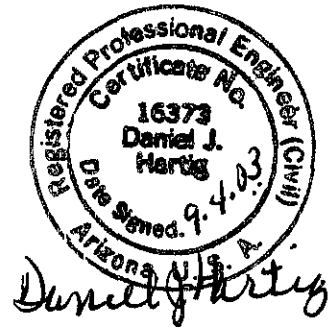

Transportation Impact and
Mitigation Analysis for

Waterfront Development

FINAL REPORT

Scottsdale, Arizona

September 2003



Submitted to:

Submitted by:



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EXECUTIVE SUMMARY

Introduction

The City of Scottsdale received an application for a development plan under the Infill Incentive District on an 11.3-acre parcel located on the southwest corner of Scottsdale Road and Camelback Road. This traffic study analyzes the impacts from the traffic that would result from the approval of this infill development.

The following tasks comprised the scope of the traffic analysis:

1. Existing daily traffic volumes were collected on Camelback Road, Scottsdale Road, and Goldwater Boulevard. Turning movement counts during the peak hours were obtained at the intersections of Camelback Road and Goldwater Boulevard, Camelback Road and Marshall Way, Camelback Road and Scottsdale Road, and Goldwater Boulevard and Montecito Avenue.
2. The projected traffic volumes that will be generated by the proposed development were estimated using the *ITE Trip Generation Handbook, 6th Edition*. These traffic projections were utilized to estimate peak hour volumes at the site driveways.
3. Comparison of the estimated traffic generated by the proposed development to estimated trip generation for previously approved development plans.
4. Estimate future traffic volumes on Camelback Road, Scottsdale Road, Goldwater Boulevard, Marshall Way, and Montecito Avenue using a year 2010 horizon.
5. Completion of capacity analyses at the signalized intersection of Camelback Road and Goldwater Boulevard, Camelback Road and Marshall Way, Camelback Road and Scottsdale Road, and Goldwater Boulevard and Montecito Avenue, and at the site driveway on Camelback Road. Level-of-service was determined for both the morning and evening peak hours at all study intersections. The analyses were based on methodology contained in the *2000 Highway Capacity Manual*.
6. Analysis of the traffic impacts assuming that the Marshall Way bridge over the Arizona Canal is open to vehicular traffic (not restricted to trolley and pedestrians).
7. Recommendations regarding minimum acceptable site access, site driveway location and configuration, modifications to existing left-turn lane lengths, and installation of deceleration lanes.
8. Analysis of the on-site circulation including parking structure entrance locations and recommendations for improvements if necessary.
9. Provide an overview of the alternative transportation opportunities for the site and proposed development with recommendations to enhance pedestrian and bicycle access.

Site Information

The location of the proposed development is shown in Figure 1. The specific site plan considered by this report is depicted in Figure 2. The development consists of approximately 94,400 square feet of general office space, 110,900 square feet of retail space and 366 residential condominium units in eleven multi-story buildings on this 11.3-acre parcel of land. The roadway network around the development to serve the site traffic includes Camelback Road, Scottsdale Road, and Goldwater Boulevard. The site traffic will have a right/left turn in/right turn out driveway on Camelback Road and two signalized intersections, Camelback Road/Marshall Way and Goldwater Boulevard/Montecito Avenue, to enter and exit the site.

Principal Findings

This traffic report contains the findings of the traffic impact study prepared for the proposed development. Overall, the development in the SWC of Scottsdale Road and Camelback Road will not adversely impact the surrounding street system adjacent to the site. Only one location, Scottsdale Road and Camelback Road, will experience a decrease in level of service (D to E), and that will only occur in the p.m. peak hour.

The analysis was based on projected traffic volumes on Camelback Road, Scottsdale Road and Goldwater Boulevard that result from the proposed development and anticipated future development in the area. The study included a capacity analysis at the intersections of Camelback Road and Goldwater Boulevard, Camelback Road and Marshall Way, Camelback Road and Scottsdale Road, and Goldwater Boulevard and Montecito Avenue, as well as at the proposed site driveway.

Conclusions

Overall, the development in the SWC of Scottsdale Road and Camelback Road will not adversely impact the surrounding street system adjacent to the site. Only one intersection, Scottsdale Road and Camelback Road, will experience a decrease in level of service (D to E), and that will only occur in the p.m. peak hour.

Site access and circulation should be reviewed further as more detailed development plans are available. The function and capacity of the Marshall Way/Montecito loop roadway needs to accommodate 700 vehicles in the p.m. peak hour and provide adequate access for emergency vehicles.

A number of geometric and operation improvements as described in the next section should also be considered to improve the overall efficiency of the transportation system.

Recommendations

- Provide two left turn lanes eastbound on Camelback Road at Scottsdale Road.
- Continue the third eastbound through lane on Camelback Road from Marshall Way to Scottsdale Road where it would become a right turn only lane.
- Incorporate a bus stop for eastbound Camelback Road into the project design.
- Close the existing median opening on Camelback Road between Scottsdale Road and Marshall Way.
- The Marshall Way approach to the signalized intersection at Camelback Road should have two lanes for approximately 110 feet.
- The Montecito approach to the signalized intersection at Goldwater Boulevard should have two lanes for approximately 150 feet.
- Adequate transitions should be provided from the two lane to four lane sections on the Marshall Way/Montecito loop.
- The entrances to the site garages from Marshall Way/Montecito should be treated as stop controlled intersections. Angle parking should not be permitted in the intersection area and the locations should align with the fashion square locations where practical. The south garage entrance on Marshall Way should align with Montecito.
- The number of lanes exiting the garage should be examined once the final size is known to ensure adequate queue lengths are provided.
- The geometry of Marshall Way south of Camelback Road and the entrance to Fashion Square north of Camelback Road must provide lane alignment that minimizes the offset for north-south movements.

PROPOSED DEVELOPMENT

Introduction

The purpose of this report is to address the impacts that a new mixed-use development located in the southwest corner of Camelback Road and Scottsdale Road will have on the adjacent street system and at the intersections surrounding the site. To determine these impacts, site traffic will be determined based on the current site plan and assigned to the streets and the intersections surrounding the site. Capacity analysis will be conducted for the a.m. and p.m. peak hour conditions for all the adjacent intersections including the site driveway.

Description

The proposed development is located on a parcel of land at the southwest corner of Camelback Road and Scottsdale Road as shown in Figure 1. The specific site plan considered by this report is depicted in Figure 1a. The development consists of approximately 94,400 square feet of general office space, 110,900 square feet of retail space, and 366 residential condominium units in eleven multi-story buildings on the site.

The roadway network serving the site includes Camelback Road, Scottsdale Road and Goldwater Boulevard. The site plan includes one driveway (Driveway A) onto Camelback Road. Site traffic will use Driveway A onto Camelback Road and two signalized intersections to enter and exit the site. The signalized intersections are Camelback Road and Marshall Way, and Goldwater Boulevard and Montecito.

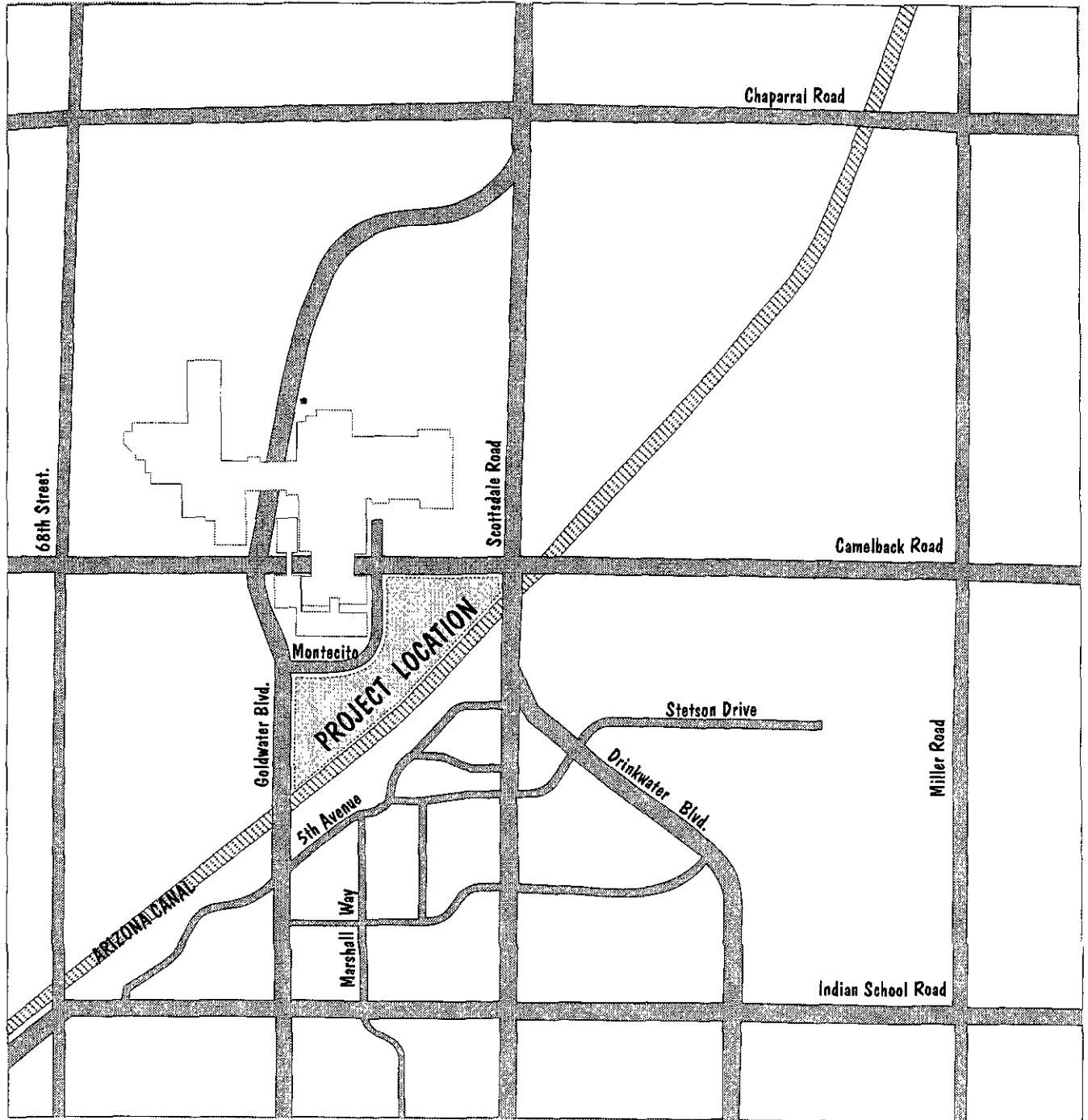
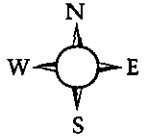
AREA CONDITIONS

Study Area

The study area is located in the downtown area of the city. The site is located south of the Scottsdale Fashion Square retail center located at Camelback Road and Scottsdale Road. The study area includes the following intersections:

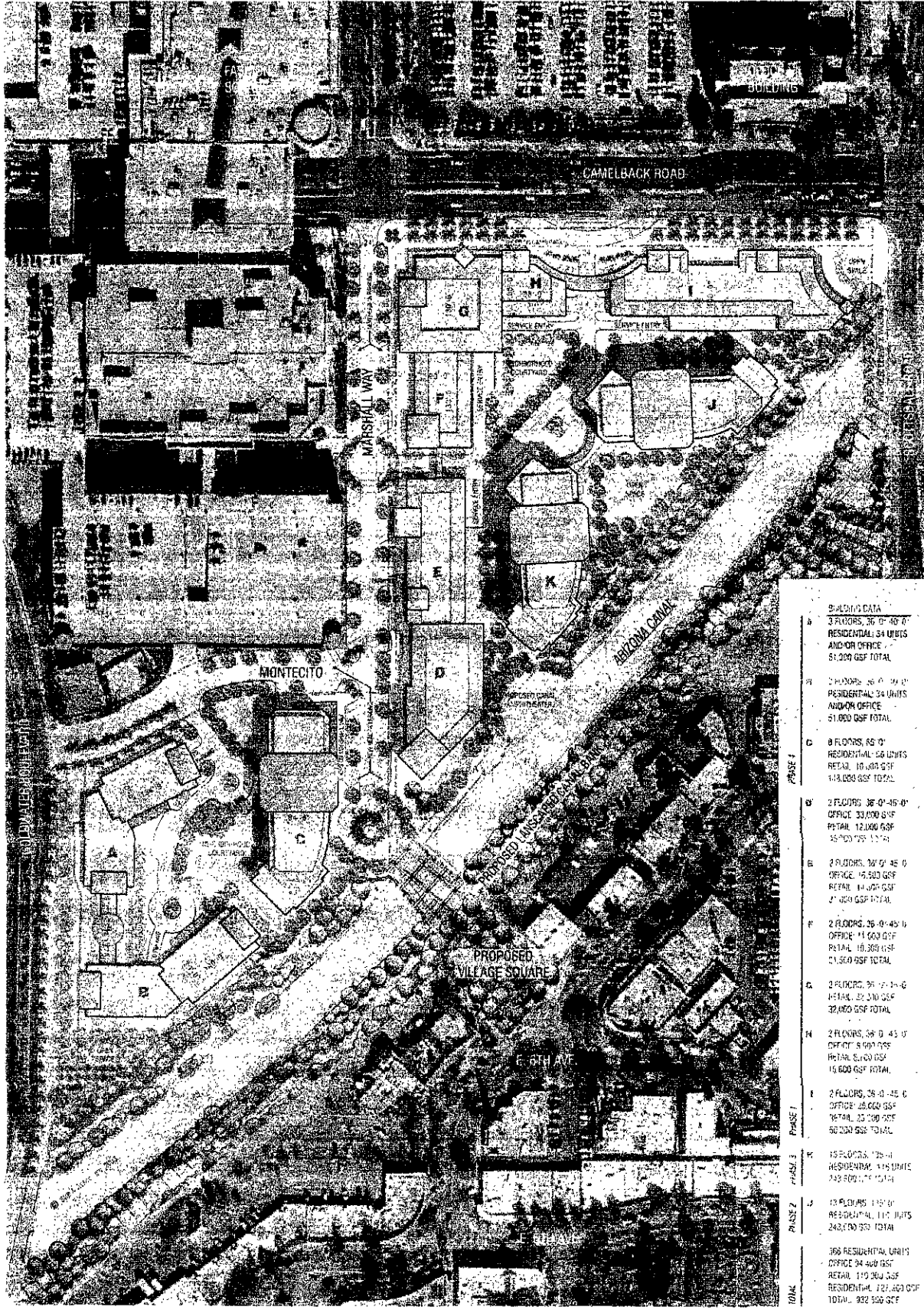
- Camelback Road and Goldwater Boulevard
- Camelback Road and Marshall Way
- Camelback Road and Scottsdale Road
- Goldwater Boulevard and Montecito Avenue

Waterfront TIMA



Vicinity Map

FIGURE 1



PHASE	DESCRIPTION	RESIDENTIAL UNITS	OFFICE	RETAIL	TOTAL GSF	
PHASE 1	B	3 FLOORS, 36'-0" x 48'-0"	RESIDENTIAL 34 UNITS	AND/OR OFFICE	51,200 GSF TOTAL	
	F1	2 FLOORS, 26'-0" x 34'-0"	RESIDENTIAL 34 UNITS	AND/OR OFFICE	51,000 GSF TOTAL	
	C	8 FLOORS, 65'-0"	RESIDENTIAL 56 UNITS	RETAIL, 16,000 GSF	118,000 GSF TOTAL	
PHASE 2	B'	3 FLOORS, 36'-0" x 48'-0"	OFFICE 33,000 GSF	RETAIL 12,000 GSF	45,000 GSF TOTAL	
	E	2 FLOORS, 26'-0" x 45'-0"	OFFICE 15,500 GSF	RETAIL 14,000 GSF	29,500 GSF TOTAL	
	F	2 FLOORS, 26'-0" x 45'-0"	OFFICE 11,000 GSF	RETAIL 16,300 GSF	27,300 GSF TOTAL	
	G	3 FLOORS, 36'-0" x 45'-0"	OFFICE 22,200 GSF	RETAIL 30,000 GSF	52,200 GSF TOTAL	
	H	2 FLOORS, 36'-0" x 45'-0"	OFFICE 8,500 GSF	RETAIL 6,500 GSF	15,000 GSF TOTAL	
PHASE 3	I	2 FLOORS, 26'-0" x 45'-0"	OFFICE 25,000 GSF	RETAIL 25,000 GSF	50,000 GSF TOTAL	
	J	15 FLOORS, 120'-0"	RESIDENTIAL 415 UNITS		742,500 GSF TOTAL	
TOTAL		106 RESIDENTIAL UNITS	OFFICE 94,400 GSF	RETAIL 110,300 GSF	RESIDENTIAL 127,000 GSF	TOTAL 932,500 GSF

SITE PLAN
 Scottsdale Waterfront
 Scottsdale Waterfront, L.L.C.

Site Plan

FIGURE 1A

08.07.03 0' 50' 100'

Land Use

The 11.3-acre area available for the proposed development is zoned for Residential/Commercial/Office (D/RCO-2 PBD) land use. The current land uses for the surrounding areas are:

- To the north-Retail
- To the south-Retail
- To the east-Office/residential
- To the west-Office

Site Accessibility

Area Roadway System

The existing major roadways that will provide access to the site are Camelback Road, Scottsdale Road and Goldwater Boulevard.

Camelback Road is an east-west minor arterial with a posted speed limit of 35 MPH in the vicinity of the site. There are three westbound lanes and two eastbound lanes adjacent to the site. There is an existing bus route on Camelback Road.

Scottsdale Road is a north-south major arterial with a posted speed limit of 40 MPH in the vicinity of the site. It is located east of the proposed site. Adjacent to the site, there are three lanes northbound and two lanes southbound. There is an existing bus route on Scottsdale Road.

Goldwater Boulevard is a north-south major arterial with a posted speed limit of 35 MPH in the vicinity of the site. It is located west of the proposed site. Adjacent to the site, there are three lanes northbound and two lanes southbound.

Marshall Way is classified as a local street in the north-south direction with a design speed of 25 MPH throughout the site. It will be used as one of the north access points to the proposed site. It contains two lanes in each direction and connects with Montecito Avenue at the south end of the road.

Montecito Avenue is classified as a local street in the east-west direction with a design speed of 25 MPH throughout the site. It will be used as the west access point to the proposed site. It contains two lanes in each direction and connects with Marshall Way at the east end of the road.

The signalized intersection of Camelback Road and Goldwater Boulevard is located northwest of the site. Camelback Road in the eastbound direction consists of one dedicated left turn lane, three through lanes and a dedicated right turn lane. In the westbound direction, Camelback Road consists of one dedicated left turn

lane, two through lanes, and a shared through/right turn lane. Goldwater Boulevard in the northbound direction consists of dual left turn lanes, two through lane and a dedicated right turn lane. The southbound direction, Goldwater Boulevard consists of dual left turn lanes, three through lanes and a dedicated right turn lane.

The signalized intersection of Camelback Road and Scottsdale Road is located to the north-east of the site. Camelback Road in the eastbound direction consists of a dedicated left turn lane, two through lanes and a dedicated right turn lane. In the westbound direction, Camelback Road consists of a dedicated left turn lane, two through lanes, and a shared through/right turn lane. Scottsdale Road in the northbound direction consists of dual left turn lanes, two through lanes, and a shared through/right turn lane. In the southbound direction, Scottsdale Road consists of dual left turn lanes, two through lanes and a dedicated right turn lane.

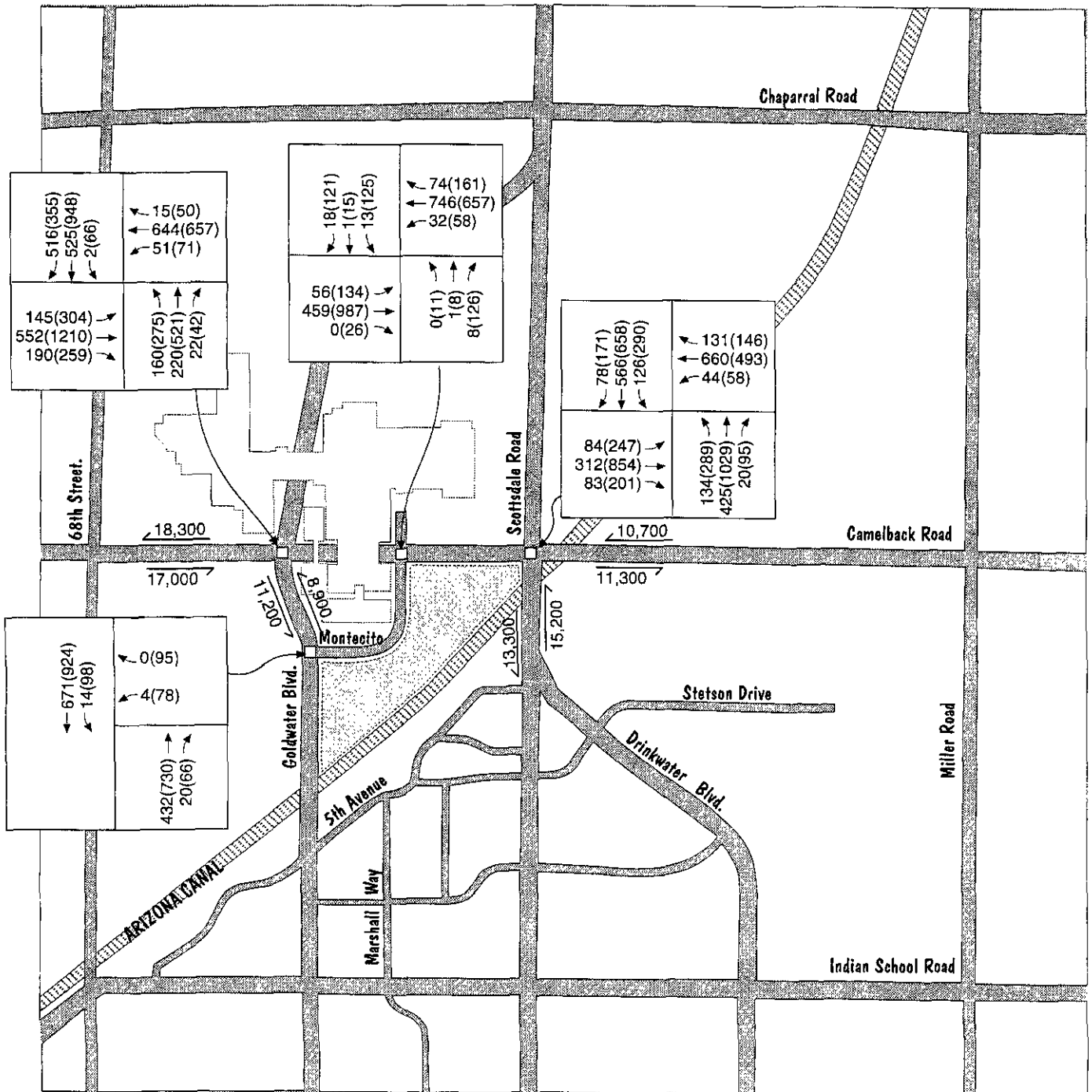
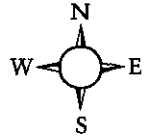
The Camelback Road and Marshall Way intersection provides access to the Fashion Square shopping center both north and south of Camelback Road. It is a signalized intersection that will also be used as an access point to the proposed site. The eastbound direction consists of a dedicated left turn lane, two through lanes and a dedicated right turn lane. The westbound direction consists of a dedicated left turn lane, three through lanes and a dedicated right turn lane. The north and south legs consist of two lanes that are shared between the through and their respective turning movements.

Goldwater Boulevard and Montecito Avenue, west of the site, also provides access to Fashion Square south of Camelback Road. It is a signalized intersection that will also be used as an access for the proposed site. On Goldwater Boulevard, the southbound direction consists of a dedicated left turn lane and three through lanes. In the northbound direction there are two through lanes and a dedicated right turn lane. On Montecito Avenue, there is a left turn lane and a right turn lane.

Traffic Volumes

Existing daily traffic volumes were collected on Camelback Road, Scottsdale Road, and Goldwater Boulevard on June 11, 2003. Turning movement counts were conducted during the morning (7 to 9 a.m.) and afternoon (4 to 6 p.m.) peak hours at the intersections of Camelback Road and Goldwater Boulevard, Camelback Road and Marshall Way, Camelback Road and Scottsdale Road, and Goldwater Boulevard and Montecito Avenue on June 12, 2003. Figure 2 depicts the peak hour and daily traffic volumes. The AM peak hour was from 7:45 to 8:45 at two locations and from 7:30 and 8:30 at the other two locations. The PM peak hour was from 4:45 and 5:45 at all four locations. The actual traffic counts are attached in Appendix A of this report. It should be noted that the actual volumes were increased two percent to represent the annual average.

Waterfront TIMA



(##) = A.M.(P.M.)

= Directional Daily Volume

Existing Traffic Volumes

FIGURE 2

The existing daily traffic volumes are shown in Table 1. The directional daily volumes and the 'k' factor are shown for each location. The 'k' factor represents the percentage of daily traffic that occurs in the a.m. or p.m. peak hour.

TABLE 1: EXISTING DAILY TRAFFIC VOLUMES

Locations	Direction	Volume	Percent of Total	K Factor	
				AM	PM
Camelback Road-west of Goldwater Boulevard	EB	17,000	48%	7%	9%
	WB	18,300	52%	7%	7%
Camelback Road-east of Scottsdale Road	EB	11,300	51%	5%	10%
	WB	10,700	49%	7%	8%
Scottsdale Road-south of Camelback Road	NB	15,200	53%	7%	8%
	SB	13,300	47%	6%	7%
Goldwater Boulevard- south of Camelback Road	NB	8,900	44%	6%	9%
	SB	11,200	56%	5%	8%

As can be seen from Table 1, the largest daily volume imbalance is on Goldwater Boulevard south of Camelback Road. This is not surprising since Goldwater Boulevard has three southbound lanes and two northbound lanes. The highest 'k' factor was found on Camelback Road, east of Scottsdale Road in the p.m. peak, while the lowest was found at the same location in the a.m. peak as well as on Goldwater Boulevard, south of Camelback Road in the a.m. peak. It is very common for the a.m. peak to have a lower 'k' factor than the p.m. peak, which is consistent with the data shown in Table 1.

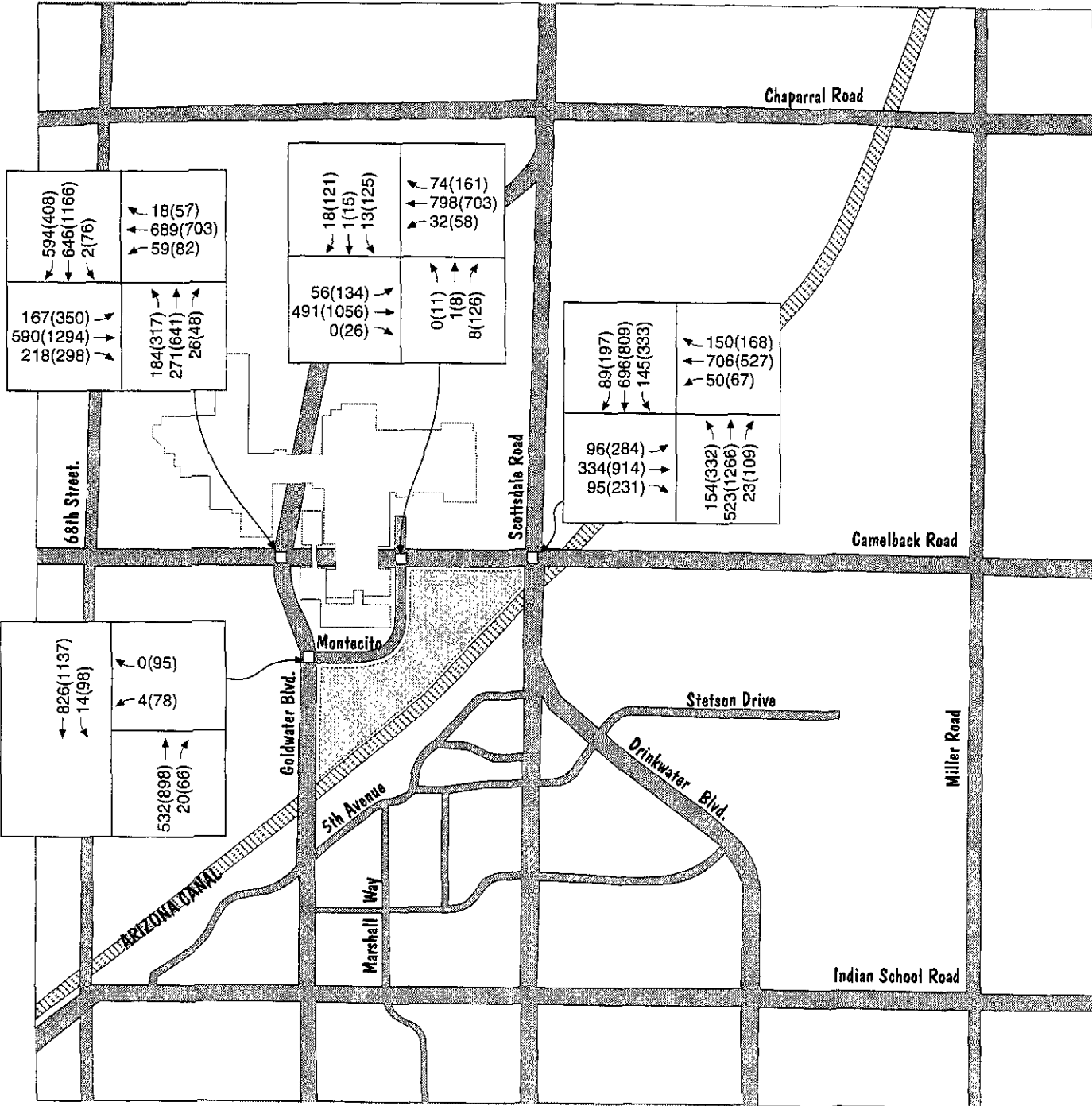
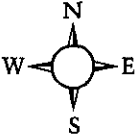
PROJECTED TRAFFIC

Background Traffic

Average daily traffic on Camelback Road for the horizon year of 2010 was obtained by assuming a growth rate of one percent a year. Average daily traffic on Scottsdale Road and Goldwater Boulevard for the horizon year of 2010 was obtained by assuming a growth rate of three percent a year. Using these rates, the turning movements at the study intersections were increased by the appropriate percent for the horizon year of 2010. The traffic volumes on Marshall Way and Montecito Avenue were not adjusted for growth.

Figure 3 shows the future base traffic for the study intersections in the a.m. and p.m. peak hours.

Waterfront TIMA



##(##) = A.M.(P.M.)

Future Base Traffic Volumes

FIGURE 3

Site Traffic

Trip Generation

The number of trips that would be generated by a proposed development can be estimated using trip generation rates published by the Institute of Transportation Engineers (ITE) in *Trip Generation, 6th Edition* (1997). These rates represent years of data collection for a variety of different land uses around the country. For the land uses proposed for the site, ITE Land Use Codes 230 (Residential Condominium/Townhouse), 710 (General Office), and 820 (Shopping Center) were selected for the site uses. Table 2 presents a summary of the trip generation estimate for the site.

Also included in Table 2 is an estimate of internal trips and trips made on other modes. Internal trips are ones that are made between uses within the site or to Fashion Square and do not impact the surrounding street system. This was estimated to be 10 percent. Trips made on other modes can include transit, bicycle, walking, trolley, and was estimated to be five percent. The external trips from the site are estimated to be 8,992 on a daily basis with 419 (five percent) in the a.m. peak hour and 888 (10 percent) in the p.m. peak hour.

TABLE 2: TRIP GENERATION

Development	ITE Land Use Code	Size	Trips Generated						
			Daily	AM Peak Hour			PM Peak Hour		
				Enter	Exit	Total	Enter	Exit	Total
Residential Condominium/Townhouse	230	366 D.U.	1,961	24	118	142	120	59	179
General Office	710	94,400 S.F.	1,270	157	21	178	31	154	185
Shopping Center	820	110,900 S.F.	7,286	104	66	170	323	350	673
Sub-Total			10,517	285	205	490	474	563	1,037
Internal Trips (10%)			-1,052	-28	-21	-49	-47	-56	-103
Other Modal Trips (5%)			-473	-13	-9	-22	-21	-25	-46
Total External			8,992	244	175	419	406	482	888

The estimated traffic that would be generated by the proposed development was compared to the estimated trip generation for the previously approved development plan known as Waterfront Mixed-Use Development. The traffic report was dated December 1995 and prepared by Kenig, Lindgren, O'Hara, Aboona, Inc. The previous approved development plan included 208,480 SF of retail; 60,000 SF of restaurant; 134,900 SF of office; and 70,000 SF of movie theater. It is interesting

to note that all of these uses were located in the eastern portion of the property and did not include the portion west of Marshall Way. For comparison purposes, the portion of the proposed site west of Marshall Way was subtracted from the trip generation shown in Table 2. The comparison of the current plan (east of Marshall Way) with the previously approved development is presented in Table 3.

TABLE 3: COMPARISON OF TRIPS GENERATED BY PREVIOUSLY APPROVED PLAN

	ITE Land Use Codes	Trips Generated						
		Daily	AM Peak Hour			PM Peak Hour		
			Enter	Exit	Total	Enter	Exit	Total
Current Plan	230, 710, and 820	8,666	209	169	378	393	451	844
Previous Plan	820, 832, 710, 220, and 444	19,674	392	141	533	1020	925	1945
Difference	NA	-11,008	-183	28	-155	-627	-474	-1,101

As can be seen from Table 3, the proposed use will generate substantially fewer trips than the previously approved development both on a daily basis and in each peak hour.

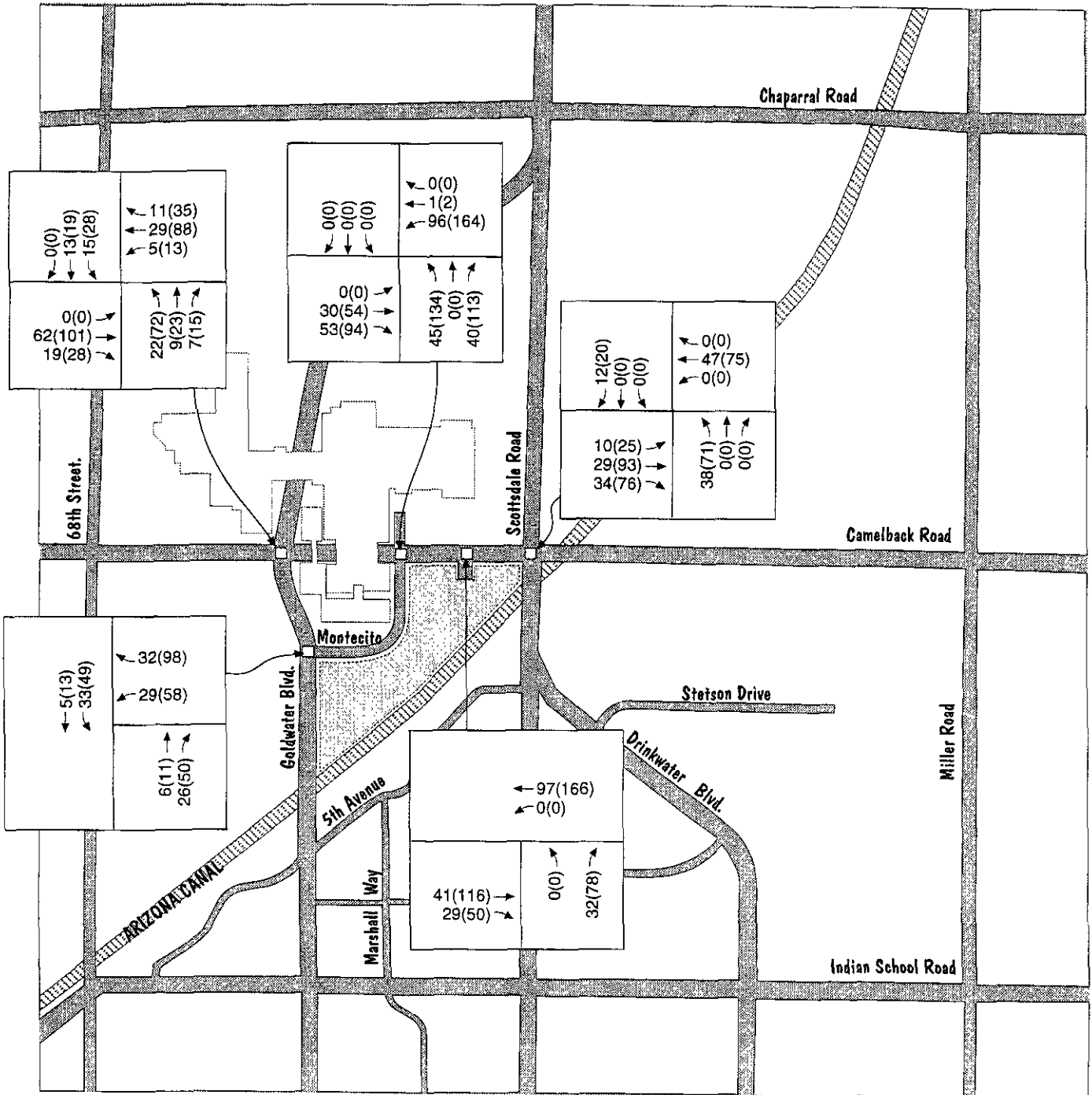
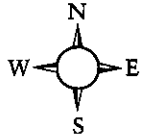
Trip Distribution

The trip distribution for site traffic was estimated by examining the population and employment distribution within a 5-mile and 15-mile radius around the site. The five-mile radius was used to estimate trips to/from the retail uses, while the 15-mile radius was used to estimate trips to/from the residential and office uses. Using these 5-mile and 15-mile radius circles, the proportion of population and employment within each quarter of the circles were calculated to estimate the distribution of trips by cardinal direction. As a result of this analysis, it is estimated that 17 percent of the traffic would be to/from the east, 30 percent would be to/from the west, 16 percent would be to/from the north, and 37 percent of the traffic would be to/from the south.

Trip Assignment

The site traffic was then assigned to the street network and to the site access points on Camelback Road and Goldwater Boulevard based on the trip distribution. The assignment of the site traffic is shown in Figure 4 for the a.m. and p.m. peak hour. It should be noted that the trip assignment does not necessarily match the trip distribution because of the street pattern. For example, traffic from the west could use SR 202 and SR 101, exit at Indian School Road and actually approach the site from the south.

Waterfront TIMA



##(##) = A.M.(P.M.)

Site Traffic Assignment

FIGURE 4

Total Traffic

The total 2010 traffic volumes were obtained by adding the background traffic and the site traffic at the study intersections. Figure 5 shows the estimated total traffic at the study intersections for the a.m. and p.m. peak hours in the year 2010.

TRAFFIC ANALYSIS

Site Access/Circulation

The site is separated into two distinct sections, east of Marshall Way and south of Montecito. Either section can be accessed from Camelback Road or Goldwater Boulevard because Montecito and Marshall Way form a continuous route northwest of the site.

The current site plan shows two access points from the east section of the site onto Marshall Way, one opposite the Fashion Square garage and one just north of the Montecito/Marshall Way intersection. Consideration should be given to adjusting the southern access point to be opposite Montecito to provide a four legged intersection.

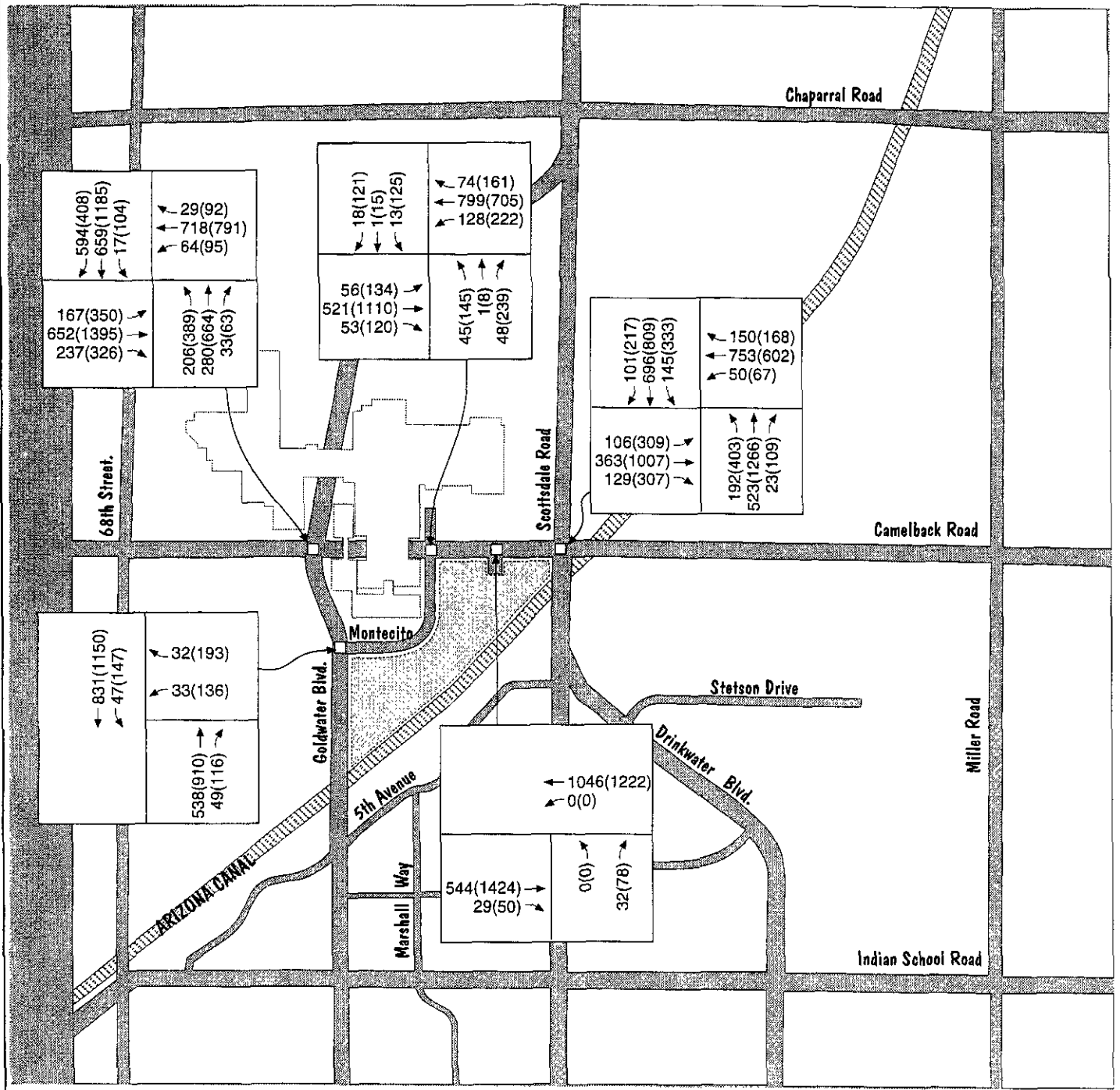
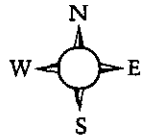
There is also one right turn in/right turn out driveway directly onto Camelback Road. The existing median opening on Camelback Road at this driveway should be closed.

There is one access point from the south section onto Montecito. This should align with the south garage access to Fashion Square.

The proposed site plan shows narrowing Marshal Way/Montecito to provide one lane in each direction with on-street angle parking. Angle parking should not be permitted within the intersection area at each access point. At a minimum, two northbound exit lanes should be provided approaching Camelback Road. The lane configuration north of Camelback Road may need to be adjusted to provide proper alignment with the final configuration on the south side. Two westbound exit lanes should be provided approaching Goldwater Boulevard.

Currently, there is a northbound right turn lane from Goldwater Boulevard onto Montecito and an eastbound right turn lane from Camelback Road onto Marshall Way. The Marshall Way/Montecito loop is four lanes with a raised median. There are two exiting lanes on Marshall Way at Camelback Road and from Montecito onto Goldwater Boulevard.

Waterfront TIMA



##(##) = A.M.(P.M.)

Future Base + Site Traffic Volumes

FIGURE 5

Level of Service

The intersections of Camelback Road and Goldwater Boulevard, Camelback Road and Marshall Way, Camelback Road and the site driveway, Goldwater Boulevard and Montecito Avenue, and Camelback Road and Scottsdale Road were analyzed to determine the current level of service and the expected level of service for the projected Year 2010 volumes.

The signalized intersections were analyzed using a 102 second cycle length. The intersection of Camelback and the site driveway is assumed to be stop-controlled with the stop signs located on the driveway. This driveway does not exist today and was only analyzed for the year 2010 with site condition.

Level of Service (LOS) is a term used to describe traffic operations. The various levels of service, which range from A to F, are generally defined as follows:

- **LEVEL OF SERVICE A** represents free flow operation.
- **LEVEL OF SERVICE B** is in the range of free flow, but the presence of other users in the traffic stream begins to be noticeable.
- **LEVEL OF SERVICE C** is in the range of stable flow, but marks the beginning of the range in which the operation of individual users becomes significantly affected by others.
- **LEVEL OF SERVICE D** represents high density but stable flow. Speed and freedom to maneuver are severely restricted, and the driver experiences a generally poor level of comfort and convenience.
- **LEVEL OF SERVICE E** represents operating conditions at or near the capacity level. All speed is reduced to a low but relatively uniform value.
- **LEVEL OF SERVICE F** is used to define forced or stop and go travel. This condition exists wherever the amount of traffic approaching a point exceeds the amount that can traverse the point.

The level of service analysis for signalized intersections was performed utilizing the methodology presented in the 2000 Highway Capacity Manual. This method uses the critical volumes passing through the intersection in one hour and compares those volumes to the capacity of the intersection and an associated delay. The analysis incorporates the effects of traffic volumes, geometry, traffic signal operation, truck and local bus volumes, pedestrian activity, and peaking characteristics. The result is a level of service determination for each approach and for the intersection as a whole.

The capacity criteria are presented in terms of average vehicle delay in Table 4.

TABLE 4: CAPACITY CRITERIA FOR SIGNALIZED INTERSECTIONS*

Level of Service (LOS)	Control Delay per Vehicle (sec)
A	less than 10
B	10.1-20
C	20.1-35
D	35.1-55
E	55.1-80
F	over 80

*Source: Highway Capacity Manual

For unsignalized intersections with two-way stop control, the Highway Capacity Manual (HCM) procedure uses the conflicting flow and critical gap of an approach to calculate the capacity of the approach. The capacity is compared to the existing or projected demand to determine the available reserve capacity, which can be used to estimate a range of traffic delay and level of service for each approach. The level of service categories are defined in Table 5.

TABLE 5: LEVEL OF SERVICE CRITERIA FOR UNSIGNALIZED INTERSECTIONS*

Level of Service	Expected Delay to Minor Street Traffic
A	little or no delay
B	short delays
C	average delays
D	long delays
E	very long delays
F	**

*SOURCE: Highway Capacity Manual

** When demand volume exceeds the capacity of the lane, extreme delays will be encountered with queuing, which may cause severe congestion affecting other traffic movements in the intersection.

The Level of Service calculations were completed for both the signalized and stop sign controlled intersections based on the operational analysis method set forth in Synchro 5, which is based on the HCM procedure. The results of the analysis are summarized in Table 6. As can be seen from Table 6, all the study intersections are projected to operate at level of service D or better with the site traffic included, except Scottsdale Road and Camelback Road in the p.m. peak hour, which would operate at level of service E in 2010.

TABLE 6 – INTERSECTION LEVEL OF SERVICE SUMMARY

Intersection/ Approach Movement	2003		2010 base		2010 with site	
	AM	PM	AM	PM	AM	PM
Camelback Rd/ Driveway A (unsignalized)						
EB: right					A	A
NB: right					B	C
Camelback Rd/Goldwater Blvd (signalized)						
EB Approach:	C	D	C	D	C	C
WB Approach:	B	C	B	C	B	D
NB Approach	C	D	C	D	D	D
SB Approach:	E	C	E	C	E	D
Intersection	D	D	D	D	D	D
Camelback Rd/Marshall Way (signalized)						
EB Approach:	A	A	A	B	A	A
WB Approach:	A	A	A	A	A	A
NB Approach	D	E	D	D	D	F
SB Approach:	D	F	D	F	D	E
Intersection	A	D	A	B	A	C

TABLE 6 – INTERSECTION LEVEL OF SERVICE SUMMARY (continued)

Intersection/ Approach Movement	2003		2010 base		2010 with site	
	AM	PM	AM	PM	AM	PM
Camelback Rd/Scottsdale Rd (signalized)						
EB Approach:	C	F	D	D	E	D
WB Approach:	E	D	D	D	F	E
NB Approach	C	D	C	D	C	D
SB Approach:	C	D	C	D	C	E
Intersection	D	E	D	D	D	E
Goldwater Blvd/Montecito Ave (signalized)						
WB Approach:	D	E	D	D	D	E
NB Approach	A	A	A	A	A	A
SB Approach:	A	A	A	B	A	A
Intersection	A	B	A	B	A	B

Accident Analysis

Accident data for the years 2000 and 2002 were reviewed using the City of Scottsdale 2002 Traffic Volume and Accident Rate Data report (June 2003). On the segment of Camelback Road west of Scottsdale Road, there were 10 accidents in 2002. At the intersection of Camelback Road and Scottsdale Road, there were 19 collisions and at the intersection of Camelback Road and Goldwater Boulevard, there were 15 collisions in 2002.

The accident rates for 2000 and 2002 are summarized in Table 7.

TABLE 7 - ACCIDENT SUMMARY

Location	2000	2002
	Accident rate*	Accident rate*
<u>Segment</u> : Camelback Road from Goldwater Blvd to Scottsdale Rd	7.34	3.93
<u>Intersection</u> : Camelback Rd & Scottsdale Rd	0.79	0.90
<u>Intersection</u> : Camelback Rd & Goldwater Blvd	1.18	0.70

*Segment accident rate is accidents per million vehicle miles traveled and intersection accident rate is accidents per million entering vehicles.

The city-wide average intersection accident rate for the year 2002 was 0.54 accidents per million entering vehicles. The city-wide average segment accident rate for the year 2002 was 1.49 accidents per million vehicle miles traveled. The rates listed in Table 7 are higher than the city-wide averages.

IMPROVEMENT ANALYSIS

The results of the traffic analysis summarized in Table 6 shows that the addition of the site traffic in 2010 will decrease the overall intersection level of service at Scottsdale Road and Camelback Road from D to E in the p.m. peak hour. The average intersection delay increases from 47 seconds per vehicle to 57 seconds per vehicle with the addition of the site traffic. All other study intersections will operate at level of service D or better in both peak hours with site traffic.

Several options can be considered for Scottsdale Road and Camelback Road:

- Allow vehicular traffic on the planned bridge over the Arizona Canal connecting Marshall Way. This would divert site traffic as well as Fashion Square traffic from Scottsdale Road and Camelback Road to Marshall Way. The disadvantage of this option is additional traffic in the 5th Avenue shopping district.
- Modify the west leg of Camelback Road to provide two eastbound left turn lanes. A preliminary look at the intersection geometry indicates this option is viable.
- Accept a level of service E during one hour of the day in the year 2010.

The left turn storage lengths needed to accommodate the 2010 volumes with the site traffic were determined using the SYNCRO analysis. They are as follows.

- EB Camelback Road left turn at Scottsdale Road – 180' (assumes dual left turn lanes)
- WB Camelback Road left turn at Marshall Way – 165'
- WB Camelback Road left turn at Goldwater Boulevard – 290'
- NB Goldwater Boulevard left turn at Camelback Road – 195'
- SB Goldwater Boulevard left turn at Montecito – 112'

A review of the current lane configuration shows that adequate left turn storage can be provided at all locations except the southbound Goldwater Boulevard left turn at Montecito and the westbound Camelback Road left turn at Goldwater Boulevard. The southbound Goldwater left turn lane is approximately 60' long today and is 'back to back' with a northbound left turn lane to an existing parking garage. Some options to consider are:

- Relocate the northbound left turn to provide longer southbound storage
- Widen to provide dual left turn lanes southbound
- Provide a separate left turn phase for the southbound movement

The westbound Camelback Road left turn lane at Goldwater is 230 feet and is back to back with the eastbound left turn lane at Marshall Way, which is approximately 140 feet. The eastbound left turn lane at Marshall Way could be reduced to 100 feet, which would increase the westbound left turn at Goldwater Boulevard to 270 feet.

CONCLUSIONS

Overall, the development in the SWC of Scottsdale Road and Camelback Road will not adversely impact the surrounding street system adjacent to the site. Only one intersection, Scottsdale Road and Camelback Road, will experience a decrease in level of service (D to E), and that will only occur in the p.m. peak hour.

Site access and circulation should be reviewed further as more detailed development plans are available. The function and capacity of the Marshall Way/Montecito loop roadway needs to accommodate 700 vehicles in the p.m. peak hour and provide adequate access for emergency vehicles.

A number of geometric and operation improvements as described in the next section should also be considered to improve the overall efficiency of the transportation system.

RECOMMENDATIONS

- Provide two left turn lanes eastbound on Camelback Road at Scottsdale Road.
- Continue the third eastbound through lane on Camelback Road from Marshall Way to Scottsdale Road where it would become a right turn only lane.
- Incorporate a bus stop for eastbound Camelback Road into the project design.
- Close the existing median opening on Camelback Road between Scottsdale Road and Marshall Way.
- The Marshall Way approach to the signalized intersection at Camelback Road should have two lanes for approximately 110 feet.
- The Montecito approach to the signalized intersection at Goldwater Boulevard should have two lanes for approximately 150 feet.
- Adequate transitions should be provided from the two lane to four lane sections on the Marshall Way/Montecito loop.
- The entrances to the site garages from Marshall Way/Montecito should be treated as stop controlled intersections. Angle parking should not be permitted in the intersection area and the locations should align with the fashion square locations where practical. The south garage entrance on Marshall Way should align with Montecito.
- The number of lanes exiting the garage should be examined once the final size is known to ensure adequate queue lengths are provided.
- The geometry of Marshall Way south of Camelback Road and the entrance to Fashion Square north of Camelback Road must provide lane alignment that minimizes the offset for north-south movements.

APPENDIX A

DAILY AND PEAK HOUR TURNING MOVEMENT DATA COLLECTED ON JUNE
11 AND 12, 2003

Start Time	Wed	11-Jun-03		Thu	12-Jun-03		Fri	13-Jun-03		Daily Average	
	A.M.	P.M.		A.M.	P.M.		A.M.	P.M.	A.M.	P.M.	
12:00	*	285		59	*		*	*		59	285
12:15	*	289		40	*		*	*		40	289
12:30	*	305		53	*		*	*		53	305
12:45	*	281		37	*		*	*		37	281
01:00	*	271		89	*		*	*		89	271
01:15	*	271		90	*		*	*		90	271
01:30	*	251		42	*		*	*		42	251
01:45	*	277		27	*		*	*		27	277
02:00	*	276		18	*		*	*		18	276
02:15	*	220		24	*		*	*		24	220
02:30	*	278		20	*		*	*		20	278
02:45	*	257		14	*		*	*		14	257
03:00	*	259		5	*		*	*		5	259
03:15	*	261		7	*		*	*		7	261
03:30	*	282		10	*		*	*		10	282
03:45	*	243		5	*		*	*		5	243
04:00	*	323		9	*		*	*		9	323
04:15	*	284		8	*		*	*		8	284
04:30	*	314		20	*		*	*		20	314
04:45	*	293		30	*		*	*		30	293
05:00	*	381		18	*		*	*		18	381
05:15	*	387		24	*		*	*		24	387
05:30	*	336		36	*		*	*		36	336
05:45	*	261		39	*		*	*		39	261
06:00	*	257		52	*		*	*		52	257
06:15	*	223		57	*		*	*		57	223
06:30	*	203		71	*		*	*		71	203
06:45	*	212		73	*		*	*		73	212
07:00	*	215		89	*		*	*		89	215
07:15	*	211		105	*		*	*		105	211
07:30	*	214		149	*		*	*		149	214
07:45	*	169		139	*		*	*		139	169
08:00	*	162		147	*		*	*		147	162
08:15	*	164		145	*		*	*		145	164
08:30	*	172		138	*		*	*		138	172
08:45	*	150		175	*		*	*		175	150
09:00	*	158		150	*		*	*		150	158
09:15	*	152		165	*		*	*		165	152
09:30	*	123		172	*		*	*		172	123
09:45	*	149		165	*		*	*		165	149
10:00	*	120		158	*		*	*		158	120
10:15	*	89		160	*		*	*		160	89
10:30	*	83		210	*		*	*		210	83
10:45	*	91		190	*		*	*		190	91
11:00	*	70		230	*		*	*		230	70
11:15	*	69		255	*		*	*		255	69
11:30	*	54		284	*		*	*		284	54
11:45	*	63		281	*		*	*		281	63
Total		0	10458	4484	0		0	0		4484	10458
Combined Total		10458		4484			0			14942	
Peak		04:45		11:00						11:00	04:45
Volume		1397		1050						1050	1397
P.H.F.		0.902		0.924						0.924	0.902
ADT		Not Calculated									

Traffic Research & Analysis, Inc.
 3844 East Indian School Road
 Phoenix, Arizona 85018
 (602) 840-1500 & Fax: (602) 840-1577

Page 1
 Station ID: 3
 Site Code: 0611033626
 Latitude: 33' 50151.000 North
 Longitude: 111' 92867.000 West
 MARSHALL WAY SO CAMELBACK RD
 NB

Start Time	Wed	11-Jun-03	Thu	12-Jun-03	Fri	13-Jun-03	Daily Average	
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	*	22	1	*	*	*	1	22
12:15	*	25	2	*	*	*	2	25
12:30	*	18	7	*	*	*	7	18
12:45	*	29	2	*	*	*	2	29
01:00	*	34	0	*	*	*	0	34
01:15	*	25	2	*	*	*	2	25
01:30	*	26	1	*	*	*	1	26
01:45	*	39	0	*	*	*	0	39
02:00	*	29	0	*	*	*	0	29
02:15	*	34	0	*	*	*	0	34
02:30	*	35	0	*	*	*	0	35
02:45	*	22	0	*	*	*	0	22
03:00	*	29	0	*	*	*	0	29
03:15	*	32	0	*	*	*	0	32
03:30	*	38	0	*	*	*	0	38
03:45	*	42	0	*	*	*	0	42
04:00	*	30	0	*	*	*	0	30
04:15	*	57	0	*	*	*	0	57
04:30	*	28	0	*	*	*	0	28
04:45	*	34	0	*	*	*	0	34
05:00	*	41	0	*	*	*	0	41
05:15	*	42	0	*	*	*	0	42
05:30	*	48	0	*	*	*	0	48
05:45	*	32	0	*	*	*	0	32
06:00	*	25	0	*	*	*	0	25
06:15	*	38	0	*	*	*	0	38
06:30	*	38	2	*	*	*	2	38
06:45	*	27	4	*	*	*	4	27
07:00	*	38	2	*	*	*	2	38
07:15	*	27	1	*	*	*	1	27
07:30	*	23	0	*	*	*	0	23
07:45	*	30	2	*	*	*	2	30
08:00	*	15	3	*	*	*	3	15
08:15	*	21	3	*	*	*	3	21
08:30	*	20	1	*	*	*	1	20
08:45	*	19	3	*	*	*	3	19
09:00	*	42	5	*	*	*	5	42
09:15	*	20	7	*	*	*	7	20
09:30	*	29	5	*	*	*	5	29
09:45	*	16	11	*	*	*	11	16
10:00	*	7	5	*	*	*	5	7
10:15	*	7	9	*	*	*	9	7
10:30	*	3	7	*	*	*	7	3
10:45	*	3	22	*	*	*	22	3
11:00	*	4	22	*	*	*	22	4
11:15	*	2	22	*	*	*	22	2
11:30	*	3	23	*	*	*	23	3
11:45	*	2	12	*	*	*	12	2
Total	0	1250	186	0	0	0	186	1250
Combined Total		1250	186		0		1436	
Peak		03:30	10:45				10:45	03:30
Volume		167	89				89	167
P.H.F.		0.732	0.967				0.967	0.732
ADT		Not Calculated						

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 3844 East Indian School Road
 Phoenix, Arizona 85018
 (602) 840-1500 & Fax: (602) 840-1577

Page 1
 Station ID: 3
 Site Code: 0611033626
 Latitude: 33' 50151.000 North
 Longitude: 111' 92867.000 West
 MARSHALL WAY SO CAMELBACK RD
 SB

Start Time	Wed	11-Jun-03	Thu	12-Jun-03	Fri	13-Jun-03	Daily Average	
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	*	53	1	*	*	*	1	53
12:15	*	42	1	*	*	*	1	42
12:30	*	58	0	*	*	*	0	58
12:45	*	34	2	*	*	*	2	34
01:00	*	36	2	*	*	*	2	36
01:15	*	42	1	*	*	*	1	42
01:30	*	31	2	*	*	*	2	31
01:45	*	32	1	*	*	*	1	32
02:00	*	32	0	*	*	*	0	32
02:15	*	30	0	*	*	*	0	30
02:30	*	31	0	*	*	*	0	31
02:45	*	23	0	*	*	*	0	23
03:00	*	35	1	*	*	*	1	35
03:15	*	34	0	*	*	*	0	34
03:30	*	28	1	*	*	*	1	28
03:45	*	27	0	*	*	*	0	27
04:00	*	27	0	*	*	*	0	27
04:15	*	22	1	*	*	*	1	22
04:30	*	33	0	*	*	*	0	33
04:45	*	36	0	*	*	*	0	36
05:00	*	24	0	*	*	*	0	24
05:15	*	24	1	*	*	*	1	24
05:30	*	30	0	*	*	*	0	30
05:45	*	23	0	*	*	*	0	23
06:00	*	25	0	*	*	*	0	25
06:15	*	27	2	*	*	*	2	27
06:30	*	20	1	*	*	*	1	20
06:45	*	26	4	*	*	*	4	26
07:00	*	27	6	*	*	*	6	27
07:15	*	17	2	*	*	*	2	17
07:30	*	16	10	*	*	*	10	16
07:45	*	6	9	*	*	*	9	6
08:00	*	16	6	*	*	*	6	16
08:15	*	14	9	*	*	*	9	14
08:30	*	10	9	*	*	*	9	10
08:45	*	9	10	*	*	*	10	9
09:00	*	6	8	*	*	*	8	6
09:15	*	5	13	*	*	*	13	5
09:30	*	3	13	*	*	*	13	3
09:45	*	1	33	*	*	*	33	1
10:00	*	2	25	*	*	*	25	2
10:15	*	3	21	*	*	*	21	3
10:30	*	0	24	*	*	*	24	0
10:45	*	2	41	*	*	*	41	2
11:00	*	3	41	*	*	*	41	3
11:15	*	1	30	*	*	*	30	1
11:30	*	2	40	*	*	*	40	2
11:45	*	0	47	*	*	*	47	0
Total	0	1028	418	0	0	0	418	1028
Combined Total	1028		418		0		1446	
Peak		12:00	11:00				11:00	12:00
Volume		187	158				158	187
P.H.F.		0.806	0.840				0.840	0.806
ADT		Not Calculated						

Traffic Research & Analysis, Inc.
 3844 East Indian School Road
 Phoenix, Arizona 85018
 (602) 840-1500 & Fax: (602) 840-1577

Page 1
 Station ID: 4
 Site Code: 0611033077
 Latitude: 33' 50036.000 North
 Longitude: 111' 92930.000 West
 MONTECITO AVE EO GOLDWATER BLVD
 WB

Start Time	Wed	11-Jun-03	Thu	12-Jun-03	Fri	13-Jun-03	Daily Average	
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	*	90	0	*	*	*	0	90
12:15	*	99	1	*	*	*	1	99
12:30	*	86	1	*	*	*	1	86
12:45	*	102	2	*	*	*	2	102
01:00	*	76	2	*	*	*	2	76
01:15	*	81	1	*	*	*	1	81
01:30	*	82	3	*	*	*	3	82
01:45	*	74	0	*	*	*	0	74
02:00	*	58	0	*	*	*	0	58
02:15	*	74	0	*	*	*	0	74
02:30	*	56	0	*	*	*	0	56
02:45	*	71	0	*	*	*	0	71
03:00	*	68	4	*	*	*	4	68
03:15	*	61	0	*	*	*	0	61
03:30	*	50	0	*	*	*	0	50
03:45	*	43	2	*	*	*	2	43
04:00	*	63	3	*	*	*	3	63
04:15	*	68	2	*	*	*	2	68
04:30	*	42	3	*	*	*	3	42
04:45	*	48	2	*	*	*	2	48
05:00	*	85	4	*	*	*	4	85
05:15	*	52	0	*	*	*	0	52
05:30	*	78	5	*	*	*	5	78
05:45	*	64	2	*	*	*	2	64
06:00	*	39	6	*	*	*	6	39
06:15	*	47	12	*	*	*	12	47
06:30	*	40	10	*	*	*	10	40
06:45	*	33	19	*	*	*	19	33
07:00	*	46	16	*	*	*	16	46
07:15	*	24	9	*	*	*	9	24
07:30	*	22	1	*	*	*	1	22
07:45	*	19	14	*	*	*	14	19
08:00	*	28	10	*	*	*	10	28
08:15	*	15	21	*	*	*	21	15
08:30	*	11	24	*	*	*	24	11
08:45	*	7	21	*	*	*	21	7
09:00	*	16	14	*	*	*	14	16
09:15	*	10	35	*	*	*	35	10
09:30	*	12	29	*	*	*	29	12
09:45	*	5	43	*	*	*	43	5
10:00	*	3	52	*	*	*	52	3
10:15	*	8	49	*	*	*	49	8
10:30	*	5	69	*	*	*	69	5
10:45	*	6	91	*	*	*	91	6
11:00	*	3	68	*	*	*	68	3
11:15	*	3	61	*	*	*	61	3
11:30	*	4	116	*	*	*	116	4
11:45	*	5	87	*	*	*	87	5
Total	0	2082	914	0	0	0	914	2082
Combined Total		2082	914		0		2996	
Peak		12:00	10:45				10:45	12:00
Volume		377	336				336	377
P.H.F.		0.924	0.724				0.724	0.924
ADT		Not Calculated						

Traffic Research & Analysis, Inc.
 3844 East Indian School Road
 Phoenix, Arizona 85018
 (602) 840-1500 & Fax: (602) 840-1577

Page 1
 Station ID: 6
 Site Code: 0611033104
 Latitude: 33' 50222.000 North
 Longitude: 111' 93324.000 West
 CAMELBACK RD WO GOLDWATER BLVD
 EB

Start Time	Wed	11-Jun-03	Thu	12-Jun-03	Fri	13-Jun-03	Daily Average	
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	*	310	42	*	*	*	42	310
12:15	*	309	27	*	*	*	27	309
12:30	*	308	23	*	*	*	23	308
12:45	*	274	18	*	*	*	18	274
01:00	*	278	38	*	*	*	38	278
01:15	*	283	19	*	*	*	19	283
01:30	*	275	24	*	*	*	24	275
01:45	*	321	17	*	*	*	17	321
02:00	*	295	7	*	*	*	7	295
02:15	*	293	12	*	*	*	12	293
02:30	*	296	12	*	*	*	12	296
02:45	*	308	8	*	*	*	8	308
03:00	*	311	5	*	*	*	5	311
03:15	*	292	8	*	*	*	8	292
03:30	*	302	5	*	*	*	5	302
03:45	*	302	7	*	*	*	7	302
04:00	*	320	11	*	*	*	11	320
04:15	*	334	10	*	*	*	10	334
04:30	*	313	21	*	*	*	21	313
04:45	*	344	27	*	*	*	27	344
05:00	*	417	32	*	*	*	32	417
05:15	*	510	35	*	*	*	35	510
05:30	*	458	42	*	*	*	42	458
05:45	*	441	82	*	*	*	82	441
06:00	*	365	72	*	*	*	72	365
06:15	*	309	81	*	*	*	81	309
06:30	*	342	136	*	*	*	136	342
06:45	*	285	165	*	*	*	165	285
07:00	*	243	148	*	*	*	148	243
07:15	*	196	173	*	*	*	173	196
07:30	*	195	185	*	*	*	185	195
07:45	*	188	263	*	*	*	263	188
08:00	*	177	205	*	*	*	205	177
08:15	*	151	192	*	*	*	192	151
08:30	*	178	252	*	*	*	252	178
08:45	*	117	241	*	*	*	241	117
09:00	*	142	236	*	*	*	236	142
09:15	*	125	232	*	*	*	232	125
09:30	*	119	238	*	*	*	238	119
09:45	*	146	310	*	*	*	310	146
10:00	*	130	230	*	*	*	230	130
10:15	*	112	228	*	*	*	228	112
10:30	*	100	236	*	*	*	236	100
10:45	*	91	282	*	*	*	282	91
11:00	*	86	297	*	*	*	297	86
11:15	*	54	331	*	*	*	331	54
11:30	*	45	402	*	*	*	402	45
11:45	*	44	407	*	*	*	407	44
Total	0	11834	6074	0	0	0	6074	11834
Combined Total		11834	6074		0		17908	
Peak		05:00	11:00				11:00	05:00
Volume		1826	1437				1437	1826
P.H.F.		0.895	0.883				0.883	0.895
ADT		Not Calculated						

Start Time	Wed	11-Jun-03	Thu	12-Jun-03	Fri	13-Jun-03	Daily Average	
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	*	279	54	*	*	*	54	279
12:15	*	248	50	*	*	*	50	248
12:30	*	257	33	*	*	*	33	257
12:45	*	270	36	*	*	*	36	270
01:00	*	275	32	*	*	*	32	275
01:15	*	300	58	*	*	*	58	300
01:30	*	305	29	*	*	*	29	305
01:45	*	280	29	*	*	*	29	280
02:00	*	262	20	*	*	*	20	262
02:15	*	275	26	*	*	*	26	275
02:30	*	291	21	*	*	*	21	291
02:45	*	249	9	*	*	*	9	249
03:00	*	293	9	*	*	*	9	293
03:15	*	281	4	*	*	*	4	281
03:30	*	263	11	*	*	*	11	263
03:45	*	256	5	*	*	*	5	256
04:00	*	276	14	*	*	*	14	276
04:15	*	363	10	*	*	*	10	363
04:30	*	306	18	*	*	*	18	306
04:45	*	274	38	*	*	*	38	274
05:00	*	354	26	*	*	*	26	354
05:15	*	356	41	*	*	*	41	356
05:30	*	299	64	*	*	*	64	299
05:45	*	287	69	*	*	*	69	287
06:00	*	258	81	*	*	*	81	258
06:15	*	245	117	*	*	*	117	245
06:30	*	210	159	*	*	*	159	210
06:45	*	212	172	*	*	*	172	212
07:00	*	202	192	*	*	*	192	202
07:15	*	189	256	*	*	*	256	189
07:30	*	170	300	*	*	*	300	170
07:45	*	146	317	*	*	*	317	146
08:00	*	156	307	*	*	*	307	156
08:15	*	184	281	*	*	*	281	184
08:30	*	196	286	*	*	*	286	196
08:45	*	191	265	*	*	*	265	191
09:00	*	190	202	*	*	*	202	190
09:15	*	194	218	*	*	*	218	194
09:30	*	168	201	*	*	*	201	168
09:45	*	153	202	*	*	*	202	153
10:00	*	129	189	*	*	*	189	129
10:15	*	154	214	*	*	*	214	154
10:30	*	117	190	*	*	*	190	117
10:45	*	90	212	*	*	*	212	90
11:00	*	83	226	*	*	*	226	83
11:15	*	72	217	*	*	*	217	72
11:30	*	56	232	*	*	*	232	56
11:45	*	68	242	*	*	*	242	68
Total	0	10732	5984	0	0	0	5984	10732
Combined Total	10732		5984		0		16716	
Peak		04:15	07:30				07:30	04:15
Volume		1297	1205				1205	1297
P.H.F.		0.893	0.950				0.950	0.893
ADT		Not Calculated						

Traffic Research & Analysis, Inc.
 3844 East Indian School Road
 Phoenix, Arizona 85018
 (602) 840-1500 & Fax: (602) 840-1577

File Name : 03359UA
 Site Code : 00000000
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 Page No : 1

Groups Printed- 1 - Unshifted

Start Time	GOLDWATER BLVD From North					CAMELBACK RD From East					GOLDWATER BLVD From South					CAMELBACK RD From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
07:00	0	74	60	0	134	8	118	1	0	127	17	28	3	0	48	22	86	29	0	137	446
07:15	0	123	152	0	275	13	130	3	0	146	31	40	5	0	76	34	127	30	0	191	688
07:30	0	125	140	0	265	12	180	2	0	194	35	41	3	0	79	32	121	32	0	185	723
07:45	0	157	124	0	281	13	153	1	0	167	42	66	13	0	121	50	160	66	0	276	845
Total	0	479	476	0	955	46	581	7	0	634	125	175	24	0	324	138	494	157	0	789	2702
08:00	1	134	121	0	256	13	148	6	0	167	42	63	0	0	105	24	113	52	0	189	717
08:15	1	99	121	0	221	12	150	6	0	168	38	46	6	0	90	36	147	36	0	219	698
08:30	0	123	91	0	214	7	155	6	0	168	42	61	5	0	108	43	146	32	0	221	711
08:45	0	90	101	0	191	20	147	8	0	175	42	79	11	0	132	55	157	50	0	262	760
Total	2	446	434	0	882	52	600	26	0	678	164	249	22	0	435	158	563	170	0	891	2886
Grand Total	2	925	910	0	1837	98	1181	33	0	1312	289	424	46	0	759	296	1057	327	0	1680	5588
Apprch %	0.1	50.4	49.5	0.0		7.5	90.0	2.5	0.0		38.1	55.9	6.1	0.0		17.6	62.9	19.5	0.0		
Total %	0.0	16.6	16.3	0.0	32.9	1.8	21.1	0.6	0.0	23.5	5.2	7.6	0.8	0.0	13.6	5.3	18.9	5.9	0.0	30.1	

Traffic Research & Analysis, Inc.
 3844 East Indian School Road
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File Name : 03359UP
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Groups Printed- 1 - Unshifted

Start Time	GOLDWATER BLVD From North					CAMELBACK RD From East					GOLDWATER BLVD From South					CAMELBACK RD From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
15:00	3	125	67	0	195	13	143	20	0	176	53	113	8	0	174	38	139	54	0	231	776
15:15	6	125	83	0	214	9	135	14	0	158	50	100	7	0	157	64	192	39	0	295	824
15:30	11	166	73	0	250	21	159	14	0	194	66	86	10	0	162	59	170	62	0	291	897
15:45	14	174	53	0	241	11	163	19	0	193	59	116	11	0	186	60	183	40	0	283	903
Total	34	590	276	0	900	54	600	67	0	721	228	415	36	0	679	221	684	195	0	1100	3400
16:00	27	187	74	0	288	9	164	24	0	197	79	101	8	0	188	37	232	43	0	312	985
16:15	19	177	60	0	256	10	145	8	0	163	67	95	2	0	164	45	224	23	0	292	875
16:30	10	175	88	0	273	5	120	13	0	138	51	137	6	0	194	80	197	49	0	326	931
16:45	12	185	74	0	271	10	160	14	0	184	51	139	16	0	206	74	213	74	0	361	1022
Total	68	724	296	0	1088	34	589	59	0	682	248	472	32	0	752	236	866	189	0	1291	3813
17:00	25	276	108	0	409	15	180	13	0	208	97	142	8	0	247	70	308	73	0	451	1315
17:15	22	244	102	0	368	21	164	7	0	192	69	116	14	0	199	76	326	71	0	473	1232
17:30	6	224	64	0	294	24	140	15	0	179	53	114	3	0	170	78	339	36	0	453	1096
17:45	25	135	60	0	220	20	120	8	0	148	54	96	8	0	158	68	252	26	0	346	872
Total	78	879	334	0	1291	80	604	43	0	727	273	468	33	0	774	292	1225	206	0	1723	4515
Grand Total	180	2193	906	0	3279	168	1793	169	0	2130	749	1355	101	0	2205	749	2775	590	0	4114	11728
Apprch %	5.5	66.9	27.6	0.0		7.9	84.2	7.9	0.0		34.0	61.5	4.6	0.0		18.2	67.5	14.3	0.0		
Total %	1.5	18.7	7.7	0.0	28.0	1.4	15.3	1.4	0.0	18.2	6.4	11.6	0.9	0.0	18.8	6.4	23.7	5.0	0.0	35.1	

Traffic Research & Analysis, Inc.
 3844 East Indian School Road
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 (602) 840-1500 & Fax: (602) 840-1577

File Name : 03360UA
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Groups Printed: 1 - Unshifted

Start Time	SCOTTSDALE RD From North					CAMELBACK RD From East					SCOTTSDALE RD From South					CAMELBACK RD From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
07:00	18	78	13	0	109	6	117	17	0	140	15	89	6	0	90	19	49	20	0	88	427
07:15	15	122	15	0	152	7	123	19	0	149	14	92	2	0	108	22	59	21	0	102	511
07:30	22	145	19	0	186	9	167	29	0	205	23	127	2	0	152	21	55	24	0	100	643
07:45	34	159	19	0	212	14	174	35	0	223	28	102	7	0	137	25	104	26	0	155	727
Total	89	504	66	0	659	36	581	100	0	717	80	390	17	0	487	87	267	91	0	446	2308
08:00	38	129	23	0	190	8	146	29	0	183	41	89	4	0	134	16	63	15	0	94	601
08:15	30	122	15	0	167	12	160	35	0	207	39	99	7	0	145	20	84	16	0	120	639
08:30	37	130	16	0	183	22	136	37	0	195	24	99	5	0	128	30	70	32	0	132	638
08:45	37	143	19	0	199	13	149	29	0	191	40	128	5	0	173	30	89	20	0	139	702
Total	142	524	73	0	739	55	591	130	0	776	144	415	21	0	580	96	306	83	0	485	2580
Grand Total	231	1028	139	0	1398	91	1172	230	0	1493	224	805	38	0	1067	183	573	174	0	930	4888
Apprch %	16.5	73.5	9.9	0.0		6.1	78.5	15.4	0.0		21.0	75.4	3.6	0.0		19.7	61.6	18.7	0.0		
Total %	4.7	21.0	2.8	0.0	28.6	1.9	24.0	4.7	0.0	30.5	4.6	16.5	0.8	0.0	21.8	3.7	11.7	3.6	0.0	19.0	

Traffic Research & Analysis, Inc.
 3844 East Indian School Road
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Groups Printed- 1 - Unshifted

Start Time	SCOTTSDALE RD From North					CAMELBACK RD From East					SCOTTSDALE RD From South					CAMELBACK RD From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
15:00	44	157	50	0	251	19	124	42	0	185	66	195	13	0	274	50	116	54	0	220	930
15:15	48	142	44	0	234	22	106	32	0	160	68	181	32	0	281	48	130	49	0	227	902
15:30	53	151	49	0	253	10	101	40	0	151	70	201	21	0	292	64	126	48	0	238	934
15:45	43	169	53	0	265	15	120	45	0	180	79	188	22	0	289	57	158	48	0	263	997
Total	188	619	196	0	1003	66	451	159	0	676	283	765	88	0	1136	219	530	199	0	948	3763
16:00	61	151	40	0	252	16	110	33	0	159	69	220	16	0	305	56	158	50	0	264	980
16:15	54	128	41	0	223	11	104	31	0	146	71	183	18	0	272	56	188	49	0	293	934
16:30	77	146	43	0	266	8	115	38	0	161	57	204	22	0	283	54	166	41	0	261	971
16:45	71	166	46	0	283	8	132	33	0	173	68	246	15	0	329	61	179	41	0	281	1066
Total	263	591	170	0	1024	43	461	135	0	639	265	853	71	0	1189	227	691	181	0	1099	3951
17:00	80	160	45	0	285	13	108	36	0	157	91	283	29	0	403	55	194	51	0	300	1145
17:15	56	163	37	0	256	18	121	41	0	180	63	257	24	0	344	63	231	48	0	342	1122
17:30	77	156	40	0	273	18	122	33	0	173	61	223	25	0	309	63	233	57	0	353	1108
17:45	47	170	45	0	262	15	111	44	0	170	64	179	20	0	263	46	195	55	0	296	991
Total	260	649	167	0	1076	64	462	154	0	680	279	942	98	0	1319	227	853	211	0	1291	4366
Grand Total	711	1859	533	0	3103	173	1374	448	0	1995	827	2560	257	0	3644	673	2074	591	0	3338	12080
Apprch %	22.9	59.9	17.2	0.0		8.7	68.9	22.5	0.0		22.7	70.3	7.1	0.0		20.2	62.1	17.7	0.0		
Total %	5.9	15.4	4.4	0.0	25.7	1.4	11.4	3.7	0.0	16.5	6.8	21.2	2.1	0.0	30.2	5.6	17.2	4.9	0.0	27.6	

Traffic Research & Analysis, Inc.
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 (602) 840-1500 & Fax: (602) 840-1577

File Name : 03361UA
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 Page No : 1

Groups Printed- 1 - Unshifted

Start Time	GOLDWATER BLVD From North					MONTECITO AVE From East					GOLDWATER BLVD From South					NONE From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
07:00	6	111	0	0	117	1	0	1	0	2	0	50	4	0	54	0	0	0	0	0	173
07:15	2	126	0	0	128	0	0	0	0	0	0	82	3	0	85	0	0	0	0	0	213
07:30	1	147	0	0	148	1	0	0	0	1	0	85	1	0	86	0	0	0	0	0	235
07:45	5	209	0	0	214	1	0	0	0	1	0	118	6	0	124	0	0	0	0	0	339
Total	14	593	0	0	607	3	0	1	0	4	0	335	14	0	349	0	0	0	0	0	960
08:00	3	179	0	0	182	0	0	0	0	0	0	125	3	0	128	0	0	0	0	0	310
08:15	5	123	0	0	128	2	0	0	0	2	0	96	10	0	106	0	0	0	0	0	236
08:30	11	143	0	0	154	1	0	2	0	3	0	120	6	0	126	0	0	0	0	0	283
08:45	10	126	0	0	136	1	0	0	0	1	0	109	4	0	113	0	0	0	0	0	250
Total	29	571	0	0	600	4	0	2	0	6	0	450	23	0	473	0	0	0	0	0	1079
Grand Total	43	1164	0	0	1207	7	0	3	0	10	0	785	37	0	822	0	0	0	0	0	2039
Approch %	3.6	96.4	0.0	0.0		70.0	0.0	30.0	0.0		0.0	95.5	4.5	0.0		0.0	0.0	0.0	0.0		
Total %	2.1	57.1	0.0	0.0	59.2	0.3	0.0	0.1	0.0	0.5	0.0	38.5	1.8	0.0	40.3	0.0	0.0	0.0	0.0	0.0	

Traffic Research & Analysis, Inc.
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Groups Printed- 1 - Unshifted

Start Time	GOLDWATER BLVD From North					MONTECITO AVE From East					GOLDWATER BLVD From South					NONE From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
15:00	34	169	0	0	203	28	0	26	0	54	0	136	15	0	151	0	0	0	0	0	408
15:15	24	133	0	0	157	15	0	27	0	42	0	121	14	0	135	0	0	0	0	0	334
15:30	39	166	0	0	205	14	0	19	0	33	0	124	19	0	143	0	0	0	0	0	381
15:45	25	164	0	0	189	20	0	33	0	53	0	173	9	0	182	0	0	0	0	0	424
Total	122	632	0	0	754	77	0	105	0	182	0	554	57	0	611	0	0	0	0	0	1547
16:00	19	177	0	0	196	22	0	36	0	58	0	149	16	0	165	0	0	0	0	0	419
16:15	29	163	0	0	192	16	0	26	0	42	0	135	15	0	150	0	0	0	0	0	384
16:30	22	165	0	0	187	16	0	19	0	35	0	191	12	0	203	0	0	0	0	0	425
16:45	24	190	0	0	214	21	0	30	0	51	0	169	18	0	187	0	0	0	0	0	452
Total	94	695	0	0	789	75	0	111	0	186	0	644	61	0	705	0	0	0	0	0	1680
17:00	18	227	0	0	245	14	0	31	0	45	0	214	14	0	228	0	0	0	0	0	518
17:15	25	247	0	0	272	16	0	16	0	32	0	181	14	0	195	0	0	0	0	0	499
17:30	29	242	0	0	271	25	0	16	0	41	0	152	19	0	171	0	0	0	0	0	483
17:45	16	182	0	0	198	15	0	19	0	34	0	146	24	0	170	0	0	0	0	0	402
Total	88	898	0	0	986	70	0	82	0	152	0	693	71	0	764	0	0	0	0	0	1902
Grand Total	304	2225	0	0	2529	222	0	298	0	520	0	1891	189	0	2080	0	0	0	0	0	5129
Apprch %	12.0	88.0	0.0	0.0		42.7	0.0	57.3	0.0		0.0	90.9	9.1	0.0		0.0	0.0	0.0	0.0		
Total %	5.9	43.4	0.0	0.0	49.3	4.3	0.0	5.8	0.0	10.1	0.0	36.9	3.7	0.0	40.6	0.0	0.0	0.0	0.0	0.0	

Traffic Research & Analysis, Inc.
 3844 East Indian School Road
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Groups Printed- 1 - Unshifted

Start Time	MARSHALL WAY From North					CAMELBACK RD From East					MARSHALL WAY From South					CAMELBACK RD From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
07:00	2	0	1	0	3	8	105	11	0	124	0	0	1	0	1	8	91	0	0	99	227
07:15	1	1	9	0	11	4	132	14	0	150	0	0	2	0	2	7	94	0	0	101	264
07:30	4	0	2	0	6	6	184	14	0	204	0	0	0	0	0	9	104	0	0	113	323
07:45	4	0	7	0	11	11	182	25	0	218	0	0	2	0	2	18	139	0	0	157	388
Total	11	1	19	0	31	29	603	64	0	696	0	0	5	0	5	42	428	0	0	470	1202
08:00	2	0	8	0	10	4	178	18	0	200	0	1	3	0	4	10	94	0	0	104	318
08:15	3	1	1	0	5	10	187	16	0	213	0	0	3	0	3	18	113	0	0	131	352
08:30	5	0	8	0	13	4	177	9	0	190	0	0	1	0	1	10	120	2	0	132	336
08:45	2	1	14	0	17	10	170	22	0	202	0	0	1	0	1	14	129	0	0	143	363
Total	12	2	31	0	45	28	712	65	0	805	0	1	8	0	9	52	456	2	0	510	1369
Grand Total	23	3	50	0	76	57	1315	129	0	1501	0	1	13	0	14	94	884	2	0	980	2571
Apprch %	30.3	3.9	65.8	0.0		3.8	87.6	8.6	0.0		0.0	7.1	92.9	0.0		9.6	90.2	0.2	0.0		
Total %	0.9	0.1	1.9	0.0	3.0	2.2	51.1	5.0	0.0	58.4	0.0	0.0	0.5	0.0	0.5	3.7	34.4	0.1	0.0	38.1	

Traffic Research & Analysis, Inc.
 3844 East Indian School Road
 Phoenix, Arizona 85018
 (602) 840-1500 & Fax: (602) 840-1577

File Name : 03362UP
 Site Code : 00000000
 Start Date : 06/12/2003
 Page No : 1

Groups Printed- 1 - Unshifted

Start Time	MARSHALL WAY From North					CAMELBACK RD From East					MARSHALL WAY From South					CAMELBACK RD From West					Int. Total
	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
15:00	28	3	22	0	53	23	149	34	0	206	0	1	26	0	27	13	137	7	0	157	443
15:15	34	3	14	0	51	19	139	34	0	192	4	3	22	0	29	23	148	7	0	178	450
15:30	35	1	13	0	49	13	135	32	0	180	1	0	22	0	23	14	150	1	0	165	417
15:45	25	7	26	0	58	16	159	37	0	212	0	10	23	0	33	29	178	10	0	217	520
Total	122	14	75	0	211	71	582	137	0	790	5	14	93	0	112	79	613	25	0	717	1830
16:00	29	7	37	0	73	15	149	39	0	203	2	2	16	0	20	19	159	5	0	183	479
16:15	41	7	24	0	72	12	144	37	0	193	2	0	33	0	35	22	186	4	0	212	512
16:30	30	4	25	0	59	10	138	33	0	181	1	4	33	0	38	23	177	5	0	205	483
16:45	28	4	37	0	69	24	152	44	0	220	2	4	47	0	53	35	190	5	0	230	572
Total	128	22	123	0	273	61	583	153	0	797	7	10	129	0	146	99	712	19	0	830	2046
17:00	33	1	31	0	65	10	181	36	0	227	3	3	21	0	27	28	268	3	0	299	618
17:15	32	5	26	0	63	12	156	38	0	206	5	1	24	0	30	29	244	9	0	282	581
17:30	30	5	25	0	60	11	155	40	0	206	1	0	32	0	33	39	266	8	0	313	612
17:45	23	3	28	0	54	14	136	35	0	185	0	2	28	0	30	35	234	5	0	274	543
Total	118	14	110	0	242	47	628	149	0	824	9	6	105	0	120	131	1012	25	0	1168	2354
Grand Total	368	50	308	0	726	179	1793	439	0	2411	21	30	327	0	378	309	2337	69	0	2715	6230
Apprch %	50.7	6.9	42.4	0.0		7.4	74.4	18.2	0.0		5.6	7.9	86.5	0.0		11.4	86.1	2.5	0.0		
Total %	5.9	0.8	4.9	0.0	11.7	2.9	28.8	7.0	0.0	38.7	0.3	0.5	5.2	0.0	6.1	5.0	37.5	1.1	0.0	43.6	

APPENDIX B

LEVEL OF SERVICE ANALYSIS – SYNCHRO 5.0 OUTPUT

HCM Signalized Intersection Capacity Analysis
 6: Camelback Road & Goldwater Blvd

Existing AM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖↖↖	↗	↖	↖↖↖		↖↖	↖↖	↗	↖↖	↖↖↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91		0.97	0.95	1.00	0.97	0.91	1.00
Fr't	1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1583	1770	5068		3433	3539	1583	3433	5085	1583
Flt Permitted	0.31	1.00	1.00	0.33	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	580	5085	1583	609	5068		3433	3539	1583	3433	5085	1583
Volume (vph)	145	552	190	51	644	15	160	220	22	2	525	371
Peak-hour factor, PHF	0.79	0.79	0.79	0.90	0.90	0.90	0.82	0.82	0.82	0.91	0.91	0.91
Adj. Flow (vph)	184	699	241	57	716	17	195	268	27	2	577	408
Lane Group Flow (vph)	184	699	241	57	733	0	195	268	27	2	577	408
Turn Type	pm+pt		Perm	pm+pt			Prot		Perm	Prot		Perm
Protected Phases	6	1		2	5		4	7		8	3	
Permitted Phases	1		1	5				7				3
Actuated Green, G (s)	48.5	42.2	42.2	48.5	42.2		12.8	28.5	28.5	5.0	20.7	20.7
Effective Green, g (s)	50.5	44.2	44.2	50.5	44.2		12.8	30.5	30.5	5.0	22.7	22.7
Actuated g/C Ratio	0.50	0.43	0.43	0.50	0.43		0.13	0.30	0.30	0.05	0.22	0.22
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0	6.0	4.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	361	2204	686	373	2196		431	1058	473	168	1132	352
v/s Ratio Prot	c0.03	0.14		0.01	0.14		c0.06	0.08		0.00	0.11	
v/s Ratio Perm	c0.22		0.15	0.07					0.02			0.26
v/c Ratio	0.51	0.32	0.35	0.15	0.33		0.45	0.25	0.06	0.01	0.51	1.16
Uniform Delay, d1	22.4	19.0	19.3	17.4	19.1		41.4	27.1	25.5	46.1	34.8	39.6
Progression Factor	1.00	1.00	1.00	1.04	0.88		1.02	0.99	0.94	1.00	1.00	1.00
Incremental Delay, d2	1.1	0.4	1.4	0.2	0.4		0.8	0.1	0.1	0.0	0.4	98.6
Delay (s)	23.5	19.4	20.7	18.3	17.2		42.7	26.8	24.1	46.2	35.1	138.3
Level of Service	C	B	C	B	B		D	C	C	D	D	F
Approach Delay (s)		20.3			17.3			33.0			77.8	
Approach LOS		C			B			C			E	

Intersection Summary		
HCM Average Control Delay	38.2	HCM Level of Service
HCM Volume to Capacity ratio	0.67	
Actuated Cycle Length (s)	102.0	Sum of lost time (s)
Intersection Capacity Utilization	55.0%	ICU Level of Service
c Critical Lane Group		

HCM Signalized Intersection Capacity Analysis
 8: Montecito Ave & Goldwater Blvd

Existing AM PK
 9/4/2003

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗	↕	↖	↗	↕
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0		4.0	4.0	4.0	4.0
Lane Util. Factor	1.00		0.95	1.00	1.00	0.91
Fr't	1.00		1.00	0.85	1.00	1.00
Flt Protected	0.95		1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770		3539	1583	1770	5085
Flt Permitted	0.95		1.00	1.00	0.49	1.00
Satd. Flow (perm)	1770		3539	1583	920	5085
Volume (vph)	4	0	424	20	14	658
Peak-hour factor, PHF	0.50	0.50	0.95	0.95	0.79	0.79
Adj. Flow (vph)	8	0	446	21	18	833
Lane Group Flow (vph)	8	0	446	21	18	833
Turn Type		Perm		Perm	Perm	
Protected Phases	4		2			2
Permitted Phases		4		2	2	
Actuated Green, G (s)	1.4		89.6	89.6	89.6	89.6
Effective Green, g (s)	2.4		91.6	91.6	91.6	91.6
Actuated g/C Ratio	0.02		0.90	0.90	0.90	0.90
Clearance Time (s)	5.0		6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	42		3178	1422	826	4567
v/s Ratio Prot	c0.00		0.13			c0.16
v/s Ratio Perm				0.01	0.02	
v/c Ratio	0.19		0.14	0.01	0.02	0.18
Uniform Delay, d1	48.8		0.6	0.5	0.5	0.6
Progression Factor	1.00		1.00	1.00	2.43	2.55
Incremental Delay, d2	2.2		0.1	0.0	0.0	0.1
Delay (s)	51.1		0.7	0.6	1.4	1.7
Level of Service	D		A	A	A	A
Approach Delay (s)	51.1		0.7			1.7
Approach LOS	D		A			A

Intersection Summary

HCM Average Control Delay	1.6	HCM Level of Service	A
HCM Volume to Capacity ratio	0.18		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	26.1%	ICU Level of Service	A
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 10: Camelback Road & Marshall Way

Existing AM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↵	↕↕	↗	↵	↕↕↕	↗		↕↗			↕↗	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0		4.0	4.0	4.0		4.0			4.0	
Lane Util. Factor	1.00	0.95		1.00	0.91	1.00		0.95			0.95	
Frt	1.00	1.00		1.00	1.00	0.85		0.87			0.91	
Flt Protected	0.95	1.00		0.95	1.00	1.00		1.00			0.98	
Satd. Flow (prot)	1770	3539		1770	5085	1583		3079			3172	
Flt Permitted	0.34	1.00		0.45	1.00	1.00		1.00			0.85	
Satd. Flow (perm)	642	3539		837	5085	1583		3079			2736	
Volume (vph)	55	450	0	31	731	73	0	1	8	13	1	18
Peak-hour factor, PHF	0.83	0.83	0.83	0.94	0.94	0.94	0.63	0.63	0.63	0.75	0.75	0.75
Adj. Flow (vph)	66	542	0	33	778	78	0	2	13	17	1	24
Lane Group Flow (vph)	66	542	0	33	778	78	0	15	0	0	42	0
Turn Type	pm+pt		Perm	pm+pt		Perm	Perm			Perm		
Protected Phases	3	2		3	2			4			4	
Permitted Phases	2		2	2		2	4			4		
Actuated Green, G (s)	81.4	78.2		81.4	78.2	78.2		4.6			4.6	
Effective Green, g (s)	83.4	80.2		83.4	80.2	80.2		6.6			6.6	
Actuated g/C Ratio	0.82	0.79		0.82	0.79	0.79		0.06			0.06	
Clearance Time (s)	4.0	6.0		4.0	6.0	6.0		6.0			6.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	560	2783		714	3998	1245		199			177	
v/s Ratio Prot	c0.00	c0.15		0.00	0.15			0.00				
v/s Ratio Perm	0.09			0.04		0.05					c0.02	
v/c Ratio	0.12	0.19		0.05	0.19	0.06		0.08			0.24	
Uniform Delay, d1	1.9	2.8		1.8	2.8	2.5		44.8			45.3	
Progression Factor	4.12	2.70		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.1	0.2		0.0	0.1	0.1		0.2			0.7	
Delay (s)	8.0	7.6		1.8	2.9	2.5		45.0			46.0	
Level of Service	A	A		A	A	A		D			D	
Approach Delay (s)		7.6			2.8			45.0			46.0	
Approach LOS		A			A			D			D	

Intersection Summary

HCM Average Control Delay	6.3	HCM Level of Service	A
HCM Volume to Capacity ratio	0.20		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	32.0%	ICU Level of Service	A
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 3: Camelback Road & Scottsdale Road

Existing AM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖↖	↖	↖	↖↖		↖↖	↖↖↖		↖↖	↖↖	↖
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.97	0.91		0.97	0.95	1.00
Frt	1.00	1.00	0.85	1.00	0.98		1.00	0.99		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3451		3433	5050		3433	3539	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1770	3539	1583	1770	3451		3433	5050		3433	3539	1583
Volume (vph)	84	312	83	44	660	131	134	425	20	126	566	78
Peak-hour factor, PHF	0.76	0.76	0.76	0.92	0.92	0.92	0.93	0.93	0.93	0.89	0.89	0.89
Adj. Flow (vph)	111	411	109	48	717	142	144	457	22	142	636	88
Lane Group Flow (vph)	111	411	109	48	859	0	144	479	0	142	636	88
Turn Type	Prot		Perm	Prot			Prot			Prot		Perm
Protected Phases	4	7		8	3		6	1		2	5	
Permitted Phases			7									5
Actuated Green, G (s)	11.1	26.0	26.0	9.1	24.0		9.0	37.9		9.0	37.9	37.9
Effective Green, g (s)	11.1	28.0	28.0	9.1	26.0		9.0	39.9		9.0	39.9	39.9
Actuated g/C Ratio	0.11	0.27	0.27	0.09	0.25		0.09	0.39		0.09	0.39	0.39
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0		4.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	193	971	435	158	880		303	1975		303	1384	619
v/s Ratio Prot	c0.06	0.12		0.03	c0.25		c0.04	0.09		0.04	c0.18	
v/s Ratio Perm			0.07									0.06
v/c Ratio	0.58	0.42	0.25	0.30	0.98		0.48	0.24		0.47	0.46	0.14
Uniform Delay, d1	43.2	30.4	28.8	43.5	37.7		44.3	20.9		44.2	23.0	20.0
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	4.1	0.3	0.3	1.1	24.4		1.2	0.3		1.1	1.1	0.5
Delay (s)	47.3	30.7	29.1	44.6	62.1		45.4	21.2		45.4	24.1	20.5
Level of Service	D	C	C	D	E		D	C		D	C	C
Approach Delay (s)		33.3			61.1			26.8			27.3	
Approach LOS		C			E			C			C	

Intersection Summary			
HCM Average Control Delay	38.6	HCM Level of Service	D
HCM Volume to Capacity ratio	0.60		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	65.5%	ICU Level of Service	B
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 6: Camelback Road & Goldwater Blvd

Existing PM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖↖↖	↗	↖	↖↖↖		↗↗	↖↖	↗	↗↗	↖↖↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91		0.97	0.95	1.00	0.97	0.91	1.00
Fr't	1.00	1.00	0.85	1.00	0.99		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1583	1770	5031		3433	3539	1583	3433	5085	1583
Flt Permitted	0.27	1.00	1.00	0.10	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	504	5085	1583	193	5031		3433	3539	1583	3433	5085	1583
Volume (vph)	304	1210	259	71	657	50	275	521	42	66	948	355
Peak-hour factor, PHF	0.79	0.79	0.79	0.90	0.90	0.90	0.82	0.82	0.82	0.91	0.91	0.91
Adj. Flow (vph)	385	1532	328	79	730	56	335	635	51	73	1042	390
Lane Group Flow (vph)	385	1532	328	79	786	0	335	635	51	73	1042	390
Turn Type	pm+pt		Perm	pm+pt			Prot		Perm	Prot		pm+ov
Protected Phases	6	1		2	5		4	7		8	3	6
Permitted Phases	1		1	5				7				3
Actuated Green, G (s)	44.6	36.6	36.6	44.6	36.6		10.8	26.4	26.4	11.0	26.6	34.6
Effective Green, g (s)	46.6	38.6	38.6	46.6	38.6		10.8	28.4	28.4	11.0	28.6	36.6
Actuated g/C Ratio	0.46	0.38	0.38	0.46	0.38		0.11	0.28	0.28	0.11	0.28	0.36
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0	6.0	4.0	6.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	330	1924	599	212	1904		363	985	441	370	1426	630
v/s Ratio Prot	c0.09	0.30		0.03	0.16		c0.10	0.18		0.02	c0.20	0.05
v/s Ratio Perm	c0.44		0.21	0.14					0.03			0.20
v/c Ratio	1.17	0.80	0.55	0.37	0.41		0.92	0.64	0.12	0.20	0.73	0.62
Uniform Delay, d1	30.8	28.2	24.9	36.9	23.4		45.2	32.4	27.4	41.5	33.2	27.0
Progression Factor	1.00	1.00	1.00	0.97	0.91		1.17	1.03	1.49	1.00	1.00	1.00
Incremental Delay, d2	102.8	3.5	3.6	1.1	0.6		27.8	1.4	0.1	0.3	2.0	1.8
Delay (s)	133.6	31.7	28.4	37.0	21.8		80.7	34.8	41.1	41.7	35.2	28.8
Level of Service	F	C	C	D	C		F	C	D	D	D	C
Approach Delay (s)		48.7			23.2			50.1			33.8	
Approach LOS		D			C			D			C	

Intersection Summary

HCM Average Control Delay	41.1	HCM Level of Service	D
HCM Volume to Capacity ratio	0.99		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	16.0
Intersection Capacity Utilization	79.7%	ICU Level of Service	C
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 8: Montecito Ave & Goldwater Blvd

Existing PM PK
 9/4/2003

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗	↕	↗	↖	↕
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.91
Fr't	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1583	3539	1583	1770	5085
Flt Permitted	0.95	1.00	1.00	1.00	0.35	1.00
Satd. Flow (perm)	1770	1583	3539	1583	652	5085
Volume (vph)	78	95	730	66	98	924
Peak-hour factor, PHF	0.50	0.50	0.95	0.95	0.79	0.79
Adj. Flow (vph)	156	190	768	69	124	1170
Lane Group Flow (vph)	156	190	768	69	124	1170
Turn Type		Perm		Perm	Perm	
Protected Phases	4		2			2
Permitted Phases		4		2	2	
Actuated Green, G (s)	12.9	12.9	78.1	78.1	78.1	78.1
Effective Green, g (s)	13.9	13.9	80.1	80.1	80.1	80.1
Actuated g/C Ratio	0.14	0.14	0.79	0.79	0.79	0.79
Clearance Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	241	216	2779	1243	512	3993
v/s Ratio Prot	0.09		0.22			c0.23
v/s Ratio Perm		0.12		0.04	0.19	
v/c Ratio	0.65	0.88	0.28	0.06	0.24	0.29
Uniform Delay, d1	41.7	43.2	3.0	2.5	2.9	3.1
Progression Factor	1.00	1.00	1.00	1.00	2.89	2.93
Incremental Delay, d2	5.9	30.8	0.2	0.1	0.9	0.2
Delay (s)	47.6	74.0	3.2	2.5	9.3	9.1
Level of Service	D	E	A	A	A	A
Approach Delay (s)	62.1		3.2			9.1
Approach LOS	E		A			A

Intersection Summary			
HCM Average Control Delay	14.5	HCM Level of Service	B
HCM Volume to Capacity ratio	0.38		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	46.8%	ICU Level of Service	A
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 10: Camelback Road & Marshall Way

Existing PM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↶	↷	↶	↶	↷	↶		↷			↷	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0		4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	1.00		0.95			0.95	
Frt	1.00	1.00	0.85	1.00	1.00	0.85		0.87			0.93	
Fit Protected	0.95	1.00	1.00	0.95	1.00	1.00		1.00			0.98	
Satd. Flow (prot)	1770	3539	1583	1770	5085	1583		3066			3216	
Fit Permitted	0.37	1.00	1.00	0.20	1.00	1.00		0.74			0.67	
Satd. Flow (perm)	691	3539	1583	377	5085	1583		2282			2210	
Volume (vph)	134	987	26	58	657	161	11	8	126	125	15	121
Peak-hour factor, PHF	0.83	0.83	0.83	0.94	0.94	0.94	0.63	0.63	0.63	0.75	0.75	0.75
Adj. Flow (vph)	161	1189	31	62	699	171	17	13	200	167	20	161
Lane Group Flow (vph)	161	1189	31	62	699	171	0	230	0	0	348	0
Turn Type	pm+pt		Perm	pm+pt		Perm	Perm				Perm	
Protected Phases	3	2		3	2			4				4
Permitted Phases	2		2	2		2	4			4		
Actuated Green, G (s)	76.0	72.0	72.0	76.0	72.0	72.0		10.0			10.0	
Effective Green, g (s)	78.0	74.0	74.0	78.0	74.0	74.0		12.0			12.0	
Actuated g/C Ratio	0.76	0.73	0.73	0.76	0.73	0.73		0.12			0.12	
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0	6.0		6.0			6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	571	2568	1148	343	3689	1148		268			260	
v/s Ratio Prot	c0.01	c0.34		0.01	0.14							
v/s Ratio Perm	0.20		0.02	0.13		0.11		0.10			c0.16	
v/c Ratio	0.28	0.46	0.03	0.18	0.19	0.15		1.05dr			1.58dl	
Uniform Delay, d1	3.9	5.8	3.9	7.0	4.5	4.3		44.2			45.0	
Progression Factor	0.23	0.56	0.07	1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.2	0.4	0.0	0.3	0.1	0.3		22.8			176.0	
Delay (s)	1.1	3.6	0.3	7.2	4.6	4.6		66.9			221.0	
Level of Service	A	A	A	A	A	A		E			F	
Approach Delay (s)		3.3			4.7			66.9			221.0	
Approach LOS		A			A			E			F	

Intersection Summary

HCM Average Control Delay	35.0	HCM Level of Service	D
HCM Volume to Capacity ratio	0.57		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	67.6%	ICU Level of Service	B

- dl Defacto Left Lane. Recode with 1 though lane as a left lane.
- dr Defacto Right Lane. Recode with 1 though lane as a right lane.
- c Critical Lane Group

HCM Signalized Intersection Capacity Analysis
 3: Camelback Road & Scottsdale Road

Existing PM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗	↘	↖	↗		↖	↗		↖	↗	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.97	0.91		0.97	0.95	1.00
Frt	1.00	1.00	0.85	1.00	0.97		1.00	0.99		1.00	1.00	0.85
Fit Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3418		3433	5021		3433	3539	1583
Fit Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1770	3539	1583	1770	3418		3433	5021		3433	3539	1583
Volume (vph)	247	854	201	58	493	146	289	1029	95	290	658	171
Peak-hour factor, PHF	0.76	0.76	0.76	0.92	0.92	0.92	0.93	0.93	0.93	0.89	0.89	0.89
Adj. Flow (vph)	325	1124	264	63	536	159	311	1106	102	326	739	192
Lane Group Flow (vph)	325	1124	264	63	695	0	311	1208	0	326	739	192
Turn Type	Prot		Perm	Prot			Prot			Prot		Perm
Protected Phases	4	7		8	3		6	1		2	5	
Permitted Phases			7									5
Actuated Green, G (s)	10.8	25.9	25.9	7.9	23.0		10.0	38.2		10.0	38.2	38.2
Effective Green, g (s)	10.8	27.9	27.9	7.9	25.0		10.0	40.2		10.0	40.2	40.2
Actuated g/C Ratio	0.11	0.27	0.27	0.08	0.25		0.10	0.39		0.10	0.39	0.39
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0		4.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	187	968	433	137	838		337	1979		337	1395	624
v/s Ratio Prot	c0.18	c0.32		0.04	0.20		0.09	c0.24		c0.09	0.21	
v/s Ratio Perm			0.17									0.12
v/c Ratio	1.74	1.16	0.61	0.46	0.83		0.92	0.61		0.97	0.53	0.31
Uniform Delay, d1	45.6	37.1	32.3	45.0	36.5		45.6	24.7		45.8	23.7	21.3
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	353.4	84.1	2.4	2.4	6.8		29.9	1.4		39.9	1.4	1.3
Delay (s)	399.0	121.1	34.7	47.4	43.3		75.5	26.1		85.7	25.1	22.6
Level of Service	F	F	C	D	D		E	C		F	C	C
Approach Delay (s)		160.5			43.6			36.2			40.4	
Approach LOS		F			D			D			D	

Intersection Summary

HCM Average Control Delay	78.9	HCM Level of Service	E
HCM Volume to Capacity ratio	0.97		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	16.0
Intersection Capacity Utilization	84.2%	ICU Level of Service	D
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 6: Camelback Road & Goldwater Blvd

2010 BASE AM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖↖↖	↗	↖	↖↖↖		↖↖	↖↖	↗	↖↖	↖↖↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91		0.97	0.95	1.00	0.97	0.91	1.00
Frt	1.00	1.00	0.85	1.00	1.00		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1583	1770	5066		3433	3539	1583	3433	5085	1583
Flt Permitted	0.30	1.00	1.00	0.36	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	558	5085	1583	670	5066		3433	3539	1583	3433	5085	1583
Volume (vph)	167	590	218	59	689	18	184	271	26	2	646	427
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	176	621	229	62	725	19	194	285	27	2	680	449
Lane Group Flow (vph)	176	621	229	62	744	0	194	285	27	2	680	449
Turn Type	pm+pt		Perm	pm+pt		Prot		Perm	Prot		Perm	Prot
Protected Phases	6	1		2	5	4		7	8		3	
Permitted Phases	1		1	5				7				3
Actuated Green, G (s)	46.0	39.7	39.7	46.0	39.7		12.7	31.1	31.1	4.9	23.3	23.3
Effective Green, g (s)	48.0	41.7	41.7	48.0	41.7		12.7	33.1	33.1	4.9	25.3	25.3
Actuated g/C Ratio	0.47	0.41	0.41	0.47	0.41		0.12	0.32	0.32	0.05	0.25	0.25
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0	6.0	4.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	337	2079	647	383	2071		427	1148	514	165	1261	393
v/s Ratio Prot	0.03	0.12		0.01	0.15		0.06	0.08		0.00	0.13	
v/s Ratio Perm	0.21		0.14	0.07					0.02			0.28
v/c Ratio	0.52	0.30	0.35	0.16	0.36		0.45	0.25	0.05	0.01	0.54	1.14
Uniform Delay, d1	24.6	20.3	20.8	18.4	20.9		41.4	25.3	23.7	46.2	33.3	38.4
Progression Factor	1.00	1.00	1.00	1.05	0.89		1.01	0.98	0.95	1.00	1.00	1.00
Incremental Delay, d2	1.5	0.4	1.5	0.2	0.5		0.8	0.1	0.0	0.0	0.4	90.2
Delay (s)	26.1	20.7	22.4	19.5	19.1		42.8	25.0	22.4	46.3	33.7	128.6
Level of Service	C	C	C	B	B		D	C	C	D	C	F
Approach Delay (s)		22.0			19.1			31.7			71.4	
Approach LOS		C			B			C			E	

Intersection Summary		
HCM Average Control Delay	38.8	HCM Level of Service
HCM Volume to Capacity ratio	0.69	D
Actuated Cycle Length (s)	102.0	Sum of lost time (s)
Intersection Capacity Utilization	57.8%	16.0
c Critical Lane Group		ICU Level of Service
		A

HCM Signalized Intersection Capacity Analysis
 8: Montecito Ave & Goldwater Blvd

2010 BASE AM PK
 9/4/2003

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↘	↗	↕	↗	↘	↕
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0		4.0	4.0	4.0	4.0
Lane Util. Factor	1.00		0.95	1.00	1.00	0.91
Frt	1.00		1.00	0.85	1.00	1.00
Flt Protected	0.95		1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770		3539	1583	1770	5085
Flt Permitted	0.95		1.00	1.00	0.45	1.00
Satd. Flow (perm)	1770		3539	1583	832	5085
Volume (vph)	4	0	522	20	14	809
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	4	0	549	21	15	852
Lane Group Flow (vph)	4	0	549	21	15	852
Turn Type		Perm		Perm	Perm	
Protected Phases	4		2			2
Permitted Phases		4		2	2	
Actuated Green, G (s)	1.3		89.7	89.7	89.7	89.7
Effective Green, g (s)	2.3		91.7	91.7	91.7	91.7
Actuated g/C Ratio	0.02		0.90	0.90	0.90	0.90
Clearance Time (s)	5.0		6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	40		3182	1423	748	4572
v/s Ratio Prot	c0.00		0.16			c0.17
v/s Ratio Perm				0.01	0.02	
v/c Ratio	0.10		0.17	0.01	0.02	0.19
Uniform Delay, d1	48.8		0.6	0.5	0.5	0.6
Progression Factor	1.00		1.00	1.00	3.41	3.68
Incremental Delay, d2	1.1		0.1	0.0	0.0	0.1
Delay (s)	49.9		0.7	0.5	1.9	2.4
Level of Service	D		A	A	A	A
Approach Delay (s)	49.9		0.7			2.4
Approach LOS	D		A			A

Intersection Summary			
HCM Average Control Delay	1.9	HCM Level of Service	A
HCM Volume to Capacity ratio	0.18		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	26.5%	ICU Level of Service	A
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 10: Camelback Road & Marshall Way

2010 BASE AM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗	↘	↖	↗	↘	↖	↗	↘	↖	↗	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0		4.0	4.0	4.0		4.0			4.0	
Lane Util. Factor	1.00	0.95		1.00	0.91	1.00		0.95			0.95	
Frt	1.00	1.00		1.00	1.00	0.85		0.87			0.92	
Fit Protected	0.95	1.00		0.95	1.00	1.00		1.00			0.98	
Satd. Flow (prot)	1770	3539		1770	5085	1583		3067			3177	
Fit Permitted	0.33	1.00		0.47	1.00	1.00		1.00			0.85	
Satd. Flow (perm)	611	3539		867	5085	1583		3067			2746	
Volume (vph)	55	482	0	31	782	73	0	1	8	13	1	18
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	58	507	0	33	823	77	0	1	8	14	1	19
Lane Group Flow (vph)	58	507	0	33	823	77	0	9	0	0	34	0
Turn Type	pm+pt		Perm	pm+pt		Perm	Perm			Perm		
Protected Phases	3	2		3	2			4				4
Permitted Phases	2		2	2		2	4			4		
Actuated Green, G (s)	81.6	78.4		81.6	78.4	78.4		4.4				4.4
Effective Green, g (s)	83.6	80.4		83.6	80.4	80.4		6.4				6.4
Actuated g/C Ratio	0.82	0.79		0.82	0.79	0.79		0.06				0.06
Clearance Time (s)	4.0	6.0		4.0	6.0	6.0		6.0				6.0
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0				3.0
Lane Grp Cap (vph)	537	2790		739	4008	1248		192				172
v/s Ratio Prot	c0.00	0.14		0.00	c0.16			0.00				
v/s Ratio Perm	0.09			0.04		0.05						c0.01
v/c Ratio	0.11	0.18		0.04	0.21	0.06		0.05				0.20
Uniform Delay, d1	1.9	2.7		1.7	2.7	2.4		44.9				45.4
Progression Factor	5.77	3.12		1.00	1.00	1.00		1.00				1.00
Incremental Delay, d2	0.1	0.1		0.0	0.1	0.1		0.1				0.6
Delay (s)	11.1	8.5		1.8	2.8	2.5		45.0				45.9
Level of Service	B	A		A	A	A		D				D
Approach Delay (s)		8.7			2.8			45.0				45.9
Approach LOS		A			A			D				D

Intersection Summary

HCM Average Control Delay	6.2	HCM Level of Service	A
HCM Volume to Capacity ratio	0.20		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	25.9%	ICU Level of Service	A
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 3: Camelback Road & Scottsdale Road

2010 BASE AM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↙	↕	↘	↙	↕	↘	↙	↕	↘	↙	↕	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.97	0.91		0.97	0.95	1.00
Frt	1.00	1.00	0.85	1.00	0.97		1.00	0.99		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3446		3433	5053		3433	3539	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1770	3539	1583	1770	3446		3433	5053		3433	3539	1583
Volume (vph)	96	334	95	50	706	150	154	523	23	145	696	89
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	101	352	100	53	743	158	162	551	24	153	733	94
Lane Group Flow (vph)	101	352	100	53	901	0	162	575	0	153	733	94
Turn Type	Prot		Perm	Prot			Prot			Prot		Perm
Protected Phases	4	7		8	3		6	1		2	5	
Permitted Phases			7									5
Actuated Green, G (s)	7.9	18.0	18.0	17.5	27.6		9.2	37.3		9.2	37.3	37.3
Effective Green, g (s)	7.9	20.0	20.0	17.5	29.6		9.2	39.3		9.2	39.3	39.3
Actuated g/C Ratio	0.08	0.20	0.20	0.17	0.29		0.09	0.39		0.09	0.39	0.39
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0		4.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	137	694	310	304	1000		310	1947		310	1364	610
v/s Ratio Prot	c0.06	0.10		0.03	c0.26		c0.05	0.11		0.04	c0.21	
v/s Ratio Perm			0.06									0.06
v/c Ratio	0.74	0.51	0.32	0.17	0.90		0.52	0.30		0.49	0.54	0.15
Uniform Delay, d1	46.0	36.6	35.2	36.1	34.8		44.3	21.7		44.2	24.3	20.5
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	18.5	0.6	0.6	0.3	11.0		1.6	0.4		1.2	1.5	0.5
Delay (s)	64.6	37.2	35.8	36.4	45.8		45.9	22.1		45.4	25.8	21.0
Level of Service	E	D	D	D	D		D	C		D	C	C
Approach Delay (s)		41.9			45.3			27.4			28.4	
Approach LOS		D			D			C			C	

Intersection Summary			
HCM Average Control Delay	35.5	HCM Level of Service	D
HCM Volume to Capacity ratio	0.68		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	16.0
Intersection Capacity Utilization	69.4%	ICU Level of Service	B
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
6: Camelback Road & Goldwater Blvd

2010 BASE PM PK
9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖↖↖	↗	↖	↖↖↖		↖↖	↖↖	↗	↖↖	↖↖↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91		0.97	0.95	1.00	0.97	0.91	1.00
Fr't	1.00	1.00	0.85	1.00	0.99		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1583	1770	5028		3433	3539	1583	3433	5085	1583
Flt Permitted	0.25	1.00	1.00	0.11	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	474	5085	1583	207	5028		3433	3539	1583	3433	5085	1583
Volume (vph)	350	1294	298	82	703	57	317	641	48	76	1166	204
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	368	1362	314	86	740	60	334	675	51	80	1227	215
Lane Group Flow (vph)	368	1362	314	86	800	0	334	675	51	80	1227	215
Turn Type	pm+pt		Perm	pm+pt			Prot		Perm	Prot		Perm
Protected Phases	6	1		2	5		4	7		8	3	
Permitted Phases	1		1	5				7				3
Actuated Green, G (s)	42.0	34.0	34.0	42.0	34.0		10.7	28.9	28.9	11.1	29.3	29.3
Effective Green, g (s)	44.0	36.0	36.0	44.0	36.0		10.7	30.9	30.9	11.1	31.3	31.3
Actuated g/C Ratio	0.43	0.35	0.35	0.43	0.35		0.10	0.30	0.30	0.11	0.31	0.31
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0	6.0	4.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	306	1795	559	212	1775		360	1072	480	374	1560	486
v/s Ratio Prot	c0.09	0.27		0.03	0.16		c0.10	0.19		0.02	c0.24	
v/s Ratio Perm	c0.42		0.20	0.14					0.03			0.14
v/c Ratio	1.20	0.76	0.56	0.41	0.45		0.93	0.63	0.11	0.21	0.79	0.44
Uniform Delay, d1	32.4	29.2	26.6	36.6	25.4		45.3	30.6	25.6	41.5	32.3	28.4
Progression Factor	1.00	1.00	1.00	1.18	0.94		1.03	1.12	1.96	1.00	1.00	1.00
Incremental Delay, d2	118.1	3.1	4.0	1.2	0.8		28.7	1.1	0.1	0.3	2.7	0.6
Delay (s)	150.6	32.2	30.7	44.6	24.6		75.2	35.6	50.2	41.8	35.0	29.0
Level of Service	F	C	C	D	C		E	D	D	D	C	C
Approach Delay (s)		53.3			26.5			48.8			34.5	
Approach LOS		D			C			D			C	

Intersection Summary			
HCM Average Control Delay	42.9	HCM Level of Service	D
HCM Volume to Capacity ratio	1.02		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	16.0
Intersection Capacity Utilization	82.6%	ICU Level of Service	D
c Critical Lane Group			

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↘	↗	↕	↗	↘	↕
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.91
Fr't	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1583	3539	1583	1770	5085
Flt Permitted	0.95	1.00	1.00	1.00	0.29	1.00
Satd. Flow (perm)	1770	1583	3539	1583	542	5085
Volume (vph)	78	95	898	66	98	1137
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	82	100	945	69	103	1197
Lane Group Flow (vph)	82	100	945	69	103	1197
Turn Type		Perm		Perm	Perm	
Protected Phases	4		2			2
Permitted Phases		4		2	2	
Actuated Green, G (s)	9.8	9.8	81.2	81.2	81.2	81.2
Effective Green, g (s)	10.8	10.8	83.2	83.2	83.2	83.2
Actuated g/C Ratio	0.11	0.11	0.82	0.82	0.82	0.82
Clearance Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	187	168	2887	1291	442	4148
v/s Ratio Prot	0.05		0.27			0.24
v/s Ratio Perm		0.06		0.04	0.19	
v/c Ratio	0.44	0.60	0.33	0.05	0.23	0.29
Uniform Delay, d1	42.8	43.5	2.4	1.8	2.1	2.3
Progression Factor	1.00	1.00	1.00	1.00	4.77	5.11
Incremental Delay, d2	1.6	5.6	0.3	0.1	0.9	0.1
Delay (s)	44.4	49.1	2.7	1.9	11.1	11.7
Level of Service	D	D	A	A	B	B
Approach Delay (s)	47.0		2.6			11.7
Approach LOS	D		A			B

Intersection Summary			
HCM Average Control Delay	10.6	HCM Level of Service	B
HCM Volume to Capacity ratio	0.36		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	46.4%	ICU Level of Service	A
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 10: Camelback Road & Marshall Way

2010 BASE PM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↵	↕	↗	↵	↕	↗		↕			↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0		4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	1.00		0.95			0.95	
Frt	1.00	1.00	0.85	1.00	1.00	0.85		0.87			0.93	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00		1.00			0.98	
Satd. Flow (prot)	1770	3539	1583	1770	5085	1583		3066			3217	
Flt Permitted	0.36	1.00	1.00	0.23	1.00	1.00		0.83			0.76	
Satd. Flow (perm)	661	3539	1583	420	5085	1583		2554			2497	
Volume (vph)	134	1056	26	58	703	161	11	8	126	125	15	121
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	141	1112	27	61	740	169	12	8	133	132	16	127
Lane Group Flow (vph)	141	1112	27	61	740	169	0	153	0	0	275	0
Turn Type	pm+pt		Perm	pm+pt		Perm	Perm				Perm	
Protected Phases	3	2		3	2			4				4
Permitted Phases	2		2	2		2	4			4		
Actuated Green, G (s)	76.9	72.9	72.9	76.9	72.9	72.9		9.1			9.1	
Effective Green, g (s)	78.9	74.9	74.9	78.9	74.9	74.9		11.1			11.1	
Actuated g/C Ratio	0.77	0.73	0.73	0.77	0.73	0.73		0.11			0.11	
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0	6.0		6.0			6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	555	2599	1162	378	3734	1162		278			272	
v/s Ratio Prot	c0.01	c0.31		0.01	0.15							
v/s Ratio Perm	0.19		0.02	0.12		0.11		0.06			c0.11	
v/c Ratio	0.25	0.43	0.02	0.16	0.20	0.15		0.55			1.01	
Uniform Delay, d1	3.5	5.2	3.7	5.8	4.2	4.0		43.1			45.4	
Progression Factor	2.34	1.99	0.78	1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	0.2	0.4	0.0	0.2	0.1	0.3		2.4			57.4	
Delay (s)	8.5	10.8	2.9	6.0	4.3	4.3		45.4			102.9	
Level of Service	A	B	A	A	A	A		D			F	
Approach Delay (s)		10.4			4.4			45.4			102.9	
Approach LOS		B			A			D			F	

Intersection Summary			
HCM Average Control Delay	19.7	HCM Level of Service	B
HCM Volume to Capacity ratio	0.49		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	60.7%	ICU Level of Service	B
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 3: Camelback Road & Scottsdale Road

2010 BASE PM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↕	↗	↘	↕	↗	↘	↕	↗	↘	↕	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.97	0.91		0.97	0.95	1.00
Frt	1.00	1.00	0.85	1.00	0.96		1.00	0.99		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3411		3433	5025		3433	3539	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1770	3539	1583	1770	3411		3433	5025		3433	3539	1583
Volume (vph)	284	914	231	67	527	168	331	1266	109	333	809	197
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	299	962	243	71	555	177	348	1333	115	351	852	207
Lane Group Flow (vph)	299	962	243	71	732	0	348	1448	0	351	852	207
Turn Type	Prot		Perm	Prot			Prot			Prot		Perm
Protected Phases	4	7		8	3		6	1		2	5	
Permitted Phases			7									5
Actuated Green, G (s)	16.9	34.1	34.1	6.9	24.1		12.8	28.2		12.8	28.2	28.2
Effective Green, g (s)	16.9	36.1	36.1	6.9	26.1		12.8	30.2		12.8	30.2	30.2
Actuated g/C Ratio	0.17	0.35	0.35	0.07	0.26		0.13	0.30		0.13	0.30	0.30
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0		4.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	293	1253	560	120	873		431	1488		431	1048	469
v/s Ratio Prot	c0.17	0.27		0.04	c0.21		0.10	c0.29		c0.10	0.24	
v/s Ratio Perm			0.15									0.13
v/c Ratio	1.02	0.77	0.43	0.59	0.84		0.81	0.97		0.81	0.81	0.44
Uniform Delay, d1	42.6	29.2	25.2	46.2	36.0		43.4	35.5		43.4	33.3	29.1
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	57.9	2.9	0.5	7.6	7.1		10.6	17.7		11.2	6.9	3.0
Delay (s)	100.5	32.1	25.7	53.8	43.1		54.0	53.2		54.7	40.2	32.1
Level of Service	F	C	C	D	D		D	D		D	D	C
Approach Delay (s)		44.7			44.0			53.4			42.6	
Approach LOS		D			D			D			D	

Intersection Summary		
HCM Average Control Delay	46.9	HCM Level of Service D
HCM Volume to Capacity ratio	0.92	
Actuated Cycle Length (s)	102.0	Sum of lost time (s) 16.0
Intersection Capacity Utilization	89.2%	ICU Level of Service D
c Critical Lane Group		

HCM Signalized Intersection Capacity Analysis
6: Camelback Road & Goldwater Blvd

2010 WITH SITE AM PK
9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖↖↖	↗	↖	↖↖↖		↖↖	↖↖	↗	↖↖	↖↖↖	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91		0.97	0.95	1.00	0.97	0.91	1.00
Frt	1.00	1.00	0.85	1.00	0.99		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1583	1770	5055		3433	3539	1583	3433	5085	1583
Flt Permitted	0.28	1.00	1.00	0.33	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	523	5085	1583	609	5055		3433	3539	1583	3433	5085	1583
Volume (vph)	167	652	237	64	718	29	206	280	33	17	659	427
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	176	686	249	67	756	31	217	295	35	18	694	449
Lane Group Flow (vph)	176	686	249	67	787	0	217	295	35	18	694	449
Turn Type	pm+pt		Perm	pm+pt		Prot		Perm	Prot		Perm	
Protected Phases	6	1		2	5	4		7	8		3	
Permitted Phases	1		1	5				7				3
Actuated Green, G (s)	46.4	39.8	39.8	46.4	39.8	12.0	25.5	25.5	10.1	23.6	23.6	
Effective Green, g (s)	48.4	41.8	41.8	48.4	41.8	12.0	27.5	27.5	10.1	25.6	25.6	
Actuated g/C Ratio	0.47	0.41	0.41	0.47	0.41	0.12	0.27	0.27	0.10	0.25	0.25	
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0	4.0	6.0	6.0	4.0	6.0	6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	329	2084	649	364	2072		404	954	427	340	1276	397
v/s Ratio Prot	c0.03	0.13		0.01	0.16		c0.06	0.08		0.01	0.14	
v/s Ratio Perm	c0.22		0.16	0.08					0.02			0.28
v/c Ratio	0.53	0.33	0.38	0.18	0.38		0.54	0.31	0.08	0.05	0.54	1.13
Uniform Delay, d1	25.3	20.5	21.1	19.1	21.0		42.4	29.7	27.8	41.6	33.1	38.2
Progression Factor	1.00	1.00	1.00	1.02	0.71		1.20	1.03	1.14	1.00	1.00	1.00
Incremental Delay, d2	1.7	0.4	1.7	0.2	0.5		1.4	0.2	0.1	0.1	0.5	85.8
Delay (s)	26.9	21.0	22.8	19.6	15.5		52.2	30.7	31.7	41.7	33.6	124.0
Level of Service	C	C	C	B	B		D	C	C	D	C	F
Approach Delay (s)		22.3			15.8			39.3			68.7	
Approach LOS		C			B			D			E	

Intersection Summary			
HCM Average Control Delay	38.0	HCM Level of Service	D
HCM Volume to Capacity ratio	0.68		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	59.3%	ICU Level of Service	A
c Critical Lane Group			



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↵	↗	↕	↗	↵	↗↗↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.91
Frt	1.00	0.85	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1583	3539	1583	1770	5085
Flt Permitted	0.95	1.00	1.00	1.00	0.44	1.00
Satd. Flow (perm)	1770	1583	3539	1583	827	5085
Volume (vph)	33	32	528	49	47	814
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	35	34	556	52	49	857
Lane Group Flow (vph)	35	34	556	52	49	857
Turn Type		Perm		Perm	Perm	
Protected Phases	4		2			2
Permitted Phases		4		2	2	
Actuated Green, G (s)	6.4	6.4	84.6	84.6	84.6	84.6
Effective Green, g (s)	7.4	7.4	86.6	86.6	86.6	86.6
Actuated g/C Ratio	0.07	0.07	0.85	0.85	0.85	0.85
Clearance Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	128	115	3005	1344	702	4317
v/s Ratio Prot	0.02		0.16			c0.17
v/s Ratio Perm		0.02		0.03	0.06	
v/c Ratio	0.27	0.30	0.19	0.04	0.07	0.20
Uniform Delay, d1	44.8	44.8	1.4	1.2	1.2	1.4
Progression Factor	1.00	1.00	1.00	1.00	2.34	2.14
Incremental Delay, d2	1.2	1.4	0.1	0.1	0.2	0.1
Delay (s)	45.9	46.3	1.5	1.3	3.1	3.1
Level of Service	D	D	A	A	A	A
Approach Delay (s)	46.1		1.5			3.1
Approach LOS	D		A			A

Intersection Summary			
HCM Average Control Delay	4.4	HCM Level of Service	A
HCM Volume to Capacity ratio	0.21		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	26.6%	ICU Level of Service	A
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 10: Camelback Road & Marshall Way

2010 WITH SITE AM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↵	↕	↗	↵	↕	↗		↕			↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0		4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	1.00		0.95			0.95	
Flt	1.00	1.00	0.85	1.00	1.00	0.85		0.92			0.92	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00		0.98			0.98	
Satd. Flow (prot)	1770	3539	1583	1770	5085	1583		3190			3177	
Flt Permitted	0.32	1.00	1.00	0.44	1.00	1.00		0.82			0.81	
Satd. Flow (perm)	595	3539	1583	826	5085	1583		2676			2624	
Volume (vph)	56	521	53	128	799	74	45	1	48	13	1	18
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	59	548	56	135	841	78	47	1	51	14	1	19
Lane Group Flow (vph)	59	548	56	135	841	78	0	99	0	0	34	0
Turn Type	pm+pt		Perm	pm+pt		Perm	Perm			Perm		
Protected Phases	3	2		3	2			4			4	
Permitted Phases	2		2	2		2	4			4		
Actuated Green, G (s)	79.7	75.7	75.7	79.7	75.7	75.7		6.3			6.3	
Effective Green, g (s)	81.7	77.7	77.7	81.7	77.7	77.7		8.3			8.3	
Actuated g/C Ratio	0.80	0.76	0.76	0.80	0.76	0.76		0.08			0.08	
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0	6.0		6.0			6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	523	2696	1206	699	3874	1206		218			214	
v/s Ratio Prot	0.00	0.15		c0.01	c0.17							
v/s Ratio Perm	0.09		0.04	0.15		0.05		c0.04			0.01	
v/c Ratio	0.11	0.20	0.05	0.19	0.22	0.06		0.45			0.16	
Uniform Delay, d1	2.5	3.4	3.0	2.6	3.5	3.0		44.7			43.6	
Progression Factor	0.33	0.46	0.23	1.62	0.78	1.47		1.00			1.00	
Incremental Delay, d2	0.1	0.2	0.1	0.1	0.1	0.1		1.5			0.3	
Delay (s)	0.9	1.7	0.8	4.2	2.8	4.5		46.2			44.0	
Level of Service	A	A	A	A	A	A		D			D	
Approach Delay (s)		1.6			3.1			46.2			44.0	
Approach LOS		A			A			D			D	

Intersection Summary

HCM Average Control Delay	5.6	HCM Level of Service	A
HCM Volume to Capacity ratio	0.24		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	36.0%	ICU Level of Service	A
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 3: Camelback Road & Scottsdale Road

2010 WITH SITE AM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗	↘	↖	↗	↘	↖	↗	↘	↖	↗	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.97	0.91		0.97	0.95	1.00
Frt	1.00	1.00	0.85	1.00	0.98		1.00	0.99		1.00	1.00	0.85
Fit Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3451		3433	5053		3433	3539	1583
Fit Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	1770	3539	1583	1770	3451		3433	5053		3433	3539	1583
Volume (vph)	106	363	65	50	753	150	192	523	23	145	696	101
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	112	382	68	53	793	158	202	551	24	153	733	106
Lane Group Flow (vph)	112	382	68	53	951	0	202	575	0	153	733	106
Turn Type	Prot		Perm	Prot			Prot			Prot		Perm
Protected Phases	4	7		8	3		6	1		2	5	
Permitted Phases			7									5
Actuated Green, G (s)	10.2	21.0	21.0	13.2	24.0		9.6	38.2		9.6	38.2	38.2
Effective Green, g (s)	10.2	23.0	23.0	13.2	26.0		9.6	40.2		9.6	40.2	40.2
Actuated g/C Ratio	0.10	0.23	0.23	0.13	0.25		0.09	0.39		0.09	0.39	0.39
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0		4.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	177	798	357	229	880		323	1991		323	1395	624
v/s Ratio Prot	c0.06	0.11		c0.03	c0.28		c0.06	0.11		0.04	c0.21	
v/s Ratio Perm			0.04									0.07
v/c Ratio	0.63	0.48	0.19	0.23	1.08		0.63	0.29		0.47	0.53	0.17
Uniform Delay, d1	44.1	34.3	32.0	39.8	38.0		44.5	21.1		43.8	23.6	20.1
Progression Factor	1.42	1.43	3.27	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	7.1	0.5	0.3	0.5	54.5		3.7	0.4		1.1	1.4	0.6
Delay (s)	69.9	49.5	104.7	40.4	92.5		48.2	21.5		44.9	25.0	20.7
Level of Service	E	D	F	D	F		D	C		D	C	C
Approach Delay (s)		60.2			89.8			28.4			27.6	
Approach LOS		E			F			C			C	

Intersection Summary

HCM Average Control Delay	52.0	HCM Level of Service	D
HCM Volume to Capacity ratio	0.73		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	20.0
Intersection Capacity Utilization	72.5%	ICU Level of Service	C
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
6: Camelback Road & Goldwater Blvd

2010 WITH SITE PM PK
9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗	↘	↖	↗	↘	↖	↗	↘	↖	↗	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91		0.97	0.95	1.00	0.97	0.91	1.00
Frt	1.00	1.00	0.85	1.00	0.98		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	5085	1583	1770	5006		3433	3539	1583	3433	5085	1583
Flt Permitted	0.15	1.00	1.00	0.17	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	277	5085	1583	325	5006		3433	3539	1583	3433	5085	1583
Volume (vph)	350	1395	326	95	791	92	389	664	63	104	1185	204
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	368	1468	343	100	833	97	409	699	66	109	1247	215
Lane Group Flow (vph)	368	1468	343	100	930	0	409	699	66	109	1247	215
Turn Type	pm+pt		Perm	pm+pt			Prot		Perm	Prot		pm+ov
Protected Phases	6	1		2	5		4	7		8	3	6
Permitted Phases	1		1	5					7			3
Actuated Green, G (s)	48.4	35.9	35.9	27.4	20.9		14.6	24.9	24.9	14.7	25.0	46.5
Effective Green, g (s)	48.4	37.9	37.9	29.4	22.9		14.6	26.9	26.9	14.7	27.0	48.5
Actuated g/C Ratio	0.47	0.37	0.37	0.29	0.22		0.14	0.26	0.26	0.14	0.26	0.48
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0	6.0	4.0	6.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	446	1889	588	186	1124		491	933	417	495	1346	815
v/s Ratio Prot	c0.17	0.29		0.03	0.19		c0.12	0.20		0.03	c0.25	0.06
v/s Ratio Perm	c0.22		0.22	0.12					0.04			0.08
v/c Ratio	0.83	0.78	0.58	0.54	0.83		0.83	0.75	0.16	0.22	0.93	0.26
Uniform Delay, d1	31.6	28.3	25.7	47.4	37.7		42.5	34.5	28.9	38.6	36.5	16.0
Progression Factor	1.00	1.00	1.00	1.38	0.90		0.92	1.30	2.31	1.00	1.00	1.00
Incremental Delay, d2	11.8	3.2	4.2	2.9	6.9		11.1	3.2	0.2	0.2	11.0	0.2
Delay (s)	43.4	31.5	29.9	68.1	40.8		50.3	48.1	66.8	38.8	47.6	16.2
Level of Service	D	C	C	E	D		D	D	E	D	D	B
Approach Delay (s)		33.3			43.5			49.9			42.7	
Approach LOS		C			D			D			D	

Intersection Summary		
HCM Average Control Delay	40.8	HCM Level of Service
HCM Volume to Capacity ratio	0.84	
Actuated Cycle Length (s)	102.0	Sum of lost time (s)
Intersection Capacity Utilization	87.8%	ICU Level of Service
c Critical Lane Group		

HCM Signalized Intersection Capacity Analysis
 8: Montecito Ave & Goldwater Blvd

2010 WITH SITE PM PK
 9/4/2003

Movement	↙	↘	↑	↗	↖	↓
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↙	↘	↑↑	↗	↖	↑↑↑
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.91
Frt	1.00	0.85	1.00	0.85	1.00	1.00
Fit Protected	0.95	1.00	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1583	3539	1583	1770	5085
Fit Permitted	0.95	1.00	1.00	1.00	0.28	1.00
Satd. Flow (perm)	1770	1583	3539	1583	525	5085
Volume (vph)	136	193	910	116	147	1150
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	143	203	958	122	155	1211
Lane Group Flow (vph)	143	203	958	122	155	1211
Turn Type		Perm		Perm	Perm	
Protected Phases	4		2			2
Permitted Phases		4		2	2	
Actuated Green, G (s)	12.9	12.9	78.1	78.1	78.1	78.1
Effective Green, g (s)	13.9	13.9	80.1	80.1	80.1	80.1
Actuated g/C Ratio	0.14	0.14	0.79	0.79	0.79	0.79
Clearance Time (s)	5.0	5.0	6.0	6.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	241	216	2779	1243	412	3993
v/s Ratio Prot	0.08		0.27			0.24
v/s Ratio Perm		0.13		0.08	0.30	
v/c Ratio	0.59	0.94	0.34	0.10	0.38	0.30
Uniform Delay, d1	41.4	43.6	3.2	2.5	3.3	3.1
Progression Factor	1.00	1.00	1.00	1.00	2.88	2.40
Incremental Delay, d2	3.9	44.0	0.3	0.2	1.5	0.1
Delay (s)	45.3	87.7	3.6	2.7	11.1	7.5
Level of Service	D	F	A	A	B	A
Approach Delay (s)	70.1		3.5			7.9
Approach LOS	E		A			A

Intersection Summary

HCM Average Control Delay	13.9	HCM Level of Service	B
HCM Volume to Capacity ratio	0.46		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	53.0%	ICU Level of Service	A
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 10: Camelback Road & Marshall Way

2010 WITH SITE PM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↵	↕↕	↗	↵	↕↕↕	↗		↕↕			↕↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0		4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.91	1.00		0.95			0.95	
Fr't	1.00	1.00	0.85	1.00	1.00	0.85		0.91			0.93	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00		0.98			0.98	
Satd. Flow (prot)	1770	3539	1583	1770	5085	1583		3157			3217	
Flt Permitted	0.34	1.00	1.00	0.18	1.00	1.00		0.68			0.60	
Satd. Flow (perm)	638	3539	1583	338	5085	1583		2178			1973	
Volume (vph)	134	1110	120	222	705	161	145	8	239	125	15	121
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	141	1168	126	234	742	169	153	8	252	132	16	127
Lane Group Flow (vph)	141	1168	126	234	742	169	0	413	0	0	275	0
Turn Type	pm+pt		Perm	pm+pt		Perm	Perm			Perm		
Protected Phases	3	2		3	2			4			4	
Permitted Phases	2		2	2		2	4			4		
Actuated Green, G (s)	71.1	61.6	61.6	71.1	61.6	61.6		14.9			14.9	
Effective Green, g (s)	73.1	63.6	63.6	73.1	63.6	63.6		16.9			16.9	
Actuated g/C Ratio	0.72	0.62	0.62	0.72	0.62	0.62		0.17			0.17	
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0	6.0		6.0			6.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	563	2207	987	376	3171	987		361			327	
v/s Ratio Prot	0.02	0.33		c0.06	0.15							
v/s Ratio Perm	0.16		0.08	c0.39		0.11		c0.19			0.14	
v/c Ratio	0.25	0.53	0.13	0.62	0.23	0.17		1.14			0.84	
Uniform Delay, d1	6.3	10.8	7.9	20.6	8.5	8.1		42.6			41.2	
Progression Factor	0.05	0.08	0.00	1.23	0.40	0.51		1.00			1.00	
Incremental Delay, d2	0.2	0.6	0.2	2.3	0.1	0.3		92.6			17.4	
Delay (s)	0.5	1.5	0.2	27.6	3.5	4.4		135.1			58.7	
Level of Service	A	A	A	C	A	A		F			E	
Approach Delay (s)		1.3			8.6			135.1			58.7	
Approach LOS		A			A			F			E	

Intersection Summary

HCM Average Control Delay	25.6	HCM Level of Service	C
HCM Volume to Capacity ratio	0.72		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	79.7%	ICU Level of Service	C
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 3: Camelback Road & Scottsdale Road

2010 WITH SITE PM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗	↘	↖	↗		↖	↗		↖	↗	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95		0.97	0.91		0.97	0.95	1.00
Flt Protected	1.00	1.00	0.85	1.00	0.97		1.00	0.99		1.00	1.00	0.85
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	1770	3539	1583	1770	3423		3433	5025		3433	3539	1583
Satd. Flow (perm)	1770	3539	1583	1770	3423		3433	5025		3433	3539	1583
Volume (vph)	309	1007	154	67	602	168	403	1266	109	333	809	109
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	325	1060	162	71	634	177	424	1333	115	351	852	115
Lane Group Flow (vph)	325	1060	162	71	811	0	424	1448	0	351	852	115
Turn Type	Prot		Perm	Prot			Prot			Prot		Perm
Protected Phases	4	7		8	3		6	1		2	5	
Permitted Phases			7									5
Actuated Green, G (s)	19.8	34.8	34.8	8.0	23.0		16.0	28.2		11.0	23.2	23.2
Effective Green, g (s)	19.8	36.8	36.8	8.0	25.0		16.0	30.2		11.0	25.2	25.2
Actuated g/C Ratio	0.19	0.36	0.36	0.08	0.25		0.16	0.30		0.11	0.25	0.25
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0		4.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	344	1277	571	139	839		539	1488		370	874	391
v/s Ratio Prot	c0.18	0.30		0.04	c0.24		c0.12	c0.29		0.10	0.24	
v/s Ratio Perm			0.10									0.07
v/c Ratio	0.94	0.83	0.28	0.51	0.97		0.79	0.97		0.95	0.97	0.29
Uniform Delay, d1	40.6	29.7	23.2	45.1	38.1		41.4	35.5		45.2	38.1	31.2
Progression Factor	1.42	1.34	1.67	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	31.7	4.2	0.2	3.1	23.0		7.4	17.7		33.3	24.9	1.9
Delay (s)	89.3	44.0	38.9	48.3	61.1		48.8	53.2		78.5	63.0	33.1
Level of Service	F	D	D	D	E		D	D		E	E	C
Approach Delay (s)		53.0			60.0			52.2			64.5	
Approach LOS		D			E			D			E	

Intersection Summary			
HCM Average Control Delay	56.6	HCM Level of Service	E
HCM Volume to Capacity ratio	0.92		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	12.0
Intersection Capacity Utilization	92.8%	ICU Level of Service	E
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis
 3: Camelback Road & Scottsdale Road

2010 WITH SITE PM PK
 9/4/2003

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔↔	↕↕	↗	↖	↕↕		↔↔	↕↕↕		↔↔	↕↕	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	4.0
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95		0.97	0.91		0.97	0.95	1.00
Fr't	1.00	1.00	0.85	1.00	0.97		1.00	0.99		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	3433	3539	1583	1770	3423		3433	5025		3433	3539	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	3433	3539	1583	1770	3423		3433	5025		3433	3539	1583
Volume (vph)	309	1007	154	67	602	168	403	1266	109	333	809	109
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	325	1060	162	71	634	177	424	1333	115	351	852	115
Lane Group Flow (vph)	325	1060	162	71	811	0	424	1448	0	351	852	115
Turn Type	Prot		Perm	Prot			Prot			Prot		Perm
Protected Phases	4	7		8	3		6	1		2	5	
Permitted Phases			7									5
Actuated Green, G (s)	12.7	33.2	33.2	4.8	25.3		17.8	31.2		12.8	26.2	26.2
Effective Green, g (s)	12.7	35.2	35.2	4.8	27.3		17.8	33.2		12.8	28.2	28.2
Actuated g/C Ratio	0.12	0.35	0.35	0.05	0.27		0.17	0.33		0.13	0.28	0.28
Clearance Time (s)	4.0	6.0	6.0	4.0	6.0		4.0	6.0		4.0	6.0	6.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	427	1221	546	83	916		599	1636		431	978	438
v/s Ratio Prot	c0.09	c0.30		0.04	0.24		c0.12	c0.29		0.10	0.24	
v/s Ratio Perm			0.10									0.07
v/c Ratio	0.76	0.87	0.30	0.86	0.89		0.71	0.89		0.81	0.87	0.26
Uniform Delay, d1	43.2	31.2	24.4	48.3	35.8		39.7	32.6		43.4	35.2	28.8
Progression Factor	1.38	1.37	1.79	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	7.0	6.1	0.3	53.5	10.2		3.8	7.4		11.2	10.5	1.5
Delay (s)	66.6	48.9	43.8	101.8	46.1		43.5	40.0		54.7	45.7	30.2
Level of Service	E	D	D	F	D		D	D		D	D	C
Approach Delay (s)		52.1			50.6			40.8			46.7	
Approach LOS		D			D			D			D	

Intersection Summary

HCM Average Control Delay	46.8	HCM Level of Service	D
HCM Volume to Capacity ratio	0.81		
Actuated Cycle Length (s)	102.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	84.8%	ICU Level of Service	D
c Critical Lane Group			