

**CONCEPTUAL  
MASTER DRAINAGE REPORT  
FOR  
SERENO CANYON**

May 12, 2006  
WP# 042054

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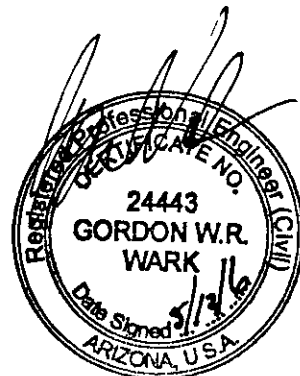


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## 1.0 INTRODUCTION

### 1.1 Project Description

Sereno Canyon is a planned 330-acre custom lot residential subdivision located in the northeastern portion of Scottsdale, Arizona. The site is located in Section 11, Township 4 North, Range 5 East of the Gila and Salt River Base and Meridian. The site is currently an assemblage of undeveloped parcels bound to the west by the existing Sonoran Crest Development (122<sup>nd</sup> Street alignment), to the east by the 128<sup>th</sup> Street alignment, to the north by the Happy Valley Road alignment, and to the south by the McDowell Mountain Sonoran Preserve. Access to the development is planned from the west via the ½-mile section roadway, Alameda Road. Plate 1 provides a vicinity map for the project and surrounding areas.

Sereno Canyon is a proposed custom lot sub-division nestled at the northern base of the McDowell Mountains. Development of the project is planned to occur in four (4) phases. Please refer to Plate 1A for a phasing map of the project. The development includes 122 lots ranging in size from 2 to 3 acres and a clubhouse with amenities such as jacuzzis, pools, water falls, and restaurant facilities. Interpretive trails and scattered pocket parks with water features will also be incorporated into the site plan.

### 1.2 Type of Report

This report is being prepared as a Conceptual Master Drainage Plan for the proposed Sereno Canyon Project.

### 1.3 Purpose

This Conceptual Master Drainage Report for the proposed Sereno Canyon project has been prepared to meet the master drainage plan requirements outlined in Chapter 4 of the *City of Scottsdale Design Standards and Policies Manual (DS&PM)*. This report presents description of the hydrological and hydraulic modeling of the proposed drainage systems.

The main purpose of this report is to illustrate the following

- Compliance with the City of Scottsdale's Drainage Ordinance storm water storage requirements by providing retention for 100-year, 2-hour storm event runoff for improved areas computed by Rational Method
- Reduction of post-development flows to at or below the pre-development flows for 2-year, 10-year and 100-year, 6-hour storm events Hence, the downstream properties are provided with similar or better flood protection as the pre-development condition
- Maintenance of the major water courses traversing through the site in their natural location
- Delineation of the 100-year water surface extents along with determination of the 100-year water surface elevations for the major drainage courses using HEC RAS.

## 2.0 EXISTING ON-SITE DRAINAGE CONDITIONS AND CHARACTERISTICS

### 2.1 On-site Drainage

The proposed project lies in the northern planning section of the City of Scottsdale. The Elevations range from 2,830 feet in the south to 2,675 feet in the northeast. Based on the existing topography of the site, a ridgeline runs through the property from the southern property boundary (approximately at the center) in the northwest direction.

Vegetation is typical Sonoran Desert type with creosote bush, jumping cholla, saguaro cacti, palo verde, ironwood, and mesquite trees. The project site lies within the areas identified as Environmentally Sensitive Lands (ESL) by the City of Scottsdale. The site is further classified as 'Upper Desert' within the ESL areas as shown in Plate 2 – *ESL Classification*.

Plate 3 presents the soils classification (type D) based on the Soil Conservation Service (SCS) Map for the project area. SCS is now referred to as Natural Resource Conservation Service (NRCS). The site photos for the project site are included in Appendix A. The cover type and hydrologic condition were estimated as desert shrub areas with poor hydrologic conditions. From Figure 4-6 of the *City of Scottsdale DS&PM*, a runoff curve number of 88 was used for the corresponding soils group D that occurs within the watershed area.

Currently, the site contains no development and hence an effective impervious area of zero percent was used. The soil characteristics of the site are included in the model in terms of the curve number discussed above. Also, minor rock outcroppings on the site are not considered a factor in computing the percent effective impervious areas as they do not interfere with the drainage pattern and are not hydraulically connected to the sub basin concentration points. This conclusion is made based on the photographs of the site and washes included in Appendix A. Please note that the site is not mass graded and the outcroppings will be the same in both the existing and proposed conditions and does not impact the pre- and post-development runoff comparison.



There are no washes on the site categorized as a 'Vista Corridor' as the calculated 100-year, 6-hour flows are less than 750 cfs for all washes. The major watercourses traversing the project with a 100-year, 6-hour flow greater than 50 cfs are identified as the site lies within ESL areas. These washes are identified on Plate 9 – *Pre-Development Grading and Drainage Plan*. These washes will be maintained in their natural location and will not be re-aligned.

## 2.2 Existing On-site Drainage Network

Existing on-site drainage sub-basin boundaries were identified using aerial mapped 1-foot contours. Please refer to Plate 5 – *Color Topographic Aerial Photograph*. Please refer to Plate 8 – *Pre-Development Drainage Site Plan* for the on-site sub-basins and concentration points.

Runoff generated on-site drains northeasterly or northwesterly from the ridgeline. Washes C1, D1, E1, E2, and F1, with a 100-year, 6-hour flow greater than 50 cfs, traverse the site in the northeast direction and exit at the property boundary. The other minor washes (100-year, 6-hour flow less than 50 cfs) traverse either northeasterly or northwesterly until they exit at the property boundary.

A delineation of the 100-year, 6-hour water surface ~~entents~~ for washes with 100-year, 6-hour peak flow of 50 cfs or greater has been drawn based on the HEC-RAS analysis. Water surface elevations and top widths for pre-development flows at each cross section are included on Plate 9 – *Pre-Development Grading and Drainage Plan*. The hydrologic and hydraulic analysis procedures are discussed in Section 5.0. The boundary conditions water surface elevations were determined using normal depth for the given channel slope in HEC RAS.

## 2.3 Off-site Watershed

The off-site areas impacting the drainage pattern of the proposed development lie in the northern planning section of the City of Scottsdale. The off-site watershed includes a portion of the 40-acre Sonoran Sky development, an additional 20-acre parcel and mountainous terrain located within the McDowell Sonoran Preserve. Currently, the entire off-site watershed is undeveloped desert land. The aforementioned two properties may be developed in the future. The off-site area impacting the drainage patterns for the proposed

developments are identified on Plate 6 – *Off-Site Watershed Area Map*. Vegetation is typical Sonoran Desert type with creosote bush, jumping cholla, saguaro cacti, palo verde, ironwood, and mesquite trees. The off-site areas lie within the ‘Upper Desert’ and ‘Hillside’ (ESL) landform areas.

There are no off-site washes categorized as a ‘Vista Corridor’ that impact the site, as the calculated 100-year, 6-hour flow is less than 750 cfs. The major watercourses traversing the project with a 100-year, 6-hour flow greater than 50 cfs are identified. These washes will be maintained in their natural location and will not be re-aligned.

Based on the SCS Map, the cover type and hydrologic condition were estimated as desert shrub areas with poor hydrologic conditions. From Figure 4-6 of the *City of Scottsdale’s DS&PM*, a runoff curve number of 88 was used for the corresponding soils group D that occurs within the off-site watershed area.

#### **2.4 Existing Off-site Drainage Network**

City of Scottsdale Quarter Section Maps with 2-foot contour interval topography were used to identify the off-site drainage sub-basins impacting the proposed development. Please refer to Plate 6 – *Off-Site Watershed Area Map* for the limits of the drainage sub-basins and concentration points.

Off-site flows enter the site along the southern property boundary along their natural course as overland flows and concentrated flows. The offsite flows impacting the project traverse the site through washes A1, A2, B1, B2, C, H1, H2, I, J and R. Washes A1, A2, B1, B2, C and R traverse the site in the northeast direction and exit at the east property boundary. Washes H1, H2, I and J traverse the site in the northwest direction until they exit at the west property boundary.

#### **2.5 Existing Drainage Relative to Adjacent Projects**

Existing washes exit the site as concentrated flows at various locations on the eastern, western and northern property boundaries. These locations will be maintained with peak flows for the 2-year, 10-year and 100-year, 6-hour storm events at or below pre-developed amounts.

## 2.6 Flood Insurance Rate Map (FIRM)

The Flood Insurance Rate Maps (FIRM) for Maricopa County, Arizona and incorporated areas, Panel Numbers 04013C1255G and 04013C1260F, effective date September 30, 2005 indicates the site is within Zone "X" (shaded), and Zone "D"

Zone "X" (shaded) is defined by FEMA as follows

*Areas of 500-year flood, areas of 100-year flood with average depths of less than 1 foot or with drainage areas less than 1 square mile, and areas protected by levees from 100-year flood*

Zone "D" is defined by FEMA as follows

*Areas in which flood hazards are undetermined*

The location of the parcel relative to the FIRM panels is illustrated on Plate 4 – *Flood Insurance Rate Map (FIRM)* using the FEMA Firmettes

## 3.0 PROPOSED MASTER DRAINAGE PLAN

### 3.1 General Description of Proposed Drainage System

The proposed drainage system will allow existing drainage patterns to be maintained in their natural location and condition where possible. The site is being developed as large custom lots with a minimum lot size of approximately 2 acres. In the event of any future development within the off-site watershed areas, it will be the responsibility of the future developer to attenuate the post-development flows to at or below the pre-development flows.

The offsite runoff impacting the site will traverse the site through natural washes A1, A2, B1, B2, C, H1, H2, I, J and R. Washes A1, A2, B1, B2, C and R traverse through the site in the northeast direction and exit at the east property boundary. Washes H1, H2, I and J traverse the site in the northwest direction until they exit at the west property boundary.

The on-site drainage system will be composed primarily of the existing on-site washes conveying the runoff through the site and exiting at the property boundary at their historic locations. According to the City's Drainage Ordinance, stormwater storage is to be

provided for the 100-year 2-hour storm event for improved areas Rational Method was used to estimate the required stormwater storage volumes The procedure is discussed in detail in Section 5 2 4 Retention/detention basins with bleed-off pipes are proposed at strategic locations to provide stormwater storage for the runoff generated during the 100-year, 2-hour storm event Several existing on-site washes are being utilized as retention basin locations due to the steep slopes encountered within the site Rock walls will be used to mitigate the need for large constructed slopes to attain ponding areas

According to the City's Drainage Ordinance, all runoff generated from the developed portion of the site must be managed and the peak discharge from the site reduced to at least pre-development values With the development being sparse in nature at approximately 0 37 dwelling units per acre and no mass grading being proposed, post-development flows are only slightly higher than pre-development flows The proposed on-line retention basins along several existing on-site washes will also act to attenuate the post-development peak discharges to at or below the pre-development values Please note that the proposed storage basins act as retention basins up to a height of 3 feet and as detention basins above 3 feet A detailed description on the design and analysis of these basins is provided in Section 3 5 1 Also included is a discussion on how retention and detention is achieved through these storage basins

Roadway runoff will be conveyed along the street up to the capacity of the street within the required stipulations outlined in Figure 4-2 – *Hydraulic Design Criteria* of the *City of Scottsdale DS&PM* Please refer to Appendix A for a copy of this figure The additional runoff exceeding the street capacity will be released through a curb opening or storm drain system Curb openings will occur where there is a wash adjacent to the roadway or at culvert crossings

### 3.2 Future Conditions

Currently, the offsite areas impacting the site are undeveloped desert lands The areas proposed to be developed in the future are the 40-acre Sonoran Sky development and the 20-acre parcel at the southern property boundary Currently no approved drainage reports exist for these two developments It will be the responsibility of these developments to maintain their post-development flow peaks and volume at or below the pre-development flow peaks and volume when they develop The proposed drainage systems are designed

assuming that there will be no increase in peak flows during the 100-year, 6-hour event upstream of the project site

The drainage plan for Sereno Canyon proposes to maintain the 2-year, 10-year and 100-year, 6-hour storm event post-development peak discharges at or below the pre-development flows. Also, the drainage courses will be maintained at their historic locations at the property boundary. Thus, no adverse impact is anticipated on the adjacent properties downstream of the project site for the 2-year, 10-year and 100-year, 6-hour storm events.

### 3.3 Stormwater Storage Requirements

Based on the City of Scottsdale's Drainage Ordinance storm water storage requirements, on-site storm water storage is to be provided for runoff generated during the 100-year 2-hour storm event for improved areas. Rational Method is used to estimate the 100-year 2-hour storm water storage volumes. 100-year, 2-hour event storm water runoff is proposed to be retained on site through retention basins upstream of culvert crossings and at other strategic locations along existing on-site washes. Several basins were designed to retain runoff at a depth of four feet (4') in order to meet the volume provided requirements, and because of ESL and NAOS constraints. During final design when roadway profile and pad elevations have been established, these basins will be reconfigured to pond a maximum depth of three feet (3'). The precipitation values for the 100-year, 2-hour storm event were obtained from the NOAA Atlas 2 Volume VIII maps. The equations and the assumptions used to calculate the required and provided retention volumes are discussed in Section 5.2.4.

Provided below is a summary of the storm water storage required and provided for each individual drainage basin. The following table, Table 3.3 below indicates that the storm water storage provided is in excess of the storm water storage required for the entire site by 2.28 ac-ft. Please refer to Appendix E for detailed storm water storage calculations.

#### 3.3.1 Dissipation of Stormwater

18-inch bleedoff pipes with removable orifice plates are proposed for the dissipation of the stormwater retained within the stipulated time of 36 hours per the City of Scottsdale Drainage Ordinance. Thus, no dry wells will be required. The orifice plates are preliminarily sized using the orifice equation. A 6-inch diameter orifice plate is proposed to dissipate at a rate of 1.1 cfs. The calculation is included in Appendix F.

Table 3.3: Storm Water Storage

Drainage Boundary	Total Development Area (Acre)	100-yr, 2-hr Required Volume (acre-ft)	100-yr, 2-hr. Provided Volume (acre-ft)	Volume Shortage / Excess ± (acre-ft)
A1	3.79	0.53	1.04	0.51
A2	7.17	1.01	1.01	0.00
B	10.94	1.54	1.68	0.14
C	3.65	0.51	0.79	0.28
D	7.73	1.09	1.21	0.12
E1	14.24	2.01	2.03	0.02
E2	13.23	1.87	1.93	0.06
E3	2.19	0.32	0.34	0.02
F1	10.24	1.44	1.60	0.16
F2	3.56	0.5	0.78	0.28
F3	2.93	0.41	0.56	0.15
G	3.7	0.52	0.59	0.07
HI	12.94	1.82	1.94	0.12
H2	8.5	1.2	1.21	0.01
I	4.44	0.63	0.64	0.01
J	4.81	0.68	0.71	0.03
K	2.06	0.29	0.38	0.09
L	1.49	0.21	0.23	0.02
M	1.44	0.2	0.28	0.08
N	2.81	0.4	0.42	0.02
O	0.55	0.08	0.09	0.01
P	1.73	0.24	0.27	0.03
Q	0.52	0.07	0.07	0.00
R	0.18	0.02	0.05	0.03
<b>Total</b>	<b>125</b>	<b>17.59</b>	<b>19.87</b>	<b>2.28</b>

### 3.4 Pre- and Post- Runoff Characteristics

The U S Army Corps of Engineers' HEC-1 computer analysis program was used for hydrologic modeling including routing of flow through storage basins and combining hydrographs. The HEC-1 model was also used to compare the pre- and post-development runoff flows. Based on the site topographic conditions, Kinematic Wave routing technique was used for both the existing and proposed conditions hydrologic modeling. Runoff for each drainage sub-basin was computed and then routed, if required, through downstream drainage sub-basins where the hydrographs were then combined. The parameters were selected per the guidelines provided in the *City of Scottsdale DS&PM*. The parameters selected and the inputs for the HEC-1 models are discussed in Sections 5.1. The parameters changed to model developed conditions are also discussed in Section 5.1.1.

Tables 3.4-1, 3.4-2, and 3.4-3 provide the comparative peak flows for the pre- vs post-developed conditions for 2-year, 10-year, and 100-year, 6-hour events respectively at each concentration point. For the location of these concentration points and corresponding 100-year, 6-hour flow values, please refer to Plates 8 and 9 for existing conditions, and Plates 10 and 11 for developed conditions. The developed conditions HEC-1 model with on-line retention upstream of the culvert road crossings and other strategic locations reduced the combined peak flow to below the pre-development peak flow for the 2-year, 10-year, and 100-year, 6-hour storm events. Further, the post-development flows were reduced to at or below the pre-development flows at every drainage basin for the 2-year, 10-year and 100-year 6-hour storm events.

The HEC-1 input data and output files for the existing and developed conditions are included in Appendices B and C, respectively. The data analysis procedures are discussed in Section 5.0. Hydrographs comparing both pre-development and post-development 100-year flows at concentration points are included in Appendix D. These hydrographs graphically illustrate that the post-development 100-year, 6-hour flows are at or below the pre-development flows. Further, they also illustrate the time to peak and the duration of the storm event.

**Table 3.4-1  
2-Year, 6-Hour Flow Summary**

<b>Concentration Point</b>	<b>Existing Flow (cfs)</b>	<b>Developed Flow (cfs)</b>	<b>Net Downstream Effect (cfs)</b>
A1	43	39	-4
A2	20	14	-6
B1	16	9	-7
COMB B	34	23	-11
C	16	11	-5
D	16	9	-7
E1	24	13	-11
E2	25	17	-8
COMBE	45	21	-24
E3	5	0	-5
F1	18	12	-6
F2	8	5	-3
F3	7	4	-3
G	7	0	-7
H1	26	4	-22
H2	29	16	-13
I	13	2	-11
J	17	9	-8
K	9	0	-9
L	7	3	-4
M	6	0	-6
N	7	0	-7
O	2	2	0
P	5	5	0
Q	3	3	0
R	89	83	-6



**Table 3.4-3  
100-Year, 6-Hour Flow Summary**

<b>Concentration Point</b>	<b>Existing Flow (cfs)</b>	<b>Developed Flow (cfs)</b>	<b>Net Downstream Effect (cfs)</b>
A1	225	223	-2
A2	103	100	-3
B1	79	73	-6
COMB B	192	186	-6
C	85	78	-7
D	88	82	-6
E1	136	131	-5
E2	129	123	-6
COMBE	258	233	-25
E3	26	15	-11
F1	93	88	-5
F2	42	41	-1
F3	38	34	-4
G	39	25	-14
H1	143	117	-26
H2	161	154	-7
I	70	49	-21
J	81	73	-8
K	34	20	-14
L	31	29	-2
M	27	18	-9
N	34	19	-15
O	11	9	-2
P	24	21	-3
Q	12	10	-2
R	429	402	-27

### 3.5 Proposed Drainage Structures

#### 3.5.1 On-site Retention/Detention

On-site retention/detention basins are proposed at the upstream end of culvert roadway crossings and at other strategic locations along existing on-site washes to provide storm water storage as discussed in Section 3.3. Please refer to Appendix E for the 100-year, 2-hour retention volume calculations for all retention basins.

These basins are designed to retain the volume collected for the first three feet of basin depth, and detain any additional volume above 3 feet (3') deep. This is accomplished through an outlet structure consisting of a 3 foot (3') high weir. Please refer to Figure 3 5 1 for an illustration of this design. Further, an 18-inch bleed-off pipe with a removable orifice plate is proposed at the outlet to drain the water retained in the retention basin behind the weir within the stipulated time of 36 hours. Storm water volumes will be retained in the basin up to a height of 3 feet (3'). Several basins were designed to retain runoff at a depth of four feet (4') in order to meet the volume provided requirements, and because of ESL and NAOS constraints. During final design when roadway profile and pad elevations have been established, these basins will be reconfigured to pond a maximum depth of three feet (3'). The bleed-off pipe with orifice plate will drain the water into the natural watercourses downstream of the basin. Thus no dry wells are needed for draining the basins as required by the drainage ordinance of the City of Scottsdale. The orifice plate will only be removed for maintenance purposes.

Storm water volumes above 3 feet (3') high will overflow over the weir. For the 2-year, 10-year, and 100-year, 6-hour storm events, these basins act as detention basins to attenuate the post-development flows to at or below the pre-development flows. Peak flows are routed through these basins acting as on-line detention for the 2-year, 10-year, and 100-year, 6-hour storm events. 100-year, 6-hour peak flows overtop the weirs detained at a depth less than 2 feet (2') as shown in Table 3 4-1. The peak flow volume during the smaller events (2-year, 6-hour) is completely retained in the basins as indicated by zero discharge at post-development conditions in the HEC-1 model. The volume retained will be drained through the bleed-off pipe. This attenuation is modeled using HEC-1.

Provided below in Figures 3.5 1A and 3 5 1B is a graphical illustration of storm water routing/storage through a detention basin

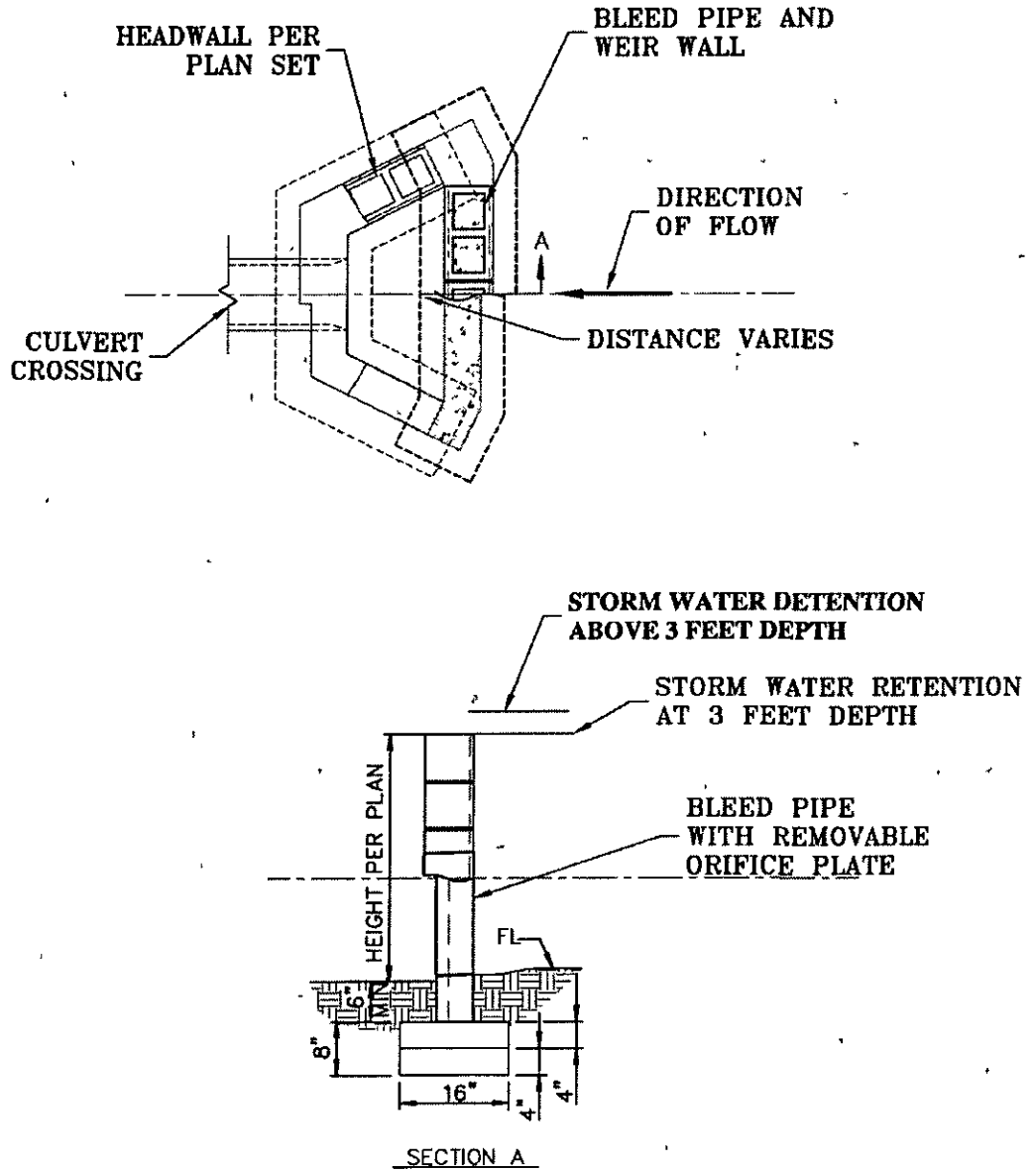
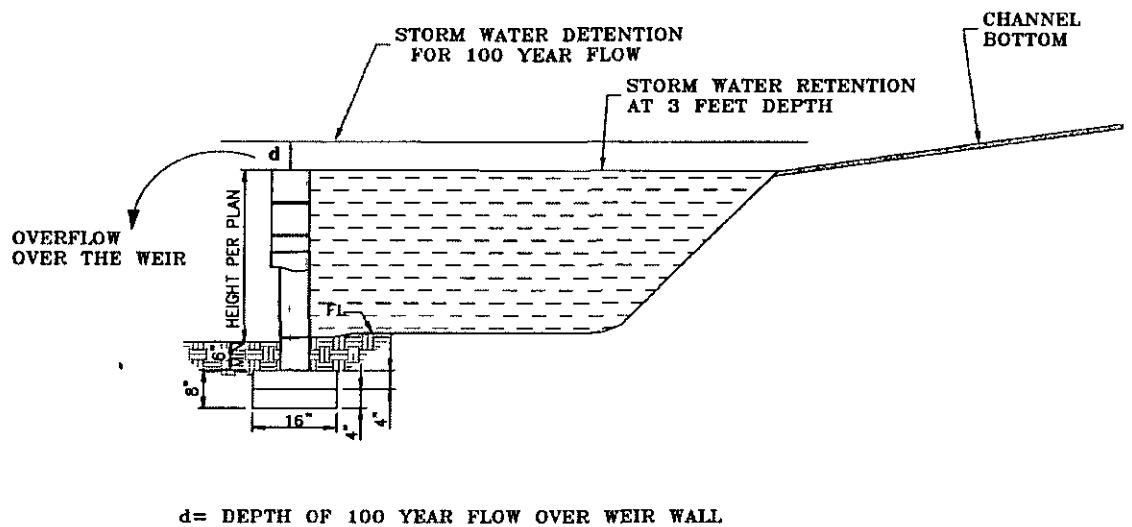


Figure 3.5.1A- Weir Wall and Bleed Pipe at Headwall



**Figure 3.5.1B – Storm Water Routing through a Detention Basin**

### 3.5.2 Roadway Crossing Structures

Please note that the culvert crossings under the roadways were not sized for this report. Culverts will be designed with the final drainage reports, and with the final design of the roadway profiles. The culverts will be designed to convey the 100-year flow downstream of the basin outlet structures. However, care should be taken that the headwater behind the culvert does not inundate the building envelopes. Also, where weirs are present, the headwater elevation at the culvert should be at or below the water surface elevation over the weir during the 100-year, 6-hour storm event. HY-8 culvert analysis program developed for the Federal Highway Administration will be used for the analysis of the culverts proposed with the improvement plans. The approximate 100-year backwater limits (ponding) will also be incorporated within the proposed floodplain delineations.

Rip rap will be utilized within the channels to dissipate velocities and as erosion protection on the upstream and downstream sides of the proposed culverts. The actual design and rip rap sizing will be based on approved practices and will be completed at the time of improvement plan submittal.

Street runoff conveyance and street capacities will be calculated with the final drainage reports prepared for the improvement plan submittals. Curb openings or storm drain systems will be designed to convey the excess 100-year flow so that street flow depths do not exceed eight inches (8") and the 10-year flow is contained within the curbs. The actual design will be based on approved practices and will be completed at the time of improvement plan submittal.

The drainage system for local residential roadways will be designed consistent with the *City of Scottsdale DS&PM*. The methodology developed by the Federal Highway Administration will be used to calculate, in spreadsheet format, the allowable street cross-section capacities.

### 3.6 Project Phasing

The project site is proposed to be developed in four phases. For the completion of this project, a drainage report will be prepared for each phase. The drainage structures proposed within each phase will be designed to meet the *City of Scottsdale DS&PM* guidelines. The impact of the phased construction and the required interim condition drainage systems proposed will be discussed in the drainage report for each phase. The development requirements will be met independently for each phase.

## 4.0 SPECIAL CONDITIONS

### 4.1 404 Washes

All washes within the project were investigated to identify those that may be deemed jurisdictional. The United States Army Corp of Engineers approved 404 Jurisdictional Delineation is presented on Plate 7 – *404 Washes*. This plate provides the approved 404 wash locations within the project site. The 404 washes are also illustrated on the pre- and post-development grading and drainage plans.

## 5.0 DATA ANALYSIS METHODS

### 5.1 Hydrologic Procedures

The U.S. Army Corps of Engineers' HEC-1 hydrologic model is used to compute the 2-year, 10-year and 100-year events, and 6-hour storm events runoff as outlined in Chapter 4 of the *City of Scottsdale DS&PM*. Further, Rational Method is used to compute the 100-year, 2-hour storm event storage volume required. Below is a description of the hydrologic procedures specific to this project.

#### 5.1.1 HEC-1 Procedures

The U.S. Army Corps of Engineers' HEC-1 hydrologic model is used for computing hydrographs routing the flow through sub-basins and combining hydrographs. Based on the site topographic conditions the Kinematic Wave routing technique was used for both the existing and proposed conditions hydrologic modeling. The main components for runoff computation are overland flow, collector channel flow and main channel flow. Input data necessary for the computer analysis included definition and measurement of the overland flow characteristics, drainage sub-basins, assignment of soil curve numbers and calculations of the proposed impervious factors. Provided below is a more detailed explanation of the parameter selection and the data inputs for HEC-1 modeling.

##### 5.1.1.1 Rainfall Event

The precipitation amount for the 2-year, 10-year and 100-year events, 6-hour duration period was obtained from NOAA Atlas 2 Volume VIII maps located in the *City of Scottsdale DS&PM*. An illustration of the location of the site on these maps is provided in Appendix A. These values were further analyzed using the Maricopa County Drainage Design Management Systems for Windows (DDMSW) program to determine the precipitation depths for intervals lower than 1 hour, which are input values in the HEC-1 model. The DDMSW report of this analysis is included in Appendix A. The total calculated rainfall depths for the 2-year, 10-year, and 100-year, 6-hour storm events are 1.57, 2.29, and 3.37 inches respectively. The drainage sub-basins have also been depicted on the NOAA maps. Based on the size of the drainage area, a

single point rainfall amount without aerial reduction was justified for each storm event analyzed

#### 5.1.1.2 Soil Curve Numbers

Soil curve numbers are used to account for interception/infiltration losses. Soil curve numbers for the HEC-1 models were calculated based on published guidelines and engineering experience for the type of soils present within the drainage sub-basins. The cover type (less than 30% vegetation cover due to wild fire) and hydrologic condition were estimated as desert shrub areas with poor hydrologic conditions. Plate 3 presents the soils classification (type D) based on the SCS Map for the project area. A runoff curve number of 88 was used for the corresponding soils group D that occurs within the watershed area. Figure 4-6 "Runoff Curve Numbers for Arid and Semi-Arid Rangelands" located in the *City of Scottsdale DS&PM* was used to determine the soil curve number and has been included in Appendix A. Wood/Patel is in agreement with the City's published curve numbers.

#### 5.1.1.3 Impervious Area

Impervious percentages were determined for each sub-basin for the developed conditions HEC-1 model. Currently, the site contains no development and hence an effective impervious area of zero percent was used. The rock outcroppings on the site are not considered a factor in computing the percent impervious as they do not have a pattern that may impact the drainage characteristics and are not hydraulically connected to the basin outlets. Please refer to photographs of the site and washes in Appendix A. For the on-site developed conditions, impervious percentages were calculated utilizing Figure 2-2-16 "Percent of Impervious Area vs Dwelling Units/Acre", in Chapter 2 of the old *City of Scottsdale DS&PM*. Please refer to Appendix A for a copy of this figure. Based on the lot sizes (Average lot size = 101,500 sq ft), an impervious percentage of 10 percent was used for the proposed developed areas.



#### 5.1.1.4 Overland Flow Resistance Factor

The resistance factor for overland flow was estimated based on Table 3.5 provided in the HEC-1 manual. A copy of this table is included in Appendix A. Based on the site conditions illustrated by the photographs in Appendix A, the site is considered to be sparse rangeland with debris and 20 percent cover. From the aforementioned table in the HEC-1 manual an average resistance factor for overland flow of 15 was chosen.

#### 5.1.1.5 HEC-1 Input Records

This section provides additional description for the input records used in the HEC-1 modeling for this project. The computation time interval for hydrograph generation used was 2 minutes which is within the range of 1 to 5 minutes specified in the *City of Scottsdale DS & PM*.

##### BA Record

This record is a direct input for computing runoff. The element of this record is the drainage area in square miles estimated by measuring the area of the drainage boundary.

##### LS Record

This record is a loss rate data record and computes rainfall losses based on the geological characteristics of the site. The elements on this record are the soil curve numbers and the percent impervious. Both of these data are estimated based on the soil characteristics and site conditions. The derivation of these parameters is discussed in detail in Section 5.1.1.

##### UK Record

This record defines the characteristics of overland flow. The elements on this record are the overland flow length, slope, resistance factor, and percent of sub-basin area represented. All of the input values are measurements and calculated values except for the resistance factor. The resistance factor for overland flow was estimated based on Table 3.5 in the HEC-1 manual. Please note that the values in Table 3.5 are not specific to Arizona. The closest category and value applicable to the site was chosen.

#### RK Record

This record defines the characteristics of the main channel. The elements on this record are the channel length, channel slope, channel roughness, and geometry of the channel. All of the values were measured and calculated values. The channel roughness ('n' value) was chosen based on the site conditions as discussed in Section 5.2. Per the HEC-1 manual, the 'n' value that best represents the channel was chosen. The 'n' value for the channel bottom, 0.032, was chosen to best represent the channel characteristics. Also, this value is conservative as it results in a higher estimate of peak flows. The HEC-1 model was calibrated to the HEC RAS hydraulic model as accurately as possible. However, the HEC-1 model is simplified by using average dimensions of the channel to account for the irregularity in the channel dimensions.

#### SE Record

This record is used for defining the parameters to facilitate routing of flows through a storage basin. The elements on this record are the elevations corresponding to values in the same field in the SV and SQ Records. These values are obtained from the detention basin elevations and/or head water elevations where storage routing is proposed. Please refer to Appendices F and G for the rating calculations and detention basin calculations respectively. Please note that the elevation values are extrapolated for modeling purposes to determine the headwater elevation over the weir and the limits of inundation.

#### SV Record

This record is used for defining the storage volumes in the storage routing of flows. These values are obtained from the storage volumes at different elevations of the ponding at the control structures provided as defined by the SE records. These values are calculated and are provided in Appendix G. Please note that the storage volumes are extrapolated to represent the proposed detention volumes.

### SQ Record

This record is used to define outflow discharge data for storage routing. Please refer to Appendix F for the rating curves for storage intervals. The elements in this field indicate the discharge at different stages for the proposed structures. Appendix F includes calculations for weir flows.

#### **5.1.2 Parameter Changes for Developed Conditions**

The parameter changes depicting the developed conditions are based on the guidelines provided in the *City of Scottsdale DS & PM*. The percent impervious used in the HEC-1 modeling was increased to depict the developed conditions. For the on-site developed conditions, effective impervious percentages were calculated utilizing Figure 2.2-16 "Percent of Impervious Area vs Dwelling Units/Acre", in Chapter 2 of the old *City of Scottsdale DS&PM*. Please refer to Appendix A for a copy of this figure. Based on the lot sizes (Average lot size = 101,500 sq ft), and dwelling density of 0.37 du/acre, an effective impervious percentage of 10 percent was used for the proposed developed areas in the HEC-1 model. An effective impervious area of 10 percent was used for the developed areas within the project site, while 0 percent was used for the current and proposed undeveloped areas. A weighted percent impervious value was used in HEC-1 model for each drainage basin based on the proposed developed and undeveloped areas.

#### **5.1.3 Rational Method**

The Rational Method was used to compute peak discharges for watersheds less than or equal to 160 acres as a comparison with HEC-1 results. Parameters necessary for this procedure are measurement of drainage sub-basin areas, runoff coefficient ("C" values) and calculation of rainfall intensity. Runoff values are calculated using the following 100-year "C" values:

- 0.56 for undeveloped desert land with slopes less than 10% - Hydrologic Soil Group D
- 0.59 for single family residential area R1-130 – Hydrologic Soil Group D

These values are based on Figure 4-5 "Runoff Coefficients for Use with Rational Method" in the *City of Scottsdale DS&PM*. This figure is included in Appendix A. The Maricopa County Flood Control District's Drainage Design Management

System (DDMS) software was used to determine the peak discharges by Rational Method

#### 5.1.4 Stormwater Storage

Based on the City of Scottsdale's Drainage Ordinance storm water storage requirements, on-site storm water storage is proposed to be provided for 100-year, 2-hour storm events. The Rational Method is used to estimate the 100-year, 2-hour storm water storage volumes. 100-year, 2-hour storm water retention volume is to be provided for the proposed development at a depth not to exceed three feet (3'). The precipitation values for the 100-year, 2-hour storm event were obtained from the NOAA Atlas 2 Volume VIII maps. The equations used to calculate the required and provided retention volumes are presented below.

##### 5.1.4.1 Required Retention Volume

Retention volume required for the 100-year, 2-hour event is

$$V_{\text{REQUIRED}} = C * \left( \frac{P}{12} \right) * A$$

Where

- V is the required retention volume in acre-feet
- C is the weighted "C" coefficient,  
0.59 for the proposed developed areas
- P is the precipitation in inches for the 100-year, 2-hour rainfall,  
2.82 inches for the proposed developed areas
- A is the drainage area in acres

##### 5.1.4.2 Provided Retention Volume

Retention volume provided for the 100-year, 2-hour event is

$$V_{\text{PROVIDED}} = \left[ \left( \frac{A_1 + A_2}{2} \right) * (ELEV_2 - ELEV_1) \right]$$

Where

- V is the provided retention volume in acre-feet
- A is the contour area in acres
- ELEV is the contour elevation

Section 3.3 of this report describes the storm water storage provided through proposed detention basins. Assumptions that were made when determining provided volume were maximum side slopes of 4:1 and a maximum ponding depth of three (3) feet

Please refer to retention basin volume calculations in Appendix G

## 5.2 Hydraulic Procedures

### 5.2.1 Hydraulic Analysis of Open Channels

The U S Army Corps of Engineers' HEC-RAS computer program was used for the hydraulic analysis of the washes greater than 50 cfs. HEC-RAS was used to determine the 100-year water surface elevations. Cross-sections were placed such that significant variations in the channel cross-sectional geometry are adequately represented. The Manning's 'n' value was estimated using Cowan Method based on the following parameters:

- Channel material
- Degree of irregularity
- Effects of obstruction
- Vegetation
- Variations in channel cross section
- Degree of meandering

The investigation of site photos presented in Appendix A and aerial photos provided on Plate 5, indicated gradual variations in channel cross-sections within the site and a minor degree of meandering. The vegetation on the site was minimal due to the wild fire that occurred recently and is typical of the local desert climate. The channel material was coarse gravel and the degree of irregularity was minor. Based on the above, a 'n' value of 0.032 and 0.045 were chosen for channel bottom and channel over banks respectively. Site Photos have been included representative of on-site washes that support the "n" value determination. The site photos are included in Appendix A.

Cross-sections were placed such that significant variations in the channel cross-sectional geometry are adequately represented. Due to the steep slopes on the project site, it was not feasible to put enough cross-sections such that the

difference in energy grade elevations is less than one foot (1') between cross-sections. The HEC-RAS computer program gives an error message for a difference in energy grade elevation of greater than one foot (1') between cross-sections. Hence, a wash was analyzed at an elevation difference of one foot (1') to demonstrate the impact of additional cross-sections on the HEC-RAS analysis. It was found that increasing the number of cross sections did not impact the flow depths as long as adequate cross-sections representative of the geometry of the channel are included.

### 5.2.2 Hydraulic Analysis of Culverts

For analysis of culvert roadway crossings the computer program HY-8 will be used during final design. HY-8 is a culvert analysis program that was developed for the Federal Highway Administration. Site characteristics and flow are entered into the program and the resulting flow regime and headwater values are calculated.

## 6.0 DISCUSSION ON MODELING RESULTS

### 6.1 Rational Method vs. HEC-1 Flows

100-year, 6-hour peak flows were calculated using both the Rational Method and HEC-1 for the off-site basins for comparison. The rational method results are included in Appendix E - *100-year Rational Method*. Provided below in Table 6.2 is a comparison of the off-site flows using the Rational Method and HEC-1 results for the 100-year 6-hour storm event

**Table 6.2:**  
**Rational Method vs. HEC-1 Flows**

Concentration Point	Rational Method Flow (cfs)	HEC-1 Flow (cfs)	Difference Rational - Hec1 (cfs)
A1	238	225	13
A2	112	103	9
B	210	192	18
H2	193	161	32
I	76	70	6
J	86	81	5
R	409	429	-20

The table indicates that for the 100-year 6-hour storm event the flow values determined by Rational Method were similar to flow values determined using the HEC-1 analysis. The flow values determined by Rational Method were higher for smaller basins and the flow values calculated by HEC-1 were higher for larger basins. A HEC-RAS hydraulic analysis was performed to determine the impact of this variation in flow values using the different hydrological methods. The 100-year water surface elevations were determined using these flow values and the results are discussed in the subsequent section.

### 6.2 100-Year Floodplain Delineation

The U S Army Corps of Engineers' HEC-RAS Version 3.1.2 was used to generate the water surface profiles for washes with 100-year flows in excess of 50 cfs. The 100-year floodplain delineation for these washes has been revised to reflect the limits of ponding

that occurs because of the proposed retention/detention basins. At this point, the final location of the roadways has not been finalized and therefore a post-development HEC-RAS model that includes cross sections representative of the culverts has not been prepared. It is anticipated that the 100-year floodplain delineation for post-development conditions will be approximately the same as the pre-development delineation with the exception of some ponding at culvert crossings. Water surface elevations and top widths for each cross section are included on Plate 11 – *Post-Development Grading and Drainage Plan*.

The 100-year water surface elevations were computed using the peak flow values from HEC-1 and Rational Method results. Provided below is a summary of the HEC-1 and Rational Method flows and the corresponding water surface elevations.

**Table 6.3: Variation in WSE for Flows from Rational Method and HEC-1**

Concentration Point	100-year flow		Water Surface Elevations		Difference in WSE
	Rational Method Flow (cfs)	HEC-1 Flow (cfs)	Rational Method (ft)	HEC-1 (ft)	Rational Method - HEC-1 (ft)
A1	238	225	2754.31	2754.27	0.03
A2	112	103	2734.62	2734.57	0.04
B	210	192	2747.05	2746.98	0.07
H2	193	161	2731.65	2731.61	0.04
I	76	70	2755.89	2755.85	0.03

The table indicates that the water surface elevations change by only up to 0.07 feet for increases in flow up to 32 cfs. Appendix F provides the detailed HEC-RAS results for all the washes.



## **7.0 DRAINAGE PLAN REQUIREMENTS**

### **7.1 Drainage System Requirements**

The existing drainage patterns will be maintained in their natural location and condition where possible. The site is being developed as large custom lots with an average lot size of 101,500 sq ft. As lots are developed, individual lot engineering will be required to provide drainage documentation to substantiate the development of the lot. In the event of any future development within the off-site watershed areas, it will be the responsibility of the future developer to attenuate the post-development flows and volume to at or below the pre-development values.

### **7.2 Easement Requirements**

Where flows from the 100-year storm event are greater than 50 cfs, drainage easements will be provided. In addition, drainage easements shall be dedicated to the limits of inundation for the 100-year, 2-hour storm event in each retention basin.

### **7.3 Roadway Crossing Requirements**

In all cases the depth of flow over streets will be in accordance with City of Scottsdale Flood Plain and Drainage Ordinance.

### **7.4 Maintenance**

Ongoing maintenance of the designed or recommended drainage systems is required to preserve the design integrity and purpose of the drainage system. Failure to provide maintenance can prevent the drainage system from performing to its intended design purpose and can result in reduced performance. It is the responsibility of private developers, homeowner associations, etc. to maintain facilities on private property, within all drainage easements, private streets, and right-of-ways unless accepted by the City for maintenance. A regular maintenance program is required so that drainage systems perform to the level of protection or service as presented in this report and the project's plans and specifications.

## 7.5 Scour Protection

The need for scour protection will be identified as part of the final drainage reports for each phase of this project. Because of the relatively small peak flow values and incised nature of the on-site washes, development setbacks will most likely be utilized rather than bank protection schemes if required.

## 7.6 Annual Sediment Yield

Sediment yield is defined as that portion of eroded material that travels through a drainage system to a downstream measuring or control point (*Bureau of Reclamation 1987*). The units used for this study are acre-feet/square mile/year (ac-ft/sq mi /yr). The prediction of sediment yields is not an exact science. Many complex variables influence the erosion process and any sediment yields analysis requires considerable engineering judgment. For this reason, two (2) methods were used to arrive at a conservative estimate of the average annual sediment yield rate. The following sections provided a detailed discussion of each method and its results.

Sediment yield was calculated using the Pacific Southwest Inter-Agency Committee (PSIAC) method. This method was obtained from the following source: Pacific Southwest Inter-Agency Committee, *Report of the Water Management Subcommittee on Factors Affecting Sediment Yield in the Pacific Southwest Area and Selection and Evaluation of Measures for Reduction of Erosion and Sediment Yield*, October, 1968.

Nine (9) factors are evaluated in determining the sediment yield classification. These are geology, soils, climate, runoff, topography, ground cover, land use, upland erosion, and channel erosion and sediment transport. Characteristics of each of the nine factors, which give that factor high-, moderate-, or low-sediment yield, are evaluated and the factor is assigned a numerical value representing its significance in the yield rating. The yield rating is the sum of values for the appropriate characteristics for each of the nine factors. For the purpose of avoiding complexity, the factors are generally described as independently influencing the amount of sediment yield. Table 7.6 provides the sediment yield classification based on sediment yield rating.

**Table 7.6 – Sediment Yield Classification Table**

Classification	Rating	Sediment Yield acre-ft/sq. mile
1	> 100	3 0
2	75 – 100	1 0 – 3 0
3	50 – 75	0 5 – 1 0
4	25 – 50	0 2 – 0 5
5	0 – 25	< 0 2

Application of the nine sediment yield factors to the watershed discussed in this report generated a total numerical rating of 77 to 85, the actual rating being dependant upon the interpretation of the guidelines that are used to assign rating values to each factor. Regardless of the actual number, i.e. 77 or 85, both values fall within Classification 2 (75 – 100), which indicates a sediment yield range of 1 0 to 3 0 ac-ft/sq mi./yr.

#### 7.6.1 Bureau of Reclamation Sediment Surveys

The 1987 edition of *Design of Small Dams* (BuRec) publishes data on sediment measurements from 28 reservoirs in semi-arid regions of the United States. A regression line through these data points produces the following equation:

$$Q_s = 1.84 A^{-0.24}$$

Where

$Q_s$  = annual sediment yield (ac-ft/sq mi /yr)

$A$  = drainage area (sq mi)

Using a drainage area size of 1 95 sq mi, this equation predicts an annual sediment yield of 1 57 ac-ft/sq mi /yr or 4,920 cubic yards.

#### 7.6.2 Recommended Annual Sediment Yield

In consideration of the complex processes under investigation, the two (2) methods used exhibit fairly good agreement. Based on comparison of sediment yield calculations to actual sediment yield measurements, Renard & Stone (1981) concluded that “the PSIAC method appears to give the best results for the amount of work required to make the estimate.” Based on the results of the sediment yield equations, a design value of 1 6 ac-ft/sq mi /yr was selected.

### 7.6.3 Short-/Long-Term Aggradation, Degradation, Local Scour, and Deposition

Sediment transport analyses need to distinguish between short-term and long-term changes. Short-term changes are event specific and occur to some extent during each flood hydrograph. Examples of short-term changes would be local scour, general scour, bend scour, bedform troughs, and to some extent low-flow channel incisement. With the exception of low-flow channel incisement, any visual signs of these processes may be difficult to detect after the flow has subsided. Short-term scour processes can usually be quantified with empirical and/or theoretical relationships.

Vertical incisement of the channel bed can occur in response to the following six (6) processes:

- 1 Long-term degradation,
- 2 Local scour,
- 3 General scour,
- 4 Bend scour,
- 5 Low-flow channel incisement, and
- 6 Antidune troughs

Precise prediction of long-term channel impacts can be much more elusive than their short-term counterparts because of the time span involved and the numerous variables that impact long-term changes. The major drainage corridors within the project site will be designed as protected channels.

## 7.7 Protective Devices

Roadway crossing culverts will incorporate riprap at both the inlet and the outlet to dissipate energy and provide flow line scour protection. Retention basins upstream of the roadways will incorporate a protected overflow area (using native riprap and filter fabric) in the event of overtopping. The bleed-off pipes will also incorporate riprap protection.

## 8.0 CONCLUSIONS

### 8.1 Overall Project

1. The project site is located within FEMA Zone "X" (shaded), and Zone "D" as shown on Plate 2
2. The major watercourses traversing through the project will be maintained in their natural location and condition, and will not be re-aligned wherever possible
3. Drainage corridors have been designated for the identified washes in accordance with the appropriate City of Scottsdale ordinance requirements
4. It is being proposed that online retention/detention on the upstream side of the road culvert crossings and other strategic locations be provided to reduce post-development flows to at or below pre-development levels, and for 100-year, 2-hour storm water retention
5. Several basins were designed to retain runoff at a depth of four feet (4') in order to meet the volume provided requirements, and because of ESL and NAOS constraints During final design when roadway profile and pad elevations have been established, these basins will be reconfigured to pond a maximum depth of three feet (3')
6. The post-development peak flows for the 2-year, 10-year and 100-year, 6-hour storm events are reduced to at or below the pre-development flows
7. The stormwater retained will be drained within 36 hours through bleed-off pipes with orifice plates
8. The design of hydraulic structures is to be based on generally accepted engineering practices and in accordance with City of Scottsdale requirements
9. On-going maintenance is required for all drainage systems in order to assure design performance
10. All finished floor elevations are to be designed to be above the 100-year water surface elevation

## 8.2 Project Phasing

The project site is proposed to be developed in four phases. For the completion of the site, a final drainage report will be prepared for each phase of the project. The development requirements will be met independently for each phase.

## 9.0 REFERENCES

- 1 City of Scottsdale, *Design Standards and Policies Manual Chapter 2 Drainage*, December, 1999.
2. City of Scottsdale, *Design Standards and Policies Manual Chapter 4 Drainage*, June, 2005.
3. Flood Control District of Maricopa County, *Drainage Design Manual for Maricopa County, Arizona Volume I – Hydrology*, revised January 1995
4. Flood Control District of Maricopa County, *Drainage Design Manual for Maricopa County, Arizona Volume II – Hydraulics*, January 28, 1996
5. U S Army Corps of Engineers, *HEC-1, Flood Hydrograph Package*, June 1998
6. U S Army Corps of Engineers, *HEC-RAS, Version 3 1 2*, April 2004
7. Arizona Department of Water Resources State Standards for Floodplain Management
8. HEC-1 Manual, Flood Hydrograph Package User's Manual US Army Corps of Engineers Hydrologic Engineering Center
- 9 US Army Corps of Engineers Hydrologic Engineering Center Introduction and Application of Kinematic Wave Routing Techniques using HEC-1.
10. Flood Control District of Maricopa County, Drainage Design Management System for Windows
11. Flood Control District of Maricopa County, Drainage Design Management System
- 12 Pacific Southwest Inter-Agency Committee, *Report of the Water Management Subcommittee on Factors Affecting Sediment Yield in the Pacific Southwest Area and Selection and Evaluation of Measures for Reduction of Erosion and Sediment Yield*, October, 1968
- 13 Design of Small Dams, 1987 Edition

## **APPENDIX A**

### **Supporting Information for Parameter Selection**

- Site Photo Vicinity Map
- Site Photos
- Soil Survey Data
- FCDMC Rainfall Data
- Precipitation Maps
- Runoff Curve Numbers and Percent Impervious Area Chart
- Resistance Factor for Overland Flow
- Runoff Coefficients for use with Rational Method





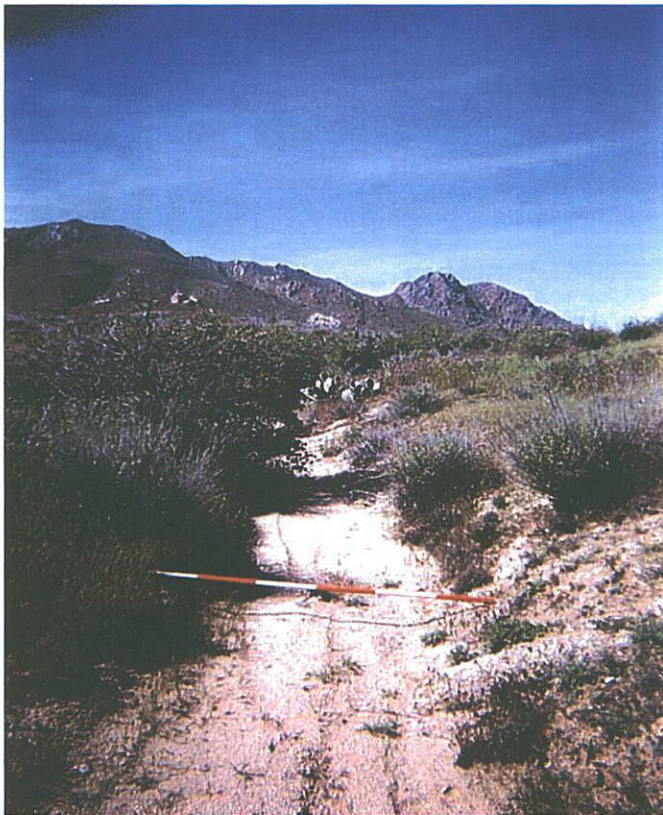
**Photo 1**



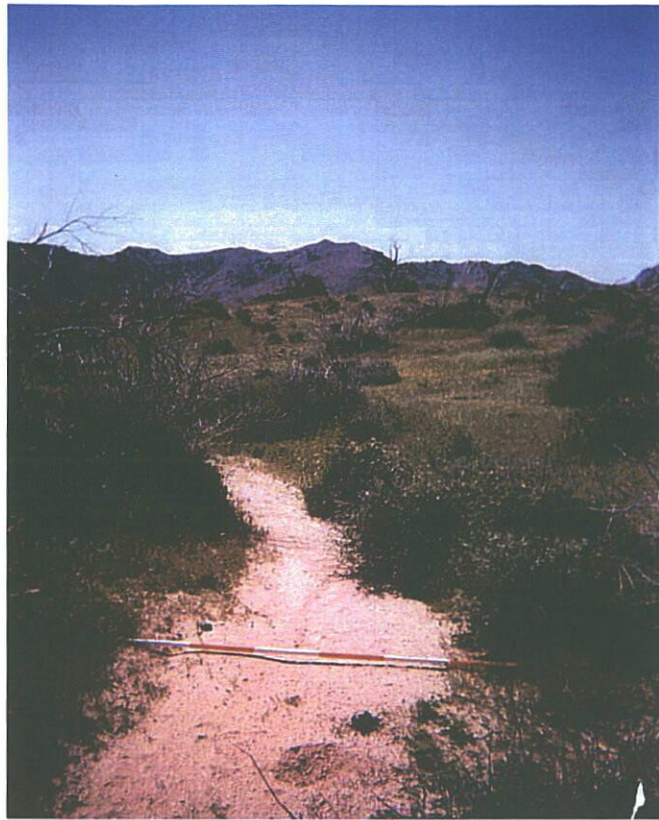
**Photo 2**



**Photo 3**



**Photo 4**



**Photo 11**



**Photo 12**



**Photo 13**



**Photo 14**

TABLE 15.--SOIL AND WATER FEATURES--Continued

Soil name and map symbol	Hydro-logic group	Flooding			Bedrock		Cemented pan		Risk of corrosion	
		Frequency	Duration	Months	Depth	Hardness	Depth	Hardness	Uncoated steel	Concrete
					In		In			
61*, 59* Lenure	B	None	---	---	>60	---	---	---	High	Low.
60* Glenbar Noncalcareous	B	Rare	---	---	>60	---	---	---	High	Low.
Glenbar Calcareous	B	Rare	---	---	>60	---	---	---	High	Low
62*, 62* Wickenburg	D	None	---	---	20-40	Hard	---	---	High	Low.
Wickenburg	D	None	---	---	3-20	Soft	---	---	High	Low.
63*, 64* Wickenburg	D	None	---	---	20-40	Hard	---	---	High	Low.
Wickenburg	D	None	---	---	3-20	Soft	---	---	High	Low
Rock outcrop.										
65* Greyeagle	D	None	---	---	>60	---	4-20	Thick	High	Low.
Continental	C	None	---	---	>60	---	---	---	High	Low.
Nickel	B	None	---	---	>60	---	---	---	High	Low.
66* Greyeagle	D	None	---	---	>60	---	4-20	Thick	High	Low.
Suncity Variant	D	None	---	---	>60	---	5-20	Thick	High	Low.
67* Forest	D	Rare	---	---	>60	---	---	---	High	Low.
68*, 69* Gunsight	B	None	---	---	>60	---	---	---	High	Low.
69* Priano	D	None	---	---	>60	---	5-20	Thick	High	Low.
70*, 71* Gunsight	B	None	---	---	>60	---	---	---	High	Low.
71* Allito	B	None	---	---	>60	---	---	---	High	Low.
72*, 73* Lehmans	D	None	---	---	6-20	Hard	---	---	High	Low.
Rock outcrop										
74* Luke	C	None	---	---	>60	---	20-40	Thick	High	Low.
74* Priano	D	None	---	---	>60	---	4-20	Thick	High	Low.
75, 76, 77, 78, 79* Mohall	B	None	---	---	>60	---	---	---	High	Low
80*, 81* Mohall	B	None	---	---	>60	---	---	---	High	Low

See footnote at end of table

Flood Control District of Mancopa County

Rainfall Data

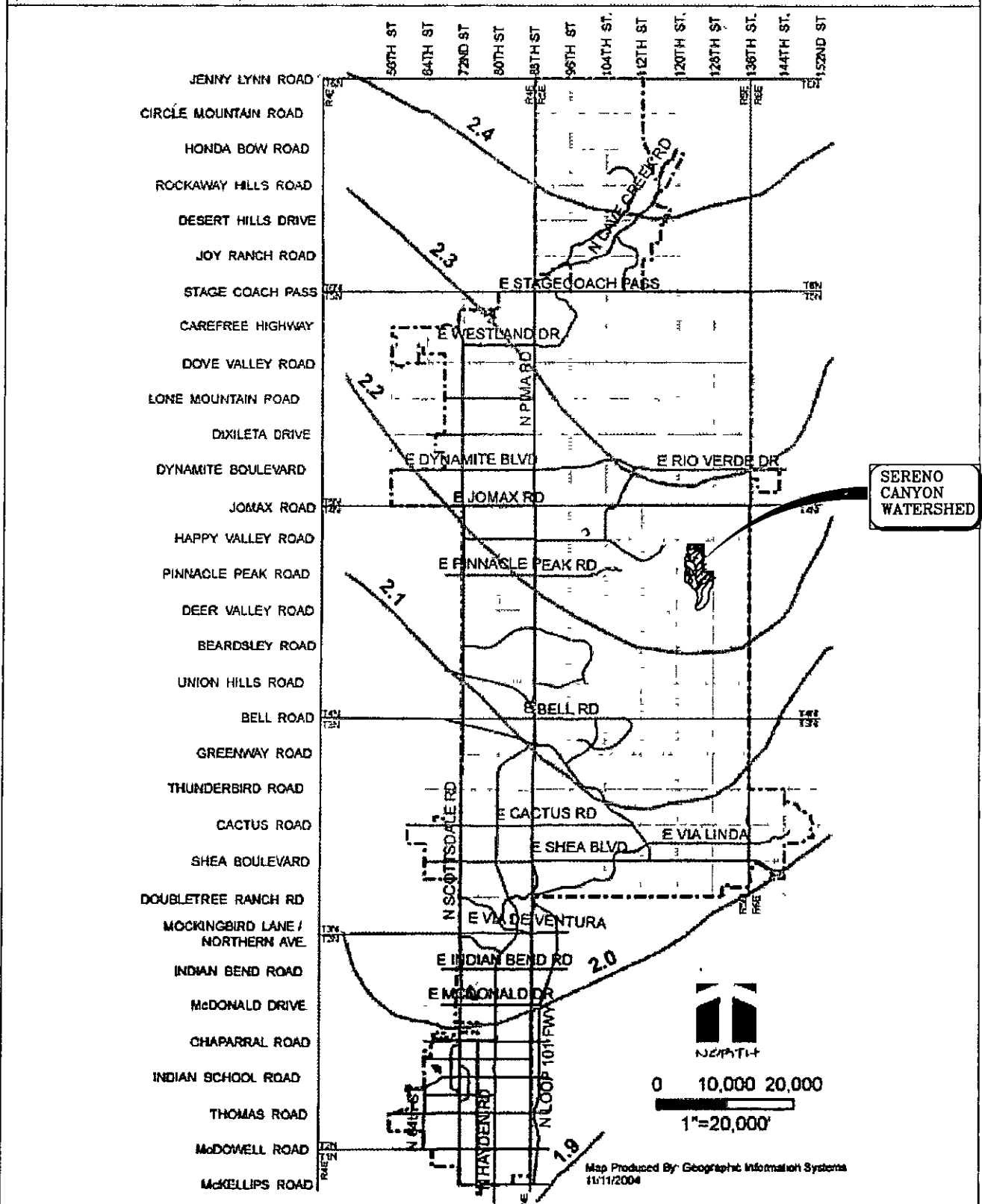
Primary Zone Number 7 Latitude 0 0 Elevation 0  
 Short Duration Zone Number 8 Longitude 0 0

Duration	Point Values (in)					
	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
5 MIN	0 38	0 46	0 52	0 60	0 66	0 73
10 MIN	0 57	0 70	0 78	0 91	1 01	1 11
15 MIN	0 70	0 87	0 99	1 16	1 29	1 43
30 MIN	0 92	1 16	1 33	1 56	1 75	1 94
1 HOUR	1 12	1 43	1 64	1 95	2 19	2 42
2 HOUR	1 27	1 62	1 87	2 21	2 48	2 74
3 HOUR	1 38	1 75	2 01	2 38	2 67	2 96
6 HOUR	1 57	2 00	2 29	2 71	3 04	3 37
12 HOUR	1 81	2 31	2 66	3 15	3 53	3 91
24 HOUR	2 05	2 62	3 02	3 58	4 02	4 45



# Grading & Drainage – Appendix 4-D ISOPLUVIALS

## 10 Year 6 Hour Precipitation in Inches



SERENO  
CANYON  
WATERSHED

Map Produced By: Geographic Information Systems  
11/11/2004

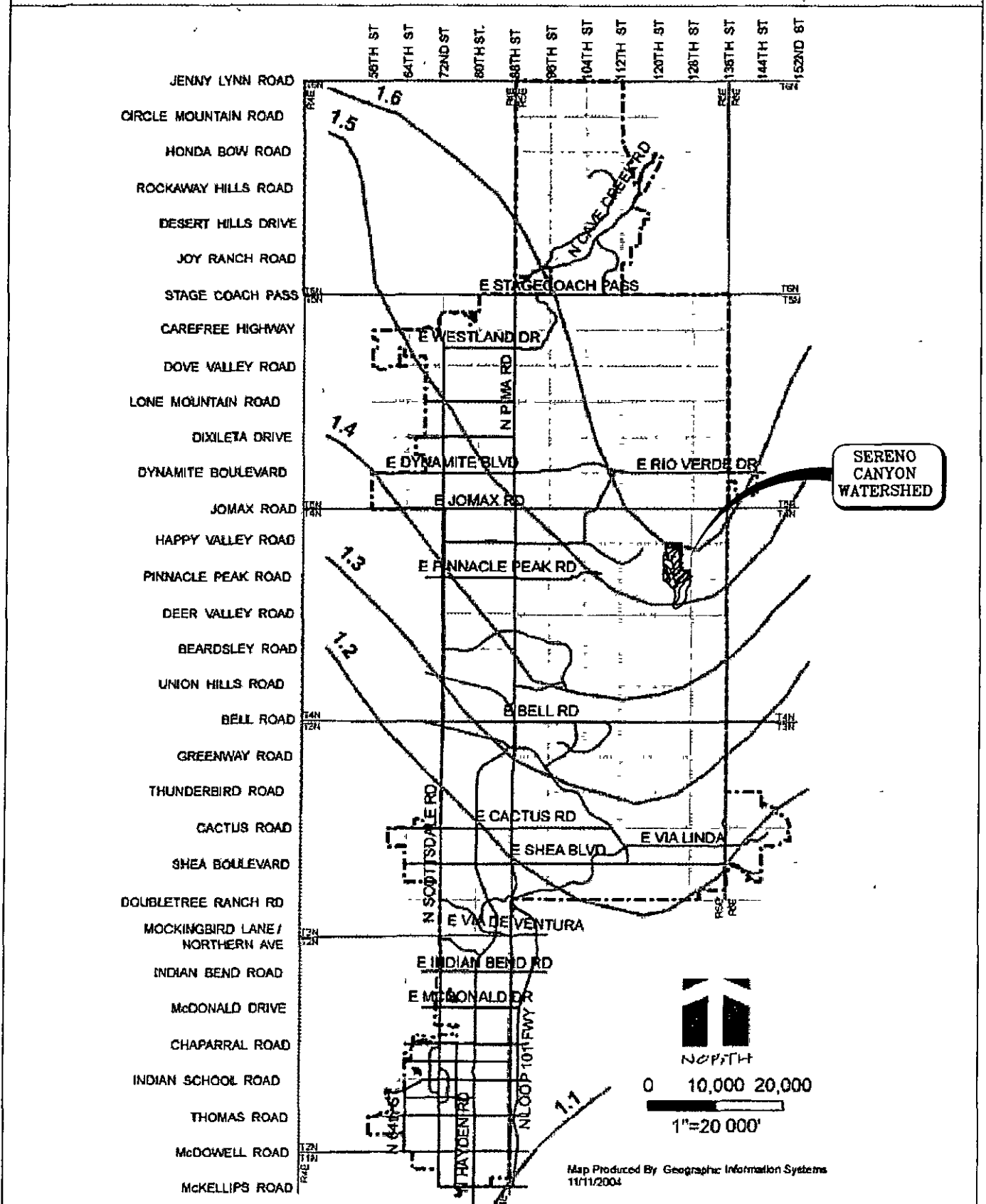






# ISOPLUVIALS

## 2 Year 6 Hour Precipitation in Inches



Map Produced By Geographic Information Systems  
11/11/2004

**2. Infiltration**

Infiltration or soil losses will be determined using Green and Ampt (G&A) procedures per FCDMC Hydrology Manual Use the most recent published SCS soil survey maps of the area to determine the hydrologic soil group or surface soil texture for the G&A procedures Use USDA Natural Resources Conservation Services (previously the Soil Conservation Services) maps, Soil Survey of Aguila-Carefree Area, Parts of Maricopa and Pinal Counties, or the Soil Survey of Eastern Maricopa and Northern Pinal Counties, Arizona, depending on what part of the city you are located

**3. Runoff Curve Numbers**

Where detailed mapping is not available from the SCS or other sources, the use of curve numbers is acceptable When using runoff curve numbers (ROCN) within Scottsdale

- 1 Assume poor hydrologic condition and desert shrub cover type for natural undisturbed desert conditions in Figure 4-6
- 2 For lawns, golf courses, and other grassed open space areas, assume good condition in Figure 4-7 to determine the ROCN, then adjust the ROCN to antecedent moisture condition III (use Figure 4-8)
- 3 For developed conditions, increase the percent impervious on the LS card without changing the ROCN (except in the case of grassed areas, in which the curve number should be adjusted up according to the above) Use the actual or estimated percent impervious, a minimum of 85 percent for commercial and 72 percent industrial

<b>RUNOFF CURVE NUMBERS FOR ARID &amp; SEMIARID RANGELANDS<sup>1</sup></b>					
Cover Type and Hydrologic Condition	Hydrologic Condition <sup>2</sup>	Curve Numbers for Hydrologic Soil Group			
		A <sup>3</sup>	B	C	D
Herbaceous mixture of grass, weeds, and low-growing brush, with brush the minor element	Poor		80	87	93
	Fair		71	81	89
	Good		62	74	85
Oak-aspen mountain brush mixture of oak brush, aspen, mountain mahogany, bitter brush, maple, and other brush	Poor		66	74	79
	Fair		48	57	63
	Good		30	41	48
Pinyon-juniper pinyon, juniper, or both, grass understory	Poor		75	85	89
	Fair		58	73	80
	Good		41	61	71
Sagebrush with grass understory	Poor		67	80	85
	Fair		51	63	70
	Good		35	47	55
Desert shrub major plants include saltbush, greasewood, creosote bush, blackbrush, bursage, Palo Verde, mesquite, and cactus	Poor	63	77	85	88
	Fair	55	72	81	86
	Good	49	68	79	84

<sup>1</sup> Average Runoff Condition, and I<sub>a</sub> = 0.2S

<sup>2</sup> Poor < 30% ground cover (litter, grass, and brush overstory)

Fair 30 to 70% ground cover (not applicable in Scottsdale)

Good >70% ground cover (not applicable in Scottsdale)

<sup>3</sup> Curve Numbers for Group A have been developed only for desert shrub

**FIGURE 4-6 RUNOFF CURVE NUMBERS FOR ARID & SEMIARID RANGELANDS**

**2. Infiltration**

Infiltration or soil losses will be determined using Green and Ampt (G&A) procedures per FCDMC Hydrology Manual Use the most recent published SCS soil survey maps of the area to determine the hydrologic soil group or surface soil texture for the G&A procedures Use USDA Natural Resources Conservation Services (previously the Soil Conservation Services) maps, Soil Survey of Aguila-Carefree Area, Parts of Maricopa and Pinal Counties, or the Soil Survey of Eastern Maricopa and Northern Pinal Counties, Arizona, depending on what part of the city you are located

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RUNOFF CURVE NUMBERS FOR ARID & SEMIARID RANGELANDS <sup>1</sup>					
Cover Type and Hydrologic Condition	Hydrologic Condition <sup>2</sup>	Curve Numbers for Hydrologic Soil Group			
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	Fair		71	81	89
	Good		62	74	85
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	Fair		48	57	63
	Good		30	41	48
Pinyon-juniper pinyon, juniper, or both, grass understory	Poor		75	85	89
	Fair		58	73	80
	Good		41	61	71
Sagebrush with grass understory	Poor		67	80	85
	Fair		51	63	70
	Good		35	47	55
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	Fair	55	72	81	86
	Good	49	68	79	84

<sup>1</sup> Average Runoff Condition, and  $I_a = 0.2S$

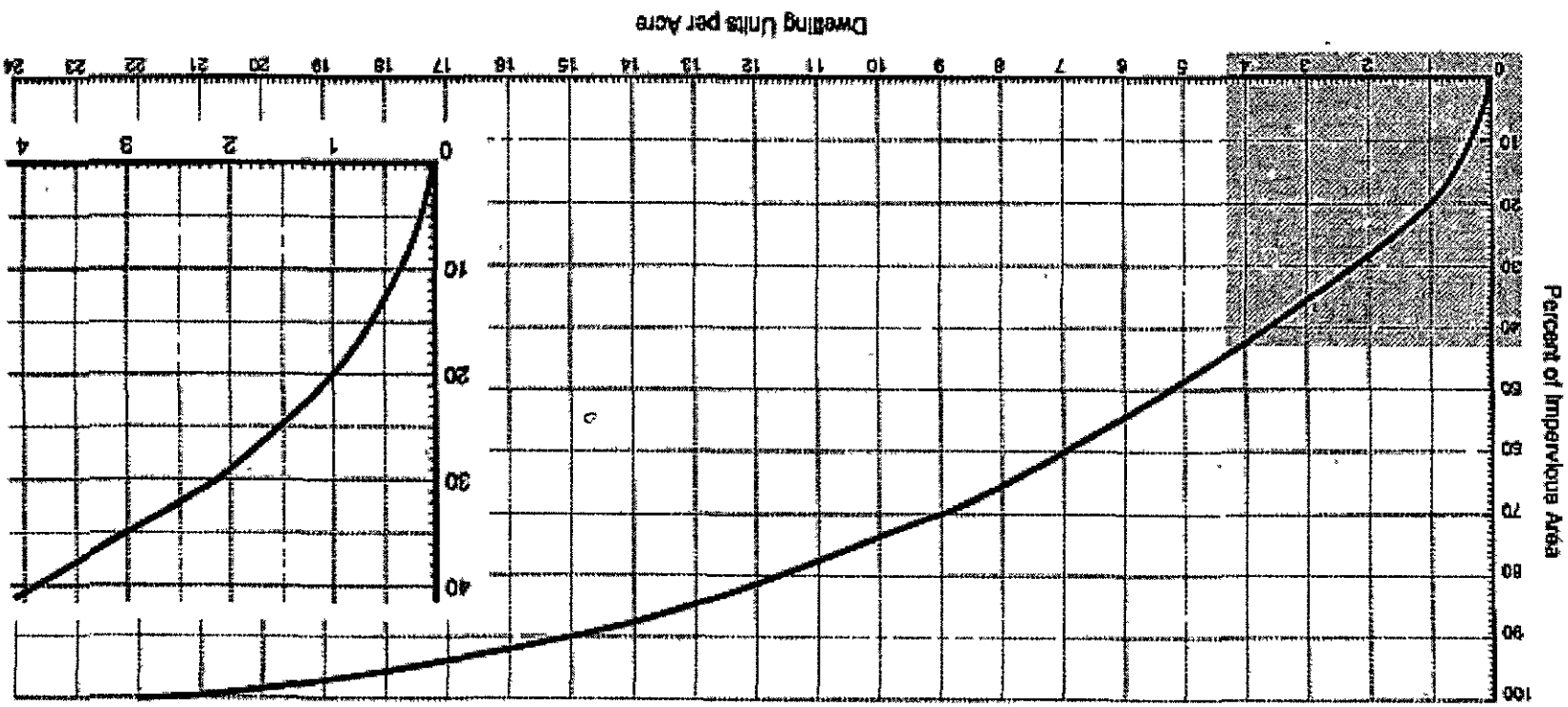
<sup>2</sup> Poor < 30% ground cover (litter, grass, and brush overstory)  
 Fair 30 to 70% ground cover (not applicable in Scottsdale)  
 Good >70% ground cover (not applicable in Scottsdale)

<sup>3</sup> Curve Numbers for Group A have been developed only for desert shrub

**FIGURE 4-6 RUNOFF CURVE NUMBERS FOR ARID & SEMIARID RANGELANDS**

Percent of Impervious Area vs. Dwelling Density

FIGURE 2.2-16



The model requires that at least one overland flow plane and one main channel be used in kinematic wave applications. In the above example, fewer elements might have been used depending on the level of detail required for the hydrologic analysis.

### 3.5 Base Flow

Two distinguishable contributions to a stream flow hydrograph are direct runoff (described earlier) and base flow which results from releases of water from subsurface storage. The HEC-1 model provides means to include the effects of base flow on the streamflow hydrograph as a function of three input parameters, STRTQ, QRCSN and RTIOR. Figure 3.8 defines the relation between the streamflow hydrograph and these variables.

Table 3.6  
Typical Kinematic Wave/Muskingum-Cunge Data

Overland Flow Plane Data					
Identification	Overland Flow Length (ft)	Average Slope (ft/ft)	Roughness Coefficient	Percentage of Subbasin Area	
Pervious Area	200	01	3	80%	
Impervious Area	100	01	1	20%	
Channel Data					
	Channel Length (ft)	Channel Slope (ft/ft)	Channel Roughness	Contributing Area (sq mi)	Shape
Collector Channel	500	005	02	05	2.0 (ft) (Diameter)
Collector Channel	1500	001	015	25	2.0 (ft) (Diameter)
**Main Channel	4000	001	03	1.0 *	Trapezoidal

\* Main channel always assumed to service total subbasin area.

\*\* Note main channel may be eight-point cross section when using Muskingum-Cunge routing. Muskingum-Cunge and kinematic wave channel elements cannot be inter-mixed.

LAND USE Composite Area-wide Values	Hydrologic Soil Group		
	B	C	D
<b>Commercial &amp; Industrial Areas</b>	0.90		
<b>Residential Areas-Single Family (av. lot size)</b>			
R1-1-1901	0.33	0.50	0.58
R1-130	0.35	0.51	0.59
R1-70	0.37	0.52	0.60
R1-43	0.38	0.55	0.61
R1-35 (35,000 sq ft./lot)	0.40	0.56	0.62
R1-18 (18,000 sq ft./lot)	0.43	0.58	0.64
R1-10 (10,000 sq ft./lot)	0.47	0.62	0.67
R1-7 (7,000 sq ft./lot)	0.51	0.64	0.70
<b>Townhouses (R-2, R-4)</b>	0.63	0.74	0.78
<b>Apartments &amp; Condominiums (R-3, R-5)</b>	0.76	0.83	0.87
<b>Specific Surface Type Values</b>			
Paved streets, parking lots (concrete or asphalt), roofs, & driveways, etc	0.95		
Lawns, golf courses, & parks (grassed areas)	0.33	0.56	0.66
Undisturbed natural desert or desert landscaping (no impervious weed barrier)	0.31	0.48	0.56
Desert landscaping (with impervious weed barrier)	0.83	0.83	0.83
Mountain terrain – slopes greater than 10%	0.70	0.70	0.70
Agricultural areas (flood-irrigated fields)	0.20	0.20	0.20

FIGURE 4-5 RUNOFF COEFFICIENTS FOR USE WITH RATIONAL METHOD

#### D. The Army Corps of Engineer's HEC-1 Computer Model

HEC-1 procedures are applicable for any watershed area over 160 acres and up to 100 square miles in size. HEC-1 is required for analyzing drainage areas over 160 acres in size. **HEC-HMS is not an acceptable substitute for HEC-1.** Minimum required submittals when using HEC-1 are:

- A printout of the input data
- A schematic (routing) diagram of the stream network
- The runoff summary output table
- Electronic input file(s), on CD or floppy
- Supporting Documentation and Source Material for parameter selection

##### 1. Precipitation

Precipitation values for HEC-1 modeling shall be determined using the Flood Control Manual, specifically PD and JD records for point rainfall and area reduction factors. Capital Projects shall use the ADOT manual and methodology when specified. Precipitation values are to be obtained from the Isopluvial maps for the specific frequency desired (see Appendix 4-D)

## **APPENDIX B**

### **Existing Condition Hydrologic Calculations**

- 2-year HEC-1 Model
- 10-year HEC-1 Model
- 100-year HEC-1 Model

```

1*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4 1 *
* RUN DATE 04MAY06 TIME 17 46 07 *
*****
    
```

```

*****
* U S ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW  
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE  
 THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81 THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS WRITE STAGE FREQUENCY,  
 DSS READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE NEW FINITE DIFFERENCE ALGORITHM

1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	HEC-1 MODEL FOR SERENO CANYON									
2	ID	2-YEAR, 6-HOUR STORM									
3	ID	RAINFALL FROM NOAA ATLAS									
4	ID	SCS CURVE NUMBER SOIL LOSS PARAMETERS									
5	ID	KINEMATIC WAVE HYDROGRAPH ROUTING									
6	ID	EXISTING CONDITIONS									
7	ID										
8	ID	PREPARED BY WOOD/PATEL, 8 4 2004									
9	ID	FILE NAME EX-100 DAT									
10	ID										
11	*DIAGRAM										
12	IT	2									
13	IO	5									
14	JD	01									
15	PH		38	0 7	1 12	1 27	1 38	1 57			
16	KK	A1									
17	KM	RUNOFF FROM SUB-BASIN A1									
18	BA	084									
19	LS		88								
20	UK	260	025	0 15	100						
21	RK	3520	023	032	TRAP	12	5				
22	KO				22						
23	KK	A2									
24	KM	RUNOFF FROM SUB-BASIN A2									
25	BA	036									
26	LS		88								
27	UK	235	025	0 15	100						
28	RK	2000	02	032	TRAP	8	5				
29	KO				22						
30	KK	B1									
31	KM	RUNOFF FROM SUB-BASIN B1									
32	BA	027									
33	LS		88								
34	UK	200	025	0 15	100						
35	RK	2500	028	032	TRAP	8	5				
36	KO				22						
37	KK	B2									
38	KM	RUNOFF FROM SUB-BASIN B2									
39	BA	050									
40	LS		88								
41	UK	400	025	0 15	100						
42	RK	2340	024	032	TRAP	8	5				
43	KK	COMBB									
44	KM	COMBINE SECTIONS B1 AND B2									
45	HC	2									
46	KO				22						

1

LINE	ID	1	2	3	4	5	6	7	8	9	10
46	KK	C									
47	KM	RUNOFF FROM SUB-BASIN C									
48	BA	03									
49	LS		88								
50	UK	250	023	0 15	100						
51	RK	1570	017	032	TRAP	5	5				
52	KO				22						
53	KK	D									
54	KM	RUNOFF FROM SUB-BASIN D									
55	BA	034									
56	LS		88								



EX-2 OH1

57	UK	300	02	0 15	100					
58	RK	1425	014	032		TRAP	6	5		
59	KO					22				
60	KK	E1								
61	KM	RUNOFF	FROM	SUB-BASIN E1						
62	BA	061								
63	LS		88							
64	UK	400	02	0 15	100					
65	RK	2050	013	032		TRAP	7	5		
66	KO					22				
67	KK	E2								
68	KM	RUNOFF	FROM	SUB-BASIN E2						
69	BA	046								
70	LS		88							
71	UK	200	02	0 15	100					
72	RK	2370	013	032		TRAP	10	5		
73	KO					22				
74	KK	COMBE								
75	KM	COMBINE	SECTIONS	E1 AND E2						
76	HC	2								
77	KO					22				
78	KK	CLEAR								
79	KM	CLEAR	HYDROGRAPH	STACK						
80	HC	6								
	*									

81	KK	E3								
82	KM	RUNOFF	FROM	SUB-BASIN E3						
83	BA	008								
84	LS		88							
85	UK	175	025	0 15	100					
86	RK	750	032	032		TRAP	6	5		
87	KO					22				

HEC-1 INPUT

PAGE 3

1 LINE ID 1 2 3 4 5 6 7 8 9 10

88	KK	F1								
89	KM	RUNOFF	FROM	SUB-BASIN F1						
90	BA	036								
91	LS		88							
92	UK	225	018	0 15	100					
93	RK	2500	012	032		TRAP	7	5		
94	KO					22				
95	KK	F2								
96	KM	RUNOFF	FROM	SUB-BASIN F2						
97	BA	014								
98	LS		88							
99	UK	200	018	0 15	100					
100	RK	1440	028	032		TRAP	6	5		
101	KO					22				
102	KK	F3								
103	KM	RUNOFF	FROM	SUB-BASIN F3						
104	BA	013								
105	LS		88							
106	UK	225	018	0 15	100					
107	RK	850	026	032		TRAP	6	5		
108	KO					22				
109	KK	G								
110	KM	RUNOFF	FROM	SUB-BASIN G						
111	BA	0 016								
112	LS		88							
113	UK	400	025	0 15	100					
114	RK	720	022	032		TRAP	4	5		
115	KO					22				

116	KK	H1								
117	KM	RUNOFF	FROM	SUB-BASIN H1						
118	BA	059								
119	LS		88							
120	UK	375	025	0 15	100					
121	RK	2210	019	032		TRAP	8	5		
122	KO					22				

123	KK	H2								
124	KM	RUNOFF	FROM	SUB-BASIN H2						
125	BA	072								
126	LS		88							
127	UK	400	025	0 15	100					
128	RK	3480	019	032		TRAP	12	5		
129	KO					22				

130	KK	I								
131	KM	RUNOFF	FROM	SUB-BASIN I						
132	BA	025								
133	LS		88							
134	UK	225	02	0 15	100					
135	RK	2100	026	032		TRAP	10	5		

HEC-1 INPUT

PAGE 4

1 LINE ID 1 2 3 4 5 6 7 8 9 10

136	KO					22				
137	KK	CLEAR								
138	KM	CLEAR	HYDROGRAPH	STACK						
139	HC	8								

140	KK	J								
141	KM	RUNOFF	FROM	SUB-BASIN	J					
142	BA	024								
143	LS		88	0						
144	UK	145	02	0 15	100					
145	RK	965	026	032		TRAP	5		5	
146	KO					22				
147	KK	K								
148	KM	RUNOFF	FROM	SUB-BASIN	K					
149	BA	009								
150	LS		88	0						
151	UK	81	025	0 15	100					
152	RK	715	022	032		TRAP	5		5	
153	KO					22				
154	KK	L								
155	KM	RUNOFF	FROM	SUB-BASIN	L					
156	BA	009								
157	LS		88	0						
158	UK	113	018	0 15	100					
159	RK	450	012	032		TRAP	5		5	
160	KO					22				
161	KK	M								
162	KM	RUNOFF	FROM	SUB-BASIN	M					
163	BA	008								
164	LS		88	0						
165	UK	137	018	0 15	100					
166	RK	750	012	032		TRAP	4		5	
167	KO					22				
168	KK	N								
169	KM	RUNOFF	FROM	SUB-BASIN	N					
170	BA	01								
171	LS		88	0						
172	UK	117	018	0 15	100					
173	RK	1036	012	032		TRAP	5		5	
174	KO					22				
175	KK	O								
176	KM	RUNOFF	FROM	SUB-BASIN	O					
177	BA	0029								
178	LS		88	0						
179	UK	108	02	0 15	100					
180	RK	174	014	032		TRAP	4		5	
181	KO					22				

HEC-1 INPUT

PAGE 5

1

LINE ID 1 2 3 4 5 6 7 8 9 10

182	KK	P								
183	KM	RUNOFF	FROM	SUB-BASIN	P					
184	BA	0069								
185	LS		88	0						
186	UK	135	02	0 15	100					
187	RK	354	014	032		TRAP	5		5	
188	KO					22				
189	KK	CLEAR								
190	KM	CLEAR HYDROGRAPH STACK								
191	HC	8								
192	KK	Q								
193	KM	RUNOFF	FROM	SUB-BASIN	Q					
194	BA	0032								
195	LS		88	0						
196	UK	86	02	0 15	100					
197	RK	160	014	032		TRAP	4		5	
198	KO					22				
199	KK	R								
200	KM	RUNOFF	FROM	SUB-BASIN	R					
201	BA	15								
202	LS		88							
203	UK	180	025	0 15	100					
204	RK	4300	023	032		TRAP	15		5	
205	KO					22				
206	ZZ									

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW  
 NO ( ) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

15

A1

22

A2

29

B1

36

B2

42

COMBB

46

C

53

EX-2 OH1

```

60                                     E1
67                                     E2
74                                     COMBE
78 CLEAR
81                                     E3
88                                     F1
95                                     F2
102                                    F3
109                                    G
116                                    H1
123                                    H2
130                                    I
137 CLEAR
140                                    J
147                                    K
154                                    L
161                                    M
168                                    N
175                                    O
182                                    P
189 CLEAR
192                                    Q
199                                    R

```

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4 1 *
* RUN DATE 04MAY06 TIME 17 46 07 *
*****

```

```

*****
* U S ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

HEC-1 MODEL FOR SERENO CANYON  
100-YEAR, 6-HOUR STORM  
RAINFALL FROM NOAA ATLAS  
SCS CURVE NUMBER SOIL LOSS PARAMETERS  
KINEMATIC WAVE HYDROGRAPH ROUTING  
EXISTING CONDITIONS

PREPARED BY WOOD/PATEL, 8 4 2004  
FILE NAME EX-100 DAT

```

12 IO OUTPUT CONTROL VARIABLES
      IPRNT 5 PRINT CONTROL
      IPLOT 0 PLOT CONTROL
      QSCAL 0 HYDROGRAPH PLOT SCALE

```

```

IT HYDROGRAPH TIME DATA
    NMIN 2 MINUTES IN COMPUTATION INTERVAL
    IDATE 1 0 STARTING DATE
    ITIME 0000 STARTING TIME
    NQ 900 NUMBER OF HYDROGRAPH ORDINATES

```

NDDATE 2 0 ENDING DATE  
NDTIME 0558 ENDING TIME  
ICENT 19 CENTURY MARK

EX-2 OH1

COMPUTATION INTERVAL 03 HOURS  
TOTAL TIME BASE 29 97 HOURS

ENGLISH UNITS  
DRAINAGE AREA SQUARE MILES  
PRECIPITATION DEPTH INCHES  
LENGTH, ELEVATION FEET  
FLOW CUBIC FEET PER SECOND  
STORAGE VOLUME ACRE-Feet  
SURFACE AREA ACRES  
TEMPERATURE DEGREES FAHRENHEIT

13 JD INDEX STORM NO 1  
STRM 1 57 PRECIPITATION DEPTH  
TRDA 01 TRANSPOSITION DRAINAGE AREA

14 PI PRECIPITATION PATTERN  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
01 01 01 01 01 01 01 01 01 01  
02 02 02 02 03 03 04 06 08 15  
15 12 07 05 03 03 03 02 02 02  
01 01 01 01 01 01 01 01 01 01  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
15 KK \* A1 \*  
\* \*  
\*\*\*\*\*

21 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

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\* \*  
22 KK \* A2 \*  
\* \*  
\*\*\*\*\*

28 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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\*\*\*\*\*  
\* \*  
29 KK \* B1 \*  
\* \*  
\*\*\*\*\*

35 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

42 KK \*\*\*\*\*  
\* \*  
\* COMBB \*  
\* \*  
\*\*\*\*\*

45 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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46 KK \*\*\*\*\*  
\* \*  
\* C \*  
\* \*  
\*\*\*\*\*

52 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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53 KK \*\*\*\*\*  
\* \*  
\* D \*  
\* \*  
\*\*\*\*\*

59 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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60 KK \*\*\*\*\*  
\* \*  
\* E1 \*  
\* \*  
\*\*\*\*\*

66 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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67 KK \*\*\*\*\*  
\* \*  
\* E2 \*  
\* \*  
\*\*\*\*\*

73 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED

ISAV2  
TIMINT

900 LAST ORDINATE PUNCHED OR SAVED EX-2 OH1  
033 TIME INTERVAL IN HOURS

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\* \*  
74 KK \* COMBE \*  
\* \*  
\*\*\*\*\*

77 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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\* \*  
81 KK \* E3 \*  
\* \*  
\*\*\*\*\*

87 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
88 KK \* F1 \*  
\* \*  
\*\*\*\*\*

94 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
95 KK \* F2 \*  
\* \*  
\*\*\*\*\*

101 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
102 KK \* F3 \*  
\* \*  
\*\*\*\*\*

108 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL

QSCAL	0	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	033	TIME INTERVAL IN HOURS

\*\*\* \*\*

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*****
*      *
109 KK *      G *
*      *
*****

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115 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     033  TIME INTERVAL IN HOURS

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*****
*      *
116 KK *      H1 *
*      *
*****

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122 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     033  TIME INTERVAL IN HOURS

```

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*****
*      *
123 KK *      H2 *
*      *
*****

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```

129 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     033  TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*      *
130 KK *      I *
*      *
*****

```

```

136 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     033  TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*      *
140 KK *      J *
*      *
*****

```

146 KO            OUTPUT CONTROL VARIABLES  
                  IPRNT            5    PRINT CONTROL  
                  IPLOT            0    PLOT CONTROL  
                  QSCAL            0    HYDROGRAPH PLOT SCALE  
                  IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                  IOUT            22    SAVE HYDROGRAPH ON THIS UNIT  
                  ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                  ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
                  TIMINT           033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \* \*  
 147 KK        \*        K \*  
 \* \*  
 \*\*\*\*\*

153 KO            OUTPUT CONTROL VARIABLES  
                  IPRNT            5    PRINT CONTROL  
                  IPLOT            0    PLOT CONTROL  
                  QSCAL            0    HYDROGRAPH PLOT SCALE  
                  IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                  IOUT            22    SAVE HYDROGRAPH ON THIS UNIT  
                  ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                  ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
                  TIMINT           033    TIME INTERVAL IN HOURS

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\*\*\*\*\*  
 \* \*  
 154 KK        \*        L \*  
 \* \*  
 \*\*\*\*\*

160 KO            OUTPUT CONTROL VARIABLES  
                  IPRNT            5    PRINT CONTROL  
                  IPLOT            0    PLOT CONTROL  
                  QSCAL            0    HYDROGRAPH PLOT SCALE  
                  IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                  IOUT            22    SAVE HYDROGRAPH ON THIS UNIT  
                  ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                  ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
                  TIMINT           033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \* \*  
 161 KK        \*        M \*  
 \* \*  
 \*\*\*\*\*

167 KO            OUTPUT CONTROL VARIABLES  
                  IPRNT            5    PRINT CONTROL  
                  IPLOT            0    PLOT CONTROL  
                  QSCAL            0    HYDROGRAPH PLOT SCALE  
                  IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                  IOUT            22    SAVE HYDROGRAPH ON THIS UNIT  
                  ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                  ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
                  TIMINT           033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \* \*  
 168 KK        \*        N \*  
 \* \*  
 \*\*\*\*\*

174 KO            OUTPUT CONTROL VARIABLES  
                  IPRNT            5    PRINT CONTROL  
                  IPLOT            0    PLOT CONTROL  
                  QSCAL            0    HYDROGRAPH PLOT SCALE  
                  IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                  IOUT            22    SAVE HYDROGRAPH ON THIS UNIT  
                  ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                  ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
                  TIMINT           033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*



175 KK \* \*  
 \* O \*  
 \* \*  
 \*\*\*\*\*

181 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0 HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

182 KK \* \*  
 \* P \*  
 \* \*  
 \*\*\*\*\*

188 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0 HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

192 KK \* \*  
 \* Q \*  
 \* \*  
 \*\*\*\*\*

198 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0 HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

199 KK \* \*  
 \* R \*  
 \* \*  
 \*\*\*\*\*

205 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0 HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 033 TIME INTERVAL IN HOURS

1

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

+	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	A1	43	3 33	6	1	1	08		
+	HYDROGRAPH AT	A2	20	3 30	2	1	0	04		
+	HYDROGRAPH AT	B1	16	3 27	2	0	0	03		
+	HYDROGRAPH AT	B2	21	3 43	3	1	1	05		
+	2 COMBINED AT	COMBB	34	3 33	5	1	1	08		
	HYDROGRAPH AT									

					EX-2 OH1			
+		C	16	3 30	2	1	0	03
+	HYDROGRAPH AT	D	16	3 37	2	1	0	03
+	HYDROGRAPH AT	E1	24	3 47	4	1	1	06
+	HYDROGRAPH AT	E2	25	3 30	3	1	1	05
+	2 COMBINED AT	COMBE	45	3 37	7	2	1	11
+	6 COMBINED AT	CLEAR	174	3 33	25	6	5	37
+	HYDROGRAPH AT	E3	5	3 20	1	0	0	01
+	HYDROGRAPH AT	F1	18	3 37	2	1	0	04
+	HYDROGRAPH AT	F2	8	3 30	1	0	0	01
+	HYDROGRAPH AT	F3	7	3 30	1	0	0	01
+	HYDROGRAPH AT	G	7	3 40	1	0	0	02
+	HYDROGRAPH AT	H1	26	3 43	4	1	1	06
+	HYDROGRAPH AT	H2	29	3 47	5	1	1	07
+	HYDROGRAPH AT	I	13	3 33	2	0	0	03
+	8 COMBINED AT	CLEAR	105	3 37	16	4	3	24
+	HYDROGRAPH AT	J	17	3 20	2	0	0	02
+	HYDROGRAPH AT	K	9	3 13	1	0	0	01
+	HYDROGRAPH AT	L	7	3 17	1	0	0	01
+	HYDROGRAPH AT	M	6	3 20	1	0	0	01
+	HYDROGRAPH AT	N	7	3 20	1	0	0	01
+	HYDROGRAPH AT	O	2	3 17	0	0	0	00
+	HYDROGRAPH AT	P	5	3 17	0	0	0	01
+	8 COMBINED AT	CLEAR	134	3 33	21	5	4	31
+	HYDROGRAPH AT	Q	3	3 13	0	0	0	00
+	HYDROGRAPH AT	R	89	3 30	10	3	2	15

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION INTERVAL PEAK	INTERVAL TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1	STORM AREA (SQ MI) =			01					
A1	MANE	1 86	43 40	201 06	63	2 00	43 09	200 00	63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2833E+01 OUTFLOW= 2811E+01 BASIN STORAGE= 1422E-02 PERCENT ERROR= 7

FOR STORM = 1	STORM AREA (SQ MI) =			01					
A2	MANE	2 00	20 40	197 76	63	2 00	20 32	198 00	63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1214E+01 OUTFLOW= 1205E+01 BASIN STORAGE= 4691E-03 PERCENT ERROR= 7

FOR STORM = 1	STORM AREA (SQ MI) =			01					
B1	MANE	2 00	15 93	195 89	63	2 00	15 93	196 00	63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9106E+00 OUTFLOW= 9030E+00 BASIN STORAGE= 2704E-03 PERCENT ERROR= 8

FOR STORM = 1	STORM AREA (SQ MI) =			01					
B2	MANE	1 77	21 18	206 52	63	2 00	21 08	206 00	63

EX-2 OH1

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1686E+01 OUTFLOW= 1676E+01 BASIN STORAGE= 1425E-02 PERCENT ERROR= 6  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 C MANE 1 84 16 24 197 74 63 2 00 16 24 198 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1012E+01 OUTFLOW= 1004E+01 BASIN STORAGE= 4595E-03 PERCENT ERROR= 7  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 D MANE 1 74 16 01 201 93 63 2 00 16 00 202 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1147E+01 OUTFLOW= 1139E+01 BASIN STORAGE= 6731E-03 PERCENT ERROR= 6  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E1 MANE 1 87 24 22 208 02 63 2 00 24 21 208 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2057E+01 OUTFLOW= 2046E+01 BASIN STORAGE= 1822E-02 PERCENT ERROR= 4  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E2 MANE 2 00 25 67 199 33 63 2 00 25 49 198 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1551E+01 OUTFLOW= 1540E+01 BASIN STORAGE= 6121E-03 PERCENT ERROR= 7  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E3 MANE 93 5 49 192 15 63 2 00 5 47 192 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2698E+00 OUTFLOW= 2681E+00 BASIN STORAGE= 6786E-04 PERCENT ERROR= 6  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F1 MANE 1 95 18 10 202 12 63 2 00 18 09 202 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1214E+01 OUTFLOW= 1207E+01 BASIN STORAGE= 5783E-03 PERCENT ERROR= 6  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F2 MANE 1 72 8 06 198 12 63 2 00 8 06 198 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4722E+00 OUTFLOW= 4691E+00 BASIN STORAGE= .1912E-03 PERCENT ERROR= 6  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F3 MANE 1 02 7 10 197 64 63 2 00 7 09 198 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4385E+00 OUTFLOW= 4359E+00 BASIN STORAGE= 1862E-03 PERCENT ERROR= 5  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 G MANE 1 02 6 92 203 91 63 2 00 6 91 204 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5396E+00 OUTFLOW= 5363E+00 BASIN STORAGE= 4299E-03 PERCENT ERROR= 5  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 H1 MANE 2 00 25 66 205 92 63 2 00 25 63 206 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1990E+01 OUTFLOW= 1975E+01 BASIN STORAGE= 1494E-02 PERCENT ERROR= 7  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 H2 MANE 2 00 29 40 208 72 63 2 00 29 35 208 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2428E+01 OUTFLOW= 2411E+01 BASIN STORAGE= 2200E-02 PERCENT ERROR= 6  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 I MANE 1 77 13 52 198 63 63 2 00 13 43 200 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 8432E+00 OUTFLOW= 8364E+00 BASIN STORAGE= 3859E-03 PERCENT ERROR= 8  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 J MANE 94 17 19 192 27 63 2 00 17 19 192 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 8095E+00 OUTFLOW= 8051E+00 BASIN STORAGE= 1749E-03 PERCENT ERROR= 5  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 K MANE 96 8 95 187 64 63 2 00 8 90 188 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3035E+00 OUTFLOW= 3024E+00 BASIN STORAGE= 2356E-04 PERCENT ERROR= 4  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 L MANE 71 7 23 190 82 63 2 00 7 19 190 00 63

EX-2 OH1

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3035E+00 OUTFLOW= 3020E+00 BASIN STORAGE= 4446E-04 PERCENT ERROR= 5

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 M MANE 1 33 5 68 193 42 63 2 00 5 63 192 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2698E+00 OUTFLOW= 2683E+00 BASIN STORAGE= 6045E-04 PERCENT ERROR= 5

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 N MANE 1 63 7 42 191 98 63 2 00 7 42 192 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3373E+00 OUTFLOW= 3352E+00 BASIN STORAGE= 5862E-04 PERCENT ERROR= 6

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 O MANE 44 2 47 189 94 63 2 00 2 45 190 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9781E-01 OUTFLOW= 9730E-01 BASIN STORAGE= 1338E-04 PERCENT ERROR= 5

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 P MANE 62 5 25 190 85 63 2 00 5 18 190 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2327E+00 OUTFLOW= 2314E+00 BASIN STORAGE= 4631E-04 PERCENT ERROR= 5

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 Q MANE 39 3 06 187 56 63 2 00 3 00 188 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1079E+00 OUTFLOW= 1075E+00 BASIN STORAGE= 9685E-05 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 R MANE 1 84 89 97 197 31 63 2 00 89 03 198 00 63

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5059E+01 OUTFLOW= 5017E+01 BASIN STORAGE= 1598E-02 PERCENT ERROR= 8

\*\*\* NORMAL END OF HEC-1 \*\*\*

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4 1 *
* RUN DATE 04MAY06 TIME 17 47 14 *
*****
    
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*****
* U S ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW  
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE  
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81 THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS WRITE STAGE FREQUENCY,  
 DSS READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE NEW FINITE DIFFERENCE ALGORITHM

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	HEC-1 MODEL FOR SERENO CANYON									
2	ID	10-YEAR, 6-HOUR STORM									
3	ID	RAINFALL FROM NOAA ATLAS									
4	ID	SCS CURVE NUMBER SOIL LOSS PARAMETERS									
5	ID	KINEMATIC WAVE HYDROGRAPH ROUTING									
6	ID	EXISTING CONDITIONS									
7	ID										
8	ID	PREPARED BY WOOD/PATEL, 8 4 2004									
9	ID	FILE NAME EX-100 DAT									
10	ID										
11	ID	*DIAGRAM									
12	IT	2			900						
13	IO	5									
14	JD		01								
15	PH			0 52	0 99	1 64	1 87	2 01	2 29		
16	KK	A1									
17	KM	RUNOFF FROM SUB-BASIN A1									
18	BA	084									
19	LS		88								
20	UK	260	025	0 15	100						
21	RK	3520	023	032		TRAP	12	5			
22	KO					22					
23	KK	A2									
24	KM	RUNOFF FROM SUB-BASIN A2									
25	BA	036									
26	LS		88								
27	UK	235	025	0 15	100						
28	RK	2000	02	032		TRAP	8	5			
29	KO					22					
30	KK	B1									
31	KM	RUNOFF FROM SUB-BASIN B1									
32	BA	027									
33	LS		88								
34	UK	200	025	0 15	100						
35	RK	2500	028	032		TRAP	8	5			
36	KO					22					
37	KK	B2									
38	KM	RUNOFF FROM SUB-BASIN B2									
39	BA	050									
40	LS		88								
41	UK	400	025	0 15	100						
42	RK	2340	024	032		TRAP	8	5			
43	KK	COMBB									
44	KM	COMBINE SECTIONS B1 AND B2									
45	HC	2									
46	KO										

LINE	ID	1	2	3	4	5	6	7	8	9	10
46	KK	C									
47	KM	RUNOFF FROM SUB-BASIN C									
48	BA	03									
49	LS		88								
50	UK	250	023	0 15	100						
51	RK	1570	017	032		TRAP	5	5			
52	KO					22					
53	KK	D									
54	KM	RUNOFF FROM SUB-BASIN D									
55	BA	034									
56	LS		88								

57	UK	300	02	0 15	100	EX-10 OH1													
58	RK	1425	014	032		TRAP	6												
59	KO					22													
60	KK	E1																	
61	KM	RUNOFF	FROM	SUB-BASIN E1															
62	BA	061																	
63	LS		88																
64	UK	400	02	0 15	100														
65	RK	2050	013	032		TRAP	7												
66	KO					22													
67	KK	E2																	
68	KM	RUNOFF	FROM	SUB-BASIN E2															
69	BA	046																	
70	LS		88																
71	UK	200	02	0 15	100														
72	RK	2370	013	032		TRAP	10												
73	KO					22													
74	KK	COMBE																	
75	KM	COMBINE	SECTIONS	E1 AND E2															
76	HC	2																	
77	KO					22													
78	KK	CLEAR																	
79	KM	CLEAR	HYDROGRAPH	STACK															
80	HC	6																	
81	KK	E3																	
82	KM	RUNOFF	FROM	SUB-BASIN E3															
83	BA	008																	
84	LS		88																
85	UK	175	025	0 15	100														
86	RK	750	032	032		TRAP	6												
87	KO					22													

PAGE 3

1

LINE	ID	1	2	3	4	5	6	7	8	9	10								
88	KK	F1																	
89	KM	RUNOFF	FROM	SUB-BASIN F1															
90	BA	036																	
91	LS		88																
92	UK	225	018	0 15	100														
93	RK	2500	012	032		TRAP	7												
94	KO					22													
95	KK	F2																	
96	KM	RUNOFF	FROM	SUB-BASIN F2															
97	BA	014																	
98	LS		88																
99	UK	200	018	0 15	100														
100	RK	1440	028	032		TRAP	6												
101	KO					22													
102	KK	F3																	
103	KM	RUNOFF	FROM	SUB-BASIN F3															
104	BA	013																	
105	LS		88																
106	UK	225	018	0 15	100														
107	RK	850	026	032		TRAP	6												
108	KO					22													
109	KK	G																	
110	KM	RUNOFF	FROM	SUB-BASIN G															
111	BA	0 016																	
112	LS		88																
113	UK	400	025	0 15	100														
114	RK	720	022	032		TRAP	4												
115	KO					22													
116	KK	H1																	
117	KM	RUNOFF	FROM	SUB-BASIN H1															
118	BA	059																	
119	LS		88																
120	UK	375	025	0 15	100														
121	RK	2210	019	032		TRAP	8												
122	KO					22													
123	KK	H2																	
124	KM	RUNOFF	FROM	SUB-BASIN H2															
125	BA	072																	
126	LS		88																
127	UK	400	025	0 15	100														
128	RK	3480	019	032		TRAP	12												
129	KO					22													
130	KK	I																	
131	KM	RUNOFF	FROM	SUB-BASIN I															
132	BA	025																	
133	LS		88																
134	UK	225	02	0 15	100														
135	RK	2100	026	032		TRAP	10												

PAGE 4

1

LINE	ID	1	2	3	4	5	6	7	8	9	10								
136	KO					22													
137	KK	CLEAR																	
138	KM	CLEAR	HYDROGRAPH	STACK															
139	HC	8																	

140	KK	J								
141	KM	RUNOFF	FROM	SUB-BASIN	J					
142	BA	024								
143	LS		88	0		100				
144	UK	145	02	0 15						
145	RK	965	026	0 32			TRAP	5	5	
146	KO						22			
147	KK	K								
148	KM	RUNOFF	FROM	SUB-BASIN	K					
149	BA	009								
150	LS		88	0		100				
151	UK	81	025	0 15						
152	RK	715	022	0 32			TRAP	5	5	
153	KO						22			
154	KK	L								
155	KM	RUNOFF	FROM	SUB-BASIN	L					
156	BA	009								
157	LS		88	0		100				
158	UK	113	018	0 15						
159	RK	450	012	0 32			TRAP	5	5	
160	KO						22			
161	KK	M								
162	KM	RUNOFF	FROM	SUB-BASIN	M					
163	BA	008								
164	LS		88	0		100				
165	UK	137	018	0 15						
166	RK	750	012	0 32			TRAP	4	5	
167	KO						22			
168	KK	N								
169	KM	RUNOFF	FROM	SUB-BASIN	N					
170	BA	01								
171	LS		88	0		100				
172	UK	117	018	0 15						
173	RK	1036	012	0 32			TRAP	5	5	
174	KO						22			
175	KK	O								
176	KM	RUNOFF	FROM	SUB-BASIN	O					
177	BA	0029								
178	LS		88	0		100				
179	UK	108	02	0 15						
180	RK	174	014	0 32			TRAP	4	5	
181	KO						22			

1

HEC-1 INPUT

PAGE 5

LINE ID 1 2 3 4 5 6 7 8 9 10

182	KK	P								
183	KM	RUNOFF	FROM	SUB-BASIN	P					
184	BA	0069								
185	LS		88	0		100				
186	UK	135	02	0 15						
187	RK	354	014	0 32			TRAP	5	5	
188	KO						22			
189	KK	CLEAR								
190	KM	CLEAR	HYDROGRAPH	STACK						
191	HC	8								
192	KK	Q								
193	KM	RUNOFF	FROM	SUB-BASIN	Q					
194	BA	0032								
195	LS		88	0		100				
196	UK	86	02	0 15						
197	RK	160	014	0 32			TRAP	4	5	
198	KO						22			
199	KK	R								
200	KM	RUNOFF	FROM	SUB-BASIN	R					
201	BA	15								
202	LS		88	0		100				
203	UK	180	025	0 15						
204	RK	4300	023	0 32			TRAP	15	5	
205	KO						22			
206	ZZ									

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW  
 NO ( ) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

15 A1

22 A2

29 B1

36 B2

42 COMBB

46 C

53 D Page 3

EX-10 OH1

60

E1

67

E2

74

COMBE

78

CLEAR

81

E3

88

F1

95

F2

102

F3

109

G

116

H1

123

H2

130

I

137

CLEAR

140

J

147

K

154

L

161

M

168

N

175

O

182

P

189

CLEAR

192

Q

199

R

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4 1 *
* RUN DATE 04MAY06 TIME 17 47 14 *
*****

```

```

*****
* U S ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

HEC-1 MODEL FOR SERENO CANYON  
 100-YEAR, 6-HOUR STORM  
 RAINFALL FROM NOAA ATLAS  
 SCS CURVE NUMBER SOIL LOSS PARAMETERS  
 KINEMATIC WAVE HYDROGRAPH ROUTING  
 EXISTING CONDITIONS

PREPARED BY WOOD/PATEL, 8 4 2004  
 FILE NAME EX-100 DAT

12 IO

OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0	HYDROGRAPH PLOT SCALE

IT

HYDROGRAPH TIME DATA

NMIN	2	MINUTES IN COMPUTATION INTERVAL
IDATE	1	STARTING DATE
ITIME	0000	STARTING TIME
NQ	900	NUMBER OF HYDROGRAPH ORDINATES



NDDATE 2 0 ENDING DATE
NDTIME 0558 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 03 HOURS
TOTAL TIME BASE 29 97 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

13 JD INDEX STORM NO 1
STRM 2 29 PRECIPITATION DEPTH
TRDA 01 TRANSPOSITION DRAINAGE AREA

Table with 10 columns of precipitation data for station 14 PI. Values range from 00 to 21.

\*\*\* \*\* separator line

15 KK \*\*\*\*\*
\* \*
\* A1 \*
\* \*
\*\*\*\*\*

21 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\* separator line

22 KK \*\*\*\*\*
\* \*
\* A2 \*
\* \*
\*\*\*\*\*

28 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\* separator line

29 KK \*\*\*\*\*
\* \*
\* B1 \*
\* \*
\*\*\*\*\*

35 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
42 KK \* COMBB \*  
\* \*  
\*\*\*\*\*

45 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
46 KK \* C \*  
\* \*  
\*\*\*\*\*

52 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
53 KK \* D \*  
\* \*  
\*\*\*\*\*

59 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
60 KK \* E1 \*  
\* \*  
\*\*\*\*\*

66 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
67 KK \* E2 \*  
\* \*  
\*\*\*\*\*

73 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED

ISAV2 900 LAST ORDINATE PUNCHED OR SAVED EX-10 OH1  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
74 KK \* COMBE \*  
\* \*  
\*\*\*\*\*

77 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
81 KK \* E3 \*  
\* \*  
\*\*\*\*\*

87 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
88 KK \* F1 \*  
\* \*  
\*\*\*\*\*

94 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
95 KK \* F2 \*  
\* \*  
\*\*\*\*\*

101 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
102 KK \* F3 \*  
\* \*  
\*\*\*\*\*

108 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL

```

QSCAL      0  HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
TIMINT     033 TIME INTERVAL IN HOURS

```

EX-10 OH1

\*\*\* \*\*

```

*****
* *
*   G *
* *
*****

```

```

115 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     033 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
* *
*   H1 *
* *
*****

```

```

122 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     033 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
* *
*   H2 *
* *
*****

```

```

129 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     033 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
* *
*   I *
* *
*****

```

```

136 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     033 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
* *
*   J *
* *
*****

```

146 KO            OUTPUT CONTROL VARIABLES  
                   IPRNT            5    PRINT CONTROL  
                   IPLOT            0    PLOT CONTROL  
                   QSCAL            0    HYDROGRAPH PLOT SCALE  
                   IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                   IOUT            22    SAVE HYDROGRAPH ON THIS UNIT  
                   ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                   ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
                   TIMINT            033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 147 KK       \*            K       \*  
 \*            \*  
 \*\*\*\*\*

153 KO            OUTPUT CONTROL VARIABLES  
                   IPRNT            5    PRINT CONTROL  
                   IPLOT            0    PLOT CONTROL  
                   QSCAL            0    HYDROGRAPH PLOT SCALE  
                   IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                   IOUT            22    SAVE HYDROGRAPH ON THIS UNIT  
                   ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                   ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
                   TIMINT            033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 154 KK       \*            L       \*  
 \*            \*  
 \*\*\*\*\*

160 KO            OUTPUT CONTROL VARIABLES  
                   IPRNT            5    PRINT CONTROL  
                   IPLOT            0    PLOT CONTROL  
                   QSCAL            0    HYDROGRAPH PLOT SCALE  
                   IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                   IOUT            22    SAVE HYDROGRAPH ON THIS UNIT  
                   ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                   ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
                   TIMINT            033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 161 KK       \*            M       \*  
 \*            \*  
 \*\*\*\*\*

167 KO            OUTPUT CONTROL VARIABLES  
                   IPRNT            5    PRINT CONTROL  
                   IPLOT            0    PLOT CONTROL  
                   QSCAL            0    HYDROGRAPH PLOT SCALE  
                   IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                   IOUT            22    SAVE HYDROGRAPH ON THIS UNIT  
                   ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                   ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
                   TIMINT            033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 168 KK       \*            N       \*  
 \*            \*  
 \*\*\*\*\*

174 KO            OUTPUT CONTROL VARIABLES  
                   IPRNT            5    PRINT CONTROL  
                   IPLOT            0    PLOT CONTROL  
                   QSCAL            0    HYDROGRAPH PLOT SCALE  
                   IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
                   IOUT            22    SAVE HYDROGRAPH ON THIS UNIT  
                   ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
                   ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
                   TIMINT            033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*

175 KK \* \*  
 \* O \*  
 \* \*  
 \*\*\*\*\*

181 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0 HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

182 KK \* \*  
 \* P \*  
 \* \*  
 \*\*\*\*\*

188 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0 HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

192 KK \* \*  
 \* Q \*  
 \* \*  
 \*\*\*\*\*

198 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0 HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

199 KK \* \*  
 \* R \*  
 \* \*  
 \*\*\*\*\*

205 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0 HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 033 TIME INTERVAL IN HOURS

1

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

+	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	A1	106	3 23	11	3	2	08		
+	HYDROGRAPH AT	A2	51	3 20	5	1	1	04		
+	HYDROGRAPH AT	B1	39	3 20	3	1	1	03		
+	HYDROGRAPH AT	B2	53	3 27	6	2	1	05		
+	2 COMBINED AT	COMBB	87	3 23	10	2	2	08		
	HYDROGRAPH AT									

					EX-10 OH1			
+		C	40	3 20	4	1	1	03
	HYDROGRAPH AT							
+		D	41	3 23	4	1	1	03
	HYDROGRAPH AT							
+		E1	62	3 30	8	2	2	06
	HYDROGRAPH AT							
+		E2	62	3 20	6	1	1	05
	2 COMBINED AT							
+		COMBE	117	3 23	14	3	3	11
	6 COMBINED AT							
+		CLEAR	438	3 23	47	12	9	37
	HYDROGRAPH AT							
+		E3	13	3 13	1	0	0	01
	HYDROGRAPH AT							
+		F1	45	3 23	5	1	1	04
	HYDROGRAPH AT							
+		F2	20	3 20	2	0	0	01
	HYDROGRAPH AT							
+		F3	18	3 20	2	0	0	01
	HYDROGRAPH AT							
+		G	18	3 27	2	1	0	02
	HYDROGRAPH AT							
+		H1	65	3 27	8	2	2	06
	HYDROGRAPH AT							
+		H2	74	3 33	9	2	2	07
	HYDROGRAPH AT							
+		I	33	3 23	3	1	1	03
	8 COMBINED AT							
+		CLEAR	267	3 27	31	8	6	24
	HYDROGRAPH AT							
+		J	41	3 13	3	1	1	02
	HYDROGRAPH AT							
+		K	19	3 10	1	0	0	01
	HYDROGRAPH AT							
+		L	16	3 10	1	0	0	01
	HYDROGRAPH AT							
+		M	13	3 13	1	0	0	01
	HYDROGRAPH AT							
+		N	17	3 13	1	0	0	01
	HYDROGRAPH AT							
+		O	6	3 10	0	0	0	00
	HYDROGRAPH AT							
+		P	12	3 13	1	0	0	01
	8 COMBINED AT							
+		CLEAR	341	3 23	40	10	8	31
	HYDROGRAPH AT							
+		Q	6	3 10	0	0	0	00
	HYDROGRAPH AT							
+		R	216	3 20	19	5	4	15

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION PEAK	INTERVAL TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1	STORM AREA (SQ MI) =			01					
A1	MANE	1 84	107 02	194 53	1 19	2 00	106 46	194 00	1 19

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5392E+01 OUTFLOW= 5353E+01 BASIN STORAGE= 1364E-02 PERCENT ERROR= 7

FOR STORM = 1	STORM AREA (SQ MI) =			01					
A2	MANE	1 75	50 71	191 79	1 20	2 00	50 52	192 00	1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2311E+01 OUTFLOW= 2297E+01 BASIN STORAGE= 5304E-03 PERCENT ERROR= 6

FOR STORM = 1	STORM AREA (SQ MI) =			01					
B1	MANE	2 00	39 12	191 38	1 20	2 00	38 66	192 00	1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1733E+01 OUTFLOW= 1724E+01 BASIN STORAGE= 3053E-03 PERCENT ERROR= 5

FOR STORM = 1	STORM AREA (SQ MI) =			01					
B2	MANE	1 74	53 59	197 45	1 20	2 00	53 26	196 00	1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3209E+01 OUTFLOW= 3189E+01 BASIN STORAGE= 1515E-02 PERCENT ERROR= 6

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 C MANE 1 47 40 57 192 64 1 20 2 00 40 37 192 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1926E+01 OUTFLOW= 1915E+01 BASIN STORAGE= 4279E-03 PERCENT ERROR= 5

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 D MANE 1 48 40 88 194 62 1 20 2 00 40 61 194 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2182E+01 OUTFLOW= 2171E+01 BASIN STORAGE= 6675E-03 PERCENT ERROR= 5

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E1 MANE 1 90 62 27 198 97 1 20 2 00 61 72 198 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3915E+01 OUTFLOW= 3896E+01 BASIN STORAGE= 1974E-02 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E2 MANE 2 00 62 46 191 99 1 20 2 00 62 45 192 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2953E+01 OUTFLOW= 2935E+01 BASIN STORAGE= 5918E-03 PERCENT ERROR= 6

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E3 MANE 85 13 38 188 41 1 20 2 00 13 30 188 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5135E+00 OUTFLOW= 5118E+00 BASIN STORAGE= 6173E-04 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F1 MANE 1 92 44 93 195 61 1 20 2 00 44 61 194 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2311E+01 OUTFLOW= 2295E+01 BASIN STORAGE= 5646E-03 PERCENT ERROR= 6

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F2 MANE 1 34 19 98 191 29 1 20 2 00 19 66 192 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 8986E+00 OUTFLOW= 8943E+00 BASIN STORAGE= 1658E-03 PERCENT ERROR= 5

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F3 MANE 96 17 90 191 29 1 20 2 00 17 72 192 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 8344E+00 OUTFLOW= 8306E+00 BASIN STORAGE= 1816E-03 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 G MANE 80 17 64 196 31 1 20 2 00 17 63 196 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1027E+01 OUTFLOW= 1021E+01 BASIN STORAGE= 4573E-03 PERCENT ERROR= 6

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 H1 MANE 1 72 65 74 196 70 1 20 2 00 65 49 196 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3787E+01 OUTFLOW= 3769E+01 BASIN STORAGE= 1393E-02 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 H2 MANE 2 00 74 37 199 20 1 20 2 00 73 61 200 00 1 19

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4621E+01 OUTFLOW= 4590E+01 BASIN STORAGE= 2307E-02 PERCENT ERROR= 6

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 I MANE 1 77 33 15 193 81 1 20 2 00 32 93 194 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1605E+01 OUTFLOW= 1595E+01 BASIN STORAGE= 3572E-03 PERCENT ERROR= 6

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 J MANE 81 41 44 187 95 1 20 2 00 41 39 188 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1540E+01 OUTFLOW= 1535E+01 BASIN STORAGE= 1760E-03 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 K MANE 84 18 86 185 93 1 20 2 00 18 81 186 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5777E+00 OUTFLOW= 5767E+00 BASIN STORAGE= 2665E-04 PERCENT ERROR= 2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 L MANE 57 16 78 187 06 1 20 2 00 16 44 186 00 1 20



EX-10 OH1

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5777E+00 OUTFLOW= 5758E+00 BASIN STORAGE= 4539E-04 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) = 01  
M MANE 1 04 13 56 188 38 1 20 2 00 13 42 188 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5135E+00 OUTFLOW= 5117E+00 BASIN STORAGE= 6097E-04 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) = 01  
N MANE 1 44 17 49 188 21 1 20 2 00 17 32 188 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 6419E+00 OUTFLOW= 6397E+00 BASIN STORAGE= 6706E-04 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) = 01  
O MANE 35 5 62 185 78 1 20 2 00 5 60 186 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1861E+00 OUTFLOW= 1856E+00 BASIN STORAGE= 1265E-04 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) = 01  
P MANE 53 12 32 187 06 1 20 2 00 12 07 188 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4429E+00 OUTFLOW= 4413E+00 BASIN STORAGE= 4819E-04 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) = 01  
Q MANE 25 6 56 184 90 1 20 2 00 6 43 186 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2054E+00 OUTFLOW= 2049E+00 BASIN STORAGE= 1025E-04 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) = 01  
R MANE 1 86 217 34 191 54 1 20 2 00 216 26 192 00 1 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9628E+01 OUTFLOW= 9581E+01 BASIN STORAGE= 1564E-02 PERCENT ERROR= 5

\*\*\* NORMAL END OF HEC-1 \*\*\*

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4 1 *
* RUN DATE 04MAY06 TIME 13 31 20 *
*****
    
```

```

*****
* U S ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

```

X X XXXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXXX XXXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW  
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE  
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81 THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS WRITE STAGE FREQUENCY,  
 DSS READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	HEC-1 MODEL FOR SERENO CANYON									
2	ID	100-YEAR, 6-HOUR STORM									
3	ID	RAINFALL FROM NOAA ATLAS									
4	ID	SCS CURVE NUMBER SOIL LOSS PARAMETERS									
5	ID	KINEMATIC WAVE HYDROGRAPH ROUTING									
6	ID	EXISTING CONDITIONS									
7	ID										
8	ID	PREPARED BY WOOD/PATEL, 8 4 2004									
9	ID	FILE NAME EX-100 DAT									
10	ID										
11	ID	*DIAGRAM									
12	IT	2			900						
13	JO	5									
14	JD		01								
14	PH			73	1 43	2 42	2 74	2 96	3 37		
15	KK	A1									
16	KM	RUNOFF FROM SUB-BASIN A1									
17	BA	084									
18	LS		88								
19	UK	260	025	0 15	100						
20	RK	3520	023	032		TRAP	12	5			
21	KO					22					
22	KK	A2									
23	KM	RUNOFF FROM SUB-BASIN A2									
24	BA	036									
25	LS		88								
26	UK	235	025	0 15	100						
27	RK	2000	02	032		TRAP	8	5			
28	KO					22					
29	KK	B1									
30	KM	RUNOFF FROM SUB-BASIN B1									
31	BA	027									
32	LS		88								
33	UK	200	025	0 15	100						
34	RK	2500	028	032		TRAP	8	5			
35	KO					22					
36	KK	B2									
37	KM	RUNOFF FROM SUB-BASIN B2									
38	BA	050									
39	LS		88								
40	UK	400	025	0 15	100						
41	RK	2340	024	032		TRAP	8	5			
42	KK	COMBB									
43	KM	COMBINE SECTIONS B1 AND B2									
44	HC	2									
45	KO										

1

HEC-1 INPUT

PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
46	KK	C									
47	KM	RUNOFF FROM SUB-BASIN C									
48	BA	03									
49	LS		88								
50	UK	250	023	0 15	100						
51	RK	1570	017	032		TRAP	5	5			
52	KO					22					
53	KK	D									
54	KM	RUNOFF FROM SUB-BASIN D									
55	BA	034									
56	LS		88								

57 UK 300 02 0 15 100 EX-100 OH1  
 58 RK 1425 014 032 TRAP 22 6 5  
 59 KO  
 60 KK E1  
 61 KM RUNOFF FROM SUB-BASIN E1  
 62 BA 061  
 63 LS 88  
 64 UK 400 02 0 15 100  
 65 RK 2050 013 032 TRAP 22 7 5  
 66 KO  
 67 KK E2  
 68 KM RUNOFF FROM SUB-BASIN E2  
 69 BA 046  
 70 LS 88  
 71 UK 200 02 0 15 100  
 72 RK 2370 013 032 TRAP 22 10 5  
 73 KO  
 74 KK COMBE  
 75 KM COMBINE SECTIONS E1 AND E2  
 76 HC 2  
 77 KO 22  
 78 KK CLEAR  
 79 KM CLEAR HYDROGRAPH STACK  
 80 HC 6\*  
 81 KK E3  
 82 KM RUNOFF FROM SUB-BASIN E3  
 83 BA 008  
 84 LS 88  
 85 UK 175 025 0 15 100  
 86 RK 750 032 TRAP 22 6 5  
 87 KO HEC-1 INPUT

PAGE 3

1 LINE ID 1 2 3 4 5 6 7 8 9 10

88 KK F1  
 89 KM RUNOFF FROM SUB-BASIN F1  
 90 BA 036  
 91 LS 88  
 92 UK 225 018 0 15 100  
 93 RK 2500 012 TRAP 22 7 5  
 94 KO  
 95 KK F2  
 96 KM RUNOFF FROM SUB-BASIN F2  
 97 BA 014  
 98 LS 88  
 99 UK 200 018 0 15 100  
 100 RK 1440 028 TRAP 22 6 5  
 101 KO  
 102 KK F3  
 103 KM RUNOFF FROM SUB-BASIN F3  
 104 BA 013  
 105 LS 88  
 106 UK 225 018 0 15 100  
 107 RK 850 026 TRAP 22 6 5  
 108 KO  
 109 KK G  
 110 KM RUNOFF FROM SUB-BASIN G  
 111 BA 0 016  
 112 LS 88  
 113 UK 400 025 0 15 100  
 114 RK 720 022 TRAP 22 4 5  
 115 KO  
 116 KK H1  
 117 KM RUNOFF FROM SUB-BASIN H1  
 118 BA 059  
 119 LS 88  
 120 UK 375 025 0 15 100  
 121 RK 2210 019 TRAP 22 8 5  
 122 KO  
 123 KK H2  
 124 KM RUNOFF FROM SUB-BASIN H2  
 125 BA 072  
 126 LS 88  
 127 UK 400 025 0 15 100  
 128 RK 3480 019 TRAP 22 12 5  
 129 KO  
 130 KK I  
 131 KM RUNOFF FROM SUB-BASIN I  
 132 BA .025  
 133 LS 88  
 134 UK 225 02 0 15 100  
 135 RK 2100 026 TRAP 22 10 5

PAGE 4

1 LINE ID 1 2 3 4 5 6 7 8 9 10

136 KO 22  
 137 KK CLEAR  
 138 KM CLEAR HYDROGRAPH STACK  
 139 HC 8

140	KK	J									
141	KM	RUNOFF	FROM	SUB-BASIN	J						
142	BA	024									
143	LS		88	0		100					
144	UK	145	02	0 15							
145	RK	965	026	032			TRAP	5		5	
146	KO						22				
147	KK	K									
148	KM	RUNOFF	FROM	SUB-BASIN	K						
149	BA	009									
150	LS		88	0		100					
151	UK	81	025	0 15							
152	RK	715	022	032			TRAP	5		5	
153	KO						22				
154	KK	L									
155	KM	RUNOFF	FROM	SUB-BASIN	L						
156	BA	009									
157	LS		88	0		100					
158	UK	113	018	0 15							
159	RK	450	012	032			TRAP	5		5	
160	KO						22				
161	KK	M									
162	KM	RUNOFF	FROM	SUB-BASIN	M						
163	BA	008									
164	LS		88	0		100					
165	UK	137	018	0 15							
166	RK	750	012	032			TRAP	4		5	
167	KO						22				
168	KK	N									
169	KM	RUNOFF	FROM	SUB-BASIN	N						
170	BA	01									
171	LS		88	0		100					
172	UK	117	018	0 15							
173	RK	1036	012	032			TRAP	5		5	
174	KO						22				
175	KK	O									
176	KM	RUNOFF	FROM	SUB-BASIN	O						
177	BA	0029									
178	LS		88	0		100					
179	UK	108	02	0 15							
180	RK	174	014	032			TRAP	4		5	
181	KO						22				

1

HEC-1 INPUT

PAGE 5

LINE ID 1 2 3 4 5 6 7 8 9 10

182	KK	P									
183	KM	RUNOFF	FROM	SUB-BASIN	P						
184	BA	0069									
185	LS		88	0		100					
186	UK	135	02	0 15							
187	RK	354	014	032			TRAP	5		5	
188	KO						22				
189	KK	CLEAR									
190	KM	CLEAR	HYDROGRAPH	STACK							
191	HC	8									
192	KK	Q									
193	KM	RUNOFF	FROM	SUB-BASIN	Q						
194	BA	0032									
195	LS		88	0		100					
196	UK	86	02	0 15							
197	RK	160	014	032			TRAP	4		5	
198	KO						22				
199	KK	R									
200	KM	RUNOFF	FROM	SUB-BASIN	R						
201	BA	15									
202	LS		88	0		100					
203	UK	180	025	0 15							
204	RK	4300	023	032			TRAP	15		5	
205	KO						22				
206	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW  
 NO ( ) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

15 A1

22 A2

29 B1

36 B2

42 COMBB

46 C

53 D Page 3

EX-100 OH1

```

60                                     E1
67                                     E2
74                                     COMBE
78 CLEAR
81                                     E3
88                                     F1
95                                     F2
102                                    F3
109                                     G
116                                    H1
123                                    H2
130                                    I
137 CLEAR
140                                     J
147                                     K
154                                     L
161                                     M
168                                     N
175                                     O
182                                     P
189 CLEAR
192                                     Q
199                                     R

```

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   JUN 1998 *
* VERSION 4 1 *
* RUN DATE 04MAY06 TIME 13 31 20 *
*****

```

```

*****
* U S ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

HEC-1 MODEL FOR SERENO CANYON  
100-YEAR, 6-HOUR STORM  
RAINFALL FROM NOAA ATLAS  
SCS CURVE NUMBER SOIL LOSS PARAMETERS  
KINEMATIC WAVE HYDROGRAPH ROUTING  
EXISTING CONDITIONS

PREPARED BY WOOD/PATEL, 8 4 2004  
FILE NAME EX-100 DAT

```

12 IO OUTPUT CONTROL VARIABLES
      IPRNT 5 PRINT CONTROL
      IPLOT 0 PLOT CONTROL
      QSCAL 0 HYDROGRAPH PLOT SCALE

```

```

IT HYDROGRAPH TIME DATA
    NMIN 2 MINUTES IN COMPUTATION INTERVAL
    IDATE 1 0 STARTING DATE
    ITIME 0000 STARTING TIME
    NQ 900 NUMBER OF HYDROGRAPH ORDINATES

```

NDDATE 2 0 ENDING DATE  
NDTIME 0558 ENDING TIME  
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 03 HOURS  
TOTAL TIME BASE 29 97 HOURS

ENGLISH UNITS  
DRAINAGE AREA SQUARE MILES  
PRECIPITATION DEPTH INCHES  
LENGTH, ELEVATION FEET  
FLOW CUBIC FEET PER SECOND  
STORAGE VOLUME ACRE-FEET  
SURFACE AREA ACRES  
TEMPERATURE DEGREES FAHRENHEIT

13 JD INDEX STORM NO 1  
STRM 3 37 PRECIPITATION DEPTH  
TRDA 01 TRANSPOSITION DRAINAGE AREA

14 PI PRECIPITATION PATTERN

00	00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00	00
00	00	01	01	01	01	01	01	01	01	01
01	01	01	01	01	01	01	01	01	01	01
01	01	01	01	01	01	01	01	01	01	01
01	01	01	01	01	01	01	01	01	01	01
01	01	01	01	01	01	03	03	03	03	03
04	04	04	06	06	07	09	12	17	29	29
29	24	15	11	07	07	06	05	04	04	04
04	03	03	03	03	01	01	01	01	01	01
01	01	01	01	01	01	01	01	01	01	01
01	01	01	01	01	01	01	01	01	01	01
01	01	01	01	01	01	01	01	01	01	01
01	01	01	01	01	01	01	01	00	00	00
00	00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00	00

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15 KK \* A1 \*  
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21 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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22 KK \* A2 \*  
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28 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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29 KK \* B1 \*  
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35 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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\* \*  
42 KK \* COMBB \*  
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45 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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46 KK \* C \*  
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52 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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53 KK \* D \*  
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59 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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60 KK \* E1 \*  
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66 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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67 KK \* E2 \*  
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73 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED

ISAV2 900 LAST ORDINATE PUNCHED OR SAVED EX-100 OH1  
TIMINT 033 TIME INTERVAL IN HOURS

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\* \*  
74 KK \* COMBE \*  
\* \*  
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77 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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81 KK \* E3 \*  
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87 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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88 KK \* F1 \*  
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94 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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\* \*  
95 KK \* F2 \*  
\* \*  
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101 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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\* \*  
102 KK \* F3 \*  
\* \*  
\*\*\*\*\*

108 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL



QSCAL	0	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	033	TIME INTERVAL IN HOURS

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* *
109 KK *   G *
* *
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115 KO   OUTPUT CONTROL VARIABLES
        IPRNT      5  PRINT CONTROL
        IPLOT      0  PLOT CONTROL
        QSCAL      0  HYDROGRAPH PLOT SCALE
        IPNCH      0  PUNCH COMPUTED HYDROGRAPH
        IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2     900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT     033 TIME INTERVAL IN HOURS

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* *
116 KK *  H1 *
* *
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122 KO   OUTPUT CONTROL VARIABLES
        IPRNT      5  PRINT CONTROL
        IPLOT      0  PLOT CONTROL
        QSCAL      0  HYDROGRAPH PLOT SCALE
        IPNCH      0  PUNCH COMPUTED HYDROGRAPH
        IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2     900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT     033 TIME INTERVAL IN HOURS

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*****
* *
123 KK *  H2 *
* *
*****

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129 KO   OUTPUT CONTROL VARIABLES
        IPRNT      5  PRINT CONTROL
        IPLOT      0  PLOT CONTROL
        QSCAL      0  HYDROGRAPH PLOT SCALE
        IPNCH      0  PUNCH COMPUTED HYDROGRAPH
        IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2     900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT     033 TIME INTERVAL IN HOURS

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*****
* *
130 KK *   I *
* *
*****

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136 KO   OUTPUT CONTROL VARIABLES
        IPRNT      5  PRINT CONTROL
        IPLOT      0  PLOT CONTROL
        QSCAL      0  HYDROGRAPH PLOT SCALE
        IPNCH      0  PUNCH COMPUTED HYDROGRAPH
        IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2     900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT     033 TIME INTERVAL IN HOURS

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*****
* *
140 KK *   J *
* *
*****

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146 KO            OUTPUT CONTROL VARIABLES  
 IPRNT            5    PRINT CONTROL  
 IPLOT            0    PLOT CONTROL  
 QSCAL            0    HYDROGRAPH PLOT SCALE  
 IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
 IOUT             22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
 TIMINT           033    TIME INTERVAL IN HOURS

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 \*            \*  
 147 KK       \*            K       \*  
 \*            \*  
 \*\*\*\*\*

153 KO            OUTPUT CONTROL VARIABLES  
 IPRNT            5    PRINT CONTROL  
 IPLOT            0    PLOT CONTROL  
 QSCAL            0    HYDROGRAPH PLOT SCALE  
 IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
 IOUT             22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
 TIMINT           033    TIME INTERVAL IN HOURS

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 \*            \*  
 154 KK       \*            L       \*  
 \*            \*  
 \*\*\*\*\*

160 KO            OUTPUT CONTROL VARIABLES  
 IPRNT            5    PRINT CONTROL  
 IPLOT            0    PLOT CONTROL  
 QSCAL            0    HYDROGRAPH PLOT SCALE  
 IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
 IOUT             22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
 TIMINT           033    TIME INTERVAL IN HOURS

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 \*            \*  
 161 KK       \*            M       \*  
 \*            \*  
 \*\*\*\*\*

167 KO            OUTPUT CONTROL VARIABLES  
 IPRNT            5    PRINT CONTROL  
 IPLOT            0    PLOT CONTROL  
 QSCAL            0    HYDROGRAPH PLOT SCALE  
 IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
 IOUT             22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
 TIMINT           033    TIME INTERVAL IN HOURS

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 \*            \*  
 168 KK       \*            N       \*  
 \*            \*  
 \*\*\*\*\*

174 KO            OUTPUT CONTROL VARIABLES  
 IPRNT            5    PRINT CONTROL  
 IPLOT            0    PLOT CONTROL  
 QSCAL            0    HYDROGRAPH PLOT SCALE  
 IPNCH            0    PUNCH COMPUTED HYDROGRAPH  
 IOUT             22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1            1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2            900    LAST ORDINATE PUNCHED OR SAVED  
 TIMINT           033    TIME INTERVAL IN HOURS

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175 KK \* \*  
\* O \*  
\* \*  
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181 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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182 KK \* \*  
\* P \*  
\* \*  
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188 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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192 KK \* \*  
\* Q \*  
\* \*  
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198 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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199 KK \* \*  
\* R \*  
\* \*  
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205 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

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RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

+	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	A1	225	3 17	19	5	4	08		
+	HYDROGRAPH AT	A2	103	3 13	8	2	2	04		
+	HYDROGRAPH AT	B1	79	3 13	6	2	1	03		
+	HYDROGRAPH AT	B2	117	3 20	11	3	2	05		
+	2 COMBINED AT	COMBB	192	3 17	18	4	4	08		
	HYDROGRAPH AT									

					EX-100 OH1			
+		C	85	3 13	7	2	1	03
	HYDROGRAPH AT							
+		D	88	3 17	8	2	2	03
	HYDROGRAPH AT							
+		E1	136	3 20	14	4	3	06
	HYDROGRAPH AT							
+		E2	129	3 17	11	3	2	05
	2 COMBINED AT							
+		COMBE	258	3 17	25	6	5	11
	6 COMBINED AT							
+		CLEAR	948	3 17	84	21	17	37
	HYDROGRAPH AT							
+		E3	26	3 10	2	0	0	01
	HYDROGRAPH AT							
+		F1	93	3 17	8	2	2	04
	HYDROGRAPH AT							
+		F2	42	3 13	3	1	1	01
	HYDROGRAPH AT							
+		F3	38	3 13	3	1	1	01
	HYDROGRAPH AT							
+		G	39	3 17	4	1	1	02
	HYDROGRAPH AT							
+		H1	143	3 17	13	3	3	06
	HYDROGRAPH AT							
+		H2	161	3 23	16	4	3	07
	HYDROGRAPH AT							
+		I	70	3 17	6	1	1	03
	8 COMBINED AT							
+		CLEAR	584	3 17	56	14	11	24
	HYDROGRAPH AT							
+		J	81	3 10	6	1	1	02
	HYDROGRAPH AT							
+		K	34	3 07	2	1	0	01
	HYDROGRAPH AT							
+		L	31	3 10	2	1	0	01
	HYDROGRAPH AT							
+		M	27	3 10	2	0	0	01
	HYDROGRAPH AT							
+		N	34	3 10	2	1	0	01
	HYDROGRAPH AT							
+		O	11	3 07	1	0	0	00
	HYDROGRAPH AT							
+		P	24	3 10	2	0	0	01
	8 COMBINED AT							
+		CLEAR	758	3 17	72	18	14	31
	HYDROGRAPH AT							
+		Q	12	3 07	1	0	0	00
	HYDROGRAPH AT							
+		R	429	3 17	34	9	7	15

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO		VOLUME (IN)
							COMPUTATION PEAK (CFS)	INTERVAL TIME TO PEAK (MIN)	
FOR STORM = 1	STORM AREA (SQ MI) =			01					
A1	MANE	1 85	225 20	189 38	2 14	2 00	224 80	190 00	2 14
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9633E+01 OUTFLOW= 9604E+01 BASIN STORAGE= 1456E-02 PERCENT ERROR= 3									
FOR STORM = 1	STORM AREA (SQ MI) =			01					
A2	MANE	1 35	104 77	188 43	2 14	2 00	103 27	188 00	2 15
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4129E+01 OUTFLOW= 4118E+01 BASIN STORAGE= 4516E-03 PERCENT ERROR= 2									
FOR STORM = 1	STORM AREA (SQ MI) =			01					
B1	MANE	1 60	79 81	189 36	2 15	2 00	78 60	188 00	2 14
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3096E+01 OUTFLOW= 3089E+01 BASIN STORAGE= 2797E-03 PERCENT ERROR= 2									
FOR STORM = 1	STORM AREA (SQ MI) =			01					
B2	MANE	1 48	118 10	190 89	2 14	2 00	117 30	192 00	2 14

EX-100 OH1

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5734E+01 OUTFLOW= 5701E+01 BASIN STORAGE= 1520E-02 PERCENT ERROR= 5  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 C MANE 1 23 85 59 188 20 2 15 2 00 84 94 188 00 2 14

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3440E+01 OUTFLOW= 3432E+01 BASIN STORAGE= 4680E-03 PERCENT ERROR= 2  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 D MANE 1 11 88 51 190 19 2 14 2 00 88 33 190 00 2 14

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3899E+01 OUTFLOW= 3887E+01 BASIN STORAGE= 7476E-03 PERCENT ERROR= 3  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E1 MANE 1 51 136 89 192 60 2 14 2 00 136 17 192 00 2 14

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 6996E+01 OUTFLOW= 6958E+01 BASIN STORAGE= 2145E-02 PERCENT ERROR= 5  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E2 MANE 1 81 130 97 189 38 2 15 2 00 128 84 190 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5275E+01 OUTFLOW= 5263E+01 BASIN STORAGE= 5523E-03 PERCENT ERROR= 2  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E3 MANE 67 26 55 186 39 2 15 2 00 26 42 186 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9174E+00 OUTFLOW= 9155E+00 BASIN STORAGE= 6540E-04 PERCENT ERROR= 2  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F1 MANE 2 00 95 18 190 56 2 14 2 00 93 24 190 00 2 14

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4129E+01 OUTFLOW= 4114E+01 BASIN STORAGE= 5693E-03 PERCENT ERROR= 3  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F2 MANE 1 06 41 56 187 91 2 15 2 00 41 51 188 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1606E+01 OUTFLOW= 1602E+01 BASIN STORAGE= 1682E-03 PERCENT ERROR= 2  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F3 MANE 68 37 67 188 19 2 15 2 00 37 65 188 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1491E+01 OUTFLOW= 1487E+01 BASIN STORAGE= 1820E-03 PERCENT ERROR= 2  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 G MANE 68 38 57 190 59 2 14 2 00 38 55 190 00 2 14

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1835E+01 OUTFLOW= 1825E+01 BASIN STORAGE= 4585E-03 PERCENT ERROR= 5  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 H1 MANE 1 45 144 51 190 20 2 14 2 00 143 37 190 00 2 14

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 6766E+01 OUTFLOW= 6737E+01 BASIN STORAGE= 1486E-02 PERCENT ERROR= 4  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 H2 MANE 2 00 162 43 193 50 2 14 2 00 160 92 194 00 2 14

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 8257E+01 OUTFLOW= 8205E+01 BASIN STORAGE= 2345E-02 PERCENT ERROR= 6  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 I MANE 1 51 70 53 189 63 2 14 2 00 69 68 190 00 2 14

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2867E+01 OUTFLOW= 2858E+01 BASIN STORAGE= 4040E-03 PERCENT ERROR= 3  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 J MANE 63 81 49 185 81 2 15 2 00 81 34 186 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2752E+01 OUTFLOW= 2749E+01 BASIN STORAGE= 1872E-03 PERCENT ERROR= 1  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 K MANE 69 34 56 184 48 2 15 2 00 34 08 184 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1032E+01 OUTFLOW= 1032E+01 BASIN STORAGE= 2351E-04 PERCENT ERROR= 0  
 FOR STORM = 1 STORM AREA (SQ MI) = 01  
 L MANE 58 32 05 185 30 2 15 2 00 31 42 186 00 2 15

EX-100 OH1

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1032E+01 OUTFLOW= 1030E+01 BASIN STORAGE= 5207E-04 PERCENT ERROR= 2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
M MANE 86 26 78 186 51 2 15 2 00 26 60 186 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9174E+00 OUTFLOW= 9164E+00 BASIN STORAGE= 6575E-04 PERCENT ERROR= 1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
N MANE 1 22 33 96 186 87 2 15 2 00 33 62 186 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1147E+01 OUTFLOW= 1146E+01 BASIN STORAGE= 5856E-04 PERCENT ERROR= 1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
O MANE 25 10 68 184 44 2 15 2 00 10 55 184 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3326E+00 OUTFLOW= 3320E+00 BASIN STORAGE= 1358E-04 PERCENT ERROR= .2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
P MANE 42 24 13 185 54 2 15 2 00 23 71 186 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 7913E+00 OUTFLOW= 7897E+00 BASIN STORAGE= 4642E-04 PERCENT ERROR= 2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
Q MANE 28 12 25 184 34 2 15 2 00 12 20 184 00 2 15

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3670E+00 OUTFLOW= 3665E+00 BASIN STORAGE= 1048E-04 PERCENT ERROR= 1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
R MANE 1 93 438 22 189 49 2 14 2 00 429 09 190 00 2 14

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1720E+02 OUTFLOW= 1715E+02 BASIN STORAGE= 1546E-02 PERCENT ERROR= 3

\*\*\* NORMAL END OF HEC-1 \*\*\*

## APPENDIX C

### Proposed Condition Hydrologic Calculations

- 2-year HEC-1 Model
- 10-year HEC-1 Model
- 100-year HEC-1 Model

1\*\*\*\*\*  
 \* FLOOD HYDROGRAPH PACKAGE (HEC-1) \*  
 \* JUN 1998 \*  
 \* VERSION 4 1 \*  
 \* RUN DATE 08MAY06 TIME 09 29 20 \*  
 \*\*\*\*\*

\*\*\*\*\*  
 \* U S ARMY CORPS OF ENGINEERS \*  
 \* HYDROLOGIC ENGINEERING CENTER \*  
 \* 609 SECOND STREET \*  
 \* DAVIS, CALIFORNIA 95616 \*  
 \* (916) 756-1104 \*  
 \*\*\*\*\*

```

X   X XXXXXXXX XXXXX   X
X   X X       X   X   XX
X   X X       X       X
XXXXXXXX XXXX   X   XXXXX X
X   X X       X       X
X   X X       X   X   X
X   X XXXXXXXX XXXXX   XXX
  
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW  
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE  
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81 THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS WRITE STAGE FREQUENCY,  
 DSS READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE NEW FINITE DIFFERENCE ALGORITHM

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	HEC-1 MODEL FOR SERENO CANYON									
2	ID	2-YEAR, 6-HOUR STORM									
3	ID	RAINFALL FROM NOAA ATLAS									
4	ID	SCS CURVE NUMBER SOIL LOSS PARAMETERS									
5	ID	KINEMATIC WAVE HYDROGRAPH ROUTING									
6	ID	DEVELOPED CONDITIONS									
7	ID	10% IMPERVIOUS AREA USED FOR SITE									
8	ID										
9	ID	PREPARED BY WOOD/PATEL, 1 10 2005									
10	ID	FILE NAME DEV-100 DAT									
11	ID										
12	IT	2			900						
13	IO	5									
14	JD		01								
15	PH			0 38	0 7	1 12	1 27	1 38	1 57		
16	KK	A1									
17	KM	RUNOFF FROM SUB-BASIN A1									
18	BA	0 084									
19	LS		88	1 1							
20	UK	260	025	0 15	100						
21	RK	3520	023	032		TRAP	12	5			
22	KK	BASA1									
23	KM	29' WEIR	AT HEADWALL								
24	RS	1	STOR	0							
25	SV	0	151	342	0 574	0 807	1 04				
26	SE	0	1	2	3	4	5				
27	SQ	0	0	0	0	81	230				
28	KK	A2									
29	KM	RUNOFF FROM SUB-BASIN A2									
30	BA	036									
31	LS		88	7 4							
32	UK	235	025	0 15	100						
33	RK	2000	02	032		TRAP	8	5			
34	KK	BASA2	-5								
35	KM	14' WEIR	AT OUTLET								
36	RS	1	STOR	0							
37	SV	0	0 125	0 273	0 448	0 623	0 798				
38	SE	2772	2773	2774	2775	2776	2777				
39	SQ	0	0	0	0	39	111				
40	KK	B1									
41	KM	RUNOFF FROM SUB-BASIN B1									
42	BA	027									
43	LS		88	3 7							
44	UK	200	025	0 15	100						
45	RK	2500	028	032		TRAP	8	5			

LINE	ID	1	2	3	4	5	6	7	8	9	10
46	KK	BASB	-3								
47	KM	11' WEIR	AT HEADWALL								
48	RS	1	STOR	0							
49	SV	0	089	0 2	338	0 49	0 643				
50	SE	2781	2782	2783	2784	2785	2786				
51	SQ	0	0	0	0	31	87				
52	KK	B2									
53	KM	RUNOFF FROM SUB-BASIN B2									



54 BA 050  
 55 LS 88 6 7  
 56 UK 400 025 0 15 100  
 57 RK 2340 024 032 TRAP 8 5  
 \*

DEV-2 OH1

58 KK COMBB  
 59 KM COMBINE SECTIONS B1 AND B2  
 60 HC 2  
 \*

61 KK BASB -5  
 62 KM 23 WEIR  
 63 RS 1 STOR 0  
 64 SV 0 21 459 750 1 04 1 33  
 65 SE 2746 2747 2748 2749 2750 2751  
 66 SQ 0 0 0 0 64 182  
 \*

67 KK C  
 68 KM RUNOFF FROM SUB-BASIN C  
 69 BA 03  
 70 LS 88 5 9  
 71 UK 250 023 0 15 100  
 72 RK 1570 017 032 TRAP 5 5  
 \*

73 KK BASC -1  
 74 KM 11 WEIR  
 75 RS 1 STOR 0  
 76 SV 0 107 236 388 540 0 745  
 77 SE 2779 2780 2781 2782 2783 2784  
 78 SQ 0 0 0 0 31 87  
 \*

79 KK D  
 80 KM RUNOFF FROM SUB-BASIN D  
 81 BA 034  
 82 LS 88 10  
 83 UK 300 02 0 15 100  
 84 RK 1425 014 032 TRAP 6 5  
 \*

HEC-1 INPUT

PAGE 3

1 LINE ID 1 2 3 4 5 6 7 8 9 10

85 KK BASD-1  
 86 KM 12 WEIR AT HEAD WALL  
 87 RS 1 STOR 0  
 88 SV 0 086 131 227 324 0 472  
 89 SE 2749 2750 2751 2752 2753 2754  
 90 SQ 0 0 0 0 34 95  
 \*

91 KK BASD -2  
 92 KM 10 WEIR AT HEAD WALL  
 93 RS 1 STOR 0  
 94 SV 0 097 218 364 0 51 0 657  
 95 SE 2724 2725 2726 2727 2728 2729  
 96 SQ 0 0 0 0 28 79  
 \*

97 KK E1 5  
 98 KM RUNOFF FROM SUB-BASIN E1  
 99 BA 061  
 100 LS 88 10  
 101 UK 400 02 0 15 100  
 102 RK 2050 013 032 TRAP 7 5  
 \*

103 KK BASE1- 2  
 104 KM 19 WEIR AT HEADWALL  
 105 RS 1 STOR 0  
 106 SV 0 181 396 648 0 9 1 152  
 107 SE 2716 2717 2718 2719 2720 2721  
 108 SQ 0 0 0 0 53 150  
 \*

109 KK BASE1- 3  
 110 KM 18 WEIR AT HEAD WALL  
 111 RS 1 STOR 0  
 112 SV 0 138 316 532 747 963  
 113 SE 2695 2696 2697 2698 2699 2700  
 114 SQ 0 0 0 0 50 143  
 \*

115 KK E2  
 116 KM RUNOFF FROM SUB-BASIN E2  
 117 BA 046  
 118 LS 88 10  
 119 UK 200 02 0 15 100  
 120 RK 2370 013 032 TRAP 10 5  
 \*

121 KK BASE2  
 122 KM 18 WEIR AT HEAD WALL  
 123 RS 1 STOR 0  
 124 SV 0 176 385 632 919 1 205  
 125 SE 2694 2695 2696 2697 2698 2699  
 126 SQ 0 0 0 0 50 143  
 \*

HEC-1 INPUT

PAGE 4

1 LINE ID 1 2 3 4 5 6 7 8 9 10

127 KK COMBE  
 128 KM COMBINE SECTIONS E1 AND E2  
 129 HC 2

130 KK CLEAR  
 131 KM CLEAR HYDROGRAPH STACK  
 132 HC 6  
 \*

133 KK E3  
 134 KM RUNOFF FROM SUB-BASIN E3  
 135 BA 008  
 136 LS 88 10  
 137 UK 175 025 0 15 100  
 138 RK 750 032 032 TRAP 6 5  
 \*

139 KK BASE3  
 140 KM 10' WEIR AT HEAD WALL  
 141 RS 1 STOR 0  
 142 SV 0 082 188  
 143 SE 2711 2712 2713 2714 0 456 2715  
 144 SQ 0 0 0 0 28  
 \*

145 KK F1  
 146 KM RUNOFF FROM SUB-BASIN F1  
 147 BA 036  
 148 LS 88 10  
 149 UK 225 018 0 15 100  
 150 RK 2500 012 032 TRAP 7 5  
 \*

151 KK BASF1  
 152 KM 12' WEIR AT HEAD WALL  
 153 RS 1 STOR 0  
 154 SV 0 132 293  
 155 SE 2696 2697 2698 2699 0 706 2700 2701  
 156 SQ 0 0 0 0 34 95  
 \*

157 KK F2  
 158 KM RUNOFF FROM SUB-BASIN F2  
 159 BA 014  
 160 LS 88 10  
 161 UK 200 018 0 15 100  
 162 RK 1440 028 032 TRAP 6 5  
 \*

163 KK BASF2  
 164 KM 16' WEIR AT HEAD WALL  
 165 RS 1 STOR 0  
 166 SV 0 055 135  
 167 SE 2701 2702 2703 2704 0 345 2705  
 168 SQ 0 0 0 0 45  
 \*

169 KK F3  
 170 KM RUNOFF FROM SUB-BASIN F3  
 171 BA 013  
 172 LS 88 10  
 173 UK 225 018 0 15 100  
 174 RK 850 026 032 TRAP 6 5  
 \*

175 KK BASF3  
 176 KM 14' WEIR AT HEAD WALL  
 177 RS 1 STOR 0  
 178 SV 0 055 133  
 179 SE 2706 2707 2708 2709 0 378 2710  
 180 SQ 0 0 0 0 39  
 \*

181 KK G  
 182 KM RUNOFF FROM SUB-BASIN G  
 183 BA 0 016  
 184 LS 88 10  
 185 UK 400 025 0 15 100  
 186 RK 720 022 032 TRAP 4 5  
 \*

187 KK BASG  
 188 KM 14' WEIR AT HEAD WALL  
 189 RS 1 STOR 0  
 190 SV 0 0 159 356  
 191 SE 2723 2724 2725 2726 0 828 2727  
 192 SQ 0 0 0 0 39  
 \*

193 KK H1  
 194 KM RUNOFF FROM SUB-BASIN H1  
 195 BA 059  
 196 LS 88 9 4  
 197 UK 375 025 0 15 100  
 198 RK 2210 019 032 TRAP 8 5  
 \*

199 KK BASH1-  
 200 KM 20' WEIR  
 201 RS 1 STOR 0  
 202 SV 0 169 363  
 203 SE 2735 2736 2737 2738 0 807 2739 2740  
 204 SQ 0 0 0 0 56 158  
 HEC-1 INPUT

1

LINE ID 1 2 3 4 5 6 7 8 9 10

1

LINE ID 1 2 3 4 5 6 7 8 9 10  
 Page 3

DEV-2 OH1

205 KK BASH1- 3  
 206 KM 19' WEIR  
 207 RS 1 STOR 0  
 208 SV 0 086 192 321 0 474 0 627  
 209 SE 2736 2737 2738 2739 2740 2741  
 210 SQ 0 0 0 0 53 150

211 KK BASH1- 5  
 212 KM 19' WEIR  
 213 RS 1 STOR 0  
 214 SV 0 219 467 747 1 026 1 306  
 215 SE 2727 2728 2729 2730 2731 2732  
 216 SQ 0 0 0 0 53 150  
 \*

217 KK H2  
 218 KM RUNOFF FROM SUB-BASIN H2  
 219 BA 072  
 220 LS 88 4 78  
 221 UK 400 025 0 15 100  
 222 RK 3480 019 032 TRAP 12 5

223 KK BASH2- 1  
 224 KM 21' WEIR AT HEADWALL  
 225 RS 1 STOR 0  
 226 SV 0 175 382 624 905 1 185  
 227 SE 0 2772 2773 2774 2775 2776  
 228 SQ 0 0 0 0 59 166

229 KK BASH2- 2  
 230 KM 21' WEIR AT HEADWALL  
 231 RS 1 STOR 0  
 232 SV 0 165 361 590 820 1 049  
 233 SE 0 2785 2786 2787 2788 2789  
 234 SQ 0 0 0 0 59 166  
 \*

235 KK I  
 236 KM RUNOFF FROM SUB-BASIN I  
 237 BA 025  
 238 LS 88 7 4  
 239 UK 225 02 0 15 100  
 240 RK 2100 026 032 TRAP 10 5  
 \*

241 KK BASI  
 242 KM 10' WEIR  
 243 RS 1 STOR 0  
 244 SV 0 186 399 640 0 881 1 122  
 245 SE 2755 2756 2757 2758 2759 2760  
 246 SQ 0 0 0 0 28 79  
 \*

HEC-1 INPUT

PAGE 7

1 LINE ID 1 2 3 4 5 6 7 8 9 10

247 KK CLEAR  
 248 KM CLEAR HYDROGRAPH STACK  
 249 HC 8  
 \*

250 KK J  
 251 KM RUNOFF FROM SUB-BASIN J  
 252 BA 024  
 253 LS 88 7 9  
 254 UK 145 02 0 15 100  
 255 RK 965 026 032 TRAP 5 5  
 \*

256 KK BASJ  
 257 KM 11' WEIR  
 258 RS 1 STOR 0  
 259 SV 0 098 217 359 528 0 698  
 260 SE 2769 2770 2771 2772 2773 2774  
 261 SQ 0 0 0 0 31 87  
 \*

262 KK K  
 263 KM RUNOFF FROM SUB-BASIN K  
 264 BA 009  
 265 LS 88 10  
 266 UK 81 025 0 15 100  
 267 RK 715 022 032 TRAP 5 5  
 \*

268 KK BASK  
 269 KM 13' WEIR  
 270 RS 1 STOR 0  
 271 SV 0 106 232 378 0 525  
 272 SE 2737 2738 2739 2740 2741  
 273 SQ 0 0 0 0 36  
 \*

274 KK L  
 275 KM RUNOFF FROM SUB-BASIN L  
 276 BA 009  
 277 LS 88 10  
 278 UK 113 018 0 15 100  
 279 RK 450 012 032 TRAP 5 5  
 \*

280 KK BASL  
 281 KM 12' WEIR

282 RS 1 STOR 0  
 283 SV 0 035 096 188 0 28  
 284 SE 2722 2723 2724 2725 2726  
 285 SQ 0 0 0 0 34

DEV-2 OH1

1 HEC-1 INPUT PAGE 8

LINE ID 1 2 3 4 5 6 7 8 9 10

286 KK M  
 287 KM RUNOFF FROM SUB-BASIN M  
 288 BA 008  
 289 LS 88 10  
 290 UK 137 018 0 15 100  
 291 RK 750 012 032 TRAP 4 5

292 KK BASM  
 293 KM 10 WEIR  
 294 RS 1 STOR 0  
 295 SV 0 069 160 278 0 396  
 296 SE 2721 2722 2723 2724 2725  
 297 SQ 0 0 0 0 28

298 KK N  
 299 KM RUNOFF FROM SUB-BASIN N  
 300 BA 01  
 301 LS 88 10  
 302 UK 117 018 0 15 100  
 303 RK 1036 012 032 TRAP 5 5

304 KK BASN  
 305 KM 13 WEIR  
 306 RS 1 STOR 0  
 307 SV 0 11 25 422 0 594  
 308 SE 2691 2692 2693 2694 2695  
 309 SQ 0 0 0 0 36

310 KK O  
 311 KM RUNOFF FROM SUB-BASIN O  
 312 KM Area of Basin subtrating the area to be retained for 100 year event  
 313 BA 0 0024 9  
 314 LS 88 10  
 315 UK 108 02 0 15 100  
 316 RK 174 014 032 TRAP 4 5

317 KK P  
 318 KM RUNOFF FROM SUB-BASIN P  
 319 KM Area of Basin subtrating the area to be retained for 100 year event  
 320 BA 0058  
 321 LS 88 10  
 322 UK 135 02 0 15 100  
 323 RK 354 014 032 TRAP 5 5

324 KK CLEAR  
 325 KM CLEAR HYDROGRAPH STACK  
 326 HC 8

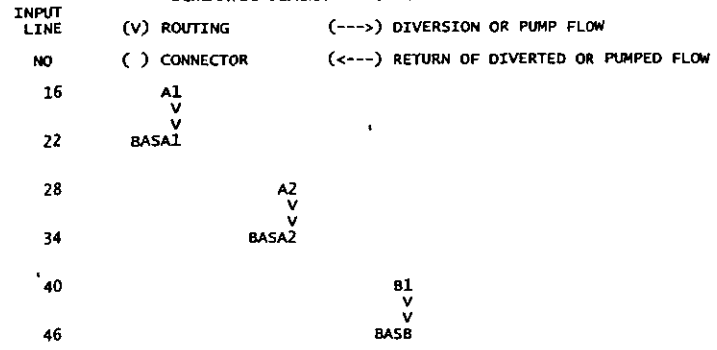
1 HEC-1 INPUT PAGE 9

LINE ID 1 2 3 4 5 6 7 8 9 10

327 KK Q  
 328 KM RUNOFF FROM SUB-BASIN Q  
 329 KM Area of Basin subtrating the area to be retained for 100 year event  
 330 BA 0024 9  
 331 LS 88 10  
 332 UK 86 02 0 15 100  
 333 RK 160 014 032 TRAP 4 5

334 KK R  
 335 KM RUNOFF FROM SUB-BASIN R  
 336 KM Area of Basin subtrating the area to be retained for 100 year event  
 337 BA 14 8  
 338 LS 88 14  
 339 UK 180 025 0 15 100  
 340 RK 4300 023 032 TRAP 15 5  
 341 ZZ

1 SCHEMATIC DIAGRAM OF STREAM NETWORK



DEV-2 OH1

52

B2

58

COMBB  
V  
V  
BASB

61

67

C  
V  
V  
BASC

73

79

D  
V  
V  
BASD-1  
V  
V  
BASD

85

91

E1  
V  
V  
BASE1-  
V  
V  
BASE1-

97

103

109

115

E2  
V  
V  
BASE2

121

127

COMBE

130

CLEAR

133

E3  
V  
V  
BASE3

139

145

F1  
V  
V  
BASF1

151

157

F2  
V  
V  
BASF2

163

169

F3  
V  
V  
BASF3

175

181

G  
V  
V  
BASG

187

193

H1  
V  
V  
BASH1-  
V  
V  
BASH1-  
V  
V  
BASH1-

199

205

211

217

H2  
V  
V  
BASH2-  
V  
V  
BASH2-

223

229

235

I  
V  
V  
BASI

241

247

CLEAR

DEV-2 OH1

250

J  
V  
V  
BASJ

256

262

K  
V  
V  
BASK

268

274

L  
V  
V  
BASL

280

286

M  
V  
V  
BASM

292

298

N  
V  
V  
BASN

304

310

O

317

P

324

CLEAR

327

Q

334

R

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4 1 *
* RUN DATE 08MAY06 TIME 09 29 20 *
*****

```

```

*****
* U S ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

HEC-1 MODEL FOR SERENO CANYON  
 100-YEAR, 6-HOUR STORM  
 RAINFALL FROM NOAA ATLAS  
 SCS CURVE NUMBER SOIL LOSS PARAMETERS  
 KINEMATIC WAVE HYDROGRAPH ROUTING  
 DEVELOPED CONDITIONS  
 10% IMPERVIOUS AREA USED FOR SITE  
 PREPARED BY WOOD/PATEL, 1 10 2005  
 FILE NAME DEV-100 DAT

13 IO

OUTPUT CONTROL VARIABLES

IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0 HYDROGRAPH PLOT SCALE

IT

HYDROGRAPH TIME DATA

NMIN 2 MINUTES IN COMPUTATION INTERVAL  
 IDATE 1 0 STARTING DATE  
 ITIME 0000 STARTING TIME  
 NQ 900 NUMBER OF HYDROGRAPH ORDINATES  
 NDDATE 2 0 ENDING DATE  
 NDTIME 0558 ENDING TIME  
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 03 HOURS  
 TOTAL TIME BASE 29 97 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES  
 PRECIPITATION DEPTH INCHES  
 LENGTH, ELEVATION FEET  
 FLOW CUBIC FEET PER SECOND  
 STORAGE VOLUME ACRE-FEET  
 SURFACE AREA ACRES  
 TEMPERATURE DEGREES FAHRENHEIT

14 JD

INDEX STORM NO 1

STRM 1 57 PRECIPITATION DEPTH  
 TRDA 01 TRANSPOSITION DRAINAGE AREA

15 PI

PRECIPITATION PATTERN

00	00	00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00	00	00



DEV-2 OH1

+	ROUTED TO	BASF1	12	3 57	2	0	0	04	2699 37	3 57
+	HYDROGRAPH AT	F2	10	3 23	1	0	0	01		
+	ROUTED TO	BASF2	5	3 50	1	0	0	01	2704 11	3 50
+	HYDROGRAPH AT	F3	9	3 27	1	0	0	01		
+	ROUTED TO	BASF3	4	3 53	1	0	0	01	2709 10	3 53
+	HYDROGRAPH AT	G	8	3 33	1	0	0	02		
+	ROUTED TO	BASG	0	7 03	0	0	0	02	2726 00	7 00
+	HYDROGRAPH AT	H1	31	3 37	4	1	1	06		
+	ROUTED TO	BASH1-	26	3 50	3	1	1	06	2738 46	3 50
+	ROUTED TO	BASH1-	19	3 63	3	1	1	06	2739 36	3 63
+	ROUTED TO	BASH1-	4	4 57	1	0	0	06	2730 08	4 57
+	HYDROGRAPH AT	H2	33	3 43	5	1	1	07		
+	ROUTED TO	BASH2-	27	3 60	4	1	1	07	2774 45	3 60
+	ROUTED TO	BASH2-	16	3 90	3	1	1	07	2787 26	3 90
+	HYDROGRAPH AT	I	16	3 30	2	0	0	03		
+	ROUTED TO	BASI	2	4 20	1	0	0	03	2758 08	4 20
+	8 COMBINED AT	CLEAR	26	3 87	7	2	2	24		
+	HYDROGRAPH AT	J	20	3 17	2	0	0	02		
+	ROUTED TO	BASJ	9	3 40	1	0	0	02	2772 28	3 40
+	HYDROGRAPH AT	K	10	3 13	1	0	0	01		
+	ROUTED TO	BASK	0	00	0	0	0	01	2739 79	23 63
+	HYDROGRAPH AT	L	8	3 13	1	0	0	01		
+	ROUTED TO	BASL	3	3 47	0	0	0	01	2725 07	3 47
+	HYDROGRAPH AT	M	7	3 17	1	0	0	01		
+	ROUTED TO	BASM	0	5 43	0	0	0	01	2724 01	5 40
+	HYDROGRAPH AT	N	9	3 20	1	0	0	01		
+	ROUTED TO	BASN	0	00	0	0	0	01	2693 79	29 40
+	HYDROGRAPH AT	O	2	3 13	0	0	0	00		
+	HYDROGRAPH AT	P	5	3 17	0	0	0	01		
+	8 COMBINED AT	CLEAR	32	3 53	9	2	2	31		
+	HYDROGRAPH AT									





	H1	MANE	2 18	31 15	201 14	DEV-2 OH1 72	2 00	31 08	202 00	72	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2267E+01 OUTFLOW= 2255E+01 BASIN STORAGE= 1559E-02 PERCENT ERROR= 5										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	H2	MANE	2 00	32.86	206 02	67	2 00	32 85	206 00	67	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2600E+01 OUTFLOW= 2587E+01 BASIN STORAGE= 2161E-02 PERCENT ERROR= 4										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	I	MANE	2 00	15 82	197 38	70	2 00	15 64	198 00	70	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9357E+00 OUTFLOW= 9317E+00 BASIN STORAGE= 3884E-03 PERCENT ERROR= 4										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	J	MANE	1 03	19 95	190 49	70	2 00	19 75	190 00	70	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9043E+00 OUTFLOW= 9008E+00 BASIN STORAGE= 1637E-03 PERCENT ERROR= 4										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	K	MANE	97	10 24	187 13	72	2 00	9 96	188 00	72	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3485E+00 OUTFLOW= 3477E+00 BASIN STORAGE= 2342E-04 PERCENT ERROR= 2										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	L	MANE	70	8 57	188 86	72	2 00	8 40	188 00	72	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3485E+00 OUTFLOW= 3475E+00 BASIN STORAGE= 4365E-04 PERCENT ERROR= 3										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	M	MANE	1 27	6 79	191 05	72	2 00	6 67	190 00	72	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3098E+00 OUTFLOW= 3086E+00 BASIN STORAGE= 5717E-04 PERCENT ERROR= 4										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	N	MANE	1 55	8 92	190 45	72	2 00	8 79	192 00	72	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3873E+00 OUTFLOW= 3862E+00 BASIN STORAGE= 6205E-04 PERCENT ERROR= 3										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	O	MANE	36	2 41	188 39	72	2 00	2 40	188 00	72	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9295E-01 OUTFLOW= 9265E-01 BASIN STORAGE= 1053E-04 PERCENT ERROR= 3										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	P	MANE	67	5 20	189 25	72	2 00	5 17	190 00	72	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2246E+00 OUTFLOW= 2237E+00 BASIN STORAGE= 3648E-04 PERCENT ERROR= 4										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	Q	MANE	35	2 65	187 27	72	2 00	2 62	186 00	72	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9295E-01 OUTFLOW= 9267E-01 BASIN STORAGE= 7453E-05 PERCENT ERROR= 3										
	FOR STORM = 1	STORM AREA (SQ MI) =		01							
	R	MANE	1 91	83 84	197 65	63	2 00	83 28	198 00	63	
	CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4732E+01 OUTFLOW= 4696E+01 BASIN STORAGE= 1577E-02 PERCENT ERROR= 7										

\*\*\* NORMAL END OF HEC-1 \*\*\*

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4 1
* RUN DATE 05MAY06 TIME 08 37 11
*****
    
```

```

*****
* U S ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*****
    
```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW  
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE  
 THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81 THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS WRITE STAGE FREQUENCY,  
 DSS READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE NEW FINITE DIFFERENCE ALGORITHM

1

LINE	ID	1	2	3	4	5	6	7	8	9	10	
1	ID	HEC-1 MODEL FOR SERENO CANYON										
2	ID	10-YEAR, 6-HOUR STORM										
3	ID	RAINFALL FROM NOAA ATLAS										
4	ID	SCS CURVE NUMBER SOIL LOSS PARAMETERS										
5	ID	KINEMATIC WAVE HYDROGRAPH ROUTING										
6	ID	DEVELOPED CONDITIONS										
7	ID	10% IMPERVIOUS AREA USED FOR SITE										
8	ID											
9	ID	PREPARED BY WOOD/PATEL, 1 10 2005										
10	ID	FILE NAME DEV-100 DAT										
11	ID											
12	IT	2										
13	IO	5										
14	JD		01									
15	PH			0 52	0 99	1 64	1 87	2 01	2 29			
16	KK	A1										
17	KM	RUNOFF FROM SUB-BASIN A1										
18	BA	0 084										
19	LS		88	1 1								
20	UK	260	025	0 15	100							
21	RK	3520	023	032		TRAP	12	5				
22	KK	BASA1										
23	KM	29' WEIR AT HEADWALL										
24	RS	1	STOR	0								
25	SV	0	151	342	0 574	0 807	1 04					
26	SE	0	1	2	3	4	5					
27	SQ	0	0	0	0	81	230					
28	KK	A2										
29	KM	RUNOFF FROM SUB-BASIN A2										
30	BA	036										
31	LS		88	7 4								
32	UK	235	025	0 15	100							
33	RK	2000	02	032		TRAP	8	5				
34	KK	BASA2										
35	KM	14' WEIR AT OUTLET										
36	RS	1	STOR	0								
37	SV	0	0 125	0 273	0 448	0 623	0 798					
38	SE	2772	2773	2774	2775	2776	2777					
39	SQ	0	0	0	0	39	111					
40	KK	B1										
41	KM	RUNOFF FROM SUB-BASIN B1										
42	BA	027										
43	LS		88	3 7								
44	UK	200	025	0 15	100							
45	RK	2500	028	032		TRAP	8	5				

PAGE 2

1

LINE	ID	1	2	3	4	5	6	7	8	9	10
46	KK	BASB									
47	KM	11' WEIR AT HEADWALL									
48	RS	1	STOR	0							
49	SV	0	089	0 2	338	0 49	0 643				
50	SE	2781	2782	2783	2784	2785	2786	2787			
51	SQ	0	0	0	0	31	87				
52	KK	B2									
53	KM	RUNOFF FROM SUB-BASIN B2									

54 BA 050  
 55 LS 88 6 7  
 56 UK 400 025 0 15 100  
 57 RK 2340 024 032 TRAP 8 5  
 \*

DEV-10 OH1

58 KK COMBB  
 59 KM COMBINE SECTIONS B1 AND B2  
 60 HC 2  
 \*

61 KK BASB -5  
 62 KM 23' WEIR  
 63 RS 1 STOR 0  
 64 SV 0 21 459 750 1 04 1 33  
 65 SE 2746 2747 2748 2749 2750 2751  
 66 SQ 0 0 0 0 64 182  
 \*

67 KK C  
 68 KM RUNOFF FROM SUB-BASIN C  
 69 BA 03  
 70 LS 88 5 9  
 71 UK 250 023 0 15 100  
 72 RK 1570 017 032 TRAP 5 5  
 \*

73 KK BASC -1  
 74 KM 11' WEIR  
 75 RS 1 STOR 0  
 76 SV 0 107 236 388 540 0 745  
 77 SE 2779 2780 2781 2782 2783 2784  
 78 SQ 0 0 0 0 31 87  
 \*

79 KK D  
 80 KM RUNOFF FROM SUB-BASIN D  
 81 BA 034  
 82 LS 88 10  
 83 UK 300 02 0 15 100  
 84 RK 1425 014 032 TRAP 6 5  
 \*

HEC-1 INPUT

PAGE 3

1 LINE ID 1 2 3 4 5 6 7 8 9 10

85 KK BASD-1  
 86 KM 12' WEIR AT HEAD WALL  
 87 RS 1 STOR 0  
 88 SV 0 056 131 227 324 0 472  
 89 SE 2749 2750 2751 2752 2753 2754  
 90 SQ 0 0 0 0 34 95  
 \*

91 KK BASD -2  
 92 KM 10' WEIR AT HEAD WALL  
 93 RS 1 STOR 0  
 94 SV 0 097 218 364 0 51 0 657  
 95 SE 2724 2725 2726 2727 2728 2729  
 96 SQ 0 0 0 0 28 79  
 \*

97 KK E1 5  
 98 KM RUNOFF FROM SUB-BASIN E1  
 99 BA 061  
 100 LS 88 10  
 101 UK 400 02 0 15 100  
 102 RK 2050 013 032 TRAP 7 5  
 \*

103 KK BASE1- 2  
 104 KM 19' WEIR AT HEADWALL  
 105 RS 1 STOR 0  
 106 SV 0 181 396 648 0 9 1 152  
 107 SE 2716 2717 2718 2719 2720 2721  
 108 SQ 0 0 0 0 53 150  
 \*

109 KK BASE1- 3  
 110 KM 18' WEIR AT HEAD WALL  
 111 RS 1 STOR 0  
 112 SV 0 138 316 532 747 963  
 113 SE 2695 2696 2697 2698 2699 2700  
 114 SQ 0 0 0 0 50 143  
 \*

115 KK E2  
 116 KM RUNOFF FROM SUB-BASIN E2  
 117 BA 046  
 118 LS 88 10  
 119 UK 200 02 0 15 100  
 120 RK 2370 013 032 TRAP 10 5  
 \*

121 KK BASE2  
 122 KM 18' WEIR AT HEAD WALL  
 123 RS 1 STOR 0  
 124 SV 0 176 385 632 919 1 205  
 125 SE 2694 2695 2696 2697 2698 2699  
 126 SQ 0 0 0 0 50 143  
 \*

HEC-1 INPUT

PAGE 4

1 LINE ID 1 2 3 4 5 6 7 8 9 10

127 KK COMBE  
 128 KM COMBINE SECTIONS E1 AND E2  
 129 HC 2

130 KK CLEAR  
 131 KM CLEAR HYDROGRAPH STACK  
 132 HC 6  
 \*

133 KK E3  
 134 KM RUNOFF FROM SUB-BASIN E3  
 135 BA 008  
 136 LS 88 10  
 137 UK 175 025 0 15 100  
 138 RK 750 032 032 TRAP 6 5  
 \*

139 KK BASE3  
 140 KM 10' WEIR AT HEAD WALL  
 141 RS 1 STOR 0  
 142 SV 0 082 188 322 0 456  
 143 SE 2711 2712 2713 2714 2715  
 144 SQ 0 0 0 0 28  
 \*

145 KK F1  
 146 KM RUNOFF FROM SUB-BASIN F1  
 147 BA 036  
 148 LS 88 10  
 149 UK 225 018 0 15 100  
 150 RK 2500 012 032 TRAP 7 5  
 \*

151 KK BASF1  
 152 KM 12' WEIR AT HEAD WALL  
 153 RS 1 STOR 0  
 154 SV 0 132 293 483 0 706 0 929  
 155 SE 2696 2697 2698 2699 2700 2701  
 156 SQ 0 0 0 0 34 95  
 \*

157 KK F2  
 158 KM RUNOFF FROM SUB-BASIN F2  
 159 BA 014  
 160 LS 88 10  
 161 UK 200 018 0 15 100  
 162 RK 1440 028 032 TRAP 6 5  
 \*

163 KK BASF2  
 164 KM 16' WEIR AT HEAD WALL  
 165 RS 1 STOR 0  
 166 SV 0 055 135 240 0 345  
 167 SE 2701 2702 2703 2704 2705  
 168 SQ 0 0 0 0 45  
 \*

169 KK F3  
 170 KM RUNOFF FROM SUB-BASIN F3  
 171 BA 013  
 172 LS 88 10  
 173 UK 225 018 0 15 100  
 174 RK 850 026 032 TRAP 6 5  
 \*

175 KK BASF3  
 176 KM 14' WEIR AT HEAD WALL  
 177 RS 1 STOR 0  
 178 SV 0 055 133 239 378  
 179 SE 2706 2707 2708 2709 2710  
 180 SQ 0 0 0 0 39  
 \*

181 KK G  
 182 KM RUNOFF FROM SUB-BASIN G  
 183 BA 0 016  
 184 LS 88 10  
 185 UK 400 025 0 15 100  
 186 RK 720 022 032 TRAP 4 5  
 \*

187 KK BASG  
 188 KM 14' WEIR AT HEAD WALL  
 189 RS 1 STOR 0  
 190 SV 0 0 159 356 592 0 828  
 191 SE 2723 2724 2725 2726 2727  
 192 SQ 0 0 0 0 39  
 \*

193 KK H1  
 194 KM RUNOFF FROM SUB-BASIN H1  
 195 BA 059  
 196 LS 88 9 4  
 197 UK 375 025 0 15 100  
 198 RK 2210 019 032 TRAP 8 5  
 \*

199 KK BASH1-  
 200 KM 20' WEIR  
 201 RS 1 STOR 0  
 202 SV 0 169 363 585 0 807 1 029  
 203 SE 2735 2736 2737 2738 2739 2740  
 204 SQ 0 0 0 0 56 158  
 \*

1 LINE ID 1 2 3 4 5 6 7 8 9 10  
 HEC-1 INPUT  
 Page 3

205	KK	BASH1-	3						
206	KM	19' WEIR							
207	RS	1	STOR	0					
208	SV	0	086	192	321	0 474	0 627		
209	SE	2736	2737	2738	2739	2740	2741		
210	SQ	0	0	0	0	53	150		

211	KK	BASH1-	5						
212	KM	19' WEIR							
213	RS	1	STOR	0					
214	SV	0	219	467	747	1 026	1 306		
215	SE	2727	2728	2729	2730	2731	2732		
216	SQ	0	0	0	0	53	150		

217	KK	H2							
218	KM	RUNOFF FROM SUB-BASIN H2							
219	BA	072							
220	LS		88	4 78					
221	UK	400	025	0 15	100				
222	RK	3480	019	032		TRAP	12	5	

223	KK	BASH2-	1						
224	KM	21' WEIR AT HEADWALL							
225	RS	1	STOR	0					
226	SV	0	175	382	624	905	1 185		
227	SE	0	2772	2773	2774	2775	2776		
228	SQ	0	0	0	0	59	166		

229	KK	BASH2-	2						
230	KM	21' WEIR AT HEADWALL							
231	RS	1	STOR	0					
232	SV	0	165	361	590	820	1 049		
233	SE	0	2785	2786	2787	2788	2789		
234	SQ	0	0	0	0	59	166		

235	KK	I							
236	KM	RUNOFF FROM SUB-BASIN I							
237	BA	025							
238	LS		88	7 4					
239	UK	225	02	0 15	100				
240	RK	2100	026	032		TRAP	10	5	

241	KK	BASI							
242	KM	10' WEIR							
243	RS	1	STOR	0					
244	SV	0	186	399	640	0 881	1 122		
245	SE	2755	2756	2757	2758	2759	2760		
246	SQ	0	0	0	0	28	79		

1

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
247	KK	CLEAR									
248	KM	CLEAR HYDROGRAPH STACK									
249	HC	8									
250	KK	J									
251	KM	RUNOFF FROM SUB-BASIN J									
252	BA	024									
253	LS		88	7 9							
254	UK	145	02	0 15	100						
255	RK	965	026	032		TRAP	5	5			
256	KK	BASJ									
257	KM	11' WEIR									
258	RS	1	STOR	0							
259	SV	0	098	217	359	528	0 698				
260	SE	2769	2770	2771	2772	2773	2774				
261	SQ	0	0	0	0	31	87				
262	KK	K									
263	KM	RUNOFF FROM SUB-BASIN K									
264	BA	009									
265	LS		88	10							
266	UK	81	025	0 15	100						
267	RK	715	022	032		TRAP	5	5			
268	KK	BASK									
269	KM	13' WEIR									
270	RS	1	STOR	0							
271	SV	0	106	232	378	0 525					
272	SE	2737	2738	2739	2740	2741					
273	SQ	0	0	0	0	36					
274	KK	L									
275	KM	RUNOFF FROM SUB-BASIN L									
276	BA	009									
277	LS		88	10							
278	UK	113	018	0 15	100						
279	RK	450	012	032		TRAP	5	5			
280	KK	BASL									
281	KM	12' WEIR									

282 RS 1 STOR 0  
 283 SV 0 035 096 188 0 28  
 284 SE 2722 2723 2724 2725 2726  
 285 SQ 0 0 0 0 34

DEV-10 OH1

1

HEC-1 INPUT

PAGE 8

LINE	ID	1	2	3	4	5	6	7	8	9	10
286	KK	M									
287	KM	RUNOFF FROM SUB-BASIN M									
288	BA	008									
289	LS		88	10							
290	UK	137	018	0 15	100						
291	RK	750	012	032		TRAP	4	5			
292	KK	BASM									
293	KM	10' WEIR									
294	RS	1	STOR	0							
295	SV	0	069	160	278	0 396					
296	SE	2721	2722	2723	2724	2725					
297	SQ	0	0	0	0	28					
298	KK	N									
299	KM	RUNOFF FROM SUB-BASIN N									
300	BA	01									
301	LS		88	10							
302	UK	117	018	0 15	100						
303	RK	1036	012	032		TRAP	5	5			
304	KK	BASN									
305	KM	13' WEIR									
306	RS	1	STOR	0							
307	SV	0	11	25	422	0 594					
308	SE	2691	2692	2693	2694	2695					
309	SQ	0	0	0	0	36					

310	KK	0									
311	KM	RUNOFF FROM SUB-BASIN O									
312	KM	Area of Basin subtrating									the area to be retained for 100 year event
313	BA	0 0024	9								
314	LS		88	10							
315	UK	108	02	0 15	100						
316	RK	174	014	032		TRAP	4	5			
317	KK	P									
318	KM	RUNOFF FROM SUB-BASIN P									
319	KM	Area of Basin subtrating									the area to be retained for 100 year event
320	BA	0058									
321	LS		88	10							
322	UK	135	02	0 15	100						
323	RK	354	014	032		TRAP	5	5			
324	KK	CLEAR									
325	KM	CLEAR HYDROGRAPH STACK									
326	HC	8									

1

HEC-1 INPUT

PAGE 9

LINE	ID	1	2	3	4	5	6	7	8	9	10
327	KK	Q									
328	KM	RUNOFF FROM SUB-BASIN Q									
329	KM	Area of Basin subtrating									the area to be retained for 100 year event
330	BA	0024									
331	LS		88	10							
332	UK	86	02	0 15	100						
333	RK	160	014	032		TRAP	4	5			
334	KK	R									
335	KM	RUNOFF FROM SUB-BASIN R									
336	KM	Area of Basin subtrating									the area to be retained for 100 year event
337	BA	14	8								
338	LS		88	14							
339	UK	180	025	0 15	100						
340	RK	4300	023	032		TRAP	15	5			
341	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW  
 NO ( ) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

16 A1  
 V  
 BASA1

28 A2  
 V  
 BASA2

40 B1  
 V  
 BASB

52  
58  
61  
67  
73  
79  
85  
91  
97  
103  
109  
115  
121  
127  
130  
133  
139  
145  
151  
157  
163  
169  
175  
181  
187  
193  
199  
205  
211  
217  
223  
229  
235  
241  
247

B2

COMBB  
V  
V  
BASS

C  
V  
V  
BASC

D  
V  
V  
BASD-1  
V  
V  
BASD

E1  
V  
V  
BASE1-  
V  
V  
BASE1-

E2  
V  
V  
BASE2

COMBE

CLEAR

E3  
V  
V  
BASE3

F1  
V  
V  
BASF1

F2  
V  
V  
BASF2

F3  
V  
V  
BASF3

G  
V  
V  
BASG

H1  
V  
V  
BASH1-  
V  
V  
BASH1-  
V  
V  
BASH1-

H2  
V  
V  
BASH2-  
V  
V  
BASH2-

I  
V  
V  
BAS I

CLEAR



DEV-10 OH1

250  
256  
262  
268  
274  
280  
286  
292  
298  
304  
310  
317  
324  
327  
334

J  
V  
V  
BASJ

K  
V  
V  
BASK

L  
V  
V  
BASL

M  
V  
V  
BASM

N  
V  
V  
BASN

O

P

CLEAR

Q

R

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION  
1\*\*\*\*\*  
\* FLOOD HYDROGRAPH PACKAGE (HEC-1) \*  
\* JUN 1998 \*  
\* VERSION 4 1 \*  
\* RUN DATE 05MAY06 TIME 08 37 11 \*  
\*\*\*\*\*

\*\*\*\*\*  
\* U S ARMY CORPS OF ENGINEERS \*  
\* HYDROLOGIC ENGINEERING CENTER \*  
\* 609 SECOND STREET \*  
\* DAVIS, CALIFORNIA 95616 \*  
\* (916) 756-1104 \*  
\*\*\*\*\*

HEC-1 MODEL FOR SERENO CANYON  
100-YEAR, 6-HOUR STORM  
RAINFALL FROM NOAA ATLAS  
SCS CURVE NUMBER SOIL LOSS PARAMETERS  
KINEMATIC WAVE HYDROGRAPH ROUTING  
DEVELOPED CONDITIONS  
10% IMPERVIOUS AREA USED FOR SITE  
  
PREPARED BY WOOD/PATEL, 1 10 2005  
FILE NAME DEV-100 DAT

13 IO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
  
IT HYDROGRAPH TIME DATA  
NMIN 2 MINUTES IN COMPUTATION INTERVAL  
IDATE 1 0 STARTING DATE  
ITIME 0000 STARTING TIME  
NQ 900 NUMBER OF HYDROGRAPH ORDINATES  
NDDATE 2 0 ENDING DATE  
NDTIME 0558 ENDING TIME  
ICENT 19 CENTURY MARK  
  
COMPUTATION INTERVAL 03 HOURS  
TOTAL TIME BASE 29 97 HOURS

ENGLISH UNITS  
DRAINAGE AREA SQUARE MILES  
PRECIPITATION DEPTH INCHES  
LENGTH, ELEVATION FEET  
FLOW CUBIC FEET PER SECOND  
STORAGE VOLUME ACRE-FEET  
SURFACE AREA ACRES  
TEMPERATURE DEGREES FAHRENHEIT

14 JD INDEX STORM NO 1  
STRM 2 29 PRECIPITATION DEPTH  
TRDA 01 TRANSPOSITION DRAINAGE AREA

15 PI PRECIPITATION PATTERN  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00  
00 00 00 00 00 00 00 00 00 00

00	00	00	00	DEV-10	OH1	00	00	00	00
00	00	00	00	00	00	00	01	01	01
01	01	01	01	01	01	01	01	01	01
01	01	01	01	01	01	02	02	02	02
02	03	03	04	04	05	06	08	11	21
21	17	10	08	05	04	04	04	03	02
02	02	02	02	02	01	01	01	01	01
01	01	01	01	01	01	01	01	01	01
01	01	01	01	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00
00	00	00	00	00	00	00	00	00	00

1

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	108	3 23	11	3	2	08		
ROUTED TO	BASA1	105	3 27	10	2	2	08	4 16	3 27
HYDROGRAPH AT	A2	53	3 20	5	1	1	04		
ROUTED TO	BASA2	44	3 27	4	1	1	04	2776 07	3 27
HYDROGRAPH AT	B1	40	3 20	4	1	1	03		
ROUTED TO	BASB	30	3 30	3	1	1	03	2784 96	3 30
HYDROGRAPH AT	B2	58	3 27	7	2	1	05		
2 COMBINED AT	COMBB	87	3 27	10	2	2	08		
ROUTED TO	BASB	77	3 37	8	2	2	08	2750 11	3 37
HYDROGRAPH AT	C	43	3 20	4	1	1	03		
ROUTED TO	BASC	33	3 30	3	1	1	03	2783 04	3 30
HYDROGRAPH AT	D	45	3 23	5	1	1	03		
ROUTED TO	BASD-1	44	3 27	4	1	1	03	2753 16	3 27
ROUTED TO	BASD	34	3 37	4	1	1	03	2728 11	3 37
HYDROGRAPH AT	E1	69	3 30	8	2	2	06		
ROUTED TO	BASE1-	64	3 37	7	2	1	06	2720 11	3 37
ROUTED TO	BASE1-	50	3 50	6	2	1	06	2699 00	3 50
HYDROGRAPH AT	E2	68	3 20	6	2	1	05		
ROUTED TO	BASE2	49	3 33	5	1	1	05	2697 98	3 33
2 COMBINED AT	COMBE	86	3 47	11	3	2	11		
6 COMBINED AT	CLEAR	317	3 40	40	10	8	37		
HYDROGRAPH AT	E3	14	3 13	1	0	0	01		
ROUTED TO	BASE3	3	3 50	0	0	0	01	2714 13	3 50
HYDROGRAPH AT	F1	50	3 23	5	1	1	04		

DEV-10 OH1

+	ROUTED TO	BASF1	37	3 37	4	1	1	04	2700 04	3 37
+	HYDROGRAPH AT	F2	22	3 17	2	0	0	01		
+	ROUTED TO	BASF2	18	3 27	1	0	0	01	2704 40	3 27
+	HYDROGRAPH AT	F3	19	3 17	2	0	0	01		
+	ROUTED TO	BASF3	15	3 27	1	0	0	01	2709 37	3 27
+	HYDROGRAPH AT	G	20	3 23	2	1	0	02		
+	ROUTED TO	BASG	7	3 63	1	0	0	02	2726 17	3 63
+	HYDROGRAPH AT	H1	72	3 27	8	2	2	06		
+	ROUTED TO	BASH1-	69	3 30	7	2	1	06	2739 12	3 30
+	ROUTED TO	BASH1-	63	3 37	6	2	1	06	2740 11	3 37
+	ROUTED TO	BASH1-	37	3 60	5	1	1	06	2730 70	3 60
+	HYDROGRAPH AT	H2	79	3 30	10	2	2	07		
+	ROUTED TO	BASH2-	73	3 37	8	2	2	07	2775 13	3 37
+	ROUTED TO	BASH2-	58	3 50	7	2	1	07	2787 99	3 50
+	HYDROGRAPH AT	I	36	3 20	3	1	1	03		
+	ROUTED TO	BASI	16	3 47	2	1	0	03	2758 55	3 47
+	8 COMBINED AT	CLEAR	152	3 53	23	6	5	24		
+	HYDROGRAPH AT	J	44	3 13	3	1	1	02		
+	ROUTED TO	BASJ	29	3 23	3	1	1	02	2772 93	3 23
+	HYDROGRAPH AT	K	20	3 10	1	0	0	01		
+	ROUTED TO	BASK	4	3 43	1	0	0	01	2740 11	3 43
+	HYDROGRAPH AT	L	18	3 10	1	0	0	01		
+	ROUTED TO	BASL	12	3 20	1	0	0	01	2725 37	3 20
+	HYDROGRAPH AT	M	15	3 13	1	0	0	01		
+	ROUTED TO	BASM	5	3 40	1	0	0	01	2724 17	3 40
+	HYDROGRAPH AT	N	19	3 13	1	0	0	01		
+	ROUTED TO	BASN	4	3 53	1	0	0	01	2694 11	3 53
+	HYDROGRAPH AT	O	5	3 10	0	0	0	00		
+	HYDROGRAPH AT	P	11	3 10	1	0	0	01		
+	8 COMBINED AT	CLEAR	181	3 53	29	7	6	31		
+	HYDROGRAPH AT									

+ Q 5 3 07 0 0 0 00  
+ HYDROGRAPH AT R 201 3 20 18 5 4 14  
1

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO COMPUTATION INTERVAL		VOLUME (IN)
							PEAK (CFS)	TIME TO PEAK (MIN)	
FOR STORM = 1 STORM AREA (SQ MI) =				01					
A1	MANE	1 72	108 55	193 45	1 21	2 00	108 01	194 00	1 21

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5445E+01 OUTFLOW= 5419E+01 BASIN STORAGE= 1398E-02 PERCENT ERROR= 5

FOR STORM = 1 STORM AREA (SQ MI) =				01					
A2	MANE	1 72	54 13	190 95	1 28	2 00	53 34	192 00	1 28

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2465E+01 OUTFLOW= 2454E+01 BASIN STORAGE= 5265E-03 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) =				01					
B1	MANE	2 00	40 63	190 88	1 24	2 00	40 17	192 00	1 24

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1791E+01 OUTFLOW= 1782E+01 BASIN STORAGE= 3254E-03 PERCENT ERROR= 5

FOR STORM = 1 STORM AREA (SQ MI) =				01					
B2	MANE	1 73	58 08	197 19	1 27	2 00	58 01	196 00	1 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3403E+01 OUTFLOW= 3391E+01 BASIN STORAGE= 1518E-02 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) =				01					
C	MANE	1 51	43 22	191 39	1 26	2 00	42 84	192 00	1 26

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2028E+01 OUTFLOW= 2019E+01 BASIN STORAGE= 4990E-03 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) =				01					
D	MANE	1 32	45 23	193 38	1 31	2 00	44 83	194 00	1 31

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2379E+01 OUTFLOW= 2372E+01 BASIN STORAGE= 7570E-03 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) =				01					
E1	MANE	1 88	69 25	196 46	1 31	2 00	68 98	198 00	1 31

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4269E+01 OUTFLOW= 4252E+01 BASIN STORAGE= 2035E-02 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) =				01					
E2	MANE	2 00	67 82	190 94	1 30	2 00	67 80	192 00	1 30

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3219E+01 OUTFLOW= 3196E+01 BASIN STORAGE= 5915E-03 PERCENT ERROR= 7

FOR STORM = 1 STORM AREA (SQ MI) =				01					
E3	MANE	74	14 47	187 88	1 31	2 00	14 43	188 00	1 31

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5598E+00 OUTFLOW= 5586E+00 BASIN STORAGE= 7059E-04 PERCENT ERROR= 2

FOR STORM = 1 STORM AREA (SQ MI) =				01					
F1	MANE	2 00	49 85	193 84	1 31	2 00	49 70	194 00	1 31

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2519E+01 OUTFLOW= 2507E+01 BASIN STORAGE= 5659E-03 PERCENT ERROR= 5

FOR STORM = 1 STORM AREA (SQ MI) =				01					
F2	MANE	1 33	21 77	190 64	1 31	2 00	21 66	190 00	1 31

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9797E+00 OUTFLOW= 9752E+00 BASIN STORAGE= 1820E-03 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) =				01					
F3	MANE	82	19 45	190 00	1 31	2 00	19 45	190 00	1 31

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9098E+00 OUTFLOW= 9062E+00 BASIN STORAGE= 1824E-03 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) =				01					
G	MANE	84	19 78	194 57	1 31	2 00	19 72	194 00	1 31

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1120E+01 OUTFLOW= 1115E+01 BASIN STORAGE= 4378E-03 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) = 01

						DEV-10 OH1						
HI	MANE	1 71	73 19	195 47	1 30	2 00	72 42	196 00		1 30		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4108E+01 OUTFLOW= 4096E+01 BASIN STORAGE= 1496E-02 PERCENT ERROR= 3												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	H2	MANE	1 97	79 42	197 45	1 25	2 00	79 26	198 00	1 25		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4821E+01 OUTFLOW= 4799E+01 BASIN STORAGE= 2063E-02 PERCENT ERROR= 4												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	I	MANE	1 80	36 22	191 38	1 28	2 00	36 00	192 00	1 28		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1712E+01 OUTFLOW= 1704E+01 BASIN STORAGE= 4051E-03 PERCENT ERROR= 4												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	J	MANE	79	44 02	187 79	1 29	2 00	43 80	188 00	1 28		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1650E+01 OUTFLOW= 1647E+01 BASIN STORAGE= 1790E-03 PERCENT ERROR= 2												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	K	MANE	75	20 24	185 09	1 31	2 00	19 86	186 00	1 31		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 6298E+00 OUTFLOW= 6294E+00 BASIN STORAGE= 2209E-04 PERCENT ERROR= 1												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	L	MANE	70	18 01	186 91	1 31	2 00	17 87	186 00	1 31		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 6298E+00 OUTFLOW= 6286E+00 BASIN STORAGE= 5025E-04 PERCENT ERROR= 2												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	M	MANE	1 09	14 81	187 46	1 31	2 00	14 71	188 00	1 31		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5598E+00 OUTFLOW= 5587E+00 BASIN STORAGE= 5786E-04 PERCENT ERROR= 2												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	N	MANE	1 27	18 77	188 39	1 31	2 00	18 69	188 00	1 31		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 6998E+00 OUTFLOW= 6988E+00 BASIN STORAGE= 5759E-04 PERCENT ERROR= 1												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	O	MANE	37	5 03	185 12	1 31	2 00	4 98	186 00	1 31		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1680E+00 OUTFLOW= 1677E+00 BASIN STORAGE= 1040E-04 PERCENT ERROR= 1												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	P	MANE	53	11 16	187 13	1 31	2 00	11 07	186 00	1 31		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4059E+00 OUTFLOW= 4050E+00 BASIN STORAGE= 3361E-04 PERCENT ERROR= 2												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	Q	MANE	33	5 33	184 83	1 31	2 00	5 15	184 00	1 31		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1680E+00 OUTFLOW= 1678E+00 BASIN STORAGE= 7015E-05 PERCENT ERROR= 1												
	FOR STORM = 1	STORM AREA (SQ MI) =		01								
	R	MANE	1 80	202 31	192 29	1 20	2 00	200 54	192 00	1 20		
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 8998E+01 OUTFLOW= 8955E+01 BASIN STORAGE= 1432E-02 PERCENT ERROR= 5												

\*\*\* NORMAL END OF HEC-1 \*\*\*

\*\*\*\*\*  
 \* FLOOD HYDROGRAPH PACKAGE (HEC-1) \*  
 \* JUN 1998 \*  
 \* VERSION 4 1 \*  
 \* RUN DATE 05MAY06 TIME 10 18 38 \*  
 \*\*\*\*\*

\*\*\*\*\*  
 \* U S ARMY CORPS OF ENGINEERS \*  
 \* HYDROLOGIC ENGINEERING CENTER \*  
 \* 609 SECOND STREET \*  
 \* DAVIS, CALIFORNIA 95616 \*  
 \* (916) 756-1104 \*  
 \*\*\*\*\*

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X   X  XXXXXXX  XXXXX      X
X   X  X        X   X      XX
X   X  X        X           X
XXXXXXXX XXXX  X   XXXXX  X
X   X  X        X           X
X   X  X        X   X      X
X   X  XXXXXXX  XXXXX      XXX
  
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW  
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE  
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81 THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS WRITE STAGE FREQUENCY,  
 DSS READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	HEC-1 MODEL FOR SERENO CANYON									
2	ID	100-YEAR, 6-HOUR STORM									
3	ID	RAINFALL FROM NOAA ATLAS									
4	ID	SCS CURVE NUMBER SOIL LOSS PARAMETERS									
5	ID	KINEMATIC WAVE HYDROGRAPH ROUTING									
6	ID	DEVELOPED CONDITIONS									
7	ID	10% IMPERVIOUS AREA USED FOR SITE									
8	ID										
9	ID	PREPARED BY WOOD/PATEL, 1 10 2005									
10	ID	FILE NAME DEV-100 DAT									
11	ID										
12	DIAGRAM										
13	IT	2			900						
14	IO	5									
15	JD		01								
16	PH			73	1 43	2 42	2 74	2 96	3 37		
17	KK	A1									
18	KM	RUNOFF FROM SUB-BASIN A1									
19	BA	0 084									
20	LS		88	1 1							
21	UK	260	025	0 15	100						
22	RK	3520	023	032		TRAP	12	5			
23	KK	BASA1									
24	KM	29	WEIR AT HEADWALL								
25	RS	1	STOR 0								
26	SV	0	151	342	0 574	0 807	1 04				
27	SE	0	1	2	3	4	5				
28	SQ	0	0	0	0	81	230				
29	KO					22					
30	KK	A2									
31	KM	RUNOFF FROM SUB-BASIN A2									
32	BA	036									
33	LS		88	7 4							
34	UK	235	025	0 15	100						
35	RK	2000	02	032		TRAP	8	5			
36	KK	BASA2									
37	KM	14'	WEIR AT OUTLET								
38	RS	1	STOR 0								
39	SV	0	0 125	0 273	0 448	0 623	0 798				
40	SE	2772	2773	2774	2775	2776	2777				
41	SQ	0	0	0	0	39	111				
42	KO					22					
43	KK	B1									
44	KM	RUNOFF FROM SUB-BASIN B1									
45	BA	027									
46	LS		88	3 7							
47	UK	200	025	0 15	100						
48	RK	2500	028	032		TRAP	8	5			

1

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
48	KK	BASB-3									
49	KM	11' WEIR AT HEADWALL									
50	RS	1	STOR 0								
51	SV	0	089	0 2	338	0 49	0 643				
52	SE	2781	2782	2783	2784	2785	2786				
53	SQ	0	0	0	0	31	87				
54	KO					22					

55 KK B2  
 56 KM RUNOFF FROM SUB-BASIN B2  
 57 BA 050  
 58 LS 88 6 7  
 59 UK 400 025 0 15 100  
 60 RK 2340 024 032 TRAP 8 5  
 \*

61 KK COMBB  
 62 KM COMBINE SECTIONS B1 AND B2  
 63 HC 2  
 \*

64 KK BASB-5  
 65 KM 23' WEIR  
 66 RS 1 STOR 0  
 67 SV 0 21 459 750 1 04 1 33  
 68 SE 2746 2747 2748 2749 2750 2751  
 69 SQ 0 0 0 0 64 182  
 70 KO 22  
 \*

71 KK C  
 72 KM RUNOFF FROM SUB-BASIN C  
 73 BA 03  
 74 LS 88 5 9  
 75 UK 250 023 0 15 100  
 76 RK 1570 017 032 TRAP 5 5  
 \*

77 KK BASC -1  
 78 KM 11' WEIR  
 79 RS 1 STOR 0  
 80 SV 0 107 236 388 540 0 745  
 81 SE 2779 2780 2781 2782 2783 2784  
 82 SQ 0 0 0 0 31 87  
 83 KO 22  
 \*

84 KK D  
 85 KM RUNOFF FROM SUB-BASIN D  
 86 BA 034  
 87 LS 88 10  
 88 UK 300 02 0 15 100  
 89 RK 1425 014 032 TRAP 6 5  
 \*

HEC-1 INPUT

1

LINE ID 1 2 3 4 5 6 7 8 9 10

90 KK BASD-1  
 91 KM 12' WEIR AT HEAD WALL  
 92 RS 1 STOR 0  
 93 SV 0 056 131 227 324 0 472  
 94 SE 2749 2750 2751 2752 2753 2754  
 95 SQ 0 0 0 0 34 95  
 \*

96 KK BASD -2  
 97 KM 10' WEIR AT HEAD WALL  
 98 RS 1 STOR 0  
 99 SV 0 097 218 364 0 51 0 657  
 100 SE 2724 2725 2726 2727 2728 2729  
 101 SQ 0 0 0 0 28 79  
 102 KO 22  
 \*

103 KK E1 5  
 104 KM RUNOFF FROM SUB-BASIN E1  
 105 BA 061  
 106 LS 88 10  
 107 UK 400 02 0 15 100  
 108 RK 2050 013 032 TRAP 7 5  
 \*

109 KK BASE1- 2  
 110 KM 19' WEIR AT HEADWALL  
 111 RS 1 STOR 0  
 112 SV 0 181 396 648 0 9 1 152  
 113 SE 2716 2717 2718 2719 2720 2721  
 114 SQ 0 0 0 0 53 150  
 \*

115 KK BASE1- 3  
 116 KM 18' WEIR AT HEAD WALL  
 117 RS 1 STOR 0  
 118 SV 0 138 316 532 747 963  
 119 SE 2695 2696 2697 2698 2699 2700  
 120 SQ 0 0 0 0 50 143  
 121 KO 22  
 \*

122 KK E2  
 123 KM RUNOFF FROM SUB-BASIN E2  
 124 BA 046  
 125 LS 88 10  
 126 UK 200 02 0 15 100  
 127 RK 2370 013 032 TRAP 10 5  
 \*

128 KK BASE2  
 129 KM 18' WEIR AT HEAD WALL  
 130 RS 1 STOR 0  
 131 SV 0 176 385 632 919 1 205  
 132 SE 2694 2695 2696 2697 2698 2699  
 133 SQ 0 0 0 0 50 143  
 134 KO 22  
 \*

1

LINE	ID	1	2	3	4	5	6	7	8	9	10
135	KK	COMBE									
136	KM	COMBINE SECTIONS E1 AND E2									
137	HC	2									
138	KO	22									
139	KK	CLEAR									
140	KM	CLEAR HYDROGRAPH STACK									
141	HC	6									
142	KK	E3									
143	KM	RUNOFF FROM SUB-BASIN E3									
144	BA	008									
145	LS	88 10									
146	UK	175 025 0 15 100									
147	RK	750 032 032 TRAP 6 5									
148	KK	BASE3									
149	KM	10' WEIR AT HEAD WALL									
150	RS	1 STOR 0									
151	SV	0 082 188 322 0 456									
152	SE	2711 2712 2713 2714 2715									
153	SQ	0 0 0 0 28									
154	KO	22									
155	KK	F1									
156	KM	RUNOFF FROM SUB-BASIN F1									
157	BA	036									
158	LS	88 10									
159	UK	225 018 0 15 100									
160	RK	2500 012 032 TRAP 7 5									
161	KK	BASF1									
162	KM	12' WEIR AT HEAD WALL									
163	RS	1 STOR 0									
164	SV	0 132 293 483 0 706 0 929									
165	SE	2696 2697 2698 2699 2700 2701									
166	SQ	0 0 0 0 34 95									
167	KO	22									
168	KK	F2									
169	KM	RUNOFF FROM SUB-BASIN F2									
170	BA	014									
171	LS	88 10									
172	UK	200 018 0 15 100									
173	RK	1440 028 032 TRAP 6 5									
174	KK	BASF2									
175	KM	16' WEIR AT HEAD WALL									
176	RS	1 STOR 0									
177	SV	0 055 135 240 0 345									
178	SE	2701 2702 2703 2704 2705									
179	SQ	0 0 0 0 45 22									
180	KO	22									
181	KK	F3									
182	KM	RUNOFF FROM SUB-BASIN F3									
183	BA	013									
184	LS	88 10									
185	UK	225 018 0 15 100									
186	RK	850 026 032 TRAP 6 5									
187	KK	BASF3									
188	KM	14' WEIR AT HEAD WALL									
189	RS	1 STOR 0									
190	SV	0 055 133 239 378									
191	SE	2706 2707 2708 2709 2710									
192	SQ	0 0 0 0 39 22									
193	KO	22									
194	KK	G									
195	KM	RUNOFF FROM SUB-BASIN G									
196	BA	0 016									
197	LS	88 10									
198	UK	400 025 0 15 100									
199	RK	720 022 032 TRAP 4 5									
200	KK	BASG									
201	KM	14' WEIR AT HEAD WALL									
202	RS	1 STOR 0									
203	SV	0 0 159 356 592 0 828									
204	SE	2723 2724 2725 2726 2727									
205	SQ	0 0 0 0 39 22									
206	KO	22									
207	KK	H1									
208	KM	RUNOFF FROM SUB-BASIN H1									

1



1 209 BA 059 88 9 4  
 210 LS 375 025 0 15 100  
 211 UK 2210 019 032 HEC-1 TRAP 8 5  
 212 RK PAGE 6

LINE ID 1 2 3 4 5 6 7 8 9 10

213 KK BASH1- 4  
 214 KM 20' WEIR  
 215 RS 1 STOR 0  
 216 SV 0 169 363 585 0 807 1 029  
 217 SE 2735 2736 2737 2738 2739 2740  
 218 SQ 0 0 0 0 56 158

219 KK BASH1- 3  
 220 KM 19' WEIR  
 221 RS 1 STOR 0  
 222 SV 0 086 192 321 0 474 0 627  
 223 SE 2736 2737 2738 2739 2740 2741  
 224 SQ 0 0 0 0 53 150

225 KK BASH1- 5  
 226 KM 19' WEIR  
 227 RS 1 STOR 0  
 228 SV 0 219 467 747 1 026 1 306  
 229 SE 2727 2728 2729 2730 2731 2732  
 230 SQ 0 0 0 0 53 150  
 231 KO 22

232 KK H2  
 233 KM RUNOFF FROM SUB-BASIN H2  
 234 BA 072  
 235 LS 88 4 78  
 236 UK 400 025 0 15 100  
 237 RK 3480 019 032 TRAP 12 5

238 KK BASH2- 1  
 239 KM 21' WEIR AT HEADWALL  
 240 RS 1 STOR 0  
 241 SV 0 175 382 624 905 1 185  
 242 SE 0 2772 2773 2774 2775 2776  
 243 SQ 0 0 0 0 59 166

244 KK BASH2- 2  
 245 KM 21' WEIR AT HEADWALL  
 246 RS 1 STOR 0  
 247 SV 0 165 361 590 820 1 049  
 248 SE 0 2785 2786 2787 2788 2789  
 249 SQ 0 0 0 0 59 166  
 250 KO 22

251 KK I  
 252 KM RUNOFF FROM SUB-BASIN I  
 253 BA 025  
 254 LS 88 7 4  
 255 UK 225 02 0 15 100  
 256 RK 2100 026 032 TRAP 10 5

1 HEC-1 INPUT PAGE 7

LINE ID 1 2 3 4 5 6 7 8 9 10

257 KK BASI  
 258 KM 10' WEIR  
 259 RS 1 STOR 0  
 260 SV 0 186 399 640 0 881 1 122  
 261 SE 2755 2756 2757 2758 2759 2760  
 262 SQ 0 0 0 0 28 79  
 263 KO 22

264 KK CLEAR  
 265 KM CLEAR HYDROGRAPH STACK  
 266 HC 8

267 KK J  
 268 KM RUNOFF FROM SUB-BASIN J  
 269 BA 024  
 270 LS 88 7 9  
 271 UK 145 02 0 15 100  
 272 RK 965 026 032 TRAP 5 5

273 KK BASJ  
 274 KM 11' WEIR  
 275 RS 1 STOR 0  
 276 SV 0 098 217 359 528 0 698  
 277 SE 2769 2770 2771 2772 2773 2774  
 278 SQ 0 0 0 0 31 87  
 279 KO 22

280 KK K  
 281 KM RUNOFF FROM SUB-BASIN K  
 282 BA 009  
 283 LS 88 10  
 284 UK 81 025 0 15 100  
 285 RK 715 022 032 TRAP 5 5

286 KK BASK  
 287 KM 13' WEIR  
 288 RS 1 STOR 0  
 289 SV 0 106 232 378 0 525  
 290 SE 2737 2738 2739 2740 2741  
 291 SQ 0 0 0 0 36  
 292 KO 22  
 \*

293 KK L  
 294 KM RUNOFF FROM SUB-BASIN L  
 295 BA 009  
 296 LS 88 10  
 297 UK 113 018 0 15 100  
 298 RK 450 012 032 TRAP 5 5  
 \*

1

HEC-1 INPUT

LINE ID 1 2 3 4 5 6 7 8 9 10

299 KK BASK  
 300 KM 12' WEIR  
 301 RS 1 STOR 0  
 302 SV 0 035 096 188 0 28  
 303 SE 2722 2723 2724 2725 2726  
 304 SQ 0 0 0 0 34  
 305 KO 22  
 \*

306 KK M  
 307 KM RUNOFF FROM SUB-BASIN M  
 308 BA 008  
 309 LS 88 10  
 310 UK 137 018 0 15 100  
 311 RK 750 012 032 TRAP 4 5  
 \*

312 KK BASK  
 313 KM 10' WEIR  
 314 RS 1 STOR 0  
 315 SV 0 069 160 278 0 396  
 316 SE 2721 2722 2723 2724 2725  
 317 SQ 0 0 0 0 28  
 318 KO 22  
 \*

319 KK N  
 320 KM RUNOFF FROM SUB-BASIN N  
 321 BA 01  
 322 LS 88 10  
 323 UK 117 018 0 15 100  
 324 RK 1036 012 032 TRAP 5 5  
 \*

325 KK BASK  
 326 KM 13' WEIR  
 327 RS 1 STOR 0  
 328 SV 0 11 25 422 0 594  
 329 SE 2691 2692 2693 2694 2695  
 330 SQ 0 0 0 0 36  
 331 KO 22  
 \*

332 KK O  
 333 KM RUNOFF FROM SUB-BASIN O  
 334 KM Area of Basin subtrating the area to be retained for 100 year event  
 335 BA 0 0024 9  
 336 LS 88 10  
 337 UK 108 02 0 15 100  
 338 RK 174 014 032 TRAP 4 5  
 339 KO 22  
 \*

1

HEC-1 INPUT

LINE ID 1 2 3 4 5 6 7 8 9 10

340 KK P  
 341 KM RUNOFF FROM SUB-BASIN P  
 342 KM Area of Basin subtrating the area to be retained for 100 year event  
 343 BA 0058 9  
 344 LS 88 10  
 345 UK 135 02 0 15 100  
 346 RK 354 014 032 TRAP 5 5  
 347 KO 22  
 \*

348 KK CLEAR  
 349 KM CLEAR HYDROGRAPH STACK  
 350 HC 8

351 KK Q  
 352 KM RUNOFF FROM SUB-BASIN Q  
 353 KM Area of Basin subtrating the area to be retained for 100 year event  
 354 BA 0024 9  
 355 LS 88 10  
 356 UK 86 02 0 15 100  
 357 RK 160 014 032 TRAP 4 5  
 358 KO 22  
 \*

359 KK R  
 360 KM RUNOFF FROM SUB-BASIN R  
 361 KM Area of Basin subtrating the area to be retained for 100 year event  
 362 BA 14 8  
 363 LS 88 14  
 364 UK 180 025 0 15 100  
 365 RK 4300 023 032 TRAP 15 5  
 \*

366  
367

KO  
ZZ

DEV-100 OH1  
22

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT  
LINE

(V) ROUTING (--->) DIVERSION OR PUMP FLOW  
( ) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

NO  
16  
22  
29  
35  
42  
48  
55  
61  
64  
71  
77  
84  
90  
96  
103  
109  
115  
122  
128  
135  
139  
142  
148  
155  
161  
168  
174  
181  
187  
194  
200  
207  
213

A1  
V  
V  
BASA1

A2  
V  
V  
BASAZ

B1  
V  
V  
BASB-3

B2

COMBB  
V  
V  
BASB-5

C  
V  
V  
BASC

D  
V  
V  
BASD-1  
V  
V  
BASD

E1  
V  
V  
BASE1-  
V  
V  
BASE1-

E2  
V  
V  
BASE2

COMBE

CLEAR

E3  
V  
V  
BASE3

F1  
V  
V  
BASF1

F2  
V  
V  
BASF2

F3  
V  
V  
BASF3

G  
V  
V  
BASG

H1  
V  
V  
BASH1-

DEV-100 OH1

219

V  
V  
BASH1-  
V  
V  
BASH1-

225

232

H2  
V  
BASH2-  
V  
V  
BASH2-

238

244

251

I  
V  
V  
BASI

257

CLEAR

267

J  
V  
V  
BASJ

273

280

K  
V  
V  
BASK

286

293

L  
V  
V  
BASL

299

306

M  
V  
V  
BASM

312

319

N  
V  
V  
BASN

325

332

O

340

P

348

CLEAR

351

Q

359

R

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

\*\*\*\*\*  
\* FLOOD HYDROGRAPH PACKAGE (HEC-1) \*  
\* JUN 1998 \*  
\* VERSION 4 1 \*  
\* RUN DATE 05MAY06 TIME 10 18 38 \*  
\*\*\*\*\*

\*\*\*\*\*  
\* U S ARMY CORPS OF ENGINEERS \*  
\* HYDROLOGIC ENGINEERING CENTER \*  
\* 609 SECOND STREET \*  
\* DAVIS, CALIFORNIA 95616 \*  
\* (916) 756-1104 \*  
\*\*\*\*\*

HEC-1 MODEL FOR SERENO CANYON  
100-YEAR, 6-HOUR STORM  
RAINFALL FROM NOAA ATLAS  
SCS CURVE NUMBER SOIL LOSS PARAMETERS  
KINEMATIC WAVE HYDROGRAPH ROUTING  
DEVELOPED CONDITIONS  
10% IMPERVIOUS AREA USED FOR SITE

PREPARED BY WOOD/PATEL, 1 10 2005  
FILE NAME DEV-100 DAT

13 IO

OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE

IT

HYDROGRAPH TIME DATA  
NMIN 2 MINUTES IN COMPUTATION INTERVAL  
IDATE 1 0 STARTING DATE  
ITIME 0000 STARTING TIME

\*\*\* \*\*

64 KK \*\*\*\*\*
\* \*
\* BASB-5 \*
\* \*
\*\*\*\*\*

70 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT 033 TIME INTERVAL IN HOURS

WARNING --- ROUTED OUTFLOW ( 186 ) IS GREATER THAN MAXIMUM OUTFLOW ( 182 ) IN STORAGE-OUTFLOW TABLE

\*\*\* \*\*

77 KK \*\*\*\*\*
\* \*
\* BASC \* -1
\* \*
\*\*\*\*\*

83 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

96 KK \*\*\*\*\*
\* \*
\* BASD \* -2
\* \*
\*\*\*\*\*

102 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT 033 TIME INTERVAL IN HOURS

WARNING --- ROUTED OUTFLOW ( 82 ) IS GREATER THAN MAXIMUM OUTFLOW ( 79 ) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 80 ) IS GREATER THAN MAXIMUM OUTFLOW ( 79 ) IN STORAGE-OUTFLOW TABLE

\*\*\* \*\*

115 KK \*\*\*\*\*
\* \*
\* BASE1- \* 3
\* \*
\*\*\*\*\*

121 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

128 KK \*\*\*\*\*
\* \*
\* BASE2 \*
\* \*
\*\*\*\*\*

134 KO        OUTPUT CONTROL VARIABLES  
 IPRNT        5    PRINT CONTROL  
 IPLOT        0    PLOT CONTROL  
 QSCAL        0    HYDROGRAPH PLOT SCALE  
 IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
 IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2        900    LAST ORDINATE PUNCHED OR SAVED  
 TIMINT       033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 135 KK       \*    COMBE    \*  
 \*            \*  
 \*\*\*\*\*

138 KO        OUTPUT CONTROL VARIABLES  
 IPRNT        5    PRINT CONTROL  
 IPLOT        0    PLOT CONTROL  
 QSCAL        0    HYDROGRAPH PLOT SCALE  
 IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
 IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2        900    LAST ORDINATE PUNCHED OR SAVED  
 TIMINT       033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 148 KK       \*    BASE3    \*  
 \*            \*  
 \*\*\*\*\*

154 KO        OUTPUT CONTROL VARIABLES  
 IPRNT        5    PRINT CONTROL  
 IPLOT        0    PLOT CONTROL  
 QSCAL        0    HYDROGRAPH PLOT SCALE  
 IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
 IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2        900    LAST ORDINATE PUNCHED OR SAVED  
 TIMINT       033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 161 KK       \*    BASF1    \*  
 \*            \*  
 \*\*\*\*\*

167 KO        OUTPUT CONTROL VARIABLES  
 IPRNT        5    PRINT CONTROL  
 IPLOT        0    PLOT CONTROL  
 QSCAL        0    HYDROGRAPH PLOT SCALE  
 IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
 IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2        900    LAST ORDINATE PUNCHED OR SAVED  
 TIMINT       033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*  
 174 KK       \*    BASF2    \*  
 \*            \*  
 \*\*\*\*\*

180 KO        OUTPUT CONTROL VARIABLES  
 IPRNT        5    PRINT CONTROL  
 IPLOT        0    PLOT CONTROL  
 QSCAL        0    HYDROGRAPH PLOT SCALE  
 IPNCH        0    PUNCH COMPUTED HYDROGRAPH  
 IOUT        22    SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1        1    FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2        900    LAST ORDINATE PUNCHED OR SAVED  
 TIMINT       033    TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
 \*            \*

187 KK \* BASF3 \*  
\* \*  
\*\*\*\*\*

193 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
200 KK \* BASG \*  
\* \*  
\*\*\*\*\*

206 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
225 KK \* BASH1- \* 5  
\* \*  
\*\*\*\*\*

231 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
244 KK \* BASH2- \* 2  
\* \*  
\*\*\*\*\*

250 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
257 KK \* BASI \*  
\* \*  
\*\*\*\*\*

263 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*





\*\*\* \*\*

332 KK \*\*\*\*\*  
\* \*  
\* 0 \*  
\* \*  
\*\*\*\*\*

339 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

340 KK \*\*\*\*\*  
\* \*  
\* P \*  
\* \*  
\*\*\*\*\*

347 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

351 KK \*\*\*\*\*  
\* \*  
\* Q \*  
\* \*  
\*\*\*\*\*

358 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

\*\*\* \*\*

359 KK \*\*\*\*\*  
\* \*  
\* R \*  
\* \*  
\*\*\*\*\*

366 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0 HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 033 TIME INTERVAL IN HOURS

1

RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+		A1	227	3 17	19	5	4	08	
+	ROUTED TO								
+		BAS1	223	3 20	18	5	4	08	4 95 3 20
+	HYDROGRAPH AT								
+		A2	107	3 13	9	2	2	04	

DEV-100 OH1

+	ROUTED TO	BASA2	100	3 17	8	2	2	04	2776 85	3 17
+	HYDROGRAPH AT	B1	81	3 13	6	2	1	03		
+	ROUTED TO	BASB-3	73	3 20	6	1	1	03	2785 76	3 20
+	HYDROGRAPH AT	B2	122	3 20	12	3	2	05		
+	2 COMBINED AT	COMBB	195	3 20	18	4	4	08		
+	ROUTED TO	BASB-5	186	3 23	16	4	3	08	2751 03	3 23
+	HYDROGRAPH AT	C	87	3 13	7	2	1	03		
+	ROUTED TO	BASC	78	3 20	6	2	1	03	2783 83	3 20
+	HYDROGRAPH AT	D	92	3 17	8	2	2	03		
+	ROUTED TO	BASD-1	89	3 20	8	2	2	03	2753 90	3 20
+	ROUTED TO	BASD	82	3 23	7	2	1	03	2729 05	3 23
+	HYDROGRAPH AT	E1	144	3 20	15	4	3	06		
+	ROUTED TO	BASE1-	139	3 23	13	3	3	06	2720 89	3 23
+	ROUTED TO	BASE1-	131	3 30	12	3	2	06	2699 87	3 30
+	HYDROGRAPH AT	E2	135	3 17	11	3	2	05		
+	ROUTED TO	BASE2	123	3 20	10	2	2	05	2698 78	3 20
+	2 COMBINED AT	COMBE	233	3 27	22	6	4	11		
+	6 COMBINED AT	CLEAR	853	3 23	78	20	16	37		
+	HYDROGRAPH AT	E3	27	3 10	2	0	0	01		
+	ROUTED TO	BASE3	15	3 23	1	0	0	01	2714 55	3 23
+	HYDROGRAPH AT	F1	99	3 17	9	2	2	04		
+	ROUTED TO	BASF1	88	3 23	8	2	2	04	2700 88	3 23
+	HYDROGRAPH AT	F2	43	3 13	3	1	1	01		
+	ROUTED TO	BASF2	41	3 17	3	1	1	01	2704 91	3 17
+	HYDROGRAPH AT	F3	39	3 13	3	1	1	01		
+	ROUTED TO	BASF3	34	3 17	3	1	1	01	2709 88	3 17
+	HYDROGRAPH AT	G	41	3 17	4	1	1	02		
+	ROUTED TO	BASG	25	3 30	3	1	1	02	2726 65	3 30
+	HYDROGRAPH AT	H1	151	3 17	14	4	3	06		
+	ROUTED TO	BASH1-	147	3 20	13	3	3	06	2739 89	3 20

DEV-100 OHL

+	ROUTED TO	BASH1-	145	3 23	12	3	3	06	2740 95	3 23
+	ROUTED TO	BASH1-	117	3 33	11	3	2	06	2731 66	3 33
+	HYDROGRAPH AT	H2	166	3 20	17	4	3	07		
+	ROUTED TO	BASH2-	160	3 27	16	4	3	07	2775 94	3 27
+	ROUTED TO	BASH2-	154	3 30	15	4	3	07	2788 88	3 30
+	HYDROGRAPH AT	I	72	3 17	6	2	1	03		
+	ROUTED TO	BASI	49	3 27	5	1	1	03	2759 42	3 27
+	8 COMBINED AT	CLEAR	474	3 30	47	12	10	24		
+	HYDROGRAPH AT	J	84	3 10	6	1	1	02		
+	ROUTED TO	BASJ	73	3 13	5	1	1	02	2773 74	3 13
+	HYDROGRAPH AT	K	36	3 07	2	1	0	01		
+	ROUTED TO	BASK	20	3 17	1	0	0	01	2740 56	3 17
+	HYDROGRAPH AT	L	33	3 10	2	1	0	01		
+	ROUTED TO	BASL	29	3 13	2	0	0	01	2725 86	3 13
+	HYDROGRAPH AT	M	28	3 10	2	0	0	01		
+	ROUTED TO	BASM	18	3 20	1	0	0	01	2724 64	3 20
+	HYDROGRAPH AT	N	35	3 10	2	1	0	01		
+	ROUTED TO	BASN	19	3 23	2	0	0	01	2694 52	3 23
+	HYDROGRAPH AT	O	9	3 07	1	0	0	00		
+	HYDROGRAPH AT	P	21	3 10	1	0	0	01		
+	8 COMBINED AT	CLEAR	574	3 30	61	15	12	31		
+	HYDROGRAPH AT	Q	10	3 07	1	0	0	00		
+	HYDROGRAPH AT	R	402	3 17	32	8	6	14		

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION INTERVAL		VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	(IN)
FOR STORM = 1	STORM AREA (SQ MI) =			01					
A1	MANE	1 82	228 04	190 15	2 16	2 00	227 13	190 00	2 16

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9693E+01 OUTFLOW= 9660E+01 BASIN STORAGE= 1348E-02 PERCENT ERROR= 3

FOR STORM = 1	STORM AREA (SQ MI) =			01					
A2	MANE	1 35	107 48	187 90	2 24	2 00	107 42	188 00	2 24

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4302E+01 OUTFLOW= 4297E+01 BASIN STORAGE= 4761E-03 PERCENT ERROR= 1

FOR STORM = 1	STORM AREA (SQ MI) =			01					
B1	MANE	1 65	81 16	187 86	2 19	2 00	81 11	188 00	2 19

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3161E+01 OUTFLOW= 3155E+01 BASIN STORAGE= 2836E-03 PERCENT ERROR= 2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 B2 MANE 1 43 123 24 191 05 2 22 2 00 121 98 192 00 2 22

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5952E+01 OUTFLOW= 5927E+01 BASIN STORAGE= 1419E-02 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 C MANE 1 27 87 99 188 69 2 22 2 00 87 35 188 00 2 22

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 3556E+01 OUTFLOW= 3548E+01 BASIN STORAGE= 4233E-03 PERCENT ERROR= 2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 D MANE 1 12 92 55 189 50 2 27 2 00 92 44 190 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4120E+01 OUTFLOW= 4112E+01 BASIN STORAGE= 7089E-03 PERCENT ERROR= .2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E1 MANE 1 44 143 79 192 60 2 26 2 00 143 60 192 00 2 26

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 7392E+01 OUTFLOW= 7358E+01 BASIN STORAGE= 2255E-02 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E2 MANE 1 86 136 41 188 26 2 27 2 00 135 19 190 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 5574E+01 OUTFLOW= 5562E+01 BASIN STORAGE= 6104E-03 PERCENT ERROR= 2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 E3 MANE 58 27 46 186 21 2 27 2 00 27 42 186 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9695E+00 OUTFLOW= 9681E+00 BASIN STORAGE= 7116E-04 PERCENT ERROR= 1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F1 MANE 2 12 99 31 189 90 2 27 2 00 99 17 190 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 4363E+01 OUTFLOW= 4350E+01 BASIN STORAGE= 6177E-03 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F2 MANE 1 18 43 68 187 44 2 27 2 00 43 43 188 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1697E+01 OUTFLOW= 1693E+01 BASIN STORAGE= 1776E-03 PERCENT ERROR= 2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 F3 MANE 73 39 42 187 56 2 27 2 00 39 12 188 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1575E+01 OUTFLOW= 1573E+01 BASIN STORAGE= 2010E-03 PERCENT ERROR= 2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 G MANE 62 40 97 190 48 2 27 2 00 40 95 190 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1939E+01 OUTFLOW= 1933E+01 BASIN STORAGE= 4768E-03 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 H1 MANE 1 42 151 12 191 31 2 26 2 00 150 68 190 00 2 26

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 7127E+01 OUTFLOW= 7102E+01 BASIN STORAGE= 1651E-02 PERCENT ERROR= 3

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 H2 MANE 2 00 166 95 192 21 2 20 2 00 165 66 192 00 2 20

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 8481E+01 OUTFLOW= 8442E+01 BASIN STORAGE= 2229E-02 PERCENT ERROR= 4

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 I MANE 1 53 73 51 188 62 2 24 2 00 71 95 190 00 2 23

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2987E+01 OUTFLOW= 2982E+01 BASIN STORAGE= 4029E-03 PERCENT ERROR= 2

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 J MANE 68 83 64 186 16 2 24 2 00 83 52 186 00 2 25

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2876E+01 OUTFLOW= 2873E+01 BASIN STORAGE= 1637E-03 PERCENT ERROR= 1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 K MANE 62 36 08 184 33 2 27 2 00 35 83 184 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1091E+01 OUTFLOW= 1090E+01 BASIN STORAGE= 2309E-04 PERCENT ERROR= 0

DEV-100 OH1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 L MANE 59 33 32 184 89 2 27 2 00 32 61 186 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1091E+01 OUTFLOW= 1090E+01 BASIN STORAGE= 5388E-04 PERCENT ERROR= 1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 M MANE 83 27 89 186 03 2 27 2 00 27 86 186 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 9695E+00 OUTFLOW= 9682E+00 BASIN STORAGE= 6146E-04 PERCENT ERROR= 1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 N MANE 1 12 34 95 185 84 2 27 2 00 34 93 186 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1212E+01 OUTFLOW= 1212E+01 BASIN STORAGE= 5737E-04 PERCENT ERROR= 0

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 O MANE 33 9 15 184 64 2 27 2 00 9 05 184 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2908E+00 OUTFLOW= 2905E+00 BASIN STORAGE= 1154E-04 PERCENT ERROR= 1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 P MANE 52 21 03 185 26 2 27 2 00 20 69 186 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 7029E+00 OUTFLOW= 7022E+00 BASIN STORAGE= 3389E-04 PERCENT ERROR= 1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 Q MANE 30 9 52 184 04 2 27 2 00 9 52 184 00 2 27

CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 2908E+00 OUTFLOW= 2905E+00 BASIN STORAGE= 7960E-05 PERCENT ERROR= 1

FOR STORM = 1 STORM AREA (SQ MI) = 01  
 R MANE 1 98 409 02 189 54 2 15 2 00 401 91 190 00 2 15

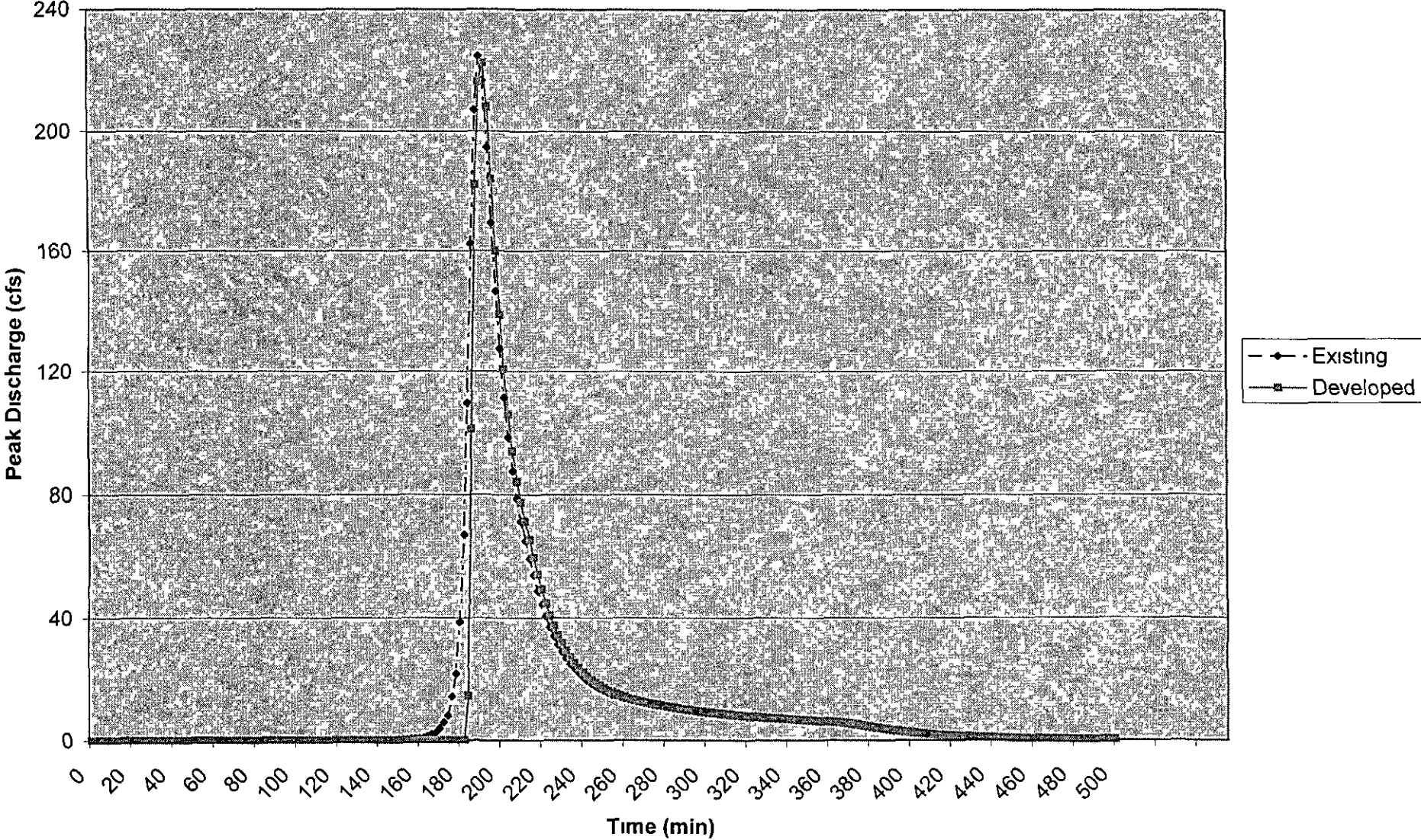
CONTINUITY SUMMARY (AC-FT) - INFLOW= 0000E+00 EXCESS= 1607E+02 OUTFLOW= 1602E+02 BASIN STORAGE= 1407E-02 PERCENT ERROR= 3

\*\*\* NORMAL END OF HEC-1 \*\*\*

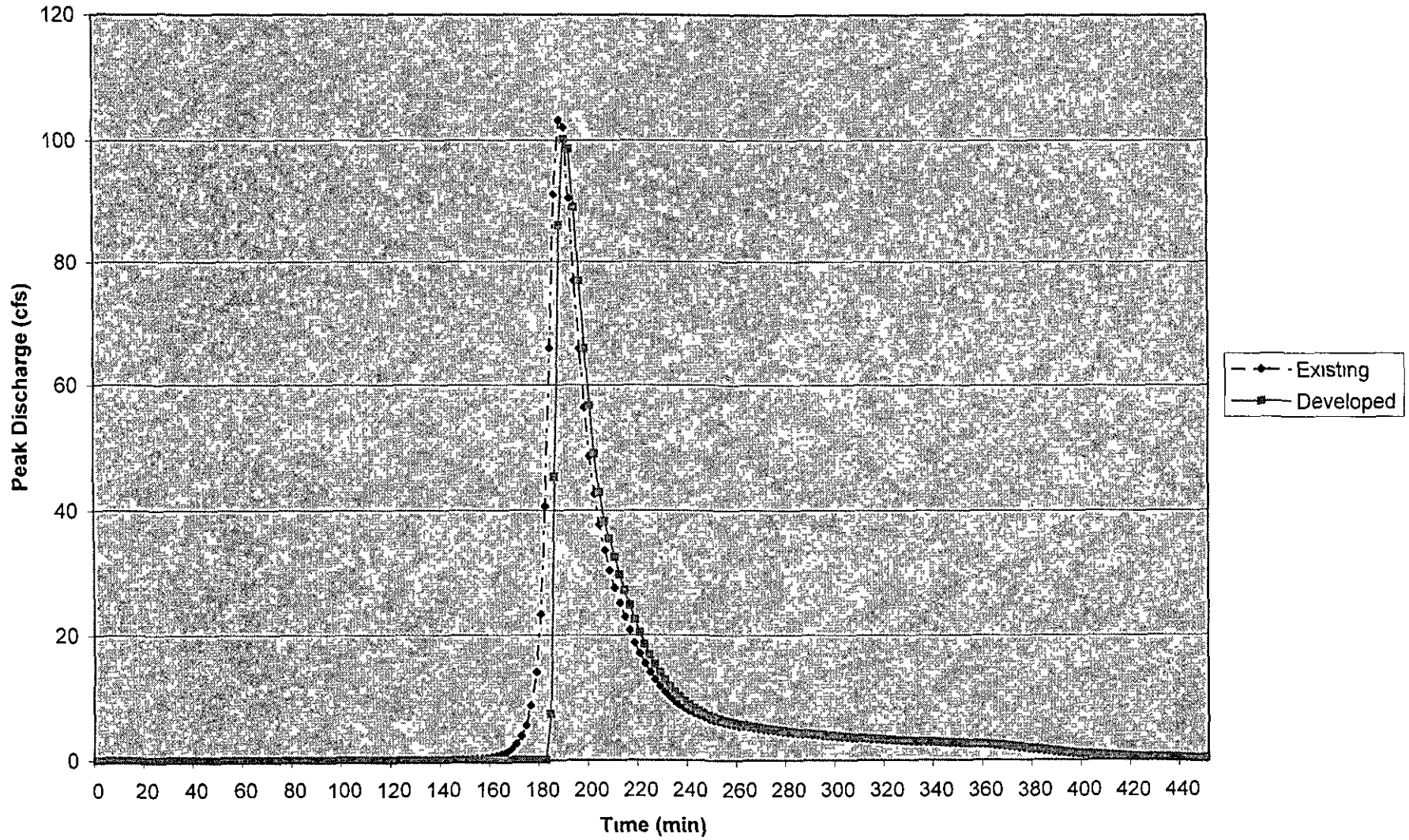
**APPENDIX D**

**Hydrographs**

# Hydrograph - A1

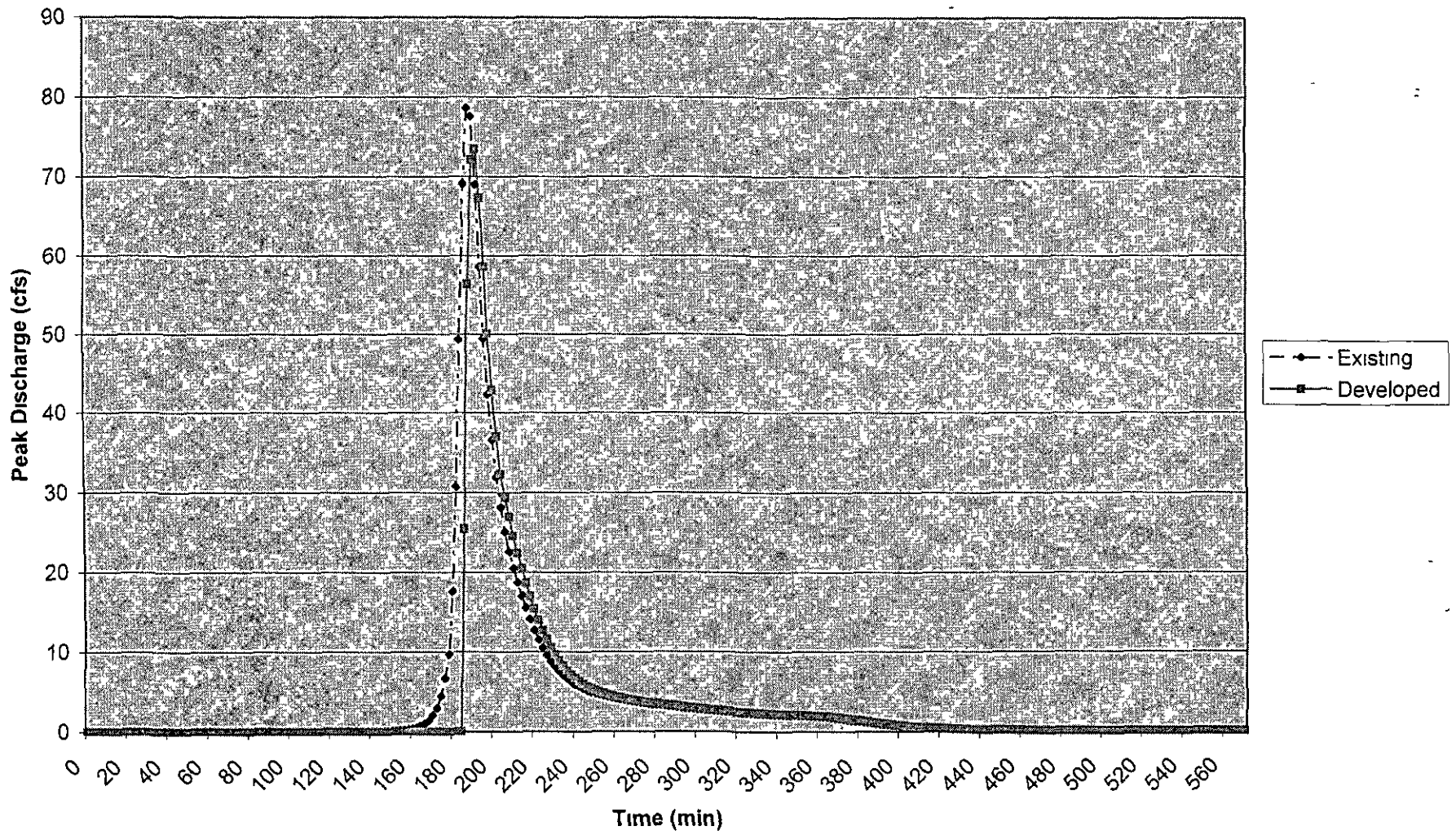


# Hydrograph - A2

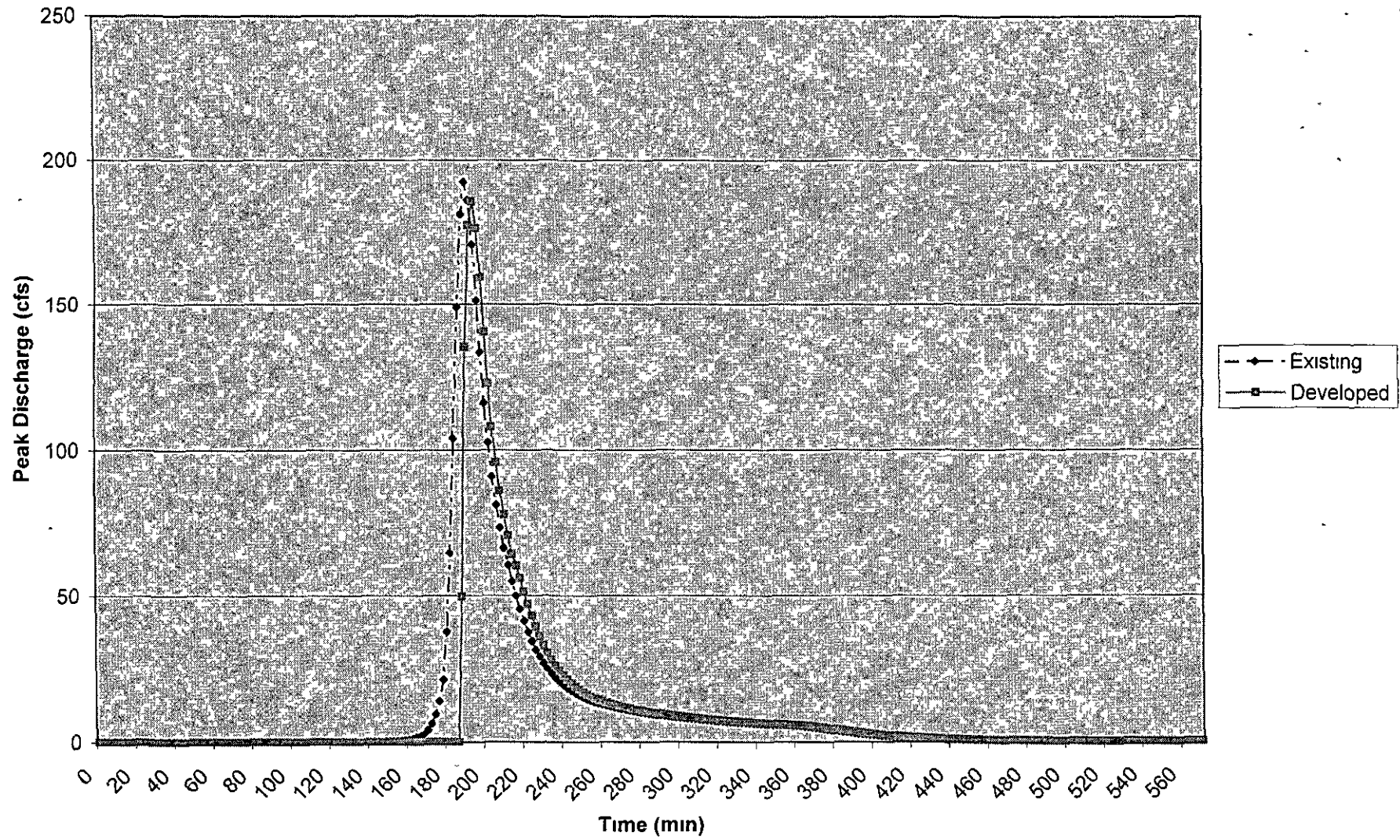




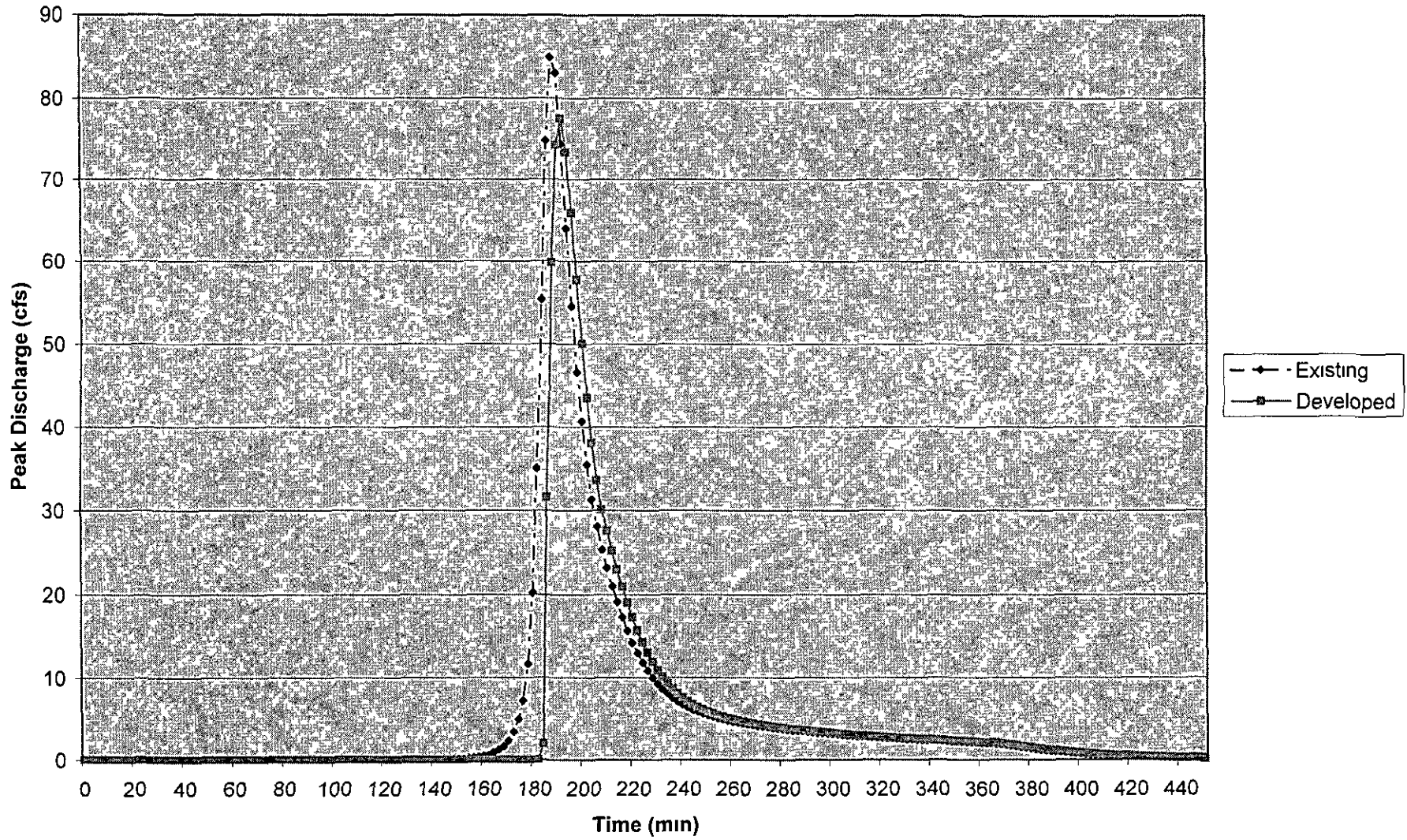
# Hydrograph - B1



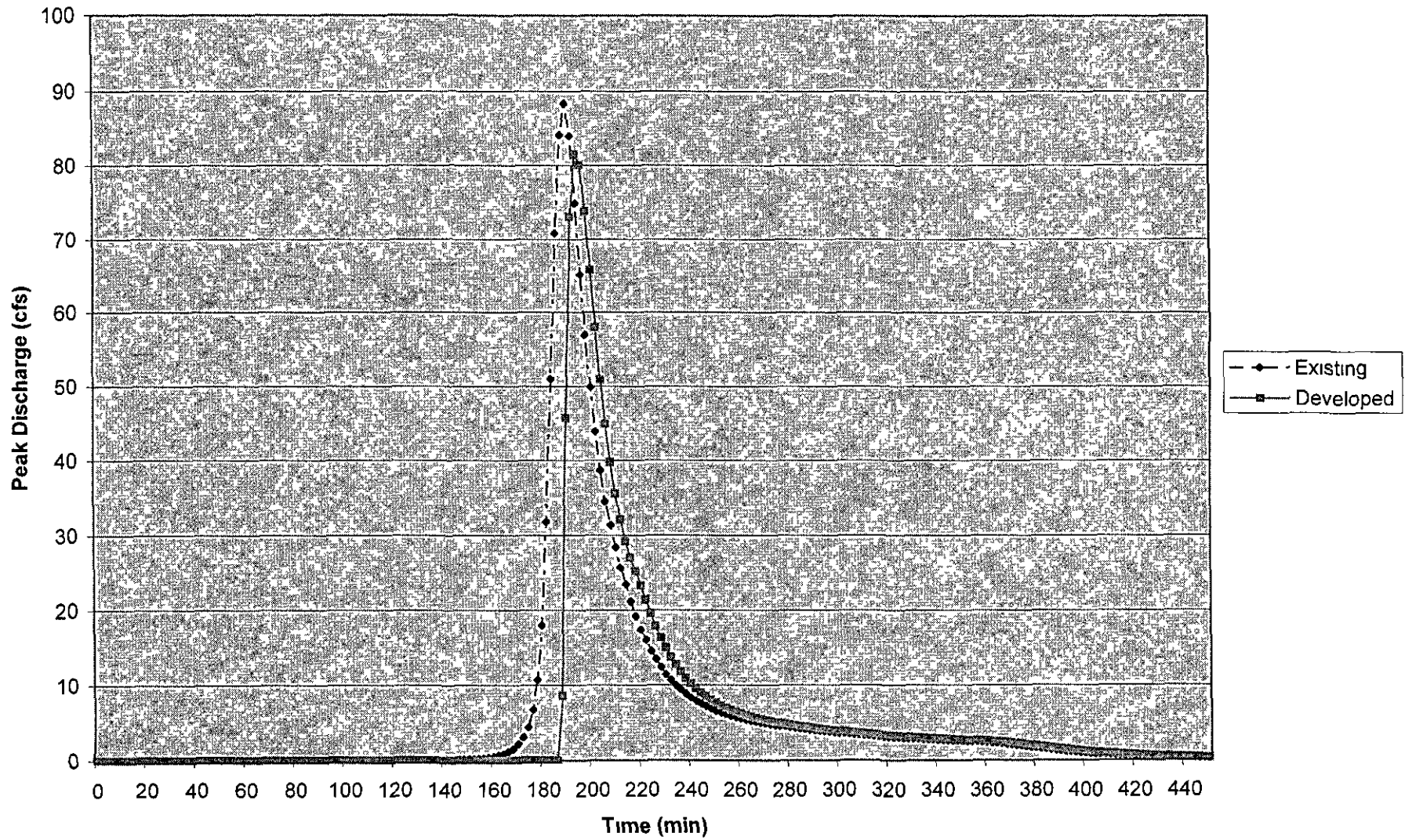
# Hydrograph - COMB



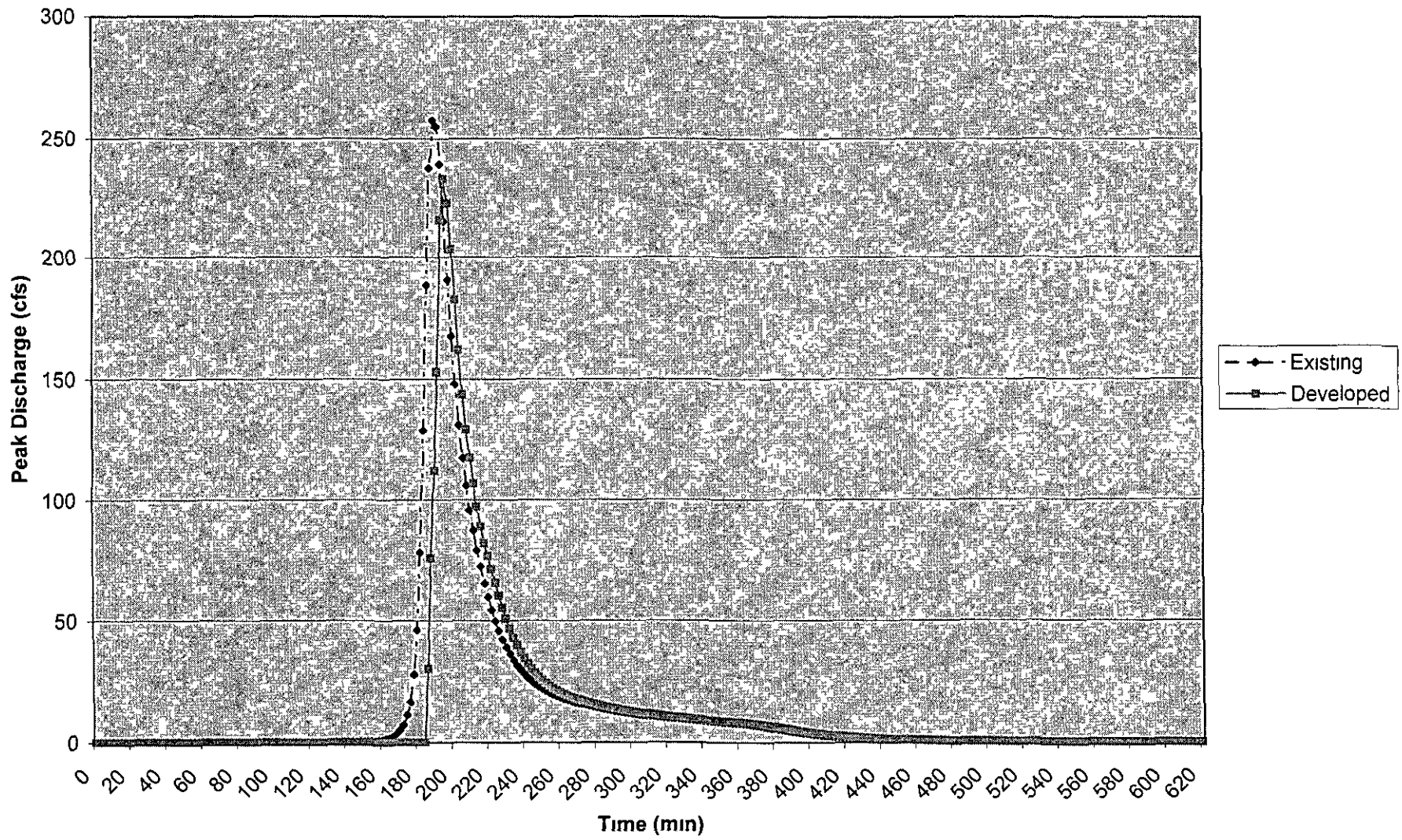
# Hydrograph - C



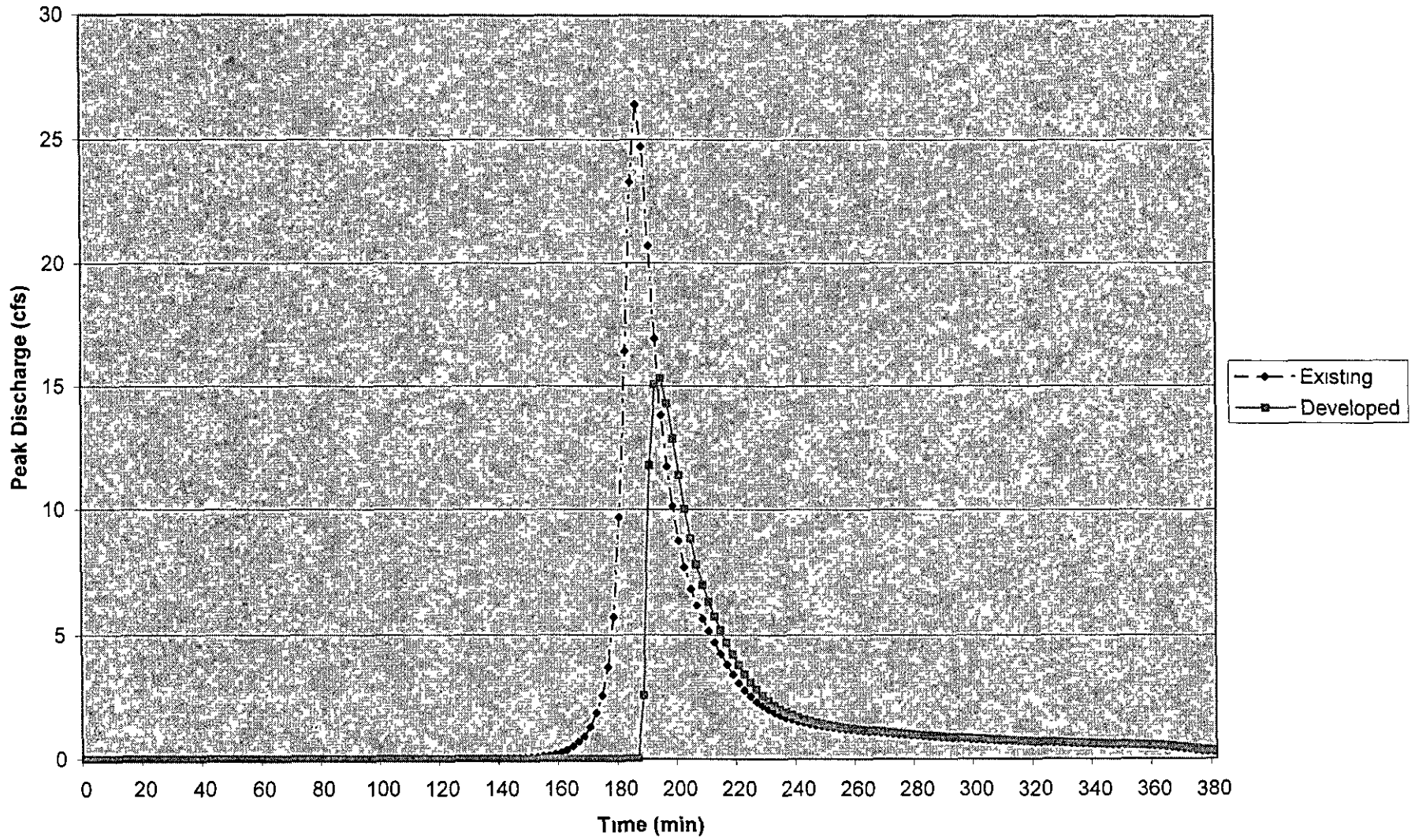
# Hydrograph - D



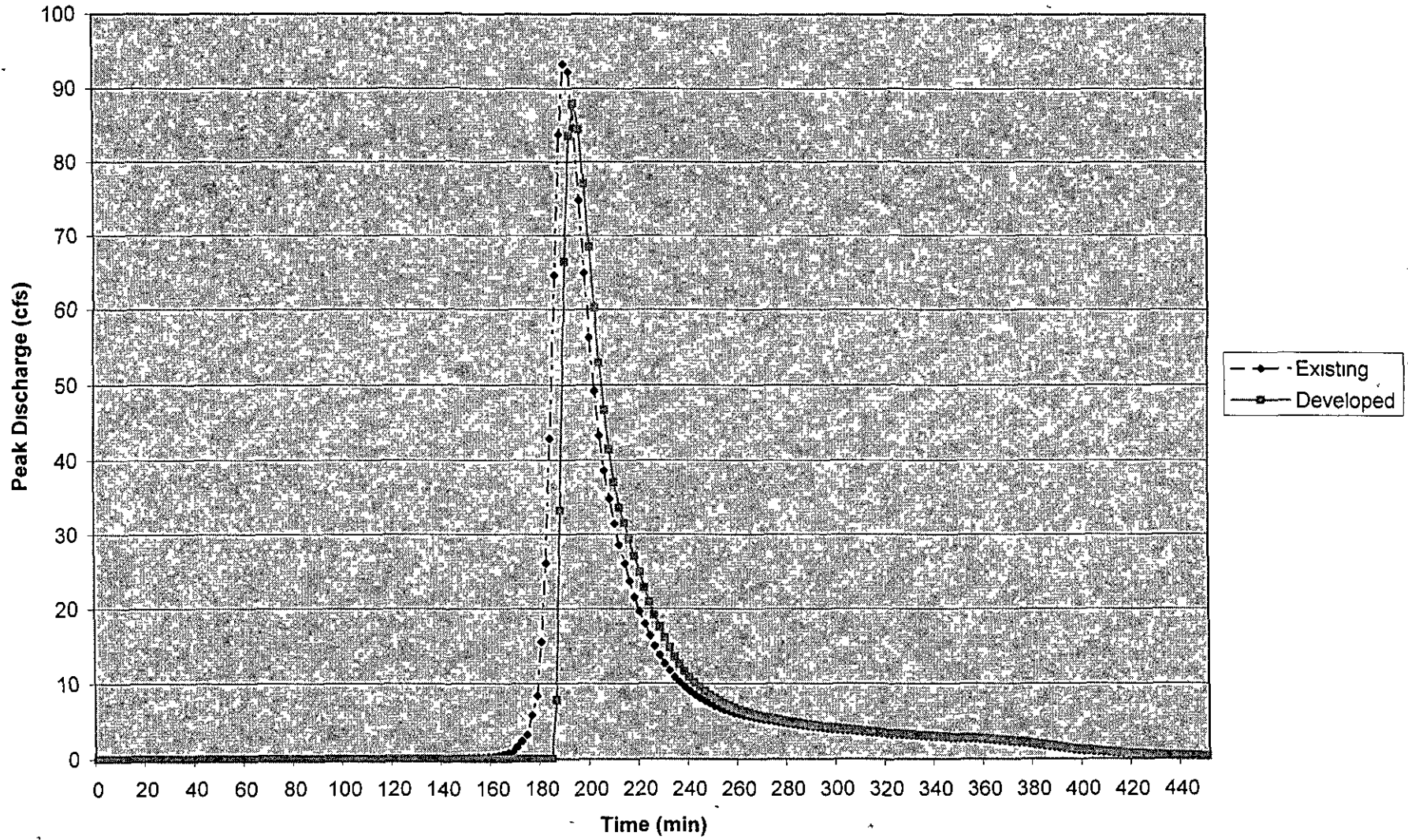
# Hydrograph - COMB E



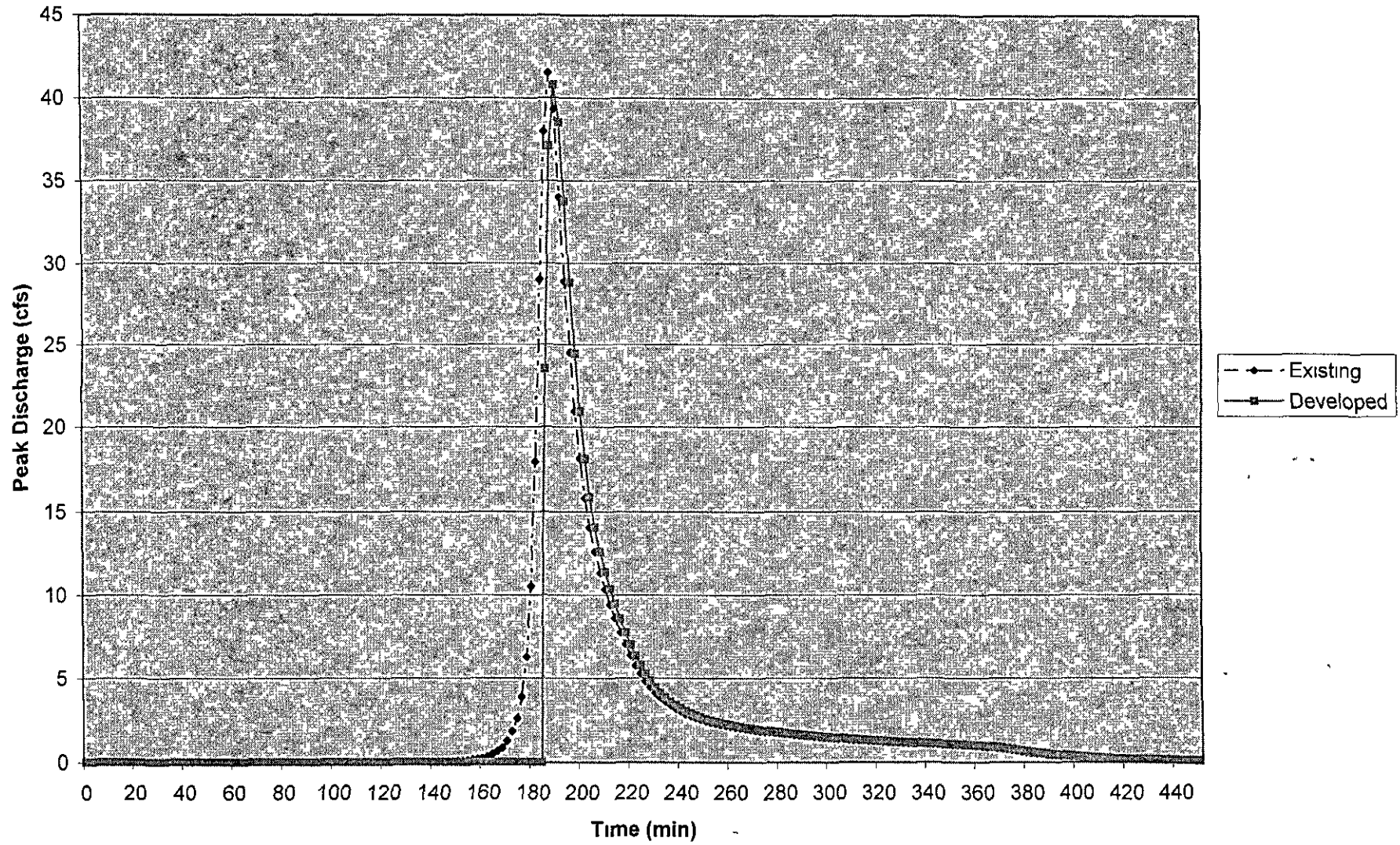
# Hydrograph - E3



# Hydrograph - F1

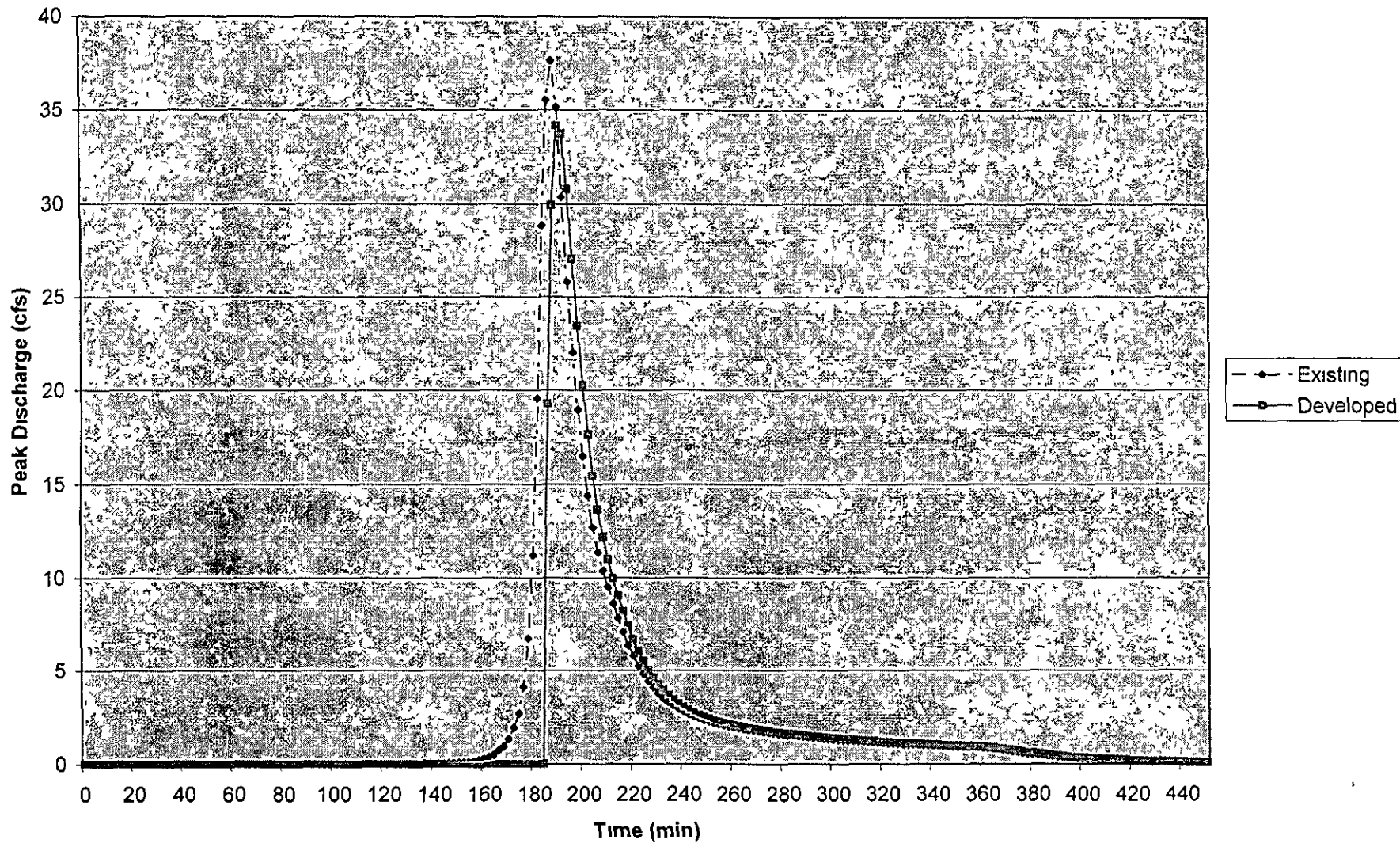


# Hydrograph - F2

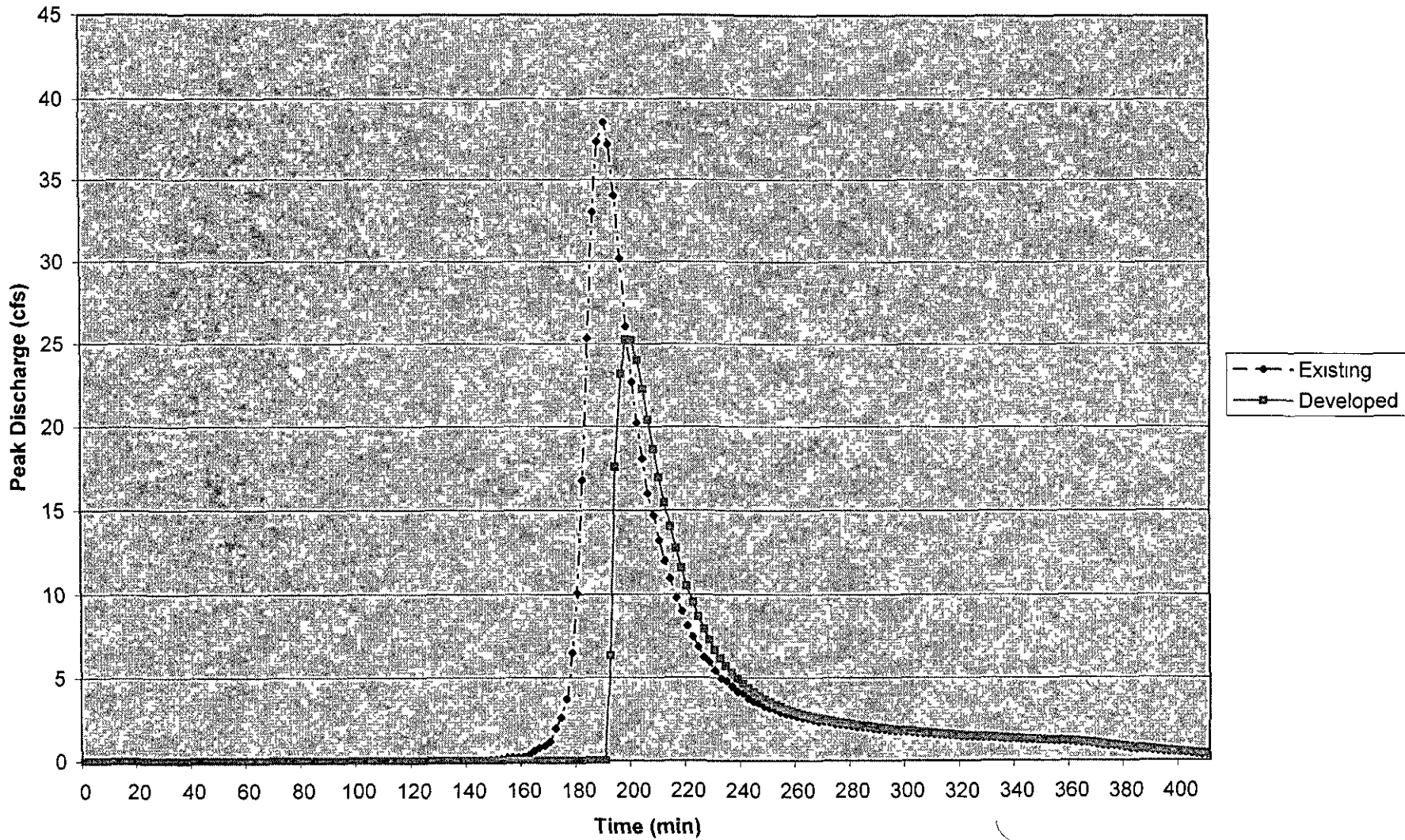




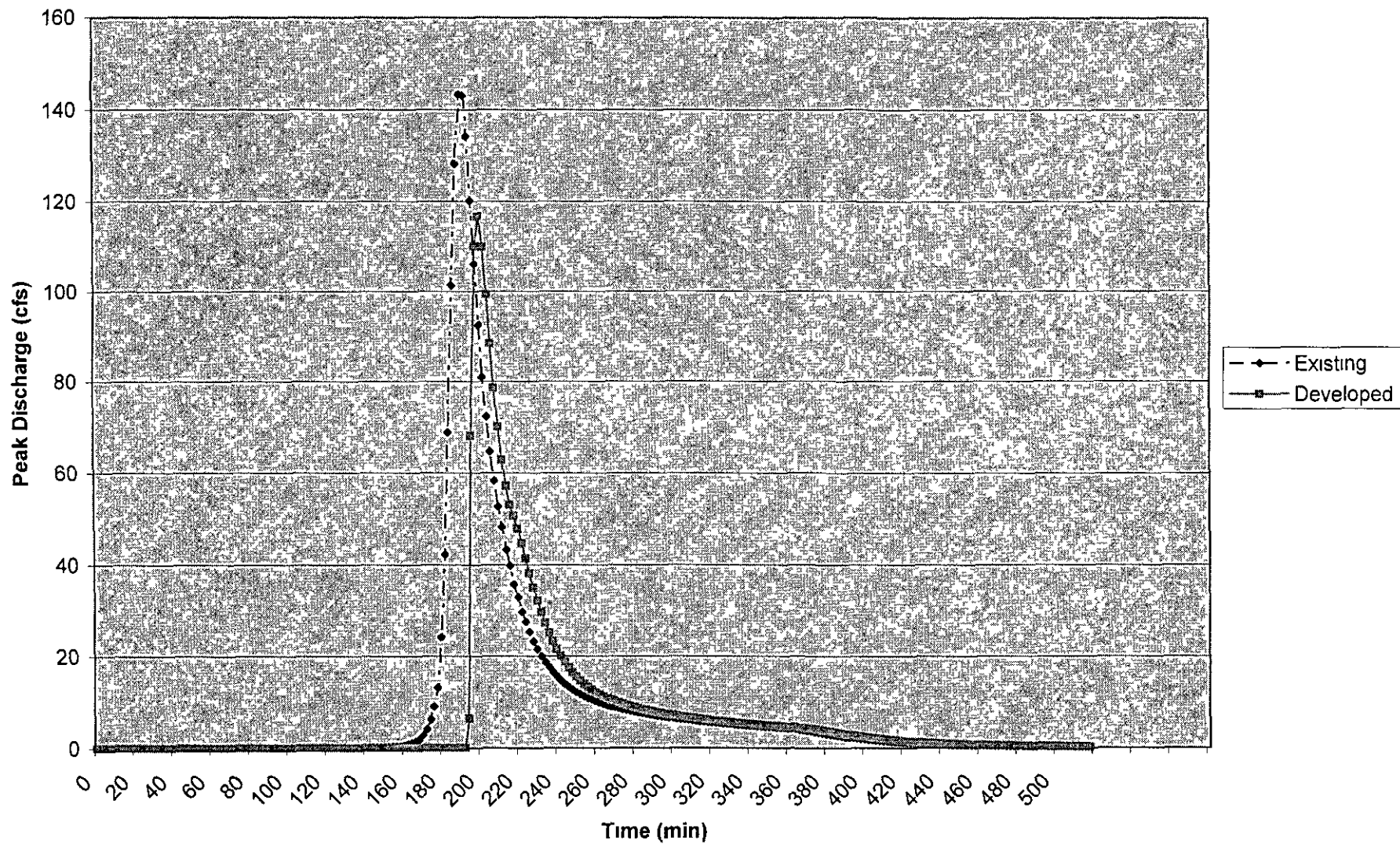
### Hydrograph - F3



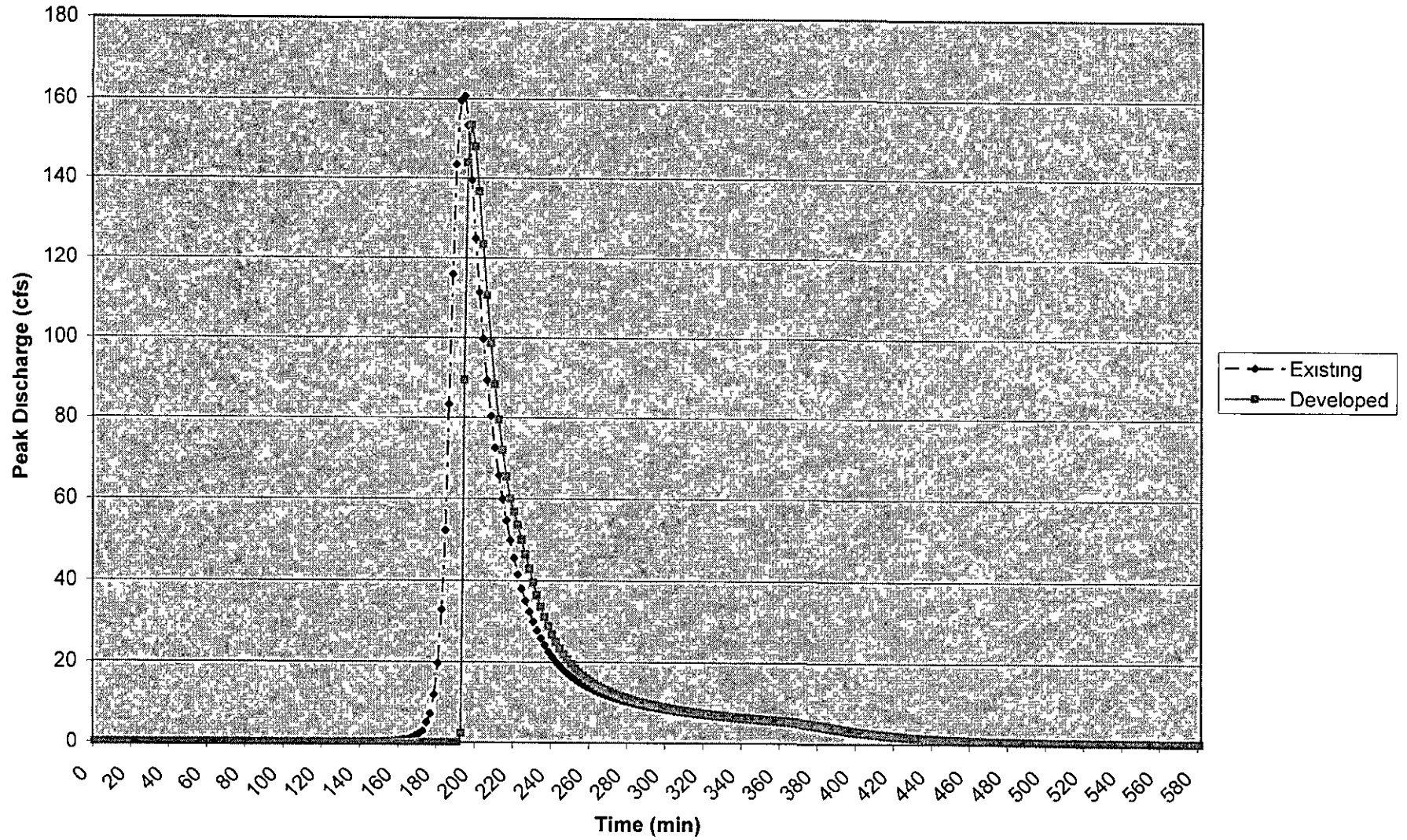
# Hydrograph - G



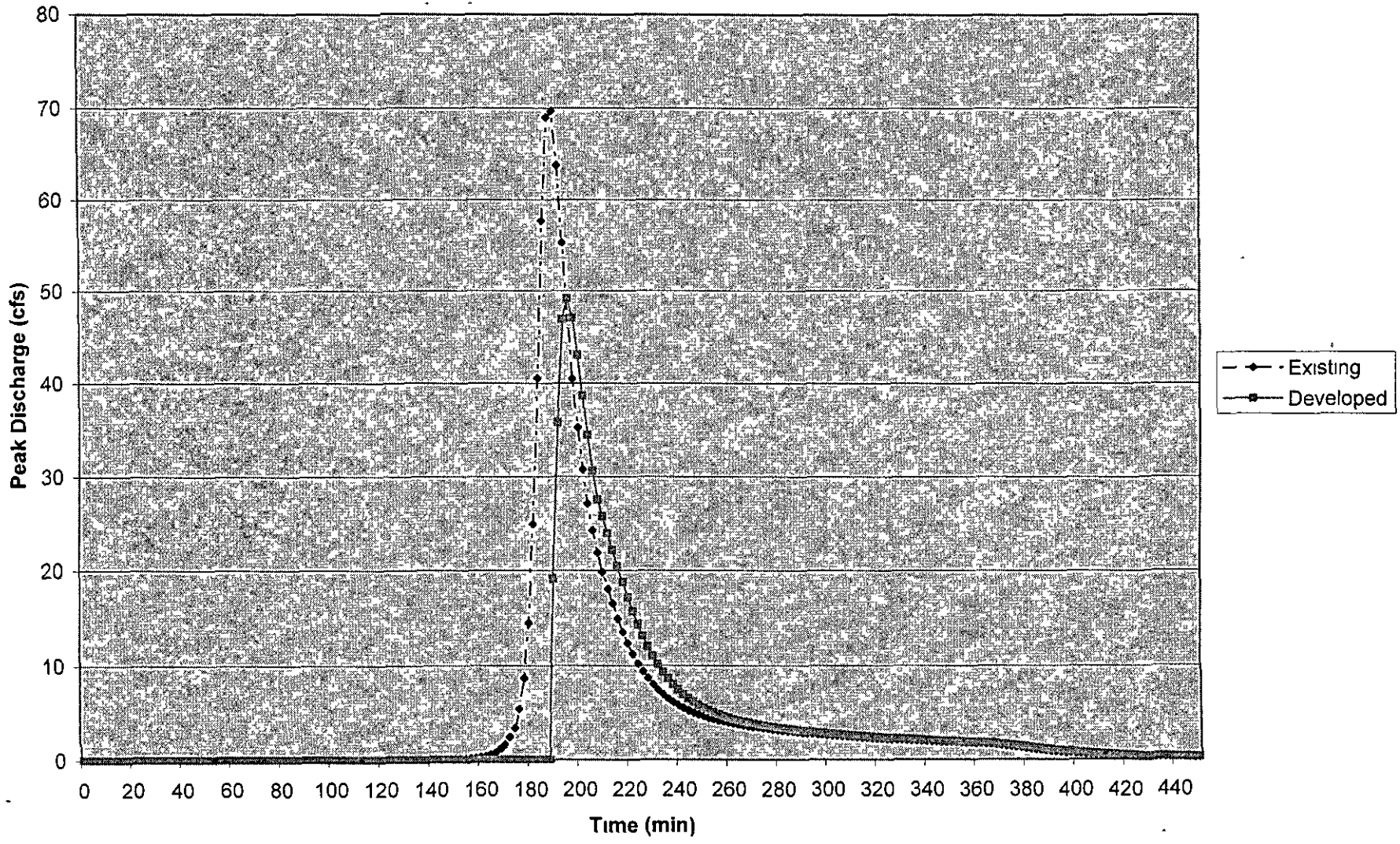
# Hydrograph - H1



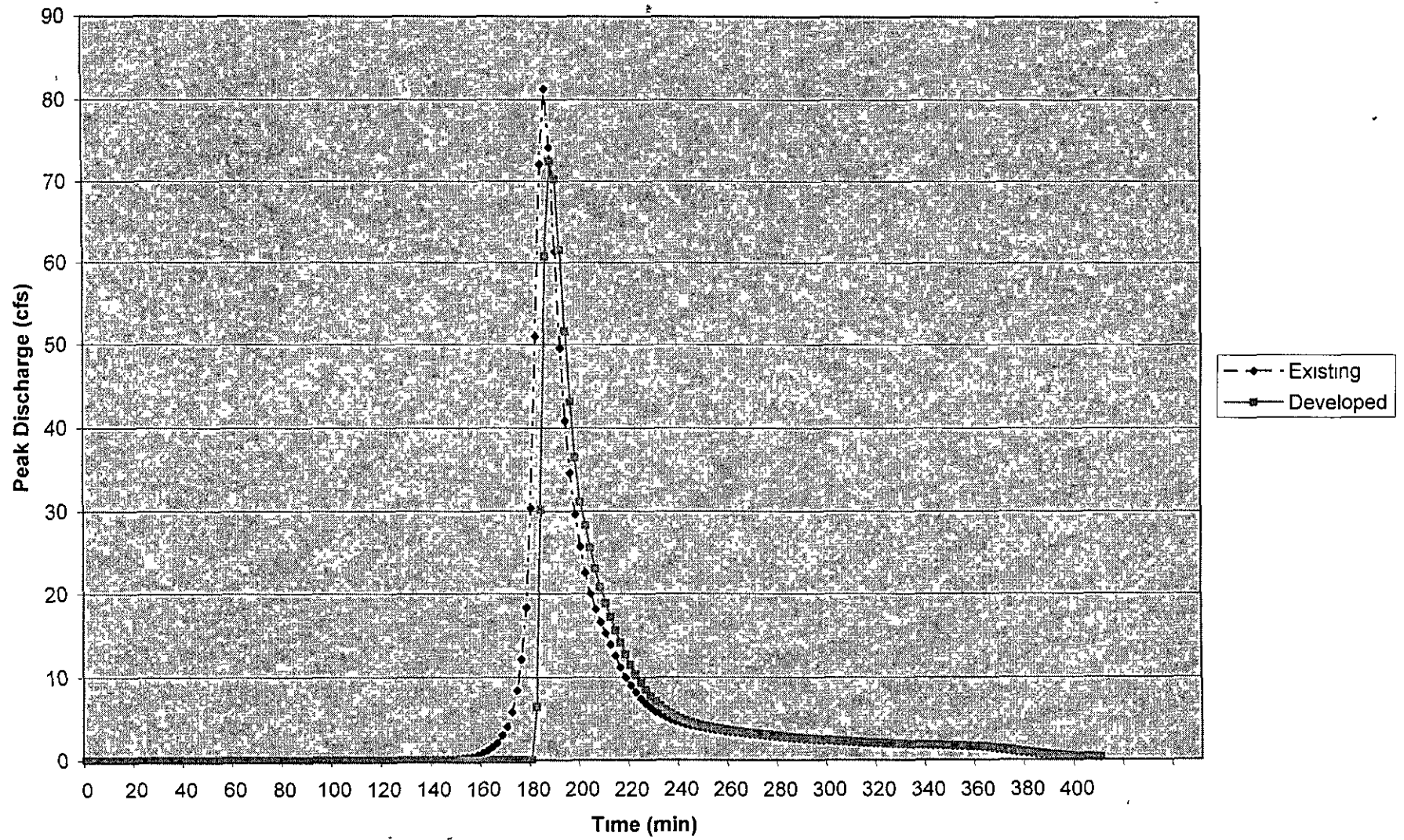
## Hydrograph - H2



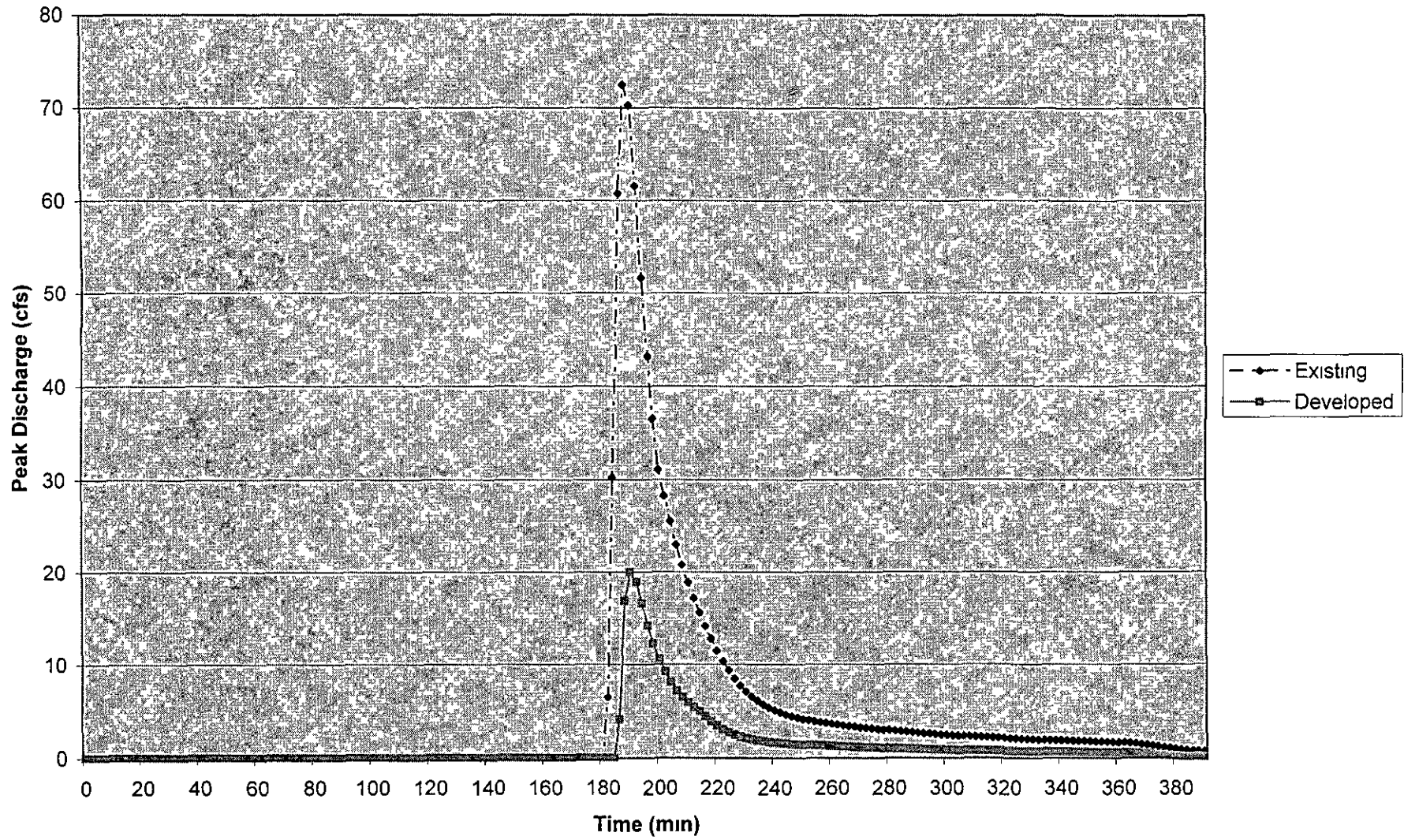
# Hydrograph - I



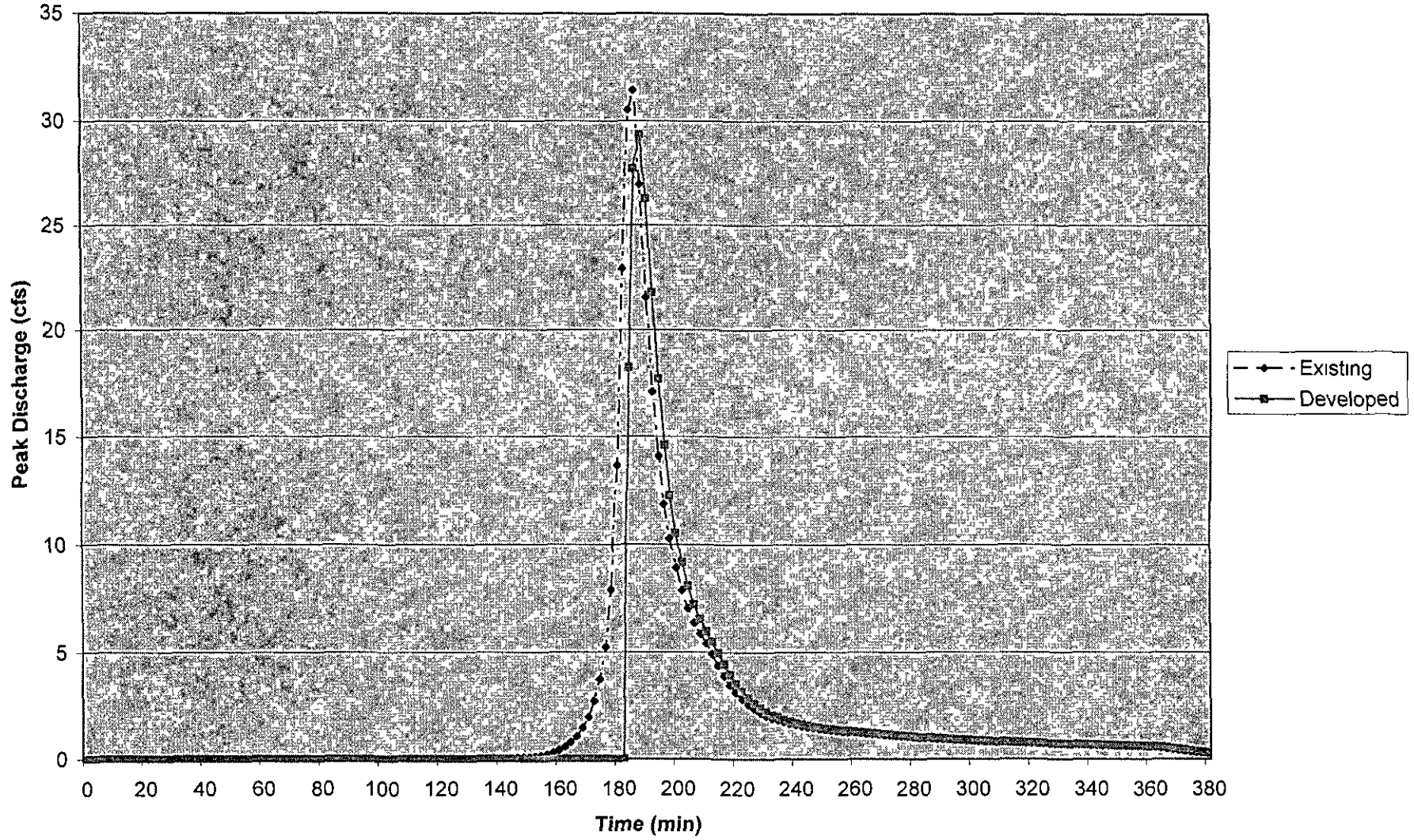
# Hydrograph - J



# Hydrograph - K

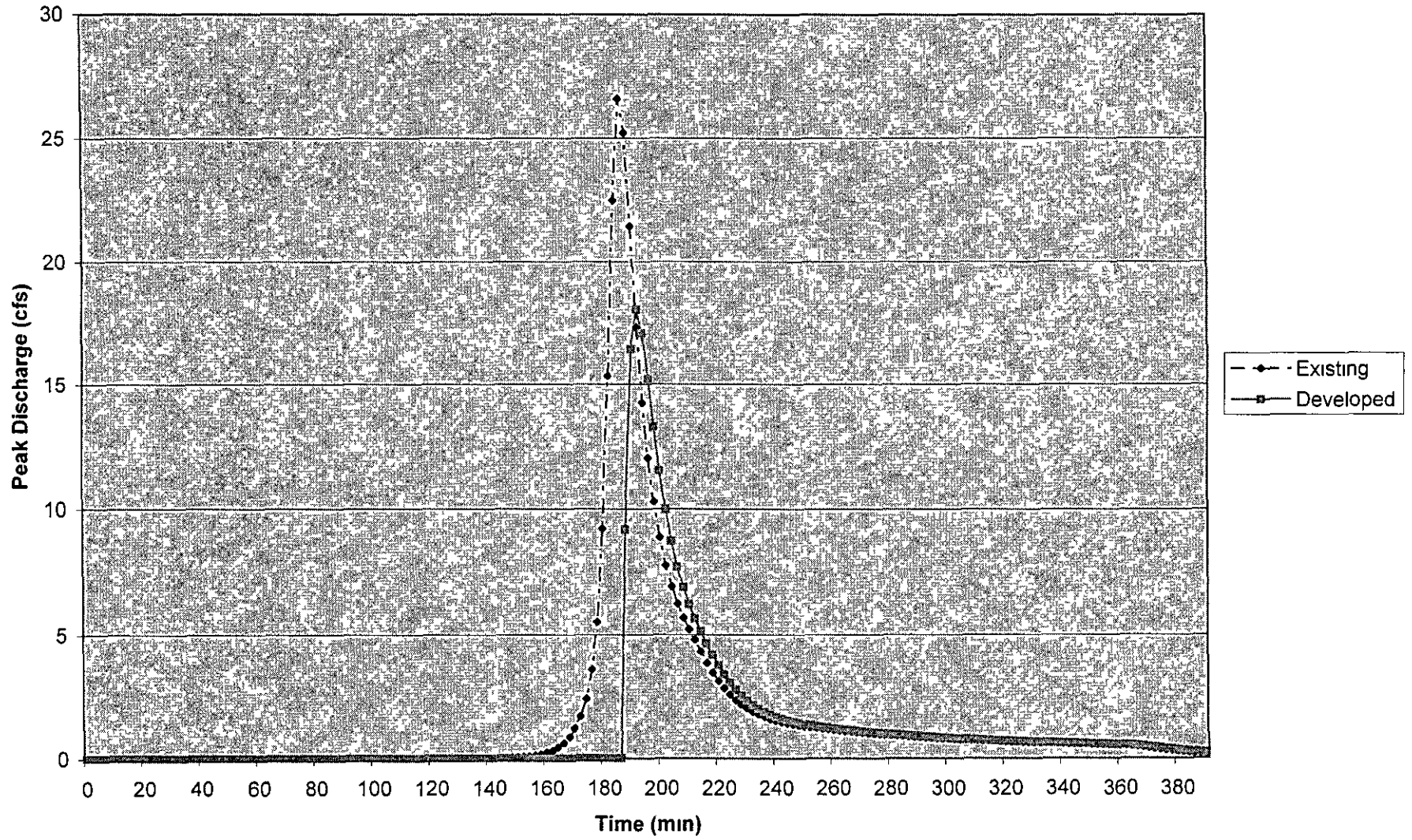


# Hydrograph - L

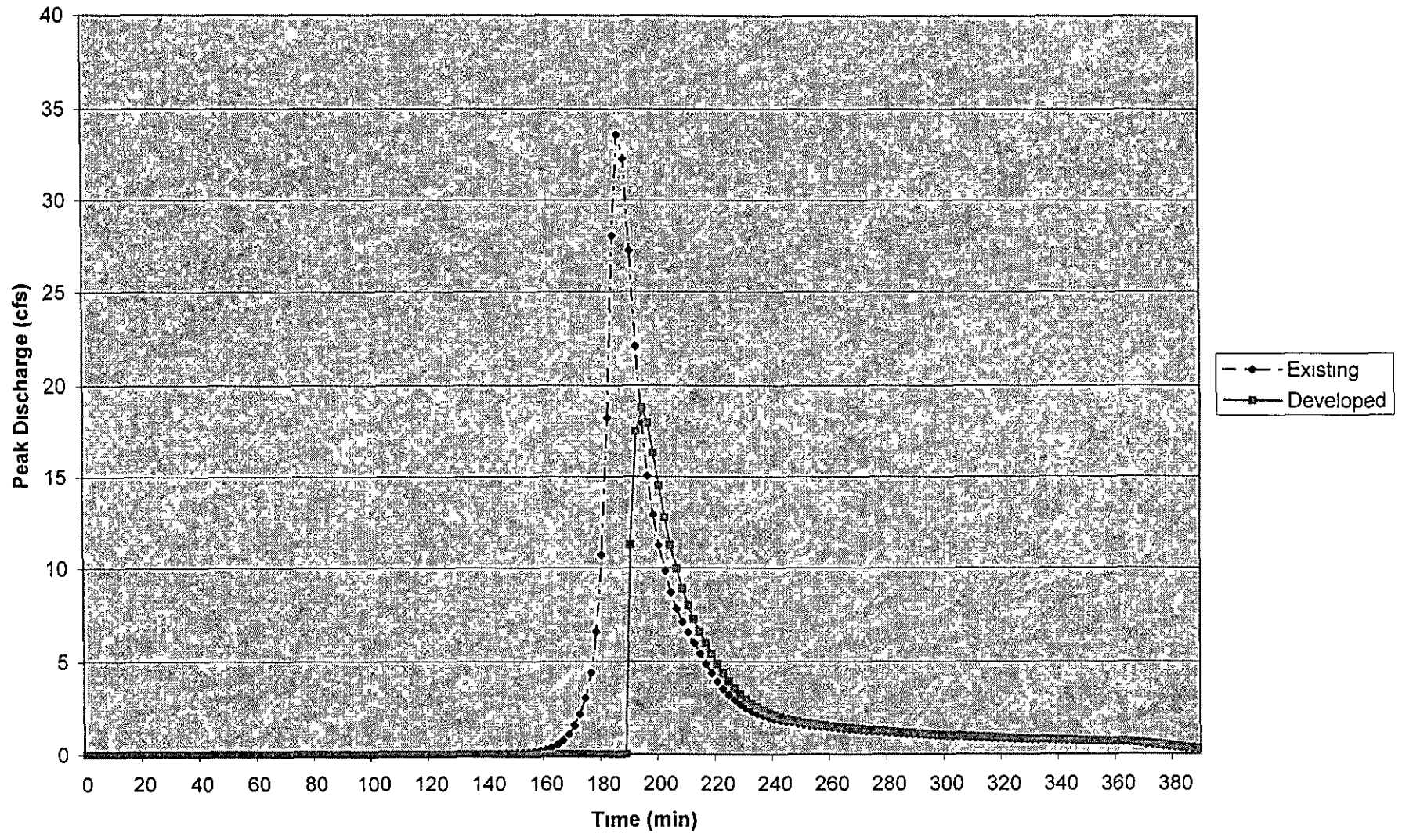




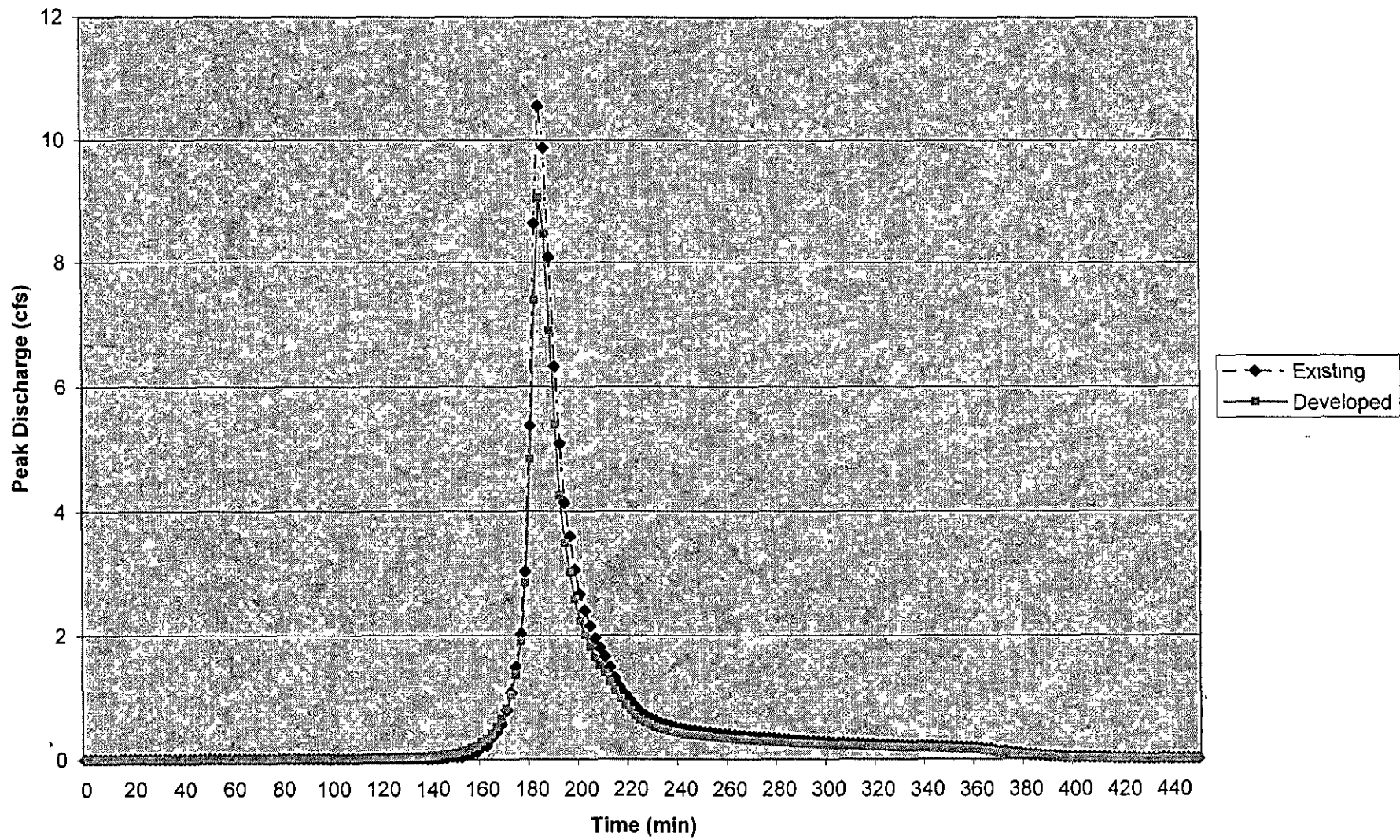
# Hydrograph - M



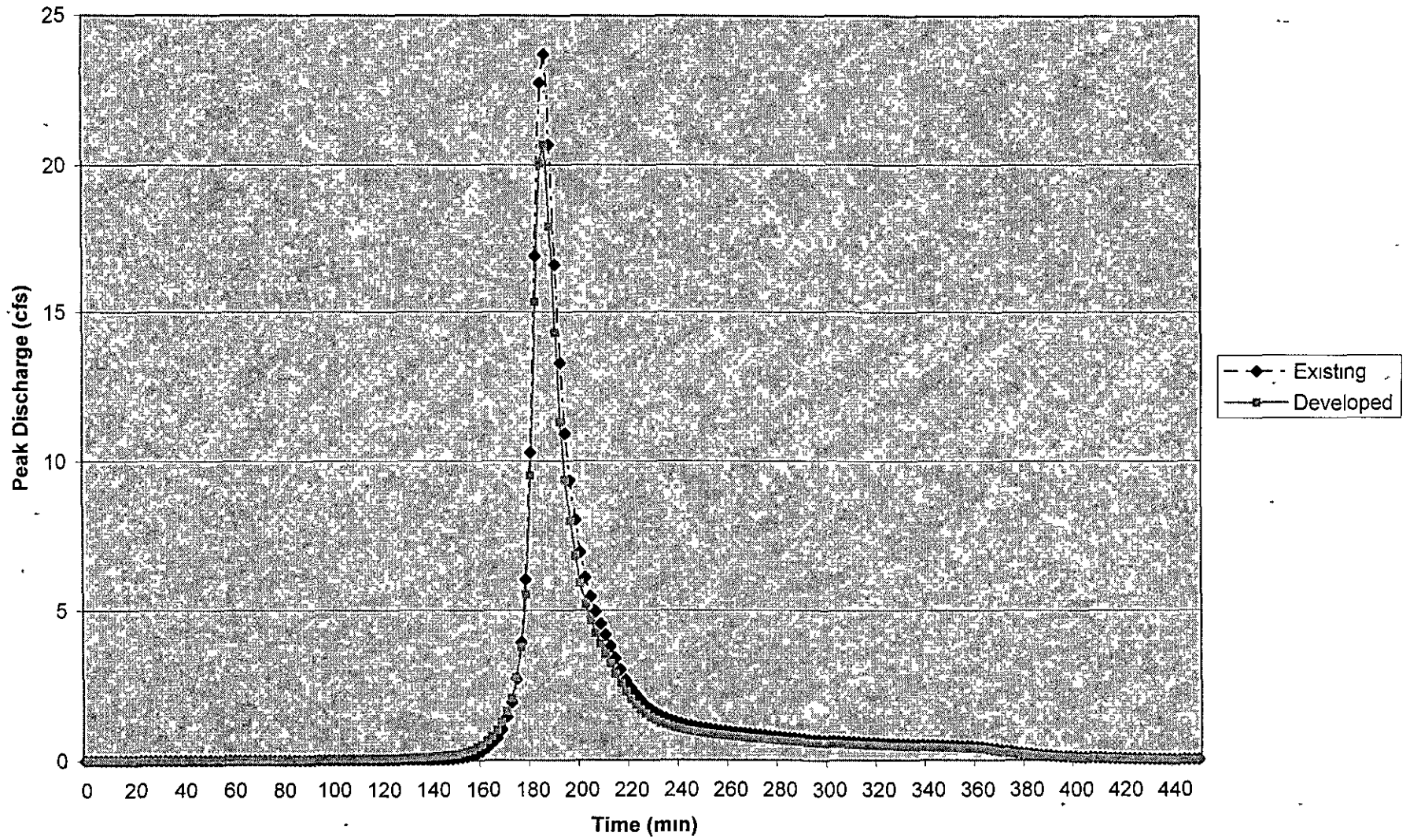
# Hydrograph - N



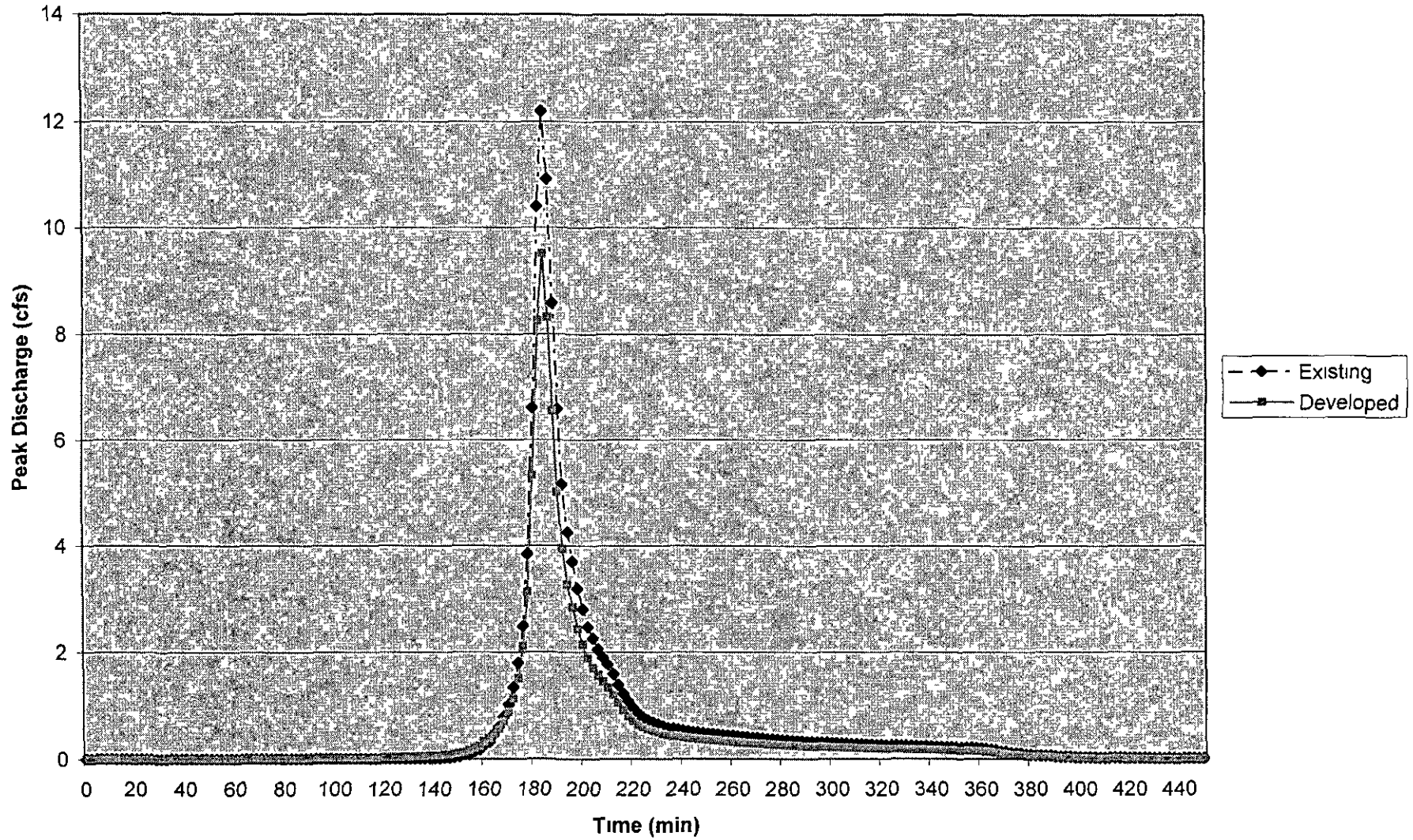
# Hydrograph - O



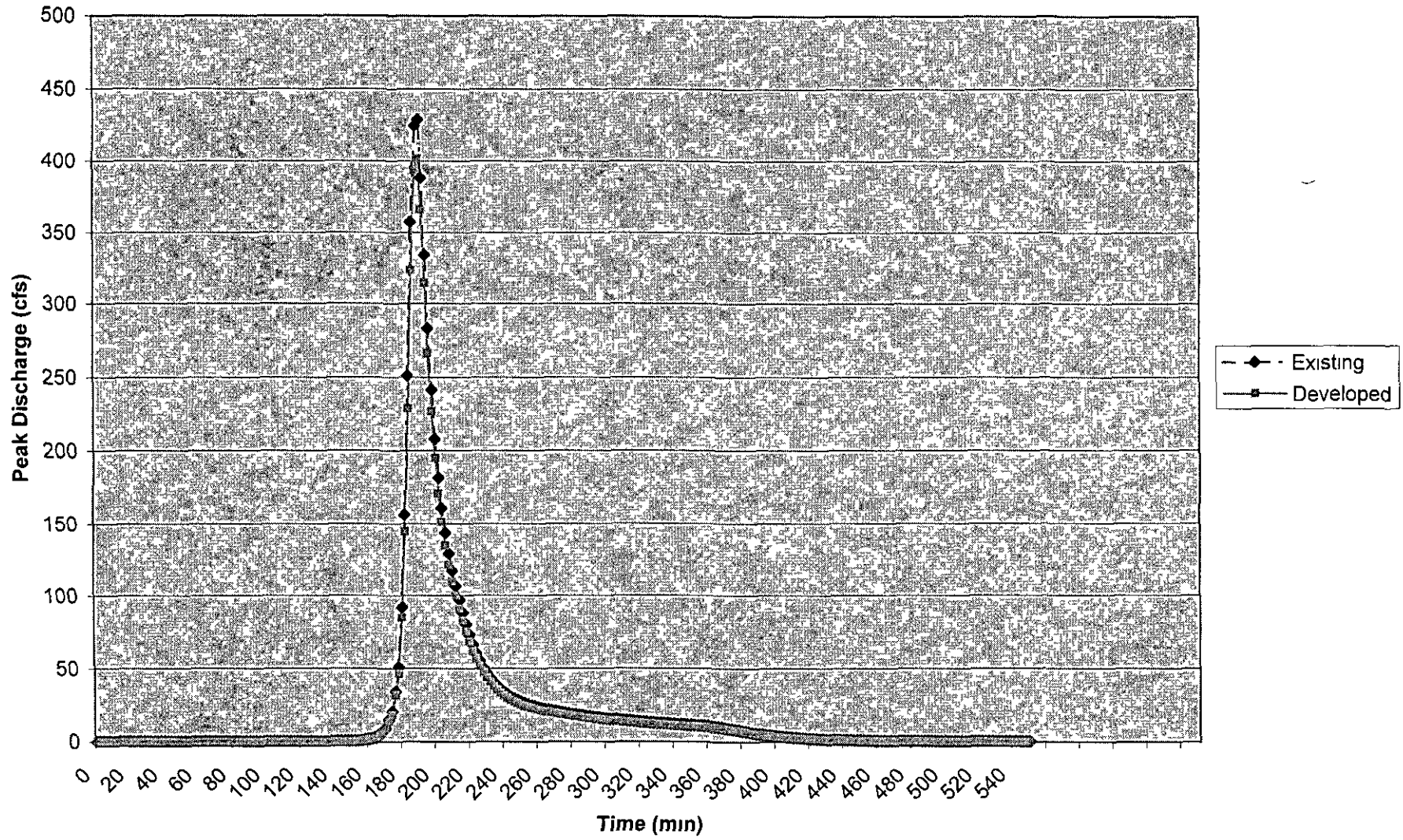
# Hydrograph - P



# Hydrograph - Subbasin Q



# Hydrograph - Subbasin R



**APPENDIX E**

**100-year Rational Method**

# WOOD/PATEL

CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project *SERENO CANYON*

Location *City of Scottsdale*

Date *09-May-06*

References *Drainage Design Manual for Maricopa County, Volume I-Hydrology*  
*City of Scottsdale Engineering & Design Standards*

Project Number *042054*

Project Engineer *Gordon W Wark, P E*

## RATIONAL METHOD CALCULATIONS: OFF-SITE FLOW

\* Summary of Rational Method values using Flood Control District of Maricopa County DDMSW Software

Concentration Point	Area		"C" Value	Length of Area (ft)	2 - Year			10-Year			100-Year		
	(sq ft)	(ac)			T <sub>c</sub> (min)	Intensity (in/hr)	Q (cfs)	T <sub>c</sub> (min)	Intensity (in/hr)	Q (cfs)	T <sub>c</sub> (min)	Intensity (in/hr)	Q (cfs)
A1	2,502,397	57.4	0.56	4,754	16	2.5	65	13	4.5	116	11	7.4	238
A2	989,311	22.7	0.56	2,100	10	3.2	33	8	5.4	55	7	8.0	112
B	2,153,597	49.4	0.56	3,200	15	2.6	58	12	4.6	102	10	7.6	210
H2	1,998,065	45.9	0.56	3,350	15	2.6	54	12	4.6	95	10	7.5	193
I	700,376	16.1	0.56	2,012	11	3.0	22	9	5.1	37	8	8.4	76
J	678,947	15.6	0.56	920	6	3.8	27	5	6.3	44	5	9.9	86
R	4,238,208	97.3	0.56	4,300	16	2.5	109	13	4.5	197	10	7.5	409

### Notes

"C" value is based on hydrologic soil group D from Figure 4-5 in the City of Scottsdale DSPM, according to the SCS Classification of Soil #61, Gran Wickenburg



Flood Control District of Maricopa County  
 042054 - Sereno Canyon  
 Sub Basin Data - Rational Method

Sub Basin	Sub Basin Parameters				Kb	Return Period (Years)					
	Area (acres)	Length (ft)	Slope (ft/ft)	Q (cfs)		2	5	10	25	50	100
A1	57.40	4,754	0.0747	0.056	Q (cfs)	65	96	116	164	205	238
A2	22.70	2,100	0.0833	0.061	Q (cfs)	33	46	55	77	97	112
B	49.40	3,200	0.0469	0.057	Q (cfs)	58	84	102	143	179	210
H2	45.90	3,350	0.0493	0.057	Q (cfs)	54	76	95	133	166	193
I	16.10	2,012	0.0572	0.063	Q (cfs)	22	30	37	52	65	76
J	15.60	920	0.0761	0.064	Q (cfs)	27	37	44	61	75	86
R	97.30	4,300	0.0605	0.053	Q (cfs)	109	162	197	277	347	409

## **APPENDIX F**

### **Hydraulic Calculations**

- Storage Rating Curves
- HEC-RAS Output Files
- Orifice Plate Calculation

**Culvert Rating Curves for Stage-Storage Intervals**

## WOOD/PATEL

CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

**Project:** *Sereno Canyon*  
**Location:** *City of Scottsdale*  
**Reference:** *Rectangular Weir Equation*

**WIP#:** 042054  
**Date:** 5/11/2006

$$Q = CLH^{3/2}$$

Where

C = Weir Coefficient = 2.80  
L = Length of Weir = 10.00 ft  
H = Head above Weir = 1.50 ft  
Q = Weir Flow = 51.44 cfs

\* The above equation is used to determine the rating curves for weir flow

# WOOD/PATEL

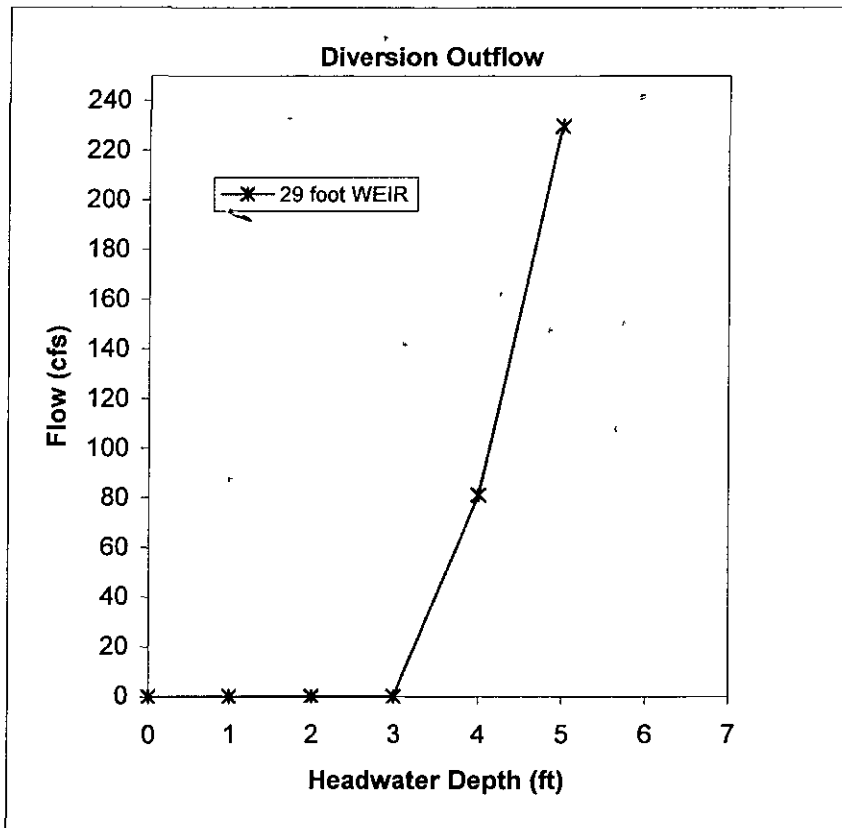
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BASA1

29	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	81
5	230



# WOOD/PATEL

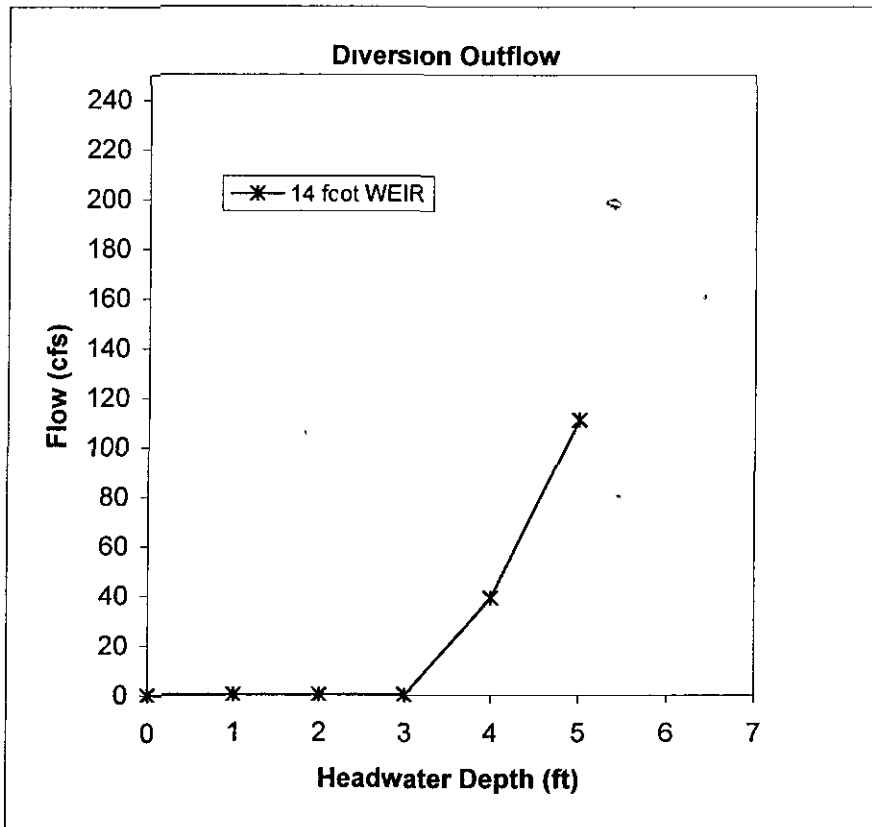
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

W/P#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS A2-5

14 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	39
5	111



# WOOD/PATEL

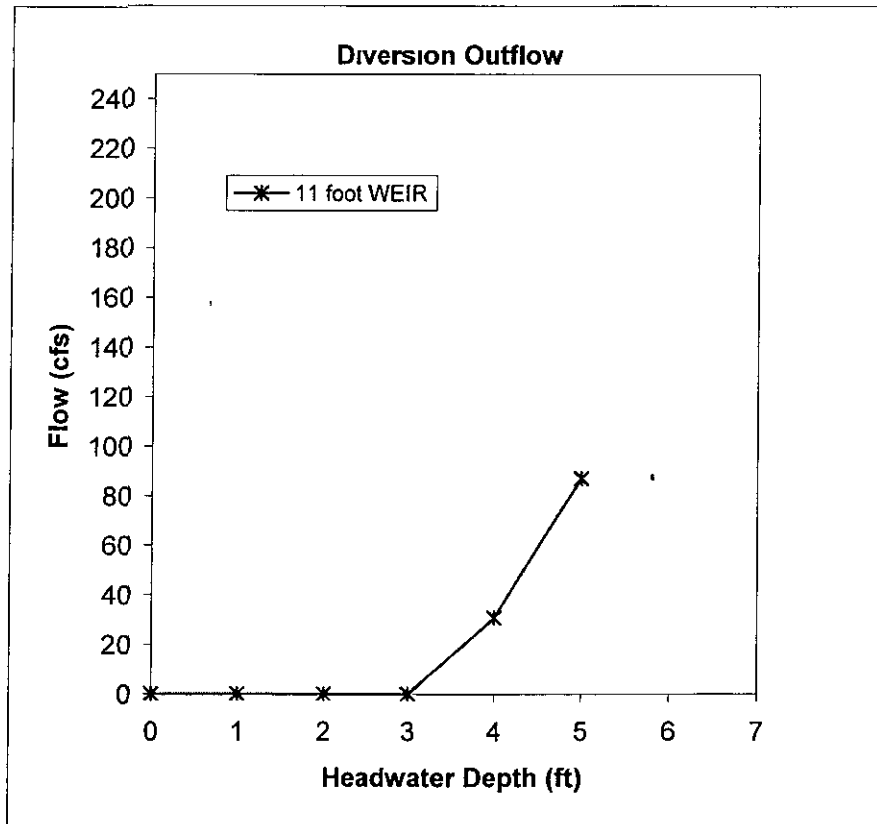
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS B-3

11	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	31
5	87



# WOOD/PATEL

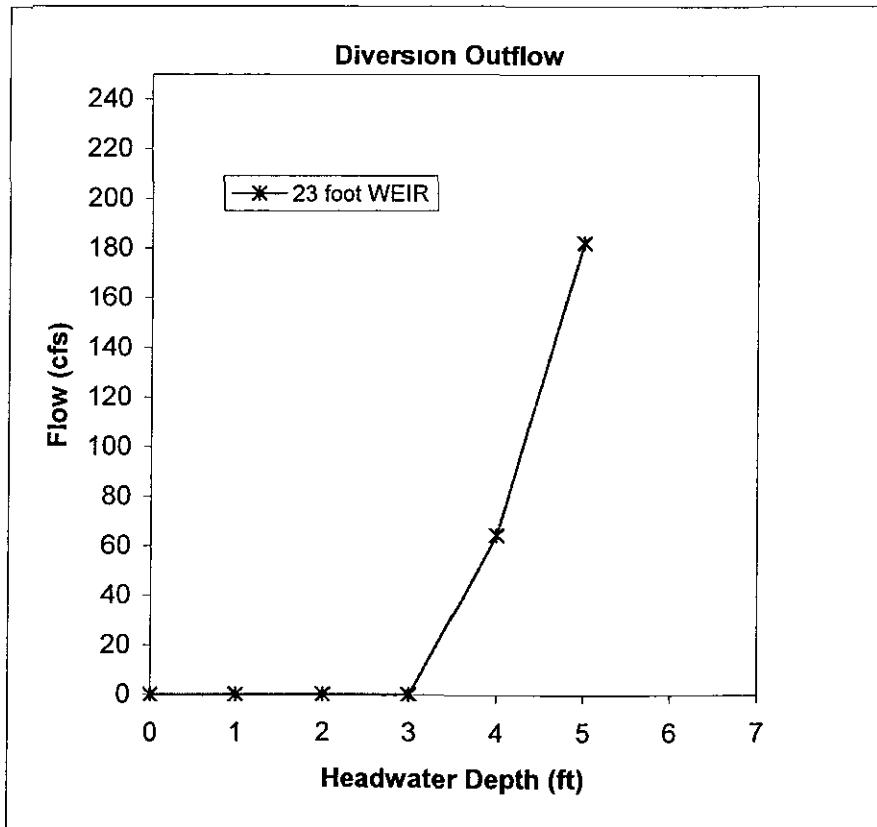
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

**Project:** Sereno Canyon  
**Location:** City of Scottsdale

**WIP#:** 042054  
**Date:** 5/9/2006

## Rating Curve for Diversion BAS B-4

23 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	64
5	182





# WOOD/PATEL

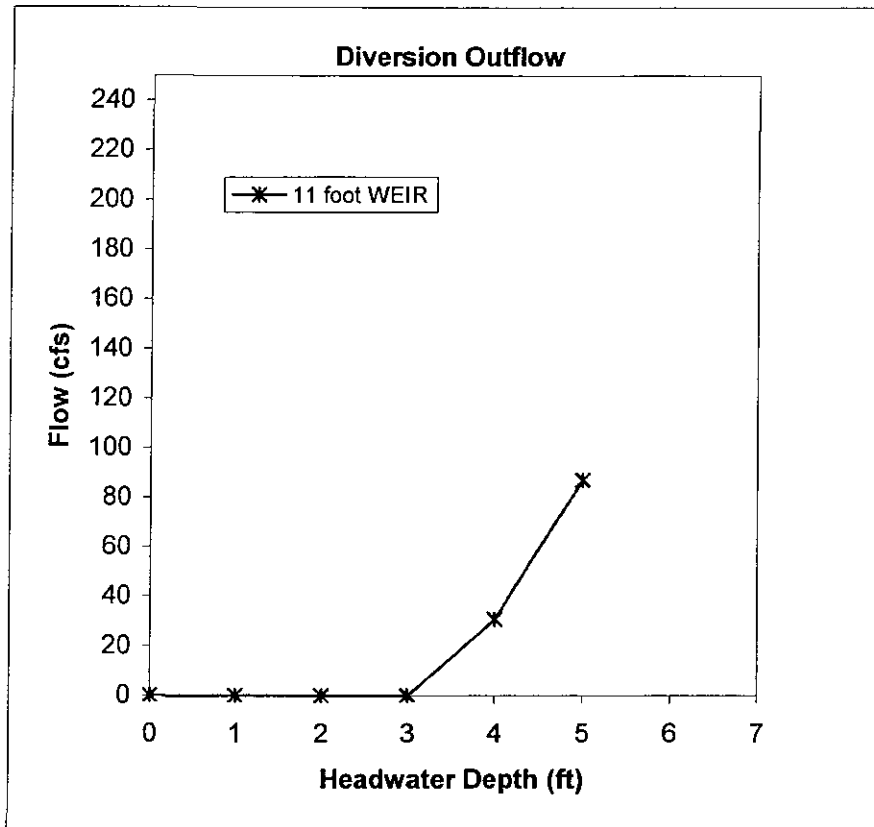
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

**Project:** Sereno Canyon  
**Location:** City of Scottsdale

**WIP#:** 042054  
**Date:** 5/9/2006

## Rating Curve for Diversion BAS C-1

11 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	31
5	87



# WOOD/PATEL

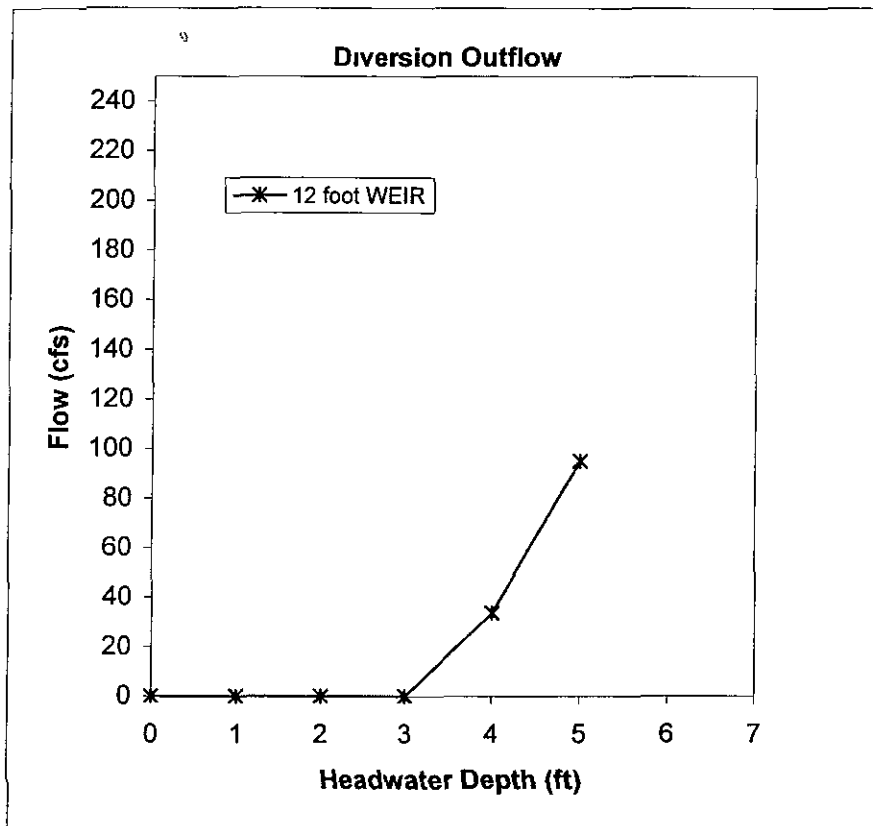
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS D-1

12 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	34
5	95



# WOOD/PATEL

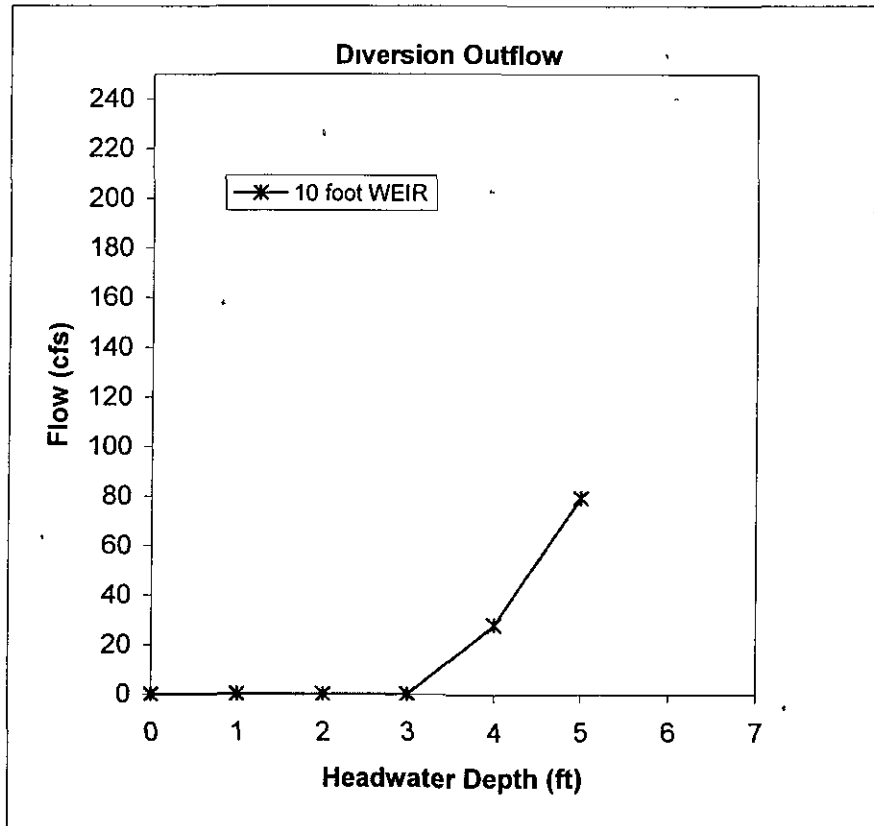
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

**Project:** Sereno Canyon  
**Location:** City of Scottsdale

**WIP#:** 042054  
**Date:** 5/9/2006

## Rating Curve for Diversion BAS D-1

10 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	28
5	79



# WOOD/PATEL

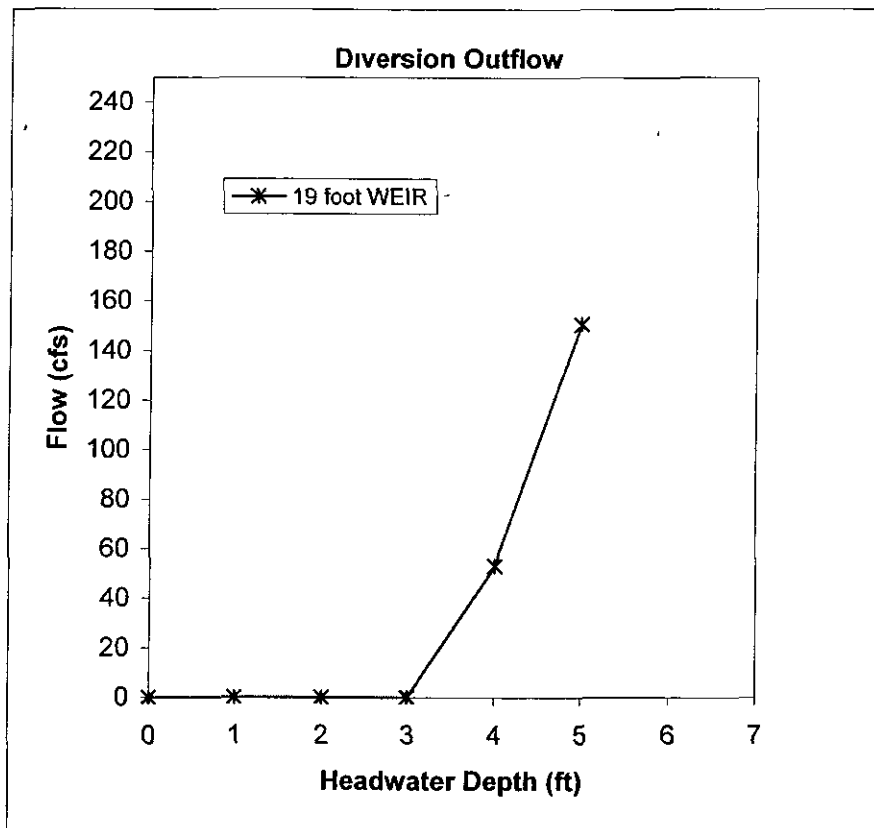
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS E1-2

19	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	53
5	150



# WOOD/PATEL

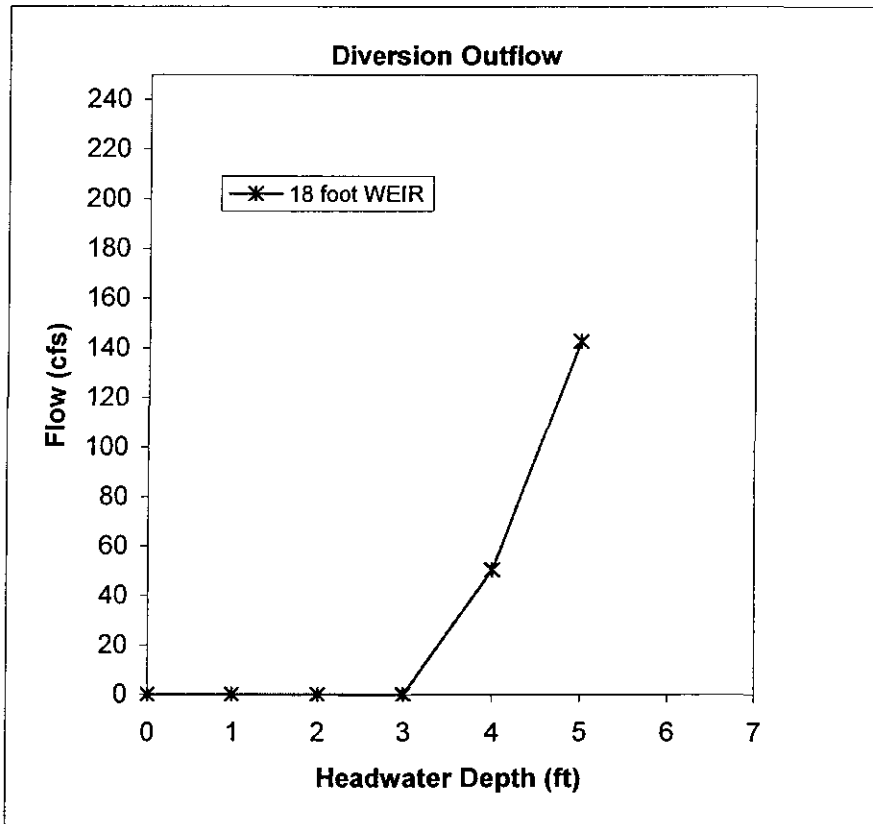
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

**Project:** Sereno Canyon  
**Location:** City of Scottsdale

**WIP#:** 042054  
**Date:** 5/9/2006

## Rating Curve for Diversion BAS E1-3

18 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	50
5	143



# WOOD/PATEL

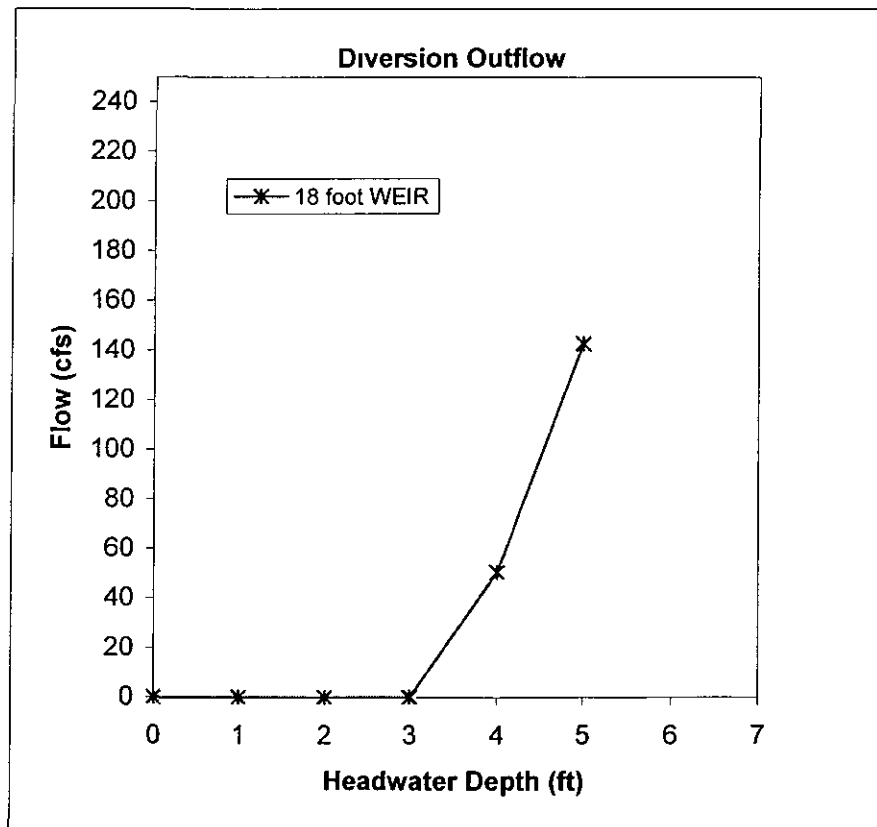
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

**Project:** *Sereno Canyon*  
**Location:** *City of Scottsdale*

**WIP#:** 042054  
**Date:** 5/9/2006

## Rating Curve for Diversion BAS E2-2

18 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	50
5	143



# WOOD/PATEL

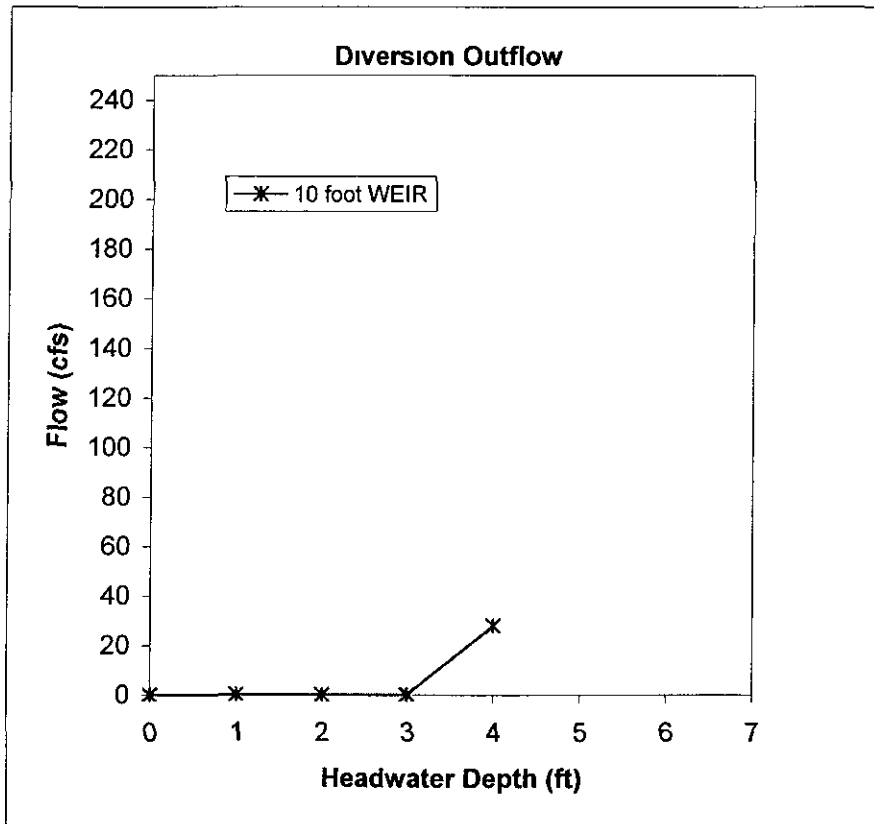
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

**Project:** Sereno Canyon  
**Location:** City of Scottsdale

**WIP#:** 042054  
**Date:** 5/9/2006

## Rating Curve for Diversion BAS E3

10 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	28



# WOOD/PATEL

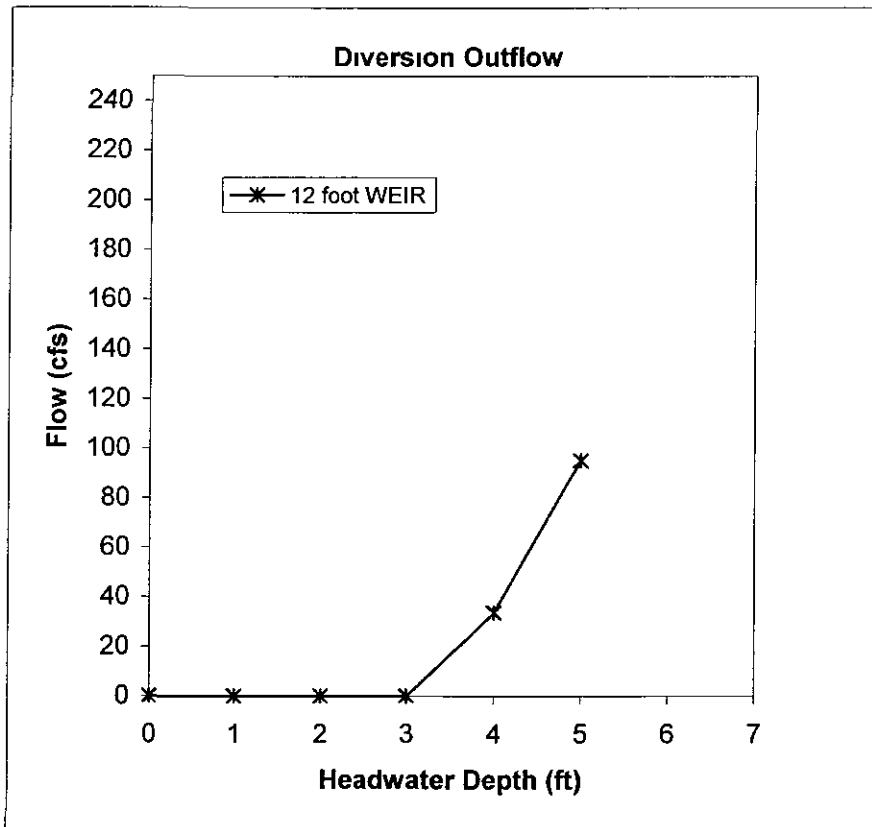
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

W/P#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS F1-3

12 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	34
5	95





# WOOD/PATEL

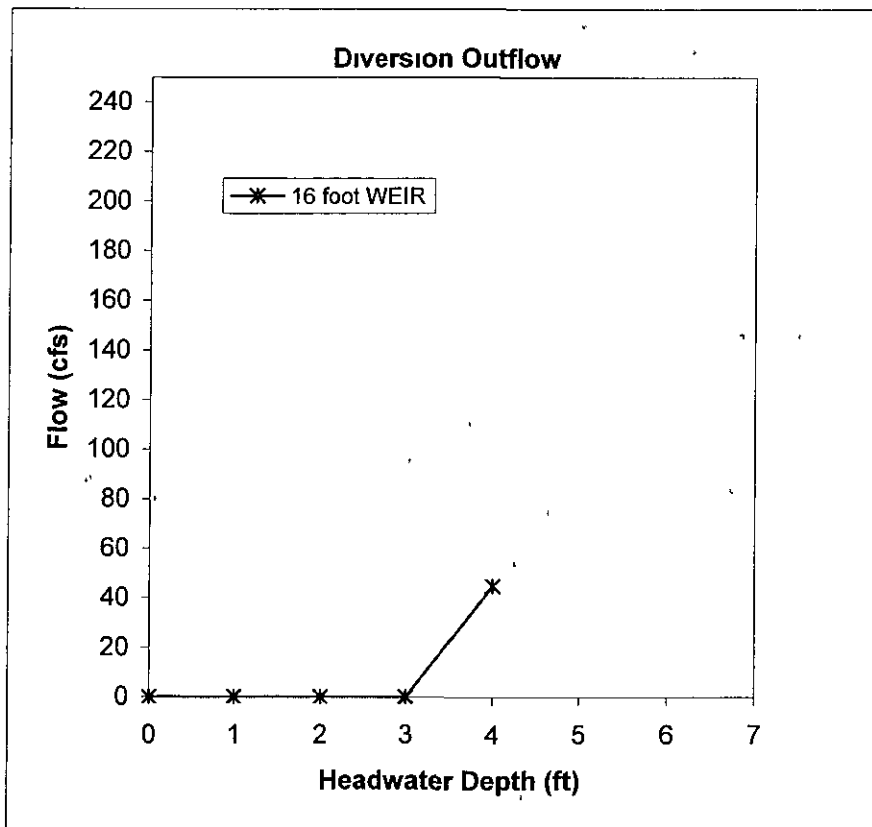
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

WIP#: 042054  
Date: 5/10/2006

## Rating Curve for Diversion BAS F2-1

16	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	45



# WOOD/PATEL

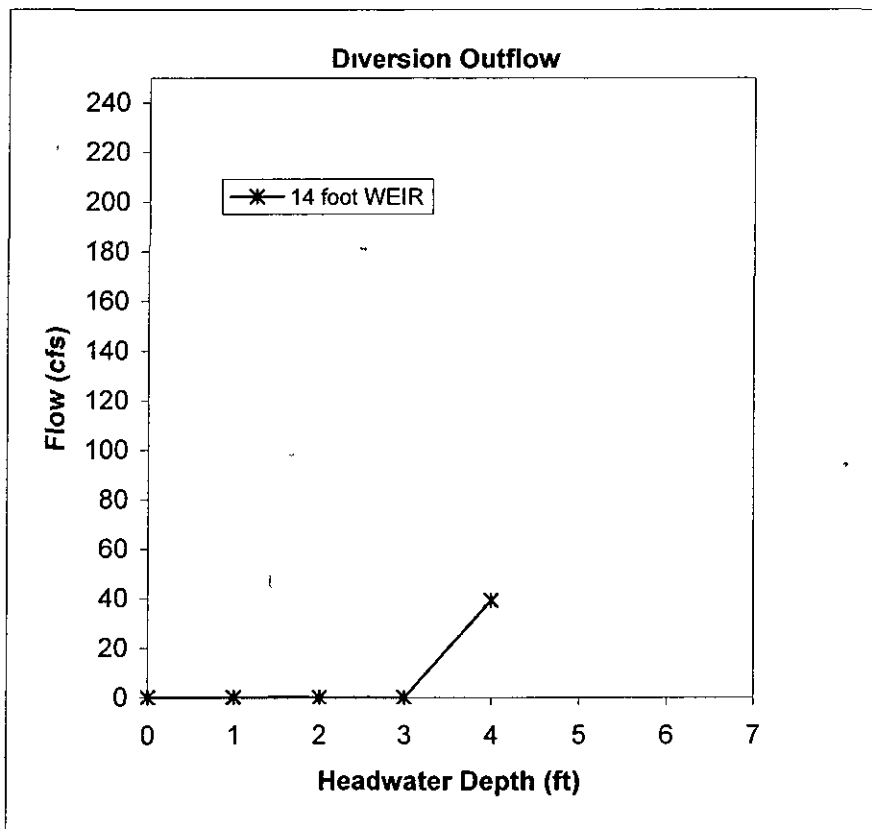
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

W/P#: 042054  
Date: 5/10/2006

## Rating Curve for Diversion BAS F3-2

14 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	39



# WOOD/PATEL

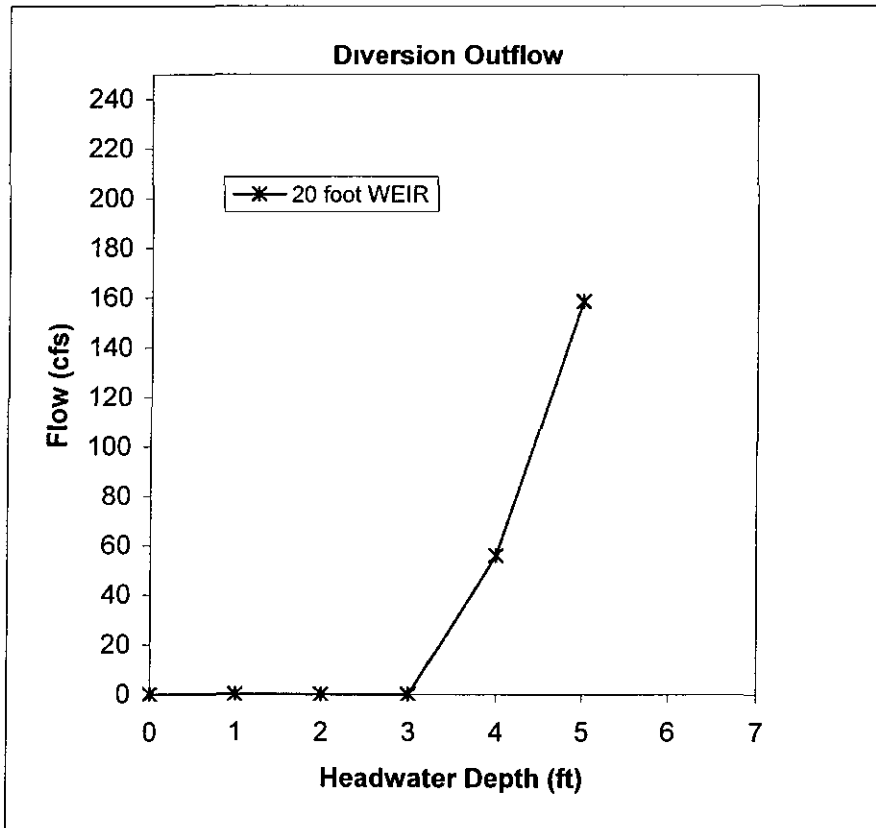
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: Sereno Canyon  
Location: City of Scottsdale

W/P#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS H1-4

20 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	56
5	158



# WOOD/PATEL

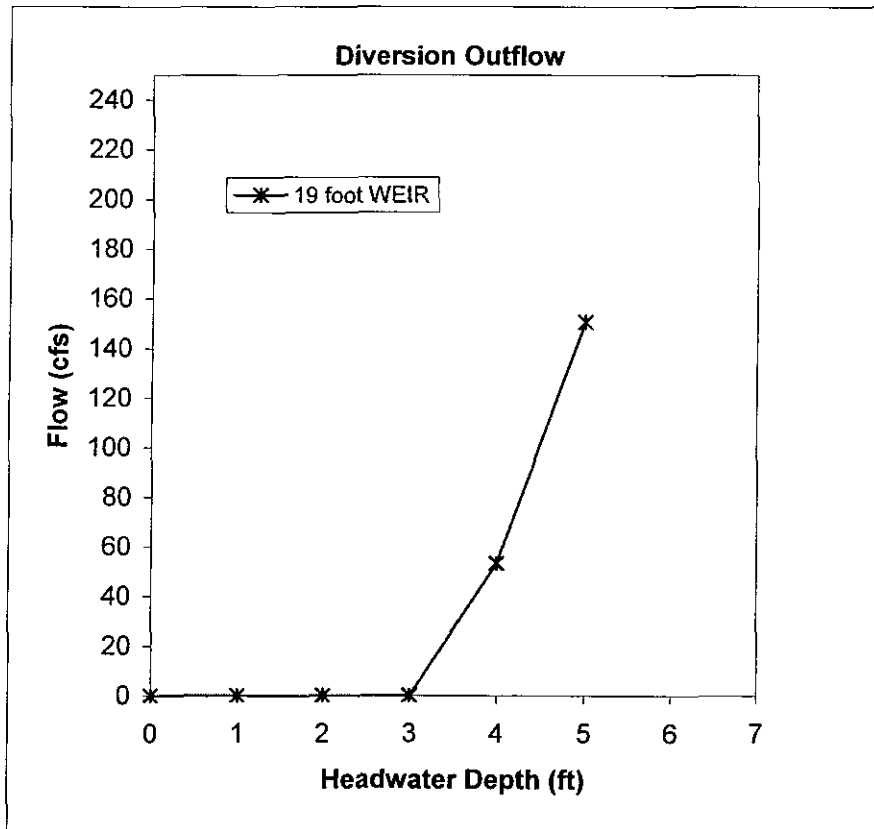
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: Sereno Canyon  
Location: City of Scottsdale

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS H1-3

19 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	53
5	150



# WOOD/PATEL

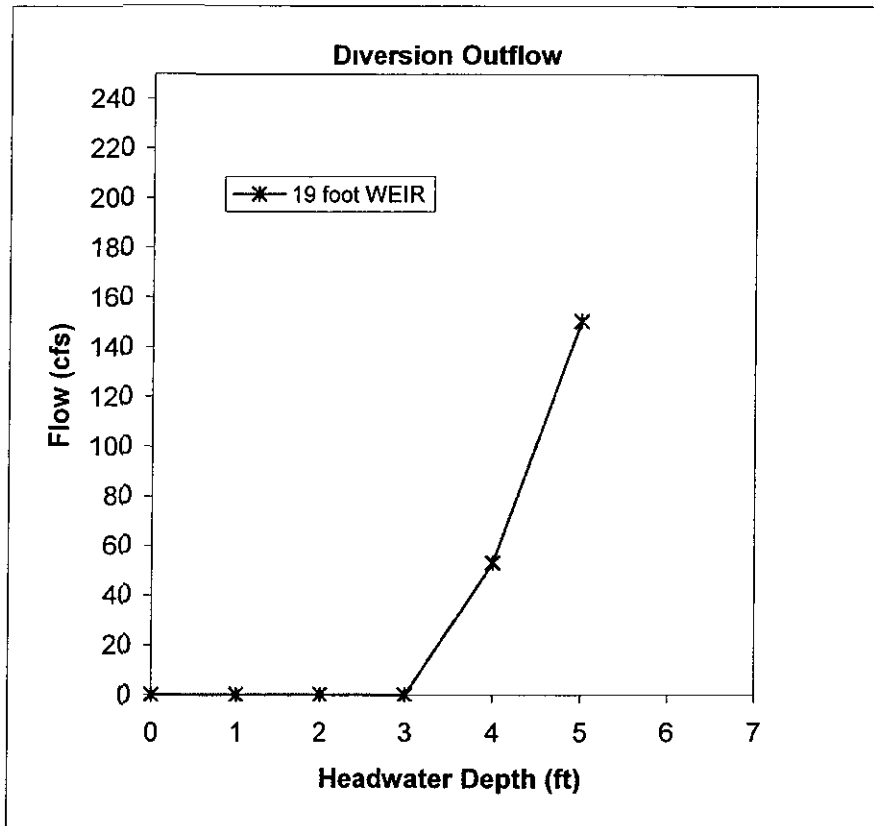
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS H1-5

19	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	53
5	150



# WOOD/PATEL

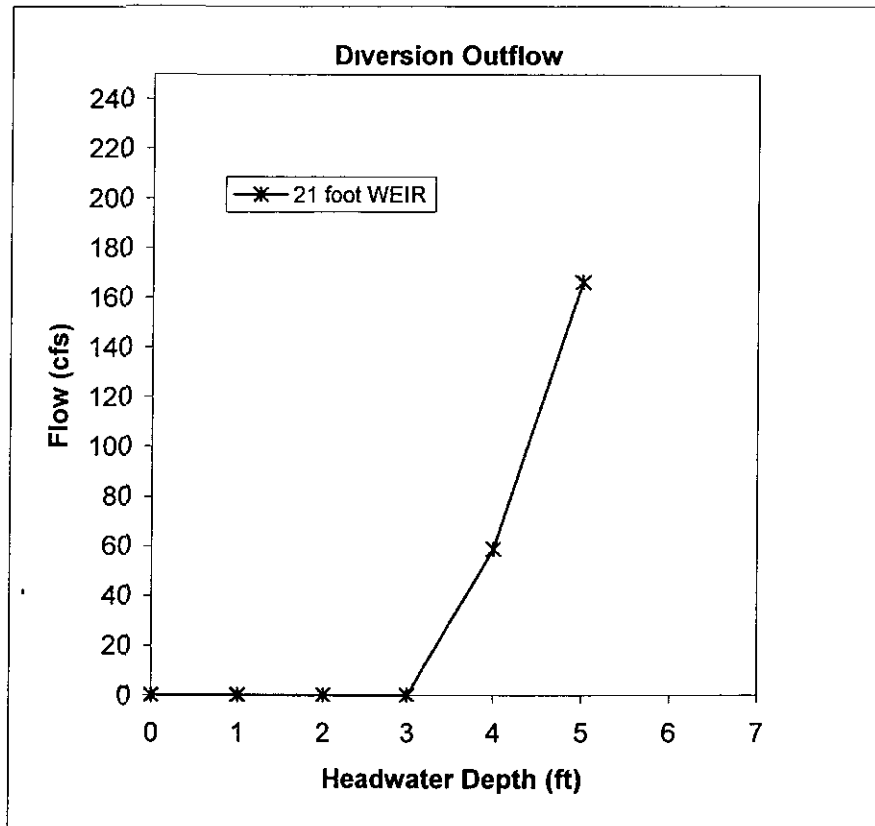
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS H2-1

21	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	59
5	166



# WOOD/PATEL

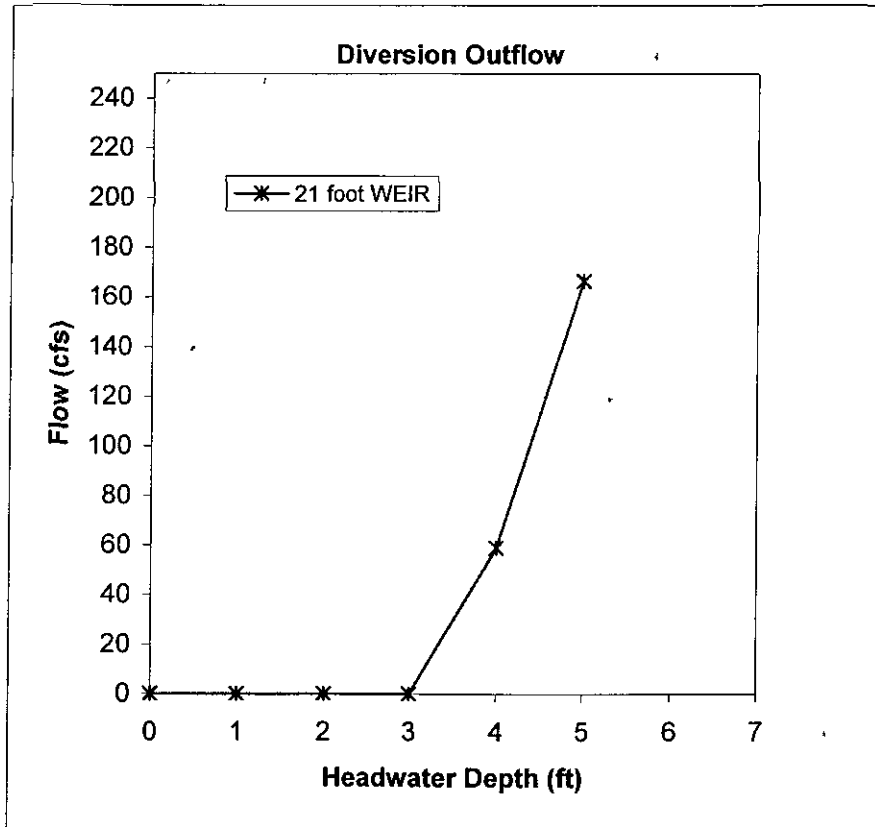
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS H2-2

21 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	59
5	166



# WOOD/PATEL

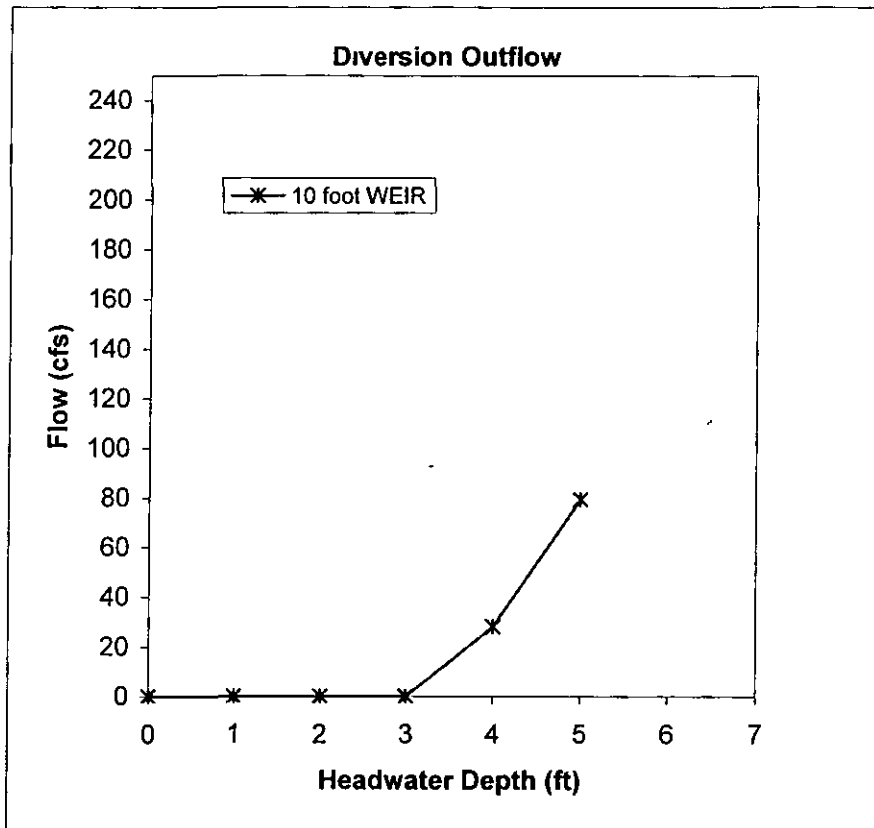
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: Sereno Canyon  
Location: City of Scottsdale

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS I

10 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	28
5	79





# WOOD/PATEL

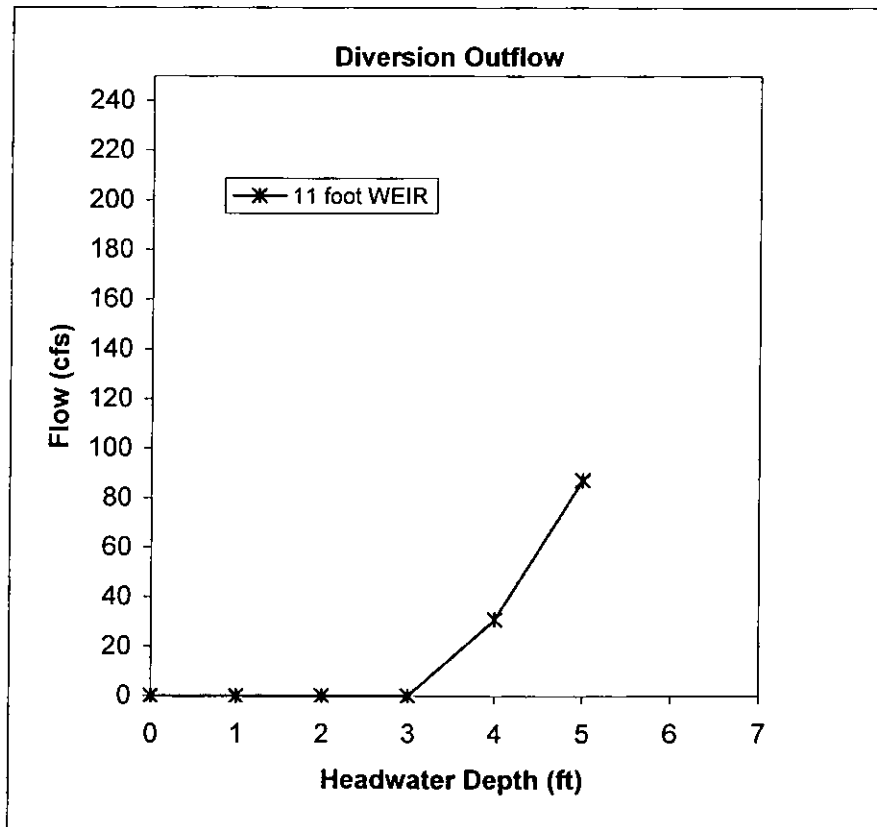
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: Sereno Canyon  
Location: City of Scottsdale

WIP#. 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS J

11 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	31
5	87



# WOOD/PATEL

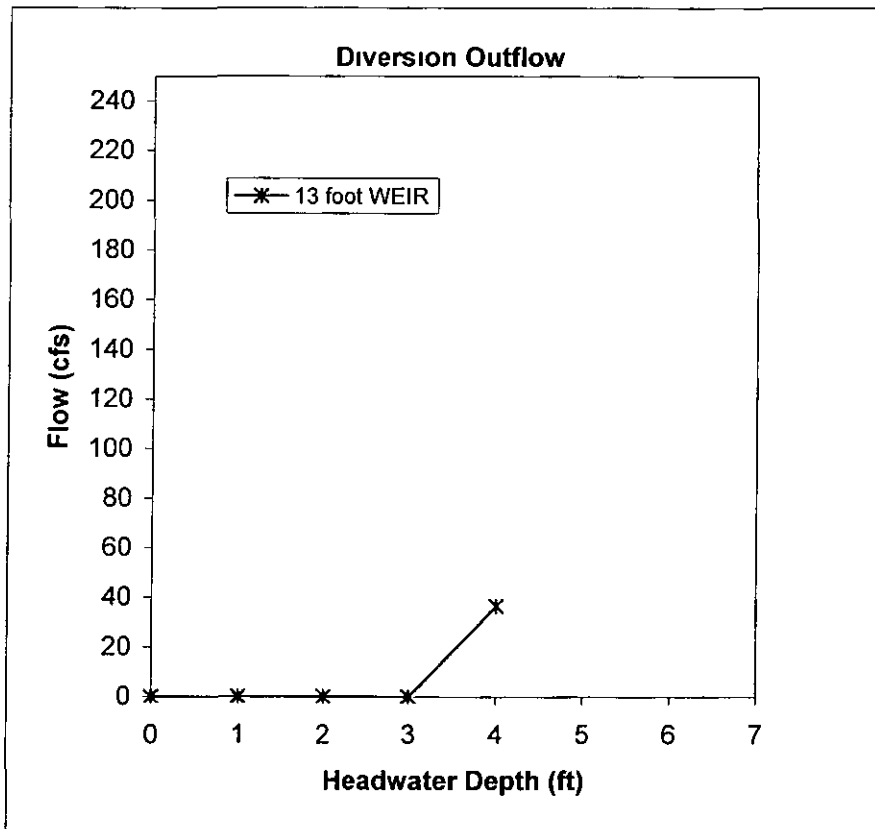
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: Sereno Canyon  
Location: City of Scottsdale

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS K

13	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	36



# WOOD/PATEL

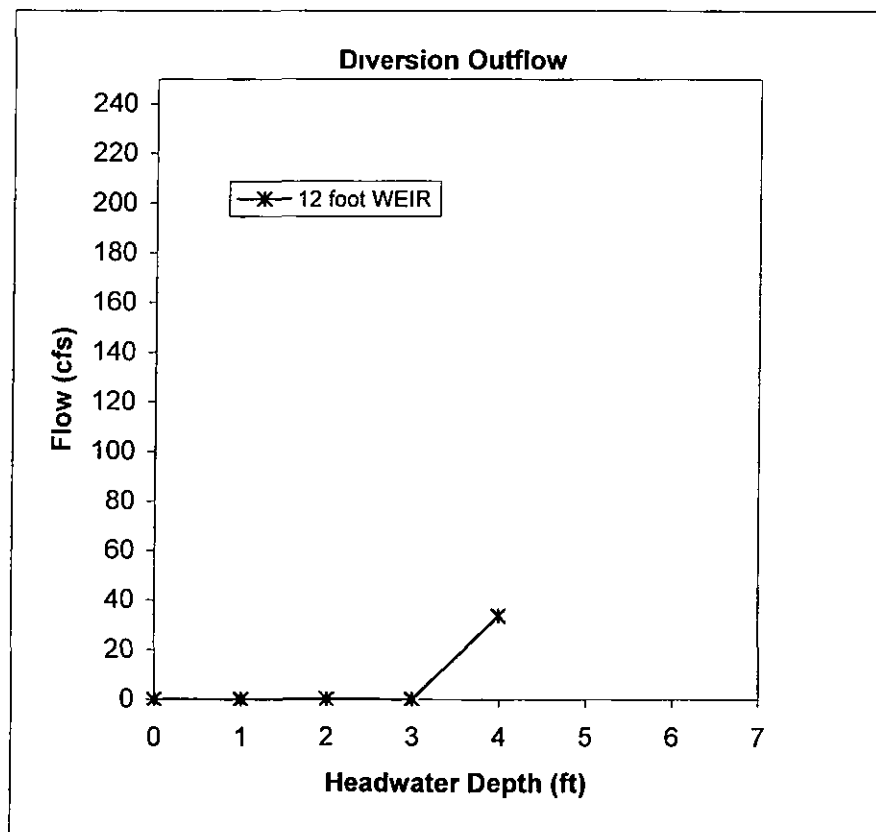
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS L

12 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	34



# WOOD/PATEL

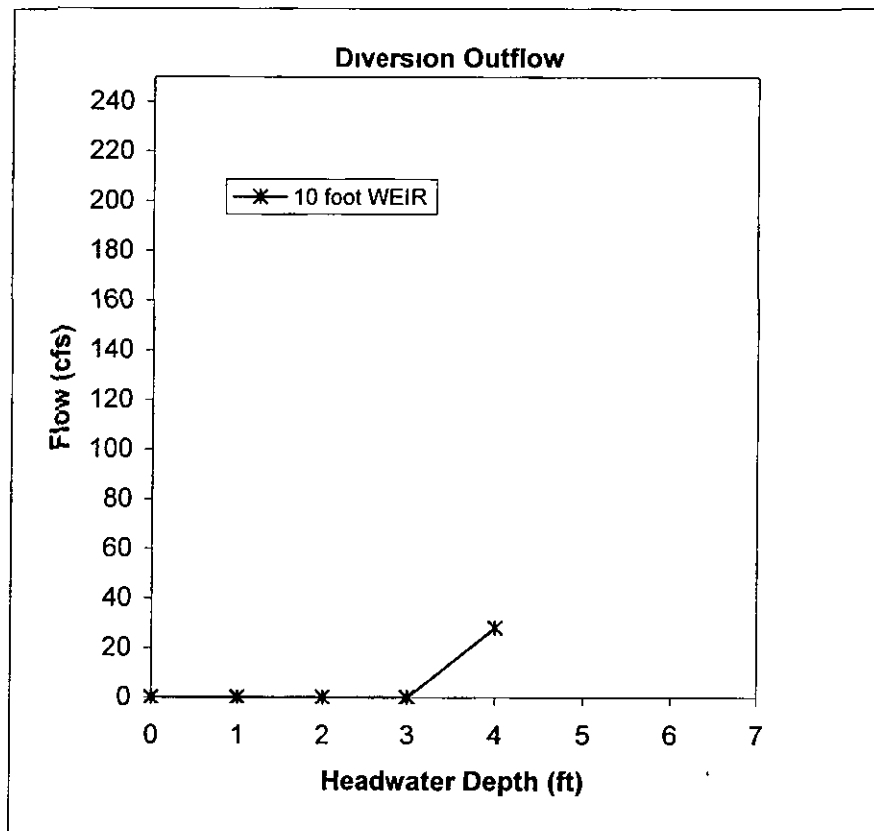
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: Sereno Canyon  
Location: City of Scottsdale

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS M

10 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	28



# WOOD/PATEL

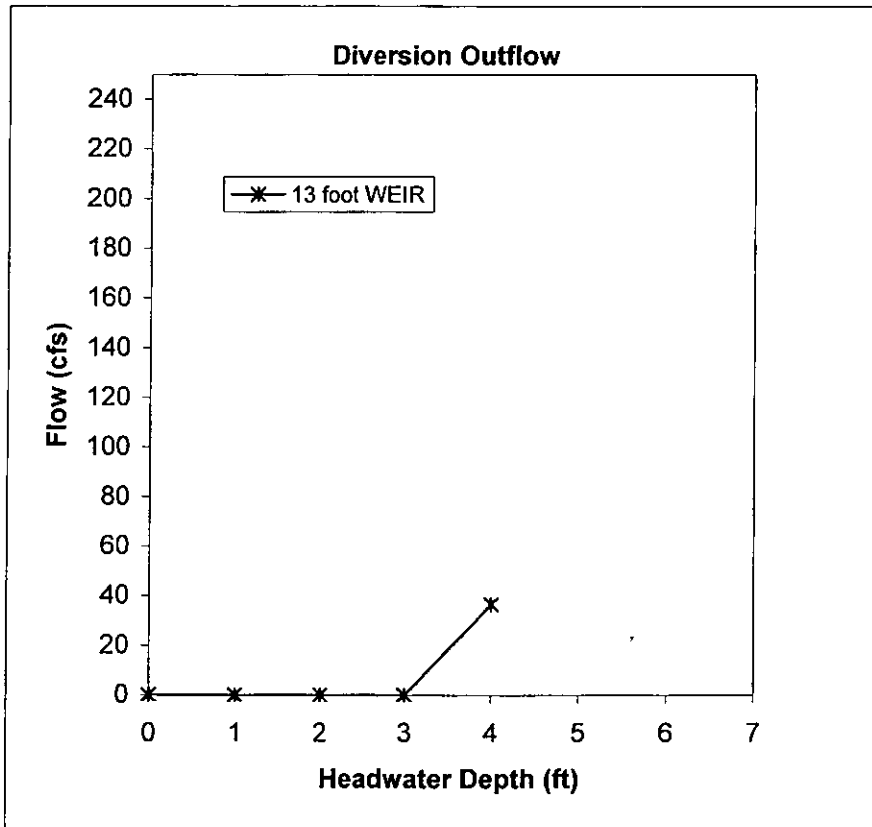
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Sereno Canyon*  
Location: *City of Scottsdale*

WIP#: 042054  
Date: 5/9/2006

## Rating Curve for Diversion BAS N

13 foot WEIR	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	36

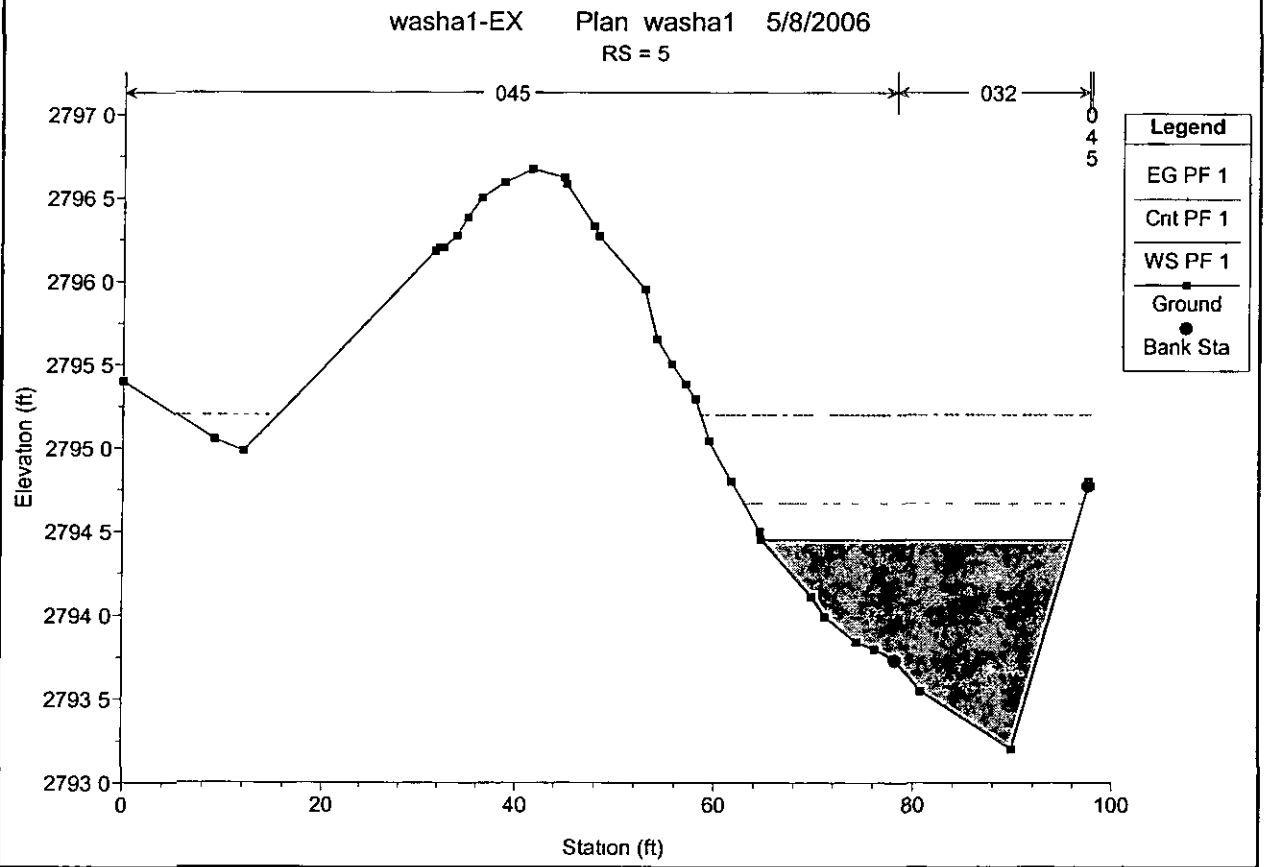
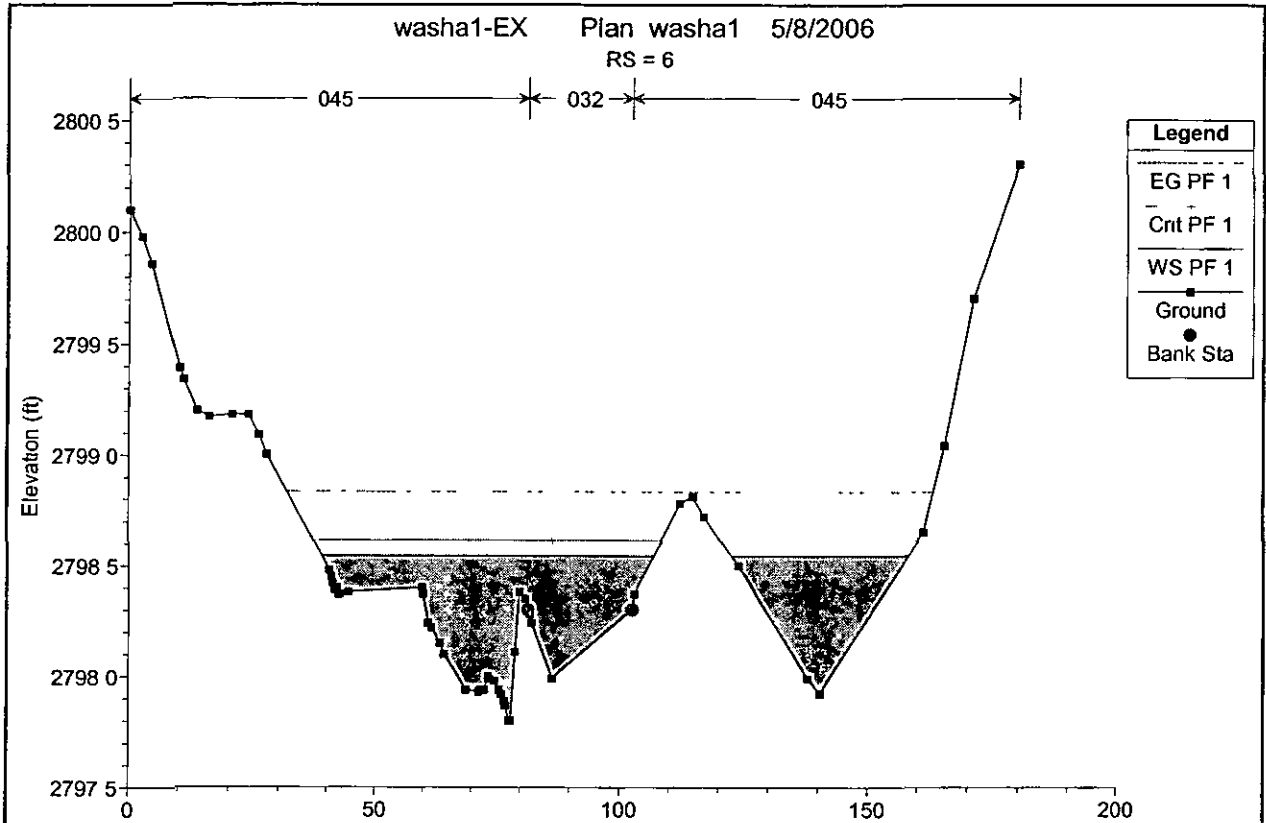


**HEC-RAS Output Files**

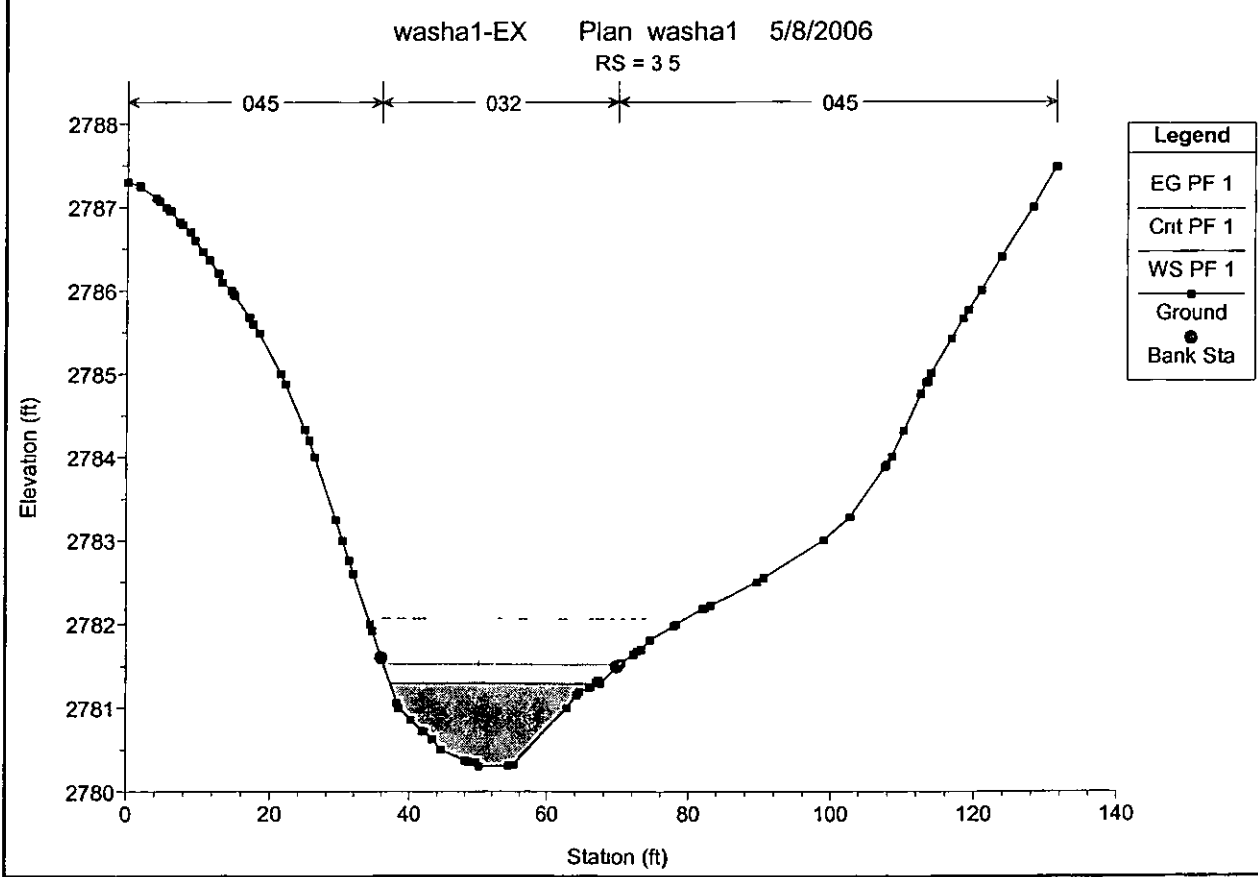
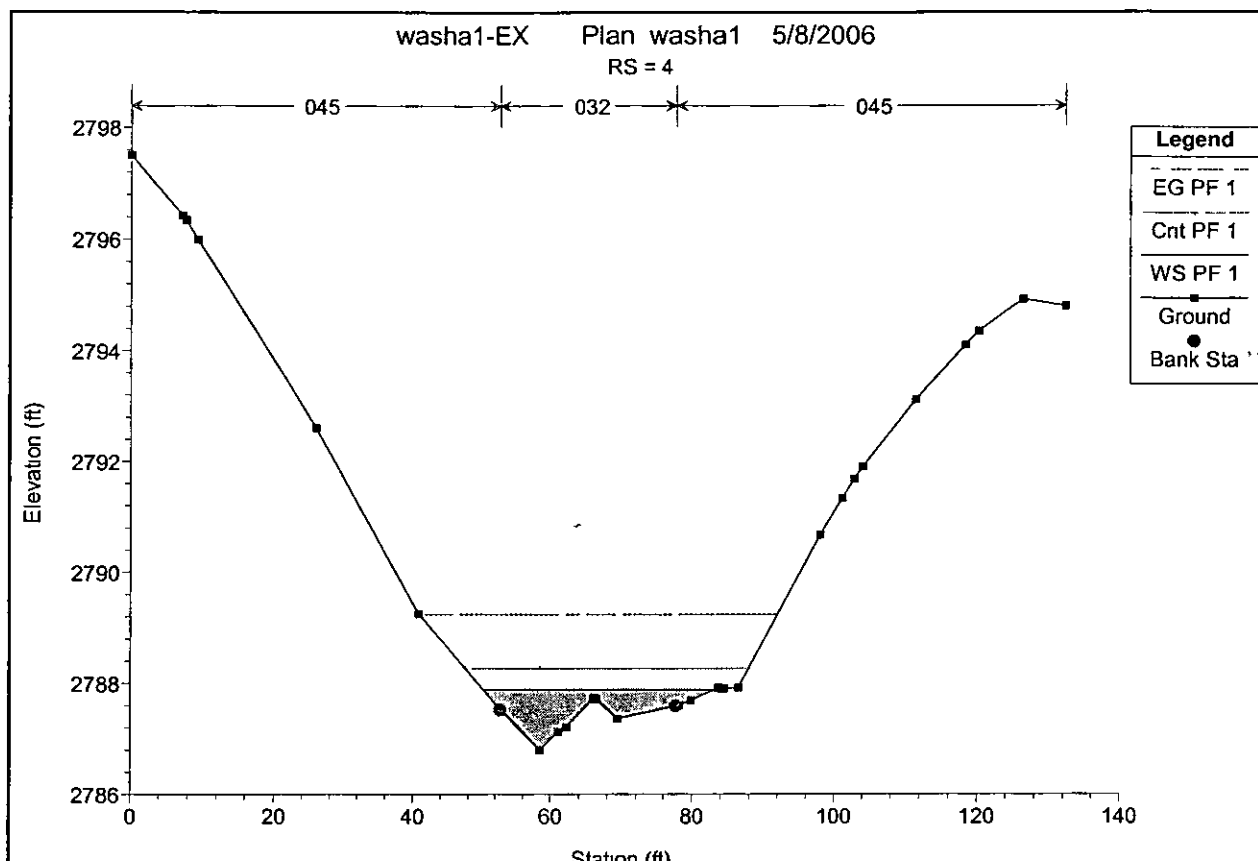
# WASH AI

HEC-RAS Plan wasa1 River RIVER 1 Reach Reach-1 Profile PF 1

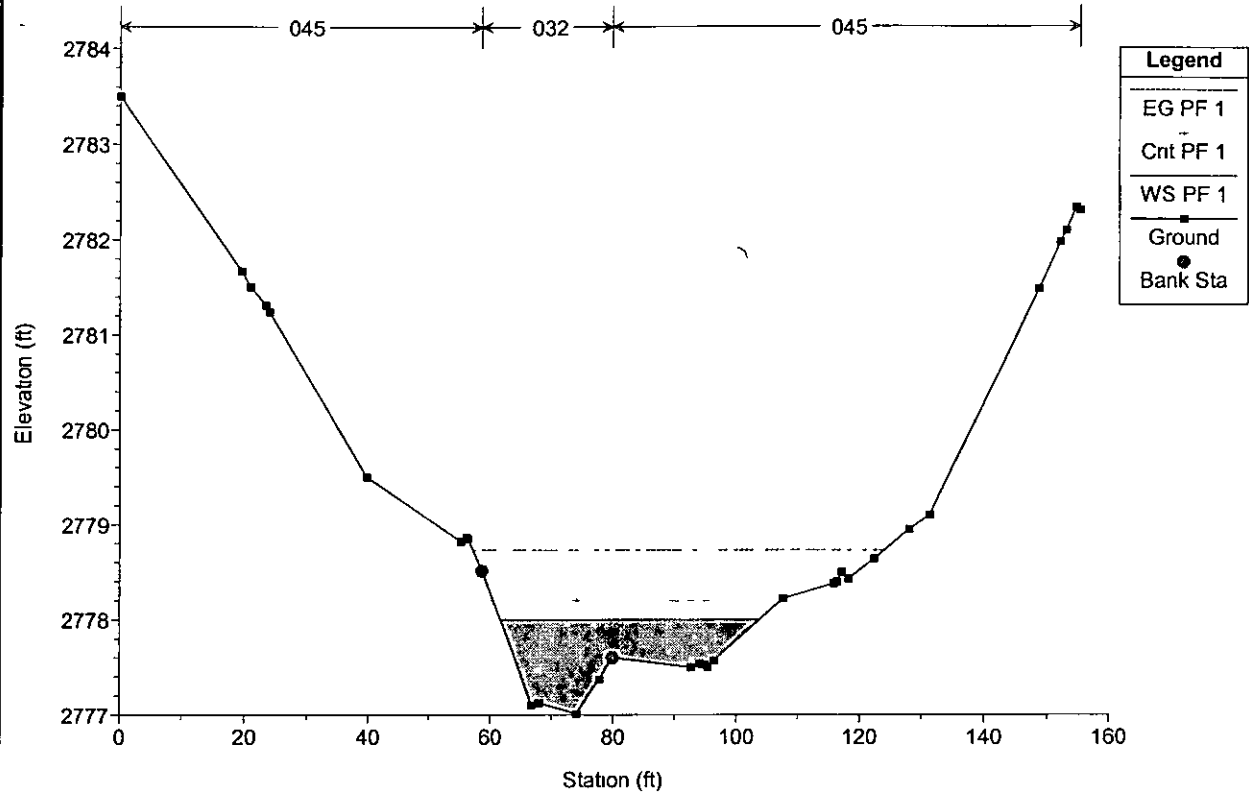
Reach	River Sta	Profile	TQ Total (cfs)	Min Ch El (ft)	W S Elev (ft)	Crit W S (ft)	E G Elev (ft)	E G Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	6	PF_1	133.00	2797.99	2798.54	2798.61	2798.83	0.050016	5.63	33.66	103.73	1.57
Reach-1	5	PF_1	133.00	2793.20	2794.45	2794.67	2795.20	0.029978	7.35	21.29	31.22	1.38
Reach-1	4	PF_1	133.00	2786.79	2787.89	2788.26	2789.23	0.093236	9.42	15.00	33.03	2.25
Reach-1	3.5	PF_1	133.00	2780.30	2781.30	2781.53	2782.07	0.042079	7.04	18.89	29.62	1.55
Reach-1	3	PF_1	133.00	2777.01	2778.00	2778.20	2778.73	0.041792	7.60	22.37	42.27	1.58
Reach-1	2.5	PF_1	133.00	2772.00	2772.83	2773.19	2773.87	0.067610	8.23	16.36	31.78	1.93
Reach-1	2	PF_1	225.00	2761.68	2763.49	2764.10	2765.36	0.052876	11.96	24.91	36.49	1.92
Reach-1	1.5	PF_1	225.00	2759.00	2760.13	2760.26	2760.62	0.033317	5.58	40.31	75.30	1.34
Reach-1	1	PF_1	225.00	2753.00	2754.28	2754.64	2755.47	0.048122	9.80	31.86	57.99	1.78



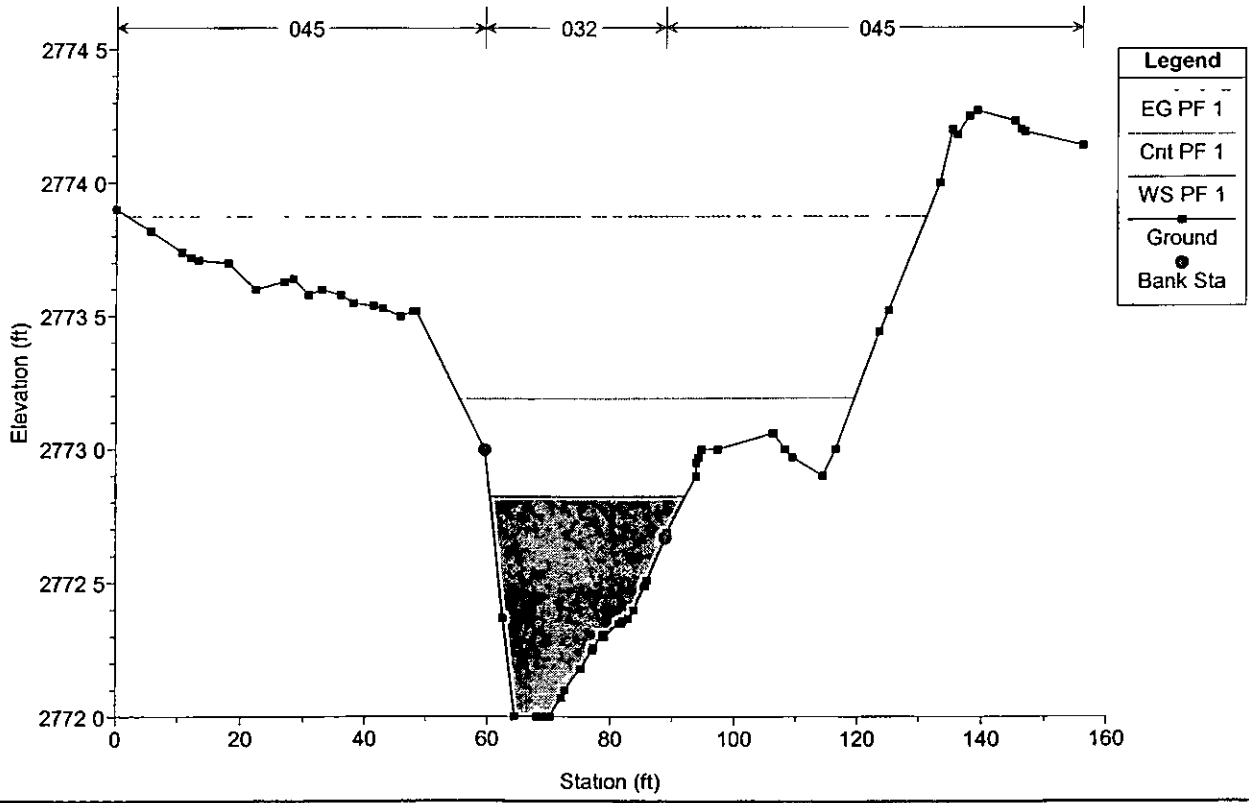


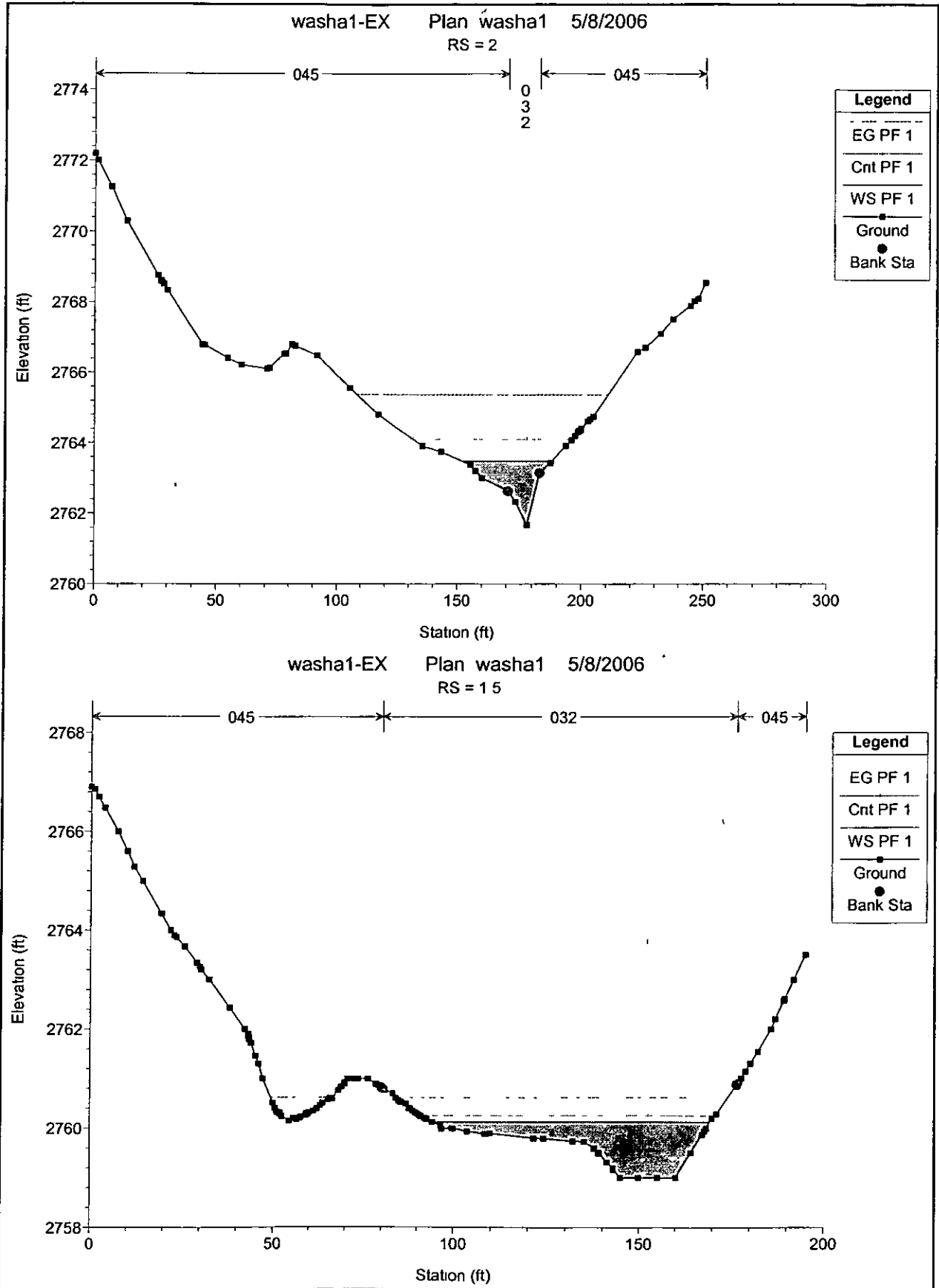


washa1-EX Plan washa1 5/8/2006  
RS = 3

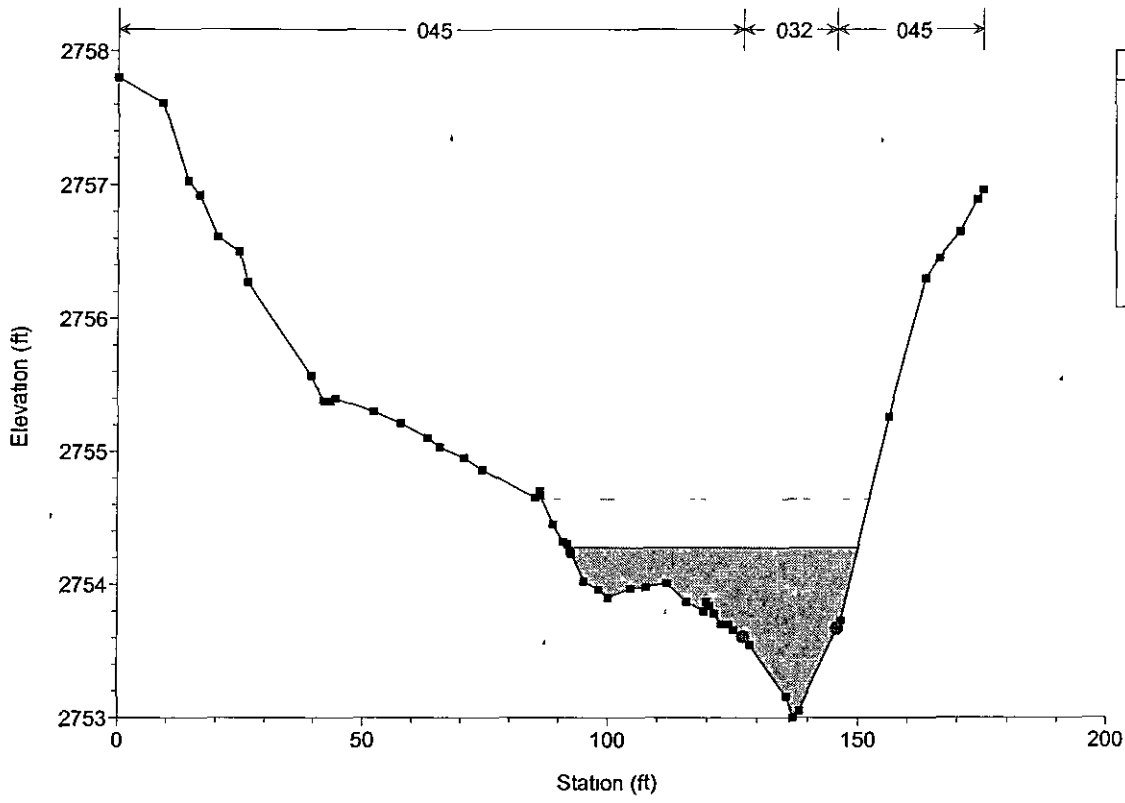


washa1-EX Plan washa1 5/8/2006  
RS = 25





washa1-EX Plan washa1 5/8/2006  
RS = 1

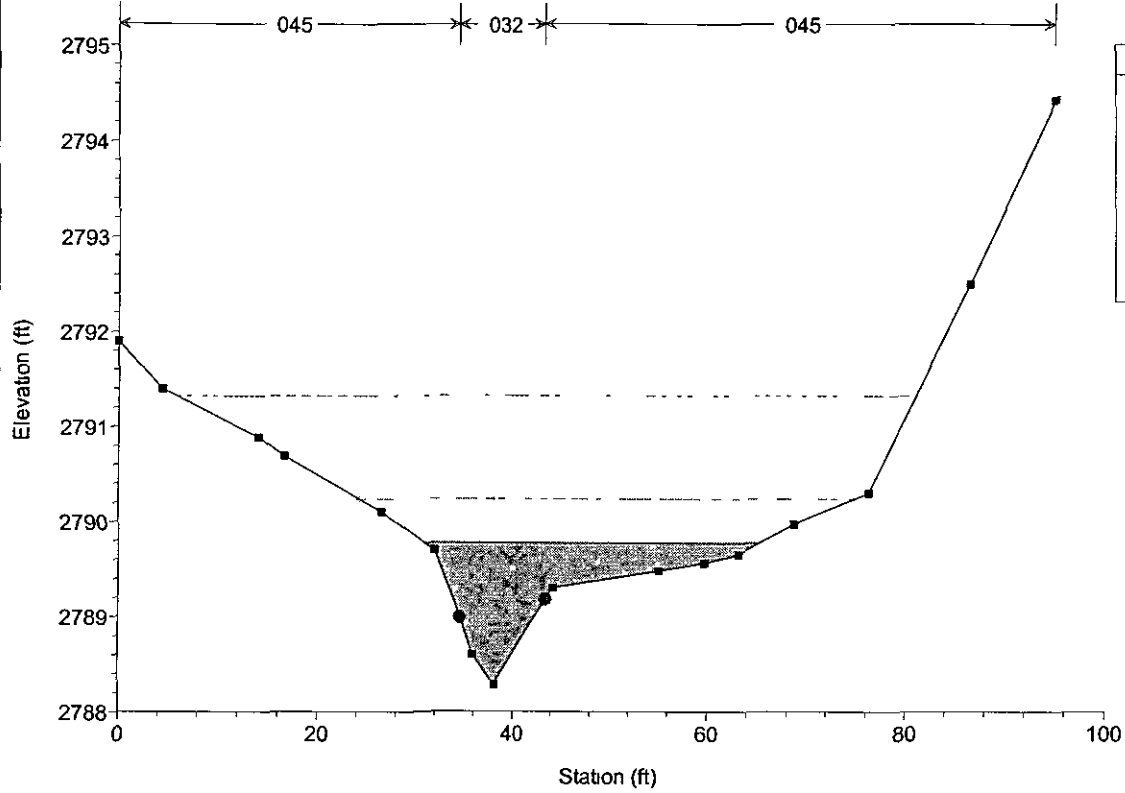


# WASH A1-1

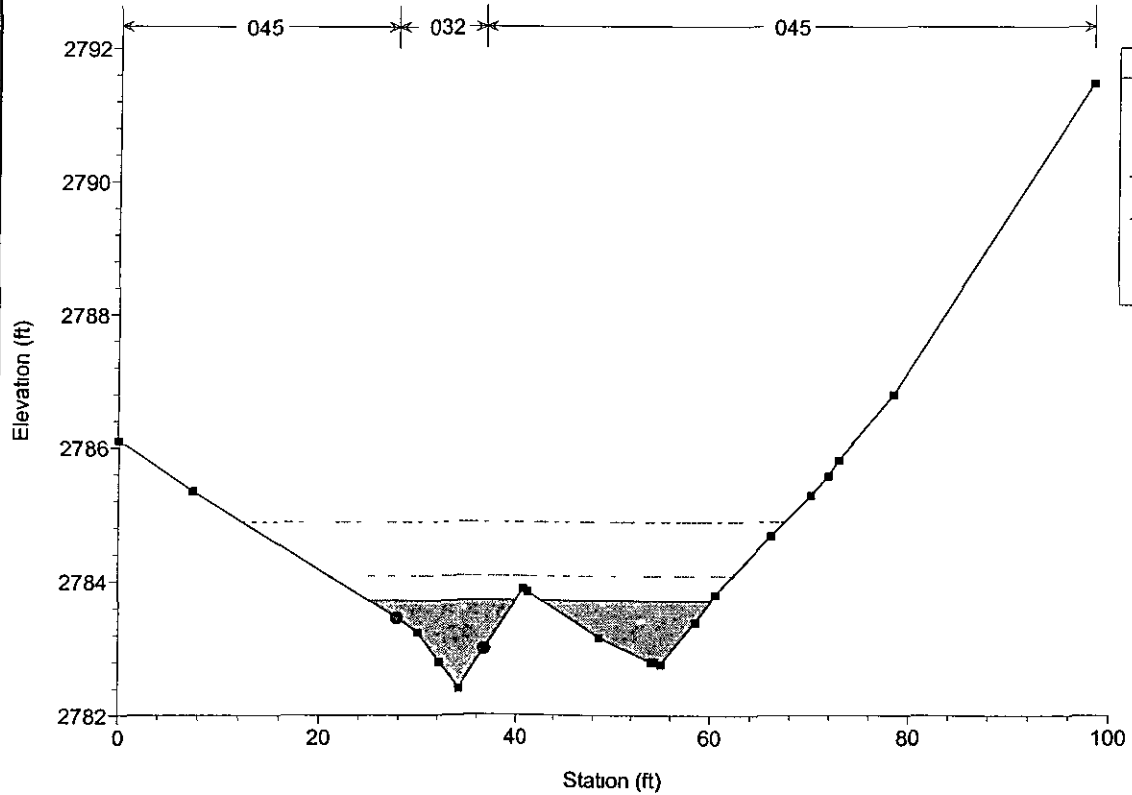
HEC-RAS Plan Plan 02 River RIVER-1 Reach Reach-1 Profile PF 1

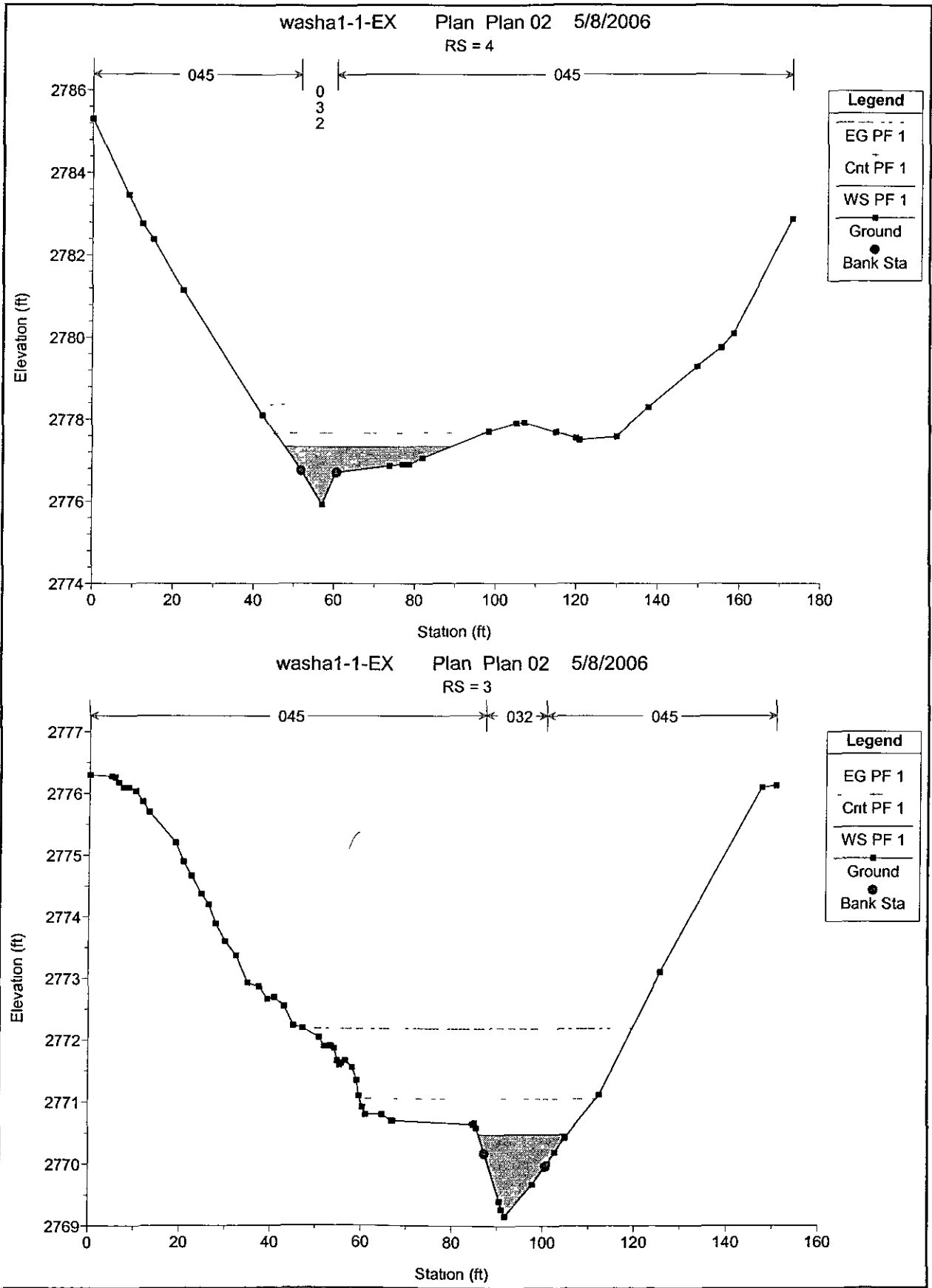
Reach	River Sta	Profile	Q Total (cfs)	Min Ch Elev (ft)	W S Elev (ft)	Crit W S (ft)	E G Elev (ft)	E G Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	6	PF_1	133.00	2788.28	2789.77	2790.23	2791.32	0.050087	10.97	17.44	34.33	1.84
Reach-1	5	PF_1	133.00	2782.40	2783.72	2784.08	2784.89	0.068076	10.46	17.98	32.56	2.04
Reach-1	4	PF_1	133.00	2775.92	2777.33	2777.66	2778.36	0.045547	9.75	21.33	41.25	1.72
Reach-1	3	PF_1	133.00	2769.16	2770.48	2771.05	2772.19	0.061709	10.66	13.50	19.67	1.98
Reach-1	2.5	PF_1	133.00	2766.00	2767.19	2767.51	2768.15	0.057608	7.85	16.99	29.16	1.79
Reach-1	2	PF_1	133.00	2761.84	2763.45	2763.69	2764.23	0.028466	7.15	19.39	28.35	1.34
Reach-1	1.5	PF_1	133.00	2759.00	2759.88	2760.04	2760.39	0.048351	5.71	23.31	55.71	1.55
Reach-1	1	PF_1	225.00	2753.00	2754.35	2754.62	2755.30	0.035741	8.97	36.07	59.94	1.55

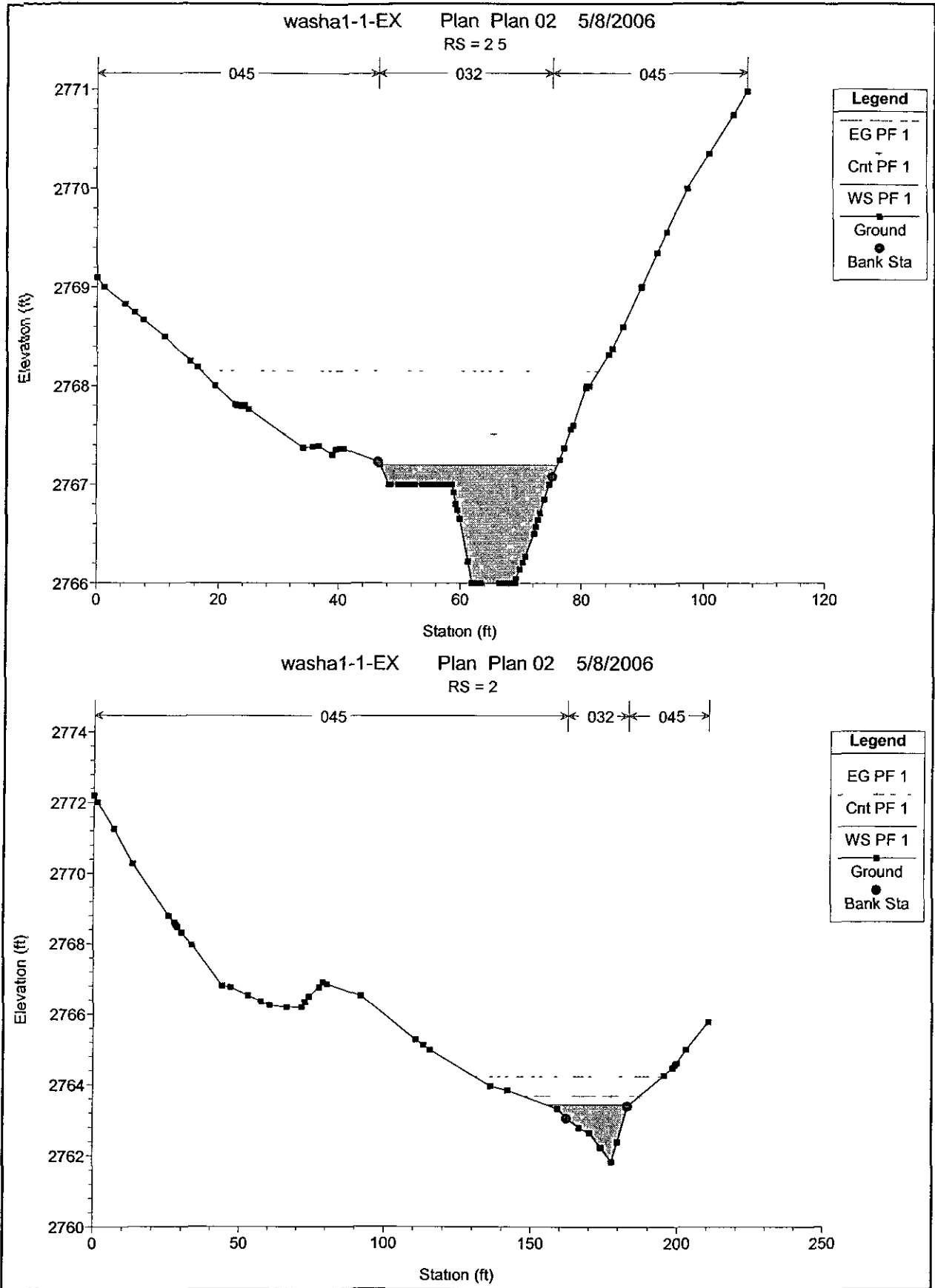
washa1-1-EX Plan Plan 02 5/8/2006  
RS = 6



washa1-1-EX Plan Plan 02 5/8/2006  
RS = 5

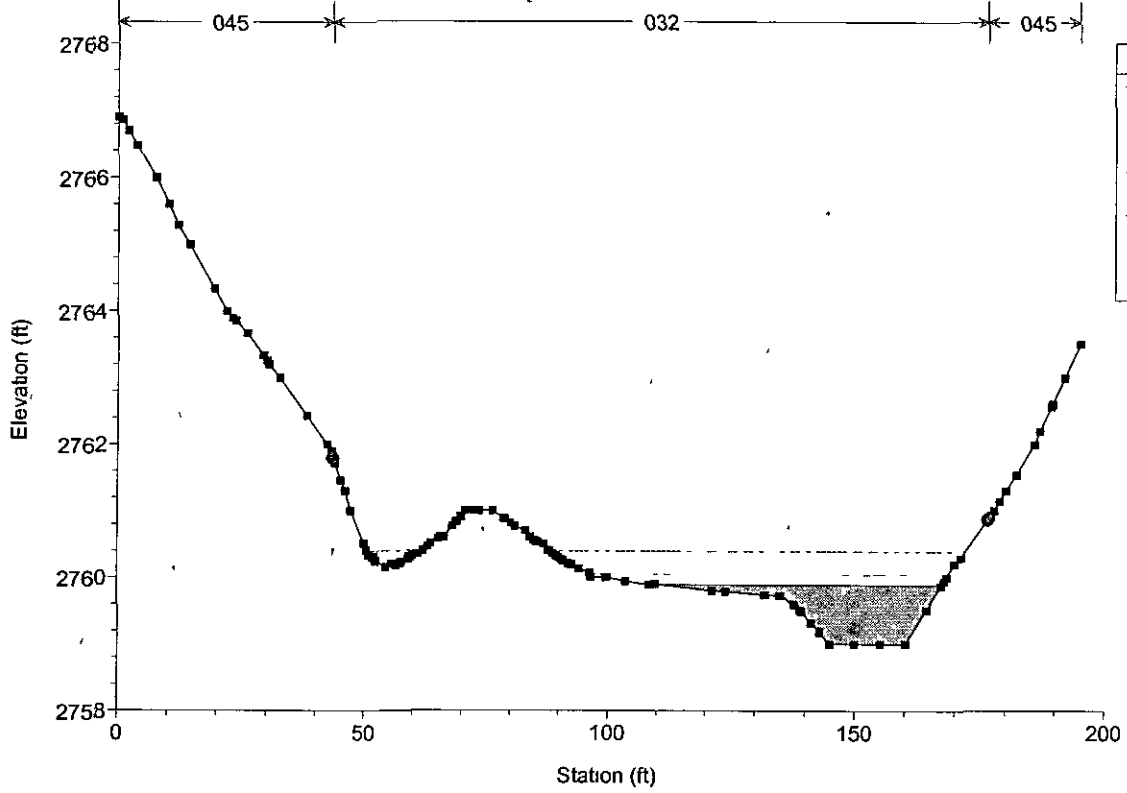




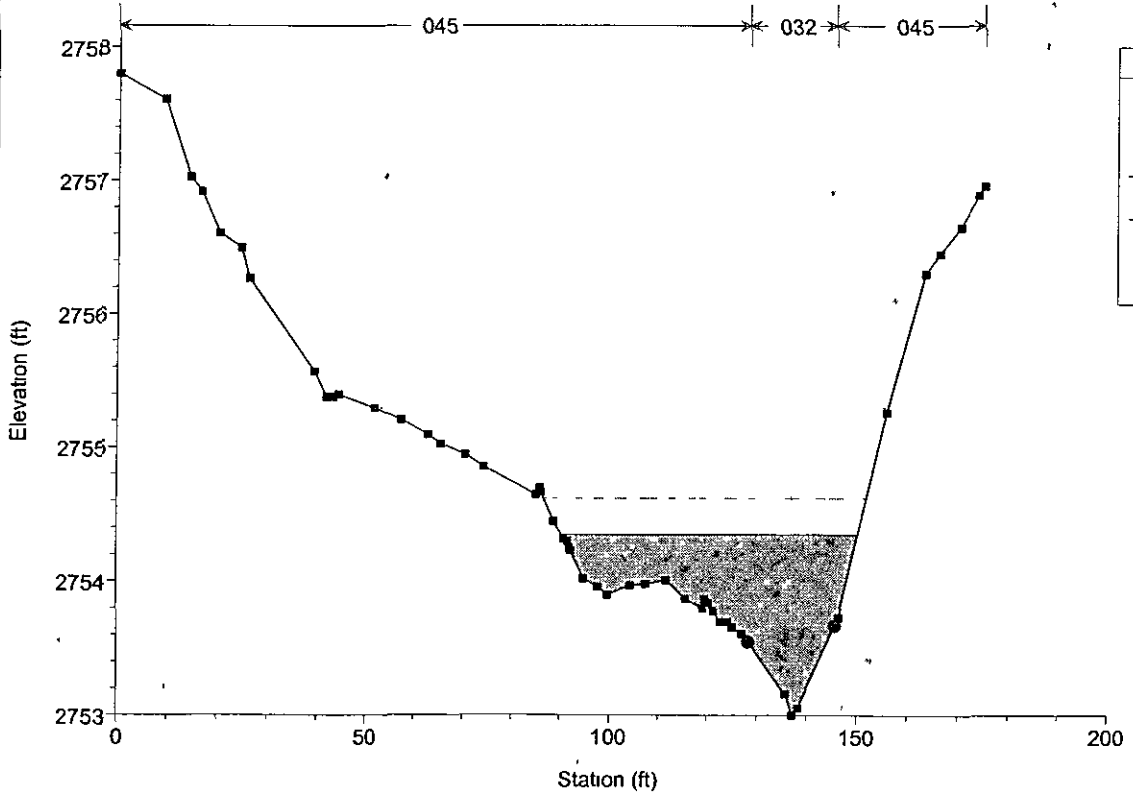




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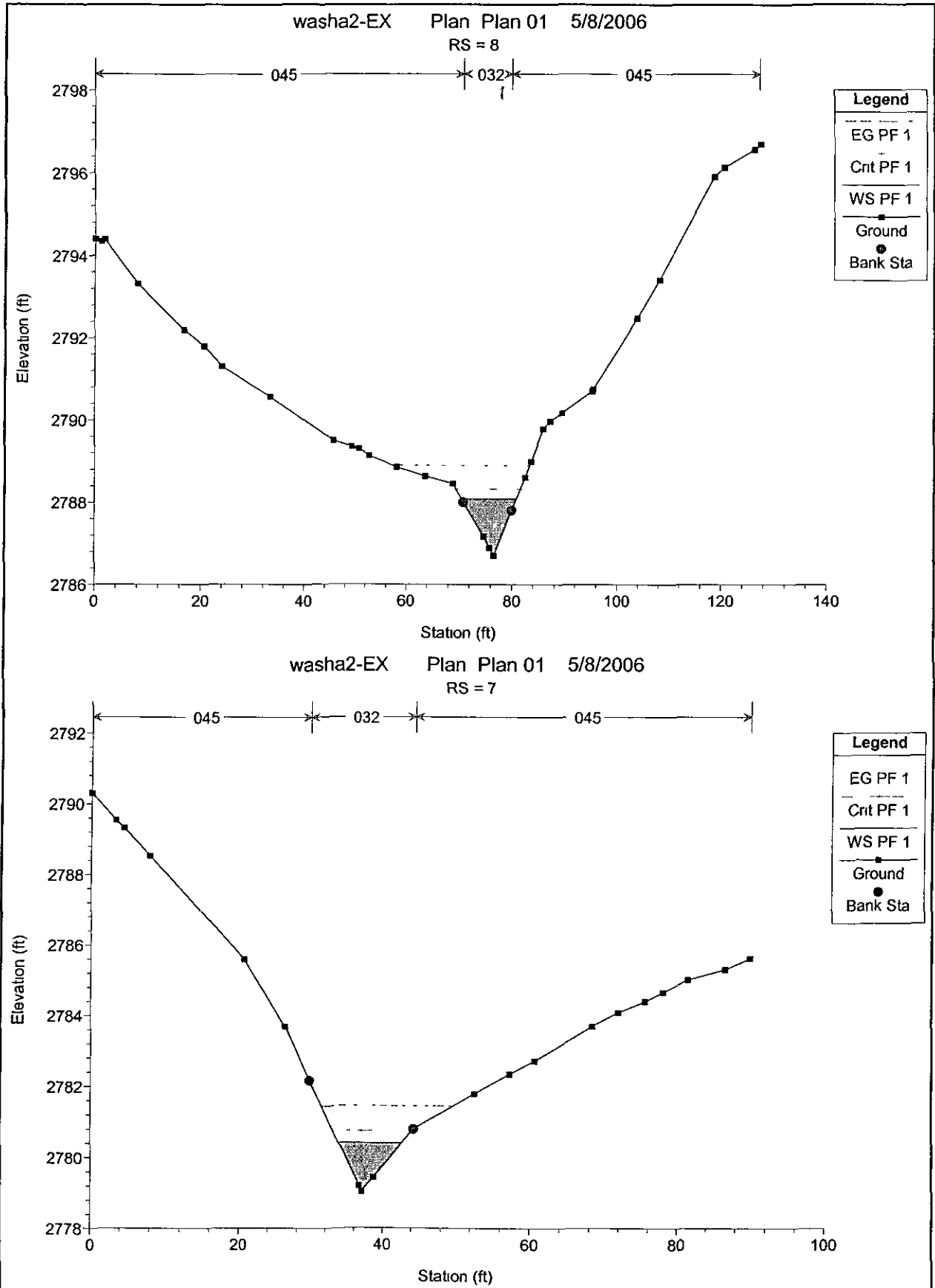
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RS = 1



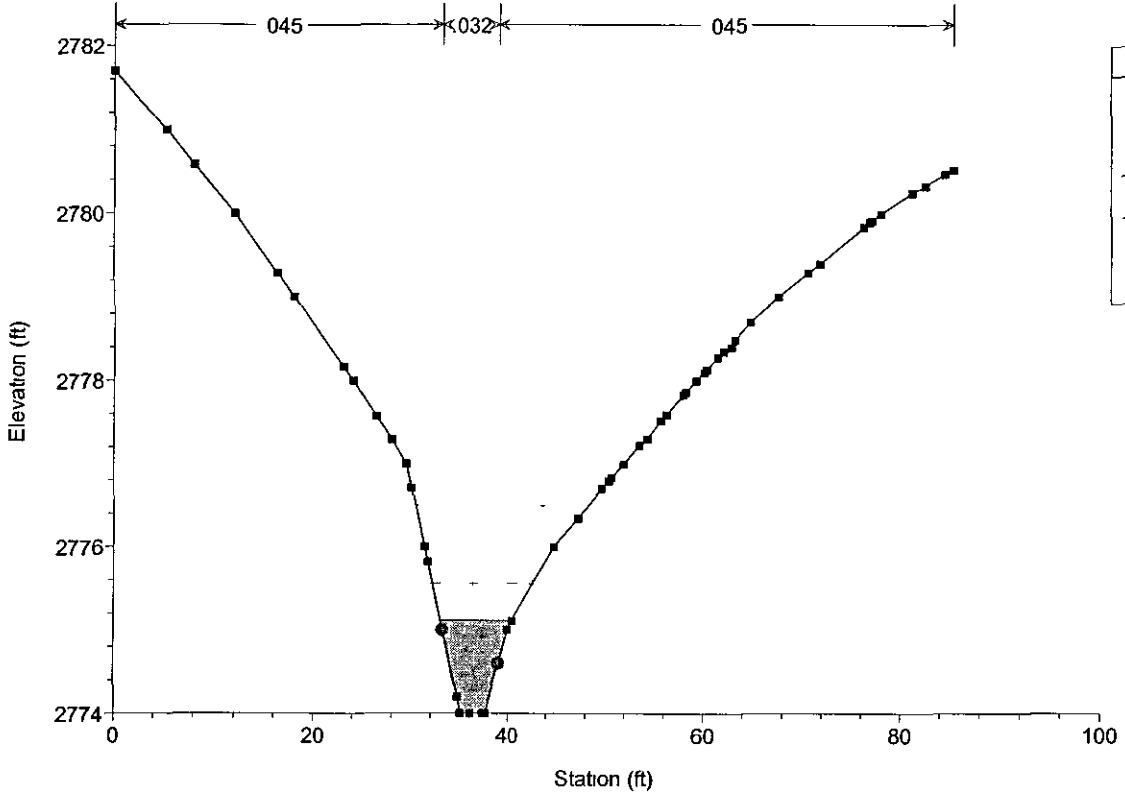
# WASH A2

HEC-RAS Plan Plan 01 River RIVER-1 Reach Reach 1 Profile PF 1

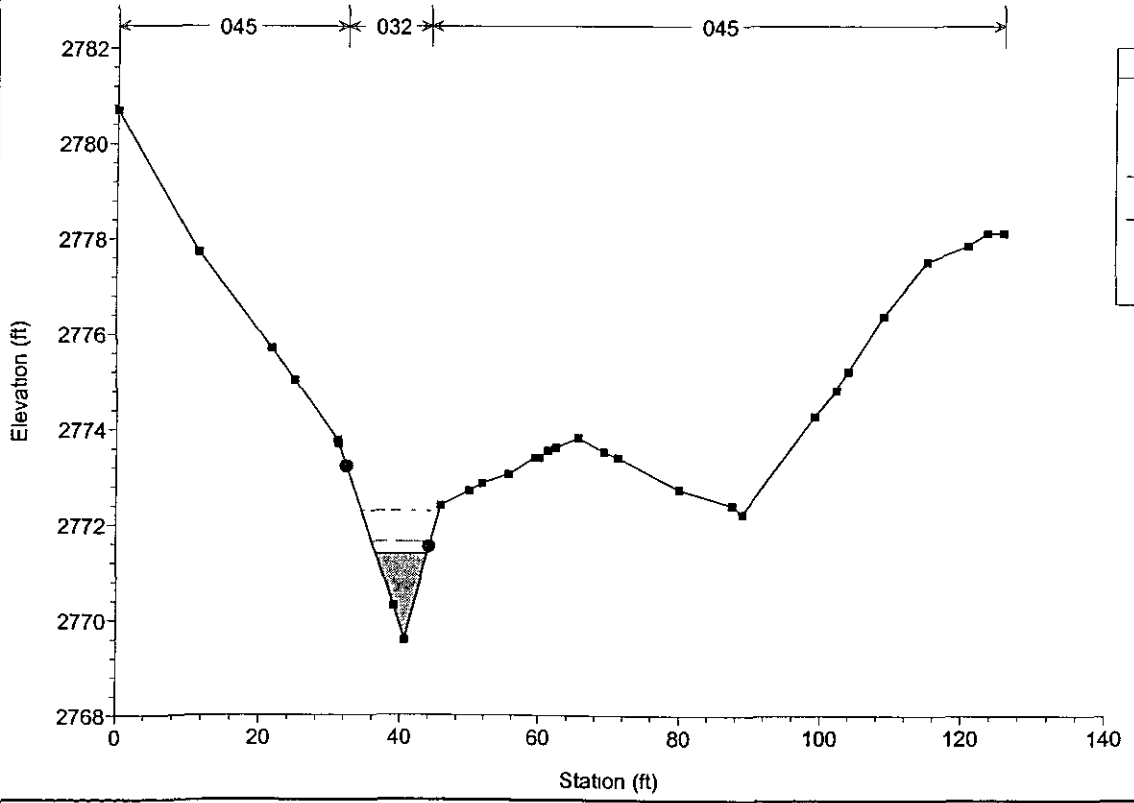
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W S Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	6	PF 1	50 00	2786 70	2788 08	2788 32	2788 89	0 037052	7 25	7 00	10 29	1 47
Reach-1	7	PF 1	50 00	2779 03	2780 41	2780 75	2781 45	0 054930	8 20	6 09	8 86	1 74
Reach-1	6 5	PF 1	50 00	2774 00	2775 12	2775 56	2776 49	0 053771	9 50	5 51	7 56	1 78
Reach-1	6	PF 1	50 00	2769 58	2771 38	2771 63	2772 27	0 035829	7 60	6 58	7 35	1 41
Reach-1	5 5	PF 1	50 00	2766 00	2767 37	2767 82	2768 78	0 045155	9 98	5 95	8 05	1 70
Reach-1	5	PF 1	65 00	2760 85	2762 45	2762 98	2764 14	0 060416	10 47	6 36	7 80	1 89
Reach-1	4	PF 1	65 00	2754 30	2756 01	2756 26	2756 93	0 034820	7 73	8 44	9 82	1 44
Reach-1	3	PF 1	65 00	2749 02	2750 53	2750 80	2751 42	0 041151	7 55	8 61	11 61	1 55
Reach-1	2	PF 1	103 00	2742 10	2743 49	2743 84	2744 51	0 040565	8 21	13 15	20 48	1 59
Reach-1	1 5	PF 1	103 00	2739 00	2740 24	2740 46	2740 96	0 023639	6 94	17 39	33 29	1 24
Reach-1	1	PF 1	103 00	2733 30	2734 58	2735 00	2735 99	0 080509	9 58	10 96	20 87	2 13

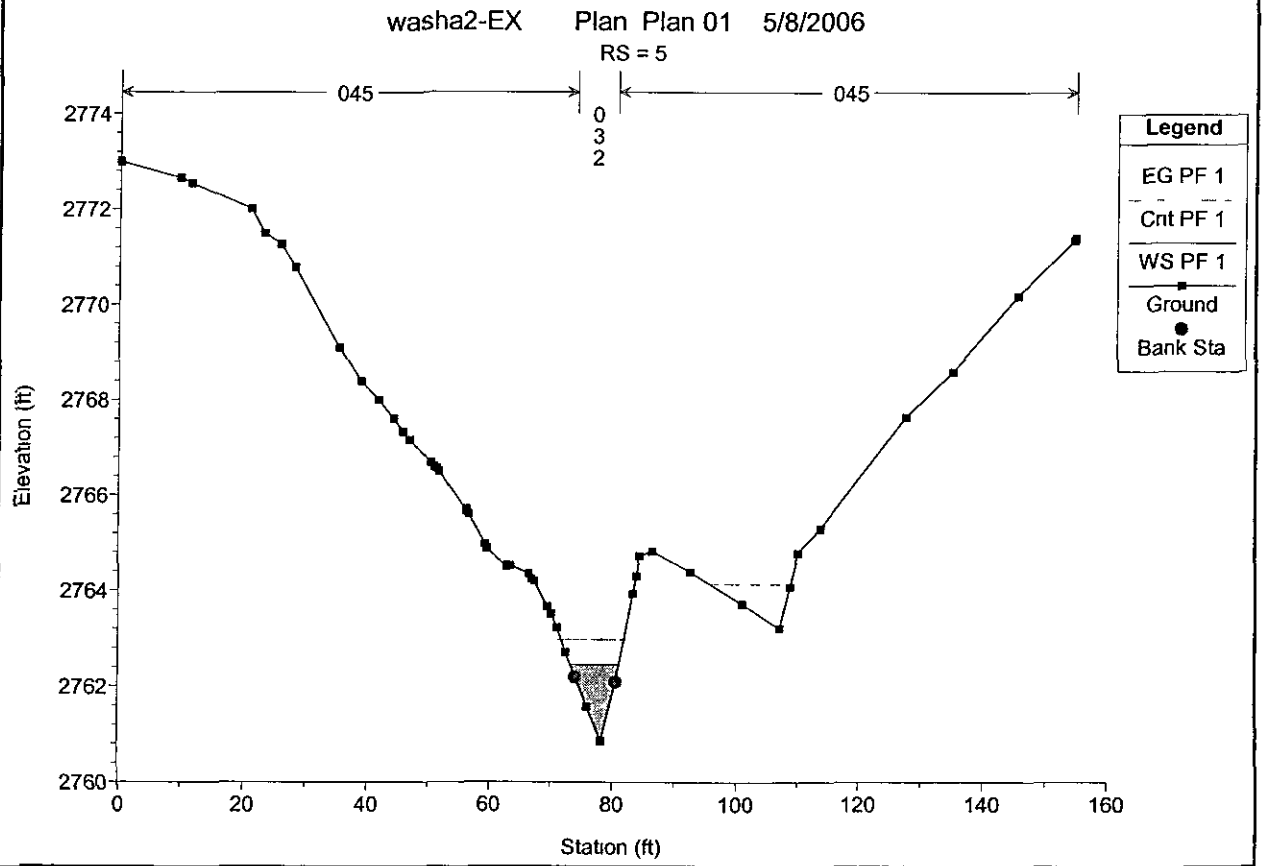
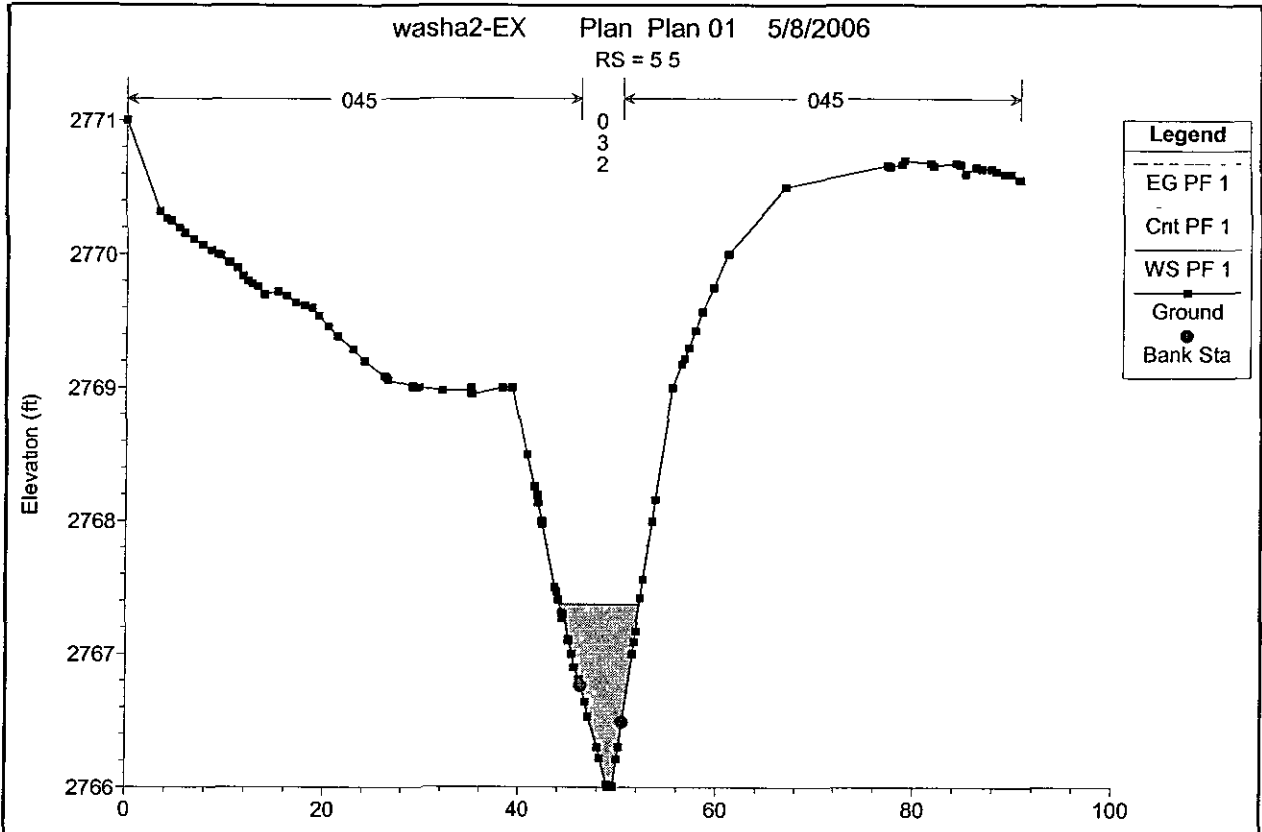


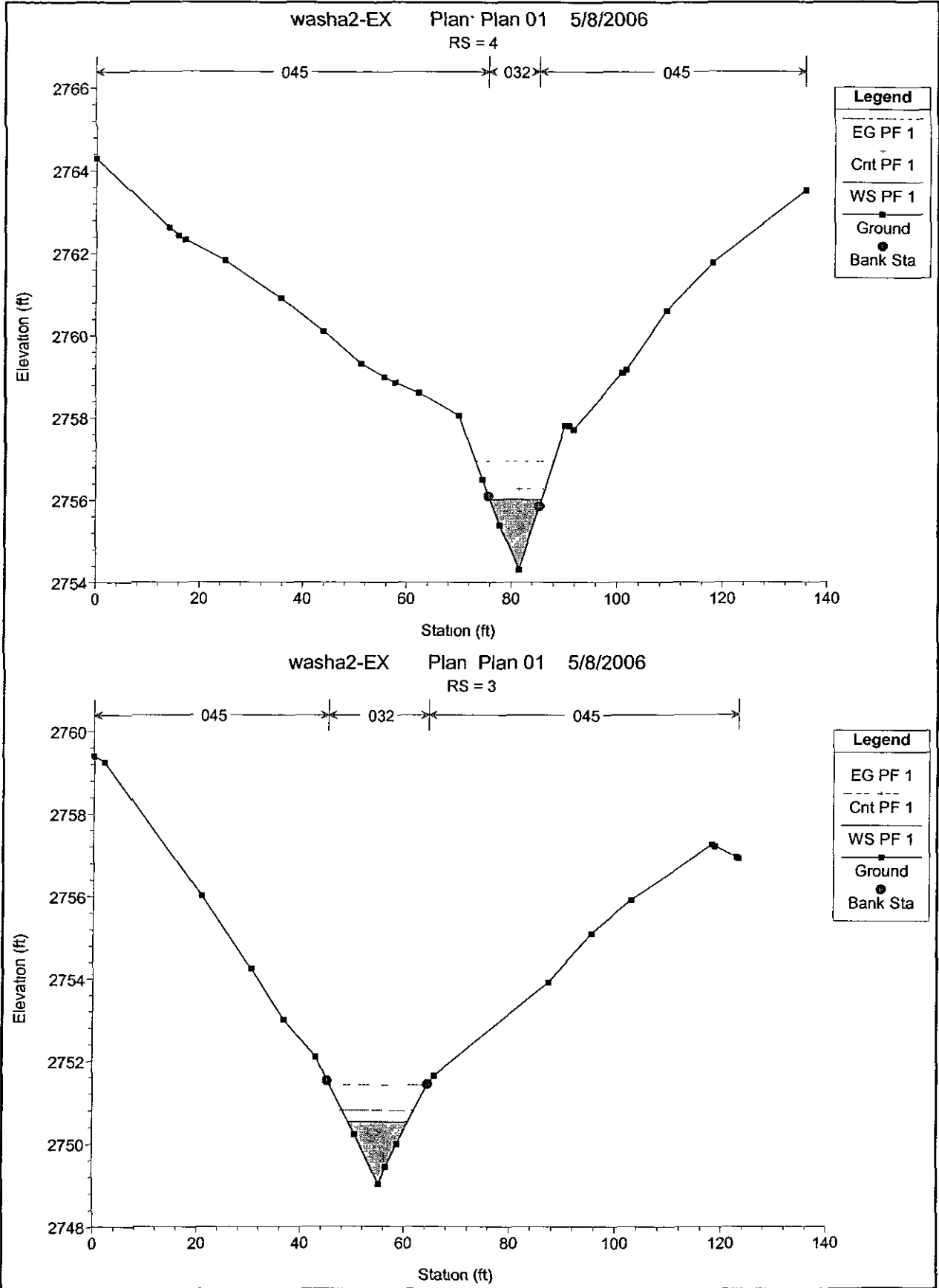
washa2-EX Plan Plan 01 5/8/2006  
RS = 6.5



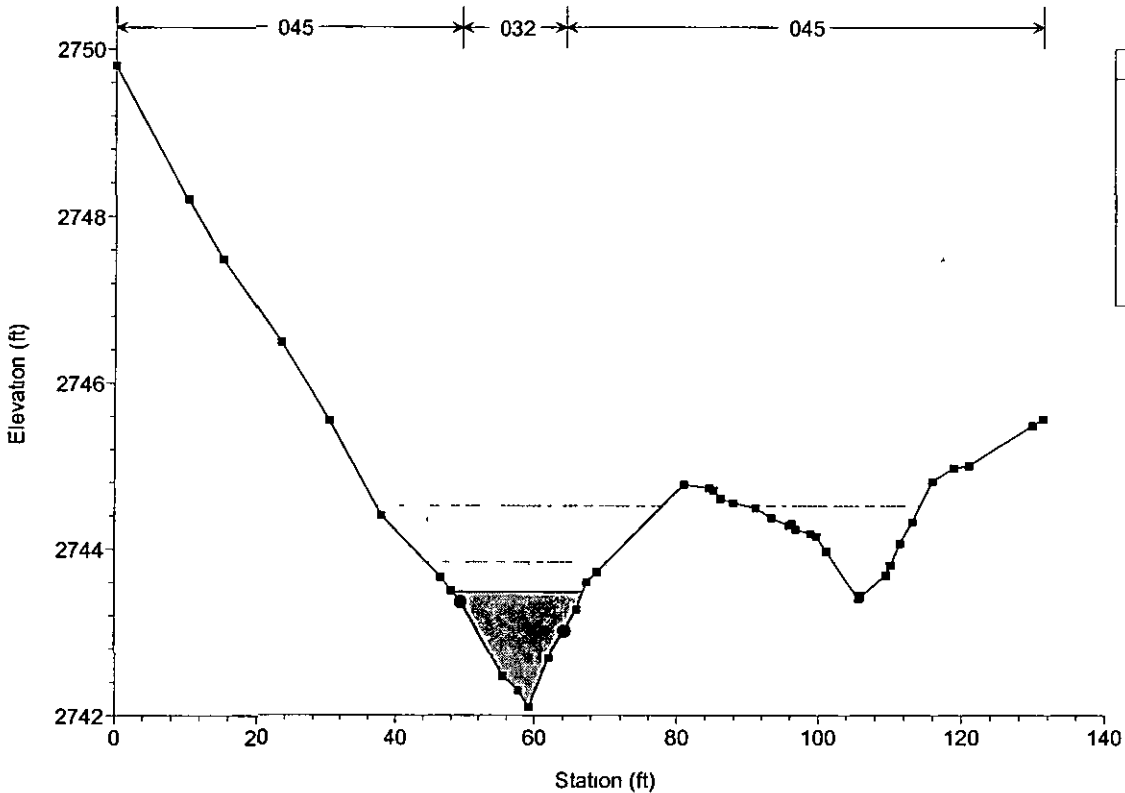
washa2-EX Plan Plan 01 5/8/2006  
RS = 6





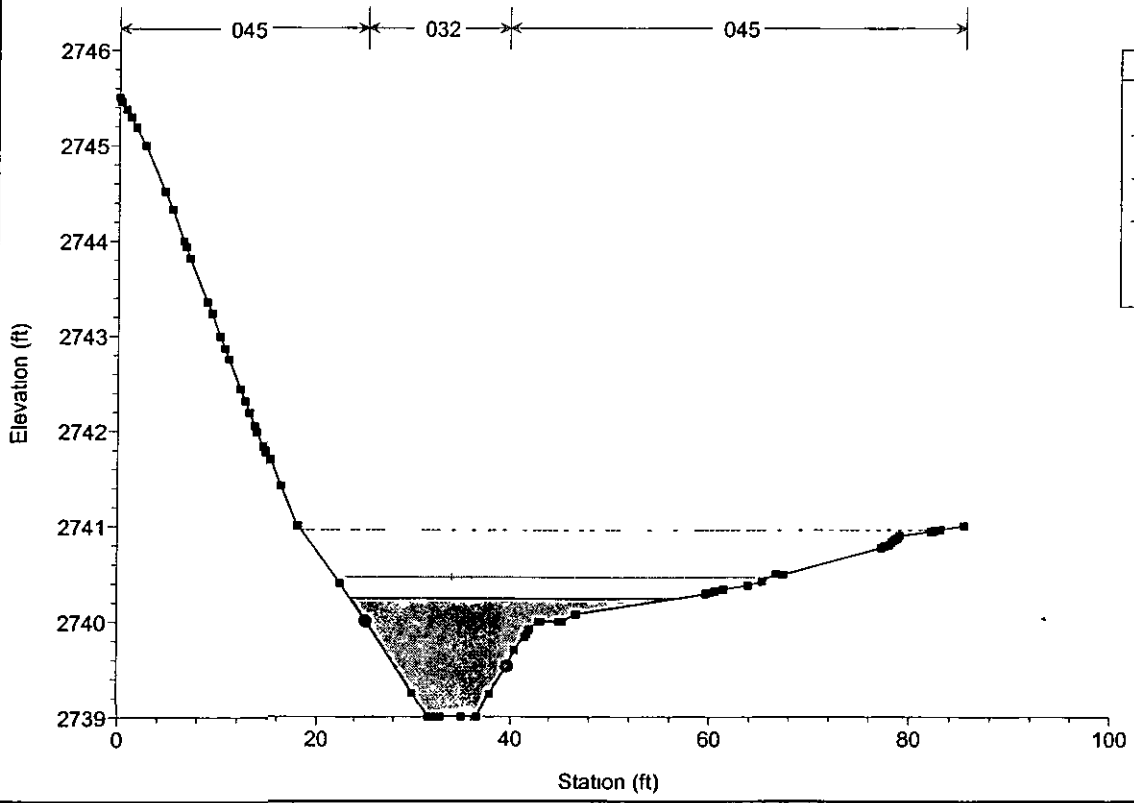


washa2-EX Plan Plan 01 5/8/2006  
RS = 2



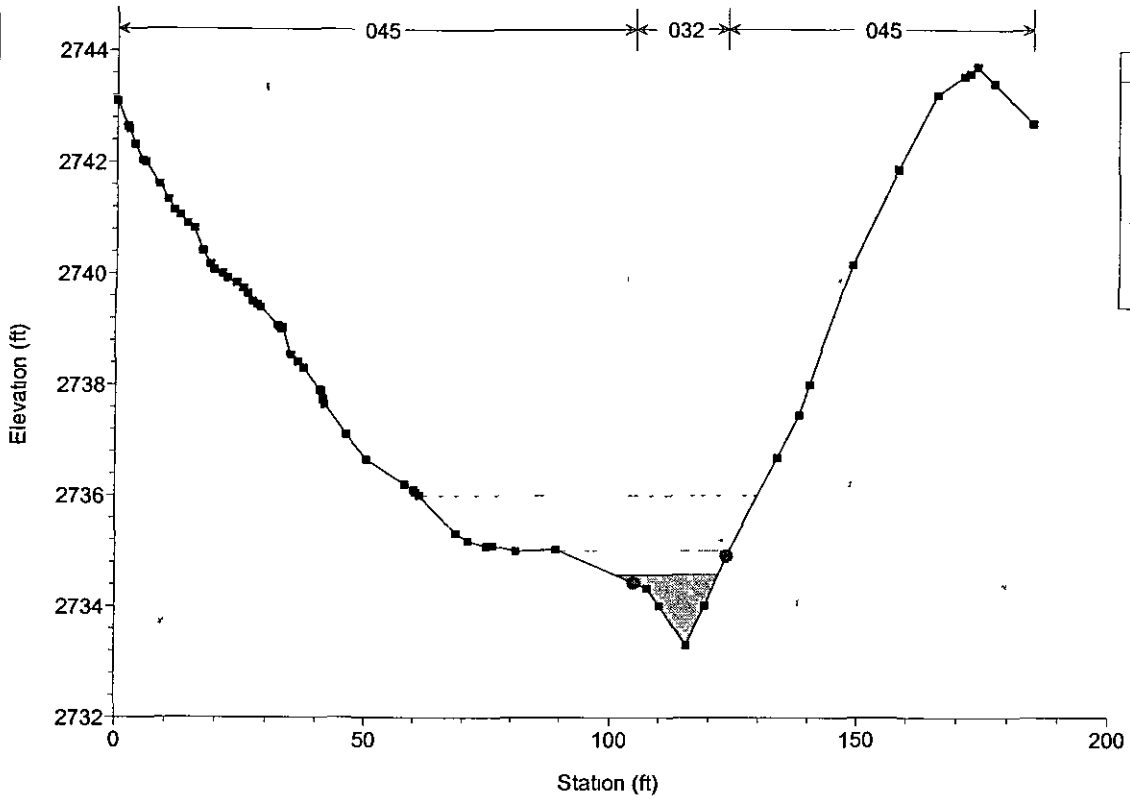
Legend	
—	EG PF 1
- - -	Cnt PF 1
—	WS PF 1
■	Ground
●	Bank Sta

washa2-EX Plan Plan 01 5/8/2006  
RS = 15



Legend	
—	EG PF 1
- - -	Cnt PF 1
—	WS PF 1
■	Ground
●	Bank Sta

washa2-EX Plan. Plan 01 5/8/2006  
RS = 1



Legend	
---	EG PF 1
- - -	Cnt PF 1
---	WS PF 1
■	Ground
●	Bank Sta

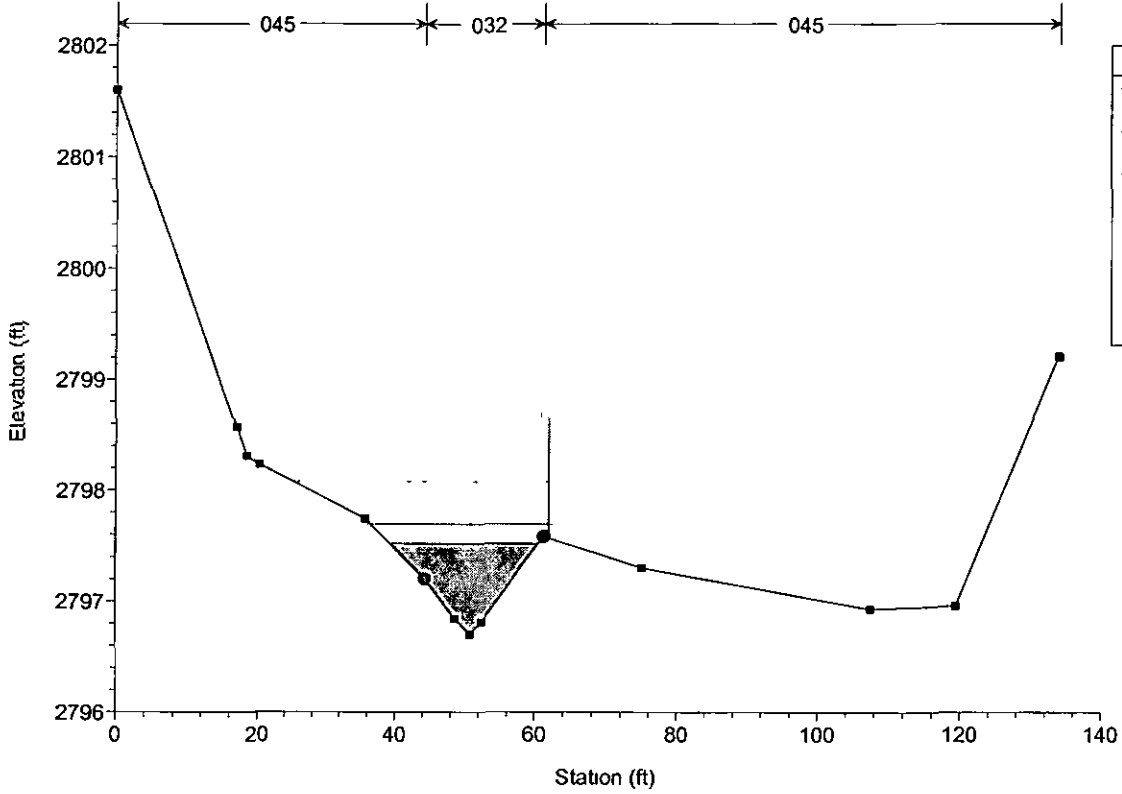


# WASH B1

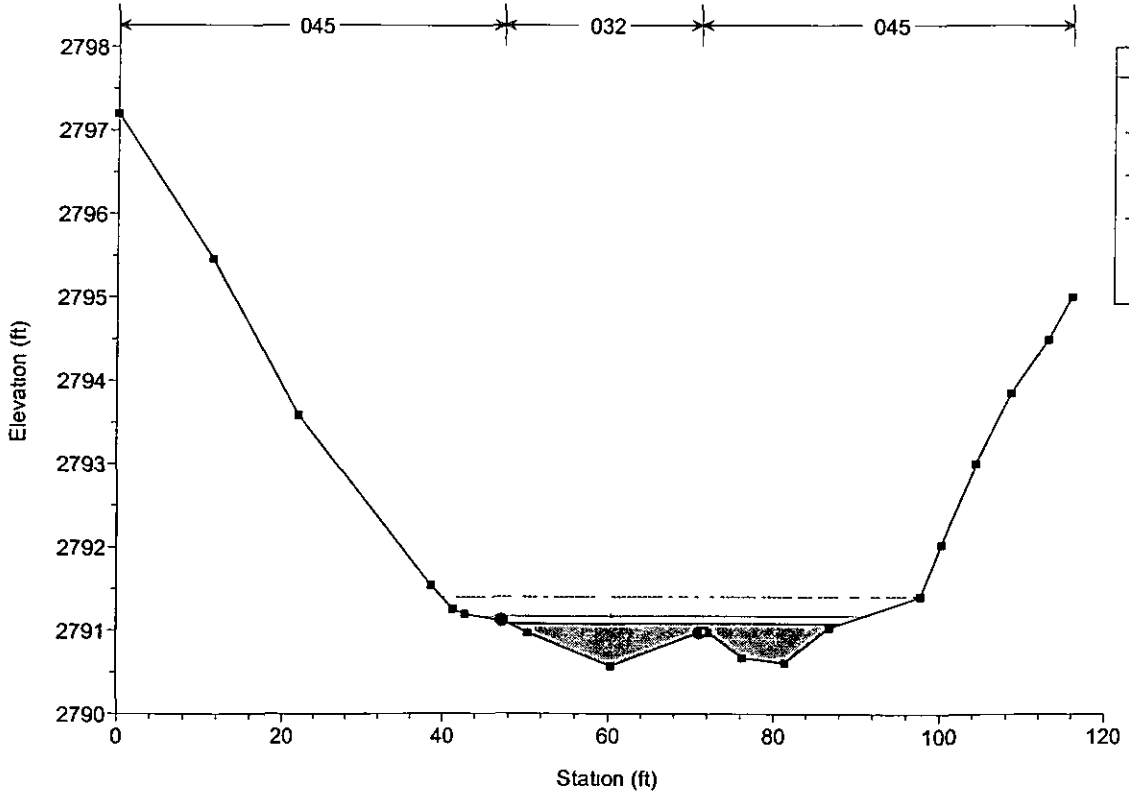
HEC-RAS Plan Plan01 River RIVER-1 Reach Reach-1 Profile PF 1

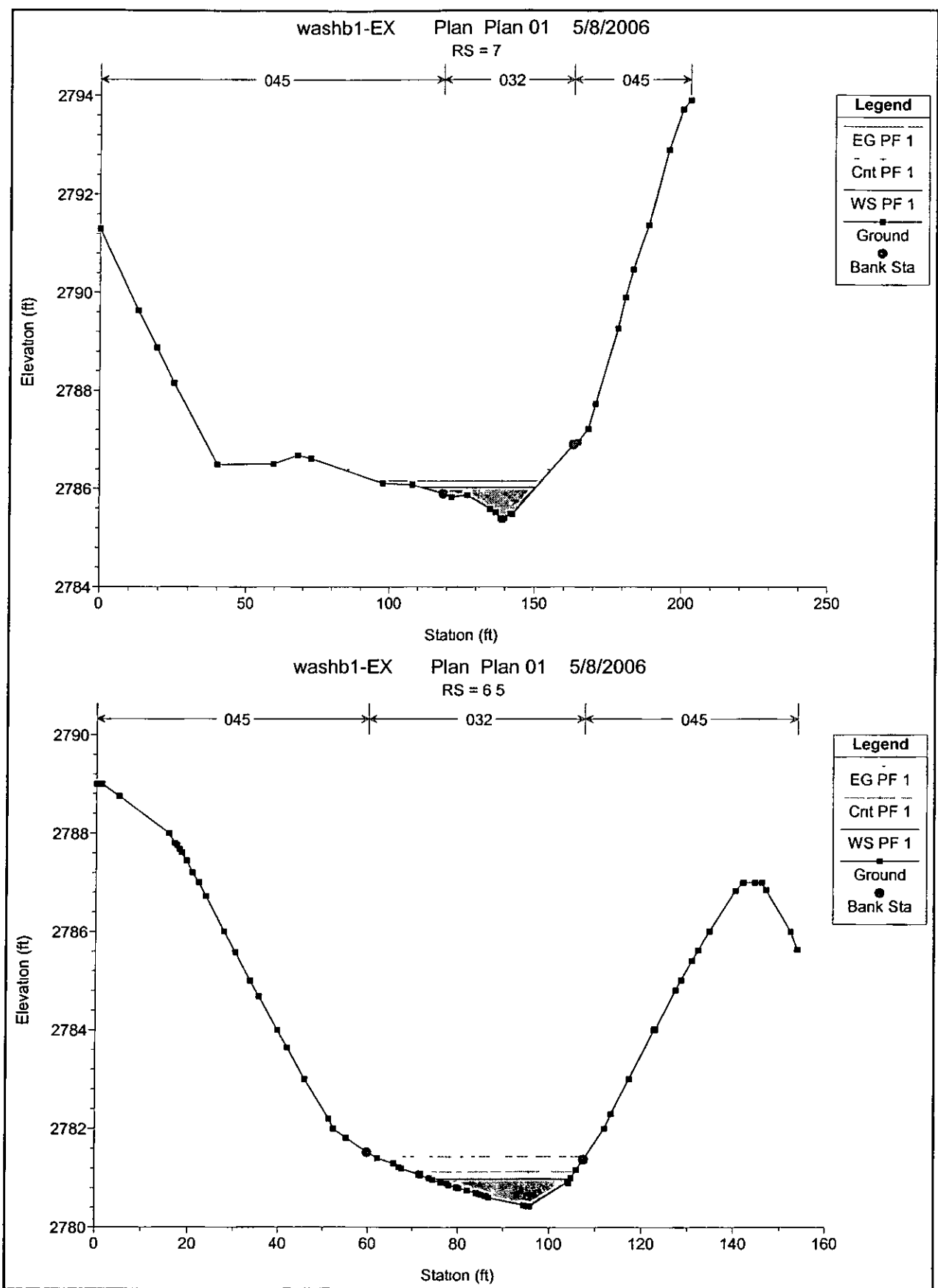
Reach	River Sta	Profile	Q Total (cfs)	Min Ch Elev (ft)	W S Elev (ft)	Crit W S (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	9	PF.1	50.00	2796.70	2797.52	2797.69	2798.08	0.045030	6.09	8.74	21.32	1.54
Reach-1	8	PF.1	50.00	2790.57	2791.08	2791.17	2791.40	0.059767	4.94	11.57	40.41	1.62
Reach-1	7	PF.1	50.00	2785.37	2786.03	2786.15	2786.38	0.048169	4.80	10.80	39.56	1.49
Reach-1	6.5	PF.1	50.00	2780.43	2780.99	2781.13	2781.43	0.065773	5.37	9.31	30.67	1.72
Reach-1	6	PF.1	50.00	2776.08	2776.54	2776.59	2776.81	0.033697	4.19	11.94	34.58	1.26
Reach-1	5	PF.1	65.00	2771.10	2771.91	2772.14	2772.65	0.053926	7.02	10.88	49.99	1.70
Reach-1	4.5	PF.1	65.00	2768.38	2768.90	2769.02	2769.32	0.046654	5.18	12.54	33.73	1.50
Reach-1	4	PF.1	65.00	2764.68	2765.43	2765.61	2766.02	0.043096	7.04	12.39	29.28	1.57
Reach-1	3	PF.1	65.00	2759.60	2760.51	2760.70	2761.15	0.041337	6.54	10.57	22.11	1.52
Reach-1	2	PF.1	79.00	2754.10	2754.85	2755.02	2755.43	0.047512	6.18	13.44	33.39	1.58
Reach-1	1	PF.1	192.00	2745.67	2746.98	2747.15	2747.49	0.048446	7.72	39.04	85.79	1.66

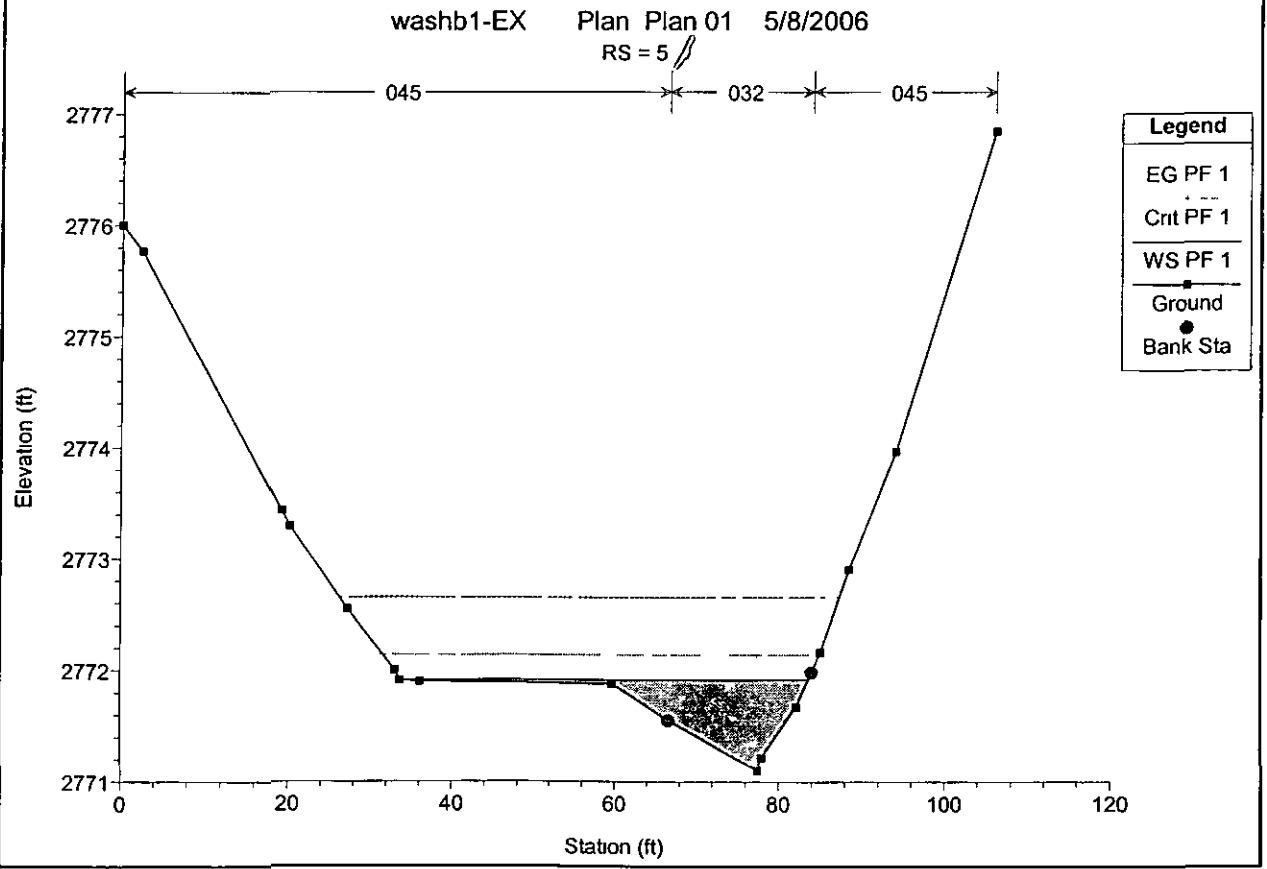
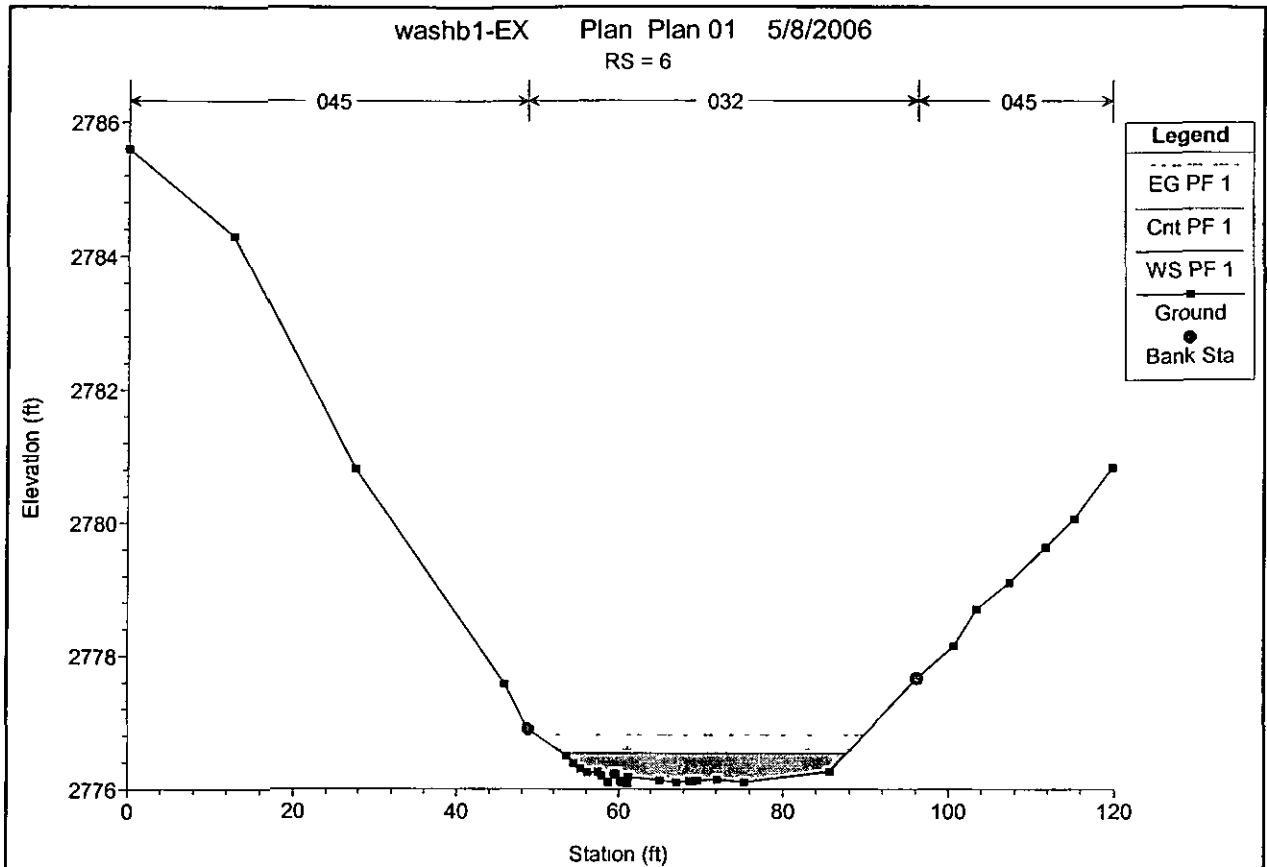
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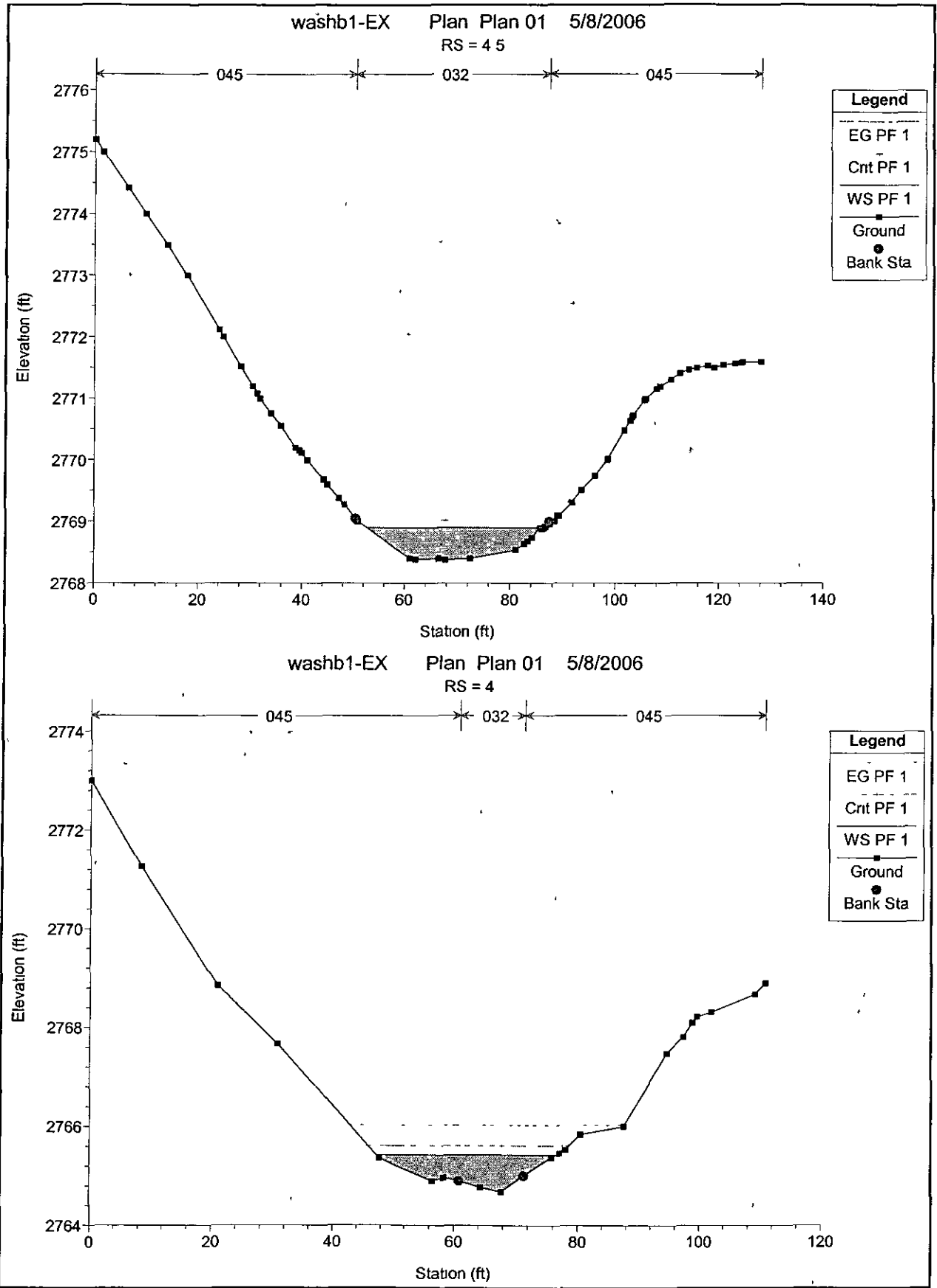


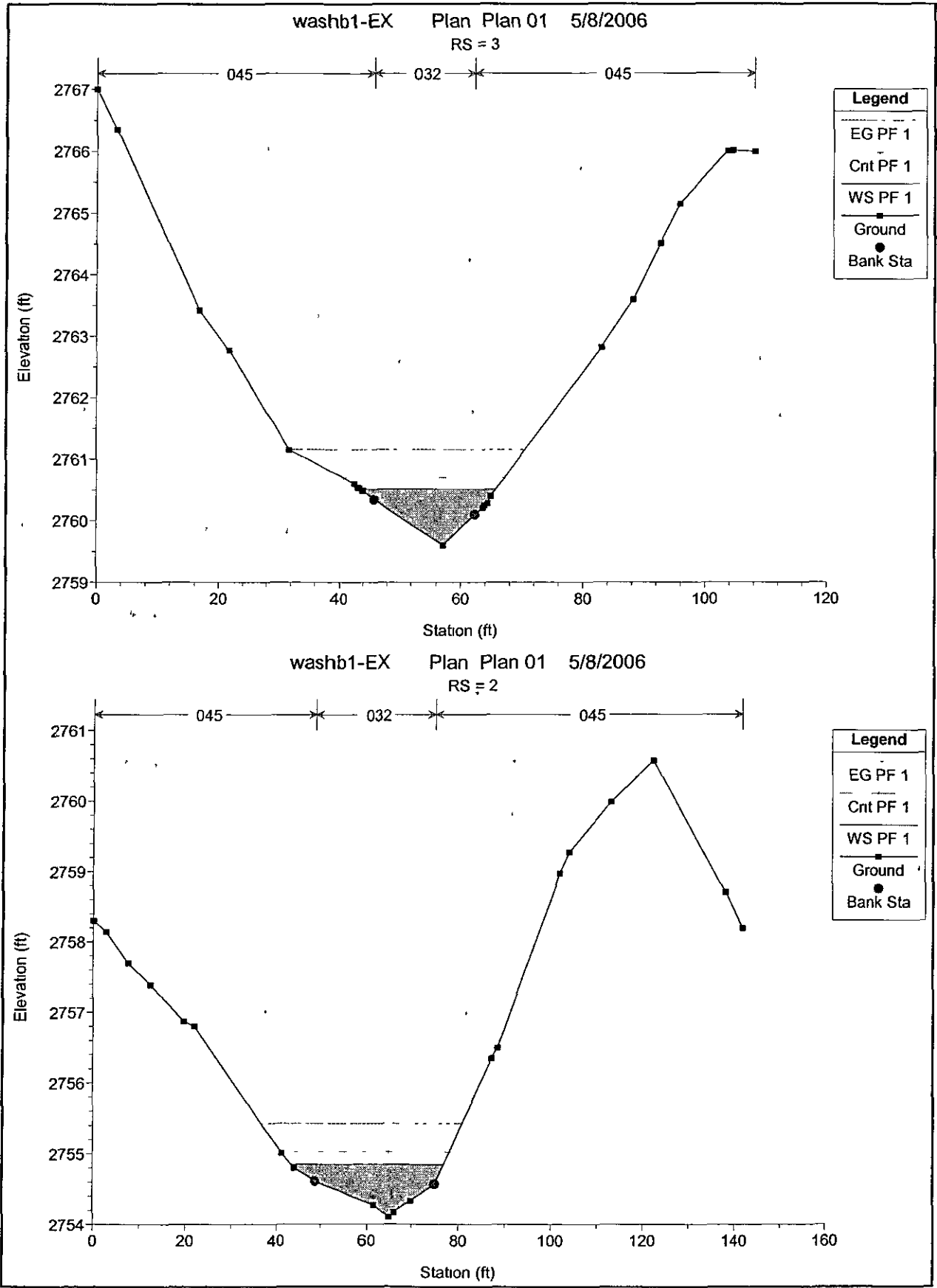
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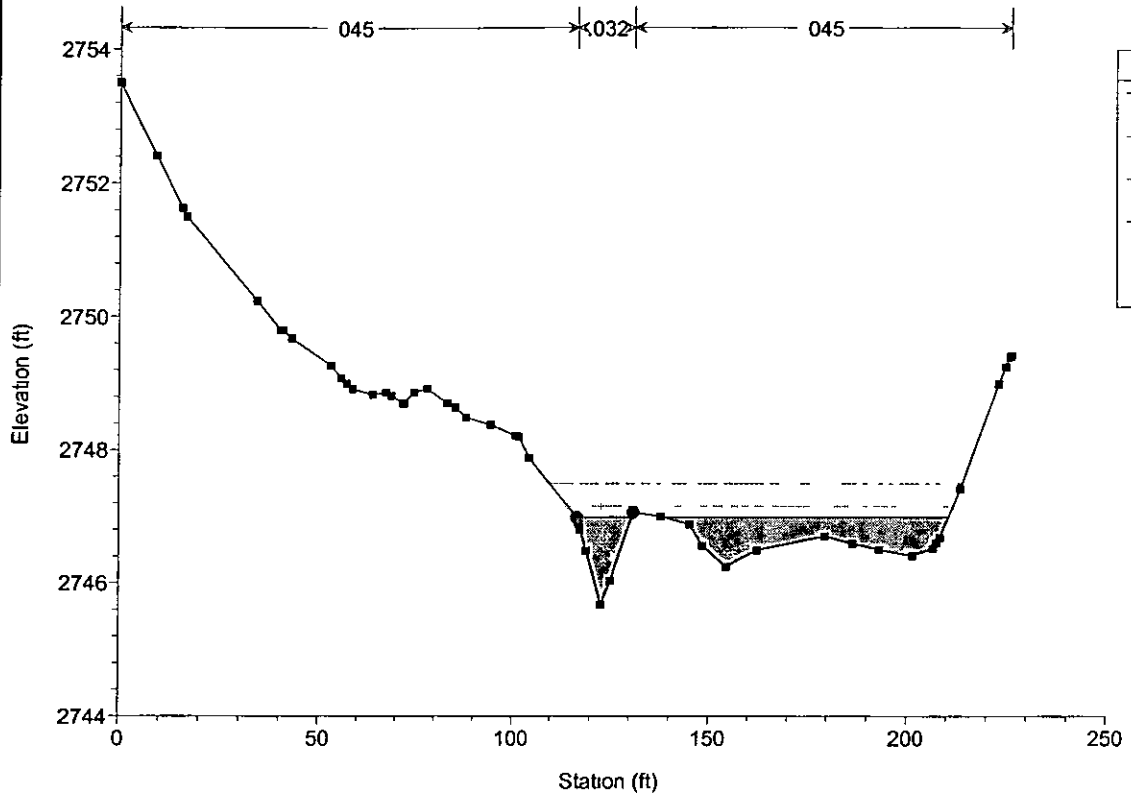






washb1-EX Plan Plan 01 5/8/2006

RS = 1



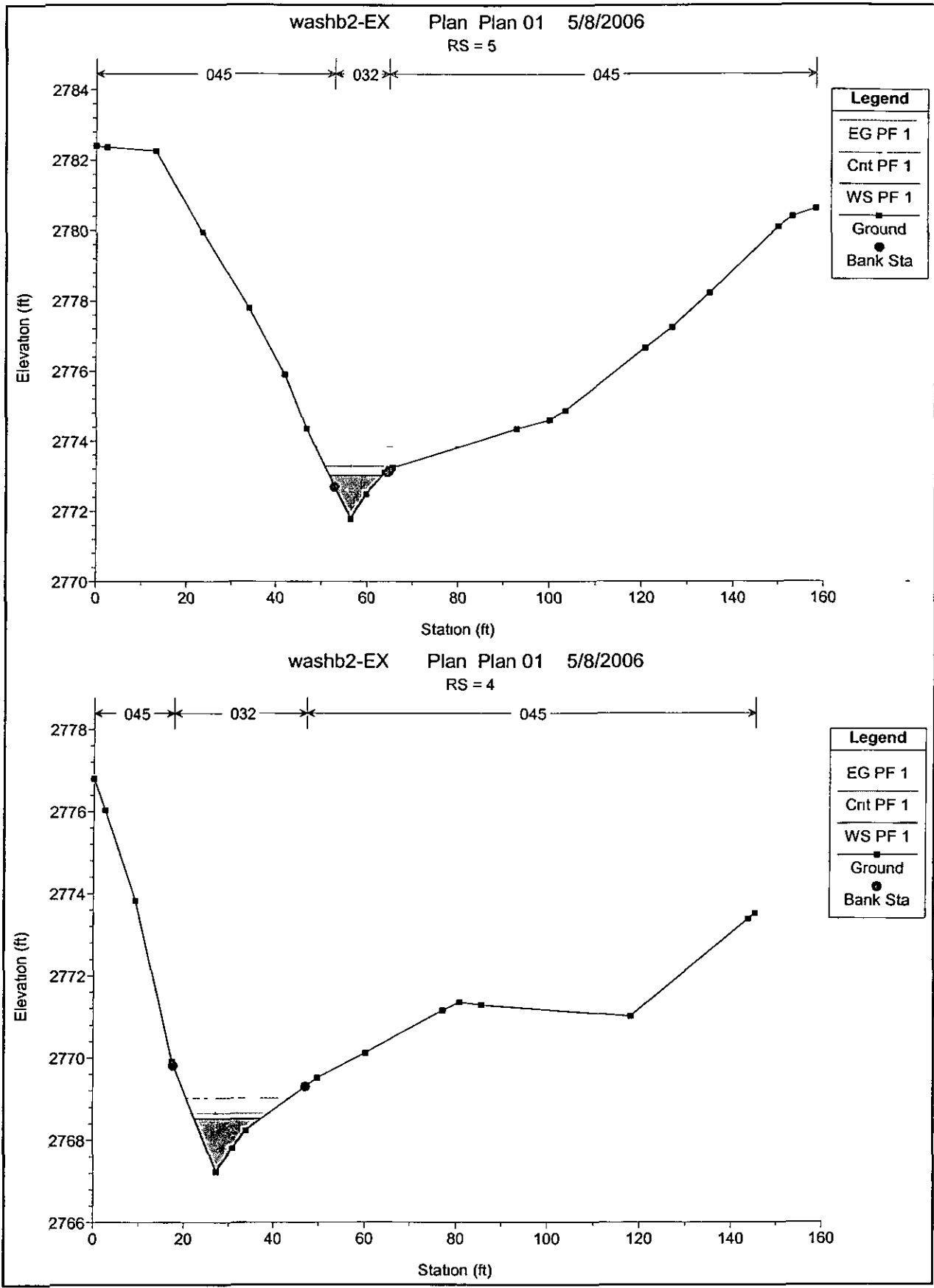
Legend	
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—	Cnt PF 1
■	WS PF 1
■	Ground
●	Bank Sta

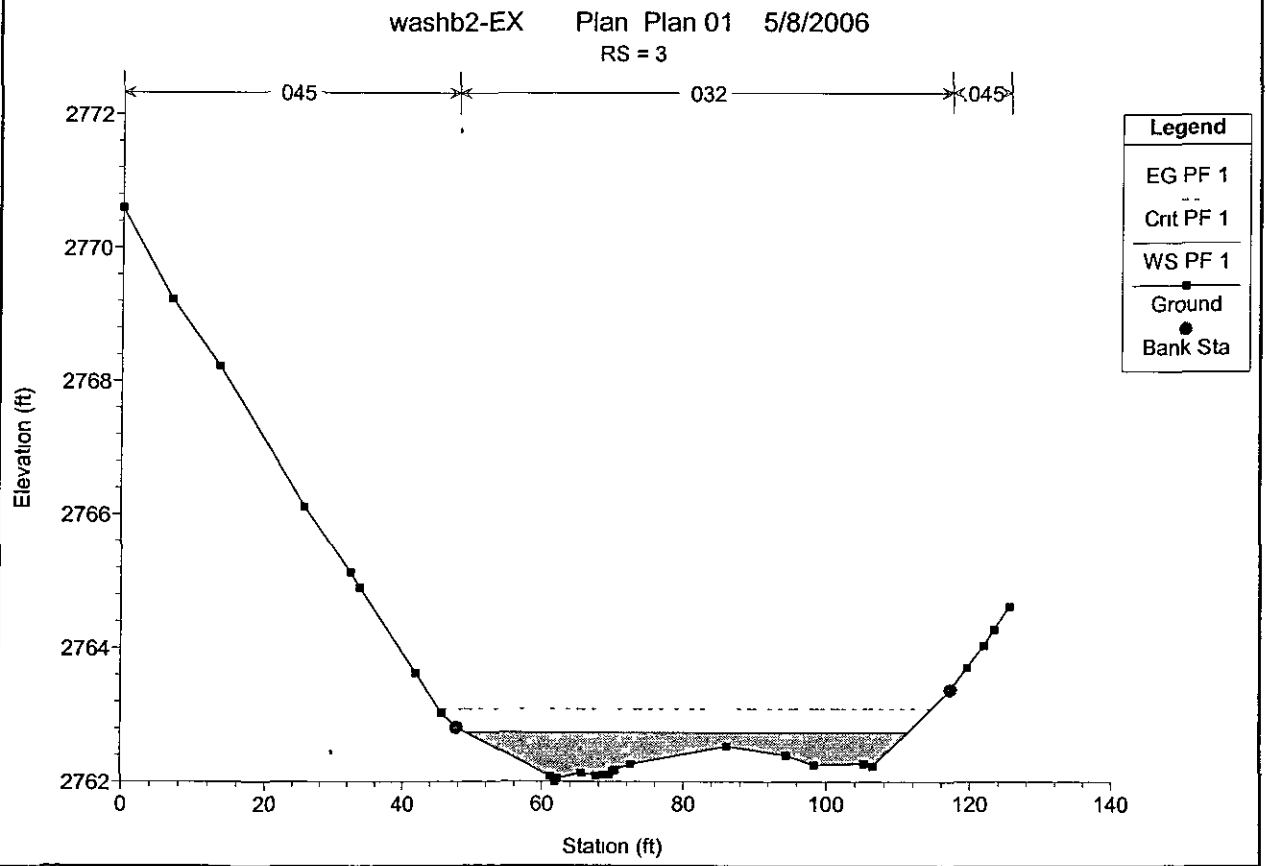
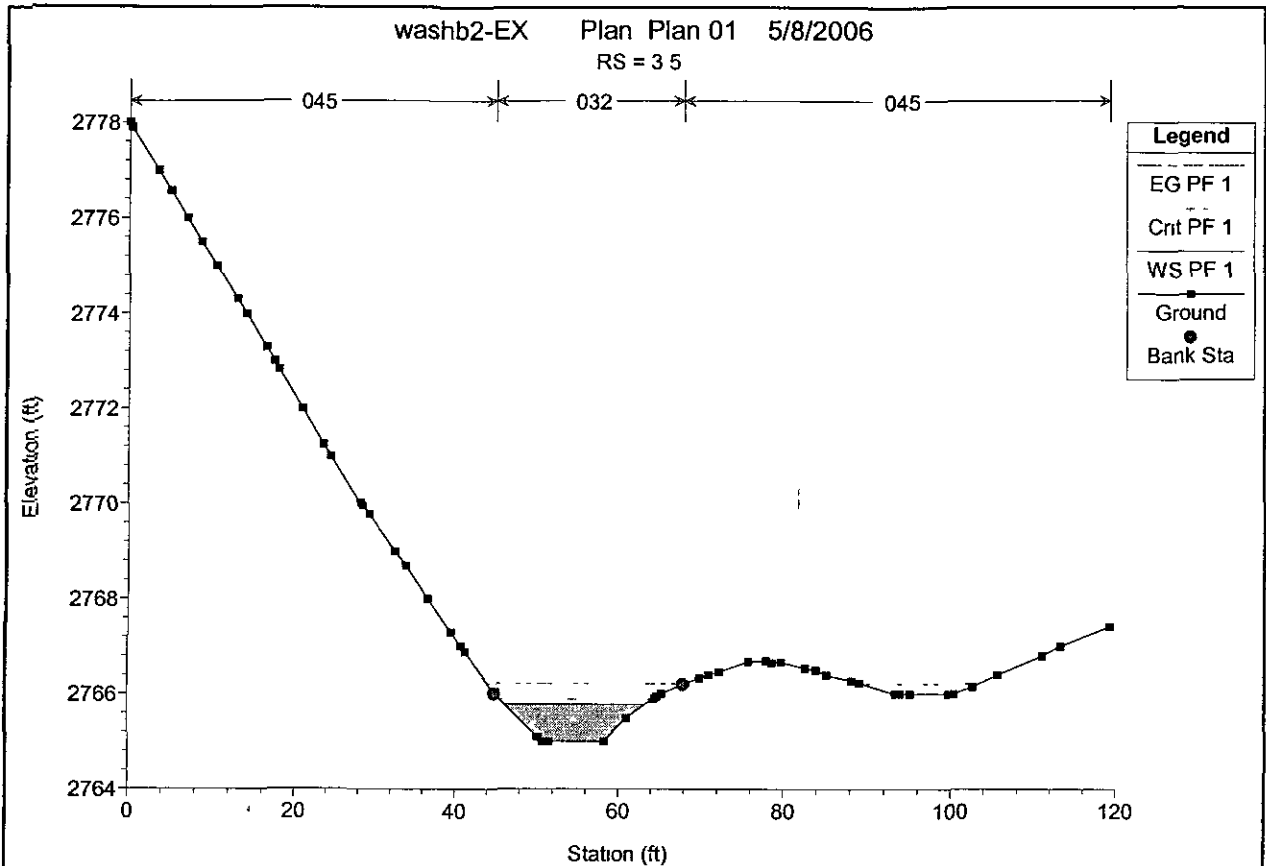
WASH B2

HEC-RAS Plan Plan 01 River RIVER 1 Reach Reach-1 Profile PF 1

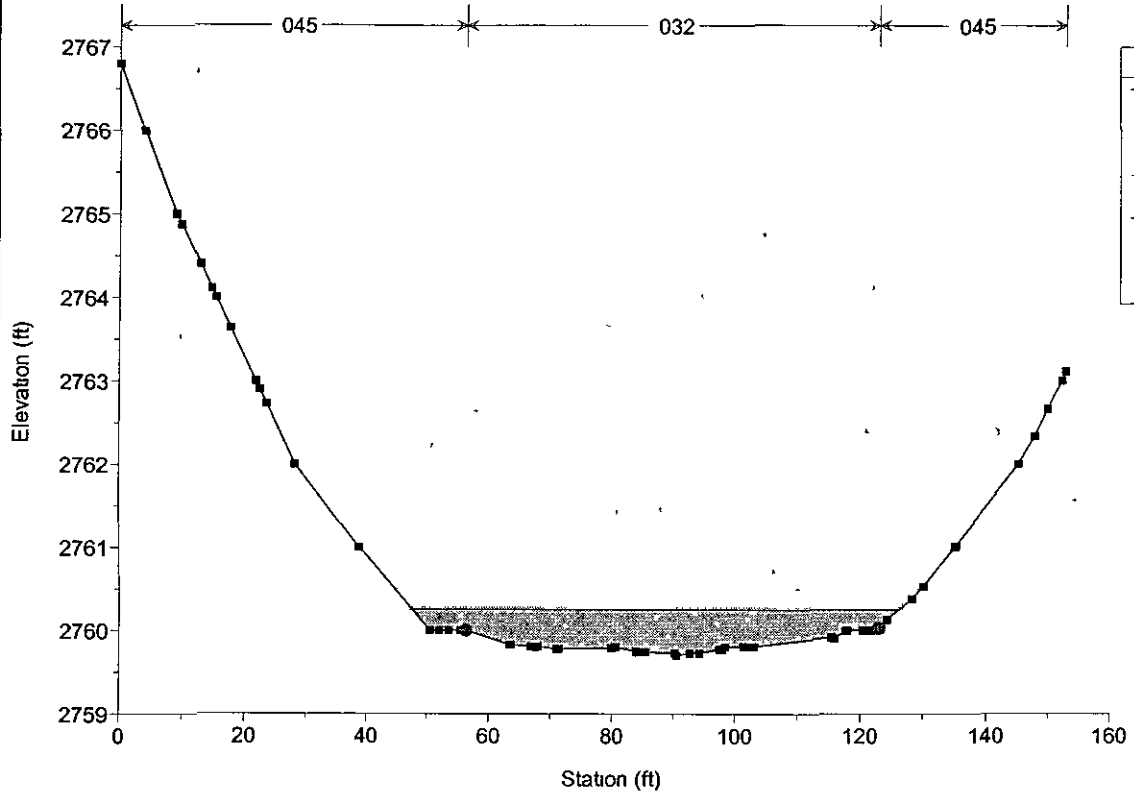
Reach	River Sta	Profile	Q Total (cfs)	Min Ch Elev (ft)	W S Elev (ft)	Crit W S Elev (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	5	PF_1	50.00	2771.78	2773.02	2773.28	2773.84	0.045021	7.27	7.03	11.78	1.59
Reach-1	4	PF_1	50.00	2787.23	2768.51	2768.64	2769.01	0.030968	5.67	8.82	15.02	1.30
Reach-1	3.5	PF_1	50.00	2765.00	2765.78	2765.88	2766.22	0.029393	5.31	9.41	17.14	1.26
Reach-1	3	PF_1	117.00	2762.00	2762.74	2762.82	2763.09	0.036286	4.76	24.56	62.10	1.33
Reach-1	2.5	PF_1	117.00	2759.70	2760.25	2760.28	2760.50	0.023772	4.03	30.32	78.68	1.09
Reach-1	2	PF_1	117.00	2755.39	2756.72	2757.05	2757.64	0.046277	7.69	15.28	24.82	1.69
Reach-1	1.5	PF_1	117.00	2753.00	2754.53	2754.78	2755.29	0.022422	8.31	22.76	37.34	1.27
Reach-1	1	PF_1	192.00	2745.67	2747.00	2747.15	2747.47	0.044213	7.46	40.39	87.15	1.59



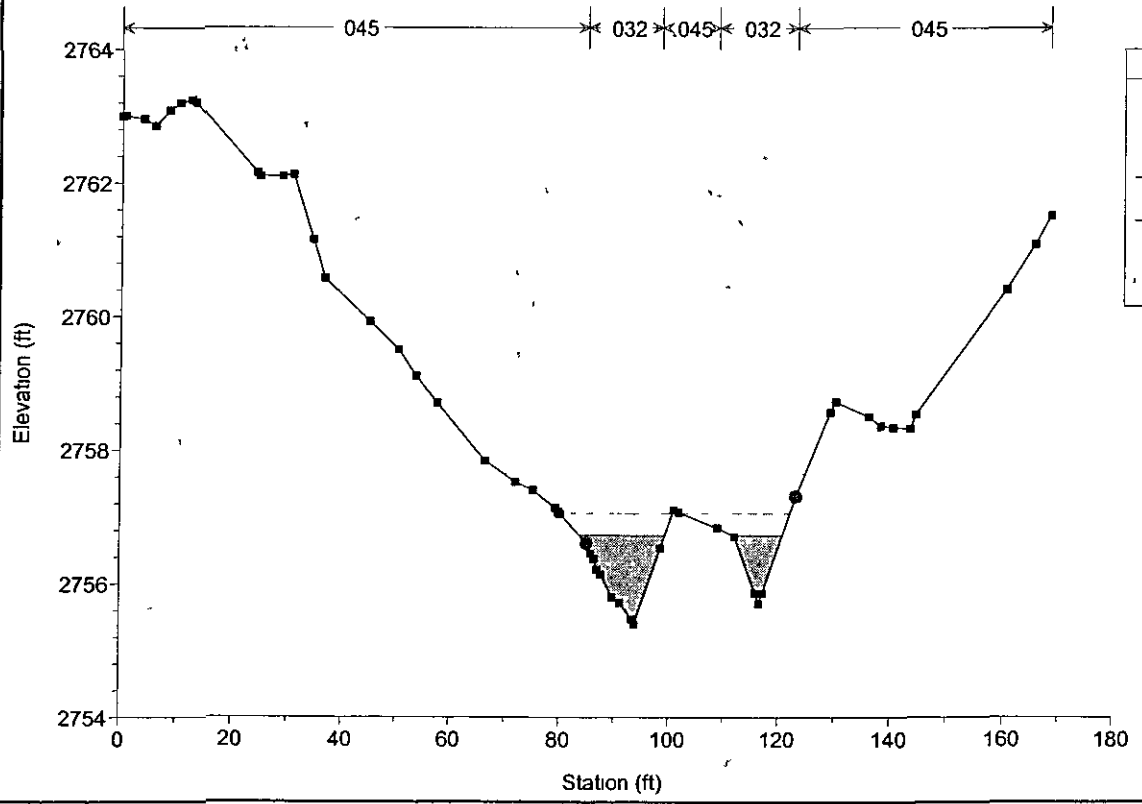


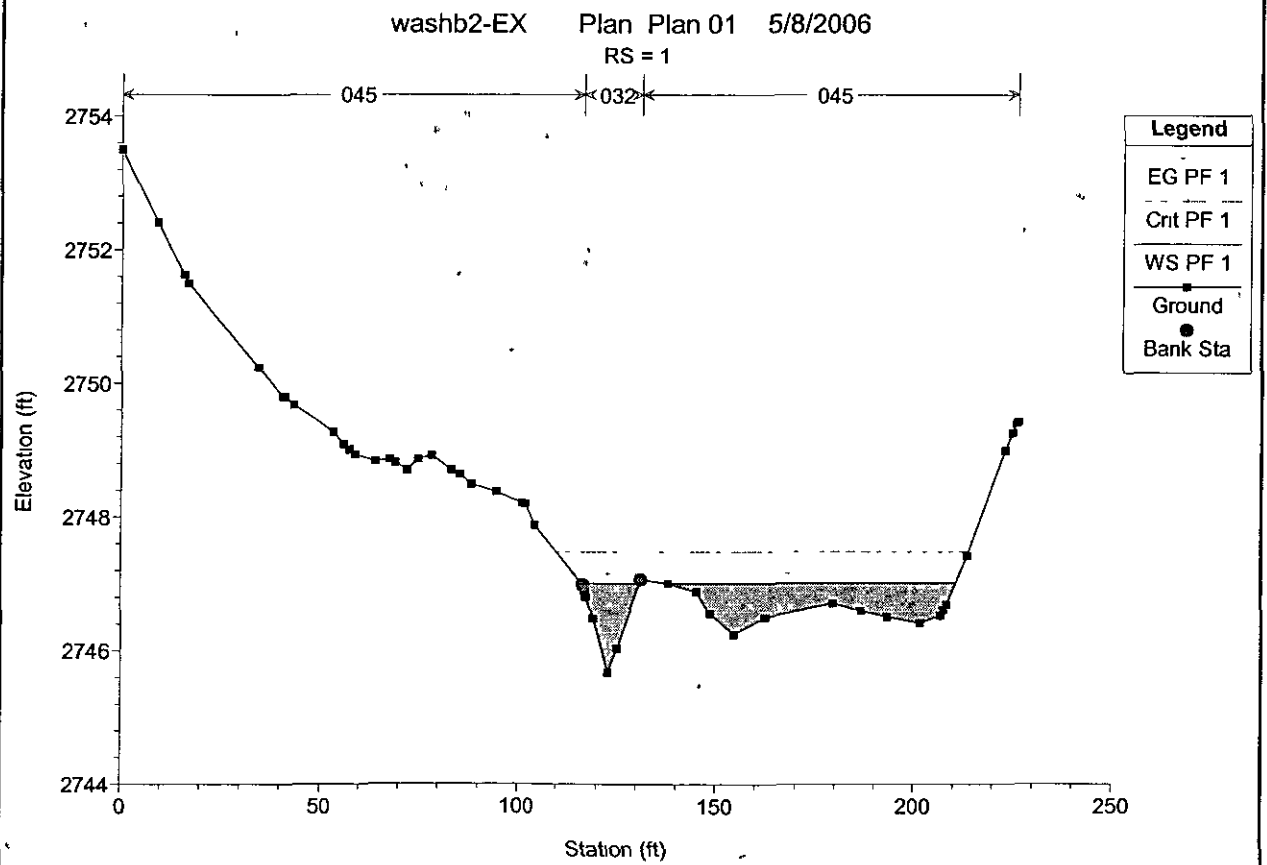
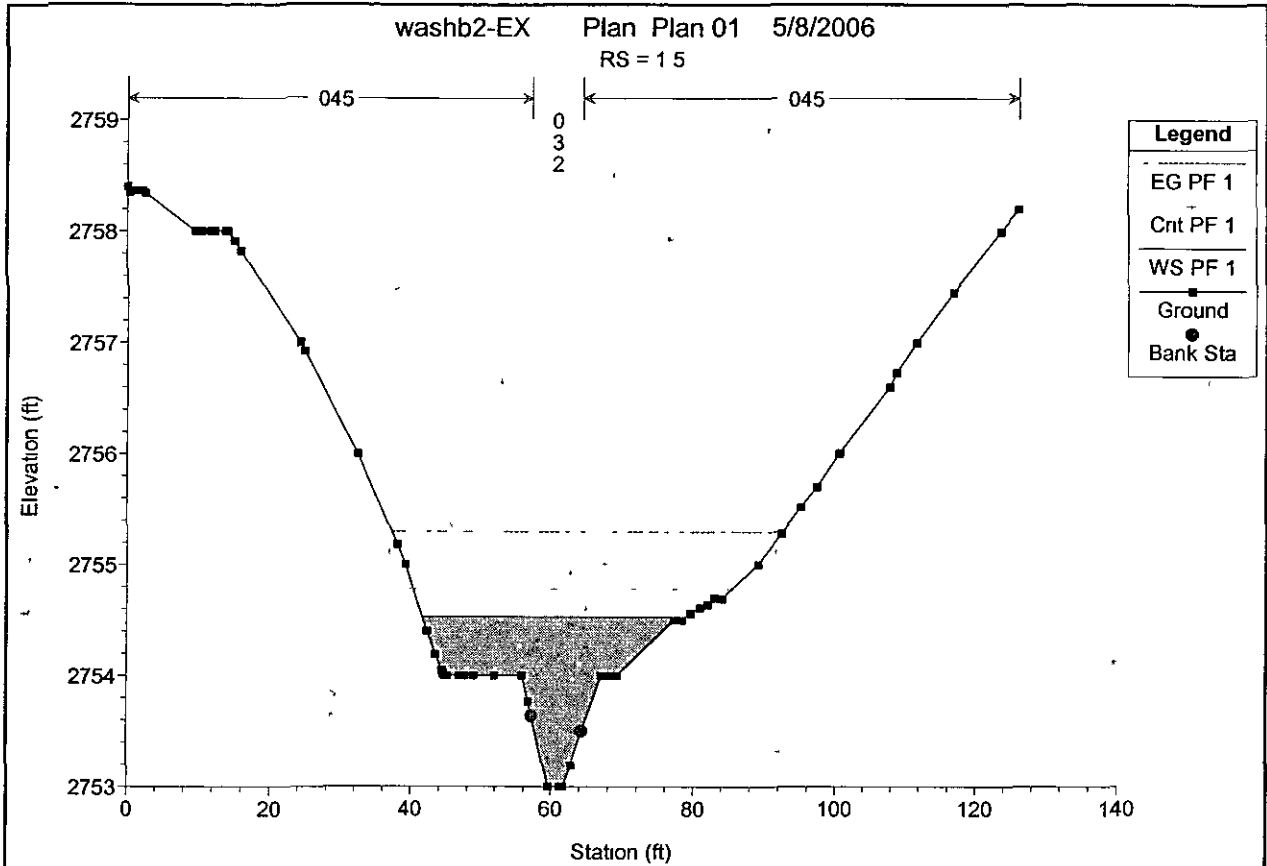


washb2-EX Plan Plan 01 5/8/2006  
RS = 2.5



washb2-EX Plan Plan 01 5/8/2006  
RS = 2



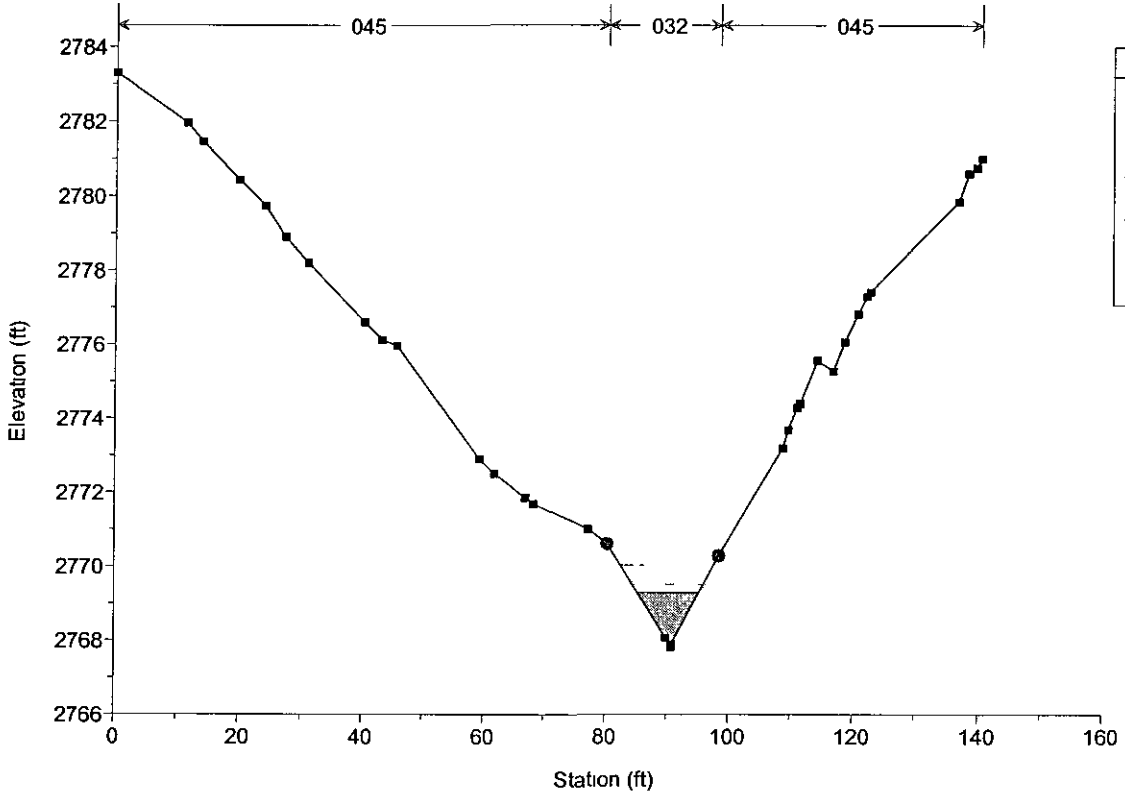


WASH C

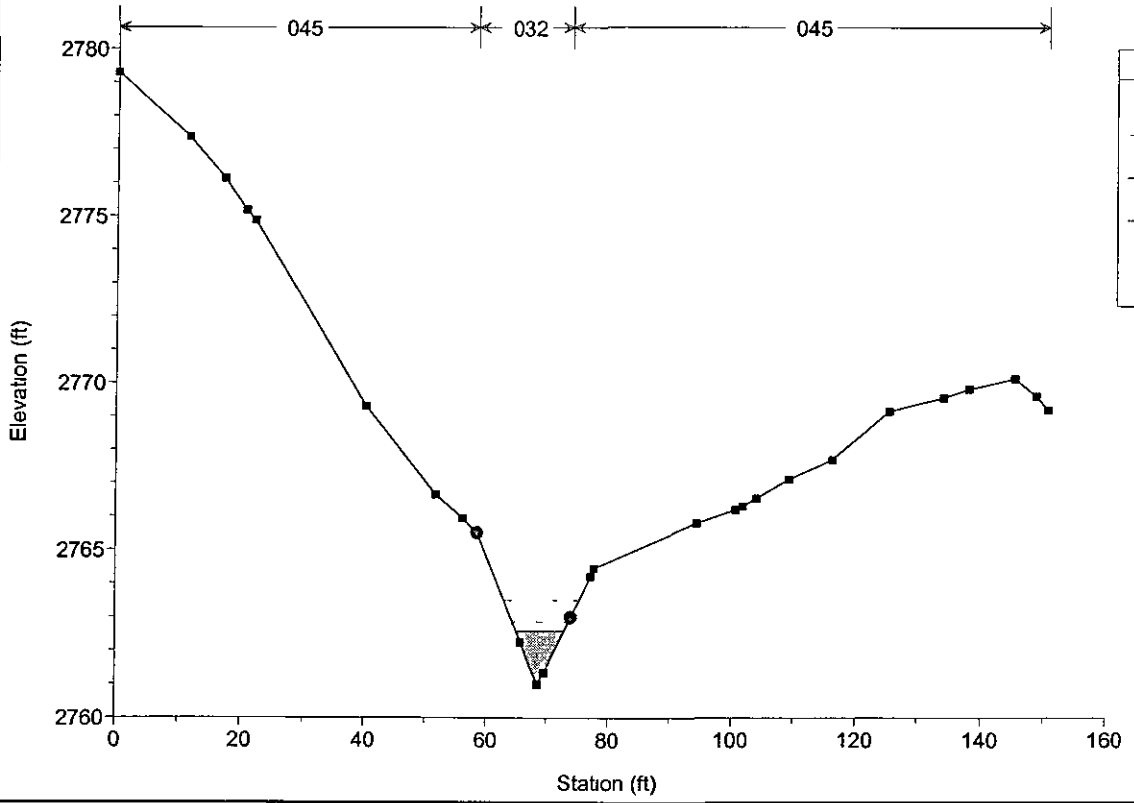
HEC-RAS Plan Plan 01 River RIVER-1 Reach Reach-1 Profile PF 1

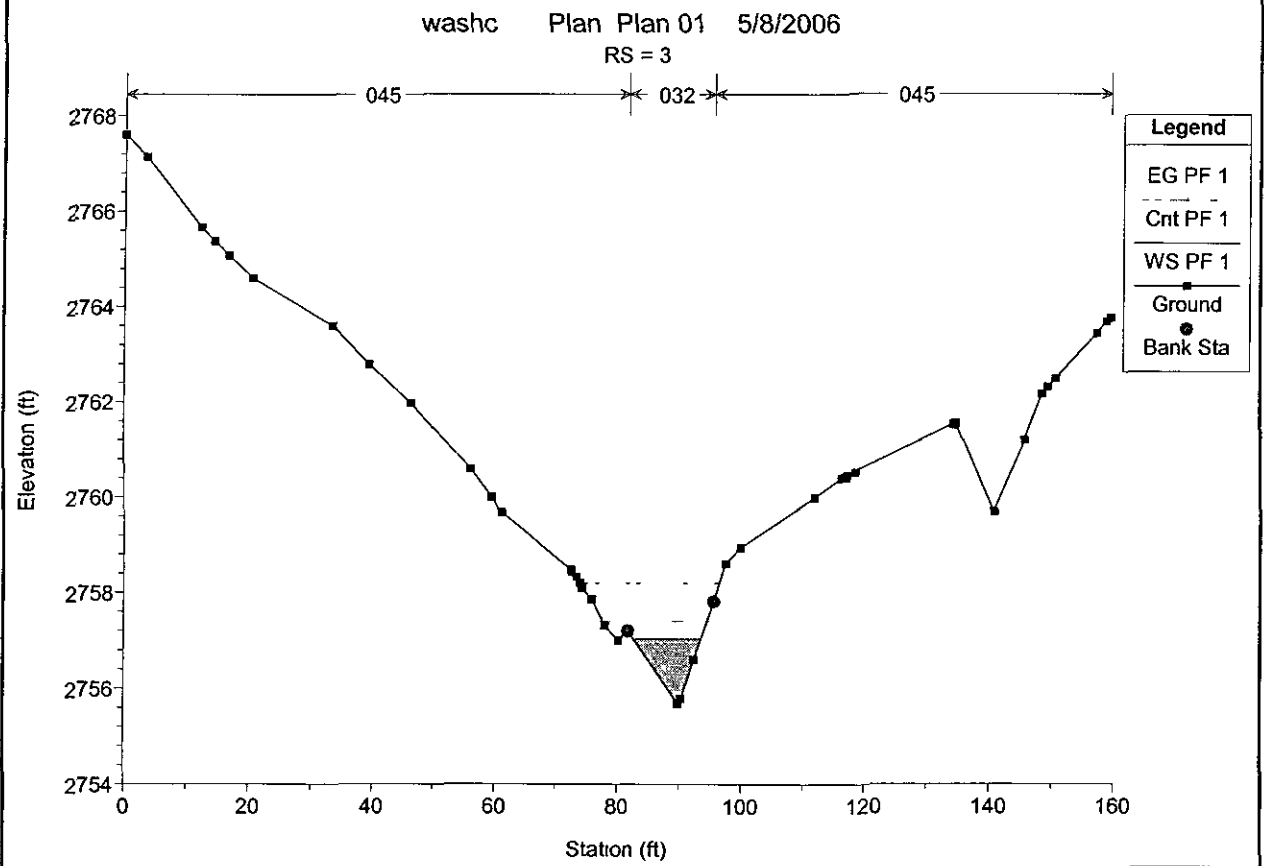
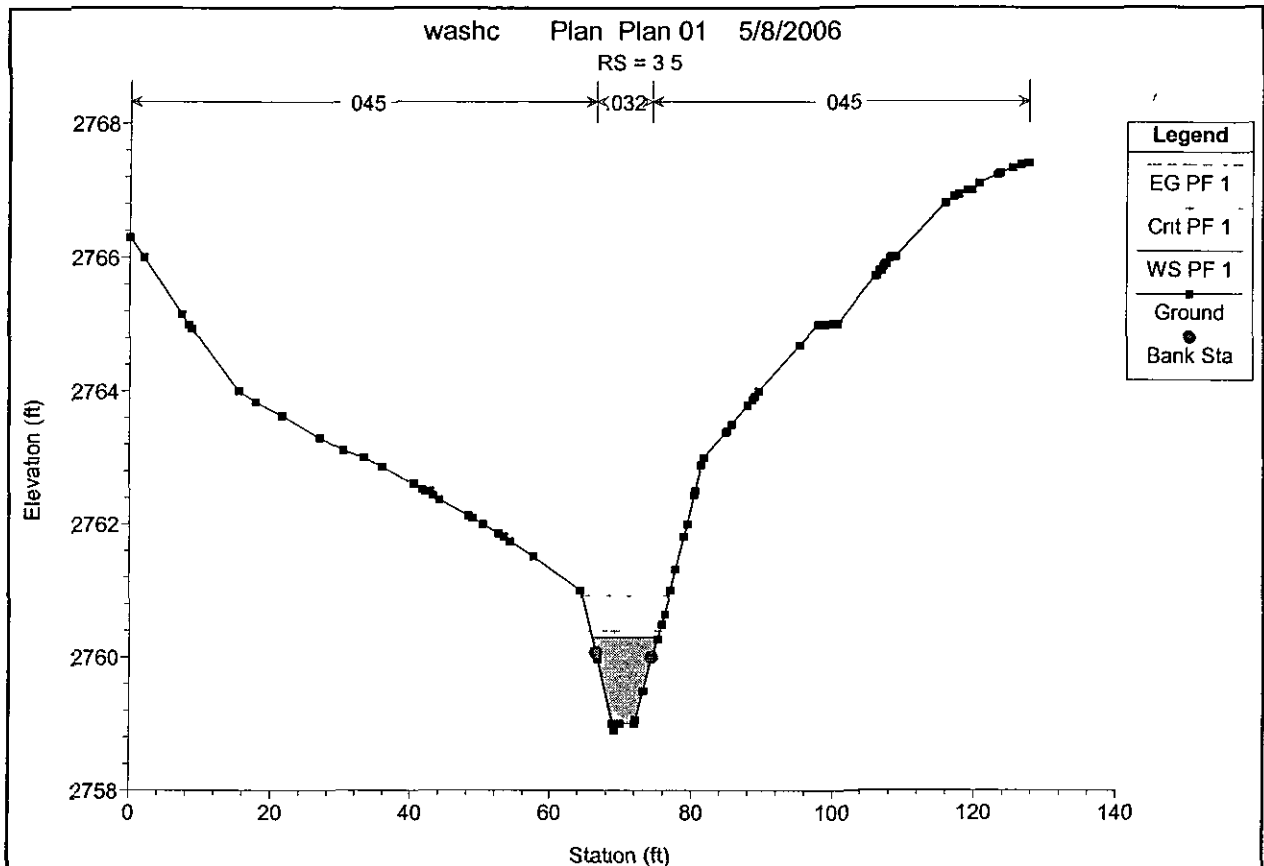
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W S Elev (ft)	Crit W S (ft)	E G Elev (ft)	E G Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	5	PF 1	50.00	2767.82	2769.29	2769.50	2770.03	0.036028	6.90	7.25	10.04	1.43
Reach-1	4	PF 1	50.00	2761.00	2762.59	2762.87	2763.50	0.039393	7.65	6.54	8.03	1.49
Reach-1	3.5	PF 1	50.00	2758.90	2760.30	2760.39	2760.92	0.019976	6.32	8.08	9.29	1.11
Reach-1	3	PF 1	65.00	2755.68	2757.04	2757.41	2758.20	0.059581	8.65	7.52	11.30	1.84
Reach-1	2	PF 1	65.00	2749.90	2751.82	2751.88	2752.40	0.019726	6.09	10.67	11.11	1.10
Reach-1	1.5	PF 1	65.00	2748.00	2748.96	2749.27	2749.92	0.058207	7.87	8.26	13.81	1.79
Reach-1	1	PF 1	85.00	2743.99	2745.62	2745.79	2746.33	0.026155	6.77	12.64	15.95	1.28

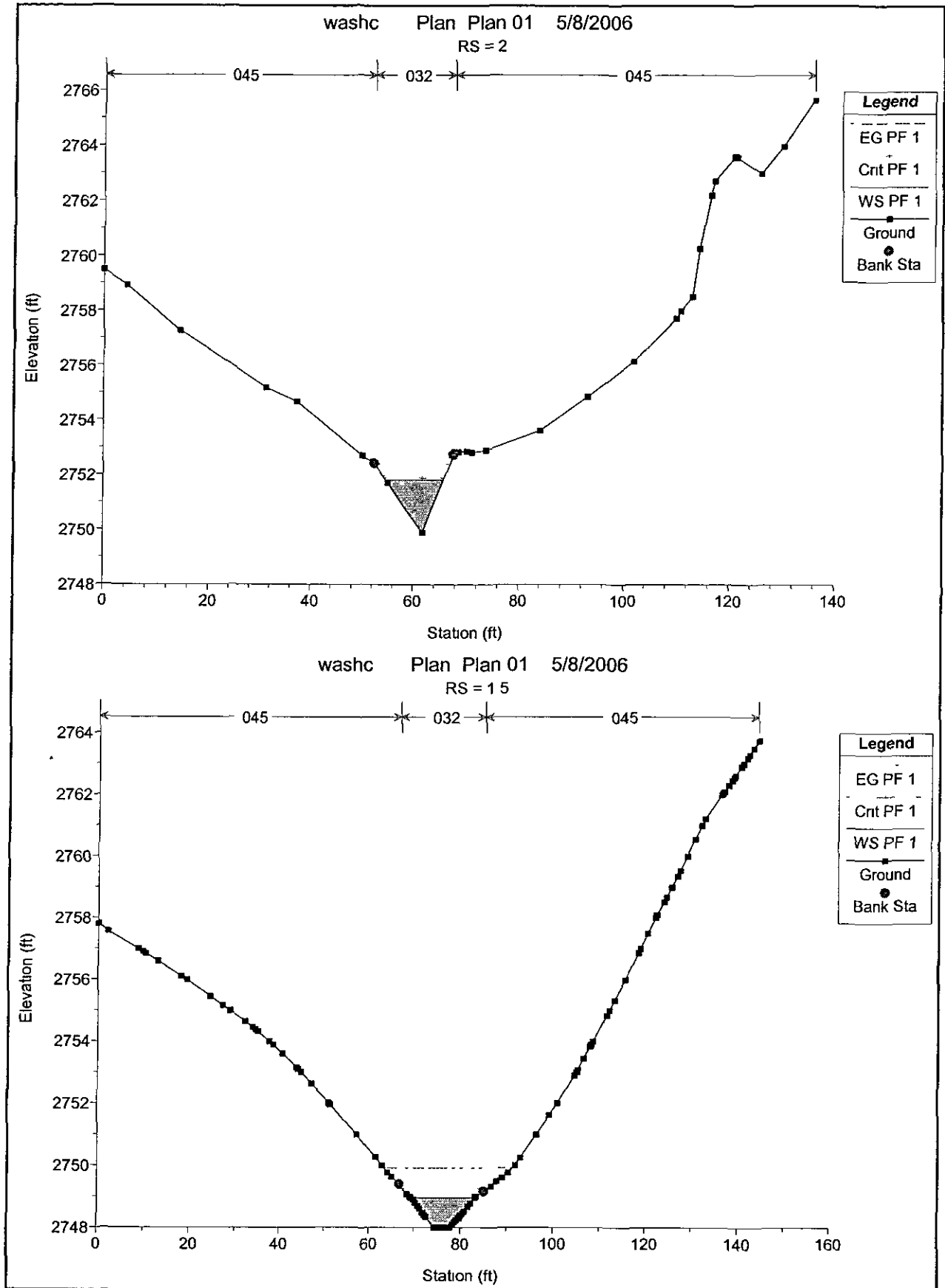
washc Plan Plan 01 5/8/2006  
RS = 5



washc Plan Plan 01 5/8/2006  
RS = 4



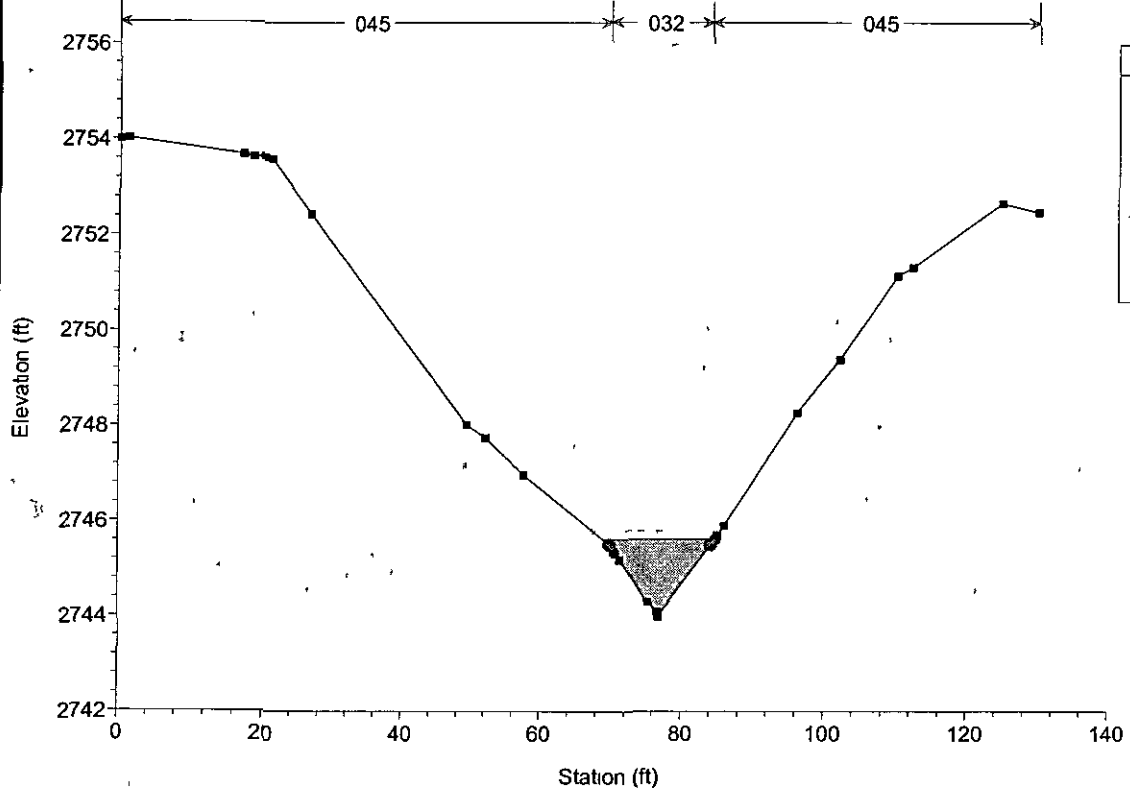






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RS = 1

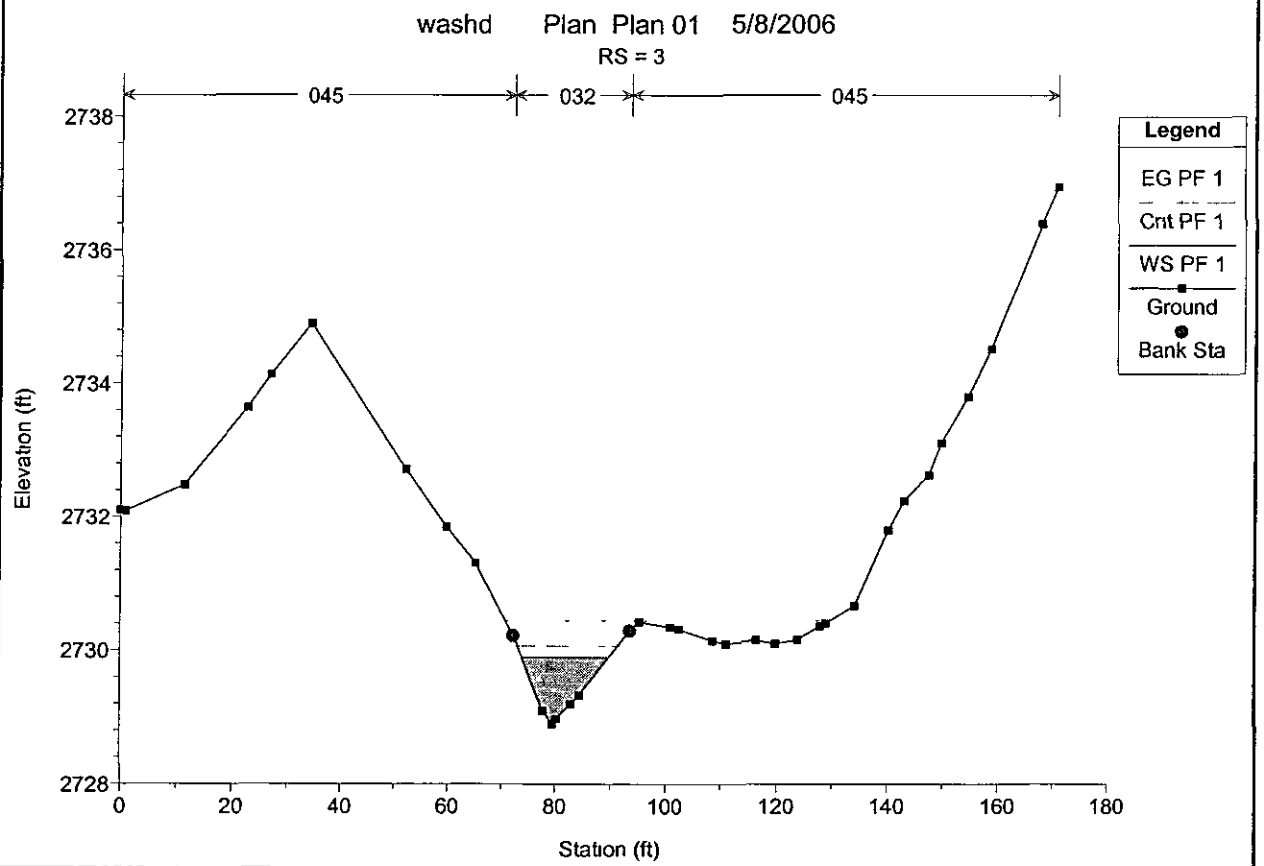
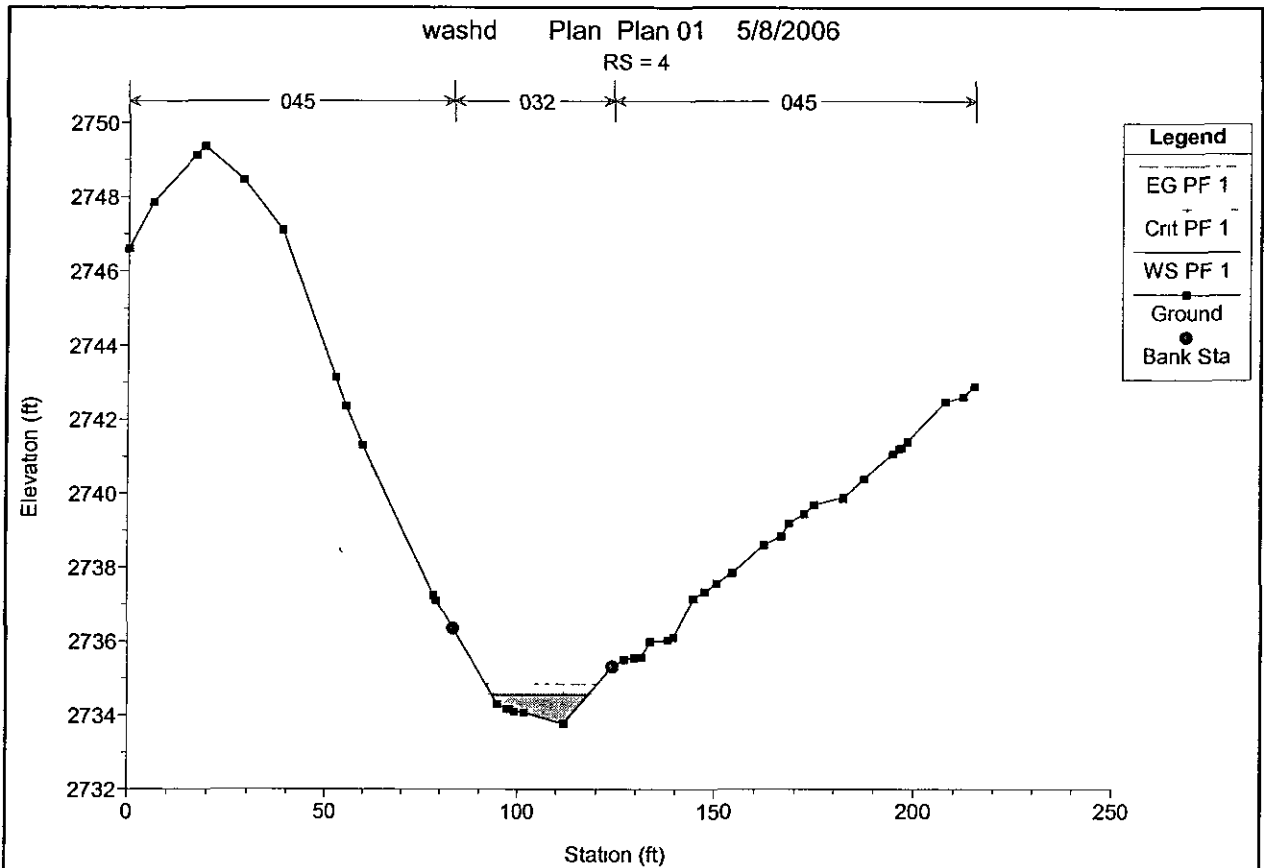


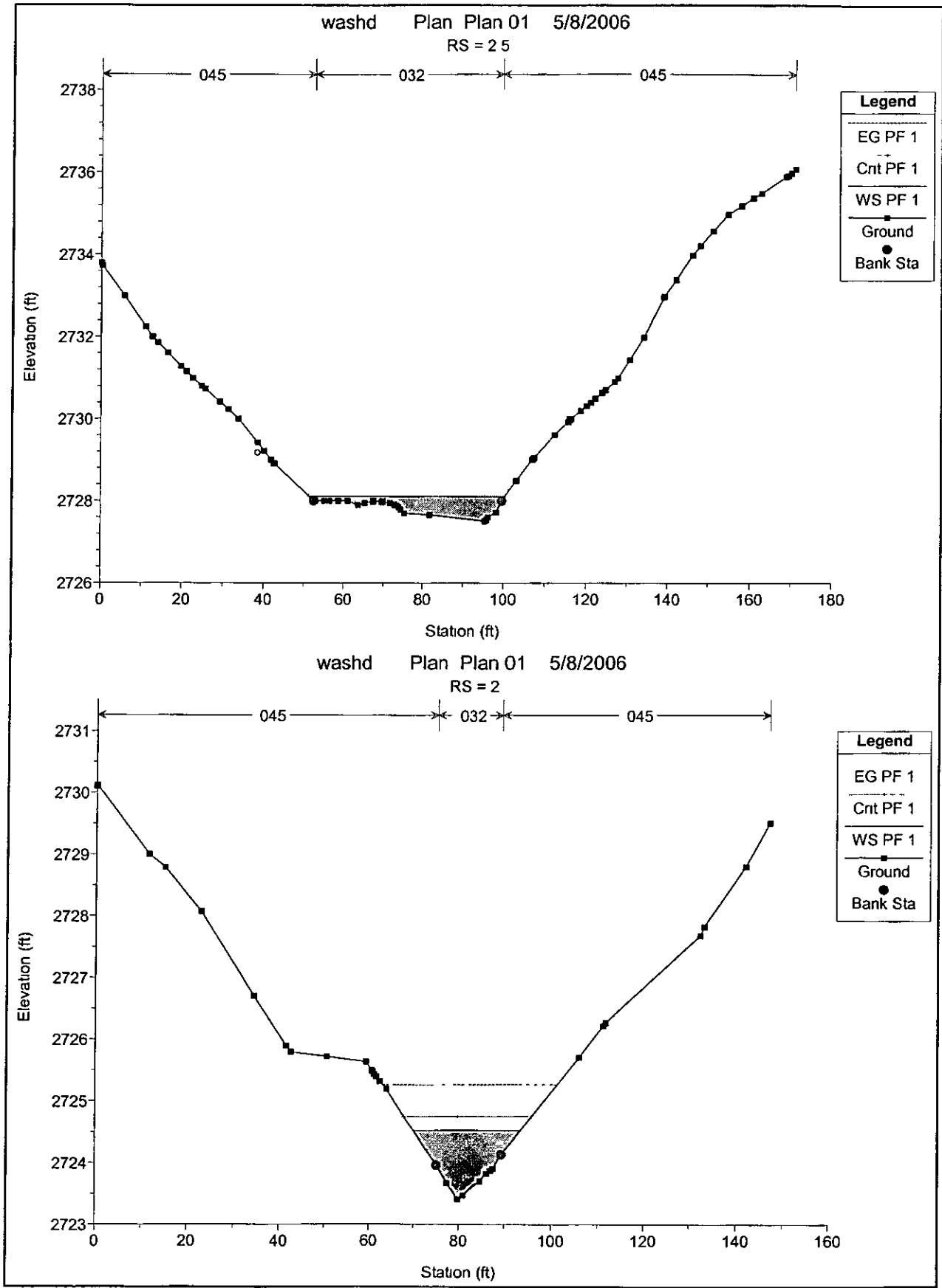
Legend	
---	EG PF 1
---	Crit PF 1
---	WS PF 1
■	Ground
●	Bank Sta

WASH D

HEC-RAS Plan Plan 01 Rver RIVER 1 Reach Reach-1 Profile PF 1

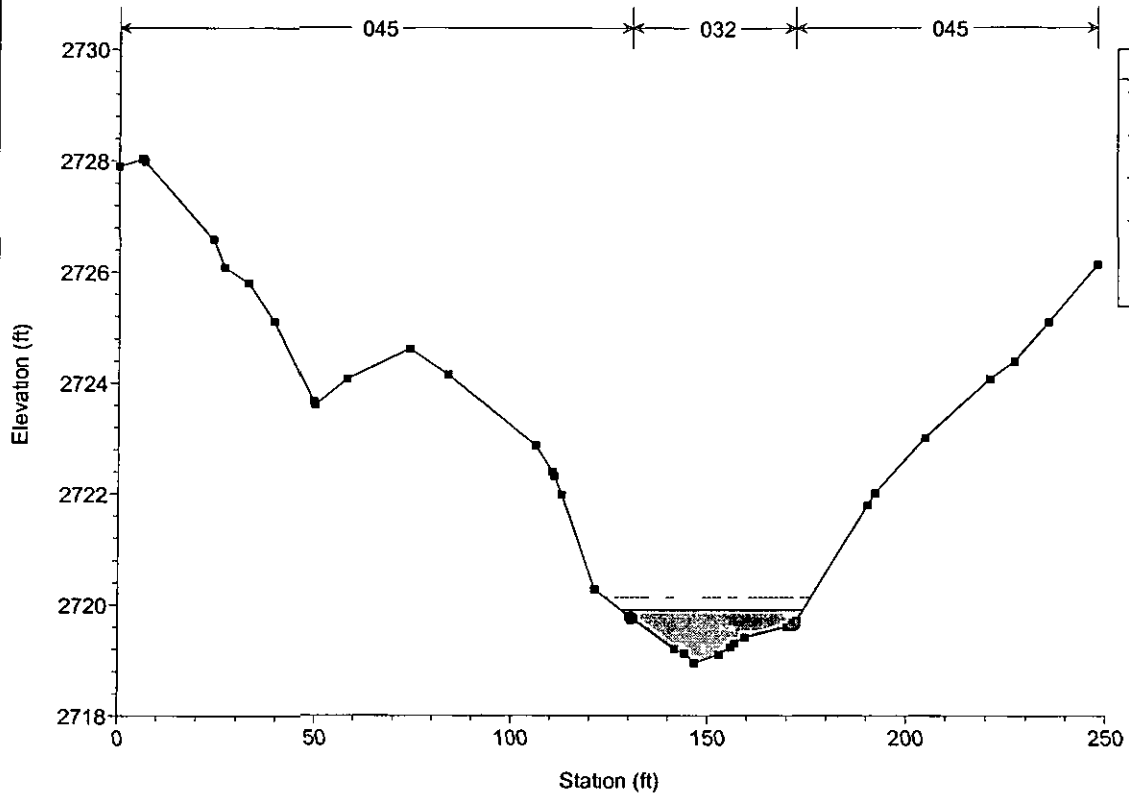
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W S Elev (ft)	Crit W S (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	4	PF_1	50.00	2733.77	2734.54	2734.58	2734.64	0.025006	4.39	11.38	24.53	1.14
Reach-1	3	PF_1	50.00	2728.90	2729.90	2730.06	2730.45	0.038474	5.94	8.42	15.86	1.44
Reach-1	2.5	PF_1	50.00	2727.51	2728.11	2728.12	2728.28	0.023407	3.32	15.16	49.07	1.03
Reach-1	2	PF_1	88.00	2723.41	2724.53	2724.75	2725.26	0.030279	7.07	14.00	23.61	1.37
Reach-1	1	PF_1	88.00	2718.95	2719.91	2719.86	2720.13	0.014006	3.75	23.83	46.23	0.88





washd Plan Plan 01 5/8/2006

RS = 1

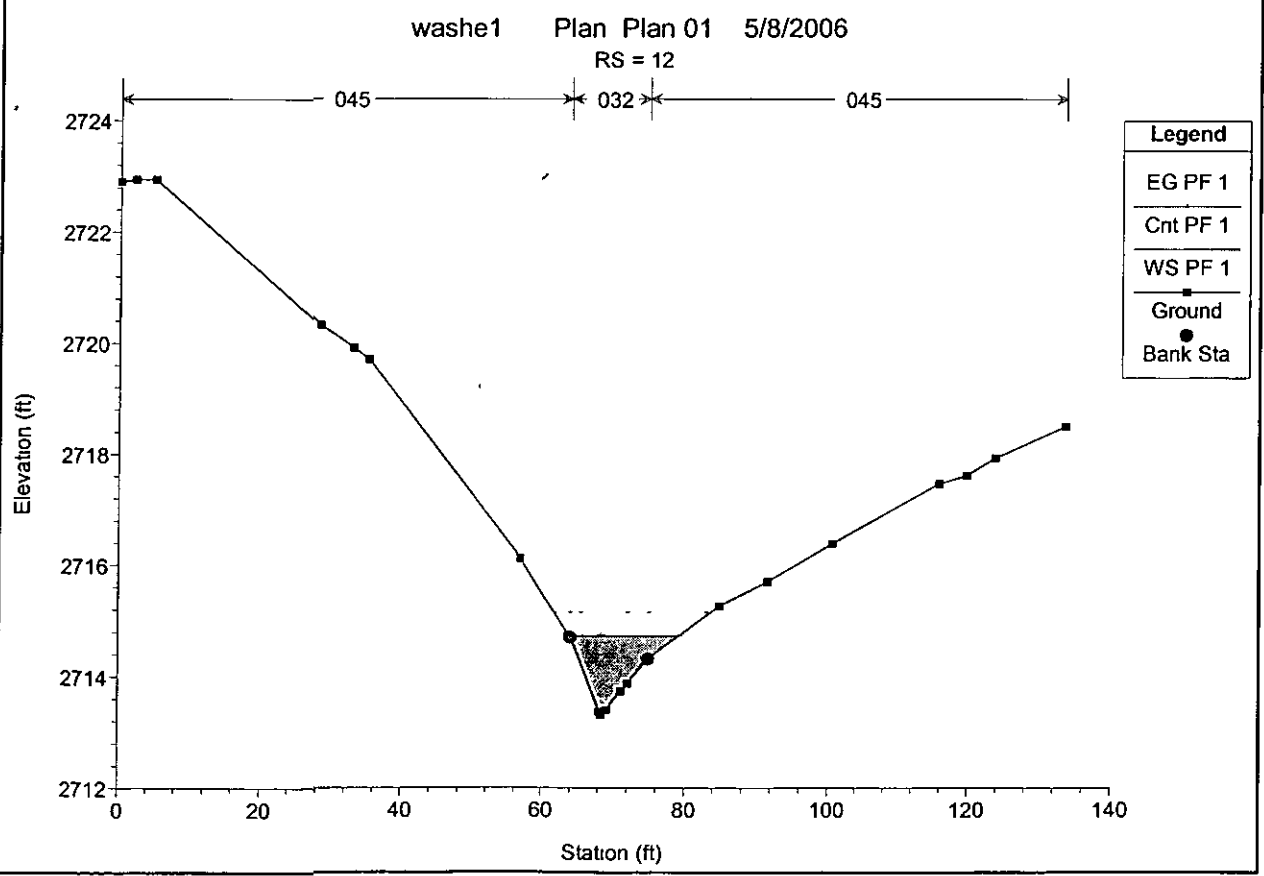
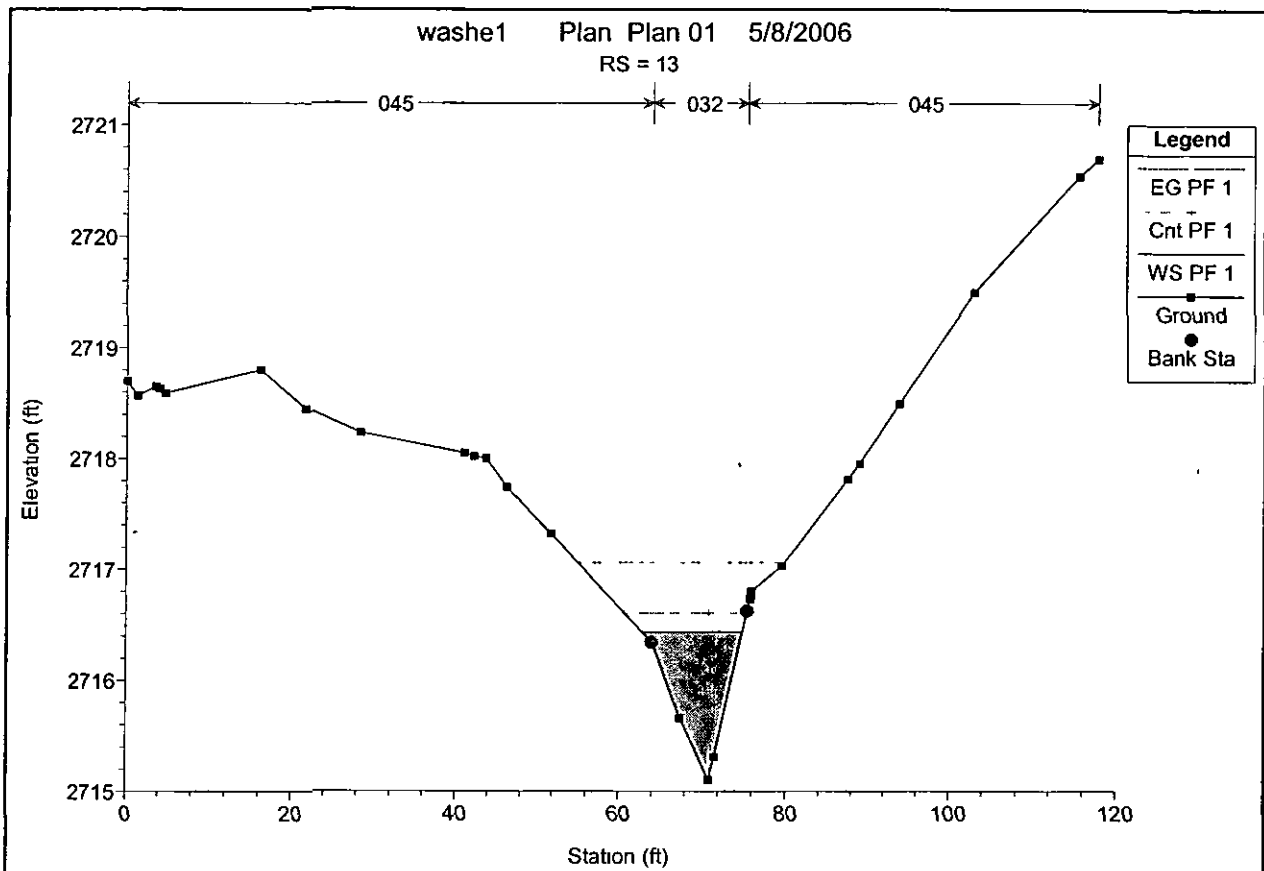


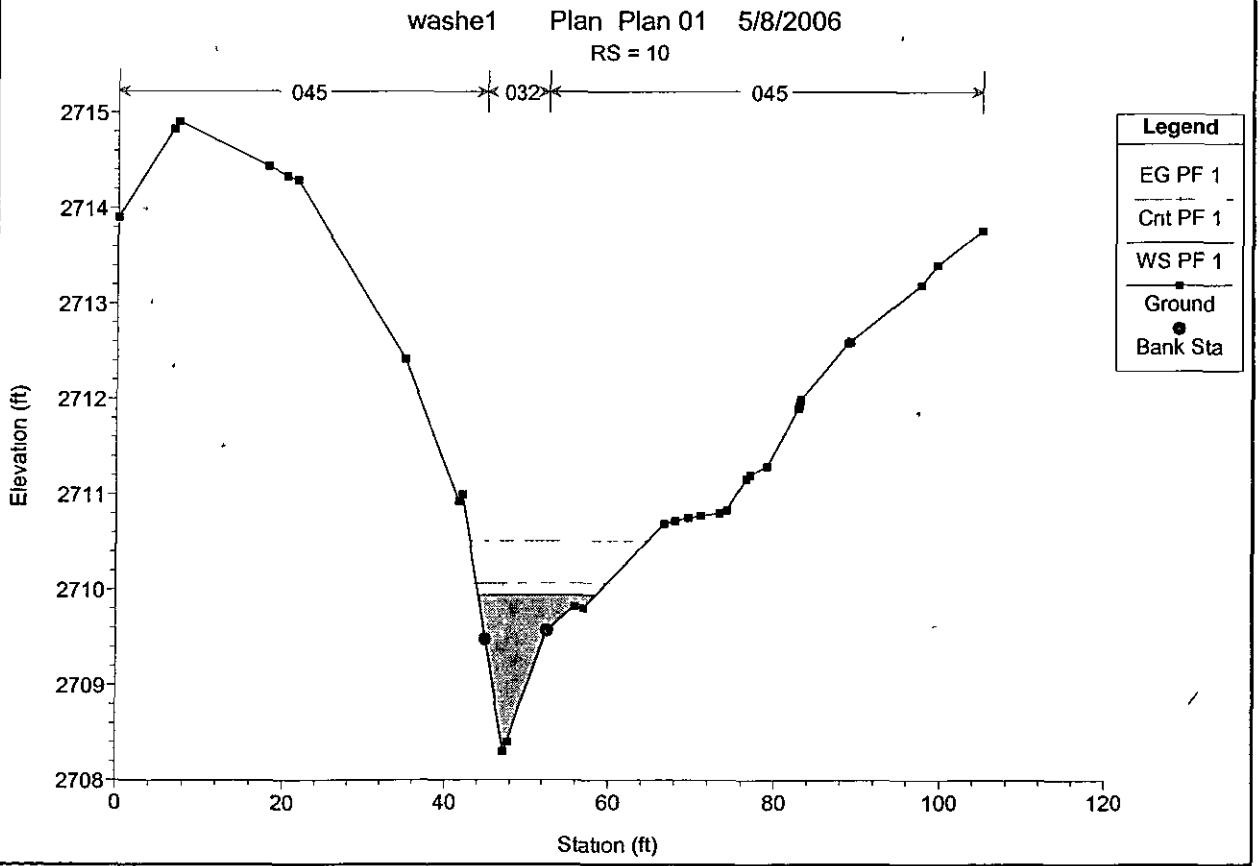
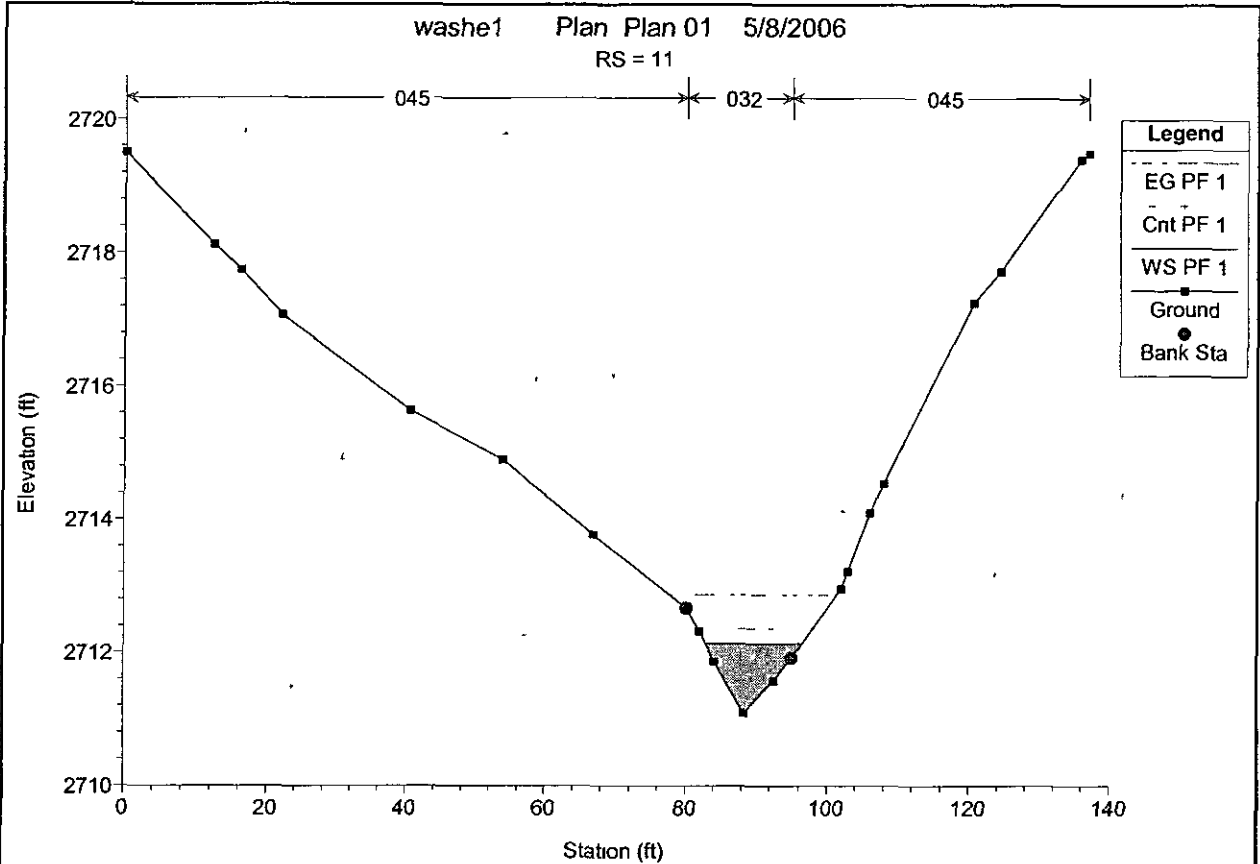
Legend	
EG PF 1	
WS PF 1	
Crit PF 1	
Ground	●
Bank Sta	●

# WASH EI

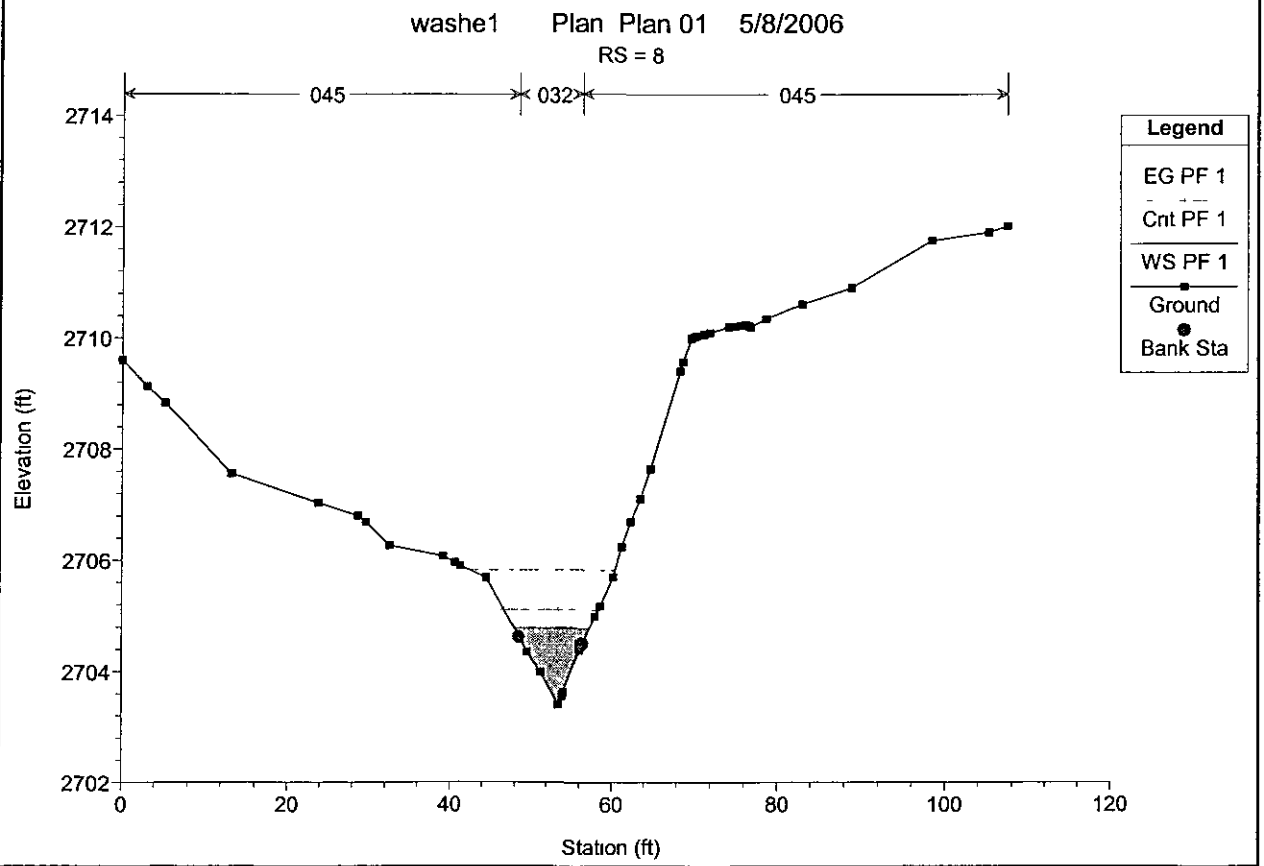
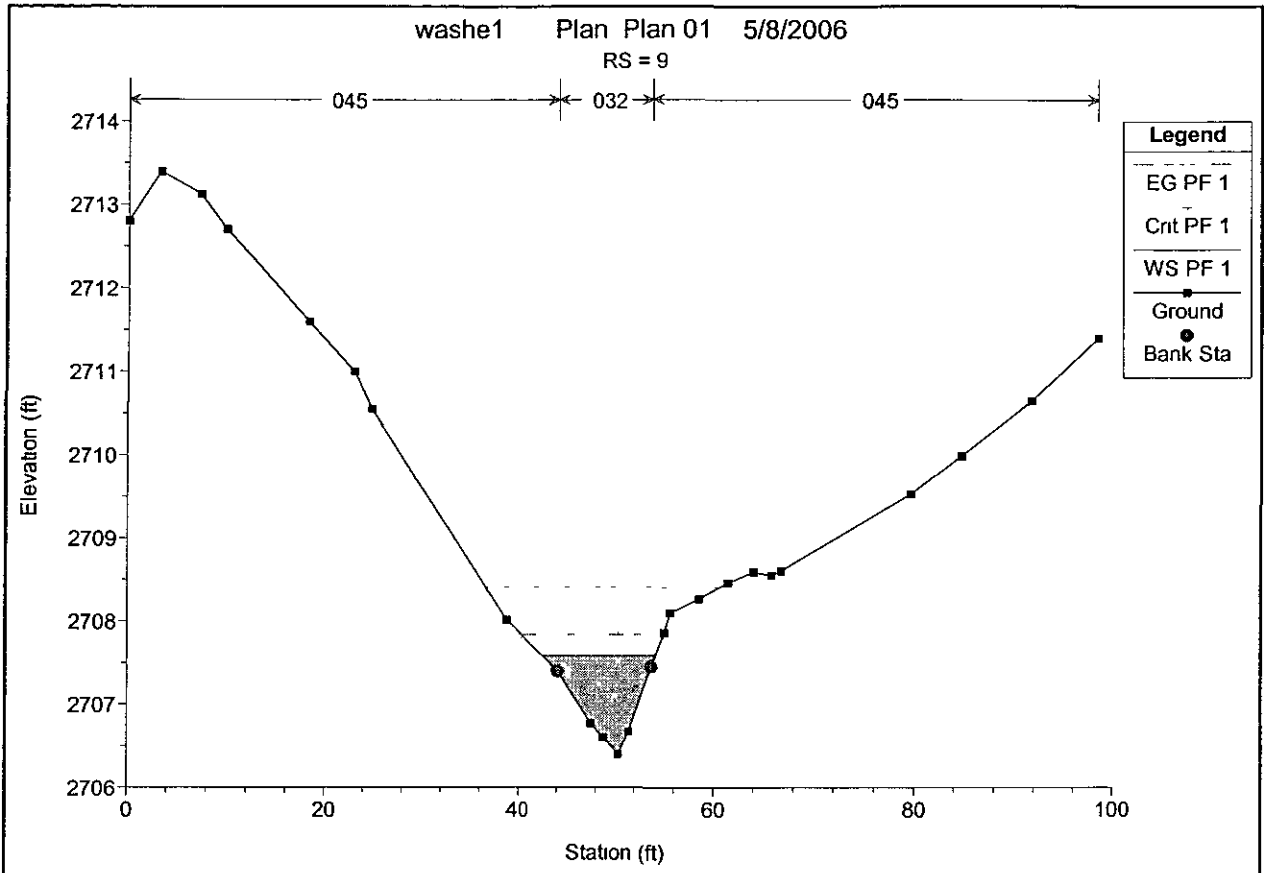
HEC-RAS Plan Plan 01 River RIVER 1 Reach Reach 1 Profile PF 1

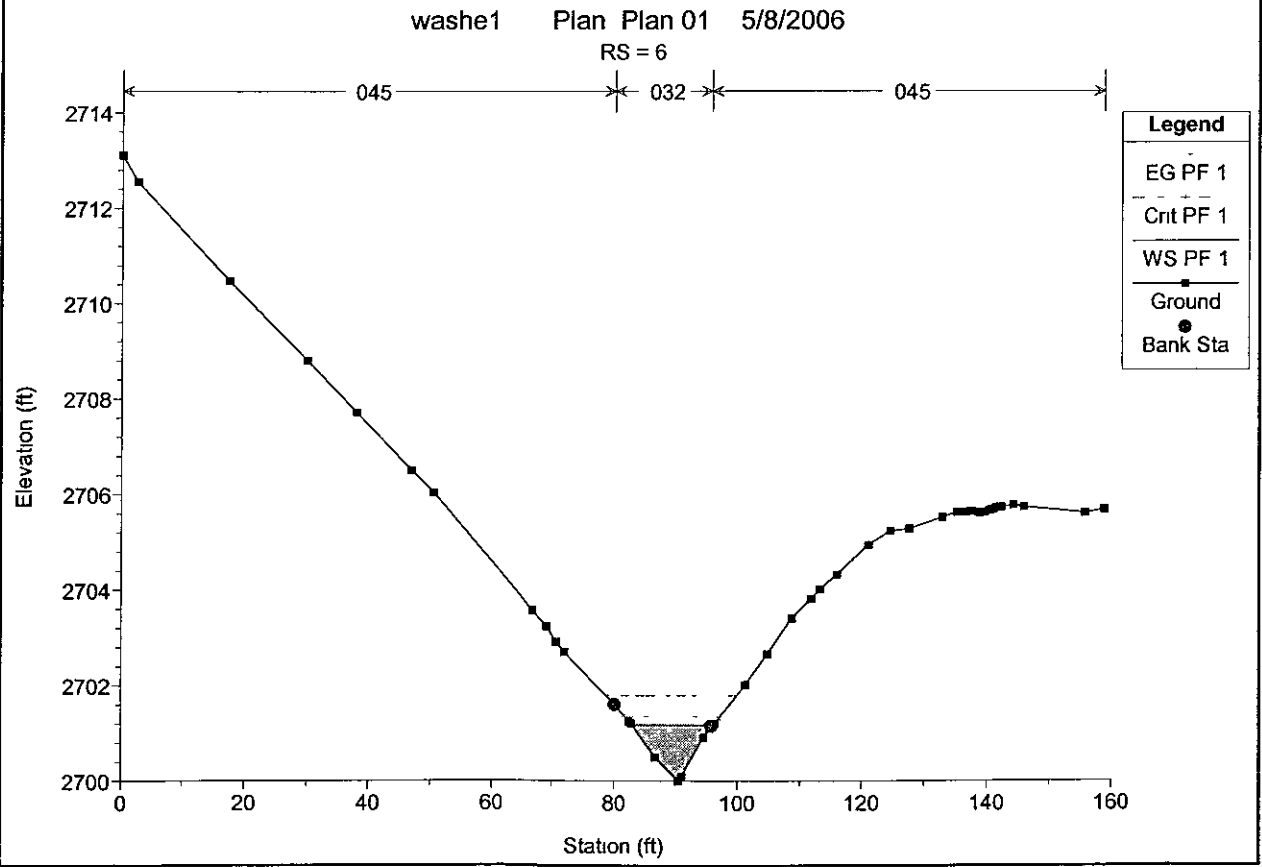
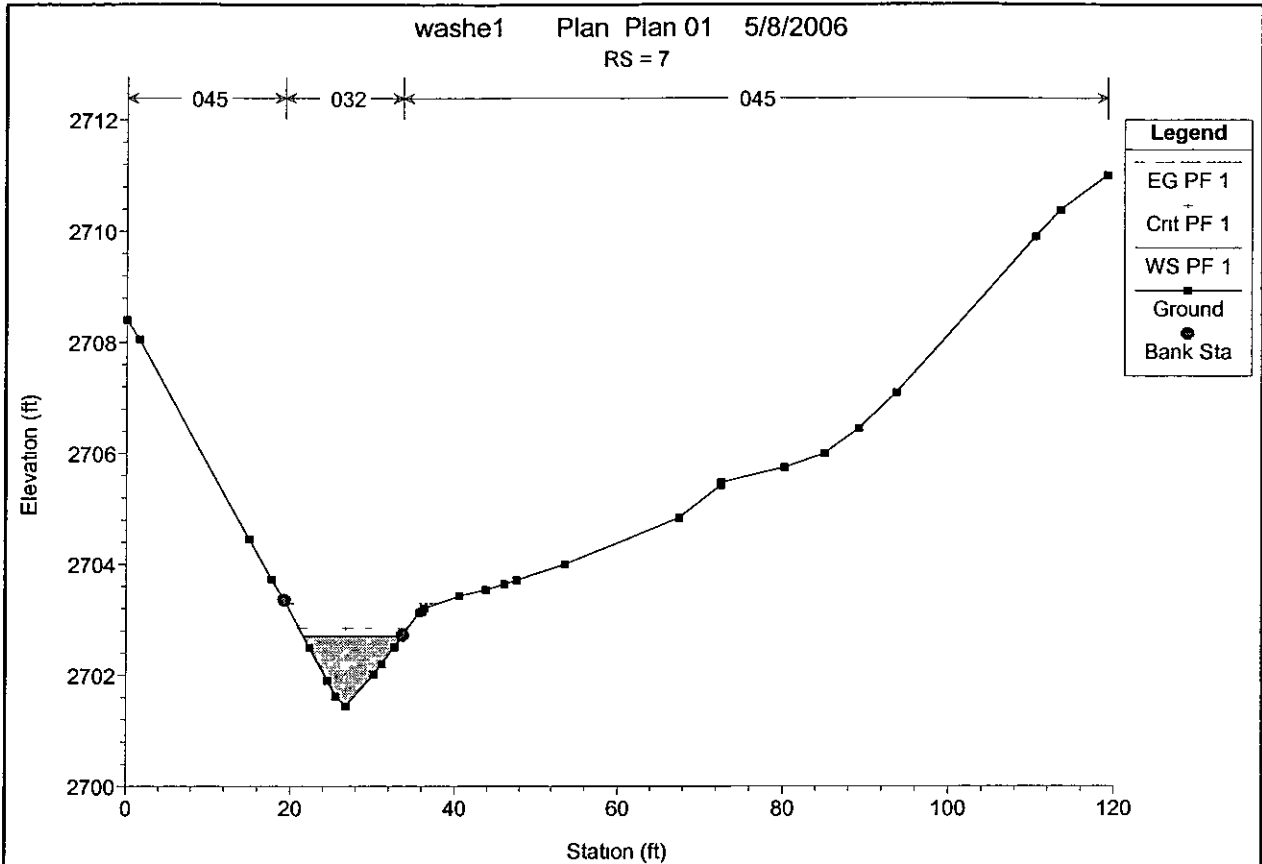
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W S Elev (ft)	Cent W S (ft)	E.G Elev (ft)	E.G Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Ch
Reach-1	13	PF_1	50.00	2715.10	2716.43	2716.60	2717.06	0.030018	6.34	7.93	12.10	1.32
Reach-1	12	PF_1	50.00	2713.32	2714.73	2714.77	2715.16	0.017142	5.31	10.05	15.35	1.02
Reach-1	11	PF_1	50.00	2711.10	2712.13	2712.36	2712.87	0.045013	6.89	7.38	13.63	1.58
Reach-1	10	PF_1	50.00	2708.30	2709.94	2710.07	2710.51	0.018336	6.16	9.08	14.42	1.07
Reach-1	9	PF_1	50.00	2706.40	2707.58	2707.84	2708.40	0.040012	7.29	7.00	11.58	1.52
Reach-1	8	PF_1	50.00	2703.40	2704.77	2705.10	2705.82	0.046780	8.23	6.21	9.25	1.65
Reach-1	7	PF_1	50.00	2701.44	2702.70	2702.85	2703.29	0.030785	6.17	8.11	12.04	1.32
Reach-1	6	PF_1	50.00	2700.00	2701.17	2701.35	2701.79	0.036001	6.31	7.92	13.05	1.42
Reach-1	5	PF_1	110.00	2696.59	2698.30	2698.33	2698.80	0.017684	5.71	19.43	23.16	1.06
Reach-1	4	PF_1	110.00	2692.81	2694.46	2694.74	2695.37	0.030727	7.70	14.71	19.32	1.40
Reach-1	3	PF_1	110.00	2692.00	2692.95	2692.96	2693.32	0.014354	5.00	24.98	39.27	0.95
Reach-1	3	PF_1	136.00	2689.21	2690.89	2691.20	2691.69	0.023022	7.41	21.69	36.01	1.25
Reach-1	2	PF_1	258.00	2679.83	2681.80	2682.30	2683.31	0.034090	9.93	27.09	26.99	1.55
Reach-1	1	PF_1	258.00	2677.00	2679.30	2679.52	2680.19	0.016095	8.28	41.22	40.05	1.11

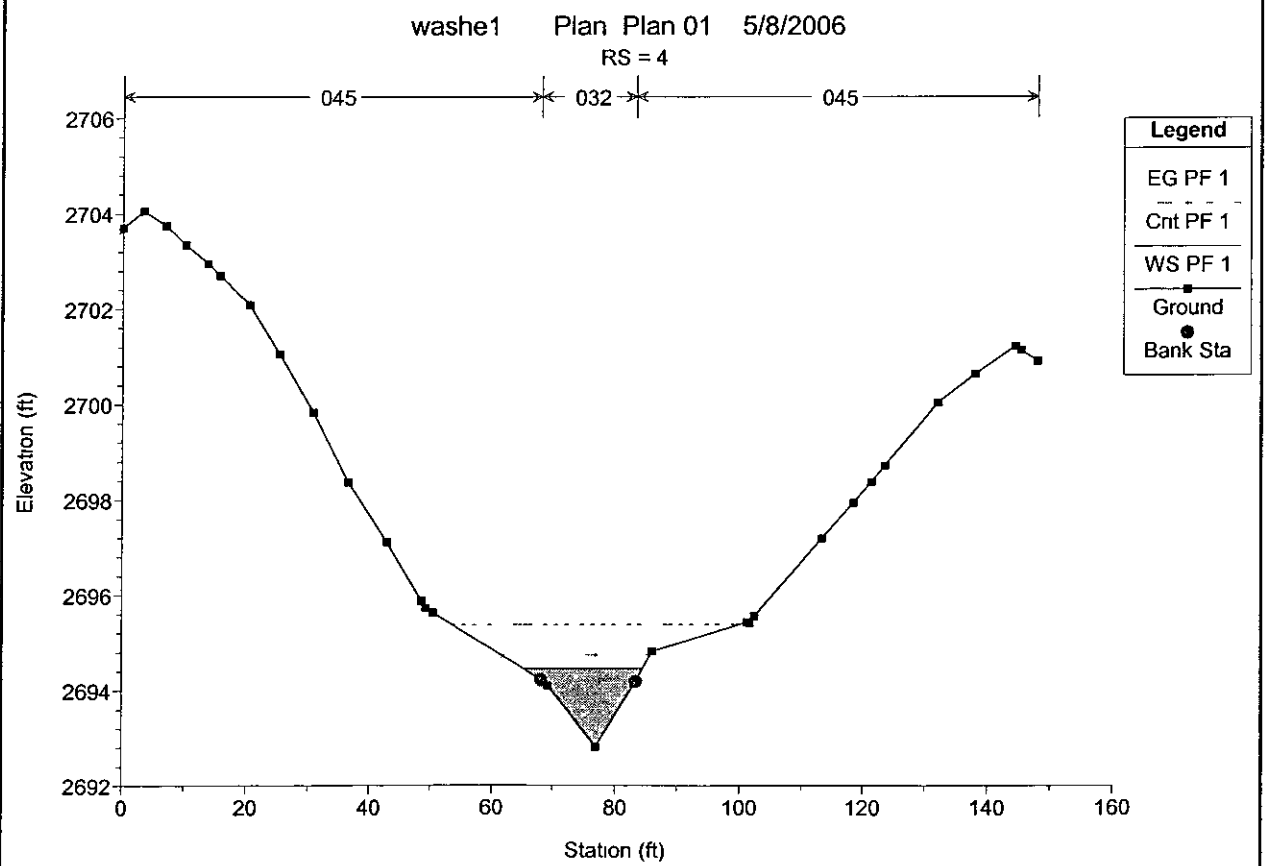
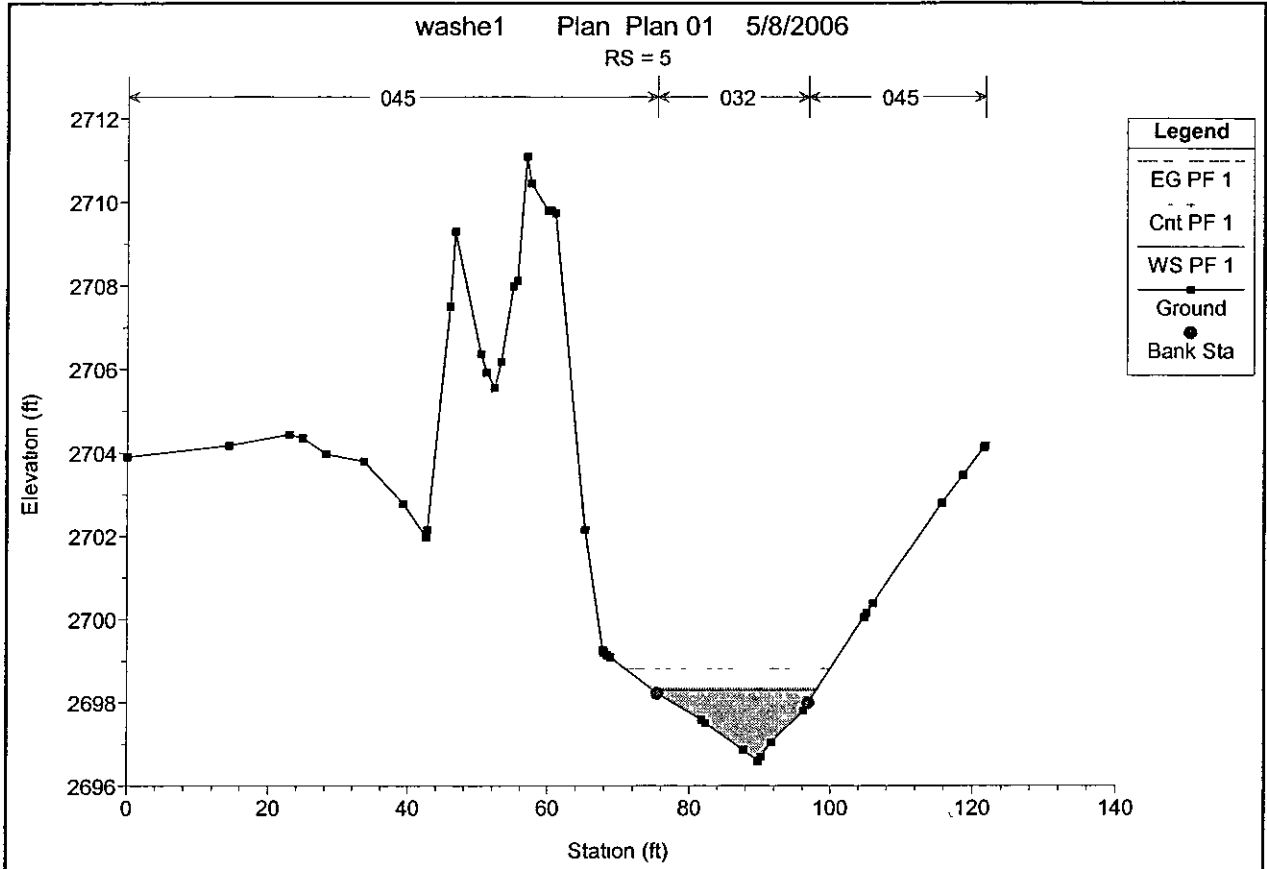


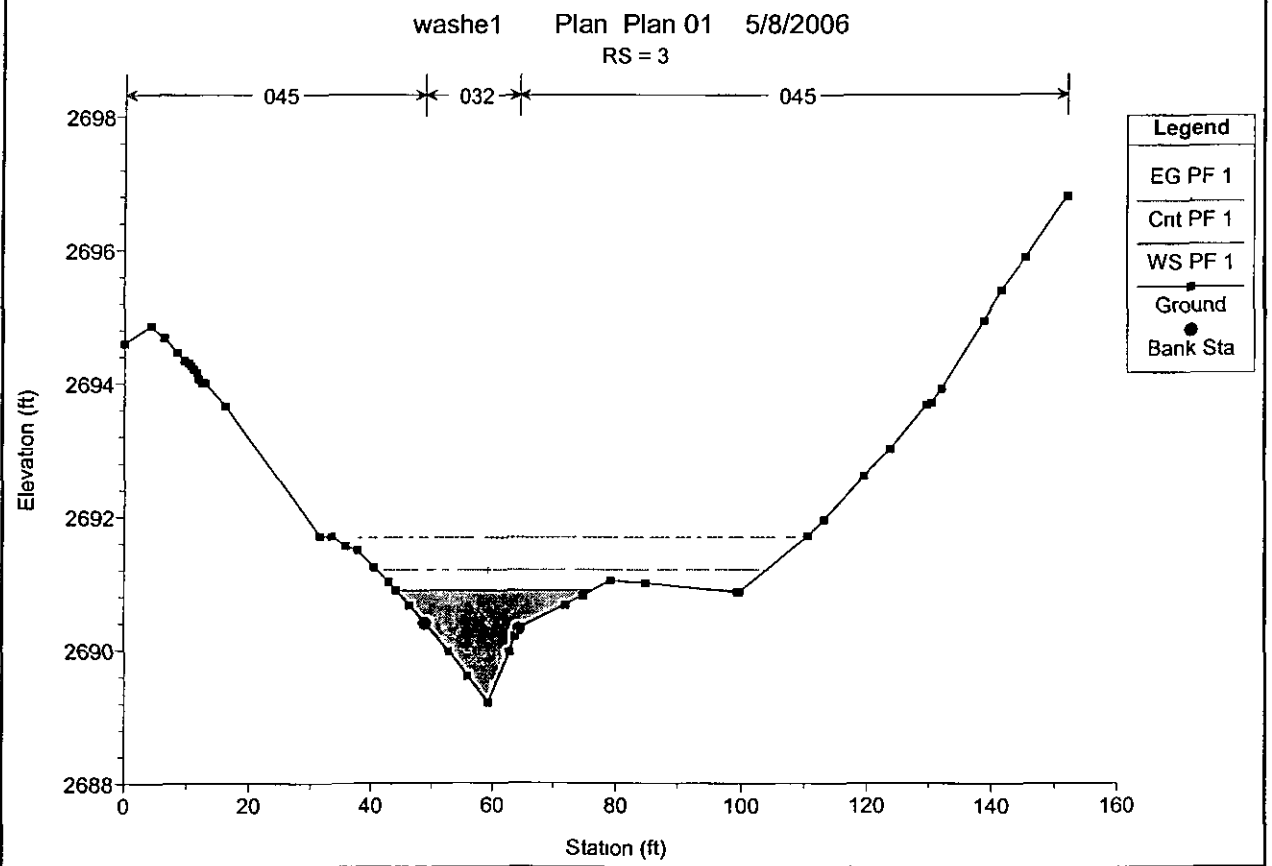
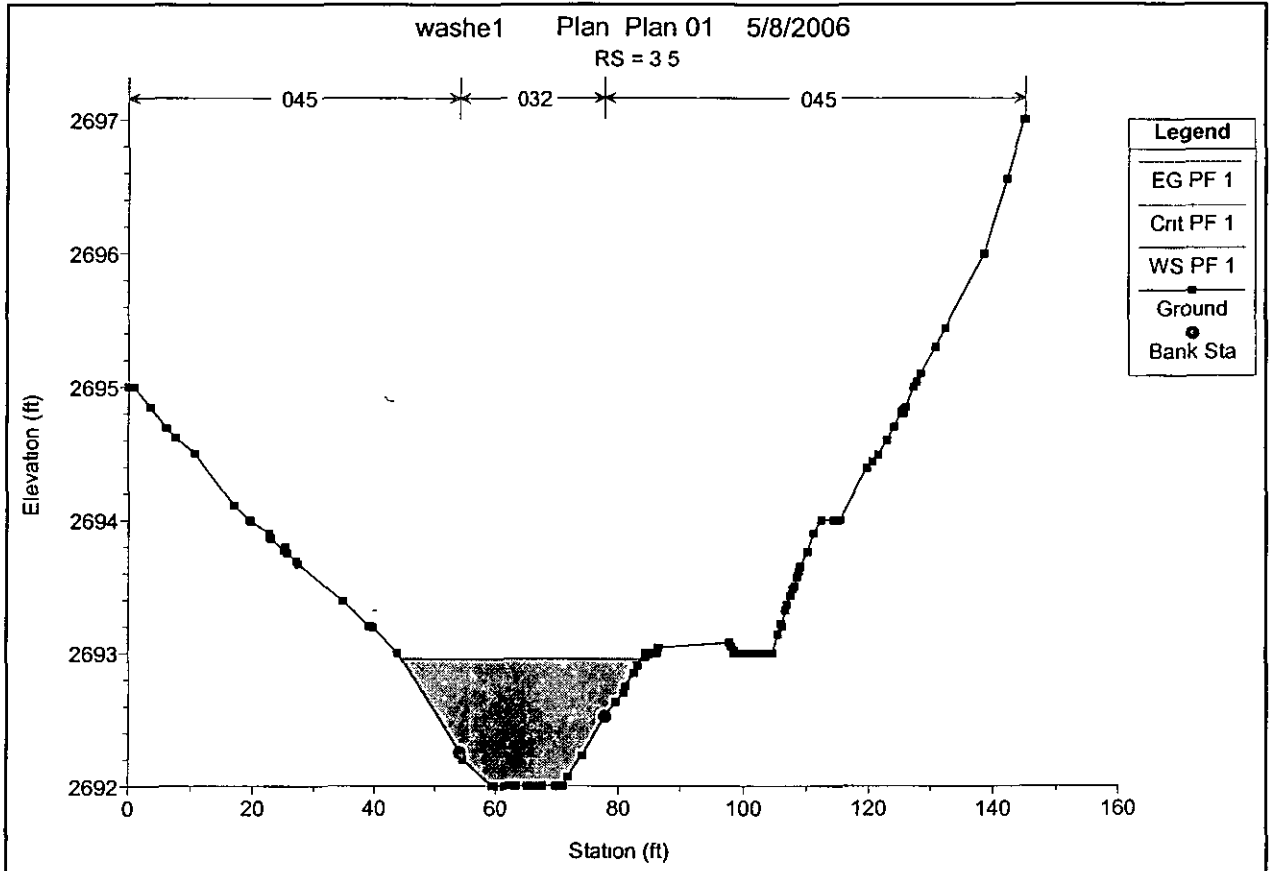


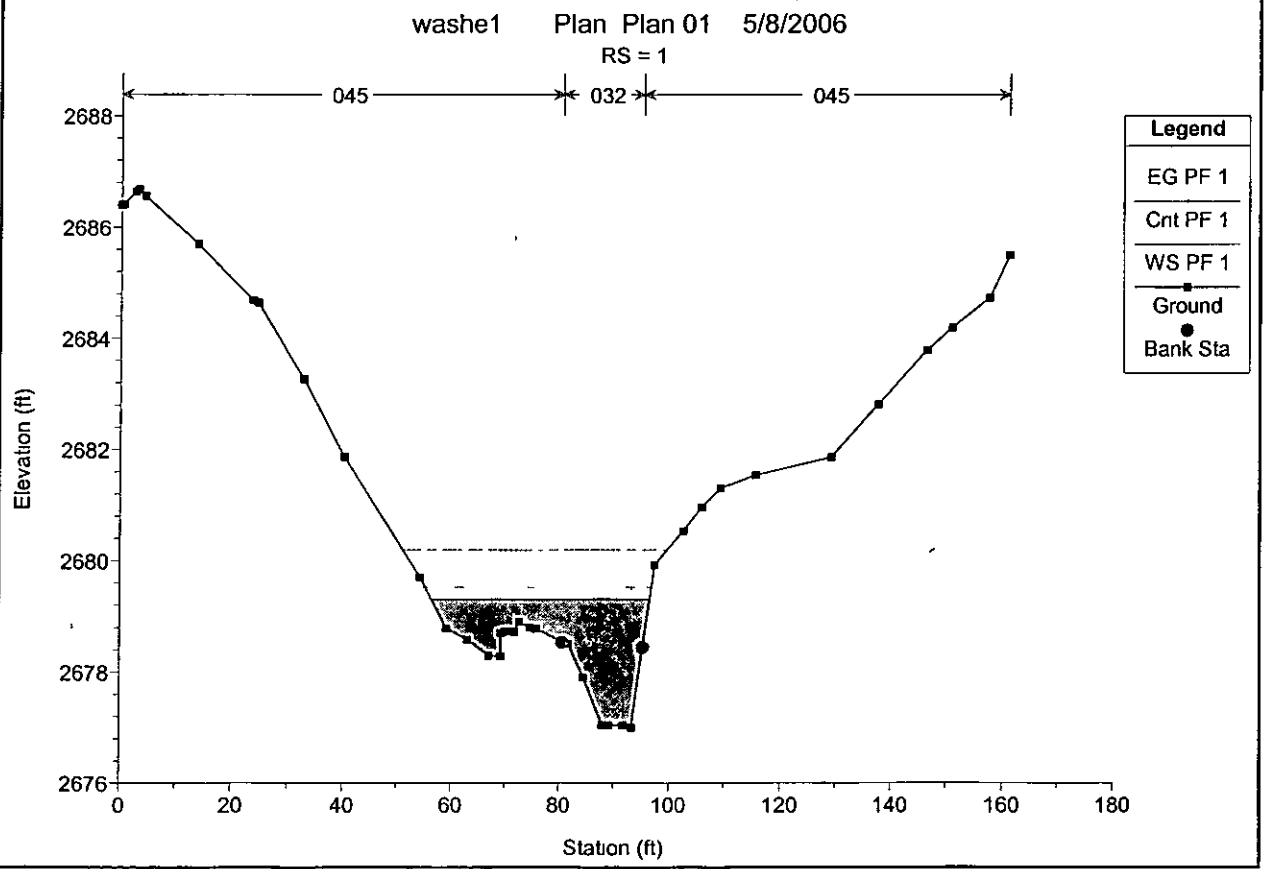
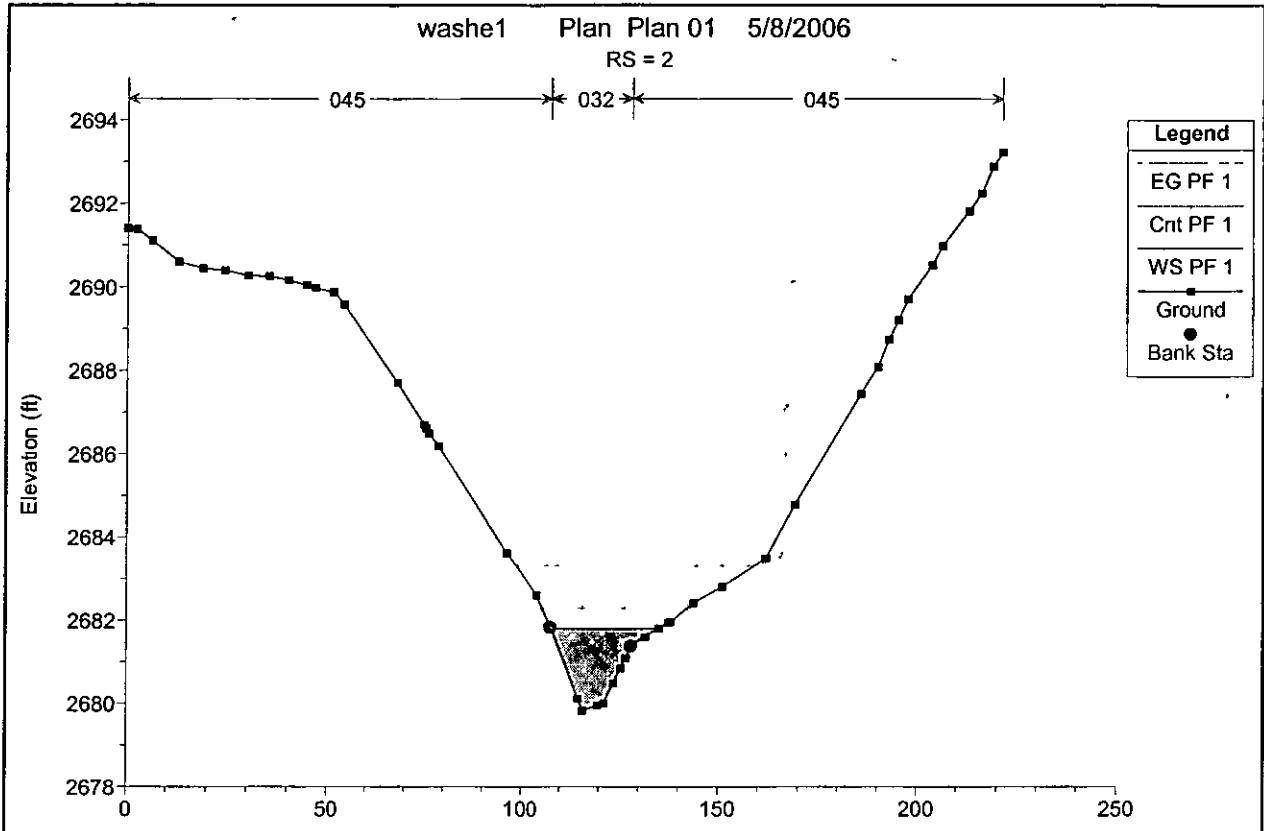








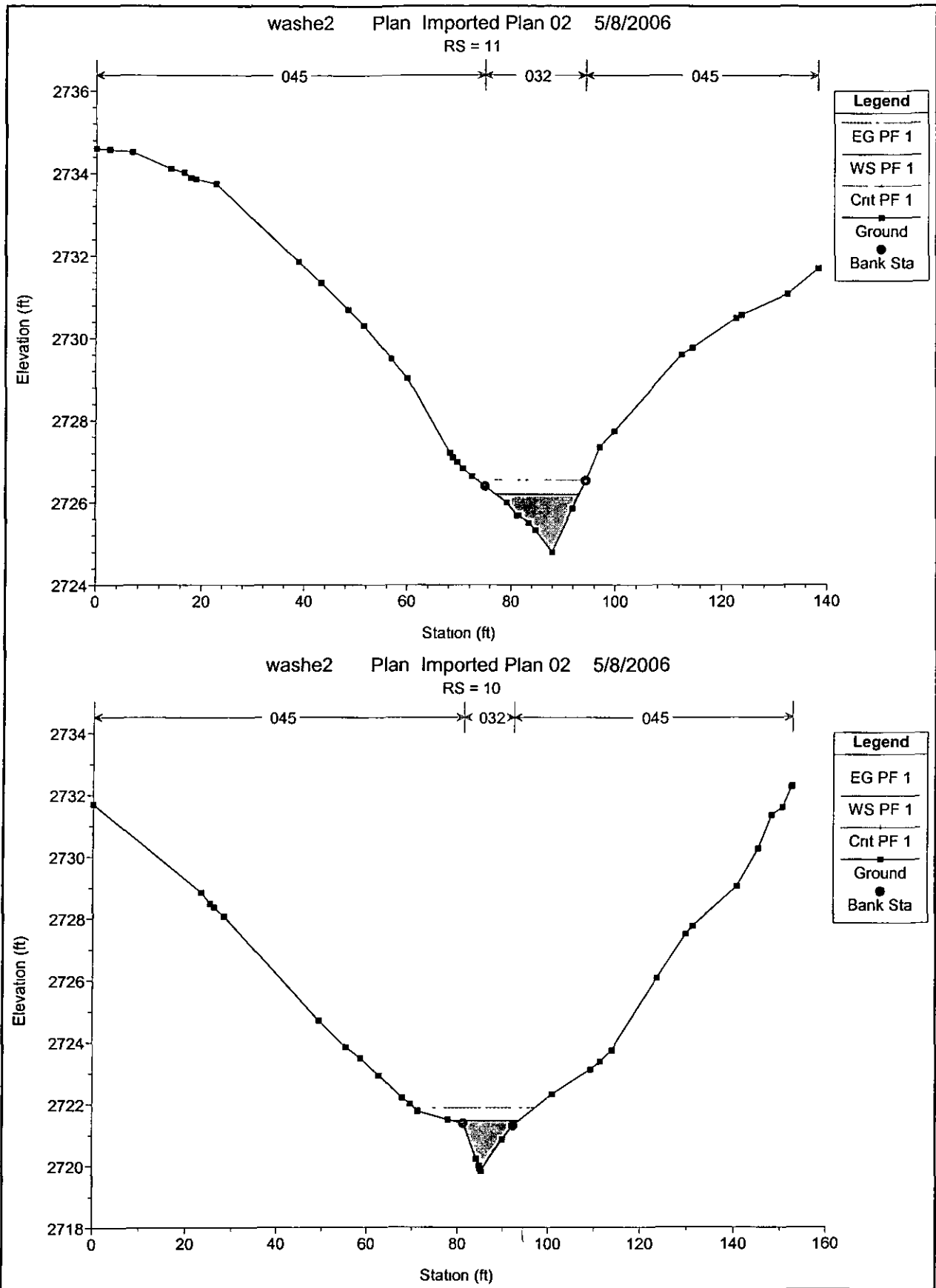


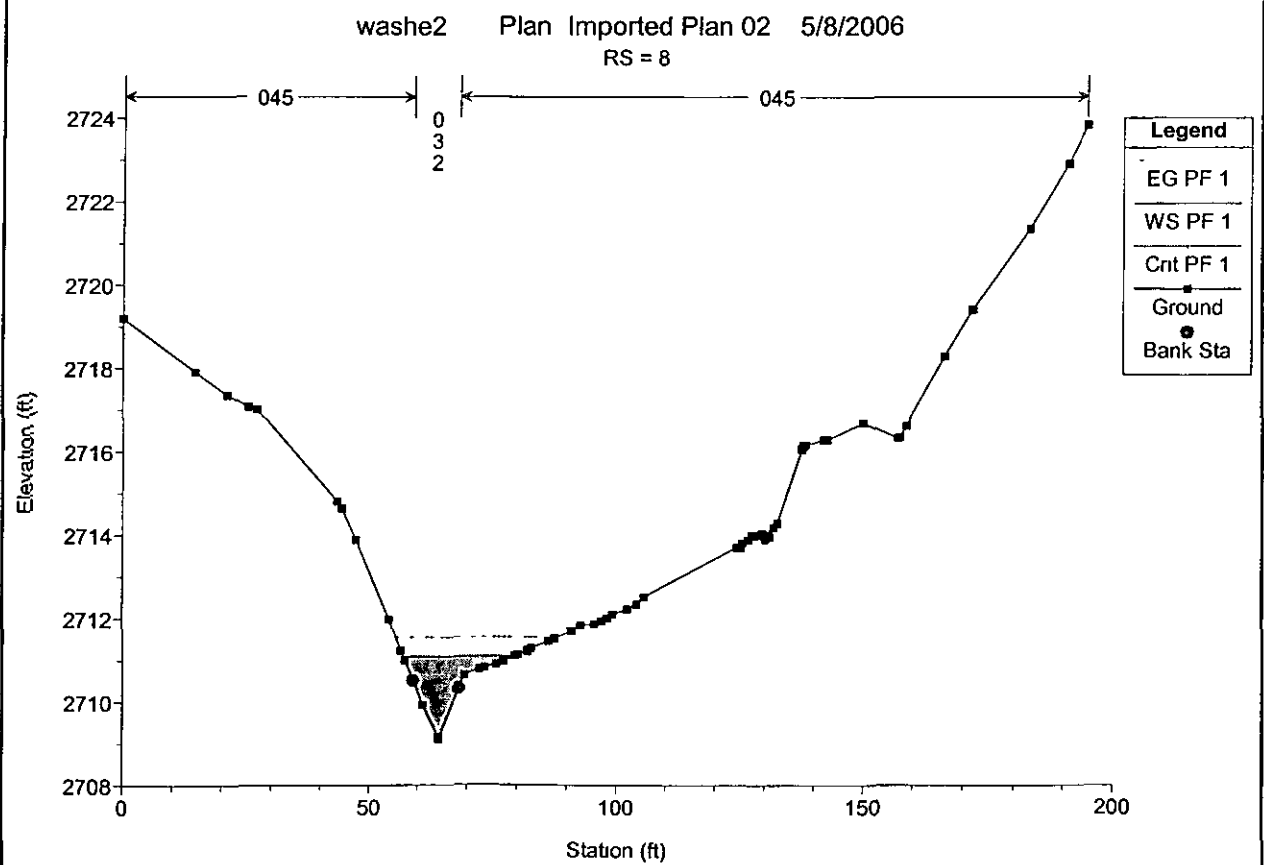
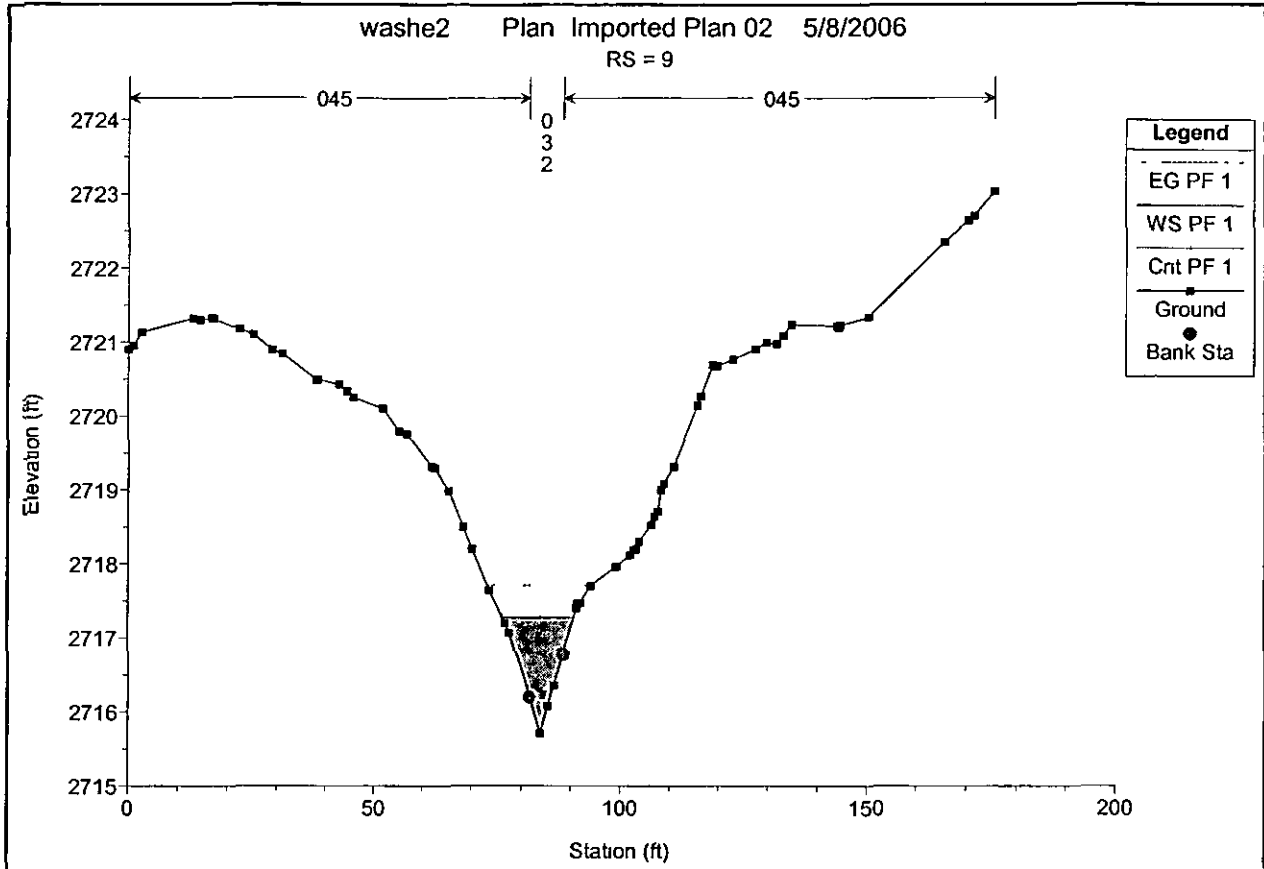


# WASH E2

HEC-RAS Plan Imported Pla River RIVER-1 Reach Reach-1 Profile PF 1

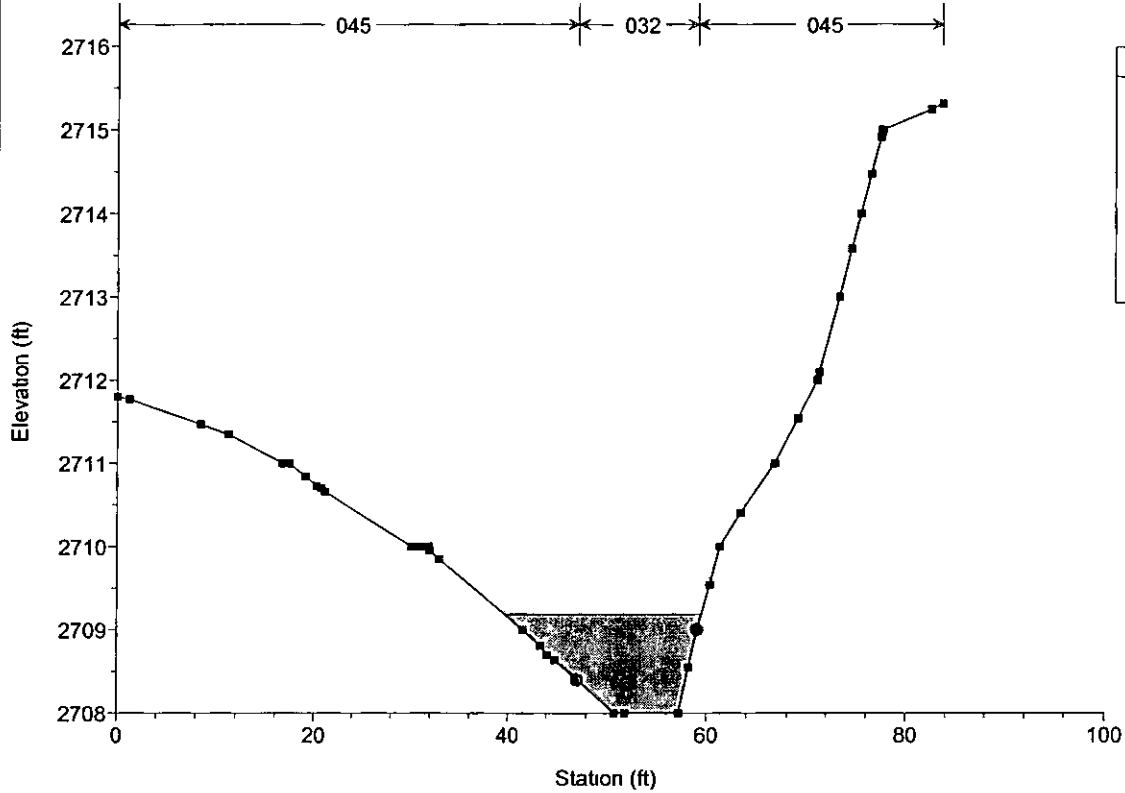
Reach	River Sta	Profile	Q Total (cfs)	Min Ch Elev (ft)	W S Elev (ft)	Crit W S (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach 1	11	PF 1	50.00	2724.80	2726.21	2726.21	2726.55	0.018084	4.69	10.67	16.13	1.02
Reach 1	10	PF 1	50.00	2719.82	2721.45	2721.45	2721.87	0.015739	5.17	9.85	14.80	0.98
Reach 1	9	PF 1	50.00	2715.71	2717.28	2717.28	2717.71	0.013081	5.64	10.87	14.46	0.94
Reach 1	8	PF 1	75.00	2709.10	2711.09	2711.09	2711.55	0.010257	5.57	16.18	22.55	0.85
Reach 1	7.5	PF 1	75.00	2708.00	2709.19	2709.19	2709.62	0.013563	5.49	15.49	19.83	0.95
Reach 1	7	PF 1	75.00	2705.46	2706.94	2706.94	2707.36	0.015127	5.18	14.90	20.27	0.98
Reach 1	6	PF 1	75.00	2701.53	2703.58	2703.58	2704.11	0.014543	5.84	13.21	14.90	0.97
Reach 1	5	PF 1	75.00	2697.79	2699.88	2699.88	2700.43	0.010621	6.48	15.38	16.38	0.89
Reach 1	4.5	PF 1	75.00	2697.00	2698.52	2698.52	2699.00	0.012220	5.77	14.89	17.87	0.92
Reach 1	4	PF 1	75.00	2694.91	2696.38	2696.38	2696.78	0.016000	5.09	14.92	21.12	0.99
Reach 1	3	PF 1	129.00	2687.45	2689.80	2689.80	2690.42	0.012827	6.38	21.52	20.91	0.95
Reach 1	2	PF 1	258.00	2679.83	2682.33	2682.33	2683.02	0.009910	6.95	44.53	37.38	0.89
Reach 1	1	PF 1	258.00	2677.00	2679.50	2679.50	2680.04	0.016298	6.88	49.30	41.36	0.82



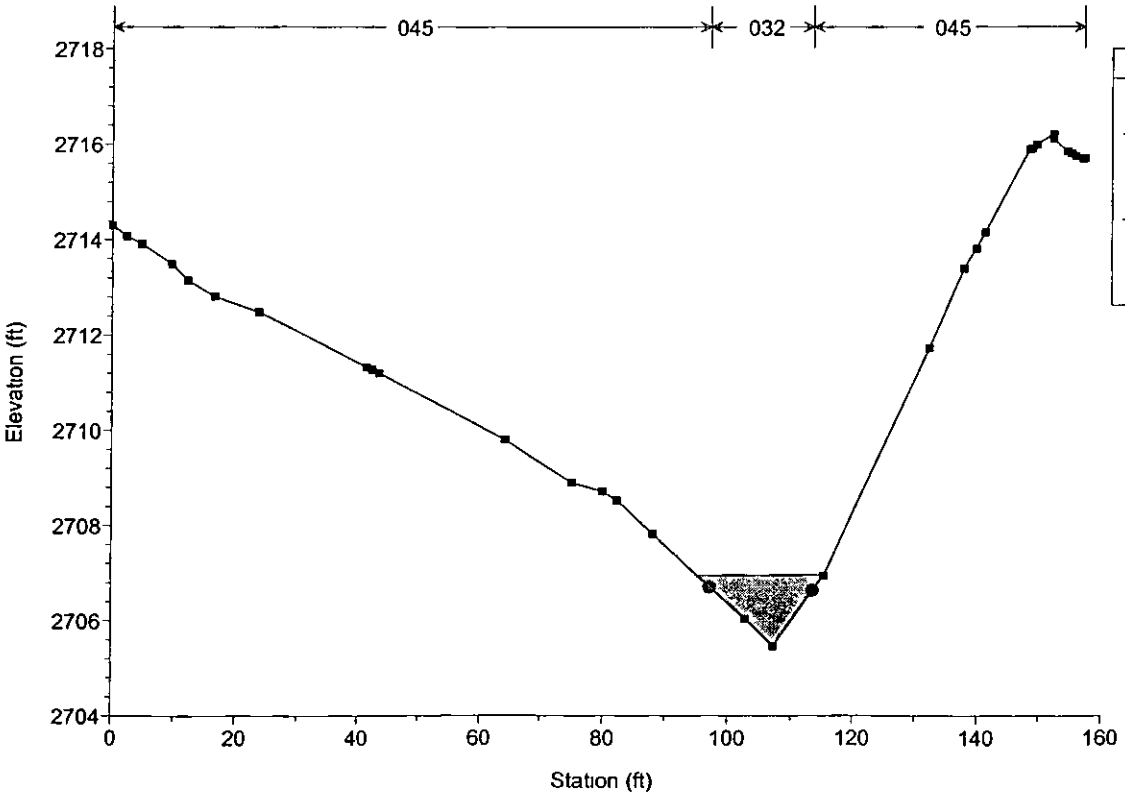




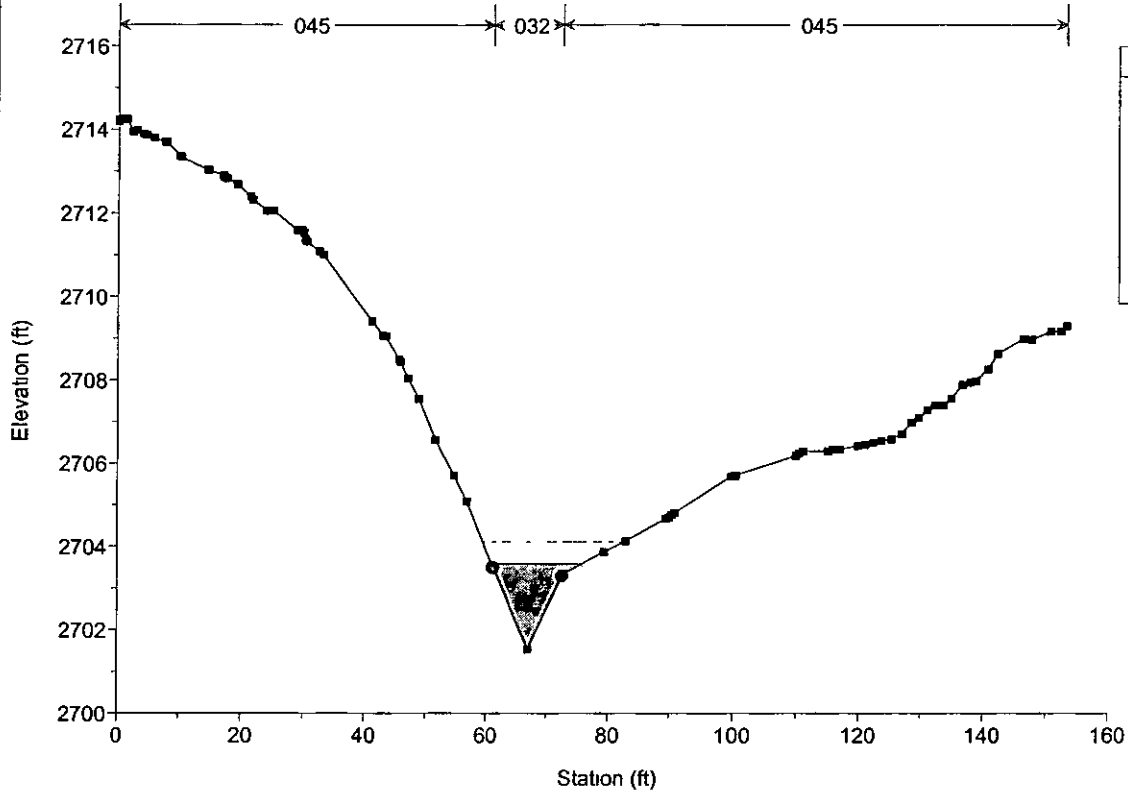
washe2 Plan Imported Plan 02 5/8/2006  
RS = 7.5



washe2 Plan Imported Plan 02 5/8/2006  
RS = 7

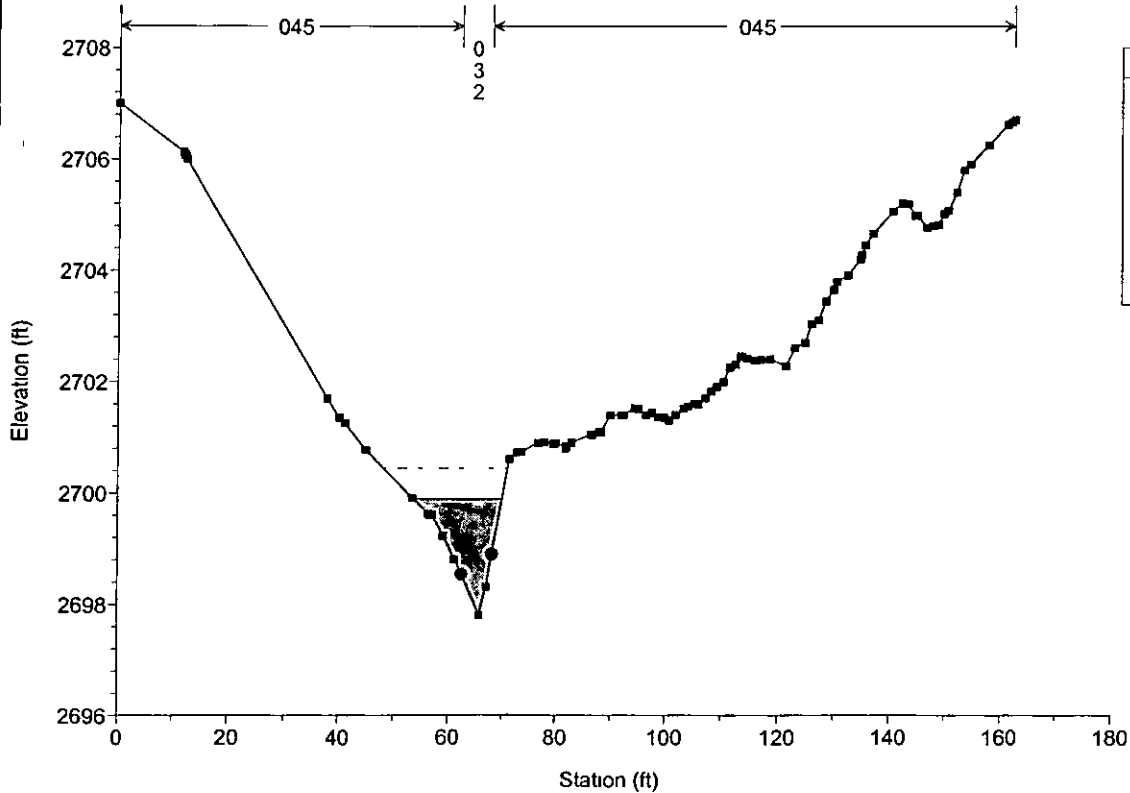


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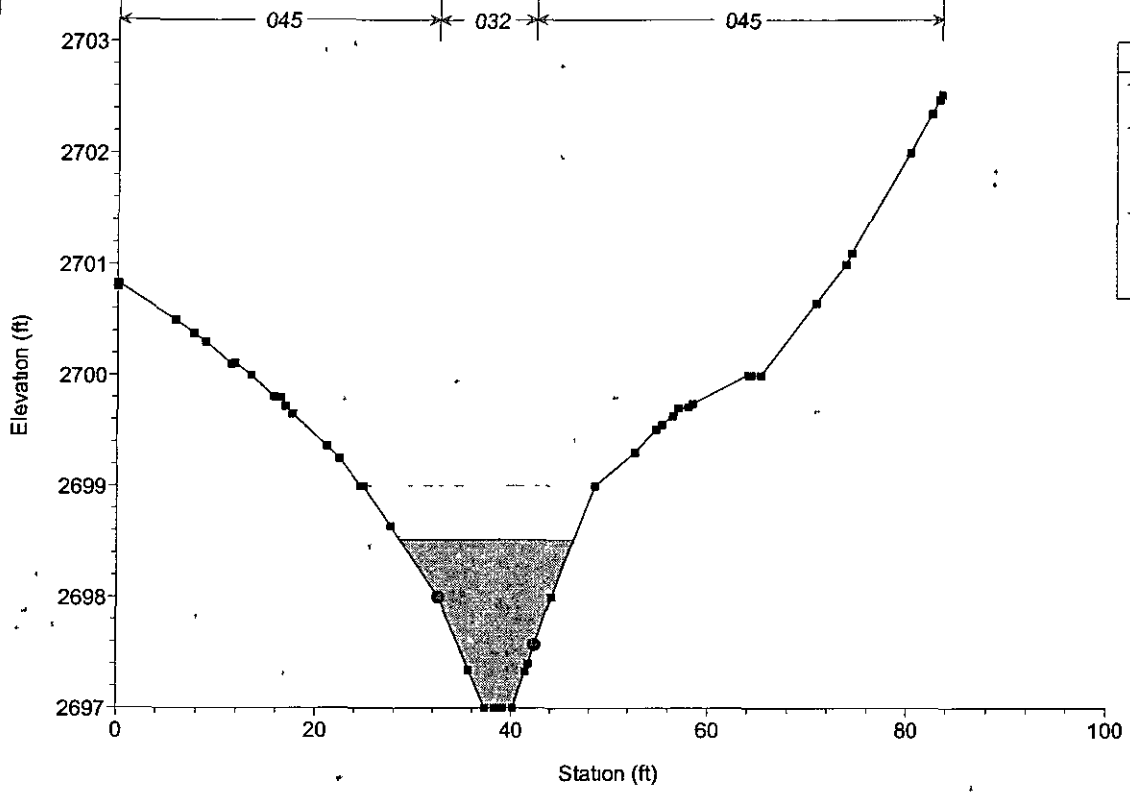
Legend	
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- - -	WS PF 1
- - -	Cnt PF 1
■	Ground
●	Bank Sta

washe2 Plan Imported Plan 02 5/8/2006  
RS = 5

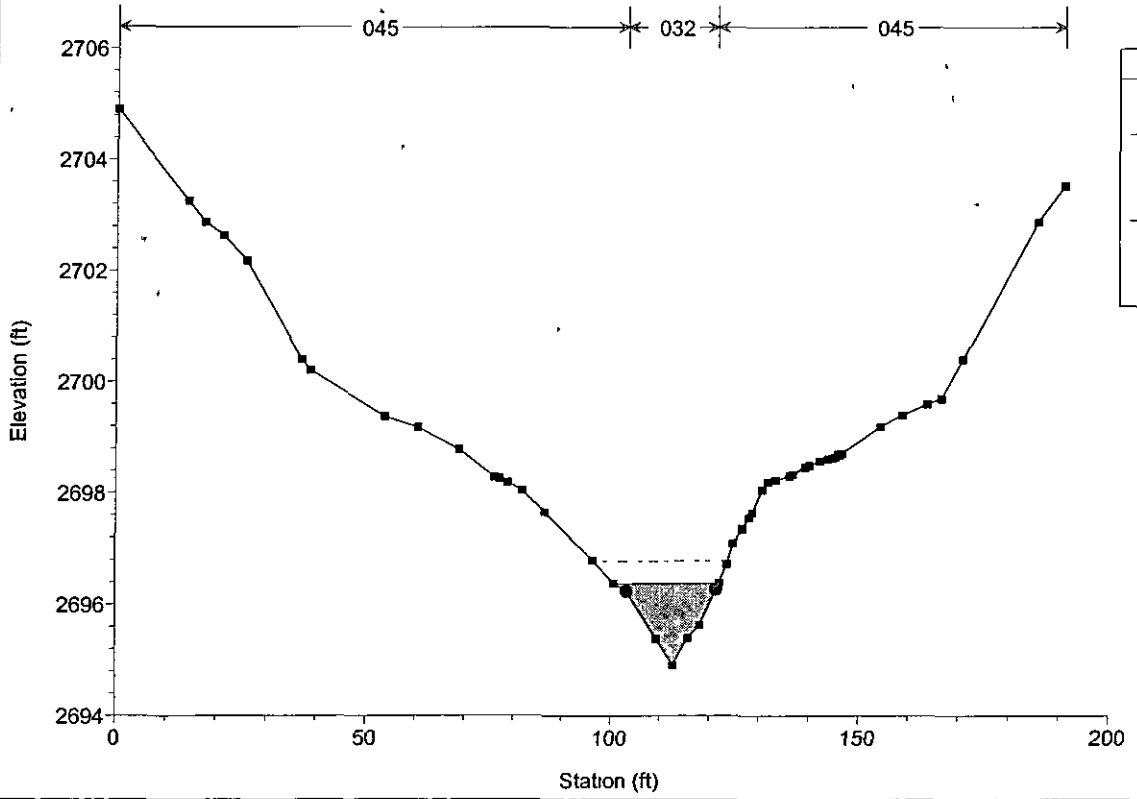


Legend	
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- - -	WS PF 1
- - -	Cnt PF 1
■	Ground
●	Bank Sta

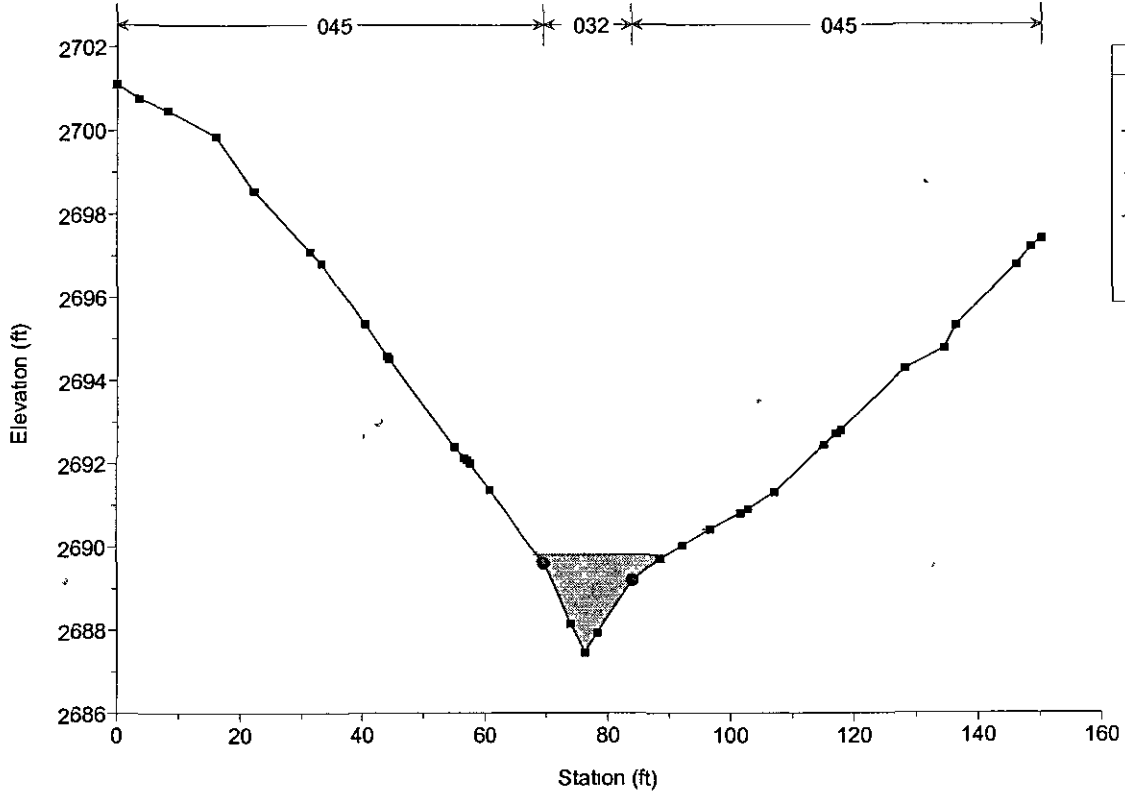
washe2 Plan Imported Plan 02 5/8/2006  
RS = 4.5



washe2 Plan Imported Plan 02 5/8/2006  
RS = 4

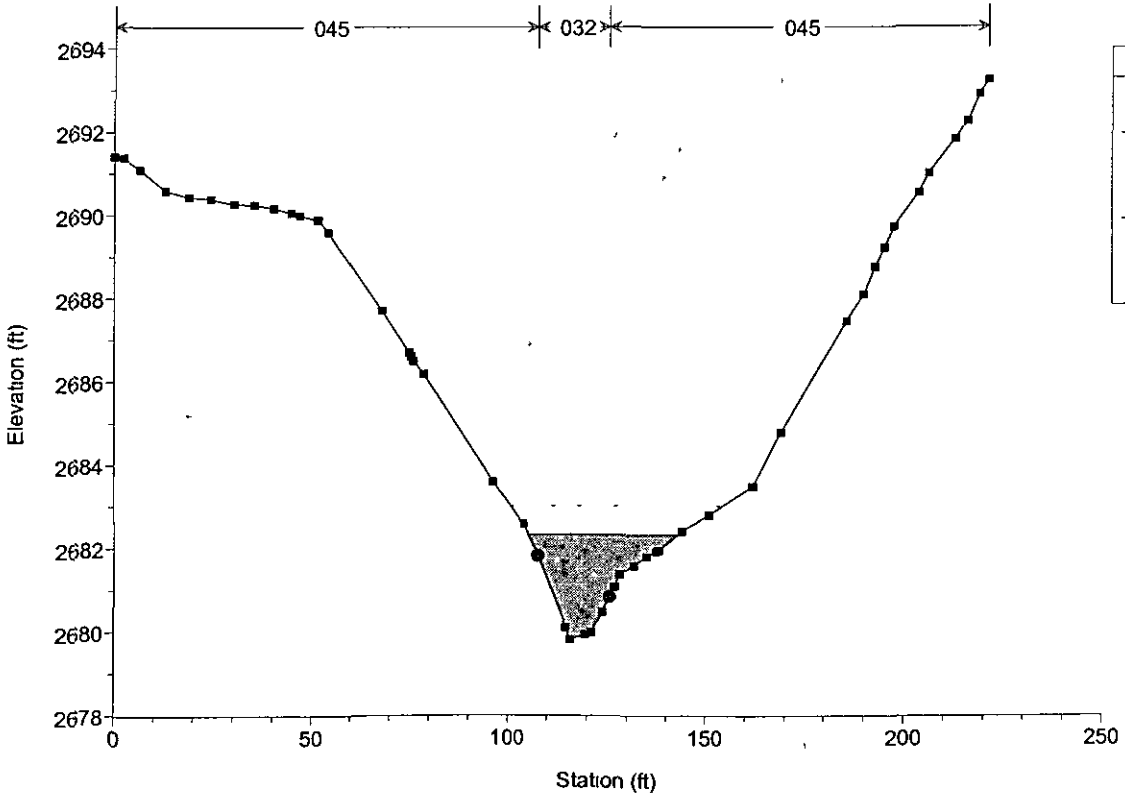


washe2 Plan Imported Plan 02 5/8/2006  
RS = 3



Legend	
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—	WS PF 1
—	Crit PF 1
■	Ground
●	Bank Sta

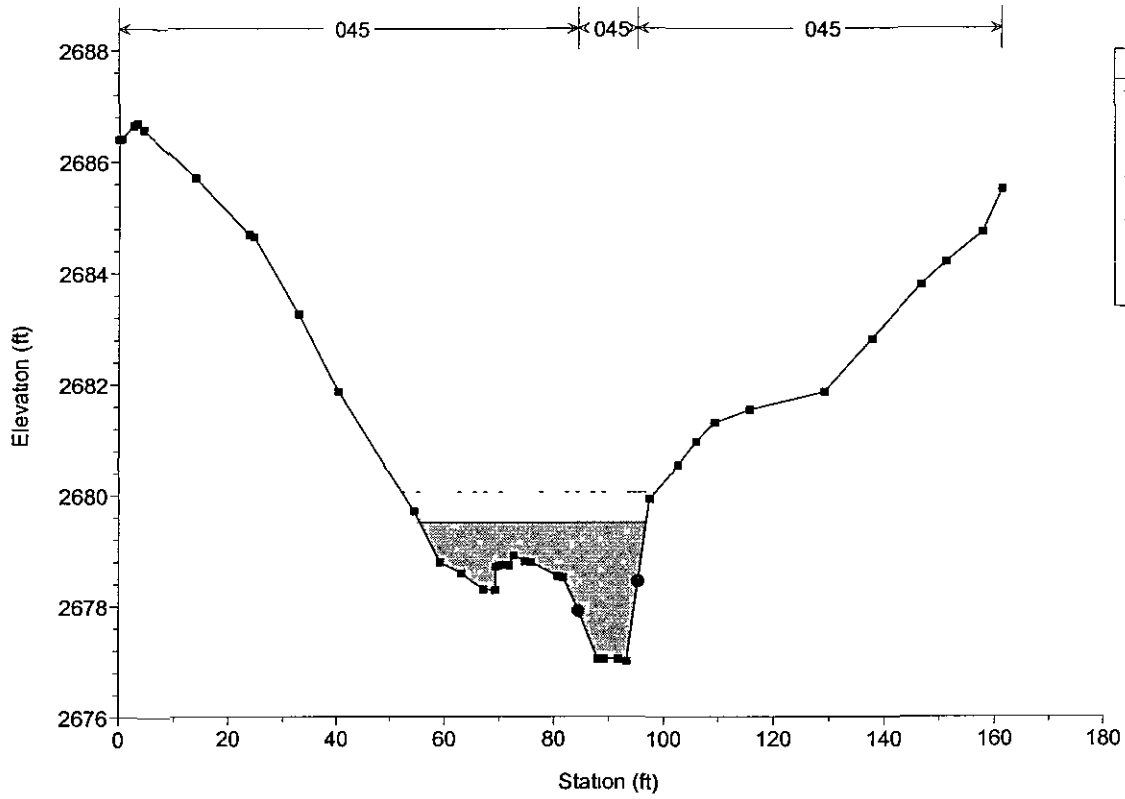
washe2 Plan Imported Plan 02 5/8/2006  
RS = 2



Legend	
—	EG PF 1
—	WS PF 1
—	Crit PF 1
■	Ground
●	Bank Sta

washe2 Plan Imported Plan 02 5/8/2006

RS = 1



Legend	
---	EG PF 1
- - - -	WS PF 1
- - - -	Crit PF 1
■	Ground
●	Bank Sta

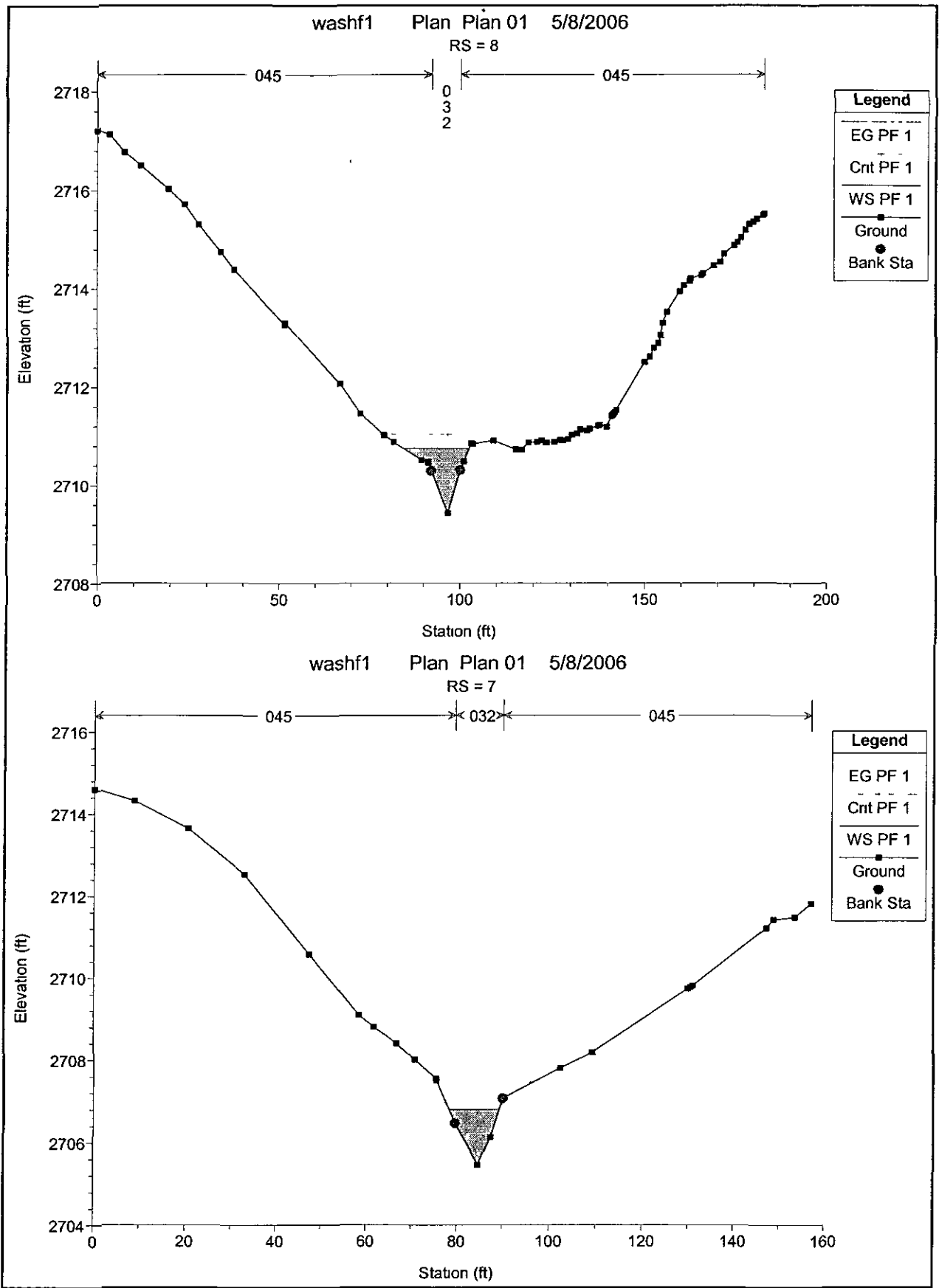
HEC-RAS Plan Imported Plan River RIVER-1 Reach Reach-1 Profile PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch Elev (ft)	W S Elev (ft)	Crit W S (ft)	E.G. Elev (ft)	E G Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Ch
Reach-1	11	PF 1	50.00	2724.80	2726.21	2726.21	2726.55	0.018084	4.69	10.67	16.13	1.02
Reach-1	10	PF 1	50.00	2719.82	2721.45	2721.45	2721.87	0.015739	5.17	9.85	14.80	0.98
Reach-1	9	PF 1	50.00	2715.71	2717.28	2717.28	2717.71	0.013081	5.64	10.87	14.46	0.94
Reach 1	8	PF 1	75.00	2709.10	2711.09	2711.09	2711.55	0.010257	5.57	16.18	22.55	0.85
Reach 1	7.5	PF 1	75.00	2708.00	2709.19	2709.19	2709.62	0.013563	5.49	15.49	19.83	0.96
Reach-1	7	PF 1	75.00	2705.46	2706.94	2706.94	2707.36	0.015127	5.18	14.90	20.27	0.98
Reach 1	6	PF 1	75.00	2701.53	2703.58	2703.58	2704.11	0.014543	5.84	13.21	14.90	0.97
Reach-1	5	PF 1	75.00	2697.79	2699.88	2699.88	2700.43	0.010621	6.48	15.38	16.38	0.89
Reach 1	4.5	PF 1	75.00	2697.00	2698.52	2698.52	2699.00	0.012220	5.77	14.89	17.87	0.92
Reach-1	4	PF 1	75.00	2694.91	2696.38	2696.38	2696.78	0.016000	5.09	14.92	21.12	0.99
Reach-1	3	PF 1	129.00	2687.45	2689.80	2689.80	2690.42	0.012827	6.38	21.52	20.91	0.95
Reach-1	2	PF 1	258.00	2679.83	2682.33	2682.33	2683.02	0.009910	6.95	44.53	37.38	0.89
Reach-1	1	PF 1	258.00	2677.00	2679.50	2679.50	2680.04	0.016298	6.88	49.30	41.36	0.82

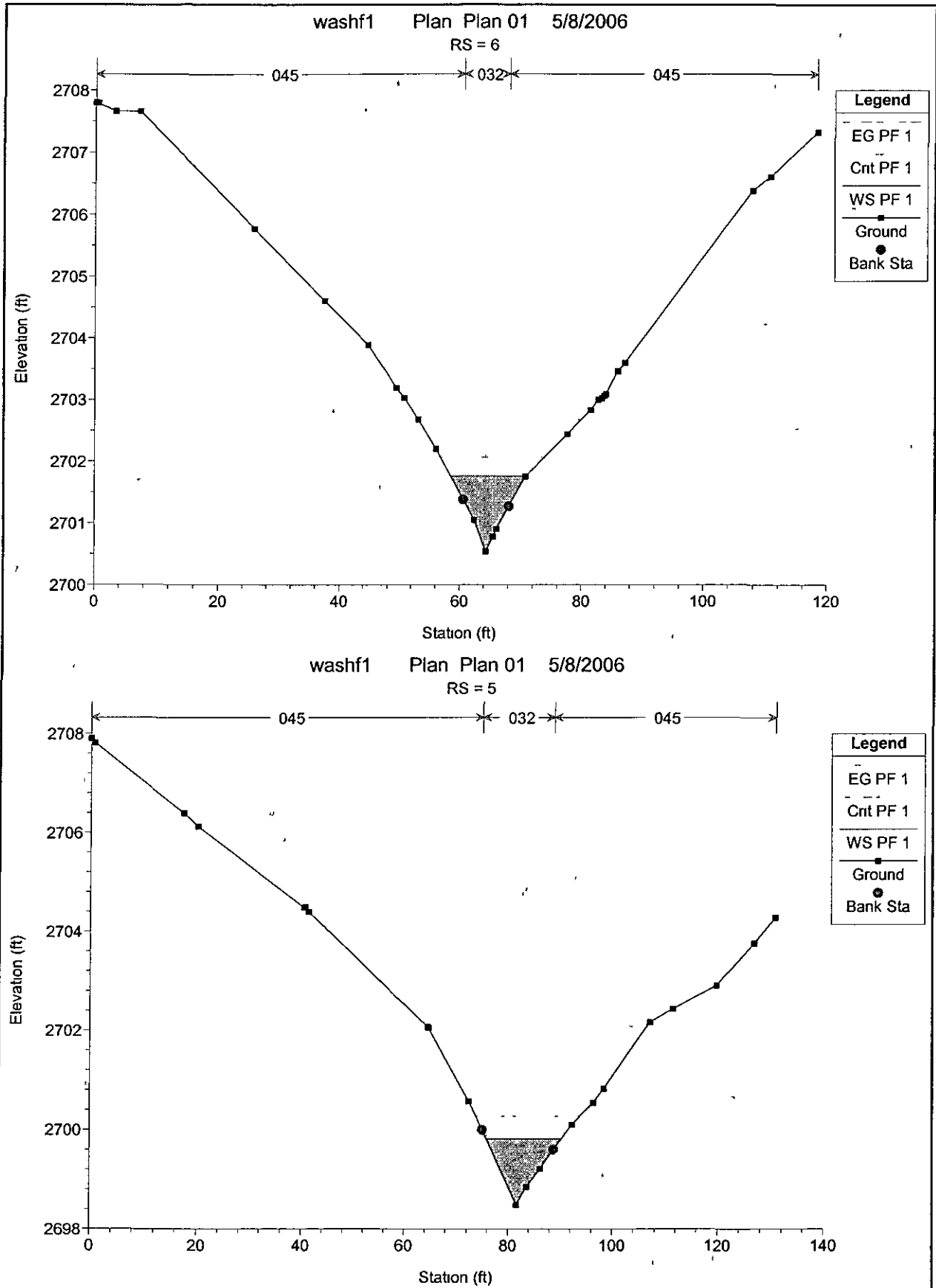
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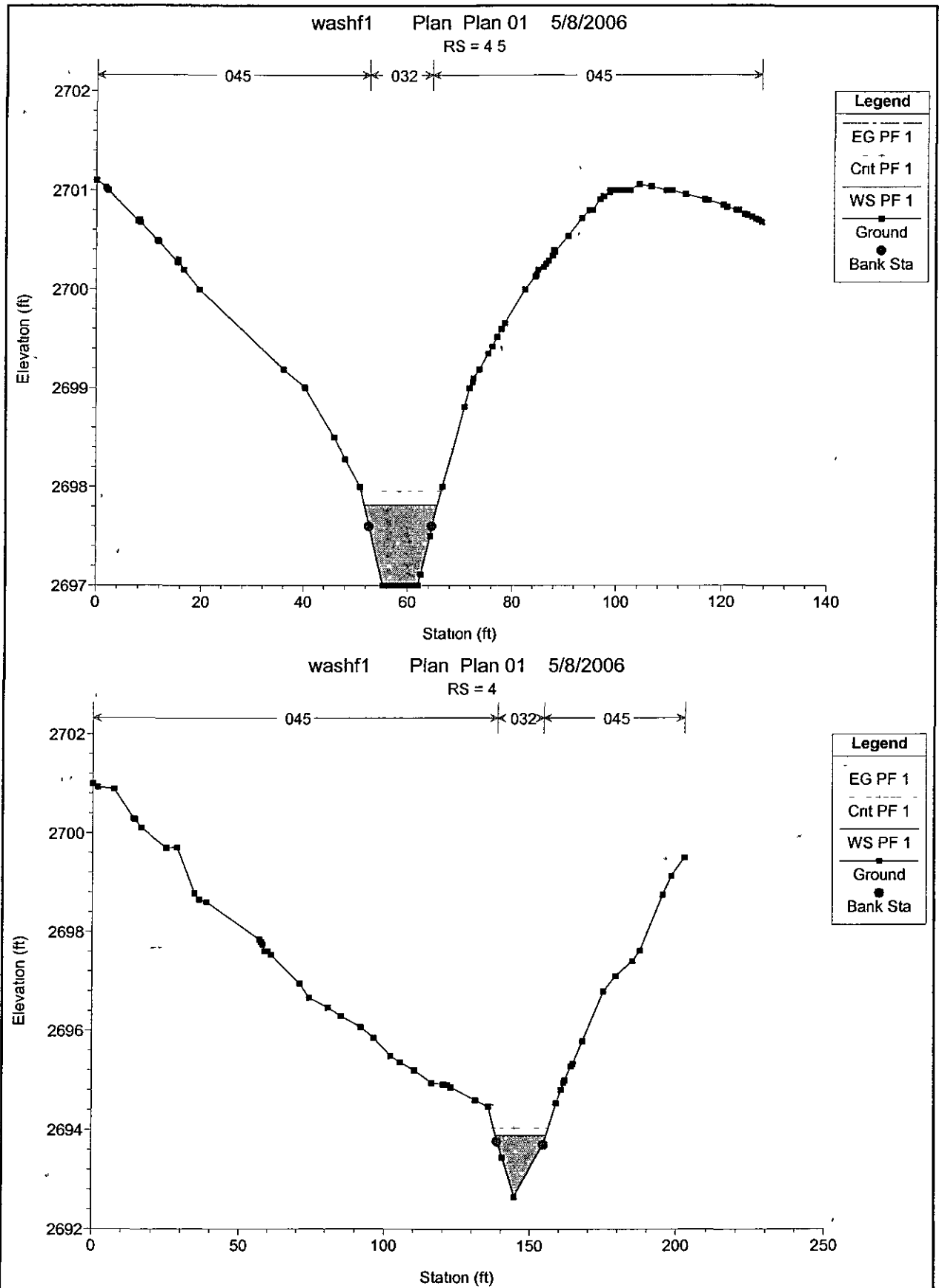
HEC-RAS Plan Plan 01 River RIVER-1 Reach Reach-1 Profile PF 1

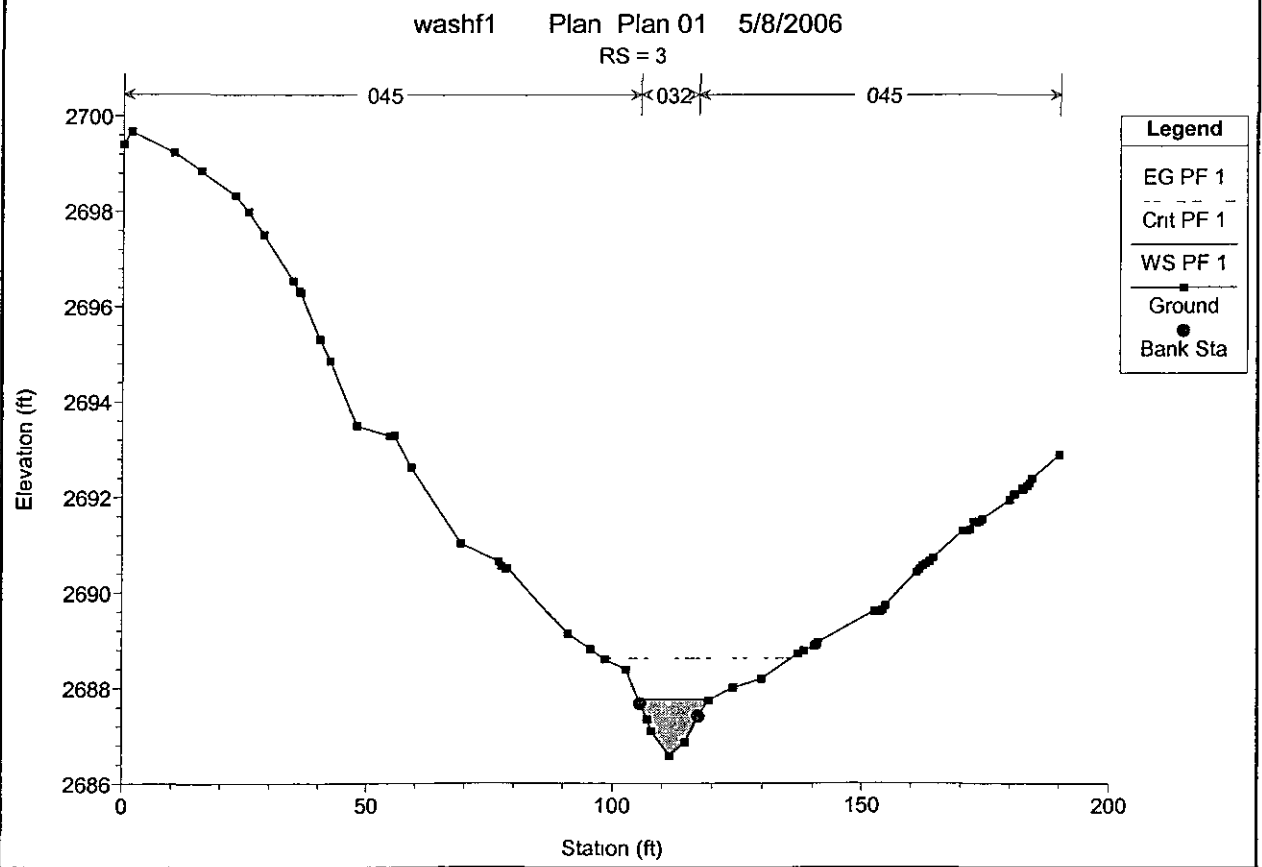
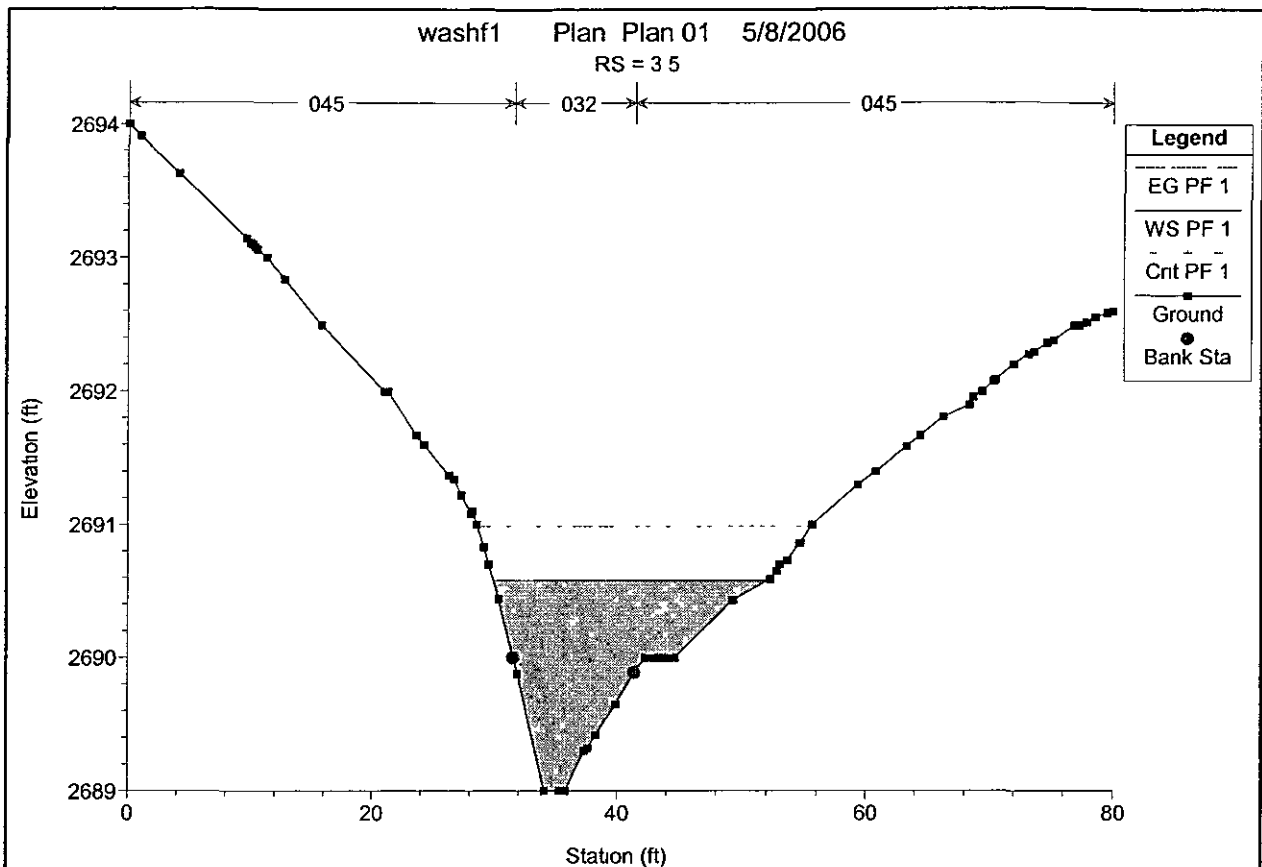
Reach	River Sta	Profile	Q Total (cfs)	Min Ch Elev (ft)	W S Elev (ft)	Ch W S (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Ch
Reach-1	8	PF 1	50.00	2709.42	2710.74	2711.03	2711.38	0.025021	6.65	8.95	20.46	1.25
Reach-1	7	PF 1	50.00	2705.46	2706.81	2706.97	2707.46	0.027912	6.49	7.87	11.03	1.29
Reach-1	6	PF 1	50.00	2700.54	2701.76	2702.06	2702.68	0.039439	7.86	7.10	12.34	1.54
Reach-1	5	PF 1	50.00	2698.48	2699.81	2699.89	2700.28	0.022728	5.49	9.24	14.33	1.15
Reach-1	4.5	PF 1	50.00	2697.00	2697.81	2697.86	2698.38	0.028656	6.06	8.42	13.96	1.29
Reach-1	4	PF 1	70.00	2692.65	2693.89	2694.04	2694.51	0.030515	6.32	11.17	17.42	1.34
Reach-1	3.5	PF 1	70.00	2689.00	2690.58	2690.58	2690.99	0.011182	5.40	15.90	22.12	0.87
Reach-1	3	PF 1	70.00	2686.57	2687.76	2688.03	2688.62	0.036115	7.47	9.68	14.54	1.48
Reach-1	2	PF 1	70.00	2681.90	2683.30	2683.31	2683.60	0.016888	4.45	17.26	34.61	0.98
Reach-1	1	PF 1	93.00	2677.60	2678.78	2679.00	2679.47	0.035483	6.95	17.02	53.71	1.45



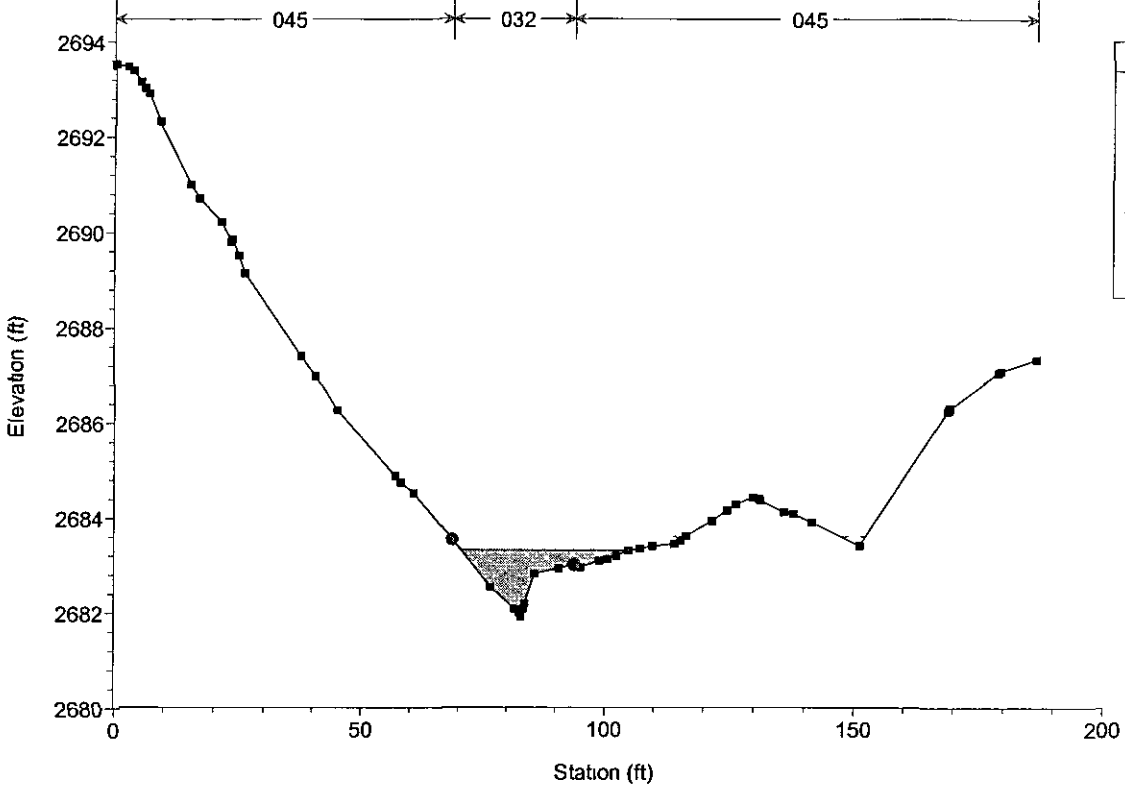






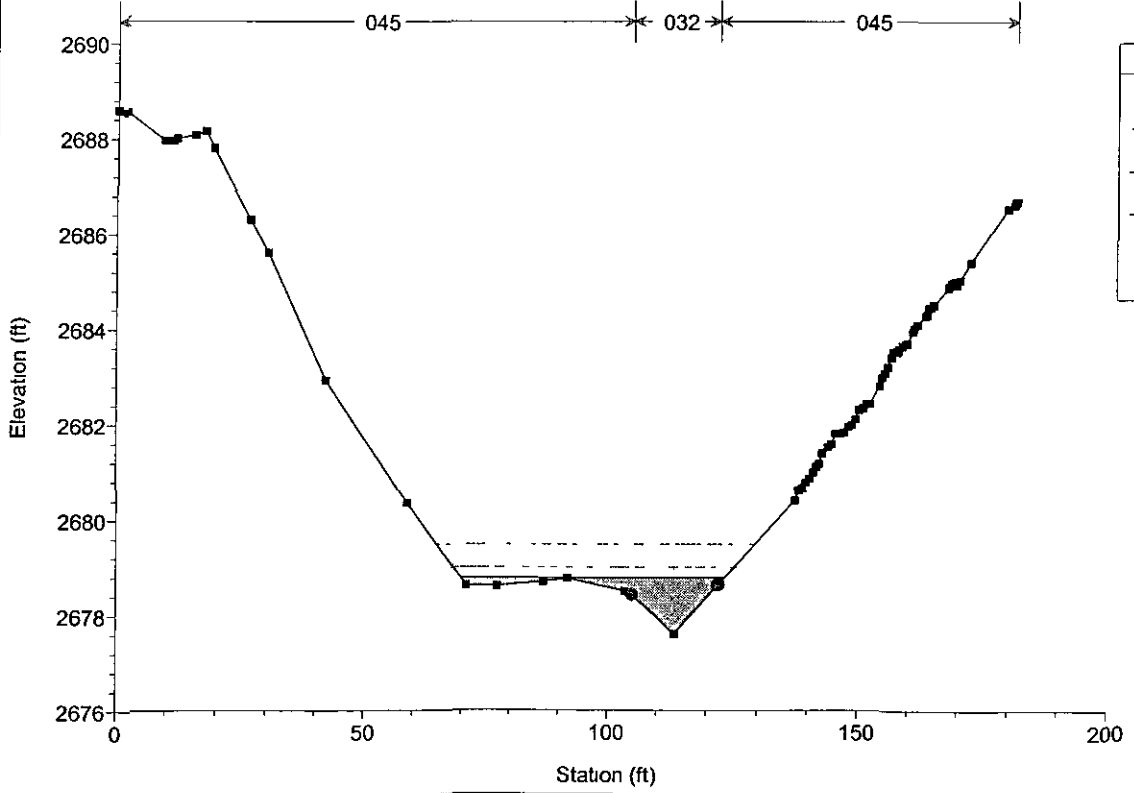


washf1 Plan Plan 01 5/8/2006  
RS = 2



Legend	
EG PF 1	—
Cnt PF 1	- - -
WS PF 1	—
Ground	■
Bank Sta	●

washf1 Plan Plan 01 5/8/2006  
RS = 1

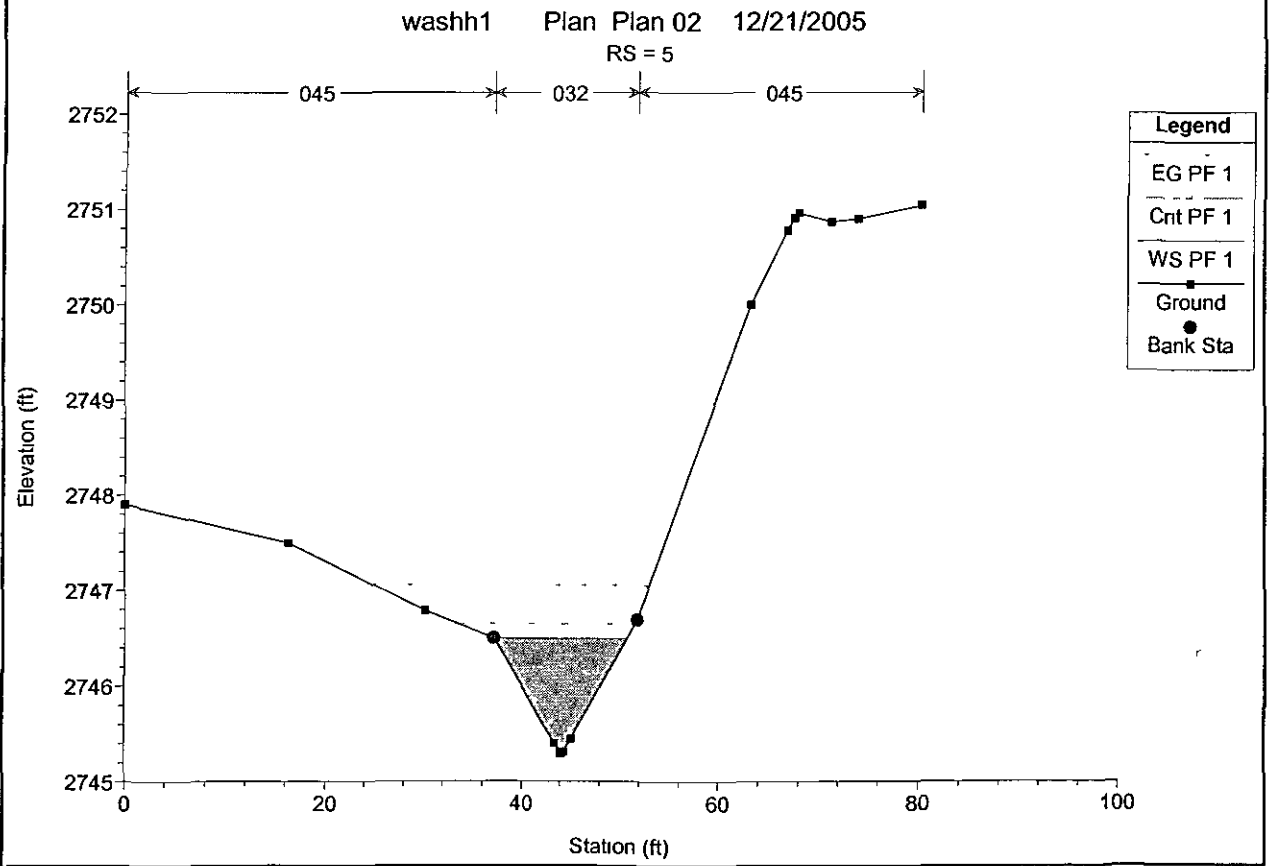
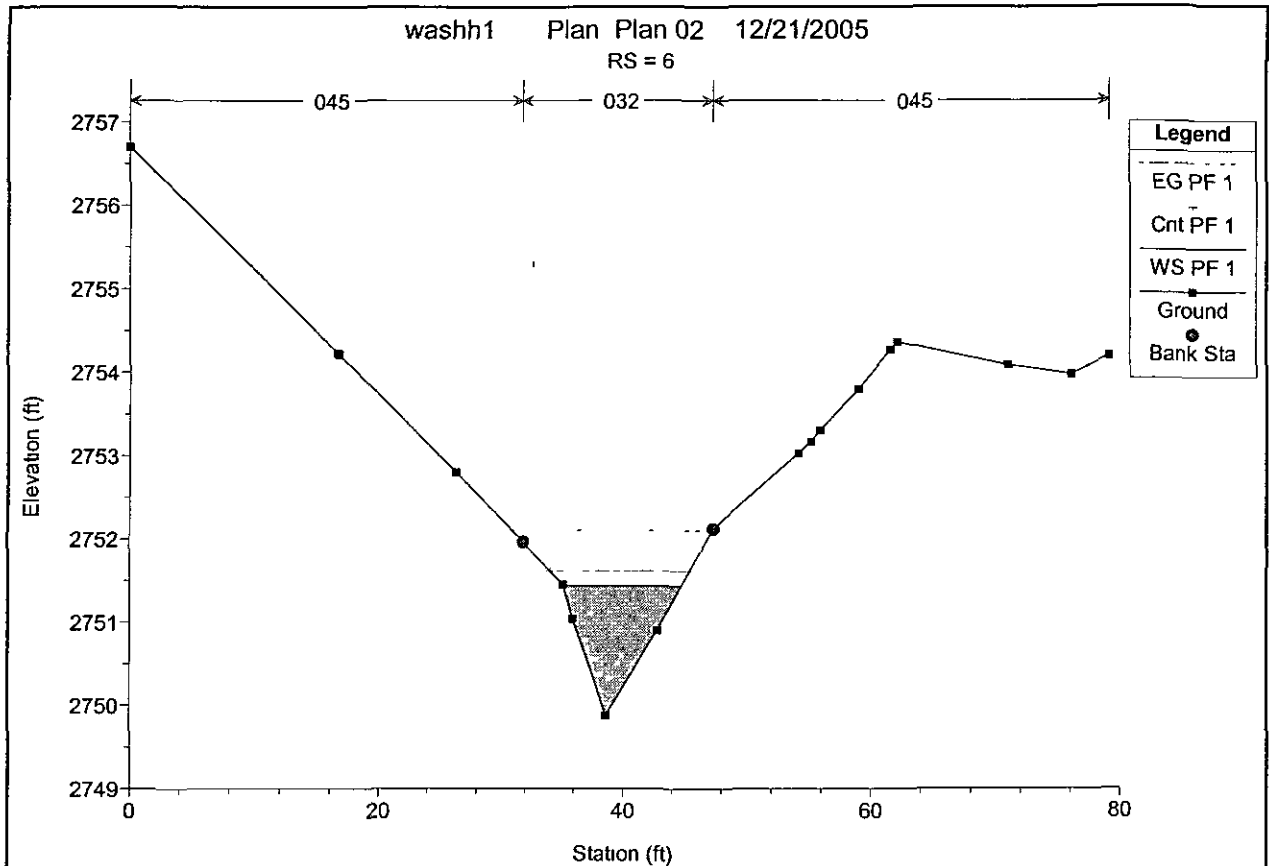


Legend	
EG PF 1	—
Cnt PF 1	- - -
WS PF 1	—
Ground	■
Bank Sta	●

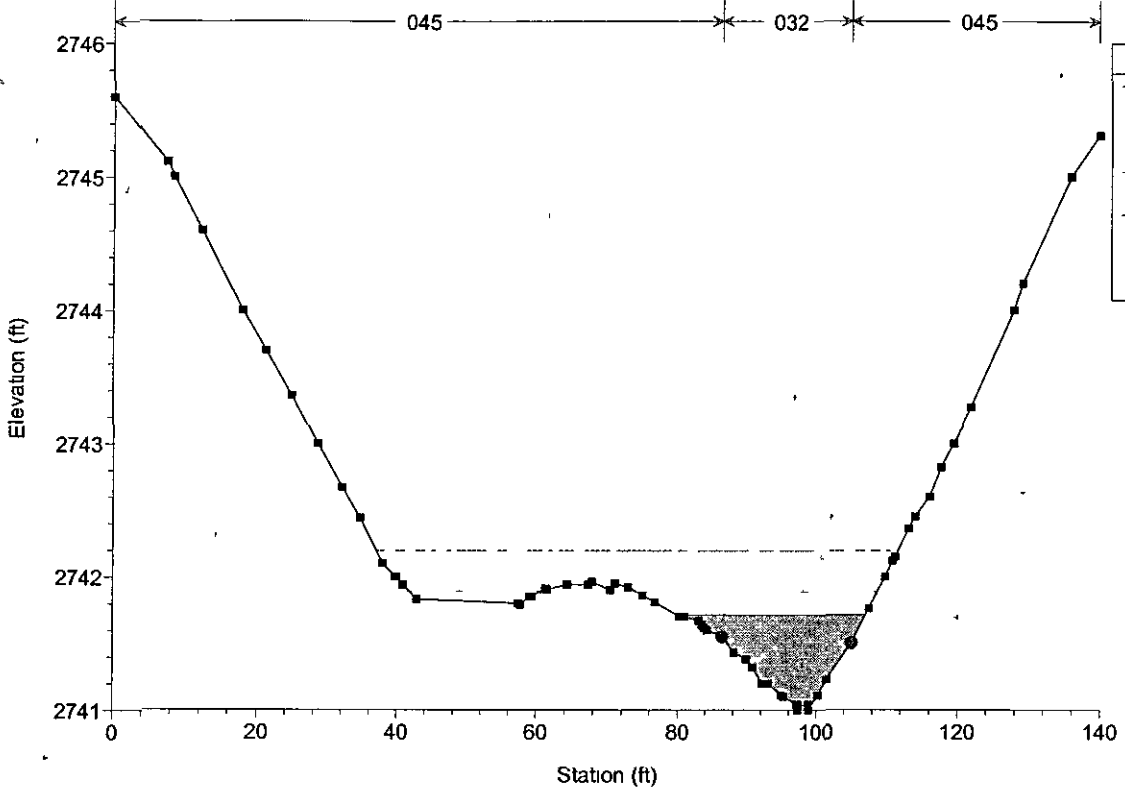
# WASH HI

HEC-RAS Plan Plan 02 River RIVER 1 Reach Reach-1 Profile PF 1

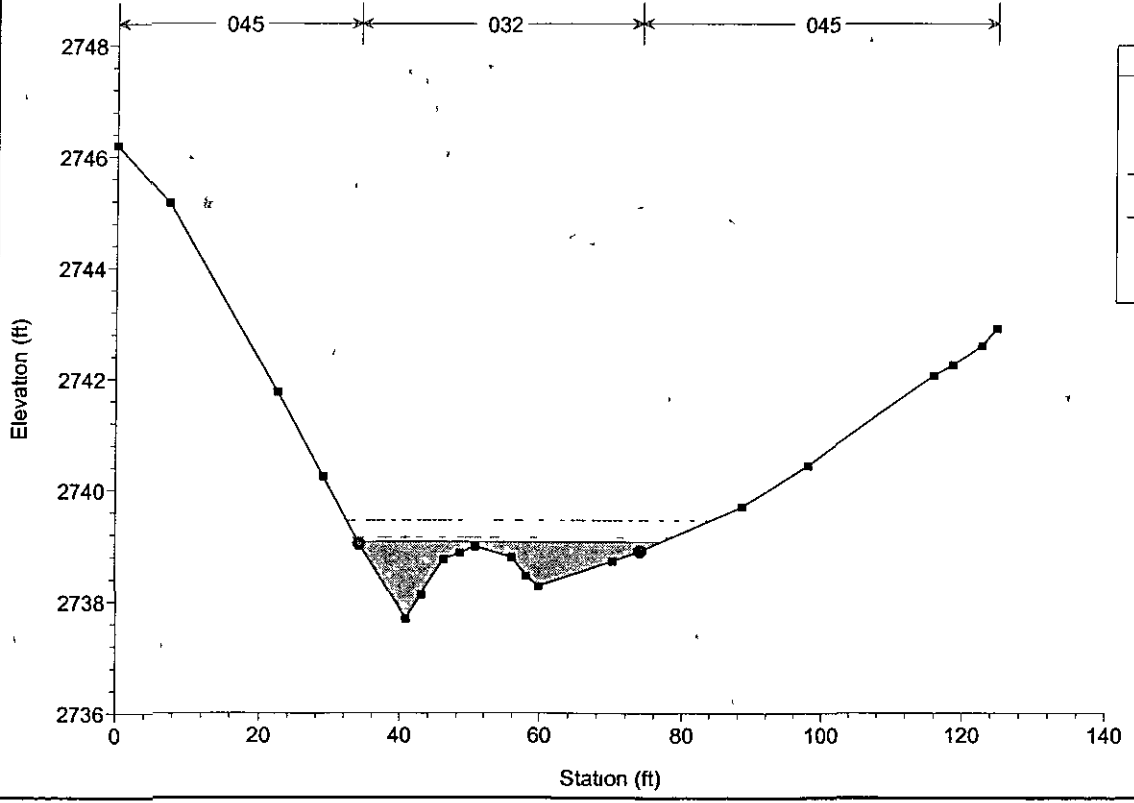
Reach	River Sta	Profile	Q Total (cfs)	Min Chl El (ft)	W S Elev (ft)	Crit W S (ft)	E G Elev (ft)	E G Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	6	PF 1	50.00	2749.88	2751.43	2751.61	2752.10	0.028975	6.55	7.63	9.59	1.29
Reach-1	5	PF 1	50.00	2745.29	2746.49	2746.65	2747.06	0.033087	6.02	8.30	13.57	1.36
Reach-1	4.5	PF 1	50.00	2741.00	2741.71	2741.89	2742.19	0.040197	5.61	9.42	27.11	1.44
Reach-1	4	PF 1	100.00	2737.70	2739.07	2739.15	2739.46	0.029073	4.98	20.28	43.02	1.24
Reach-1	3	PF 1	100.00	2732.24	2733.47	2733.71	2734.20	0.032501	6.95	16.31	45.91	1.40
Reach-1	2	PF 1	143.00	2725.11	2726.77	2726.94	2727.42	0.035977	6.47	22.20	36.12	1.43
Reach 1	1.5	PF 1	143.00	2723.00	2724.13	2724.22	2724.63	0.017888	5.81	27.34	40.51	1.07
Reach 1	1	PF 1	143.00	2720.27	2720.72	2720.96	2721.55	0.204067	7.72	19.60	64.59	2.88



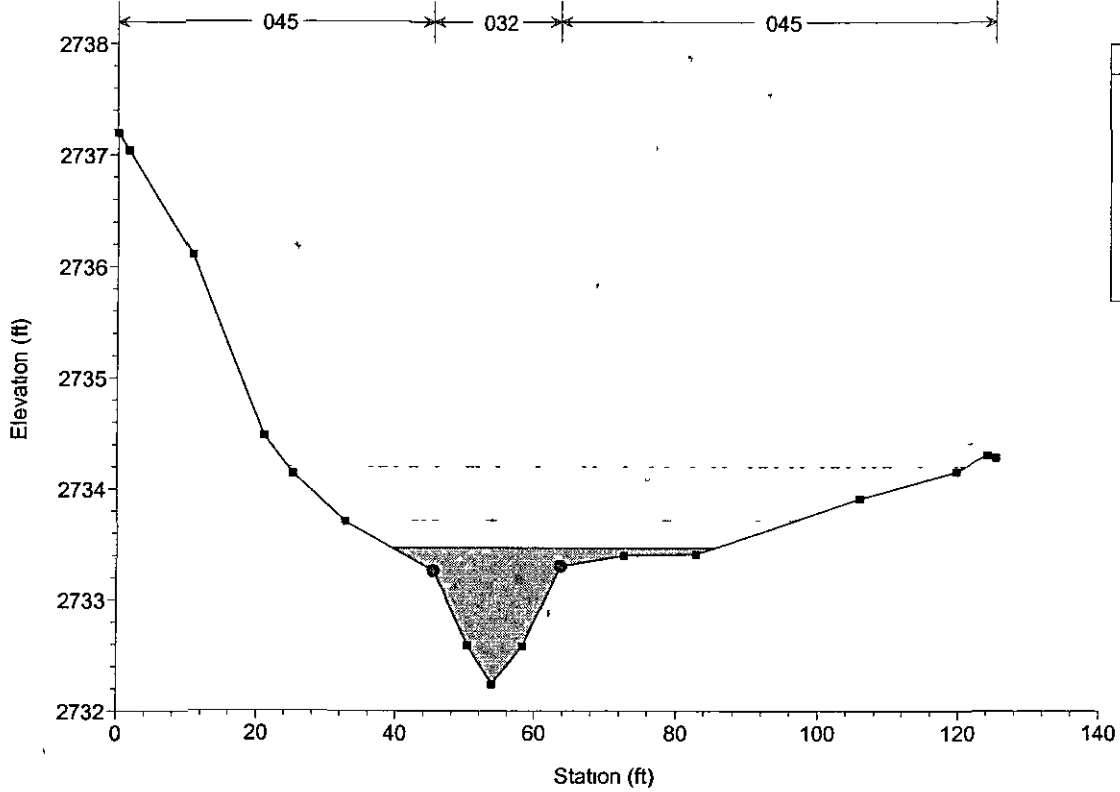
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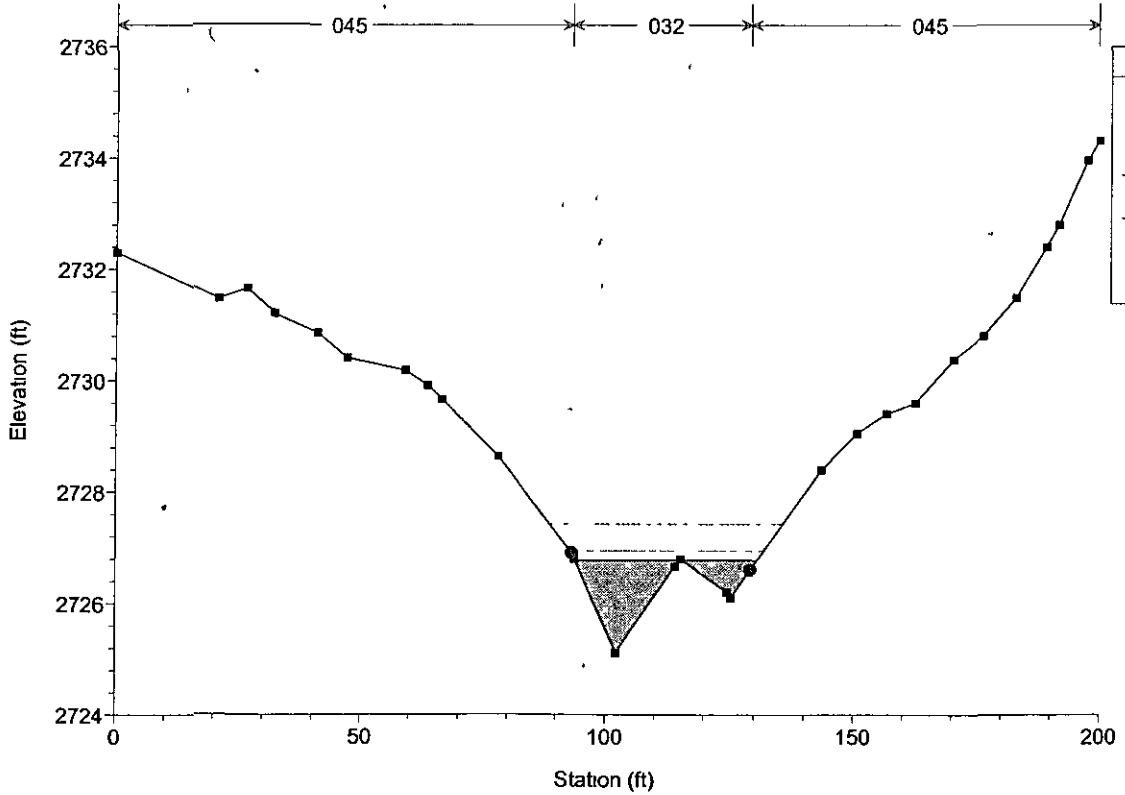
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washh1 Plan Plan 02 12/21/2005  
RS = 3

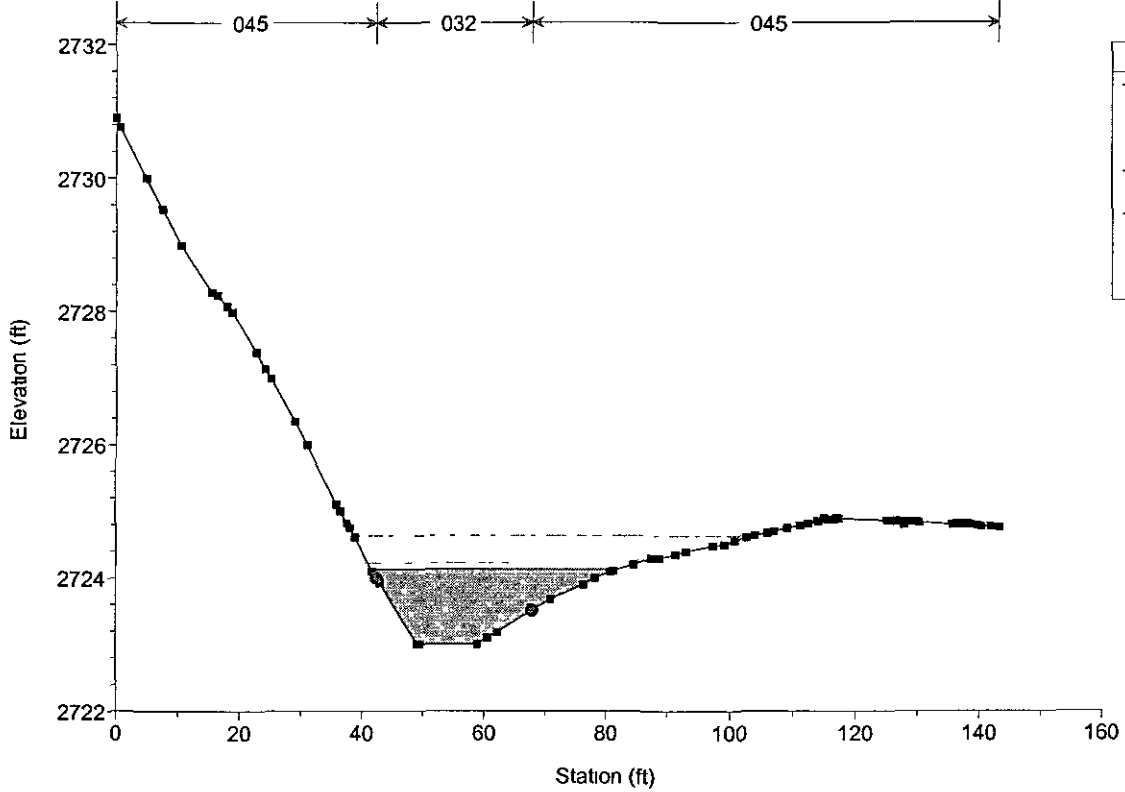


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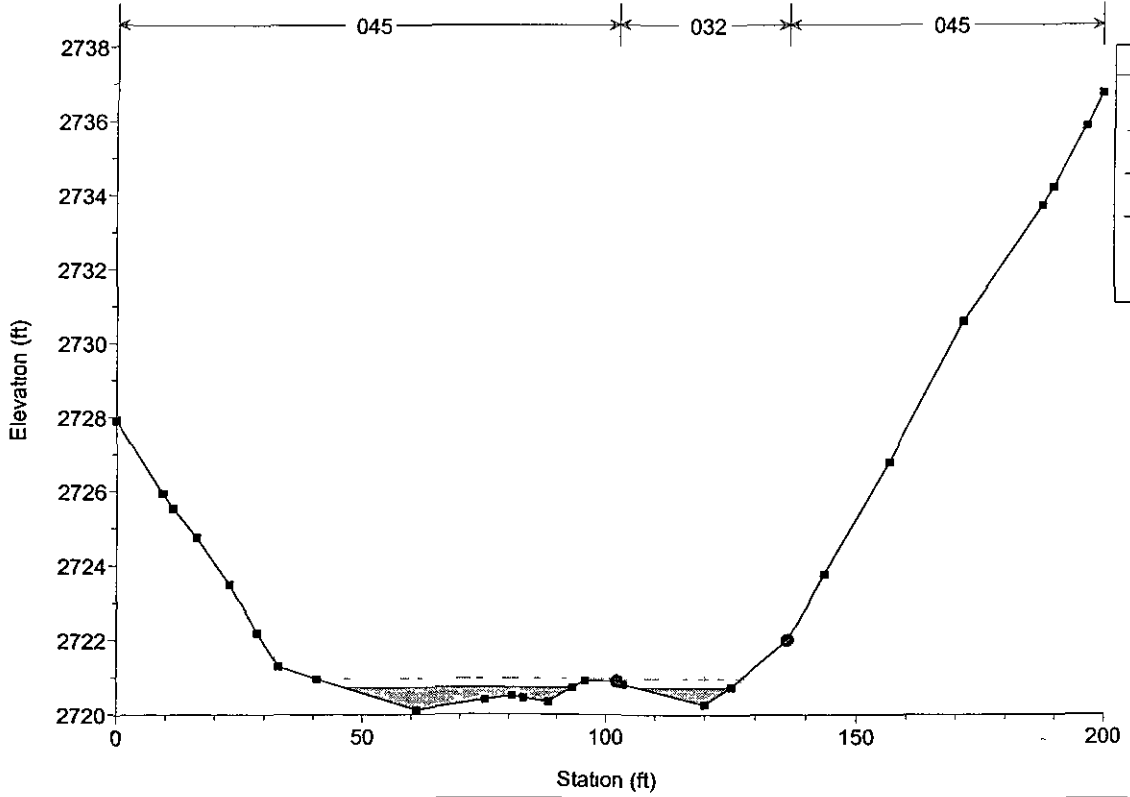




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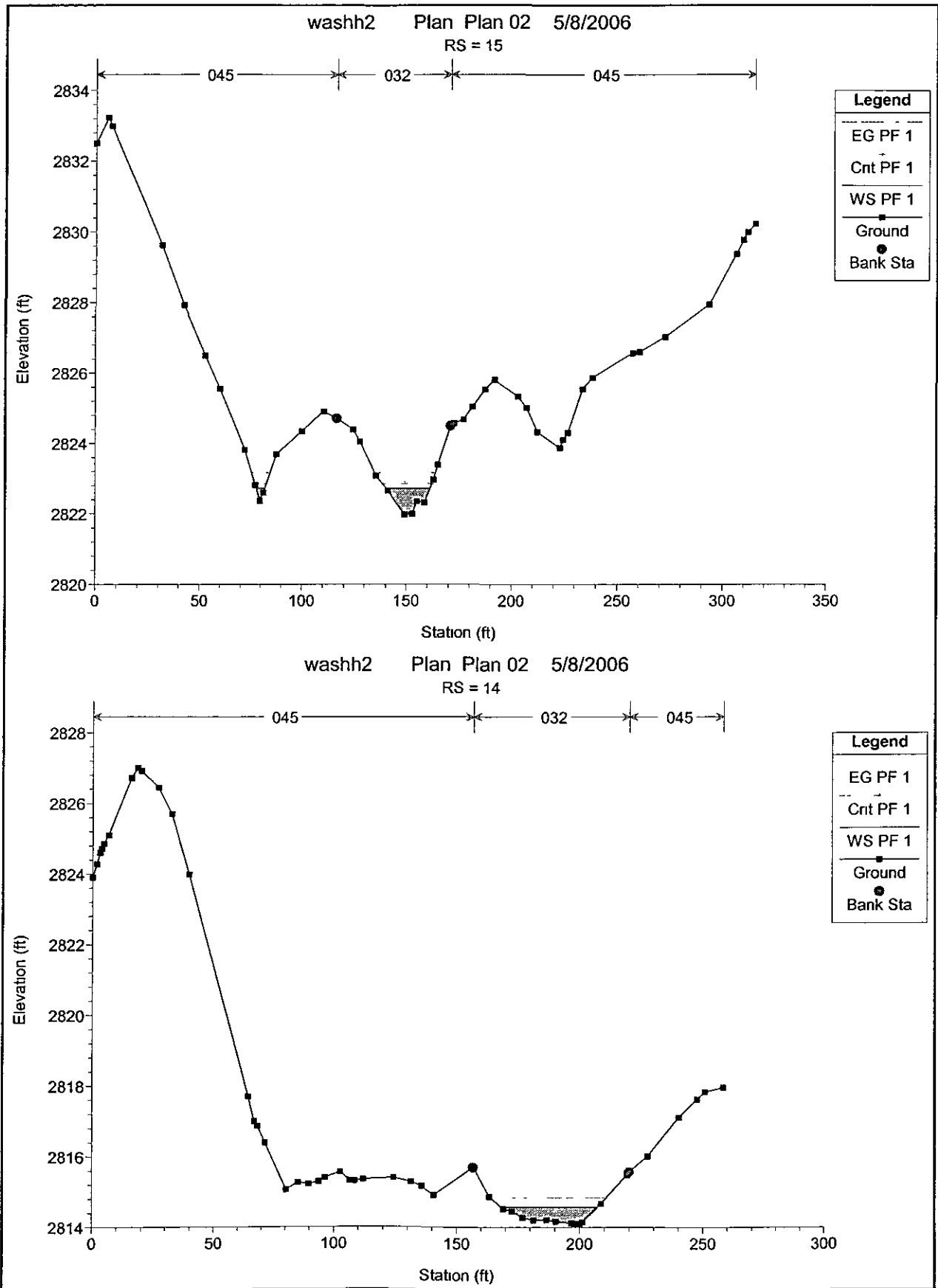
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RS = 1

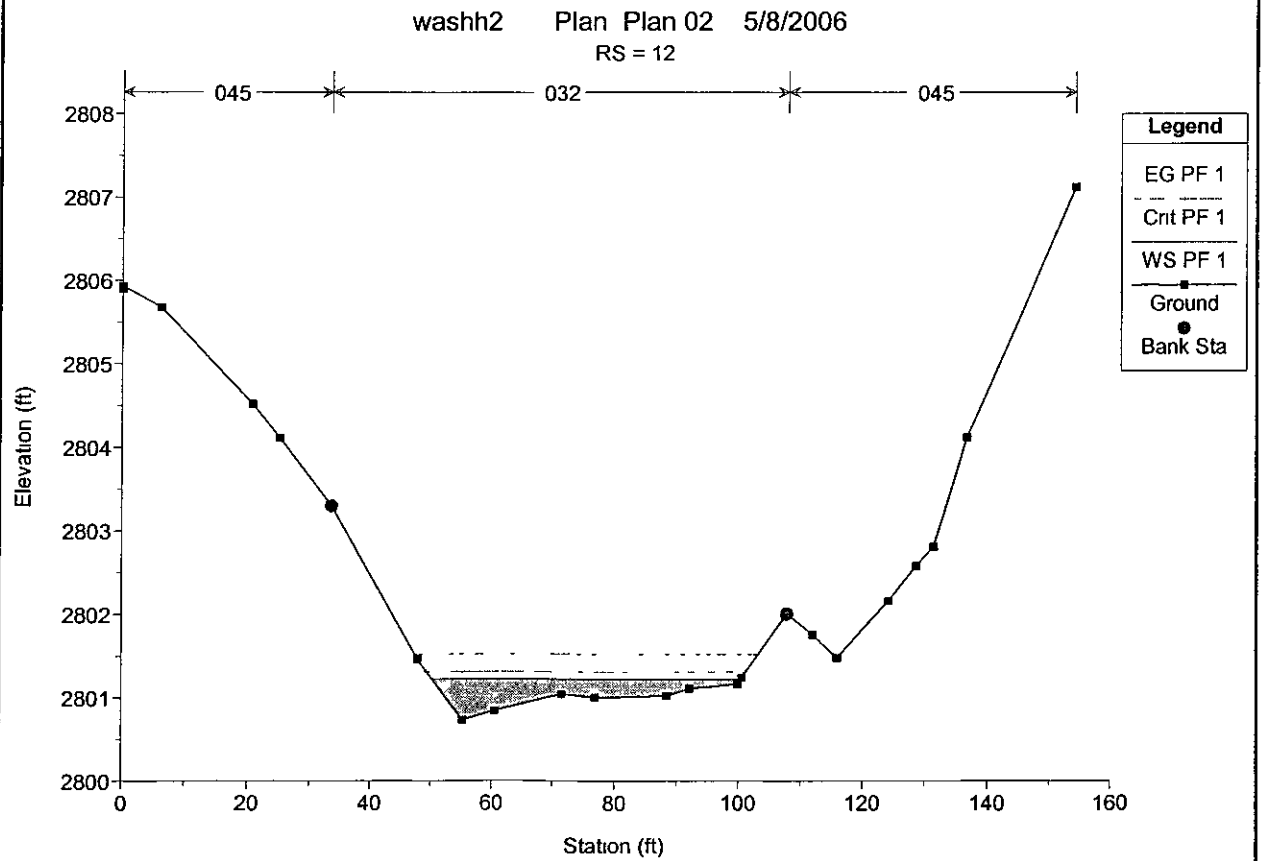
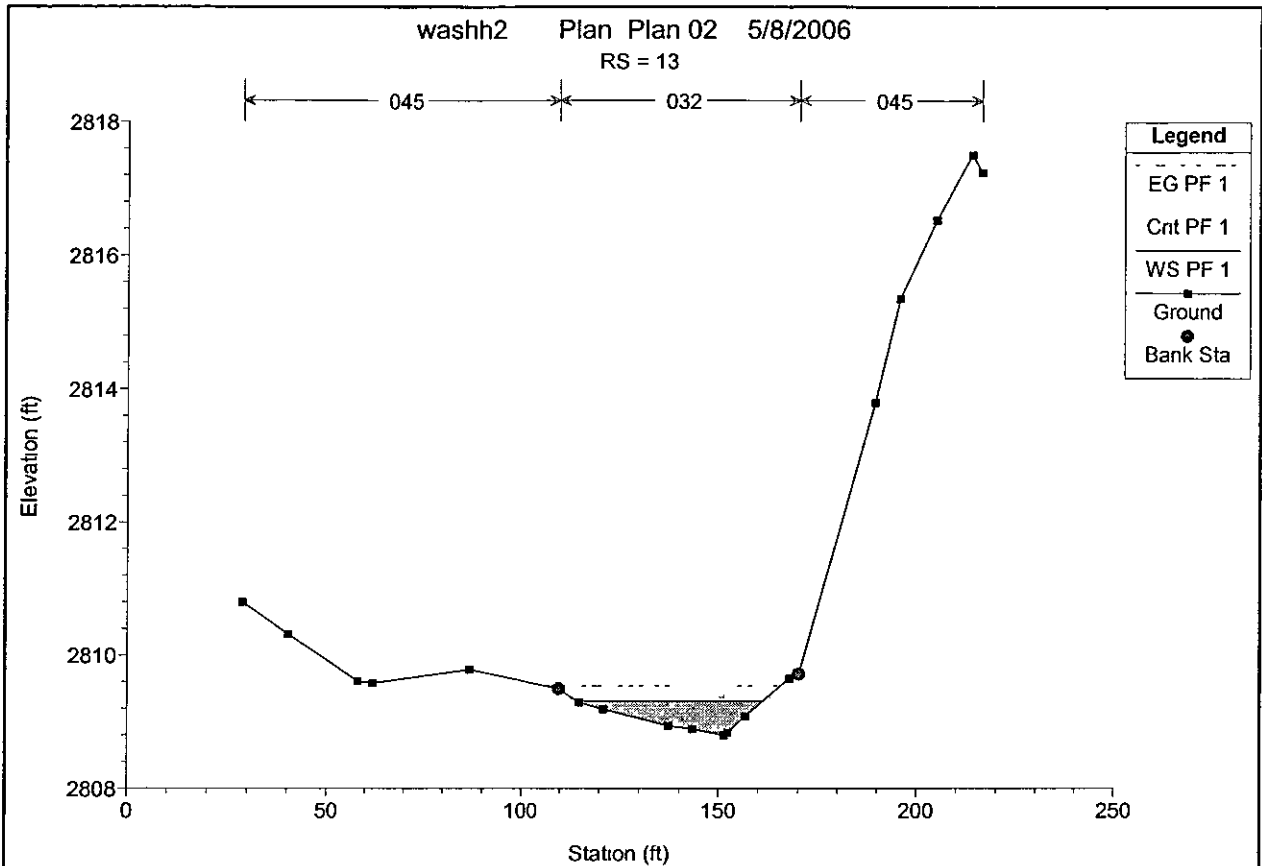


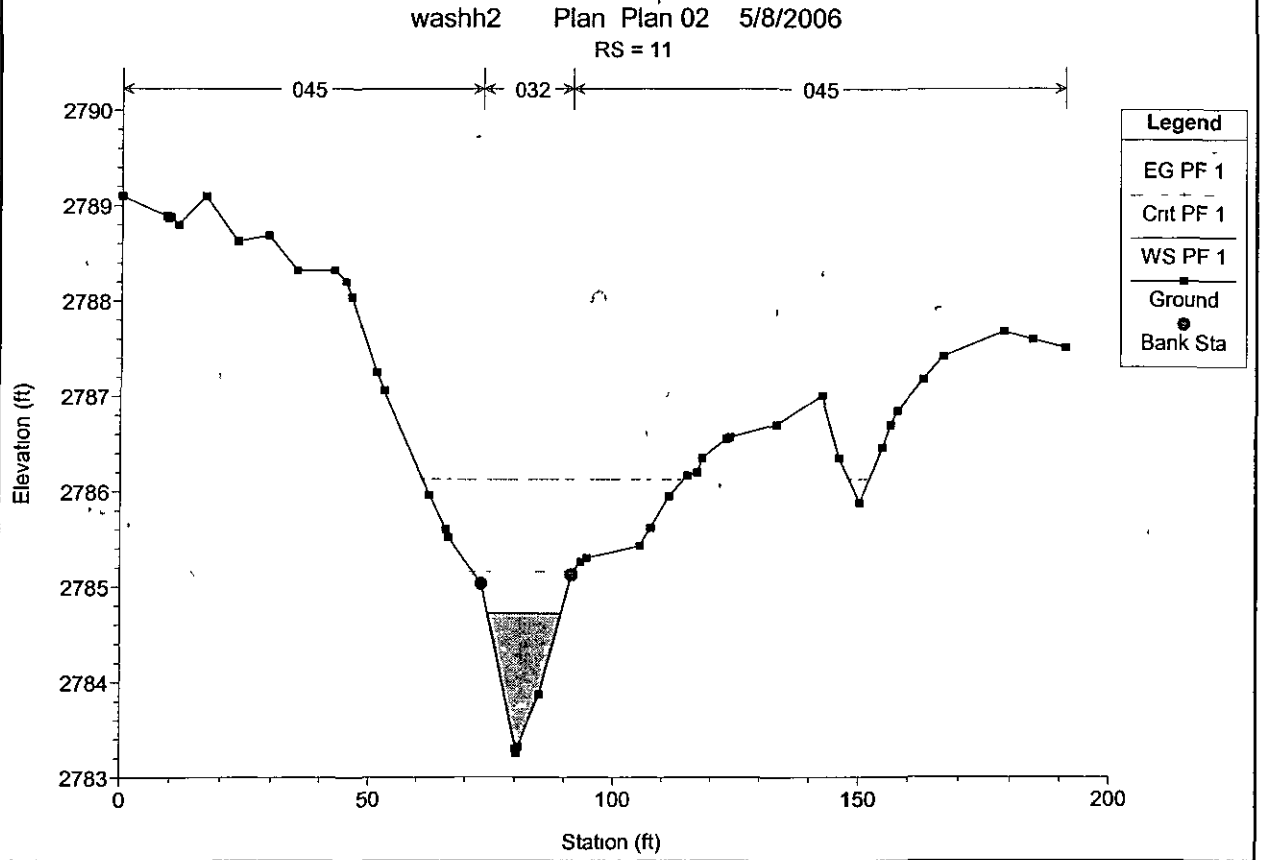
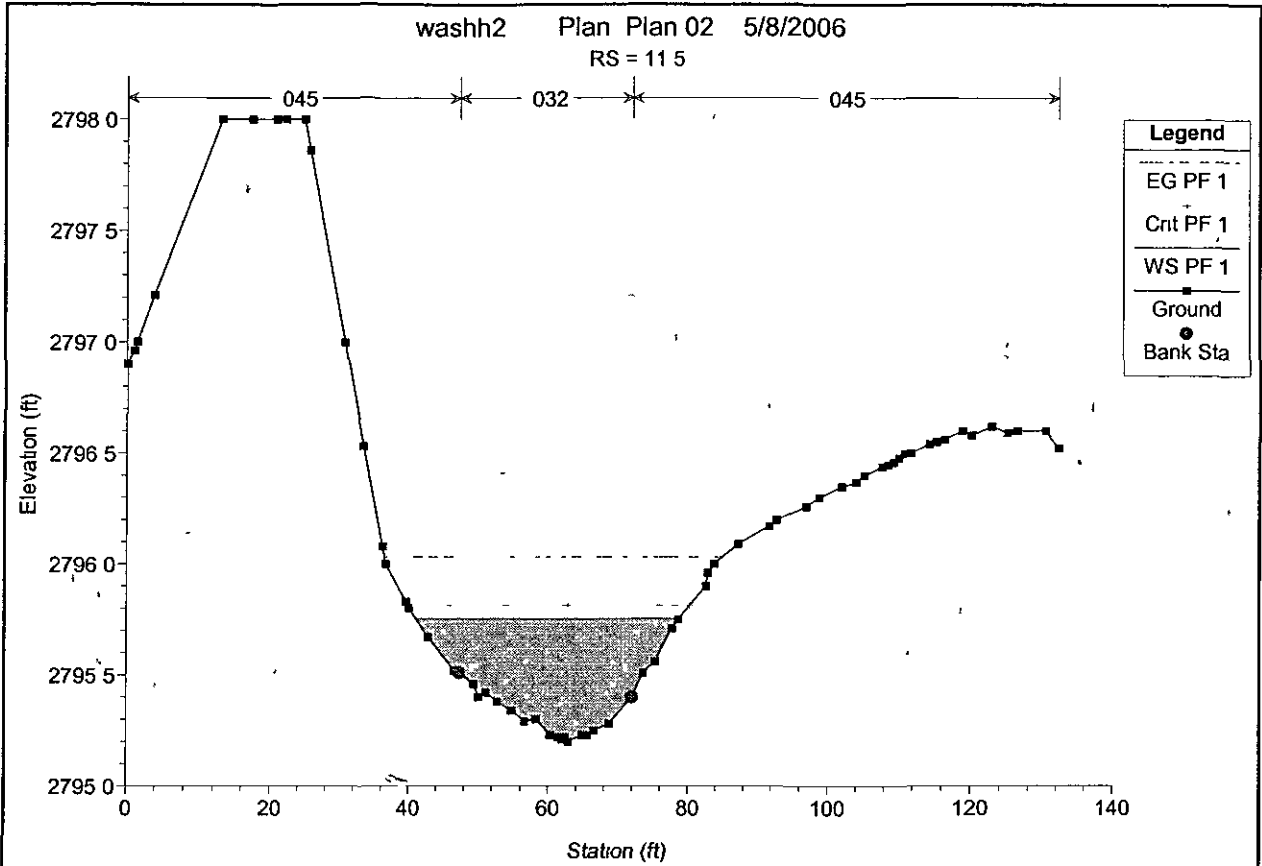
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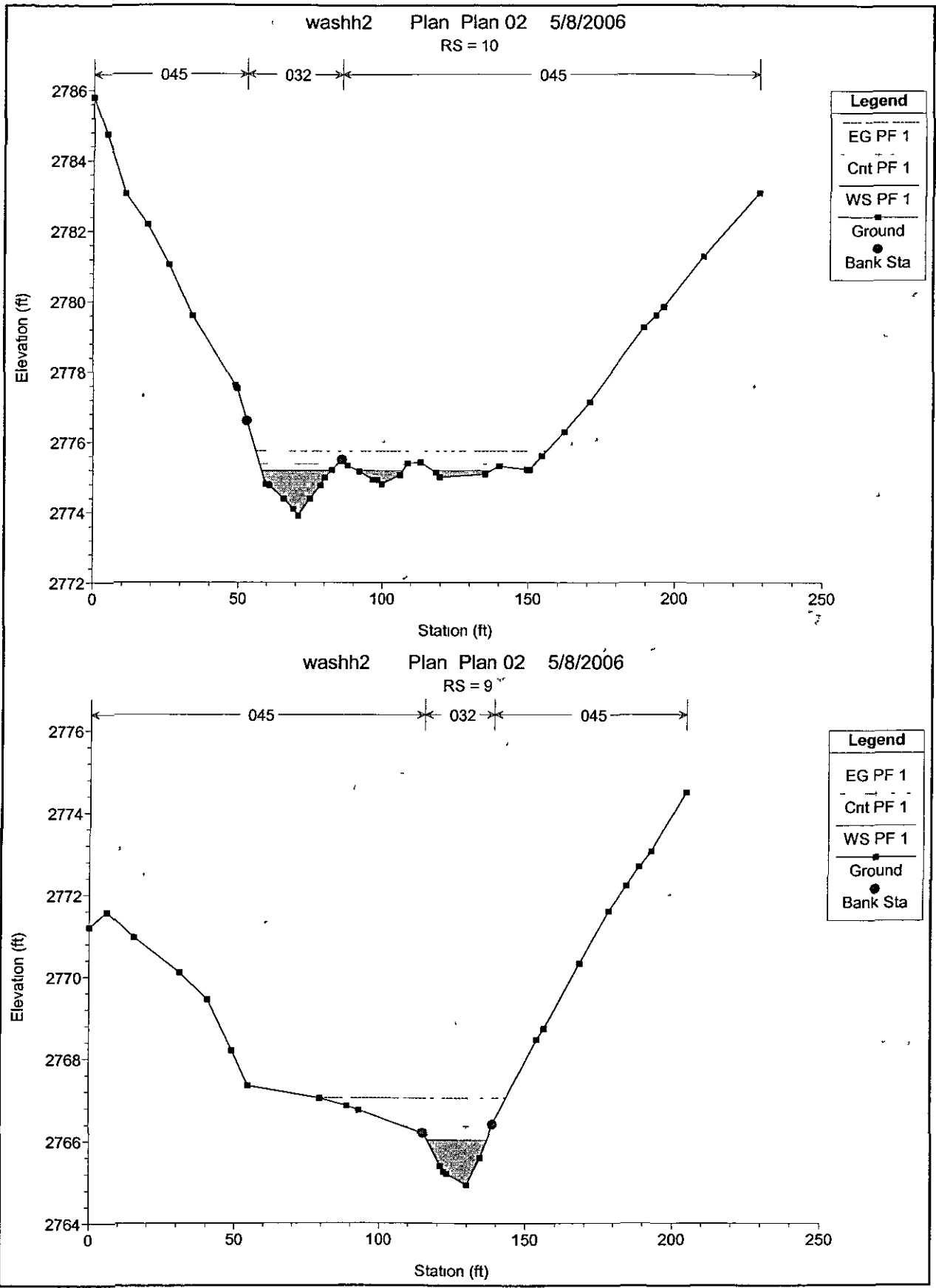
HEC-RAS Plan Plan 02 River RIVER 1 Reach Reach-1 Profile PF 1

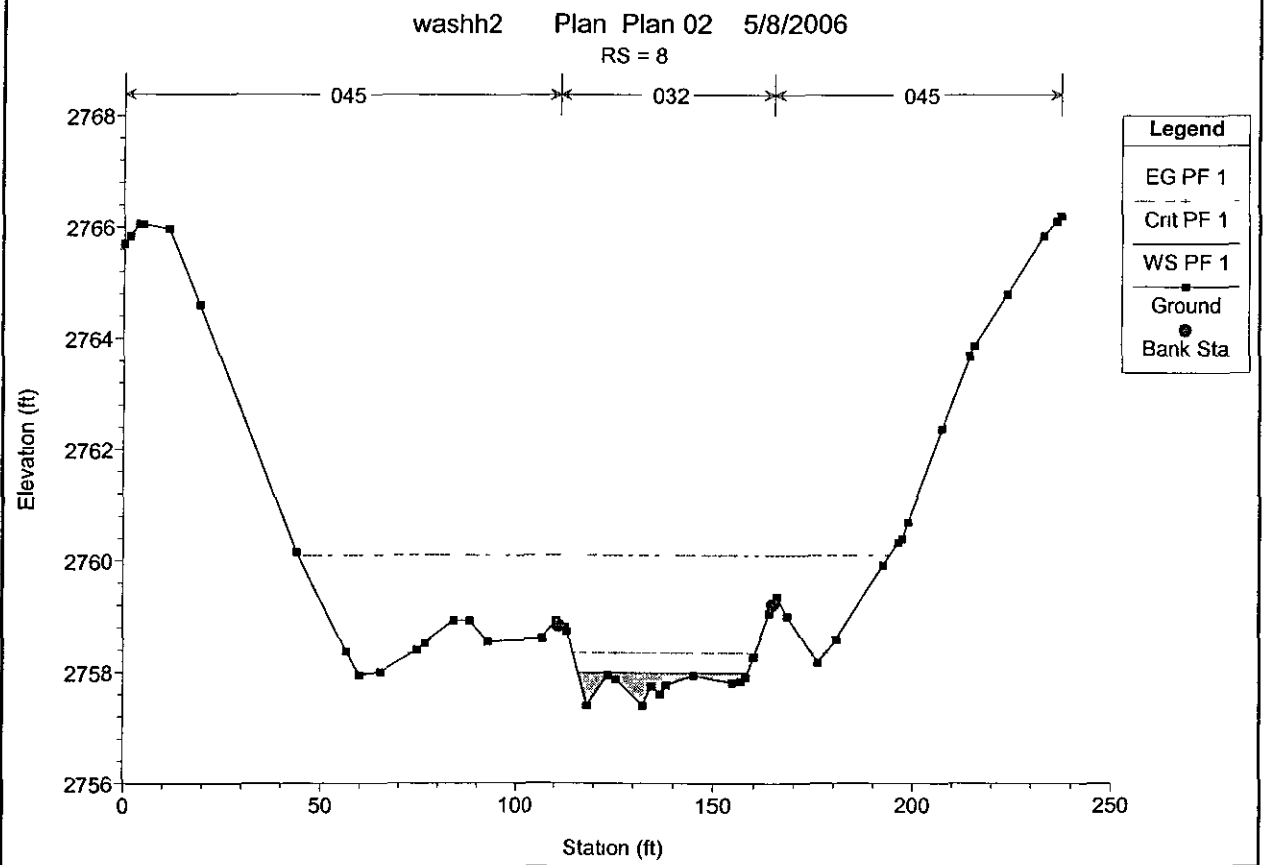
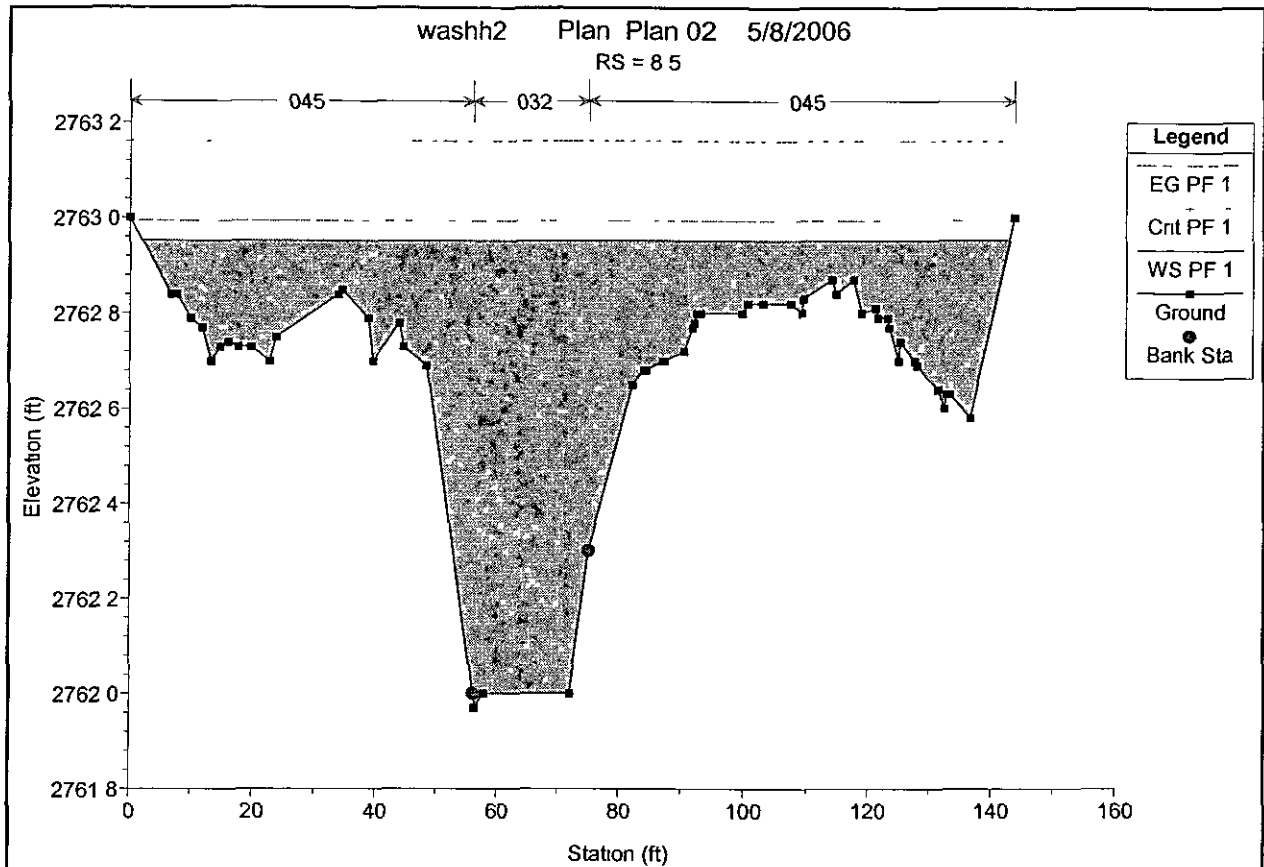
Reach	River Sta	Profile	Q Total (cfs)	Min Ch Elev (ft)	W S Elev (ft)	Crit W S (ft)	E.G. Elev (ft)	E.G. Slope (ft/R)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	15	PF_1	50.00	2821.98	2822.72	2822.85	2823.16	0.043048	5.45	9.64	25.05	1.47
Reach-1	14	PF_1	50.00	2814.10	2814.57	2814.63	2814.83	0.038257	4.15	12.04	38.90	1.32
Reach-1	13	PF_1	50.00	2808.80	2809.31	2809.36	2809.54	0.036980	3.82	13.10	46.83	1.27
Reach-1	12	PF_1	50.00	2800.73	2801.22	2801.30	2801.52	0.063174	4.36	11.47	50.18	1.61
Reach-1	11.5	PF_1	50.00	2795.20	2795.75	2795.81	2796.03	0.026258	4.34	12.81	37.70	1.16
Reach-1	11	PF_1	110.00	2783.25	2784.72	2785.16	2786.12	0.060907	9.51	11.56	14.98	1.91
Reach-1	10	PF_1	110.00	2773.90	2775.18	2775.37	2775.73	0.031554	6.25	21.64	59.62	1.35
Reach-1	9	PF_1	110.00	2764.93	2766.03	2766.36	2767.05	0.052813	8.09	13.60	20.48	1.75
Reach-1	8.5	PF_1	110.00	2761.97	2762.95	2762.99	2763.16	0.009532	4.32	45.50	141.00	0.79
Reach-1	8	PF_1	110.00	2757.40	2757.99	2758.35	2760.10	0.469743	11.66	9.51	46.80	4.35
Reach-1	7	PF_1	110.00	2751.66	2752.64	2752.66	2752.98	0.016236	4.71	24.75	45.62	0.98
Reach-1	6	PF_1	110.00	2749.39	2749.94	2750.26	2751.09	0.119556	8.63	12.75	32.33	2.42
Reach-1	5	PF_1	110.00	2746.81	2747.80	2747.91	2748.30	0.028449	5.70	19.29	30.93	1.27
Reach-1	4	PF_1	110.00	2744.09	2744.94	2745.29	2746.05	0.056779	8.44	13.03	19.34	1.81
Reach-1	3	PF_1	110.00	2739.56	2740.88	2741.04	2741.47	0.031414	6.12	17.97	28.08	1.34
Reach-1	2	PF_1	161.00	2735.23	2736.20	2736.52	2737.24	0.053455	8.19	19.67	29.43	1.76
Reach-1	1	PF_1	161.00	2731.01	2731.61	2731.79	2732.24	0.047739	6.39	25.19	50.01	1.59

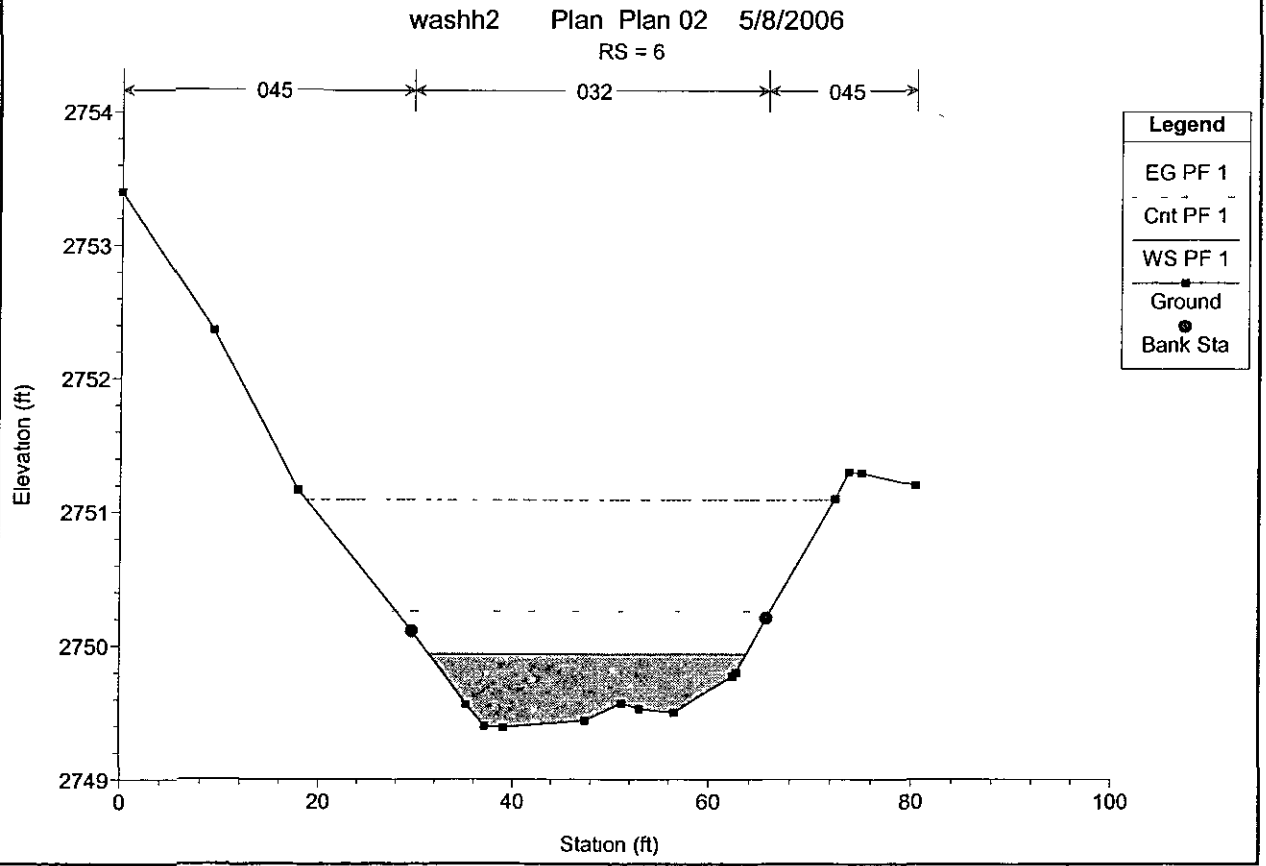
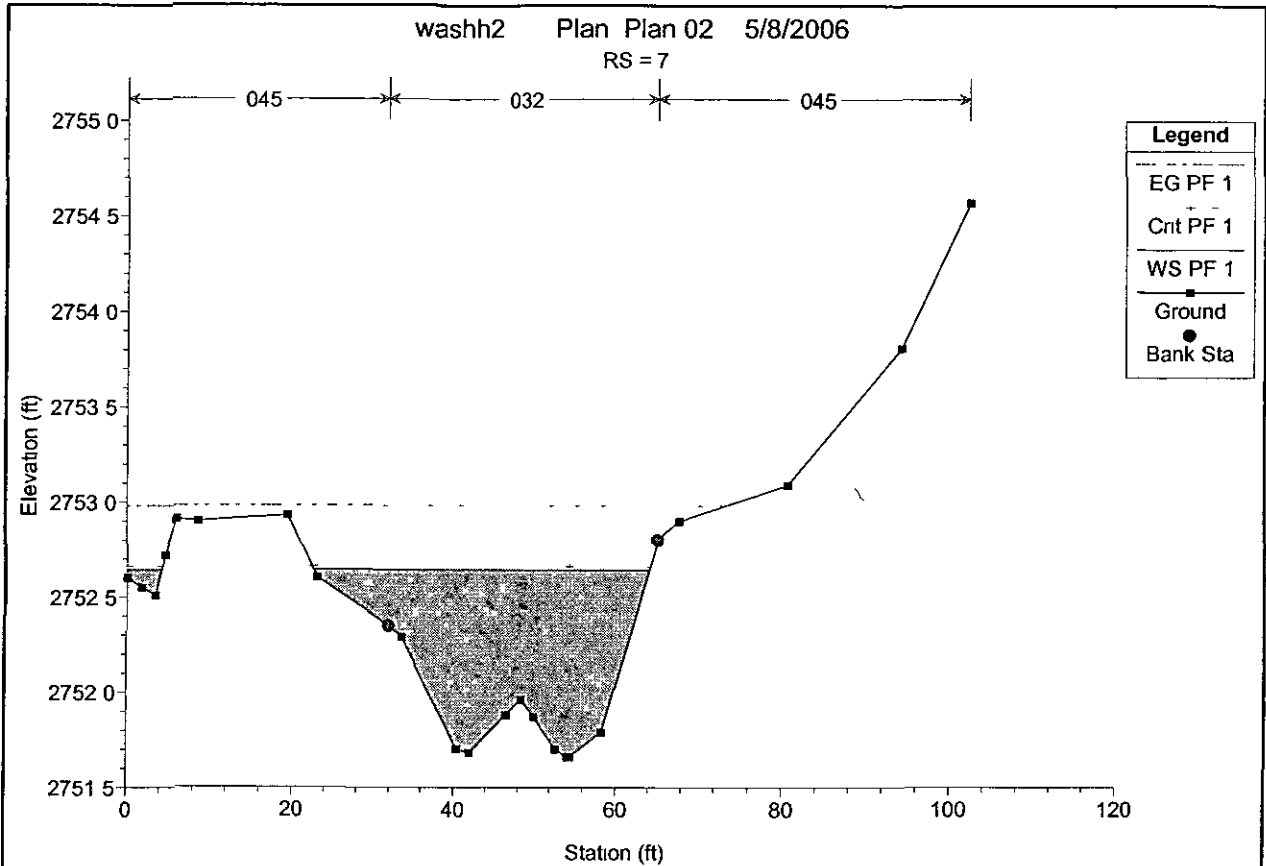




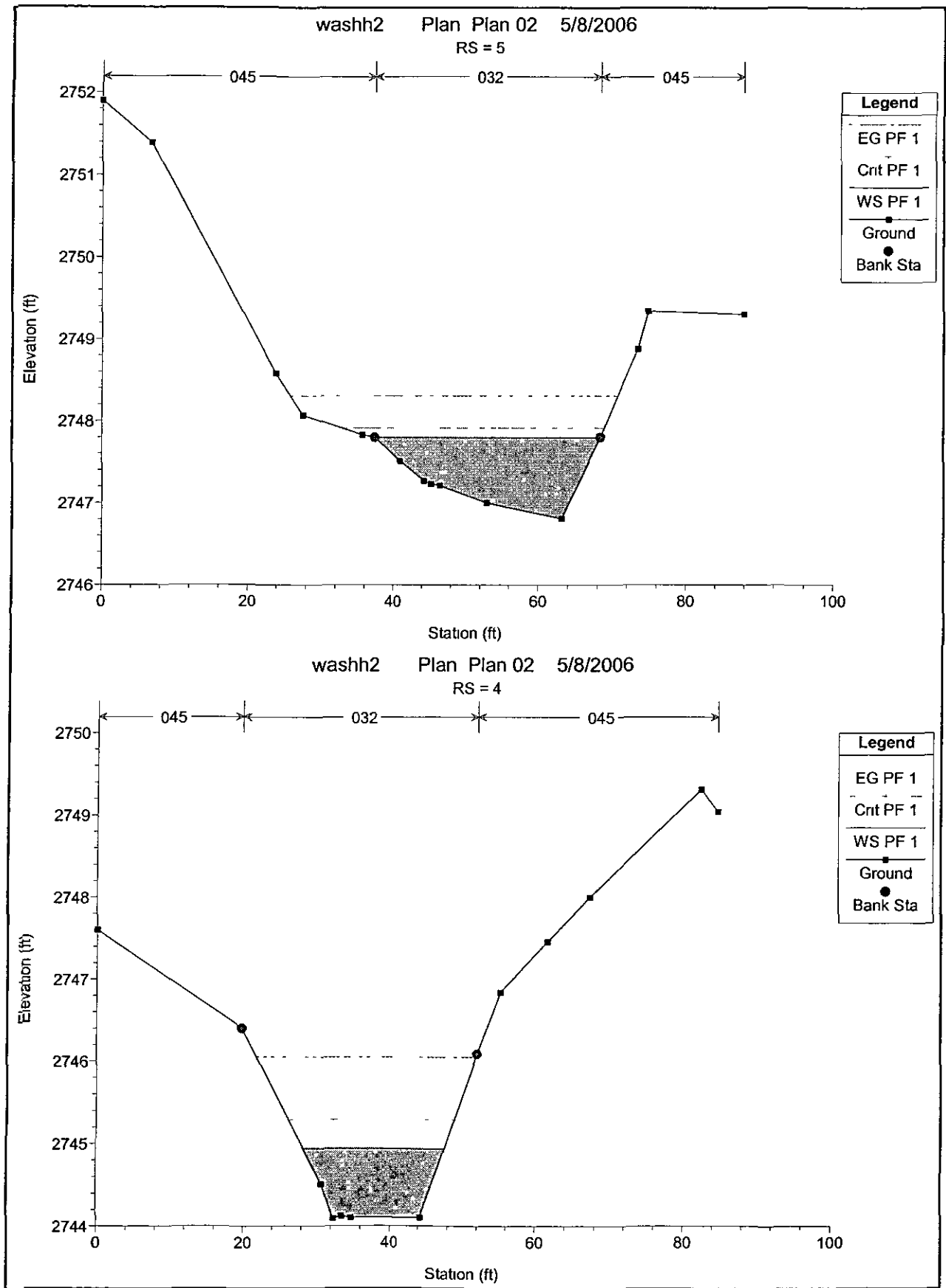




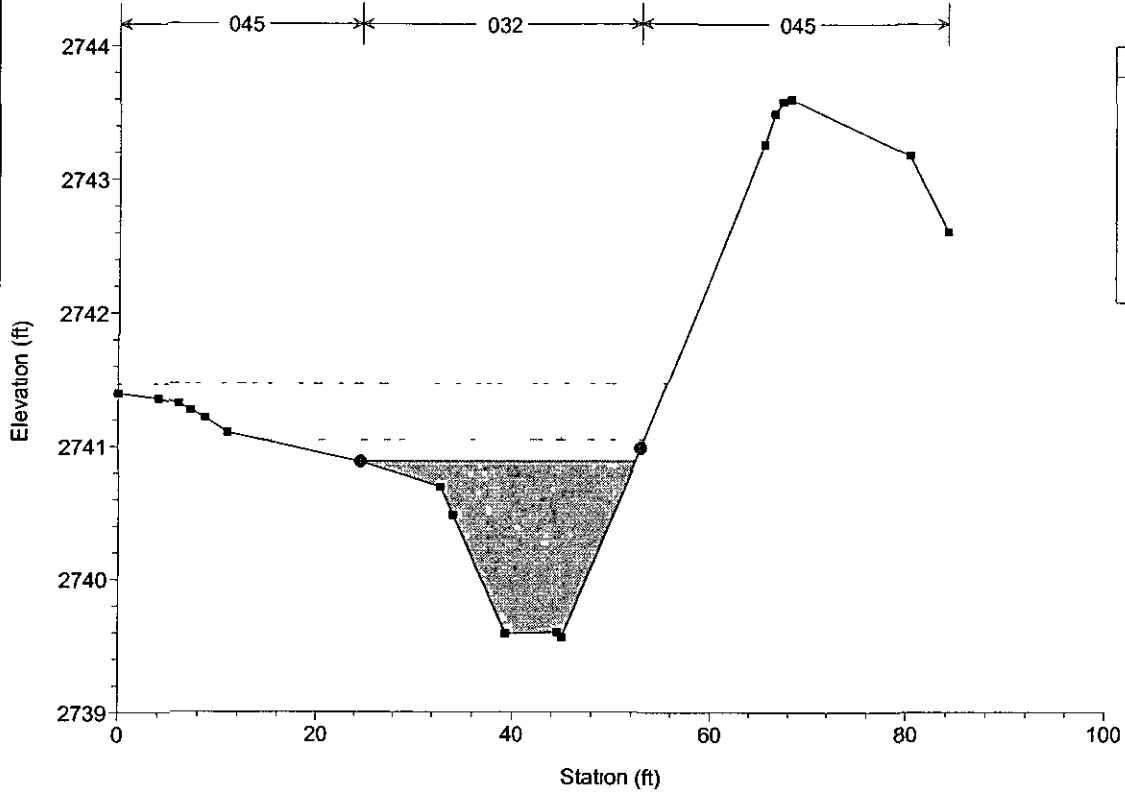




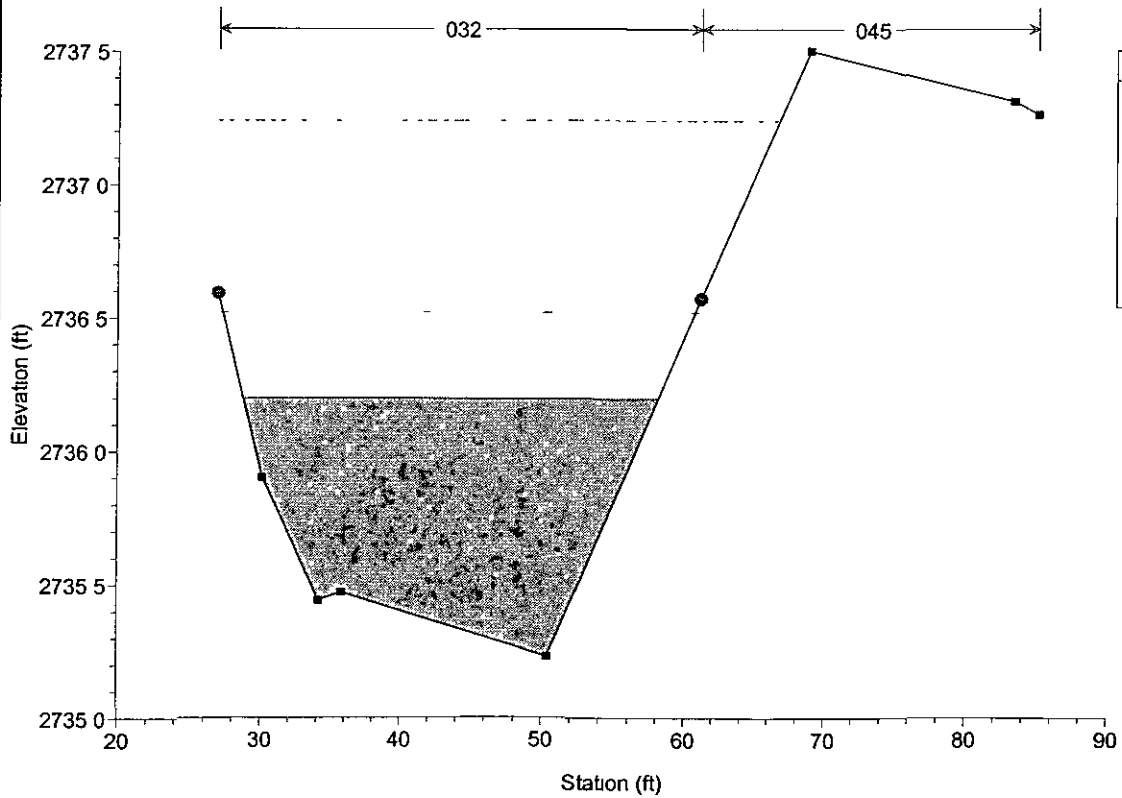




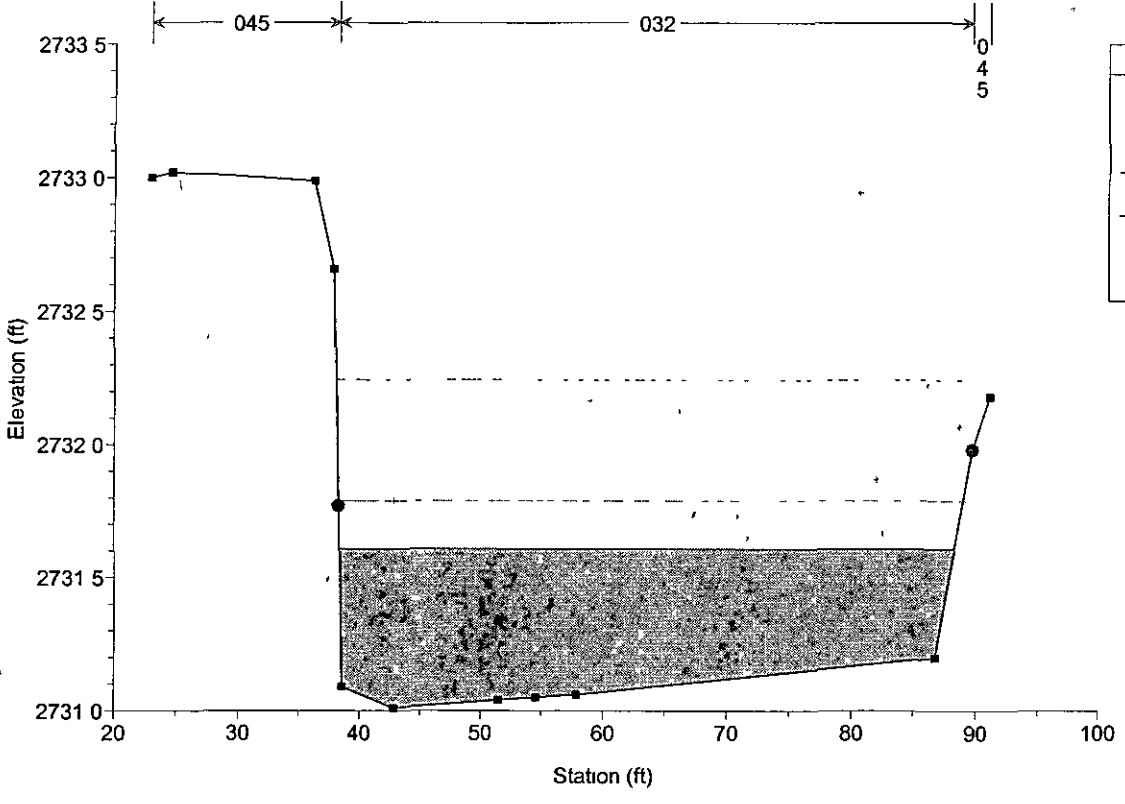
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washh2 Plan Plan 02 5/8/2006  
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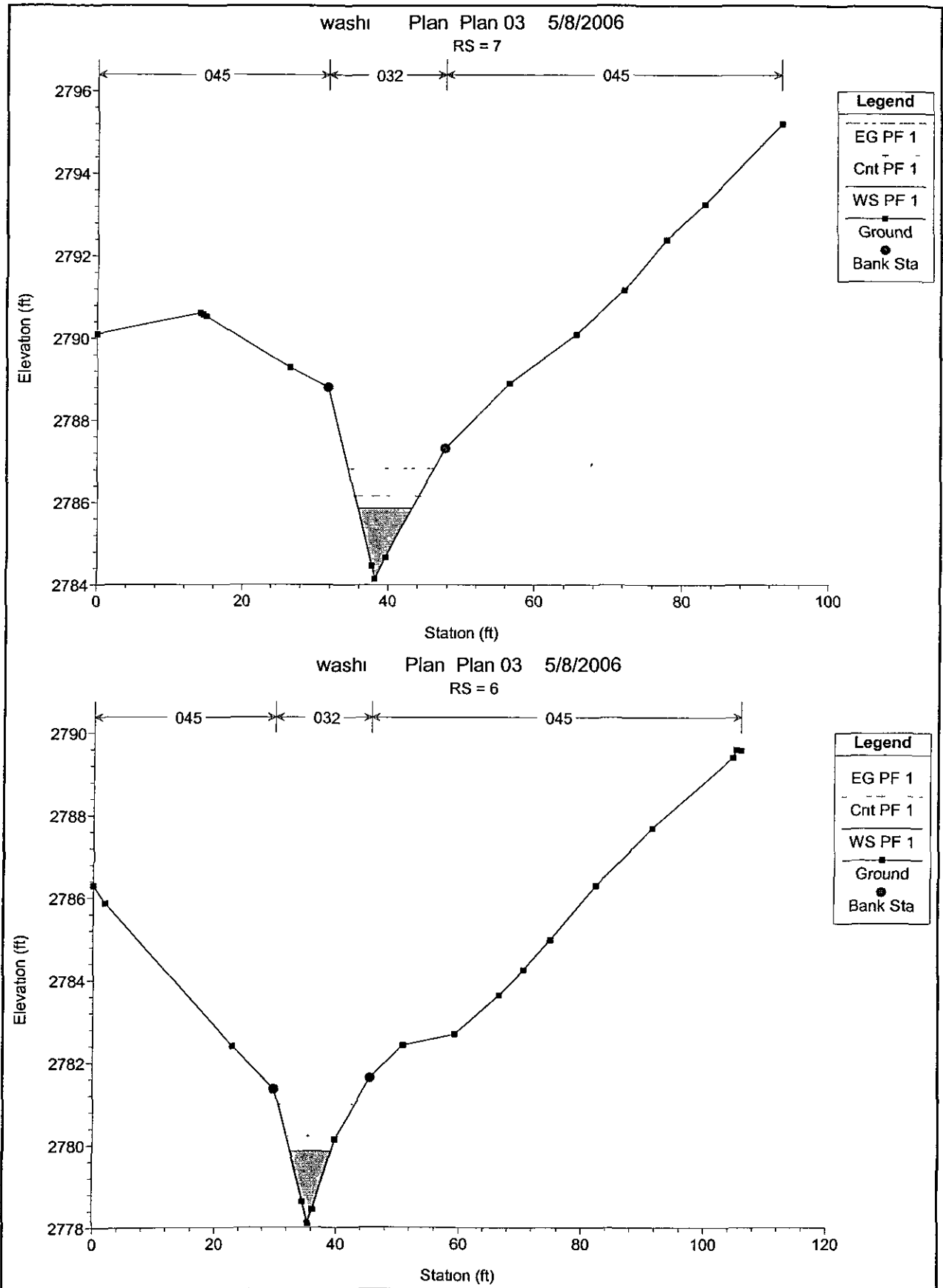


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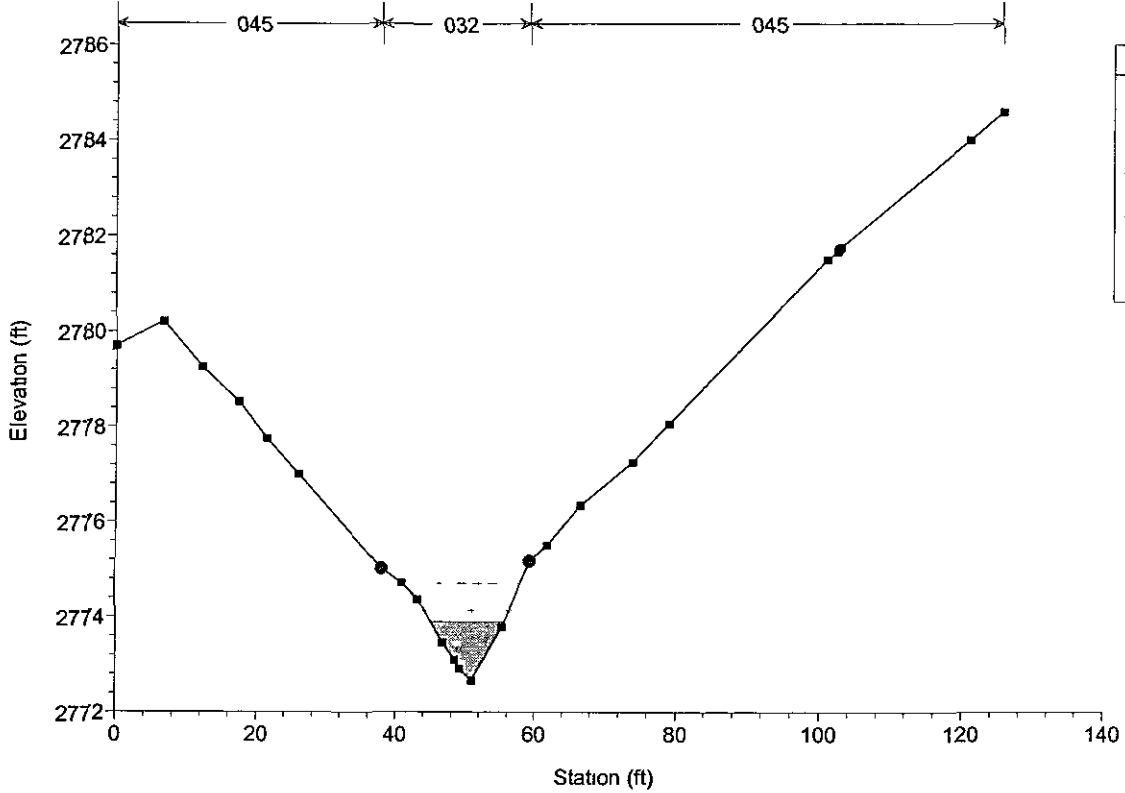


HEC-RAS Plan Plan 03 Rver RIVER-1 Reach Reach-1 Profile PF 1

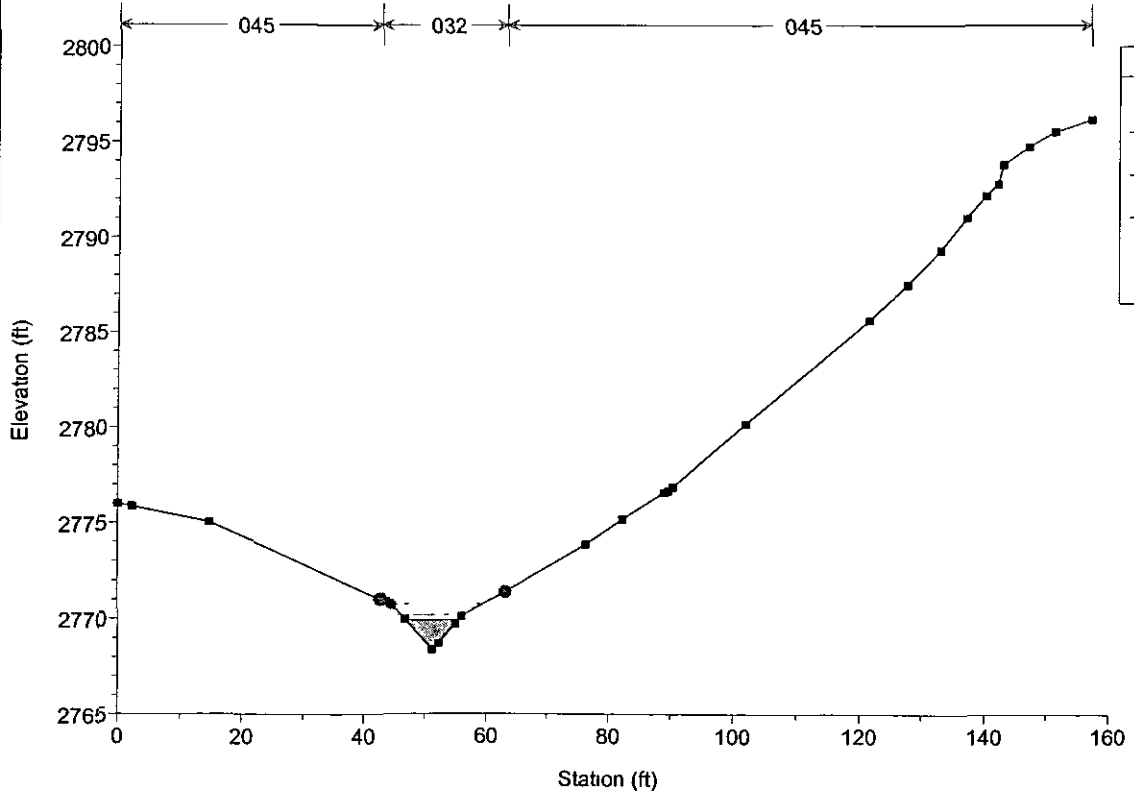
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W S Elev (ft)	Crit W S (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	7	PF 1	50.00	2784.14	2785.86	2786.15	2786.82	0.041077	7.86	6.36	7.51	1.50
Reach-1	6	PF 1	50.00	2778.10	2779.85	2780.22	2780.99	0.047685	8.57	5.84	6.64	1.61
Reach-1	5	PF 1	50.00	2772.66	2773.89	2774.14	2774.70	0.043982	7.22	6.92	10.54	1.57
Reach-1	4	PF 1	50.00	2768.37	2769.92	2770.18	2770.77	0.038173	7.42	6.74	8.58	1.47
Reach-1	3	PF 1	51.00	2764.31	2765.70	2766.03	2766.69	0.049526	7.97	6.40	9.02	1.67
Reach-1	2	PF 1	51.00	2760.53	2761.93	2762.26	2762.86	0.049033	7.75	6.58	9.66	1.65
Reach-1	1.5	PF 1	51.00	2757.88	2758.42	2758.54	2758.75	0.058439	4.61	11.07	42.05	1.58
Reach-1	1	PF 1	70.00	2754.90	2755.86	2755.96	2756.22	0.034324	4.81	14.55	34.53	1.31



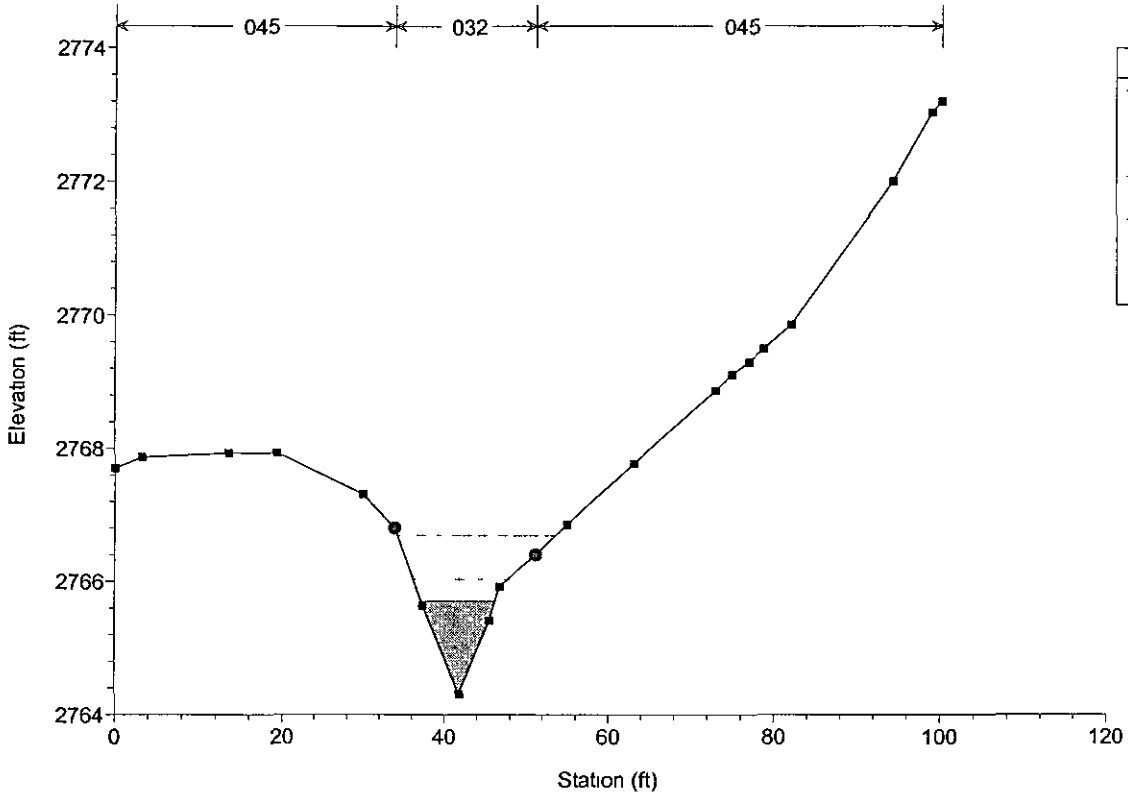
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washi Plan Plan 03 5/8/2006  
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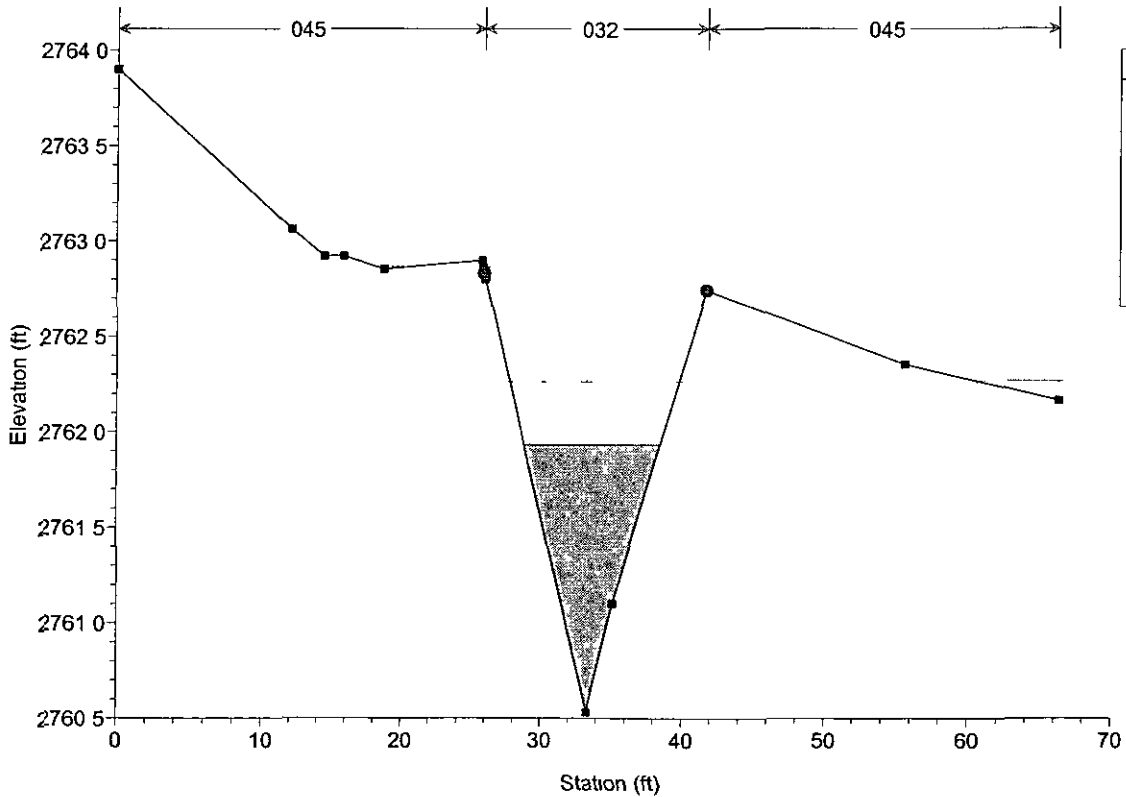


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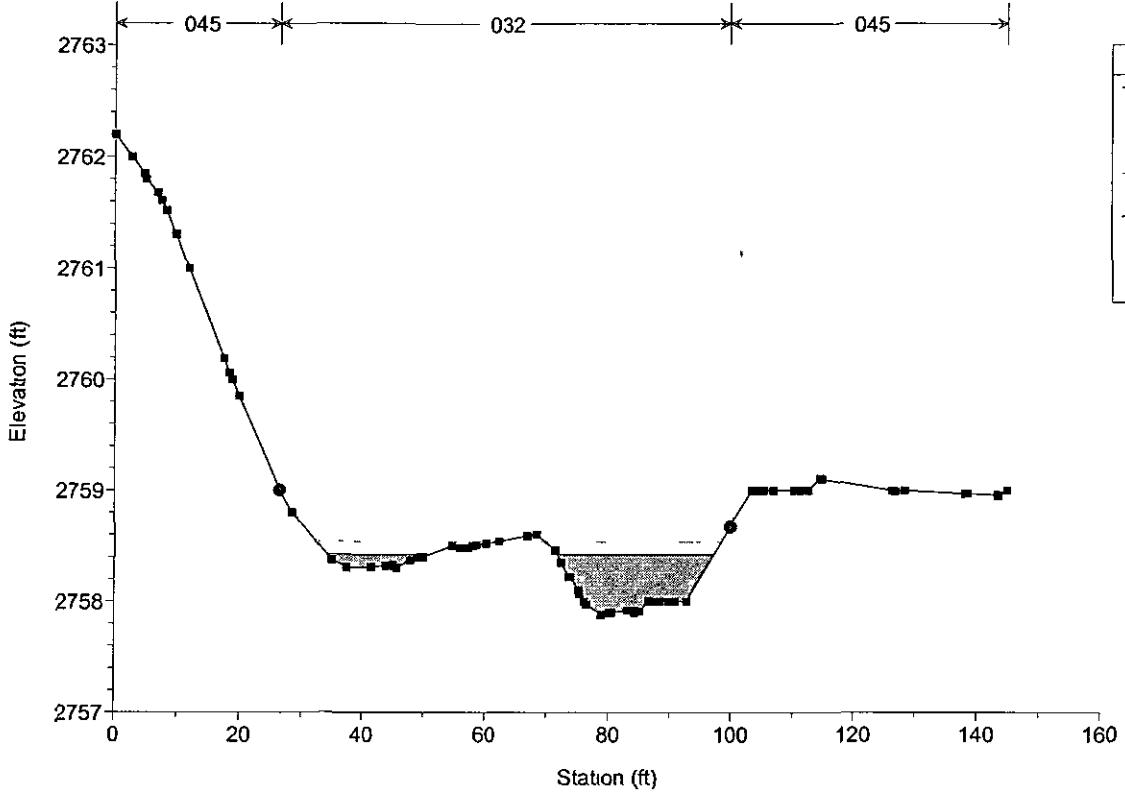
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—■—	WS PF 1
●	Ground
●	Bank Sta

washi Plan Plan 03 5/8/2006  
RS = 2

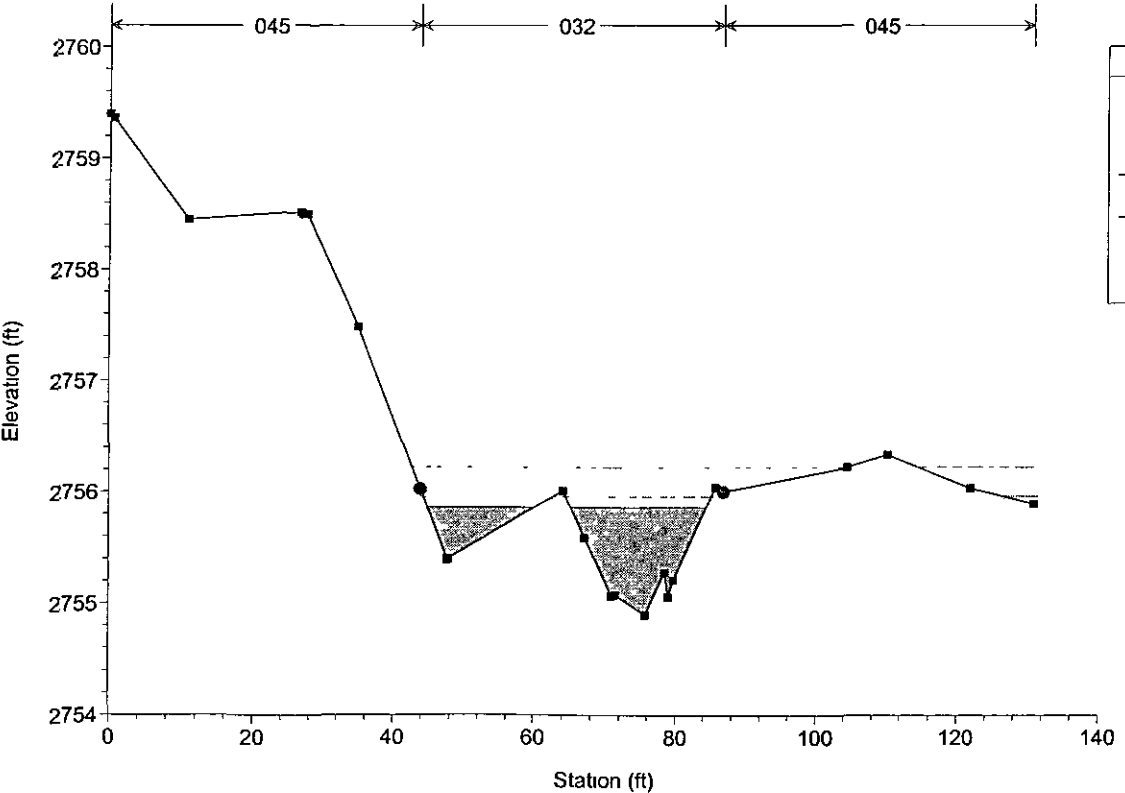


Legend	
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—	Cnt PF 1
—■—	WS PF 1
●	Ground
●	Bank Sta

washi Plan Plan 03 5/8/2006  
RS = 1 5



washi Plan Plan 03 5/8/2006  
RS = 1





**Orifice Plate Calculation**

# WOOD/PATEL

CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

**Project:** *Sereno Canyon*

**WIP#:** 042054

**Location:** *City of Scottsdale*

**Date:** 5/11/2006

**Reference:** *Orifice Equation*

$$Q = CA\sqrt{2gh}$$

Where

C = Orifice Coefficient = 0.60

d = Diameter of Orifice = 6.00 in

A = Cross-sectional area of orifice = 0.20 Sq ft

H = Depth above Orifice = 1.50 ft

Q = Orifice Flow = 1.16 cfs

Time to Drain 2 ac-ft = 20.9 hours

A 6 inch diameter orifice plate will bleed at a rate of 1.16 cfs

**APPENDIX G**

**Retention Basin Volume Calculations**

**Sereno Canyon  
Detention Basin Volume Calculations**

Drainage Basin	Total Development Area (Acre)	100yr-2 hr Volume (acre-ft)	Provided Volume (acre-ft)	Volume Shortage / Excess ± (acre-ft)
A1	3.79	0.53	1.04	0.51
A2	7.17	1.01	1.01	0.00
B	10.94	1.54	1.68	0.14
C	3.65	0.51	0.79	0.28
D	7.73	1.09	1.21	0.12
E1	14.24	2.01	2.03	0.02
E2	13.23	1.87	1.93	0.06
E3	2.19	0.32	0.34	0.02
F1	10.24	1.44	1.60	0.16
F2	3.56	0.5	0.78	0.28
F3	2.93	0.41	0.56	0.15
G	3.7	0.52	0.59	0.07
H1	12.94	1.82	1.94	0.12
H2	8.5	1.2	1.21	0.01
1	4.44	0.63	0.64	0.01
J	4.81	0.68	0.71	0.03
K	2.06	0.29	0.38	0.09
L	1.49	0.21	0.23	0.02
M	1.44	0.2	0.28	0.08
N	2.81	0.4	0.42	0.02
O	0.55	0.08	0.09	0.01
P	1.73	0.24	0.27	0.03
Q	0.52	0.07	0.07	0.00
R	0.18	0.02	0.05	0.03
Total	125	17.59	19.87	2.28

**Q=(CIA)/12**

Q is the volume in acre-ft

C = 0.6

I = 2.82 in

A = building envelope & roadway area in ACRES

\*Total Development Area Includes the Right-of-Way for the Roads plus the Building Envelopes\*

Project *Sereno Canyon*  
 Location *City of Scottsdale*  
 Date *11-May-06*

Project Number *042054*  
 Project Engineer *Gordon W Wark, P E*

**DETENTION BASIN VOLUME CALCULATIONS**

BASIN - A1-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2772	1604	0.037	0.000	0.000
2773	2151	0.049	0.043	0.043
2774	2763	0.063	0.056	0.100
2777	3441	0.079	0.214	0.313

TOTAL PROVIDED VOLUME 0.313

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr Provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
A1	A1-1	0.0790	0.313		
	A1-2	0.1757	0.726		
<b>TOTALS</b>		<b>0.2547</b>	<b>1.040</b>	<b>0.53</b>	<b>0.510</b>

BASIN - A1-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2756	4199	0.096	0.000	0.000
2757	5251	0.121	0.108	0.108
2758	6404	0.147	0.134	0.242
2761	7655	0.176	0.484	0.726

TOTAL PROVIDED VOLUME 0.726

BASIN - A2-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2758	351	0.008	0.000	0.000
2759	702	0.016	0.012	0.012
2760	1209	0.028	0.022	0.034
2761	1823	0.042	0.035	0.069

TOTAL PROVIDED VOLUME 0.069

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
A2	A2-1	0.0419	0.069		
	A2-2	0.0284	0.040		
	A2-3	0.1476	0.407		
	A2-4	0.0376	0.0504		
	A2-5	0.1885	0.4484		
	A2-6	0.1104	0.0000		
<b>TOTALS</b>		<b>0.5544</b>	<b>1.01</b>	<b>1.01</b>	<b>0.0</b>

BASIN - A2-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2758	112	0.003	0.000	0.000
2759	358	0.008	0.005	0.005
2760	716	0.016	0.012	0.018
2761	1236	0.028	0.022	0.040

TOTAL PROVIDED VOLUME 0.040

BASIN - A2-3				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2733	2653	0.061	0.000	0.000
2734	3460	0.079	0.070	0.070
2735	4363	0.100	0.090	0.160
2736	5357	0.123	0.112	0.272
2737	6431	0.148	0.135	0.407

TOTAL PROVIDED VOLUME 0.407

BASIN - A2-4				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2792	168	0.004	0.000	0.000
2793	425	0.010	0.007	0.007
2794	866	0.020	0.015	0.022
2795	1638	0.038	0.029	0.050

TOTAL PROVIDED VOLUME 0.050

BASIN - A2-5				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2772	4926	0.113	0.000	0.000
2773	5935	0.136	0.125	0.125
2774	7026	0.161	0.149	0.273
2775	8213	0.189	0.175	0.448

TOTAL PROVIDED VOLUME 0.448

BASIN - A2-6				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2772	2085	0.048	0.000	0.000
2773	2843	0.065	0.057	0.057
2774	3745	0.086	0.076	0.132
2777	4811	0.110	0.295	0.427

TOTAL PROVIDED VOLUME 0.427

BASIN - B-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2798	809	0.019	0.000	0.000
2799	1241	0.028	0.024	0.024
2800	1804	0.041	0.035	0.058
2801	2456	0.056	0.049	0.107

TOTAL PROVIDED VOLUME: 0.107

BASIN - B-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2798	2378	0.055	0.000	0.000
2799	3095	0.071	0.063	0.063
2800	3952	0.091	0.081	0.144
2801	4986	0.114	0.103	0.246

TOTAL PROVIDED VOLUME: 0.246

BASIN - B-3				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2781	3418	0.078	0.000	0.000
2782	4333	0.099	0.089	0.089
2783	5371	0.123	0.111	0.200
2784	6633	0.152	0.138	0.338

TOTAL PROVIDED VOLUME: 0.338

BASIN - B-4				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2768	2154	0.049	0.000	0.000
2769	2990	0.069	0.059	0.059
2770	3950	0.091	0.080	0.139
2771	4983	0.114	0.103	0.241

TOTAL PROVIDED VOLUME: 0.241

BASIN - B-5				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2746	8320	0.191	0.000	0.000
2747	9979	0.229	0.210	0.210
2748	11724	0.269	0.249	0.459
2749	13573	0.312	0.290	0.750

TOTAL PROVIDED VOLUME: 0.750

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
B	B-1	0.0564	0.107		
	B-2	0.1145	0.246		
	B-3	0.1523	0.338		
	B-4	0.1144	0.241		
	B-5	0.3116	0.750		
TOTAL		0.7491	1.683	1.54	0.143

BASIN - C-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2779	4198	0.096	0.000	0.000
2780	5112	0.117	0.107	0.107
2781	6102	0.140	0.129	0.236
2782	7169	0.165	0.152	0.388
2783	8375	0.192	0.178	0.566

TOTAL PROVIDED VOLUME: 0.566

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
C	C-1	0.1923	0.566		
	C-2	0.1121	0.225		
TOTAL		0.3044	0.791	0.51	0.281

BASIN - C-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2754	1955	0.045	0.000	0.000
2755	2712	0.062	0.054	0.054
2756	3657	0.084	0.073	0.127
2757	4885	0.112	0.098	0.225

TOTAL PROVIDED VOLUME: 0.225

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
D	D-1	0.1361	0.349		
	D-2	0.1594	0.364		
	D-3	0.2176	0.500		
TOTAL		0.5131	1.213	1.09	0.123

BASIN - D-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2749	2054	0.047	0.000	0.000
2750	2819	0.065	0.056	0.056
2751	3699	0.085	0.075	0.131
2752	4712	0.108	0.097	0.227
2753	5928	0.136	0.122	0.349

TOTAL PROVIDED VOLUME: 0.349

BASIN - D-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2724	3734	0.086	0.000	0.000
2725	4728	0.109	0.097	0.097
2726	5799	0.133	0.121	0.218
2727	6943	0.159	0.146	0.364

TOTAL PROVIDED VOLUME: 0.364

BASIN - D-3				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2736	5188	0.119	0.000	0.000
2737	6505	0.149	0.134	0.134
2738	7928	0.182	0.166	0.300
2739	9480	0.218	0.200	0.500

TOTAL PROVIDED VOLUME: 0.500

BASIN - E1-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2724	5286	0.121	0.000	0.000
2725	6163	0.141	0.131	0.131
2726	7100	0.163	0.152	0.284
2727	8095	0.186	0.174	0.458

TOTAL PROVIDED VOLUME: 0.458

BASIN - E1-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2716	7154	0.164	0.000	0.000
2717	6605	0.198	0.181	0.181
2718	10153	0.233	0.215	0.396
2719	11781	0.270	0.252	0.648

TOTAL PROVIDED VOLUME: 0.648

BASIN - E1-3				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2695	5080	0.117	0.000	0.000
2696	6952	0.160	0.138	0.138
2697	8543	0.196	0.178	0.316
2698	10236	0.235	0.216	0.532

TOTAL PROVIDED VOLUME: 0.532

BASIN - E1-4				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2689	4285	0.098	0.000	0.000
2690	5165	0.119	0.108	0.108
2691	6138	0.141	0.130	0.238
2692	7207	0.165	0.153	0.391

TOTAL PROVIDED VOLUME: 0.391

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
E1	E1-1	0.1858	0.458		
	E1-2	0.2705	0.648		
	E1-3	0.2350	0.532		
	E1-4	0.1654	0.391		
<b>TOTAL</b>		<b>0.8567</b>	<b>2.029</b>	<b>2.01</b>	<b>0.019</b>

BASIN - E2-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2710	4399	0.101	0.000	0.000
2711	5399	0.124	0.112	0.112
2712	6476	0.149	0.136	0.249
2713	7667	0.176	0.162	0.411
2714	9046	0.208	0.192	0.603

TOTAL PROVIDED VOLUME: 0.603

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
E2	E2-1	0.2077	0.603		
	E2-2	0.3069	0.919		
	E2-3	0.1862	0.407		
<b>TOTAL</b>		<b>0.7007</b>	<b>1.929</b>	<b>1.87</b>	<b>0.059</b>

BASIN - E2-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2694	6967	0.160	0.000	0.000
2695	8344	0.192	0.176	0.176
2696	9919	0.228	0.210	0.385
2697	11588	0.266	0.247	0.632
2698	13368	0.307	0.286	0.919

TOTAL PROVIDED VOLUME: 0.919

BASIN - E2-3				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2682	3978	0.091	0.000	0.000
2683	5153	0.118	0.105	0.105
2684	6529	0.150	0.134	0.239
2685	8110	0.186	0.168	0.407

TOTAL PROVIDED VOLUME. 0.407

BASIN - E3-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2711	3064	0.070	0.000	0.000
2712	4065	0.093	0.082	0.082
2713	5197	0.119	0.106	0.188
2714	6456	0.148	0.134	0.322

TOTAL PROVIDED VOLUME. 0.322

BASIN - E3-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2676	817	0.019	0.000	0.000
2677	1226	0.028	0.023	0.023

1.0' depth

TOTAL PROVIDED VOLUME. 0.023

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
E3	E3-1	0.1482	0.322		
	E3-2	0.0280	0.023		
TOTAL		0.1762	0.345	0.32	0.025

BASIN - F1-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2726	4563	0.105	0.000	0.000
2727	5553	0.127	0.118	0.116
2728	6670	0.153	0.140	0.256
2729	7918	0.182	0.167	0.424

TOTAL PROVIDED VOLUME. 0.424

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
F1	F1-1	0.1818	0.424		
	F1-2	0.1987	0.462		
	F1-3	0.2059	0.483		
	F1-4	0.1128	0.227		
TOTAL		0.6992	1.596	1.44	0.156

BASIN - F1-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2710	4891	0.112	0.000	0.000
2711	6044	0.139	0.128	0.126
2712	7298	0.168	0.153	0.279
2713	8656	0.199	0.183	0.462

TOTAL PROVIDED VOLUME. 0.462

BASIN - F1-3				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2696	5180	0.119	0.000	0.000
2697	6353	0.146	0.132	0.132
2698	7624	0.175	0.160	0.293
2699	8989	0.206	0.190	0.483

TOTAL PROVIDED VOLUME. 0.483

BASIN - F1-4				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2686	1919	0.044	0.000	0.000
2687	2757	0.063	0.054	0.054
2688	3733	0.086	0.074	0.128
2689	4913	0.113	0.099	0.227

TOTAL PROVIDED VOLUME. 0.227

BASIN - F2-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2701	1923	0.044	0.000	0.000
2702	2906	0.067	0.055	0.055
2703	3987	0.092	0.079	0.135
2706	5172	0.119	0.315	0.450

TOTAL PROVIDED VOLUME. 0.450

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
F2	F2-1	0.1187	0.450		
	F2-2	0.1329	0.254		
	F2-3	0.0462	0.075		
TOTAL		0.2978	0.778	0.5	0.278



BASIN - F2-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2682	1841	0.042	0.000	0.000
2683	2963	0.068	0.055	0.055
2684	4269	0.098	0.083	0.138
2685	5789	0.133	0.115	0.254

TOTAL PROVIDED VOLUME 0.254

BASIN - F2-3				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2692	353	0.008	0.000	0.000
2693	770	0.018	0.013	0.013
2694	1302	0.030	0.024	0.037
2695	2011	0.046	0.038	0.075

TOTAL PROVIDED VOLUME 0.075

BASIN - F3-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2724	178	0.004	0.000	0.000
2725	410	0.009	0.007	0.007
2726	746	0.017	0.013	0.020
2727	1279	0.029	0.023	0.043

TOTAL PROVIDED VOLUME 0.043

BASIN - F3-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2706	1932	0.044	0.000	0.000
2707	2855	0.066	0.055	0.055
2708	3941	0.090	0.078	0.133
2709	5286	0.121	0.106	0.239
2711	6815	0.156	0.278	0.517

TOTAL PROVIDED VOLUME 0.517

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
F3	F3-1	0.0294	0.043		
	F3-2	0.1565	0.517		
TOTAL		0.1858	0.560	0.41	0.150

BASIN - G-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2723	8168	0.142	0.000	0.000
2724	7712	0.177	0.159	0.159
2725	9387	0.215	0.196	0.356
2726	11197	0.257	0.236	0.592

TOTAL PROVIDED VOLUME 0.592

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
G	G-1	0.2570	0.592		
TOTAL		0.2570	0.592	0.52	0.072

BASIN - H1-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2770	524	0.012	0.000	0.000
2771	1010	0.023	0.018	0.018
2772	1621	0.037	0.030	0.048
2773	2383	0.055	0.046	0.094

TOTAL PROVIDED VOLUME 0.094

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
H1	H1-1	0.0547	0.094		
	H1-2	0.0307	0.044		
	H1-3	0.1655	0.474		
	H1-4	0.2362	0.5850		
	H1-5	0.2956	0.7467		
TOTAL		0.7827	1.94	1.82	0.12

BASIN - H1-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2766	179	0.004	0.000	0.000
2767	418	0.010	0.007	0.007
2768	757	0.017	0.013	0.020
2769	1339	0.031	0.024	0.044

TOTAL PROVIDED VOLUME 0.044

BASIN - H1-3				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2736	3296	0.076	0.000	0.000
2737	4159	0.095	0.086	0.086
2738	5099	0.117	0.106	0.192
2739	6118	0.140	0.129	0.321
2740	7208	0.165	0.153	0.474

TOTAL PROVIDED VOLUME 0.474

BASIN - H1-4				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2735	6797	0 156	0 000	0 000
2736	7887	0 181	0 169	0 169
2737	9051	0 208	0 194	0 363
2738	10290	0 236	0 222	0 585

TOTAL PROVIDED VOLUME. 0 585

BASIN - H1-5				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2727	8891	0 204	0 000	0 000
2728	10158	0 233	0 219	0 219
2729	11486	0 264	0 248	0 467
2730	12876	0 296	0 280	0 747

TOTAL PROVIDED VOLUME. 0 747

BASIN - H2-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2772	6928	0 159	0 000	0 000
2773	8286	0 190	0 175	0 175
2774	9774	0 224	0 207	0 382
2775	11346	0 260	0 242	0 624

TOTAL PROVIDED VOLUME. 0 624

BASIN - H2-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2785	6526	0 150	0 000	0 000
2786	7846	0 180	0 165	0 165
2787	9247	0 212	0 196	0 361
2788	10730	0 246	0 229	0 590

TOTAL PROVIDED VOLUME. 0 590

BASIN - I-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2755	7532	0 173	0 000	0 000
2756	8664	0 199	0 186	0 186
2757	9863	0 226	0 213	0 399
2758	11132	0 256	0 241	0 640

TOTAL PROVIDED VOLUME 0 640

BASIN - J-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2769	3832	0 088	0 000	0 000
2770	4689	0 108	0 098	0 098
2771	5667	0 130	0 119	0 217
2772	6755	0 155	0 143	0 359
2773	7983	0 183	0 169	0 528

TOTAL PROVIDED VOLUME: 0 528

BASIN - J-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2762	1719	0 039	0 000	0 000
2763	2323	0 053	0 046	0 046
2764	3026	0 069	0 061	0 108
2765	3836	0 088	0 079	0 187

TOTAL PROVIDED VOLUME 0 187

BASIN - K-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2737	4211	0 097	0 000	0 000
2738	5026	0 115	0 106	0 106
2739	5909	0 136	0 126	0 232
2740	6860	0 157	0 147	0 378

TOTAL PROVIDED VOLUME 0.378

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
H2	H2-1	0 2605	0 624		
	H2-2	0 2463	0 590		
TOTAL		0 5068	1 215	1 2	0 015

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
I	I-1	0 2556	0 640		
TOTAL		0 2556	0 640	0 63	0 010

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
J	J-1	0 1833	0 528		
	J-2	0 0881	0 187		
TOTAL		0 2713	0 715	0 68	0 035

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
K	K-1	0 1575	0 378		
TOTAL		0 1575	0 378	0 29	0 088

BASIN - L1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2744	1366	0 031	0 000	0 000
2745	1889	0 043	0 037	0 037

TOTAL PROVIDED VOLUME 0.037

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
L	L-1	0 0434	0 037		
	L-2	0 1084	0 188		
TOTAL		0 1518	0 225	0 21	0 015

BASIN - L2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2722	1003	0 023	0 000	0 000
2723	2030	0 047	0 035	0 035
2724	3288	0 075	0 061	0 096
2725	4723	0 108	0 092	0 188

TOTAL PROVIDED VOLUME 0 188

BASIN - M-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2721	2543	0 058	0 000	0 000
2722	3462	0 079	0 069	0 069
2723	4509	0 104	0 091	0 160
2724	5737	0 132	0 118	0 278

TOTAL PROVIDED VOLUME 0 278

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
M	M-1	0 1317	0 278		
TOTAL		0 1317	0 278	0 2	0 078

BASIN - N-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2691	4188	0 096	0 000	0 000
2692	5407	0 124	0 110	0 110
2693	6760	0 155	0 140	0 250
2694	8217	0 189	0 172	0 422

TOTAL PROVIDED VOLUME 0 422

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
N	N-1	0 1886	0 422		
TOTAL		0 1886	0 422	0 4	0 022

BASIN - O-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2721	3682	0 085	0 000	0 000
2722	4568	0 105	0 095	0 095

TOTAL PROVIDED VOLUME 0 095

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
O	O-1	0 1049	0 095		
TOTAL		0 1049	0 095	0 08	0 015

BASIN - P-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2764	2082	0 048	0 000	0 000
2765	2732	0 063	0 055	0 055

TOTAL PROVIDED VOLUME 0.055

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
P	P-1	0 0627	0 055		
	P-2	0 0647	0 056		
	P-3	0 1693	0 156		
TOTAL		0 2967	0 268	0 24	0 028

BASIN - P-2				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2749	2063	0 047	0 000	0 000
2750	2818	0 065	0 056	0 056

TOTAL PROVIDED VOLUME 0.056 acre-feet

BASIN - P-3				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2744	6240	0 143	0 000	0 000
2745	7374	0 169	0 156	0 156

TOTAL PROVIDED VOLUME 0 156

BASIN - Q-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2774	2722	0 062	0 000	0 000
2775	3568	0 082	0 072	0 072

TOTAL PROVIDED VOLUME 0 072

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
Q	Q-1	0 0819	0 072		
TOTAL		0 0819	0 072	0 07	0 002

BASIN - R-1				
STAGE	AREA		VOLUME	
	[ft <sup>2</sup> ]	[acres]	INC [acre-feet]	CUM [acre-feet]
2775	2030	0 047	0 000	0 000
2776	2639	0 061	0 054	0 054

TOTAL PROVIDED VOLUME 0 054

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
S	S-1	0 0606	0 054		
TOTAL		0 0606	0 054	0 02	0 034

**Disturbance Calculations:**

**Site Gross Area**  
 14,388,631 5541 Sq Ft  
 330 3175 Ac

**Basin Disturbance:**  
 8 5394 Ac

**Disturbance:**  
 2 59%

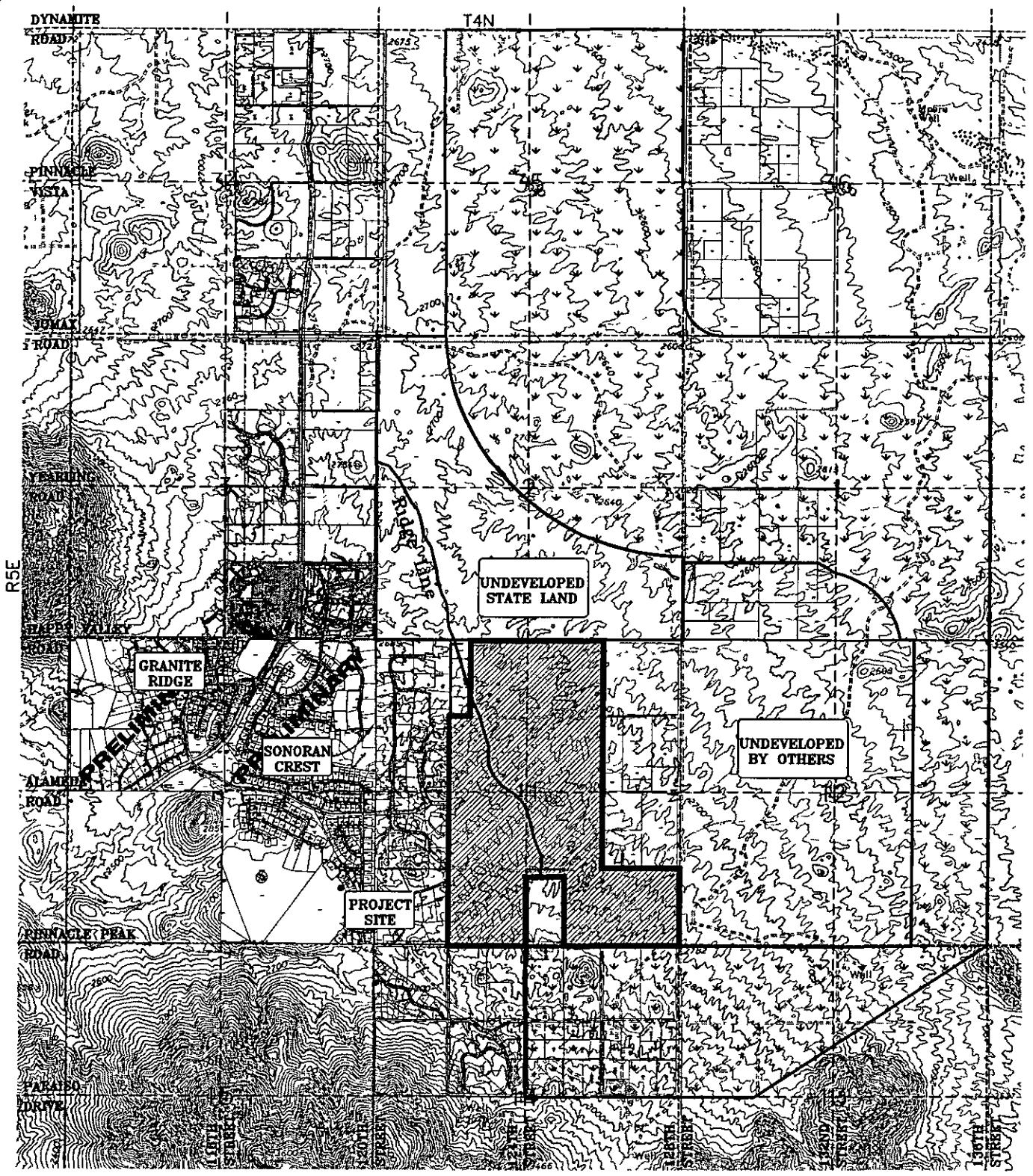
*Basin locations & volumes are conceptual & are subject to change with Final Improvement Plan Design*

**VOLUME BREAKDOWN TOTALS**

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr provided ac-ft	100yr-2hr Required ac-ft	Volume Shortage / Excess ±
A1	A1-1 - A1-2	0 2547	1 04	0 53	0 510
A2	A2-1 - A2-3	0 5544	1 01	1 01	0 00
B	B1 - B5	0 7491	1 68	1 54	0 143
C	C1 - C2	0 3044	0 79	0 51	0 281
D	D1 - D2	0 5131	1 21	1 09	0 123
E1	E1-1 - E1-4	0 8567	2 03	2 01	0 019
E2	E2-1 - E2-2	0 7007	1 93	1 87	0 059
E3	E3-1-E3-2	0 1762	0 34	0 32	0 025
F1	F1-1 - F1-4	0 6992	1 60	1 44	0 156
F2	F2-1 - F2-3	0 2978	0 78	0 5	0 278
F3	F3-1 - F3-2	0 1858	0 56	0 41	0 150
G	G1	0 2570	0 59	0 52	0 072
H1	H1-1 - H1-3	0 7827	1 94	1 82	0 123
H2	H2-1 - H2-2	0 5068	1 21	1 2	0 015
I	I1	0 2556	0 64	0 63	0 010
J	J1 - J2	0 2713	0 71	0 68	0 035
K	K1	0 1575	0 38	0 29	0 088
L	L1 - L2	0 1518	0 23	0 21	0 015
M	M1	0 1317	0 28	0 2	0 078
N	N1	0 1886	0 42	0 4	0 022
O	O1	0 1049	0 09	0 08	0 015
P	P1-P3	0 2967	0 27	0 24	0 028
Q	Q1	0 0819	0 07	0 07	0 002
R	R1	0 0606	0 05	0 02	0 034
<b>TOTAL</b>		<b>8 5394</b>	<b>19 87</b>	<b>17 59</b>	<b>2 284</b>

**PLATE 1**

**Vicinity Map**



UNDEVELOPED STATE LAND




UNDEVELOPED BY OTHERS

GRANITE RIDGE

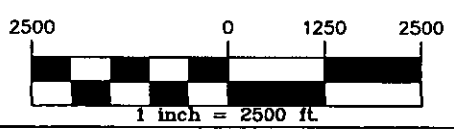
SONORAN CREST

PROJECT SITE

**LEGEND**

-  Project Boundry
-  Project Site
-  McDowell Sonoran Preserve

 Existing 5 Foot Contours



**CROWN**  
COMMUNITY DEVELOPMENT  
*A Honey Brown Company*

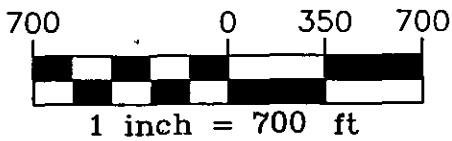
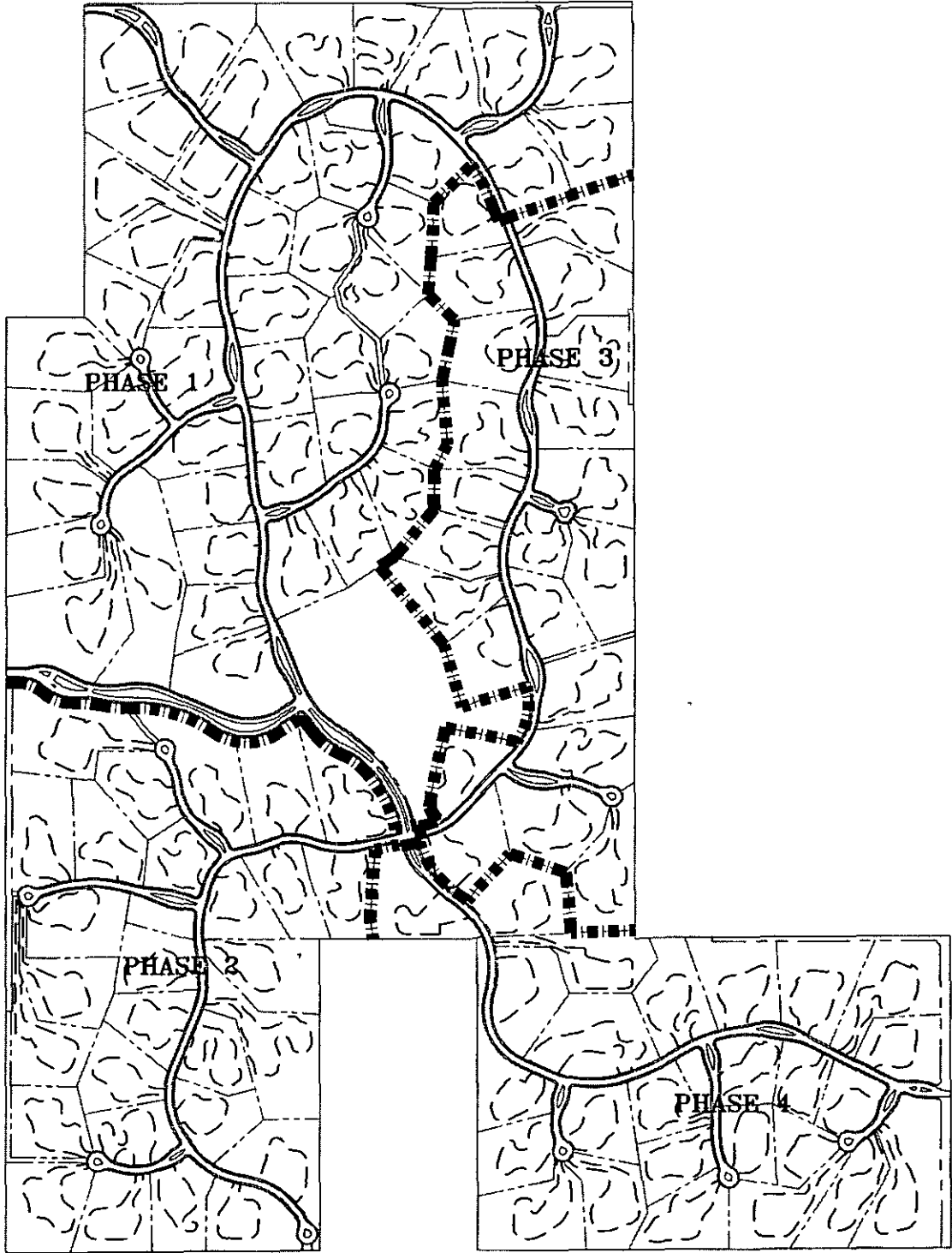
**SERENO CANYON**

Plate 1  
"Vicinity Map"

**WOOD/PATEL**  
LAND DEVELOPMENT • WATER RESOURCES  
TRANSPORTATION / TRAFFIC  
WATER / WASTEWATER • SURVEYING  
CONSTRUCTION MANAGEMENT  
(602) 335-8500  
PHOENIX • MESA • TUCSON

**PLATE 1A**

**Phasing Map**



**CROWN**  
 COMMUNITY DEVELOPMENT  
*A Hensley Brown Company*

**SERENO CANYON**

Plate 1A  
 "Phasing Map"

**WOOD/PATEL**

LAND DEVELOPMENT • WATER RESOURCES  
 TRANSPORTATION / TRAFFIC  
 WATER / WASTEWATER • SURVEYING  
 CONSTRUCTION MANAGEMENT

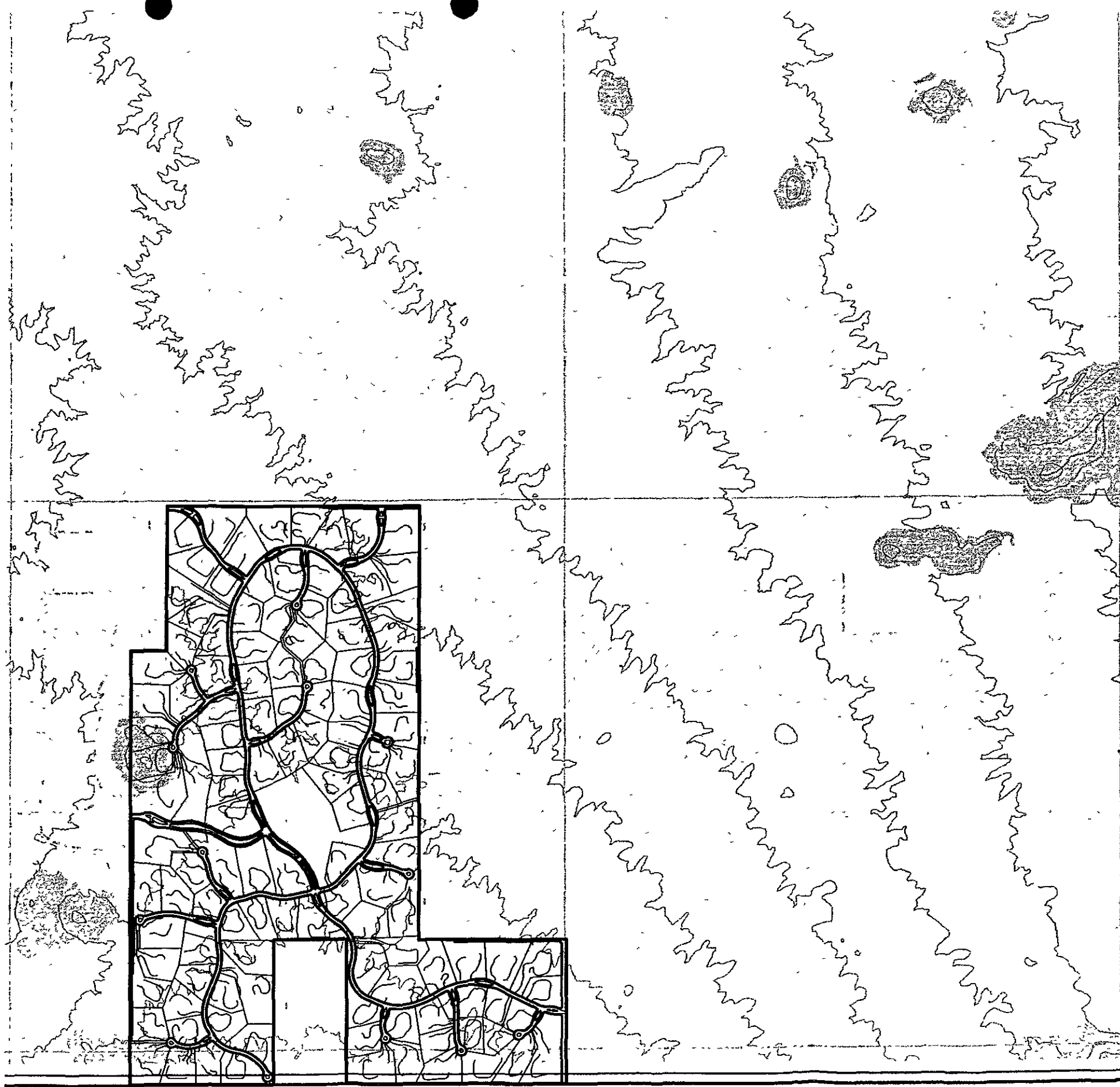
(602) 335-8500

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**PLATE 2**

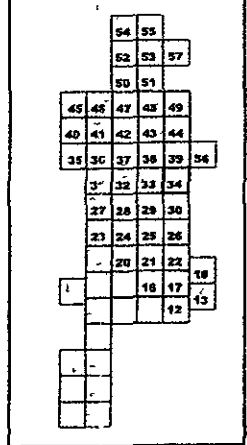
**ESL Classification**



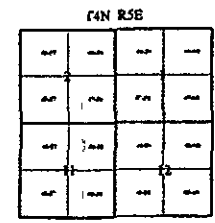
### ESL Landforms and Protected Peaks and Ridges

- City Limits
- Section Boundary
- Streets
- Landforms
  - Lower Desert
  - Upper Desert
  - Hillside
- Elevation
  - 50' Contour
  - 20' Contour
- Protected Peaks and Ridges
  - Peak or Ridge Line

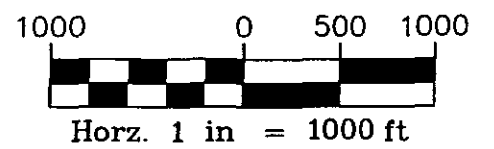
Area of Map Indicated in red




Official Detail Map  
**Map 34**



Data Source: City of Scottsdale



**PLATE 2**  
**ESL CLASSIFICATION**



**CROWN**  
COMMUNITY DEVELOPMENT  
*A Henry Crown Company*

**WOOD/PATEL**  
LAND DEVELOPMENT • WATER RESOURCES  
TRANSPORTATION/TRAFFIC  
WATER / WASTEWATER • SURVEYING  
CONSTRUCTION MANAGEMENT  
(602) 955-8800  
PHOENIX • MESA • TUCSON

**PLATE 3**

**Soils Classification Map**

UNDEVELOPED  
STATE LAND

UNDEVELOPED  
BY OTHERS

SONORAN  
CREST

61

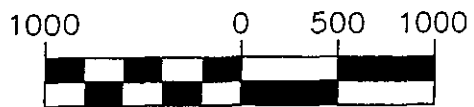
61 - GRAN-WICKENBURG COMPLEX, 1:  
TO 10 PERCENT SLOPES. SEE UNITED  
STATES DEPARTMENT OF  
AGRICULTURE'S SOIL SURVEY OF  
AGUILA-CAREFREE AREA, PARTS OF  
MARICOPA AND PINAL COUNTIES,  
ARIZONA BOOK.

121

63

UNDEVELOPED  
BY OTHERS

61



Horz 1 in = 1000 ft

**CROWN**  
COMMUNITY DEVELOPMENT  
*A Henry Crown Company*

### SERENO CANYON

Plate 3  
"Soils Classification"

### WOOD/PATEL

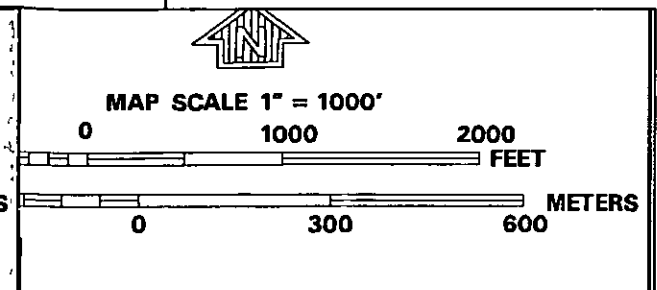
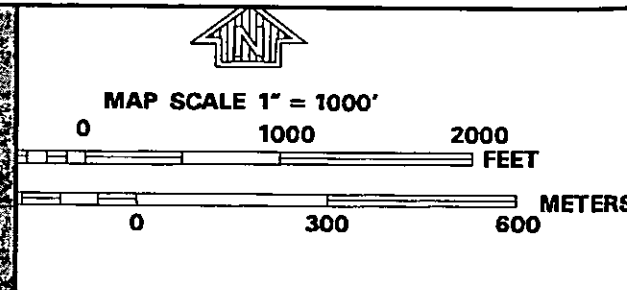
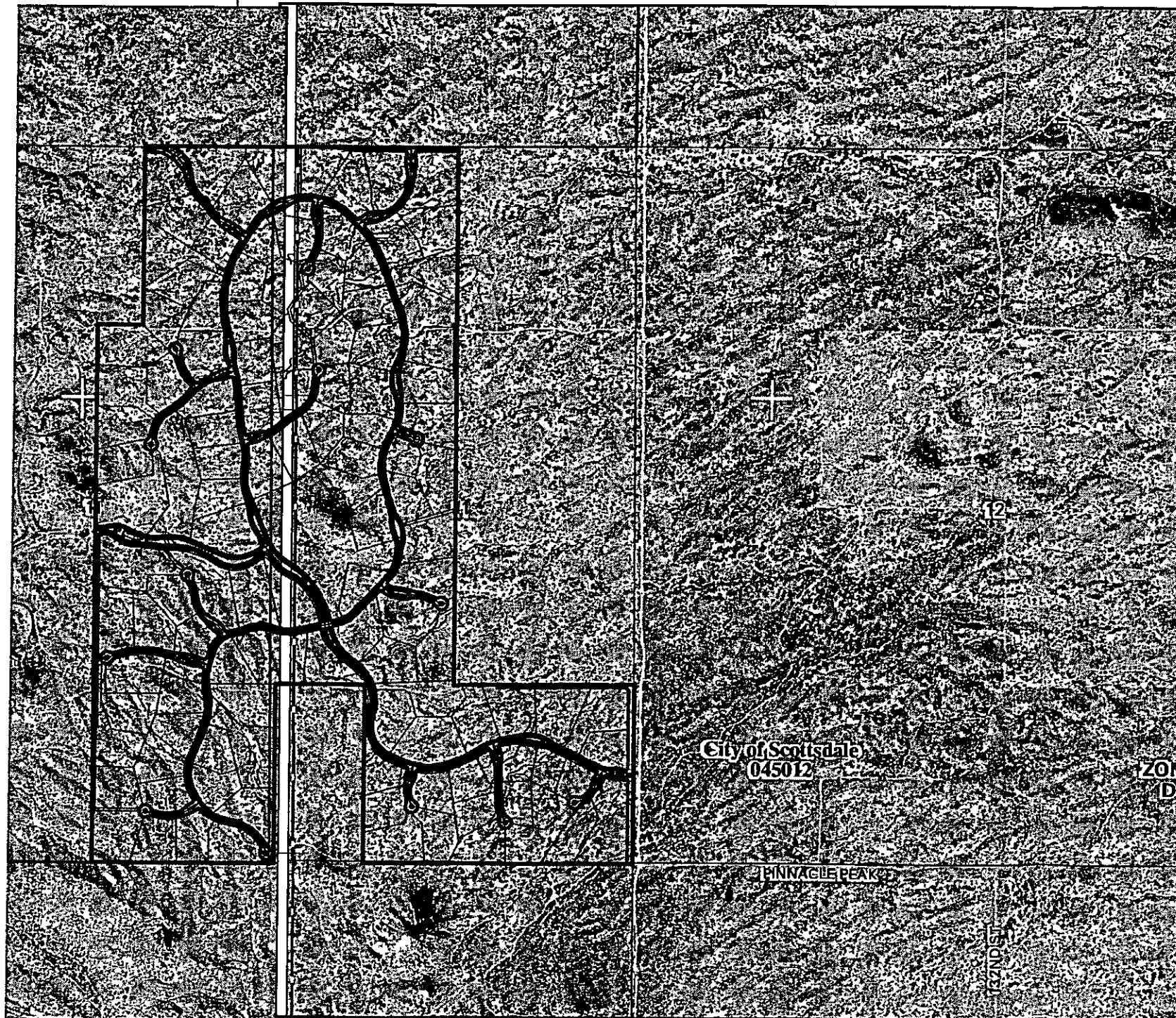
LAND DEVELOPMENT • WATER RESOURCES  
TRANSPORTATION/TRAFFIC  
WATER/WASTEWATER • SURVEYING  
CONSTRUCTION MANAGEMENT

(602) 335-8500

PHOENIX • MESA • GILSON

**PLATE 4**

**Flood Insurance Rate Map (FIRM)**



**NFP** PANEL 1260F

**FIRM  
FLOOD INSURANCE RATE MAP  
MARICOPA COUNTY,  
ARIZONA  
AND INCORPORATED AREAS**


**PANEL 1260 OF 4350**

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

**CONTAINS**

COMMUNITY	NUMBER	PANEL	SUFFIX
MARICOPA COUNTY	040032	1260	F
SCOTTSDALE, CITY OF	045012	1260	F

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.

 **MAP NUMBER  
04013C1260F  
MAP REVISED  
SEPTEMBER 30, 2005**  
Federal Emergency Management Agency

**NFP** PANEL 1255G

**FIRM  
FLOOD INSURANCE RATE MAP  
MARICOPA COUNTY,  
ARIZONA  
AND INCORPORATED AREAS**


**PANEL 1255 OF 4350**

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

**CONTAINS**

COMMUNITY	NUMBER	PANEL	SUFFIX
SCOTTSDALE, CITY OF	045012	1255	G

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.

 **MAP NUMBER  
04013C1255G  
MAP REVISED  
SEPTEMBER 30, 2005**  
Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at [www.msc.fema.gov](http://www.msc.fema.gov)

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at [www.msc.fema.gov](http://www.msc.fema.gov)

**PLATE 5**

**Color Topographic Aerial Photograph**

*(In with plans)*

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**PLATE 6**

**Off-Site Watershed Area Map**

*(with plans)*



**PLATE 7**

**404 Washes**

*(with plans)*

**PLATE 8**

**Pre-Development Drainage Site Plan**

*(with plans)*

**PLATE 9**

**Pre-Development Grading and Drainage Plan**

*(with plans)*

**PLATE 10**

**Post-Development Drainage Site Plan**

*(with plans)*

**PLATE 11**

**Post-Development Grading and Drainage Plan**

(with plans)