# CONCEPTUAL MASTER DRAINAGE REPORT FOR SERENO CANYON

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# TABLE OF CONTENTS

1.0	INTR	ODUCTION	1
2.0	GEN	ERAL LOCATION AND DESCRIPTION	2
•	2.1	•	
	2.2	Flood Insurance Rate Map (FIRM)	2
3.0	HYD	ROLOGY	
•	3.1	Methodology	3
	3.2	Drainage Sub-Basins	3
		3.2.1 Off-site Contributing Areas	3
	*	3.2.2 On-site Contributing Areas	3
	3.3	Soil Curve Numbers	
	3.4	Impervious Area	4
	3.5	Rainfall Event	4
	3.6	On-site Detention	
	3.7	Summary of Modeling Results	4
4.0	PROI	POSED DRAINAGE SYSTEMS	6
	4.1	Identification of Major Drainage Courses	
	4.2	404 Washes	6
	4.3	Drainage System Requirements	6
	4.4	Easement Requirements	6
	4.5	Roadway Crossing Requirements.	6
	4.6	Maintenance	7
5.0	HYD	RAULICS	8
	. 5.1	100-Year Floodplain Delineation	8
	5.2	Roadway Crossing Structures	8
	5.3	Scour Protection	10
	5.4	Annual Sediment Yield	10
		5.4.1 Bureau of Reclamation Sediment Surveys	11
		5.4.2 Recommended Annual Sediment Yield	11
	ų.	5.4.3 Short-/Long-Term Aggradation, Degradation, Local Scour, and Deposition	11
,	5.5	Protective Devices	12
6.0	CON	CLUSIONS	13
7.0	REFI	ERENCES	14
		·	

# **APPENDICES**

Appendix A	HEC-1 Input Parameters
Appendix B	Existing HEC-1 Model
Appendix C	Proposed HEC-1 Model
Appendix D	Hydraulics: Culvert Rating Curves for Stage - Storage Intervals
	HEC-RAS Output Files
Appendix E	Detention Basin Volume Calculation

# **PLATES**

Plate 1	Vicinity Map
Plate 1A	Phasing Map
Plate 2	Flood Insurance Rate Map (FIRM)
Plate 3	Soils Classification Map
Plate 4	404 Washes
Plate 5	Color Topographic Aerial Photograph
Plate 6	Off-Site Watershed Area Map
Plate 7	Pre-Development Drainage Site Plan
Plate 8	Pre-Development Grading and Drainage Plan
Plate 9	Post-Development Drainage Site Plan
Plate 10	Post-Development Grading and Drainage Plan

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#### 1.0 INTRODUCTION

This Conceptual Master Drainage Report has been prepared to meet the master drainage plan requirements, in accordance with the City of Scottsdale development criteria for the proposed Sereno Canyon Project. Sereno Canyon is a planned 330-acre custom lot residential subdivision located in the northeastern portion of Scottsdale, Arizona. This report presents the hydrological and hydraulic modeling and storm water storage requirements.

Sereno Canyon is located in Section 11, Township 4 North, Range 5 East. The site is currently an assemblage of undeveloped parcels bound to the west by the existing Sonoran Crest Development (122<sup>nd</sup> Street alignment), to the east by the 128<sup>th</sup> Street alignment, to the north by the Happy Valley Road alignment, and to the south by the McDowell Mountain Sonoran Preserve. Access to the development is planned from the west via the ½ -mile section roadway, Alameda Road. Plate 1 provides a Vicinity Map for the project and surrounding areas.

Sereno Canyon is a proposed custom lot sub-division, nestled at the northern base of the McDowell Mountains. Development of the project is planned to occur in four phases. Please refer to Plate 1A for a *Phasing Map* of the Project. The development includes approximately 122 lots ranging in size from 2 to 3 acres and a Clubhouse with amenities such as jacuzzis, pools, water falls, and restaurant facilities. Interpretive trails and scattered pocket parks with water features will also be incorporated into the site plan.

# 2.0 GENERAL LOCATION AND DESCRIPTION

#### 2.1 Site Features

The proposed project lies in the northern planning section of the City of Scottsdale. The site drains from the south to the north. Elevations range from 2,830 in the south to 2,675 feet in the northeast. Vegetation is typical Sonoran Desert type with creosote bush, jumping cholla, saguaro cacti, palo verde, ironwood and mesquite trees.

# 2.2 Flood Insurance Rate Map (FIRM)

The Flood Insurance Rate Maps (FIRM) for Maricopa County, Arizona and incorporated areas, Map Numbers 04013C1255F and 04013C1260E, dated July 19, 2001 indicates the site is within Zone "X" (shaded), and Zone "D".

Zone "X" (shaded) is defined by FEMA as follows:

Areas of 500-year flood; areas of 100-year flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 100-year flood.

Zone "D" is defined by FEMA as follows:

Areas in which flood hazards are undetermined.

The location of the parcel relative to the FIRM panels is illustrated on Plate 2 – Flood Insurance Rate Map (FIRM).

#### 3.0 HYDROLOGY

#### 3.1 Methodology

The U.S. Army Corps of Engineers' HEC-1 hydrologic model was utilized to compute the pre- and post-development 100-year, 6-hour storm runoff discharge rates. The preparation of input data necessary for the computer analysis included definition and measurement of the drainage sub-basins, assignment of soil curve numbers and calculations of the proposed impervious factors. The City of Scottsdale modeling procedures were followed as outlined in Chapter 4 of the City of Scottsdale Design Standards and Policies Manual (DSPM).

Provided below is a more detailed explanation of the methods utilized to compute the definition of drainage sub-basins, determination of soil curve numbers, impervious percentages for the study area and the selection of the design rainfall event.

#### 3.2 Drainage Sub-Basins

# 3.2.1 Off-site Contributing Areas

U.S.G.S. quad maps with 2-foot contour interval topography were utilized to define the drainage sub-basins for the off-site portions of the drainage areas. Please refer to Plate 6 - Off-Site Watershed Area Map for the limits of the drainage sub-basins and concentration points.

#### 3.2.2 On-site Contributing Areas

Digital topography with 1-foot contours was used to define the shape of the on-site drainage areas. Please refer to Plate 7 - *Pre-Development Drainage Site Plan* for the limits of the on-site sub-basins and concentration points.

#### 3.3 Soil Curve Numbers

Soil curve numbers for the HEC-1 models were calculated based on published guidelines and engineering experience for the type of soils present within the drainage sub-basins. Plate 3 presents the soils classification (typed) based on the SCS Map for the project area. Figure 4.6 "Runoff Curve Numbers for Arid and Semi-Arid Rangelands" located in the 2004 City of Scottsdale DSPM was used to determine the soil curve number.

The cover type (less than 30% vegetation cover due to wild fire) and hydrologic condition were estimated as desert shrub areas with poor hydrologic conditions. Runoff curve number of 88 was used for the corresponding soils group D that occurs within the watershed area. Wood/Patel is in agreement with the City's published curve numbers.

#### 3.4 Impervious Area

Impervious percentages were determined for each sub-basin for the developed condition HEC-1 model. Presently no development consists on the site. For the on-site developed conditions, impervious percentages were calculated utilizing the "Percent of Impervious Area vs. Dwelling Units/Acre" (Figure 2.2-16), as contained in Chapter 2 of the old *City of Scottsdale Drainage DSPM*. Based on the lot sizes, an impervious percentage of 10 percent was used for the developed areas.

#### 3.5 Rainfall Event

The precipitation amount for the 100-year, 6-hour return period was obtained from NOAA Atlas 2 Volume VIII maps located in the *City of Scottsdale Manual*. A copy of this map is included in Appendix A. The total calculated 100-year rainfall depth is 3.37 inches for the 100-year, 6-hour rainfall event.

#### 3.6 On-site Detention

According to the City's Drainage Ordinance, all runoff generated from the developed portion of the site must be managed and the peak discharge rate from the site reduced to at least pre-development values. With the development being sparse in nature at approximately 0.37 dwelling units per acre and no mass grading being proposed, post-development flows are only slightly higher than pre-development flows. On-line detention basins, located immediately upstream of culverted roadway crossings and at other strategic locations, are proposed to store the runoff associated with the 100-year, 2-hour storm event from disturbed areas and building envelopes. Please refer to Appendix E for the 100-year 2-hour detention volume calculations for all detention basins.

#### 3.7 Summary of Modeling Results

As previously discussed, the U.S. Army Corps of Engineers' HEC-1 computer analysis program was utilized to compute the peak storm water discharge rates for both existing and proposed conditions. Runoff for each drainage sub-basin was computed and then

routed, if required, through downstream drainage sub-basins where the hydrographs were then combined.

Table 3.1 provides the comparative peak discharge rates for the pre- vs. post-developed conditions at each concentration point. For the location of these concentration points and corresponding flow values, please refer to Plates 7 and 8 for existing conditions, and Plates 9 and 10 for developed conditions. As mentioned previously, the developed conditions HEC-1 model with on-line detention upstream of the culverted road crossings reduced the peak flows at or below the pre-development peak flows at all concentration points. The actual HEC-1 input data and result files for the existing and developed conditions are included in Appendices B and C, respectively.

Table 3.1: Flow Summary

100-year Flow								
Concentration Point	Existing Flow cfs	Developed Flow cfs						
A1	223	220						
A2	. 102	100						
В	181	181						
C	83	83						
D .	87	78						
E1	136	123						
E2	128	123						
COMBE	255	226						
E3	26	16						
F1	92	88						
F2	41	38						
F3	38	30						
G	41	38						
H1	144	144						
H2	161	143						
I	69	61						
J	81	60						
K	34	21						
L	32	25						
M	26	19						
N	33 .	17						
0	_10	10						
. P	24	21						
Q	12	9						
R	482	448						

#### 4.0 PROPOSED DRAINAGE SYSTEMS

# 4.1 Identification of Major Drainage Courses

There are no washes on the site with an anticipated 100-year flow equal to or greater than 750 cfs, therefore no washes are categorized as a Vista Corridor. The major water courses that traverse through the project have been identified as washes with a 100-year flow greater than 50 cfs. These washes will be maintained in their natural location and will not be re-aligned.

#### 4.2 404 Washes

A preliminary investigation has been done on the major washes within the project to identify the washes that may be deemed jurisdictional. A request for 404 Jurisdictional Delineation Verification has been submitted to the Army Corp or Engineers for review and approval. Plate 4 - 404 Washes provides the preliminary 404 wash locations in reference to the aerial photograph for the site. The preliminary 404 washes are also illustrated on the Pre- and Post-Development Grading and Drainage Plans.

# 4.3 Drainage System Requirements

The existing drainage patterns will be maintained in their natural location and condition where possible. The site is being developed as large custom lots. Therefore, as lots are developed individual lot engineers will be required to provide drainage documentation to substantiate the development of the lot.

# 4.4 Easement Requirements

Where flows from the 100-year storm event are greater than 50 cfs, drainage easements have been provided. In addition, drainage easements shall be dedicated to the limits of inundation for the 100-year, 2-hour storm event in each detention basin.

#### 4.5 Roadway Crossing Requirements

In all cases the depth of flow over streets is in accordance with City of Scottsdale Flood Plain and Drainage Ordinance.

6

# 4.6 Maintenance

Ongoing maintenance of the designed or recommended drainage systems is required to preserve the design integrity and purpose of the drainage system. Failure to provide maintenance can prevent the drainage system from performing to its intended design purpose and can result in reduced performance. It is the responsibility of private developers, homeowner associations, etc. for facilities on private property, within all drainage easements, private streets, and right-of-ways unless accepted by the City for maintenance. A regular maintenance program is required to have drainage systems perform to the level of protection or service as presented in this report and the project's plans and specifications.

#### 5.0 HYDRAULICS

# 5.1 100-Year Floodplain Delineation

The U.S. Army Corps of Engineers' HEC-RAS Version 3.1.2 was used to generate the water surface profiles for washes with 100-year flows in excess of 50 cfs. Manning's 'N' values were assigned based on field observation and the review of aerial photography. The majority of the washes contain the 100-year flows within incised channels. Accordingly, an 'N' value of 0.032 was utilized for the incised portion of the wash and 0.045 was used for the over bank condition. The starting water surface elevations were determined using slope area method. A delineation of the 100-year water surface elevation for these washes that experience pre-development peak flows has been drawn based on results from the HEC-RAS analysis. Water surface elevations and top widths during pre-development flows for each cross section are included on Plate 8 - Pre-Development Grading and Drainage Plan.

The 100-year floodplain delineation for these washes that experience post-development peak flows has been revised to reflect the limits of ponding that occurs on the upstream side of the culverted road crossings. At this point, the location of the roadways has not been finalized and therefore a post-development HEC-RAS model that includes cross sections representative of the culverts has not been prepared. It is anticipated that the 100-year floodplain delineation for post-development conditions will be approximately the same delineation as the pre-development delineation with the exception of the widened areas at ponding locations. Water surface elevations and top widths for each cross section are included on Plate 10 – Post-Development Grading and Drainage Plan.

Cross-sections were placed such that significant variations in the channel cross-sectional geometry are adequately represented. Due to the steep slopes on the project site, it is not feasible to put enough cross-sections such that the difference in energy grade elevations is less than one (1) foot.

#### 5.2 Roadway Crossing Structures

Roadway crossing structures were designed to convey the anticipated 100-year flows. Fourteen (14) culvert crossings have been designed and are identified on Plate 10. The approximate 100-year backwater limits (ponding) have been incorporated with the proposed floodplain delineations shown on the post-development Plates 9 and 10. The supporting hydraulic calculations are included in Appendix D.

These culverts are designed to provide storm water storage on the upstream side of the culvert. The difference between the inflow and outflow rates at the culverts is identified in Table 5.2-1 on the following page. Please refer to Plate 10 for the location of the culverts according to their ID.

Rip rap will be utilized within the channels to dissipate velocities on the upstream and downstream sides of the proposed culverts. The actual design of rip rap sizing will be based on approved practices and will be completed at the time of improvement plan submittal.

Table 5.2-1:
Culvert Inflow vs. Outflow

Basin	Culvert ID	Description	Inflow (cfs)	Outflow (cfs)	Inflow - Outflow (cfs)
M	1	(1) 30" Pipe	27	19	8
F3	2	(1) 30" Pipe	39	30	9
F2	3	(1) 30" Pipe	43	38	5
F1	4	(2) 48" Pipes	96	88	. 8
E2	5	(2) 42" Pipes	128	123	. 5
EI	6	(2) 36" Pipes	124	123	1.
El	. 7	(2) 36" Pipes	144	124	- 20
E3	8	(1) 24" Pipe	27	16	11
D	. 9	(2) 36" Pipes	80	78	2
D	10	(2) 36" Pipes	93	80	13
С	11	(2) 30" Pipes	86	84	. 2
В	. 12	(2) 42" Pipes	187	181	6 .
A2	13	(2) 36" Pipes	107	100	7
I	14	(2) 54" Pipes	72	61	11
H2	15	(3) 54" Pipes	165	143	22
H1	16	(2) 54" Pipes	149	144	5
G1	17	(1) 24" Pipe	38	38	0

#### 5.3 Scour Protection

The need for scour protection will be identified as part of the Final Drainage Report. Because of the relatively small flow values and incised nature of the sites washes, development setbacks would be utilized rather than bank protection schemes, if required.

#### 5.4 Annual Sediment Yield

Sediment yield is defined as that portion of eroded material that travels through a drainage system to a downstream measuring or control point (*Bureau of Reclamation 1987*). The units used for this study are acre-feet/square mile/year (AF/sq.mi./yr.). The prediction of sediment yields is not an exact science. Many complex variables influence the erosion process and any sediment yields analysis requires considerable engineering judgment. For this reason, two (2) methods were used to arrive at a conservative estimate of the average annual sediment yield rate. The following sections provided a detailed discussion of each method and its results.

Sediment yield was calculated using the Pacific Southwest Inter-Agency Committee (PSIAC) method. This method was obtained from the following source: Pacific Southwest Inter-Agency Committee, Report of the Water Management Subcommittee on Factors Affecting Sediment Yield in the Pacific Southwest Area and Selection and Evaluation of Measures for Reduction of Erosion and Sediment Yield, October, 1968.

Nine (9) factors are evaluated in determining the sediment yield classification. These are geology, soils, climate, runoff, topography, ground cover, land use, upland erosion, and channel erosion and sediment transport. Characteristics of each of the nine factors, which give that factor high, moderate, or low sediment yield, are evaluated and the factor is assigned a numerical value representing its significance in the yield rating. The yield rating is the sum of values for the appropriate characteristics for each of the nine factors. For the purpose of avoiding complexity, the factors are generally described as independently influencing the amount of sediment yield.

Classification	Rating	Sediment Yield acre-ft/sq. mile
1	> 100	3.0
2	75 – 100	1.0 - 3.0
3	50 – 75	0.5 – 1.0
4	25 - 50	0.2 - 0.5
5	0 - 25	< 0.2

Application of the nine sediment yield factors to the watershed discussed in this report generated a total numerical rating of 77 to 85, the actual rating being dependant upon the interpretation of the guidelines that are used to assign rating values to each factor. Regardless of the actual number, i.e. 77 or 85, both values fall within Classification 2 (75 -100), which indicates a sediment yield range of 1.0 to 3.0 AF/sq.mi./yr.

# 5.4.1 Bureau of Reclamation Sediment Surveys

The 1987 edition of *Design of Small Dams* (BuRec) publishes data on sediment measurements from 28 reservoirs in semi-arid regions of the United States. A regression line through these data points produces the following equation:

$$Q_s = 1.84 A^{-0.24}$$

Where:

 $Q_s$  = annual sediment yield (AF/sq.mi./yr.)

A = drainage area (sq.mi.)

Using a drainage area size of 1.629 sq.mi. (with Reata Pass improvements), this equation predicts an annual sediment yield of 1.67 AF/sq.mi./yr. or 4,390 cubic yards.

#### 5.4.2 Recommended Annual Sediment Yield

In consideration of the complex processes under investigation, the two (2) methods used exhibit fairly good agreement. Based on comparison of sediment yield calculations to actual sediment yield measurements, Renard & Stone (1981) concluded that "the PSIAC method appears to give the best results for the amount of work required to make the estimate." Based on the results of the sediment yield equations, a design value of 1.7 AF/sq.mi./yr. was selected for use in Planning Units II and IV.

#### 5.4.3 Short-/Long-Term Aggradation, Degradation, Local Scour, and Deposition

Sediment transport analyses need to distinguish between short-term and long-term changes. Short-term changes are event specific and occur to some extent during each flood hydrograph. Examples of short-term changes would be local scour, general scour, bend scour, bedform troughs, and to some extent low-flow channel incisement. With the exception of low-flow channel incisement, any

visual signs of these processes may be difficult to detect after the flow has subsided. Short-term scour processes can usually be quantified with empirical and/or theoretical relationships.

Vertical incisement of the channel bed can occur in response to the following six (6) processes:

- 1. Long-term degradation,
- 2. Local scour,
- 3. General scour.
- 4. Bend scour,
- 5. Low-flow channel incisement, and
- 6. Antidune troughs.

Precise prediction of long-term channel impacts can be much more elusive than their short-term counterparts because of the time span involved and the numerous variables that impact long-term changes. The major drainage corridors within Planning Units II and IV were designed as protected channels.

#### 5.5 Protective Devices

Roadway crossing pipes will incorporate riprap at both inlet and outlet to dissipate energy and provide flow line scour protection. Detention basins upstream of roadways will incorporate a protected overflow area (using native riprap and filter fabric) in the event of overtopping. The bleedoff pipes for other detention basins will also incorporate riprap protection.

#### 6.0 CONCLUSIONS

- 1. The project site located within FEMA Zone "X" (shaded), and Zone "D" designated flood zones as shown on Plate 2.
- 2. Drainage corridors have been designated for the identified washes in accordance with the appropriate City of Scottsdale ordinance requirements.
- 3. The differences of the peak flow rates for the pre-versus post-development conditions for the 100-year, 6-hour storm event is negligible in instances where the post-development flows have increased over the pre-development conditions.
- 4. It is being proposed that in lieu of providing 100-year, 2-hour detention, online detention on the upstream side of the road culvert crossings be provided to reduce post-development flows to at or below pre-development levels. A storm water storage waiver has been submitted to the City of Scottsdale.
- 5. The design of hydraulic structures are to be based on generally accepted engineering practices and in accordance with City of Scottsdale requirements.
- 6. On-going maintenance is required for all drainage systems in order to assure design performance.
- 7. All finished floor elevations are to be designed to be above the 100-year water surface elevation.

# 7.0 REFERENCES

- 1. City of Scottsdale, Design Standards and Policies Manual Chapter 2 Drainage, December, 1999.
- 2. Flood Control District of Maricopa County, *Drainage Design Manual for Maricopa County, Arizona: Volume I.– Hydrology*, revised January 1995.
- 3. Flood Control District of Maricopa County, *Drainage Design Manual for Maricopa County*, *Arizona: Volume II Hydraulics*, January 28, 1996.
- 4. U.S. Army Corps of Engineers, HEC-1, Flood Hydrograph Package, June 1998.
- 5. U.S. Army Corps of Engineers, HEC-RAS, Version 3.1.2, April 2004.

APPENDIX A

**HEC-1 Input Parameters** 

# TABLE 15.--SOIL AND WATER FEATURES--Continued

		F	looding		B€	drock	Ce	emented	Risk of	corrosion
Soll name and map symbol	Hydro-  logic  group	Frequency	Duration	Months	Depth	Hardness	Depth	pan Hardness	Uncoated steel	Concreté
,					<u>In</u>		<u>In</u>			
58*,59*: Denure	В	None		 	>60				High	Low.
60*: Glenbar Noncalcareous	B	Rare	<del></del>		>60		   	<b>_</b>	High	Low.
Glenbar Calcareous	В	Rare	<del></del>		>60			 	High	Low.
61*, 62*: Gran	p	None			20-40	Hard		) 	  High	Low.
Wickenburg	Đ	None	<del></del>		   3-20!	Soft			High	Low.
63*, 64*: Gran	D ,	None	·		20-40	Hard			High	Low.
Wickenburg	p	None	,		3-20	Soft			High	Low.
Rock outcrop.				}	•					
65*:			_	}	,					
Greyeagle	Þ	None	- <del></del>		>60 		4-20	Thick	High	
Continental	С	None			>60			- <b></b>	High	÷
Nickel	В	None			>60	·		<del></del>	H1gh	Low.
66*: Greyeagle	D	None	<del></del>		>60		4-20	Th1ck	High	Low.
Suncity Variant	D	None			>60		5-20	Thick	High	Low.
67Guest	D	Rare			>60				High	Low.
68*, 69*: Gunsight	В	None			>60				High	Low.
C1priano	D	None			>60		5-20	Thick	High	Low.
70*, 71*: Gunsight	В	   None	- <del></del>		>60				High	Low.
Rillito	В	Non e	·	}	>60				High	Low.
72*, 73*: Lehmans	D	None			6-20	Hard			High	Low.
Rock outerop.	}	, !		)			}			
74*: Luke	C .	None	 		>60		20-40	Thick	  H1gh	Low.
Cipriano	, D	None			>60		4-20	Thick	High	Low.
75, 76, 77, 78, 79 Mohall	В	None			>60	<del></del>			H1gh	Low.
80*, 81*: Mohall	В	None			>60				High	Low.

See footnote at end of table.

# Flood Control District of Maricopa County

# Rainfall Data

Page 1 12/14/2004

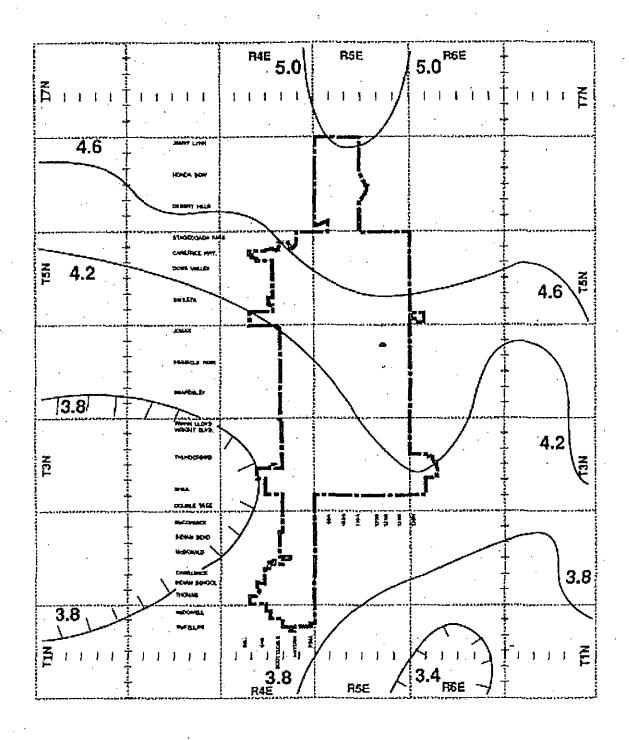
Elevation: .

0

Primary Zone Number: 7 Latitude: 0.0

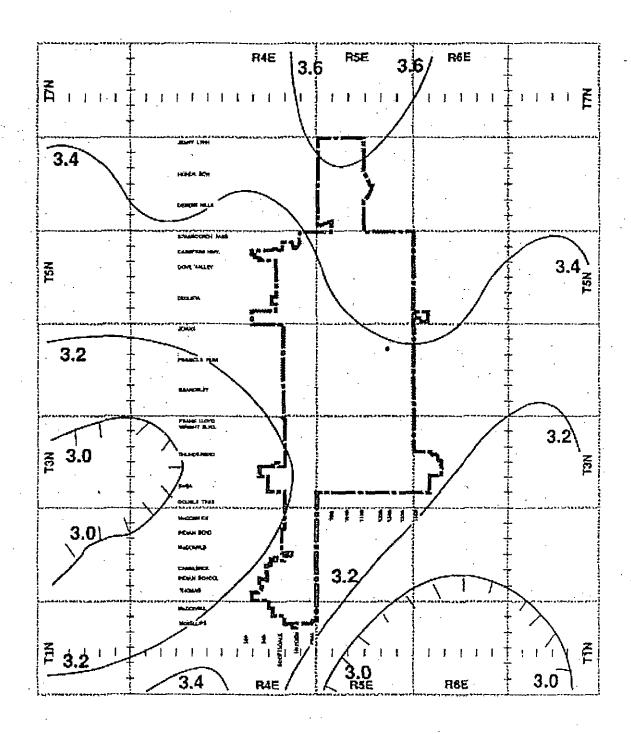
Short Duration Zone Number: 8 Longitude: 0.0

	•			Point Va	alues (in)			
,	Duration	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	
	5 MIN	0.38	0.46	0.52	0.60	0.66	0.73	
	·, 10 MIN	0.57	0.70	0.78	0.91	1.01	1,11	
	15 MIN	0.70	0.87	. 0.99	1.16 -	1.29	1.43	
	30 MIN	0.92	1.16	1.33	1.56	1.75	1.94	
	· 1 HOUR	1.12	1.43	1.64	1.95	2.19	2.42	
	2 HOUR	1.27	1.62	1.87	2.21	2.48	2.74	
	3 HOUR	1.38	1.75	2.01	2.38	2.67	2.96	
	6 HOUR	1.57	2.00	2.29	2.71	3.04	3.37	
•	12 HOUR	1.81	2.31	2.66	3.15	3.53	3.91	
	24 HOUR	2.05 -	2.62	3.02	3.58	4.02	4.45	



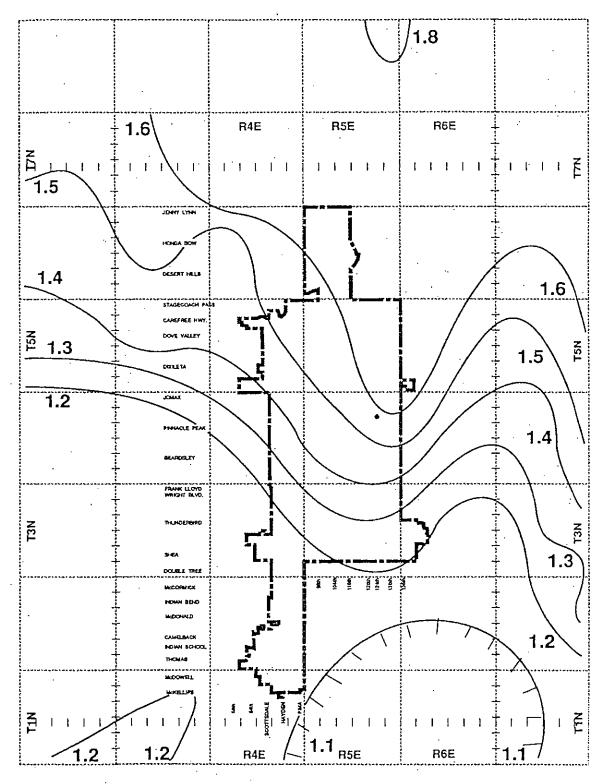
# **FIGURE 2.2-12**

Isopluvials 100 Year 24 Hour Precipitation in Inches
Rainfall Data From NOAA Atlas 2, Vol. VIII



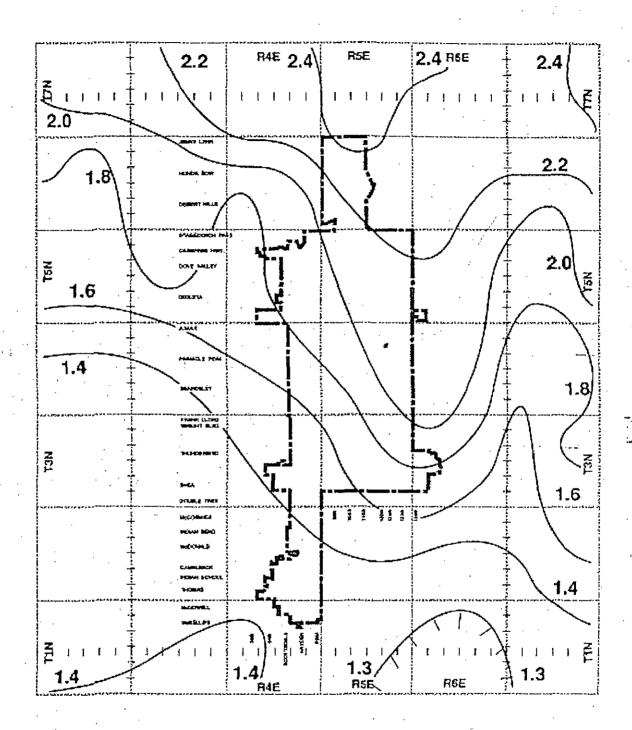
# **FIGURE 2.2-6**

Isopluvials 100 Year 6 Hour Precipitation in Inches
Rainfall Data From NOAA Altas 2, Vol. VIII



**FIGURE 2.2-1** 

Isopluvials 2 Year 6 Hour Precipitation in Inches Rainfall Data From NOAA Atlas 2, Vol. VIII



# **FIGURE 2.2-7**

Isopluvials 2 Year 24 Hour Precipitation in Inches Rainfall Data From NOAA Allas 2, Vol. VIII

# Runoff Curve Numbers for Urban Areas<sup>1</sup>

Open space (lawns, parks, golf courses, cemeteries, etc.)  Poor condition (grass cover less that 50%)  Fair condition (grass cover 50-75%)  Good condition (grass cover greater than 75%)  Impervious areas:  Paved parking lots, roads, driveways, etc.  (excld. right-of-way)  Sieels and roads:  Paved; curbs and storm sewer (excld. right-of-way)  Paved; open dilibes (including right-of-way)  Oirt (including right-of-way)  Dirt (including right-of-way)  Vestern desert landscaping (pervious areas only)  Antificial desert landscaping (impervious weed barrier, desert shrub with 1 to 2-inch sand or gravel mulch and basin borders)  Irban districts:  Commercial and business  Industrial  Townhouse, duplexes  Multi-Family  tesidential districts by average lot size: (See Figure 2.2-16)	Average % Impervious Area <sup>2</sup>	- hydrol			
,	711144	A	В	С	D
Fully developed urban areas with vegetation established					
Open space (lawns, parks, golf courses, cemeteries, etc.)3:					
Poor condition (grass cover less that 50%)		88	79		
Fair condition (grass cover 50-75%)		49	69		
Good condition (grass cover greater than 75%)		39	61	74	80
Impervious areas:					
Paved parking lots, roads, driveways, etc.					
(excid. right-ol-way) ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		98	98	98	98
		98	98	98	98
	•	<b>53</b>			93
		76			
Dirt (including right-oi-way)		72	82		89
Western desert urban areas: Natural desert landscaping (pervious areas only) <sup>4</sup>		63	77	85	88
basin borders)		98	96	96	98
Urban districts.					
Commercial and business ,	85			•	
Industrial	85	Mod	Ann	icabi	e in
Townhouse, duplexes	65			scale	
Multi-Family	85		~~		
Residential districts by average lot size: (See Figure 2.2-16)		-			
Developing Urban Areas					
Newly graded areas					
		77	86		. 94

cover type.

# **FIGURE 2.2-19**

Runoff Curve Numbers for Urban Areas<sup>1</sup>

<sup>&</sup>lt;sup>1</sup>Average runoff condition, and i<sub>8</sub> = 0.25; Table 2-2a, 210-VI-TR55, Second Ed., June 1986.

<sup>2</sup>The average percent impervious area shown was used to develop the compatie CN's. Other assumptions are as follows: imparvious areas are directly connected to the drainage system, impervious areas have a CN of 98, and pervious areas are considered equivalent to open space in good hydrotogic condition (not applicable in Scottadale).

<sup>3</sup>CN's shown are equalvalent to those of pasture. Composite CN's may be computed for other combinations of open space.

<sup>\*</sup>Composite CN's for natural desert landscaping should be computed based on the impervious area percentage (CN = 98) and the pervious area CN. The pervious area CN's are assumed equivalent to desert shrub in poor hydrologic condition. 5Composite CNPs to use for the design of temporary measures during grading and construction should be computed based on the degree of development (Impervious area percentage) and the CN's for the newly graded pervious areas.

Runoff Curve Numbers for Arid and Semiarid Rangelands1

Cover type and hydrologic condition	Hydrologic				s lor group
	Condition2	A	В	C	D
Herbaceous - mixture of grass, weeds, and	Poor		80	87	93
low-growing brush, with brush the minor element.	Fair Good		71 62	81 74	89 85
,		•			
Oak-aspen - mountain brush mixture of oak brush,	Poor Fair		65 48	74 57	79 63
aspen, mountain mahogany, bilter brush, maple, and other brush.	Good		30	41	48
Panyon-juniper - pinyon, juniper, or both:	Poor		75	85	89
grass understory.	Fair		58		80
	Good		41	51	71
Sagebrush with grass understory.	Poor	-	67	80	65
	_Fair	•	51	63	70
•	Good		35	47	55
Desert strub - major plants include satibush.	Poor	63	77	85	88
greasewood, creosotebush, blackbrush, bursage,	Fair	55	72	81	88
palo verde, mequite, and cactus.	Good	49	68	79	84

**FIGURE 2.2-20** 

Runoff Curve Numbers for Arid and Semiarid Rangelands<sup>1</sup>

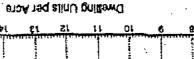
TAverage runoit condition, and I₂ = 0.2S; Table 2-2d, 210-VI-TRSS, Second Ed., June 1988. ₹Poor: <30% ground cover (filter, grass, and brush overatory).

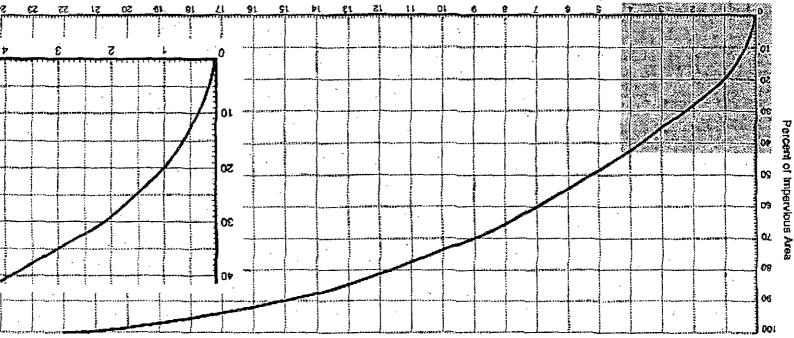
Fair: 30 to 70% ground cover (not applicable in Scottsdale).

Good: >70% ground cover (not applicable in Scottsdale).

3Curve Numbers for group A have been developed only for desert shrub.

# FIGURE 2.2-16





APPENDIX B

Existing HEC-1 Model

EX-TEST.OUT

FLOOD HYDROGRAPH PACKAGE (HEC-1) JUN 1998 VERSIGN 4.1

RUN DATE 16DEC05 TIME 09:24:56

U.S. ARMY CORPS OF ENGINEERS HYDROLOGIC ENGINEERING CENTER 609 SECOND STREET DAVIS, CALIFORNIA 95616 (916) 756-1104

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WAITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESTRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

	•				,		•							
						HEC-1	INPUT			-			PAGE	1
LINE		ID	1	2	3	4	5	6	7	8	9	.10		
1 2 3 4 5 6 7 8 9 10	. ·	ID I	100 RA SC KI EX: PR: FI	C-1 MODEL 0-YEAR, ( INFALL FI S CURVE I NEMATIC V ISTING CO EPARED BY LE NAME:	5-HOUR S ROM NOAA NUMBER S VAVE HYD ONDITION / WOOD/P	TORM ATLAS OIL LOSS PROGRAPH IS ATEL, 8.	PARAME ROUTING	BACKBOWL TERS						
11 12 13 14		IO JD PH	5	.01	.73	1.43	2.42	2.74	2.96	3.37				
15 16		KK KM *	A1 RUN	OFF FROM	SUB-BAS	IN Al								
17 18 19 20	•	BA LS UK RK	.084 260 3520	88 .025 .023	.15	106	TRAP	. <i>.</i> 15	5					
21 22 23 24 25 26	-	KK KM BA LS UK RK	A2 RUN .036 235 2000	0FF FROM 88 .025 .02	.15 .032	100	TRAP	15	5					
27 28 29 30 31 32		KK KM BA LS UK RK	B RUN . 077 400 2420	0FF FROM 88 .025 .024	SUB-8AS	100	TRAP	15	5					
33 34 35 36 37 38		KK KM . BA LS UK RK	c	0FF FROM 88 .023 .017	·	IN C	TRAP	15	. 5		•	,	,	
39 40 41		KK KM BA	Ð	OFF FROM	SUB-BAS	SIN D								
42 43 44		LS UK RK	300 1425	. 88 .02 .014	.032	100 HEC-1	TRAP INPUT	15	5				· . PAGE	5
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51 52 53 54 55		KK KM BA LS UK	E2 RUN .046 200	OFF FROM 88 .02	SUB-BAS	5IN E2 100	Pa	je 1					•	

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60 61 62	KK KM HC *	CLEAR CLEAR 6	HYDROGRAP	PH STACK							
63 64 65 66 67	KK KM BA LS	. E3 RUNOFF .008	FROM SUB-	BASIN E3							
67 68	. UK RK	175 750	025 .1 032 .03	15 100 32	TRAP	15	5				
69 70 71 72 73	KK KM BA	F1 RUNOFF .036	FROM SUB-	BASIN F1	•						
73 74	LS UK RK *	225 .0 2500 .0	88 018 .1 012 .03	15 100 32	TRAP	15	5				
75 76 77 78	KK KM BA	F2 RUNGFF .014	FROM SUB-	-BASIN FZ \		*					
78 79 80	LS UK RK *	200 .	88 018 .1 028 .03	15 100 32	TRAP	15	5				
81 82 83	KK KM BA	F3 RUNOFF .013	FROM SUB-	-BASIN F3							
84 85 86	L5 UK RK	. 225 .	88 018 .1 026 .0	LS 100 32	TRAP	15	5				
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90 91 92 -	LS UK RK	400 720	88 025 .1 022 .03	15 100 32	TRAP	15 .	5				
93 94 95	KK KM BA	H1 RUNOFF .059.	FROM SUB-	-BASIN H1							
96 97 98	LS UK RK	375 .	88 025 019 0	15 100 32	TRAP	15	S				
99 100 101	KK KM BA	H2 . RUNOFF .072	FROM SUB-	-BASIN HZ							
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137		JK RK	750	.012	.15 .032	100	TRAP	15	5		
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					UMMARY ET PER SECOND N IN SQUARE M				
OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FI 6-HOUR	LOW FOR MAXIM	10M PERIOD 72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
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HYDROGRAPH AT	A2	102.	3.17	8.	. 2.	1.	.04		
HYDROGRAPH AT	В	181.	3.20	18.	4.	2.	.08		
HYDROGRAPH AT	. с	83.	3.17	7.	2.	1.	.03		r
HYDROGRAPH AT	Đ	87.	3.17	8.	2.	1.	.03		
HYDROGRAPH AT	E1	136.	3.20	14.	4	1.	.06		
HYDROGRAPH AT	, . E2	128.	3.17	11.	3.	1.	.05	٠	•
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6 COMBINED AT	CLEAR '		3.17,	84.	21.	8.	. 37		_
HYDROGRAPH AT	E3	26.	3.10	2.	0.	0.	.01		
HYDROGRAPH AT	F1	92.	3.20	8.	2.	. 1.	. 04		•
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HYDROGRAPH AT	F3	38.	3.13	3.	· 1.	0.	.01		
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HYDROGRAPH AT	. н1	144.	•	14.	3.	1.	.06		
HYDROGRAPH AT	н2		3.23	16.	. 4.	1.	. 07		
HYDROGRAPH AT	1	69.	3.17	6.	1.	1.	.03		
8 COMBINED AT	CLEAR	581.	3.17	56.	14	5.	. 24	•	
HYDROGRAPH AT	. ]	81.	3.10	6.	1.	. 0.	.02		
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8 COMBINED AT				2.	0.			-	•
HYDROGRAPH AT	CLEAR	758.	3.17	72.	18.	7.	.31		

HYDROGRAPH AT

482 3.10 SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW) DT . ISTAO ELEMENT VOLUME DΤ PEAK VOLUME (CFS) (MIN) (MIN) (CFS) (MIN) (IN) (MIN) (III) FOR STORM = 1 STORM AREA (SQ MI) = 01 A1 MANE 1.92 225.80 190.40 2.14 2.00 223.31 190.00 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9633E+01 OUTFLOW= .9599E+01 BASIN STORAGE= .3600E-03 PERCENT ERROR= FOR STORM =  $1^{\circ}$  STORM AREA (SQ MI) = .01 A2 MANE 1.43 104.13 · 188.54 2.15 102.13 2.00 190.00 CONTINUITY SUMMARY (AC-FT) - INFLOW: .0000E+00 EXCESS: .4129E+01 OUTFLOW: .4119E+01 BASIN STORAGE: .1150E-03 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = B MANE 1.39 182.80 .01 191.08 2.14 2.00 180.83 192.00 2.14 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .8830E+01 OUTFLOW= .8782E+01 BASIN STORAGE= .5870E-03 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = C MANE 1.28 84.68 .01 188.46 2.15 190.00 2.00 83.44 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3440E+01 OUTFLOW= .3432E+01 BASIN STORAGE= .1204E-03 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = 0 MANE 1.30 87.59 .01 190.71 87.45 2.14 2.00 190,00 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3899E+01 OUTFLOW= .3885E+01 BASIN STORAGE= .1913E-03 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = 136.49 .01 193.57 2.14 135.65 2.00 192.00 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6996E+01 OUTFLOW= .6955E+01 BASIN STORAGE= .5465E-03 PERCENT ERROR FOR STORM = 1 STORM AREA (SQ MI) \* E2 MANE 1.80 129.68 .01 189.33 2.14 2.00 127.91 190.00 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5275E+01 OUTFLOW= .5262E+01 BASIN STORAGE= .1409E-03 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = E3 MANE .76 26.38 .01 186.17 2.15 2.00 26.25 186.00 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9174E+00 OUTFLOW= .9157E+00 BASIN STORAGE= .1670E-04 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = 2.00 93.25 .01 191.67 2.14 2.00 92.18 192.00 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4129E+01 OUTFLOW= .4112E+01 BASIN STORAGE= .1467E-03 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = 1.28 41.26 .01 188.54 2.14 40.94 2.00 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1606E+01 OUTFLOW= .1601E+01 BASIN STORAGE: .4319E-04 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = .76 37.65 .01 188.33 2.15 2.00 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1491E+01 OUTFLOW= .1488E+01 BASIN STORAGE= .4644E-04 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = G MANE .70 40.99 .01 191.07 2.00 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1950E+01 OUTFLOW= .1939E+01 BASIN STORAGE= .1238E-03 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = H1 MANE 1.43 144.55 .01 191.09 2.14 2.00 143.52 CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6766E+01 OUTFLOW= .6737E+01 BASIN STORAGE= .3777E-03 PERCENT ERROR= FOR STORM = 1 STORM AREA (SQ MI) = 12.00 161.86 .01 192.60

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Page 6

#### EX-TEST.OUT

CONTI	NUITY SUMMARY (AC-FT) ~ INFLOW	= ,0000E+00 EXC	ESS= .8257E-	+01 OUTFLOW:	.8201E+	01 BASIN	STORAGE=	.5785E-03	PERCENT	ERROR=	. 7
	FOR STORM = 1 STORM AREA I MANE 1.	(SQ MI) = 50 69.38	.01 188.68	2.14	2.00	69.26	190.00	2.14			
CONTI	NUITY SUMMARY (AC-FT) - INFLOW	= .0000£+00 EXC	ESS= .2867E	+01 OUTFLOW:	≈ .2859E+	01 BASIN	STORAGE=	.1014E-03	PERCENT	ERROR=	. 3
	FOR STORM = 1 STORM AREA J MANE .	(SQ MI) = 69 80.91	.01 185.91	2.15	2.00	80.83	186.00	2.15			
CONT	NUITY SUMMARY (AC-FT) - INFLOW	= .0000£+00 EXC	ESS= .2752E	+01 OUTFLOW:	≈ .2749E+	01 BASIN	STORAGE≖	.4778E-04	PERCENT	ERROR=	.1
	FOR STORM = 1 STORM AREA K MANE	(SQ MI) = 73 34.32	.01 184.84	2.15	2.00	33.78	184.00	2.15			
CONT	NUITY SUMMARY (AC-FT) - INFLOW	= .0000£+00 EXC	ESS= .1032£	+01 OUTFLOW	≈ .1032€+	01 BASIN	STORAGE=	.6088E-05	PERCENT	ERROR=	.0
	FOR STORM = $1$ STORM AREA $\ell$ MANE .	(SQ MI) ≠ 67 31.90	.01 185.18	2.15	2.00	31.51	186.00	2.19	; ;		
CONT	NUITY SUMMARY (AC-FT) - INFLOW	= .0000£+00 EXC	ESS= .1032E	+01 OUTFLOW	≖ .1031E+	01 BASIN	STORAGE=	.1334E-04	PERCENT	ERROR=	.1
	FOR STORM = 1 STORM AREA M MANE 1.	(SQ MI) = 00 26.37	.01 186.99	2.15	2.00	26.23	186.00	2.15	;		
CONT	NUITY SUMMARY (AC-FT) - INFLOW	'≃ .0000£+00 EXC	ESS= .9174E	+00 OUTFLOW	= .9164E+	00 BASIN	STORAGE≃	.1694E-04	PERCENT	ERROR≠	.1
	FOR STORM = 1 STORM AREA N MANE 1.	(SQ MI) = 27 33.61	.01 186.96	2.15	2.00	32.90	186.00	2.15	i ,		
CONT	NUITY SUMMARY (AC-FT) - INFLOW	= .0000E+00 EXC	ESS= .1147E	+01 OUTFLOW	= .1146E+	01 BASIN	STORAGE=	.1533E-04	PERCENT	ERROR=	.1
	FOR STORM = I STORM AREA O MANE .	(SQ MI) = 40 10.64	.01 184.55	2.15	2.00	10.48	184.00	2.15			
CONT	NUITY SUMMARY (AC-FT) - INFLOW	= .0000E+00 EXC	ESS= .3326E	+00 OUTFLOW:	= .3322E+	00 BASIN	STORAGE=	.3447E~05	PERCENT	ERROR=	.1
	FOR STORM = 1 STORM AREA P MANE	(SQ MI) = 43 24.11	.01 185.50	2.15	2.00	23.85	186.00	2.15			
CONT	NUITY SUMMARY (AC-FT) - INFLOW	± .0000E+00 EXC	ESS= .7913E	+00 OUTFLOW	≖ .7899E+	00 BASIN	STORAGE=	.1181E-04	PERCENT	ERROR≖ .	.2
	FOR STORM = 1 STORM AREA Q MANE .	(SQ MI) = 39 12.21	.01 184.17	2.15	2.00	12.19	184.00	2.15	i		
CONT	NUITY SUMMARY (AC-FT) - INFLOW	= .0000E+00 EXC	ESS= .3670E	+00 OUTFLOW	= .3667E+	OO BASIN	STORAGE=	.2684E-05	PERCENT	ERROR≠	.1
	FOR STORM = 1 STORM AREA R MANE	(SQ MI) ⇒ . 91 488.31	.01 186.83	2.15	2.00	481.77	186.00	2.15	;		
CONT:	NUITY SUMMARY (AC-FT) - INFLOW	/= .0000£+00 EXC	ESS= .1720E	+02 OUTFLOW	= .1717E+	-02 BASIN	STORAGE⇒	.3282E-03	PERCENT	ERROR=	. 2

\*\*\* NORMAL END OF HEC-1 \*\*\*

APPENDIX C

**Proposed HEC-1 Model** 

FLOOD HYDROGRAPH PACKAGE (HEC-1)
JUN 1998
VERSION 4.1

RUN DATE 16DECQ5 TIME 09:14:58

U.S. ARMY CORPS OF ENGINEERS HYDROLOGIC ENGINEERING CENTER 609 SECOND STREET DAVIS, CALIFORNIA 95616 (916) 756-1104

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

	HEC-1 INPUT	PAGE	1
LINE	. ID12345678910		
1 2 3 4 5 6 7 8	ID HEC-1 MODEL FOR MCDOWELL MOUNTAIN BACKBOWL ID 100-YEAR, 6-HOUR STORM ID RAINFALL FROM NOAA ATLAS ID SCS CURVE NUMBER SOIL LOSS PARAMETERS ID KINEMATIC WAVE HYDROGRAPH ROUTING ID DEVELOPED CONDITIONS ID 10% IMPERVIOUS AREA USED FOR SITE	,	-
9 10 11	ID PREPARED BY WOOD/PATEL, 1.10.2005 ID FILE NAME: DEV-100.DAT ID	•	
12 13 14 15	*DIAGRAM IT 2 2000 IO 5 JD .01 PH .73 1.43 2.42 2.74 2.96 3.37		
16 17 18 19 20 21	KK Al KM RUNOFF FROM SUB-BASIN Al BA .083 LS 88 1.1 UK 260 .025 .15 100 RK .3520 .023 .032 TRAP 15 5		
22 23 24 25 26 27	KK BASA1 KM 1-12' PIPE AND 28' WIER AT HEADWALL RS 1 STOR 0 SV 0 .151 .342 .575 SE 0 1 2 3 SQ 0 2 83 228		
28 29 30 31 32 33	KK A2 KM RUNOFF FROM SUB-BASIN A2 BA .036 LS 88 7.4 UK 235 .025 .15 100 RK 2000 .02 .032 TRAP 15 5	·.	
34 35 36 37 38 39	KK BASA2-1 KM 2-36" PIPES KM 1-12' PIPE AND 15' WEIR AT HEADWALL RS 1 STOR 0 SV 0 .127 .292 .504 SE 2772 2773 2774 2775 SQ 0 2.2 46.5 124.8		,
41 42 43 44 45 46 47	KK BASA2-2  KM 2-36" PIPES  KM 1-12' PIPE AND 15' WEIR AT HEADWALL  RS 1 STOR 0  SV 0 0.7 .16 .272 .407  SE 2733 2734 2735 2736 2737  SQ 0 2.2 46.5 124.8 225.3  HEC-1 INPUT	PAGE	. 2
LINE	ID12345678910		
48 49 50 51 52 53	KK B KM RUNOFF FROM SUB-BASIN B BA .077 LS 88 5.7 UK 400 .025 .15 100 RK 2420 .024 .032 TRAP 15 5		
54 55	KK BASB-1 KM 3-42" PIPES Page 1		

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DEV-NEWB.OUT
WEIR AT HEADWALL
                                                              B-2

3-42" PIPES

1-12" PIPE AND 28' WEIR AT HEADWALL

1 STOR 0

0 .21 .459 .750

6 .2747 .2748 .2749

0 .2 83 .228
    68
69
70
71
72
73
                                                        C RUNOFF FROM SUB-BASIN C
                                      KK
KM
BA
LS
UK
RK
                                                                           88
.023
.017
                                                                                               5.9
.15
.032
                                                    BASC-1
2-30" PIPES
1-12" PIPE AND 15' WEIR AT HEADWALL
1 5TOR 0
0 .107 .236 .388
2779 2780 2781 2782
0 2.2 46.5 124.8
    74
75
76
77
78
79
80
                                      KK
KM
KM
RS
SV
SE
SQ
                                                    BASC-2
2-30" PIPES
1-12" PIPE AND 15' WEIR AT HEADWALL
1 STOR 0
0 .054 .127 .225
2754 2755 2756 2757
0 2.2 46.5 124.8
    81
82
83
84
85
86
87
                                      KK
KM
RS
SV
SE
SQ
                                                     D RUNOFF FROM SUB-BASIN D
    88
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KM
BA
LS
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.014
                                                                                              .15
.032
                                                     300
1425
                                                                                                                  HEC-1 INPUT
LINE
                                      ID.,
                                                     BASD-1
2-36" PIPES
1-12" PIPE AND 10' WEIR AT HEADWALL
1 STOR 0
0 .134 .300 .500
2736 2737 2738 2739
0 3 54 82
 94
95
96
97
98
99
                                      KK
KM
RS
SV
SE
SQ
                                                  BASD-2
2-36" PIPES
1-12" PIPE AND 10' WEIR AT HEADWALL
1 STOR 0
0 .097 .218 .364
2724 2725 2726 2727
0 1 31 82
  101
102
103
104
105
106
  108
109
110
111
112
113
                                      KK
KM
BA
LS
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                                                      El 5
RUNOFF FROM SUB-BASIN El
.061
                                                                          88
.02
.013
                                                      400
2050
                                                 BASE1-1
2-36" PIPES
1-12" PIPE AND 15' WEIR AT HEADWALL
1 STOR 0
0 .319 .712 1.18
2716 2717 2718 2719
0 2.2 46.5 124.8
                                      KK
KM
KM
RS
SV
SE
SQ
                                                BASE1-2
2-36" PIPES
1-12" PIPE AND 15' WEIR AT HEADWALL
1 STOR
0 .108 .238 .391
2689 2690 2691 2692
0 2.2 46.5 124.8
 121
122
123
124
125
126
127
                                      KK
KM
KM
RS
SV
SE
SQ
 128
129
130
131
132
133
                                                      E2
RUNOFF FROM SUB-BASIN E2
.046
                                                                           .02
.013
                                                                                               10
.15
.032
                                                                                                                   HEC-1 INPUT
                                                                                                                                                                                                                                                                   PAGE
LINE
                                                   BASE2
2-42" PIPES
 134
135
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Page 2

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DEV-NEWB.OUT
1-12" PIPE AND 13' WEIR AT HEADWALL
I STOR 0
0.176.385.632.919
4.2695.2696.2697.2698
0.2.2.40.9.108.95.196.24
 136
137
138
139
140
                                  KM
RS
SV
SE
SQ
                                   KK
KM
HC
*
                                               COMBE COMBINE SECTIONS E1 AND E2 2
  144
145
146
                                   KK
KM
HC
                                                        CLEAR HYDROGRAPH STACK
                                                 E3
RUNOFF FROM SUB-BASIN E3
.008
                                                                    .025
.032
                                                                                        10
.15
.032
                                                   175
750
                                                                                                            100
                                               BASE3

1-24" PIPE

1 STOR

0 .082

2711 2712

0 4.2
 153
154
155
156
157
158
                                  KK
KM
RS
SV
SE
SQ
 159
160
161
162
163
164
                                  KK
KM
BA
LS
UK
RK
                                                 F1
RUNOFF FROM SUB-BASIN F1
.036
                                                                    88
.018
.012
                                                                                        10
.15
.032
                                                 225
2500
                                                                                                            100
                                             Z-48" PIPES

1 STOR

0 .132

2696 2697

0 16
                                   KK
KM
RS
SV
SE
SQ
                                                                                       . 293
2698
46
                                                                                                         .483
2699
92
                                                                                                         HEC-1 INPUT
LINE
 171
172
173
174
175
176
                                                 FZ
RUNOFF FROM SUB-BASIN FZ
.014
                                  KK
KM
BA
LS
UK
RK
                                                                    .018
.028
                                                                                      10
.15
.032
                                                200
1440
                                             BASF2
1-30" PIPE
V-NOTCH WEIR WITH 95 DEGREE ANGLE AT HEADWALL
1 STOR 0
0 .055 .135 .240
2701 2702 2703 2704
0 2.7 15.4 42.4
 177
178
179
180
181
182
183
                                   KK
KM
RS
SV
SE
SQ
  184
185
186
187
188
189
                                   KK
KM
BA
LS
UK
RK
                                                  F3
RUNOFF FROM SUB-BASIN F3
.013
                                                                                        10
.15
.032
                                                                    .018
.026
                                                   225
850
                                                                                                            100
                                             BASF3
1-30" PIPE
V-NOTCH WEIR WITH 50 DEGREE ANGLE AT HEADWALL
1 STOR 0
0 .055 .133 .239 .378
2706 2707 2708 2709 2710
0 1.2 6.6 18 37.2
 190
191
192
193
194
195
196
                                   KK
KM
KM
RS
SV
SE
SQ
  197
198
199
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KM
BA
LS
UK
RK
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RUNOFF FROM SUB-BASIN G
.015
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. 022
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. 032
                                                   400
720
                                                                                                            100
                                                                                                                                                  15
                                  KK
KM
BA
LS
  203
204
205
206
                                               G-1
RUNGFF FROM SUB-BASIN G-1
.0014
                                                                         88
                                                                                           10
                                             BASIN G-1
1-24" PIPE
1 STOR
0 .159
2723 2724
  207
208
209
210
211
                                   KK
KM
RS
SV
SE
                                                                                                                                  Page 3
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212	SQ	o	4.2	12.5	20.2	DEV-NEV	WB.OUT				
LTAIR			_	_		INPUT	_	-			PAGE 6
LINE	10	1	2	3	4 .			/	8	910	
213 214 215 216 217 218	KK KM RS RC RX RY	RG-1 ROU 1 .45 100 10	TE BASIN FLOW .35 110 99	G-1 THR 0 .45 120 98	OUGH G 610 138 95	.048 148 95	2744 166 98	176 99	186 100		,
219 220 221	KK KM HC *	CPG CONCEN 2	TRATION	POINT G				•	,		
222 223 224 225 226 227	KK KM BA LS UK RX	H1 RUN .059 375 2210	0FF FROM 88 .025 .019	9.4 .15 .032	IN H1	TRAP	15	. 5			
228 229 230 231 232 233	KK KM RS SV SE SQ	BASH1 2 1 0 2736 0	-54"PIPE STOR .086 2737 20	0 .192 2738 52	.321 2739 104	.474 2740 162				·	•
234 235 236 237 238 239	KK KM BA LS UK RK *	H2 RUN .072 400 3480	0FF FROM 88 .025 .019	SUB~BAS 4.78 .15 .032	IN H2	TRAP	15	5			•
240 241 242 243 244 245	KK KM RS SV SE SE SQ	BASH2-1 3- 1 0 0 0	54" PIPE STOR .34 1 30	s .743 2 .78	1.214 3 156						*
246 247 248 249 250 251	KK XM BA LS UK RK	1 RUN .025 225 2100	0FF FROM 88 .02 .026	5UB-BAS 7.4 .15 .032	100	TRAP	15	. 5		·	
LINE	ID.	1	2	3		INPUT5	6	7	8	910	PAGE 7
252 253 254 255 256 257 258	. KK KM KM KS SV SE SQ	BASI 2- 1- 0 2755 0	30" PIPE 12" PIPE \$TOR .186 2756 2	AND 9' 0 .399 2757 35.1	WEIR AT .640 2758 87.4	HEADWALL		,			
259 260 261	KK KM HC *	CLEAR CL 6	EAR HYDR	OGRAPH S	TACK -					,	٠.,
262 263 264 265 266 267	KK KM BA LS UK RK	J RUN .024 145 965	0FF FROM 88 .02 .026	7.9 .15 .032	IN J 100	TRAP	15	. \$	, ,		
268 269 270 271 272 273	KK KM RS SV SE SQ	BASJ	42" PIPE STOR .098 .2770 8	0 .217 2771 20	.359 2772 41	.528 2773 60		,			,
274 275 276 277 278 279	KK KM BA LS UK RK *	K RUN .009 81 715	0FF FROM 88 .025 .022	SUB-BAS 10 .15 .032	IN K	TRAP	15	5			٠,
280 281 282 283	KK KM RS SV	BASK 1- 1 0	30" PIPE STOR .106	.232	. 378	Pag	e 4				

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284
285
                                                                         2740
28
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287
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291
                                   L
RUNOFF FROM SUB-BASIN L
.009
                                               .018
.012
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                                                                                      TRAP
LINE
                         ID..
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                                 292
293
294
295
296
297
                         KK
KM
RS
SV
SE
SQ
                                  BASL
                                         1-30"PIPE
STOR
.035
2723
  298
299
300
301
302
303
                         KK
KM
BA
LS
UK
RK
                                   M
RUNOFF FROM SUB-BASIN M
.008
                                                .018
.012
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                                        1-30"PIPE
1 STOR
0 .069
1 2722
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  304
305
306
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308
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                         KK
KM
RS
SV
SE
SQ
                                  BASM
  310
311
312
313
314
315
                        KK
KM
BA
LS
UK
RK
                                    N
RUNOFF FROM SUB-BASIN N
.01
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.012
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1036
                                                             .15
.032
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                                 BASN

1-24" PIPE

1 STOR

0 .11

2691 2692

1 4.2
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317
318
319
320
321
                         KK
KM
RS
SV
SE
SQ
  322
323
324
325
326
327
328
                         KK
KM
KM
BA
LS
UK
RX
                                BASO
RUNOFF FROM SUB-BASIN O
Area of Basin subtrating the area to be retained for 100 year event
.00265
                                               .02
.014
                                                            10
.15
.032
                                    108
174
                                                                                                      15
                                                                                      TRAP
  329
330
331
332
333
334
                                 P
RUNOFF FROM SUB-BASIN P
Area of Basin subtrating the area to be retained for 100 year event
.0058
                                                 .88
.02 、
                                    135
                                                                          100
                                                                         HEC-1 INPUT
                                                                                                                                                                    PAGE 9
LINE
                         ID.,....1.....2.....3......4.....5......6..
  335
                         RK
                                    354
                                               .014
                                                            .032
  336
337
338
                         KK
KM
HC
                                 CLEAR
                                       CLEAR HYDROGRAPH STACK
  339
340
341
342
343
344
345
                         KK
KM
KM
BA
LS
UK
RK
                                 Q
RUNOFF FROM SUB-BASIN Q
Area of Basin subtrating the area to be retained for 100 year event
.0024
                                                .02
.014
                                                            10
,15
.032
                                    86
160
  346
347
348
349
350
351
352
353
                         KK
KM
KM
BA
LS
UK
RK
ZZ
                                    R RUNOFF FROM SUB-BASIN R Area of Basin subtrating the area to be retained for 100 year event .148
                                               88
.025
.023
                                  180
1840
                                                             .15
.032
           SCHEMATIC DIAGRAM OF STREAM NETWORK
  (V) ROUTING
                                    (--->) DIVERSION OR PUMP FLOW
  (.) CONNECTOR
                                    (<---) RETURN OF DIVERTED OR PUMPED FLOW
```

INPUT

LINE

NO.

16

		•			DEV-NI	EWB.OUT	
22	BASA1						
28	:	A2 V					
34	:	BASA2					
41	:	V V BASA2					
	:						
48	:		B V				
54	*	•	BASB V				
61	:		V BASB				
68	:	•	:	٠ .			
	:	:	:	· c v			
74			•	BASC			
81	:	•	•	BASC .			
. 88	:		:		. D		
94	:	• .	:	:	V V BASD		
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101	:				BASD .		
108	:	· ·	:	:	. :	E1	
114 、		:	:	:	:	V BASE1	
	:		:	:	:	BASE1	•
121	- :	•		. :	:	BASEL	
128	:		:				EŻ V
134	:		:	`:		: '	BASE2
141	:				:	COMBE	
144	CLEAŘ.	•	;	:-	:		
	CLEAR						
147	:	E3 V ·			,		
153	. :	BASE3					
159	:	:	F1 V				
165	:		BASF1				
	:	:					
171	:		:	F2 V			
177		•		8ASFŽ		-	
184					F3	-	
190	:	, :	:		F3 V V BASF3		•
	:	• • • • • •			•	_	•
197	:	•	•			G :	
203	:	:			•	•	G-1 V
207	:		:	:	•	•	BASIN V
213		. :	:	•	:	`.	RG-1
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219	:	•	•	:	•	CPG	* * * * * * * * * *
222	:	· ·	:	:	•	•	H1 V
228	:	:	:		•	:	V BASH1
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234	•	•				age 6	

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2	46	: :		•	•		•	: :	I V	
2	52	:		:		•		: :	V	
2	32	:				•			BASI	
2	59	: :		CLEAR.						
2	62	:		•		-				
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									<b>V</b>	
***	116 HEC1 ERROR S	*** TOO MAN	Y HYDROGRAPHS	. COMBINE MOR	E OFTEN.	•			BASN	•
		:	, ,			•	•	: :		
***	22 HEC1 ERROR 5	*** TOO MANY	Y HYDROGRAPHS	. COMBINE MOR	E OFTEN.	,	•		BASO	
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3	29	:		•		· ·		: :	P ,	
3	36	CLEAR	₹							
2	39 .		·	_						
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3	146	:		R		•	•		•	
(**	*) RUNOFF ALS	O COMPUTED AT	T THIS LOCATI	ION			٠	•	•	
1***	2 ERRORS	IN STREAM 5	YSTEM	•	•			******	*******	******
*	FLOOD HYDROG	RAPH PACKAGE	(HEC-1) *	: :			•		ARMY CORPS OF ENGINEE	
*	VER	JUN 1998 RSION 4.1	,	t '			•	*	OLOGIC ENGINEERING CEN 609 SECOND STREET	*
*		DECOS TIME	09:14:58	† †				* D	AVIS, CALIFORNIA 95616 (916) 756-1104	*
**	*******	*****	* **********	* •				*****	******	*****
						,				
			,			•,				
			HEC-1 MOX	DEL FOR MCDOWE	LL MOUNTAIN	N BACKBOWL				
			RAINFALL	, 6-HOUR STORM FROM NOAA ATL	.AS	ETEDC				
			KINEMATIO	E NUMBER SOIL C WAVE HYDROGR D CONDITIONS	APH ROUTIN	G				
				RVIOUS AREA US	ED FOR SIT	Ē	·* .			
			PREPARED FILE NAME	BY WOOD/PATEL	, 1.10.2009	5 .		•		
	٠.									
3	13 10	IPRNT	ROL VARIABLES 5	PRINT CONTRO	L					
٠.		IPLOT QSCAL	0	PLOT CONTROL HYDROGRAPH P						
	_				_					

COMPUTATION INTERVAL TOTAL TIME BASE .03 HOURS 66.63 HOURS

DATA

2 MINUTES IN COMPUTATION INTERVAL
1 O STARTING DATE
2000 STARTING TIME
2000 NUMBER OF HYDROGRAPH ORDINATES
3 O ENDING DATE
1838 ENDING TIME
19 CENTURY MARK

HYDROGRAPH TIME DATA
NNIN
IDATE 1
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NQ 20
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	ENGLISH UNITS  DRAINAGE AREA PRECIPITATION DEPTH LENGTH, ELEVATION FLOW STORAGE VOLUME SURFACE AREA TEMPERATURE	INCH FEET CUBI ACRE ACRE	C FEET PE -FEET		DEY-IVEN					
14 JO	INDEX STORM NO. 1 STRM TRDA	3.37 .01		ATION DEPT ITION DRAI	TH INAGE AREA	,				
15 PI	.00 .00 .00 .00 .00 .00 .00 .00 .00 .00	00 00 00 00 00 01 01 01 04 24 03 01	.00 .00 .00 .01 .01 .01 .01 .03 .03 .01 .01 .01 .01 .00	.00 .00 .00 .01 .01 .01 .01 .06 .11 .01 .01 .01 .01	.00 .00 .00 .01 .01 .01 .01 .06 .07 .03 .01 .01 .01 .01	.00 .00 .00 .01 .01 .01 .03 .07 .07 .01 .01 .01	.00 .00 .00 .01 .01 .01 .03 .09 .06 .01 .01 .01	.00 .00 .00 .01 .01 .01 .03 .12 .05 .01 .01 .01	.00 .00 .00 .01 .01 .01 .03 .17 .04 .01 .01 .00	.00 .00 .00 .01 .01 .01 .03 .29 .04 .01 .01 .01 .00 .00

<sup>\*\*\*</sup> FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION USED - ITERATION=  $\,1\,$ 

<sup>\*\*\*</sup> FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION USED - ITERATION= 1

	RUNOFF SUMMARY
FLOW	IN CUBIC FEET PER SECOND
TIME IN	HOURS. AREA IN SQUARE MILES

					•				
ADEDATION		PEAK	TIME OF	AVERAGE FL	OW FOR MAXI	MUM PERIOD	BASIN	MAXIMUM	TIME OF
OPERATION	STATION	FLOW	PEAK	5-HOUR	24-HOUR	72-HOUR	AREA	STAGE	MAX STAGE
HYDROGRAPH AT				*			•		
ILLONOGWALH WI	A1	224.	3.17	19.	5.	2.	.08		
ROUTED TO								•	
	BASAl	220.	3.20	19.	5.	· 2.	.08		• • •
•				٠,				2.94	3.20
HYDROGRAPH AT	A2	107.	3.13	9	2.	1.	.04		
BOUTED TO	AL.	107.	3.13	J	2.	1.	.04	•	•
ROUTED TO	BASAZ -	100.	3.17	9.	2.	1.	.04		
								2774.69	3.17
ROUTED TO	_			_	_	_			
	BASA2	100.	3.20	9.	2.	1.	.04	2735.68	3.20
HYDROGRAPH AT									
	`8	187.	3.20	18.	5.	2.	.08		
ROUTED TO					•				
	BASB	183.	3.20	18.	5.	2.	.08	2769.90	3.20
		•						2703.30	3.20
ROUTED TO	BASB	181.	3.23	18.	5.	2.	, 08		
-								2748.67	3.23
HYDROGRAPH AT	-			_	_	-		•	
	С	86.	3.13	7.	. 2.	1.	.03		
ROUTED TO	0456	84.	3.17	7.	2,	1.	.03		
	BASC		3.17	/.	۷.	1.	.03	2781.48	3.17
ROUTED TO									
	BASC :	83.	3.20	7.	2.	1. ,	٠03	2756.46	3.20
* *								2730.40	3.20
HYDROGRAPH AT	D	93.	3.17	8.	2.	1	.03		•
DOUTED TO				•			103		
ROUTED TO	BASD	80.	3.23	8.	. 2.	1.	.03		
								2738.92	3.23
ROUTED TO		7.0	,	•				•	
	BASD	78.	3.27	8.	2.	1.	.03	2726.92	3.27
				·	Page &				

<sup>\*\*\*</sup> FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION USED - ITERATION= 1

				DEV	-NEWB.OUT	•			
HYDROGRAPH AT	E1	144.	3.20	15.	4.	1.	.06		
ROUTED TO	8ASE1	124.	3.27	14.	4.	1.	.06	2719.00	3.27
ROUTED TO	BASE1	123.	3.30	14.	4.	1.	.06	2691.98	3.30
HYDROGRAPH AT	E2	133.	3.17	11.	3.	· 1.	`.05		
ROUTED TO	BASE2	123.	3.20	11.	3.	1.	.05	2697,16	3.20
2 COMBINED AT	COMBE	226.	3.27	25.	7.	2.	.11	-44.1	
6 COMBINED AT	CLEAR	861.	3.23	86.	. 22.	8.	. 37		
HYDROGRAPH AT	E3	27.	. 3.10	2.	0.	0	.01		
ROUTED TO	BASE3	· 16.	3.23	2.	٥.	0.	.01	2713.42	3.23
HYDROGRAPH AT	F1 .	96.	3.17	9.	2.	1.	.04		
ROUTED TO	BASF1	. 88_	3.23	9	2.	1	. 04	2698.91	3.23
HYDROGRAPH AT	F2	43.	3.13	3.	1.	0.	.01		•
ROUTED TO	BASF2	38.	3.20	, 3.	<sub>1.</sub>	٥.	.01	2703.83	3.20
HYDROGRAPH AT	F3	, , 39.	3.13	3	1.	0.	.01	-	
ROUTED TO .	BASF3	. 30.	3.20	3.	1.	. 0.	.01	2709.60	3.20
HYDROGRAPH AT	G	38.	3.17	4.	1.	0.	.01		
HYDROGRAPH AT	G-1	0.	.00	0.	0.	0.	.00		,
ROUTED TO	BASIN	0.	.00	<b>.</b>	0.	0.	.00	2723.00	.00
ROUTED TO	RG-1	0.	00	٠. ٥.	0.	· 0.	.00	. 10.00	.00
2 COMBINED AT	. CPG	38.	3.17	4.	1.	0.	. 02		
HYDROGRAPH AT	н1	149.	3.17	14.	4	1.	.06	-	
ROUTED TO	BASH1	144.	3.20	14.	4.	1.	.06	2739.69	3.20
HYDROGRAPH AT	н2	165.	3.23	17.	4.	2.	.07	•	
ROUTED TO	BASH2	143.	3.30	17.	4	2.	.07	2.84	3.30
HYDROGRAPH AT	I	72.	3.17	6.	2.	1.	.03	•	
ROUTED TO	BASI	61.	3.20	6.	. 2.	1.	Ò3	2757.50	3.20
6 COMBINED AT	CLEAR	430.	3.23	47.	12.	4.	. 20		
HYDROGRAPH AT	,	83	3.10	6.	1.	1.	.02	•	
ROUTED TO	BASI	60.	3.17	6.	. 1	1.	.02	2772.99	3.17
HYDROGRAPH AT	ĸ	36.	3.07	. 2.	1.	<b>o.</b>	.01		
ROUTED TO	BASK	. 21.	. 3.17	2.	· 1. Page 9	0.	.01		

					DEV-NEW	B.OUT			3720 30	2 17	
HYDROGRAPH	I AT	L	33.	3.10	2.	1.	0.	.01	2739.39	3.17	
ROUTED TO		BASL	25.	3.17	Ζ,	1.	0.	10.			
		DAJL	23.	3.17	۷.		ν.	.02	2724.73	3.17	
HYDROGRAPH -	I AT	M	27.	3.10	2.	٥.	0.	.01			
ROUTED TO	-	BASM	19.	3.20	2.	0.	0.	.01	2723.27 '	3.20	
· HYDROGRAPH	I AT	N	34.	3.10	2.	1.	·o.	.01			
ROUTED TO	•	BASN	17.	3.27	3.	1.	1.	.01			
HYDROGRAPH	I AT	• •							2693.57	3.27	`.
HYDROGRAPH	AT	BASO	10.	3.07	1.	0.	0.	.00	•		
9 COMPTHE		, Р	21.	3.10	1.	0.	0.	.01	\$		
8 COMBINED HYDROGRAPH		CLEAR	574.	3.23	64.	17	. , <b>7.</b>	. 27			
HYDROGRAPH		`	9.	3.07	1.	0.	0.	.00			
, Transonarii	יאי	' R	448.	3.10	32.	8.	3.	. 14			
. ,		•	SUMP		EMATIC WAVE - IRECT RUNOFF W				•		
ISTAQ	ELEME	NT DT	. PEA	K TIME T PEAK		DT .	COMPUTATION PEAK	INTERVAL TIME TO PEAK	VOLUME ^		•
		(MIN)	•		N) (IN)	(MIN)	(CFS)	(MIN)	(IN)		
	! = 1 MANE	STORM AREA 1.9	(SQ MI) = 7 224.			2.00	223.88	190.00	2.16		
CONTINUITY SUMMARY	(AC-FT	) - INFLOW=	.0000E+0	O EXCESS≖ .	9578E+01 OUTFL	.ow= .95401	E+01 BASIN	STORAGE= .	3266E-03 PERCENT	ERROR=	.4
	I ⇒ 1 MANE	STORM AREA	(SQ MI) = 7 108.	.01 11 188.7		2.00	106,51	188.00	2.24		
CONTINUITY SUMMARY						-			-	F0909-	.1
	•	STORM AREA	٠.			.042 .4250	. ,	21010AGE- 1		EKKOK-	••
	MANE	1.3	8 188.			2.00	187.40	192.00	2.21		
CONTINUITY SUMMARY	(AC-FT	) - INFLOW=	.0000E+0	0 EXCESS= .	9116E+01 OUTFL	.ow= .9090	E+01 BASIN	STORAGE= .	5599E-03 PERCENT	ERROR=	. 3
FOR STORM C	1 = 1 MANE	STORM AREA	(SQ MI) ⇒ 3 87.	.01 41 188.6		2.00	86,27	188.00	2.22		
CONTINUITY SUMMARY	(AC-FT	) - INFLOW=	.0000E+0	0 EXCESS= .	3556E+01 OUTFL	.ow≂ .3548	E+01 BASIN	STORAGE= .	1096E-03 PERCENT	ERROR=	. 2
	1 ≈ 1 MANE	STORM AREA				2.00	92,69	190.00	2.29		*
CONTINUITY SUMMARY		,								FPROD-	.3
	,						CAGE DIGE		,	ERNOR-	••
	MANE	STORM AREA 1.5	0 144.	.01 09 192.2		2.00	143.65	192.00	2.26		
CONTINUITY SUMMARY	(AC-FT	) - INFLOW=	.0000E+0	0 EXCESS= .	7392E+01 OUTFL	.ow= .7359	E+01 BASIN	STORAGE≖ .	\$820E-03 PERCENT	ERROR=	.4
FOR STORM EZ	= 1 MANE	STORM AREA 1.9	(SQ MI) ≠ 0 134.	.01 16 ( 188.5		2.00	133,10	190.00	2.27		
CONTINUITY SUMMARY	(AC-FT	) - INFLOW=	.0000E+0	0 EXCESS= .	5574E+01 OUTFL	.ow= .5561	E+01 BASIN	STORAGE= .	1525E-03 PERCENT	ERROR≠	.2
	! = 1 MANE	STORM AREA	(SQ_MI) = 8 27.	.01 54 185.2		. 2.00	27,41	186.00	2.27		
CONTINUITY SUMMARY					•					FRROP-	.1
						,			CALCAI	2	• • •
	MANE	STORM AREA			8 2.27	2.00	95.80	190.00	2.27		
					Page	10					

#### DEV-NEWB, OUT

	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	E+00 EXCESS= .4363	E+01 OUTFLO	w= ,4350E	+01 BASIN	STORAGE≖	.1591E-03 PERCENT	ERROR=	. 3
	FOR STORM = 1 STORM AREA (SQ MI F2 MANE 1.22	) = .01 42.84 188.53	2.27	2.00	42.60	188.00	2.27		
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	E+00 EXCESS= .1697	E+01 OUTFLO	w= .1693E	+01 BASIN	STORAGE=	.4564E-04 PERCENT	ERROR=	.2
	FOR STORM = 1 STORM AREA (SQ MI F3 MANE .75	) = .01 39.32 187.69	2.27	2.00	39.09	188.00	2.27		
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	E+00 EXCESS= .1575	E+01 OUTFLO	w= .1573E	+01 BASIN	STORAGE=	.5118E-04 PERCENT	ERROR=	. 2
	FOR STORM = 1 STORM AREA (SQ MI G MANE .77	) = .01 38.40 190.65	2.27	2.00	38.38	190.00	2.27		
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	E+00 EXCESS= .1818	E+01 OUTFLO	w= .1813E	+01 BASIN	STORAGE=	.1139E-03 PERCENT	ERROR≈	.3
	FOR STORM = 1 STORM AREA (SQ MI H1 MANE 1.43 1	) = .01 50.60 191.55	2.26	2.00	149.35	190.00	2.26		
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	E+00 EXCESS= .7127	E+01 OUTFLO	w= .710SE	+01 BASIN	STORAGE=	.4181E-03 PERCENT	ERROR=	. 3
	FOR STORM = 1 STORM AREA (SQ MI HZ MANE 2.00 1	) = .01 67.87 193.01	2.20	2.00	165.00	194.00	2.20		
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	E+00 EXCESS= .8481	E+01 OUTFLO	₩= .8441E	+01 BASIN	STORAGE=	.5510e-03 PERCENT	ERROR=	.5
	FOR STORM = 1 STORM AREA (SQ MI I MANE 1.48	) = .01 72.37 188.53	2.23	2.00	71.73	190.00	2.23		
	CONTINUITY SUMMARY (AC-FT) - INFLOW0000	E+00 EXCESS= .2987	E+01 OUTFLO	w= .2980E	+01 BASIN	STORAGE≃	.101ZE-03 PERCENT	ERROR=	. 3
	FOR STORM = 1 STORM AREA (SQ M) J MANE .75	0) = .01 83.27 185.90	2.25	2.00	83.14	186.00	2.25		
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	E+00 EXCESS= .2876	E+01 OUTFLO	)W⇒ .2874E	+01 BASIN	STORAGE=	.4203E-04 PERCENT	ERROR=	.1
	FOR STORM = 1 STORM AREA (SQ M) K MANE .77	) = .01 35.81 184.12	2.27	2.00	35.58	184.00	2.27		
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	0E+00 EXCESS= .1091	E+01 OUTFLO	ow= .1090E	±+01 BASIN	STORAGE=	.5989E-05 PERCENT	ERROR=	.0
	FOR STORM = 1 STORM AREA (SQ MI L MANE .66	0) = .01 33.19 184.98	2.27	2.00	32.61	186.00	2.27		
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	E+00 EXCESS= .1091	E+01 OUTFLO	ow= .1090£	+01 BASIN	STORAGE=	.1377E-04 PERCENT	ERROR=	.ó
	FOR STORM = 1 STORM AREA (SQ M M MANE 1.04	() = .01 27.69 186.68	2.27	2.00	27.29	186.00	2.27		
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	PE+00 EXCESS= .9695	E+00 OUTFLO	)W≃ .9685 <i>E</i>	+00 BASIN	STORAGE=	.1590E-04 PERCENT	ERROR=	.1
	FOR STORM = 1 STORM AREA (SQ M) N MANE 1.26	) =01 34.49 187.00	2.27	2.00	34.09	186.00	2.27		
,	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	DE+00 EXCESS= .1212	E+01 OUTFLO	OW⇒ ,1211£	E+01 BASIN	STORAGE=	.1508E-04 PERCENT	ERROR=	.1
	FOR STORM = 1 STORM AREA (SQ M) BASO MANE .31	) = .01 10.07 184.74	2.27	2.00	9.97	184.00	2.27		
	CONTINUITY SUMMARY (AC-FT) -, INFLOW= .0000	0E+00 EXCESS= .3211	E+00 OUTFLO	ow= .3207€	+00 BASIN	STORAĞE=	.3235E-05 PERCENT	ERROR=	.1
	FOR STORM = 1 STORM AREA (SQ M P MANE .50	i) = 01 21.02 185.36	2.27	2.00	20.67	186.00	2.27		-
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000	DE+00 EXCESS= .7029	E+00 OUTFLO	Dw= .7020E	E+00 BASIN	STORAGE≈	.8658E-05 PERCENT	ERROR= .	.1
	FOR STORM = 1 STORM AREA (SQ M: Q MANE .41	() = .01 · 9.50 183.99	2.27	2.00	9.49	184.00	2.27		
	CONTINUITY SUMMARY (AC-FT) - INFLOW= .000	DE+00 EXCESS= .2908	SE+00 OUTFLO	ow= .2906	E+00 BASIN	STORAGE=	.2034E-05 PERCENT	ERROR≠	.1
	FOR STORM = 1 STORM AREA (SQ M R MANE .82	() = .01 153.82 187.00	2.15	2.00	447.75	186.00	2.15		
			•		,				

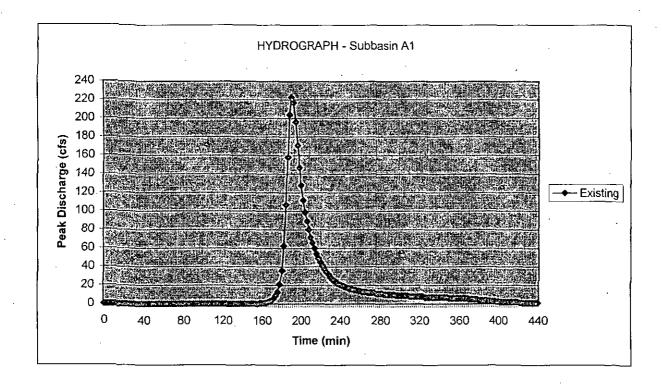
Page 11

DEV-NEWB.OUT
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1607E+02 OUTFLOW= .1605E+02 BASIN STORAGE= .2979E-03 PERCENT ERROR\* .1

\*\*\* NORMAL END OF HEC-1 \*\*\*

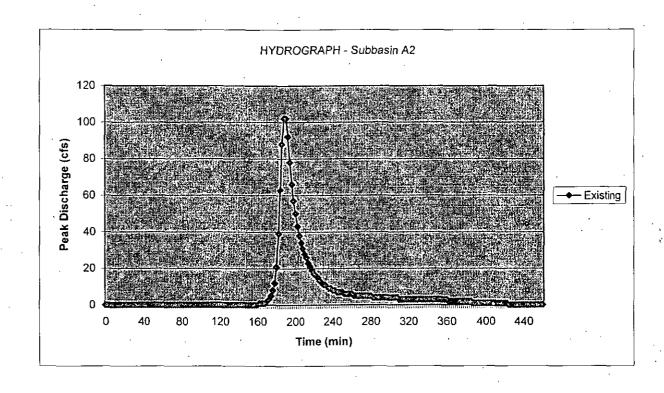
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: Sereno Canyon Location: City of Scottsdale



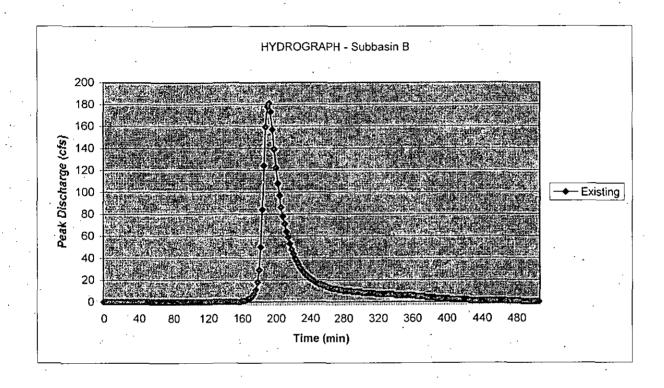
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

**Project:** Sereno Canyon **Location:** City of Scottsdale



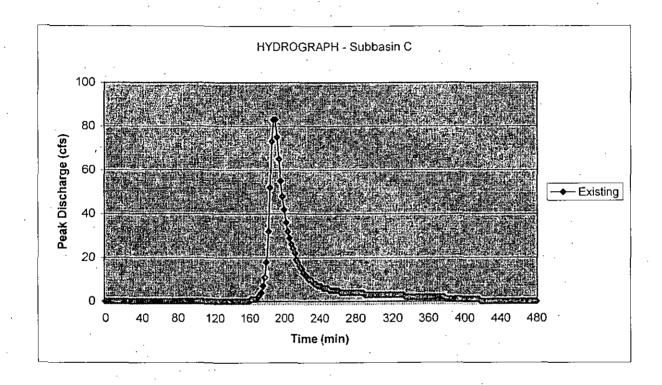
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Project: Sereno Canyon Location: City of Scottsdale



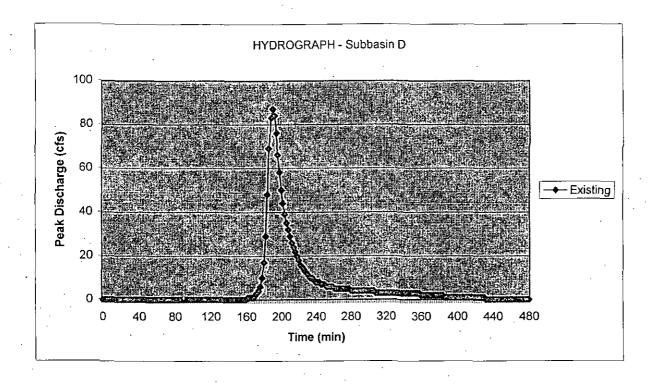
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Project: Sereno Canyon Location: City of Scottsdale



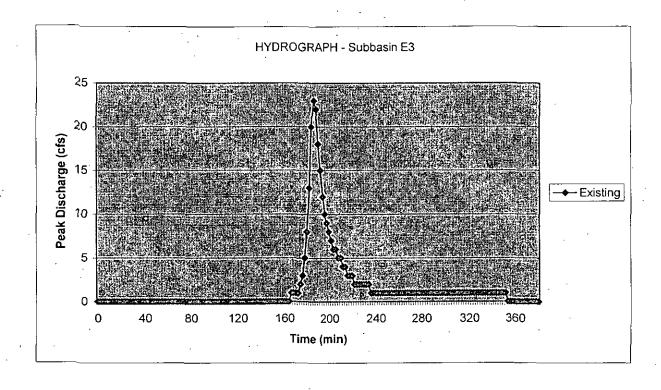
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Project: Sereno Canyon Location: City of Scottsdale



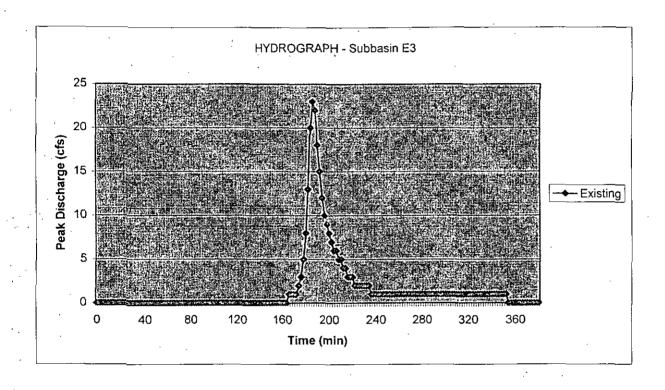
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Project: Sereno Canyon Location: City of Scottsdale



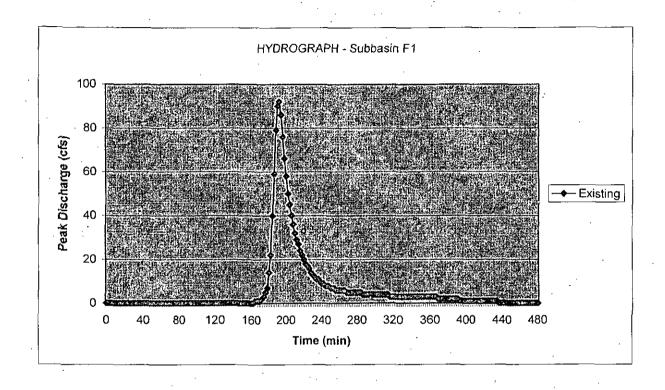
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Project: Sereno Canyon Location: City of Scottsdale



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Project: Sereno Canyon Location: City of Scottsdale



# APPENDIX D

Hydraulics: Culvert Rating Curves for Stage-Storage Intervals

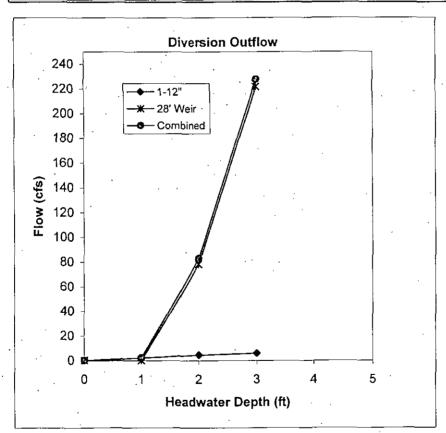
#### CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

**Project:** Sereno Canyon **Location:** City of Scottsdale

W/P#: 042054 Date: 12/16/2005

## Rating Curve for Diversion BASA1

1-1	2"	28' V	Veir	Comb	ined Flows
Headwater Depth	Flow	Headwater Depth	Flow.	Headwater Depth	Flow
ft	ft cfs		cfs	ft	cfs
0	0	0	0	0	Ö
1	2.2	1	0	11	2
2 .	4.5	2	78	2	83
3 6		3	222	3	228

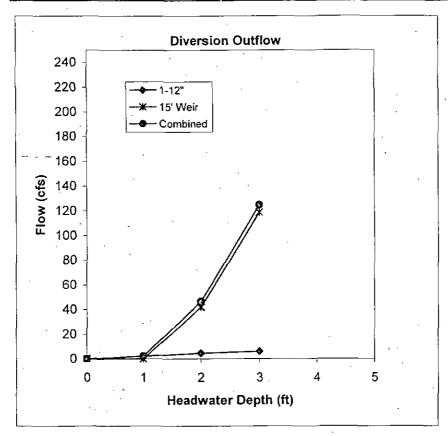


## CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: Sereno Canyon Location: City of Scottsdale W/P#: 042054 Date: 12/16/2005

#### Rating Curve for Diversion BASA2

1-1	2"	15' V	Veir	Combi	ned Flows
Headwater Depth ft	Flow cfs	Headwater Depth ft	Flow cfs	Headwater Depth ft	Flow
0	0	0	0	0	0
- 1	2.2	1	0	1	2
2	4.5	2	42	2	47
3	3 6		119	3	125



#### CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: Sereno Canyon

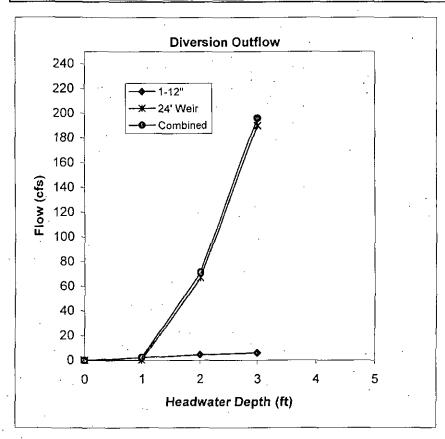
W/P#: 042054

Location: City of Scottsdale

Date: 12/16/2005

## Rating Curve for Diversion BASB

1-1	2"	24' V	Veir	Combined Flows			
Headwater Depth	Flow	Headwater Depth	Flow	Headwater Depth	Flow		
<u> </u>	CIS		013	R	<u> </u>		
0	0	0	. 0	0	0		
1	2.2	_ 1 _	0	_ 1 _	_ 2		
2	4.5	2 .	. 67	2	72		
3	6	3	190	3	196		



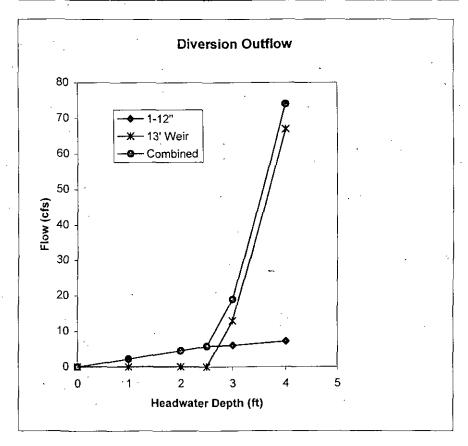
CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: Sereno Canyon Location: City of Scottsdale W/P#: 042054

Date: 12/16/2005

#### Rating Curve for Diversion BASH2

1-1	2"	13' W	/eir	Combined Flows			
Headwater Depth	Flow	Headwater Flow		Headwater Depth	Flow		
ft	cfs	ft	cfs	ft	cfs_		
0	0	0	0	0	0.0		
1	2.2	1	0	1	2.2		
2	4.5	2	0	2	4.5		
2.5	5.75	2.5	0	2.5	5.8		
3	6	3	13	3	18.9		
4	7.1	4	67	4	74.0		
5	8.2	5	144	5	152.1		



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Project: Sereno Canyon

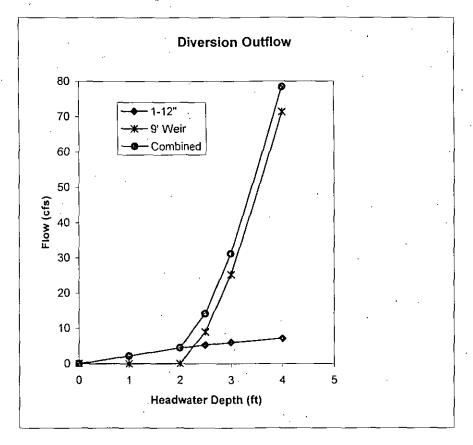
W/P#: 042054

Location: City of Scottsdale

Date: 12/16/2005

#### Rating Curve for Diversion BASI

1-1	2"	9' W	/eir	Combined Flows		
Headwater Depth	Flow	Headwater Depth	Flow	Headwater Depth	Flow	
ft	cfs	ft	cfs_	ft	cfs	
0	_ 0	0	0	0	0	
1	2.2	1	0	1	2	
2	4.5	2	0	2	5	
2.5	5.25	2.5	9	2.5	14	
3	6	3	25_	3	31	
4	7.1	4	71	4	78	



**HEC-RAS Output Files** 

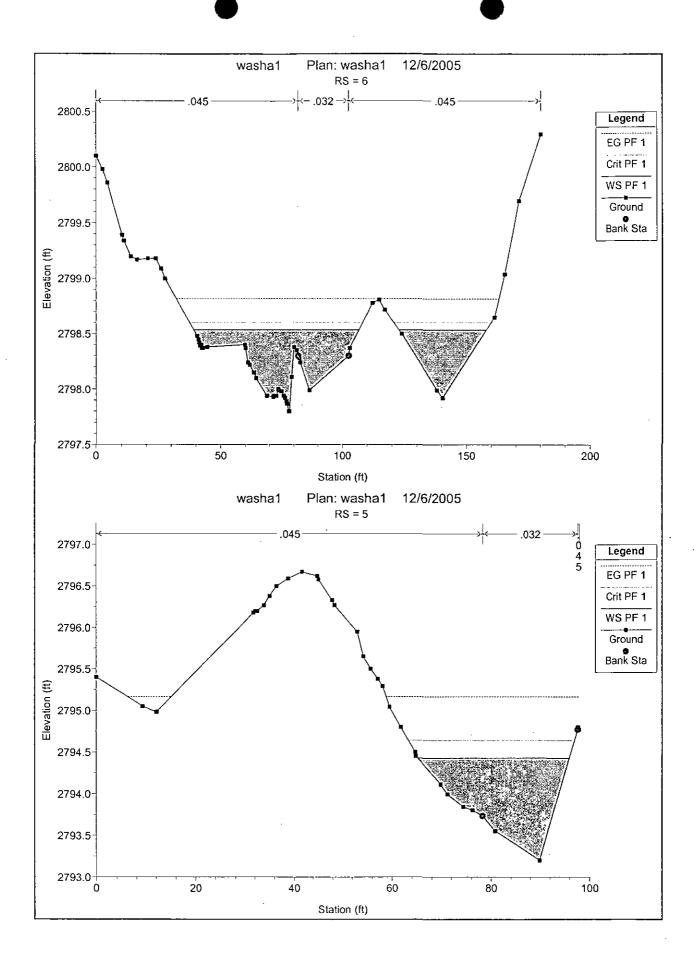
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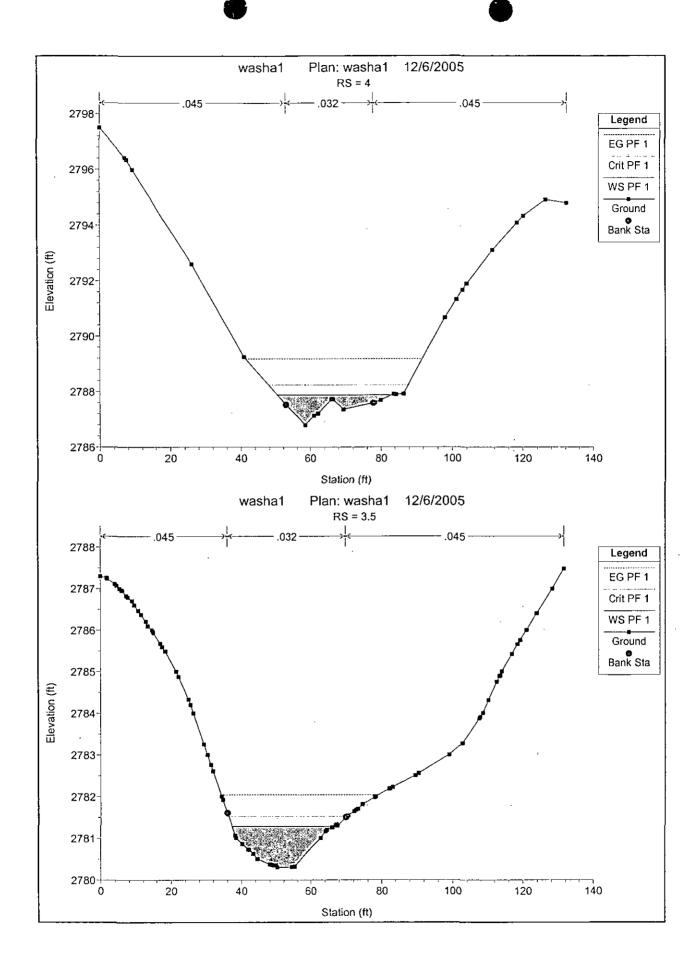
HEC-RAS Plan: wasa1   F	Piver RIVER-1	Reach Reach 1	Profile: DE 1

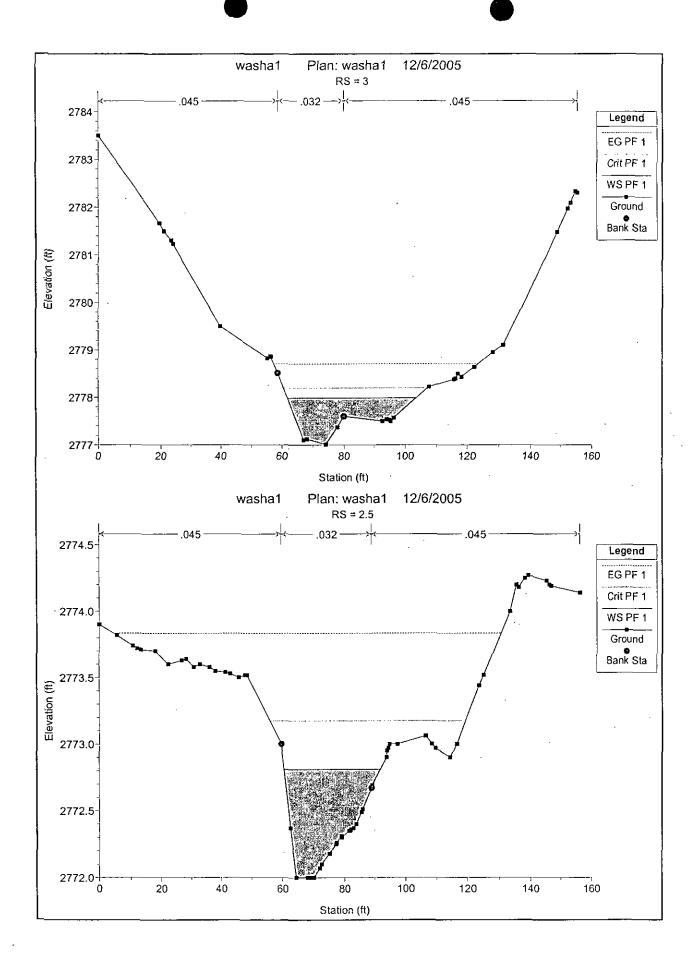
₩ Reach %	∄ River, Sta \$	Piofile 送名	🕾 Q Total 🥋	≱ Min Ch El∯	'⊭W.S. Elev .	₹Crit W.S.‰	∦E.G. Elev∄	¥ E.G.¹Slope ≨	∜Vel Chnl	Flow Area 🖫	%Top Width ≇	Froude # Chl
<b>新港市政策29</b> 年	[45]主起战强战	transp.	类率(cfs)。概	题读 (ft) 学會	好某些(ft) 70%37	表表: (ft) 的数2:	1324 (ft) 空歌	<sup>注题性(fl/fl)</sup> 乳酸	重(ft/s)統領	<b>深感 (sq ft)</b> もぎ	1985 (ft) 最高的	(基於聖]的《經濟
Reach-12条	6 基件域等值	PE.1编数编	128.00	2797.99	2798.54	2798 60	2798.82	0.050032	5.55	32.78	102.82	1.56
Reach-1957	5 科斯隆武器	PF.1流流。数(	128.00	2793.20	2794,43	2794 64	2795.16	0.030199	7.29	20.61	30.80	1.38
Reach-1347				2786.79	2787.88	2788 24	2789.18	0.092876	9.28	14.63	32.77	2.24
Reach-1	3.5 333 333	PF。1855年起	128.00	2780.30	2781.28	2781.51	2782.04	0.041984	6,96	18.39	29.28	1.55
Reach-1	3 7 22 27 1	PF。1號監察	128.00	2777,01	2777.99	2778.19	2778.70	0.041823	7.52	21.73	41.91	1.57
Reach-1736	2.5	PE,1844	128.00	2772 00	2772.81	2773.17	2773 83	0.068010	8.13	15.91	31.40	1.93
Reach-1提印	2 4 4 4 4	PF,1词型图式学	213.00	2761.68	2763 46	2764 06	2765.28	0.053087	11.76	23.67	34 94	1.91
	1.5 《神事》			2759 00	2760.11	2760.23	2760.58	0.032801	5.46	39.01	74.45	1.33
Reach-1	1.正相關語	PF.1 湯湯雪	213 00	2753 00	2754 25	2754.61	2755.43	0.048899	9.70	30.35	57.61	1.78

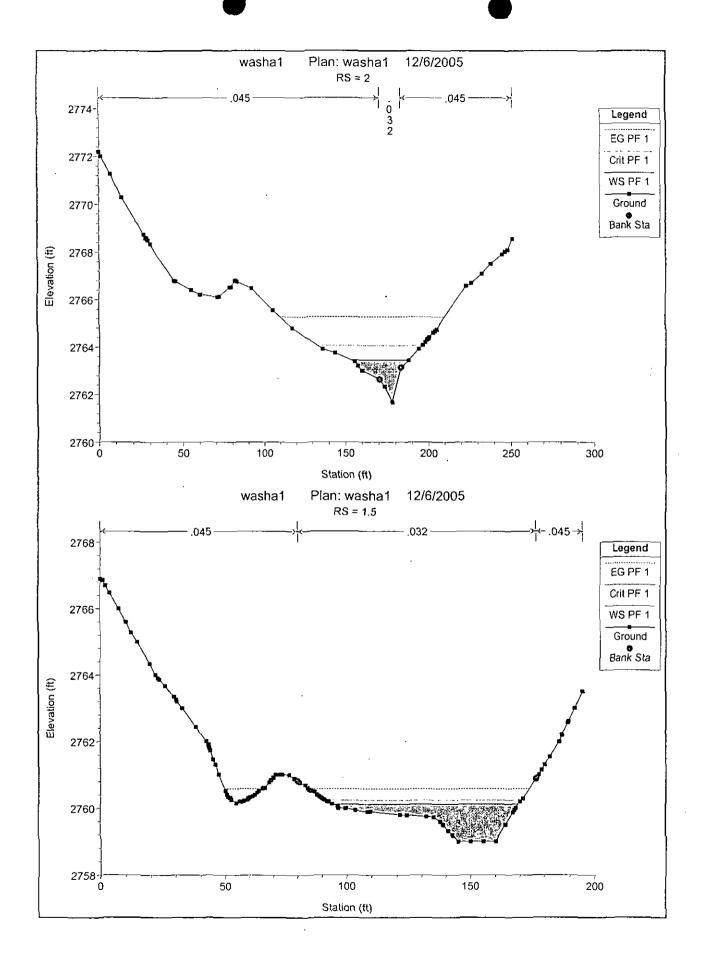
Errors Warnings and Notes for Plan: wasa1

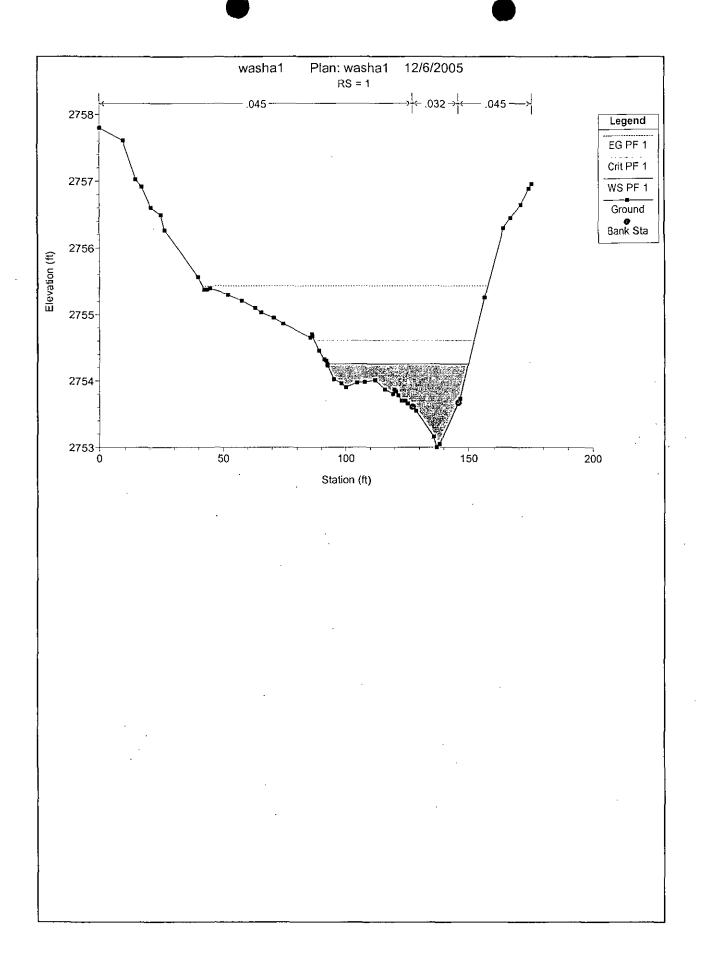
Errors Warn	ings and Notes for Plan: wasa1
Location:	River: RIVER-1 Reach: Reach-1 RS: 6 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
Location:	River: RIVER-1 Reach: Reach-1 RS: 5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 4 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
* .	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
-	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS; 3.5 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS; 3 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 2.5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 1.5 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 1 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.











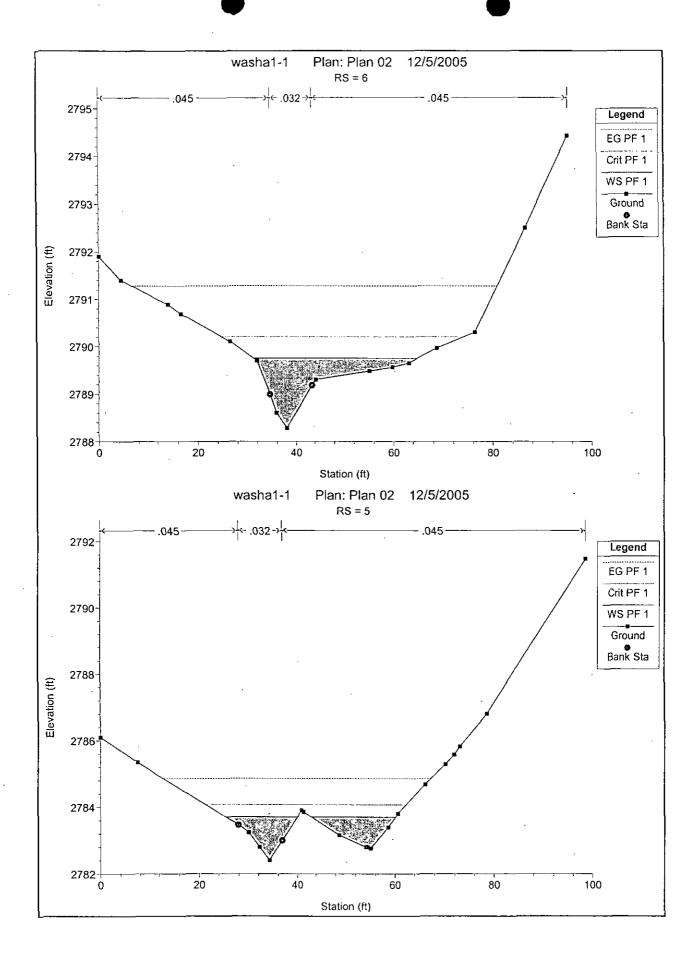
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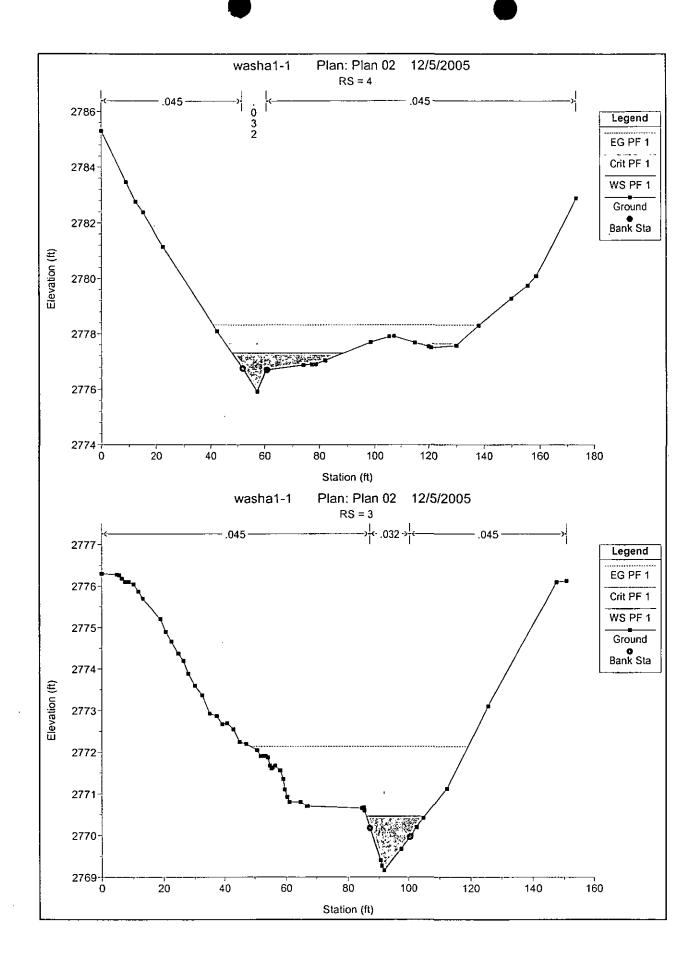
HEC-RAS	Plan 02	River: RIVER-1	Reach: Reach-1	Profile: PF 1

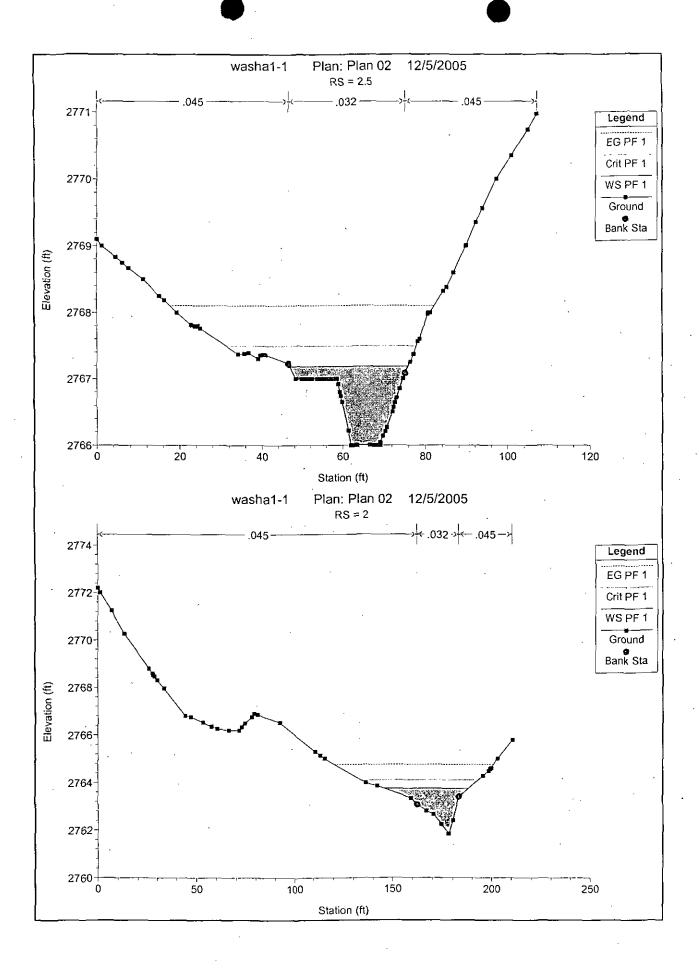
≰ Reach %is	River Sta 🖟	Fix Profile **	∠ Q Total al <sup>7</sup>	£Min Ch Eb	⇒ W.S. Elev.	準Crit W.S語	E.G. Elevid	₿E.G. Slope ::	→ Vel Chnl	∴Flow Area >	Top Width &	Froude # Chl
40亿型"数"员	Januar Paras Jan Kanada Kanada	提供加速支	/漢(cfs)素。/	一卷线(ft)架孔。	性模型(ft)(对自	型器学(ft)的新兴	上海(ft)多项	「製造(ft/ft) 密禁	為多(ft/s)。	学ば(sq ft) 場と	記事(ft)の数	地名中国 (晚天)片
Reach-1覆绿	6 1 1 1 1 1	PF.1要期5抽2	128 00	2788 28	2789.75	2790.21	2791.28	0.050053	10.84	16.83	33.79	1.83
Reach-1	5/#/J####	PF 1= SEF	128.00	2782 40	2783.71	2784 06	2784.85	0.068334	10.34	17,44	32.03	2.04
Reach-110	ATHICK MI	PF 1	128 00	2775.92	2777.31	2777.64	2778.33	0.045358	9.63	20.71	40.76	1,72
Reach-1924	3/445926	PF.1 - Sand	128 00	2759.16	2770 46	2771.03	2772 13	0.052347	10.54	13.08	19 34	1.98
Reach-15%	2.51961418.	PF,1652	128 00	2766 00	2767 18	2767.49	2768 10	0.056771	7.70	16.65	28.99	1.77
Reach-1	2 LAL #4 E	PF。13版等等	213.00	2761.84	2763.74	2764.10	2764.75	0.025862	8 25	29.71	42.13	1.34
Reach-1	1.5	PF.1中部4英	213.00	2759 00	2760.02	2760.24	2760.71	0.061570	6.69	31.84	71.84	1.77
Reach-1	17/5/4/10	PF 11	213.00	2753 00	2754.35	2754 61	2755 20	0.031929	8.48	36.11	59.96	1.47

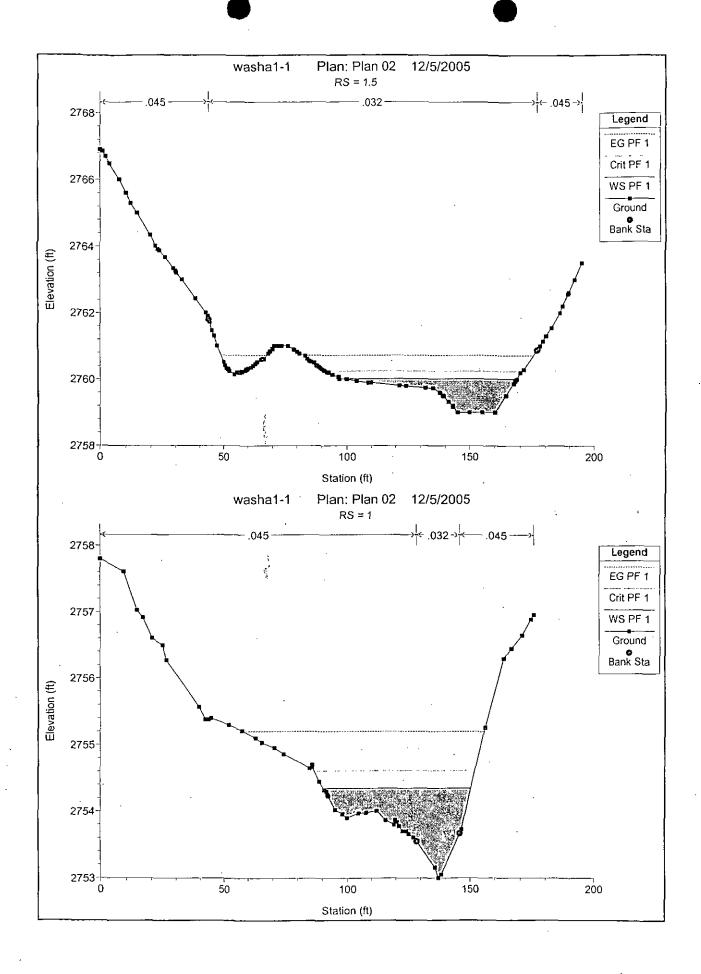
Errors Warnings and Notes for Plan: Plan 02

Endia Waniii	igs and Notes for Plant. Plant 02
Location:	River; RIVER-1 Reach: Reach-1 RS: 5 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 4 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 2.5 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
·	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 1.5 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
<u> </u>	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS; 1 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.









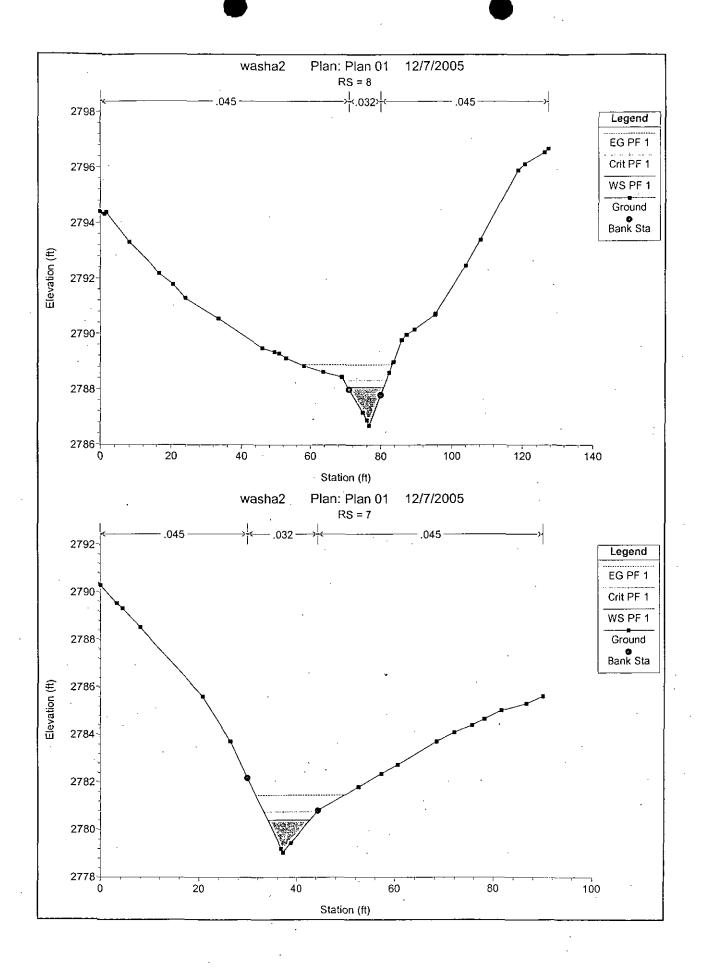
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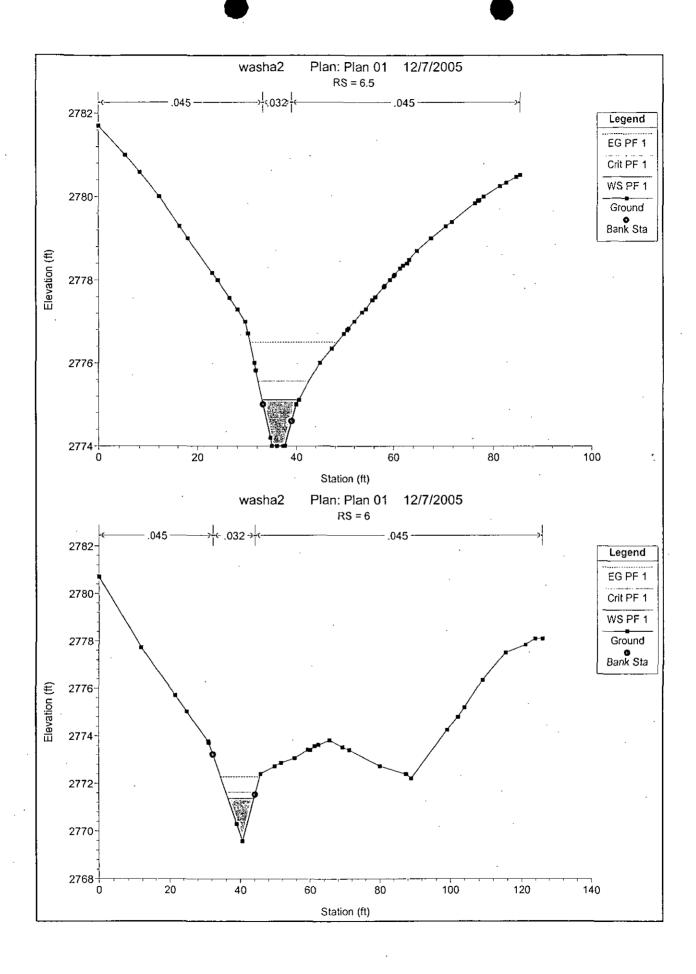
HEC-RAS Plan: Plan 01 River RIVER-1 Reach: Reach-1 Profile RE 1

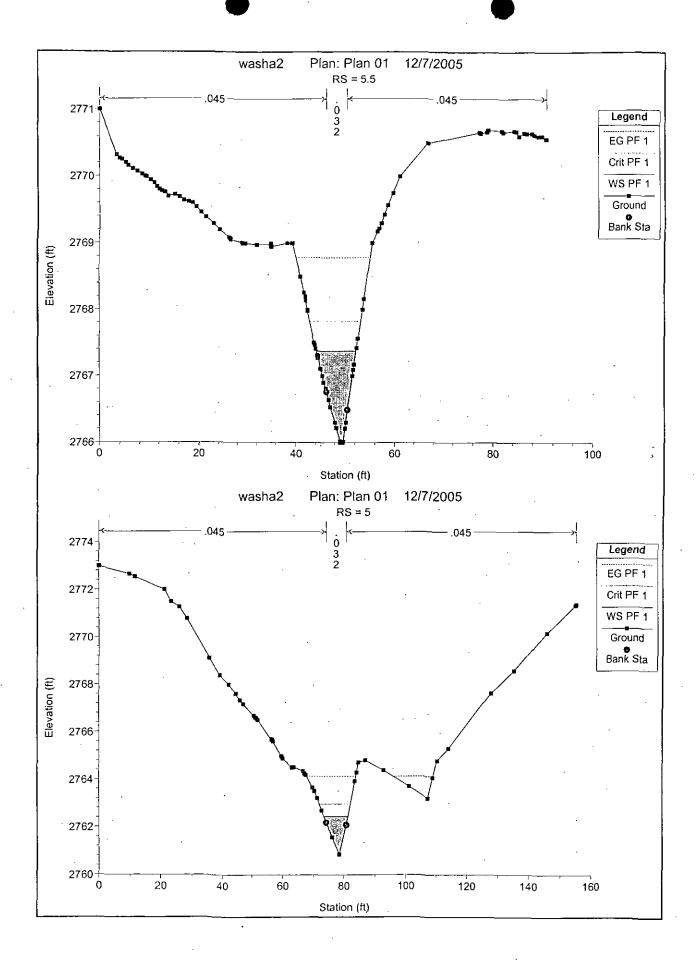
		TOVEL TOTAL	Treatin, Treati									
Reach	River Sta	₃ ﷺ Profile ≔	STO Total	Min Ch Eli.	W.S. Elev	音Crit W.S.影	.∉E.G.º Elev ≇	%E.G. Slope 🖟	涯Vel Chint	Plow Area	Top Width?	্Froude # Chl≫
	\$54.5.TM	S CHESTOSC	(cfs) 🚟	差集(ft) 7等安	高层(ft)上标	為被(ft)協議		銀子(fVft)数字	∰% (ft/s)®%,	ে (sq ft) ইণ্ডি	斯辛拉(ft) 提及F	CANADA TARE
Reach-1號管	8 FE S M S	** PF 155%学0	50 00	2786 70	2788 08	2788 32	2788 89	0.037052	7.25	7.00	10 29	1.47
Reach-1	7.345.850	PEIIU基础	50.00	2779 03	2780.41	2780.75	2781.45	0 054930	8 20	6 09	8 86	1.74
Reach-1 🚟	6.5 沙路 (社)	性 PF.1类型6类	50 00	2774.00	2775.12	2775.56	2776.49	0.053771	9.50	5,51	7.56	1.78
Reach-1奇。	8 th 1000 feet	年 PE 1位标准等	50.00	2769 58	2771.38	2771,63	2772 27	0.035829	7.60	6.58	7.35	1,41
Reach-1	5.5	》PF。1954课稿	50.00	2766 00	2767.37	2767 82	2768.78	0.045155	9 98	5,95	8.05	1.70
Reach 1	51,500 10	到PE1透透透	65.00	2760.85	2762.45	2762.98	2764.14	0 060416	10.47	6 36	7.80	1 89
Reach-1 💝 🏋	41.	PE.1	65 00	2754.30	2756.01	2756.26	2756.93	0 034820	7,73	8.44	9.82	1,44
Reach-1编辑	35 40.23	C PE-1888	65 00	2749.02	2750.53	2750.80	2751.42	0.041151	7.55	8.61	11.61	1.55
Reach-1 📆 🙏	2000年代日	2 PF, 1134 214	100.00	2742.10	2743.47	2743 82	2744.48	0 040885	8 14	12.82	19 90	1.59
Reach 1F	1.5	J PF.1: Sign	100.00	2739.00	2740.23	2740.46	2740.93	0 023444	6 85	16 97	32.42	1 24
Reach-1	1 ACCOUNT FOR THE PARTY OF THE	PF.15	100.00	2733.30	2734.56	2734.98	2735.97	0 081796	9.53	10.68	20.42	2.14

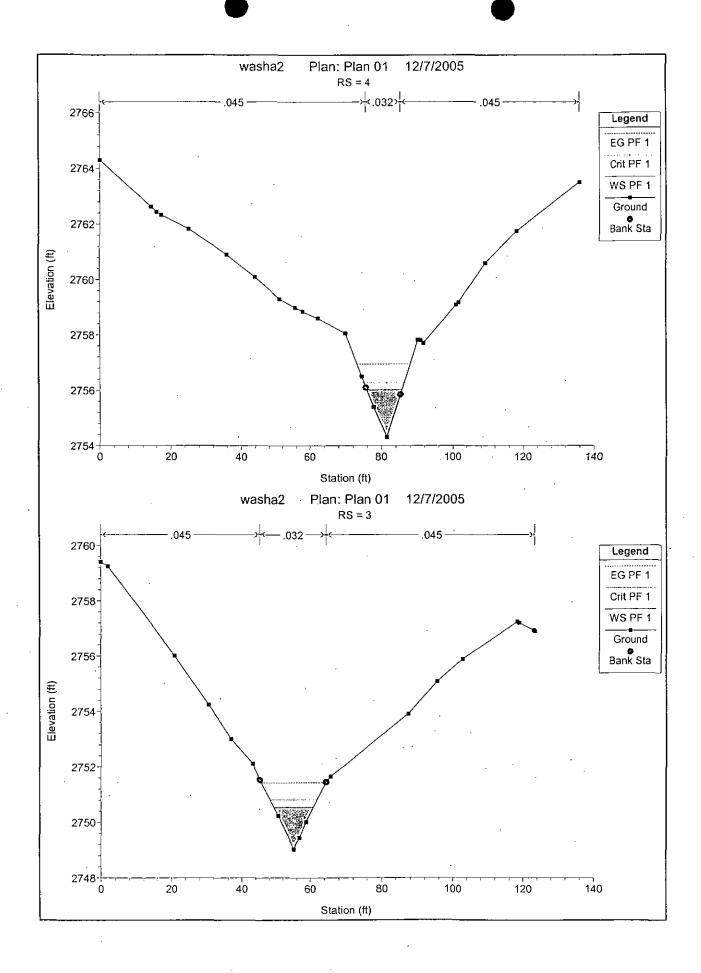
Errors Warnings and Notes for Plan: Plan 01

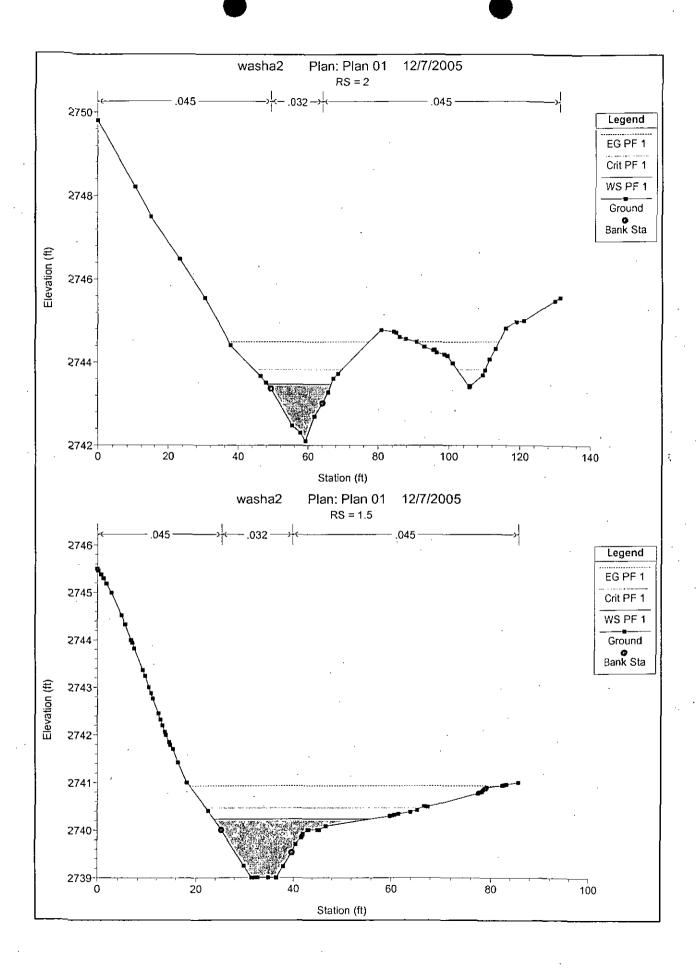
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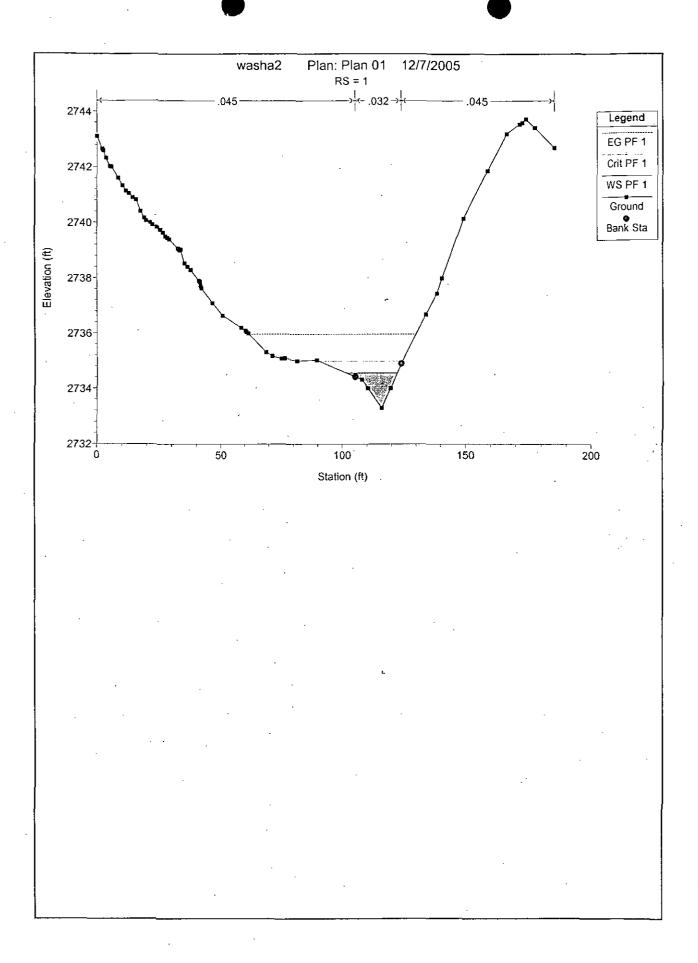










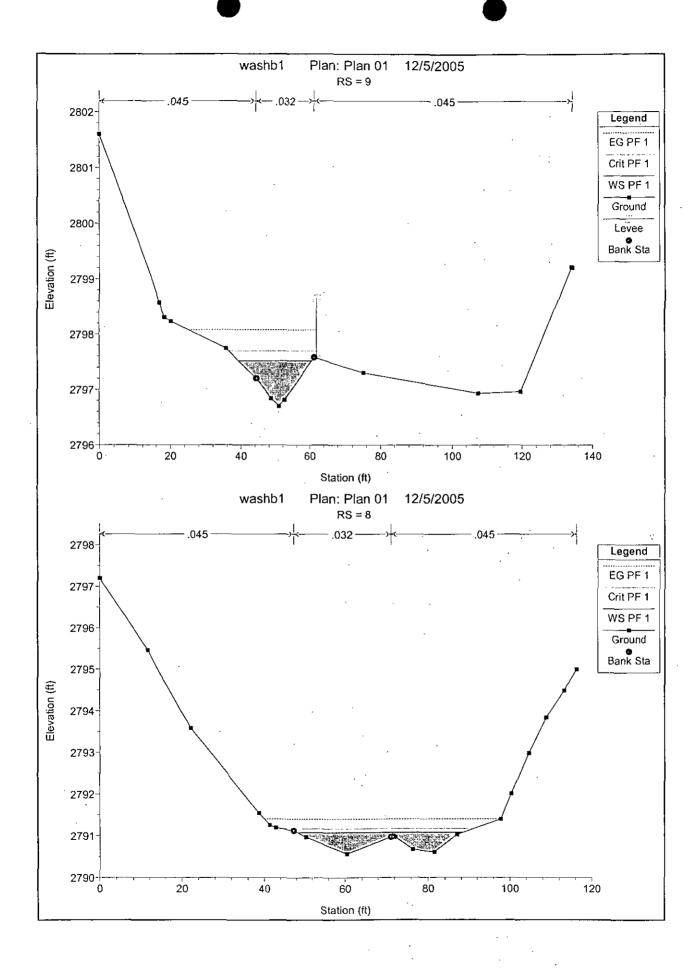


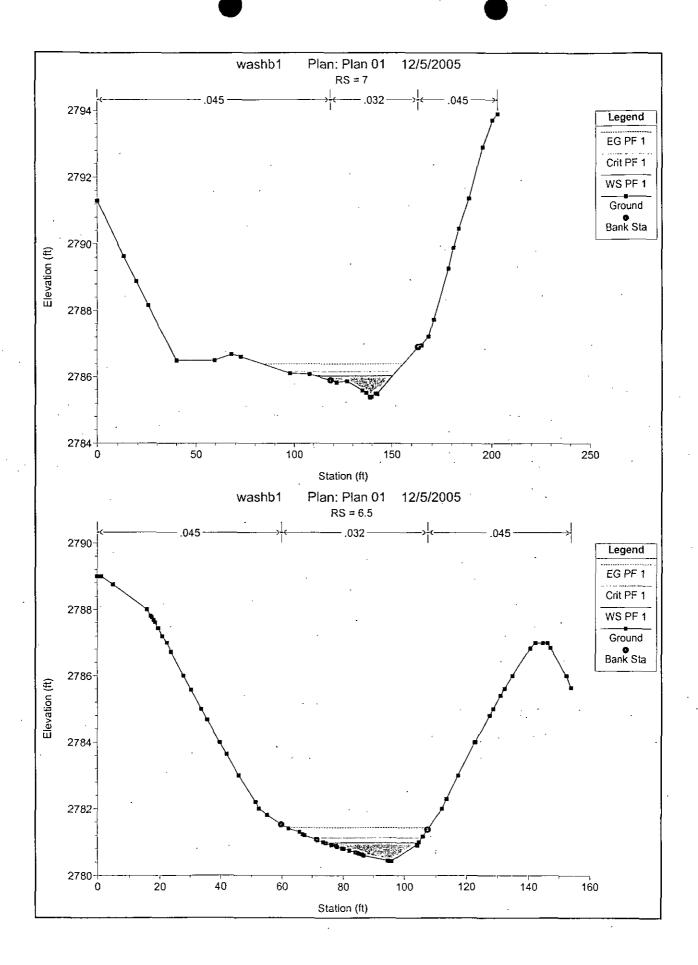
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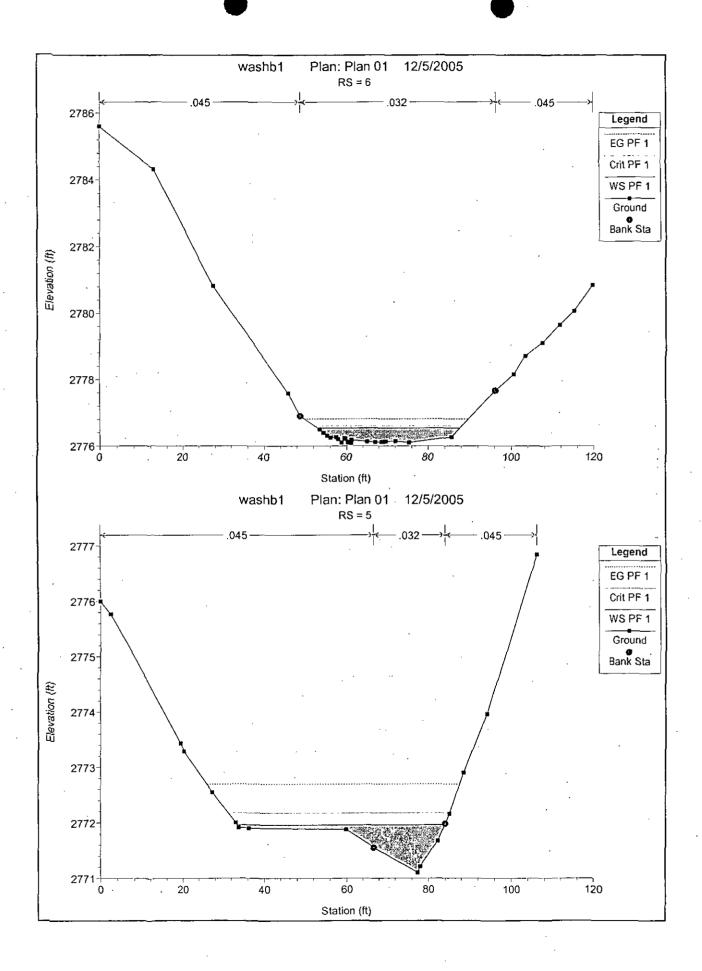
HEC-RAS Pla	in: Plan 01 F	River: RIVER-1	Reach: Reac	h-1 Profile: F	F !							
- <sup>ag</sup> Reach ≸a	. River Sta €	***Profile 語	₹ Q Total 📆	黎Min Ch El %	. W.S. Elev ₩	《Crit W S.零	⊮E.G."Elev⊠	E.G. Slope 13	₩.Vei Chnl 8	⊈Flow Area 🖺	≨∗Top Width ≩	를 Froude # Chl 등
THE PROPERTY AND ADDRESS.	第6次3414 英	<b>的。我們對於家</b>	MG (cfs) √+	servy (ft) 品能 。	运行(ft)等主流	品层(ft)制造	排犯(ft)顯原	Con (ft/ft) S.At	ેલે (ft/s)ફેક્સ	. 🖈 (sq ft) 🕸 🗟	斯姆(ft)李雪	are of the
Reach-1激展	9 & M. (E)	PF_1ttblase	50.00	2796.70	2797 52	2797.69	2798.08	0 045030	6.09	8 74	21.32	1.54
Reach-13	Br. Therm	PF.13866.93	50.00	2790.57	2791.08	2791.17	2791.40	0 059767	4 94	11.57	40.41	1.62
		PF,1 30		2785.37	2786.03	2786 15	2786.38	0 048169	4 80	10.80	39.56	1,49
Reach-1	6.5 22 (6.8)	PF.1550	50.00	2780 43	2780.99	2781 13	2781.43	0.065773	5.37	9.31	30.67	1 72
Reach-1	6場數分經濟	PF.1TEE	50.00	2776 08	2776 54	2776 59	2776.81	0.033697	4.19	11.94	34,58	1.20
Reach-1	5 % BESS	PF-1型製造電	75.00	2771.10	2771.96	2772.17	2772.70	0 051225	7.15	13.13	50.55	1.68
		PF.1至 議院		2768.38	2768.93	2769 06	2769,40	0.048075	5 47	13.70	34.74	1.54
Reach-1990	4 2 4 1 3	PF.11.	75.00	2764.68	2765.48	2765.67	2766.11	0.041720	7.30	13.92	30.42	1.56
Reach-1	3 70 7 12 70	PF.1 14 7573	75 00	2759.60	2760.56	2760.78	2761.28	0 041947	6.93	. 11.63	23 08	1 55
		PE-1 TARRES		2754.10	2754.83	2755.00	2755 40	0.049622	6.14	12.78	32 98	1.50
	Laboration Street, Co. of	1 A Sept 25 (15 o. 15)										

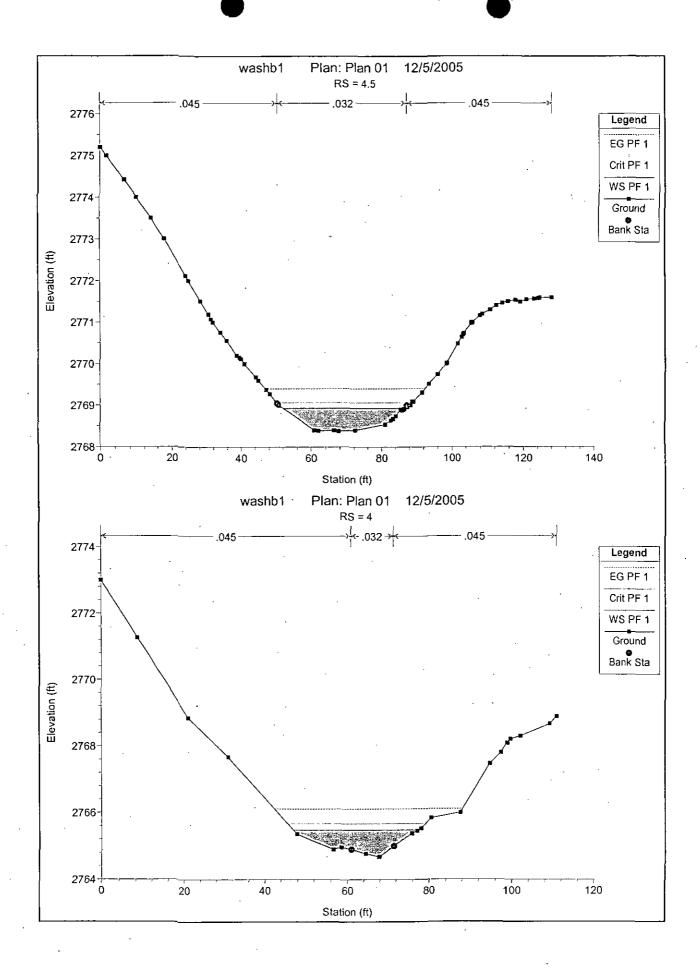
Errors Warnings and Notes for Plan : Plan 01

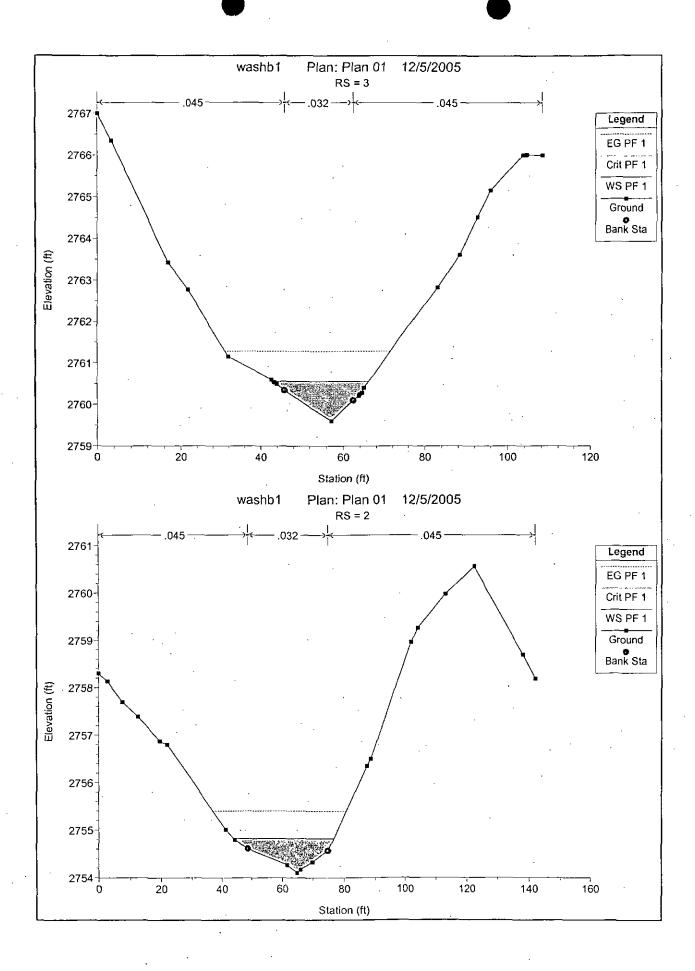
	ings and roles for Flatt of
Location:	River: RIVER-1 Reach: Reach-1 RS: 9 Profile: PF 1
Note:	Multiple critical depths were found at this location. The critical depth with the lowest, valid, water
	surface was used.
Location:	River: RIVER-1 Reach: Reach-1 RS: 8 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 7 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 6.5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 6 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
_ocation:	River: RIVER-1 Reach: Reach-1 RS: 5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 4.5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 4 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
_ocation:	River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
_ocation:	River: RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1
Naming:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
_ocation:	River: RIVER-1 Reach: Reach-1 RS: 1 Profile: PF 1
Narning:	Divided flow computed for this cross-section.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.

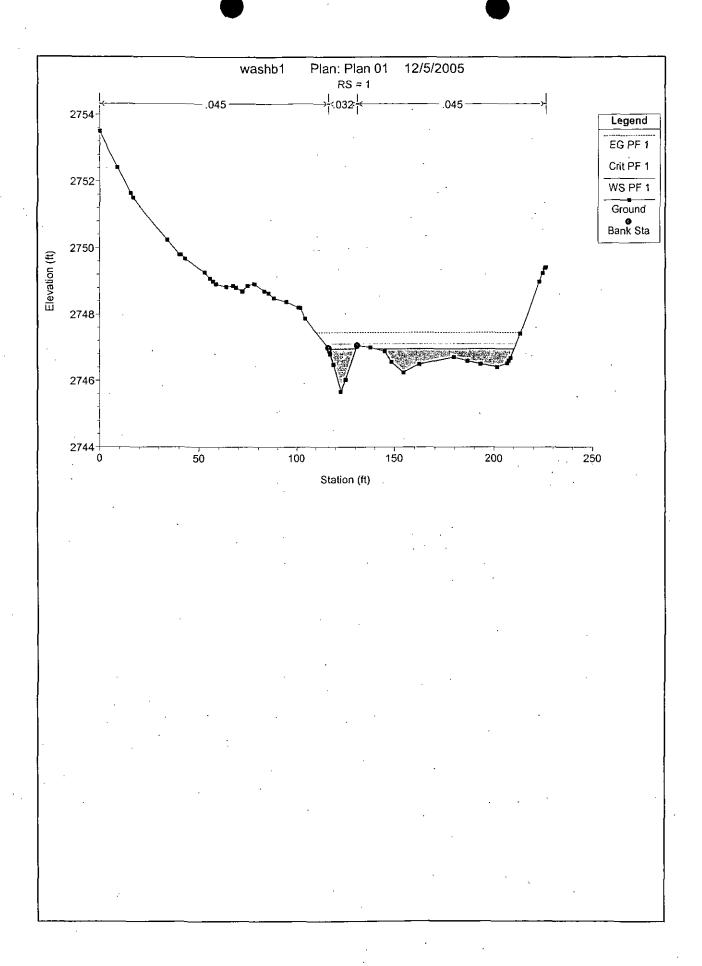










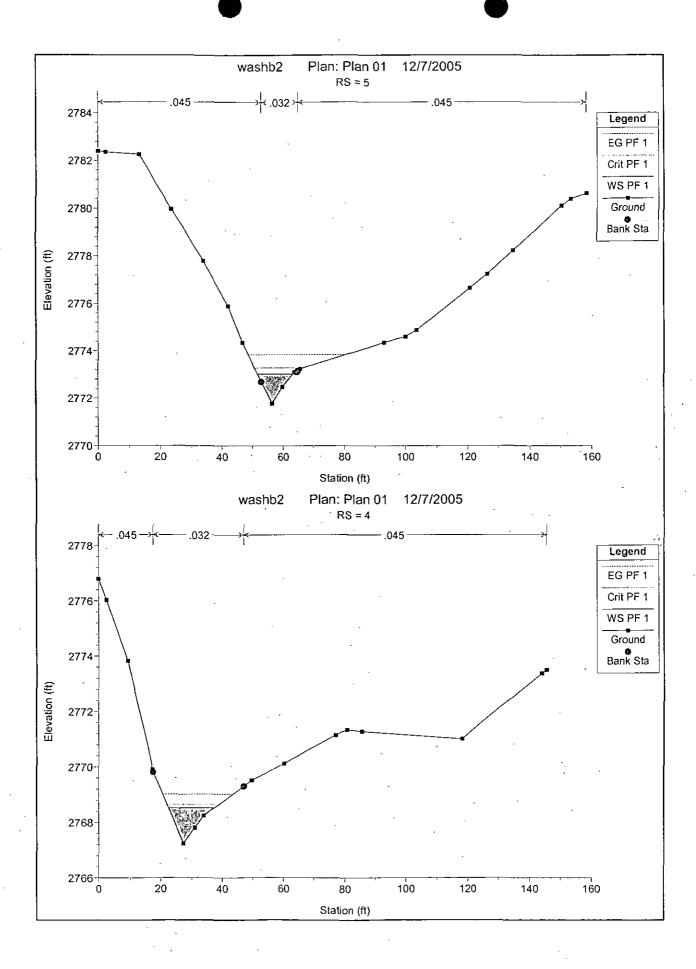


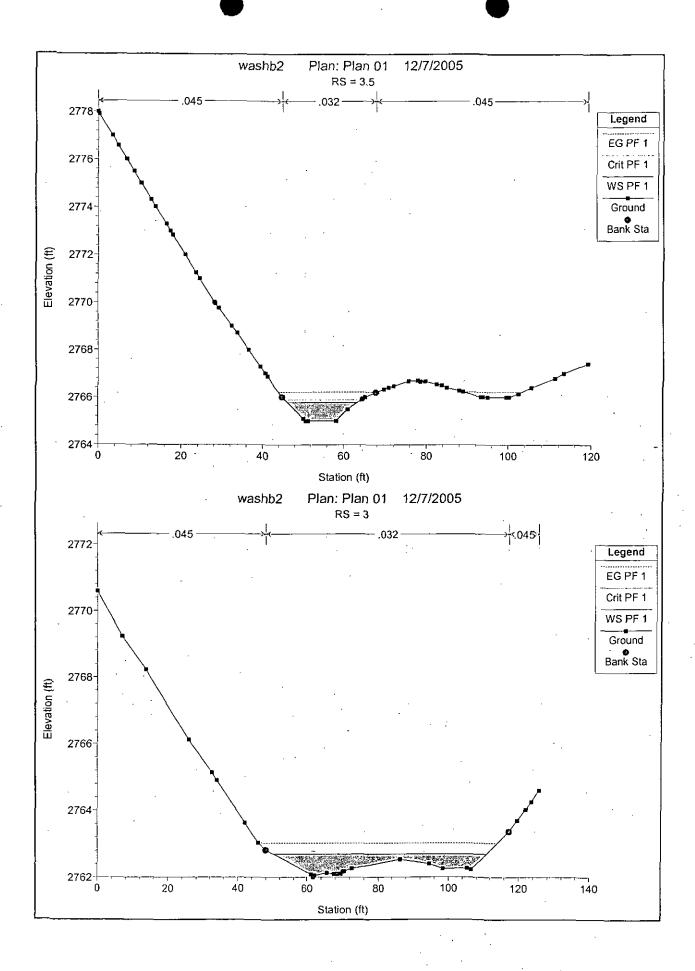
### WASH BZ

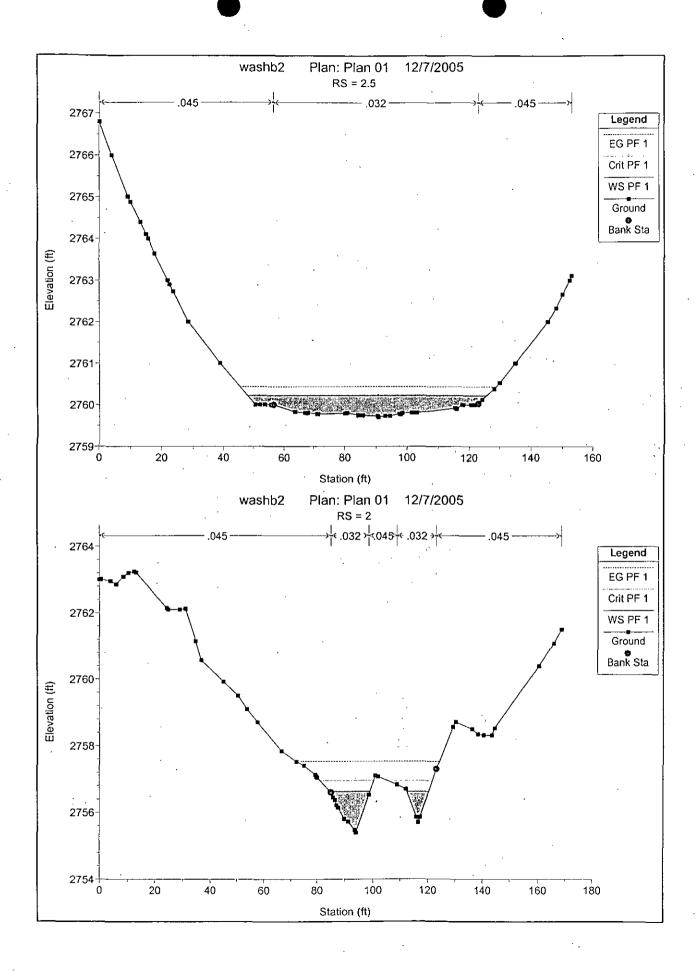
HEC-RAS Pla		River: RIVER-1	Reach; Reac									
Reach	# River Sta	∰ Profile ?	G Total	@Min Ch El ₹	W.S. Elev	Crit W.S.	∤ E.G. Elev.	& E.G. Slope ≗	€Vel Chnl,%	% Flow Area is		Froude # Chl
1418 47	PARTY	LOW THE PARTY	(cls)	2巻1(ft)19.85	解例(ft) 活蛋	500 (ft) 654	· 豫 (ft) 計論	27 18 (ft/ft) 28%	流流(ft/s)本記	(1277 (sq ft) 135	逐。(ft)等档	A PARTY N
Reach-1 (at 1)									7.27	7.03		
Reach-1				2767 23	2768 51	2768.64	2769.01	0.030968	5.67	8.82	15 02	1.30
Reach 1955				2765.00	2765 78	2765.88	2766.22	0 029393	5.31	9.41	17.14	1.20
Reach-1				2762.00	2762.69	2762.78	2763.02	0.037825	4 56	21.91	60.90	1.34
Reach-1	2.5	PF.15	100.00	2759,70	2760.22	2760.24	2760.43	0.022853	3.74	27.78	77.79	1.00
Reach-1	2	PF 1522	100,00	2755.39	2756.63	2756.95	2757.52	. 0.051075	7.59	13.19	22,30	1.73
Reach-127 Ed				2753.00	2754.46	2754.68	2755.16	0.021251	7.80	20 25	34.63	1.22
Peach 14701				2745 67	2746.97	2747 11	2747 43	0.044086	7.32	38.04	84.83	1,59

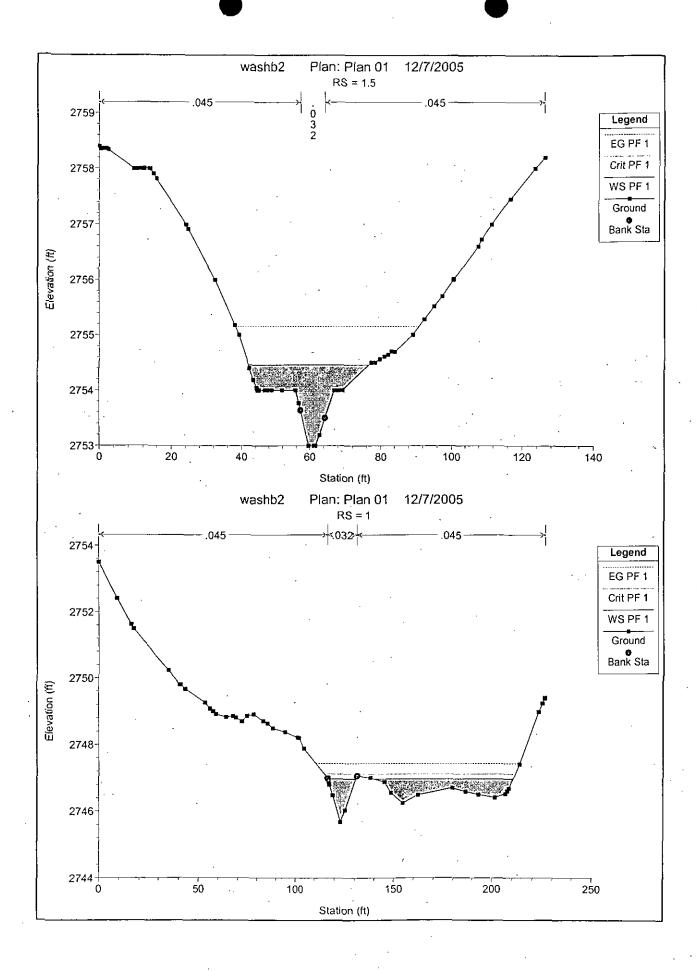
Errors W	'arnings	and	Notes	for	Plan:	Plan	01
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Location:	River: RIVER-1 Reach: Reach-1 RS: 4 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 3.5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
`	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 2.5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1
Warning;	Divided flow computed for this cross-section.
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections,
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RiVER-1 Reach: Reach-1 RS: 1.5 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 1 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.









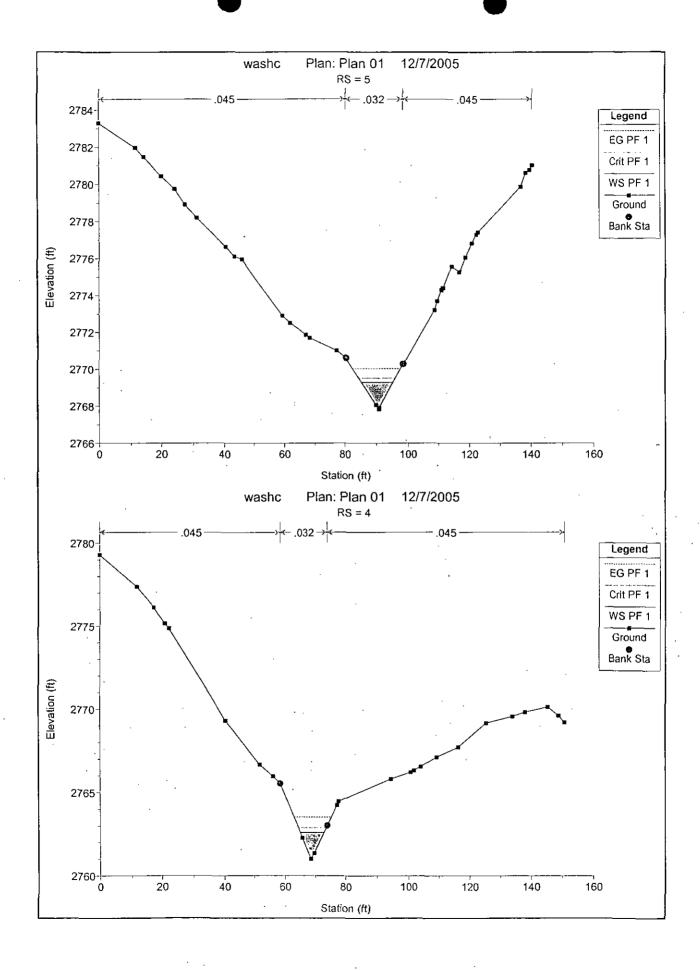
# WASHC

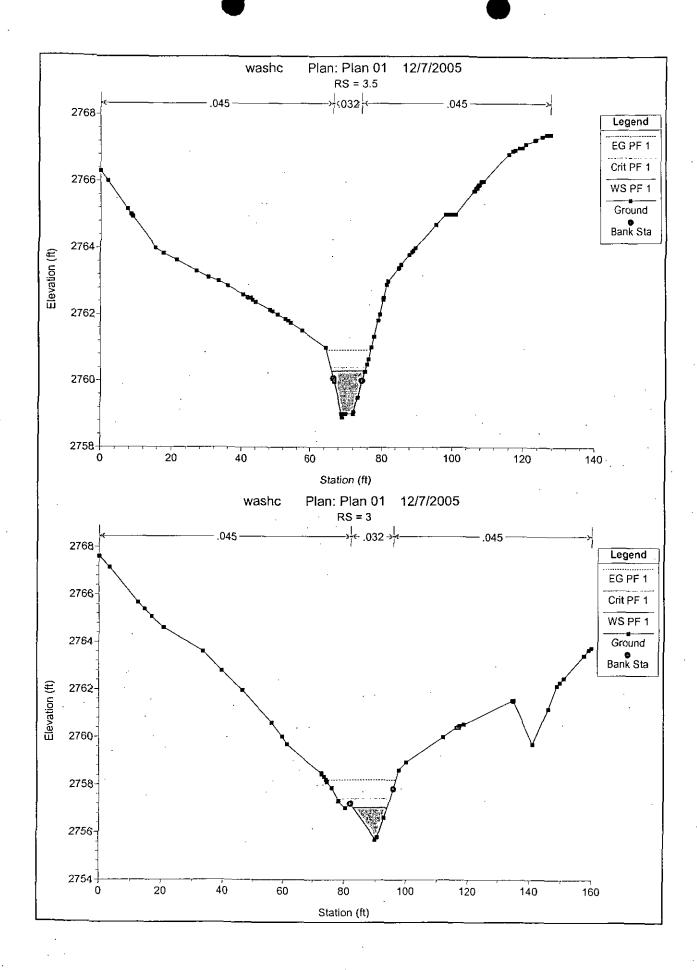
HEC-RAS	Plan: Plan 01	River RIVER-1	Reach: Reach-1	Profile PF 1

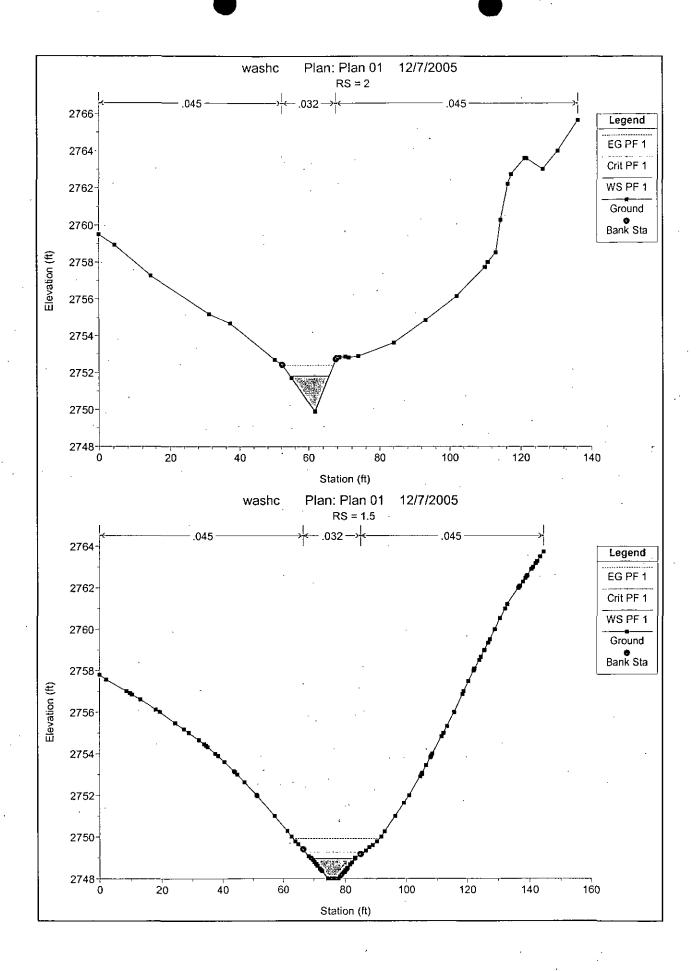
Reach 🥳	∄River Sta	** Profile	<sup>3</sup> Q Total : ?	3 Min Ch Eti	≾W.S. Elevi	*Crit W.S.	≝ E.G. Elev ä	&E.G. Slope	©Vei Chnl ജ	1 Flow Area ₽	Top Width	∄Froude # Chl¾
1655年1875年11	<b>水炉和新</b> 柜		(cls) 🐺 .	部位(h)定理	以第三(h)空物道	美沙兰(ft)流言	⊒#*(ft):⊒:	凝基(lvft)至增	透点(tvs)/砂り	型数(sq ft) 型	是含在(ft)。多种	<b>扩展的支撑</b>
Reach-1	5 發媒地交換媒	PE.15种验验	50.00	2767 82	2769,29	2769.50	2770.03	0.036028	6.90	7.25	10 04	1.43
Reach-19-18	4号高级图图	PF 1 型砂學期	50.00	2761.00	2762.59	2762 87	2763 50	0.039393	7.65	5.54	8.03	1.49
Reach-1%	3.5 溢光	PF.1級規學的	50.00	2758.90	2760.30	2760 39	2760.92	0.019976	6 32	8.08	9.29	1,11
Reach-11	311 1000	PF。13学/第2	65.00	2755 68	2757.04	2757.41	2758.20	0.059581	8 65	7.52	11.30	1 84
Reach-1	25-27-2	PE_1320 51.6	65 00	2749.90	2751.82	2751.88	2752.40	0.019726	6.09	10,67	11.11	1 10
Reach-136	1.5	PF.1%	65 00	2748.00	2748.96	2749.27	2749.92	0,058207	7.87	8 26	13.81	1.79
Reach 1:24	1/1/2/27/2000	PF_11555555	83 00	2743.99	2745.61	2745.77	2746.31	0.026195	6.71	12,44	15.76	1.27

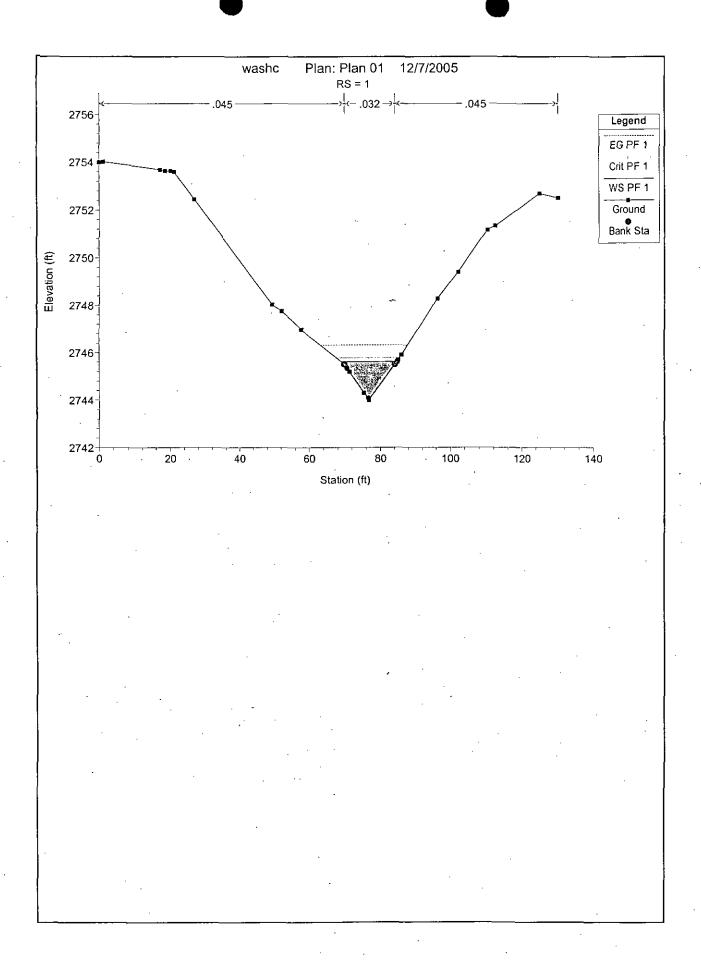
Errors Warnings and Notes for Plan: Plan 01

C11013 • VAIII	ings and reces for half .1 fail of
Location:	River: RIVER-1 Reach: Reach-1 RS: 4 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 3.5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections,
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 1.5 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
<u> </u>	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 1 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.









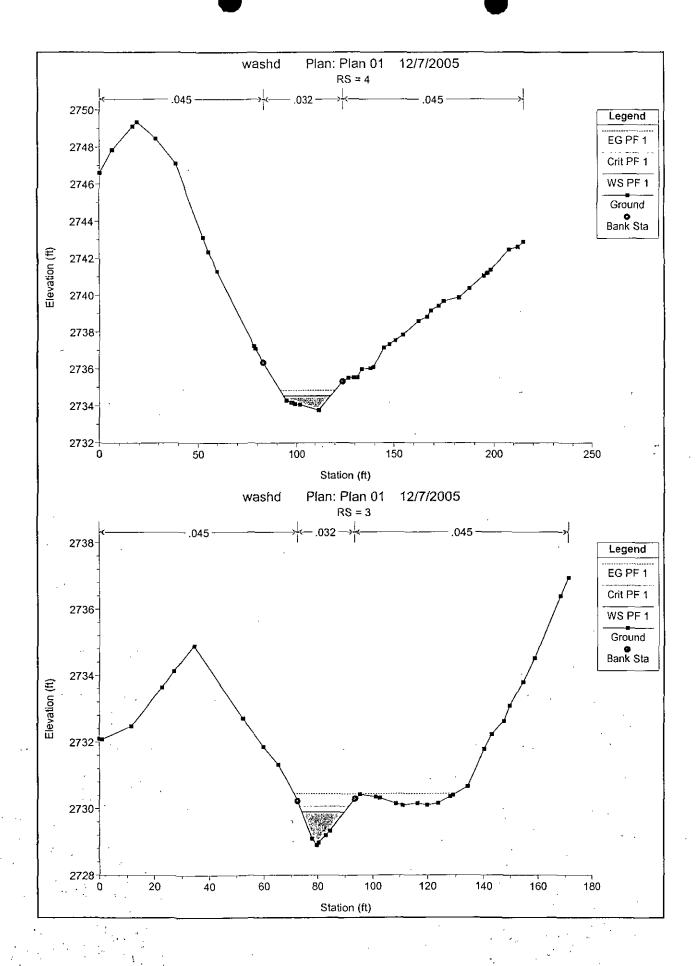
### WASH D

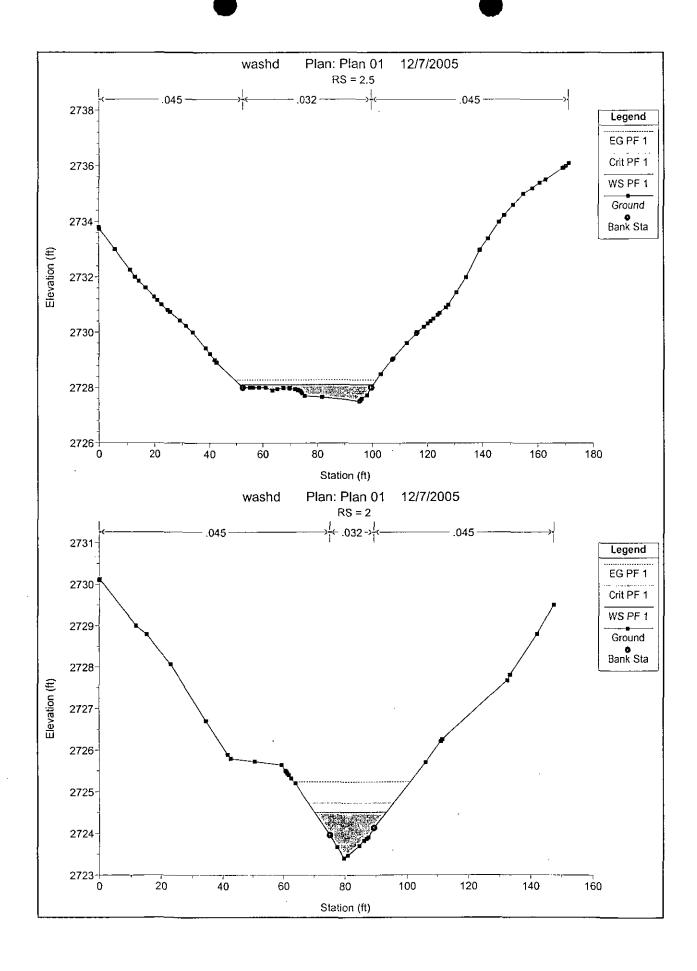
HEC-RAS Plant Plan 01 River: RIVER-1 Reach: Reach-1 Profile: PE 1

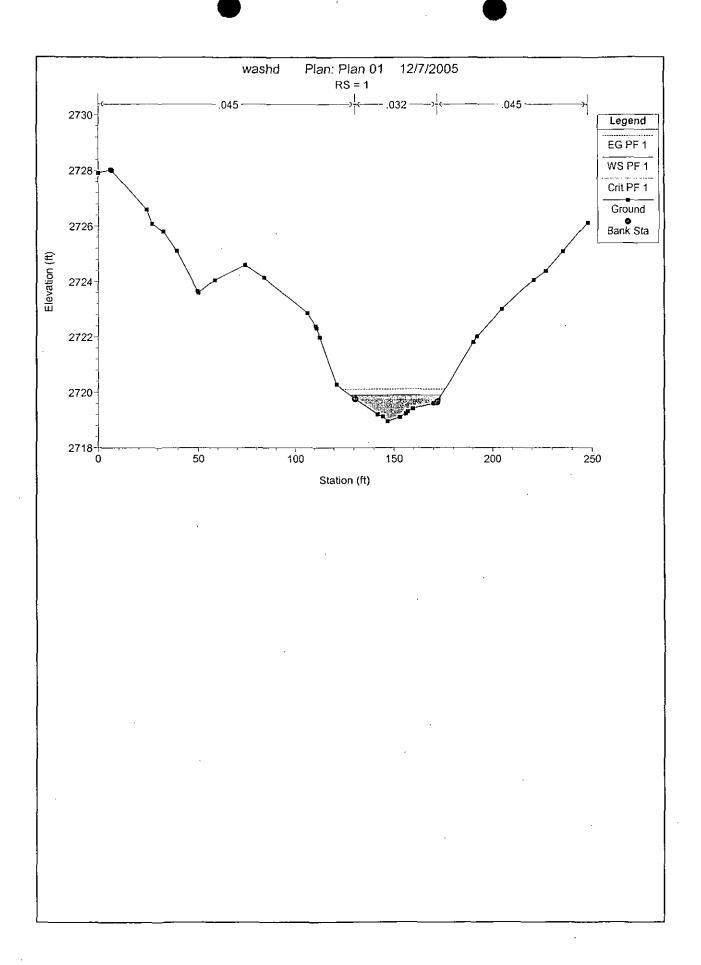
Reach 💨	:- River Sta	《Profile 論論	M.Q Total与	É Min Ch El%	≨W.S. Elev.®	氯Crit W.S.종	됐E.G. Elev	≨ E.G. Slope ⅓	海Vel Chnlឡ	Flow Area	∰Top Width =	€ Froude # Chi 🗒
TO PROPERTY	TA SPANIE	CTCHARCE:	ि ग (cfs) 🙉	:20 (ft) war	·李泰 (ft) 慰証器	新漢(ft)統領	金·(ft)安全	遊泳 (lvft) 強乙	禁止(fl/s)發展	* (sq ft)	表述(ft)预算	MARKE SAL
Reach-1.	4/4/4/5/5	PF. 1 11 11 11	50.00	2733.77	2734.54	2734.58	2734.84	0.025006	4.39	11.38	24.53	1.14
Reach-1				2728.90	2729.90	2730.06	2730 45	0.038474	5 94	8.42	15.86	1.44
Reach 1350				2727.51	2728 11	2728.12	2728.28	0.023407	3 32	15.16	49.07	1.03
Reach 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				2723 41	2724.51	2724.73	2725.24	0 030841	7.02	13.55	23 22	1.38
Reach-1試練	12dspage	PF_1被编码数	85.00	2718.95	2719.90	2719.85	2720.11	0.014008	3.70	23.30	45 93	88 0

Errors Warnings and Notes for Plan : Plan 01

Location:	River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1									
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.									
	This may indicate the need for additional cross sections.									
Location:	River: RIVER-1 Reach: Reach-1 RS: 2.5 Profile: PF 1									
Warning;	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.									
	This may indicate the need for additional cross sections.									
Location:	River: RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1									
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for									
	additional cross sections.									
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7									
	or greater than 1.4. This may indicate the need for additional cross sections.									
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.									
	This may indicate the need for additional cross sections.									
Location:	River: RIVER-1 Reach: Reach-1 RS: 1 Profile: PF 1									
Note:	Hydraulic jump has occurred between this cross section and the previous upstream section.									







## WASH EI

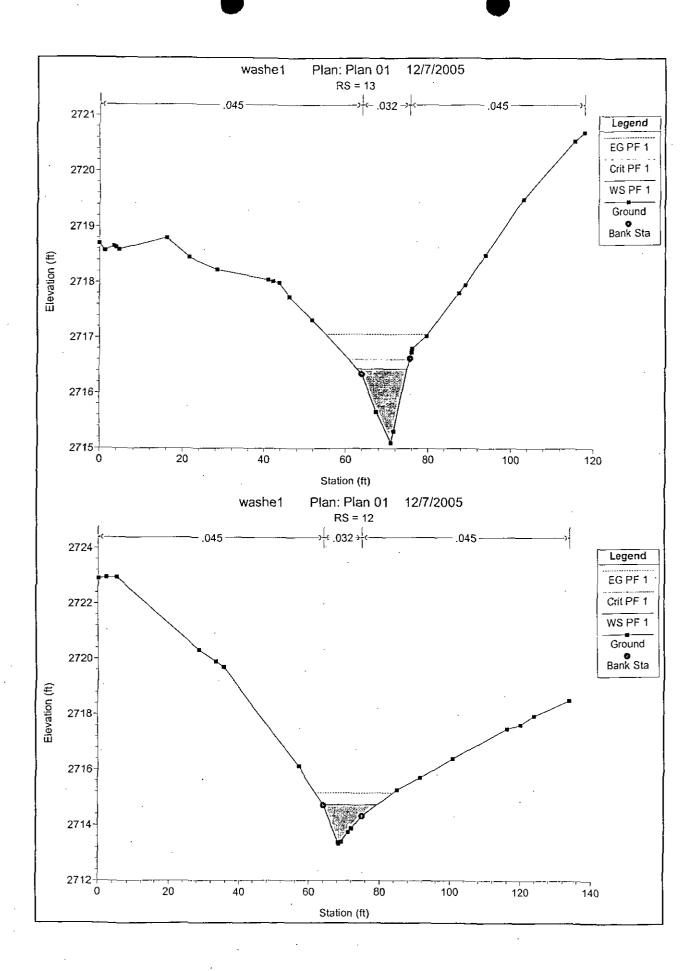
HEC-RAS Pla	an: Plan 01 F	River: RIVER-1	Reach, Reac	h-1 Profile: P	F 1							
Reach &	River Sta	Profile 1	Q Total	3 Min Ch Ei≱	W.S. Elev.	Crit W.S.	E.G. Elev		"≂Vel Chnl'±	Flow Area ≝	Top Width -	# Froude # Chi
224 x25 x 4x	2007503	25:2 <b>7</b>	🤼 (cls), 🚉	到海(ft)严闷	福運(ft)参覧	· (ft) 物質	ತ್ರಕ್ಕ್ (ft) ವ್ಯ. ೆ	系图(ft/ft)图题	<b>建定 (fl/s) を</b>	sizi (sq ft)	10 1 (ft)	温が強いない
Reach-1	13號學學學	PF,1	50.00	2715.10	2716 43	2716 60	2717.06	0.030018	6.34	7 93	12,10	1.32
Reach-1 36	1212512	PF_102966	50.00	2713.32	2714.73	2714.77	2715.16	0.017142	5.31	10.05	15 35	1.02
Reach-1	1159/李公元第	PF.1:3000	50.00	2711.10	2712.13	2712.36	2712 87	0.045013	6.89	7.38	13 63	1.58
Reach-1	10 3 7 3 2 1	PF 1	50.00	2708.30	2709.94	2710.07	2710 51	0 018336	6,16	9.08	14.42	1.07
Reach-17.76	9:HQU-H	PE.1 经等品证	50.00	2706.40	2707 58	2707.84	2708.40	0.040012	7.29	7.00	11.58	1.52
Reach-15-35	83,4145	PF_1deting	50.00	2703.40	2704.77	2705, 10	2705.82	0.046780	8.23	6 21	9 25	1 65
Reach-1 ##	7-20-50-50	PF 145、16型	50.00	2701.44	2702.70	2702.85	2703 29	0.030785	6.17	8.11	12.04	1.32
Reach-1	6 3 1 1 1 E	PE.1 令页数	50.00	2700.00	2701.17	2701.35	2701.79	0.036001	6.31	7.92	13.05	1 42
Reach-165	5 19.14.7 19.5	PF.1	110.00	2696.59	2698.30	2698.33	2698 80	0,017684	5 71	19.43	23.16	1 06
Reach-1275	4 2 45 5	1 PF.1% (5%)	110.00	2692.81	2694.46	2694.74	2695.37	0.030727	7.70	14 71	19.32	1.40
Reach 1 66 2	3.5	PF.1 (50)	110.00	2692.00	2692.95	2692.96	2693.32	0 014354	5.00	24 98	39 27	0.95
Reach-1	3 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	PE.1	133.00	2689.21	2690.87	2691.19	2691.67	0 023564	7.41	21.04	33 22	1.26
Reach-1强胀	2	PE.1%	251.00	2679.83	2681.78	2682.26	2683 26	0.033851	9.83	26.57	26.60	1.54
Reach-1	15956.571479	PF-19-14-14-1	251.00	2677.00	2679.28	2679.50	2680.16	0.016076	B.21	40.37	39.91	1.10

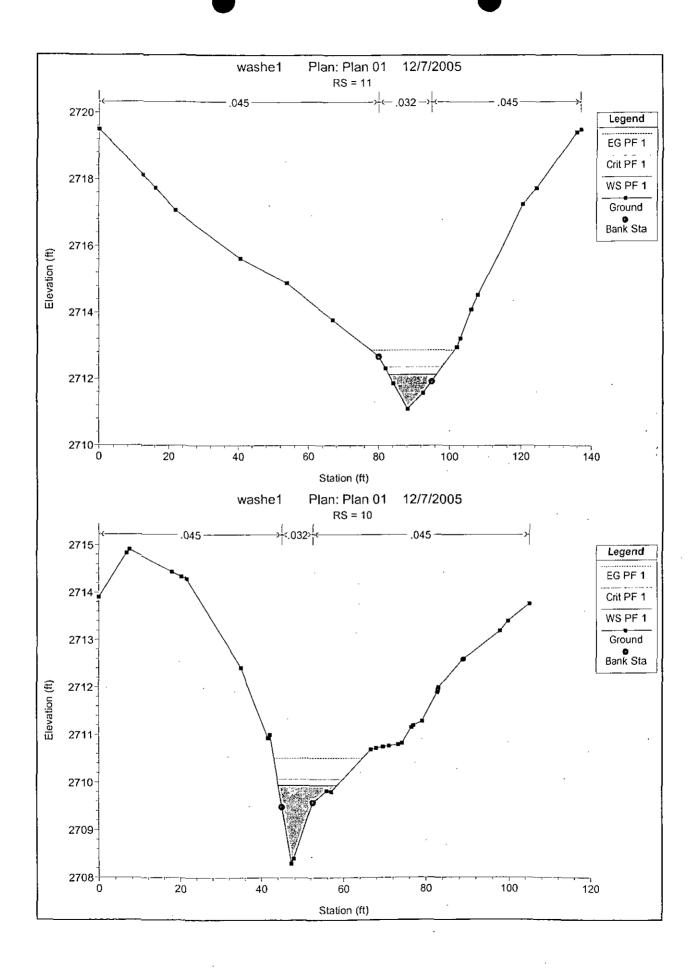
Errors Warnings and Notes for Plan : Plan 01

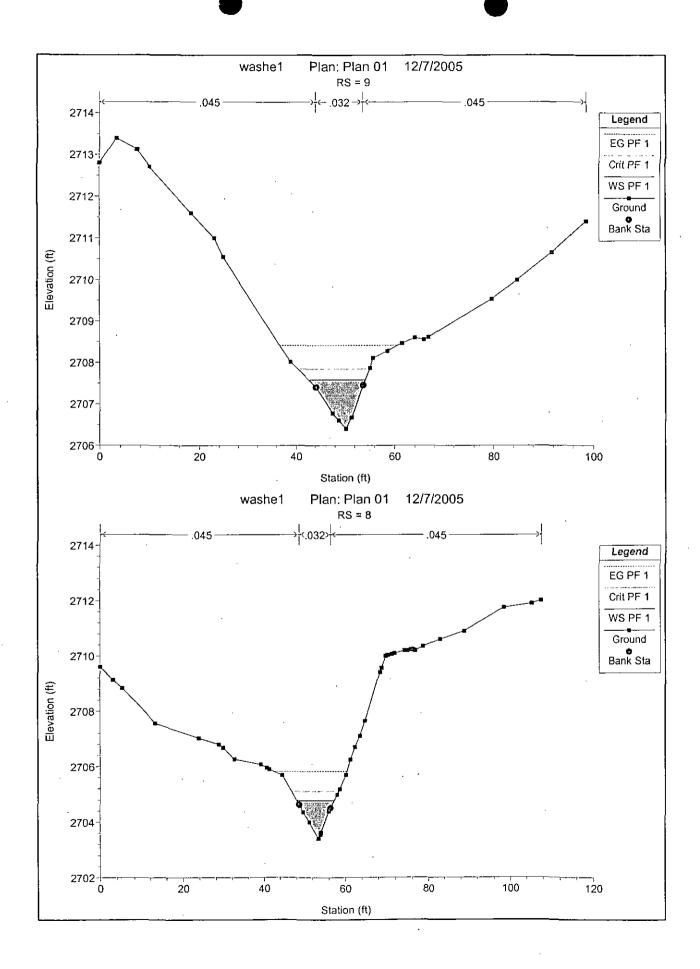
Errois warn	lings and Notes for Plan : Plan 01
Location:	River: RIVER-1 Reach: Reach-1 RS: 12 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 11 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 10 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 9 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
· · · · · · · · · · · · · · · · · · ·	This may indicate the need for additional cross sections.
Location:	
Warning:	
vvarring.	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
Location:	This may indicate the need for additional cross sections.
	River: RIVER-1 Reach: Reach-1 RS: 7 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 6 Profile: PF 1
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
	selected the water surface that had the least amount of error between computed and assumed
	values.
Warning:	The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 5 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 4 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River; RIVER-1 Reach: Reach-1 RS: 3.5 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
_ocation:	River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
Training.	conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7

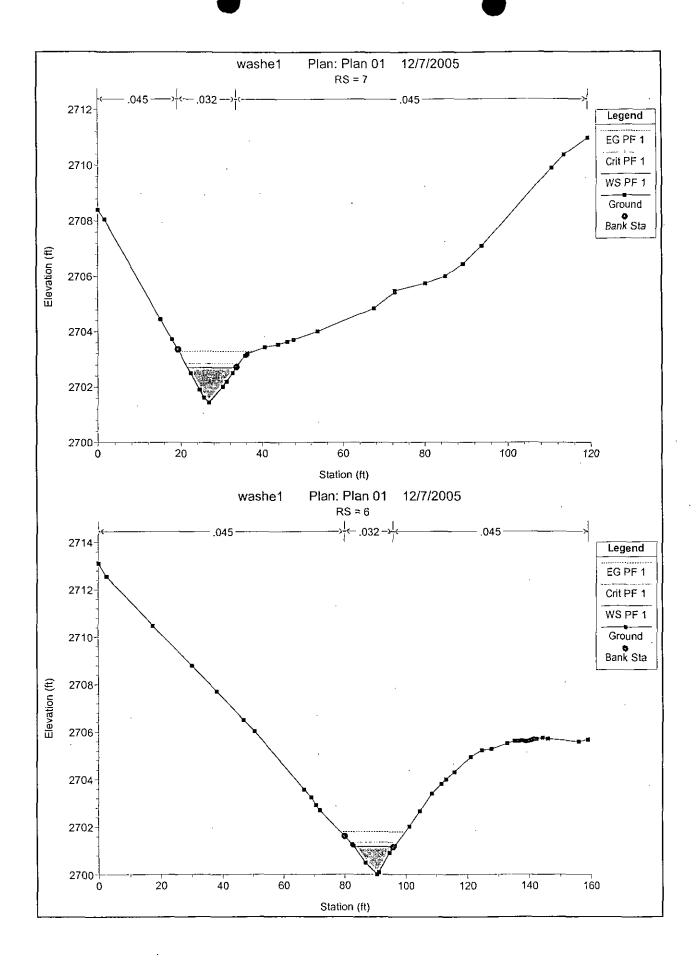
Errors Warnings and Notes for Plan: Plan 01 (Continued)

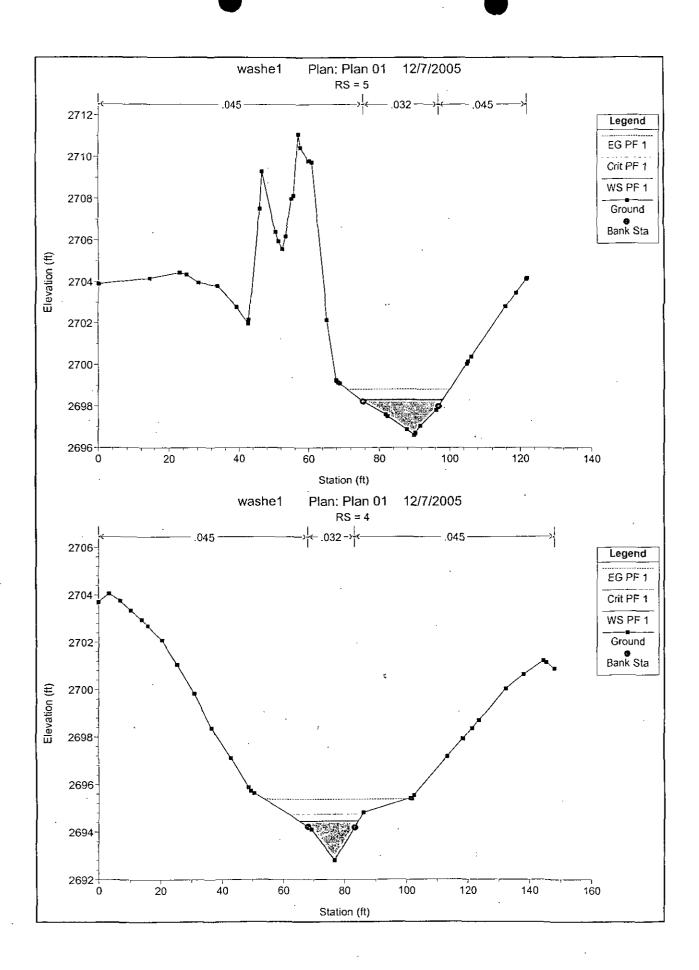
·· <del>-</del>	or greater than 1.4. This may indicate the need for additional cross sections.								
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.								
	This may indicate the need for additional cross sections.								
Location:	River: RIVER-1 Reach: Reach-1 RS: 1 Profile: PF 1								
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for								
	additional cross sections.								
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7								
	or greater than 1.4. This may indicate the need for additional cross sections.								
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.								
	This may indicate the need for additional cross sections.								

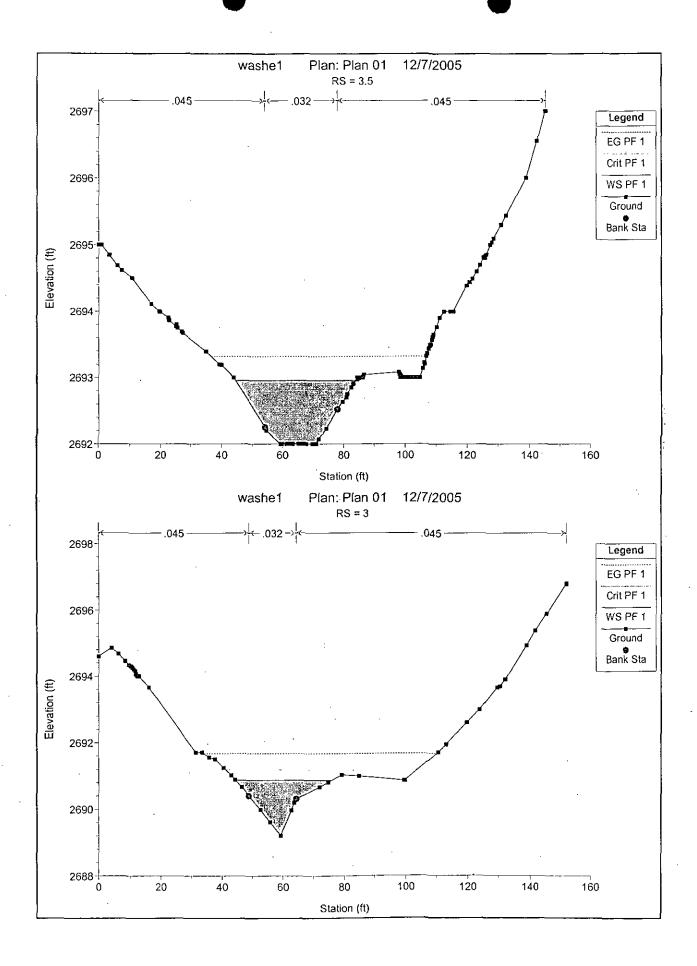


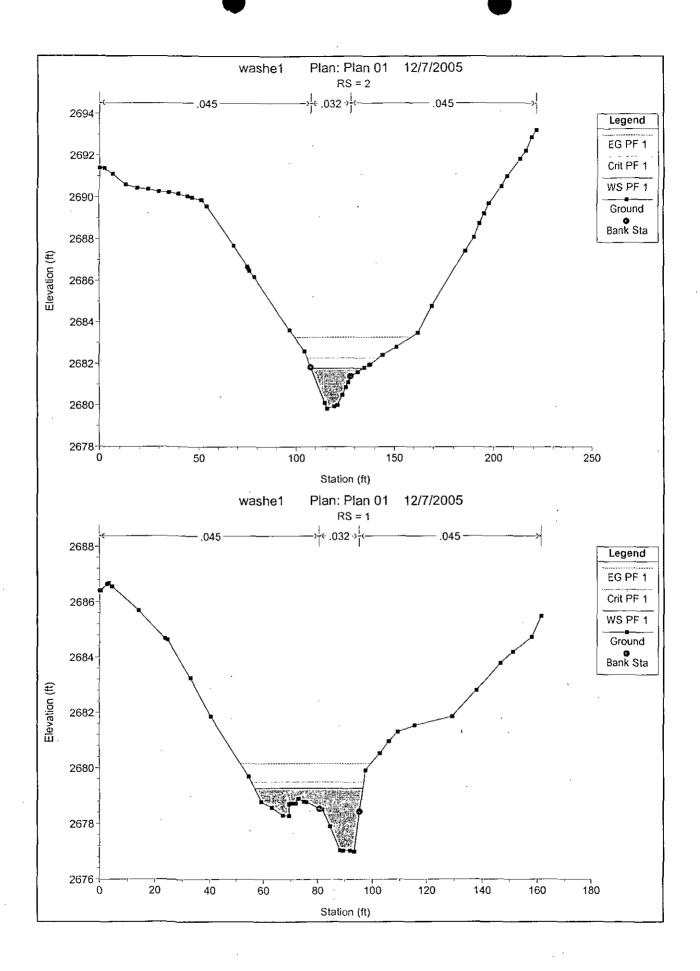












## WASH EZ

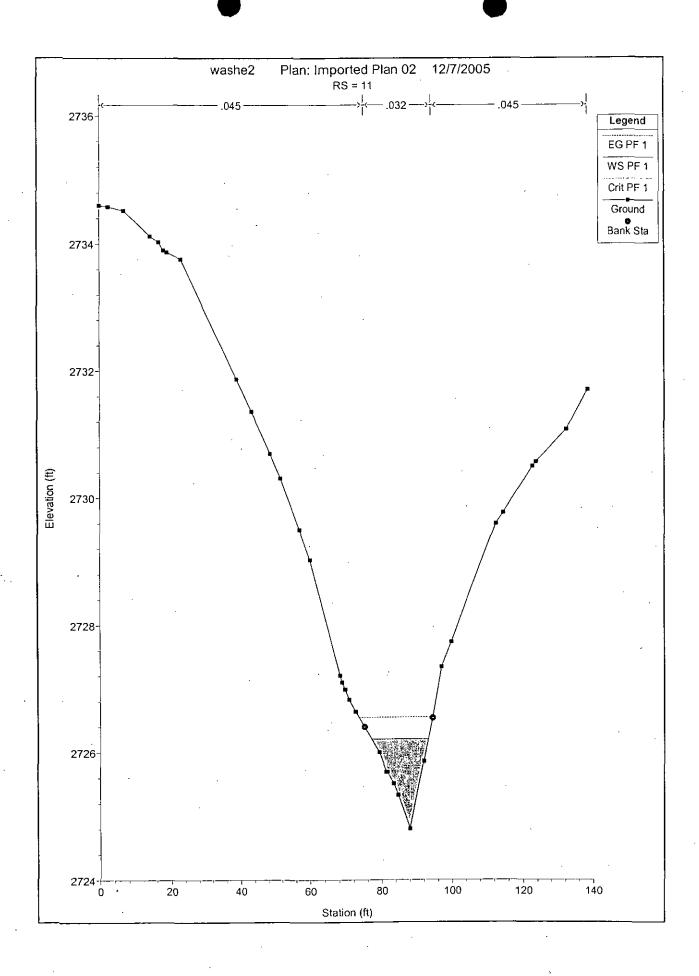
HEC-RAS PIE					ile: PF 1							
, Reach	≅River Sta⊋	黑 Profile 型	.EQ Total %	高Min Ch 白青	. W.S. Elev							s Froude # Chl €
APERT T	Gare distri	22.13c 12.15	② (cfs) 沙。	等性(ft) 議会	会情(机)等新	老家*(ft) : [新	磨⊯。(tt)高微。	立合(ft/ft)で続	હેં∉ુઁ(ft/s) ἔ∷ૈ	(sq ft) :- 2.	设施(ft)制造	<b>阿拉拉斯</b>
		PE 1257 Since		2724 80	2726.21	2726.21	2726 55	0.018084	4.69	10 67	16.13	1.02
Reach-1	10 美级级	PE 1	50 00	271982	2721.45	2721.45	2721 87	0.015739	5 17	9.85	14 80	0 98
Reach-15%, 2	9.其实是其实	PF 150 140 150	50 00	2715.71	2717.28	2717.28	2717.71	0.013081	5.64	10.87	14.46	0.94
Reach-1 知意	81/7/22/85	PF.1 智·隐藏	50.00	2709.10	2710.75	2710.73	2711.18	0.013703	5.29	9.88	13 61	0 93
Reach-1	7.5 心图处理	PF. 1沙族等於	50.00	2708.00	2708.95	2708.95	2709.30	0.014860	4.88	11.16	16 95	0.95
Reach-100	7 西南山野洋流山	PF.1:1984555	50.00	2705.46	2706.72	2706.72	2707.06	0.018115	4,66	10.75	17.20	1.02
Reach-1	615,349,5	PF_1-08: 1%	50 00	2701.53	2703.27	2703.27	2703.72	0.017169	5 37	9.32	10.70	1.01
Reach-1	5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	PE. I SEE	50.00	2697.79	2699.50	2699.50	2700.00	0.012670	5 97	10.10	11.77	0.93
Reach-1	4.5 次起 5 成一	PF. 1 35 - 155 -	50.00	2697.00	2698.23	2698 23	2698.64	, 0.014274	5 21	10.31	14.42	0.95
Reach-1	41251F85F15	PE.165	50.00	2694,91	2696.16	2696.16	2696.49	0.018131	4.61	10.84	16.93	1.02
Reach-1488	383 7.734	PF.11	50.00	2687,45	2689.05	2689.05	2689 47	0.017317	5.17	9,67	12.00	1.01
Reach-1	2 13 3	PF 1計算。	50.00	2679.83	2680.97	2680.94	2681 31	0.015193	4 66	10.76	14.98	0.95
Reach-1	1:30,000	PF.1	50 00	2677.00	2678 17	2678.17	2678 60	0.031321	5.29	9.56	11,63	0 98

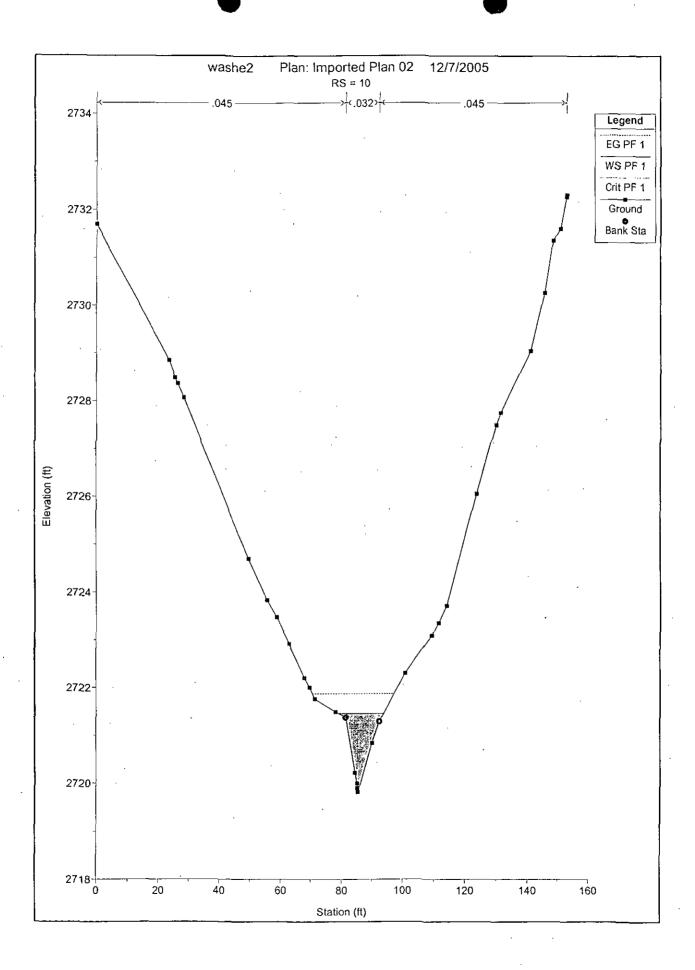
Errors Warnings and Notes for Plan: Imported Pla

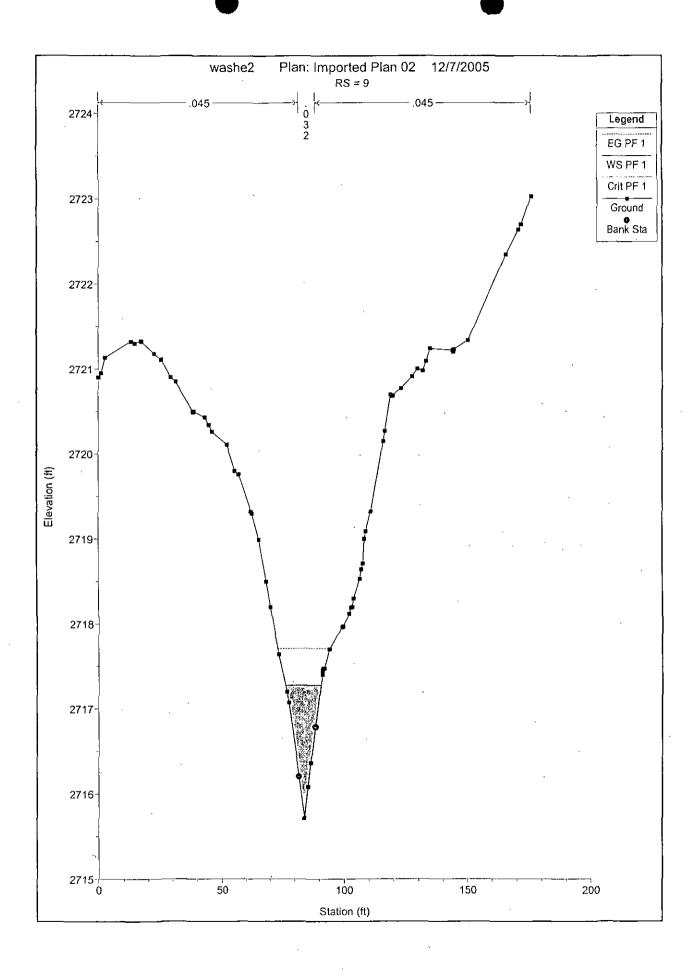
LITOIS TYBIII	ings and votes for Frant : imported Fra
Location:	River: RIVER-1 Reach: Reach-1 RS: 11 Profile: PF 1
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
	used critical depth for the water surface and continued on with the calculations.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
-	This may indicate the need for additional cross sections.
Warning:	During the standard step iterations, when the assumed water surface was set equal to critical depth,
	the calculated water surface came back below critical depth. This indicates that there is not a valid
	subcritical answer. The program defaulted to critical depth.
Location:	River: RIVER-1 Reach: Reach-1 RS: 10 Profile: PF 1
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
	used critical depth for the water surface and continued on with the calculations.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
<u> </u>	This may indicate the need for additional cross sections.
Warning:	During the standard step iterations, when the assumed water surface was set equal to critical depth,
9.	the calculated water surface came back below critical depth. This indicates that there is not a valid
	subcritical answer. The program defaulted to critical depth.
Location:	River: RIVER-1 Reach: Reach-1 RS: 9 Profile: PF 1
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
arming.	used critical depth for the water surface and continued on with the calculations.
Warning:	
rraining.	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.  This may indicate the need for additional cross sections.
Warning:	
waning.	During the standard step iterations, when the assumed water surface was set equal to critical depth,
	the calculated water surface came back below critical depth. This indicates that there is not a valid
Logotie	subcritical answer. The program defaulted to critical depth.
Location:	River: RIVER-1 Reach: Reach-1 RS: 8 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 7.5 Profile: PF 1
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
· · ·	used critical depth for the water surface and continued on with the calculations.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Warning:	During the standard step iterations, when the assumed water surface was set equal to critical depth,
	the calculated water surface came back below critical depth. This indicates that there is not a valid
	subcritical answer. The program defaulted to critical depth.
Location:	River: RIVER-1 Reach: Reach-1 RS: 7 Profile: PF 1
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
	used critical depth for the water surface and continued on with the calculations.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Warning:	During the standard step iterations, when the assumed water surface was set equal to critical depth,
	the calculated water surface came back below critical depth. This indicates that there is not a valid
	subcritical answer. The program defaulted to critical depth.
Location:	River: RIVER-1 Reach: Reach-1 RS: 6 Profile: PF 1
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
	used critical depth for the water surface and continued on with the calculations.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
<u> </u>	This may indicate the need for additional cross sections.
	During the standard step iterations, when the assumed water surface was set equal to critical depth,
Warning.	
warning:	Ithe calculated water surface came back below critical denth. This indicates that there is not a valid
Warning:	the calculated water surface came back below critical depth. This indicates that there is not a valid
Location:	the calculated water surface came back below critical depth. This indicates that there is not a valid subcritical answer. The program defaulted to critical depth.  River: RIVER-1 Reach; Reach-1 RS: 5 Profile: PF 1

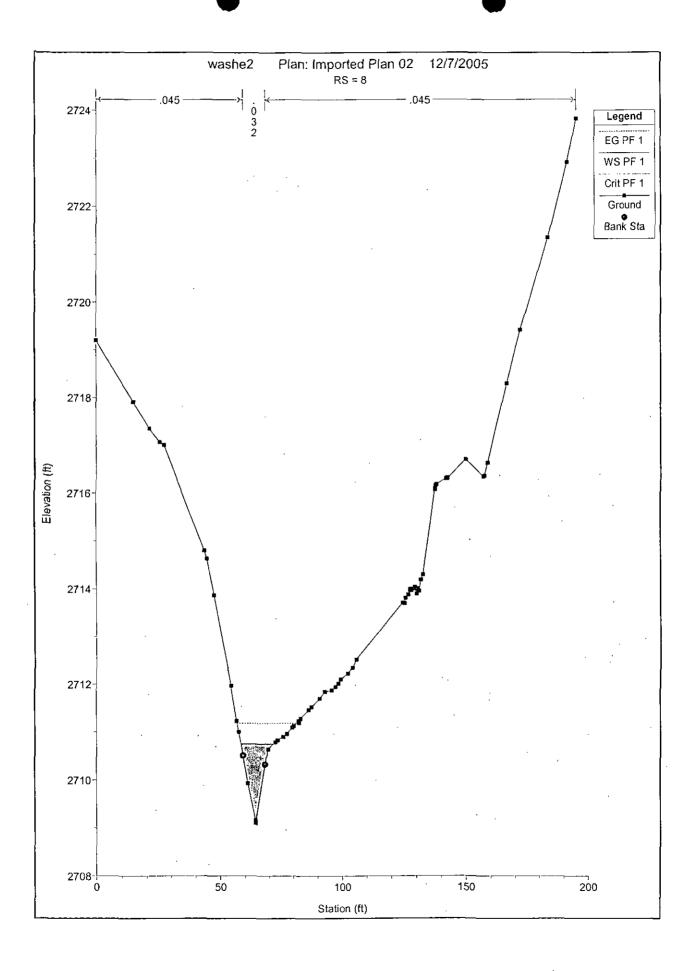
Errors Warnings	and Notes for Plan	· Imported Pla (Continued)
LUUIS Wallillus	and Miles for Plan	THURDARA PIS (CANTINUES)

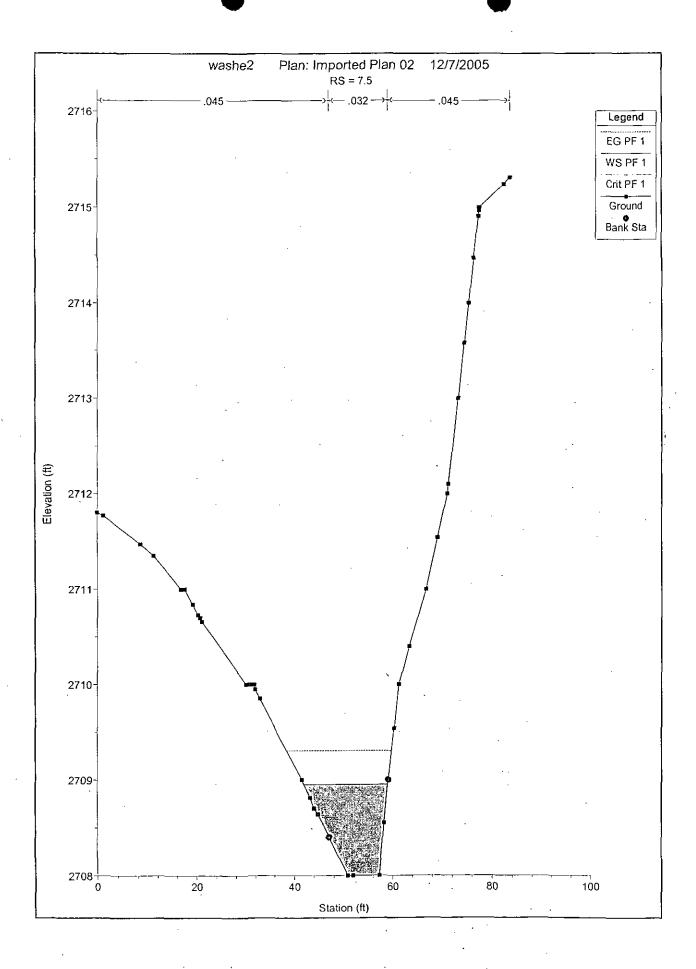
selected the water surface that had the least amount of error between computed and assumed						
values.						
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.						
This may indicate the need for additional cross sections.						
During the standard step iterations, when the assumed water surface was set equal to critical depth,						
the calculated water surface came back below critical depth. This indicates that there is not a valid						
subcritical answer. The program defaulted to critical depth.						
River: RIVER-1 Reach: Reach-1 RS: 4.5 Profile: PF 1						
The energy equation could not be balanced within the specified number of iterations. The program						
used critical depth for the water surface and continued on with the calculations.						
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.						
This may indicate the need for additional cross sections.						
During the standard step iterations, when the assumed water surface was set equal to critical depth,						
the calculated water surface came back below critical depth. This indicates that there is not a valid						
subcritical answer. The program defaulted to critical depth.						
River: RIVER-1 Reach: Reach-1 RS: 4 Profile: PF 1						
The energy equation could not be balanced within the specified number of iterations. The program						
used critical depth for the water surface and continued on with the calculations.						
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.						
This may indicate the need for additional cross sections.						
During the standard step iterations, when the assumed water surface was set equal to critical depth,						
the calculated water surface came back below critical depth. This indicates that there is not a valid						
subcritical answer. The program defaulted to critical depth.						
River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1						
The energy equation could not be balanced within the specified number of iterations. The program						
used critical depth for the water surface and continued on with the calculations.						
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.						
This may indicate the need for additional cross sections.						
During the standard step iterations, when the assumed water surface was set equal to critical depth,						
the calculated water surface came back below critical depth. This indicates that there is not a valid						
subcritical answer. The program defaulted to critical depth.						
River; RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1						
The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7						
or greater than 1.4. This may indicate the need for additional cross sections.						
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.						
This may indicate the need for additional cross sections.						

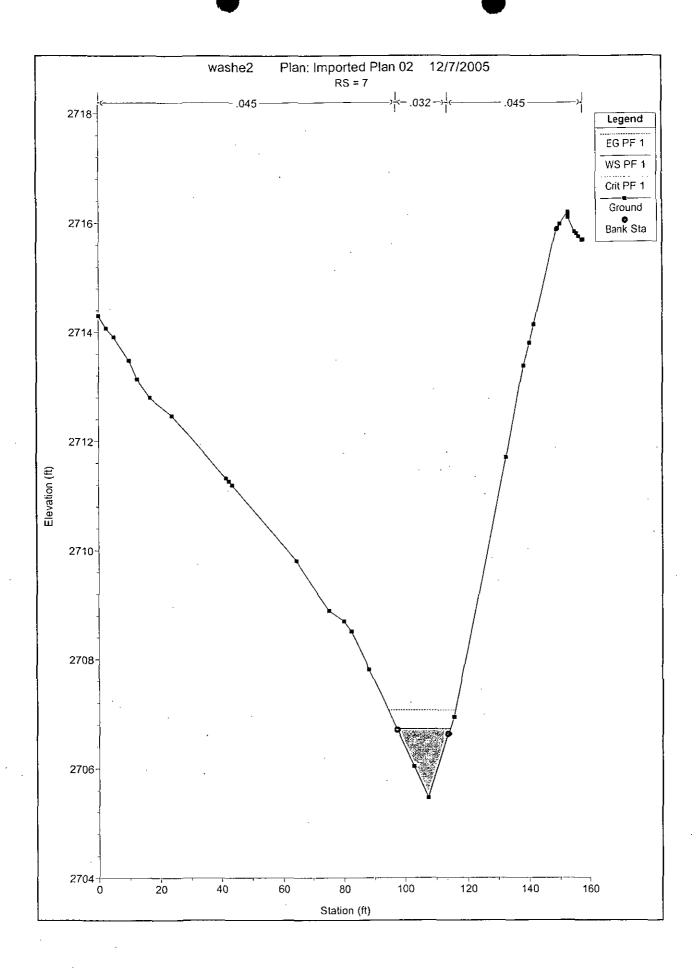


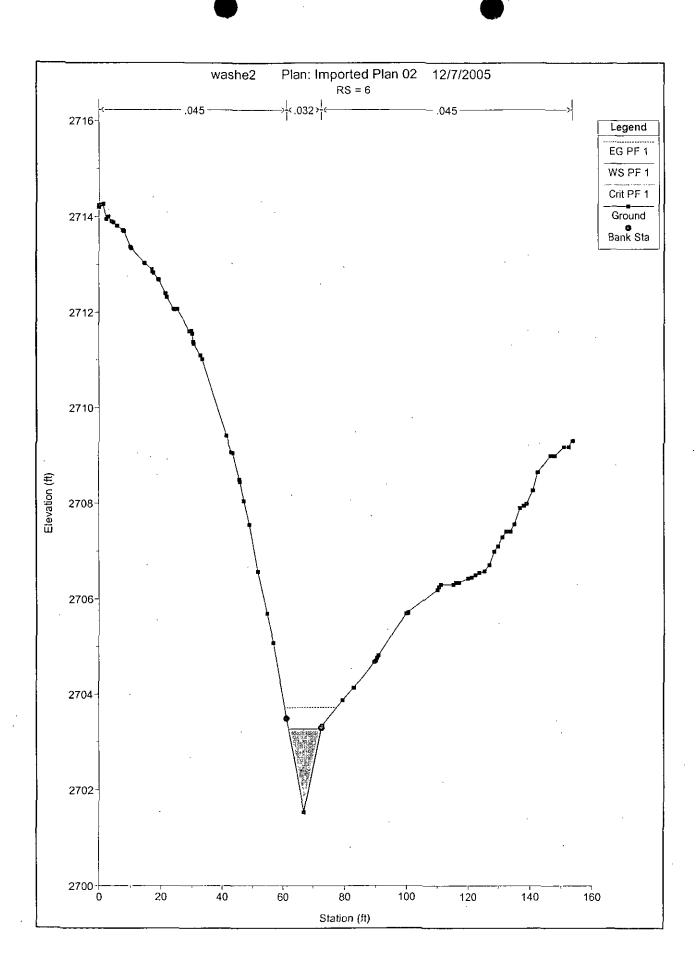


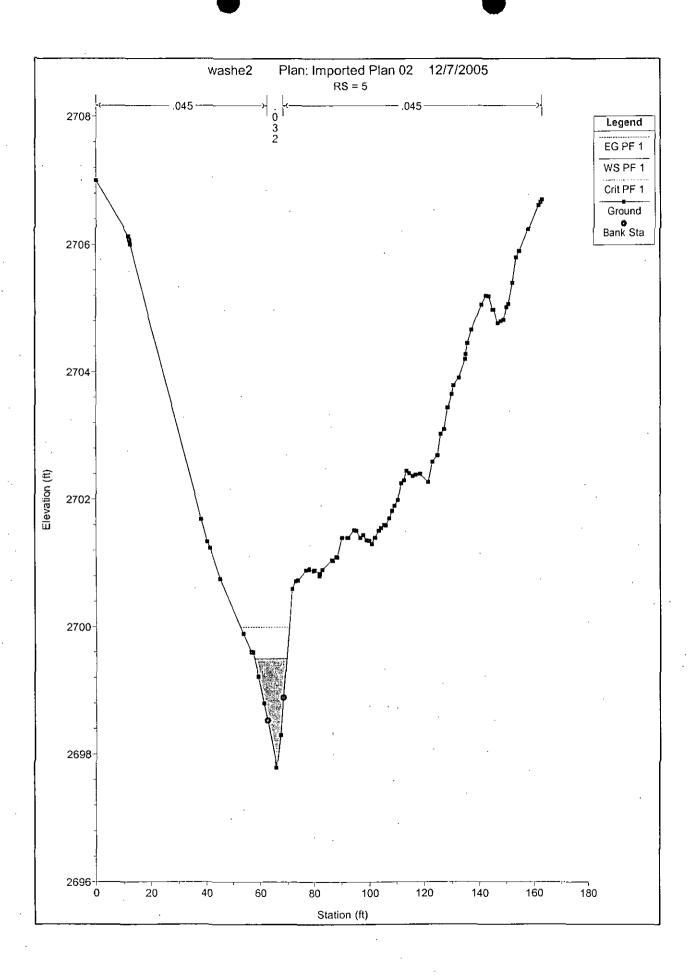


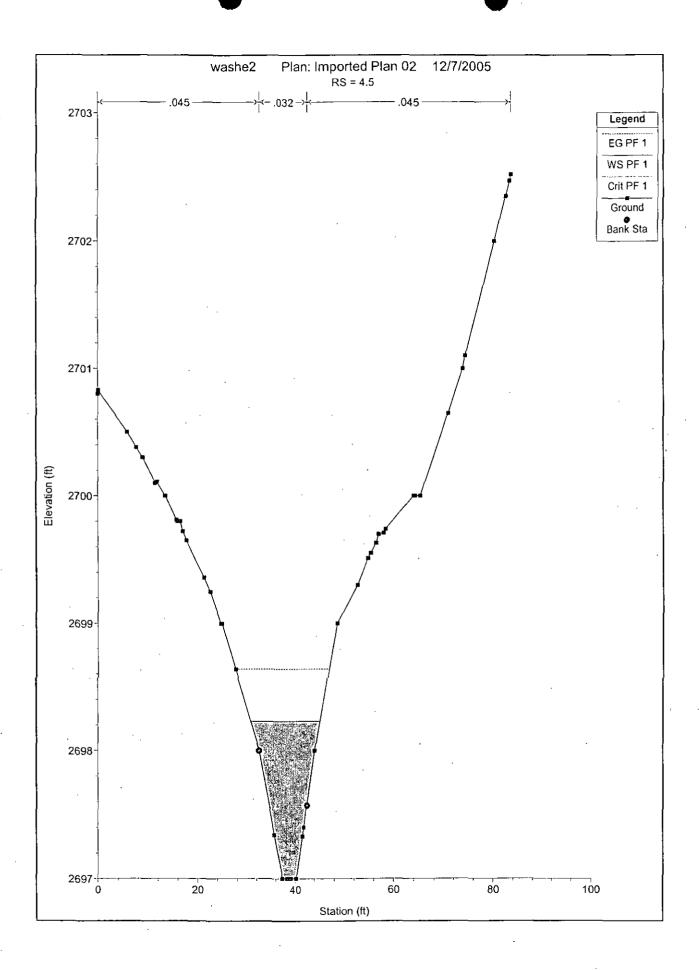


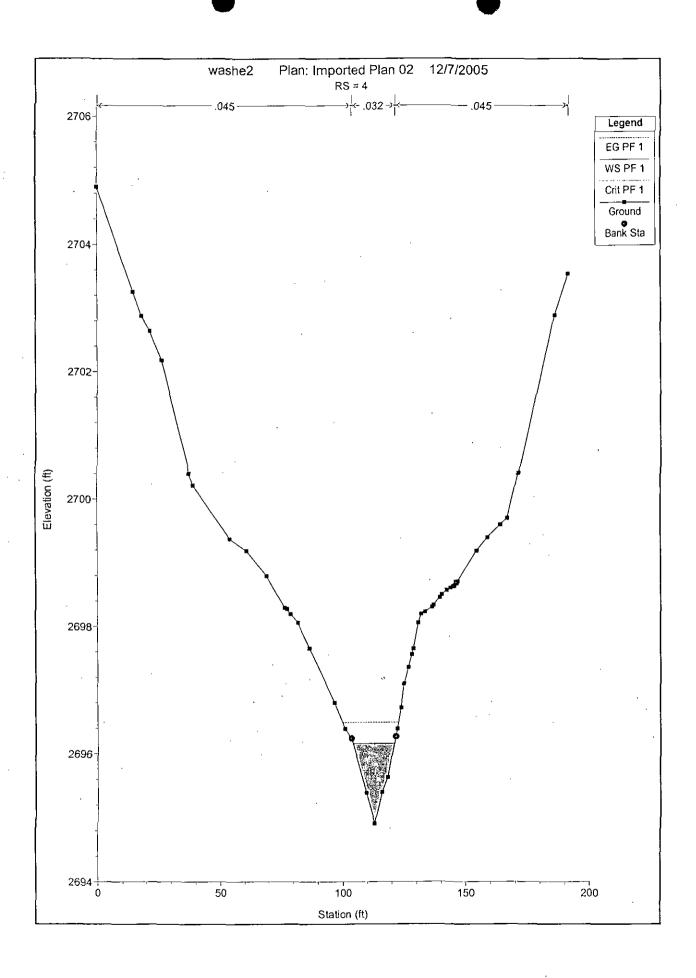


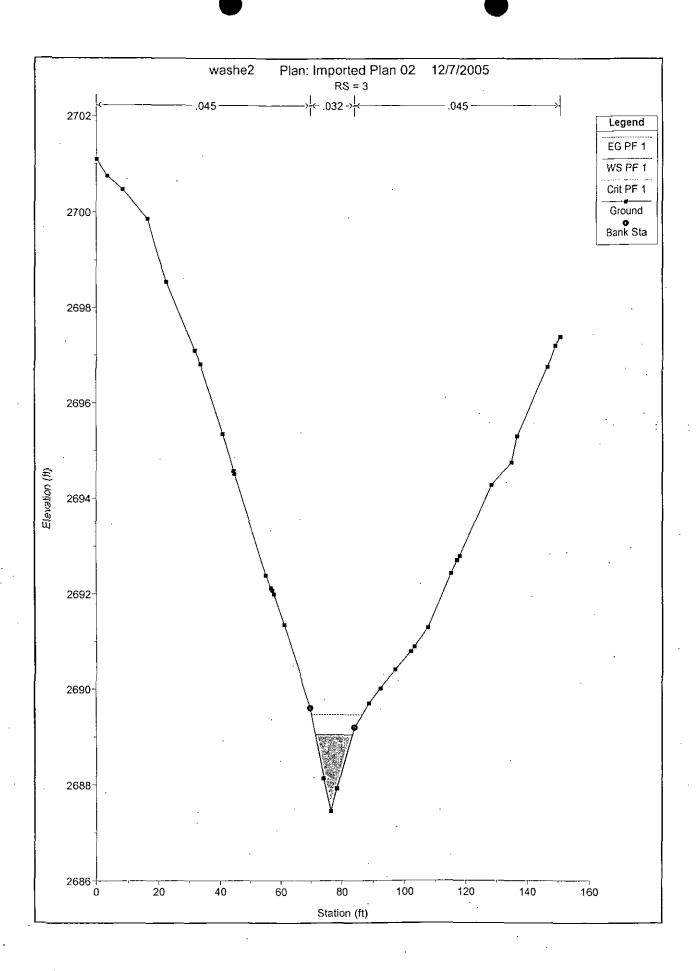


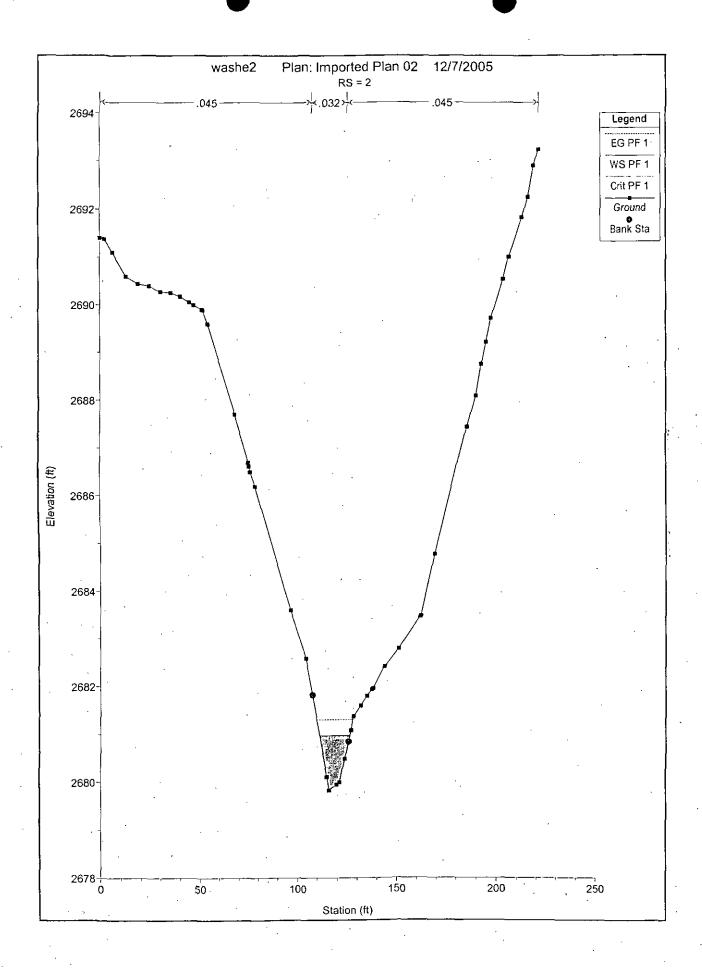


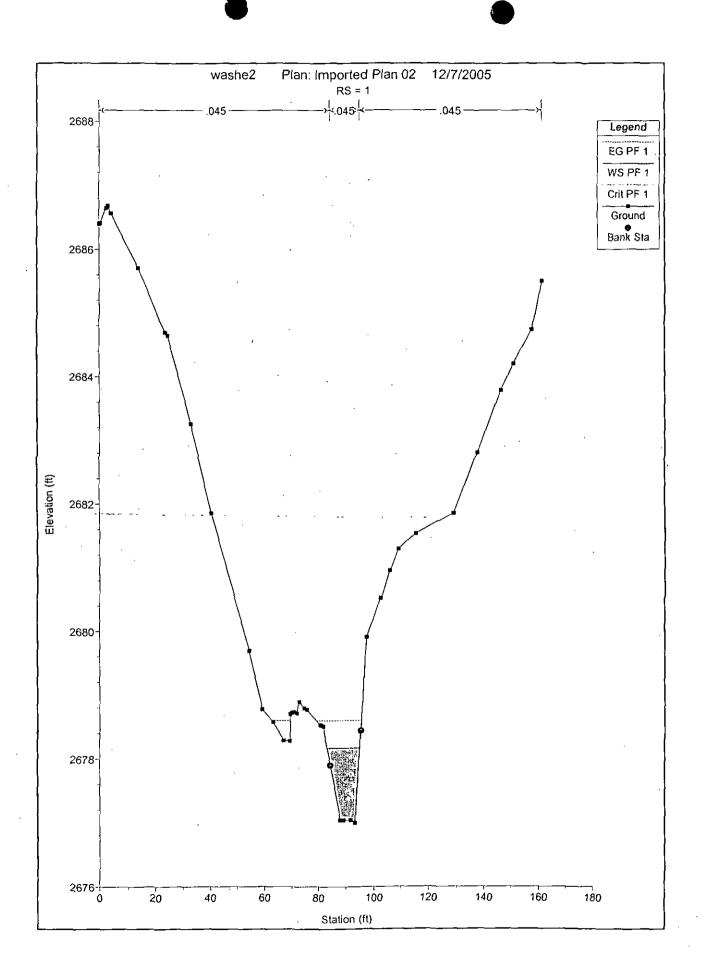










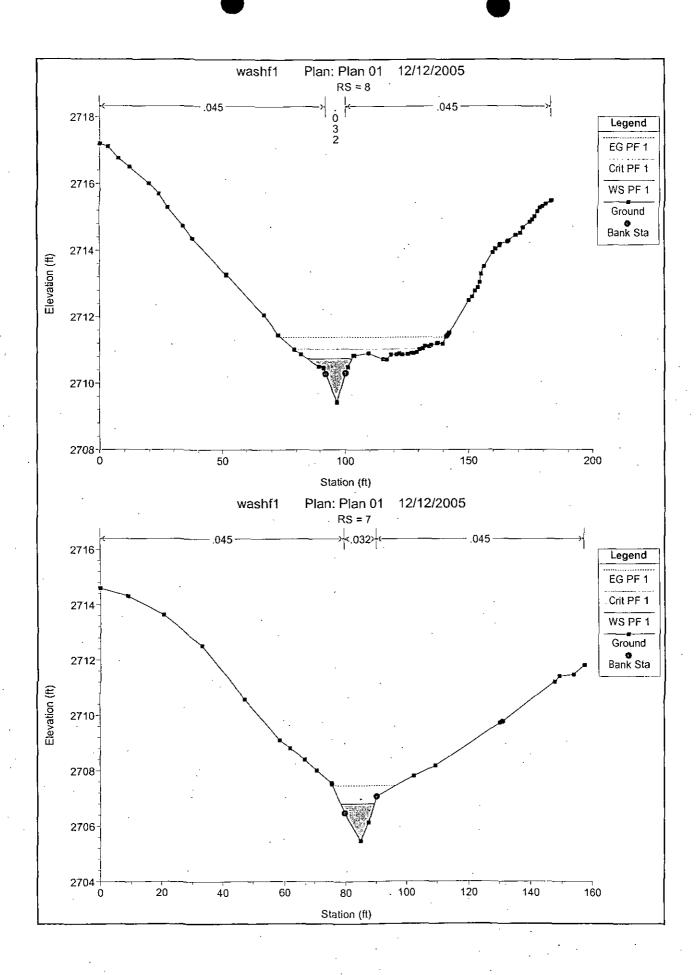


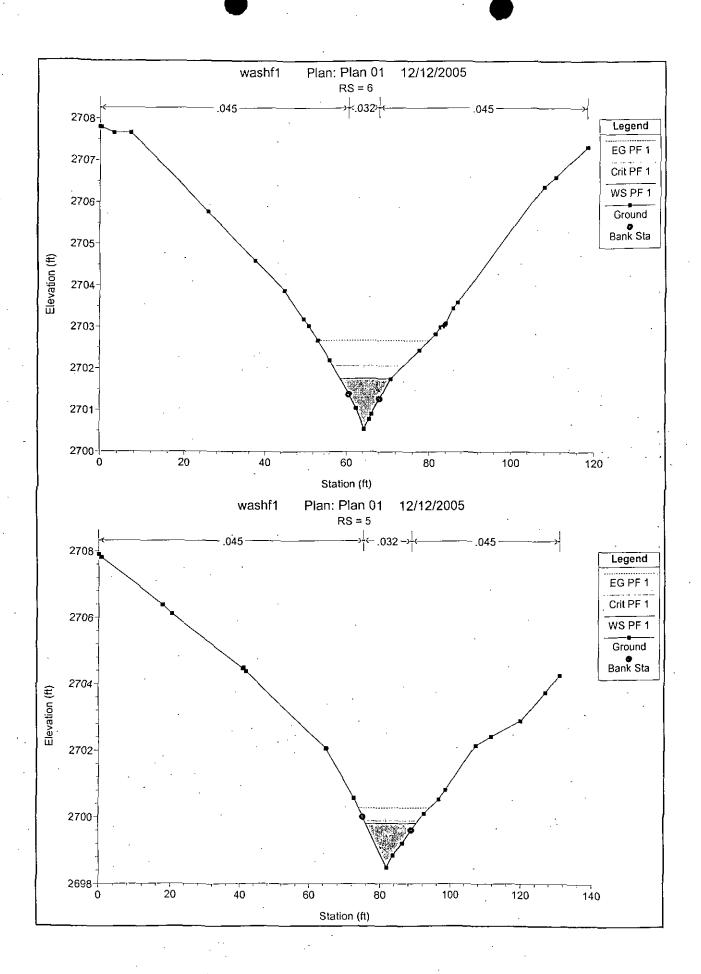
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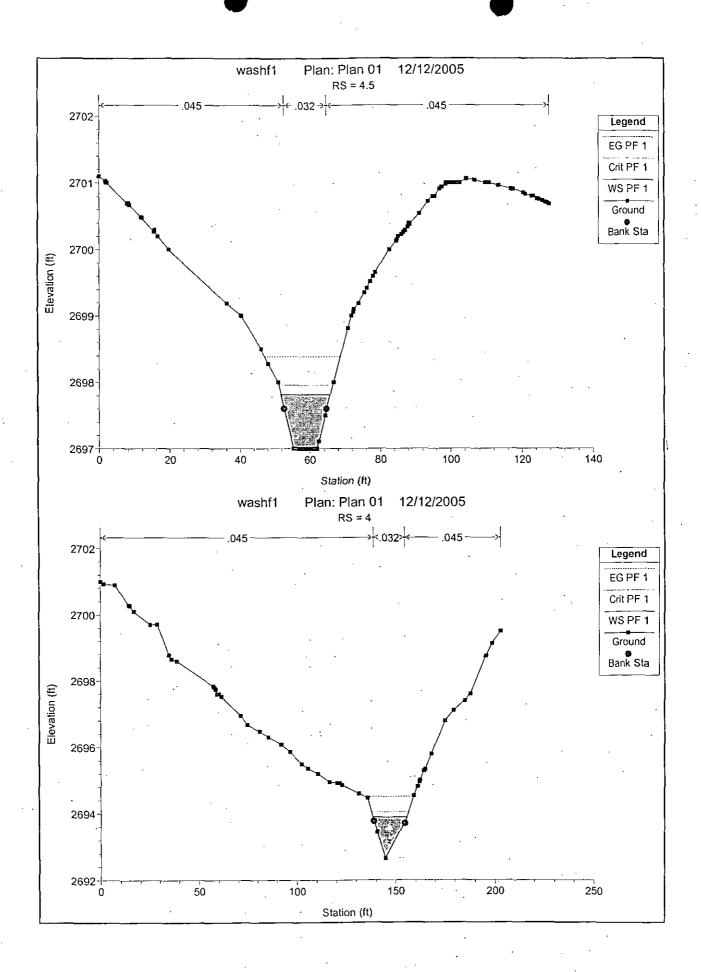
Reach 🦃	₹River Sta	Profile . %	C Total	Min Ch El	W.S. Elev.	GAL W.S.	.⊈E.G. Elev.β	E.G. Slope	業: Vel Chrift	ri Flow Area !!	走Top Width被	Froude # Chi
R SECTION	Fig. 14	<b>台州建筑湖</b> 市	程序 (cfs) 許成	4450 (h) 6529	图址(ft)运运	云為魏(ff)於監急	· 通線(h)登場	Fig. (fvft) : 25%	<b>變 (fl/s) 經濟</b>	Se (sq ft)	证法公(用)下学者	PART 1994
Reach-1 🗯	8	PE.1以统治在	50.00	2709 42					6.65			
		PF 1 # ALAN		2705.46	2706 81	2706.97	2707.45	0.027912	6.49	7 87	11.03	1 29
		PF 1000 16		2700.54	2701.76	2702.06	2702.68	0.039439	7.86	7.10	12.34	1.54
Reach-1+	5111911112	PF.1	50.00	2698 48	2699.81	2699.89	2700.28	0.022728	5.49	9.24	14.33	1.15
		PF,1 (5)		2697.00	2697.81	2697.96	2698.38	0.028656	6.06	8 42	13,96	1 29
Reach-1	4世紀が上が	PF.14%	70 00	2692 65	2693 89	2694 04	2694.51	0.030515	6.32	11.17	17.42	1,34
Reach-1	3.5	PF.1科学题	. 70.00	2589.00	2690.58	2590.58	2690.99	0.011182	5 40	15.90	22.12	0,87
		PE 1		2686.57	2687.76	2688.03	2688.62	0.036115	7 47	9 68	14 54	1 48
Reach-1	24.30.00	PF.1(18)***	70,00	2681,90	2683.30	2683 31	2683.60	0.016888	4.45	17.26	34.61	0.98
Peach 13 dill	451 841 3 45 5 5	LIDE 4% MARKETON	97.00	2077.00	2020.75	0070.00	2670.45	0.037047	6.04	45.40	62.07	

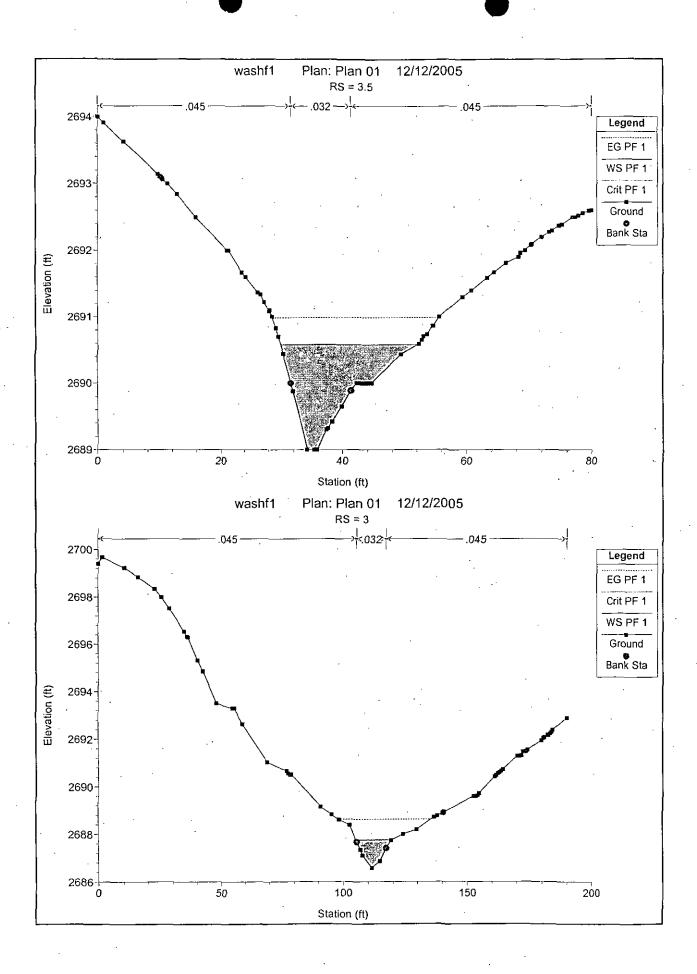
Errors Warnings and Notes for Plan: Plan 01

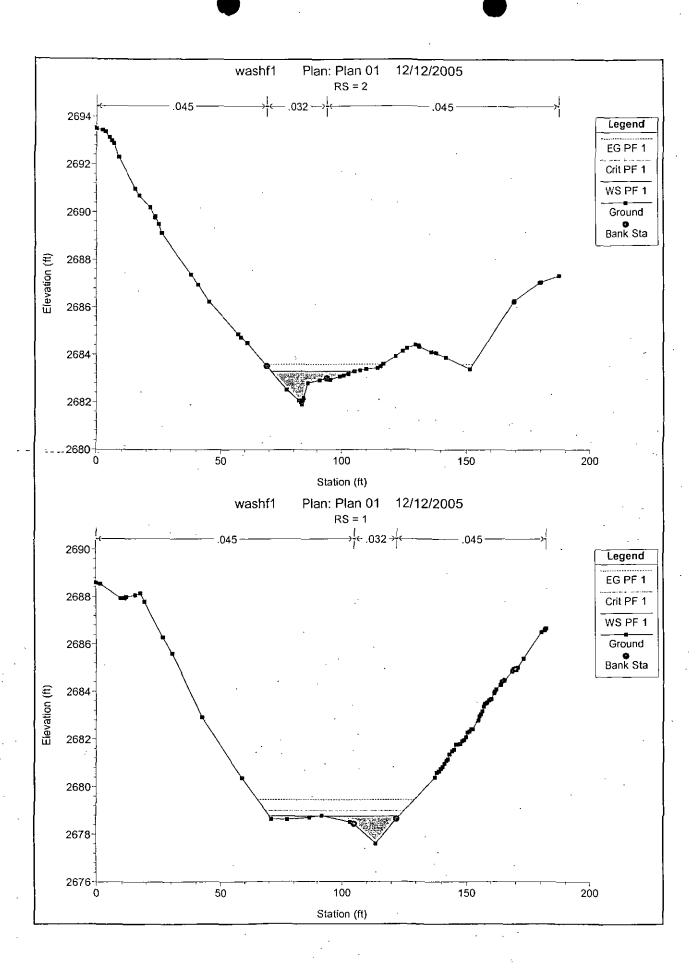
	ings and notes for Plant: Plant VT
Location:	River: RIVER-1 Reach: Reach-1 RS: 8 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
Location:	River: RIVER-1 Reach: Reach-1 RS: 7 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 6 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 5 Profile: PF 1
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
	selected the water surface that had the least amount of error between computed and assumed
	values.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
<u>gr</u>	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 4.5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS; 4 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
ing.	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 R\$; 3.5 Profile: PF 1
Warning:	The energy equation could not be balanced within the specified number of iterations. The program
770mm,mng.	used critical depth for the water surface and continued on with the calculations.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
Truming.	This may indicate the need for additional cross sections.
Warning:	During the standard step iterations, when the assumed water surface was set equal to critical depth,
177 s iii ig.	the calculated water surface came back below critical depth. This indicates that there is not a valid
<del></del>	subcritical answer. The program defaulted to critical depth.
Location:	River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
rruning.	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
rran inig.	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS; 2 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
Training.	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
Truming.	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
TTGITING.	This may indicate the need for additional cross sections.
Location	
Location:	
Warning:	Divided flow computed for this cross-section.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
L	This may indicate the need for additional cross sections.











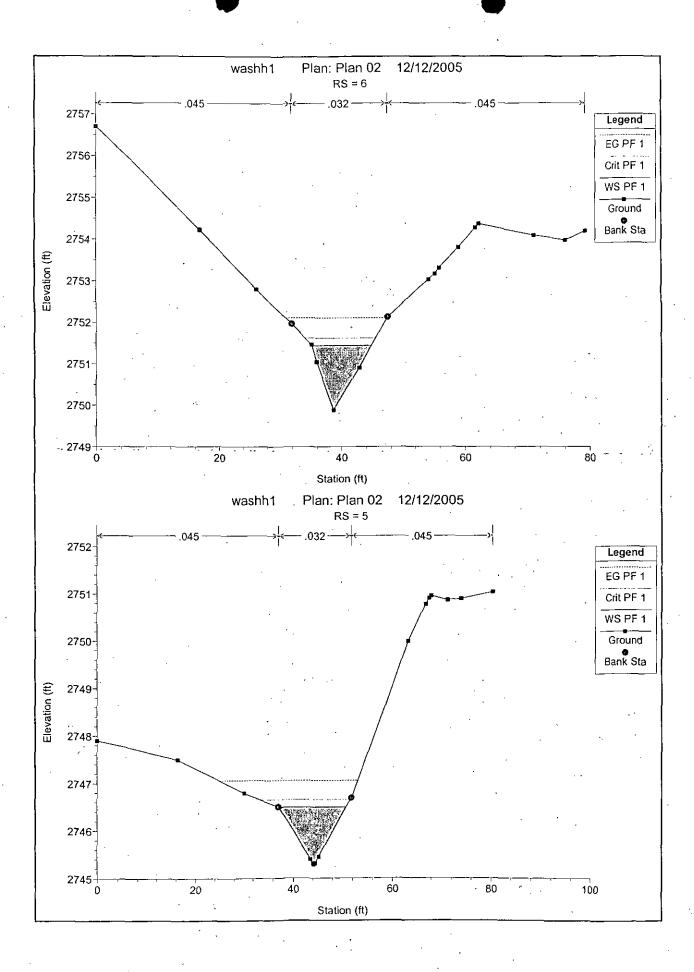
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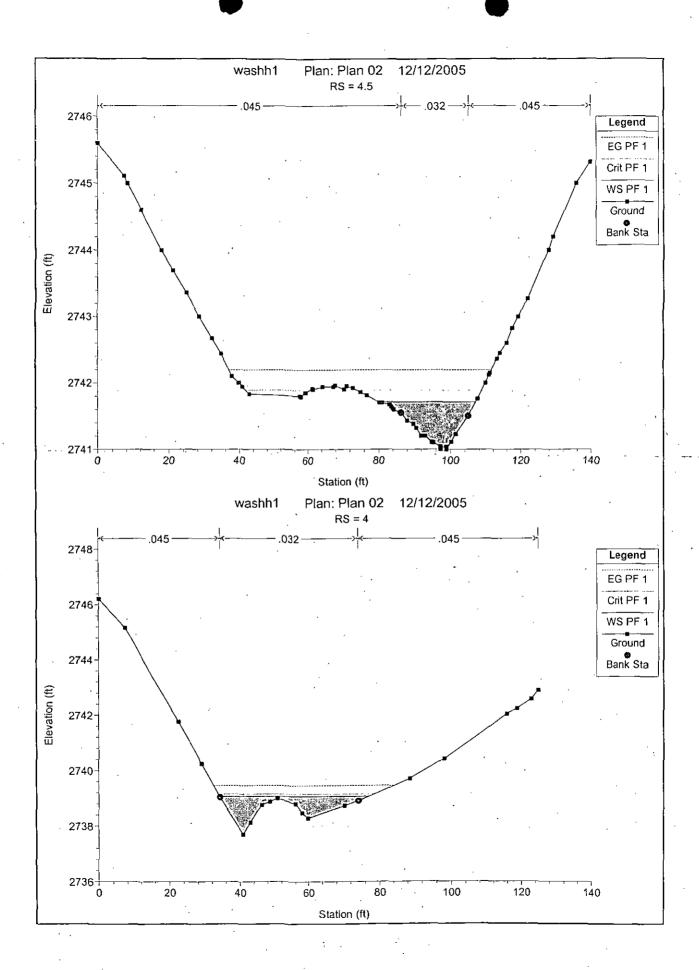
HEC-RAS	Pian: Pian 02	River: RIVER-1	Reach: Reach-1	Profile PF 1

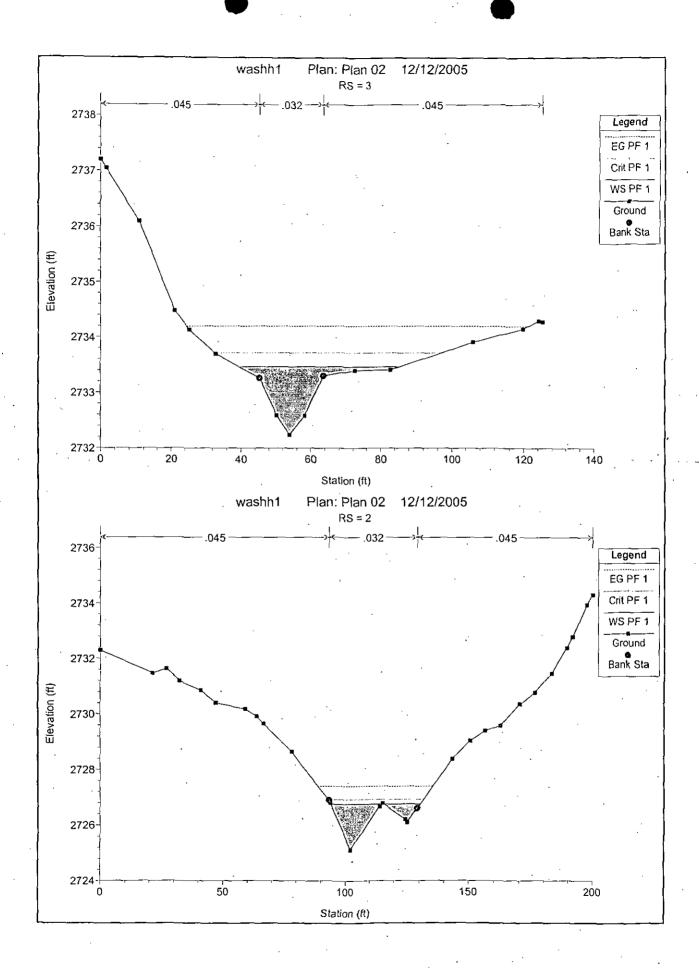
Reach	≒River Sta ≒	Profile 🗟	🚣 Q Total 🧐	高Min Ch 白夏	∛W.S. Elev.∄	Crit W.S.%	E.G. Elev®	≛ E.G. Slope 🖫	燕Vel Chnl:為	≩ Flow Area :∘	-≋Top Width #	≩Froude # Chl ⅓
a42446463			趣 (cfs) 淀	為定(作)等機	建压(ft)汽车	(ft) 经资	(ft) 海道	罗斯 (fuft)毛压	(ft/s)美術	- € (sq.ft) 🐫	\$4. (h) \$4.5	PR 15 17 15 15 1
Reach-1 🛴				2749.88	2751.43	2751.61	2752.10	0.028975	6.55	7.63	9.59	1.29
Reach-1				2745 29	2746 49	2746 65	2747.06	0.033087	6 02	8.30	13.57	1.36
Reach-1 200				2741.00	2741.71	2741,89	2742.19	0.040197	5.61	9.42	27.11	1.44
Reach-1				2737.70	2739.07	2739.15	2739.46	0.029073	4.98	20.28	43 02	1,24
Reach-1115				2732.24	2733.47	2733.71	2734 20	0.032501	6 95	16,31	45.91	1.40
Reach-12	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	PF_11%5522	140.00	2725.11	2726.76	2726.92	2727.40	0.035100	6 44	21.81	35 71	1.43
Reach-1	1.5	PF_1166	140.00	2723.00	2724.12	2724.21	2724.61	0.017869	5.76	26 91	40.07	1.07
Reach-1	1.等。施工计	PF_1:2500	140 00	2720.27	2720.72	2720.96	2721.50	0.190207	7.49	19.79	64.81	2.78

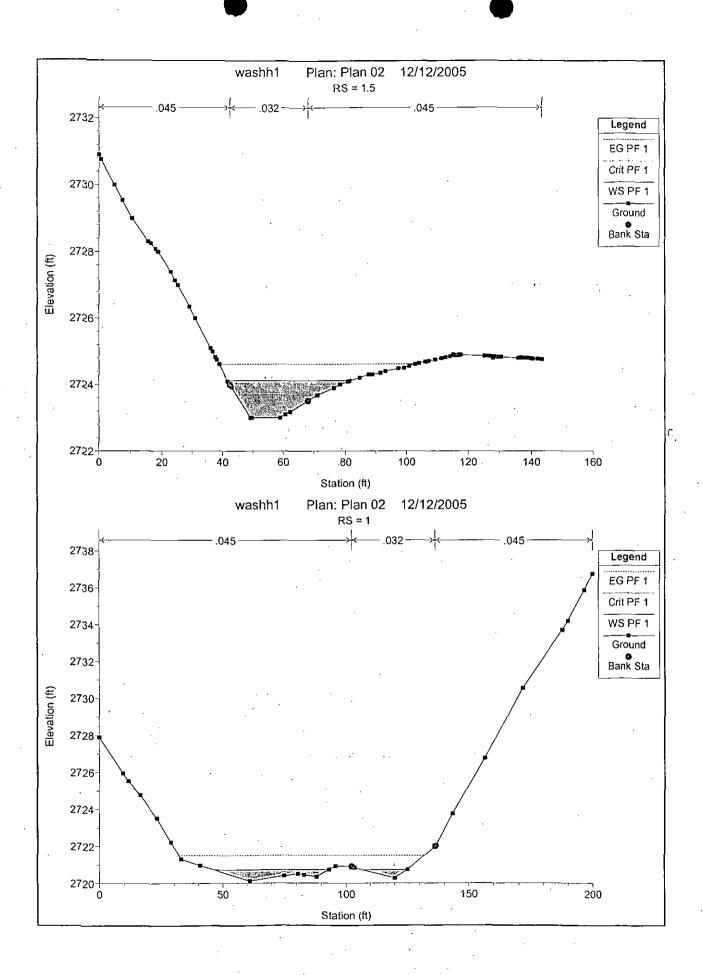
Errors Warnings and Notes for Plan: Plan 02

Location:	River: RIVER-1 Reach: Reach-1 RS: 5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 4.5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 4 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 1.5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 1 Profile: PF 1
Waming:	The energy equation could not be balanced within the specified number of iterations. The program
. <u> </u>	selected the water surface that had the least amount of error between computed and assumed
	values.
Warning:	Divided flow computed for this cross-section.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.









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HEC-RAS	Plan: Plan 02	River RIVER-1	Reach Reach-1	Profite PF 1	

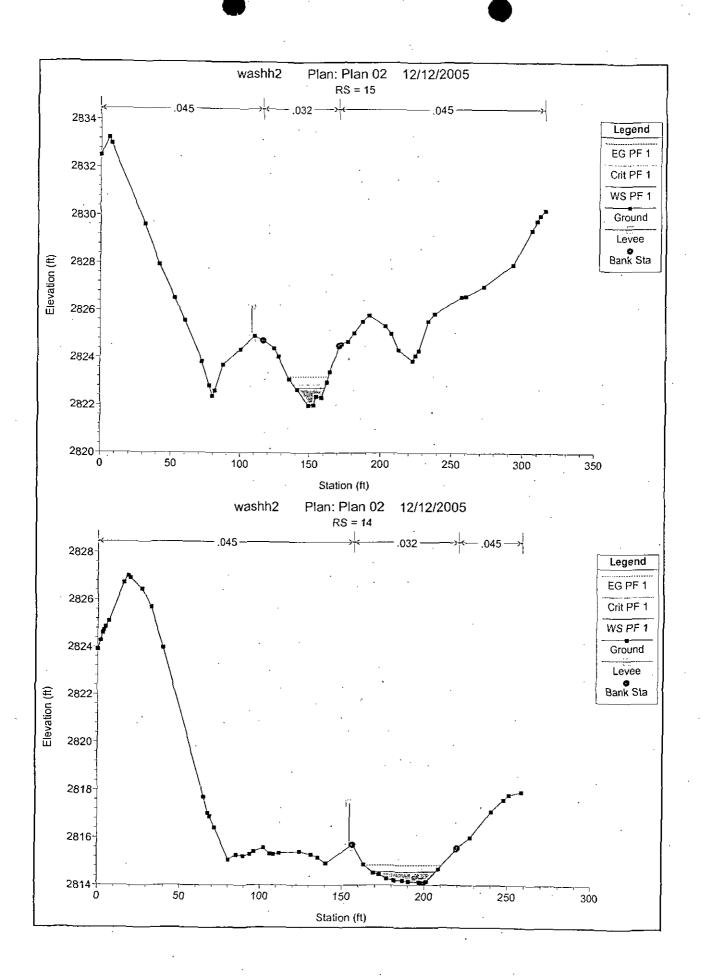
						, <del></del>	,				r	
												系Froude # Chl会
机器分割较	deservice.		神道 (cfs) 🖫 🖫	uše (ft) 🙉 🗆	(f) 图点	(ft) 海道	Berg (ft) \$128	說於(fl/ft)多數	监禁(ft/s) %、	(\$q ft) (\$\$	Exact (ft) 文字。	10000000000000000000000000000000000000
	15世纪20			2821.98	2822.73	2822.87	2823 20	0.043010	5.50	9.10	21.01	1.47
	14 图 图 2			2814.10	2814 57	2814.63	2814 83	0.038439	4.16	12.02	38 88	1.32
	13月37年第1月			2808.80	2809.31	2809.36	2809.54	0,036980	3 82	13 10	46 83	1.27
	12			2800.73	280122	2801.30	2801.52	0.063174	4.36	11.47	50.18	1.61
	11.5			2795 20	2795 75	2795.81	2796.03	0.026258	4.34	12.81	37.70	1,16
Reach-1	115次数据是2	PF.1; C. Pag	110.00	2783.25	2784.72	2785.16	2786 12	0 060907	9.51	11,56	14.98	1.91
Reach 1	10 24 74 19	PF.18001224	110.00	2773.90	2775.20	2775 38	2775.77	0,031318	6 28	19.82	40.55	1.35
Reach 1 🎏	9点他细胞	PF_11285128	110.00	2764.93	· 2766 03	2756.36	2767.06	0 053740	8.14	13.52	20.43	1.76
Reach-1	8.5	PF.1字经数	110.00	2761.97	2762 95	2762.99	2763.16	0.009485	4.31	45 60	141.04	0.79
Reach-1	8 10 48 4	PF.1	110,00	2757.40	. 2757.99	2758.36	2760.10	0.470942	11.67	9 42	42.33	4.36
Reach-1 消滅	7. 海海等心袋	PE.1	110 00	2751.66	2752.64	2752.67	2752.98	0.016247	4.71	24.44	41.48	0.98
Reach-12	6 LA CARTA	PF.1"	110.00	2749 39	2749.94	2750,26	2751.09	0.119556	8.63	12.75	32 33	2.42
Reach-1	5131240	PF.1	110.00	2746.81	2747.80	. 2747.91	2748.30	0 028449	5.70	19.29	30.93	1,27
Reach-1	4	PF 1	110.00	2744.09	2744 94	2745.29	2746 05	0.056779	8.44	13 03	19 34	1.81
Reach-1	3 1487.03	PF.1#3855	110.00	2739 56	2740.88	2741.04	2741.47	0.031414	6.12	17.97	28 08	1.34
Reach-1	2 图 图 图	PF.1部課職	155.00	2735 23	2736.17	2736.50	2737.21	0.054958	8.16	18.99	29.13	1.78
Reach-1	188766646	PF/13/FFFE	155.00	2731.01	2731,60	2731.77	2732,21	0.046564	6.25	24.80	49.97	1.56

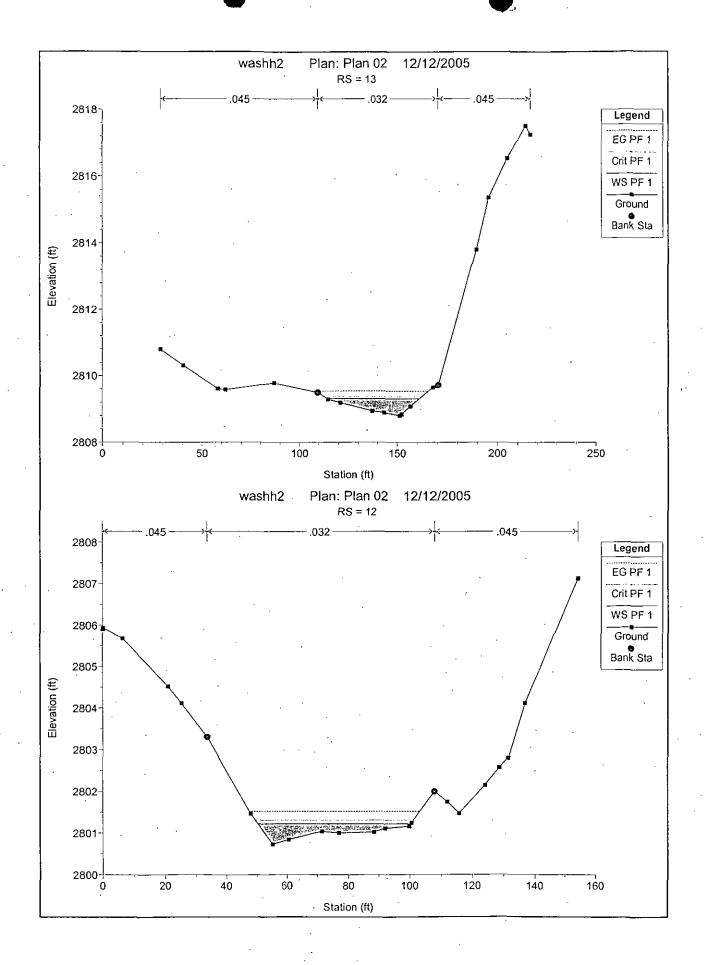
Errors Warnings and Notes for Plan: Plan 02

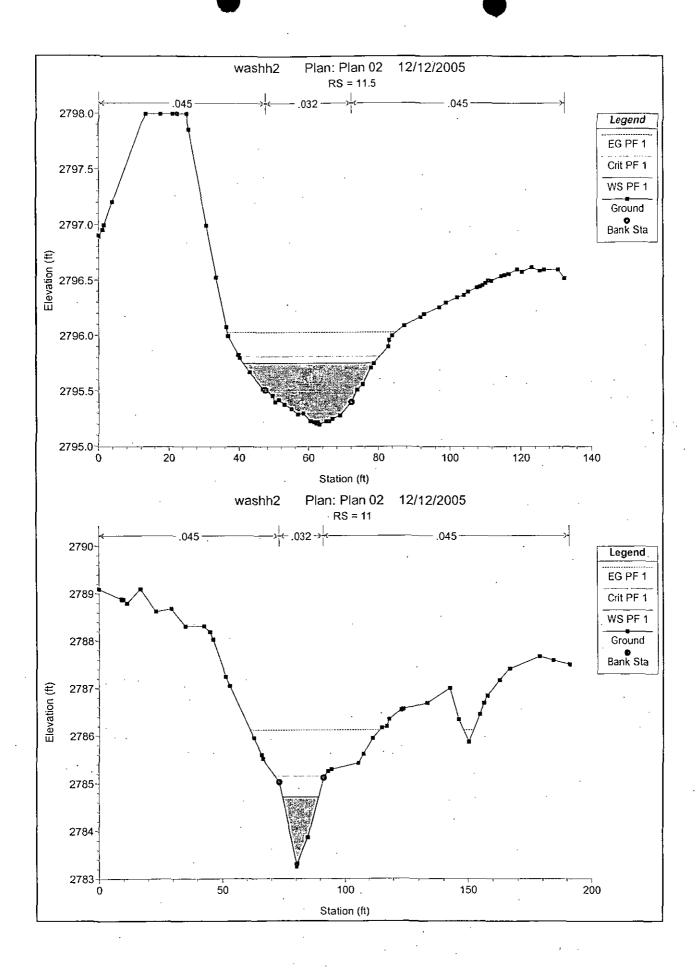
	ings and Notes for Flan 02
Location:	River: RIVER-1 Reach: Reach-1 RS: 15 Profile: PF 1
Note:	Multiple critical depths were found at this location. The critical depth with the lowest, valid, water
	surface was used.
Location:	River: RIVER-1 Reach: Reach-1 RS: 14 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Note:	Multiple critical depths were found at this location. The critical depth with the lowest, valid, water
	surface was used.
Location:	River: RIVER-1 Reach: Reach-1 RS: 13 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 12 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 11.5 Profile: PF 1
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 11 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 10 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
— <u> </u>	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Note:	Multiple critical depths were found at this location. The critical depth with the lowest, valid, water
	surface was used.
Location:	River: RIVER-1 Reach: Reach-1 RS: 9 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
· · · · · · · · · · · · · · · · · · ·	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 8.5 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
- Litting:	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
· · · unining:	This may indicate the need for additional cross sections.
 Location:	River: RiVER-1 Reach: Reach-1 RS: 8 Profile: PF 1
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
··aning.	additional cross sections.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
waning.	or greater than 1.4. This may indicate the need for additional cross sections.
Marning	-   - · · · · · · · · · · · · · · · · ·
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
Note:	This may indicate the need for additional cross sections.  Multiple estimated deaths used found at this location. The critical death with the lowest valid water.
Note:	Multiple critical depths were found at this location. The critical depth with the lowest, valid, water
	surface Was used.

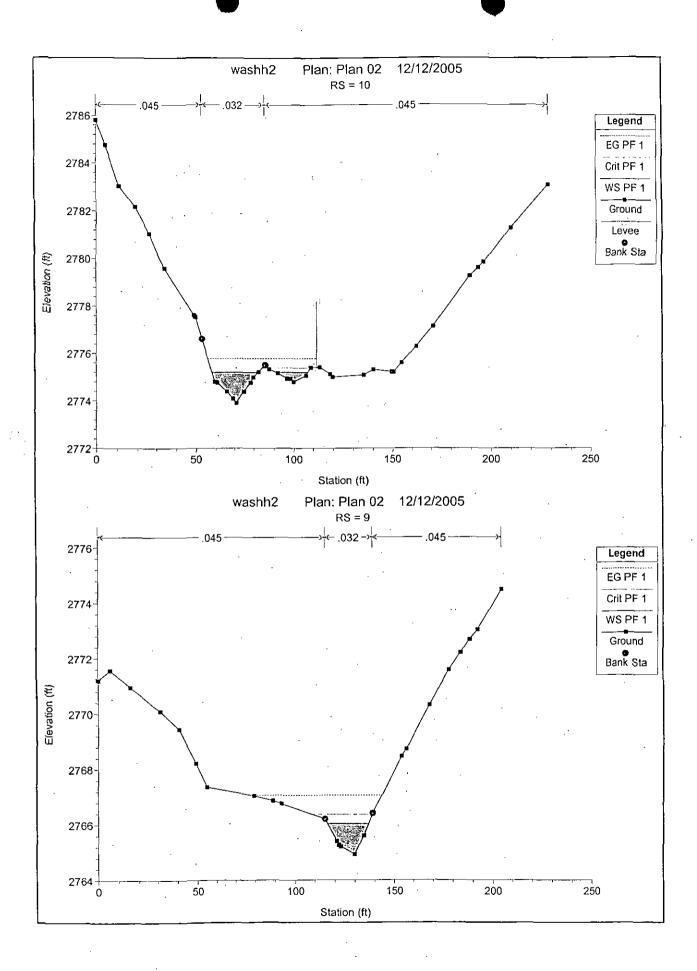
Errors Warnings and Notes for Plan: Plan 02 (Continued)

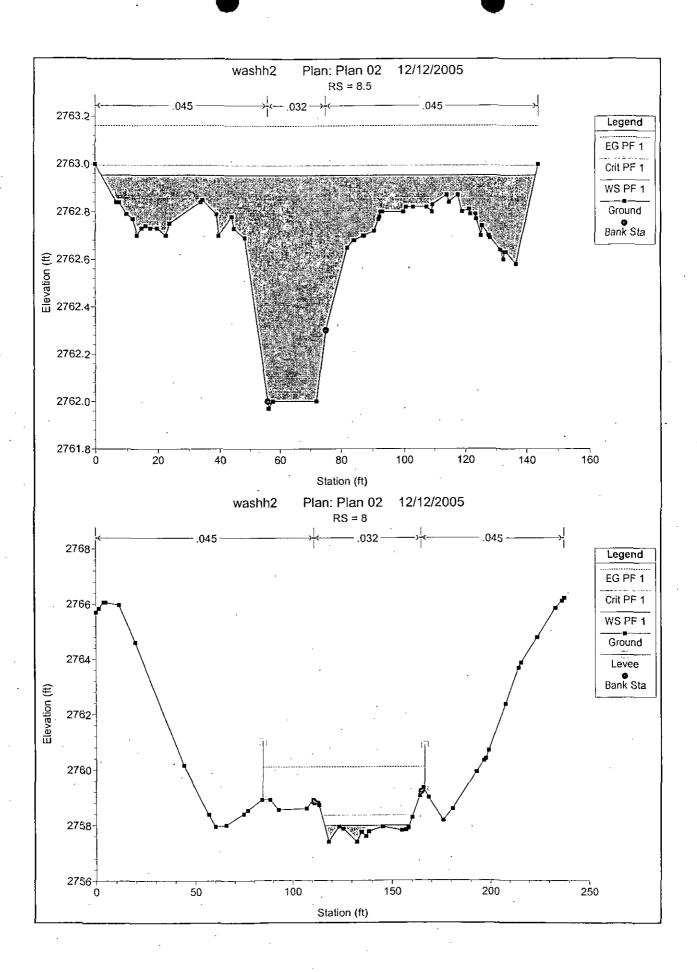
Tigs and Notes for Fight : Flan 02 (Continued)
River: RIVER-1 Reach-1 RS: 7 Profile: PF 1
The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
additional cross sections.
The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
or greater than 1.4. This may indicate the need for additional cross sections.
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
This may indicate the need for additional cross sections
Multiple critical depths were found at this location. The critical depth with the lowest, valid, water
surface was used.
River: RIVER-1 Reach: Reach-1 RS: 6 Profile: PF 1
The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
additional cross sections.
The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
or greater than 1.4. This may indicate the need for additional cross sections.
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
This may indicate the need for additional cross sections.
River: RIVER-1 Reach: Reach-1 RS: 5 Profile: PF 1
The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
additional cross sections.
The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
or greater than 1.4. This may indicate the need for additional cross sections.
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
This may indicate the need for additional cross sections.
River: RIVER-1 Reach: Reach-1 RS: 4 Profile: PF 1
The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
additional cross sections.
The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
or greater than 1.4. This may indicate the need for additional cross sections.
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
This may indicate the need for additional cross sections.
River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1
The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
additional cross sections.
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
This may indicate the need for additional cross sections.
River: RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
This may indicate the need for additional cross sections.
River: RIVER-1 Reach: Reach-1 RS: 1 Profile: PF 1
The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.

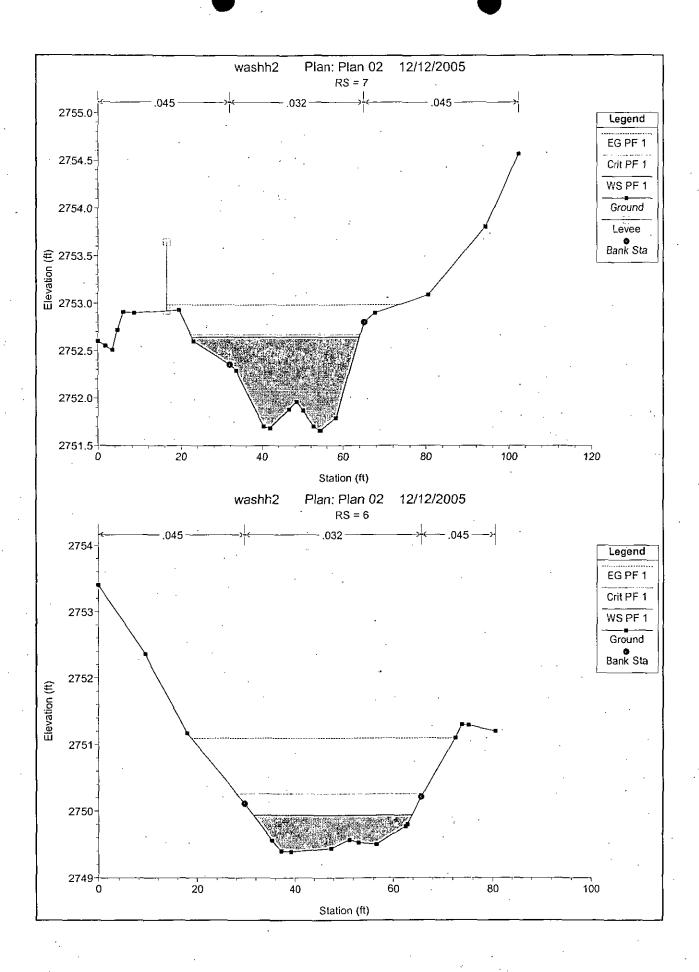


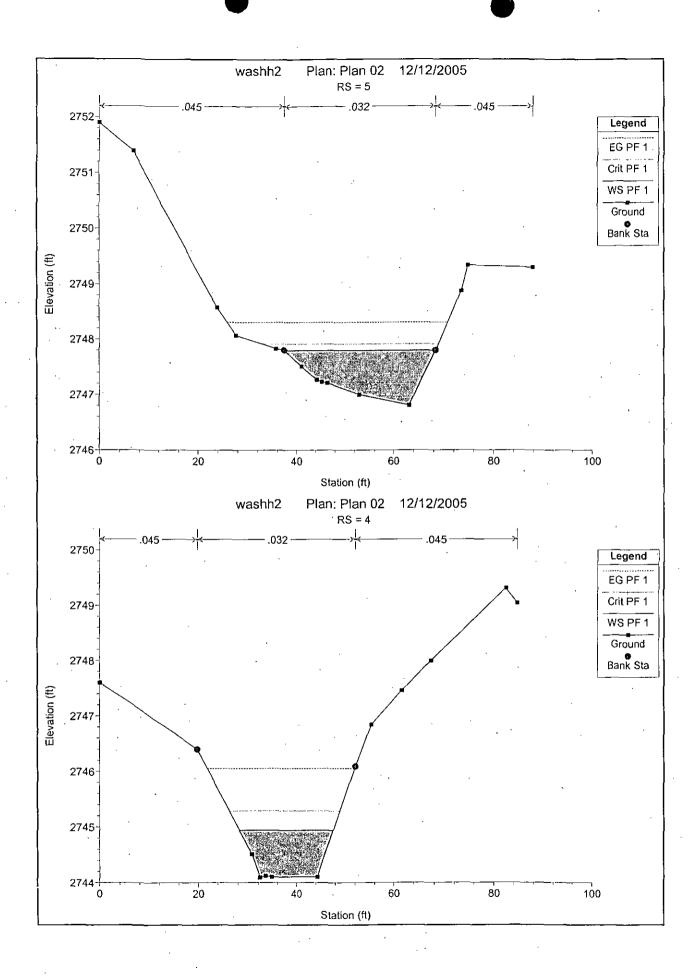


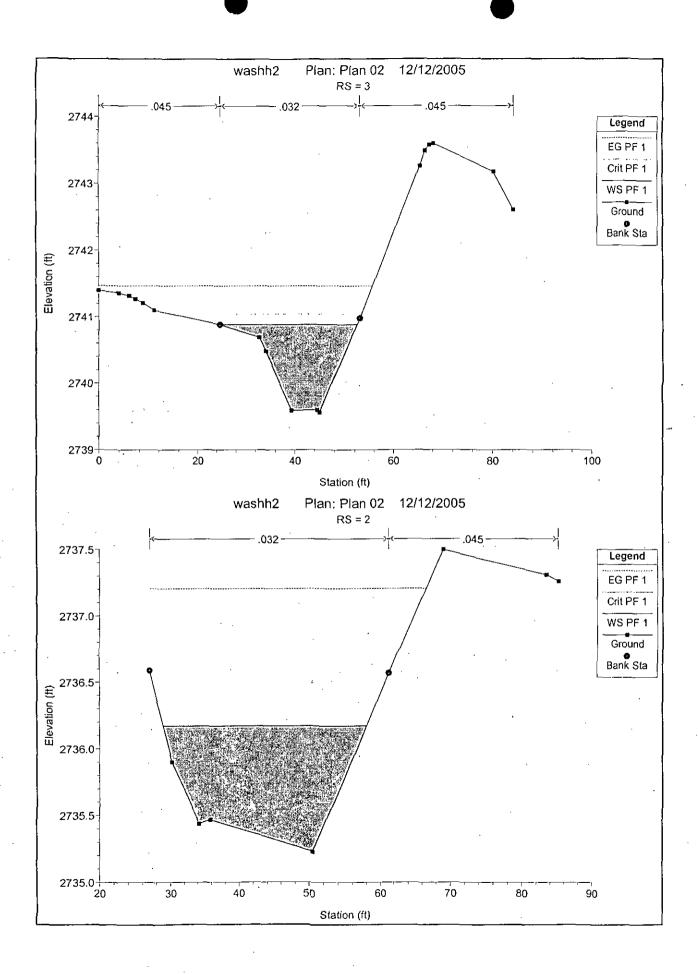


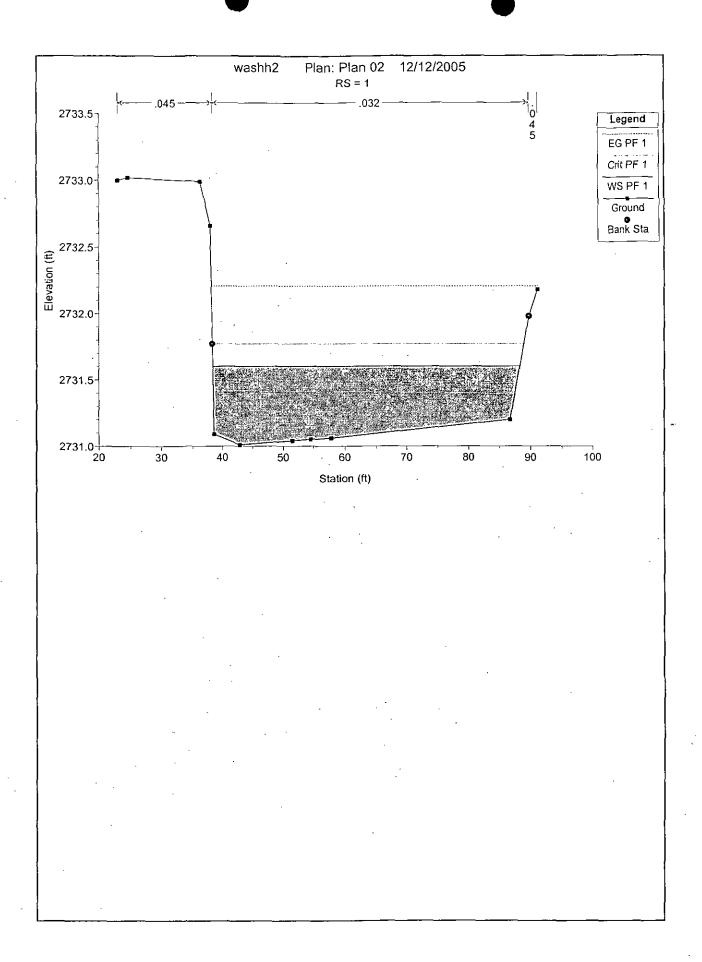












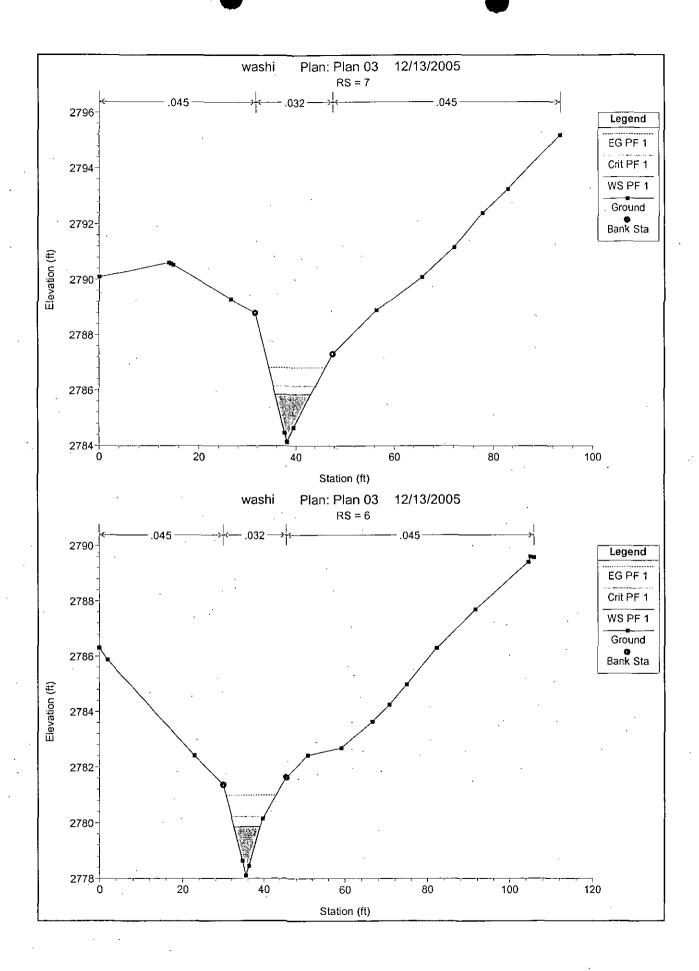
# WASHI

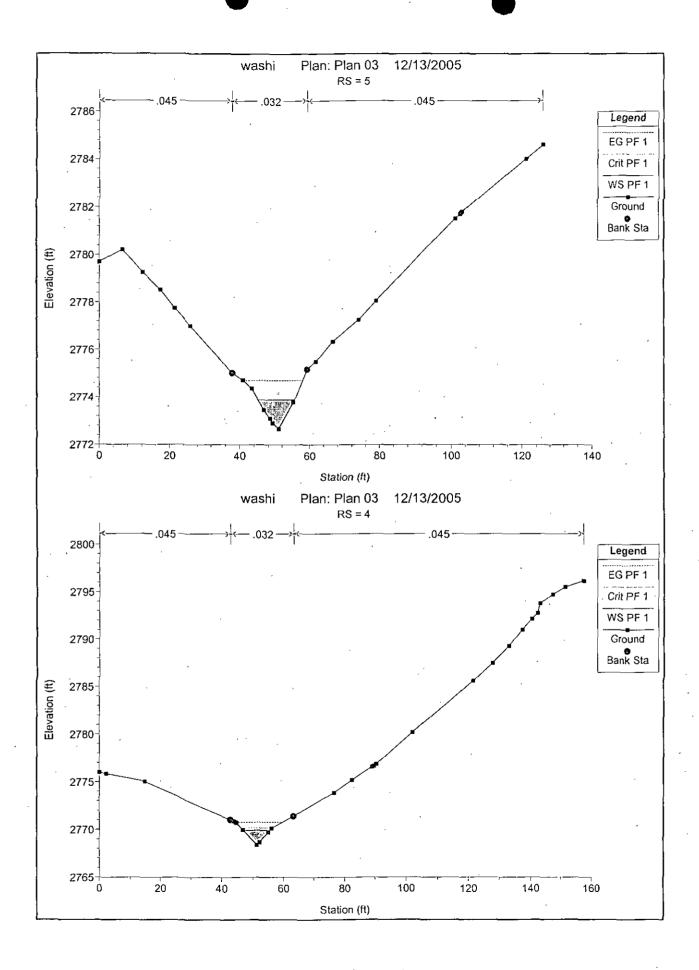
HEC-RAS	Plan: Plan 03	River RIVER-1	Reach: Reach-1	Profile: PF 1

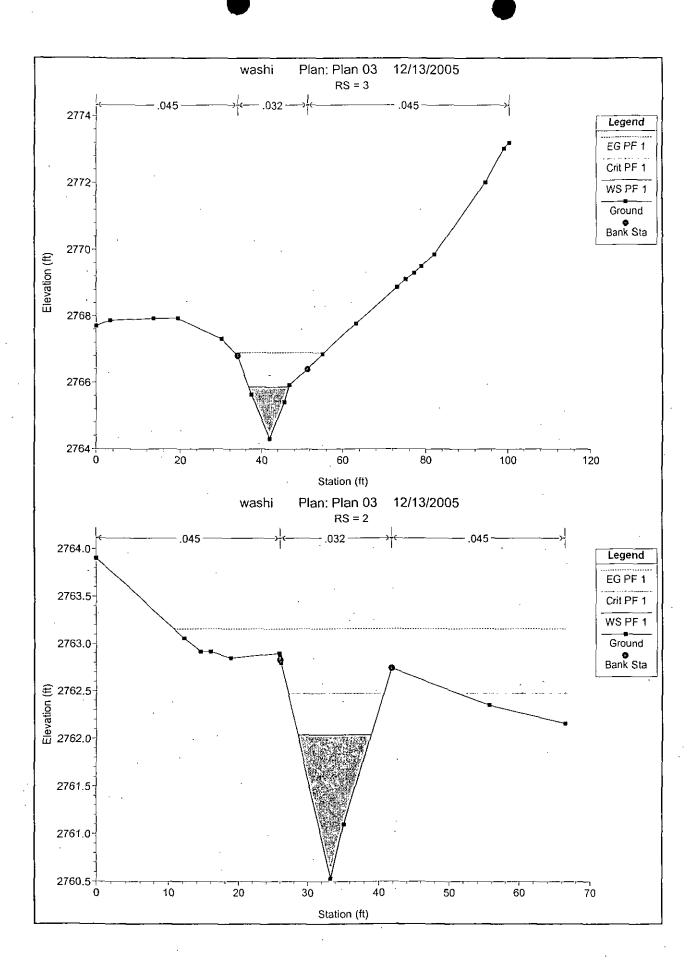
												≨ Froude # Chl ₫
total rate	green and		配包 (cfs)[254]	[編集(ft)] [1]	现程(ft)汽车	第5字(ft)记载至	经现象(ft) 完整	部落(fl/fl) 字》	(lt/s)	યુક્ત (sq ft) 🐝	系(W (ft) 智慧	是自己的政策的
Reach-1% ##			50.00	2784 14	2785.86	2786.15	2786 82	0.041077	7.86	6.36	7,51	1.50
Reach-1				2778.10	2779 85	2780 22	2780.99	0.047685	8 57	5.84	6 64	1.61
Reach-1988				2772.66	2773 89	2774.14	2774.70	0 043982	7 22	6.92	10.54	1 57
Reach-12				2768.37	2769.92	2770.18	2770.77	0.038173	7.42	6,74	8.58	1.47
Reach-1#				2764.31	2765.87	2766.22	2766.90	0.043829	8.14	7.99	9.95	1.60
Reach-1				2760,53	2762 04	2762.47	2763.16	0.053335	8 49	7.66	10.43	1.75
Reach-1			65.00	2757.88	2758.45	2758 55	2758.76	0.054721	4.64	15.30	60.02	1.55
Reach-1	160 2	PF_1/38 (3)	79.00	2754.90	2755.91	2756.06	2756 37	0 031284	5 78	16.24	37.92	1 32

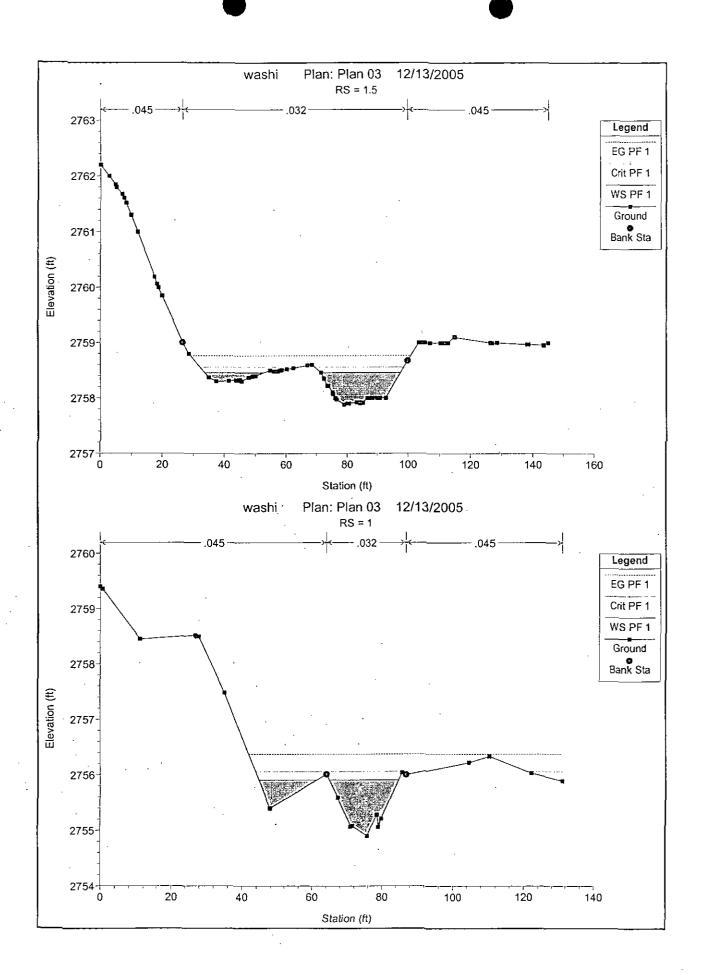
Errors Warnings and Notes for Plan: Plan 03

	3
Location:	River: RIVER-1 Reach: Reach-1 RS: 6 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 5 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 4 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 3 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 2 Profile: PF 1
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 1.5 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
Warning:	The cross-section end points had to be extended vertically for the computed water surface.
Warning:	The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for
	additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.
Location:	River: RIVER-1 Reach: Reach-1 RS: 1 Profile: PF 1
Warning:	Divided flow computed for this cross-section.
Warning:	The cross-section end points had to be extended vertically for the computed water surface.
Warning:	The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7
	or greater than 1.4. This may indicate the need for additional cross sections.
Warning:	The energy loss was greater than 1.0 ft (0.3 m), between the current and previous cross section.
	This may indicate the need for additional cross sections.









## APPENDIX E

**Detention Basin Volume Calculations** 

### Sereno Canyon

### Online Detention Volume Calculations

PROVIDED VOLUME ESTIMATE						
		BASIN - A1				
	A	REA	VÕI.	UME		
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]		
2772	1604	0 037	0.000	0.000		
2773	2151	0.049	0.043	0.043		
2774	2763	0.063	0.056	0.100		
2775	3441	0.079	0.071	0,171		

TOTAL	PROVIDED	voi	IIMF:

0.171 acre-feet

PROVIDED VOLUME ESTIMATE	
BASIN . A1-7	

BASIN - A1-2						
	Ā	REA	VOI.	UME		
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM (acre-feet)		
2756	4199	0.095	0 000	0.000		
2757	5251	0,121	0.108	0.108		
2758	6404	0.147	0.134	0 242		
2759	7655	0,176	0 161	0 404		

DED VOLUME:	0.404	acre-le

F	ROVIDE	D NOTOW	E ESTIMAT	Ε	
,		BASIN - AZ	E-1	•	(7+ T ·
<u> </u>	A	REA	VOL	UME	l
\$TAGE	[ft²]	[acres]	INC	CUM	
ļ ,	[ո ]	(acres)	[acre-feet]	(acre-teet)	ŀ
2759	323	0.007	0.000	0.000	l
2759	625	0 014	0.011	0 011	
2760	1109	0 025	0 020	0.031	[
2761	1768	0.041	0.033	0.064	1

TOTAL PROVIDED VOLUME:

ROVIDED VOLUME:	•	0.004	au b-ie
PROVIDED VOLUME	ESTIMATE		

PROVIDED VOEDINE ESTIMATE						
BASIN - A2-2						
	Á	VOL	UME			
STAGE [	[ft <sup>2</sup> ]	[acres]	INC lacre-feeth	CUM lacre-teeth		
2758	47	0.001	0 000	0 000		
2759	187	0 004	0 003	0 003		
2760	430	0.010	0 007	0.010		
2761	950	0 022	0 0 1 6	0 026		

TOTAL PROVIDED VOLUME

IE:	0.026	acre-fe

PROVIDED VOLUME ESTIMATE							
	BASIN - A2-3						
	Ā	REA	VOL	UME			
STAGE	(ft²)	[acres]	INC [acre-feet]	CUM [acre-feet]			
2733	2653	0.061	0.000	0 000			
2734	3460	0.079	0 070	0 070			
2735	4363	0.100	0 090	0.160			
2736	5357	0.123	0.112	0 272			
2727	8474	0.140	0.125	0.407			

acre-feet

TOTAL PRO	0.407			
F	ROVIDE	D VOLUM	E ESTIMAT	E
		BASIN - B	-1	
	A	REA	VOL	UME
STAGE	[ft²]	[acres]	INC (acre-feet)	CUM (acre-feet)
2700	800	0.010	0.000	7.70

TOTAL PROVIDED VOLUME:

## PROVIDED VOLUME ESTIMATE

BASIN - B-2					
AREA		REA	VOL	DLUME	
STAGE	[ft²]	[acres]	INC (acre-feet)	CUM [acre-feet]	
2798	2378	0 055	0 000	0.000	
2799	3095	0.071	0 063	0 063	
2500	3952	0 091	0.081	0.144	
2301	4986	0.114	0.103	0 246	

TOTAL PROVIDED VOLUME:

PROVIDED VOLUME ESTIMATE BASIN - 8-3						
STAGE	[n²]	[acres]	INC [acre-feet]	CUM [acre-feet]		
2781	3419	0 078	0.000	0 000		
2782	4333	0 099	0.083	680 0		

TOTAL PROVIDED VOLUME:

0.338

		BASIN - B	-4	
AREA VOLUME				
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2768	2154	0 049	0.000	0.000
2769	2950	0.069	0.059	0.059
2770	3950	0.091	0.080	0.139
2771	4983	0.114	0 103	0 241

TOTAL PROVIDED VOLUME:

0.241 acre-feet

	PROVIDE	D AOLOW	E ESTIMAT	E
		BASIN - B	-6	
AREA VOLUME				
STAGE	[tt <sub>2</sub> ]	{acres}	INC [acre-feet]	CUM [acre-teet]
2745	8320	0.191	0.000	0 000
2747	9979	0 229	0 210	0210
2749	11724	0 269	0 249	0.459
2749	13573	0 312	0 290	0.750

TOTAL PROVIDED VOLUME:

0.750 acre-feet

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. Proved ac/ft	100yr-2hr. Required ac/ft	Volume Shortage / Excess ±
A1	A1-1 A1-2	0.0790 0.1757	0,171 0.404		
	TOTAL	0.2547	0 574	0.53	0 044

VOLUME BREAKDOWN

r-	Orainage	Sub-Basin	Disturbance	100yr-2hr.	100yr-2hr.	Valume
	Boundary	Volumes	Areas (acres)	Proved ac/ft	Required acrit	Shortage / Excess ±
	A2	A2-1	0.0406	0.064	<u> </u>	\
1		A2-2	0.0218	0.026	!	
		A2-3	0.1476	0 407	}	1
ŭ.		A2-4	0.0376	0.0504	ነ	}
1		A2-5	0.1274	0.2743	1	
ı		A2-6	0.1104	0 2304		
		TOTAL	0.4855	1.051	1,02	0.031

PROVIDED VOLUME ESTIMATE

BASIN - A2-4					
	Af	₹EA	VOL	UME	
STAGE	[ft²]	[acres]	INC (acre-feet)	CUM [acre-teet]	
2792	168	0.004	0.000	0.000	
2793	425	0 010	0.007	0 007	
2794	866	0 020	0.015	0.022	
2735	1638	0.038	0.029	0.050	

TOTAL PROVIDED VOLUME:

	PROVIDE	D VOLUME ES	TIMATE		
BASIN - A2-S					
	AF	REA	VOL	UMÉ	
STAGE	[ft²]	[acras]	INC (acre-feet)	CUM [acre-feet]	
2772	2611	0.060	0 000	0 000	
2773	3465	0 080	0.070	0.070	
2774	4402	0.101	0 090	0.160	
2776	EE40	0.117	0.114	0.274	

TOTAL PROVIDED VOLUME:

0.274

acre-feet

PROVIDED VOLUME ESTIMATE

BASIN - A2-6					
	AREA		VOLUME		
STAGE	(n²)	(acres)	INC [acre-feet]	CUM [acre-teet]	
2772	2085	0.048	0.000	0,000	
2773	2843	0.065	0.057	0.057	
2774	3745	0.085	0.076	0.132	
2775	4811	0.110	860.0	0 230	

TOTAL PROVIDED VOLUME:

0.230

		AOCOME B	CEARDOWN		
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr, Proved ac/ft	100yr-2hr. Required ac/ft	Volume Shortage / Excess ±
В	8-1 B-2 B-3 B-4 B-5	0.0564 0.1145 0.1523 0.1144 0.3116	0,107 0,246 0,338 0,241 0,750		
	TOTAL	0.7491	1.683	1,54	0.143

N 12004 042054 Dwg Design/Retention volume calc ds

PROVIDED VOLUME ESTIMATE

THOUSE TO LOTHER TO							
	BASIN - C-1						
	VOL	JME					
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]			
2779	4198	0.096	0.000	0.000			
2780	5112	0.117	0.107	0.107			
2761	6102	0.140	0.129	0.236			
2782	7169	0.165	0.152	0.388			

TOTAL PROVIDED VOLUME:

0.388

acre-feet

PROVIDED VOLUME ESTIMATE						
	BASIN - C-2					
	AREA		VOLUME			
STAGE	44.21	[acres]	INC	CUM		
	[ft²]		[acre-feet]	[acre-feet]		
2754	1955	0 045	0.000	0.000		

TOTAL PROVIDED VOLUME:

acre-feet

0.225

PROVIDED VOLUME ESTIMATE					
	(c	BASIN - D			
	Al	VOLUME			
STAGE	[ft²]	[acres]	INC {acre-feet}	CUM [acre_feet]	
2749	2054	0.047	0.000	0.000	
2750	2819	0.065	0.056	0 056	
2751	3699	0.085	0.075	0.131	
2752	4712	0.108	0.097	0 227	

TOTAL PROVIDED VOLUME:

0.227 acre-feet

PROVIDED VOLUME ESTIMATE							
	BASIN - D-2						
	A	VOL	UME				
STAGE	[ft²]	{acres}	INC (acre-feet)	CUM [acre-feet]			
2724	3734	0.086	0 000	0 000			
2725	4728	0 109	0 097	0 097			
2726	5799	0.133	0 121	0218			
2727	6943	0 159	0.146	0.364			

TOTAL PROVIDED VOLUME:

acre-feet

0.364

0.500

PROVIDED VOLUME ESTIMATE							
BASIN - D-3							
STAGE	A	REA	VOLUME				
	[ft²] [acres]	INC	CUM				
i	[it ]	[acres]	[acre-feet]	[acre-feet]			
2736	5188	0 119	0 000	0 000			
2737	6505	0,149	0.134	0.134			
0720	7000	0.400	0.466	0.200			

2739 9480 0218 0200

TOTAL PROVIDED VOLUME:

	PROVIDE	D VOLUM	E ESTIMAT	E
		BASIN - E		· — · · · · · ·
	A	REA	VOLUME	
STAGE	[ft²] [acres]		INC [acra-feet]	CUM [acre-feet]
2724	5286	0 121	0.000	0.000
2725	6163	0.141	0.131	0.131
2726	7100	0.163	0.152	0 284
2727	2006	A 196	0.174	0.450

TOTAL PROVIDED VOLUME:

0.458

PROVIDED VOLUME ESTIMATE

BASIN - E1-2

	AREA		VOLUME	
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2716	7154	0.164	0,000	0.000
2717	8605	0.198	0.181	0.181
2718	10153	0.233	0.215	0 396
2719	11781	0 270	0 252	0 648

TOTAL PROVIDED VOLUME:

0.648 acre-feet

PROVIDED VOLUME ESTIMATE BASIN - E1-3 AREA STAGE [ft²] [acres] 5080 6952 8543 10236 0.117

TOTAL PROVIDED VOLUME:

0.532 acre-feet

PROVIDED VOLUME ESTIMATE BASIN - E1-4 AREA STAGE CUM [ft²] [acres] [acre-feet] [acre-feet] 0 000 4285 8600 0 108 0 238 0 391 0.119 0.108 0.141

TOTAL PROVIDED VOLUME:

0.391

		PROVIDE	D AOT NW	E ESTIMAT	E	
j			BASIN - E2	-1		1
ł		AREA		VOL	UME	1
- }	STAGE	{ft²}	[acres]	INC	CUM	7
ì			[acre-feet]	[acre-feet]	)	
ii	2710	4399	0.101	0.000	0.000	],
• 1	2711	5399	0.124	0.112	0 112	Ĭ
ļ	2712	6476	0.149	0.136	0 249	Ĭ
ı	2713	7667	0.176	0.162	0 411	1
	2714	9046	0.208	0 192	0 603	4,010

TOTAL PRO	OVIDED VO	LUME	. 0 132	0 603	4,01deema	
TOTACTAL	31.000 10	CUME.		0,800	aue-reet	•

	LUCAIDE	D AOTOW	E ESTIMAT	E _	
		BASIN - E	2.2		โ
	A	REA	VOL	UME	1
STAGE	(ft²)	[acres]	INC	CUM	1
			[acre-feet]	[acre-feet]	f
2694	6967	0.160_	_0 000	0.000	]
2595	8344	0.192	0 176	0.176	N.
2696	9919	0 228	0.210	0.385	<b>\</b>
2697	11588	0.266	0 247	0.632	1 .
2698	13358	0.307	0.286	0.919	a or or orn I

TOTAL PROVIDED VOLUME:

0,919

			VOLUME 8	REAKDOWN		
	Drainage	Sub-Basin	Disturbance	100yr-2hr.	100yr-2hr.	Volume
i	Boundary	Volumes	Areas (acres)	Proved ac/ft	Required ac/ft	Shonage / Excess ±
	E2	E2-1	0.2077	0,603	T	1
		E2-2	0.3069	0.919		\
•		E2-3	0 1862	0 407 .		
		TOTAL	0.7007	1.929	. 1.87	0.059 ,
			1			

**VOLUME BREAKDOWN** 

VOLUME BREAKDOWN

VOLUME BREAKDOWN

Disturbance

0.1858 0.2705 0.2350

0 1654

0.8567

100yr-2hr. Proved ac/ft 0.458 0.648 0.532

0.391

2.029

Disturbance Areas (acres) 0.1082

0.1594 0.2176

0.4852

100yr-2hr. Proved ac/fi 0 227

0 364 0 500

1.091

Disturbance

0.1646 0.1121 0.2767

100yr-2hi

Proved acft 0.388 0.225 0.613

100yr-2hr.

Required ac/ft

100yr-2hr. Required ac/ft

1.09

100yr-2hr. Required ac/ft

Volume

Shortage / Excess

0.103

Volume

Shortage / Excess

0.001

Volume

Shortage / Excess

Sub-Basin

Valumes C-1 C-2 TOTAL

Sub-Basin Volumes D-1

D-2 O-3

TÖTAL

Sub-Basin

Volumes E1-1

E1-2 E1-3 E1-4

TOTAL

Drainage

Boundary

Boundary D

Drainage

Boundary E1

		D VOLUME ES	TIMATE	
		BASIN - E2-3		
· ·	Af	REA	VOL	UME
STAGE	[tt²]	(acres)	INC [acre-feet]	CUM (acre-feet)
2082	397B	0 091	0 000	0 000
2683	5153	0.118	0.105	0.105
2684	6529	0.150	0.134	0 239
ORGE	0140	0.400		

TOTAL PROVIDED VOLUME:

PROVIDED VOLUME ESTIMATE

BASIN - E3-1							
	A	REA	VOLUME				
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM {acre-feet}			
2711	3064	0.070	0.000	0.000			
2712	4065	0 093	0.082	0.082			
2713	5197	0,119	_0.106	0.188			
2714	6456	0 148	0.134	0.322			

	VOLUME BREAKDOWN						
Drainage Boundary E3	Sub-Basin Volumes E3-1	Oisturbance Areas (acres) 0.1482	100yr-2hr. Proved ac/ft 0.322	100yr-2hr. Required ac/ft	Volume Shortage / Excess ±		
·····	TOTAL	0.1482	0.322	0,31	0.012		

TOTAL PROVIDED VOLUME:

0.322 acre-feet

BASIN - F1-1							
	A	REA	VOLUME				
STAGE	STAGE [ft²]		INC [scre-feet]	CUM [acre-feet]			
2/26	4563	0 105	0.000	0.000			
2727	5553 0.127	0.127	0 116	0.116			
2728	6670	0.153	0.140	0 256			
27.29	7918	0.182	0 167	0.424			

	. VOLUME BREAKDOWN							
ſ	Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. Proved ac/ft	100yr-2hr. Required acrit	Volume Shortage / Excess ±		
	F1	F1-1 F1-2 F1-3 F1-4	0.1818 0.1987 0.2059 0.1128	0 424 0 462 0 483 0.227				
_		TOTAL	0.6992	1.596	1.44	0.156		

TOTAL PROVIDED VOLUME:

PROVIDED VOLUME ESTIMATE					
		BASIN - F1			
	A	REA VOLUME		UMĒ	
STAGE	[ft²]	[acres]	INC (acre-feet)	CUM [acre-feet]	
2710	4891	0.112	0.000	0.000	
2711	6044	0.139	0.126	0.126	
2712	7298	0.168	0.153	0.279	
2713	8656	0.199	0.183	0 462	

TOTAL PROVIDED VOLUME:

0.462

PROVIDED VOLUME ESTIMATE					
BASIN - F1-3					
	Ä	ŔĔĀ	VOL	UME	
STAGE	[ft²]	[acres]	INC	CUM	
	fir l	[acres]	[acre-feet]	[acre-feet]	
2696	5180	0.119	0.000	0.000	
2697	6353	0.146	0.132	0.132	
2698	7824	0.175	0.160	0 293	
2600	<b>8969</b>	0.206	0 190	0.483	

TOTAL PROVIDED VOLUME:

0.483

PROVIDED VOLUME ESTIMATE

BASIN - F1-4					
	A	AREA		UME	
STAGE	{ft²}	[acres]	INC [acre-feet]	CUM [acre-feet]	
2686	1919	0.044	0.000	0 000	
2687	2757	0.063	0 054	0.054	
2688	3733	0 086	0.074	0.128	
2689	4913	0.113	0.099	0.227	

TOTAL PROVIDED VOLUME:

0.227 acre-feet

PROVIDED VOLUME ESTIMATE				
BASIN - F2-1				
1	AREA	VOL	UME	
F 07.05		18.10	01111	

'n		A	REA	VOLUME	
I	STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
ľ	2701	1923	0.044	0.000	0 000
ľ	2702	2906	0.067	0.055	0 055
۲	2703	3987	0 092	0 079	0.135
Ĺ	2704	5172	0.119	0 105	0 240

PROVIDED VOLUME ESTIMATE

Drainage	Sub-Basin	Disturbance	100yr-2hr.	100yr-2hr.	Volume
Boundary	Volumes	Areas (acres)	Proved ac/ft	Required acth	Shortage / Excess ±
F2	F2-1	0.1187	0 240		- ""
•	F2-2	0.1329	0.254		
	F2-3	0.0462	0.075		L
	TOTAL	0.2978	0.568	0.5	0.068

**VOLUME BREAKDOWN** 

TOTAL PROVIDED VOLUME:

0.240 acre-feet

[ft²] [acre-feet] 0.000 1841 2963

2080	3/09	Ų. į S.
TOTAL PR	OVIDED VO	LUME:

	SKONIDE	D AOTOW	EESIIMAT	E
		BASIN - F2	-3	
	. Al	REA	VOLUME	
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM (acre-feet)
2692	353	0 008	0 000	0.000
2693	770	0.018	0.013	0.013
_2694	1302	0.030	0.024	0.037
2695	2011	0.046	0.038	0.075

TOTAL PROVIDED VOLUME:

	PROVIDE	D VOLUM	E ESTIMAT	E
		BASIN - F3	l-1	
	AREA		VOLUME	
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM (acre-fee

	AREA		VOLUME	
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM (acre-feet)
2724	178	0 004	0 000	0.000
_ 2725	410	0.009	0.007	0.007
2726	746	0.017	0.013	0.020
2727	1279	0.029	0 023	0.043

TOTAL PROVIDED VOLUME:

PROVIDED VOLUME ESTIMATE

BASIN - F3-2					
	A	REA	VOLUME		
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]	
2706	1932	0.044	0.000	0 000	
2707	2855	0.066	0 055	0 055	
2708	3941	0.090	0.078	0.133	
2709	5286	0.121	0 106	D 239	
2710	6815	0.156	0 139	0 378	

TOTAL PROVIDED VOLUME:

VOLUME BREAKDOWN Drainage Boundary F3 100yr-2hr. 100yr-2hr. Volume Areas (acres) 0.0294 0.1565 0.1858 0.043 0.378 0.421

PROVIDED VOLUME ESTIMATE					
		BASIN - G			
	AREA		VOL	UME	
STAGE	[ft²]	[acres]	INC	CUM	
	[a]	(BW C3)	(acre-feet)	[acre-feet]	
2723	6168	0.142	0.000	0.000	
2724	7712	D 177	0,159	0.159	
2725	9387	0 215	0.196	0 356	
2726	11197	0 257	0.236	0.592	

**VOLUME BREAKDOWN** 

	Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. Proved ac/ft	100yr-2hr. Required ac/ft	Volume Shortage / Excess ±
Ì	G	G-1	0,2570	0 592		
		TOTAL	0.2570	0.592	0.52	0.072

TOTAL PROVIDED VOLUME:

0.592

PROVIDED VOLUME ESTIMATE

		BASIN - H1		
	A	AREA VOLUM		UME
STAGE	in²1	[acres]	INC	CUM
	fir 1	[scres]	[acre-feet]	[acre-feet]
2770	524	0.012	0.000	0.000
2771	1010	0 023	0 0 18	0.018
2772	1621	0 037	0.030	0 048
2773	2383	0.055	0.046	0.094

100yr-2hr. Required ac/ft 100yr-2hr Volume Shortage / Excess :

Areas (acres)
0 0547
0.0307
0.1655
0 2362
0 2956 Volumes H1-1 H1-2 H1-3 H1-4 H1-5 Proved ac/ft 0,094 0,044 0,474 0.5850 0.7467 1.943 TOTAL 0.123

VOLUME BREAKDOWN

TOTAL PROVIDED VOLUME:

0.094	acre-feet

PROVIDED VOLUME ESTIMATE								
	BASIN - H1-2							
	Α	REA	VOLUME					
STAGE	[#²]	[acres]	INC (acre-feet)	CUM [acre-feet]				
2766	179	0 004	0 000	0 000				
2767	418 .	0 010	0.007	0.007				
2768	757	0.017	0.013	0.020				

BASIN - H1-4						
	AREA		VOL	JME		
STAGE	l ter l factael l ' '''	, INC (acre-feel)	CUM " [acre-feet]			
2735	5797	0.156	0.000	0 000		
2735	7887	0.181	0 169	0.169		
2737	9051-	0.208	0.194	0 363		
2738	10290	0.236	0.222	0 585		

PROVIDED VOLUME ESTIMATE BASIN - H1-5

[acres]

TOTAL PROVIDED VOLUME:

0.044 acre-feet TOTAL PROVIDED VOLUME:

STAGE

Drainage

0.585

PROVIDED VOLUME ESTIMATE

BASIN - H1-3						
1	A	REA	VOL	UME		
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]		
2736	3296	0 076	0.000	0 000		
2737	4159	0 095	0.086	0.086		
2738	5099	0.117	0.106	0 192		
2739	6118	0.140	0.129	0.321		
2740	7208	0.165	0 153	0.474		

[ft²] 8891 10158 11486 12876

Sub-Basin

TOTAL PROVIDED VOLUME:

0.474

4.0 ception

TOTAL PROVIDED VOLUME:

0.747

CUM

acre-feet

	PROVIDED VOLUME ESTIMATE						
		BASIN - H	2-1				
	AF	REA	VOLUME				
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]			
2772	6928	0.159	0 000	0 000			
2773	8286	0 190	0 175	0.175			
2774	9774	0.224	0 207	0.382			
2775	11346	0.260	0.242	0.624			

VOLUME BREAKDOWN

			1		
Drainage	Sub-Basin	Disturbance	100yr-2hr.	100yr-2hr,	Volume
Boundary	Volumes	Areas (acres)	Proved ac/ft	Required acrit	Shortage / Excess ±
H2	H2-1	0,2605	0.624		T
DW	H2-2	0.2463	0.590		
	TOTAL	0.5068	1 215	12	0.015

INC

[acre-feet] 0.000 0.219 0.248 0.280

TOTAL PROVIDED VOLUME:

0.624

PROVIDED VOLUME ESTIMATE

(		BASIN - H	2-2	
	Ai	REA	VOL	UME
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM (acre-feet)
2785	6526	0 150	0.000	0.000
2786	7846	0 180	0.165	0.165
2787	9247	0.212	0.196	0361
2788	10730	0.246	0 229	0.590

PROVIDED VOLUME ESTIMATE BASIN - I-1 AREA

[acres]

0 173 0.199

TOTAL PROVIDED VOLUME:

acre-feet

0.590

CUM

0.640

[acre-feet] 0 000 0.186

		VOLUME BE	REAKDOWN		
Drainage	Sub-Basin	Disturbance	100yr-2hr.	100yr-2hr.	Volume
Boundary	Volumes	Areas (acres)	Proved ac/ft	Required ac/ft	Shortage / Excess ±
1	<u>i-1</u>	0.2556	0.640		
					<u> </u>
	TOTAL	0.2556	0.640	0.63	0.010

TOTAL PROVIDED VOLUME:

[ft²]

7532

STAGE

Drainage Boundary I	Sub-Basin Volumes I-1	Disturbance Areas (acres) 0.2556	100yr-2hr. Proved ac/ft 0.640	100yr-2hr. Required ac/ft	Volume Shortage / Excess ±
	TOTAL	0.2556	0.640	0.63	0.010

	KOVIDE	BASIN - J	E ESTIMAT	E	
	A	REA	VOLUME		
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM (acre-feet)	
2769	3832	0.088	0.000	0.000	
2770	4689	0.108	0.098	D 098	
2771	5667	0 130	0 119	0 217	
2772	6755	0.155	0 143	0.359	
2773	7093	0.183	0.160	0.520	

**VOLUME BREAKDOWN** 

Drainage	Sub-Basin	Disturbance	100yr-2hr.	100yr-2hr.	Volume					
Boundary	Volumes	Areas (acres)	Proved acft	Required acrit	Shortage / Excess :					
J	J-1	0.1833	0.528							
	J-2	0 0881	0 187							
	TOTAL	0.2713	0.715	0.68	0.035					

TOTAL PROVIDED VOLUME:

0.528

PROVIDED		
	RASIN - L2	

		BASIN - J	-2		
	A	REA	VOLUME		
STAGE	[ft²}	[acres]	INC [acre-feet]	CUM [acre-teet]	
2762	1719	0.039	0 000	0.000	
2763	2323	0.053	0.046	0 046	
2764	3026	0.069	0.061	0.108	
2765	3836	0.088	0 079	0.187	

PROVIDED VOLUME ESTIMATE BASIN - K-1 AREA

0.097 0.115

TOTAL PROVIDED VOLUME:

0.187

			VOLUME BE	REAKDOWN		
	Orainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. Proved ac/ft	100yr-2hr. Required ac/ft	Volume Shortage / Excess ±
•	- K	W-3	0.1575	0.378		
		TOTAL.	0.1575	0.378	0 29	0 088

0.378 acre-feet

·			EESTIMAT	<u>-</u>
		BASIN - L	.1	
	_ A	REA	VOLUME	
STAGE	[ft <sup>7</sup> ]	(acres)	INC [acre-feet]	.CUM (acre-feet)
2744	1366	0.031	0 000	0 000
2745	1889	0.043	0.037	0 037

TOTAL PROVIDED VOLUME:

10 depth?

VOLUME BREAKDOWN									
Drainage	Sub-Basin	Disturbance	100yr-2hr.	100yr-2hr.	Valume				
Boundary	Volumes	Areas (acres)	Proved ac/ft	Required ac/ft	Shortage / Excess ±				
1 L	L-1	0.0434	0 037	\	}				
	L-2	0.1084	0 188	<u></u>	[				
-	TOTAL	0.1518	0.225	0.21	0.015				

PROVIDED VOLUME ESTIMATE

| CUM | (acre-feet) | (acre-fe

THE TOTAL PROPERTY.									
BASIN - L2									
	AREA		VOLUME						
STAGE	(ft²)	[acres]	INC [acre-feet]	CUM lacre-feet)					
2722	1003	0.023	0 000	0.000					
2723	2030	0.047	0.035	0 035					
2724	3288	0.075	0.061	0.096					
2725	4723	0.108	0.092	0.188					

TOTAL PROVIDED VOLUME:

PROVIDED VOLUME ESTIMATE

BASIN - M-1								
	Ai	REA	VOLUME					
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-leet]				
2721	2543	0.058	0.000	0.000				
2722	3462	0.079	0.069	0.069				
2723	4509	0.104	0.091	0.160				
2724	5737	0.132	0.118	0.278				

VOLUME BREAKDOWN									
Orainage	Sub-Basin	Disturbance	100yr-2hr.	100yr-2hr.	Volume				
Boundary	Volumes	Areas (acres)	Proved ac/ft	Required ac/ft	Shortage / Excess ±				
М	M-1	0.1317	0.278	1	1				
		<u></u>		<u> </u>					
	TOTAL	0.1317	0.278	02	0.078				

TOTAL PROVIDED VOLUME:

0.278

PROVIDED VOLUME ESTIMATE								
BASIN - N-1								
		AREA	VOL	UME				
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]				

	·	BASIN - N	-1							
	A	REA	VOL	UME	VOLUME BREAKDOWN					
STAGE	[ft²]	[acres]	INC	CUM	Drainage	Sub-Basin	Disturbance	100yr-2hr.	100yr-2hr.	Volume
	fir 1	[60/03]	[acre-feet]	[acre-feet]	Boundary	Volumes	Areas (acres)	Proved ac/ft	Required ac/ft	Shortage / Exc
2691	4188	0.096	0 000	0.000	14	N-1	0.1886	0.422		
2692	5407	0.124	0.110	0 110			L		<u> </u>	
2693	6760	0,155	0.140	0 250	<del></del>	TOTAL	0.1886	0.422	0.4	0.022
7894	8217	0.189	0.172	0.422						

TOTAL PROVIDED VOLUME:

_ D	DOMAGE	DVOL	ILBAC	FSTIM	ΔTF

		BASIN - C	-1			
	AREA		VOF	VOLUME		
STAGE	[ft²]	[acres]	INC (acre-feet)	CUM [acre-teet]		
2676	817	0 019	0.000	0 000		
2677	1226	0 028	0.023	0 023		
					r.o.depth	

			AOLOWE BY	KEAKDOWN		
	Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr, Proved ac/ft	100yr-2hr. Required ac/ft	Volume Shortage / Excess ±
•	Ö	1 O-1	0.0281	₹ 0.023		
		TOTAL	0.0281	0.023	0.01	0.013

TOTAL PROVIDED VOLUME:

0.023

DDOMMED	MOL	LIBERT	ECTHMATE	

		BASIN - P			9
	Α	REA	VOL	UME	
STAGE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]	İ
2721	3682	0.085	0.000	0 000	l
2722	4568	0.105	0 095	0.095	
			<u> </u>		1,0 dep

			VOLUME B	REAKDOWN		
	Drainage Boundary	Sub-Basın Volumes	Disturbance Areas (acres)	100yr-2hr. Proved ac/ft	100yr-2hr, Required ac/ft	Volume Shortage / Excess ±
١	Þ	P-1	0.1049	0.095		}
٠		TOTAL	0.1049	0.095	80.0	0.015

TOTAL PROVIDED VOLUME:

### PROVIDED VOLUME ESTIMATE

l .	1				
	A	REA	VOL	ì	
STAGE	[ft²] [acres]		INC [acre-feet]	CUM [acre-feet]	
2764	2082	0 048	0.000	0.000	J
2765	2732	0.063	0.055	0.055	1.0 depth

VOLUME BREAKDOWN

Orainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. Proved ac/ft	100yr-2hr. Required ac/ft	Volume Shortage / Excess ±
<u> </u>	Q-1	0.0627	0.055		
	Q-2	0.0647	0.056		i
	Q-3	0.1693	0.156		
	TOTAL	0.2967	0.268	0.24	0 028

TOTAL PROVIDED VOLUME:

0.055

PRO	OVID	ED	VOL	UME	ES	TIMA	4TE

i		Ī				
į		Α	REA	VOL		
	STAGE	[ft²] [acres]		INC [acre-feet]	CUM [acre-feet]	
Ϊ	2749	2063	0 047	0.000	0.000	
Į	2750	2818	0 065	0.056	0.056	1.0 depth

TOTAL PROVIDED VOLUME:

 _		-		-		-	-		
				_		11	-	_	_
				М.	Δ.	-15			ш

		PROVIDE	D VOLUM	E ESTIMAT	E	_
			BASIN - C	1-3		]
		Ā	REA	VOL	) F	
STA	GE	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]	
274	14	6240	D.143	0 000	0.000	10 depth
274	5	7374	0.169	0,156	0.156	

PROVIDED VOLUME ESTIMATE

TOTAL PROVIDED VOLUME:

0.156

VOLUME BREAKDOWN

BASIN - R-	-1		,	VOLUME BREAKDOWN					
REA VOLUME			Drainage	Sub-Basin	Disturbance	100yr-2hr.	100yr-2hr.	Volume	
[acres]	INC	ÇUM		Boundary	Volumes	Areas (acres)	Proved ac/ft	Required acrit	Shortage / Excess ±
[acres]	[acre-feet]	(acre-feet)		R	R-1	0.0819	0.072		1
0.062	0 000	0.000				1			
0.082	0.072	0.072	1.0 desiti 3		TOTAL	0.0819	0.072	0,07	0.002
			,	•					

TOTAL PROVIDED VOLUME:

[ft²]

STAGE

0.072

	ROVIDE	D VOLUM	E ESTIMAT	E
		BASIN - S	-1	·
	AF	REA	VOL	UME
STAGE	[ft²] ·	[acres]	INC [acre-feet]	CUM [acre-feet]
0775	2020	0.047	0.000	2.000

- 1		BASIN - S-1						
ſ		AF	REA	VOLUME				
ļ	STAGE	[ft²] .	[acres]	INC [acre-feet]	CUM [acre-feet]			
Ì	2775	2030	0.047	0.000	0.000	1		
Į	2776	2639	0.061	0 054	0.054	1.0" depth 3		

VOLUME BREAKDOWN						
Drainage Boundary S	Sub-Basin Volumes S-1	Disturbance Areas (acres) 0.0606	100yr-2hr, Proved ac/ft 0.054	100yr-2hr. Required ac/ft	Volume Shortage / Excess ±	
<del></del>	TOTAL	. 0.0606	0.054	0.02	0.034	

TOTAL PROVIDED VOLUME:

0.054 acre-feet

### Retention Volume Calculations: Sereno Canyon **Disturbance Calculations:**

Sq. Ft.
Ac.

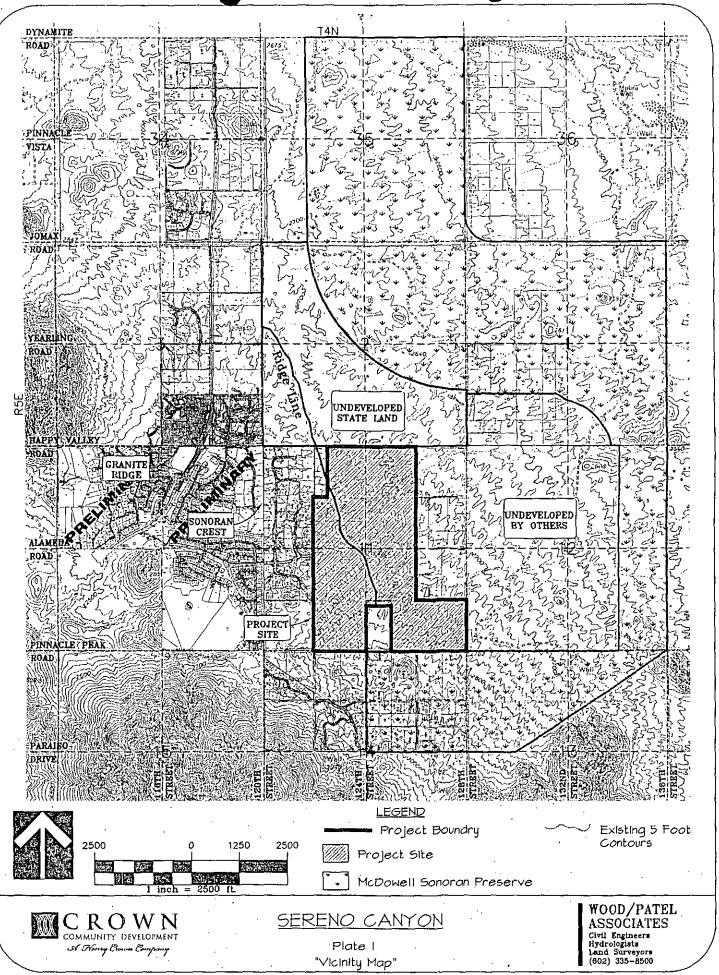
Basin Disturbance: 8.4149

Disturbance: 2.55%

Drainage	Sub-Basin	Disturbance i	100yr-2hr,	100yr-2hr.	Valume
Boundary	Volumes	Areas (acres)	Proved ac/ft	Required ac/ft	Shortage / Excess :
* A1 ·	A1-1 - A1-2	0.2547	0.574 -	0.53	0.044
A2	A2-1 - A2-3	0.4855	1 051	1.02	2 031
В	B1 - B5	0 7491	1 683	1.54	0 143
C	C1 - C2	0 2767	0.613	0.51	0.103
0	D1 - D2	0 4852	1.091	1.09	0 001
E1	E1-1 - E1-4	0.8567	2 029	2.01	0.019
E2	E2-1 - E2-2	0,7007	1,929	1 87	0.059
E3	E3-1	0 1482	0.322	0.31	0 012
F1	F1-1 - F1-4	0.6992	1.596	1,44	0.156
F2	F2-1 - F2-3	0 2978	0 568	0.5	0.068
F3	F3-1 - F3-2	0 1858	0 421	0.41	0.011
G	G1	0.2570	0.592	0.52	0 072
H1	H1-1 - H1-3	0.7827	1.943	1.82	0.123
H2	H2-1 - H2-2	0.5068	1.215	1.2	0.015
<u></u>	. (1	0 2556	0.640	0 63	0.010
J	J1 - J2	0.2713	0.715	0.68	0 035
K K	K1	D 1575	0.378	0 29	0 088
L	L1 - L2	0.1518	0.225	0.21	0 015
M	M1	0.1317	0 278	02	0.078
N	N1	0.1886	0 422	0.4	0.022
0	O1	0.0281	0 023	0.01	0 013
Р	)P1	0.1049	0.095	0.08	0 0 1 5
	Q1 - Q3	0 2967	0 268	0.24	0 028
R	R1	0 0819	0.072	0.07	0.002
S	S1	0.0606	0 054	0.02	0 034
	TOTAL	0.4440	40.700	47.	1.400

to change with Final Improvement Plan Design.

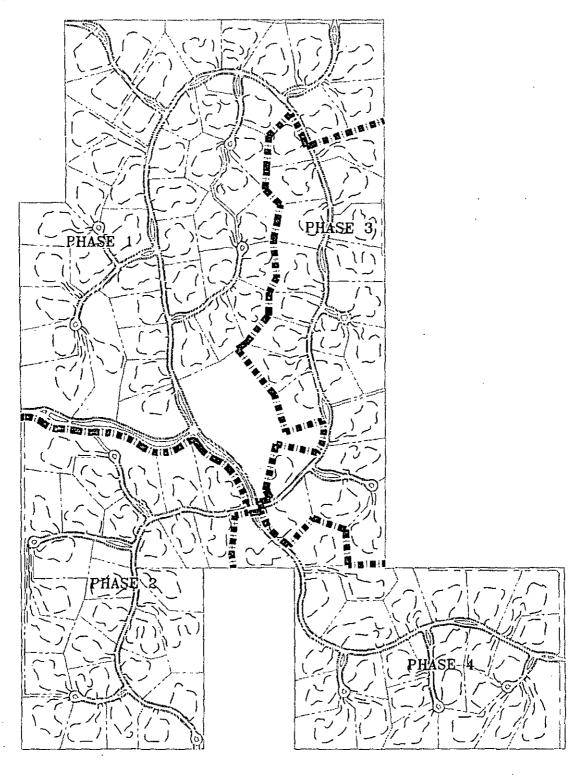
Vicinity Map



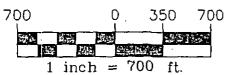
N.\2004\042054\Project Support\Hydro\Exhibits\FDR\ PLATE OI-VICINITYMAP.dug - Sop 282005

PLATE 1A

Phasing Map









### SERENO CANYON

Plate IA "Phasing Map"

### WOOD/PATEL

LAND DEVELOPMENT • WATER RESOURCES TRANSPORTATION/TRAFFIC WATER (WASTEWATER • SURVEYING CONSTRUCTION MANAGEMENT

(602) 335-8500

PHOLNIX • MESA • TUCSON

Flood Insurance Rate Map (FIRM)

OTTY OF SOUTSDALE

SERENO CANYON Plate 2 "Flood Insurance Rate Map"

CROWN COMMUNITY DEVELOPMENT

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Soils Classification Map

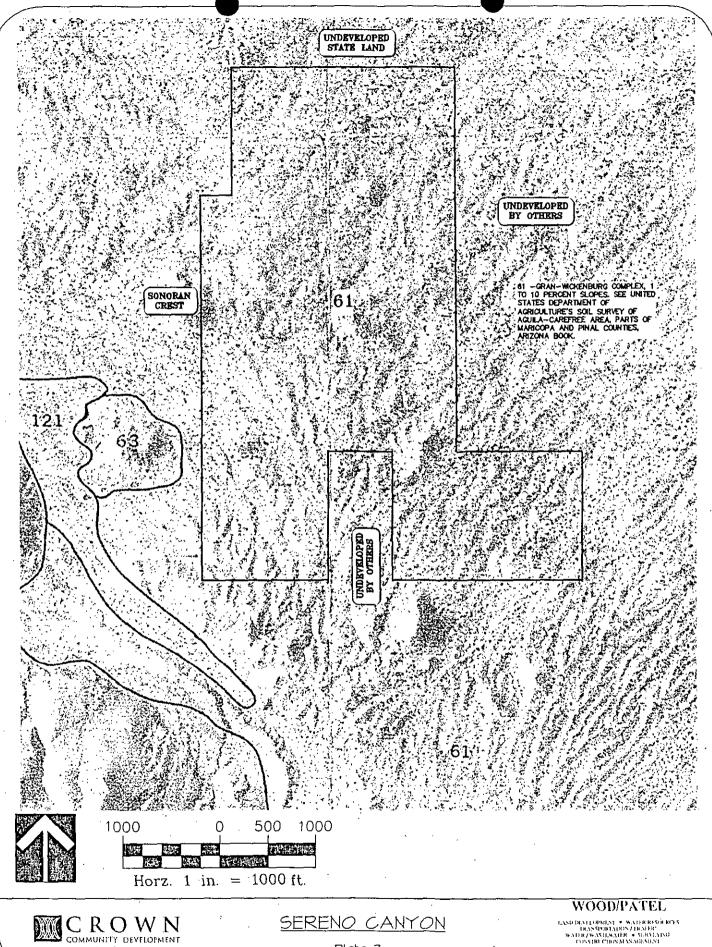




Plate 3 "Soils Classification"

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PROENTY • MESA • TECSON

404 Washes

Color Topographic Aerial Photograph

O

Off-Site Watershed Area Map

**Pre-Development Drainage Site Plan** 

**Pre-Development Grading and Drainage Plan** 

Post-Development Drainage Site Plan

Post-Development Grading and Drainage Plan