

**CONCEPTUAL
MASTER DRAINAGE REPORT
FOR
SERENO CANYON**

November 15, 2006

WP# 042054

Prepared for:

Crown Community Development
1751A West Diehl Road
Naperville, IL 60563
Phone: (630) 851-5490
Fax: (630) 898-0480

Submitted to:

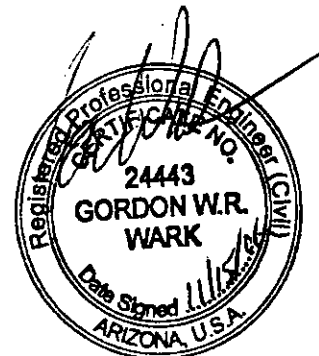
City of Scottsdale
7447 East Indian School Road
Scottsdale, AZ 85251
Phone: (480) 312-7080
Fax: (480) 312-7781

Prepared by:

Wood, Patel & Associates, Inc.
2051 West Northern Avenue
Suite 100
Phoenix, AZ 85021
Phone: (602) 335-8500
Fax: (602) 335-8580
Website: www.woodpatel.com



Peer Reviewer



Engineer

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WOOD/PATEL

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1.0 INTRODUCTION

1.1 Project Description

Sereno Canyon is a planned 330-acre custom lot residential subdivision located in the northeastern portion of Scottsdale, Arizona. The site is located in Section 11, Township 4 North, Range 5 East of the Gila and Salt River Base and Meridian. The site is currently an assemblage of undeveloped parcels bound to the west by the existing Sonoran Crest Development (122nd Street alignment), to the east by the 128th Street alignment, to the north by the Happy Valley Road alignment, and to the south by the McDowell Mountain Sonoran Preserve. Access to the development is planned from the west via the ½-mile section roadway, Alameda Road. Plate 1 provides a vicinity map for the project and surrounding areas.

Sereno Canyon is a proposed custom lot sub-division nestled at the northern base of the McDowell Mountains. Development of the project is planned to occur in four (4) phases. Please refer to Plate 1A for a phasing map of the project. The development includes 122 lots ranging in size from 2 to 3 acres and a clubhouse with amenities such as jacuzzis, pools, water falls, and restaurant facilities. Interpretive trails and scattered pocket parks with water features will also be incorporated into the site plan.

1.2 Type of Report

This report is being prepared as a Conceptual Master Drainage Plan for the proposed Sereno Canyon Project.

1.3 Purpose

This Conceptual Master Drainage Report for the proposed Sereno Canyon project has been prepared to meet the master drainage plan requirements outlined in Chapter 4 of the *City of Scottsdale Design Standards and Policies Manual (DS&PM)* effective September 25, 2006. This report presents description of the hydrological and hydraulic modeling of the proposed drainage systems.

The main purpose of this report is to illustrate the following:

- Compliance with the City of Scottsdale's Drainage Ordinance stormwater storage requirements by providing stormwater storage for 100-year, 2-hour storm event runoff for improved areas computed by Rational Method.
- Reduction of post-development flows to at or below the pre-development flows for 2-year, 10-year and 100-year, 6-hour storm events. Hence, the downstream properties are provided with similar or better flood protection as the pre-development condition.
- Maintenance of the major water courses traversing through the site in their natural location.
- Delineation of the 100-year water surface extents along with determination of the 100-year water surface elevations for the major drainage courses using HEC RAS computer program.

2.0 EXISTING ON-SITE DRAINAGE CONDITIONS AND CHARACTERISTICS

2.1 On-Site Drainage

The proposed project lies in the northern planning section of the City of Scottsdale. The Elevations range from 2,830 feet in the south to 2,675 feet in the northeast. Based on the existing topography of the site, a ridgeline runs through the property from the southern property boundary (approximately at the center) in the northwest direction.

Vegetation is typical Sonoran Desert type with creosote bush, jumping cholla, saguaro cacti, palo verde, ironwood, and mesquite trees. The project site lies within the areas identified as Environmentally Sensitive Lands (ESL) by the City of Scottsdale. The site is further classified as 'Upper Desert' within the ESL areas as shown in Plate 2 – *ESL Classification*.

Plate 3 presents the soils classification (type D) based on the Soil Conservation Service (SCS) Map for the project area. SCS is now referred to as Natural Resource Conservation Service (NRCS). The site photos for the project site are included in Appendix A. The cover type and hydrologic condition were estimated as desert shrub areas with poor hydrologic conditions. From Figure 4-6 of the *City of Scottsdale DS&PM*, a runoff curve number of 88 was used for the corresponding soils group D that occurs within the watershed area.

Currently, the site contains no development and hence an effective impervious area of zero percent was used. The soil characteristics of the site are included in the model in terms of the curve number discussed above. Also, minor rock outcroppings on the site are not considered a factor in computing the percent effective impervious areas as they do not interfere with the drainage pattern and are not hydraulically connected to the sub basin concentration points. This conclusion is made based on the photographs of the site and washes included in Appendix A. Please note that the site will not be mass graded and the outcroppings will be the same in both the existing and proposed conditions and do not impact the pre- and post-development runoff comparison.

There are no washes on the site categorized as a 'Vista Corridor' as the calculated 100-year, 6-hour flow is less than 750 cfs for all washes. The major watercourses traversing the project with a 100-year, 6-hour flow greater than 50 cfs are identified as the site lies within ESL areas. These washes are identified on Plate 9 – *Pre-Development Grading and Drainage Plan*. These washes will be maintained in their natural location and will not be re-aligned.

2.2 Existing On-Site Drainage Network

Existing on-site drainage sub-basin boundaries were identified using aerial mapped 1-foot contours. Please refer to Plate 5 – *Color Topographic Aerial Photograph*. Please refer to Plate 8 – *Pre-Development Drainage Site Plan* for the on-site sub-basins and concentration points.

Runoff generated on-site drains northeasterly or northwesterly from the ridgeline. Washes C1, D1, E1, E2, and F1, with a 100-year, 6-hour flow greater than 50 cfs, traverse the site in the northeast direction and exit at the property boundary. The other minor washes (100-year, 6-hour flow less than 50 cfs) traverse either northeasterly or northwesterly until they exit at the property boundary.

A delineation of water surface extents for washes with 100-year, 6-hour peak flow of 50 cfs or greater has been drawn based on the HEC-RAS analysis. Water surface elevations and top widths for pre-development flows at each cross-section are included on Plate 9 – *Pre-Development Grading and Drainage Plan*. The hydrologic and hydraulic analysis procedures are discussed in Section 5.0. The boundary conditions water surface elevations were determined using normal depth for the given channel slope in HEC RAS.

2.3 Off-Site Watershed

The off-site areas impacting the drainage pattern of the proposed development lie in the northern planning section of the City of Scottsdale. The off-site watershed includes a portion of the 40-acre Sonoran Sky development, an additional 20-acre parcel and mountainous terrain located within the McDowell Sonoran Preserve. Currently, the entire off-site watershed is undeveloped desert land. The aforementioned two properties may be developed in the future. The off-site area impacting the drainage patterns for the proposed

developments are identified on Plate 6 – *Off-Site Watershed Area Map*. Vegetation is typical Sonoran Desert type with creosote bush, jumping cholla, saguaro cacti, palo verde, ironwood, and mesquite trees. The off-site areas lie within the ‘Upper Desert’ and ‘Hillside’ (ESL) landform areas.

There are no off-site washes categorized as a ‘Vista Corridor’ that impact the site, as the calculated 100-year, 6-hour flow is less than 750 cfs. The major watercourses traversing the project with a 100-year, 6-hour flow greater than 50 cfs are identified. These washes will be maintained in their natural location and will not be re-aligned.

Based on the SCS Map, the cover type and hydrologic condition were estimated as desert shrub areas with poor hydrologic conditions. From Figure 4-6 of the *City of Scottsdale’s DS&PM*, a runoff curve number of 88 was used for the corresponding soils group D that occurs within the off-site watershed area.

2.4 Existing Off-Site Drainage Network

City of Scottsdale Quarter Section Maps with 2-foot contour interval topography were used to identify the off-site drainage sub-basins impacting the proposed development. Please refer to Plate 6 – *Off-Site Watershed Area Map* for the limits of the drainage sub-basins and concentration points.

Off-site flows enter the site along the southern property boundary along their natural course as overland and concentrated flows. The offsite flows impacting the project traverse the site through washes A1, A2, B1, B2, C, H1, H2, I, J and R. Washes A1, A2, B1, B2, C and R traverse the site in the northeast direction and exit at the east property boundary. Washes H1, H2, I and J traverse the site in the northwest direction until they exit at the west property boundary.

2.5 Existing Drainage Relative to Adjacent Projects

Existing drainage courses exit the site as concentrated flows at various locations on the eastern, western and northern property boundaries. These locations will be maintained with peak flows for the 2-year, 10-year and 100-year, 6-hour storm events at or below pre-developed amounts.

2.6 Flood Insurance Rate Map (FIRM)

The Flood Insurance Rate Maps (FIRM) for Maricopa County, Arizona and incorporated areas, Panel Numbers 04013C1255G and 04013C1260F, effective date September 30, 2005 indicates the site is within Zone "X" (shaded), and Zone "D".

Zone "X" (shaded) is defined by FEMA as follows:

Areas of 500-year flood; areas of 100-year flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 100-year flood.

Zone "D" is defined by FEMA as follows:

Areas in which flood hazards are undetermined.

The location of the parcel relative to the FIRM panels is illustrated on Plate 4 – *Flood Insurance Rate Map (FIRM)* using the FEMA Firmettes.

3.0 PROPOSED MASTER DRAINAGE PLAN

3.1 General Description of Proposed Drainage System

The proposed drainage system will allow existing drainage patterns to be maintained in their natural location and condition where possible. The site is being developed as large custom lots with a minimum lot size of approximately 2 acres. In the event of any future development within the off-site watershed areas, it will be the responsibility of the future developer to attenuate the post-development flows to at or below the pre-development flows.

The off-site runoff impacting the site will traverse the site through natural washes A1, A2, B1, B2, C, H1, H2, I, J and R. Washes A1, A2, B1, B2, C and R traverse through the site in the northeast direction and exit at the east property boundary. Washes H1, H2, I and J traverse the site in the northwest direction until they exit at the west property boundary.

The on-site drainage system will be composed primarily of the existing on-site washes conveying the runoff through the site and exiting at the property boundary at their historic locations. According to the City's Drainage Ordinance, stormwater storage is to be provided for the 100-year, 2-hour storm event for improved areas. Rational Method was used to estimate the required stormwater storage volumes. The procedure is discussed in detail in Section 5.2.4. Detention basins with bleed-off pipes are proposed at strategic locations to provide stormwater storage for the runoff generated during the 100-year, 2-hour storm event. Several existing on-site washes are being utilized as detention basin locations due to the steep slopes encountered within the site. Rock walls will be used to mitigate the need for large constructed slopes to attain ponding areas.

According to the City's Drainage Ordinance, all runoff generated from the developed portion of the site must be managed and the peak discharge from the site reduced to at least pre-development values. With the development being sparse in nature at approximately 0.37 dwelling units per acre and no mass grading being proposed, post-development flows are only slightly higher than pre-development flows. The proposed on-line detention basins along several existing on-site washes will also act to attenuate the post-development peak discharges to at or below the pre-development values. A

detailed description on the design and analysis of these basins is provided in Section 3.5.1.

Roadway runoff will be conveyed along the street up to the capacity of the street within the required stipulations outlined in Figure 4-2 – *Hydraulic Design Criteria* of the *City of Scottsdale DS&PM*. Please refer to Appendix A for a copy of this figure. The additional runoff exceeding the street capacity will be released through curb openings or storm drain system. Curb openings will occur where there is a wash adjacent to the roadway or at culvert crossings.

3.2 Future Conditions

Currently, the off-site areas impacting the site are undeveloped desert lands. The areas proposed to be developed in the future are the 40-acre Sonoran Sky development and the 20-acre parcel at the southern property boundary. Currently no approved drainage reports exist for these two developments. It will be the responsibility of these developments to maintain their post-development flow peaks and volume at or below the pre-development flow peaks and volume when they develop. The proposed drainage systems are designed assuming that there will be no increase in peak flows during the 100-year, 6-hour event upstream of the project site.

The drainage plan for Sereno Canyon proposes to maintain the 2-year, 10-year and 100-year, 6-hour storm event post-development peak discharges at or below the pre-development flows. Also, the drainage courses will be maintained at their historic locations at the property boundary. Thus, no adverse impact is anticipated on the adjacent properties downstream of the project site for the 2-year, 10-year and 100-year, 6-hour storm events.

3.3 Stormwater Storage Requirements

Based on the City of Scottsdale's Drainage Ordinance stormwater storage requirements, on-site stormwater storage is to be provided for runoff generated during the 100-year, 2-hour storm event for improved areas. Rational Method is used to estimate the 100-year, 2-hour stormwater storage volumes. 100-year, 2-hour event stormwater runoff is proposed to be detained on site through detention basins upstream of culvert crossings and at other strategic locations along existing on-site washes. Several of the detention

basins were designed to detain runoff at a depth greater than three feet (3') in order to meet the volume provided requirements within the Environmentally Sensitive Lands Ordinance (ESLO) and Natural Area Open Space (NAOS) constraints. Detention basins were sized at a depth greater than three feet (3'), as grading the detention basins results in large disturbed areas, which are not in accordance with the ESLO intent. Based on the topographical conditions, washes are incised and typically greater than two feet (2') deep. Hence, berms are provided for the detention basins to blend with the natural topography, such that the emergency overflow discharges into the existing wash flow path. During final design when roadway profile and pad elevations have been established, these basins will be reconfigured. The precipitation values for the 100-year, 2-hour storm event were obtained from the NOAA Atlas 2 Volume VIII maps. The equations and the assumptions used to calculate the required and provided stormwater storage volumes are discussed in Section 5.2.4.

Provided below is a summary of the stormwater storage required and provided for each individual drainage basin. Table 3.3, on the following page, indicates that the stormwater storage provided is in excess of the stormwater storage required for the entire site by 2.17 ac-ft. Please refer to Appendix E for detailed stormwater storage calculations.

3.3.1 Dissipation of Stormwater

18-inch bleedoff pipes with removable orifice plates are proposed for the dissipation of the stormwater detained within the stipulated time of 12 to 24 hours per the City of Scottsdale Drainage Ordinance; thus, no dry wells will be required. The orifice plates are preliminarily sized using the orifice equation. A 6-inch diameter orifice plate is proposed to dissipate the detained volume at a rate of not greater than 1.5 cfs. The calculation is included in Appendix F.

Table 3.3
Stormwater Storage

Drainage Boundary	Total Development Area (Acre)	100-yr, 2-hr Required Volume (acre-ft)	100-yr, 2-hr Provided Volume (acre-ft)	Volume Shortage / Excess $\pm$ (acre-ft)
A1	3.79	0.53	0.6	0.07
A2	7.17	1.01	1.01	0.00
B	10.94	1.54	1.78	0.24
C	3.65	0.51	0.57	0.06
D	7.73	1.09	1.25	0.16
E1	14.24	2.01	2.01	0.0
E2	13.23	1.87	2.02	0.15
E3	2.19	0.32	0.58	0.26
F1	10.24	1.44	1.99	0.55
F2	3.56	0.5	0.51	0.01
F3	2.93	0.41	0.40	0.0
G	3.7	0.52	0.51	0.0
HI	12.94	1.82	1.98	0.16
H2	8.5	1.2	1.31	0.11
I	4.44	0.63	0.64	0.01
J	4.81	0.68	0.66	0.02
K	2.06	0.29	0.38	0.09
L	1.49	0.21	0.20	0.0
M	1.44	0.2	0.2	0.0
N	2.81	0.4	0.49	0.09
O	0.55	0.08	0.22	0.14
P	1.73	0.24	0.37	0.13
Q	0.52	0.07	0.07	0.00
R	0.18	0.02	0.0	-0.02
Total	125	17.59	19.76	2.17

3.4 Pre- and Post- Runoff Characteristics

The U.S. Army Corps of Engineers' HEC-1 computer analysis program was used for hydrologic modeling including routing of flow through storage basins and combining hydrographs. The HEC-1 model was also used to compare the pre- and post-development runoff flows. Based on the site topographic conditions, Kinematic Wave routing technique was used for both the existing and proposed conditions hydrologic modeling. Runoff for each drainage sub-basin was computed and then routed, if required, through downstream drainage sub-basins where the hydrographs were then combined. The parameters were selected per the guidelines provided in the *City of Scottsdale DS&PM*. The parameters selected and the inputs for the HEC-1 models are discussed in Sections 5.1. The parameters changed to model developed conditions are also discussed in Section 5.1.1

Tables 3.4-1, 3.4-2, and 3.4-3 provide the comparative peak flows for the pre- vs. post-developed conditions for 2-year, 10-year, and 100-year, 6-hour events respectively at each concentration point. For the location of these concentration points and corresponding 100-year, 6-hour flow values, please refer to Plates 8 and 9 for existing conditions, and Plates 10 and 11 for developed conditions. The developed conditions HEC-1 model with on-line detention upstream of the culvert road crossings and other strategic locations reduced the post-development flows to below the pre-development flows at each concentration point exiting the site for the 2-year, 10-year, and 100-year, 6-hour storm events.

The HEC-1 input data and output files for the existing and developed conditions are included in Appendices B and C, respectively. The data analysis procedures are discussed in Section 5.0. Hydrographs comparing both pre-development and post-development 100-year, 6-hour event flows at each concentration point exiting the site are included in Appendix D. These hydrographs graphically illustrate that the post-development 100-year, 6-hour flows are at or below the pre-development flows. Further, they also illustrate the time to peak and the duration of the storm event.

Table 3.4-1
2-Year, 6-Hour Flow Summary

Concentration Point	Existing Flow (cfs)	Developed Flow (cfs)	Net Downstream Effect (cfs)
A1	46	39	-7
A2	21	15	-6
COMB B	37	28	-9
C	17	10	-7
D	17	15	-2
COMB E	49	26	-23
E3	6	0	-6
F1	19	13	-6
F2	9	1	-8
F3	8	6	-2
G	7	1	-6
H1	28	7	-21
H2	32	19	-13
I	14	2	-12
J	18	9	-9
K	9	0	-9
L	8	2	-6
M	6	1	-5
N	8	0	-8
O	3	3	0
P	6	6	0
Q	3	3	0
R	94	88	-6

Table 3.4-2
10-Year, 6-Hour Flow Summary

Concentration Point	Existing Flow (cfs)	Developed Flow (cfs)	Net Downstream Effect (cfs)
A1	111	109	-2
A2	52	48	-4
COMB B	93	85	-8
C	43	33	-10
D	43	37	-6
COMB E	126	68	-58
E3	14	4	-10
F1	47	37	-10
F2	21	11	-10
F3	19	18	-1
G	19	9	-10
H1	70	54	-16
H2	79	66	-13
I	35	16	-19
J	43	30	-13
K	19	4	-15
L	17	22	-6
M	14	7	-7
N	18	2	-16
O	6	5	-1
P	13	12	-1
Q	7	5	-2
R	220	209	-11

Table 3.4-3
100-Year, 6-Hour Flow Summary

Concentration Point	Existing Flow (cfs)	Developed Flow (cfs)	Net Downstream Effect (cfs)
A1	235	228	-7
A2	109	108	-1
COMB B	204	198	-6
C	89	80	-9
D	93	79	14
COMB E	275	246	-29
E3	28	16	-12
F1	99	88	-11
F2	43	35	-8
F3	39	38	-1
G	41	31	-10
H1	152	147	-5
H2	172	165	-7
I	73	50	-13
J	83	75	-8
K	35	20	-15
L	32	29	-3
M	28	19	-9
N	35	19	-16
O	11	9	-2
P	24	21	-3
Q	13	10	-3
R	444	411	-33

3.5 Proposed Drainage Structures

3.5.1 On-Site Detention

On-site detention basins are proposed at the upstream end of roadway culvert crossings and at other strategic locations along existing on-site washes to provide stormwater storage as discussed in Section 3.3. Please refer to Appendix E for the 100-year, 2-hour stormwater storage volume calculations for all detention basins.

Stormwater volumes for storm events greater than the 100-year, 2-hour event will overflow over the rock berm. For the 2-year, 10-year, and 100-year, 6-hour storm events, these basins attenuate the post-development flows to at or below the pre-development flows. Peak flows are routed through these basins for the 2-year, 10-year, and 100-year, 6-hour storm events. The peak flow volume during the smaller events (2-year, 6-hour) will not overflow the rock berm and will be drained through the bleed-off pipe. This attenuation is modeled using HEC-1.

Provided in Figure 3.5.1 is a graphical illustration of stormwater routing/storage through a detention basin. Weir walls are proposed at the outlet of the basins. Storage routing curves were developed for the detention basins specific to the outlet structure. Please refer to Appendix F – *Hydraulic Calculations* for weir wall storage routing curves used in the HEC-1 modeling.

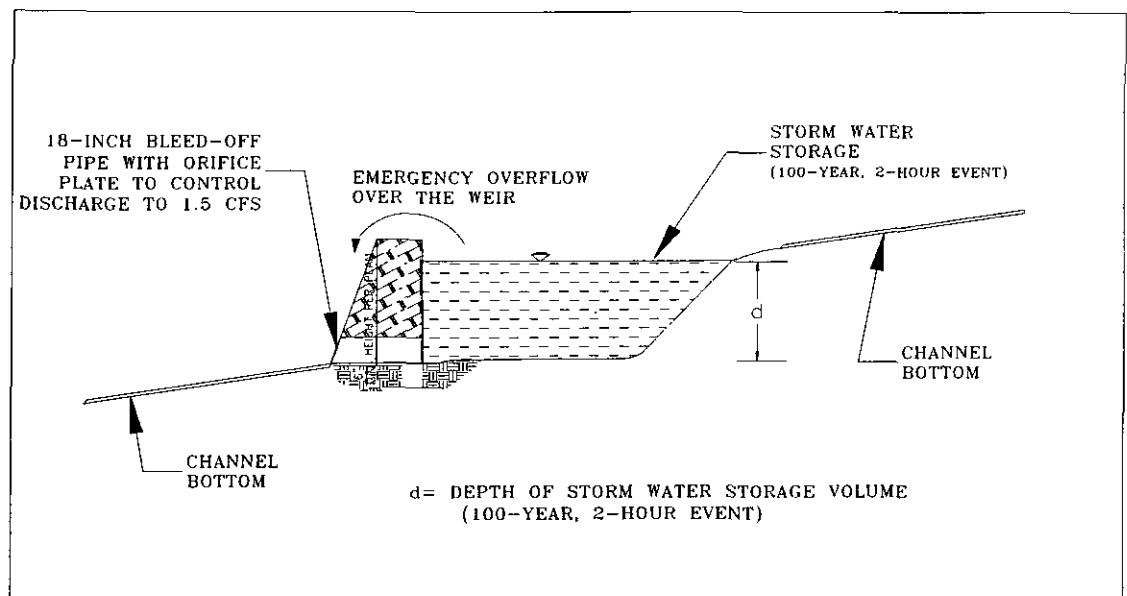


Figure 3.5.1 – Stormwater Routing through a Detention Basin

The bleed-off pipe is sized at 18 inches, based on the minimum pipe size requirements by the City of Scottsdale. As the flows cannot be metered by this bleed-off pipe, an orifice plate is proposed to meter the discharge flows at a rate not greater than 1.5 cfs. The bleed-off pipe with orifice plate drains the water into the natural watercourses downstream of the basin. The proposed orifice plates will be sized such that the detained water is drained at a rate not exceeding 1.5 cfs and the drain time is between 12 to 24 hours.

3.5.2 Roadway Crossing Structures

Culverts will be designed with the final drainage reports, and with the final design of the roadway profiles. The culverts will be designed to convey the 100-year flow downstream of the basin outlet structures, which is less than the existing 100-year flow. Culverts are proposed to be sized such that the headwater behind the culvert does not inundate the building envelopes. Also, the calculated headwater elevation at the culvert is used to define the ponding limits for the 100-year, 6-hour storm event. HY-8 culvert analysis program developed for the Federal Highway Administration will be used for the analysis of the culverts proposed with the improvement plans. The approximate 100-year backwater limits (ponding) will also be incorporated within the proposed floodplain delineations.

Rip rap will be utilized within the channels to dissipate velocities and as erosion protection on the upstream and downstream sides of the proposed culverts. The actual design and rip rap sizing will be based on approved practices and will be completed at the time of improvement plan submittal.

Street runoff conveyance and street capacities will be calculated with the final drainage reports prepared for the improvement plan submittals. Curb openings or storm drain systems will be designed to convey the excess 100-year flow so that street flow depths do not exceed eight inches (8") and the 10-year flow is contained within the curbs. The actual design will be based on approved practices and will be completed at the time of improvement plan submittal.

The drainage system for local residential roadways will be designed consistent with the *City of Scottsdale DS&PM*. The methodology developed by the Federal Highway Administration will be used to calculate, in spreadsheet format, the allowable street cross-section capacities.

3.6 Project Phasing

The project site is proposed to be developed in four phases. For the completion of this project, a drainage report will be prepared for each phase. The drainage structures proposed within each phase will be designed to meet the *City of Scottsdale DS&PM* guidelines. The impact of the phased construction and the required interim condition drainage systems proposed will be discussed in the drainage report for each phase. The development requirements will be met independently for each phase.

4.0 SPECIAL CONDITIONS

4.1 Section 404 Washes

All washes within the project were investigated to identify those that may be deemed jurisdictional. The United States Army Corp of Engineers approved Section 404 Jurisdictional Delineation is presented on Plate 7 – *Section 404 Washes*. This plate provides the approved Section 404 wash locations within the project site. The Section 404 washes are also illustrated on the pre- and post-development grading and drainage plans.

5.0 DATA ANALYSIS METHODS

5.1 Hydrologic Procedures

The U.S. Army Corps of Engineers' HEC-1 hydrologic model is used to compute the 2-year, 10-year and 100-year events, and 6-hour storm events runoff as outlined in Chapter 4 of the *City of Scottsdale DS&PM*. Further, Rational Method is used to compute the 100-year, 2-hour storm event storage volume required. Below is a description of the hydrologic procedures specific to this project.

5.1.1 HEC-1 Procedures

The U.S. Army Corps of Engineers' HEC-1 hydrologic model is used for computing hydrographs, routing the flow through sub-basins, and combining hydrographs. Based on the site topographic conditions the Kinematic Wave routing technique was used for both the existing and proposed conditions hydrologic modeling. The main components for runoff computation are overland flow, collector channel flow and main channel flow. Input data necessary for the computer analysis included definition and measurement of the overland flow characteristics, drainage sub-basins, assignment of soil curve numbers and calculations of the proposed impervious factors. Provided below is a more detailed explanation of the parameter selection and the data inputs for HEC-1 modeling.

5.1.1.1 Rainfall Event

The precipitation amount for the 2-year, 10-year and 100-year events, 6-hour duration period was obtained from NOAA Atlas 2 Volume VIII maps located in the *City of Scottsdale DS&PM*. An illustration of the location of the site on these maps is provided in Appendix A. These values were further analyzed using the Maricopa County Drainage Design Management Systems for Windows (DDMSW) program to determine the precipitation depths for intervals lower than 1 hour, which are input values in the HEC-1 model. The DDMSW report of this analysis is included in Appendix A. The total calculated rainfall depths for the 2-year, 10-year, and 100-year, 6-hour storm events are 1.57, 2.29, and 3.37 inches respectively. The drainage sub-basins have also been depicted on the NOAA maps. Based on the size of the drainage area, a

single point rainfall amount without aerial reduction was justified for each storm event analyzed.

5.1.1.2 Soil Curve Numbers

Soil curve numbers are used to account for interception/infiltration losses. Soil curve numbers for the HEC-1 models were calculated based on published guidelines and engineering experience for the type of soils present within the drainage sub-basins. The cover type (less than 30% vegetation cover due to wild fire) and hydrologic condition were estimated as desert shrub areas with poor hydrologic conditions. Plate 3 presents the soils classification (type D) based on the SCS Map for the project area. A runoff curve number of 88 was used for the corresponding soils group D that occurs within the watershed area. Figure 4-6 "Runoff Curve Numbers for Arid and Semi-Arid Rangelands" located in the *City of Scottsdale DS&PM* was used to determine the soil curve number and has been included in Appendix A. Wood/Patel is in agreement with the City's published curve numbers.

5.1.1.3 Impervious Area

Impervious percentages were determined for each sub-basin for the developed conditions HEC-1 model. Currently, the site contains no development and hence a effective impervious area of zero percent was used. The rock outcroppings on the site are not considered a factor in computing the percent impervious as they do not have a pattern that may impact the drainage characteristics and are not hydraulically connected to the basin outlets. Please refer to photographs of the site and washes in Appendix A. For the on-site developed conditions, impervious percentages were calculated utilizing Figure 2.2-16 "Percent of Impervious Area vs. Dwelling Units/Acre", in Chapter 2 of the old *City of Scottsdale DS&PM*. Please refer to Appendix A for a copy of this figure. Based on the lot sizes (Average lot size = 101,500 sq.ft), an impervious percentage of 10 percent was used for the proposed developed areas.

5.1.1.4 Overland Flow Resistance Factor

The resistance factor for overland flow was estimated with guidance from descriptive information and site photographs. Based on the site conditions illustrated by the photographs in Appendix A, the site is considered to most closely resemble the sparse rangeland with Debris-20% cover, as well as the sparse vegetation categories identified in Table 3.5 of the HEC-1 manual. Based on the site photos and field observations, vegetative cover is estimated to be greater than 20%. Thus, a mid-range value from the sparse rangeland with Debris-20% cover category was deemed appropriate. Since a mid-range value from this category corresponded with the upper value of the sparse vegetation category, a value of 0.13 was chosen to best fit both categories with actual site conditions. From Table 3.5 in the HEC-1 manual, a resistance factor for overland flow of 0.13 was chosen. A copy of this table is included in Appendix A.

5.1.1.5 HEC-1 Input Records

This section provides additional description for the input Records used in the HEC-1 modeling for this project. The computation time interval for hydrograph generation used was 2 minutes which is within the range of 1 to 5 minutes specified in the *City of Scottsdale DS & PM*.

BA Record

This record is a direct input for computing runoff. The element of this record is the drainage area in square miles estimated by measuring the area of the drainage boundary.

LS Record

This record is a loss rate data record and computes rainfall losses based on the geological characteristics of the site. The elements on this record are the soil curve numbers and the percent impervious. Both of these data are estimated based on the soil characteristics and site conditions. The derivation of these parameters is discussed in detail in Section 5.1.1.

UK Record

This record defines the characteristics of overland flow. The elements on this record are the overland flow length, slope, resistance factor, and percent of sub-basin area represented. All of the input values are measurements and calculated values except for the resistance factor. The resistance factor for overland flow was estimated based on Table 3.5 in the HEC-1 manual. Based on the site conditions illustrated by the photographs in Appendix A, a resistance factor value of 0.13 was chosen. Further discussion on overland flow resistance factor was provided in Section 5.1.1.4.

RK Record

This record defines the characteristics of the main channel. The elements on this record are the channel length, channel slope, channel roughness, and geometry of the channel. All of the values were measured and calculated values. The channel roughness ('n' value) was chosen based on the site conditions as discussed in Section 5.2. Per the HEC-1 manual, the 'n' value that best represents the channel was chosen. The 'n' value for the channel bottom, 0.032, was chosen to best represent the channel characteristics. Also, this value is conservative as it results in a higher estimate of peak flows. The HEC-1 model was calibrated to the HEC RAS hydraulic model as accurately as possible. However, the HEC-1 model is simplified by using average dimensions of the channel to account for the irregularity in the channel dimensions.

SE Record

This record is used for defining the parameters to facilitate routing of flows through a storage basin. The elements on this record are the elevations corresponding to values in the same field in the SV and SQ Records. These values are obtained from the detention basin elevations and/or head water elevations where storage routing is proposed. Please refer to Appendices F and G for the rating calculations and detention basin calculations respectively. Please note that the elevation values are extrapolated for modeling purposes to determine the headwater elevation over the weir and the limits of inundation.

SV Record

This record is used for defining the storage volumes in the storage routing of flows. These values are obtained from the storage volumes at different elevations of the ponding at the control structures provided as defined by the SE records. These values are calculated and are provided in Appendix G. Please note that the storage volumes are extrapolated to represent the proposed detention volumes.

SQ Record

This record is used to define outflow discharge data for storage routing. Please refer to Appendix F for the rating curves for storage intervals. The elements in this field indicate the discharge at different stages for the proposed structures. Appendix F includes calculations for weir flows.

5.1.2 Parameter Changes for Developed Conditions

The parameter changes depicting the developed conditions are based on the guidelines provided in the *City of Scottsdale DS & PM*. The percent impervious used in the HEC-1 modeling was increased to depict the developed conditions. For the on-site developed conditions, effective impervious percentages were calculated utilizing Figure 2.2-16 "Percent of Impervious Area vs. Dwelling Units/Acre", in Chapter 2 of the old *City of Scottsdale DS&PM*. Please refer to Appendix A for a copy of this figure.

The proposed development includes 122 lots and a clubhouse, which is considered to be an additional lot. The dwelling density is calculated as 0.373 du/acre, for a total of 123 lots and 330 acres. Based on the lot sizes (average lot size = 101,500 sq.ft), and dwelling density of 0.37 du/acre an effective impervious percentage of 10 percent was used for the proposed developed areas in the HEC-1 model. An effective impervious area of 10 percent was used for the developed areas within the project site, while 0 percent was used for the current and proposed undeveloped areas. A weighted percent impervious value was used in HEC-1 model for each drainage basin based on the proposed developed and undeveloped areas.

5.1.3 Rational Method

The Rational Method was used to compute peak discharges for watersheds less than or equal to 160 acres as a comparison with HEC-1 results. Parameters necessary for this procedure are measurement of drainage sub-basin areas, runoff coefficient ("C" values) and calculation of rainfall intensity. Runoff values are calculated using the following 100-year "C" values:

- 0.56 for undeveloped desert land with slopes less than 10% - Hydrologic Soil Group D
- 0.59 for single family residential area R1-130 – Hydrologic Soil Group D

These values are based on Figure 4-5 "Runoff Coefficients for Use with Rational Method" in the *City of Scottsdale DS&PM*. This figure is included in Appendix A. The Maricopa County Flood Control District's Drainage Design Management System (DDMS) software was used to determine the peak discharges by Rational Method.

5.1.4 Stormwater Storage

Based on the City of Scottsdale's Drainage Ordinance stormwater storage requirements, on-site stormwater storage is proposed to be provided for 100-year, 2-hour storm events. The Rational Method is used to estimate the 100-year, 2-hour stormwater storage volumes. The precipitation values for the 100-year, 2-hour storm event were obtained from the NOAA Atlas 2 Volume VIII maps. The equations used to calculate the required and provided stormwater storage volumes are presented below.

5.1.4.1 Required Stormwater Storage Volume

Stormwater storage volume required for the 100-year, 2-hour event is:

$$V_{REQUIRED} = C * \left(\frac{P}{12} \right) * A$$

Where:

- V is the required stormwater storage volume in acre-feet
- C is the weighted "C" coefficient, 0.59 for the proposed developed areas.
- P is the precipitation in inches for the 100-year, 2-hour rainfall, 2.82 inches for the proposed developed areas.
- A is the drainage area in acres

5.1.4.2 Provided Stormwater Storage Volume

Stormwater storage volume provided for the 100-year, 2-hour event is:

$$V_{PROVIDED} = \left[\left(\frac{A_1 + A_2}{2} \right) * (ELEV_2 - ELEV_1) \right]$$

Where:

- V is the provided stormwater storage volume in acre-feet
- A is the contour area in acres
- ELEV is the contour elevation

Section 3.3 of this report describes the stormwater storage provided through proposed detention basins. Assumptions that were made when determining provided volume were maximum side slopes of 4:1. Please refer to Appendix G for 100-year, 2 hour stormwater storage volume calculations.

5.2 Hydraulic Procedures

5.2.1 Hydraulic Analysis of Open Channels

The U.S. Army Corps of Engineers' HEC-RAS computer program was used for the hydraulic analysis of the washes greater than 50 cfs. HEC-RAS was used to determine the 100-year water surface elevations. Cross-sections were placed such that significant variations in the channel cross-sectional geometry are adequately represented. The Manning's 'n' value was estimated using Cowan Method based on the following parameters:

- Channel material
- Degree of irregularity
- Effects of obstruction
- Vegetation
- Variations in channel cross section
- Degree of meandering

The investigation of site photos presented in Appendix A and aerial photos provided on Plate 5, indicated gradual variations in channel cross-sections within the site and a minor degree of meandering. The vegetation on the site was minimal due to the wild fire that occurred recently and is typical of the local desert climate. The channel material was coarse gravel and the degree of irregularity was minor. Based on the above, a 'n' value of 0.032 and 0.045 were

chosen for channel bottom and channel over banks respectively. Site Photos have been included representative of on-site washes that support the "n" value determination. The site photos are included in Appendix A.

Cross-sections were placed such that significant variations in the channel cross-sectional geometry are adequately represented. Due to the steep slopes on the project site, it was not feasible to put enough cross-sections such that the difference in energy grade elevations is less than one foot (1') between cross-sections. The HEC-RAS computer program gives an error message for a difference in energy grade elevation of greater than one foot (1') between cross-sections. Hence, a wash was analyzed at an elevation difference of one foot (1') to demonstrate the impact of additional cross-sections on the HEC-RAS analysis. It was found that increasing the number of cross sections did not impact the flow depths as long as adequate cross-sections representative of the geometry of the channel are included.

5.2.2 Hydraulic Analysis of Culverts

For analysis of culvert roadway crossings the computer program HY-8 will be used during final design. HY-8 is a culvert analysis program that was developed for the Federal Highway Administration. Site characteristics and flow are entered into the program and the resulting flow regime and headwater values are calculated.

6.0 DISCUSSION ON MODELING RESULTS

6.1 Rational Method vs. HEC-1 Flows

100-year, 6-hour peak flows were calculated using both the Rational Method and HEC-1 for the off-site basins for comparison. The rational method results are included in Appendix E - *100-year Rational Method*. Provided below in Table 6.2 is a comparison of the off-site flows using the Rational Method and HEC-1 results for the 100-year 6-hour storm event.

Table 6.2:
Rational Method vs. HEC-1 Flows

Concentration Point	Rational Method Flow (cfs)	HEC-1 Flow (cfs)	Difference Rational – Hec1 (cfs)
A1	238	225	13
A2	112	103	9
B	210	192	18
H2	193	161	32
I	76	70	6
J	86	81	5
R	409	429	-20

The table indicates that for the 100-year 6-hour storm event the flow values determined by Rational Method were similar to flow values determined using the HEC-1 analysis. The flow values determined by Rational Method were higher for smaller basins and the flow values calculated by HEC-1 were higher for larger basins. A HEC-RAS hydraulic analysis was performed to determine the impact of this variation in flow values using the different hydrological methods. The 100-year water surface elevations were determined using these flow values and the results are discussed in the subsequent section.

6.2 100-Year Floodplain Delineation

The U.S. Army Corps of Engineers' HEC-RAS Version 3.1.2 was used to generate the water surface profiles for washes with 100-year flows in excess of 50 cfs. The 100-year floodplain delineation for these washes has been revised to reflect the limits of ponding that occurs because of the proposed detention basins. At this point, the final location of

the roadways has not been finalized and therefore a post-development HEC-RAS model that includes cross sections representative of the culverts has not been prepared. It is anticipated that the 100-year floodplain delineation for post-development conditions will be approximately the same as the pre-development delineation with the exception of some ponding at culvert crossings. Water surface elevations and top widths for each cross section are included on Plate 11 – *Post-Development Grading and Drainage Plan*.

The 100-year water surface elevations were computed using the peak flow values from HEC-1 and Rational Method results. Provided below is a summary of the HEC-1 and Rational Method flows and the corresponding water surface elevations.

Table 6.3:
Variation in WSE for Flows from Rational Method and HEC-1

Concentration Point	100-year flow		Water Surface Elevations		Difference in WSE
	Rational Method Flow (cfs)	HEC-1 Flow (cfs)	Rational Method (ft)	HEC-1 (ft)	Rational Method – HEC-1 (ft)
A1	238	235	2754.31	2754.3	0.03
A2	112	109	2734.62	2734.6	0.04
B	210	204	2747.05	2747.0	0.07
H2	193	172	2731.65	2731.60	0.04
I	76	73	2755.89	2755.89	0.03

The table indicates that the water surface elevations calculated using flows estimated by Rational Method and HEC-1 change by only up to 0.05 feet. Appendix F provides the detailed HEC-RAS results for all the washes.

7.0 DRAINAGE PLAN REQUIREMENTS

7.1 Drainage System Requirements

The existing drainage patterns will be maintained in their natural location and condition where possible. The site is being developed as large custom lots with an average lot size of 101,500 sq.ft. As lots are developed, individual lot engineering will be required to provide drainage documentation to substantiate the development of the lot. In the event of any future development within the off-site watershed areas, it will be the responsibility of the future developer to attenuate the post-development flows and volume to at or below the pre-development values.

7.2 Easement Requirements

Where flows from the 100-year storm event are greater than 50 cfs, drainage easements will be provided. In addition, drainage easements shall be dedicated to the limits of inundation for the 100-year, 2-hour storm event in each detention basin.

7.3 Roadway Crossing Requirements

In all cases the depth of flow over streets will be in accordance with City of Scottsdale Flood Plain and Drainage Ordinance.

7.4 Maintenance

Ongoing maintenance of the designed or recommended drainage systems is required to preserve the design integrity and purpose of the drainage system. Failure to provide maintenance can prevent the drainage system from performing to its intended design purpose and can result in reduced performance. It is the responsibility of private developers, homeowner associations, etc. to maintain facilities on private property, within all drainage easements, private streets, and right-of-ways unless accepted by the City for maintenance. A regular maintenance program is required so that drainage systems perform to the level of protection or service as presented in this report and the project's plans and specifications.

7.5 Scour Protection

The need for scour protection will be identified as part of the final drainage reports for each phase of this project. Because of the relatively small peak flow values and incised nature of the on-site washes, development setbacks will most likely be utilized rather than bank protection schemes if required.

7.6 Annual Sediment Yield

Sediment yield is defined as that portion of eroded material that travels through a drainage system to a downstream measuring or control point (*Bureau of Reclamation 1987*). The units used for this study are acre-feet/square mile/year (ac-ft/sq.mi./yr.). The prediction of sediment yields is not an exact science. Many complex variables influence the erosion process and any sediment yields analysis requires considerable engineering judgment. For this reason, two (2) methods were used to arrive at a conservative estimate of the average annual sediment yield rate. The following sections provided a detailed discussion of each method and its results.

Sediment yield was calculated using the Pacific Southwest Inter-Agency Committee (PSIAC) method. This method was obtained from the following source: Pacific Southwest Inter-Agency Committee, *Report of the Water Management Subcommittee on Factors Affecting Sediment Yield in the Pacific Southwest Area and Selection and Evaluation of Measures for Reduction of Erosion and Sediment Yield*, October, 1968.

Nine (9) factors are evaluated in determining the sediment yield classification. These are geology, soils, climate, runoff, topography, ground cover, land use, upland erosion, and channel erosion and sediment transport. Characteristics of each of the nine factors, which give that factor high-, moderate-, or low-sediment yield, are evaluated and the factor is assigned a numerical value representing its significance in the yield rating. The yield rating is the sum of values for the appropriate characteristics for each of the nine factors. For the purpose of avoiding complexity, the factors are generally described as independently influencing the amount of sediment yield. Table 7.6 provides the sediment yield classification based on sediment yield rating.

Table 7.6
Sediment Yield Classification Table

Classification	Rating	Sediment Yield acre-ft/sq. mile
1	> 100	3.0
2	75 – 100	1.0 – 3.0
3	50 – 75	0.5 – 1.0
4	25 – 50	0.2 – 0.5
5	0 – 25	< 0.2

Application of the nine sediment yield factors to the watershed discussed in this report generated a total numerical rating of 77 to 85, the actual rating being dependant upon the interpretation of the guidelines that are used to assign rating values to each factor. Regardless of the actual number, i.e. 77 or 85, both values fall within Classification 2 (75 – 100), which indicates a sediment yield range of 1.0 to 3.0 ac-ft/sq.mi./yr.

7.6.1 Bureau of Reclamation Sediment Surveys

The 1987 edition of *Design of Small Dams* (BuRec) publishes data on sediment measurements from 28 reservoirs in semi-arid regions of the United States. A regression line through these data points produces the following equation:

$$Q_s = 1.84 A^{0.24}$$

Where:

Q_s = annual sediment yield (ac-ft/sq.mi./yr.)

A = drainage area (sq.mi.)

Using a drainage area size of 1.95 sq.mi., this equation predicts an annual sediment yield of 1.57 ac-ft/sq.mi./yr. or 4,920 cubic yards.

7.6.2 Recommended Annual Sediment Yield

In consideration of the complex processes under investigation, the two (2) methods used exhibit fairly good agreement. Based on comparison of sediment yield calculations to actual sediment yield measurements, Renard & Stone (1981) concluded that "the PSIAC method appears to give the best results for the amount of work required to make the estimate." Based on the results of the sediment yield equations, a design value of 1.6 ac-ft/sq.mi./yr. was selected.

7.6.3 Short-/Long-Term Aggradation, Degradation, Local Scour, and Deposition

Sediment transport analyses need to distinguish between short-term and long-term changes. Short-term changes are event specific and occur to some extent during each flood hydrograph. Examples of short-term changes would be local scour, general scour, bend scour, bedform troughs, and to some extent low-flow channel incisement. With the exception of low-flow channel incisement, any visual signs of these processes may be difficult to detect after the flow has subsided. Short-term scour processes can usually be quantified with empirical and/or theoretical relationships.

Vertical incisement of the channel bed can occur in response to the following six (6) processes:

1. Long-term degradation,
2. Local scour,
3. General scour,
4. Bend scour,
5. Low-flow channel incisement, and
6. Antidune troughs.

Precise prediction of long-term channel impacts can be much more elusive than their short-term counterparts because of the time span involved and the numerous variables that impact long-term changes. The major drainage corridors within the project site will be designed as protected channels.

7.7 Protective Devices

Roadway crossing culverts will incorporate rip rap at both the inlet and the outlet to dissipate energy and provide flow line scour protection. Detention basins upstream of the roadways will incorporate a protected overflow area (using native rip rap and filter fabric) in the event of overtopping. The bleed-off pipes will also incorporate rip rap protection.

8.0 CONCLUSIONS

8.1 Overall Project

1. The project site is located within FEMA Zone "X" (shaded), and Zone "D" as shown on Plate 2.
2. The major watercourses traversing through the project will be maintained in their natural location and condition, and will not be re-aligned wherever possible.
3. Drainage corridors have been designated for the identified washes in accordance with the appropriate City of Scottsdale ordinance requirements.
4. It is being proposed that online detention on the upstream side of the road culvert crossings and other strategic locations be provided to reduce post-development flows to at or below pre-development levels, and for 100-year, 2-hour stormwater storage. Some of the detention basins are sized at a depth greater than three feet (3'), due to ESLO and NAOS constraints.
5. The post-development peak flows for the 2-year, 10-year and 100-year, 6-hour storm events are reduced to at or below the pre-development flows.
6. The stormwater detained will be drained within 24 hours through bleed-off pipes with orifice plates.
7. The design of hydraulic structures is to be based on generally accepted engineering practices and in accordance with City of Scottsdale requirements.
8. On-going maintenance is required for all drainage systems in order to assure design performance.
9. All finished floor elevations are to be designed to be above the 100-year water surface elevation.

8.2 Project Phasing

The project site is proposed to be developed in four phases. For the completion of the site, a final drainage report will be prepared for each phase of the project. The development requirements will be met independently for each phase.

9.0 REFERENCES

1. City of Scottsdale, *Design Standards and Policies Manual Chapter 2 Drainage*, December, 1999.
2. City of Scottsdale, *Design Standards and Policies Manual Chapter 4 Drainage*, June, 2005.
3. Flood Control District of Maricopa County, *Drainage Design Manual for Maricopa County, Arizona: Volume I – Hydrology*, revised January 1995.
4. Flood Control District of Maricopa County, *Drainage Design Manual for Maricopa County, Arizona: Volume II – Hydraulics*, January 28, 1996.
5. U.S. Army Corps of Engineers, *HEC-1, Flood Hydrograph Package*, June 1998.
6. U.S. Army Corps of Engineers, *HEC-RAS, Version 3.1.2*, April 2004.
7. Arizona Department of Water Resources State Standards for Floodplain Management.
8. HEC-1 Manual, Flood Hydrograph Package User's Manual. US Army Corps of Engineers Hydrologic Engineering Center.
9. US Army Corps of Engineers Hydrologic Engineering Center. Introduction and Application of Kinematic Wave Routing Techniques using HEC-1.
10. Flood Control District of Maricopa County, Drainage Design Management System for Windows.
11. Flood Control District of Maricopa County, Drainage Design Management System.
12. Pacific Southwest Inter-Agency Committee, *Report of the Water Management Subcommittee on Factors Affecting Sediment Yield in the Pacific Southwest Area and Selection and Evaluation of Measures for Reduction of Erosion and Sediment Yield*, October 1968.
13. Design of Small Dams, 1987 Edition.

APPENDIX A

Supporting Information for Parameter Selection

- Site Photo Vicinity Map
- Site Photos
- Soil Survey Data
- FCDMC Rainfall Data
- Precipitation Maps
- Runoff Curve Numbers and Percent Impervious Area Chart
- Resistance Factor for Overland Flow
- Runoff Coefficients for use with Rational Method

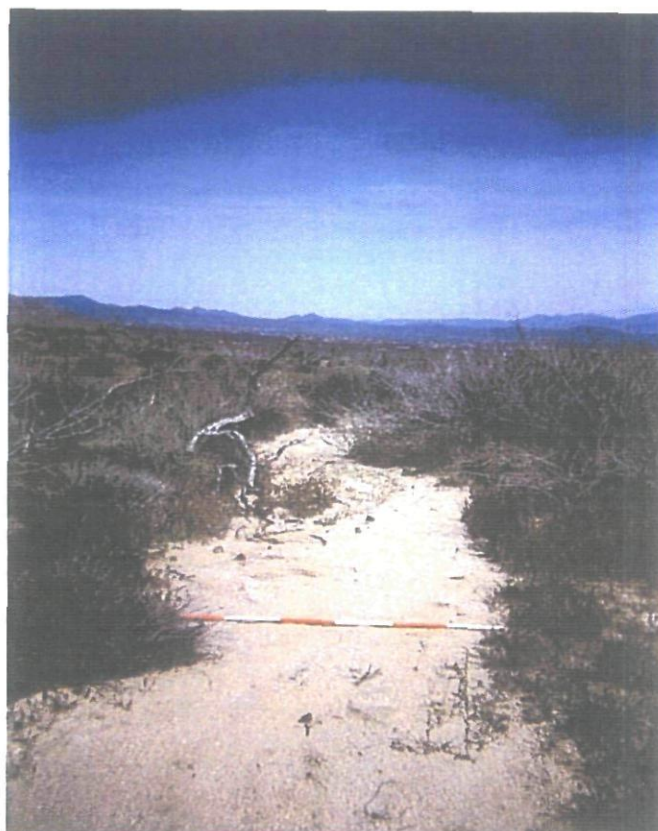


Photo 1



Photo 2

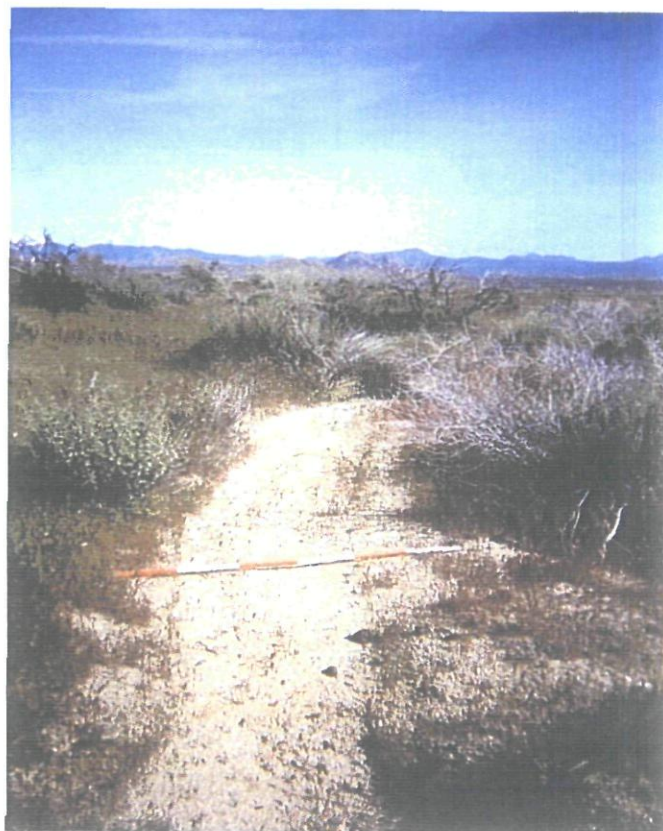


Photo 3

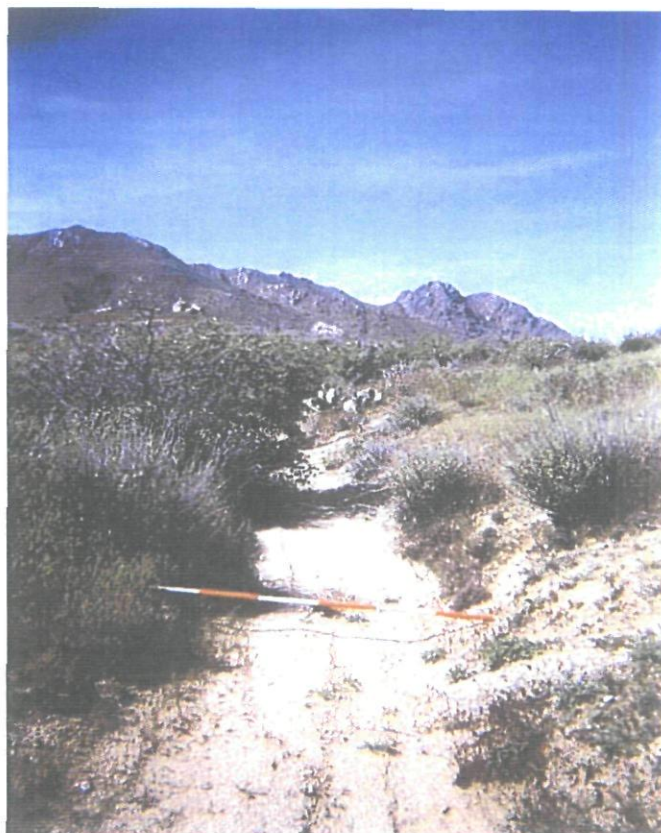


Photo 4



Photo 5



Photo 6



Photo 7

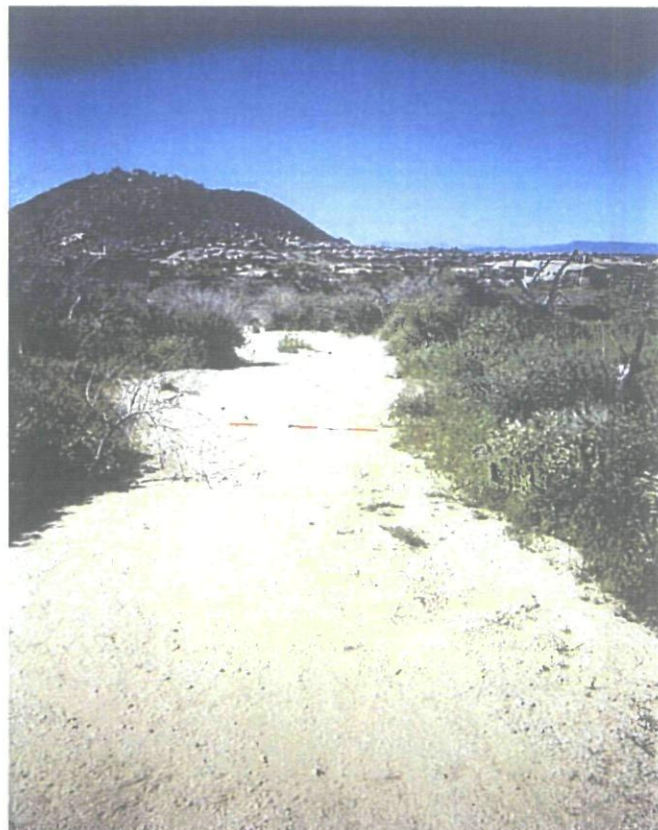


Photo 8



Photo 9



Photo 10

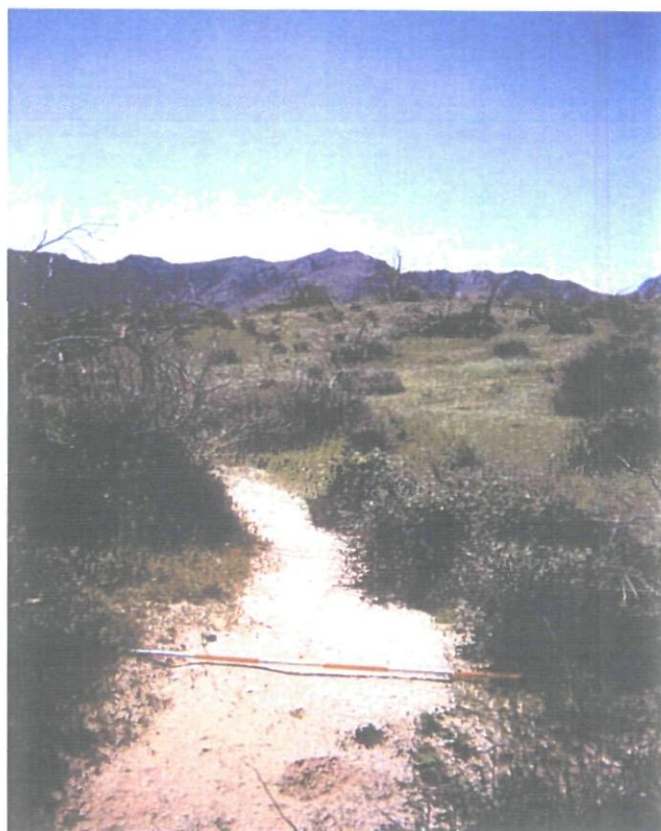


Photo 11



Photo 12



Photo 13



Photo 14

TABLE 15.--SOIL AND WATER FEATURES--Continued

Soil name and map symbol	Hydro-logic group	Flooding			Bedrock		Cemented pan		Risk of corrosion	
		Frequency	Duration	Months	Depth	Hardness	Depth	Hardness	Uncoated steel	Concrete
					In		In			
859*: Dunbar	B	None	---	---	>60	---	---	---	High	Low.
0*: Glenbar Noncalcareous	B	Rare	---	---	>60	---	---	---	High	Low.
Glenbar Calcareous	B	Rare	---	---	>60	---	---	---	High	Low.
162*: Glen	D	None	---	---	20-40	Hard	---	---	High	Low.
Wickenburg	D	None	---	---	3-20	Soft	---	---	High	Low.
364*: Glen	D	None	---	---	20-40	Hard	---	---	High	Low.
Wickenburg	D	None	---	---	3-20	Soft	---	---	High	Low.
Rock outcrop.										
5*: Greyeagle	D	None	---	---	>60	---	4-20	Thick	High	Low.
Continental	C	None	---	---	>60	---	---	---	High	Low.
Nickel	B	None	---	---	>60	---	---	---	High	Low.
6*: Greyeagle	D	None	---	---	>60	---	4-20	Thick	High	Low.
Uncity Variant	D	None	---	---	>60	---	5-20	Thick	High	Low.
7*: Forest	D	Rare	---	---	>60	---	---	---	High	Low.
369*: Dunsight	B	None	---	---	>60	---	---	---	High	Low.
Mariano	D	None	---	---	>60	---	5-20	Thick	High	Low.
071*: Dunsight	B	None	---	---	>60	---	---	---	High	Low.
Alito	B	None	---	---	>60	---	---	---	High	Low.
273*: Lehmans	D	None	---	---	6-20	Hard	---	---	High	Low.
Rock outcrop.										
4*: Lake	C	None	---	---	>60	---	20-40	Thick	High	Low.
Mariano	D	None	---	---	>60	---	4-20	Thick	High	Low.
76, 77, 78, 9 Mhall	B	None	---	---	>60	---	---	---	High	Low.
81*: Mhall	B	None	---	---	>60	---	---	---	High	Low.

See footnote at end of table.

Flood Control District of Maricopa County

Rainfall Data

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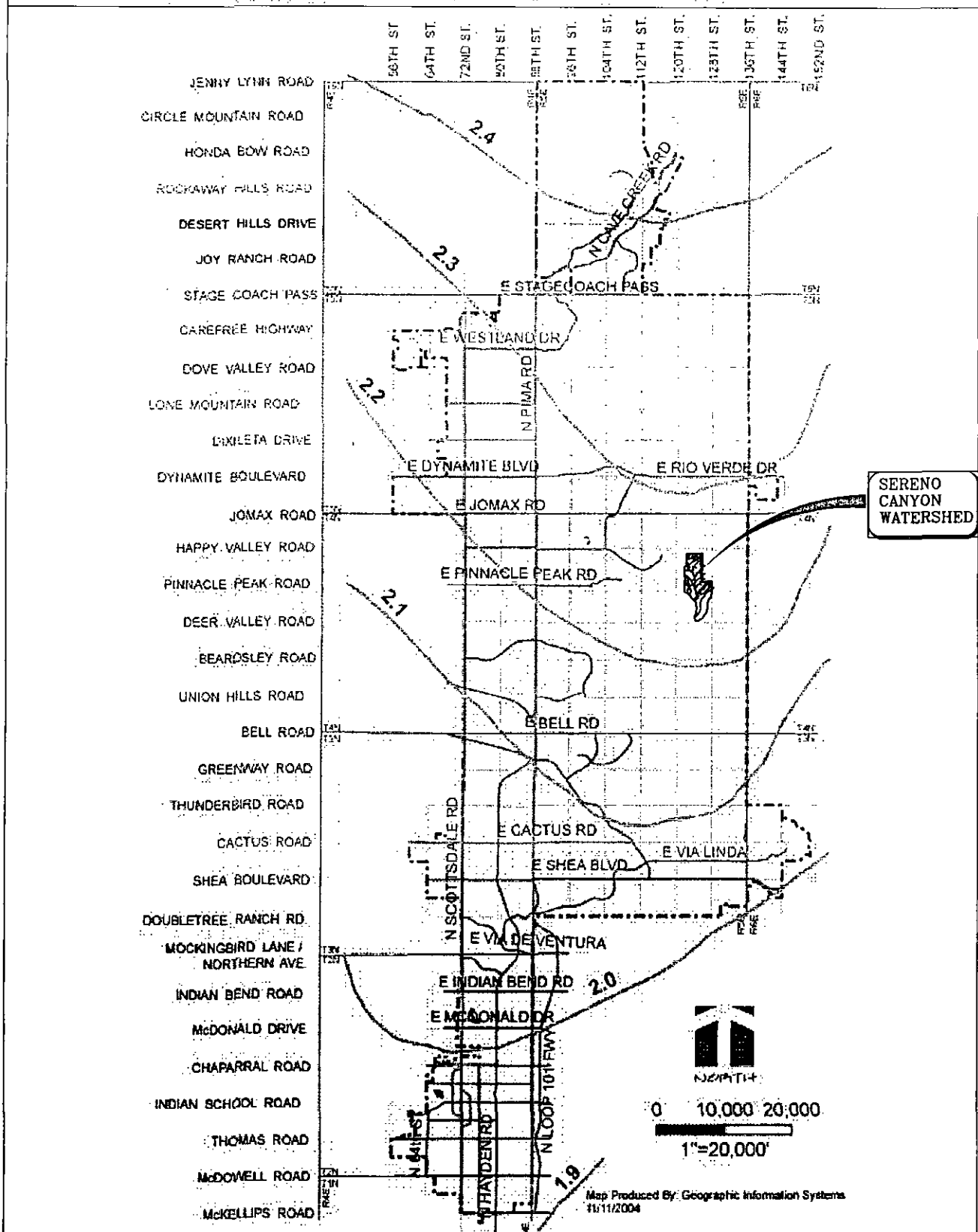
Primary Zone Number: 7 Latitude: 0.0 Elevation: 0
Short Duration Zone Number: 8 Longitude: 0.0

Duration	Point Values (in)					
	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
5 MIN	0.38	0.46	0.52	0.60	0.66	0.73
10 MIN	0.57	0.70	0.78	0.91	1.01	1.11
15 MIN	0.70	0.87	0.99	1.16	1.29	1.43
30 MIN	0.92	1.16	1.33	1.56	1.75	1.94
1 HOUR	1.12	1.43	1.64	1.95	2.19	2.42
2 HOUR	1.27	1.62	1.87	2.21	2.48	2.74
3 HOUR	1.38	1.75	2.01	2.38	2.67	2.96
6 HOUR	1.57	2.00	2.29	2.71	3.04	3.37
12 HOUR	1.81	2.31	2.66	3.15	3.53	3.91
24 HOUR	2.05	2.62	3.02	3.58	4.02	4.45

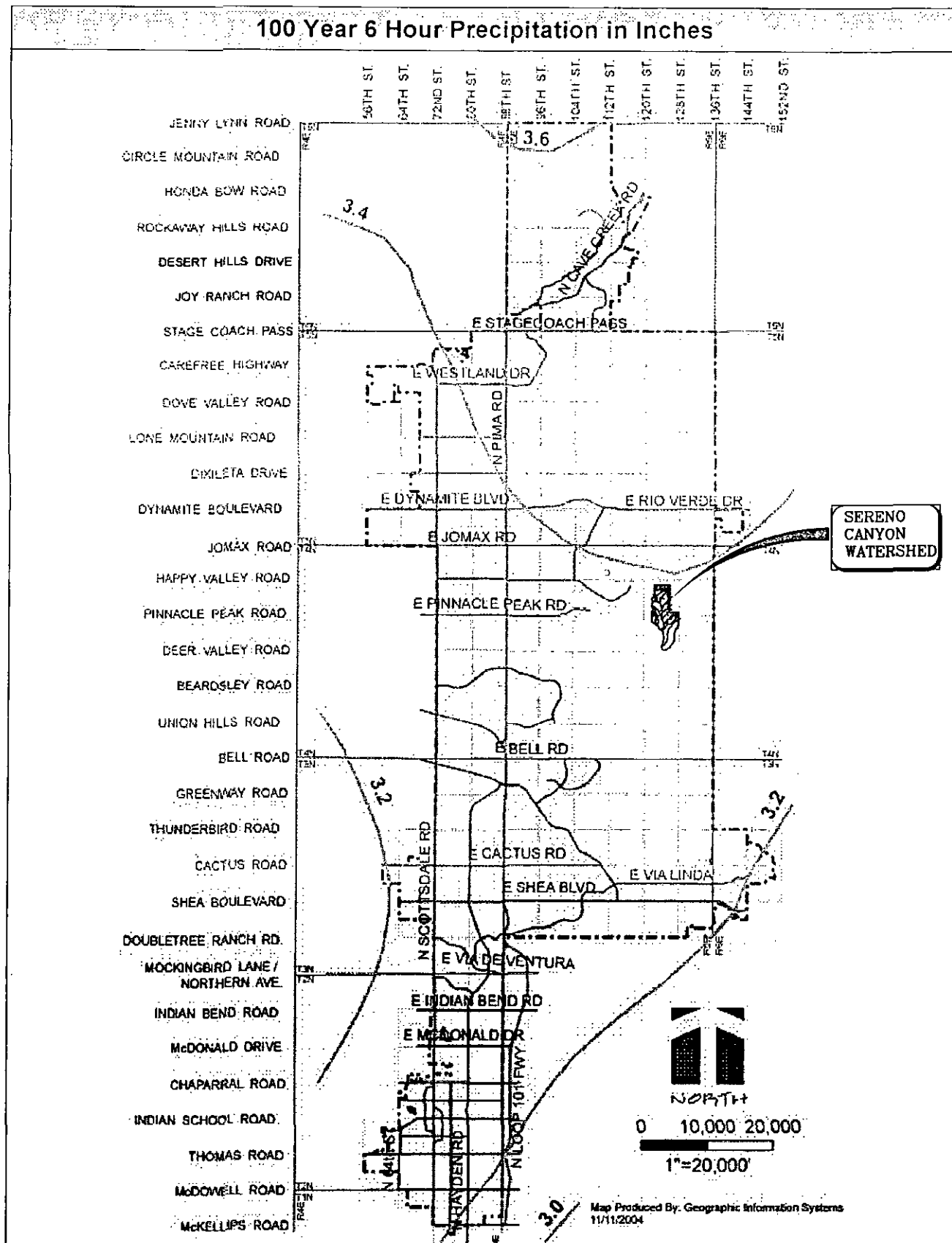


ISOPLUVIALS

10 Year 6 Hour Precipitation in Inches



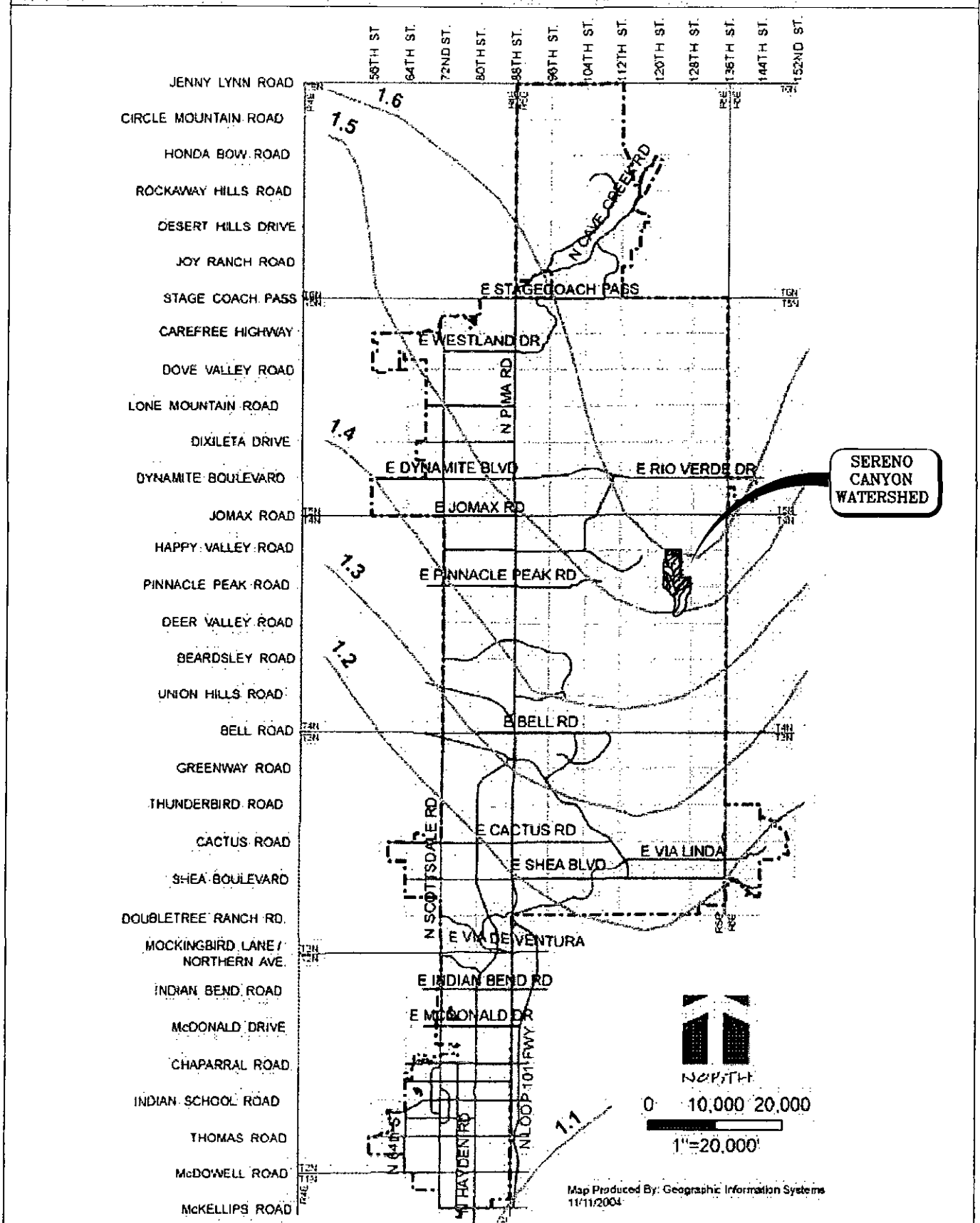
ISOPLUVIALS





ISOPLUVIALS

2 Year 6 Hour Precipitation in Inches



2. Infiltration

Infiltration or soil losses will be determined using Green and Ampt (G&A) procedures per FCDMC Hydrology Manual. Use the most recent published SCS soil survey maps of the area to determine the hydrologic soil group or surface soil texture for the G&A procedures. Use USDA Natural Resources Conservation Services (previously the Soil Conservation Services) maps, Soil Survey of Aguila-Carefree Area, Parts of Maricopa and Pinal Counties, or the Soil Survey of Eastern Maricopa and Northern Pinal Counties, Arizona, depending on what part of the city you are located.

3. Runoff Curve Numbers

Where detailed mapping is not available from the SCS or other sources, the use of curve numbers is acceptable. When using runoff curve numbers (ROCN) within Scottsdale:

1. Assume poor hydrologic condition and desert shrub cover type for natural undisturbed desert conditions in Figure 4-6.
2. For lawns, golf courses, and other grassed open space areas, assume good condition in Figure 4-7 to determine the ROCN, then adjust the ROCN to antecedent moisture condition III (use Figure 4-8).
3. For developed conditions, increase the percent impervious on the LS card without changing the ROCN (except in the case of grassed areas, in which the curve number should be adjusted up according to the above). Use the actual or estimated percent impervious; a minimum of 85 percent for commercial and 72 percent industrial.

RUNOFF CURVE NUMBERS FOR ARID & SEMIARID RANGELANDS¹

Cover Type and Hydrologic Condition	Hydrologic Condition ²	Curve Numbers for Hydrologic Soil Group			
		A ³	B	C	D
Herbaceous: mixture of grass, weeds, and low-growing brush, with brush the minor element	Poor		80	87	93
	Fair		71	81	89
	Good		62	74	85
Oak-aspen: mountain brush mixture of oak brush, aspen, mountain mahogany, bitter brush, maple, and other brush	Poor		66	74	79
	Fair		48	57	63
	Good		30	41	48
Pinyon-juniper: pinyon, juniper, or both; grass understory	Poor		75	85	89
	Fair		58	73	80
	Good		41	61	71
Sagebrush with grass understory	Poor		67	80	85
	Fair		51	63	70
	Good		35	47	55
Desert shrub: major plants include saltbush, greasewood, creosote bush, blackbrush, bursage, Palo Verde, mesquite, and cactus	Poor	63	77	85	88
	Fair	55	72	81	86
	Good	49	68	79	84

¹ Average Runoff Condition, and $I_a = 0.2S$.

² Poor: < 30% ground cover (litter, grass, and brush overstory)

Fair: 30 to 70% ground cover (not applicable in Scottsdale)

Good: >70% ground cover (not applicable in Scottsdale)

³ Curve Numbers for Group A have been developed only for desert shrub

FIGURE 4-6. RUNOFF CURVE NUMBERS FOR ARID & SEMIARID RANGELANDS

2. Infiltration

Infiltration or soil losses will be determined using Green and Ampt (G&A) procedures per FCDMC Hydrology Manual. Use the most recent published SCS soil survey maps of the area to determine the hydrologic soil group or surface soil texture for the G&A procedures. Use USDA Natural Resources Conservation Services (previously the Soil Conservation Services) maps, Soil Survey of Aguila-Carefree Area, Parts of Maricopa and Pinal Counties, or the Soil Survey of Eastern Maricopa and Northern Pinal Counties, Arizona; depending on what part of the city you are located.

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3. For developed conditions, increase the percent impervious on the LS card without changing the ROCN (except in the case of grassed areas, in which the curve number should be adjusted up according to the above). Use the actual or estimated percent impervious; a minimum of 85 percent for commercial and 72 percent industrial.

RUNOFF CURVE NUMBERS FOR ARID & SEMIARID RANGELANDS¹

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	Fair		71	81	89
	Good		62	74	85
Oak-aspen: mountain brush mixture of oak brush, aspen, mountain mahogany, bitter brush, maple, and other brush	Poor		66	74	79
	Fair		48	57	63
	Good		30	41	48
Pinyon-juniper: pinyon, juniper, or both; grass understory	Poor		75	85	89
	Fair		58	73	80
	Good		41	61	71
Sagebrush with grass understory	Poor		67	80	85
	Fair		51	63	70
	Good		35	47	55
Desert shrub: major plants include saltbush, greasewood, creosote bush, blackbrush, bursage, Palo Verde, mesquite, and cactus	Poor	63	77	85	88
	Fair	55	72	81	86
	Good	49	68	79	84

¹ Average Runoff Condition, and $I_a = 0.2S$.

² Poor: < 30% ground cover (litter, grass, and brush overstory)

Fair: 30 to 70% ground cover (not applicable in Scottsdale)

Good: >70% ground cover (not applicable in Scottsdale)

³ Curve Numbers for Group A have been developed only for desert shrub

FIGURE 4-6. RUNOFF CURVE NUMBERS FOR ARID & SEMIARID RANGELANDS

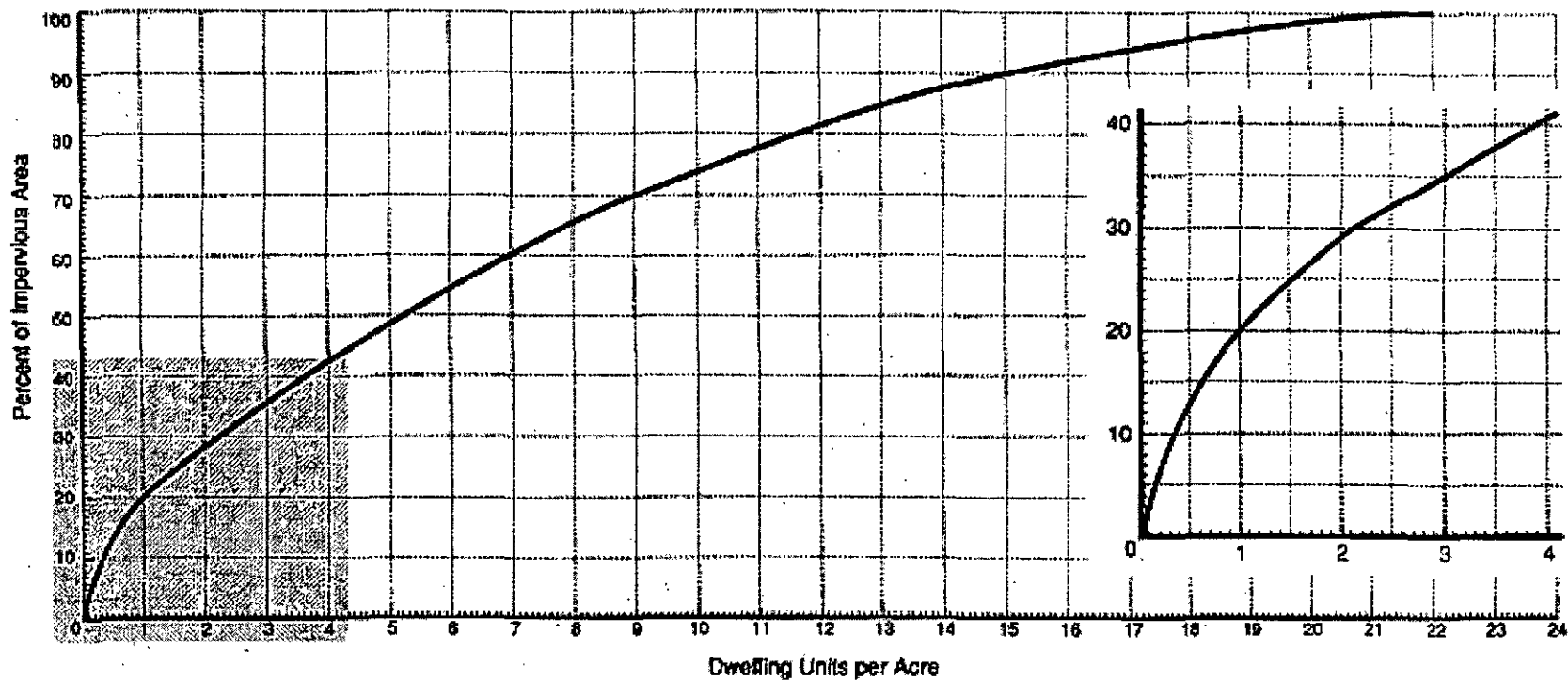


FIGURE 2.2-16

Percent of Impervious Area vs. Dwelling Density

Developed by Water Resources Associates, Inc. from data in Table 2.2a of TR-55, Urban Hydrology For Small Watersheds, and from discussions with Scottsdale city staff.

3.4.4 Element Application

(1) **Overland Flow.** The overland flow element is a wide rectangular channel of unit width; so, referring to Figure 3.6, $\alpha = 1.486S^{1/3}/N$ and $m = 5/3$. Notice that Manning's n has been replaced by an overland flow roughness factor, N . Typical values of N are shown in Table 3.5. When applying Equations (3.43) and (3.46) to an overland flow element, the lateral inflow is rainfall excess (previously computed using methods described in Section 3.2) and the outflow is a flow per unit width.

An overland flow element is described by four parameters: a typical overland flow length, L , slope and roughness factor which are used to compute α , and the percent of the subbasin area represented by this element.

Two overland flow elements may be used for each subbasin. The total discharge, Q , from each element is computed as

$$Q = q * \frac{\text{AREA}}{L} \quad (3.70)$$

Table 3.5
Resistance Factor for Overland Flow

Surface	N value	Source
Asphalt/Concrete*	0.05 - 0.15	a
Bare Packed Soil Free of Stone	0.10	c
Fallow - No Residue	0.008 - 0.012	b
Conventional Tillage - No Residue	0.06 - 0.12	b
Conventional Tillage - With Residue	0.16 - 0.22	b
Chisel Plow - No Residue	0.06 - 0.12	b
Chisel Plow - With Residue	0.10 - 0.16	b
Fall Disking - With Residue	0.30 - 0.50	b
No Till - No Residue	0.04 - 0.10	b
No Till (20-40 percent residue cover)	0.07 - 0.17	b
No Till (60-100 percent residue cover)	0.17 - 0.47	b
Sparse Rangeland with Debris:		
0 Percent Cover	0.09 - 0.34	b
20 Percent Cover	0.05 - 0.25	b
Sparse Vegetation	0.053 - 0.13	f
Short Grass Prairie	0.10 - 0.20	f
Poor Grass Cover On Moderately Rough	0.30	c
Bare Surface		
Light Turf	0.20	a
Average Grass Cover	0.4	c
Dense Turf	0.17 - 0.80	a,c,e,f
Dense Grass	0.17 - 0.30	d
Bermuda Grass	0.30 - 0.48	d
Dense Shrubbery and Forest Litter	0.4	a

Legend: a) Harley (1975), b) Engman (1986), c) Hathaway (1945), d) Palmer (1946), e) Ragan and Duru (1972), f) Woolhiser (1975). (See Hjermfelt, 1986)

*Asphalt/Concrete n value for open channel flow 0.01 - 0.016

LAND USE Composite Area-wide Values	Hydrologic Soil Group		
	B	C	D
Commercial & Industrial Areas	0.90		
Residential Areas-Single Family (av. lot size)			
R1-1-1901	0.33	0.50	0.58
R1-130	0.35	0.51	0.59
R1-70	0.37	0.52	0.60
R1-43	0.38	0.55	0.61
R1-35 (35,000 sq. ft./lot)	0.40	0.56	0.62
R1-18 (18,000 sq. ft./lot)	0.43	0.58	0.64
R1-10 (10,000 sq. ft./lot)	0.47	0.62	0.67
R1-7 (7,000 sq. ft./lot)	0.51	0.64	0.70
Townhouses (R-2, R-4)	0.63	0.74	0.78
Apartments & Condominiums (R-3, R-5)	0.76	0.83	0.87
Specific Surface Type Values			
Paved streets, parking lots (concrete or asphalt), roofs, & driveways, etc.	0.95		
Lawns, golf courses, & parks (grassed areas)	0.33	0.56	0.66
Undisturbed natural desert or desert landscaping (no impervious weed barrier)	0.31	0.48	0.56
Desert landscaping (with impervious weed barrier)	0.83	0.83	0.83
Mountain terrain – slopes greater than 10%	0.70	0.70	0.70
Agricultural areas (flood-irrigated fields)	0.20	0.20	0.20

FIGURE 4-5. RUNOFF COEFFICIENTS FOR USE WITH RATIONAL METHOD

D. The Army Corps of Engineer's HEC-1 Computer Model

HEC-1 procedures are applicable for any watershed area over 160 acres and up to 100 square miles in size. HEC-1 is required for analyzing drainage areas over 160 acres in size. **HEC-HMS is not an acceptable substitute for HEC-1.** Minimum required submittals when using HEC-1 are:

- A printout of the input data
- A schematic (routing) diagram of the stream network
- The runoff summary output table
- Electronic input file(s), on CD or floppy
- Supporting Documentation and Source Material for parameter selection

1. Precipitation

Precipitation values for HEC-1 modeling shall be determined using the Flood Control Manual, specifically PD and JD records for point rainfall and area reduction factors. Capital Projects shall use the ADOT manual and methodology when specified. Precipitation values are to be obtained from the Isopluvial maps for the specific frequency desired (see Appendix 4-D).

APPENDIX B

Existing Condition Hydrologic Calculations

- 2-year HEC-1 Model
- 10-year HEC-1 Model
- 100-year HEC-1 Model

EX-2.0H1

 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * JUN 1998 *
 * VERSION 4.1 *
 * RUN DATE 07NOV06 TIME 09:27:06 *

 * U.S. ARMY CORPS OF ENGINEERS *
 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 756-1104 *

```

      X  X  XXXXXX  XXXXX  X
      X  X  X      X  X  XX
      X  X  X      X  X  X
      XXXXXX  XXXX  X  XXXX  X
      X  X  X      X  X  X
      X  X  X      X  X  X
      X  X  XXXXXX  XXXXX  XXX
  
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	HEC-1 MODEL FOR SERENO CANYON									
2	ID	2-YEAR, 6-HOUR STORM									
3	ID	RAINFALL FROM NOAA ATLAS									
4	ID	SCS CURVE NUMBER SOIL LOSS PARAMETERS									
5	ID	KINEMATIC WAVE HYDROGRAPH ROUTING									
6	ID	EXISTING CONDITIONS									
7	ID										
8	ID	PREPARED BY WOOD/PATEL, 8.4.2004									
9	ID	FILE NAME: EX-100.DAT									
10	ID										
11	IT	2									
12	IO	5									
13	JO		.01								
14	PH			.38	0.7	1.12	1.27	1.38	1.57		
15	KK	A1									
16	KM	RUNOFF FROM SUB-BASIN A1									
17	BA	.084									
18	LS	88									
19	UK	260	.025	0.13	100						
20	RK	3520	.023	.032		TRAP	22	12	5		
21	KO										
22	KK	A2									
23	KM	RUNOFF FROM SUB-BASIN A2									
24	BA	.036									
25	LS	88									
26	UK	235	.025	0.13	100						
27	RK	2000	.02	.032		TRAP	22	8	5		
28	KO										
29	KK	B1									
30	KM	RUNOFF FROM SUB-BASIN B1									
31	BA	.027									
32	LS	88									
33	UK	200	.025	0.13	100						
34	RK	2500	.028	.032		TRAP	22	8	5		
35	KO										
36	KK	B2									
37	KM	RUNOFF FROM SUB-BASIN B2									
38	BA	.050									
39	LS	88									
40	UK	400	.025	0.13	100						
41	RK	2340	.024	.032		TRAP	22	8	5		
42	KK	COMBB									
43	KM	COMBINE SECTIONS B1 AND B2									
44	HC	2									
45	KO										

1

HEC-1 INPUT

PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
46	KK	C									
47	KM	RUNOFF FROM SUB-BASIN C									
48	BA	.03									
49	LS	88									
50	UK	250	.023	0.13	100						
51	RK	1570	.017	.032		TRAP	22	5	5		
52	KO										
53	KK	D									
54	KM	RUNOFF FROM SUB-BASIN D									
55	BA	.034									
56	LS	88									

EX-2.OH1

57	UK	300	.02	0.13	100			
58	RK	1425	.014	.032		TRAP	6	5
59	KO					22		
60	KK	E1						
61	KM	RUNOFF FROM SUB-BASIN E1						
62	BA	.061						
63	LS		88					
64	UK	400	.02	0.13	100			
65	RK	2050	.013	.032		TRAP	7	5
66	KO					22		
67	KK	E2						
68	KM	RUNOFF FROM SUB-BASIN E2						
69	BA	.046						
70	LS		88					
71	UK	200	.02	0.13	100			
72	RK	2370	.013	.032		TRAP	10	5
73	KO					22		
74	KK	COMBE						
75	KM	COMBINE SECTIONS E1 AND E2						
76	HC	2						
77	KO					22		
78	KK	CLEAR						
79	KM	CLEAR HYDROGRAPH STACK						
80	HC	6						
	*							
81	KK	E3						
82	KM	RUNOFF FROM SUB-BASIN E3						
83	BA	.008						
84	LS		88					
85	UK	175	.025	0.13	100			
86	RK	750	.032	.032		TRAP	6	5
87	KO					22		

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

88	KK	F1						
89	KM	RUNOFF FROM SUB-BASIN F1						
90	BA	.036						
91	LS		88					
92	UK	225	.018	0.13	100			
93	RK	2500	.012	.032		TRAP	7	5
94	KO					22		
95	KK	F2						
96	KM	RUNOFF FROM SUB-BASIN F2						
97	BA	.014						
98	LS		88					
99	UK	200	.018	0.13	100			
100	RK	1440	.028	.032		TRAP	6	5
101	KO					22		
102	KK	F3						
103	KM	RUNOFF FROM SUB-BASIN F3						
104	BA	.013						
105	LS		88					
106	UK	225	.018	0.13	100			
107	RK	850	.026	.032		TRAP	6	5
108	KO					22		
109	KK	G						
110	KM	RUNOFF FROM SUB-BASIN G						
111	BA	0.016						
112	LS		88					
113	UK	400	.025	0.13	100			
114	RK	720	.022	.032		TRAP	4	5
115	KO					22		
116	KK	H1						
117	KM	RUNOFF FROM SUB-BASIN H1						
118	BA	.059						
119	LS		88					
120	UK	375	.025	0.13	100			
121	RK	2210	.019	.032		TRAP	8	5
122	KO					22		
123	KK	H2						
124	KM	RUNOFF FROM SUB-BASIN H2						
125	BA	.072						
126	LS		88					
127	UK	400	.025	0.13	100			
128	RK	3480	.019	.032		TRAP	12	5
129	KO					22		
130	KK	I						
131	KM	RUNOFF FROM SUB-BASIN I						
132	BA	.025						
133	LS		88					
134	UK	225	.02	0.13	100			
135	RK	2100	.026	.032		TRAP	10	5

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

136	KO					22		
137	KK	CLEAR						
138	KM	CLEAR HYDROGRAPH STACK						
139	HC	8						

EX-2.OH1

140	KK	J							
141	KM	RUNOFF	FROM SUB-BASIN J						
142	BA	.024							
143	LS		88	0					
144	UK	145	.02	0.13	100				
145	RK	965	.026	.032		TRAP	5	5	
146	KO					22			
147	KK	K							
148	KM	RUNOFF	FROM SUB-BASIN K						
149	BA	.009							
150	LS		88	0					
151	UK	81	.025	0.13	100				
152	RK	715	.022	.032		TRAP	5	5	
153	KO					22			
154	KK	L							
155	KM	RUNOFF	FROM SUB-BASIN L						
156	BA	.009							
157	LS		88	0					
158	UK	113	.018	0.13	100				
159	RK	450	.012	.032		TRAP	5	5	
160	KO					22			
161	KK	M							
162	KM	RUNOFF	FROM SUB-BASIN M						
163	BA	.008							
164	LS		88	0					
165	UK	137	.018	0.13	100				
166	RK	750	.012	.032		TRAP	4	5	
167	KO					22			
168	KK	N							
169	KM	RUNOFF	FROM SUB-BASIN N						
170	BA	.01							
171	LS		88	0					
172	UK	117	.018	0.13	100				
173	RK	1036	.012	.032		TRAP	5	5	
174	KO					22			
175	KK	O							
176	KM	RUNOFF	FROM SUB-BASIN O						
177	BA	.0029							
178	LS		88	0					
179	UK	108	.02	0.13	100				
180	RK	174	.014	.032		TRAP	4	5	
181	KO					22			

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

182	KK	P							
183	KM	RUNOFF	FROM SUB-BASIN P						
184	BA	.0069							
185	LS		88	0					
186	UK	135	.02	0.13	100				
187	RK	354	.014	.032		TRAP	5	5	
188	KO					22			
189	KK	CLEAR							
190	KM	CLEAR HYDROGRAPH STACK							
191	HC	8							
192	KK	Q							
193	KM	RUNOFF	FROM SUB-BASIN Q						
194	BA	.0032							
195	LS		88	0					
196	UK	86	.02	0.13	100				
197	RK	160	.014	.032		TRAP	4	5	
198	KO					22			
199	KK	R							
200	KM	RUNOFF	FROM SUB-BASIN R						
201	BA	.15							
202	LS		88						
203	UK	180	.025	0.13	100				
204	RK	4300	.023	.032		TRAP	15	5	
205	KO					22			
206	ZZ								

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

15	A1								
22	.	A2							
29	.	.	B1						
36	.	.	.	B2					
42	.	.	COMBB.....						
46	.	.	.	C					
53	.	.	.	.					

60					E1				
67						E2			
74					COMBE				
78	CLEAR								
81		E3							
88			F1						
95				F2					
102					F3				
109						G			
116							H1		
123								H2	
130									I
137	CLEAR								
140			J						
147				K					
154					L				
161						M			
168							N		
175								O	
182									P
189	CLEAR								
192			Q						
199				R					

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 07NOV06 TIME 09:27:06 *
*****

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*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *

PREPARED BY WOOD/PATEL, 8.4.2004
FILE NAME: EX-100.DAT

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12 IO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE

IT         HYDROGRAPH TIME DATA
            NMNIN      2  MINUTES IN COMPUTATION INTERVAL
            IDATE      1  0  STARTING DATE
            ITIME      0000  STARTING TIME
            NQ         900  NUMBER OF HYDROGRAPH ORDINATES

```

Page 4

EX-2.OH1

NDDATE 2 0 ENDING DATE
 NDTIME 0558 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .03 HOURS
 TOTAL TIME BASE 29.97 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

13 JD INDEX STORM NO. 1
 STRM 1.57 PRECIPITATION DEPTH
 TRDA .01 TRANSPOSITION DRAINAGE AREA

14 PI PRECIPITATION PATTERN

.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.01	.01	.01	.01	.01	.01	.01	.01	.01	.01	.01
.02	.02	.02	.02	.03	.03	.03	.04	.06	.08	.15
.15	.12	.07	.05	.03	.03	.03	.02	.02	.02	.02
.01	.01	.01	.01	.01	.01	.01	.01	.01	.01	.01
.01	.01	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

*** **

 * *
 15 KK * A1 *
 * *

21 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

*** **

 * *
 27 KK * A2 *
 * *

28 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

*** **

 * *
 29 KK * B1 *
 * *

35 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

*** **

*
42 KK COMBB
*

45 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

*
46 KK C
*

52 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

*
53 KK D
*

59 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

*
60 KK E1
*

66 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

*
67 KK E2
*

73 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED

ISAV2 900 LAST ORDINATE PUNCHED OR SAVED EX-2.OH1
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
74 KK * COMBE *
* *

77 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
81 KK * E3 *
* *

87 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
88 KK * F1 *
* *

94 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION= 1

*** **

* *
95 KK * F2 *
* *

101 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
102 KK * F3 *
* *

```

108 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
*      G  *
*      *
*****

```

```

115 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
*      H1 *
*      *
*****

```

```

122 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
*      H2 *
*      *
*****

```

```

129 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
*      I  *
*      *
*****

```

```

136 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

140 KK

```
*****
*       *
*       J *
*       *
*****
```

146 KO

OUTPUT CONTROL VARIABLES

```
IPRNT      5 PRINT CONTROL
IPLOT      0 PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS
```

*** **

147 KK

```
*****
*       *
*       K *
*       *
*****
```

153 KO

OUTPUT CONTROL VARIABLES

```
IPRNT      5 PRINT CONTROL
IPLOT      0 PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS
```

*** **

154 KK

```
*****
*       *
*       L *
*       *
*****
```

160 KO

OUTPUT CONTROL VARIABLES

```
IPRNT      5 PRINT CONTROL
IPLOT      0 PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS
```

*** **

161 KK

```
*****
*       *
*       M *
*       *
*****
```

167 KO

OUTPUT CONTROL VARIABLES

```
IPRNT      5 PRINT CONTROL
IPLOT      0 PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS
```

*** **

168 KK

```
*****
*       *
*       N *
*       *
*****
```

174 KO

OUTPUT CONTROL VARIABLES

```
IPRNT      5 PRINT CONTROL
IPLOT      0 PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS
```


*** **

* *
175 KK * O *
* *

181 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
182 KK * P *
* *

188 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
192 KK * Q *
* *

198 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
199 KK * R *
* *

205 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

1

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

+	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	A1	46.	3.30	6.	1.	1.	.08		
+	HYDROGRAPH AT	A2	21.	3.27	2.	1.	0.	.04		
+	HYDROGRAPH AT	B1	17.	3.27	2.	0.	0.	.03		
+	HYDROGRAPH AT	B2	23.	3.40	3.	1.	1.	.05		

EX-2.OH1

+	2 COMBINED AT	COMBB	37.	3.30	5.	1.	1.	.08
+	HYDROGRAPH AT	C	17.	3.27	2.	1.	0.	.03
+	HYDROGRAPH AT	D	17.	3.33	2.	1.	0.	.03
+	HYDROGRAPH AT	E1	26.	3.43	4.	1.	1.	.06
+	HYDROGRAPH AT	E2	27.	3.30	3.	1.	1.	.05
+	2 COMBINED AT	COMBE	49.	3.33	7.	2.	1.	.11
+	6 COMBINED AT	CLEAR	186.	3.30	25.	6.	5.	.37
+	HYDROGRAPH AT	E3	6.	3.20	1.	0.	0.	.01
+	HYDROGRAPH AT	F1	19.	3.33	2.	1.	0.	.04
+	HYDROGRAPH AT	F2	9.	3.27	1.	0.	0.	.01
+	HYDROGRAPH AT	F3	8.	3.27	1.	0.	0.	.01
+	HYDROGRAPH AT	G	7.	3.37	1.	0.	0.	.02
+	HYDROGRAPH AT	H1	28.	3.37	4.	1.	1.	.06
+	HYDROGRAPH AT	H2	32.	3.43	5.	1.	1.	.07
+	HYDROGRAPH AT	I	14.	3.30	2.	0.	0.	.03
+	8 COMBINED AT	CLEAR	113.	3.33	16.	4.	3.	.24
+	HYDROGRAPH AT	J	18.	3.17	2.	0.	0.	.02
+	HYDROGRAPH AT	K	9.	3.13	1.	0.	0.	.01
+	HYDROGRAPH AT	L	8.	3.17	1.	0.	0.	.01
+	HYDROGRAPH AT	M	6.	3.20	1.	0.	0.	.01
+	HYDROGRAPH AT	N	8.	3.20	1.	0.	0.	.01
+	HYDROGRAPH AT	O	3.	3.13	0.	0.	0.	.00
+	HYDROGRAPH AT	P	6.	3.17	0.	0.	0.	.01
+	8 COMBINED AT	CLEAR	144.	3.30	21.	5.	4.	.31
+	HYDROGRAPH AT	Q	3.	3.10	0.	0.	0.	.00
+	HYDROGRAPH AT	R	94.	3.27	10.	3.	2.	.15

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION INTERVAL PEAK	INTERPOLATED TO COMPUTATION INTERVAL TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1	STORM AREA (SQ MI) =								
A1	MANE	2.00	45.60	197.96	.63	2.00	45.59	198.00	.63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2833E+01 OUTFLOW= .2811E+01 BASIN STORAGE= .1146E-02 PERCENT ERROR= .7

FOR STORM = 1	STORM AREA (SQ MI) =								
A2	MANE	2.00	21.65	196.72	.63	2.00	21.49	196.00	.63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1214E+01 OUTFLOW= .1203E+01 BASIN STORAGE= .4066E-03 PERCENT ERROR= .9

FOR STORM = 1	STORM AREA (SQ MI) =								
B1	MANE	1.82	17.27	195.49	.63	2.00	17.14	196.00	.63

EX-2.OH1

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9106E+00 OUTFLOW= .9045E+00 BASIN STORAGE= .2559E-03 PERCENT ERROR= .6

FOR STORM = 1 STORM AREA (SQ MI) = .01
 B2 MANE 2.00 22.90 203.78 .63 2.00 22.84 204.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1686E+01 OUTFLOW= .1674E+01 BASIN STORAGE= .1114E-02 PERCENT ERROR= .7

FOR STORM = 1 STORM AREA (SQ MI) = .01
 C MANE 1.85 17.26 197.14 .63 2.00 17.16 196.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1012E+01 OUTFLOW= .1005E+01 BASIN STORAGE= .3633E-03 PERCENT ERROR= .6

FOR STORM = 1 STORM AREA (SQ MI) = .01
 D MANE 1.82 17.39 199.15 .63 2.00 17.32 200.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1147E+01 OUTFLOW= .1139E+01 BASIN STORAGE= .5591E-03 PERCENT ERROR= .6

FOR STORM = 1 STORM AREA (SQ MI) = .01
 E1 MANE 1.85 25.97 206.70 .63 2.00 25.95 206.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2057E+01 OUTFLOW= .2042E+01 BASIN STORAGE= .1681E-02 PERCENT ERROR= .7

FOR STORM = 1 STORM AREA (SQ MI) = .01
 E2 MANE 2.00 27.30 196.30 .63 2.00 27.05 198.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1551E+01 OUTFLOW= .1541E+01 BASIN STORAGE= .5151E-03 PERCENT ERROR= .7

FOR STORM = 1 STORM AREA (SQ MI) = .01
 E3 MANE 1.00 5.96 191.22 .63 2.00 5.94 192.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2698E+00 OUTFLOW= .2685E+00 BASIN STORAGE= .5610E-04 PERCENT ERROR= .5

FOR STORM = 1 STORM AREA (SQ MI) = .01
 F1 MANE 1.98 19.30 199.10 .63 2.00 19.19 200.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1214E+01 OUTFLOW= .1204E+01 BASIN STORAGE= .5067E-03 PERCENT ERROR= .8

FOR STORM = 1 STORM AREA (SQ MI) = .01
 F2 MANE 1.71 8.55 195.83 .63 2.00 8.53 196.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4722E+00 OUTFLOW= .4686E+00 BASIN STORAGE= .1519E-03 PERCENT ERROR= .7

FOR STORM = 1 STORM AREA (SQ MI) = .01
 F3 MANE 1.06 7.64 196.27 .63 2.00 7.63 196.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4385E+00 OUTFLOW= .4352E+00 BASIN STORAGE= .1636E-03 PERCENT ERROR= .7

FOR STORM = 1 STORM AREA (SQ MI) = .01
 G MANE 1.01 7.50 201.08 .63 2.00 7.49 202.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5396E+00 OUTFLOW= .5361E+00 BASIN STORAGE= .3341E-03 PERCENT ERROR= .6

FOR STORM = 1 STORM AREA (SQ MI) = .01
 H1 MANE 2.00 27.81 202.87 .63 2.00 27.65 202.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1990E+01 OUTFLOW= .1974E+01 BASIN STORAGE= .1180E-02 PERCENT ERROR= .7

FOR STORM = 1 STORM AREA (SQ MI) = .01
 H2 MANE 2.00 31.67 205.47 .63 2.00 31.56 206.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2428E+01 OUTFLOW= .2411E+01 BASIN STORAGE= .1729E-02 PERCENT ERROR= .7

FOR STORM = 1 STORM AREA (SQ MI) = .01
 I MANE 1.74 14.44 197.94 .63 2.00 14.42 198.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .8432E+00 OUTFLOW= .8362E+00 BASIN STORAGE= .3168E-03 PERCENT ERROR= .8

FOR STORM = 1 STORM AREA (SQ MI) = .01
 J MANE .89 18.64 191.55 .63 2.00 18.28 190.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .8095E+00 OUTFLOW= .8050E+00 BASIN STORAGE= .1481E-03 PERCENT ERROR= .5

FOR STORM = 1 STORM AREA (SQ MI) = .01
 K MANE .94 9.41 187.34 .63 2.00 9.14 188.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3035E+00 OUTFLOW= .3025E+00 BASIN STORAGE= .1753E-04 PERCENT ERROR= .3

EX-2.OH1

FOR STORM = 1 STORM AREA (SQ MI) = .01
 L MANE .75 7.78 189.63 .63 2.00 7.66 190.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3035E+00 OUTFLOW= .3020E+00 BASIN STORAGE= .3530E-04 PERCENT ERROR= .5

FOR STORM = 1 STORM AREA (SQ MI) = .01
 M MANE 1.26 6.15 191.67 .63 2.00 6.09 192.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2698E+00 OUTFLOW= .2683E+00 BASIN STORAGE= .5108E-04 PERCENT ERROR= .6

FOR STORM = 1 STORM AREA (SQ MI) = .01
 N MANE 1.73 7.98 191.88 .63 2.00 7.94 192.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3373E+00 OUTFLOW= .3355E+00 BASIN STORAGE= .5160E-04 PERCENT ERROR= .5

FOR STORM = 1 STORM AREA (SQ MI) = .01
 O MANE .47 2.67 188.19 .63 2.00 2.65 188.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9781E-01 OUTFLOW= .9736E-01 BASIN STORAGE= .1145E-04 PERCENT ERROR= .4

FOR STORM = 1 STORM AREA (SQ MI) = .01
 P MANE .68 5.61 189.50 .63 2.00 5.60 190.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2327E+00 OUTFLOW= .2314E+00 BASIN STORAGE= .3947E-04 PERCENT ERROR= .5

FOR STORM = 1 STORM AREA (SQ MI) = .01
 Q MANE .37 3.22 187.49 .63 2.00 3.19 186.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1079E+00 OUTFLOW= .1075E+00 BASIN STORAGE= .8190E-05 PERCENT ERROR= .4

FOR STORM = 1 STORM AREA (SQ MI) = .01
 R MANE 2.00 94.22 195.52 .63 2.00 93.94 196.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5059E+01 OUTFLOW= .5020E+01 BASIN STORAGE= .1201E-02 PERCENT ERROR= .7

*** NORMAL END OF HEC-1 ***

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 07NOV06 TIME 09:46:54 *
*****

```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

```

X X XXXXXXX XXXX X
X X X X X XX
X X X X X X
XXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID HEC-1 MODEL FOR SERENO CANYON
2 ID 10-YEAR, 6-HOUR STORM
3 ID RAINFALL FROM NOAA ATLAS
4 ID SCS CURVE NUMBER SOIL LOSS PARAMETERS
5 ID KINEMATIC WAVE HYDROGRAPH ROUTING
6 ID EXISTING CONDITIONS
7 ID
8 ID PREPARED BY WOOD/PATEL, 8.4.2004
9 ID FILE NAME: EX-10.DAT
10 ID
11 *DIAGRAM
12 IT 2 900
13 ID 5
14 JD .01
15 PH 0.52 0.99 1.64 1.87 2.01 2.29
16 KK A1
17 KM RUNOFF FROM SUB-BASIN A1
18 BA .084
19 LS 88
20 UK 260 .025 0.13 100
21 RK 3520 .023 .032 TRAP 12 5
22 KO
23 KK A2
24 KM RUNOFF FROM SUB-BASIN A2
25 BA .036
26 LS 88
27 UK 235 .025 0.13 100
28 RK 2000 .02 .032 TRAP 8 5
29 KO
30 KK B1
31 KM RUNOFF FROM SUB-BASIN B1
32 BA .027
33 LS 88
34 UK 200 .025 0.13 100
35 RK 2500 .028 .032 TRAP 8 5
36 KO
37 KK B2
38 KM RUNOFF FROM SUB-BASIN B2
39 BA .050
40 LS 88
41 UK 400 .025 0.13 100
42 RK 2340 .024 .032 TRAP 8 5
43 KO
44 KK COMBB
45 KM COMBINE SECTIONS B1 AND B2
46 HC 2
47 KO

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1

HEC-1 INPUT

PAGE 2

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
46 KK C
47 KM RUNOFF FROM SUB-BASIN C
48 BA .03
49 LS 88
50 UK 250 .023 0.13 100
51 RK 1570 .017 .032 TRAP 5 5
52 KO
53 KK D
54 KM RUNOFF FROM SUB-BASIN D
55 BA .034
56 LS 88

```

EX-10.OH1

57	UK	300	.02	0.13	100			
58	RK	1425	.014	.032		TRAP	6	5
59	KO					22		
60	KK	E1						
61	KM	RUNOFF FROM SUB-BASIN E1						
62	BA	.061						
63	LS	88						
64	UK	400	.02	0.13	100			
65	RK	2050	.013	.032		TRAP	7	5
66	KO					22		
67	KK	E2						
68	KM	RUNOFF FROM SUB-BASIN E2						
69	BA	.046						
70	LS	88						
71	UK	200	.02	0.13	100			
72	RK	2370	.013	.032		TRAP	10	5
73	KO					22		
74	KK	COMBE						
75	KM	COMBINE SECTIONS E1 AND E2						
76	HC	2						
77	KO					22		
78	KK	CLEAR						
79	KM	CLEAR HYDROGRAPH STACK						
80	HC	6						
	*							
81	KK	E3						
82	KM	RUNOFF FROM SUB-BASIN E3						
83	BA	.008						
84	LS	88						
85	UK	175	.025	0.13	100			
86	RK	750	.032	.032		TRAP	6	5
87	KO					22		

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

88	KK	F1						
89	KM	RUNOFF FROM SUB-BASIN F1						
90	BA	.036						
91	LS	88						
92	UK	225	.018	0.13	100			
93	RK	2500	.012	.032		TRAP	7	5
94	KO					22		
95	KK	F2						
96	KM	RUNOFF FROM SUB-BASIN F2						
97	BA	.014						
98	LS	88						
99	UK	200	.018	0.13	100			
100	RK	1440	.028	.032		TRAP	6	5
101	KO					22		
102	KK	F3						
103	KM	RUNOFF FROM SUB-BASIN F3						
104	BA	.013						
105	LS	88						
106	UK	225	.018	0.13	100			
107	RK	850	.026	.032		TRAP	6	5
108	KO					22		
109	KK	G						
110	KM	RUNOFF FROM SUB-BASIN G						
111	BA	0.016						
112	LS	88						
113	UK	400	.025	0.13	100			
114	RK	720	.022	.032		TRAP	4	5
115	KO					22		
116	KK	H1						
117	KM	RUNOFF FROM SUB-BASIN H1						
118	BA	.059						
119	LS	88						
120	UK	375	.025	0.13	100			
121	RK	2210	.019	.032		TRAP	8	5
122	KO					22		
123	KK	H2						
124	KM	RUNOFF FROM SUB-BASIN H2						
125	BA	.072						
126	LS	88						
127	UK	400	.025	0.13	100			
128	RK	3480	.019	.032		TRAP	12	5
129	KO					22		
130	KK	I						
131	KM	RUNOFF FROM SUB-BASIN I						
132	BA	.025						
133	LS	88						
134	UK	225	.02	0.13	100			
135	RK	2100	.026	.032		TRAP	10	5

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

136	KO					22		
137	KK	CLEAR						
138	KM	CLEAR HYDROGRAPH STACK						
139	HC	8						

EX-10.OH1

140	KK	J							
141	KM	RUNOFF FROM SUB-BASIN J							
142	BA	.024							
143	LS		88	0					
144	UK	145	.02	0.13	100				
145	RK	965	.026	.032		TRAP	5	5	
146	KO					22			
147	KK	K							
148	KM	RUNOFF FROM SUB-BASIN K							
149	BA	.009							
150	LS		88	0					
151	UK	81	.025	0.13	100				
152	RK	715	.022	.032		TRAP	5	5	
153	KO					22			
154	KK	L							
155	KM	RUNOFF FROM SUB-BASIN L							
156	BA	.009							
157	LS		88	0					
158	UK	113	.018	0.13	100				
159	RK	450	.012	.032		TRAP	5	5	
160	KO					22			
161	KK	M							
162	KM	RUNOFF FROM SUB-BASIN M							
163	BA	.008							
164	LS		88	0					
165	UK	137	.018	0.13	100				
166	RK	750	.012	.032		TRAP	4	5	
167	KO					22			
168	KK	N							
169	KM	RUNOFF FROM SUB-BASIN N							
170	BA	.01							
171	LS		88	0					
172	UK	117	.018	0.13	100				
173	RK	1036	.012	.032		TRAP	5	5	
174	KO					22			
175	KK	O							
176	KM	RUNOFF FROM SUB-BASIN O							
177	BA	.0029							
178	LS		88	0					
179	UK	108	.02	0.13	100				
180	RK	174	.014	.032		TRAP	4	5	
181	KO					22			

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

182	KK	P							
183	KM	RUNOFF FROM SUB-BASIN P							
184	BA	.0069							
185	LS		88	0					
186	UK	135	.02	0.13	100				
187	RK	354	.014	.032		TRAP	5	5	
188	KO					22			
189	KK	CLEAR							
190	KM	CLEAR HYDROGRAPH STACK							
191	HC	8							
192	KK	Q							
193	KM	RUNOFF FROM SUB-BASIN Q							
194	BA	.0032							
195	LS		88	0					
196	UK	86	.02	0.13	100				
197	RK	160	.014	.032		TRAP	4	5	
198	KO					22			
199	KK	R							
200	KM	RUNOFF FROM SUB-BASIN R							
201	BA	.15							
202	LS		88						
203	UK	180	.025	0.13	100				
204	RK	4300	.023	.032		TRAP	15	5	
205	KO					22			
206	ZZ								

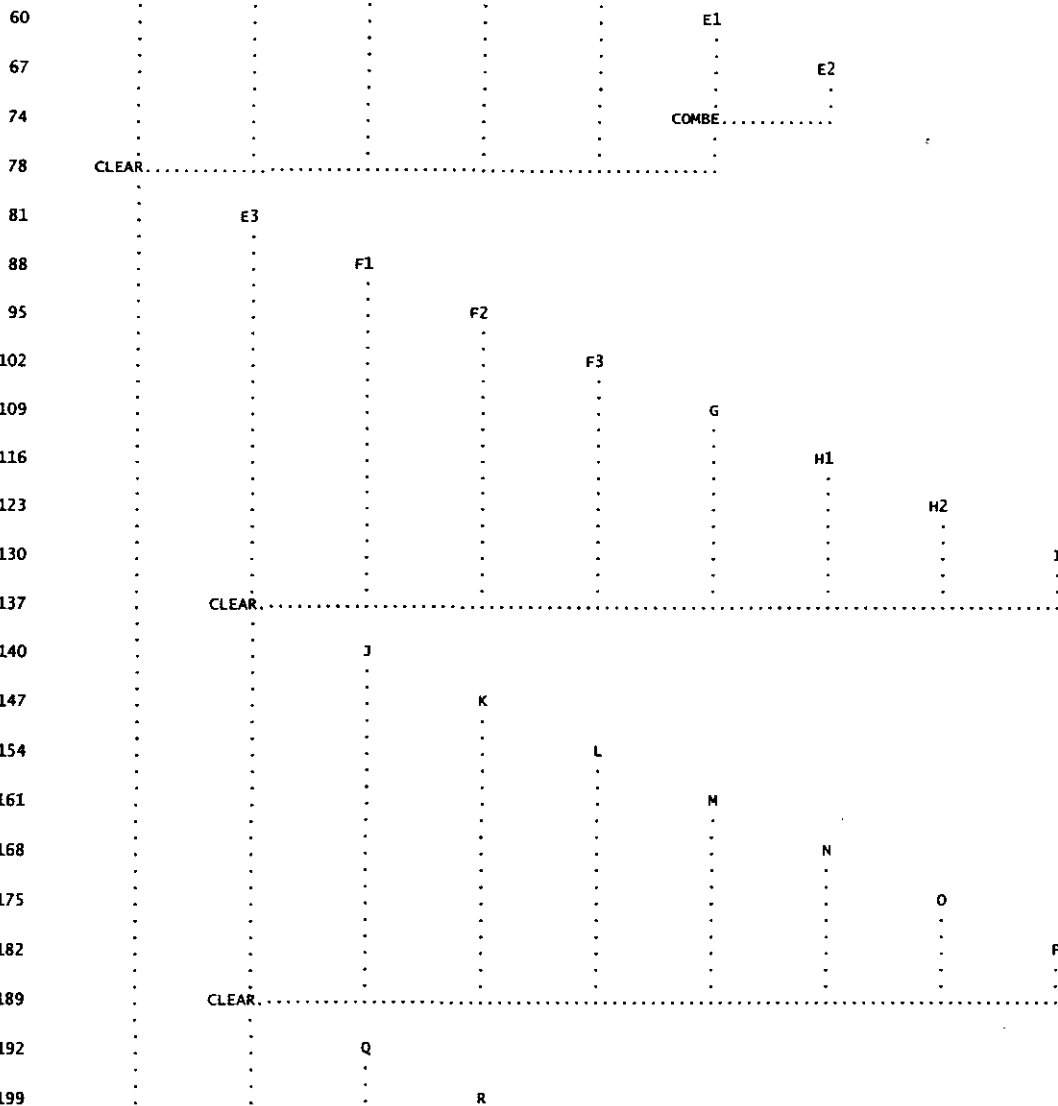
SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
LINE

(V) ROUTING (--->) DIVERSION OR PUMP FLOW
(.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

15	A1								
22	.	A2							
29	.	.	B1						
36	.	.	.	B2					
42	.	.	COMBB.	.					
46	.	.	.	.	C				
53	.	.	.	.	.				

EX-10.0H1



(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * JUN 1998 *
 * VERSION 4.1 *
 * RUN DATE 07NOV06 TIME 09:46:54 *

 * U.S. ARMY CORPS OF ENGINEERS *
 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 756-1104 *

HEC-1 MODEL FOR SERENO CANYON
 10-YEAR, 6-HOUR STORM
 RAINFALL FROM NOAA ATLAS
 SCS CURVE NUMBER SOIL LOSS PARAMETERS
 KINEMATIC WAVE HYDROGRAPH ROUTING
 EXISTING CONDITIONS

PREPARED BY WOOD/PATEL, 8.4.2004
 FILE NAME: EX-10.DAT

12 IO OUTPUT CONTROL VARIABLES
 IPRINT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IT HYDROGRAPH TIME DATA
 NMIN 2 MINUTES IN COMPUTATION INTERVAL
 IDATE 1 0 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 900 NUMBER OF HYDROGRAPH ORDINATES
 Page 4

NDDATE 2 0 ENDING DATE
 NDTIME 0558 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .03 HOURS
 TOTAL TIME BASE 29.97 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

13 JD INDEX STORM NO. 1
 STRM 2.29 PRECIPITATION DEPTH
 TRDA .01 TRANSPOSITION DRAINAGE AREA

14 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .01 .01 .01 .01 .01 .01 .01 .01 .01 .01
 .01 .01 .01 .01 .01 .01 .01 .01 .01 .01
 .02 .03 .03 .04 .04 .04 .04 .04 .04 .04
 .21 .17 .10 .08 .05 .04 .04 .04 .04 .04
 .02 .02 .02 .02 .02 .01 .01 .01 .01 .01
 .01 .01 .01 .01 .01 .01 .01 .01 .01 .01
 .01 .01 .01 .01 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
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 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00

*** **

 * *
 15 KK * A1 *
 * *

21 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 * *
 22 KK * A2 *
 * *

28 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 * *
 29 KK * B1 *
 * *

35 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

```
*****
*
42 KK  * COMBB *
*
*****
```

```
45 KO  OUTPUT CONTROL VARIABLES
        IPRINT      5  PRINT CONTROL
        IPLOT       0  PLOT CONTROL
        QSCAL       0. HYDROGRAPH PLOT SCALE
        IPNCH       0  PUNCH COMPUTED HYDROGRAPH
        IOUT        22 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1       1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT      .033 TIME INTERVAL IN HOURS
```

*** **

```
*****
*
46 KK  * C *
*
*****
```

```
52 KO  OUTPUT CONTROL VARIABLES
        IPRINT      5  PRINT CONTROL
        IPLOT       0  PLOT CONTROL
        QSCAL       0. HYDROGRAPH PLOT SCALE
        IPNCH       0  PUNCH COMPUTED HYDROGRAPH
        IOUT        22 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1       1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT      .033 TIME INTERVAL IN HOURS
```

*** **

```
*****
*
53 KK  * D *
*
*****
```

```
59 KO  OUTPUT CONTROL VARIABLES
        IPRINT      5  PRINT CONTROL
        IPLOT       0  PLOT CONTROL
        QSCAL       0. HYDROGRAPH PLOT SCALE
        IPNCH       0  PUNCH COMPUTED HYDROGRAPH
        IOUT        22 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1       1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT      .033 TIME INTERVAL IN HOURS
```

*** **

```
*****
*
60 KK  * E1 *
*
*****
```

```
66 KO  OUTPUT CONTROL VARIABLES
        IPRINT      5  PRINT CONTROL
        IPLOT       0  PLOT CONTROL
        QSCAL       0. HYDROGRAPH PLOT SCALE
        IPNCH       0  PUNCH COMPUTED HYDROGRAPH
        IOUT        22 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1       1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT      .033 TIME INTERVAL IN HOURS
```

*** **

```
*****
*
67 KK  * E2 *
*
*****
```

```
73 KO  OUTPUT CONTROL VARIABLES
        IPRINT      5  PRINT CONTROL
        IPLOT       0  PLOT CONTROL
        QSCAL       0. HYDROGRAPH PLOT SCALE
        IPNCH       0  PUNCH COMPUTED HYDROGRAPH
        IOUT        22 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1       1  FIRST ORDINATE PUNCHED OR SAVED
```

ISAV2
TIMINT

900 LAST ORDINATE PUNCHED OR SAVED
.033 TIME INTERVAL IN HOURS

EX-10.OH1

*** **

* *
74 KK * COMBE *
* *

77 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
81 KK * E3 *
* *

87 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
88 KK * F1 *
* *

94 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
95 KK * F2 *
* *

101 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
102 KK * F3 *
* *

108 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL

EX-10.0H1

QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

*** **

* *
109 KK * G *
* *

115 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
116 KK * H1 *
* *

122 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
123 KK * H2 *
* *

129 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
130 KK * I *
* *

136 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
140 KK * J *
* *

146 K0 OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 * *
 147 KK * K *
 * *

153 K0 OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 * *
 154 KK * L *
 * *

160 K0 OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 * *
 161 KK * M *
 * *

167 K0 OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 * *
 168 KK * N *
 * *

174 K0 OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

175 KK * *
 * 0 *
 * * *

181 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

182 KK *****
 * *
 * p *
 * * *

188 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

192 KK *****
 * *
 * Q *
 * * *

198 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

199 KK *****
 * *
 * R *
 * * *

205 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+	A1	111.	3.23	11.	3.	2.	.08		
+	HYDROGRAPH AT								
+	A2	52.	3.20	5.	1.	1.	.04		
+	HYDROGRAPH AT								
+	B1	40.	3.17	3.	1.	1.	.03		
+	HYDROGRAPH AT								
+	B2	57.	3.27	6.	2.	1.	.05		
+	2 COMBINED AT								
+	COMBB	93.	3.23	10.	2.	2.	.08		
	HYDROGRAPH AT								

					EX-10.0H1			
+		C	43.	3.20	4.	1.	1.	.03
+	HYDROGRAPH AT	D	43.	3.20	4.	1.	1.	.03
+	HYDROGRAPH AT	E1	67.	3.27	8.	2.	2.	.06
+	HYDROGRAPH AT	E2	65.	3.20	6.	1.	1.	.05
+	2 COMBINED AT	COMBE	126.	3.23	14.	3.	3.	.11
+	6 COMBINED AT	CLEAR	463.	3.20	47.	12.	9.	.37
+	HYDROGRAPH AT	E3	14.	3.13	1.	0.	0.	.01
+	HYDROGRAPH AT	F1	47.	3.23	5.	1.	1.	.04
+	HYDROGRAPH AT	F2	21.	3.17	2.	0.	0.	.01
+	HYDROGRAPH AT	F3	19.	3.17	2.	0.	0.	.01
+	HYDROGRAPH AT	G	19.	3.23	2.	1.	0.	.02
+	HYDROGRAPH AT	H1	70.	3.23	8.	2.	2.	.06
+	HYDROGRAPH AT	H2	79.	3.30	9.	2.	2.	.07
+	HYDROGRAPH AT	I	35.	3.20	3.	1.	1.	.03
+	8 COMBINED AT	CLEAR	285.	3.23	31.	8.	6.	.24
+	HYDROGRAPH AT	J	43.	3.13	3.	1.	1.	.02
+	HYDROGRAPH AT	K	19.	3.10	1.	0.	0.	.01
+	HYDROGRAPH AT	L	17.	3.10	1.	0.	0.	.01
+	HYDROGRAPH AT	M	14.	3.13	1.	0.	0.	.01
+	HYDROGRAPH AT	N	18.	3.13	1.	0.	0.	.01
+	HYDROGRAPH AT	O	6.	3.10	0.	0.	0.	.00
+	HYDROGRAPH AT	P	13.	3.10	1.	0.	0.	.01
+	8 COMBINED AT	CLEAR	364.	3.20	40.	10.	8.	.31
+	HYDROGRAPH AT	Q	7.	3.07	0.	0.	0.	.00
+	HYDROGRAPH AT	R	220.	3.20	19.	5.	4.	.15

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	INTERPOLATED TO COMPUTATION INTERVAL			
						DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)
FOR STORM = 1	STORM AREA (SQ MI) =			.01					
A1	MANE	2.00	113.23	193.02	1.20	2.00	110.67	194.00	1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5392E+01 OUTFLOW= .5360E+01 BASIN STORAGE= .1203E-02 PERCENT ERROR= .6

FOR STORM = 1	STORM AREA (SQ MI) =			.01					
A2	MANE	1.72	53.31	191.26	1.20	2.00	52.34	192.00	1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2311E+01 OUTFLOW= .2300E+01 BASIN STORAGE= .4152E-03 PERCENT ERROR= .4

FOR STORM = 1	STORM AREA (SQ MI) =			.01					
B1	MANE	1.97	41.51	190.99	1.20	2.00	40.35	190.00	1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1733E+01 OUTFLOW= .1725E+01 BASIN STORAGE= .2299E-03 PERCENT ERROR= .5

FOR STORM = 1	STORM AREA (SQ MI) =			.01					
B2	MANE	1.73	57.59	194.86	1.20	2.00	57.02	196.00	1.20

EX-10.0H1

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3209E+01 OUTFLOW= .3188E+01 BASIN STORAGE= .1093E-02 PERCENT ERROR= .6

FOR STORM = 1 STORM AREA (SQ MI) = .01
C MANE 1.39 43.38 190.70 1.20 2.00 42.69 192.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1926E+01 OUTFLOW= .1917E+01 BASIN STORAGE= .3510E-03 PERCENT ERROR= .4

FOR STORM = 1 STORM AREA (SQ MI) = .01
D MANE 1.49 43.45 192.11 1.20 2.00 43.28 192.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2182E+01 OUTFLOW= .2168E+01 BASIN STORAGE= .6112E-03 PERCENT ERROR= .6

FOR STORM = 1 STORM AREA (SQ MI) = .01
E1 MANE 1.86 66.55 195.99 1.20 2.00 66.55 196.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3915E+01 OUTFLOW= .3895E+01 BASIN STORAGE= .1651E-02 PERCENT ERROR= .5

FOR STORM = 1 STORM AREA (SQ MI) = .01
E2 MANE 2.00 65.93 191.46 1.20 2.00 65.44 192.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2953E+01 OUTFLOW= .2937E+01 BASIN STORAGE= .5186E-03 PERCENT ERROR= .5

FOR STORM = 1 STORM AREA (SQ MI) = .01
E3 MANE .77 14.06 187.27 1.20 2.00 13.99 188.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5135E+00 OUTFLOW= .5118E+00 BASIN STORAGE= .4884E-04 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
F1 MANE 1.88 47.77 193.48 1.20 2.00 47.39 194.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2311E+01 OUTFLOW= .2297E+01 BASIN STORAGE= .5291E-03 PERCENT ERROR= .6

FOR STORM = 1 STORM AREA (SQ MI) = .01
F2 MANE 1.37 21.16 190.57 1.20 2.00 21.06 190.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .8986E+00 OUTFLOW= .8950E+00 BASIN STORAGE= .1544E-03 PERCENT ERROR= .4

FOR STORM = 1 STORM AREA (SQ MI) = .01
F3 MANE .85 19.08 190.44 1.20 2.00 19.04 190.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .8344E+00 OUTFLOW= .8306E+00 BASIN STORAGE= .1711E-03 PERCENT ERROR= .4

FOR STORM = 1 STORM AREA (SQ MI) = .01
G MANE .84 18.87 194.76 1.20 2.00 18.84 194.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1027E+01 OUTFLOW= .1020E+01 BASIN STORAGE= .3250E-03 PERCENT ERROR= .6

FOR STORM = 1 STORM AREA (SQ MI) = .01
H1 MANE 1.68 70.36 194.24 1.20 2.00 69.96 194.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3787E+01 OUTFLOW= .3764E+01 BASIN STORAGE= .1280E-02 PERCENT ERROR= .6

FOR STORM = 1 STORM AREA (SQ MI) = .01
H2 MANE 2.00 79.06 198.18 1.19 2.00 79.00 198.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4621E+01 OUTFLOW= .4588E+01 BASIN STORAGE= .1717E-02 PERCENT ERROR= .7

FOR STORM = 1 STORM AREA (SQ MI) = .01
I MANE 1.73 35.72 191.54 1.20 2.00 35.41 192.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1605E+01 OUTFLOW= .1596E+01 BASIN STORAGE= .2887E-03 PERCENT ERROR= .5

FOR STORM = 1 STORM AREA (SQ MI) = .01
J MANE .82 43.52 187.61 1.20 2.00 42.95 188.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1540E+01 OUTFLOW= .1535E+01 BASIN STORAGE= .1366E-03 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
K MANE .84 19.33 185.62 1.20 2.00 18.99 186.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5777E+00 OUTFLOW= .5771E+00 BASIN STORAGE= .1799E-04 PERCENT ERROR= .1

FOR STORM = 1 STORM AREA (SQ MI) = .01
L MANE .61 17.53 186.45 1.20 2.00 17.46 186.00 1.20

EX-10.0H1

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5777E+00 OUTFLOW= .5764E+00 BASIN STORAGE= .4197E-04 PERCENT ERROR= .2

FOR STORM = 1 STORM AREA (SQ MI) = .01
M MANE 1.10 14.36 187.63 1.20 2.00 14.28 188.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5135E+00 OUTFLOW= .5119E+00 BASIN STORAGE= .4761E-04 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
N MANE 1.44 18.20 187.71 1.20 2.00 18.08 188.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6419E+00 OUTFLOW= .6400E+00 BASIN STORAGE= .5152E-04 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
O MANE .35 5.86 185.68 1.20 2.00 5.79 186.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1861E+00 OUTFLOW= .1856E+00 BASIN STORAGE= .1133E-04 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
P MANE .48 12.94 186.82 1.20 2.00 12.84 186.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4429E+00 OUTFLOW= .4414E+00 BASIN STORAGE= .3229E-04 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
Q MANE .35 6.82 184.66 1.20 2.00 6.64 184.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2054E+00 OUTFLOW= .2051E+00 BASIN STORAGE= .8253E-05 PERCENT ERROR= .1

FOR STORM = 1 STORM AREA (SQ MI) = .01
R MANE 2.00 224.79 190.71 1.20 2.00 220.38 192.00 1.20

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9628E+01 OUTFLOW= .9574E+01 BASIN STORAGE= .1305E-02 PERCENT ERROR= .5

*** NORMAL END OF HEC-1 ***

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 31OCT06 TIME 11:12:07 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1XW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION. NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE:GREEN AND AMPT INFILTRATION, KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID HEC-1 MODEL FOR SERENO CANYON
2 ID 100-YEAR, 6-HOUR STORM
3 ID RAINFALL FROM NOAA ATLAS
4 ID SCS CURVE NUMBER SOIL LOSS PARAMETERS
5 ID KINEMATIC WAVE HYDROGRAPH ROUTING
6 ID EXISTING CONDITIONS
7 ID
8 ID PREPARED BY WOOD/PATEL, 8.4.2004
9 ID FILE NAME: EX-100.DAT
10 ID
11 *DIAGRAM
12 IT 2 900
13 ID 5
14 ID .01 .73 1.43 2.42 2.74 2.96 3.37
15 PH
16 KK A1
17 KM RUNOFF FROM SUB-BASIN A1
18 BA .084
19 LS 88
20 UK 260 .025 0.13 100
21 RK 3520 .023 .032 TRAP 12 5
22 KO
23 KK A2
24 KM RUNOFF FROM SUB-BASIN A2
25 BA .036
26 LS 88
27 UK 235 .025 0.13 100
28 RK 2000 .02 .032 TRAP 8 5
29 KO
30 KK B1
31 KM RUNOFF FROM SUB-BASIN B1
32 BA .027
33 LS 88
34 UK 200 .025 0.13 100
35 RK 2500 .028 .032 TRAP 8 5
36 KO
37 KK B2
38 KM RUNOFF FROM SUB-BASIN B2
39 BA .050
40 LS 88
41 UK 400 .025 0.13 100
42 RK 2340 .024 .032 TRAP 8 5
43 KO
44 KK COMBB
45 KM COMBINE SECTIONS B1 AND B2
46 HC 2
47 KO

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1 HEC-1 INPUT PAGE 2

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
46 KK C
47 KM RUNOFF FROM SUB-BASIN C
48 BA .03
49 LS 88
50 UK 250 .023 0.13 100
51 RK 1570 .017 .032 TRAP 5 5
52 KO
53 KK D
54 KM RUNOFF FROM SUB-BASIN D
55 BA .034
56 LS 88

```

EX-100.QH1

57	UK	300	.02	0.13	100			
58	RK	1425	.014	.032		TRAP	6	5
59	KO					22		
60	KK	E1						
61	KM	RUNOFF FROM SUB-BASIN E1						
62	BA	.061						
63	LS	88						
64	UK	400	.02	0.13	100			
65	RK	2050	.013	.032		TRAP	7	5
66	KO					22		
67	KK	E2						
68	KM	RUNOFF FROM SUB-BASIN E2						
69	BA	.046						
70	LS	88						
71	UK	200	.02	0.13	100			
72	RK	2370	.013	.032		TRAP	10	5
73	KO					22		
74	KK	COMBE						
75	KM	COMBINE SECTIONS E1 AND E2						
76	HC	2						
77	KO					22		
78	KK	CLEAR						
79	KM	CLEAR HYDROGRAPH STACK						
80	HC	6						
81	KK	E3						
82	KM	RUNOFF FROM SUB-BASIN E3						
83	BA	.008						
84	LS	88						
85	UK	175	.025	0.13	100			
86	RK	750	.032	.032		TRAP	6	5
87	KO					22		

HEC-1 INPUT

PAGE 3

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

88	KK	F1						
89	KM	RUNOFF FROM SUB-BASIN F1						
90	BA	.036						
91	LS	88						
92	UK	225	.018	0.13	100			
93	RK	2500	.012	.032		TRAP	7	5
94	KO					22		
95	KK	F2						
96	KM	RUNOFF FROM SUB-BASIN F2						
97	BA	.014						
98	LS	88						
99	UK	200	.018	0.13	100			
100	RK	1440	.028	.032		TRAP	6	5
101	KO					22		
102	KK	F3						
103	KM	RUNOFF FROM SUB-BASIN F3						
104	BA	.013						
105	LS	88						
106	UK	225	.018	0.13	100			
107	RK	850	.026	.032		TRAP	6	5
108	KO					22		
109	KK	G						
110	KM	RUNOFF FROM SUB-BASIN G						
111	BA	0.016						
112	LS	88						
113	UK	400	.025	0.13	100			
114	RK	720	.022	.032		TRAP	4	5
115	KO					22		
116	KK	H1						
117	KM	RUNOFF FROM SUB-BASIN H1						
118	BA	.059						
119	LS	88						
120	UK	375	.025	0.13	100			
121	RK	2210	.019	.032		TRAP	8	5
122	KO					22		
123	KK	H2						
124	KM	RUNOFF FROM SUB-BASIN H2						
125	BA	.072						
126	LS	88						
127	UK	400	.025	0.13	100			
128	RK	3480	.019	.032		TRAP	12	5
129	KO					22		
130	KK	I						
131	KM	RUNOFF FROM SUB-BASIN I						
132	BA	.025						
133	LS	88						
134	UK	225	.02	0.13	100			
135	RK	2100	.026	.032		TRAP	10	5

HEC-1 INPUT

PAGE 4

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

136	KO					22		
137	KK	CLEAR						
138	KM	CLEAR HYDROGRAPH STACK						
139	HC	8						

EX-100.OH1

140	KK	J							
141	KM	RUNOFF	FROM SUB-BASIN J						
142	BA	.024							
143	LS		88	0					
144	UK	145	.02	0.13	100				
145	RK	965	.026	.032		TRAP	5	5	
146	KO					22			
147	KK	K							
148	KM	RUNOFF	FROM SUB-BASIN K						
149	BA	.009							
150	LS		88	0					
151	UK	81	.025	0.13	100				
152	RK	715	.022	.032		TRAP	5	5	
153	KO					22			
154	KK	L							
155	KM	RUNOFF	FROM SUB-BASIN L						
156	BA	.009							
157	LS		88	0					
158	UK	113	.018	0.13	100				
159	RK	450	.012	.032		TRAP	5	5	
160	KO					22			
161	KK	M							
162	KM	RUNOFF	FROM SUB-BASIN M						
163	BA	.008							
164	LS		88	0					
165	UK	137	.018	0.13	100				
166	RK	750	.012	.032		TRAP	4	5	
167	KO					22			
168	KK	N							
169	KM	RUNOFF	FROM SUB-BASIN N						
170	BA	.01							
171	LS		88	0					
172	UK	117	.018	0.13	100				
173	RK	1036	.012	.032		TRAP	5	5	
174	KO					22			
175	KK	O							
176	KM	RUNOFF	FROM SUB-BASIN O						
177	BA	.0029							
178	LS		88	0					
179	UK	108	.02	0.13	100				
180	RK	174	.014	.032		TRAP	4	5	
181	KO					22			

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

182	KK	P							
183	KM	RUNOFF	FROM SUB-BASIN P						
184	BA	.0069							
185	LS		88	0					
186	UK	135	.02	0.13	100				
187	RK	354	.014	.032		TRAP	5	5	
188	KO					22			
189	KK	CLEAR							
190	KM	CLEAR HYDROGRAPH STACK							
191	HC	8							
192	KK	Q							
193	KM	RUNOFF	FROM SUB-BASIN Q						
194	BA	.0032							
195	LS		88	0					
196	UK	86	.02	0.13	100				
197	RK	160	.014	.032		TRAP	4	5	
198	KO					22			
199	KK	R							
200	KM	RUNOFF	FROM SUB-BASIN R						
201	BA	.15							
202	LS		88						
203	UK	180	.025	0.13	100				
204	RK	4300	.023	.032		TRAP	15	5	
205	KO					22			
206	ZZ								

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

15  A1
    .
22  .      A2
    .
29  .      .      B1
    .      .      .
36  .      .      .      B2
    .      .      .      .
42  .      .      COMBB.....
    .      .      .
46  .      .      .      C
    .      .      .      .
53  .      .      .      .
  
```

EX-100.OH1

NDDATE 2 0 ENDING DATE
NDTIME 0558 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .03 HOURS
TOTAL TIME BASE 29.97 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-Feet
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

13 JD INDEX STORM NO. 1
STRM 3.37 PRECIPITATION DEPTH
TRDA .01 TRANSPOSITION DRAINAGE AREA

14 PI PRECIPITATION PATTERN
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.01 .01 .01 .01 .01 .01 .01 .01 .01 .01
.01 .01 .01 .01 .01 .01 .01 .01 .01 .01
.01 .01 .01 .01 .01 .01 .01 .01 .01 .01
.01 .01 .01 .01 .01 .01 .01 .01 .01 .01
.04 .04 .04 .06 .06 .07 .09 .12 .17 .29
.29 .24 .15 .11 .07 .07 .06 .05 .04 .04
.04 .03 .03 .03 .03 .01 .01 .01 .01 .01
.01 .01 .01 .01 .01 .01 .01 .01 .01 .01
.01 .01 .01 .01 .01 .01 .01 .01 .01 .01
.01 .01 .01 .01 .01 .01 .01 .01 .01 .01
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00

*** **

* *
15 KK * A1 *
* *

21 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
22 KK * A2 *
* *

28 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
29 KK * B1 *
* *

35 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

42 KK

```
*****
*          *
*   COMBB  *
*          *
*****
```

45 KO

OUTPUT CONTROL VARIABLES

```
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0.  HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS
```

46 KK

```
*****
*          *
*    C     *
*          *
*****
```

52 KO

OUTPUT CONTROL VARIABLES

```
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0.  HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS
```

53 KK

```
*****
*          *
*    D     *
*          *
*****
```

59 KO

OUTPUT CONTROL VARIABLES

```
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0.  HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS
```

60 KK

```
*****
*          *
*   E1     *
*          *
*****
```

66 KO

OUTPUT CONTROL VARIABLES

```
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0.  HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS
```

67 KK

```
*****
*          *
*   E2     *
*          *
*****
```

73 KO

OUTPUT CONTROL VARIABLES

```
IPRNT      5  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0.  HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
```

ISAV2 900 LAST ORDINATE PUNCHED OR SAVED EX-100.OM1
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
74 KK * COMBE *
* *

77 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
81 KK * E3 *
* *

87 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
88 KK * F1 *
* *

94 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
95 KK * F2 *
* *

101 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
102 KK * F3 *
* *

108 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL

EX-100.0H1

QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

*** **

109 KK *****
* *
* G *
* *

115 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

*** **

116 KK *****
* *
* H1 *
* *

122 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

*** **

123 KK *****
* *
* H2 *
* *

129 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

*** **

130 KK *****
* *
* I *
* *

136 KO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	0	PUNCH COMPUTED HYDROGRAPH
IOUT	22	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	900	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

*** **

140 KK *****
* *
* J *
* *

146 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 * *
 147 KK * K *
 * *

153 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 * *
 154 KK * L *
 * *

160 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 * *
 161 KK * M *
 * *

167 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 * *
 168 KK * N *
 * *

174 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

					EX-100.OH1			
+		C	89.	3.13	7.	2.	1.	.03
+	HYDROGRAPH AT	D	93.	3.17	8.	2.	2.	.03
+	HYDROGRAPH AT	E1	146.	3.20	14.	4.	3.	.06
+	HYDROGRAPH AT	E2	131.	3.13	11.	3.	2.	.05
+	2 COMBINED AT	COMBE	275.	3.17	25.	6.	5.	.11
+	6 COMBINED AT	CLEAR	994.	3.17	84.	21.	17.	.37
+	HYDROGRAPH AT	E3	28.	3.10	2.	0.	0.	.01
+	HYDROGRAPH AT	F1	99.	3.17	8.	2.	2.	.04
+	HYDROGRAPH AT	F2	43.	3.13	3.	1.	1.	.01
+	HYDROGRAPH AT	F3	39.	3.13	3.	1.	1.	.01
+	HYDROGRAPH AT	G	41.	3.17	4.	1.	1.	.02
+	HYDROGRAPH AT	H1	152.	3.17	14.	3.	3.	.06
+	HYDROGRAPH AT	H2	172.	3.20	16.	4.	3.	.07
+	HYDROGRAPH AT	I	73.	3.13	6.	1.	1.	.03
+	8 COMBINED AT	CLEAR	619.	3.17	56.	14.	11.	.24
+	HYDROGRAPH AT	J	83.	3.10	6.	1.	1.	.02
+	HYDROGRAPH AT	K	35.	3.07	2.	1.	0.	.01
+	HYDROGRAPH AT	L	32.	3.10	2.	1.	0.	.01
+	HYDROGRAPH AT	M	28.	3.10	2.	0.	0.	.01
+	HYDROGRAPH AT	N	35.	3.10	2.	1.	0.	.01
+	HYDROGRAPH AT	O	11.	3.07	1.	0.	0.	.00
+	HYDROGRAPH AT	P	24.	3.07	2.	0.	0.	.01
+	8 COMBINED AT	CLEAR	801.	3.13	72.	18.	14.	.31
+	HYDROGRAPH AT	Q	13.	3.07	1.	0.	0.	.00
+	HYDROGRAPH AT	R	444.	3.13	34.	9.	7.	.15

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION INTERVAL PEAK	INTERPOLATED TO COMPUTATION INTERVAL TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1	STORM AREA (SQ MI) =			.01					
A1	MANE	1.93	236.19	188.91	2.14	2.00	234.69	190.00	2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9633E+01 OUTFLOW= .9599E+01 BASIN STORAGE= .1058E-02 PERCENT ERROR= .3

FOR STORM = 1	STORM AREA (SQ MI) =			.01					
A2	MANE	1.40	109.51	187.98	2.15	2.00	109.47	188.00	2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4129E+01 OUTFLOW= .4119E+01 BASIN STORAGE= .3972E-03 PERCENT ERROR= .2

FOR STORM = 1	STORM AREA (SQ MI) =			.01					
B1	MANE	1.65	83.67	187.47	2.15	2.00	83.24	188.00	2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3096E+01 OUTFLOW= .3090E+01 BASIN STORAGE= .2639E-03 PERCENT ERROR= .2

FOR STORM = 1	STORM AREA (SQ MI) =			.01					
B2	MANE	1.47	126.47	191.43	2.14	2.00	125.94	190.00	2.14

EX-100.OH1

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5734E+01 OUTFLOW= .5714E+01 BASIN STORAGE= .1075E-02 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
C MANE 1.21 89.27 188.60 2.14 2.00 89.14 188.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3440E+01 OUTFLOW= .3431E+01 BASIN STORAGE= .3398E-03 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
D MANE 1.09 93.69 189.80 2.14 2.00 93.00 190.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3899E+01 OUTFLOW= .3887E+01 BASIN STORAGE= .5549E-03 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
E1 MANE 1.50 146.93 191.32 2.14 2.00 145.90 192.00 2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6996E+01 OUTFLOW= .6964E+01 BASIN STORAGE= .1588E-02 PERCENT ERROR= .4

FOR STORM = 1 STORM AREA (SQ MI) = .01
E2 MANE 1.80 135.47 188.91 2.14 2.00 131.38 188.00 2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5275E+01 OUTFLOW= .5261E+01 BASIN STORAGE= .4665E-03 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
E3 MANE .59 27.61 185.93 2.15 2.00 27.58 186.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9174E+00 OUTFLOW= .9163E+00 BASIN STORAGE= .5292E-04 PERCENT ERROR= .1

FOR STORM = 1 STORM AREA (SQ MI) = .01
F1 MANE 2.00 99.30 190.12 2.14 2.00 98.91 190.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4129E+01 OUTFLOW= .4114E+01 BASIN STORAGE= .4961E-03 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
F2 MANE 1.05 42.99 186.95 2.15 2.00 42.74 188.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1606E+01 OUTFLOW= .1603E+01 BASIN STORAGE= .1464E-03 PERCENT ERROR= .2

FOR STORM = 1 STORM AREA (SQ MI) = .01
F3 MANE .79 39.63 187.38 2.15 2.00 39.33 188.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1491E+01 OUTFLOW= .1487E+01 BASIN STORAGE= .1603E-03 PERCENT ERROR= .2

FOR STORM = 1 STORM AREA (SQ MI) = .01
G MANE .64 41.98 189.23 2.14 2.00 41.19 190.00 2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1835E+01 OUTFLOW= .1829E+01 BASIN STORAGE= .3183E-03 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
H1 MANE 1.38 152.54 190.65 2.14 2.00 152.20 190.00 2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6766E+01 OUTFLOW= .6742E+01 BASIN STORAGE= .1371E-02 PERCENT ERROR= .3

FOR STORM = 1 STORM AREA (SQ MI) = .01
H2 MANE 2.00 172.46 193.00 2.14 2.00 171.53 192.00 2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .8257E+01 OUTFLOW= .8222E+01 BASIN STORAGE= .1669E-02 PERCENT ERROR= .4

FOR STORM = 1 STORM AREA (SQ MI) = .01
I MANE 1.43 73.88 189.00 2.14 2.00 72.74 188.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2867E+01 OUTFLOW= .2860E+01 BASIN STORAGE= .3037E-03 PERCENT ERROR= .2

FOR STORM = 1 STORM AREA (SQ MI) = .01
J MANE .72 84.27 185.42 2.15 2.00 83.48 186.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2752E+01 OUTFLOW= .2749E+01 BASIN STORAGE= .1338E-03 PERCENT ERROR= .1

FOR STORM = 1 STORM AREA (SQ MI) = .01
K MANE .72 35.23 184.51 2.15 2.00 35.04 184.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1032E+01 OUTFLOW= .1032E+01 BASIN STORAGE= .1840E-04 PERCENT ERROR= .0

FOR STORM = 1 STORM AREA (SQ MI) = .01
L MANE .48 33.06 185.01 2.15 2.00 31.77 186.00 2.15

EX-100.OH1

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1032E+01 OUTFLOW= .1031E+01 BASIN STORAGE= .3573E-04 PERCENT ERROR= .2

FOR STORM = 1 STORM AREA (SQ MI) = .01
 M MANE .83 27.65 185.81 2.15 2.00 27.57 186.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9174E+00 OUTFLOW= .9164E+00 BASIN STORAGE= .4951E-04 PERCENT ERROR= .1

FOR STORM = 1 STORM AREA (SQ MI) = .01
 N MANE 1.23 35.08 186.42 2.15 2.00 34.66 186.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1147E+01 OUTFLOW= .1146E+01 BASIN STORAGE= .4527E-04 PERCENT ERROR= .1

FOR STORM = 1 STORM AREA (SQ MI) = .01
 O MANE .37 10.92 184.32 2.15 2.00 10.86 184.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3326E+00 OUTFLOW= .3323E+00 BASIN STORAGE= .1057E-04 PERCENT ERROR= .1

FOR STORM = 1 STORM AREA (SQ MI) = .01
 P MANE .42 24.72 185.07 2.15 2.00 24.05 184.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .7913E+00 OUTFLOW= .7899E+00 BASIN STORAGE= .3787E-04 PERCENT ERROR= .2

FOR STORM = 1 STORM AREA (SQ MI) = .01
 Q MANE .28 12.54 183.97 2.15 2.00 12.53 184.00 2.15

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3670E+00 OUTFLOW= .3666E+00 BASIN STORAGE= .8886E-05 PERCENT ERROR= .1

FOR STORM = 1 STORM AREA (SQ MI) = .01
 R MANE 1.93 455.07 188.88 2.14 2.00 443.55 188.00 2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1720E+02 OUTFLOW= .1715E+02 BASIN STORAGE= .1227E-02 PERCENT ERROR= .3

*** NORMAL END OF HEC-1 ***

APPENDIX C

Proposed Condition Hydrologic Calculations

- 2-year HEC-1 Model
- 10-year HEC-1 Model
- 100-year HEC-1 Model

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 14NOV06 TIME 17:51:49 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

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X X XXXXXX XXXX X
X X X X X XX
X X X X X
XXXXXXX XXXX X XXXX X
X X X X X
X X X X X
X X XXXXXX XXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID HEC-1 MODEL FOR SERENO CANYON
2 ID 2-YEAR, 6-HOUR STORM
3 ID RAINFALL FROM NOAA ATLAS
4 ID SCS CURVE NUMBER SOIL LOSS PARAMETERS
5 ID KINEMATIC WAVE HYDROGRAPH ROUTING
6 ID DEVELOPED CONDITIONS
7 ID 10% IMPERVIOUS AREA USED FOR SITE
8 ID
9 ID PREPARED BY WOOD/PATEL, 1.10.2005
10 ID FILE NAME: DEV-2.DAT
11 ID
12 *DIAGRAM
13 IT 2 900
14 IO 5
15 JD .01 0.38 0.7 1.12 1.27 1.38 1.57
16 PH
17 KK A1
18 KM RUNOFF FROM SUB-BASIN A1
19 BA 0.084
20 LS 88 1.1
21 UK 260 .025 0.13 100
22 RK 3520 .023 .032 TRAP 12 5
23 *
24 KK BASA1
25 KM 29' WEIR AT HEADWALL
26 RS 1 STOR 0
27 SV 0 0.108 0.242 0.404 0.6 0.841 1.081
28 SE 2756 2757 2758 2759 2760 2761 2762
29 SQ 0 0 0 0 0 81 230
30 KO 22
31 KK A2
32 KM RUNOFF FROM SUB-BASIN A2
33 BA .036
34 LS 88 7.4
35 UK 235 .025 0.13 100
36 RK 2000 .02 .032 TRAP 8 5
37 *
38 KK BAS A2 -3
39 KM 14' WEIR AT OUTLET
40 RS 1 STOR 0
41 SV 0 0.059 0.135 0.228 0.34 0.474 0.608 0.742
42 SE 2733 2734 2735 2736 2737 2738 2739 2740
43 SQ 0 0 0 0 0 0 39 111
44 KO 22
45 *
46 KK B1
47 KM RUNOFF FROM SUB-BASIN B1
48 BA .027
49 LS 88 3.7
50 UK 200 .025 0.13 100
51 RK 2500 .028 .032 TRAP 8 5

```

PAGE 2

1

HEC-1 INPUT

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
48 KK BASB-3
49 KM 11' WEIR AT HEADWALL
50 RS 1 STOR 0
51 SV 0 .089 0.2 .338 0.49 0.643
52 SE 2781 2782 2783 2784 2785 2786
53 SQ 0 0 0 0 31 87
54 KO 22

```

DEV-2.OH1

55 KK B2
56 KM RUNOFF FROM SUB-BASIN B2
57 BA .050
58 LS 88 6.7
59 UK 400 .025 0.13 100
60 RK 2340 .024 .032 TRAP 8 5
*

61 KK COMBB
62 KM COMBINE SECTIONS B1 AND B2
63 HC 2
*

64 KK BASB-5
65 KM 25' WEIR
66 RS 1 STOR 0
67 SV 0 0.146 0.333 0.565 0.843 1.121
68 SE 2742 2743 2744 2745 2746 2747
69 SQ 0 0 0 0 70 198
70 KO 22
*

71 KK C
72 KM RUNOFF FROM SUB-BASIN C
73 BA .03
74 LS 88 5.9
75 UK 250 .023 0.13 100
76 RK 1570 .017 .032 TRAP 5 5
*

77 KK BASC-1
78 KM 11' WEIR
79 RS 1 STOR 0
80 SV 0 .107 .236 .388 0.566 0.745
81 SE 2779 2780 2781 2782 2783 2784
82 SQ 0 0 0 0 31 87
83 KO 22
*

84 KK D
85 KM RUNOFF FROM SUB-BASIN D
86 BA .034
87 LS 88 10
88 UK 300 .02 0.13 100
89 RK 1425 .014 .032 TRAP 6 5
*

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

90 KK BASD-2
91 KM 11' WEIR AT HEAD WALL
92 RS 1 STOR 0
93 SV 0 0.039 0.13 0.29 0.541 0.898
94 SE 2719 2720 2721 2722 2723 2724
95 SQ 0 0 0 0 31 87
96 KO 22
*

97 KK E1
98 KM RUNOFF FROM SUB-BASIN E1
99 BA .061
100 LS 88 10
101 UK 400 .02 0.13 100
102 RK 2050 .013 .032 TRAP 7 5
*

103 KK BASE1-2
104 KM 19' WEIR AT HEAD WALL
105 RS 1 STOR 0
106 SV 0 0.147 0.334 0.56 0.826 1.135
107 SE 2695 2696 2697 2698 2699 2700
108 SQ 0 0 0 0 53 150
109 KO 22
*

110 KK E2
111 KM RUNOFF FROM SUB-BASIN E2
112 BA .046
113 LS 88 10
114 UK 200 .02 0.13 100
115 RK 2370 .013 .032 TRAP 10 5
*

116 KK BASE2
117 KM FLOW ROUTED THROUGH DETENTION BASIN E2-1
118 KM 2-42" PIPE CULVERT
119 RS 1 STOR 0
120 SV 0 0.094 0.211 0.379 0.641 1.026 1.546 1.861 2.22
121 SE 2696 2697 2698 2699 2700 2701 2702 2702.5 2703
122 SQ 0 0 0 0 0 0 0 0 180
123 KO 22
*

124 KK COMBE
125 KM COMBINE SECTIONS E1 AND E2
126 HC 2
127 KO 22
*

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

128 KK CLEAR

DEV-2.OH1

129 KM CLEAR HYDROGRAPH STACK
130 HC 6
*

131 KK E3
132 KM RUNOFF FROM SUB-BASIN E3
133 BA .008
134 LS 88 10
135 UK 175 .025 0.13 100
136 RK 750 .032 .032 TRAP 6 5
*

137 KK BASE3 -1
138 KM 10' WEIR AT HEAD WALL
139 RS 1 STOR 0
140 SV 0 .082 .188 .322 0.456
141 SE 2711 2712 2713 2714 2715
142 SQ 0 0 0 0 28
143 KO 22
*

144 KK F1
145 KM RUNOFF FROM SUB-BASIN F1
146 BA .036
147 LS 88 10
148 UK 225 .018 0.13 100
149 RK 2500 .012 .032 TRAP 7 5
*

150 KK BASF1 -3
151 KM 12' WEIR AT HEAD WALL
152 RS 1 STOR 0
153 SV 0 0.21 0.443 0.701 0.958
154 SE 2696 2697 2698 2699 2700
155 SQ 0 0 0 34 95
156 KO 22
*

157 KK F2
158 KM RUNOFF FROM SUB-BASIN F2
159 BA .014
160 LS 88 10
161 UK 200 .018 0.13 100
162 RK 1440 .028 .032 TRAP 6 5
*

163 KK BASF2 -2
164 KM 16' WEIR AT HEAD WALL
165 RS 1 STOR 0
166 SV 0 0.112 0.243 0.396 0.549
167 SE 2701 2702 2703 2704 2705
168 SQ 0 0 0 0 45
169 KO 22
*

HEC-1 INPUT

PAGE 5

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

170 KK F3
171 KM RUNOFF FROM SUB-BASIN F3
172 BA .013
173 LS 88 10
174 UK 225 .018 0.13 100
175 RK 850 .026 .032 TRAP 6 5
*

176 KK BASF3 -1
177 KM 14' WEIR AT HEAD WALL
178 RS 1 STOR 0
179 SV 0 0.038 0.095 0.172 0.274
180 SE 2704 2705 2706 2707 2708
181 SQ 0 0 0 0 39
182 KO 22
*

183 KK G
184 KM RUNOFF FROM SUB-BASIN G
185 BA 0.016
186 LS 88 10
187 UK 400 .025 0.13 100
188 RK 720 .022 .032 TRAP 4 5
*

189 KK BASG -1
190 KM 15' WEIR
191 RS 1 STOR 0
192 SV 0 0.094 0.209 0.347 0.512 0.71
193 SE 2722 2723 2724 2725 2726 2727
194 SQ 0 0 0 0 0 42
195 KO 22
*

196 KK H1
197 KM RUNOFF FROM SUB-BASIN H1
198 BA .059
199 LS 88 9.4
200 UK 375 .025 0.13 100
201 RK 2210 .019 .032 TRAP 8 5
*

202 KK BASH1 -1
203 KM FLOW ROUTED THROUGH DETENTION BASIN H1-1
204 KM 2-36" PIPE CULVERT
205 RS 1 STOR 0
206 SV 0 0.009 0.046 0.132 0.284 0.51 0.822 1.24 1.497 1.809
207 SE 2738 2739 2740 2741 2742 2743 2744 2745 2745.5 2746
Page 3

208 SQ 0 0 0 0 DEV-2.OH1 0 0 0 0 170
 209 KO 22

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

210 KK H2
 211 KM RUNOFF FROM SUB-BASIN H2
 212 BA .072
 213 LS 88 4.78
 214 UK 400 .025 0.13 100
 215 RK 3480 .019 .032 TRAP 12 5

216 KK BASH2- 1
 217 KM 22' WEIR AT HEADWALL
 218 RS 1 STOR 0
 219 SV 0 0.15 0.318 0.507 0.718 0.929
 220 SE 0 2772 2773 2774 2775 2776
 221 SQ 0 0 0 0 62 174

222 KK BASH2- 2
 223 KM 21' WEIR AT HEADWALL
 224 RS 1 STOR 0
 225 SV 0 .165 .361 .590 .820 1.049
 226 SE 0 2785 2786 2787 2788 2789
 227 SQ 0 0 0 0 59 166
 228 KO 22

229 KK I
 230 KM RUNOFF FROM SUB-BASIN I
 231 BA .025
 232 LS 88 7.4
 233 UK 225 .02 0.13 100
 234 RK 2100 .026 .032 TRAP 10 5

235 KK BASI
 236 KM 10' WEIR
 237 RS 1 STOR 0
 238 SV 0 186 .399 .640 0.881 1.122
 239 SE 2755 2756 2757 2758 2759 2760
 240 SQ 0 0 0 0 28 79
 241 KO 22

242 KK CLEAR
 243 KM CLEAR HYDROGRAPH STACK
 244 HC 8

245 KK J
 246 KM RUNOFF FROM SUB-BASIN J
 247 BA .024
 248 LS 88 7.9
 249 UK 145 .02 0.13 100
 250 RK 965 .026 .032 TRAP 5 5

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

251 KK BASJ
 252 KM 11' WEIR
 253 RS 1 STOR 0
 254 SV 0 .098 .217 .359 .528 0.698
 255 SE 2769 2770 2771 2772 2773 2774
 256 SQ 0 0 0 0 31 87
 257 KO 22

258 KK K
 259 KM RUNOFF FROM SUB-BASIN K
 260 BA .009
 261 LS 88 10
 262 UK 81 .025 0.13 100
 263 RK 715 .022 .032 TRAP 5 5

264 KK BASK
 265 KM 13' WEIR
 266 RS 1 STOR 0
 267 SV 0 106 .232 .378 0.525
 268 SE 2737 2738 2739 2740 2741
 269 SQ 0 0 0 0 36
 270 KO 22

271 KK L
 272 KM RUNOFF FROM SUB-BASIN L
 273 BA .009
 274 LS 88 10
 275 UK 113 .018 0.13 100
 276 RK 450 .012 .032 TRAP 5 5

277 KK BASL
 278 KM 13' WEIR
 279 RS 1 STOR 0
 280 SV 0 0.041 0.106 0.2 0.326
 281 SE 2722 2723 2724 2725 2726
 282 SQ 0 0 0 0 36
 283 KO 22

284 KK M
285 KM RUNOFF FROM SUB-BASIN M
286 BA .008
287 LS 88 10
288 UK 137 .018 0.13 100
289 RK 750 .012 .032 TRAP 4 5
*

HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

290 KK BASM
291 KM 7' WEIR
292 RS 1 STOR 0
293 SV 0 0.056 0.124 0.205 0.301 0.413
294 SE 2713 2714 2715 2716 2717 2718
295 SQ 0 0 0 0 20 55
296 KO 22
*

297 KK N
298 KM RUNOFF FROM SUB-BASIN N
299 BA .01
300 LS 88 10
301 UK 117 .018 0.13 100
302 RK 1036 .012 .032 TRAP 5 5
*

303 KK BASM
304 KM 13' WEIR
305 RS 1 STOR 0
306 SV 0 0.083 0.191 0.326 0.49 0.686
307 SE 2688 2689 2690 2691 2692 2693
308 SQ 0 0 0 0 0 36
309 KO 22
*

310 KK O
311 KM RUNOFF FROM SUB-BASIN O
312 KM Area of Basin subtrating the area to be retained for 100 year event
313 BA 0.0024 9
314 LS 88 10
315 UK 108 .02 0.13 100
316 RK 174 .014 .032 TRAP 4 5
317 KO 22

318 KK P
319 KM RUNOFF FROM SUB-BASIN P
320 KM Area of Basin subtrating the area to be retained for 100 year event
321 BA .0058
322 LS 88 10
323 UK 135 .02 0.13 100
324 RK 354 .014 .032 TRAP 5 5
325 KO 22

326 KK CLEAR
327 KM CLEAR HYDROGRAPH STACK
328 HC 8

329 KK Q
330 KM RUNOFF FROM SUB-BASIN Q
331 KM Area of Basin subtrating the area to be retained for 100 year event
332 BA .0024
333 LS 88 10
334 UK 86 .02 0.13 100

HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

335 RK 160 .014 .032 TRAP 4 5
336 KO 22

337 KK R
338 KM RUNOFF FROM SUB-BASIN R
339 KM Area of Basin subtrating the area to be retained for 100 year event
340 BA .14 8
341 LS 88 .14
342 UK 180 .025 0.13 100
343 RK 4300 .023 .032 TRAP 15 5
344 KO 22
345 ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

16 A1
V
V
22 BASA1
.
.
29 A2
V
V
35 BAS A2
.
.
42 B1
V
V

48
55
61
64
71
77
84
90
97
103
110
116
124
128
131
137
144
150
157
163
170
176
183
189
196
202
210
216
222
229
235
242
245
251
258
264

DEV-2.OH1

BASB-3

B2

COMBB

BASB-5

BASC-1

BASD-2

BASE1-

BASE2

COMBE

CLEAR

E3
V
V
BASE3

F1
V
V
BASF1

F2
V
V
BASF2

F3
V
V
BASF3

G
V
V
BASG

H1
V
V
BASH1

H2
V
V
BASH2-
V
V
BASH2-

I
V
V
BASI

CLEAR

J
V
V
BASJ

K
V
V
BASK

DEV-2.OH1

```

271      .      .      .      .      .      .      .      .      .      .
277      .      .      .      .      .      .      .      .      .      .
284      .      .      .      .      .      .      .      .      .      .
290      .      .      .      .      .      .      .      .      .      .
297      .      .      .      .      .      .      .      .      .      .
303      .      .      .      .      .      .      .      .      .      .
310      .      .      .      .      .      .      .      .      .      .
318      .      .      .      .      .      .      .      .      .      .
326      .      .      .      .      .      .      .      .      .      .
329      .      .      .      .      .      .      .      .      .      .
337      .      .      .      .      .      .      .      .      .      .

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION
 1*****
 * FLOOD HYDROGRAPH PACKAGE (HEC-1)
 * JUN 1998
 * VERSION 4.1
 * RUN DATE 14NOV06 TIME 17:51:49

 * U.S. ARMY CORPS OF ENGINEERS
 * HYDROLOGIC ENGINEERING CENTER
 * 609 SECOND STREET
 * DAVIS, CALIFORNIA 95616
 * (916) 756-1104

HEC-1 MODEL FOR SERENO CANYON
 2-YEAR, 6-HOUR STORM
 RAINFALL FROM NOAA ATLAS
 SCS CURVE NUMBER SOIL LOSS PARAMETERS
 KINEMATIC WAVE HYDROGRAPH ROUTING
 DEVELOPED CONDITIONS
 10% IMPERVIOUS AREA USED FOR SITE
 PREPARED BY WOOD/PATEL, 1.10.2005
 FILE NAME: DEV-2.DAT

13 IO OUTPUT CONTROL VARIABLES
 IPRINT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IT HYDROGRAPH TIME DATA
 NMIN 2 MINUTES IN COMPUTATION INTERVAL
 IDATE 1 0 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 900 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 2 0 ENDING DATE
 NDTIME 0558 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .03 HOURS
 TOTAL TIME BASE 29.97 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

14 ID INDEX STORM NO. 1
 STRM 1.57 PRECIPITATION DEPTH
 TRDA .01 TRANSPOSITION DRAINAGE AREA

15 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
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 .01 .01 .01 .01 .01 .01 .01 .01 .01 .01
 .02 .02 .02 .02 .03 .03 .04 .06 .08 .15
 .15 .12 .07 .05 .03 .03 .02 .02 .02 .02
 .01 .01 .01 .01 .01 .01 .01 .01 .01 .01
 .01 .01 .00 .00 .00 .00 .00 .00 .00 .00
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 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00

				DEV-2.0H1					
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

*** **

 *
 22 KK * BASA1 *
 *

28 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 *
 35 KK * BAS A2 * -3
 *

41 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 *
 48 KK * BASB-3 *
 *

54 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 *
 64 KK * BASB-5 *
 *

70 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH
 IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
 TIMINT .033 TIME INTERVAL IN HOURS

*** **

 *
 77 KK * BASC-1 *
 *

83 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL

```

                                DEV-2.0H1
IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
90 KK  *  BASD-2  *
*
*****

```

```

96 KO  OUTPUT CONTROL VARIABLES
        IPRNT      5  PRINT CONTROL
        IPLOT      0  PLOT CONTROL
        QSCAL      0. HYDROGRAPH PLOT SCALE
        IPNCH      0  PUNCH COMPUTED HYDROGRAPH
        IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
        ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
103 KK *  BASE1-  *
*
*****

```

2

```

109 KO  OUTPUT CONTROL VARIABLES
        IPRNT      5  PRINT CONTROL
        IPLOT      0  PLOT CONTROL
        QSCAL      0. HYDROGRAPH PLOT SCALE
        IPNCH      0  PUNCH COMPUTED HYDROGRAPH
        IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
        ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
116 KK *  BASE2  *
*
*****

```

```

123 KO  OUTPUT CONTROL VARIABLES
        IPRNT      5  PRINT CONTROL
        IPLOT      0  PLOT CONTROL
        QSCAL      0. HYDROGRAPH PLOT SCALE
        IPNCH      0  PUNCH COMPUTED HYDROGRAPH
        IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
        ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
124 KK *  COMBE  *
*
*****

```

```

127 KO  OUTPUT CONTROL VARIABLES
        IPRNT      5  PRINT CONTROL
        IPLOT      0  PLOT CONTROL
        QSCAL      0. HYDROGRAPH PLOT SCALE
        IPNCH      0  PUNCH COMPUTED HYDROGRAPH
        IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
        ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
        TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
137 KK *  BASE3  *
*

```

-1

```

143 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
150 KK  *  BASF1  *      -3
*      *
*****

```

```

156 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
163 KK  *  BASF2  *      -2
*      *
*****

```

```

169 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
176 KK  *  BASF3  *      -1
*      *
*****

```

```

182 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
189 KK  *  BASG   *      -1
*      *
*****

```

```

195 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **


```

*****
*      *
202 KK *  BASH1 *
*      *
*****

```

-1

```

209 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
222 KK *  BASH2- *
*      *
*****

```

2

```

228 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
235 KK *  BASI *
*      *
*****

```

```

241 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
251 KK *  BASJ *
*      *
*****

```

```

257 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
264 KK *  BASK *
*      *
*****

```

```

270 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

* *
277 KK * BASL *
* *

283 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
290 KK * BASM *
* *

296 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
303 KK * BASN *
* *

309 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
310 KK * 0 *
* *

317 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
318 KK * P *
* *

325 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
Page 12

TIMINT .033 TIME INTERVAL IN HOURS DEV-2.OH1

*** **

329 KK *****
* Q *

336 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

337 KK *****
* R *

344 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

1										
RUNOFF SUMMARY										
FLOW IN CUBIC FEET PER SECOND										
TIME IN HOURS, AREA IN SQUARE MILES										

DEV-2.OH1

+	HYDROGRAPH AT	E2	33.	3.27	4.	1.	1.	.05		
+	ROUTED TO	BASE2	0.	.00	0.	0.	0.	.05	2702.36	26.43
+	2 COMBINED AT	COMBE	26.	3.50	4.	1.	1.	.11		
+	6 COMBINED AT	CLEAR	127.	3.47	17.	4.	3.	.37		
+	HYDROGRAPH AT	E3	7.	3.17	1.	0.	0.	.01		
+	ROUTED TO	BASE3	0.	.00	0.	0.	0.	.01	2713.90	24.67
+	HYDROGRAPH AT	F1	23.	3.30	3.	1.	1.	.04		
+	ROUTED TO	BASF1	13.	3.53	2.	0.	0.	.04	2698.39	3.53
+	HYDROGRAPH AT	F2	10.	3.23	1.	0.	0.	.01		
+	ROUTED TO	BASF2	1.	4.13	0.	0.	0.	.01	2704.03	4.13
+	HYDROGRAPH AT	F3	9.	3.23	1.	0.	0.	.01		
+	ROUTED TO	BASF3	6.	3.37	1.	0.	0.	.01	2707.16	3.37
+	HYDROGRAPH AT	G	9.	3.30	1.	0.	0.	.02		
+	ROUTED TO	BASG	1.	5.00	0.	0.	0.	.02	2726.02	5.00
+	HYDROGRAPH AT	H1	34.	3.33	4.	1.	1.	.06		
+	ROUTED TO	BASH1	7.	4.03	1.	0.	0.	.06	2745.52	4.03
+	HYDROGRAPH AT	H2	35.	3.40	5.	1.	1.	.07		
+	ROUTED TO	BASH2-	32.	3.50	4.	1.	1.	.07	2774.51	3.50
+	ROUTED TO	BASH2-	19.	3.77	3.	1.	1.	.07	2787.32	3.77
+	HYDROGRAPH AT	I	16.	3.27	2.	0.	0.	.03		
+	ROUTED TO	BASI	2.	4.17	1.	0.	0.	.03	2758.08	4.17
+	8 COMBINED AT	CLEAR	30.	3.73	8.	2.	2.	.24		
+	HYDROGRAPH AT	J	21.	3.17	2.	0.	0.	.02		
+	ROUTED TO	BASJ	9.	3.40	1.	0.	0.	.02	2772.29	3.40
+	HYDROGRAPH AT	K	11.	3.10	1.	0.	0.	.01		
+	ROUTED TO	BASK	0.	.00	0.	0.	0.	.01	2739.79	27.97
+	HYDROGRAPH AT	L	9.	3.13	1.	0.	0.	.01		
+	ROUTED TO	BASL	2.	3.50	0.	0.	0.	.01	2725.06	3.50
+	HYDROGRAPH AT	M	7.	3.17	1.	0.	0.	.01		
+	ROUTED TO	BASM	1.	3.77	0.	0.	0.	.01	2716.05	3.77
+	HYDROGRAPH AT									

		N	9.	3.17	1.	DEV-2.0H1	0.	0.	.01		
+	ROUTED TO										
+		BASN	0.	.00	0.	0.	0.	0.	.01	2691.37	26.53
+	HYDROGRAPH AT	O	3.	3.13	0.	0.	0.	0.	.00		
+	HYDROGRAPH AT	P	6.	3.13	0.	0.	0.	0.	.01		
+	B COMBINED AT										
+		CLEAR	37.	3.73	10.	3.	2.		.31		
+	HYDROGRAPH AT	Q	3.	3.10	0.	0.	0.	0.	.00		
+	HYDROGRAPH AT	R	88.	3.27	9.	2.	2.		.14		

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION PEAK	INTERVAL TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1	STORM AREA (SQ MI) =								
A1 MANE	2.00	47.04	.01	198.45	.64	2.00	46.72	198.00	.64
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2879E+01 OUTFLOW= .2857E+01 BASIN STORAGE= .1235E-02 PERCENT ERROR= .7									
FOR STORM = 1	STORM AREA (SQ MI) =								
A2 MANE	2.04	25.25	.01	194.06	.70	2.00	25.20	194.00	.70
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1347E+01 OUTFLOW= .1341E+01 BASIN STORAGE= .3797E-03 PERCENT ERROR= .5									
FOR STORM = 1	STORM AREA (SQ MI) =								
B1 MANE	1.92	18.63	.01	195.16	.66	2.00	18.43	194.00	.66
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9606E+00 OUTFLOW= .9551E+00 BASIN STORAGE= .2359E-03 PERCENT ERROR= .5									
FOR STORM = 1	STORM AREA (SQ MI) =								
B2 MANE	2.00	26.14	.01	201.02	.69	2.00	26.01	202.00	.69
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1854E+01 OUTFLOW= .1842E+01 BASIN STORAGE= .1126E-02 PERCENT ERROR= .6									
FOR STORM = 1	STORM AREA (SQ MI) =								
C MANE	1.75	19.66	.01	195.10	.68	2.00	19.65	196.00	.68
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1100E+01 OUTFLOW= .1093E+01 BASIN STORAGE= .3456E-03 PERCENT ERROR= .6									
FOR STORM = 1	STORM AREA (SQ MI) =								
D MANE	1.62	21.12	.01	196.56	.72	2.00	21.07	198.00	.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1317E+01 OUTFLOW= .1311E+01 BASIN STORAGE= .6177E-03 PERCENT ERROR= .4									
FOR STORM = 1	STORM AREA (SQ MI) =								
E1 MANE	2.27	31.91	.01	202.20	.72	2.00	31.86	202.00	.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2362E+01 OUTFLOW= .2349E+01 BASIN STORAGE= .1516E-02 PERCENT ERROR= .5									
FOR STORM = 1	STORM AREA (SQ MI) =								
E2 MANE	2.00	33.11	.01	195.30	.72	2.00	32.69	196.00	.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1781E+01 OUTFLOW= .1774E+01 BASIN STORAGE= .4574E-03 PERCENT ERROR= .4									
FOR STORM = 1	STORM AREA (SQ MI) =								
E3 MANE	.86	7.09	.01	190.52	.72	2.00	7.08	190.00	.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3098E+00 OUTFLOW= .3086E+00 BASIN STORAGE= .4967E-04 PERCENT ERROR= .4									
FOR STORM = 1	STORM AREA (SQ MI) =								
F1 MANE	1.89	23.39	.01	198.26	.72	2.00	23.38	198.00	.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1394E+01 OUTFLOW= .1388E+01 BASIN STORAGE= .5221E-03 PERCENT ERROR= .4									
FOR STORM = 1	STORM AREA (SQ MI) =								
F2 MANE	1.59	10.47	.01	193.53	.72	2.00	10.40	194.00	.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5422E+00 OUTFLOW= .5395E+00 BASIN STORAGE= .1396E-03 PERCENT ERROR= .5									

DEV-2.OH1

FOR STORM = 1	STORM AREA (SQ MI) =	.01							
F3 MANE	1.03	9.22	193.01	.72	2.00	9.21	194.00	.72	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5035E+00 OUTFLOW= .5017E+00 BASIN STORAGE= .1687E-03 PERCENT ERROR= .3									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
G MANE	.86	9.19	198.87	.72	2.00	9.13	198.00	.72	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6196E+00 OUTFLOW= .6177E+00 BASIN STORAGE= .3650E-03 PERCENT ERROR= .3									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
H1 MANE	2.00	33.69	199.70	.72	2.00	33.56	200.00	.72	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2267E+01 OUTFLOW= .2254E+01 BASIN STORAGE= .1203E-02 PERCENT ERROR= .5									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
H2 MANE	2.16	35.31	204.39	.67	2.00	35.24	204.00	.67	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2600E+01 OUTFLOW= .2584E+01 BASIN STORAGE= .1757E-02 PERCENT ERROR= .6									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
I MANE	2.00	16.55	195.48	.70	2.00	16.48	196.00	.70	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9357E+00 OUTFLOW= .9298E+00 BASIN STORAGE= .2830E-03 PERCENT ERROR= .6									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
J MANE	.97	21.46	190.09	.70	2.00	21.44	190.00	.71	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9043E+00 OUTFLOW= .9018E+00 BASIN STORAGE= .1448E-03 PERCENT ERROR= .3									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
K MANE	.92	10.75	186.30	.72	2.00	10.58	186.00	.72	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3485E+00 OUTFLOW= .3479E+00 BASIN STORAGE= .1758E-04 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
L MANE	.70	9.15	188.53	.72	2.00	9.13	188.00	.72	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3485E+00 OUTFLOW= .3477E+00 BASIN STORAGE= .4296E-04 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
M MANE	1.28	7.32	189.98	.72	2.00	7.32	190.00	.72	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3098E+00 OUTFLOW= .3087E+00 BASIN STORAGE= .5202E-04 PERCENT ERROR= .3									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
N MANE	1.55	9.31	190.77	.72	2.00	9.23	190.00	.72	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3873E+00 OUTFLOW= .3863E+00 BASIN STORAGE= .4529E-04 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
O MANE	.46	2.56	187.54	.72	2.00	2.53	188.00	.72	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9295E-01 OUTFLOW= .9274E-01 BASIN STORAGE= .9717E-05 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
P MANE	.65	5.59	188.96	.72	2.00	5.52	188.00	.72	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2246E+00 OUTFLOW= .2239E+00 BASIN STORAGE= .3152E-04 PERCENT ERROR= .3									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
Q MANE	.37	2.78	186.06	.72	2.00	2.78	186.00	.72	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9295E-01 OUTFLOW= .9268E-01 BASIN STORAGE= .6752E-05 PERCENT ERROR= .3									
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
R MANE	1.97	88.00	197.39	.63	2.00	87.79	196.00	.63	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4732E+01 OUTFLOW= .4696E+01 BASIN STORAGE= .1127E-02 PERCENT ERROR= .7									

*** NORMAL END OF HEC-1 ***

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 14NOV06 TIME 17:51:27 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION. NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE:GREEN AND AMPT INFILTRATION, KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID HEC-1 MODEL FOR SERENO CANYON
2 ID 10-YEAR, 6-HOUR STORM
3 ID RAINFALL FROM NOAA ATLAS
4 ID SCS CURVE NUMBER SOIL LOSS PARAMETERS
5 ID KINEMATIC WAVE HYDROGRAPH ROUTING
6 ID DEVELOPED CONDITIONS
7 ID 10% IMPERVIOUS AREA USED FOR SITE
8 ID
9 ID PREPARED BY WOOD/PATEL, 1.10.2005
10 ID FILE NAME: DEV-10.DAT
11 ID
12 *DIAGRAM
13 IT 2 900
14 IO 5
15 JD .01 0.52 0.99 1.64 1.87 2.01 2.29
16 PH
17 KK A1
18 KM RUNOFF FROM SUB-BASIN A1
19 BA 0.084
20 LS 88 1.1
21 UK 260 .025 0.13 100
22 RK 3520 .023 .032 TRAP 12 5
23 *
24 KK BASA1
25 KM 29' WEIR AT HEADWALL
26 RS 1 STOR 0
27 SV 0 0.108 0.242 0.404 0.6 0.841 1.081
28 SE 2756 2757 2758 2759 2760 2761 2762
29 SQ 0 0 0 0 0 81 230
30 KO 22
31 KK A2
32 KM RUNOFF FROM SUB-BASIN A2
33 BA .036
34 LS 88 7.4
35 UK 235 .025 0.13 100
36 RK 2000 .02 .032 TRAP 8 5
37 *
38 KK BAS A2 -3
39 KM 14' WEIR AT OUTLET
40 RS 1 STOR 0
41 SV 0 0.059 0.135 0.228 0.34 0.474 0.608 0.742
42 SE 2733 2734 2735 2736 2737 2738 2739 2740
43 SQ 0 0 0 0 0 0 39 111
44 KO 22
45 *
46 KK B1
47 KM RUNOFF FROM SUB-BASIN B1
48 BA .027
49 LS 88 3.7
50 UK 200 .025 0.13 100
51 RK 2500 .028 .032 TRAP 8 5

```

1 HEC-1 INPUT PAGE 2

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
48 KK BASB-3
49 KM 11' WEIR AT HEADWALL
50 RS 1 STOR 0
51 SV 0 .089 0.2 .338 0.49 0.643
52 SE 2781 2782 2783 2784 2785 2786
53 SQ 0 0 0 0 0 31 87
54 KO 22

```

DEV-10.OH1

55 KK B2
56 KM RUNOFF FROM SUB-BASIN B2
57 BA .050
58 LS 88 6.7
59 UK 400 .025 0.13 100
60 RK 2340 .024 .032 TRAP 8 5
*

61 KK COMBB
62 KM COMBINE SECTIONS B1 AND B2
63 HC 2
*

64 KK BASB-5
65 KM 25' WEIR
66 RS 1 STOR 0
67 SV 0 0.146 0.333 0.565 0.843 1.121
68 SE 2742 2743 2744 2745 2746 2747
69 SQ 0 0 0 0 70 198
70 KO 22
*

71 KK C
72 KM RUNOFF FROM SUB-BASIN C
73 BA .03
74 LS 88 5.9
75 UK 250 .023 0.13 100
76 RK 1570 .017 .032 TRAP 5 5
*

77 KK BASC-1
78 KM 11' WEIR
79 RS 1 STOR 0
80 SV 0 0.107 0.236 0.388 0.566 0.745
81 SE 2779 2780 2781 2782 2783 2784
82 SQ 0 0 0 0 31 87
83 KO 22
*

84 KK D
85 KM RUNOFF FROM SUB-BASIN D
86 BA .034
87 LS 88 10
88 UK 300 .02 0.13 100
89 RK 1425 .014 .032 TRAP 6 5
*

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

90 KK BASD-2
91 KM 11' WEIR AT HEAD WALL
92 RS 1 STOR 0
93 SV 0 0.039 0.13 0.29 0.541 0.898
94 SE 2719 2720 2721 2722 2723 2724
95 SQ 0 0 0 0 31 87
96 KO 22
*

97 KK E1
98 KM RUNOFF FROM SUB-BASIN E1
99 BA .061
100 LS 88 10
101 UK 400 .02 0.13 100
102 RK 2050 .013 .032 TRAP 7 5
*

103 KK BASE1- 2
104 KM 19' WEIR AT HEAD WALL
105 RS 1 STOR 0
106 SV 0 0.147 0.334 0.56 0.826 1.135
107 SE 2695 2696 2697 2698 2699 2700
108 SQ 0 0 0 0 53 150
109 KO 22
*

110 KK E2
111 KM RUNOFF FROM SUB-BASIN E2
112 BA .046
113 LS 88 10
114 UK 200 .02 0.13 100
115 RK 2370 .013 .032 TRAP 10 5
*

116 KK BASE2
117 KM FLOW ROUTED THROUGH DETENTION BASIN E2-1
118 KM 2-42" PIPE CULVERT
119 RS 1 STOR 0
120 SV 0 0.094 0.211 0.379 0.641 1.026 1.546 1.861 2.22
121 SE 2696 2697 2698 2699 2700 2701 2702 2702.5 2703
122 SQ 0 0 0 0 0 0 0 0 180
123 KO 22
*

124 KK COMBE
125 KM COMBINE SECTIONS E1 AND E2
126 HC 2
127 KO 22
*

HEC-1 INPUT

PAGE 4

LINE * ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

128 KK CLEAR

129 KM CLEAR HYDROGRAPH STACK DEV-10.OH1
130 HC 6

131 KK E3
132 KM RUNOFF FROM SUB-BASIN E3
133 BA .008
134 LS 88 10
135 UK 175 .025 0.13 100
136 RK 750 .032 .032 TRAP 6 5

137 KK BASE3 -1
138 KM 10' WEIR AT HEAD WALL
139 RS 1 STOR 0
140 SV 0 .082 .188 .322 0.456
141 SE 2711 2712 2713 2714 2715
142 SQ 0 0 0 0 28
143 KO 22

144 KK F1
145 KM RUNOFF FROM SUB-BASIN F1
146 BA .036
147 LS 88 10
148 UK 225 .018 0.13 100
149 RK 2500 .012 .032 TRAP 7 5

150 KK BASF1 -3
151 KM 12' WEIR AT HEAD WALL
152 RS 1 STOR 0
153 SV 0 0.21 0.443 0.701 0.958
154 SE 2696 2697 2698 2699 2700
155 SQ 0 0 0 34 95
156 KO 22

157 KK F2
158 KM RUNOFF FROM SUB-BASIN F2
159 BA .014
160 LS 88 10
161 UK 200 .018 0.13 100
162 RK 1440 .028 .032 TRAP 6 5

163 KK BASF2 -2
164 KM 16' WEIR AT HEAD WALL
165 RS 1 STOR 0
166 SV 0 0.112 0.243 0.396 0.549
167 SE 2701 2702 2703 2704 2705
168 SQ 0 0 0 0 45
169 KO 22

170 KK F3
171 KM RUNOFF FROM SUB-BASIN F3
172 BA .013
173 LS 88 10
174 UK 225 .018 0.13 100
175 RK 850 .026 .032 TRAP 6 5

176 KK BASF3 -1
177 KM 14' WEIR AT HEAD WALL
178 RS 1 STOR 0
179 SV 0 0.038 0.095 0.172 0.274
180 SE 2704 2705 2706 2707 2708
181 SQ 0 0 0 0 39
182 KO 22

183 KK G
184 KM RUNOFF FROM SUB-BASIN G
185 BA 0.016
186 LS 88 10
187 UK 400 .025 0.13 100
188 RK 720 .022 .032 TRAP 4 5

189 KK BASG -1
190 KM 15' WEIR
191 RS 1 STOR 0
192 SV 0 0.094 0.209 0.347 0.512 0.71
193 SE 2722 2723 2724 2725 2726 2727
194 SQ 0 0 0 0 0 42
195 KO 22

196 KK H1
197 KM RUNOFF FROM SUB-BASIN H1
198 BA .059
199 LS 88 9.4
200 UK 375 .025 0.13 100
201 RK 2210 .019 .032 TRAP 8 5

202 KK BASH1 -1
203 KM FLOW ROUTED THROUGH DETENTION BASIN H1-1
204 KM 2-36" PIPE CULVERT
205 RS 1 STOR 0
206 SV 0 0.009 0.046 0.132 0.284 0.51 0.822 1.24 1.497 1.809
207 SE 2738 2739 2740 2741 2742 2743 2744 2745 2745.5 2746

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

208 SQ 0 0 0 0 DEV-10.OH1 0 0 0 0 170
 209 KO 22

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

210 KK H2
 211 KM RUNOFF FROM SUB-BASIN H2
 212 BA .072
 213 LS 88 4.78
 214 UK 400 .025 0.13 100
 215 RK 3480 .019 .032 TRAP 12 5

216 KK BASH2- 1
 217 KM 22' WEIR AT HEADWALL
 218 RS 1 STOR 0
 219 SV 0 0.15 0.318 0.507 0.718 0.929
 220 SE 0 2772 2773 2774 2775 2776
 221 SQ 0 0 0 62 174

222 KK BASH2- 2
 223 KM 21' WEIR AT HEADWALL
 224 RS 1 STOR 0
 225 SV 0 .165 .361 .590 .820 1.049
 226 SE 0 2785 2786 2787 2788 2789
 227 SQ 0 0 0 59 166
 228 KO 22

229 KK I
 230 KM RUNOFF FROM SUB-BASIN I
 231 BA .025
 232 LS 88 7.4
 233 UK 225 .02 0.13 100
 234 RK 2100 .026 .032 TRAP 10 5

235 KK BASI
 236 KM 10' WEIR
 237 RS 1 STOR 0
 238 SV 0 .186 .399 .640 0.881 1.122
 239 SE 2755 2756 2757 2758 2759 2760
 240 SQ 0 0 0 28 79
 241 KO 22

242 KK CLEAR
 243 KM CLEAR HYDROGRAPH STACK
 244 HC 8

245 KK J
 246 KM RUNOFF FROM SUB-BASIN J
 247 BA .024
 248 LS 88 7.9
 249 UK 145 .02 0.13 100
 250 RK 965 .026 .032 TRAP 5 5

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

251 KK BASJ
 252 KM 11' WEIR
 253 RS 1 STOR 0
 254 SV 0 .098 .217 .359 .528 0.698
 255 SE 2769 2770 2771 2772 2773 2774
 256 SQ 0 0 0 31 87
 257 KO 22

258 KK K
 259 KM RUNOFF FROM SUB-BASIN K
 260 BA .009
 261 LS 88 10
 262 UK 81 .025 0.13 100
 263 RK 715 .022 .032 TRAP 5 5

264 KK BASK
 265 KM 13' WEIR
 266 RS 1 STOR 0
 267 SV 0 .106 .232 .378 0.525
 268 SE 2737 2738 2739 2740 2741
 269 SQ 0 0 0 36
 270 KO 22

271 KK L
 272 KM RUNOFF FROM SUB-BASIN L
 273 BA .009
 274 LS 88 10
 275 UK 113 .018 0.13 100
 276 RK 450 .012 .032 TRAP 5 5

277 KK BASL
 278 KM 13' WEIR
 279 RS 1 STOR 0
 280 SV 0 0.041 0.106 0.2 0.326
 281 SE 2722 2723 2724 2725 2726
 282 SQ 0 0 0 36
 283 KO 22

284 KK M
285 KM RUNOFF FROM SUB-BASIN M
286 BA .008
287 LS 88 10
288 UK 137 .018 0.13 100
289 RK 750 .012 .032 TRAP 4 5
*

HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

290 KK BASIN
291 KM 7' WEIR
292 RS 1 STOR 0
293 SV 0 0.056 0.124 0.205 0.301 0.413
294 SE 2713 2714 2715 2716 2717 2718
295 SQ 0 0 0 0 20 55
296 KO 22
*

297 KK N
298 KM RUNOFF FROM SUB-BASIN N
299 BA .01
300 LS 88 10
301 UK 117 .018 0.13 100
302 RK 1036 .012 .032 TRAP 5 5
*

303 KK BASIN
304 KM 13' WEIR
305 RS 1 STOR 0
306 SV 0 0.083 0.191 0.326 0.49 0.686
307 SE 2688 2689 2690 2691 2692 2693
308 SQ 0 0 0 0 0 36
309 KO 22
*

310 KK O
311 KM RUNOFF FROM SUB-BASIN O
312 KM Area of Basin subtrating the area to be retained for 100 year event
313 BA 0.0024 9
314 LS 88 10
315 UK 108 .02 0.13 100
316 RK 174 .014 .032 TRAP 4 5
317 KO 22

318 KK P
319 KM RUNOFF FROM SUB-BASIN P
320 KM Area of Basin subtrating the area to be retained for 100 year event
321 BA .0058 8
322 LS 88 10
323 UK 135 .02 0.13 100
324 RK 354 .014 .032 TRAP 5 5
325 KO 22

326 KK CLEAR
327 KM CLEAR HYDROGRAPH STACK
328 HC 8

329 KK Q
330 KM RUNOFF FROM SUB-BASIN Q
331 KM Area of Basin subtrating the area to be retained for 100 year event
332 BA .0024 8
333 LS 88 10
334 UK 86 .02 0.13 100
HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

335 RK 160 .014 .032 TRAP 4 5
336 KO 22

337 KK R
338 KM RUNOFF FROM SUB-BASIN R
339 KM Area of Basin subtrating the area to be retained for 100 year event
340 BA .14 8
341 LS 88 .14
342 UK 180 .025 0.13 100
343 RK 4300 .023 .032 TRAP 15 5
344 KO 22
345 ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
LINE

(V) ROUTING (--->) DIVERSION OR PUMP FLOW
(.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

NO.

16 A1
V
22 BASA1

29 A2
V
35 BAS A2

42 B1
V
V

48
55
61
64
71
77
84
90
97
103
110
116
124
128
131
137
144
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202
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242
245
251
258
264

271	.	.	.	.	L V V				
277	.	.	.	.	BASL				
284	.	.	.	.		M V V			
290	.	.	.	.		BASM			
297	.	.	.	.			N V V		
303	.	.	.	.			BASN		
310	.	.	.	.				O	
318	.	.	.	.					P
326	CLEAR.	.	.	.					
329	.	Q	.	.					
337	.	.	R	.					

```
*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 14NOV06 TIME 17:51:27
*****
```

★

★ U.S. ARMY CORPS OF ENGINEERS ★

★ HYDROLOGIC ENGINEERING CENTER ★

★ 609 SECOND STREET ★

★ DAVIS, CALIFORNIA 95616 ★

★ (916) 756-1104 ★

PREPARED BY WOOD/PATEL, 1.10.2005
FILE NAME: DEV-10.DAT

ENGLISH UNITS	
DRAINAGE AREA	SQUARE MILES
PRECIPITATION DEPTH	INCHES
LENGTH, ELEVATION	FEET
FLOW	CUBIC FEET PER SECOND
STORAGE VOLUME	ACRE-FeET
SURFACE AREA	ACRES
TEMPERATURE	DEGREES FAHRENHEIT

[illegible]

				DEV-10.OH1						
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

*** **

```

*****
*
22 KK  *   BASA1  *
*   *
*****

```

```

28 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
35 KK  *   BAS A2  *   -3
*   *
*****

```

```

41 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
48 KK  *   BASB-3  *
*   *
*****

```

```

54 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
64 KK  *   BASB-5  *
*   *
*****

```

```

70 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
77 KK  *   BASC-1  *
*   *
*****

```

```

83 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL

```

```

IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
90 KK  *  BASD-2  *
*      *
*****

```

```

96 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
103 KK *  BASE1-  *      2
*      *
*****

```

```

109 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
116 KK *  BASE2  *
*      *
*****

```

```

123 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
124 KK *  COMBE  *
*      *
*****

```

```

127 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
137 KK *  BASE3  *      -1
*      *

```

```

143 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

150 KK      *
            * BASF1 *
            *

```

-3

```

156 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

163 KK      *
            * BASF2 *
            *

```

-2

```

169 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

176 KK      *
            * BASF3 *
            *

```

-1

```

182 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

189 KK      *
            * BASG *
            *

```

-1

```

195 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **


```

*****
*      *
* 202 KK  BASH1  *      -1
*      *
*****

```

```

209 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
* 222 KK  BASH2- *      2
*      *
*****

```

```

228 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
* 235 KK  BASI   *
*      *
*****

```

```

241 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
* 251 KK  BASJ   *
*      *
*****

```

```

257 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*      *
* 264 KK  BASK   *
*      *
*****

```

```

270 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

* *
277 KK * BASL *
* *

283 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
290 KK * BASM *
* *

296 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

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* *
303 KK * BASN *
* *

309 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
310 KK * O *
* *

317 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

* *
318 KK * P *
* *

325 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED

TIMINT .033 TIME INTERVAL IN HOURS DEV-10.OH1

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* *
329 KK * Q *
* *

336 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

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* *
337 KK * R *
* *

344 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

1

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	A1	113.	3.23	11.	3.	2.	.08		
+	ROUTED TO	BASA1	109.	3.27	10.	2.	2.	.08	2761.19	3.27
+	HYDROGRAPH AT	A2	57.	3.17	5.	1.	1.	.04		
+	ROUTED TO	BAS A2	48.	3.23	4.	1.	1.	.04	2739.13	3.23
+	HYDROGRAPH AT	B1	42.	3.17	4.	1.	1.	.03		
+	ROUTED TO	BASB-3	31.	3.27	3.	1.	1.	.03	2784.99	3.27
+	HYDROGRAPH AT	B2	62.	3.23	7.	2.	1.	.05		
+	2 COMBINED AT	COMBB	92.	3.27	10.	2.	2.	.08		
+	ROUTED TO	BASB-5	85.	3.30	9.	2.	2.	.08	2746.12	3.30
+	HYDROGRAPH AT	C	46.	3.17	4.	1.	1.	.03		
+	ROUTED TO	BASC-1	33.	3.30	3.	1.	1.	.03	2783.04	3.30
+	HYDROGRAPH AT	D	48.	3.20	5.	1.	1.	.03		
+	ROUTED TO	BASD-2	37.	3.30	4.	1.	1.	.03	2723.11	3.30
+	HYDROGRAPH AT	E1	74.	3.27	8.	2.	2.	.06		
+	ROUTED TO	BASE1-	68.	3.33	7.	2.	1.	.06	2699.15	3.33

DEV-10.0H1

+	HYDROGRAPH AT	E2	71.	3.20	6.	2.	1.	.05		
+	ROUTED TO	BASE2	21.	3.57	3.	1.	1.	.05	2702.56	3.57
+	2 COMBINED AT	COMBE	68.	3.33	10.	3.	2.	.11		
+	6 COMBINED AT	CLEAR	364.	3.30	40.	10.	8.	.37		
+	HYDROGRAPH AT	E3	15.	3.13	1.	0.	0.	.01		
+	ROUTED TO	BASE3	4.	3.47	0.	0.	0.	.01	2714.13	3.47
+	HYDROGRAPH AT	F1	52.	3.20	5.	1.	1.	.04		
+	ROUTED TO	BASF1	37.	3.33	4.	1.	1.	.04	2699.05	3.33
+	HYDROGRAPH AT	F2	23.	3.17	2.	0.	0.	.01		
+	ROUTED TO	BASF2	11.	3.33	1.	0.	0.	.01	2704.25	3.33
+	HYDROGRAPH AT	F3	21.	3.17	2.	0.	0.	.01		
+	ROUTED TO	BASF3	18.	3.20	1.	0.	0.	.01	2707.47	3.20
+	HYDROGRAPH AT	G	21.	3.20	2.	1.	0.	.02		
+	ROUTED TO	BASG	9.	3.47	1.	0.	0.	.02	2726.22	3.47
+	HYDROGRAPH AT	H1	78.	3.23	8.	2.	2.	.06		
+	ROUTED TO	BASH1	54.	3.37	5.	1.	1.	.06	2745.66	3.37
+	HYDROGRAPH AT	H2	84.	3.27	10.	2.	2.	.07		
+	ROUTED TO	BASH2-	81.	3.30	9.	2.	2.	.07	2775.17	3.30
+	ROUTED TO	BASH2-	66.	3.43	7.	2.	1.	.07	2788.06	3.43
+	HYDROGRAPH AT	I	38.	3.20	3.	1.	1.	.03		
+	ROUTED TO	BASI	16.	3.43	2.	1.	0.	.03	2758.57	3.43
+	8 COMBINED AT	CLEAR	193.	3.40	23.	6.	5.	.24		
+	HYDROGRAPH AT	J	45.	3.13	3.	1.	1.	.02		
+	ROUTED TO	BASJ	30.	3.20	3.	1.	1.	.02	2772.96	3.20
+	HYDROGRAPH AT	K	20.	3.10	1.	0.	0.	.01		
+	ROUTED TO	BASK	4.	3.40	1.	0.	0.	.01	2740.11	3.40
+	HYDROGRAPH AT	L	19.	3.10	1.	0.	0.	.01		
+	ROUTED TO	BASL	11.	3.20	1.	0.	0.	.01	2725.31	3.20
+	HYDROGRAPH AT	M	15.	3.13	1.	0.	0.	.01		
+	ROUTED TO	BASM	7.	3.30	1.	0.	0.	.01	2716.36	3.30
+	HYDROGRAPH AT									

		N	19.	3.13	1.	DEV-10.OH1	0.	0.	.01
ROUTED TO	BASN	2.	3.70	0.	0.	0.	0.	.01	2692.06 3.70
HYDROGRAPH AT	O	5.	3.10	0.	0.	0.	0.	.00	
HYDROGRAPH AT	P	12.	3.10	1.	0.	0.	0.	.01	
8 COMBINED AT	CLEAR	229.	3.40	29.	7.	6.	6.	.31	
HYDROGRAPH AT	Q	5.	3.07	0.	0.	0.	0.	.00	
HYDROGRAPH AT	R	209.	3.20	18.	5.	4.	4.	.14	

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION INTERVAL PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1	STORM AREA (SQ MI) =								
A1 MANE	2.00	115.18	.01	193.37	1.21	2.00	113.02	194.00	1.21

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5445E+01 OUTFLOW= .5413E+01 BASIN STORAGE= .1061E-02 PERCENT ERROR= .6

FOR STORM = 1	STORM AREA (SQ MI) =								
A2 MANE	1.68	57.43	.01	190.51	1.28	2.00	56.73	190.00	1.28

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2465E+01 OUTFLOW= .2458E+01 BASIN STORAGE= .3728E-03 PERCENT ERROR= .3

FOR STORM = 1	STORM AREA (SQ MI) =								
B1 MANE	1.87	42.57	.01	190.13	1.24	2.00	42.28	190.00	1.24

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1791E+01 OUTFLOW= .1781E+01 BASIN STORAGE= .2570E-03 PERCENT ERROR= .6

FOR STORM = 1	STORM AREA (SQ MI) =								
B2 MANE	1.70	62.30	.01	195.09	1.27	2.00	62.22	194.00	1.27

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3403E+01 OUTFLOW= .3386E+01 BASIN STORAGE= .1074E-02 PERCENT ERROR= .5

FOR STORM = 1	STORM AREA (SQ MI) =								
C MANE	1.43	45.89	.01	190.10	1.26	2.00	45.76	190.00	1.26

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2028E+01 OUTFLOW= .2021E+01 BASIN STORAGE= .3612E-03 PERCENT ERROR= .4

FOR STORM = 1	STORM AREA (SQ MI) =								
D MANE	1.39	48.14	.01	191.54	1.31	2.00	48.10	192.00	1.31

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2379E+01 OUTFLOW= .2369E+01 BASIN STORAGE= .6044E-03 PERCENT ERROR= .4

FOR STORM = 1	STORM AREA (SQ MI) =								
E1 MANE	1.75	74.37	.01	195.04	1.31	2.00	73.87	196.00	1.31

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4269E+01 OUTFLOW= .4253E+01 BASIN STORAGE= .1659E-02 PERCENT ERROR= .3

FOR STORM = 1	STORM AREA (SQ MI) =								
E2 MANE	2.00	72.17	.01	190.70	1.30	2.00	70.61	192.00	1.30

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3219E+01 OUTFLOW= .3199E+01 BASIN STORAGE= .5220E-03 PERCENT ERROR= .6

FOR STORM = 1	STORM AREA (SQ MI) =								
E3 MANE	.72	15.34	.01	187.59	1.31	2.00	15.08	188.00	1.31

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5598E+00 OUTFLOW= .5589E+00 BASIN STORAGE= .5716E-04 PERCENT ERROR= .2

FOR STORM = 1	STORM AREA (SQ MI) =								
F1 MANE	1.91	52.45	.01	192.51	1.30	2.00	51.64	192.00	1.30

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2519E+01 OUTFLOW= .2500E+01 BASIN STORAGE= .5312E-03 PERCENT ERROR= .8

FOR STORM = 1	STORM AREA (SQ MI) =								
F2 MANE	1.30	23.06	.01	189.57	1.31	2.00	22.91	190.00	1.31

CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9797E+00 OUTFLOW= .9755E+00 BASIN STORAGE= .1414E-03 PERCENT ERROR= .4

DEV-10.OH1

STORM	AREA (SQ MI)	INFL	EXCESS	OUTFLOW	BASIN STORAGE	PERCENT ERROR
FOR STORM = 1 F3 MANE	.78 20.87	.01 189.80	1.30	2.00	20.72 190.00	1.30
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9098E+00 OUTFLOW= .9034E+00 BASIN STORAGE= .1756E-03 PERCENT ERROR= .7						
FOR STORM = 1 G MANE	.84 20.91	.01 191.73	1.30	2.00	20.91 192.00	1.30
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1120E+01 OUTFLOW= .1113E+01 BASIN STORAGE= .3816E-03 PERCENT ERROR= .5						
FOR STORM = 1 H1 MANE	1.73 77.78	.01 193.87	1.30	2.00	77.56 194.00	1.30
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4108E+01 OUTFLOW= .4096E+01 BASIN STORAGE= .1136E-02 PERCENT ERROR= .3						
FOR STORM = 1 H2 MANE	2.01 84.00	.01 195.55	1.25	2.00	83.97 196.00	1.25
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4821E+01 OUTFLOW= .4796E+01 BASIN STORAGE= .1696E-02 PERCENT ERROR= .5						
FOR STORM = 1 I MANE	1.85 38.26	.01 191.32	1.28	2.00	37.58 192.00	1.28
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1712E+01 OUTFLOW= .1705E+01 BASIN STORAGE= .3085E-03 PERCENT ERROR= .4						
FOR STORM = 1 J MANE	.84 45.84	.01 186.87	1.29	2.00	45.03 188.00	1.29
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1650E+01 OUTFLOW= .1648E+01 BASIN STORAGE= .1536E-03 PERCENT ERROR= .1						
FOR STORM = 1 K MANE	.81 20.79	.01 184.71	1.31	2.00	20.13 186.00	1.31
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6298E+00 OUTFLOW= .6293E+00 BASIN STORAGE= .1887E-04 PERCENT ERROR= .1						
FOR STORM = 1 L MANE	.69 18.68	.01 185.87	1.31	2.00	18.66 186.00	1.31
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6298E+00 OUTFLOW= .6292E+00 BASIN STORAGE= .4243E-04 PERCENT ERROR= .1						
FOR STORM = 1 M MANE	.98 15.38	.01 187.16	1.31	2.00	15.23 188.00	1.31
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5598E+00 OUTFLOW= .5583E+00 BASIN STORAGE= .4692E-04 PERCENT ERROR= .3						
FOR STORM = 1 N MANE	1.38 19.77	.01 187.36	1.31	2.00	19.48 188.00	1.31
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .6998E+00 OUTFLOW= .6992E+00 BASIN STORAGE= .5151E-04 PERCENT ERROR= .1						
FOR STORM = 1 O MANE	.39 5.21	.01 185.54	1.31	2.00	5.10 186.00	1.31
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1680E+00 OUTFLOW= .1676E+00 BASIN STORAGE= .8995E-05 PERCENT ERROR= .2						
FOR STORM = 1 P MANE	.58 11.76	.01 186.23	1.31	2.00	11.73 186.00	1.31
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4059E+00 OUTFLOW= .4052E+00 BASIN STORAGE= .3030E-04 PERCENT ERROR= .2						
FOR STORM = 1 Q MANE	.25 5.51	.01 184.71	1.31	2.00	5.42 184.00	1.31
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1680E+00 OUTFLOW= .1676E+00 BASIN STORAGE= .6537E-05 PERCENT ERROR= .2						
FOR STORM = 1 R MANE	1.84 210.47	.01 191.74	1.20	2.00	208.88 192.00	1.20
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .8998E+01 OUTFLOW= .8958E+01 BASIN STORAGE= .1237E-02 PERCENT ERROR= .4						

*** NORMAL END OF HEC-1 ***

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
* RUN DATE 14NOV06 TIME 17:50:45
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*****

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION. NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID HEC-1 MODEL FOR SERENO CANYON
2 ID 100-YEAR, 6-HOUR STORM
3 ID RAINFALL FROM NOAA ATLAS
4 ID SCS CURVE NUMBER SOIL LOSS PARAMETERS
5 ID KINEMATIC WAVE HYDROGRAPH ROUTING
6 ID DEVELOPED CONDITIONS
7 ID 10% IMPERVIOUS AREA USED FOR SITE
8 ID
9 ID PREPARED BY WOOD/PATEL, 1.10.2005
10 ID FILE NAME: DEV-100.DAT
11 ID
12 IT 2 900
13 IO 5
14 JD .01
15 PH .73 1.43 2.42 2.74 2.96 3.37
16 KK A1
17 KM RUNOFF FROM SUB-BASIN A1
18 BA 0.084
19 LS 88 1.1
20 UK 260 .025 0.13 100
21 RK 3520 .023 .032 TRAP 12 5
22 KK BASA1
23 KM 29' WEIR AT HEADWALL
24 RS 1 STOR 0
25 SV 0 0.108 0.242 0.404 0.6 0.841 1.081
26 SE 2756 2757 2758 2759 2760 2761 2762
27 SQ 0 0 0 0 0 81 230
28 KO 22
29 KK A2
30 KM RUNOFF FROM SUB-BASIN A2
31 BA .036
32 LS 88 7.4
33 UK 235 .025 0.13 100
34 RK 2000 .02 .032 TRAP 8 5
35 KK BAS A2 -3
36 KM 14' WEIR AT OUTLET
37 RS 1 STOR 0
38 SV 0 0.059 0.135 0.228 0.34 0.474 0.608 0.742
39 SE 2733 2734 2735 2736 2737 2738 2739 2740
40 SQ 0 0 0 0 0 0 39 111
41 KO 22
42 KK B1
43 KM RUNOFF FROM SUB-BASIN B1
44 BA .027
45 LS 88 3.7
46 UK 200 .025 0.13 100
47 RK 2500 .028 .032 TRAP 8 5

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PAGE 2

1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
48 KK BASB-3
49 KM 11' WEIR AT HEADWALL
50 RS 1 STOR 0
51 SV 0 .089 0.2 .338 0.49 0.643
52 SE 2781 2782 2783 2784 2785 2786
53 SQ 0 0 0 0 31 87
54 KO 22

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DEV-100.OH1

55 KK B2
56 KM RUNOFF FROM SUB-BASIN B2
57 BA .050
58 LS 88 6.7
59 UK 400 .025 0.13 100
60 RK 2340 .024 .032 TRAP 8 5
*

61 KK COMBB
62 KM COMBINE SECTIONS B1 AND B2
63 HC 2
*

64 KK BASB-5
65 KM 25' WEIR
66 RS 1 STOR 0
67 SV 0 0.146 0.333 0.565 0.843 1.121
68 SE 2742 2743 2744 2745 2746 2747
69 SQ 0 0 0 0 70 198
70 KO 22
*

71 KK C
72 KM RUNOFF FROM SUB-BASIN C
73 BA .03
74 LS 88 5.9
75 UK 250 .023 0.13 100
76 RK 1570 .017 .032 TRAP 5 5
*

77 KK BASC-1
78 KM 11' WEIR
79 RS 1 STOR 0
80 SV 0 .107 .236 .388 0.566 0.745
81 SE 2779 2780 2781 2782 2783 2784
82 SQ 0 0 0 0 31 87
83 KO 22
*

84 KK D
85 KM RUNOFF FROM SUB-BASIN D
86 BA .034
87 LS 88 10
88 UK 300 .02 0.13 100
89 RK 1425 .014 .032 TRAP 6 5
*

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

90 KK BASD-2
91 KM 11' WEIR AT HEAD WALL
92 RS 1 STOR 0
93 SV 0 0.039 0.13 0.29 0.541 0.898
94 SE 2719 2720 2721 2722 2723 2724
95 SQ 0 0 0 0 31 87
96 KO 22
*

97 KK E1
98 KM RUNOFF FROM SUB-BASIN E1
99 BA .061
100 LS 88 10
101 UK 400 .02 0.13 100
102 RK 2050 .013 .032 TRAP 7 5
*

103 KK BASE1-2
104 KM 19' WEIR AT HEAD WALL
105 RS 1 STOR 0
106 SV 0 0.147 0.334 0.56 0.826 1.135
107 SE 2695 2696 2697 2698 2699 2700
108 SQ 0 0 0 0 53 150
109 KO 22
*

110 KK E2
111 KM RUNOFF FROM SUB-BASIN E2
112 BA .046
113 LS 88 10
114 UK 200 .02 0.13 100
115 RK 2370 .013 .032 TRAP 10 5
*

116 KK BASE2
117 KM FLOW ROUTED THROUGH DETENTION BASIN E2-1
118 KM 2-42" PIPE CULVERT
119 RS 1 STOR 0
120 SV 0 0.094 0.211 0.379 0.641 1.026 1.546 1.861 2.22
121 SE 2696 2697 2698 2699 2700 2701 2702 2702.5 2703
122 SQ 0 0 0 0 0 0 0 0 180
123 KO 22
*

124 KK COMBE
125 KM COMBINE SECTIONS E1 AND E2
126 HC 2
127 KO 22
*

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

128 KK CLEAR

129
130

KM CLEAR HYDROGRAPH STACK
HC 6
*

DEV-100.OH1

131
132
133
134
135
136

KK E3
KM RUNOFF FROM SUB-BASIN E3
BA .008
LS 88 10
UK 175 .025 0.13 100
RK 750 .032 .032
*

TRAP 6 5

137
138
139
140
141
142
143

KK BASE3 -1
KM 10' WEIR AT HEAD WALL
RS 1 STOR 0
SV 0 .082 .188 .322 0.456
SE 2711 2712 2713 2714 2715
SQ 0 0 0 0 28
KO 22
*

144
145
146
147
148
149

KK F1
KM RUNOFF FROM SUB-BASIN F1
BA .036
LS 88 10
UK 225 .018 0.13 100
RK 2500 .012 .032
*

TRAP 7 5

150
151
152
153
154
155
156

KK BASF1 -3
KM 12' WEIR AT HEAD WALL
RS 1 STOR 0
SV 0 0.21 0.443 0.701 0.958
SE 2696 2697 2698 2699 2700
SQ 0 0 0 34 95
KO 22
*

157
158
159
160
161
162

KK F2
KM RUNOFF FROM SUB-BASIN F2
BA .014
LS 88 10
UK 200 .018 0.13 100
RK 1440 .028 .032
*

TRAP 6 5

163
164
165
166
167
168
169

KK BASF2 -2
KM 16' WEIR AT HEAD WALL
RS 1 STOR 0
SV 0 0.112 0.243 0.396 0.549
SE 2701 2702 2703 2704 2705
SQ 0 0 0 0 45
KO 22
*

1

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

170
171
172
173
174
175

KK F3
KM RUNOFF FROM SUB-BASIN F3
BA .013
LS 88 10
UK 225 .018 0.13 100
RK 850 .026 .032
*

TRAP 6 5

176
177
178
179
180
181
182

KK BASF3 -1
KM 14' WEIR AT HEAD WALL
RS 1 STOR 0
SV 0 0.038 0.095 0.172 0.274
SE 2704 2705 2706 2707 2708
SQ 0 0 0 0 39
KO 22
*

183
184
185
186
187
188

KK G
KM RUNOFF FROM SUB-BASIN G
BA 0.016
LS 88 10
UK 400 .025 0.13 100
RK 720 .022 .032
*

TRAP 4 5

189
190
191
192
193
194
195

KK BASG -1
KM 15' WEIR
RS 1 STOR 0
SV 0 0.094 0.209 0.347 0.512 0.71
SE 2722 2723 2724 2725 2726 2727
SQ 0 0 0 0 0 42
KO 22
*

196
197
198
199
200
201

KK H1
KM RUNOFF FROM SUB-BASIN H1
BA .059
LS 88 9.4
UK 375 .025 0.13 100
RK 2210 .019 .032
*

TRAP 8 5

202
203
204
205
206
207

KK BASH1 -1
KM FLOW ROUTED THROUGH DETENTION BASIN H1-1
KM 2-36" PIPE CULVERT
RS 1 STOR 0
SV 0 0.009 0.046 0.132 0.284 0.51 0.822 1.24 1.497 1.809
SE 2738 2739 2740 2741 2742 2743 2744 2745 2745.5 2746

Page 3

208 SQ 0 0 0 0 DEV-100.OH1 0 0 0 0 170
209 KO 22

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

210 KK H2
211 KM RUNOFF FROM SUB-BASIN H2
212 BA .072
213 LS 88 4.78
214 UK 400 .025 0.13 100
215 RK 3480 .019 .032 TRAP 12 5

216 KK BASH2- 1
217 KM 22' WEIR AT HEADWALL
218 RS 1 STOR 0
219 SV 0 0.15 0.318 0.507 0.718 0.929
220 SE 0 2772 2773 2774 2775 2776
221 SQ 0 0 0 0 62 174

222 KK BASH2- 2
223 KM 21' WEIR AT HEADWALL
224 RS 1 STOR 0
225 SV 0 .165 .361 .590 .820 1.049
226 SE 0 2785 2786 2787 2788 2789
227 SQ 0 0 0 0 59 166
228 KO 22

229 KK I
230 KM RUNOFF FROM SUB-BASIN I
231 BA .025
232 LS 88 7.4
233 UK 225 .02 0.13 100
234 RK 2100 .026 .032 TRAP 10 5

235 KK BASI
236 KM 10' WEIR
237 RS 1 STOR 0
238 SV 0 .186 .399 .640 0.881 1.122
239 SE 2755 2756 2757 2758 2759 2760
240 SQ 0 0 0 0 28 79
241 KO 22

242 KK CLEAR
243 KM CLEAR HYDROGRAPH STACK
244 HC 8

245 KK J
246 KM RUNOFF FROM SUB-BASIN J
247 BA .024
248 LS 88 7.9
249 UK 145 .02 0.13 100
250 RK 965 .026 .032 TRAP 5 5

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

251 KK BASJ
252 KM 11' WEIR
253 RS 1 STOR 0
254 SV 0 .098 .217 .359 .528 0.698
255 SE 2769 2770 2771 2772 2773 2774
256 SQ 0 0 0 0 31 87
257 KO 22

258 KK K
259 KM RUNOFF FROM SUB-BASIN K
260 BA .009
261 LS 88 10
262 UK 81 .025 0.13 100
263 RK 715 .022 .032 TRAP 5 5

264 KK BASK
265 KM 13' WEIR
266 RS 1 STOR 0
267 SV 0 .106 .232 .378 0.525
268 SE 2737 2738 2739 2740 2741
269 SQ 0 0 0 0 36
270 KO 22

271 KK L
272 KM RUNOFF FROM SUB-BASIN L
273 BA .009
274 LS 88 10
275 UK 113 .018 0.13 100
276 RK 450 .012 .032 TRAP 5 5

277 KK BASL
278 KM 13' WEIR
279 RS 1 STOR 0
280 SV 0 0.041 0.106 0.2 0.326
281 SE 2722 2723 2724 2725 2726
282 SQ 0 0 0 0 36
283 KO 22

284 KK M
285 KM RUNOFF FROM SUB-BASIN M
286 BA .008
287 LS 88 10
288 UK 137 .018 0.13 100
289 RK 750 .012 .032 TRAP 4 5
*

HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

290 KK BASM
291 KM 7' WEIR
292 RS 1 STOR 0
293 SV 0 0.056 0.124 0.205 0.301 0.413
294 SE 2713 2714 2715 2716 2717 2718
295 SQ 0 0 0 0 20 55
296 KO 22
*

297 KK N
298 KM RUNOFF FROM SUB-BASIN N
299 BA .01
300 LS 88 10
301 UK 117 .018 0.13 100
302 RK 1036 .012 .032 TRAP 5 5
*

303 KK BASN
304 KM 13' WEIR
305 RS 1 STOR 0
306 SV 0 0.083 0.191 0.326 0.49 0.686
307 SE 2688 2689 2690 2691 2692 2693
308 SQ 0 0 0 0 0 36
309 KO 22
*

310 KK O
311 KM RUNOFF FROM SUB-BASIN O
312 KM Area of Basin subtrating the area to be retained for 100 year event
313 BA 0.0024 9
314 LS 88 10
315 UK 108 .02 0.13 100
316 RK 174 .014 .032 TRAP 4 5
317 KO 22

318 KK P
319 KM RUNOFF FROM SUB-BASIN P
320 KM Area of Basin subtrating the area to be retained for 100 year event
321 BA .0058
322 LS 88 10
323 UK 135 .02 0.13 100
324 RK 354 .014 .032 TRAP 5 5
325 KO 22

326 KK CLEAR
327 KM CLEAR HYDROGRAPH STACK
328 HC 8

329 KK Q
330 KM RUNOFF FROM SUB-BASIN Q
331 KM Area of Basin subtrating the area to be retained for 100 year event
332 BA .0024
333 LS 88 10
334 UK 86 .02 0.13 100
HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

335 RK 160 .014 .032 TRAP 4 5
336 KO 22

337 KK R
338 KM RUNOFF FROM SUB-BASIN R
339 KM Area of Basin subtrating the area to be retained for 100 year event
340 BA .14 8
341 LS 88 .14
342 UK 180 .025 0.13 100
343 RK 4300 .023 .032 TRAP 15 5
344 KO 22
345 ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

16 A1
V
22 BASA1
.
.
29 A2
V
35 BAS A2
.
.
42 B1
V
V

48 BASB-3
55 B2
61 COMBB
64 V
71 BASB-5
77 C
84 V
90 V
97 BASC-1
103 D
110 V
116 BASD-2
124 E1
128 V
131 BASE1-
137 E2
144 V
150 BASE2
157 COMBE
163 CLEAR
170 E3
176 V
183 BASE3
189 F1
196 V
202 BASF1
210 F2
216 V
222 BASF2
229 F3
235 V
242 BASF3
245 G
251 V
258 BASG
264 H1
271 V
278 BASH1
285 H2
292 V
299 BASH2-
306 V
313 BASH2-
320 I
327 V
334 BASI
341 CLEAR
348 J
355 V
362 BASJ
369 K
376 V
383 BASK

271	.	.	.	.	L V V			
277	.	.	.	.	BASL			
284	.	.	.	.		M V V		
290	.	.	.	.		BASM		
297	.	.	.	.			N V V	
303	.	.	.	.			BASN	
310	.	.	.	.				O
318	.	.	.	.				.
326	CLEAR.....	.	.	.	.	.	.	.
329	.	Q	.	.	.	.	.	.
337	.	.	R	.	.	.	.	.

```
*****
*                                     *
*   FLOOD HYDROGRAPH PACKAGE   (HEC-1)   *
*           JUN 1998               *
*         VERSION 4.1              *
*                                     *
*   RUN DATE   14NOV06   TIME  17:50:45   *
*                                     *
*****
```

U.S. ARMY CORPS OF ENGINEERS
HYDROLOGIC ENGINEERING CENTER
609 SECOND STREET
DAVIS, CALIFORNIA 95616
(916) 756-1104

PREPARED BY WOOD/PATEL, 1.10.2005
FILE NAME: DEV-100.DAT

ENGLISH UNITS	
DRAINAGE AREA	SQUARE MILES
PRECIPITATION DEPTH	INCHES
LENGTH, ELEVATION	FEET
FLOW	CUBIC FEET PER SECOND
STORAGE VOLUME	ACRE-Feet
SURFACE AREA	ACRES
TEMPERATURE	DEGREES FAHRENHEIT

[illegible]

				DEV-100.OH1						
.01	.01	.01	.01	.01	.01	.01	.01	.00	.00	
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	

*** **

*
22 KK * BASA1 *
*

28 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

*
35 KK * BAS A2 * -3
*

41 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

*
48 KK * BASB-3 *
*

54 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

*
64 KK * BASB-5 *
*

70 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

*
77 KK * BASC-1 *
*

83 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL


```

IPLOT      0  PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0  PUNCH COMPUTED HYDROGRAPH
IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
90 KK  *   BASD-2  *
*
*****

```

```

96 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
103 KK *   BASE1-  *      2
*
*****

```

```

109 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
116 KK *   BASE2  *
*
*****

```

```

123 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
124 KK *   COMBE  *
*
*****

```

```

127 KO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      0  PUNCH COMPUTED HYDROGRAPH
          IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*
137 KK *   BASE3  *      -1
*

```

```

*****
143 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*          *
150 KK      * BASF1 *      -3
*          *
*****

```

```

156 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*          *
163 KK      * BASF2 *      -2
*          *
*****

```

```

169 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*          *
176 KK      * BASF3 *      -1
*          *
*****

```

```

182 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*          *
189 KK      * BASG *      -1
*          *
*****

```

```

195 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2      900 LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **


```

*****
*          *
202 KK    * BASH1 *      -1
*          *
*****

```

```

209 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*          *
222 KK    * BASH2- *      2
*          *
*****

```

```

228 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*          *
235 KK    * BASI *
*          *
*****

```

```

241 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*          *
251 KK    * BASJ *
*          *
*****

```

```

257 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*** **

```

*****
*          *
264 KK    * BASK *
*          *
*****

```

```

270 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
            IPNCH      0  PUNCH COMPUTED HYDROGRAPH
            IOUT       22  SAVE HYDROGRAPH ON THIS UNIT
            ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
            ISAV2     900  LAST ORDINATE PUNCHED OR SAVED
            TIMINT     .033 TIME INTERVAL IN HOURS

```

*
277 KK * BASL *
*

283 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*
290 KK * BASM *
*

296 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*
303 KK * BASN *
*

309 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*
310 KK * O *
*

317 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*
318 KK * P *
*

325 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED

TIMINT .033 TIME INTERVAL IN HOURS DEV-100.OH1

*** **

329 KK

*
* Q *
*

336 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

*** **

337 KK

*
* R *
*

344 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 22 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 900 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

1									
RUNOFF SUMMARY									
FLOW IN CUBIC FEET PER SECOND									
TIME IN HOURS, AREA IN SQUARE MILES									
OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+		A1	236.	3.17	19.	5.	4.	.08	
+	ROUTED TO	BASA1	228.	3.17	18.	5.	4.	.08	
+								2761.99	3.17
+	HYDROGRAPH AT								
+		A2	112.	3.13	9.	2.	2.	.04	
+	ROUTED TO	BAS A2	108.	3.17	8.	2.	2.	.04	
+								2739.95	3.17
+	HYDROGRAPH AT								
+		B1	84.	3.13	6.	2.	1.	.03	
+	ROUTED TO	BASB-3	76.	3.17	6.	1.	1.	.03	
+								2785.80	3.17
+	HYDROGRAPH AT								
+		B2	131.	3.17	12.	3.	2.	.05	
+	2 COMBINED AT	COMBB	207.	3.17	18.	4.	4.	.08	
+	ROUTED TO	BASB-5	198.	3.20	16.	4.	3.	.08	
+								2747.00	3.20
+	HYDROGRAPH AT								
+		C	92.	3.13	7.	2.	1.	.03	
+	ROUTED TO	BASC-1	80.	3.20	6.	2.	1.	.03	
+								2783.88	3.20
+	HYDROGRAPH AT								
+		D	98.	3.13	8.	2.	2.	.03	
+	ROUTED TO	BASD-2	79.	3.23	8.	2.	2.	.03	
+								2723.85	3.23
+	HYDROGRAPH AT								
+		E1	154.	3.17	15.	4.	3.	.06	
+	ROUTED TO	BASE1-	145.	3.23	14.	3.	3.	.06	
+								2699.95	3.23

DEV-100.OH1

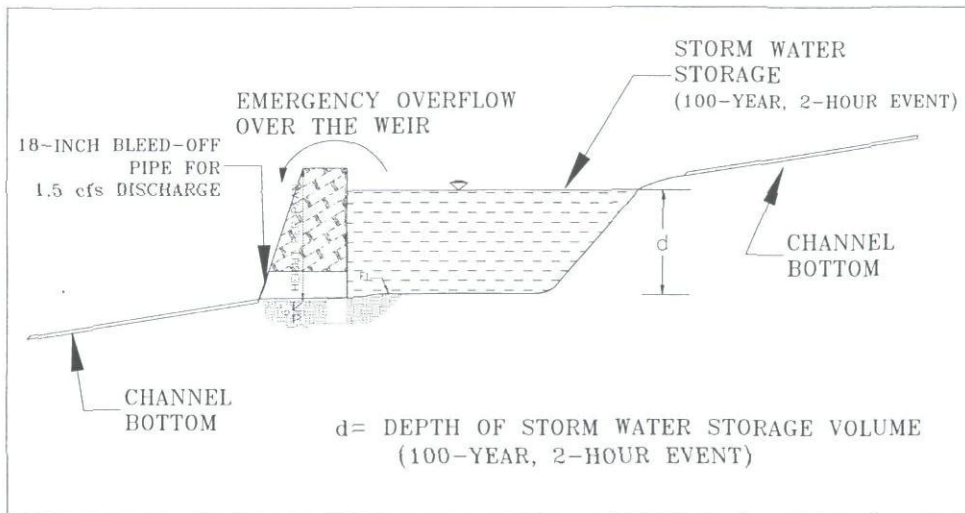
+	HYDROGRAPH AT	E2	138.	3.13	11.	3.	2.	.05		
+	ROUTED TO	BASE2	101.	3.23	7.	2.	1.	.05	2702.78	3.23
+	2 COMBINED AT	COMBE	246.	3.23	21.	5.	4.	.11		
+	6 COMBINED AT	CLEAR	885.	3.23	78.	19.	16.	.37		
+	HYDROGRAPH AT	E3	29.	3.10	2.	0.	0.	.01		
+	ROUTED TO	BASE3	16.	3.20	1.	0.	0.	.01	2714.56	3.20
+	HYDROGRAPH AT	F1	102.	3.17	9.	2.	2.	.04		
+	ROUTED TO	BASF1	88.	3.23	8.	2.	2.	.04	2699.89	3.23
+	HYDROGRAPH AT	F2	45.	3.13	3.	1.	1.	.01		
+	ROUTED TO	BASF2	35.	3.20	3.	1.	1.	.01	2704.78	3.20
+	HYDROGRAPH AT	F3	41.	3.13	3.	1.	1.	.01		
+	ROUTED TO	BASF3	38.	3.17	3.	1.	1.	.01	2707.98	3.17
+	HYDROGRAPH AT	G	43.	3.13	4.	1.	1.	.02		
+	ROUTED TO	BASG	31.	3.27	3.	1.	1.	.02	2726.74	3.27
+	HYDROGRAPH AT	H1	160.	3.17	14.	4.	3.	.06		
+	ROUTED TO	BASH1	147.	3.20	11.	3.	2.	.06	2745.93	3.20
+	HYDROGRAPH AT	H2	176.	3.20	17.	4.	3.	.07		
+	ROUTED TO	BASH2-	172.	3.23	16.	4.	3.	.07	2775.99	3.23
+	ROUTED TO	BASH2-	165.	3.27	15.	4.	3.	.07	2788.99	3.27
+	HYDROGRAPH AT	I	76.	3.13	6.	2.	1.	.03		
+	ROUTED TO	BASI	50.	3.23	5.	1.	1.	.03	2759.43	3.23
+	8 COMBINED AT	CLEAR	543.	3.23	48.	12.	10.	.24		
+	HYDROGRAPH AT	J	86.	3.10	6.	1.	1.	.02		
+	ROUTED TO	BASJ	75.	3.13	5.	1.	1.	.02	2773.79	3.13
+	HYDROGRAPH AT	K	36.	3.07	2.	1.	0.	.01		
+	ROUTED TO	BASK	20.	3.17	1.	0.	0.	.01	2740.57	3.17
+	HYDROGRAPH AT	L	34.	3.07	2.	1.	0.	.01		
+	ROUTED TO	BASL	28.	3.13	2.	0.	0.	.01	2725.78	3.13
+	HYDROGRAPH AT	M	29.	3.10	2.	0.	0.	.01		
+	ROUTED TO	BASM	21.	3.17	2.	0.	0.	.01	2717.03	3.17
+	HYDROGRAPH AT									

DEV-100.OH1									
	N	36.	3.10	2.	1.	0.	.01		
ROUTED TO	BASN	16.	3.27	1.	0.	0.	.01	2692.43	3.27
HYDROGRAPH AT	O	9.	3.07	1.	0.	0.	.00		
HYDROGRAPH AT	P	21.	3.07	1.	0.	0.	.01		
8 COMBINED AT	CLEAR	673.	3.23	61.	15.	12.	.31		
HYDROGRAPH AT	Q	10.	3.07	1.	0.	0.	.00		
HYDROGRAPH AT	R	411.	3.13	32.	8.	6.	.14		
SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)									
ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION INTERVAL PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1	STORM AREA (SQ MI) =								
A1 MANE	1.88	239.78	.01	189.45	2.16	2.00	236.42	190.00	2.16
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9693E+01 OUTFLOW= .9674E+01 BASIN STORAGE= .1217E-02 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =								
A2 MANE	1.44	112.74	.01	188.42	2.24	2.00	112.13	188.00	2.24
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4302E+01 OUTFLOW= .4291E+01 BASIN STORAGE= .3724E-03 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =								
B1 MANE	1.70	84.46	.01	188.49	2.19	2.00	83.67	188.00	2.19
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3161E+01 OUTFLOW= .3155E+01 BASIN STORAGE= .2478E-03 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =								
B2 MANE	1.43	130.88	.01	189.97	2.22	2.00	130.85	190.00	2.22
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5952E+01 OUTFLOW= .5933E+01 BASIN STORAGE= .1117E-02 PERCENT ERROR= .3									
FOR STORM = 1	STORM AREA (SQ MI) =								
C MANE	1.13	92.24	.01	188.19	2.22	2.00	91.80	188.00	2.22
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .3556E+01 OUTFLOW= .3549E+01 BASIN STORAGE= .4105E-03 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =								
D MANE	1.07	98.44	.01	188.54	2.27	2.00	97.55	188.00	2.27
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4120E+01 OUTFLOW= .4113E+01 BASIN STORAGE= .6457E-03 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =								
E1 MANE	1.46	154.45	.01	191.31	2.27	2.00	153.59	190.00	2.27
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .7392E+01 OUTFLOW= .7370E+01 BASIN STORAGE= .1786E-02 PERCENT ERROR= .3									
FOR STORM = 1	STORM AREA (SQ MI) =								
E2 MANE	1.74	141.10	.01	188.83	2.27	2.00	137.93	188.00	2.27
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .5574E+01 OUTFLOW= .5564E+01 BASIN STORAGE= .4962E-03 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =								
E3 MANE	.61	28.67	.01	185.81	2.27	2.00	28.52	186.00	2.27
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9695E+00 OUTFLOW= .9686E+00 BASIN STORAGE= .5751E-04 PERCENT ERROR= .1									
FOR STORM = 1	STORM AREA (SQ MI) =								
F1 MANE	2.00	102.90	.01	190.64	2.27	2.00	102.36	190.00	2.27
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .4363E+01 OUTFLOW= .4352E+01 BASIN STORAGE= .5158E-03 PERCENT ERROR= .2									
FOR STORM = 1	STORM AREA (SQ MI) =								
F2 MANE	1.19	44.77	.01	187.96	2.27	2.00	44.71	188.00	2.27
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1697E+01 OUTFLOW= .1695E+01 BASIN STORAGE= .1539E-03 PERCENT ERROR= .1									

DEV-100.OH1

FOR STORM = 1	STORM AREA (SQ MI) =	.01							
F3 MANE	.68	41.31	186.98	2.27	2.00	40.52	188.00	2.27	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1575E+01 OUTFLOW= .1573E+01 BASIN STORAGE= .1671E-03 PERCENT ERROR=									.1
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
G MANE	.69	43.96	189.19	2.27	2.00	43.37	188.00	2.27	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1939E+01 OUTFLOW= .1934E+01 BASIN STORAGE= .3297E-03 PERCENT ERROR=									.3
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
H1 MANE	1.32	160.19	190.42	2.26	2.00	159.95	190.00	2.26	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .7127E+01 OUTFLOW= .7113E+01 BASIN STORAGE= .1335E-02 PERCENT ERROR=									.2
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
H2 MANE	2.00	176.79	191.31	2.20	2.00	175.93	192.00	2.20	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .8481E+01 OUTFLOW= .8448E+01 BASIN STORAGE= .1838E-02 PERCENT ERROR=									.4
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
I MANE	1.43	76.57	188.46	2.24	2.00	75.65	188.00	2.24	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2987E+01 OUTFLOW= .2981E+01 BASIN STORAGE= .2899E-03 PERCENT ERROR=									.2
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
J MANE	.67	86.10	185.83	2.24	2.00	85.63	186.00	2.25	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2876E+01 OUTFLOW= .2873E+01 BASIN STORAGE= .1339E-03 PERCENT ERROR=									.1
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
K MANE	.67	36.44	184.23	2.27	2.00	36.31	184.00	2.27	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1091E+01 OUTFLOW= .1091E+01 BASIN STORAGE= .1964E-04 PERCENT ERROR=									.0
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
L MANE	.50	34.08	184.38	2.27	2.00	33.57	184.00	2.27	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1091E+01 OUTFLOW= .1089E+01 BASIN STORAGE= .4161E-04 PERCENT ERROR=									.1
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
M MANE	.92	28.71	186.11	2.27	2.00	28.71	186.00	2.27	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .9695E+00 OUTFLOW= .9685E+00 BASIN STORAGE= .4774E-04 PERCENT ERROR=									.1
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
N MANE	1.07	36.42	185.94	2.27	2.00	36.37	186.00	2.27	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1212E+01 OUTFLOW= .1212E+01 BASIN STORAGE= .4858E-04 PERCENT ERROR=									.0
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
O MANE	.34	9.35	184.29	2.27	2.00	9.30	184.00	2.27	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2908E+00 OUTFLOW= .2905E+00 BASIN STORAGE= .8448E-05 PERCENT ERROR=									.1
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
P MANE	.46	21.58	184.91	2.27	2.00	21.09	184.00	2.27	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .7029E+00 OUTFLOW= .7020E+00 BASIN STORAGE= .2793E-04 PERCENT ERROR=									.1
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
Q MANE	.32	9.68	183.90	2.27	2.00	9.65	184.00	2.27	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .2908E+00 OUTFLOW= .2907E+00 BASIN STORAGE= .5883E-05 PERCENT ERROR=									.1
FOR STORM = 1	STORM AREA (SQ MI) =	.01							
R MANE	1.96	420.39	189.24	2.15	2.00	411.28	188.00	2.14	
CONTINUITY SUMMARY (AC-FT) - INFLOW= .0000E+00 EXCESS= .1607E+02 OUTFLOW= .1602E+02 BASIN STORAGE= .1145E-02 PERCENT ERROR=									.3

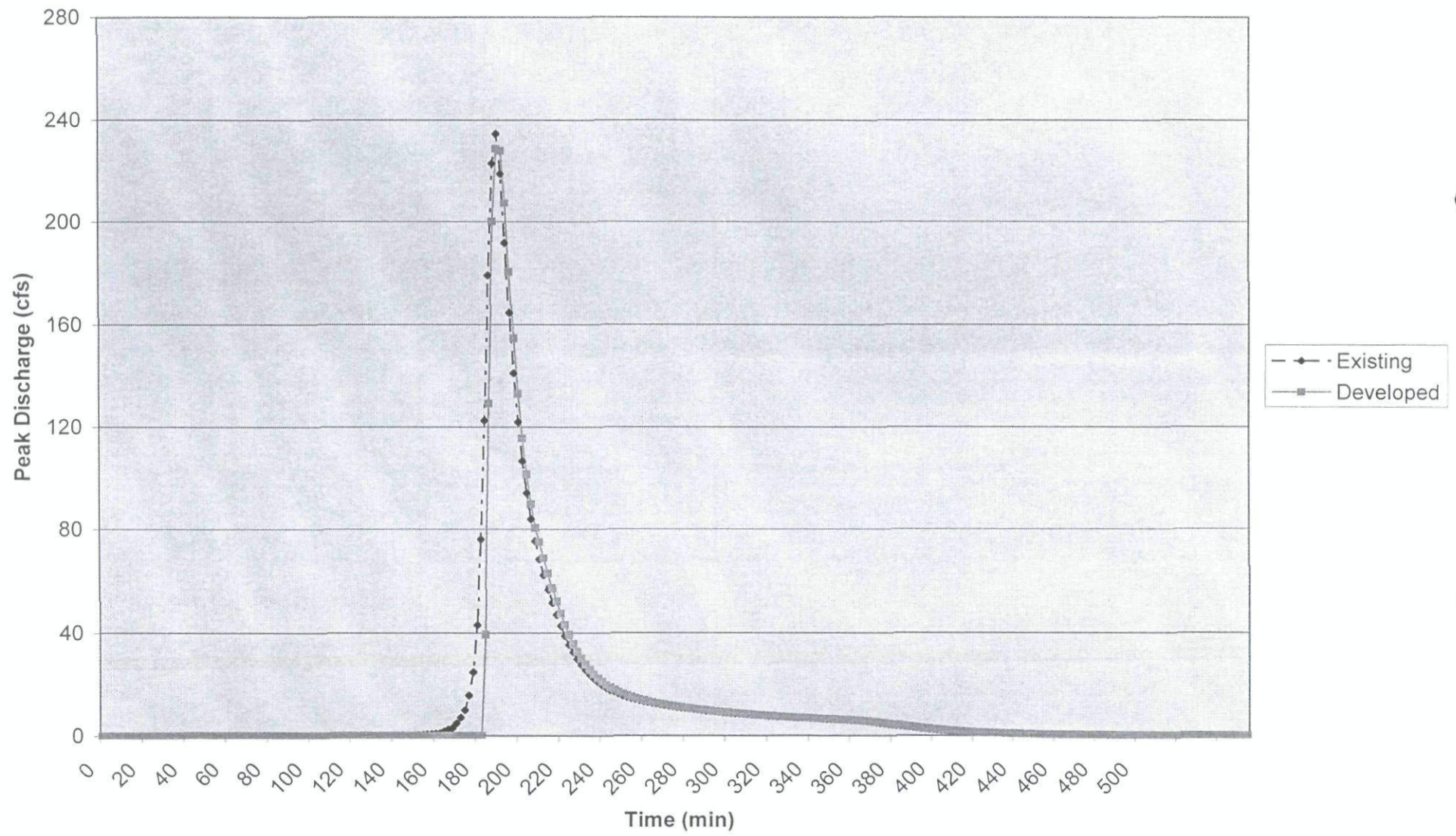
*** NORMAL END OF HEC-1 ***



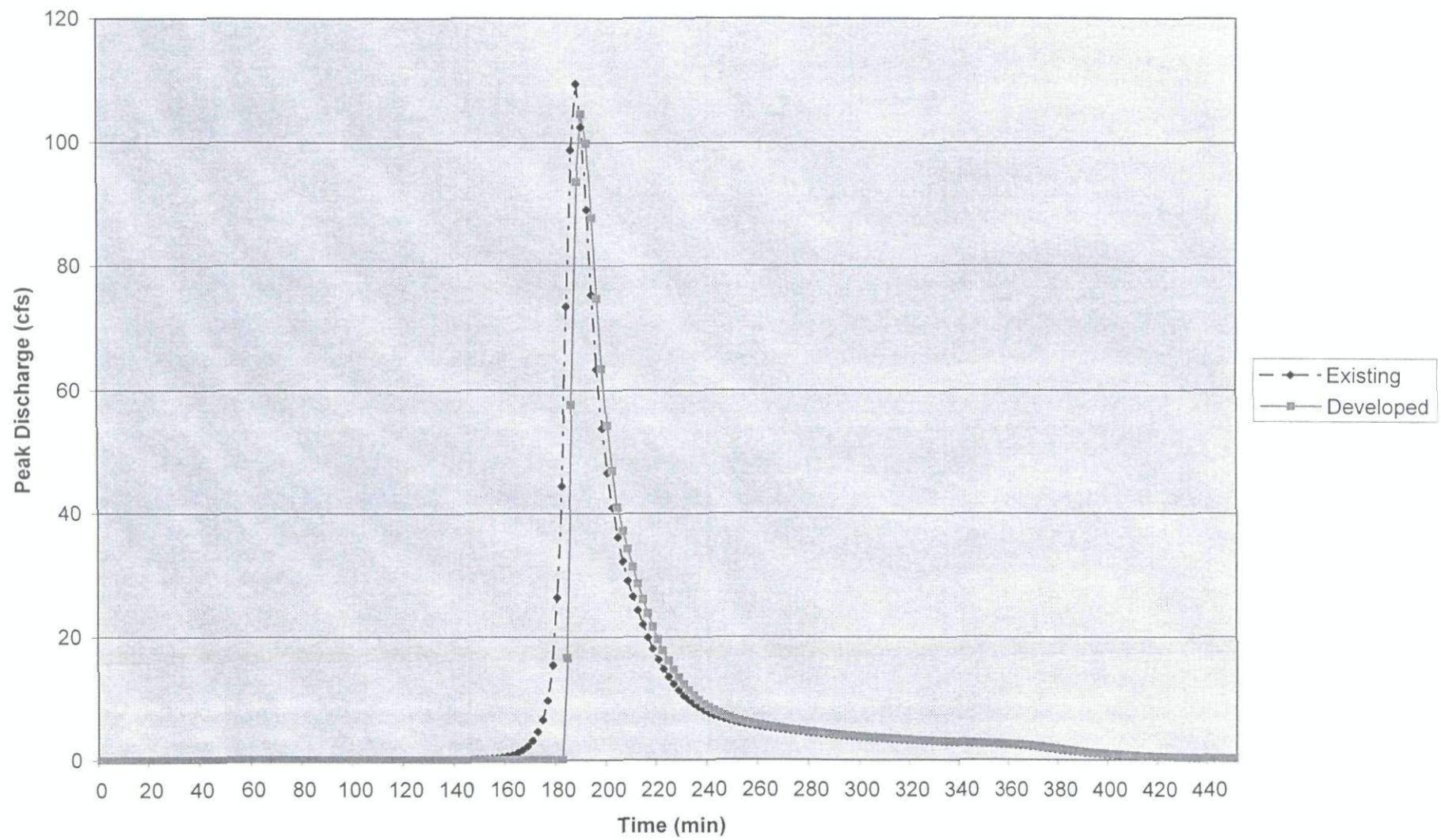
APPENDIX D

Hydrographs

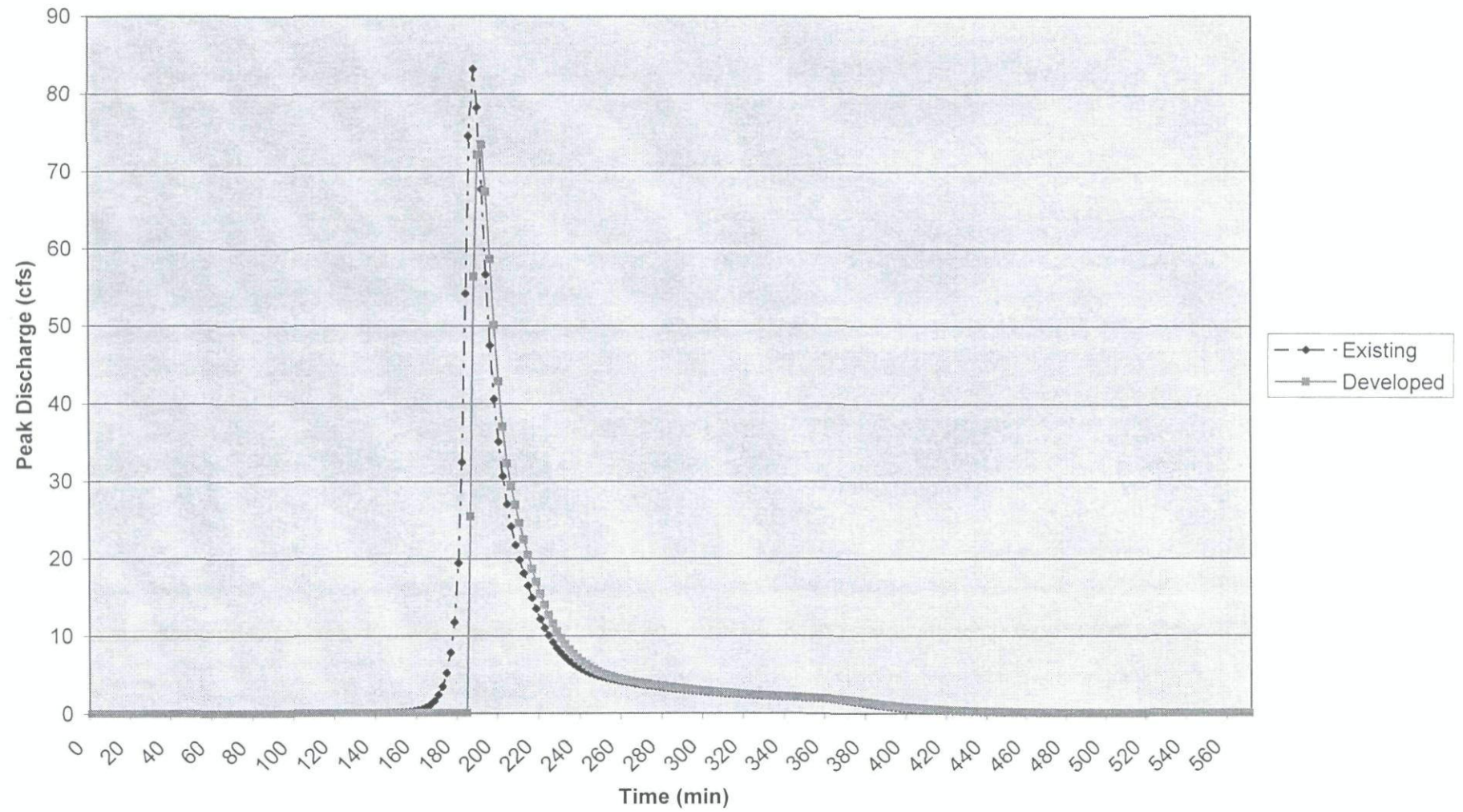
Hydrograph - A1



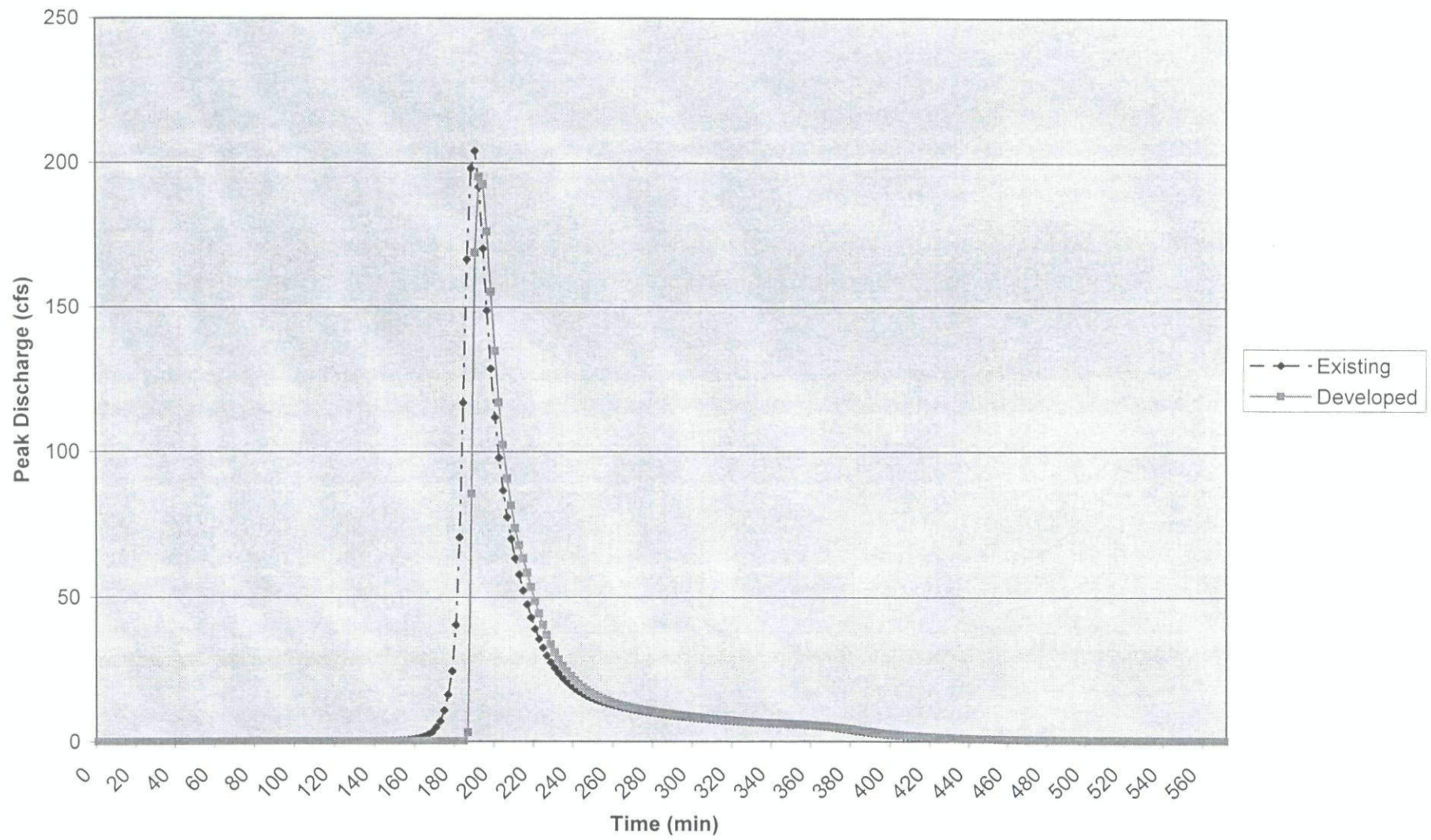
Hydrograph - A2



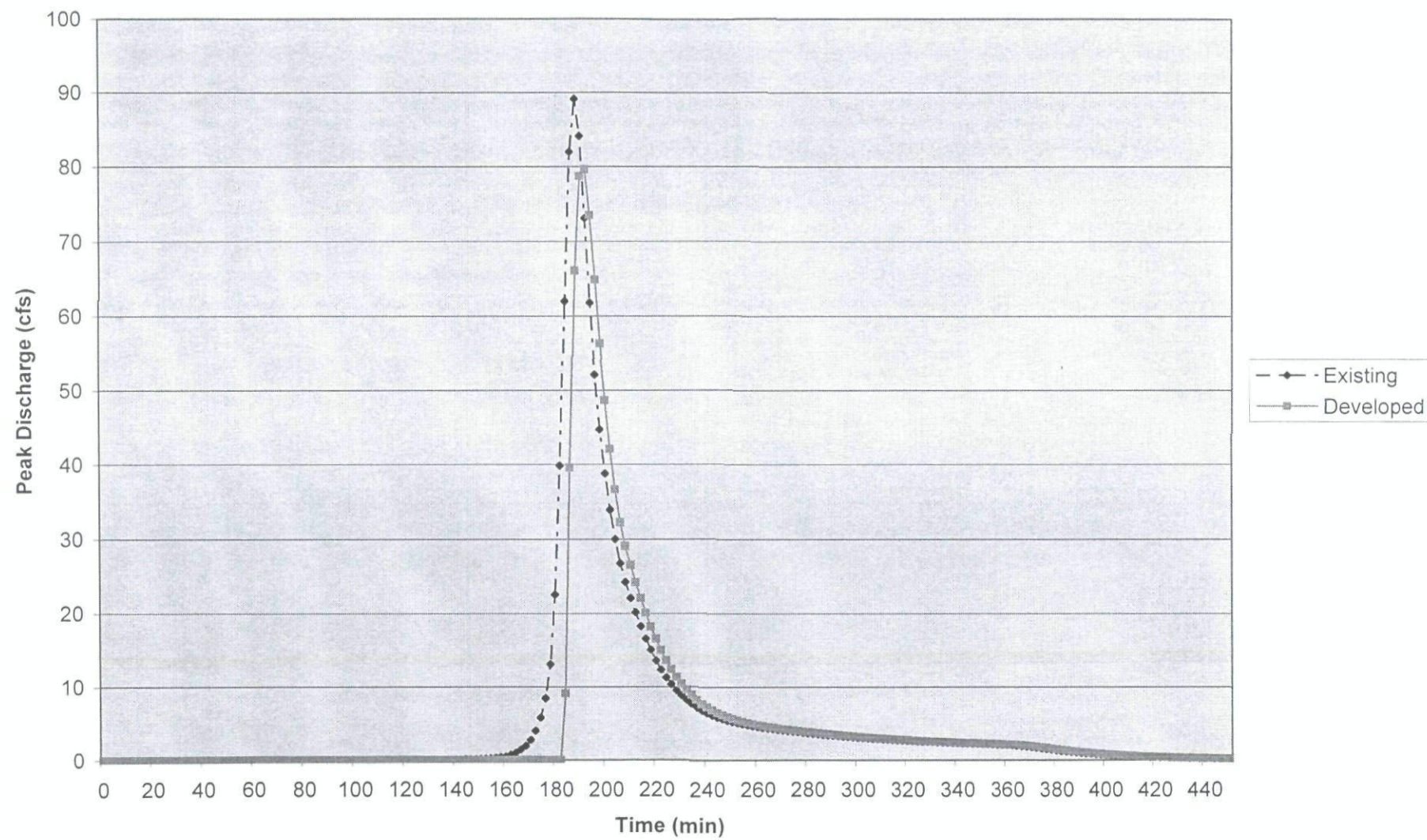
Hydrograph - B1



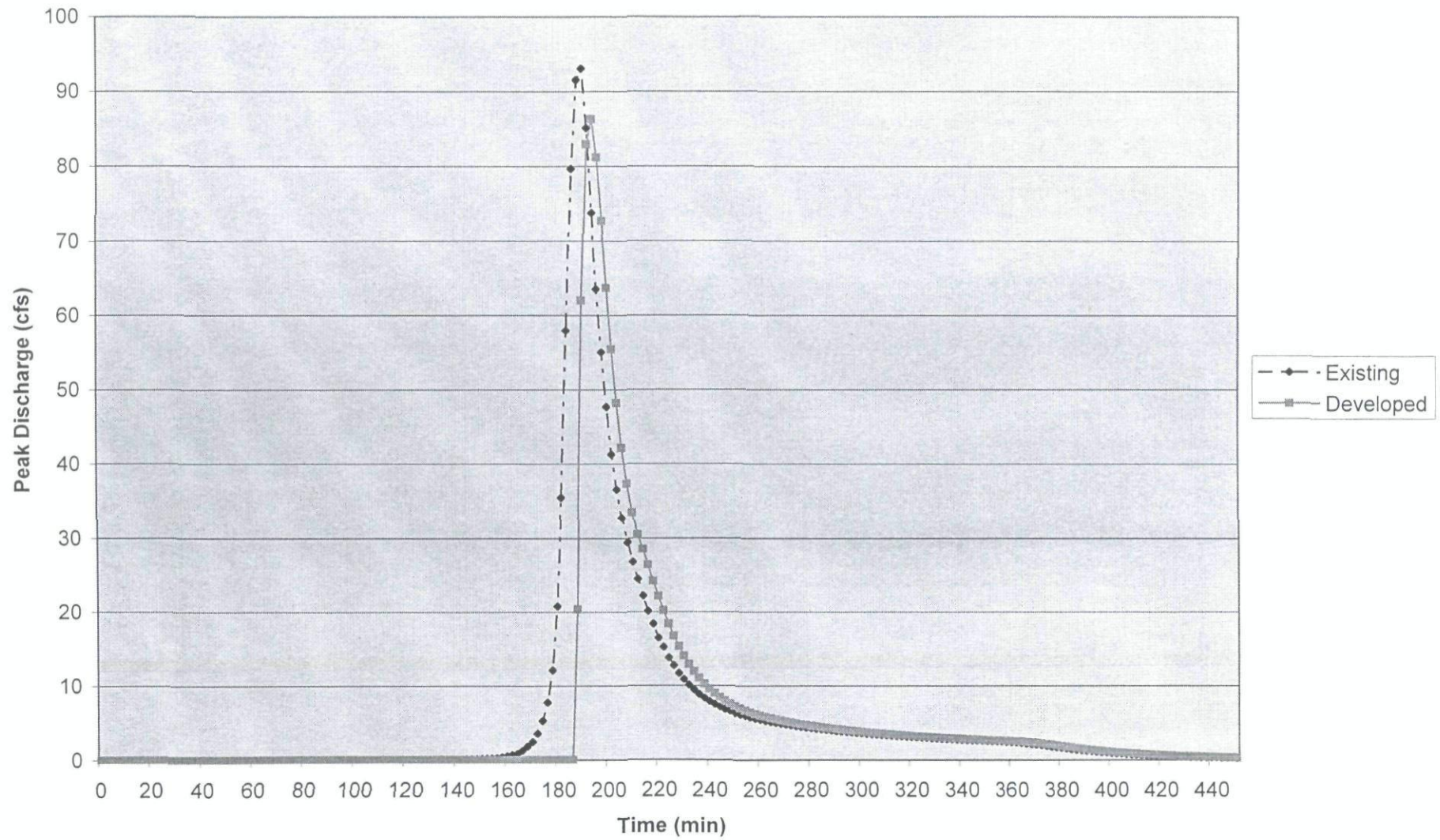
Hydrograph - COMB



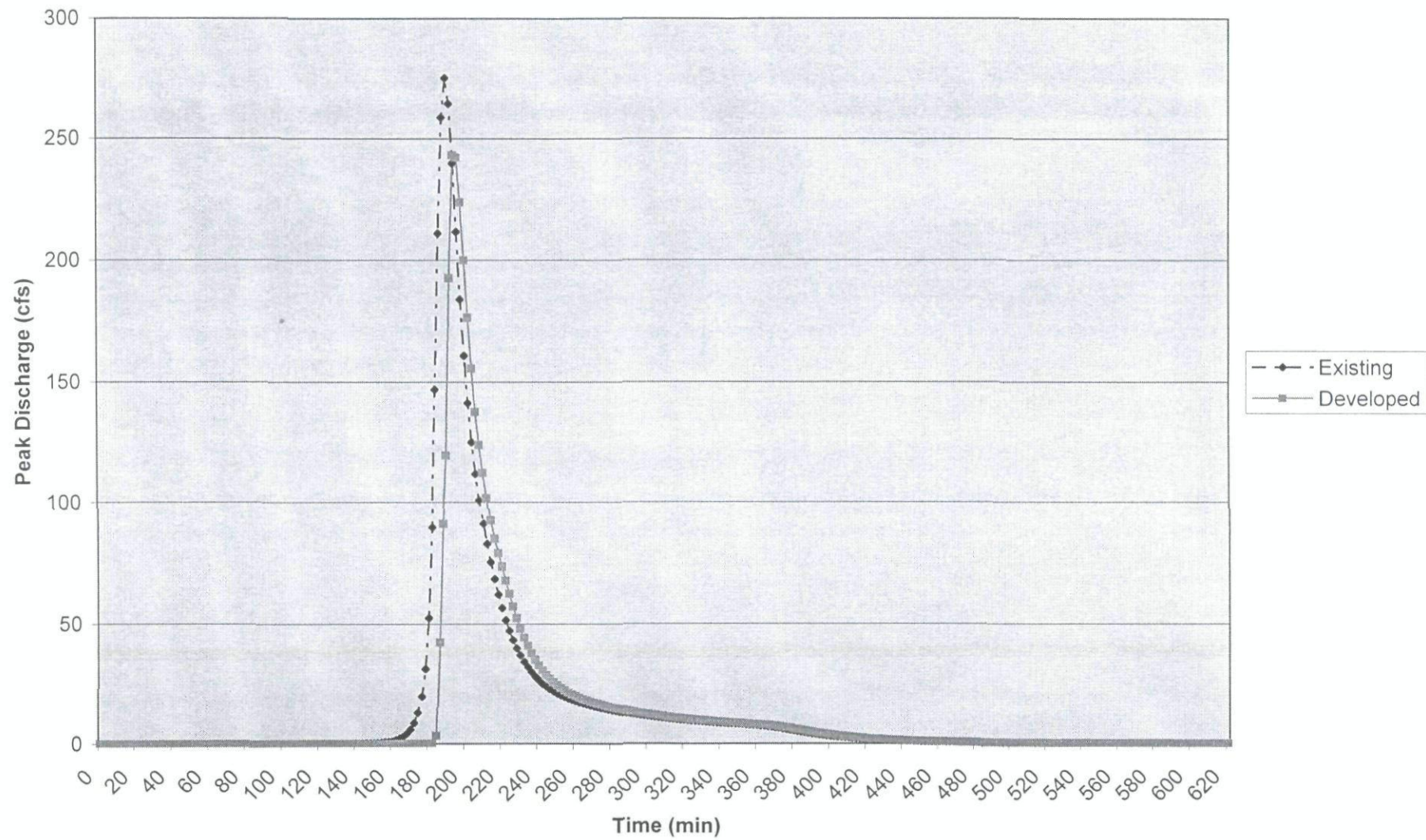
Hydrograph - C



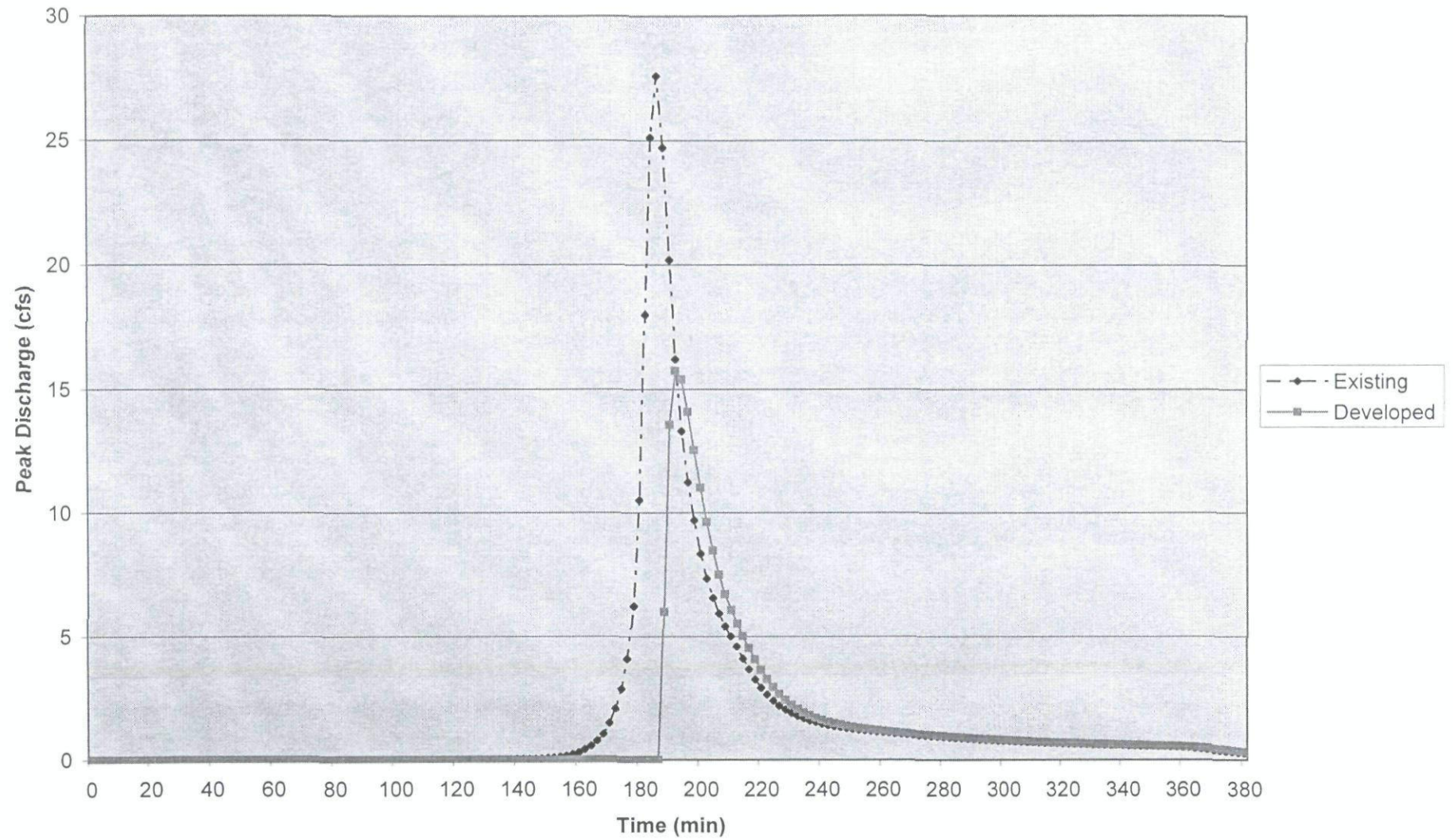
Hydrograph - D



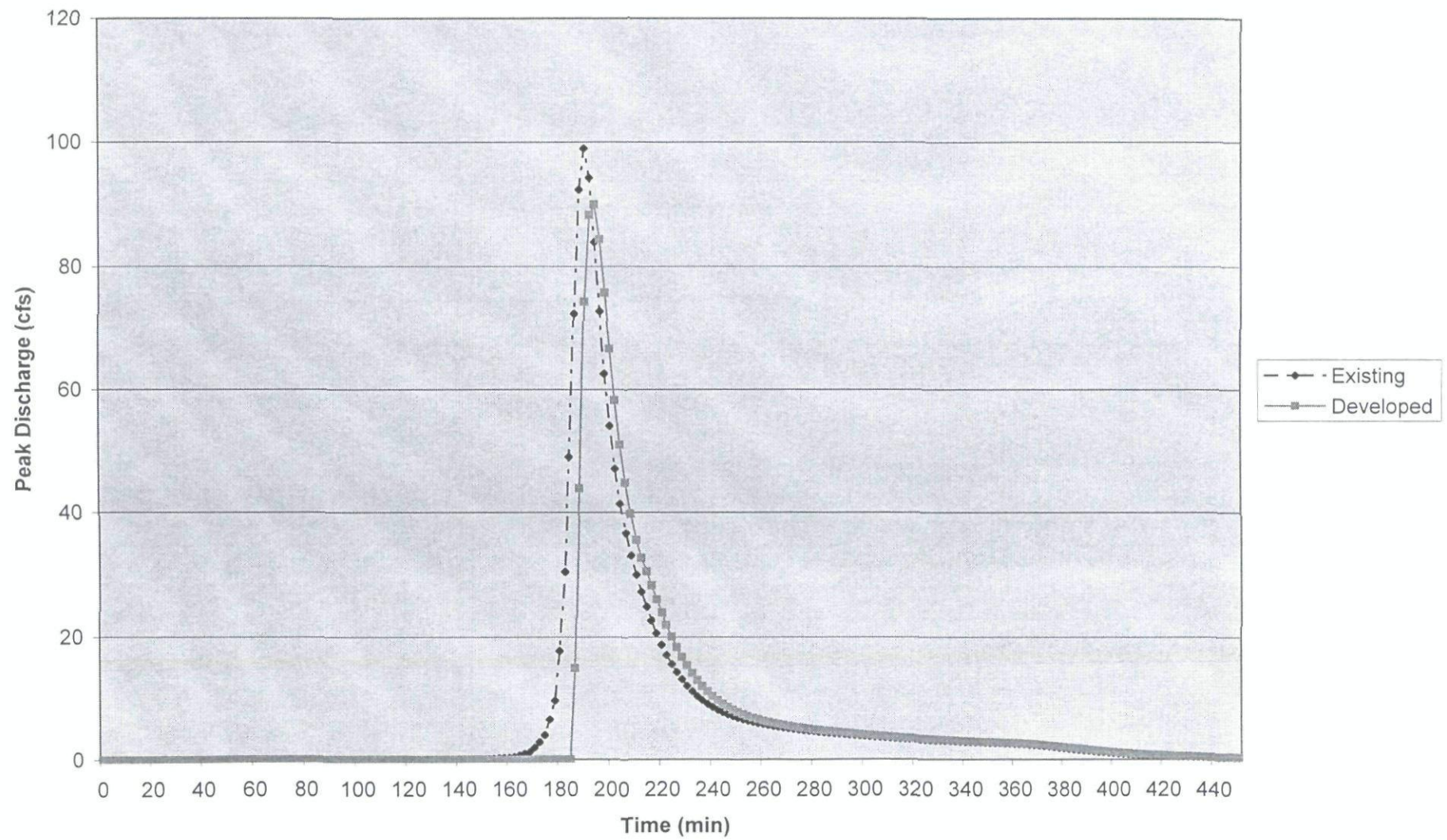
Hydrograph - COMB E



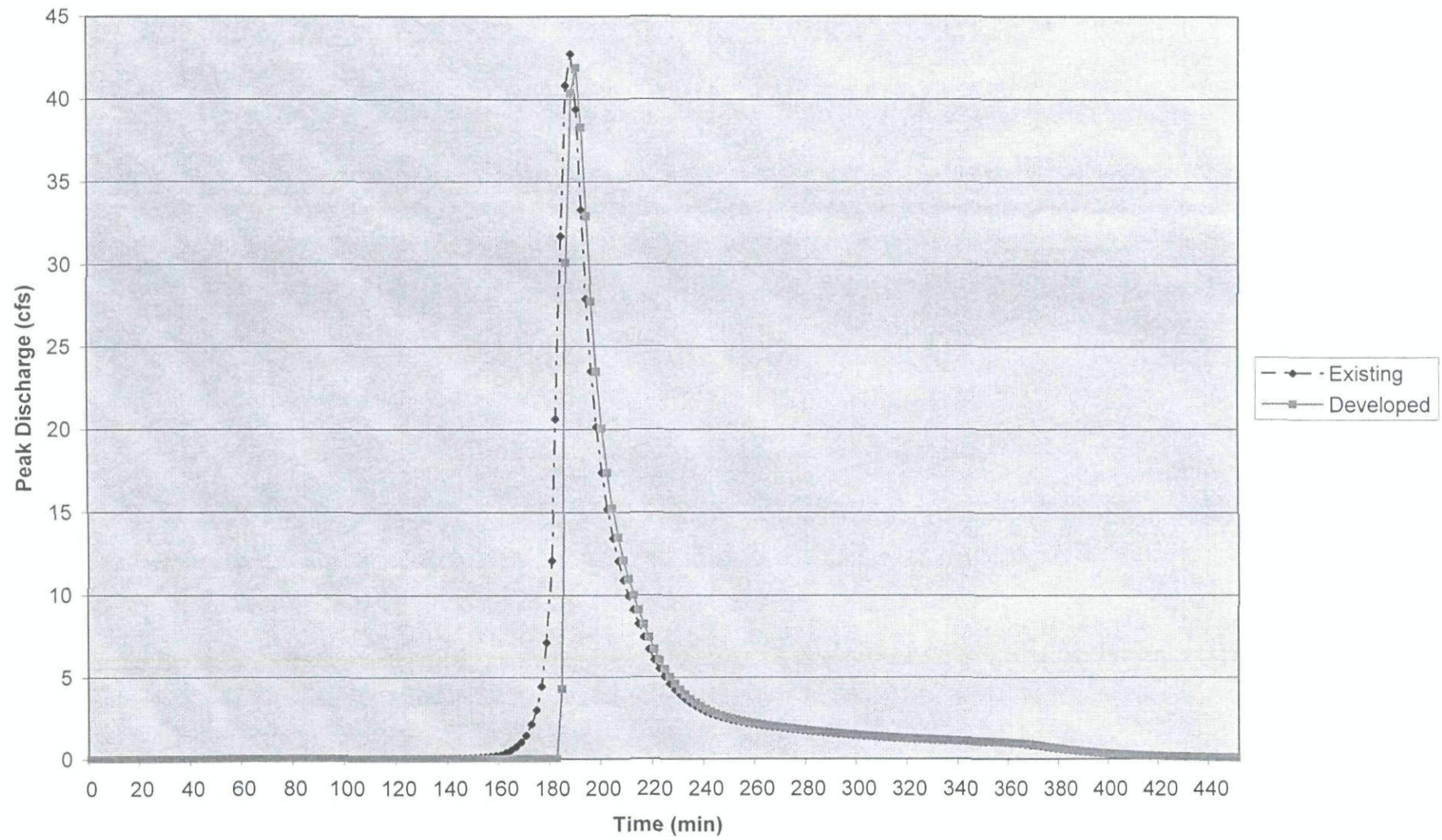
Hydrograph - E3



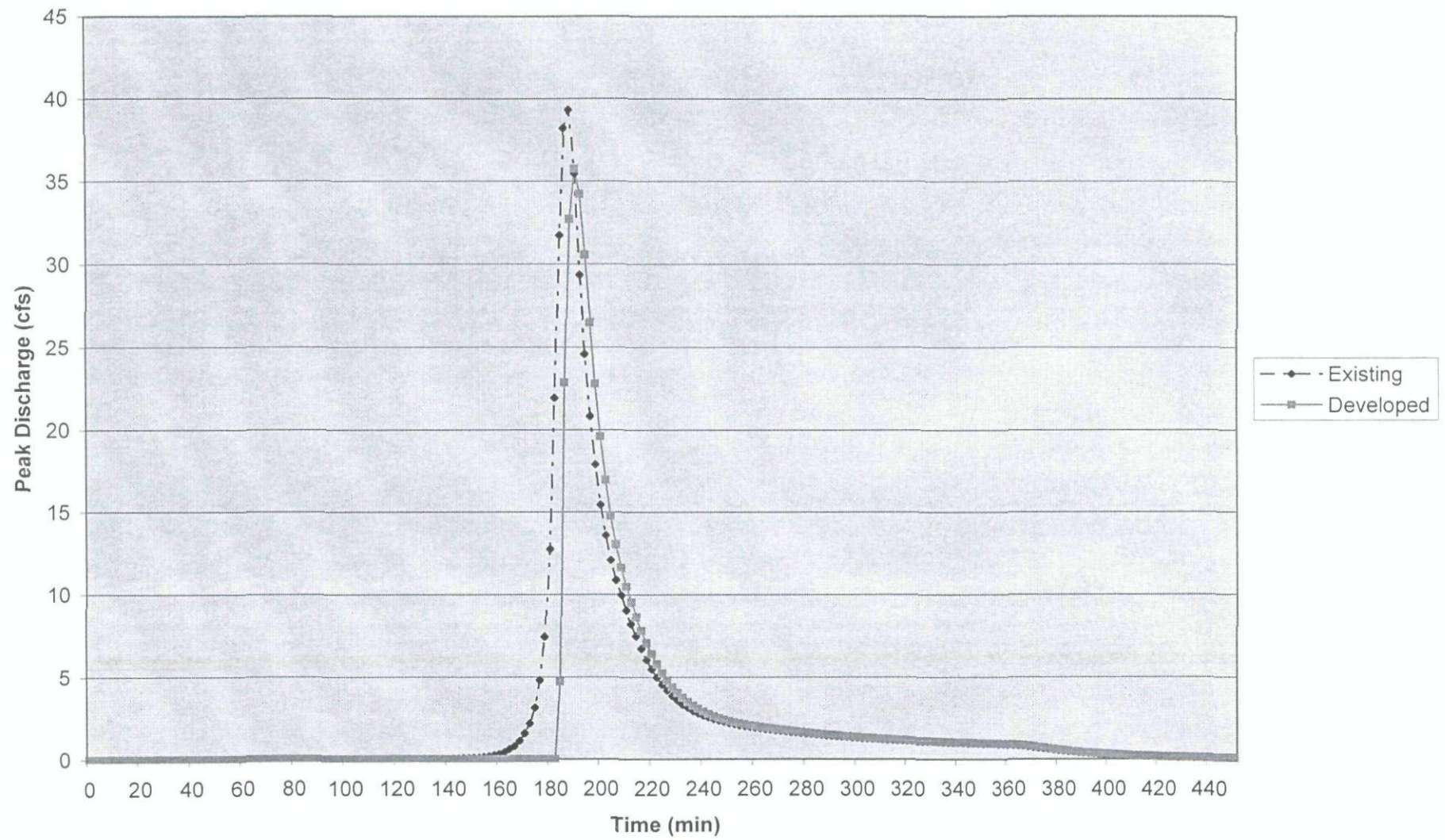
Hydrograph - F1



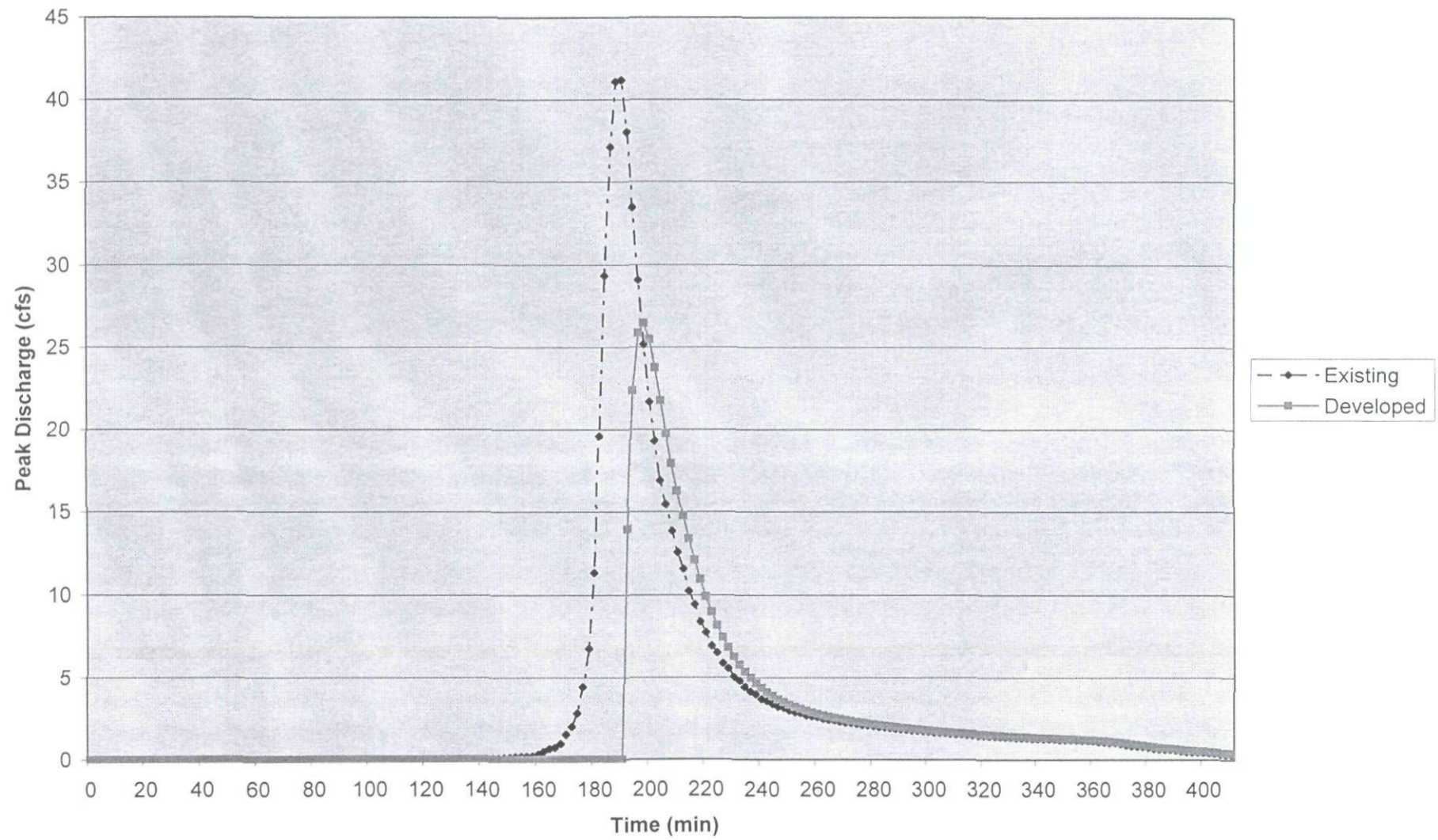
Hydrograph - F2



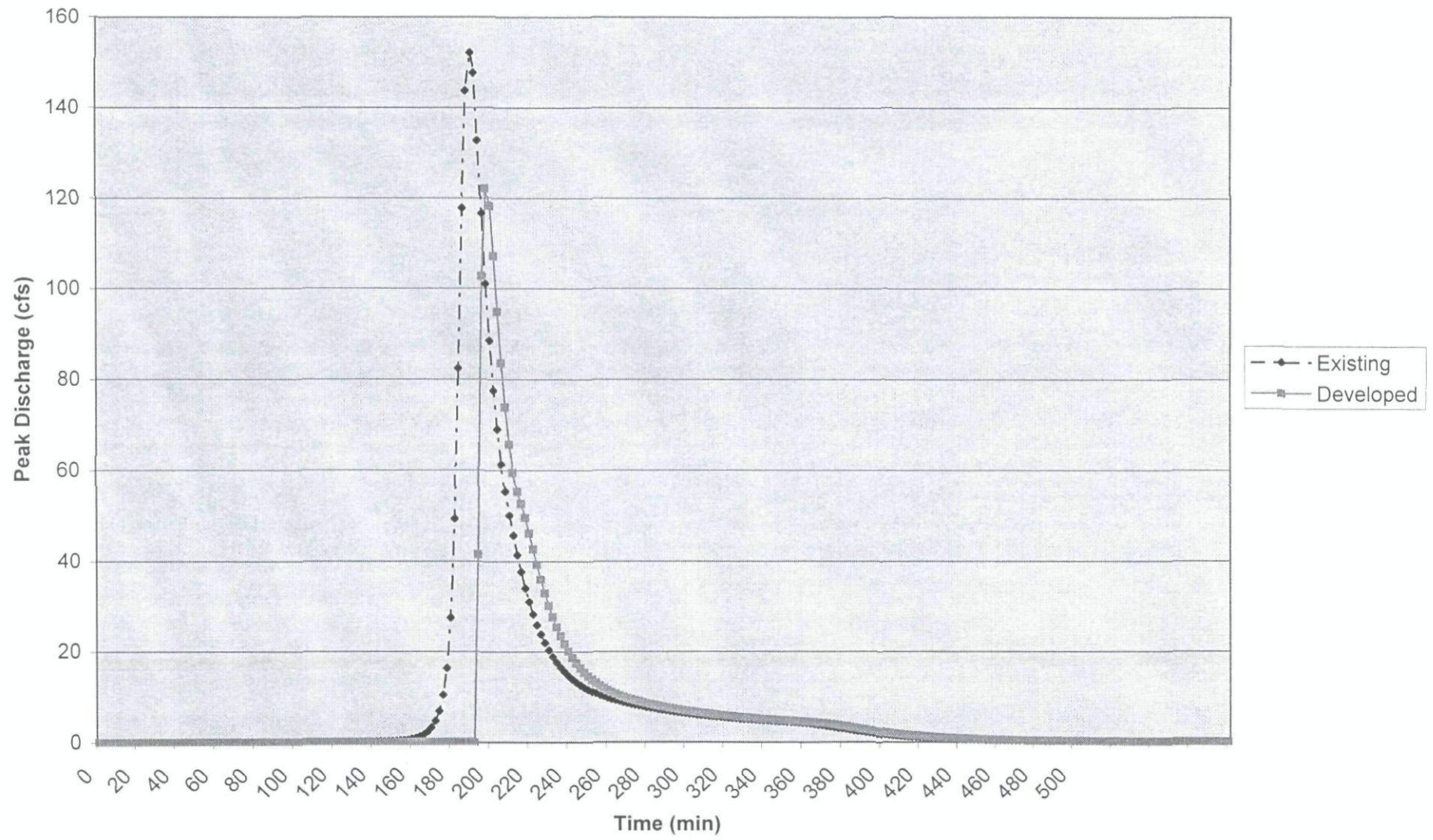
Hydrograph - F3



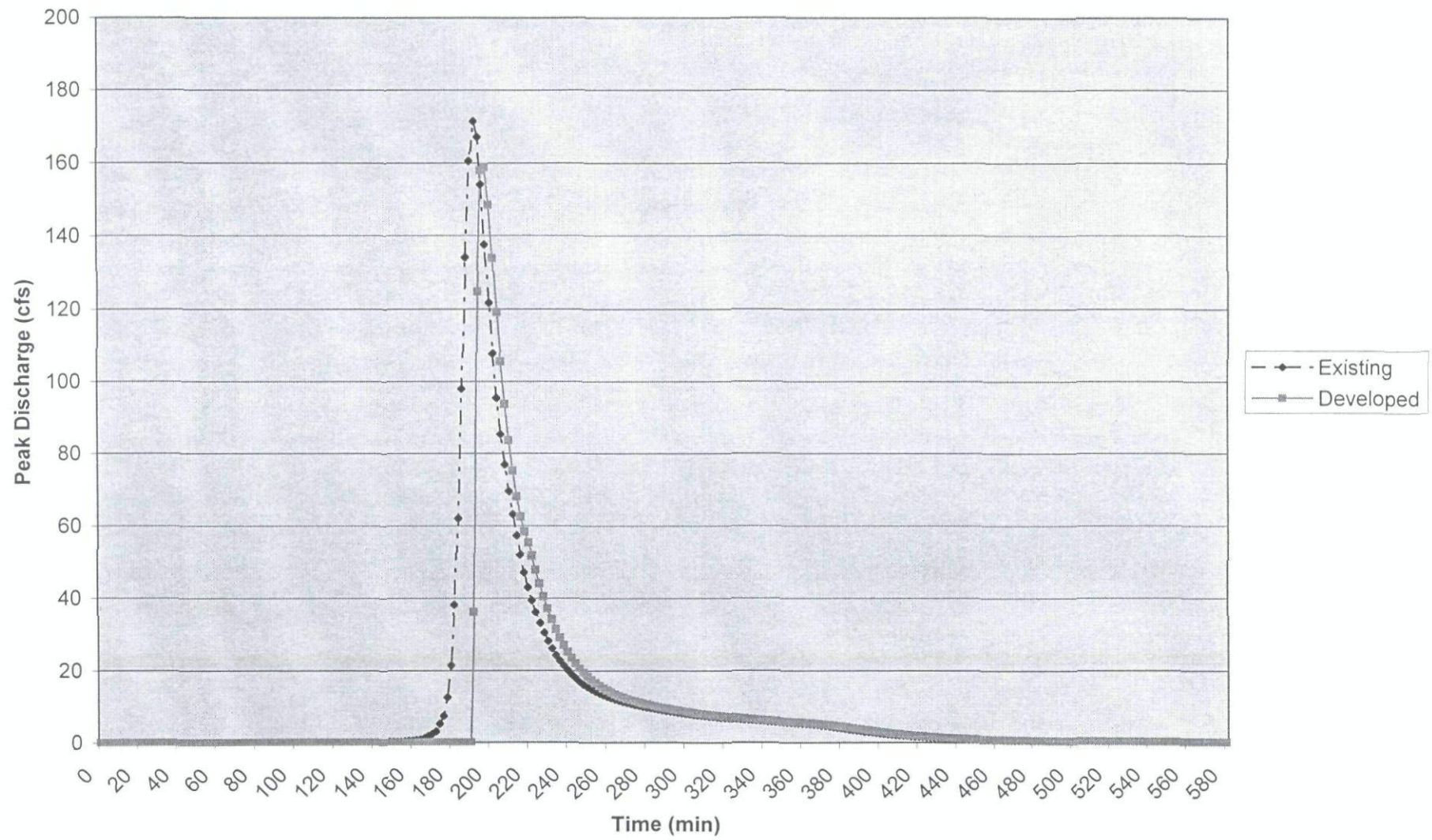
Hydrograph - G



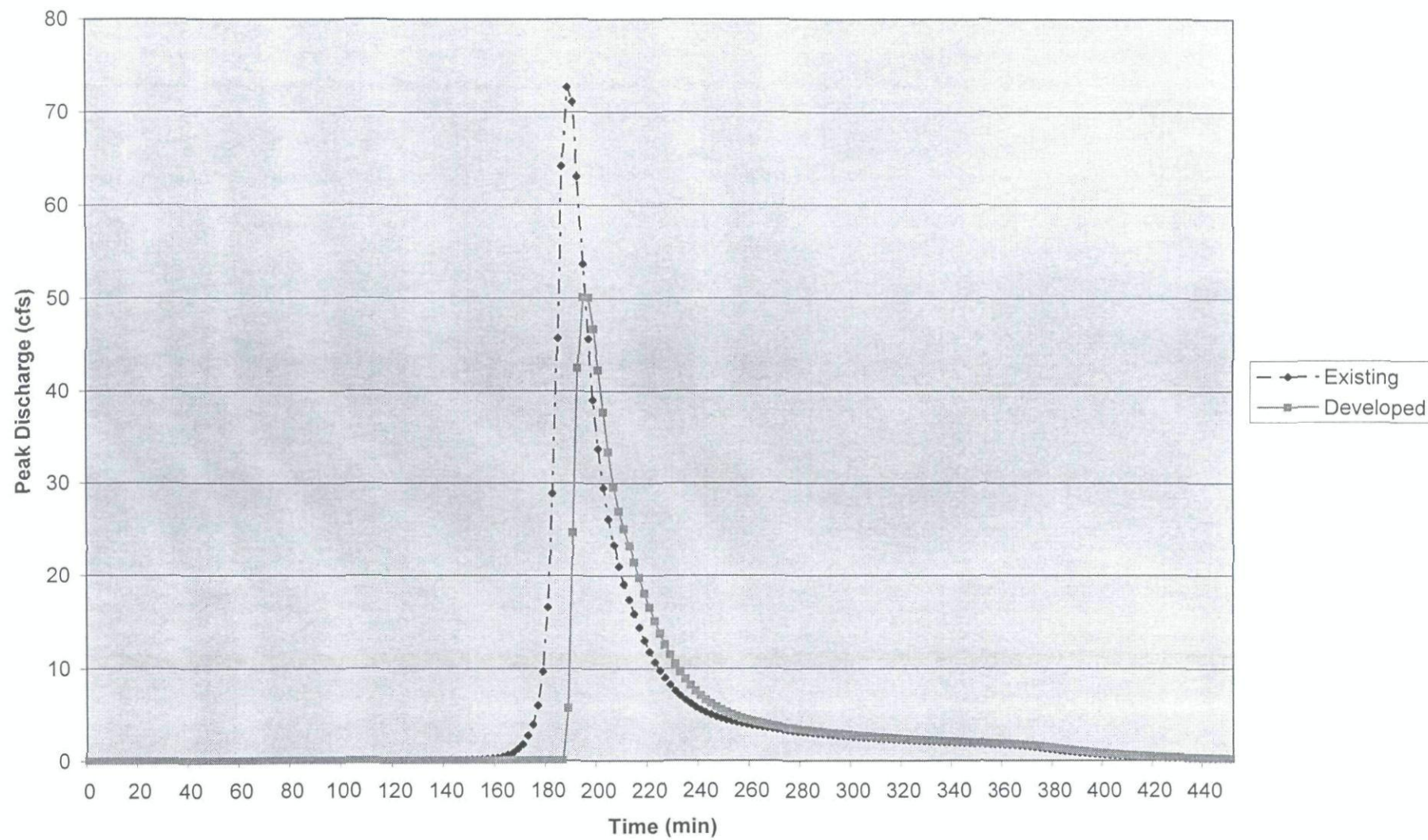
Hydrograph - H1



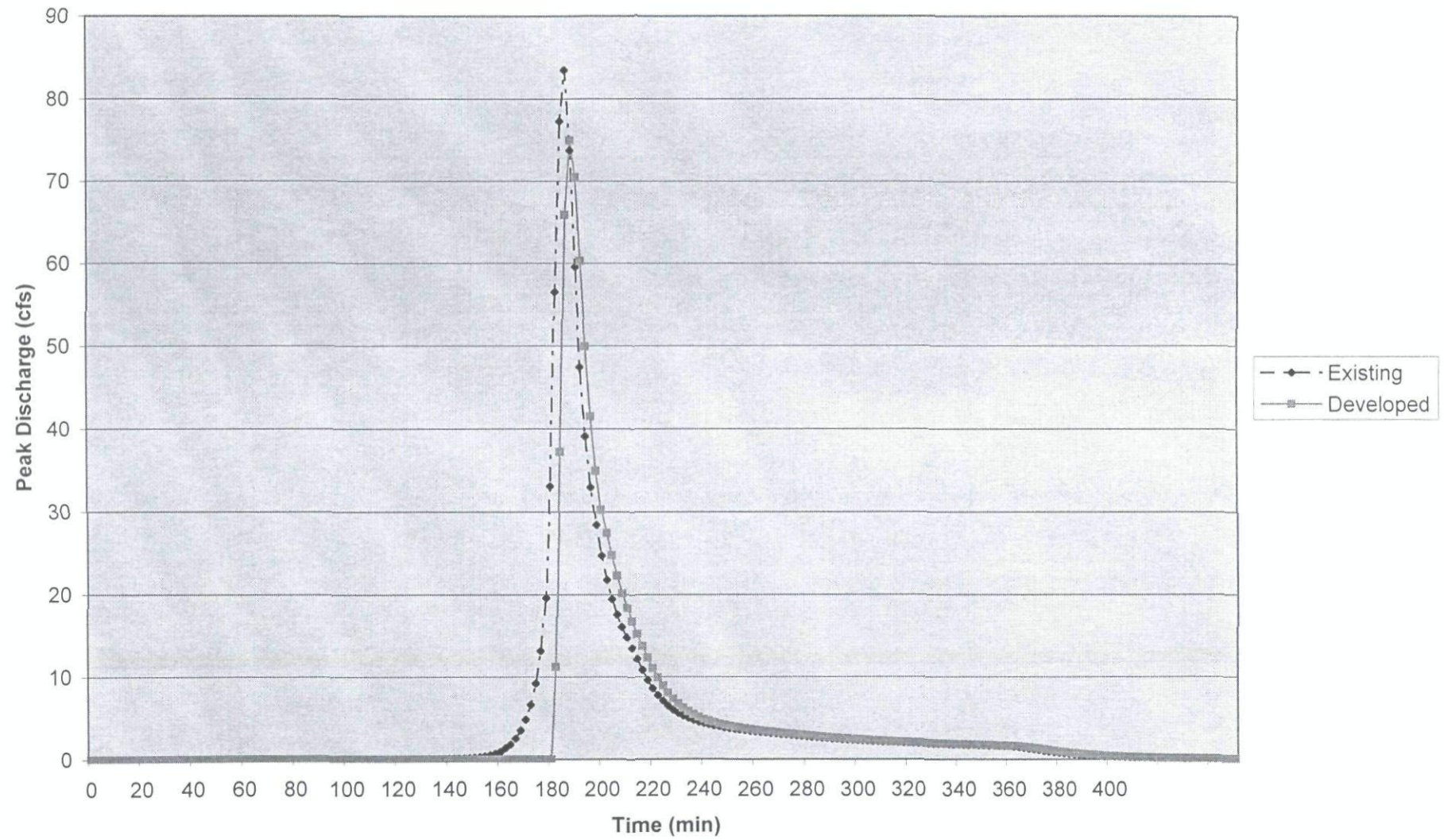
Hydrograph - H2



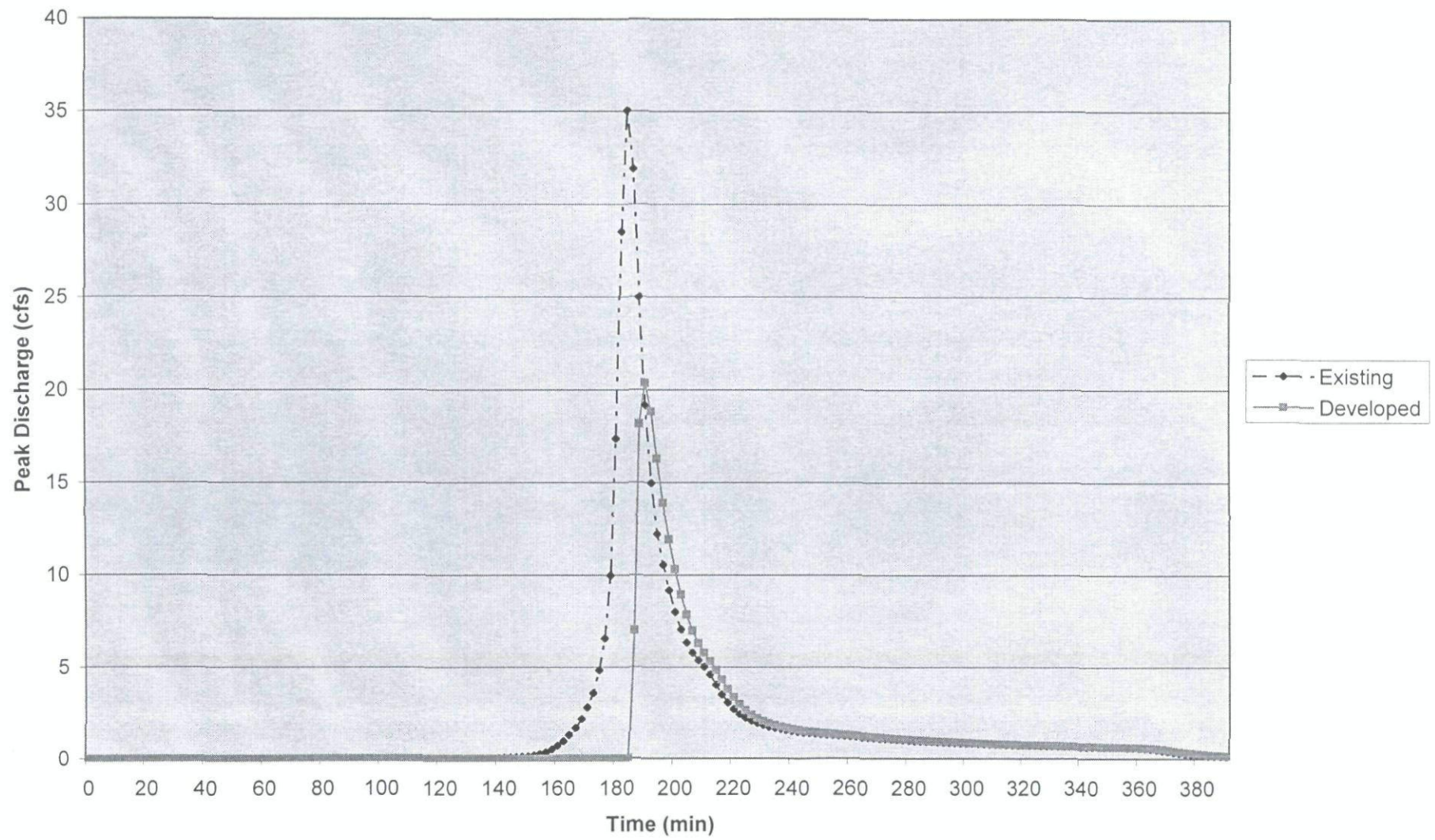
Hydrograph - I



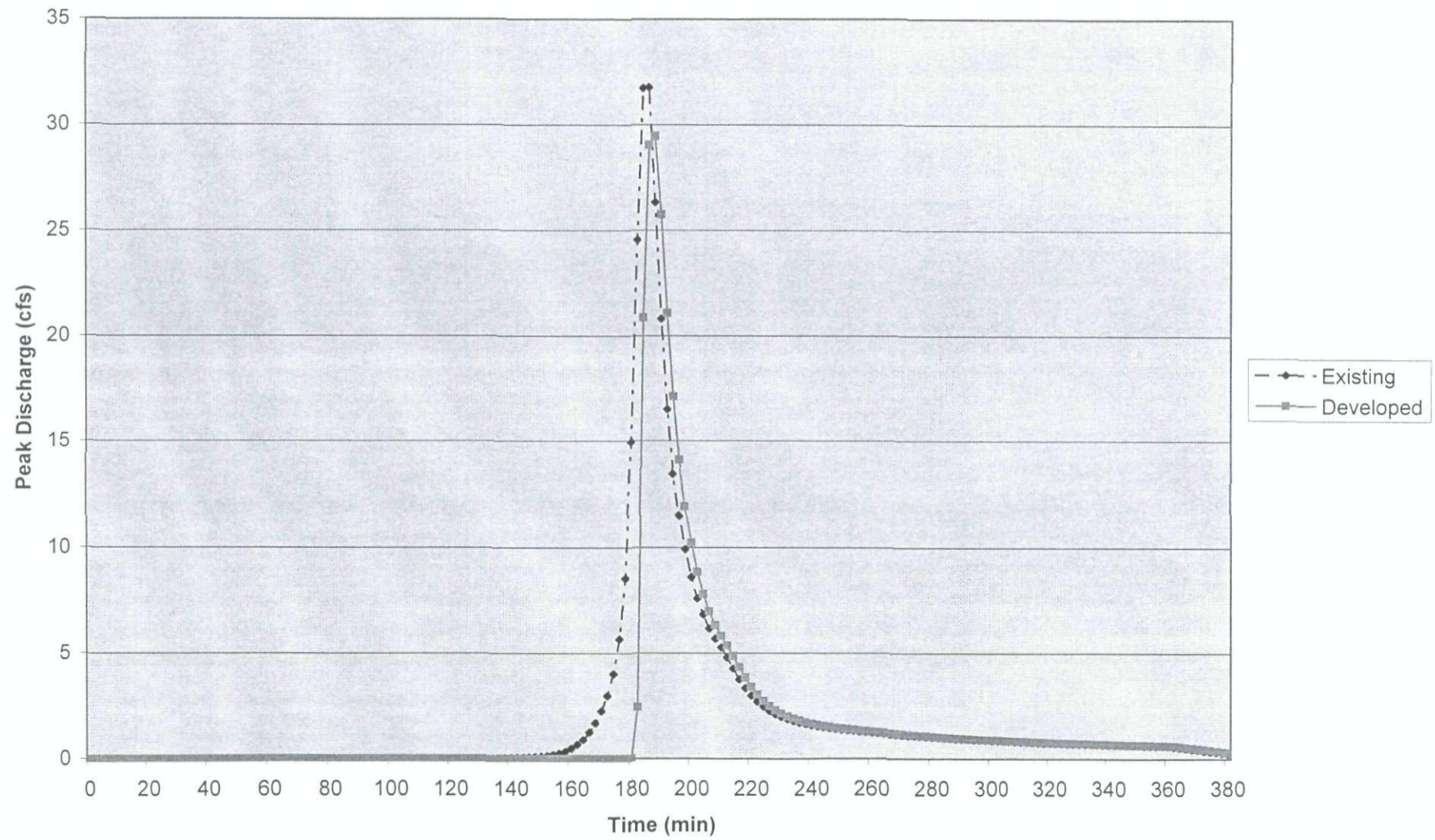
Hydrograph - J



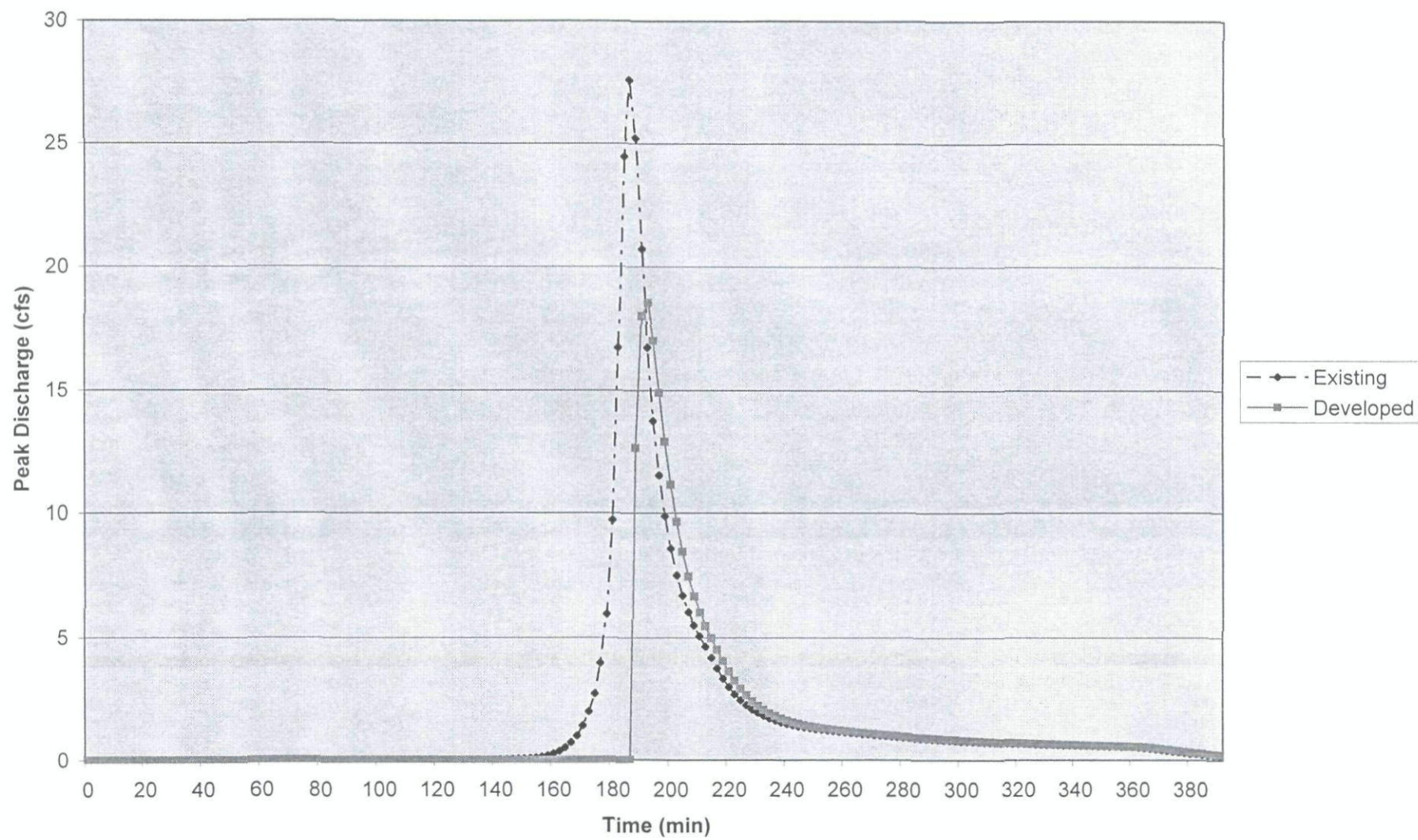
Hydrograph - K



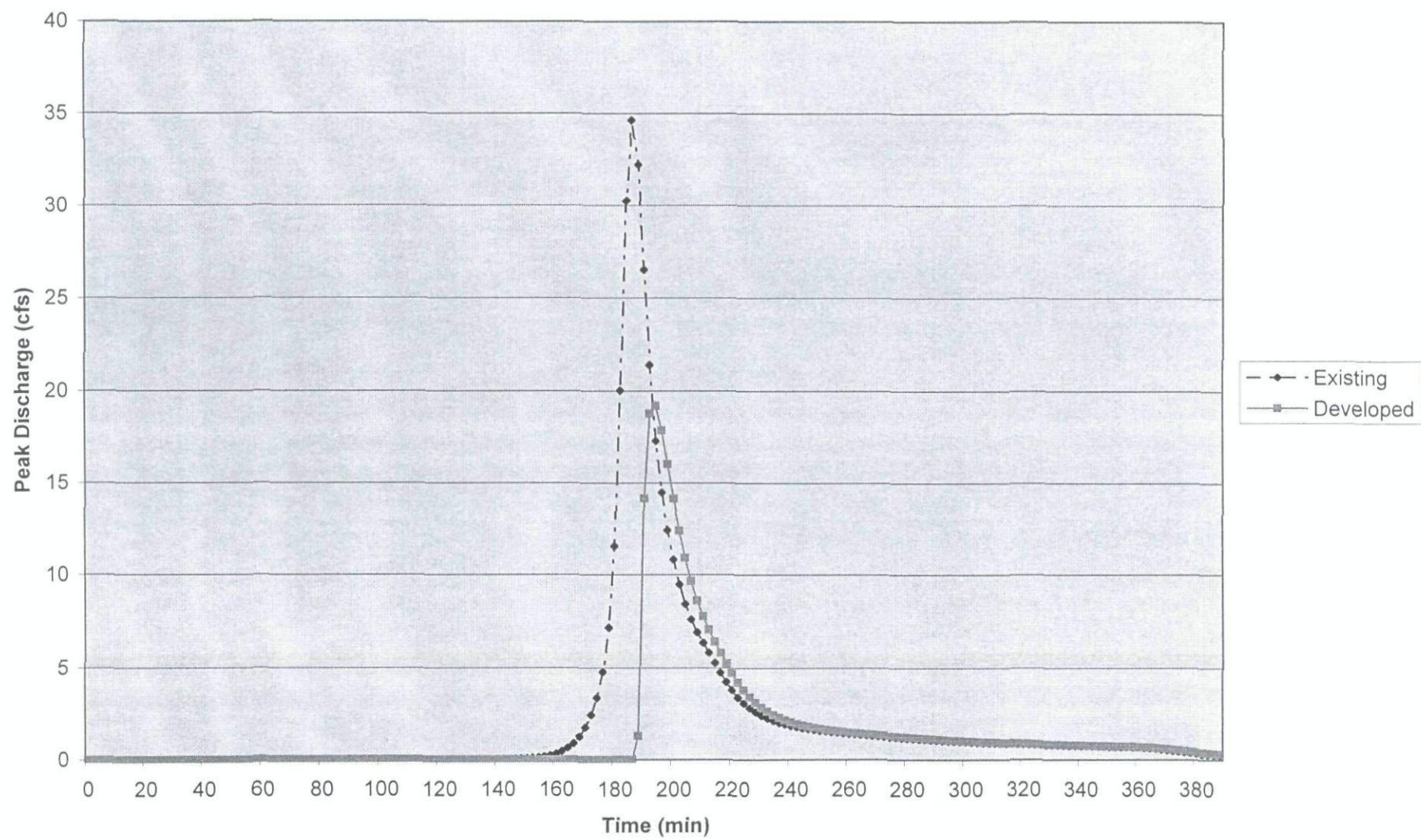
Hydrograph - L



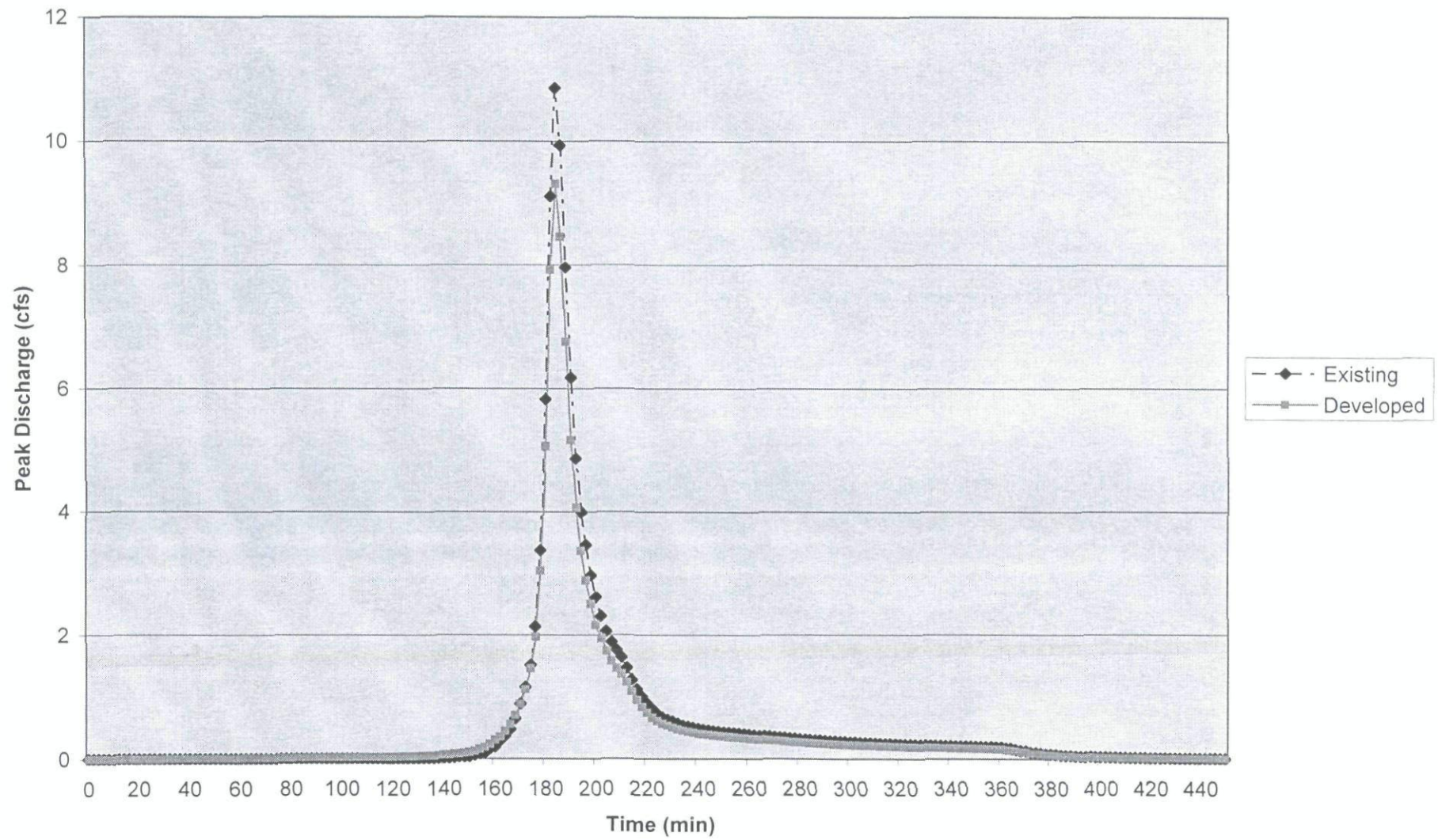
Hydrograph - M



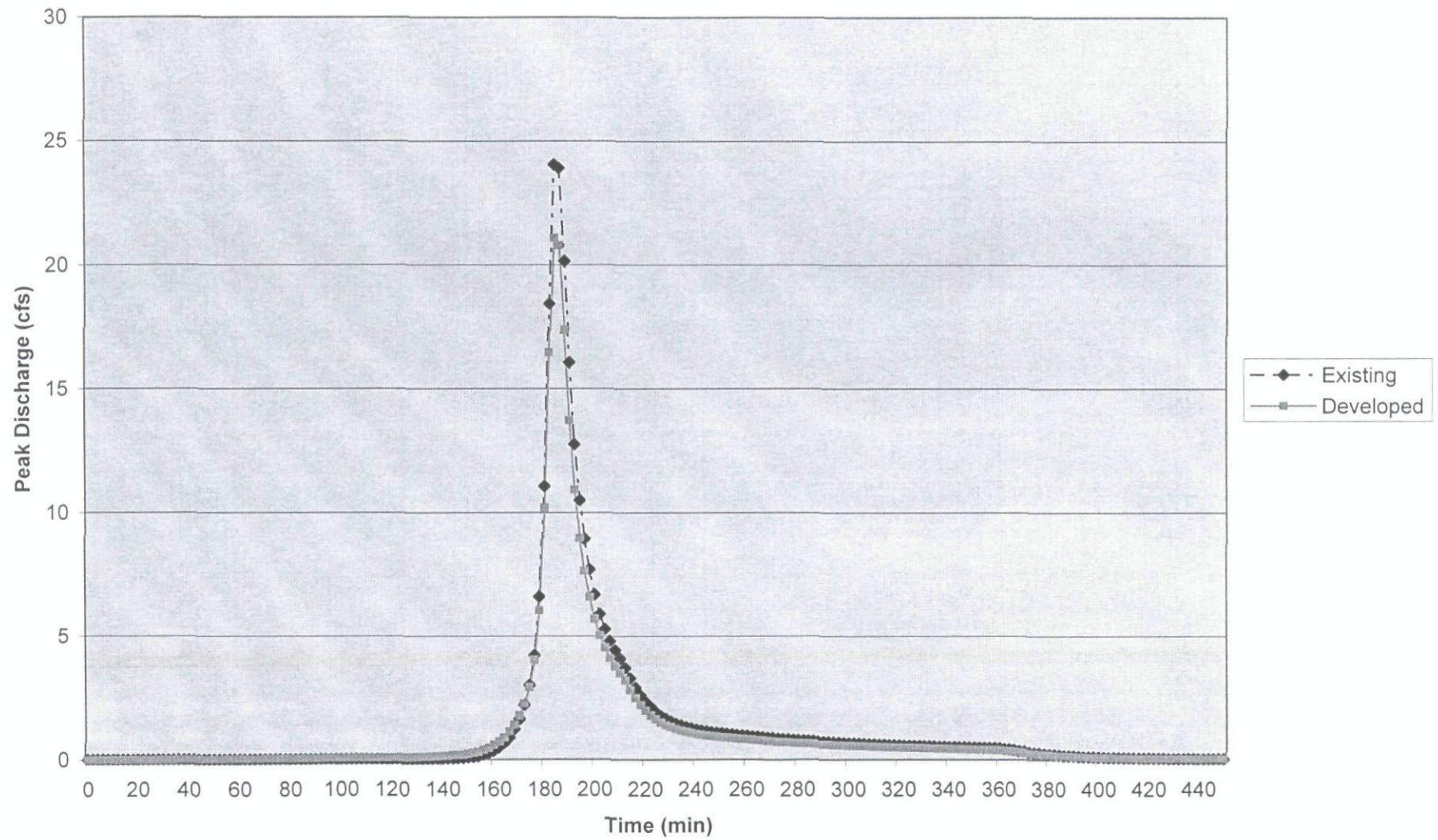
Hydrograph - N



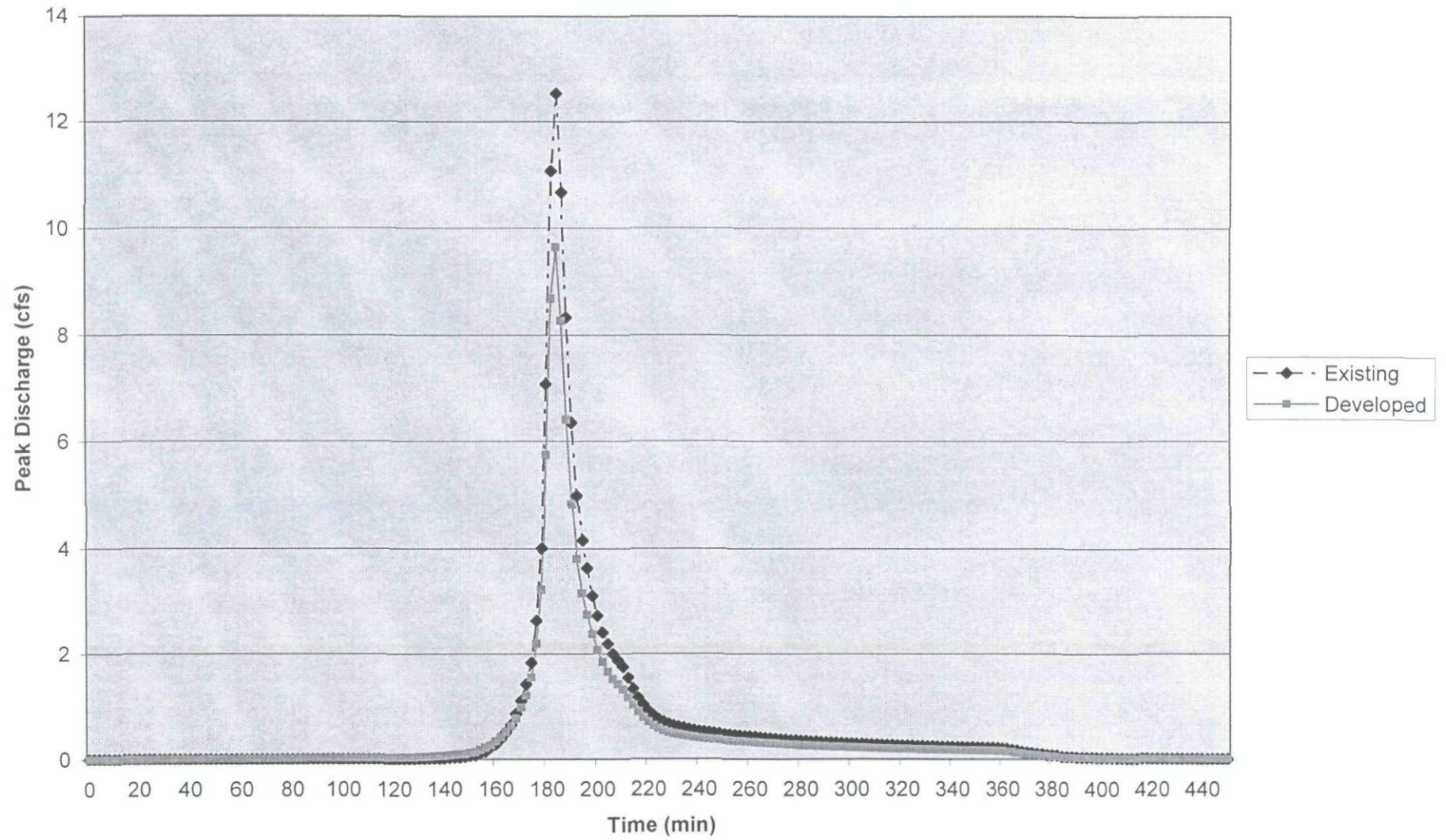
Hydrograph - O



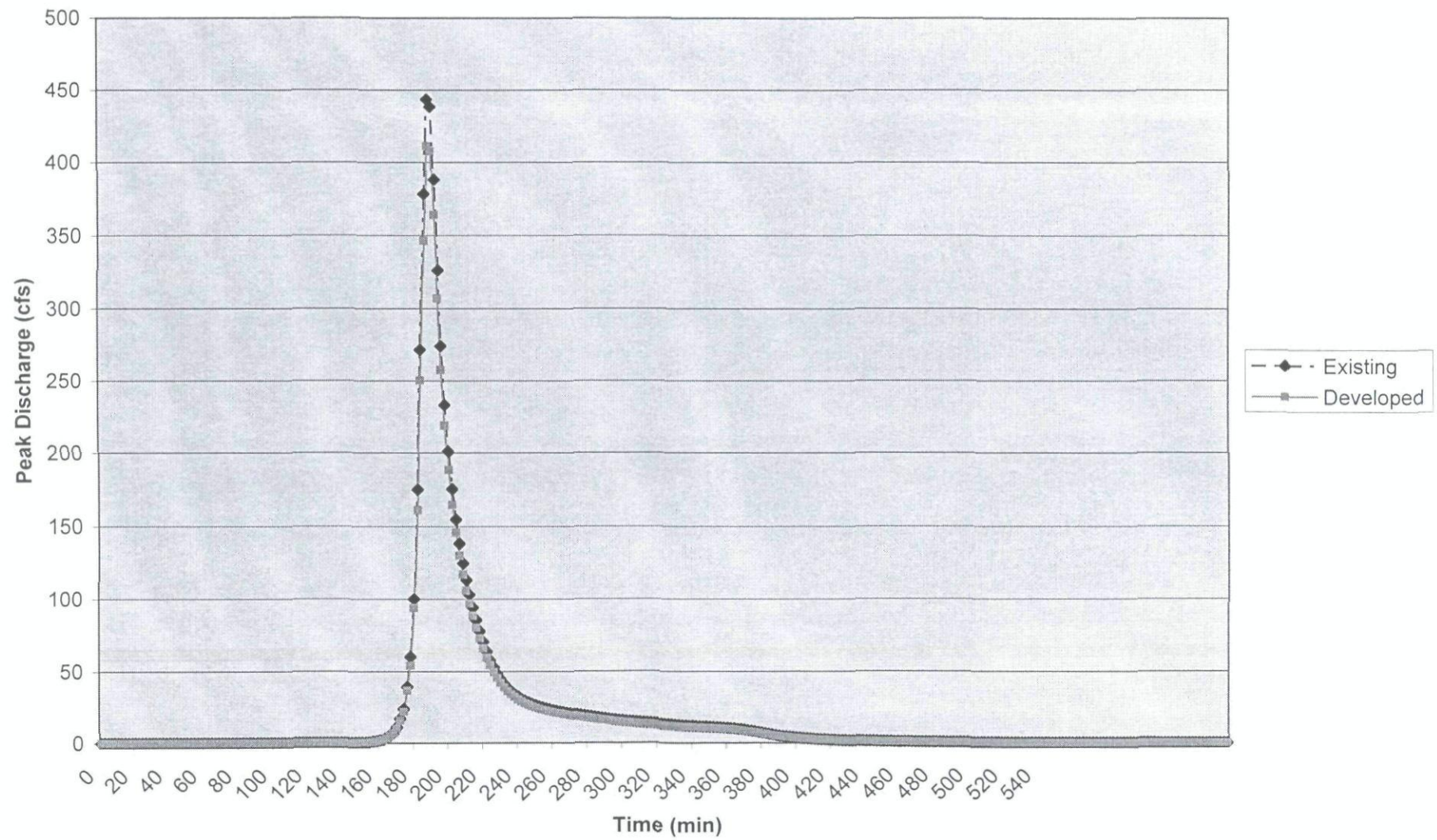
Hydrograph - P



Hydrograph - Subbasin Q



Hydrograph - Subbasin R



APPENDIX E

100-year Rational Method

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: SERENO CANYON

Location: City of Scottsdale

Date: 09-May-06

References: Drainage Design Manual for Maricopa County, Volume I-Hydrology
City of Scottsdale Engineering & Design Standards

Project Number: 042054

Project Engineer: Gordon W. Wark, P.E.

RATIONAL METHOD CALCULATIONS: OFF-SITE FLOW

* Summary of Rational Method values using Flood Control District of Maricopa County DDMSW Software

Concentration Point	Area		"C" Value	Length of Area (ft)	2 - Year			10-Year			100-Year		
	(sq ft)	(ac)			T _c (min)	Intensity (in/hr)	Q (cfs)	T _c (min)	Intensity (in/hr)	Q (cfs)	T _c (min)	Intensity (in/hr)	Q (cfs)
A1	2,502,397	57.4	0.56	4,754	16	2.5	65	13	4.5	116	11	7.4	238
A2	989,311	22.7	0.56	2,100	10	3.2	33	8	5.4	55	7	88.0	112
B	2,153,597	49.4	0.56	3,200	15	2.6	58	12	4.6	102	10	7.6	210
H2	1,998,065	45.9	0.56	3,350	15	2.6	54	12	4.6	95	10	7.5	193
I	700,376	16.1	0.56	2,012	11	3.0	22	9	5.1	37	8	8.4	76
J	678,947	15.6	0.56	920	6	3.8	27	5	6.3	44	5	9.9	86
R	4,238,208	97.3	0.56	4,300	16	2.5	109	13	4.5	197	10	7.5	409

Notes:

"C" value is based on hydrologic soil group D from Figure 4-5 in the City of Scottsdale DSPM, according to the SCS Classification of Soil #61, Gran Wickenburg.

Flood Control District of Maricopa County
042054 - Sereno Canyon
Sub Basin Data - Rational Method

Page 1

5/11/2006

Sub Basin	Sub Basin Parameters					Return Period (Years)					
	Area (acres)	Length (ft)	Slope (ft/ft)	Kb		2	5	10	25	50	100
A1	57.40	4,754	0.0747	0.056	Q (cfs)	65	96	116	164	205	238
A2	22.70	2,100	0.0833	0.061	Q (cfs)	33	46	55	77	97	112
B	49.40	3,200	0.0469	0.057	Q (cfs)	58	84	102	143	179	210
H2	45.90	3,350	0.0493	0.057	Q (cfs)	54	76	95	133	166	193
I	16.10	2,012	0.0572	0.063	Q (cfs)	22	30	37	52	65	76
J	15.60	920	0.0761	0.064	Q (cfs)	27	37	44	61	75	86
R	97.30	4,300	0.0605	0.053	Q (cfs)	109	162	197	277	347	409

APPENDIX F

Hydraulic Calculations

- Storage Rating Curves
- HEC-RAS Output Files
- Orifice Plate Calculation

Culvert Rating Curves for Stage-Storage Intervals

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: *Sereno Canyon*
Location: *City of Scottsdale*
Reference: *Rectangular Weir Equation*

W/P#: 042054
Date: 5/11/2006

$$Q = CLH^{3/2}$$

Where:

C = Weir Coefficient =	2.80	
L = Length of Weir =	10.00	ft
H = Head above Weir =	1.50	ft
Q = Weir Flow =	51.44	cfs

* The above equation is used to determine the rating curves for weir flow

WOOD/PATEL

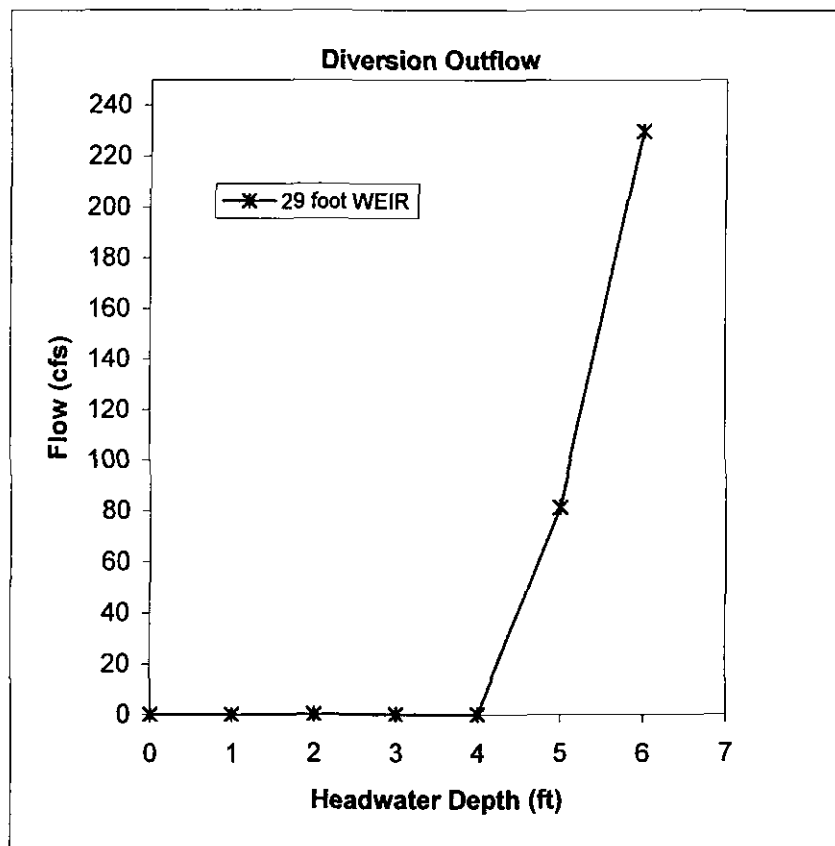
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: *Sereno Canyon*
Location: *City of Scottsdale*

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BASA1

29	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	0
5	81
6	230



WOOD/PATEL

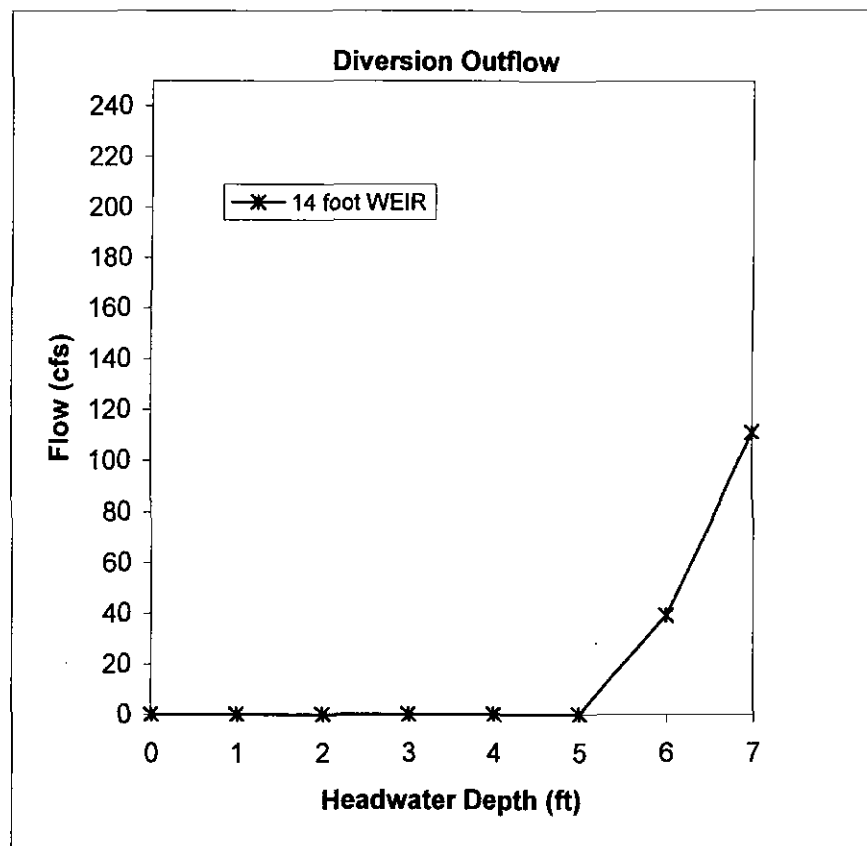
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: *Sereno Canyon*
Location: *City of Scottsdale*

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS A2-3

14	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	0
5	0
6	39
7	111



WOOD/PATEL

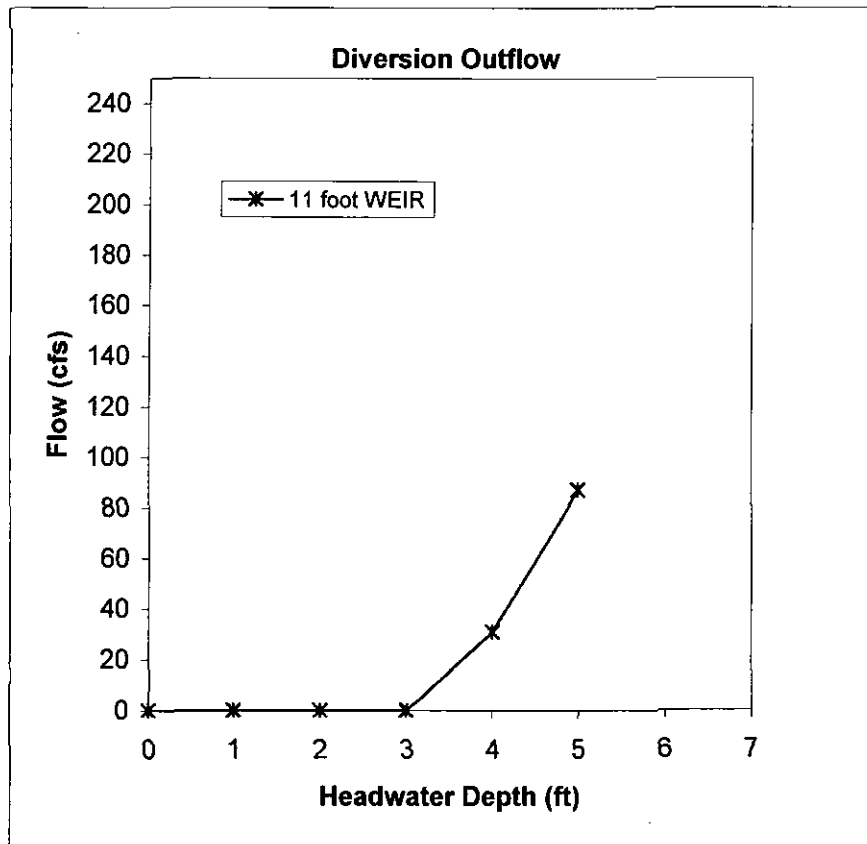
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS B-3

11	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	31
5	87



WOOD/PATEL

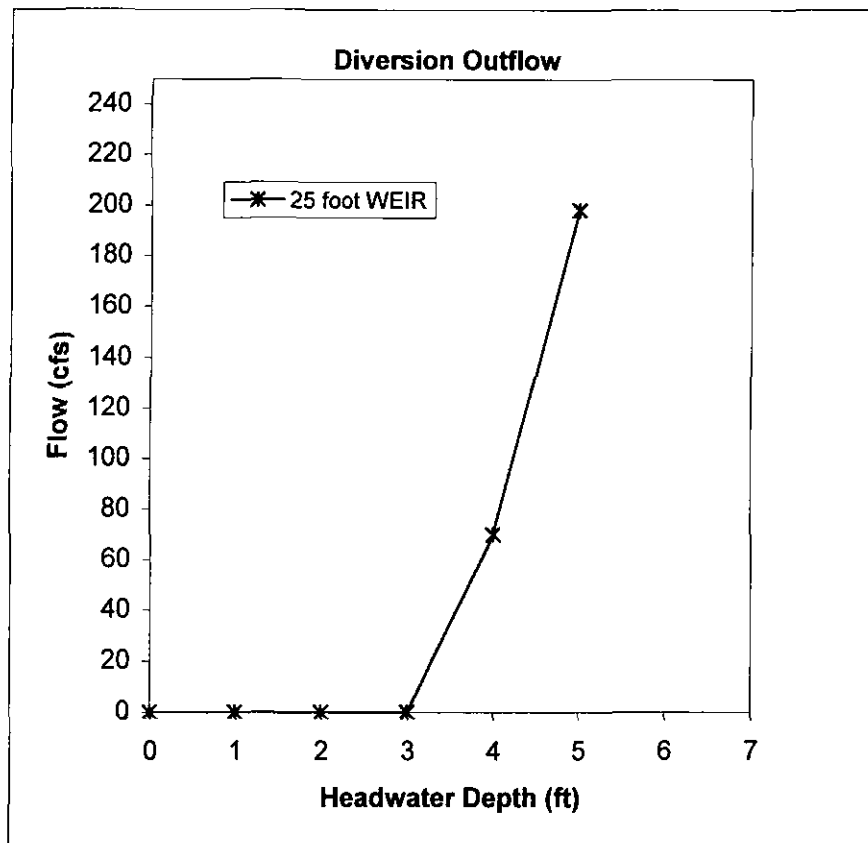
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS B-4

25	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	70
5	198



WOOD/PATEL

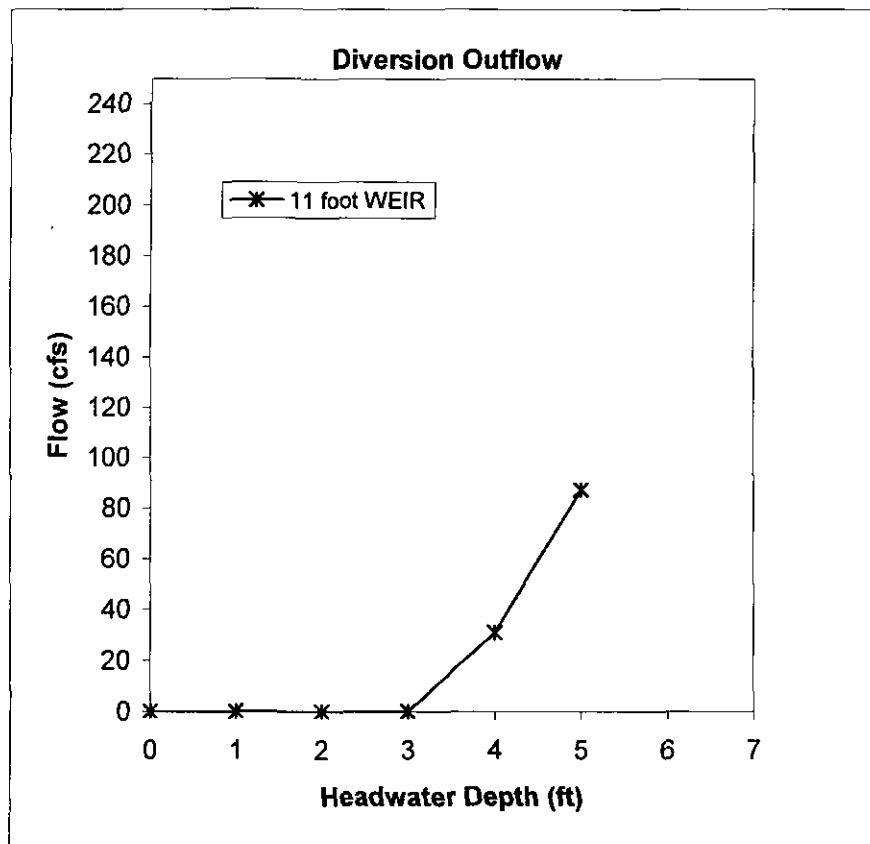
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS C-1

11	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	31
5	87



WOOD/PATEL

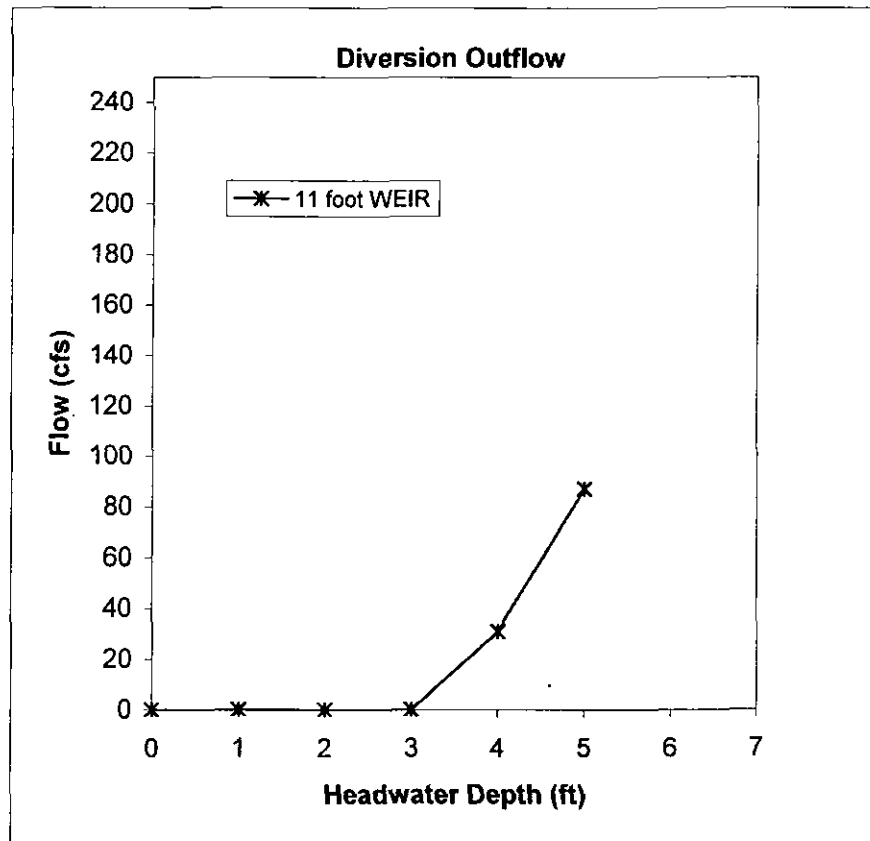
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

W/P#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS D-1

11	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	31
5	87



WOOD/PATEL

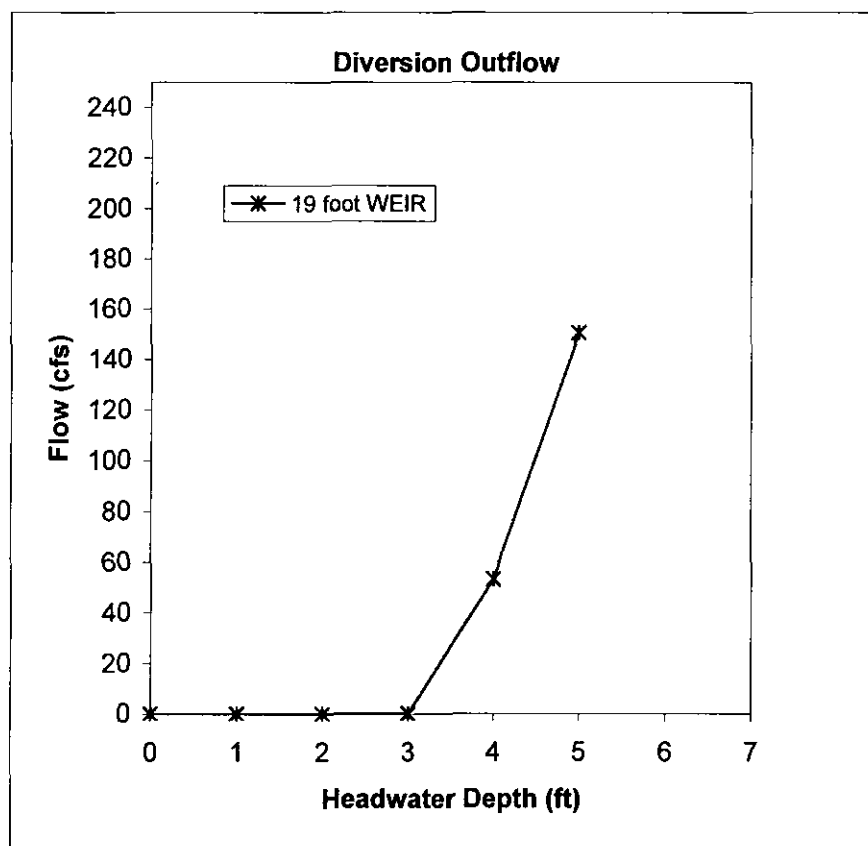
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS E1-2

19	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	53
5	150



WOOD/PATEL

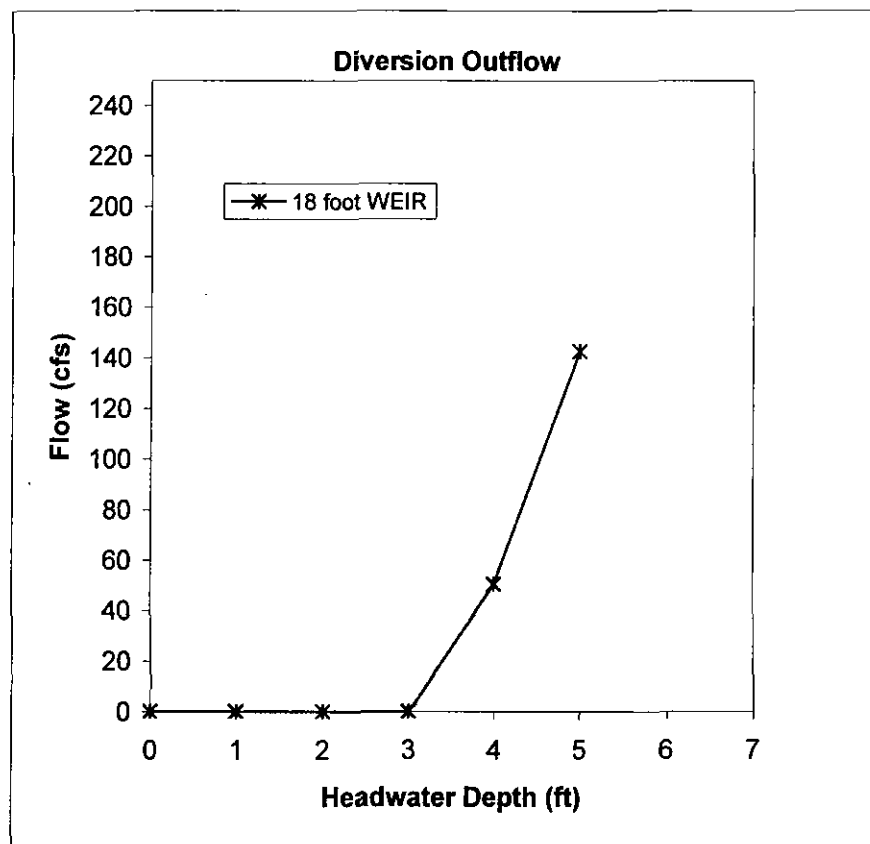
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS E2-1

18	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	50
5	143



WOOD/PATEL

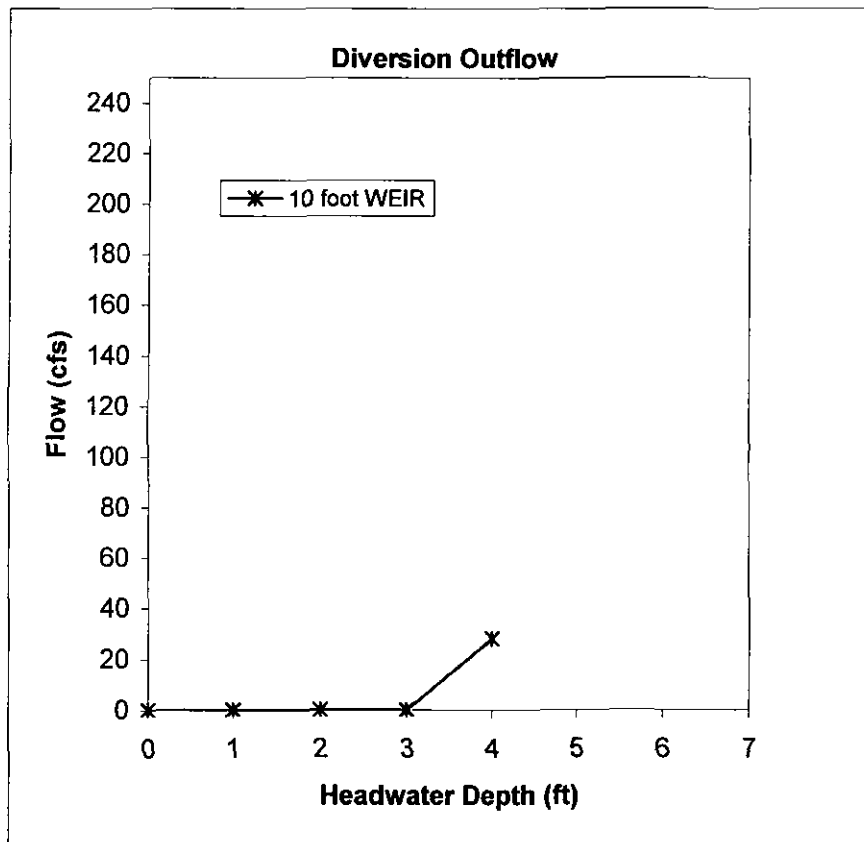
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: *Sereno Canyon*
Location: *City of Scottsdale*

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS E3-1

10	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	28



WOOD/PATEL

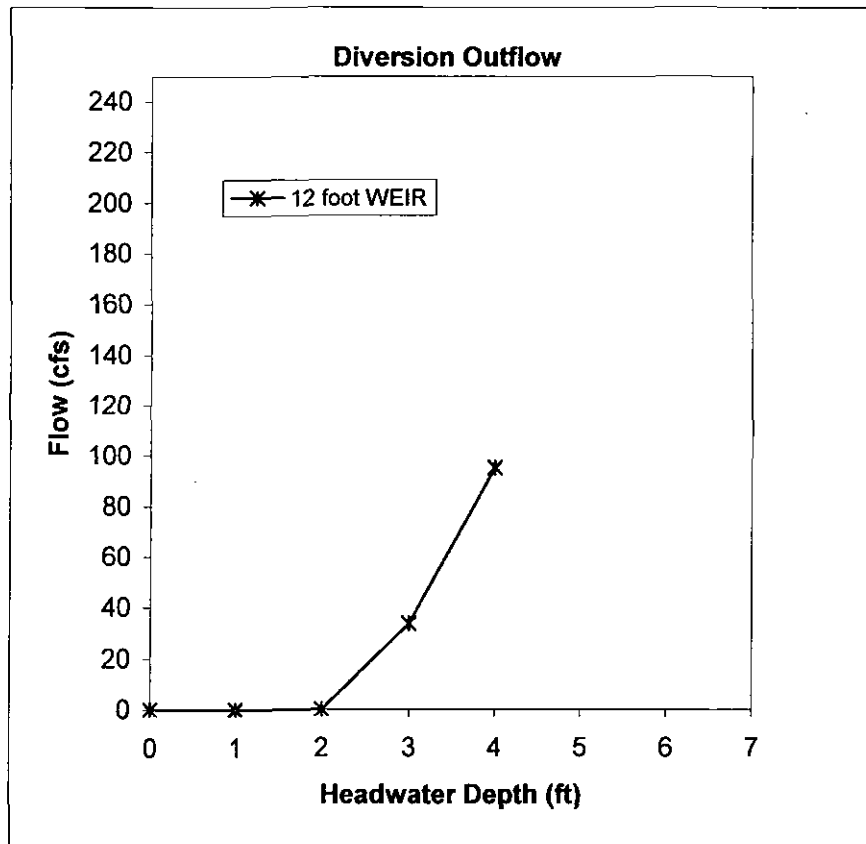
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS F1-3

12	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	34
4	95



WOOD/PATEL

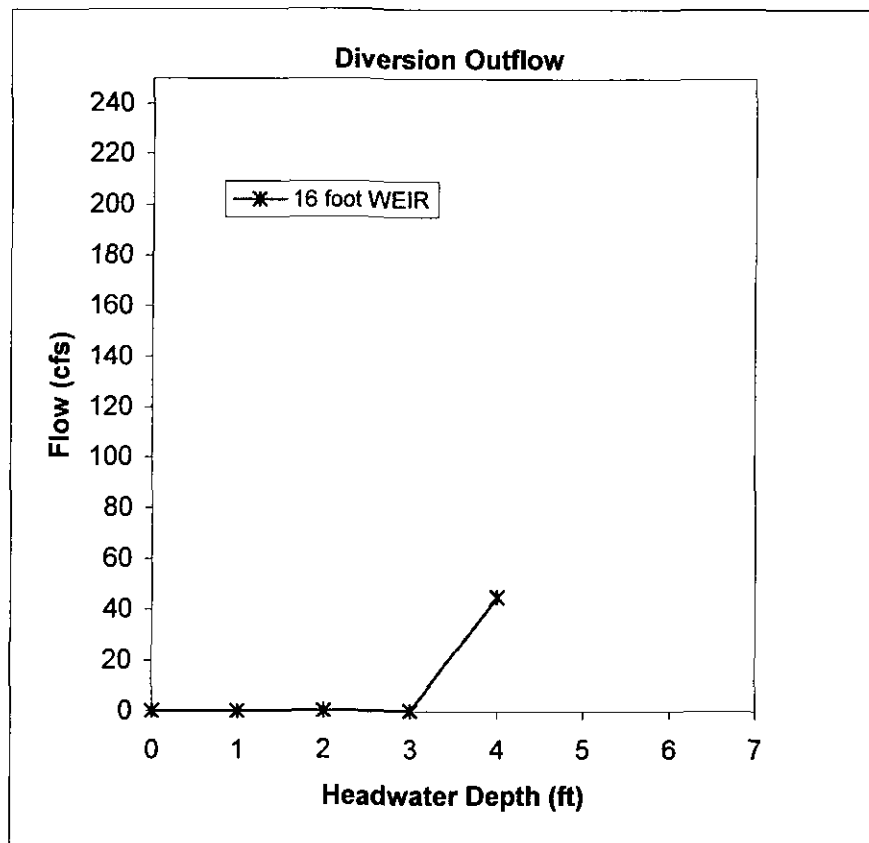
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: *Sereno Canyon*
Location: *City of Scottsdale*

W/P#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS F2-2

16	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	45



WOOD/PATEL

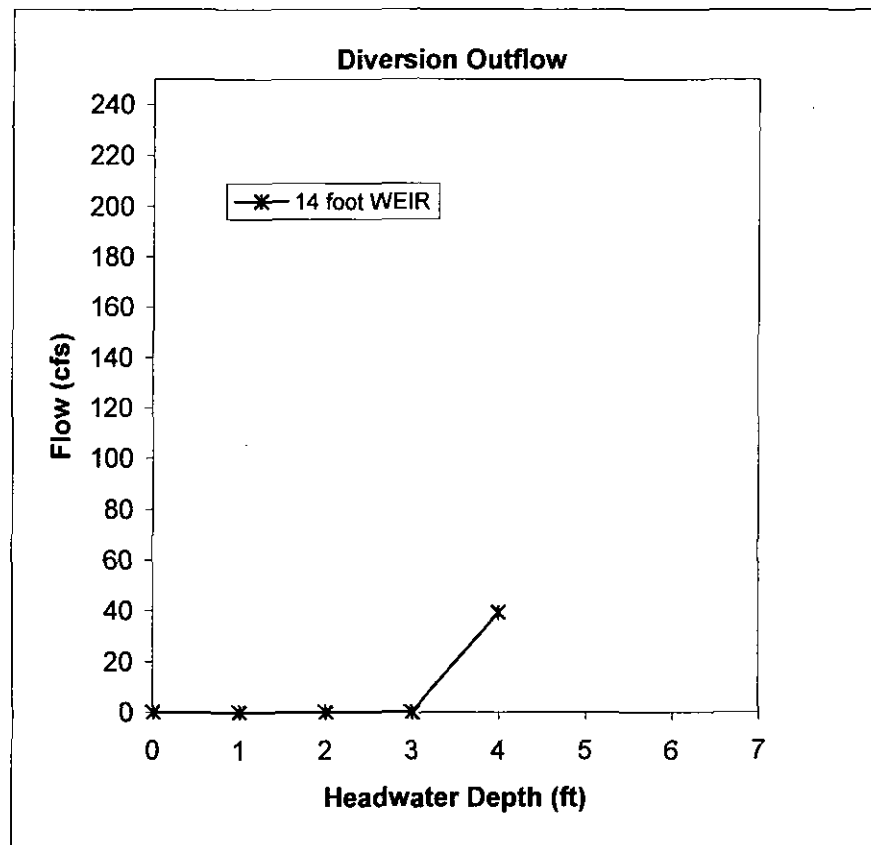
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS F3-1

14	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	39



WOOD/PATEL

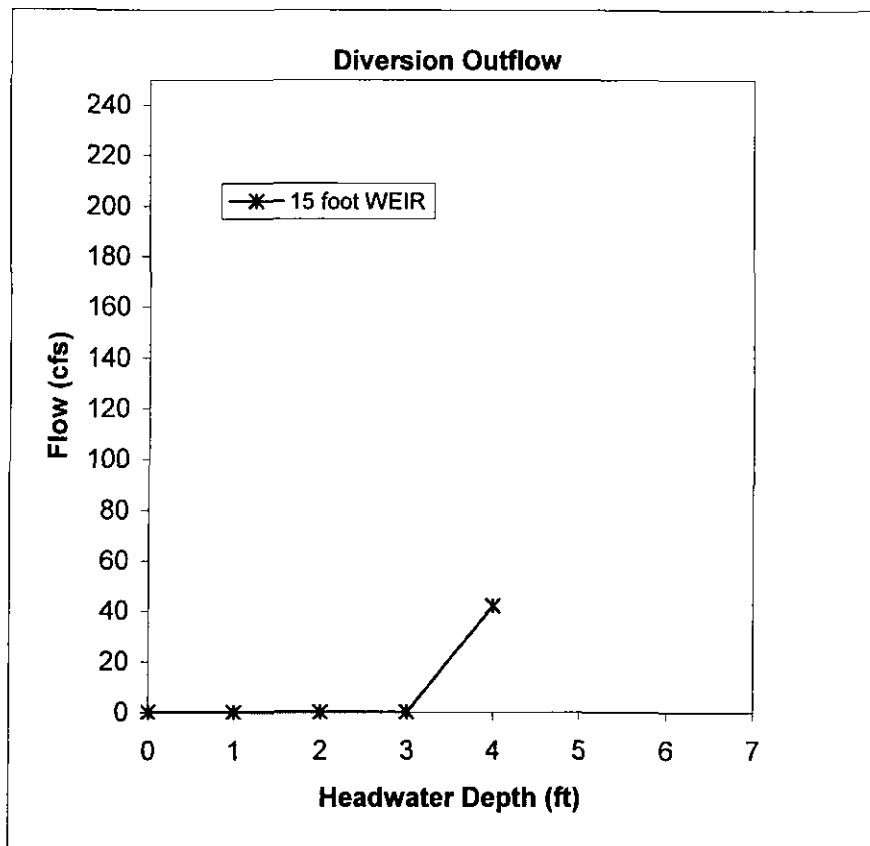
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS G

15	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	42



WOOD/PATEL

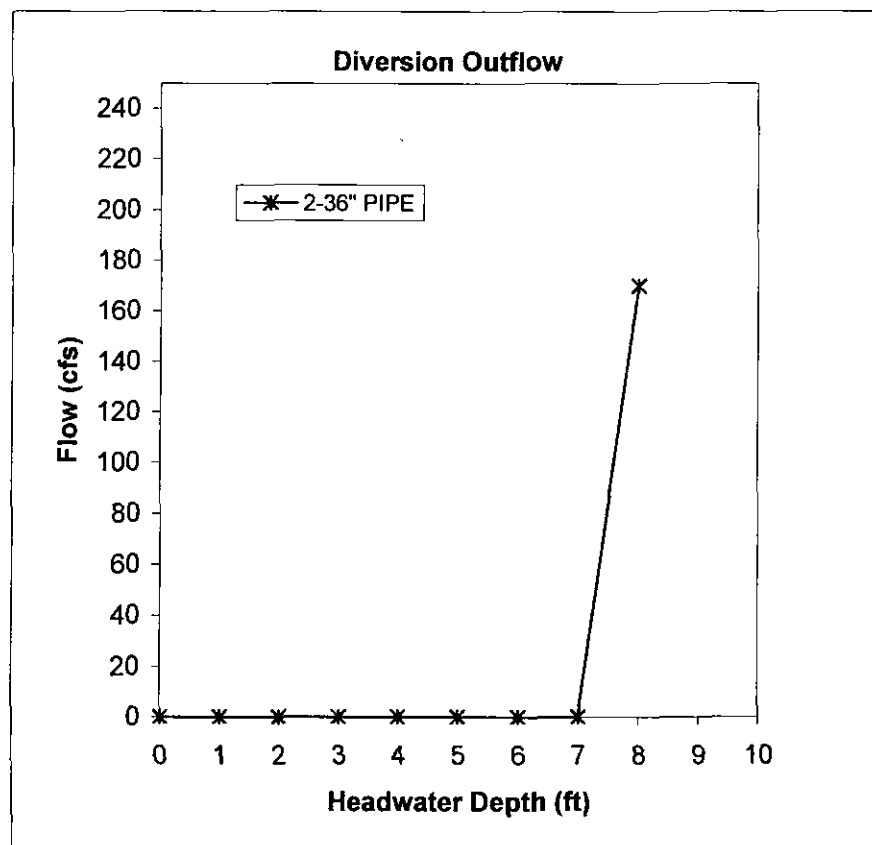
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

W/P#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS H1-1

2-36" PIPE	
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	0
5	0
6	0
7	0
8	170



WOOD/PATEL

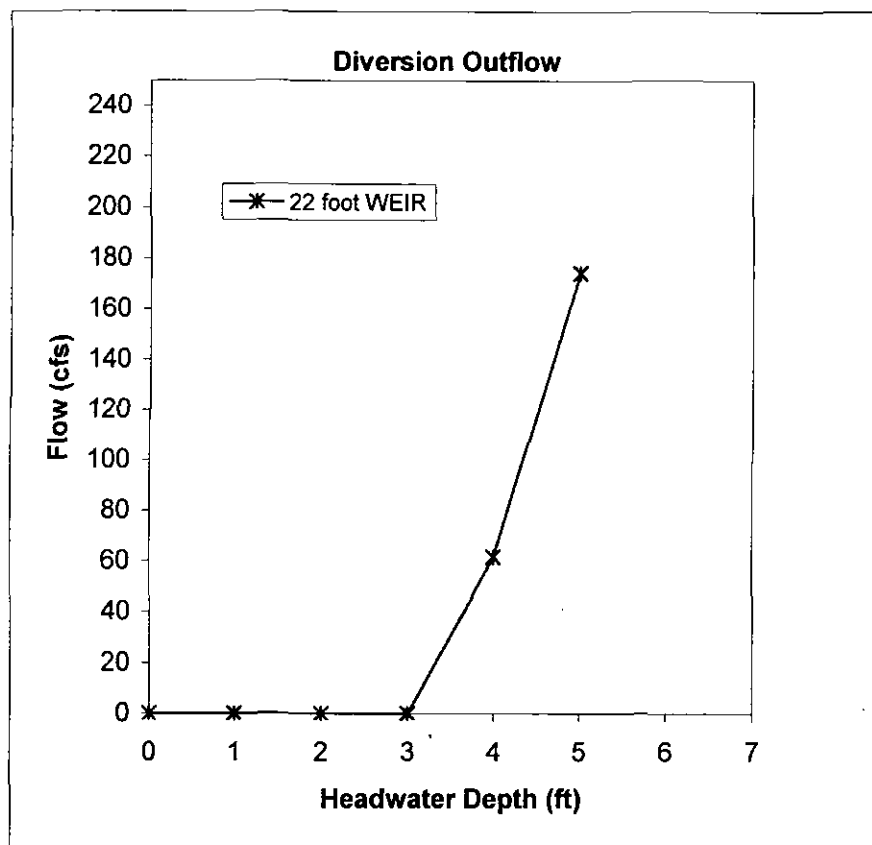
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS H2-1

22	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	62
5	174



WOOD/PATEL

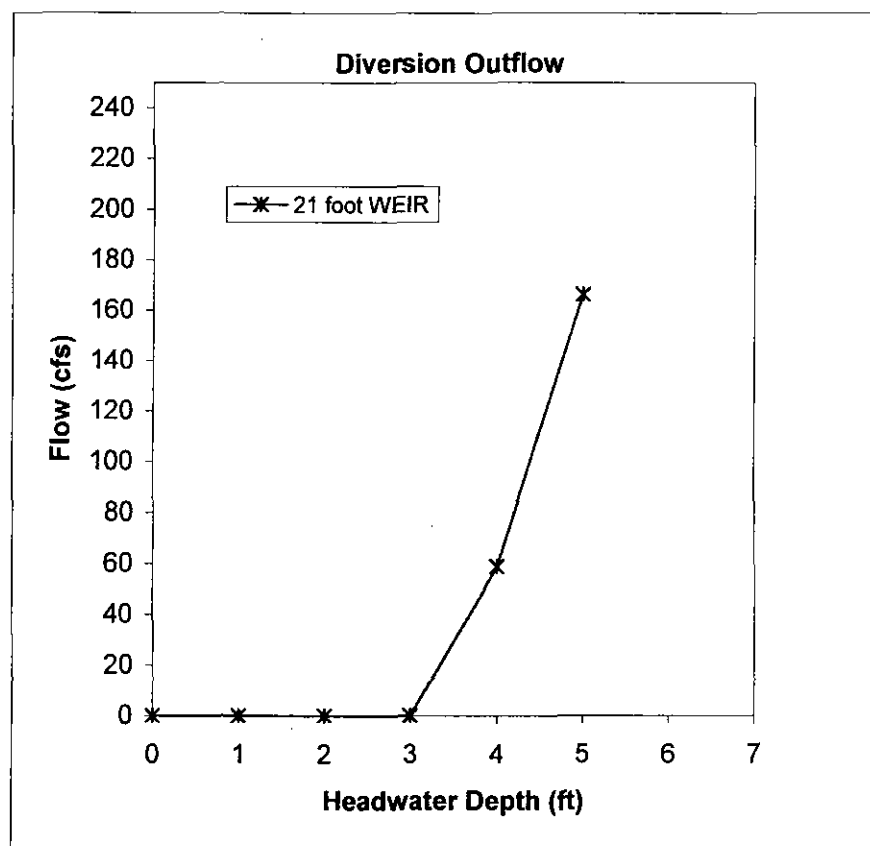
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS H2-2

21	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	59
5	166



WOOD/PATEL

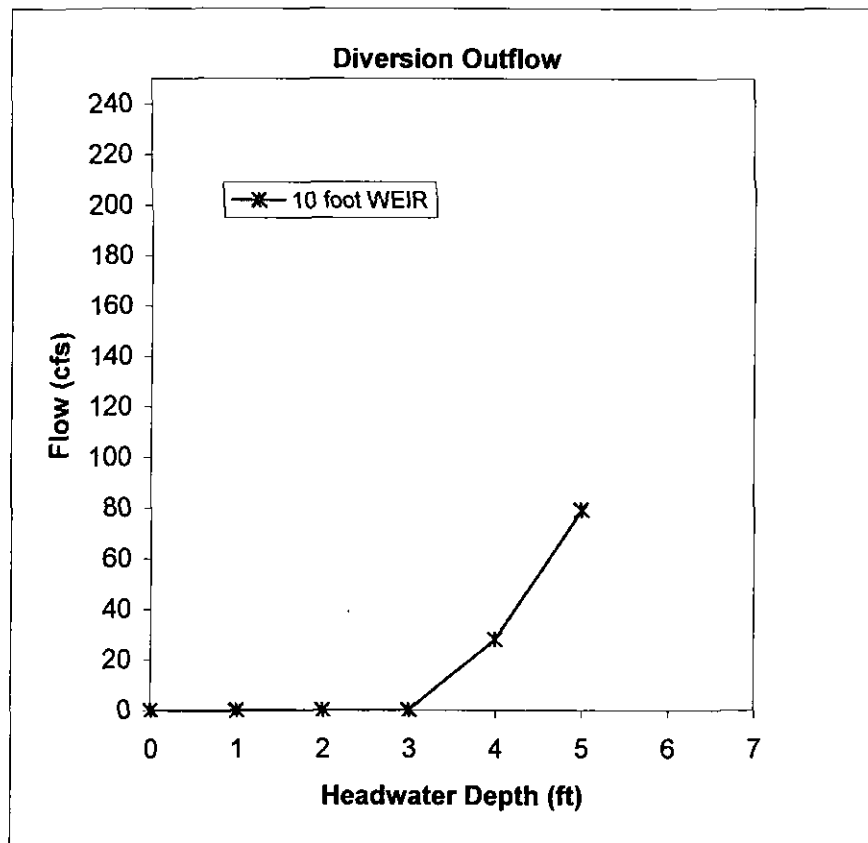
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS I

10	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	28
5	79



WOOD/PATEL

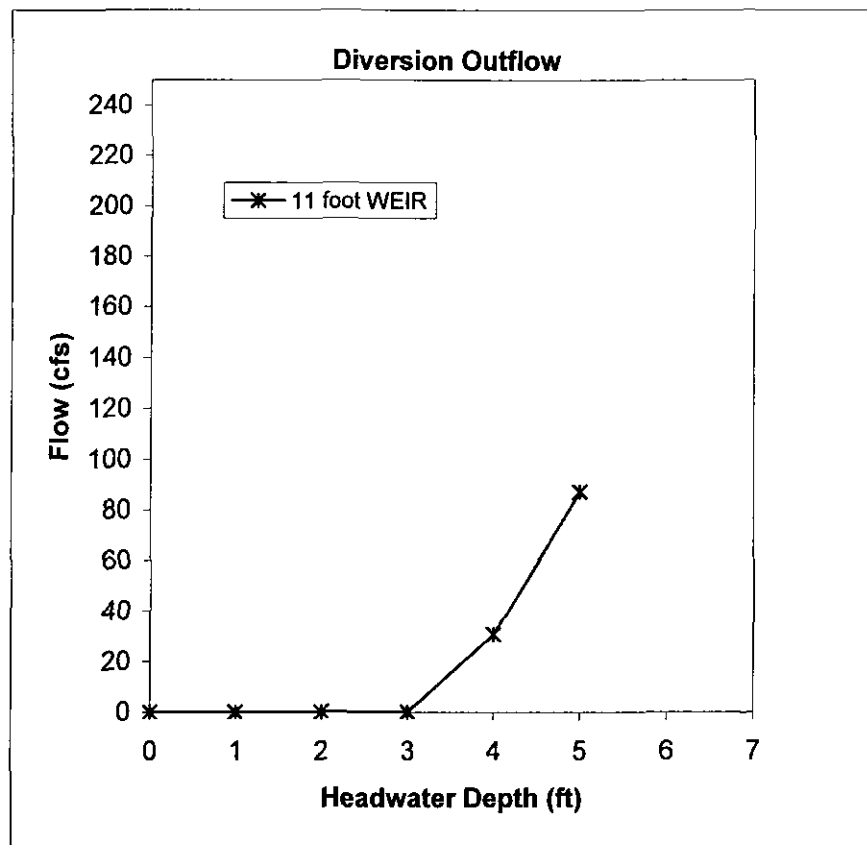
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: *Sereno Canyon*
Location: *City of Scottsdale*

W/P#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS J

11	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	31
5	87



WOOD/PATEL

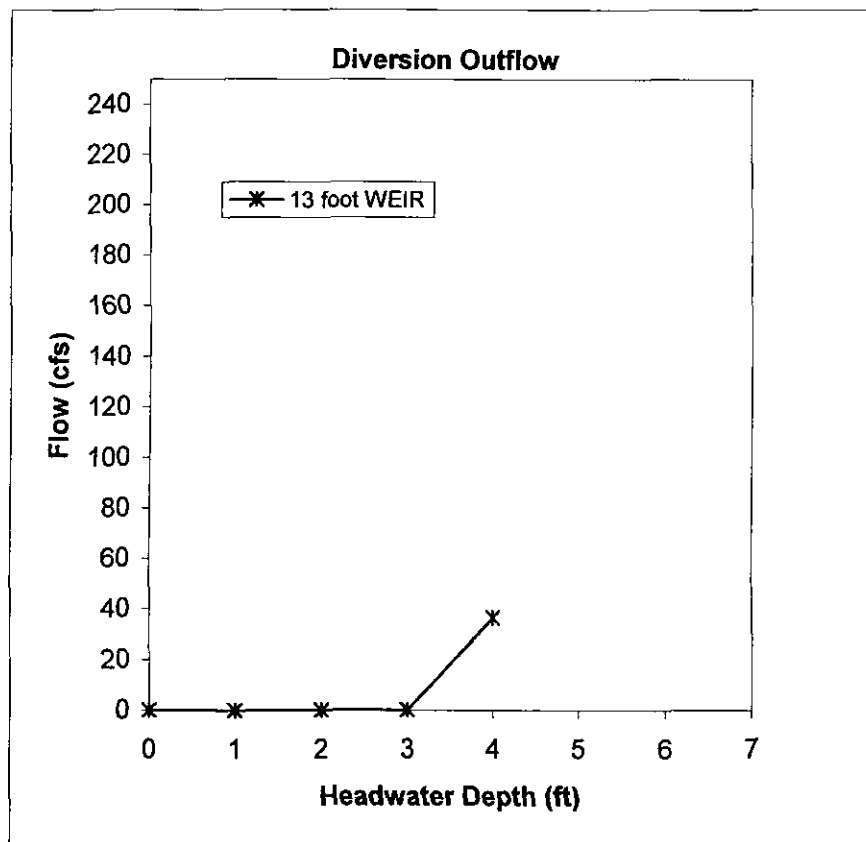
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS K

13	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	36



WOOD/PATEL

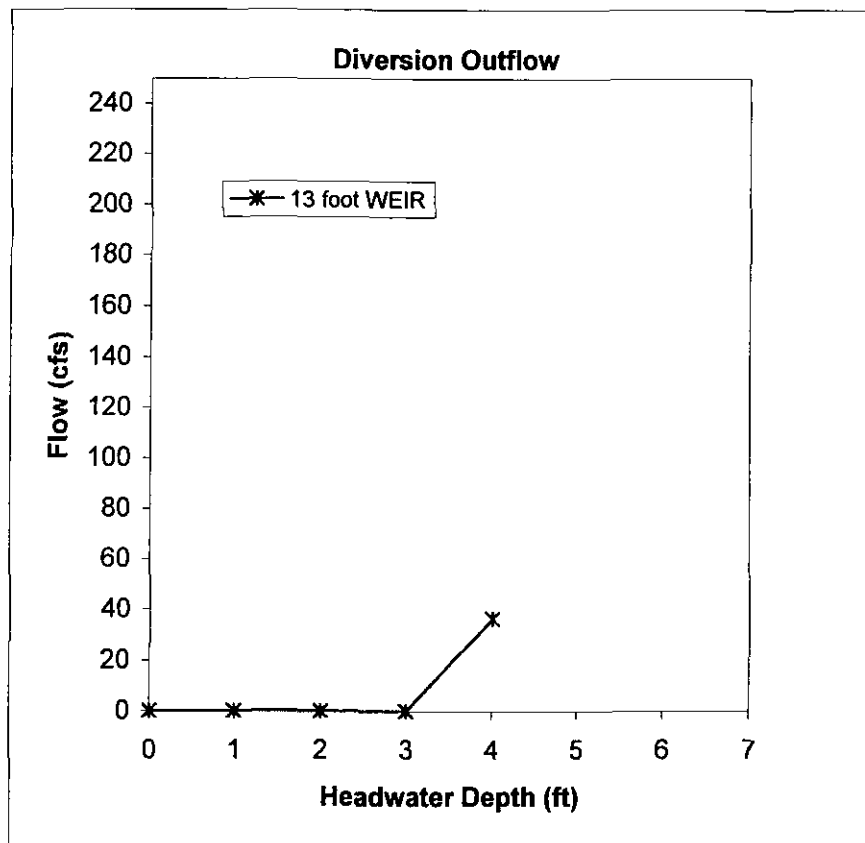
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: *Sereno Canyon*
Location: *City of Scottsdale*

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS L

13	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	36



WOOD/PATEL

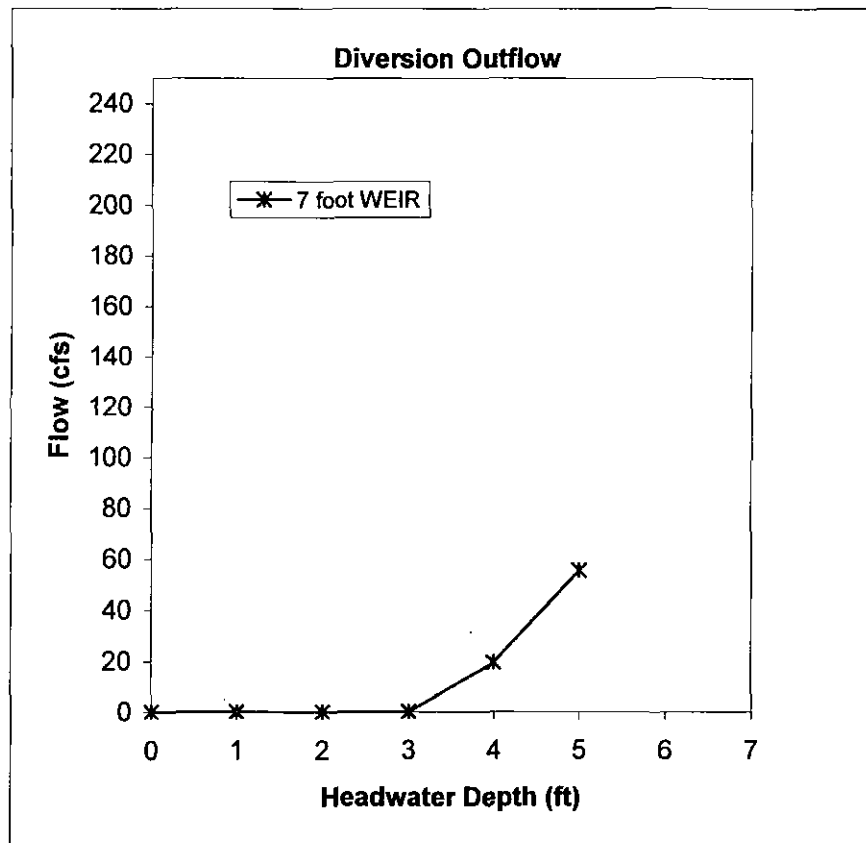
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

W/P#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS M

7	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	20
5	55



WOOD/PATEL

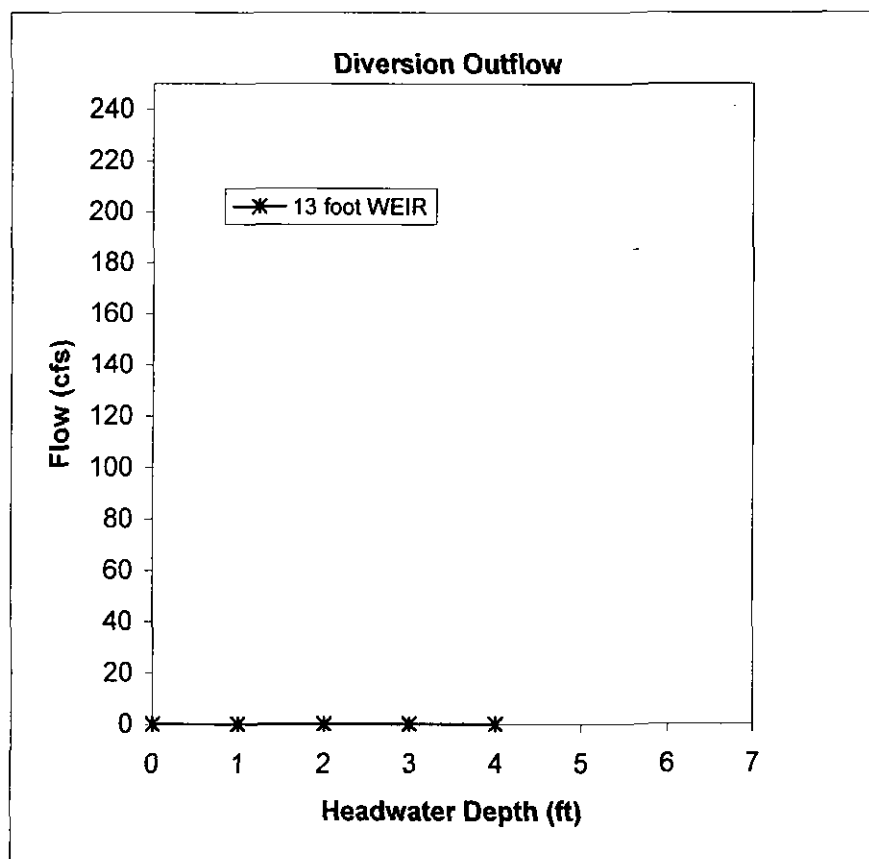
CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale

WIP#: 042054
Date: 11/15/2006

Rating Curve for Diversion BAS N

13	foot WEIR
Headwater Depth	Flow
ft	cfs
0	0
1	0
2	0
3	0
4	0
5	36



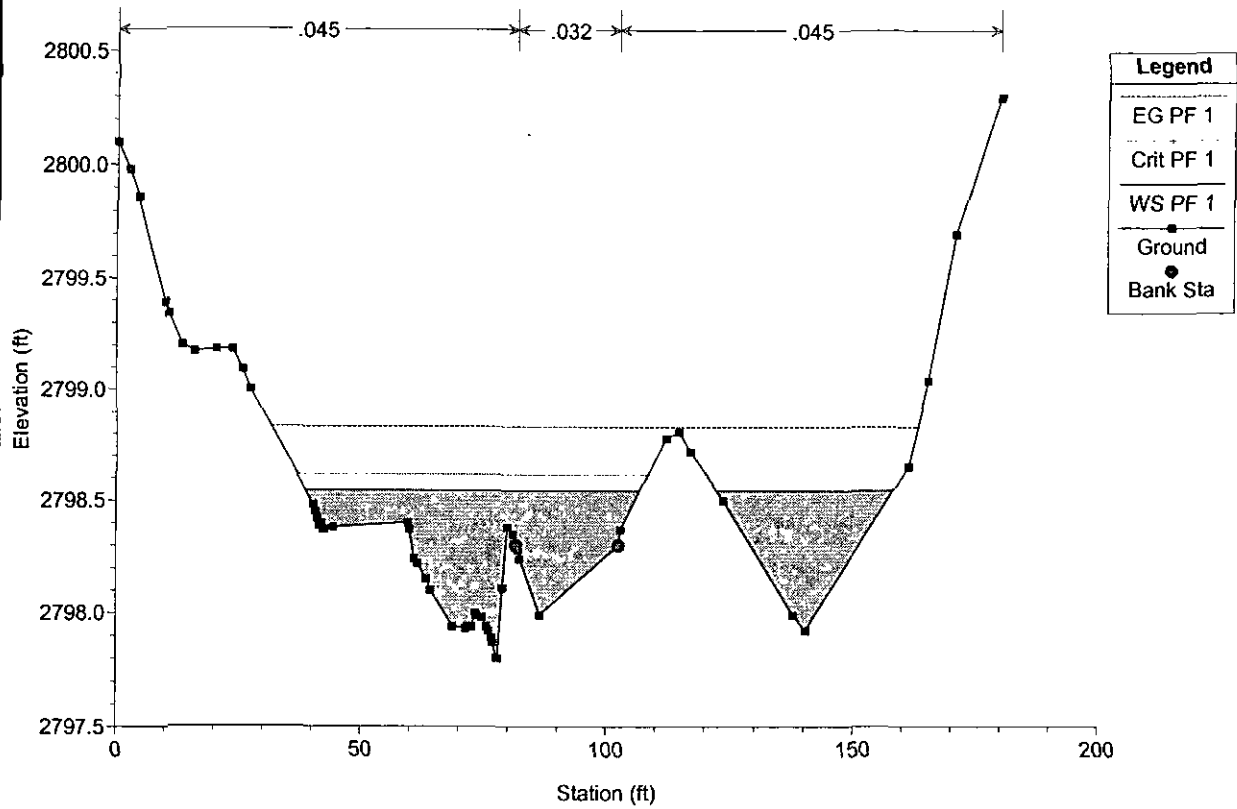
HEC-RAS Output Files

WASH A1

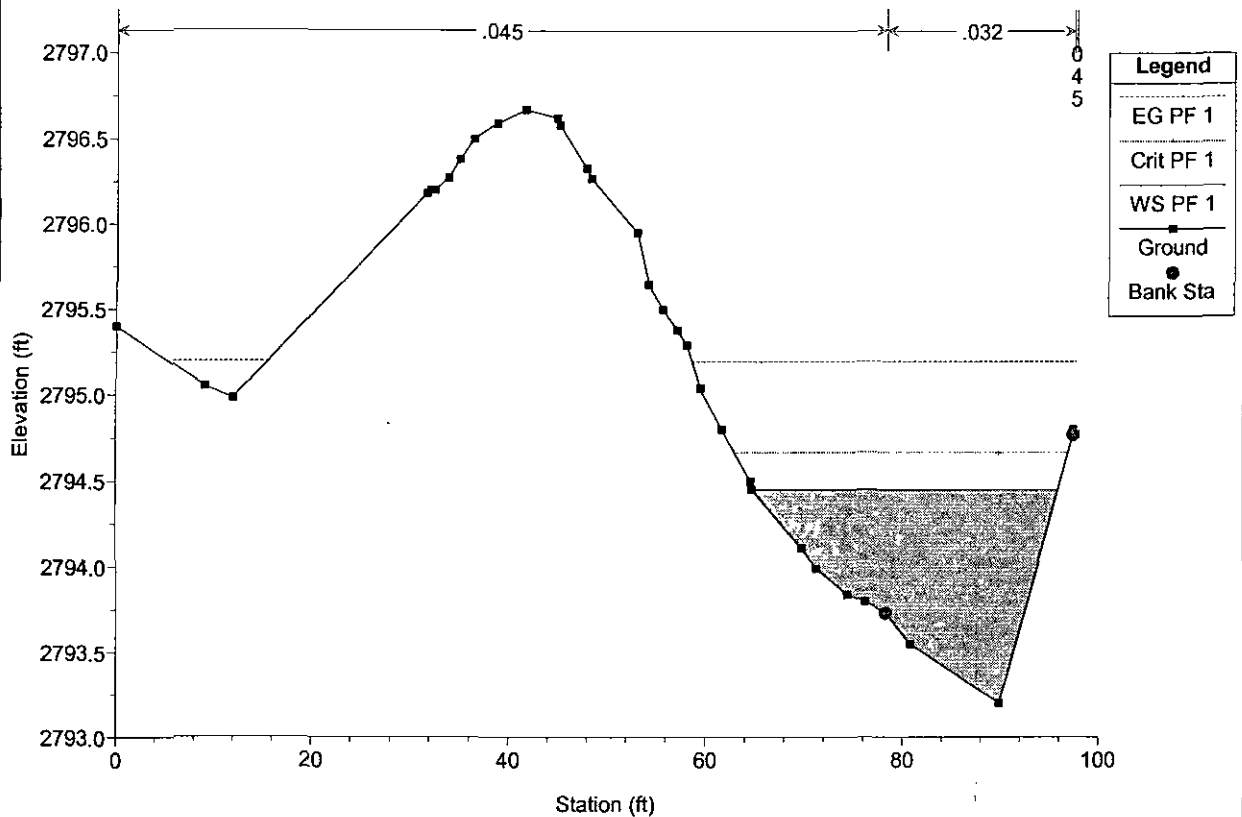
HEC-RAS Plan: wasa1 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	6	PF 1	133.00	2797.99	2798.54	2798.61	2798.83	0.050016	5.63	33.66	103.73	1.57
Reach-1	5	PF 1	133.00	2793.20	2794.45	2794.67	2795.20	0.029978	7.35	21.29	31.22	1.38
Reach-1	4	PF 1	133.00	2786.79	2787.89	2788.26	2789.23	0.093236	9.42	15.00	33.03	2.25
Reach-1	3.5	PF 1	133.00	2780.30	2781.30	2781.53	2782.07	0.042079	7.04	18.89	29.62	1.55
Reach-1	3	PF 1	133.00	2777.01	2778.00	2778.20	2778.73	0.041792	7.60	22.37	42.27	1.58
Reach-1	2.5	PF 1	133.00	2772.00	2772.83	2773.19	2773.87	0.067610	8.23	16.36	31.78	1.93
Reach-1	2	PF 1	235.00	2761.68	2763.52	2764.13	2765.42	0.052499	12.11	25.99	37.79	1.92
Reach-1	1.5	PF 1	235.00	2759.00	2760.14	2760.27	2760.65	0.033817	5.69	41.27	75.65	1.36
Reach-1	1	PF 1	235.00	2753.00	2754.30	2754.63	2755.50	0.047422	9.88	33.13	58.31	1.77

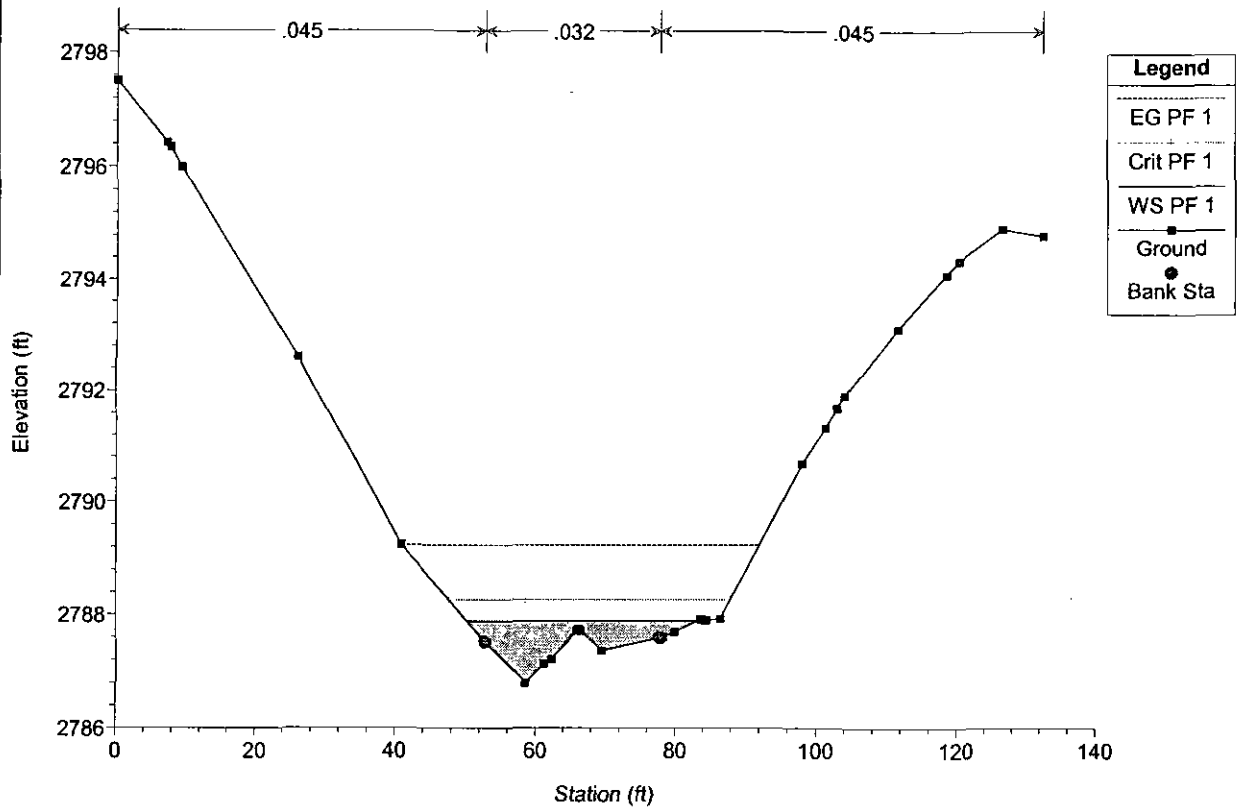
washa1-EX Plan: washa1 11/8/2006



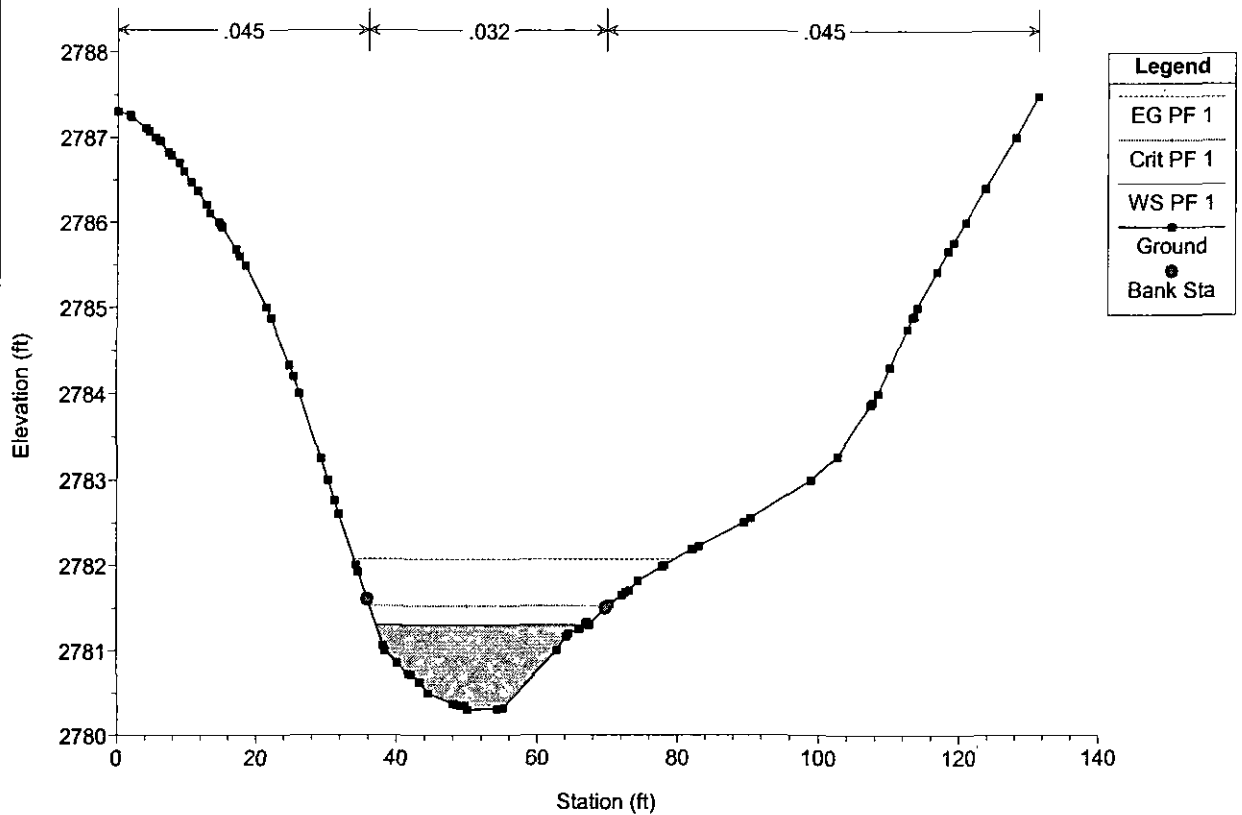
washa1-EX Plan: washa1 11/8/2006



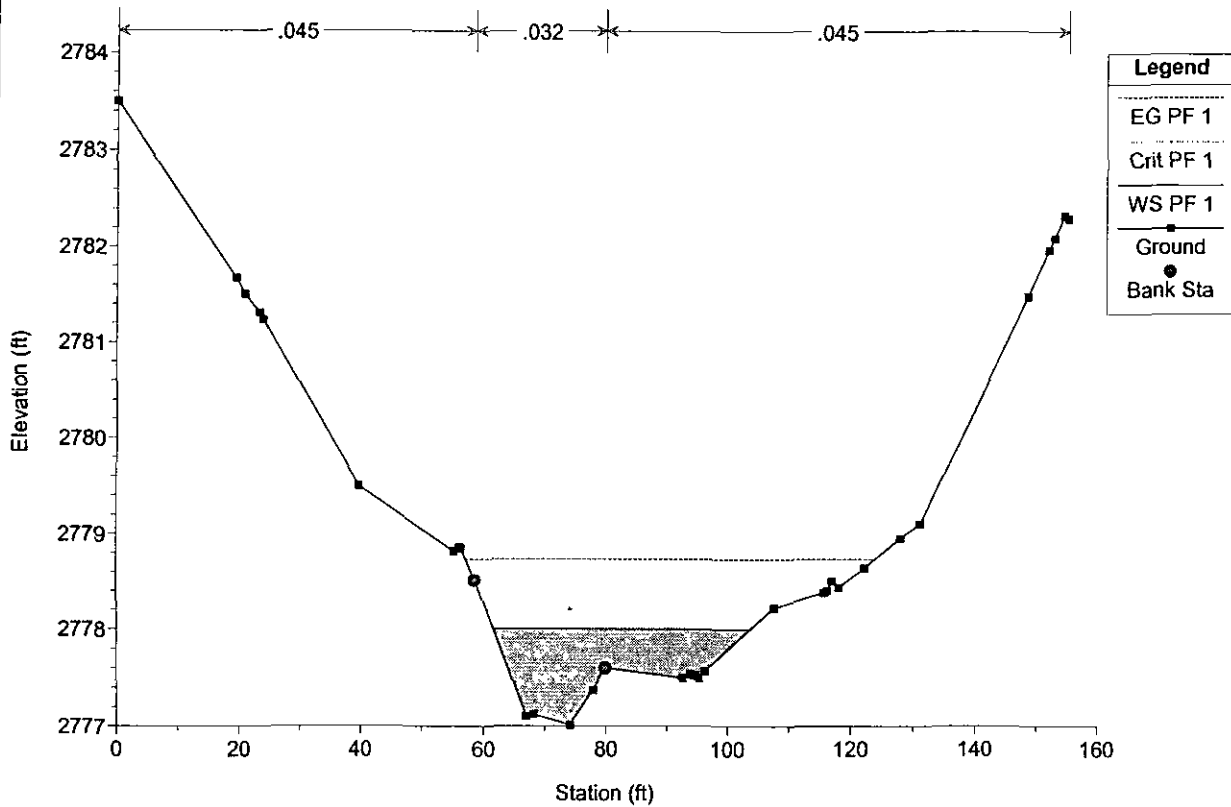
washa1-EX Plan: washa1 11/8/2006



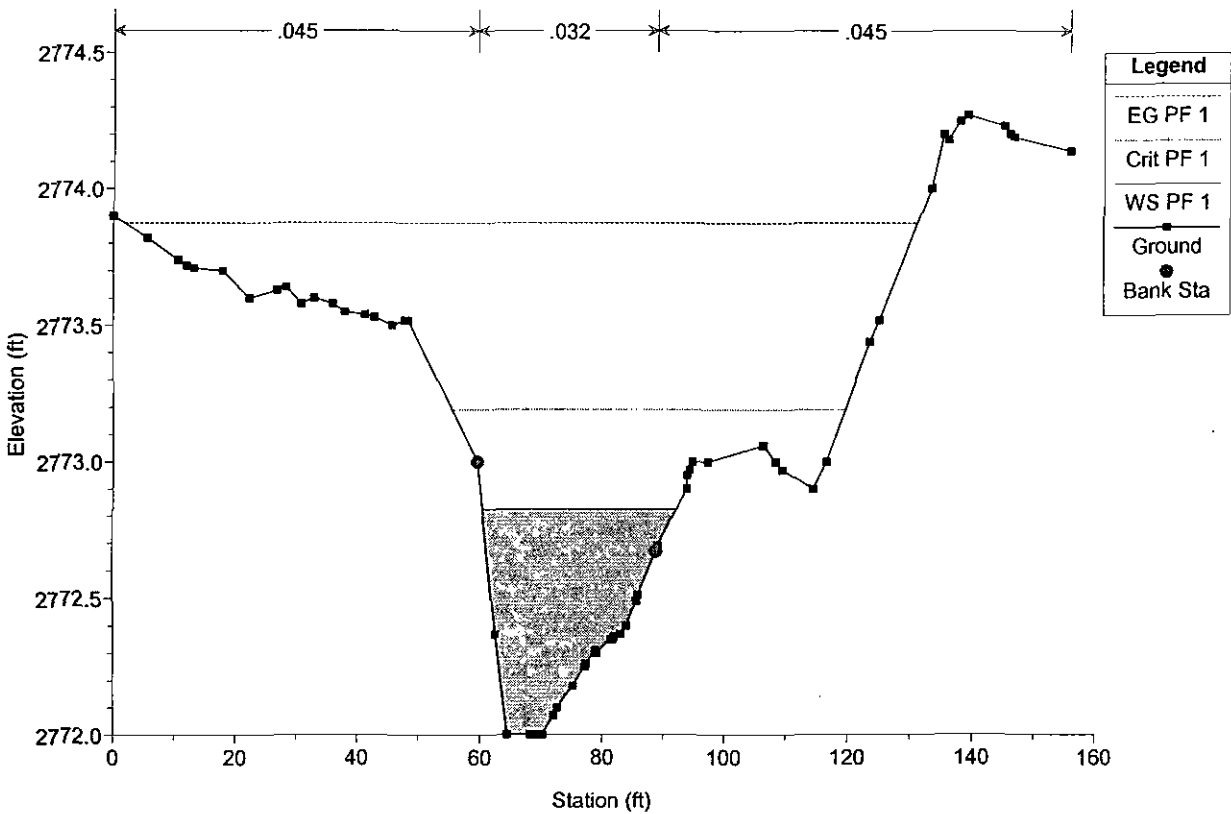
washa1-EX Plan: washa1 11/8/2006



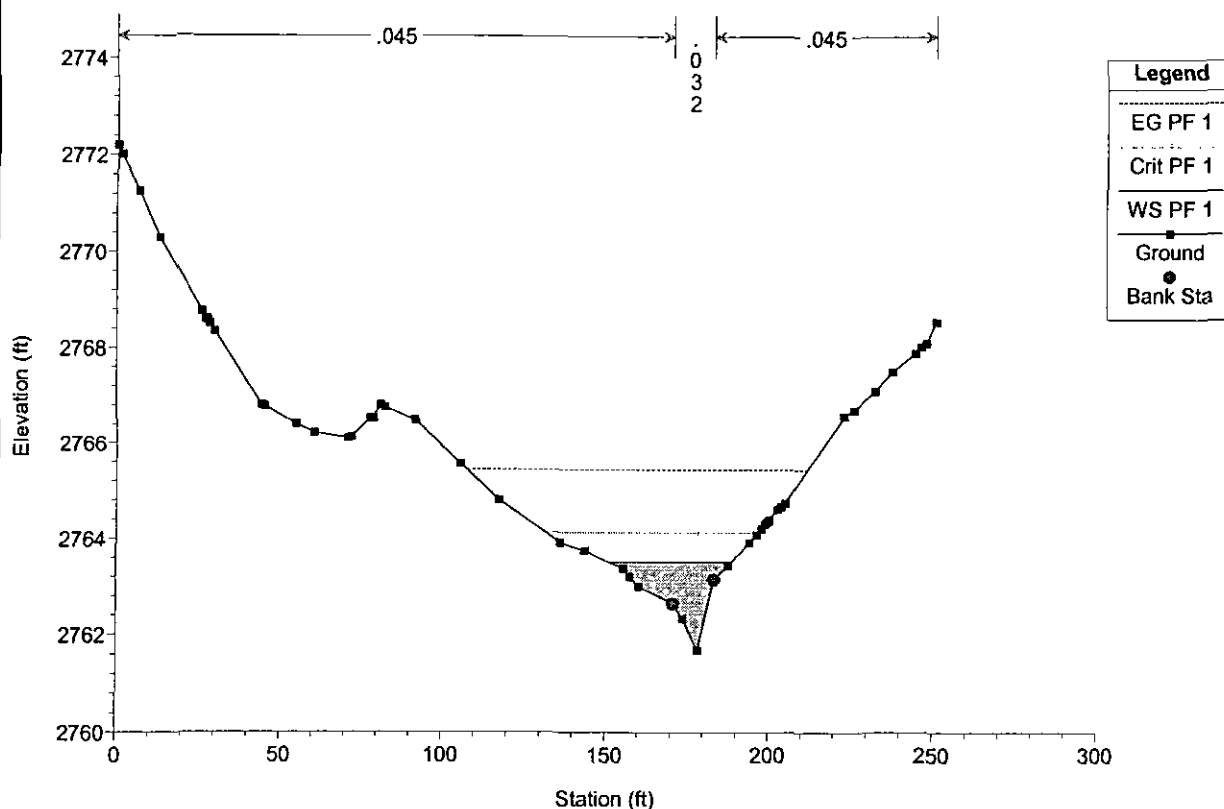
washa1-EX Plan: washa1 11/8/2006



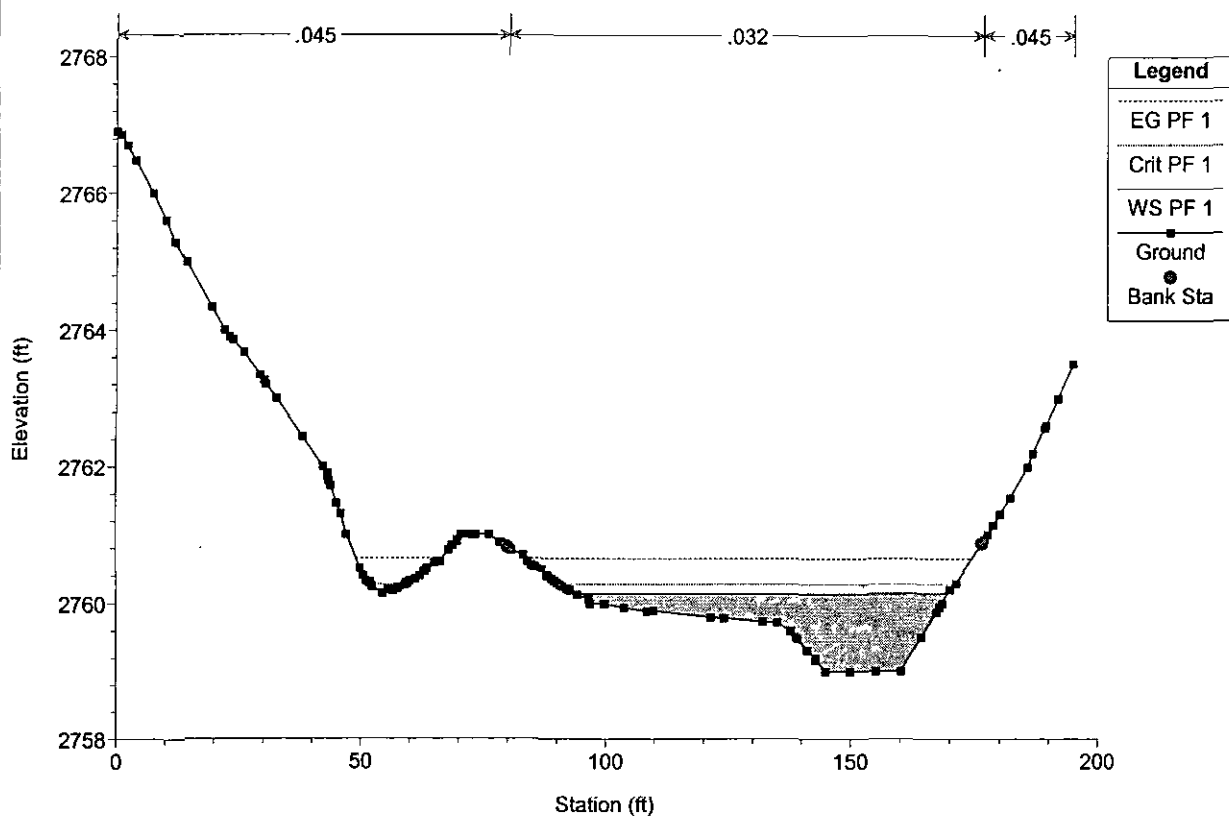
washa1-EX Plan: washa1 11/8/2006



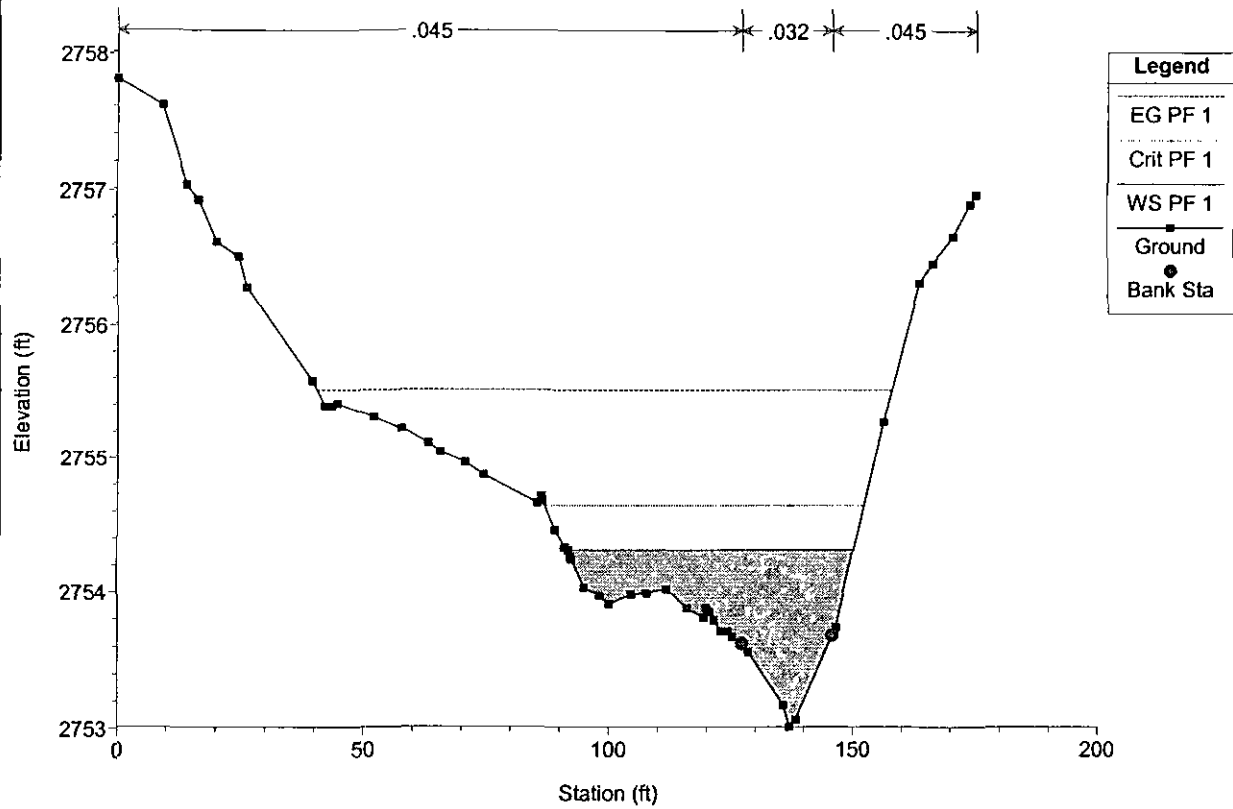
washa1-EX Plan: washa1 11/8/2006



washa1-EX Plan: washa1 11/8/2006



washa1-EX Plan: washa1 11/8/2006

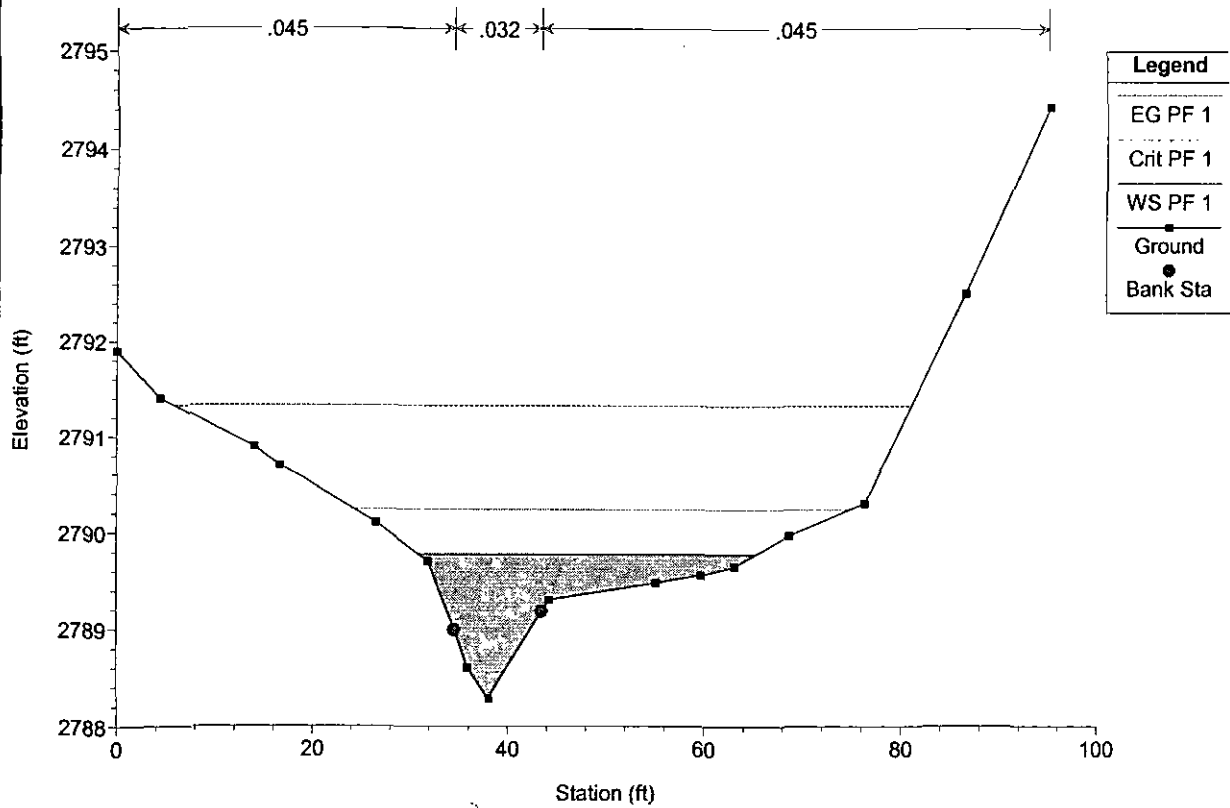


WASH A1-1

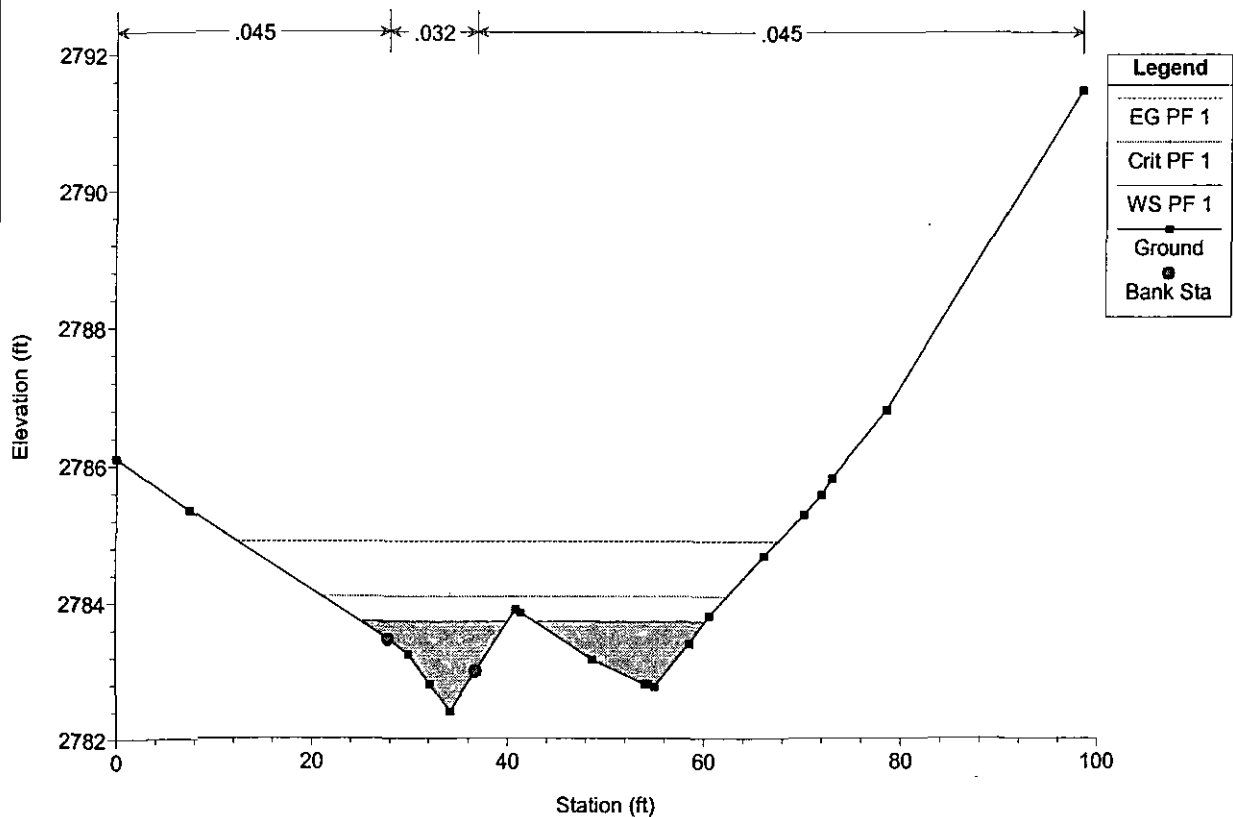
HEC-RAS Plan: Plan 02 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/n)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	6	PF 1	133.00	2788.28	2789.77	2790.23	2791.32	0.050087	10.97	17.44	34.33	1.84
Reach-1	5	PF 1	133.00	2782.40	2783.72	2784.08	2784.89	0.068076	10.46	17.98	32.56	2.04
Reach-1	4	PF 1	133.00	2775.92	2777.33	2777.66	2778.36	0.045547	9.75	21.33	41.25	1.72
Reach-1	3	PF 1	133.00	2769.16	2770.48	2771.05	2772.19	0.061709	10.66	13.50	19.67	1.98
Reach-1	2.5	PF 1	133.00	2766.00	2767.19	2767.51	2768.15	0.057608	7.85	16.99	29.16	1.79
Reach-1	2	PF 1	133.00	2761.84	2763.45	2763.69	2764.23	0.028466	7.15	19.38	28.35	1.34
Reach-1	1.5	PF 1	133.00	2759.00	2759.88	2760.03	2760.39	0.048351	5.71	23.31	55.71	1.55
Reach-1	1	PF 1	235.00	2753.00	2754.37	2754.64	2755.33	0.035550	9.07	37.36	60.40	1.55

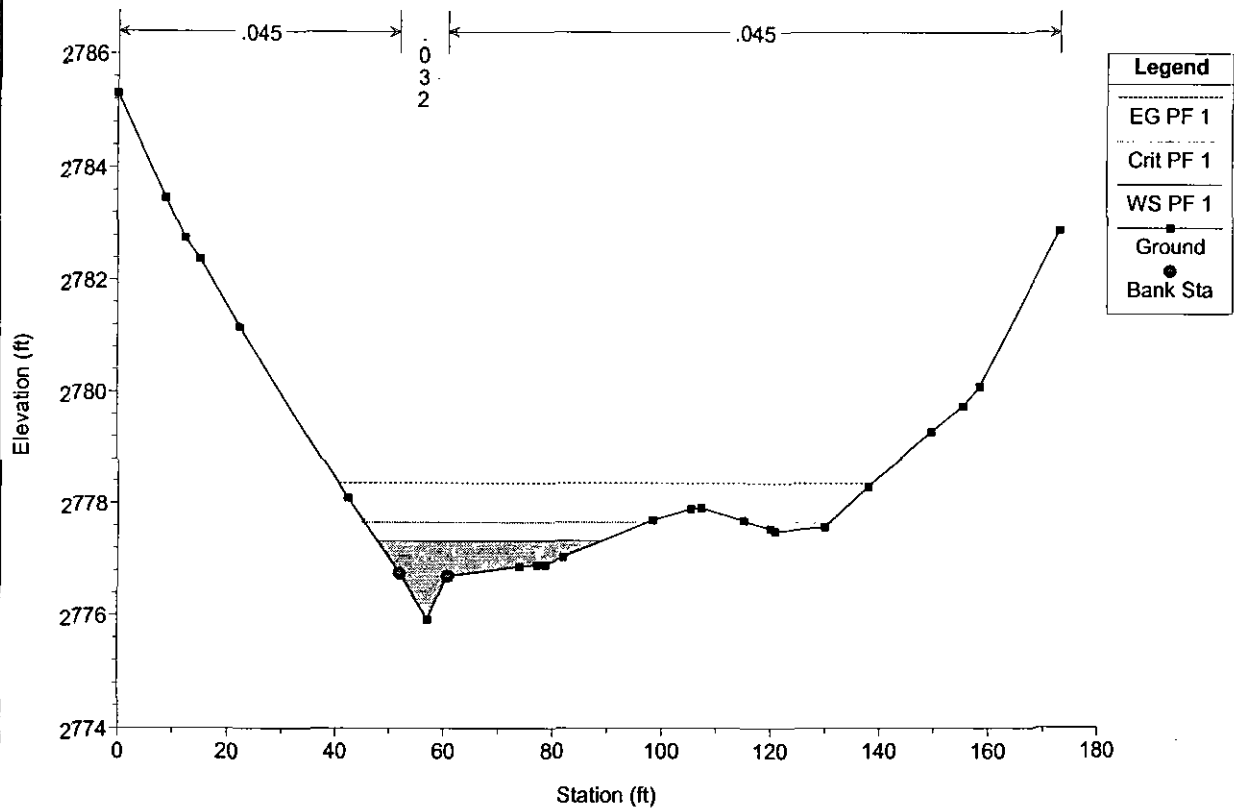
washa1-1-EX Plan: Plan 02 11/7/2006



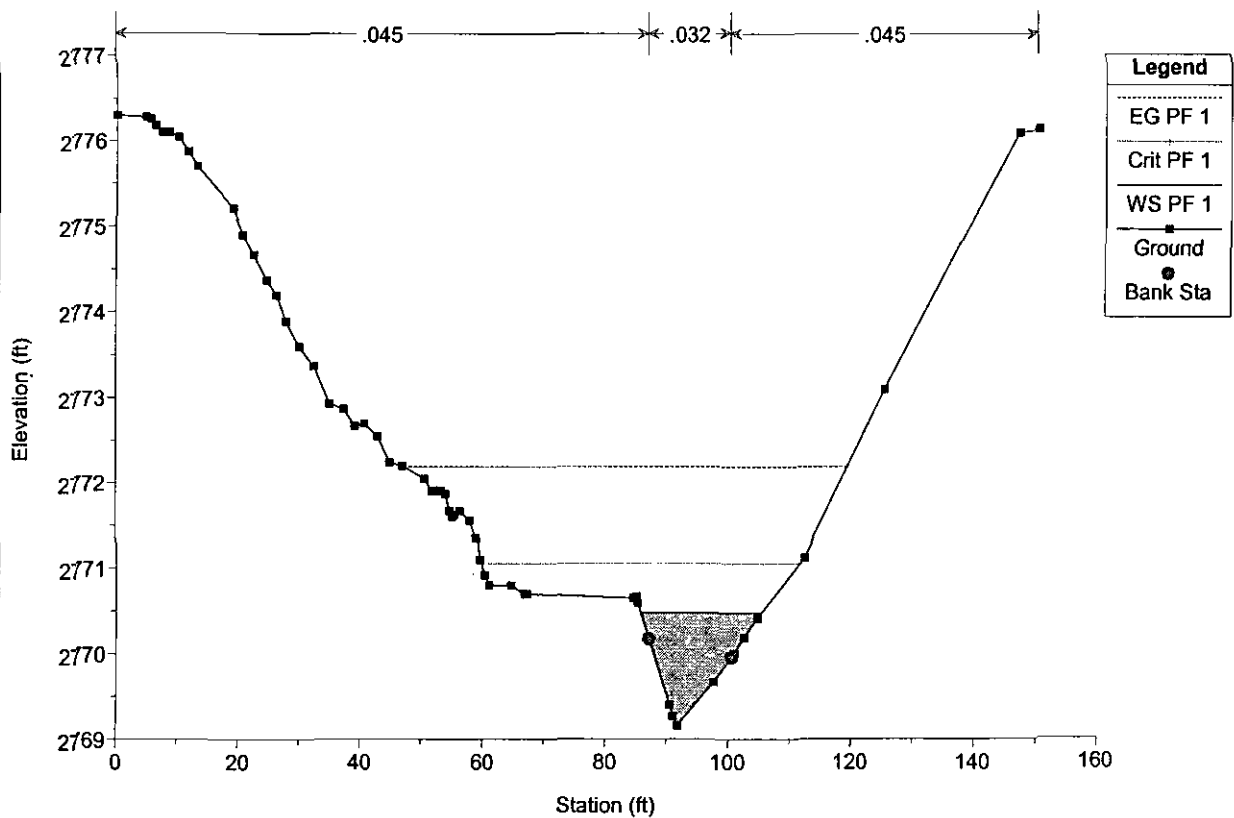
washa1-1-EX Plan: Plan 02 11/7/2006



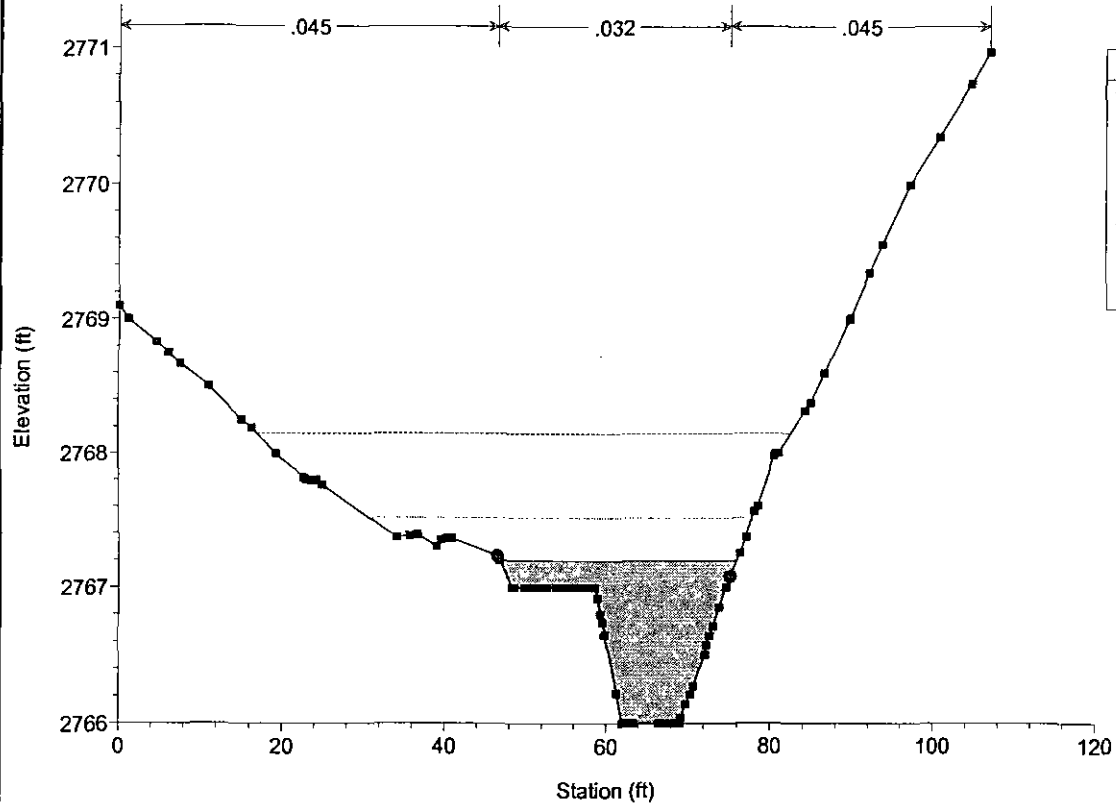
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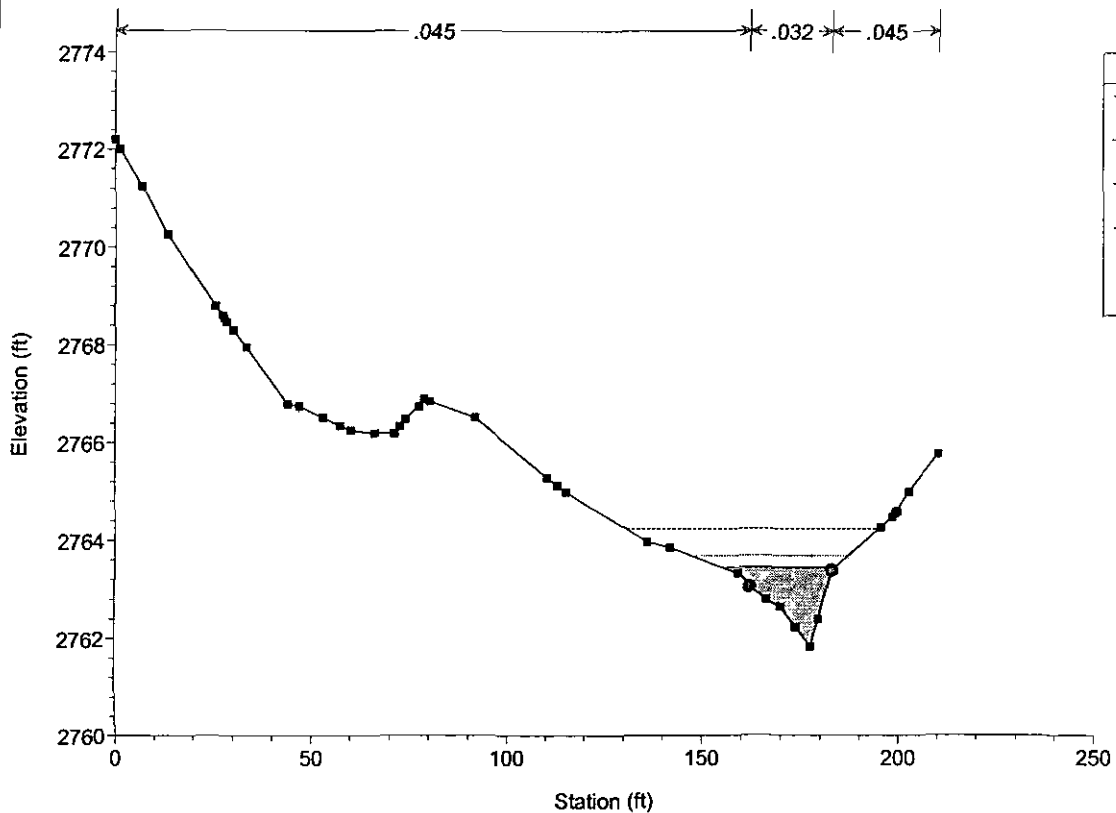
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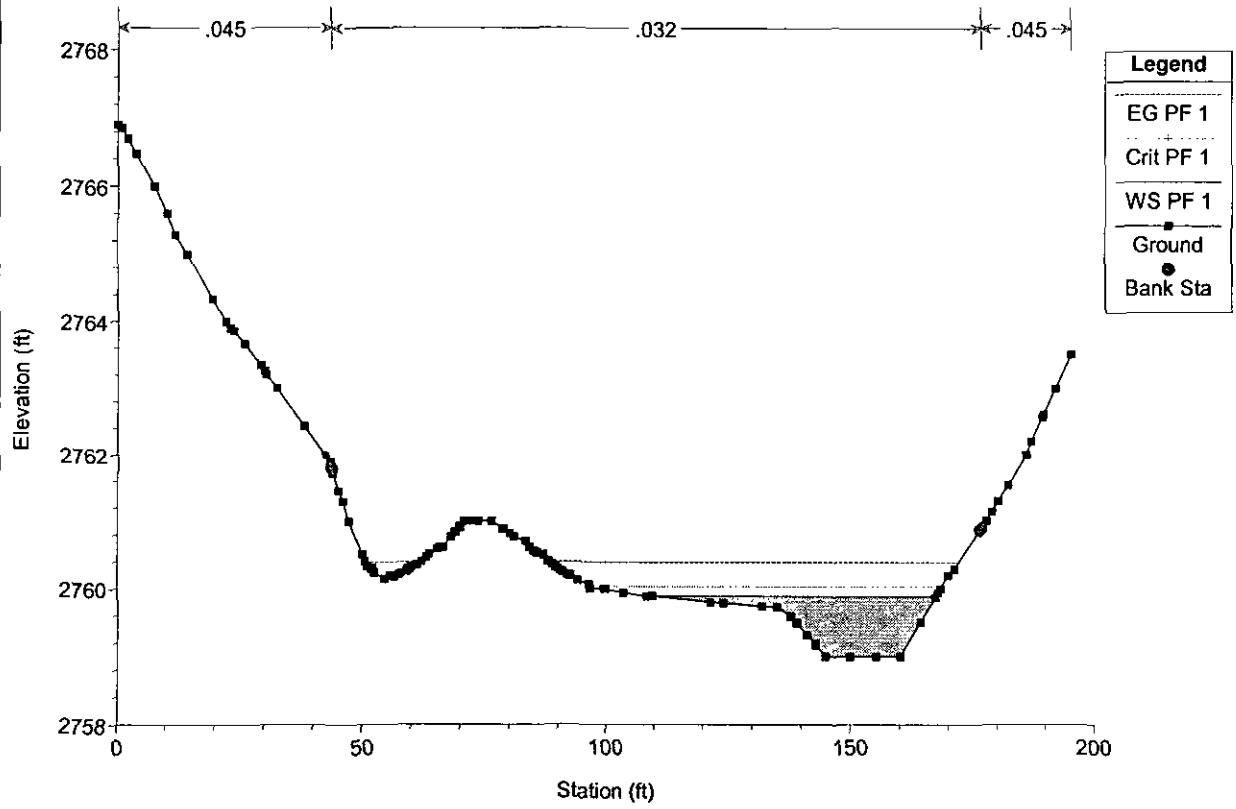
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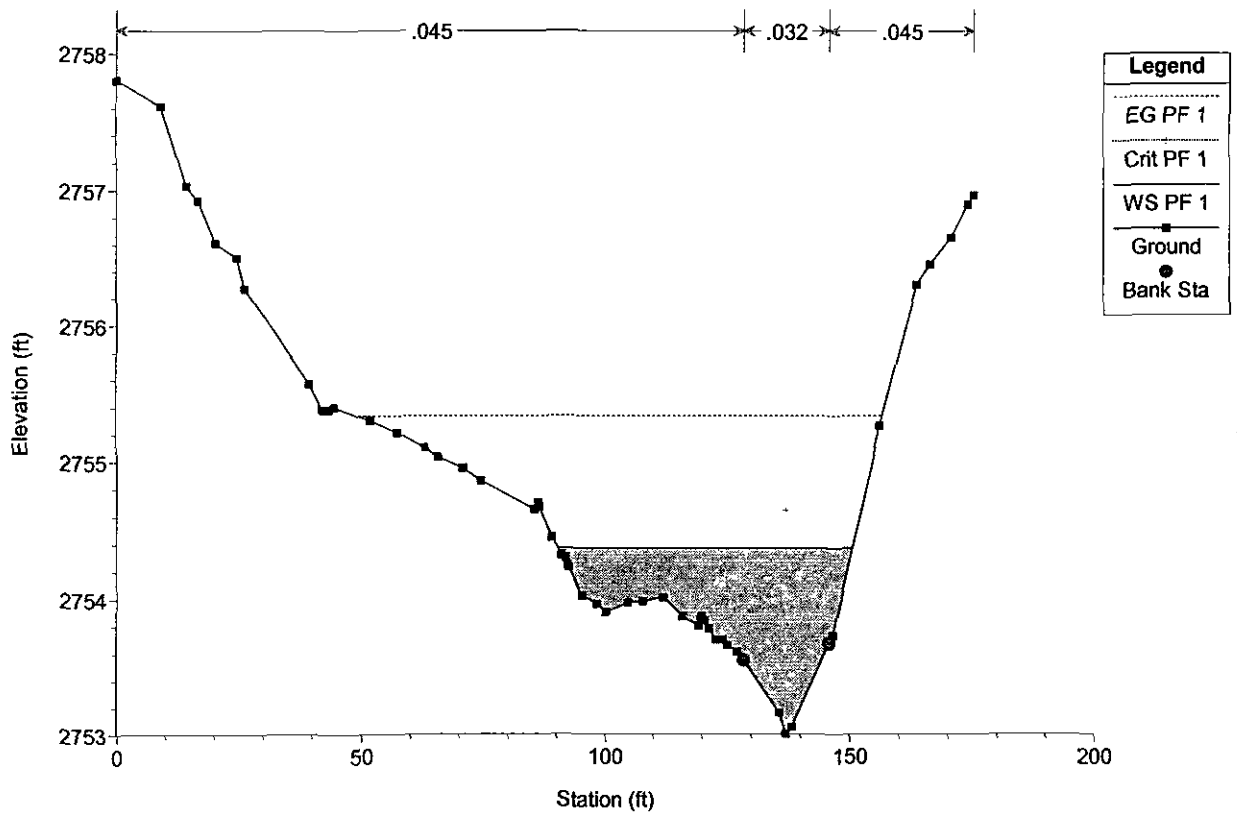
washa1-1-EX Plan: Plan 02 11/7/2006



washa1-1-EX Plan: Plan 02 11/7/2006



washa1-1-EX Plan: Plan 02 11/7/2006

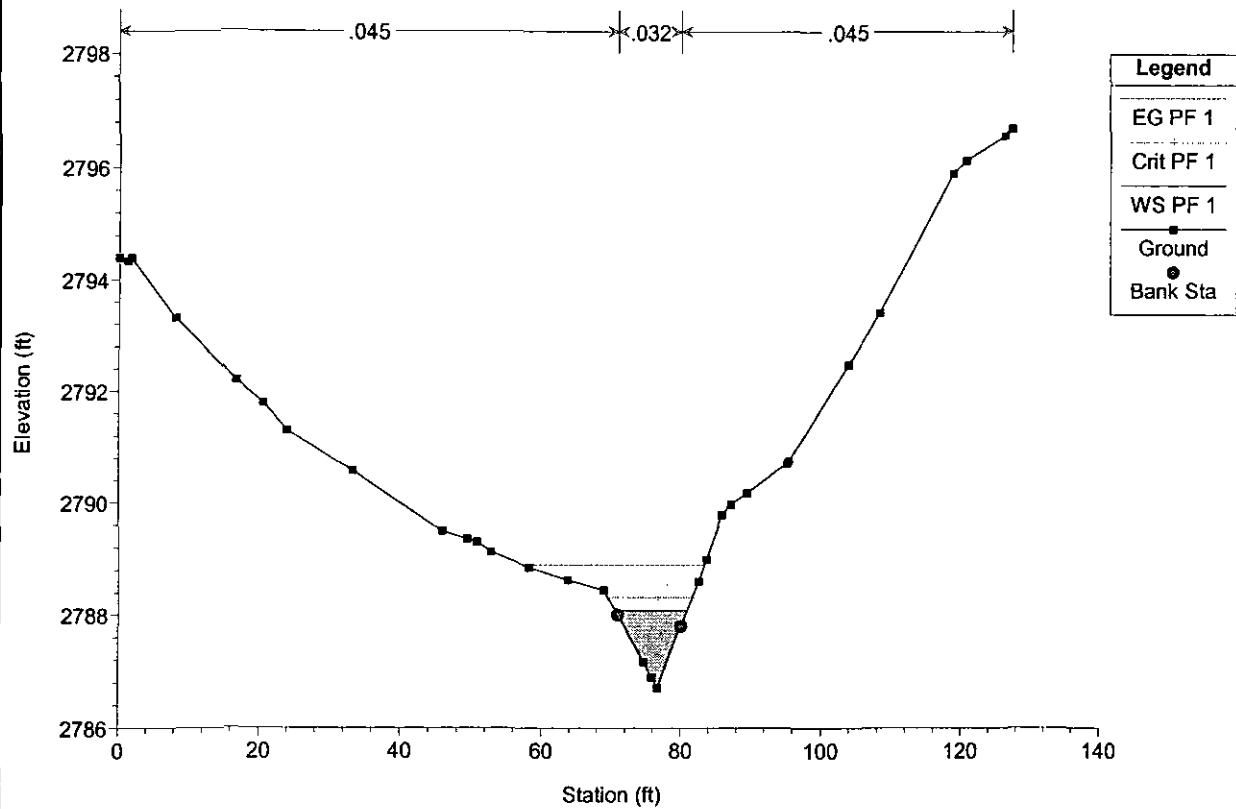


WASH A2

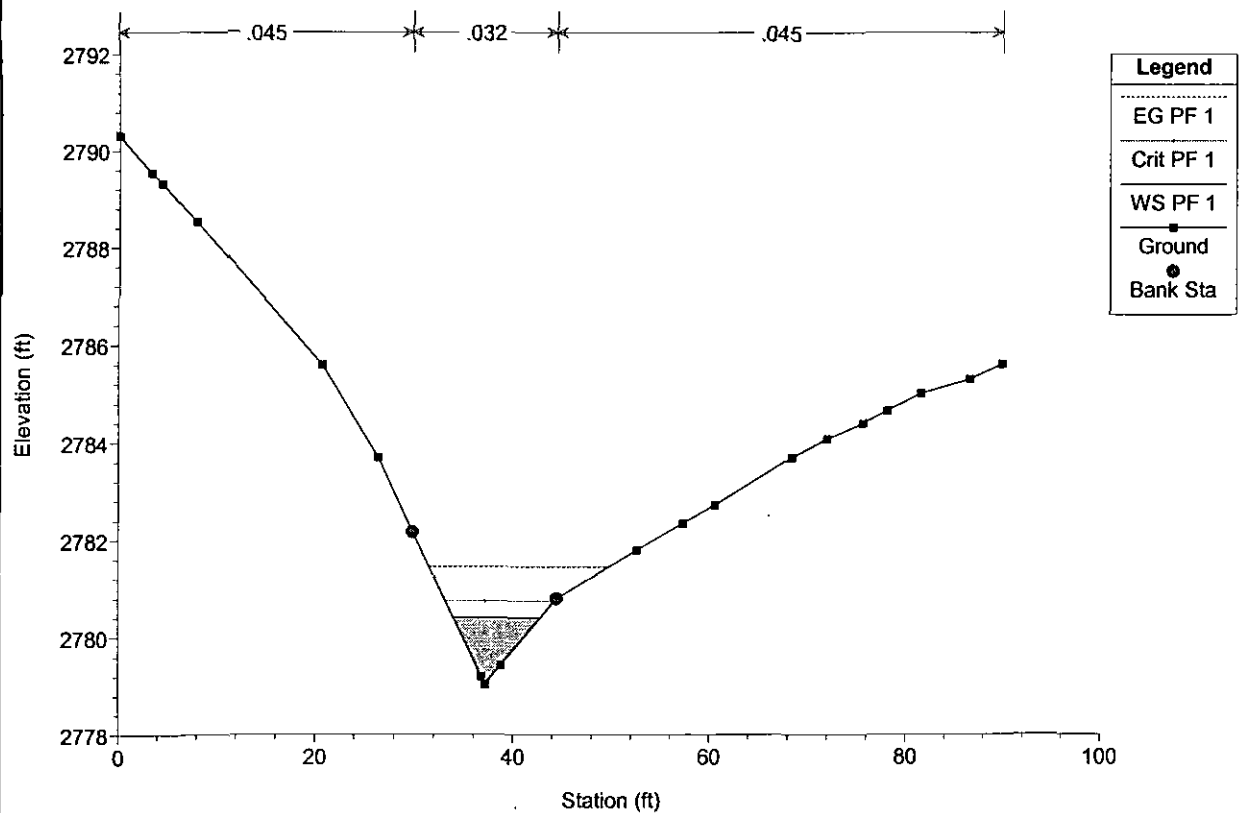
HEC-RAS Plan: Plan 01 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	8	PF 1	50.00	2786.70	2788.08	2788.32	2788.89	0.037052	7.25	7.00	10.29	1.47
Reach-1	7	PF 1	50.00	2779.03	2780.41	2780.75	2781.45	0.054930	8.20	6.09	8.86	1.74
Reach-1	6.5	PF 1	50.00	2774.00	2775.12	2775.56	2776.49	0.053771	9.50	5.51	7.56	1.78
Reach-1	6	PF 1	50.00	2769.58	2771.38	2771.63	2772.27	0.035829	7.60	6.58	7.35	1.47
Reach-1	5.5	PF 1	50.00	2766.00	2767.37	2767.82	2768.78	0.045155	9.90	5.95	8.05	1.70
Reach-1	5	PF 1	65.00	2760.85	2762.45	2762.98	2764.14	0.060416	10.47	6.36	7.80	1.89
Reach-1	4	PF 1	65.00	2754.30	2756.01	2756.26	2756.93	0.034820	7.73	8.44	9.82	1.44
Reach-1	3	PF 1	65.00	2749.02	2750.53	2750.80	2751.42	0.041151	7.55	8.61	11.61	1.55
Reach-1	2	PF 1	109.00	2742.10	2743.52	2743.88	2744.58	0.039966	8.35	13.80	21.60	1.58
Reach-1	1.5	PF 1	109.00	2739.00	2740.27	2740.51	2741.01	0.023936	7.10	18.25	35.03	1.26
Reach-1	1	PF 1	109.00	2733.30	2734.60	2735.10	2736.05	0.078453	9.68	11.58	21.73	2.12

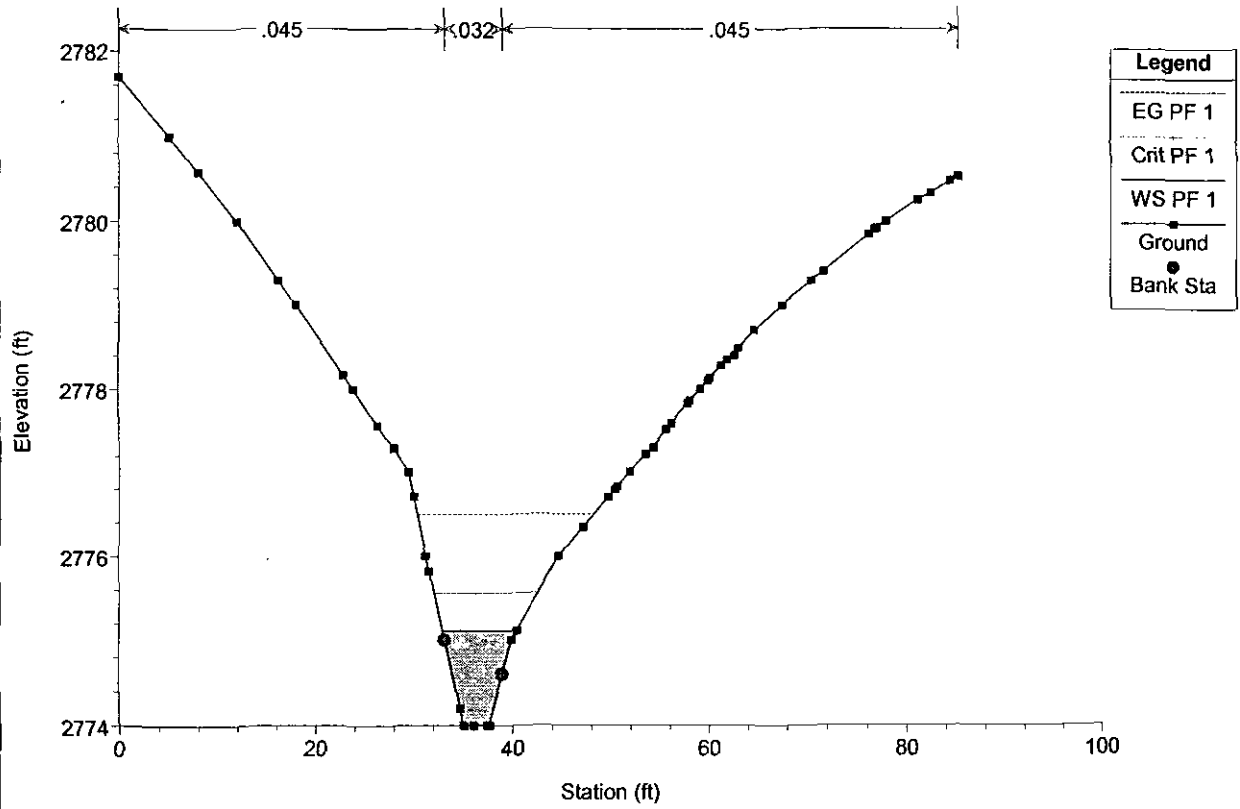
washa2-EX Plan: Plan 01 11/7/2006



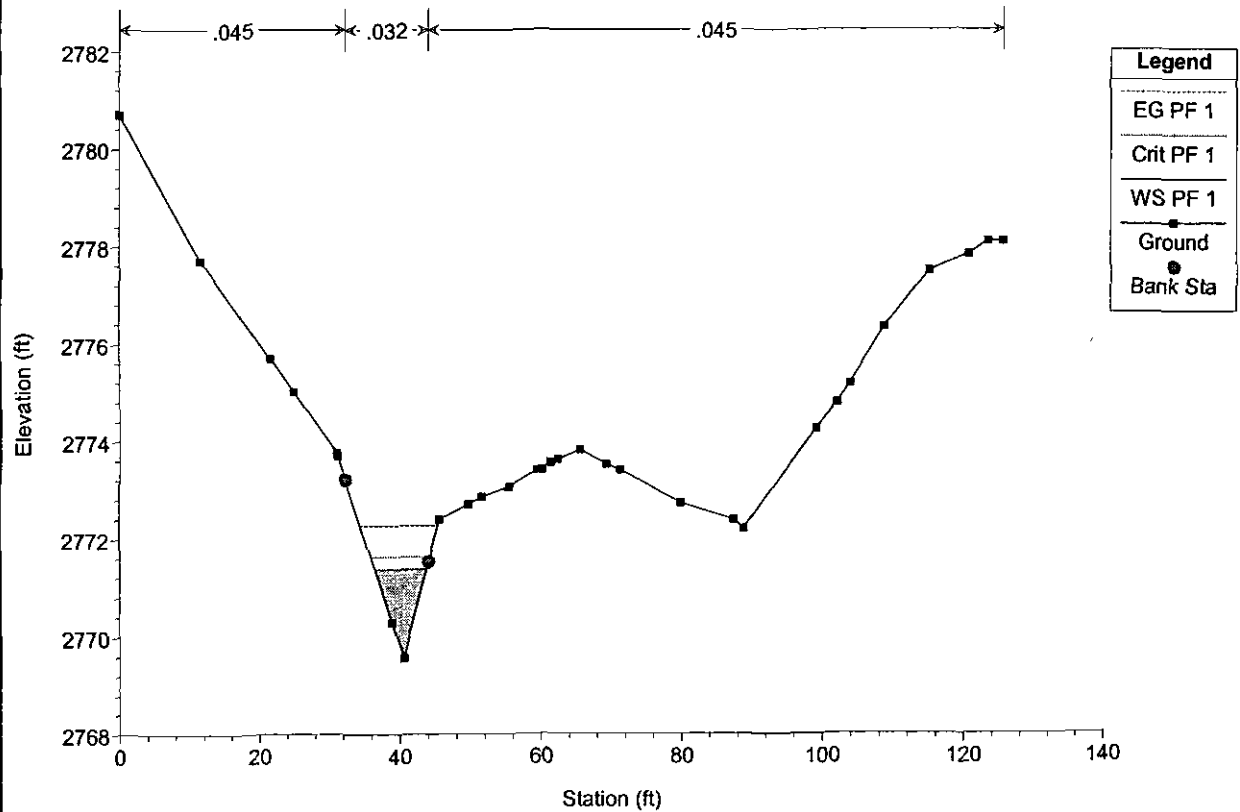
washa2-EX Plan: Plan 01 11/7/2006



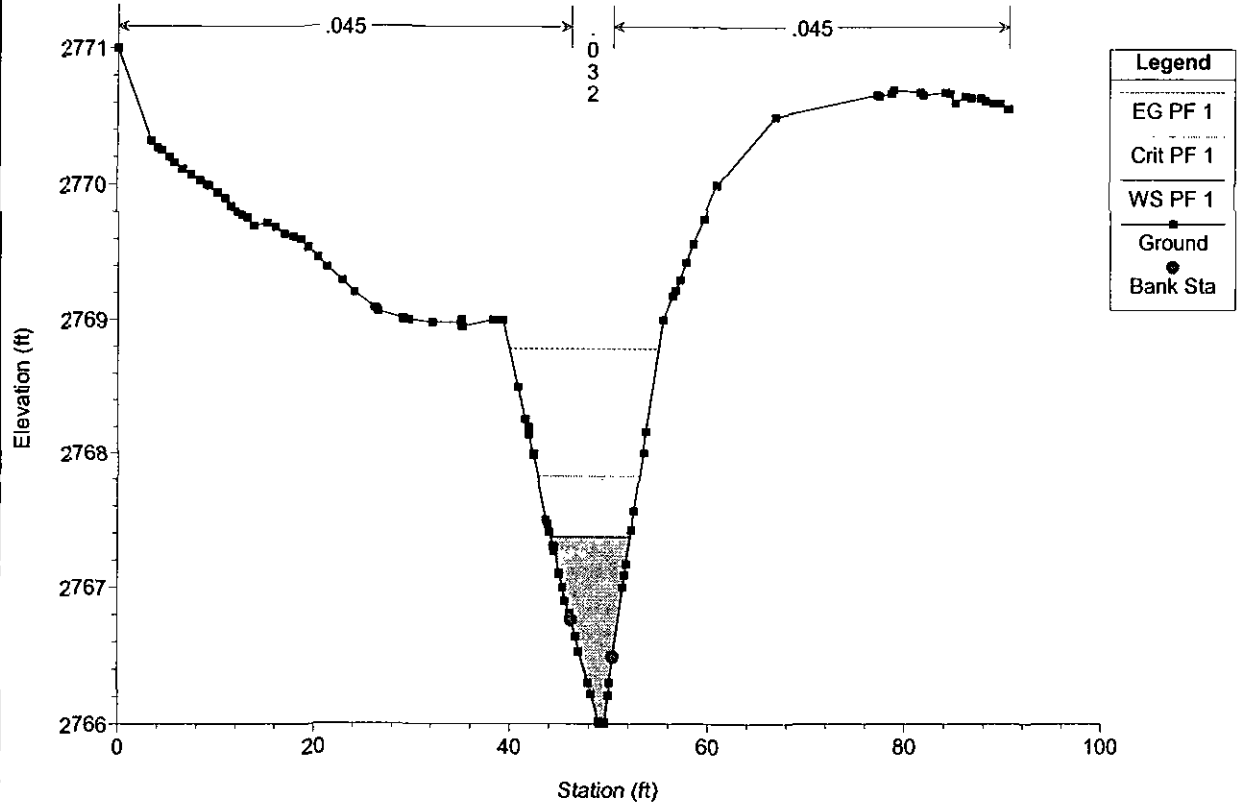
washa2-EX Plan: Plan 01 11/7/2006



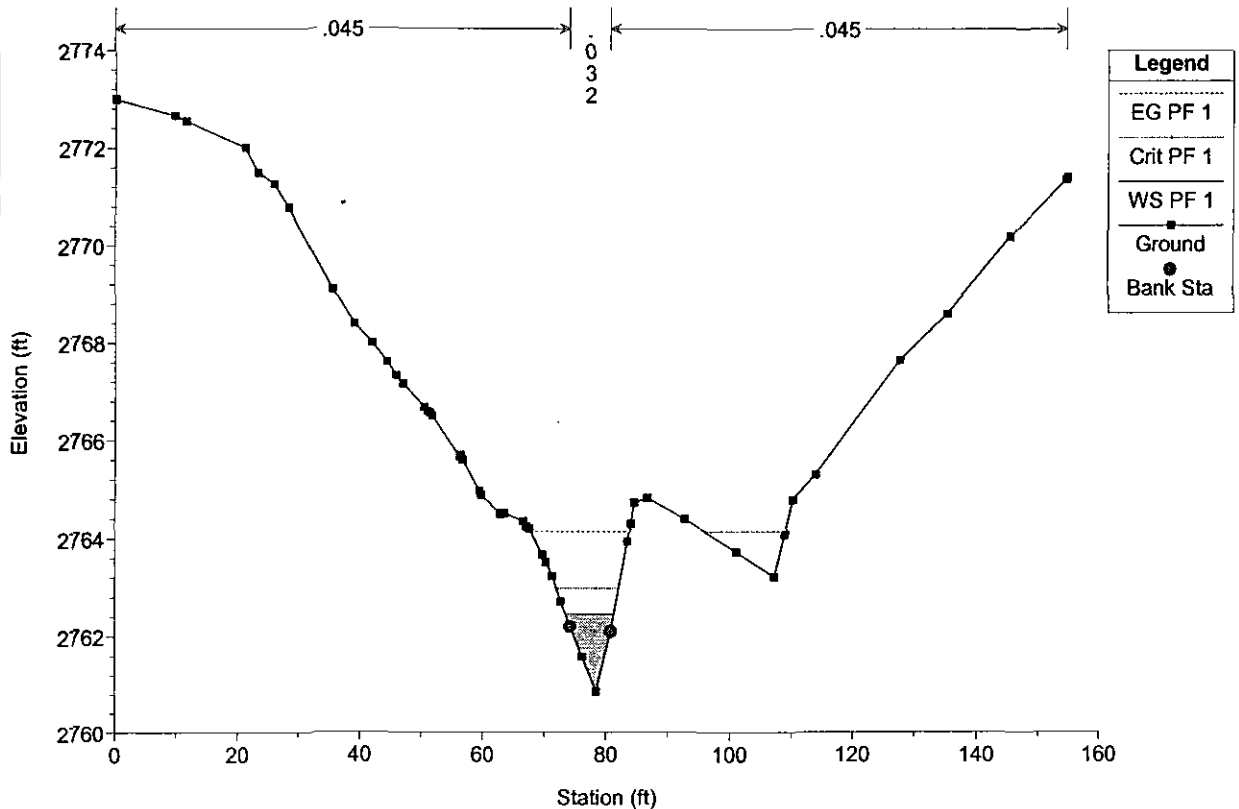
washa2-EX Plan: Plan 01 11/7/2006



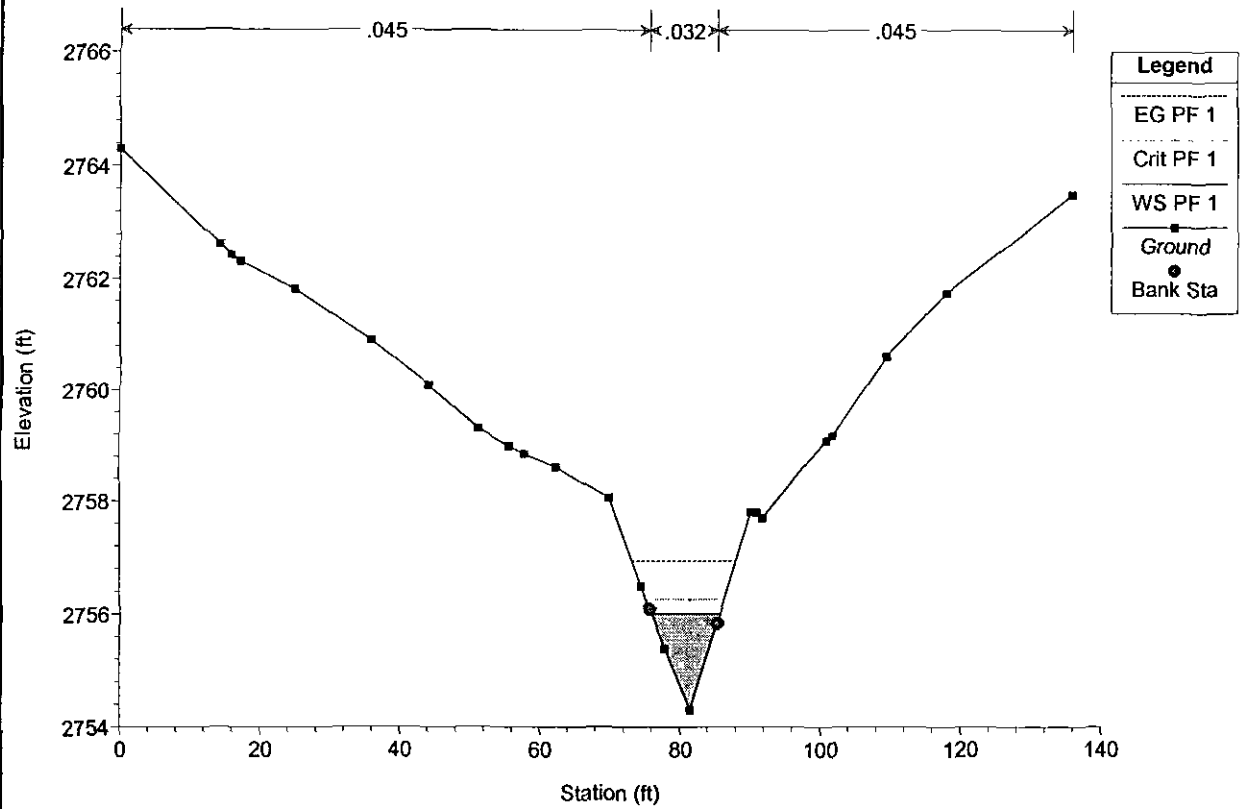
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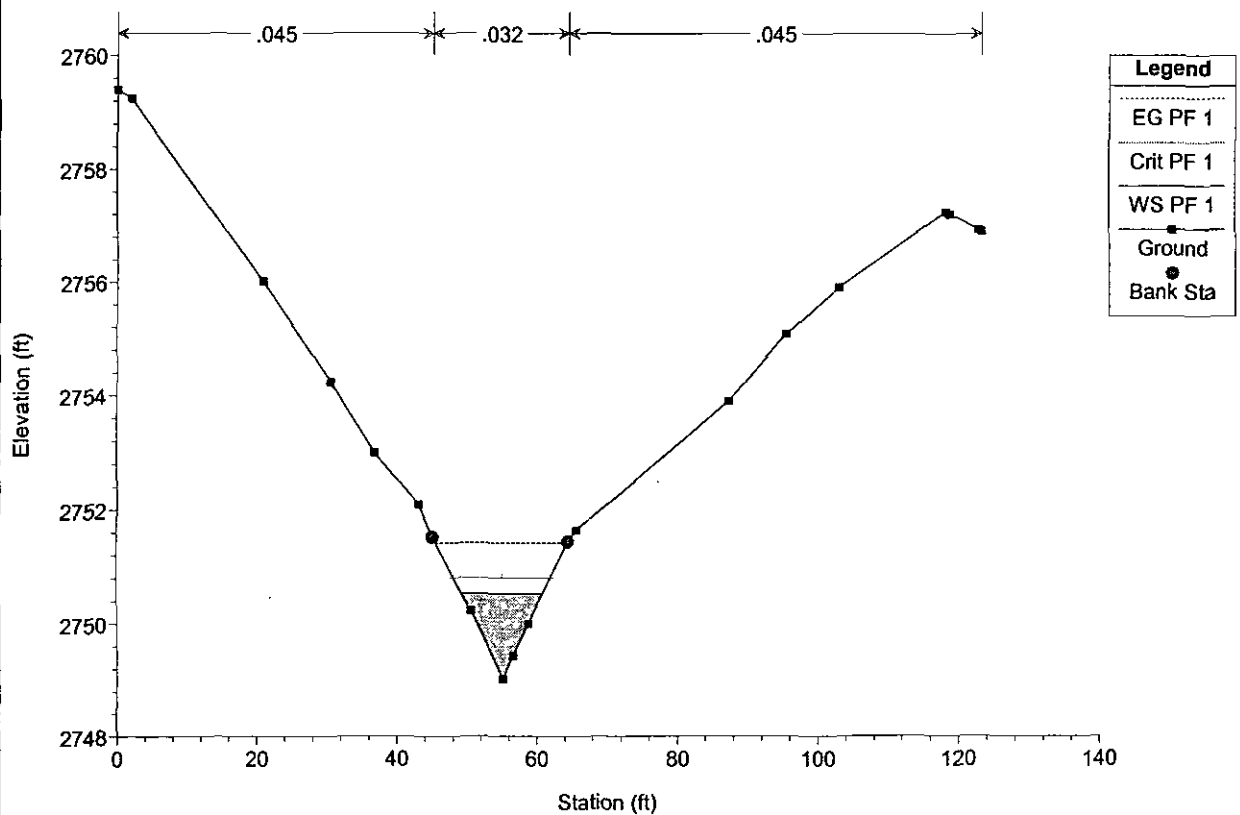
washa2-EX Plan: Plan 01 11/7/2006



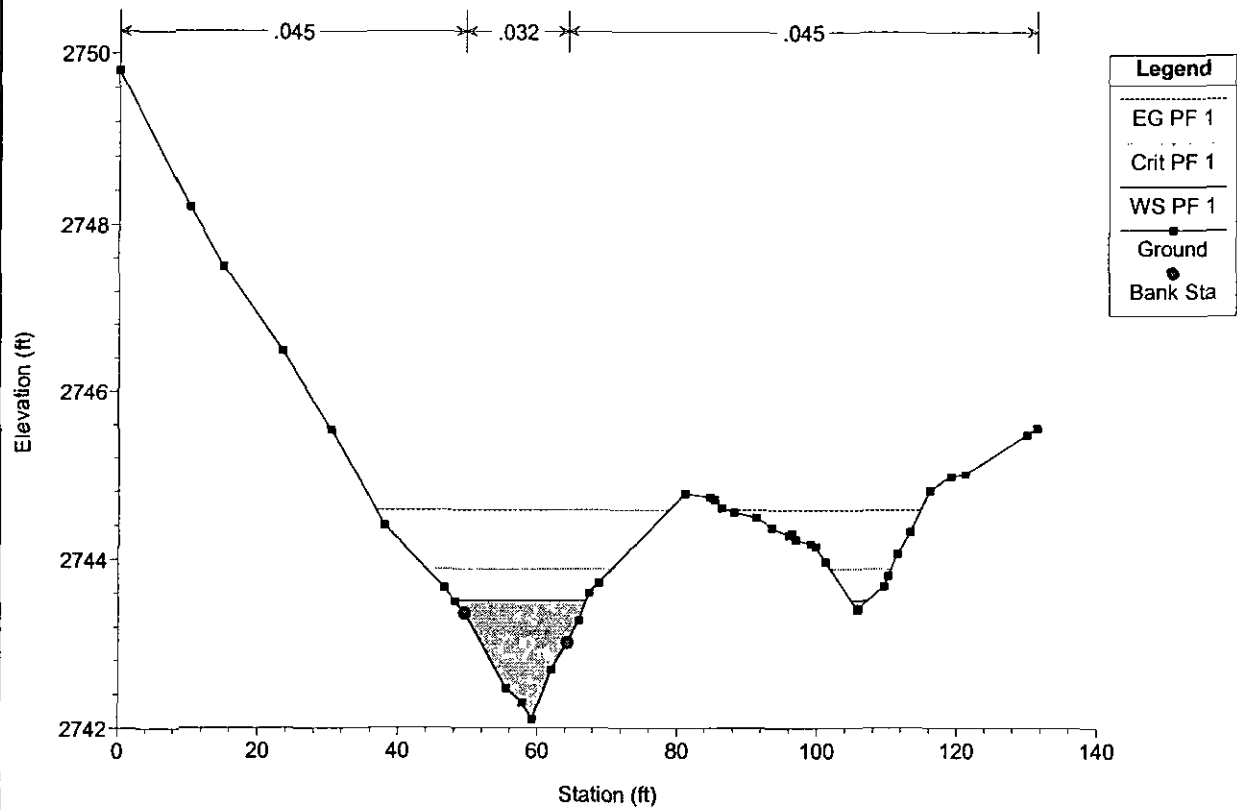
washa2-EX Plan: Plan 01 11/7/2006



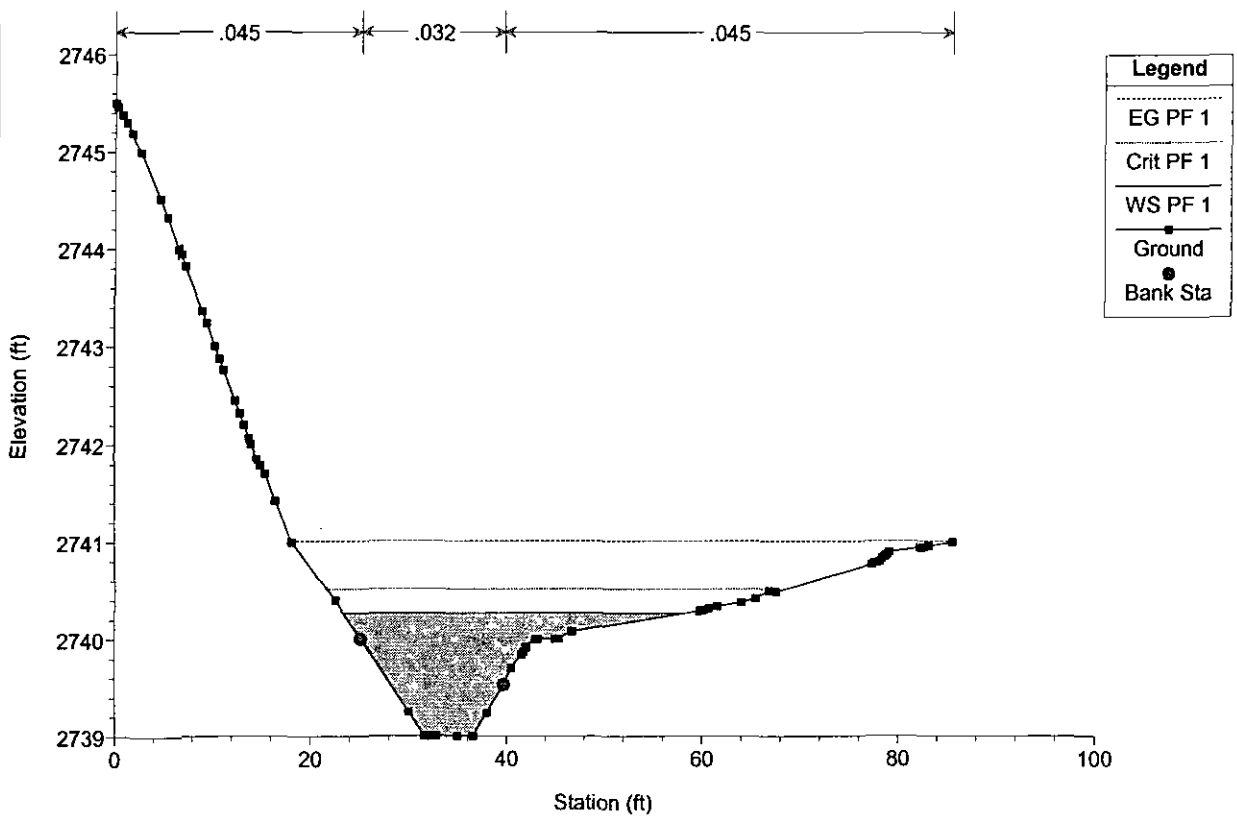
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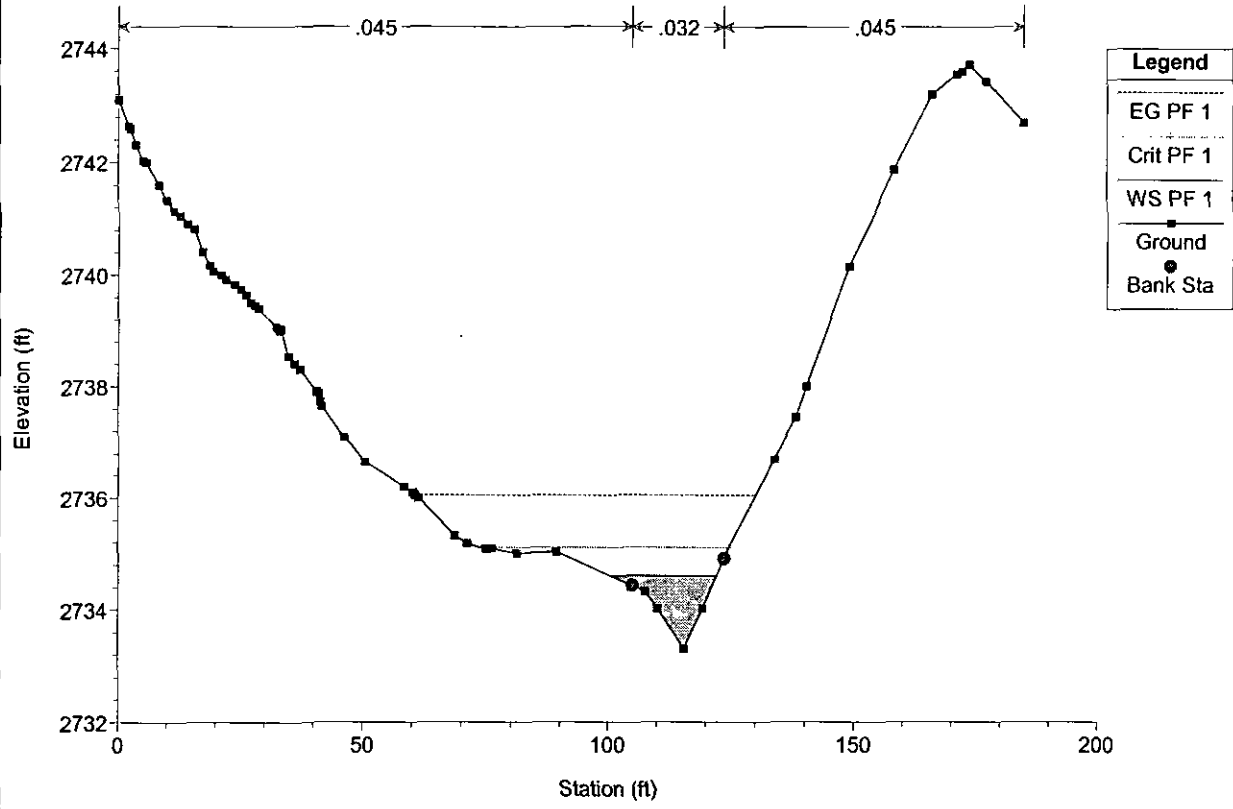
washa2-EX Plan: Plan 01 11/7/2006



washa2-EX Plan: Plan 01 11/7/2006



washa2-EX Plan: Plan 01 11/7/2006

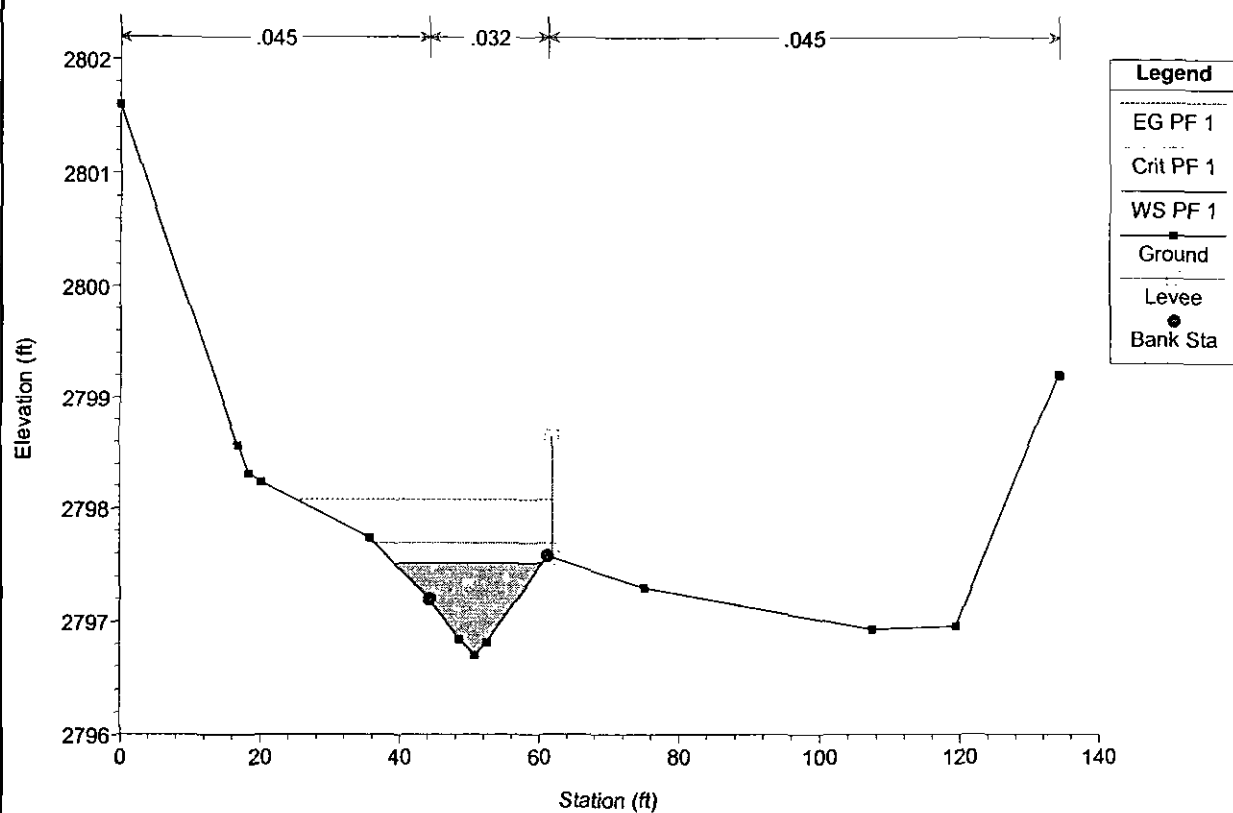


WASH B1

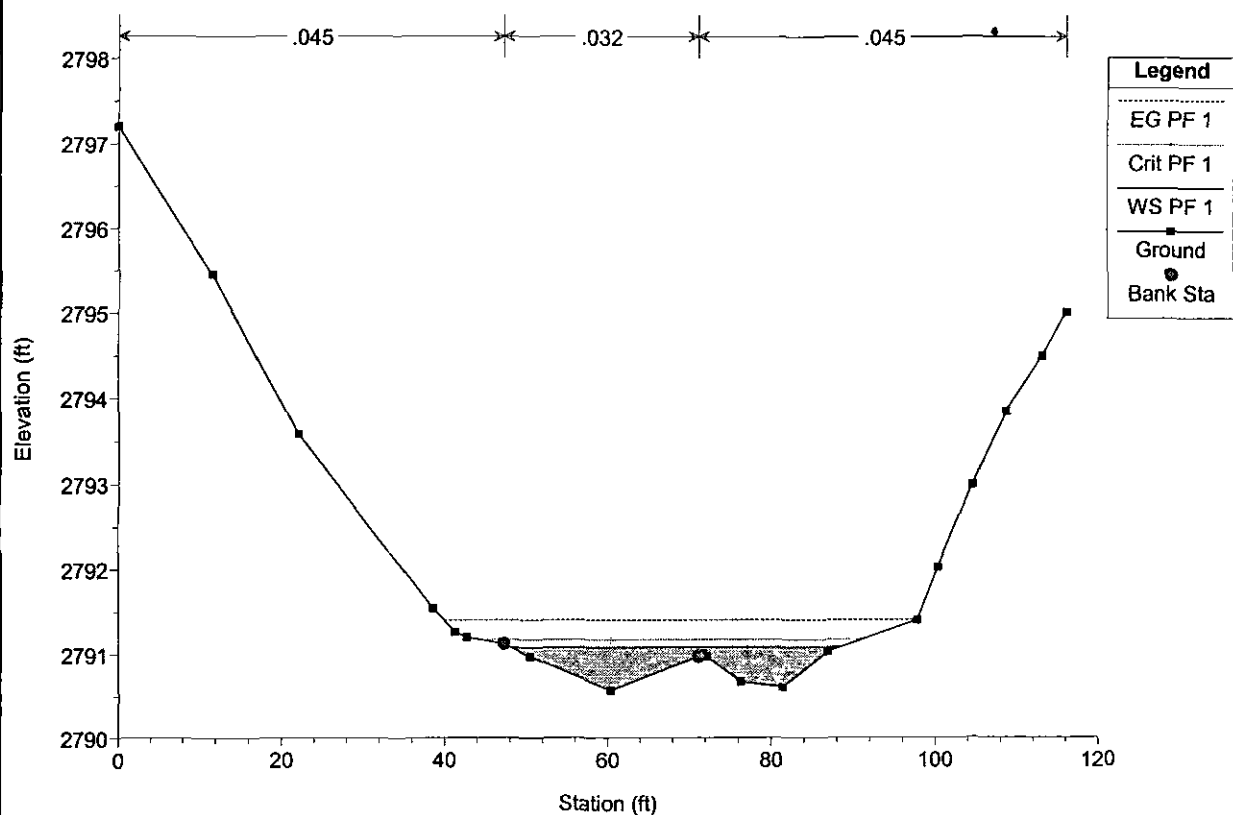
HEC-RAS Plan: Plan 01 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Ch
Reach-1	9	PF.1	50.00	2796.70	2797.52	2797.69	2798.08	0.045030	6.09	8.74	21.32	1.54
Reach-1	8	PF.1	50.00	2790.57	2791.08	2791.17	2791.40	0.059767	4.94	11.57	40.41	1.62
Reach-1	7	PF.1	50.00	2785.37	2786.03	2786.15	2786.38	0.048169	4.80	10.80	39.56	1.49
Reach-1	6.5	PF.1	50.00	2780.43	2780.99	2781.13	2781.43	0.065773	5.37	9.31	30.67	1.72
Reach-1	6	PF.1	50.00	2776.08	2776.54	2776.59	2776.81	0.033697	4.19	11.94	34.58	1.26
Reach-1	5	PF.1	65.00	2771.10	2771.91	2772.14	2772.65	0.053926	7.02	10.88	49.99	1.70
Reach-1	4.5	PF.1	65.00	2768.38	2768.90	2769.02	2769.32	0.046654	5.18	12.54	33.73	1.50
Reach-1	4	PF.1	65.00	2764.68	2765.43	2765.61	2766.02	0.043096	7.04	12.39	29.28	1.57
Reach-1	3.5	PF.1	65.00	2759.60	2760.51	2760.70	2761.15	0.041337	6.54	10.57	22.11	1.52
Reach-1	2	PF.1	83.00	2754.10	2754.86	2755.04	2755.46	0.046948	6.27	13.96	33.71	1.57
Reach-1	1	PF.1	204.00	2745.67	2747.01	2747.17	2747.53	0.048666	7.85	40.62	87.83	1.67

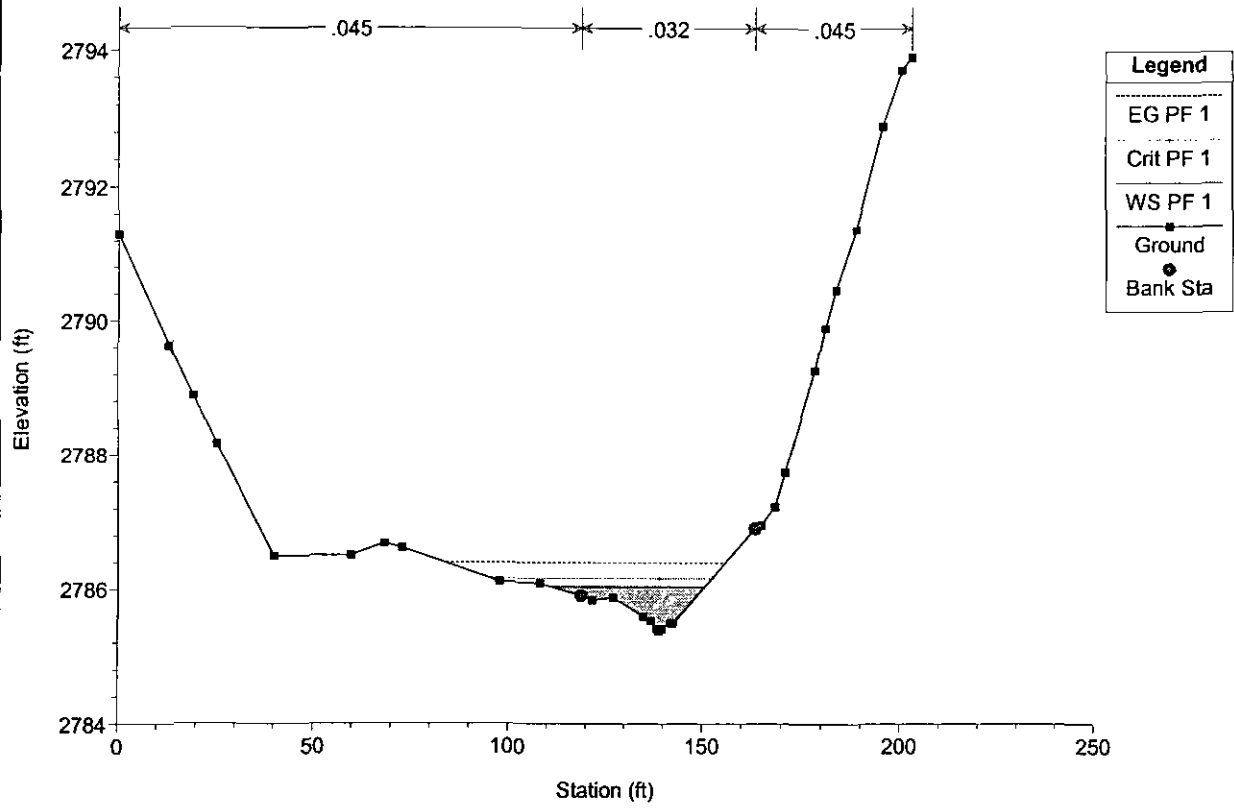
washb1-EX Plan: Plan 01 11/7/2006



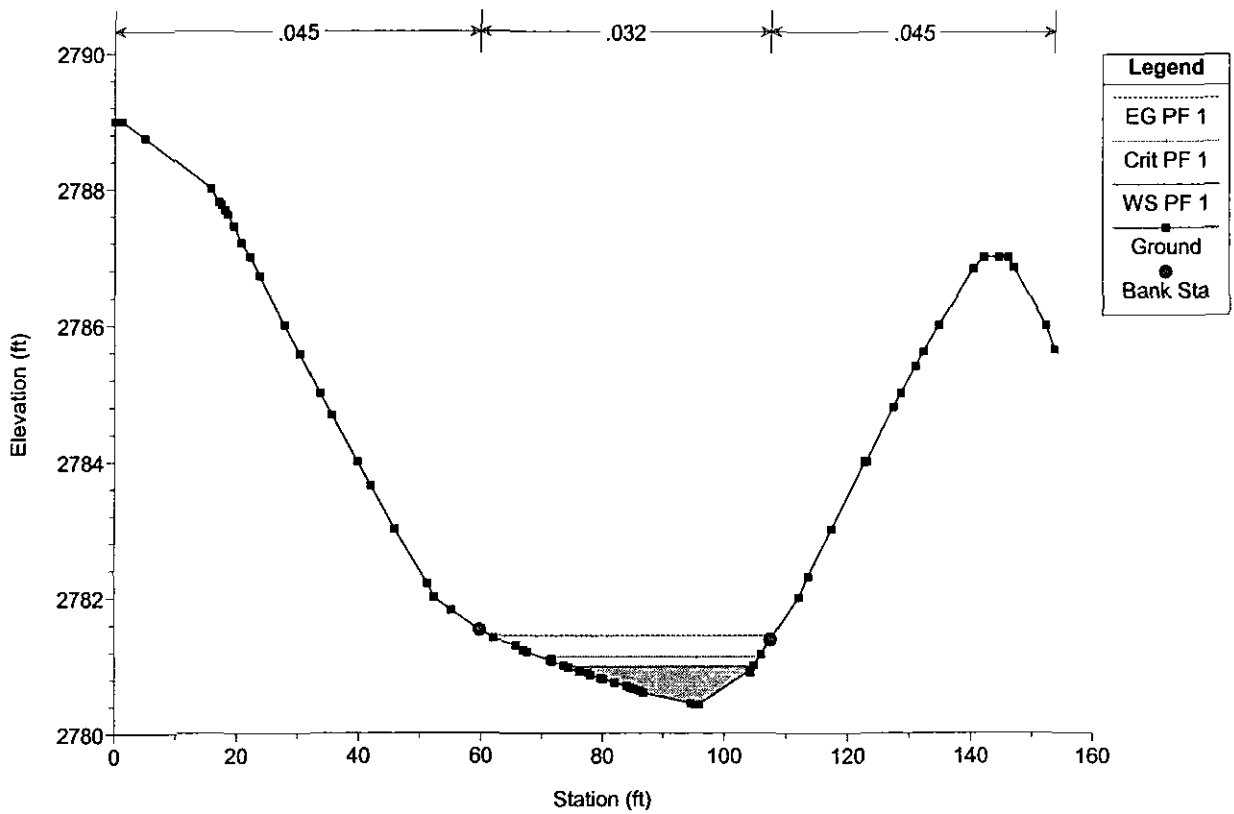
washb1-EX Plan: Plan 01 11/7/2006



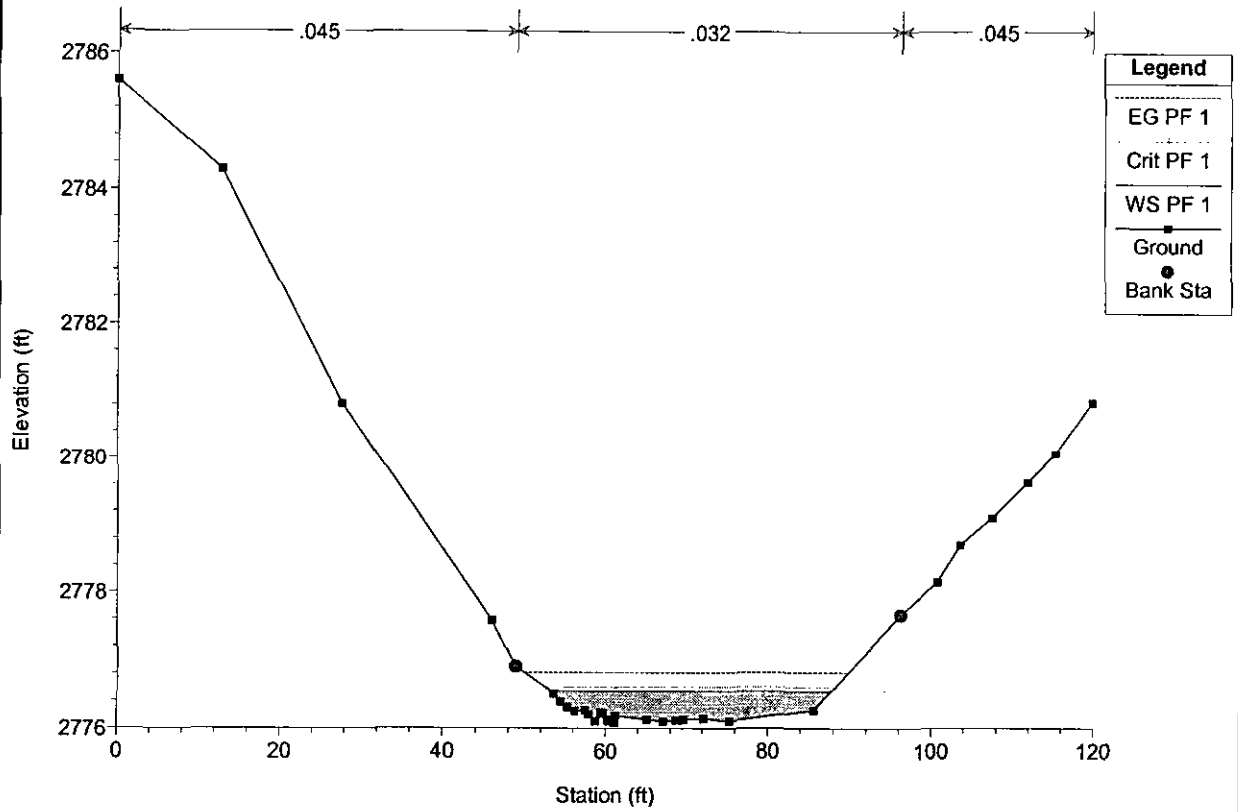
washb1-EX Plan: Plan 01 11/7/2006



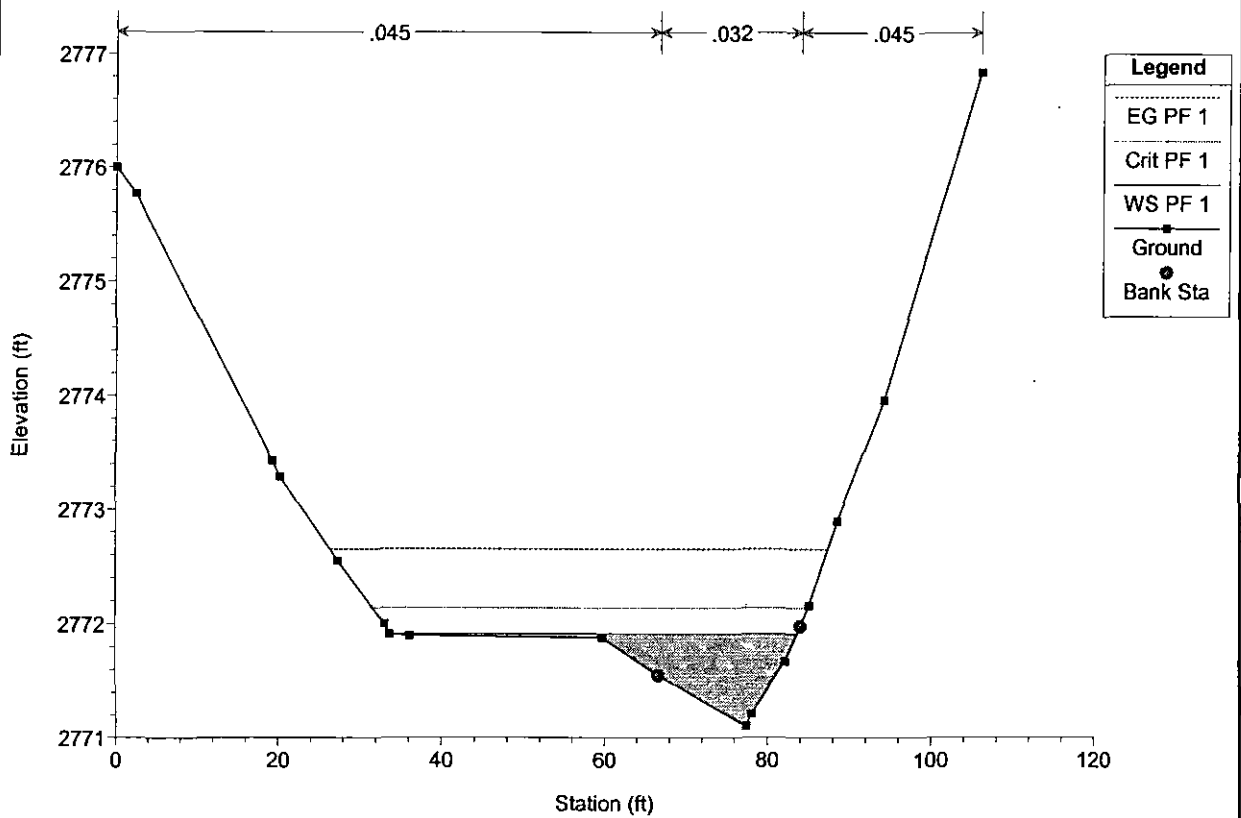
washb1-EX Plan: Plan 01 11/7/2006



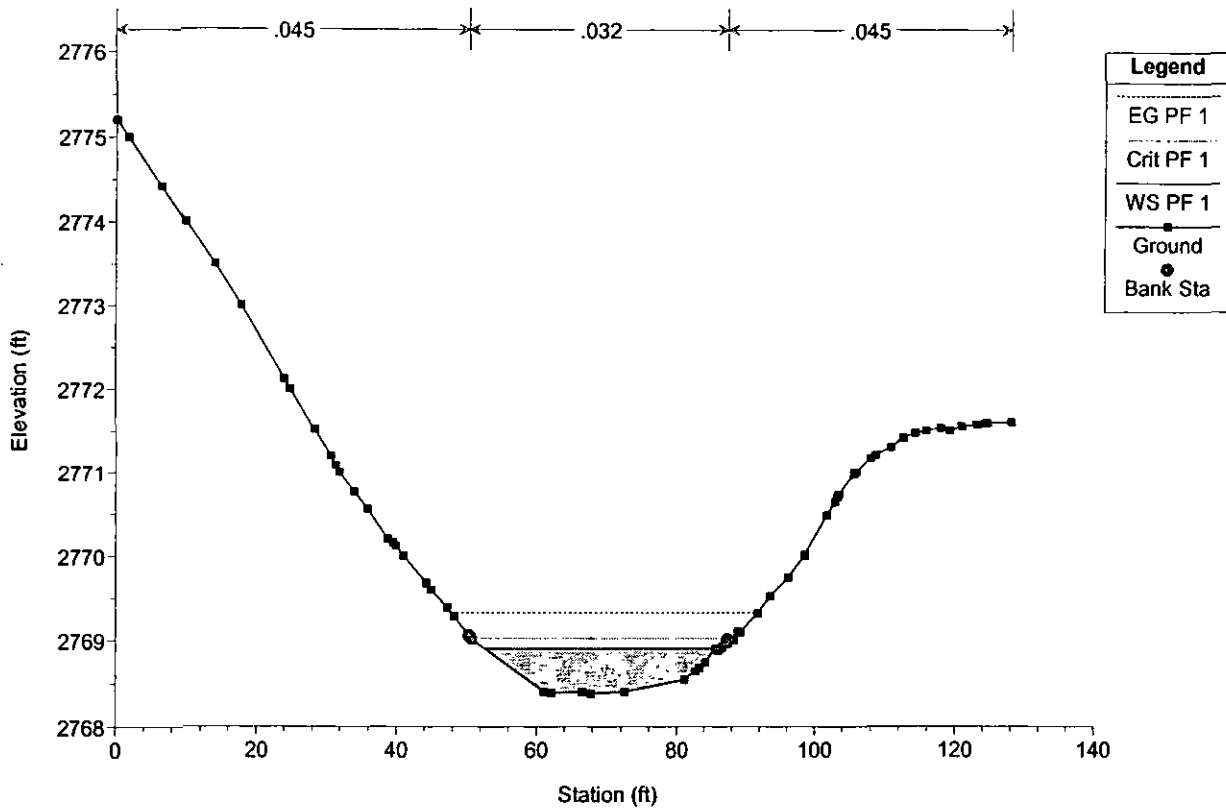
washb1-EX Plan: Plan 01 11/7/2006



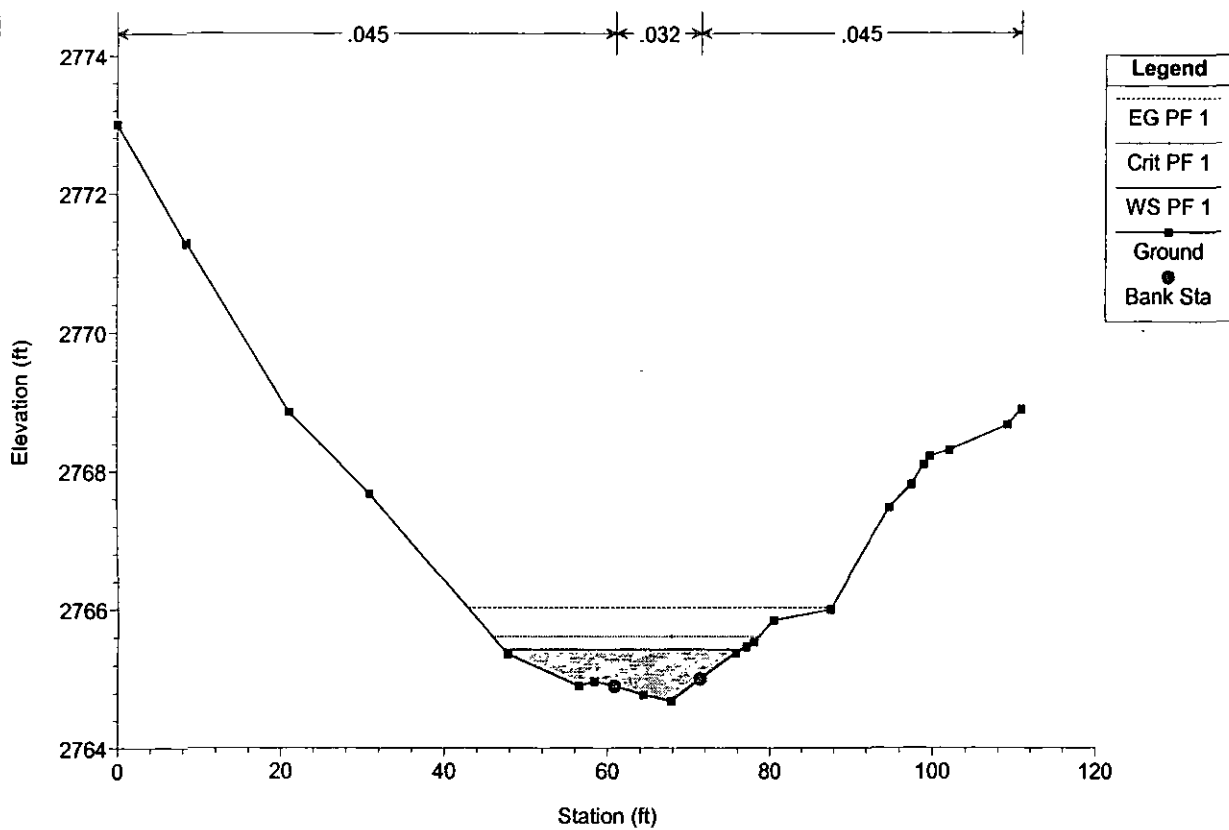
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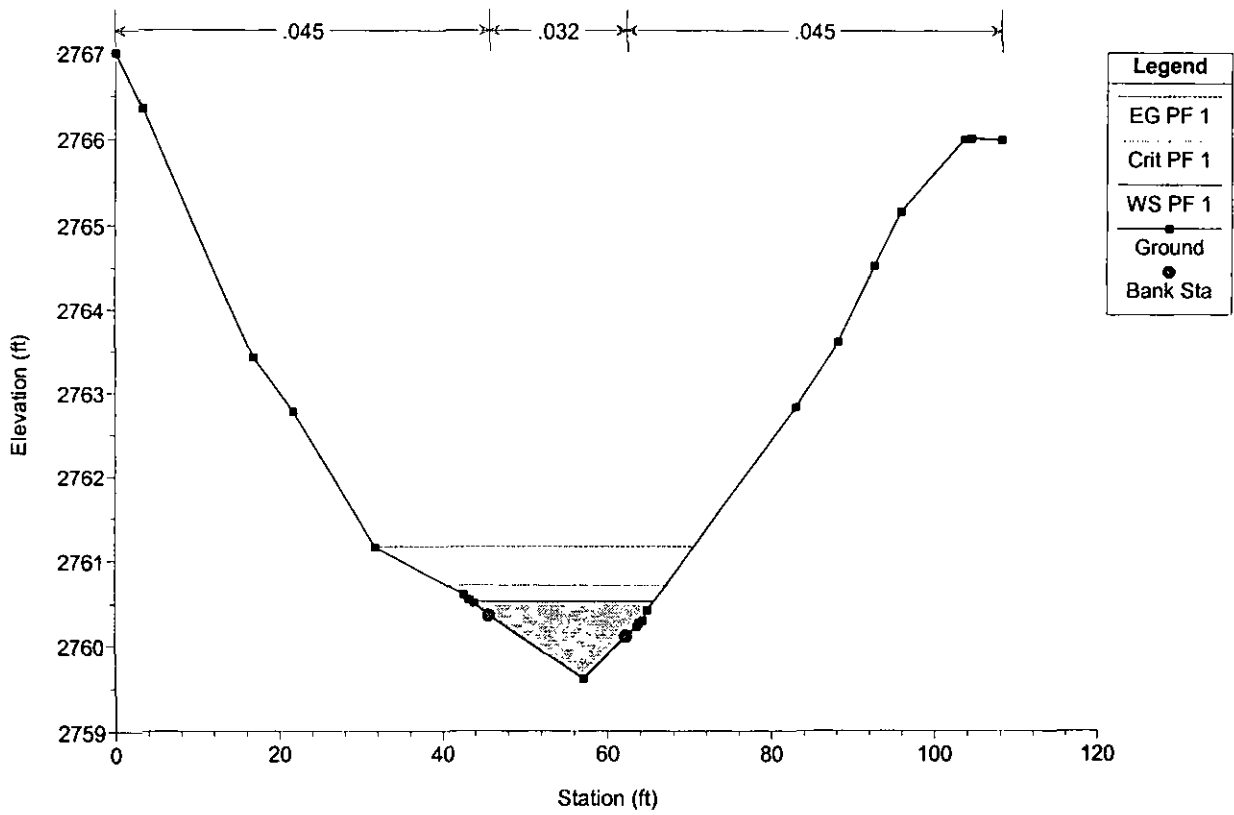
washb1-EX Plan: Plan 01 11/7/2006



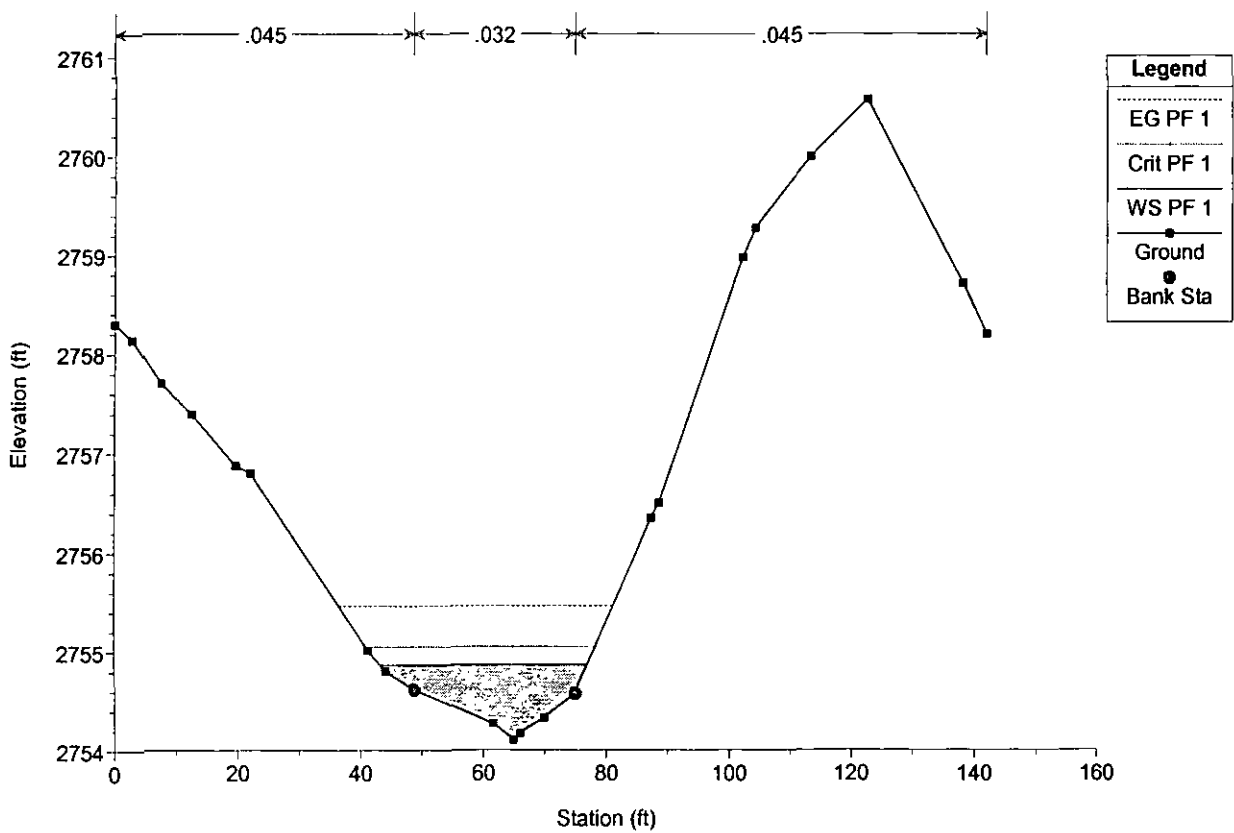
washb1-EX Plan: Plan 01 11/7/2006



washb1-EX Plan: Plan 01 11/7/2006



washb1-EX Plan: Plan 01 11/7/2006

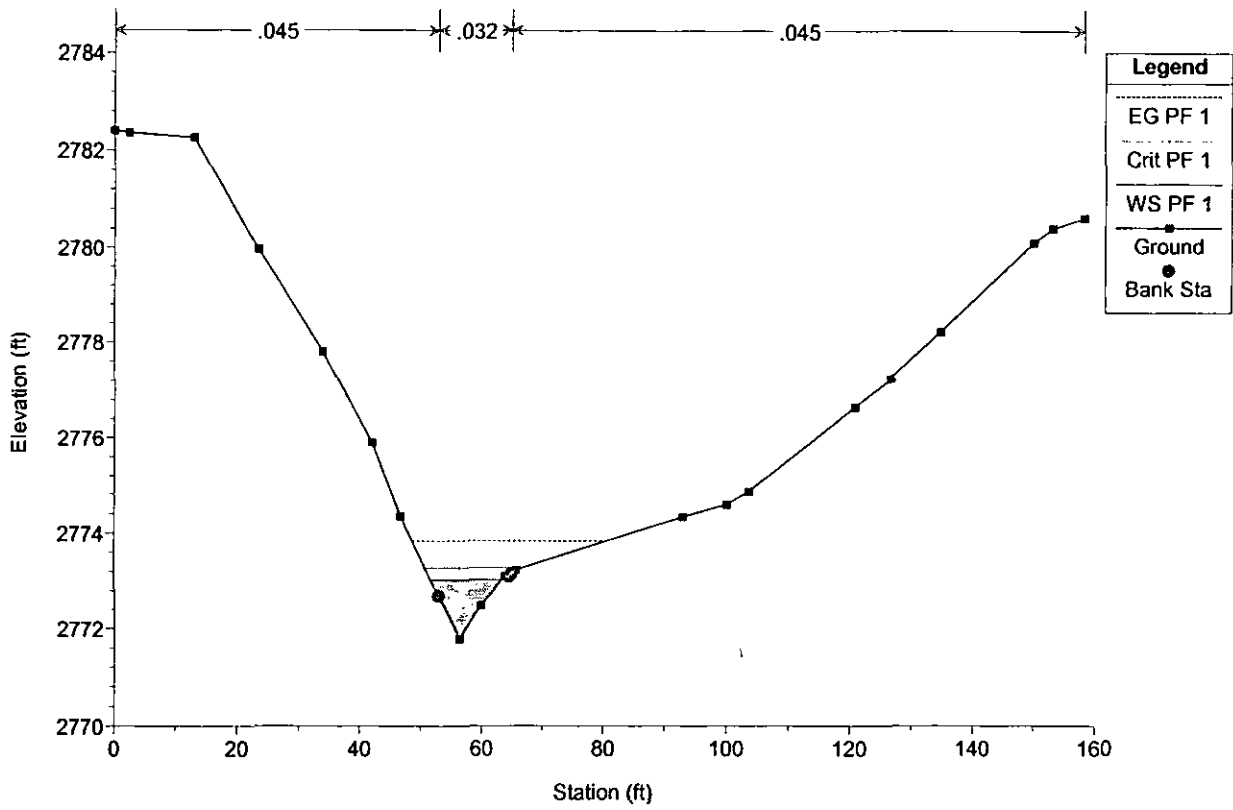


WASH B2

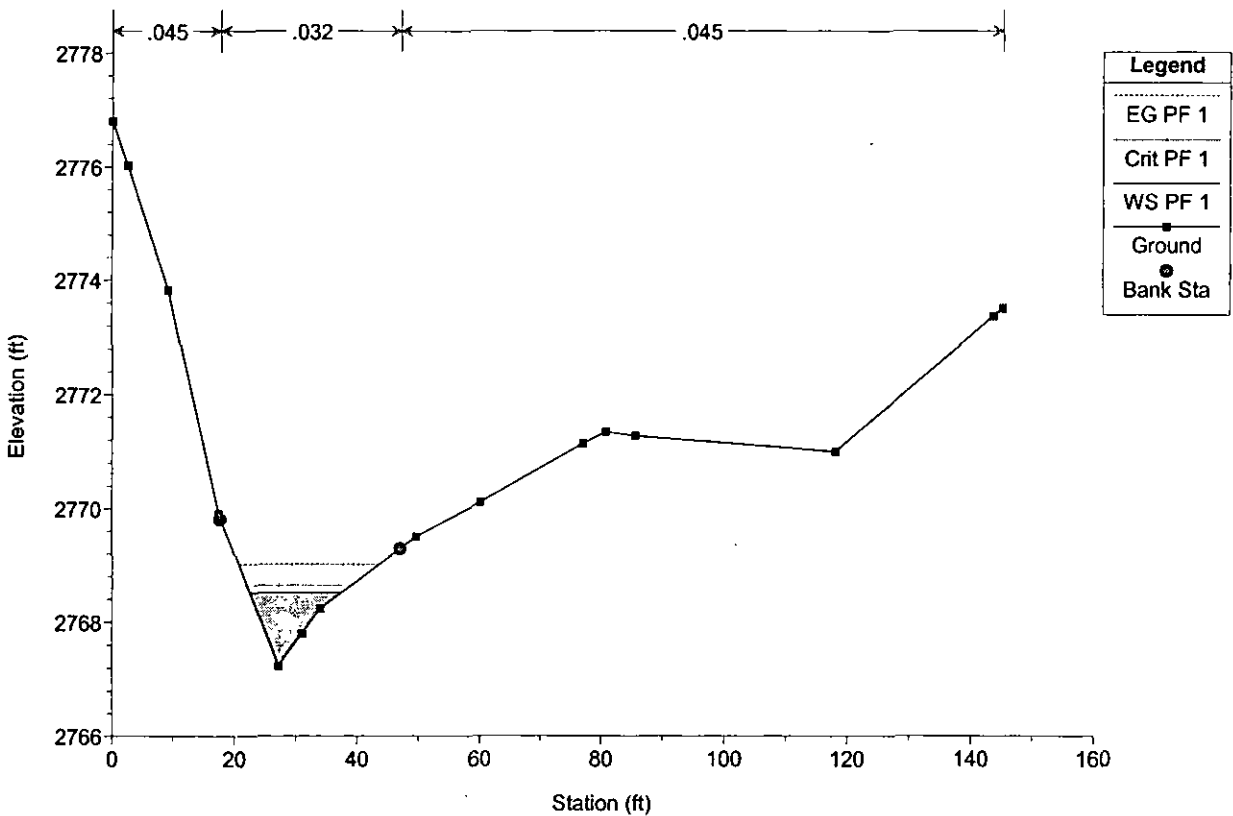
HEC-RAS Plan: Plan 01 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	5	PF 1	50.00	2771.78	2773.02	2773.28	2773.84	0.045021	7.27	7.03	11.78	1.59
Reach-1	4	PF 1	50.00	2767.23	2768.51	2768.64	2769.01	0.030968	5.67	8.82	15.02	1.30
Reach-1	3.5	PF 1	50.00	2765.00	2765.78	2765.88	2766.22	0.029393	5.31	9.41	17.14	1.26
Reach-1	3	PF 1	126.00	2762.00	2762.76	2762.84	2763.13	0.035706	4.86	25.91	62.70	1.33
Reach-1	2.5	PF 1	126.00	2759.70	2760.27	2760.30	2760.53	0.024149	4.17	31.63	79.13	1.11
Reach-1	2	PF 1	126.00	2755.39	2756.77	2757.08	2757.69	0.044435	7.69	16.51	26.88	1.68
Reach-1	1.5	PF 1	126.00	2753.00	2754.56	2754.82	2755.36	0.022867	8.53	24.06	38.24	1.29
Reach-1	1	PF 1	204.00	2745.67	2747.03	2747.17	2747.50	0.043743	7.55	42.67	90.73	1.59

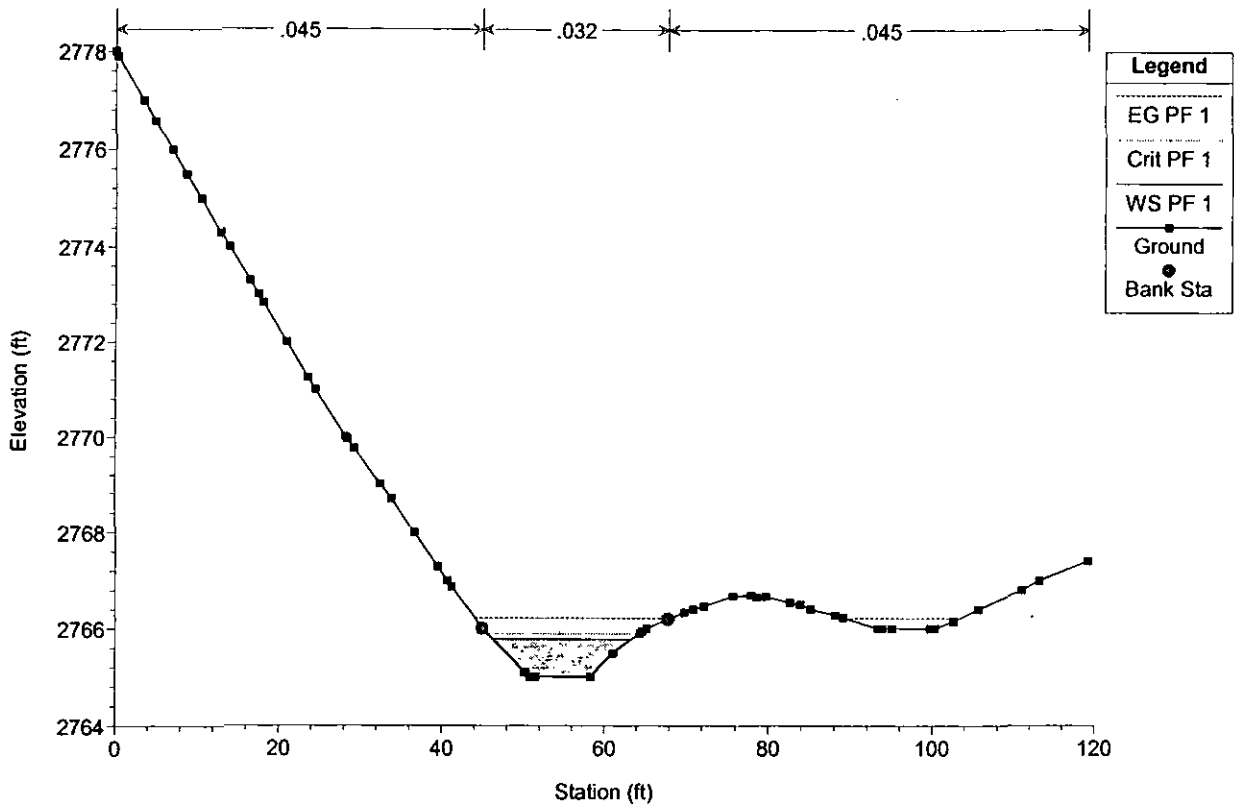
washb2-EX Plan: Plan 01 11/7/2006



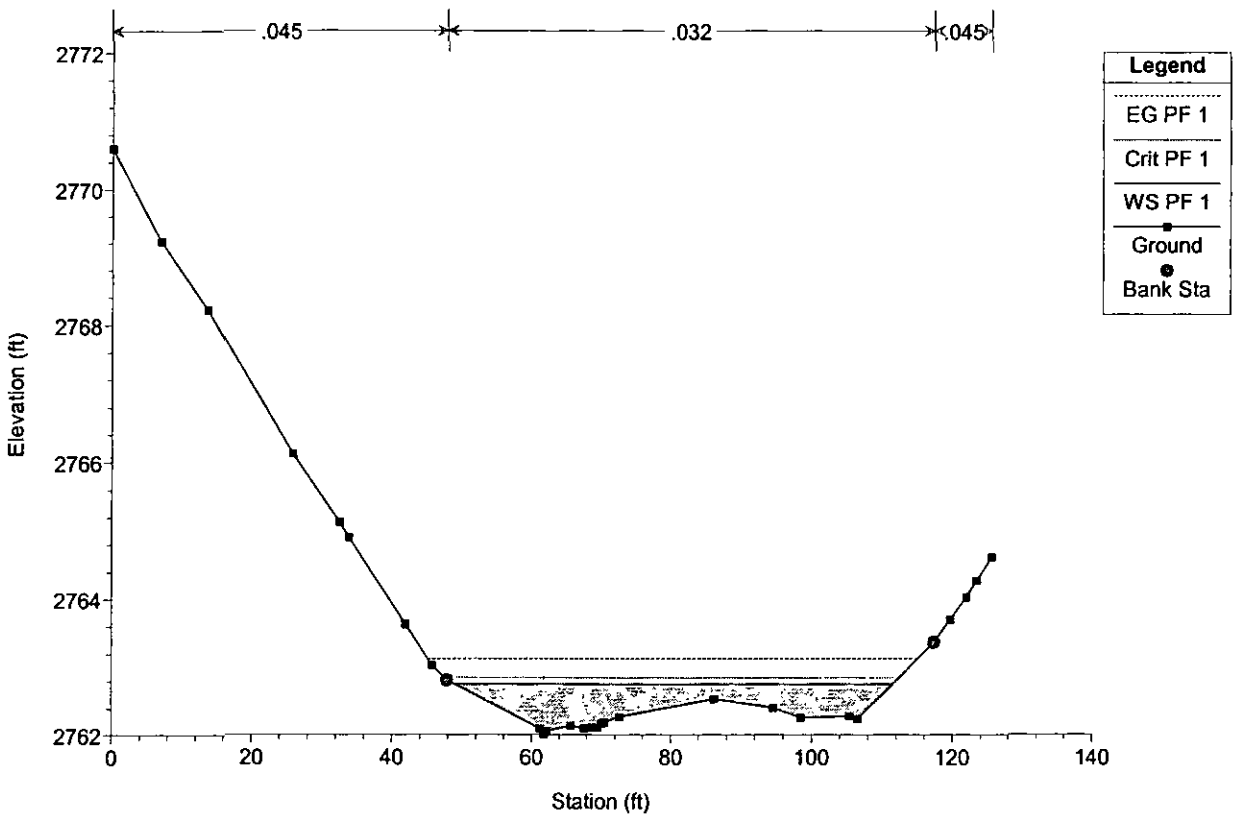
washb2-EX Plan: Plan 01 11/7/2006



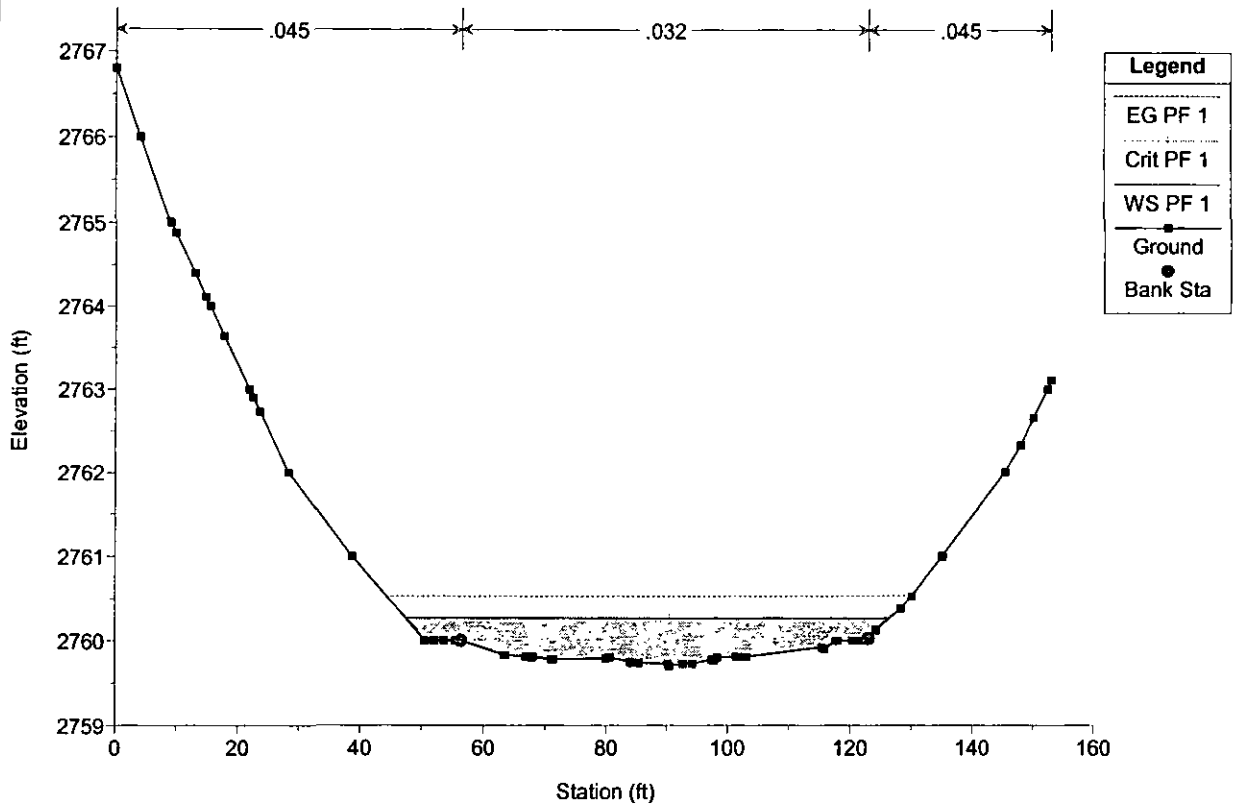
washb2-EX Plan: Plan 01 11/7/2006



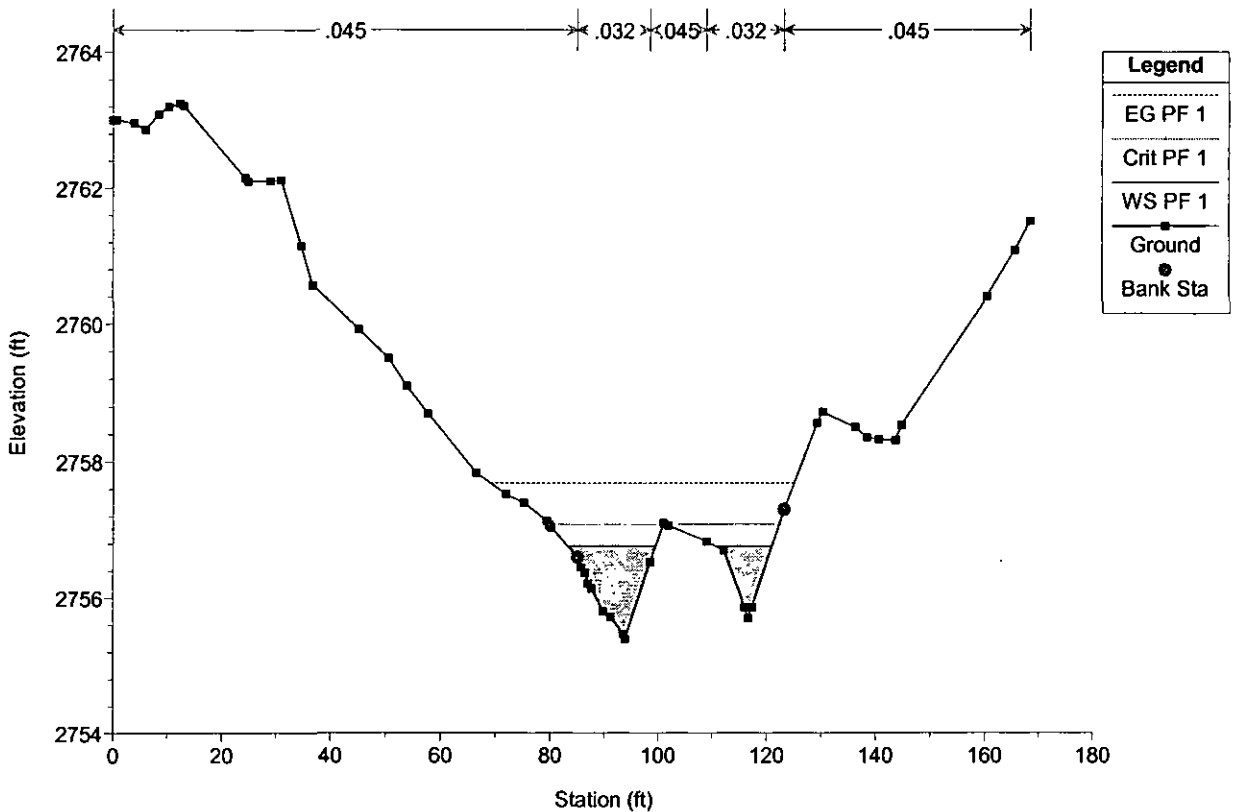
washb2-EX Plan: Plan 01 11/7/2006



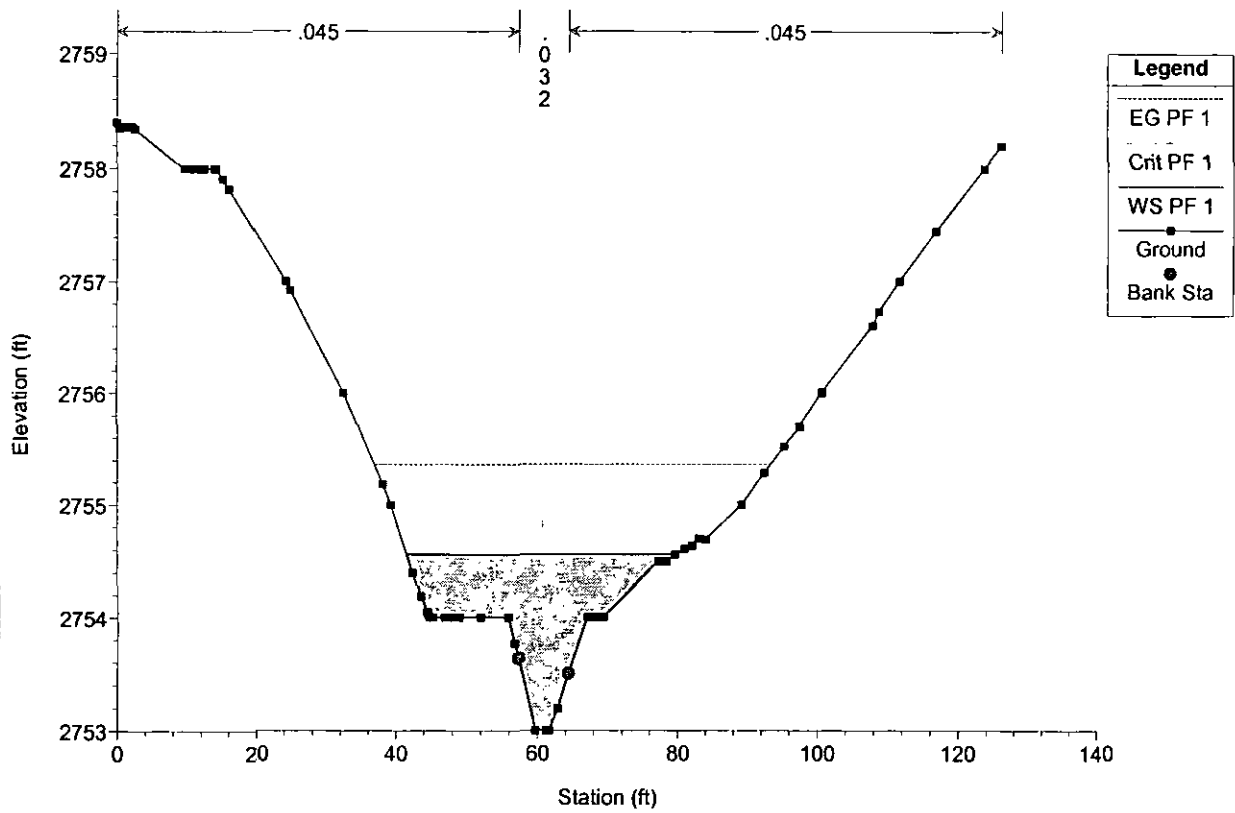
washb2-EX Plan: Plan 01 11/7/2006



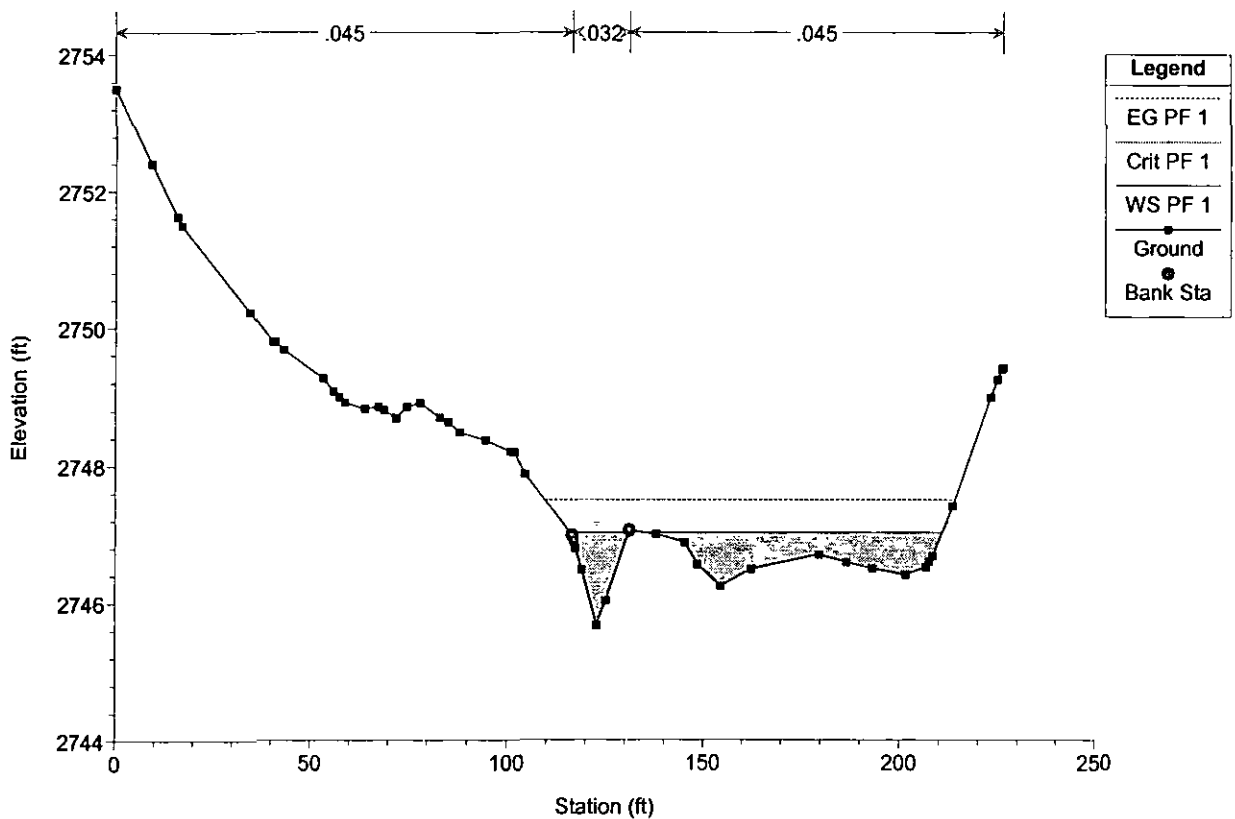
washb2-EX Plan: Plan 01 11/7/2006



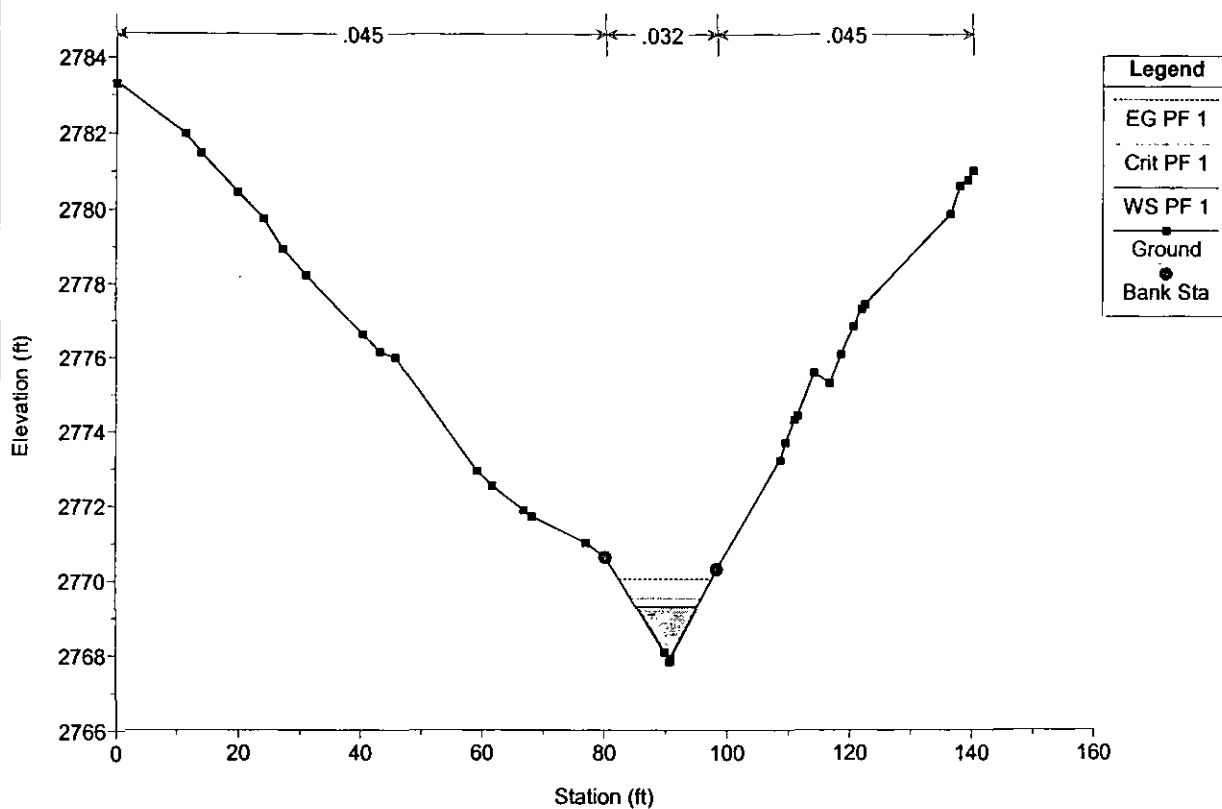
washb2-EX Plan: Plan 01 11/7/2006



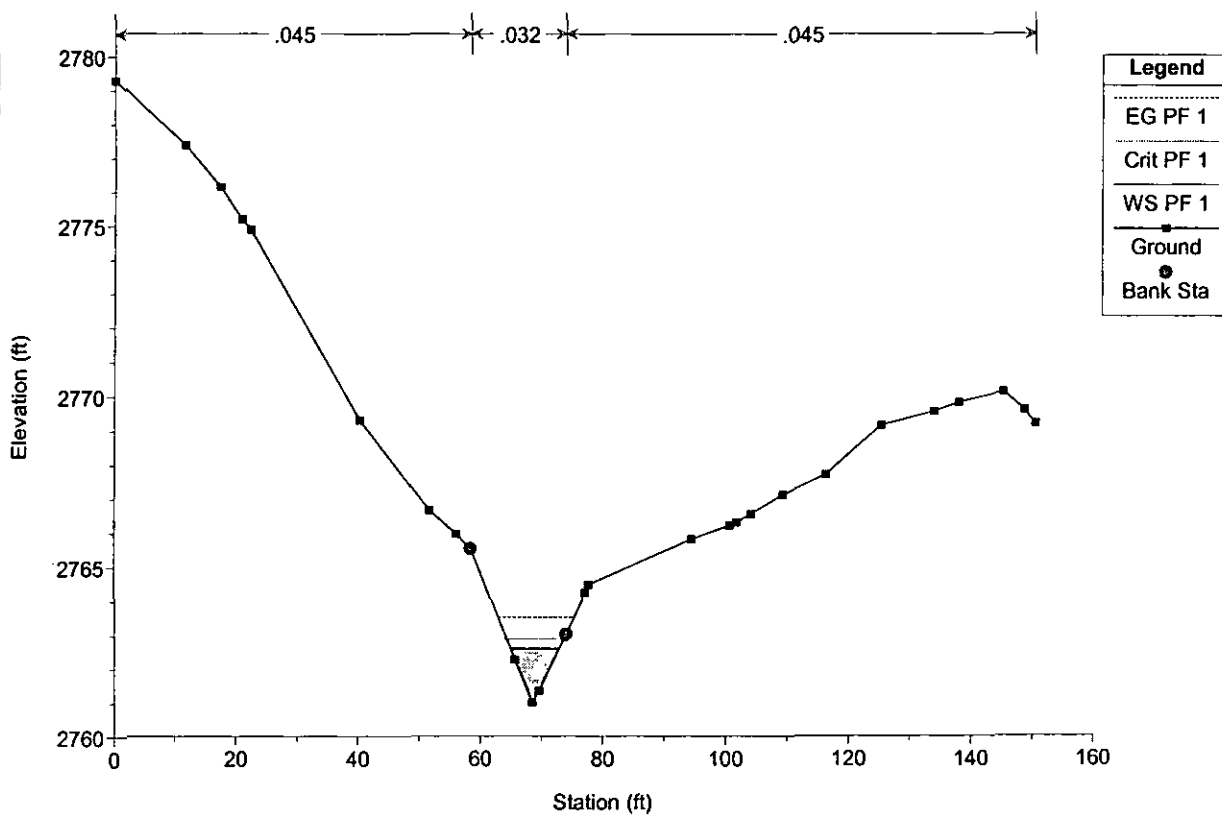
washb2-EX Plan: Plan 01 11/7/2006



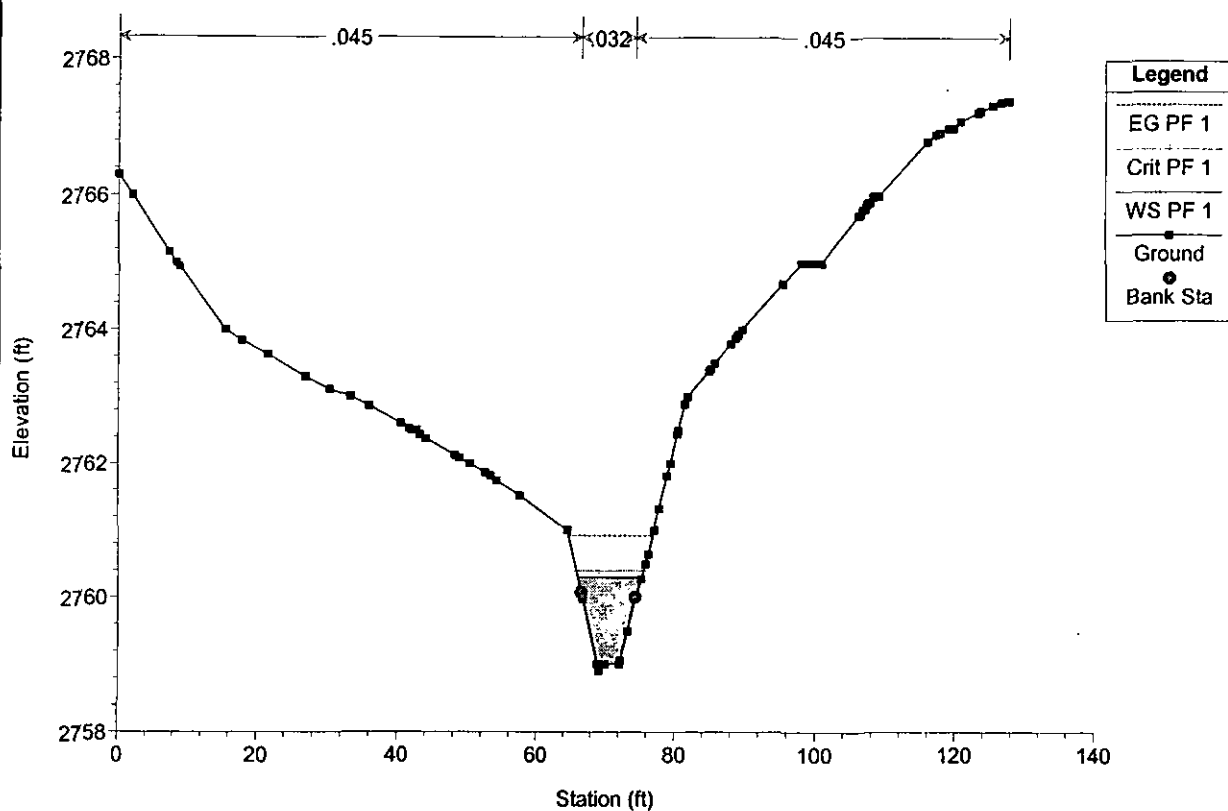
washc Plan: Plan 01 11/7/2006



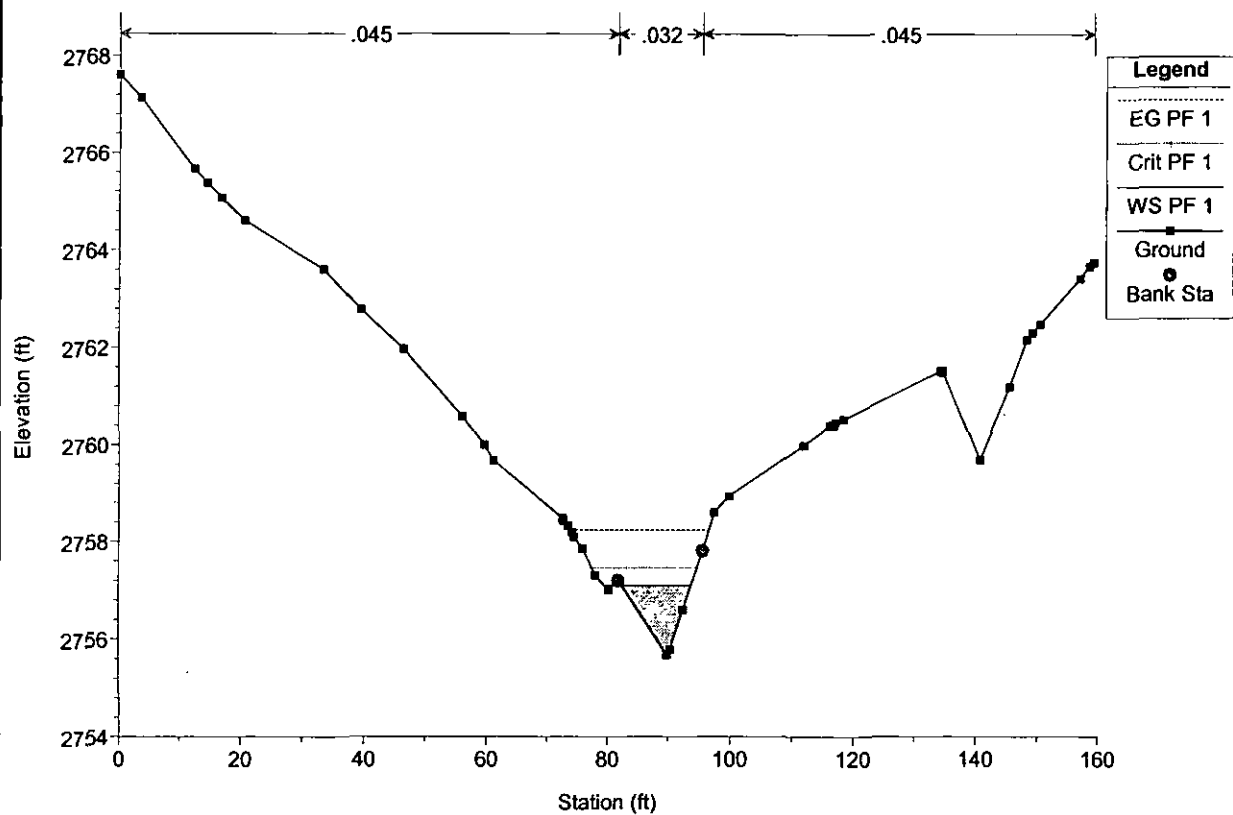
washc Plan: Plan 01 11/7/2006



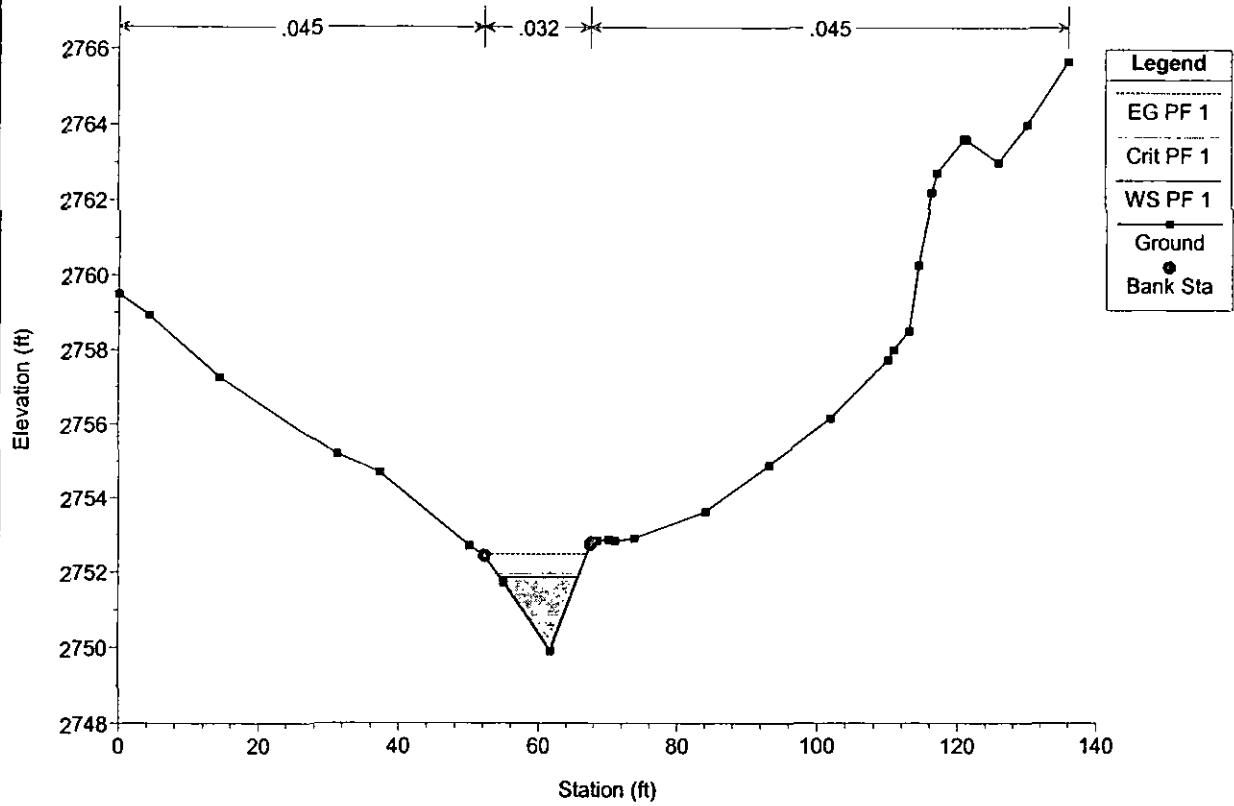
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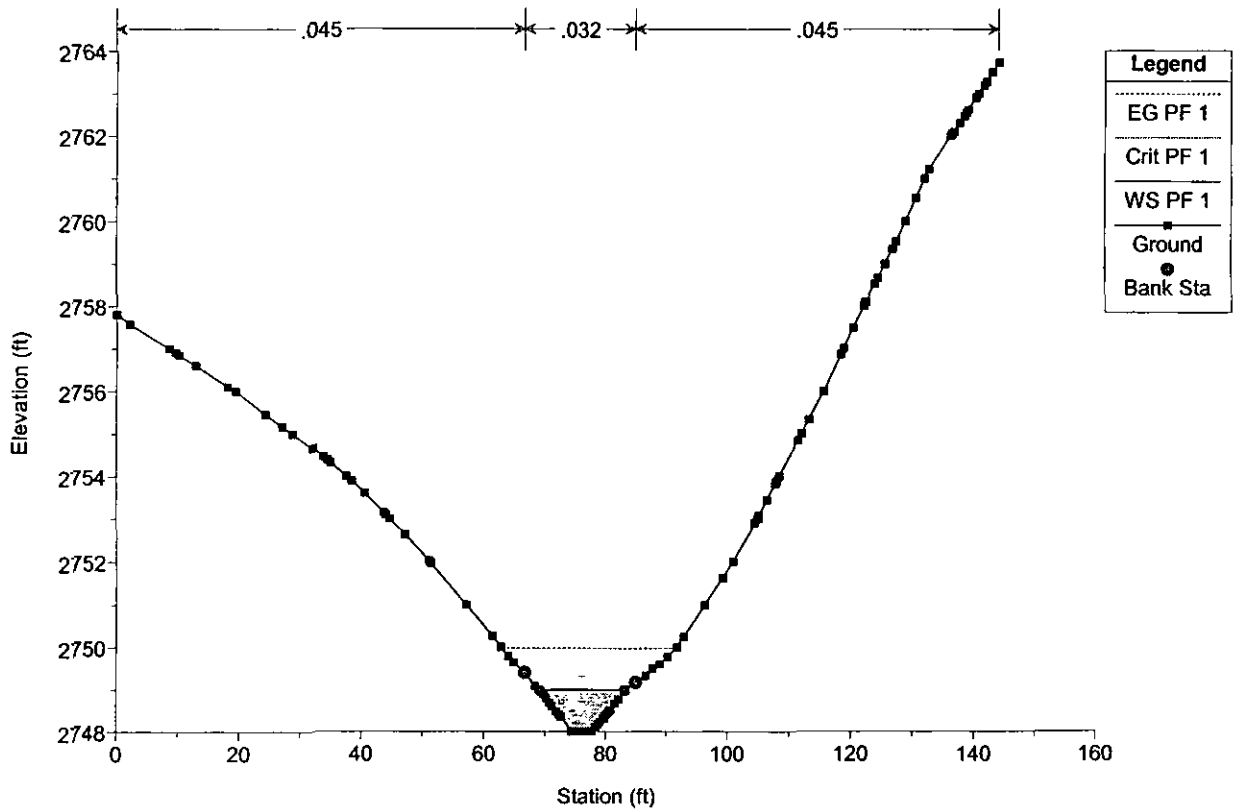
washc Plan: Plan 01 11/7/2006



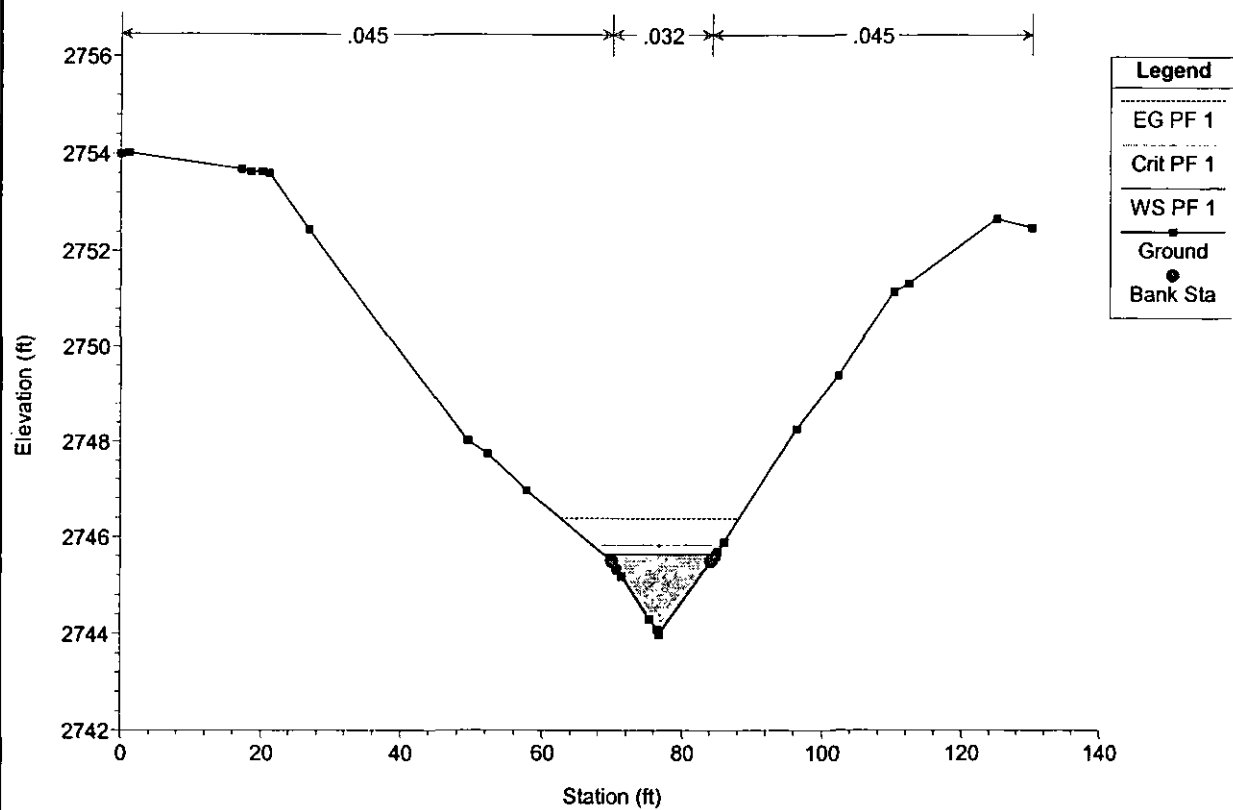
washc Plan: Plan 01 11/7/2006



washc Plan: Plan 01 11/7/2006



washc Plan: Plan 01 11/7/2006

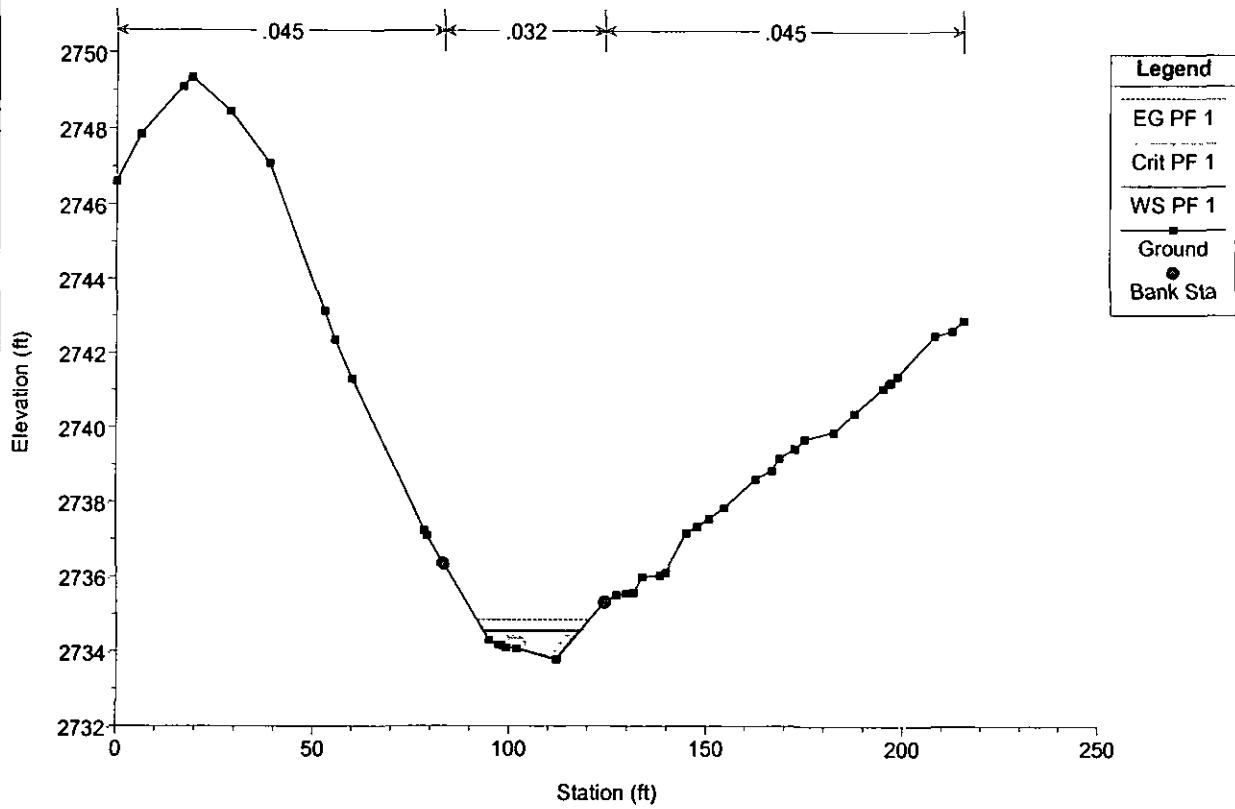


WASH D

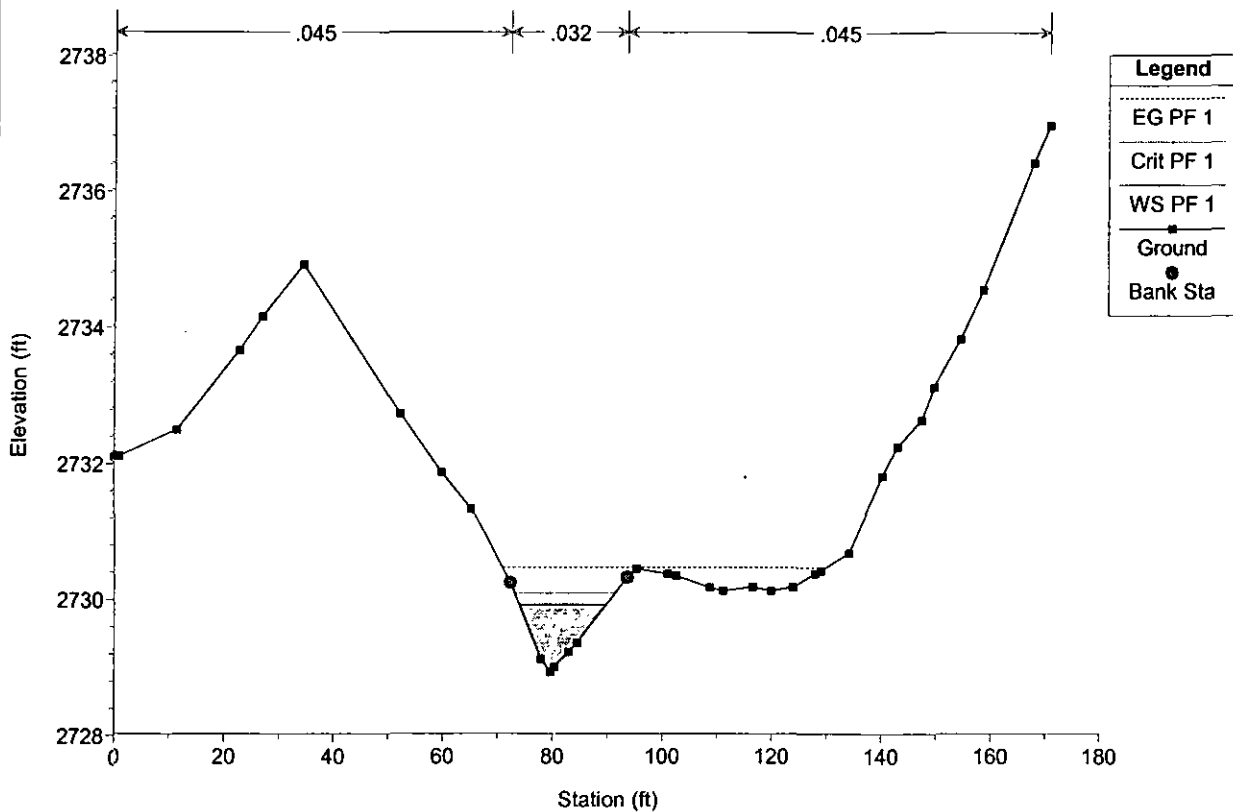
HEC-RAS Plan: Plan 01 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	4	PF 1	50.00	2733.77	2734.54	2734.58	2734.84	0.025006	4.39	11.38	24.53	1.14
Reach-1	3	PF 1	50.00	2728.90	2729.90	2730.06	2730.45	0.038474	5.94	8.42	15.86	1.44
Reach-1	2.5	PF 1	50.00	2727.51	2728.11	2728.12	2728.28	0.023407	3.32	15.16	49.07	1.03
Reach-1	2	PF 1	93.00	2723.41	2724.56	2724.78	2725.31	0.029370	7.14	14.78	24.24	1.36
Reach-1	1	PF 1	93.00	2718.95	2719.84	2719.88	2720.16	0.023707	4.50	20.86	44.50	1.12

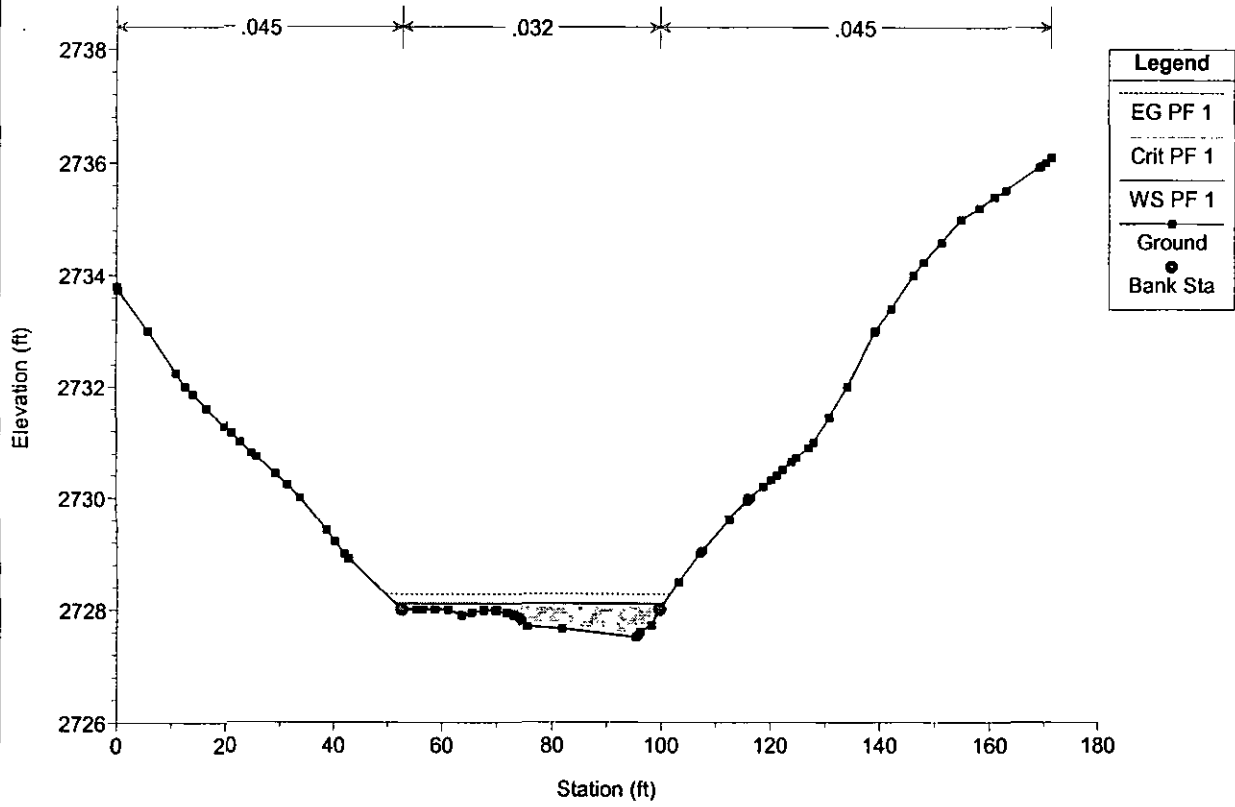
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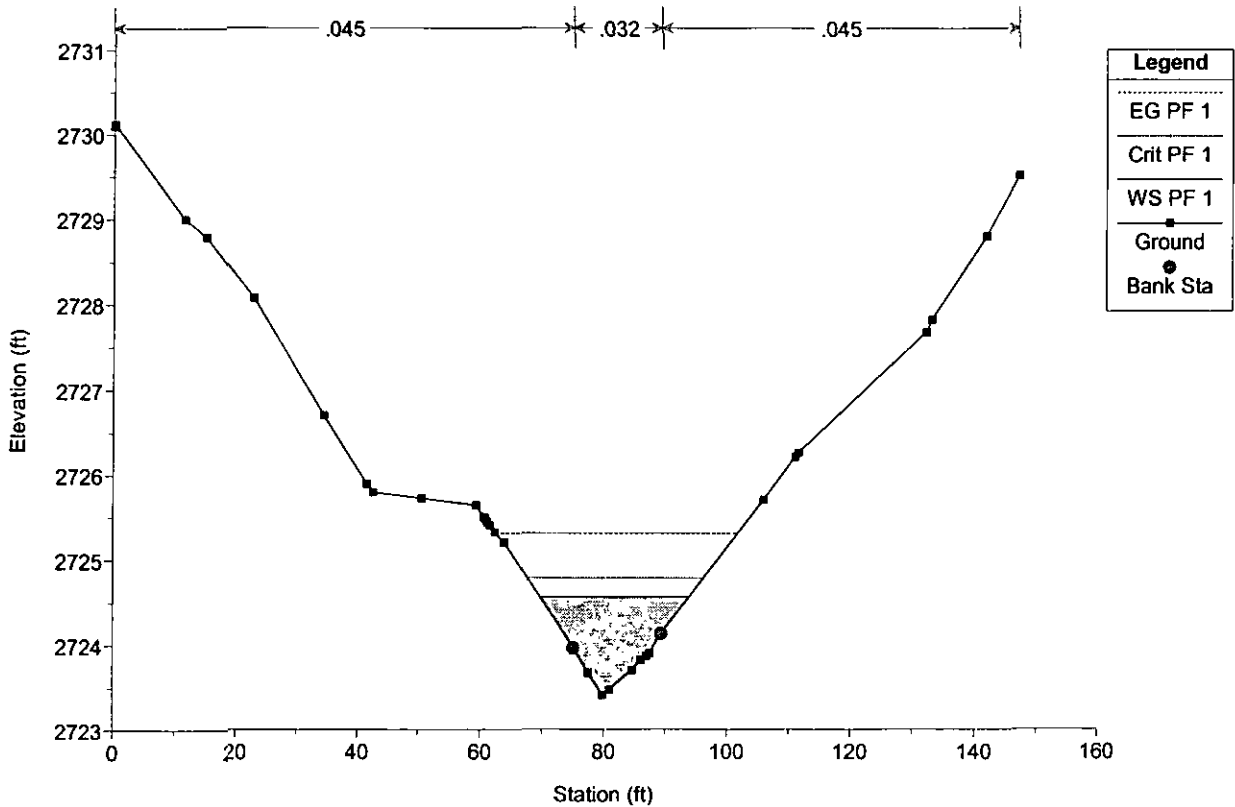
wasd Plan: Plan 01 11/7/2006



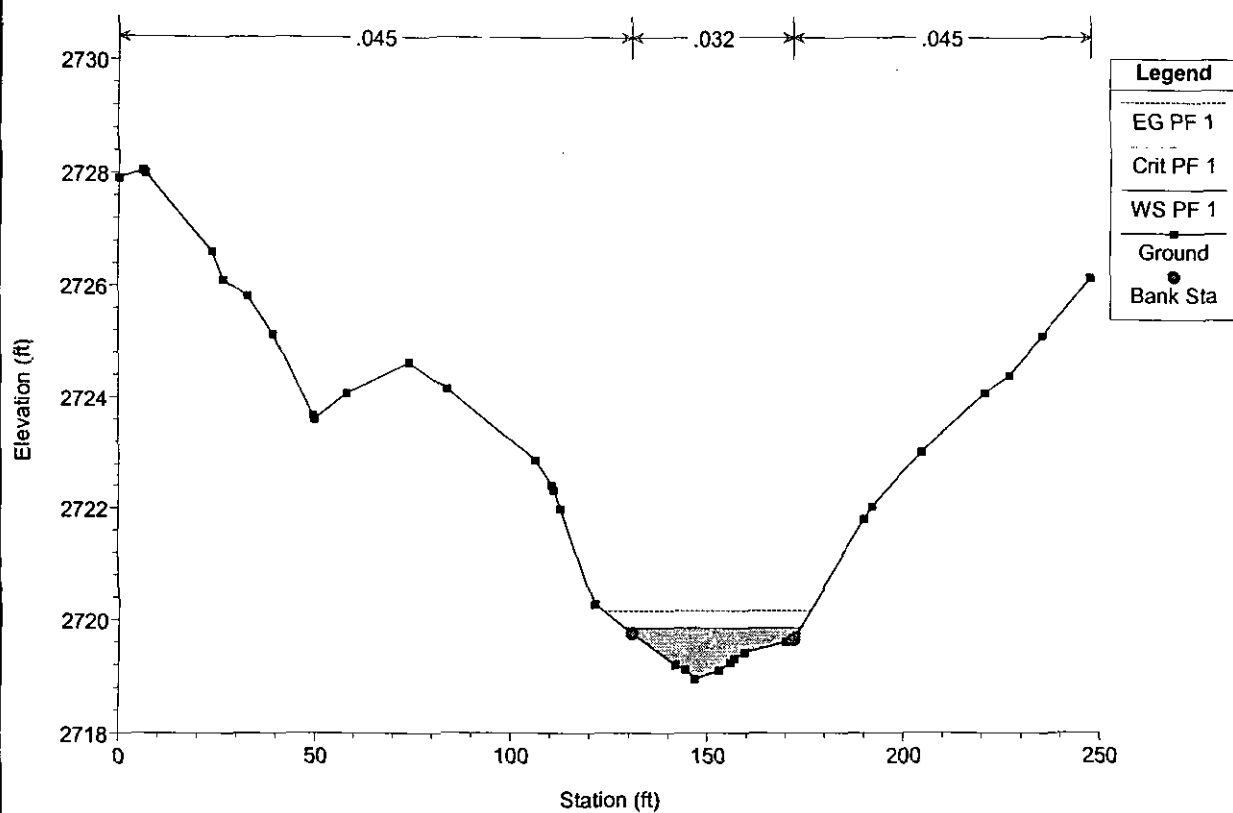
washd Plan: Plan 01 11/7/2006



washd Plan: Plan 01 11/7/2006



washd Plan: Plan 01 11/7/2006

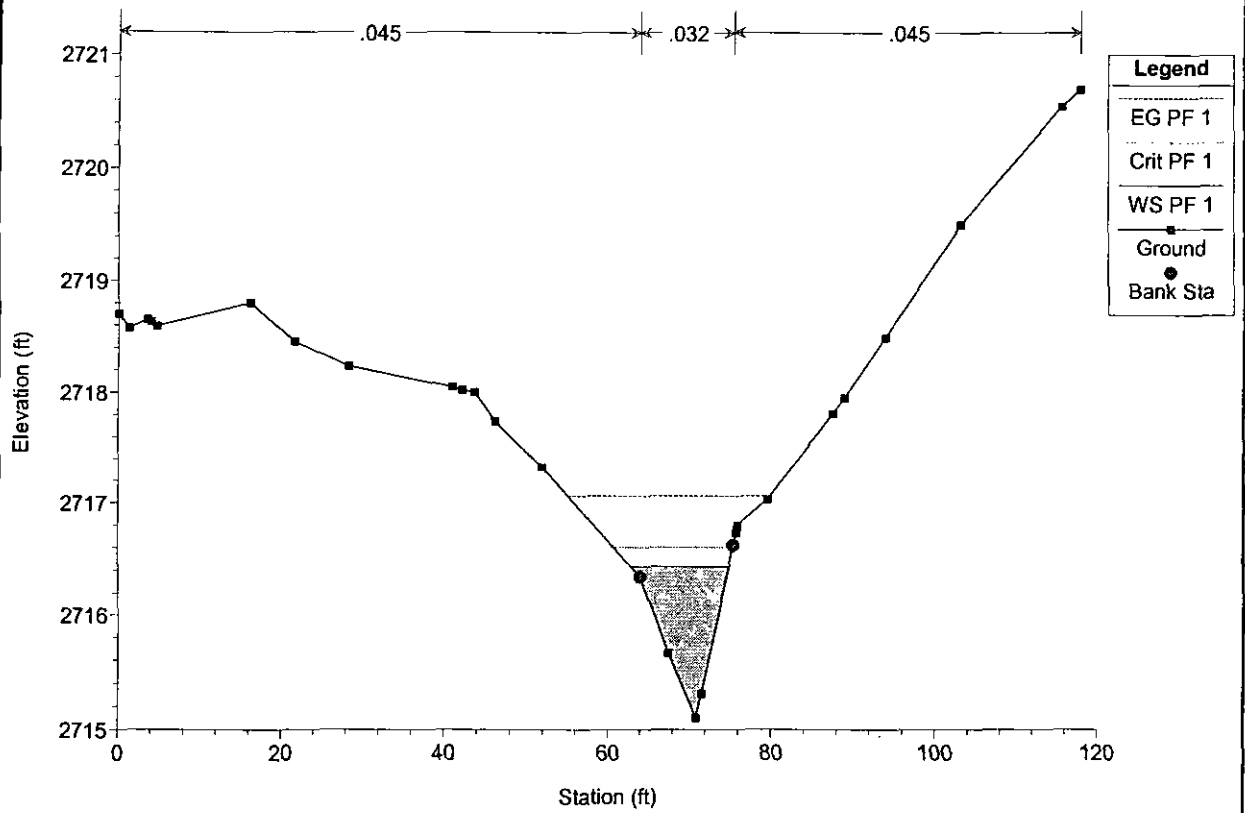


WASH E1

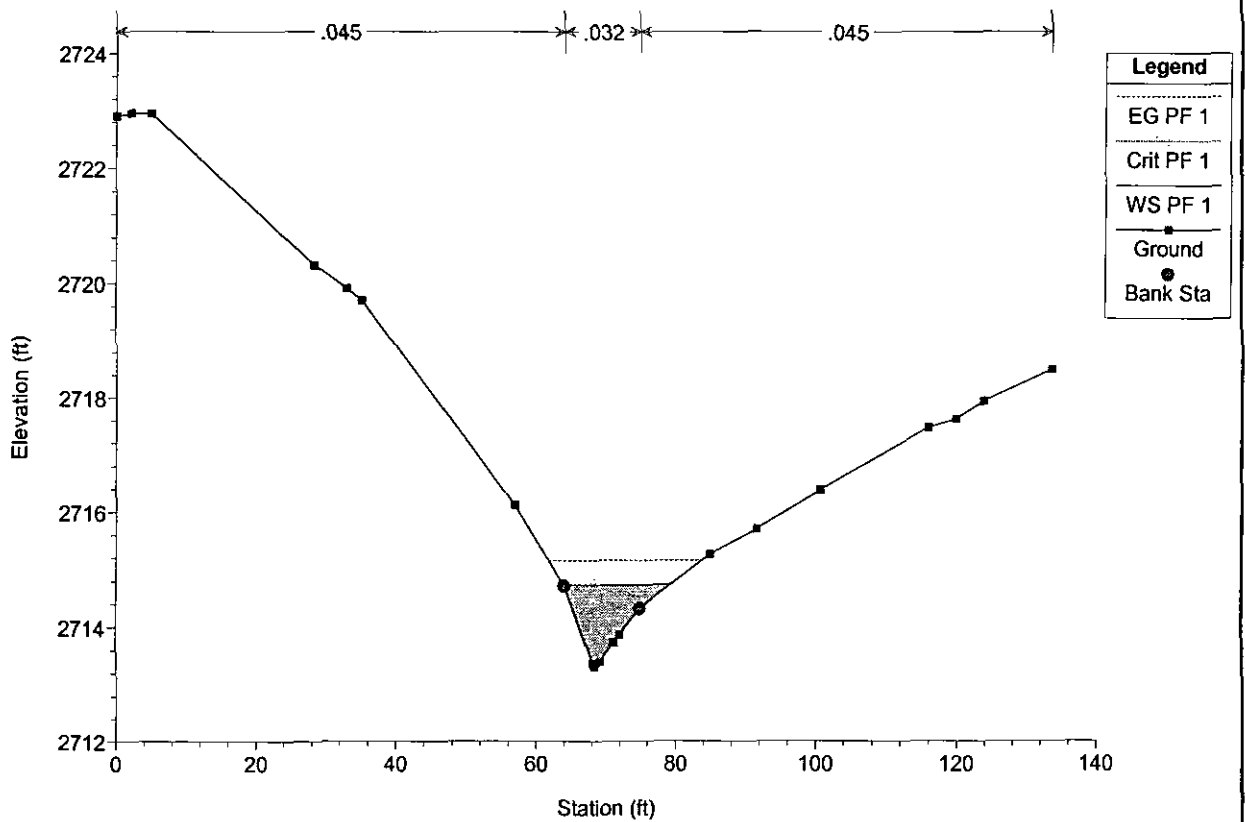
HEC-RAS Plan: Plan 01 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	13	PF 1	50.00	2715.10	2716.43	2716.60	2717.06	0.030018	6.34	7.93	12.10	1.32
Reach-1	12	PF 1	50.00	2713.32	2714.73	2714.77	2715.16	0.017142	5.31	10.05	15.35	1.02
Reach-1	11	PF 1	50.00	2711.10	2712.13	2712.36	2712.87	0.045013	6.89	7.38	13.63	1.58
Reach-1	10	PF 1	50.00	2708.30	2709.94	2710.07	2710.51	0.018336	6.16	9.08	14.42	1.07
Reach-1	9	PF 1	50.00	2706.40	2707.58	2707.84	2708.40	0.040012	7.29	7.00	11.58	1.52
Reach-1	8	PF 1	50.00	2703.40	2704.77	2705.10	2705.82	0.046780	8.23	6.21	9.25	1.65
Reach-1	7	PF 1	50.00	2701.44	2702.70	2702.85	2703.29	0.030785	6.17	8.11	12.04	1.32
Reach-1	6	PF 1	50.00	2700.00	2701.35	2701.35	2701.71	0.016870	4.84	10.45	15.48	1.00
Reach-1	5	PF 1	110.00	2696.59	2698.23	2698.33	2698.83	0.023198	6.20	17.84	22.36	1.20
Reach-1	4	PF 1	110.00	2692.81	2694.53	2694.74	2695.31	0.024021	7.14	16.08	20.48	1.25
Reach-1	3.5	PF 1	110.00	2692.00	2692.91	2692.96	2693.32	0.017181	5.30	23.38	38.13	1.04
Reach-1	3	PF 1	146.00	2689.21	2690.98	2691.23	2691.71	0.019310	7.16	25.54	49.17	1.16
Reach-1	2	PF 1	275.00	2679.83	2681.81	2682.35	2683.49	0.037223	10.45	27.53	27.33	1.63
Reach-1	1	PF 1	275.00	2677.00	2679.36	2679.56	2680.26	0.015675	8.37	43.76	40.47	1.10

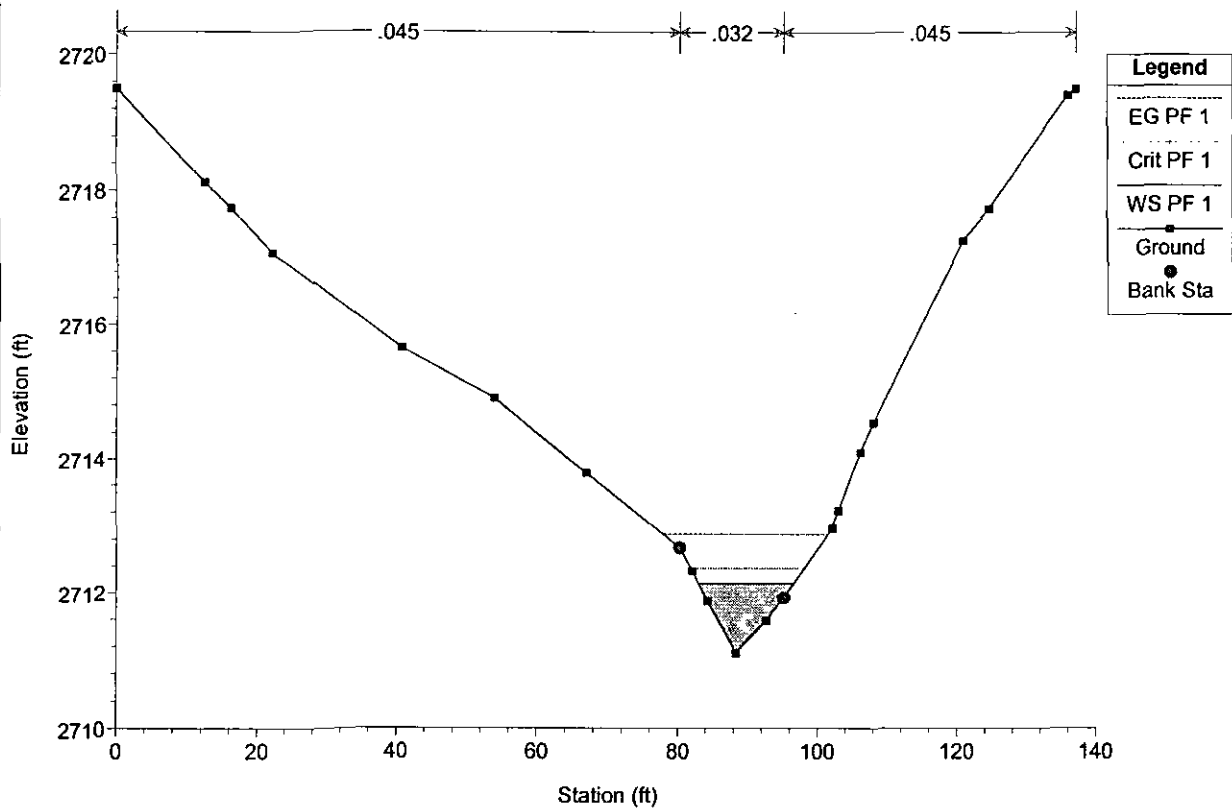
washe1 Plan: Plan 01 11/7/2006



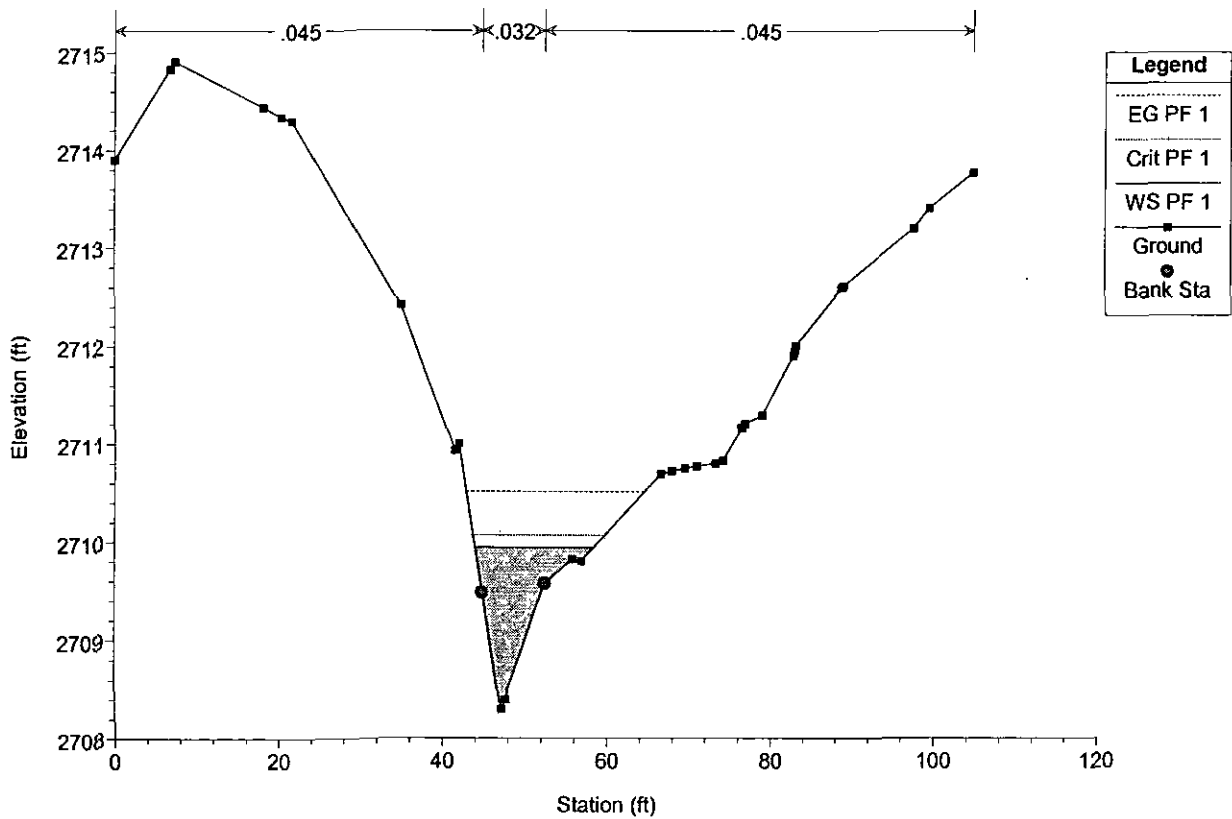
washe1 Plan: Plan 01 11/7/2006



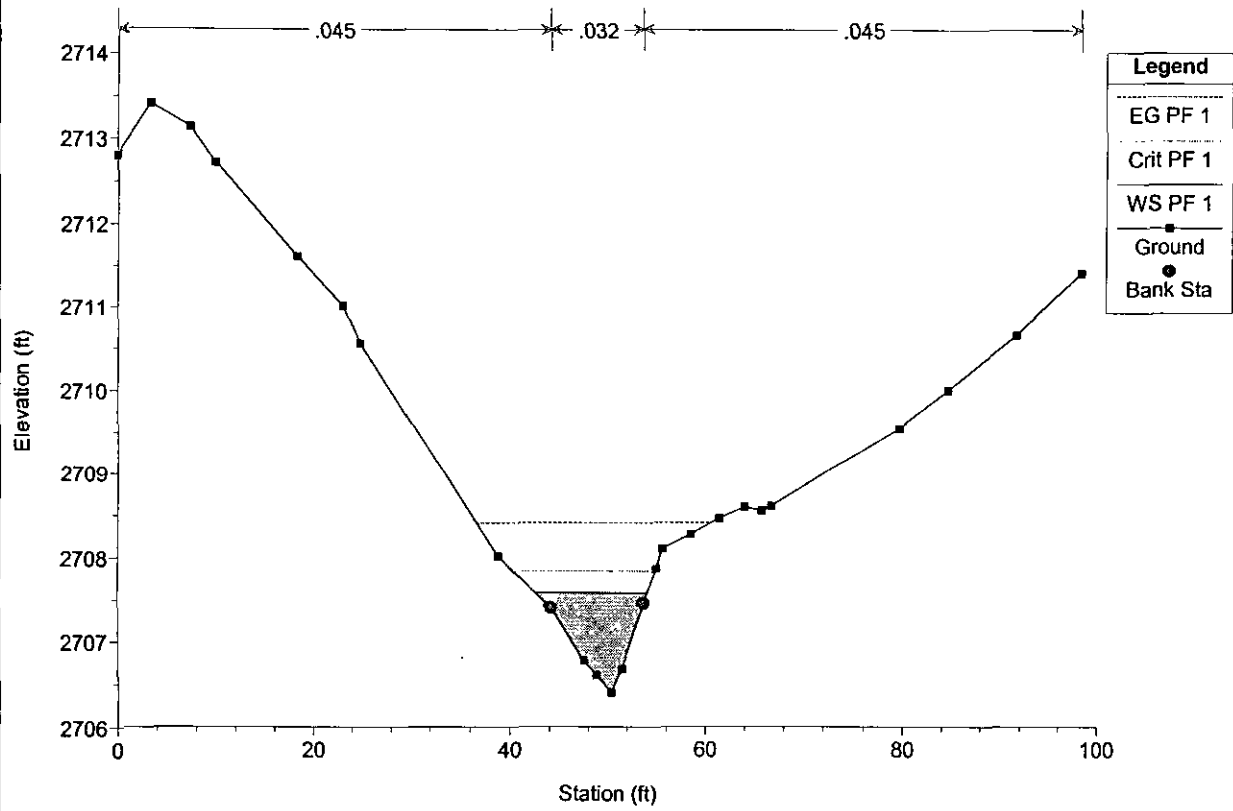
washe1 Plan: Plan 01 11/7/2006



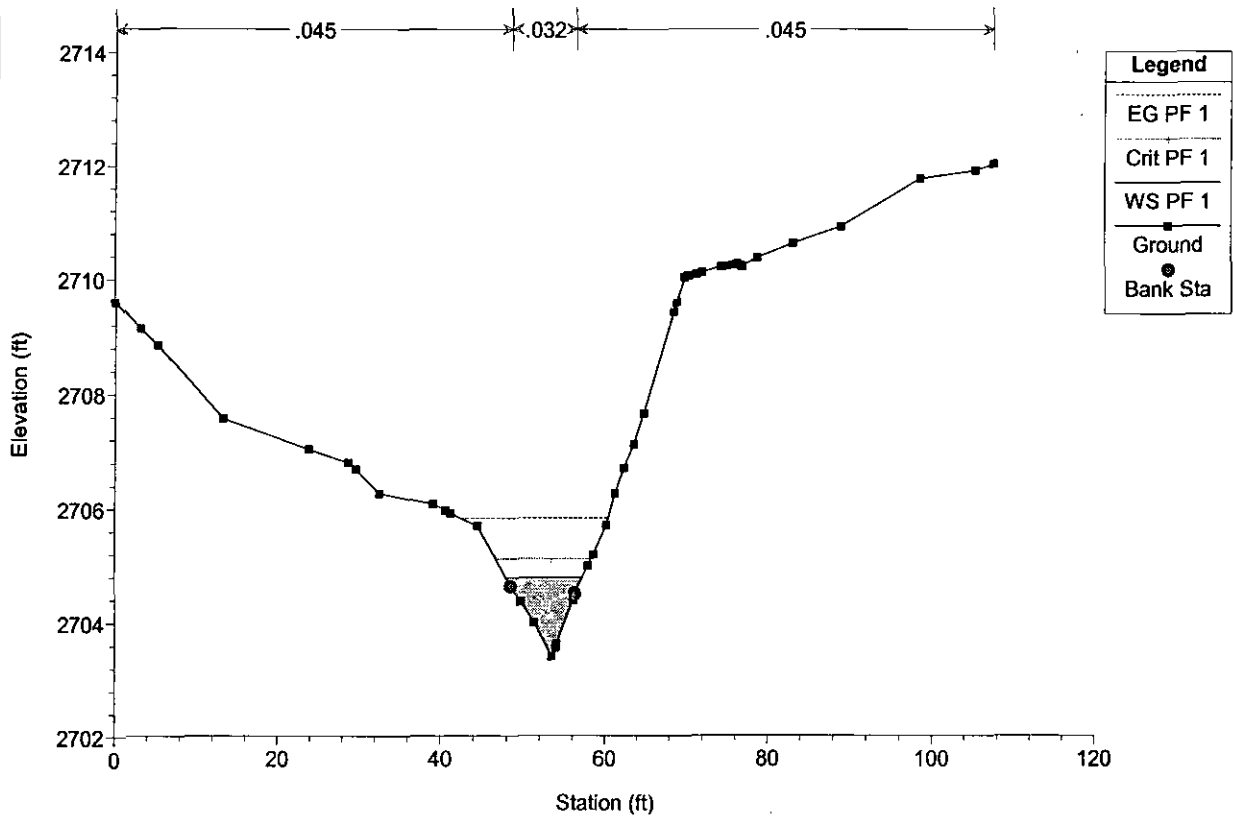
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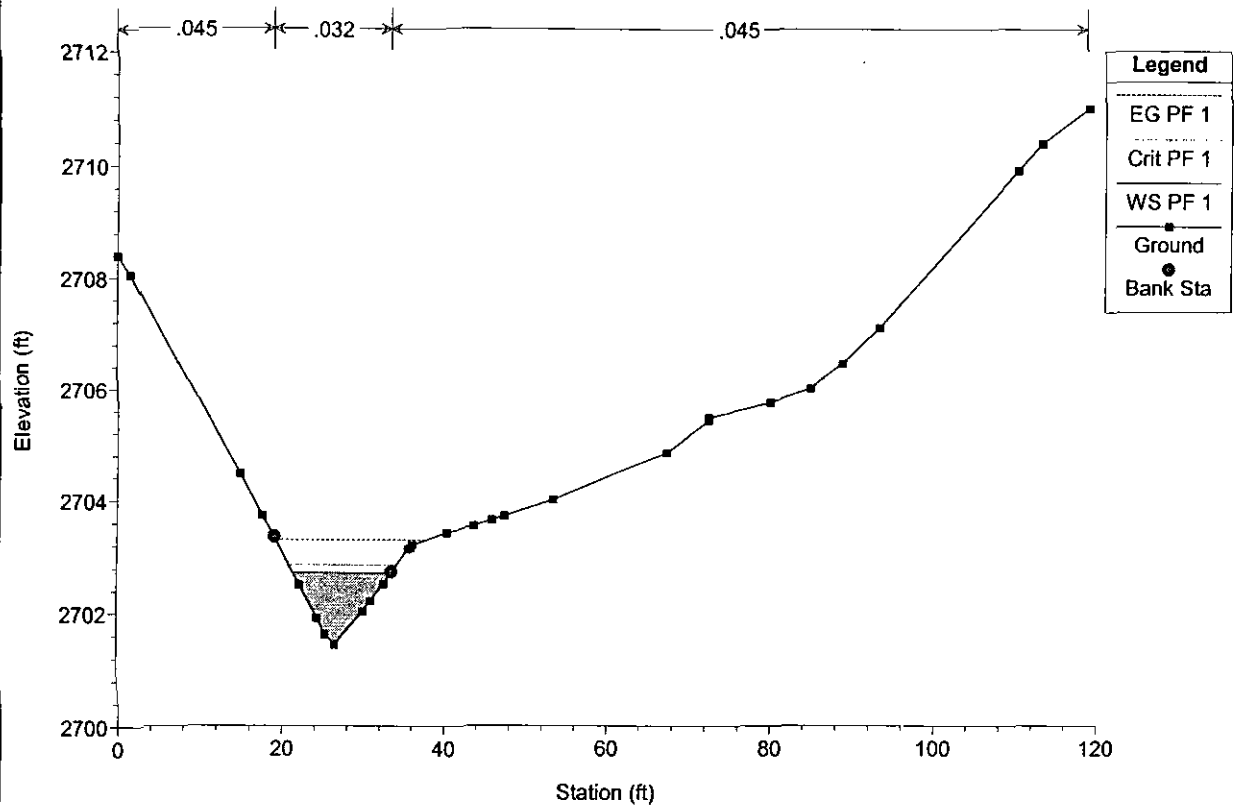
washe1 Plan: Plan 01 11/7/2006



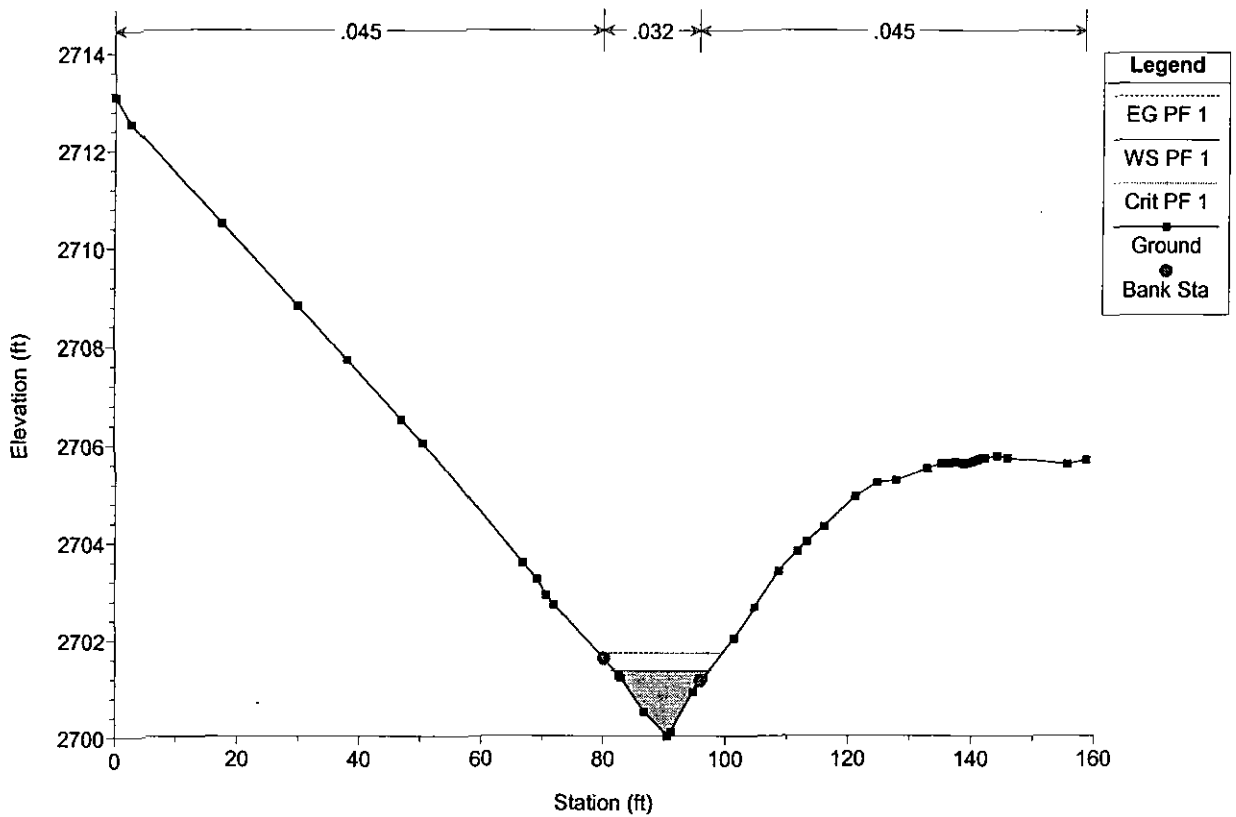
washe1 Plan: Plan 01 11/7/2006



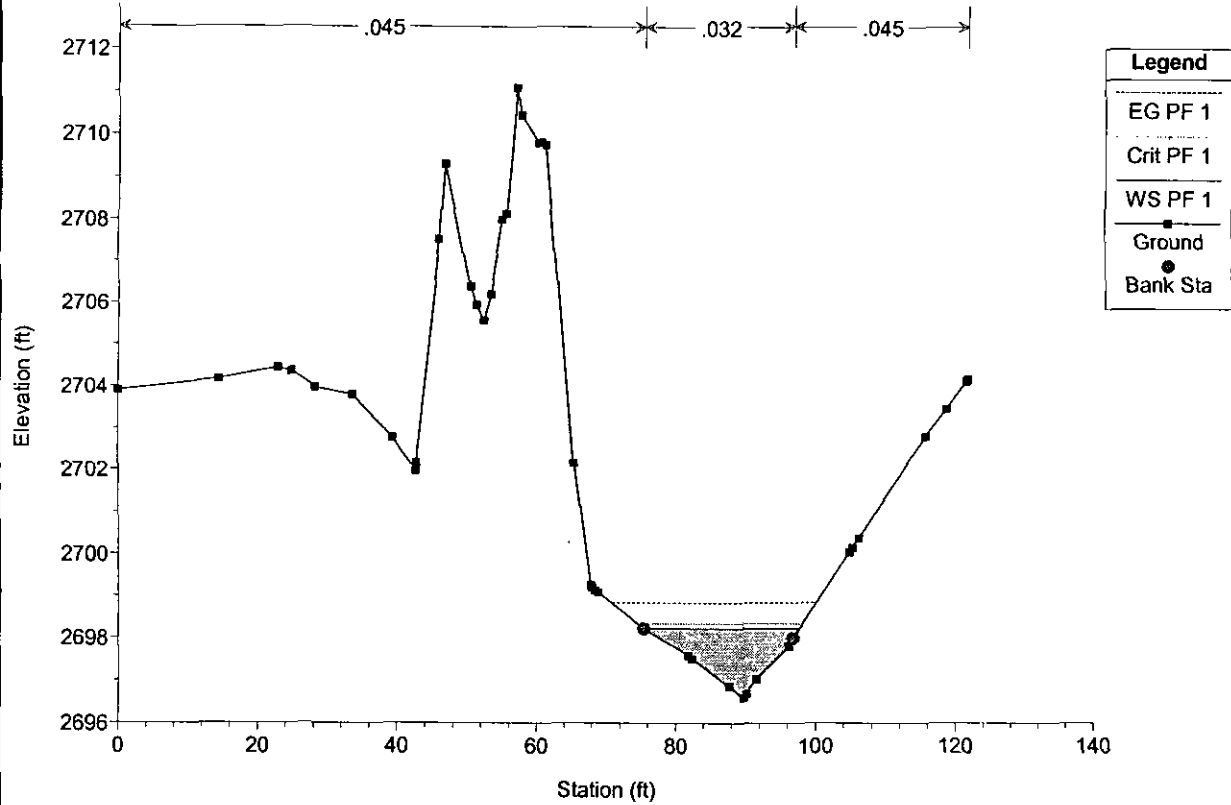
washe1 Plan: Plan 01 11/7/2006



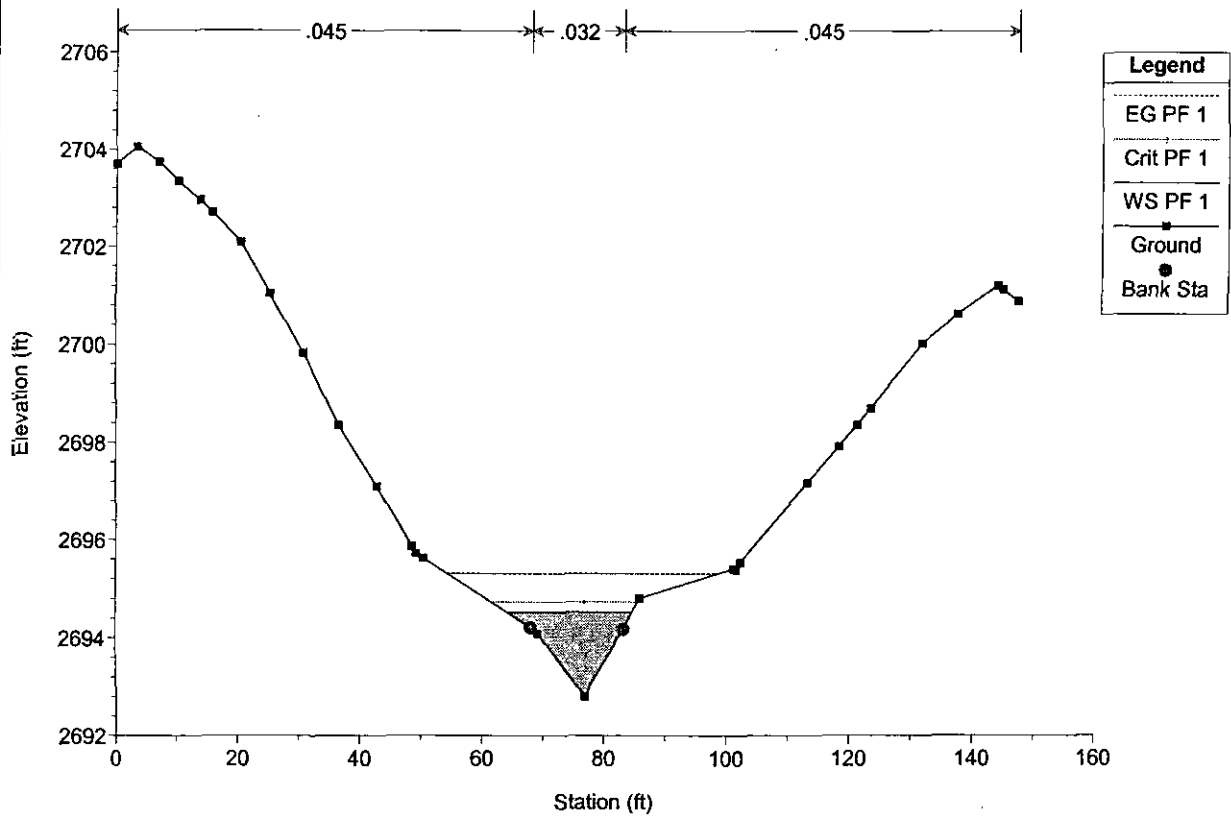
washe1 Plan: Plan 01 11/7/2006



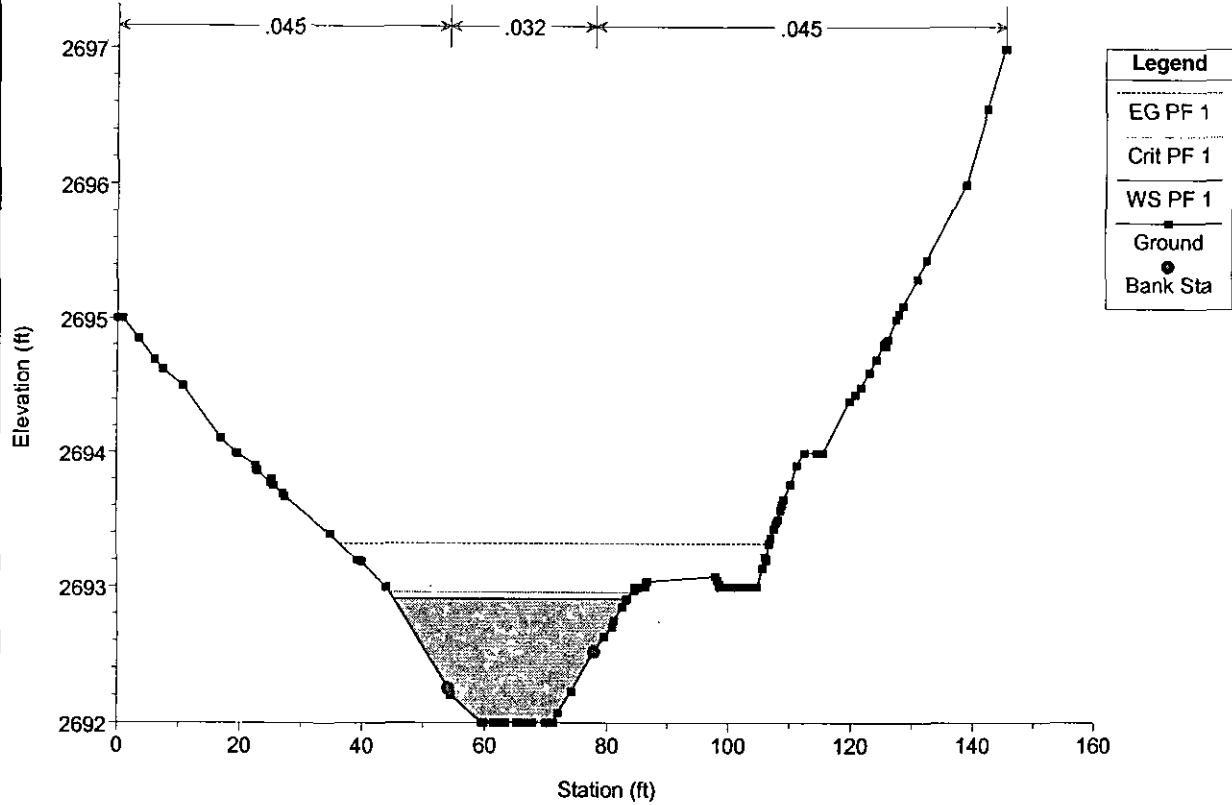
washe1 Plan: Plan 01 11/7/2006



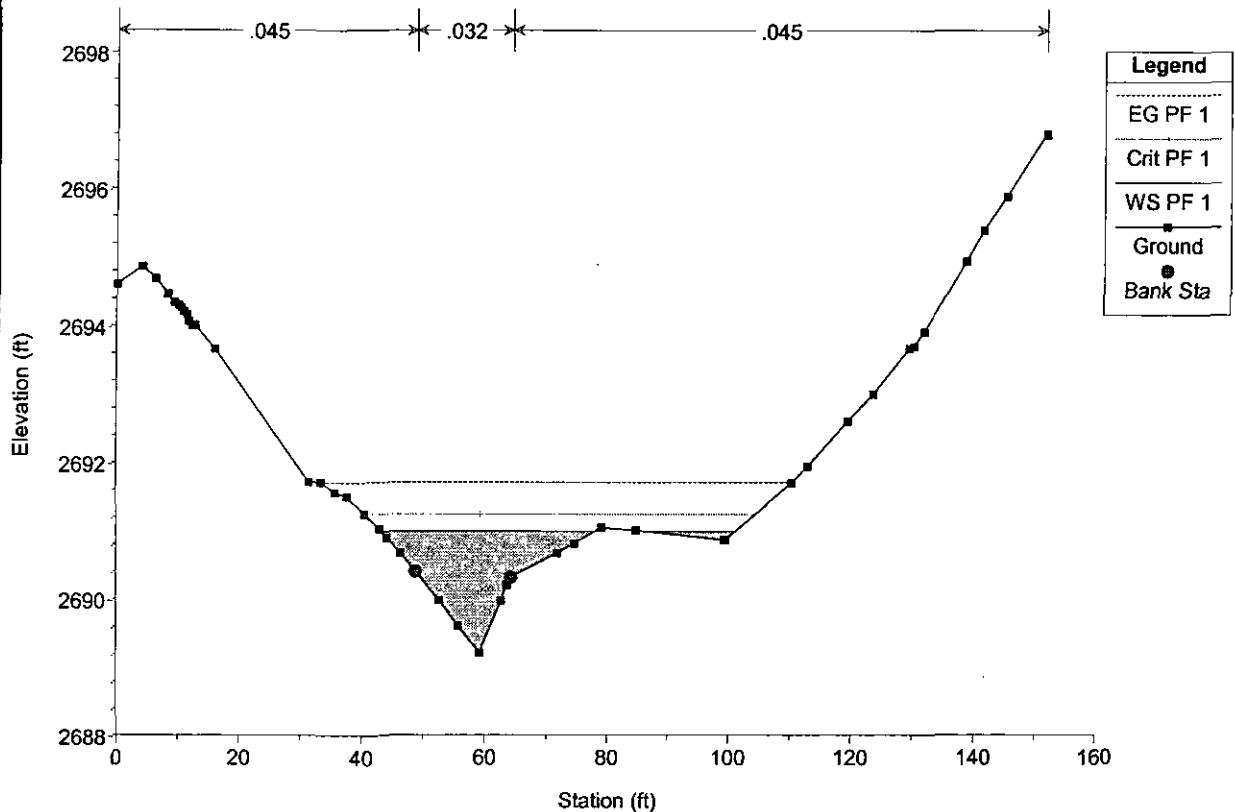
washe1 Plan: Plan 01 11/7/2006



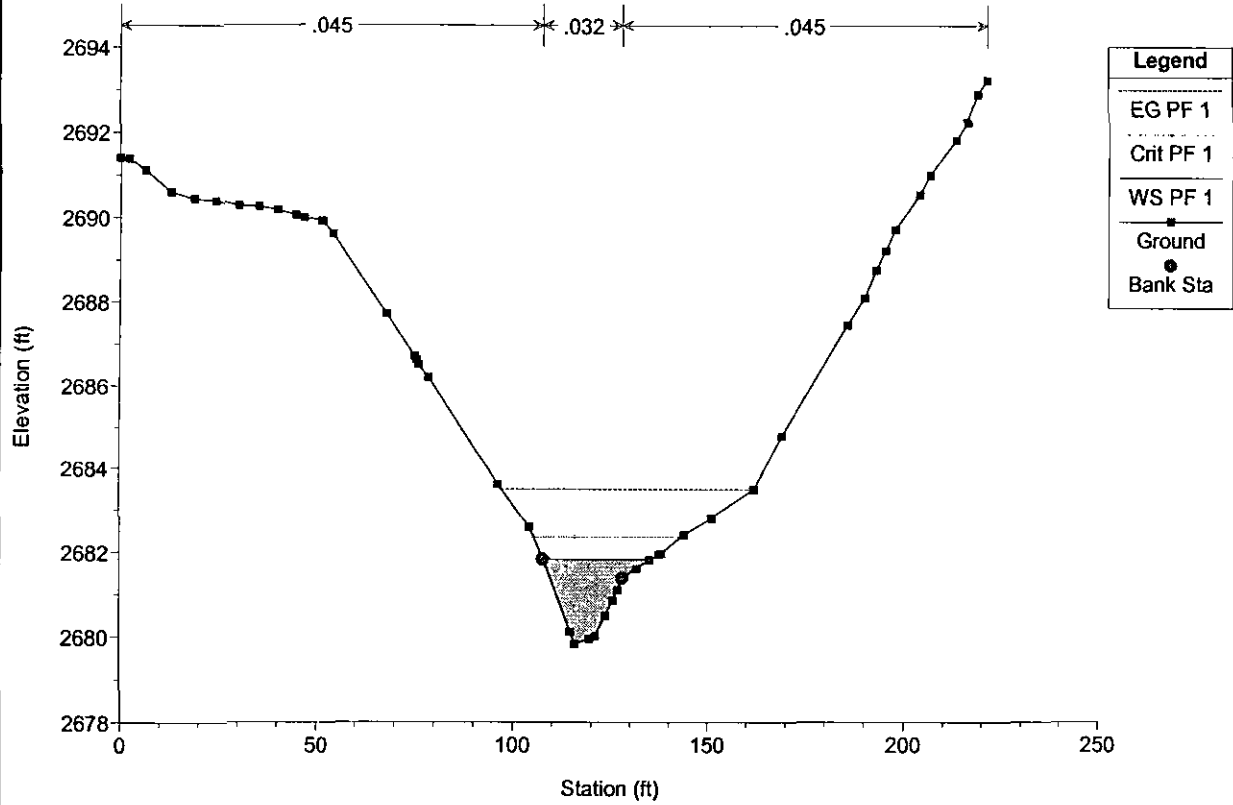
washe1 Plan: Plan 01 11/7/2006



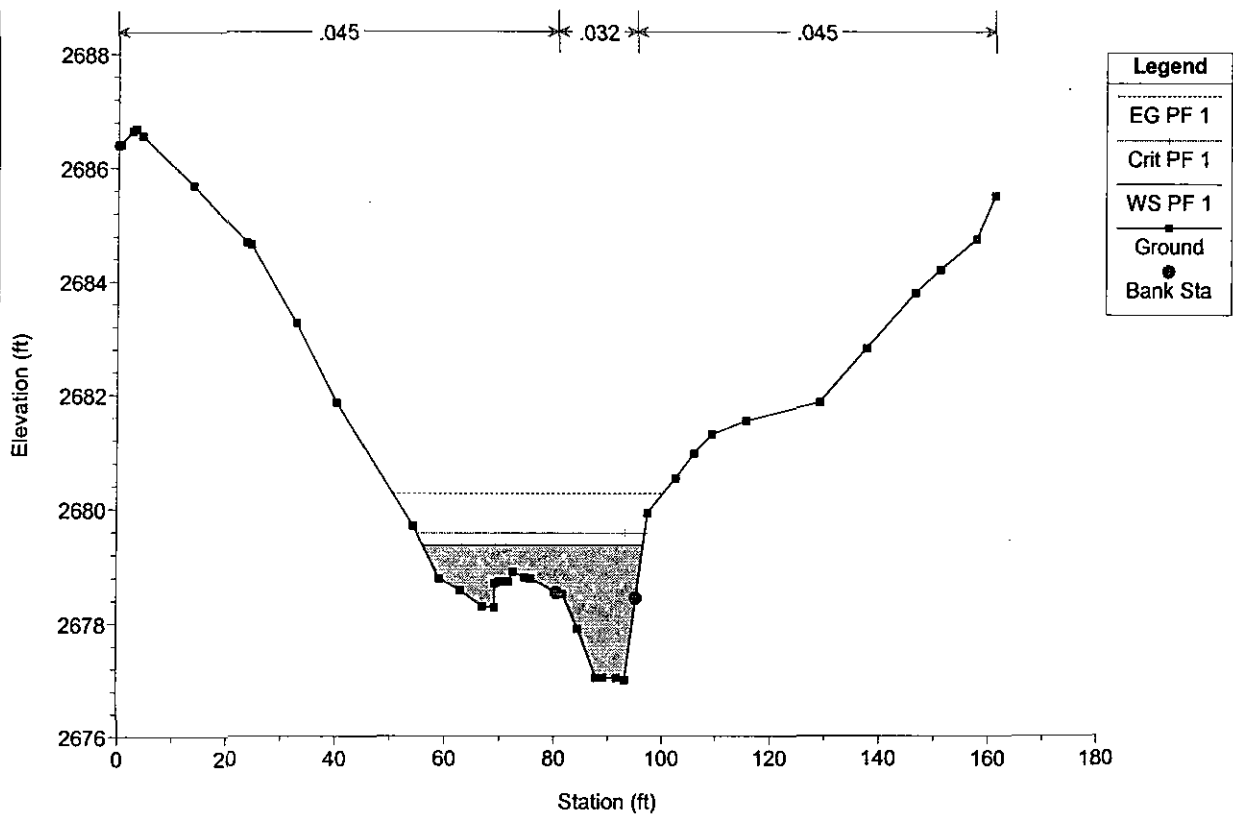
washe1 Plan: Plan 01 11/7/2006



washe1 Plan: Plan 01 11/7/2006



washe1 Plan: Plan 01 11/7/2006

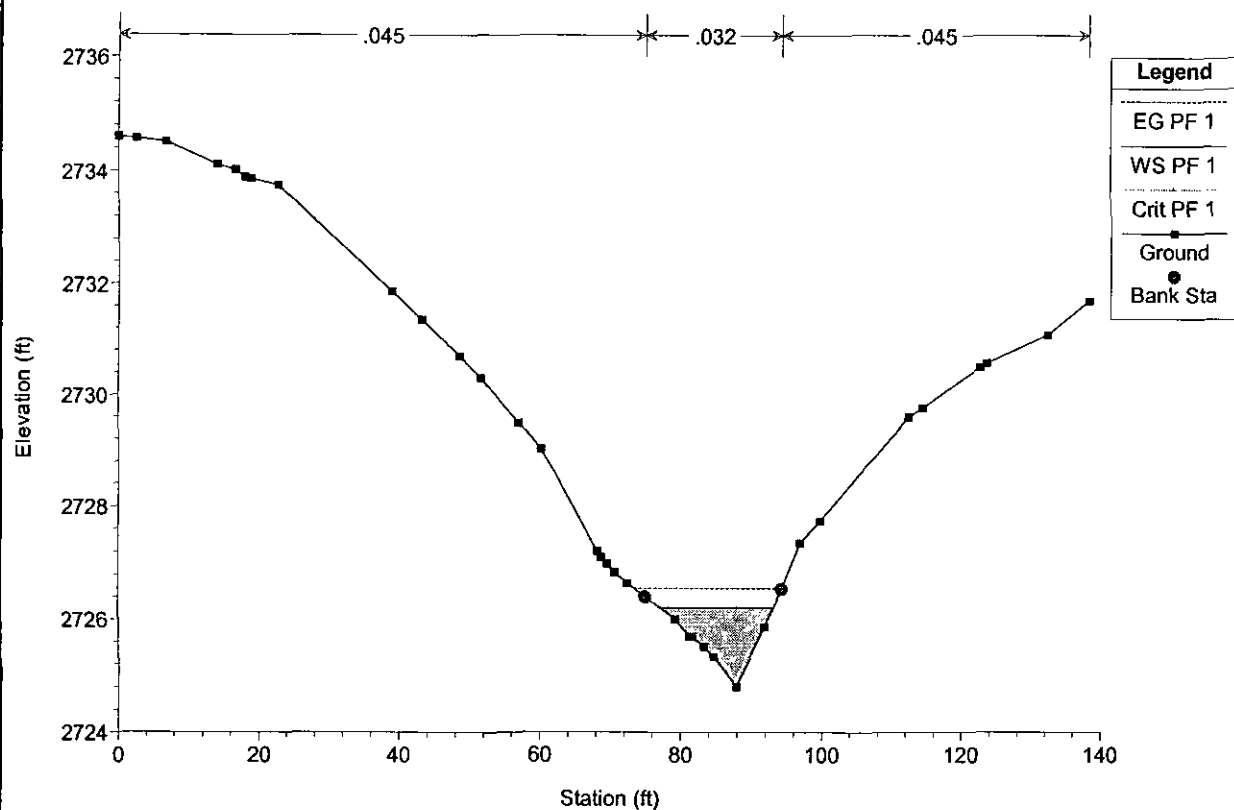


WASH E2

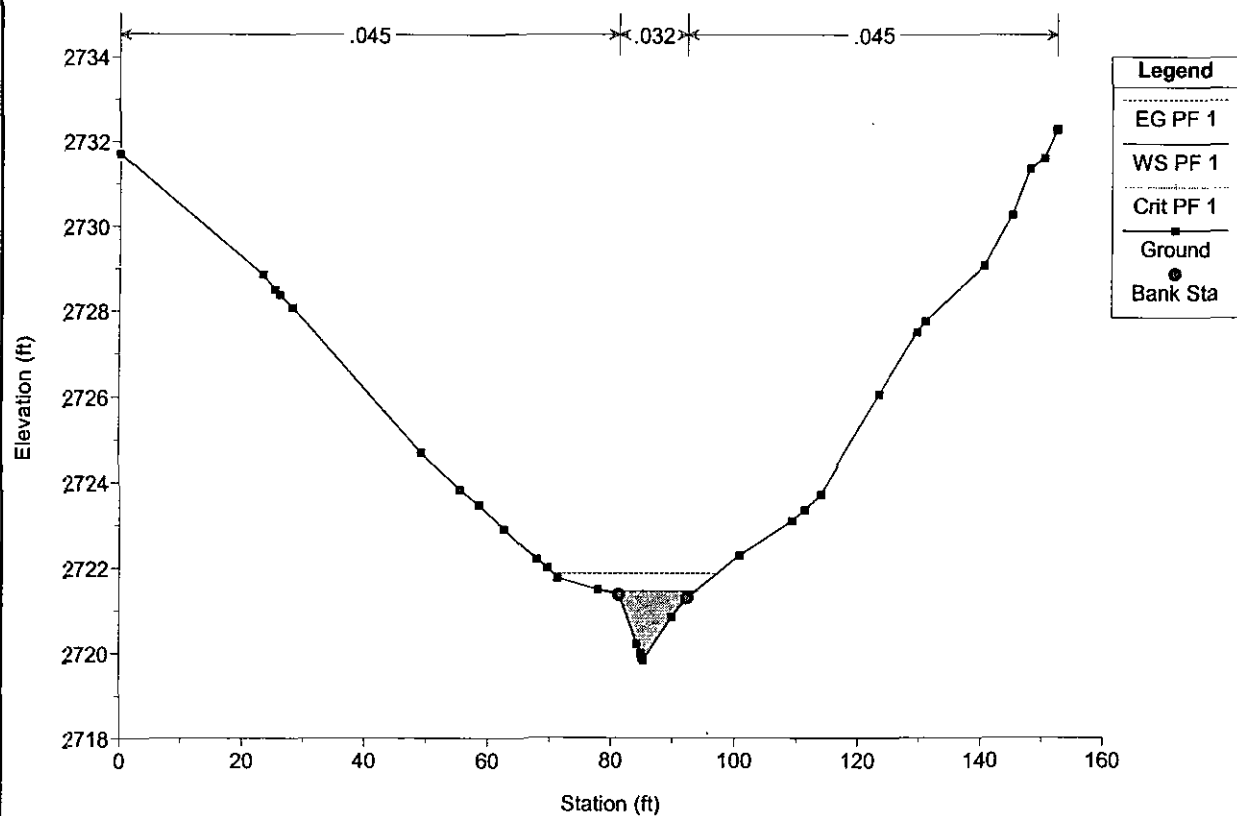
HEC-RAS Plan: Imported Pla River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch E (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	11	PF 1	50.00	2724.80	2726.21	2726.21	2726.55	0.018084	4.69	10.67	16.13	1.02
Reach-1	10	PF 1	50.00	2719.62	2721.45	2721.45	2721.67	0.015739	5.17	9.85	14.80	0.98
Reach-1	9	PF 1	50.00	2715.71	2717.28	2717.28	2717.71	0.013081	5.64	10.87	14.46	0.94
Reach-1	8	PF 1	75.00	2709.10	2711.09	2711.09	2711.55	0.010257	5.57	16.18	22.55	0.85
Reach-1	7.5	PF 1	75.00	2708.00	2709.19	2709.19	2709.62	0.013563	5.49	15.49	19.83	0.95
Reach-1	7	PF 1	75.00	2705.46	2706.94	2706.94	2707.36	0.015127	5.18	14.90	20.27	0.98
Reach-1	6	PF 1	75.00	2701.53	2703.58	2703.58	2704.11	0.014543	5.84	13.21	14.90	0.97
Reach-1	5	PF 1	75.00	2697.79	2699.88	2699.88	2700.43	0.010621	6.48	15.38	16.38	0.89
Reach-1	4.5	PF 1	75.00	2697.00	2698.52	2698.52	2699.00	0.012220	5.77	14.89	17.87	0.92
Reach-1	4	PF 1	75.00	2694.91	2696.38	2696.38	2696.78	0.016000	5.09	14.92	21.12	0.99
Reach-1	3	PF 1	131.00	2687.45	2689.81	2689.81	2690.44	0.012785	6.41	21.80	21.14	0.95
Reach-1	2	PF 1	275.00	2679.83	2682.39	2682.39	2683.11	0.009982	7.12	46.76	38.39	0.90
Reach-1	1	PF 1	275.00	2677.00	2679.55	2679.55	2680.11	0.016423	7.02	51.48	41.70	0.83

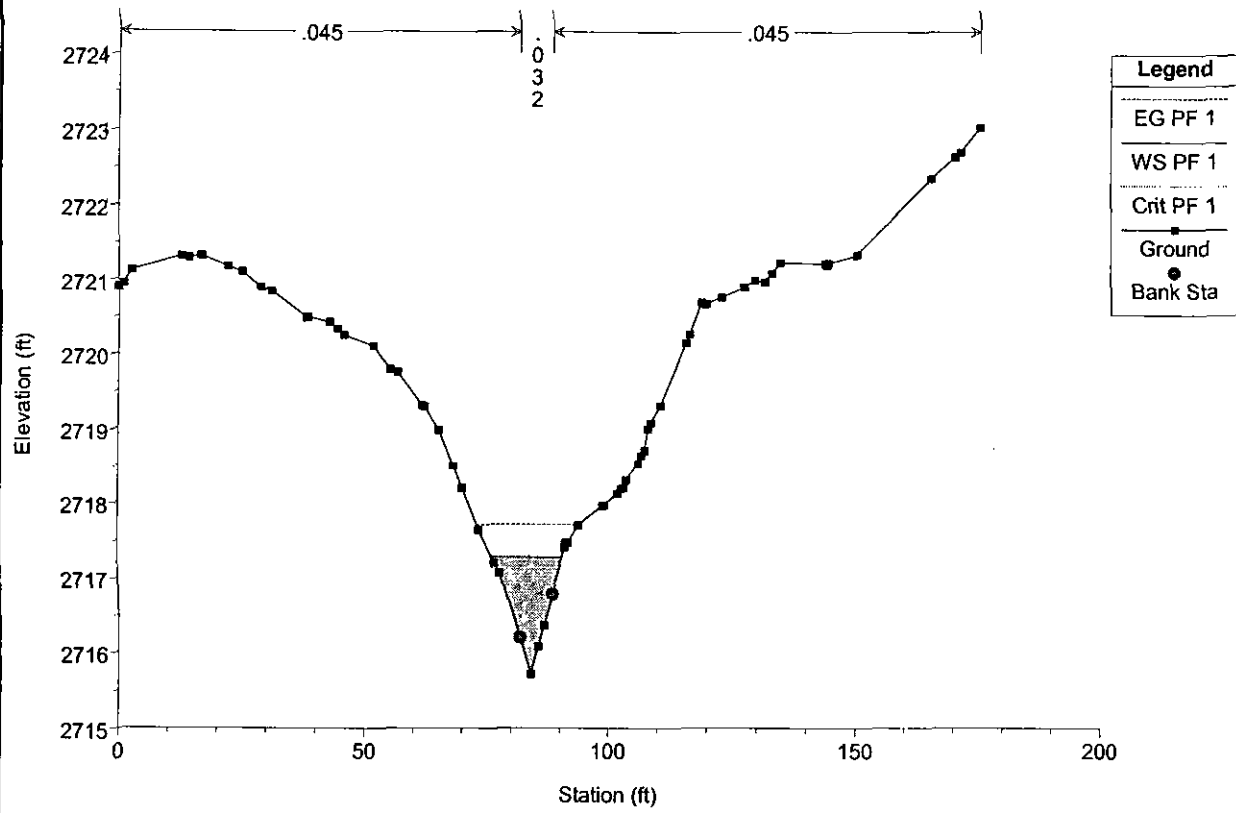
washe2 Plan: Imported Plan 02 11/7/2006



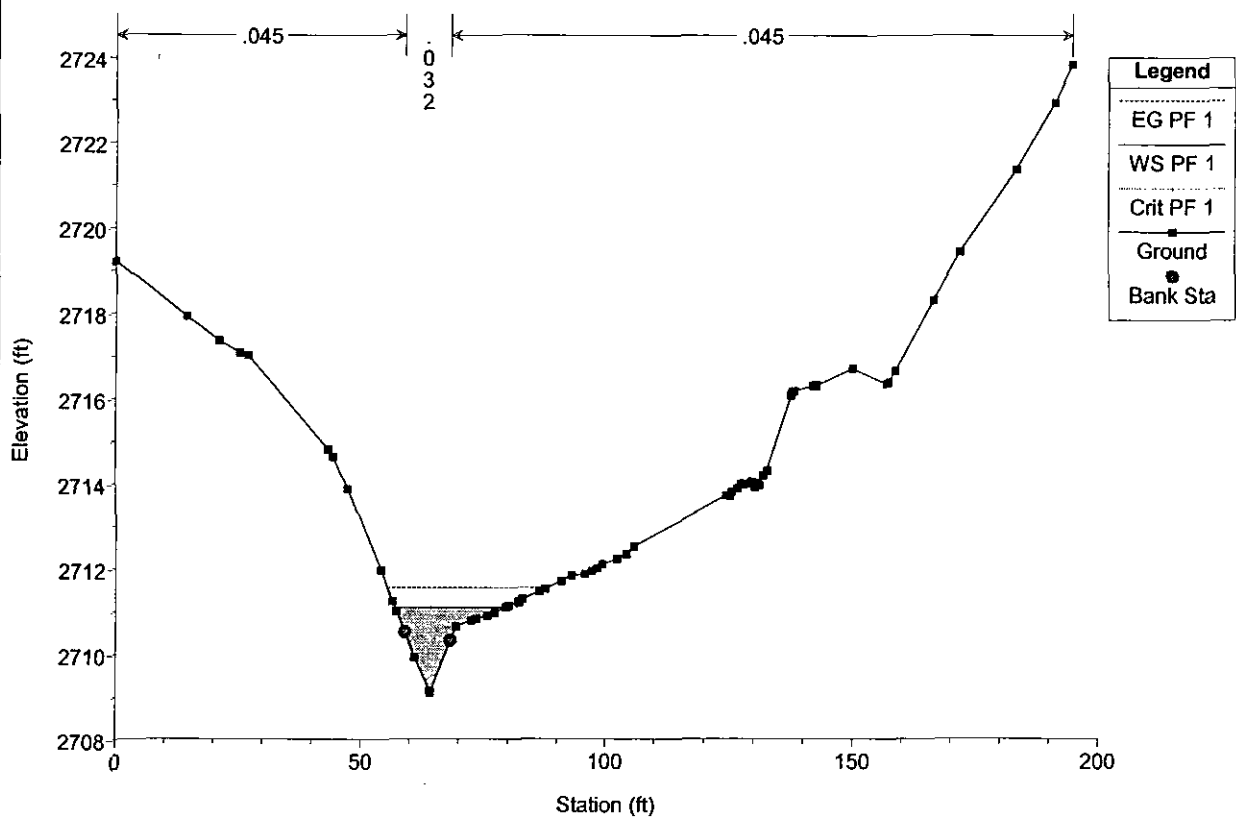
washe2 Plan: Imported Plan 02 11/7/2006



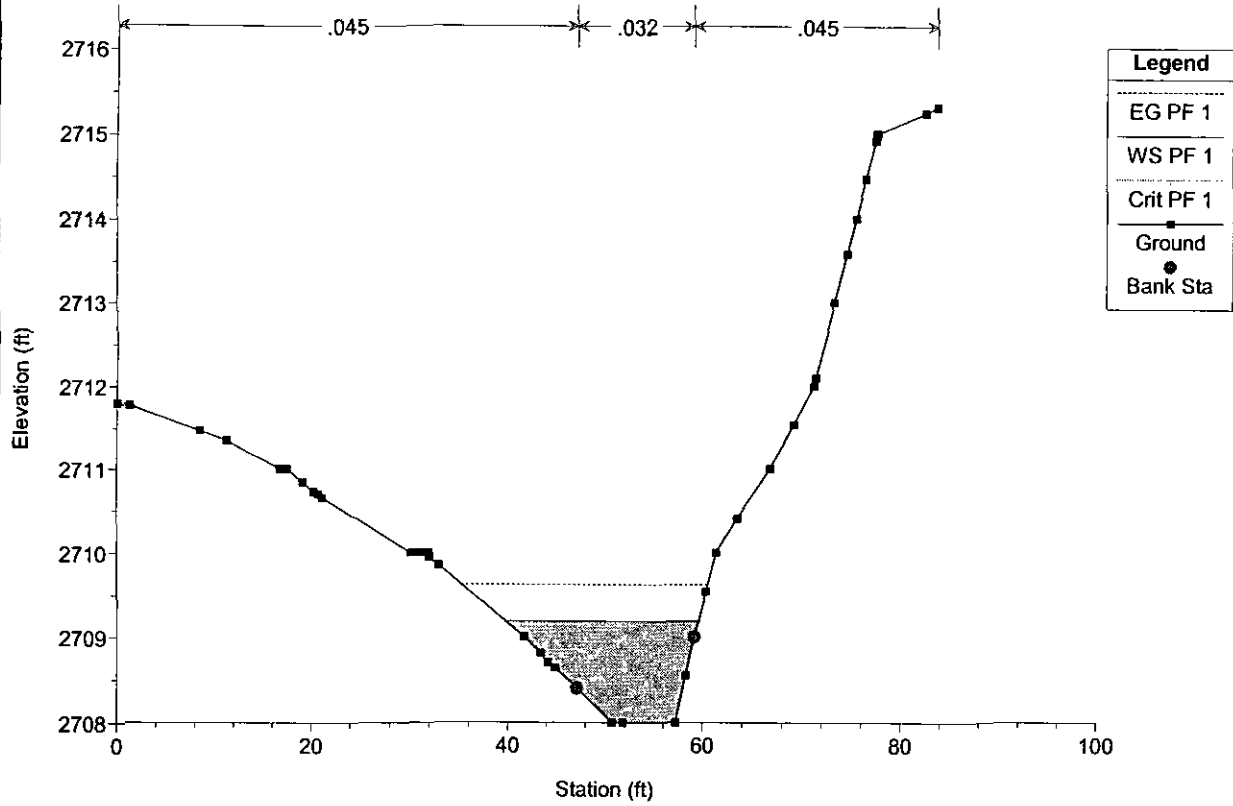
washe2 Plan: Imported Plan 02 11/7/2006



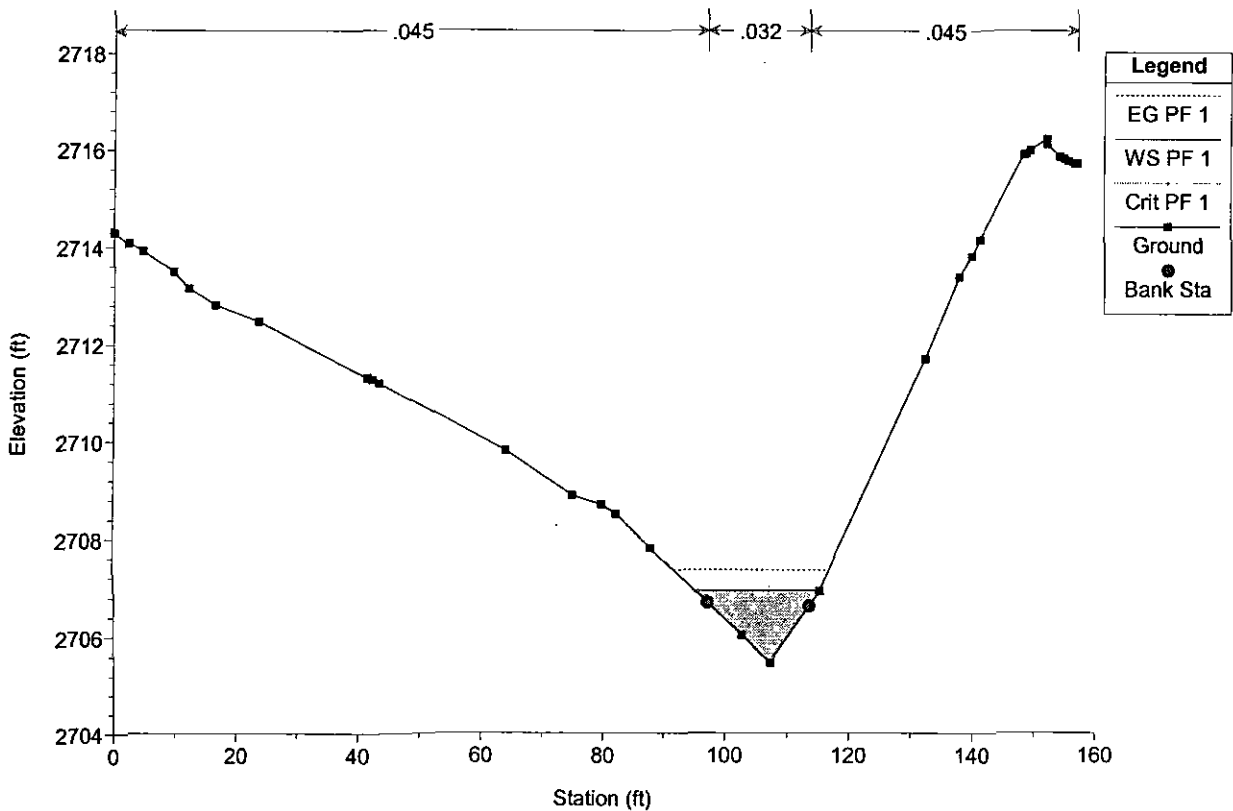
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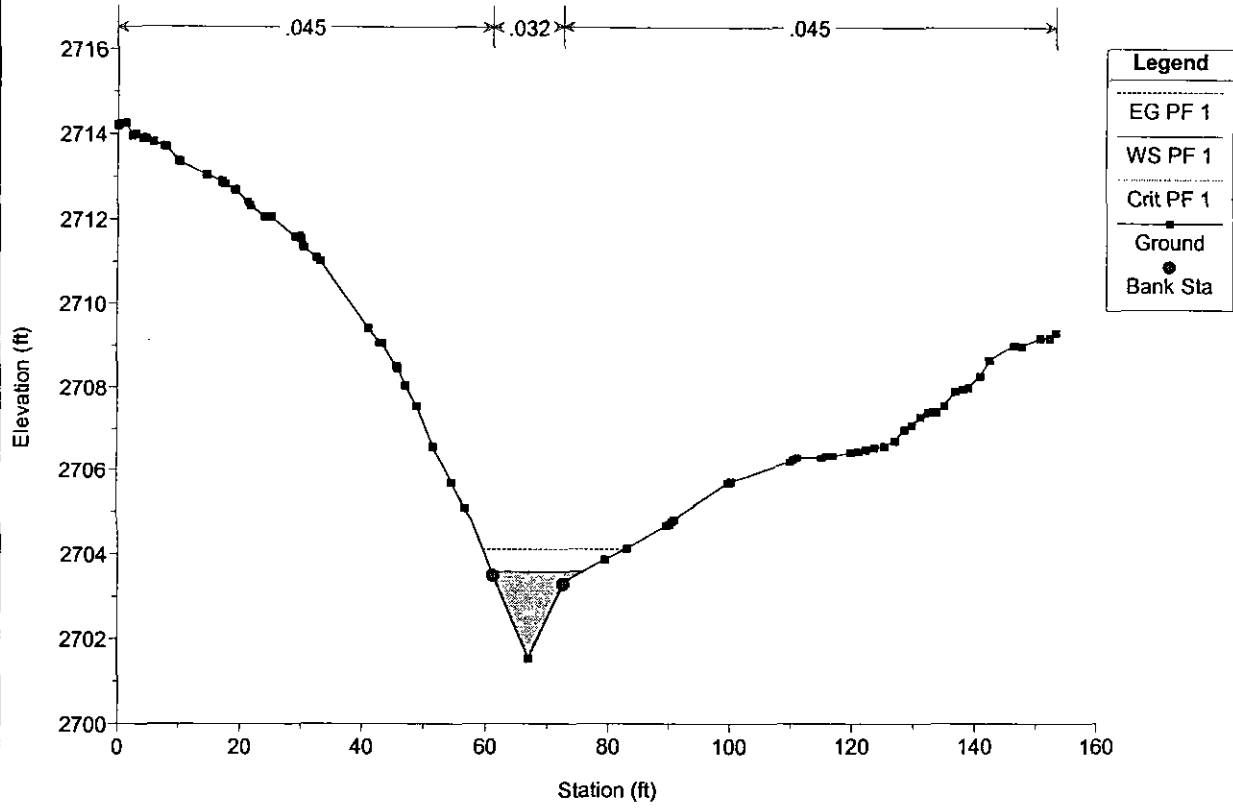
washe2 Plan: Imported Plan 02 11/7/2006



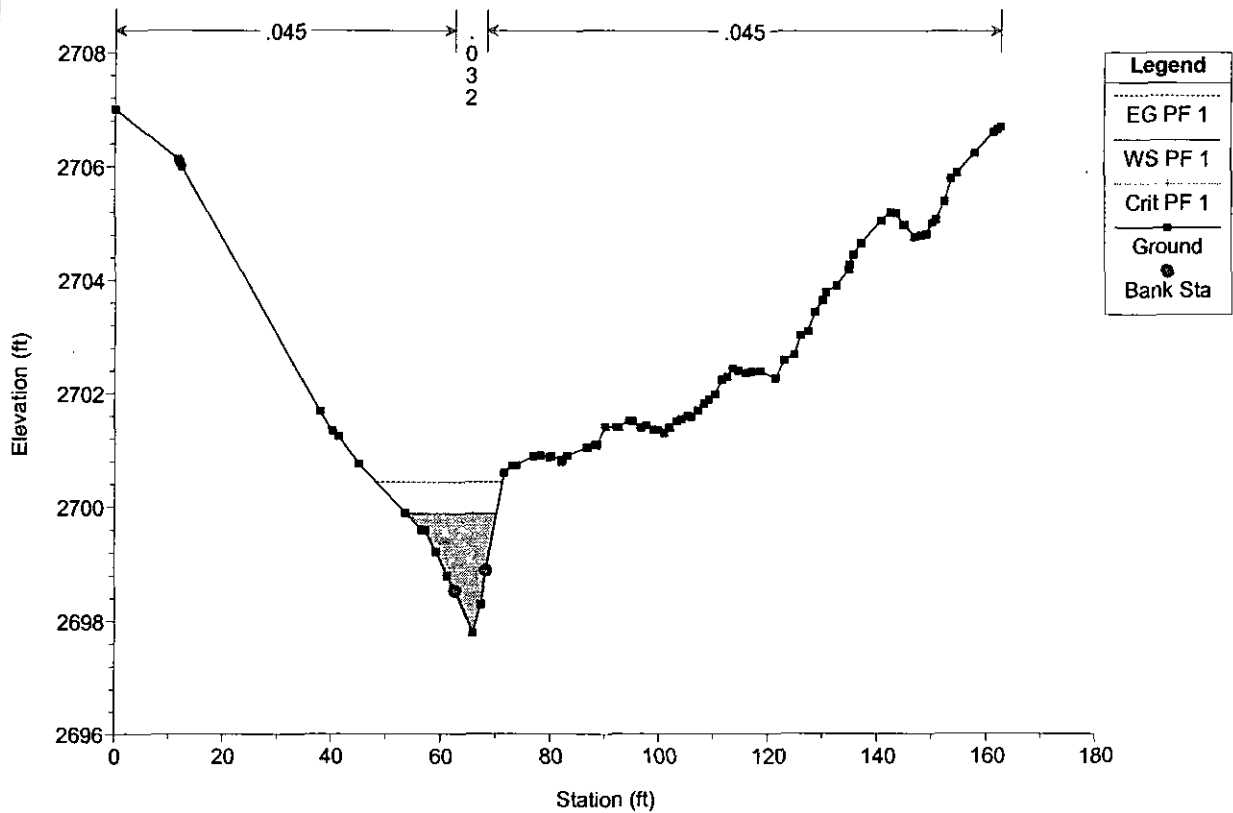
washe2 Plan: Imported Plan 02 11/7/2006



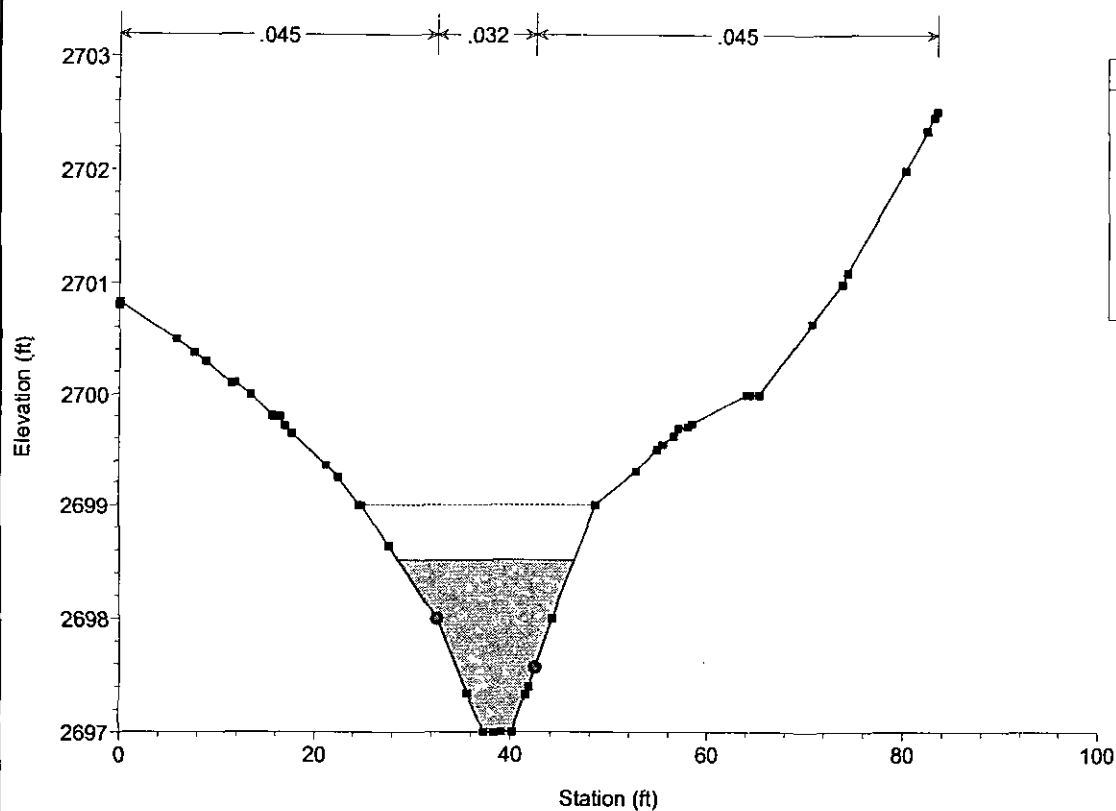
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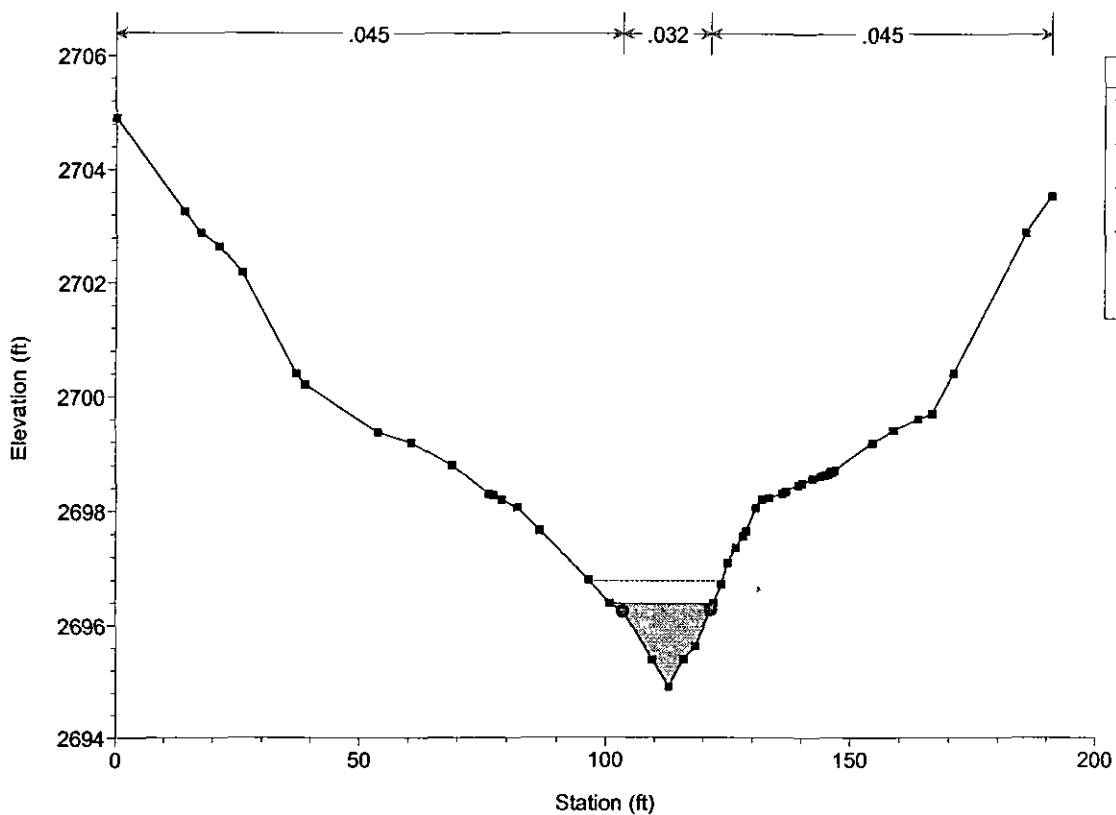
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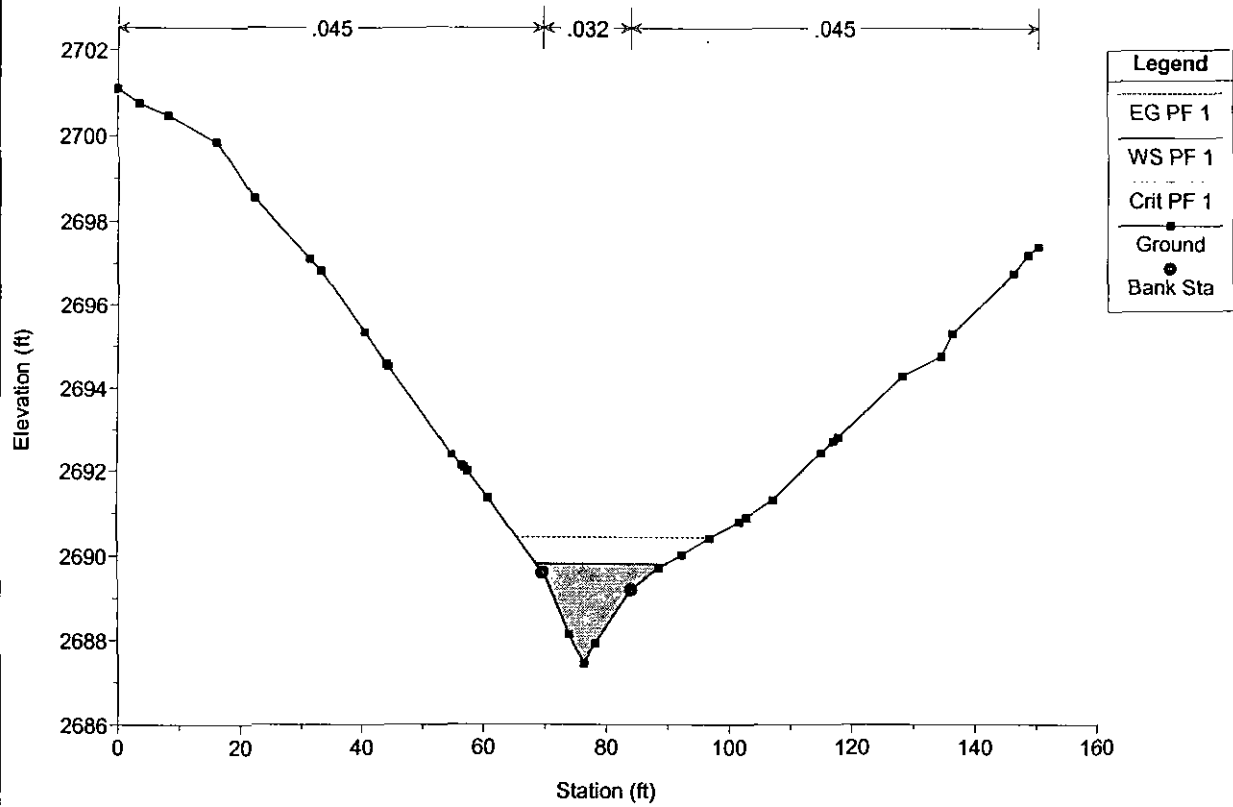
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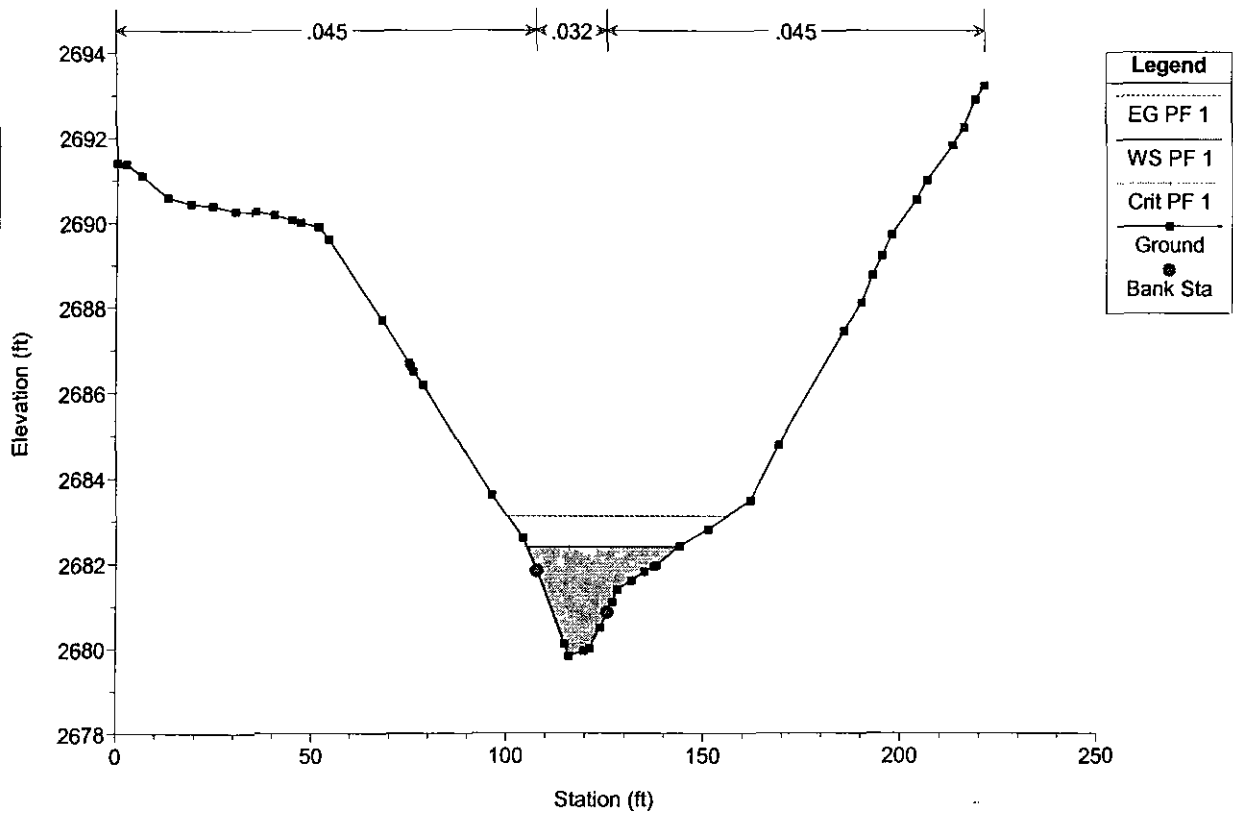
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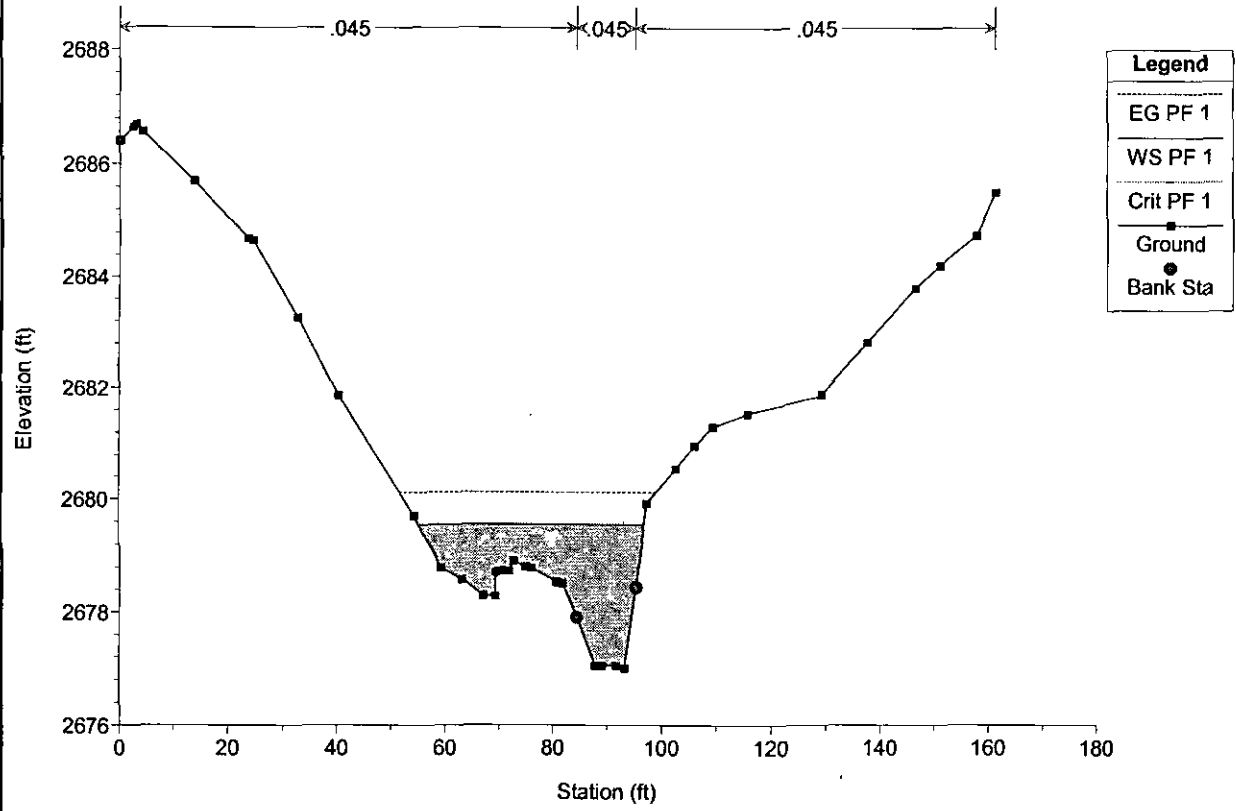
washe2 Plan: Imported Plan 02 11/7/2006



washe2 Plan: Imported Plan 02 11/7/2006



washe2 Plan: Imported Plan 02 11/7/2006

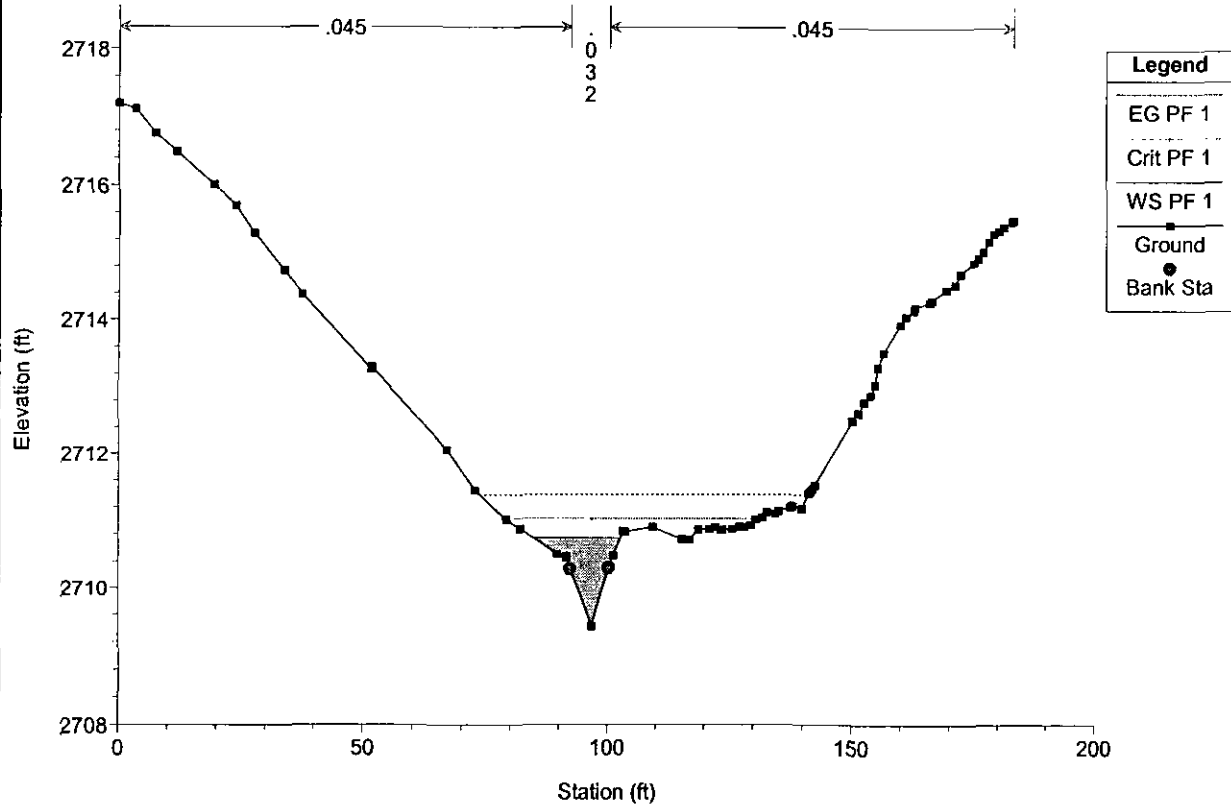


WASH F1

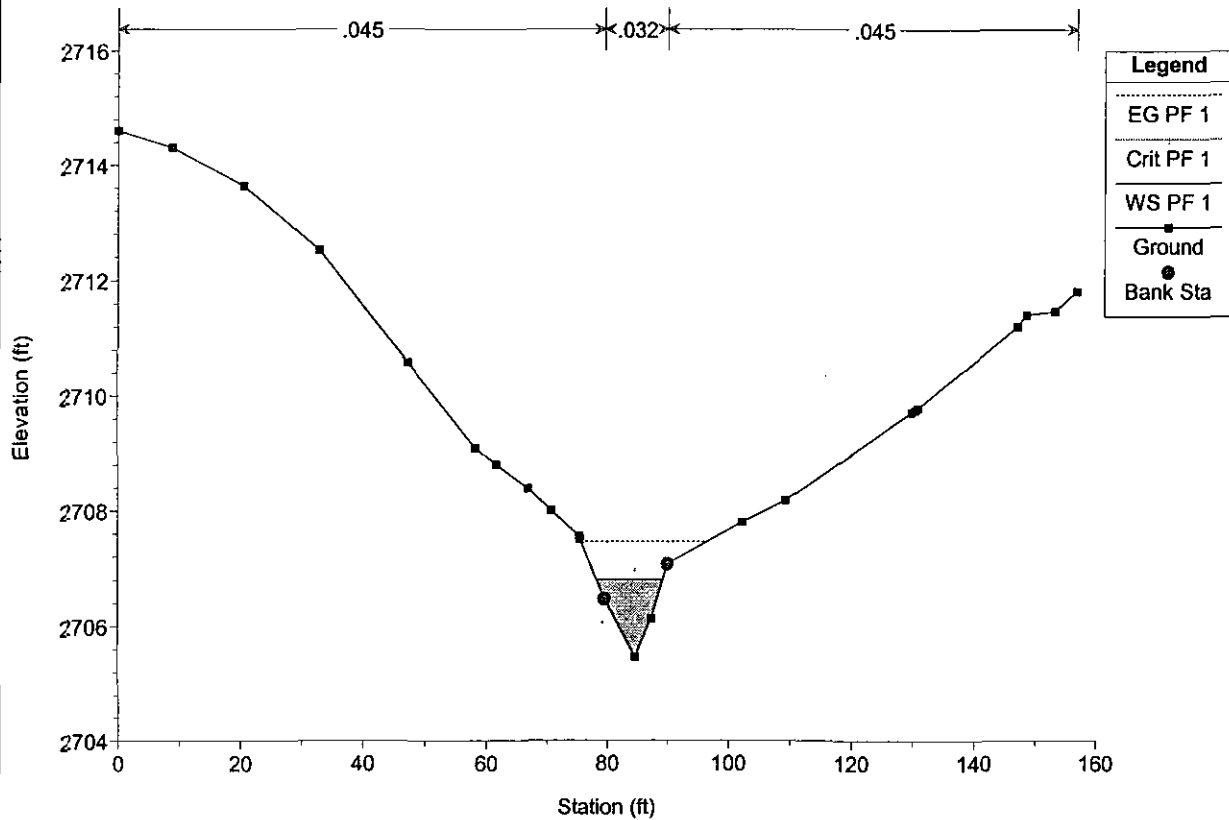
HEC-RAS Plan: Plan 01 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	8	PF 1	50.00	2709.42	2710.74	2711.03	2711.38	0.025021	6.65	8.95	20.46	1.25
Reach-1	7	PF 1	50.00	2705.46	2706.81	2706.97	2707.46	0.027912	6.49	7.87	11.03	1.29
Reach-1	6	PF 1	50.00	2700.54	2701.76	2702.06	2702.68	0.039439	7.86	7.10	12.34	1.54
Reach-1	5	PF 1	50.00	2698.48	2699.81	2699.89	2700.28	0.022728	5.49	9.24	14.33	1.15
Reach-1	4.5	PF 1	50.00	2697.00	2697.81	2697.96	2698.38	0.028656	6.06	8.42	13.96	1.29
Reach-1	4	PF 1	76.00	2692.65	2693.93	2694.09	2694.58	0.029509	6.47	11.91	17.84	1.33
Reach-1	3.5	PF 1	76.00	2689.00	2690.62	2690.62	2691.05	0.011337	5.57	16.92	22.78	0.89
Reach-1	3	PF 1	76.00	2686.57	2687.81	2688.10	2688.70	0.034574	7.61	10.42	15.66	1.46
Reach-1	2	PF 1	76.00	2681.90	2683.33	2683.35	2683.65	0.017537	4.63	18.17	36.25	1.00
Reach-1	1	PF 1	99.00	2677.60	2678.80	2679.01	2679.50	0.034655	7.02	18.30	54.08	1.44

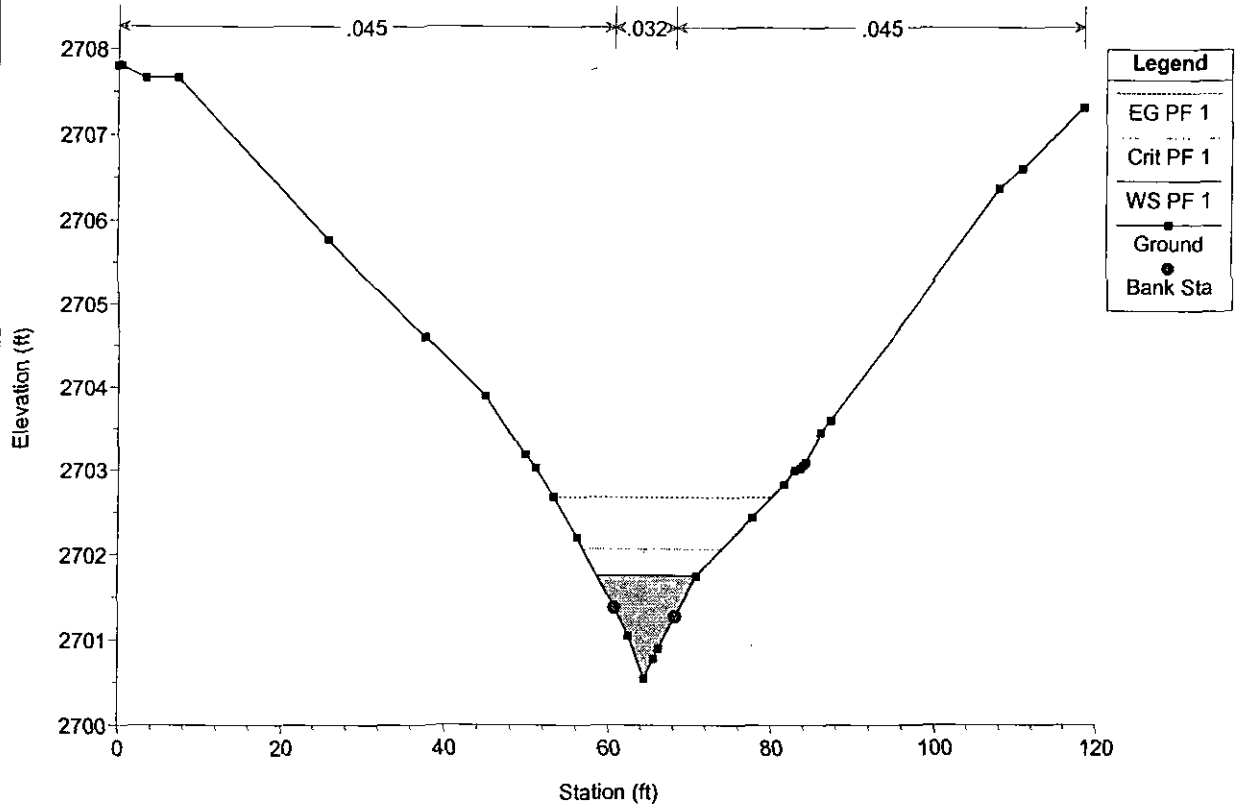
washf1 Plan: Plan 01 11/7/2006



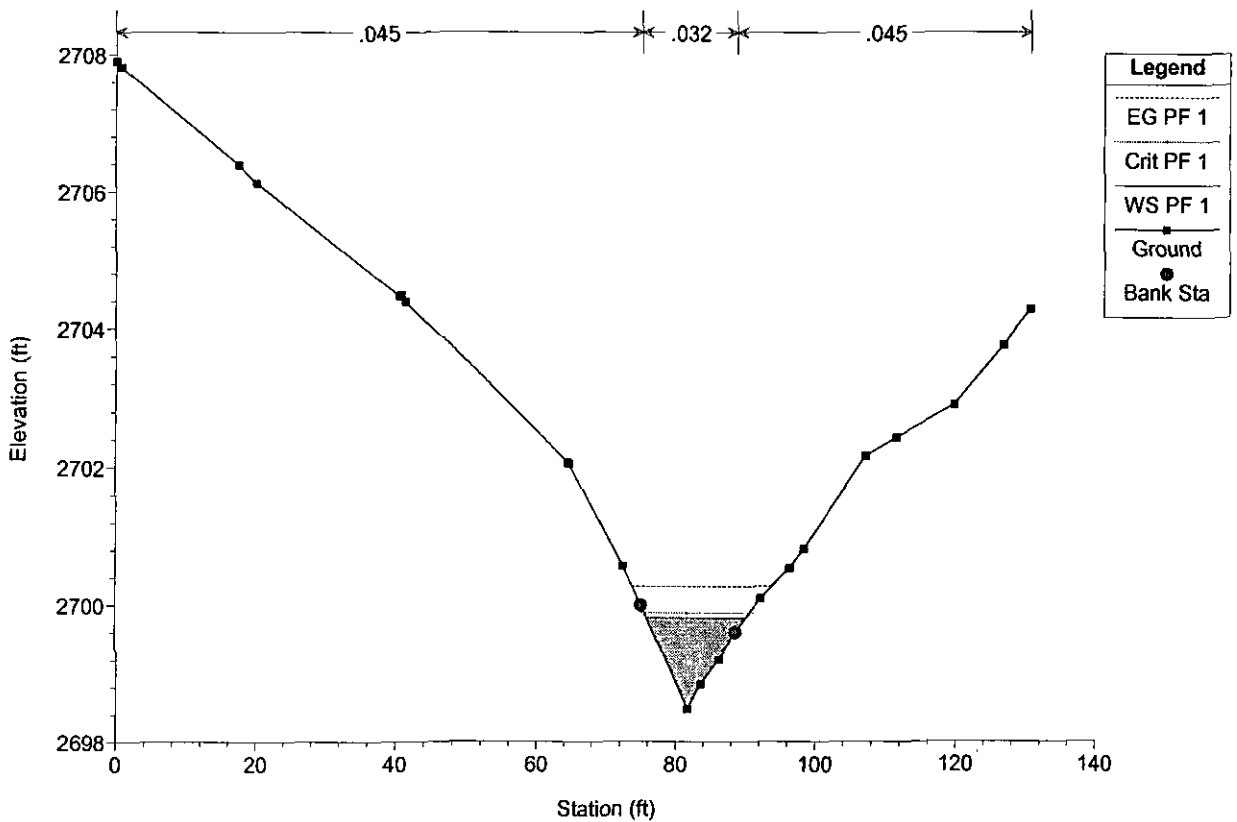
washf1 Plan: Plan 01 11/7/2006



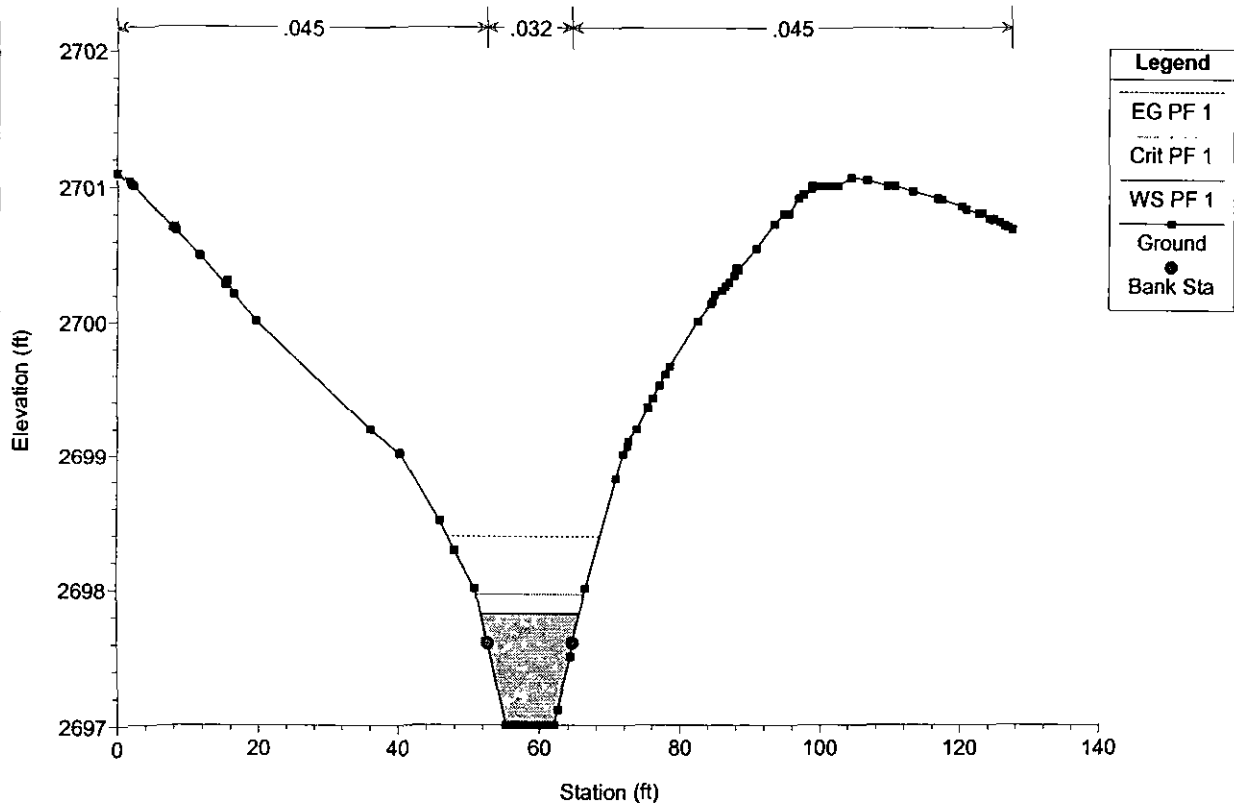
washf1 Plan: Plan 01 11/7/2006



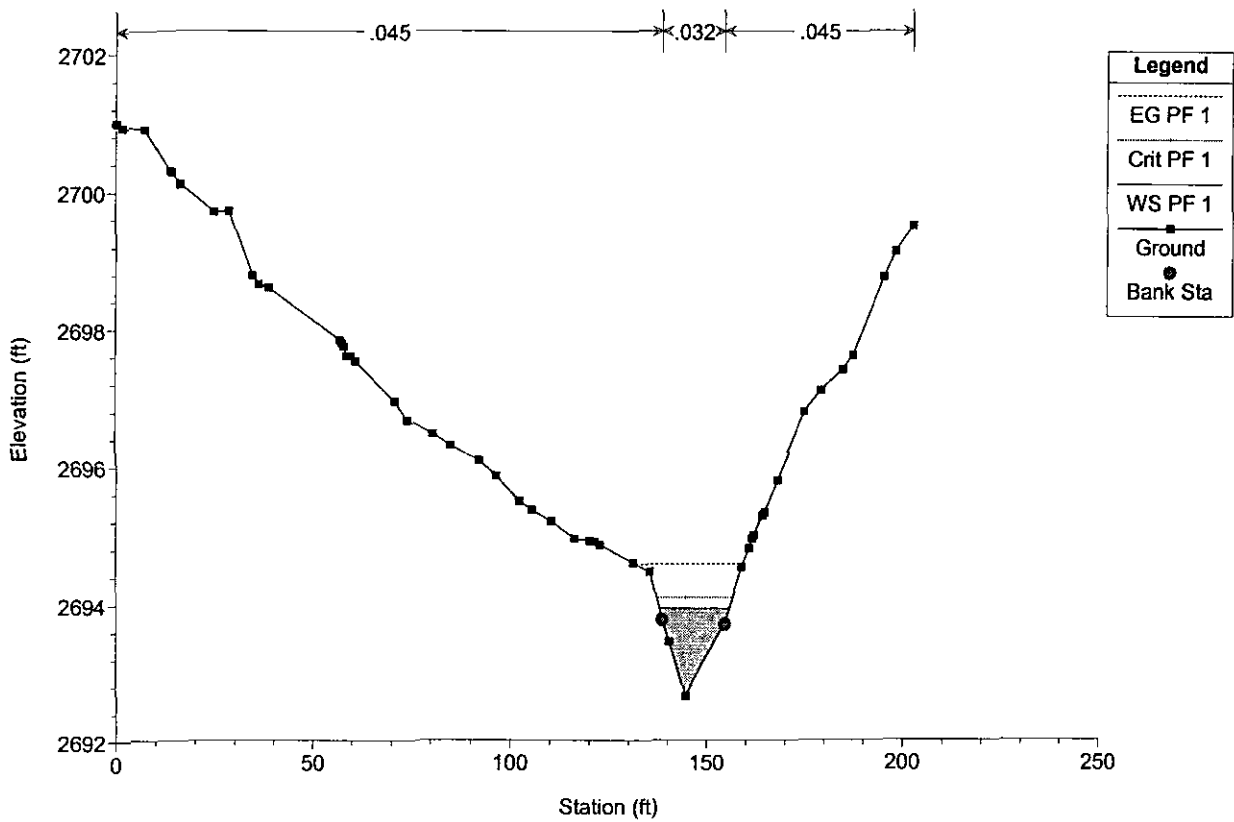
washf1 Plan: Plan 01 11/7/2006



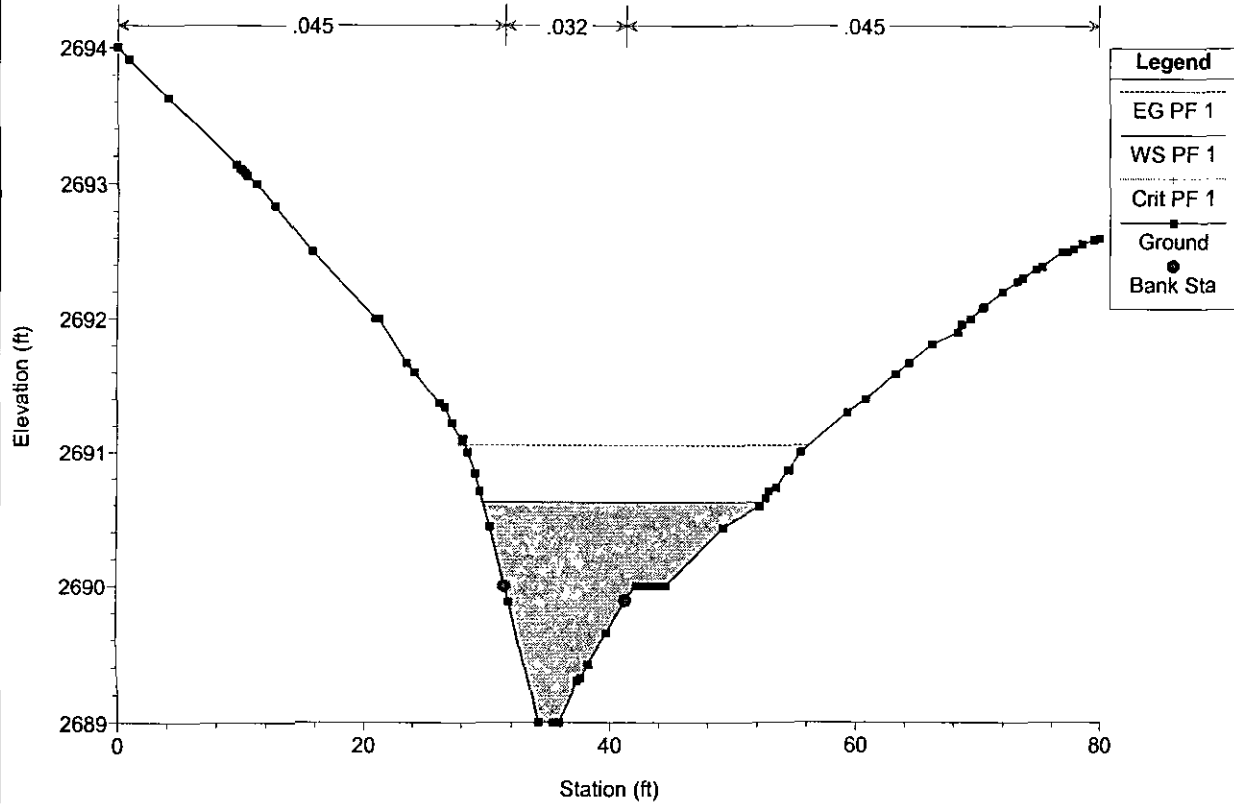
washf1 Plan: Plan 01 11/7/2006



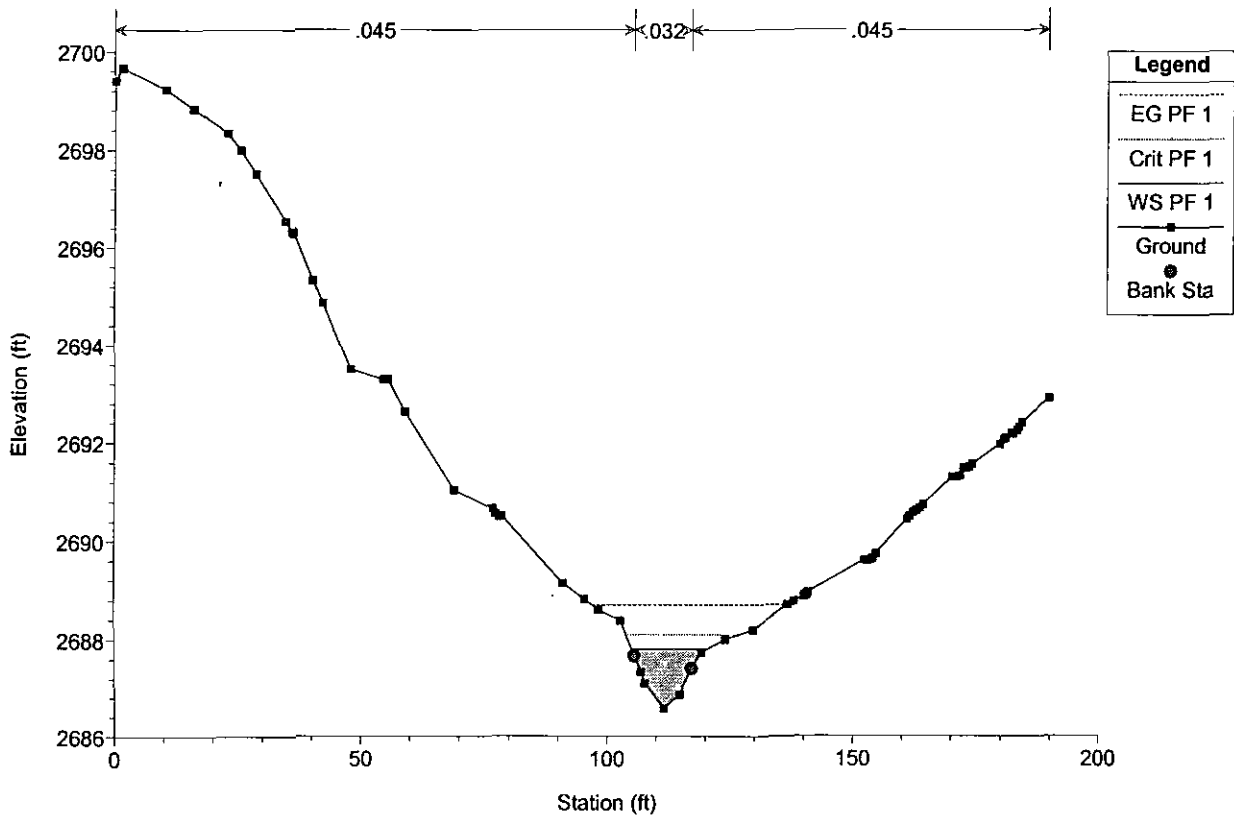
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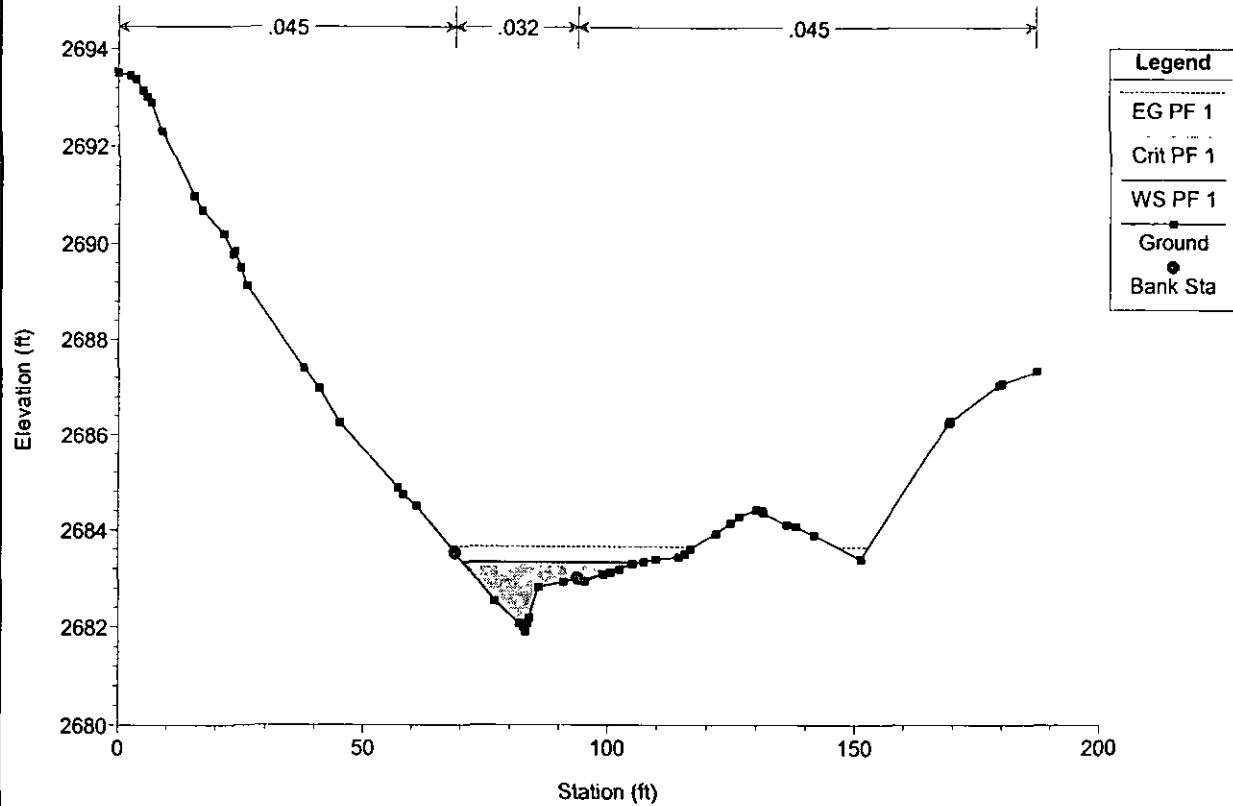
washf1 Plan: Plan 01 11/7/2006



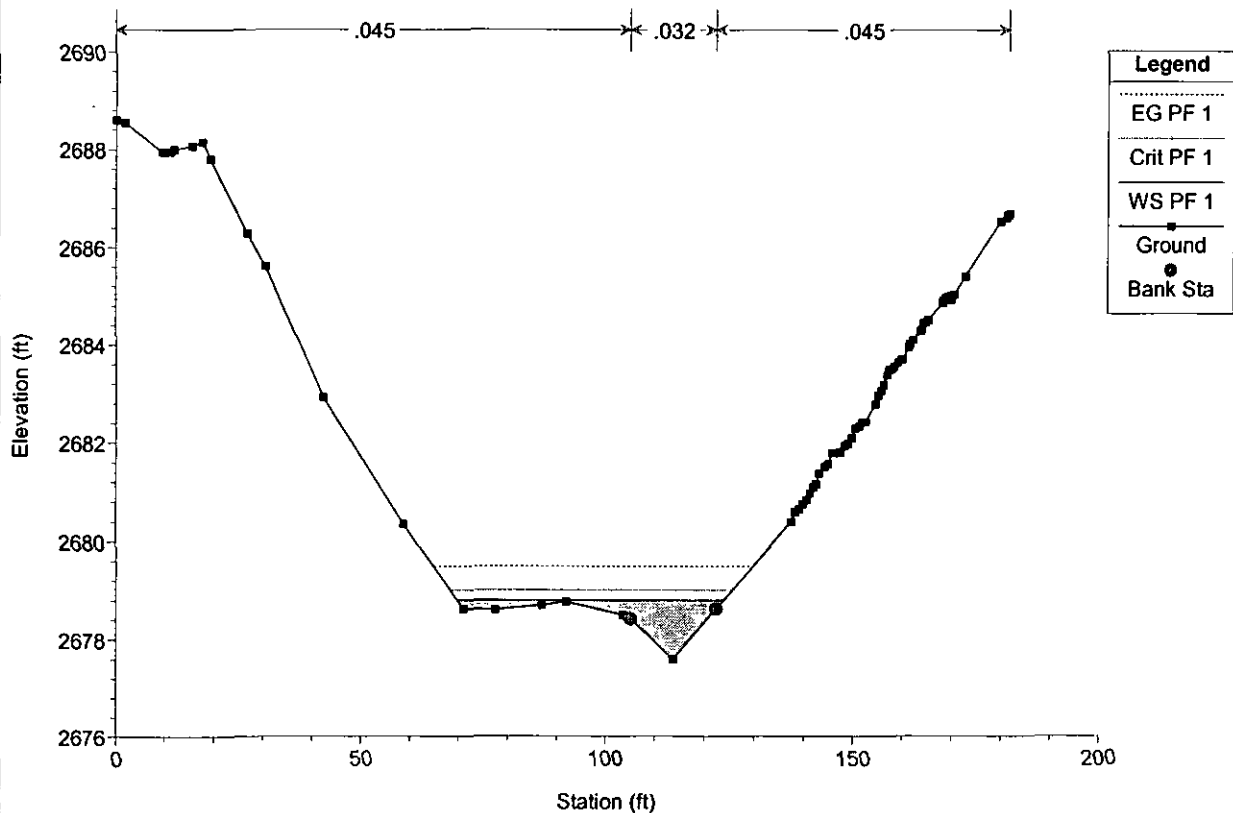
washf1 Plan: Plan 01 11/7/2006



washf1 Plan: Plan 01 11/7/2006



washf1 Plan: Plan 01 11/7/2006

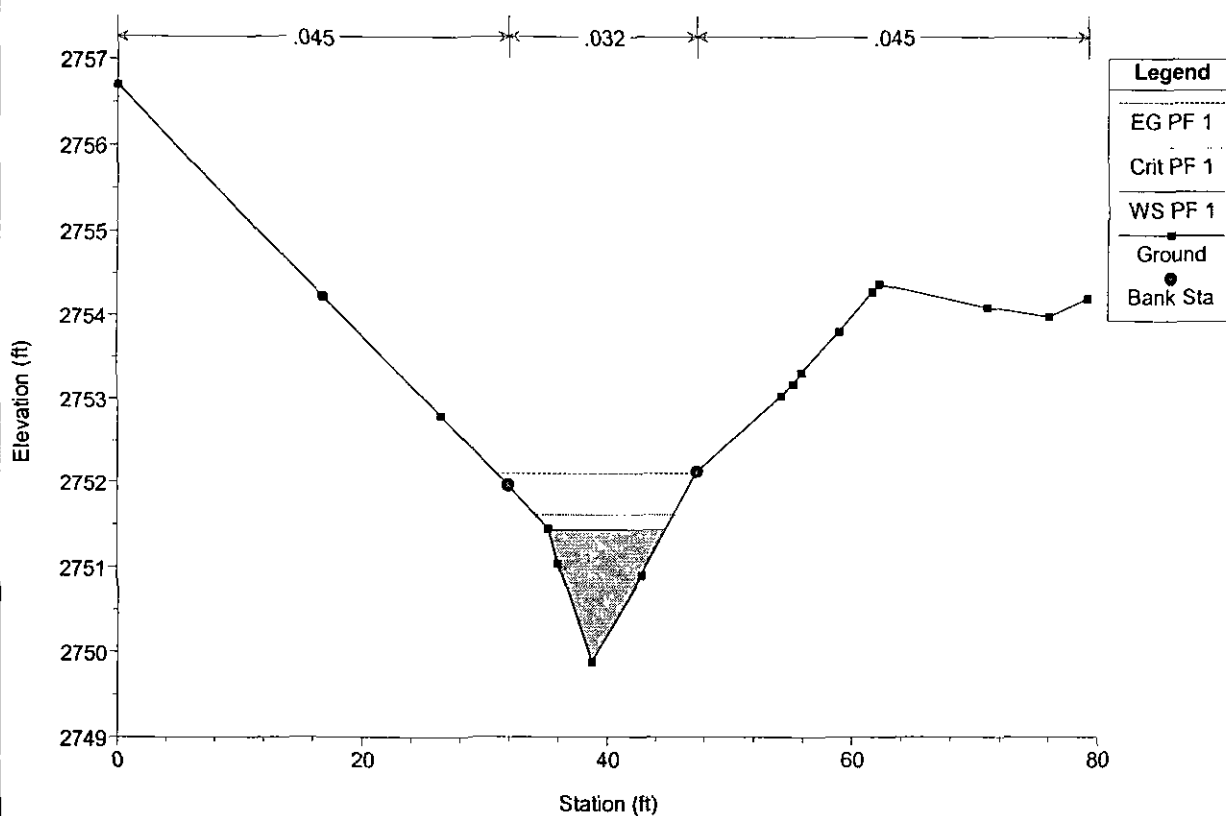


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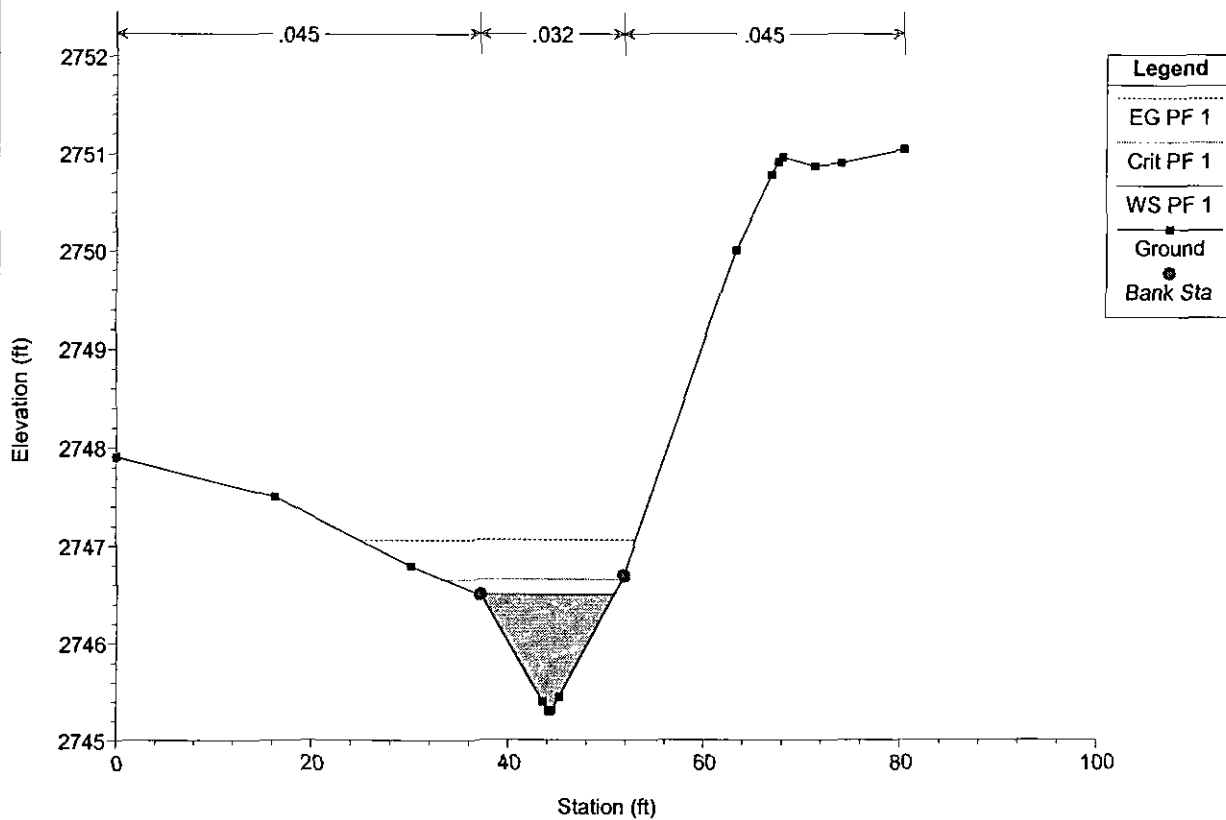
HEC-RAS Plan: Plan 02 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta.	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	61	PF 1	50.00	2749.88	2751.43	2751.61	2752.10	0.028975	6.55	7.63	9.59	1.29
Reach-1	5	PF 1	50.00	2745.29	2746.49	2746.65	2747.06	0.033087	6.02	8.30	13.57	1.36
Reach-1	4.5	PF 1	50.00	2741.00	2741.71	2741.89	2742.19	0.040197	5.61	9.42	27.11	1.44
Reach-1	4	PF 1	110.00	2737.70	2739.10	2739.19	2739.51	0.028653	5.14	21.65	43.74	1.24
Reach-1	3	PF 1	110.00	2732.24	2733.50	2733.77	2734.27	0.032757	7.19	17.98	48.55	1.42
Reach-1	2	PF 1	152.00	2725.11	2726.80	2726.97	2727.47	0.035951	6.57	23.25	37.04	1.44
Reach-1	1.5	PF 1	152.00	2723.00	2724.16	2724.25	2724.68	0.017807	5.94	28.69	41.89	1.08
Reach-1	1	PF 1	152.00	2720.27	2721.00	2720.98	2721.22	0.028998	4.37	40.92	87.59	1.20

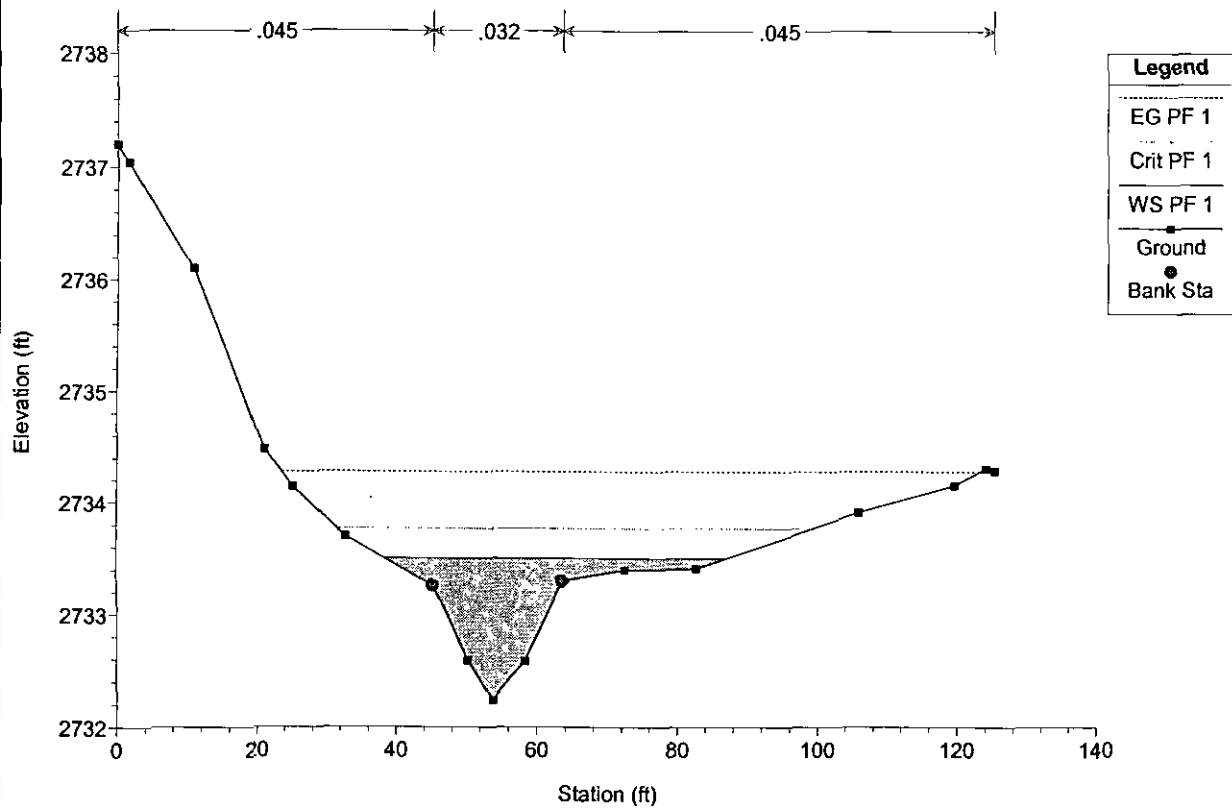
washh1 Plan: Plan 02 11/7/2006



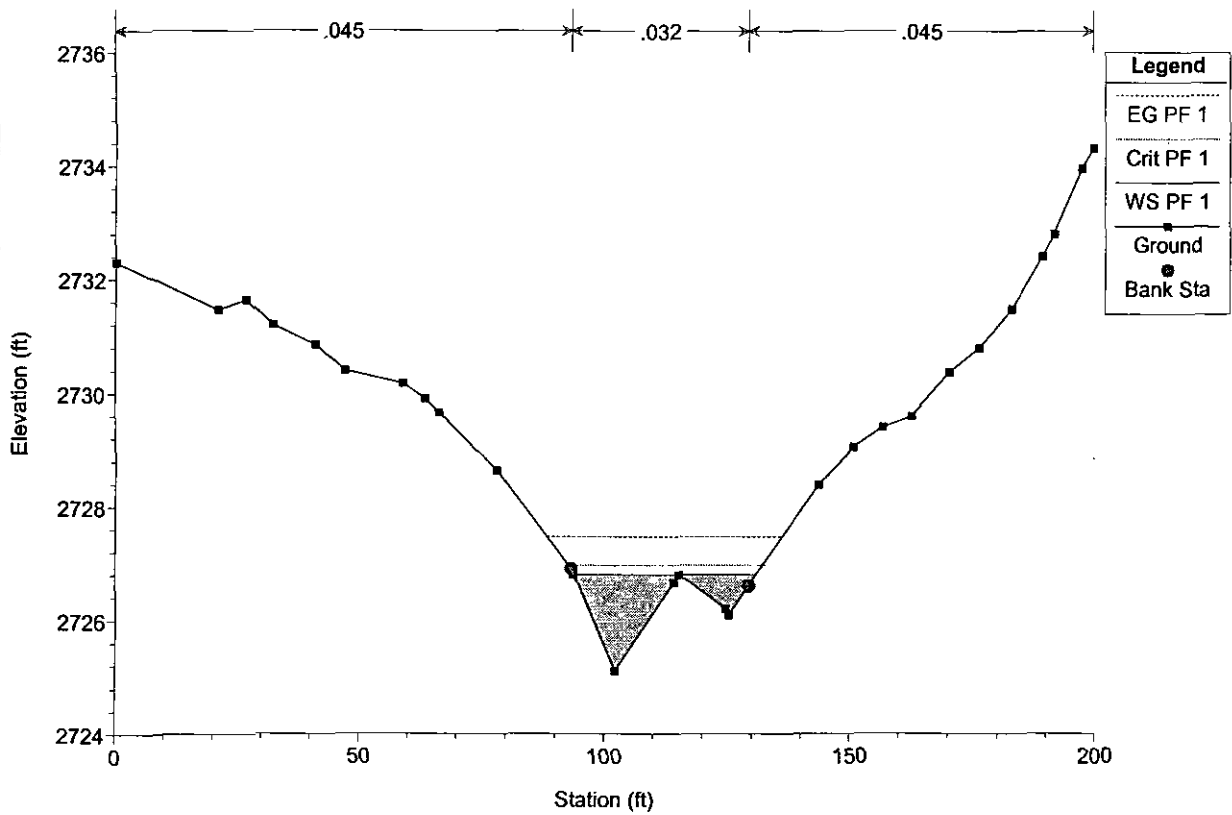
washh1 Plan: Plan 02 11/7/2006



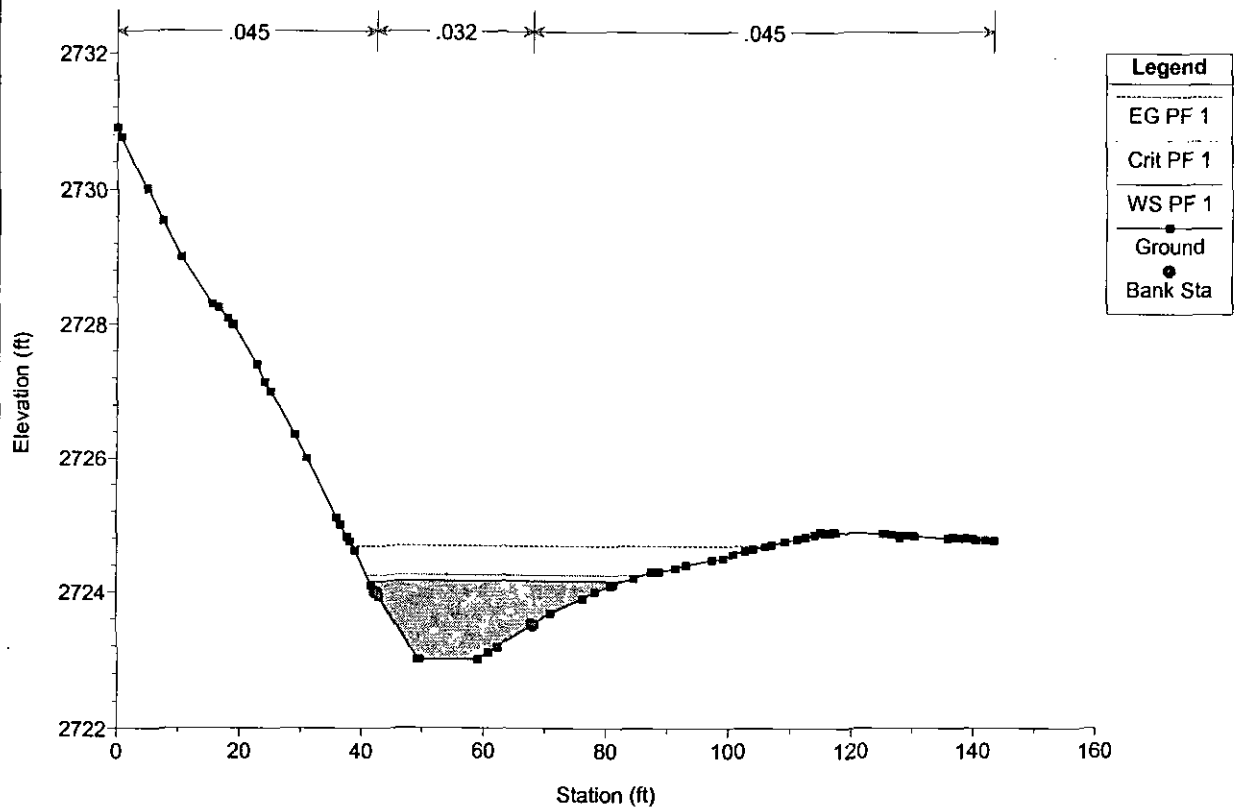
washh1 Plan: Plan 02 11/7/2006



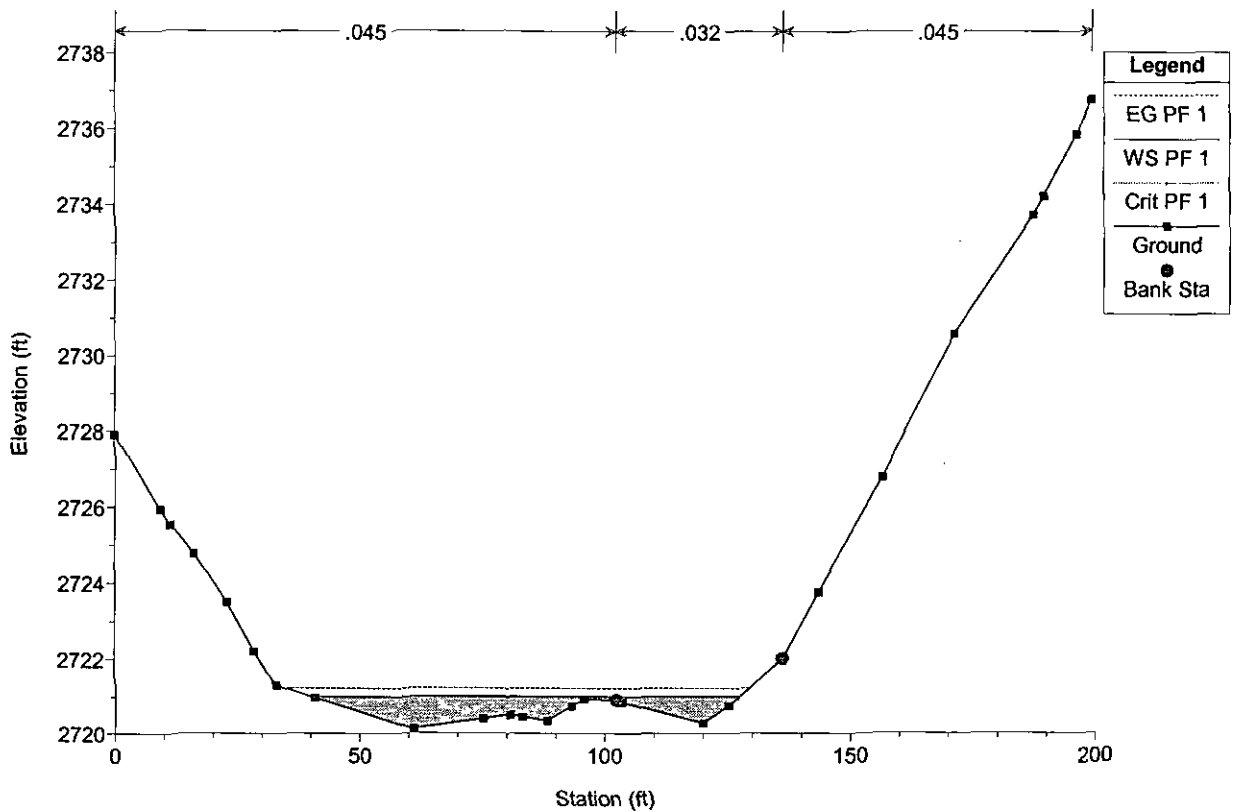
washh1 Plan: Plan 02 11/7/2006



washh1 Plan: Plan 02 11/7/2006



washh1 Plan: Plan 02 11/7/2006

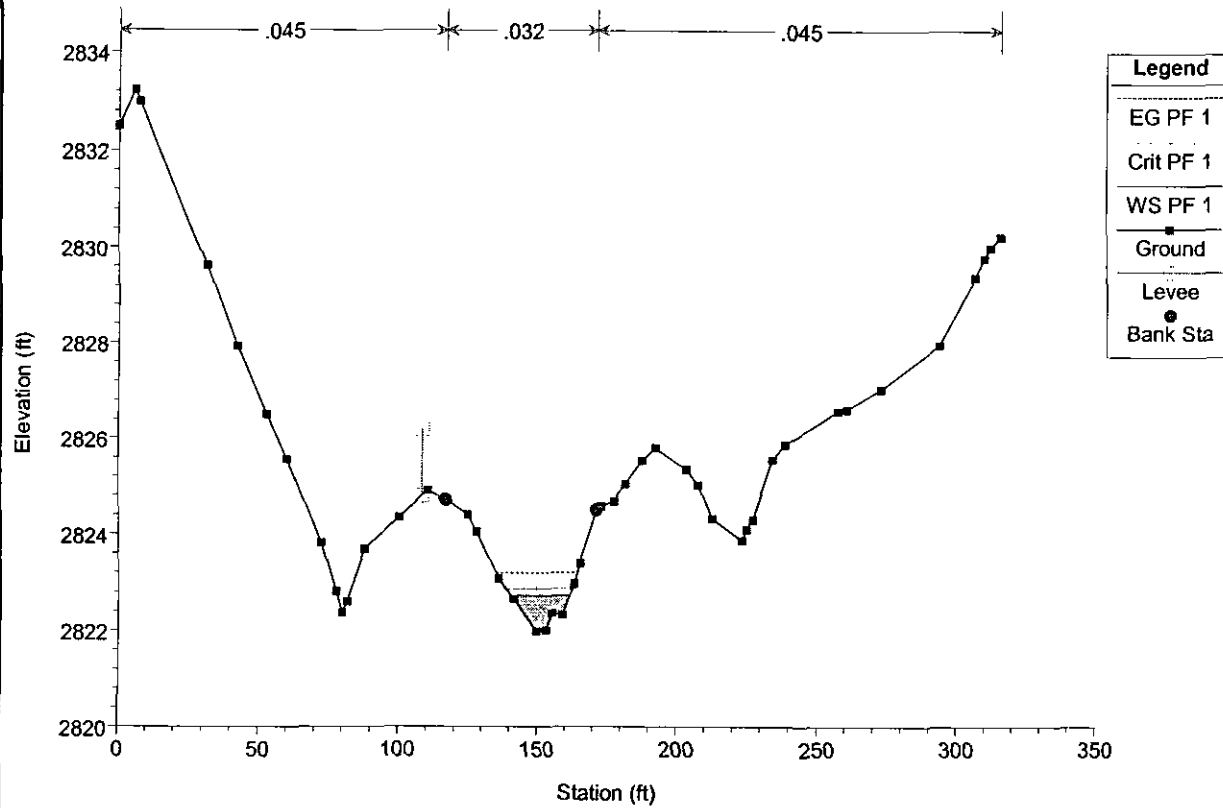


WASH H2

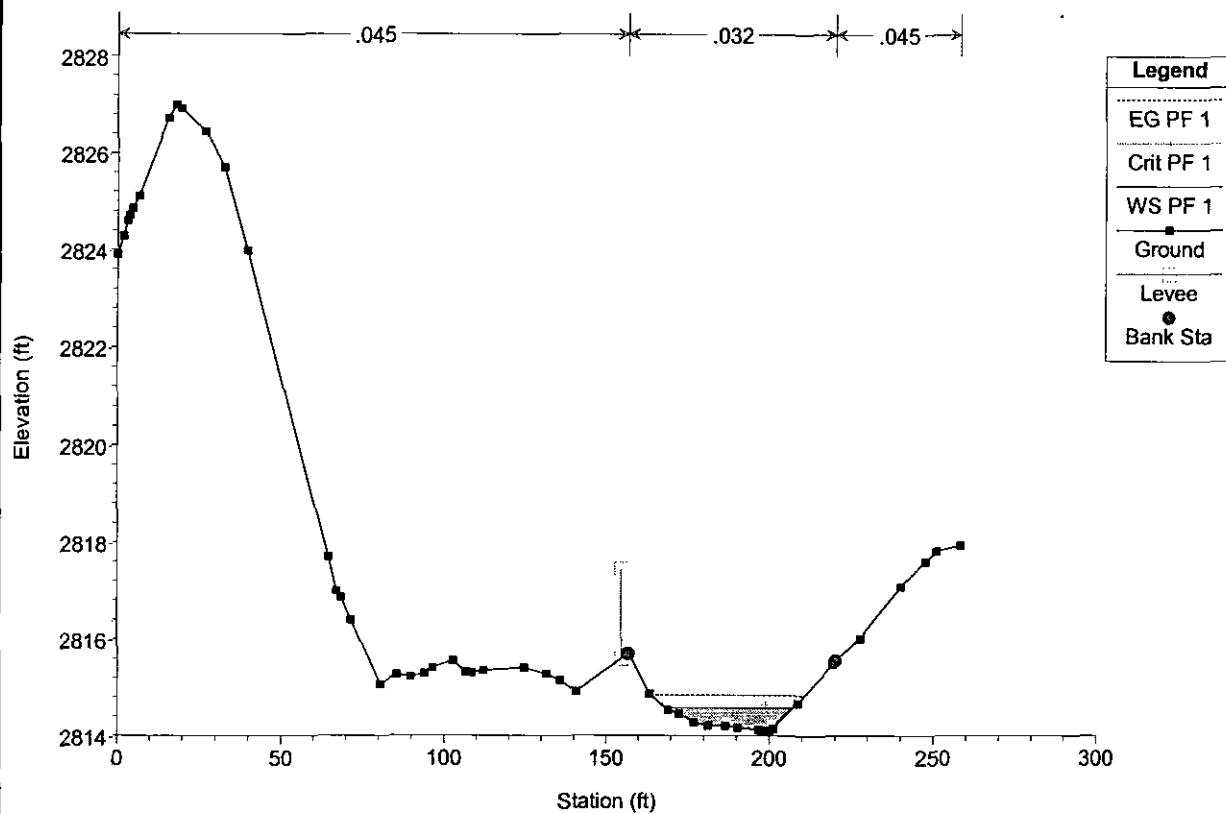
HEC-RAS Plan: Plan 02 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	15	PF-1	50.00	2821.98	2822.73	2822.87	2823.20	0.043010	5.50	9.10	21.01	1.47
Reach-1	14	PF-1	50.00	2814.10	2814.57	2814.63	2814.83	0.038439	4.16	12.02	38.88	1.32
Reach-1	13	PF-1	50.00	2808.80	2809.31	2809.36	2809.54	0.036980	3.82	13.10	46.83	1.27
Reach-1	12	PF-1	50.00	2800.73	2801.22	2801.30	2801.52	0.063174	4.36	11.47	50.18	1.61
Reach-1	11.5	PF-1	50.00	2795.20	2795.75	2795.81	2796.03	0.026258	4.34	12.81	37.70	1.16
Reach-1	11	PF-1	110.00	2783.25	2784.72	2785.16	2786.12	0.060907	9.51	11.56	14.98	1.91
Reach-1	10	PF-1	110.00	2773.90	2775.20	2775.38	2775.77	0.031318	6.28	19.82	40.55	1.35
Reach-1	9	PF-1	110.00	2764.93	2766.03	2766.36	2767.06	0.053740	8.14	13.52	20.43	1.76
Reach-1	8.5	PF-1	110.00	2761.97	2762.95	2762.99	2763.16	0.009485	4.31	45.60	141.04	0.79
Reach-1	8	PF-1	110.00	2757.40	2757.99	2758.36	2760.10	0.470942	11.67	9.42	42.33	4.36
Reach-1	7	PF-1	110.00	2751.66	2752.64	2752.67	2752.98	0.016247	4.71	24.44	41.48	0.98
Reach-1	6	PF-1	110.00	2749.39	2749.94	2750.26	2751.09	0.119556	8.63	12.75	32.33	2.42
Reach-1	5	PF-1	110.00	2746.81	2747.80	2747.91	2748.30	0.028449	5.70	19.29	30.93	1.27
Reach-1	4	PF-1	110.00	2744.09	2744.94	2745.29	2746.05	0.056779	8.44	13.03	19.34	1.81
Reach-1	3	PF-1	110.00	2739.56	2740.88	2741.04	2741.47	0.031414	6.12	17.97	28.08	1.34
Reach-1	2	PF-1	172.00	2735.23	2736.24	2736.57	2737.29	0.051494	8.26	20.84	29.92	1.74
Reach-1	1	PF-1	172.00	2731.01	2731.62	2731.82	2732.31	0.049538	6.63	25.94	50.07	1.62

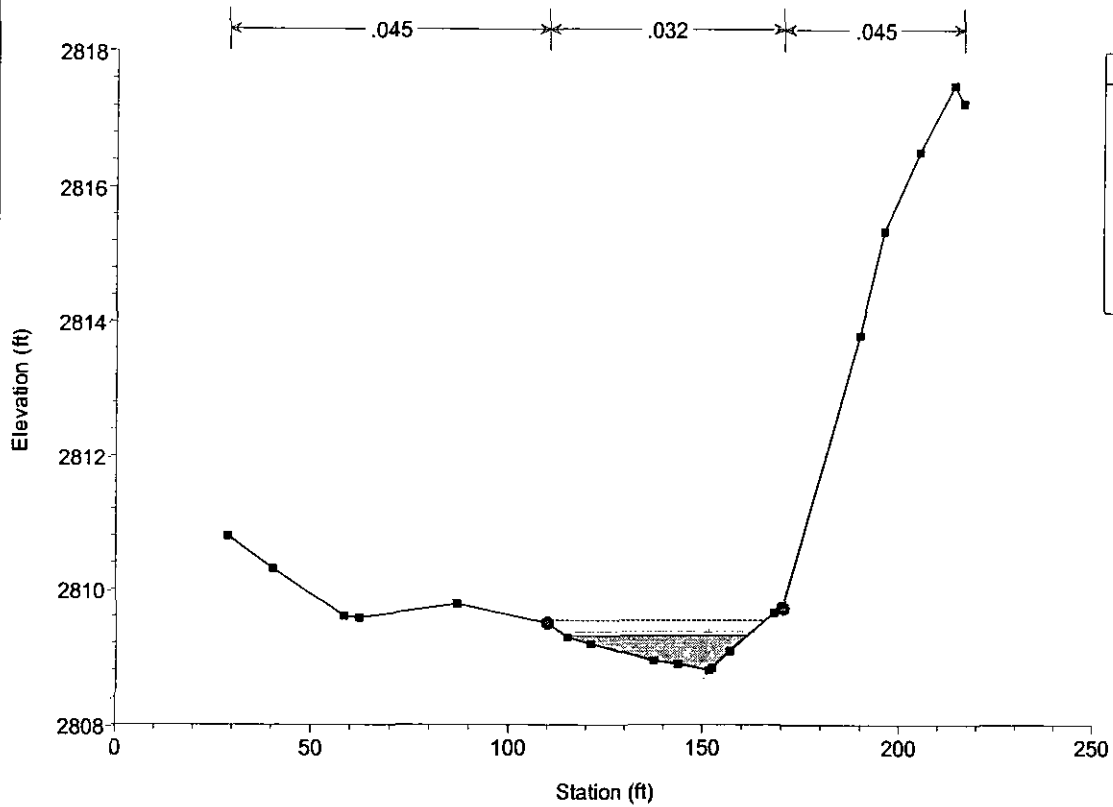
washh2 Plan: Plan 02 11/7/2006



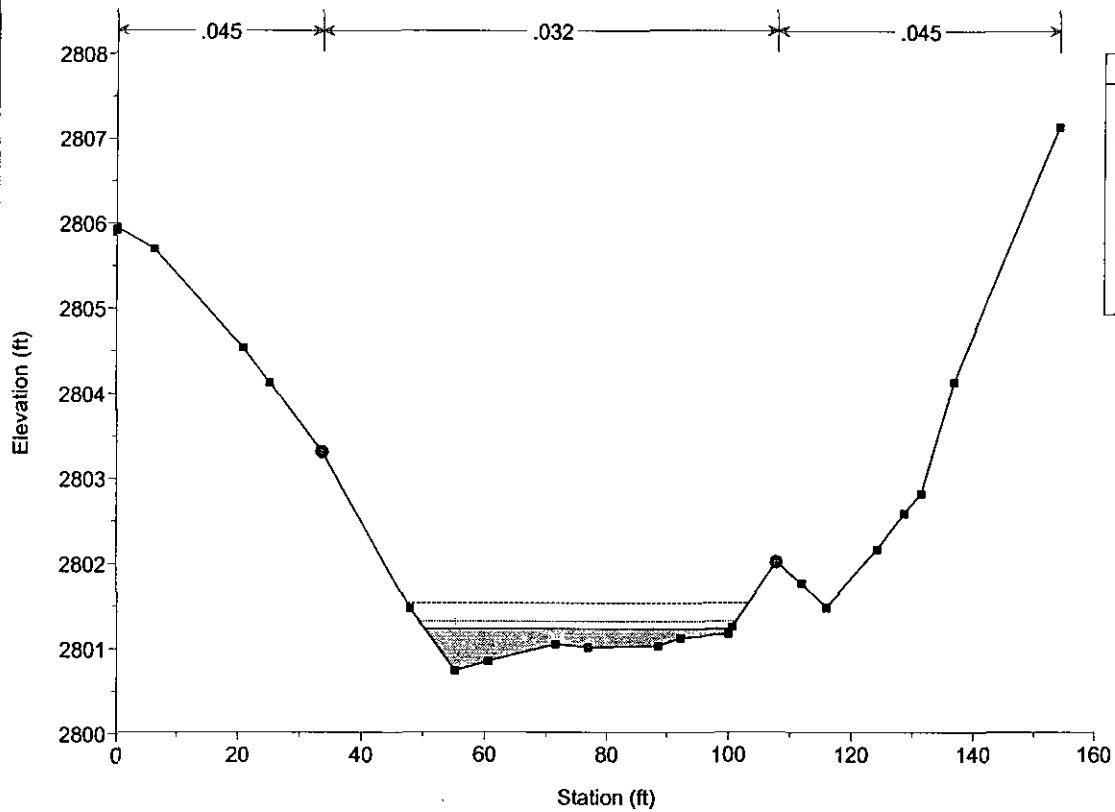
washh2 Plan: Plan 02 11/7/2006



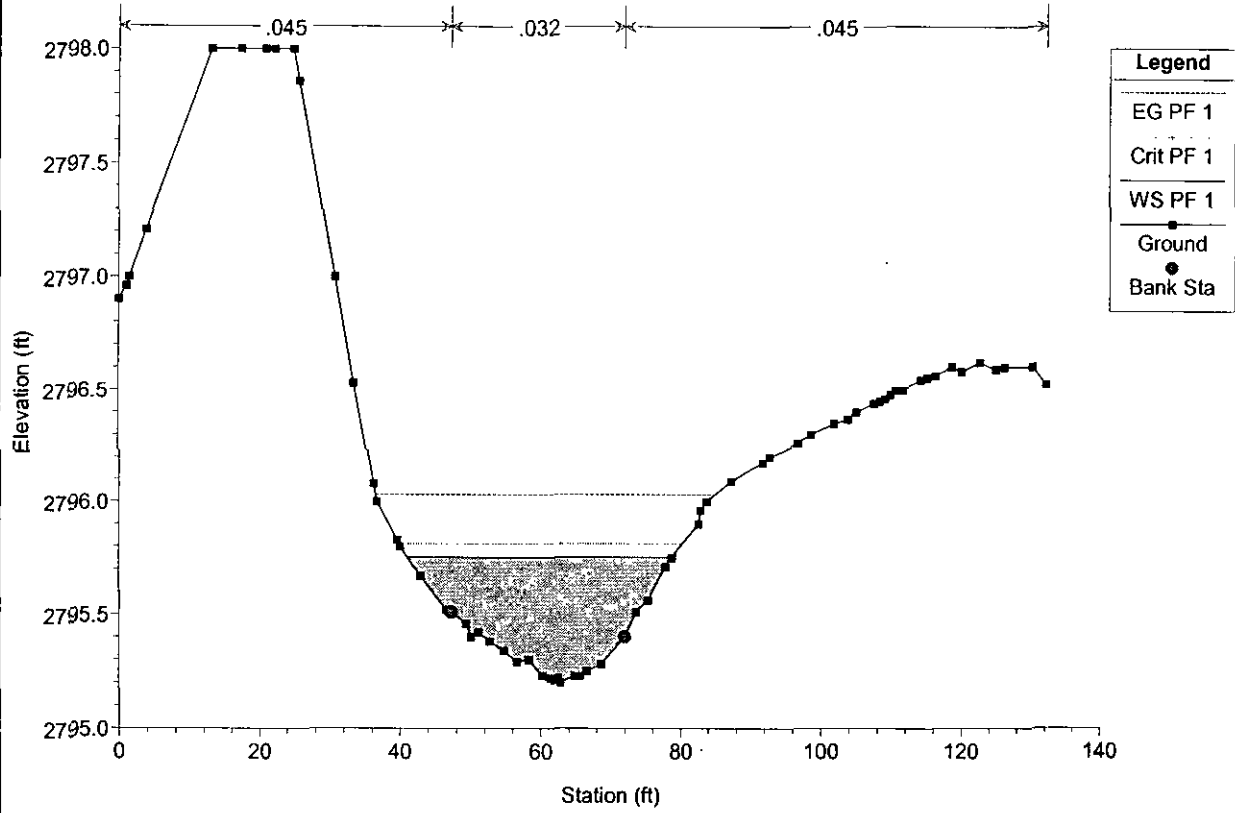
washh2 Plan: Plan 02 11/7/2006



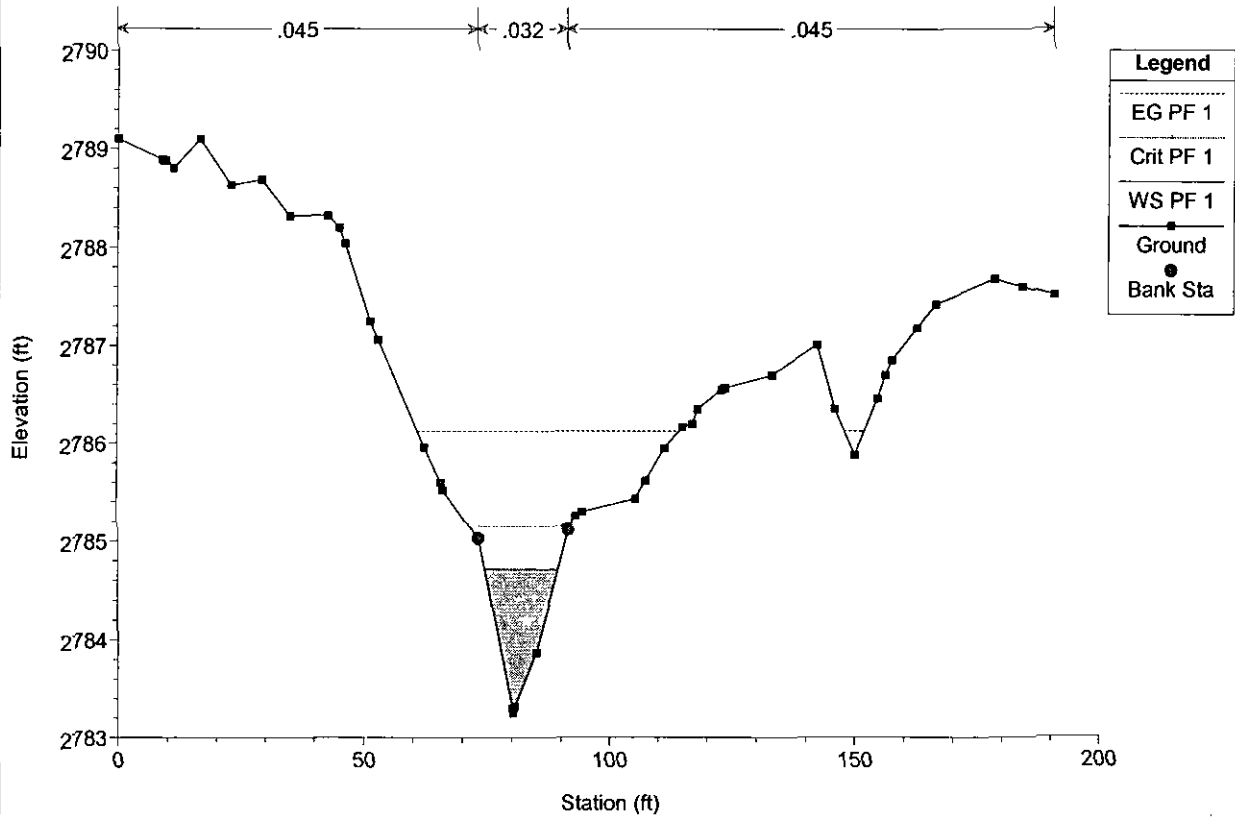
washh2 Plan: Plan 02 11/7/2006



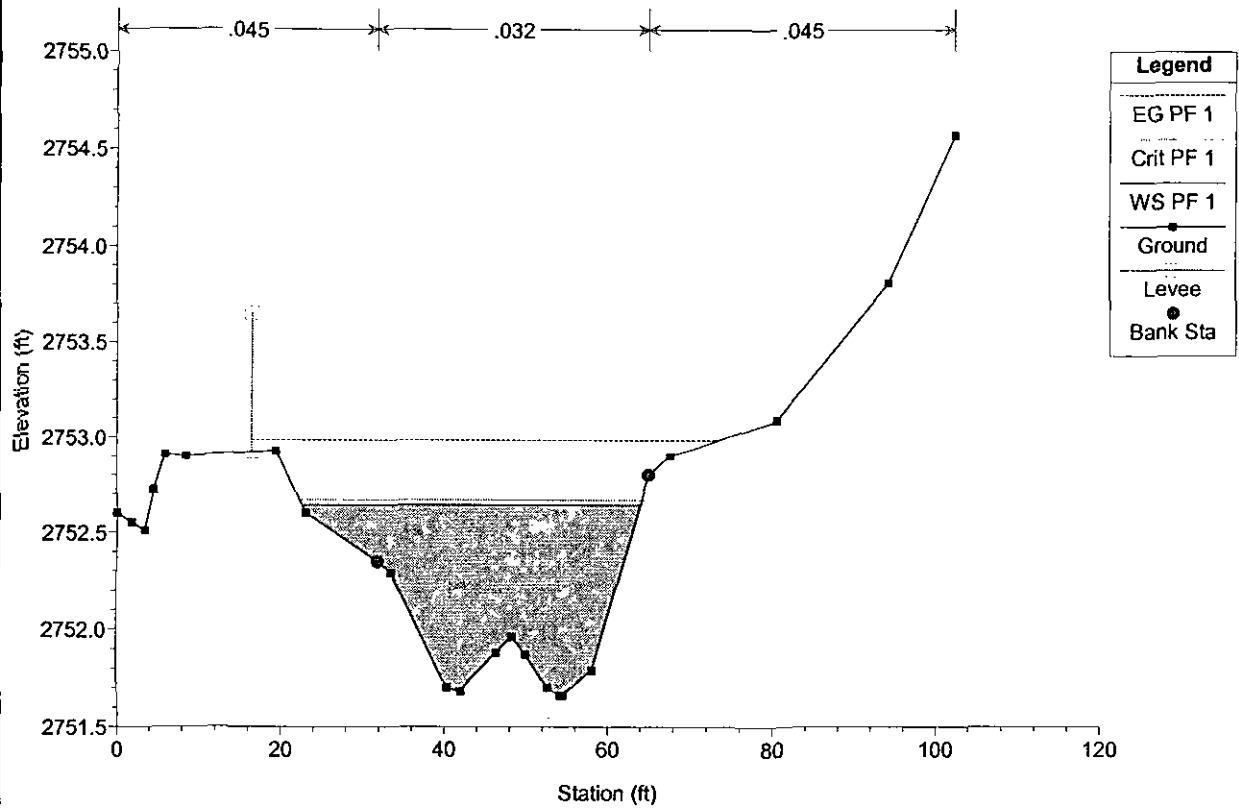
washh2 Plan: Plan 02 11/7/2006



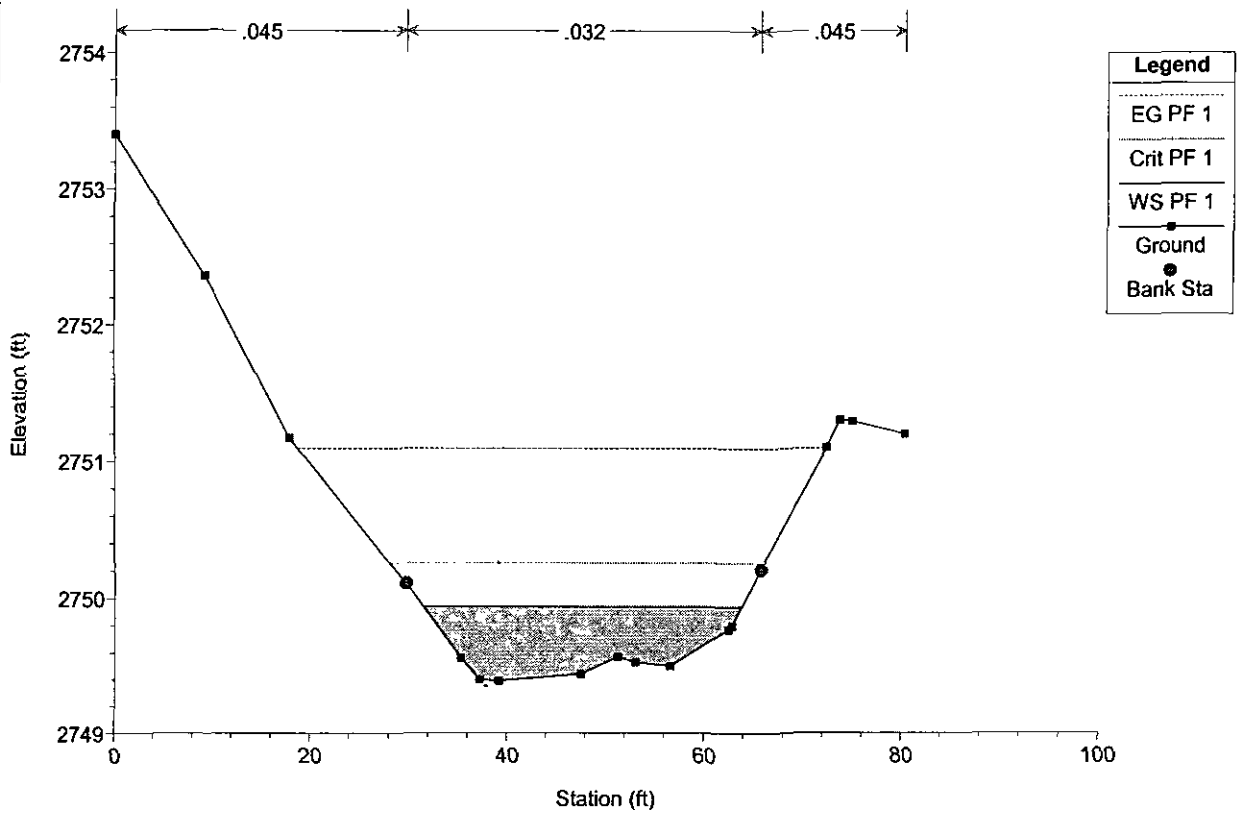
washh2 Plan: Plan 02 11/7/2006



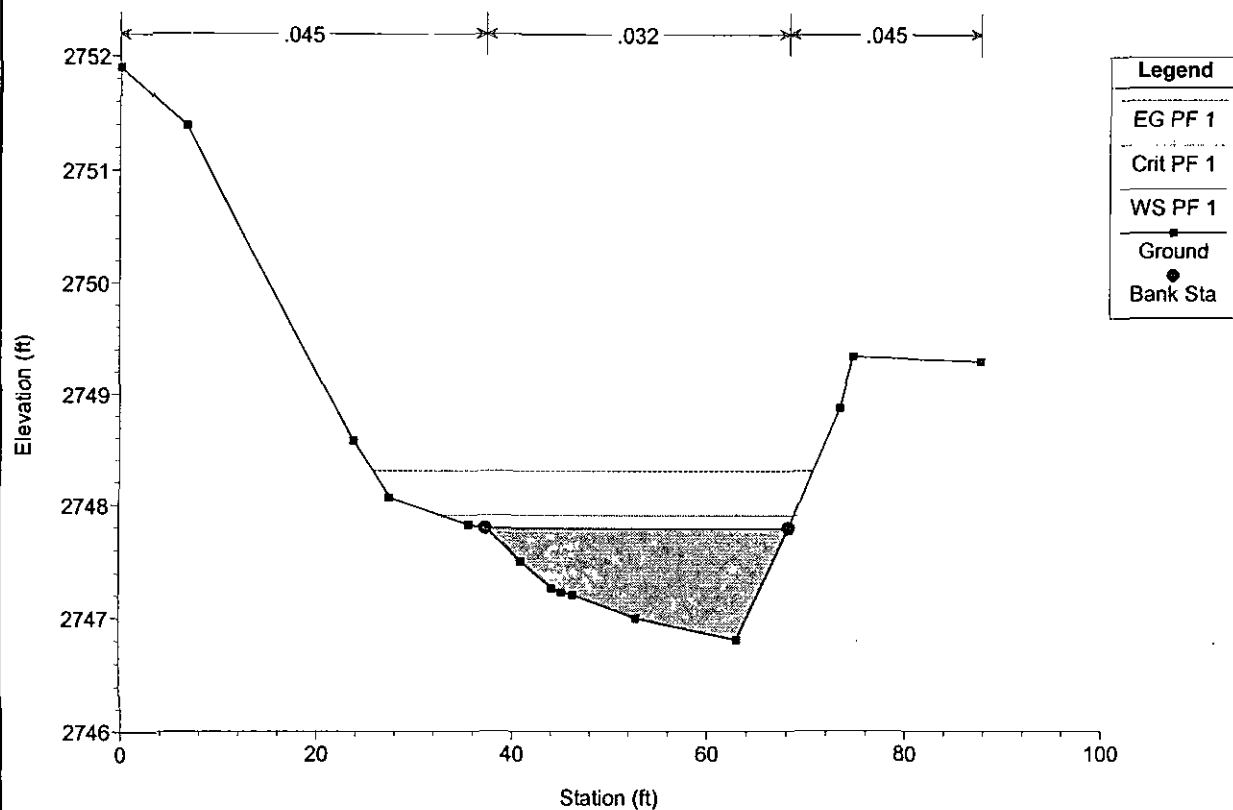
washh2 Plan: Plan 02 11/7/2006



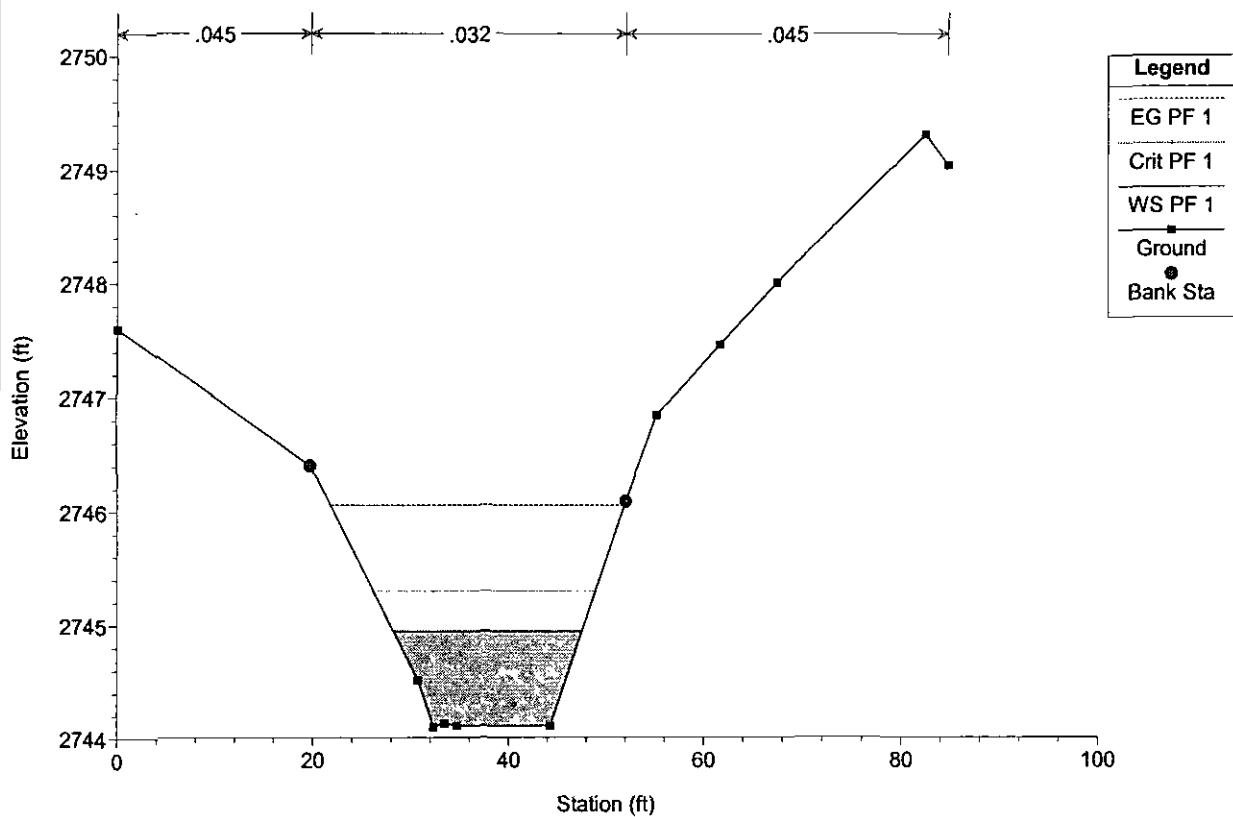
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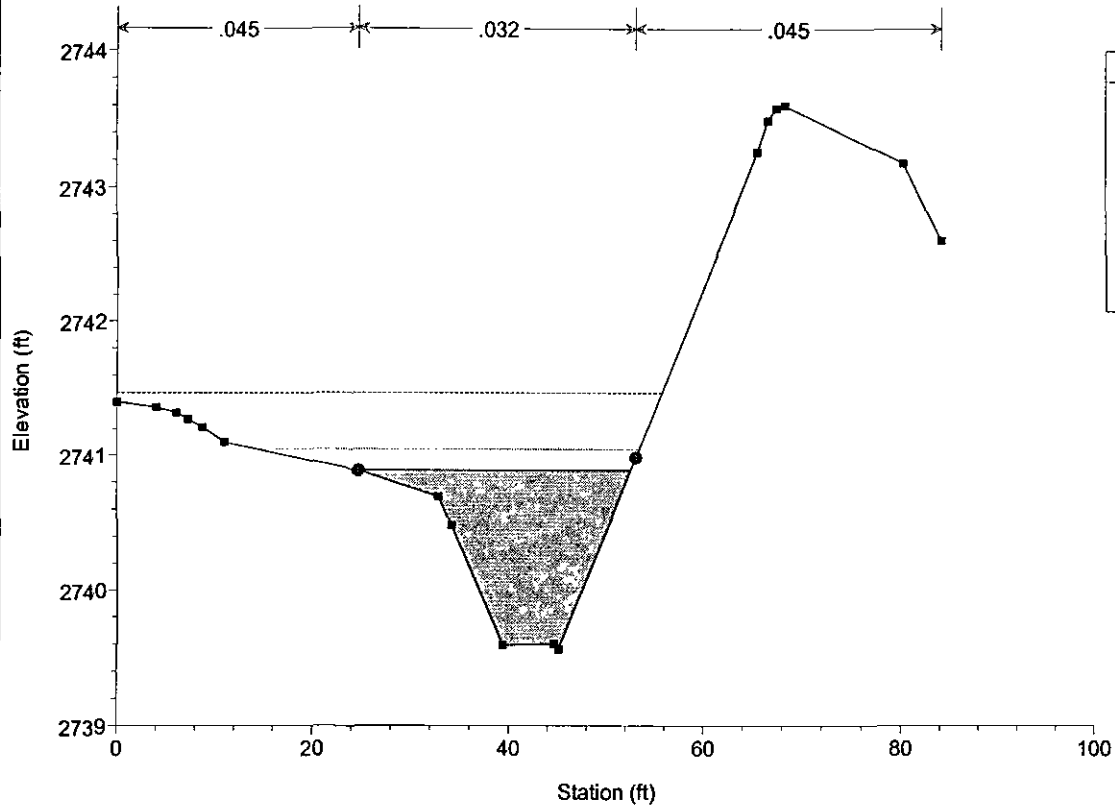
washh2 Plan: Plan 02 11/7/2006



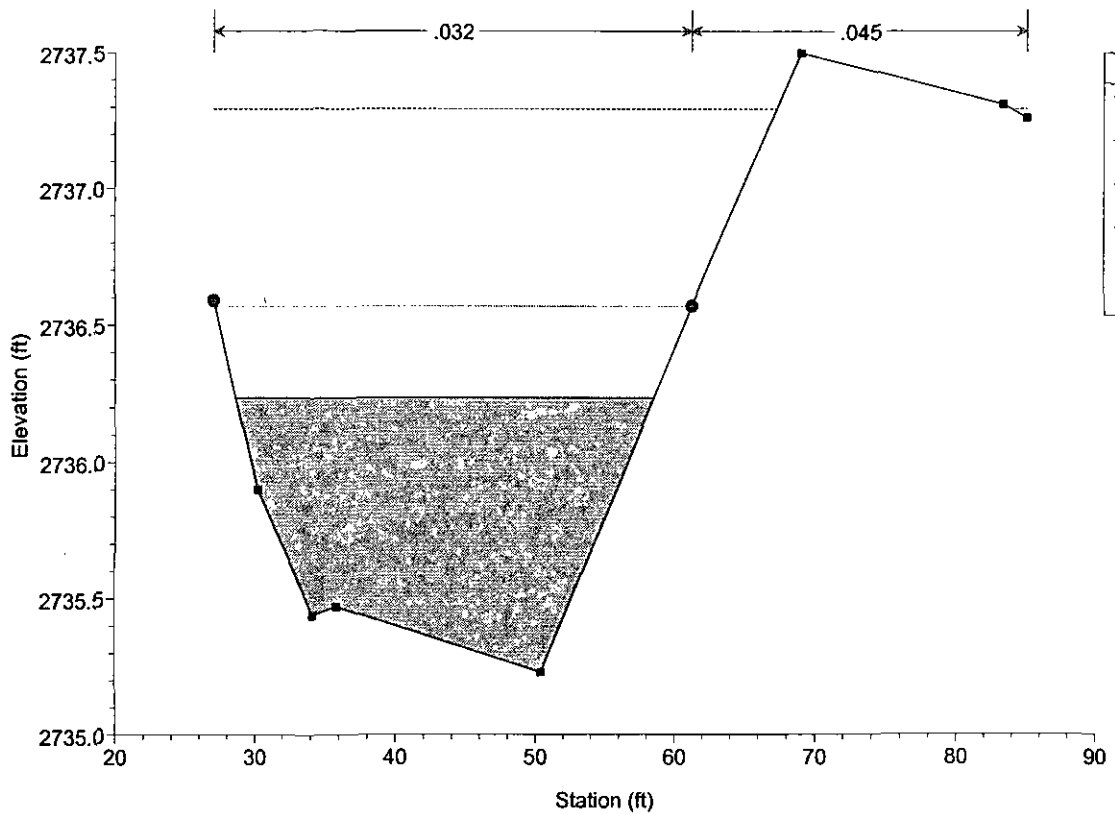
washh2 Plan: Plan 02 11/7/2006



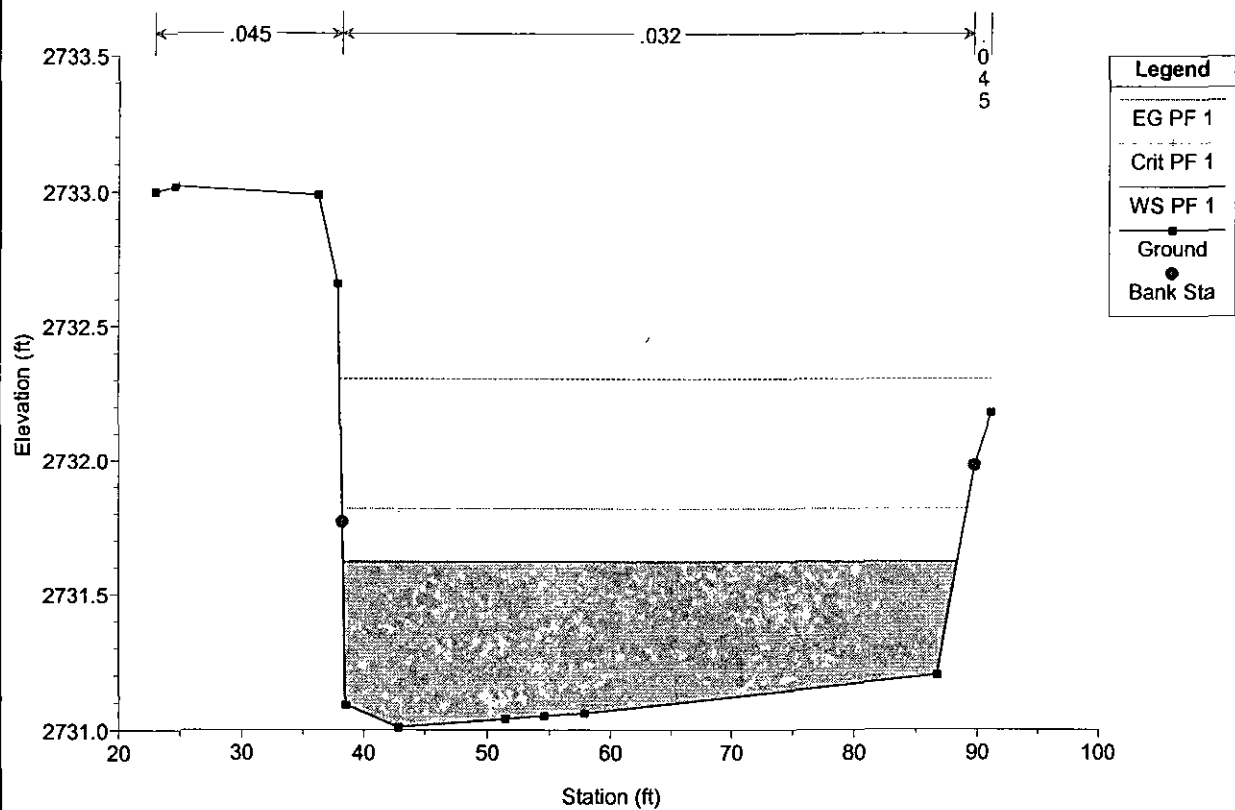
washh2 Plan: Plan 02 11/7/2006



washh2 Plan: Plan 02 11/7/2006



washh2 Plan: Plan 02 11/7/2006

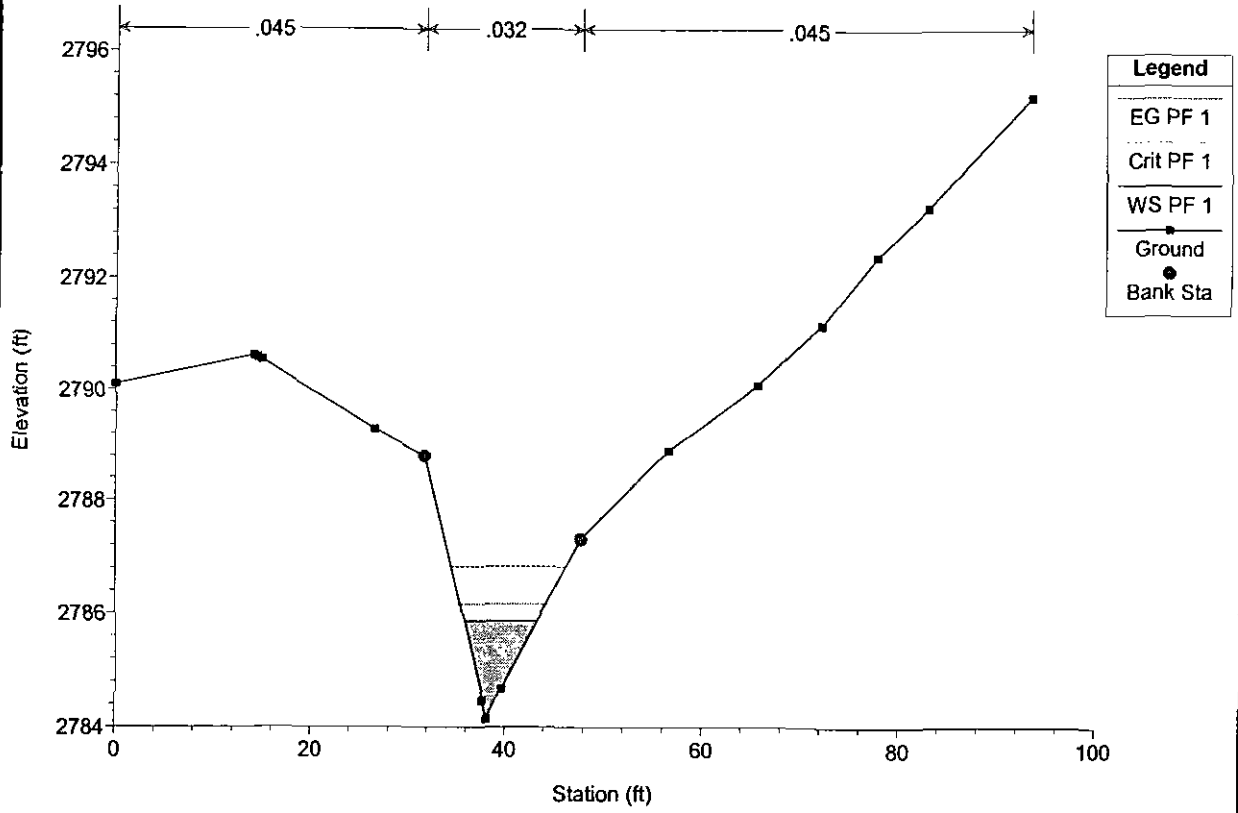


WASH I

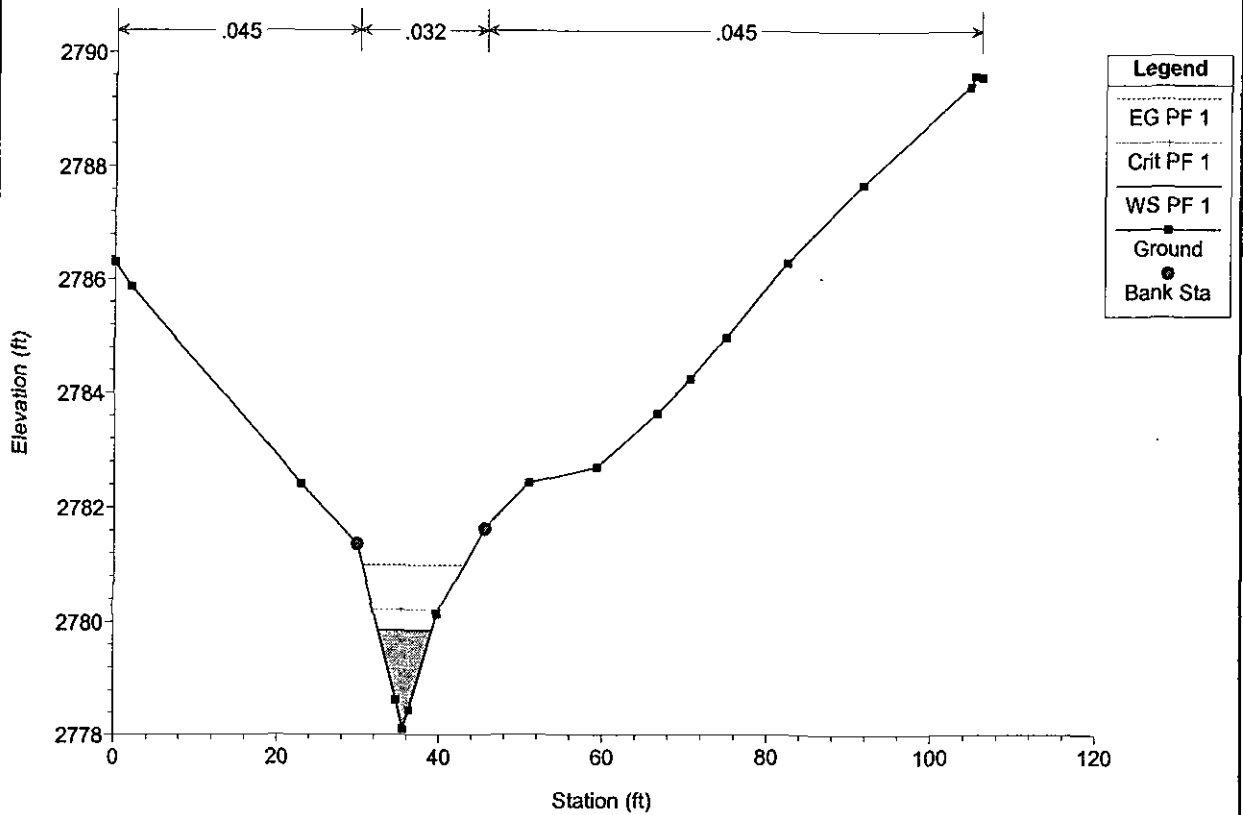
HEC-RAS Plan: Plan 03 River: RIVER-1 Reach: Reach-1 Profile: PF 1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach-1	7	PF 1	50.00	2784.14	2785.86	2786.15	2786.82	0.041077	7.86	6.36	7.51	1.50
Reach-1	6	PF 1	50.00	2778.10	2779.85	2780.22	2780.99	0.047685	8.57	5.84	6.64	1.61
Reach-1	5	PF 1	50.00	2772.66	2773.89	2774.14	2774.70	0.043982	7.22	6.92	10.54	1.57
Reach-1	4	PF 1	50.00	2768.37	2769.92	2770.18	2770.77	0.038173	7.42	6.74	8.58	1.47
Reach-1	3	PF 1	51.00	2764.31	2765.70	2766.03	2766.69	0.049526	7.97	6.40	9.02	1.67
Reach-1	2	PF 1	51.00	2760.53	2761.93	2762.26	2762.86	0.049033	7.75	6.58	9.66	1.65
Reach-1	1.5	PF 1	51.00	2757.88	2758.42	2758.54	2758.75	0.058439	4.61	11.07	42.05	1.58
Reach-1	1	PF 1	73.00	2754.90	2755.88	2755.98	2756.24	0.034060	4.84	15.07	35.24	1.30

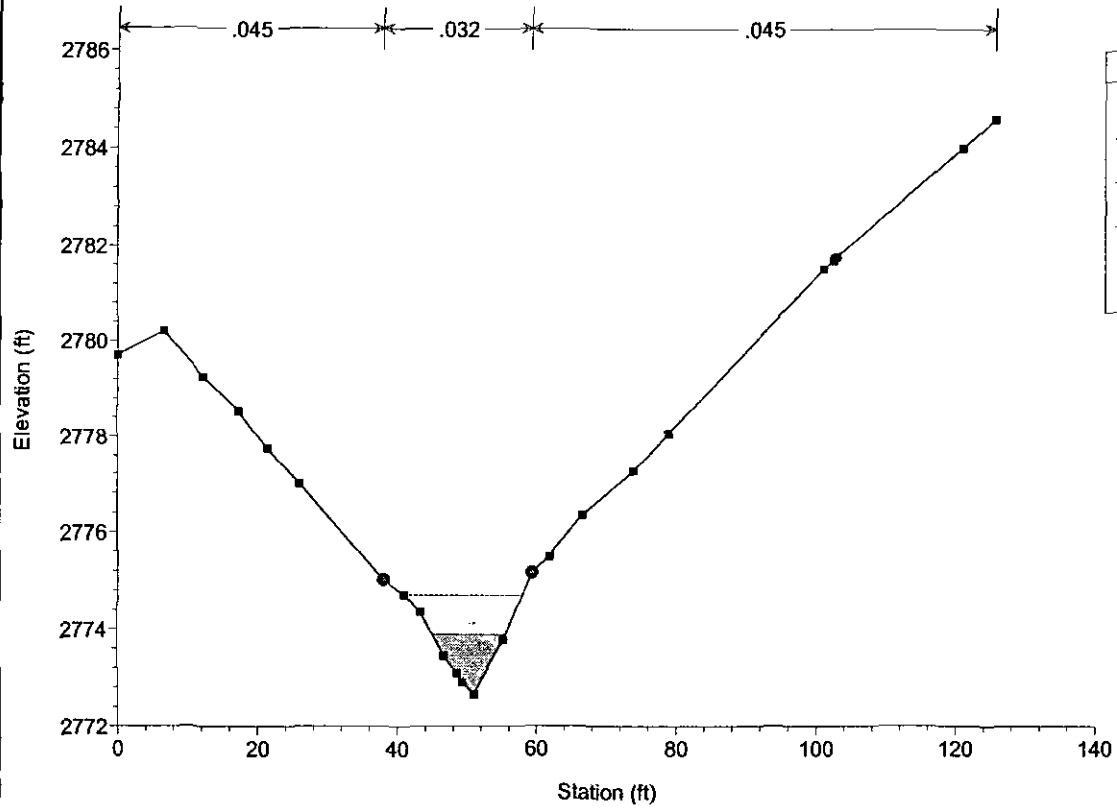
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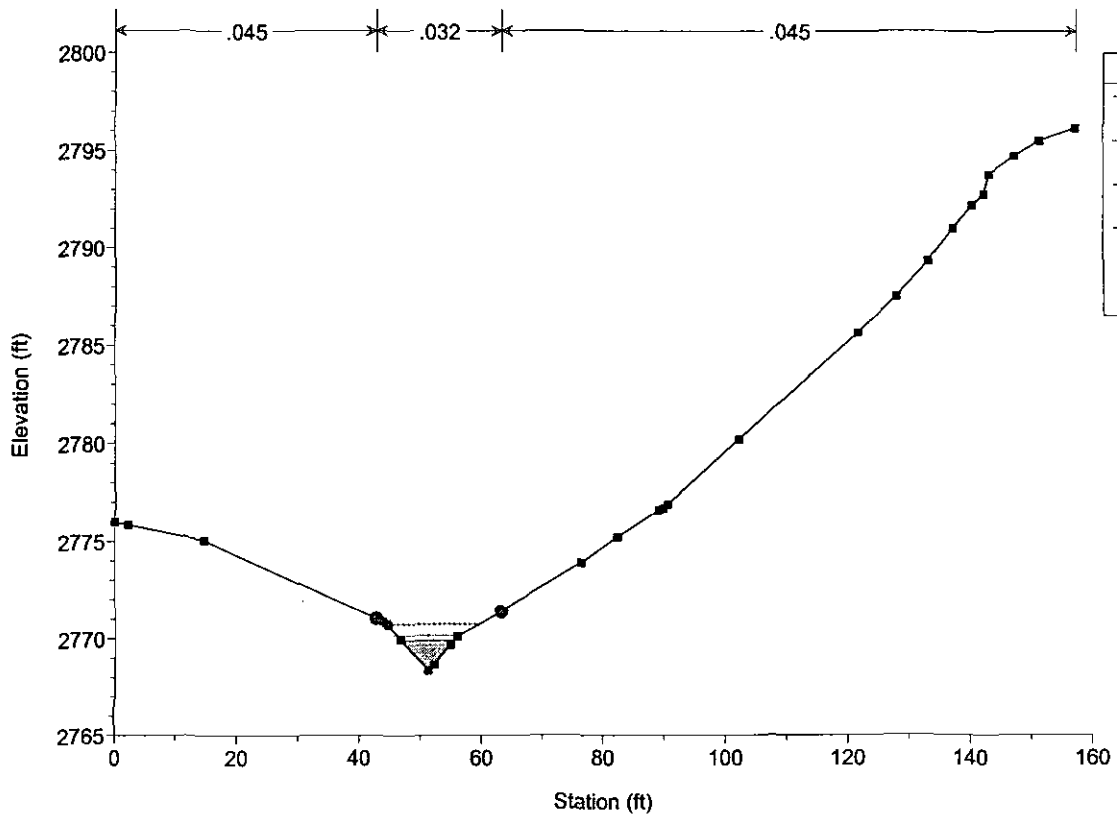
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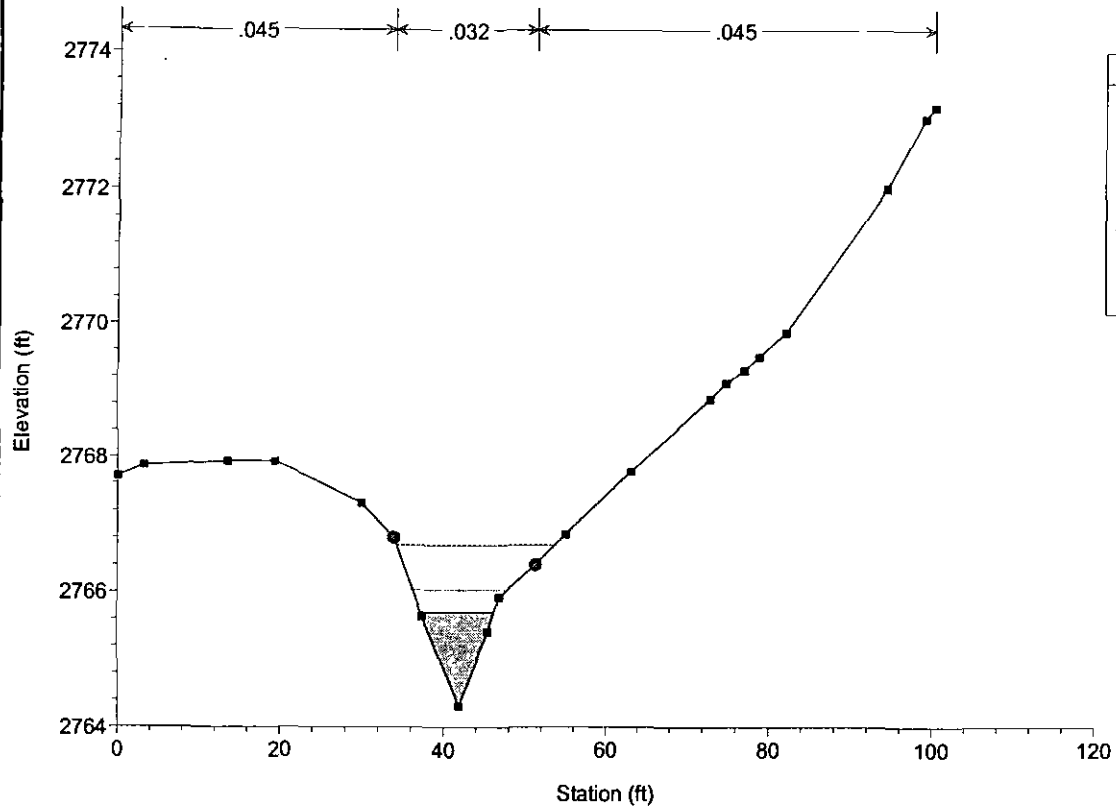
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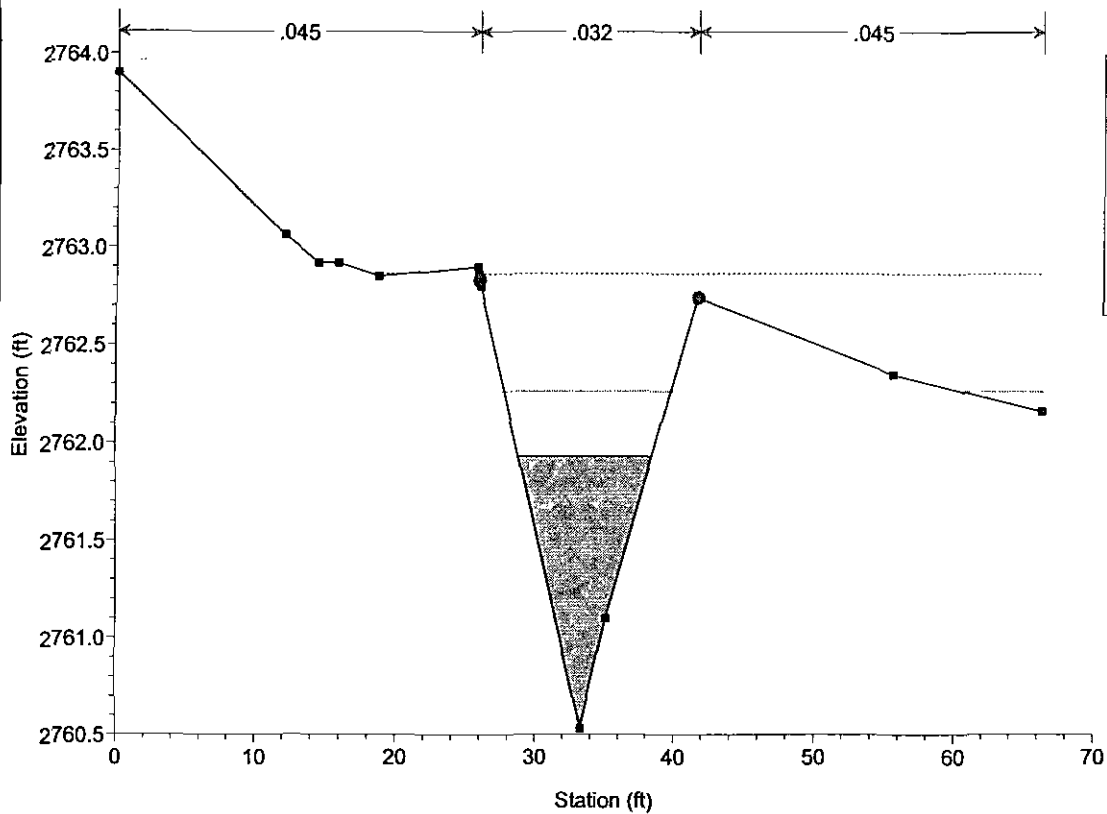
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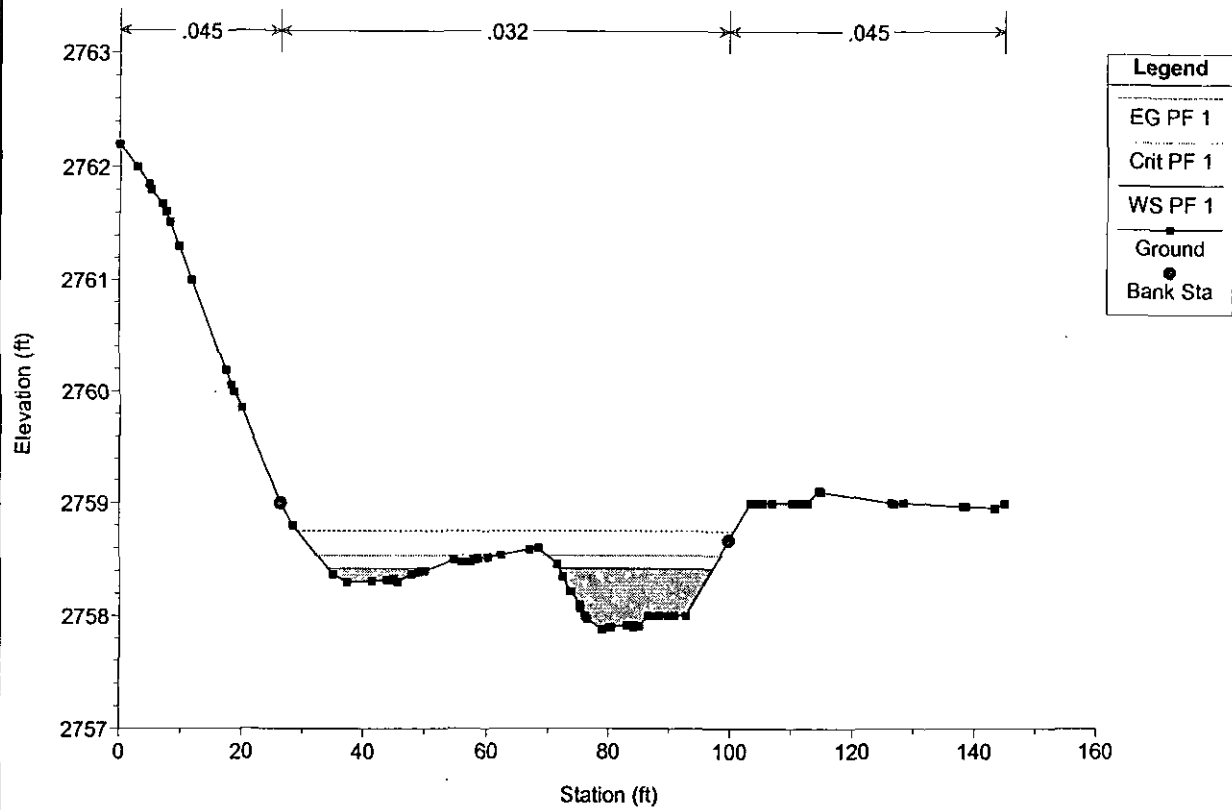
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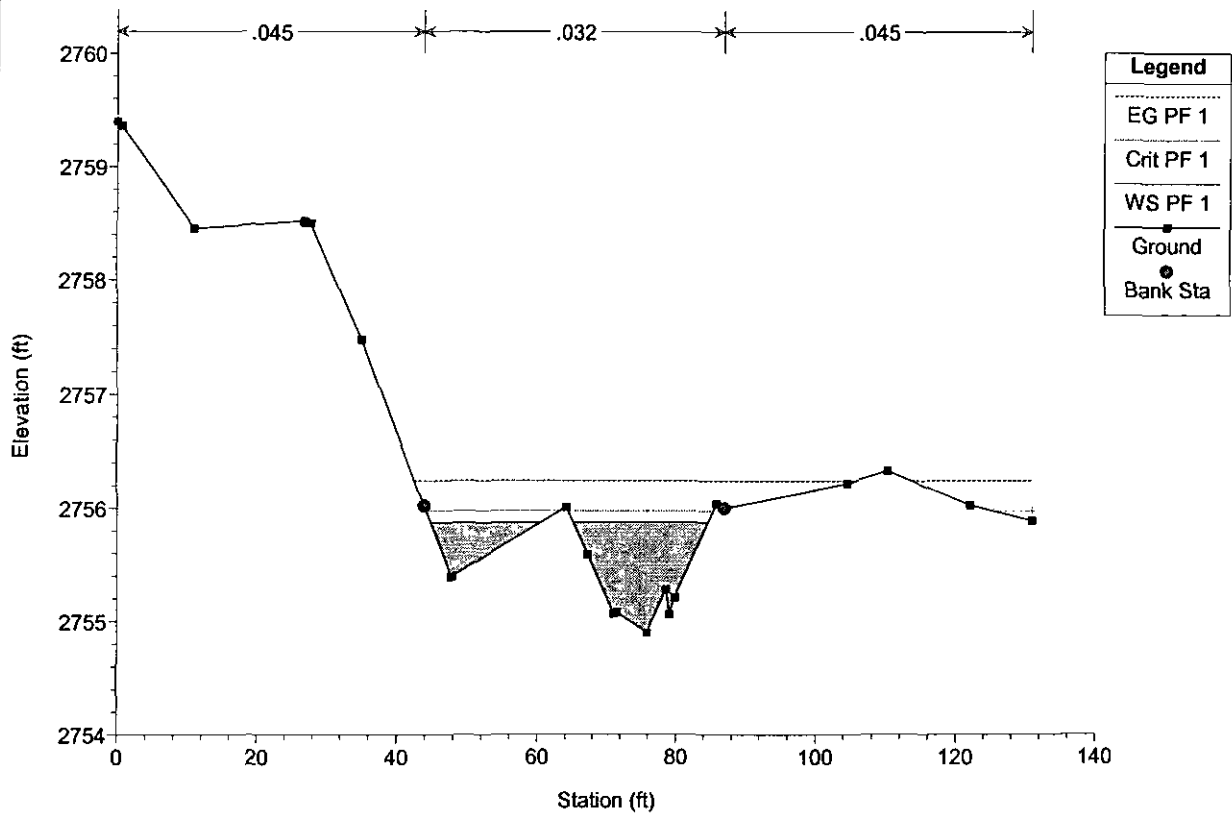
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washi Plan: Plan 03 11/7/2006



Orifice Plate Calculation

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: *Sereno Canyon*

W/P#: 042054

Location: *City of Scottsdale*

Date: 11/15/2006

Reference: *Orifice Equation*

$$Q = CA\sqrt{2gh}$$

Where:

C = Orifice Coefficient = 0.60

d = Diameter of Orifice = 6.00 in

A = Cross-sectional area of orifice = 0.20 Sq.ft

H = Depth above Orifice = 1.50 ft

Q = Orifice Flow = 1.16 cfs

Time to Drain 2 ac-ft = 20.9 hours

A 6 inch diameter orifice plate will bleed at a rate of 1.16 cfs.

APPENDIX G

Storm Water Storage Volume Calculations

Summary of 100-yr, 2-hr Storm Water Storage Calculations

Drainage Basin	Total Development Area (Acre)	100yr-2 hr. Required Volume (acre-ft)	Provided Volume (acre-ft)	Volume Excess / Shortage $\pm$ (acre-ft)
A1	3.79	0.53	0.60	0.07
A2	7.17	1.01	1.01	0.00
B	10.94	1.54	1.78	0.24
C	3.65	0.51	0.57	0.06
D	7.73	1.09	1.25	0.16
E1	14.24	2.01	2.01	0.00
E2	13.23	1.87	2.02	0.15
E3	2.19	0.32	0.58	0.26
F1	10.24	1.44	1.99	0.55
F2	3.56	0.5	0.51	0.01
F3	2.93	0.41	0.40	-0.01
G	3.7	0.52	0.51	-0.01
H1	12.94	1.82	1.98	0.16
H2	8.5	1.2	1.31	0.11
I	4.44	0.63	0.64	0.01
J	4.81	0.68	0.66	-0.02
K	2.06	0.29	0.38	0.09
L	1.49	0.21	0.20	-0.01
M	1.44	0.2	0.21	0.01
N	2.81	0.4	0.49	0.09
O	0.55	0.08	0.22	0.14
P	1.73	0.24	0.37	0.13
Q	0.52	0.07	0.07	0.00
R	0.18	0.02	0.00	-0.02
Total	125	17.59	19.76	2.17

WOOD/PATEL

CIVIL ENGINEERS • HYDROLOGISTS • LAND SURVEYORS • CONSTRUCTION MANAGERS

Project: Sereno Canyon
Location: City of Scottsdale
Date: 15-Nov-06

Project Number: 062654
Project Engineer: Geoff Brownell, P.E.

100-yr, 2-hr Storm Water Storage Calculations

BASIN A1-1				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2756	4193.3	0.096	0.000	0.000
2757	5251	0.121	0.108	0.108
2758	6404	0.147	0.134	0.242
2759	7655	0.176	0.161	0.404
2760	9474.23	0.217	0.197	0.600
2762	11468.06	0.263	0.481	1.081

TOTAL PROVIDED VOLUME: 0.600

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. Provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
A1	A1-1	0.1757	0.600		
TOTAL		0.1757	0.600	0.53	0.070

BASIN A2-1				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2760	492	0.011	0.000	0.000
2761	1062	0.024	0.018	0.018
2762	1981	0.045	0.035	0.053
2763	3373	0.077	0.061	0.114
2764	4971	0.114	0.096	0.210
2765	7030	0.161	0.138	0.348

TOTAL PROVIDED VOLUME: 0.348

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. Provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
A2	A2-1	0.1614	0.348		
	A2-2	0.0674	0.139		
	A2-3	0.1446	0.474		
	A2-4	0.0376	0.050		
TOTAL		0.4110	1.011	1.01	0.00

BASIN A2-2				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2759	200	0.005	0.000	0.000
2760	446	0.010	0.007	0.007
2761	816	0.019	0.014	0.022
2762	1277	0.029	0.024	0.046
2763	1932	0.044	0.037	0.083
2764	2937	0.067	0.056	0.139

TOTAL PROVIDED VOLUME: 0.139

BASIN A2-3				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2733	2268	0.052	0.000	0.000
2734	2913	0.067	0.059	0.059
2735	3632	0.083	0.075	0.135
2736	4466	0.103	0.093	0.228
2737	5369	0.123	0.113	0.340
2738	6297	0.145	0.134	0.474

TOTAL PROVIDED VOLUME: 0.474

BASIN A2-4				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2792	168	0.004	0.000	0.000
2793	425	0.010	0.007	0.007
2794	866	0.020	0.015	0.022
2795	1638	0.038	0.029	0.050

TOTAL PROVIDED VOLUME: 0.050

BASIN B-1				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2798	809	0.019	0.000	0.000
2799	1241	0.028	0.024	0.024
2800	1804	0.041	0.035	0.058
2801	2456	0.056	0.049	0.107

TOTAL PROVIDED VOLUME: 0.107

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
B	B-1	0.0564	0.107		
	B-2	0.1145	0.246		
	B-3	0.1523	0.338		
	B-4	0.1144	0.241		
	B-5	0.2549	0.843		
TOTAL		0.6924	1.776	1.54	0.236

BASIN B-2				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2798	2378	0.055	0.000	0.000
2799	3095	0.071	0.063	0.063
2800	3952	0.091	0.081	0.144
2801	4986	0.114	0.103	0.246

TOTAL PROVIDED VOLUME: 0.246

BASIN B-3				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2781	3419	0.078	0.000	0.000
2782	4333	0.099	0.089	0.089
2783	5371	0.123	0.111	0.200

2784	6633	0.152	0.138	0.338
2785	7269	0.167	0.160	0.498

TOTAL PROVIDED VOLUME: 0.338

BASIN - B-4				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2768	2154	0.049	0.000	0.000
2769	2990	0.069	0.059	0.059
2770	3950	0.091	0.080	0.139
2771	4983	0.114	0.103	0.241

TOTAL PROVIDED VOLUME: 0.241

BASIN - B-5				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2742	5494	0.126	0.000	0.000
2743	7220	0.166	0.146	0.146
2744	9089.9	0.209	0.187	0.333
2745	11105	0.255	0.232	0.565
2746	13101	0.301	0.278	0.843

TOTAL PROVIDED VOLUME: 0.843

BASIN - C-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2779	4198	0.096	0.000	0.000
2780	5112	0.117	0.107	0.107
2781	6102	0.140	0.129	0.236
2782	7169	0.165	0.152	0.388
2783	8375	0.192	0.178	0.566

TOTAL PROVIDED VOLUME: 0.566

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
C	C-1	0.1923	0.566		
TOTAL		0.1923	0.566	0.51	0.056

BASIN - D-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2749	2054	0.047	0.000	0.000
2750	2819	0.065	0.056	0.056
2751	3699	0.085	0.075	0.131
2752	4712	0.108	0.097	0.227
2753	5928	0.136	0.122	0.349

TOTAL PROVIDED VOLUME: 0.349

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
D	D-1	0.1361	0.349		
	D-2	0.2021	0.898		
TOTAL		0.3382	1.248	1.09	0.158

BASIN - D-2				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2719	732.75	0.017	0.000	0.000
2720	2673.44	0.061	0.039	0.039
2721	5208.95	0.120	0.090	0.130
2722	8803	0.202	0.161	0.290
2723	13007	0.299	0.250	0.541
2724	18123.75	0.416	0.357	0.898

TOTAL PROVIDED VOLUME: 0.898

BASIN - E1-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2723	5198	0.119	0.000	0.000
2724	6100	0.140	0.130	0.130
2725	7068	0.162	0.151	0.281
2726	8098	0.186	0.174	0.455
2727	9189	0.211	0.198	0.653
2728	10351	0.238	0.224	0.878

TOTAL PROVIDED VOLUME: 0.878

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
E1	E1-1	0.1859	0.878		
	E1-2	0.3307	1.135		
TOTAL		0.5166	2.013	2.010	0.003

BASIN - E1-2				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2695	5427	0.125	0.000	0.000
2696	7366	0.169	0.147	0.147
2697	8975	0.206	0.188	0.334
2698	10686	0.245	0.226	0.560
2699	12497	0.287	0.266	0.826
2700	14406	0.331	0.309	1.135

TOTAL PROVIDED VOLUME: 1.135

BASIN - E2-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2693	13886	0.319	0.000	0.000

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
E2	E2-1	0.4963	2.020	1.87	

2694	15293	0.351	0.335	0.335
2695	16761	0.385	0.368	0.703
2696	18290	0.420	0.402	1.105
2697	19880	0.456	0.438	1.543
2698	21620	0.496	0.476	2.020

TOTAL		0.4963	2.020	1.870	0.150

TOTAL PROVIDED VOLUME: 2.020

BASIN E3-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2711	3064	0.070	0.000	0.000
2712	4065	0.093	0.082	0.082
2713	5197	0.119	0.106	0.188
2714	6456	0.148	0.134	0.322

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
E3	E3-1	0.1482	0.322		
	E3-2	0.1557	0.257		
TOTAL		0.3039	0.579	0.32	0.259

TOTAL PROVIDED VOLUME: 0.322

BASIN E3-2				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2683	65.87	0.002	0.000	0.000
2684	317.95	0.007	0.004	0.004
2685	1008.73	0.023	0.015	0.020
2686	2251.36	0.052	0.037	0.057
2687	4199.64	0.096	0.074	0.131
2688	6780.55	0.156	0.126	0.257

TOTAL PROVIDED VOLUME: 0.257

BASIN F1-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2725	5323.24	0.122	0.000	0.000
2726	5822.09	0.134	0.128	0.128
2727	6327.79	0.145	0.139	0.267
2728	6840.34	0.157	0.151	0.419

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
F1	F1-1	0.1570	0.419		
	F1-2	0.2161	0.537		
	F1-3	0.2707	0.701		
	F1-4	0.1630	0.338		
TOTAL		0.8067	1.995	1.44	0.555

TOTAL PROVIDED VOLUME: 0.419

BASIN F1-2				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2708	6246.38	0.143	0.000	0.000
2709	7252.9	0.167	0.155	0.155
2710	8307.83	0.191	0.179	0.334
2711	9411.15	0.216	0.203	0.537

TOTAL PROVIDED VOLUME: 0.537

BASIN F1-3				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2696	8712.31	0.200	0.000	0.000
2697	9621.58	0.221	0.210	0.210
2698	10649.01	0.244	0.233	0.443
2699	11791.2	0.271	0.258	0.701

TOTAL PROVIDED VOLUME: 0.701

BASIN F1-4				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2676	3139.86	0.072	0.000	0.000
2677	4162.11	0.096	0.084	0.084
2678	5460.19	0.125	0.110	0.194
2679	7098.73	0.163	0.144	0.338

TOTAL PROVIDED VOLUME: 0.338

BASIN F2-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2700	765	0.018	0.000	0.000
2701	1324	0.030	0.024	0.024
2702	1953.89	0.045	0.038	0.062
2703	2662.39	0.061	0.053	0.115

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
F2	F2-1	0.0611	0.115		
	F2-2	0.1637	0.396		
TOTAL		0.2248	0.511	0.5	0.011

TOTAL PROVIDED VOLUME: 0.115

BASIN F2-2				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2681	4470.86	0.103	0.000	0.000
2682	5282.49	0.121	0.112	0.112
2683	6174.37	0.142	0.132	0.243
2684	7130.31	0.164	0.153	0.396

TOTAL PROVIDED VOLUME: 0.396

BASIN F3-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2704	1240	0.028	0.000	0.000
2705	2048.96	0.047	0.038	0.038
2706	2915.75	0.067	0.057	0.095
2707	3851.96	0.088	0.078	0.172
2708	4957.71	0.114	0.101	0.274
2709	6277.08	0.144	0.129	0.403

TOTAL PROVIDED VOLUME: 0.403

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
F3	F3-1	0.0884	0.403		
TOTAL		0.0884	0.403	0.41	-0.007

BASIN G-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2722	3720	0.085	0.000	0.000
2723	4513	0.104	0.095	0.095
2724	5460	0.125	0.114	0.209
2725	6561	0.151	0.138	0.347
2726	7817	0.179	0.165	0.512
2727	9484	0.218	0.199	0.711

TOTAL PROVIDED VOLUME: 0.512

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
G	G-1	0.0000	0.512		
TOTAL		0.0000	0.512	0.520	-0.008

BASIN H1-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2738	36.36	0.001	0.000	0.000
2739	715.37	0.016	0.009	0.009
2740	2504.5	0.057	0.037	0.046
2741	5064.58	0.116	0.087	0.132
2742	8155.31	0.187	0.152	0.284
2743	11514.5	0.264	0.226	0.510
2744	15653.69	0.359	0.312	0.822
2745	20773.56	0.477	0.418	1.240
2745.5	24068.465	0.553	0.257	1.497
2746	27363.37	0.628	0.988	1.809

TOTAL PROVIDED VOLUME: 1.497

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
H1	H1-1	0.5525	1.497		
	H1-2	0.2491	0.484		
TOTAL		0.8016	1.98	1.82	0.2

BASIN H1-2				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2721	49.02	0.001	0.000	0.000
2722	1008	0.023	0.012	0.012
2723	2630	0.060	0.042	0.054
2724	4685	0.108	0.084	0.138
2725	7311	0.168	0.138	0.276
2726	10849	0.249	0.208	0.484

TOTAL PROVIDED VOLUME: 0.484

BASIN H2-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2772	6103	0.140	0.000	0.000
2773	6926	0.159	0.150	0.150
2774	7774	0.178	0.169	0.318
2775	8675	0.199	0.189	0.507
2776	9692	0.222	0.211	0.718

TOTAL PROVIDED VOLUME: 0.718

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
H2	H2-1	0.2225	0.718		
	H2-2	0.2463	0.590		
TOTAL		0.4688	1.308	1.2	0.108

BASIN H2-2				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2785	6526	0.150	0.000	0.000
2786	7846	0.180	0.165	0.165
2787	9247	0.212	0.196	0.361
2788	10730	0.246	0.229	0.590

TOTAL PROVIDED VOLUME: 0.590

BASIN I-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2755	7532	0.173	0.000	0.000
2756	8664	0.199	0.186	0.186
2757	9863	0.226	0.213	0.399
2758	11132	0.256	0.241	0.640

TOTAL PROVIDED VOLUME: 0.640

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
I	I-1	0.2556	0.640		
TOTAL		0.2556	0.640	0.63	0.010

BASIN J-1				
STAGE	AREA		VOLUME	
	[ft²]	[acres]	INC	CUM

VOLUME BREAKDOWN					
Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±

	[ft ²]	[acres]	[acre-feet]	[acre-feet]
2770	3543	0.081	0.000	0.000
2771	4264	0.098	0.090	0.090
2772	5183	0.119	0.108	0.198
2773	6092	0.140	0.129	0.327
2774	7205	0.165	0.153	0.480
2775	8610	0.198	0.182	0.662

TOTAL PROVIDED VOLUME: 0.662

J	J-1	0.1977	0.662		
TOTAL		0.1977	0.662	0.68	-0.018

BASIN K-1				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2737	4211	0.097	0.000	0.000
2738	5026	0.115	0.106	0.106
2739	5909	0.136	0.126	0.232
2740	6860	0.157	0.147	0.378

TOTAL PROVIDED VOLUME: 0.378

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
K	K-1	0.1575	0.378		
TOTAL		0.1575	0.378	0.29	-0.088

BASIN L-1				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2722	1286.47	0.030	0.000	0.000
2723	2258.57	0.052	0.041	0.041
2724	3415.36	0.078	0.065	0.106
2725	4774.68	0.110	0.094	0.200

TOTAL PROVIDED VOLUME: 0.200

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
L	L-1	0.0000	0.200		
TOTAL		0.0000	0.200	0.21	-0.010

BASIN M-1				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2713	2180	0.050	0.000	0.000
2714	2684.4	0.062	0.056	0.056
2715	3242.27	0.074	0.068	0.124
2716	3851.17	0.088	0.081	0.205

TOTAL PROVIDED VOLUME: 0.205

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
M	M-1	0.0884	0.205		
TOTAL		0.0884	0.205	0.2	-0.005

BASIN N-1				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2688	3077.83	0.071	0.000	0.000
2689	4145.22	0.095	0.083	0.083
2690	5272.59	0.121	0.108	0.191
2691	6491.16	0.149	0.135	0.326
2692	7806.7	0.179	0.164	0.490

TOTAL PROVIDED VOLUME: 0.490

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
N	N-1	0.1792	0.490		
TOTAL		0.1792	0.490	0.4	-0.090

BASIN O-1				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2721	527	0.012	0.000	0.000
2722	1798	0.041	0.027	0.027
2723	4070	0.093	0.067	0.094
2724	7173	0.165	0.129	0.223

TOTAL PROVIDED VOLUME: 0.223 acre-feet

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
O	O-1	0.1647	0.223		
TOTAL		0.1647	0.223	0.08	-0.143

BASIN P-1				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2764	2082	0.048	0.000	0.000
2765	2732	0.063	0.055	0.055

TOTAL PROVIDED VOLUME: 0.055 acre-feet

VOLUME BREAKDOWN

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
P	P-1 P-2	0.0627 0.1149	0.055 0.318		
TOTAL		0.1776	0.373	0.24	-0.133

BASIN P-2				
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2739	88.86	0.002	0.000	0.000
2740	493.65	0.011	0.007	0.007
2741	1522.96	0.035	0.023	0.030
2742	2955.84	0.068	0.051	0.081
2743	5005.28	0.115	0.091	0.173
2744	7620.68	0.175	0.145	0.318

TOTAL PROVIDED VOLUME: 0.318

VOLUME BREAKDOWN

BASIN Q-1				
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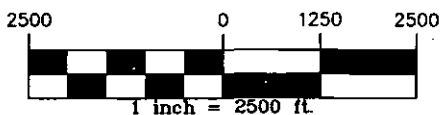
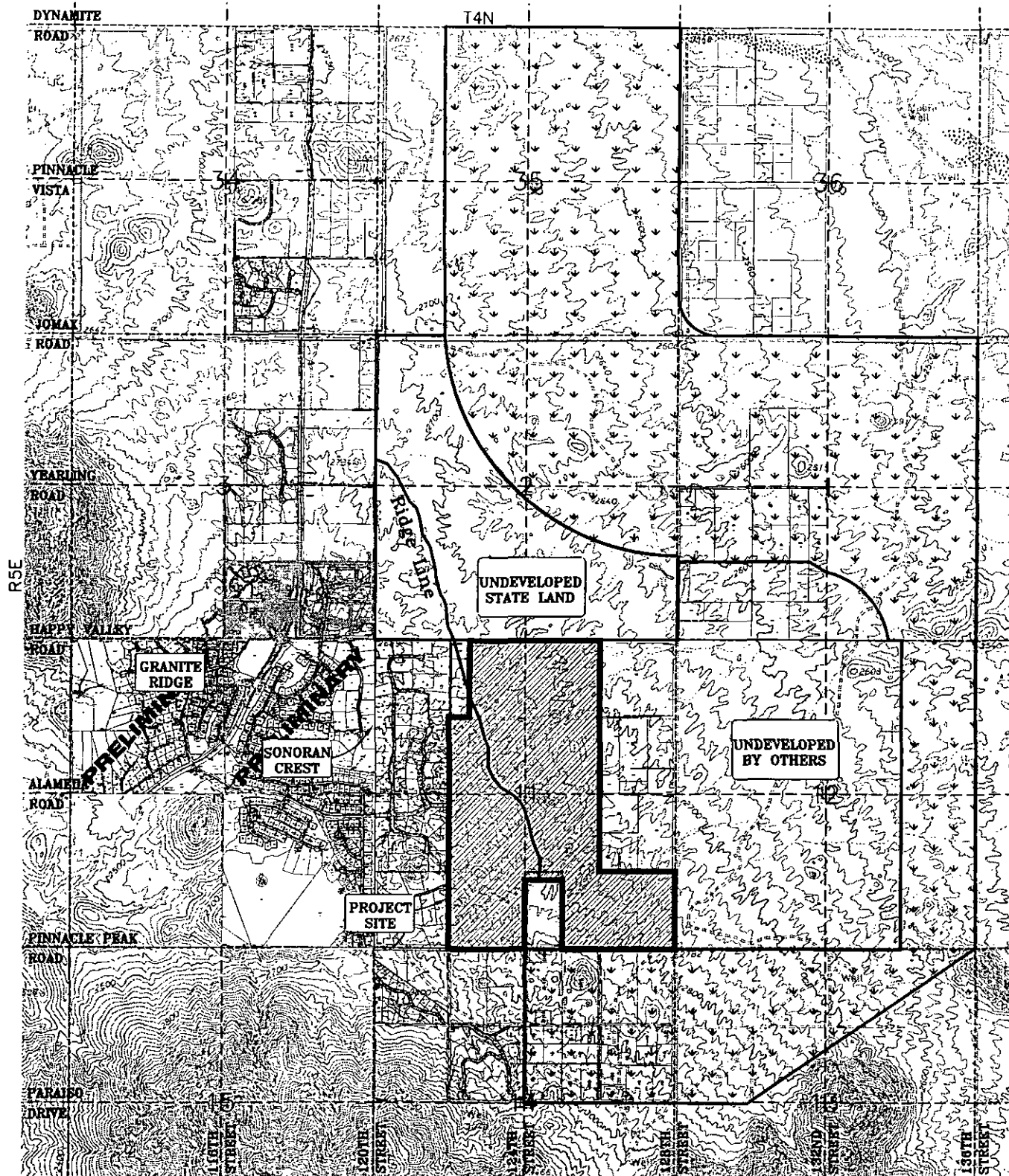
STAGE	AREA		VOLUME	
	[ft ²]	[acres]	INC [acre-feet]	CUM [acre-feet]
2774	2722	0.062	0.000	0.000
2775	3568	0.082	0.072	0.072

Drainage Boundary	Sub-Basin Volumes	Disturbance Areas (acres)	100yr-2hr. provided ac-ft	100yr-2hr. Required ac-ft	Volume Shortage / Excess ±
Q	Q-1	0.0819	0.072		
TOTAL		0.0819	0.072	0.07	0.002

TOTAL PROVIDED VOLUME: 0.072

PLATE 1

Vicinity Map



LEGEND

- Project Boundary
- Project Site
- McDowell Sonoran Preserve

Existing 5 Foot Contours

CROWN
COMMUNITY DEVELOPMENT
A Honey Creek Company

SERENO CANYON

Plate I
"Vicinity Map"

WOOD/PATEL

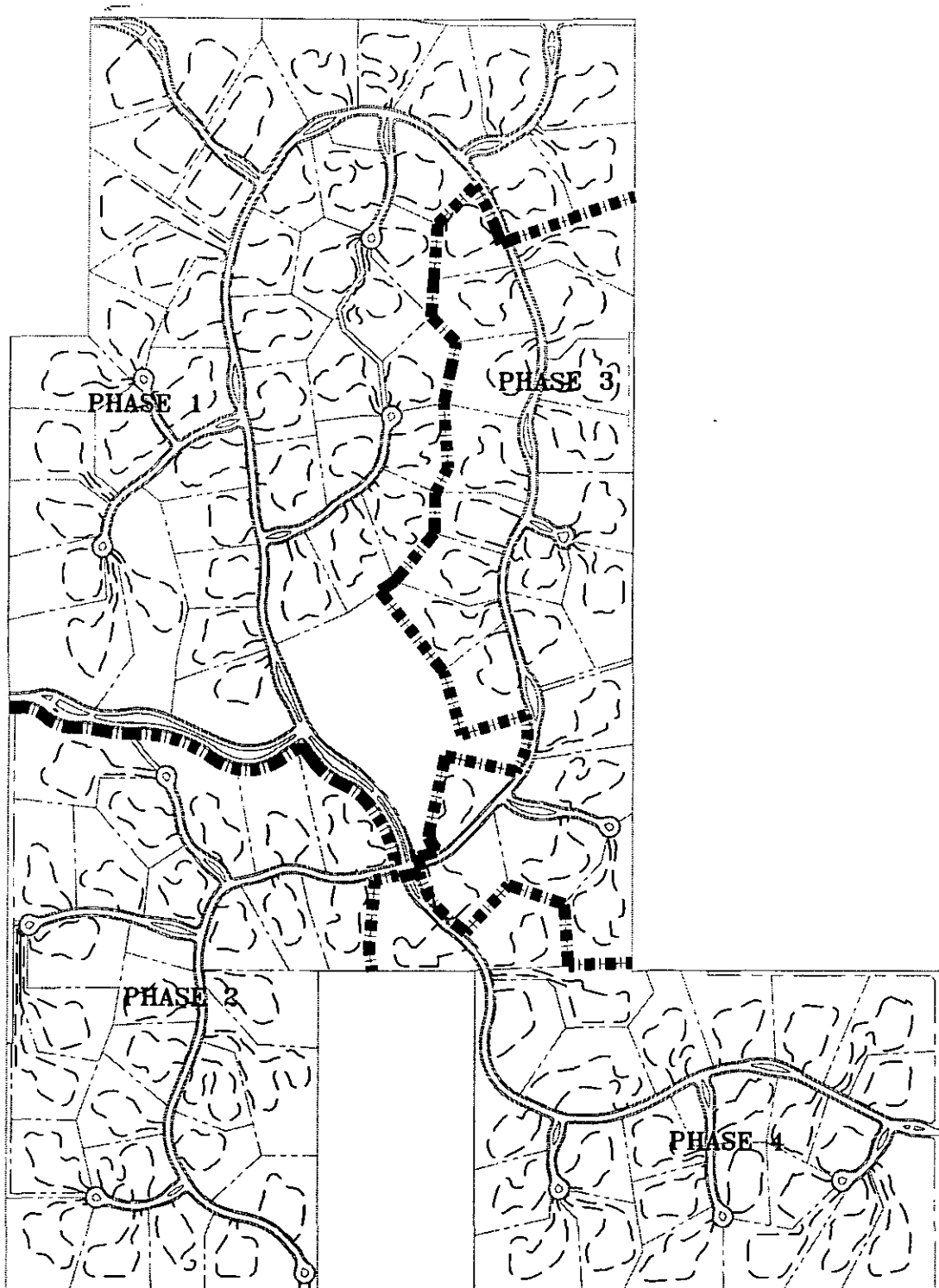
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TRANSPORTATION / TRAFFIC
WATER / WASTEWATER • SURVEYING
CONSTRUCTION MANAGEMENT

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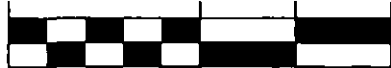
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PLATE 1A

Phasing Map



700 0 350 700



1 inch = 700 ft.



SERENO CANYON

Plate 1A
"Phasing Map"

WOOD/PATEL

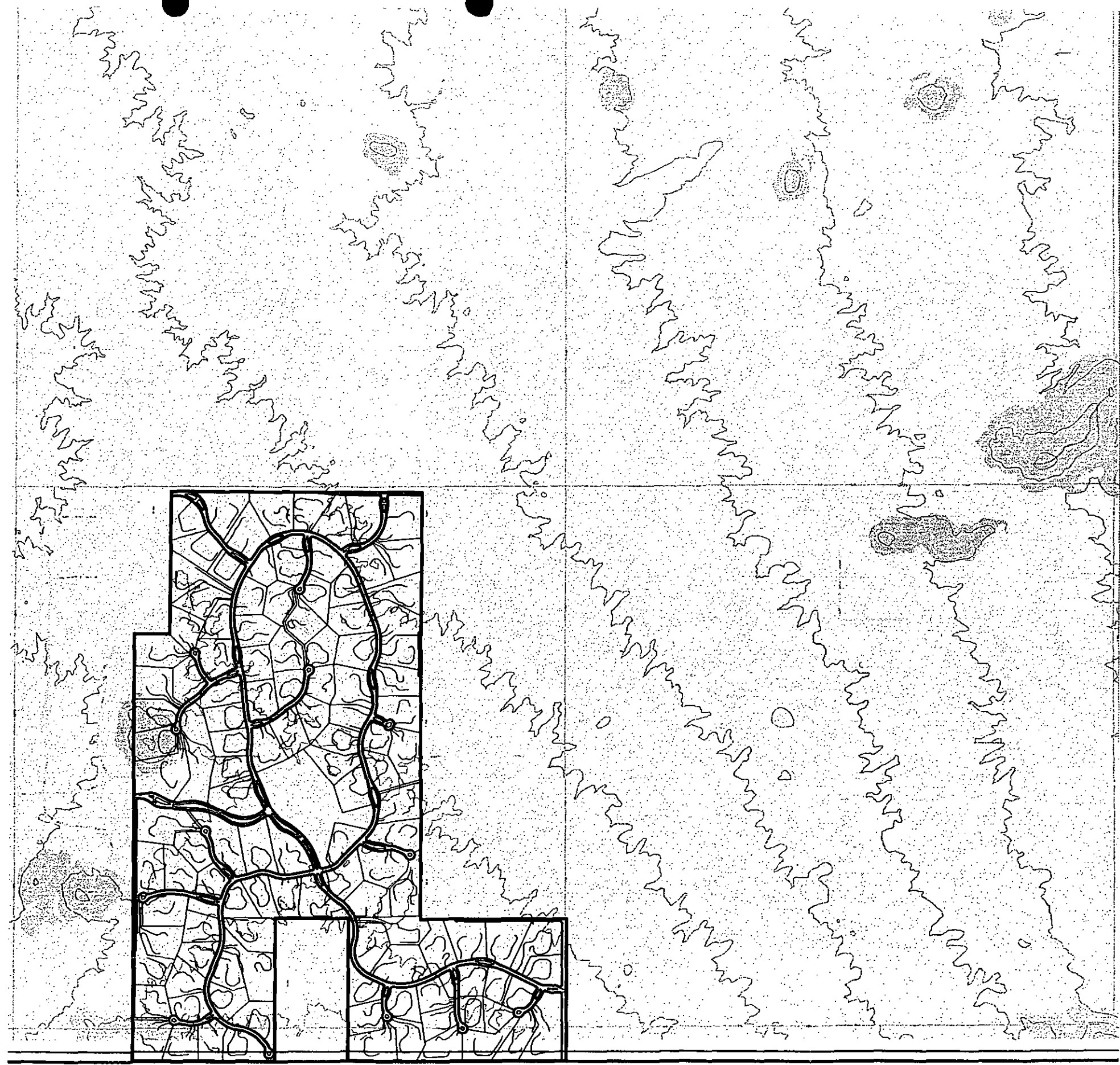
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PLATE 2

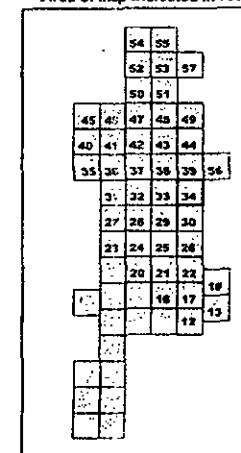
ESL Classification



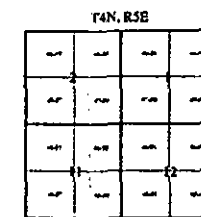
ESL Landforms and Protected Peaks and Ridges

- City Limits
- Section Boundary
- Streets
- Landforms
 - Lower Desert
 - Upper Desert
 - Hillside
- Elevation
 - 50' Contour
 - 20' Contour
- Protected Peaks and Ridges
 - Peak or Ridge Line

Area of Map Indicated in red



Official Detail Map
Map 34



Data Source: City of Scottsdale

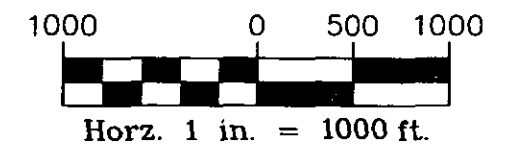


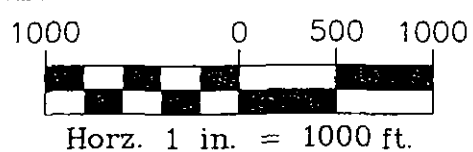
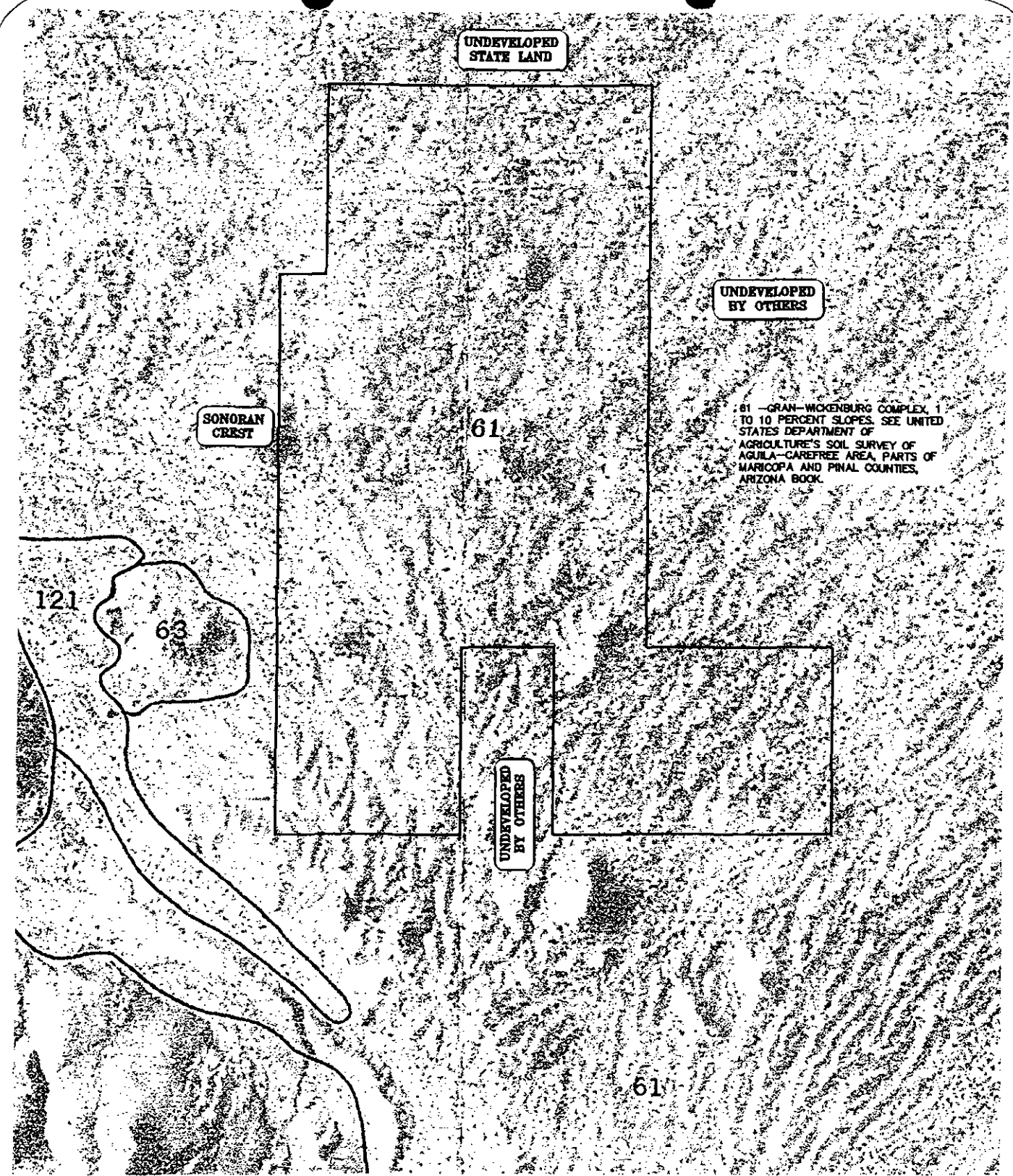
PLATE 2
ESL CLASSIFICATION



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PLATE 3

Soils Classification Map



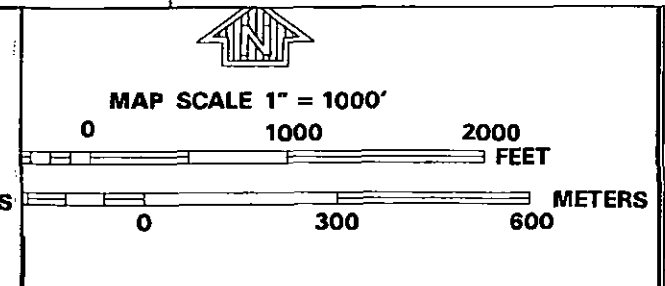
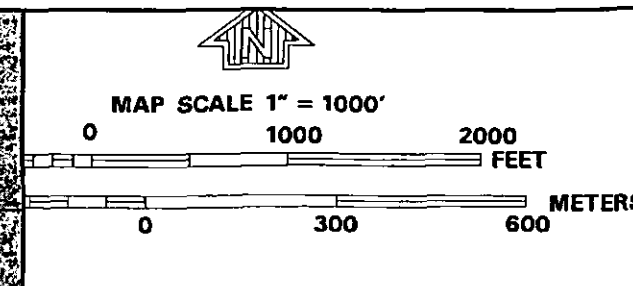
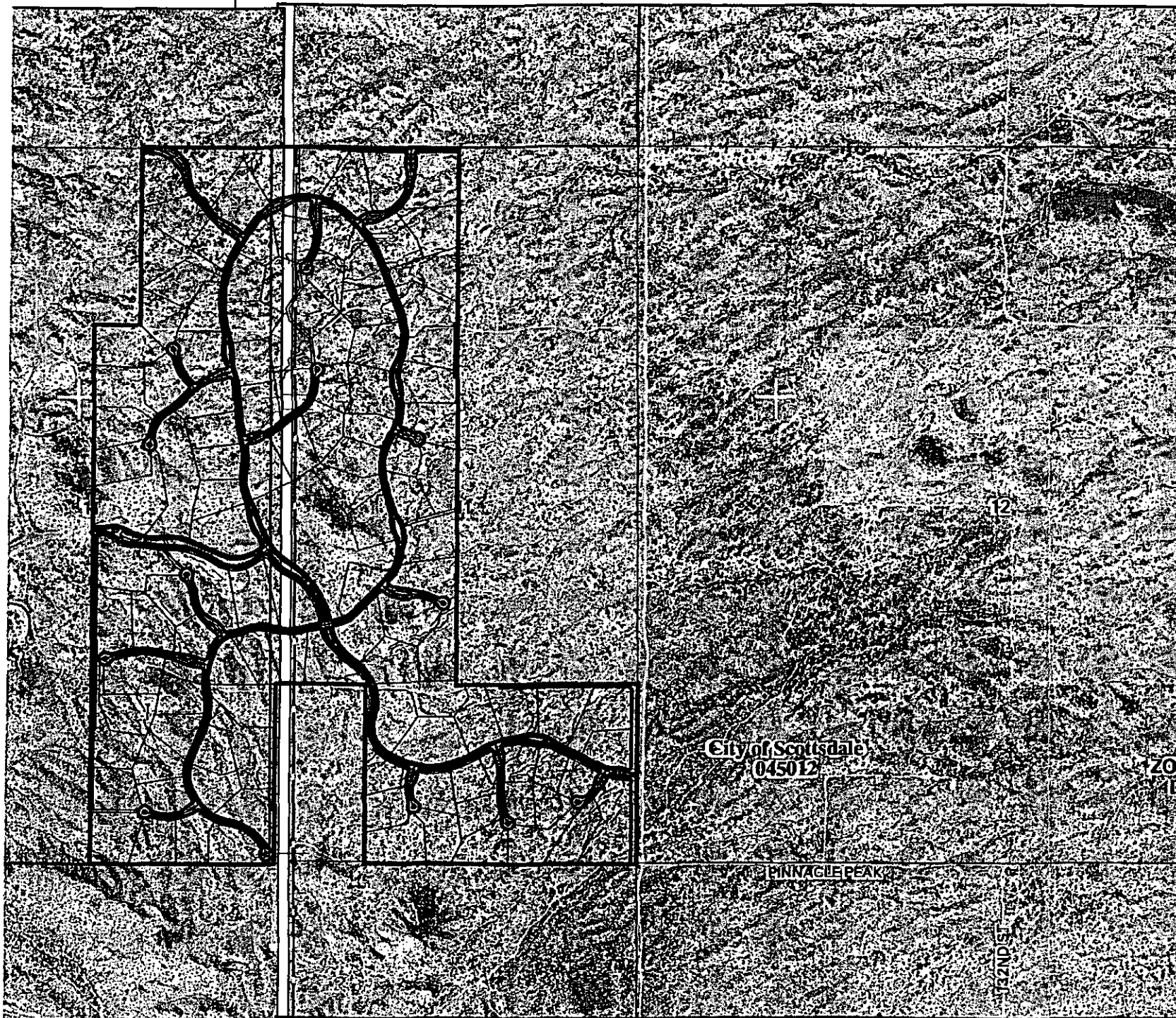
CROWN
COMMUNITY DEVELOPMENT
A Henry Crown Company

SERENO CANYON
Plate 3
"Soils Classification"

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WATER / WASTEWATER • SURVEYING
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PLATE 4

Flood Insurance Rate Map (FIRM)



NFIP

PANEL 1260F

**FIRM
FLOOD INSURANCE RATE MAP
MARICOPA COUNTY,
ARIZONA
AND INCORPORATED AREAS**

PANEL 1260 OF 4350
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
MARICOPA COUNTY	040037	1260	F
SCOTTSDALE, CITY OF	045012	1260	F

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.

 **MAP NUMBER
04013C1260F
MAP REVISED
SEPTEMBER 30, 2005**
Federal Emergency Management Agency

NFIP

PANEL 1255G

**FIRM
FLOOD INSURANCE RATE MAP
MARICOPA COUNTY,
ARIZONA
AND INCORPORATED AREAS**

PANEL 1255 OF 4350
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
SCOTTSDALE, CITY OF	045012	1255	G

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.

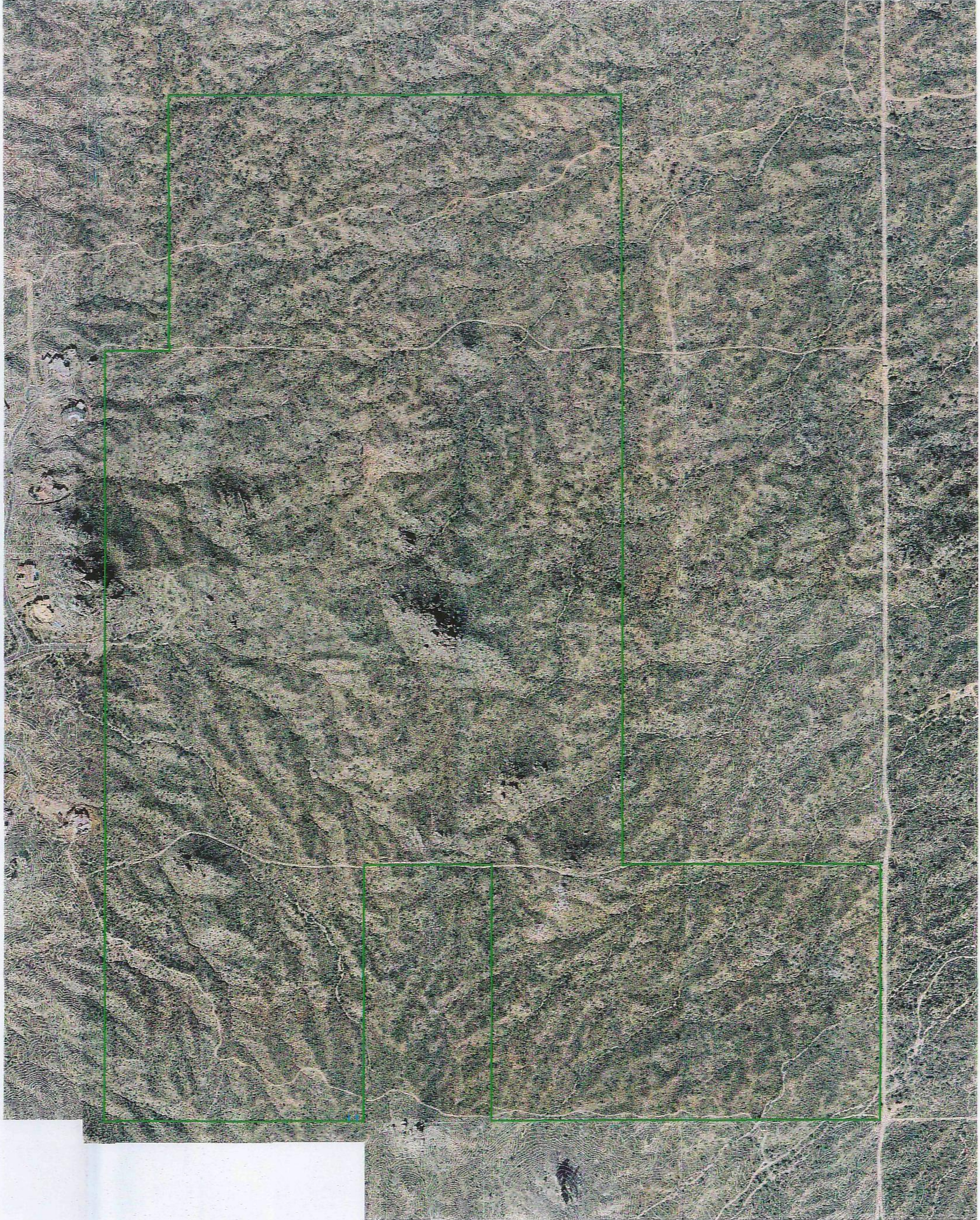
 **MAP NUMBER
04013C1255G
MAP REVISED
SEPTEMBER 30, 2005**
Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov

PLATE 5

Color Topographic Aerial Photograph



300 0 150 300
Horz. 1 in. = 300 ft.

 **CROWN**
COMMUNITY DEVELOPMENT
A Henry Crown Company

SERENO CANYON
Plate 5
"Topographic Aerial "

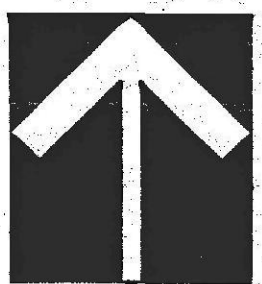
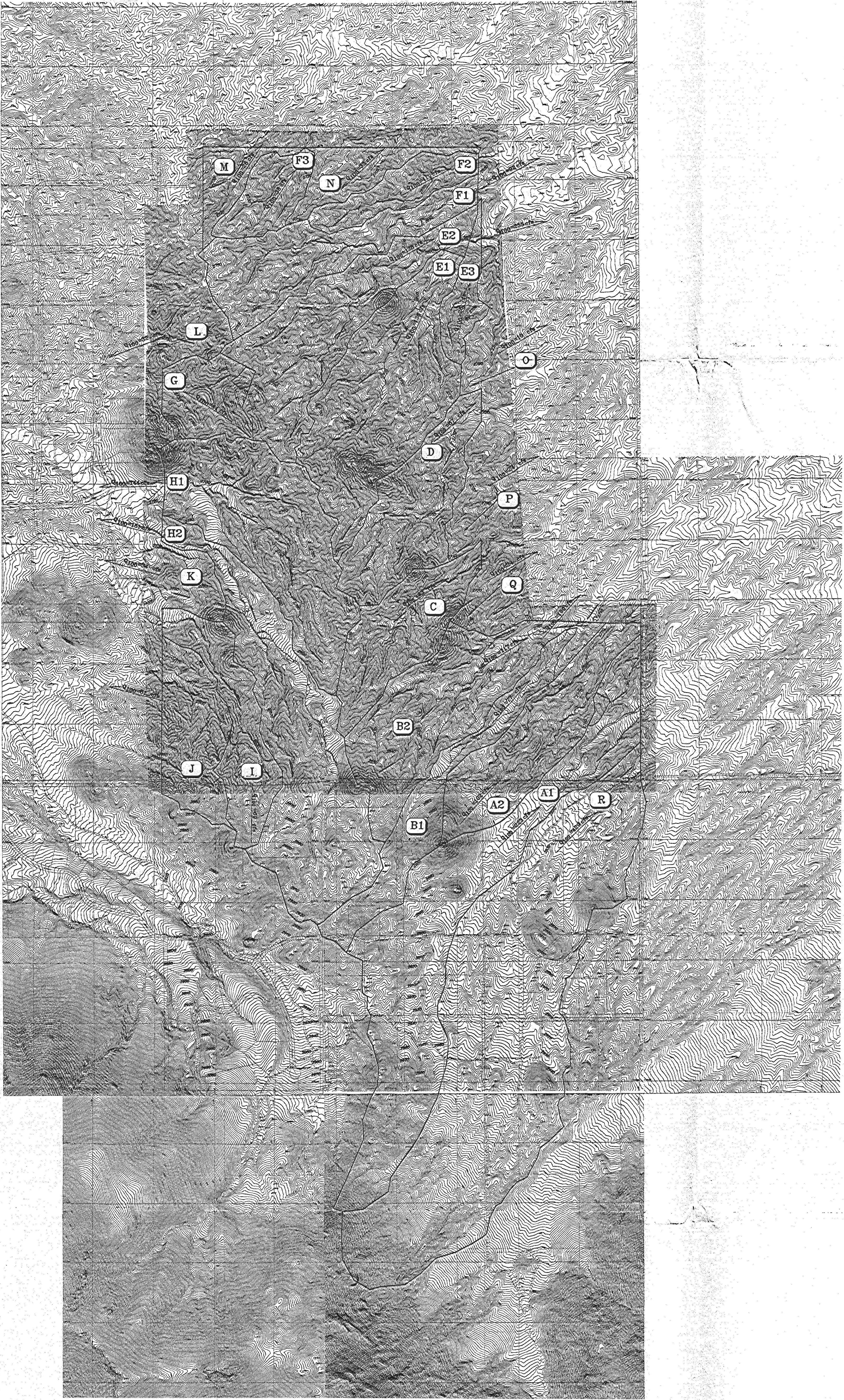
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PLATE 6

Off-Site Watershed Area Map



400 0 200 400
1 inch = 400 ft.

LEGEND



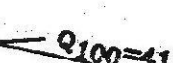
HEC-1 SUB-BASIN ID
AND CONCENTRATION POINT



SUB-BASIN BOUNDARY



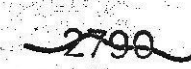
FLOWLINE



PRE-DEVELOPMENT PEAK
FLOW



PROJECT BOUNDARY



EXISTING CONTOUR
LABELS



FLOW ARROW

CROWN
COMMUNITY DEVELOPMENT
At Henry Crown Company

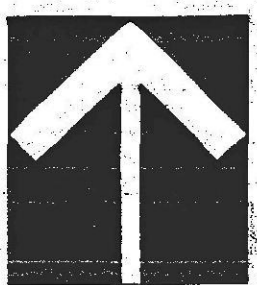
SERENO CANYON
Plate 6

"Off-Site Watershed Area Map"

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PLATE 7

Section 404 Washes



250 0 125 250
1 inch = 250 ft.

LEGEND

PRELIMINARY SECTION 404 JURISDICTIONAL
DELINEATION. A REQUEST FOR
VERIFICATION HAS BEEN SUBMITTED TO THE
A.C.O.E. FOR REVIEW AND APPROVAL.

PLATE 8

Pre-Development Drainage Site Plan

PLATE 9

Pre-Development Grading and Drainage Plan

PLATE 10

Post-Development Drainage Site Plan

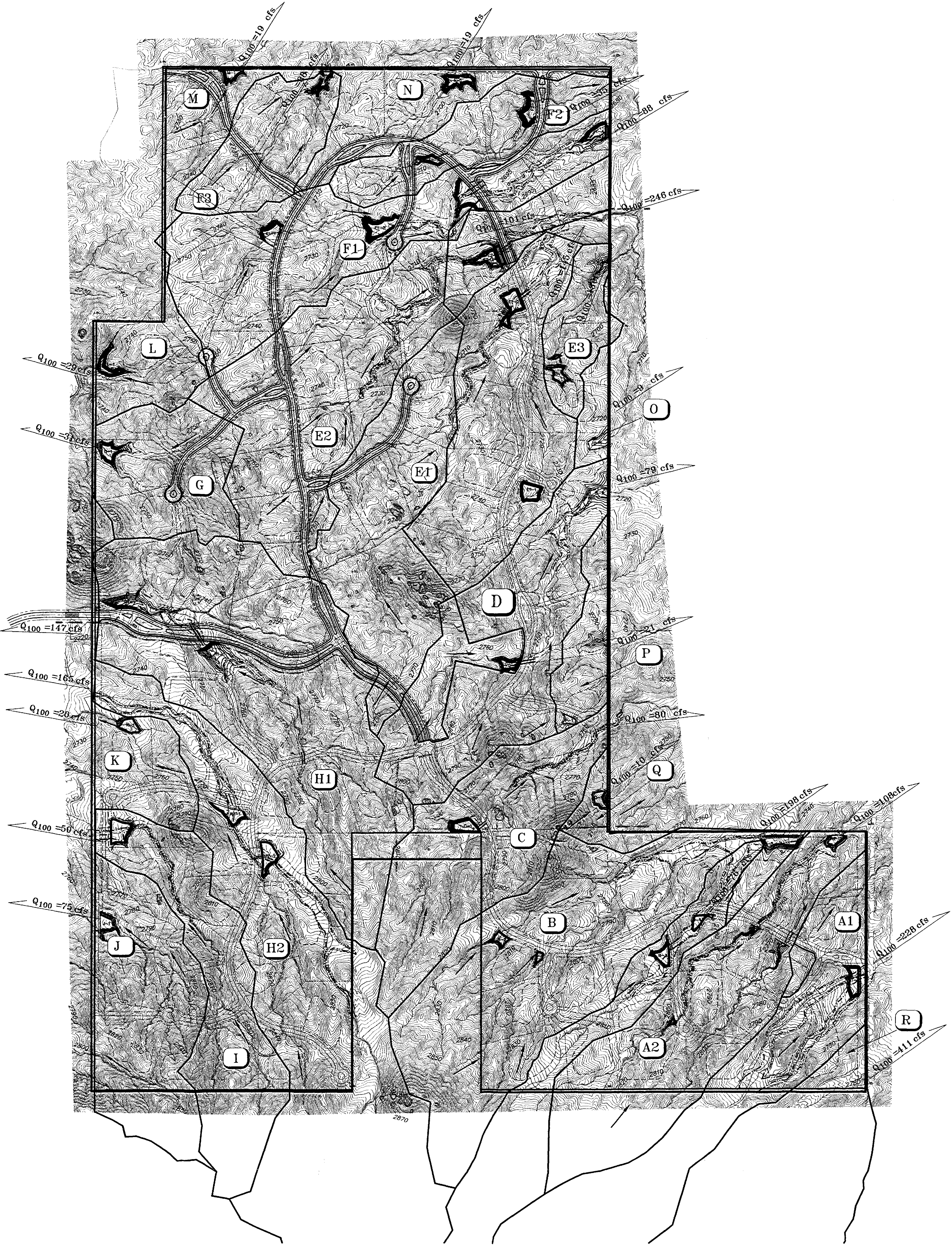
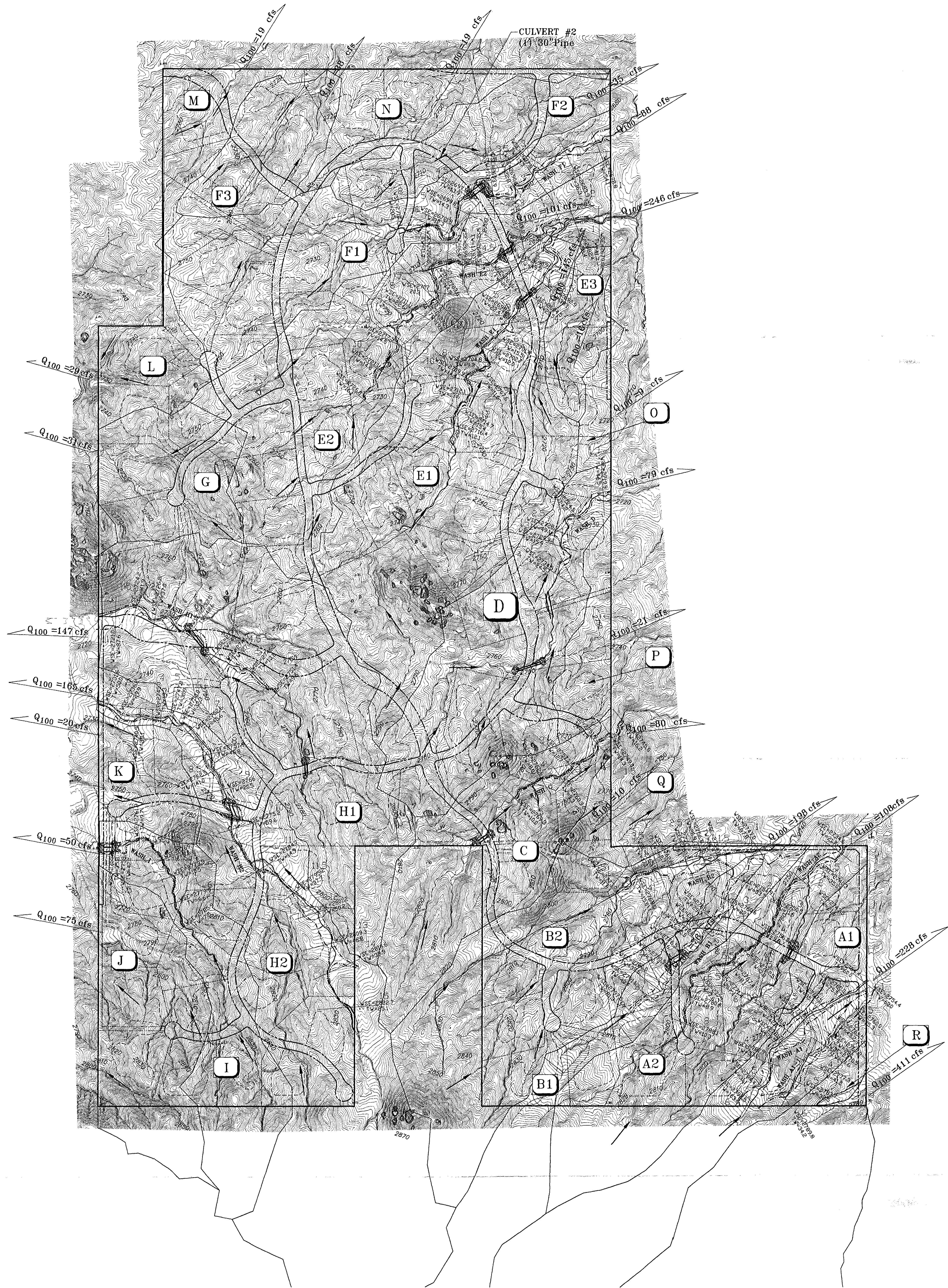


PLATE 11

Post-Development Grading and Drainage Plan



LEGEND

- | | | | |
|--|--|--|---|
| | HEC-1 SUB-BASIN ID AND CONCENTRATION POINT | | PROJECT BOUNDARY |
| | SUB-BASIN BOUNDARY | | EXISTING 1 FOOT CONTOUR LABEL |
| | FLOWLINE | | FLOW ARROW |
| | FLOODPLAIN BOUNDARY | | HEC-RAS CROSS SECTION WITH RIVER STATION, WATER SURFACE ELEVATION AND TOP WIDTH |
| | 404 WASH | | RIP RAP |
| | POST-DEVELOPMENT PEAK FLOW | | |
| | PROPOSED STORM DRAIN LOCATION | | |

