

DRAINAGE REPORTS

ABBREVEATED WATER & SEWER NEED REPORTS

WATER STUDY

WASTEWATER STUDY

STORMWATER WAIVER APPLICATION

■
Water Basis of Design Report

***Cavalliere Flats
80-Lot Proposed Subdivision***

Scottsdale, Arizona

Accepted for

**City of Scottsdale
Water Resources Administration
9379 E. San Salvador
Scottsdale, AZ 85258**

Prepared for:

Doug Mann 9.10.14
Taylor Morrison/Arizona, Inc.

Prepared by:

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191069012
August 2014
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Kimley»»Horn

**12-ZN-2014
9/8/2014**

Water Basis of Design Report

Cavalliere Flats 80-Lot Proposed Subdivision

Scottsdale, Arizona

Prepared for:

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9000 E. Pima Center Parkway, Suite 350
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1.0 INTRODUCTION

1.1 Intent

The purpose of this water report is to review the existing and proposed water system for the proposed Cavalliere Flats residential development located at the southeast corner of Alma School Parkway and Pinnacle Vista Drive in Scottsdale, Arizona. This report presents the basis of design criteria that will be used for the engineering design of the proposed development utilizing current water design standards and guidelines set forth by the City of Scottsdale, Arizona.

1.2 Project Description

The proposed project is located at the southeast corner of Alma School Parkway and Pinnacle Vista Drive in the City of Scottsdale, Arizona. It is more precisely described as a portion of the southeast quarter of Section 33, Township 5 North, Range 5 East, Maricopa County, Arizona. The proposed development consists of approximately 47 acres and is planned to include a 80-lot, single family residential subdivision. Refer to **Appendix A** for the Site Location Map.

2.0 DISTRIBUTION SYSTEM DESCRIPTION

2.1 Existing Distribution System

Per the City of Scottsdale Quarter Section Maps 49-53 and 49-54, three existing waterlines are located within Alma School Parkway to the west of the property. A 14-inch diameter asbestos concrete pipe (ACP), as well as 12-inch diameter and 20-inch diameter ductile iron pipe (DIP) mains. An 8-inch diameter to 12-inch diameter C900 Polyvinyl Chloride (PVC) waterline is located in Pinnacle Vista Drive to the north of the site. Additionally, to the west of the site within 111th Street and Bajada Drive is an 8-inch diameter C900 PVC waterline. To the south of the site exists a 12-inch ACP waterline within Jomax Road.

The subject site lies near the boundary of two separate water pressure zones and ranges in existing ground elevation from 2545 feet to 2600 feet, City of Scottsdale Datum. According to Figure 6.1-3 of the City of Scottsdale Design Standards and Policies Manual (DS&PM), the site is located within the south portion of Pressure Zone 12. The existing waterlines adjacent to the site appear to be operated in Pressure Zone 12. With the exception of the line within Jomax road, which appears to be located within Pressure Zone 11.

2.2 Proposed Distribution System

The proposed distribution system will consist of an 8" Class 350 DIP water line that will provide potable water and fire protection. The distribution system will connect to the existing 12-inch DIP (Zone 12) water main located in Alma School Parkway and loop through the project site and tie into the existing 12-inch ACP (Zone 11) water main in Jomax Road. The system will also connect to the existing 8-inch PVC (Zone 12) waterline in Bajada Drive. The system spans two pressure zones and will require a pressure reducing valve (PRV). A PRV is proposed along the waterline near lot 57. The north portion of the project will operate under Zone 12, while the south portion will operate under Zone 11. Refer to **Appendix C** for the Proposed Water System Layout Exhibit.

3.0 BASIS OF DESIGN

3.1 Design Methodology

The WaterCAD v8i water system modeling software distributed by Haestad Methods, Inc. was used to model the proposed water network. Three flow tests were performed to determine the residual and static pressures of the existing system. Flow Test #1 was taken from hydrants connected to Pressure Zone 11 along Jomax Road to the south of the site. Flow Test #2 was taken for hydrants along Bajada Drive operating in Pressure Zone 12. Flow Test #3 was taken from hydrants connected to Pressure Zone 12 along Pinnacle Drive to the north of the site. Refer to **Appendix B** for the results of the fire flow tests. According to the results of these flow tests, the maximum operating flows at 20 psi were calculated to be 4,276 gpm, 2,055 gpm and 3,059 gpm respectively.

According to Section 6-1.407 of the DSPM, distribution systems shall be designed with a minimum residual pressure of 50 psi and a maximum static pressure of 120 psi. In fire flow scenarios a minimum design pressure of 30 psi is required.

3.2 Water System Analysis

The proposed water distribution system for the project is modeled under 4 design scenarios. Average Day, Max Day, Peak Hour and Max Day plus Fire Flows scenarios are modeled. Average Day Demands are based on Figure 6.1-2 in the DS&PM, with peaking factors per section 6-1.404. A fire flow of 1,000 gpm per section 6-1.501 of the DS&PM was used. See the table below for a summary of water demands:

Water Demands								
Land Use	Dwelling units (du)	Average Daily Demand (gpd/du)	Average Daily Flow (gpd)	ADF (gpm)	Max Day Flow (gpd)	MDF (gpm)	Peak Hour Flow (gpd)	PHF (gpm)
<2 du/ac	80	485.6	38,848	27	77,696	54	135,968	94

Average Day, Max Day, and Peak Hour Demands are applied at hydraulic model nodes based on number of adjacent proposed units. Fire flow demands are applied to all junctions within the project boundary.

3.3 Results

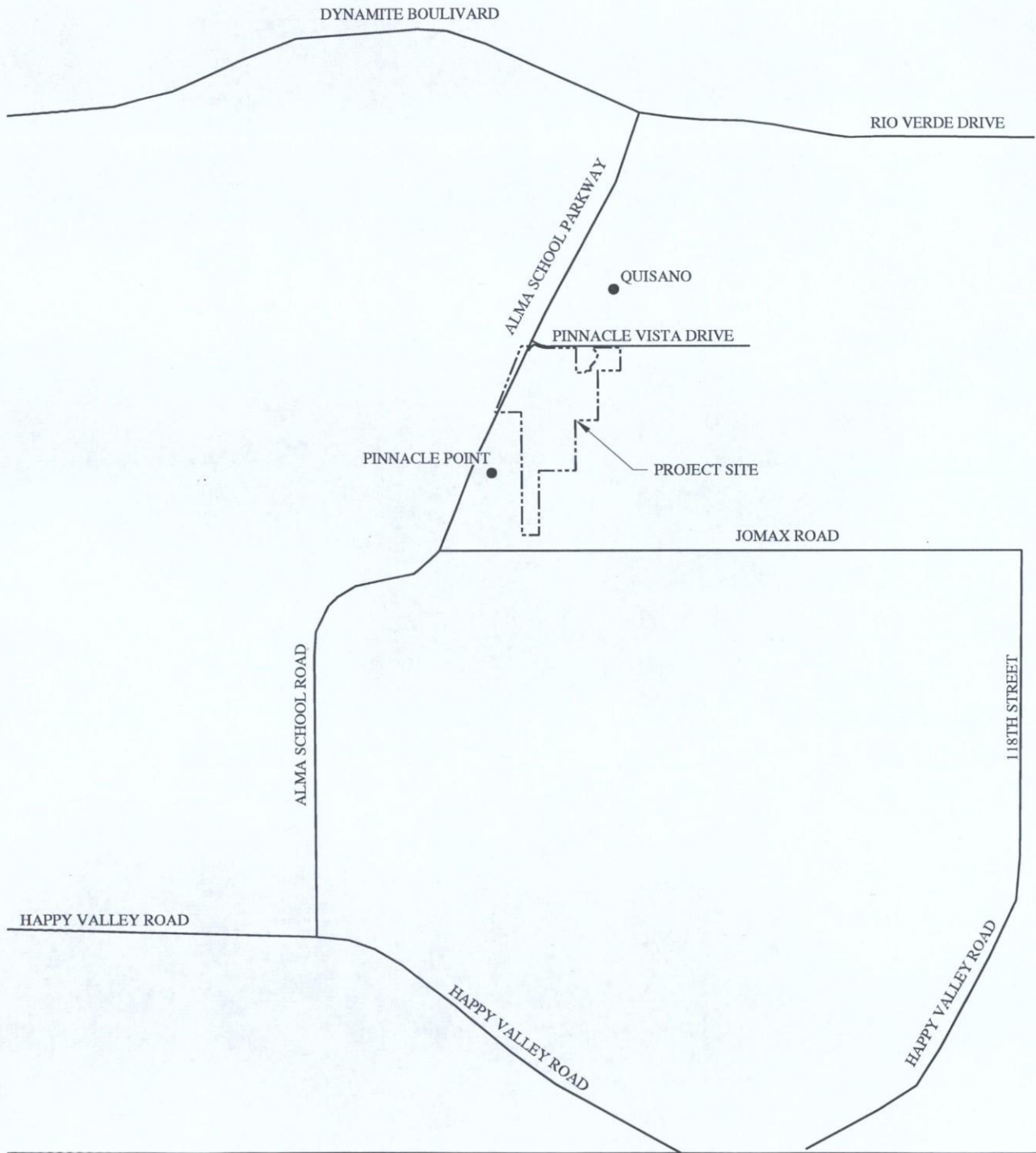
Results from the hydraulic water model indicated the system is able to supply the calculated demands at or above the minimum required pressures. The proposed PRV is set at an outflow pressure of 85 psi. This is an initial setting and may be adjusted as required by the city.

Refer to **Appendix D** for the WaterCAD node and fire flow reports.

Based on the fire flow tests performed and the results of the WaterCAD analysis, the water system appears adequate and sufficient to meet the fire flow and domestic demands of the Cavalliere Flats residential subdivision. Individual unit PRV's are required on all single family residential units.

APPENDIX A
SITE LOCATION MAP

K:\V\H\CA\181008012 - Greenwood\Reports\Water\03\Exhibit\Locality Map.dwg May 19, 2014 zash.jll
XREF: 200725W 200725W 200725P



APPENDIX B
FIRE FLOW TEST RESULTS



ALLIANCE FIRE PROTECTION CO.

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 2114 East Cedar Street • Tempe, Arizona 85281
 E-mail Address: afpc@afpc.com

AZ Lic. C-16 58130
 AZ Lic. L-16 74007
 NV Lic. C-41a 30135

FIRE HYDRANT FLOW TEST

Name: Kimley-Horn
Alma School & Lomax
Scottsdale Arizona

Date: 02/14/14
 Time: 8:00 AM
 Report # _____
 Tech: R.Pfeiff

Static Hydrant: 100 yards east of Windy Walk Dr. on north side of Jomax Flowing Hydrant: 200 yards east of Alma School on north side of Jomax

Elevation: 2596

Elevation: 2523

Dist. Between Hydrants: 500 yards

Type of Supply: CITY MAIN

Diameter of Main: _____

Static Pressure:

A	54.0	B
---	------	---

Residual Pressure:

A	40.0	B
---	------	---

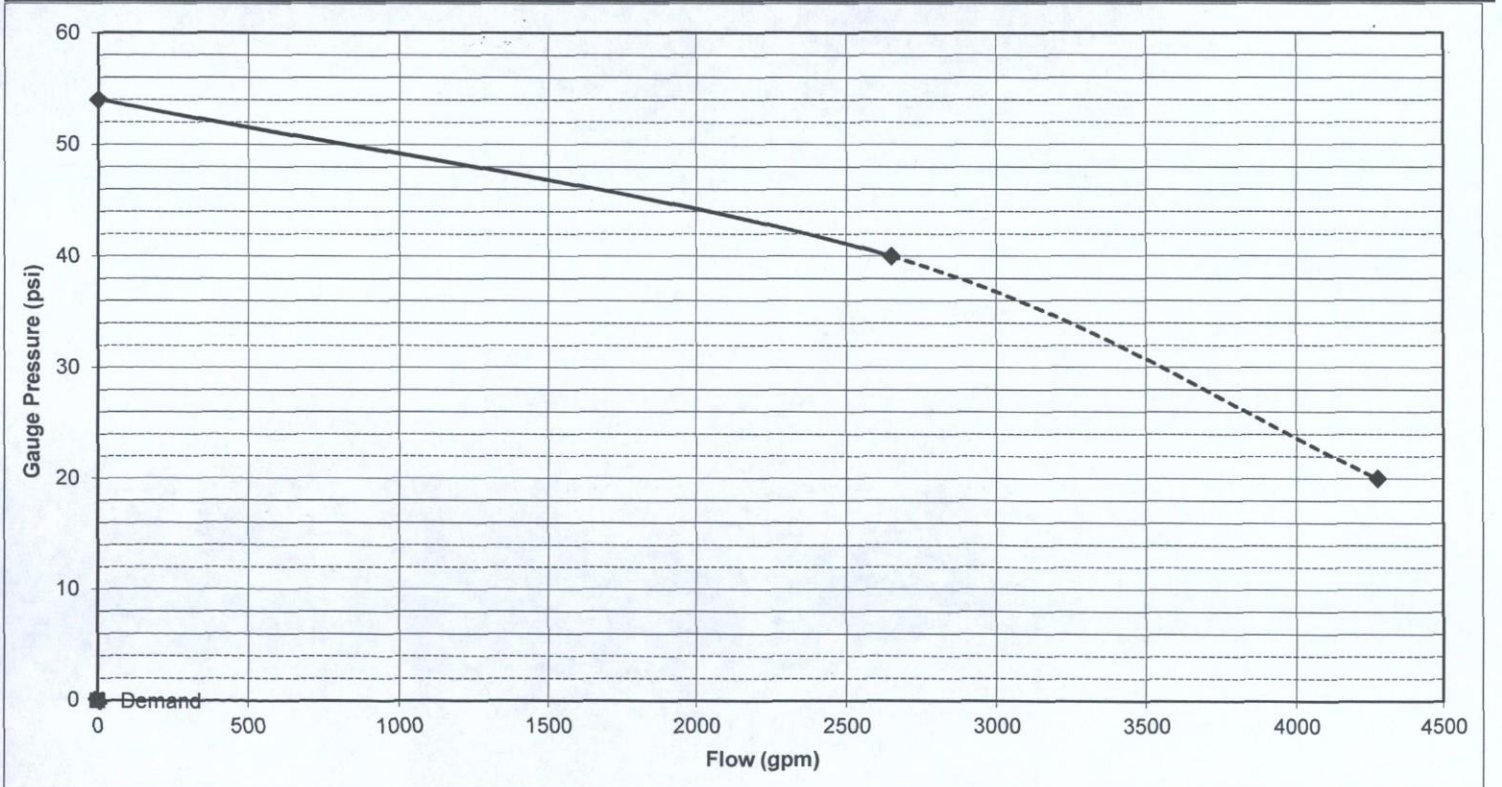
Pump Present: NO

Tank Present: NO

Req. GPM: _____ Req. PSI: _____

Hydrant:	A	A	B	B
Outlet Diameter:	4.0			
Pitot Reading:	38.0			
Coeff:	0.90			
Discharge GPM:	2648	0	0	0

	Flow A		Flow B	
Static pressure of	54	psi @ 0 gpm	0	psi @ 0 gpm
Residual pressure of	40	psi @ 2648 gpm	0	psi @ 0 gpm
Available flow @	20	psi @ 4276 gpm	20	psi @ gpm



Comments: Elevations are approximate

NOTES:

1. Flowing hydrant is assumed to be on a circulating main or downstream of the pressure test hydrant on a dead-end system.
2. Flow analysis assumes a gravity flow system with no distribution pumps and having no demand, other than the test
3. The distance between hydrants, elevations & main diameters are for information only.



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FIRE HYDRANT FLOW TEST

Name: Kimley-Horn
111th Street & Bajada
Scottsdale Arizona

Date: 02/14/14
 Time: 8:15 AM
 Report # _____
 Tech: R.Pfeiff

Static Hydrant: 150 yards west of 111th St. on Flowing Hydrant: 300 yards west of 111th St. on
South side of Bajada South side of Bajada

Elevation: 2602

Elevation: 2581

Dist. Between Hydrants: 150 yards

Type of Supply: CITY MAIN

Diameter of Main: _____

Hydrant:	A	A	B	B
Outlet Diameter:	4.0			
Pitot Reading:	16.0			
Coeff:	0.90			
Discharge GPM:	1718	0	0	0

Static Pressure:

A	105.0	B
---	-------	---

Residual Pressure:

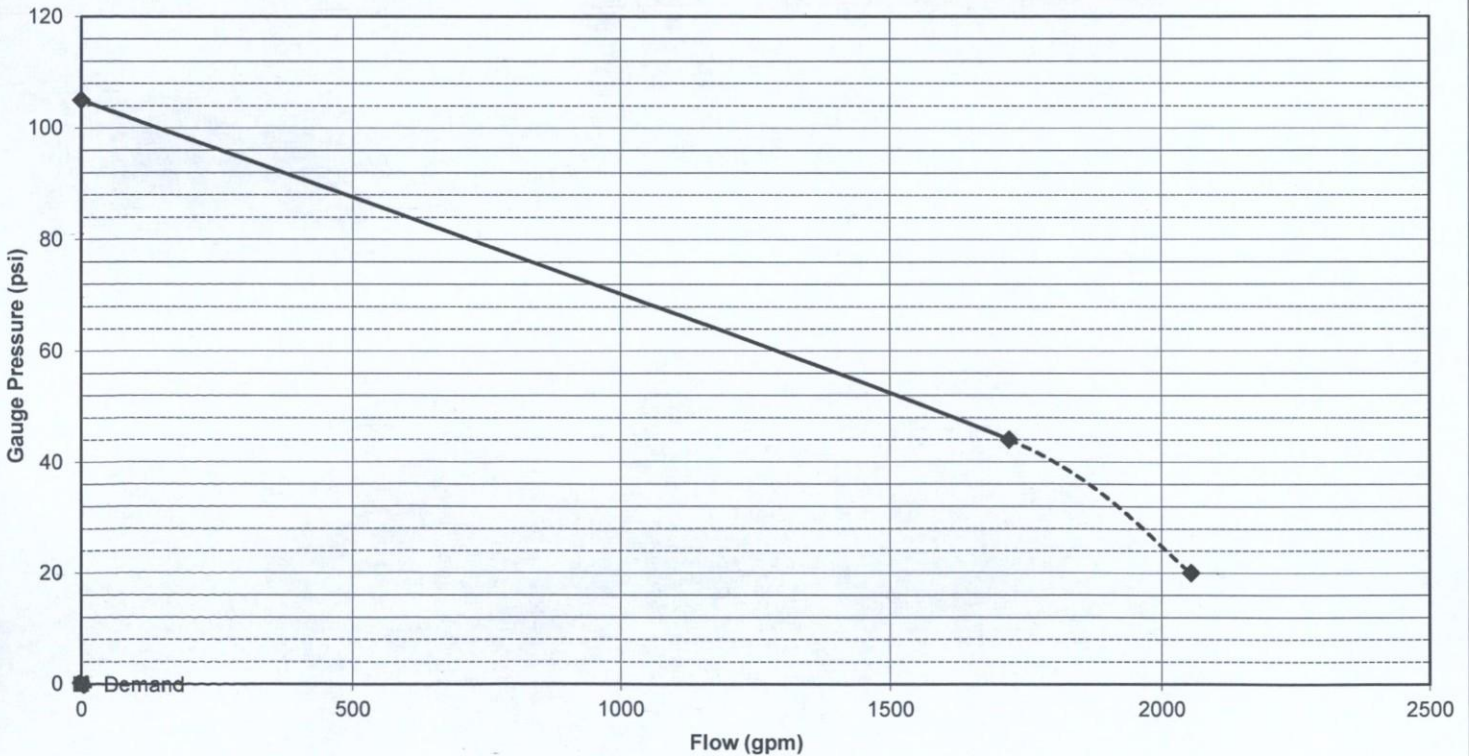
A	44.0	B
---	------	---

Pump Present: NO

Tank Present: NO

Req. GPM: _____ Req. PSI: _____

	Flow A	Flow B
Static pressure of	105 psi @ 0 gpm	0 psi @ 0 gpm
Residual pressure of	44 psi @ 1718 gpm	0 psi @ 0 gpm
Available flow @	20 psi @ 2055 gpm	20 psi @ 0 gpm



Comments: Elevations are approximate

NOTES:

1. Flowing hydrant is assumed to be on a circulating main or downstream of the pressure test hydrant on a dead-end system.
2. Flow analysis assumes a gravity flow system with no distribution pumps and having no demand, other than the test
3. The distance between hydrants, elevations & main diameters are for information only.



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 NV Lic. C-41a 30135

FIRE HYDRANT FLOW TEST

Name: Kimley-Horn
Alma School & Pinnacle Vista
Scottsdale Arizona

Date: 02/14/14
 Time: 8:15 AM
 Report # _____
 Tech: R.Pfeiff

Static Hydrant: 100 yards east of Alma School on
North side of Pinnacle Vista

Flowing Hydrant: 500 yards east of Alma School on
North side of Pinnacle Vista

Elevation: 2616

Elevation: 2592

Dist. Between Hydrants: 400 yards

Type of Supply: CITY MAIN

Diameter of Main: _____

Static Pressure:

A	98.0	B
---	------	---

Residual Pressure:

A	84.0	B
---	------	---

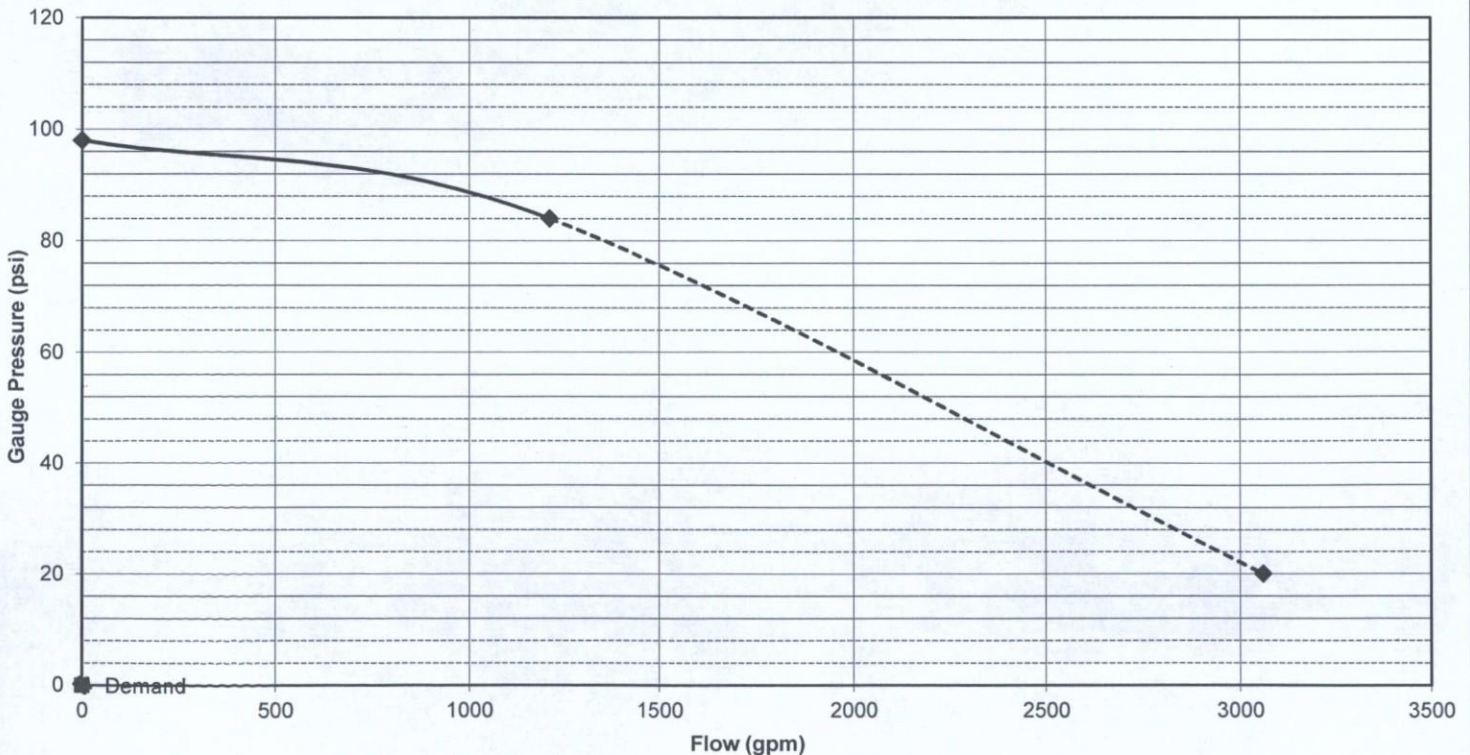
Pump Present: NO

Tank Present: NO

Req. GPM: _____ Req. PSI: _____

Hydrant:	A	A	B	B
Outlet Diameter:	2.5			
Pitot Reading:	52.0			
Coeff:	0.90			
Discharge GPM:	1210	0	0	0

	Flow A		Flow B	
Static pressure of	98 psi @	0 gpm	0 psi @	0 gpm
Residual pressure of	84 psi @	1210 gpm	0 psi @	0 gpm
Available flow @	20 psi @	3059 gpm	20 psi @	gpm



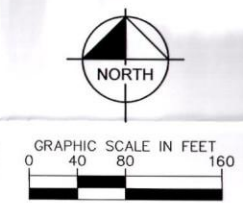
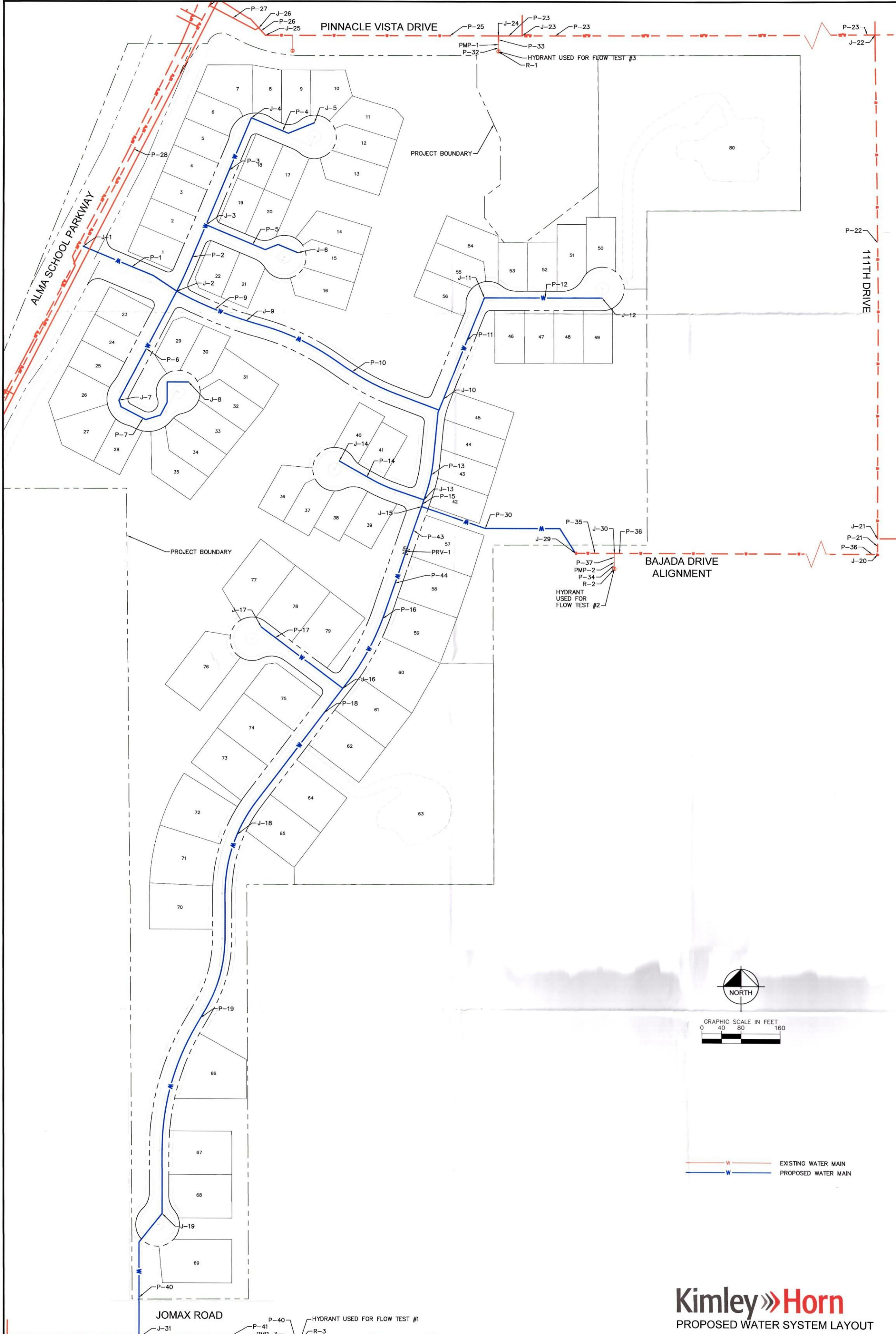
Comments: Elevations are approximate

NOTES:

1. Flowing hydrant is assumed to be on a circulating main or downstream of the pressure test hydrant on a dead-end system.
2. Flow analysis assumes a gravity flow system with no distribution pumps and having no demand, other than the test
3. The distance between hydrants, elevations & main diameters are for information only.

APPENDIX C

PROPOSED WATER SYSTEM LAYOUT



— W — EXISTING WATER MAIN
 — W — PROPOSED WATER MAIN

APPENDIX D
WATERCAD ANALYSIS RESULTS

FlexTable: Junction Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Average Day

Current Time: 0.000 hours

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)	Zone
J-1	2,591.00	0	2,846.02	110.3	Zone 12
J-2	2,581.00	0	2,846.01	114.7	Zone 12
J-3	2,586.00	2	2,846.01	112.5	Zone 12
J-4	2,591.00	2	2,846.01	110.3	Zone 12
J-5	2,585.00	2	2,846.01	112.9	Zone 12
J-6	2,580.00	2	2,846.01	115.1	Zone 12
J-7	2,578.00	2	2,846.01	116.0	Zone 12
J-8	2,570.00	2	2,846.01	119.4	Zone 12
J-9	2,576.00	0	2,846.01	116.8	Zone 12
J-10	2,575.00	1	2,846.01	117.3	Zone 12
J-11	2,576.00	2	2,846.01	116.8	Zone 12
J-12	2,583.00	2	2,846.01	113.8	Zone 12
J-13	2,569.00	1	2,846.01	119.9	Zone 12
J-14	2,568.00	2	2,846.01	120.3	Zone 12
J-15	2,569.00	0	2,846.01	119.9	Zone 12
J-16	2,557.00	2	2,762.53	88.9	Zone 11
J-17	2,558.00	2	2,762.53	88.5	Zone 11
J-18	2,548.00	3	2,762.53	92.8	Zone 11
J-19	2,535.00	1	2,762.53	98.4	Zone 11
J-20	2,620.00	0	2,846.02	97.8	Zone 12
J-21	2,622.00	0	2,846.02	96.9	Zone 12
J-22	2,635.00	0	2,846.02	91.3	Zone 12
J-23	2,618.00	0	2,846.02	98.7	Zone 12
J-24	2,616.00	0	2,846.02	99.5	Zone 12
J-25	2,592.00	0	2,846.02	109.9	Zone 12
J-26	2,593.00	0	2,846.02	109.5	Zone 12
J-27	2,598.00	0	2,846.02	107.3	Zone 12
J-29	2,582.00	0	2,846.01	114.2	Zone 12
J-30	2,590.00	0	2,846.01	110.8	Zone 12
J-31	2,533.00	0	2,762.53	99.3	Zone 11

FlexTable: Pipe Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Average Day

Current Time: 0.000 hours

Label	Length (Scaled) (ft)	Start Node	Stop Node	Diameter (in)	Hazen- Williams C	Flow (gpm)	Velocity (ft/s)	Headloss Gradient (ft/ft)
P-1	215	J-1	J-2	8.0	130.0	17	0.11	0.000
P-2	151	J-2	J-3	8.0	130.0	7	0.05	0.000
P-3	240	J-3	J-4	8.0	130.0	3	0.02	0.000
P-4	139	J-4	J-5	8.0	130.0	2	0.01	0.000
P-5	199	J-3	J-6	8.0	130.0	2	0.01	0.000
P-6	256	J-2	J-7	8.0	130.0	4	0.03	0.000
P-7	154	J-7	J-8	8.0	130.0	2	0.02	0.000
P-9	152	J-9	J-2	8.0	130.0	-5	0.03	0.000
P-10	449	J-9	J-10	8.0	130.0	5	0.03	0.000
P-11	226	J-10	J-11	8.0	130.0	4	0.03	0.000
P-12	240	J-11	J-12	8.0	130.0	2	0.02	0.000
P-13	213	J-10	J-13	8.0	130.0	1	0.00	0.000
P-14	193	J-13	J-14	8.0	130.0	2	0.01	0.000
P-15	85	J-13	J-15	8.0	130.0	-2	0.02	0.000
P-17	245	J-16	J-17	8.0	130.0	2	0.01	0.000
P-18	337	J-16	J-18	8.0	130.0	4	0.03	0.000
P-19	846	J-18	J-19	8.0	130.0	1	0.01	0.000
P-21	33	J-20	J-21	8.0	130.0	-10	0.06	0.000
P-22	1,042	J-21	J-22	8.0	130.0	-10	0.06	0.000
P-23	1,209	J-22	J-23	12.0	130.0	-10	0.03	0.000
P-24	50	J-23	J-24	12.0	130.0	-10	0.03	0.000
P-25	477	J-24	J-25	8.0	130.0	17	0.11	0.000
P-26	20	J-25	J-26	8.0	130.0	17	0.11	0.000
P-27	113	J-26	J-27	8.0	130.0	17	0.11	0.000
P-28	562	J-27	J-1	12.0	130.0	17	0.05	0.000
P-30	174	J-15	J-29	8.0	130.0	-10	0.06	0.000
P-32	22	PMP-1	R-1	48.0	130.0	-27	0.00	0.000
P-33	30	PMP-1	J-24	48.0	130.0	27	0.00	0.000
P-34	37	R-2	PMP-2	48.0	130.0	0	0.00	0.000
P-35	250	J-29	J-30	8.0	130.0	-10	0.06	0.000
P-36	1,023	J-30	J-20	8.0	130.0	-10	0.06	0.000
P-37	36	PMP-2	J-30	48.0	130.0	0	0.00	0.000
P-40	195	J-19	J-31	6.0	130.0	0	0.00	0.000
P-41	513	J-31	PMP-3	8.0	130.0	(N/A)	(N/A)	(N/A)
P-42	93	PMP-3	R-3	6.0	130.0	(N/A)	(N/A)	(N/A)
P-43	97	J-15	PRV-1	8.0	130.0	7	0.05	0.000
P-44	244	PRV-1	J-16	8.0	130.0	7	0.05	0.000

FlexTable: PRV Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Average Day

Current Time: 0.000 hours

Label	Elevation (ft)	Pressure (From) (psi)	Pressure (To) (psi)	Flow (gpm)	Headloss (ft)	Zone
PRV-1	2,566.00	121.1	85.0	7	83.48	Zone 11

FlexTable: Pump Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Average Day

Current Time: 0.000 hours

Label	Elevation (ft)	Hydraulic Grade (Suction) (ft)	Hydraulic Grade (Discharge) (ft)	Flow (Total) (gpm)	Pump Head (ft)	Flow (Design) (gpm)	Head (Design) (ft)	Zone
PMP-1	2,616.00	2,620.00	2,846.02	27	226.02	1,210	193.75	Zone 12
PMP-2	2,590.00	2,592.00	2,846.01	0	0.00	1,718	101.49	Zone 12
PMP-3	2,596.00	(N/A)	(N/A)	(N/A)	(N/A)	2,648	92.40	Zone 11

FlexTable: Reservoir Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Average Day

Current Time: 0.000 hours

Label	Elevation (ft)	Zone	Flow (Out net) (gpm)	Hydraulic Grade (ft)
R-1	2,620.00	Zone 12	27	2,620.00
R-2	2,592.00	Zone 12	0	2,592.00
R-3	2,600.00	Zone 11	(N/A)	(N/A)

FlexTable: Junction Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Max Day

Current Time: 0.000 hours

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)	Zone
J-1	2,591.00	0	2,845.92	110.3	Zone 12
J-2	2,581.00	0	2,845.92	114.6	Zone 12
J-3	2,586.00	5	2,845.92	112.5	Zone 12
J-4	2,591.00	3	2,845.91	110.3	Zone 12
J-5	2,585.00	3	2,845.91	112.9	Zone 12
J-6	2,580.00	3	2,845.92	115.0	Zone 12
J-7	2,578.00	4	2,845.92	115.9	Zone 12
J-8	2,570.00	5	2,845.92	119.4	Zone 12
J-9	2,576.00	0	2,845.92	116.8	Zone 12
J-10	2,575.00	1	2,845.91	117.2	Zone 12
J-11	2,576.00	3	2,845.91	116.8	Zone 12
J-12	2,583.00	5	2,845.91	113.8	Zone 12
J-13	2,569.00	2	2,845.91	119.8	Zone 12
J-14	2,568.00	4	2,845.91	120.2	Zone 12
J-15	2,569.00	0	2,845.91	119.8	Zone 12
J-16	2,557.00	3	2,762.53	88.9	Zone 11
J-17	2,558.00	3	2,762.53	88.5	Zone 11
J-18	2,548.00	5	2,762.53	92.8	Zone 11
J-19	2,535.00	3	2,762.53	98.4	Zone 11
J-20	2,620.00	0	2,845.93	97.8	Zone 12
J-21	2,622.00	0	2,845.93	96.9	Zone 12
J-22	2,635.00	0	2,845.95	91.3	Zone 12
J-23	2,618.00	0	2,845.95	98.6	Zone 12
J-24	2,616.00	0	2,845.95	99.5	Zone 12
J-25	2,592.00	0	2,845.93	109.9	Zone 12
J-26	2,593.00	0	2,845.93	109.4	Zone 12
J-27	2,598.00	0	2,845.93	107.3	Zone 12
J-29	2,582.00	0	2,845.92	114.2	Zone 12
J-30	2,590.00	0	2,845.92	110.7	Zone 12
J-31	2,533.00	0	2,762.53	99.3	Zone 11

FlexTable: Pipe Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Max Day

Current Time: 0.000 hours

Label	Length (Scaled) (ft)	Start Node	Stop Node	Diameter (in)	Hazen- Williams C	Flow (gpm)	Velocity (ft/s)	Headloss Gradient (ft/ft)
P-1	215	J-1	J-2	8.0	130.0	34	0.22	0.000
P-2	151	J-2	J-3	8.0	130.0	15	0.09	0.000
P-3	240	J-3	J-4	8.0	130.0	7	0.04	0.000
P-4	139	J-4	J-5	8.0	130.0	3	0.02	0.000
P-5	199	J-3	J-6	8.0	130.0	3	0.02	0.000
P-6	256	J-2	J-7	8.0	130.0	9	0.06	0.000
P-7	154	J-7	J-8	8.0	130.0	5	0.03	0.000
P-9	152	J-9	J-2	8.0	130.0	-11	0.07	0.000
P-10	449	J-9	J-10	8.0	130.0	11	0.07	0.000
P-11	226	J-10	J-11	8.0	130.0	8	0.05	0.000
P-12	240	J-11	J-12	8.0	130.0	5	0.03	0.000
P-13	213	J-10	J-13	8.0	130.0	-1	0.01	0.000
P-14	193	J-13	J-14	8.0	130.0	4	0.03	0.000
P-15	85	J-13	J-15	8.0	130.0	-5	0.03	0.000
P-17	245	J-16	J-17	8.0	130.0	3	0.02	0.000
P-18	337	J-16	J-18	8.0	130.0	8	0.05	0.000
P-19	846	J-18	J-19	8.0	130.0	3	0.02	0.000
P-21	33	J-20	J-21	8.0	130.0	-20	0.13	0.000
P-22	1,042	J-21	J-22	8.0	130.0	-20	0.13	0.000
P-23	1,209	J-22	J-23	12.0	130.0	-20	0.06	0.000
P-24	50	J-23	J-24	12.0	130.0	-20	0.06	0.000
P-25	477	J-24	J-25	8.0	130.0	34	0.22	0.000
P-26	20	J-25	J-26	8.0	130.0	34	0.22	0.000
P-27	113	J-26	J-27	8.0	130.0	34	0.22	0.000
P-28	562	J-27	J-1	12.0	130.0	34	0.10	0.000
P-30	174	J-15	J-29	8.0	130.0	-20	0.13	0.000
P-32	22	PMP-1	R-1	48.0	130.0	-54	0.01	0.000
P-33	30	PMP-1	J-24	48.0	130.0	54	0.01	0.000
P-34	37	R-2	PMP-2	48.0	130.0	0	0.00	0.000
P-35	250	J-29	J-30	8.0	130.0	-20	0.13	0.000
P-36	1,023	J-30	J-20	8.0	130.0	-20	0.13	0.000
P-37	36	PMP-2	J-30	48.0	130.0	0	0.00	0.000
P-40	195	J-19	J-31	6.0	130.0	0	0.00	0.000
P-41	513	J-31	PMP-3	8.0	130.0	(N/A)	(N/A)	(N/A)
P-42	93	PMP-3	R-3	6.0	130.0	(N/A)	(N/A)	(N/A)
P-43	97	J-15	PRV-1	8.0	130.0	15	0.09	0.000
P-44	244	PRV-1	J-16	8.0	130.0	15	0.09	0.000

FlexTable: PRV Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Max Day

Current Time: 0.000 hours

Label	Elevation (ft)	Pressure (From) (psi)	Pressure (To) (psi)	Flow (gpm)	Headloss (ft)	Zone
PRV-1	2,566.00	121.1	85.0	15	83.38	Zone 11

FlexTable: Pump Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Max Day

Current Time: 0.000 hours

Label	Elevation (ft)	Hydraulic Grade (Suction) (ft)	Hydraulic Grade (Discharge) (ft)	Flow (Total) (gpm)	Pump Head (ft)	Flow (Design) (gpm)	Head (Design) (ft)	Zone
PMP-1	2,616.00	2,620.00	2,845.95	54	225.95	1,210	193.75	Zone 12
PMP-2	2,590.00	2,592.00	2,845.92	0	0.00	1,718	101.49	Zone 12
PMP-3	2,596.00	(N/A)	(N/A)	(N/A)	(N/A)	2,648	92.40	Zone 11

FlexTable: Reservoir Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Max Day

Current Time: 0.000 hours

Label	Elevation (ft)	Zone	Flow (Out net) (gpm)	Hydraulic Grade (ft)
R-1	2,620.00	Zone 12	54	2,620.00
R-2	2,592.00	Zone 12	0	2,592.00
R-3	2,600.00	Zone 11	(N/A)	(N/A)

FlexTable: Junction Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Peak Hour

Current Time: 0.000 hours

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)	Zone
J-1	2,591.00	0	2,845.69	110.2	Zone 12
J-2	2,581.00	0	2,845.67	114.5	Zone 12
J-3	2,586.00	8	2,845.67	112.3	Zone 12
J-4	2,591.00	6	2,845.67	110.2	Zone 12
J-5	2,585.00	6	2,845.67	112.8	Zone 12
J-6	2,580.00	6	2,845.67	114.9	Zone 12
J-7	2,578.00	7	2,845.67	115.8	Zone 12
J-8	2,570.00	8	2,845.67	119.3	Zone 12
J-9	2,576.00	0	2,845.67	116.7	Zone 12
J-10	2,575.00	2	2,845.67	117.1	Zone 12
J-11	2,576.00	6	2,845.66	116.7	Zone 12
J-12	2,583.00	8	2,845.66	113.6	Zone 12
J-13	2,569.00	4	2,845.67	119.7	Zone 12
J-14	2,568.00	7	2,845.67	120.1	Zone 12
J-15	2,569.00	0	2,845.67	119.7	Zone 12
J-16	2,557.00	6	2,762.53	88.9	Zone 11
J-17	2,558.00	6	2,762.53	88.5	Zone 11
J-18	2,548.00	9	2,762.53	92.8	Zone 11
J-19	2,535.00	5	2,762.52	98.4	Zone 11
J-20	2,620.00	0	2,845.72	97.7	Zone 12
J-21	2,622.00	0	2,845.72	96.8	Zone 12
J-22	2,635.00	0	2,845.76	91.2	Zone 12
J-23	2,618.00	0	2,845.76	98.5	Zone 12
J-24	2,616.00	0	2,845.76	99.4	Zone 12
J-25	2,592.00	0	2,845.72	109.8	Zone 12
J-26	2,593.00	0	2,845.71	109.3	Zone 12
J-27	2,598.00	0	2,845.70	107.2	Zone 12
J-29	2,582.00	0	2,845.67	114.1	Zone 12
J-30	2,590.00	0	2,845.68	110.6	Zone 12
J-31	2,533.00	0	2,762.52	99.3	Zone 11

FlexTable: Pipe Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Peak Hour

Current Time: 0.000 hours

Label	Length (Scaled) (ft)	Start Node	Stop Node	Diameter (in)	Hazen- Williams C	Flow (gpm)	Velocity (ft/s)	Headloss Gradient (ft/ft)
P-1	215	J-1	J-2	8.0	130.0	60	0.38	0.000
P-2	151	J-2	J-3	8.0	130.0	26	0.17	0.000
P-3	240	J-3	J-4	8.0	130.0	12	0.08	0.000
P-4	139	J-4	J-5	8.0	130.0	6	0.04	0.000
P-5	199	J-3	J-6	8.0	130.0	6	0.04	0.000
P-6	256	J-2	J-7	8.0	130.0	15	0.10	0.000
P-7	154	J-7	J-8	8.0	130.0	8	0.05	0.000
P-9	152	J-9	J-2	8.0	130.0	-19	0.12	0.000
P-10	449	J-9	J-10	8.0	130.0	19	0.12	0.000
P-11	226	J-10	J-11	8.0	130.0	14	0.09	0.000
P-12	240	J-11	J-12	8.0	130.0	8	0.05	0.000
P-13	213	J-10	J-13	8.0	130.0	2	0.01	0.000
P-14	193	J-13	J-14	8.0	130.0	7	0.05	0.000
P-15	85	J-13	J-15	8.0	130.0	-9	0.05	0.000
P-17	245	J-16	J-17	8.0	130.0	6	0.04	0.000
P-18	337	J-16	J-18	8.0	130.0	14	0.09	0.000
P-19	846	J-18	J-19	8.0	130.0	5	0.03	0.000
P-21	33	J-20	J-21	8.0	130.0	-35	0.22	0.000
P-22	1,042	J-21	J-22	8.0	130.0	-35	0.22	0.000
P-23	1,209	J-22	J-23	12.0	130.0	-35	0.10	0.000
P-24	50	J-23	J-24	12.0	130.0	-35	0.10	0.000
P-25	477	J-24	J-25	8.0	130.0	60	0.38	0.000
P-26	20	J-25	J-26	8.0	130.0	60	0.38	0.000
P-27	113	J-26	J-27	8.0	130.0	60	0.38	0.000
P-28	562	J-27	J-1	12.0	130.0	60	0.17	0.000
P-30	174	J-15	J-29	8.0	130.0	-35	0.22	0.000
P-32	22	PMP-1	R-1	48.0	130.0	-94	0.02	0.000
P-33	30	PMP-1	J-24	48.0	130.0	94	0.02	0.000
P-34	37	R-2	PMP-2	48.0	130.0	0	0.00	0.000
P-35	250	J-29	J-30	8.0	130.0	-35	0.22	0.000
P-36	1,023	J-30	J-20	8.0	130.0	-35	0.22	0.000
P-37	36	PMP-2	J-30	48.0	130.0	0	0.00	0.000
P-40	195	J-19	J-31	6.0	130.0	0	0.00	0.000
P-41	513	J-31	PMP-3	8.0	130.0	(N/A)	(N/A)	(N/A)
P-42	93	PMP-3	R-3	6.0	130.0	(N/A)	(N/A)	(N/A)
P-43	97	J-15	PRV-1	8.0	130.0	26	0.17	0.000
P-44	244	PRV-1	J-16	8.0	130.0	26	0.17	0.000

FlexTable: Pump Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Peak Hour

Current Time: 0.000 hours

Label	Elevation (ft)	Hydraulic Grade (Suction) (ft)	Hydraulic Grade (Discharge) (ft)	Flow (Total) (gpm)	Pump Head (ft)	Flow (Design) (gpm)	Head (Design) (ft)	Zone
PMP-1	2,616.00	2,620.00	2,845.76	94	225.76	1,210	193.75	Zone 12
PMP-2	2,590.00	2,592.00	2,845.68	0	0.00	1,718	101.49	Zone 12
PMP-3	2,596.00	(N/A)	(N/A)	(N/A)	(N/A)	2,648	92.40	Zone 11

FlexTable: Reservoir Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Peak Hour

Current Time: 0.000 hours

Label	Elevation (ft)	Zone	Flow (Out net) (gpm)	Hydraulic Grade (ft)
R-1	2,620.00	Zone 12	94	2,620.00
R-2	2,592.00	Zone 12	0	2,592.00
R-3	2,600.00	Zone 11	(N/A)	(N/A)

FlexTable: PRV Table (Greasewood-WaterCAD-Model.wtg)

Active Scenario: Peak Hour

Current Time: 0.000 hours

Label	Elevation (ft)	Pressure (From) (psi)	Pressure (To) (psi)	Flow (gpm)	Headloss (ft)	Zone
PRV-1	2,566.00	121.0	85.0	26	83.13	Zone 11

Fire Flow Node FlexTable: Fire Flow Report (GreaseWood-WaterCAD-Model.wtg)

Active Scenario: Max Day + FF

Current Time: 0.000 hours

Label	Elevation (ft)	Fire Flow (Needed) (gpm)	Fire Flow (Available) (gpm)	Pressure (Calculated Zone Lower Limit @ Total Flow Needed) (psi)	Satisfies Fire Flow Constraints?
J-1	2,591.00	1,000	3,930	84.8	True
J-2	2,581.00	1,000	3,853	84.9	True
J-3	2,586.00	1,000	3,509	84.9	True
J-4	2,591.00	1,000	3,109	84.9	True
J-5	2,585.00	1,000	2,982	84.9	True
J-6	2,580.00	1,000	3,272	84.9	True
J-7	2,578.00	1,000	3,445	84.9	True
J-8	2,570.00	1,000	3,273	84.9	True
J-9	2,576.00	1,000	3,951	85.0	True
J-10	2,575.00	1,000	3,958	85.2	True
J-11	2,576.00	1,000	3,458	85.2	True
J-12	2,583.00	1,000	3,087	85.2	True
J-13	2,569.00	1,000	4,150	85.4	True
J-14	2,568.00	1,000	3,730	85.4	True
J-15	2,569.00	1,000	4,234	85.4	True
J-16	2,557.00	1,000	3,567	86.5	True
J-17	2,558.00	1,000	3,198	86.9	True
J-18	2,548.00	1,000	3,163	86.5	True
J-19	2,535.00	1,000	2,540	86.5	True
J-20	2,620.00	1,000	3,468	84.8	True
J-21	2,622.00	1,000	3,447	84.7	True
J-22	2,635.00	1,000	3,801	89.8	True
J-23	2,618.00	1,000	4,131	84.4	True
J-24	2,616.00	1,000	4,147	84.5	True
J-25	2,592.00	1,000	4,029	84.7	True
J-26	2,593.00	1,000	4,007	84.7	True
J-27	2,598.00	1,000	3,887	84.8	True
J-29	2,582.00	1,000	4,219	85.5	True
J-30	2,590.00	1,000	4,338	85.7	True
J-31	2,533.00	1,000	2,096	86.5	True

■
Sewer Basis of Design Report

***Cavalliere Flats
80-Lot Proposed Subdivision***

Scottsdale, Arizona

Accepted for
City of Scottsdale
Water Resources Administration
9379 E. San Salvador
Scottsdale, AZ 85258

Prepared for:

Doug Mann 9.10.14
Taylor Morrison/Arizona, Inc.

Prepared by:

Kimley-Horn and Associates, Inc.
*7740 North 16th Street, Suite 300
Phoenix, AZ 85020*

191069012
August 2014
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Kimley»»Horn

**12-ZN-2014
9/8/2014**

Sewer Basis of Design Report

Cavalliere Flats 80-Lot Proposed Subdivision

Scottsdale, Arizona

Prepared for:

Taylor Morrison/Arizona, Inc.
9000 E. Pima Center Parkway, Suite 350
Scottsdale, Arizona 85258

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3.0	BASIS OF DESIGN.....	2

LIST OF APPENDICES

- Appendix A: Site Location Map
- Appendix B: Proposed Wastewater System Layout
- Appendix C: Flowmaster Calculations

1.0 INTRODUCTION

1.1 Intent

The purpose of this water report is to review the existing and proposed wastewater system for the proposed Cavalliere Flats residential development located at the southeast corner of Alma School Parkway and Pinnacle Vista Drive in Scottsdale, Arizona. This report presents the basis of design criteria that will be used for the engineering design of the proposed development utilizing current wastewater design standards and guidelines set forth by the City of Scottsdale, Arizona.

1.2 Project Description

The proposed project is located at the southeast corner of Alma School Parkway and Pinnacle Vista Drive in the City of Scottsdale, Arizona. It is more precisely described as a portion of the southeast quarter of Section 33, Township 5 North, Range 5 East, Maricopa County, Arizona. The proposed development consists of approximately 47 acres and is planned to include a 80-lot, single family residential subdivision. Refer to **Appendix A** for the Site Location Map.

2.0 COLLECTION SYSTEM DESCRIPTION

2.1 Existing Collection System

Per the City of Scottsdale Sewer Quarter Section Map 49-53, an 8-inch ~~asbestos concrete pipe (ACP)~~ ^{YCP} sewer main is located in Alma School Parkway to the west of the site. Additionally an 8-inch diameter (PVC) sewer line is located along Jomax Road to the south of the site. The line is located within the north, unpaved half-street of Jomax Road.

2.2 Proposed Collection System

The proposed ~~distribution~~ ^{Collection} system will consist of 8" PVC gravity sewer line that will collect and convey wastewater flows generated by the site. The collection system will connect to the existing 8-inch PVC ~~water~~ ^{Sewer} line stub located in Jomax Road. The system also provides a stub to Bajada Drive to the east of the site for a future connection. Refer to **Appendix B** for the Proposed ~~Water~~ ^{Sewer} System Layout Exhibit.

3.0 BASIS OF DESIGN

3.1 Design Criteria

Average Day Demand design flows are calculated based on design criteria detailed within the City of Scottsdale Design Standards and Polices Manual (DS&PM). Per DS&PM Chapter 7, a design flow of 100 gallons per capita per day (gpcpd) shall be used. The DS&PM also requires a peaking factor of 4.0 and a residential density of 2.5 persons per dwelling unit. A summary of design criteria and calculated demands is included in the table below:

	No. of Units	Density	Population	Average Day Demand (GPD)	Peak Flow (GPD)	Peak Flow (GPM)
8-inch Sewer	80	2.5	200	20,000	8,000	55.6

Per the DS&PM proposed sewer lines were designed to achieve a full flow velocity of between 2.5 and 10 feet per second and maintain a maximum d/D ratio 0.65 when calculated with a Manning's "n" value of 0.013. To satisfy these requirements the proposed public 8-inch sewer will meet be design with a minimum slope of 0.0052 ft/ft (0.52%) and a maximum slope of 0.0833 ft/ft (8.33%). See **Appendix C** for pipe slope calculations.

3.2 Wastewater System Analysis and Results

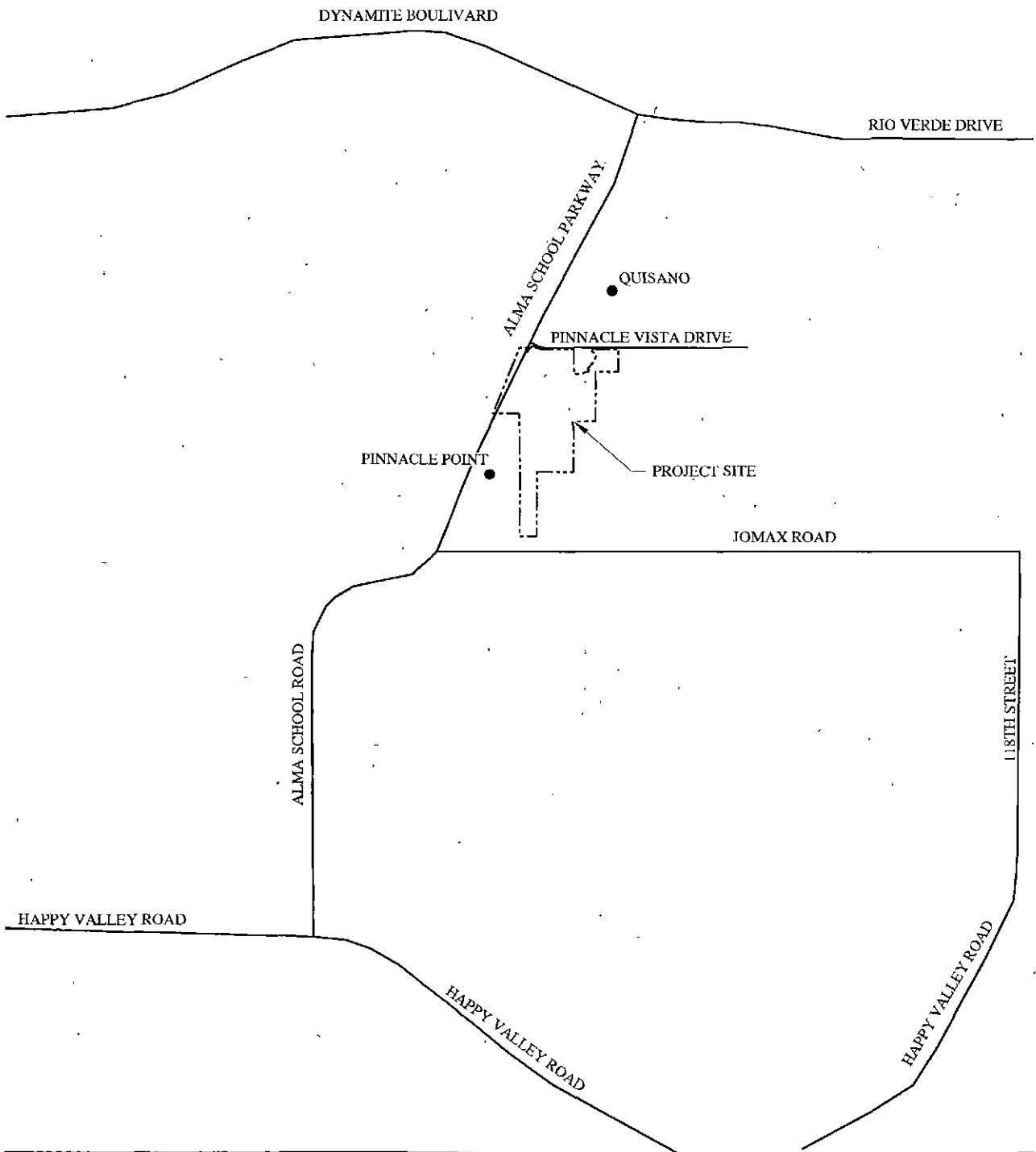
To determine the capacity of the proposed wastewater collection system, the peak design flow was analyzed within the minimum design pipe slope. At the minimum design slope of 0.0052 ft/ft an 8-inch line has the capacity to convey approximately 563,000 gallons per day. An 8-inch line at the minimum design slope can convey the proposed peak design flow of 80,000 gallons per day at a normal depth of 0.17' or a d/D ratio of 0.25, at a velocity of 1.77 ft/s. **See Appendix C – Flowmaster Calculations** for pipe capacity calculations.

3.3 Conclusions

The proposed wastewater collection system will collect and convey wastewater flows from the proposed 80 unit residential subdivision, Cavalliere Flats. The system will discharge to the existing City of Scottsdale sewer system using a connection point at an existing 8-inch sewer stub within Jomax Road. Based on the calculations included within, the proposed system has the capacity to convey the calculated peak flow. The sizes and slopes of the proposed system have been design in accordance with City of Scottsdale design requirements per the DS&PM. The capacity of the sewer system downstream of the proposed connection location has not been analyzed.

APPENDIX A
SITE LOCATION MAP

U:\WORK\CH\101000010 - Operations\Reports\Water\001\Exhibit\Map\Map.dwg May 19, 2014 2:04:18 PM
PLOT: 10/19/2014 09:50:04 AM 1000127P



APPENDIX B
PROPOSED WASTEWATER SYSTEM LAYOUT

APPENDIX C
FLOWMASTER CALCULATIONS

Worksheet for 8-Inch Full - Min

Project Description

Friction Method Manning Formula
Solve For Full Flow Capacity

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00520	ft/ft
Normal Depth	0.67	ft
Diameter	0.67	ft
Discharge	563167.59	gal/day

Results

Discharge	563167.59	gal/day
Normal Depth	0.67	ft
Flow Area	0.35	ft ²
Wetted Perimeter	2.09	ft
Hydraulic Radius	0.17	ft
Top Width	0.00	ft
Critical Depth	0.44	ft
Percent Full	100.0	%
Critical Slope	0.00857	ft/ft
Velocity	2.50	ft/s
Velocity Head	0.10	ft
Specific Energy	0.76	ft
Froude Number	0.00	
Maximum Discharge	0.94	ft ³ /s
Discharge Full	0.87	ft ³ /s
Slope Full	0.00520	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%

Worksheet for 8-Inch Full - Min

GVF Output Data

Normal Depth Over Rise	100.00	%
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.67	ft
Critical Depth	0.44	ft
Channel Slope	0.00520	ft/ft
Critical Slope	0.00857	ft/ft

Worksheet for 8-Inch Full -Max

Project Description

Friction Method Manning Formula
Solve For Full Flow Capacity

Input Data

Roughness Coefficient 0.013
Channel Slope 0.08330 ft/ft
Normal Depth 0.67 ft
Diameter 0.67 ft
Discharge 2254023.72 gal/day

Results

Discharge 2254023.72 gal/day
Normal Depth 0.67 ft
Flow Area 0.35 ft²
Wetted Perimeter 2.09 ft
Hydraulic Radius 0.17 ft
Top Width 0.00 ft
Critical Depth 0.66 ft
Percent Full 100.0 %
Critical Slope 0.07763 ft/ft
Velocity 9.99 ft/s
Velocity Head 1.55 ft
Specific Energy 2.22 ft
Froude Number 0.00
Maximum Discharge 3.75 ft³/s
Discharge Full 3.49 ft³/s
Slope Full 0.08330 ft/ft
Flow Type SubCritical

GVF Input Data

Downstream Depth 0.00 ft
Length 0.00 ft
Number Of Steps 0

GVF Output Data

Upstream Depth 0.00 ft
Profile Description
Profile Headloss 0.00 ft
Average End Depth Over Rise 0.00 %

Worksheet for 8-Inch Full -Max

GVF Output Data

Normal Depth Over Rise	100.00	%
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.67	ft
Critical Depth	0.66	ft
Channel Slope	0.08330	ft/ft
Critical Slope	0.07763	ft/ft

Worksheet for 8-Inch Capacity

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient 0.013
Channel Slope 0.00520 ft/ft
Diameter 0.67 ft
Discharge 80000.00 gal/day

Results

Normal Depth 0.17 ft
Flow Area 0.07 ft²
Wetted Perimeter 0.71 ft
Hydraulic Radius 0.10 ft
Top Width 0.58 ft
Critical Depth 0.16 ft
Percent Full 25.5 %
Critical Slope 0.00650 ft/ft
Velocity 1.77 ft/s
Velocity Head 0.05 ft
Specific Energy 0.22 ft
Froude Number 0.90
Maximum Discharge 0.94 ft³/s
Discharge Full 0.87 ft³/s
Slope Full 0.00010 ft/ft
Flow Type SubCritical

GVF Input Data

Downstream Depth 0.00 ft
Length 0.00 ft
Number Of Steps 0

GVF Output Data

Upstream Depth 0.00 ft
Profile Description
Profile Headloss 0.00 ft
Average End Depth Over Rise 0.00 %
Normal Depth Over Rise 25.46 %
Downstream Velocity Infinity ft/s



Worksheet for 8-Inch Capacity

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.17	ft
Critical Depth	0.16	ft
Channel Slope	0.00520	ft/ft
Critical Slope	0.00650	ft/ft





100

100

100



File Copy

Case Drainage Report Review – Cavalliere Flats

PROJECT NAME: CAVALLIERE FLATS

LOCATION: SOUTHEAST CORNER OF ALMA SCHOOL PARKWAY AND EAST PINNACLE VISTA DRIVE

CASE NUMBERS: 12-ZN-2014

Review comments for case drainage report by Kimley-Horn and Associates, Inc. sealed August 28, 2014. The drainage report was received by the City of Scottsdale (City) on September 8, 2014. The date of the review is September 30, 2014. Our review comments are as follows:

1. The City will continue to work with the applicant and his engineer on the location, design and requirements for the crossing of stormwater outflows from Cavalliere Park across Pinnacle Vista Drive. The applicant and engineer should be aware that the outcome of these discussions may have a significant impact on the layout and design of the development.
2. It is unclear how stormwater flows from the existing east culvert on Pinnacle Vista Drive and the northeast corner of the development parcel will drain through the development. Figure 4 shows a 100-year flow of 200 cfs for this wash based on the overflow of the outflow from Doc Cavalliere Park. Existing flows, excluding the 200 cfs overflow, are likely on the order of 30 cfs based on the size of the existing culvert and the contributing area to the east. Figure 4 shows this flow being routed through a proposed out parcel for the development.
3. It is unclear how stormwater flows from the west crossing Alma School Parkway will drain through the development. The drainage report shows 100-year flows of around 45 cfs from two locations entering the development site. However, there does not appear to be adequate area within the development site to route these flows to the south then east around the proposed lots.
4. Due to the conceptual nature of the drainage report, it is likely there will be a number of future significant review comments and issues resulting from the review of the preliminary drainage report and preliminary grading and drainage plan to be prepared and submitted in support of the preliminary plat case for this development.

Richard M. Anderson, P.E.
Senior Civil Engineer
Stormwater Management Division
City of Scottsdale
Phone: 480-312-2729
Fax: 480-312-9202

Response to Drainage Comments:

1. It is understood that more detail will be required for the preliminary plat applications. As mentioned in comment 3, the storage requirement for pre verses post will be included in the preliminary drainage report included with preliminary plat. Discussion with City staff on the offsite flows that impact the site from the North is ongoing and will be addressed further at the preliminary plat as well.
2. An offsite drainage exhibit has been provided as Figure 2. Excerpts from the *Addendum to the Preliminary Drainage Report for the Rocks at Reata Pass* have been included in the report to help validate the offsite flows from the west. Figures 3 and 4 have been revised to show all required items.
3. As discussed in the response to comment 1, a pre verses post development analysis will be conducted as part of the preliminary plat submittal to show that the project will not increase flows downstream.
4. The discussion of a channel to convey 200 cfs has been removed from the report due to the ongoing discussion with City staff to identify a different solution for flows crossing Pinnacle Vista Drive.
5. Discussion is currently underway with City staff to determine a mutually acceptable solution to the flows crossing Pinnacle Vista Drive. A solution will be shown at the preliminary plat submittal.
6. The HEC-RAS model has been updated with the revised manning's n values. The manning's n values have been varied based on aerial photography.
7. A disc containing HEC-1 and HEC-RAS models are included with the report for City review.

**12-ZN-2014
9/8/2014**



Preliminary Drainage Report
Prepared: August 2014

Cavalliere Flats

Prepared for:

Taylor Morrison/Arizona, Inc.
9000 E. Pima Center Parkway, Suite 350
Scottsdale, Arizona 85258

Prepared by:

Kimley-Horn and Associates, Inc.
7740 N. 16th Street, Suite 300
Phoenix, AZ 85020
(602) 944-5500

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August, 2014.

Kimley »» Horn

12-ZN-2014
9/8/2014



Preliminary Drainage Report

Prepared: August 2014

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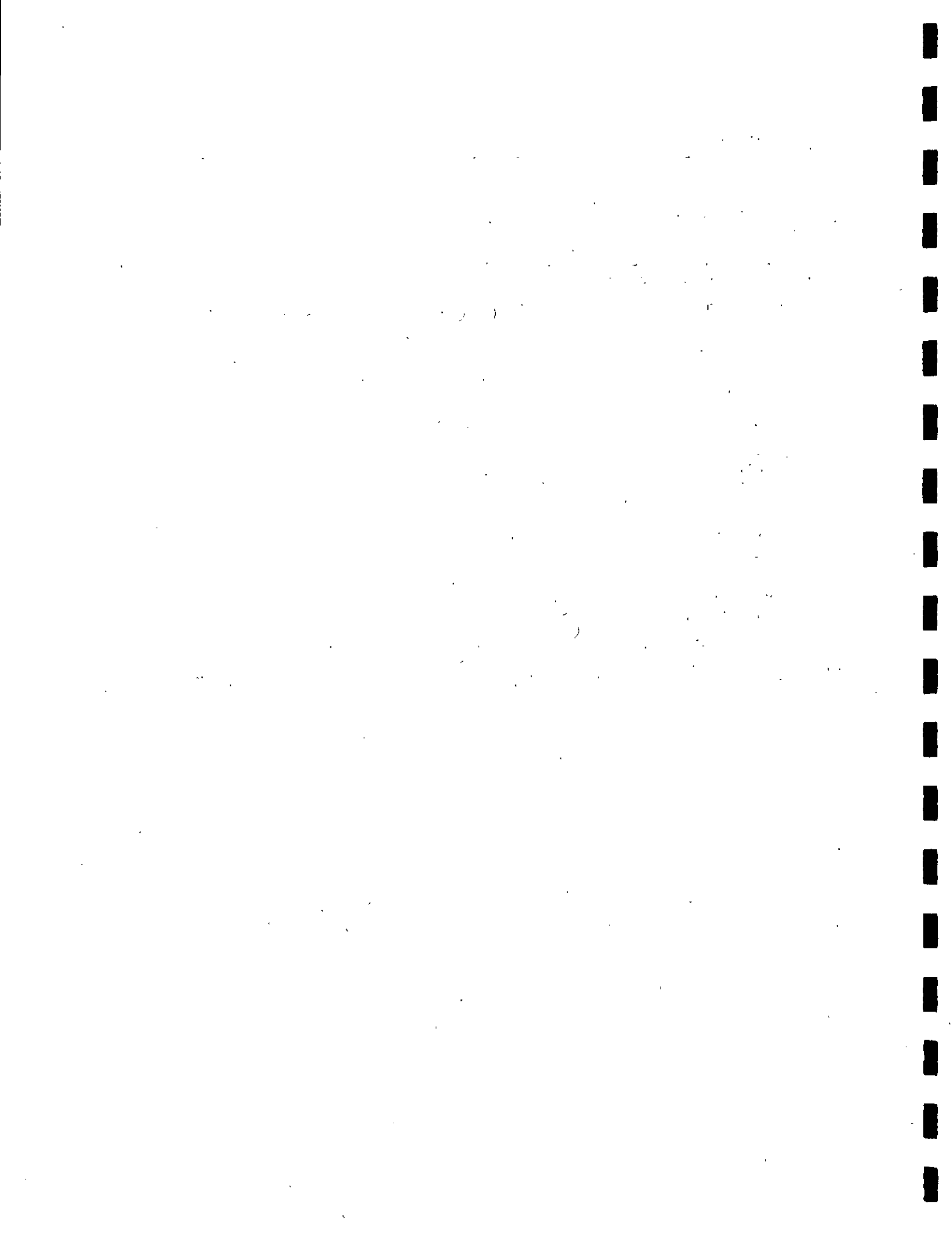
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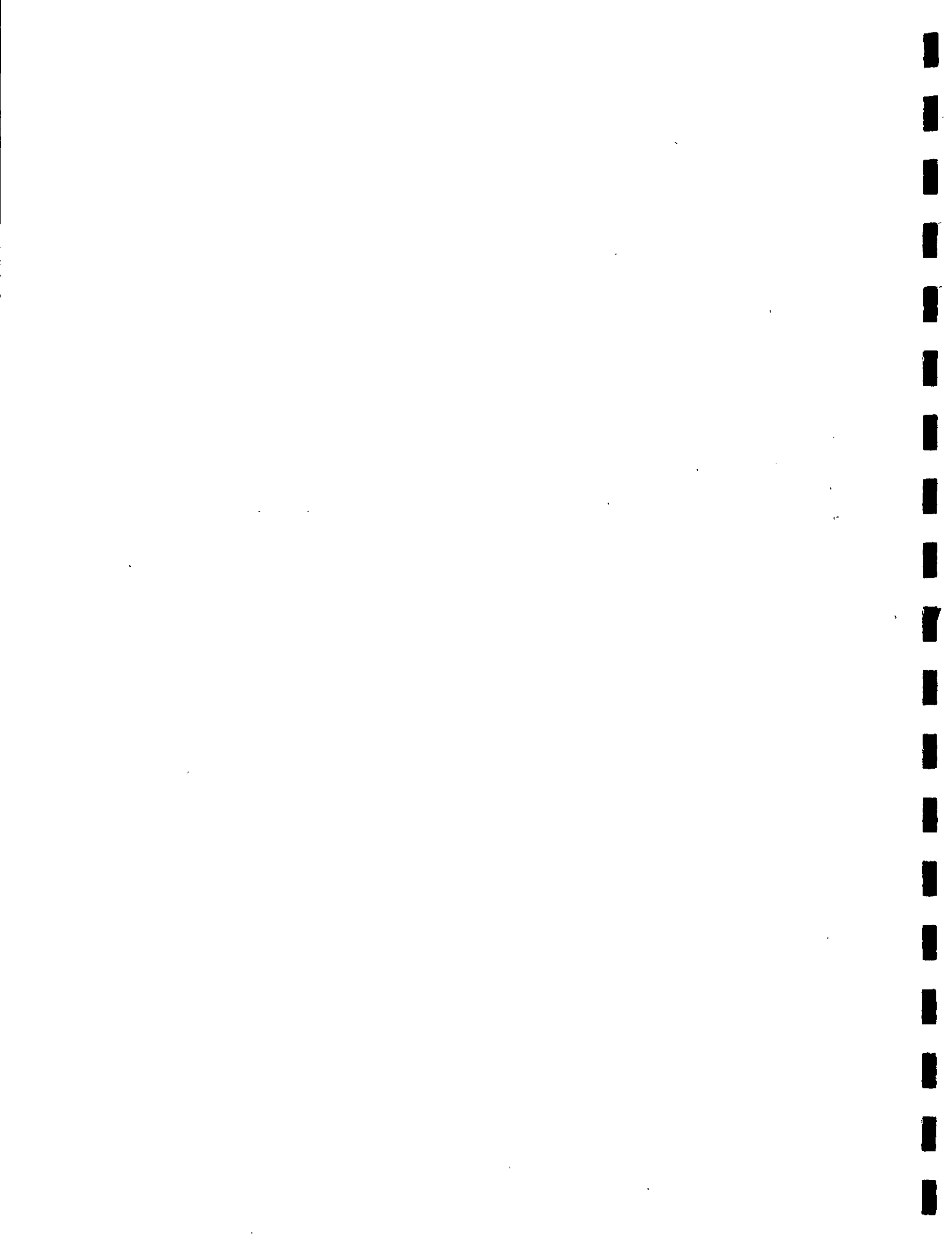
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1.0 INTRODUCTION

1.1 Project Description

This Preliminary Drainage report has been prepared for the proposed Cavalliere Flats residential subdivision development. Cavalliere Flats is a proposed 47-acre single family residential subdivision. The proposed development consists of 80 single family residential units. The proposed site is located within the City of Scottsdale and falls under the city's Environmentally Sensitive Lands Ordinance (ESLO)

1.2 Project Location and Description

Cavalliere Flats is located within Section 33 of Township 5 North, Range 5 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. The site is bound to the north by the Pinnacle Vista Drive and the west by Alma School Parkway. Jomax Road bounds the site to the south, with the majority of the land south of the site being undeveloped desert. An existing condominium development called Pinnacle Point is located to the southwest of the site. Scattered single family residential dwellings are located to the west of the site. North of Pinnacle Vista Drive is Quisana; an existing single family residential subdivision. To the east of Quisana and northeast of the site is Troon North Park, which also serves as a City of Scottsdale regional detention basin. (see Figure 1. Vicinity Map).

The project property is home to the existing Greasewood Flats bar and restaurant and the existing Reata Pass Steakhouse. Also located on the property are multiple single family residences, as well as other miscellaneous structures. Minor improvements such as parking lots, street lights and fencing scatter the property.

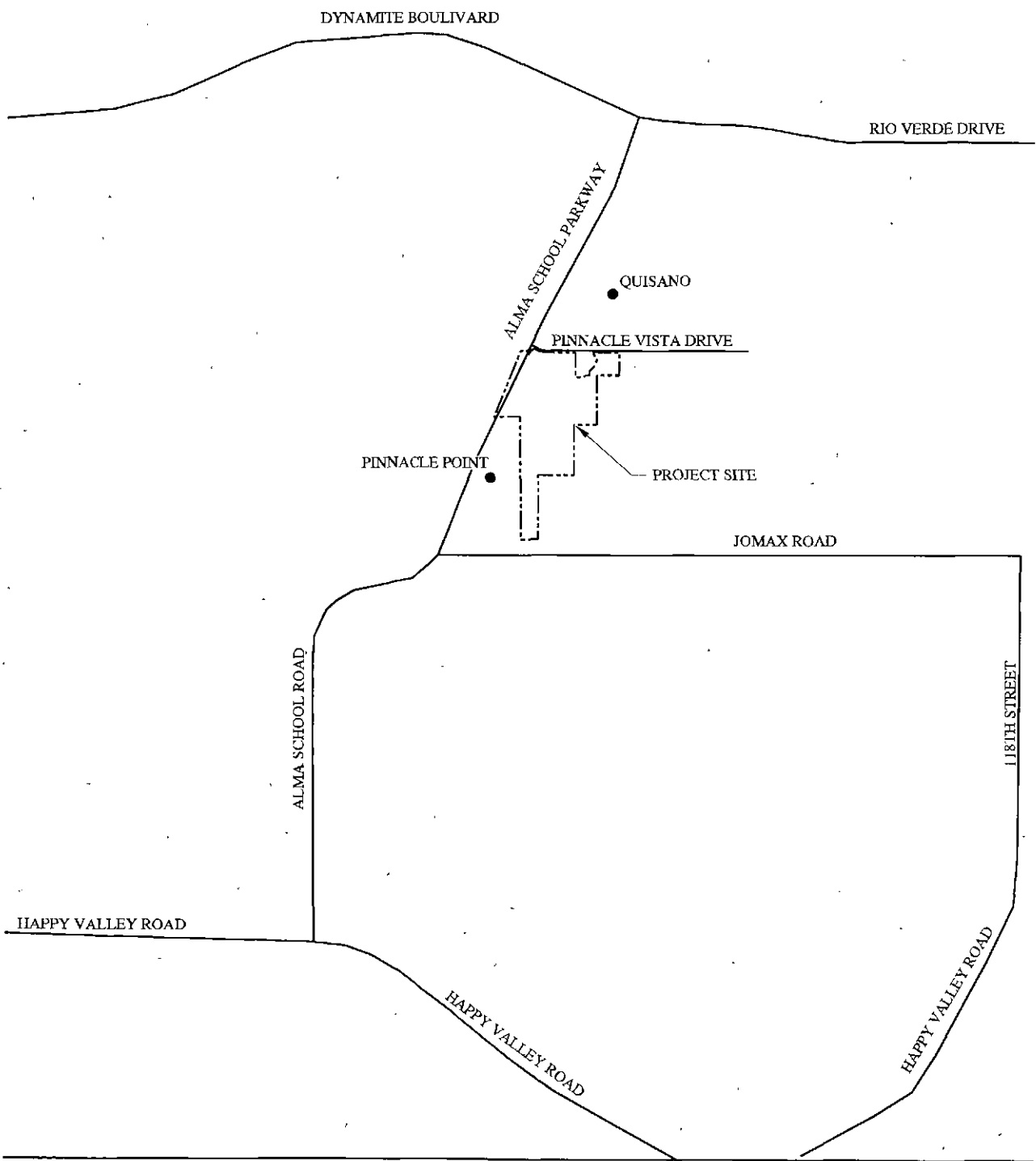
1.3 Scope of Drainage Report

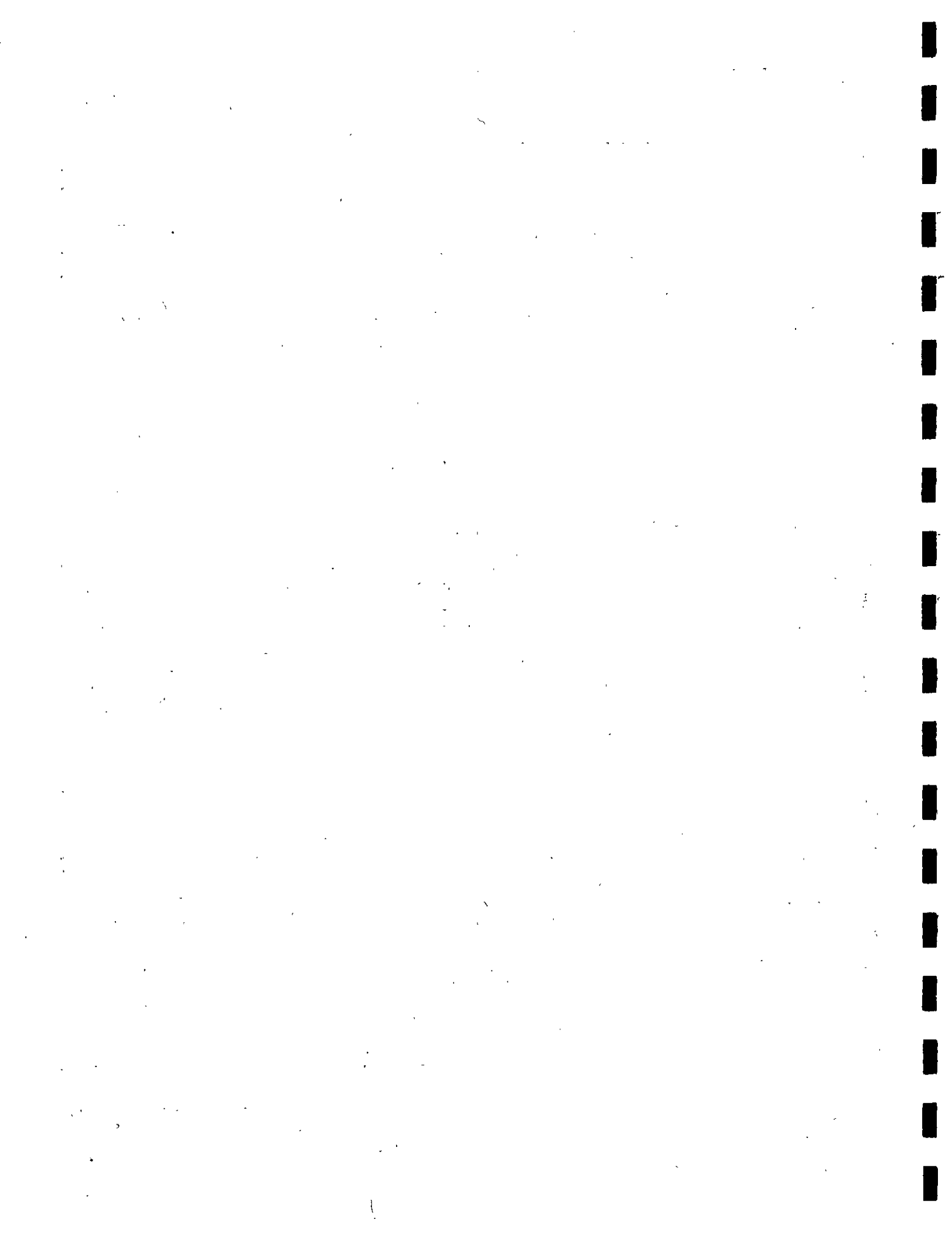
This Preliminary Drainage report for Cavalliere Flats establishes preliminary drainage parameters and criteria for the development of the site. This report demonstrates compliance with the City of Scottsdale Design Standards and Polices Manual (DS&PM) as well as Flood Control District of Maricopa County (FCDMC) drainage polices.

This report will quantify existing offsite flows currently being conveyed through the development area and onto the property and evaluate existing and proposed drainage paths. The report will also present the preliminary hydrologic analysis of both on-site and off-site runoff for the pre-developed and post-developed conditions.



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2.0 DESCRIPTION OF EXISTING DRAINAGE CONDITIONS AND CHARACTERISTICS

2.1 Existing Site Conditions

The site property is occupied by the existing Greasewood Flats bar and restaurant and the Reata Pass Steakhouse. These two structures are located near the western edge of the site with access to Alma School Parkway. Associated with these buildings are typical improvements associated with commercial locations including paved parking lots, street lights, and fencing. Both structures include outdoor patio seating which does not appear to be an impervious surface.

There are also two existing single family residences located in the northeast portion of the project. The western residence of these two properties will be excluded from the development area and not included as part of the development. The eastern residence will be included in the development with the existing dwelling unit remaining in its current location.

Other minor improvements and non-native materials scatter the site. These include small shed-like structures, power and light poles, telephone poles, animal fences and pens, abandoned vehicles and other large scrap metal items.

2.2 Existing On-Site Drainage Conditions

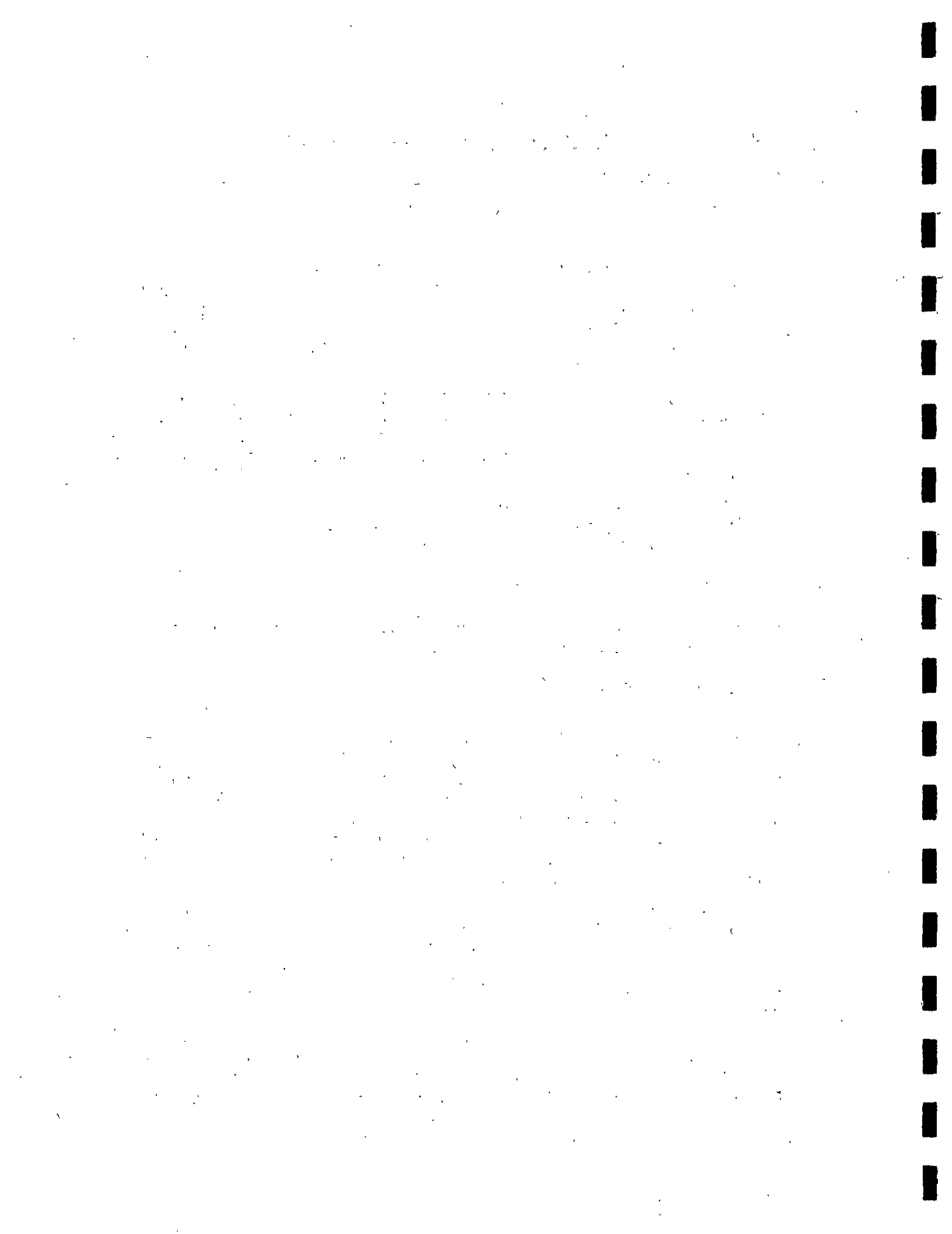
Topography of the site generally slopes towards the single major wash which bisects the site. The wash and major drainage way flows southwest through the entirety of the site.

2.3 Existing Off-Site Drainage Conditions

Off-site flows impact the site from the north, east and west. Flows to the west originate in the Rocks at Reata Pass development and are conveyed through a portion of the Four Seasons Resort property. Two existing culverts convey the offsite flow across Alma School Parkway onto the site. According to the Addendum to the Preliminary Drainage Report for the Rocks at Reata Pass, approximately 33 cfs exits the Rocks at Reata Pass property. Due to the uncertainty of how the offsite flow is conveyed through the Four Seasons Resort, the offsite flow from the west was determined based on the capacity of each culvert. Excerpts from the addendum are included in **Appendix B**.

The largest impact to the site comes from the northern property boundary across Pinnacle Vista Drive. Off-site flows from both the existing Quisana development and the Troon North Park detention basin are channelized north of Pinnacle Vista Drive. At approximately 500 feet east of Alma School Parkway the flows cross Pinnacle Vista Drive and impact the site. This flow is conveyed through the site within the existing onsite wash.

According to the Conceptual Drainage Report prepared for Quisana, the Troon North Park outlet channel has the capacity to convey the 100-year storm event. The capacity of the channel was determined to be 348 cfs using a normal depth analysis. According to this analysis, approximately 200 cfs will overtop the channel during the 100-year event. This overtopping flow does not assume any other failures along the channel, however,



according to discussion with City staff, the channel experiences failures at more frequent storm events. Discussion with City staff on a solution to this offsite drainage issue is ongoing and will be presented in future drainage reports for preliminary plat. For the analysis associated with this report, it is assumed that 200 cfs will overtop the channel and be routed south separately across the site.

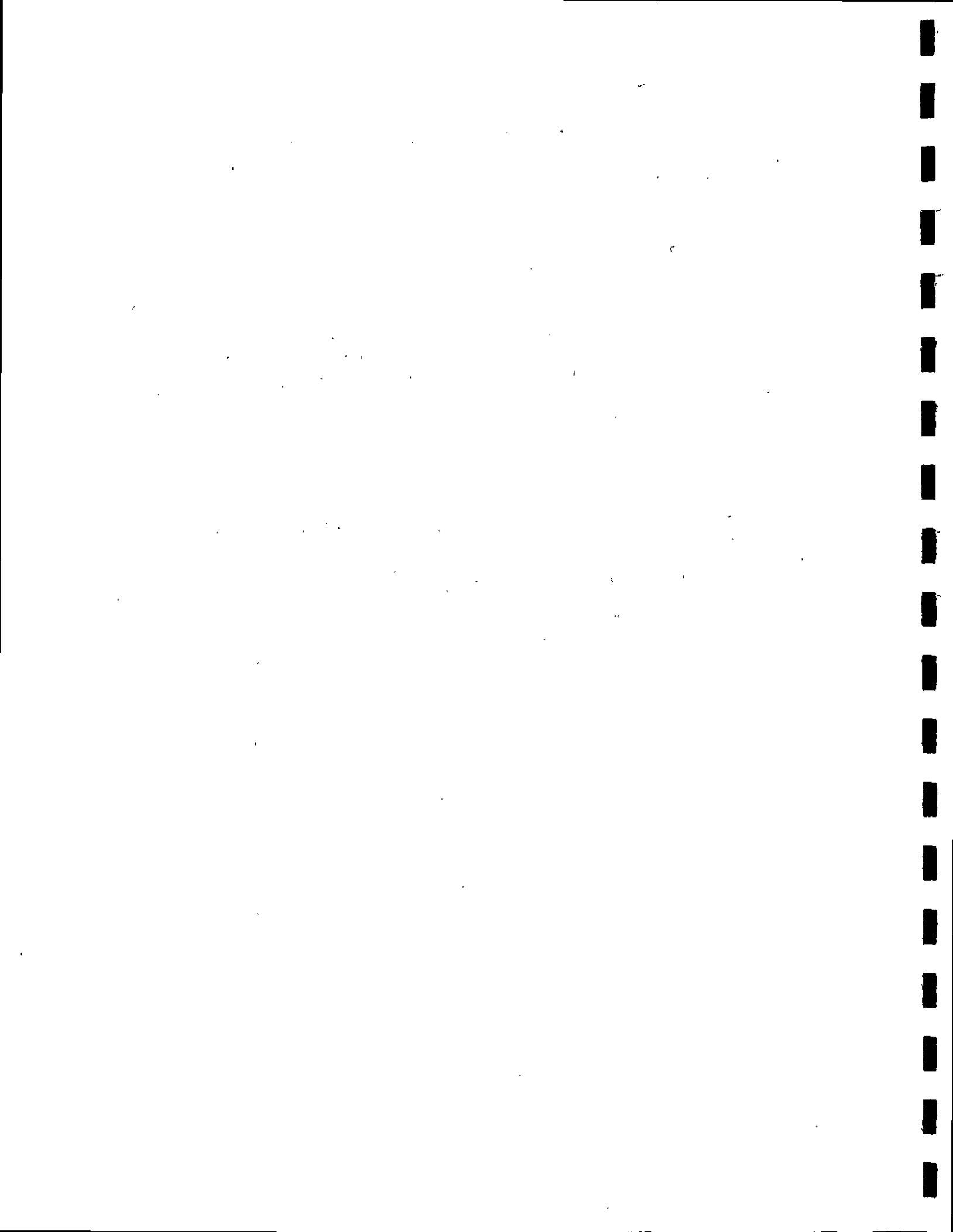
From the east minor washes convey local flows onto the site. A single large wash conveys a large regional flow onto the site, just north of the existing Greasewood Flats bar

Flows from the north and east were determined using a HEC-1 analysis based on the models included with the Troon North Park Hydrology Study Addendum Report prepared by Argus Consulting in September 2011. Excerpts from this report are included in **Appendix B**. See Figure 2 for the offsite drainage exhibit. Existing condition flows are detailed on Figure 3, for complete existing hydrology and hydraulics see **Appendix C** and **Appendix D** respectively

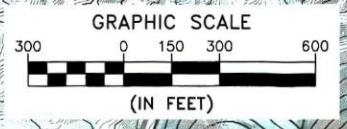
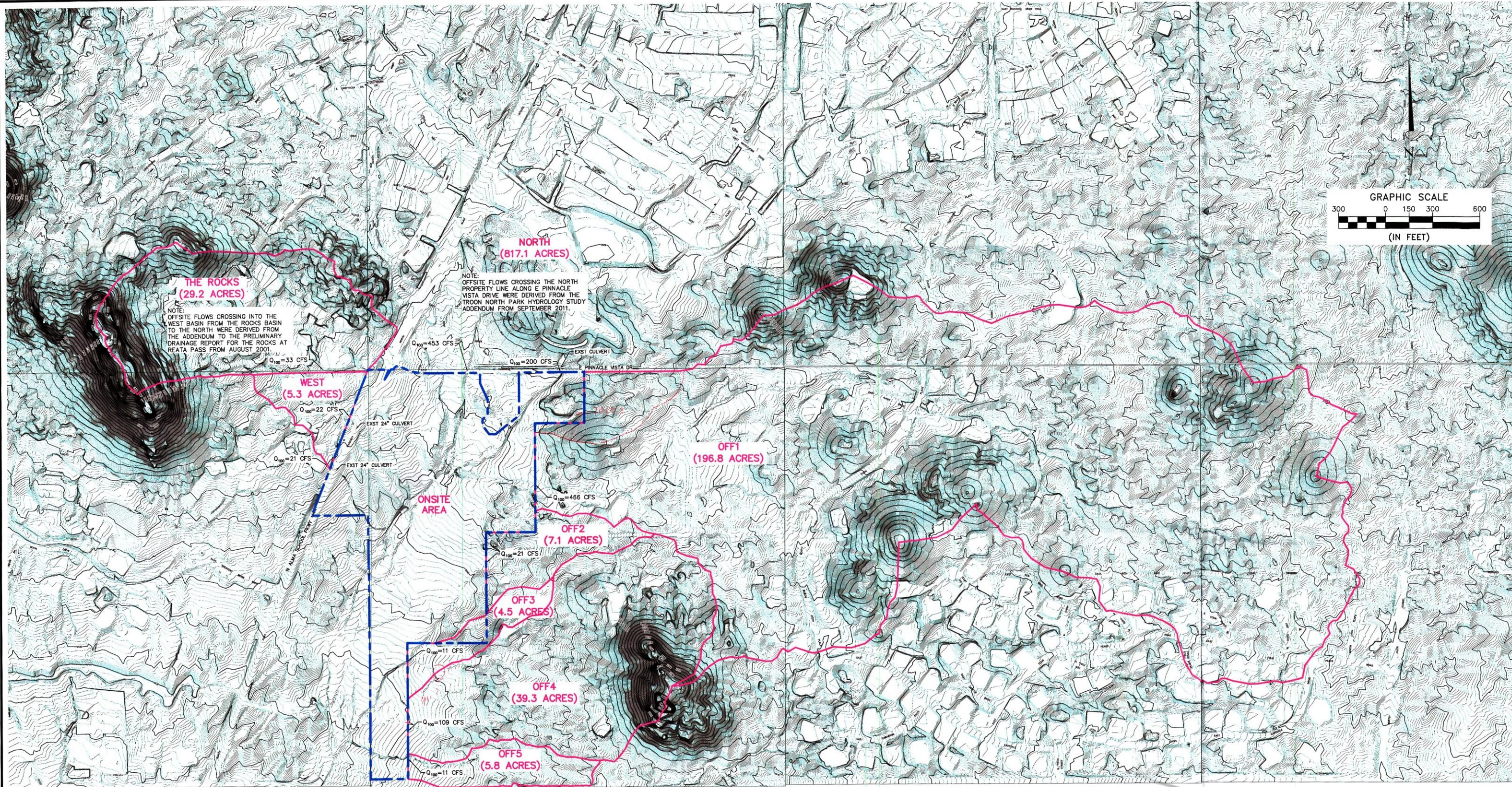
2.4 Flood hazards Zones

The development is located within one flood zones as shown on Flood Insurance Rate Map (FIRM) panel number 04013C1330L, dated October 16, 2013 (see **Appendix A** for FIRM). Fema defines the flood zone for the site area as:

"Other Flood Areas" Zone X – "Areas of 0.2% annual chance flood, areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood."

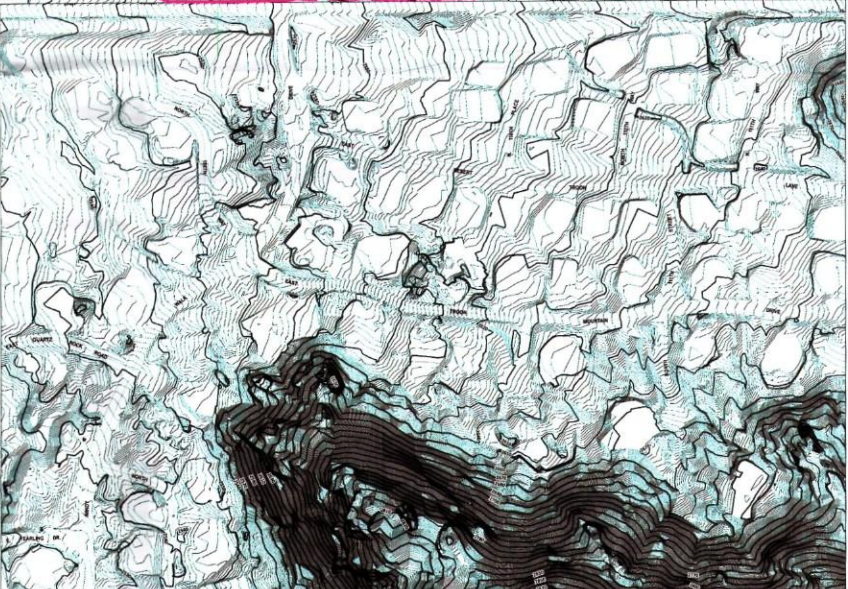


K:\PHX_Civil\191069102 - Greasewood Reports\Drawings\Emmits\Figure 2 - Offsite.dwg Aug 27, 2014 nick.stafford
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LEGEND

- EX. OFFSITE HEC-1 SUB-BASINS
- - - - - PROJECT BOUNDARY



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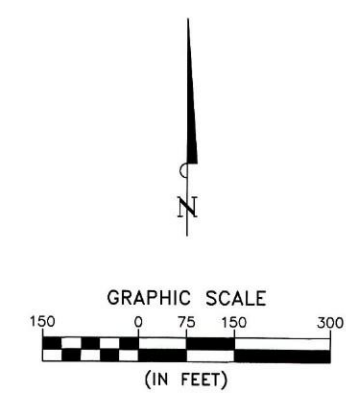
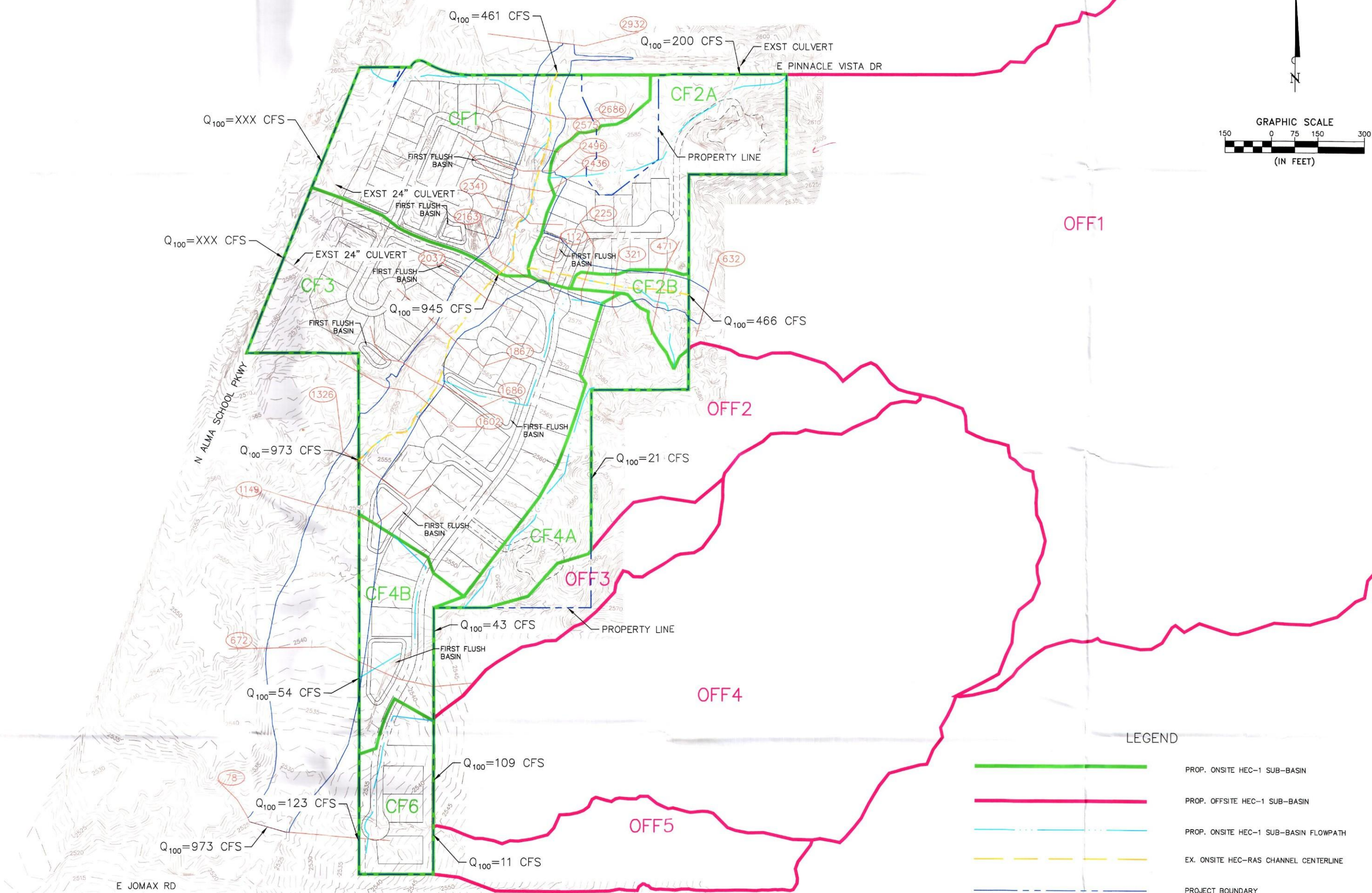
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 DRAWN BY: NAS
 CHECKED BY: MAW
 DATE: AUG 2014

CAVALLIERE FLATS
OFFSITE DRAINAGE CONDITIONS
FIGURE 2
 SCOTTSDALE, ARIZONA

PROJECT NO.
191069102
 DRAWING NAME
FIGURE 2
 1 OF 1

Kimley»Horn
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 7878 North 16th Street, Suite 300
 Phoenix, Arizona 85020 (602) 944-5500

K:\Phd_civa\191069102 - Grasswood\Reports\Drainage\Exhibits\Figure 4 - Prop Conditions Aug 21, 2014_nick.stofford
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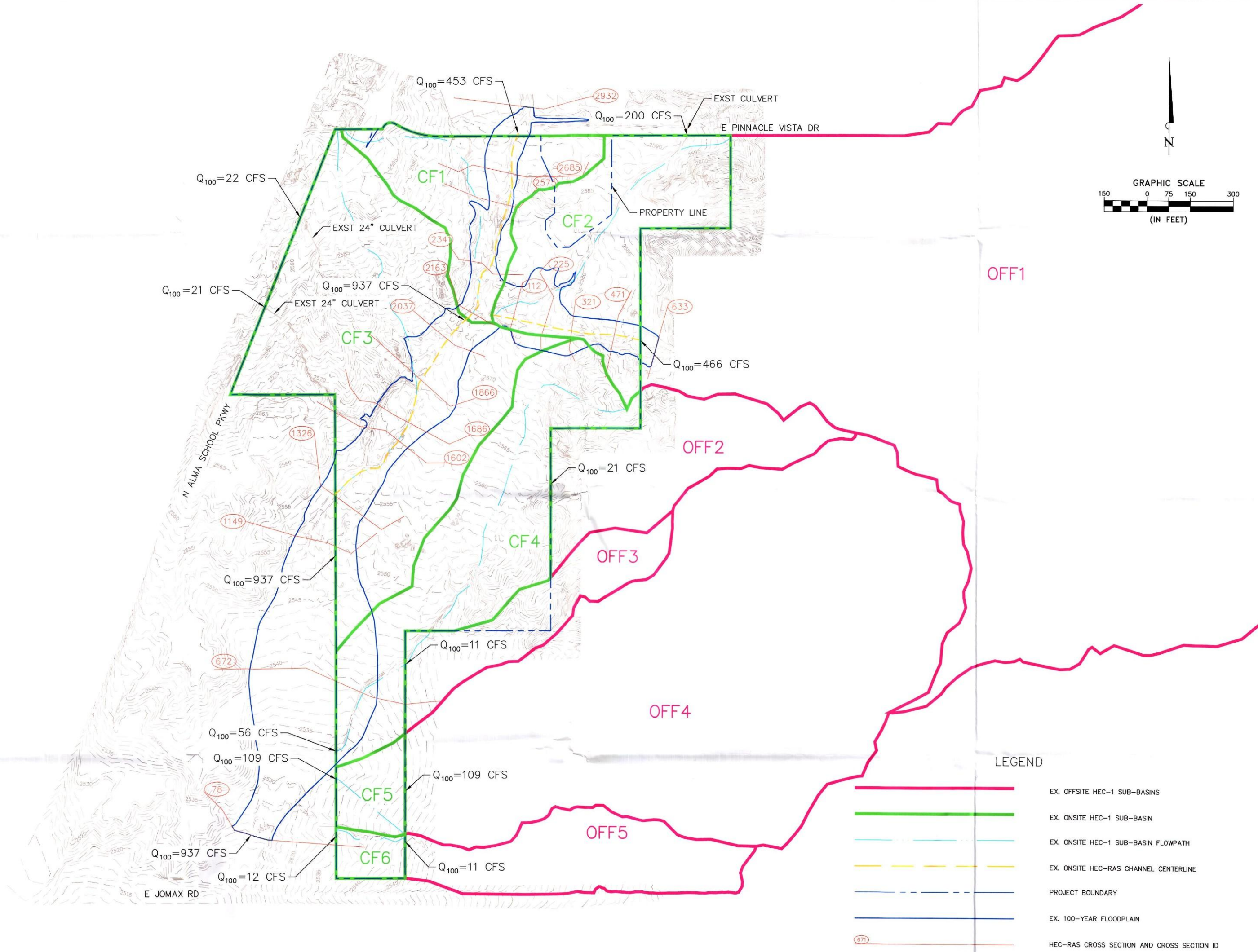


LEGEND

	PROP. ONSITE HEC-1 SUB-BASIN
	PROP. OFFSITE HEC-1 SUB-BASIN
	PROP. ONSITE HEC-1 SUB-BASIN FLOWPATH
	EX. ONSITE HEC-RAS CHANNEL CENTERLINE
	PROJECT BOUNDARY
	PROP. 100-YEAR FLOODPLAIN
	HEC-RAS CROSS SECTION AND CROSS SECTION ID

<p>Kimley»Horn</p> <p>© 2014 KIMLEY-HORN AND ASSOCIATES, INC. 7878 North 16th Street, Suite 300 Phoenix, Arizona 85020 (602) 944-5500</p>	<p>SCALE (H): 1"=150' SCALE (V): NONE DESIGNED BY: MAW DRAWN BY: NAS CHECKED BY: MAW DATE: AUG 2014</p>
<p>CAVALLIERE FLATS PROPOSED DRAINAGE CONDITIONS FIGURE 4 SCOTTSDALE, ARIZONA</p>	
<p>PROJECT NO. 191069102</p>	<p>BY DATE APPR. REVISION NO.</p>
<p>DRAWING NAME FIGURE 4</p>	
<p>1 OF 1</p>	

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 DRAWN BY: NAS
 CHECKED BY: MAW
 DATE: AUG 2014

**CAVALLIERE FLATS
 EXISTING DRAINAGE CONDITIONS
 FIGURE 3
 SCOTTSDALE, ARIZONA**

3.0 PROPOSED PRELIMINARY DRAINAGE PLAN

3.1 General Discussion

Stormwater runoff generally flows south through the site. Following development of Cavalliere Flats, runoff will be conveyed in the streets and/or in on-site swales and storm drain systems to several first flush detention basins located throughout the project. These basins will discharge into the existing central wash. The post development flows exiting the site will be attenuated through the first flush basin to a level equal to or less than pre-development flows

3.2 Methodology

When stormwater is conveyed through streets the 10-year flow will be contained within the curb, and the 100-year flow will be contained within the street right of way or tract at a maximum depth of 8 inches above the street.

For cross road culvert crossings the 10-year flow will be conveyed under the road, while the 100-year storm will be conveyed with a maximum depth over the road of 12-inches. All lots and structures will be accessible by at least one route with a depth of flow no greater than 1 foot during the 100-year event.

Culverts have been sized for crossings that will convey the offsite flows. Additional culvert crossings for local on-site flows will be sized during final design. The culverts conveying runoff in the two previously mentioned large off-site washes were sized as part of the proposed conditions HEC-RAS model to be 3 – 6-ft x 5-ft reinforced concrete box culverts and 4 – 36-in pipe culverts for the flows coming from the north and east, respectively. An additional culvert crossing was sized for off-site flows being conveyed from east across the southern portion of the site. This culvert was sized to be 1 – 36-in pipe in HY-8. See **Appendix D** for HY-8 output

The site will be protected from off-site runoff that crosses Alma School Road and overtops the Pinnacle Vista Drive channel by drainage channels and swales that will direct the runoff to the large off-site washes. Sizing of these channels and swales will be completed during final design

3.3 Stormwater Storage Requirements

On-site first flush basins will be provided for the development of the Site. First flush basin will be designed to detain the first half inch of runoff generated on the site. First flush basins will discharge into the existing central wash on-site. Discharge will be controlled with a bleed off pipe with attached orifice plate and overflow weir if necessary. The first flush basins will be used to attenuate post development flows leaving the site. Additional basin size beyond the required first flush volume may be required to achieve pre vs. post compliance. See Figure 4 – Preliminary Drainage Plan Map for first flush basin locations and post development flows. For complete proposed condition hydrology and hydraulics see **Appendix C** and **Appendix D** respectively.

3.4 Erosion Setback Analysis

A Level I erosion setback analysis was performed on the two large off-site washes. The analysis followed the requirement in the Arizona Department of Water Resources (ADWR) State Standard Attachment 5-96. Based on this analysis, a setback of 22 feet

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4.0 DATA ANALYSIS METHODS

4.1 General Discussion

Detailed hydrologic and hydraulic models were prepared for the two large washes that traverse the Site. The sections below provide the hydrology and hydraulic methodology.

As mentioned in Section 2.3, the Quisana Conceptual Drainage Report states that the Troon North Park outlet channel north of Pinnacle Vista Drive has the capacity to convey the 100-year storm runoff. The channel capacity was analyzed and determined that approximately 200 cfs will overtop the channel in the 100-year storm event. This revision has been reflected in the hydrology and hydraulic analysis.

4.2 Hydrology

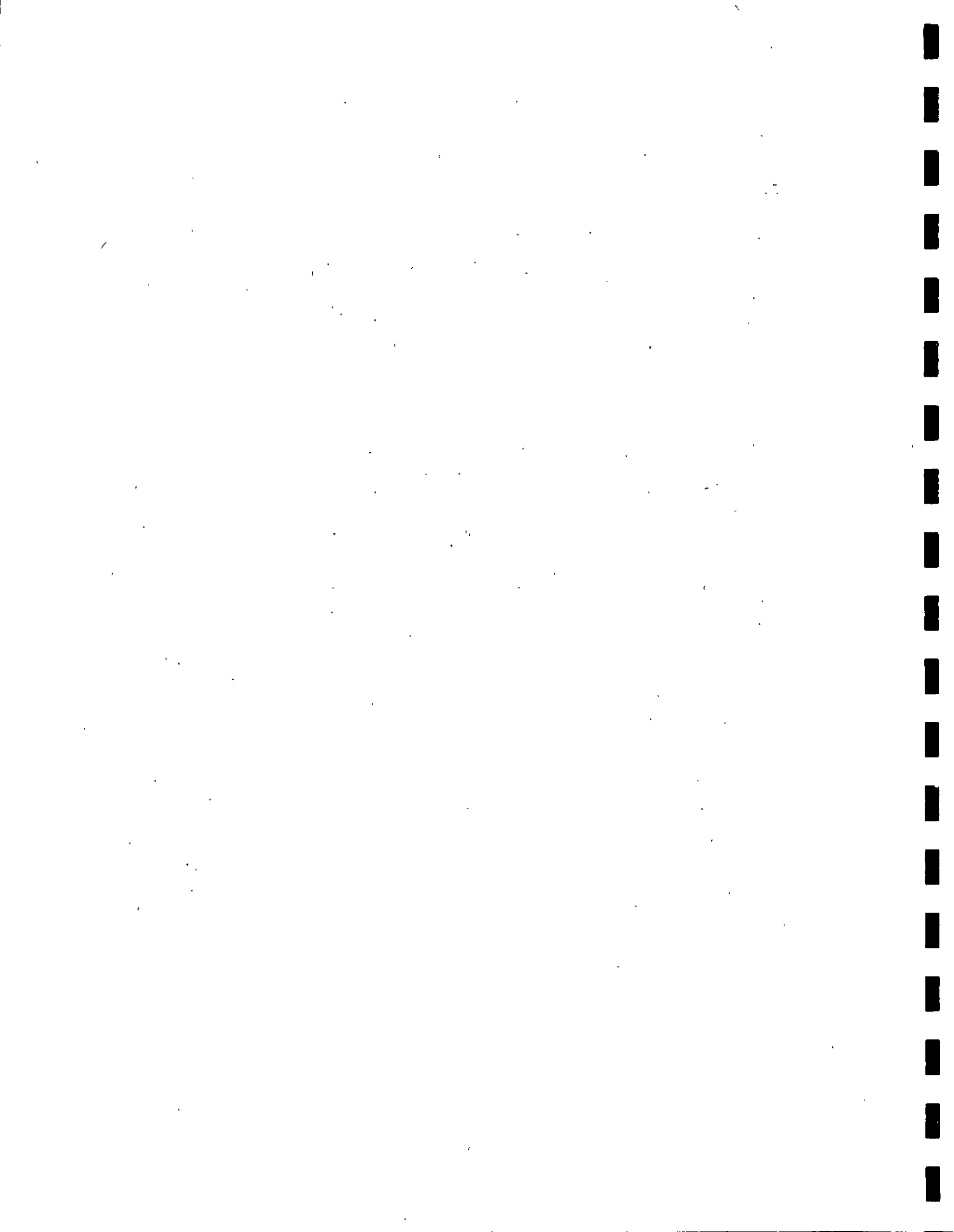
The U.S. Army Corps of Engineers HEC-1 hydrologic computer program was used to determine the 10- and 100-year peak discharges for off-site and on-site flows. HEC-1 models were prepared for the existing and proposed development conditions based on the hydrology models prepared for Troon North Park. The Troon North Park models contain the drainage areas north of Pinnacle Vista Road, including the regional detention facilities. The Drainage Design Management System for Windows (DDMSW) program was used to develop the hydrologic parameters for the on-site drainage areas and off-site drainage areas east of the Site. Green and Ampt rainfall loss parameters were estimated using DDMSW and the Flood Control District of Maricopa County (FCDMC) Drainage Design Manual – Hydrology. The HEC-1 models were prepared using the Clark Unit Hydrograph. Rainfall depths were not changed from the Troon North Park hydrology study.

Four different soil types were identified for the on-site and eastern off-site watersheds using the web soil survey from the National Resource Conservation Service (NRCS). A map showing the different soil types is shown in **Appendix C**. A list of the soils found in the watershed is shown below:

- Anthony-Arizo complex
- Gran-Wickenburg complex, 1 to 10 percent slopes
- Gran-Wickenburg-Rock outcrop complex, 1 to 7 percent slopes
- Pinaleno-Tres Hermanos complex, 1 to 10 percent slopes

Land use types for the HEC-1 models were based on the DS&PM and DDMSW codes. The proposed development is a combination of Zones R-4, R1-10 and R1-190. Land use maps for the existing and proposed development conditions are provided in **Appendix C**.

As discussed in Section 2.3, the Troon North Park outlet channel north of Pinnacle Vista Drive does not have the capacity to convey the 100-year runoff. A diversion record was added to the upstream portion of the model to model the conditions of the channel more accurately. The capacity of the outlet channel was calculated with FlowMaster at the most constricted location of the channel based on the detailed mapping obtained for this project. FlowMaster output is provided in **Appendix C**.



4.3 Hydraulics

The existing 100-year BFEs for the two large on-site washes were established using the U.S. Army Corps of Engineers HEC-RAS (v4.1.1) computer program. Cross sections were cut for the existing wash using the detailed mapping obtained for this project. The hydraulic models were run using sub-critical conditions with the normal depth boundary condition. In the proposed conditions model, culvert crossings at the proposed local road crossings were modeled. Expansion and contraction coefficients were determined from Table 3-2 of the HEC-RAS Hydraulic Reference Manual. Manning's 'n' coefficients for the channels vary from 0.035 to 0.045 and values for the overbanks are 0.050. Based on field observations and aerial photography the large wash is an undisturbed natural desert with an impervious weed barrier. Refer to **Appendix D** for the results of the hydraulic modeling.

4.4 Stormwater Storage Method

The existing property is a part of the Environmentally Sensitive Lands Ordinance (ESLO). Based on new City ordinances, a waiver will need to be obtained for any volume less than the 100-year, 2-hour volume. However, there is no waiver fee associated with the volumes that do not result in an increase in downstream runoff. See **Appendix E** for a copy of the waiver. Refer to **Appendix C** for the pre- and post-development hydrologic model results.



5.0 CONCLUSION

- 1) Two large washes cross the development. Impacts to the washes include two roadway crossing and encroachment from proposed development. A hydraulic model for the existing and proposed conditions was prepared to show the 100-year inundation limits and establish BFEs for the existing washes.
- 2) According to the Quisana Conceptual Drainage Report, the Troon North Park outlet channel north of Pinnacle Vista Drive has the capacity to convey the 100-year storm runoff. The channel capacity was analyzed and determined that approximately 200 cfs will overtop the channel in the 100-year storm event. Discussion with City staff indicate that the channel has experienced failures in the past and a solution to this offsite drainage issue will be presented as part of the preliminary plat.
- 3) Onsite runoff will be conveyed through the local streets, storm drains and culverts to the major wash corridors.
- 4) Seven detention basins are located on the development to control runoff exiting the property. The detention basins will use orifice plates and concrete weir structures to control post-development runoff.
- 5) Hydrologic models were prepared for the on-site and off-site areas for the pre- and post-development conditions.
- 6) The individual lots will be graded at a later date but will follow the patterns set forth in this report. The proposed pad elevations are established a minimum of one foot above the BFE in the large wash. The finished floors for the future structure will be established as part of the single lot grading plans.
- 7) A Level I Erosion Setback analysis was performed on the two large washes conveying off-site runoff. Locations where the setback is located within a lot will require an erosion cutoff wall. The erosion cutoff walls will be designed during final design.



6.0 REFERENCES

City of Scottsdale, *Addendum to the Preliminary Drainage Report for the Rocks at Reata Pass*, prepared by Gannett Fleming, Inc., revised August 2001.

City of Scottsdale, *Conceptual Drainage Report for Quisana*, prepared by Southwest Land Consulting, P.C., July 2005

City of Scottsdale, *Design Standards and Policies Manual*, January 2010

City of Scottsdale, *Troon North Park Hydrology Study Addendum*, prepared by Argus Consulting, September 2011

Federal Emergency Management Agency, *Flood Insurance Rate Map Panel No. 04013C1330L*, October 16, 2013

Flood Control District of Maricopa County, *Drainage Design Manual – Hydrology*, updated August 15, 2013

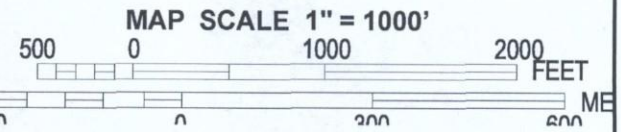
U.S Army Corps of Engineers, Hydrologic Engineering Center, *HEC-RAS, River Analysis System Hydraulic Reference Manual Version 4.1*, January 2010.

U.S. Army Corps of Engineers, Hydrologic Engineering Center, *HEC-RAS, River Analysis System User's Manual Version 4.1*, January 2010.



Appendix A – FLOOD INSURANCE RATE MAPS





NFIP

PANEL 1330L

NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP
MARICOPA COUNTY,
ARIZONA
 AND INCORPORATED AREAS

PANEL 1330 OF 4425
 (SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
SCOTTSDALE, CITY OF	045012	1330	L

Notice to User: The Map Number shown below should be used when placing map orders, the Community Number shown above should be used on insurance applications for the subject community.



MAP NUMBER
04013C1330L
MAP REVISED
OCTOBER 16, 2013

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov



Appendix B- EXCERPTS FROM OFFSITE DRAINAGE REPORTS

Addendum to the Preliminary Drainage Report for the Rocks at Reata Pass

Conceptual Drainage Report for Quisana

Troon North Park Hydrology Study and Addendum



REVISED 08/27/01

ADDENDUM TO THE

PRELIMINARY DRAINAGE REPORT

FOR

THE ROCKS

AT

REATA PASS

Monarch
Communities, LLC
SCOTTSDALE, ARIZONA



Prepared By:
Gannett Fleming, Inc.
3001 East Camelback Road, Suite 130
Phoenix, Arizona 85016-4498

GF Job Number 37500.013
JULY 2001

115 DR 2000 #2

1945

1. The first part of the report deals with the general situation in the country. It is noted that the economy is in a state of depression and that the government is facing a serious financial crisis. The report also mentions that the population is suffering from widespread poverty and unemployment.

2. The second part of the report discusses the political situation. It is noted that the government is weak and that there is a lack of political stability. The report also mentions that there are rumors of a coup d'état.

3. The third part of the report discusses the social situation. It is noted that there is a high level of social inequality and that the poor are suffering from discrimination. The report also mentions that there is a high level of crime and that the justice system is corrupt.

4. The fourth part of the report discusses the economic situation. It is noted that the economy is in a state of depression and that there is a high level of unemployment. The report also mentions that the government is facing a serious financial crisis.

5. The fifth part of the report discusses the military situation. It is noted that the military is weak and that there is a lack of military stability. The report also mentions that there are rumors of a coup d'état.

6. The sixth part of the report discusses the international situation. It is noted that the country is isolated and that there is a lack of international support. The report also mentions that the country is facing a serious diplomatic crisis.

7. The seventh part of the report discusses the future of the country. It is noted that the country is in a state of crisis and that there is a need for reform. The report also mentions that there are rumors of a coup d'état.



THE ROCKS AT REATA PASS

HEC-1 SUMMARY

	Pre-Development	Post-Development	Pre-Development	Post-Development	Pre-Development	Post-Development
	2yr-6hr	2yr-6hr	10yr-6hr	10yr-6hr	100yr-6hr	100yr-6hr
Det Basin B						
Runoff Curve Number	88	88	88	88	88	88
Percent Impervious	-	27	-	27	-	27
Total Runoff	6	9	13	15	22	24
Storage Available(CUFT)	-	2047	-	2047	-	2047
Det Basin Outflow (CFS)	-	6	-	13	-	20

	Pre-Development	Post-Development	Pre-Development	Post-Development	Pre-Development	Post-Development
	2yr-6hr	2yr-6hr	10yr-6hr	10yr-6hr	100yr-6hr	100yr-6hr
Det Basin C						
Runoff Curve Number	88	88	88	88	88	88
Percent Impervious	-	27	-	27	-	27
Total Runoff	1	1	1	2	2	2
Storage Available(CUFT)	-	958	-	958	-	958
Det Basin Outflow (CFS)	-	1	-	1	-	2

	Pre-Development	Post-Development	Pre-Development	Post-Development	Pre-Development	Post-Development
	2yr-6hr	2yr-6hr	10yr-6hr	10yr-6hr	100yr-6hr	100yr-6hr
Det Basin F						
Runoff Curve Number	88	88	88	88	88	88
Percent Impervious	-	27	-	27	-	27
Total Runoff	2	2	3	4	6	6
Storage Available(CUFT)	-	2744	-	2745	-	2746
Det Basin Outflow (CFS)	-	1	-	3	-	4

	Pre-Development	Post-Development	Pre-Development	Post-Development	Pre-Development	Post-Development
	2yr-6hr	2yr-6hr	10yr-6hr	10yr-6hr	100yr-6hr	100yr-6hr
Det Basin I-3						
Runoff Curve Number	88	88	88	88	88	88
Percent Impervious	-	27	-	27	-	27
Total Runoff	6	8	13	15	21	23
Storage Available(CUFT)	-	1307	-	1307	-	1307
Det Basin Outflow (CFS)	-	7	-	13	-	20

ROUTED TO 2

	Pre-Development	Post-Development	Pre-Development	Post-Development	Pre-Development	Post-Development
	2yr-6hr	2yr-6hr	10yr-6hr	10yr-6hr	100yr-6hr	100yr-6hr
Det Basin I (Parking Lot)						
Runoff Curve Number	88	88	88	88	88	88
Percent Impervious	-	27	-	27	-	27
Total Runoff	7	9	18	17	28	25
Storage Available(CUFT)	-	1307	-	1307	-	1307
Det Basin Outflow (CFS)	-	7	-	7	-	7

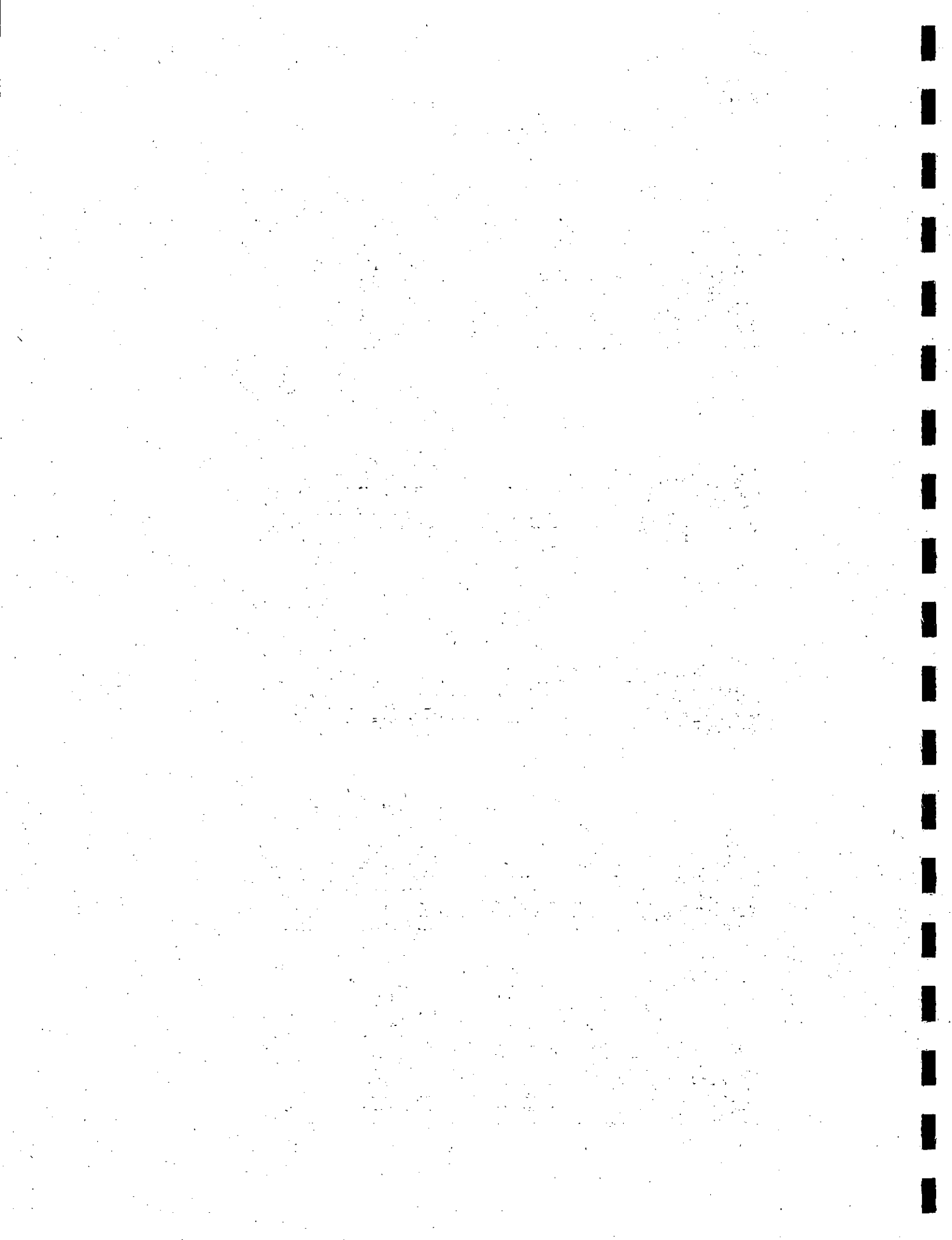
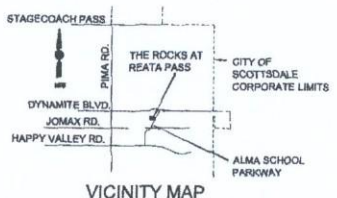


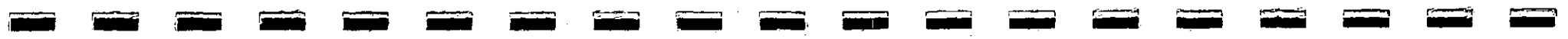
EXHIBIT 2



- LEGEND**
- DRAINAGE BASIN BOUNDARY
 - CONCENTRATION POINT
 - DIRECTION OF FLOW
 - PARCEL BOUNDARY

ONSITE DRAINAGE MAP

DATE	REVISED	BY
MONARCH COMMUNITIES, LLC. 3000 E. BERRY HILLS SUITE SCOTTSDALE, ARIZONA PHOENIX, ARIZONA 85251-4000		
PROJECT TITLE		
THE ROCKS AT REATA PASS		
SCALE	DATE	BY
1"=40'	8-27-01	
DATE OF ISSUE	PROJECT NO.	SHEET NO.
8-27-01	37500.013	1 of 1



I. INTRODUCTION

This parcel is a proposed single-family residence located at the intersection of Alma School Parkway and Pinnacle Vista Drive in Scottsdale, Arizona. The land is situated within a portion of Section 33, Township 5N, Range 5E, and is bounded by Pinnacle Vista Drive to the south, a fire station to the north, Alma School Parkway to the west and a future city park to the east (see Vicinity Map, Figure 1). This parcel is comprised of 13.0-acres zoned for S-R, Service Residential.

The land consists of natural desert with typical upper desert vegetation and slopes to the south at a gradient of approximately 2.5%. A small hill is located partially on the property with exposed rock along the east property line. Based on a field walk, an aerial photo and current topography (see Figures 2, 3 and 4) there is evidence of several washes which affect the proposed onsite development.

All onsite development has been designed to comply with the Drainage Regulations for the Flood Control District of Maricopa County (FCDMC) and the City of Scottsdale's Design Standards and Policies Manual (References 1, 2 and 3) while maintaining the original intent of the Troon North Drainage Master Plan.

II. FLOOD PLAIN DESIGNATION

The site is located within Zone "X" as shown on the FEMA Flood Insurance Rate Map 04013C1255F dated 6/19/01 (see Figure 5). Flood Zone "X" is defined as:

"areas of 500-year flood; areas of 100-year flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 100-year flood."

III. OFFSITE DRAINAGE

A drainage report was previously completed for this property, which will be the basis of the preliminary drainage design for this project (see Appendix B and reference 4). During the preliminary plat submittal a more detailed report will be completed as the design becomes more in depth. There are two natural washes that enter the site via culvert crossings beneath Alma School Parkway. The main wash to the north discharges from an existing 10'x3' RBC and the second discharges from 2-30" RCP's. Both of these washes intercept runoff from developments to the west and also contain storm water retention/detention facilities. Street runoff will enter the site within the two existing scuppers provided in Alma School Parkway at the north and south ends of the project. Based on the size of the second crossing and the 100-year peak discharge there may be considerable overtopping of Alma School Parkway near the south end of the project. Therefore, the proposed onsite development will minimize disturbance in this area to eliminate flooding onsite and an alteration of the

existing drainage patterns. During improvements to Pinnacle Vista Drive a channel was constructed along the north side of the paved surface and partially located on the subject parcel. This channel is an outlet for the existing dam located within the proposed City Park to the east. The channel was constructed



as a public improvement by the City of Scottsdale and will fully contain the 100-year peak discharge.

IV. PRE-DEVELOPED ONSITE CONDITIONS

Currently, the site is mostly natural desert with natural upper desert vegetation. However, several man-made structures have been added over the years in the form of a Troon North sign and vertical ramps for local bicycle riders. All of the mentioned items will be removed during the onsite development. A field walk revealed that the main wash (Wash 1) flowing diagonally thru the site is showing signs of severe erosion just downstream of the box culvert outlet (see Figures 6 & 7). In this region Wash 1 has eroded to a depth of 3'-4' deep and 3' wide since the installation of the box culvert many years ago. One of the reasons for this issue is the box appears to have been installed below the natural grade without a positive outfall. Over time during the large storms a path for positive drainage was established via erosion and Wash 1 has returned to its historic location. The narrowing of the channel width has decreased the active floodway/floodplain which can result in inundation of the parcel beyond what occurred in the past. There seems to be evidence of this in the above average vegetation along this reach of the watercourse. Further downstream Wash 1 has returned to the characteristics of similar washes in the region, a sandy bottom with gradually sloping sides containing typical vegetation (see Figure 8). The secondary wash (Wash 2) to the south enters the site and meanders across the southwest corner. Wash 2 shows minor signs of erosion typical for the region but nothing unusual (see Figure 9). Evidence to demonstrate that overtopping of Alma School Parkway occurs due to an undersized crossing at this location is not apparent. Therefore, only during a major storm of considerable duration might this occur. Both washes exit the site via the provided wet crossings within Pinnacle Vista Drive and continue downstream to a confluence location just south of this project.

V. POST-DEVELOPED ONSITE CONDITIONS

A full retention waiver was submitted with the original layout for this parcel and will be requested with the latest design (see Appendix B and Reference 4). The current zoning district along with the large areas of wash and hillside conservation make this the most sensitive way to develop the parcel while maintaining the theme of the Troon North Master Plan. During the development of the adjacent parcels a portion of upstream contributing areas to this project were diverted into the future park site. The reduction in watershed allows this development to waive the storm water storage requirement without increasing post-development discharges above historic flow rates.

A. Lot Drainage

Lots will ultimately be graded to drain the rear yard to the front and into the street. A time of concentration of 10 minutes will be used for lot drainage for determining rainfall intensity during final design.



Troon North Park

Hydrology Study Addendum

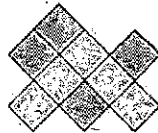


EXPIRES 12/31/12

prepared for:

City of Scottsdale
7447 E. Indian School Road
Scottsdale, Arizona 85251

prepared by:



ARGUS CONSULTING
A CIVIL ENGINEERING COMPANY

10115 E. Bell Road, Suite 107 - #104
Scottsdale, Arizona 85260
480 596 1131

September 2011



3.0 MAPPING AND SURVEYING INFORMATION

3.1 Mapping

The available mapping utilized in this study was as follows:

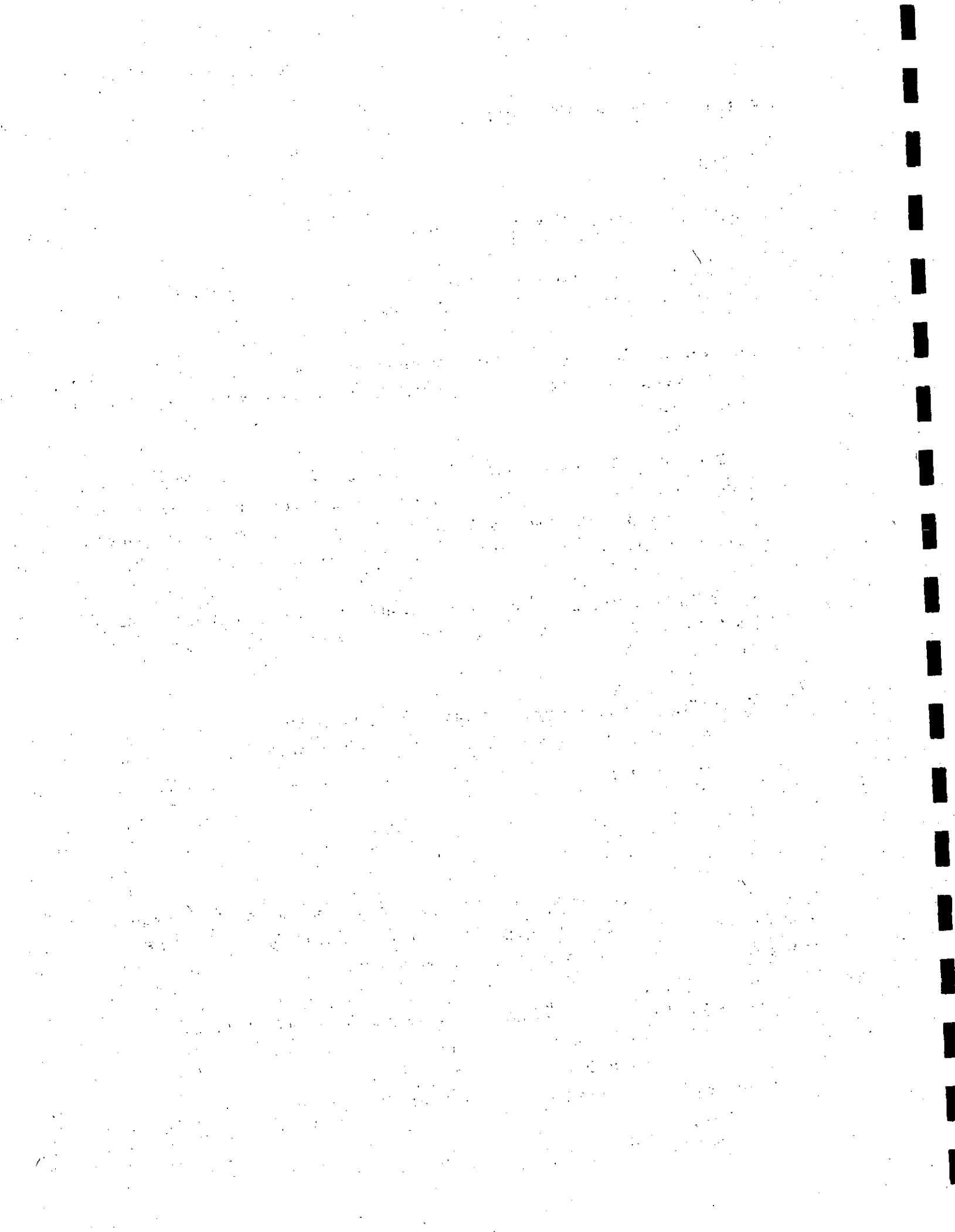
1. **Topographic Map:** City of Scottsdale digital mapping with contour of 1-foot was used for drainage areas to delineate watershed boundaries and subbasins.
2. **Aerial Photos:** City of Scottsdale aerial photos were used to provide vegetation cover patterns and identify existing flow path locations. They also help to verify land uses and densities and existing drainage infrastructure.
3. **SCS Soils Maps:** The Soil Survey of Aguila-Carefree Area (Ref.1) was used to identify the soils map units in each sub-basin. The general soil map of this survey shows one soil association over the study area. This association normally consists of one or more major soils and at least one minor soil, and is named for the major soil. The study area has the following soil map units:
 - Gran-Wickenburg complex: This map unit is about 40% Gran very gravelly sandy loam and 35% Wickenburg gravelly sandy loam. Included in this unit are small areas of Eba, Pinaleno, and Arizo soils, and Rock outcrops.
 - Pinaleno-Tres Hermanos complex: This map unit is about 45% Pinaleno very gravelly clay loam and 40% Tres Hermanos gravelly loam. Runoff is slow, and the hazard for water erosion slight.

4.0 HYDROLOGIC ANALYSIS

4.1 General

The hydrologic analyses presented for this report were developed using the current guidelines and procedures presented in the Drainage Design Manual for Maricopa County, Hydrology (Ref.2). The following outlines the design criteria used to develop the hydrologic analysis:

- ▶ Hydrologic calculations were completed for 2-, 10-, 50-, and 100-Year storm frequency.
- ▶ Storm duration of 6 hours was investigated.
- ▶ Storm distribution patterns for a 6-hour local storm were utilized.
- ▶ Green and Ampt method was used to estimate rainfall loss rates.



- ▶ Clark Unit Hydrograph method was utilized to generate hydrographs.
- ▶ The Normal Depth method was utilized for hydrologic routing.
- ▶ Reservoir routing was used to model detention basins.
- ▶ The U.S. Army Corps of Engineers HEC-1 computer program (Ref.3) was utilized for runoff calculations.

Appendix E contains input and output data for the HEC-1 models, hydrographs, and peak flow summary tables. Estimated peak flows impacting Pinnacle Vista Drive roadway are summarized below:

Table 4.1 – Peak Flow Summary at Pinnacle Vista Drive

	Concentration Point C042			Concentration Point C043		
	Peak Flow cfs	Time of Peak hr	Area Sq.Mi.	Peak Flow cfs	Time of Peak hr	Area Sq.Mi.
2-Yr/6-Hr	206	4.83	1.20	25	4.17	0.05
10-Yr/6-Hr	308	5.25	1.20	47	4.17	0.05
50-Yr/6-Hr	427	5.50	1.20	72	4.17	0.05
100-Yr/6-Hr	548	5.17	1.20	83	4.17	0.05

Table 4.2 – Combined Peak Flow at Pinnacle Vista Drive

	Concentration Point COMB		
	Peak Flow cfs	Time of Peak hr	Area Sq.Mi.
2-Yr/6-Hr	213	4.83	1.25
10-Yr/6-Hr	313	5.17	1.25
50-Yr/6-Hr	431	5.50	1.25
100-Yr/6-Hr	558	5.08	1.25

4.2 Hydrologic Parameters

4.2.1 Rainfall

The watershed area (less than 40 Square Miles) falls in the category of local storms. Therefore, a 6-Hour duration storm event was modeled to represent the local storm for this watershed. NOAA Precipitation-Frequency Atlas No.14 was used for obtaining rainfall data (Appendix A). Flood Control District of Maricopa County (FCDMC) is currently operating and maintaining a rain and a stream gage at Detention No.2 in Troon North Park. Appendix A also contains information on the precipitation gage data for the site.

4.2.2 Rainfall Losses

Rainfall losses are generally considered to be the result of evaporation of water from the land surface, interception of rainfall by vegetal cover, depression storage on the land surface (paved or unpaved), and the infiltration of water into the soil matrix (Ref.2). The selection of parameters to simulate rainfall losses was performed using the Drainage Design Management System for Windows (DDMSW) computer program available through FCDMC. Urban development was represented by adding a percentage of impervious cover to the drainage subbasin (Plate B). Results of parameters estimation can be found in Appendix B.

4.2.3 Hydrograph Generation

For this project the Clark Unit Hydrograph method was utilized for generation of hydrographs. Two numeric parameters, Time of Concentration (T_c) and Storage Coefficient (R), and a graphical parameter, the time-area relations were determined for each subbasin area using DDMSW computer program (Appendix B).

4.2.4 Runoff Routing

The Normal Depth routing method was selected for flow routing. This method is based on the principle of hydraulic diffusivity, which simulates an attenuation of the flood peak through the routing reach. This method can be used for both man-made and natural channels where overbank flow is expected, provided the conveyance can be accurately described with an eight-point cross section (Ref.2). Channel routing parameters were based on natural topography/channels (Plate C).

4.2.5 Reservoir Routing

Detention/Retention Basins located throughout the watershed were modeled using the Level Pool Reservoir Routing routines in the HEC-1 computer program. Data and Exhibits of modeled detention and retention basins can be found in Appendix C.

4.3 Troon North Park Regional Basins

Regional stormwater detention basins located within Troon North Park collect rainfall runoff from approximately 1.12 square miles, or 89.6%, of the study area. These basins temporarily store floodwaters by providing a controlled outflow to downstream waterways. The combined stormwater storage capacity at Troon North Park is 49.4 Ac-Ft (Ref.4).

4.4 Special Considerations

4.4.1 Jurisdictional Dam

In Arizona all dams, except those owned or operated by a government agency, are under the jurisdiction of the Arizona Department of Water Resources (ADWR). A detention or retention basin that impounds stormwater above the natural ground surface may be considered as being a dam under the authority of ADWR.

A "Jurisdictional Dam" is classified as a structure that is either 25 or more feet in height or has capacity to store more than 50 acre-feet. Height is measured as the vertical distance from the lowest point on the downstream toe (at natural ground) to the emergency spillway crest. Capacity is calculated as the maximum storage that can be impounded when there is no discharge of water.

Appendix C- HYDROLOGY

Parameters

Existing Conditions

Proposed Conditions

Troon North Park

Hydrology Study Addendum

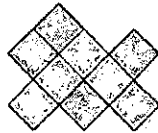


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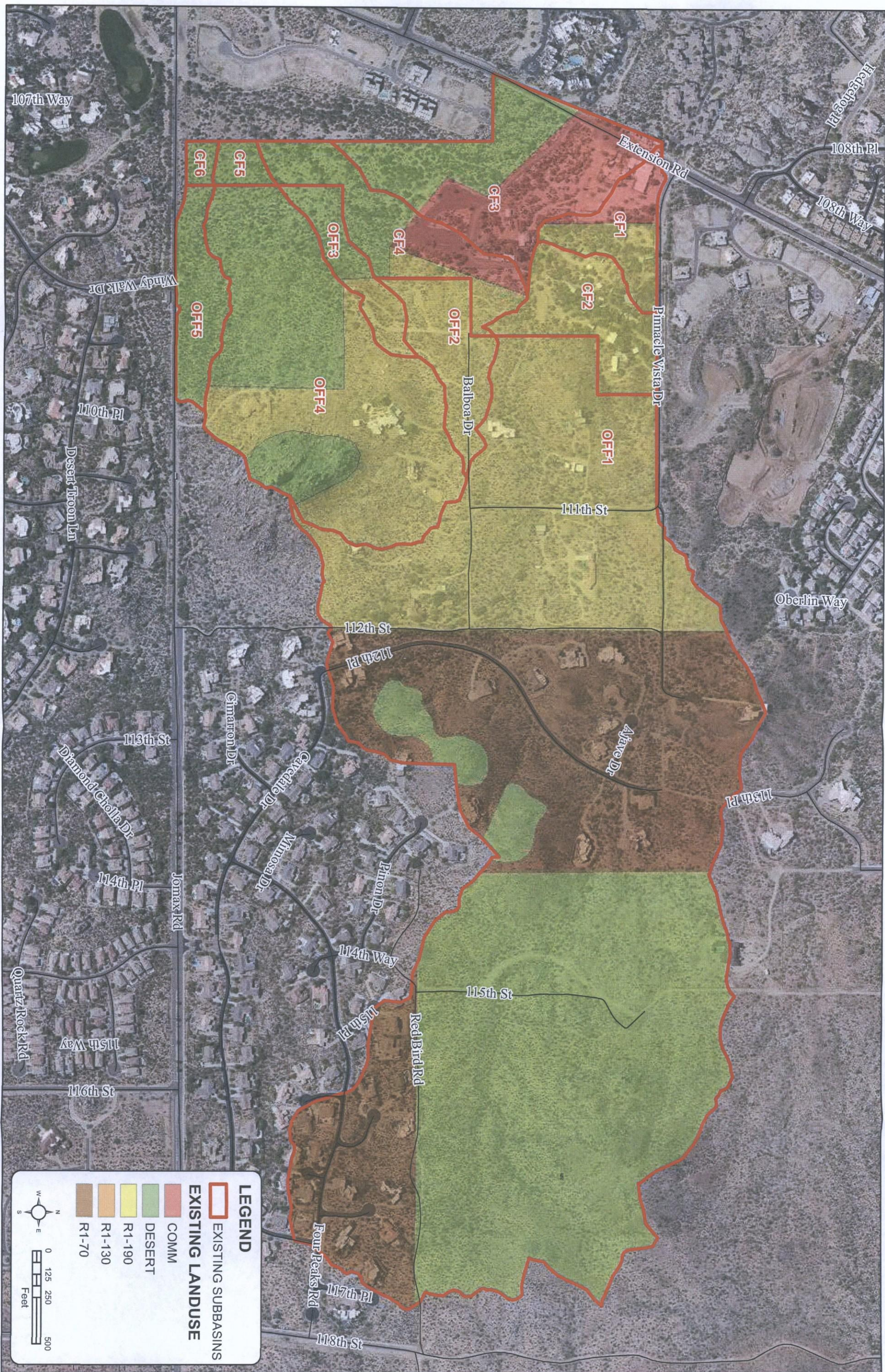
September 2011

Flood Control District of Maricopa County
 Drainage Design Management System
 RAINFALL DATA
 Project Reference: TROON NORTH PARK

ID	Method	Duration	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
DEFAULT	NOAA14	5 MIN	0.313	0.422	0.504	0.612	0.694	0.777
	NOAA14	10 MIN	0.476	0.642	0.767	0.932	1.057	1.182
	NOAA14	15 MIN	0.590	0.796	0.951	1.155	1.310	1.466
	NOAA14	30 MIN	0.795	1.072	1.281	1.556	1.764	1.974
	NOAA14	1 HOUR	0.984	1.326	1.585	1.925	2.183	2.443
	NOAA14	2 HOUR	1.128	1.496	1.778	2.159	2.448	2.746
	NOAA14	3 HOUR	1.198	1.560	1.847	2.247	2.564	2.891
	NOAA14	6 HOUR	1.417	1.796	2.100	2.518	2.845	3.183
	NOAA14	12 HOUR	1.701	2.134	2.479	2.946	3.306	3.675
	NOAA14	24 HOUR	2.072	2.702	3.212	3.936	4.524	5.147

City of Scottsdale
 Drainage Design Management System
 SOILS

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)
Major Basin ID: 01								
CF1	645	6	6456	0.000	4.30	0.62	-	100
	645	61	64561	0.009	95.70	0.15	-	100
CF2	645	6	6456	0.000	2.10	0.62	-	100
	645	61	64561	0.014	97.90	0.15	-	100
CF3	645	6	6456	0.007	22.00	0.62	-	100
	645	61	64561	0.016	54.30	0.15	-	100
	645	96	64596	0.007	23.70	0.07	-	100
CF4	645	6	6456	0.001	7.20	0.62	-	100
	645	61	64561	0.016	92.80	0.15	-	100
CF5	645	6	6456	0.000	8.70	0.62	-	100
	645	61	64561	0.002	91.30	0.15	-	100
CF6	645	6	6456	0.001	78.60	0.62	-	100
	645	61	64561	0.000	21.40	0.15	-	100
OFF1	645	61	64561	0.261	84.90	0.15	-	100
	645	63	64563	0.047	15.10	0.14	25.00	100
OFF2	645	61	64561	0.011	100.00	0.15	-	100
OFF3	645	61	64561	0.007	100.00	0.15	-	100
OFF4	645	61	64561	0.040	65.70	0.15	-	100
	645	63	64563	0.021	34.30	0.14	25.00	100
OFF5	645	6	6456	0.004	42.90	0.62	-	100
	645	61	64561	0.005	50.50	0.15	-	100
	645	63	64563	0.001	6.60	0.14	25.00	100



LEGEND

- EXISTING SUBBASINS
- EXISTING LANDUSE
- COMM
- DESERT
- R1-190
- R1-130
- R1-70

0 125 250 500
Feet

W N
E S

**CAVALLIRE FLATS
EXISTING CONDITIONS
LAND USE MAP**

SCALE(H): 1" = 500'
SCALE(V): N/A
DESIGNED BY: MAW
DRAWN BY: JAG
CHECKED BY: MAW
DATE: MAY 2014



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Engineering, Planning and Environmental Consultants
7740 North 16th Street, Suite 300
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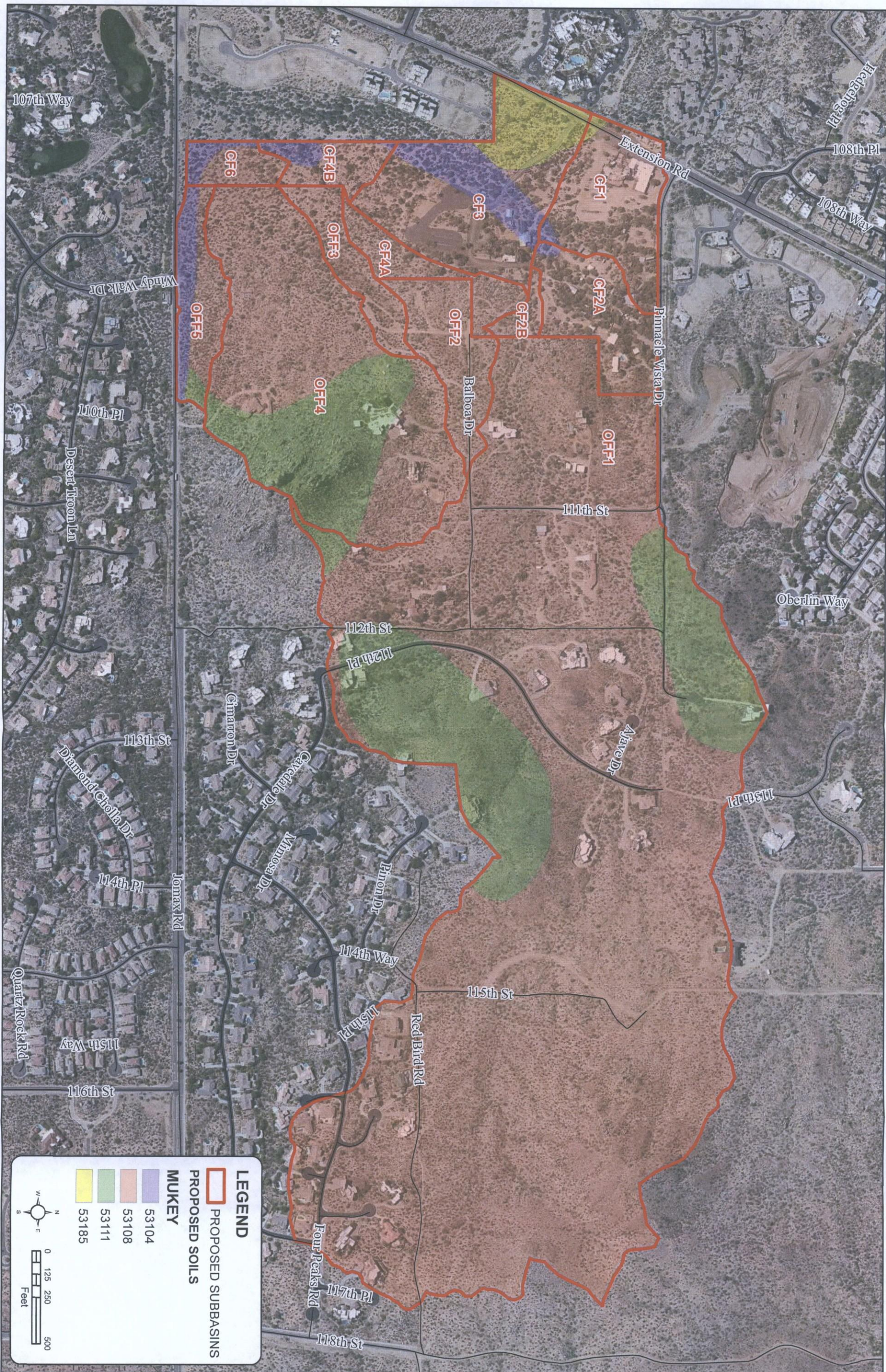
NO.	REVISION	BY	DATE	APPR.

PROJECT NO.
191060012
DRAWING NAME
Existing Land Use Map

City of Scottsdale
 Drainage Design Management System
 LAND USE
 Project Reference: CAVALLIERE FLATS-EX

Sub Basin	Land Use Code	Area (sq mi)	Area Initial Loss (IA) (%)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	
Major Basin ID: 01								
CF1	COMM R1-190	0.004	39.8	0.10	85	75.0	NORMAL	0.035
		0.006	60.2	0.30	6	20.0	NORMAL	0.035
		0.009	100.0					
CF2	R1-190	0.014	100.0	0.30	6	20.0	NORMAL	0.034
		0.014	100.0					
CF3	COMM DESERT	0.017	56.8	0.10	85	75.0	NORMAL	0.032
		0.013	43.2	0.35	0	25.0	DRY	0.062
		0.030	100.0					
CF4	COMM DESERT R1-190	0.005	26.8	0.10	85	75.0	NORMAL	0.034
		0.009	51.2	0.35	0	25.0	DRY	0.066
		0.004	22.0	0.30	6	20.0	NORMAL	0.034
		0.017	100.0					
CF5	DESERT	0.002	100.0	0.35	0	25.0	DRY	0.079
		0.002	100.0					
CF6	DESERT	0.001	100.0	0.35	0	25.0	DRY	0.083
		0.001	100.0					
OFF1	DESERT R1-190 R1-70	0.128	41.7	0.35	0	25.0	DRY	0.048
		0.072	23.5	0.30	6	20.0	NORMAL	0.026
		0.107	34.8	0.30	12	20.0	NORMAL	0.026
		0.308	100.0					
OFF2	R1-190	0.011	100.0	0.30	6	20.0	NORMAL	0.035
		0.011	100.0					
OFF3	DESERT R1-190	0.004	57.7	0.35	0	25.0	DRY	0.071
		0.003	42.3	0.30	6	20.0	NORMAL	0.036
		0.007	100.0					
OFF4	DESERT R1-190	0.030	49.3	0.35	0	25.0	DRY	0.058
		0.031	50.7	0.30	6	20.0	NORMAL	0.030
		0.061	100.0					
OFF5	DESERT	0.009	100.0	0.35	0	25.0	DRY	0.070
		0.009	100.0					

* Non default value



LEGEND

- PROPOSED SUBBASINS
- PROPOSED SOILS

MUKEY

- 53104
- 53108
- 53111
- 53185

0 125 250 500
Feet

W
N
E
S

**CAVALLIRE FLATS
PROPOSED CONDITIONS
SOILS USE MAP**

SCALE(H): 1"= 500'
SCALE(V): N/A
DESIGNED BY: MAW
DRAWN BY: JAG
CHECKED BY: MAW
DATE: MAY 2014

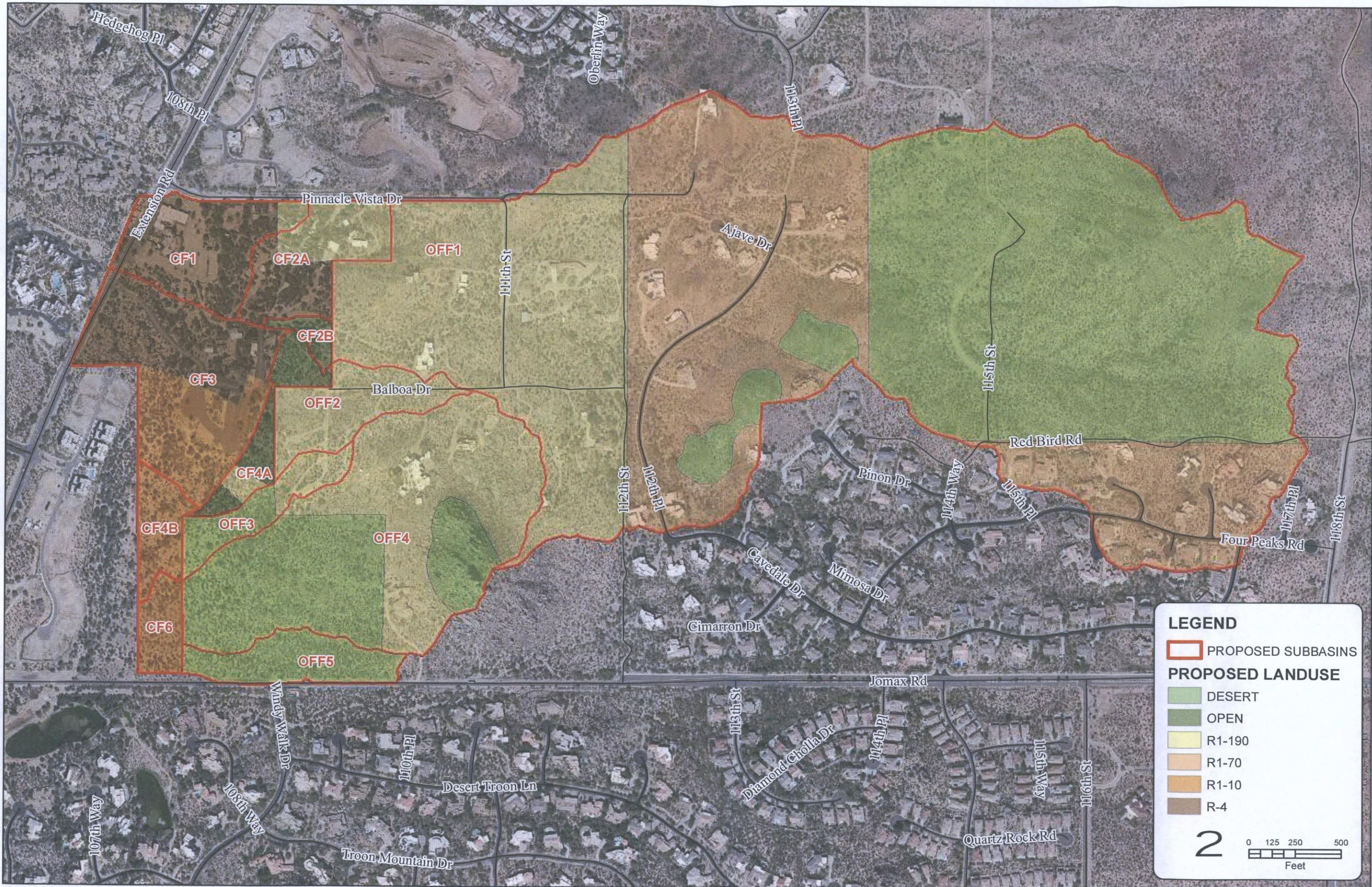
Kimley»Horn
2014 KIMLEY-HORN AND ASSOCIATES, INC.
Engineering, Planning and Environmental Consultants
7740 North 16th Street, Suite 300
Phoenix, Arizona 85020 (602) 944-5500

NO.	REVISION	BY	DATE	APPR.

PROJECT NO.: 191068012
DRAWING NAME: Proposed Soils.mxd

City of Scottsdale
 Drainage Design Management System
 SOILS

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)
Major Basin ID: 01								
CF1	645	6	6456	0.000	2.60	0.62	-	100
	645	61	64561	0.015	95.50	0.15	-	100
	645	96	64596	0.000	1.90	0.07	-	100
CF2A	645	6	6456	0.000	2.40	0.62	-	100
	645	61	64561	0.012	97.60	0.15	-	100
CF2B	645	61	64561	0.002	100.00	0.15	-	100
CF3	645	6	6456	0.007	23.00	0.62	-	100
	645	61	64561	0.015	53.20	0.15	-	100
	645	96	64596	0.007	23.80	0.07	-	100
CF4A	645	61	64561	0.007	100.00	0.15	-	100
CF4B	645	6	6456	0.001	24.50	0.62	-	100
	645	61	64561	0.004	75.50	0.15	-	100
CF6	645	6	6456	0.001	31.00	0.62	-	100
	645	61	64561	0.003	69.00	0.15	-	100
OFF1	645	61	64561	0.261	84.90	0.15	-	100
	645	63	64563	0.047	15.10	0.14	25.00	100
OFF2	645	61	64561	0.011	100.00	0.15	-	100
OFF3	645	61	64561	0.007	100.00	0.15	-	100
OFF4	645	61	64561	0.040	65.70	0.15	-	100
	645	63	64563	0.021	34.30	0.14	25.00	100
OFF5	645	6	6456	0.004	42.90	0.62	-	100
	645	61	64561	0.005	50.50	0.15	-	100
	645	63	64563	0.001	6.60	0.14	25.00	100



LEGEND

PROPOSED SUBBASINS
PROPOSED LANDUSE
 DESERT
 OPEN
 R1-190
 R1-70
 R1-10
 R-4

2

0 125 250 500
Feet

**CAVALLIERE FLATS
PROPOSED CONDITIONS
LAND USE MAP**

SCALE: 1" = 500'
 SCALE: 1/4" = 125'
 DESIGNED BY: MAW
 DRAWN BY: JAG
 CHECKED BY: MAW
 DATE: MAY 2014

Kimley»Horn

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 7740 North 16th Street, Suite 300
 Phoenix, Arizona 85020 (602) 944-5900
 Environmental Consultants
 Engineering, Planning and
 Environmental Consultants

NO.	REVISION	BY	DATE	APPR.

PROJECT NO.
 191069012
 DRAWING NAME
 Proposed_Landuse.mxd

City of Scottsdale
 Drainage Design Management System
 LAND USE
 Project Reference: CAVALLIERE FLATS-PR

Sub Basin	Land Use Code	Area (sq mi)	Area Initial Loss (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb
Major Basin ID: 01								
CF1	R1-190	0.001	5.8	0.30	6	20.0	NORMAL	0.034
	R-4	0.015	94.2	0.25	65	50.0	NORMAL	0.034
		0.016	100.0					
CF2A	R1-190	0.006	51.6	0.30	6	20.0	NORMAL	0.034
	R-4	0.006	48.4	0.25	65	50.0	NORMAL	0.034
		0.012	100.0					
CF2B	OPEN	0.002	100.0	0.10	0	10.0	DRY	0.039
		0.002	100.0					
CF3	R1-10	0.013	44.5	0.25	36	50.0	NORMAL	0.032
	R-4	0.016	55.5	0.25	65	50.0	NORMAL	0.032
		0.028	100.0					
CF4A	OPEN	0.006	84.6	0.10	0	10.0	DRY	0.036
	R1-190	0.001	15.4	0.30	6	20.0	NORMAL	0.036
		0.007	100.0					
CF4B	R1-10	0.005	100.0	0.25	36	50.0	NORMAL	0.037
		0.005	100.0					
CF6	R1-10	0.004	100.0	0.25	36	50.0	NORMAL	0.037
		0.004	100.0					
OFF1	DESERT	0.128	41.7	0.35	0	25.0	DRY	0.048
	R1-190	0.072	23.5	0.30	6	20.0	NORMAL	0.026
	R1-70	0.107	34.8	0.30	12	20.0	NORMAL	0.026
		0.308	100.0					
OFF2	R1-190	0.011	100.0	0.30	6	20.0	NORMAL	0.035
		0.011	100.0					
OFF3	DESERT	0.004	57.7	0.35	0	25.0	DRY	0.071
	R1-190	0.003	42.3	0.30	6	20.0	NORMAL	0.036
		0.007	100.0					
OFF4	DESERT	0.030	49.3	0.35	0	25.0	DRY	0.058
	R1-190	0.031	50.7	0.30	6	20.0	NORMAL	0.030
		0.061	100.0					
OFF5	DESERT	0.009	100.0	0.35	0	25.0	DRY	0.070
		0.009	100.0					

City of Scottsdale
 Drainage Design Management System
 SUB BASINS

Area ID	Sub Basin Parameters					Kb	Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area		IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr	
Major Basin ID: 01																		
CF1	0.016	0.23	168.9	168.9	URBAN	0.034	0.25	0.25	6.00	0.21	62	Tc (Hrs)	0.183*	0.176*	0.164*	0.151*	0.143*	0.136*
												Vel (f/s)	1.84	1.92	2.06	2.23	2.36	2.48
												R (Hrs)	0.183	0.176	0.162	0.148	0.139	0.132
CF2A	0.012	0.17	132.5	132.5	URBAN	0.034	0.28	0.25	5.85	0.20	35	Tc (Hrs)	0.184*	0.175*	0.161*	0.146*	0.137*	0.130*
												Vel (f/s)	1.36	1.42	1.55	1.71	1.82	1.92
												R (Hrs)	0.170	0.161	0.147	0.132	0.123	0.116
CF2B	0.002	0.10	153.1	153.1	URBAN	0.039	0.10	0.40	6.00	0.15		Tc (Hrs)	0.160*	0.151*	0.136*	0.120*	0.112*	0.105*
												Vel (f/s)	0.92	0.97	1.08	1.22	1.31	1.40
												R (Hrs)	0.266	0.249	0.221	0.193	0.178	0.167
CF3	0.028	0.22	119.4	119.4	URBAN	0.032	0.25	0.25	5.71	0.24	52	Tc (Hrs)	0.200*	0.192*	0.178*	0.163*	0.153*	0.146*
												Vel (f/s)	1.61	1.68	1.81	1.98	2.11	2.21
												R (Hrs)	0.141	0.135	0.125	0.113	0.106	0.100
CF4A	0.007	0.16	197.5	197.5	Natural	0.036	0.13	0.38	6.00	0.15	1	Tc (Hrs)	0.179*	0.169*	0.152*	0.135*	0.125*	0.118*
												Vel (f/s)	1.31	1.39	1.54	1.74	1.88	1.99
												R (Hrs)	0.214	0.201	0.178	0.156	0.144	0.135
CF4B	0.005	0.12	100.0	100.0	URBAN	0.037	0.25	0.25	5.24	0.30	36	Tc (Hrs)	0.179*	0.171*	0.157*	0.143*	0.134*	0.126*
												Vel (f/s)	0.98	1.03	1.12	1.23	1.31	1.40
												R (Hrs)	0.206	0.195	0.179	0.160	0.149	0.140
CF6	0.004	0.11	35.7	35.7	URBAN	0.037	0.25	0.25	5.05	0.33	36	Tc (Hrs)	0.237*	0.226*	0.208*	0.189*	0.177*	0.167*
												Vel (f/s)	0.68	0.71	0.78	0.85	0.91	0.97
												R (Hrs)	0.298	0.283	0.258	0.232	0.216	0.202
OFF1	0.307	1.20	141.3	141.3	Natural	0.035	0.32	0.31	6.00	0.17	9	Tc (Hrs)	0.542	0.512	0.462	0.409	0.379	0.356
												Vel (f/s)	3.25	3.44	3.81	4.30	4.64	4.94
												R (Hrs)	0.425	0.399	0.356	0.311	0.286	0.267
OFF2	0.011	0.23	219.8	218.0	Natural	0.035	0.30	0.25	6.00	0.17	6	Tc (Hrs)	0.204*	0.193*	0.173*	0.154*	0.143*	0.135*
												Vel (f/s)	1.65	1.75	1.95	2.19	2.36	2.50
												R (Hrs)	0.256	0.240	0.213	0.187	0.172	0.161

* Non default value or value out of range

City of Scottsdale
 Drainage Design Management System
 SUB BASINS

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters					
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin ID: 01																	
OFF3	0.007	0.21	240.5	233.7	Natural	0.056	0.33	0.34	6.00	0.17	3	Tc (Hrs) 0.257*	0.242*	0.217*	0.191*	0.176*	0.165*
												Vel (f/s) 1.20	1.27	1.42	1.61	1.75	1.87
												R (Hrs) 0.398	0.371	0.330	0.286	0.261	0.243
OFF4	0.061	0.53	337.1	274.7	Natural	0.044	0.32	0.32	6.00	0.17	12	Tc (Hrs) 0.326	0.308	0.279*	0.248*	0.230*	0.216*
												Vel (f/s) 2.38	2.52	2.79	3.13	3.38	3.60
												R (Hrs) 0.316	0.297	0.266	0.233	0.214	0.200
OFF5	0.009	0.26	245.1	236.8	Natural	0.070	0.35	0.35	4.65	0.33	2	Tc (Hrs) 0.357	0.329	0.292*	0.257*	0.237*	0.219*
												Vel (f/s) 1.07	1.16	1.31	1.48	1.61	1.74
												R (Hrs) 0.589	0.537	0.470	0.409	0.373	0.342

* Non default value or value out of range

City of Scottsdale
 Drainage Design Management System
 HEC-1 ROUTING DATA
 Project Reference: CAVALLIERE FLATS-PR

Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)		1.	2.	3.	4.	5.	6.	7.	8.
NORMAL DEPTH															
Major Basin 01															
RCF1	0.035	0.035	0.035	690.00	0.0200	-	X:	-	30.00	67.50	72.50	77.50	82.50	120.00	150.00
							Y:	3.00	2.50	2.00	-	-	2.00	2.50	3.00
RCF2A	0.035	0.035	0.035	990.00	0.0200	-	X:	-	20.00	40.00	45.00	55.00	60.00	80.00	100.00
							Y:	2.00	1.50	1.00	-	-	1.00	1.50	2.00
RCF2B	0.035	0.035	0.035	530.00	0.0270	-	X:	-	15.00	30.00	35.00	90.00	95.00	110.00	125.00
							Y:	2.00	1.50	1.00	-	-	1.00	1.50	2.00
RCF3	0.035	0.035	0.035	795.00	0.0380	-	X:	-	35.00	80.50	100.00	106.00	125.50	565.00	600.00
							Y:	5.00	4.00	3.00	-	-	3.00	4.00	5.00
RCF4A	0.035	0.035	0.035	1,230.00	0.0250	-	X:	-	25.00	50.00	55.00	65.00	70.00	95.00	120.00
							Y:	3.00	2.50	2.00	-	-	2.00	2.50	3.00
RCF6	0.035	0.035	0.035	270.00	0.0300	-	X:	-	50.00	100.00	105.00	115.00	120.00	170.00	220.00
							Y:	1.00	0.75	0.50	-	-	0.50	0.75	1.00

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 02JUN14 TIME 14:32:05
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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Cavalliere Flats Rezoning									
2	ID	City of Scottsdale									
3	ID	Prepared by: Kimley-Horn and Associates, Inc.									
4	ID										
5	ID	Existing Conditions model									
6	ID	10-year, 6-hour Storm event									
7	ID										
8	ID	Based on Troon North Park Model:									
9	ID										
10	ID	PROJECT NAME: TROON NORTH PARK					JOB#: 236-08				
11	ID	PREPARED FOR: CITY OF SCOTTSDALE									
12	ID	PREPARED BY : ARGUS CONSULTING, P.C.									
13	ID										
14	ID	FILE NAME : TNP10YR.DAT									
15	ID	STORM EVENT : 10-YR/6-HR									
16	ID	Unit Hydrograph: Clark									
17	ID	01/24/2011									
18	IT	5	06JUN99	0000	2000						
19	IN	15									
20	IO	5									
	*DIAGRAM										
21	KK	001	BASIN								
22	BA	0.112									
23	PB	2.071									
24	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.075
25	PC	0.087	0.100	0.119	0.151	0.235	0.415	0.762	0.873	0.915	0.944
26	PC	0.956	0.967	0.979	0.989	1.000					
27	LG	0.30	0.25	6.00	0.18	3					
28	UC	0.344	0.316								
29	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
30	UA	100									
31	KK	002	BASIN								
32	BA	0.0167									
33	LG	0.30	0.25	6.00	0.18	7					
34	UC	0.262	0.408								
35	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
36	UA	100									
37	KK	LP002	ROUTE INFLOW THRU STORAGE AREA								
38	KM	SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS									
39	KM	LOW FLOW OUTLET = 30" RCP									
40	RS	1	STOR 0.0								
41	SA	.003	.005	.026	.059						
42	SQ	0	5	15	30						
43	SE	2722.0	2723.0	2724.0	2725.0						

1

HEC-1 INPUT

PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
44	ST	2725.0	40	2.7	1.5						
	*										
45	KK	C001 COMBINE HYDROGRAPHS FROM SUB 001 AND LP002									
46	HC	2									
	*										
47	KK	LPC001	ROUTE INFLOW THRU STORAGE AREA								
48	KM	SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS									
49	KM	LOW FLOW OUTLET - OPEN SECTION BETWEEN BERM AND WALL									
50	RS	1	STOR	0.0							
51	SA	.001	.004	.013	.104	.150					
52	SQ	0	6.5	23	50	90					
53	SE	2713.0	2714.0	2715.0	2716.0	2717.0					
54	ST	2717.0	45	2.7	1.5						
	*										
55	KK	RLPC01	ROUTE FLOW THRU SUB 007								
56	RS	1	FLOW	0							
57	RC	0.070	0.050	0.070	1736	0.025					
58	RX	1000	1001	1034	1047.5	1052.5	1066	1099	1100		
59	RY	100	95.5	94.5	90	90	94.5	95.5	100		
	*										
60	KK	007	BASIN								
61	BA	0.018									
62	LG	0.27	0.25	6.00	0.20	25					
63	UC	0.241	0.340								
64	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
65	UA	100									
	*										
66	KK	C007	COMBINE HYDROGRAPHS FROM SUB 007 AND RLPC01								
67	HC	2									
	*										
68	KK	LPC007	ROUTE INFLOW THRU STORAGE AREA								
69	KM	SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS									
70	KM	LOW FLOW OUTLET = 2-36" RCP									
71	RS	1	STOR	0.0							
72	SA	.001	.002	.004	.025	.040	.060	.100			
73	SQ	0	18	35	70	100	125	143			
74	SE	68.0	69.0	70.0	71.0	72.0	73.0	74.0			
75	ST	74.0	45	2.7	1.5						
	*										
76	KK	003	BASIN								
77	BA	0.018									
78	LG	0.30	0.25	6.00	0.18	2					
79	UC	0.206	0.235								
80	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
81	UA	100									
	*										

1

HEC-1 INPUT

PAGE 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
82	KK	R003	ROUTE FLOW THRU SUB 005								
83	RS	1	FLOW	0							
84	RC	0.070	0.050	0.070	470	0.023					
85	RX	1000	1001	1045.5	1050	1055	1059.5	1099	1100		
86	RY	100	92.5	91.5	90	90	91.5	92.5	100		
	*										
87	KK	005	BASIN								
88	BA	0.007									
89	LG	0.27	0.25	6.00	0.20	23					
90	UC	0.171	0.213								
91	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
92	UA	100									
	*										
93	KK	C005	COMBINE HYDROGRAPHS FROM SUB 005 AND R003								
94	HC	2									
	*										
95	KK	RC005	ROUTE FLOW THRU SUB 006								
96	RS	1	FLOW	0							
97	RC	0.070	0.050	0.070	450	0.022					
98	RX	1000	1001	1045.5	1050	1055	1059.5	1099	1100		
99	RY	100	92.5	91.5	90	90	91.5	92.5	100		
	*										
100	KK	LPC005	ROUTE INFLOW THRU STORAGE AREA								
101	KM	SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS									
102	KM	LOW FLOW OUTLET = 2-24" RCP									
103	RS	1	STOR	0							
104	SA	.001	.002	.030	.090	.190	.271				

167 KK 008 BASIN
 168 BA 0.003
 169 LG 0.27 0.25 6.00 0.20 25
 170 UC 0.181 0.386
 171 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 172 UA 100
 *

173 KK 010 BASIN
 174 BA 0.003
 175 LG 0.27 0.25 6.00 0.20 19
 176 UC 0.155 0.219
 177 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 178 UA 100
 *

179 KK C010 COMBINE HYDROGRAPHS FROM SUB 008 AND SUB 010
 180 HC 2
 *

181 KK RC010 ROUTE FLOW THRU SUB 011
 182 RS 1 FLOW 0
 183 RC 0.070 0.050 0.070 390 0.020
 184 RX 1000 1001 1039 1045 1055 1061 1099 1100
 185 RY 100 93 92 90 90 92 93 100
 *

186 KK 011 BASIN
 187 BA 0.005
 188 LG 0.26 0.25 6.00 0.20 25
 189 UC 0.168 0.205
 190 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 191 UA 100
 *

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

192 KK C011 COMBINE HYDROGRAPHS FROM SUB 011 AND RC010
 193 HC 2
 *

194 KK C013I COMBINE HYDROGRAPHS C011, LP012 AND LPC007
 195 HC 3
 *

196 KK LPC131 ROUTE INFLOW THRU STORAGE AREA
 197 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 198 KM LOW FLOW OUTLET = 10'w x 3'h RCB
 199 RS 1 STOR 0
 200 SA .001 .005 .029 .082 .216 .432
 201 SQ 0 40 90 150 210 266
 202 SE 61.0 62.0 63.0 64.0 65.0 66.0
 203 ST 66.0 45 2.7 1.5
 *

204 KK 013 BASIN
 205 BA 0.041
 206 LG 0.26 0.25 6.00 0.20 27
 207 UC 0.255 0.255
 208 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 209 UA 100
 *

210 KK C013 COMBINE HYDROGRAPHS FROM SUB 013 AND LPC131
 211 HC 2
 *

212 KK LPC013 ROUTE INFLOW THRU STORAGE AREA
 213 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 214 KM LOW FLOW OUTLET = 10'w x 3'h RCB
 215 RS 1 STOR 0
 216 SA .001 .003 .005 .024 .100 .355
 217 SQ 0 40 100 160 220 272
 218 SE 55.8 57.0 58.0 59.0 60.0 61.0
 219 ST 61.0 45 2.7 1.5
 *

220 KK RLPC13 ROUTE FLOW THRU SUB 018
 221 RS 1 FLOW 0
 222 RC 0.045 0.040 0.045 2178 0.020
 223 RX 1000 1001 1010 1022 1032 1044 1053 1054
 224 RY 95.5 95.0 94.0 90.0 90.0 94.0 95.0 95.5
 *

225 KK 014 BASIN
 226 BA 0.018
 227 LG 0.26 0.25 6.00 0.20 25
 228 UC 0.197 0.179
 229 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 230 UA 100

291 KK 018 BASIN
 292 BA 0.050
 293 LG 0.22 0.21 6.40 0.18 33
 294 UC 0.277 0.277
 295 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 296 UA 100
 *

297 KK C018 COMBINE HYDROGRAPHS FROM SUB 018, RC017 AND RLPC13
 298 KM THIS IS TNP INFLOW FROM ALMA SCHOOL RD. CULVERT
 299 HC 3
 *

300 KK 019 BASIN
 301 BA 0.050
 302 LG 0.26 0.25 6.00 0.20 57
 303 UC 0.238 0.215
 304 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 305 UA 100
 *

HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

306 KK 020 BASIN
 307 BA 0.016
 308 LG 0.28 0.25 6.00 0.20 18
 309 UC 0.298 0.566
 310 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 311 UA 100
 *

312 KK C020 COMBINE HYDROGRAPHS FROM SUB 019 AND SUB 020
 313 HC 2
 *

314 KK RC020 NOTE: "n" VALUE AND GEOMETRY MODIFIED FROM PRE-DEVELOPED MODEL
 315 KM TO REFLECT EXISTING DEVELOPMENT.
 316 RS 1 FLOW 0
 317 RC 0.070 0.040 0.070 880 0.018
 318 RX 1000 1001 1034 1047.5 1052.5 1066 1099 1100
 319 RY 100 95.5 94.5 90 90 94.5 95.5 100
 *

320 KK 021 BASIN
 321 BA 0.024
 322 LG 0.26 0.25 6.00 0.20 56
 323 UC 0.225 0.250
 324 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 325 UA 100
 *

326 KK C021 COMBINE HYDROGRAPHS FROM SUB 021 AND RC020
 327 KM THIS IS TNP INFLOW FROM C021
 328 HC 2
 *

329 KK 022 BASIN
 330 BA 0.067
 331 LG 0.29 0.25 6.00 0.19 10
 332 UC 0.318 0.364
 333 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 334 UA 100
 *

335 KK LP022 ROUTE INFLOW THRU STORAGE AREA
 336 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 337 KM LOW FLOW OUTLET = 4-24" RCP
 338 RS 1 STOR 0
 339 SA .001 .004 .009 .011 .013 .060 .140
 340 SQ 0 17 48.5 79.5 97.0 113 127
 341 SE 71.0 72.0 73.0 74.0 75.0 76.0 77.0
 342 ST 77.0 45 2.7 1.5
 *

HEC-1 INPUT

PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

343 KK 023 BASIN
 344 BA 0.063
 345 LG 0.27 0.25 6.00 0.20 22
 346 UC 0.276 0.261
 347 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 348 UA 100
 *

349 KK LP023 ROUTE INFLOW THRU STORAGE AREA
 350 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 351 KY LOW FLOW OUTLET = 3-30" RCP
 352 RS 1 STOR 0
 353 SA .001 .052 .288 .518 .751

354	SQ	0	15	48	84	113			
355	SE	73.0	74.0	75.0	76.0	77.0			
356	ST	77.0	45	2.7	1.5				
	*								
357	KK	C023 COMBINE HYDROGRAPHS FROM LP022 AND LP023							
358	HC	2							
	*								
359	KK	RLPC23							
360	RS	2	FLOW	0					
361	RC	0.070	0.050	0.070	1235	0.022			
362	RX	1000	1001	1034	1047.5	1052.5	1066	1099	1100
363	RY	100	95.5	94.5	90	90	94.5	95.5	100
	*								

364	KK	024	BASIN						
365	BA	0.017							
366	LG	0.28	0.25	6.00	0.19	35			
367	UC	0.217	0.243						
368	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0
369	UA	100							94.0
	*								97.0

370 KK C024 COMBINE HYDROGRAPHS FROM SUB 024 AND RLPC23
 371 HC 2
 *

372	KK	LPC024	ROUTE INFLOW THRU STORAGE AREA						
373	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS						
374	KM		LOW FLOW OUTLET = NONE						
375	RS	1	STOR	0					
376	SA	.108	.346	.566	.815	1.05			
377	SQ	0	0	0	56	230			
378	SE	45.0	46.0	47.0	48.0	49.0			
	*								

HEC-1 INPUT

PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

379	KK	RLPC24	NOTE: "n" VALUE AND GEOMETRY MODIFIED FROM PRE-DEVELOPED MODEL						
380	KM		TO REFLECT EXISTING DEVELOPMENT.						
381	RS	3	FLOW	0					
382	RC	0.070	0.040	0.070	1244	0.022			
383	RX	1000	1001	1022.0	1040	1055	1073.0	1099	1100
384	RY	100	99.0	96.0	90	90	96.0	99.0	100
	*								

385	KK	025	BASIN						
386	BA	0.029							
387	LG	0.26	0.25	6.00	0.20	57			
388	UC	0.227	0.263						
389	JA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0
390	JA	100							94.0
	*								97.0

391 KK C025 COMBINE HYDROGRAPHS FROM SUB 025 AND RR
 392 KM THIS TNP INFLOW FROM C025
 393 HC 2
 *

394	KK	026	BASIN						
395	BA	0.039							
396	LG	0.28	0.25	6.00	0.19	11			
397	UC	0.229	0.225						
398	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0
399	UA	100							94.0
	*								97.0

400	KK	LP026	ROUTE INFLOW THRU STORAGE AREA						
401	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS						
402	KM		LOW FLOW OUTLET = NONE						
403	RS	1	STOR	0					
404	SA	.142	.186	.210	.238				
405	SQ	0	0	0	78				
406	SE	2700.0	2701.0	2702.0	2703.0				
	*								

407	KK	RLP026							
408	RS	4	FLOW	0					
409	RC	0.070	0.050	0.070	2870	0.024			
410	RX	1000	1001	1045.5	1050	1055	1059.5	1099	1100
411	RY	100	92.5	91.5	90	90	91.5	92.5	100
	*								

412	KK	028	BASIN						
413	BA	0.004							
414	LG	0.30	0.25	6.00	0.20	9			
415	UC	0.120	0.130						
416	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0
417	UA	100							94.0
	*								97.0


```

*
482 KK C032 COMBINE HYDROGRAPHS FROM SUB 032 AND RC031
483 HC 2
*

484 KK LPC032 ROUTE INFLOW THRU STORAGE AREA
485 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
486 KM LOW FLOW OUTLET - 42" RCP
487 RS 1 STOR 0
488 SA .001 .007 .070 .136 .270
489 SQ 0 11 23 46 63
490 SE 77.0 78.0 79.0 80.0 81.0
491 ST 81.0 45 2.7 1.5
*
    
```

1 HEC-1 INPUT PAGE 14

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

492 KK RLPC32
493 KM ROUTE FLOW THRU SUB 033
494 RS 4 FLOW 0
495 RC 0.070 0.050 0.070 2176 0.021
496 RX 1000 1001 1039 1045 1055 1061 1099 1100
497 RY 100 93 92 90 90 92 93 100
*
    
```

```

498 KK 033 BASIN
499 BA 0.147
500 LG 0.27 0.25 6.00 0.20 36
501 UC 0.338 0.277
502 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
503 UA 100
*
    
```

```

504 KK C033 COMBINE HYDROGRAPHS FROM SUB 033, RLPC32 AND RLP026
505 HC 3
*
    
```

```

506 KK 034 BASIN
507 BA 0.093
508 TG 0.30 0.25 6.00 0.18 1
509 UC 0.272 0.231
510 JA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
511 JA 100
*
    
```

```

512 KK R034 ROUTE FLOW THRU SUB 035
513 RS 1 FLOW 0
514 RC 0.045 0.040 0.045 2178 0.020
515 RX 1000 1001 1010 1022 1032 1044 1053 1054
516 RY 95.5 95.0 94.0 90.0 90.0 94.0 95.0 95.5
*
    
```

```

517 KK 035 BASIN
518 BA 0.022
519 LG 0.30 0.23 6.20 0.17 5
520 UC 0.187 0.194
521 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
522 UA 100
*
    
```

```

523 KK 036 BASIN
524 BA 0.008
525 LG 0.29 0.23 6.20 0.18 23
526 UC 0.140 0.165
527 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
528 UA 100
*
    
```

1 HEC-1 INPUT PAGE 15

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

529 KK C036 COMBINE HYDROGRAPHS FROM SUB 035, SUB 036, C033 AND R034
530 KM THIS IS TNP INFLOW FROM C036
531 HC 4
*
    
```

```

532 KK 037 BASIN
533 BA 0.041
534 LG 0.30 0.23 6.20 0.17 1
535 UC 0.187 0.146
536 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
537 UA 100
*
    
```

```

538 KK C037 COMBINE ALL HYDROGRAPHS
539 KM THIS REPRESENTS TOTAL INFLOW TO BASIN No.1
540 HC 5
541 ZW A=TRON B=TNFLOW C=FLOW
*
    
```

542	KK	DET#1	ROUTE INFLOW THRU DET BASIN #1 (UPPER BASIN)									
543	KM		LOW FLOW OUTLET = 4-42" RCP									
544	KM		INLET INVERT ELEV = 2598.80									
545	KM		OUTLET INVERT ELEV = 2592.0									
546	KM		SPILLWAY ELEV = 2610.0									
547	KM		WEIR LENGTH = 50'									
548	RS	1	STOR 0.0									
549	SA	0.023	0.11	0.49	1.23	3.50	4.00	4.24	4.57	4.85	5.33	
550	SA	5.95	6.67	7.60	8.95							
551	SQ	0	45	103.5	185	264	328.5	382	428.5	470	507	
552	SQ	543	575	725	999							
553	SE	2599	2600	2601	2602	2603	2604	2605	2606	2607	2608	
554	SE	2609	2610	2611	2612							
555	ZW		A=TROON			B=DET#1		C=FLOW				

556	KK	DET#2	ROUTE INFLOW THRU DET BASIN #2 (LOWER BASIN)									
557	KM		LOW FLOW OUTLET = 1-60" RCP									
558	KM		INLET INVERT ELEV = 2590.36									
559	KM		OUTLET INVERT ELEV = 2589.95									
560	KM		SPILLWAY ELEV = 2607.0									
561	KM		WEIR LENGTH = 35'									
562	RS	1	STOR 0.0									
563	SA	0.023	0.048	0.496	0.876	1.119	1.277	1.414	1.558	1.705	1.820	
564	SA	1.930	2.042	2.365	2.487	2.615	2.770	2.935	3.258			
565	SQ	0	11.5	29	47	78	112.5	148	177	204	226	
566	SQ	248	266	283	300.5	315	330	345	378			
567	SE	2590.3	2591	2592	2593	2594	2595	2596	2597	2598	2599	
568	SE	2600	2601	2602	2603	2604	2605	2606	2607			
569	ST	2607	35	3.0	1.5							
570	ZW		A=TROON			B=DET#2		C=FLOW				

 ***** START CAVALLIERE FLATS MODEL CHANGES *****

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

571	KK	DDDET#2										
572	KM		Outlet channel capacity is 348 cfs. Divert overtopping flow.									
573	DT	OVER										
574	DI	0	348	348.1	10000							
575	DQ	0	0	0.1	9652							

 ***** END CAVALLIERE FLATS MODEL CHANGES *****

576	KK	RDET#2	ROUTE FLOW THRU SUB 042									
577	KM		OUTLET CHANNEL TROON NORTH PARK BASIN									
578	RS	1	FLOW 0									
579	RC	0.040	0.035	0.040	594	0.010						
580	RX	990	992	994	1000	1010	1016	1018	1020			
581	RY	14.0	13.5	13.0	10.0	10.0	13.0	13.5	14.0			

 ***** STARTS DOWNSTREAM OFFSITE AREAS MODEL *****

582	KK	038	BASIN									
583	BA	0.047										
584	LG	0.28	0.25	6.00	0.21	26						
585	UC	0.234	0.266									
586	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0	
587	UA	100										

588	KK	LP038	ROUTE INFLOW THRU STORAGE AREA									
589	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS									
590	KM		LOW FLOW OUTLET = 3-36" RCP									
591	RS	1	STOR 0									
592	SA	.001	.007	.047	.086	.160						
593	SQ	0	33	82	132	165						
594	SE	24.6	26.0	27.0	28.0	29.0						
595	ST	29.0	45	2.7	1.5							

596	KK	040	BASIN									
597	BA	0.018										
598	LG	0.26	0.25	6.00	0.20	49						

599	UC	0.184	0.202								
600	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
601	UA	100									

HEC-1 INPUT

PAGE 17

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

602	KK	C040	COMBINE HYDROGRAPHS FROM SUB 040 AND LP038								
603	HC	2									

604	KK	LP040	ROUTE INFLOW THRU STORAGE AREA								
605	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
606	KM		LOW FLOW OUTLET = 3-36" RCP								
607	RS	1	STOR 0								
608	SA	.001	.024	.077	.127						
609	SQ	0	82	132	165						
610	SE	06.6	09.0	10.0	11.0						
611	ST	11.0	45	2.7	1.5						

612	KK	RLP040	ROUTE FLOW THRU SUB 042								
613	RS	1	FLOW C								
614	RC	0.045	0.040	0.045	927	0.016					
615	RX	990	992	994	1000	1012	1018	1020	1022		
616	RY	13.2	13.1	13.0	10.0	10.0	13.0	13.1	13.2		

617	KK	042	BASIN								
618	BA	0.020									
619	LG	0.27	0.25	6.00	0.20	20					
620	UC	0.289	0.433								
621	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
622	UA	100									

623	KK	C042	COMBINE HYDROGRAPHS FROM RDET#2, SUB 042 AND RLP040								
624	HC	3									
625	ZW		A=EVISTA	B=C042	C=FLOW						

626	KK	039	BASIN								
627	BA	0.035									
628	LG	0.26	0.25	6.00	0.20	39					
629	UC	0.225	0.292								
630	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
631	UA	100									

632	KK	LP039	ROUTE INFLOW THRU STORAGE AREA								
633	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
634	KM		LOW FLOW OUTLET = 3-30" RCP								
635	RS	1	STOR 0								
636	SA	.001	.007	.015	.024	.037					
637	SQ	0	49	84	112	135					
638	SE	17.0	19.0	20.0	21.0	22.0					
639	ST	22.0	45	2.7	1.5						

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

640	KK	041	BASIN								
641	BA	0.011									
642	LG	0.28	0.25	6.00	0.19	33					
643	UC	0.231	0.353								
644	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
645	UA	100									

646	KK	C041	COMBINE HYDROGRAPHS FROM SUB 041 AND LP039								
647	HC	2									

648	KK	LP041	ROUTE INFLOW THRU STORAGE AREA								
649	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
650	KM		LOW FLOW OUTLET = 3-30" RCP								
651	RS	1	STOR 0								
652	SA	.001	.020	.039	.072	.104					
653	SQ	0	14	49	84	113					
654	SE	07.0	08.0	09.0	10.0	11.0					
655	ST	11.0	45	2.7	1.5						

656	KK	RLP041	ROUTE FLOW THRU SUB 041/042								
657	RS	1	FLOW 0								
658	RC	0.045	0.040	0.045	1314	0.017					
659	RX	990	992	994	1000	1008	1014	1016	1018		
660	RY	13.2	13.1	13.0	10.0	10.0	13.0	13.1	13.2		

661 KK 043 BASIN
 662 BA 0.008
 663 LG 0.26 0.25 6.00 0.20 29
 664 UC 0.231 0.376
 665 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 666 UA 100
 *

667 KK C043 COMBINE HYDROGRAPHS FROM SUB 043 AND RLP041
 668 HC 2
 669 ZW A=PVISTA B=C043 C=FLOW
 *

670 KK COMB COMBINE HYDROGRAPHS FROM C042 AND C043
 671 KM THIS REPRESENTS COMBINED FLOWS AT PINNACLE VISTA ROAD
 672 HC 2
 673 ZW A=PVISTA B=COMB C=FLOW
 *

* *****
 * *****
 * ***** START CAVALLIERE FLATS MODEL CHANGES *****
 * *****
 * *****
 *

1

HEC-1 INPUT

PAGE 19

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

674 KK RCF1 ROUTE
 675 RS 1 FLOW
 676 RC 0.035 0.035 0.035 690 0.0200 0.00
 677 RX 0.00 30.00 67.50 72.50 77.50 82.50 120.00 150.00
 678 RY 3.00 2.50 2.00 0.00 0.00 2.00 2.50 3.00
 *

679 KK CF1 BASIN
 680 BA 0.009
 681 LG 0.22 0.25 5.85 0.22 37
 682 UC 0.158 0.193
 683 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 684 UA 100
 *

685 KK CCF1 COMBINE
 686 HC 2
 *

687 KK ODET#2
 688 KM Retrieve flow overtopping Troon North Park Basin outlet channel
 689 DR OVER
 *

690 KK RCF2A ROUTE
 691 RS 1 FLOW
 692 RC 0.035 0.035 0.035 990 0.0200 0.00
 693 RX 0.00 20.00 40.00 45.00 55.00 60.00 80.00 100.00
 694 RY 2.00 1.50 1.00 0.00 0.00 1.00 1.50 2.00
 *

695 KK OFF1 BASIN
 696 BA 0.307
 697 LG 0.32 0.31 6.00 0.17 9
 698 UC 0.462 0.356
 699 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 700 UA 100
 *

701 KK RCF2B ROUTE
 702 RS 1 FLOW
 703 RC 0.035 0.035 0.035 530 0.0270 0.00
 704 RX 0.00 15.00 30.00 35.00 90.00 95.00 110.00 125.00
 705 RY 2.00 1.50 1.00 0.00 0.00 1.00 1.50 2.00
 *

706 KK CF2 BASIN
 707 BA 0.014
 708 LG 0.30 0.25 5.85 0.18 6
 709 UC 0.187 0.188
 710 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 711 UA 100
 *

1

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

712 KK CCF2A COMBINE
 713 HC 3
 *

714 KK CCF2B COMBINE

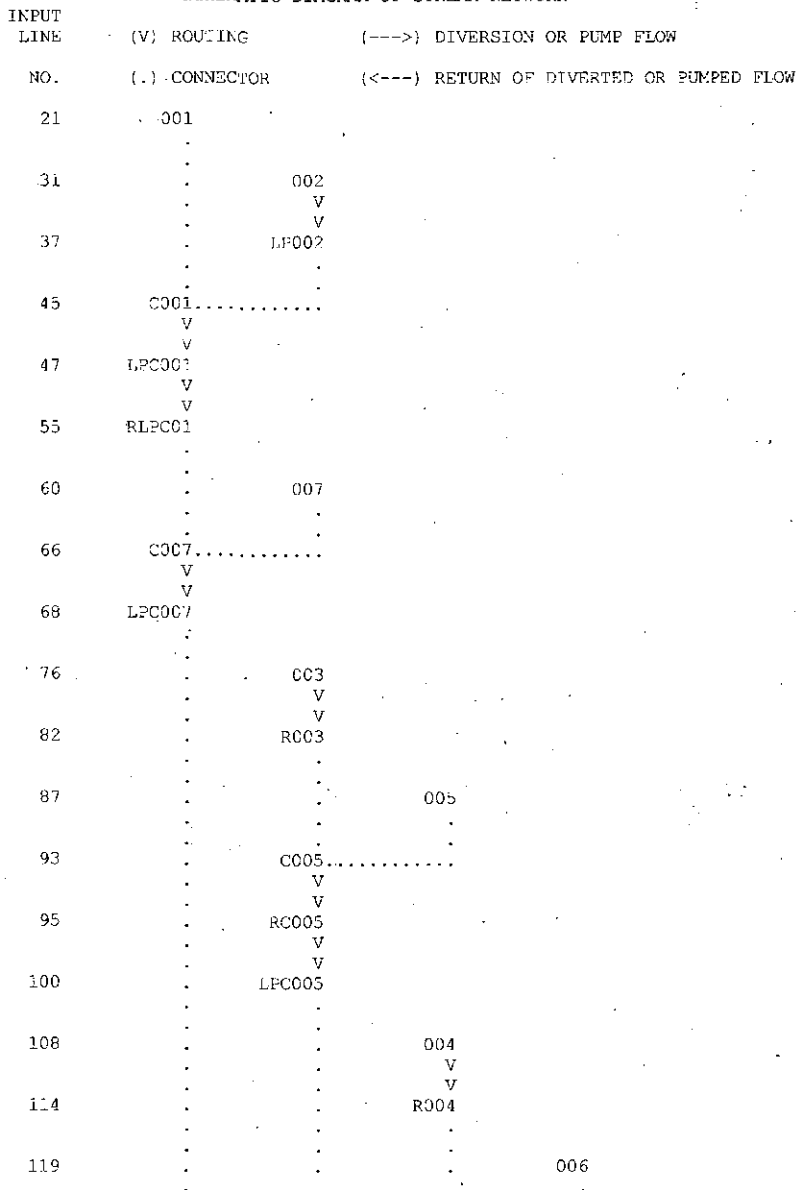
778	KK	OFF5	BASIN									
779	EA	0.009										
780	LG	0.35	0.35	4.65	0.33	2						
781	JC	0.292	0.470									
782	JA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
783	JA	100										

HEC-1 INPUT

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LINE	ID	1	2	3	4	5	6	7	8	9	10
784	KK	RCF6	ROUTE								
785	RS		FLOW								
786	RC	0.035	0.035	0.035	260	0.0350	0.00				
787	RX	0.90	50.00	100.00	105.00	110.00	115.00	165.00	215.00		
788	RY	3.00	2.50	2.00	0.00	0.00	2.00	2.50	3.00		
789	KK	CF6	BASIN								
790	EA	0.001									
791	LG	0.35	0.35	3.75	0.54	0					
792	UC	0.163	0.231								
793	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
794	JA	100									
795	KK	CCF6	COMBINE								
796	HC	2									
797	??										

SCHEMATIC DIAGRAM OF STREAM NETWORK



125	.	.	C006.....	.
	.	.	V	.
	.	.	V	.
127	.	.	LPC006	.

135	.	.	.	009

141	.	.	C009.....	.
	.	.	V	.
	.	.	V	.
143	.	.	LPC009	.

151	.	.	.	012

157	.	.	C012.....	.
	.	.	V	.
	.	.	V	.
159	.	.	LPC012	.

167	.	.	.	008

173	.	.	.	010

179	.	.	C010.....	.
	.	.	V	.
	.	.	V	.
181	.	.	RC010	.

186	.	.	.	011

192	.	.	C011.....	.

194	.	.	C013I.....	.
	.	.	V	.
	.	.	V	.
196	.	.	LPC13I	.

204	.	.	.	013

210	.	.	C013.....	.
	.	.	V	.
	.	.	V	.
212	.	.	LPC013	.
	.	.	V	.
	.	.	V	.
220	.	.	RLPC13	.

225	.	.	.	014
	.	.	V	.
	.	.	V	.
231	.	.	LP014	.
	.	.	V	.
	.	.	V	.
239	.	.	RLP014	.

244	.	.	.	016
	.	.	V	.
	.	.	V	.
250	.	.	LP016	.
	.	.	V	.
	.	.	V	.
257	.	.	RLP016	.

262	.	.	.	015

268	.	.	.	017

280	.	.	.	-----> DR017
274	.	.	DV017	.

283	.	.	C017.....	.
	.	.	V	.
	.	.	V	.
285	.	.	RC017	.

291	.	.	018	.
297	C018
300	.	019	.	.
306	.	.	020	.
312	.	C020
314	.	V	.	.
	.	V	.	.
	.	RC020	.	.
320	.	.	021	.
326	.	C021
329	.	.	022	.
	.	V	.	.
	.	V	.	.
335	.	LP022	.	.
343	.	.	023	.
	.	V	.	.
	.	V	.	.
349	.	LP023	.	.
357	.	C023
	.	V	.	.
	.	V	.	.
359	.	RLPC23	.	.
364	.	.	024	.
370	.	C024
	.	V	.	.
	.	V	.	.
372	.	LPC024	.	.
	.	V	.	.
	.	V	.	.
379	.	RLPC24	.	.
385	.	.	025	.
391	.	C025
394	.	.	026	.
	.	V	.	.
	.	V	.	.
400	.	LP026	.	.
	.	V	.	.
	.	V	.	.
407	.	RLP026	.	.
412	.	.	028	.
	.	V	.	.
	.	V	.	.
418	.	LP028	.	.
426	.	.	030	.
	.	V	.	.
	.	V	.	.
432	.	LP030	.	.
440	.	.	.	029
446	.	C029
	.	V	.	.
	.	V	.	.
448	.	LPC029	.	.
456	.	.	027	.

462					031
468				C031	
				V	
				V	
470				RC031	
476					032
482				C032	
				V	
				V	
484				LPC032	
				V	
				V	
492				RLPC32	
498					033
504				C033	
506				034	
				V	
				V	
512				R034	
517					035
523					036
529				C036	
532				037	
538				C037	
				V	
				V	
542				DET#1	
				V	
				V	
556				DET#2	
573					
571				DDET#2	
				V	
				V	
576				RDET#2	
582				038	
				V	
				V	
588				LP038	
596					040
602				C040	
				V	
				V	
604				LP040	
				V	
				V	
612				RLP040	
617					042
623				C042	
626				039	
				V	
				V	
632				LP039	

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640      .      .      041
        .      .      .
646      .      C041.....
        .      V
        .      V
648      .      LP041
        .      V
        .      V
656      .      RLP041
        .      .
661      .      .      043
        .      .      .
667      .      C043.....
        .      .
670      .      COMB.....
        .      V
        .      V
674      .      RCF1
        .      .
679      .      .      CF1
        .      .
685      .      CCF1.....
        .      .
689      .      .      <----- OVER
687      .      ODET#2
        .      V
        .      V
690      .      RCF2A
        .      .
695      .      .      OFF1
        .      .      V
701      .      .      RCF2B
        .      .      V
706      .      .      .      CF2
        .      .      .
712      .      CCF2A.....
        .      .
714      .      CCF2B.....
        .      V
        .      V
716      .      RCF3
        .      .
721      .      .      CF3
        .      .
727      .      CCF3.....
        .      .
729      .      .      OFF2
        .      .      V
735      .      .      RCF4A
        .      .
740      .      .      .      OFF3
        .      .      .      V
746      .      .      .      RCF4B
        .      .      .      V
751      .      .      .      .      CF4
        .      .      .      .
757      .      .      CCF4.....
        .      .
759      .      .      .      OFF4
        .      .      .      V
765      .      .      .      RCF5
        .      .      .
770      .      .      .      .      CF5
        .      .      .      .
776      .      .      .      CCF5.....
        .      .
778      .      .      .      .      OFF5
        .      .      .      .      V
```

784 V
RC76
789
CF6
795
CCF6.....

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 02JUN14 TIME 14:32:05 *

* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *

Cavaliere Flats Rezoning
City of Scottsdale
Prepared by: Kimley-Horn and Associates, Inc.

Existing Conditions model
10-year, 6-hour Storm event

Based on Troon North Park Model:

PROJECT NAME: TROON NORTH PARK
PREPARED FOR: CITY OF SCOTTSDALE
PREPARED BY : ARGUS CONSULTING, P.C.

JOB#: 236-08

FILE NAME : TNP10YR.DAT
STORM EVENT : 10-YR/6-HR
Unit Hydrograph: Clark
01/24/2011

20 IO OUTPUT CONTROL VARIABLES
IPRNT 3 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
NMIN 5 MINUTES IN COMPUTATION INTERVAL
IDATE 6JUN99 STARTING DATE
ITIME 0000 STARTING TIME
NQ 2000 NUMBER OF HYDROGRAPH ORDINATES
NDDATE 12JUN99 ENDING DATE
NDTIME 2235 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .08 HOURS
TOTAL TIME BASE 166.58 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-Feet
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

-----DSS---ZOPEN: Existing File Opened, File: 01-10.DSS
Unit: 71; DSS Version: 6-JG

-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/INFLOW/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/INFLOW/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/INFLOW/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/INFLOW/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/INFLOW/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/INFLOW/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/INFLOW/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/INFLOW/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#1/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#1/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#1/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#1/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#1/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#1/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#1/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#1/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#2/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#2/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#2/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#2/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#2/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#2/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#2/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 11: /TROON/DET#2/FLOW/12JUN1999/5MIN//

+		.006	16.	4.08	2.	0.	0.	.01		
+	2 COMBINED AT	CC06	25.	4.08	3.	1.	0.	.03		
	ROUTED TO	LPC006	24.	4.17	3.	1.	0.	.03	82.19	4.17
	HYDROGRAPH AT	009	7.	4.08	1.	0.	0.	.01		
+	3 COMBINED AT	C009	52.	4.17	6.	1.	0.	.06		
+	ROUTED TO	LPC009	52.	4.17	6.	1.	0.	.06	77.35	4.17
	HYDROGRAPH AT	012	4.	4.00	0.	0.	0.	.00		
+	2 COMBINED AT	C012	56.	4.17	6.	2.	1.	.06		
+	ROUTED TO	LPC012	56.	4.17	6.	2.	1.	.06	70.03	4.17
	HYDROGRAPH AT	008	2.	4.08	0.	0.	0.	.00		
+	HYDROGRAPH AT	010	3.	4.08	0.	0.	0.	.00		
+	2 COMBINED AT	C010	5.	4.08	1.	0.	0.	.01		
+	ROUTED TO	RC010	5.	4.08	1.	0.	0.	.01	90.18	4.08
	HYDROGRAPH AT	011	5.	4.08	1.	0.	0.	.00		
+	2 COMBINED AT	C011	10.	4.08	1.	0.	0.	.01		
+	3 COMBINED AT	C013I	160.	4.25	21.	5.	2.	.22		
+	ROUTED TO	LPC13I	159.	4.25	21.	5.	2.	.22	64.15	4.25
	HYDROGRAPH AT	013	39.	4.08	5.	1.	0.	.04		
+	2 COMBINED AT	C013	190.	4.25	25.	6.	2.	.26		
+	ROUTED TO	LPC013	191.	4.25	25.	6.	2.	.26	59.51	4.25
+	ROUTED TO	RLPC13	185.	4.33	25.	6.	2.	.26	91.85	4.33
	HYDROGRAPH AT	014	19.	4.08	2.	1.	0.	.02		
+	ROUTED TO	LP014	20.	4.08	2.	1.	0.	.02	66.81	4.08
+	ROUTED TO	RLPQ14	17.	4.17	2.	1.	0.	.02	90.73	4.17
	HYDROGRAPH AT	016	31.	4.08	3.	1.	0.	.03		
+	ROUTED TO	LP016	28.	4.17	2.	1.	0.	.03	43.15	4.17
+	ROUTED TO	RLP016	26.	4.25	2.	1.	0.	.03	91.04	4.25
	HYDROGRAPH AT	015	58.	4.08	7.	2.	1.	.06		

+	HYDROGRAPH AT										
		017	15.	4.08	2.	0.	0.	.02			
+	DIVERSION TO										
		DR017	15.	.00	2.	0.	0.	.02			
+	HYDROGRAPH AT										
		DV017	0.	.00	0.	0.	0.	.02			
+	4 COMBINED AT										
		C017	97.	4.17	11.	3.	1.	.13			
+	ROUTED TO										
		RC017	94.	4.17	11.	3.	1.	.13	91.24	4.17	
+	HYDROGRAPH AT										
		018	49.	4.08	7.	2.	1.	.05			
+	3 COMBINED AT										
		C018	317.	4.25	43.	11.	4.	.44			
+	HYDROGRAPH AT										
		019	59.	4.08	8.	2.	1.	.05			
+	HYDROGRAPH AT										
		020	9.	4.17	2.	0.	0.	.02			
+	2 COMBINED AT										
		C020	68.	4.08	10.	2.	1.	.07			
+	ROUTED TO										
		RC020	65.	4.17	10.	2.	1.	.07	91.42	4.17	
+	HYDROGRAPH AT										
		021	27.	4.08	4.	1.	0.	.02			
+	2 COMBINED AT										
		C021	92.	4.08	13.	3.	1.	.09			
+	HYDROGRAPH AT										
		022	48.	4.17	6.	2.	1.	.07			
+	ROUTED TO										
		LP022	48.	4.17	6.	2.	1.	.07	73.00	4.17	
+	HYDROGRAPH AT										
		023	59.	4.08	7.	2.	1.	.06			
+	ROUTED TO										
		LP023	54.	4.17	7.	2.	1.	.06	75.17	4.17	
+	2 COMBINED AT										
		C023	102.	4.17	13.	3.	1.	.13			
+	ROUTED TO										
		RLPC23	101.	4.25	13.	3.	1.	.13	91.87	4.25	
+	HYDROGRAPH AT										
		024	17.	4.08	2.	1.	0.	.02			
+	2 COMBINED AT										
		C024	114.	4.25	15.	4.	1.	.15			
+	ROUTED TO										
		LPC024	110.	4.25	14.	4.	1.	.15	48.31	4.25	
+	ROUTED TO										
		RLPC24	113.	4.33	14.	4.	1.	.15	91.13	4.33	
+	HYDROGRAPH AT										
		025	32.	4.08	5.	1.	0.	.03			
+	2 COMBINED AT										
		C025	133.	4.33	19.	5.	2.	.18			
+	HYDROGRAPH AT										
		026	37.	4.08	4.	1.	0.	.04			
+	ROUTED TO										
		LP026	36.	4.08	3.	1.	0.	.04	2702.46	4.08	
+	ROUTED TO										
		RLP026	32.	4.33	3.	1.	0.	.04	91.04	4.33	

+	HYDROGRAPH AT	028	5.	4.00	0.	0.	0.	.00		
	ROUTED TO	LP028	0.	.00	0.	0.	0.	.00	30.19	6.75
+	HYDROGRAPH AT	030	2.	4.00	0.	0.	0.	.00		
+	ROUTED TO	LP030	0.	.00	0.	0.	0.	.00	15.54	7.00
+	HYDROGRAPH AT	029	4.	4.08	0.	0.	0.	.00		
+	3 COMBINED AT	C029	4.	4.08	0.	0.	0.	.01		
+	ROUTED TO	LPC029	4.	4.08	0.	0.	0.	.01	1.66	4.08
+	HYDROGRAPH AT	027	20.	4.08	2.	0.	0.	.02		
+	HYDROGRAPH AT	031	10.	4.00	1.	0.	0.	.01		
+	3 COMBINED AT	C031	35.	4.08	3.	1.	0.	.05		
+	ROUTED TO	RC031	34.	4.08	3.	1.	0.	.05	90.81	4.08
+	HYDROGRAPH AT	032	13.	4.08	2.	0.	0.	.01		
+	2 COMBINED AT	C032	47.	4.08	5.	1.	0.	.06		
+	ROUTED TO	LPC032	45.	4.17	5.	1.	0.	.06	79.94	4.17
+	ROUTED TO	RLPC32	43.	4.33	5.	1.	0.	.06	90.91	4.33
+	HYDROGRAPH AT	033	137.	4.17	19.	5.	2.	.15		
+	3 COMBINED AT	C033	191.	4.25	27.	7.	2.	.25		
+	HYDROGRAPH AT	034	82.	4.08	8.	2.	1.	.09		
+	ROUTED TO	R034	72.	4.17	8.	2.	1.	.09	91.11	4.17
+	HYDROGRAPH AT	035	22.	4.08	2.	0.	0.	.02		
+	HYDROGRAPH AT	036	9.	4.00	1.	0.	0.	.01		
+	4 COMBINED AT	C036	281.	4.25	37.	9.	3.	.37		
+	HYDROGRAPH AT	037	45.	4.00	4.	1.	0.	.04		
+	5 COMBINED AT	C037	824.	4.25	116.	29.	10.	1.12		
+	ROUTED TO	DET#1	446.	4.58	116.	29.	10.	1.12	2606.42	4.58
+	ROUTED TO	DET#2	303.	5.33	115.	29.	10.	1.12	2603.18	5.33
+	DIVERSION TO	OVER	0.	5.33	0.	0.	0.	1.12		
+	HYDROGRAPH AT	DDET#2	303.	5.33	115.	29.	10.	1.12		

	ROUTED TO									
+		RDET#2	303.	5.33	115.	29.	10.	1.12		
+									12.87	5.33
	HYDROGRAPH AT									
+		038	43.	4.08	5.	1.	0.	.05		
	ROUTED TO									
+		LP038	43.	4.08	5.	1.	0.	.05		
+									26.21	4.08
	HYDROGRAPH AT									
+		040	21.	4.06	3.	1.	0.	.02		
	2 COMBINED AT									
+		C040	64.	4.08	8.	2.	1.	.06		
	ROUTED TO									
+		LP040	64.	4.08	8.	2.	1.	.06		
+									8.48	4.08
	ROUTED TO									
+		RLP040	62.	4.17	8.	2.	1.	.06		
+									11.02	4.17
	HYDROGRAPH AT									
+		042	14.	4.17	2.	1.	0.	.02		
	3 COMBINED AT									
+		C042	308.	5.25	125.	32.	11.	1.20		
	HYDROGRAPH AT									
+		039	34.	4.08	5.	1.	0.	.04		
	ROUTED TO									
+		LP039	34.	4.08	5.	1.	0.	.04		
+									18.38	4.08
	HYDROGRAPH AT									
+		041	9.	4.08	1.	0.	0.	.01		
	2 COMBINED AT									
+		C041	43.	4.08	6.	2.	1.	.05		
	ROUTED TO									
+		LP041	43.	4.08	6.	2.	1.	.05		
+									8.82	4.08
	ROUTED TO									
+		RLP041	41.	4.17	6.	2.	1.	.05		
+									10.99	4.17
	HYDROGRAPH AT									
+		043	6.	4.08	1.	0.	0.	.01		
	2 COMBINED AT									
+		C043	47.	4.17	7.	2.	1.	.05		
	2 COMBINED AT									
+		COMB	313.	5.17	132.	33.	11.	1.25		
	ROUTED TO									
+		RCF1	313.	5.17	132.	33.	11.	1.25		
+									2.55	5.17
	HYDROGRAPH AT									
+		CF1	10.	4.00	1.	0.	0.	.01		
	2 COMBINED AT									
+		CCF1	314.	5.17	134.	34.	12.	1.26		
	HYDROGRAPH AT									
+		ODET#2	0.	.00	0.	0.	0.	.00		
	ROUTED TO									
+		RCF2A	0.	.00	0.	0.	0.	.00		
	HYDROGRAPH AT									
+		OFF1	210.	4.33	27.	7.	2.	.31		
	ROUTED TO									
+		RCF2B	211.	4.33	27.	7.	2.	.31		
+									.69	4.33
	HYDROGRAPH AT									
+		CE2	14.	4.08	1.	0.	0.	.01		
	3 COMBINED AT									
+		CCF2A	217.	4.33	28.	7.	2.	.32		
	2 COMBINED AT									
+		CCF2B	517.	4.33	161.	41.	14.	1.58		

ROUTED TO	RCF3	515.	4.33	161.	41.	14.	1.58		
								2.36	4.33
HYDROGRAPH AT	CF3	34.	4.08	4.	1.	0.	.03		
2 COMBINED AT	CCF3	531.	4.33	165.	42.	14.	1.61		
HYDROGRAPH AT	OFF2	11.	4.08	1.	0.	0.	.01		
ROUTED TO	RCF4A	10.	4.17	1.	0.	0.	.01		
								.31	4.17
HYDROGRAPH AT	OFF3	5.	4.17	1.	0.	0.	.01		
ROUTED TO	RCF4B	5.	4.17	1.	0.	0.	.01		
								.20	4.17
HYDROGRAPH AT	CF4	12.	4.17	2.	0.	0.	.02		
3 COMBINED AT	CCF4	26.	4.17	3.	1.	0.	.04		
HYDROGRAPH AT	OFF4	51.	4.17	6.	1.	0.	.06		
ROUTED TO	RCF5	50.	4.17	6.	1.	0.	.06		
								.62	4.17
HYDROGRAPH AT	CF5	2.	4.08	0.	0.	0.	.00		
2 COMBINED AT	CCF5	51.	4.17	6.	1.	0.	.06		
HYDROGRAPH AT	OFF5	4.	4.25	0.	0.	0.	.01		
ROUTED TO	RCF6	4.	4.25	0.	0.	0.	.01		
								.22	4.25
HYDROGRAPH AT	CF6	1.	4.08	0.	0.	0.	.00		
2 COMBINED AT	CCF6	4.	4.17	0.	0.	0.	.01		

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP002
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	2722.00	2725.00	2725.00
STORAGE	0.	0.	0.
OUTFLOW	0.	30.	30.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	2723.63	.00	0.	11.	.00	4.17	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC001
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	2713.00	2717.00	2717.00
STORAGE	0.	0.	0.
OUTFLOW	0.	90.	90.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	2717.03	.03	0.	92.	.08	4.17	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC007
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	68.00	74.00	74.00
STORAGE	0.	0.	0.

	OUTFLOW	0.	143.	143.			
RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	72.00	.00	0.	100.	.00	4.25	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC005 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	83.00	88.00	88.00
STORAGE	0.	0.	0.
OUTFLOW	0.	62.	62.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	84.76	.00	0.	22.	.00	4.17	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC006 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	80.00	86.00	86.00
STORAGE	0.	1.	1.
OUTFLOW	0.	51.	51.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	82.49	.00	0.	24.	.00	4.17	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC009 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	75.00	80.00	80.00
STORAGE	0.	1.	1.
OUTFLOW	0.	124.	124.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	77.35	.00	0.	52.	.00	4.17	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC012 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	67.50	73.00	73.00
STORAGE	0.	0.	0.
OUTFLOW	0.	133.	133.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	70.03	.00	0.	56.	.00	4.17	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC131 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	61.00	66.00	66.00
STORAGE	0.	1.	1.
OUTFLOW	0.	266.	266.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	64.15	.00	0.	159.	.00	4.25	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC013 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	55.80	61.00	61.00
STORAGE	0.	0.	0.
OUTFLOW	0.	272.	272.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	59.51	.00	0.	191.	.00	4.25	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP014 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	64.50	70.00	70.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	48.	48.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	66.81	.00	0.	20.	.00	4.08	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP022 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	71.00	77.00	77.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	127.	127.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	73.00	.00	0.	48.	.00	4.17	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP023 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	73.00	77.00	77.00
	STORAGE	0.	1.	1.
	OUTFLOW	0.	113.	113.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	75.17	.00	0.	54.	.00	4.17	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP028 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	28.00	32.00	32.00
	STORAGE	0.	1.	1.
	OUTFLOW	0.	0.	0.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	30.19	.00	0.	0.	.00	.00	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP030 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	11.00	16.00	16.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	0.	0.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	15.54	.00	0.	0.	.00	.00	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP029 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	1.00	8.00	8.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	38.	38.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1 1.00 1.66 .00 0. 4. .00 4.08 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC032
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	77.00	81.00	81.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	63.	63.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1 1.00 79.94 .00 0. 45. .00 4.17 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DET#2
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	2590.30	2607.00	2607.00
	STORAGE	0.	29.	29.
	OUTFLOW	0.	378.	378.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1 1.00 2603.18 .00 18. 303. .00 5.33 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP038
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	24.60	29.00	29.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	165.	165.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1 1.00 26.21 .00 0. 43. .00 4.08 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP040
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	6.60	11.00	11.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	165.	165.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1 1.00 8.48 .00 0. 64. .00 4.08 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP039
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	17.00	22.00	22.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	135.	135.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1 1.00 18.38 .00 0. 34. .00 4.08 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP041
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	7.00	11.00	11.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	113.	113.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1.00 8.82 .00 0. 43. .00 4.08 .00

*** NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 01-10.DSS
Pointer Utilization: .27
Number of Records: 48
File Size: 92.1 Kbytes
Percent Inactive: .0

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 02JUN14 TIME 14:32:22
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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Cavalliere Flats Rezoning
2 ID City of Scottsdale
3 ID Prepared by: Kimley-Horn and Associates, Inc.
4 ID
5 ID Existing Conditions model
6 ID 100-year, 6-hour Storm event
7 ID
8 ID Based on Troon North Park Model:
9 ID
10 ID PROJECT NAME: TROON NORTH PARK JOB#: 236-08
11 ID PREPARED FOR: CITY OF SCOTTSDALE
12 ID PREPARED BY : ARGUS CONSULTING, P.C.
13 ID
14 ID FILE NAME : TNP100YR.DAT
15 ID STORM EVENT : 100-YR/6-HR
16 ID Unit Hydrograph: Clark
17 ID 01/24/2011
*
*
*
18 IT 5 06JUN99 0000 2000
19 IN 15
20 IO 5
*DIAGRAM
*
*
21 KK 001 BASIN
22 BA 0.112
23 PB 3.136
24 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.075
25 PC 0.087 0.100 0.119 0.151 0.235 0.415 0.762 0.873 0.915 0.944
26 PC 0.956 0.967 0.979 0.989 1.000
27 LG 0.30 0.25 6.00 0.18 3
28 UC 0.261 0.233
29 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
30 UA 100
*
31 KK 002 BASIN
32 BA 0.0167
33 LG 0.30 0.25 6.00 0.18 7
34 UC 0.201 0.303
35 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
36 UA 100
*
37 KK LP002 ROUTE INFLOW THRU STORAGE AREA
38 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
39 KM LOW FLOW OUTLET = 30" RCP
40 RS 1 STOR 0.0
41 SA .003 .005 .026 .059
42 SQ 0 5 15 30
43 SE 2722.0 2723.0 2724.0 2725.0
    
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1 HEC-1 INPUT PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
44	ST	2725.0	40	2.7	1.5						
	*										
45	KK	C001	COMBINE HYDROGRAPHS FROM SUB 001 AND LPC02								
46	HC	2									
	*										
47	KK	LPC001	ROUTE INFLOW THRU STORAGE AREA								
48	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
49	KM		LOW FLOW OUTLET = OPEN SECTION BETWEEN BERM AND WALL								
50	RS	1	STOR	0.0							
51	SA	.001	.004	.013	.104	.150					
52	SQ	0	6.5	23	50	90					
53	SE	2713.0	2714.0	2715.0	2716.0	2717.0					
54	ST	2717.0	45	2.7	1.5						
	*										
55	KK	RLPC01	ROUTE FLOW THRU SUB 007								
56	RS	1	FLOW	0							
57	RC	0.070	0.050	0.070	1736	0.025					
58	RX	1000	1001	1034	1047.5	1052.5	1066	1099	1100		
59	RY	100	95.5	94.5	90	90	94.5	95.5	100		
	*										
60	KK	007	BASIN								
61	BA	0.018									
62	LG	0.27	0.25	6.00	0.20	25					
63	UC	0.137	0.167								
64	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
65	UA	100									
	*										
66	KK	C007	COMBINE HYDROGRAPHS FROM SUB 007 AND RLPC01								
67	HC	2									
	*										
68	KK	LPC007	ROUTE INFLOW THRU STORAGE AREA								
69	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
70	KM		LOW FLOW OUTLET - 2-36" RCP								
71	RS	1	STOR	0.0							
72	SA	.001	.002	.004	.025	.040	.060	.100			
73	SQ	0	18	35	70	100	125	143			
74	SE	68.0	69.0	70.0	71.0	72.0	73.0	74.0			
75	ST	74.0	45	2.7	1.5						
	*										
76	KK	003	BASIN								
77	BA	0.018									
78	LG	0.30	0.25	6.00	0.18	2					
79	UC	0.156	0.173								
80	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
81	UA	100									
	*										

1 HEC-1 INPUT PAGE 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
82	KK	R003	ROUTE FLOW THRU SUB 005								
83	RS	1	FLOW	0							
84	RC	0.070	0.050	0.070	470	0.023					
85	RX	1000	1001	1045.5	1050	1055	1059.5	1099	1100		
86	RY	100	92.5	91.5	90	90	91.5	92.5	100		
	*										
87	KK	005	BASIN								
88	BA	0.007									
89	LG	0.27	0.25	6.00	0.20	23					
90	UC	0.134	0.162								
91	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
92	UA	100									
	*										
93	KK	C005	COMBINE HYDROGRAPHS FROM SUB 005 AND R003								
94	HC	2									
	*										
95	KK	RC005	ROUTE FLOW THRU SUB 006								
96	RS	1	FLOW	0							
97	RC	0.070	0.050	0.070	450	0.022					
98	RX	1000	1001	1045.5	1050	1055	1059.5	1099	1100		
99	RY	100	92.5	91.5	90	90	91.5	92.5	100		
	*										
100	KK	LPC005	ROUTE INFLOW THRU STORAGE AREA								
101	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
102	KM		LOW FLOW OUTLET = 2-24" RCP								
103	RS	1	STOR	0							
104	SA	.001	.002	.030	.090	.190	.271				

167 KK 008 BASIN
 168 BA 0.003
 169 LG 0.27 0.25 6.00 0.20 25
 170 UC 0.142 0.295
 171 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 172 UA 100
 *

173 KK 010 BASIN
 174 BA 0.003
 175 LG 0.27 0.25 6.00 0.20 19
 176 UC 0.121 0.166
 177 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 178 UA 100
 *

179 KK C010 COMBINE HYDROGRAPHS FROM SUB 008 AND SUB 010
 180 HC 2
 *

181 KK RC010 ROUTE FLOW THRU SUB 011
 182 RS 1 FLOW 0
 183 RC 0.070 0.050 0.070 390 0.020
 184 RX 1000 1001 1039 1045 1055 1061 1099 1100
 185 RY 100 93 92 90 90 92 93 100
 *

186 KK 011 BASIN
 187 BA 0.005
 188 LG 0.26 0.25 6.00 0.20 25
 189 UC 0.132 0.157
 190 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 191 UA 100
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

192 KK C011 COMBINE HYDROGRAPHS FROM SUB 011 AND RC010
 193 HC 2
 *

194 KK C013I COMBINE HYDROGRAPHS C011, LP012 AND LPC007
 195 HC 3
 *

196 KK LPC13I ROUTE INFLOW THRU STORAGE AREA
 197 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 198 KM LOW FLOW OUTLET = 10'w x 3'h RCB
 199 RS 1 STOR 0
 200 SA .001 .005 .029 .082 .216 .432
 201 SQ 0 40 90 150 210 266
 202 SE 61.0 62.0 63.0 64.0 65.0 66.0
 203 ST 66.0 45 2.7 1.5
 *

204 KK 013 BASIN
 205 BA 0.041
 206 LG 0.26 0.25 6.00 0.20 27
 207 UC 0.201 0.196
 208 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 209 UA 100
 *

210 KK C013 COMBINE HYDROGRAPHS FROM SUB 013 AND LPC13I
 211 HC 2
 *

212 KK LPC013 ROUTE INFLOW THRU STORAGE AREA
 213 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 214 KM LOW FLOW OUTLET = 10'w x 3'h RCB
 215 RS 1 STOR 0
 216 SA .001 .003 .005 .024 .100 .355
 217 SQ 0 40 100 160 220 272
 218 SE 55.8 57.0 58.0 59.0 60.0 61.0
 219 ST 61.0 45 2.7 1.5
 *

220 KK RLP013 ROUTE FLOW THRU SUB 018
 221 RS 1 FLOW 0
 222 RC 0.045 0.040 0.045 2178 0.020
 223 RX 1000 1001 1010 1022 1032 1044 1053 1054
 224 RY 95.5 95.0 94.0 90.0 90.0 94.0 95.0 95.5
 *

225 KK 014 BASIN
 226 BA 0.018
 227 LG 0.26 0.25 6.00 0.20 25
 228 UC 0.155 0.137
 229 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 230 UA 100

291	KK	018	BASIN									
292	BA	0.050										
293	LG	0.22	0.21	6.40	0.18	33						
294	UC	0.223	0.219									
295	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0	
296	UA	100										
	*											

297 KK C018 COMBINE HYDROGRAPHS FROM SUB 018, RC017 AND RLPC13
 298 KM THIS IS TNP INFLOW FROM ALMA SCHOOL RD. CULVERT
 299 HC 3
 *

300	KK	019	BASIN									
301	BA	0.050										
302	LG	0.26	0.25	6.00	0.20	57						
303	UC	0.195	0.172									
304	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0	
305	UA	100										
	*											

1 HEC-1 INPUT PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

306	KK	020	BASIN									
307	BA	0.016										
308	LG	0.28	0.25	6.00	0.20	18						
309	UC	0.231	0.427									
310	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0	
311	UA	100										
	*											

312 KK C020 COMBINE HYDROGRAPHS FROM SUB 019 AND SUB 020
 313 HC 2
 *

314	KK	RC020	NOTE: "n" VALUE AND GEOMETRY MODIFIED FROM PRE-DEVELOPED MODEL									
315	KM		TO REFLECT EXISTING DEVELOPMENT.									
316	RS	1	FLOW	0								
317	RC	0.070	0.040	0.070	880	0.018						
318	RX	1000	1001	1034	1047.5	1052.5	1066	1099	1100			
319	RY	100	95.5	94.5	90	90	94.5	95.5	100			
	*											

320	KK	021	BASIN									
321	BA	0.024										
322	LG	0.26	0.25	6.00	0.20	56						
323	UC	0.184	0.200									
324	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0	
325	UA	100										
	*											

326 KK C021 COMBINE HYDROGRAPHS FROM SUB 021 AND RC020
 327 KM THIS IS TNP INFLOW FROM C021
 328 HC 2
 *

329	KK	022	BASIN									
330	BA	0.067										
331	LG	0.29	0.25	6.00	0.19	10						
332	UC	0.244	0.272									
333	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0	
334	UA	100										
	*											

335	KK	LP022	ROUTE INFLOW THRU STORAGE AREA									
336	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS									
337	KM		LOW FLOW OUTLET = 4-24" RCP									
338	RS	1	STOR	0								
339	SA	.001	.004	.009	.011	.013	.060	.140				
340	SQ	0	17	48.5	79.5	97.0	113	127				
341	SE	71.0	72.0	73.0	74.0	75.0	76.0	77.0				
342	ST	77.0	45	2.7	1.5							
	*											

HEC-1 INPUT PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

343	KK	023	BASIN									
344	BA	0.065										
345	LG	0.27	0.25	6.00	0.20	22						
346	UC	0.216	0.198									
347	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0	
348	UA	100										
	*											

349	KK	LP023	ROUTE INFLOW THRU STORAGE AREA									
350	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS									
351	KM		LOW FLOW OUTLET = 3-30" RCP									
352	RS	1	STOR	0								
353	SA	.001	.052	.288	.518	.751						

1 HEC-1 INPUT PAGE 12

LINE	ID	1	2	3	4	5	6	7	8	9	10
418	KK	LP028	ROUTE INFLOW THRU STORAGE AREA								
419	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
420	KM		LOW FLOW OUTLET = NONE								
421	RS	1	STOR 0								
422	SA	.020	.067	.124	.181	.275					
423	SQ	0	0	0	0	0					
424	SE	28.0	29.0	30.0	31.0	32.0					
425	ST	32.0	41	2.7	1.5						
	*										
426	KK	030	BASIN								
427	BA	0.002									
428	LG	0.28	3.25	6.00	0.19	16					
429	JC	0.092	0.132								
430	JA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
431	JA	100									
	*										
432	KK	LP030	ROUTE INFLOW THRU STORAGE AREA								
433	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
434	KM		LOW FLOW OUTLET = NONE								
435	RS	1	STOR 0								
436	SA	.001	.004	.013	.027	.046	.072				
437	SQ	0	0	0	0	0					
438	SE	11.0	12.0	13.0	14.0	15.0	16.0				
439	ST	16.0	45	2.7	1.5						
	*										
440	KK	029	BASIN								
441	BA	0.005									
442	LG	0.28	0.25	6.00	0.19	13					
443	UC	0.135	0.218								
444	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
445	UA	100									
	*										
446	KK	C029	COMBINE HYDROGRAPHS FROM SUB 029, LP028 AND LP029								
447	HC	3									
	*										
448	KK	LPC029	ROUTE INFLOW THRU STORAGE AREA								
449	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
450	KM		LOW FLOW OUTLET = 24" RCP								
451	RS	1	STOR 0								
452	SA	.001	.002	.010	.039	.067	.100	.148			
453	SQ	0	13	20	26	31	35	38			
454	SE	01	03	04	05	06	07	08			
455	ST	08	40	2.7	1.5						
	*										

1 HEC-1 INPUT PAGE 13

LINE	ID	1	2	3	4	5	6	7	8	9	10
456	KK	027	BASIN								
457	BA	0.024									
458	LG	0.30	0.25	6.00	0.18	2					
459	UC	0.169	0.191								
460	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
461	UA	100									
	*										
462	KK	031	BASIN								
463	BA	0.010									
464	LG	0.30	0.25	6.00	0.18	2					
465	UC	0.126	0.130								
466	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
467	UA	100									
	*										
468	KK	C031	COMBINE HYDROGRAPHS FROM SUB 027, SUB 031 AND LPC029								
469	HC	3									
	*										
470	KK	RC031	ROUTE FLOW THRU SUB 032								
471	KM		FLOW 0								
472	RS	4	FLOW 0								
473	RC	0.070	0.050	0.070	894	0.019					
474	RX	1000	1001	1039	1045	1055	1061	1099	1100		
475	RY	100	93	92	90	90	92	93	100		
	*										
476	KK	032	BASIN								
477	BA	0.014									
478	LG	0.27	0.25	6.00	0.20	21					
479	UC	0.161	0.201								
480	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
481	UA	100									

482 KK C032 COMBINE HYDROGRAPHS FROM SUB 032 AND RC031
 483 HC 2
 *

484 KK RLPC32 ROUTE INFLOW THRU STORAGE AREA
 485 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 486 KM LOW FLOW OUTLET = 42" RCP
 487 RS 1 STOR 0
 488 SA .001 .007 .070 .136 .270
 489 SQ 0 11 23 46 63
 490 SE 77.0 78.0 79.0 80.0 81.0
 491 ST 81.0 45 2.7 1.5

HEC-1 INPUT

PAGE 14

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

492 KK RLPC32
 493 KM ROUTE FLOW THRU SUB 033
 494 RS 4 FLOW 0
 495 RC 0.070 0.050 0.070 2176 0.021
 496 RX 1000 1001 1039 1045 1055 1061 1099 1100
 497 RY 100 93 92 90 90 92 93 100
 *

498 KK 033 BASIN
 499 BA 0.147
 500 LG 0.27 0.25 6.00 0.20 36
 501 UC 0.270 0.216
 502 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 503 JA 100
 *

504 KK C033 COMBINE HYDROGRAPHS FROM SUB 033, RLPC32 AND RLP026
 505 HC 3
 *

506 KK 034 BASIN
 507 BA 0.093
 508 LG 0.30 0.25 6.00 0.18 1
 509 UC 0.206 0.169
 510 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 511 UA 100
 *

512 KK R034 ROUTE FLOW THRU SUB 035
 513 RS 1 FLOW 0
 514 RC 0.045 0.040 0.045 2178 0.020
 515 RX 1000 1001 1010 1022 1032 1044 1053 1054
 516 RY 95.5 95.0 94.0 90.0 90.0 94.0 95.0 95.5
 *

517 KK 035 BASIN
 518 BA 0.022
 519 LG 0.30 0.23 6.20 0.17 5
 520 UC 0.144 0.145
 521 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 522 UA 100
 *

523 KK 036 BASIN
 524 BA 0.008
 525 LG 0.29 0.23 6.20 0.18 23
 526 UC 0.110 0.127
 527 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 528 UA 100
 *

HEC-1 INPUT

PAGE 15

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

529 KK C036 COMBINE HYDROGRAPHS FROM SUB 035, SUB 036, C033 AND R034
 530 KM THIS IS TNP INFLOW FROM C036
 531 HC 4
 *

532 KK 037 BASIN
 533 BA 0.041
 534 LG 0.30 0.23 6.20 0.17 1
 535 UC 0.143 0.108
 536 JA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 537 JA 100
 *

538 KK C037 COMBINE ALL HYDROGRAPHS
 539 KM THIS REPRESENTS TOTAL INFLOW TO BASIN No.1
 540 HC 5
 541 ZW A=TRON B=INFLOW C=FLOW
 *

542 KK DET#1 ROUTE INFLOW THRU DET BASIN #1 (UPPER BASIN)
 543 KM LOW FLOW OUTLET = 4-42" RCP
 544 KM INLET INVERT ELEV = 2598.80
 545 KM OUTLET INVERT ELEV = 2592.0
 546 KM SPILLWAY ELEV = 2610.0
 547 KM WEIR LENGTH = 50'
 548 RS 1 STOR 0.0
 549 SA 0.023 0.11 0.49 1.23 3.50 4.00 4.24 4.57 4.85 5:33
 550 SA 5.95 6.67 7.80 8.95
 551 SQ 0 45 103.5 185 264 328.5 382 428.5 470 507
 552 SQ 543 575 725 999
 553 SE 2599 2600 2601 2602 2603 2604 2605 2606 2607 2608
 554 SE 2609 2610 2611 2612
 555 ZW A=TROON B=DET#1 C=FLOW
 *

556 KK DET#2 ROUTE INFLOW THRU DET BASIN #2 (LOWER BASIN)
 557 KM LOW FLOW OUTLET = 1-60" RCP
 558 KM INLET INVERT ELEV = 2590.36
 559 KM OUTLET INVERT ELEV = 2589.95
 560 KM SPILLWAY ELEV = 2607.0
 561 KM WEIR LENGTH = 35'
 562 RS 1 STOR 0.0
 563 SA 0.023 0.048 0.496 0.876 1.12 1.28 1.41 1.56 1.70 1.82
 564 SA 1.93 2.04 2.36 2.49 2.61 2.77 2.93 3.26
 565 SQ 0 11.5 .29 .47 78 112.5 148 177 204 226
 566 SQ 248 266 283 300.5 315 330 345 378
 567 SE 2590.3 2591 2592 2593 2594 2595 2596 2597 2598 2599
 568 SE 2600 2601 2602 2603 2604 2605 2606 2607
 569 ST 2607 35 3.0 1.5
 570 ZW A=TROON B=DET#2 C=FLOW
 *

 ***** START CAVALLIERE FLATS MODEL CHANGES *****

HEC-1 INPUT

PAGE 16

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

571 KK DDET#2
 572 KM Outlet channel capacity is 348 cfs. Divert overtopping flow.
 573 DT OVER
 574 DI 0 348 348.1 10000
 575 DQ 0 0 0.1 9652
 *

 ***** END CAVALLIERE FLATS MODEL CHANGES *****

576 KK RDET#2 ROUTE FLOW THRU SUB C42
 577 KM OUTLET CHANNEL TROON NORTH PARK BASIN
 578 RS 1 FLOW 0
 579 RC 0.040 0.035 0.040 594 0.010
 580 RX 990 992 994 1000 1010 1016 1018 1020
 581 RY 14.0 13.5 13.0 10.0 10.0 13.0 13.5 14.0
 *

 ***** STARTS DOWNSTREAM OFFSITE AREAS MODEL *****

582 KK 038 BASIN
 583 BA 0.047
 584 LG 0.23 0.25 6.00 0.21 26
 585 UC 0.234 0.266
 586 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 587 UA 100
 *

588 KK LP038 ROUTE INFLOW THRU STORAGE AREA
 589 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 590 KM LOW FLOW OUTLET = 3-36" RCP
 591 RS 1 STOR 0
 592 SA .001 .007 .047 .086 .160
 593 SQ 0 33 82 132 165
 594 SE 24.6 26.0 27.0 28.0 29.0
 595 ST 29.0 45 2.7 1.5
 *

596 KK 040 BASIN
 597 BA 0.018
 598 LG 0.26 0.25 6.00 0.20 49

599	UC	0.184	0.202								
600	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
601	UA	100									

1

HEC-1 INPUT

PAGE 17

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

602	KK	C040	COMBINE HYDROGRAPHS FROM SUB 040 AND LP038								
603	HC	2									

604	KK	LP040	ROUTE INFLOW THRU STORAGE AREA								
605	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
606	KM		LOW FLOW OUTLET = 3-36" RCP								
607	RS	1	STOR	0							
608	SA	.001	.024	.077	.127						
609	SQ	0	82	132	165						
610	SE	06.6	09.0	10.0	11.0						
611	ST	11.0	45	2.7	1.5						

612	KK	RLP040	ROUTE FLOW THRU SUB 042								
613	RS	1	FLOW	0							
614	RC	0.045	0.040	0.045	927	0.016					
615	RX	990	992	994	1000	1012	1018	1020	1022		
616	RY	13.2	13.1	13.0	10.0	10.0	13.0	13.1	13.2		

617	KK	042	BASIN								
618	BA	0.020									
619	LG	0.27	0.25	6.00	0.20	20					
620	UC	0.289	0.433								
621	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
622	UA	100									

623	KK	C042	COMBINE HYDROGRAPHS FROM RDET#2, SUB 042 AND RLP040								
624	HC	3									
625	HW		A-PVISTA	B=C042	C-FLOW						

626	KK	039	BASIN								
627	BA	0.035									
628	LG	0.26	0.25	6.00	0.20	39					
629	UC	0.225	0.292								
630	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
631	UA	100									

632	KK	LP039	ROUTE INFLOW THRU STORAGE AREA								
633	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
634	KM		LOW FLOW OUTLET = 3-30" RCP								
635	RS	1	STOR	0							
636	SA	.001	.007	.015	.024	.037					
637	SQ	0	49	84	112	135					
638	SE	17.0	19.0	20.0	21.0	22.0					
639	ST	22.0	45	2.7	1.5						

1

HEC-1 INPUT

PAGE 18

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

640	KK	041	BASIN								
641	BA	0.011									
642	LG	0.28	0.25	6.00	0.19	33					
643	JC	0.231	0.353								
644	JA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
645	JA	100									

646	KK	C041	COMBINE HYDROGRAPHS FROM SUB 041 AND LP039								
647	HC	2									

648	KK	LP041	ROUTE INFLOW THRU STORAGE AREA								
649	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
650	KM		LOW FLOW OUTLET = 3-30" RCP								
651	RS	1	STOR	0							
652	SA	.001	.020	.039	.072	.104					
653	SQ	0	14	49	84	113					
654	SE	07.0	08.0	09.0	10.0	11.0					
655	ST	11.0	45	2.7	1.5						

656	KK	RLP041	ROUTE FLOW THRU SUB 041/042								
657	RS	1	FLOW	0							
658	RC	0.045	0.040	0.045	1314	0.017					
659	RX	990	992	994	1000	1008	1014	1016	1018		
660	RY	13.2	13.1	13.0	10.0	10.0	13.0	13.1	13.2		

661 KK 043 BASIN
 662 BA 0.008
 663 LG 0.26 0.25 6.00 0.20 29
 664 UC 0.183 0.290
 665 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 666 UA 100
 *

667 KK C043 COMBINE HYDROGRAPHS FROM SUB 043 AND RLP041
 668 HC 2
 669 ZW A=PVISTA B=C043 C=FLOW
 *

670 KK COMB COMBINE HYDROGRAPHS FROM C042 AND C043
 671 KM THIS REPRESENTS COMBINED FLOWS AT PINNACLE VISTA ROAD
 672 HC 2
 673 ZW A=PVISTA B=COMB C=FLOW
 *

 * START CAVALIERE FLATS MODEL CHANGES *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

674 KK RCF1 ROUTE
 675 RS 1 FLOW
 676 RC 0.035 0.035 0.035 690 0.0200 0.00
 677 RX 0.00 30.00 67.50 72.50 77.50 82.50 120.00 150.00
 678 RY 3.00 2.50 2.00 0.00 0.00 2.00 2.50 3.00
 *

679 KK CF1 BASIN
 680 BA 0.009
 681 LG 0.22 0.25 5.85 0.22 37
 682 UC 0.128 0.152
 683 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 684 UA 100
 *

685 KK CCF1 COMBINE
 686 HC 2
 *

687 KK ODEF#2
 688 KM Retrieve flow overtopping Troon North Park Basin outlet channel
 689 DR OVER
 *

690 KK RCF2A ROUTE
 691 RS 1 FLOW
 692 RC 0.035 0.035 0.035 990 0.0200 0.00
 693 RX 0.00 20.00 40.00 45.00 55.00 60.00 80.00 100.00
 694 RY 2.00 1.50 1.00 0.00 0.00 1.00 1.50 2.00
 *

695 KK OFF1 BASIN
 696 BA 0.307
 697 LG 0.32 0.31 6.00 0.17 9
 698 UC 0.356 0.267
 699 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 700 UA 100
 *

701 KK RCF2B ROUTE
 702 RS 1 FLOW
 703 RC 0.035 0.035 0.035 530 0.0270 0.00
 704 RX 0.00 15.00 30.00 35.00 90.00 95.00 110.00 125.00
 705 RY 2.00 1.50 1.00 0.00 0.00 1.00 1.50 2.00
 *

706 KK CF2 BASIN
 707 BA 0.014
 708 LG 0.30 0.25 5.85 0.18 9
 709 UC 0.144 0.141
 710 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 711 UA 100
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

712 KK CCF2A COMBINE
 713 HC 3
 *

777	HC	2										
	*											
778	KK	OFF5	BASIN									
779	BA	0.009										
780	LG	0.35	0.35	4.65	0.33	2						
781	UC	0.219	0.342									
782	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
783	UA	100										
	*											

HEC-1 INPUT

PAGE 22

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

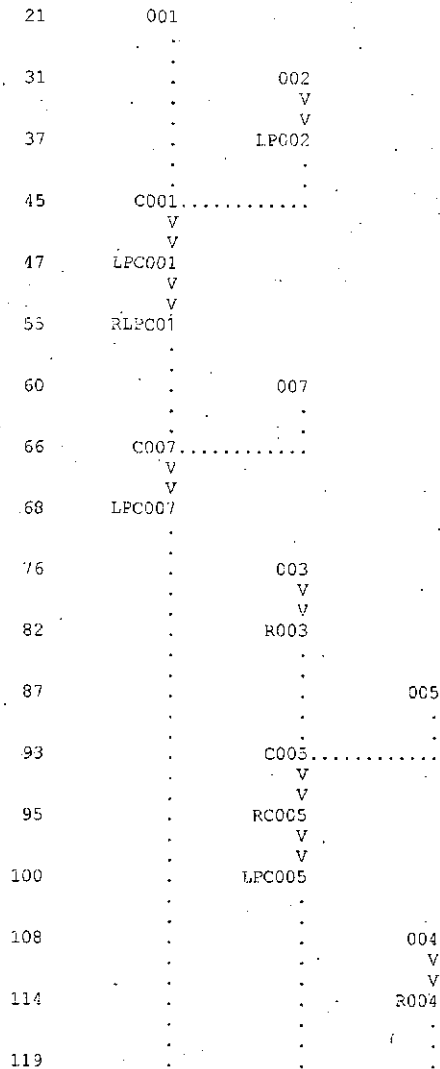
784	KK	RCP6	ROUTE									
785	RS	1	FLOW									
786	RC	0.035	0.035	0.035	260	0.0350	0.00					
787	RX	0.00	50.00	100.00	105.00	110.00	115.00	165.00	215.00			
788	RY	3.00	2.50	2.00	0.00	0.00	2.00	2.50	3.00			
	*											

789	KK	CP6	BASIN									
790	BA	0.001										
791	LG	0.35	0.35	3.75	0.54	0						
792	UC	0.121	0.165									
793	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
794	UA	100										
	*											

795	KK	CCP6	COMBINE									
796	HC	2										
	*											
797	ZZ											

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW



```
125 . . . . . C006.....  
    . . . . . V  
    . . . . . V  
127 . . . . . LPC006  
    . . . . .  
    . . . . .  
135 . . . . . 009  
    . . . . .  
    . . . . .  
141 . . . . . C009.....  
    . . . . . V  
    . . . . . V  
143 . . . . . LPC009  
    . . . . .  
    . . . . .  
151 . . . . . 012  
    . . . . .  
    . . . . .  
157 . . . . . C012.....  
    . . . . . V  
    . . . . . V  
159 . . . . . LPC012  
    . . . . .  
    . . . . .  
167 . . . . . 008  
    . . . . .  
    . . . . .  
173 . . . . . 010  
    . . . . .  
    . . . . .  
179 . . . . . C010.....  
    . . . . . V  
    . . . . . V  
181 . . . . . RC010  
    . . . . .  
    . . . . .  
186 . . . . . 011  
    . . . . .  
    . . . . .  
192 . . . . . C011.....  
    . . . . .  
    . . . . .  
194 . . . . . C013I.....  
    . . . . . V  
    . . . . . V  
196 . . . . . LPC13I  
    . . . . .  
    . . . . .  
204 . . . . . 013  
    . . . . .  
    . . . . .  
210 . . . . . C013.....  
    . . . . . V  
    . . . . . V  
212 . . . . . LPC013  
    . . . . . V  
    . . . . . V  
220 . . . . . RLPC13  
    . . . . .  
    . . . . .  
225 . . . . . 014  
    . . . . . V  
    . . . . . V  
231 . . . . . LP014  
    . . . . . V  
    . . . . . V  
239 . . . . . RLP014  
    . . . . .  
    . . . . .  
244 . . . . . 016  
    . . . . . V  
    . . . . . V  
250 . . . . . LP016  
    . . . . . V  
    . . . . . V  
257 . . . . . RLP016  
    . . . . .  
    . . . . .  
262 . . . . . 015  
    . . . . .  
    . . . . .  
268 . . . . . 017  
    . . . . .  
    . . . . .  
280 . . . . . -----> DR017  
274 . . . . . DV017  
    . . . . .  
    . . . . .  
283 . . . . . C017.....  
    . . . . . V  
    . . . . . V
```

285	RC017		
291		018	
297	C018		
300		019	
306			020
312	C020		
	V		
	V		
314	RC020		
320			021
326	C021		
329			022
			V
			V
335	LP022		
343			023
			V
			V
349		LP023	
357	C023		
	V		
	V		
359	RLPC23		
364			024
370	C024		
	V		
	V		
372	LP024		
	V		
	V		
379	RLPC24		
385			025
391	C025		
394			026
			V
			V
400		LP026	
			V
			V
407		RLP026	
412			028
			V
			V
418		LP028	
426			030
			V
			V
432		LP030	
440			029
446			C029
			V
			V
448			LP029

456	027	.
462	031
468	C031
	V		.
	V		.
470	RC031		.
476	032

482	C032
	V		.
	V		.
484	LPC032		.
	V		.
	V		.
492	RLPC32		.
498	033

504	C033
506
	034		.
	V		.
	V		.
512	R034		.
517	035

523	036

529	C036
532
	037		.
538
	C037
	V	
	V	
542	DET#1	
	V	
	V	
556	DET#2	

573	.	----->	OVER
571	DDET#2	
	V	
	V	
576	RDET#2	
582	.	038
	.	V
	.	V
588	.	LP038
596	040

602	.	C040
	.	V	
	.	V	
604	.	LP040	
	.	V	
	.	V	
612	.	RLP040	
617	042

623
	C042
626	.	039
	.	V
	.	V
632	.	LP039

640	.	.	041
646	.	CO41
	.	V	
	.	V	
648	.	LPC41	
	.	V	
	.	V	
656	.	RLP041	
661	.	.	C43
667	.	CO43
670	.	COMP
	.	V	
	.	V	
674	.	RCF1	
679	.	CF1	
685	.	CCF1
689	.	.	OVER
687	.	ODET#2	←-----
	.	V	
	.	V	
690	.	RCF2A	
695	.	.	OFF1
	.	V	
	.	V	
701	.	RCF2B	
706	.	.	CF2
712	.	CCF2A
714	.	CCF2B
	.	V	
	.	V	
716	.	RCF3	
721	.	CF3	
727	.	CCF3
729	.	OFF2	
	.	V	
	.	V	
735	.	RCF4A	
740	.	.	OFF3
	.	V	
	.	V	
746	.	RCF4B	
751	.	.	CF4
757	.	CCF4
759	.	.	OFF4
	.	V	
	.	V	
765	.	RCF5	
770	.	.	CF5
776	.	CCF5
778	.	.	OFF5

```

      . . . . . V
      . . . . . V
784 . . . . . RCF6
      . . . . .
      . . . . .
789 . . . . .
      . . . . .
      . . . . .
      . . . . .
795 . . . . . CCF6.....

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   JUN 1998                       *
*   VERSION 4.1                     *
*
* RUN DATE 02JUN14 TIME 14:32:22 *
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

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Cavaliere Flats Rezoning
City of Scottsdale
Prepared by: Kimley-Horn and Associates, Inc.

Existing Conditions model
100-year, 6-hour Storm event

Based on Troon North Park Model:

PROJECT NAME: TROON NORTH PARK
PREPARED FOR: CITY OF SCOTTSDALE
PREPARED BY: ARGUS CONSULTING, P.C.

JOB#: 236-00

FILE NAME : TNP100YR.DAT
STORM EVENT : 100-YR/6-HR
Unit Hydrograph: Clark
01/24/2011

```

20 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN       5  MINUTES IN COMPUTATION INTERVAL
          IDATE      6JUN99  STARTING DATE
          ITIME      0000  STARTING TIME
          NQ         2000  NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     12JUN99  ENDING DATE
          NDTIME     2235  ENDING TIME
          ICENT      19  CENTURY MARK

          COMPUTATION INTERVAL .08 HOURS
          TOTAL TIME BASE 166.58 HOURS

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ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH  INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME     ACRE-FEET
SURFACE AREA       ACRES
TEMPERATURE        DEGREES FAHRENHEIT

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-----DSS---ZOPEN: Existing File Opened, File: 01-100.DSS
Unit: 71; DSS Version: 6-JG
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/INFLOW/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/INFLOW/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/INFLOW/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/INFLOW/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/INFLOW/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/INFLOW/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/INFLOW/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/INFLOW/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH1/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH1/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH1/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH1/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH1/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH1/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH1/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH1/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH2/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH2/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH2/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH2/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH2/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH2/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 16: /TROON/DETH2/FLOW/11JUN1999/5MIN//

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-----DSS--ZWRITE Unit 71; Vers. 16: /TROON/DFT#2/ELOW/12JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO42/ELOW/05JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO42/ELOW/06JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO42/ELOW/07JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO42/ELOW/08JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO42/ELOW/09JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO42/ELOW/10JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO42/ELOW/11JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO42/ELOW/12JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO43/ELOW/05JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO43/ELOW/06JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO43/ELOW/07JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO43/ELOW/08JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO43/ELOW/09JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO43/ELOW/10JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO43/ELOW/11JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/CO43/ELOW/12JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/COMB/ELOW/05JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/COMB/ELOW/06JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/COMB/ELOW/07JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/COMB/ELOW/08JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/COMB/ELOW/09JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/COMB/ELOW/10JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/COMB/ELOW/11JUN1999/5MIN//
 -----DSS--ZWRITE Unit 71; Vers. 8: /PVISTA/COMB/ELOW/12JUN1999/5MIN//

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	001	185.	4.08	20.	5.	2.	.11		
HYDROGRAPH AT	002	25.	4.08	3.	1.	0.	.02		
ROUTED TO	LPC002	24.	4.08	3.	1.	0.	.02	2724.63	4.08
2 COMBINED AT	C001	209.	4.08	23.	6.	2.	.13		
ROUTED TO	LPC001	206.	4.08	23.	6.	2.	.13	2717.79	4.08
ROUTED TO	RJLPC01	200.	4.17	23.	6.	2.	.13	92.53	4.17
HYDROGRAPH AT	C07	36.	4.00	4.	1.	0.	.02		
2 COMBINED AT	C007	226.	4.17	26.	7.	2.	.15		
ROUTED TO	LPC007	222.	4.08	26.	7.	2.	.15	74.67	4.08
HYDROGRAPH AT	003	34.	4.00	3.	1.	0.	.02		
ROUTED TO	R003	34.	4.08	3.	1.	0.	.02	91.08	4.08
HYDROGRAPH AT	005	14.	4.00	1.	0.	0.	.01		
2 COMBINED AT	C005	47.	4.08	5.	1.	0.	.02		
ROUTED TO	RC005	47.	4.08	5.	1.	0.	.02	91.27	4.08
ROUTED TO	LPC005	44.	4.17	5.	1.	0.	.02	86.16	4.17
HYDROGRAPH AT	004	23.	4.00	2.	1.	0.	.01		
ROUTED TO	R004	23.	4.08	2.	1.	0.	.01	90.84	4.08

		015	112.	4.08	13.	3.	1.	.06		
+	HYDROGRAPH AT		017	29.	4.08	4.	1.	0.	.02	
+	DIVERSION TO		DR017	29.	.00	4.	1.	0.	.02	
+	HYDROGRAPH AT		DV017	0.	.00	0.	0.	0.	.02	
+	4 COMBINED AT		C017	199.	4.08	22.	5.	2.	.13	
-	ROUTED TO		RC017	195.	4.08	22.	5.	2.	.13	91.87 4.08
+	HYDROGRAPH AT		018	92.	4.08	11.	3.	1.	.05	
+	3 COMBINED AT		C018	610.	4.17	83.	21.	7.	.44	
+	HYDROGRAPH AT		019	102.	4.00	13.	3.	1.	.05	
+	HYDROGRAPH AT		020	20.	4.08	3.	1.	0.	.02	
+	2 COMBINED AT		C020	122.	4.08	16.	4.	1.	.07	
+	ROUTED TO		RC020	122.	4.08	16.	4.	1.	.07	91.94 4.08
+	HYDROGRAPH AT		021	47.	4.08	6.	2.	1.	.02	
+	2 COMBINED AT		C021	169.	4.08	22.	6.	2.	.09	
+	HYDROGRAPH AT		022	105.	4.08	12.	3.	1.	.07	
+	ROUTED TO		LP022	104.	4.08	12.	3.	1.	.07	75.45 4.08
+	HYDROGRAPH AT		023	118.	4.08	13.	3.	1.	.06	
+	ROUTED TO		LP023	97.	4.17	13.	3.	1.	.06	76.45 4.17
+	2 COMBINED AT		C023	199.	4.17	25.	6.	2.	.13	
+	ROUTED TO		RLPC23	198.	4.17	25.	6.	2.	.13	92.60 4.17
+	HYDROGRAPH AT		024	33.	4.00	4.	1.	0.	.02	
+	2 COMBINED AT		C024	225.	4.17	29.	7.	2.	.15	
+	ROUTED TO		LPC024	218.	4.25	28.	7.	2.	.15	48.93 4.25
+	ROUTED TO		RLPC24	219.	4.25	28.	7.	2.	.15	91.65 4.25
+	HYDROGRAPH AT		025	56.	4.08	8.	2.	1.	.03	
+	2 COMBINED AT		C025	258.	4.25	36.	9.	3.	.18	
+	HYDROGRAPH AT		026	74.	4.00	7.	2.	1.	.04	
-	ROUTED TO		LP026	74.	4.08	7.	2.	1.	.04	2702.94 4.08
-	ROUTED TO		RLP026	68.	4.17	7.	2.	1.	.04	

+		DDET#2	348.	4.67	187.	48.	16.	1.12		
	ROUTED TO									
+		RDET#2	349.	4.75	187.	48.	16.	1.12		
+									13.07	4.75
	HYDROGRAPH AT									
+		038	77.	4.08	10.	2.	1.	.05		
	ROUTED TO									
+		LP038	76.	4.08	10.	2.	1.	.05		
+									26.89	4.08
	HYDROGRAPH AT									
+		040	35.	4.08	4.	1.	0.	.02		
	2 COMBINED AT									
+		C040	111.	4.08	14.	4.	1.	.06		
	ROUTED TO									
+		LP040	110.	4.08	14.	4.	1.	.06		
+									9.56	4.08
	ROUTED TO									
+		RLP040	107.	4.17	14.	4.	1.	.06		
+									11.41	4.17
	HYDROGRAPH AT									
+		042	25.	4.17	4.	1.	0.	.02		
	3 COMBINED AT									
+		C042	386.	4.67	205.	52.	17.	1.20		
	HYDROGRAPH AT									
+		039	58.	4.08	8.	2.	1.	.04		
	ROUTED TO									
+		LP039	58.	4.08	8.	2.	1.	.04		
+									19.26	4.08
	HYDROGRAPH AT									
+		041	16.	4.08	2.	1.	0.	.01		
	2 COMBINED AT									
+		C041	74.	4.08	11.	3.	1.	.05		
	ROUTED TO									
+		LP041	73.	4.08	11.	3.	1.	.05		
+									9.68	4.08
	ROUTED TO									
+		RLP041	71.	4.17	11.	3.	1.	.05		
+									11.35	4.17
	HYDROGRAPH AT									
+		043	13.	4.08	2.	0.	0.	.01		
	2 COMBINED AT									
+		C043	83.	4.17	12.	3.	1.	.05		
	2 COMBINED AT									
+		COMB	443.	4.25	217.	55.	18.	1.25		
	ROUTED TO									
+		RCF1	443.	4.25	217.	55.	18.	1.25		
+									2.76	4.25
	HYDROGRAPH AT									
+		CF1	19.	4.00	2.	1.	0.	.01		
	2 COMBINED AT									
+		CCF1	453.	4.25	219.	56.	19.	1.26		
	HYDROGRAPH AT									
+		ODET#2	190.	5.17	28.	7.	2.	.00		
	ROUTED TO									
+		RCF2A	188.	5.17	28.	7.	2.	.00		
+									1.47	5.17
	HYDROGRAPH AT									
+		OFF1	466.	4.25	55.	14.	5.	.31		
	ROUTED TO									
+		RCF2B	470.	4.25	55.	14.	5.	.31		
+									1.10	4.25
	HYDROGRAPH AT									
+		CF2	29.	4.00	3.	1.	0.	.01		
	3 COMBINED AT									
+		CCF2A	484.	4.25	86.	21.	7.	.32		

+	2 COMBINED AT	CCF2B	937.	4.25	304.	77.	26.	1.58		
	ROUTED TO	RCF3	937.	4.25	304.	77.	26.	1.58	3.03	4.25
-	HYDROGRAPH AT	CF3	64.	4.00	7.	2.	1.	.03		
+	2 COMBINED AT	CCF3	971.	4.25	311.	79.	26.	1.61		
	HYDROGRAPH AT	OFF2	21.	4.08	2.	1.	0.	.01		
-	ROUTED TO	RCF4A	20.	4.08	2.	1.	0.	.01	.47	4.08
	HYDROGRAPH AT	OFF3	11.	4.08	1.	0.	0.	.01		
+	ROUTED TO	RCF4B	11.	4.17	1.	0.	0.	.01	.34	4.17
	HYDROGRAPH AT	CF4	25.	4.17	3.	1.	0.	.02		
+	3 COMBINED AT	CCF4	56.	4.17	6.	2.	1.	.04		
	HYDROGRAPH AT	OFF4	109.	4.08	11.	3.	1.	.06		
+	ROUTED TO	RCF5	107.	4.17	11.	3.	1.	.06	.79	4.17
	HYDROGRAPH AT	CF5	4.	4.00	0.	0.	0.	.00		
-	2 COMBINED AT	CCF5	109.	4.17	11.	3.	1.	.06		
	HYDROGRAPH AT	OFF5	11.	4.17	1.	0.	0.	.01		
+	ROUTED TO	RCF6	11.	4.17	1.	0.	0.	.01	.43	4.17
	HYDROGRAPH AT	CF6	1.	4.00	0.	0.	0.	.00		
+	2 COMBINED AT	CCF6	12.	4.17	1.	0.	0.	.01		

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPO02
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1

ELEVATION	INITIAL VALUE	SPELLWAY CREST	TOP OF DAM
STORAGE	2722.00	2725.00	2725.00
OUTFLOW	0.	0.	0.
		30.	30.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	2724.63	.00	0.	24.	.00	4.08	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC001
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1

ELEVATION	INITIAL VALUE	SPELLWAY CREST	TOP OF DAM
STORAGE	2713.00	2717.00	2717.00
OUTFLOW	0.	0.	0.
		90.	90.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	2717.79	.79	0.	206.	.58	4.08	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC007
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1

INITIAL VALUE	SPELLWAY CREST	TOP OF DAM
---------------	----------------	------------

ELEVATION	68.00	74.00	74.00
STORAGE	0.	0.	0.
OUTFLOW	0.	143.	143.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
--------------	----------------------------	------------------------	-----------------------	---------------------	-------------------------	---------------------------	-----------------------

1.00	74.67	.67	0.	222.	.50	4.08	.00
------	-------	-----	----	------	-----	------	-----

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC005
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM

ELEVATION	83.00	88.00	88.00
STORAGE	0.	0.	0.
OUTFLOW	0.	62.	62.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1.00	86.16	.00	0.	41.	.00	4.17	.00
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SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC006
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM

ELEVATION	80.00	86.00	86.00
STORAGE	0.	1.	1.
OUTFLOW	0.	51.	51.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1.00	84.78	.00	0.	43.	.00	4.17	.00
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SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC009
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM

ELEVATION	75.00	80.00	80.00
STORAGE	0.	1.	1.
OUTFLOW	0.	124.	124.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1.00	78.71	.00	0.	96.	.00	4.17	.00
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SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC012
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM

ELEVATION	67.50	73.00	73.00
STORAGE	0.	0.	0.
OUTFLOW	0.	133.	133.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1.00	71.49	.00	0.	100.	.00	4.25	.00
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SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC013
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM

ELEVATION	61.00	66.00	66.00
STORAGE	0.	1.	1.
OUTFLOW	0.	266.	266.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1.00	66.51	.51	1.	338.	.33	4.17	.00
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SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC013
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM

ELEVATION	55.80	61.00	61.00
STORAGE	0.	0.	0.
OUTFLOW	0.	272.	272.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	61.76	.76	1.	392.	.42	4.25	.00
	SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP014 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	64.50	70.00	70.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	48.	48.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	68.40	.00	0.	37.	.00	4.08	.00
	SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP022 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	71.00	77.00	77.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	127.	127.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	75.45	.00	0.	104.	.00	4.08	.00
	SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP023 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	73.00	77.00	77.00
	STORAGE	0.	1.	1.
	OUTFLOW	0.	113.	113.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	76.45	.00	1.	97.	.00	4.17	.00
	SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP028 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	28.00	32.00	32.00
	STORAGE	0.	1.	1.
	OUTFLOW	0.	0.	0.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	31.25	.00	0.	0.	.00	.00	.00
	SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP030 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	11.00	16.00	16.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	0.	0.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	16.08	.08	0.	3.	162.50	4.17	.00
	SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC029 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)							

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	1.00	8.00	8.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	38.	38.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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	RATIO OF PMF	RESERVOIR W.S.ELEV	DEPTH OVER DAM	STORAGE AC-FT	OUTFLOW CFS	OVER TOP HOURS	MAX OUTFLOW HOURS	FAILURE HOURS
1	1.00	2.50	.00	0.	10.	.00	4.17	.00
		SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPO32						
		(PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)						

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	77.00	81.00	81.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	63.	63.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	81.36	.36	0.	95.	.25	4.08	.00
		SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DET#2						
		(PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)						

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	2590.30	2607.00	2607.00
	STORAGE	0.	29.	29.
	OUTFLOW	0.	378.	378.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	2608.11	1.11	33.	538.	1.33	5.17	.00
		SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP038						
		(PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)						

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	24.60	29.00	29.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	165.	165.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	26.89	.00	0.	76.	.00	4.08	.00
		SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP040						
		(PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)						

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	6.60	11.00	11.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	165.	165.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	9.56	.00	0.	110.	.00	4.08	.00
		SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP039						
		(PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)						

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	17.00	22.00	22.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	135.	135.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	19.26	.00	0.	58.	.00	4.08	.00
		SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP041						
		(PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)						

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	7.00	11.00	11.00
	STORAGE	0.	0.	0.
	OUTFLOW	0.	113.	113.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1.00 9.68 .00 0. 73. .00 4.08 .00

*** NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 01-100.DSS
Pointer Utilization: .27
Number of Records: 48
File Size: 92.1 Kbytes
Percent Inactive: .0

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 21AUG14 TIME 15:10:15
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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
    
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION. NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Cavalliere Flats Rezoning
2 ID City of Scottsdale
3 ID Prepared by: Kimley-Horn and Associates, Inc.
4 ID
5 ID Proposed Conditions model
6 ID 10-year, 6-hour Storm event
7 ID
8 ID Based on Troon North Park Model:
9 ID
10 ID PROJECT NAME: TROON NORTH PARK JOB#: 236-08
11 ID PREPARED FOR: CITY OF SCOTTSDALE
12 ID PREPARED BY : ARGUS CONSULTING, P.C.
13 ID
14 ID FILE NAME : TNP10YR.DAT
15 ID STORM EVENT : 10-YR/6-HR
16 ID Unit Hydrograph: Clark
17 ID 01/24/2011
*
*
*
18 IT 5 06JUN99 0000 2000
19 IN 15
20 TO 5
*DIAGRAM
*
21 KK 001 BASIN
22 BA 0.112
23 PB 2.071
24 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.075
25 PC 0.087 0.100 0.119 0.151 0.235 0.415 0.762 0.873 0.915 0.944
26 PC 0.936 0.967 0.979 0.989 1.000
27 LG 0.30 0.25 6.00 0.18 3
28 UC 0.344 0.316
29 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
30 UA 100
*
31 KK 002 BASIN
32 BA 0.0167
33 LG 0.30 0.25 6.00 0.18 7
34 UC 0.262 0.408
35 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
36 UA 100
*
37 KK LP002 ROUTE INFLOW THRU STORAGE AREA
38 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS.GIS MAPS
39 KM LOW FLOW OUTLET = 30" RCP
40 RS 1 STOR 0.0
41 SA .003 .005 .026 .059
42 SQ 0 5 15 30
43 SE 2722.0 2723.0 2724.0 2725.0
    
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HEC-1 INPUT

PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
44	ST	2725.0	40	2.7	1.5						
	*										
45	KK	C001	COMBINE HYDROGRAPHS FROM SUB 001 AND LP002								
46	HC	2									
	*										
47	KK	LPC001	ROUTE INFLOW THRU STORAGE AREA								
48	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
49	KM		LOW FLOW OUTLET = OPEN SECTION BETWEEN BERM AND WALL								
50	RS	1	STOR	0.0							
51	SA	.001	.004	.013	.104	.150					
52	SQ	0	6.5	23	50	90					
53	SE	2713.0	2714.0	2715.0	2716.0	2717.0					
54	ST	2717.0	45	2.7	1.5						
	*										
55	KK	RLPC01	ROUTE FLOW THRU SUB 007								
56	RS	1	FLOW	0							
57	RC	0.070	0.050	0.070	1736	0.025					
58	RX	1000	1001	1034	1047.5	1052.5	1066	1099	1100		
59	RY	100	95.5	94.5	90	90	94.5	95.5	100		
	*										
60	KK	007	BASIN								
61	BA	0.018									
62	LG	0.27	0.25	6.00	0.20	25					
63	UC	0.241	0.340								
64	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
65	UA	100									
	*										
66	KK	C007	COMBINE HYDROGRAPHS FROM SUB 007 AND RLPC01								
67	HC	2									
	*										
68	KK	LPC007	ROUTE INFLOW THRU STORAGE AREA								
69	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
70	KM		LOW FLOW OUTLET = 2-36" RCP								
71	RS	1	STOR	0.0							
72	SA	.001	.002	.004	.025	.040	.060	.100			
73	SQ	0	18	35	70	100	125	143			
74	SE	68.0	69.0	70.0	71.0	72.0	73.0	74.0			
75	ST	74.0	45	2.7	1.5						
	*										
76	KK	003	BASIN								
77	BA	0.018									
78	LG	0.30	0.25	6.00	0.18	2					
79	UC	0.206	0.235								
80	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
81	UA	100									
	*										

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HEC-1 INPUT

PAGE 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
82	KK	R003	ROUTE FLOW THRU SUB 005								
83	RS	1	FLOW	0							
84	RC	0.070	0.050	0.070	470	0.023					
85	RX	1000	1001	1045.5	1050	1055	1059.5	1099	1100		
86	RY	100	92.5	91.5	90	90	91.5	92.5	100		
	*										
87	KK	005	BASIN								
88	BA	0.007									
89	LG	0.27	0.25	6.00	0.20	23					
90	UC	0.171	0.213								
91	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
92	UA	100									
	*										
93	KK	C005	COMBINE HYDROGRAPHS FROM SUB 005 AND R003								
94	HC	2									
	*										
95	KK	RC005	ROUTE FLOW THRU SUB 006								
96	RS	1	FLOW	0							
97	RC	0.070	0.050	0.070	450	0.022					
98	RX	1000	1001	1045.5	1050	1055	1059.5	1099	1100		
99	RY	100	92.5	91.5	90	90	91.5	92.5	100		
	*										
100	KK	LPCC05	ROUTE INFLOW THRU STORAGE AREA								
101	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
102	KM		LOW FLOW OUTLET = 2-24" RCP								
103	RS	1	STOR	0							
104	SA	.001	.002	.030	.090	.190	.271				

1 HEC-1 INPUT PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

231 KK LP014 ROUTE INFLOW THRU STORAGE AREA
 232 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 233 KM LOW FLOW OUTLET = 30" RCP
 234 RS .1 STOR 0
 235 SA .001 .002 .003 .014 .054 .117
 236 SQ 0 10 22 33 42 48
 237 SE 64.5 66.0 67 68 69 70.0
 238 ST 70.0 45 2.7 1.5
 *

239 KK RLP014 ROUTE FLOW THRU SUB 015
 240 RS 1 FLOW 0
 241 RC 0.070 0.050 0.070 1764 0.021
 242 RX 1000 1001 1045.5 1050 1055 1059.5 1099 1100
 243 RY 100 92.5 91.5 90 90 91.5 92.5 100
 *

244 KK 016 BASIN
 245 BA 0.031
 246 LG 0.28 0.25 6.00 0.22 19
 247 UC 0.225 0.190
 248 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 249 UA 100
 *

250 KK LPO16 ROUTE INFLOW THRU STORAGE AREA
 251 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 252 KM LOW FLOW OUTLET = NONE
 253 RS 1 STOR 0
 254 SA .0891 .939 1.154
 255 SQ 0 0 185
 256 SE 42.0 43.0 44.0
 *

257 KK RLP016 ROUTE FLOW THRU SUB 015
 258 RS 1 FLOW 0
 259 RC 0.070 0.050 0.070 730 0.016
 260 RX 1000 1001 1045.5 1050 1055 1059.5 1099 1100
 261 RY 100 92.5 91.5 90 90 91.5 92.5 100
 *

262 KK 015 BASIN
 263 BA 0.060
 264 LG 0.28 0.25 6.00 0.20 30
 265 UC 0.278 0.251
 266 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 267 UA 100
 *

1 HEC-1 INPUT PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

268 KK 017 BASIN
 269 BA 0.017
 270 LG 0.28 0.25 6.00 0.23 28
 271 UC 0.217 0.306
 272 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 273 UA 100
 *

274 KK DV017 SPLIT HYDROGRAPH SB017. THAT PORTION INTERCEPTED BY DOUBLE CATCH
 275 KM BASIN AND STORM DRAIN @ INTERSECTION OF 108TH WY AND GRAYTHORN DR
 276 KM IS DIVERTED FROM PRE-DEVELOPED FLOW PATH TO SBC65. FLOW-BY
 277 KM HYDROGRAPH WILL BE LABELED "DR140" WHICH WILL BE RETRIEVED LATER
 278 KM AND ROUTED THRU SB145 AS "RR145". DI-DQ RECORD IS FROM GILBERTSON
 279 KM AND ASSOC PARCEL "J" HEC-1 MODEL 90092100.DAT DATED 01-29-96.
 280 DT DR017

* KK DIV1
 * KM DIVERT 32 CFS TO CP2I
 * DT DIV1
 281 DI 0 32 45 100
 282 DQ 0 32 32 32
 *

283 KK C017 COMBINE HYDROGRAPHS FROM DV017, SUB 015, RLP016 AND RLP014
 284 HC 4
 *

285 KK RC017 "n" VALUE AND GEOMETRY MODIFIED FROM PRE-DEVELOPED MODEL TO REFLECT
 286 KM ANTICIPATED DEVELOPMENT.
 287 RS 1 FLOW 0
 288 RC 0.070 0.040 0.070 690 0.011
 289 RX 1000 1001 1025.0 1040 1055 1070.0 1099 1100
 290 RY 100 97.0 95.0 90 90 95.0 97.0 100
 *

291 KK 018 BASIN
 292 BA 0.050
 293 LG 0.22 0.21 6.40 0.18 33
 294 UC 0.277 0.277
 295 JA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 296 JA 100
 *

297 KK C018 COMBINE HYDROGRAPHS FROM SUB 018, RC017 AND RLPC13
 298 KM THIS IS TNP INFLOW FROM ALMA SCHOOL RD. CULVERT
 299 HC 3
 *

300 KK 019 BASIN
 301 BA 0.050
 302 LG 0.26 0.25 6.00 0.20 57
 303 UC 0.238 0.215
 304 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 305 UA 100
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

306 KK 020 BASIN
 307 BA 0.016
 308 LG 0.28 0.25 6.00 0.20 18
 309 UC 0.298 0.566
 310 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 311 UA 100
 *

312 KK C020 COMBINE HYDROGRAPHS FROM SUB 019 AND SUB 020
 313 HC 2
 *

314 KK RCC20 NOTE: "n" VALUE AND GEOMETRY MODIFIED FROM PRE-DEVELOPED MODEL
 315 KM TO REFLECT EXISTING DEVELOPMENT.
 316 RS 1 FLOW 0
 317 RC 0.070 0.040 0.070 880 0.018
 318 RX 1000 1001 1034 1047.5 1052.5 1066 1099 1100
 319 RY 100 95.5 94.5 90 90 94.5 95.5 100
 *

320 KK 021 BASIN
 321 BA 0.024
 322 LG 0.26 0.25 6.00 0.20 56
 323 UC 0.225 0.250
 324 JA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 325 UA 100
 *

326 KK C021 COMBINE HYDROGRAPHS FROM SUB 021 AND RC020
 327 KM THIS IS TNP INFLOW FROM C021
 328 HC 2
 *

329 KK 022 BASIN
 330 BA 0.067
 331 LG 0.29 0.25 6.00 0.19 10
 332 UC 0.318 0.364
 333 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 334 UA 100
 *

335 KK LP022 ROUTE INFLOW THRU STORAGE AREA
 336 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 337 KM LOW FLOW OUTLET = 4-24" RCP
 338 RS 1 STOR 0
 339 SA .001 .004 .009 .011 .013 .060 .140
 340 SQ 0 17 48.5 79.5 97.0 113 127
 341 SE 71.0 72.0 73.0 74.0 75.0 76.0 77.0
 342 ST 77.0 45 2.7 1.5
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

343 KK 023 BASIN
 344 BA 0.065
 345 LG 0.27 0.25 6.00 0.20 22
 346 UC 0.276 0.261
 347 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 348 UA 100
 *

349 KK LP023 ROUTE INFLOW THRU STORAGE AREA
 350 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 351 KM LOW FLOW OUTLET = 3-30" RCP
 352 RS 1 STOR 0
 353 SA .001 .052 .288 .518 .751


```

*
482 KK C032 COMBINE HYDROGRAPHS FROM SUB 032 AND RC031
483 HC 2
*
484 KK LPC032 ROUTE INFLOW THRU STORAGE AREA
485 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
486 KM LOW FLOW OUTLET = 42" RCP
487 RS 1 STOR 0
488 SA .001 .007 .070 .136 .270
489 SQ 0 11 23 46 63
490 SE 77.0 78.0 79.0 80.0 81.0
491 ST 81.0 45 2.7 1.5
*
    
```

1 HEC-1 INPUT PAGE 14

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

492 KK RLPC32
493 KM ROUTE FLOW THRU SUB 033
494 RS 4 FLOW 0
495 RC 0.070 0.050 0.070 2176 0.021
496 RX 1000 1001 1039 1045 1055 1061 1099 1100
497 RY 100 93 92 90 90 92 93 100
*
    
```

```

498 KK 033 BASIN
499 BA 0.147
500 LG 0.27 0.25 6.00 0.20 36
501 UC 0.338 0.277
502 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
503 UA 100
*
    
```

```

504 KK C033 COMBINE HYDROGRAPHS FROM SUB 033, RLPC32 AND RLP026
505 HC 3
*
    
```

```

506 KK 034 BASIN
507 BA 0.093
508 LG 0.30 0.25 6.00 0.18 1
509 UC 0.272 0.231
510 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
511 UA 100
*
    
```

```

512 KK R034 ROUTE FLOW THRU SUB 035
513 RS 1 FLOW 0
514 RC 0.045 0.040 0.045 2178 0.020
515 RX 1000 1001 1010 1022 1032 1044 1053 1054
516 RY 95.5 95.0 94.0 90.0 90.0 94.0 95.0 95.5
*
    
```

```

517 KK 035 BASIN
518 BA 0.022
519 LG 0.30 0.23 6.20 0.17 5
520 UC 0.187 0.194
521 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
522 UA 100
*
    
```

```

523 KK 036 BASIN
524 BA 0.008
525 LG 0.29 0.23 6.20 0.18 23
526 UC 0.140 0.165
527 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
528 UA 100
*
    
```

1 HEC-1 INPUT PAGE 15

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

529 KK C036 COMBINE HYDROGRAPHS FROM SUB 035, SUB 036, C033 AND R034
530 KM THIS IS TNP INFLOW FROM C036
531 HC 4
*
    
```

```

532 KK 037 BASIN
533 BA 0.041
534 LG 0.30 0.23 6.20 0.17 1
535 UC 0.187 0.146
536 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
537 UA 100
*
    
```

```

538 KK C037 COMBINE ALL HYDROGRAPHS
539 KM THIS REPRESENTS TOTAL INFLOW TO BASIN No.1
540 HC 5
541 ZW A=TROON B=INFLOW C=FLOW
*
    
```

542 KK DET#1 ROUTE INFLOW THRU DET BASIN #1 (UPPER BASIN)
 543 KM LOW FLOW OUTLET = 4-42" RCP
 544 KM INLET INVERT ELEV = 2598.80
 545 KM OUTLET INVERT ELEV = 2592.0
 546 KM SPILLWAY ELEV = 2610.0
 547 KM WEIR LENGTH = 50'
 548 RS 1 STOR 0.0
 549 SA 0.023 0.11 0.49 1.23 3.50 4.00 4.24 4.57 4.85 5.33
 550 SA 5.95 6.67 7.80 8.95
 551 SQ 0 45 103.5 185 264 328.5 382 428.5 470 507
 552 SQ 543 575 725 999
 553 SE 2599 2600 2601 2602 2603 2604 2605 2606 2607 2608
 554 SE 2609 2610 2611 2612
 555 ZW A=TROON B=DETH1 C=FLOW
 *

556 KK DET#2 ROUTE INFLOW THRU DET BASIN #2 (LOWER BASIN)
 557 KM LOW FLOW OUTLET = 1-60" RCP
 558 KM INLET INVERT ELEV = 2590.36
 559 KM OUTLET INVERT ELEV = 2589.95
 560 KM SPILLWAY ELEV = 2607.0
 561 KM WEIR LENGTH = 35'
 562 RS 1 STOR 0.0
 563 SA 0.023 0.048 0.496 0.876 1.119 1.277 1.414 1.558 1.705 1.820
 564 SA 1.930 2.042 2.365 2.487 2.615 2.770 2.935 3.258
 565 SQ 0 11.5 29 47 78 112.5 148 177 204 226
 566 SQ 248 266 283 300.5 315 330 345 378
 567 SE 2590.3 2591 2592 2593 2594 2595 2596 2597 2598 2599
 568 SE 2600 2601 2602 2603 2604 2605 2606 2607
 569 ST 2607 35 3.0 1.5
 570 ZW A=TROON B=DETH2 C=FLOW
 *

* *****
 * *****
 * ***** START CAVALLIERE FLATS MODEL CHANGES *****
 * *****
 * *****
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

571 KK DDET#2
 572 KM Outlet channel capacity is 348 cfs. Divert overlapping flow.
 573 DT OVER
 574 DI 0 348 348.1 10000
 575 DQ 0 0 0.1 9652
 *

* *****
 * *****
 * ***** END CAVALLIERE FLATS MODEL CHANGES *****
 * *****
 * *****
 *

576 KK RDET#2 ROUTE FLOW THRU SUB 042
 577 KM OUTLET CHANNEL TROON NORTH PARK BASIN
 578 RS 1 FLOW 0
 579 RC 0.040 0.035 0.040 594 0.010
 580 RX 990 992 994 1000 1010 1016 1018 1020
 581 RY 14.0 13.5 13.0 10.0 10.0 13.0 13.5 14.0
 *

* *****
 * *****
 * ***** STARTS DOWNSTREAM OFFSITE AREAS MODEL *****
 * *****
 * *****
 *

582 KK 038 BASIN
 583 BA 0.047
 584 LG 0.28 0.25 6.00 0.21 26
 585 UC 0.234 0.266
 586 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 587 CA 100
 *

588 KK LPC38 ROUTE INFLOW THRU STORAGE AREA
 589 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 590 KM LOW FLOW OUTLET = 3-36" RCP
 591 RS 1 STOR 0
 592 SA .001 .007 .047 .086 .160
 593 SQ 0 33 82 132 165
 594 SE 24.6 26.0 27.0 28.0 29.0
 595 ST 29.0 45 2.7 1.5
 *

596 KK 040 BASIN
 597 BA 0.018
 598 LG 0.26 0.25 6.00 0.20 49

599 UC 0.184 0.202
 600 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 601 JA 100
 *

HEC-1 INPUT

PAGE 17

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

602 KK C040 COMBINE HYDROGRAPHS FROM SUB 040 AND LP038
 603 HC 2
 *

604 KK LP040 ROUTE INFLOW THRU STORAGE AREA
 605 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 606 KM LOW FLOW OUTLET 3-36" RCP
 607 RS 1 STOR 0
 608 SA .001 .024 .077 .127
 609 SQ 0 82 132 165
 610 SE 06.6 09.0 10.0 11.0
 611 ST 11.0 45 2.7 1.5
 *

612 KK RLP040 ROUTE FLOW THRU SUB 042
 613 RS 1 FLOW 0
 614 RC 0.045 0.040 0.045 927 0.016
 615 RX 990 992 994 1000 1012 1018 1020 1022
 616 RY 13.2 13.1 13.0 10.0 10.0 13.0 13.1 13.2
 *

617 KK 042 BASIN
 618 BA 0.020
 619 LG 0.27 0.25 6.00 0.20 20
 620 UC 0.289 0.433
 621 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 622 UA 100
 *

623 KK C042 COMBINE HYDROGRAPHS FROM RDET#2, SUB 042 AND RLPC40
 624 HC 3
 625 ZW A=PVISTA B=C042 C=FLOW
 *

626 KK 039 BASIN
 627 BA 0.035
 628 LG 0.26 0.25 6.00 0.20 39
 629 UC 0.225 0.292
 630 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 631 UA 100
 *

632 KK LP039 ROUTE INFLOW THRU STORAGE AREA
 633 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 634 KM LOW FLOW OUTLET = 3-30" RCP
 635 RS 1 STOR 0
 636 SA .001 .007 .015 .024 .037
 637 SQ 0 49 84 112 135
 638 SE 17.0 19.0 20.0 21.0 22.0
 639 ST 22.0 45 2.7 1.5
 *

HEC-1 INPUT

PAGE 18

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

640 KK 041 BASIN
 641 BA 0.011
 642 LG 0.28 0.25 6.00 0.19 33
 643 UC 0.231 0.353
 644 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 645 UA 100
 *

646 KK C041 COMBINE HYDROGRAPHS FROM SUB 041 AND LP039
 647 HC 2
 *

648 KK LP041 ROUTE INFLOW THRU STORAGE AREA
 649 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 650 KM LOW FLOW OUTLET = 3-30" RCP
 651 RS 1 STOR 0
 652 SA .001 .020 .039 .072 .104
 653 SQ 0 14 49 84 113
 654 SE 07.0 08.0 09.0 10.0 11.0
 655 ST 11.0 45 2.7 1.5
 *

656 KK RLP041 ROUTE FLOW THRU SUB 041/042
 657 RS 1 FLOW 0
 658 RC 0.045 0.040 0.045 1314 0.017
 659 RX 990 992 994 1000 1008 1014 1016 1018
 660 RY 13.2 13.1 13.0 10.0 10.0 13.0 13.1 13.2
 *

661 KK 043 BASIN
 662 RA 0.008
 663 LG 0.26 0.25 6.00 0.20 29
 664 UC 0.231 0.376
 665 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 666 UA 100
 *

667 KK C043 COMBINE HYDROGRAPHS FROM SUB 043 AND RLP041
 668 HC 2
 669 ZW A=PVISTA B=C043 C=FLOW
 *

670 KK COMB COMBINE HYDROGRAPHS FROM C042 AND C043
 671 KM THIS REPRESENTS COMBINED FLOWS AT PINNACLE VISTA ROAD
 672 HC 2
 673 ZW A=PVISTA B=COMB C=FLOW
 *

* *****
 * *****
 * ***** START CAVALLIERS FLATS MODEL CHANGES *****
 * *****
 * *****
 * *****
 *

1

HEC-1 INPUT

PAGE 19

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

674 KK RCF1 ROUTE
 675 RS 1 FLOW
 676 RC 0.035 0.035 0.035 690 0.0200 0.00
 677 RX 0.00 30.00 67.50 72.50 77.50 82.50 120.00 150.00
 678 RY 3.00 2.50 2.00 0.00 0.00 2.00 2.50 3.00
 *

679 KK CF1 BASIN
 680 BA 0.016
 681 LG 0.25 0.25 6.00 0.21 62
 682 UC 0.164 0.162
 683 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 684 UA 100
 *

685 KK CCF1A COMBINE
 686 HC 2
 *

687 KK ODET#2
 688 KM Retrieve flow overtopping Troon North Park Basin outlet channel
 689 DR OVER
 *

690 KK RCF2A ROUTE
 691 RS 1 FLOW
 692 RC 0.035 0.035 0.035 990 0.0200 0.00
 693 RX 0.00 20.00 40.00 45.00 55.00 60.00 80.00 100.00
 694 RY 2.00 1.50 1.00 0.00 0.00 1.00 1.50 2.00
 *

695 KK CF2A BASIN
 696 BA 0.012
 697 LG 0.28 0.25 5.85 0.20 35
 698 UC 0.161 0.147
 699 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 700 UA 100
 *

701 KK OFF1 BASIN
 702 BA 0.307
 703 LG 0.32 0.31 6.00 0.17 9
 704 UC 0.462 0.356
 705 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 706 UA 100
 *

707 KK RCF2B ROUTE
 708 RS 1 FLOW
 709 RC 0.035 0.035 0.035 530 0.0270 0.00
 710 RX 0.00 15.00 30.00 35.00 50.00 95.00 110.00 125.00
 711 RY 2.00 1.50 1.00 0.00 0.00 1.00 1.50 2.00
 *

1

HEC-1 INPUT

PAGE 20

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

712 KK CF2B BASIN
 713 BA 0.002
 714 LG 0.10 0.40 6.00 0.15 0
 715 UC 0.136 0.221

776	KK	OFF5	BASIN									
777	BA	0.009										
778	LG	0.35	0.35	4.65	0.33	2						
779	JC	0.292	0.470									
780	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
781	UA	100										

782	KK	COFF4	COMBINE									
783	HC	2										

HEC-1 INPUT

PAGE 22

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

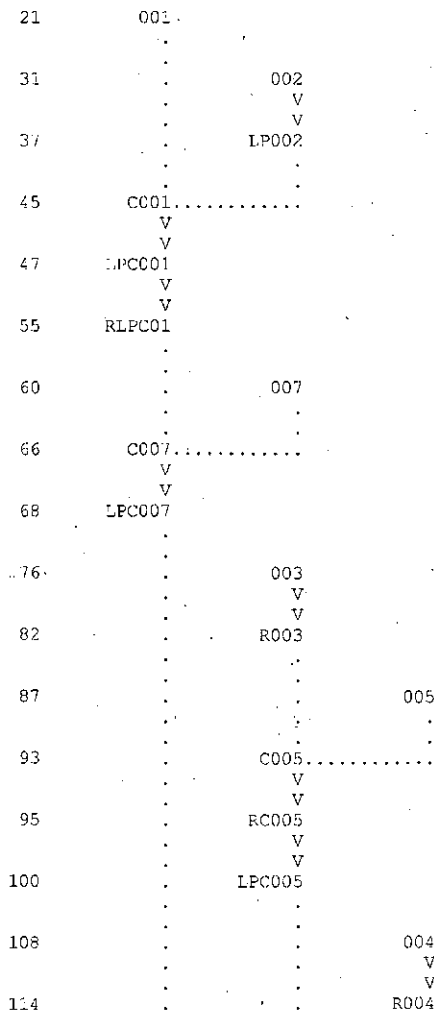
784	KK	RCF6	ROUTE									
785	RS	1	FLOW									
786	RC	0.035	0.035	0.035	270	0.0300	0.00					
787	RX	0.00	50.00	100.00	105.00	115.00	120.00	170.00	220.00			
788	RY	1.00	0.75	0.50	0.00	0.00	0.50	0.75	1.00			

789	KK	CF6	BASIN									
790	BA	0.004										
791	LG	0.25	0.25	5.05	0.33	36						
792	UC	0.208	0.258									
793	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0	
794	UA	100										

795	KK	CCF6	COMBINE									
796	HC	2										
797	ZZ											

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW



119				006
125		C006		
		V		
		V		
127		LPC006		
135				009
141		C009		
		V		
		V		
143		LPC009		
151			012	
157		C012		
		V		
		V		
159		LPC012		
167			008	
173				010
179		C010		
		V		
		V		
181		RC010		
186				011
192		C011		
194	C013T			
	V			
	V			
196	LPC13T			
204		013		
210	C013			
	V			
	V			
212	LPC013			
	V			
	V			
220	RLPC13			
225		014		
		V		
		V		
231		LP014		
		V		
		V		
239		RLP014		
244			016	
		V		
		V		
250		LP016		
		V		
		V		
257		RLP016		
262				015
268				017
280				DR017
274			DV017	
283		C017		

```

      V
      V
285   .   RC017
      .
291   .           018
      .
297   C018.....
      .
300   .   019
      .
306   .           020
      .
312   .   C020.....
      .
314   .   V
      .   V
      .   RC020
      .
320   .           021
      .
326   .   C021.....
      .
329   .           022
      .   V
      .   V
335   .   LP022
      .
343   .           023
      .   V
      .   V
349   .   LP023
      .
357   .   C023.....
      .   V
      .   V
359   .   RLPC23
      .
364   .           024
      .
370   .   C024.....
      .   V
      .   V
372   .   LPC024
      .   V
      .   V
379   .   RLPC24
      .
385   .           025
      .
391   .   C025.....
      .
394   .           026
      .   V
      .   V
400   .   LP026
      .   V
      .   V
407   .   RLP026
      .
412   .           028
      .   V
      .   V
418   .   LP028
      .
426   .           030
      .   V
      .   V
432   .   LP030
      .
440   .           029
      .
446   .   C029.....
      .   V
      .   V
448   .   LPC029
```



```
632 . LP039
    .
640 . . 041
    . . .
646 . C041.....
    . V
    . V
648 . LP041
    . V
    . V
656 . RLP041
    .
661 . . 043
    . . .
667 . C043.....
    .
670 . COMB.....
    . V
    . V
674 . RCF1
    .
679 . CF1
    .
685 . CCF1A.....
    .
689 . <----- OVER
687 . ODET#2
    . V
    . V
690 . RCF2A
    .
695 . CF2A
    .
701 . . OFF1
    . . V
    . . V
707 . . RCF2B
    . .
712 . . CF2B
    . .
718 . . CCF2B.....
    .
720 . CCF2A.....
    .
722 . CCF1B.....
    . V
    . V
724 . RCF3
    .
729 . CF3
    .
735 . CCF3.....
    .
737 . OFF2
    . V
    . V
743 . RCF4A
    .
748 . CF4A
    .
754 . . OFF3
    . .
760 . COFF3.....
    .
762 . CF4B
    .
768 . CCF4B.....
    .
770 . OFF4
    .
```

```
776 . . . OFF5
. . . .
782 . . COFF4.....
. . V
. . V
784 . . RCF6
. . .
789 . . . CF6
. . .
795 . . CCF6.....
```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```
1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
*
* RUN DATE 21AUG14 TIME 15:10:15 *
*
*****
```

```
*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****
```

Cavalliere Flats Rezoning
City of Scottsdale
Prepared by: Kimley-Horn and Associates, Inc.

Proposed Conditions model
10-year, 6-hour Storm event

Based on Troon North Park Model:

PROJECT NAME: TROON NORTH PARK
PREPARED FOR: CITY OF SCOTTSDALE
PREPARED BY : ARGUS CONSULTING, P.C.

JOB#: 236-08

FILE NAME : TNP10YR.DAT
STORM EVENT : 10-YR/6-HR
Unit Hydrograph: Clark
01/24/2011

```
20 IO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0. HYDROGRAPH PLOT SCALE
```

```
IT HYDROGRAPH TIME DATA
      NMIN      5 MINUTES IN COMPUTATION INTERVAL
      IDATE     6JUN99 STARTING DATE
      ITIME     0000 STARTING TIME
      NQ        2000 NUMBER OF HYDROGRAPH ORDINATES
      NDDATE    12JUN99 ENDING DATE
      NDTIME    2235 ENDING TIME
      ICENT     19 CENTURY MARK
```

COMPUTATION INTERVAL .08 HOURS
TOTAL TIME BASE 166.58 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

-----DSS---ZOPEN: Existing File Opened, File: 01-10.DSS
Unit: 71; DSS Version: 6-JG

```
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/06JUN1999/5MIN//
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-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C042/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C042/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C042/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C042/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C042/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C042/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C042/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C042/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C043/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C043/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C043/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C043/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C043/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C043/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C043/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/C043/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/COMB/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/COMB/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/COMB/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/COMB/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/COMB/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/COMB/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/COMB/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /PVISTA/COMB/FLOW/12JUN1999/5MIN//
    
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RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	001	83.	4.17	9.	2.	1.	.11		
HYDROGRAPH AT	002	11.	4.17	1.	0.	0.	.02		
ROUTED TO	LP002	11.	4.17	2.	0.	0.	.02	2723.63	4.17
2 COMBINED AT	C001	94.	4.17	11.	3.	1.	.13		
ROUTED TO	LPC001	92.	4.17	11.	3.	1.	.13	2717.03	4.17
ROUTED TO	RLPC01	88.	4.25	11.	3.	1.	.13	91.70	4.25
HYDROGRAPH AT	007	15.	4.08	2.	1.	0.	.02		
2 COMBINED AT	C007	100.	4.25	13.	3.	1.	.15		
ROUTED TO	LPC007	100.	4.25	13.	3.	1.	.15	72.00	4.25
HYDROGRAPH AT	003	16.	4.08	1.	0.	0.	.02		
ROUTED TO	R003	16.	4.08	1.	0.	0.	.02	90.69	4.08
HYDROGRAPH AT	005	7.	4.08	1.	0.	0.	.01		
2 COMBINED AT	C005	23.	4.08	2.	1.	0.	.02		
ROUTED TO	RC005	22.	4.08	2.	1.	0.	.02	90.85	4.08
ROUTED TO	LPC005	22.	4.17	2.	1.	0.	.02	84.76	4.17
HYDROGRAPH AT	004	11.	4.08	1.	0.	0.	.01		

+	ROUTED TO	R004	10.	4.17	1.	0.	0.	.01		
+									90.56	4.17
	HYDROGRAPH AT	006	16.	4.08	2.	0.	0.	.01		
+	2 COMBINED AT	C006	25.	4.08	3.	1.	0.	.03		
	ROUTED TO	LPC006	24.	4.17	3.	1.	0.	.03		
+									82.49	4.17
+	HYDROGRAPH AT	009	7.	4.08	1.	0.	0.	.01		
+	3 COMBINED AT	C009	52.	4.17	6.	1.	0.	.06		
	ROUTED TO	LPC009	52.	4.17	6.	1.	0.	.06		
+									77.35	4.17
+	HYDROGRAPH AT	012	4.	4.00	0.	0.	0.	.00		
+	2 COMBINED AT	C012	56.	4.17	6.	2.	1.	.06		
	ROUTED TO	LPC012	56.	4.17	6.	2.	1.	.06		
+									70.03	4.17
+	HYDROGRAPH AT	008	2.	4.08	0.	0.	0.	.00		
+	HYDROGRAPH AT	010	3.	4.08	0.	0.	0.	.00		
+	2 COMBINED AT	C010	5.	4.08	1.	0.	0.	.01		
	ROUTED TO	RC010	5.	4.08	1.	0.	0.	.01		
+									90.18	4.08
+	HYDROGRAPH AT	011	5.	4.08	1.	0.	0.	.00		
+	2 COMBINED AT	C011	10.	4.08	1.	0.	0.	.01		
+	3 COMBINED AT	C013I	160.	4.25	21.	5.	2.	.22		
	ROUTED TO	LPC13I	159.	4.25	21.	5.	2.	.22		
+									64.15	4.25
+	HYDROGRAPH AT	013	39.	4.08	5.	1.	0.	.04		
+	2 COMBINED AT	C013	190.	4.25	25.	6.	2.	.26		
	ROUTED TO	LPC013	191.	4.25	25.	6.	2.	.26		
+									59.51	4.25
+	ROUTED TO	RLPC13	185.	4.33	25.	6.	2.	.26		
+									91.85	4.33
+	HYDROGRAPH AT	014	19.	4.08	2.	1.	0.	.02		
	ROUTED TO	LP014	20.	4.08	2.	1.	0.	.02		
+									66.81	4.08
+	ROUTED TO	RLP014	17.	4.17	2.	1.	0.	.02		
+									90.73	4.17
+	HYDROGRAPH AT	016	31.	4.08	3.	1.	0.	.03		
	ROUTED TO	LP016	28.	4.17	2.	1.	0.	.03		
+									43.15	4.17

+		LP026	36.	4.08	3.	1.	0.	.04		
+									2702.46	4.08
		ROUTED TO								
+		RLP026	32.	4.33	3.	1.	0.	.04		
+									91.04	4.33
		HYDROGRAPH AT								
+		028	5.	4.00	0.	0.	0.	.00		
		ROUTED TO								
+		LP028	0.	.00	0.	0.	0.	.00		
+									30.19	6.75
		HYDROGRAPH AT								
+		030	2.	4.00	0.	0.	0.	.00		
		ROUTED TO								
+		LP030	0.	.00	0.	0.	0.	.00		
+									15.54	7.00
		HYDROGRAPH AT								
+		029	4.	4.08	0.	0.	0.	.00		
		3 COMBINED AT								
+		C029	4.	4.08	0.	0.	0.	.01		
		ROUTED TO								
+		LPC029	4.	4.08	0.	0.	0.	.01		
+									1.66	4.08
		HYDROGRAPH AT								
-		027	20.	4.08	2.	0.	0.	.02		
		HYDROGRAPH AT								
-		031	10.	4.00	1.	0.	0.	.01		
		3 COMBINED AT								
+		C031	35.	4.08	3.	1.	0.	.05		
		ROUTED TO								
+		RC031	34.	4.08	3.	1.	0.	.05		
+									90.81	4.08
		HYDROGRAPH AT								
-		032	13.	4.08	2.	0.	0.	.01		
		2 COMBINED AT								
+		C032	47.	4.08	5.	1.	0.	.06		
		ROUTED TO								
+		LPC032	45.	4.17	5.	1.	0.	.06		
+									79.94	4.17
		ROUTED TO								
+		RLPC32	43.	4.33	5.	1.	0.	.06		
+									90.91	4.33
		HYDROGRAPH AT								
+		033	137.	4.17	19.	5.	2.	.15		
		3 COMBINED AT								
+		C033	191.	4.25	27.	7.	2.	.25		
		HYDROGRAPH AT								
-		034	82.	4.08	8.	2.	1.	.09		
		ROUTED TO								
+		R034	72.	4.17	8.	2.	1.	.09		
+									91.11	4.17
		HYDROGRAPH AT								
+		035	22.	4.08	2.	0.	0.	.02		
		HYDROGRAPH AT								
+		036	9.	4.00	1.	0.	0.	.01		
		4 COMBINED AT								
+		C036	281.	4.25	37.	9.	3.	.37		
		HYDROGRAPH AT								
+		037	45.	4.00	4.	1.	0.	.04		
		5 COMBINED AT								
+		C037	824.	4.25	116.	29.	10.	1.12		
		ROUTED TO								
+		DET#1	446.	4.58	116.	29.	10.	1.12		
+									2606.42	4.58
		ROUTED TO								
+		DET#2	303.	5.33	115.	29.	10.	1.12		
+									2603.18	5.33

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC007
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 68.00	74.00	74.00
	STORAGE 0.	0.	0.
	OUTFLOW 0.	143.	143.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	72.00	.00	0.	100.	.00	4.25	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC005
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 83.00	88.00	88.00
	STORAGE 0.	0.	0.
	OUTFLOW 0.	62.	62.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	84.76	.00	0.	22.	.00	4.17	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC006
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 80.00	86.00	86.00
	STORAGE 0.	1.	1.
	OUTFLOW 0.	51.	51.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	82.49	.00	0.	24.	.00	4.17	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC009
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 75.00	80.00	80.00
	STORAGE 0.	1.	1.
	OUTFLOW 0.	124.	124.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	77.35	.00	0.	52.	.00	4.17	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC012
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 67.50	73.00	73.00
	STORAGE 0.	0.	0.
	OUTFLOW 0.	133.	133.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	70.03	.00	0.	56.	.00	4.17	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC131
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 67.00	66.00	66.00
	STORAGE 0.	1.	1.
	OUTFLOW 0.	266.	266.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	64.15	.00	0.	159.	.00	4.25	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC013
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 55.80	61.00	61.00
	STORAGE 0.	0.	0.
	OUTFLOW 0.	272.	272.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	59.51	.00	0.	191.	.00	4.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP014
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 64.50	70.00	70.00
	STORAGE 0.	0.	0.
	OUTFLOW 0.	48.	48.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	66.81	.00	0.	20.	.00	4.08	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC22
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 71.00	77.00	77.00
	STORAGE 0.	0.	0.
	OUTFLOW 0.	127.	127.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	73.00	.00	0.	48.	.00	4.17	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP023
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 73.00	77.00	77.00
	STORAGE 0.	1.	1.
	OUTFLOW 0.	113.	113.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	75.17	.00	0.	54.	.00	4.17	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP028
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 28.00	32.00	32.00
	STORAGE 0.	1.	1.
	OUTFLOW 0.	0.	0.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	30.19	.00	0.	0.	.00	.00	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP030
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 11.00	16.00	16.00
	STORAGE 0.	0.	0.
	OUTFLOW 0.	0.	0.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	15.54	.00	0.	0.	.00	.00	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC029
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
1	ELEVATION 1.00	8.00	8.00

STORAGE 0. 0. 0.
 OUTFLOW 0. 38. 38.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 1.66
 MAXIMUM DEPTH OVER DAM .00
 MAXIMUM STORAGE AC-FT 0.
 MAXIMUM OUTFLOW CFS 4.
 DURATION OVER TOP HOURS .00
 TIME OF MAX OUTFLOW HOURS 4.08
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC032
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1
 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 77.00 81.00 81.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 63. 63.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 79.94
 MAXIMUM DEPTH OVER DAM .00
 MAXIMUM STORAGE AC-FT 0.
 MAXIMUM OUTFLOW CFS 45.
 DURATION OVER TOP HOURS .00
 TIME OF MAX OUTFLOW HOURS 4.17
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DETH2
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1
 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 2590.30 2607.00 2607.00
 STORAGE 0. 29. 29.
 OUTFLOW 0. 378. 378.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 2603.16
 MAXIMUM DEPTH OVER DAM .00
 MAXIMUM STORAGE AC-FT 18.
 MAXIMUM OUTFLOW CFS 303.
 DURATION OVER TOP HOURS .00
 TIME OF MAX OUTFLOW HOURS 5.33
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP038
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1
 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 24.60 29.00 29.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 165. 165.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 26.21
 MAXIMUM DEPTH OVER DAM .00
 MAXIMUM STORAGE AC-FT 0.
 MAXIMUM OUTFLOW CFS 43.
 DURATION OVER TOP HOURS .00
 TIME OF MAX OUTFLOW HOURS 4.08
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP040
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1
 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 6.60 11.00 11.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 165. 165.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 8.48
 MAXIMUM DEPTH OVER DAM .00
 MAXIMUM STORAGE AC-FT 0.
 MAXIMUM OUTFLOW CFS 64.
 DURATION OVER TOP HOURS .00
 TIME OF MAX OUTFLOW HOURS 4.08
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP039
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1
 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 17.00 22.00 22.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 135. 135.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 18.38
 MAXIMUM DEPTH OVER DAM .00
 MAXIMUM STORAGE AC-FT 0.
 MAXIMUM OUTFLOW CFS 34.
 DURATION OVER TOP HOURS .00
 TIME OF MAX OUTFLOW HOURS 4.08
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP041
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1
 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 7.00 11.00 11.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 113. 113.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	8.82	.00	0.	43.	.00	4.08	.00

*** NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 01-10.DSS
Pointer Utilization: .27
Number of Records: 48
File Size: 92.1 Kbytes
Percent Inactive: .0

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* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 21AUG14 TIME 15:10:36 *
*****
    
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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
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X X XXXXXXX XXXXX X
X X X X X XX
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X X X X X X
X X X X X X
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION. NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Cavalliere Flats Rezoning
2 ID City of Scottsdale
3 ID Prepared by: Kimley-Horn and Associates, Inc.
4 ID
5 ID Proposed Conditions model
6 ID 100-year, 6-hour Storm event
7 ID
8 ID Based on Troon North Park Model:
9 ID
10 ID PROJECT NAME: TROON NORTH PARK JOB#: 236-08
11 ID PREPARED FOR: CITY OF SCOTTSDALE
12 ID PREPARED BY : ARGUS CONSULTING, P.C.
13 ID
14 ID FILE NAME : INP10CYR.DAT
15 ID STORM EVENT : 100-YR/6-HR
16 ID Unit Hydrograph: Clark
17 ID 01/24/2011
*
*
*
18 IT 5 06JUN99 0000 2000
19 IN 15
20 IO 5
*DIAGRAM
*
*
21 KK 001 BASIN
22 BA 0.112
23 PB 3.136
24 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.075
25 EC 0.087 0.100 0.119 0.151 0.235 0.415 0.762 0.873 0.915 0.944
26 FC 0.956 0.967 0.979 0.989 1.000
27 LG 0.30 0.25 6.00 0.18 3
28 UC 0.261 0.233
29 JA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
30 JA 100
*
*
31 KK 002 BASIN
32 BA 0.0167
33 LG 0.30 0.25 6.00 0.18 7
34 UC 0.201 0.303
35 JA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
36 JA 100
*
*
37 KK LP002 ROUTE INFLOW THRU STORAGE AREA
38 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
39 KM LOW FLOW OUTLET = 30" RCP
40 RS 1 STOR 0.0
41 SA .003 .005 .026 .059
42 SQ 0 5 15 30
43 SE 2722.0 2723.0 2724.0 2725.0
    
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1 HEC-1 INPUT PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
44	ST	2725.0	40	2.7	1.5						
45	KK	C001	COMBINE HYDROGRAPHS FROM SUB C01 AND LPC02								
46	HC	2									
47	KK	LPC001	ROUTE INFLOW THRU STORAGE AREA								
48	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
49	KM		LOW FLOW OUTLET = OPEN SECTION BETWEEN BERM AND WALL								
50	RS	1	STOR 0.0								
51	SA	.001	.004	.013	.104	.150					
52	SQ	0	6.5	23	50	90					
53	SE	2713.0	2714.0	2715.0	2716.0	2717.0					
54	ST	2717.0	45	2.7	1.5						
55	KK	R LPC01	ROUTE FLOW THRU SUB 007								
56	RS	1	FLOW 0								
57	RC	0.070	0.050	0.070	1736	0.025					
58	RX	1000	1001	1034	1047.5	1052.5	1066	1099	1100		
59	RY	100	95.5	94.5	90	90	94.5	95.5	100		
60	KK	C07	BASIN								
61	BA	0.018									
62	LG	0.27	0.25	6.00	0.20	25					
63	UC	0.137	0.167								
64	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	
65	UA	100								97.0	
66	KK	C007	COMBINE HYDROGRAPHS FROM SUB 007 AND R LPC01								
67	HC	2									
68	KK	LPC007	ROUTE INFLOW THRU STORAGE AREA								
69	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
70	KM		LOW FLOW OUTLET = 2-36" RCP								
71	RS	1	STOR 0.0								
72	SA	.001	.002	.004	.025	.040	.060	.100			
73	SQ	0	18	35	70	100	125	143			
74	SE	68.0	69.0	70.0	71.0	72.0	73.0	74.0			
75	ST	74.0	45	2.7	1.5						
76	KK	C03	BASIN								
77	BA	0.018									
78	LG	0.30	0.25	6.00	0.18	2					
79	UC	0.156	0.173								
80	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	
81	UA	100								97.0	

1 HEC-1 INPUT PAGE 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
82	KK	R003	ROUTE FLOW THRU SUB C05								
83	RS	1	FLOW 0								
84	RC	0.070	0.050	0.070	470	0.023					
85	RX	1000	1001	1045.5	1050	1055	1059.5	1099	1100		
86	RY	100	92.5	91.5	90	90	91.5	92.5	100		
87	KK	C05	BASIN								
88	BA	0.007									
89	LG	0.27	0.25	6.00	0.20	23					
90	UC	0.134	0.162								
91	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	
92	UA	100								97.0	
93	KK	C005	COMBINE HYDROGRAPHS FROM SUB 005 AND R003								
94	HC	2									
95	KK	R005	ROUTE FLOW THRU SUB 006								
96	RS	1	FLOW 0								
97	RC	0.070	0.050	0.070	450	0.022					
98	RX	1000	1001	1045.5	1050	1055	1059.5	1099	1100		
99	RY	100	92.5	91.5	90	90	91.5	92.5	100		
100	KK	LPC005	ROUTE INFLOW THRU STORAGE AREA								
101	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
102	KM		LOW FLOW OUTLET = 2-24" RCP								
103	RS	1	STOR 0								
104	SA	.001	.002	.030	.090	.190	.271				

291 KK 018 BASIN
 292 BA 0.050
 293 LG 0.22 0.21 6.40 0.18 33
 294 UC 0.223 0.219
 295 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 296 UA 100
 *

297 KK C018 COMBINE HYDROGRAPHS FROM SUB 018, RC017 AND RLPC13
 298 KM THIS IS TNP INFLOW FROM ALMA SCHOOL RD. CULVERT
 299 HC 3
 *

300 KK 019 BASIN
 301 BA 0.050
 302 LG 0.26 0.25 6.00 0.20 57
 303 UC 0.195 0.172
 304 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 305 UA 100
 *

1 HEC-1 INPUT PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

306 KK 020 BASIN
 307 BA 0.016
 308 LG 0.28 0.25 6.00 0.20 18
 309 UC 0.231 0.427
 310 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 311 UA 100
 *

312 KK C020 COMBINE HYDROGRAPHS FROM SUB 019 AND SUB 020
 313 HC 2
 *

314 KK RC020 NOTE: "n" VALUE AND GEOMETRY MODIFIED FROM PRE-DEVELOPED MODEL
 315 KM TO REFLECT EXISTING DEVELOPMENT.
 316 RS 1 FLOW 0
 317 RC 0.070 0.040 0.070 880 0.018
 318 RX 1000 1001 1034 1047.5 1052.5 1066 1099 1100
 319 RY 100 95.5 94.5 90 90 94.5 95.5 100
 *

320 KK 021 BASIN
 321 BA 0.024
 322 LG 0.26 0.25 6.00 0.20 56
 323 UC 0.184 0.200
 324 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 325 UA 100
 *

326 KK C021 COMBINE HYDROGRAPHS FROM SUB 021 AND RC020
 327 KM THIS IS TNP INFLOW FROM C021
 328 HC 2
 *

329 KK 022 BASIN
 330 BA 0.067
 331 LG 0.29 0.25 6.00 0.19 10
 332 UC 0.244 0.272
 333 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 334 UA 100
 *

335 KK LP022 ROUTE INFLOW THRU STORAGE AREA
 336 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 337 KM LOW FLOW OUTLET = 4-24" RCP
 338 RS 1 STOR 0
 339 SA .001 .004 .009 .011 .013 .060 .140
 340 SQ 0 17 48.5 79.5 97.0 113 127
 341 SE 71.0 72.0 73.0 74.0 75.0 76.0 77.0
 342 ST 77.0 45 2.7 1.5
 *

1 HEC-1 INPUT PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

343 KK 023 BASIN
 344 BA 0.065
 345 LG 0.27 0.25 6.00 0.20 22
 346 UC 0.216 0.198
 347 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 348 UA 100
 *

349 KK LP023 ROUTE INFLOW THRU STORAGE AREA
 350 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 351 KM LOW FLOW OUTLET = 3-30" RCP
 352 RS 1 STOR 0
 353 SA .001 .052 .288 .518 .751

*
 482 KK C032 COMBINE HYDROGRAPHS FROM SUB 032 AND RC031
 483 HC 2
 *
 484 KK LPC032 ROUTE INFLOW THRU STORAGE AREA
 485 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GTS MAPS
 486 KM LOW FLOW OUTLET = 42" RCP
 487 RS 1 STOR 0
 488 SA .001 .007 .070 .136 .270
 489 SQ 0 11 23 46 63
 490 SE 77.0 78.0 79.0 80.0 81.0
 491 SP 81.0 45 2.7 1.5
 *

HEC-1 INPUT

PAGE 14

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

492 KK RLPC32
 493 KM ROUTE FLOW THRU SUB 033
 494 RS 4 FLOW 0
 495 RC 0.070 0.050 0.070 2176 0.021
 496 RX 1000 1031 1039 1045 1055 1061 1099 1100
 497 RY 100 .93 92 90 90 92 93 100
 *

498 KK 033 BASIN
 499 BA 0.147
 500 LG 0.27 0.25 6.00 0.20 36
 501 UC 0.270 0.216
 502 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 503 UA 100
 *

504 KK C033 COMBINE HYDROGRAPHS FROM SUB 033, RLPC32 AND RLP026
 505 HC 3
 *

506 KK 034 BASIN
 507 BA 0.093
 508 LG 0.30 0.25 6.00 0.18 1
 509 UC 0.206 0.169
 510 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 511 UA 100
 *

512 KK R034 ROUTE FLOW THRU SUB 035
 513 RS 1 FLOW 0
 514 RC 0.045 0.040 0.045 2178 0.020
 515 RX 1000 1001 1010 1022 1032 1044 1053 1054
 516 RY 95.5 95.0 94.0 90.0 90.0 94.0 95.0 95.5
 *

517 KK 035 BASIN
 518 BA 0.022
 519 LG 0.30 0.23 6.20 0.17 5
 520 UC 0.144 0.145
 521 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 522 UA 100
 *

523 KK 036 BASIN
 524 BA 0.008
 525 LG 0.29 0.23 6.20 0.18 23
 526 UC 0.110 0.127
 527 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 528 UA 100
 *

HEC-1 INPUT

PAGE 15

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

529 KK C036 COMBINE HYDROGRAPHS FROM SUB 035, SUB 036, C033 AND R034
 530 KM THIS IS TNP INFLOW FROM C036
 531 HC 4
 *

532 KK 037 BASIN
 533 BA 0.041
 534 LG 0.30 0.23 6.20 0.17 1
 535 UC 0.143 0.108
 536 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 537 UA 100
 *

538 KK C037 COMBINE ALL HYDROGRAPHS
 539 KM THIS REPRESENTS TOTAL INFLOW TO BASIN No.1
 540 HC 5
 541 ?W A=TROON B=INFLOW C=FLOW
 *

542	KK	DET#1	ROUTE INFLOW THRU DET BASIN #1 (UPPER BASIN)								
543	KM		LOW FLOW OUTLET = 4-42" RCP								
544	KM		INLET INVERT ELEV = 2598.80								
545	KM		OUTLET INVERT ELEV = 2592.0								
546	KM		SPILLWAY ELEV = 2610.0								
547	KM		WEIR LENGTH = 50'								
548	RS	1	STOR 0.0								
549	SA	0.023	0.11	0.49	1.23	3.50	4.00	4.24	4.57	4.85	5.33
550	SA	5.95	6.67	7.80	8.95						
551	SQ	0	45	103.5	185	264	328.5	382	428.5	470	507
552	SQ	543	575	725	999						
553	SE	2599	2600	2601	2602	2603	2604	2605	2606	2607	2608
554	SF	2609	2610	2611	2612						
555	ZW		A=TROON	B=DET#1	C=FLOW						

556	KK	DET#2	ROUTE INFLOW THRU DET BASIN #2 (LOWER BASIN)								
557	KM		LOW FLOW OUTLET = 1-60" RCP								
558	KM		INLET INVERT ELEV = 2590.36								
559	KM		OUTLET INVERT ELEV = 2589.95								
560	KM		SPILLWAY ELEV = 2607.0								
561	KM		WEIR LENGTH = 35'								
562	RS	1	STOR 0.0								
563	SA	0.023	0.048	0.496	0.876	1.12	1.28	1.41	1.56	1.70	1.82
564	SA	1.93	2.04	2.36	2.49	2.61	2.77	2.93	3.26		
565	SQ	0	11.5	29	47	78	112.5	148	177	204	226
566	SQ	248	266	283	300.5	315	330	345	378		
567	SE	2590.3	2591	2592	2593	2594	2595	2596	2597	2598	2599
568	SE	2600	2601	2602	2603	2604	2605	2606	2607		
569	ST	2607	35	3.0	1.5						
570	ZW		A=TROON	B=DET#2	C=FLOW						

 ***** START CAVALLIERE FLATS MODEL CHANGES *****

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

571	KK	DDET#2	Outlet channel capacity is 348 cfs. Divert overtopping flow.								
572	KM	OVER									
573	DI	0	348	348.1	10000						
574	DQ	0	0	0.1	9652						

 ***** END CAVALLIERE FLATS MODEL CHANGES *****

576	KK	RDET#2	ROUTE FLOW THRU SUB 042								
577	KM		OUTLET CHANNEL TROON NORTH PARK BASIN								
578	RS	1	FLOW 0								
579	RC	0.040	0.035	0.040	594	0.010					
580	RX	990	992	994	1000	1010	1016	1018	1020		
581	RY	14.0	13.5	13.0	10.0	10.0	13.0	13.5	14.0		

 ***** STARTS DOWNSTREAM OFFSITE AREAS MODEL *****

582	KK	038	BASIN								
583	BA	0.047									
584	LG	0.28	0.25	6.00	0.21	26					
585	UC	0.234	0.266								
586	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
587	JA	100									

588	KK	LP038	ROUTE INFLOW THRU STORAGE AREA								
589	KM		SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS								
590	KM		LOW FLOW OUTLET = 3-36" RCP								
591	RS	1	STOR 0								
592	SA	.001	.007	.047	.086	.160					
593	SQ	0	33	82	132	165					
594	SE	24.6	26.0	27.0	28.0	29.0					
595	ST	29.0	45	2.7	1.5						

596	KK	040	BASIN								
597	BA	0.018									
598	LG	0.26	0.25	6.00	0.20	49					

599 UC 0.184 C.202
 600 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 601 UA 100
 *

HEC-1 INPUT

PAGE 17

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

602 KK C040 COMBINE HYDROGRAPHS FROM SUB 040 AND LP038
 603 HC 2
 *

604 KK LF040 ROUTE INFLOW THRU STORAGE AREA
 605 KY SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 606 KY LOW FLOW OUTLET = 3-36" RCP
 607 RS 1 STOR 0
 608 SA .001 .024 .077 .127
 609 SQ 0 82 132 165
 610 SF 06.6 09.0 10.0 11.0
 611 ST 11.0 45 2.7 1.5
 *

612 KK RLP040 ROUTE FLOW THRU SUB 042
 613 RS 1 FLOW 0
 614 RC 0.045 0.040 0.045 927 0.016
 615 RX 990 992 994 1000 1012 1018 1020 1022
 616 RY 13.2 13.1 13.0 10.0 10.0 13.0 13.1 13.2
 *

617 KK 042 BASIN
 618 BA 0.020
 619 LG 0.27 0.25 6.00 0.20 20
 620 UC 0.289 0.433
 621 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 622 UA 100
 *

623 KK C042 COMBINE HYDROGRAPHS FROM RDET#2, SUB 042 AND RLP040
 624 HC 3
 625 ZW A-PVISTA B=C042 C-FLOW
 *

626 KK 039 BASIN
 627 BA 0.035
 628 LG 0.26 0.25 6.00 0.20 39
 629 UC 0.225 0.292
 630 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 631 UA 100
 *

632 KK LP039 ROUTE INFLOW THRU STORAGE AREA
 633 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 634 KM LOW FLOW OUTLET = 3-30" RCP
 635 RS 1 STOR 0
 636 SA .001 .007 .015 .024 .037
 637 SQ 0 49 84 112 135
 638 SF 17.0 19.0 20.0 21.0 22.0
 639 ST 22.0 45 2.7 1.5
 *

HEC-1 INPUT

PAGE 18

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

640 KK 041 BASIN
 641 BA 0.011
 642 LG 0.28 0.25 6.00 0.19 33
 643 UC 0.231 0.353
 644 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 645 UA 100
 *

646 KK C041 COMBINE HYDROGRAPHS FROM SUB 041 AND LP039
 647 HC 2
 *

648 KK LPC41 ROUTE INFLOW THRU STORAGE AREA
 649 KM SURFACE AREA-ELEVATION RELATIONSHIPS FROM COS GIS MAPS
 650 KM LOW FLOW OUTLET = 3-30" RCP
 651 RS 1 STOR 0
 652 SA .001 .020 .039 .072 .104
 653 SQ 0 14 49 84 113
 654 SF 07.0 08.0 09.0 10.0 11.0
 655 ST 11.0 45 2.7 1.5
 *

656 KK RLP041 ROUTE FLOW THRU SUB 041/042
 657 RS 1 FLOW 0
 658 RC 0.045 0.040 0.045 1314 0.017
 659 RX 990 992 994 1000 1008 1014 1016 1018
 660 RY 13.2 13.1 13.0 10.0 10.0 13.0 13.1 13.2
 *

661 KK 043 BASIN
 662 BA 0.008
 663 LG 0.26 0.25 6.00 0.20 29
 664 UC 0.183 0.290
 665 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 666 JA 100
 *

667 KK C043 COMBINE HYDROGRAPHS FROM SUB 043 AND RLP041
 668 HC 2
 669 ZW A=PVISTA B=C043 C=FLOW
 *

670 KK COMB COMBINE HYDROGRAPHS FROM C042 AND C043
 671 KM THIS REPRESENTS COMBINED FLOWS AT PINNACLE VISTA ROAD
 672 HC 2
 673 ZW A=PVISTA B=COMB C=FLOW
 *

* *****
 * *****
 * ***** START CAVALIERE FLATS MODEL CHANGES *****
 * *****
 * *****
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

674 KK RCF1 ROUTE
 675 RS 1 FLOW
 676 RC 0.035 0.035 0.035 690 0.0200 0.00
 677 RX 0.00 30.00 67.50 72.50 77.50 82.50 120.00 150.00
 678 RY 3.00 2.50 2.00 0.00 0.00 2.00 2.50 3.00
 *

679 KK CF1 BASIN
 680 BA 0.016
 681 LG 0.25 0.25 6.00 0.21 62
 682 UC 0.136 0.132
 683 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 684 UA 100
 *

685 KK CCF1A COMBINE
 686 HC 2
 *

687 KK ODET#2
 688 KM Retrieve flow overtopping Trcon North Park Basin outlet channel
 689 DR OVER
 *

690 KK RCF2A ROUTE
 691 RS 1 FLOW
 692 RC 0.035 0.035 0.035 990 0.0200 0.00
 693 RX 0.00 20.00 40.00 45.00 55.00 60.00 80.00 100.00
 694 RY 2.00 1.50 1.00 0.00 0.00 1.00 1.50 2.00
 *

695 KK CF2A BASIN
 696 BA 0.012
 697 LG 0.28 0.25 5.85 0.20 35
 698 UC 0.130 0.116
 699 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 700 UA 100
 *

701 KK OFF1 BASIN
 702 BA 0.307
 703 LG 0.32 0.31 6.00 0.17 9
 704 UC 0.356 0.267
 705 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 706 UA 100
 *

707 KK RCF2B ROUTE
 708 RS 1 FLOW
 709 RC 0.035 0.035 0.035 530 0.0270 0.00
 710 RX 0.00 15.00 30.00 35.00 90.00 95.00 110.00 125.00
 711 RY 2.00 1.50 1.00 0.00 0.00 1.00 1.50 2.00
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

712 KK CF2B BASIN
 713 BA 0.002
 714 LG 0.10 0.40 6.00 0.15 0
 715 UC 0.105 0.167

776 KK OFF5 BASIN
 777 BA 0.009
 778 LG 0.35 0.35 4.65 0.33 2
 779 UC 0.219 0.342
 780 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 781 UA 100
 *

782 KK COFF4 COMBINE
 783 HC 2
 *

1 HEC-1 INPUT PAGE 22

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

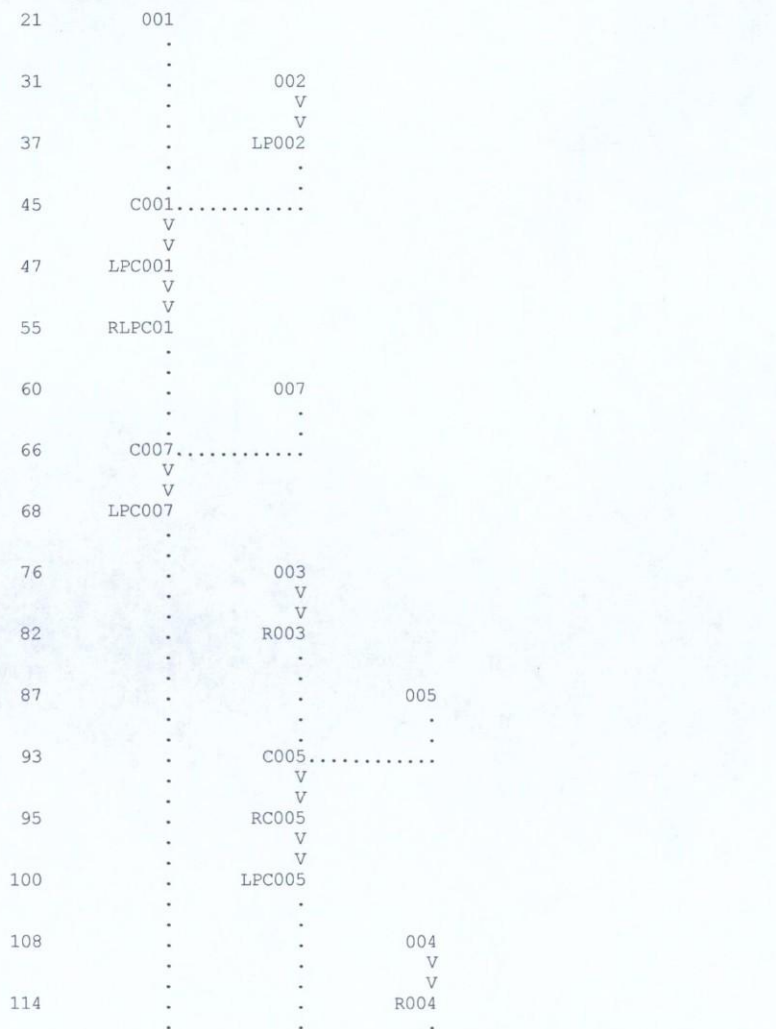
784 KK RCF6 ROUTE
 785 RS 1 FLOW
 786 RC 0.035 0.035 0.035 270 0.0300 0.00
 787 RX 0.00 50.00 100.00 105.00 115.00 120.00 170.00 220.00
 788 RY 1.00 0.75 0.50 0.00 0.00 0.50 0.75 1.00
 *

789 KK CF6 BASIN
 790 BA 0.004
 791 LG 0.25 0.25 5.05 0.33 36
 792 UC 0.167 0.202
 793 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 794 UA 100
 *

795 KK CCF6 COMBINE
 796 HC 2
 *
 797 ZZ

1 SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW



119	.	.	.	006	.
125	.	.	C006.....	.	.
	.	.	V	.	.
127	.	.	V	.	.
	.	.	LPC006	.	.
135	.	.	.	009	.
141	.	.	C009.....	.	.
	.	.	V	.	.
143	.	.	V	.	.
	.	.	LPC009	.	.
151	.	.	.	012	.
157	.	.	C012.....	.	.
	.	.	V	.	.
159	.	.	V	.	.
	.	.	LPC012	.	.
167	.	.	.	008	.
173	010
179	.	.	C010.....	.	.
	.	.	V	.	.
181	.	.	V	.	.
	.	.	RC010	.	.
186	011
192	.	.	C011.....	.	.
194	.	.	C013.....	.	.
	.	.	V	.	.
196	.	.	V	.	.
	.	.	LPC131	.	.
204	.	.	.	013	.
210	.	.	C013.....	.	.
	.	.	V	.	.
212	.	.	V	.	.
	.	.	LPC013	.	.
	.	.	V	.	.
220	.	.	V	.	.
	.	.	RLPC13	.	.
225	.	.	.	014	.
	.	.	V	.	.
231	.	.	V	.	.
	.	.	LP014	.	.
	.	.	V	.	.
239	.	.	V	.	.
	.	.	RLP014	.	.
244	.	.	.	016	.
	.	.	V	.	.
250	.	.	V	.	.
	.	.	LP016	.	.
	.	.	V	.	.
257	.	.	V	.	.
	.	.	RLP016	.	.
262	015
268	017
280
274
	DV017
	DR017
283	.	.	C017.....	.	.

285	V V RC017			
291			018	
297	C018			
300		019		
306			020	
312		C020		
314	V V RC020			
320			021	
326		C021		
329			022	
335		V V LP022		
343			023	
349			V LP023	
357		C023		
359		V V RLPC23		
364			024	
370		C024		
372		V V LPC024		
379		V V RLPC24		
385			025	
391		C025		
394			026	
400			V V LP026	
407			V V RLP026	
412				028
418			V V LP028	
426				030
432			V V LP030	
440				029
446				
448			C029	
			V V LPC029	

456	027	.
462	031
468	CC31
	V	.	.
	V	.	.
470	RC031	.	.
476	032
482	C032
	V	.	.
	V	.	.
484	LPC032	.	.
	V	.	.
	V	.	.
492	RLPC32	.	.
498	033
504	C033
506	034
	V	.	.
	V	.	.
512	R034	.	.
517	035
523	036
529	C036
532	037
538	C037
	V	.	.
	V	.	.
542	DET#1	.	.
	V	.	.
	V	.	.
556	DET#2	.	.

573	----->	OVER	.
571	DDET#2	.	.
	V	.	.
	V	.	.
576	RDET#2	.	.
582	038
	V	.	.
	V	.	.
588	LP038	.	.
596	040
602	C040
	V	.	.
	V	.	.
604	LP040	.	.
	V	.	.
	V	.	.
612	RLPC40	.	.
617	042
623	C042
626	039
	V	.	.
	V	.	.

```
632      .      LP039
        .
640      .      .      041
        .      .
646      .      C041.....
        .      V
        .      V
648      .      LP041
        .      V
        .      V
656      .      RLP041
        .
661      .      .      043
        .      .
667      .      C043.....
        .
670      .      COMB.....
        .      V
        .      V
674      .      RCF1
        .
679      .      .      CF1
        .      .
685      .      CCF1A.....
        .
689      .      .      <----- OVER
687      .      ODET#2
        .      V
        .      V
690      .      RCF2A
        .
695      .      .      CF2A
        .      .
701      .      .      .      OFF1
        .      .      .      V
707      .      .      .      RCF2B
        .      .      .      V
712      .      .      .      .      CF2B
        .      .      .      .
718      .      .      .      CCF2B.....
        .      .      .
720      .      CCF2A.....
        .
722      .      CCF1B.....
        .      V
        .      V
724      .      RCF3
        .
729      .      .      CF3
        .      .
735      .      CCF3.....
        .
737      .      .      OFF2
        .      .      V
        .      .      V
743      .      .      RCF4A
        .      .
748      .      .      .      CF4A
        .      .      .
754      .      .      .      .      OFF3
        .      .      .      .
760      .      .      COFF3.....
        .      .
762      .      .      .      CF4B
        .      .      .
768      .      .      CCF4B.....
        .      .
770      .      .      .      OFF4
        .      .
```

```

776 . . . . . OFF5
782 . . . . . COFF4.....
      . . . . . V
      . . . . . V
784 . . . . . RCF6
789 . . . . . CF6
795 . . . . . CCF6.....
    
```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 21AUG14 TIME 15:10:36 *
*****
    
```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

Cavalliere Flats Rezoning
 City of Scottsdale
 Prepared by: Kimley-Horn and Associates, Inc.

Proposed Conditions model
 100-year, 6-hour Storm event

Based on Troon North Park Model:

PROJECT NAME: TROON NORTH PARK
 PREPARED FOR: CITY OF SCOTTSDALE
 PREPARED BY : ARGUS CONSULTING, P.C.

JOB#: 236-08

FILE NAME : TNE100YR.DAT
 STORM EVENT : 100-YR/6-HR
 Unit Hydrograph: Clark
 01/24/2011

```

20 10 OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
      NMIN      5 MINUTES IN COMPUTATION INTERVAL
      IDATE     6JUN99 STARTING DATE
      ITIME     0000 STARTING TIME
      NQ       2000 NUMBER OF HYDROGRAPH ORDINATES
      NDDATE    12JUN99 ENDING DATE
      NDTIME    2235 ENDING TIME
      ICENT     19 CENTURY MARK

      COMPUTATION INTERVAL .08 HOURS
      TOTAL TIME BASE 166.58 HOURS
    
```

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILLS
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

```

-----DSS---ZOPEN: Existing file opened, file: 01-100.DSS
Unit: 71; DSS Version: 6-JG
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/INFLOW/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#1/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/06JUN1999/5MIN//
    
```

```

-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 14: /TROON/DET#2/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C042/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C042/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C042/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C042/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C042/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C042/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C042/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C042/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C043/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C043/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C043/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C043/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C043/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C043/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C043/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/C043/FLOW/12JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/COMB/FLOW/05JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/COMB/FLOW/06JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/COMB/FLOW/07JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/COMB/FLOW/08JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/COMB/FLOW/09JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/COMB/FLOW/10JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/COMB/FLOW/11JUN1999/5MIN//
-----DSS---ZWRITE Unit 71; Vers. 6: /FVISTA/COMB/FLOW/12JUN1999/5MIN//
    
```

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+									
+	HYDROGRAPH AT								
		001	185.	4.08	20.	5.	2.	.11	
+	HYDROGRAPH AT								
		002	25.	4.08	3.	1.	0.	.02	
+	ROUTED TO								
+		LP002	24.	4.08	3.	1.	0.	.02	2724.63 4.08
+	2 COMBINED AT								
		C001	209.	4.08	23.	6.	2.	.13	
+	ROUTED TO								
+		LPC001	206.	4.08	23.	6.	2.	.13	2717.79 4.08
+	ROUTED TO								
+		RLPC01	200.	4.17	23.	6.	2.	.13	92.53 4.17
+	HYDROGRAPH AT								
		007	36.	4.00	4.	1.	0.	.02	
+	2 COMBINED AT								
		C007	226.	4.17	26.	7.	2.	.15	
+	ROUTED TO								
+		LPC007	222.	4.08	26.	7.	2.	.15	74.67 4.08
+	HYDROGRAPH AT								
		003	34.	4.00	3.	1.	0.	.02	
+	ROUTED TO								
+		R003	34.	4.08	3.	1.	0.	.02	91.08 4.08
+	HYDROGRAPH AT								
		005	14.	4.00	1.	0.	0.	.01	
+	2 COMBINED AT								
		C005	47.	4.08	5.	1.	0.	.02	
+	ROUTED TO								
+		RC005	47.	4.08	5.	1.	0.	.02	91.27 4.08
+	ROUTED TO								
+		LPC005	44.	4.17	5.	1.	0.	.02	86.16 4.17
+	HYDROGRAPH AT								
		004	23.	4.00	2.	1.	0.	.01	

+	ROUTED TO	R004	23.	4.08	2.	1.	0.	.01	90.84	4.08
+	HYDROGRAPH AT	006	31.	4.00	3.	1.	0.	.01		
+	2 COMBINED AT	C006	52.	4.08	5.	1.	0.	.03		
+	ROUTED TO	LPC006	43.	4.17	5.	1.	0.	.03	84.78	4.17
+	HYDROGRAPH AT	009	14.	4.00	1.	0.	0.	.01		
+	3 COMBINED AT	C009	99.	4.08	11.	3.	1.	.06		
+	ROUTED TO	LPC009	96.	4.17	11.	3.	1.	.06	78.71	4.17
+	HYDROGRAPH AT	012	8.	4.00	1.	0.	0.	.00		
+	2 COMBINED AT	C012	102.	4.17	12.	3.	1.	.06		
+	ROUTED TO	LPC012	100.	4.25	12.	3.	1.	.06	71.49	4.25
+	HYDROGRAPH AT	008	5.	4.08	1.	0.	0.	.00		
+	HYDROGRAPH AT	010	6.	4.00	1.	0.	0.	.00		
+	2 COMBINED AT	C010	11.	4.00	1.	0.	0.	.01		
+	ROUTED TO	RC010	10.	4.08	1.	0.	0.	.01	90.36	4.08
+	HYDROGRAPH AT	011	10.	4.00	1.	0.	0.	.00		
+	2 COMBINED AT	C011	20.	4.08	2.	1.	0.	.01		
+	3 COMBINED AT	C013I	338.	4.17	41.	10.	3.	.22		
+	ROUTED TO	LPC13I	338.	4.17	41.	10.	3.	.22	66.51	4.17
+	HYDROGRAPH AT	013	76.	4.08	9.	2.	1.	.04		
+	2 COMBINED AT	C013	402.	4.17	49.	12.	4.	.26		
+	ROUTED TO	LPC013	392.	4.25	50.	12.	4.	.26	61.76	4.25
+	ROUTED TO	RLPC13	378.	4.25	50.	12.	4.	.26	92.68	4.25
+	HYDROGRAPH AT	014	38.	4.00	4.	1.	0.	.02		
+	ROUTED TO	LP014	37.	4.08	4.	1.	0.	.02	68.40	4.08
+	ROUTED TO	RLP014	33.	4.08	4.	1.	0.	.02	91.08	4.08
+	HYDROGRAPH AT	016	62.	4.00	6.	1.	0.	.03		
+	ROUTED TO	LP016	59.	4.08	5.	1.	0.	.03	43.32	4.08

+		LP026	74.	4.08	7.	2.	1.	.04	2702.94	4.08
+										
	ROUTED TO									
+		RLP026	68.	4.17	7.	2.	1.	.04	91.50	4.17
+										
	HYDROGRAPH AT									
+		028	9.	4.00	1.	0.	0.	.00		
+	ROUTED TO									
+		LP028	0.	.00	0.	0.	0.	.00	31.25	6.58
+										
	HYDROGRAPH AT									
+		030	4.	4.00	0.	0.	0.	.00		
+	ROUTED TO									
+		LP030	3.	4.17	0.	0.	0.	.00	16.08	4.17
+										
	HYDROGRAPH AT									
+		029	9.	4.00	1.	0.	0.	.00		
+	3 COMBINED AT									
+		C029	10.	4.17	1.	0.	0.	.01		
+	ROUTED TO									
+		LPC029	10.	4.17	1.	0.	0.	.01	2.50	4.17
+										
	HYDROGRAPH AT									
+		027	43.	4.00	4.	1.	0.	.02		
+	HYDROGRAPH AT									
+		031	21.	4.00	2.	0.	0.	.01		
+	3 COMBINED AT									
+		C031	73.	4.00	7.	2.	1.	.05		
+	ROUTED TO									
+		RC031	73.	4.08	7.	2.	1.	.05	91.27	4.08
+										
	HYDROGRAPH AT									
+		032	26.	4.00	3.	1.	0.	.01		
+	2 COMBINED AT									
+		C032	99.	4.08	10.	2.	1.	.06		
+	ROUTED TO									
+		LPC032	95.	4.08	10.	2.	1.	.06	81.36	4.08
+										
	ROUTED TO									
+		RLPC32	91.	4.25	10.	2.	1.	.06	91.40	4.25
+										
	HYDROGRAPH AT									
+		033	267.	4.08	33.	8.	3.	.15		
+	3 COMBINED AT									
+		C033	400.	4.17	50.	12.	4.	.25		
+	HYDROGRAPH AT									
+		034	170.	4.08	16.	4.	1.	.09		
+	ROUTED TO									
+		R034	160.	4.08	16.	4.	1.	.09	91.72	4.08
+										
	HYDROGRAPH AT									
+		035	45.	4.00	4.	1.	0.	.02		
+	HYDROGRAPH AT									
+		036	18.	4.00	2.	0.	0.	.01		
+	4 COMBINED AT									
+		C036	602.	4.08	71.	18.	6.	.37		
+	HYDROGRAPH AT									
+		037	90.	4.00	7.	2.	1.	.04		
+	5 COMBINED AT									
+		C037	1658.	4.17	219.	55.	18.	1.12		
+	ROUTED TO									
+		DET#1	810.	4.50	219.	55.	18.	1.12	2611.31	4.50
+										
	ROUTED TO									
+		DET#2	538.	5.17	216.	55.	18.	1.12	2608.11	5.17

+	DIVERSION TO	OVER	190.	4.67	28.	7.	2.	1.12		
+	HYDROGRAPH AT	DDET#2	348.	4.67	187.	48.	16.	1.12		
+	ROUTED TO	RDET#2	349.	4.75	187.	48.	16.	1.12	13.07	4.75
+	HYDROGRAPH AT	038	77.	4.08	10.	2.	1.	.05		
+	ROUTED TO	LP038	76.	4.08	10.	2.	1.	.05	26.89	4.08
+	HYDROGRAPH AT	040	35.	4.08	4.	1.	0.	.02		
+	2 COMBINED AT	C040	111.	4.08	14.	4.	1.	.06		
+	ROUTED TO	LP040	110.	4.08	14.	4.	1.	.06	9.56	4.08
+	ROUTED TO	RLP040	107.	4.17	14.	4.	1.	.06	11.41	4.17
+	HYDROGRAPH AT	042	25.	4.17	4.	1.	0.	.02		
+	3 COMBINED AT	C042	386.	4.67	205.	52.	17.	1.20		
+	HYDROGRAPH AT	039	58.	4.08	8.	2.	1.	.04		
+	ROUTED TO	LP039	58.	4.08	8.	2.	1.	.04	19.26	4.08
+	HYDROGRAPH AT	041	16.	4.08	2.	1.	0.	.01		
+	2 COMBINED AT	C041	74.	4.08	11.	3.	1.	.05		
+	ROUTED TO	LP041	73.	4.08	11.	3.	1.	.05	9.68	4.08
+	ROUTED TO	RLP041	71.	4.17	11.	3.	1.	.05	11.35	4.17
+	HYDROGRAPH AT	043	13.	4.08	2.	0.	0.	.01		
+	2 COMBINED AT	C043	83.	4.17	12.	3.	1.	.05		
+	2 COMBINED AT	COMB	443.	4.25	217.	55.	18.	1.25		
+	ROUTED TO	RCF1	443.	4.25	217.	55.	18.	1.25	2.76	4.25
+	HYDROGRAPH AT	CF1	37.	4.00	4.	1.	0.	.02		
+	2 COMBINED AT	CCF1A	461.	4.25	221.	57.	19.	1.27		
+	HYDROGRAPH AT	ODET#2	190.	5.17	28.	7.	2.	.00		
+	ROUTED TO	RCF2A	188.	5.17	28.	7.	2.	.00	1.47	5.17
+	HYDROGRAPH AT	CF2A	27.	4.00	3.	1.	0.	.01		
+	HYDROGRAPH AT	OFF1	466.	4.25	55.	14.	5.	.31		
+	ROUTED TO	RCF2B	470.	4.25	55.	14.	5.	.31		

1 1.00 2717.79 .79 0. 206. .58 4.08 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC007
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM			
	ELEVATION	68.00	74.00	74.00			
	STORAGE	0.	0.	0.			
	OUTFLOW	0.	143.	143.			
RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS

1 1.00 74.67 .67 0. 222. .50 4.08 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC005
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM			
	ELEVATION	83.00	88.00	88.00			
	STORAGE	0.	0.	0.			
	OUTFLOW	0.	62.	62.			
RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS

1 1.00 86.16 .00 0. 44. .00 4.17 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC006
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM			
	ELEVATION	80.00	86.00	86.00			
	STORAGE	0.	1.	1.			
	OUTFLOW	0.	51.	51.			
RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS

1 1.00 84.78 .00 0. 43. .00 4.17 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC009
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM			
	ELEVATION	75.00	80.00	80.00			
	STORAGE	0.	1.	1.			
	OUTFLOW	0.	124.	124.			
RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS

1 1.00 78.71 .00 0. 96. .00 4.17 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC012
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM			
	ELEVATION	67.50	73.00	73.00			
	STORAGE	0.	0.	0.			
	OUTFLOW	0.	133.	133.			
RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS

1 1.00 71.49 .00 0. 100. .00 4.25 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC131
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM			
	ELEVATION	61.00	66.00	66.00			
	STORAGE	0.	1.	1.			
	OUTFLOW	0.	266.	266.			
RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS

1 1.00 66.51 .51 1. 338. .33 4.17 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC013
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN	INITIAL VALUE	SPILLWAY CREST		TOP OF DAM				
1	ELEVATION	55.80	61.00	61.00				
	STORAGE	0.	0.	0.				
	OUTFLOW	0.	272.	272.				
	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	61.76	.76	1.	392.	.42	4.25	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP014 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN	INITIAL VALUE	SPILLWAY CREST		TOP OF DAM				
1	ELEVATION	64.50	70.00	70.00				
	STORAGE	0.	0.	0.				
	OUTFLOW	0.	48.	48.				
	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	68.40	.00	0.	37.	.00	4.08	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP022 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN	INITIAL VALUE	SPILLWAY CREST		TOP OF DAM				
1	ELEVATION	71.00	77.00	77.00				
	STORAGE	0.	0.	0.				
	OUTFLOW	0.	127.	127.				
	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	75.45	.00	0.	104.	.00	4.08	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP023 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN	INITIAL VALUE	SPILLWAY CREST		TOP OF DAM				
1	ELEVATION	73.00	77.00	77.00				
	STORAGE	0.	1.	1.				
	OUTFLOW	0.	113.	113.				
	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	76.45	.00	1.	97.	.00	4.17	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP028 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN	INITIAL VALUE	SPILLWAY CREST		TOP OF DAM				
1	ELEVATION	28.00	32.00	32.00				
	STORAGE	0.	1.	1.				
	OUTFLOW	0.	0.	0.				
	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	31.25	.00	0.	0.	.00	.00	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP030 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN	INITIAL VALUE	SPILLWAY CREST		TOP OF DAM				
1	ELEVATION	11.00	16.00	16.00				
	STORAGE	0.	0.	0.				
	OUTFLOW	0.	0.	0.				
	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	16.08	.08	0.	3.	162.50	4.17	.00
SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC029 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)								

PLAN	INITIAL VALUE	SPILLWAY CREST		TOP OF DAM			
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ELEVATION 1.00 8.00 8.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 38. 38.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 2.50
 MAXIMUM DEPTH OVER DAM .00
 MAXIMUM STORAGE AC-FT 0.
 MAXIMUM OUTFLOW CFS 10.
 DURATION OVER TOP HOURS .00
 TIME OF MAX OUTFLOW HOURS 4.17
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LPC032
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 77.00 81.00 81.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 63. 63.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 81.36
 MAXIMUM DEPTH OVER DAM .36
 MAXIMUM STORAGE AC-FT 0.
 MAXIMUM OUTFLOW CFS 95.
 DURATION OVER TOP HOURS .25
 TIME OF MAX OUTFLOW HOURS 4.08
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DET#2
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 2590.20 2607.00 2607.00
 STORAGE 0. 29. 29.
 OUTFLOW 0. 378. 378.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 2608.11
 MAXIMUM DEPTH OVER DAM 1.11
 MAXIMUM STORAGE AC-FT 33.
 MAXIMUM OUTFLOW CFS 538.
 DURATION OVER TOP HOURS 1.33
 TIME OF MAX OUTFLOW HOURS 5.17
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP038
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 24.60 29.00 29.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 165. 165.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 26.89
 MAXIMUM DEPTH OVER DAM .00
 MAXIMUM STORAGE AC-FT 0.
 MAXIMUM OUTFLOW CFS 76.
 DURATION OVER TOP HOURS .00
 TIME OF MAX OUTFLOW HOURS 4.08
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP040
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 6.60 11.00 11.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 165. 165.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 9.56
 MAXIMUM DEPTH OVER DAM .00
 MAXIMUM STORAGE AC-FT 0.
 MAXIMUM OUTFLOW CFS 110.
 DURATION OVER TOP HOURS .00
 TIME OF MAX OUTFLOW HOURS 4.08
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP039
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 17.00 22.00 22.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 135. 135.

RATIO OF PMF 1.00
 MAXIMUM RESERVOIR W.S.ELEV 19.26
 MAXIMUM DEPTH OVER DAM .00
 MAXIMUM STORAGE AC-FT 0.
 MAXIMUM OUTFLOW CFS 58.
 DURATION OVER TOP HOURS .00
 TIME OF MAX OUTFLOW HOURS 4.08
 TIME OF FAILURE HOURS .00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION LP041
 (PEAKS SHOWN ARE FOR INTERNAL TIME STEP USED DURING BREACH FORMATION)

PLAN 1 INITIAL VALUE SPILLWAY CREST TOP OF DAM
 ELEVATION 7.00 11.00 11.00
 STORAGE 0. 0. 0.
 OUTFLOW 0. 113. 113.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	9.68	.00	0.	73.	.00	4.08	.00

*** NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 01-100.DSS
Pointer Utilization: .27
Number of Records: 48
File Size: 92.1 Kbytes
Percent Inactive: .0

Appendix D-HYDRUALICS

Existing Conditions

Proposed Conditions

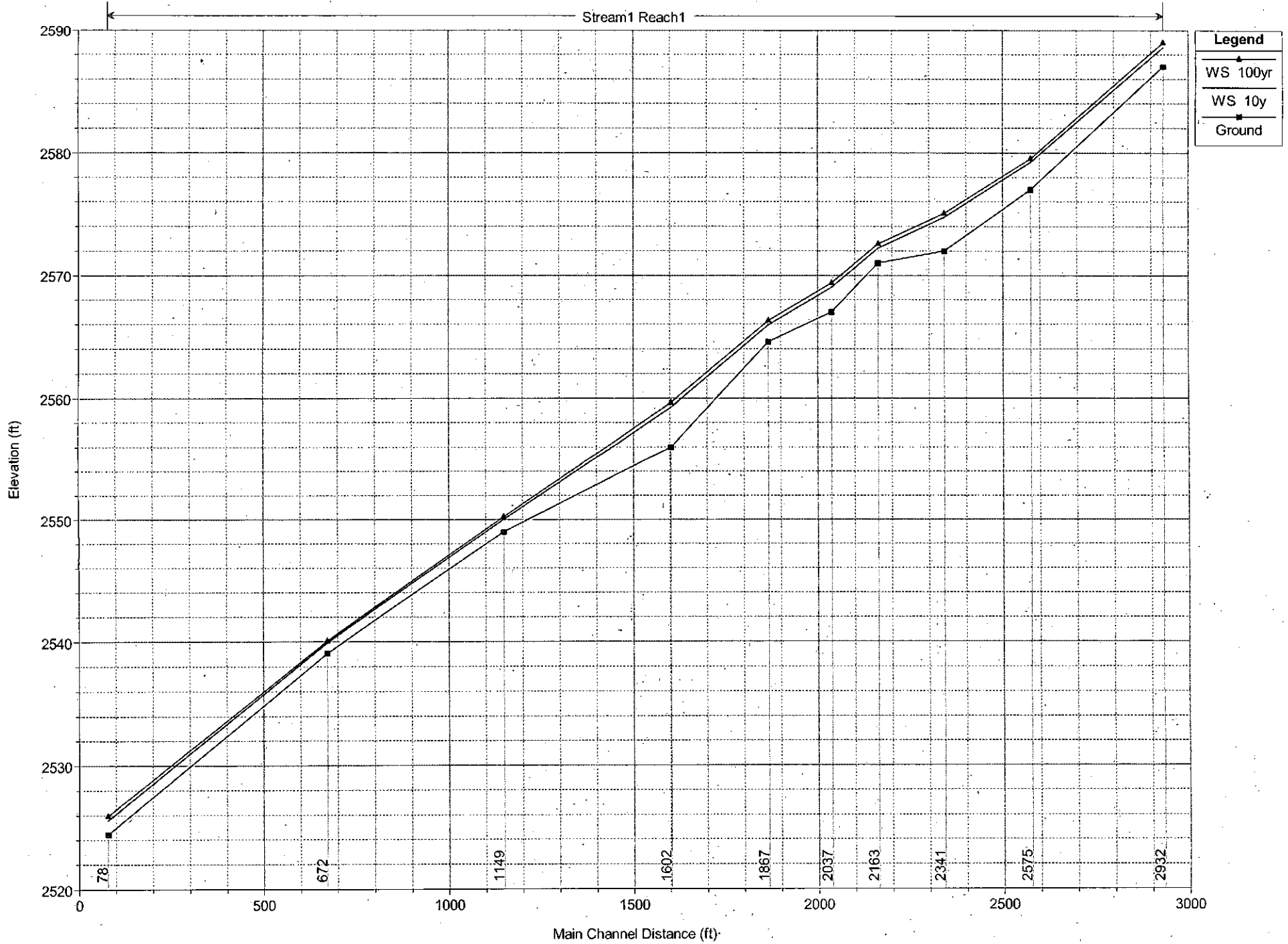
HY-8 Output

Erosion Setback Analysis

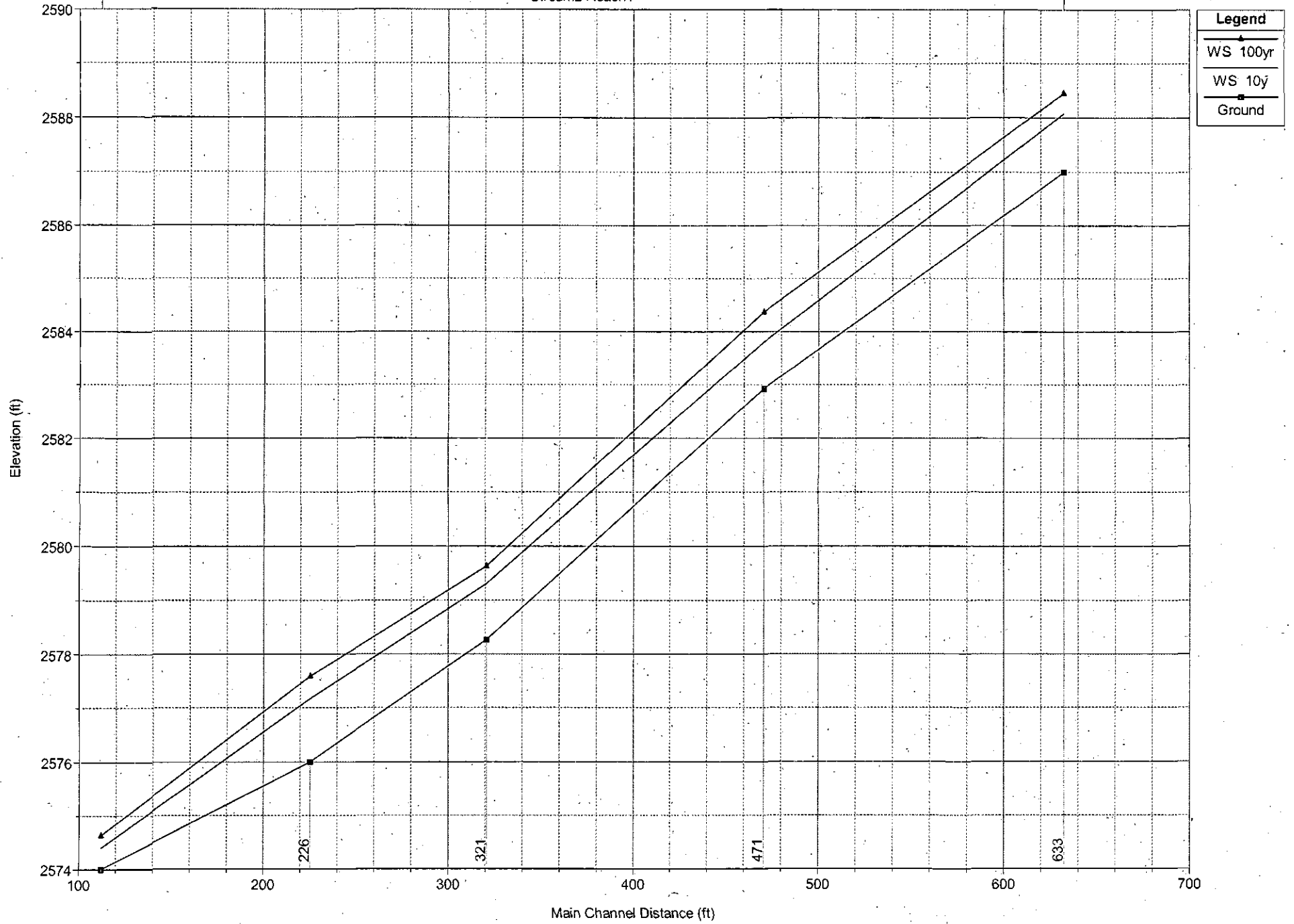


HEC-RAS Plan: GW_EX

River	Reach	River Sta.	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit.W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq/ft)	Top Width (ft)	Froude # Chl
Stream2	Reach1	633	10y	217.00	2587.00	2588.08	2588.08	2588.34	0.023202	4.16	52.15	100.64	1.02
Stream2	Reach1	633	100yr	484.00	2587.00	2588.45	2588.45	2588.89	0.019264	5.32	91.05	105.75	1.01
Stream2	Reach1	471	10y	217.00	2582.93	2583.81	2583.81	2584.18	0.020306	4.87	44.53	61.37	1.01
Stream2	Reach1	471	100yr	484.00	2582.93	2584.37	2584.37	2584.79	0.019404	5.18	93.35	113.41	1.01
Stream2	Reach1	321	10y	217.00	2578.27	2579.31	2579.31	2579.53	0.022280	3.82	56.74	120.97	0.98
Stream2	Reach1	321	100yr	484.00	2578.27	2579.63	2579.63	2580.01	0.020433	4.93	98.14	133.86	1.02
Stream2	Reach1	226	10y	217.00	2576.00	2577.17	2577.17	2577.43	0.021548	4.07	53.26	183.63	0.99
Stream2	Reach1	226	100yr	484.00	2576.00	2577.59	2577.59	2577.88	0.021377	4.34	111.54	307.26	1.00
Stream2	Reach1	112	10y	217.00	2574.00	2574.41	2574.37	2574.55	0.020022	3.08	70.44	191.66	0.90
Stream2	Reach1	112	100yr	484.00	2574.00	2574.64	2574.62	2574.91	0.020018	4.18	115.71	198.73	0.97
Stream1	Reach1	2932	10y	314.00	2587.00	2588.56	2588.56	2589.23	0.016859	6.54	48.00	36.60	1.01
Stream1	Reach1	2932	100yr	453.00	2587.00	2588.93	2588.93	2589.77	0.016064	7.34	61.75	38.01	1.01
Stream1	Reach1	2575	10y	314.00	2577.00	2579.24	2579.24	2579.68	0.018891	5.35	58.67	66.02	1.00
Stream1	Reach1	2575	100yr	453.00	2577.00	2579.50	2579.50	2580.03	0.019183	5.85	77.38	77.08	1.03
Stream1	Reach1	2341	10y	314.00	2572.00	2574.76		2574.94	0.007334	3.36	93.42	104.17	0.63
Stream1	Reach1	2341	100yr	453.00	2572.00	2575.06	2574.69	2575.26	0.006463	3.58	126.65	117.09	0.61
Stream1	Reach1	2163	10y	531.00	2571.00	2572.22	2572.22	2572.58	0.020492	4.86	109.20	152.48	1.01
Stream1	Reach1	2163	100yr	937.00	2571.00	2572.56	2572.56	2573.07	0.018050	5.75	162.95	160.85	1.01
Stream1	Reach1	2037	10y	531.00	2567.00	2569.02	2568.92	2569.38	0.015676	4.78	111.13	130.12	0.91
Stream1	Reach1	2037	100yr	937.00	2567.00	2569.37	2569.36	2569.90	0.017601	5.86	159.82	150.23	1.00
Stream1	Reach1	1867	10y	531.00	2564.58	2565.93	2565.93	2566.30	0.021062	4.86	109.15	155.54	1.02
Stream1	Reach1	1867	100yr	937.00	2564.58	2566.28	2566.28	2566.74	0.019337	5.44	172.40	195.11	1.02
Stream1	Reach1	1602	10y	531.00	2556.00	2559.26	2559.26	2559.65	0.021043	5.01	105.99	143.44	1.03
Stream1	Reach1	1602	100yr	937.00	2556.00	2559.63	2559.63	2560.14	0.018470	5.71	164.14	165.77	1.01
Stream1	Reach1	1149	10y	531.00	2549.00	2550.02	2549.97	2550.27	0.017690	4.03	131.89	218.97	0.91
Stream1	Reach1	1149	100yr	937.00	2549.00	2550.24	2550.24	2550.65	0.019805	5.14	182.45	228.86	1.01
Stream1	Reach1	672	10y	531.00	2539.07	2539.89	2539.89	2540.07	0.026181	3.41	155.67	444.85	1.02
Stream1	Reach1	672	100yr	937.00	2539.07	2540.05	2540.05	2540.31	0.023619	4.10	228.79	460.06	1.02
Stream1	Reach1	78	10y	531.00	2524.44	2525.56	2525.56	2525.95	0.019800	5.00	106.18	138.56	1.01
Stream1	Reach1	78	100yr	937.00	2524.44	2525.93	2525.93	2526.47	0.017859	5.89	159.06	150.25	1.01

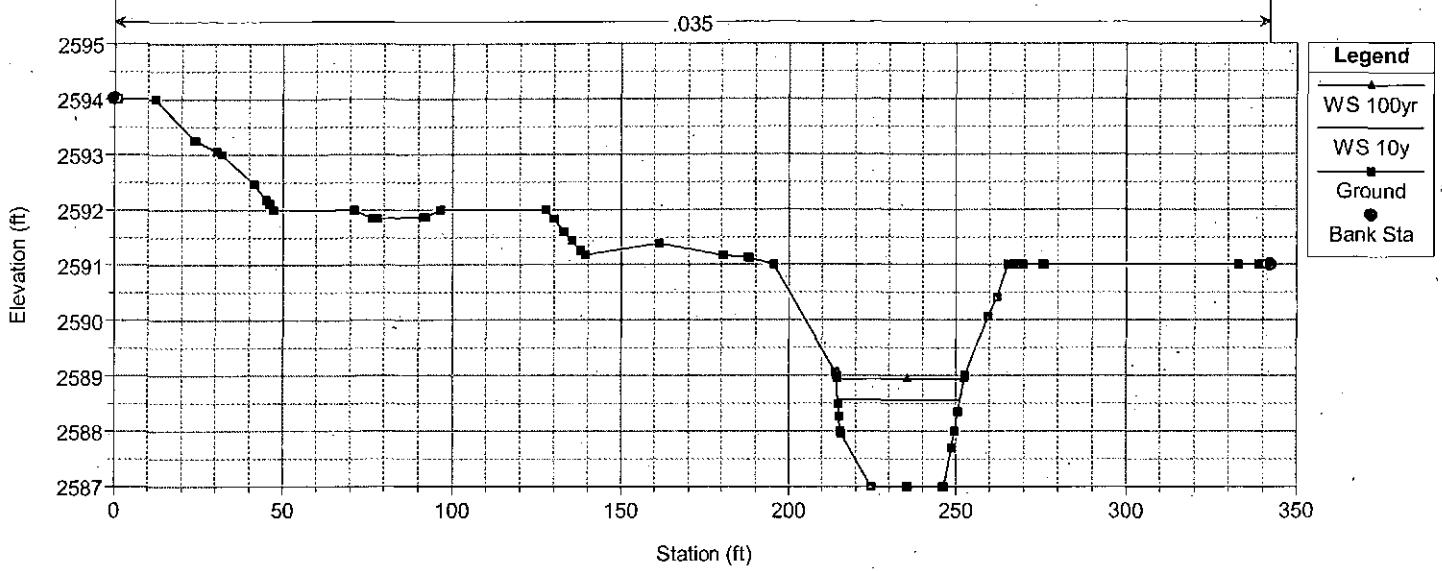


Stream2 Reach1



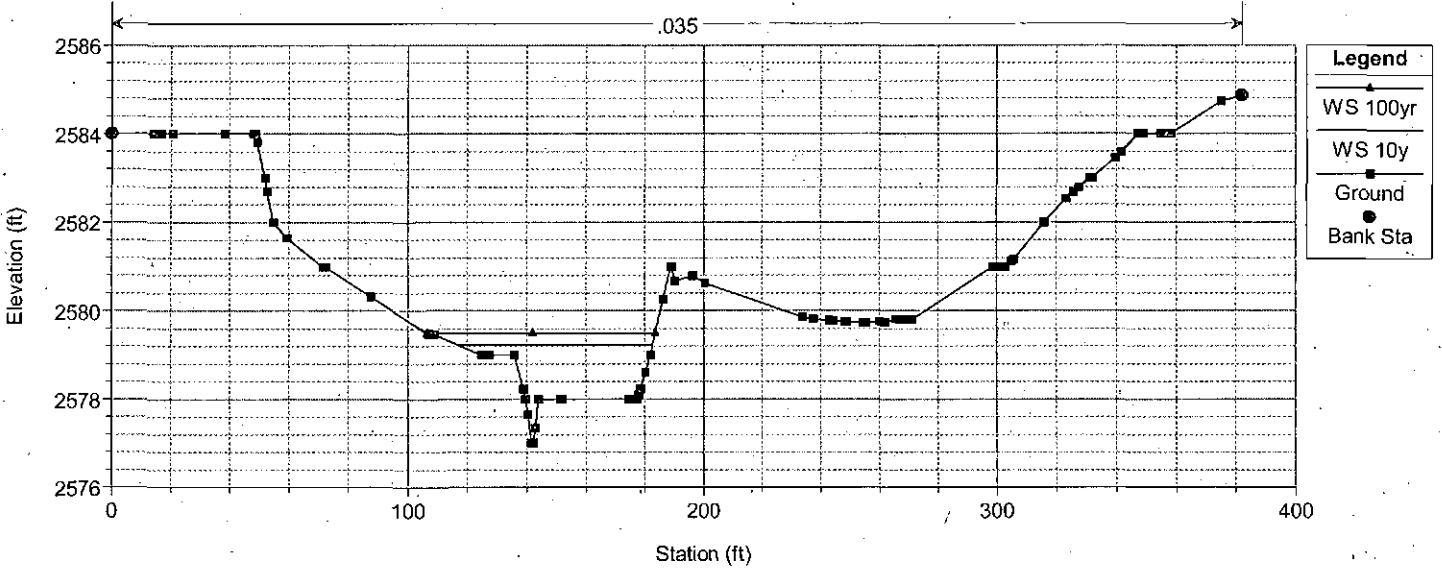
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream1 Reach = Reach1 RS = 2932



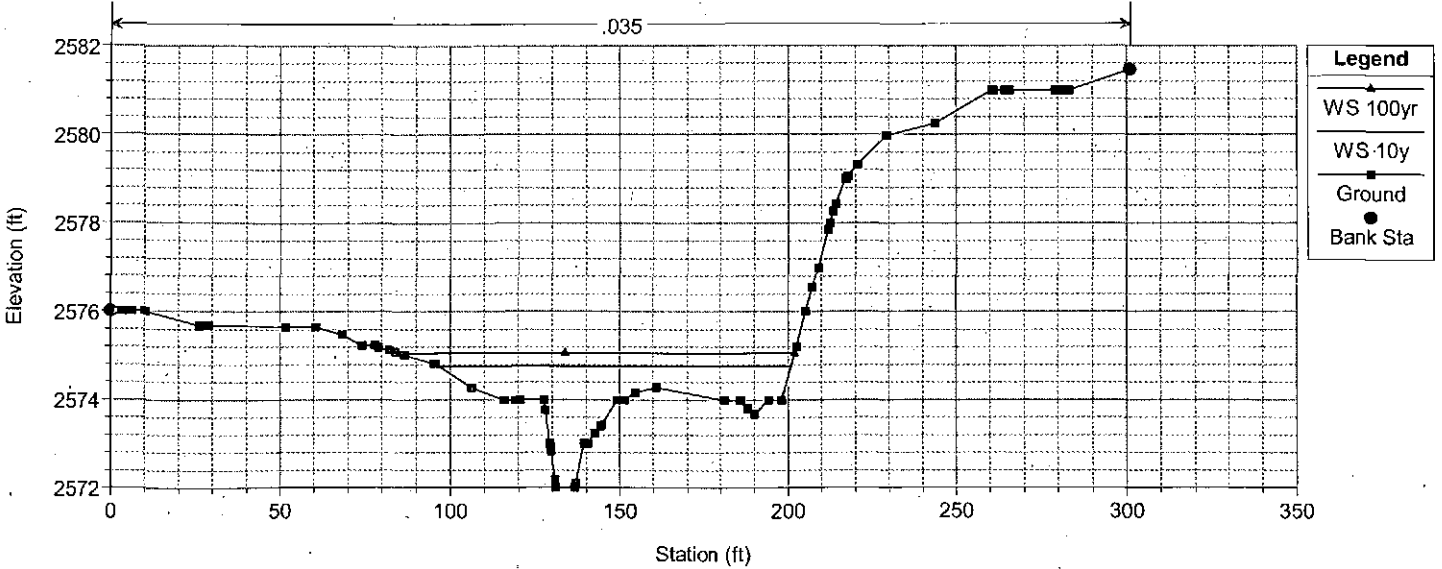
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream1 Reach = Reach1 RS = 2575



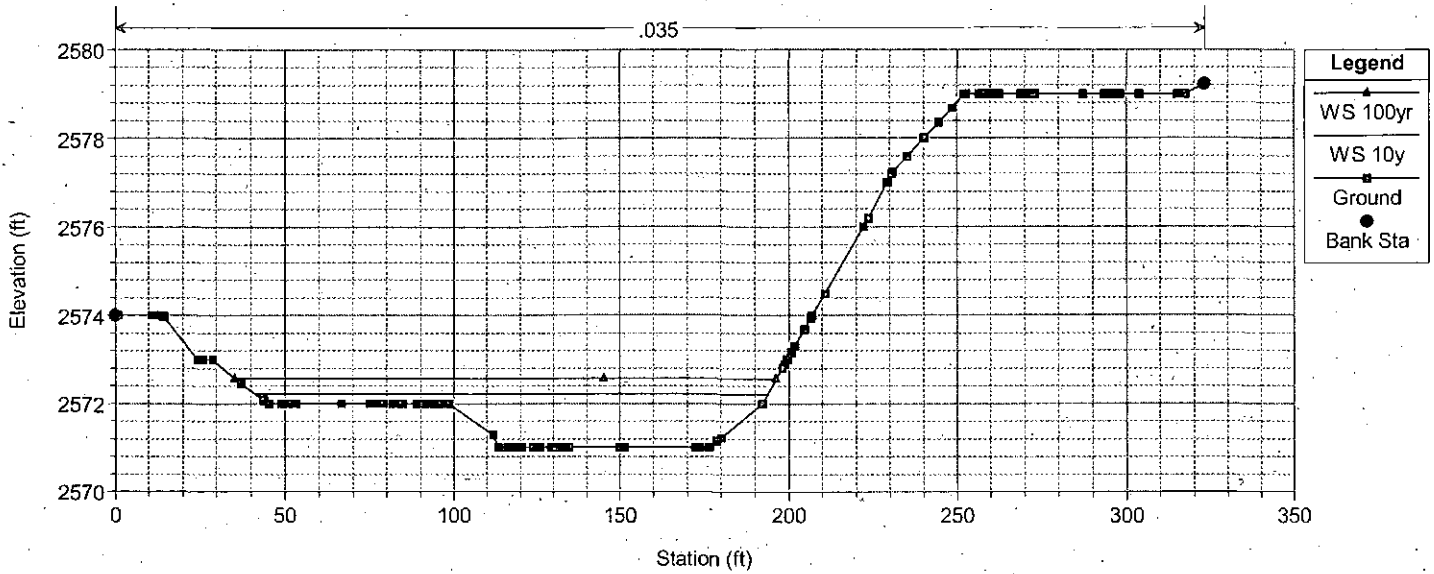
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream1 Reach = Reach1 RS = 2341



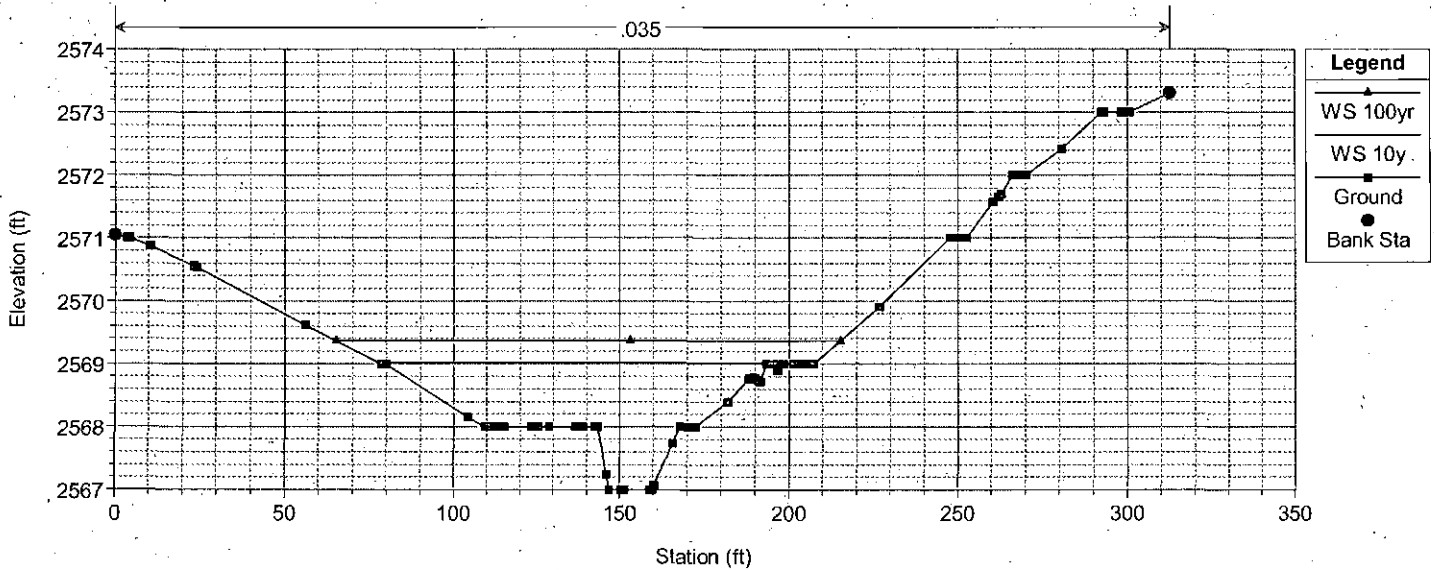
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream1 Reach = Reach1 RS = 2163



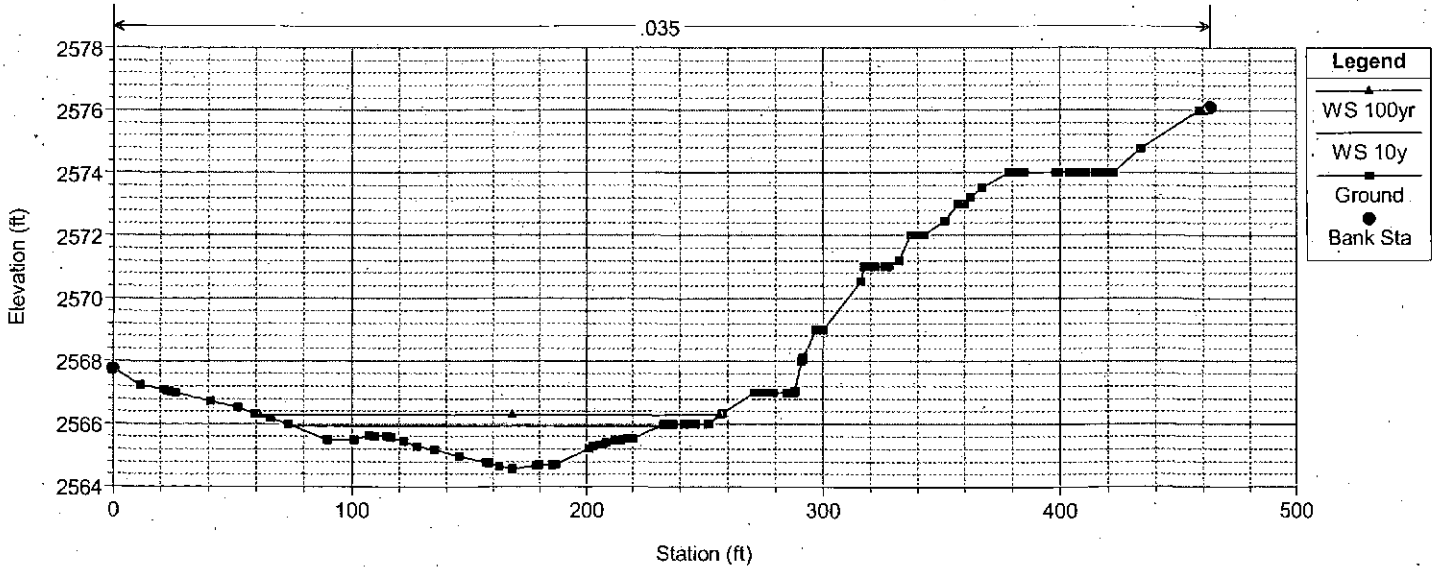
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream1 Reach = Reach1 RS = 2037



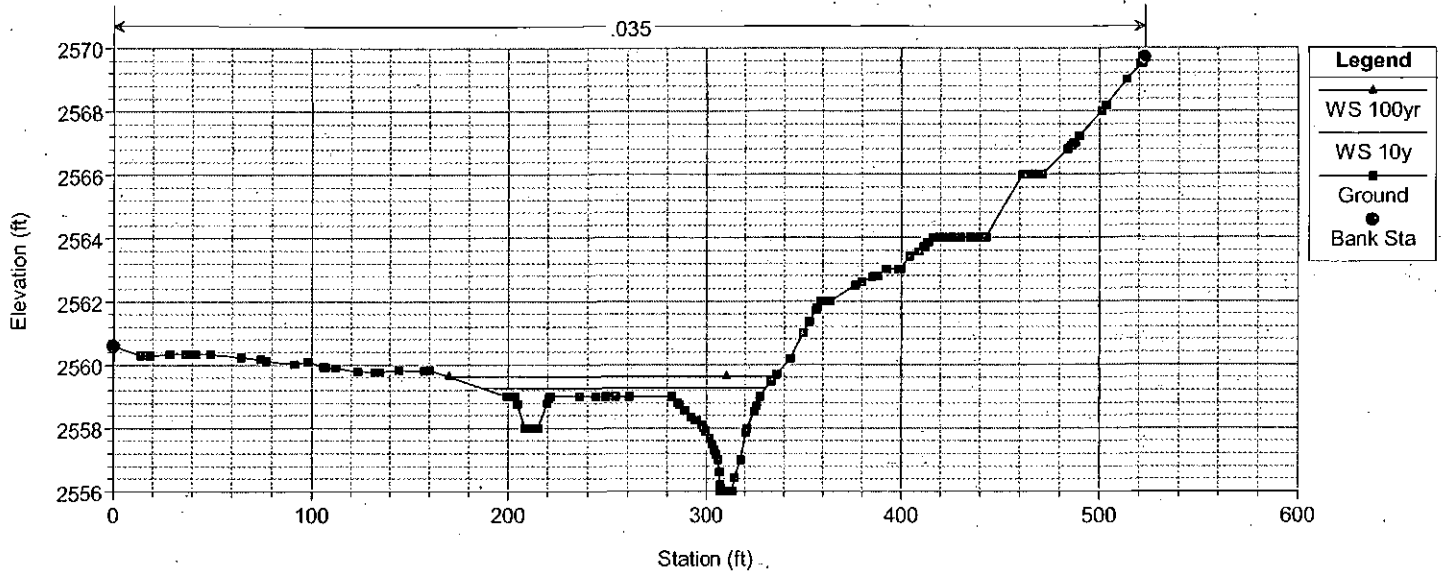
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream1 Reach = Reach1 RS = 1867



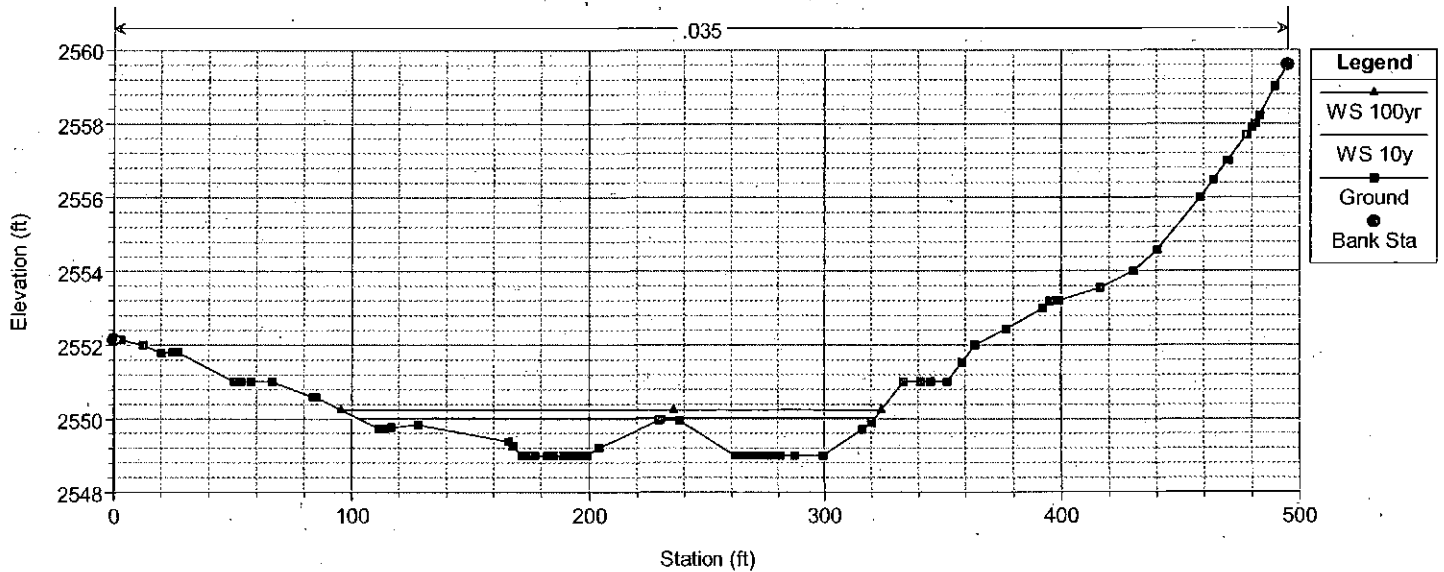
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream1 Reach = Reach1 RS = 1602



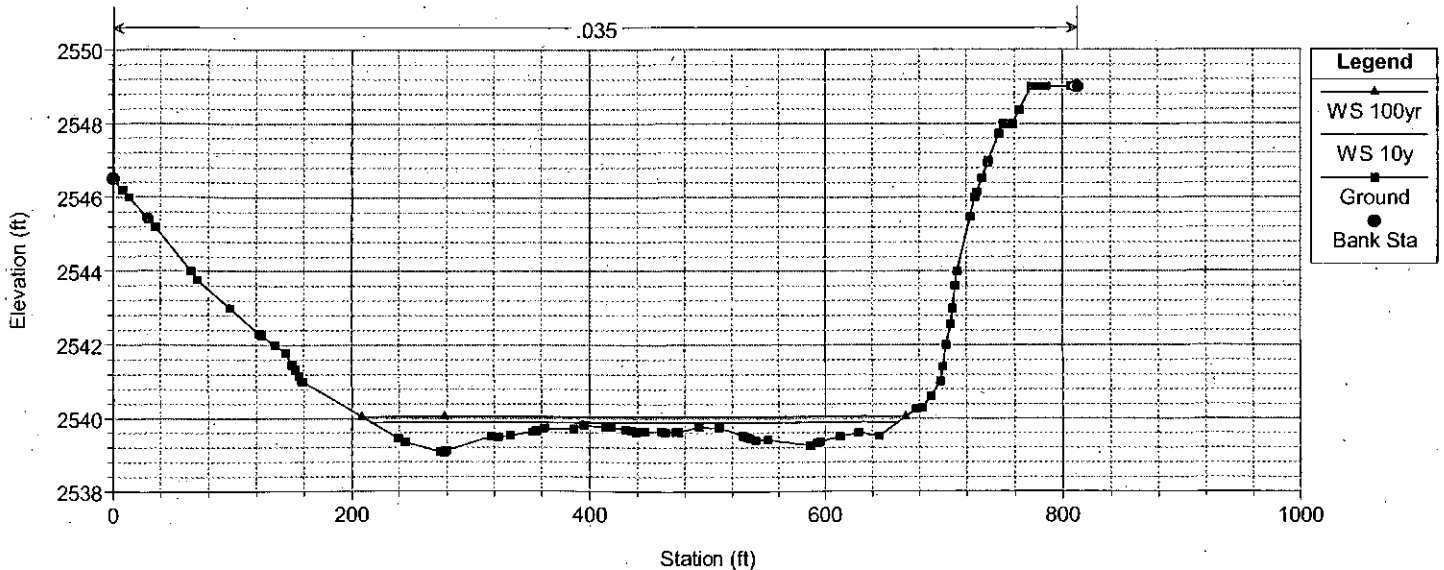
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream1 Reach = Reach1 RS = 1149



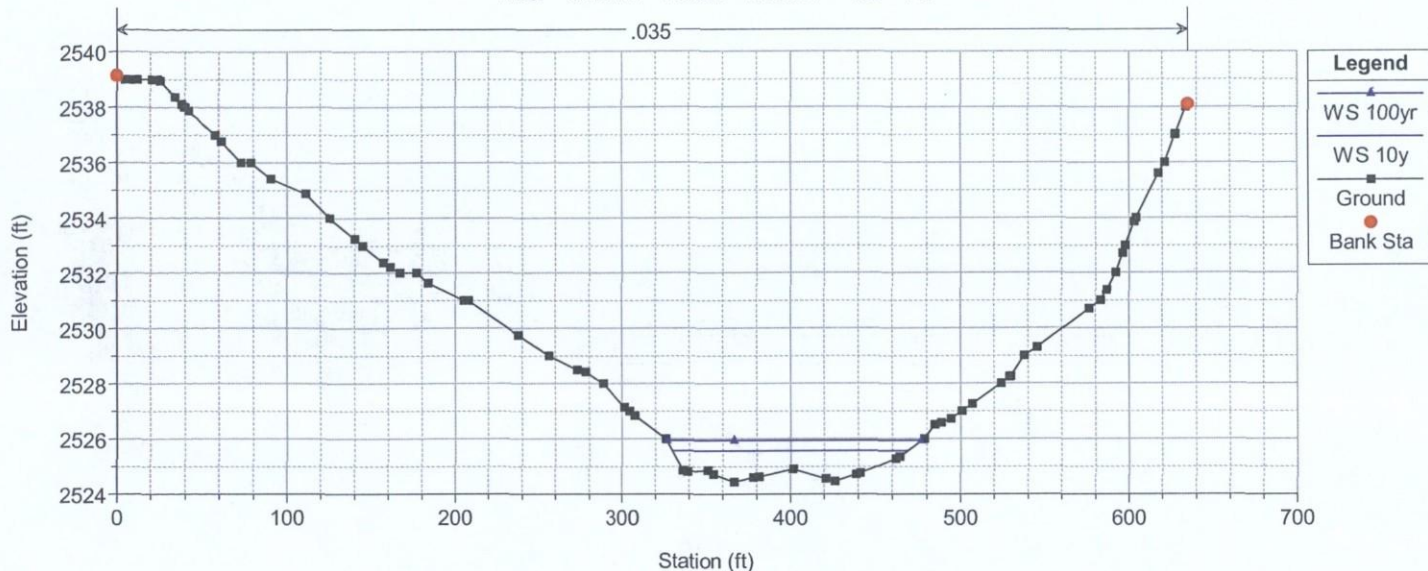
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream1 Reach = Reach1 RS = 672



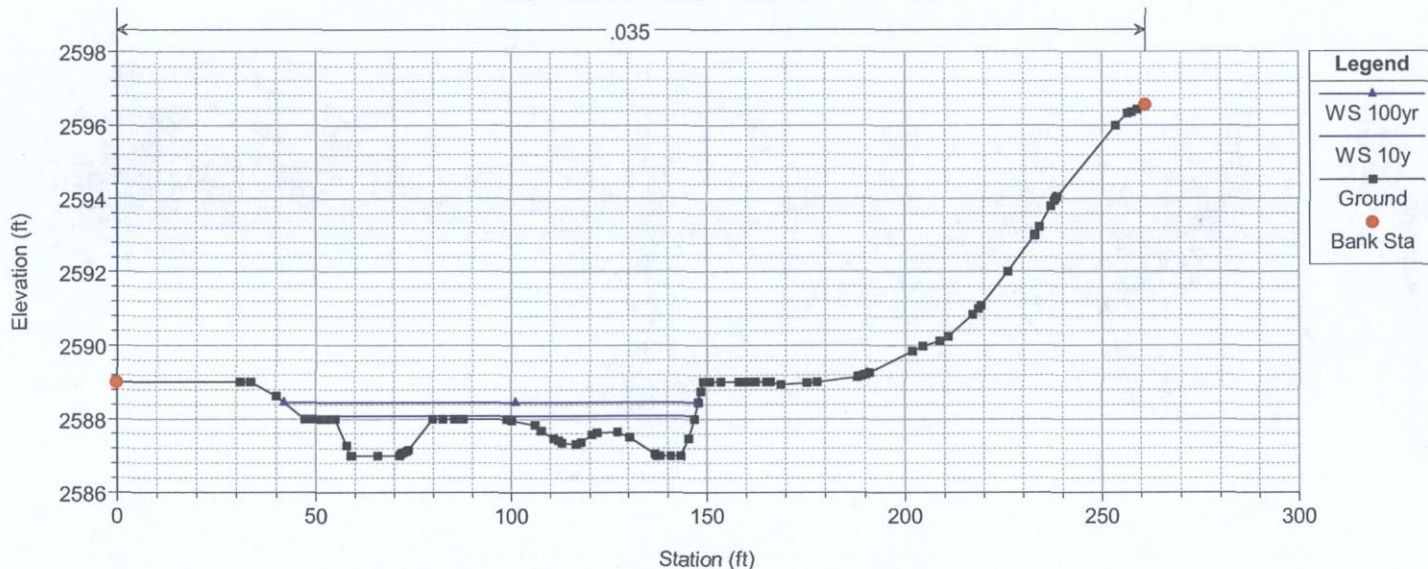
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream1 Reach = Reach1 RS = 78



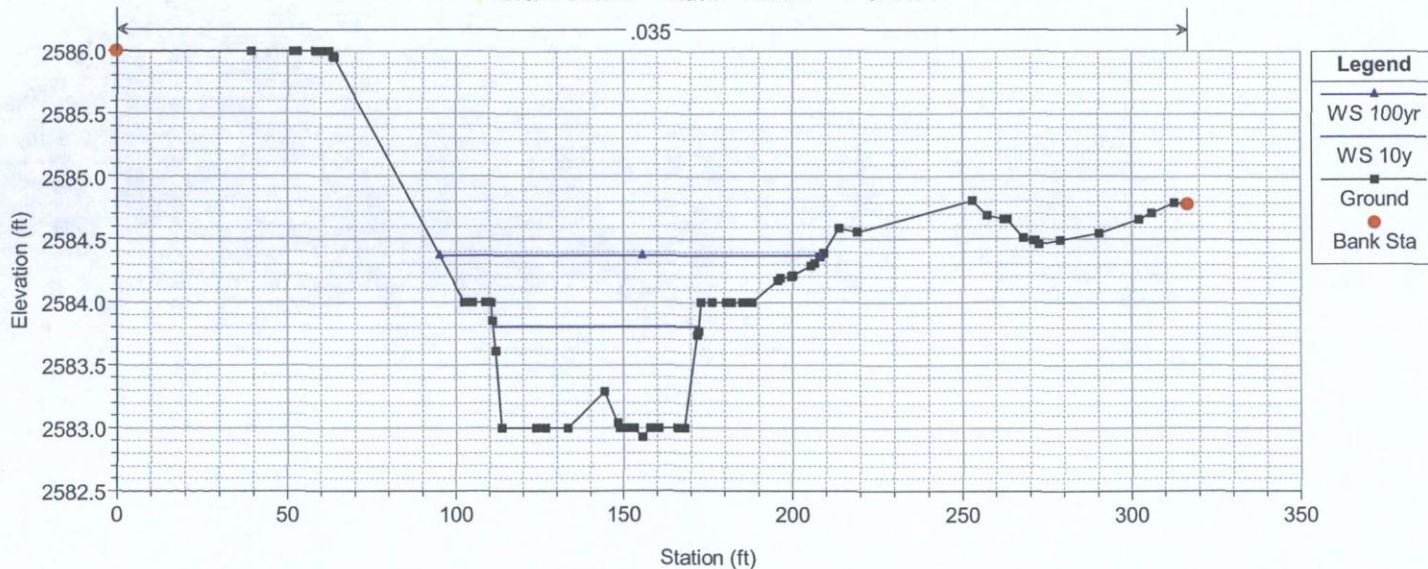
Greasewood Plan: Greasewood_EX 5/30/2014

River = Stream2 Reach = Reach1 RS = 633

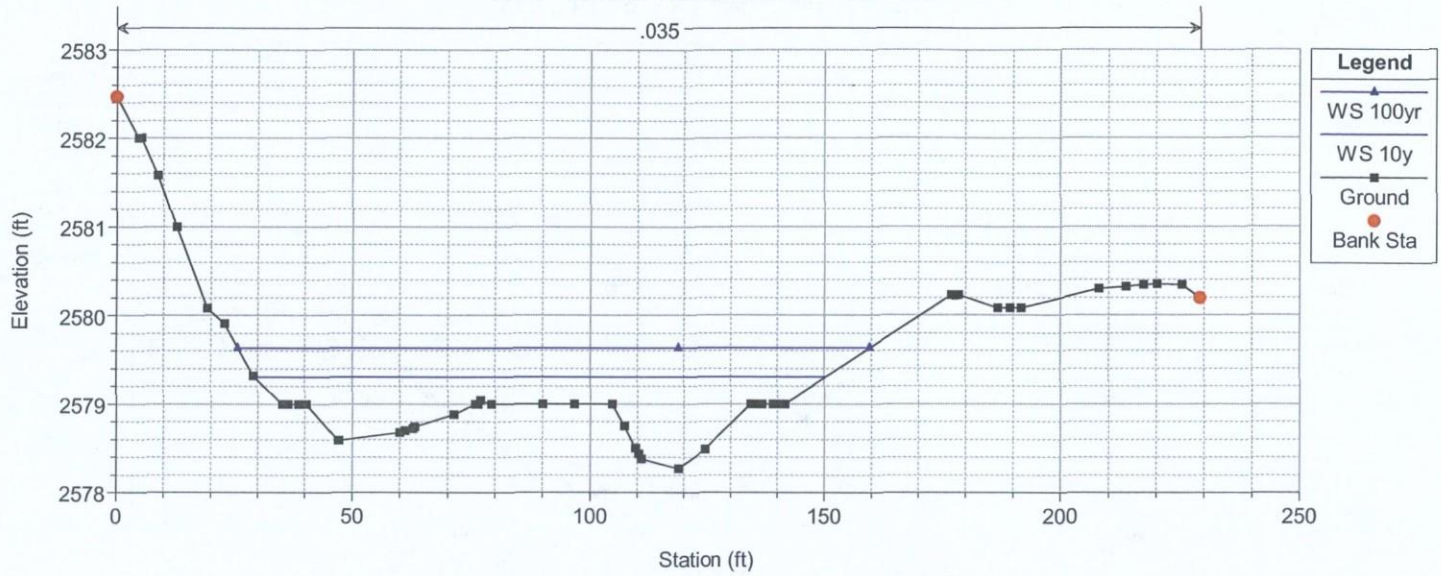


Greasewood Plan: Greasewood_EX 5/30/2014

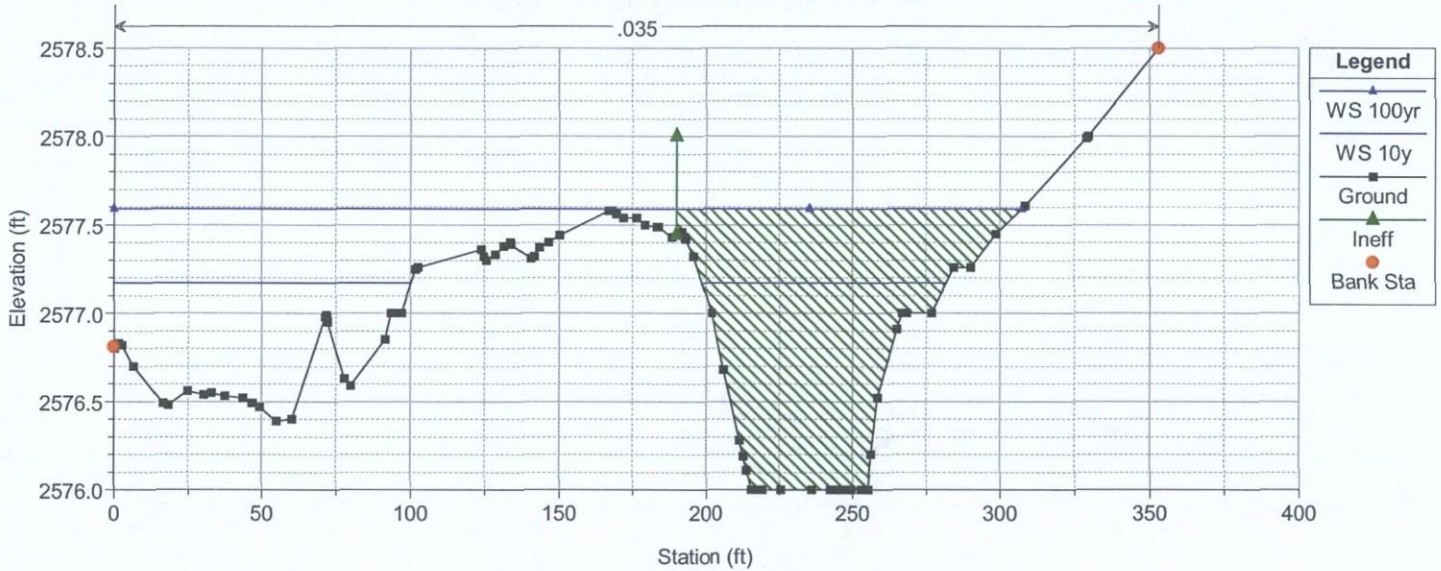
River = Stream2 Reach = Reach1 RS = 471



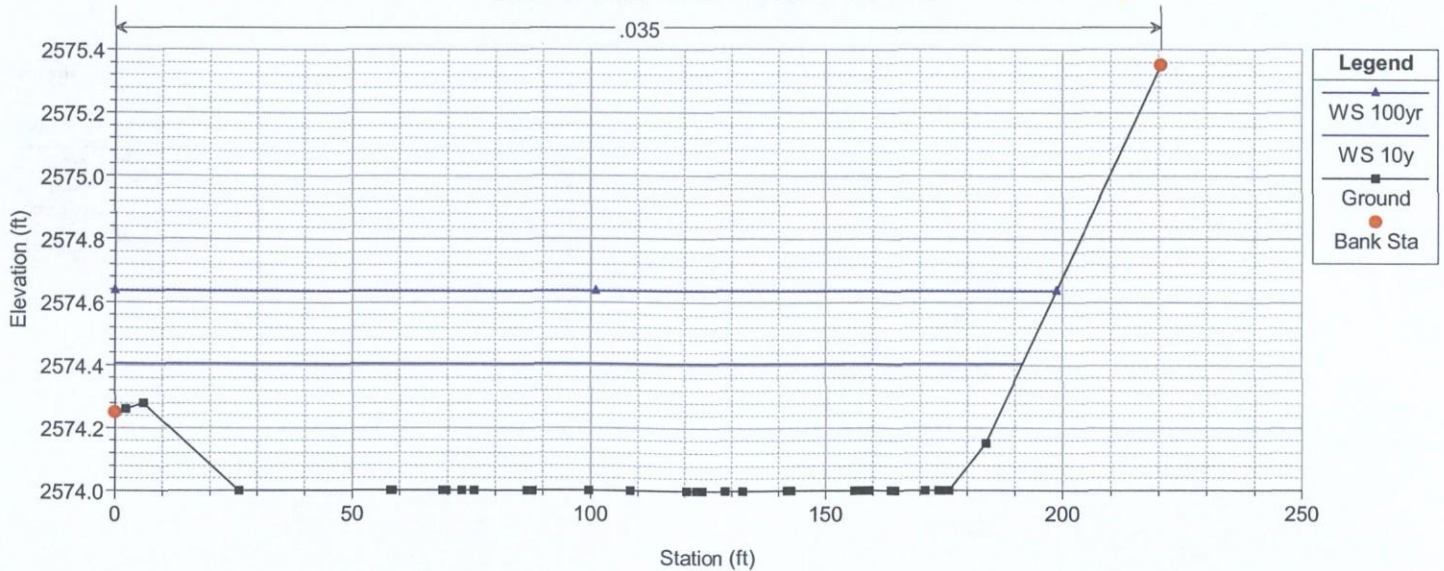
Greasewood Plan: Greasewood_EX 5/30/2014
 River = Stream2 Reach = Reach1 RS = 321



Greasewood Plan: Greasewood_EX 5/30/2014
 River = Stream2 Reach = Reach1 RS = 226

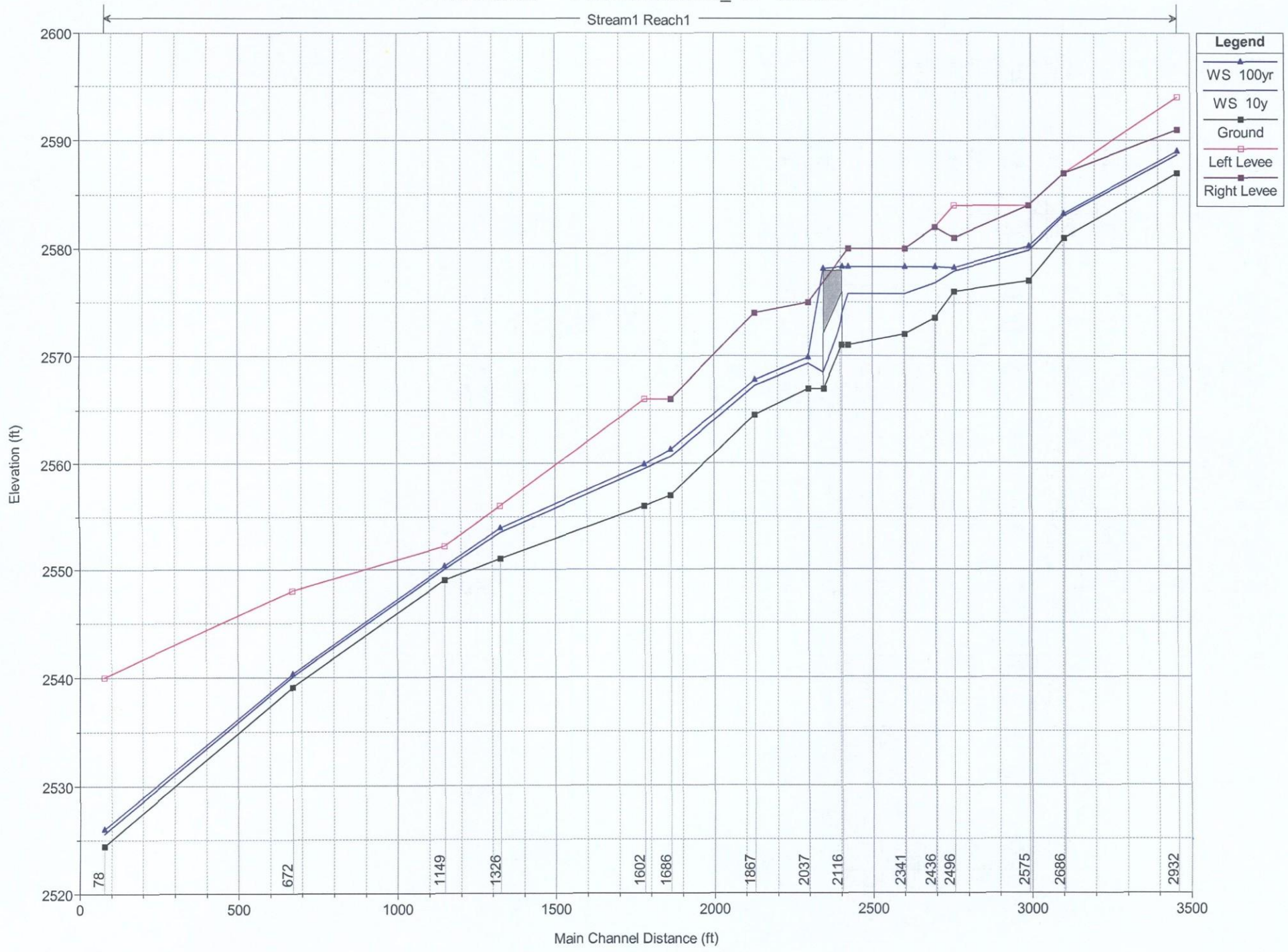


Greasewood Plan: Greasewood_EX 5/30/2014
 River = Stream2 Reach = Reach1 RS = 112

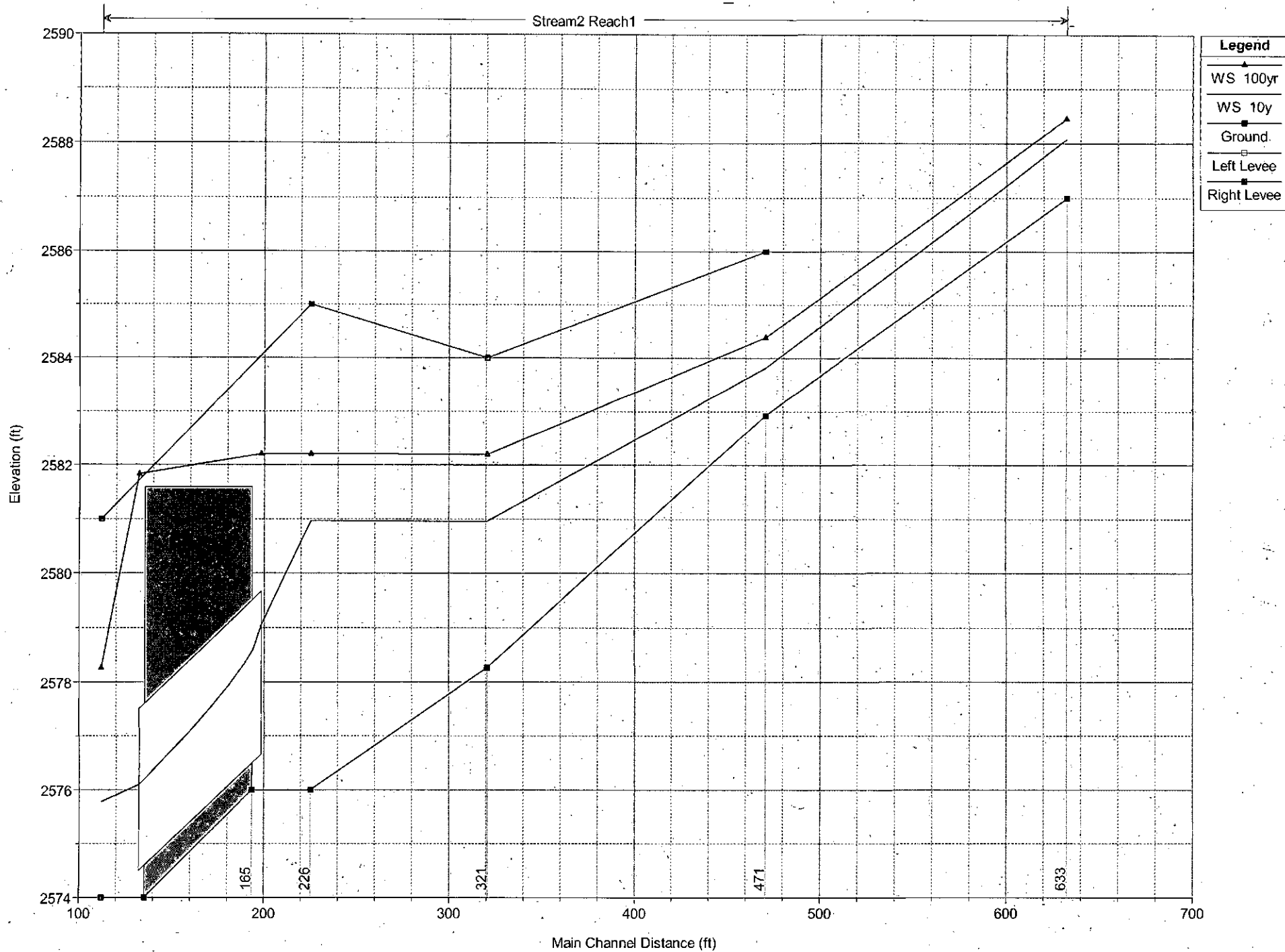


HEC-RAS Plan: GW_PR River: Stream1 Reach: Reach1

Reach	River Sta	Profile	Q Total (cfs)	Min.Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev. (ft)	E.G. Slope (ft/ft)	Vel.Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Reach1	2932	10y	314.00	2587.00	2588.68	2588.57	2589.24	0.012810	5.99	52.43	37.06	0.89
Reach1	2932	100yr	461.00	2587.00	2588.99	2588.99	2589.80	0.015067	7.23	63.76	38.21	0.99
Reach1	2686	10y	314.00	2581.00	2583.05	2583.05	2583.36	0.021427	4.47	70.23	113.84	1.00
Reach1	2686	100yr	461.00	2581.00	2583.24	2583.24	2583.63	0.019195	5.01	91.94	115.37	0.99
Reach1	2575	10y	314.00	2577.00	2579.85	2579.24	2579.99	0.003756	2.96	105.93	86.24	0.47
Reach1	2575	100yr	461.00	2577.00	2580.25	2579.54	2580.41	0.003760	3.24	142.50	101.80	0.48
Reach1	2496	10y	314.00	2576.00	2577.90	2577.87	2578.33	0.017004	5.21	60.29	64.69	0.95
Reach1	2496	100yr	461.00	2576.00	2578.22	2578.22	2578.69	0.018665	5.53	83.40	88.15	1.00
Reach1	2436	10y	314.00	2573.56	2576.81	2576.81	2577.22	0.019784	5.10	61.55	76.14	1.00
Reach1	2436	100yr	461.00	2573.56	2578.31	2577.12	2578.35	0.001098	1.71	269.25	197.00	0.26
Reach1	2341	10y	314.00	2572.00	2575.79	2574.50	2575.82	0.000665	1.40	224.07	153.45	0.20
Reach1	2341	100yr	461.00	2572.00	2578.32	2574.71	2578.33	0.000052	0.75	616.04	155.00	0.07
Reach1	2163	10y	533.00	2571.00	2575.79	2572.23	2575.79	0.000049	0.73	731.53	188.59	0.07
Reach1	2163	100yr	973.00	2571.00	2578.31	2572.60	2578.32	0.000034	0.79	1233.02	210.00	0.06
Reach1	2116		Culvert									
Reach1	2037	10y	533.00	2567.00	2569.34	2568.92	2569.52	0.006198	3.44	155.05	148.38	0.59
Reach1	2037	100yr	973.00	2567.00	2569.89	2569.40	2570.14	0.005396	4.03	241.36	163.46	0.58
Reach1	1867	10y	533.00	2564.58	2567.28	2567.28	2567.82	0.017551	5.85	91.12	83.88	0.99
Reach1	1867	100yr	973.00	2564.58	2567.81	2567.81	2568.61	0.016052	7.17	135.64	85.42	1.00
Reach1	1686	10y	533.00	2557.00	2560.68	2560.68	2561.25	0.017178	6.05	88.08	76.16	0.99
Reach1	1686	100yr	973.00	2557.00	2561.33	2561.33	2561.87	0.017119	5.90	164.88	148.90	0.99
Reach1	1602	10y	533.00	2556.00	2559.48	2559.26	2559.72	0.008786	3.93	135.52	136.49	0.70
Reach1	1602	100yr	973.00	2556.00	2559.88	2559.67	2560.28	0.009687	5.10	190.81	141.85	0.77
Reach1	1326	10y	533.00	2551.00	2553.50	2553.50	2553.89	0.020546	5.01	106.38	141.43	1.02
Reach1	1326	100yr	973.00	2551.00	2553.89	2553.89	2554.44	0.017372	5.97	162.96	146.48	1.00
Reach1	1149	10y	533.00	2549.00	2550.01	2549.98	2550.27	0.018747	4.11	129.80	218.55	0.94
Reach1	1149	100yr	973.00	2549.00	2550.28	2550.28	2550.68	0.018653	5.11	190.53	230.40	0.99
Reach1	672	10y	533.00	2539.07	2540.06	2540.06	2540.28	0.023448	3.81	140.02	312.12	1.00
Reach1	672	100yr	973.00	2539.07	2540.29	2540.29	2540.61	0.019662	4.51	215.60	325.99	0.98
Reach1	78	10y	533.00	2524.44	2525.57	2525.57	2525.95	0.019201	4.96	107.50	138.86	0.99
Reach1	78	100yr	973.00	2524.44	2525.97	2525.97	2526.51	0.016947	5.66	165.92	151.70	0.99

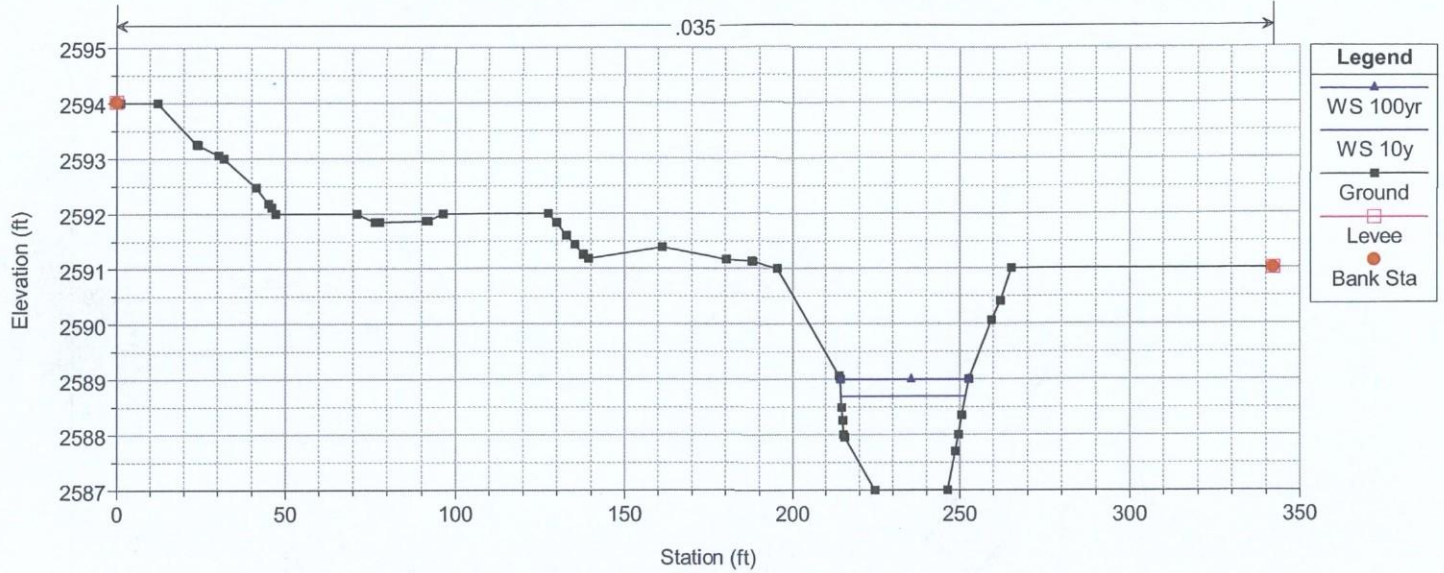


Greasewood Plan: Greasewood_PR 6/2/2014



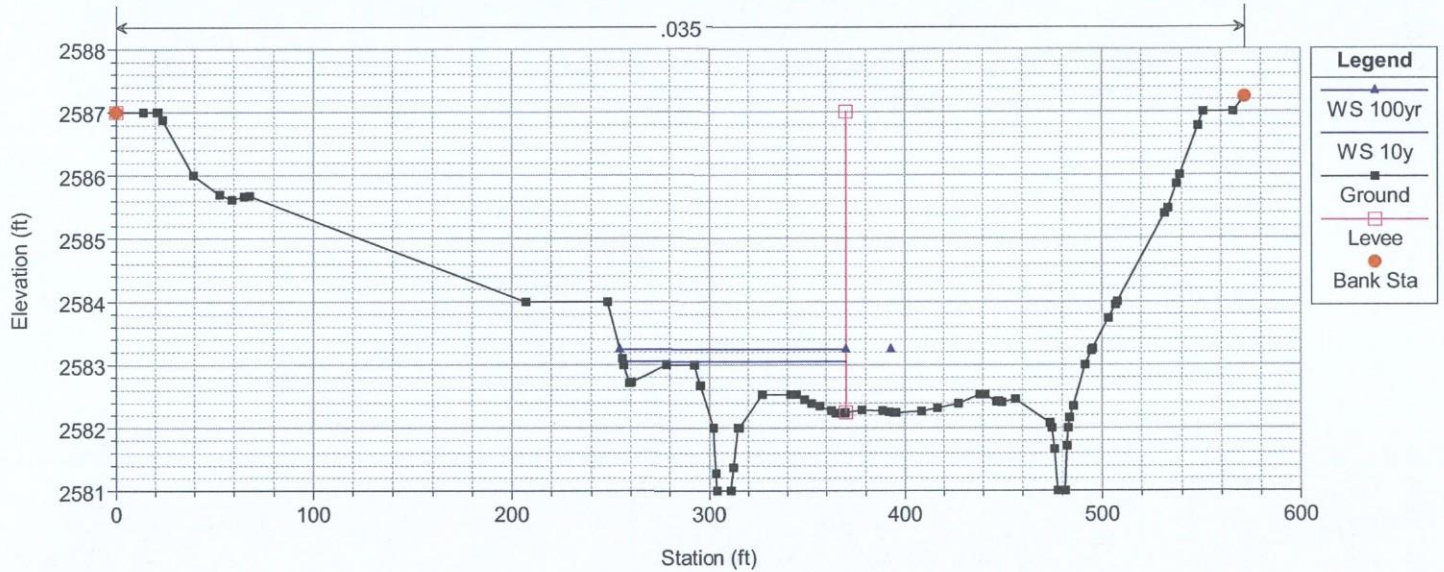
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream1 Reach = Reach1 RS = 2932



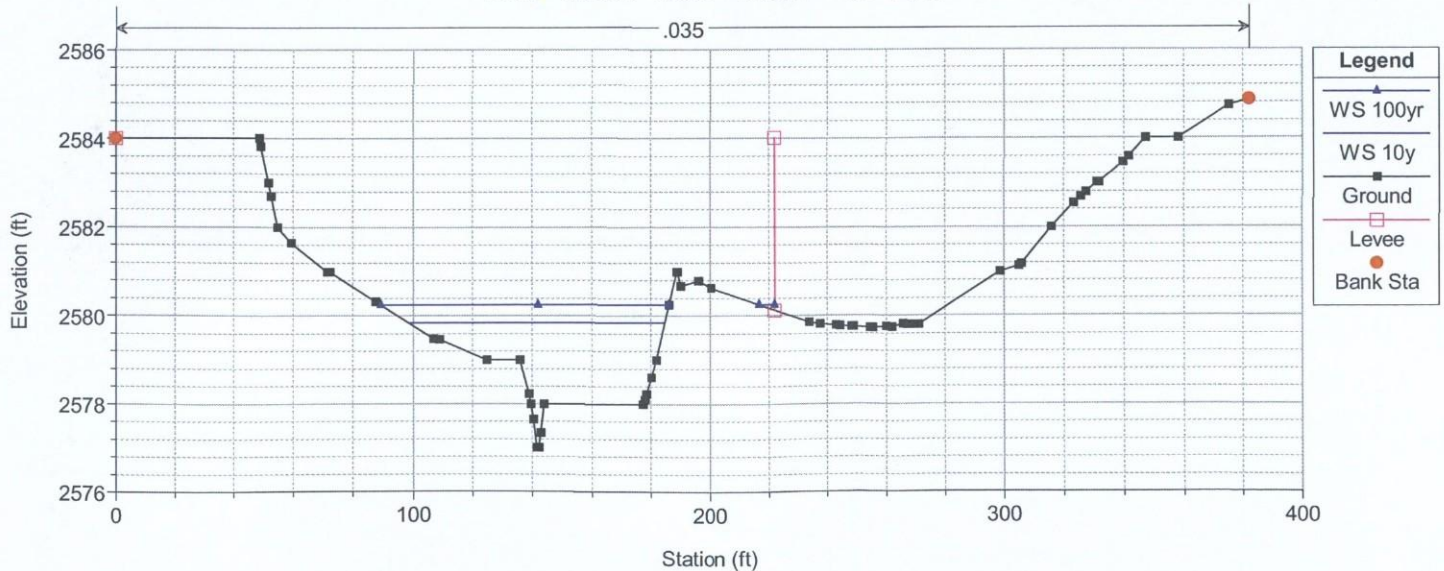
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream1 Reach = Reach1 RS = 2686



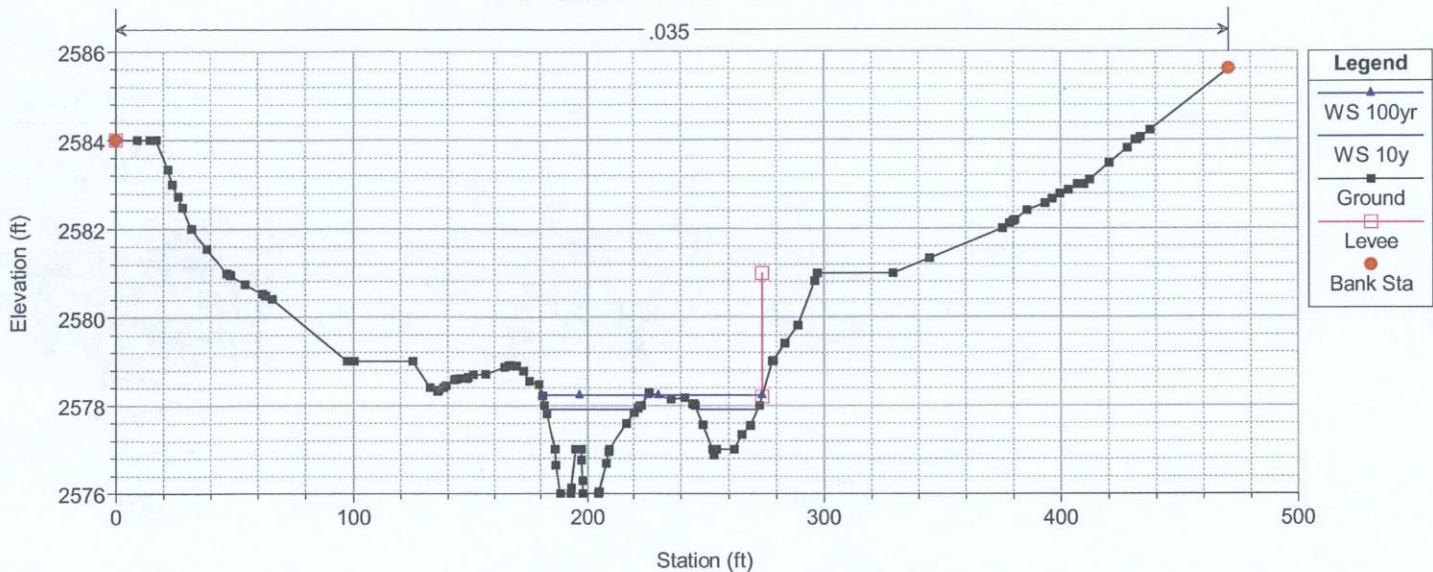
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream1 Reach = Reach1 RS = 2575



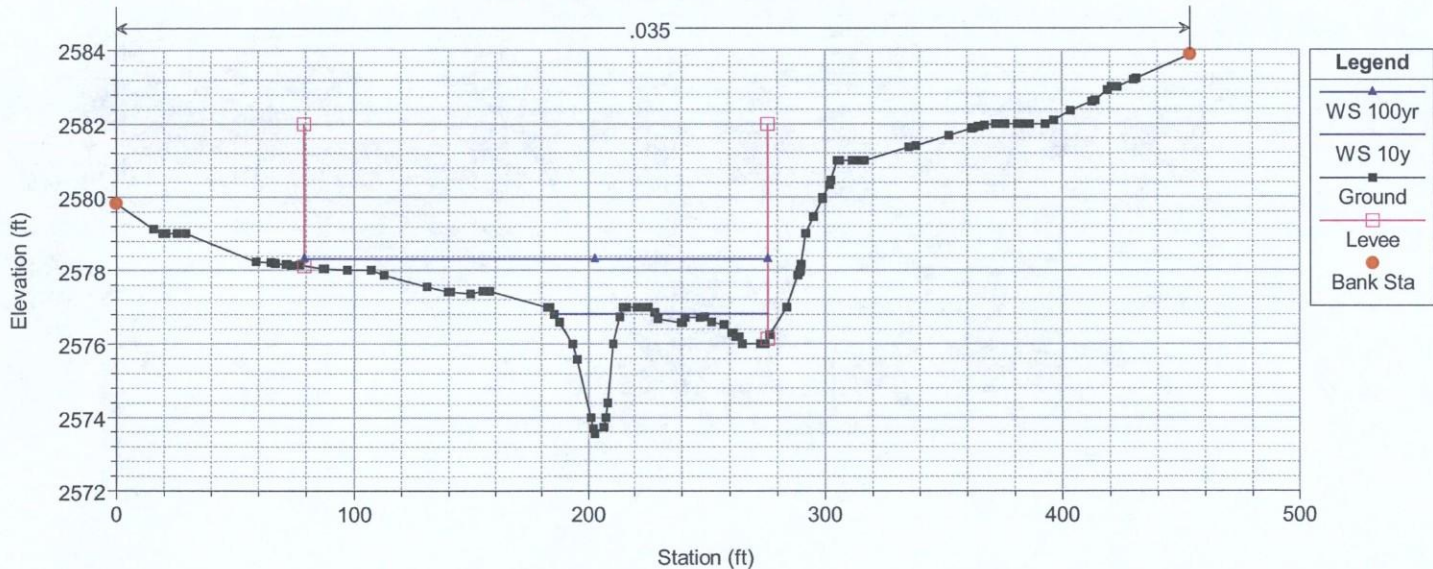
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream1 Reach = Reach1 RS = 2496



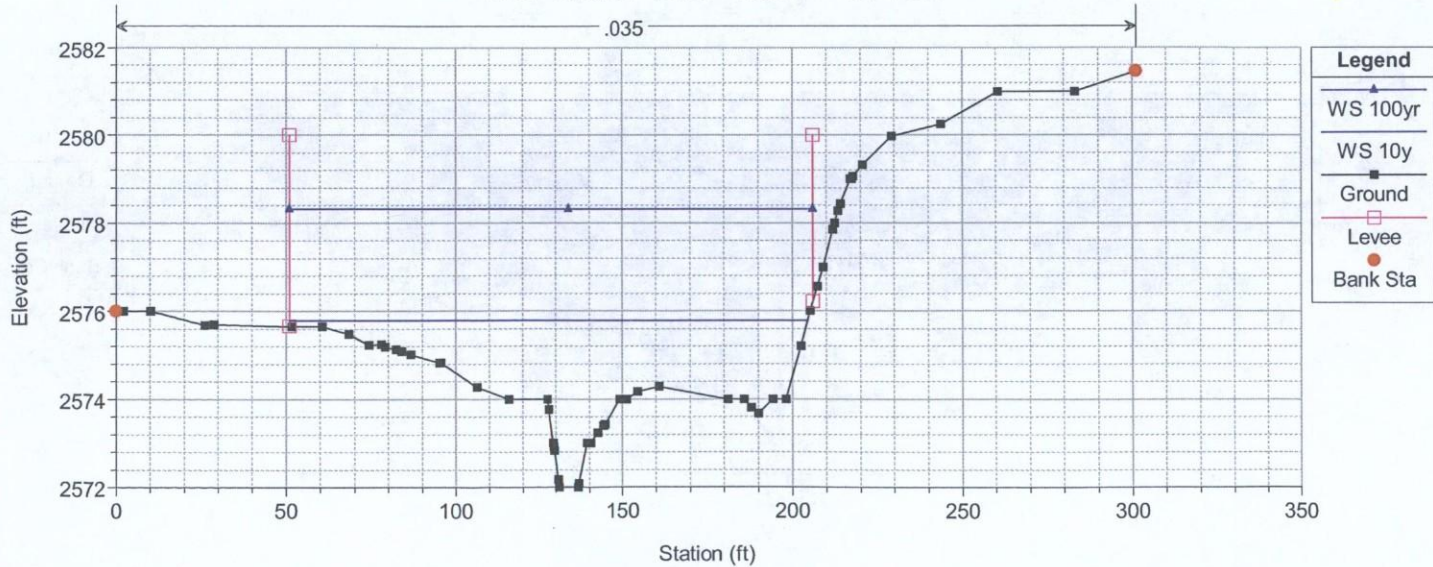
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream1 Reach = Reach1 RS = 2436

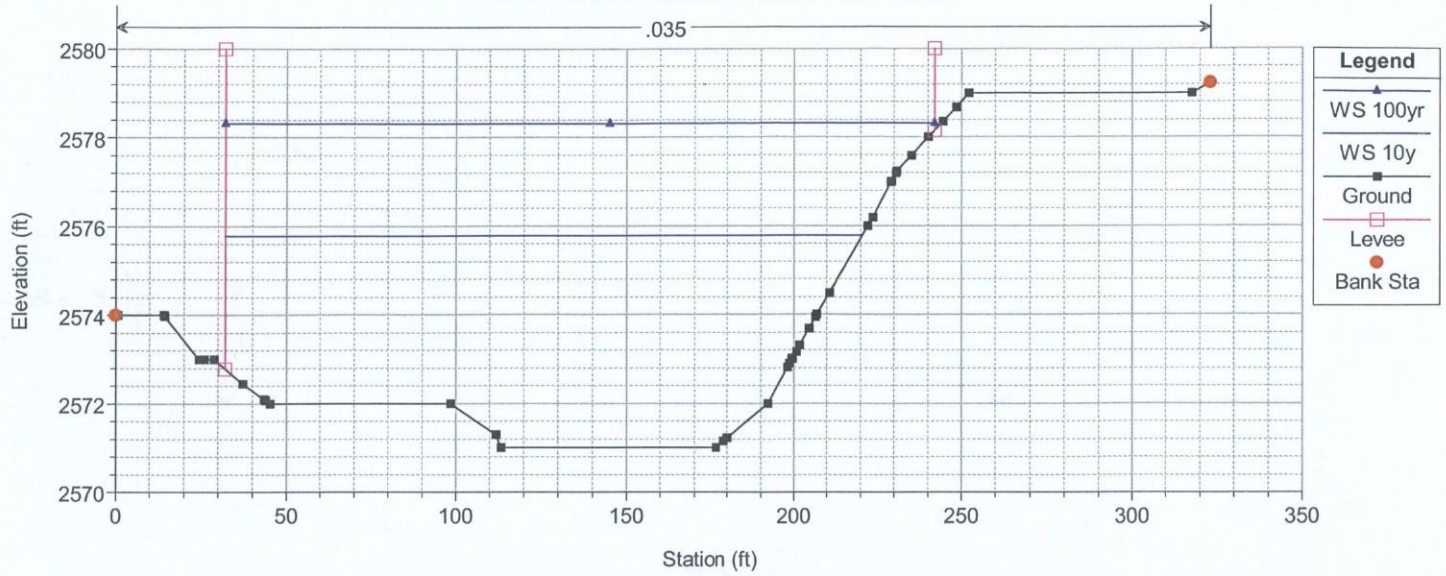


Greasewood Plan: Greasewood_PR 6/2/2014

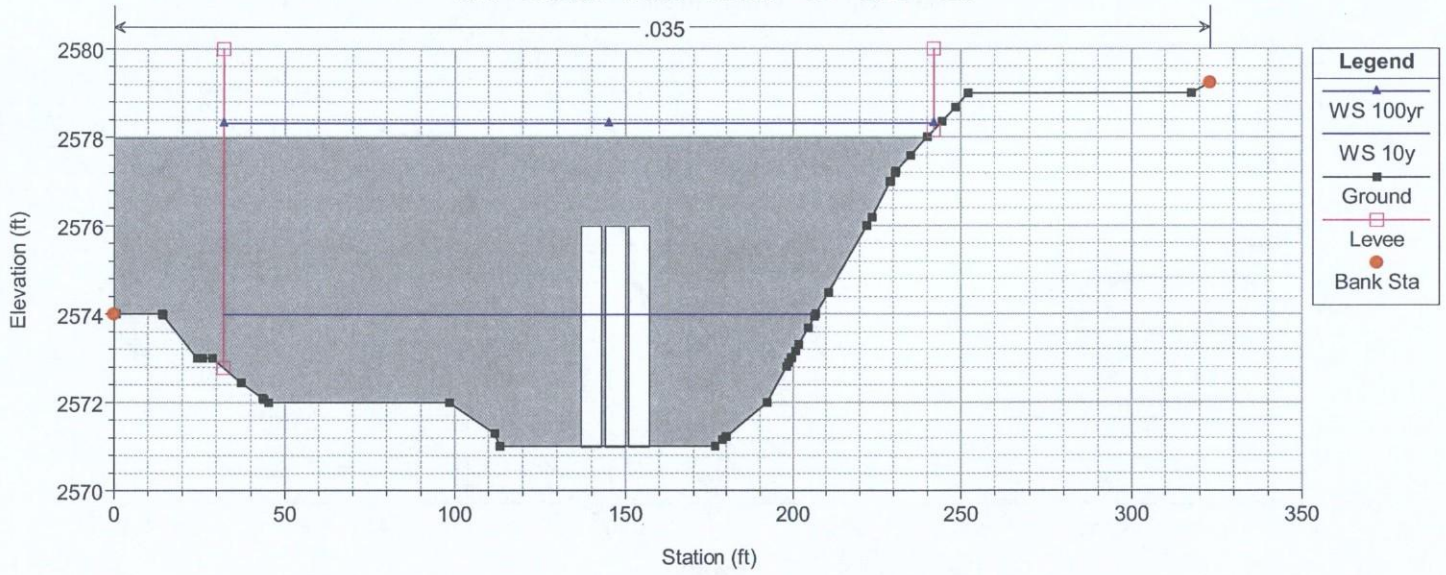
River = Stream1 Reach = Reach1 RS = 2341



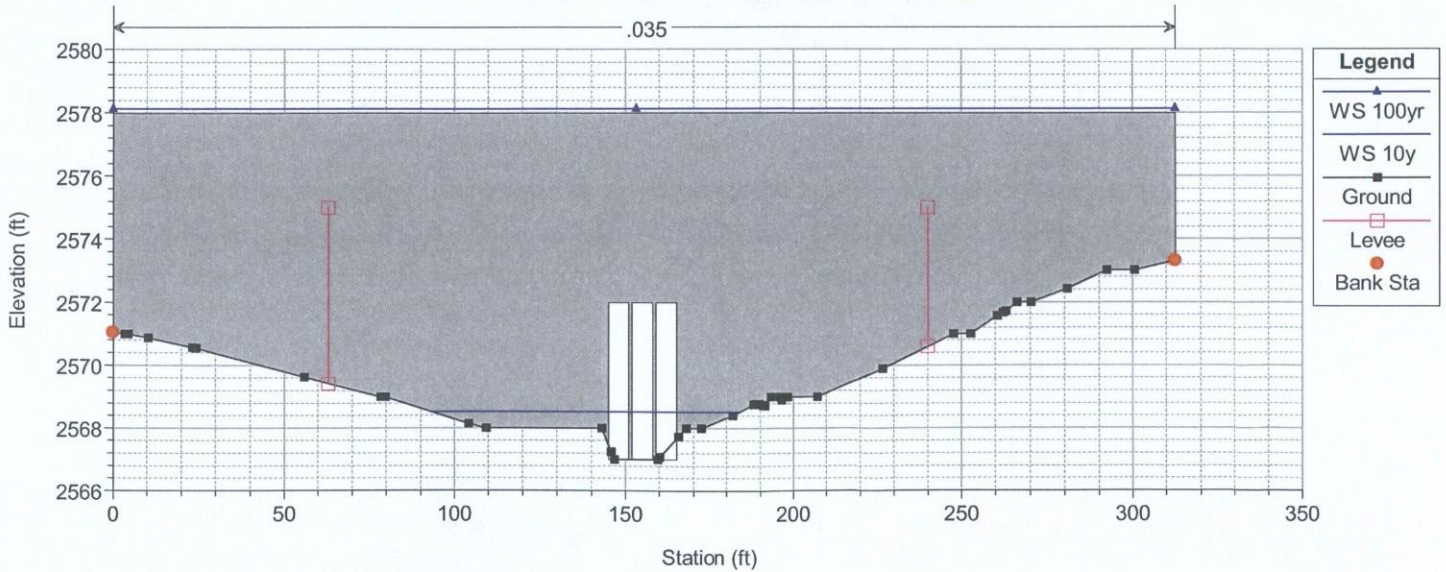
Greasewood Plan: Greasewood_PR 6/2/2014
 River = Stream1 Reach = Reach1 RS = 2163



Greasewood Plan: Greasewood_PR 6/2/2014
 River = Stream1 Reach = Reach1 RS = 2116 Culv

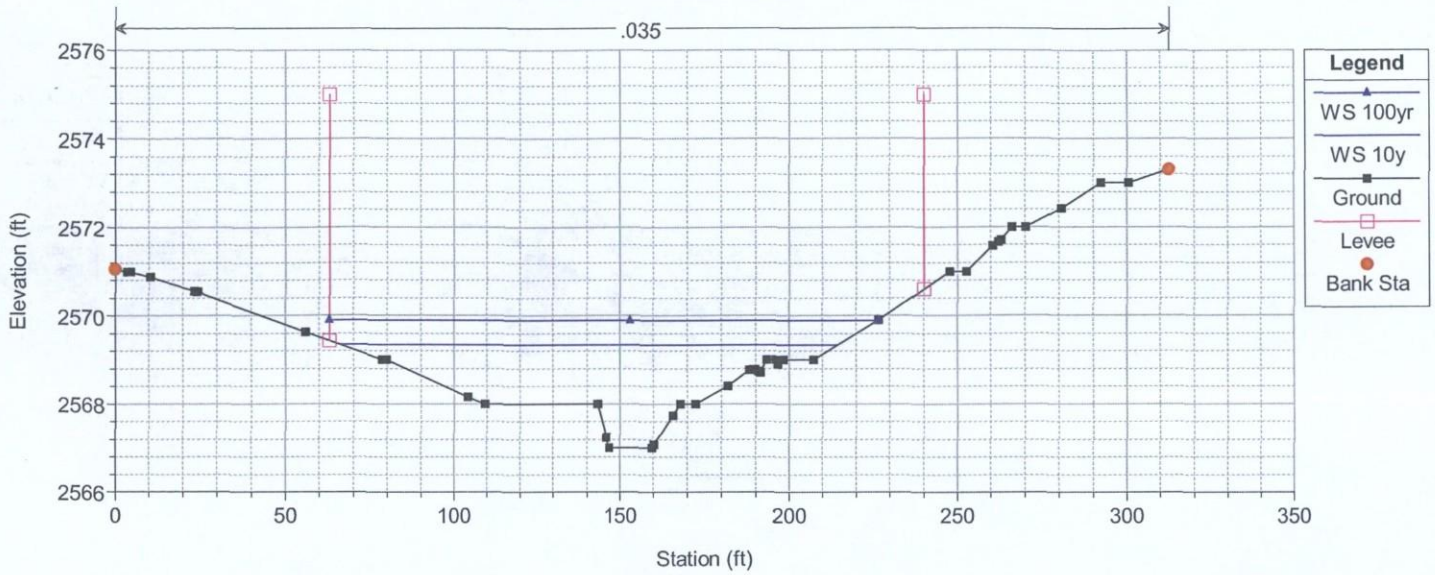


Greasewood Plan: Greasewood_PR 6/2/2014
 River = Stream1 Reach = Reach1 RS = 2116 Culv



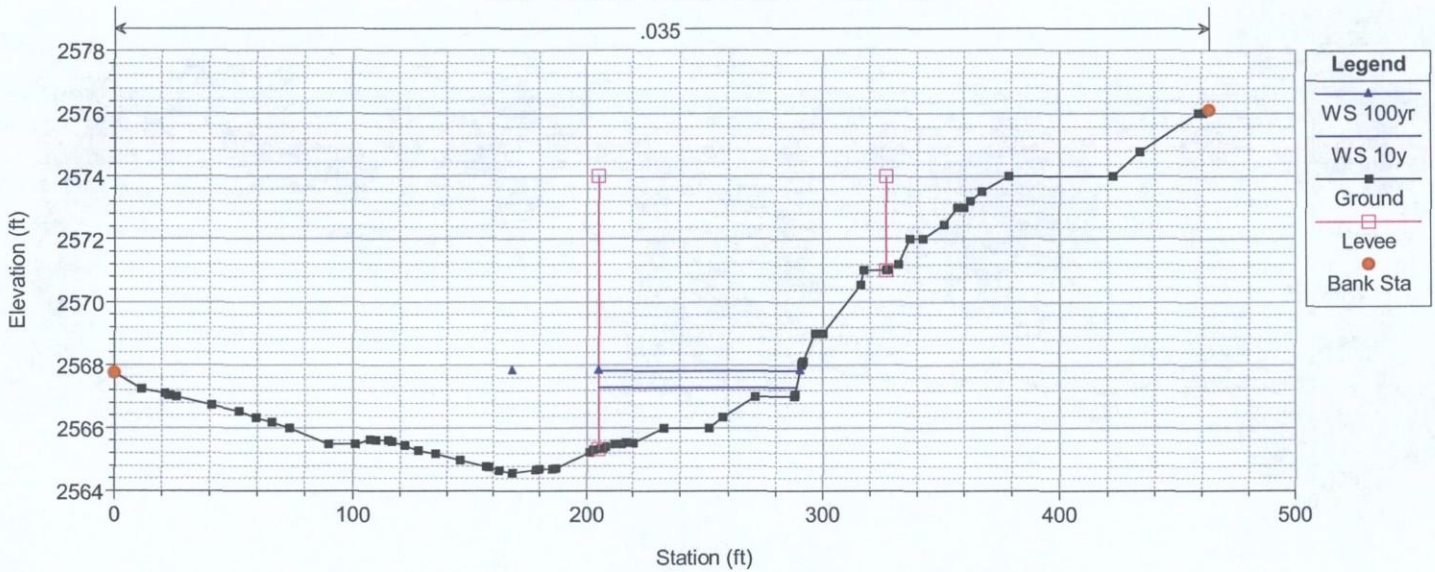
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream1 Reach = Reach1 RS = 2037



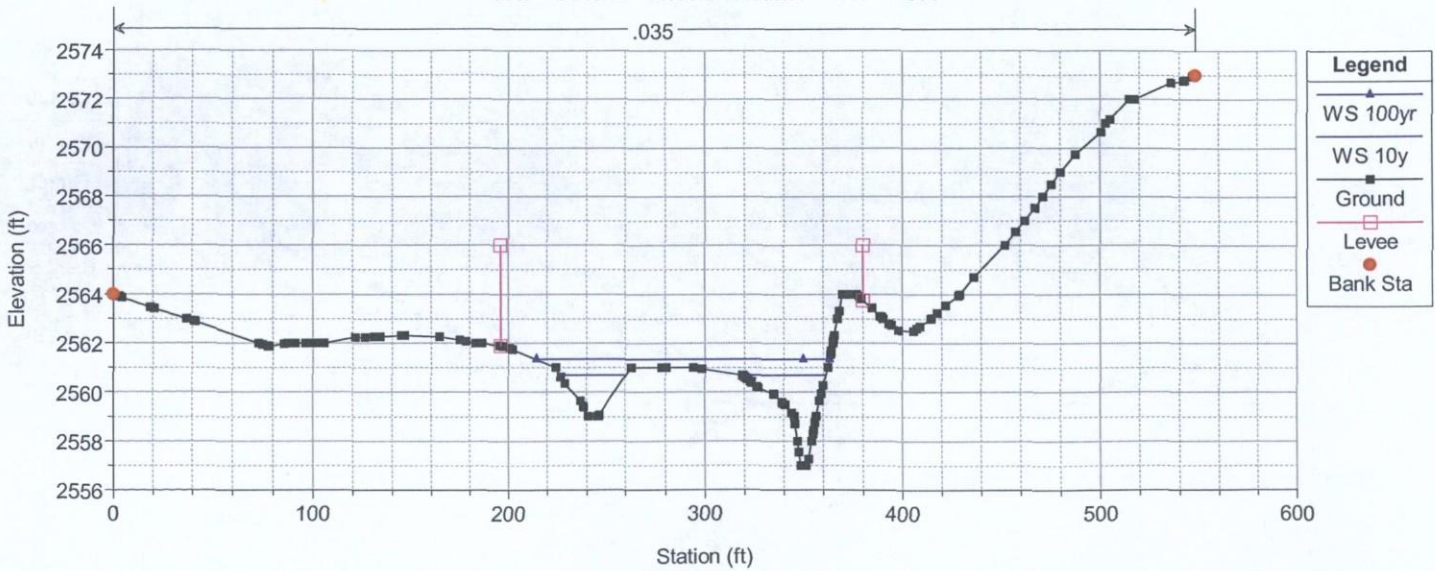
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream1 Reach = Reach1 RS = 1867

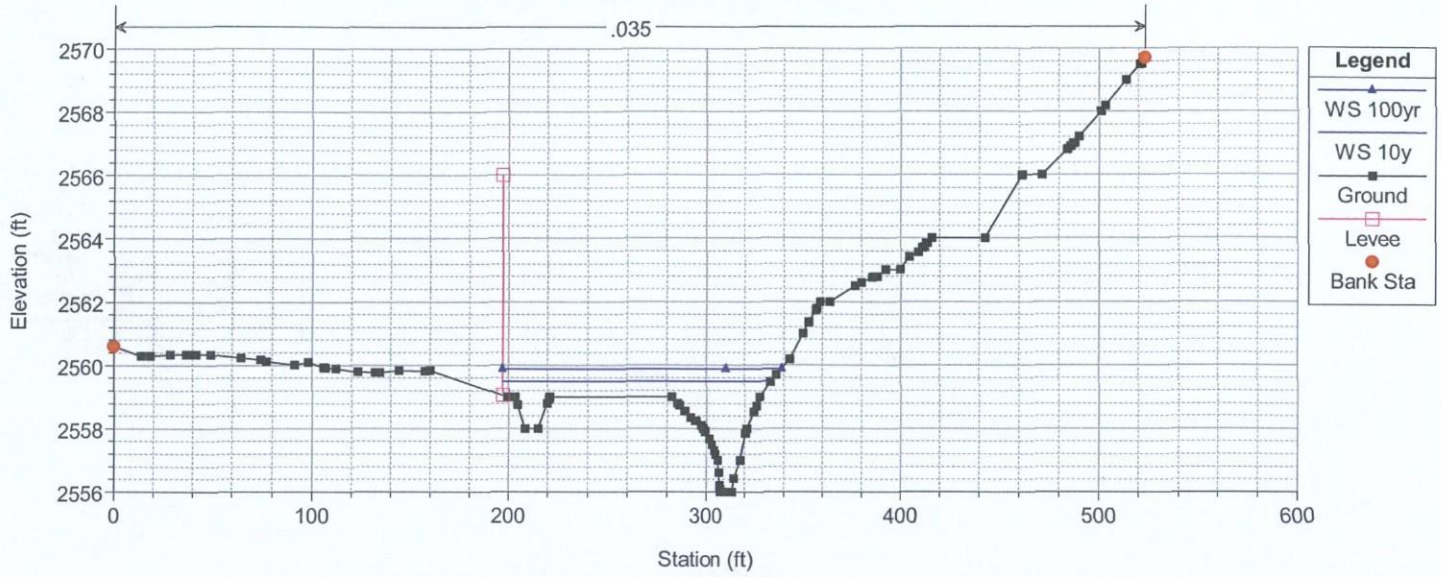


Greasewood Plan: Greasewood_PR 6/2/2014

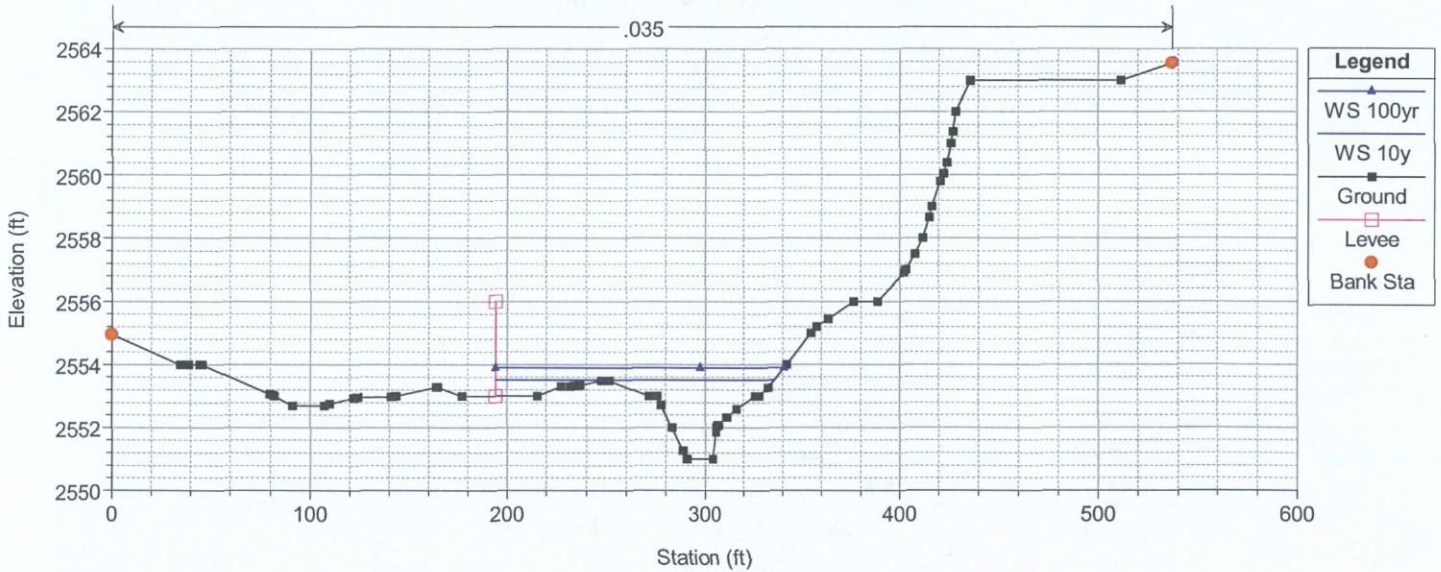
River = Stream1 Reach = Reach1 RS = 1686



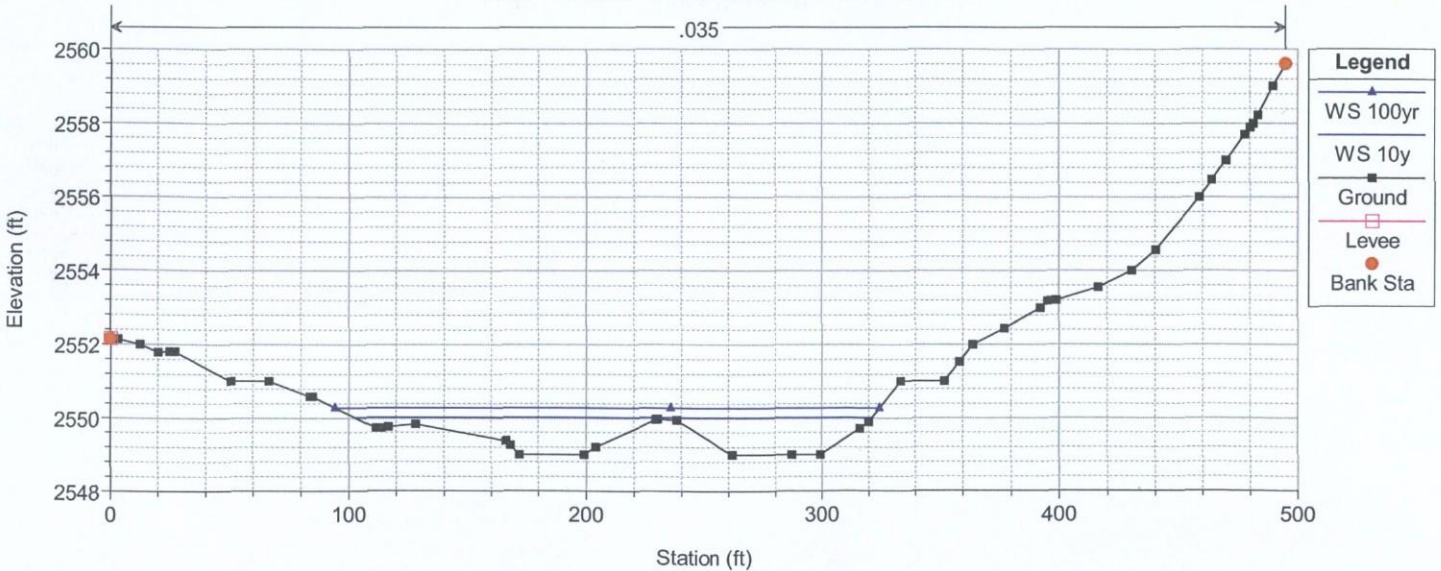
Greasewood Plan: Greasewood_PR 6/2/2014
 River = Stream1 Reach = Reach1 RS = 1602



Greasewood Plan: Greasewood_PR 6/2/2014
 River = Stream1 Reach = Reach1 RS = 1326

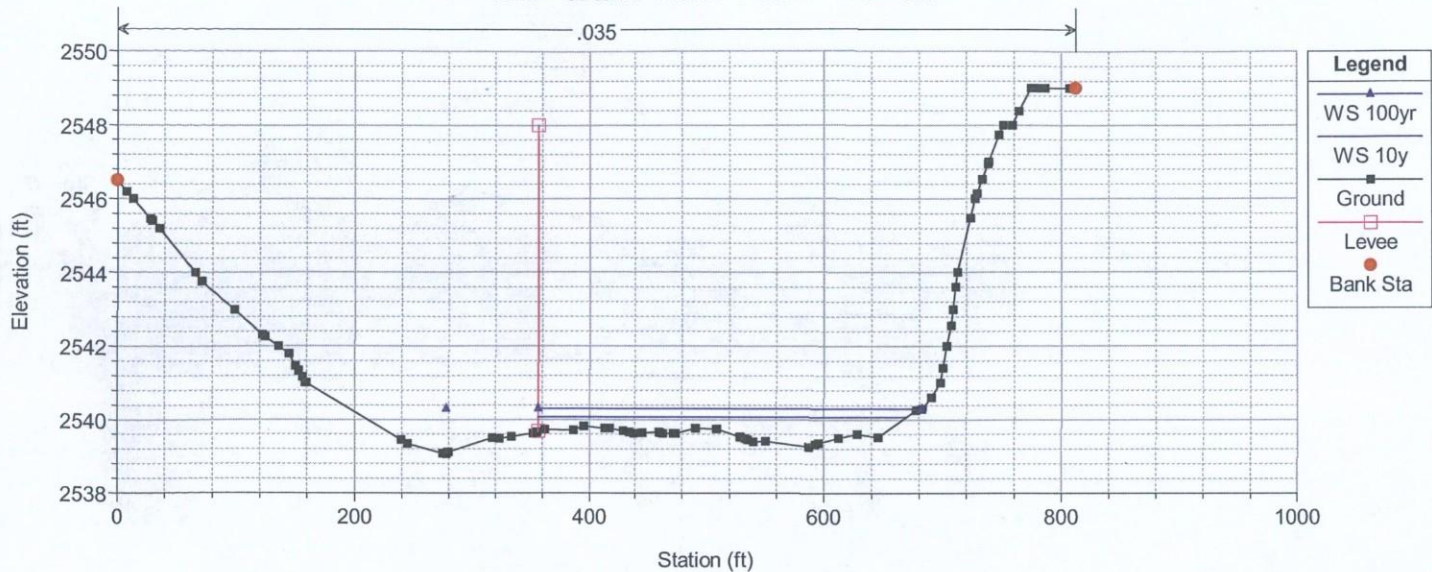


Greasewood Plan: Greasewood_PR 6/2/2014
 River = Stream1 Reach = Reach1 RS = 1149



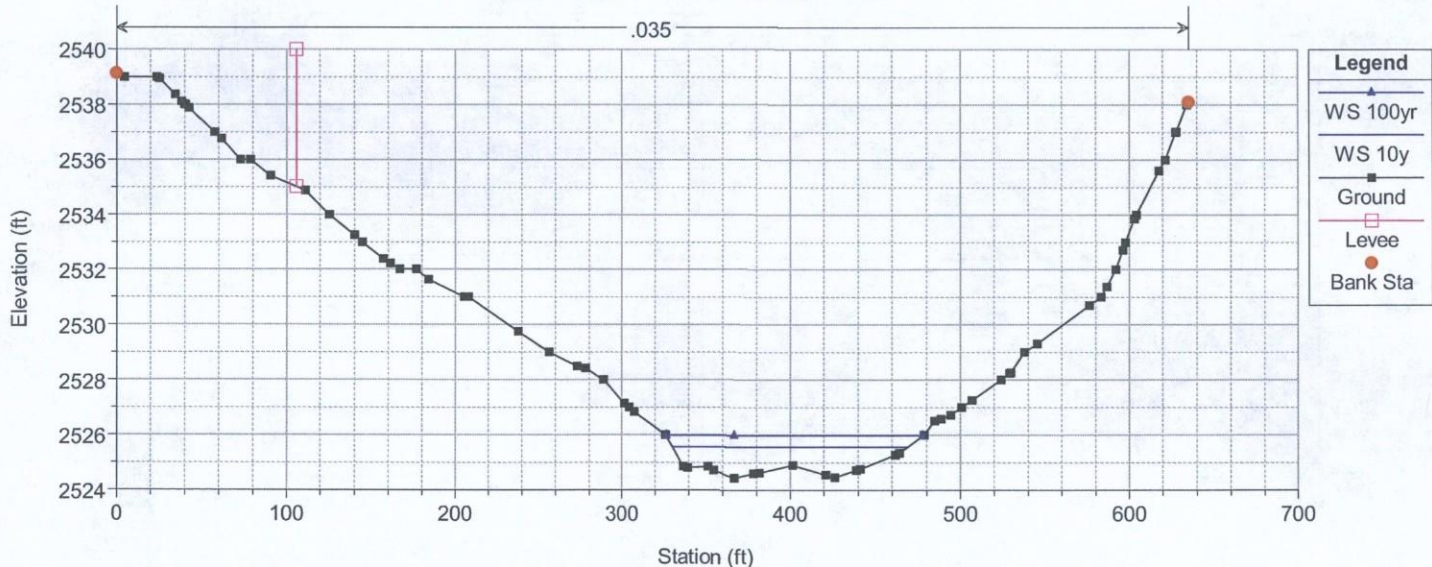
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream1 Reach = Reach1 RS = 672



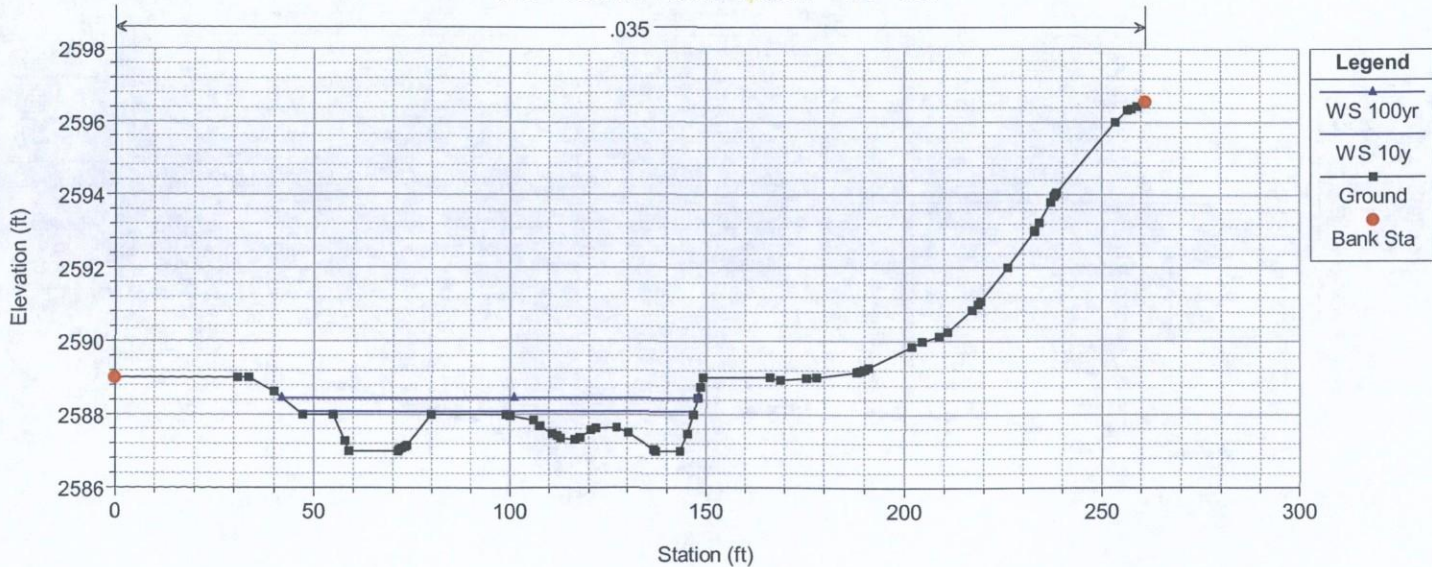
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream1 Reach = Reach1 RS = 78

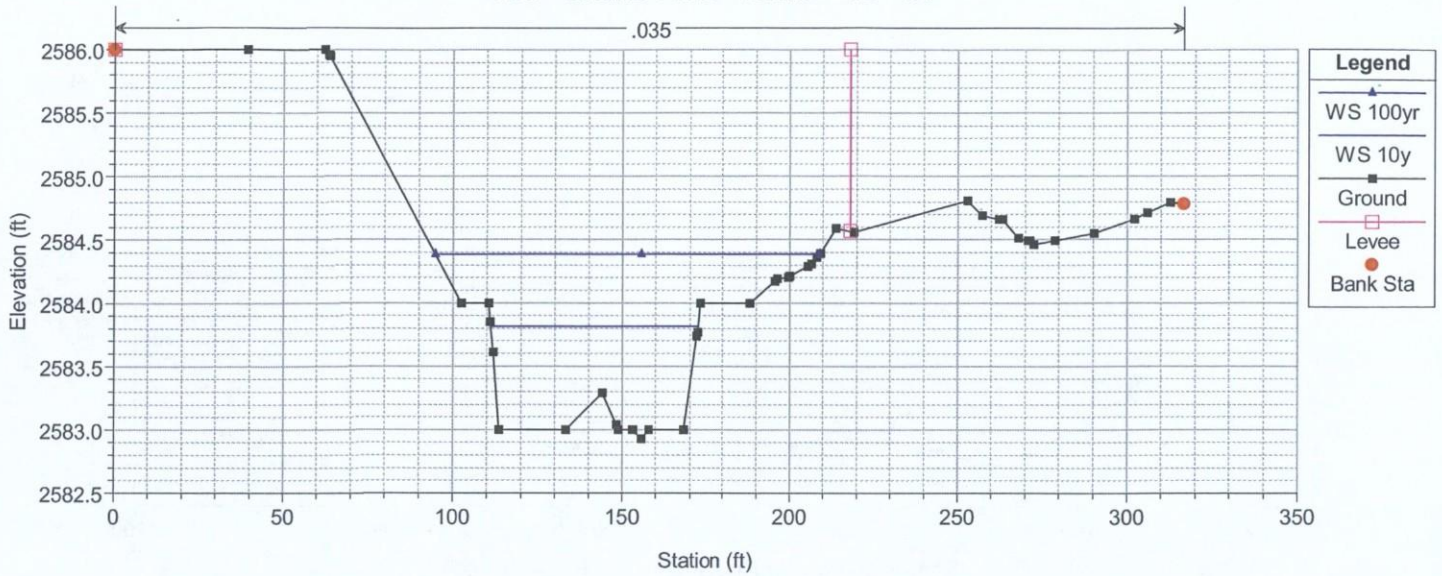


Greasewood Plan: Greasewood_PR 6/2/2014

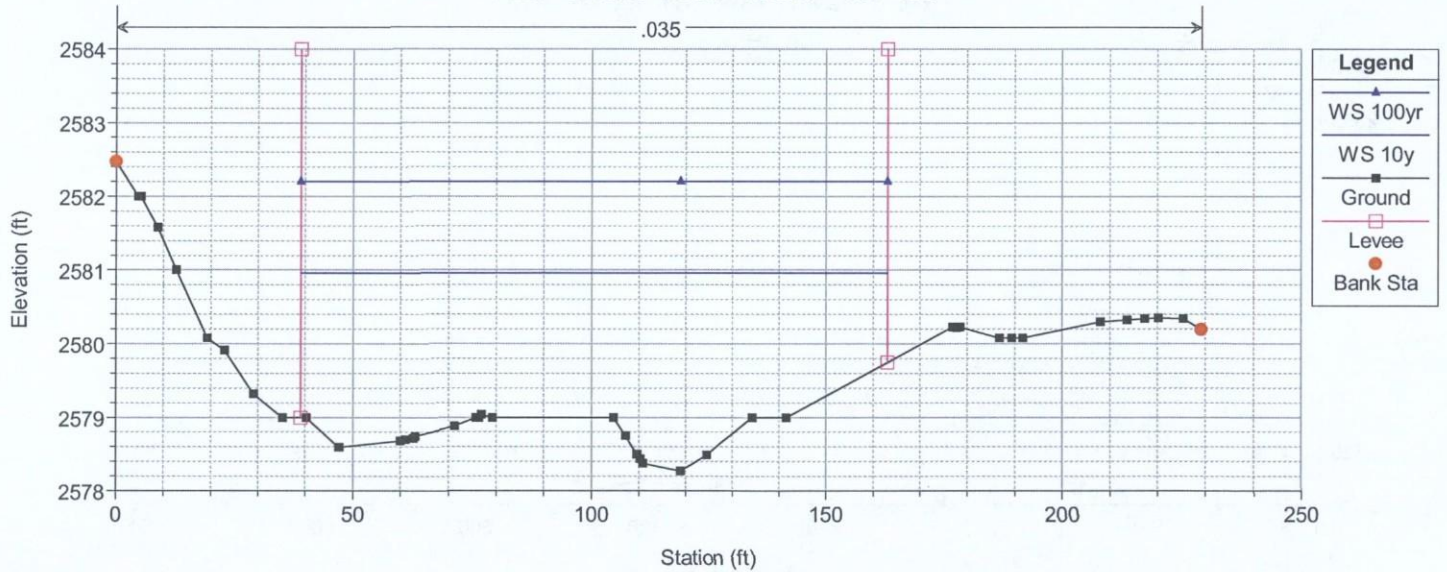
River = Stream2 Reach = Reach1 RS = 633



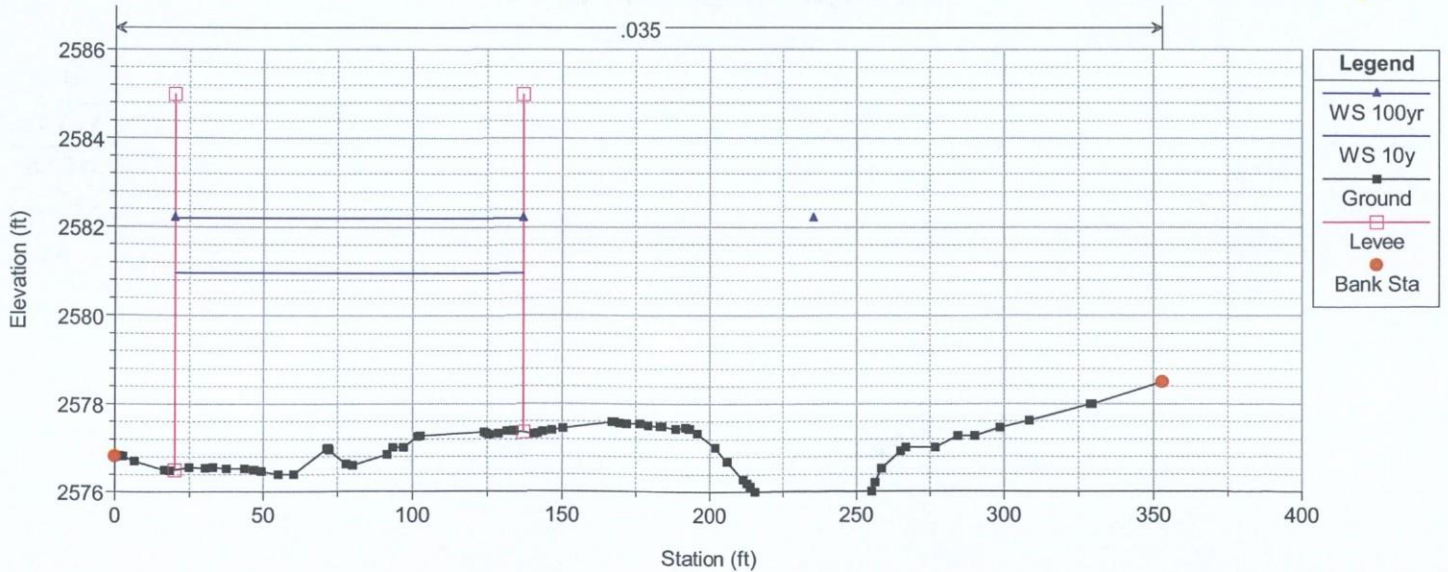
Greasewood Plan: Greasewood_PR 6/2/2014
 River = Stream2 Reach = Reach1 RS = 471



Greasewood Plan: Greasewood_PR 6/2/2014
 River = Stream2 Reach = Reach1 RS = 321

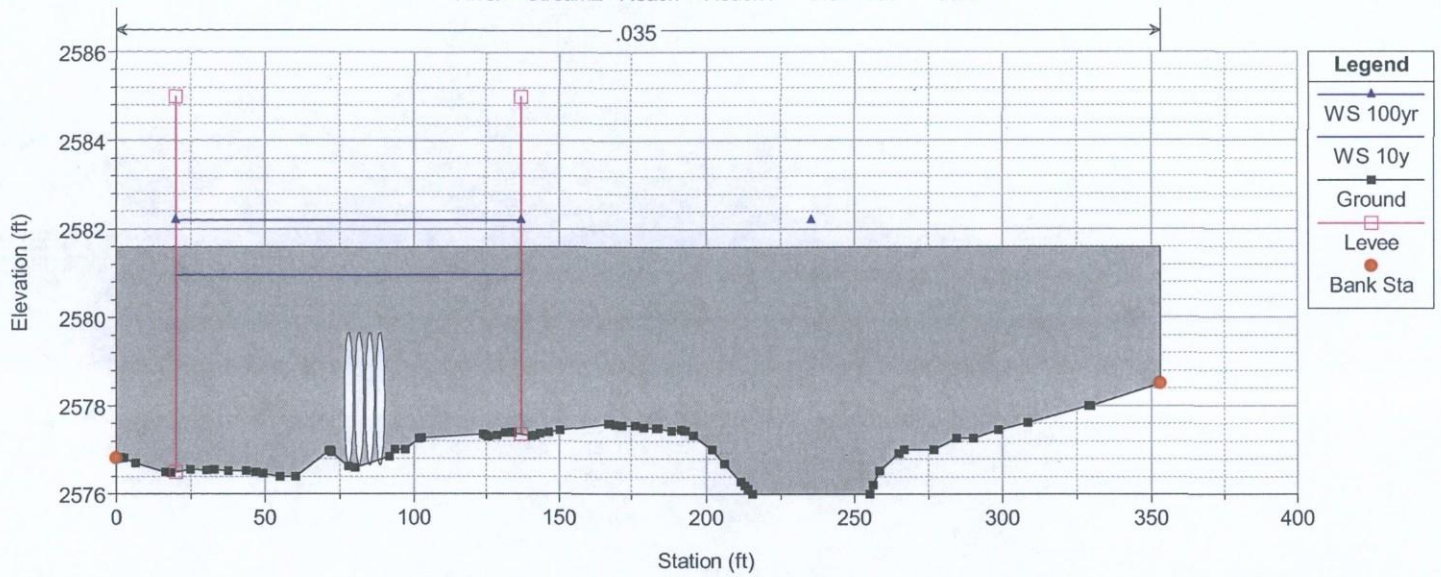


Greasewood Plan: Greasewood_PR 6/2/2014
 River = Stream2 Reach = Reach1 RS = 226



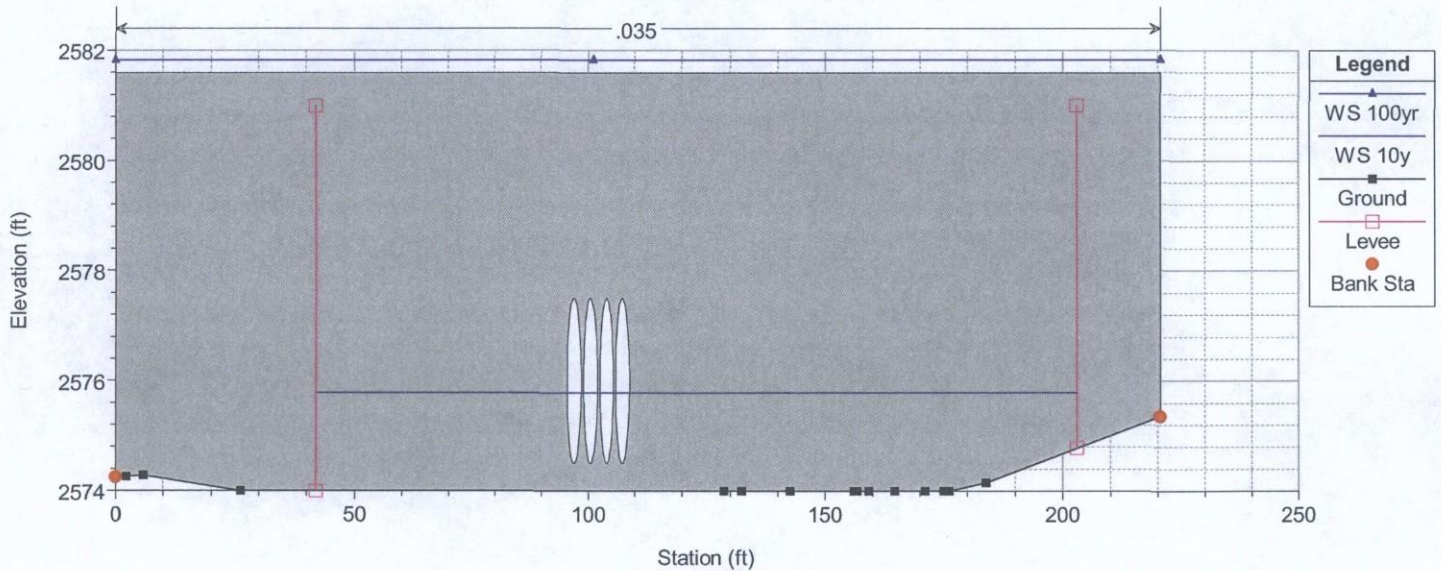
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream2 Reach = Reach1 RS = 165 Culv



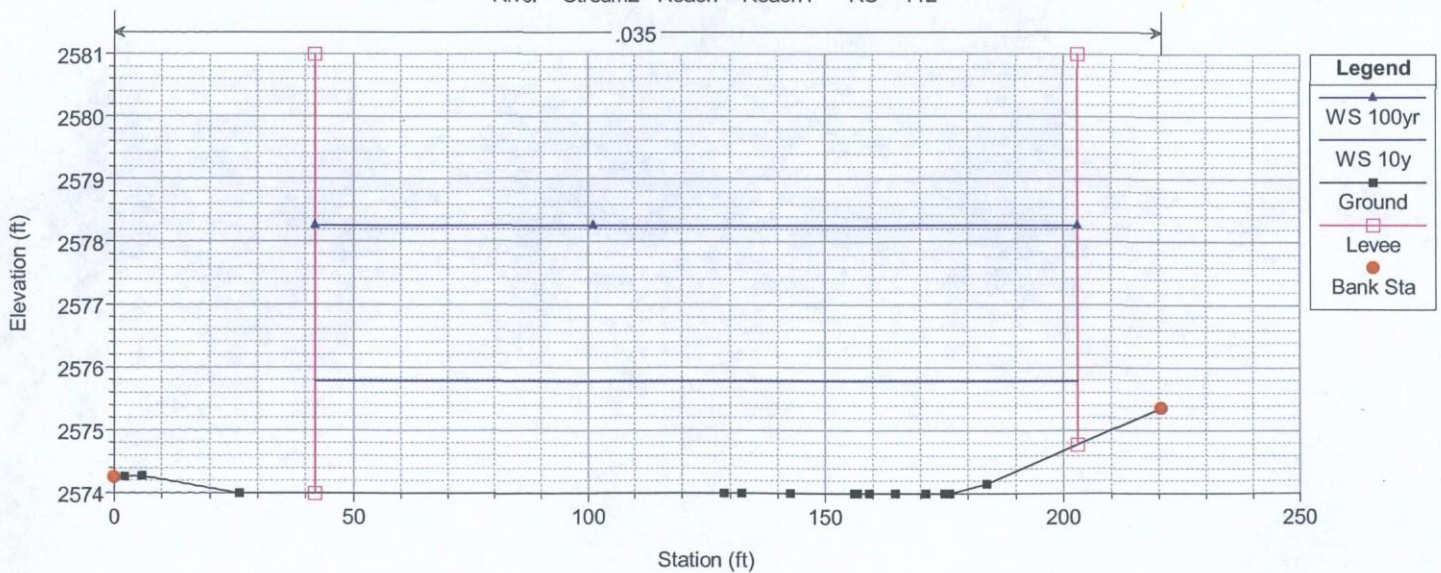
Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream2 Reach = Reach1 RS = 165 Culv



Greasewood Plan: Greasewood_PR 6/2/2014

River = Stream2 Reach = Reach1 RS = 112



HY-8 Culvert Analysis Report



Table 1 - Summary of Culvert Flows at Crossing: Off-site Crossing - 36" pipe

Headwater Elevation (ft)	Total Discharge (cfs)	Off-site Crossing - 36" pipe Discharge (cfs)	Roadway Discharge (cfs)	Iterations
2534.50	0.00	0.00	0.00	1
2536.01	11.90	11.90	0.00	1
2536.82	23.80	23.80	0.00	1
2537.52	35.70	35.70	0.00	1
2538.31	47.60	47.60	0.00	1
2538.89	55.00	55.00	0.00	1
2539.11	71.40	57.55	13.71	8
2539.17	83.30	58.16	24.85	4
2539.22	95.20	58.70	36.37	4
2539.26	107.10	59.17	47.88	4
2539.30	119.00	59.59	59.22	3
2539.00	56.25	56.25	0.00	Overtopping

Rating Curve Plot for Crossing: Off-site Crossing - 36" pipe

Total Rating Curve
Crossing: Off-site Crossing - 36" pipe

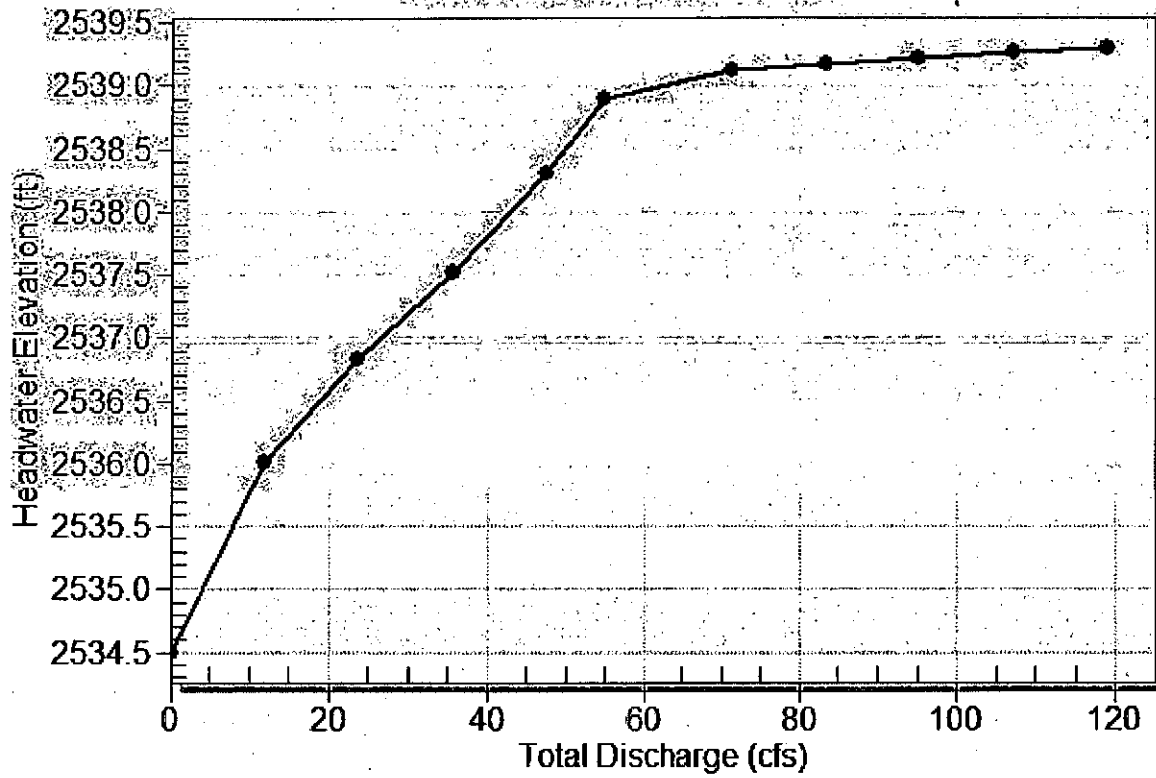


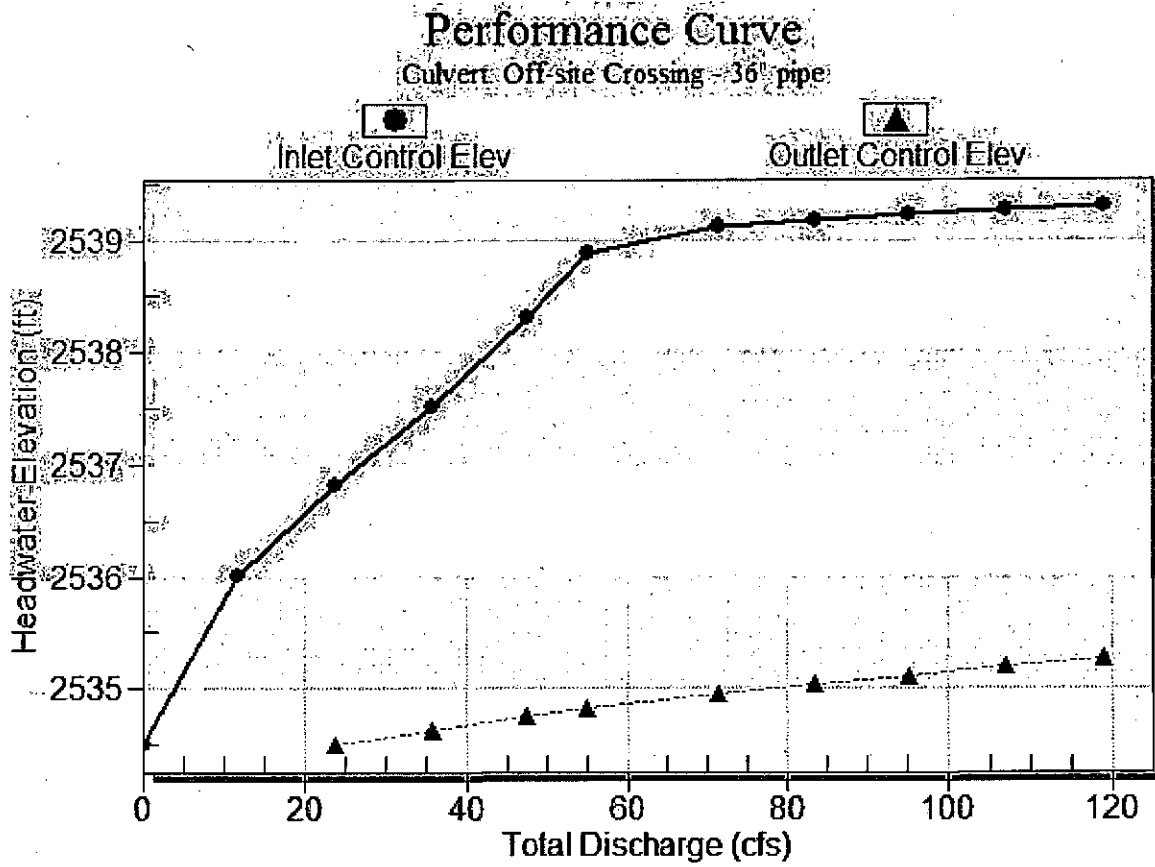
Table 2 - Culvert Summary Table: Off-site Crossing - 36" pipe

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.00	0.00	2534.50	0.000	0.0*	0-NF	0.000	0.000	0.000	0.000	0.000	0.000
11.90	11.90	2536.01	1.507	0.0*	1-S2n	0.835	1.086	0.836	0.332	7.357	3.357
23.80	23.80	2536.82	2.324	0.001	1-S2n	1.214	1.567	1.277	0.501	8.292	4.319
35.70	35.70	2537.52	3.023	0.136	5-S2n	1.530	1.936	1.618	0.636	9.190	4.983
47.60	47.60	2538.31	3.807	0.252	5-S2n	1.832	2.238	1.930	0.752	9.918	5.503
55.00	55.00	2538.89	4.392	0.318	5-S2n	2.027	2.405	2.115	0.818	10.322	5.779
71.40	57.55	2539.11	4.614	0.451	5-S2n	2.093	2.446	2.193	0.951	10.414	6.307
83.30	58.16	2539.17	4.670	0.539	5-S2n	2.111	2.456	2.203	1.039	10.476	6.635
95.20	58.70	2539.22	4.718	0.622	5-S2n	2.128	2.465	2.218	1.122	10.501	6.929
107.10	59.17	2539.26	4.761	0.700	5-S2n	2.142	2.472	2.229	1.200	10.529	7.198
119.00	59.59	2539.30	4.801	0.774	5-S2n	2.156	2.479	2.241	1.274	10.548	7.444

* theoretical depth is impractical. Depth reported is corrected.

Inlet Elevation (invert): 2534.50 ft, Outlet Elevation (invert): 2534.00 ft
Culvert Length: 55.00 ft, Culvert Slope: 0.0091

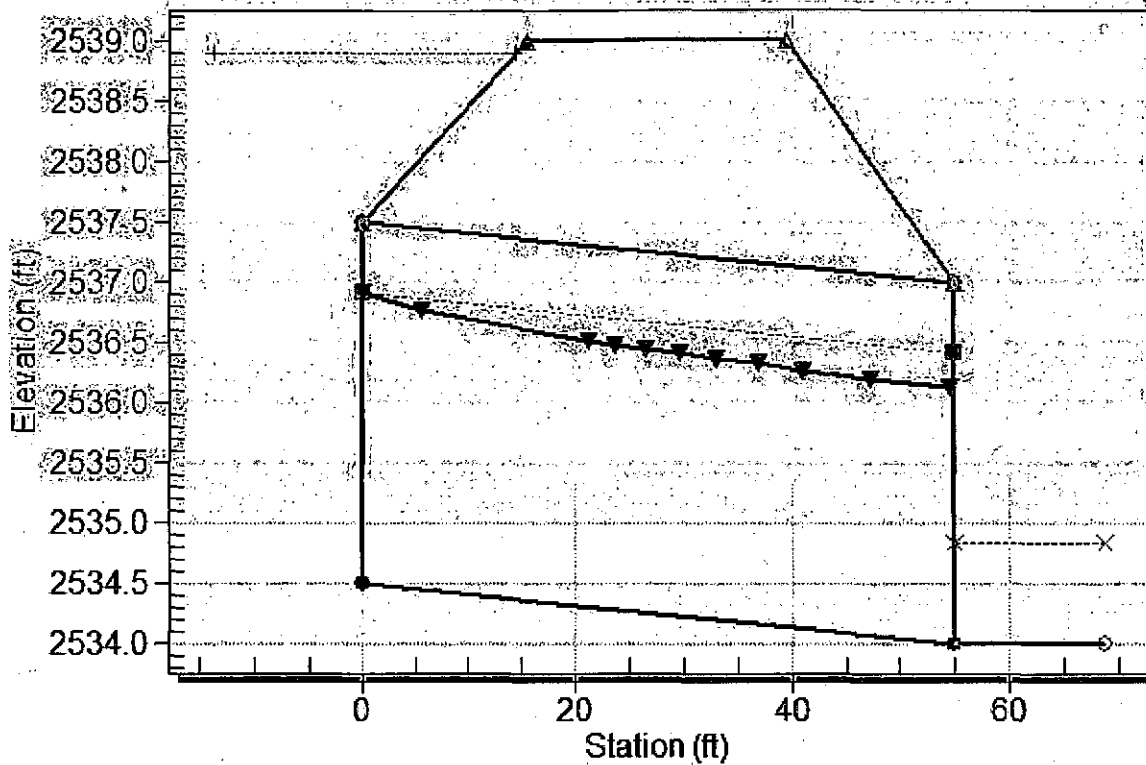
Culvert Performance Curve Plot: Off-site Crossing - 36" pipe



Water Surface Profile Plot for Culvert: Off-site Crossing - 36" pipe

Crossing - Off-site Crossing - 36" pipe, Design Discharge - 55.0 cfs

Culvert - Off-site Crossing - 36" pipe, Culvert Discharge - 55.0 cfs



Site Data - Off-site Crossing - 36" pipe

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 2534.50 ft

Outlet Station: 55.00 ft

Outlet Elevation: 2534.00 ft

Number of Barrels: 1

Culvert Data Summary - Off-site Crossing - 36" pipe

Barrel Shape: Circular

Barrel Diameter: 3.00 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0120

Inlet Type: Conventional

Inlet Edge Condition: Square Edge with Headwall

Inlet Depression: NONE

Table 3 - Downstream Channel Rating Curve (Crossing: Off-site Crossing - 36" pipe)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
0.00	2534.00	0.00	0.00	0.00	0.00
11.90	2534.33	0.33	3.36	0.62	1.06
23.80	2534.50	0.50	4.32	0.94	1.12
35.70	2534.64	0.64	4.98	1.19	1.16
47.60	2534.75	0.75	5.50	1.41	1.19
55.00	2534.82	0.82	5.78	1.53	1.20
71.40	2534.95	0.95	6.31	1.78	1.23
83.30	2535.04	1.04	6.64	1.95	1.24
95.20	2535.12	1.12	6.93	2.10	1.25
107.10	2535.20	1.20	7.20	2.25	1.27
119.00	2535.27	1.27	7.44	2.38	1.27

Tailwater Channel Data - Off-site Crossing - 36" pipe

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 10.00 ft

Side Slope (H:V): 2.00 (2:1)

Channel Slope: 0.0300

Channel Manning's n: 0.0350

Channel Invert Elevation: 2534.00 ft

Roadway Data for Crossing: Off-site Crossing - 36" pipe

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 120.00 ft

Crest Elevation: 2539.00 ft

Roadway Surface: Paved

Roadway Top Width: 24.00 ft

Project: **Cavalliere Flats**

Subject: **Erosion Setback Calculation**

Designed by: **MAW**

Date: 6/2/2014

Checked by:

Date:

Setback calculated according to *State Standard for Watercourse System System Sediment Balance (SSA 5-96)*,
Level 1 Analysis for straight channel reaches or reaches with minor curvature

$$\text{Setback} = 1 \times (100\text{-year Peak Discharge})^{0.5}$$

Calculate Erosion Setbacks

Wash ID	Peak Discharge ¹ (cfs)	Setback (ft)
North Wash, upstream of confluence	463	22
North Wash, downstream of confluence	973	31
East Wash	484	22

Notes:

¹ Proposed conditions peak discharge used for calculation

Appendix E- STORMWATER STORAGE WAIVER



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

___ - PA - ___ ___ - ZN - ___ ___ - UP - ___ ___ - DR - ___ ___ - PP - ___ PC# ___

The applicant/developer must complete and submit this form to the city for processing and obtain approval of waiver request **before submitting improvement plans**. Denial of the waiver may require the developer to submit a revised site plan to the Development Review Board.

Date 8/28/14 Project Name Cavalliere Flats

Project Location South of Pinnacle Vista Drive and east of Alma School Parkway

Applicant Contact Jason Burm Company Name Kimley-Horn and Associates, Inc.

Phone _____ Fax _____ E-mail Jason.Burm@kimley-horn.com

Address 7740 N. 16th Street, Suite 300, Phoenix, AZ 85020

Waiver Criteria

A project must meet at least one of three criteria listed below for the city to consider waiving some or all required stormwater storage. **However, regardless of the criteria, a waiver will only be granted if the applicant can demonstrate that the effect of a waiver will not increase the potential for flooding on any property.** Check the applicable box and provide a signed engineering report and supporting engineering analysis that demonstrate the project meets the criteria and that the effect of a waiver will not increase the potential for flooding on any property.

If the runoff for the project has been included in a storage facility at another location, the applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility.

- 1. The development is adjacent to a conveyance facility that an engineering analysis shows is designed and constructed to handle the additional runoff from the site as a result of development.
- 2. The development is on a parcel less than one-half acre in size.
- 3. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO).

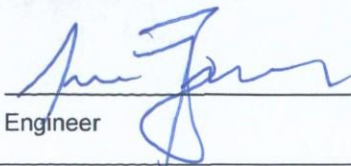
For a full storage waiver, a conflict with ESLO is limited to:

- Property located in the hillside landform as defined in the city Zoning Ordinance
- Property in the upper desert landform that has a land slope steeper than 5% as defined in the city Zoning Ordinance
- Property within the ESL zoning overlay district where the only viable location for a stormwater storage basin requires blasting

This full waiver only applies to those portions of property meeting one of these three requirements.

Partial waivers are available for projects or portions of properties within the Environmentally Sensitive Lands Zoning Overlay District, not meeting any of the three full waiver criteria above, if post-development peak discharge rates do not exceed pre-development conditions, based on the 10- and 100-year storm events.

By signing below, I certify that the stated project meets the waiver criteria selected above as demonstrated by the attached documentation.



Engineer

8.28.14

Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

____ - PA - ____ - ZN - ____ - UP - ____ - DR - ____ - PP - ____ PC# _____

CITY STAFF TO COMPLETE THIS PAGE

Project Name Cavalliere Ranch

Check Appropriate Boxes:

Meets waiver criteria (specify): 1 2 3

Recommend approve waiver.

Recommend deny waiver:

None of waiver criteria met.

Downstream conditions prohibit waiver of any storage.

Other:

Explain: _____

Return waiver request:

Insufficient data provided.

Other: _____

Explain: _____

Recommended Conditions of Waiver:

All storage requirements waived.

Post-development peak discharge rates do not exceed pre-development conditions.

Other:

Explain: _____

Waiver approved per above conditions.

Waiver denied.

Floodplain Administrator or Designee

Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA -

- ZN -

- UP -

- DR -

- PP -

PC#

In-Lieu Fee and In-Kind Contributions

In-lieu fees are only applicable to projects where post-development peak discharge rates exceed pre-development levels, based on the 10- and 100-year storm events. If the city grants a waiver, the developer is required to calculate and contribute an in-lieu fee based on what it would cost the city to provide a storage basin, sized as described below, including costs such as land acquisition, construction, landscaping, design, construction management, and maintenance over a 75-year design life. The fee for this cost is \$1.87 per cubic foot of stormwater storage for a virtual storage basin designed to mitigate the increase in runoff associated with the 100-year/2-hour storm event. The applicant may submit site-specific in-lieu fee calculations subject to the Floodplain Administrator's approval.

The Floodplain Administrator considers in-kind contributions on a case-by-case basis. An in-kind contribution can serve as part of or instead of the calculated in-lieu fee. In-kind contributions must be stormwater related and must constitute a public benefit. In-lieu fees and in-kind contributions are subject to the approval of the Floodplain Administrator or designee.

Project Name Cavalliere Ranch

The waived stormwater storage volume is calculated using a simplified approach as follows:

V = ΔCRA; where

V = stormwater storage volume required, in cubic feet,

ΔC = increase in weighted average runoff coefficient over disturbed area ($C_{post} - C_{pre}$),

R = 100-year/2-hour precipitation depth, in feet (DSPM, Appendix 4-1D, page 11), and

A = area of disturbed ground, in square feet

Furthermore,

R = _____

ΔC = _____

$V_w = V - V_p$; where

A = _____

V_w = volume waived,

V = _____

V = volume required, and

V_p = _____

V_p = volume provided

V_w = _____

An in-lieu fee will be paid, based on the following calculations and supporting documentation:

In-lieu fee (\$) = V_w (cu. ft.) x \$1.87 per cubic foot = _____

An in-kind contribution will be made, as follows:

No in-lieu fee is required. Reason:

Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO).

Approved by:

Floodplain Administrator or Designee

Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781





Archaeological Resources Certificate of No Effect Application

RE: PROTECTION OF ARCHAEOLOGICAL RESOURCES ORDINANCE

Parcel Number(s): 216-80-007G Acres: approx. 5 Quarter Section(s): 58-45

Street Address or Intersection: NEC of the intersection of Scottsdale Road and Westland Drive in Scottsdale, Arizona

Applicant Name: Taylor Morrison/Arizona, Inc.

Applicant Address: 9000 E. Pima Center Parkway, Suite 350, Scottsdale, Arizona 85258

Telephone Number: (480) 344-7000 E-Mail: jdeason@taylormorrison.com

Owner Name: Taylor Morrison/Arizona, Inc.

Owner Address: 9000 E. Pima Center Parkway, Suite 350, Scottsdale, Arizona 85258

Telephone Number: (480) 344-7000 E-Mail: jdeason@taylormorrison.com

Notes:

Development Project Application

- | | | |
|--|--|--|
| <input type="checkbox"/> Rezoning | <input type="checkbox"/> Use Permit | <input type="checkbox"/> General Plan |
| <input checked="" type="checkbox"/> Preliminary Plat | <input type="checkbox"/> Final Plat | <input type="checkbox"/> Master Plan |
| <input type="checkbox"/> Development Rev Board | <input type="checkbox"/> Lot Split | <input type="checkbox"/> Building Permit |
| <input type="checkbox"/> Grading Permit | <input type="checkbox"/> Infrastructure Imp. | <input type="checkbox"/> Native Plant |

Archaeological Survey & Report

Class III Archaeological Survey and Report Submitted Date: June 2014

Name of Report: A CLASS I RECORDS SEARCH OF THE GREASEWOOD FLAT PROPERTY AND A CLASS III ARCHAEOLOGICAL SURVEY OF APPROXIMATELY 5 ACRES (PARCEL NO. 216-80-007G), SCOTTSDALE, MARICOPA COUNTY, ARIZONA

Archaeologist and Firm: Alan L. Bartholomew, SWCA Environmental Consultants

Applicants/Archaeologists Stop Here

Scottsdale Historic Preservation Program

7506 E Indian School Rd, Scottsdale, AZ 85251 • Phone: 480-312-2523 • Fax: 480-312-5011

THIS SIDE OF FORM TO BE COMPLETED BY CITY

Date report sent to on-call archaeology firm for review: _____

Name of firm reviewing report: _____

Date review comments received: _____

CERTIFICATE OF NO EFFECT

Denied, report incomplete and does not meet city and state standards (see attached explanation)

Approved, in accordance with the following findings:

_____ No archaeological resources are located on the property.

_____ No significant archaeological resources are impacted.

_____ Significant archaeological resources are protected.

_____ Type of permanent protection provided

_____ Documentation of permanent protection provided and approved

Denied, Certificate of Approval Application required (see attached explanation)

City Archaeologist Signature: _____ Date: _____

NOTICE:

Date a copy of this signed application was handed or e-mailed to the owner and/or the applicant by the City Archaeologist after approval or denial: _____

APPEAL

Certificate of No Effect
Appeal Date:
HPC Hearing:
HPC Decision:
CC Hearing:
CC Decision:

Scottsdale Historic Preservation Program

7506 E Indian School Rd, Scottsdale, AZ 85251 • Phone: 480-312-2523 • Fax: 480-312-7314

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ENVIRONMENTAL CONSULTANTS

Sound Science. Creative Solutions.[®]

**A Class I Records Search of the
Greasewood Flat Property and a
Class III Archaeological Survey of
Approximately 5 Acres (Parcel
No. 216-80-007G), Scottsdale,
Maricopa County, Arizona**

Prepared for

Kimley-Horn and Associates

Prepared by

SWCA Environmental Consultants

June 2014

 **FILE COPY**

**12-ZN-2014
6/11/2014**

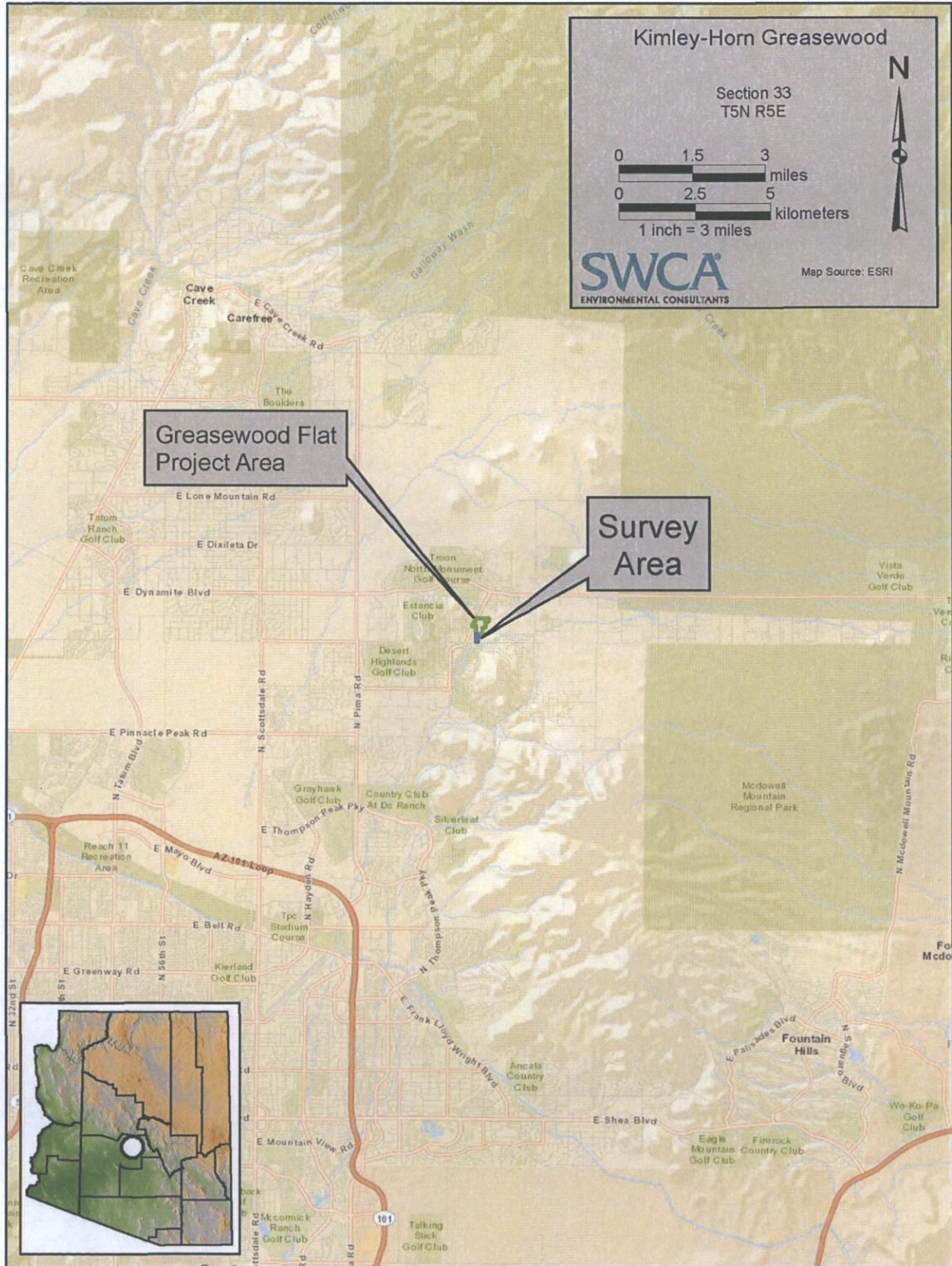


Figure 1. General project area location.

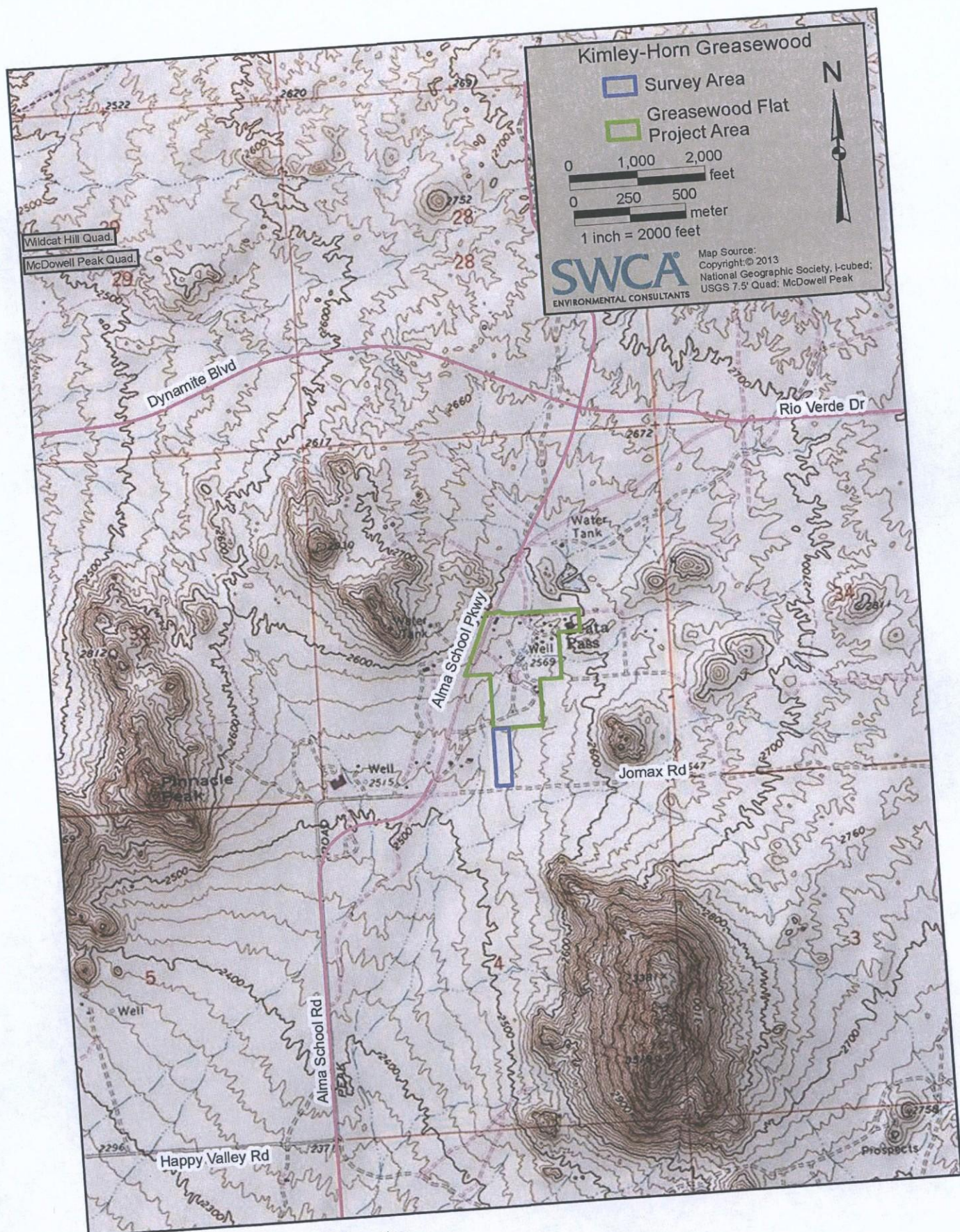


Figure 2. Specific project area location.



Figure 3. Project area overview, view facing northwest.

Hohokam cultural tradition is assigned to the Red Mountain phase (A.D. 1–500) of the Pioneer period (A.D. 1–750) (Cable and Doyel 1987; Dean 1991; Hackbarth 1992, 2001; Morris 1969). Evidence from Red Mountain phase sites indicates that people subsisted on a mix of wild resources and agricultural products. Corn was the dominant crop, along with beans, squash, and cotton. The first evidence for canal irrigation along the Salt River is attributed to this time, A.D. 400 (Ackerly and Henderson 1989). Identified house forms include small circular and “bean-shaped” pit houses (Mabry 2000).

The period between A.D. 500 and 650 is defined as the Vahki phase. It appears that by this time, irrigation had become well established. Vahki phase canals have been identified at Snaketown (Haury 1976), as well as along the edges of the Salt River floodplain (Ackerly and Henderson 1989). Domestic architecture of the Vahki phase consisted of square and rectangular pit houses of various sizes (Ciolek-Torrello et al. 2000; Crary and Craig 2001).

The Late Pioneer period (A.D. 650–750) saw the appearance of decorated pottery in southern Arizona. Hohokam decorated pottery had red painted designs on a light-colored buff or brown background (Abbott 2001; Haury 1976). The earliest decorated pottery types include Estrella, Sweetwater, and Snaketown Red-on-buff (Wallace 2001). House types associated with the Late Pioneer period vary greatly. Small, domed field houses made from bent poles and covered with brush served as temporary shelters in agricultural fields or at resource procurement and processing sites. Few artifacts are associated with the remains of these structures. Late Pioneer period habitation sites, on the other hand, contain moderately sized pit structures with square or rectangular floor plans and formal, plastered hearths. These are far more substantial than the field houses and were occupied for extended periods.

Late Pioneer period subsistence was based on a mixture of wild resources and agricultural produce. The use of irrigation expanded from the floodplains to include lands on terraces above rivers (Ackerly and Henderson 1989).

The Gila Butte and Santa Cruz phases make up the Colonial period (A.D. 750–950). This was a time of expansion and elaboration of the Hohokam culture, where the number and distribution of sites across the landscape increased considerably. Colonial period Hohokam artifacts have been found as far north as Prescott in north-central Arizona and as far south as northern Mexico, extending to the west of Gila Bend in southwestern Arizona and into New Mexico to the east (Haury 1976). Abbott (1994, 2001) argues that the center for most of the decorated buff ware vessels produced during this time was the middle Gila River valley. Not only did the Hohokam expand their territory, they also intensified contact with their neighbors. Intrusive ceramics from the north, east, and west have been found at Hohokam sites dating to this time. It has been argued that Colonial period Hohokam social organization was tied to the exchange of ritual and subsistence goods (Doyel 1985). Across Arizona, networks of interaction spheres dominated the social landscape and facilitated exchange across the region. It was during this time that the Hohokam achieved their highest level of arts and crafts production. Ceramics and shell jewelry dating to this period are well made and elaborately decorated.

The large, square communal structures found in earlier times ceased to be built during the Colonial period. Instead, ball courts, which were probably first built in the early A.D. 800s, became the dominant form of public architecture (Wallace 2001). Their appearance in southern Arizona has been thought to mark the emergence of a regional system with religious, economic, and political links that crosscut geographical boundaries (Abbott 2001; Wilcox and Shenk 1977). Subsistence was based on a mixture of wild resources and agricultural crops. Some wild species (e.g., little barley [*Hordeum pusillum*]) were so intensively exploited they became as important as some domesticated species (Bohrer 1987). The use of irrigation expanded significantly throughout the Salt and Gila River valleys, and the construction and maintenance of canals had a significant impact on Hohokam social and political organization (e.g., Abbott 2000; Hunt et al. 2005).

With the onset of the Sedentary period (Sacaton phase, A.D. 950–1150), there was a decline in the quality of Hohokam material culture, especially in the production of ceramics and shell ornaments. Ball courts were still the dominant form of public architecture during the Early Sedentary period; however, by its end, few were being built. As the construction of ball courts diminished, the construction of capped mounds or platform mounds became more common. Platform mounds were built near village centers around plazas that were surrounded by domestic features. House types exhibited significant variability and were aggregated within courtyard groups or village segments (Wilcox et al. 1981).

Subsistence continued to be based on agriculture, although there was some emphasis on collecting certain wild plant species, such as cholla. Cotton was also of major importance: its fiber was used for weaving textiles and its seeds were eaten.

By the end of the Sedentary period, a major reorganization of Hohokam society had occurred. After a period of intensive growth and expansion, many villages and areas were abandoned. Populations began aggregating in larger villages along the Salt River. These changes in the social and political environment were reflected in changes in public architecture and ceramic and shell production.

The Sedentary period was followed by the Classic period, which is divided into the Soho (A.D. 1150–1300) and the Civano phases (A.D. 1300–1450). Differences in ceramic decoration and architectural styles separate these two phases. Red-on-buff ceramics continued to be produced during the Soho phase, although they occurred with lower frequency. Red wares became increasingly common during the Civano phase, and the introduction of long-necked jars marks a clear contrast with earlier ceramic styles.

Structures with post-reinforced adobe walls and surface structures were common during the Soho phase. However, during the Civano phase, adobe compounds—often containing small plazas—and adobe structures were built and used to the near exclusion of semi-subterranean structures. Puddled and coursed

adobe construction generally replaced the use of structures with pole-reinforced walls, and the number and proximity of rooms within compounds increased.

Public architecture also underwent a change during the Early Classic period. There was a significant increase in the construction and use of platform mounds (Gregory et al. 1988). At the same time, the construction of ball courts declined to its lowest point. The apex of Hohokam public architecture was achieved during the Civano phase with the building of "big houses." The only remaining example of a big house is found today at the Casa Grande Ruins on the outskirts of Coolidge. These structures probably served multiple functions. It has been argued that they were clear symbols of elite status within Hohokam society (Wilcox and Shenk 1977). Big houses often co-occur with platform mounds, with a central plaza separating the two. The introduction of big houses is as mysterious as their disappearance. The construction and use of big houses may have been the result of changes within Hohokam society, and their abandonment may have been tied to attacks from outsiders (e.g., Teague 1989).

Increase in red wares and the disappearance of buff wares mark the Civano phase, although plain wares continue to dominate the total ceramic assemblage. Polychrome pottery (in particular, Gila and Tonto polychromes) and local imitations were present after A.D. 1320 (Reid and Whittlesey 1992).

Canal irrigation was still very important during the Civano phase. Civano phase Hohokam depended greatly on corn, beans, and squash as the mainstays of their diet, although agave and cholla were also significant dietary components. Corn was certainly the most common domesticate, and the abundance of agave at many sites indicates that it, too, was extremely important. As evidenced at some sites, during the Late Classic period, the use of agave became increasingly important and the availability of agricultural produce declined (e.g., Miller 1994).

Hohokam social organization during the Civano phase was clearly different from what preceded and followed it. Population size and density at many of the large sites in the Salt River valley reached never-before-seen levels. Although the level of social and political organization actually achieved by the Hohokam has been much debated, some increase in social complexity was undoubtedly necessary to manage the higher population densities that developed. This greater social complexity may have been reflected in the construction and use of platform mounds and big houses.

The post-Classic period (A.D. 1450–1540) in the Phoenix Basin, referred to by some as the Polvorón phase, constitutes a somewhat hazy gap in occupation between the Late Classic period Hohokam and the first Europeans to arrive in the area (e.g., Bayman 2001; Chenault 2000; Henderson and Hackbarth 2000). Nevertheless, the traits used to identify the Polvorón phase include jacal structures, polychrome ceramics, and an abundance of obsidian. Many have argued that these characteristics are not sufficient to distinguish the Polvorón phase from the late Civano phase. Additionally, available chronological dates make it difficult to distinguish between Civano and Polvorón phase sites (Dean 1991:87).

By the late Civano phase, the success enjoyed by the Hohokam had vanished. High population densities, a decline in agricultural productivity, the failure of many irrigation systems, and the depletion of food resources, along with the presence of disease, malnutrition, flooding, and drought, are cited as reasons for the collapse of the Hohokam (e.g., Bayman 2001; Van Gerven and Sheridan 1994). Nevertheless, Bayman (2001) points out that the Hohokam may have continued to occupy the area until the early 1500s and that the debate over the cause or causes for the decline and disappearance of the Hohokam is far from resolved. Some have even argued that Hohokam and Salado may have directly encountered the Spanish (Reff 1992).

The ceramic period in the Cave Creek area can be divided into two basic time periods: the Preclassic period (A.D. 700–1150) and Classic period (A.D. 1150–1425). Preclassic period sites consist of small, dispersed communities that include Hohokam-style pit houses or semi-subterranean structures with a

masonry foundation and pole-and-brush superstructure. These Preclassic period sites are not unlike contemporaneous sites in other areas that are often considered peripheral to the Hohokam heartland in the Phoenix Basin. Many sites dating to this period appear to be composed of one or a few extended families which probably formed a primary economic unit.

Community-level integrative architecture is absent in the Cave Creek area, although a few ball courts have been documented slightly to the south in the bajada of the McDowell Mountains (e.g., Wilcox and Sternberg 1983:104). The general absence of public architecture, combined with the generally small size of settlements, may indicate a lower degree of social integration across the vast territory attributed to the Central Arizona Tradition, particularly when compared to the Phoenix Basin or Colorado Plateau.

A mixed farmer-forager subsistence economy was employed during the Preclassic period. Detailed studies of botanical remains from Preclassic period sites in the foothills bordering the Phoenix Basin suggest that maize, mesquite, and cacti were staple foods (Smith 2002:119). Given the abundance of wild plant and animal resources and minimal opportunity for irrigation agriculture in many areas, domesticates were probably less important here than in the broad desert valleys of southern Arizona.

Preclassic period sites in the Cave Creek area typically contain a handful of Hohokam red-on-buff sherds. A few Tusayan White Ware, Little Colorado White Ware, or San Juan Red Ware sherds are also occasionally present. A local decorated ceramic tradition does not appear to be present, and locally produced plain wares usually account for more than 90 percent of a site's ceramic assemblage. Likewise, exotic artifacts such as shell and turquoise are relatively rare at these sites.

During the Classic period, numerous villages consisting of one or two nucleated room blocks were present throughout central Arizona. There is a trend toward increasing aggregation through time, particularly after A.D. 1275. Early Classic period sites (A.D. 1100–1275) commonly contain fewer than 20 rooms. Prominent examples near Cave Creek include Sears Kay Ruin and Brazaletes Pueblo, the latter of which contains approximately 100 rooms (Valehrach and Valehrach 1971). Two structure types are present in Early Classic period sites and sometimes occur at the same site in contexts that appear to be contemporaneous. The first type consists of detached rooms or small room blocks with low masonry footings and ephemeral superstructures. Larger nucleated room blocks with full-height masonry walls are the second type. Although Preclassic period sites do not appear to be located with obvious concerns for defense, Classic period sites are commonly located on prominent land forms with commanding views of surrounding topography. In fact, some sites are located on pinnacles or knolls that are so difficult to access that it is hard to imagine a reason other than defense for their location.

Similar to the Preclassic period, examples of public architecture are rare or absent at Classic period sites. Exceptions include large "community" rooms (measuring approximately 30 square meters [323 square feet]), plazas, and courtyard areas. At the larger sites, several lineages may have co-resided. This aggregation of people implies that more complex forms of social organization developed during the Classic period.

Subsistence strategies continued to employ a mixed farmer-forager economy. Despite the larger size of some communities, more intensive forms of food production are not evident during the Classic period. Rather, it appears that larger Classic period communities were placed in areas that offered abundant agricultural land capable of supporting a sizable community. Like earlier times, *ak chin*, or floodwater agricultural strategies, were widespread and irrigation agriculture may have been practiced along Cave Creek.

Plain ware and red ware ceramic vessels constructed from local clay and temper sources dominate Classic period ceramics assemblages. However, Puebloan black-on-white and orange ware sherds are also present in small quantities.

During the late Classic period (A.D. 1275–1425), the Cave Creek area was abandoned along with the New River area and the Lower Verde River south of Horseshoe Basin. At this time, some residents of the Cave Creek area may have migrated back to the Salt River Valley to join the large village sites such as Pueblo Grande and Los Colinas. Others may have moved north to the large aggregated villages that appeared on Perry Mesa, in Bloody Basin, and throughout the Lower Verde River north of Horseshoe Dam. Regardless of where the prehistoric occupants migrated, the Cave Creek area was heavily depopulated or totally abandoned by A.D. 1300. Environmental factors, social conditions, or both have traditionally been invoked as causal factors in the regional abandonment during the 1400s. Wilcox et al. (2001) recently proposed that this buffer zone was necessary to help minimize conflict between the large sites farther north in the Transition Zone and the large Hohokam village sites to the south along the Salt River.

Following the collapse of the Hohokam regional system, Akimel O'odham (Pima) and Tohono O'odham (Papago) groups lived in the middle Gila River valley. For unknown reasons, the Salt River valley was either used sparingly or was abandoned following the Hohokam collapse. Akimel O'odham and Tohono O'odham groups lived in small rancherías, subsisting on agricultural products, wild plant foods, and game. The Pee Posh (Maricopa), who were migrants from the Gulf of California area, formed an alliance with the Pima in the early 1800s and have lived in the Salt-Gila Basin ever since. All these groups continue to occupy the area, living on several reservations.

During the protohistoric and historic periods, Northeastern Yavapai groups used the Cave Creek area on a seasonal basis. An occupational gap of three or more centuries likely characterized the period between the prehistoric abandonment and the appearance of the Yavapai in this area. Yavapai bands visited the Cave Creek area during the historic period primarily to collect saguaro fruit and palo verde seeds (Gifford 1936:255). Very little is known about Yavapai subsistence strategies prior to Euro-American colonization of their territory and accompanying encounters with the U.S. military ca. 1860 to 1890.

Historic Period – Phoenix/Scottsdale Area

The Spanish were the first Europeans to enter what is now Arizona. Most of their early expeditions followed either the Santa Cruz or San Pedro Rivers up to the Gila River before turning westward. In 1826, Sylvester Pattie and James Ohio Pattie were the first Euro-Americans to pass through the Phoenix Basin. On February 2, 1848, the Treaty of Guadalupe Hidalgo was signed. Its provisions called for Mexico to cede 55 percent of its territory (present-day Arizona, California, New Mexico, Texas, and parts of Colorado, Nevada, and Utah) in exchange for \$15 million in compensation. Thus, in 1848, the United States acquired most of what is now Arizona, and the rest was bought with the Gadsden Purchase in 1854. In the mid-1800s, numerous Euro-American explorers and surveyors crossed Arizona, but again, the Phoenix Basin was bypassed (Cross et al. 1960; Trimble 1977; Wagoner 1989; Walker and Bufkin 1979).

The early development and growth of central Arizona during the late 1800s and early 1900s was a direct response to national economic stimuli. The discovery of gold in the Bradshaw Mountains in 1863 drew miners, the military, ranchers, and entrepreneurs to the region (Mawn 1977; Zarbin 1978). In 1899, Phoenix became the permanent capital, and in 1912, Arizona became a state (Luckingham 1989).

The genesis of modern Phoenix lies with Jack Swilling of Wickenburg. After organizing the Swilling Irrigation Canal Company, he moved to the valley in 1867 and began digging a canal to divert water from the Salt River (Zarbin 1997). Most canals in the Phoenix Basin were constructed and operated by companies. Approximately 15 major canals appeared by 1888, with more than 400,000 acres under cultivation. Components of these systems were acquired by the federal government beginning in 1902, and under a 1917 agreement, the system is now managed by the Salt River Project.

By 1868, a small colony had arisen about 4 miles east of where Phoenix is centered today. On May 4, 1868, Phoenix was officially recognized when it became an election precinct within Yavapai County. A post office was established on June 15 of the same year, with Jack Swilling as the postmaster. In the early 1870s, the town center officially shifted to the area around present-day Washington Street and Central Avenue. By 1880, Phoenix had a population of approximately 1,700 and Mesa about 1,000. Hayden's Ferry, which later became known as Tempe, was emerging as a farming and trading center. The entire area developed into an extensive farmland of citrus orchards and cotton and lettuce fields. Territorial Governor John C. Fremont signed a bill incorporating the City of Phoenix on February 25, 1881.

The first transcontinental railroad (Southern Pacific) ran south of Phoenix. However, goods were transported to the Phoenix area via freight teams. It was not until July 1887 that the railroad arrived in Phoenix. In 1895, a second railroad linked Phoenix with the northern transcontinental railroad (Atlantic-Pacific). These railroads greatly reduced the costs of goods coming into the area. Construction of flood controls on Cave Creek prompted further development of the western valley in the early 1900s (Hackbarth 1995). During the 1870s and 1880s, demand for Arizona's gold and silver grew, as did the demand for the valley's agricultural produce (Morrow 1943).

In the 1880s, Phoenix began to take on the aura of a city as sewer and water systems were started and roadways expanded. In the late 1800s and early 1900s, national demand for Arizona's resources once again stimulated economic growth in the area (Morrow 1943). The construction of Roosevelt Dam in the early 1900s resulted in a boom in agriculture in the Phoenix Basin (Luckingham 1989), and the Enlarged Homestead Act of 1909 encouraged settlement of the arid lands around Phoenix. The area also benefited from the demand of mineral and agricultural resources during World War I. The national economic downturn in 1919-1920 and the Great Depression (1929-1939) slowed economic development in the Phoenix area. The regional economy grew, although mutedly, during World War II. With the end of the war, the area's population and economy once again rebounded (Luckingham 1989). Increased agricultural production and industrialization led to a significant and ongoing increase in the valley's population.

Ironically, much of the agricultural land has been lost to residential development, and much of the water for agriculture has been usurped for domestic, recreational, and industrial purposes. The Native Americans of the area, displaced and marginalized by the influx of Europeans and Euro-Americans, have in recent years regained some economic leverage through agriculture, water settlements, and the operation of casinos.

Scottsdale was originally established in 1868, with the Swilling Irrigation Canal Company working on improving and upgrading Hohokam Canals in the region. Scottsdale's name was changed from Orangedale in 1894 in honor of brothers George and Winfield Scott, who were major landowners and businessmen in the area. In 1937, internationally famous architect Frank Lloyd Wright established his "winter camp," which is now known as Taliesin West, in Scottsdale. Scottsdale was officially incorporated in 1951.

CLASS I RECORDS REVIEW OF THE GREASEWOOD FLAT PROJECT AREA AND THE SURVEY AREA (PARCEL NO. 216-80-007G)

Prior to the Class III pedestrian survey, SWCA consulted the AZSITE database, which includes records from the Arizona State Museum (ASM), Arizona State University, the Arizona State Historic Preservation Office (SHPO), and the Bureau of Land Management, for previously conducted surveys and previously recorded sites within a 1-mile radius of the project area.

An archaeological records search of the Greasewood Flat project area, the survey area, and of a 1-mile radius surrounding the project area indicates that 33 archaeological surveys have been conducted within 1 mile of the project area (Table 1; Figure 4).

The 42-acre Greasewood Flat project area has been previously surveyed (Bartholomew 2014). No archaeological sites were recorded.

The additional 5-acre survey area has never been previously surveyed for cultural resources.

Table 1. Previously Conducted Archaeological Surveys within a 1-Mile Radius of the Project Area

Agency Number	Project Name	Project Type
11-19-4.BLM	Unknown	Block survey
7.3111.SHPO	Unknown	Block survey
SHPO-2002-772	ALLTEL Proposed Telecommunications Site AZ-Troon-North - 49000.00-ACI-22 Construction	Block survey
1985-50.ASM	Pinnacle Peak Urban Land Sale	Block survey
1987-243.ASM	North Scottsdale Reconnaissance	Block and linear survey
1992-170.ASM	Desert Sun North Survey	Block survey
1992-187.ASM	Dynamite Road Survey	Block survey
1994-150.ASM	Crescent Moon At Pinnacle Peak	Block survey
1996-137.ASM	Yearling Estates	Block survey
1999-65.ASM	Cook Shack	Block survey
1999-236.ASM	Dynamite/112th Street	Block survey
2000-32.ASM	Survey at 118 th Street and Dynamite	Block survey
2000-76.ASM	116 th Street and Dynamite	Block survey
2000-240.ASM	Mosaic Rest Archaeological Survey	Block survey
2001-61.ASM	Nextel Communications Tower Project	Block survey
2001-201.ASM	118th Street & Redbird Road	Block survey
2002-51.ASM	ERM #49012.00-ACI-38 & 49024.00-ACI-38, AZ Troon North (Maricopa County) communications tower survey	Block survey
2003-420.ASM	Jomax and Alma School	Block survey
2003-1586.ASM	Troon North Survey	Block survey
2004-714.ASM	Survey at 118th Street and Jomax Road Survey	Block survey
2005-198.ASM	Cultural Resources Survey of Approximately 30 Acres Located Near Rio Verde Drive and 118th Street, Northern Scottsdale, Maricopa County, Arizona	Block survey
2005-233.ASM	Scottsdale Waterline Survey	Linear survey
2005-327.ASM	Alma School Water Transmission Line	Linear survey
2005-586.ASM	Survey at 118th Street and Redbird Road	Block survey
2005-1055.ASM	Stoplight North	Block survey
2008-791.ASM	Pinnacle Peak Resort 10 Acres Survey	Block survey
2009-793.ASM	A Class I and Class III Cultural Resources Assessment Survey of a Proposed Wireless Telecommunications Facility AT&T Mobility H260 Located at 10791 East Dynamite Boulevard in Scottsdale, Maricopa County, Arizona	Block survey
2009-891.ASM	AT&T H066	Block survey
2009-893.ASM	AT&T H070	Block survey

Table 1. Previously Conducted Archaeological Surveys within a 1-Mile Radius of the Project Area (Continued)

Agency Number	Project Name	Project Type
2009-894.ASM	AT&T H072	Block survey
2009-895.ASM	AT&T H317	Block survey
2010-347.ASM	Dynamite Road (Pima to Alma School)	Linear survey
Bartholomew 2014	Greasewood Flat Survey	Block survey

No known archaeological sites are located within the project area; however, eight sites have been previously recorded within a 1-mile radius of the project area (Table 2). The sites range from prehistoric habitation sites and artifact scatters to historic-era trash dumps and features. Please note that according to guidance from the ASM, archaeological sites outside the project area are not depicted on a map within this report.

Table 2. Previously Recorded Archaeological Sites within a 1-Mile Radius of the Project Area

Site Number	Site Description	NRHP Eligibility
AZ U:5:19(ASM)	Artifact scatter and feature	Not Eligible – SHPO
AZ U:5:90(ASM)	Artifact scatter	Eligible – Recorder
AZ U:5:103(ASM)	Historical trash scatter	Not Evaluated – SHPO
AZ U:5:104(ASM)	Multicomponent artifact scatter	Not Evaluated – SHPO
AZ U:5:105(ASM)	Historical tank and associated artifacts	Not Evaluated – SHPO
AZ U:5:106(ASM)	Historical thermal feature	Not Evaluated – SHPO
AZ U:5:112(ASM)	Historical trash scatter	Not Evaluated – SHPO
AZ U:5:171(ASM)	Rockshelter with artifact scatter	Eligible – Recorder

General Land Office and Historical Topographic Map Research

The General Land Office (GLO) plat map filed in 1921 depicts one unnamed historic road within the Greasewood Flat project area and immediately adjacent to the survey area (Figure 5). The 1971 McDowell Peak, Arizona 7.5-minute U.S. Geological Survey topographic map does not depict any historical structures or features within the Greasewood Flat project area or the survey area.

SURVEY EXPECTATIONS

Based on the background records search, it was expected that no prehistoric archaeological sites would be encountered; however, given the large amount of historic use in the area as depicted on GLO and historical topographic maps, historic period trash and sites were expected. No prehistoric sites or artifacts were encountered, and one historical isolate was recorded, which met expectations.

SURVEY METHODS

SWCA archaeologist Alan Bartholomew surveyed the survey area on May 30, 2014, resulting in a total of one person–field day. A total of approximately 5 acres was surveyed. General conditions for the survey were

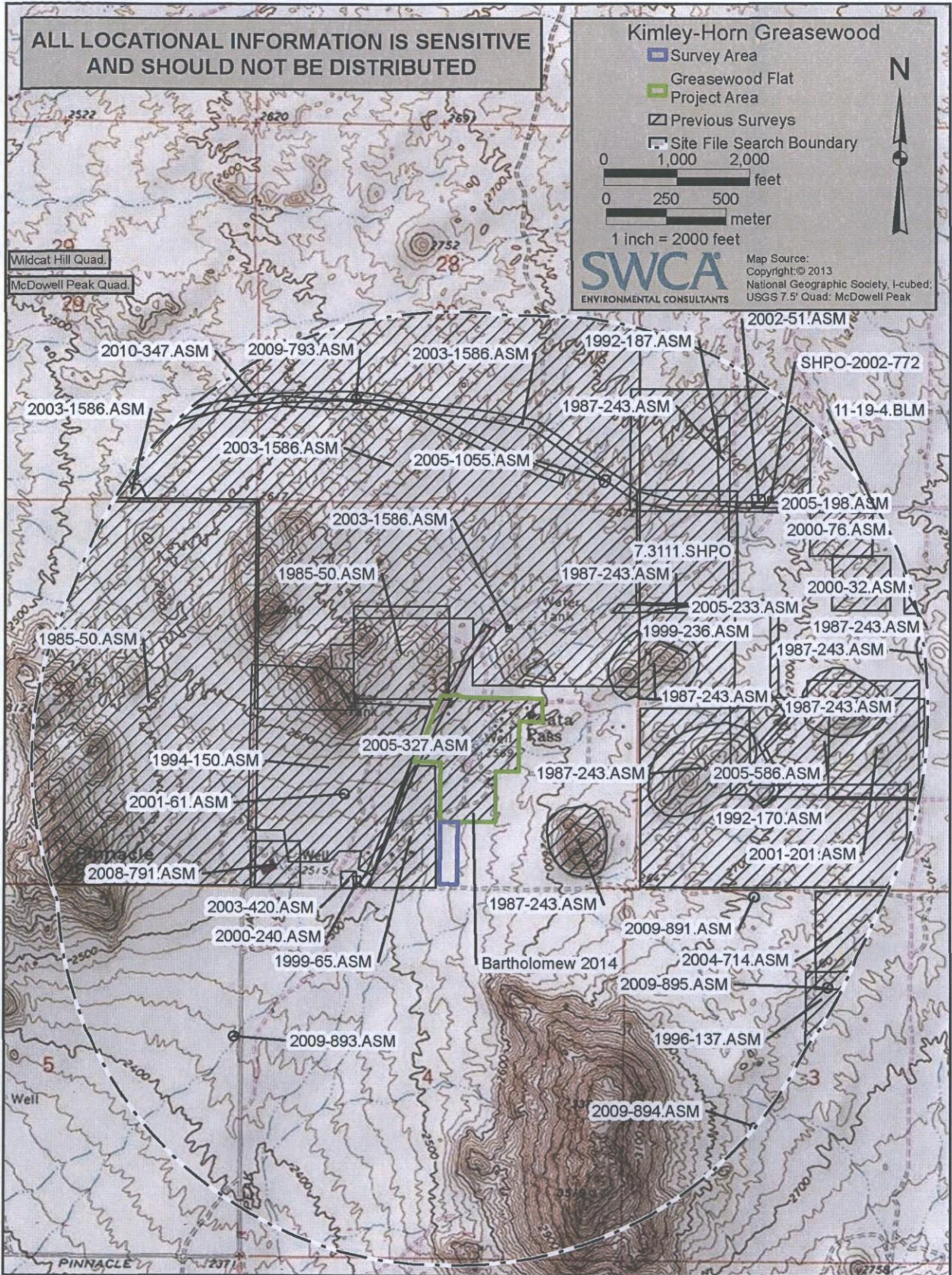


Figure 4. Class I records review results.

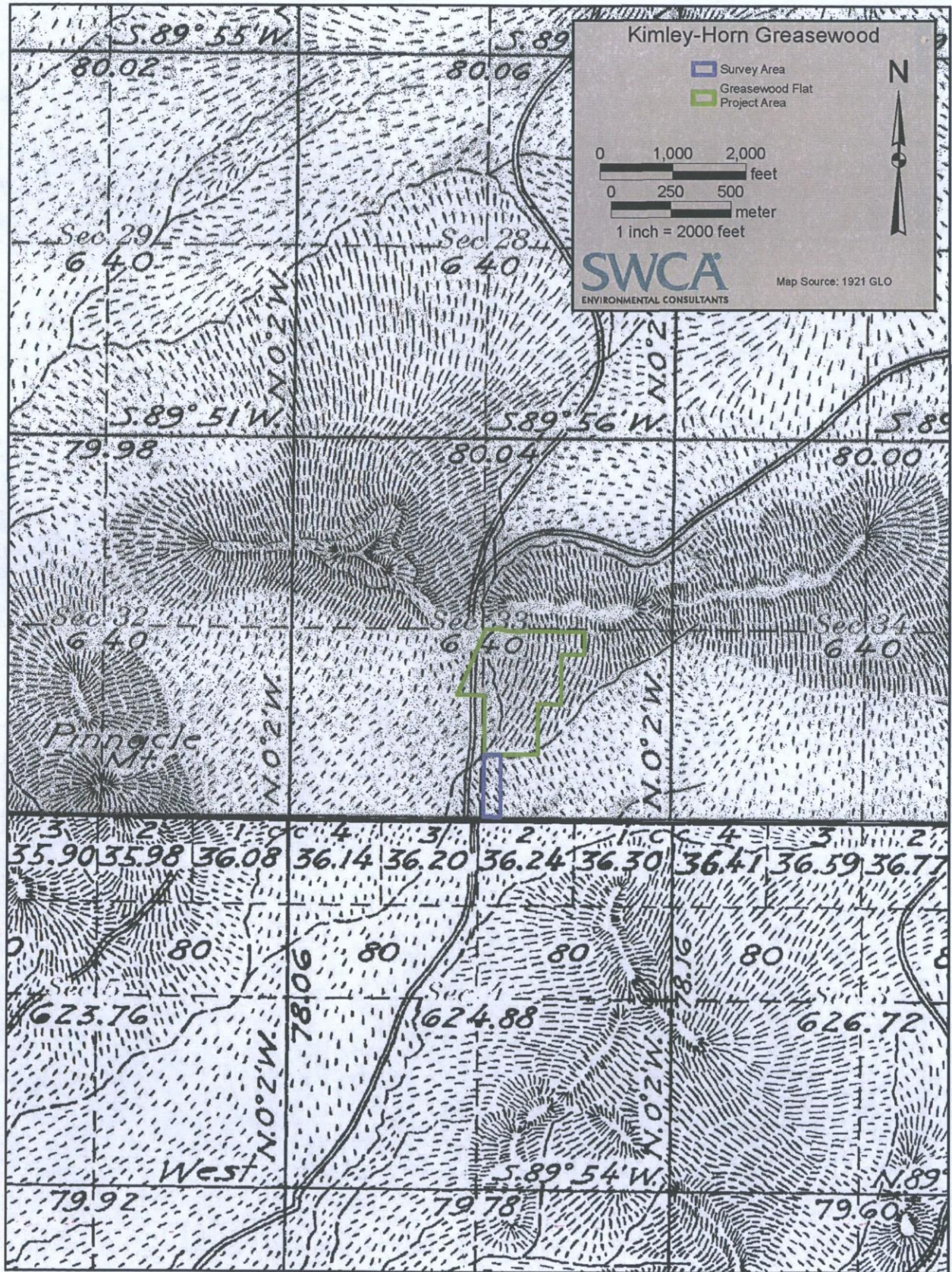


Figure 5. GLO map of the project area.

excellent, and ground visibility was generally 75 percent (see Figure 3). The survey was conducted using standard archaeological techniques following ASM guidelines for survey coverage and site recording methodologies. According to the standards for pedestrian survey established by ASM, a person conducting a pedestrian survey can achieve 100 percent coverage of a parcel by walking a series of systematic transects spaced no more than 20 meters (66 feet) apart. The survey entailed systematically walking the project area using 20-meter (66-foot) spacing in east-west and north-south directions. Evidence for cultural resources was sought in the form of artifacts (e.g., ceramics, lithics, historical metals, or glass) or features (concentrations of fire-affected rock, charcoal-stained soil, prehistoric or historic structures, or other cultural anomalies).

The ASM has established standards for evaluating materials identified during archaeological surveys. Briefly, properties of archaeological interest must contain the remains of past human activity that are at least 50 years old. Beyond this, two classes of findings are recognized: the site and the isolated occurrence (IO). To qualify as a site, a property must contain, within an area no more than 50 feet in diameter, 30 or more artifacts of a single type, unless all pieces originate from a single source (e.g., one broken bottle or ceramic vessel); or 20 or more artifacts when multiple types are present; or any number of artifacts, when a single fixed feature is present; or multiple fixed features, with or without any associated artifacts. The site can be larger than 50 feet in diameter as long as any 50-foot-diameter portion of the site meets one of these conditions. Artifact finds that do not meet these criteria but that are over 50 years old may be designated IOs. The precise locations of the IOs were point located and recorded using a handheld global positioning system (GPS) unit. Any identified artifacts were field-analyzed and then returned to their original locations.

SURVEY RESULTS

The Class III survey of the survey area resulted in the recording of one historical IO, a double punch-opened steel beverage can (Figure 6; Table 3). The IO is not eligible for listing on the NRHP.

Table 3. Isolated Occurrences

IO No.	IO Description	Easting*	Northing*
1	Double punch-opened steel beverage can	Appendix A	Appendix A

SUMMARY AND MANAGEMENT RECOMMENDATIONS

A Class I records search of the 42-acre Greasewood Flat property demonstrated that it has been completely surveyed (Bartholomew 2014). No archaeological sites have been recorded in that area.

A Class III archaeological survey of the survey area (Parcel No. 216-80-007G) resulted in the identification of one historical IO. The isolate does not meet the criteria for listing on the NRHP. No archaeological sites were recorded.

SWCA recommends a finding of No Historic Properties Affected for the proposed project. No further archaeological work is recommended for the project area. However, if previously undocumented buried cultural resources are identified during ground-disturbing activities, all work in the immediate vicinity of the discovery should stop until the find can be evaluated by a professional archaeologist.

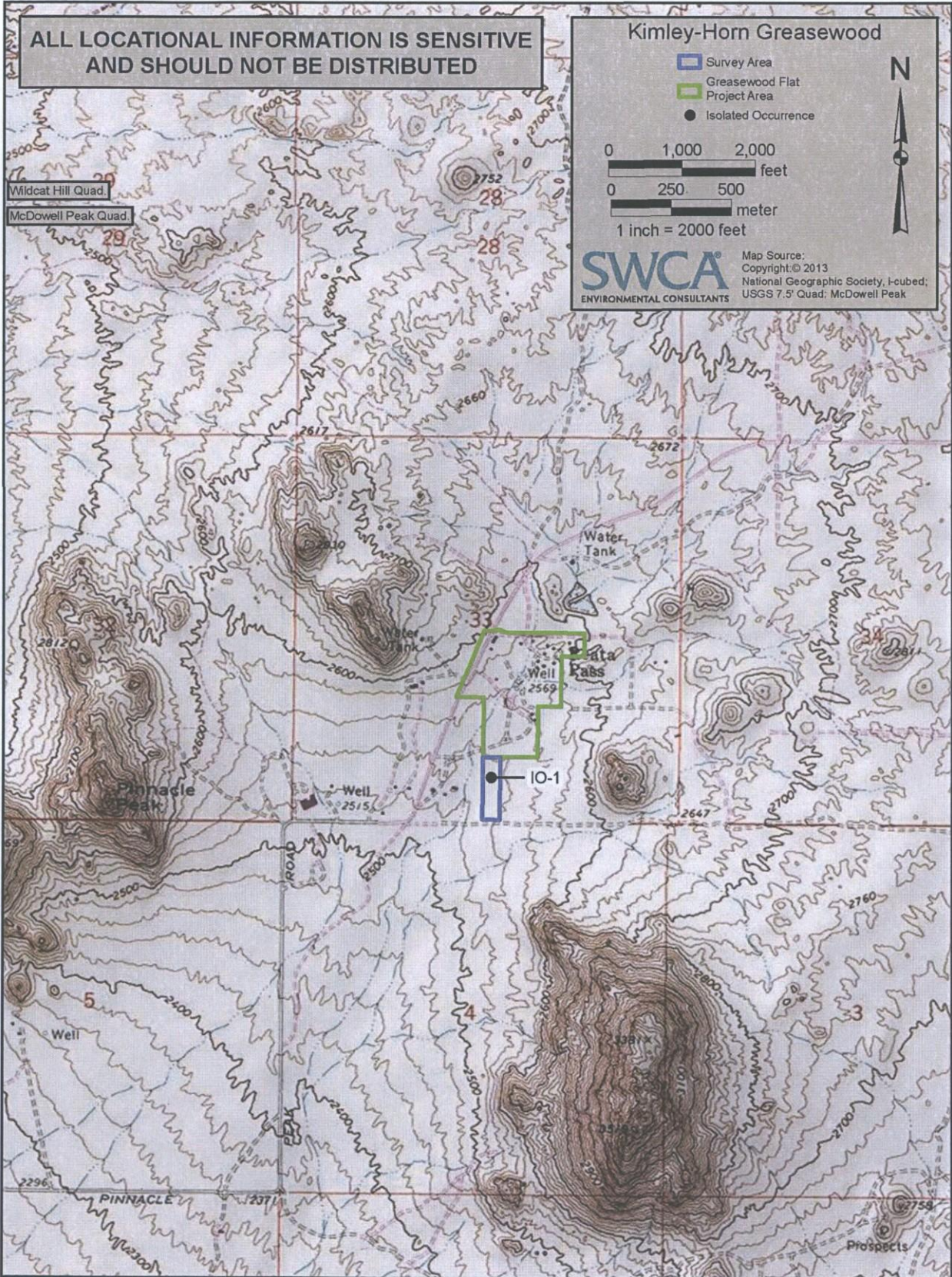


Figure 6. Class III cultural resources pedestrian survey results.

REFERENCES CITED

Abbott, David R.

- 1994 Chemical Analyses of Clay Fractions in Hohokam Pottery. In *The Pueblo Grande Project Vol. 3: Ceramics and the Production and Exchange of Pottery in the Central Phoenix Basin*, edited by David R. Abbott, pp. 91-148. Publications in Archaeology No. 20, Soil Systems, Inc., Phoenix.
- 2000 *Ceramics and Community Organization among the Hohokam*. University of Arizona Press, Tucson.
- 2001 Conclusions for the GARP Ceramic Analysis. In *The Grewe Archaeological Research Project, Volume 2, Part 1. Ceramic Studies*, edited by David R. Abbott, pp. 263-272. Anthropological Research Papers No. 99-01, Northland Research, Inc., Tempe, Arizona.

Ackerly, Neal W., and T. Kathleen Henderson (editors)

- 1989 *Prehistoric Agricultural Activities on the Lehi-Mesa Terrace: Perspectives on Hohokam Irrigation Cycles*. Northland Research, Inc., Flagstaff, Arizona.

Arizona Geological Survey

- 2014 Geologic map of Arizona. Available at: <http://services.usgin.org/azgs/geologic-map-arizona.html>. Accessed June 2, 2014.

Bartholomew, Alan

- 2014 *Archaeological Survey of Approximately 42 Acres of the Reata Pass/Greasewood Flat Property, Scottsdale, Maricopa County, Arizona*. SWCA Cultural Resources Report No. 14-78. SWCA Environmental Consultants, Phoenix.

Bayman, James M.

- 2001 The Hohokam of Southwest North America. *Journal of World Prehistory* 15:257-311.

Bohrer, Vorsila L.

- 1987 The Plant Remains from La Ciudad, a Hohokam Site in Phoenix. In *La Ciudad: Specialized Studies in the Economy, Environment, and Culture of La Ciudad, Part III*, edited by JoAnn E. Kisselburg, Glen E. Rice, and Brenda L. Shears, pp. 67-202. Anthropological Field Studies No. 20, Arizona State University, Tempe, Arizona.

Brown, David

- 1994 *Biotic Communities: Southwestern United States and Northwestern Mexico*. University of Utah Press, Salt Lake City.

Cable, John S., and David E. Doyel

- 1987 Pioneer Period Village Structure and Settlement Pattern in the Phoenix Basin. In *The Hohokam Village*, edited by David E. Doyel, pp. 21-70. Southwestern and Rocky Mountain Division of the American Association for the Advancement of Science, Glenwood Springs, Colorado.

Chenault, Mark C.

- 2000 In Defense of the Polvorón Phase. In *The Hohokam Village Revisited*, edited by David E. Doyel, Suzanne K. Fish, and Paul R. Fish, pp. 277-286. Southwestern and Rocky Mountain Division of the American Association for the Advancement of Science, Colorado State University, Fort Collins, Colorado.

- Ciolek-Torello, Richard, Eric E. Klucas, and Stephanie M. Whittlesey
 2000 Hohokam Households, Settlement Structure, and Economy in the Lower Verde Valley. In *The Hohokam Village Revisited*, edited by D. E. Doyel, S. K. Fish, and P. R. Fish, pp. 65–100. Southwestern and Rocky Mountain Division of the American Association for the Advancement of Science, Colorado State University, Fort Collins, Colorado.
- Crary, Joseph S., and Douglas B. Craig
 2001 The Design Mechanics of Hohokam Pithouses. In *The Grewe Archaeological Research Project, Vol. 1: Background and Feature Descriptions*, edited by Douglas B. Craig, pp. 39–62. Anthropological Papers 99-1, Northland Research, Inc., Tempe, Arizona.
- Cross, Jack L., Elizabeth H. Shaw, and Kathleen Scheifele (editors)
 1960 *Arizona, Its People and Resources*. University of Arizona Press, Tucson.
- Crown, Patricia L., and W. James Judge (editors)
 1991 *Chaco and the Hohokam: Prehistoric Regional Systems in the American Southwest*. School of American Research Advanced Seminary Series, School of American Research Press, Santa Fe.
- Dean, Jeffrey S.
 1991 Thoughts on Hohokam Chronology. In *Exploring the Hohokam: Prehistoric Desert Peoples of the American Southwest*, edited by G. J. Gumerman, pp. 61–149. New World Studies Series No. 1, Amerind Foundation, Dragoon, Arizona, and University of New Mexico Press, Albuquerque.
- Doyel, David E.
 1985 Summary and Discussion. In *Hohokam Settlement and Economic Systems in the Central New River Drainage, Arizona*, Vol. 1, edited by D. E. Doyel and M. D. Elson, pp. 727–734. Publications in Archaeology No. 4, Soil Systems, Inc., Phoenix.
- Fish, Paul R.
 1989 The Hohokam, 1,000 Years of Prehistory in the Sonoran Desert. In *Dynamics of Southwestern Prehistory*, edited by Linda S. Cordell and George J. Gumerman, pp. 19–63. Smithsonian Institution Press, Washington, D.C. A School of American Research Advanced Seminar Book, Santa Fe.
- Gifford, E. W.
 1936 Northeastern and Western Yavapai. *University of California Publications in American Archaeology and Ethnology* 34(4):247–354. University of California Press, Berkeley.
- Gregory, David A., William L. Deaver, Suzanne K. Fish, Ronald Gardiner, Robert W. Layhe, Fred L. Nials, and Lynn S. Teague
 1988 *The 1982–1984 Excavations at Las Colinas: The Site and its Features*. Archaeological Series 162, Vol. 2. Arizona State Museum, Tucson.
- Gumerman, George J.
 1991 *Exploring the Hohokam: Prehistoric Desert Peoples of the American Southwest*. Amerind Foundation, Dragoon, Arizona, and University of New Mexico Press, Albuquerque.
- Hackbarth, Mark R.
 1992 *Prehistoric and Historic Occupation of the Lower Verde River Valley: The State Route 87 Verde Bridge Project*. Northland Research, Inc., Flagstaff, Arizona.

- 1995 *The Historic Archaeology of Heritage Square*. Pueblo Grande Museum Anthropological Papers No. 2, Pueblo Grande Museum, Phoenix.
- 2001 Late Archaic and Red Mountain Phase Task Group Size in Paradise Valley, Arizona: Evidence from the Last Ditch Site. *Kiva* 67:81-106.
- Haury, Emil W.
1976 *The Hohokam: Desert Farmers and Craftsman; Snaketown. 1964-1965*. University of Arizona Press, Tucson.
- Henderson, T. Kathleen, and Mark R. Hackbarth
2000 What is Going on at the Hohokam Village? In *The Hohokam Village Revisited*, edited by David E. Doyel, Suzanne K. Fish, and Paul R. Fish, pp. 287-316. Southwestern and Rocky Mountain Division of the American Association for the Advancement of Science, Colorado State University, Fort Collins, Colorado.
- Huckell, Bruce B.
1982 *The Distribution of Fluted Points in Arizona: A Review and an Update*. Archaeological Series No. 145, Arizona State Museum, Tucson.
- Hunt, Robert C., David Guillet, David R. Abbott, James Bayman, Paul Fish, Suzanne Fish, Keith Kintigh, and James A. Neely
2005 Plausible Ethnographic Analogies for the Social Organization of Hohokam Canal Irrigation. *American Antiquity* 70:433-456.
- Luckingham, Bradford
1989 *Phoenix: The History of a Southwestern Metropolis*. University of Arizona Press, Tucson.
- Mabry, Jonathan B.
2000 The Red Mountain Phase and the Origins of Hohokam Villages. In *The Hohokam Village Revisited*, edited by D. E. Doyel, S. K. Fish, and P. R. Fish, pp. 37-64. Southwestern and Rocky Mountain Division of the American Association for the Advancement of Science, Colorado State University, Fort Collins, Colorado.
- Mawn, Geoffrey P.
1977 Promoters, Speculators, and the Selection of the Phoenix Townsite. *Arizona and the West* 19:207-224.
- Miller, JoAnne
1994 Pueblo Grande Flotation, Macrobotanical, and Wood Charcoal Analyses. In *The Pueblo Grande Project, Volume 5: Environment and Subsistence*, edited by S. Kwiatkowski, pp. 127-204. Publications in Archaeology No. 20, Soil Systems, Inc., Phoenix.
- Morris, Donald H.
1969 Red Mountain: An Early Pioneer Period Hohokam Site in the Salt River Valley of Central Arizona. *American Antiquity* 34:40-54.
- Morrow, Dwight W.
1943 *American Economic History*. Harper and Brothers Publishers, New York.

- Potter, James M., and Lynn A. Neal
 2000 *Tributaries, Terraces, and Tinajas: Prehistoric Cultural Variation on Three Drainages near New River, Maricopa County, Arizona*. Anthropological Research Paper No. 7, SWCA Environmental Consultants, Flagstaff, Arizona.
- Reff, D. T.
 1992 Contact Shock in Northwestern New Spain, 1518–1764. In *Disease and Demography in the Americas*, edited by J. Verano and D. Ubelaker, pp. 256–276. Smithsonian Institution Press, Washington, D.C.
- Reid, J. Jefferson, and Stephanie Whittlesey
 1992 New Evidence for Dating Gila Polychrome. In *Proceedings of the Second Salado Conference, Globe, AZ, 1992*, edited by R. C. Lang and S. Germick, pp. 223–229. Occasional Paper, Arizona Archaeological Society, Phoenix.
- Smith, Susan J.
 2002 Corn, Cacti, and Mesquite: Subsistence in the Northern Periphery. In *Phoenix Basin to Perry Mesa: Rethinking the "Northern Periphery,"* edited by Mark R. Hackbarth, Kelley Hays-Gilpin, and Lynn Neal, pp. 101–119. *The Arizona Archaeologist* 34.
- Teague, Lynn S.
 1989 The Postclassic and the Fate of the Hohokam. In *The 1982–1984 Excavations at Las Colinas, Synthesis and Conclusions*, edited by L. S. Teague and W. L. Deaver, pp. 145–168. Archaeological Series 162, Vol. 6, Arizona State Museum, Tucson.
- Trimble, Marshall
 1977 *Arizona*. Doubleday and Company, Garden City, New York.
- Valehrach, Emil M., and Bruce S. Valehrach
 1971 *Excavations at Brazaletes Pueblo*. Arizona Archaeologist Occasional Paper No. 6, Arizona Archaeological Society, Inc., Phoenix.
- Van Gerven, Dennis P., and Susan Guise Sheridan
 1994 A Biocultural Reconstruction of a Classic Period Hohokam Community. In *The Pueblo Grande Project: Volume 6: The Bioethnography of a Classic Period Hohokam Population*, edited by Dennis P. Van Gerven and Susan Guise Sheridan, pp. 123–128. Publications in Archaeology No. 20, Soil Systems, Inc., Phoenix.
- Wagoner, Jay J.
 1989 *Early Arizona: Prehistory to Civil War*. University of Arizona Press, Tucson.
- Walker, Henry P., and Don Bufkin
 1979 *Historical Atlas of Arizona*. University of Oklahoma Press, Norman.
- Wallace, Henry D.
 2001 Time Seriation and Typological Refinement of the Middle Gila Buffware Sequence: Snaketown through Soho Phase. In *The Grewe Archaeological Research Project, Vol. 2, Part I: Ceramic Studies*, edited by D. R. Abbott, pp. 177–259. Anthropological Papers 99-1, Northland Research, Inc., Tempe, Arizona.

- Wilcox, David R., Thomas R. McGuire, and Charles Sternberg
1981 *Snaketown Revisited*. Archaeological Series No. 155, Arizona State Museum, University of Arizona, Tucson.
- Wilcox, David R., Gerald Robertson, Jr., and J. Scott Wood
2001 Organized for War: The Perry Mesa Settlement System. In *Deadly Landscapes: Case Studies in Prehistoric Southwestern Warfare*, edited by Glen E. Rice and Steven A. LeBlanc, pp. 141–194. University of Utah Press, Salt Lake City.
- Wilcox, David R., and Lynette O. Shenk
1977 *The Architecture of the Casa Grande and its Interpretation*. Archaeological Series No. 115, Arizona State Museum, University of Arizona, Tucson.
- Wilcox, David R., and Charles Sternberg
1983 *Hohokam Ballcourts and Their Interpretation*. Arizona State Museum Archaeological Series No. 160, University of Arizona, Tucson.
- Wright, Thomas E.
2002 Windows to the Past: Three Rockshelters in the Northern Periphery. In *Phoenix Basin to Perry Mesa: Rethinking the "Northern Periphery,"* edited by Mark R. Hackbarth, Kelley Hays-Gilpin, and Lynn Neal, pp. 11–18. *The Arizona Archaeologist* 34.
- Zarbin, Earl A.
1978 The Swilling Legacy. *Arizona Republic* August 13–August 30. On file, Luhrs Reading Room, Hayden Library, Arizona State University, Tempe.
1997 *Two Sides of the River: Salt River Valley Canals, 1867–1902*. Salt River Project, Phoenix.

APPENDIX A
Locational Data

Table A-1. Universal Transverse Mercator (UTM) Coordinates for IO1

IO No.	Artifact Description	Easting*	Northing*
1	Steel beverage can, double punch-opened	421499	373323376

* UTM Location (Zone 12, NAD 83)



May 28, 2014

Mr. Jeff Deason
Taylor Morrison/Arizona, Inc.
9000 E. Pima Center Parkway, Suite 350
Scottsdale, Arizona 85258

**RE: Cavalliere Flats - Alma School Parkway and Pinnacle Vista Drive – Scottsdale, Arizona
Traffic Impact Statement**

Dear Mr. Deason:

The purpose of this letter is to address site specific traffic issues for the proposed Cavalliere Flats residential development located near the southeast corner of the intersection of Alma School Parkway and Pinnacle Vista Drive in Scottsdale, Arizona. This letter outlines our findings regarding the traffic generation of the proposed plan for an 80 unit single family residential development on the east side of Alma School Parkway between Pinnacle Vista Drive and Jomax Road.

The development is proposed to consist of an 80 unit gated community on the parcel currently occupied by the existing Greasewood Flats restaurant. The residential development has a single main entrance onto Alma School Parkway approximately 560 feet south of the intersection with Pinnacle Vista Drive. A second emergency-only access is provided to Jomax Road. The proposed lots are oriented along two local streets parallel to Alma School Parkway on the east and west sides of the main drainage feature through the project.

Alma School Parkway has been constructed with a single northbound and a single southbound through lane plus a continuous two way left turn lane along the project frontage. Pinnacle Vista Drive and Jomax Road are both two lane paved roadways adjacent to the site.

The current land use consists of a restaurant with a relatively small building for food and beverage service and a significant amount of adjacent outdoor seating. For the purpose of estimating the trip generation a total equivalent building size of 6,000 square feet was assumed to represent the existing trip generation of both the building and the outdoor seating area. Due to the unique characteristics of the existing facility, it is recognized that the AM peak hour volumes predicted by the ITE rates may not be representative of the peak hour of the adjacent street but would likely be more comparable to the peak hour of the generator. The existing restaurant is proposed to be replaced by the 80 single family dwelling units. The trip generation rates published by the Institute of Transportation Engineers' (ITE) Trip Generation Manual, 9th Edition were used to compare the trip generation characteristics of the existing and proposed land uses. The trip generation calculation for the existing use was based on the ITE Code 932 for High Turnover (Sit-Down) Restaurant. The trips for the proposed use were determined using ITE Code 210 for Single Family Detached Housing. The calculations are summarized in **Table 1 and Table 2.**

**12-ZN-2014
6/11/2014**

Table 1. Existing Trip Generation

Land Use Description	ITE Code	Quantity	Units	Daily Trips	AM			PM		
					In	Out	Total	In	Out	Total
High Turnover (Sit-Down) Restaurant	932	6	K sq. ft.	764	36	29	65	35	24	59

Table 2. Proposed Trip Generation

Land Use Description	ITE Code	Quantity	Units	Daily Trips	AM			PM		
					In	Out	Total	In	Out	Total
Single Family Detached Housing	210	80	D. U..	762	15	45	60	50	30	80

The trip generation calculations indicate that on an average weekday the proposed residential units would be expected to generate a similar number of daily trips as a 6,000 square foot restaurant. Peak hour and weekend trips for the restaurant use would likely be higher than the residential use.

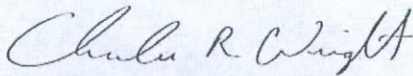
The City of Scottsdale 2010 Design Standards & Policy Manual states that if a proposed development is anticipated to generate fewer trips than the existing land use and less than 100 peak hour trips a traffic impact and mitigation analysis (TIMA) is not required. The proposed development is anticipated to meet both of these requirements, therefore a TIMA report is not required.

While no additional traffic analysis was prepared, the configuration of the proposed site entrance was reviewed. With the single access on Alma School Parkway, up to 50 vehicles could potentially be turning right from northbound Alma School Parkway into the project entrance. Since it is likely that the northbound right turn volume at the site entrance will exceed the 30 vehicle per hour threshold for installation of a deceleration lane, it is recommended that a northbound exclusive right turn lane be provided. The right turn lane should consist of a minimum 100 foot storage length with the transition prior to the storage per the City of Scottsdale standard details. Since no other access points to adjacent roadways are provided to the project, no improvements to either Pinnacle Vista Drive or Jomax Road are recommended.

If you have any further questions please feel free to contact me at (602) 944-5500.

Very truly yours,

KIMLEY-HORN AND ASSOCIATES, INC.



Charles R. Wright, P.E.

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