



Development Review (Minor) Staff Approval

97-SA-2016

Smokehaus

APPLICATION INFORMATION

LOCATION:	3636 N Scottsdale Rd	APPLICANT:	Dave Andrea
PARCEL:	130-13-033	COMPANY:	Smokehaus
Q.S.:	16-44	ADDRESS:	3636 N Scottsdale Rd Scottsdale, AZ 85251
CODE VIOLATION #:		PHONE:	602-738-1274

Request: Construct a colored shade sail over patio at restaurant

STIPULATIONS

1. The shades shall comply with the plans dated 03/22/2016.
2. The shades shall match Windscreens4less, beige dated 03/22/2016.
3. The shades shall be fire retardant, and comply with all fire and building code requirements.
4. The shade support columns shall be dark brozen or dark brown.
5. No exterior lights are approved with this application.

CONSTRUCTION DOCUMENT PLAN REVIEW SUBMITTAL REQUIREMENTS

Submit one copy of this approval letter, and a completed Owner/Builder form if applicable, and a permit application along with the following plan set(s) to the One-Stop-Shop for review:

PERMIT APPLICATION: ☒ **Completed Permit Application.** The permit application may be obtained or completed online at the following weblink:

http://www.scottsdaleaz.gov/Assets/Public+Website/bldgresources/APP_Permit_Commercial.pdf

(Please complete the permit application online prior to arriving at the City to submit your construction documents)

ARCHITECTURAL: ☒ 4 sets of architectural plans, fabric fire retardant and spread specifications, and 1 additional, elevation.

Expiration of Development Review (Minor) Approval

This approval expires two (2) years from date of approval if a permit has not been issued, or if no permit is required, work for which approval has been granted has not been completed.

Staff Signature: _____

Dan Symer, AICP

DATE: _____

3-22-2016

Planning and Development Services

7447 East Indian School Road, Suite 105, Scottsdale, Arizona 85251 Phone: 480-312-7000 Fax: 480-312-7088

City of Scottsdale's Website: www.scottsdaleaz.gov

Page 1 of 1

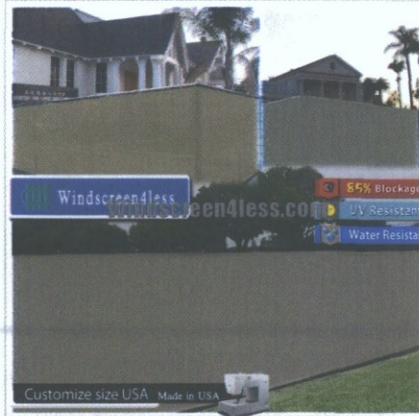
Form Revision Date: 12/11/2014

COMPARE PRODUCTS

You have no items to compare.

MY CART

You have no items in your shopping cart.



Double click on above image to view full picture



MORE VIEWS



Beige Standard 6ft by 25ft Privacy 85% Fence Screen Panel

[Email to a Friend](#)

[Be the first to review this product](#)

Availability: In stock

\$34.98

Quick Overview

- High quality polyethylene knitted design
- Visibility Blockage: 85%
- Fabricated with 1.5" double-side Beige binding
- Tape and brass grommets placed evenly 18" O.C. on all 4 sides
- Double grommets on every corner edge for extra strength

*Zip Ties Included

* Required Fields

-- Please Select --



\$34.98

Qty:

ADD TO CART

[Add to Wishlist](#)

[Add to Compare](#)

Product Description

Additional Information

Product Tags

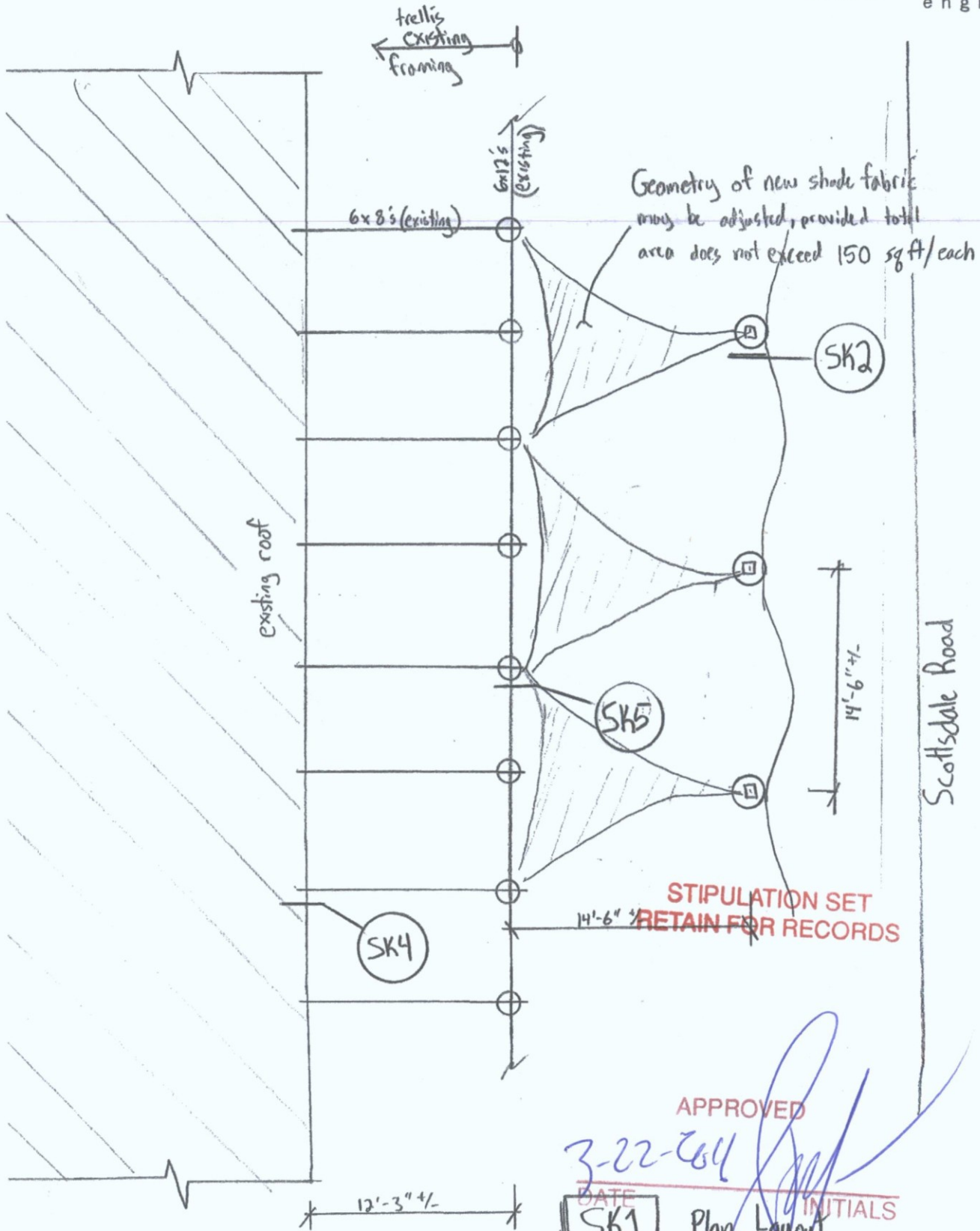
Fence Screen has taken quality and performance to a higher level, creating a super high quality knitted fence screen specifically developed as a long term solution for fence privacy and shade protection. For years we have been working to develop the perfect fence privacy screen. Our goal was to create a product that would combine privacy, air-flow and longevity; a product that could truly hold up to any outdoor climate. Years of R&D have resulted in a truly revolutionary new product for the fencing industry. Our fence utilizes the latest advancements in manufacturing technologies, combining High Density Polyethylene and advanced Ultra Violet (UV) stabilizer and pigments.

STIPULATION SET
RETAIN FOR RECORDS

APPROVED

3-22-2016
DATE

INITIALS



PROJECT:

3636 N. Scottsdale Road

NO:

SHEET:

1



DAN SYMER

Pre-Application Request

Purpose:

The purpose of the Pre-Application submittal, and meeting, is for the applicant and City Staff to discuss a proposed Development Application, and the information and process that is necessary for City Staff to process the proposal.

In accordance with the Zoning Ordinance, no development application shall be accepted before a Pre-Application has been submitted, and a Pre-Application meeting has been conducted with City Staff, unless the Pre-Application meeting has been waived by the Zoning Administrator.

Submittal:

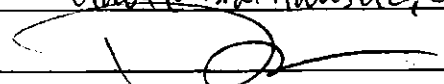
The completed Pre-Application request form, all required materials and fees should be submitted in person to the One-Stop-Shop located at 7447 East Indian School Road; or, may they be submitted digitally at following website:

<https://eservices.scottsdaleaz.gov/eServices/PreApps/Default.aspx>

All checks shall be payable to "City of Scottsdale."

Scheduling

After the Pre-Application packet has been accepted at the One-Stop-Shop, a staff member will contact the Applicant within five (5) Staff Working Days to schedule a Pre-Application meeting with the assigned staff member(s). Generally, a Pre-Application meeting is scheduled within five (5) to fifteen (15) Staff Working Days from the date of the submittal.

Project Name: <u>SMOKEHAUS</u>	
Property's Address: <u>3636 N. SCOTTSDALE RD</u>	APN: _____
Property's Zoning District Designation: _____	
Property Details:	
<input type="checkbox"/> Single-Family Residential <input type="checkbox"/> Multi-Family Residential <input checked="" type="checkbox"/> Commercial <input type="checkbox"/> Industrial <input type="checkbox"/> Other	
Has a 'Notice of Compliance' been issued? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If yes, provide a copy with this submittal	
Owner: <u>DAVE ANDRETTA</u>	Applicant: _____
Company: <u>BRAE HAUS BRAND, LLC</u>	Company: _____
Address: <u>3636 N. SCOTTSDALE RD</u>	Address: _____
Phone: <u>602 738 1274</u> Fax: _____	Phone: _____ Fax: _____
E-mail: <u>dave@braehausaz.com</u>	E-mail: _____
	Applicant Signature _____
Official Use Only	Submittal Date: _____ Application No.: <u>115</u> -PA- <u>2016</u>
Project Coordinator: _____	

Planning and Development Services Department

7447 E Indian School Road Suite 105, Scottsdale, Arizona 85251 Phone: 480-312-7000 Fax: 480-312-7088



Pre-Application Request

Development Application Type:

Please check the appropriate box of the Type(s) of Application(s) you are requesting

Zoning	Development Review	Signs
<input type="checkbox"/> Text Amendment (TA)	<input type="checkbox"/> Development Review (Major) (DR)	<input type="checkbox"/> Master Sign Program (MS)
<input type="checkbox"/> Rezoning (ZN)	<input checked="" type="checkbox"/> Development Review (Minor) (SA)	<input type="checkbox"/> Community Sign District (MS)
<input type="checkbox"/> In-fill Incentive (II)	<input type="checkbox"/> Wash Modification (WM)	Other
<input type="checkbox"/> Conditional Use Permit (UP)	<input type="checkbox"/> Historic Property (HP)	<input type="checkbox"/> Annexation/De-annexation (AN)
Exemptions to the Zoning Ordinance	Land Divisions	<input type="checkbox"/> General Plan Amendment (GP)
<input type="checkbox"/> Hardship Exemption (HE)	<input type="checkbox"/> Subdivisions (PP)	<input type="checkbox"/> In-Lieu Parking (IP)
<input type="checkbox"/> Special Exception (SX)	<input type="checkbox"/> Subdivision (Minor) (MD)	<input type="checkbox"/> Abandonment (AB)
<input type="checkbox"/> Variance (BA)	<input type="checkbox"/> Perimeter Exceptions (PE)	<input type="checkbox"/> Single-Family Residential
<input type="checkbox"/> Minor Amendment (MA)		<input type="checkbox"/> Other

Submittal Requirements: (fees subject to change every July)

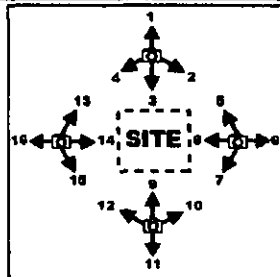
☒ Pre-Application Fee: \$ _____
(No fees are charged for Historic Preservation (HP) properties.)

☒ Records Packet Fee: \$ _____
Processed by staff. The applicant need not visit the Records desk to obtain the packet.
(Only required for ZN, II, UP, DR, PP, AB applications, or otherwise required by Staff)

☒ Application Narrative:
The narrative shall describe the purpose of the request, and all pertinent information related to the request, such as, but not limited to, site circulation, parking and design, drainage, architecture, proposed land use, and lot design.

☐ Property Owner Authorization Letter
(Required for the SA and MS Pre-Applications)

- ☐ Site / Context Photographs
- Provide color photographs showing the site and the surrounding properties. Use the guidelines below for photos.
 - Photos shall be taken looking in towards the project site and adjacent to the site.
 - Photos should show adjacent improvements and existing on-site conditions.
 - Each photograph shall include a number and direction.
 - Sites greater than 500 ft. in length, also take the photo locations shown in the dashed lines.
 - Photos shall be provided 8 1/2 x 11 paper, max. two per page.



☐ Other

- The following list of Additional Submittal Information is not required for a Pre-Application meeting, unless indicated below by staff prior to the submittal of this request.
- Applicants are advised to provide any additional information listed below. This will assist staff to provide the applicant with direction regarding an application.

Additional Submittal Information

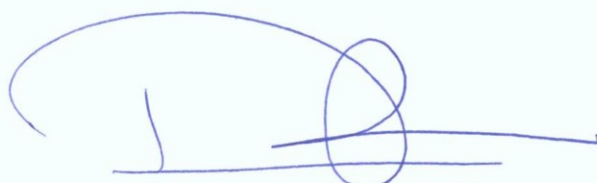
- ☐ Site Plan
- ☐ Subdivision plan
- ☐ Floor Plans
- ☐ Elevations
- ☐ Landscape plans
- ☐ H.O.A. Approval letter
- ☐ Sign Criteria Regulations & Language
- ☐ Material Samples – color chips, awning fabric, etc.
- ☐ Cross Sections – for all cuts and fills
- ☐ Conceptual Grading & Drainage Plan
- ☐ Exterior Lighting – provide cut sheets, details and photometrics for any proposed exterior lighting.
- ☐ Boundary Survey (required for minor land divisions)
- ☐ Areal of property that includes property lines and highlighted area abandonment request.
- ☐ One copy of the recorded document for the area that is requested to be abandoned. Such as: subdivision plat, map of dedication, GLO (General Land Office) federal patent roadway easement, or separate dedication document. A copy of most recorded documents to be abandoned may be purchased at the City of Scottsdale Records Dept. (480-312-2356), or the Maricopa County Recorder's Office (602-506-3535). A copy of the General Land Office (GLO) federal patent roadway easement may be purchased from the Bureau of Land Management (602-417-9200).

Planning and Development Services Department

7447 E Indian School Road Suite 105, Scottsdale, Arizona 85251 Phone: 480-312-7000 Fax: 480-312-7088

2/8/16

This preapplication request is to
put a shade sail over my patio
area @ 3636 N. Scottsdale Rd
The Smokehaus.



DAVE ANDERSON



Phoenix

7400 W. Detroit St
Suite 170
Chandler, AZ 85226
(p) 480.483.6111
(f) 480.483.6112

Las Vegas

1635 Village Center Cir
Suite 200
Las Vegas, NV 89134
(p) 702.933.7000
(f) 702.933.7001
800.933.7611
wrightengineers.com

Irvine

2 Venture, Suite 200
Irvine, CA 92618
(p) 949.477.4001
(f) 949.477.4009

Salt Lake

9160 South 300 West
Suite 2
Sandy, UT 84070
(p) 801.352.2001
(f) 801.352.2006

Tucson

2200 East River Road
Suite 104
Tucson, AZ 85718
(p) 520.468.7400

STRUCTURAL CALCULATIONS

PROJECT: **Smokehaus - Fabric Shade Attachment**
3636 N Scottsdale Road
Scottsdale, AZ 85251

PROJECT No: **160262**

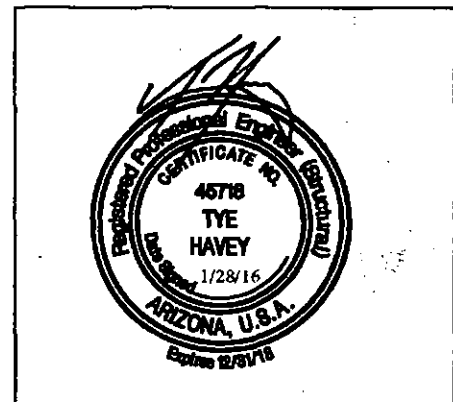
CLIENT: **Smokehaus**

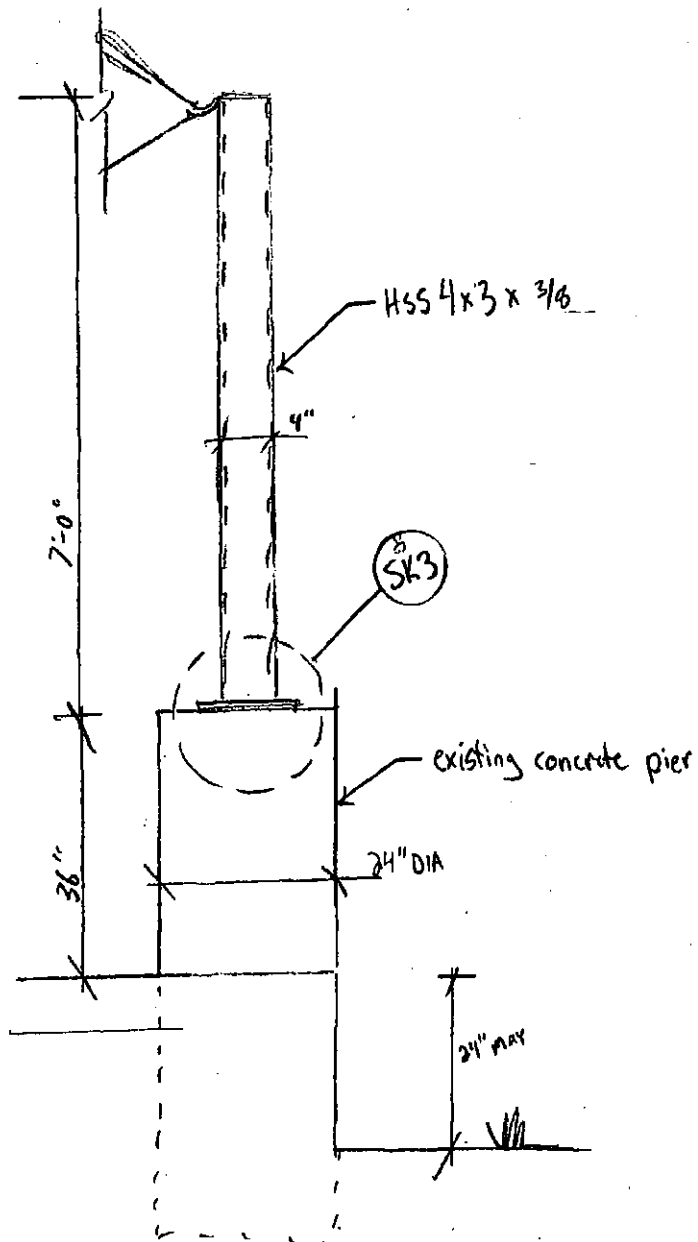
DATE: **January 28, 2016**

SHEET INDEX

LAYOUT	1
STRUCTURAL SKETCHES	2
CALCULATIONS	6

These calculations are the sole property of WRIGHT ENGINEERS and may not be reproduced in whole or part without written permission. Calculations are valid only for the above named project and location and are not valid unless engineer's original wet signed seal is affixed.





SK2

PROJECT:

No:

SHEET:

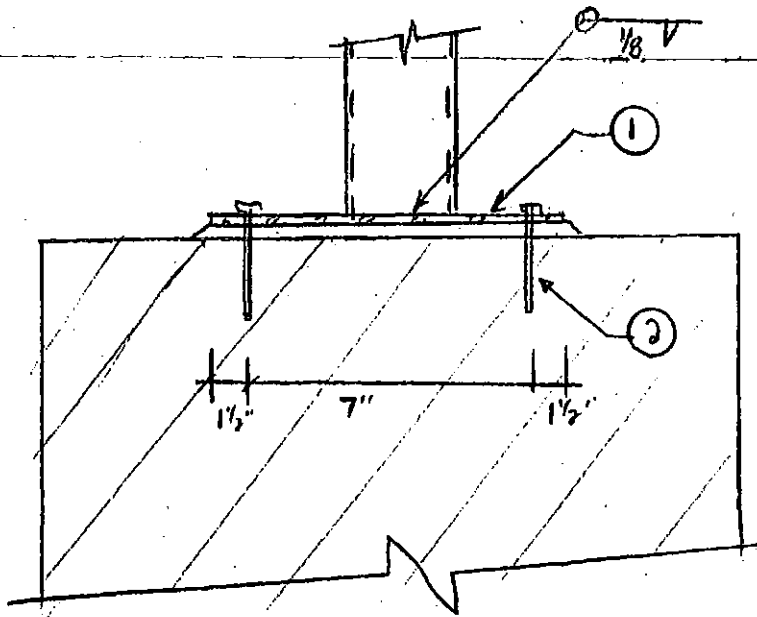
2

① $\frac{1}{4}$ " x 10" x 10" steel plate

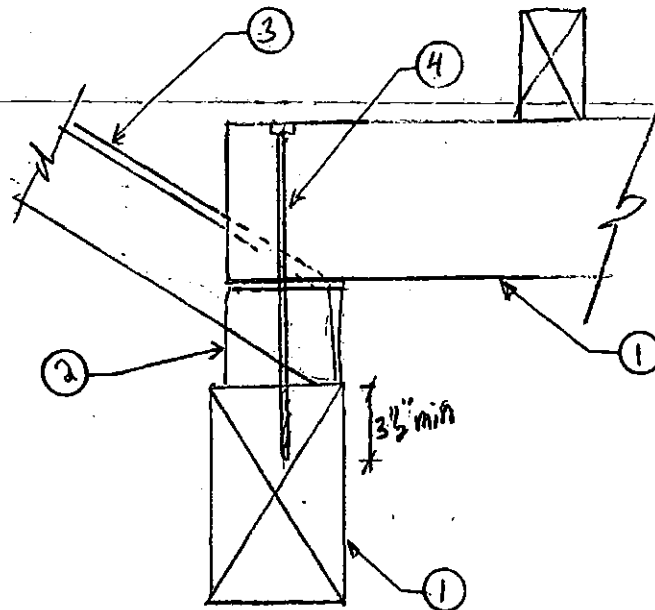
② (4) anchor bolts, use:

$\frac{1}{2}$ " DIA x 4" Tapcon⁺ by Redhead
or

$\frac{1}{2}$ " DIA x 4" Titen Screw Anchor
by Simpson Strong Tie

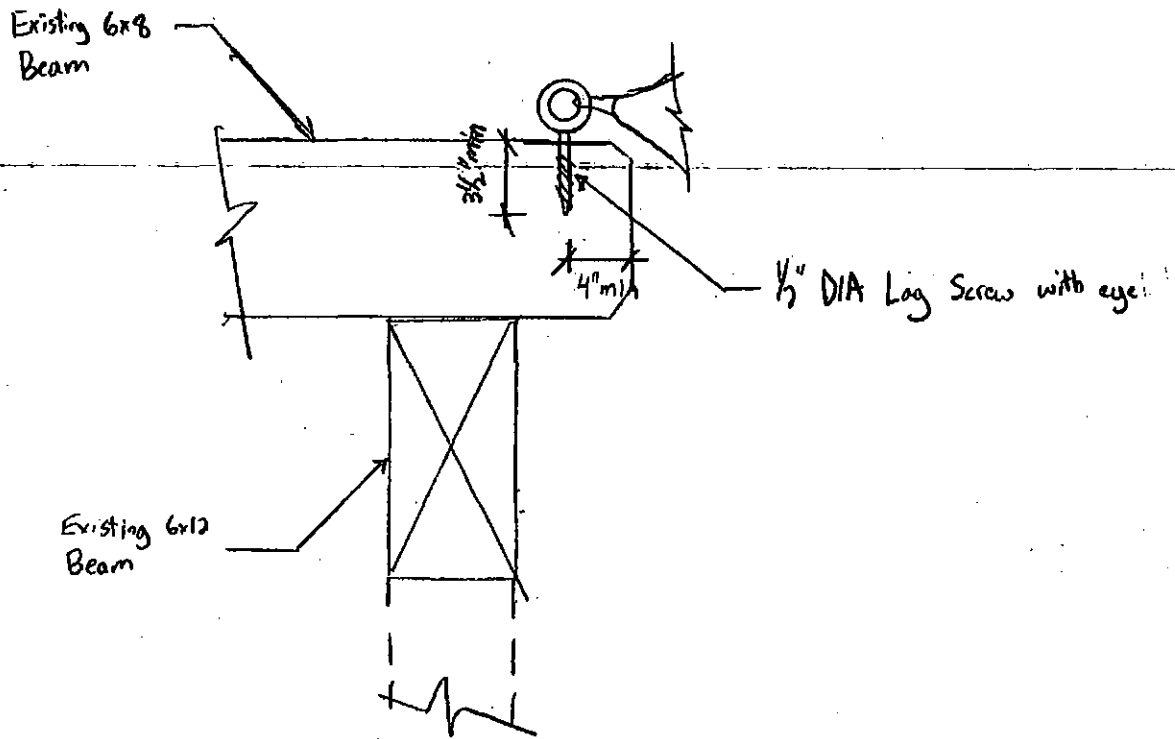


SK3



- ① Existing 6x beam
- ② Existing Shim Block
- ③ Existing roof sheathing and frame
- ④ 1/2" DIA lag screw with 3 1/2" minimum embedment into main member

SK4



SK5

PROJECT:

No:

SHEET:

5

2012 IBCDesign Wind Loads

$$P = q_h G C_N$$

C27.4-3

$$V_{ult} = 105 \text{ mph} \rightarrow (\text{based on category})$$

$$Exp = B$$

$$q_h = 0.00256 K_z K_{zt} K_d V^2 = 20.4 \text{ psf}$$

$$K_z = 0.85 \quad (\text{Table 27.3-1})$$

$$K_{zt} = 1.0 \quad (26.8.2)$$

$$K_d = 0.85 \quad (26.6-1)$$

$$G = 0.85 \quad (26.9)$$

$$C_N = \begin{matrix} 0.8 \\ -0.8 \end{matrix} \quad (\text{Table 27.4})$$

$$P = (20.4 \text{ psf})(0.85)(\pm 0.8) = 13.9 \text{ psf (ult)}$$

$$= 0.6 (13.9 \text{ psf}) = 8.3 \text{ psf (allowable)}$$

use 10.0 psf

DESCRIPTION : Smokehaus Fabric Shade Support
 3636 N Scottsdale Road
 Scottsdale, AZ

WIND LOADS

P = 10.0 psf (from previous sheet)

LOADS TO COLUMN

LOC.	A (FT^2)	YBAR (FT)	P (PSF)	F (K)	M (K-FT)
Sails	150.0	7.0	10.0	0.50	3.5
Pole seff	2.7	4.0	12.2	0.03	0.1

TOTAL 0.53 3.63
 YBAR (FT) : 6.82

STEEL COLUMN DESIGN:

Type HSS4x3x3/8 ← SK 2
 S 3.97 in^3
 Z 5.12 in^3
 I 7.93 in^4
 Fy 46 ksi
 Mu 5.8 kft
 Phi Mn 19.6 kft OK
 deflection 0.42 in
 L/ 199 OK

FOOTING DESIGN:

Pedestal H 6 ft
 Dia 2 ft
 W 2826 lb
 Mr 4.0 kft
 uplift 500 lb
 SF uplift 5.7 OK

Anchor Calculation Tapcon+ - Carbon 1/2"

Product information

Tapcon+ - Carbon

Material	Carbon
Type	Mechanical anchor
Approval	RedHead - Tapcon+, Sammys
Drill hole depth	4 1/4"
Nominal drill bit diameter	1/2"
Nominal anchor depth	4"
Effective anchor depth	3.02"



Material

Concrete (Normal Weight)	
Concrete Compressive Strength	2500 psi
Zone	Uncracked Concrete

Concrete Reinforcement	
Reinforcement of tension forces	No
Reinforcement of shear forces	No
Reinforcement to control splitting	No
Do not evaluate concrete breakout in tension	No
Do not evaluate concrete breakout in shear	No

Geometry

Anchor		
Anchor arrangement	Group of four without slotted holes	
Rotation	0 °	
Eccentricity		
Displacements	y	0.000 in
Displacements	z	0.000 in
Anchor spacings		
Anchor spacing	y ₁	7.000 in
Anchor spacing	z ₁	7.000 in
Edge Distances / Concrete Thickness		
Edge distance left		8.000 in
Edge distance right		8.000 in
Edge distance top		8.000 in
Edge distance bottom		8.000 in
Concrete thickness	h	36.000 in
Anchor plate dimensions		
Anchor plate width	y	10.000 in

ITW Commercial Construction, North America

 700 High Grove Blvd
 (P) 1-800-726-7386
 www.itwredhead.com

 Glendale Heights, IL 60139
 (F) 1-630-893-1291
 tech@itwccna.com


Anchor Calculation Tapcon+ - Carbon 1/2"

Anchor plate dimensions

Anchor plate length	z	10.000 in
Anchor plate thickness		0.250 in

Connected profile - Eccentricity

Displacements	y	0.000 in
Displacements	z	0.000 in

Load

Load

Tension	N_u	0 lbf
Shear	V_{ux}	848 lbf
Shear	V_{uy}	0 lbf
Bending moment	M_{ux}	0.0 lbf-ft
Bending moment	M_{uy}	0.0 lbf-ft
Bending moment	M_{uz}	5808.0 lbf-ft

Tension load conditions

Sustained Tension	No
-------------------	----

Shear load conditions

Use anchors with built-up grout pads	No
--------------------------------------	----

Load combination

Load factor	ACI 318 Chapter 9.2
-------------	---------------------

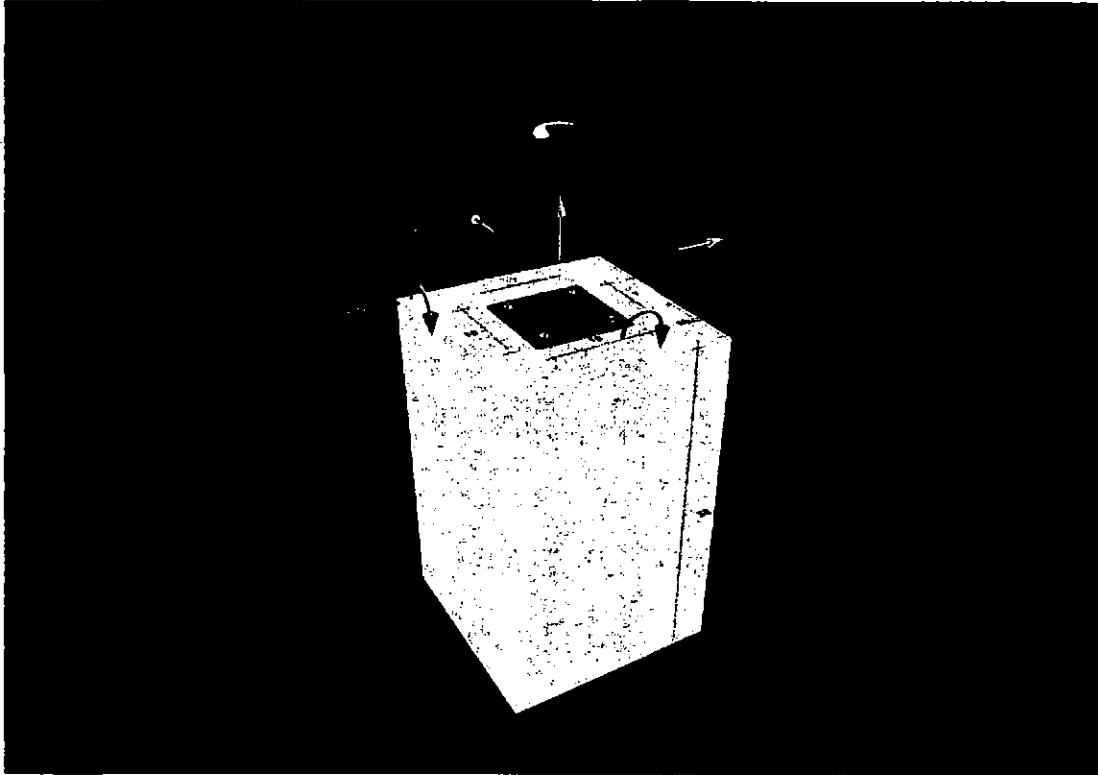
Seismic

Seismic category C, D, E or F	No
-------------------------------	----

Section forces

Anchor Nr.	Tension [lbf]	Shear [lbf]
1	4526 lbf	212 lbf
2	-491 lbf	212 lbf
3	4526 lbf	212 lbf
4	-491 lbf	212 lbf

Anchor Calculation Tapcon+ - Carbon 1/2"



Verifications ACI 318-11

Total capacity due to steel failure

$$\beta_N = \frac{N_{ua}}{\Phi N_{sa}} = \frac{4526 \text{ lbf}}{15031 \text{ lbf}}$$

	N_{ua} [lbf]	Φ	N_{sa} [lbf]	ΦN_{sa} [lbf]	β_N [%]
	4526	0.65	23125	15031	30.11

ITW Commercial Construction, North America

700 High Grove Blvd
(P) 1-800-726-7386
www.itwredhead.com

Glendale Heights, IL 60139
(F) 1-630-893-1291
tech@itwccna.com



Anchor Calculation Tapcon+ - Carbon 1/2"

Total capacity due to concrete failure (Controlling anchors: 1, 3)

$$N_b = k_c \cdot \lambda_a \cdot \sqrt{f'_c} \cdot h_{ef}^{1.5}$$

k_c	λ_a	f'_c [psi]	h_{ef} [in]	N_b [lbf]
30	1.00	2500	3.020	7872

$$N_{cb} = \frac{A_{Nc}}{A_{Nc0}} \cdot \psi_{ec,N} \cdot \psi_{ed,N} \cdot \psi_{c,N} \cdot \psi_{cp,N} \cdot N_b$$

A_{Nc} [in ²]	A_{Nc0} [in ²]	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$
145.504	82.084	1.000	1.000	1.000	1.000
N_b [lbf]	N_{cb} [lbf]				
7872	13955				

$$\beta_N = \frac{N_{ua}}{\phi N_{cb}} = \frac{9051 \text{ lbf}}{9071 \text{ lbf}}$$

N_{ua} [lbf]	ϕ	N_{cb} [lbf]	ϕN_{cb} [lbf]	β_N [%]
9051	0.65	13955	9071	99.79

Total capacity due to steel failure at shear force

$$\beta_V = \frac{V_{ua}}{\phi V_{sa}} = \frac{212 \text{ lbf}}{7566 \text{ lbf}}$$

V_{ua} [lbf]	ϕ	V_{sa} [lbf]	ϕV_{sa} [lbf]	β_V [%]
212	0.60	12610	7566	2.80

Anchor Calculation Tapcon+ - Carbon 1/2"

Total capacity due to concrete edge failure - Edge right (Controlling anchors: 2, 4)

$$V_b = 7 \cdot \left(\frac{h_{ef}}{d} \right)^{0.2} \cdot \lambda_a \cdot \sqrt{d} \cdot \sqrt{f_c} \cdot c_{a1}^{1.5}$$

	h_{ef} [in]	d [in]	λ_a	f_c [psi]	c_{a1} [in]	V_b [lbf]
	3.020	0.500	1.00	2500.00	8.000	8024

$$V_{cb} = \frac{A_{Vc}}{A_{Vc0}} \cdot \psi_{ec,V} \cdot \psi_{ed,V} \cdot \psi_{c,V} \cdot \psi_{h,V} \cdot V_b$$

	A_{Vc} [in ²]	A_{Vc0} [in ²]	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$
	276.000	288.000	1.000	0.900	1.400	1.000
	V_b [lbf]	V_{cb} [lbf]				
	8024	9689				

$$\beta_V = \sqrt{\left(\frac{V_{ua} \cdot \cos \alpha}{\Phi V_{cb}} \right)^2 + \left(\frac{V_{ua} \cdot \sin \alpha}{2 \Phi V_{cb}} \right)^2}$$

	V_{ua} [lbf]	α [°]	Φ	V_{cb} [lbf]	ΦV_{cb} [lbf]	β_V [%]
	848	0.00	0.70	9689	6782	12.50

Anchor Calculation Tapcon+ - Carbon 1/2"

Concrete pryout failure - Shear - Anchor group (Controlling anchors: 1, 2, 4, 3)

$$N_b = k_c \cdot \lambda_a \cdot \sqrt{f_c} \cdot h_{ef}^{1.5}$$

k_c	λ_a	f_c [psi]	h_{ef} [in]	N_b [lbf]
30	1.00	2500	3.020	7872

$$N_{cbg} = \frac{A_{Nc}}{A_{Nc0}} \cdot \psi_{ec,N} \cdot \psi_{ed,N} \cdot \psi_{c,N} \cdot \psi_{cp,N} \cdot N_b$$

A_{Nc} [in]	A_{Nc0} [in]	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$
257.924	82.084	1.000	1.000	1.000	1.000
N_b [lbf]	N_{cbg} [lbf]				
7872	24736				

$$V_{cpg} = k_{cp} \cdot N_{cbg}$$

k_{cp}	N_{cbg} [lbf]	V_{cpg} [lbf]
2.00	24736	49473

$$\beta_N = \frac{V_{ua}}{\Phi V_{cpg}} = \frac{848 \text{ lbf}}{34631 \text{ lbf}}$$

V_{ua} [lbf]	Φ	V_{cpg} [lbf]	ΦV_{cpg} [lbf]	β_N [%]
848	0.70	49473	34631	2.45

The selected Anchor is applicable.

Anchor Calculation Tapcon+ - Carbon 1/2"

Hints

Notes about the calculations:

The following documents are referenced for the verification of the anchor load-bearing capacity:

- Anchor approval

The following assumptions are made for the calculations:

- The selected building material class has been verified
- all of the anchors in a group are of the same type and size
- The anchor plate remains plane during loading

The verification of the local transfer of loads into the anchorage material has been performed. The transfer of these loads to the rest of the structure must be shown.

The design is based on numerous anchor-specific values. If the selected anchor is substituted for another or if the input values are changed, the design must be repeated. Additional requirements of the anchor approvals have to be taken into account, especially if the anchors are loaded dynamically.

THIS SOFTWARE APPLICATION AND THE RESULTS DERIVED FROM ITS UTILIZATION ARE INTENDED ONLY FOR USE BY PROFESSIONAL USERS WITH EXPERT KNOWLEDGE IN THE AREA OF THE INTENDED APPLICATION. USERS MUST INDEPENDENTLY VERIFY THE RESULTS BEFORE ANY USE AND TAKE INTO ACCOUNT THE SITE AND APPLICATION CONDITIONS, PRODUCT INFORMATION AND LITERATURE, TECHNICAL STATE OF THE ART AS WELL AS LOCAL APPLICABLE STANDARDS AND REGULATIONS.

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 Version 2.4.5673.2

Company:		Date:	1/28/2016
Engineer:		Page:	1/4
Project:			
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
 Customer contact name:
 Customer e-mail:
 Comment:

Project description:
 Location:
 Fastening description:

2. Input Data & Anchor Parameters**General**

Design method: ACI 318-11
 Units: Imperial units

Anchor Information:

Anchor type: Concrete screw
 Material: Carbon Steel
 Diameter (inch): 0.500
 Nominal Embedment depth (inch): 4.000
 Effective Embedment depth, h_{ef} (inch): 2.990
 Code report: ICC-ES ESR-2713
 Anchor category: 1
 Anchor ductility: No
 h_{min} (inch): 6.25
 c_{ac} (inch): 4.50
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Base Material

Concrete: Normal-weight
 Concrete thickness, h (inch): 36.00
 State: Uncracked
 Compressive strength, f_c (psi): 2500
 Ψ_{cv} : 1.4
 Reinforcement condition: A tension, A shear
 Supplemental reinforcement: No
 Reinforcement provided at corners: No
 Do not evaluate concrete breakout in tension: No
 Do not evaluate concrete breakout in shear: No
 Ignore 6do requirement: Not applicable
 Build-up grout pad: No

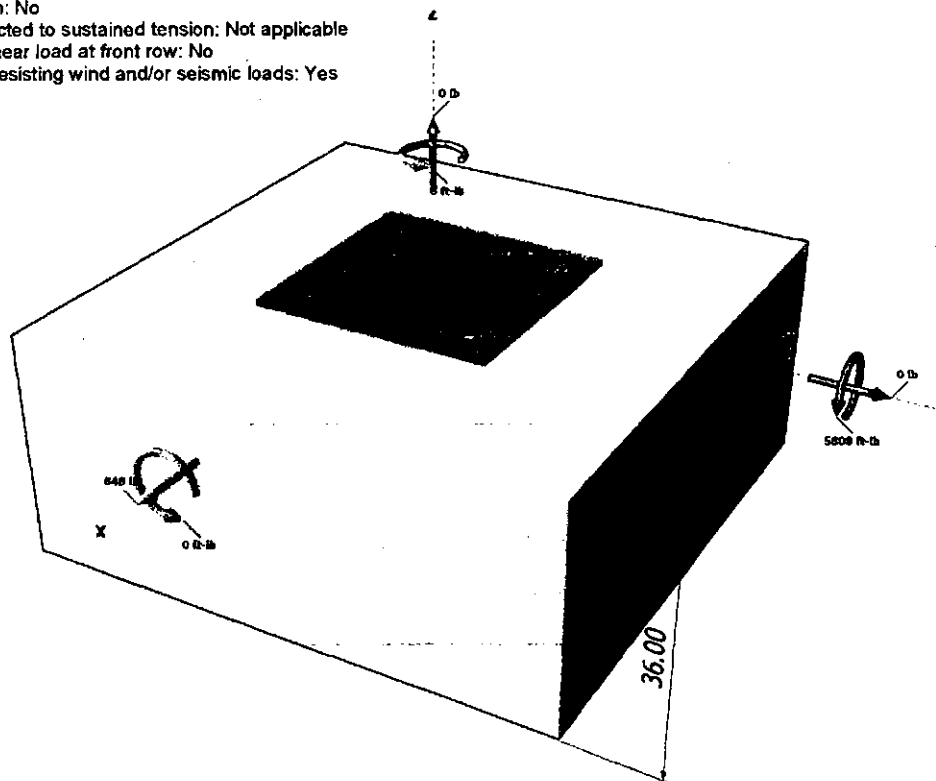
Base Plate

Length x Width x Thickness (inch): 10.00 x 10.00 x 0.25

Load and Geometry

Load factor source: ACI 318 Appendix C
 Load combination: not set
 Seismic design: No
 Anchors subjected to sustained tension: Not applicable
 Apply entire shear load at front row: No
 Anchors only resisting wind and/or seismic loads: Yes

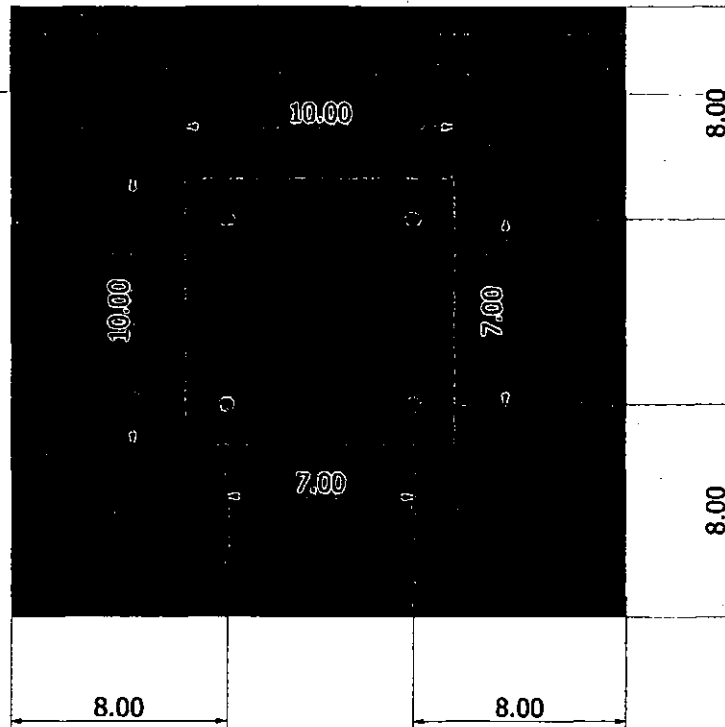
<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.
 Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com

Company:		Date:	1/28/2016
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Project:			
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: Titen HD® - 1/2"Ø Titen HD, hnom:4" (102mm)
 Code Report: ICC-ES ESR-2713

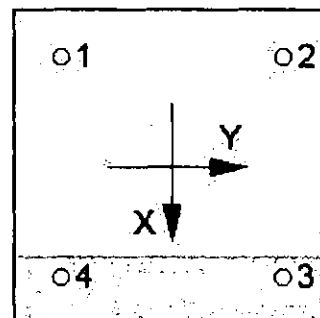


3. Resulting Anchor Forces

Anchor	Tension load, N _{ux} (lb)	Shear load x, V _{ux} (lb)	Shear load y, V _{uy} (lb)	Shear load combined, $\sqrt{(V_{ux})^2 + (V_{uy})^2}$ (lb)
1	4481.0	212.0	0.0	212.0
2	4481.0	212.0	0.0	212.0
3	0.0	212.0	0.0	212.0
4	0.0	212.0	0.0	212.0
Sum	8962.0	848.0	0.0	848.0

Maximum concrete compression strain (ϵ_c): 0.19
Maximum concrete compression stress (psi): 826
Resultant tension force (lb): 8962
Resultant compression force (lb): 8962
Eccentricity of resultant tension forces in x-axis, e'_{nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	ϕ	ϕN_{sa} (lb)
20130	0.70	14091

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$N_b = k_c \lambda_a \sqrt{f_c} h_{ef}^{1.5}$ (Eq. D-5)

k _c	λ_a	f _c (psi)	h _{ef} (in)	N _b (lb)
24.0	1.00	2500	2.990	6204

$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b$ (Sec. D.4.1 & Eq. D-4)

A _{Nc} (in ²)	A _{Nco} (in ²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N _b (lb)	ϕ	ϕN_{cbg} (lb)
143.25	80.46	1.000	1.000	1.00	1.000	6204	0.85	9389

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V _{sa} (lb)	ϕ_{gross}	ϕ	$\phi_{gross} \phi V_{sa}$ (lb)
7455	1.0	0.65	4846

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$V_{bx} = \min[7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}]$ (Eq. D-33 & Eq. D-34)

l _e (in)	d _a (in)	λ_a	f _c (psi)	c _{a1} (in)	V _{bx} (lb)
2.99	0.50	1.00	2500	15.00	20560

$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{bx}$ (Sec. D.4.1 & Eq. D-31)

A _{Vc} (in ²)	A _{Vco} (in ²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V _{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
517.50	1012.50	1.000	0.807	1.400	1.000	20560	0.85	10088

Shear parallel to edge in x-direction:

$V_{by} = \min[7(l_e / d_a)^{0.2} \sqrt{d_a} \lambda_a \sqrt{f_c} c_{a1}^{1.5}; 9 \lambda_a \sqrt{f_c} c_{a1}^{1.5}]$ (Eq. D-33 & Eq. D-34)

l _e (in)	d _a (in)	λ_a	f _c (psi)	c _{a1} (in)	V _{by} (lb)
2.99	0.50	1.00	2500	8.00	8008

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$$\phi V_{cbgr} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 \& Eq. D-31)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgr} (lb)
276.00	288.00	1.000	1.000	1.400	1.000	8008	0.85	18265

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi k_{cp} N_{cbg} = \phi k_{cp} (A_{nc} / A_{nco}) \psi_{ec,n} \psi_{ed,n} \psi_{c,n} \psi_{cr,n} N_b \text{ (Eq. D-41)}$$

k_{cp}	A_{nc} (in ²)	A_{nco} (in ²)	$\psi_{ec,n}$	$\psi_{ed,n}$	$\psi_{c,n}$	$\psi_{cr,n}$	N_b (lb)	ϕ	ϕV_{cpq} (lb)
2.0	255.04	80.46	1.000	1.000	1.000	1.000	6204	0.75	29499

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_u (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	4481	14091	0.32	Pass	
Concrete breakout	8962	9389	0.95	Pass (Governs)	
Shear	Factored Load, V_u (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	212	4846	0.04	Pass	
T Concrete breakout x+	848	10088	0.08	Pass (Governs)	
Concrete breakout y-	424	18265	0.02	Pass (Governs)	
Pryout	848	29499	0.03	Pass	
Interaction check	$N_u/\phi N_n$	$V_u/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.95	0.00	95.5 %	1.0	Pass

1/2"Ø Titen HD, hnom:4" (102mm) meets the selected design criteria.

12. Warnings

- Minimum spacing and edge distance requirement of 6da per ACI 318 Sections D.8.1 and D.8.2 for torqued cast-in-place anchor is waived per designer option.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.

SK4

Shear transfer of 500#

Use $\frac{1}{2}$ " DIA lag screw with 3" min embedment into main member
capacity = 631 # ok (per attached NDS calculation)

SK5

Shear transfer of 500#

Use eye bolt with $\frac{1}{2}$ " dia threaded portion w/ $3\frac{1}{2}$ " min embedment
ok by comparison w/ SK4

WOOD FASTENERS

<table border="0"> <tr> <th style="text-align: left;">MEMBER THICKNESS</th> <th></th> </tr> <tr> <td>SIDE</td> <td>6"</td> </tr> <tr> <td>MAIN</td> <td>8.0"</td> </tr> </table>	MEMBER THICKNESS		SIDE	6"	MAIN	8.0"	<table border="0"> <tr> <th style="text-align: left;">MATERIAL</th> <th style="text-align: left;">F_e (PSI)</th> <th style="text-align: left;">θ</th> <th style="text-align: left;">A (IN²)</th> <th style="text-align: left;">E (PSI)</th> </tr> <tr> <td>Douglas Fir-Larch</td> <td>5,600</td> <td>0</td> <td></td> <td></td> </tr> <tr> <td>Douglas Fir-Larch</td> <td>5,600</td> <td>0</td> <td></td> <td></td> </tr> </table>	MATERIAL	F _e (PSI)	θ	A (IN ²)	E (PSI)	Douglas Fir-Larch	5,600	0			Douglas Fir-Larch	5,600	0			<p style="text-align: center;">A & E FOR MULT. FASTENERS CALC. ONLY</p>																												
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