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Wastewater Study

Stormwater Waiver Application

**A Class III Cultural Resources Survey of 91 Acres for DM19, LLC
at Desert Mountain near Cave Creek Road and Pima Road,
Scottsdale, Maricopa County, Arizona**

Prepared for:

DM19, LLC

Prepared by:

Rebecca Hill, M.A., RPA



Logan Simpson
51 West Third Street, Suite 450
Tempe, AZ 85281

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LSD Technical Report No. 165088

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ABSTRACT AND MANAGEMENT SUMMARY

Report Title	A Class III Cultural Resources Survey of 91 Acres for DM19, LLC at Desert Mountain near Cave Creek Road and Pima Road, Scottsdale, Maricopa County, Arizona
Report Date	March 15, 2016 (Submittal #1)
Agencies Involved	N/A
Land Ownership	Private, Desert Mountain Home Owner's Association
LSD Project No.	165088
Project Description	DM19, LLC requested Logan Simpson conduct a Class III archaeological survey of the 91-acre Desert Mountain parcel as part of the due diligence process prior to a proposed purchase of the parcel. The Desert Mountain parcel is zoned for residential housing.
Project Location	Within the City of Scottsdale, Arizona, along Cave Creek Road and near Pima Road. Within portions of the NW¼ and the N½ of the SW¼ of Section 31, T6N, R5E, Gila and Salt River Baseline and Meridian (USGS 7.5' Cave Creek, Ariz., 1965/1981).
Project UTM's	NAD83 Northwest corner – 417533 mE, 3743129 mN Zone 12 Northeast corner – 417827 mE, 3743365 mN Southeast corner – 418350 mE, 3742868 mN Southwest corner – 417541 mE, 3742678 mN
Fieldwork Dates	March 10, 2014
Acres Surveyed	91
Methods	Pedestrian survey spaced at 15-m intervals
Number of Cultural Resources	1 previously recorded site and 9 isolated occurrences (IOs)
Register eligibility Indeterminate sites	1 AZ U:1:433(ASM)

Summary

The Class III cultural resources survey of the 91-acre parcel resulted in the identification of 1 previously recorded site—AZ U:1:433(ASM). In addition to the site, 9 IOs were identified. The IOs are not eligible for inclusion in the Arizona Register of Historic Places (ARHP) or National Register of Historic Places (NRHP), and no additional research or preservation of the IOs is required.

A formal recommendation of eligibility for AZ U:1:433(ASM) could not be made based on surface observations. An eligibility recommendation would only be possible following the implementation of an archaeological testing program. If avoidance is not possible AZ U:1:433(ASM) should be subjected to an appropriate archaeological testing program to determine whether buried deposits are present and to formally evaluate its NRHP eligibility.

If previously unrecorded cultural resources are encountered during ground-disturbing activities, these activities must be discontinued in the immediate vicinity of the discovery, and work should not resume until a qualified archaeologist has been notified and allowed time to properly address the nature and significance of the discovery.

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INTRODUCTION

DM19, LLC requested that Logan Simpson perform a Class III cultural resources survey of the approximate 91-acre Desert Mountain parcel as due diligence for a proposed private development project. The parcel is situated at the western edge of the Desert Mountain development (Photograph 1), which is located north of the intersection of Pima and Cave Creek roads in north Scottsdale, Maricopa County (Figure 1). The survey area is an irregular polygon that measures approximately 0.5 mile north-south and 0.5 mile east-west and is within Section 31, T6N, R5E, Gila and Salt River Baseline and Meridian (G&SRB&M) (USGS 7.5' Cave Creek 1965/1981) (Figure 2). Five pumping stations and an existing fire station within the parcel are excluded from the survey area.

All land within the project area is private property owned by the Desert Mountain Home Owner's Association. DM19, LLC is considering purchase of the parcel for future development. As such, the cultural resources survey was conducted in voluntary compliance with the Arizona Burial Law (A.R.S. §41-844 and §41-865), and as due diligence prior to purchase of the property.



Photograph 1. Representative overview of the project area, view to north.

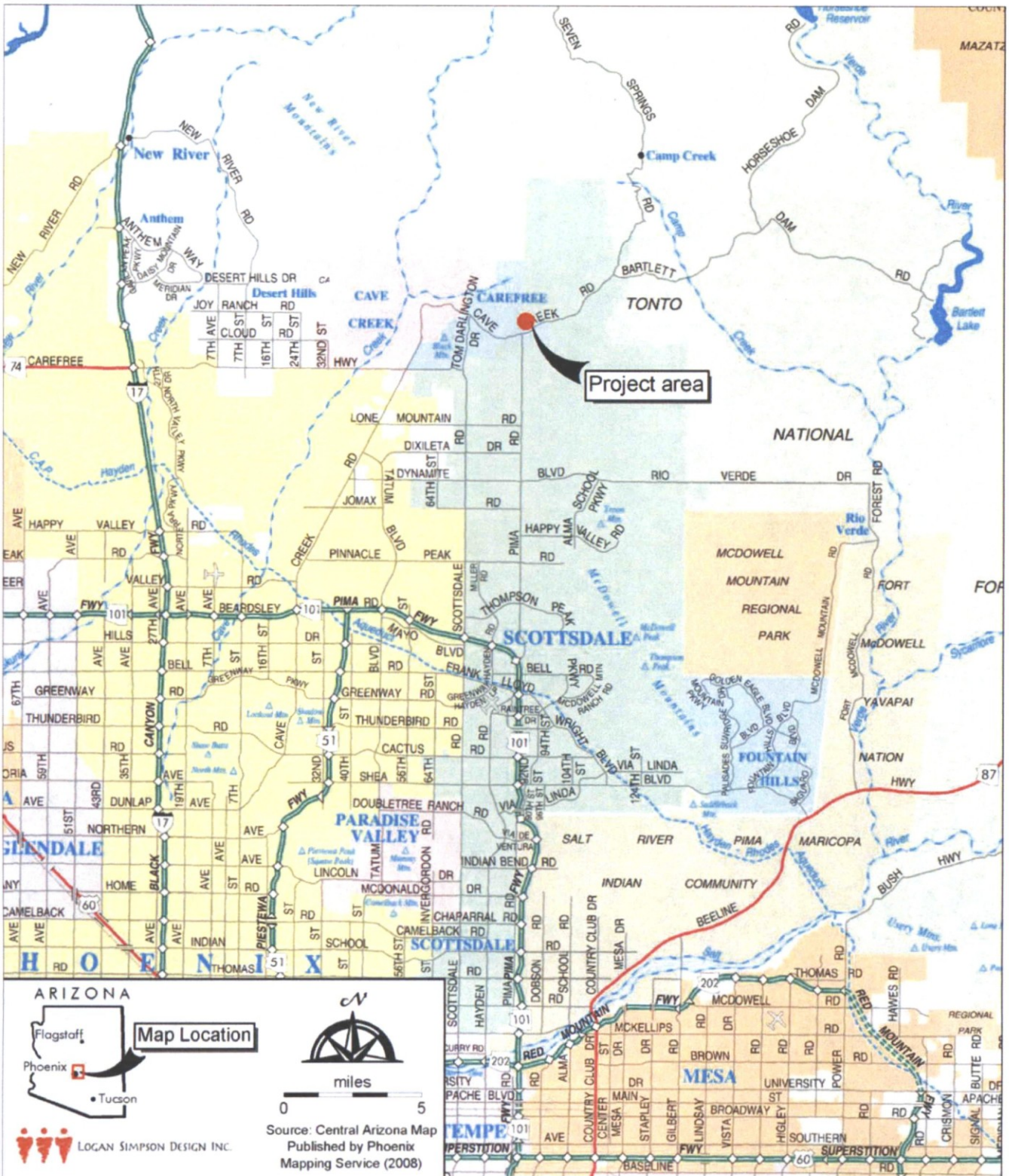
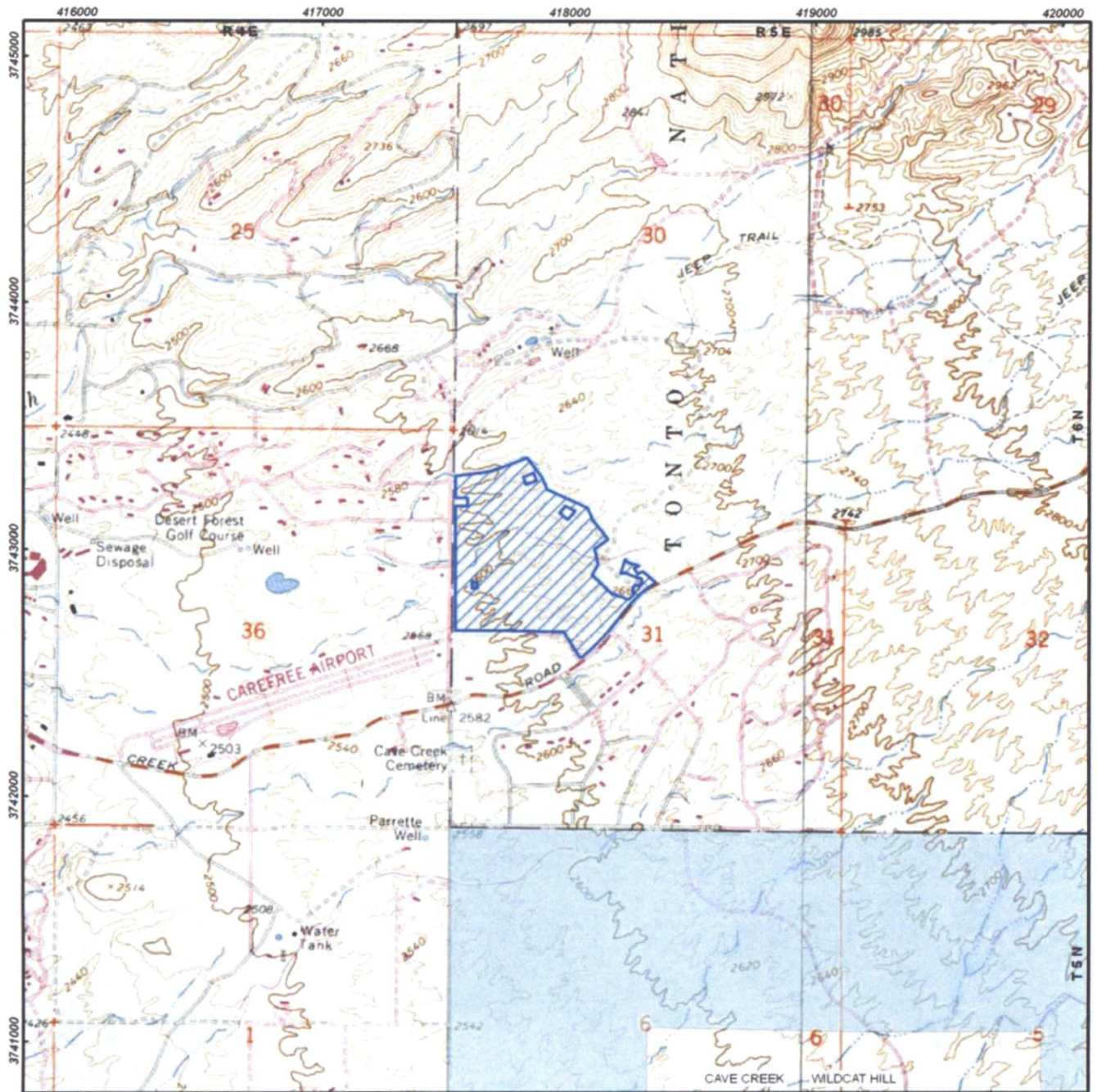


Figure 1. Project location.



Source: USGS 7.5' Quadrangles
 Cave Creek, Ariz. (1965, 1981).
 Wildcat Hill, Ariz. (1965, 1981)
 NAD 1983, UTM Zone 12

Key		Land Jurisdiction	
	Survey area		Private Land
			State Trust Land

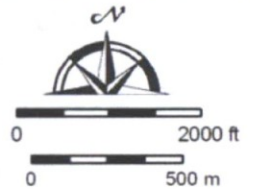


Figure 2. Location of survey area and land jurisdiction.

PHYSICAL SETTING

The project area is situated at an approximate elevation of 2,600 ft above mean sea level (amsl) and is located in the Basin and Range physiographic province, which is characterized by low desert surrounded by fault-block mountain ranges (Chronic 1983). The region is part of the Lower Colorado River Valley subdivision of the Sonoran Desertscrub biotic community (Turner and Brown 1994), which has high temperatures and generally low precipitation. The vegetation in the area includes yucca, palo verde trees, mesquite trees, saguaro, ocotillo, and cholla. The largest drainage in the vicinity is an unnamed branch of the Galloway Wash that runs east-west through the center of the project area. Topographic features surrounding the project area include Lone Mountain approximately 1.5 miles to the northeast and Black Mountain approximately 3 miles to the southwest. The local geology consists of pre-Cambrian granite and metasedimentary rocks and Cenozoic alluvial deposits. Ground visibility was fair, averaging 50 to 75 percent open.

CULTURAL HISTORY

Human presence in the Southwest began as long as 11,000 years ago. The initial period of occupation, during the Paleoindian period dating from approximately 9500 B.C. to 8500 B.C., appears to have been intermittent, given the limited amount of recovered evidence. The evidence consists primarily of isolated surface finds of Clovis points, as well as buried megafaunal kill sites in alluvial contexts that have yielded associated lithic assemblages (Haynes 1980). Based on these scant data, the period appears to be characterized by dispersed mobile groups that primarily hunted now-extinct megafauna and possibly supplemented their diet with collected wild plant materials (Waters 1986). Although only a few artifactual surface finds have been reported in the general region (Agenbroad 1967; Huckell 1982; North et al. 2004; North et al. 2005), it is likely that most Paleoindian period remains are currently buried by substantial Holocene alluvial deposits or destroyed by erosion. Three isolated points have been found in the northern periphery of the Phoenix Basin (Crownover 1994; Huckell 1982; North et al. 2005), representing the only known evidence, to date, of Paleoindian occupation or use of the Phoenix Basin.

Following climatic amelioration and the extinction of the previously exploited fauna, a new cultural pattern emerged, the Southwestern Archaic, manifested by small, mobile, residential groups that hunted medium-sized to small game and foraged for a diversity of floral resources. This adaptive pattern persisted through the Early (8500 B.C. to 5000 B.C.) and Middle (5000 B.C. to 1500 B.C.) Archaic periods, although there was a trend toward an increased reliance on migratory patterns based on seasonally available foodstuffs, as evidenced by the increased prevalence of grinding tools in the artifact assemblage. By the Late Archaic period (1500 B.C. to A.D. 1/150), some of the Archaic groups occupied well-watered upland locations or locales along primary or secondary stream courses where crops could be planted. In these locations they adopted maize horticulture, maintained substantial storage facilities, and established a semisedentary subsistence-settlement pattern (Huckell 1995; Mabry 1998). Around 500 B.C. large, seasonally occupied villages, some with communal structures, were established in some areas of southern Arizona (Mabry 1998). These villages centered around floodplain maize agriculture and riparian resources but continued the exploitation of upland bajada resources. In nonriverine desert areas, however, the tradition of hunting and foraging, accompanied by increased sedentism, appears to have persisted at least through the end of

the Archaic period (Halbirt and Henderson 1993). Late Archaic semisedentary settlements, such as those identified in upland and riverine areas to the south in the Tucson Basin and parts of southeastern Arizona (Huckell 1995; Mabry 1998; Roth 1992), have yet to be identified in the Phoenix Basin. If present, these settlements undoubtedly would be situated in floodwater agriculture areas along river terraces that are currently deeply buried in alluvium.

The succeeding Early Formative period, dating between A.D. 1 and approximately A.D. 700, is characterized primarily by the introduction and early development of plain ware ceramics. This period can be considered a period of transition, during which sedentism and the reliance on horticulture increased throughout the general region. In specific areas, such as the Tucson Basin where both Late Archaic and Early Formative villages have been recorded, settlement location reflects a general continuity from the Late Archaic period. Current understanding of the initial phase of the early Formative period in the Salt-Gila River area, the Red Mountain phase (A.D. 1 to 450), however, is limited to data derived from a few sites and site components in the Phoenix Basin (Mabry 2000). The Red Mountain phase—which is represented by the earliest component at Pueblo Patricio (Cable and Doyel 1987; Henderson 1995), La Escuela Cuba (Hackbarth 1992), the Red Mountain site (Morris 1969), and limited activity sites (Hackbarth 1998; Kenny 1987; Phillips et al. 2001; Rogge 2009) is characterized by groups of small pit houses of varying configurations that composed small semisedentary hamlets. The mortuary pattern is characterized principally by flexed inhumations, although secondary cremations have also been documented (Mabry 2000).

The Vahki, Estrella, and Sweetwater phases, along with the Snaketown phase, traditionally make up the Pioneer period in Hohokam cultural chronology (Gladwin et al. 1937; Haury 1976). The Pioneer period has undergone some recent reevaluation regarding the origins and development of the Hohokam (Dean 1991; Wallace et al. 1995). The placement of these phases (A.D. 450 to 700) is based on the available limited excavation and artifact data and can best be characterized as a continuation of the broad regional Early Formative-period cultural development, with the beginnings of a cultural pattern that was developing in the Phoenix Basin.

During the Vahki phase, both micaceous plain ware and red ware ceramics were produced and a figurine complex developed. Other characteristics of this phase include settlements with plaza-oriented layouts, the construction of large Type P-4 houses that were first identified at Snaketown (Gladwin et al. 1937; Haury 1976:68; Wilcox et al. 1981), and a mortuary pattern that incorporated both pit and trench cremations and flexed and semiflexed inhumations (Doyel 1991). The following Estrella and Sweetwater phases are characterized primarily by the production of grooved and decorated red-on-gray ceramics. Although the square P-4 houses continued to be constructed, they were smaller in size than those of the Vahki phase and co-occurred with the smaller structures. The presence of some intrusive elements, including evidence of macaws and parrots, shell, and turquoise at principal settlements such as Snaketown, suggests the development of regional interaction patterns.

The emergence of the Hohokam as an integrated cultural pattern occurred possibly as early as A.D. 700 during the Snaketown phase, although a much earlier origin beginning in the Vahki phase originally had been proposed (Gladwin et al. 1937). Recent assessments have suggested that the suite of cultural traits and developments that marked the beginnings of regional cultural differentiation and that characterized the Hohokam does not appear to be fully established until the Snaketown phase or possibly as late as the middle of the Gila Butte phase of the Colonial period, beginning around A.D. 750 (Dean 1991; Doyel 1991; Wallace et al. 1995; Wilcox 1979; Wilcox and Sternberg 1983). These traits, which reflect the development of an integrated belief and ritual system and the inception of a regional system, include the adoption of public architectural forms, such as ballcourts, into the settlement structure and the development of a characteristic mortuary complex, large-scale irrigation agriculture, and naturalistic iconography in decorated ceramics.

Throughout the pre-Classic period, between A.D. 700 and approximately A.D. 1150, the Phoenix Basin can be considered the primary focus of Hohokam regional development. During the Snaketown phase of the Pioneer period, the emerging Hohokam cultural pattern was manifested by the first documented construction of canals (Henderson 1989; Wilcox and Shenk 1977) and urn burials (Haury 1976). Trash mounds appeared during this period, and one at Snaketown was capped with caliche, possibly a precursor to the later platform mounds (Haury 1976). Evidence of Hohokam occupation or interaction is first identified outside the Phoenix Basin in locations such as the lower Verde River and in the Tucson Basin (Crown 1991).

The establishment of numerous villages throughout the region, including peripheral areas along secondary drainages where canal irrigation was not feasible, characterizes the Colonial period. Habitation sites consisting of courtyard groups focused on a common living or workspace represented a frequent pattern of settlement organization during this period (Howard 1985; Wilcox et al. 1981). At smaller hamlets and villages consisting of one or two courtyard groups, trash mounds, cemetery areas, and roasting pits tended to be arrayed around the margins of the courtyard. At larger villages composed of clusters of courtyard groups, central plazas and communal cemeteries and work areas were probably incorporated into the village structural layout (Howard 1985; Wilcox and Sternberg 1983). The introduction of ballcourts at some of these villages, at least by the Gila Butte phase, suggests the beginnings of site functional differentiation and intercommunity integration. Ballcourts, as a widespread form of public architecture, increased in number and expanded in areal extent throughout the Colonial period. By the Sedentary period, ballcourts were represented not only in the Phoenix Basin but in surrounding areas as well.

The Santa Cruz phase of the Colonial period and the Sacaton phase of the Sedentary period were times of substantial growth in the number and size of Hohokam settlements and ballcourt villages and in the extent of the canal networks in the Phoenix Basin (Doyel 1991). In peripheral drainage areas, the number of villages, hamlets, and farmsteads also increased. Nonirrigation agricultural intensification and the intensive use of agricultural rock piles for the cultivation of crops, such as agave and cholla, appear to have developed at least by the late Sedentary or early Classic periods (Fish et al. 1992; Masse 1991).

By the beginning of the early Classic period, change in the structure of Hohokam communities is evidenced by a shift in burial practices from primarily cremations to inhumations, a change in regional exchange networks as reflected by the shift in the production and distribution of ceramic types and exotic materials, and the development of new domestic and public architectural forms, including post-reinforced and adobe-walled structures and walled compounds. The decline and eventual collapse of the ballcourt system and the development of the platform mound also occurred during this period (Gregory 1987). The platform mound architectural form, developed during the late Sedentary period, represented an important architectural component of a new community organization that was manifested in Hohokam settlements not only in the Phoenix Basin but also in other settlements over a much wider region, including the Tonto and Tucson basins and along the lower San Pedro River. The platform mound apparently evolved in function from an initial nonresidential, special-purpose facility to a residence used by a specific residential group (Gregory 1991). More recent study of the Pueblo Grande platform mound directly challenges the idea that the Civano-phase mounds were the full-time residences of elite households and further supports the proposition that power was diffuse at that time (see Downum and Bostwick 2003).

In conjunction with this Classic-period community restructuring, a hierarchy of settlement types also emerged. These included villages with only one or a few walled residential compounds; villages with one or more platform mound compounds as well as other compounds; and large settlements, such as Casa Grande, with a platform mound and numerous compounds, a ballcourt, and a Great House (Wilcox 1991). These various Classic-period settlements may have composed distinct irrigation communities: sociopolitical organizations consisting of a number of integrated villages that included one or more platform mound villages serving as administrative centers and distributed along a single canal or canal system (Howard 1987). One view of the terminal period of prehistoric occupation in the Phoenix Basin is the tentatively defined and dated Polvorón phase (Doyel 1995; Sires 1983). As originally defined, occupation during the Polvorón phase is represented by dispersed ranchería settlements, consisting of individual pit structures or jacal rooms arranged in clusters. In some cases, however, late Classic-period compounds were reoccupied (Crown and Sires 1984; Doyel 1995; Sires 1983). This perspective treats the phase as a period of abrupt change in community organization and integration in the aftermath of the collapse of the late Classic-period platform mound communities. The end of the Hohokam is explained as following a period of drought and flood conditions that substantially reduced or destroyed the irrigation systems on which these communities relied (Doyel 1995; Nials et al. 1989). However, researchers continue to debate whether the phase is valid. For example, Henderson and Hackbarth (2000), on the basis of overlapping dates between the Civano and Polvorón phases, argue that the characteristics of the latter are not temporally discrete but rather a reflection of cultural variability within the Classic period. Alternatively, Chenault (2000:277) argues “. . . that not to separate Polvorón from the Civano phase obscures variability and change at the end of the cultural sequence that may relate to the nature and causes of the Hohokam collapse.” Hill and colleagues (2004), on the other hand, take a broader perspective of regional Hohokam settlement change and suggest that population decline over multiple generations was intertwined with immigration and community coalescence.

By the time of Spanish contact in the mid- to late sixteenth century, the Pima and Maricopa occupied the middle portion of the Gila River. The Pima have been traditionally considered the descendants of the Hohokam in the Phoenix Basin (Doyel 1991; Haury 1976), although the validity of this particular prehistoric-

historic connection has been debated (Doelle 1981; Masse 1991). The mountainous areas north and west of the Salt River were largely occupied by the Yavapai. Gifford (1932, 1936) considered the Yavapai most closely aligned in terms of cultural traits with the upland Yuman Walapai and Havasupai of northwestern Arizona.

Euro-American incursion into the area occurred after 1846 as a result of the Mexican-American War and its aftermath, with incursions of the military, explorers, surveyors, immigrants, and finally settlers. The war ended in 1848 with the signing of the Treaty of Guadalupe Hidalgo. The American era (A.D. 1853–1950) began with the Gadsden Purchase of 1853, when modern-day southern Arizona became part of the United States. The late 1800s saw an influx of settlement into the Salt River Valley, encouraged by a series of national public land laws, such as the National Homestead Act (1862), Timber Culture Act (1873), Desert Land Act (1877), and Enlarged Homestead Act (1909) (Bostwick and Rice 1987; Stein 1990). The majority of homesteads filed in Arizona during this period were along the Salt River (Stein 1990). By the 1870s, many settlers in the area were extensively cultivating land (Arizona Board of Regents 1989). President Roosevelt signed the Reclamation Act of 1902, creating the first national effort to build large-scale irrigation projects in the western United States. Two dams along the Salt River (Granite Reef and Roosevelt) and an extensive canal network in the Phoenix Basin became our nation's flagship reclamation project. The irrigation and electricity provided by this, the Salt River Project, was instrumental in the development of Phoenix during the twentieth century (Zarbin 1986, 1997).

PREVIOUS RESEARCH

Before field survey, archaeological site files and inventory reports were checked at the Arizona State Historic Preservation Office (SHPO) and the Arizona State Museum (ASM) using AZSITE, the state's electronic inventory of cultural resources. The National Register Information System database and City of Scottsdale (COS) Register were also reviewed, and Bureau of Land Management (BLM) maps and title plats were reviewed electronically. The parameters of the record search included the project area and the surrounding one-mile radius. No COS Register or National Register of Historic Places (NRHP)-listed properties are located nearby. The available General Land Office (GLO) maps showing Section 31, T6N, R5E (plats 0234-A and 0234-B, filed 5/9/1960 and 5/24/1961, respectively), shows the Cave Creek Road alignment and a powerline within the project's vicinity, but not within the area to be surveyed.

The records search indicated that 24 projects were previously conducted in the project area and its vicinity (Table 1; Appendix A.1). Of these, 6 surveys occurred within the current project area (Davis 2003a, 2003b; Lausten 2004, Lundin 2001, 2002; Webb and Courtright 2002). Few cultural resources were encountered; however, one site, AZ U:1:433(ASM), was previously recorded in the project area during an 8-acre survey (Lausten 2004).

AZ U:1:433(ASM) is one of four sites that have been recorded in the project vicinity (Table 2; Appendix A.1 and A.2). The site was recorded as a 10 m by 17 m Hohokam artifact scatter consisting of ceramics and flaked stone. No ARHP and NRHP eligibility recommendation was made based on the surface

Table 1. Previous investigation in the project area vicinity.

Reference number	Author and year	Location relative to project area
1979-137.ASM	N/A	Outside
1986-123.ASM	ASM 1986	Outside
1987-243.ASM	RECON 1987	Outside
1990-18.ASM	Irwin 1990	Outside
1995-90.ASM	Crownover 1995	Outside
1997-67.ASM	Adams 1997	Outside
1999-262.ASM	Hackbarth 1999	Outside
2000-31.ASM	Hart 2000	Outside
2000-561.ASM	Lindly and Mitchell 2000	Outside
2001-448.ASM	Hart 2001	Outside
2001-687.ASM	Lundin 2001	Inside
2002-211.ASM	Marshall 2002	Outside
2002-389.ASM	Lundin 2002	Inside
2003-542.ASM	Gage 2003	Outside
2003-1102.ASM	Webb and Courtright 2002	Inside
2004-204.ASM	Davis 2003a, Davis 2003b	Inside
2004-329.ASM	Lausten 2004	Inside
2005-428.ASM	Bellavia and Mitchell 2005a	Outside
2005-1285.ASM	Moore 2005	Outside
2006-66.ASM	Bellavia and Mitchell 2005b	Outside
2006-605.ASM	Schroeder 1995	Outside
2011-236.ASM	N/A	Outside
11-19-1-BLM	N/A	Outside
SHPO-2001-3257	Slawson 2001	Outside

Table 2. Previously recorded sites within the project area vicinity.

Site number	Location	Site type	Affiliation and age	Eligibility status
AZ U:1:134(ASM)	Sec. 1 ^a	Fort McDowell/ Stoneman wagon road	Euro-American/ A.D. 1870-1890	Recommended eligible
AZ U:1:136(ASM)	Sec. 1 ^a	Trash dump	Euro-American/ A.D. 1884-1940	Not determined
AZ U:1:391(ASM)	Sec. 1 ^a	Flaked stone scatter and rock ring	Native American/ Prehistoric	Recommended not eligible
AZ U:1:433(ASM)	Sec. 31 ^b	Flaked stone and ceramic scatter	Hohokam/ A.D. 200-1500	Not determined

^a T5N, R4E, USGS 7.5' Cave Creek, Ariz., 1965/1981.

^b T6N, 5E, USGS 7.5' Cave Creek, Ariz., 1965/1981.

observations. Subsurface cultural features may be present. One other prehistoric site in the vicinity is also a limited activity area. The other two previously recorded sites are historic, consisting of one wagon road and one trash dump.

SURVEY EXPECTATIONS AND HISTORIC CONTEXT

Based on the physiographic context of the project and because of the relatively few discoveries in the project vicinity, Logan Simpson had low expectations for encountering significant prehistoric or historic cultural resources during the pedestrian survey, such as isolates and no more than two sites. If cultural resources were present, it was expected that they would represent limited activity areas rather than permanent occupations. As such, one local historic context, or theme, was identified in which to evaluate the significance and ARHP and NRHP eligibility of any cultural resources sites identified: *Prehistoric/historic land use in the Carefree Basin and the northern periphery of the Hohokam cultural area.*

SURVEY METHODS

The project area was completely surveyed on March 10, 2014. The fieldwork was supervised by Sean Teeter, M.A., RPA, a permitted field supervisor on Logan Simpson's Arizona Antiquities Act Permit (2014-007b). Crew members included Leigh Davidson, B.A. (4 years southwestern experience) and Helana Ruter, M.A. (8 years southwestern experience). The project manager is Mark Hackbarth, M.A., RPA, and the report was reviewed by Scott Courtright. Prior to survey, the project area boundary was programmed into a handheld Trimble Geo XM global positioning system (GPS) unit. The project area was surveyed by maintaining parallel transects oriented with a compass and GPS unit spaced no more than 15 m apart, resulting in 100 percent coverage. When cultural remains were encountered, they were assigned a field number, plotted on USGS topographic maps and with the GPS unit, and described in written notes. All Universal Transverse Mercator (UTM) coordinates in this document are in Zone 12 North and based on the North American Datum of 1983, Continental United States datum (NAD 83 CONUS). The Trimble Geo XM GPS unit employs real-time differential correction; any GPS positions not corrected in real-time were post corrected to an accuracy of 1 to 2 m using Pathfinder Office software. Sites, features, diagnostic artifacts, and other relevant features were documented with digital photographs. Ground-surface visibility within the survey area was relatively fair and averaged 50 to 75 percent open. Natural erosion and previous construction activities have disturbed the area.

The site was recorded following ASM site criteria using ASM site cards. Before recording, all artifacts were pin flagged to define site boundary and identify concentrations. A scaled map was drawn of the site to illustrate the artifact concentration, site boundary, and topography. In addition, GPS points were taken at the site and isolated occurrences (IOs) to accurately place the cultural resources on electronic maps. Cultural resources were documented with digital photographs.

Cultural resources were recorded following the ASM *Revised Site Definition Policy* (Fish 1995), which is accepted by the Arizona SHPO. This policy states that sites should contain:

- Physical remains of past human activity that are at least 50 years old.
- Additionally, sites should consist of at least one of the following:
 1. 30+ artifacts of a single class (i.e., 30 sherds, 30 lithics, 30 tin cans) within an area 15 meters (50 feet) in diameter, except when all pieces appear to originate from a single source (i.e., one ceramic pot, one core, one glass bottle).
 2. 20+ artifacts that include at least 2 classes of artifact types (i.e., sherds, ground stone, nails, glass) within an area 15 meters (50 feet) in diameter.
 3. One or more archaeological features in temporal association with any number of artifacts.
 4. Two or more temporally associated archaeological features without artifacts.

Cultural resources not meeting site criteria were recorded as IOs.

Identified cultural resources are evaluated for ARHP and NRHP eligibility based on their integrity and significance under the four criteria outlined in 36 CFR 60 and per guideline presented in National Register Bulletin 15, *How to Apply the National Register Criteria for Evaluation*. In order for a historic property to be considered eligible for listing in the NRHP, it must:

- Be associated with events that have made a significant contribution to the broad patterns of our history (Criterion A);
- Be associated with the lives of persons significant in our past (Criterion B); or
- Embody the distinct characteristics of a type, period, or method of construction, or that represent the work of a master, or that possess high artistic values, or that represent a significant and distinguishable entity whose components may lack individual distinction (Criterion C); or
- Yield, or be likely to yield, information important in prehistory or history (Criterion D).

SURVEY RESULTS

The survey resulted in the identification of one previously recorded site—AZ U:1:433(ASM)—and 9 IOs that include both prehistoric and historic artifacts (Appendix B.1). No new sites were discovered.

Previously Recorded Site

AZ U:1:433(ASM)

ARHP/ NRHP Eligibility: Undetermined

Land Status: Private

Landform: Level area adjacent to unnamed small wash, 2,640 ft amsl

Deposition: Alluvial

Vegetation: Dense thicket of palo verde trees, wolfberry and small shrubs

Soils: Dark brown silt with moderate to high concentration of decomposing cobbles and gravel

Site Size: 30 m by 17 m (510 m²/0.13 acre)

Site Type: Artifact scatter

Cultural/Temporal Affiliation: Hohokam/A.D. 200–1500

Description:

AZ U:1:433(ASM) consists of a previously recorded artifact scatter with no surface features. The site was originally recorded by Lausten (2004) as partially within a fire station's project area, and partially within the current project area (Figure 3; Appendix B.1). The observed low- to moderate-density artifact assemblage consists of an estimated 300 sherds and 10 flakes. The ceramic assemblage largely consists of Wingfield Plain and Gila Plain sherds and few red ware sherds. Flaked-stone artifacts consist mainly of secondary flakes, and basalt and quartzite are the material types represented. Most of the artifacts are concentrated in a bulldozed area in the northeastern area of the site, bordered by the wash to the north and the fence of a construction storage area to the south (Photograph 2). The artifact concentration (AC1) contains approximately 150 to 200 sherds within a 5 m area.

The site is in poor condition from extensive disturbance created by previous construction and erosion. The installation of a chained-off cleared area used for storage disturbed much of the site. The remainder of the site has been impacted by erosion. Ground cover is moderate to heavy and visibility ranges from 50 to 75 percent open. In addition to the dense thicket of vegetation, piles of construction material and scattered debris obscure the ground surface.

Discussion and Recommendation:

AZ U:1:433(ASM) is a Hohokam artifact scatter and may be eligible for inclusion in the ARHP and NRHP under Criterion D (information potential); however, a formal eligibility recommendation could not be made based on surface observations. An eligibility recommendation would only be possible following the implementation of an archaeological testing program. Although highly disturbed, the site retains integrity of location and setting. The site likely contains intact subsurface cultural deposits that may be able to contribute important information within the context of prehistoric land use within the Carefree Basin and the northern periphery of the Hohokam cultural area. The presence of human remains, while possible, would not be expected.

ISOLATED OCCURRENCES

In addition to the one previously recorded site, 9 IOs were identified (Table 3; Appendix B.1). The IOs are not eligible for inclusion in the ARHP or the NRHP; they have been fully recorded, and no additional research or preservation is required.

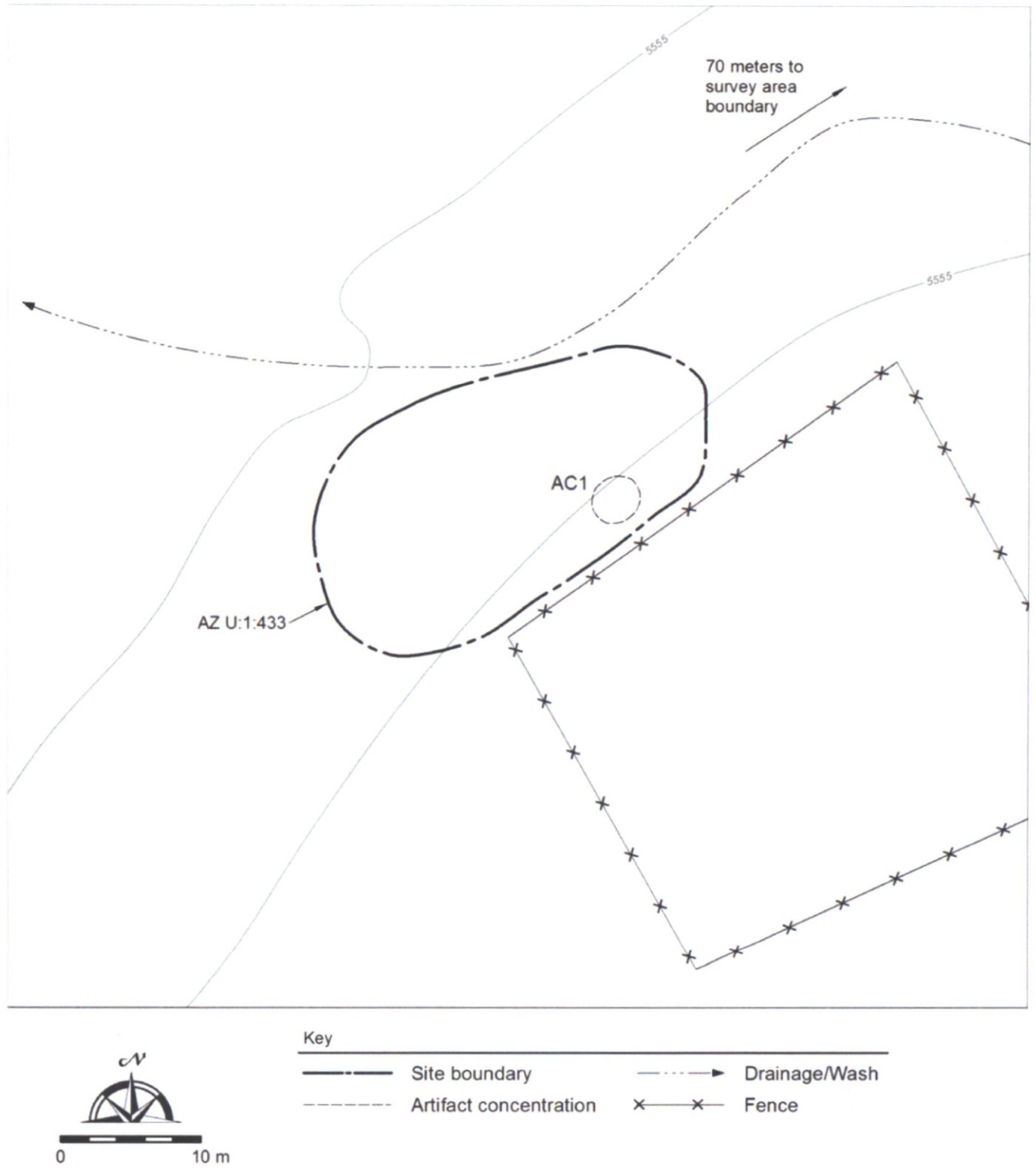


Figure 3. Site map of AZ U:1:433(ASM).



Photograph 2. Overview of AZ U:1:433(ASM), facing north.

Table 3. Isolated occurrences.

IO	Descriptions	UTMs ^a	
		Northing	Easting
1	1 church key-opened can	3742991	417662
2	1 church key-opened can	3742954	417645
3	1 church key-opened can	3742688	417654
4	1 church key-opened can	3742979	417724
5	1 basalt core	3743173	417789
6	3 church key-opened cans within 6 by 6 m area	3743044	417769
7	1 church key-opened can	3742736	417791
8	1 quartzite core	3743217	417819
9	1 church key-opened can	3743070	417894

^a UTM Zone 12, NAD 1983 CONUS.

SUMMARY AND RECOMMENDATIONS

The Class III cultural resources survey of the 91-acre parcel of private land resulted in the identification of one previously recorded site—AZ U:1:433(ASM). No new sites were discovered. In addition to the site, 9 IOs were identified. The IOs are not eligible for inclusion in the ARHP or the NRHP, and no additional research or preservation is required. The survey was conducted in voluntary compliance with the Arizona Burial Law (A.R.S. §41-844 and §41-865) and as due diligence prior to purchase of the property for a proposed private development project. It is not possible to determine whether the cultural resources would be affected by the project at this time.

A formal recommendation of eligibility for AZ U:1:433(ASM) could not be made based on surface observations. An eligibility recommendation would only be possible following the implementation of an archaeological testing program. If the parcel is developed and if avoidance is not possible, the site should be subjected to an appropriate archaeological testing program to determine whether buried deposits are present and to formally evaluate its ARHP and NRHP eligibility

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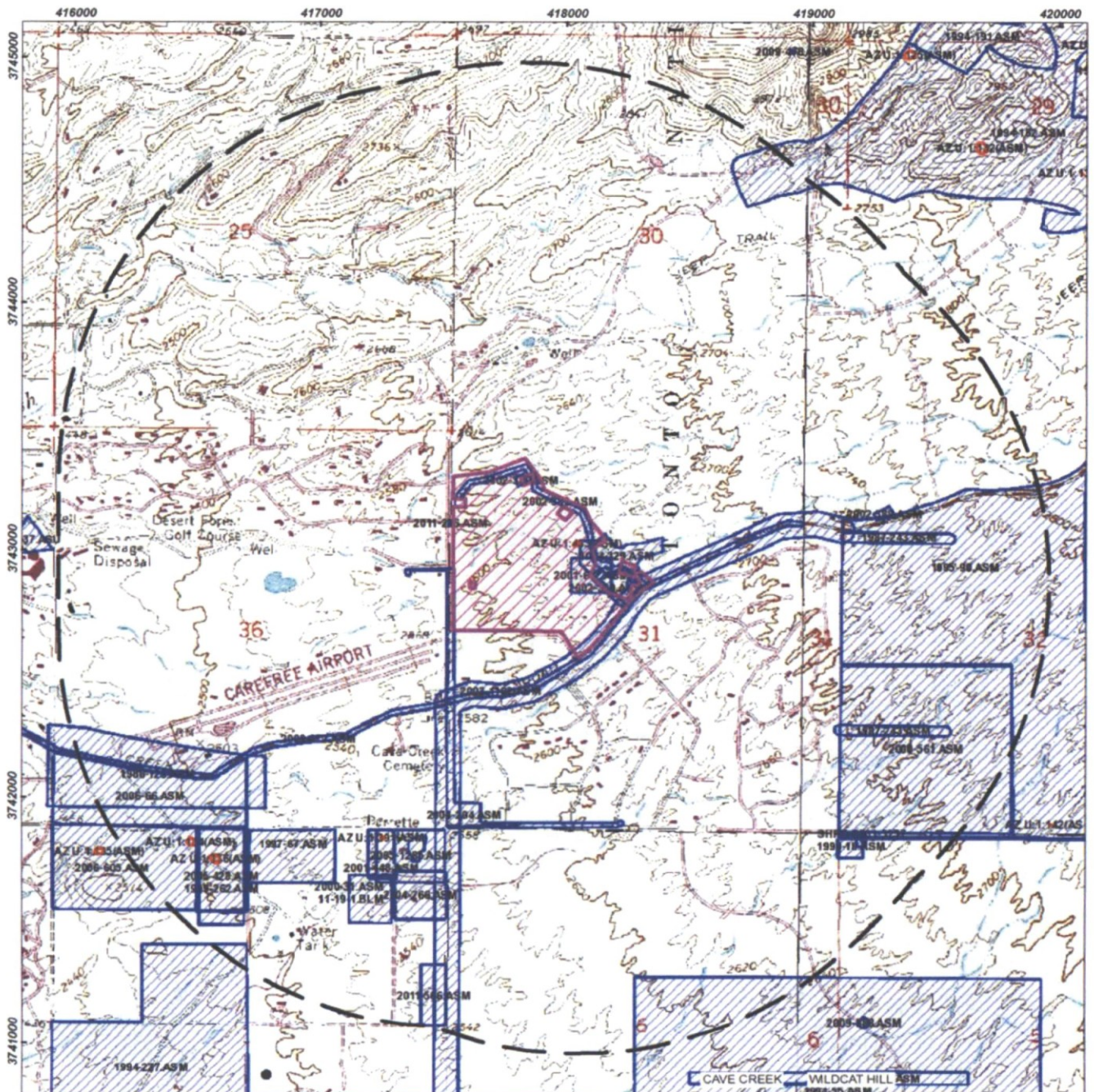
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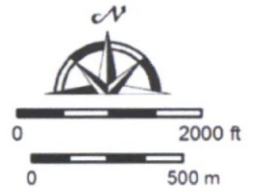
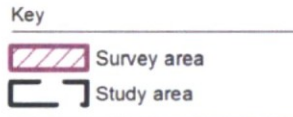
Appendices

Confidential Information

Appendix A. Previous Research Maps



Source: USGS 7.5' Quadrangles:
 Cave Creek, Ariz. (1965, 1981).
 Wildcat Hill, Ariz. (1965, 1981)
 NAD 1983, UTM Zone 12

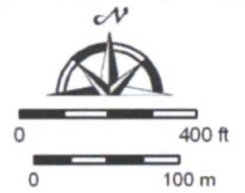


Appendix A.1 Location of previous recorded sites and projects within the project vicinity.



Source: Source: Esri, DigitalGlobe, GeoEye, i-cubed, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, and the GIS User Community

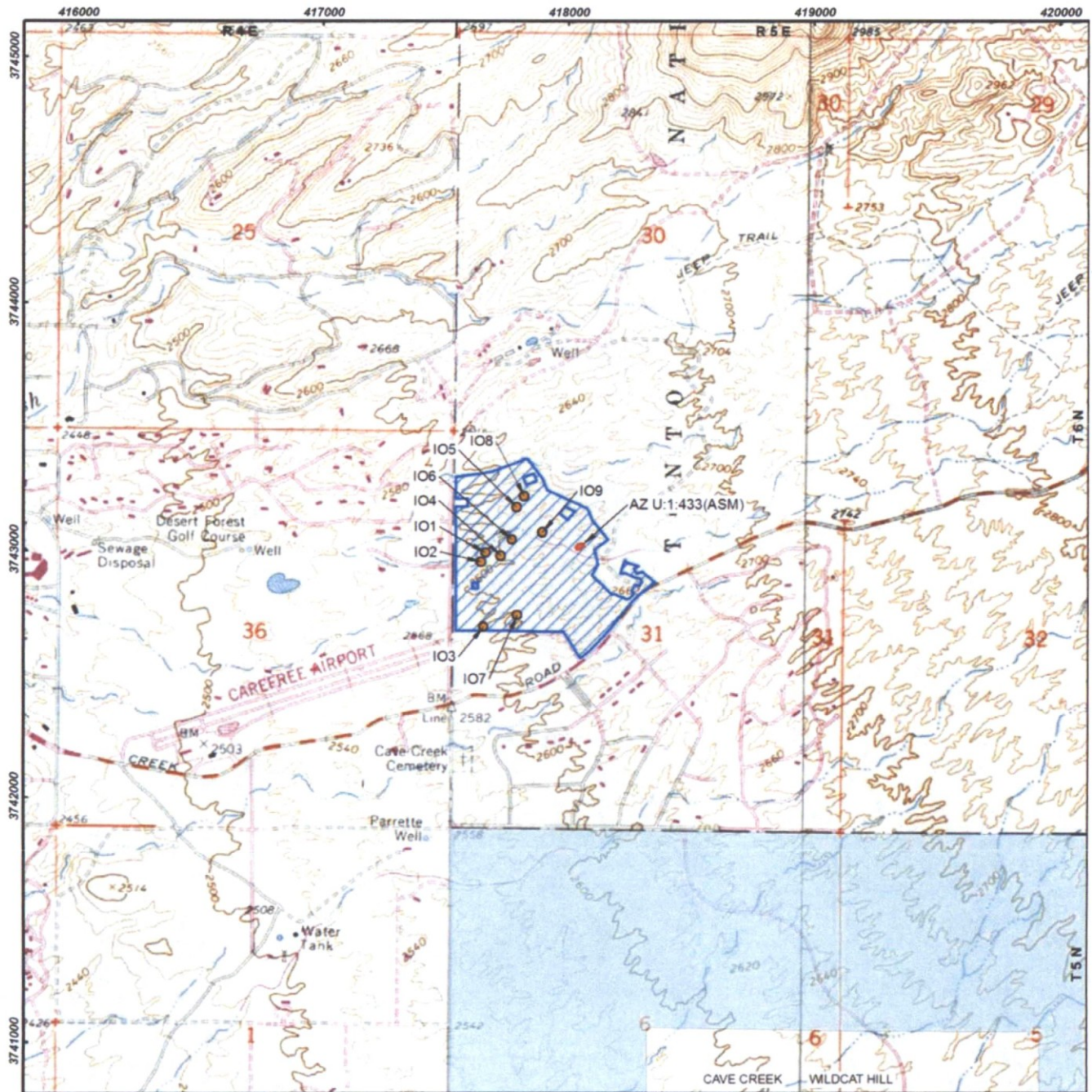
Key
 Survey area








Appendix A.2. Location of survey area and previously recorded archaeological site.

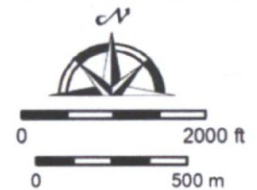
APPENDIX B.
LOCATION OF CULTURAL RESOURCES





Source: USGS 7.5' Quadrangles:
 Cave Creek, Ariz (1965, 1981);
 Wildcat Hill, Ariz (1965, 1981)
 NAD 1983, UTM Zone 12

Key		Land Jurisdiction	
	Survey area		Private Land
	Site boundary		State Trust Land
	Isolated occurrence		



Appendix B.1. Location of cultural resources.

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**PRELIMINARY DRAINAGE REPORT
FOR
DESERT MOUNTAIN PARCEL 19**

June 16, 2016
WP# 164434

Prepared for: **DM Real Estate Holdings, LLC**
10550 East Desert Hills Drive
Scottsdale, Arizona 85262
Phone: (480) 595-4030
Contact: Tom Warley

Submitted to: **City of Scottsdale**
7447 East Indian School Road
Suite 205
Scottsdale, AZ 85251
Phone: (480) 312-7080
Fax: (480) 312-7781

Prepared by: **Wood, Patel & Associates, Inc.**
2051 West Northern Avenue
Suite 100
Phoenix, AZ 85021
Phone: (602) 335-8500
Fax: (602) 335-8580
Website: www.woodpatel.com



EXP. 3-31-2018

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APPENDICES

- Appendix A Existing Condition Hydrologic Calculations
- DDMSW Output Data
 - 2-year HEC-1 Model
 - 10-year HEC-1 Model
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- Appendix B Developed Condition Hydrologic Calculations
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- Required First Flush Storm Water Calculations
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 - Scour Calculations
 - Erosion Hazard Setback Calculations
- Appendix E Electronic Files: PDF of Report
DDMSW DM 19 Existing & Proposed ZIP files,
Existing & Proposed 2-year, 10-year & 100-year HEC-1 Files
Existing Wash A & Existing & Proposed Galloway Wash HEC-RAS Files

EXHIBITS

- Exhibit 1 Vicinity Map
- Exhibit 2 ESL Classification Map
- Exhibit 3 Flood Insurance Rate Map (FIRM)
- Exhibit 4 Soils Classification Map
- Exhibit 5 Aerial Map
- Exhibit 6 Developed Conditions Land Use Map
- Exhibit 7 Existing Conditions Sub-Basin HEC-1 Map
- Exhibit 8 Developed Conditions Sub-Basin HEC-1 Map
- Exhibit 9 Existing Conditions Hydraulics Map
- Exhibit 10 Developed Conditions Hydraulics Map
- Exhibit 11 Preliminary Grading Plan



EXP. 3-31-2018

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1.0 INTRODUCTION

1.1 Project Description

Desert Mountain Parcel 19 (DM 19), herein referred to as the “Site,” is an approximate 92-acre parcel of land located in north Scottsdale and is proposed to be rezoned to R4 zoning and OS (Open Space). The Site is located in Section 31, Township 6 North, Range 5 East, of the Gila and Salt River Base and Meridian. The Site is currently an undeveloped parcel, bound to the west by Pima Road and the Carefree Fore More Development (located within the Town of Carefree), to the east by the Desert Mountain Club Golf Course, to the north by the Gambel Quail Preserve 2 Development (both located in the City of Scottsdale), and to the south and southeast by the Velvet Shadows 3 Development and two churches (Christ the Lord Lutheran Church and Our Lady of Joy Roman Catholic Church; all within the Town of Carefree). The Site was originally part of the Desert Mountain Master Development Plan. Exhibit 1 – *Vicinity Map* shows the general location of the project and surrounding areas.

1.2 Type of Report

This Report is being prepared as a Preliminary Drainage Report for the DM 19 rezoning submittal to the City of Scottsdale. At this time, only preliminary grading, roadways and a general land use plan has been completed for the Site.

1.3 Purpose

The Preliminary Drainage Report for DM 19 has been prepared to meet the drainage design requirements outlined in Chapter 4 of the *City of Scottsdale Design Standards and Policies Manual (DS&PM)*, the *Flood Control District of Maricopa County (FCDMC), Drainage Design Manual for Maricopa County, Arizona: Volume I – Hydrology*, and the *Flood Control District of Maricopa County, Drainage Design Manual for Maricopa County, Arizona: Volume II – Hydraulics*. This report presents the results of the hydrological and hydraulic modeling of the Site’s proposed preliminary drainage systems.

The main purpose of this Report is to illustrate the following:

- Compliance with the City of Scottsdale's Floodplain Ordinance stormwater storage requirements for the property subject to the Environmentally Sensitive Land Ordinance (ESLO). The Site will provide the first flush stormwater storage for the first 0.5-inch of runoff and/or first flush treatment for the property's improved areas. A Request for a Stormwater Storage Waiver is anticipated to be submitted to the City of Scottsdale.
- Reduction of post-development flows to at or below the pre-development flows for 2-year, 10-year, and 100-year, 6-hour storm events using the *U.S. Army Corps of Engineers, HEC-1, Flood Hydrograph Package*. Hence, it is anticipated that the downstream properties shall be provided with similar or potentially better flood protection than the pre-development conditions.
- Maintaining the two main water courses traversing through the Site in their natural locations (the Galloway Wash and Wash A).
- Delineation of the 100-year water surface inundation extents, along with determination of the 100-year water surface elevations for the two main drainage courses using the *U.S. Army Corps of Engineers, HEC-RAS (Version 4.1.0)*.
- Delineation of the 100-year water surface inundation extents for all washes determined to have peak flows equal to or greater than 50 cfs using hydraulic analysis software such as *AutoCad Civil 3D Hydroflow (Version 10.5)*.
- Hydraulic analyses of preliminary drainage structures and wash conveyance corridors.

2.0 EXISTING ON-SITE DRAINAGE CONDITIONS AND CHARACTERISTICS

2.1 On-Site Drainage

The Site lies in the northern planning section of the City of Scottsdale. The elevations range from 2,612 in the middle of the Site, to 2,650 feet in the east and 2,587 feet in the west. Based on the existing topography, the Site drains primarily from east to west with an approximate average slope of 2%. There are two primary outflow concentration points, one located at the northwest corner and one at the center of the western property boundary. There are several additional minor outflow concentration points along the southern and western property boundary.

Vegetation is typical Sonoran Desert type with creosote bush, jumping cholla, saguaro cacti, palo verde, ironwood, and mesquite trees. The Site lies within the areas identified as Environmentally Sensitive Lands (ESL) by the City of Scottsdale. The Site is further classified as 'Upper Desert' within the ESL areas as shown on Exhibit 2 – *ESL Classification Map*.

The USDA Natural Resources Conservation Service (NRCS), formerly known as the Soil Conservation Service (SCS), provides soils data for all of Maricopa County. This data is reproduced on Exhibit 4 – *Soils Classification Map*. Soils data is the basis of the rainfall loss parameters, as discussed in Section 5.2.3.

There is one wash, the Galloway Wash, that traverses the Site and is categorized as a 'Vista Corridor' or 'Major Wash', as the calculated 100-year, 6-hour flow is greater than 750 cfs. There is also one other significant watercourse traversing the Site that has a 100-year, 6-hour flow greater than 400 cfs and is referred to as "Wash A". The Galloway Wash and Wash A are identified on Exhibit 9 – *Existing Conditions Hydraulics Map*. Wash A was determined to have a 100-year flow of approximately 470 cfs. Both washes will be maintained in their natural locations and will not be re-aligned.

2.2 Existing On-Site Drainage Network

Existing on-site drainage sub-basin boundaries were identified using aerial mapped 1-foot contours; refer to Exhibit 7 – *Existing Conditions Sub-Basin HEC-1 Map*.

The on-site watersheds primarily drain east to west or southwest with only one wash exiting the property along the southern boundary with a 100-year peak flow greater than 50 cfs. This wash appears to enter the Velvet Shadows 3 Development and continues west combining with some additional runoff from the Site before flowing through the Our Lady of Joy Roman Catholic Parish Carefree church parking lot and ultimately over Pima Road by an at-grade drainage crossing. The flow exiting the Site has an estimated existing 100-year peak flow of 65 cfs.

Based on the results of the wash hydraulic analysis, a delineation of this wash's existing condition 100-year, 6-hour floodplain limits has been illustrated on Exhibit 9 – *Existing Conditions Hydraulic Map*. Pre-development sub-critical flow water surface elevations for each cross-section are included on Exhibit 9. The hydrologic and hydraulic analysis procedures are discussed in Section 5.0. It is anticipated that this wash will be modified and an Application for Wash Modification will be submitted to the City of Scottsdale.

2.3 Off-Site Watersheds

The off-site drainage areas impacting the Site lay to the east and are in the northern planning section of the City of Scottsdale. The off-site watersheds contain primarily large rural lot single family residential subdivisions and are part of the Desert Mountain Master Development Plan community. These drainage areas are identified on Exhibit 7. The off-site areas are also classified as 'Upper Desert' ESL landform areas by the City of Scottsdale.

2.4 Existing Off-Site Drainage Network

The *Floodplain Delineation Study of Andora Hills & Galloway Washes Technical Data Notebook* (TDN), the *Master Drainage Report for Desert Mountain Parcel C Offsite Drainage Map*, the *Master Drainage Report for Desert Mountain Development Master Development Plan* exhibit and the City of Scottsdale 2-foot contour interval topographic Quarter Section Maps were used to identify and confirm the off-site drainage areas impacting the Site. Refer to Exhibit 7 for the limits of the drainage areas and concentration points.

Off-site flows from the east enter the Site's eastern property boundary at five locations. Four of the concentration points are washes with 100-year peak discharges greater than

50 cfs and one location has a discharge of only 3 cfs. Starting at the northeast property boundary and continuing south, the washes' approximate 100-year peak flows were determined to be: Wash A's flow is 468 cfs, two unnamed washes' flows are 72 cfs and 111 cfs, and the Galloway Wash is 1,111 cfs.

Based on the results of the HEC-RAS and hydraulic wash analyses, a delineation of the existing condition 100-year, 6-hour floodplain limits for the Galloway Wash, Wash A and the two unnamed washes have been illustrated on Exhibit 9 – *Exist Condition Hydraulics Map*. Pre-development subcritical flow water surface elevations for each cross-section are included on Exhibit 9. The hydrologic and hydraulic analysis procedures are discussed in Section 5.0.

2.5 Existing Drainage Relative to Adjacent Projects

Existing washes exit the Site as concentrated flows at various locations along the western and southern property boundary. As Wash A exits the Site it immediately combines with additional flow from the north and is conveyed over Pima Road by an at-grade drainage crossing. The Galloway Wash also is conveyed over Pima Road by an at-grade drainage crossing and the preliminary hydraulic results determined that the depth of flow over Pima Road is greater than one foot. As detailed within the *Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN, when the flow leaves Pima Road it splits around the existing single-family residence before recombining downstream and continuing to the west.

There are an additional six concentrated flow exit locations along the western property boundary and three concentrated flow exit locations along the southern property boundary. These 100-year peak flows vary from a maximum discharge of 67 cfs to only 4 cfs. Refer to Exhibit 7 for the exiting concentration point locations.

There is also a small piece of the Site that is not within the City of Scottsdale and is located at the southeast corner of the property. This portion of the Site is located within the Town of Carefree and also has two existing concentration points that exit the Site to the west.

2.6 FEMA Regulated Flood Zones

The Site is located within the Flood Insurance Rate Map (FIRM) for Maricopa County, Arizona and Incorporated Areas, Panel Number 04013C0884L, effective date October 16, 2013. The FIRM, published by the Federal Emergency Management Agency (FEMA), indicates that the Site is located within Special Flood Hazard Areas (SFHAs) Zone AE, Other Flood Areas Zone "X" (Shaded) and Other Areas Zone "X".

Zone "X" is defined by FEMA as follows:

"Areas determined to be outside 500-year floodplain."

Zone "X (Shaded)" is defined by FEMA as follows:

"Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance floods."

Special Flood Hazard Areas (SFHAs) Subject To Inundation By the 1% Annual Chance Flood is defined as follows:

"The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Areas is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE AH, AO, AR, A99, V and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood."

Zone "AE" is defined as: *"Base Flood Elevations determined"*.

The approximate location of the Site relative to the FEMA FIRM panel is illustrated on Exhibit 3- *Flood Insurance Rate Map (FIRM)*.

The proposed Site's development will not alter the effective FEMA SFHA floodplain and/or floodway and will not change the effective FEMA SFHA base flood elevations. This will be documented in more detail as the Site progresses into the pre-plat phase of development.

3.0 PROPOSED DRAINAGE PLAN

3.1 General Description of Proposed Drainage System

The Site is being rezoned to approximately 44-acres of R4 zoning and approximately 48-acres of Open Space zoning. Preliminary site grading is shown on Exhibit 11 – *Preliminary Grading Plan*. The proposed drainage system is being designed to allow existing drainage patterns to be maintained in their natural location and condition where possible. Where the proposed development will disturb existing washes with 100-year peak discharges equal to or greater than 50 cfs, the washes will be modified to re-direct flow around and/or through the development to maintain historical flow patterns.

The off-site flow of the Galloway Wash will be conveyed through the Site with as little disturbance to the natural wash corridor as possible. The Galloway Wash will have two proposed 3-barrel, 10-foot wide by 4-foot high box culvert roadway drainage crossings to pass the 100-year discharge under the streets. One crossing is located near the eastern property boundary and one crossing is located within the middle of the Site. There is no anticipated disturbance to the Galloway Wash natural wash bottom between these proposed drainage crossings. Downstream of the middle box culvert roadway crossing to approximately 200-feet upstream of the western property boundary, the Galloway Wash will have a relatively small pilot channel (20-foot wide by 1-foot deep) to offset some potential open space encroachment grading proposed within the existing FEMA SHFA. Any grading proposed within the existing FEMA 100-year floodplain will require an FCDMC floodplain use permit.

The off-site flow of Wash A will be conveyed through the Site. The post-development design is not anticipated to disturb the existing natural wash corridor. In addition, any post-development adjacent to the wash is being designed to drain away from Wash A. Therefore, there is no foreseen modification to the wash and/or change to the existing 100-year peak flow of Wash A due to development.

3.2 Future Conditions

Currently, the upstream off-site watersheds located within Desert Mountain have been fully developed. The Desert Mountain development land uses were obtained from the

Floodplain Delineation Study of Andora Hills & Galloway Washes TDN and were duplicated in the HEC-1 hydrologic models.

The drainage plan for the Site proposes to maintain the 2-year, 10-year, and 100-year, 6-hour storm event post-development peak discharges at or below the pre-development flows. In addition, the two significant drainage courses, Wash A and the Galloway Wash, will be maintained in their historic locations at the property boundary. No adverse impact is anticipated on the adjacent properties downstream of the project Site for the 2-year, 10-year, and 100-year, 6-hour storm events.

3.3 Stormwater Storage Requirements

According to the City of Scottsdale's Floodplain Ordinance for property located within the ESL, all runoff generated from the developed portion of the Site must be managed, and the peak discharges from the Site reduced to at least pre-development values during the 100-year, 10-year and 2-year storm frequencies. Proposed retention and/or detention basins will be strategically located along several exiting flow locations and along the Galloway Wash which will attenuate the post-development peak discharges to at or below the pre-development values. Stormwater storage basins will also be designed to intercept flow from upstream proposed developed areas to retain the post-development required first flush retention volume and/or flow will be treated prior to exiting into the adjacent washes. 18-inch bleed-off pipes with removable 6-inch orifice plates are proposed for the dissipation of the retained storm water.

Table 3.3 below summarizes the required first flush stormwater storage for the Site and Exhibit 8 and Exhibit 11 detail the on-site drainage areas and preliminary first flush basin locations, respectively.

Table 3.3
Required First Flush Stormwater Storage

Drainage Area	Area (SF)	Area (ac)	Weighted Runoff Coefficient	First Flush Volume (ac-ft)	Required Pre vs Post Volume (ac-ft)
B5	165024	3.79	1.00	0.16	0.42
B6	239591	5.50	1.00	0.23	N/A
B8	300366	6.90	1.00	0.29	N/A
B9	217378	4.99	1.00	0.21	N/A
B10	335363	7.70	1.00	0.32	N/A
B11	269364	6.18	1.00	0.26	N/A
B13	80916	1.86	1.00	0.08	N/A
B14	1072718	24.63	1.00	1.03	N/A
B14A	115650	2.65	1.00	0.11	N/A
C1	636403	14.61	1.00	0.61	N/A

3.4 Pre- and Post-Runoff Characteristics

The U.S. Army Corps of Engineers' HEC-1 computer analysis program was used for hydrologic modeling, including routing of flow through storage basins and combining hydrographs. The HEC-1 model was also used to compare the pre- and post-development runoff at concentration points exiting the property. Runoff for each drainage sub-basin was computed and if required, hydrographs were then combined. Drainage basins were further divided into sub-basins to simulate the developed conditions. The parameters were selected per the guidelines provided in the *DS&PM*. The parameters selected and the inputs for the HEC-1 models are discussed in Section 5.1.

The HEC-1 input data and output files for the existing conditions are included in Appendix A. The developed conditions data is included in Appendix B. The data analysis procedures are discussed in Section 5.0.

Table 3.4-1 below provides the comparative peak flows for the pre- vs. post-developed conditions for 100-year, 10-year, and 2-year, 6-hour events at concentration point where flow leaves the property. For the location of these concentration points and corresponding 100-year, 6-hour flow values, refer to Exhibit 7 and Exhibit 8.

**Table 3.4-1
Pre – vs. Post-Peak Flow Analysis**

HEC-1 ID PRE/POST	100-yr, 6- hr Existing Condition Peak Flow (cfs)	100-yr, 6- hr Post- Dev. Condition Peak Flow (cfs)	100-yr, 6- hr Difference in Peak Flow (cfs)	10-yr, 6- hr Existing Condition Peak Flow (cfs)	10-yr, 6- hr Post- Dev. Condition Peak Flow (cfs)	10-yr, 6- hr Difference in Peak Flow (cfs)	2-yr, 6-hr Existing Condition Peak Flow (cfs)	2-yr, 6-hr Post-Dev. Condition Peak Flow (cfs)	2-yr, 6-hr Difference in Peak Flow (cfs)
CP-A2/CP-A2	469	467	2	261	260	1	128	128	0
B1/DVB11	35	30	5	19	15	4	8	0	8
C1/DTC11	26	5	21	14	5	9	6	5	1
CP-E5/CPGAL	1124	1025	99	549	527	22	230	222	8
F1/DTGF	4	3	1	2	1	1	1	1	0
G1/DTC-H	23	15	8	13	8	5	6	0	6
H1/DV-B5	17	17	0	10	0	10	5	0	5
I1	20			11			5		
J1	10			6			3		
K1	65			36	29		17		
I1,J1,K1/DTC- K	95	50	45	53	18	35	25	1	24
L1	10	10	0	6	6	0	3	3	0
M1	3	3	0	2	2	0	1	1	0

3.5 Proposed Drainage Structures

3.5.1 On-site First Flush Storage Basins

On-site first flush storage basins will be used to capture the first 0.5-inch of runoff from the post-development disturbed areas. The design of the basins will be at a maximum depth of 3-feet and have 4:1 side slopes. 18-inch bleed-off pipes with 6-inch orifice plates will be used to drain the stormwater storage within a 36-hour period. Outlet weirs will be incorporated into the basin design for overflow conveyance. When the capacity of the basin is reached, the runoff will overtop the weir and be released at historic flow locations. Erosion protection will be incorporated within the design of the outlets.

3.5.2 Off-Site & On-Site Galloway Wash Detention Basins

As detailed on Exhibit 8, there are two off-site off-line detention basins (Basin 3 and Basin 4) and one on-site off-line detention basin (Basin 1) proposed along the Galloway Wash. The two off-site off-line detention basins will require improved wash channel sections and erosion protected weir inlets to remove a

portion of the 100-year peak flow from the wash. The runoff will enter the detention basin and pond a maximum depth of 4-feet during the 100-year event and have a minimum of an 18-inch outflow pipe that will drain the basin within a few hours. The proposed detention basins will have storage volume capacities of approximately 2.0 acre-feet (Basin 3) and 1.4 acre-feet (Basin 4). Contractual agreements are being prepared with the upstream land owners to allow for the construction and maintenance of these off-line detention basins.

One on-site off-line detention basin will be located downstream of the Galloway Wash middle box culvert crossing and will receive flow from an 18-inch pipe located upstream of the roadway crossing. This detention basin will collect some of the low-flow stormwater runoff to reduce the 2-year and 10-year peak flow of the Galloway Wash. There will be a weir outlet from the detention basin that will drain excess flow back into the Galloway Wash. The proposed detention basin will have approximately 1.1 acre-feet of storage volume at a maximum ponding depth of 4-feet during the 100-year runoff event. The bottom 1-foot will be used for on-site first flush storage. The basin will have an 18-inch outflow pipe with a 6-inch orifice plate that will also drain back into the Galloway Wash. It is anticipated that the basin will drain within a few hours.

The three off-line detention basins were included in the post-development HEC-1 model. The hydrology results can be found within Appendix B.

In several critical locations along the wash, bank protection is necessary to protect proposed off-line detention basins. The bank protection is planned to be aesthetically pleasing and may be either integrally colored soil cement and/or reinforced gunnite/concrete, seated grouted rock riprap and/or gabion baskets/mattresses. Bank protection will be designed to extend to a maximum calculated scour depth of 4-5 feet below the wash bottom. Refer to Exhibit 10 for bank protection locations and Appendix E for scour depth calculations.

3.5.3 Roadway Crossing Structures

There are currently two proposed on-site roadway drainage crossing structures that have been designed to convey the anticipated post-development 100-year

flow of the Galloway Wash. The flows were calculated at the box culvert locations using the results of the post-development HEC-1 model analysis. Refer to Appendix B for the flow calculations.

Both crossing locations will be designed with a 3-barrel, 10-foot wide by 4-foot high box culvert roadway drainage crossing. The roadway crossings were analyzed within the wash's hydraulic analysis using the U.S. Army Corps of Engineers *HEC-RAS (Version 4.1.0)*, such that the estimated upstream 100-year peak flow is contained within the wash and no overtopping of the roadway is anticipated. The box culverts will be designed with drop inlets and a longitudinal slope such that sediment deposition is not anticipated within the culverts.

Riprap and/or erosion protection will be designed at the inlets and outlets of the box culverts to control velocities and prevent erosion. In several critical locations along the wash, bank protection is necessary to protect the proposed roadway improvements. The bank protection shall extend to a maximum calculated scour depth of 4-5 feet below the wash bottom. Refer to Exhibit 10 for bank protection locations and Appendix E for scour depth calculations.

Although the land use is still in a preliminary stage, there are additional locations within the Site that are anticipated to require culverts to pass the flow under roadways and/or to convey flow from the eastern property boundary to the Galloway Wash. These culverts will be designed using AutoCad Civil 3D *Hydroflow (Version 10.5)*, Bentley's *CulvertMaster® v3.1*, Copyright® 1995-2007, and/or the Federal Highway Administration HY-8 v7.3, 2013 program to determine the pipe sizes, headwater elevations and pipe velocities.

3.5.4 On-site Roadway Drainage Structures

3.5.4.1 Street Flow

The Rational Method will be used to calculate the storm water runoff generated from the roadways and any on-site adjacent developments that drain into the residential roadways. The 100-year and 10-year flow will be calculated for the street flow based on the FCDMC Rational Method, as discussed in Section 5.1.3.

3.5.4.2 Street Capacity Hydraulics

There are no anticipated arterial, major and/or minor collector roadways proposed within the development. For all local interior streets, the street conveyance capacity will be calculated using Manning's Equation. The streets will be designed such that the 10-year peak flow is contained within the street curbs and the 100-year peak flow does not exceed a maximum depth of 8-inches. A Manning's "n" value of 0.015 will be used for the standard street cross-section. Roadway capacities will be calculated based on 4-inch and/or 6-inch roll curb (where necessary). The drainage for local residential roadways will be designed consistent with the City of Scottsdale Ordinance 37-42(4) for the allowable depth of water on the street when the street is being utilized as a water carrier. The methodology developed by the Federal Highway Administration will be used to calculate, in spreadsheet format, the allowable street cross-section capacities.

3.5.4.3 Curb Openings

Catch basins, scuppers and/or depressed curb openings will be designed to convey runoff from the streets such that street flow depths do not exceed 8-inches for the 100-year flow, and the 10-year flow is contained within the curbs. On-site storm water runoff generated by the residential roadways is planned to exit the street via on-grade and/or low point sump curb openings. Flow will then be directed within storm drain, roadside drainage swales and/or along natural grade to the closest receiving wash, retention basin, and/or culvert inlet. All scupper and catch basin curb openings will be designed to include a 0.80 reduction factor.

4.0 SPECIAL CONDITIONS

4.1 Section 404 Washes

To the best of our knowledge, the US Army Corps of Engineers (Corps) Section 404 of the Clean Water Act (CWA) Jurisdictional Waters of the United States has not yet been determined within the Site. Potential CWA Section 404 Jurisdictional Washes are under investigation and an approved jurisdictional determination submittal inclusive of a significant nexus analysis is being prepared for the Site by Del Sol Group to identify if any of the washes may be deemed jurisdictional. It is understood that an approved jurisdictional delineation of the Waters of the United States is required for the Site.

5.0 DATA ANALYSIS METHODS

5.1 Hydrologic Method Description

This section documents the engineering procedures and methodologies used to generate the existing and developed condition hydrologic models for the Site. The results of the hydrologic models were used in the conceptual design of drainage facilities and to assure compliance with current drainage design standards.

Precipitation was input by use of the FCDMC 6-hour local storm. Key rainfall statistics were obtained from the *NOAA Atlas 14*, Arizona. The FCDMC DDMSW program was used to develop the necessary point rainfall depth-duration-frequency statistics. Table 5.1 below provides a summary of the point rainfall depth-duration-frequency data. Rainfall losses were calculated by use of the Green and Ampt infiltration equation with an allowance for surface retention loss. Synthetic unit hydrographs for each sub-basin were developed using the Phoenix Desert/Rangeland unit hydrograph as was used within the approved *Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN. Existing and proposed discharges for the 100-year, 10-year and 2-year, 6-hour storm events were modeled using HEC-1 at various concentration points as shown on Exhibit 7 and Exhibit 8, respectively. The HEC-1 results are provided within Appendix A and Appendix B for the existing and proposed condition watersheds, respectively.

Table 5.1
Point Rainfall Depth-Duration-Frequency Data

Frequency→	2-year	5-year	10-year	25-year	50-year	100-year
Duration↓	Rainfall (in)					
5 min	0.33	0.44	0.53	0.64	0.73	0.82
10 min	0.50	0.67	0.80	0.98	1.11	1.24
15 min	0.62	0.83	1.00	1.21	1.37	1.54
30 min	0.83	1.12	1.34	1.63	1.85	2.07
1 hour	1.03	1.39	1.66	2.02	2.29	2.56
2 hour	1.19	1.57	1.87	2.27	2.58	2.90
3 hour	1.26	1.64	1.94	2.36	2.70	3.04
6 hour	1.48	1.87	2.18	2.62	2.96	3.31
12 hour	1.78	2.23	2.59	3.08	3.45	3.84
24 hour	2.08	2.73	3.27	4.02	4.65	5.32

5.2 Parameter Estimation

The physical parameters of the sub-basins modeled by HEC-1 were estimated by the procedures in the *FCDMC Hydrology Manual*. The information and procedures used to estimate the aforementioned parameters are contained in the following sections. Parameter values are summarized for each sub-basin within the *FCDMC Drainage Design Management System for Windows* (DDMSW) software output located in Appendices A and Appendix B, for the existing and proposed conditions, respectively.

5.2.1 Drainage Area

For the existing conditions, the sub-basin drainage areas were determined for use in the HEC-1 model and are shown on Exhibit 7. For the proposed condition, the sub-basin drainage areas were determined for use in the HEC-1 model and are shown on Exhibit 8.

5.2.2 Precipitation

Due to the approximate one square mile size of the largest contributing watershed of the Galloway Wash, the 100-year, 6-hour storm frequency was used for the hydrology analysis. Rainfall distributions based on watershed area are furnished by the FCDMC. The contributing watershed area and corresponding precipitation pattern were determined and input into the HEC-1 model using the JD and PC record option.

Point precipitation values used in this study were derived from the isopluvial maps in the *FCDMC Hydrology Manual* which, in turn, were derived from the *NOAA Atlas XIV, Volume III*. The 100-year, 6-hour point rainfall depth used for the Site is 3.31 inches.

5.2.3 Soil Data

A description of the soils in the watershed is contained within the *NRCS Soil Survey of Aguila-Carefree Area, Parts of Maricopa and Pinal Counties, Arizona*. Based on the NRCS surveys, the Site's watersheds lie mainly within several soil map units: Unit 93, 96, 34 and for the Galloway Wash Unit 6, with the upstream watersheds consisting of primarily Unit 33. According to the soil survey, the soil

surface is primarily gravely clay and sandy loam. Exhibit 4 – *Soils Classification Map* depicts the contributing watersheds and soil designations.

5.2.4 Rainfall Losses

Rainfall losses were estimated using the Green and Ampt infiltration equation. The procedures used are described in the following paragraphs and were utilized for both existing and proposed condition HEC-1 modeling.

The composite (unadjusted) XKSAT parameter was calculated in the *FCDMC DDMSW* program using the log-average method for each sub-basin. This was accomplished by multiplying the total area of each soil map unit in the sub-basin by the common logarithm of the associated XKSAT value. The resultant products were then totaled and the sum was divided by the total area of the sub-basin. The result is the composite log-average bare ground XKSAT parameter. The log-average XKSAT parameter was then adjusted for the effects of vegetation cover using data from Figure 4.4 of the *FCDMC Hydrology Manual*. The volumetric soil moisture deficit at the start of rainfall (DTHETA) and wetting front capillary suction (PSIF) parameters are directly related to the composite bare ground hydraulic conductivity (XKSAT) by Figure 4.3 in the *FCDMC Hydrology Manual*.

The DTHETA parameters were read from lookup tables within the DDMSW program using the unadjusted XKSAT value calculated as described above. Two (2) DTHETA conditions are possible, dry and normal. The "dry" condition was used for all areas in the existing condition model for undeveloped desert areas. The "normal" condition was used for all other land uses occurring within the project watershed for the proposed condition model. DTHETA values were read from the lookup tables corresponding to the unadjusted XKSAT value, and were averaged by land use area-weighting within the DDMSW program. The value of PSIF was also read from lookup tables based on the unadjusted XKSAT value.

Initial abstraction (IA) and percent impervious (RTIMP) values correlate to soil types and land use. The following section further discusses the hydrologic significance of the IA and RTIMP parameters.

5.2.5 Land Use Characteristics

Land use characteristics upstream of the study area were obtained from the *Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN and were verified from aerial photographs. Surface characteristics affecting the hydrology include terrain (land use classification), the proportion of impervious surfaces, and vegetative cover density. *DDMSW* values for initial abstraction (IA), percent impervious (RTIMP) were obtained from the *FCDMC Hydrology Manual*. The *Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN was used to estimate the hydraulic efficiency (Kn) for the *Rural Density Residential* land use parameters. Assigned values for all parameters are shown in Appendices A and Appendix B for existing and proposed land use conditions, respectively.

5.2.6 Unit Hydrographs

To be consistent with the approved *Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN, the *FCDMC Hydrology Manual* Phoenix Desert/Rangeland unit hydrograph was used for watersheds upstream of the project Site and for the undeveloped on-site areas. The Phoenix Valley S-graph unit hydrograph was used for the post-development on-site conditions. Separate unit hydrographs are generated for each sub-basin by the use of the *DDMSW* program. This program calculates the basin lag time. Assigned values for all parameters are shown within the printouts of the *FCDMC DDMSW* located within Appendix A and Appendix B for the existing and proposed conditions, respectively.

5.2.7 Computation Time Interval

The computation time interval (NMIN) used in the HEC-1 models was based on guidelines in the *FCDMC Hydrology Manual*, which recommends an NMIN value of $0.15 * T_c$. Due to the small post-development on-site watershed areas, a minimum NMIN value of 1 minute was used and a 15-minute hydrograph time interval was used for the entire study area. For comparison purposes, a 1-minute NMIN value and a 15-minute hydrograph time interval was also used for the existing condition model.

5.2.8 Routing Parameters

Routing of sub-basin hydrographs in the study area will be performed utilizing the normal depth/storage channel routing option of HEC-1.

5.2 Rational Method

The Rational Method will be used to compute peak discharges to size on-site culverts with watersheds less than or equal to 160 acres. Parameters necessary for this procedure are the measurement of drainage sub-basin areas, runoff coefficient (“C” values), and calculation of rainfall intensity. Runoff coefficients will be calculated using the values based on Figure 4-5 “Runoff Coefficients for Use with Rational Method” in the *DS&PM*.

5.3 Storm Water Storage

Based on the City of Scottsdale’s Drainage Ordinance stormwater storage requirements, on-site first flush storm water storage is proposed to be provided for the first 0.5-inch of runoff. The Rational Method is used to estimate the first flush stormwater storage volumes. The required volume is based on the areas of the proposed disturbances within the development. The equations used to calculate the required and provided retention volumes are presented below.

5.3.1 Required Retention Volume

Retention volume required for the 100-year, 2-hour event is:

$$V_{\text{REQUIRED}} = C * \left(\frac{P}{12} \right) * A$$

Where:

- V is the required retention volume in acre-feet.
- C is the weighted “C” coefficient.
- P is the precipitation in inches for the 100-year, 2-hour rainfall; 2.70- inches for the proposed developed areas.
- A is the drainage area in acres.

5.3.2 Provided Retention Volume

Retention volume provided for the 100-year, 2-hour event is:

$$V_{\text{PROVIDED}} = \left[\left(\frac{A_1 + A_2}{2} \right) * (ELEV_2 - ELEV_1) \right]$$

Where:

- V is the provided retention volume in acre-feet.
- A is the contour area in acres.
- ELEV is the contour elevation.

Section 3.3 of this Report describes the preliminary storm water storage proposed basin locations. Assumptions that were made when determining provided volume were maximum side slopes of 4:1. Refer to stormwater storage basin volume calculations in Appendix C.

5.4 Hydraulic Procedures

5.4.1 Hydraulic Analysis of Open Channels

Due to the fact that the Galloway Wash and Wash A have significant 100-year peak flows, the U.S. Army Corps of Engineers' HEC-RAS computer program was used for the hydraulic analysis of both washes. Washes that were determined to be less than a 100-year peak flow of 150 cfs will use hydraulic analysis software such as AutoCad Civil 3D *Hydroflow* (Version 10.5) to determine their existing wash hydraulic conditions.

5.4.2 HEC-RAS Errors, Warnings, and Notes

The HEC-RAS cross sections were placed such that significant variations in the channel cross-sectional geometry are adequately represented. Due to the relatively steep slopes on the Site, it was not feasible to put enough cross-sections such that the difference in energy grade elevations is less than 1-foot between cross sections. The HEC-RAS computer program gives a warning message for a difference in energy grade elevation of greater than 1-foot between cross sections. Hence, a wash was analyzed at an elevation difference of 1-foot to demonstrate the impact of additional cross sections on the HEC-RAS analysis. It was found that increasing the number of cross-sections did not impact the flow depths, as long as adequate cross-sections representative of the geometry of the channel are included.

HEC-RAS gives multiple warning messages when the energy equation could not be balanced, resulting in the program using the critical depth for the water

surface elevation. This is due to the fact that a sub-critical flow regime was selected for the steady flow computations on a site that is relatively steep. The on-site washes are flowing super-critical, but a sub-critical flow regime was chosen because it results in the highest water surface elevations.

Since the proposed culverts generally follow the same slope as the washes, the flow in the culverts is also flowing super-critical, as noted by HEC-RAS with a note for each culvert within the model. These warnings and notes are expected when running sub-critical flow regimes for a steady flow analysis on steep sites.

HEC-RAS gives a warning message when the upstream conveyance ratio divided by the downstream conveyance ratio is less than 0.7 or greater than 1.4. Again, this is a result of the steep slopes on the Site, and it was not feasible to put enough cross-sections such that the difference in the conveyance ratio is less than 0.7 or greater than 1.4.

There are a number of warning messages generated by the HEC-RAS computer program for the hydraulic analysis of the project. These warnings do not affect the accuracy of the results and are intended to alert the user of any conditions outside of the expected norm. These warning messages, and notes for both the Developed Conditions and the Existing Conditions models were ignored, and the model was determined to be acceptable.

5.4.3 Hydraulic Analysis of Culverts

For analysis of anticipated additional culvert roadway crossings, the computer programs AutoCad Civil 3D *Hydroflow* (Version 10.5), CulvertMaster and/or HY-8 will be used for final design. Site characteristics and flow are entered into the program and the resulting pipe sizes, flow regime, headwater and tailwater values are calculated.

6.0 DRAINAGE PLAN REQUIREMENTS

6.1 Drainage System Requirements

The Site is being rezoned to approximately 44-acres of R4 zoning and approximately 48-acres of Open Space (OS) zoning. There are specific drainage system requirements in order for the proposed drainage design to be approved for the Site by the City of Scottsdale and are as follows:

1. According to the City of Scottsdale's Floodplain Ordinance for property located within the ESL, storm water storage may be waived under certain conditions and peak discharges from the Site reduced to at least pre-development values during the 100-year, 10-year and 2-year storm frequencies. The development must obtain a Stormwater Storage Waiver from the City of Scottsdale. In addition to the Stormwater Storage Waiver, the Site must either provide first-flush treatment and/or first-flush stormwater storage for the first 0.5-inch of runoff from the property's developed areas.
2. The proposed drainage system is being designed to allow existing drainage patterns to be maintained in their natural condition and location where possible. When the proposed development will disturb existing washes with peak 100-year discharges of 50 cfs or more, the washes will be modified to re-direct flow around and/or through the development to maintain historical flow patterns. The development must obtain a Wash Modification approval from the City of Scottsdale.
3. Any proposed development encroachment into the FEMA FIRM SFHAs must obtain an FCDMC Floodplain Use Permit.
4. The determination of the CWA Section 404 Jurisdictional Washes for the Site must be approved by the U.S. Army Corps.
5. Before any construction activities that will disturb one or more acres begin, these activities must be authorized by ADEQ under the Arizona Pollutant Discharge Elimination System (AZPDES) Construction General Permit. The City of Scottsdale also requires evidence of compliance before issuing development permits.

6.2 Easement Requirements

Where flows from the 100-year storm event are greater than 50 cfs, drainage easements will be required around the limits of the 100-year floodplain inundation. In addition, drainage easements will also be dedicated around the limits of the 100-year ponding for the retention and detention basins.

6.3 Roadway Crossing Requirements

In all cases, the depth of flow over streets will be in accordance with the City of Scottsdale Floodplain Ordinance and Design Standards & Policies Manual (2010).

6.4 Lowest Floor Elevations

Lowest floor (LF) elevations are to be a minimum of 12-inches above the highest adjacent 100-year water surface elevation and 14-inches above the low-site outfall. Lowest floor elevations on the grading and drainage plans for residential units reflect slab on grade conditions and cannot be lowered without agency approval in locations where 'Special Flood Hazard Areas' exist. In non-flood hazard locations, to ensure that adequate residential lot drainage can be achieved, a professional engineer should be consulted if the lowest floor elevation for the slab is proposed to be lowered, or if a basement is to be constructed.

6.5 Maintenance

Ongoing maintenance of the designed or recommended drainage systems is required to preserve the design integrity and purpose of the drainage system. Failure to provide maintenance can prevent the drainage system from performing to its intended design purpose and can result in reduced performance. Maintenance within the public right-of-way is the responsibility of the governing municipality. However, it is the responsibility of private developers, homeowner associations, etc. for facilities on private property within drainage easements and includes private streets. A regular maintenance program is required so that drainage systems perform to the level of protection or service as presented in this report and the project's plans and specifications.

Regular maintenance must be performed on detention/retention basins that are designed with sediment pools and/or are susceptible to wash sediment loads. Observation is required annually and after major storm events to monitor basin sediment load. Basins should be maintained and cleaned out in order for the drainage system to function properly.

6.6 Bank Protection

Scour protection shall be provided at all locations where the wash banks are being modified, where development is encroaching within the wash's erosion hazard setback limits and where it is necessary to protect proposed retention/detention basins. The bank protection considered may be one of a variety of choices: integrally colored soil cement and/or reinforced shotcrete/concrete, seated grouted rock riprap and/or gabion baskets. Bank protection will be designed to extend to a maximum calculated scour depth of 4-5 feet below the wash bottom. Refer to Exhibit 10 – *Developed Condition Hydraulics Map* for bank protection locations, and Appendix E for scour depth calculations.

6.7 Erosion Protection

Culverts that convey flow beneath roadways are to incorporate erosion protection at both the inlet and the outlet of the structures to dissipate energy and provide flow line scour protection. Detention and/or retention basins that utilize weir inlet and/or outlet structures will require erosion protection to prevent scour when flows overtops the weir. Bleed-off pipes will also incorporate riprap protection at pipe outlets.

7.0 CONCLUSIONS

1. The Site is located within the Flood Insurance Rate Map (FIRM) for Maricopa County, Arizona and Incorporated Areas, Panel Number 04013C0884L, effective date October 16, 2013 and is located within Special Flood Hazard Areas (SFHAs) Zone AE, Other Flood Areas Zone "X" (Shaded) and Other Areas Zone "X".
2. The proposed Site development will not alter the effective FEMA SFHA floodplain and/or floodway and will not change the effective FEMA SFHA base flood elevations. This will be documented in more detail as the Site progresses into the pre-plat phase of development.
3. The Galloway Wash and Wash 'A' will be maintained in their natural location and condition where possible and will not be re-aligned.
4. According to the City of Scottsdale's Floodplain Ordinance the Site is located within the ESL and will apply for a stormwater storage waiver
5. Peak discharges from the Site will be reduced to at least pre-development values during the 100-year, 10-year and 2-year storm frequencies.
6. The Site will must provide either first flush stormwater storage for the first 0.5-inch of runoff and/or first flush treatment for the property's developed areas.
7. The proposed drainage system is being designed to allow existing drainage patterns to be maintained in their natural location and condition where possible. Where the proposed development will disturb existing washes with peak 100-year discharges of 50 cfs or more, the washes will be modified to re-direct flow around and/or through the development to maintain historical flow patterns. The development will obtain a Wash Modification approval from the City of Scottsdale.
8. Any proposed development encroachment into the FEMA FIRM SFHAs must obtain an FCDMC Floodplain Use Permit.
9. The determination of the CWA Section 404 Jurisdictional Washes for the Site must be approved by the U.S. Army Corps.
10. Before any construction activities that will disturb one or more acres begin, these activities must be authorized by ADEQ under the Arizona Pollutant Discharge Elimination System (AZPDES) Construction General Permit. The City of Scottsdale also requires evidence of compliance before issuing development permits.

11. The design of hydraulic structures is based on generally accepted engineering practices and in accordance with City of Scottsdale's requirements.
12. The drainage for the local residential roadways will be designed consistent with City of Scottsdale Ordinance 37-42(4) for the allowable depth of water on the street when the street is being utilized as a water carrier.
13. Ongoing maintenance is required for all drainage systems in order to assure design performance. Regular maintenance must be performed on detention/retention basins that are designed with sediment pools and/or are susceptible to wash sediment loads. Observation is required annually and after major storm events to monitor basin sediment load. Basins should be maintained and cleaned out in order for the drainage system to function properly.

8.0 WARNING & DISCLAIMER OF LIABILITY

Per the requirements outlined in Chapter 4 of the City of Scottsdale Design Standards and Policies Manual (DS&PM), each drainage report must include a completed 'Warning and Disclaimer of Liability' as provided within the DS&PM Appendix 4-1C. As such, below is a City of Scottsdale 'Warning and Disclaimer of Liability' that will be completed as the Site progress into the pre-plat phase of development.



Appendix 4-1C
WARNING & DISCLAIMER OF LIABILITY

The Drainage and Floodplain Regulations and Ordinances of the City of Scottsdale are intended to "minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding caused by the surface runoff of rainfall" (Scottsdale Revised Code §37-16).

As defined in S.R.C. §37-17, a flood plain or "Special flood hazard area means an area having flood and/or flood related erosion hazards as shown on a FHBM or FIRM as zone A, AO, A1-30, AE, A99, AH, or E, and those areas identified as such by the floodplain administrator, delineated in accordance with subsection 37-18(b) and adopted by the floodplain board." It is possible that a property could be inundated by greater frequency flood events or by a flood greater in magnitude than a 100-year flood. Additionally, much of the Scottsdale area is a dynamic flood area; that is, the floodplains may shift from one location to another, over time, due to natural processes.

WARNING AND DISCLAIMER OF LIABILITY PURSUANT TO S.R.C §37-22

"The degree of flood protection provided by the requirements in this article is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by man-made or natural causes. This article (Chapter 37, Article II) shall not create liability on the part of the city, any officer or employee thereof, or the federal government for any flood damages that result from reliance on this article or any administrative decision lawfully made thereunder."

Compliance with Drainage and Floodplain Regulations and Ordinances does not insure complete protection from flooding. The Floodplain Regulations and Ordinances meet established local and federal standards for floodplain management, but neither this review nor the Regulations and Ordinances take into account such flood related problems as natural erosion, streambed meander or man-made obstructions and diversions, all of which may have an adverse affect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations.

I have read and understand the above. If I am an agent for an owner I have made the owner aware of and explained this disclaimer.

Plan Check No. Owner or Agent Date

9.0 REFERENCES

1. *Design Standards and Policies Manual Chapter 4 Drainage*, City of Scottsdale, January 2010.
2. *Drainage Design Manual for Maricopa County, Arizona: Volume I – Hydrology*, Flood Control District of Maricopa County, revised August 15, 2013.
3. *Drainage Design Manual for Maricopa County, Arizona: Volume II – Hydraulics*, Flood Control District of Maricopa County, revised August 15, 2013.
4. *HEC-1, Flood Hydrograph Package*, U.S. Army Corps of Engineers, June 1998.
5. *HEC-RAS, Version 4.1.0*, U.S. Army Corps of Engineers, January 2010.
6. *Drainage Design Management System for Windows-Version 4.624d*, Flood Control District of Maricopa County, revised September 17, 2013, by KVL Consultants, Inc.

APPENDIX A

Existing Condition Hydrologic Calculations

DDMSW Output Data

2-year HEC-1 Model

10-year HEC-1 Model

100-year HEC-1 Model

DDMSW Output Data

Flood Control District of Maricopa County
 Drainage Design Management System
 SOILS

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
A1	645	33	64533	0.085	43.00	0.230	-	100	
	645	40	64540	0.083	42.20	0.170	-	100	
	645	93	64593	0.023	11.50	0.330	-	100	
	645	96	64596	0.007	3.30	0.070	-	100	
A2	645	93	64593	0.010	69.20	0.330	-	100	
	645	96	64596	0.005	30.80	0.070	-	100	
B1	645	33	64533	0.000	0.90	0.230	-	100	
	645	93	64593	0.011	99.10	0.330	-	100	
C1	645	93	64593	0.008	100.00	0.330	-	100	
D1	645	93	64593	0.002	100.00	0.330	-	100	
E1	645	33	64533	0.022	100.00	0.230	-	100	
E2	645	33	64533	0.001	100.00	0.230	-	100	
E3	645	33	64533	0.038	100.00	0.230	-	100	
E4	645	6	6456	0.043	4.60	0.620	-	100	
	645	33	64533	0.712	77.30	0.230	-	100	
	645	34	64534	0.004	0.50	0.230	-	100	
	645	40	64540	0.014	1.50	0.170	-	100	
	645	63	64563	0.079	8.60	0.140	25.00	100	
	645	96	64596	0.069	7.50	0.070	-	100	
E5	645	6	6456	0.019	29.70	0.620	-	100	
	645	33	64533	0.013	21.40	0.230	-	100	
	645	93	64593	0.010	16.10	0.330	-	100	
	645	96	64596	0.021	32.70	0.070	-	100	
F1	645	96	64596	0.001	100.00	0.070	-	100	
G1	645	34	64534	0.007	94.40	0.230	-	100	
	645	96	64596	0.000	5.60	0.070	-	100	
H1	645	34	64534	0.005	100.00	0.230	-	100	
I1	645	34	64534	0.006	100.00	0.230	-	100	
J1	645	34	64534	0.003	94.30	0.230	-	100	
	645	96	64596	0.000	5.70	0.070	-	100	
K1	645	33	64533	0.004	17.50	0.230	-	100	
	645	34	64534	0.015	75.00	0.230	-	100	
	645	96	64596	0.002	7.50	0.070	-	100	
L1	645	33	64533	0.003	100.00	0.230	-	100	
M1	645	33	64533	0.002	100.00	0.230	-	100	

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: DM19 EX

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
Major Basin ID: 01									
A1	901	0.0181	9.2	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.1792	90.8	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		0.1973	100.0						
A2	900	0.0146	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0146	100.0						
B1	900	0.0115	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0115	100.0						
C1	900	0.0084	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0084	100.0						
D1	900	0.0024	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0024	100.0						
E1	900	0.0018	8.3	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.0045	20.7	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.0154	71.0	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		0.0217	100.0						
E2	900	0.0012	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0012	100.0						
E3	900	0.0038	10.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.0057	15.0	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.0285	75.0	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		0.0380	100.0						

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: DM19 EX

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
Major Basin ID: 01									
E4	130	0.0074	0.8	0.18	15	35.0	NORMAL	0.040	Large Lot Residential - Single Family (1 du per acre to 2 du
	150	0.0250	2.7	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	220	0.0067	0.7	0.07	80	10.0	NORMAL	0.020	Neighborhood Retail Center
	900	0.1237	13.5	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.2047	22.3	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.5446	59.3	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
	903	0.0066	0.7	0.20	0	35.0	NORMAL	0.050	Dedicated Open Space
		0.9187	100.0						
E5	900	0.0626	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0626	100.0						
F1	900	0.0012	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0012	100.0						
G1	900	0.0071	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0071	100.0						
H1	900	0.0051	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0051	100.0						
I1	900	0.0064	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0064	100.0						
J1	900	0.0035	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0035	100.0						
K1	900	0.0200	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: DM19 EX

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
Major Basin ID: 01									
		0.0200	100.0						
L1	900	0.0027	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0027	100.0						
M1	900	0.0015	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0015	100.0						

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Area ID	Sub Basin Parameters								Rainfall Losses				
	Area (sq mi)	Length (mi)	Slope (ft/mi)	S-Graph	Lca (mi)	Lag (min)	Velocity (f/s)	Kn	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)
Major Basin ID: 01													
A1	0.197	0.95	134.1	DESERT/RANGE	0.51	16.80	4.96	0.039	0.29	0.25	5.34	0.258	5
A2	0.015	0.19	118.3	DESERT/RANGE	0.10	5.10	3.20	0.040	0.35	0.37	5.24	0.250	
B1	0.011	0.22	209.1	DESERT/RANGE	0.10	4.90	3.96	0.040	0.35	0.35	4.33	0.401	
C1	0.008	0.21	216.3	DESERT/RANGE	0.09	4.60	4.00	0.040	0.35	0.35	4.33	0.403	
D1	0.002	0.09	252.9	DESERT/RANGE	0.04	2.30	3.27	0.040	0.35	0.35	4.33	0.403	
E1	0.022	0.31	184.7	DESERT/RANGE	0.15	6.40	4.35	0.038	0.28	0.26	5.05	0.306	4
E2	0.001	0.05	297.9	DESERT/RANGE	0.02	1.40	3.00	0.040	0.35	0.36	5.05	0.281	
E3	0.038	0.57	146.3	DESERT/RANGE	0.25	10.40	4.85	0.039	0.29	0.26	5.05	0.299	4
E4	0.921	3.33	259.5	DESERT/RANGE	1.69	36.70	7.99	0.038	0.28	0.27	5.24	0.281	6
E5	0.063	0.39	123.7	DESERT/RANGE	0.23	9.20	3.71	0.040	0.35	0.36	5.14	0.272	
F1	0.001	0.05	215.7	DESERT/RANGE	0.02	1.50	2.96	0.040	0.35	0.30	7.94	0.085	
G1	0.007	0.24	167.4	DESERT/RANGE	0.09	5.10	4.15	0.040	0.35	0.36	5.14	0.264	
H1	0.005	0.17	189.3	DESERT/RANGE	0.07	3.90	3.77	0.040	0.35	0.36	5.05	0.281	
I1	0.006	0.23	191.1	DESERT/RANGE	0.11	5.20	3.80	0.040	0.35	0.36	5.05	0.281	
J1	0.003	0.16	206.5	DESERT/RANGE	0.07	3.80	3.64	0.040	0.35	0.36	5.14	0.262	
K1	0.020	0.23	171.7	DESERT/RANGE	0.15	6.10	3.38	0.040	0.35	0.37	5.24	0.257	
L1	0.003	0.12	243.7	DESERT/RANGE	0.05	2.90	3.62	0.040	0.35	0.36	5.05	0.281	
M1	0.001	0.10	204.1	DESERT/RANGE	0.04	2.60	3.38	0.040	0.35	0.36	5.05	0.281	

* Non default value

2-year HEC-1 Model

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 15JUN16 TIME 20:29:19
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*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 EX - Desert Mountain 19 Existing Condition
3 ID 2 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19EX2.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 1.475 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 1.466 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 1.438 2.8
21 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
22 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
23 PC 0.950 0.963 0.975 0.988 1.000
*
24 KK A1 BASIN
25 BA 0.197
26 LG 0.29 0.25 5.34 0.26 5
27 UI 0 39 39 39 50 114 146 182 223 254
28 UI 279 323 344 355 374 378 378 369 357 338
29 UI 302 275 245 222 200 181 164 147 133 120
30 UI 108 94 88 75 74 62 61 53 42 42
31 UI 42 31 27 27 27 25 10 10 10
32 UI 10 10 10 10 10 10 10 10 10 10
33 UI 10 10 0 0 0 0 0 0 0 0
*
34 KK R-A1 ROUTE
35 RS 4 FLOW
36 RC 0.060 0.040 0.060 980 0.0224 19.00
37 RX 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
38 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*
39 KK A2 BASIN
40 BA 0.015
41 LG 0.35 0.37 5.24 0.25 0
42 UI 0 10 31 61 85 95 85 63 45 32
43 UI 22 16 11 8 6 2 2 2 2 0
44 UI 0 0 0 0 0 0 0 0 0 0
45 UI 0 0 0 0 0 0 0 0 0 0
46 UI 0 0 0 0 0 0 0 0 0 0
*

```

1

HEC-1 INPUT

PAGE 2

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
47 KK CP-A2 COMBINE
48 HC 2 .212
*
49 KK B1 BASIN
50 BA 0.011
51 LG 0.35 0.35 4.33 0.40 0

```

52	UI	0	8	25	49	67	72	62	44	31	21
53	UI	15	10	7	5	3	2	2	2	2	0
54	UI	0	0	0	0	0	0	0	0	0	0
55	UI	0	0	0	0	0	0	0	0	0	0
56	UI	0	0	0	0	0	0	0	0	0	0
	*										
57	KK	C1	BASIN								
58	BA	0.008									
59	LG	0.35	0.35	4.33	0.40	0					
60	UI	0	6	22	41	54	55	43	29	20	13
61	UI	10	6	4	3	1	1	1	0	0	0
62	UI	0	0	0	0	0	0	0	0	0	0
63	UI	0	0	0	0	0	0	0	0	0	0
64	UI	0	0	0	0	0	0	0	0	0	0
	*										
65	KK	D1	BASIN								
66	BA	0.002									
67	LG	0.35	0.35	4.33	0.40	0					
68	UI	0	7	24	24	12	6	3	1	1	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
71	UI	0	0	0	0	0	0	0	0	0	0
72	UI	0	0	0	0	0	0	0	0	0	0
	*										
73	KK	CLEAR	COMBINE								
74	HC	3									
	*										
	*										
75	KK	E1	BASIN								
76	BA	0.022									
77	LG	0.28	0.26	5.05	0.31	4					
78	UI	0	12	23	55	81	101	111	106	88	66
79	UI	51	39	29	23	17	12	9	8	5	3
80	UI	3	3	3	3	0	0	0	0	0	0
81	UI	0	0	0	0	0	0	0	0	0	0
82	UI	0	0	0	0	0	0	0	0	0	0
	*										

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HEC-1 INPUT

PAGE 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
83	KK	E2	BASIN								
84	BA	0.001									
85	LG	0.35	0.36	5.05	0.28	0					
86	UI	0	10	19	7	2	1	0	0	0	0
87	UI	0	0	0	0	0	0	0	0	0	0
88	UI	0	0	0	0	0	0	0	0	0	0
89	UI	0	0	0	0	0	0	0	0	0	0
90	UI	0	0	0	0	0	0	0	0	0	0
	*										
91	KK	E3	BASIN								
92	BA	0.038									
93	LG	0.29	0.26	5.05	0.30	4					
94	UI	0	12	12	26	47	66	83	99	110	117
95	UI	118	113	105	88	74	63	54	45	38	32
96	UI	27	23	19	17	13	13	8	8	8	5
97	UI	3	3	3	3	3	3	3	3	3	0
98	UI	0	0	0	0	0	0	0	0	0	0
	*										
99	KK	E4	BASIN								
100	BA	0.921									
101	LG	0.28	0.27	5.24	0.28	6					
102	UI	0	84	84	84	84	84	84	84	84	173
103	UI	243	243	297	329	364	405	445	470	511	545
104	UI	580	602	598	694	695	721	747	752	771	764
105	UI	810	810	810	810	810	810	803	777	766	760
106	UI	748	720	694	650	614	589	589	531	511	494
107	UI	463	445	432	399	393	367	360	329	329	299
108	UI	299	274	259	259	235	231	231	198	189	189
109	UI	188	159	159	159	159	143	130	130	130	130
110	UI	128	90	90	90	90	90	90	90	87	58
111	UI	58	58	58	58	58	58	58	58	58	58
112	UI	58	30	21	21	21	21	21	21	21	21
113	UI	21	21	21	21	21	21	21	21	21	21
114	UI	21	21	21	21	21	21	21	21	21	21
115	UI	21	21	21	21	21	0	0	0	0	0
	*										
116	KK	CP-E4	COMBINE								
117	HC	4	.982								
	*										
118	KK	R-CPE4	ROUTE								
119	RS	1	FLOW								
120	RC	0.060	0.040	0.060	2050	0.0234	2635.00				
121	RX	0.00	33.00	50.00	130.00	345.00	390.00	447.00	530.00		
122	RY	34.00	25.00	18.00	19.00	18.00	20.00	26.00	32.00		
	*										

1

HEC-1 INPUT

PAGE 4

LINE	ID	1	2	3	4	5	6	7	8	9	10
123	KK	E5	BASIN								
124	BA	0.063									
125	LG	0.35	0.36	5.14	0.27	0					
126	UI	0	23	23	65	105	143	176	204	217	221
127	UI	209	184	152	126	105	86	71	58	48	41

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW	(<---) RETURN OF DIVERTED OR PUMPED FLOW
NO.	(.) CONNECTOR		
24	A1		
	V		
	V		
34	R-A1		
	.		
39	A2		
	.		
47	CP-A2	
	.		
49	B1		
	.		
57	C1		
	.		
65	D1		
	.		
73	CLEAR	
	.		
75	E1		
	.		
83	E2		
	.		
91	E3		
	.		
99	E4		
	.		
116	CP-E4	
	V		
	V		
118	R-CPE4		
	.		
123	E5		
	.		
131	CP-E5	
	.		
133	F1		
	.		
141	G1		
	.		
149	H1		
	.		
157	I1		
	.		
165	CLR2	
	.		
167	J1		
	.		
175	K1		
	.		
183	L1		
	.		
191	M1		

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	131.	4.15	9.	2.	2.	.20		
ROUTED TO	R-A1	128.	4.22	9.	2.	2.	.20		
HYDROGRAPH AT	A2	13.	4.03	0.	0.	0.	.01		
2 COMBINED AT	CP-A2	128.	4.22	10.	2.	2.	.21		
HYDROGRAPH AT	B1	8.	4.03	0.	0.	0.	.01		
HYDROGRAPH AT	C1	6.	4.03	0.	0.	0.	.01		
HYDROGRAPH AT	D1	2.	4.02	0.	0.	0.	.00		
3 COMBINED AT	CLEAR	16.	4.03	0.	0.	0.	.02		
HYDROGRAPH AT	E1	21.	4.03	1.	0.	0.	.02		
HYDROGRAPH AT	E2	1.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	E3	31.	4.08	2.	0.	0.	.04		
HYDROGRAPH AT	E4	236.	4.43	33.	8.	6.	.92		
4 COMBINED AT	CP-E4	230.	4.43	34.	9.	6.	.98		
ROUTED TO	R-CPE4	230.	4.43	34.	9.	6.	.98		
HYDROGRAPH AT	E5	44.	4.07	2.	0.	0.	.06		
2 COMBINED AT	CP-E5	230.	4.43	35.	9.	6.	1.04		
HYDROGRAPH AT	F1	1.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	G1	6.	4.03	0.	0.	0.	.01		
HYDROGRAPH AT	H1	5.	4.03	0.	0.	0.	.00		
HYDROGRAPH AT	I1	5.	4.03	0.	0.	0.	.01		
4 COMBINED AT	CLR2	17.	4.02	1.	0.	0.	.02		
HYDROGRAPH AT	J1	3.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	K1	17.	4.05	1.	0.	0.	.02		
HYDROGRAPH AT	L1	3.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	M1	1.	4.02	0.	0.	0.	.00		

*** NORMAL END OF HEC-1 ***

10-year HEC-1 Model

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 15JUN16 TIME 20:29:28
*
*****

```

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION , DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 EX - Desert Mountain 19 Existing Condition
3 ID 10 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19EX10.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 2.182 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 2.169 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 2.127 2.8
21 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
22 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
23 PC 0.950 0.963 0.975 0.988 1.000
*
24 KK A1 BASIN
25 BA 0.197
26 LG 0.29 0.25 5.34 0.26 5
27 UI 0 39 39 39 50 114 146 182 223 254
28 UI 279 323 344 355 374 378 378 369 357 338
29 UI 302 275 245 222 200 181 164 147 133 120
30 UI 108 94 88 75 74 62 61 53 42 42
31 UI 42 31 27 27 27 27 25 10 10 10
32 UI 10 10 10 10 10 10 10 10 10 10
33 UI 10 10 0 0 0 0 0 0 0 0
*
34 KK R-A1 ROUTE
35 RS 3 FLOW
36 RC 0.060 0.040 0.060 980 0.0224 19.00
37 RX 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
38 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*
39 KK A2 BASIN
40 BA 0.015
41 LG 0.35 0.37 5.24 0.25 0
42 UI 0 10 31 61 85 95 85 63 45 32
43 UI 22 16 11 8 6 2 2 2 2 0
44 UI 0 0 0 0 0 0 0 0 0 0
45 UI 0 0 0 0 0 0 0 0 0 0
46 UI 0 0 0 0 0 0 0 0 0 0
*

```

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
47 KK CP-A2 COMBINE
48 HC 2 .212
*
49 KK B1 BASIN
50 BA 0.011
51 LG 0.35 0.35 4.33 0.40 0

```

52	UI	0	8	25	49	67	72	62	44	31	21
53	UI	15	10	7	5	3	2	2	2	2	0
54	UI	0	0	0	0	0	0	0	0	0	0
55	UI	0	0	0	0	0	0	0	0	0	0
56	UI	0	0	0	0	0	0	0	0	0	0
	*										
57	KK	C1	BASIN								
58	BA	0.008									
59	LG	0.35	0.35	4.33	0.40	0					
60	UI	0	6	22	41	54	55	43	29	20	13
61	UI	10	6	4	3	1	1	1	0	0	0
62	UI	0	0	0	0	0	0	0	0	0	0
63	UI	0	0	0	0	0	0	0	0	0	0
64	UI	0	0	0	0	0	0	0	0	0	0
	*										
65	KK	D1	BASIN								
66	BA	0.002									
67	LG	0.35	0.35	4.33	0.40	0					
68	UI	0	7	24	24	12	6	3	1	1	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
71	UI	0	0	0	0	0	0	0	0	0	0
72	UI	0	0	0	0	0	0	0	0	0	0
	*										
73	KK	CLEAR	COMBINE								
74	HC	3									
	*										
	*										
75	KK	E1	BASIN								
76	BA	0.022									
77	LG	0.28	0.26	5.05	0.31	4					
78	UI	0	12	23	55	81	101	111	106	88	66
79	UI	51	39	29	23	17	12	9	8	5	3
80	UI	3	3	3	3	0	0	0	0	0	0
81	UI	0	0	0	0	0	0	0	0	0	0
82	UI	0	0	0	0	0	0	0	0	0	0
	*										

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HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

83	KK	E2	BASIN								
84	BA	0.001									
85	LG	0.35	0.36	5.05	0.28	0					
86	UI	0	10	19	7	2	1	0	0	0	0
87	UI	0	0	0	0	0	0	0	0	0	0
88	UI	0	0	0	0	0	0	0	0	0	0
89	UI	0	0	0	0	0	0	0	0	0	0
90	UI	0	0	0	0	0	0	0	0	0	0
	*										
91	KK	E3	BASIN								
92	BA	0.038									
93	LG	0.29	0.26	5.05	0.30	4					
94	UI	0	12	12	26	47	66	83	99	110	117
95	UI	118	113	105	88	74	63	54	45	38	32
96	UI	27	23	19	17	13	13	8	8	8	5
97	UI	3	3	3	3	3	3	3	3	3	0
98	UI	0	0	0	0	0	0	0	0	0	0
	*										
99	KK	E4	BASIN								
100	BA	0.921									
101	LG	0.28	0.27	5.24	0.28	6					
102	UI	0	84	84	84	84	84	84	84	84	173
103	UI	243	243	297	329	364	405	445	470	511	545
104	UI	580	602	598	694	695	721	747	752	771	764
105	UI	810	810	810	810	810	810	803	777	766	760
106	UI	748	720	694	650	614	589	589	531	511	494
107	UI	463	445	432	399	393	367	360	329	329	299
108	UI	299	274	259	259	235	231	231	198	189	189
109	UI	188	159	159	159	159	143	130	130	130	130
110	UI	128	90	90	90	90	90	90	90	87	58
111	UI	58	58	58	58	58	58	58	58	58	58
112	UI	58	30	21	21	21	21	21	21	21	21
113	UI	21	21	21	21	21	21	21	21	21	21
114	UI	21	21	21	21	21	21	21	21	21	21
115	UI	21	21	21	21	21	0	0	0	0	0
	*										
116	KK	CP-E4	COMBINE								
117	HC	4	.982								
	*										
118	KK	R-CPE4	ROUTE								
119	RS	1	FLOW								
120	RC	0.060	0.040	0.060	2050	0.0234	2635.00				
121	RX	0.00	33.00	50.00	130.00	345.00	390.00	447.00	530.00		
122	RY	34.00	25.00	18.00	19.00	18.00	20.00	26.00	32.00		
	*										

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HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

123	KK	E5	BASIN								
124	BA	0.063									
125	LG	0.35	0.36	5.14	0.27	0					
126	UI	0	23	23	65	105	143	176	204	217	221
127	UI	209	184	152	126	105	86	71	58	48	41

SCHMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(-->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<-->) RETURN OF DIVERTED OR PUMPED FLOW
24	A1	
	V	
	V	
34	R-A1	
	.	
39	A2	
	.	
47	CP-A2	
	.	
49	B1	
	.	
57	C1	
	.	
65	D1	
	.	
73	CLEAR	
	.	
75	E1	
	.	
83	E2	
	.	
91	E3	
	.	
99	E4	
	.	
116	CP-E4	
	V	
	V	
118	R-CPE4	
	.	
123	E5	
	.	
131	CP-E5	
	.	
133	F1	
	.	
141	G1	
	.	
149	H1	
	.	
157	I1	
	.	
165	CLR2	
	.	
167	J1	
	.	
175	K1	
	.	
183	L1	
	.	
191	M1	

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+		A1	262.	4.13	19.	5.	3.	.20	
+	ROUTED TO								
+		R-A1	258.	4.18	19.	5.	3.	.20	
+	HYDROGRAPH AT								
+		A2	28.	4.02	1.	0.	0.	.01	
+	2 COMBINED AT								
+		CP-A2	261.	4.18	20.	5.	4.	.21	
+	HYDROGRAPH AT								
+		B1	19.	4.02	1.	0.	0.	.01	
+	HYDROGRAPH AT								
+		C1	14.	4.02	1.	0.	0.	.01	
+	HYDROGRAPH AT								
+		D1	4.	4.02	0.	0.	0.	.00	
+	3 COMBINED AT								
+		CLEAR	36.	4.02	1.	0.	0.	.02	
+	HYDROGRAPH AT								
+		E1	42.	4.02	2.	0.	0.	.02	
+	HYDROGRAPH AT								
+		E2	2.	4.02	0.	0.	0.	.00	
+	HYDROGRAPH AT								
+		E3	63.	4.07	3.	1.	1.	.04	
+	HYDROGRAPH AT								
+		E4	553.	4.43	78.	19.	14.	.92	
+	4 COMBINED AT								
+		CP-E4	549.	4.42	82.	20.	15.	.98	
+	ROUTED TO								
+		R-CPE4	549.	4.43	82.	20.	15.	.98	
+	HYDROGRAPH AT								
+		E5	102.	4.05	5.	1.	1.	.06	
+	2 COMBINED AT								
+		CP-E5	549.	4.42	85.	21.	15.	1.04	
+	HYDROGRAPH AT								
+		F1	2.	4.02	0.	0.	0.	.00	
+	HYDROGRAPH AT								
+		G1	13.	4.02	1.	0.	0.	.01	
+	HYDROGRAPH AT								
+		H1	10.	4.02	0.	0.	0.	.00	
+	HYDROGRAPH AT								
+		I1	11.	4.02	0.	0.	0.	.01	
+	4 COMBINED AT								
+		CLR2	36.	4.02	2.	0.	0.	.02	
+	HYDROGRAPH AT								
+		J1	6.	4.02	0.	0.	0.	.00	
+	HYDROGRAPH AT								
+		K1	36.	4.03	2.	0.	0.	.02	
+	HYDROGRAPH AT								
+		L1	6.	4.02	0.	0.	0.	.00	
+	HYDROGRAPH AT								
+		M1	2.	4.02	0.	0.	0.	.00	

*** NORMAL END OF HEC-1 ***

100-year HEC-1 Model

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 15JUN16 TIME 20:29:36
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1         ID      Flood Control District of Maricopa County
2         ID      DM19 EX - Desert Mountain 19 Existing Condition
3         ID      100 YEAR
4         ID      6 Hour Storm
5         ID      Unit Hydrograph: S-Graph
6         ID      Storm: Multiple
7         ID      06/14/2016
8         ID      WOOD/PATEL FILE NAME: DM19EX100.DAT
          *DIAGRAM
9         IT      1 1JAN99      0      2000
10        IO      5
11        IN      15
          *
12        JD      3.313 0.0001
13        PC      0.000 0.008      0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14        PC      0.087 0.099      0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15        PC      0.962 0.972      0.983 0.991 1.000
16        JD      3.293 0.5000
17        PC      0.000 0.008      0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18        PC      0.087 0.099      0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19        PC      0.962 0.972      0.983 0.991 1.000
20        JD      3.230 2.8
21        PC      0.000 0.009      0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
22        PC      0.087 0.100      0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
23        PC      0.950 0.963      0.975 0.988 1.000
          *
24        KK      A1  BASIN
25        BA      0.197
26        LG      0.29 0.25 5.34 0.26      5
27        UI      0 39 39 39 50 114 146 182 223 254
28        UI      279 323 344 355 374 378 378 369 357 338
29        UI      302 275 245 222 200 181 164 147 133 120
30        UI      108 94 88 75 74 62 61 53 42 42
31        UI      42 31 27 27 27 27 25 10 10 10
32        UI      10 10 10 10 10 10 10 10 10 10
33        UI      10 10 0 0 0 0 0 0 0 0
          *
34        KK      R-A1 ROUTE
35        RS      3 FLOW
36        RC      0.060 0.040 0.060 980 0.0224 19.00
37        RX      0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
38        RY      20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
          *
39        KK      A2  BASIN
40        BA      0.015
41        LG      0.35 0.37 5.24 0.25 0
42        UI      0 10 31 61 85 95 85 63 45 32
43        UI      22 16 11 8 6 2 2 2 2 0
44        UI      0 0 0 0 0 0 0 0 0 0
45        UI      0 0 0 0 0 0 0 0 0 0
46        UI      0 0 0 0 0 0 0 0 0 0
          *

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1

HEC-1 INPUT

PAGE 2

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
47        KK      CP-A2 COMBINE
48        HC      2
          *
49        KK      B1  BASIN
50        BA      0.011
51        LG      0.35 0.35 4.33 0.40 0

```

52	UI	0	8	25	49	67	72	62	44	31	21
53	UI	15	10	7	5	3	2	2	2	2	0
54	UI	0	0	0	0	0	0	0	0	0	0
55	UI	0	0	0	0	0	0	0	0	0	0
56	UI	0	0	0	0	0	0	0	0	0	0
	*										
57	KK	C1	BASIN								
58	BA	0.008									
59	LG	0.35	0.35	4.33	0.40	0					
60	UI	0	6	22	41	54	55	43	29	20	13
61	UI	10	6	4	3	1	1	1	0	0	0
62	UI	0	0	0	0	0	0	0	0	0	0
63	UI	0	0	0	0	0	0	0	0	0	0
64	UI	0	0	0	0	0	0	0	0	0	0
	*										
65	KK	D1	BASIN								
66	BA	0.002									
67	LG	0.35	0.35	4.33	0.40	0					
68	UI	0	7	24	24	12	6	3	1	1	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
71	UI	0	0	0	0	0	0	0	0	0	0
72	UI	0	0	0	0	0	0	0	0	0	0
	*										
73	KK	CLEAR	COMBINE								
74	HC	3									
	*										
	*										
75	KK	E1	BASIN								
76	BA	0.022									
77	LG	0.28	0.26	5.05	0.31	4					
78	UI	0	12	23	55	81	101	111	106	88	66
79	UI	51	39	29	23	17	12	9	8	5	3
80	UI	3	3	3	3	0	0	0	0	0	0
81	UI	0	0	0	0	0	0	0	0	0	0
82	UI	0	0	0	0	0	0	0	0	0	0
	*										

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

83	KK	E2	BASIN								
84	BA	0.001									
85	LG	0.35	0.36	5.05	0.28	0					
86	UI	0	10	19	7	2	1	0	0	0	0
87	UI	0	0	0	0	0	0	0	0	0	0
88	UI	0	0	0	0	0	0	0	0	0	0
89	UI	0	0	0	0	0	0	0	0	0	0
90	UI	0	0	0	0	0	0	0	0	0	0
	*										
91	KK	E3	BASIN								
92	BA	0.038									
93	LG	0.29	0.26	5.05	0.30	4					
94	UI	0	12	12	26	47	66	83	99	110	117
95	UI	118	113	105	88	74	63	54	45	38	32
96	UI	27	23	19	17	13	13	8	8	8	5
97	UI	3	3	3	3	3	3	3	3	3	0
98	UI	0	0	0	0	0	0	0	0	0	0
	*										
99	KK	E4	BASIN								
100	BA	0.921									
101	LG	0.28	0.27	5.24	0.28	6					
102	UI	0	84	84	84	84	84	84	84	84	173
103	UI	243	243	297	329	364	405	445	470	511	545
104	UI	580	602	598	694	695	721	747	752	771	764
105	UI	810	810	810	810	810	810	803	777	766	760
106	UI	748	720	694	650	614	589	589	531	511	494
107	UI	463	445	432	399	393	367	360	329	329	299
108	UI	299	274	259	259	235	231	231	198	189	189
109	UI	188	159	159	159	159	143	130	130	130	130
110	UI	128	90	90	90	90	90	90	90	87	58
111	UI	58	58	58	58	58	58	58	58	58	58
112	UI	58	30	21	21	21	21	21	21	21	21
113	UI	21	21	21	21	21	21	21	21	21	21
114	UI	21	21	21	21	21	21	21	21	21	21
115	UI	21	21	21	21	21	0	0	0	0	0
	*										
116	KK	CP-E4	COMBINE								
117	HC	4	.982								
	*										
118	KK	R-CPE4	ROUTE								
119	RS	1	FLOW								
120	RC	0.060	0.040	0.060	2050	0.0234	2635.00				
121	RX	0.00	33.00	50.00	130.00	345.00	390.00	447.00	530.00		
122	RY	34.00	25.00	18.00	19.00	18.00	20.00	26.00	32.00		
	*										

1

HEC-1 INPUT

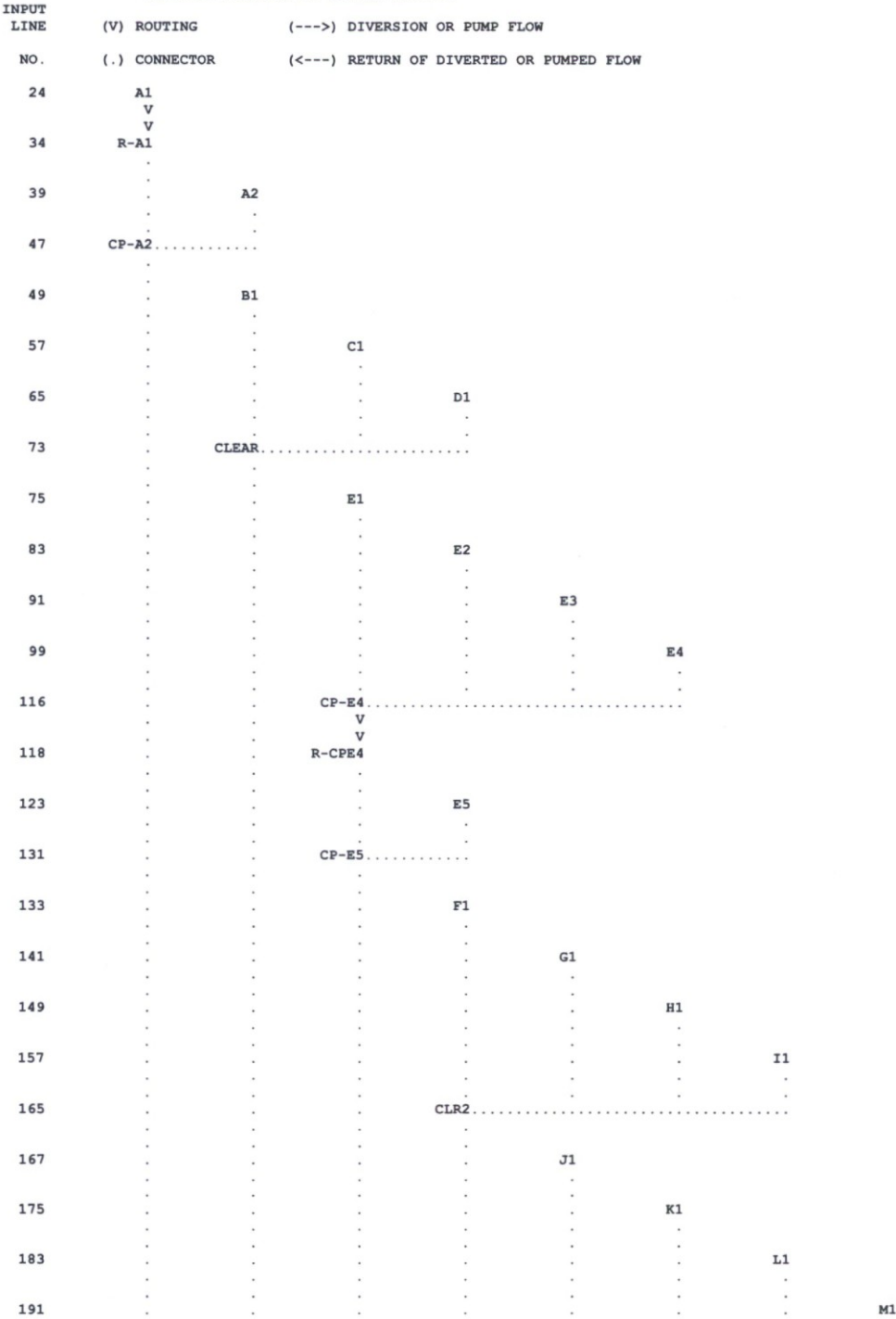
PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

123	KK	E5	BASIN								
124	BA	0.063									
125	LG	0.35	0.36	5.14	0.27	0					
126	UI	0	23	23	65	105	143	176	204	217	221
127	UI	209	184	152	126	105	86	71	58	48	41

128	UI	35	25	24	16	16	16	7	6	6	6
129	UI	6	6	6	6	0	0	0	0	0	0
130	UI	0	0	0	0	0	0	0	0	0	0
	*										
131	KK	CP-E5 COMBINE									
132	HC	2	1.044								
	*										
133	KK	F1	BASIN								
134	BA	0.001									
135	LG	0.35	0.30	7.94	0.09	0					
136	UI	0	9	19	8	2	1	0	0	0	0
137	UI	0	0	0	0	0	0	0	0	0	0
138	UI	0	0	0	0	0	0	0	0	0	0
139	UI	0	0	0	0	0	0	0	0	0	0
140	UI	0	0	0	0	0	0	0	0	0	0
	*										
141	KK	G1	BASIN								
142	BA	0.007									
143	LG	0.35	0.36	5.14	0.26	0					
144	UI	0	5	14	28	40	44	40	29	21	15
145	UI	10	8	5	4	3	1	1	1	1	0
146	UI	0	0	0	0	0	0	0	0	0	0
147	UI	0	0	0	0	0	0	0	0	0	0
148	UI	0	0	0	0	0	0	0	0	0	0
	*										
149	KK	H1	BASIN								
150	BA	0.005									
151	LG	0.35	0.36	5.05	0.28	0					
152	UI	0	5	20	35	41	34	22	14	9	6
153	UI	4	2	1	1	1	0	0	0	0	0
154	UI	0	0	0	0	0	0	0	0	0	0
155	UI	0	0	0	0	0	0	0	0	0	0
156	UI	0	0	0	0	0	0	0	0	0	0
	*										
157	KK	I1	BASIN								
158	BA	0.006									
159	LG	0.35	0.36	5.05	0.28	0					
160	UI	0	4	12	24	33	37	34	26	18	13
161	UI	9	7	5	3	3	1	1	1	1	1
162	UI	0	0	0	0	0	0	0	0	0	0
163	UI	0	0	0	0	0	0	0	0	0	0
164	UI	0	0	0	0	0	0	0	0	0	0
	*										
		HEC-1 INPUT									
LINE	ID	1	2	3	4	5	6	7	8	9	10
165	KK	CLR2 COMBINE									
166	HC	4									
	*										
167	KK	J1	BASIN								
168	BA	0.003									
169	LG	0.35	0.36	5.14	0.26	0					
170	UI	0	3	13	22	25	20	13	8	5	3
171	UI	2	1	1	1	0	0	0	0	0	0
172	UI	0	0	0	0	0	0	0	0	0	0
173	UI	0	0	0	0	0	0	0	0	0	0
174	UI	0	0	0	0	0	0	0	0	0	0
	*										
175	KK	K1	BASIN								
176	BA	0.020									
177	LG	0.35	0.37	5.24	0.26	0					
178	UI	0	11	25	55	81	100	106	97	75	57
179	UI	43	32	23	18	13	10	8	6	3	3
180	UI	3	3	3	0	0	0	0	0	0	0
181	UI	0	0	0	0	0	0	0	0	0	0
182	UI	0	0	0	0	0	0	0	0	0	0
	*										
183	KK	L1	BASIN								
184	BA	0.003									
185	LG	0.35	0.36	5.05	0.28	0					
186	UI	0	6	23	33	25	14	7	4	2	1
187	UI	1	0	0	0	0	0	0	0	0	0
188	UI	0	0	0	0	0	0	0	0	0	0
189	UI	0	0	0	0	0	0	0	0	0	0
190	UI	0	0	0	0	0	0	0	0	0	0
	*										
191	KK	M1	BASIN								
192	BA	0.001									
193	LG	0.35	0.36	5.05	0.28	0					
194	UI	0	3	9	12	7	4	2	1	0	0
195	UI	0	0	0	0	0	0	0	0	0	0
196	UI	0	0	0	0	0	0	0	0	0	0
197	UI	0	0	0	0	0	0	0	0	0	0
198	UI	0	0	0	0	0	0	0	0	0	0
	*										
199	ZZ										

SCHEMATIC DIAGRAM OF STREAM NETWORK



(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	468.	4.13	38.	10.	7.	.20		
ROUTED TO	R-A1	462.	4.18	38.	10.	7.	.20		
HYDROGRAPH AT	A2	50.	4.02	3.	1.	0.	.01		
2 COMBINED AT	CP-A2	469.	4.17	41.	10.	7.	.21		
HYDROGRAPH AT	B1	35.	4.02	2.	0.	0.	.01		
HYDROGRAPH AT	C1	26.	4.02	1.	0.	0.	.01		
HYDROGRAPH AT	D1	7.	4.02	0.	0.	0.	.00		
3 COMBINED AT	CLEAR	67.	4.02	3.	1.	1.	.02		
HYDROGRAPH AT	E1	72.	4.02	4.	1.	1.	.02		
HYDROGRAPH AT	E2	3.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	E3	111.	4.07	7.	2.	1.	.04		
HYDROGRAPH AT	E4	1111.	4.42	165.	41.	30.	.92		
4 COMBINED AT	CP-E4	1115.	4.40	174.	44.	31.	.98		
ROUTED TO	R-CPE4	1116.	4.40	174.	44.	31.	.98		
HYDROGRAPH AT	E5	186.	4.05	11.	3.	2.	.06		
2 COMBINED AT	CP-E5	1124.	4.38	183.	46.	33.	1.04		
HYDROGRAPH AT	F1	4.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	G1	23.	4.02	1.	0.	0.	.01		
HYDROGRAPH AT	H1	17.	4.02	1.	0.	0.	.00		
HYDROGRAPH AT	I1	20.	4.02	1.	0.	0.	.01		
4 COMBINED AT	CLR2	63.	4.02	3.	1.	1.	.02		
HYDROGRAPH AT	J1	10.	4.02	1.	0.	0.	.00		
HYDROGRAPH AT	K1	65.	4.02	3.	1.	1.	.02		
HYDROGRAPH AT	L1	10.	4.02	1.	0.	0.	.00		
HYDROGRAPH AT	M1	3.	4.02	0.	0.	0.	.00		

*** NORMAL END OF HEC-1 ***

APPENDIX B

Developed Condition Hydrologic Calculations

DDMSW Output Data

2-year HEC-1 Model

10-year HEC-1 Model

100-year HEC-1 Model

DDMSW Output Data

Flood Control District of Maricopa County
 Drainage Design Management System
 SOILS

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
A1	645	33	64533	0.085	43.00	0.230	-	100	
	645	40	64540	0.083	42.20	0.170	-	100	
	645	93	64593	0.023	11.50	0.330	-	100	
	645	96	64596	0.007	3.30	0.070	-	100	
A2	645	93	64593	0.010	69.20	0.330	-	100	
	645	96	64596	0.005	30.80	0.070	-	100	
B1	645	33	64533	0.022	100.00	0.230	-	100	
B10	645	93	64593	0.012	100.00	0.330	-	100	
B11	645	93	64593	0.009	92.70	0.330	-	100	
	645	34	64534	0.001	7.30	0.230	-	100	
B12	645	6	6456	0.002	49.00	0.620	-	100	
	645	93	64593	0.003	51.00	0.330	-	100	
B13	645	34	64534	0.000	3.40	0.230	-	100	
	645	96	64596	0.003	86.20	0.070	-	100	
	645	6	6456	0.000	10.30	0.620	-	100	
B14	645	6	6456	0.009	22.90	0.620	-	100	
	645	96	64596	0.016	41.60	0.070	-	100	
	645	34	64534	0.011	29.60	0.230	-	100	
	645	33	64533	0.002	5.20	0.230	-	100	
	645	93	64593	0.000	0.80	0.330	-	100	
B14A	645	96	64596	0.003	69.00	0.070	-	100	
	645	6	6456	0.001	31.00	0.620	-	100	
B2	645	33	64533	0.001	100.00	0.230	-	100	
B3	645	33	64533	0.038	100.00	0.230	-	100	
B4	645	6	6456	0.043	4.60	0.620	-	100	
	645	33	64533	0.712	77.30	0.230	-	100	
	645	34	64534	0.004	0.50	0.230	-	100	
	645	40	64540	0.014	1.50	0.170	-	100	
	645	63	64563	0.079	8.60	0.140	25.00	100	
	645	96	64596	0.069	7.50	0.070	-	100	
B5	645	34	64534	0.006	100.00	0.230	-	100	
B6	645	6	6456	0.002	20.90	0.620	-	100	
	645	33	64533	0.005	54.70	0.230	-	100	
	645	93	64593	0.002	24.40	0.330	-	100	
B6W	645	93	64593	0.000	3.30	0.330	-	100	
	645	33	64533	0.000	3.30	0.230	-	100	
	645	6	6456	0.003	93.30	0.620	-	100	
B8	645	33	64533	0.008	75.90	0.230	-	100	
	645	93	64593	0.002	22.20	0.330	-	100	
	645	6	6456	0.000	1.90	0.620	-	100	
B9	645	93	64593	0.008	100.00	0.330	-	100	
C1	645	33	64533	0.001	3.10	0.230	-	100	
	645	34	64534	0.022	94.70	0.230	-	100	
	645	96	64596	0.001	2.20	0.070	-	100	
L1	645	33	64533	0.003	100.00	0.230	-	100	
M1	645	33	64533	0.002	100.00	0.230	-	100	

Flood Control District of Maricopa County
 Drainage Design Management System
LAND USE
 Project Reference: DM19 PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
Major Basin ID: 01									
A1	901	0.0181	9.2	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.1792	90.8	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		0.1973	100.0						
A2	900	0.0110	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0110	100.0						
B1	900	0.0018	8.3	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.0045	20.7	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.0154	71.0	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		0.0217	100.0						
B10	110	0.0010	8.3	0.30	5	30.0	NORMAL	0.020	Rural Residential (<= 1/5 du per acre)
	150	0.0010	8.3	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	901	0.0100	83.3	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		0.0120	99.9						
B11	150	0.0080	80.0	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0020	20.0	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
		0.0100	100.0						
B12	730	0.0050	100.0	0.10	0	90.0	NORMAL	0.030	Passive Open Space (Includes mountain preserves and washes)
		0.0050	100.0						
B13	150	0.0004	13.3	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0015	50.0	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
	901	0.0011	36.7	0.20	0	75.0	NORMAL	0.030	Recreational Open Space

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: DM19 PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
Major Basin ID: 01									
		0.0030	100.0						
B14	150	0.0070	17.9	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	230	0.0040	10.3	0.10	80	30.0	NORMAL	0.020	Community Commercial (100,000 to 500,000 sq. ft.)
	600	0.0040	10.3	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
	901	0.0240	61.5	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		0.0390	100.0						
B14A	150	0.0040	80.0	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0010	20.0	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
		0.0050	100.0						
B2	900	0.0012	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0012	100.0						
B3	900	0.0038	10.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.0057	15.0	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.0285	75.0	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		0.0380	100.0						
B4	130	0.0074	0.8	0.18	15	35.0	NORMAL	0.040	Large Lot Residential - Single Family (1 du per acre to 2 du
	150	0.0250	2.7	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	220	0.0067	0.7	0.07	80	10.0	NORMAL	0.020	Neighborhood Retail Center
	900	0.1237	13.5	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.2047	22.3	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.5446	59.3	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: DM19 PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
Major Basin ID: 01									
B4	903	0.0066	0.7	0.20	0	35.0	NORMAL	0.050	Dedicated Open Space
		0.9187	100.0						
B5	900	0.0010	16.7	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.0050	83.3	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		0.0060	100.0						
B6	600	0.0003	3.4	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
	730	0.0001	1.1	0.10	0	90.0	NORMAL	0.030	Passive Open Space (Includes mountain preserves and washes)
	901	0.0083	95.4	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		0.0087	99.9						
B6W	600	0.0002	6.5	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
	730	0.0019	61.3	0.10	0	90.0	NORMAL	0.030	Passive Open Space (Includes mountain preserves and washes)
	901	0.0010	32.3	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		0.0031	100.1						
B8	150	0.0090	81.8	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0020	18.2	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
		0.0110	100.0						
B9	150	0.0060	75.0	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0020	25.0	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
		0.0080	100.0						
C1	150	0.0060	25.0	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0020	8.3	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: DM19 PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
Major Basin ID: 01									
C1	901	0.0160	66.7	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		0.0240	100.0						
L1	900	0.0027	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0027	100.0						
M1	900	0.0015	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		0.0015	100.0						

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Area ID	Sub Basin Parameters								Rainfall Losses				
	Area (sq mi)	Length (mi)	Slope (ft/mi)	S-Graph	Lca (mi)	Lag (min)	Velocity (f/s)	Kn	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)
Major Basin ID: 01													
B14A	0.004	0.04	394.7	VALLEY	0.02	1.10	3.17	0.035	0.13	0.26	6.16	0.160	39
A1	0.197	0.95	134.1	DESERT/RANGE	0.51	16.80	4.96	0.039	0.29	0.25	5.34	0.258	5
B6W	0.003	0.17	126.5	VALLEY	0.08	3.20	4.53	0.029	0.13	0.28	3.37	1.041	6
A2	0.011	0.19	118.3	DESERT/RANGE	0.10	5.10	3.20	0.040	0.35	0.37	5.24	0.250	
B5	0.006	0.11	136.4	VALLEY	0.06	2.70	3.60	0.032	0.23	0.27	5.05	0.377	
B6	0.009	0.11	133.9	VALLEY	0.06	2.50	4.01	0.029	0.19	0.25	4.45	0.524	3
B8	0.010	0.18	61.5	VALLEY	0.09	4.80	3.28	0.035	0.13	0.27	4.87	0.295	38
B1	0.022	0.31	184.7	DESERT/RANGE	0.15	6.40	4.35	0.038	0.28	0.26	5.05	0.306	4
B9	0.008	0.08	144.6	VALLEY	0.04	2.20	3.36	0.034	0.13	0.28	4.33	0.376	43
B2	0.001	0.05	297.9	DESERT/RANGE	0.02	1.40	3.00	0.040	0.35	0.36	5.05	0.281	
B10	0.012	0.22	113.6	VALLEY	0.11	4.30	4.53	0.030	0.20	0.25	4.33	0.541	2
B3	0.038	0.57	146.3	DESERT/RANGE	0.25	10.40	4.85	0.039	0.29	0.26	5.05	0.299	4
B11	0.010	0.18	107.3	VALLEY	0.09	4.30	3.62	0.035	0.13	0.27	4.39	0.374	39
B4	0.921	3.33	259.5	DESERT/RANGE	1.69	36.70	7.99	0.038	0.28	0.27	5.24	0.281	6
B12	0.005	0.13	150.4	DESERT/RANGE	0.07	2.80	4.15	0.030	0.10	0.25	3.79	0.851	
B13	0.003	0.05	408.2	VALLEY	0.02	.80	5.44	0.024	0.12	0.24	7.27	0.114	51
B14	0.038	0.33	121.2	VALLEY	0.16	5.50	5.29	0.029	0.17	0.26	5.58	0.266	23
C1	0.023	0.30	82.5	VALLEY	0.15	6.00	4.47	0.031	0.18	0.26	5.05	0.344	14
L1	0.003	0.12	243.7	DESERT/RANGE	0.05	2.90	3.62	0.040	0.35	0.36	5.05	0.281	
M1	0.001	0.10	204.1	DESERT/RANGE	0.04	2.60	3.38	0.040	0.35	0.36	5.05	0.281	

* Non default value

2-year HEC-1 Model

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 15JUN16 TIME 20:59:37
*
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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

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X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION

KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 PROP - Desert Mountain 19 Post Online Det Basins 1st Flush
3 ID 2 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19FT2.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 1.475 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 1.466 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 1.456 1.0
21 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.075
22 PC 0.087 0.099 0.119 0.148 0.230 0.407 0.778 0.881 0.919 0.945
23 PC 0.957 0.968 0.980 0.990 1.000
24 JD 1.438 2.8
25 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
26 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
27 PC 0.950 0.963 0.975 0.988 1.000
*
28 KK A1 BASIN
29 BA 0.197
30 LG 0.29 0.25 5.34 0.26 5
31 UI 0 39 39 39 50 114 146 182 223 254
32 UI 279 323 344 355 374 378 378 369 357 338
33 UI 302 275 245 222 200 181 164 147 133 120
34 UI 108 94 88 75 74 62 61 53 42 42
35 UI 42 31 27 27 27 27 25 10 10 10
36 UI 10 10 10 10 10 10 10 10 10 10
37 UI 10 10 0 0 0 0 0 0 0 0
*
38 KK R-A1 ROUTE
39 RS 4 FLOW
40 RC 0.060 0.040 0.060 980 0.0224 19.00
41 RX 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
42 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
43 KK A2 BASIN
44 BA 0.011
45 LG 0.35 0.37 5.24 0.25 0
46 UI 0 7 22 45 62 70 63 46 33 23
47 UI 16 12 8 6 5 2 2 2 2 0
48 UI 0 0 0 0 0 0 0 0 0 0
49 UI 0 0 0 0 0 0 0 0 0 0
50 UI 0 0 0 0 0 0 0 0 0 0
*
51 KK CP-A2 COMBINE
52 HC 2
*

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53	KK	B1	BASIN								
54	BA	0.022									
55	LG	0.28	0.26	5.05	0.31	4					
56	UI	0	12	23	55	81	101	111	106	88	66
57	UI	51	39	29	23	17	12	9	8	5	3
58	UI	3	3	3	3	0	0	0	0	0	0
59	UI	0	0	0	0	0	0	0	0	0	0
60	UI	0	0	0	0	0	0	0	0	0	0
	*										

61	KK	R-B1	ROUTE								
62	RK	500	0.0200	0.013		CIRC	3.500				
	*										

63	KK	B2	BASIN								
64	BA	0.001									
65	LG	0.35	0.36	5.05	0.28	0					
66	UI	0	10	19	7	2	1	0	0	0	0
67	UI	0	0	0	0	0	0	0	0	0	0
68	UI	0	0	0	0	0	0	0	0	0	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
	*										

71	KK	B3	BASIN								
72	BA	0.038									
73	LG	0.29	0.26	5.05	0.30	4					
74	UI	0	12	12	26	47	66	83	99	110	117
75	UI	118	113	105	88	74	63	54	45	38	32
76	UI	27	23	19	17	13	13	8	8	8	5
77	UI	3	3	3	3	3	3	3	3	3	0
78	UI	0	0	0	0	0	0	0	0	0	0
	*										

79	KK	CP-B3	COMBINE								
80	HC	2									
	*										

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

81	KK	R-CPB3	ROUTE								
82	RS	2	FLOW								
83	RC	0.060	0.040	0.060	300	0.0100	10.00				
84	RX	0.00	7.50	15.00	18.00	21.00	23.00	30.50	38.00		
85	RY	10.00	7.50	5.00	5.00	5.00	5.00	7.50	10.00		
	*										

86	KK	B4	BASIN								
87	BA	0.921									
88	LG	0.28	0.27	5.24	0.28	6					
89	UI	0	84	84	84	84	84	84	84	84	173
90	UI	243	243	297	329	364	405	445	470	511	545
91	UI	580	602	598	694	695	721	747	752	771	764
92	UI	810	810	810	810	810	810	803	777	766	760
93	UI	748	720	694	650	614	589	589	531	511	494
94	UI	463	445	432	399	393	367	360	329	329	299
95	UI	299	274	259	259	235	231	231	198	189	189
96	UI	188	159	159	159	159	143	130	130	130	130
97	UI	128	90	90	90	90	90	90	90	87	58
98	UI	58	58	58	58	58	58	58	58	58	58
99	UI	58	30	21	21	21	21	21	21	21	21
100	UI	21	21	21	21	21	21	21	21	21	21
101	UI	21	21	21	21	21	21	21	21	21	21
102	UI	21	21	21	21	21	0	0	0	0	0
	*										

103	KK	DVOC	DIVERT								
104	DT	DTOC	0.00	1050.0							
105	DI	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
106	DQ	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

107	KK	DETOD	STORAGE								
108	KO										
109	RS	1	STOR								
110	SV		0.26	0.59	0.99	1.48					
111	SQ		4.00	9.00	12.00	15.00					
112	SE		1.00	2.00	3.00	4.00					
	*										

113	KK	DVOCRETRIEVE									
114	DR	DTOC									
	*										

115	KK	B4C	COMBINE								
116	HC	2									
	*										

117	KK	DVOD	DIVERT								
118	DT	DTOD	0.00	975.0							
119	DI	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
120	DQ	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

1

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

121	KK	DETOD	STORAGE								
122	KO										
123	RS	1	STOR								
124	SV		0.42	0.89	1.42	2.01					

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

194 KK CP-V7 COMBINE
 195 HC 2
 *

196 KK DETBV7 STORAGE
 197 KO
 198 RS 1 STOR
 199 SV 0.22 0.47 0.77 1.10
 200 SQ 1.00 1.00 2.00 50.00
 201 SE 2606.0 2607.00 2608.00 2609.00 2610.00
 *

202 KK B9 BASIN
 203 BA 0.008
 204 LG 0.13 0.28 4.33 0.38 43
 205 UI 0 31 93 116 50 15 4 0 0 0
 206 UI 0 0 0 0 0 0 0 0 0 0
 207 UI 0 0 0 0 0 0 0 0 0 0
 208 UI 0 0 0 0 0 0 0 0 0 0
 209 UI 0 0 0 0 0 0 0 0 0 0
 *

210 KK DV-B9 DIVERT
 211 DT DT-B9 0.21 0.0
 212 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 213 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

214 KK CP-B9 COMBINE
 215 HC 3
 *

216 KK R-CPB9 ROUTE
 217 RS 1 FLOW
 218 RC 0.060 0.040 0.060 750 0.0300 2605.00
 219 RX 0.00 55.30 123.80 129.00 141.80 146.00 155.20 188.60
 220 RY 2608.0 2600.00 2598.00 2596.00 2596.00 2597.80 2598.00 2605.00
 *

221 KK B10 BASIN
 222 BA 0.012
 223 LG 0.20 0.25 4.33 0.54 2
 224 UI 0 10 39 58 90 102 69 47 23 13
 225 UI 7 3 3 0 0 0 0 0 0 0
 226 UI 0 0 0 0 0 0 0 0 0 0
 227 UI 0 0 0 0 0 0 0 0 0 0
 228 UI 0 0 0 0 0 0 0 0 0 0
 *

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HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

229 KK DV-B10 DIVERT
 230 DT DT-B10 0.32 0.0
 231 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 232 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

233 KK B12 BASIN
 234 BA 0.005
 235 LG 0.10 0.25 3.79 0.85 0
 236 UI 0 11 41 56 40 22 12 6 3 1
 237 UI 1 0 0 0 0 0 0 0 0 0
 238 UI 0 0 0 0 0 0 0 0 0 0
 239 UI 0 0 0 0 0 0 0 0 0 0
 240 UI 0 0 0 0 0 0 0 0 0 0
 *

241 KK B14 BASIN
 242 BA 0.038
 243 LG 0.17 0.26 5.58 0.27 23
 244 UI 0 23 67 119 155 222 274 198 149 108
 245 UI 58 39 24 12 7 7 7 0 0 0
 246 UI 0 0 0 0 0 0 0 0 0 0
 247 UI 0 0 0 0 0 0 0 0 0 0
 248 UI 0 0 0 0 0 0 0 0 0 0
 *

249 KK DV-B14 DIVERT
 250 DT DT-B14 1.03 0.0
 251 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 252 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

253 KK CP-B14 COMBINE
 254 HC 4
 *

255 KK C1 BASIN
 256 BA 0.023
 257 LG 0.18 0.26 5.05 0.34 14
 258 UI 0 13 33 61 79 104 155 128 99 76
 259 UI 55 29 22 13 8 4 4 4 0 0
 260 UI 0 0 0 0 0 0 0 0 0 0
 261 UI 0 0 0 0 0 0 0 0 0 0
 262 UI 0 0 0 0 0 0 0 0 0 0
 *

263 KK DV-C1 DIVERT
 264 DT DT-C1 0.61 0.0

265 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 266 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

267 KK DVC-K DIVERT
 268 DT DTC-K 0.00 1
 269 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 270 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

271 KK DVC-H DIVERT
 272 DT DTC-H 0.00 4
 273 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 274 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

275 KK B13 BASIN
 276 BA 0.003
 277 LG 0.12 0.24 7.27 0.11 51
 278 UI 0 82 33 0 0 0 0 0 0 0
 279 UI 0 0 0 0 0 0 0 0 0 0 0
 280 UI 0 0 0 0 0 0 0 0 0 0 0
 281 UI 0 0 0 0 0 0 0 0 0 0 0
 282 UI 0 0 0 0 0 0 0 0 0 0 0
 *

283 KK DV-B13 DIVERT
 284 DT DT-B13 0.08 0.0
 285 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 286 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

287 KK CPB13 COMBINE
 288 HC 2
 *

289 KK DVGF DIVERT
 290 DT DVGF 0.00 1
 291 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 292 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

293 KK R-B13 ROUTE
 294 RK 450 0.0090 0.013 CIRC 4.000
 *

295 KK CPGAL COMBINE
 296 HC 2
 *

297 KK B5 BASIN
 298 BA 0.006
 299 LG 0.23 0.27 5.05 0.38 0
 300 UI 0 15 45 80 54 25 10 2 2 0
 301 UI 0 0 0 0 0 0 0 0 0 0
 302 UI 0 0 0 0 0 0 0 0 0 0
 303 UI 0 0 0 0 0 0 0 0 0 0
 304 UI 0 0 0 0 0 0 0 0 0 0
 *

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

305 KK DV-B5 DIVERT
 306 DT DT-B5 0.55 0.0
 307 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 308 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

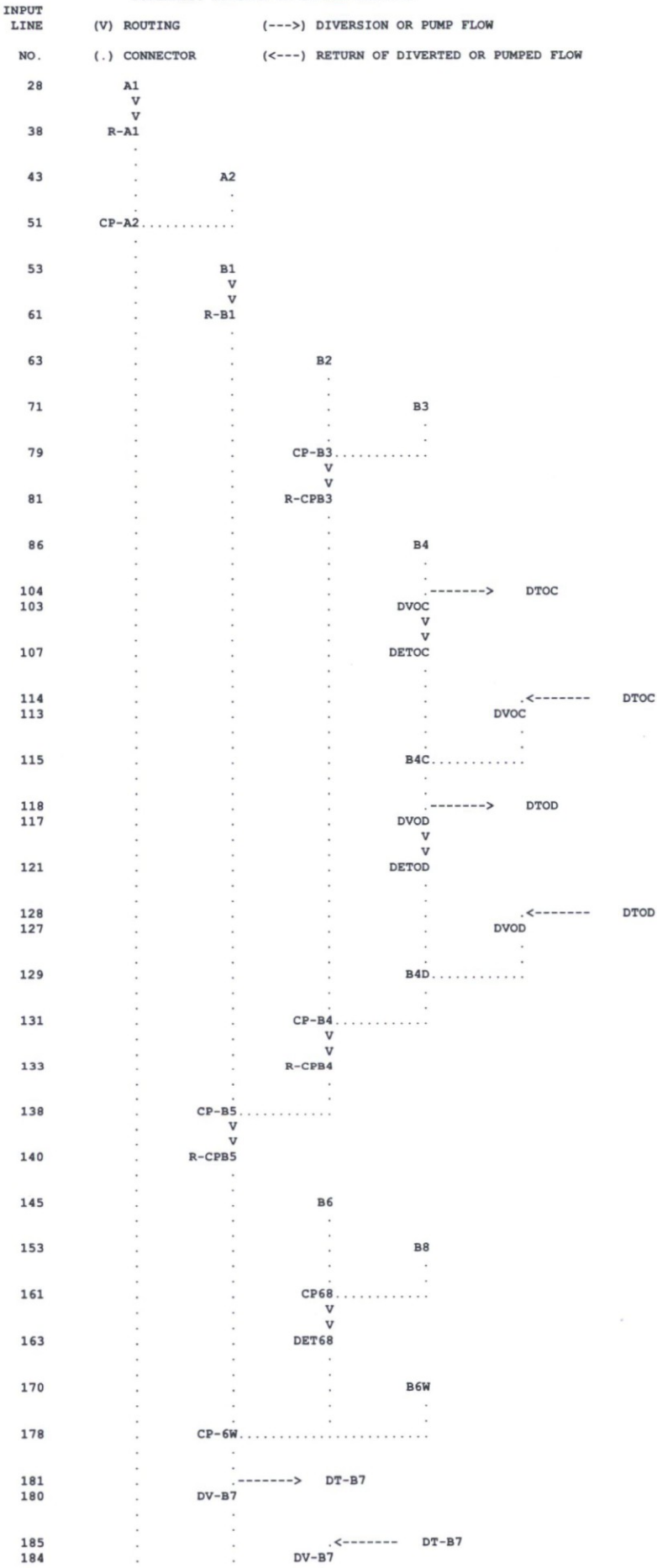
309 KK B11 BASIN
 310 BA 0.010
 311 LG 0.13 0.27 4.39 0.37 39
 312 UI 0 8 32 48 75 85 57 39 19 11
 313 UI 6 2 2 0 0 0 0 0 0 0
 314 UI 0 0 0 0 0 0 0 0 0 0
 315 UI 0 0 0 0 0 0 0 0 0 0
 316 UI 0 0 0 0 0 0 0 0 0 0
 *

317 KK DV-C11 DIVERT
 318 DT DT-C11 0.0 5.0
 319 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 320 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

321 KK DV-B11 DIVERT
 322 DT DT-B11 0.26 0.0
 323 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 324 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

325 KK L1 BASIN
 326 BA 0.003
 327 LG 0.35 0.36 5.05 0.28 0
 328 UI 0 6 23 33 25 14 7 4 2 1
 329 UI 1 0 0 0 0 0 0 0 0 0
 330 UI 0 0 0 0 0 0 0 0 0 0
 331 UI 0 0 0 0 0 0 0 0 0 0
 332 UI 0 0 0 0 0 0 0 0 0 0
 *

SCHMATIC DIAGRAM OF STREAM NETWORK



```

186 . . . . . B14A
. . . . .
194 . . . . . CP-V7
. . . . . V
196 . . . . . V
. . . . . DETBV7
. . . . .
202 . . . . . B9
. . . . .
211 . . . . . -----> DT-B9
210 . . . . . DV-B9
. . . . .
214 . . . . . CP-B9
. . . . . V
216 . . . . . V
. . . . . R-CPB9
. . . . .
221 . . . . . B10
. . . . .
230 . . . . . -----> DT-B10
229 . . . . . DV-B10
. . . . .
233 . . . . . B12
. . . . .
241 . . . . . B14
. . . . .
250 . . . . . -----> DT-B14
249 . . . . . DV-B14
. . . . .
253 . . . . . CP-B14
. . . . .
255 . . . . . C1
. . . . .
264 . . . . . -----> DT-C1
263 . . . . . DV-C1
. . . . .
268 . . . . . -----> DTC-K
267 . . . . . DVC-K
. . . . .
272 . . . . . -----> DTC-H
271 . . . . . DVC-H
. . . . .
275 . . . . . B13
. . . . .
284 . . . . . -----> DT-B13
283 . . . . . DV-B13
. . . . .
287 . . . . . CPB13
. . . . .
290 . . . . . -----> DVGF
289 . . . . . DVGF
. . . . . V
293 . . . . . V
. . . . . R-B13
. . . . .
295 . . . . . CPGAL
. . . . .
297 . . . . . B5
. . . . .
306 . . . . . -----> DT-B5
305 . . . . . DV-B5
. . . . .
309 . . . . . B11
. . . . .
318 . . . . . -----> DT-C11
317 . . . . . DV-C11
. . . . .
322 . . . . . -----> DT-B11
321 . . . . . DV-B11
. . . . .
325 . . . . . L1
. . . . .
333 . . . . .
. . . . . M1

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00

*** **

* *
107 KK * DETOC * STORAGE
* *

108 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

* *
121 KK * DETOD * STORAGE
* *

122 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

* *
163 KK * DET68 * STORAGE
* *

164 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

* *
196 KK * DETBV7 * STORAGE
* *

197 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	131.	4.15	9.	2.	2.	.20		
ROUTED TO	R-A1	128.	4.22	9.	2.	2.	.20		
HYDROGRAPH AT	A2	10.	4.03	0.	0.	0.	.01		
2 COMBINED AT	CP-A2	128.	4.22	10.	2.	2.	.21		
HYDROGRAPH AT	B1	21.	4.03	1.	0.	0.	.02		
ROUTED TO	R-B1	21.	4.05	1.	0.	0.	.02		
HYDROGRAPH AT	B2	1.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	B3	31.	4.08	2.	0.	0.	.04		
2 COMBINED AT	CP-B3	31.	4.07	2.	0.	0.	.04		
ROUTED TO	R-CPB3	31.	4.10	2.	0.	0.	.04		
HYDROGRAPH AT	B4	236.	4.43	33.	8.	6.	.92		
DIVERSION TO	DTOC	236.	4.43	33.	8.	6.	.92		
HYDROGRAPH AT	DVOC	0.	.00	0.	0.	0.	.92		
ROUTED TO	DETOC	0.	.00	0.	0.	0.	.92		
HYDROGRAPH AT	DVOC	236.	4.43	33.	8.	6.	.92		
2 COMBINED AT	B4C	236.	4.43	33.	8.	6.	.92		
DIVERSION TO	DTOD	236.	4.43	33.	8.	6.	.92		
HYDROGRAPH AT	DVOD	0.	.00	0.	0.	0.	.92		
ROUTED TO	DETOD	0.	.00	0.	0.	0.	.92		
HYDROGRAPH AT	DVOD	236.	4.43	33.	8.	6.	.92		
2 COMBINED AT	B4D	236.	4.43	33.	8.	6.	.92		
2 COMBINED AT	CP-B4	234.	4.43	34.	8.	6.	.96		
ROUTED TO	R-CPB4	233.	4.43	34.	8.	6.	.96		
2 COMBINED AT	CP-B5	231.	4.43	34.	9.	6.	.98		
ROUTED TO	R-CPB5	230.	4.47	34.	9.	6.	.98		
HYDROGRAPH AT	B6	8.	4.02	0.	0.	0.	.01		
HYDROGRAPH AT	B8	13.	4.02	1.	0.	0.	.01		
2 COMBINED AT	CP68	21.	4.02	1.	0.	0.	.02		
ROUTED TO	DET68	2.	4.03	1.	0.	0.	.02		
HYDROGRAPH AT	B6W	1.	4.03	0.	0.	0.	.00		
3 COMBINED AT	CP-6W	230.	4.47	35.	9.	6.	1.00		
DIVERSION TO	DT-B7	7.	4.47	1.	0.	0.	1.00		

+	HYDROGRAPH AT	DV-B7	223.	4.47	34.	8.	6.	1.00
+	HYDROGRAPH AT	DV-B7	7.	4.47	1.	0.	0.	1.00
+	HYDROGRAPH AT	B14A	6.	4.02	0.	0.	0.	.00
+	2 COMBINED AT	CP-V7	10.	4.45	2.	0.	0.	.00
+	ROUTED TO	DETBV7	2.	5.22	1.	0.	0.	.00
+	HYDROGRAPH AT	B9	11.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-B9	10.	3.92	0.	0.	0.	.01
+	HYDROGRAPH AT	DV-B9	11.	4.02	0.	0.	0.	.01
+	3 COMBINED AT	CP-B9	223.	4.47	34.	9.	6.	1.02
+	ROUTED TO	R-CPB9	222.	4.48	34.	9.	6.	1.02
+	HYDROGRAPH AT	B10	10.	4.03	0.	0.	0.	.01
+	DIVERSION TO	DT-B10	10.	4.03	0.	0.	0.	.01
+	HYDROGRAPH AT	DV-B10	0.	.00	0.	0.	0.	.01
+	HYDROGRAPH AT	B12	3.	4.02	0.	0.	0.	.00
+	HYDROGRAPH AT	B14	47.	4.02	3.	1.	0.	.04
+	DIVERSION TO	DT-B14	47.	4.02	2.	1.	0.	.04
+	HYDROGRAPH AT	DV-B14	37.	4.08	1.	0.	0.	.04
+	4 COMBINED AT	CP-B14	222.	4.48	34.	9.	6.	1.07
+	HYDROGRAPH AT	C1	25.	4.03	1.	0.	0.	.02
+	DIVERSION TO	DT-C1	25.	4.03	1.	0.	0.	.02
+	HYDROGRAPH AT	DV-C1	1.	4.30	0.	0.	0.	.02
+	DIVERSION TO	DTC-K	1.	4.30	0.	0.	0.	.02
+	HYDROGRAPH AT	DVC-K	0.	4.23	0.	0.	0.	.02
+	DIVERSION TO	DTC-H	0.	4.23	0.	0.	0.	.02
+	HYDROGRAPH AT	DVC-H	0.	.00	0.	0.	0.	.02
+	HYDROGRAPH AT	B13	5.	4.02	0.	0.	0.	.00
+	DIVERSION TO	DT-B13	5.	3.85	0.	0.	0.	.00
+	HYDROGRAPH AT	DV-B13	5.	4.02	0.	0.	0.	.00
+	2 COMBINED AT	CPB13	5.	4.02	0.	0.	0.	.03
+	DIVERSION TO	DVGF	1.	3.87	0.	0.	0.	.03
+	HYDROGRAPH AT	DVGF	4.	4.02	0.	0.	0.	.03
+	ROUTED TO	R-B13	4.	4.00	0.	0.	0.	.03
+	2 COMBINED AT	CPGAL	222.	4.48	34.	9.	6.	1.10
+	HYDROGRAPH AT	B5	6.	4.02	0.	0.	0.	.01
+	DIVERSION TO	DT-B5	6.	4.02	0.	0.	0.	.01

+	HYDROGRAPH AT	DV-B5	0.	.00	0.	0.	0.	.01
+	HYDROGRAPH AT	B11	13.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-C11	5.	3.83	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-C11	8.	4.02	0.	0.	0.	.01
+	DIVERSION TO	DT-B11	8.	4.02	0.	0.	0.	.01
+	HYDROGRAPH AT	DV-B11	0.	.00	0.	0.	0.	.01
+	HYDROGRAPH AT	L1	3.	4.02	0.	0.	0.	.00
+	HYDROGRAPH AT	M1	1.	4.02	0.	0.	0.	.00

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO COMPUTATION INTERVAL		VOLUME (IN)
							PEAK (CFS)	TIME TO PEAK (MIN)	
FOR STORM = 1	STORM AREA (SQ MI) =			.00					
R-B1	MANE	.31	21.32	242.49	.40	1.00	21.29	243.00	.40
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4734E+00 EXCESS= .0000E+00 OUTFLOW= .4734E+00 BASIN STORAGE= .3981E-14 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =			.50					
R-B1	MANE	.31	21.05	242.51	.40	1.00	21.01	243.00	.40
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4667E+00 EXCESS= .0000E+00 OUTFLOW= .4667E+00 BASIN STORAGE= .3958E-14 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =			1.00					
R-B1	MANE	.32	14.69	242.84	.29	1.00	14.68	243.00	.29
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3390E+00 EXCESS= .0000E+00 OUTFLOW= .3390E+00 BASIN STORAGE= .3900E-14 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =			2.80					
R-B1	MANE	.31	5.30	243.35	.13	1.00	5.26	243.00	.13
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1480E+00 EXCESS= .0000E+00 OUTFLOW= .1480E+00 BASIN STORAGE= .3907E-14 PERCENT ERROR= .0									
FOR STORM = 1	STORM AREA (SQ MI) =			.00					
R-B13	MANE	.41	3.71	240.86	.04	1.00	3.71	240.00	.04
CONTINUITY SUMMARY (AC-FT) - INFLOW= .5032E-01 EXCESS= .0000E+00 OUTFLOW= .5052E-01 BASIN STORAGE= .7473E-14 PERCENT ERROR= -.4									
FOR STORM = 2	STORM AREA (SQ MI) =			.50					
R-B13	MANE	.42	3.68	240.73	.04	1.00	3.67	240.00	.04
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4921E-01 EXCESS= .0000E+00 OUTFLOW= .4924E-01 BASIN STORAGE= .7400E-14 PERCENT ERROR= -.1									
FOR STORM = 3	STORM AREA (SQ MI) =			1.00					
R-B13	MANE	.38	2.69	240.74	.03	1.00	2.69	241.00	.03
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3451E-01 EXCESS= .0000E+00 OUTFLOW= .3471E-01 BASIN STORAGE= .7444E-14 PERCENT ERROR= -.6									
FOR STORM = 4	STORM AREA (SQ MI) =			2.80					
R-B13	MANE	.47	1.24	240.99	.01	1.00	1.24	241.00	.01
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1741E-01 EXCESS= .0000E+00 OUTFLOW= .1761E-01 BASIN STORAGE= .7703E-14 PERCENT ERROR= -1.1									

*** NORMAL END OF HEC-1 ***

10-year HEC-1 Model

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 15JUN16 TIME 20:59:46
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

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X X XXXXXXX XXXXX X
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 PROP - Desert Mountain 19 Post Online Det Basins 1st Flush
3 ID 10 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19FT10.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 2.182 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 2.169 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 2.154 1.0
21 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.075
22 PC 0.087 0.099 0.119 0.148 0.230 0.407 0.778 0.881 0.919 0.945
23 PC 0.957 0.968 0.980 0.990 1.000
24 JD 2.127 2.8
25 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
26 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
27 PC 0.950 0.963 0.975 0.988 1.000
*
28 KK A1 BASIN
29 BA 0.197
30 LG 0.29 0.25 5.34 0.26 5
31 UI 0 39 39 39 50 114 146 182 223 254
32 UI 279 323 344 355 374 378 378 369 357 338
33 UI 302 275 245 222 200 181 164 147 133 120
34 UI 108 94 88 75 74 62 61 53 42 42
35 UI 42 31 27 27 27 27 25 10 10 10
36 UI 10 10 10 10 10 10 10 10 10 10
37 UI 10 10 0 0 0 0 0 0 0 0
*
38 KK R-A1 ROUTE
39 RS 3 FLOW
40 RC 0.060 0.040 0.060 980 0.0224 19.00
41 RX 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
42 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
43 KK A2 BASIN
44 BA 0.011
45 LG 0.35 0.37 5.24 0.25 0
46 UI 0 7 22 45 62 70 63 46 33 23
47 UI 16 12 8 6 5 2 2 2 2 0
48 UI 0 0 0 0 0 0 0 0 0 0
49 UI 0 0 0 0 0 0 0 0 0 0
50 UI 0 0 0 0 0 0 0 0 0 0
*
51 KK CP-A2 COMBINE
52 HC 2
*

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53	KK	B1	BASIN								
54	BA	0.022									
55	LG	0.28	0.26	5.05	0.31	4					
56	UI	0	12	23	55	81	101	111	106	88	66
57	UI	51	39	29	23	17	12	9	8	5	3
58	UI	3	3	3	3	0	0	0	0	0	0
59	UI	0	0	0	0	0	0	0	0	0	0
60	UI	0	0	0	0	0	0	0	0	0	0
	*										

61	KK	R-B1	ROUTE								
62	RK	500	0.0200	0.013	CIRC		3.500				
	*										

63	KK	B2	BASIN								
64	BA	0.001									
65	LG	0.35	0.36	5.05	0.28	0					
66	UI	0	10	19	7	2	1	0	0	0	0
67	UI	0	0	0	0	0	0	0	0	0	0
68	UI	0	0	0	0	0	0	0	0	0	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
	*										

71	KK	B3	BASIN								
72	BA	0.038									
73	LG	0.29	0.26	5.05	0.30	4					
74	UI	0	12	12	26	47	66	83	99	110	117
75	UI	118	113	105	88	74	63	54	45	38	32
76	UI	27	23	19	17	13	13	8	8	8	5
77	UI	3	3	3	3	3	3	3	3	3	0
78	UI	0	0	0	0	0	0	0	0	0	0
	*										

79	KK	CP-B3	COMBINE								
80	HC	2									
	*										

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HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

81	KK	R-CPB3	ROUTE								
82	RS	1	FLOW								
83	RC	0.060	0.040	0.060	300	0.0100	10.00				
84	RX	0.00	7.50	15.00	18.00	21.00	23.00	30.50	38.00		
85	RY	10.00	7.50	5.00	5.00	5.00	5.00	7.50	10.00		
	*										

86	KK	B4	BASIN								
87	BA	0.921									
88	LG	0.28	0.27	5.24	0.28	6					
89	UI	0	84	84	84	84	84	84	84	84	173
90	UI	243	243	297	329	364	405	445	470	511	545
91	UI	580	602	598	694	695	721	747	752	771	764
92	UI	810	810	810	810	810	810	803	777	766	760
93	UI	748	720	694	650	614	589	589	531	511	494
94	UI	463	445	432	399	393	367	360	329	329	299
95	UI	299	274	259	259	235	231	231	198	189	189
96	UI	188	159	159	159	159	143	130	130	130	130
97	UI	128	90	90	90	90	90	90	90	87	58
98	UI	58	58	58	58	58	58	58	58	58	58
99	UI	58	30	21	21	21	21	21	21	21	21
100	UI	21	21	21	21	21	21	21	21	21	21
101	UI	21	21	21	21	21	21	21	21	21	21
102	UI	21	21	21	21	21	0	0	0	0	0
	*										

103	KK	DVOC	DIVERT								
104	DT	DTOC	0.00	1050.0							
105	DI	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
106	DQ	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

107	KK	DETOC	STORAGE								
108	KO										
109	RS	1	STOR								
110	SV	0.26	0.59	0.99	1.48						
111	SQ	4.00	9.00	12.00	15.00						
112	SE	1.00	2.00	3.00	4.00						
	*										

113	KK	DVOCRETRIEVE									
114	DR	DTOC									
	*										

115	KK	B4C	COMBINE								
116	HC	2									
	*										

117	KK	DVOD	DIVERT								
118	DT	DTOD	0.00	975.0							
119	DI	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
120	DQ	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

1

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

121	KK	DETOC	STORAGE								
122	KO										
123	RS	1	STOR								
124	SV	0.42	0.89	1.42	2.01						

125	SQ		4.00	9.00	12.00	15.00						
126	SE		1.00	2.00	3.00	4.00						
	*											
127	KK	DVODRETRIEVE										
128	DR	DTOD										
	*											
129	KK	B4D COMBINE										
130	HC	2										
	*											
131	KK	CP-B4 COMBINE										
132	HC	2										
	*											
133	KK	R-CPB4 ROUTE										
134	RS	1 FLOW										
135	RC	0.060 0.040	0.060	300	0.0300	2631.00						
136	RX	0.00 18.90	43.80	54.80	81.50	91.60	120.00	160.00				
137	RY	2635.0 2630.00	2629.00	2625.00	2625.00	2629.00	2630.00	2631.00				
	*											
138	KK	CP-B5 COMBINE										
139	HC	2										
	*											
140	KK	R-CPB5 ROUTE										
141	RS	1 FLOW										
142	RC	0.060 0.040	0.060	650	0.0300	2631.00						
143	RX	0.00 18.90	43.80	54.80	81.50	91.60	120.00	160.00				
144	RY	2635.0 2630.00	2629.00	2625.00	2625.00	2629.00	2630.00	2631.00				
	*											
145	KK	B6 BASIN										
146	BA	0.009										
147	LG	0.19 0.25	4.45	0.52	3							
148	UI	0 26	78	129	74	27	9	4	0	0		
149	UI	0 0	0	0	0	0	0	0	0	0	0	0
150	UI	0 0	0	0	0	0	0	0	0	0	0	0
151	UI	0 0	0	0	0	0	0	0	0	0	0	0
152	UI	0 0	0	0	0	0	0	0	0	0	0	0
	*											
153	KK	B8 BASIN										
154	BA	0.010										
155	LG	0.13 0.27	4.87	0.30	38							
156	UI	0 7	25	39	55	83	63	46	31	15		
157	UI	10 6	2	2	2	0	0	0	0	0	0	0
158	UI	0 0	0	0	0	0	0	0	0	0	0	0
	*											
			HEC-1 INPUT									
LINE	ID1.....2.....3.....4.....5.....6.....7.....8.....9.....10										
159	UI	0 0 0 0 0 0 0 0 0 0 0										
160	UI	0 0 0 0 0 0 0 0 0 0 0										
	*											
161	KK	CP68 COMBINE										
162	HC	2										
	*											
163	KK	DET68 STORAGE										
164	KO											
165	RS	1 STOR										
166	SV	0.09	0.21	0.37	0.58							
167	SQ	1.00	1.00	2.00	2.00							
168	SE	2608.0 2609.00	2610.00	2611.00	2612.00							
169	SS	2612.0 20.00	2.80	1.50								
	*											
170	KK	B6W BASIN										
171	BA	0.003										
172	LG	0.13 0.28	3.37	1.04	6							
173	UI	0 5	16	29	32	19	9	4	1	1		
174	UI	0 0	0	0	0	0	0	0	0	0	0	0
175	UI	0 0	0	0	0	0	0	0	0	0	0	0
176	UI	0 0	0	0	0	0	0	0	0	0	0	0
177	UI	0 0	0	0	0	0	0	0	0	0	0	0
	*											
178	KK	CP-6W COMBINE										
179	HC	3										
	*											
180	KK	DV-B7 DIVERT										
181	DT	DT-B7 0.00	0.0									
182	DI	0.0 150.0	300.0	450.0	600.0	750.0	900.0	1050.0	1200.0	0.0		
183	DQ	0.0 4.0	9.0	12.0	15.0	18.0	20.0	22.0	24.0	0.0		
	*											
184	KK	DV-B7RETRIEVE										
185	DR	DT-B7										
	*											
186	KK	B14A BASIN										
187	BA	0.004										
188	LG	0.13 0.26	6.16	0.16	39							
189	UI	0 62	83	9	0	0	0	0	0	0	0	0
190	UI	0 0	0	0	0	0	0	0	0	0	0	0
191	UI	0 0	0	0	0	0	0	0	0	0	0	0
192	UI	0 0	0	0	0	0	0	0	0	0	0	0
193	UI	0 0	0	0	0	0	0	0	0	0	0	0
	*											

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

194 KK CP-V7 COMBINE
 195 HC 2
 *

196 KK DETBV7 STORAGE
 197 KO
 198 RS 1 STOR
 199 SV 0.22 0.47 0.77 1.10
 200 SQ 1.00 1.00 2.00 50.00
 201 SE 2606.0 2607.00 2608.00 2609.00 2610.00
 *

202 KK B9 BASIN
 203 BA 0.008
 204 LG 0.13 0.28 4.33 0.38 43
 205 UI 0 31 93 116 50 15 4 0 0 0
 206 UI 0 0 0 0 0 0 0 0 0 0
 207 UI 0 0 0 0 0 0 0 0 0 0
 208 UI 0 0 0 0 0 0 0 0 0 0
 209 UI 0 0 0 0 0 0 0 0 0 0
 *

210 KK DV-B9 DIVERT
 211 DT DT-B9 0.21 0.0
 212 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 213 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

214 KK CP-B9 COMBINE
 215 HC 3
 *

216 KK R-CPB9 ROUTE
 217 RS 2 FLOW
 218 RC 0.060 0.040 0.060 750 0.0300 2605.00
 219 RX 0.00 55.30 123.80 129.00 141.80 146.00 155.20 188.60
 220 RY 2608.0 2600.00 2598.00 2596.00 2596.00 2597.80 2598.00 2605.00
 *

221 KK B10 BASIN
 222 BA 0.012
 223 LG 0.20 0.25 4.33 0.54 2
 224 UI 0 10 39 58 90 102 69 47 23 13
 225 UI 7 3 3 0 0 0 0 0 0 0
 226 UI 0 0 0 0 0 0 0 0 0 0
 227 UI 0 0 0 0 0 0 0 0 0 0
 228 UI 0 0 0 0 0 0 0 0 0 0
 *

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1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

229 KK DV-B10 DIVERT
 230 DT DT-B10 0.32 0.0
 231 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 232 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

233 KK B12 BASIN
 234 BA 0.005
 235 LG 0.10 0.25 3.79 0.85 0
 236 UI 0 11 41 56 40 22 12 6 3 1
 237 UI 1 0 0 0 0 0 0 0 0 0
 238 UI 0 0 0 0 0 0 0 0 0 0
 239 UI 0 0 0 0 0 0 0 0 0 0
 240 UI 0 0 0 0 0 0 0 0 0 0
 *

241 KK B14 BASIN
 242 BA 0.038
 243 LG 0.17 0.26 5.58 0.27 23
 244 UI 0 23 67 119 155 222 274 198 149 108
 245 UI 58 39 24 12 7 7 7 0 0 0
 246 UI 0 0 0 0 0 0 0 0 0 0
 247 UI 0 0 0 0 0 0 0 0 0 0
 248 UI 0 0 0 0 0 0 0 0 0 0
 *

249 KK DV-B14 DIVERT
 250 DT DT-B14 1.03 0.0
 251 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 252 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

253 KK CP-B14 COMBINE
 254 HC 4
 *

255 KK C1 BASIN
 256 BA 0.023
 257 LG 0.18 0.26 5.05 0.34 14
 258 UI 0 13 33 61 79 104 155 128 99 76
 259 UI 55 29 22 13 8 4 4 4 0 0
 260 UI 0 0 0 0 0 0 0 0 0 0
 261 UI 0 0 0 0 0 0 0 0 0 0
 262 UI 0 0 0 0 0 0 0 0 0 0
 *

263 KK DV-C1 DIVERT
 264 DT DT-C1 0.61 0.0

265 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 266 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

267 KK DVC-K DIVERT
 268 DT DTC-K 0.00 18
 269 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 270 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

271 KK DVC-H DIVERT
 272 DT DTC-H 0.00 8
 273 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 274 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

275 KK B13 BASIN
 276 BA 0.003
 277 LG 0.12 0.24 7.27 0.11 51
 278 UI 0 82 33 0 0 0 0 0 0 0
 279 UI 0 0 0 0 0 0 0 0 0 0
 280 UI 0 0 0 0 0 0 0 0 0 0
 281 UI 0 0 0 0 0 0 0 0 0 0
 282 UI 0 0 0 0 0 0 0 0 0 0

*

283 KK DV-B13 DIVERT
 284 DT DT-B13 0.08 0.0
 285 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 286 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

287 KK CPB13 COMBINE
 288 HC 2

*

289 KK DVGF DIVERT
 290 DT DTGF 0.00 1
 291 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 292 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

293 KK R-B13 ROUTE
 294 RK 450 0.0090 0.013 CIRC 4.000

*

295 KK CPGAL COMBINE
 296 HC 2

*

297 KK B5 BASIN
 298 BA 0.006
 299 LG 0.23 0.27 5.05 0.38 0
 300 UI 0 15 45 80 54 25 10 2 2 0
 301 UI 0 0 0 0 0 0 0 0 0 0
 302 UI 0 0 0 0 0 0 0 0 0 0
 303 UI 0 0 0 0 0 0 0 0 0 0
 304 UI 0 0 0 0 0 0 0 0 0 0

*

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

305 KK DV-B5 DIVERT
 306 DT DT-B5 0.42 0.0
 307 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 308 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

309 KK B11 BASIN
 310 BA 0.010
 311 LG 0.13 0.27 4.39 0.37 39
 312 UI 0 8 32 48 75 85 57 39 19 11
 313 UI 6 2 2 0 0 0 0 0 0 0
 314 UI 0 0 0 0 0 0 0 0 0 0
 315 UI 0 0 0 0 0 0 0 0 0 0
 316 UI 0 0 0 0 0 0 0 0 0 0

*

317 KK DV-C11 DIVERT
 318 DT DT-C11 0.0 5.0
 319 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 320 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

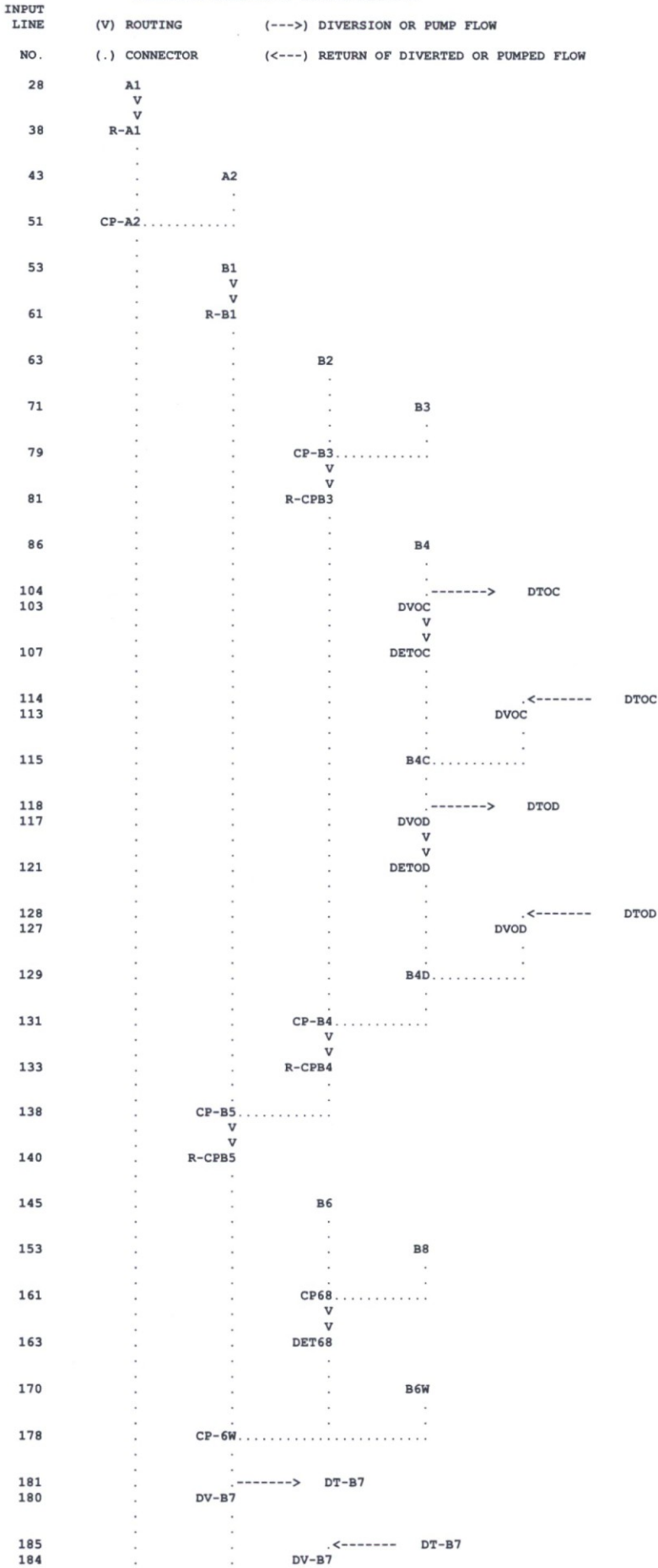
*

321 KK DV-B11 DIVERT
 322 DT DV-B11 0.26 0.0
 323 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 324 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

325 KK L1 BASIN
 326 BA 0.003
 327 LG 0.35 0.36 5.05 0.28 0
 328 UI 0 6 23 33 25 14 7 4 2 1
 329 UI 1 0 0 0 0 0 0 0 0 0
 330 UI 0 0 0 0 0 0 0 0 0 0
 331 UI 0 0 0 0 0 0 0 0 0 0
 332 UI 0 0 0 0 0 0 0 0 0 0

SCHEMATIC DIAGRAM OF STREAM NETWORK



```

186 . . . . . B14A
194 . . . . . CP-V7 . . . . .
      V
196 . . . . . DETBV7
      V
202 . . . . . B9
      .
211 . . . . . -----> DT-B9
210 . . . . . DV-B9
      .
214 . . . . . CP-B9 . . . . .
      V
216 . . . . . R-CPB9
      V
221 . . . . . B10
      .
230 . . . . . -----> DT-B10
229 . . . . . DV-B10
      .
233 . . . . . B12
      .
241 . . . . . B14
      .
250 . . . . . -----> DT-B14
249 . . . . . DV-B14
      .
253 . . . . . CP-B14 . . . . .
      .
255 . . . . . C1
      .
264 . . . . . -----> DT-C1
263 . . . . . DV-C1
      .
268 . . . . . -----> DTC-K
267 . . . . . DVC-K
      .
272 . . . . . -----> DTC-H
271 . . . . . DVC-H
      .
275 . . . . . B13
      .
284 . . . . . -----> DT-B13
283 . . . . . DV-B13
      .
287 . . . . . CPB13 . . . . .
      .
290 . . . . . -----> DTGF
289 . . . . . DVGF
      V
293 . . . . . R-B13
      V
295 . . . . . CPGAL . . . . .
      .
297 . . . . . B5
      .
306 . . . . . -----> DT-B5
305 . . . . . DV-B5
      .
309 . . . . . B11
      .
318 . . . . . -----> DT-C11
317 . . . . . DV-C11
      .
322 . . . . . -----> DV-B11
321 . . . . . DV-B11
      .
325 . . . . . L1
      .
333 . . . . .

```

M1

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00

*** **

* *
107 KK * DETOC * STORAGE
* *

108 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

* *
121 KK * DETOD * STORAGE
* *

122 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

* *
163 KK * DET68 * STORAGE
* *

164 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

* *
196 KK * DETBV7 * STORAGE
* *

197 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	262.	4.13	19.	5.	3.	.20		
ROUTED TO	R-A1	258.	4.18	19.	5.	3.	.20		
HYDROGRAPH AT	A2	21.	4.02	1.	0.	0.	.01		
2 COMBINED AT	CP-A2	260.	4.18	20.	5.	4.	.21		
HYDROGRAPH AT	B1	42.	4.02	2.	0.	0.	.02		
ROUTED TO	R-B1	42.	4.03	2.	0.	0.	.02		
HYDROGRAPH AT	B2	2.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	B3	63.	4.07	3.	1.	1.	.04		
2 COMBINED AT	CP-B3	64.	4.07	3.	1.	1.	.04		
ROUTED TO	R-CPB3	63.	4.08	3.	1.	1.	.04		
HYDROGRAPH AT	B4	546.	4.43	76.	19.	14.	.92		
DIVERSION TO	DTOC	546.	4.43	76.	19.	14.	.92		
HYDROGRAPH AT	DVOC	0.	.00	0.	0.	0.	.92		
ROUTED TO	DETOC	0.	.00	0.	0.	0.	.92		
HYDROGRAPH AT	DVOC	546.	4.43	76.	19.	14.	.92		
2 COMBINED AT	B4C	546.	4.43	76.	19.	14.	.92		
DIVERSION TO	DTOD	546.	4.43	76.	19.	14.	.92		
HYDROGRAPH AT	DVOD	0.	.00	0.	0.	0.	.92		
ROUTED TO	DETOD	0.	.00	0.	0.	0.	.92		
HYDROGRAPH AT	DVOD	546.	4.43	76.	19.	14.	.92		
2 COMBINED AT	B4D	546.	4.43	76.	19.	14.	.92		
2 COMBINED AT	CP-B4	546.	4.42	79.	20.	14.	.96		
ROUTED TO	R-CPB4	546.	4.43	79.	20.	14.	.96		
2 COMBINED AT	CP-B5	543.	4.43	80.	20.	14.	.98		
ROUTED TO	R-CPB5	543.	4.43	80.	20.	14.	.98		
HYDROGRAPH AT	B6	16.	4.02	1.	0.	0.	.01		
HYDROGRAPH AT	B8	22.	4.02	1.	0.	0.	.01		
2 COMBINED AT	CP68	39.	4.02	2.	1.	0.	.02		
ROUTED TO	DET68	2.	3.92	2.	1.	0.	.02		
HYDROGRAPH AT	B6W	4.	4.02	0.	0.	0.	.00		
3 COMBINED AT	CP-6W	543.	4.43	81.	20.	15.	1.00		
DIVERSION TO	DT-B7	14.	4.43	2.	1.	0.	1.00		

+	HYDROGRAPH AT	DV-B7	527.	4.43	79.	20.	14.	1.00
+	HYDROGRAPH AT	DV-B7	14.	4.43	2.	1.	0.	1.00
+	HYDROGRAPH AT	B14A	9.	4.02	1.	0.	0.	.00
+	2 COMBINED AT	CP-V7	16.	4.02	3.	1.	1.	.00
+	ROUTED TO	DETBV7	13.	4.65	2.	1.	1.	.00
+	HYDROGRAPH AT	B9	18.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-B9	16.	3.83	0.	0.	0.	.01
+	HYDROGRAPH AT	DV-B9	18.	4.02	1.	0.	0.	.01
+	3 COMBINED AT	CP-B9	528.	4.43	81.	21.	15.	1.02
+	ROUTED TO	R-CPB9	527.	4.47	81.	21.	15.	1.02
+	HYDROGRAPH AT	B10	21.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-B10	21.	4.00	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-B10	21.	4.03	0.	0.	0.	.01
+	HYDROGRAPH AT	B12	8.	4.02	0.	0.	0.	.00
+	HYDROGRAPH AT	B14	82.	4.02	5.	1.	1.	.04
+	DIVERSION TO	DT-B14	77.	3.93	2.	1.	0.	.04
+	HYDROGRAPH AT	DV-B14	82.	4.02	3.	1.	0.	.04
+	4 COMBINED AT	CP-B14	527.	4.47	83.	21.	15.	1.07
+	HYDROGRAPH AT	C1	47.	4.02	2.	1.	0.	.02
+	DIVERSION TO	DT-C1	45.	3.97	1.	0.	0.	.02
+	HYDROGRAPH AT	DV-C1	47.	4.02	1.	0.	0.	.02
+	DIVERSION TO	DTC-K	18.	4.00	1.	0.	0.	.02
+	HYDROGRAPH AT	DVC-K	29.	4.02	0.	0.	0.	.02
+	DIVERSION TO	DTC-H	8.	4.00	0.	0.	0.	.02
+	HYDROGRAPH AT	DVC-H	21.	4.02	0.	0.	0.	.02
+	HYDROGRAPH AT	B13	7.	4.02	1.	0.	0.	.00
+	DIVERSION TO	DT-B13	2.	3.75	0.	0.	0.	.00
+	HYDROGRAPH AT	DV-B13	7.	4.02	0.	0.	0.	.00
+	2 COMBINED AT	CPB13	28.	4.02	1.	0.	0.	.03
+	DIVERSION TO	DTGF	1.	3.78	0.	0.	0.	.03
+	HYDROGRAPH AT	DVGF	27.	4.02	1.	0.	0.	.03
+	ROUTED TO	R-B13	27.	4.02	1.	0.	0.	.03
+	2 COMBINED AT	CPGAL	527.	4.47	83.	21.	15.	1.10
+	HYDROGRAPH AT	B5	11.	4.02	0.	0.	0.	.01
+	DIVERSION TO	DT-B5	11.	4.02	0.	0.	0.	.01

+	HYDROGRAPH AT	DV-B5	0.	.00	0.	0.	0.	.01
+	HYDROGRAPH AT	B11	22.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-C11	5.	3.80	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-C11	17.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DV-B11	17.	4.02	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-B11	15.	4.05	0.	0.	0.	.01
+	HYDROGRAPH AT	L1	6.	4.02	0.	0.	0.	.00
+	HYDROGRAPH AT	M1	2.	4.02	0.	0.	0.	.00

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

I STA Q	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO COMPUTATION INTERVAL		VOLUME (IN)
							PEAK (CFS)	TIME TO PEAK (MIN)	
FOR STORM = 1	STORM AREA (SQ MI) =			.00					
R-B1	MANE	.17	42.15	241.37	.84	1.00	42.13	242.00	.84
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9822E+00 EXCESS= .0000E+00 OUTFLOW= .9825E+00 BASIN STORAGE= .3951E-14 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =			.50					
R-B1	MANE	.18	41.78	241.51	.83	1.00	41.77	242.00	.83
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9720E+00 EXCESS= .0000E+00 OUTFLOW= .9723E+00 BASIN STORAGE= .3979E-14 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =			1.00					
R-B1	MANE	.17	31.78	241.47	.70	1.00	31.77	242.00	.70
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8268E+00 EXCESS= .0000E+00 OUTFLOW= .8269E+00 BASIN STORAGE= .3940E-14 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =			2.80					
R-B1	MANE	.27	16.95	242.35	.57	1.00	16.94	242.00	.57
CONTINUITY SUMMARY (AC-FT) - INFLOW= .6687E+00 EXCESS= .0000E+00 OUTFLOW= .6687E+00 BASIN STORAGE= .3984E-14 PERCENT ERROR= .0									
FOR STORM = 1	STORM AREA (SQ MI) =			.00					
R-B13	MANE	.30	26.83	241.14	.21	1.00	26.82	241.00	.21
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2869E+00 EXCESS= .0000E+00 OUTFLOW= .2870E+00 BASIN STORAGE= .7598E-14 PERCENT ERROR= -.1									
FOR STORM = 2	STORM AREA (SQ MI) =			.50					
R-B13	MANE	.37	26.43	241.00	.20	1.00	26.43	241.00	.20
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2821E+00 EXCESS= .0000E+00 OUTFLOW= .2826E+00 BASIN STORAGE= .7502E-14 PERCENT ERROR= -.2									
FOR STORM = 3	STORM AREA (SQ MI) =			1.00					
R-B13	MANE	.34	13.15	241.76	.11	1.00	12.37	242.00	.11
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1582E+00 EXCESS= .0000E+00 OUTFLOW= .1584E+00 BASIN STORAGE= .7731E-14 PERCENT ERROR= -.2									
FOR STORM = 4	STORM AREA (SQ MI) =			2.80					
R-B13	MANE	.44	2.57	240.86	.06	1.00	2.57	240.00	.06
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8541E-01 EXCESS= .0000E+00 OUTFLOW= .8564E-01 BASIN STORAGE= .7698E-14 PERCENT ERROR= -.3									

*** NORMAL END OF HEC-1 ***

100-year HEC-1 Model

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   JUN 1998 *
*   VERSION 4.1 *
* RUN DATE 15JUN16 TIME 20:59:52 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION , DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 PROP - Desert Mountain 19 Post Online Det Basins 1st Flush
3 ID 100 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19FT100.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 3.313 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 3.293 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 3.270 1.0
21 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.075
22 PC 0.087 0.099 0.119 0.148 0.230 0.407 0.778 0.881 0.919 0.945
23 PC 0.957 0.968 0.980 0.990 1.000
24 JD 3.230 2.8
25 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
26 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
27 PC 0.950 0.963 0.975 0.988 1.000
*
28 KK A1 BASIN
29 BA 0.197
30 LG 0.29 0.25 5.34 0.26 5
31 UI 0 39 39 39 50 114 146 182 223 254
32 UI 279 323 344 355 374 378 378 369 357 338
33 UI 302 275 245 222 200 181 164 147 133 120
34 UI 108 94 88 75 74 62 61 53 42 42
35 UI 42 31 27 27 27 27 25 10 10 10
36 UI 10 10 10 10 10 10 10 10 10 10
37 UI 10 10 0 0 0 0 0 0 0 0
*
38 KK R-A1 ROUTE
39 RS 3 FLOW
40 RC 0.060 0.040 0.060 980 0.0224 19.00
41 RX 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
42 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*

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1

HEC-1 INPUT

PAGE 2

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
43 KK A2 BASIN
44 BA 0.011
45 LG 0.35 0.37 5.24 0.25 0
46 UI 0 7 22 45 62 70 63 46 33 23
47 UI 16 12 8 6 5 2 2 2 2 0
48 UI 0 0 0 0 0 0 0 0 0 0
49 UI 0 0 0 0 0 0 0 0 0 0
50 UI 0 0 0 0 0 0 0 0 0 0
*
51 KK CP-A2 COMBINE
52 HC 2
*

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53	KK	B1	BASIN								
54	BA	0.022									
55	LG	0.28	0.26	5.05	0.31	4					
56	UI	0	12	23	55	81	101	111	106	88	66
57	UI	51	39	29	23	17	12	9	8	5	3
58	UI	3	3	3	3	0	0	0	0	0	0
59	UI	0	0	0	0	0	0	0	0	0	0
60	UI	0	0	0	0	0	0	0	0	0	0
	*										

61	KK	R-B1	ROUTE								
62	RK	500	0.0200	0.013	CIRC			3.500			
	*										

63	KK	B2	BASIN								
64	BA	0.001									
65	LG	0.35	0.36	5.05	0.28	0					
66	UI	0	10	19	7	2	1	0	0	0	0
67	UI	0	0	0	0	0	0	0	0	0	0
68	UI	0	0	0	0	0	0	0	0	0	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
	*										

71	KK	B3	BASIN								
72	BA	0.038									
73	LG	0.29	0.26	5.05	0.30	4					
74	UI	0	12	12	26	47	66	83	99	110	117
75	UI	118	113	105	88	74	63	54	45	38	32
76	UI	27	23	19	17	13	13	8	8	8	5
77	UI	3	3	3	3	3	3	3	3	3	0
78	UI	0	0	0	0	0	0	0	0	0	0
	*										

79	KK	CP-B3	COMBINE								
80	HC	2									
	*										

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

81	KK	R-CPB3	ROUTE								
82	RS	1	FLOW								
83	RC	0.060	0.040	0.060	300	0.0100	10.00				
84	RX	0.00	7.50	15.00	18.00	21.00	23.00	30.50	38.00		
85	RY	10.00	7.50	5.00	5.00	5.00	5.00	7.50	10.00		
	*										

86	KK	B4	BASIN								
87	BA	0.921									
88	LG	0.28	0.27	5.24	0.28	6					
89	UI	0	84	84	84	84	84	84	84	84	173
90	UI	243	243	297	329	364	405	445	470	511	545
91	UI	580	602	598	694	695	721	747	752	771	764
92	UI	810	810	810	810	810	810	803	777	766	760
93	UI	748	720	694	650	614	589	589	531	511	494
94	UI	463	445	432	399	393	367	360	329	329	299
95	UI	299	274	259	259	235	231	231	198	189	189
96	UI	188	159	159	159	159	143	130	130	130	130
97	UI	128	90	90	90	90	90	90	90	87	58
98	UI	58	58	58	58	58	58	58	58	58	58
99	UI	58	30	21	21	21	21	21	21	21	21
100	UI	21	21	21	21	21	21	21	21	21	21
101	UI	21	21	21	21	21	21	21	21	21	21
102	UI	21	21	21	21	21	0	0	0	0	0
	*										

103	KK	DVOC	DIVERT								
104	DT	DTOC	0.00	1050.0							
105	DI	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
106	DQ	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

107	KK	DETOD STORAGE									
108	KO										
109	RS	1	STOR								
110	SV	0.26	0.59	0.99	1.48						
111	SQ	4.00	9.00	12.00	15.00						
112	SE	1.00	2.00	3.00	4.00						
	*										

113	KK	DVOCRETRIEVE									
114	DR	DTOC									
	*										

115	KK	B4C	COMBINE								
116	HC	2									
	*										

117	KK	DVOD	DIVERT								
118	DT	DTOD	0.00	975.0							
119	DI	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
120	DQ	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

121	KK	DETOD STORAGE									
122	KO										
123	RS	1	STOR								
124	SV	0.42	0.89	1.42	2.01						

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

194 KK CP-V7 COMBINE
 195 HC 2
 *

196 KK DETBV7 STORAGE
 197 KO
 198 RS 1 STOR
 199 SV 0.22 0.47 0.77 1.10
 200 SQ 1.00 1.00 2.00 50.00
 201 SE 2606.0 2607.00 2608.00 2609.00 2610.00
 *

202 KK B9 BASIN
 203 BA 0.008
 204 LG 0.13 0.28 4.33 0.38 43
 205 UI 0 31 93 116 50 15 4 0 0 0
 206 UI 0 0 0 0 0 0 0 0 0 0
 207 UI 0 0 0 0 0 0 0 0 0 0
 208 UI 0 0 0 0 0 0 0 0 0 0
 209 UI 0 0 0 0 0 0 0 0 0 0
 *

210 KK DV-B9 DIVERT
 211 DT DT-B9 0.21 0.0
 212 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 213 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

214 KK CP-B9 COMBINE
 215 HC 3
 *

216 KK R-CPB9 ROUTE
 217 RS 1 FLOW
 218 RC 0.060 0.040 0.060 750 0.0300 2605.00
 219 RX 0.00 55.30 123.80 129.00 141.80 146.00 155.20 188.60
 220 RY 2608.0 2600.00 2598.00 2596.00 2596.00 2597.80 2598.00 2605.00
 *

221 KK B10 BASIN
 222 BA 0.012
 223 LG 0.20 0.25 4.33 0.54 2
 224 UI 0 10 39 58 90 102 69 47 23 13
 225 UI 7 3 3 0 0 0 0 0 0 0
 226 UI 0 0 0 0 0 0 0 0 0 0
 227 UI 0 0 0 0 0 0 0 0 0 0
 228 UI 0 0 0 0 0 0 0 0 0 0
 *

HEC-1 INPUT

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1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

229 KK DT-B10 DIVERT
 230 DT DT-B10 0.32 0.0
 231 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 232 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

233 KK B12 BASIN
 234 BA 0.005
 235 LG 0.10 0.25 3.79 0.85 0
 236 UI 0 11 41 56 40 22 12 6 3 1
 237 UI 1 0 0 0 0 0 0 0 0 0
 238 UI 0 0 0 0 0 0 0 0 0 0
 239 UI 0 0 0 0 0 0 0 0 0 0
 240 UI 0 0 0 0 0 0 0 0 0 0
 *

241 KK B14 BASIN
 242 BA 0.038
 243 LG 0.17 0.26 5.58 0.27 23
 244 UI 0 23 67 119 155 222 274 198 149 108
 245 UI 58 39 24 12 7 7 7 0 0 0
 246 UI 0 0 0 0 0 0 0 0 0 0
 247 UI 0 0 0 0 0 0 0 0 0 0
 248 UI 0 0 0 0 0 0 0 0 0 0
 *

249 KK DV-B14 DIVERT
 250 DT DT-B14 1.03 0.0
 251 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 252 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 *

253 KK CP-B14 COMBINE
 254 HC 4
 *

255 KK C1 BASIN
 256 BA 0.023
 257 LG 0.18 0.26 5.05 0.34 14
 258 UI 0 13 33 61 79 104 155 128 99 76
 259 UI 55 29 22 13 8 4 4 4 0 0
 260 UI 0 0 0 0 0 0 0 0 0 0
 261 UI 0 0 0 0 0 0 0 0 0 0
 262 UI 0 0 0 0 0 0 0 0 0 0
 *

263 KK DV-C1 DIVERT
 264 DT DT-C1 0.61 0.0

265 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 266 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

267 KK DVC-K DIVERT
 268 DT DTC-K 0.00 50
 269 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 270 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

271 KK DVC-H DIVERT
 272 DT DTC-H 0.00 15
 273 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 274 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

275 KK B13 BASIN
 276 BA 0.003
 277 LG 0.12 0.24 7.27 0.11 51
 278 UI 0 82 33 0 0 0 0 0 0 0
 279 UI 0 0 0 0 0 0 0 0 0 0
 280 UI 0 0 0 0 0 0 0 0 0 0
 281 UI 0 0 0 0 0 0 0 0 0 0
 282 UI 0 0 0 0 0 0 0 0 0 0

*

283 KK DV-B13 DIVERT
 284 DT DT-B13 0.08 0.0
 285 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 286 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

287 KK CPB13 COMBINE
 288 HC 2

*

289 KK DVGF DIVERT
 290 DT DTGF 0.00 3
 291 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 292 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

293 KK R-B13 ROUTE
 294 RK 450 0.0090 0.013 CIRC 4.000

*

295 KK CPGAL COMBINE
 296 HC 2

*

297 KK B5 BASIN
 298 BA 0.006
 299 LG 0.23 0.27 5.05 0.38 0
 300 UI 0 15 45 80 54 25 10 2 2 0
 301 UI 0 0 0 0 0 0 0 0 0 0
 302 UI 0 0 0 0 0 0 0 0 0 0
 303 UI 0 0 0 0 0 0 0 0 0 0
 304 UI 0 0 0 0 0 0 0 0 0 0

*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

305 KK DV-B5 DIVERT
 306 DT DT-B5 0.42 0.0
 307 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 308 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

309 KK B11 BASIN
 310 BA 0.010
 311 LG 0.13 0.27 4.39 0.37 39
 312 UI 0 8 32 48 75 85 57 39 19 11
 313 UI 6 2 2 0 0 0 0 0 0 0
 314 UI 0 0 0 0 0 0 0 0 0 0
 315 UI 0 0 0 0 0 0 0 0 0 0
 316 UI 0 0 0 0 0 0 0 0 0 0

*

317 KK DV-C11 DIVERT
 318 DT DT-C11 0.0 5.0
 319 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 320 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

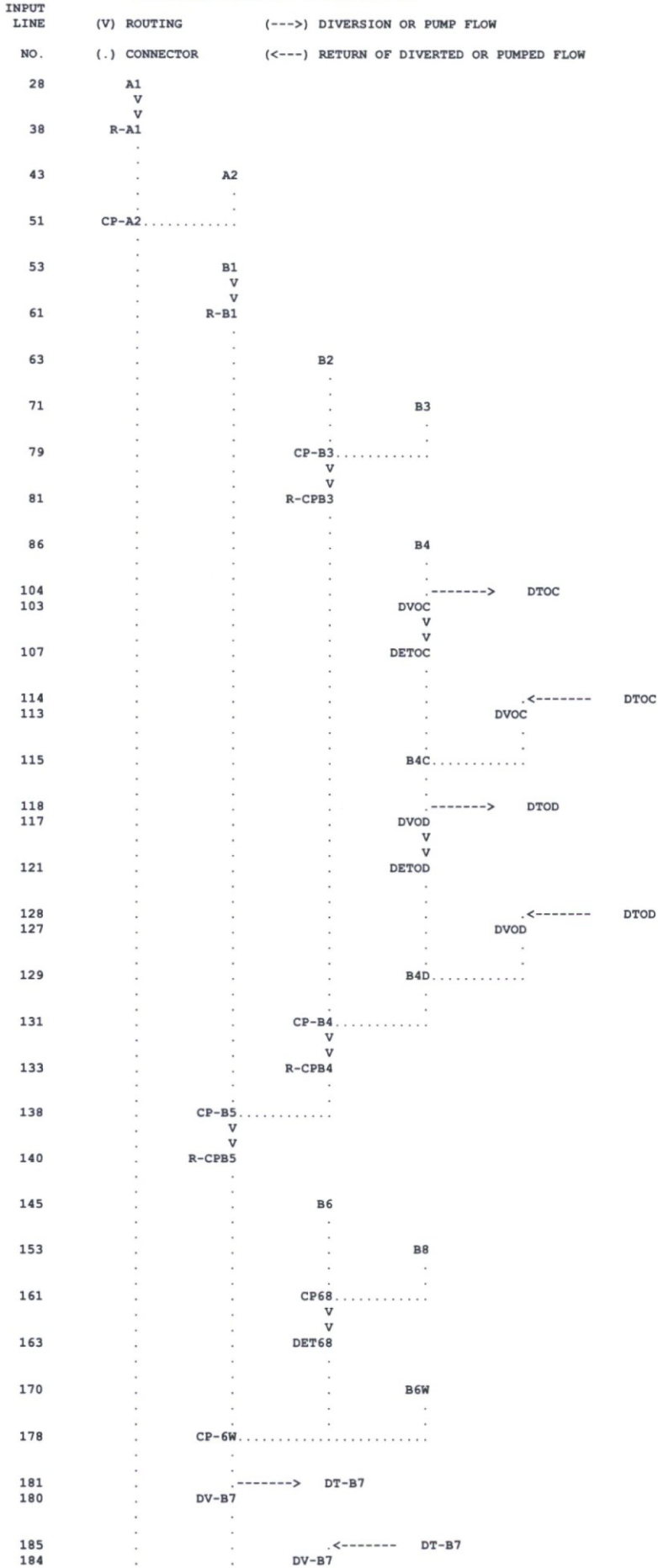
321 KK DV-B11 DIVERT
 322 DT DT-B11 0.26 0.0
 323 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
 324 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0

*

325 KK L1 BASIN
 326 BA 0.003
 327 LG 0.35 0.36 5.05 0.28 0
 328 UI 0 6 23 33 25 14 7 4 2 1
 329 UI 1 0 0 0 0 0 0 0 0 0
 330 UI 0 0 0 0 0 0 0 0 0 0
 331 UI 0 0 0 0 0 0 0 0 0 0
 332 UI 0 0 0 0 0 0 0 0 0 0

*

SCHMATIC DIAGRAM OF STREAM NETWORK



```

186 . . . . . B14A
. . . . .
194 . . . . . CP-V7
. . . . . V
196 . . . . . DETBV7
. . . . .
202 . . . . . B9
. . . . .
211 . . . . . -----> DT-B9
210 . . . . . DV-B9
. . . . .
214 . . . . . CP-B9
. . . . . V
216 . . . . . R-CPB9
. . . . .
221 . . . . . B10
. . . . .
230 . . . . . -----> DT-B10
229 . . . . . DT-B10
. . . . .
233 . . . . . B12
. . . . .
241 . . . . . B14
. . . . .
250 . . . . . -----> DT-B14
249 . . . . . DV-B14
. . . . .
253 . . . . . CP-B14
. . . . .
255 . . . . . C1
. . . . .
264 . . . . . -----> DT-C1
263 . . . . . DV-C1
. . . . .
268 . . . . . -----> DTC-K
267 . . . . . DVC-K
. . . . .
272 . . . . . -----> DTC-H
271 . . . . . DVC-H
. . . . .
275 . . . . . B13
. . . . .
284 . . . . . -----> DT-B13
283 . . . . . DV-B13
. . . . .
287 . . . . . CPB13
. . . . .
290 . . . . . -----> DTGF
289 . . . . . DVGF
. . . . . V
293 . . . . . R-B13
. . . . .
295 . . . . . CPGAL
. . . . .
297 . . . . . B5
. . . . .
306 . . . . . -----> DT-B5
305 . . . . . DV-B5
. . . . .
309 . . . . . B11
. . . . .
318 . . . . . -----> DT-C11
317 . . . . . DV-C11
. . . . .
322 . . . . . -----> DT-B11
321 . . . . . DV-B11
. . . . .
325 . . . . . L1
. . . . .
333 . . . . .

```

M1

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00

*** **

* *
107 KK * DETOC * STORAGE
* *

108 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

* *
121 KK * DETOD * STORAGE
* *

122 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

* *
163 KK * DET68 * STORAGE
* *

164 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

* *
196 KK * DETBV7 * STORAGE
* *

197 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	468.	4.13	38.	10.	7.	.20		
ROUTED TO	R-A1	462.	4.18	38.	10.	7.	.20		
HYDROGRAPH AT	A2	36.	4.02	2.	0.	0.	.01		
2 COMBINED AT	CP-A2	467.	4.18	40.	10.	7.	.21		
HYDROGRAPH AT	B1	72.	4.02	4.	1.	1.	.02		
ROUTED TO	R-B1	72.	4.03	4.	1.	1.	.02		
HYDROGRAPH AT	B2	3.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	B3	111.	4.07	7.	2.	1.	.04		
2 COMBINED AT	CP-B3	111.	4.05	7.	2.	1.	.04		
ROUTED TO	R-CPB3	111.	4.07	7.	2.	1.	.04		
HYDROGRAPH AT	B4	1110.	4.42	163.	41.	29.	.92		
DIVERSION TO	DTOC	1050.	4.33	161.	40.	29.	.92		
HYDROGRAPH AT	DVOC	60.	4.42	2.	0.	0.	.92		
ROUTED TO	DETOC	8.	4.52	2.	0.	0.	.92		
HYDROGRAPH AT	DVOC	1050.	4.33	161.	40.	29.	.92		
2 COMBINED AT	B4C	1058.	4.50	163.	41.	29.	.92		
DIVERSION TO	DTOD	975.	4.28	159.	40.	29.	.92		
HYDROGRAPH AT	DVOD	83.	4.50	3.	1.	1.	.92		
ROUTED TO	DETOD	12.	4.58	3.	1.	1.	.92		
HYDROGRAPH AT	DVOD	975.	4.28	159.	40.	29.	.92		
2 COMBINED AT	B4D	987.	4.57	162.	41.	29.	.92		
2 COMBINED AT	CP-B4	1012.	4.27	168.	42.	30.	.96		
ROUTED TO	R-CPB4	1013.	4.28	168.	42.	30.	.96		
2 COMBINED AT	CP-B5	1024.	4.28	172.	43.	31.	.98		
ROUTED TO	R-CPB5	1021.	4.30	172.	43.	31.	.98		
HYDROGRAPH AT	B6	29.	4.02	1.	0.	0.	.01		
HYDROGRAPH AT	B8	36.	4.02	2.	1.	0.	.01		
2 COMBINED AT	CP68	65.	4.02	4.	1.	1.	.02		
ROUTED TO	DET68	2.	3.83	2.	1.	1.	.02		
HYDROGRAPH AT	B6W	9.	4.02	0.	0.	0.	.00		
3 COMBINED AT	CP-6W	1022.	4.30	173.	44.	32.	1.00		
DIVERSION TO	DT-B7	22.	4.30	4.	1.	1.	1.00		

+	HYDROGRAPH AT	DV-B7	1001.	4.30	169.	43.	31.	1.00
+	HYDROGRAPH AT	DV-B7	22.	4.30	4.	1.	1.	1.00
+	HYDROGRAPH AT	B14A	15.	4.02	1.	0.	0.	.00
+	2 COMBINED AT	CP-V7	29.	4.02	5.	1.	1.	.00
+	ROUTED TO	DETBV7	22.	4.53	5.	1.	1.	.00
+	HYDROGRAPH AT	B9	29.	4.02	2.	1.	0.	.01
+	DIVERSION TO	DT-B9	8.	3.67	0.	0.	0.	.01
+	HYDROGRAPH AT	DV-B9	29.	4.02	2.	0.	0.	.01
+	3 COMBINED AT	CP-B9	1022.	4.30	174.	45.	32.	1.02
+	ROUTED TO	R-CPB9	1016.	4.33	174.	45.	32.	1.02
+	HYDROGRAPH AT	B10	38.	4.02	2.	0.	0.	.01
+	DIVERSION TO	DT-B10	35.	3.88	1.	0.	0.	.01
+	HYDROGRAPH AT	DT-B10	38.	4.02	1.	0.	0.	.01
+	HYDROGRAPH AT	B12	15.	4.02	1.	0.	0.	.00
+	HYDROGRAPH AT	B14	134.	4.02	9.	2.	2.	.04
+	DIVERSION TO	DT-B14	50.	3.82	2.	1.	0.	.04
+	HYDROGRAPH AT	DV-B14	134.	4.02	6.	2.	1.	.04
+	4 COMBINED AT	CP-B14	1027.	4.32	181.	46.	33.	1.07
+	HYDROGRAPH AT	C1	78.	4.02	5.	1.	1.	.02
+	DIVERSION TO	DT-C1	48.	3.87	1.	0.	0.	.02
+	HYDROGRAPH AT	DV-C1	78.	4.02	3.	1.	1.	.02
+	DIVERSION TO	DTC-K	50.	3.88	3.	1.	0.	.02
+	HYDROGRAPH AT	DVC-K	28.	4.02	1.	0.	0.	.02
+	DIVERSION TO	DTC-H	15.	3.92	1.	0.	0.	.02
+	HYDROGRAPH AT	DVC-H	13.	4.02	0.	0.	0.	.02
+	HYDROGRAPH AT	B13	11.	4.02	1.	0.	0.	.00
+	DIVERSION TO	DT-B13	4.	3.55	0.	0.	0.	.00
+	HYDROGRAPH AT	DV-B13	11.	4.02	1.	0.	0.	.00
+	2 COMBINED AT	CPB13	24.	4.02	1.	0.	0.	.03
+	DIVERSION TO	DTGF	3.	3.58	0.	0.	0.	.03
+	HYDROGRAPH AT	DVGF	21.	4.02	1.	0.	0.	.03
+	ROUTED TO	R-B13	21.	4.02	1.	0.	0.	.03
+	2 COMBINED AT	CPGAL	1025.	4.32	181.	46.	33.	1.10
+	HYDROGRAPH AT	B5	20.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-B5	20.	4.02	1.	0.	0.	.01

+	HYDROGRAPH AT	DV-B5	17.	4.03	0.	0.	0.	.01
+	HYDROGRAPH AT	B11	35.	4.02	2.	1.	0.	.01
+	DIVERSION TO	DT-C11	5.	3.58	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-C11	30.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-B11	28.	3.90	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-B11	30.	4.02	1.	0.	0.	.01
+	HYDROGRAPH AT	L1	10.	4.02	1.	0.	0.	.00
+	HYDROGRAPH AT	M1	3.	4.02	0.	0.	0.	.00

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
COMPUTATION INTERVAL
TIME TO
PEAK

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)
FOR STORM = 1	STORM AREA (SQ MI) =			.00					
R-B1	MANE	.30	72.25	241.40	1.72	1.00	72.12	242.00	1.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2021E+01 EXCESS= .0000E+00 OUTFLOW= .2021E+01 BASIN STORAGE= .3970E-14 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =			.50					
R-B1	MANE	.18	71.78	241.39	1.71	1.00	71.60	242.00	1.71
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2002E+01 EXCESS= .0000E+00 OUTFLOW= .2003E+01 BASIN STORAGE= .3981E-14 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =			1.00					
R-B1	MANE	.17	56.32	241.36	1.57	1.00	56.19	242.00	1.57
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1841E+01 EXCESS= .0000E+00 OUTFLOW= .1842E+01 BASIN STORAGE= .3970E-14 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =			2.80					
R-B1	MANE	.17	33.58	241.35	1.40	1.00	33.53	242.00	1.40
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1647E+01 EXCESS= .0000E+00 OUTFLOW= .1647E+01 BASIN STORAGE= .3963E-14 PERCENT ERROR= .0									
FOR STORM = 1	STORM AREA (SQ MI) =			.00					
R-B13	MANE	.35	21.50	240.86	.23	1.00	21.47	241.00	.23
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3152E+00 EXCESS= .0000E+00 OUTFLOW= .3155E+00 BASIN STORAGE= .7448E-14 PERCENT ERROR= -.1									
FOR STORM = 2	STORM AREA (SQ MI) =			.50					
R-B13	MANE	.39	20.87	240.84	.22	1.00	20.79	241.00	.22
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3053E+00 EXCESS= .0000E+00 OUTFLOW= .3054E+00 BASIN STORAGE= .7516E-14 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =			1.00					
R-B13	MANE	.31	5.94	240.94	.10	1.00	5.94	241.00	.10
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1403E+00 EXCESS= .0000E+00 OUTFLOW= .1408E+00 BASIN STORAGE= .7520E-14 PERCENT ERROR= -.4									
FOR STORM = 4	STORM AREA (SQ MI) =			2.80					
R-B13	MANE	.45	2.67	240.59	.06	1.00	2.67	240.00	.06
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8898E-01 EXCESS= .0000E+00 OUTFLOW= .8918E-01 BASIN STORAGE= .7557E-14 PERCENT ERROR= -.2									

*** NORMAL END OF HEC-1 ***

APPENDIX C

Site Retention Calculations

Required First Flush Storm Water Calculations

Provided Storm Water Calculations

Required First Flush Storm Water Calculations

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

First Flush Volume Required

Description: Desert Mountain Parcel 19
Date: 05/06/16
Location: City of Scottsdale
Reference:

City of Scottsdale, *Design Standards and Policies Manual Chapter 4 Grading & Drainage*, January 2010.

Known Values: First Flush
 0.5 inches

Calc. Values: $V=(P/12)*A*C$

V = Retention Volume Required
 C = Runoff Coefficient
 P = Precipitation amount=100-year 2 hour rainfall
 A = Area of Watershed Contributing

Drainage Area	Area (SF)	Area (ac)	Weighted Runoff Coefficient	First Flush Volume (ac-ft)	Required Pre vs Post Volume (ac ft)
B5	165024	3.79	1.00	0.16	0.42
B6	322261	5.50	1.00	0.23	N/A
B8	300366	6.90	1.00	0.29	N/A
B9	217378	4.99	1.00	0.21	N/A
B10	335363	7.70	1.00	0.32	N/A
B11	269364	6.18	1.00	0.26	N/A
B13	80916	1.86	1.00	0.08	N/A
B14	1072718	24.63	1.00	1.03	N/A
B14A	115650	2.65	1.00	0.11	N/A
C1	636403	14.61	1.00	0.61	N/A
TOTAL =				3.28	

Provided Storm Water Calculations

Off-Site Detention Basin Capacity

Description: Proposed Detention Basin Capacities
 Project: **Desert Mountain Parcel 19**
 Reference: *Drainage Design Manual for Maricopa County, Arizona - Hydrology*
 City of Scottsdale, *Design Standards and Policies Manual*

Basin IDs	Bottom Contour Area	Top Contour	Bottom Elevation	Top Elevation	Volume Provided	Volume Provided
Basin 3	17,184	19,439	0.0	1	0.42	2.01
	19,439	21,796	1.0	2	0.47	
	21,796	24,252	2.0	3	0.53	
	24,252	26,810.00	3.0	4	0.59	

Basin IDs	Bottom Contour Area	Top Contour	Bottom Elevation	Top Elevation	Volume Provided	Volume Provided
Basin 4	10,798	13,254	2680.0	2681.0	0.28	1.39
	13,254	15,028	2681.0	2682.0	0.32	
	15,028	17,134	2682.0	2683.0	0.37	
	17,134	19,270	2683.0	2684.0	0.42	
	19,270	22,350	2684.0	2684.0	0.00	

On-Site Detention Basin Capacity

Description: Proposed Detention Basin Capacities
 Project: **Desert Mountain Parcel 19**
 Reference: *Drainage Design Manual for Maricopa County, Arizona - Hydrology*
 City of Scottsdale, *Design Standards and Policies Manual*

Basin IDs	Bottom Contour Area	Top Contour	Bottom Elevation	Top Elevation	Volume Provided	Volume Provided
Basin 1	8,584	10,210	2,606.0	2,607	0.22	1.10
	10,210	11,949	2,607.0	2,608	0.25	
	11,949	13,800	2,608.0	2,609	0.30	
	13,800	15,757	2,609.0	2,610	0.34	

Basin IDs	Bottom Contour Area	Top Contour	Bottom Elevation	Top Elevation	Volume Provided	Volume Provided
Basin 2	3,232	4,553	2,608.0	2,609	0.09	0.58
	4,553	6,056	2,609.0	2,610	0.12	
	6,056	7,964	2,610.0	2,611	0.16	
	7,964	10,184	2,611.0	2,612	0.21	

APPENDIX D

Hydraulic Calculations

HEC-RAS Output Files Existing & Proposed Conditions

Scour Calculations

Erosion Hazard Setback Calculations

HEC-RAS Output Files Existing & Proposed Conditions

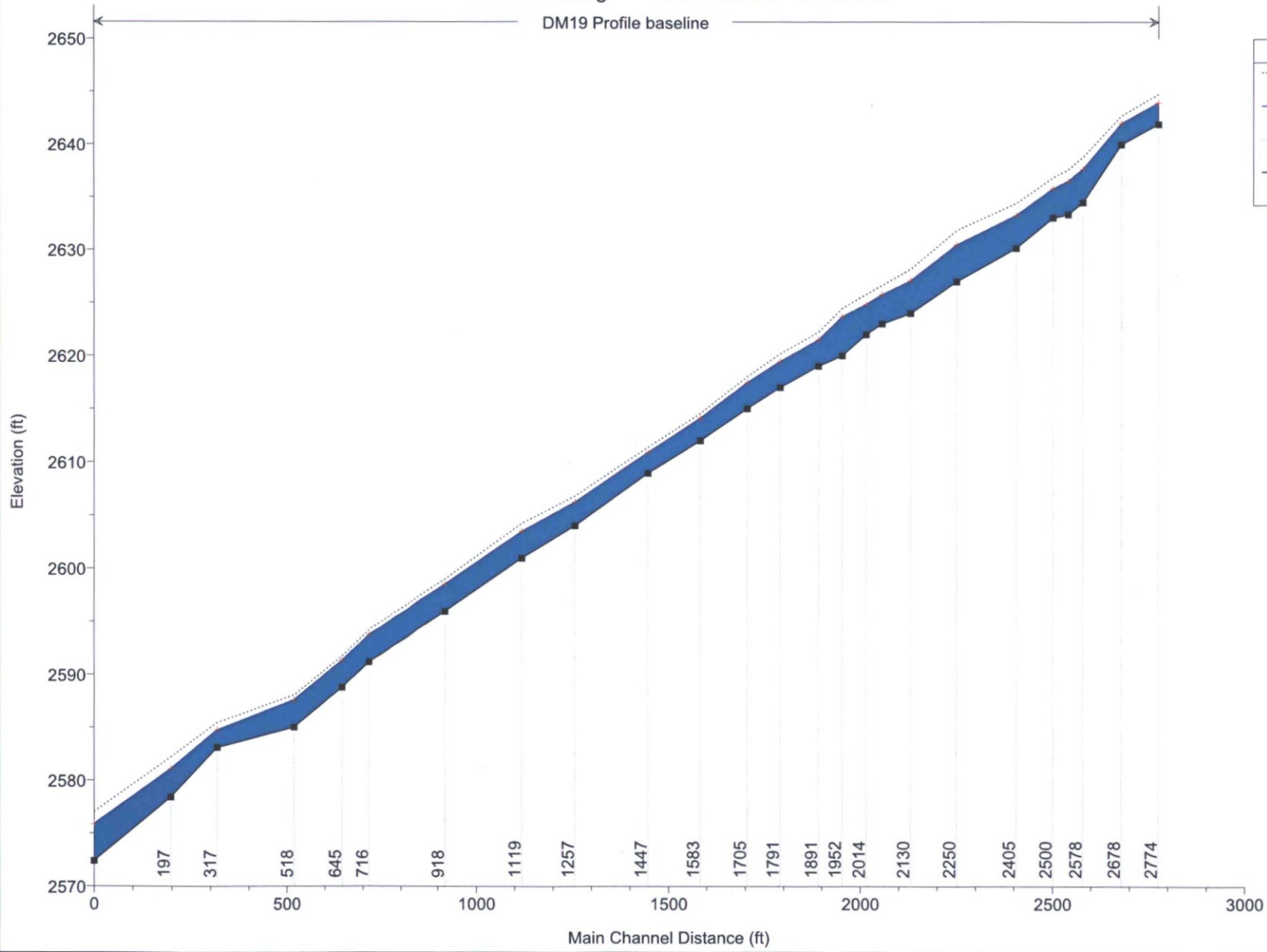
EXISTING

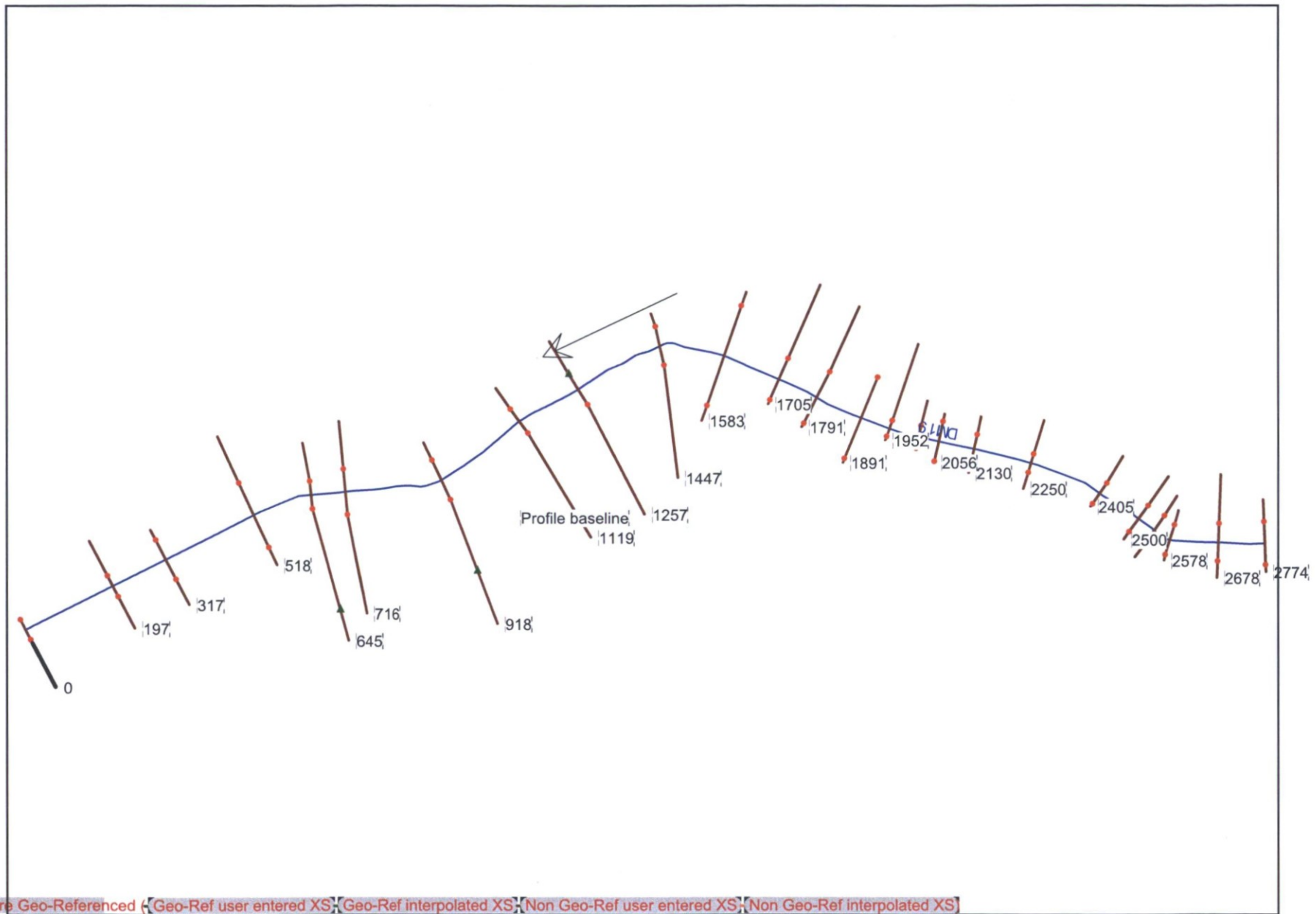
GALLOWAY WASH

DM19 Existing Plan: Plan 01 6/15/2016

DM19 Profile baseline

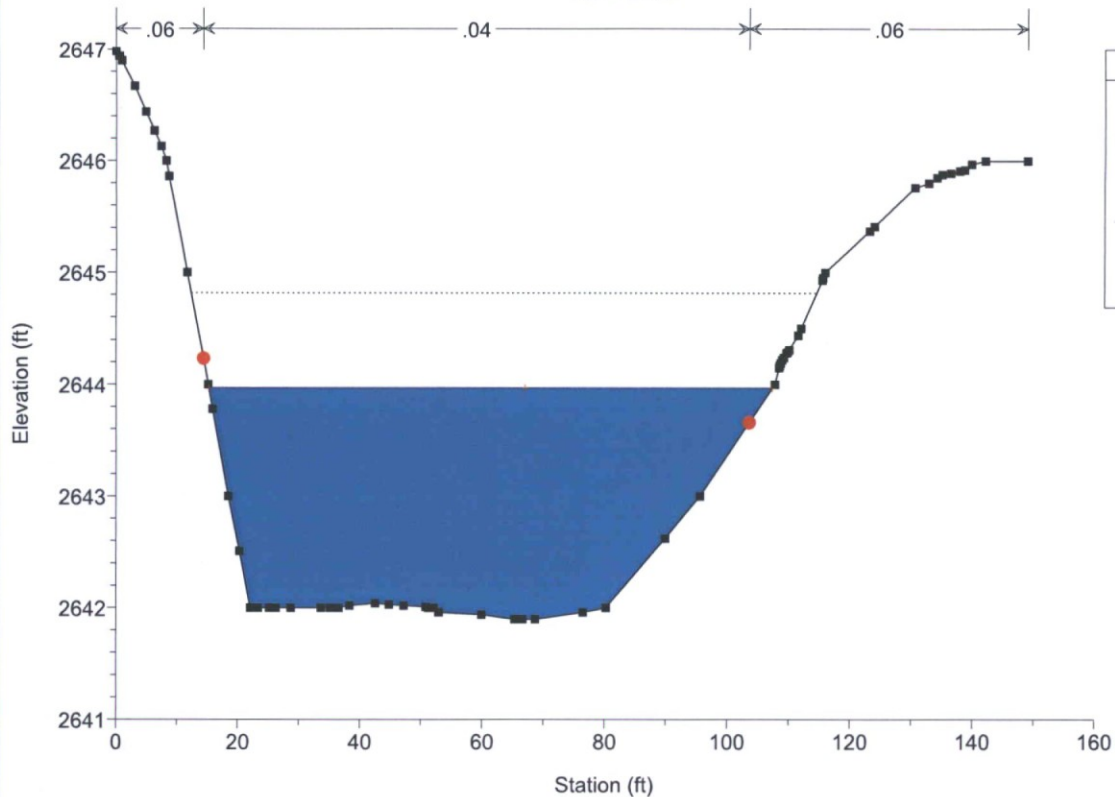
Legend	
EG PF 1
WS PF 1	————
Crit PF 1	+
Ground	■



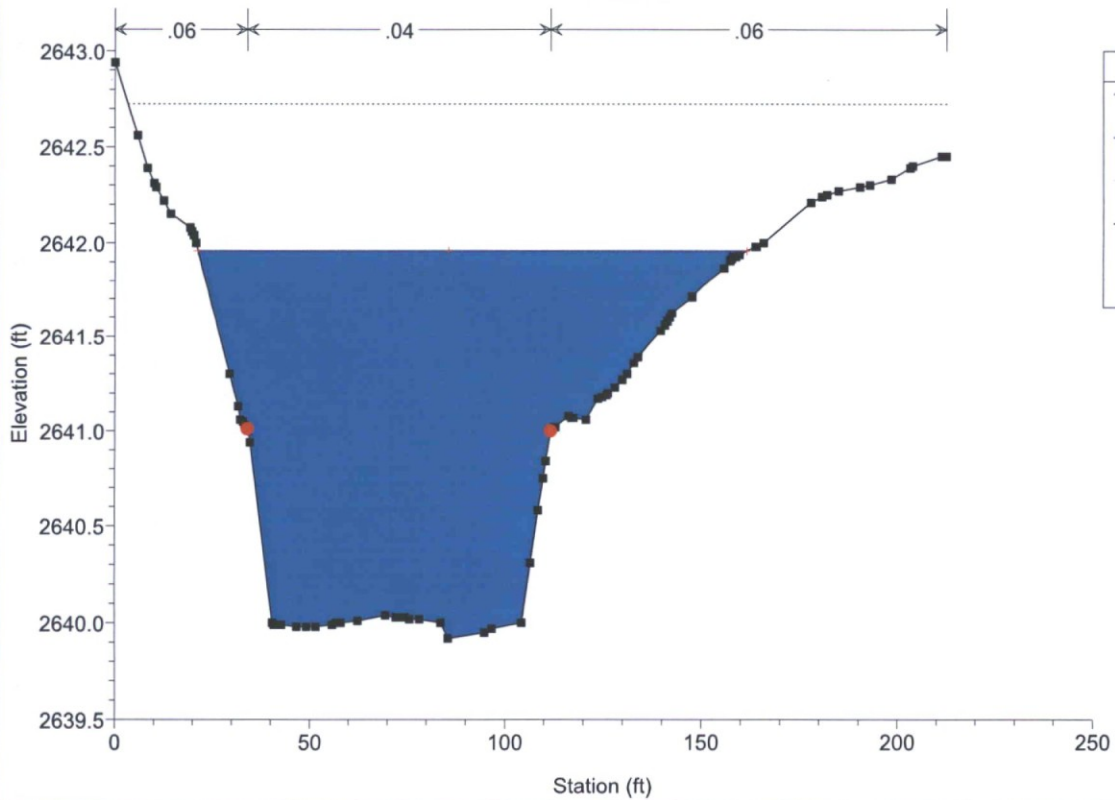


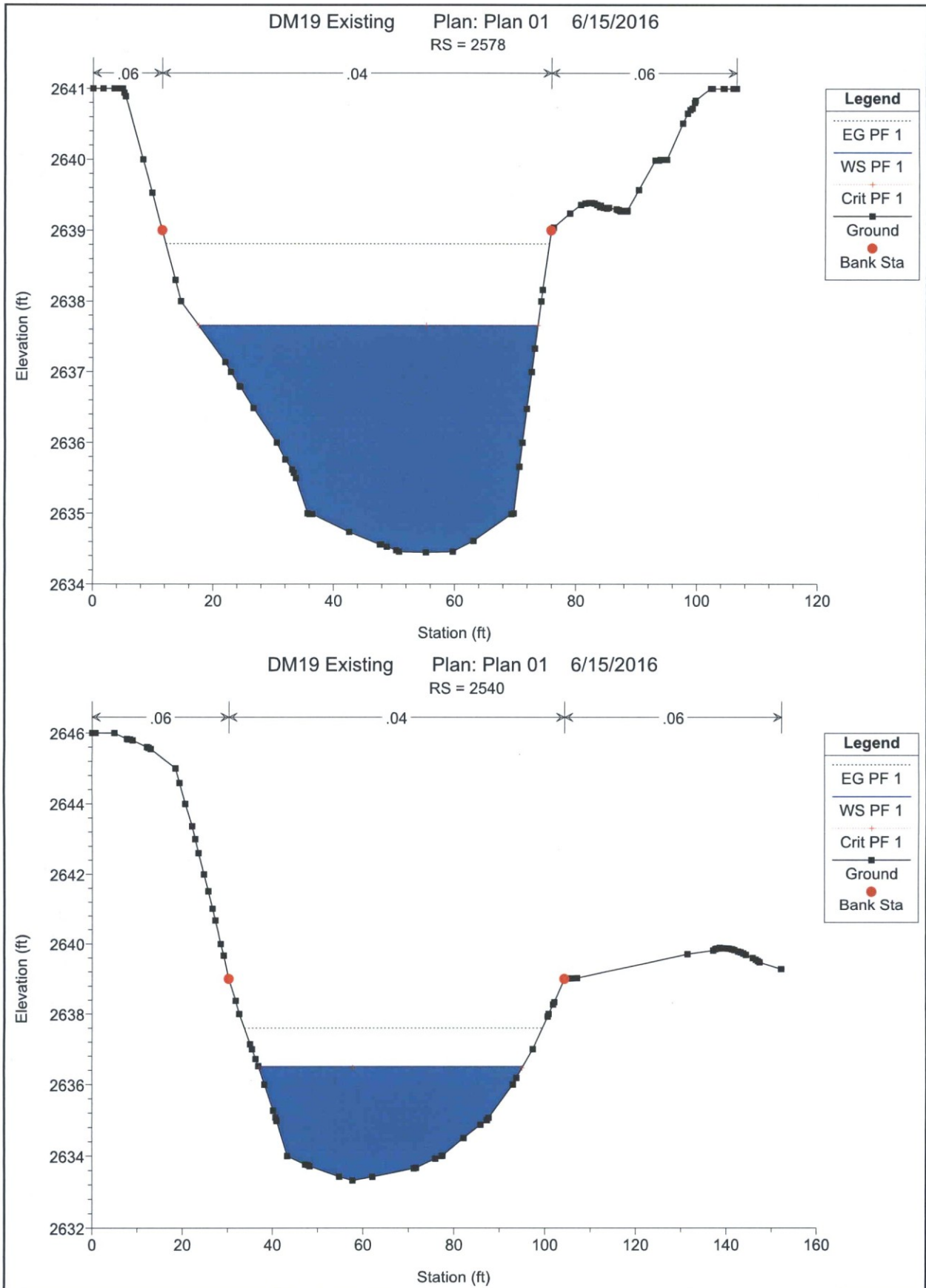
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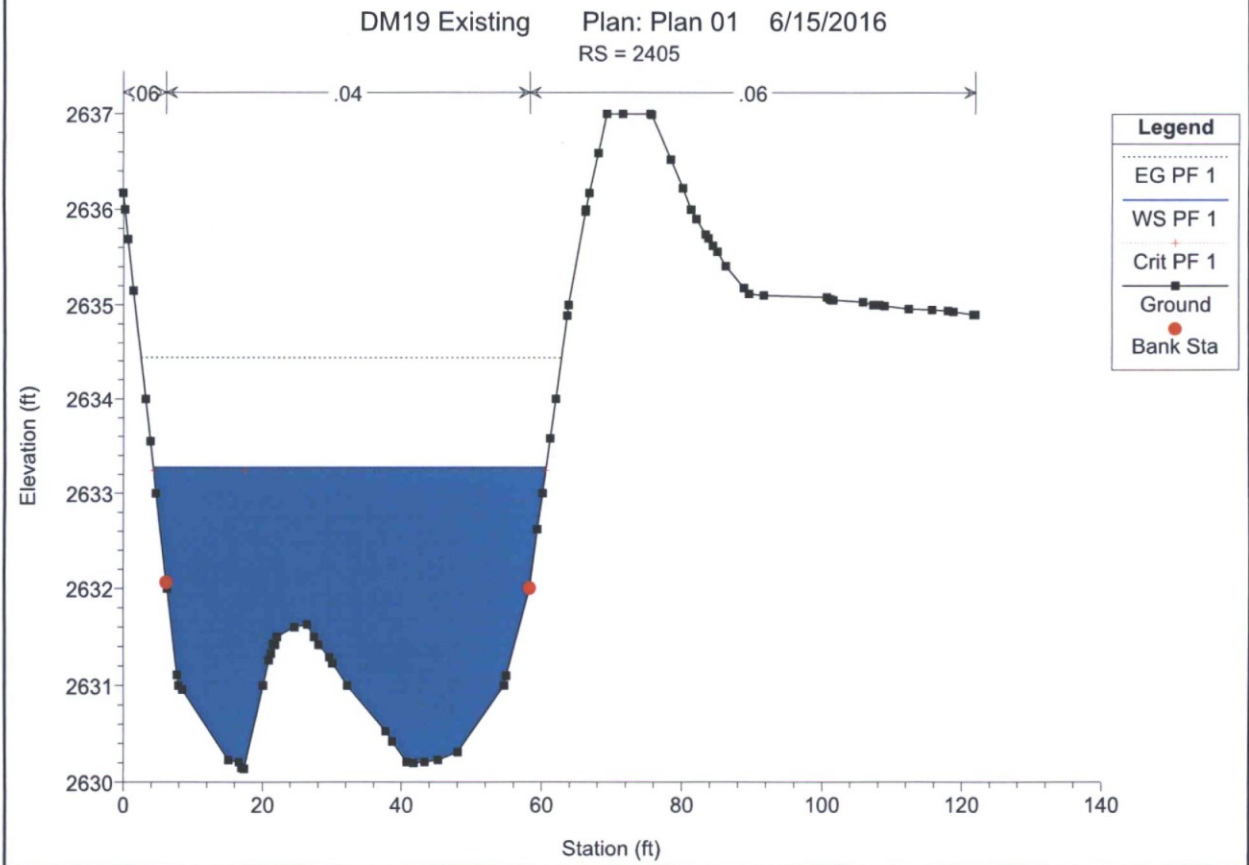
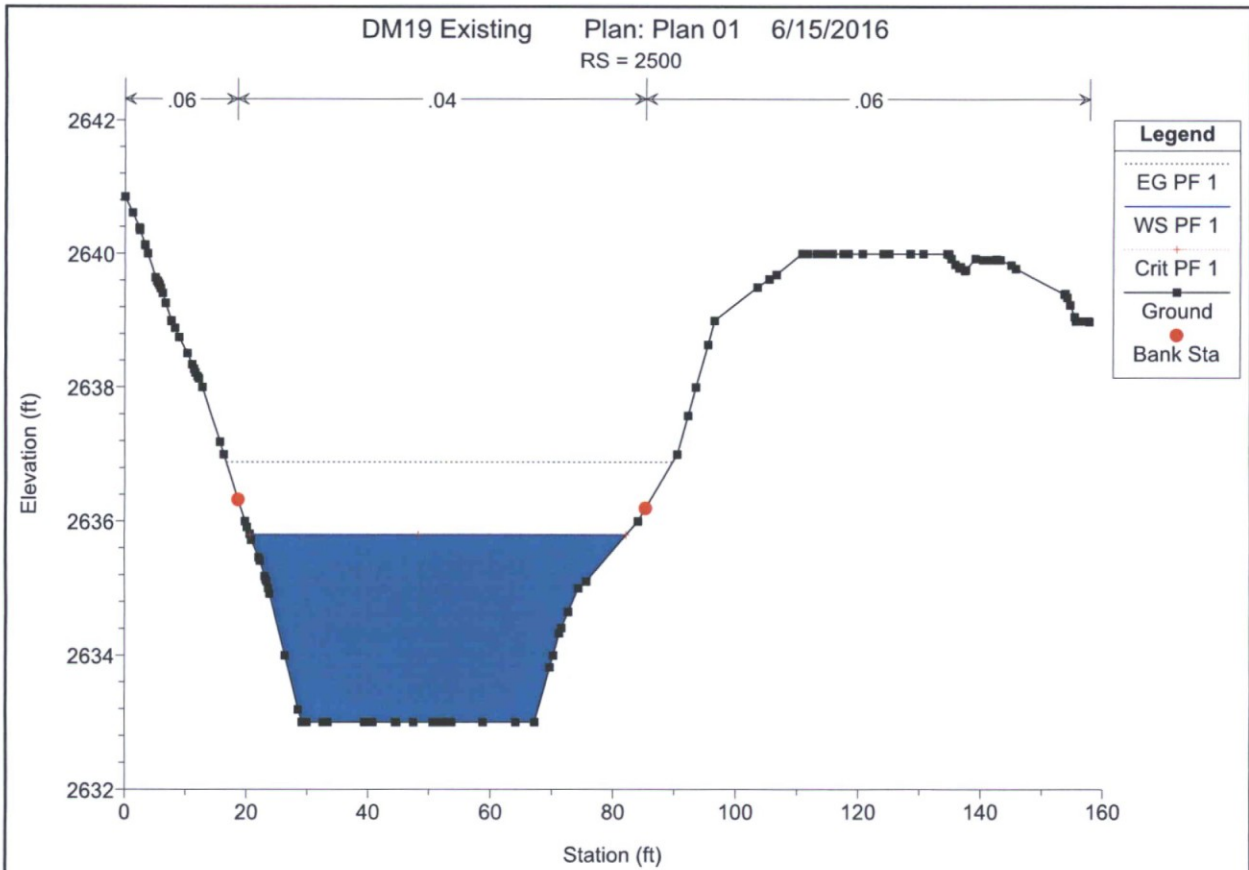
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RS = 2774

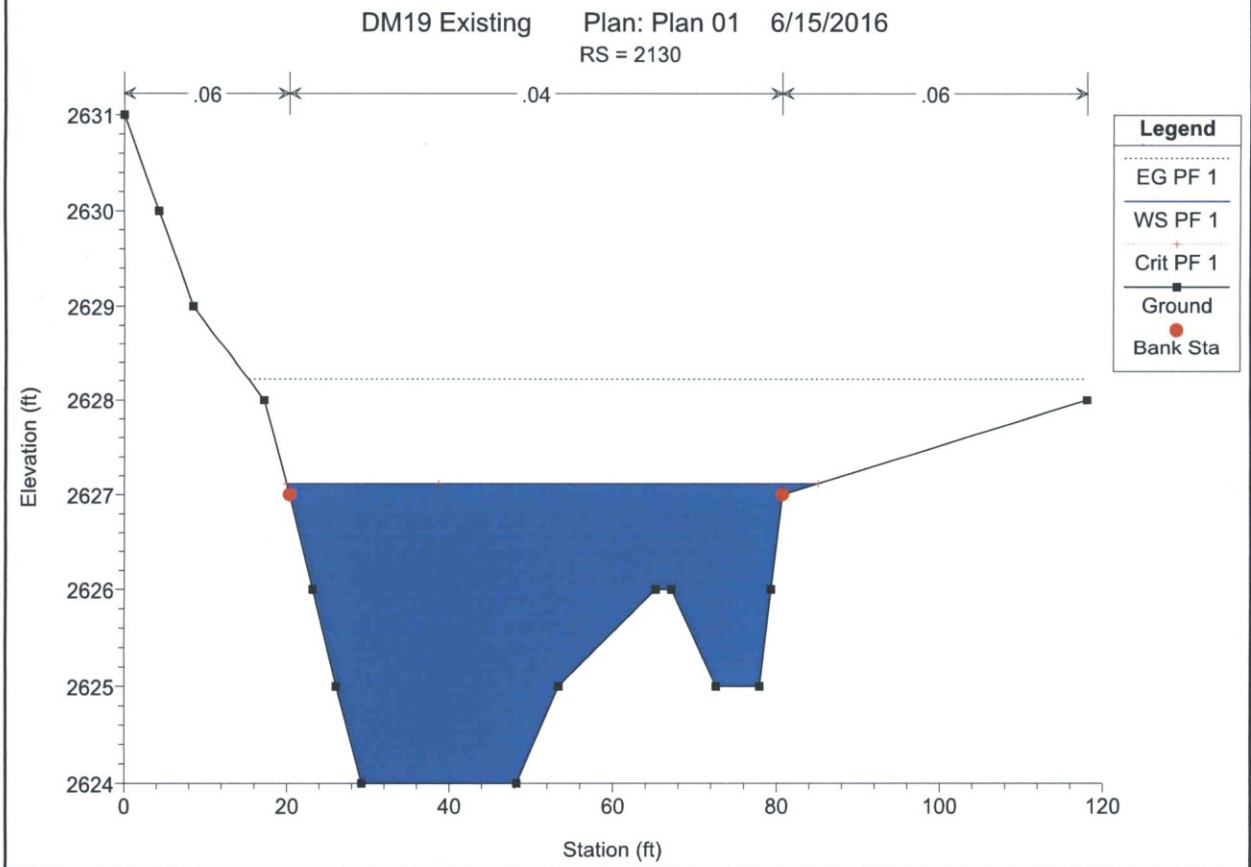
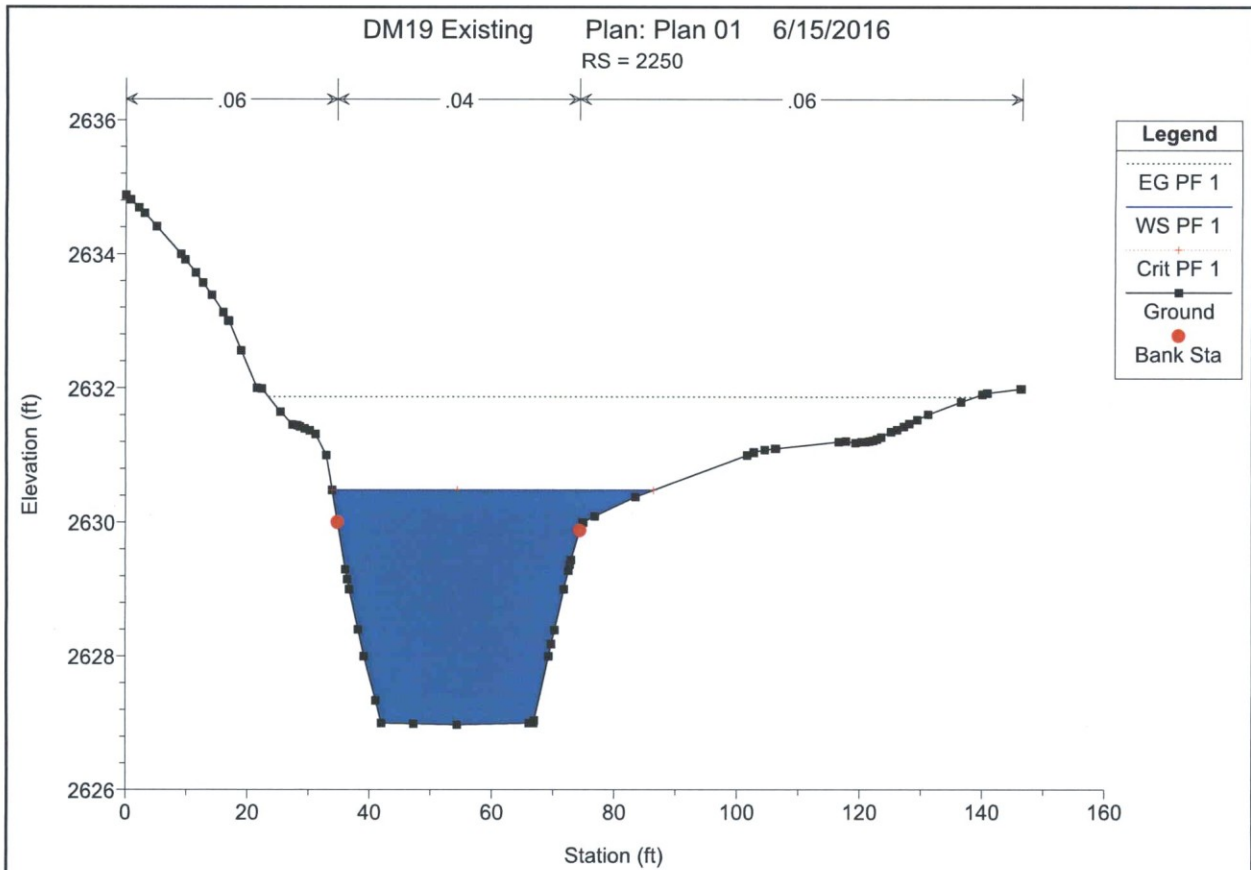


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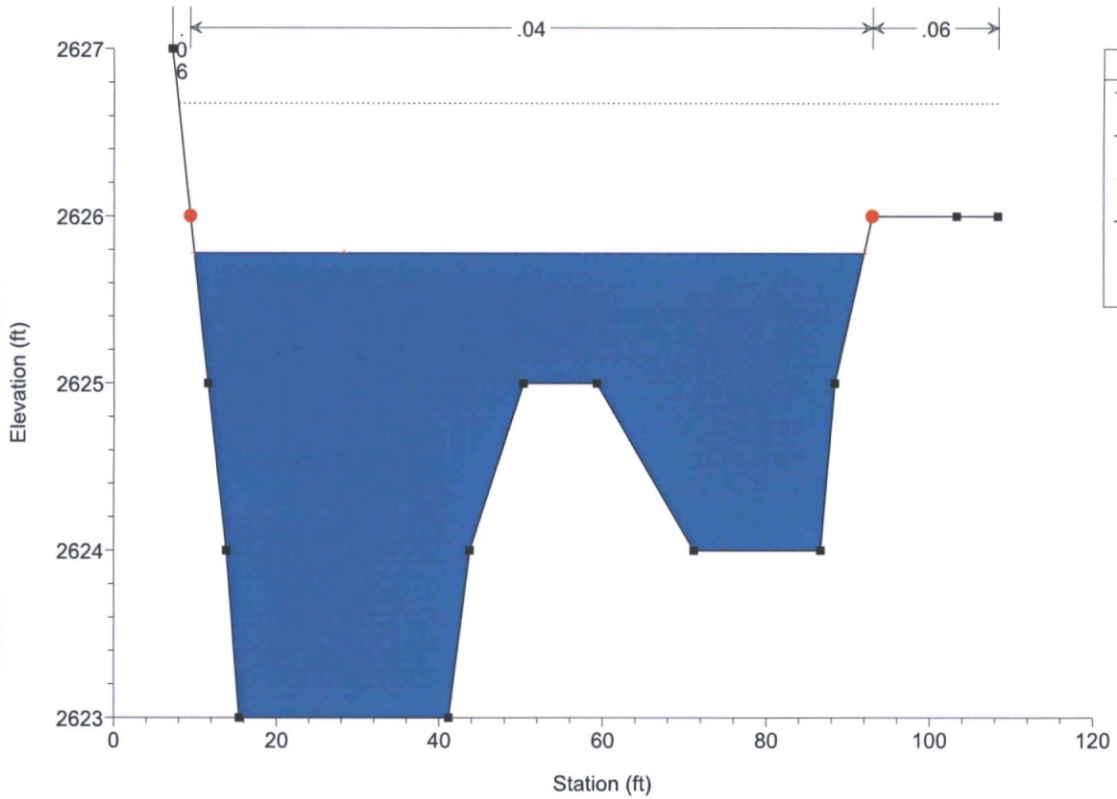






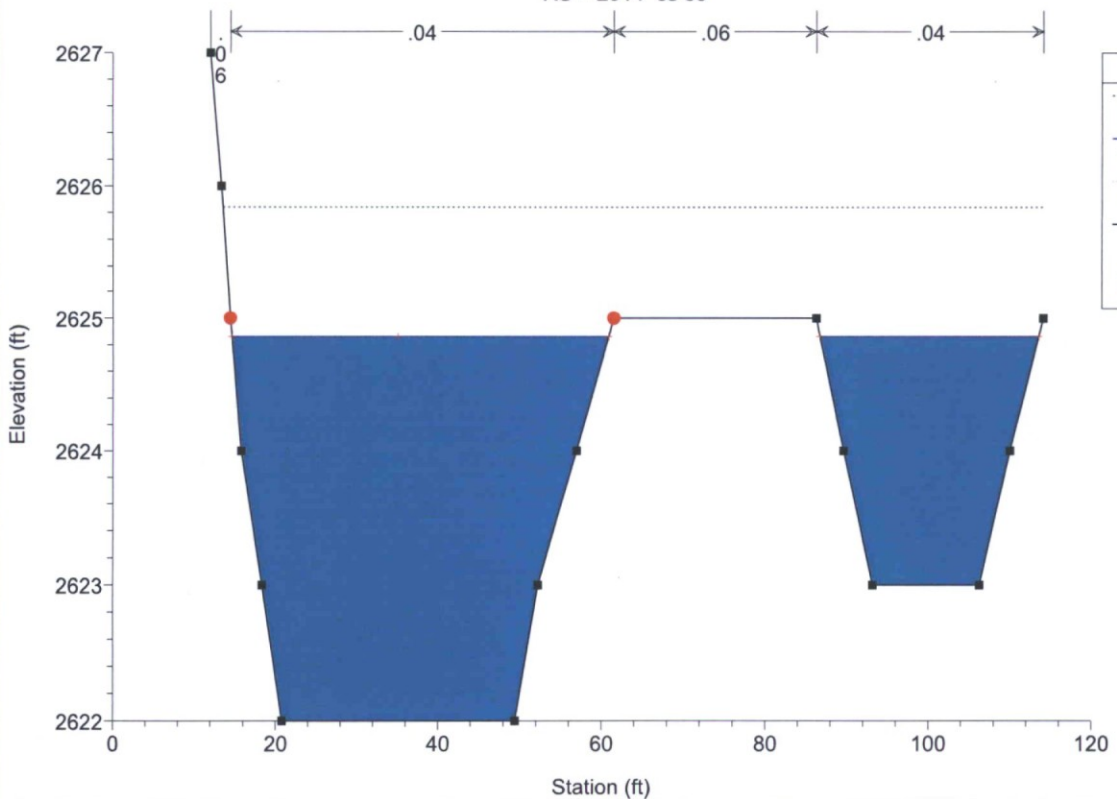


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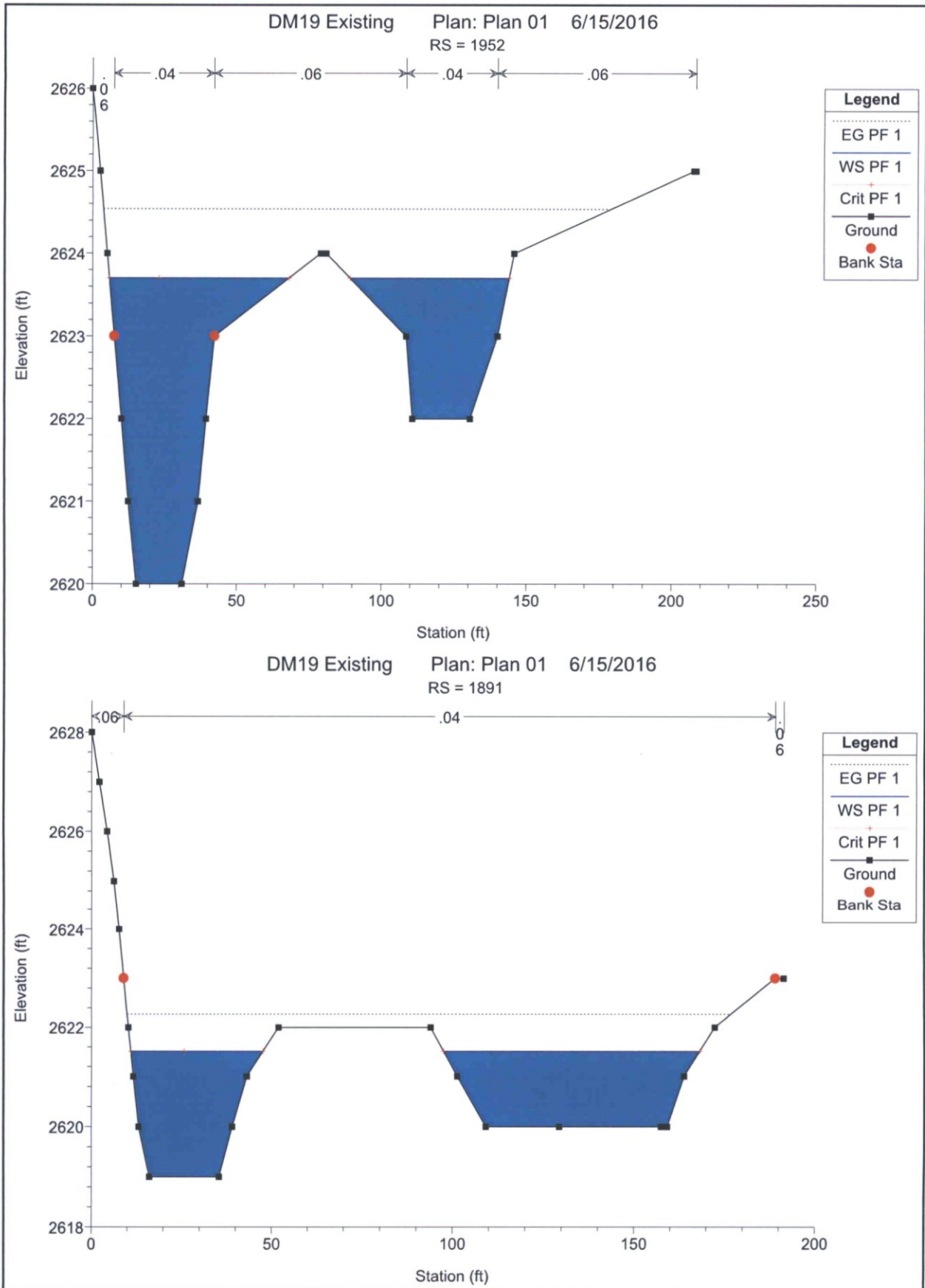


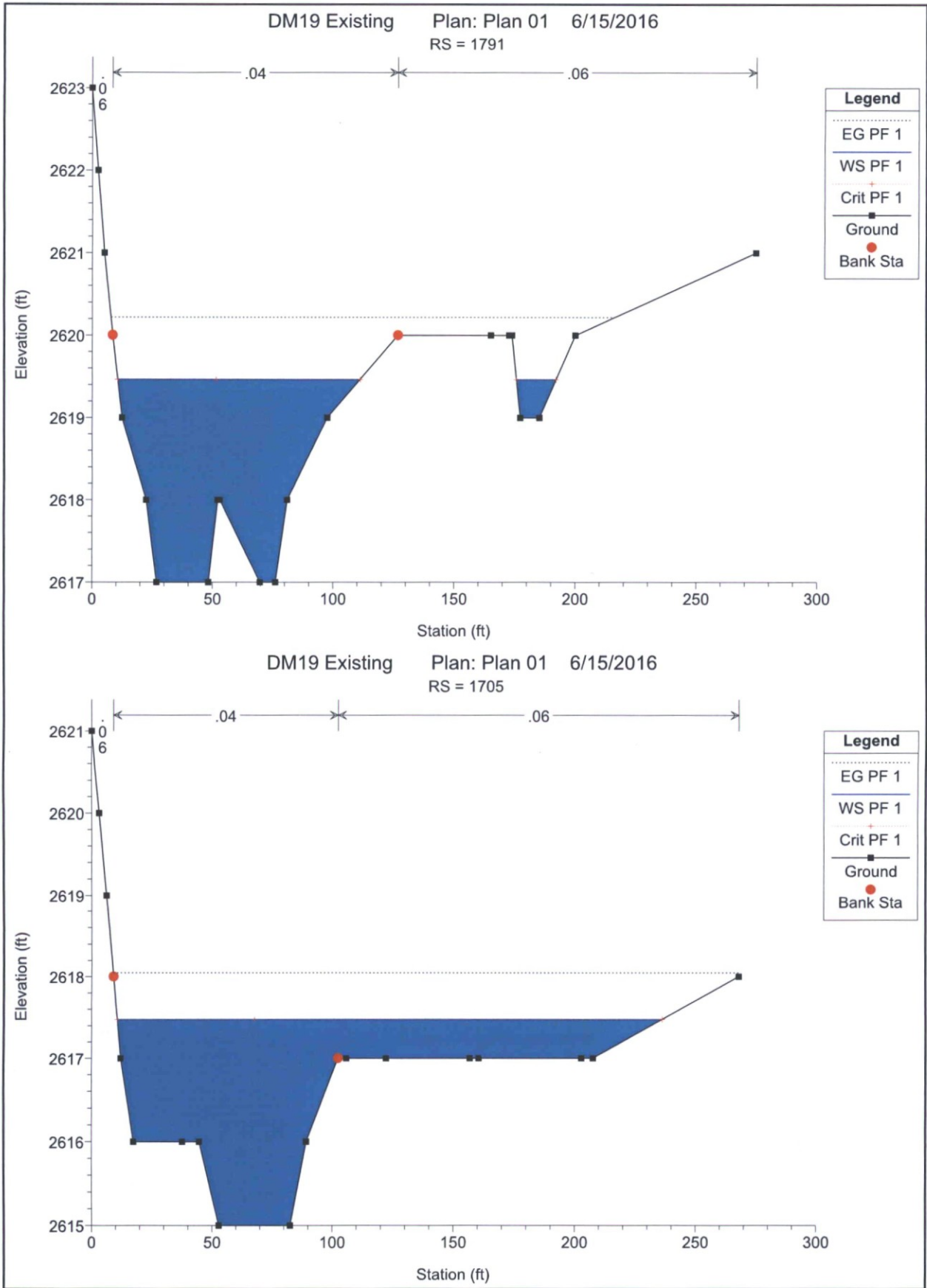
Legend	
.....	EG PF 1
-----	WS PF 1
+	Crit PF 1
■	Ground
●	Bank Sta

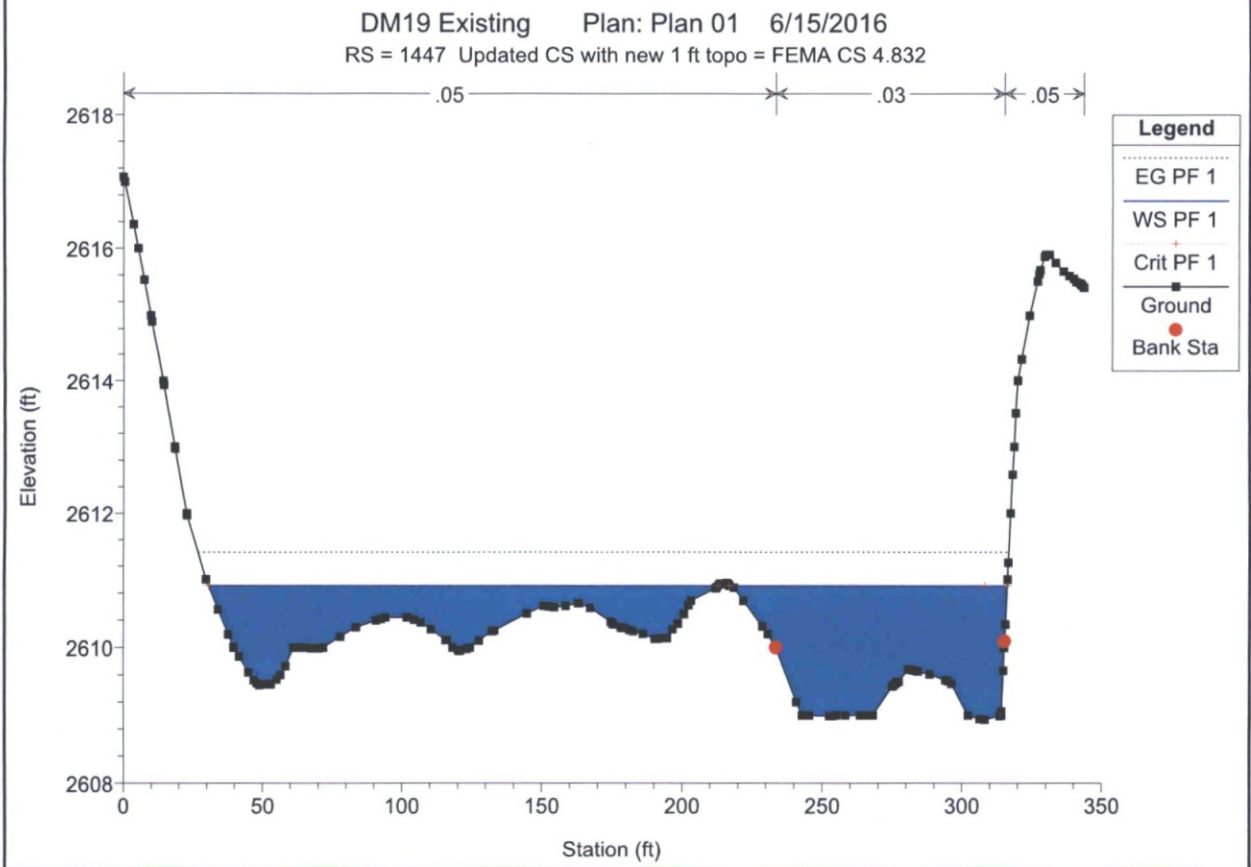
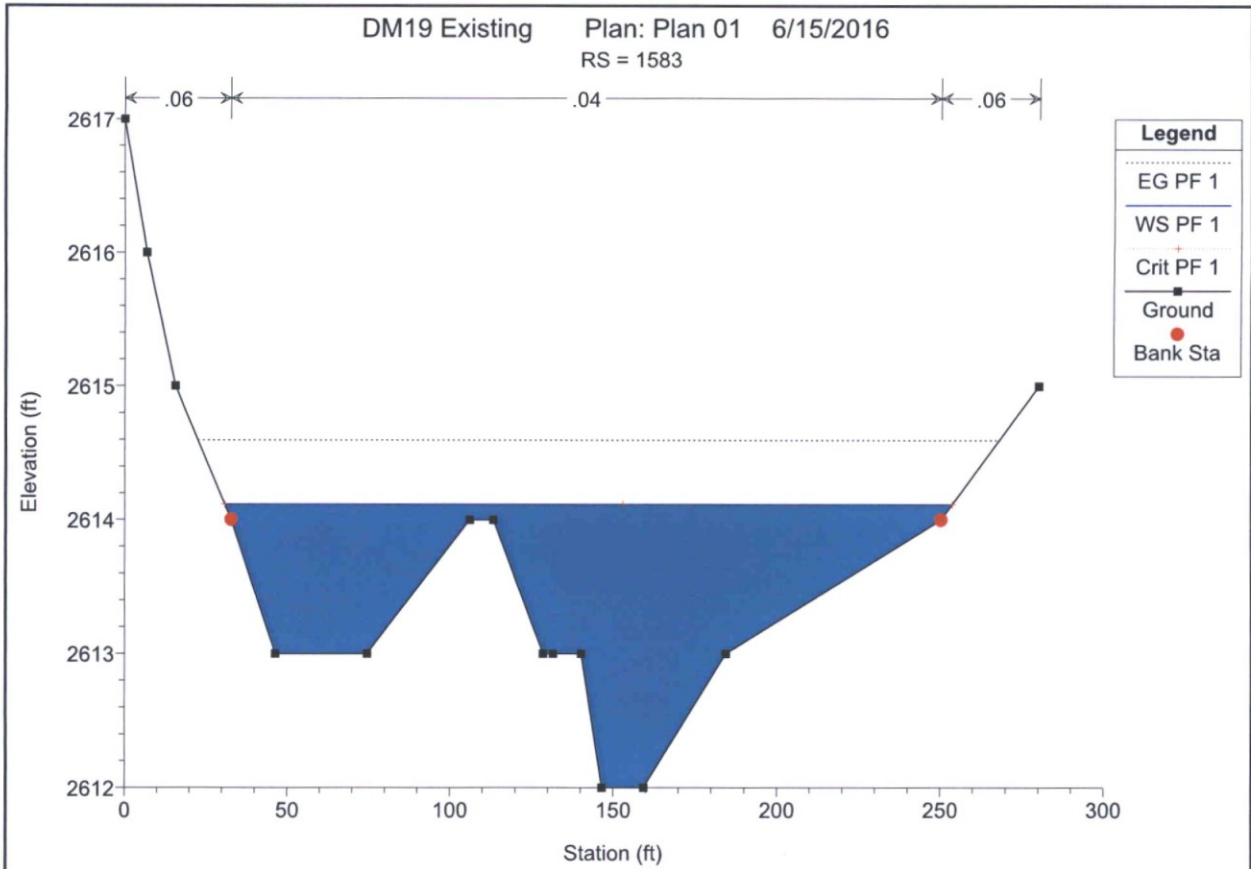
DM19 Existing Plan: Plan 01 6/15/2016
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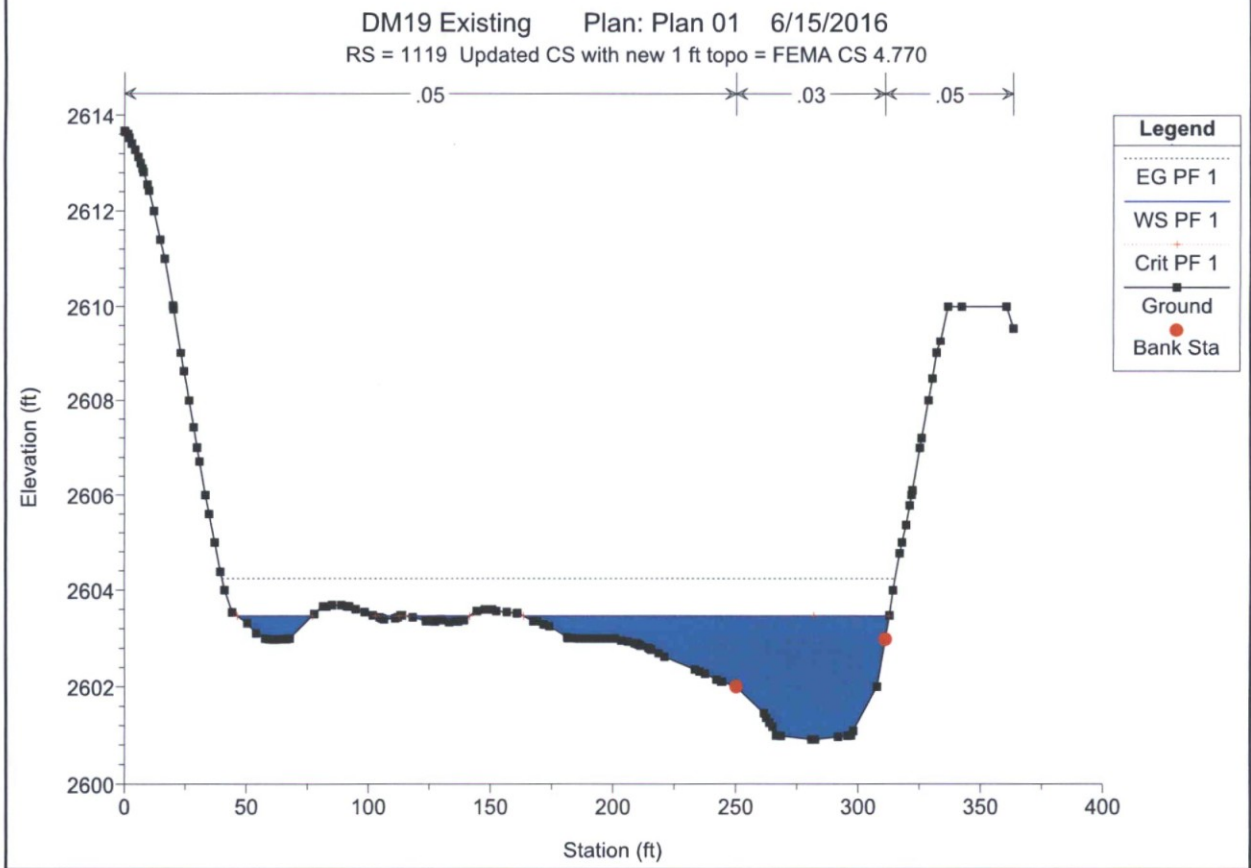
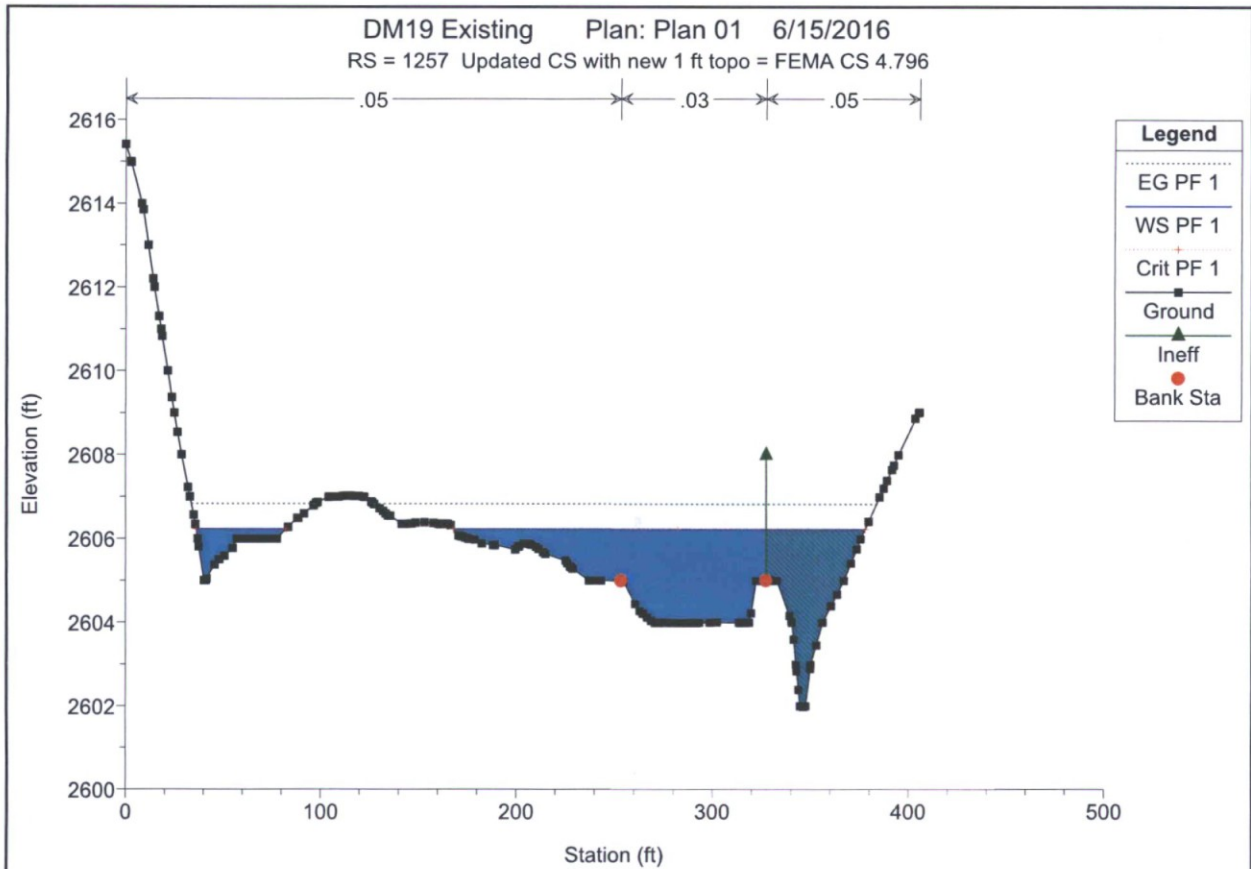


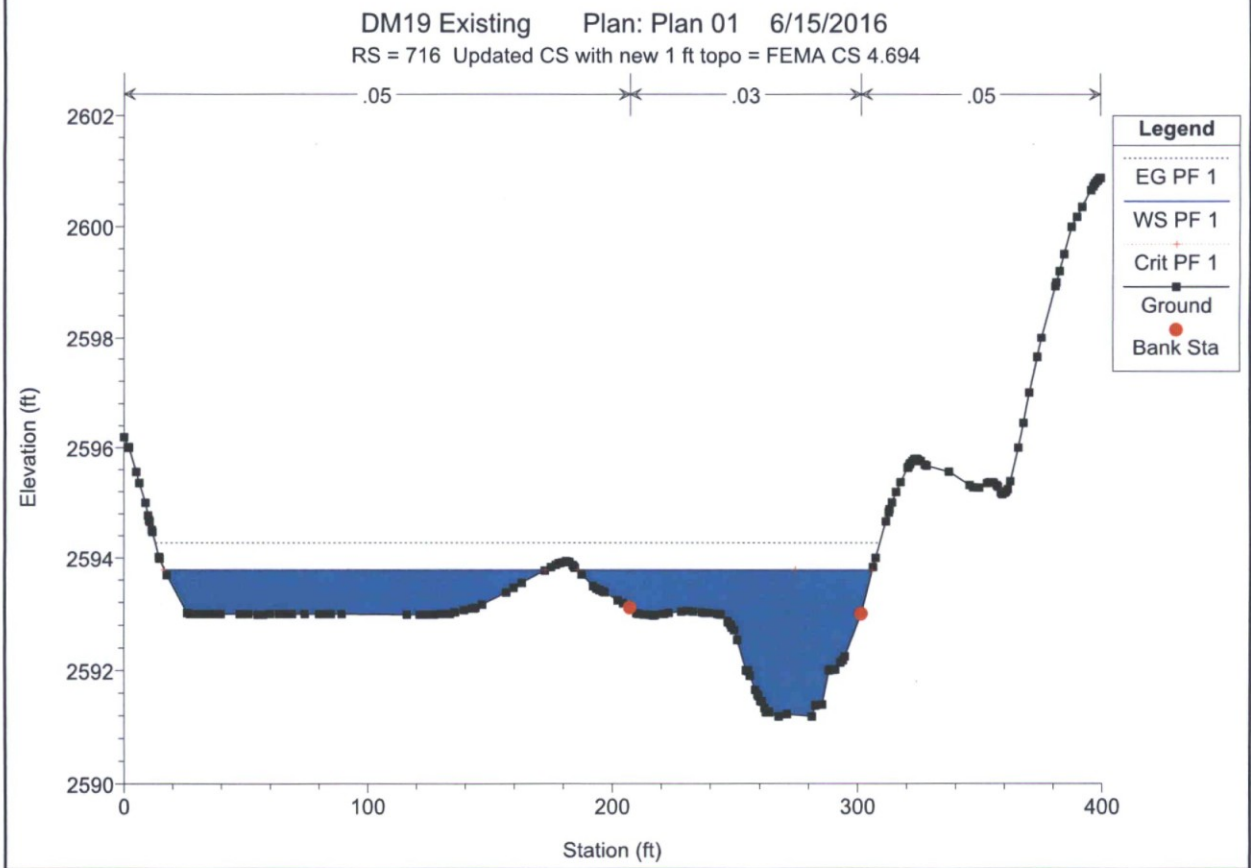
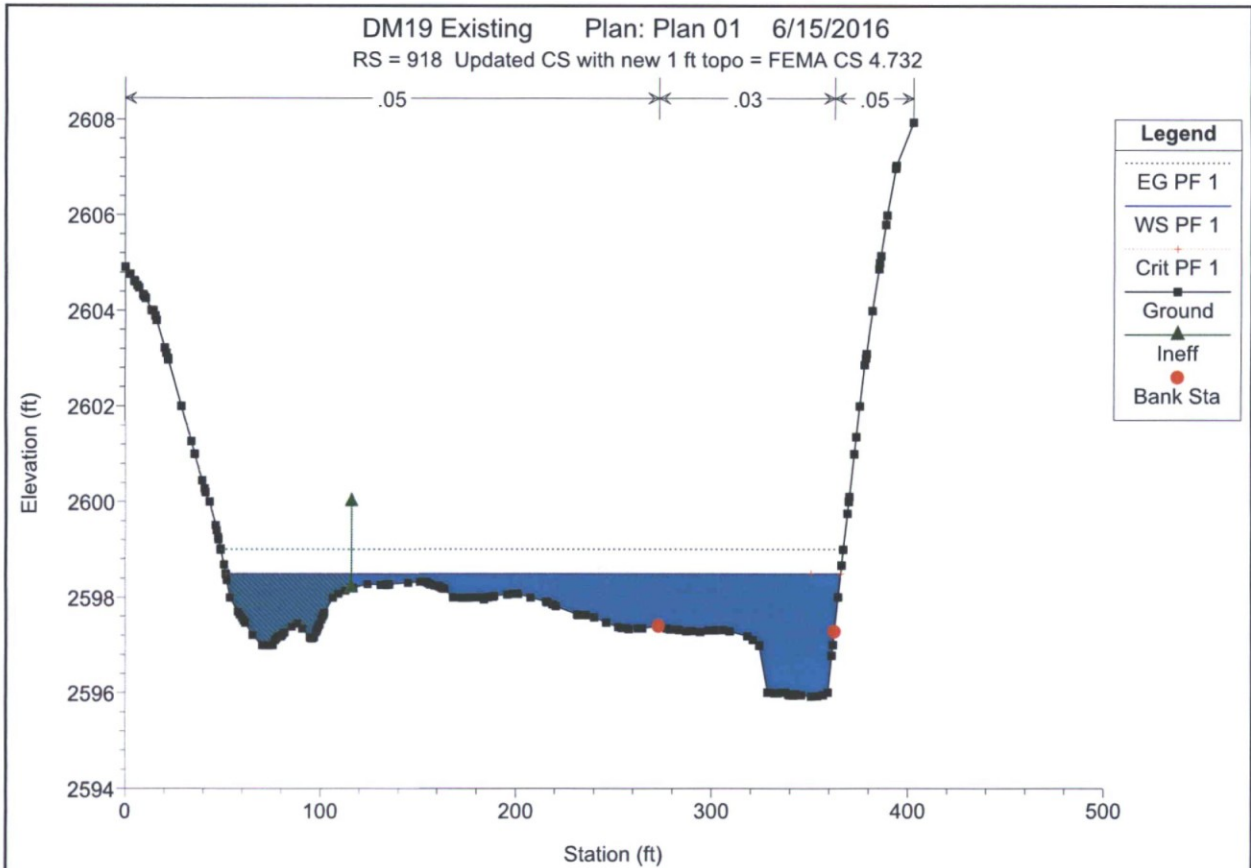
Legend	
.....	EG PF 1
-----	WS PF 1
+	Crit PF 1
■	Ground
●	Bank Sta

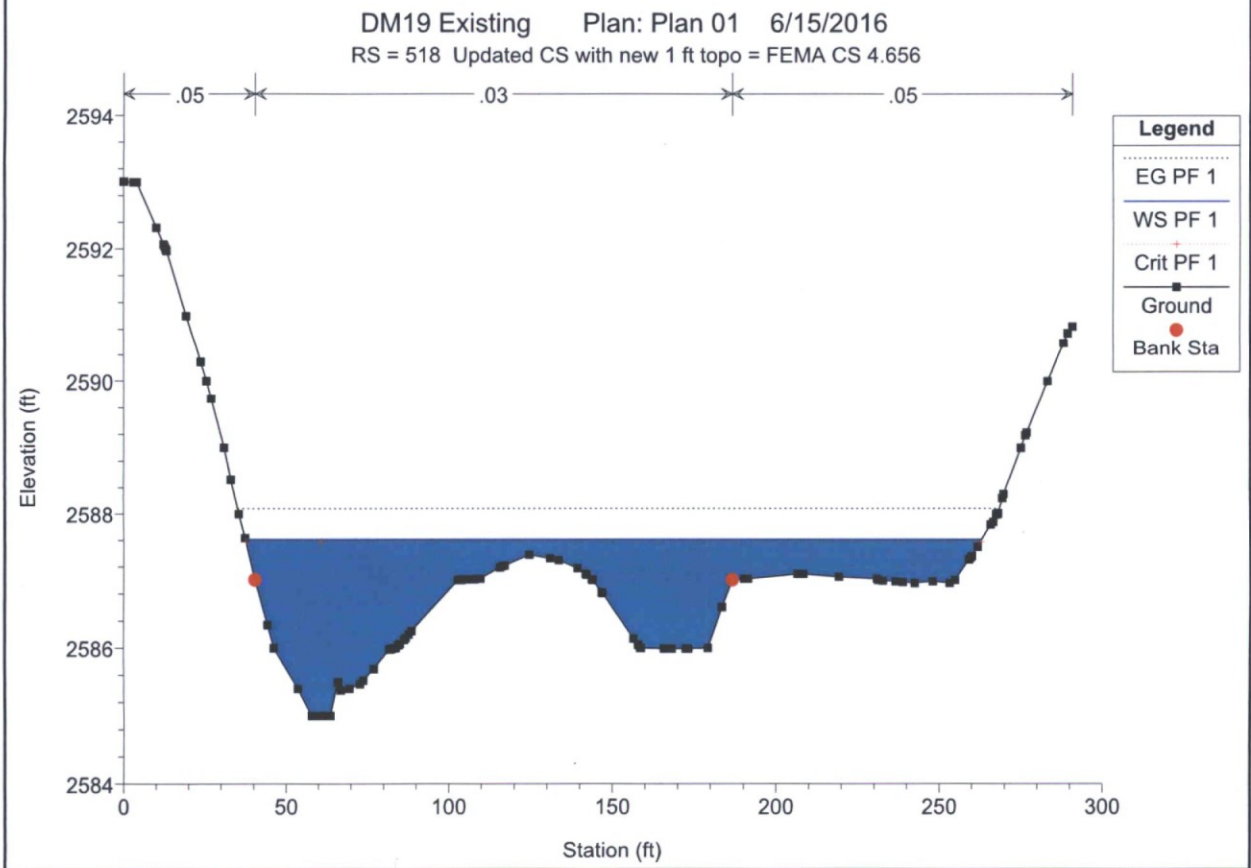
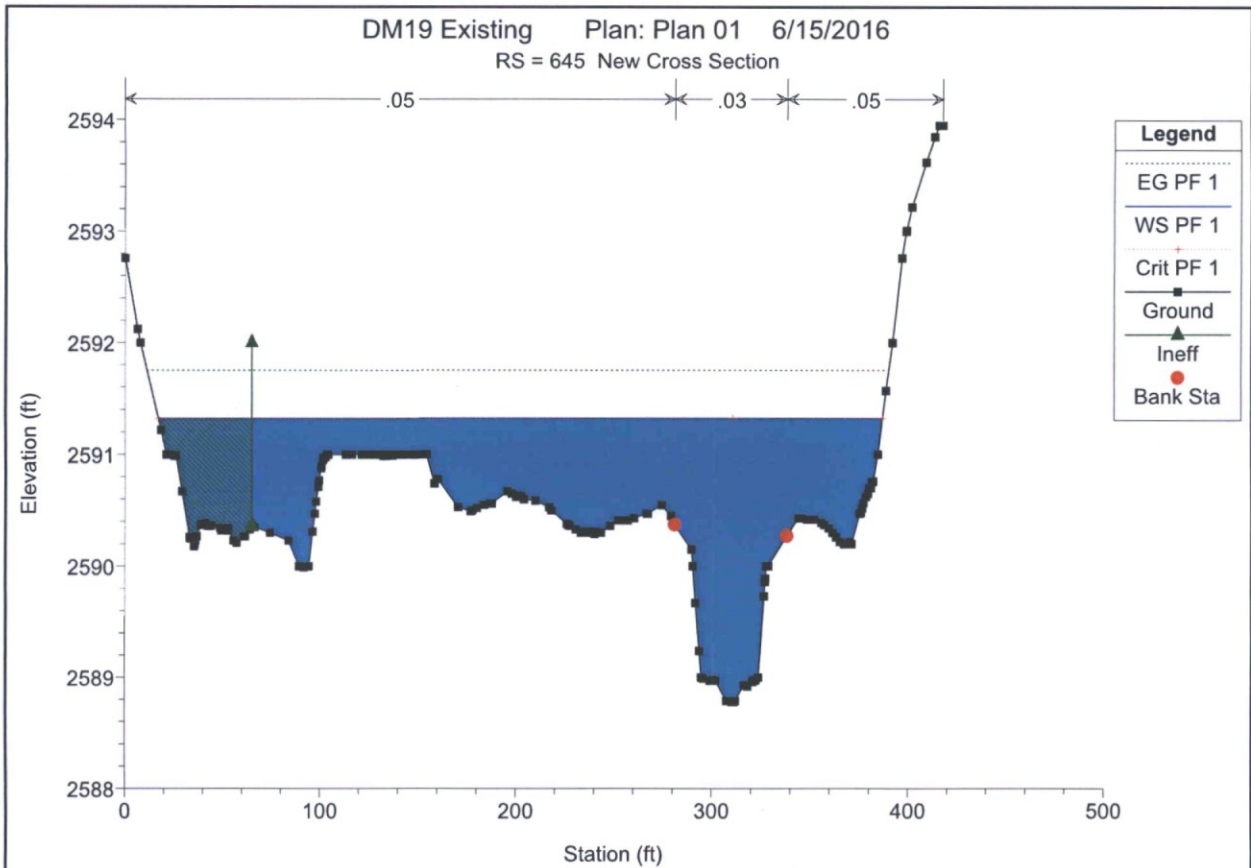




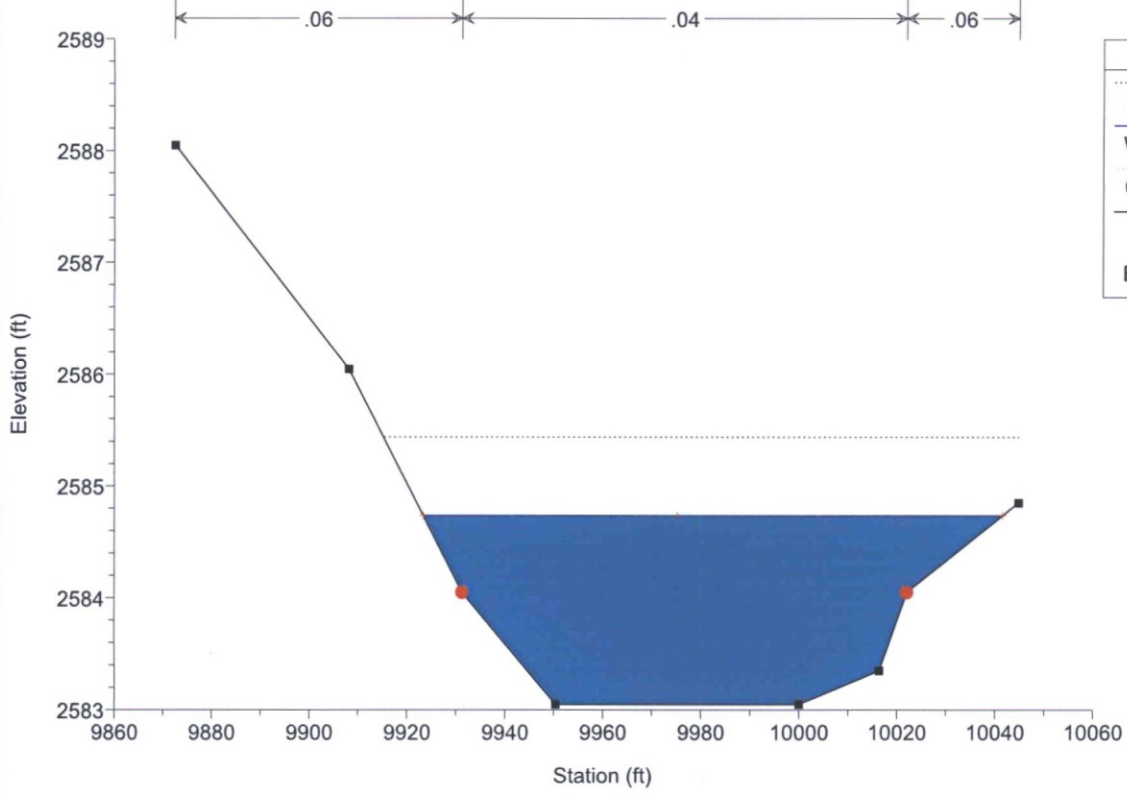






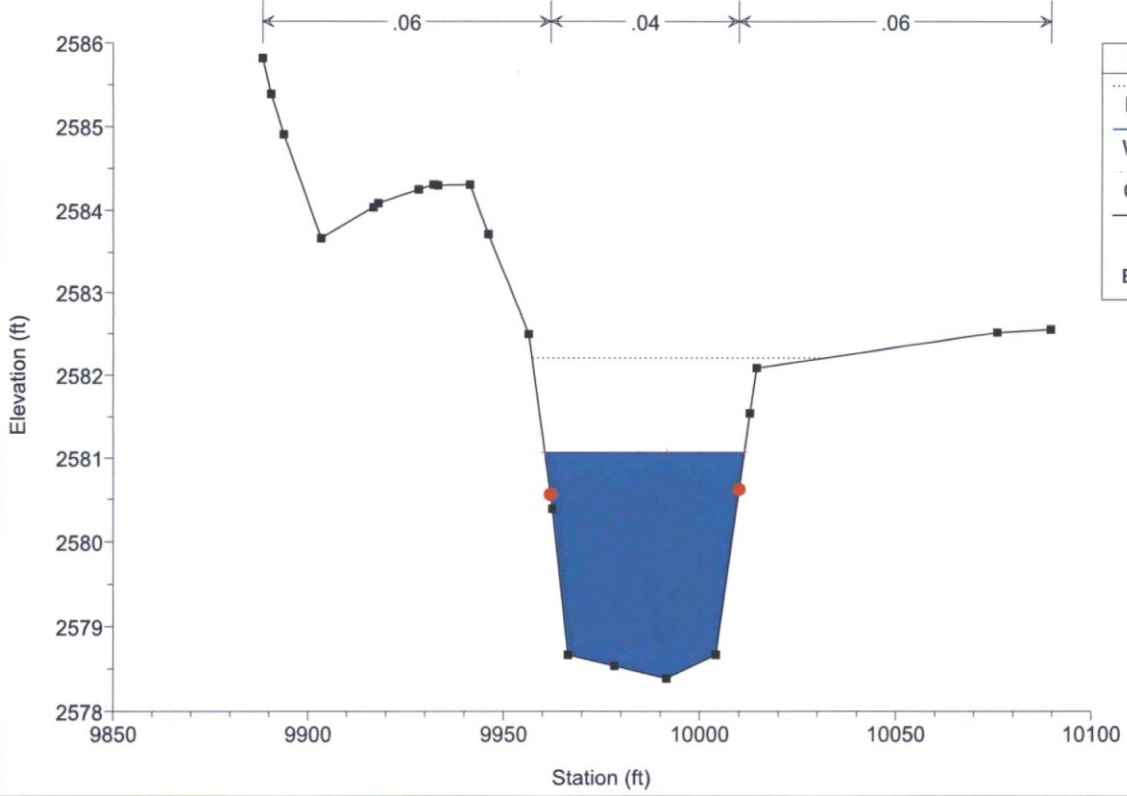


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 RS = 317 FEMA CS 4.639 - Adjust from NGVD +2.05



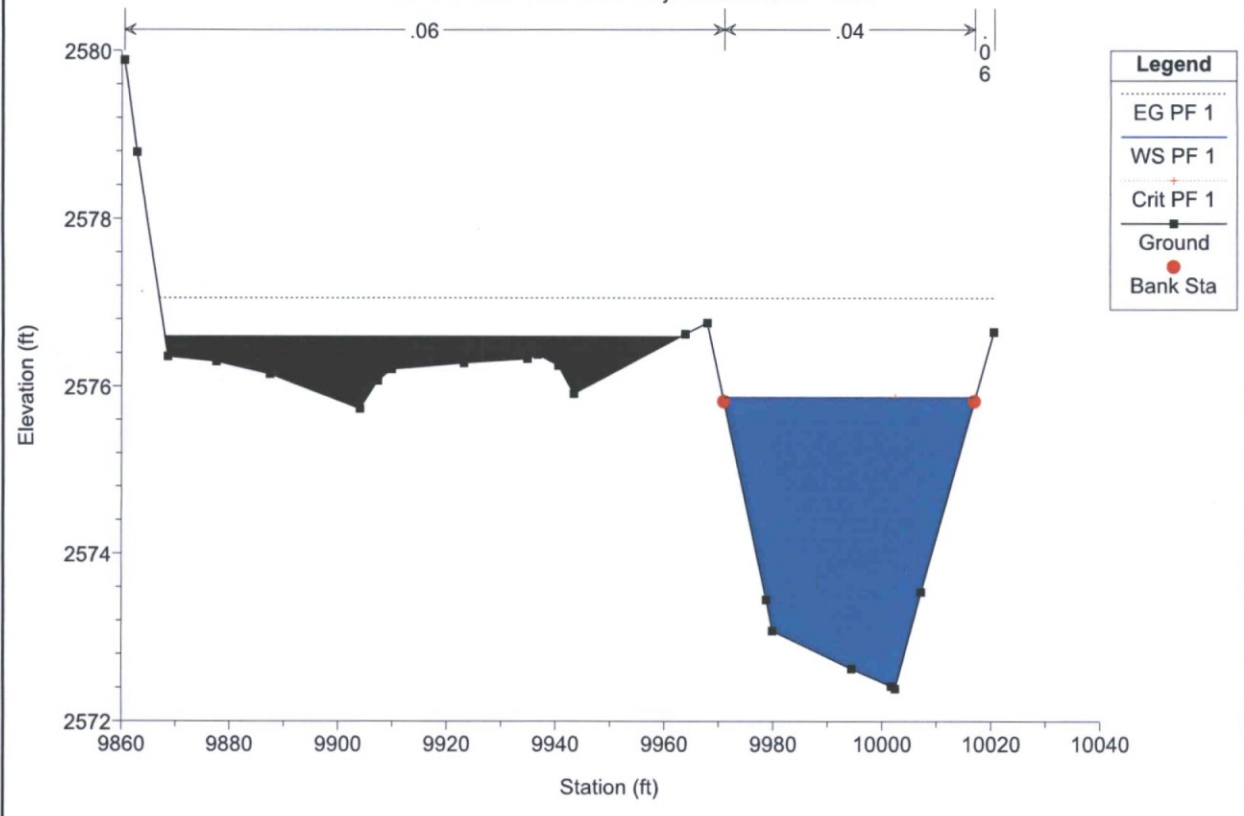
Legend	
.....	EG PF 1
————	WS PF 1
.....	Crit PF 1
■	Ground
●	Bank Sta

DM19 Existing Plan: Plan 01 6/15/2016
 RS = 197 FEMA CS 4.618- Adjust from NGVD +2.05



Legend	
.....	EG PF 1
————	WS PF 1
.....	Crit PF 1
■	Ground
●	Bank Sta

DM19 Existing Plan: Plan 01 6/15/2016
RS = 0 FEMA CS 4.581- Adjust from NGVD +2.05



HEC-RAS Plan: Plan 01 River: DM19 Reach: Profile baseline Profile: PF 1

Reach	River Sta	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Profile baseline	2774	1111.00	2641.90	2643.97	2643.97	2644.81	0.019573	7.39	150.93	92.16	1.00
Profile baseline	2678	1111.00	2639.92	2641.96	2641.96	2642.72	0.016336	7.21	176.81	140.34	0.93
Profile baseline	2578	1111.00	2634.45	2637.66	2637.66	2638.81	0.018331	8.64	128.65	56.14	1.01
Profile baseline	2540	1111.00	2633.32	2636.51	2636.47	2637.60	0.017260	8.38	132.63	58.28	0.98
Profile baseline	2500	1111.00	2633.00	2635.79	2635.79	2636.88	0.018543	8.37	132.70	61.54	1.00
Profile baseline	2405	1111.00	2630.14	2633.27	2633.24	2634.43	0.016829	8.67	130.22	56.37	0.98
Profile baseline	2250	1111.00	2626.98	2630.48	2630.48	2631.87	0.015983	9.48	119.91	52.49	0.97
Profile baseline	2130	1111.00	2624.00	2627.12	2627.12	2628.22	0.018673	8.42	132.18	65.10	1.00
Profile baseline	2056	1111.00	2623.00	2625.78	2625.78	2626.68	0.019778	7.60	146.23	82.05	1.00
Profile baseline	2014	1111.00	2622.00	2624.86	2624.86	2625.84	0.017091	8.35	143.71	73.05	0.97
Profile baseline	1952	1111.00	2620.00	2623.70	2623.70	2624.54	0.011525	8.10	169.11	117.24	0.83
Profile baseline	1891	1111.00	2619.00	2621.51	2621.51	2622.27	0.021132	6.97	159.31	107.40	1.01
Profile baseline	1791	1111.00	2617.00	2619.46	2619.46	2620.22	0.019554	6.99	163.30	116.96	0.98
Profile baseline	1705	1111.00	2615.00	2617.47	2617.47	2618.05	0.013724	6.32	217.73	225.56	0.84
Profile baseline	1583	1119.00	2612.00	2614.11	2614.11	2614.60	0.025216	5.58	200.71	222.84	1.02
Profile baseline	1447	1119.00	2608.94	2610.91	2610.91	2611.41	0.008431	6.36	261.67	281.66	0.87
Profile baseline	1257	1119.00	2603.99	2606.23	2606.23	2606.83	0.006824	6.57	226.00	256.00	0.81
Profile baseline	1119	1119.00	2600.92	2603.47	2603.47	2604.24	0.008223	7.49	202.58	215.74	0.90
Profile baseline	918	1119.00	2595.92	2598.49	2598.49	2599.01	0.007552	6.18	246.50	314.23	0.83
Profile baseline	716	1124.00	2591.19	2593.78	2593.78	2594.27	0.009506	6.27	254.38	276.24	0.91
Profile baseline	645	1124.00	2588.78	2591.32	2591.32	2591.75	0.007358	6.47	308.55	369.92	0.83
Profile baseline	518	1124.00	2585.00	2587.61	2587.58	2588.08	0.009846	5.69	226.02	225.58	0.90
Profile baseline	317	947.00	2583.05	2584.74	2584.74	2585.44	0.019229	6.79	146.67	118.24	0.97
Profile baseline	197	947.00	2578.39	2581.07	2581.07	2582.21	0.017926	8.58	110.98	50.82	1.00
Profile baseline	0	947.00	2572.39	2575.87	2575.87	2577.05	0.017992	8.73	108.53	46.37	1.00

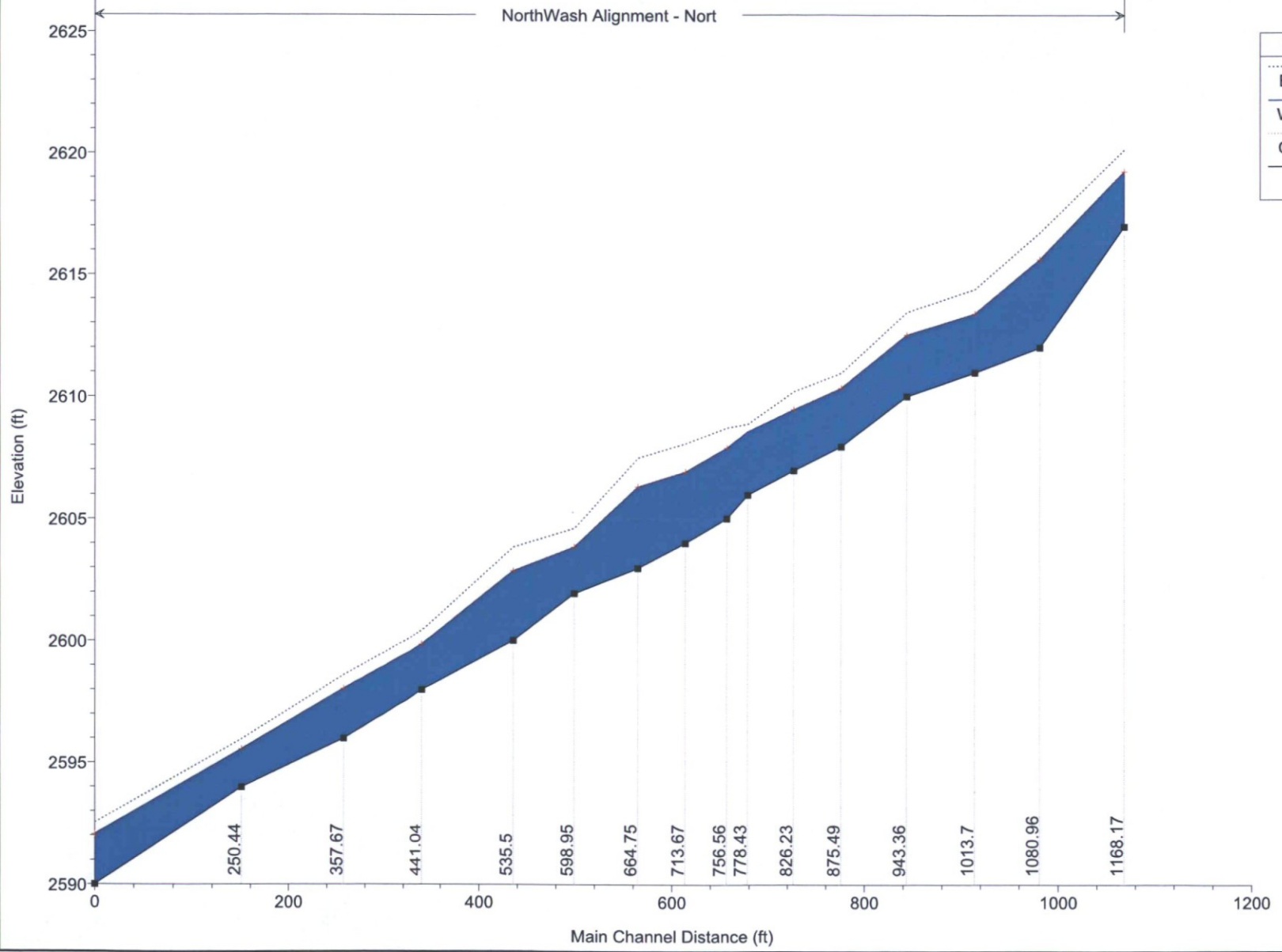
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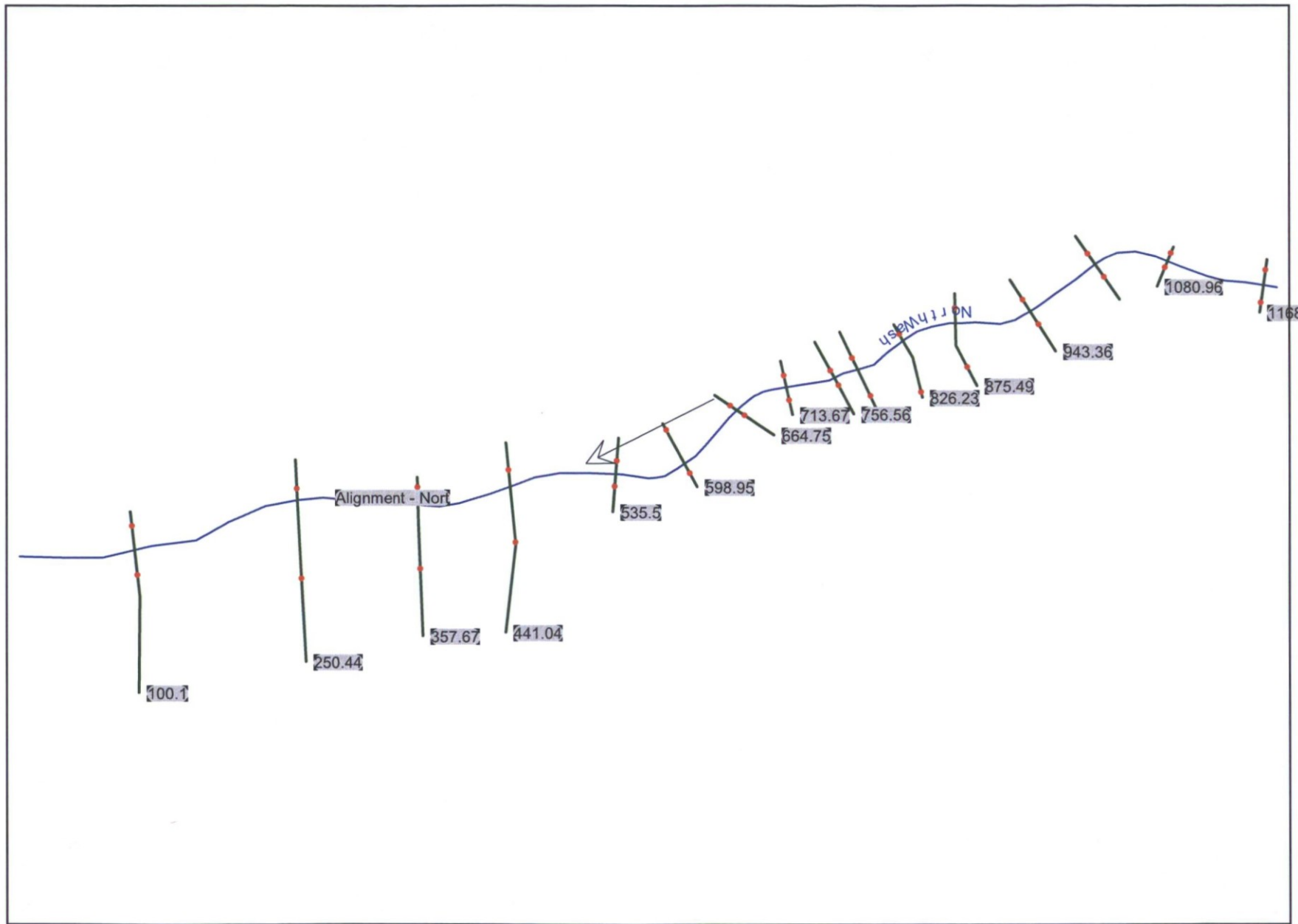
WASH A

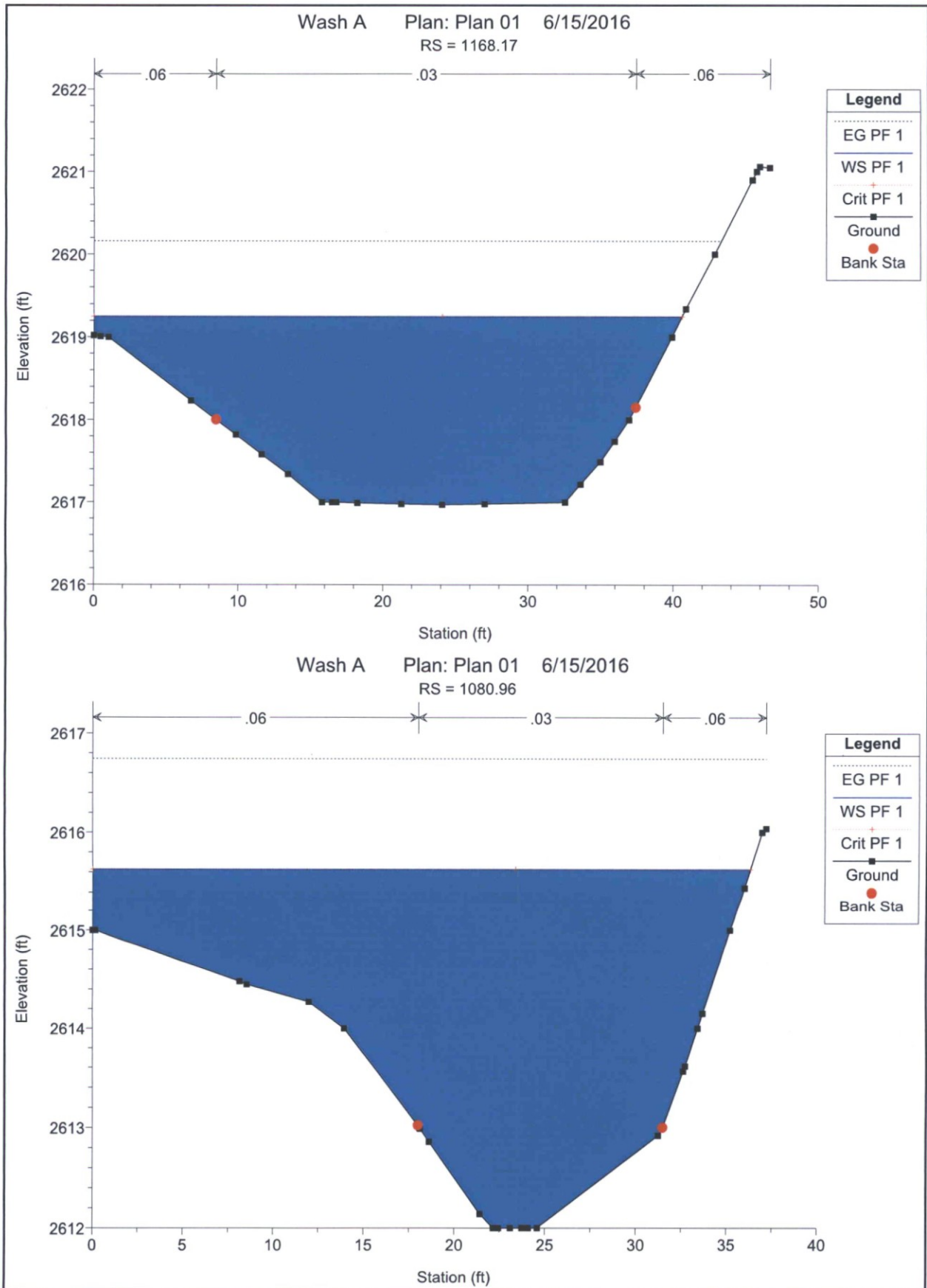
Wash A Plan: Plan 01 6/15/2016

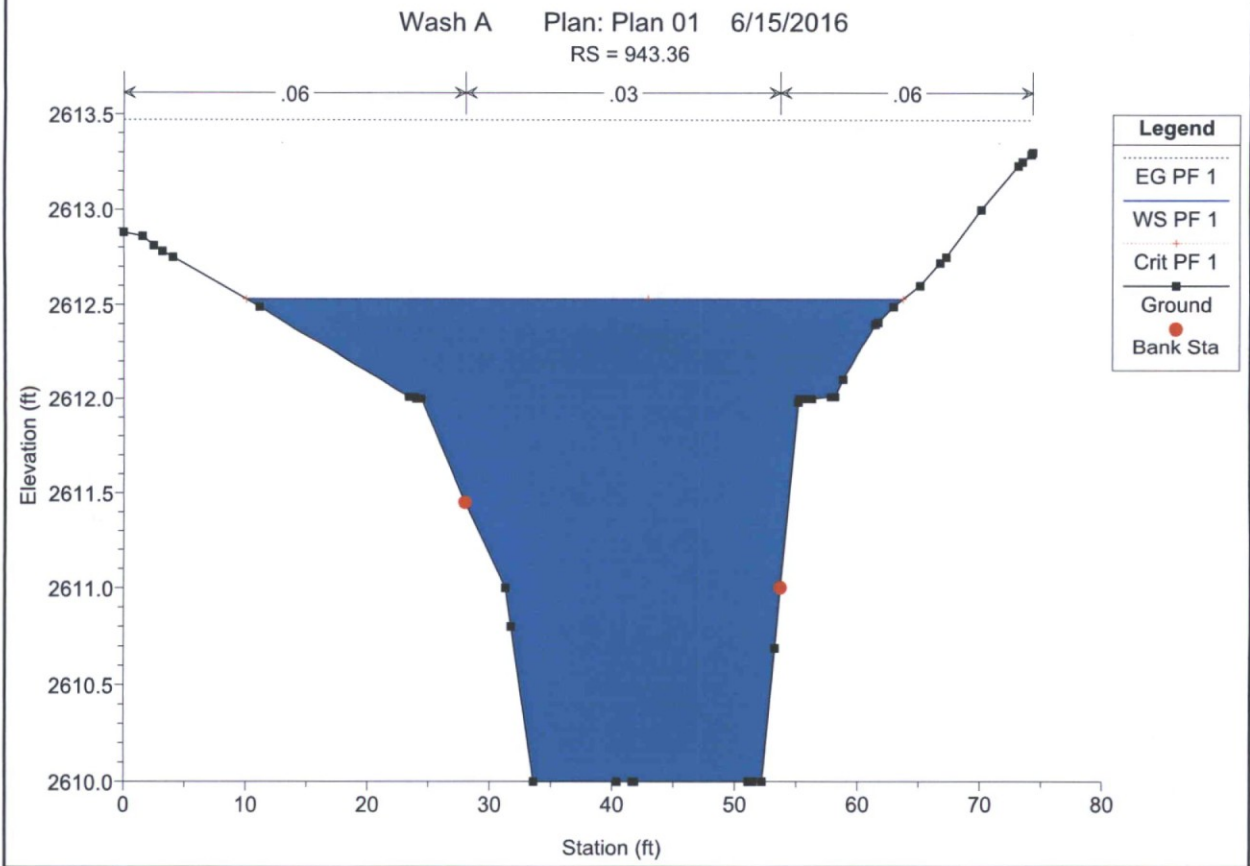
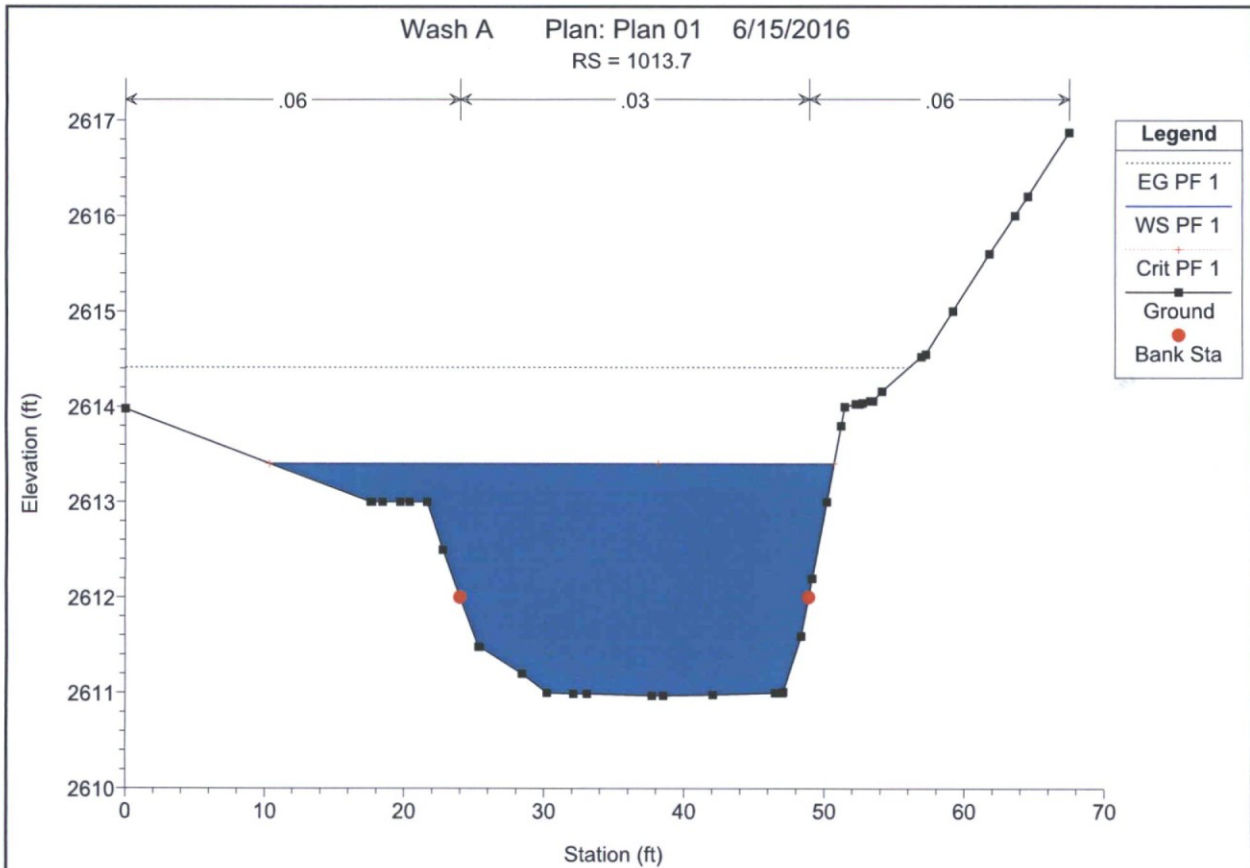
NorthWash Alignment - Nort

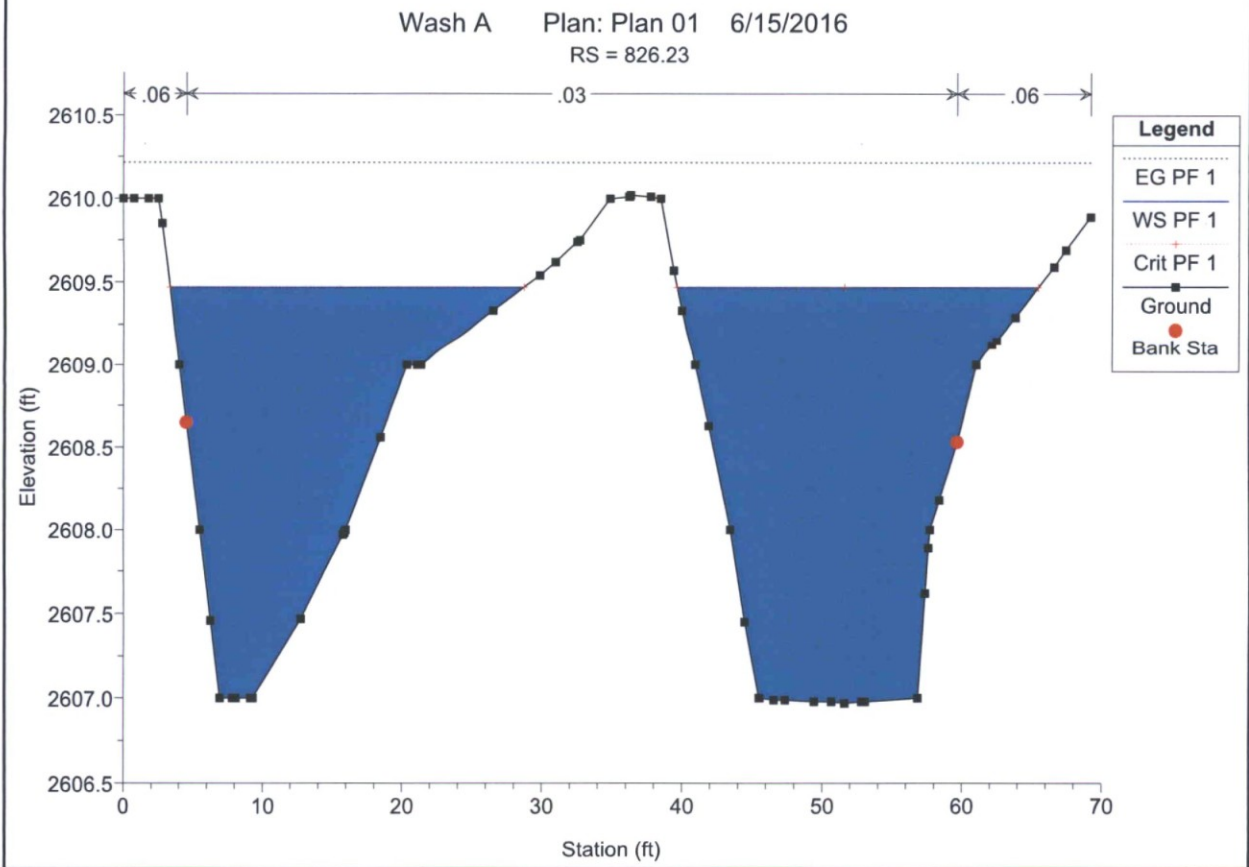
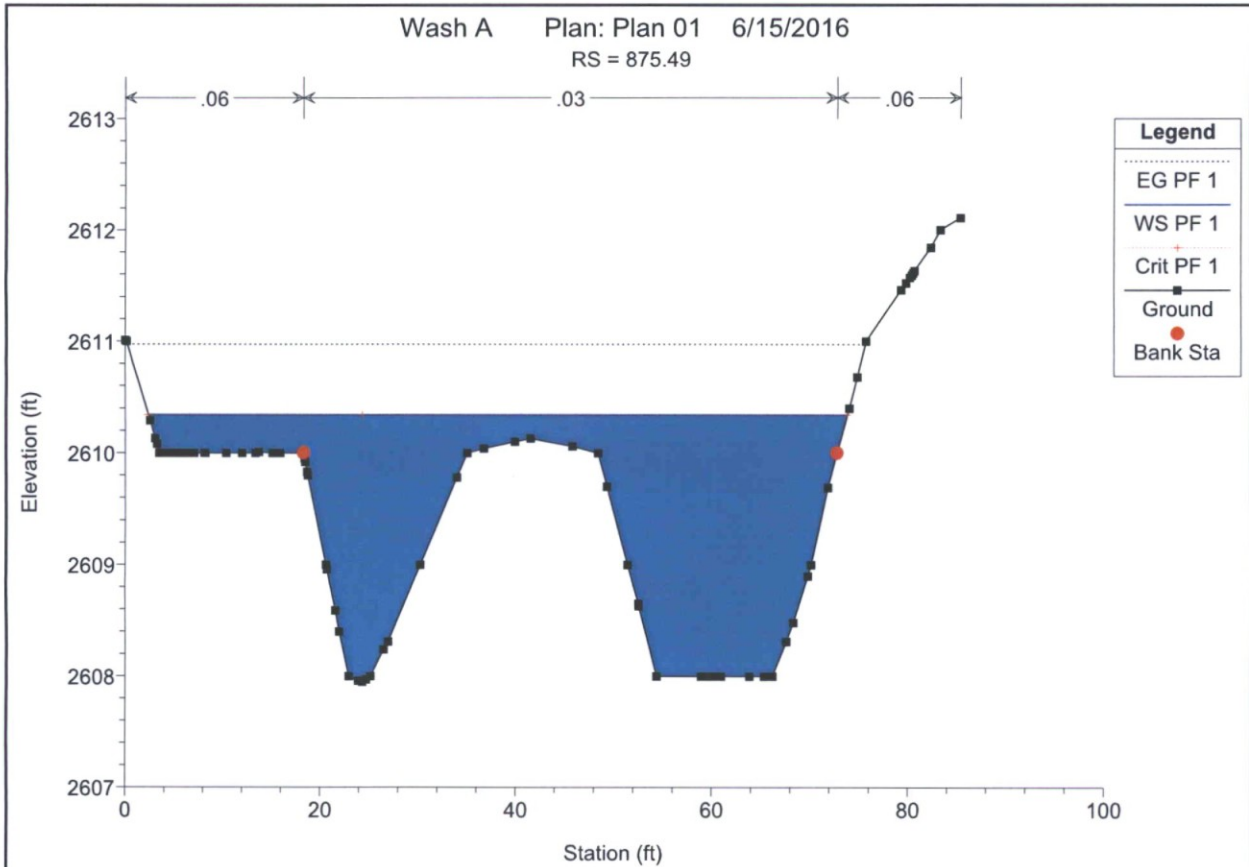
Legend	
EG PF 1
WS PF 1	————
Crit PF 1
Ground	■

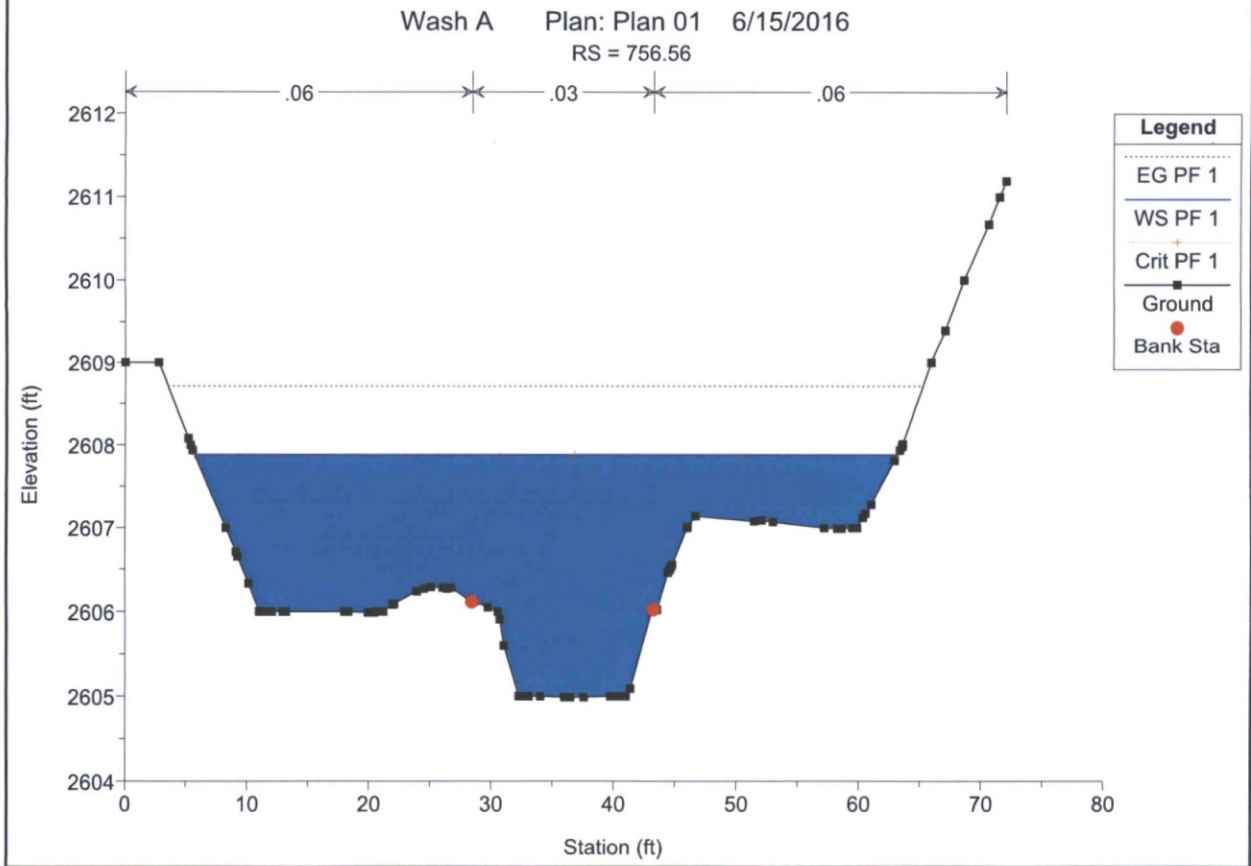
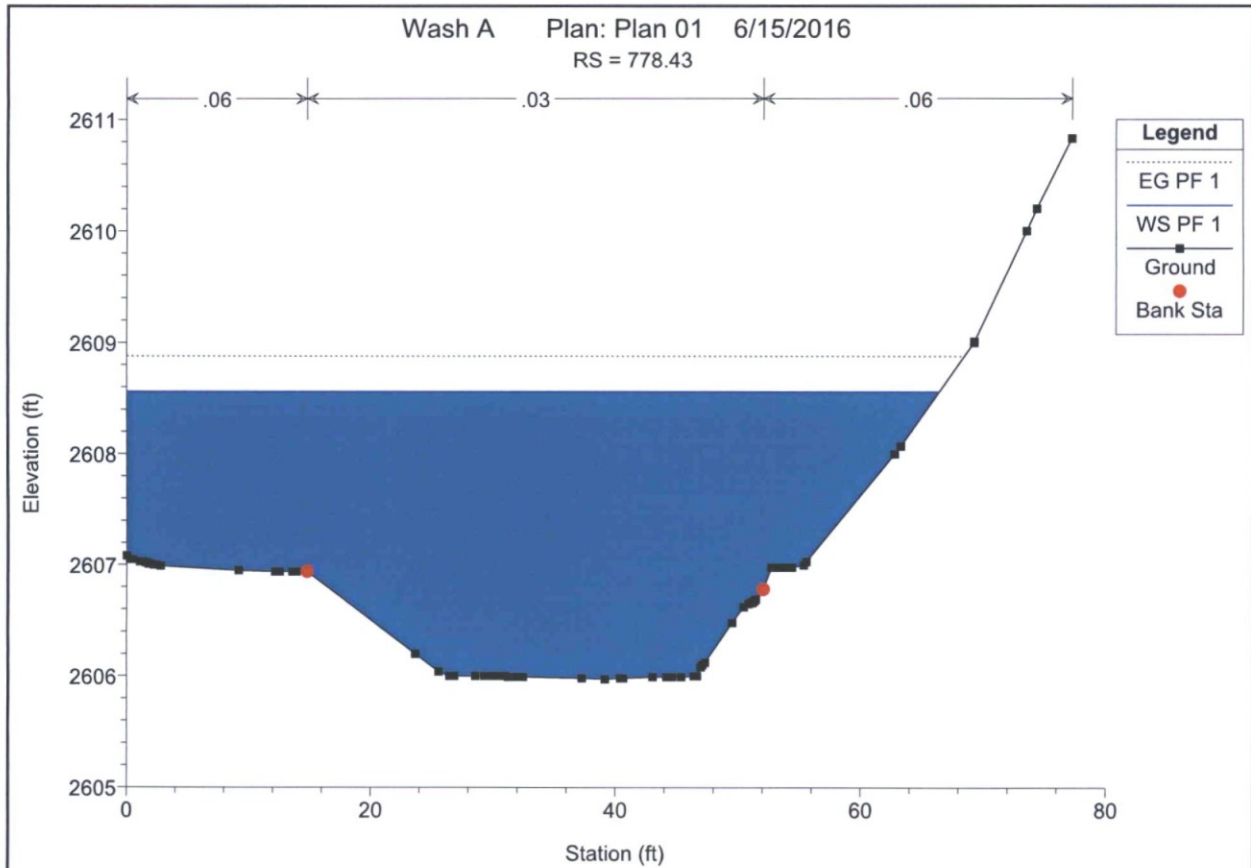


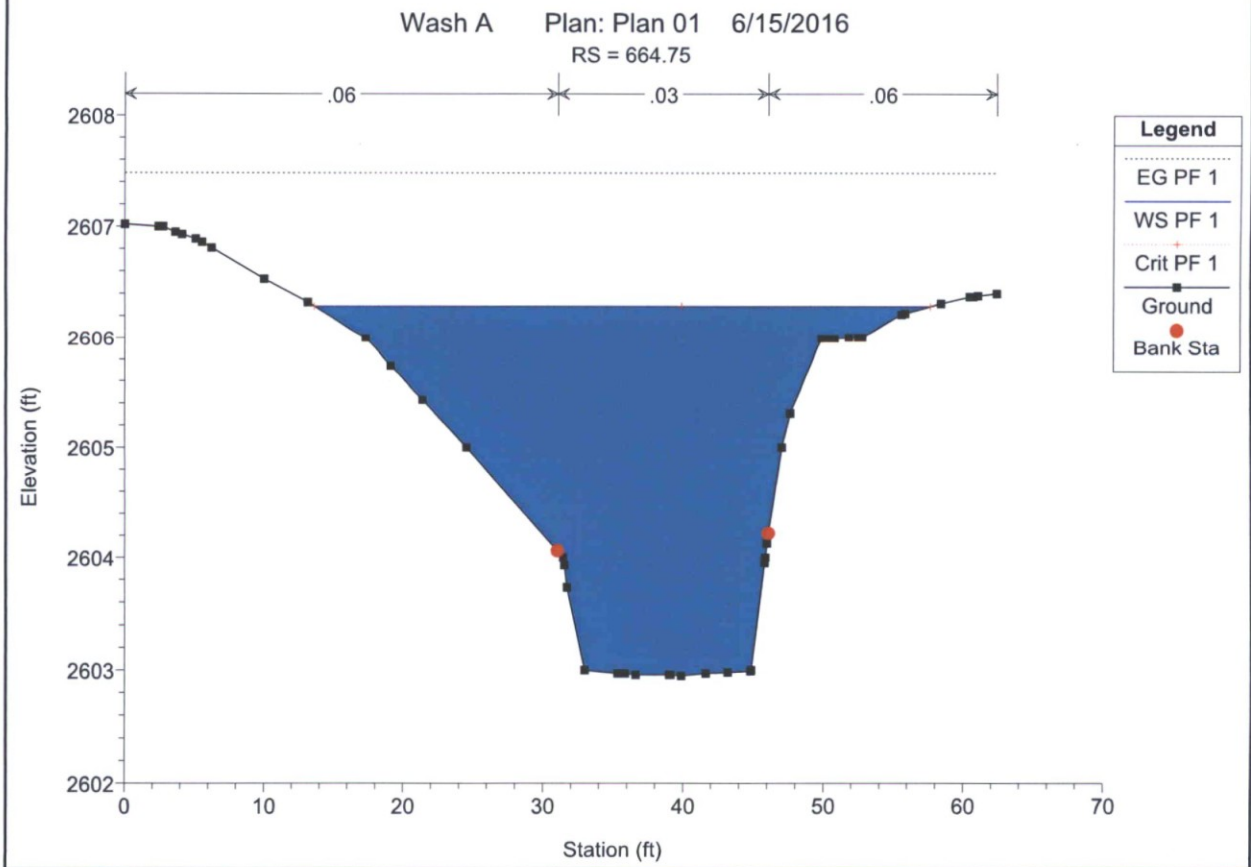
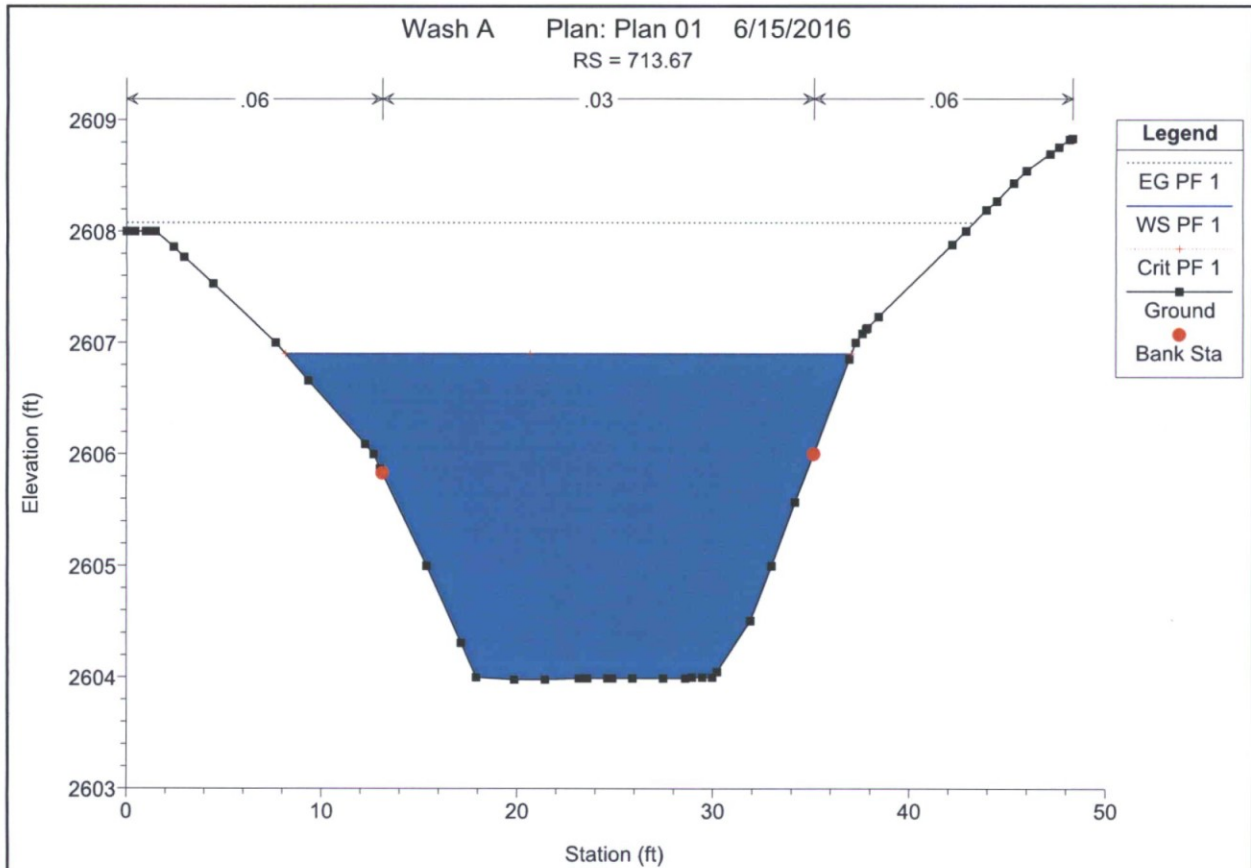


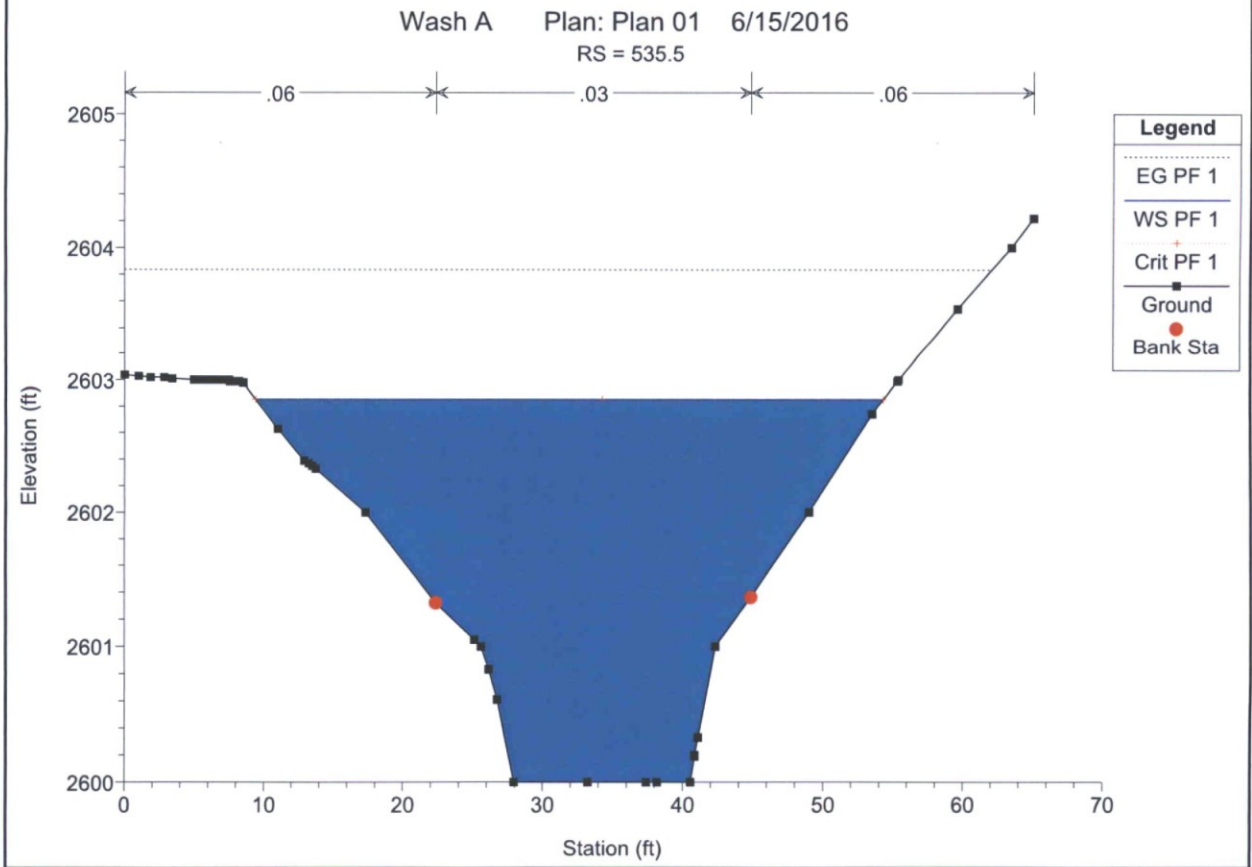
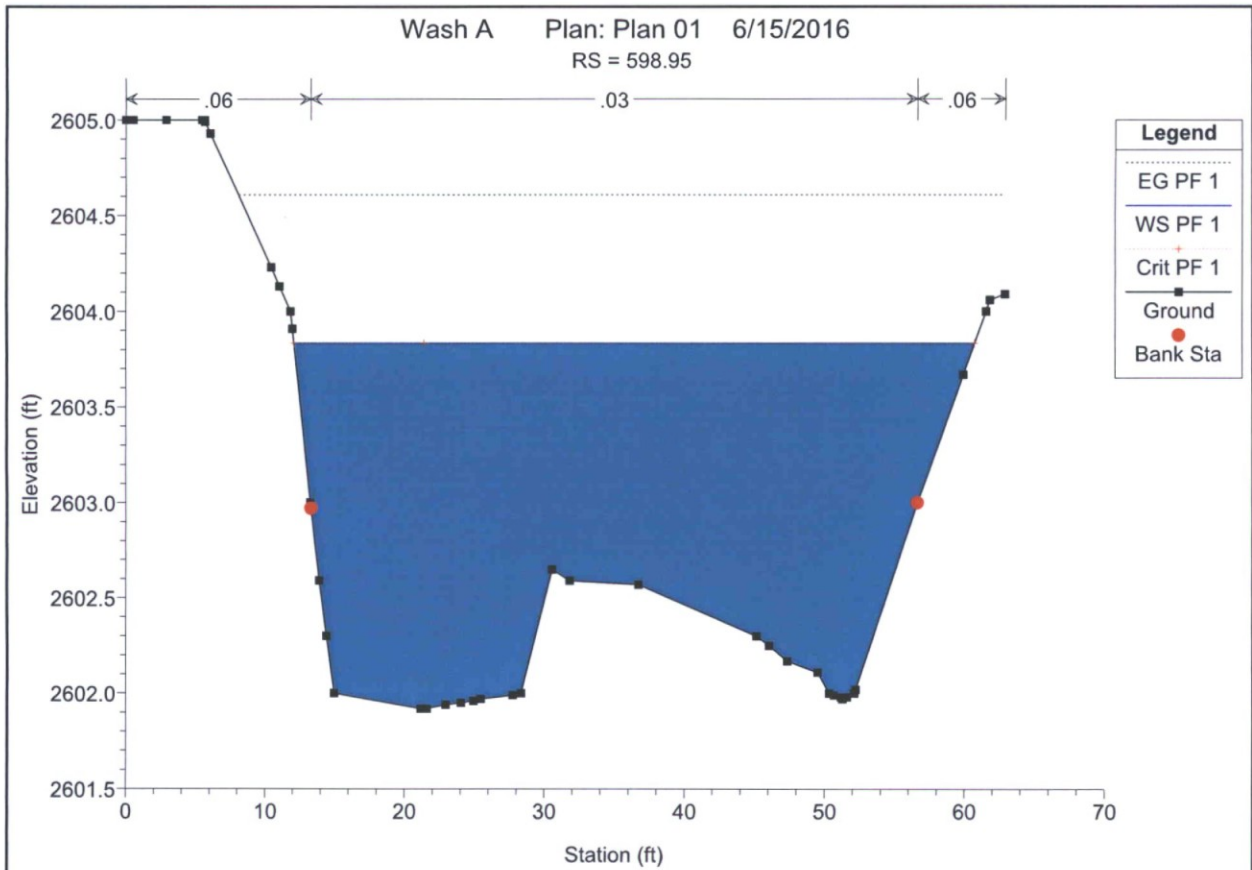


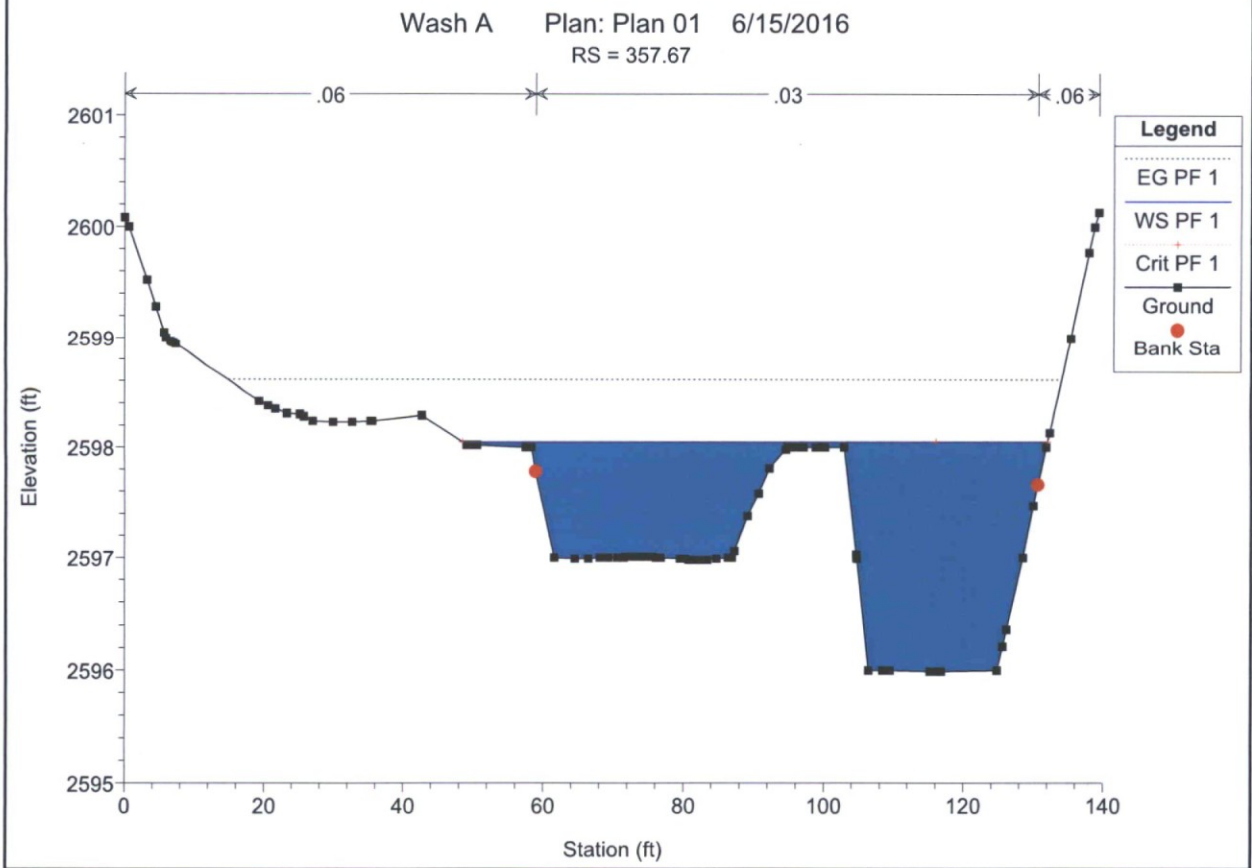
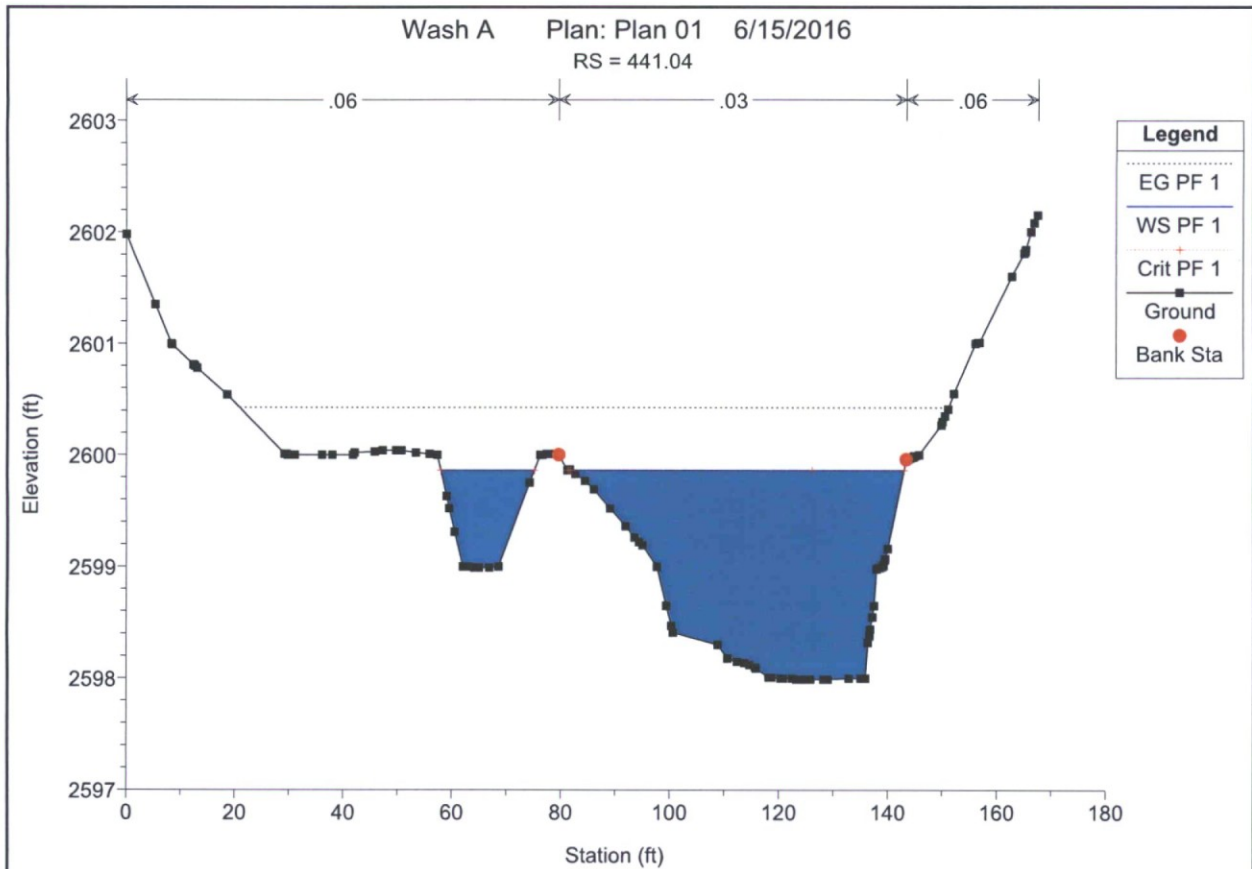


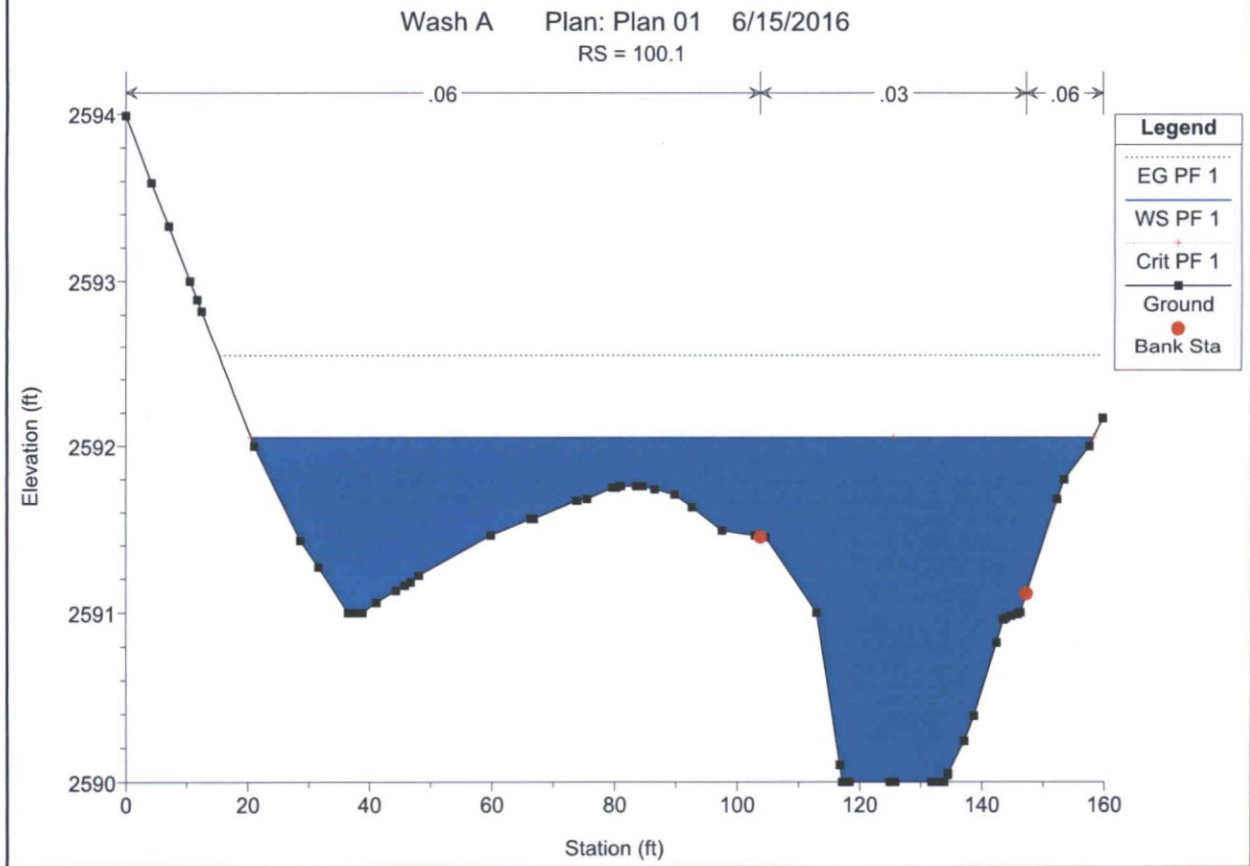
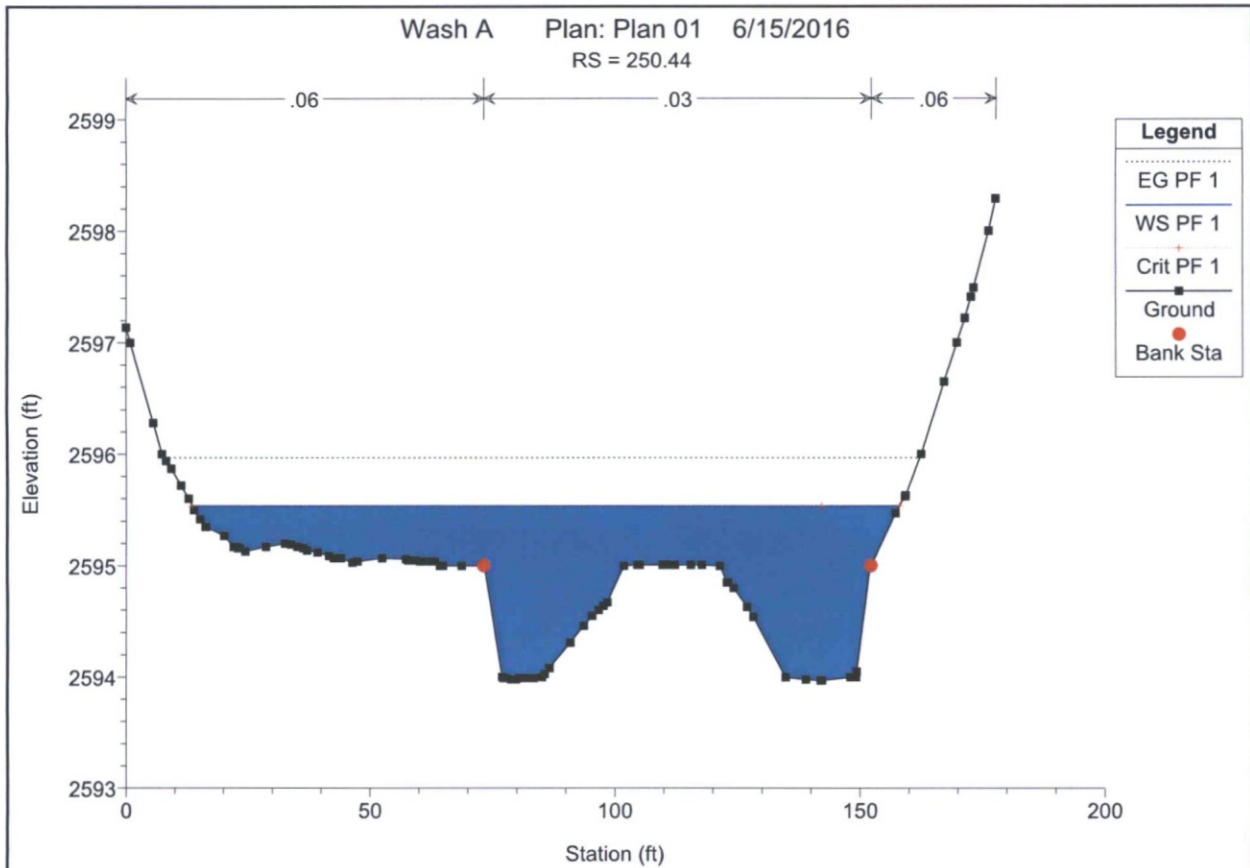












HEC-RAS Plan: Plan 01 River: NorthWash Reach: Alignment - Nort Profile: PF 1

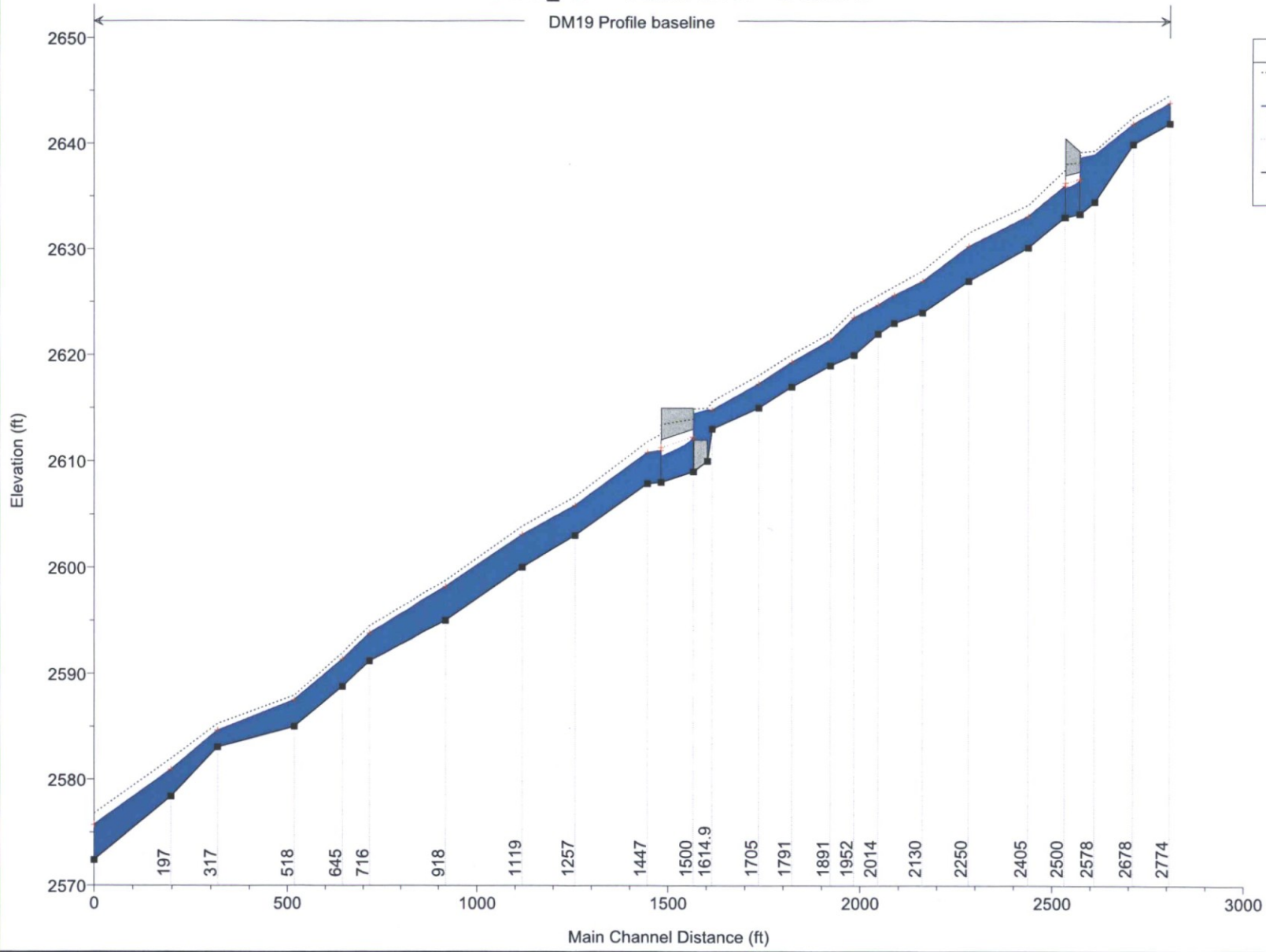
Reach	River Sta	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Alignment - Nort	1168.17	472.00	2616.97	2619.25	2619.25	2620.16	0.009615	7.77	66.58	40.60	0.96
Alignment - Nort	1080.96	472.00	2612.00	2615.63	2615.63	2616.74	0.007289	9.15	73.61	36.33	0.90
Alignment - Nort	1013.7	472.00	2610.97	2613.40	2613.40	2614.41	0.009090	8.13	63.57	40.35	0.95
Alignment - Nort	943.36	480.00	2610.00	2612.53	2612.53	2613.47	0.008585	7.89	70.29	53.67	0.92
Alignment - Nort	875.49	480.00	2607.95	2610.34	2610.34	2610.97	0.011524	6.41	79.21	71.47	0.97
Alignment - Nort	826.23	480.00	2606.97	2609.47	2609.47	2610.22	0.011584	6.96	71.02	51.19	0.98
Alignment - Nort	778.43	480.00	2605.97	2608.56		2608.88	0.002981	4.79	125.60	66.45	0.55
Alignment - Nort	756.56	480.00	2604.99	2607.89	2607.89	2608.72	0.008770	8.59	91.77	57.49	0.94
Alignment - Nort	713.67	485.00	2603.98	2606.90	2606.90	2608.08	0.009615	8.74	58.25	28.88	0.98
Alignment - Nort	664.75	485.00	2602.95	2606.28	2606.28	2607.48	0.007889	9.19	70.56	44.05	0.91
Alignment - Nort	598.95	485.00	2601.92	2603.83	2603.83	2604.61	0.011333	7.08	70.34	48.65	0.99
Alignment - Nort	535.5	485.00	2600.00	2602.85	2602.85	2603.84	0.008559	8.21	72.00	44.88	0.92
Alignment - Nort	441.04	490.00	2597.99	2599.86	2599.86	2600.43	0.011514	6.13	87.05	78.67	0.97
Alignment - Nort	357.67	490.00	2595.99	2598.05	2598.05	2598.61	0.012877	6.04	81.71	83.58	1.00
Alignment - Nort	250.44	495.00	2593.97	2595.54	2595.54	2595.97	0.011294	5.46	110.14	144.51	0.93
Alignment - Nort	100.1	495.00	2590.00	2592.05	2592.05	2592.55	0.008482	6.12	119.82	137.73	0.86

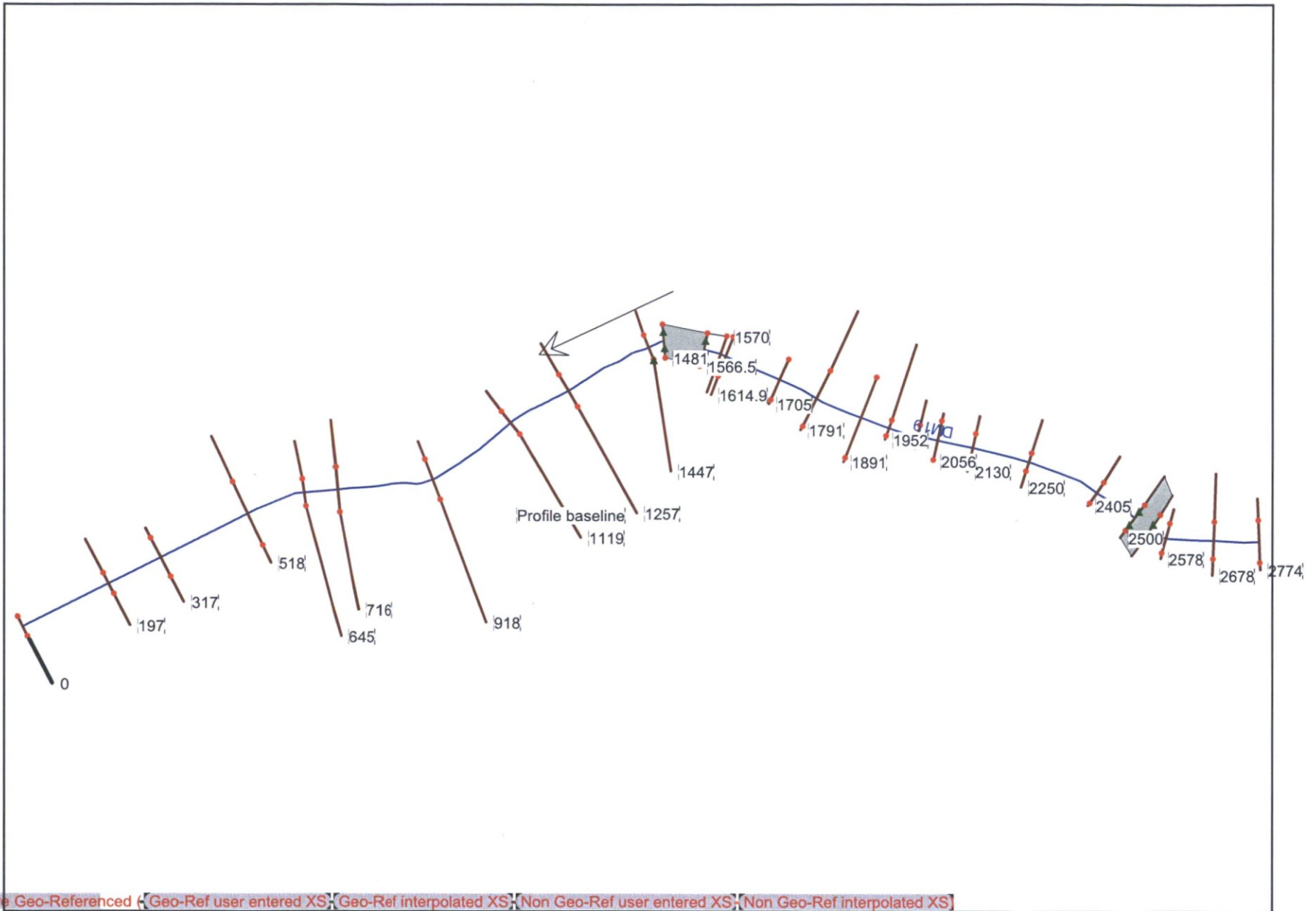
POST-DEVELOPMENT

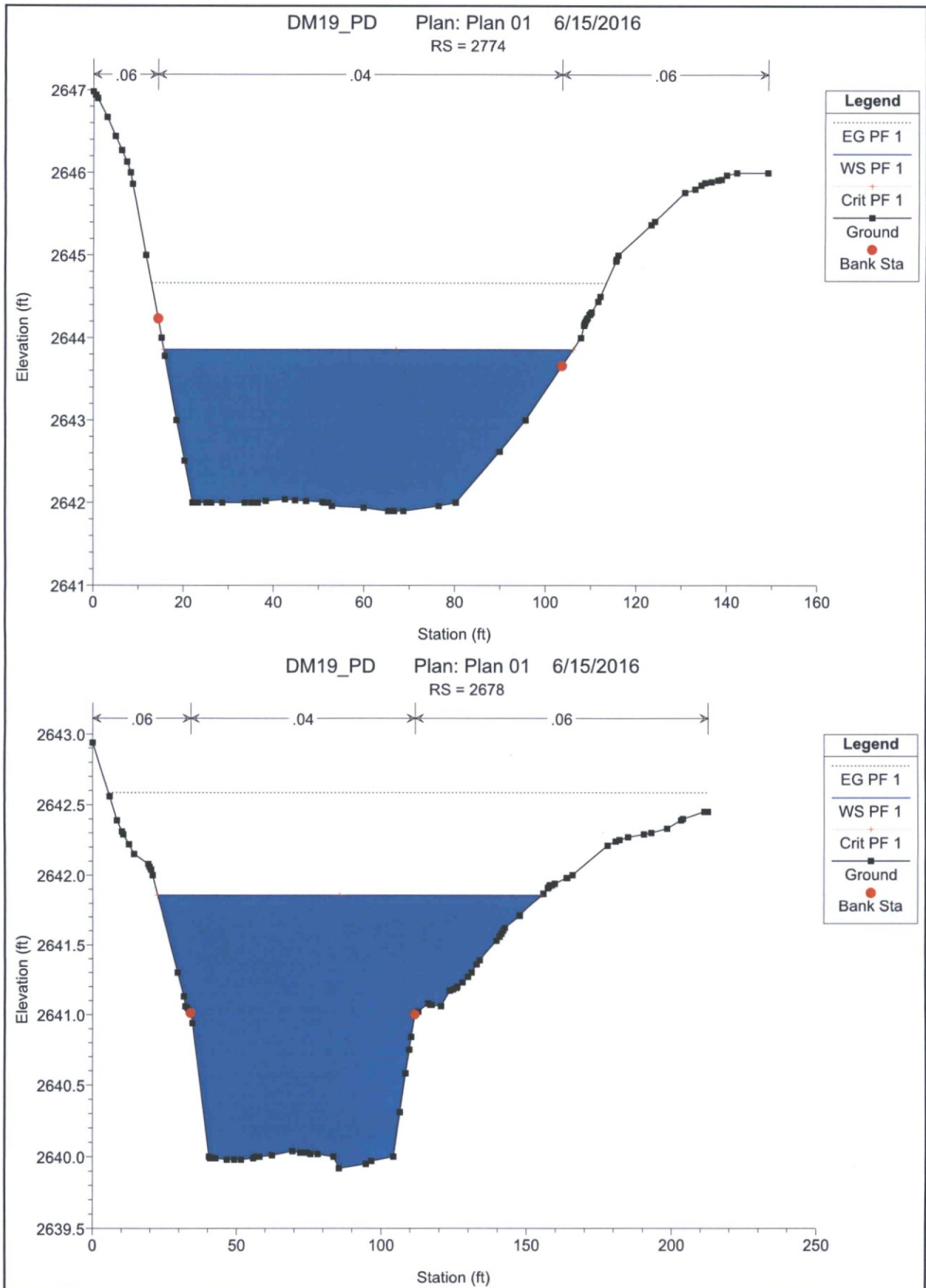
GALLOWAY WASH

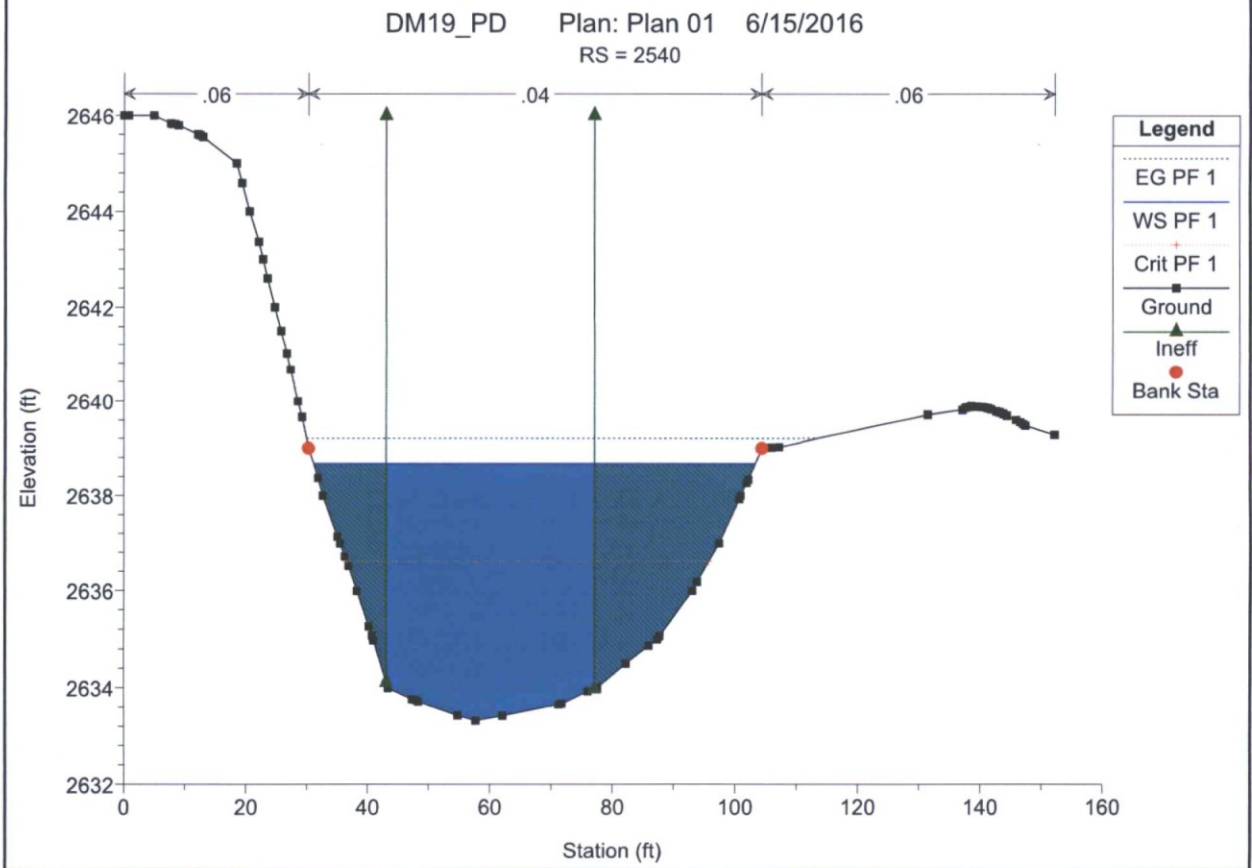
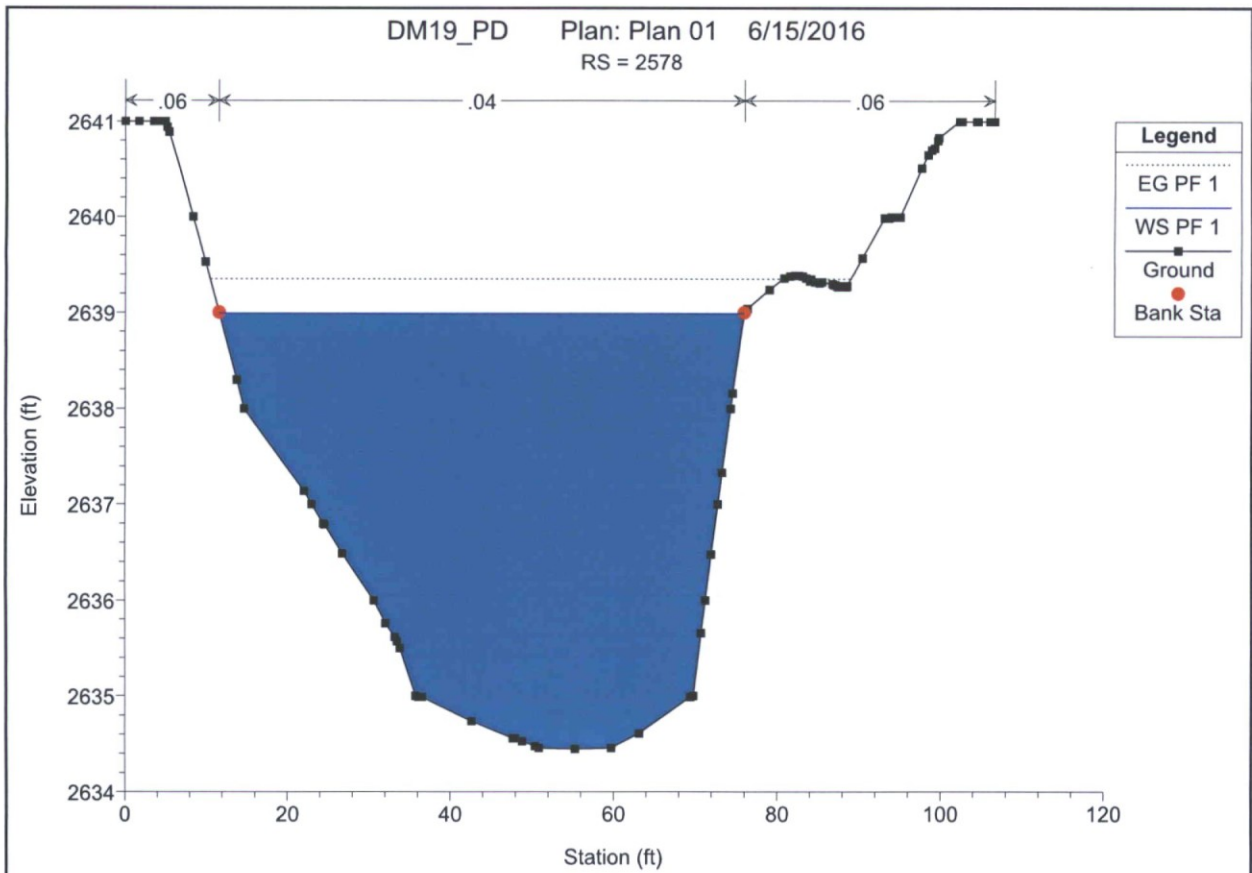
DM19 Profile baseline

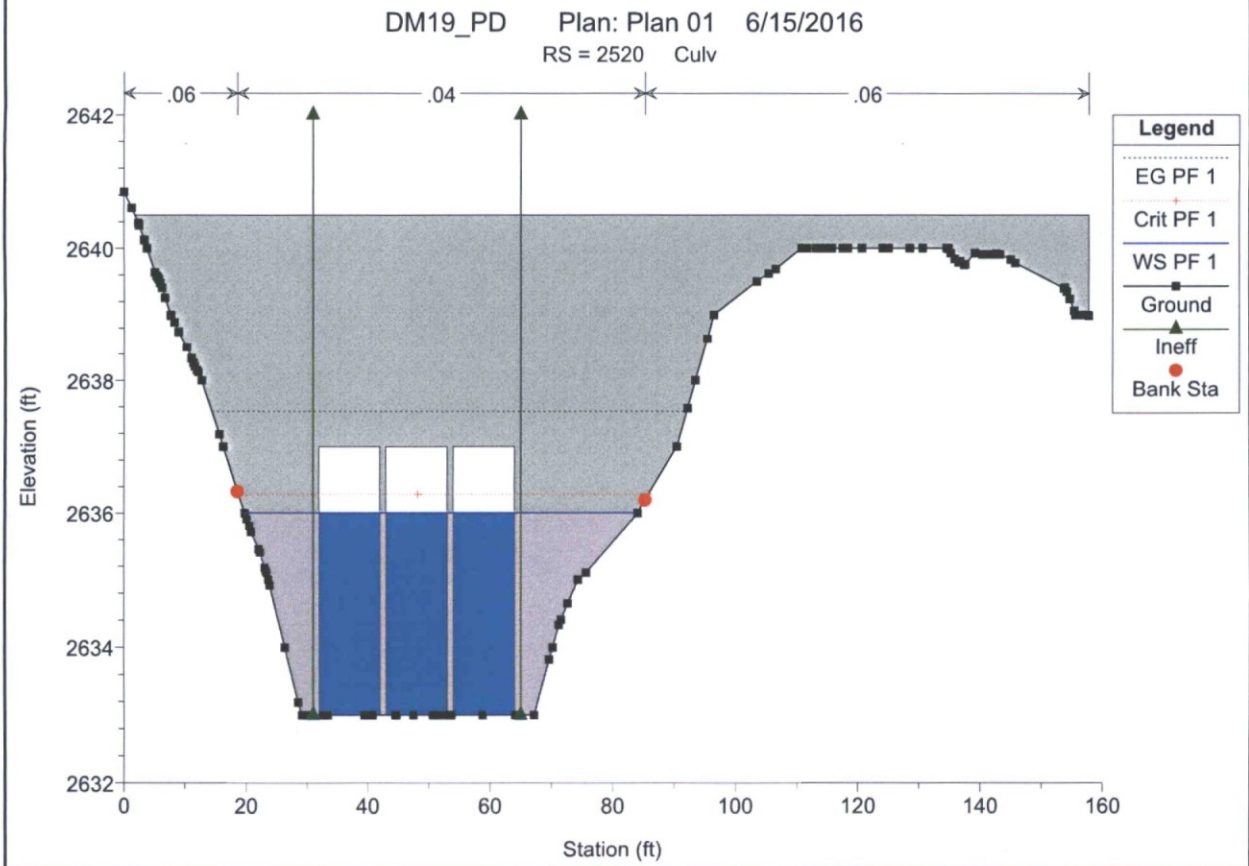
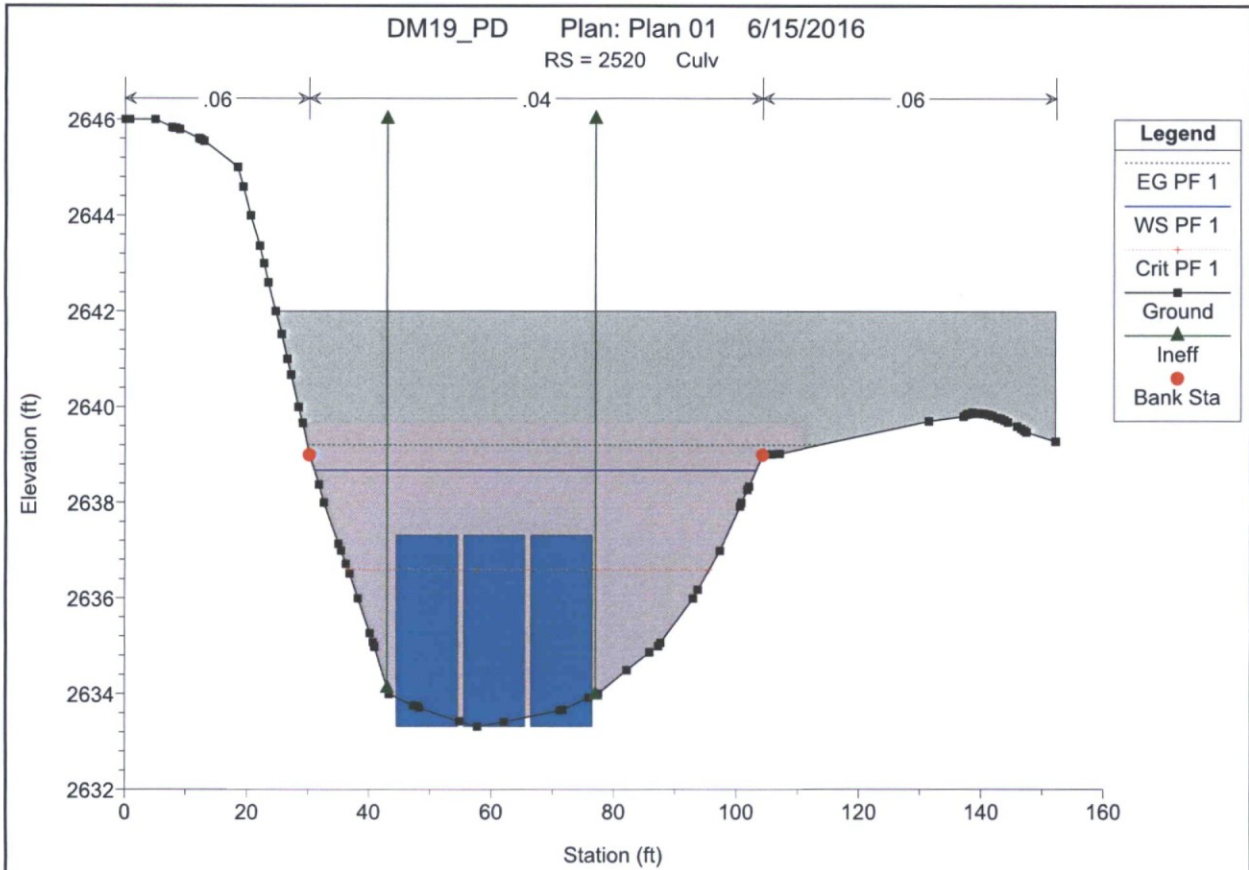
Legend	
EG PF 1
WS PF 1	————
Crit PF 1
Ground	■

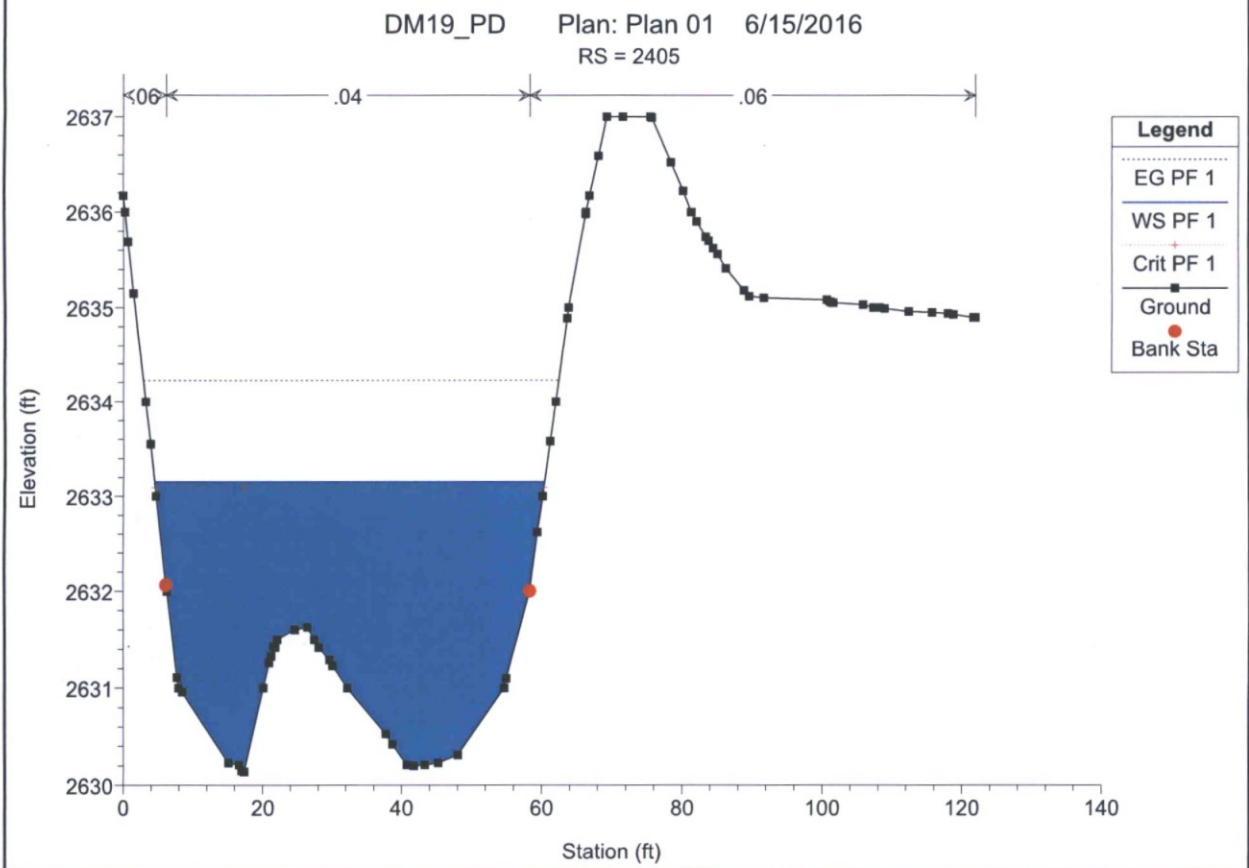
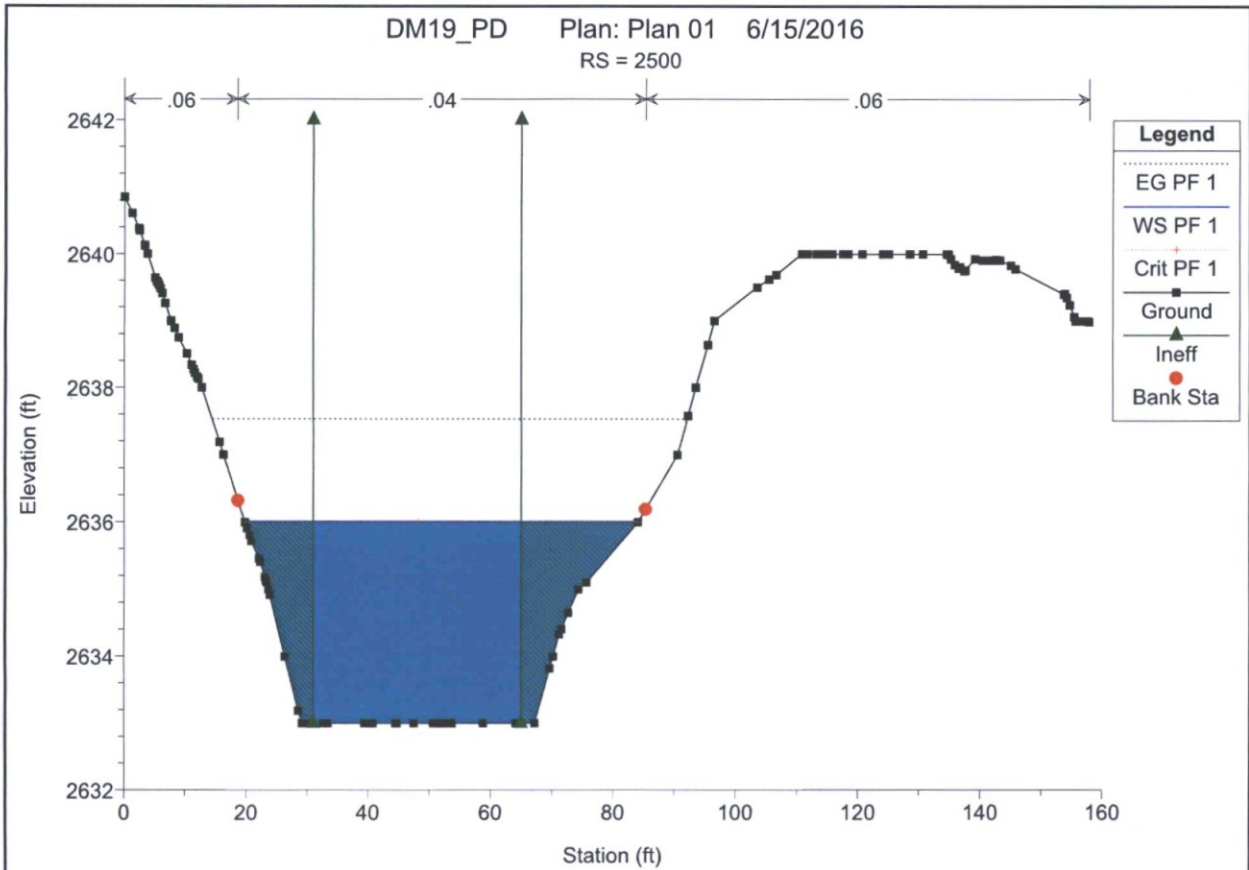


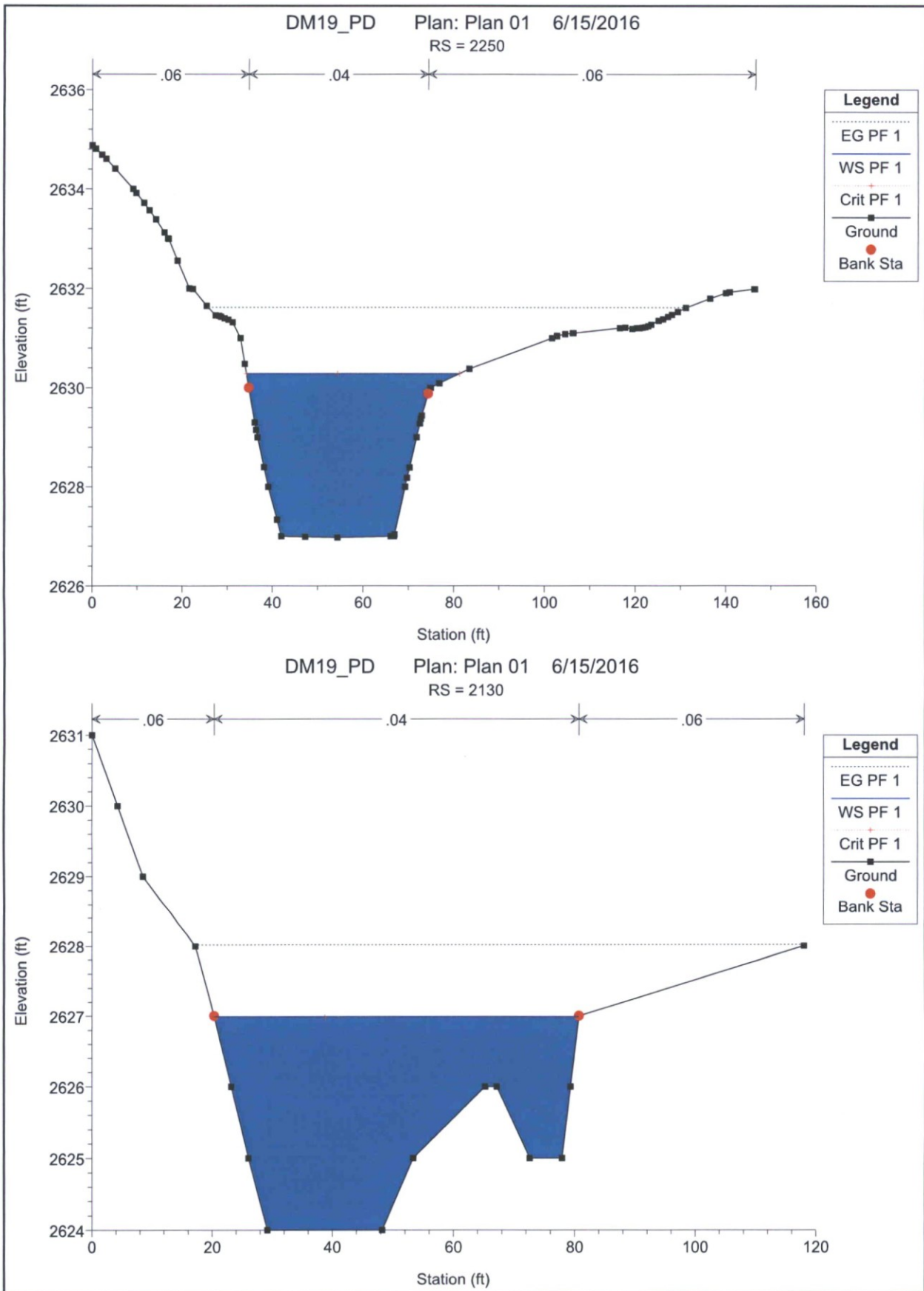


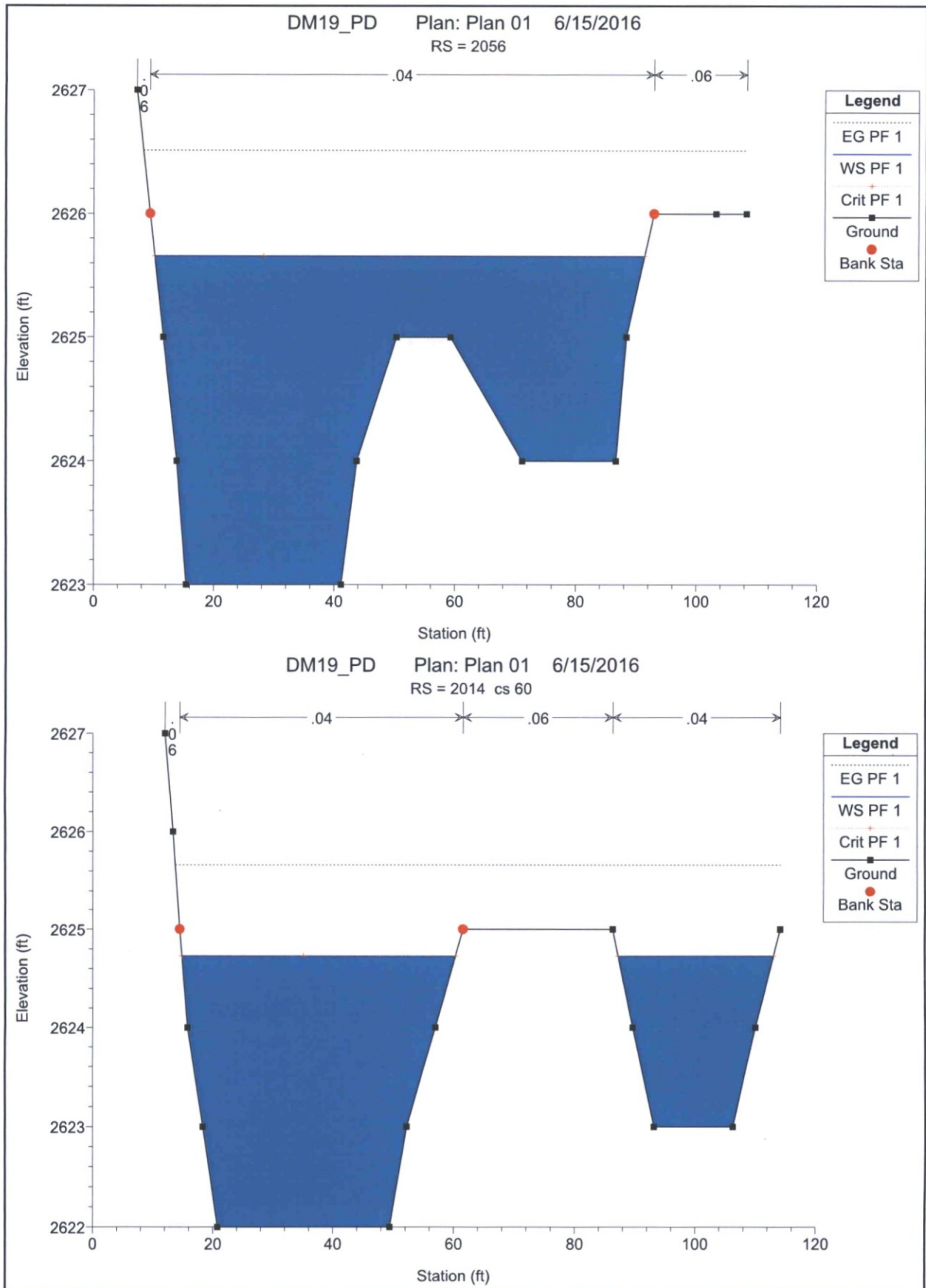


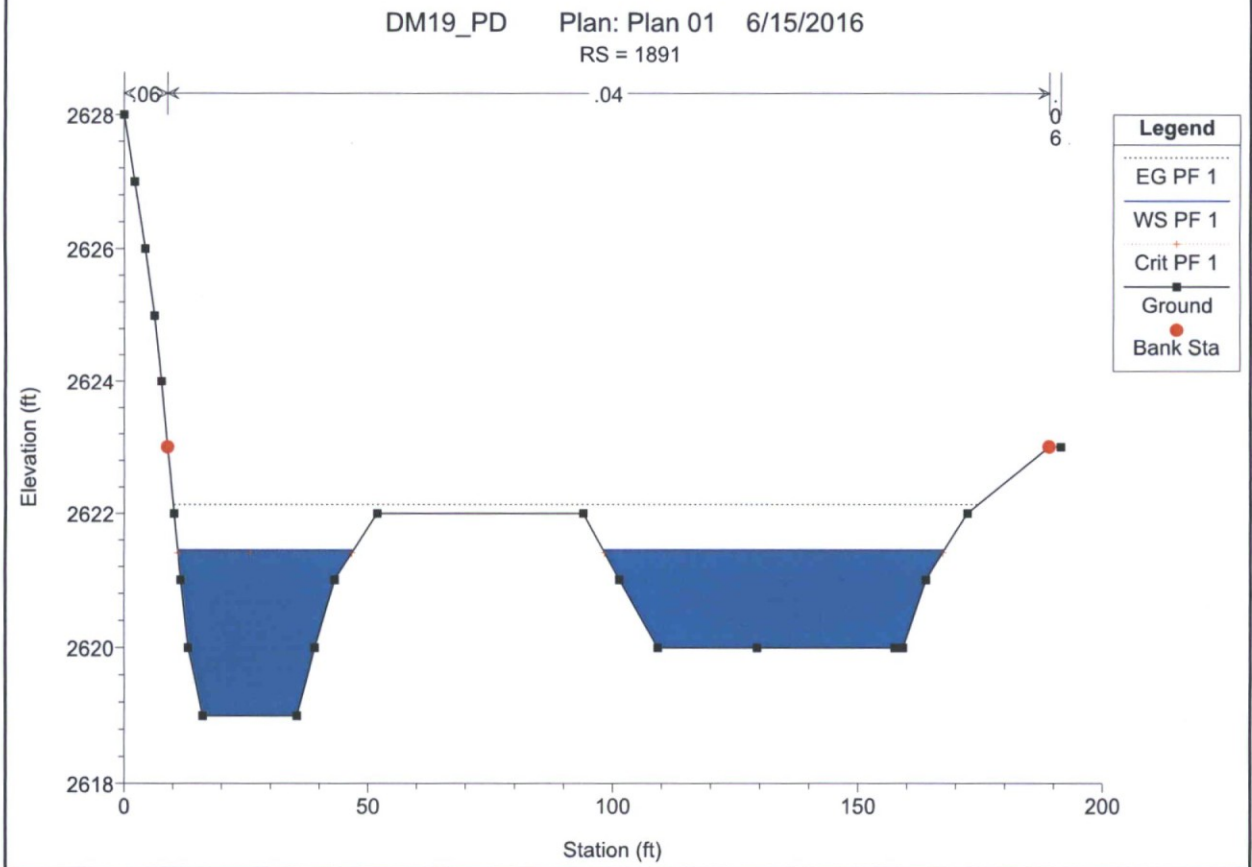
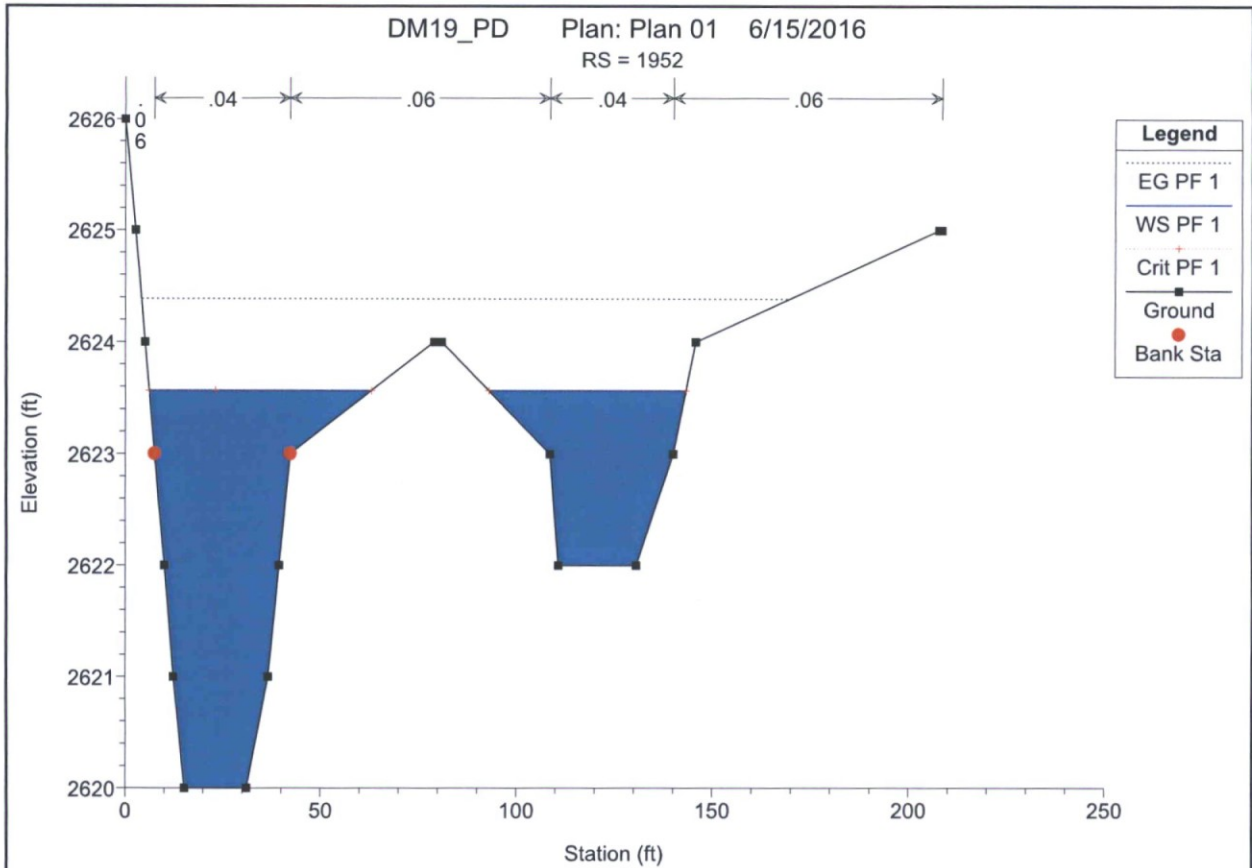


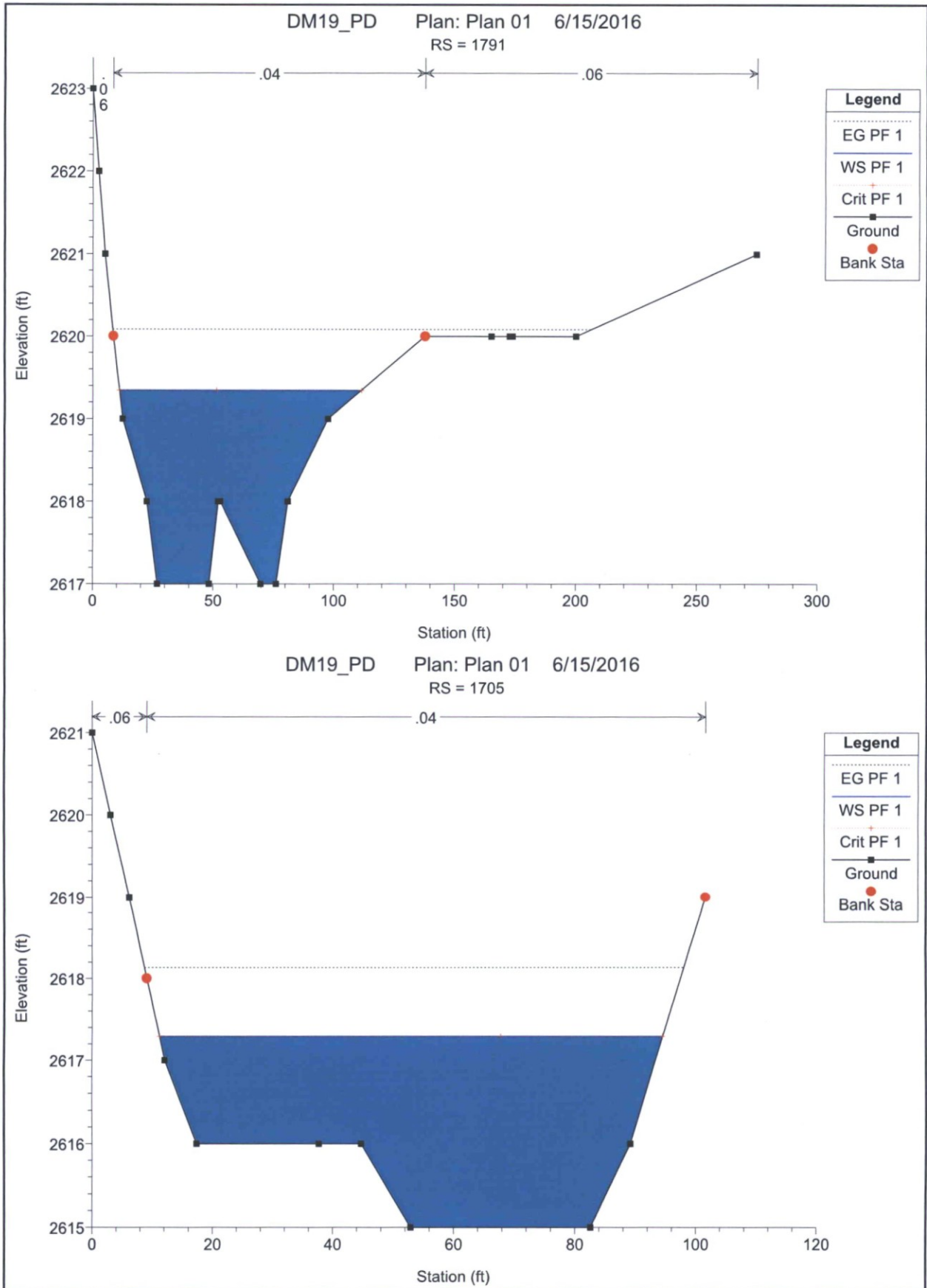




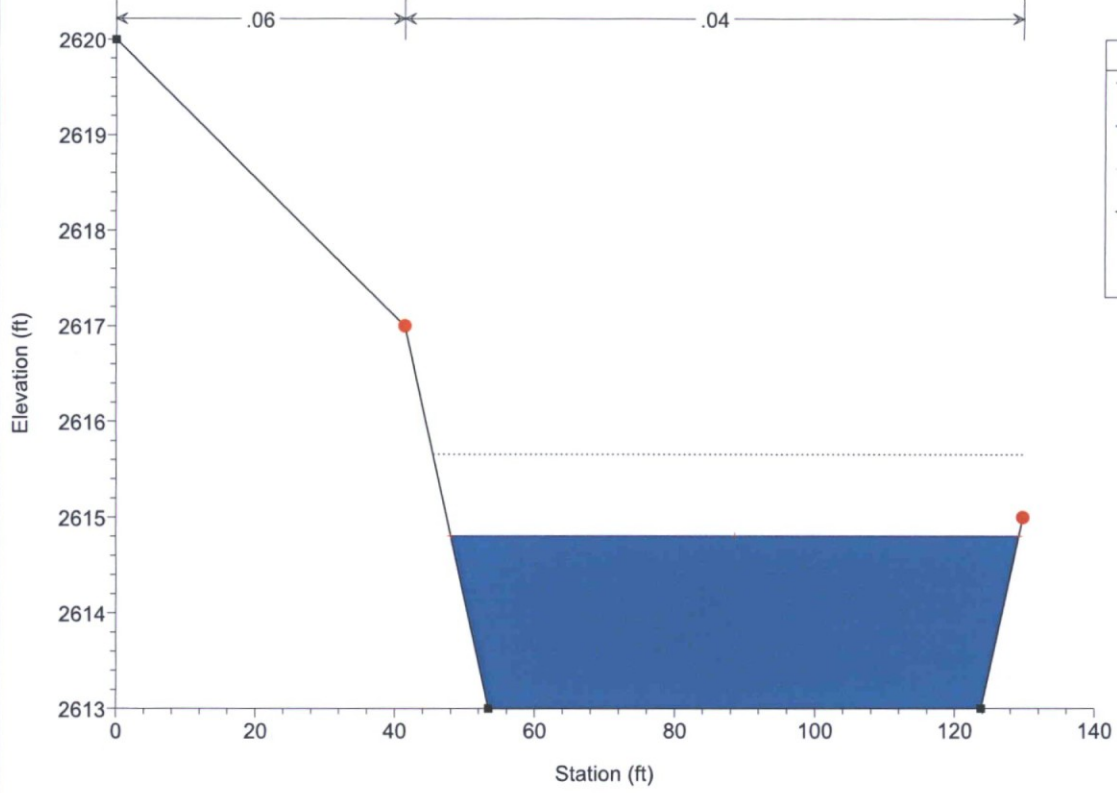




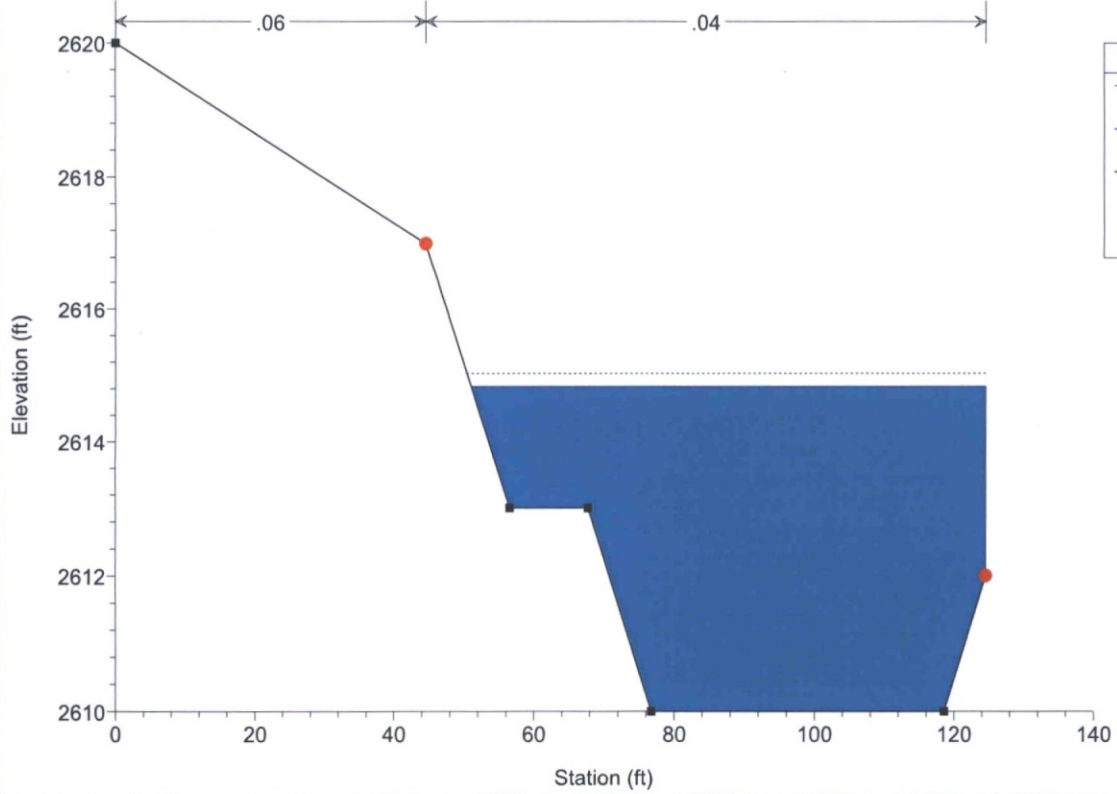




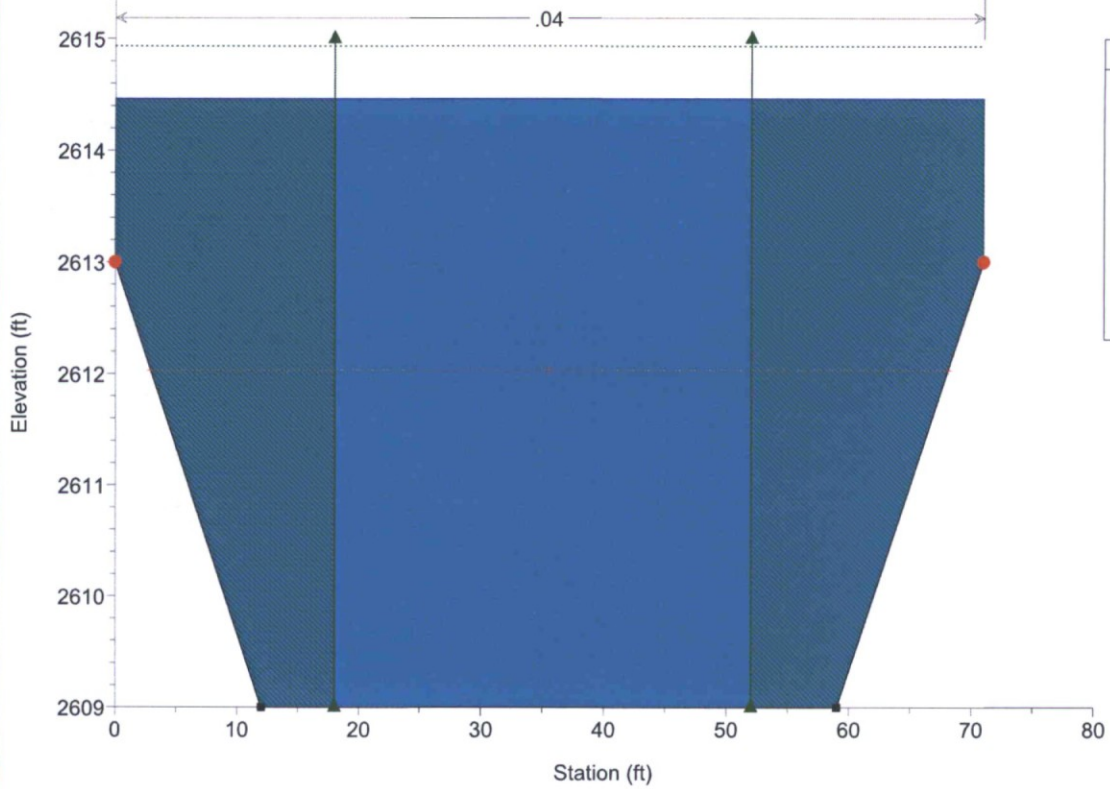
DM19_PD Plan: Plan 01 6/15/2016
RS = 1614.9



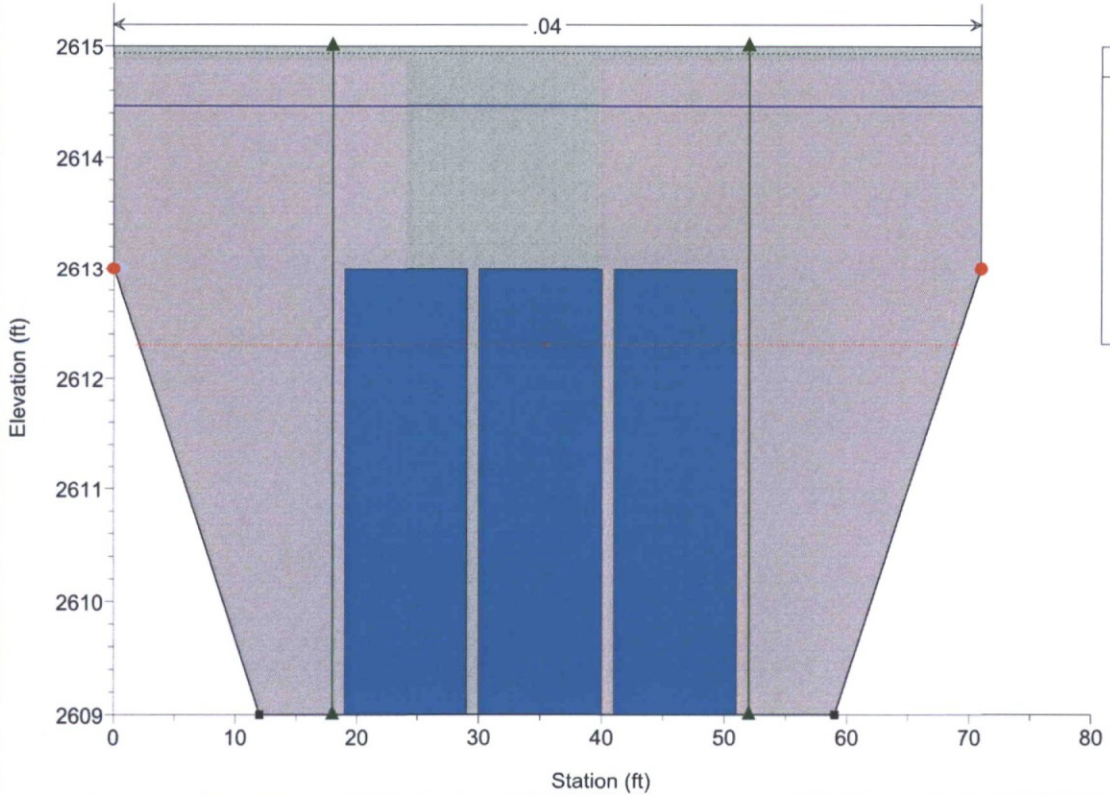
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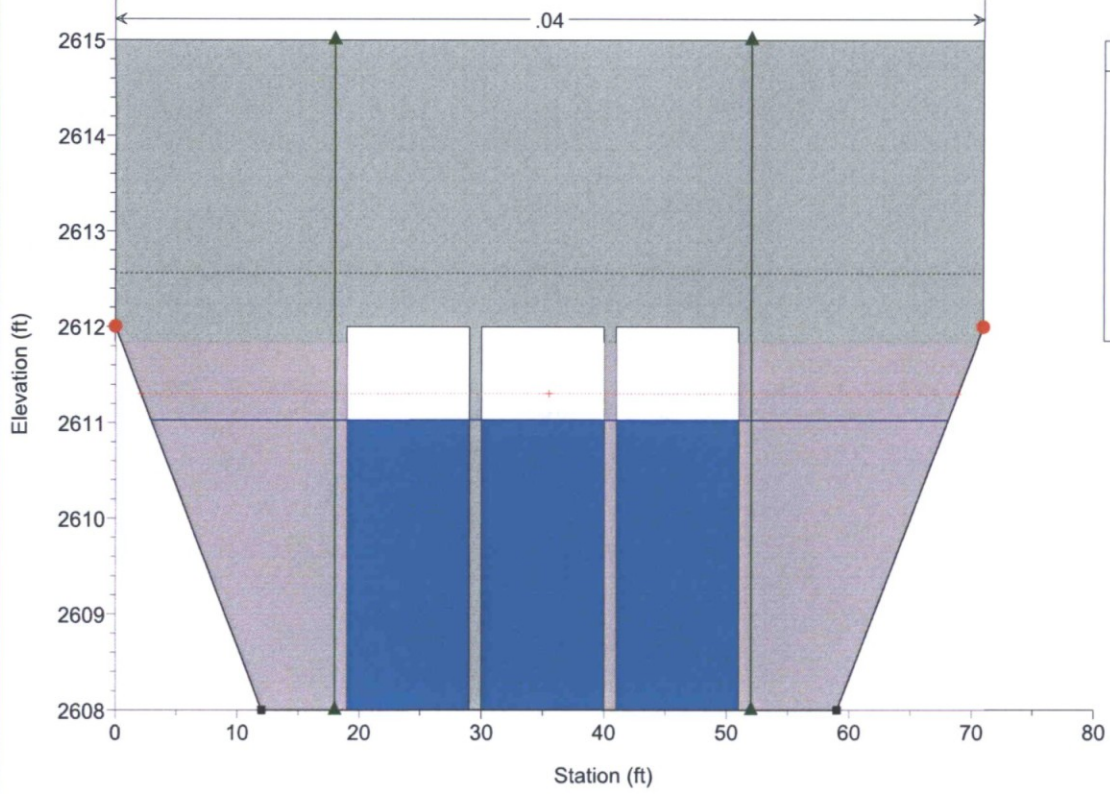
DM19_PD Plan: Plan 01 6/15/2016
RS = 1566.5



DM19_PD Plan: Plan 01 6/15/2016
RS = 1500 Culv

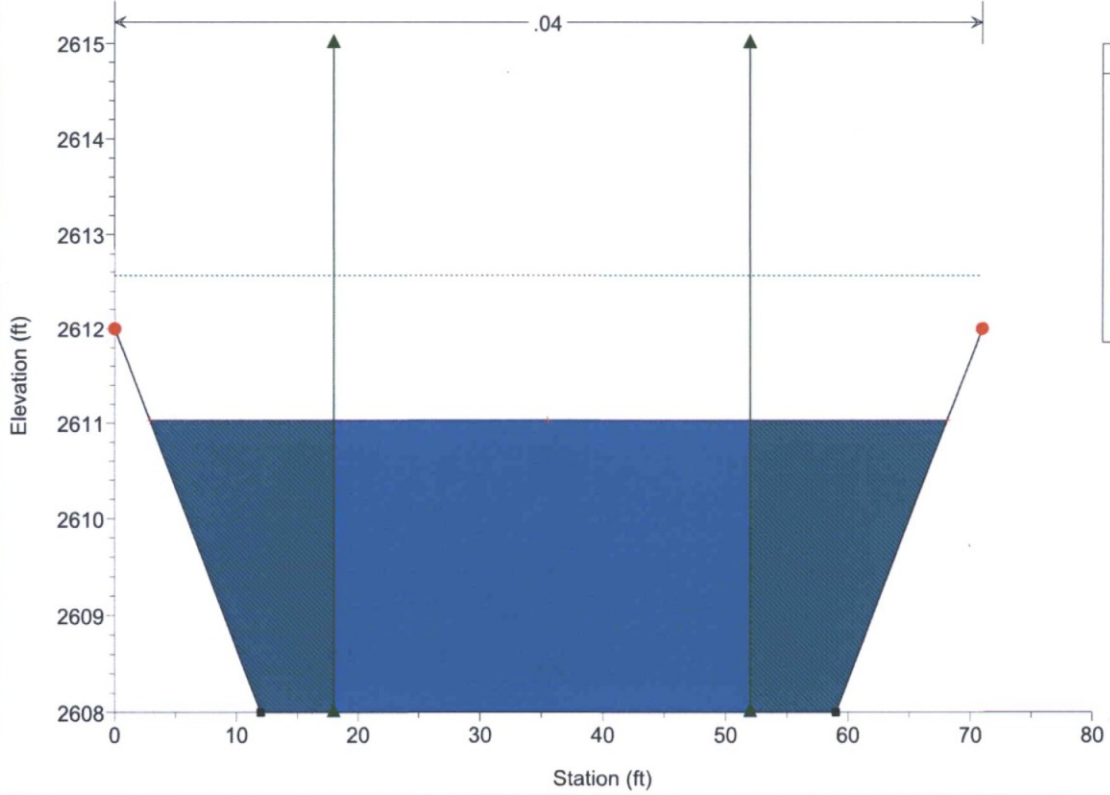


DM19_PD Plan: Plan 01 6/15/2016
RS = 1500 Culv



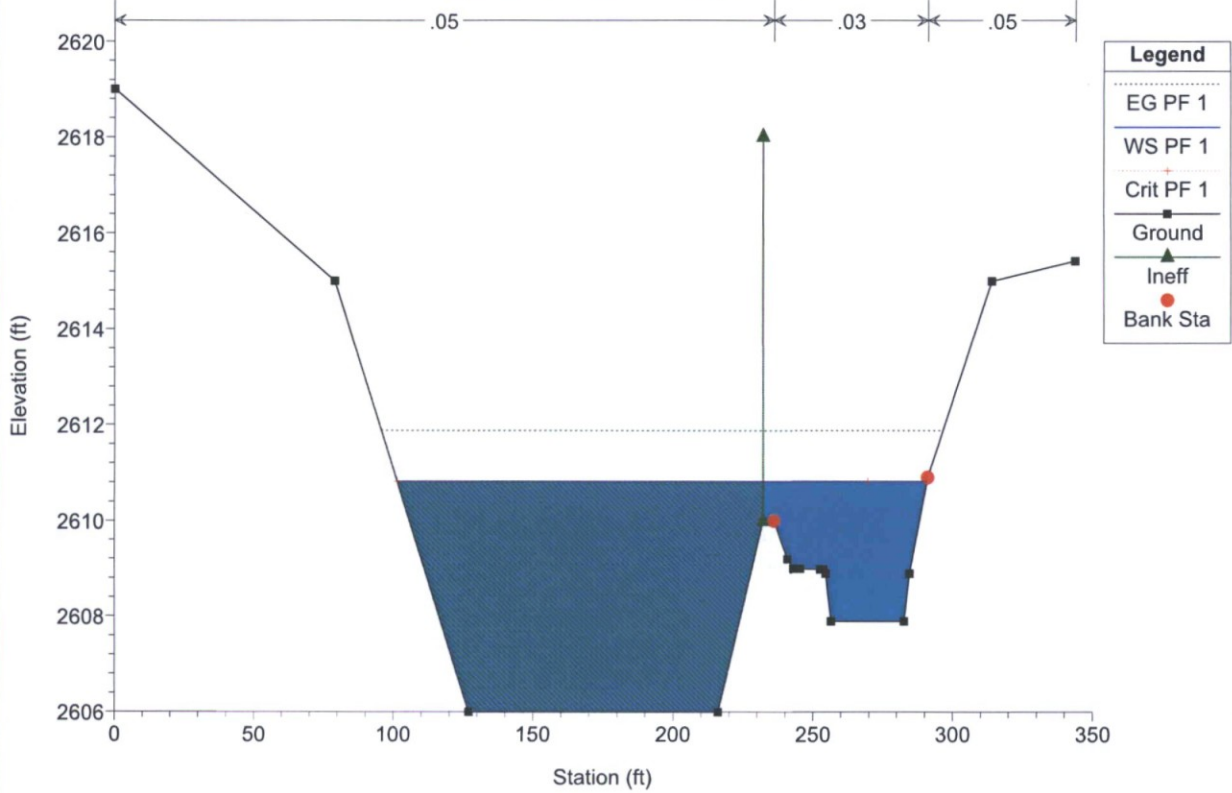
Legend	
EG PF 1	+
Crit PF 1	- - -
WS PF 1	—
Ground	—
Ineff	▲
Bank Sta	●

DM19_PD Plan: Plan 01 6/15/2016
RS = 1481

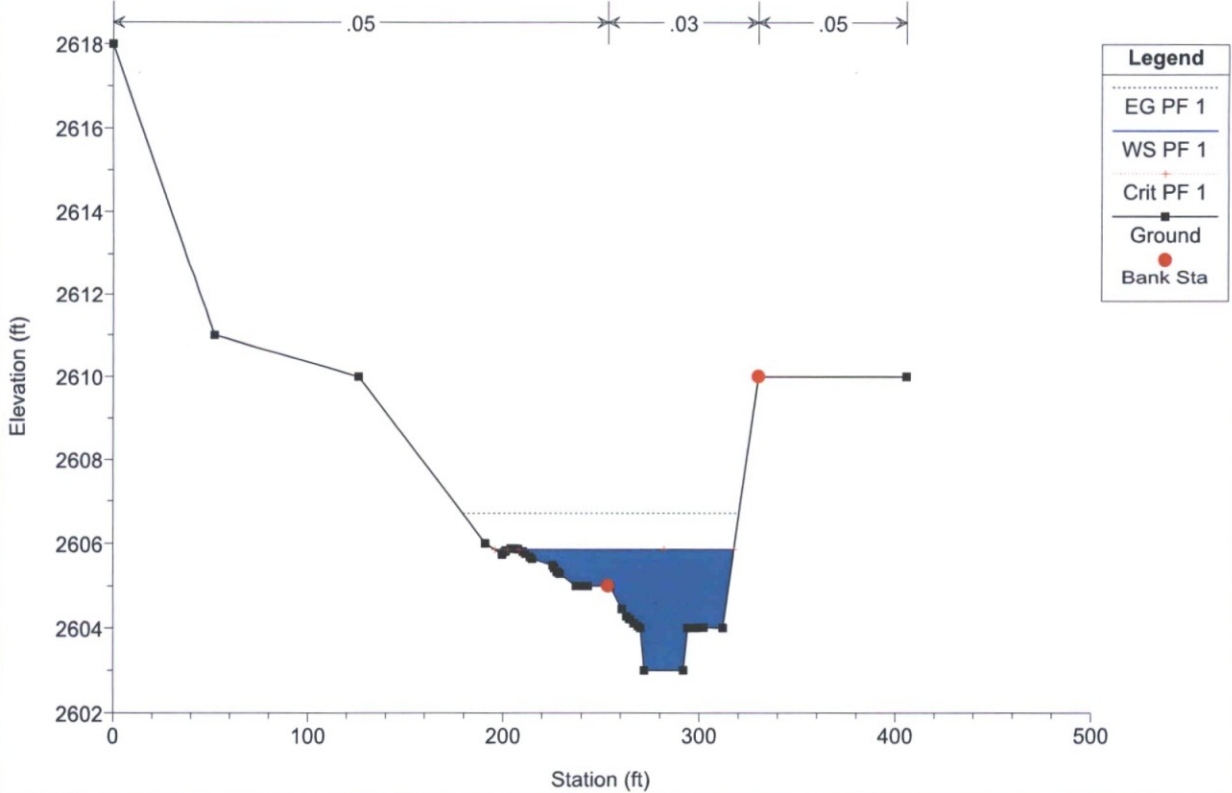


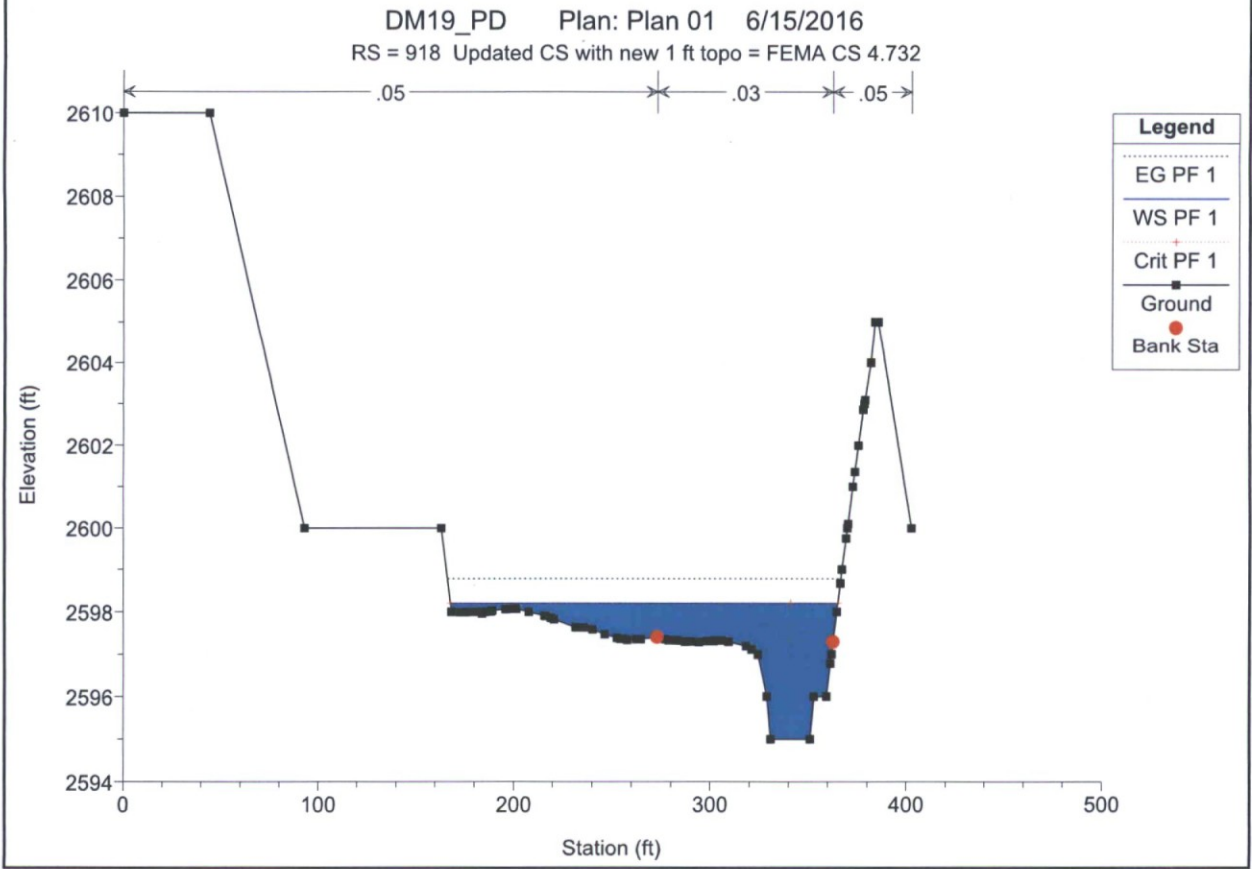
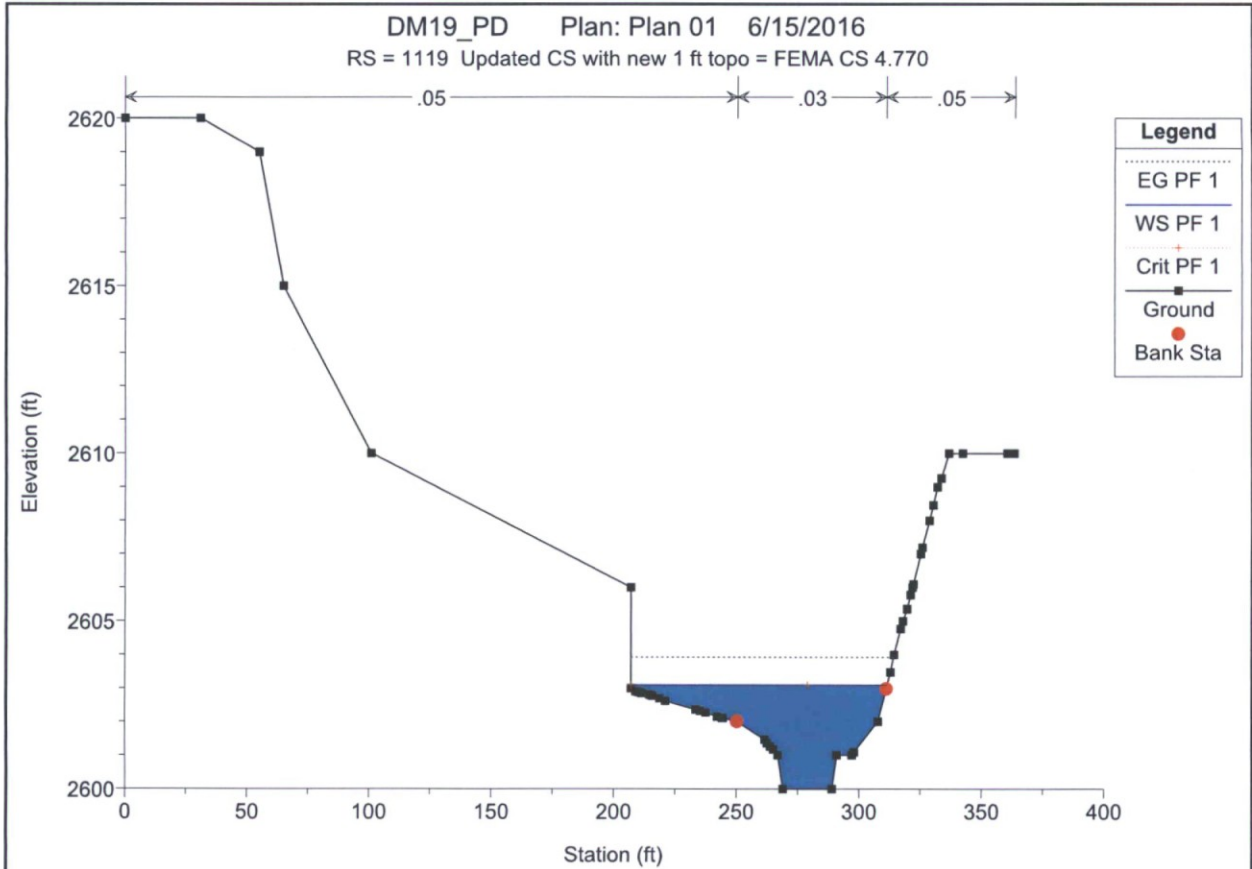
Legend	
EG PF 1	+
WS PF 1	—
Crit PF 1	- - -
Ground	—
Ineff	▲
Bank Sta	●

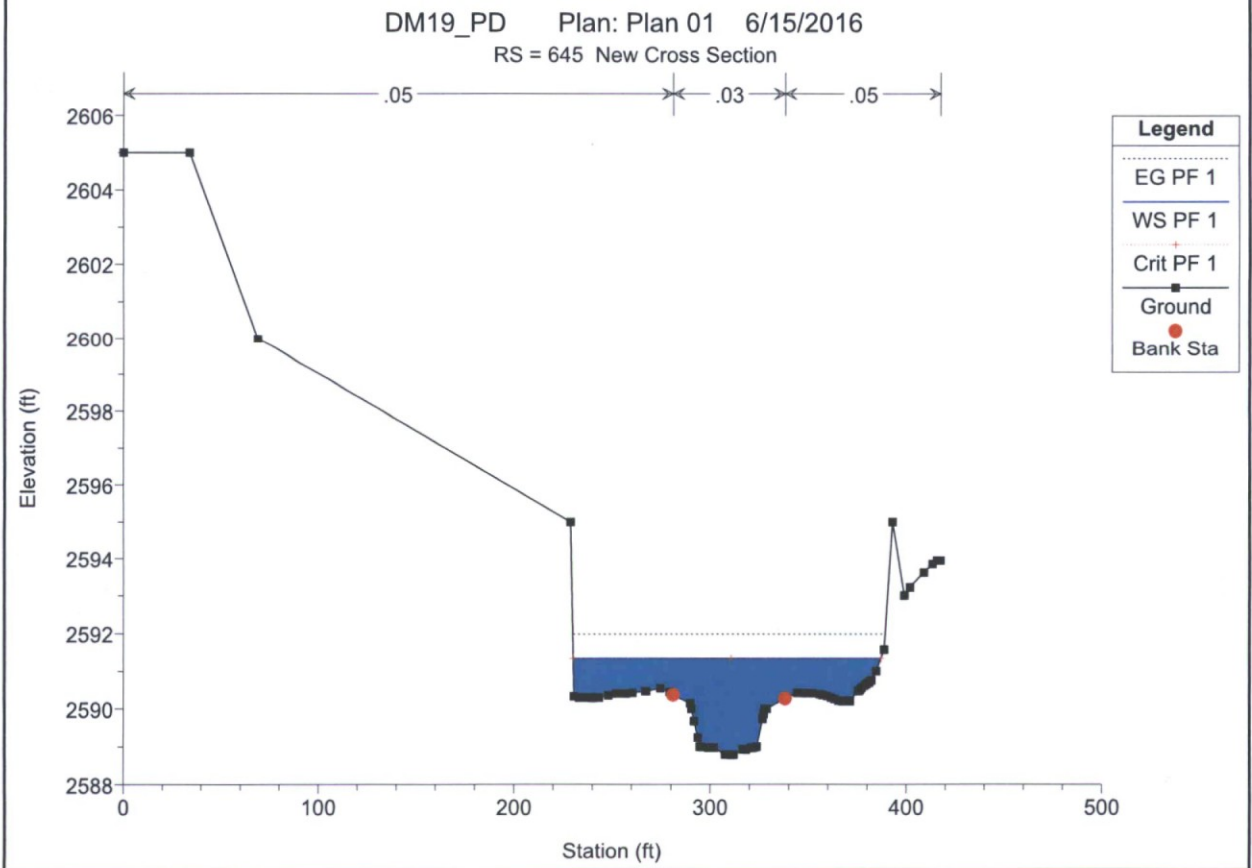
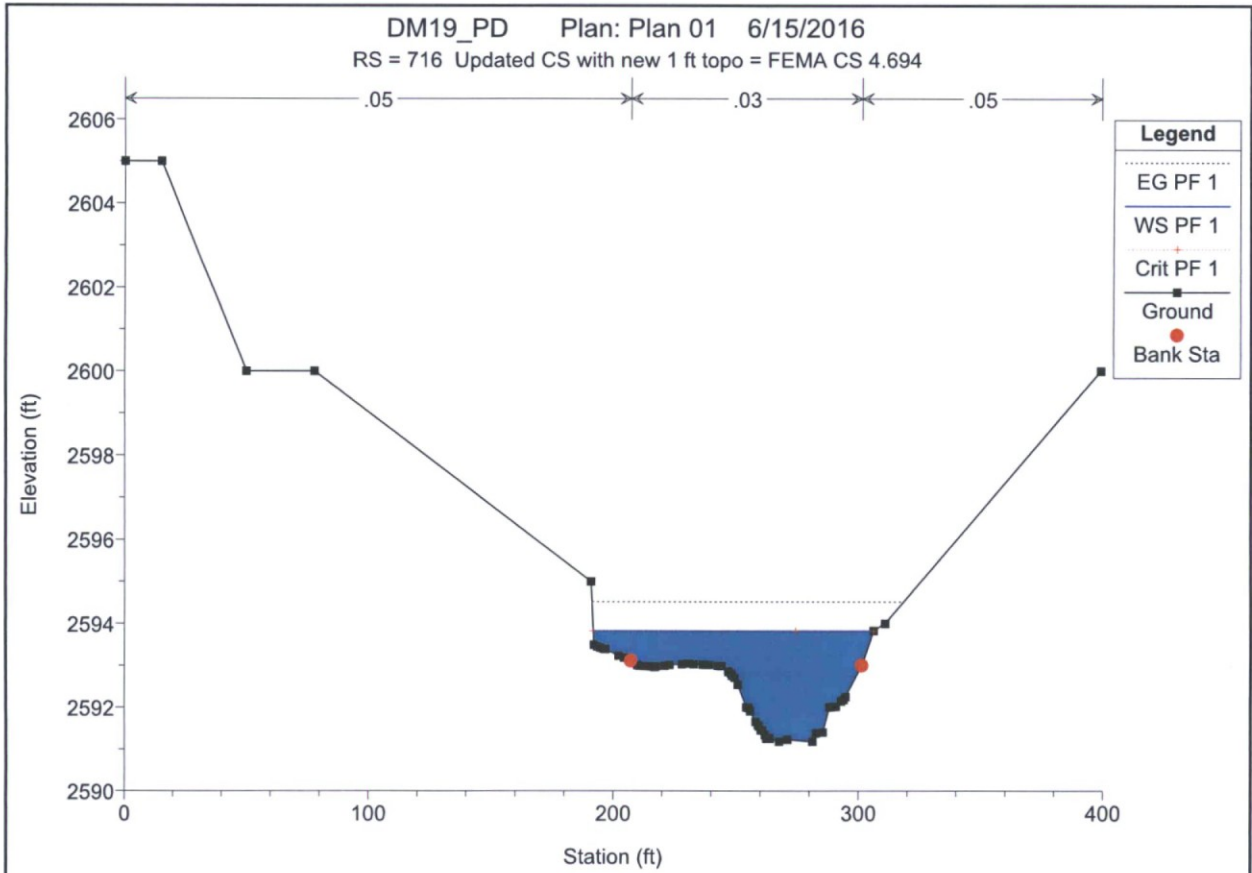
DM19_PD Plan: Plan 01 6/15/2016
 RS = 1447 Updated CS with new 1 ft topo = FEMA CS 4.832

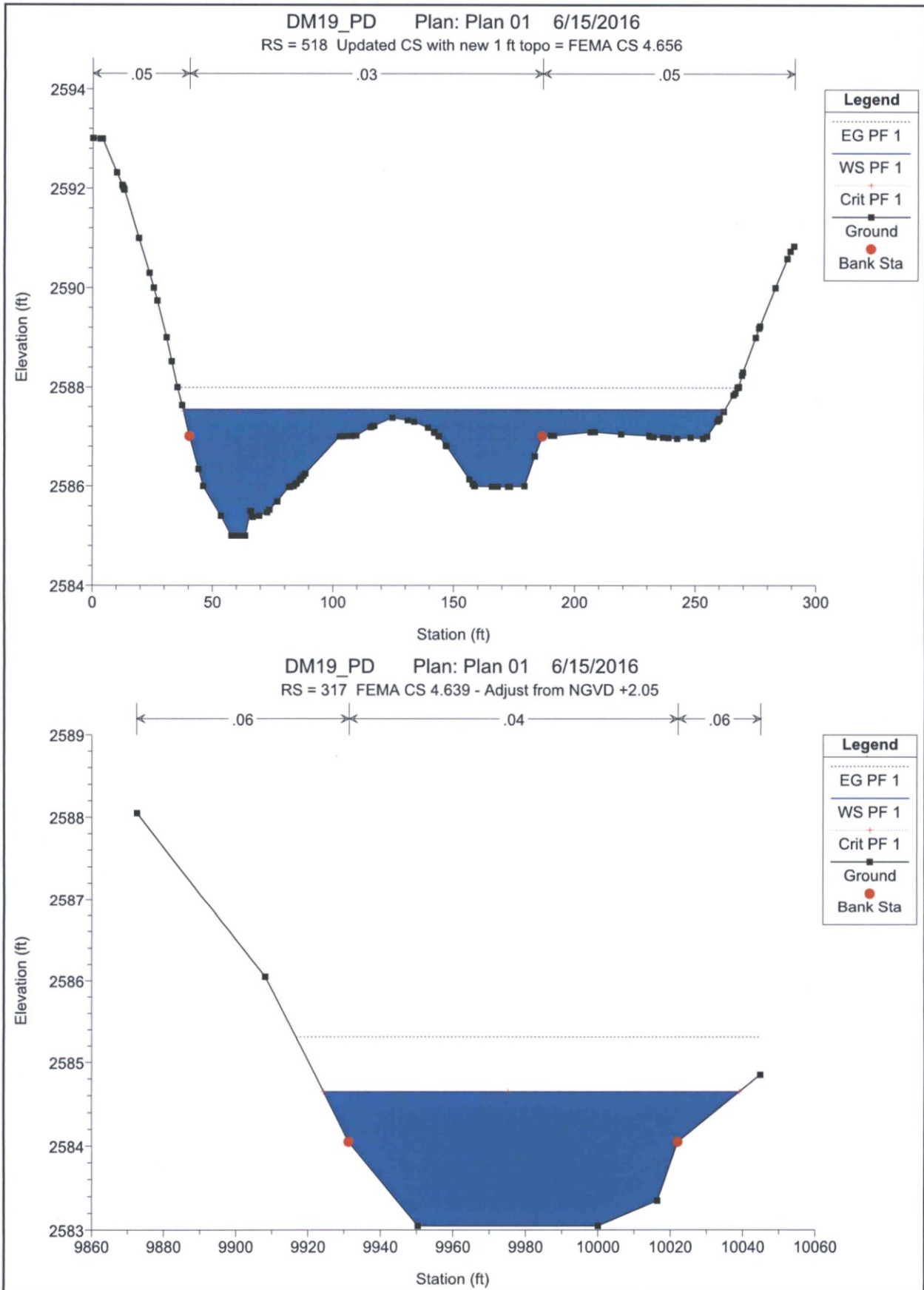


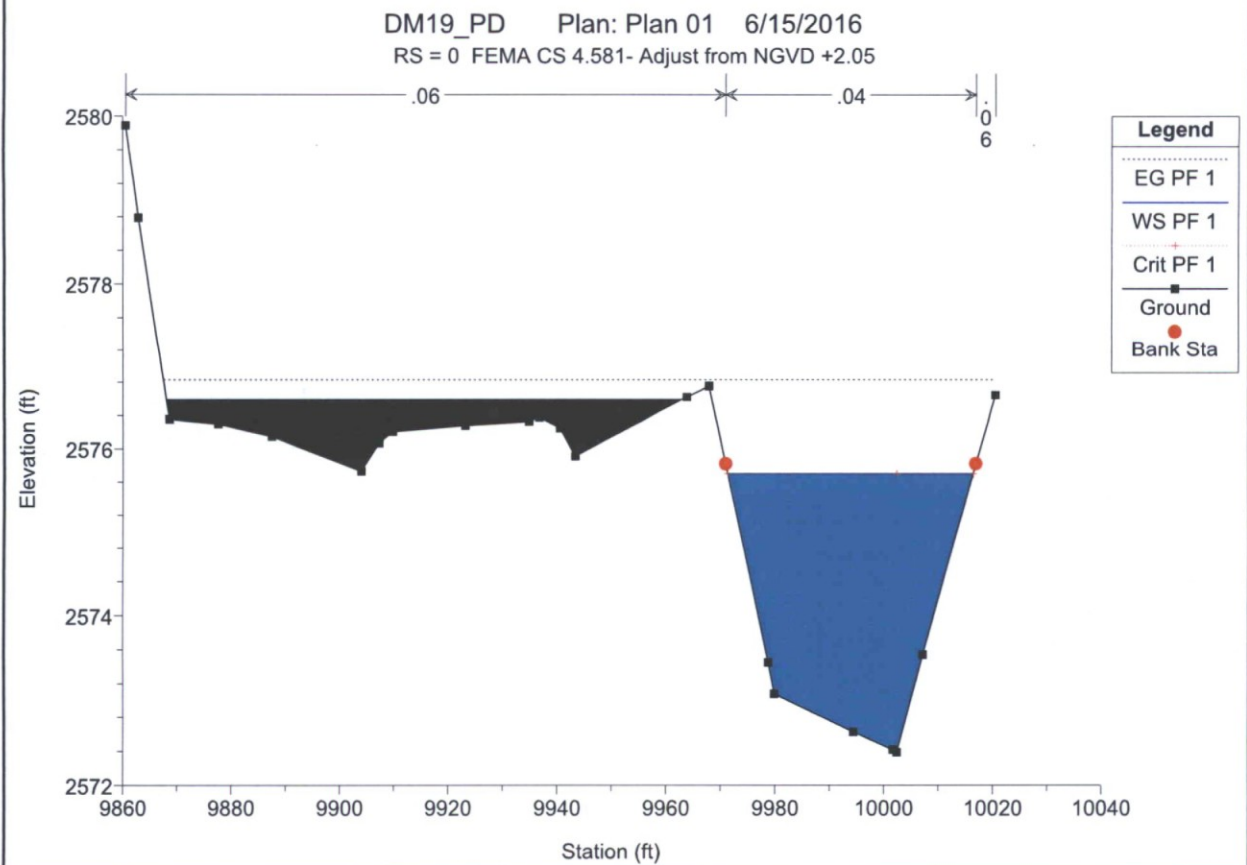
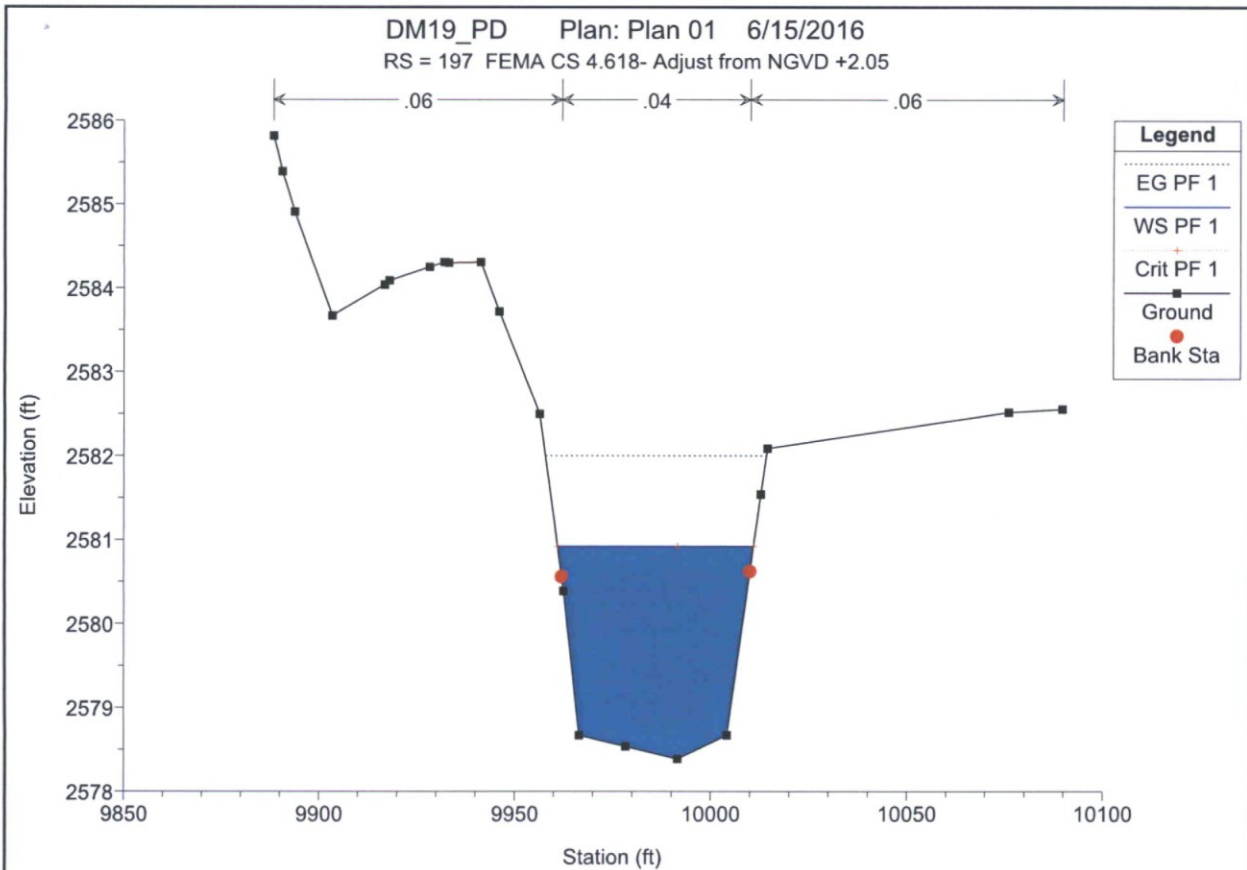
DM19_PD Plan: Plan 01 6/15/2016
 RS = 1257 Updated CS with new 1 ft topo = FEMA CS 4.796











HEC-RAS Plan: Plan 01 River: DM19 Reach: Profile baseline Profile: PF 1

Reach	River Sta	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Profile baseline	2774	1012.00	2641.90	2643.86	2643.86	2644.66	0.020167	7.19	140.92	90.46	1.00
Profile baseline	2678	1012.00	2639.92	2641.86	2641.86	2642.59	0.016480	6.99	163.50	132.82	0.92
Profile baseline	2578	1012.00	2634.45	2638.99		2639.35	0.003594	4.82	209.95	64.34	0.47
Profile baseline	2540	1012.00	2633.32	2638.68	2636.62	2639.21	0.002881	5.88	172.24	72.12	0.46
Profile baseline	2520		Culvert								
Profile baseline	2500	1012.00	2633.00	2636.01	2636.01	2637.53	0.016380	9.90	102.20	64.35	1.01
Profile baseline	2405	1012.00	2630.14	2633.15	2633.09	2634.22	0.016536	8.31	123.48	55.97	0.96
Profile baseline	2250	1012.00	2626.98	2630.28	2630.28	2631.62	0.016755	9.27	110.19	46.98	0.98
Profile baseline	2130	1012.00	2624.00	2626.99	2626.98	2628.02	0.018886	8.14	124.27	60.32	1.00
Profile baseline	2056	1012.00	2623.00	2625.66	2625.66	2626.51	0.020458	7.42	136.31	81.23	1.01
Profile baseline	2014	1012.00	2622.00	2624.73	2624.73	2625.66	0.017239	8.15	134.02	71.27	0.97
Profile baseline	1952	1012.00	2620.00	2623.56	2623.56	2624.38	0.011822	7.94	153.49	107.14	0.83
Profile baseline	1891	1012.00	2619.00	2621.45	2621.41	2622.13	0.019903	6.64	152.39	105.72	0.97
Profile baseline	1791	1012.00	2617.00	2619.35	2619.35	2620.09	0.020971	6.90	146.65	100.43	1.01
Profile baseline	1705	1012.00	2615.00	2617.29	2617.29	2618.13	0.020197	7.35	137.63	83.30	1.01
Profile baseline	1614.9	1012.00	2613.00	2614.81	2614.81	2615.66	0.019987	7.40	136.75	81.17	1.00
Profile baseline	1602.9	1012.00	2610.00	2614.83		2615.03	0.001702	3.61	280.14	73.48	0.33
Profile baseline	1570		Lat Struct								
Profile baseline	1566.5	1022.00	2609.00	2614.46	2612.03	2614.93	0.002280	5.50	185.74	71.00	0.41
Profile baseline	1500		Culvert								
Profile baseline	1481	1022.00	2608.00	2611.03	2611.03	2612.56	0.016291	9.93	102.97	65.17	1.01
Profile baseline	1447	1022.00	2607.90	2610.82	2610.82	2611.88	0.009809	8.30	125.44	189.41	0.98
Profile baseline	1257	1022.00	2603.00	2605.85	2605.85	2606.71	0.009504	7.59	153.46	115.91	0.95
Profile baseline	1119	1022.00	2600.00	2603.09	2603.09	2603.92	0.008530	7.50	155.99	104.42	0.91
Profile baseline	918	1022.00	2595.00	2598.20	2598.20	2598.78	0.008361	6.33	197.76	197.40	0.86
Profile baseline	716	1022.00	2591.19	2593.82	2593.82	2594.52	0.011845	6.70	153.95	114.34	1.00
Profile baseline	645	1022.00	2588.78	2591.34	2591.34	2591.98	0.008972	7.19	199.69	156.65	0.92
Profile baseline	518	1022.00	2585.00	2587.54	2587.51	2587.99	0.009998	5.52	210.58	224.44	0.90
Profile baseline	317	861.00	2583.05	2584.65	2584.65	2585.31	0.019510	6.57	136.54	114.75	0.97
Profile baseline	197	861.00	2578.39	2580.92	2580.92	2582.00	0.018503	8.34	103.55	49.95	1.00
Profile baseline	0	861.00	2572.39	2575.70	2575.70	2576.83	0.018487	8.54	100.84	45.09	1.01

Scour Calculations

GALLOWAY WASH

Level 1 Analysis of Stream Degradation

Reference: ADWR, Flood Warning and Dam Safety Section, 1996.
State Standard 5-96: "Watercourse System Sediment Balance - Guideline 2:
Channel Degradation Estimation for Alluvial Channels in Arizona"

Assumptions: Channel reaches without major disturbances, such as dams and bridges.

Equations:

$$D_s = D_{gs} + D_{lts}$$

where:

D_s = total scour depth, in feet;

D_{gs} = general degradation, in feet;

D_{lts} = long term degradation, in feet;

For straight channel reaches:

$$D_{gs} = 0.157 * Q_{100}^{0.4}$$

For channel reaches with curvature:

$$D_{gs} = 0.219 * Q_{100}^{0.4}$$

Long term degradation:

$$D_{lts} = 0.02 * Q_{100}^{0.6}$$

Project Name: DM 19

Location: Galloway Wash

Input Data:

$$Q_{100} = 1025 \text{ cfs}$$

$D_s = 3.79 \text{ ft}$ for straight channel

$D_s = 4.79 \text{ ft}$ otherwise

Recommended Scour Depth = 5.0 ft

Note: the minimum total scour depth, D_s , shall be 3 feet.

WASH A

Level 1 Analysis of Stream Degradation

Reference: ADWR, Flood Warning and Dam Safety Section, 1996.
State Standard 5-96: "Watercourse System Sediment Balance - Guideline 2:
Channel Degradation Estimation for Alluvial Channels in Arizona"

Assumptions: Channel reaches without major disturbances, such as dams and bridges.

Equations:

$$D_s = D_{gs} + D_{lts}$$

where:

D_s = total scour depth, in feet;
 D_{gs} = general degradation, in feet;
 D_{lts} = long term degradation, in feet;

For straight channel reaches:

$$D_{gs} = 0.157 * Q_{100}^{0.4}$$

For channel reaches with curvature:

$$D_{gs} = 0.219 * Q_{100}^{0.4}$$

Long term degradation:

$$D_{lts} = 0.02 * Q_{100}^{0.6}$$

Project Name: DM 19

Location: Wash A

Input Data:

$$Q_{100} = 468 \text{ cfs}$$

$D_s = 2.64 \text{ ft}$ for straight channel

$D_s = 3.36 \text{ ft}$ otherwise

Recommended Scour Depth = 4.0 ft

Note: the minimum total scour depth, D_s , shall be 3 feet.

Erosion Hazard Setback Calculations

GALLOWAY WASH

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: *Desert Mountain Parcel 19*
Location: *Phoenix, Maricopa County Arizona*
Date: *16-Jun-16*
References: *Storm Water Policies & Standards, City of Phoenix (12/2013)*

ADWR Erosion Hazard Setback Equations State Standard for Watercourse System Sediment Balance SSA 5-96, LMSA-5, September 1996

For straight channel reaches or reaches with minor curvature $setback = 1.0(Q100)^{0.5}$

For channels with obvious curvature or channel bend $setback = 2.5(Q100)^{0.5}$

The setback allowance is to be measured outward from the 100-year floodway
Or the top of the channel bank, whichever is greater

EROSION HAZARD SETBACK CALCULATIONS

Wash Name	Q100 Discharge (cfs)	ADWR Erosion Hazard Setback (EHS) (ft)
Galloway Wash		
Off-Site Wash	1025	80.0
(bend)		
Off-Site Wash	1025	32.0

Note:
The minimum EHS is 20' for Straight Channels

WASH A

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Project: *Desert Mountain Parcel 19*
Location: *Phoenix, Maricopa County Arizona*
Date: *16-Jun-16*
References: *Storm Water Policies & Standards, City of Phoenix (12/2013)*

ADWR Erosion Hazard Setback Equations State Standard for Watercourse System Sediment Balance SSA 5-96, LMSA-5, September 1996

For straight channel reaches or reaches with minor curvature $setback = 1.0(Q100)^{0.5}$

For channels with obvious curvature or channel bend $setback = 2.5(Q100)^{0.5}$

The setback allowance is to be measured outward from the 100-year floodway
Or the top of the channel bank, whichever is greater

EROSION HAZARD SETBACK CALCULATIONS

Wash Name	Q100 Discharge (cfs)	ADWR Erosion Hazard Setback (EHS) (ft)
Wash A		
Off-Site Wash	468	54
(bend)		
Off-Site Wash	468	22

Note:
The minimum EHS is 20' for Straight Channels

APPENDIX E

Electronic Versions:

PDF of DM 19 Preliminary Drainage Report

DDMSW DM 19 Existing & Proposed ZIP files

Existing & Proposed 2-year, 10-year & 100-year HEC-1 Files

Existing Wash A & Existing & Proposed Galloway Wash HEC-RAS Files

Electronic Files READ ME.txt

City of Scottsdale, Arizona

Project Name: PRELIMINARY DRAINAGE REPORT FOR DESERT MOUNTAIN PARCEL 19
PREPARED BY WOOD/PATEL

Date Prepared: 06/16/2016

LIST OF ITEMS ON CD:

PDF OF DRAINAGE REPORT FOR DESERT MOUNTAIN PARCEL 19

Computer Program Files

DDMSW BACKUP FILES SUBMITTED:

DM 19 EX.ZIP

DM 19 PROP.ZIP

HEC-1 FILES SUBMITTED:

PDR EXISTING MODELS HEC1

2-YEAR MODEL: DM19EX2.DAT

DM19EX2.OH1

10-YEAR MODEL: DM19EX10.DAT

DM19EX10.OH1

100-YEAR MODEL: DM19EX100.DAT

DM19EX100.OH1

PDR POST DEVELOPMENT MODELS HEC1

2-YEAR MODEL: DM19FT2.DAT

DM19FT2.OH1

10-YEAR MODEL: DM19FT10.DAT

DM19FT10.OH1

100-YEAR MODEL: DM19FT100.DAT

DM19FT100.OH1

HEC-RAS EXISTING CONDITION PROJECT Files SUBMITTED:

GALLOWAY WASH

DM19EXISTING.PRJ

DM19EXISTING.F01

DM19EXISTING.G01

DM19EXISTING.P01

DM19EXISTING.R01

DM19EXISTING.O01

WASH A

WASHA.PRJ

WASHA.F01

WASHA.G01

WASHA.P01

WASHA.R01

WASHA.O01

HEC-RAS POST-DEVELOPMENT PROJECT Files SUBMITTED:

GALLOWAY WASH

DM19_PD.PRJ

DM19_PD.F01

DM19_PD.G01

DM19_PD.P01

DM19_PD.R01

DM19_PD.O01

EXHIBIT 1

Vicinity Map



Image courtesy of USGS © 2016 Microsoft Corporation

VICINITY MAP

N.T.S.



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DESERT MOUNTAIN 19

**EXHIBIT 1
 VICINITY MAP**

DATE:
06/15/2016
 JOB NO.:
164434

SCALE:
N.T.S.
 DESIGN: RMH
 DRAWN: RMH

SHEET
1 OF 1

EXHIBIT 2

ESL Classification Map



Image courtesy of USGS © 2016 Microsoft Corporation

VICINITY MAP

N.T.S.



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DESERT MOUNTAIN 19

**EXHIBIT 1
 VICINITY MAP**

DATE:
06/15/2016
 JOB NO.:
164434

SCALE:
N.T.S.
 DESIGN: RMH
 DRAWN: RMH

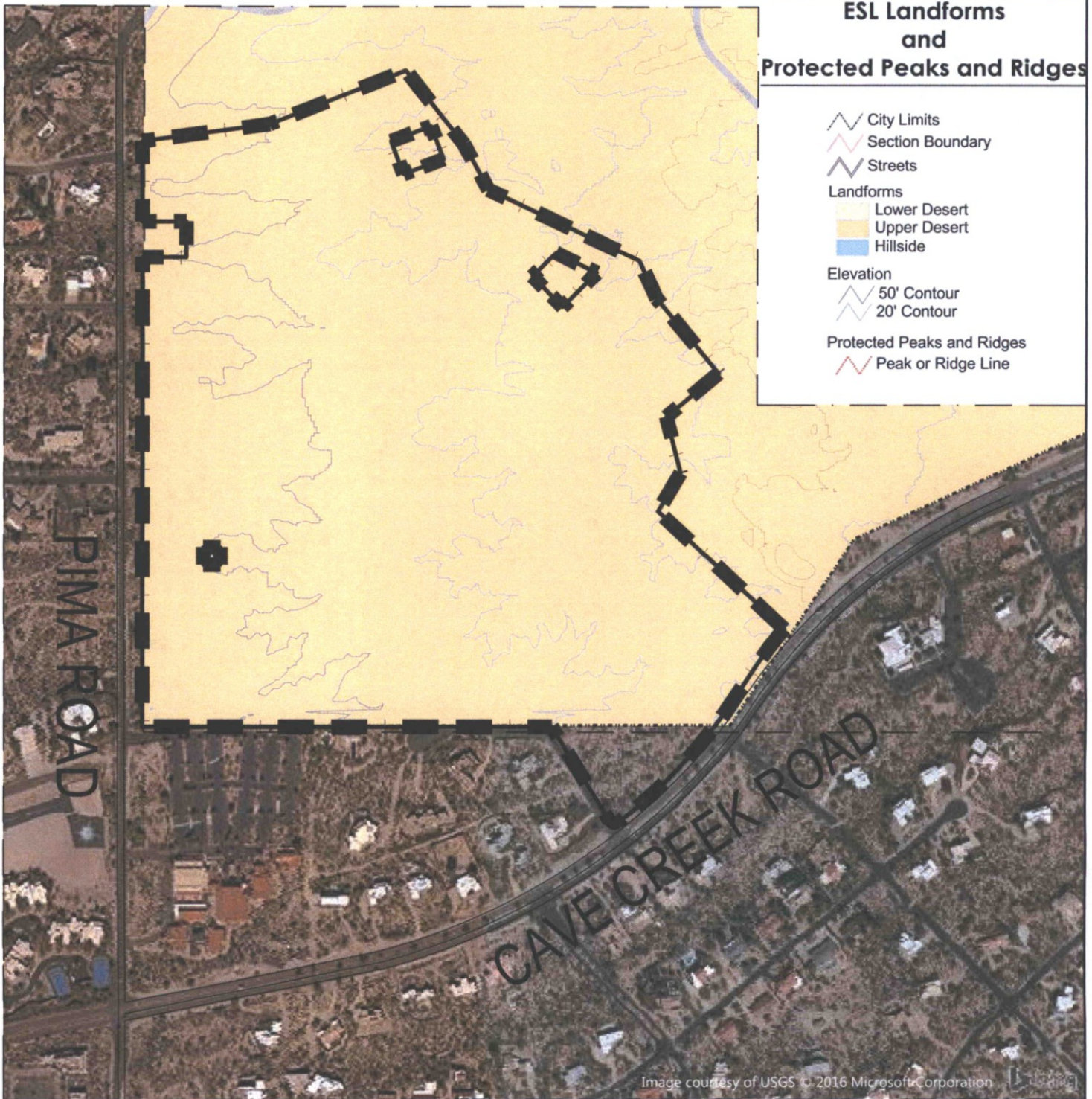
SHEET
1 OF 1

EXHIBIT 2

ESL Classification Map

ESL Landforms and Protected Peaks and Ridges

-  City Limits
-  Section Boundary
-  Streets
- Landforms**
-  Lower Desert
-  Upper Desert
-  Hillside
- Elevation**
-  50' Contour
-  20' Contour
- Protected Peaks and Ridges**
-  Peak or Ridge Line



VICINITY MAP
N.T.S.



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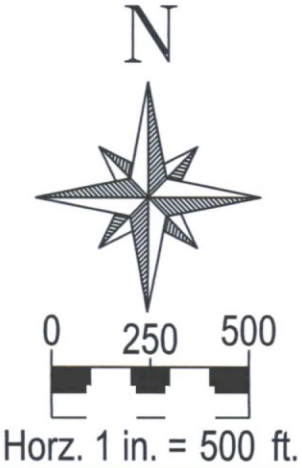
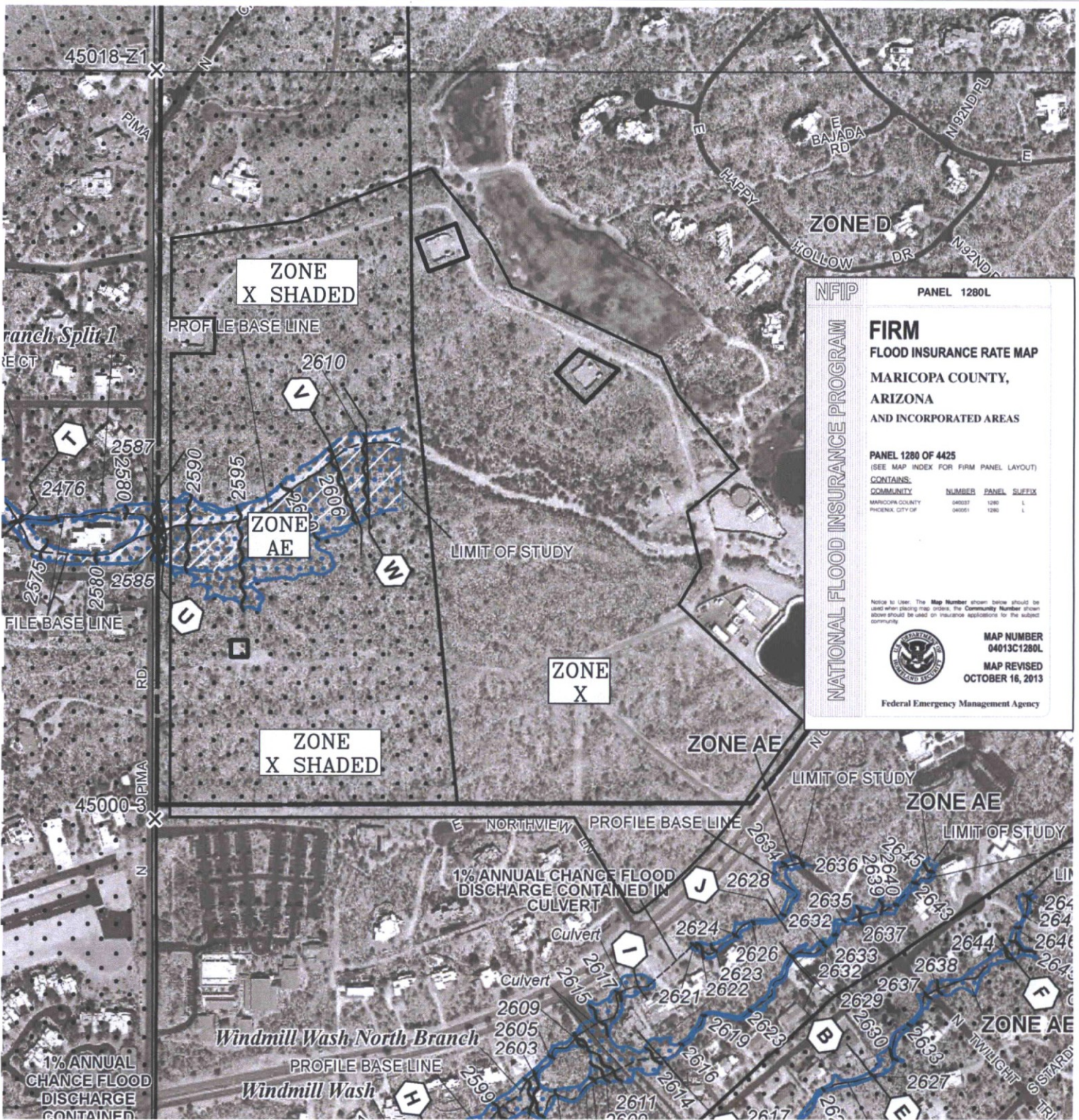
DESERT MOUNTAIN 19

EXHIBIT 2 ESL Classification Map

DATE: 06/09/2016	SCALE: N.T.S.	SHEET 1 OF 1
JOB NO.: 164434	DESIGN: SES	
	DRAWN: SES	

EXHIBIT 3

Flood Insurance Rate Map (FIRM)

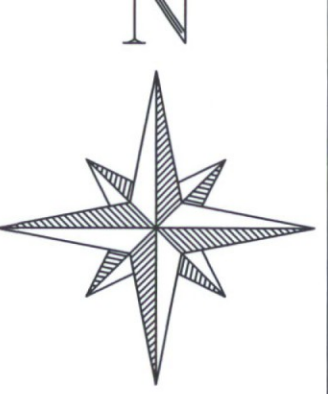


WOOD/PATEL MISSION: CLIENT SERVICE* (602) 335-8500 WWW.WOODPATEL.COM			DESERT MOUNTAIN 19		
			EXHIBIT 3 FLOOD INSURANCE RATE MAP		
DATE	SCALE	SHEET			
6/15/2016	1" = 500'	1 OF 1			
JOB NO.	DESIGN	RMH	CHECK	JCD	
164434	DRAWN	RMH	RFI #		

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EXHIBIT 4

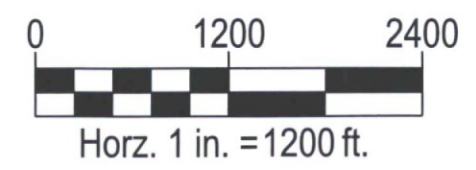
Soils Classification Map



4443... Juppo... Drainages... Exhibit... .assific... .dwg

Image courtesy of USGS © 2016 Microsoft Corporation

LEGEND	
	NRCS MAP UNIT LEGEND
	SUB BASIN ID
	SOIL BOUNDARY
	SUB BASIN BOUNDARY



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DESERT MOUNTAIN 19		
EXHIBIT 4		
SOILS CLASSIFICATION MAP		
DATE 06/09/2016	SCALE 1" = 1200'	SHEET 1 OF 1

EXHIBIT 5

Aerial Map



N



Horz. 1 in. = 500 ft.

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EXHIBIT 5 AERIAL MAP

DATE:
06/15/2016

SCALE:
1" = 500'

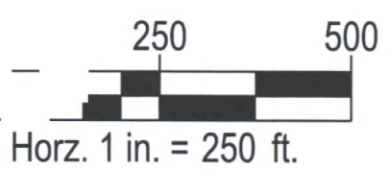
JOB NO.:
164434

DESIGN: RMH
 DRAWN: RMH

SHEET
1 OF 1

EXHIBIT 6

Developed Conditions Land Use Map

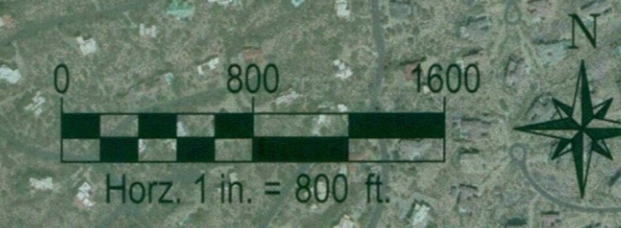
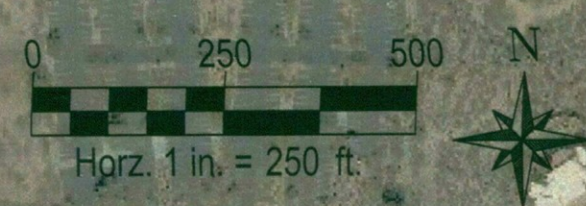
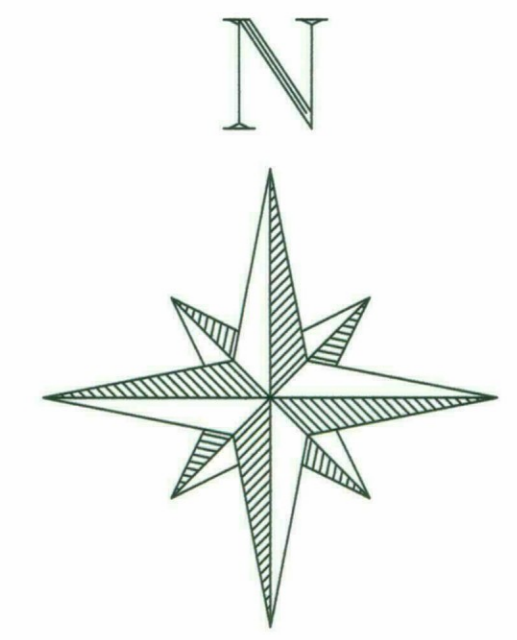


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DESERT MOUNTAIN 19			
EXHIBIT 6			
POST DEVELOPMENT LAND USE			
MAP			
DATE	SCALE	SHEET	
6/15/2016	1" = 500'	1 OF 1	
JOB NO.	DESIGN	SES	CHECK JCD
164434	DRAWN	SES	RFI #

EXHIBIT 7

Existing Conditions Sub-Basin HEC-1 Map



LEGEND

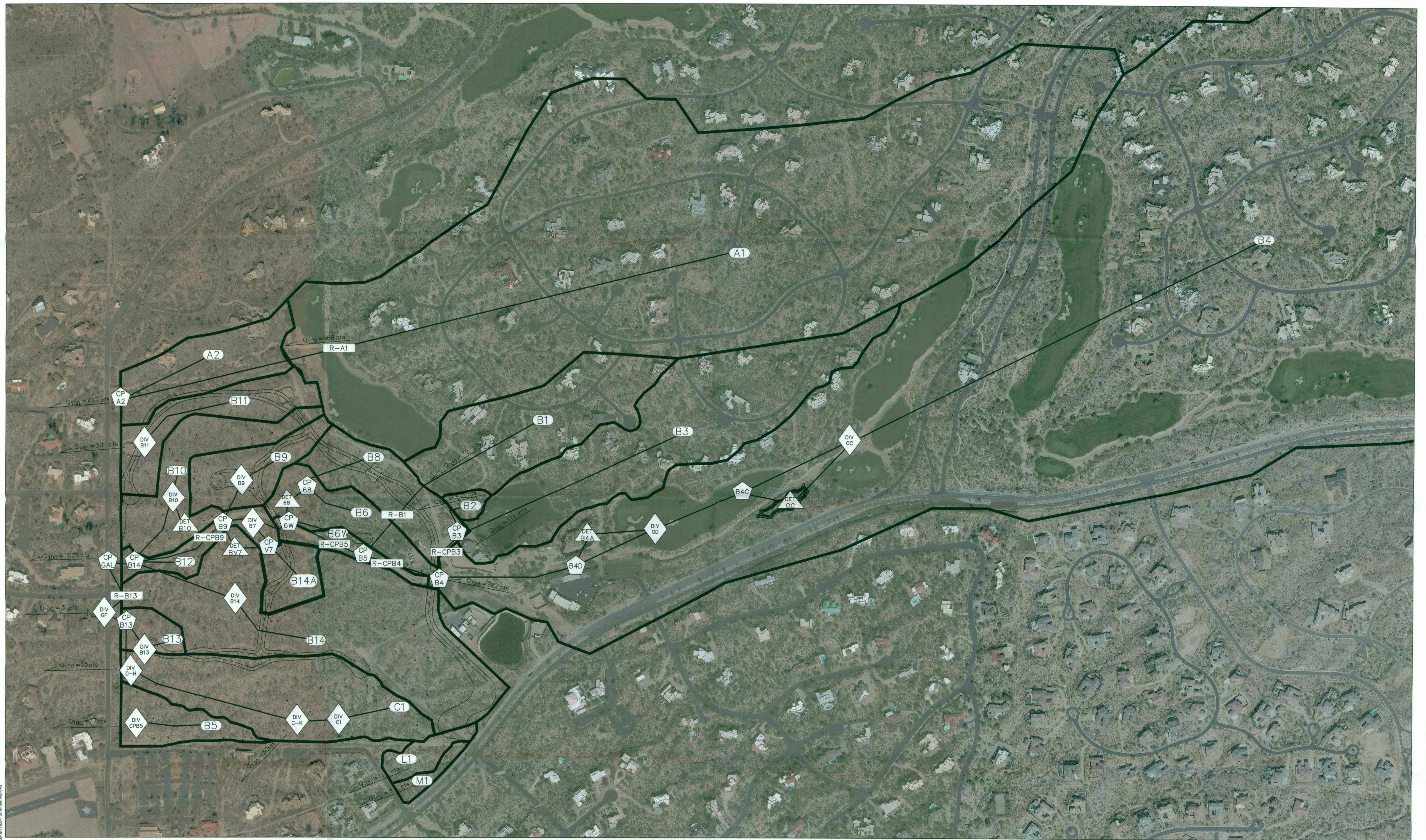
- K1 SUB-BASIN
- R-CPE4 CHANNEL ROUTING
- DET
OD STORAGE ROUTING
- CP
E5 HYDROGRAPH COMBINING
- DV
B7 FLOW DIVERSION
- DRAINAGE AREA BOUNDARY

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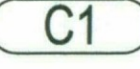
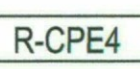




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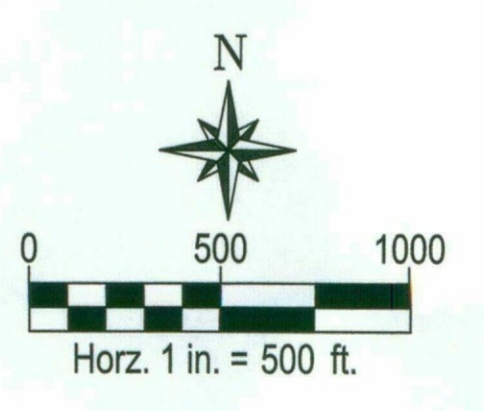
DESERT MOUNTAIN 19		
EXHIBIT 7 EXISTING CONDITIONS SUB-BASIN HEC-1 MAP		
DATE: 06-16-2016	SCALE: VARIES	SHEET 1 OF 1
JOB NO.: 164434	DESIGN: SES	
	DRAWN: SES	

N:\2010\164434\Project\SupplementalDrawings\DESERTMOUNTAIN19\Sub-Basin HEC-1 Existing Conditions HEC1 Technical Map.dwg



LEGEND

-  SUB-BASIN
-  CHANNEL ROUTING
-  STORAGE ROUTING
-  HYDROGRAPH COMBINING
-  FLOW DIVERSION
-  DRAINAGE AREA BOUNDARY

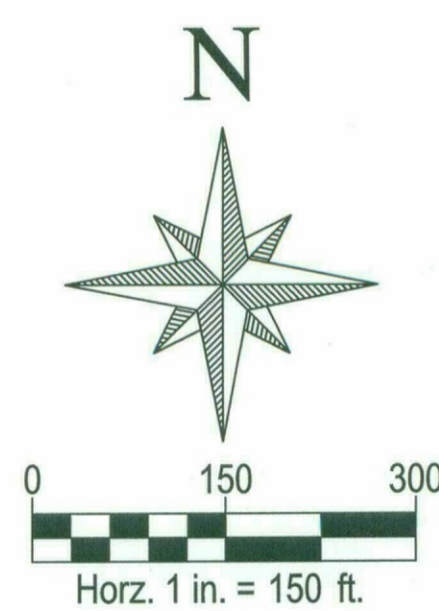


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DESERT MOUNTAIN 19		
EXHIBIT 8 DEVELOPED CONDITIONS SUB-BASIN HEC-1 MAP		
DATE: 06-16-2016	SCALE: 1" = 250'	SHEET 1 OF 1
JOB NO.: 164434	DESIGN: SES DRAWN: SES	

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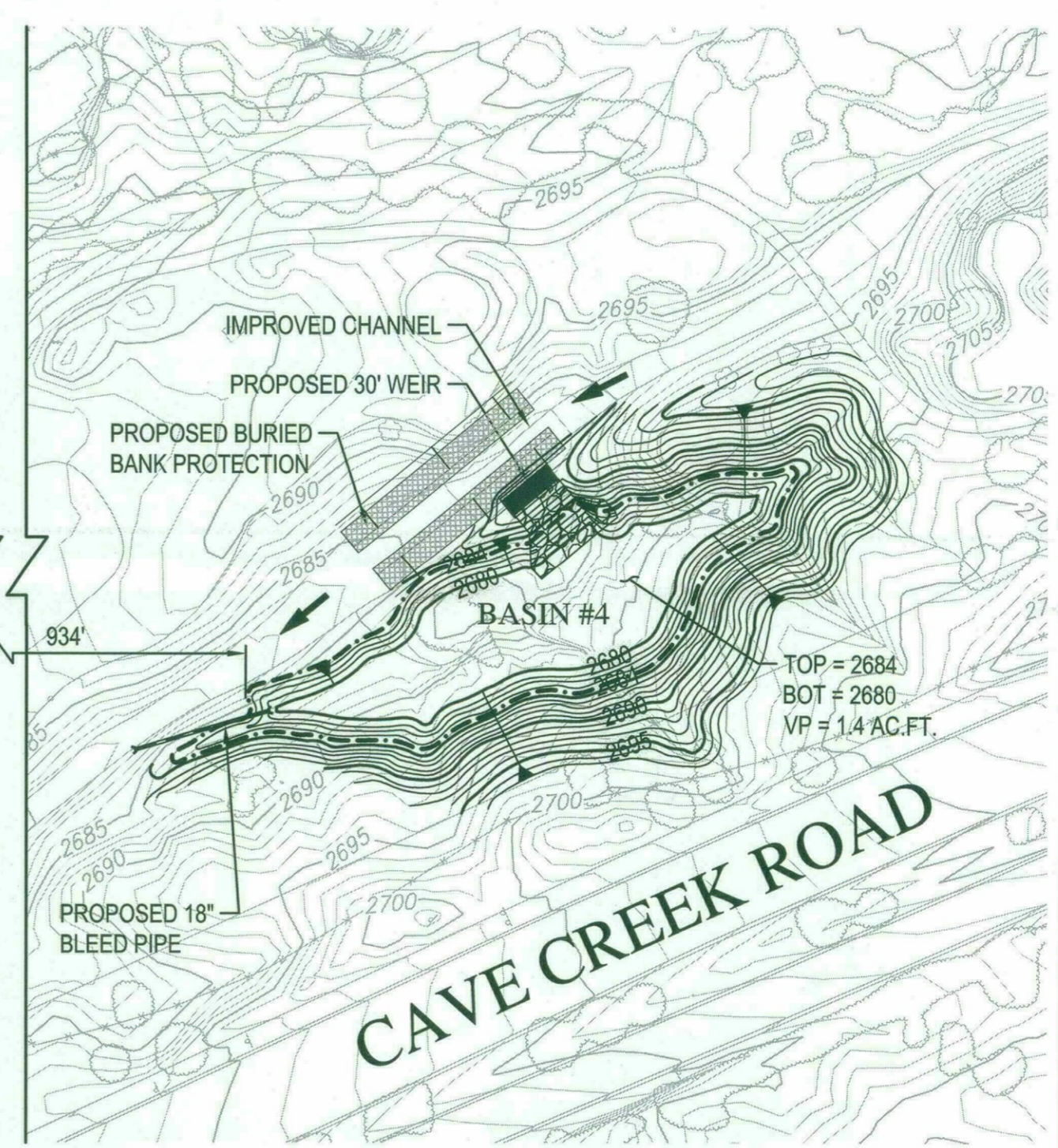
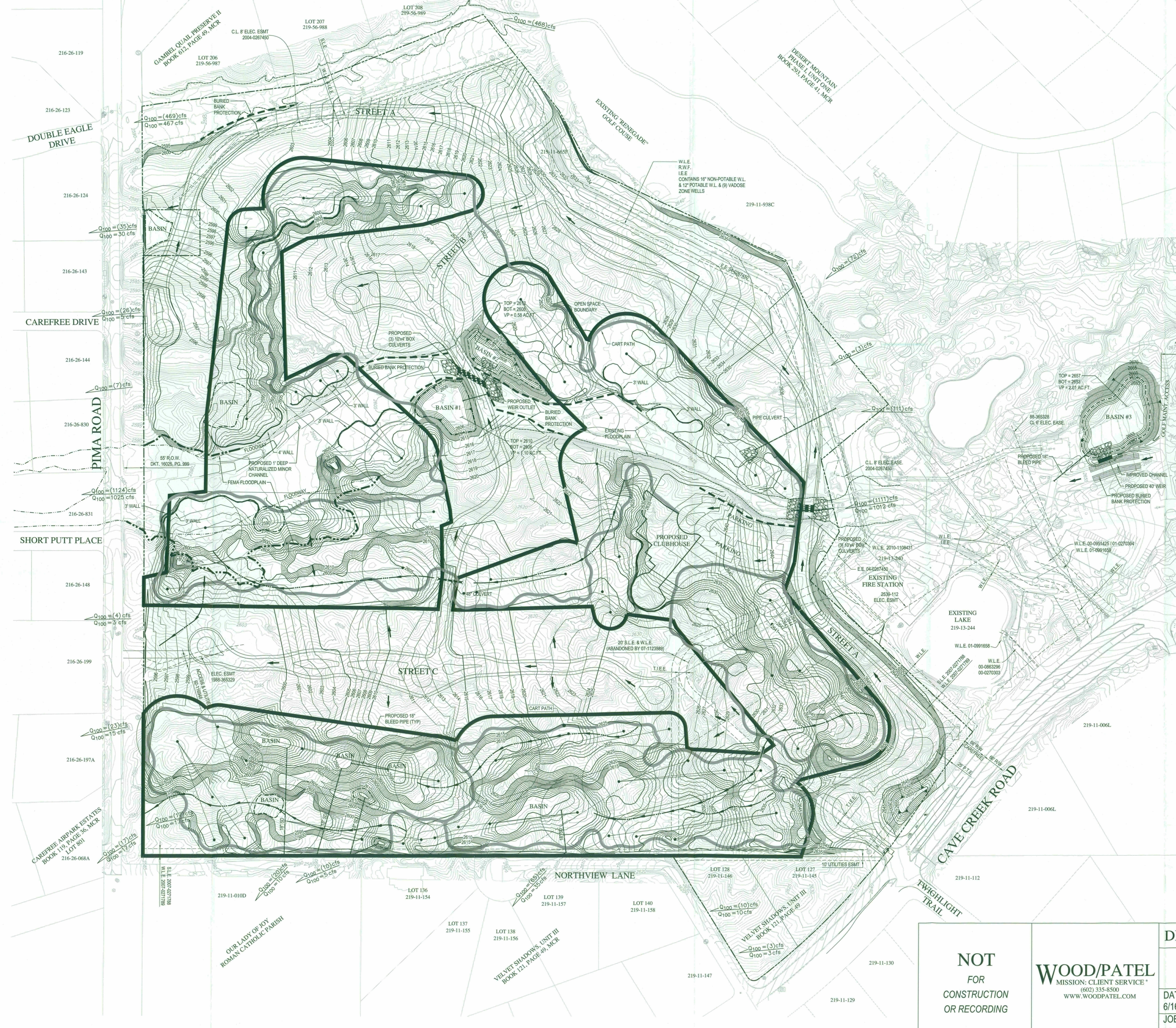
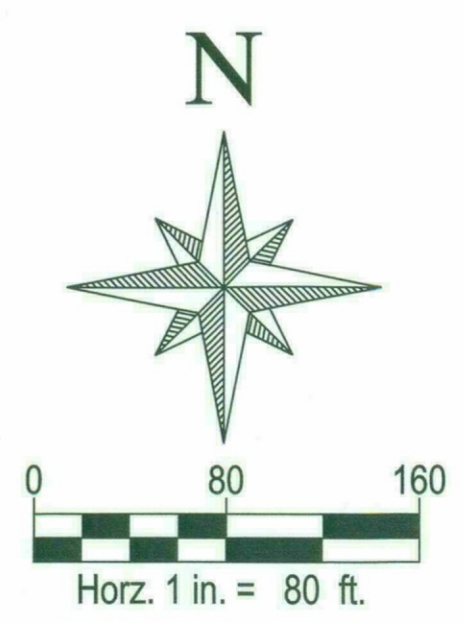


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DESERT MOUNTAIN 19		
EXHIBIT 10 DEVELOPED CONDITIONS HYRAULICS MAP		
DATE: 06-16-2016	SCALE: 1" = 150'	SHEET 1 OF 1
JOB NO.: 164434	DESIGN: JCD	
	DRAWN: JCD	

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 User: jcd
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 Plot Date: 11/20/2016 10:42:15 AM
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 Plot Size: 11.00 x 17.00
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 Plot Linetype: Solid
 Plot Font: Arial
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LEGEND

- OPEN SPACE BOUNDARY
- FEMA FLOODPLAIN
- FEMA FLOODWAY
- PRELIMINARY 100-YEAR FLOODPLAIN
- RETENTION/DETENTION BASINS
- DRAIN FLOW
- BANK PROTECTION
- WALL
- CULVERT W/ HEADWALL
- SURFACE FLOW
- EROSION ROCK
- EXISTING Q₁₀₀
- POST DEVELOPMENT Q₁₀₀

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DESERT MOUNTAIN PARCEL 19

PRELIMINARY GRADING PLAN

DATE 6/16/16	SCALE 1" = 80'	SHEET 1 OF 1
JOB NO. 164434	DESIGN DC DRAWN CD	CHECK DC RFI #

EXHIBIT 8

Developed Conditions Sub-Basin HEC-1 Map

EXHIBIT 9

Existing Conditions Hydraulics Map

EXHIBIT 10

Developed Conditions Hydraulics Map

EXHIBIT 11

Preliminary Grading Plan

contents DESERT MOUNTAIN
PARCEL 19
Appendix E - Digital Files
date 6-16-2016

WARNING: These electronic files are non-certified recordings of printed documents prepared by Wood, Patel & Associates, Inc. (Wood/Patel) and are provided only for the convenience of the receiving party and are intended solely for their exclusive use for the purposes expressly authorized. In accordance with standard industry practice, only printed documents conveyed directly by Wood/Patel may be relied upon. Wood/Patel is not responsible for deterioration or damage to file contents stored herein and use of the information obtained or derived from these electronic files will be at the receiver's sole risk. Please check this disk for virus contamination prior to use.

WP#164434

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