



Development Review (Minor) Staff Approval

226-SA-2017

SDA Academy Scoreboard

APPLICATION INFORMATION

LOCATION:	7410 E Sutton Dr	APPLICANT:	Nathan Chipman-Bonden
PARCEL:	175-04-002A	COMPANY:	Thunderbird Adventist Academy
Q.S.:	32-45	ADDRESS:	7410 E Sutton Dr. Scottsdale, AZ 85260
ZONING:	I-1 & R1-35	PHONE:	(602) 509-0832

Request: To install a scoreboard at an existing softball and baseball field. The front side of the scoreboard will face into the development and face away from existing single family residences to the south and east of the site.

STIPULATIONS

1. The scoreboard architectural elements, dimensions, materials, form, color, and texture, shall be constructed to be consistent with the Scoreboard Elevation Plan, produced by Nelsen Partners, with a City Staff approval date of August 28, 2017.
2. The scoreboard placement, setbacks, and orientation shall adhere to the Scoreboard Site Location Plan, produced by Nelsen Partners, with a City Staff approval date of August 28, 2017.
3. The maximum height of the scoreboard shall not exceed twenty (20) feet from grade.
4. The scoreboard shall utilize non-reflective surfaces or a matte finish.
5. The front face of the scoreboard shall face northwest into the development.
6. Signs and graphics are prohibited on the back side of the scoreboard.
7. A building permit and electrical permit shall be required.

CONSTRUCTION DOCUMENT PLAN REVIEW SUBMITTAL REQUIREMENTS

Submit the following plans and documents to the One Stop Shop for plan review:

PERMIT APPLICATION:	<input checked="" type="checkbox"/>	Completed Permit Application The permit application may be obtained or completed online: http://www.scottsdaleaz.gov/assets/ScottsdaleAZ/Building/APP_Permits/Permit_Commercial.pdf (Please complete the permit application prior to arriving at the One Stop Shop)
ARCHITECTURAL PLANS:	<input checked="" type="checkbox"/>	4 sets of architectural plans, including structural plans and calculations
STAFF APPROVAL LETTER:	<input checked="" type="checkbox"/>	4 copies of this Staff Approval Letter (Case# 226-SA-2017)

Expiration of Development Review (Minor) Approval

This approval expires two (2) years from date of approval if a permit has not been issued, or if no permit is required, work for which approval has been granted has not been completed.

Staff Signature: Andrew Chi Date: August 28, 2017
Andrew Chi, Planner

Planning and Development Services

7447 East Indian School Road, Suite 105, Scottsdale, Arizona 85251 Phone: 480-312-7000 Web: www.scottsdaleaz.gov

August 28, 2017

Scope of work for Thunderbird Adventist Academy

It is the proposed plan for Thunderbird Adventist Academy, to erect a scoreboard for the athletic fields on Thunderbird Campus.

- This scoreboard will be a total height of 24'-0" a.g. and have a width of 14'-0".
- The direction the scoreboard shall be facing is North/West as to provide viewing to each of our three athletic fields.
- Power will be supplied to the location of the scoreboard through 1½" underground conduit carrying stranded 8AWG wire.
- There will be a service disconnect placed at the base of the scoreboard.
- Scoreboard will be installed per engineering that has been supplied with this submittal.
- Any contract work required will be done by licensed, bonded contractors.
- Arizona Blue Stake will be contacted prior to any excavating.
- Holes will be bored by third party.
- Rebar cages provided by third party.

Submitted this 28th of August, 2017

Nathan Chipman-Bonden

Maintenance Director/Facility Manager

Thunderbird Adventist Academy

7410 E. Sutton Dr.

Scottsdale, AZ 85260

M: 602-509-0832

O: 480-948-3300

F: 480-443-4944

Email: nchipman-bonden@thunderbirdacademy.org

28th August, 2017

Authorized Agent Approval

I, Nathan Chipman-Bonden, an agent representing Thunderbird Adventist Academy, do authorize the installation of a scoreboard in proximity of our athletic fields. The installation is approved in relation to the plans and engineering which have been submitted with this letter.

Sincerely,

A handwritten signature in black ink, appearing to read 'Nathan Chipman-Bonden', with a long horizontal line extending to the right.

Nathan Chipman-Bonden

Maintenance Director/Facility Manager

Thunderbird Adventist Academy

7410 E. Sutton Dr.

Scottsdale Az, 85260

M: 602-509-0832

O: 480-948-3300

F: 480-443-4944

Nchipman-bonden@thunderbirdacademy.org

From: Projectinput
Sent: Thursday, April 06, 2017 3:16 PM
To: Projectinput
Subject: Online Pre-Application Submitted (249-PA-2017)



Pre-Application Number: **249-PA-2017**

Project Name: **Ballfield Score Board**
Location: **7410 E SUTTON DR**

Contact Name: **Nathan Chipman-Bonden**
Contact Phone: **(602) 509-0832**
Contact Email: nchipman-bonden@thunderbirdacademy.org



© 2017 City of Scottsdale. All Rights Reserved.



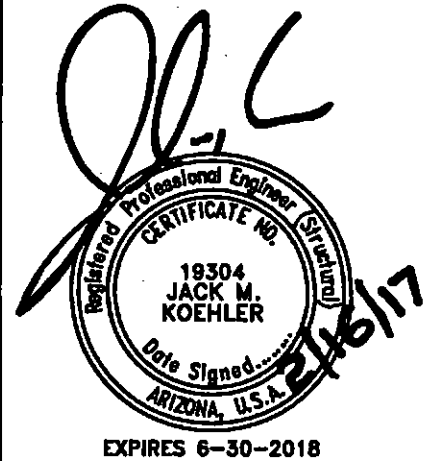
PK ASSOCIATES, LLC.
Consulting Structural Engineers
7434 E. McDonald Dr.
Scottsdale, AZ 85250

Job Name: Thunderbird Academy Score Board
Job No.: 17060 Sheet No.: cover
By: VD Date: 2/16/2017

CLIENT:

Nelsen Partners, Inc.
15210 N. Scottsdale Road, Suite #300
Scottsdale, AZ 85254

PROJECT DESCRIPTION:
THUNDERBIRD ACADEMY
SCORE BOARD
Scottsdale, AZ



BUILDING CODE: 2015 IBC

SOILS DATA: Per IBC 2015 – Sect. 1806

NOTES:

PA ASSOCIATES
7434 E. McDonald Drive
Scottsdale, AZ
480-922-8854

JOB TITLE Thunderbird Academy Score Board
Scottsdale, AZ

JOB NO. _____
CALCULATED BY _____
CHECKED BY _____

SHEET NO. _____
DATE 2/16/17
DATE _____

CS12 Ver 2012.01.24

www.struware.com

STRUCTURAL CALCULATIONS

FOR

Thunderbird Academy Score Board

Scottsdale, AZ

7434 E. McDonald Drive
Scottsdale, AZ
480-922-8854

JOB TITLE Thunderbird Academy Score Board
Scottsdale, AZ

JOB NO. _____ SHEET NO. 2
CALCULATED BY _____ DATE 2/16/17
CHECKED BY _____ DATE _____

www.struware.com

Code Search

Code: ASCE 7 - 10

Occupancy:

Occupancy Group = E Educational

Risk Category & Importance Factors:

Risk Category = III
Wind factor = 1.00
Snow factor = 1.10
Seismic factor = 1.25

Type of Construction:

Fire Rating:
Roof = 0.0 hr
Floor = 0.0 hr

Building Geometry:

Roof angle (θ) 0.00 / 12 0.0 deg
Building length (L) 14.0 ft
Least width (B) 1.0 ft
Mean Roof Ht (h) 17.0 ft
Parapet ht above grd 17.0 ft
Minimum parapet ht 0.0 ft

Live Loads:

Roof
0 to 200 sf: 20 psf
200 to 600 sf: 24 - 0.02Area, but not less than 12 psf
over 600 sf: 12 psf

Floor:

Typical Floor 40 psf
Partitions N/A
Partitions N/A
Partitions N/A
Partitions N/A

Wind Loads :

Ultimate Wind Speed 115 mph
 Directionality (Kd) 0.85
 Exposure Category C
 Enclosure Classif. Open Building
 Internal pressure +/-0.00
 Kh case 1 0.872
 Kh case 2 0.872
 Type of roof Monoslope

Topographic Factor (Kzt)

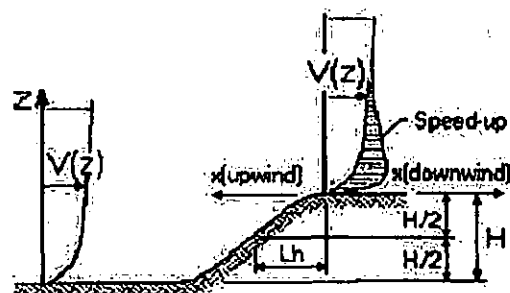
Topography Flat
 Hill Height (H) 0.0 ft
 Half Hill Length (Lh) 0.0 ft
 Actual H/Lh = 0.00
 Use H/Lh = 0.00
 Modified Lh = 0.0 ft
 From top of crest: x = 0.0 ft
 Bldg up/down wind? downwind

H/Lh = 0.00 $K_1 = 0.000$
 x/Lh = 0.00 $K_2 = 0.000$
 z/Lh = 0.00 $K_3 = 1.000$

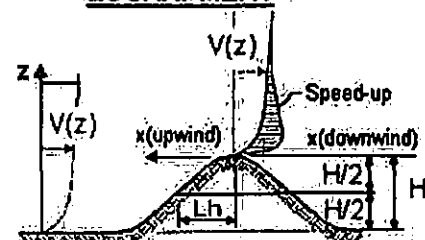
At Mean Roof Ht:

$$K_{zt} = (1 + K_1 K_2 K_3)^2 = 1.00$$

$H < 15 \text{ ft; exp C}$
 $K_{zt} = 1.0$



ESCARPMENT



2D RIDGE or 3D AXISYMMETRICAL HILL

Gust Effect Factor

h = 17.0 ft
 B = 1.0 ft
 lz (0.6h) = 15.0 ft

Flexible structure if natural frequency < 1 Hz (T > 1 second).

However, if building h/B < 4 then probably rigid structure (rule of thumb).

h/B = 17.00 May be flexible structure

G = 0.85 Using rigid structure default

Rigid Structure

$\bar{\theta} = 0.20$
 $z = 500 \text{ ft}$
 $z_{min} = 15 \text{ ft}$
 $c = 0.20$
 $g_Q, g_v = 3.4$
 $L_z = 427.1 \text{ ft}$
 $Q = 0.96$
 $I_z = 0.23$
 $G = 0.90 \text{ use } G = 0.85$

Flexible or Dynamically Sensitive Structure

Natural Frequency (η_1) = 0.0 Hz

Damping ratio (β) = 0

$f_b = 0.65$

$f_a = 0.15$

$V_z = 97.1$

$N_1 = 0.00$

$K_n = 0.000$

$K_h = 28.282$

$K_B = 28.282$

$K_L = 28.282$

$g_R = 0.000$

$R = 0.000$

$G = 0.000$

$\eta = 0.000$

$\eta = 0.000$

$\eta = 0.000$

h = 17.0 ft

Associates
7434 E. McDonald Drive
Scottsdale, AZ
480-922-8854

JOB TITLE Thunderbird Academy Score Board

Scottsdale, AZ

JOB NO.

SHEET NO. **4**

CALCULATED BY

DATE

2/16/17

CHECKED BY

DATE

Enclosure Classification

Test for Enclosed Building: A building that does not qualify as open or partially enclosed.

Test for Open Building:

All walls are at least 80% open.

$$A_o \geq 0.8A_g$$

Test for Partially Enclosed Building:

Input		Test	
Ao	0.0 sf	$A_o \geq 1.1A_{oi}$	YES
Ag	0.0 sf	$A_o > 4'$ or $0.01A_g$	NO
Aoi	0.0 sf	$A_{oi} / A_{gi} \leq 0.20$	NO
Agi	0.0 sf		

Building is NOT Partially Enclosed

Conditions to qualify as Partially Enclosed Building. Must satisfy all of the following:

$$A_o \geq 1.1A_{oi}$$

$$A_o > \text{smaller of } 4' \text{ or } 0.01 A_g$$

$$A_{oi} / A_{gi} \leq 0.20$$

Where:

Ao = the total area of openings in a wall that receives positive external pressure.

Ag = the gross area of that wall in which Ao is identified.

Aoi = the sum of the areas of openings in the building envelope (walls and roof) not including Ao.

Agi = the sum of the gross surface areas of the building envelope (walls and roof) not including Ag.

Reduction Factor for large volume partially enclosed buildings (Ri) :

If the partially enclosed building contains a single room that is unpartitioned, the internal pressure coefficient may be multiplied by the reduction factor Ri.

Total area of all wall & roof openings (Aog):

0 sf

Unpartitioned internal volume (Vi) :

0 cf

$$R_i = 1.00$$

Altitude adjustment to constant 0.00256 (caution - see code) :

Altitude = 0 feet

Constant = 0.00256

Average Air Density = 0.0765 lbm/ft³

Associates
7434 E. McDonald Drive
Scottsdale, AZ
480-922-8854

JOB TITLE Thunderbird Academy Score Board

Scottsdale, AZ

JOB NO.

SHEET NO.

CALCULATED BY

DATE

2/16/17

CHECKED BY

DATE

Wind Loads - Other Structures:

Importance Factor = 1.00
Gust Effect Factor (G) = 0.85
Kzt = 1.00
Wind Speed = 115 mph
Exposure = C

WIND
PRESSURE
APPLIES TO
SCORE BOARD

A. Solid Freestanding Walls & Solid Signs (& open signs with less than 30% open)

Dist to sign top (h) 24.0 ft
Height (s) 14.0 ft
Width (B) 14.0 ft
Wall Return (Lr) =
Directionality (Kd) 0.85
Percent of open area
to gross area 0.0%

s/h = 0.58
B/s = 1.00
Lr/s = 0.00
Kz = 0.937
qz = 27.0 psf

Open reduction
factor = 1.00

Case A & B

C_r = 1.71
F = q_z G C_f A_s = 39.2 A_s
A_s = 10.0 sf
F = 392 lbs

Case C

Case C reduction factors

Factor if s/h > 0.8 = 1.00
Wall return factor
for C_f at 0 to s = 1.00

Horiz dist from
windward edge

C_f F = q_z G C_f A_s (psf)
0 to s 2.25 51.6 A_s
s to 2s 1.50 34.4 A_s

B. Open Signs & Lattice Frameworks (openings 30% or more of gross area)

Height to centroid of A_f (z) 15.0 ft
Width (zero if round) 0.0 ft
Diameter (zero if rect) 2.0 ft
Percent of open area
to gross area 35.0%
Directionality (Kd) 0.85

D(qz)^{0.5} = 9.89
I = 0.65
C_r = 1.1

Kz = 0.849
Base pressure (qz) = 24.4 psf
F = q_z G C_r A_f = 22.8 A_f
Solid Area: A_f = 10.0 sf
F = 228 lbs

C. Chimneys, Tanks & Similar Structures

Height to centroid of A_f (z) 15.0 ft
Cross-Section Square
Directionality (Kd) 0.90
Height (h) 15.0 ft
Width (D) 1.0 ft
Type of Surface N/A

Kz = 0.849
Base pressure (qz) = 25.9 psf
h/D = 15.00

Square (wind along diagonal)

C_f = 1.28
F = q_z G C_f A_f = 28.1 A_f
A_f = sf
F = 0 lbs

Square (wind normal to face)

C_r = 1.67
F = q_z G C_r A_f = 36.6 A_f
A_f = 10.0 sf
F = 366 lbs

D. Trussed Towers

Height to centroid of A_f (z) 15.0 ft
ε = 0.27
Tower Cross Section square
Member Shape flat
Directionality (Kd) 1.00

Kz = 0.849
Base pressure (qz) = 28.7 psf
Diagonal wind factor = 1.2
Round member factor = 1.000

Square (wind along tower diagonal)

C_f = 3.24
F = q_z G C_f A_f = 79.1 A_f
Solid Area: A_f = 10.0 sf

Square (wind normal to face)

C_r = 2.70
F = q_z G C_r A_f = 65.9 A_f



PK Associates, LLC

Job Name THUNDERBIRD ACADEMY

SCORE BOARD
Job No. 17060 Sheet No. 6

By KS Date 02/16/17

SCORE BOARD SUPPORT COLUMN.

HEIGHT = 24'-0"

$$PT1 = (14\frac{1}{2} \times 14' \times 25 \text{ psf})$$

← SCORE BOARD WEIGHT.

$$= 2450 \# (W)$$

$$W = (39.2 \text{ psf}) (14\frac{1}{2})$$

$$= 275 \text{ plf (WIND)}$$

- USE W12X26 (SEE ATTACHED COMP. OUTPUT).

SCORE BOARD COLUMN CONCRETE BASE.

$$PT1 = 2450 \# \leftarrow (\text{SEE ABV.})$$

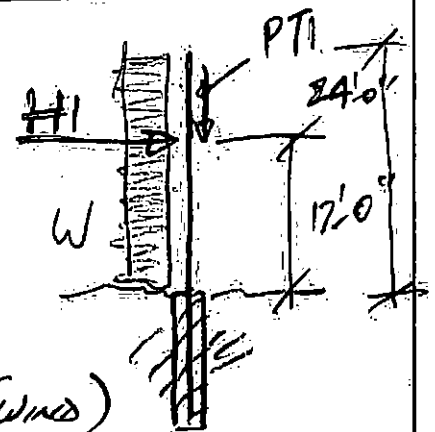
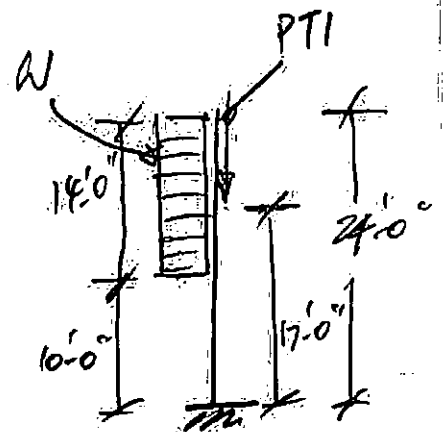
$$W = (6.5/12) (39.2 \text{ psf}) = 21.2 \text{ plf (WIND)}$$

$$H1 = (39.2 \text{ psf}) (14' \times 14\frac{1}{2}) = 3.84 \text{ k (WIND)}$$

- USE 36" ϕ X 9'-6" DEEP
CONC. BASE W/ 16 #6 VERT.

#4 TIE AT

8" O.C. (SEE ATTACHED
COMP. OUTPUT)



You can change this area
using the "Settings" menu item
and then using the "Printing &
Title Block" selection.
Title Block Line 6

Project Title:
Engineer:
Project Descr:

Project ID:

7

Printed: 16 FEB 2017, 4:21PM

Steel Column

File = c:\Users\WID\Desktop\0112-1-17\2017DE-1\THUNDE-1\EC6

ENERCALC, INC. 1983-2017, Build: 6.17.1.16, Ver: 6.17.1.16

Lic. #: KW-06004221

Licensee: PAUL KOEHLER ENGINEERS

Description: Score Board Support Columns

Code References

Calculations per AISC 360-10, IBC 2012, CBC 2013, ASCE 7-10

Load Combinations Used: IBC 2015

General Information

Steel Section Name :	W12x26	Overall Column Height	24.0 ft
Analysis Method :	Allowable Strength	Top & Bottom Fixity	Top Free, Bottom Fixed
Steel Stress Grade		Brace condition for deflection (buckling) along columns :	
Fy : Steel Yield	50.0 ksi	X-X (width) axis :	
E : Elastic Bending Modulus	29,000.0 ksi	Unbraced Length for X-X Axis buckling = 10 ft, K = 2.10	
		Y-Y (depth) axis :	
		Unbraced Length for Y-Y Axis buckling = 24.0 ft, K = 2.10	

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Column self weight included : 624.0 lbs * Dead Load Factor
AXIAL LOADS ...

Axial Load at 17.0 ft, Yecc = 7.50 in, D = 2.450 k

BENDING LOADS ...

Lat. Uniform Load from 10.0-->24.0 ft creating Mx-x, W = 0.2750 k/ft

DESIGN SUMMARY

Bending & Shear Check Results

PASS Max. Axial+Bending Stress Ratio =	0.6068 : 1	Maximum SERVICE Load Reactions ..	
Load Combination	+D+0.60W+H	Top along X-X	0.0 k
Location of max. above base	0.0 ft	Bottom along X-X	0.0 k
At maximum location values are ...		Top along Y-Y	0.0 k
Pa : Axial	3.074 k	Bottom along Y-Y	3.850 k
Pn / Omega : Allowable	41.285 k	Maximum SERVICE Load Deflections ...	
Ma-x : Applied	-40.801 k-ft	Along Y-Y	3.030 in at 24.0 ft above base
Mn-x / Omega : Allowable	71.639 k-ft	for load combination : W Only	
Ma-y : Applied	0.0 k-ft	Along X-X	0.0 in at 0.0 ft above base
Mn-y / Omega : Allowable	20.384 k-ft	for load combination :	
PASS Maximum Shear Stress Ratio =	0.04116 : 1		
Load Combination	+0.60D-0.60W+0.60H		
Location of max. above base	0.0 ft		
At maximum location values are ...			
Va : Applied	2.310 k		
Vn / Omega : Allowable	56.120 k		

Load Combination Results

Load Combination	Maximum Axial + Bending Stress Ratios			Maximum Shear Ratios		
	Stress Ratio	Status	Location	Stress Ratio	Status	Location
+D+H	0.059	PASS	24.00 ft	0.000	PASS	0.00 ft
+D+L+H	0.059	PASS	24.00 ft	0.000	PASS	0.00 ft
+D+Lr+H	0.059	PASS	24.00 ft	0.000	PASS	0.00 ft
+D+S+H	0.059	PASS	24.00 ft	0.000	PASS	0.00 ft
+D+0.750Lr+0.750L+H	0.059	PASS	24.00 ft	0.000	PASS	0.00 ft
+D+0.750L+0.750S+H	0.059	PASS	24.00 ft	0.000	PASS	0.00 ft
+D+0.60W+H	0.607	PASS	0.00 ft	0.041	PASS	0.00 ft
+D-0.60W+H	0.564	PASS	0.00 ft	0.041	PASS	0.00 ft
+D+0.70E+H	0.059	PASS	24.00 ft	0.000	PASS	0.00 ft
+D-0.70E+H	0.059	PASS	24.00 ft	0.000	PASS	0.00 ft
+D+0.750Lr+0.750L+0.450W+H	0.470	PASS	0.00 ft	0.031	PASS	0.00 ft
+D+0.750Lr+0.750L+0.450W+H	0.427	PASS	0.00 ft	0.031	PASS	0.00 ft
+D+0.750L+0.750S+0.450W+H	0.470	PASS	0.00 ft	0.031	PASS	0.00 ft
+D+0.750L+0.750S+0.450W+H	0.427	PASS	0.00 ft	0.031	PASS	0.00 ft
+D+0.750L+0.750S+0.5250E+H	0.059	PASS	24.00 ft	0.000	PASS	0.00 ft

Title Block Line 6

Project Title:
Engineer:
Project Descr:

Project ID:

⑧

Printed: 16 FEB 2017 4:21 PM

Steel Column

File = c:\Users\VID\Desktop\0112-117\2017DE-11THUNDEE\1THUNDE-1.EC8
ENERCAL C-INC 1992-2017 R-8108-17-16 DU-817116

Lic. # KW-06004221

Licensee: PAUL KOEHLER ENGINEERS

Description :	Score Board Support Columns
---------------	-----------------------------

Load Combination Results

Load Combination	Maximum Axial + Bending Stress Ratios			Maximum Shear Ratios		
	Stress Ratio	Status	Location	Stress Ratio	Status	Location
+D+0.750L+0.750S-0.5250E+H	0.059	PASS	24.00 ft	0.000	PASS	0.00 ft
+0.60D+0.60W+0.60H	0.583	PASS	0.00 ft	0.041	PASS	0.00 ft
+0.60D-0.60W+0.60H	0.558	PASS	0.00 ft	0.041	PASS	0.00 ft
+0.60D+0.70E+0.60H	0.036	PASS	24.00 ft	0.000	PASS	0.00 ft
+0.60D-0.70E+0.60H	0.036	PASS	24.00 ft	0.000	PASS	0.00 ft

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	X-X Axis Reaction		Y-Y Axis Reaction		Axial Reaction
	@ Base	@ Top	@ Base	@ Top	@ Base
+D+H		k		k	3.074 k
+D+L+H		k		k	3.074 k
+D+Lr+H		k		k	3.074 k
+D+S+H		k		k	3.074 k
+D+0.750Lr+0.750L+H		k		k	3.074 k
+D+0.750L+0.750S+H		k		k	3.074 k
+D+0.60W+H		k	2.310	k	3.074 k
+D-0.60W+H		k	-2.310	k	3.074 k
+D+0.70E+H		k		k	3.074 k
+D-0.70E+H		k		k	3.074 k
+D+0.750Lr+0.750L+0.450W+H		k	1.733	k	3.074 k
+D+0.750Lr+0.750L-0.450W+H		k	-1.733	k	3.074 k
+D+0.750L+0.750S+0.450W+H		k	1.733	k	3.074 k
+D+0.750L+0.750S-0.450W+H		k	-1.733	k	3.074 k
+D+0.750L+0.750S+0.5250E+H		k		k	3.074 k
+D+0.750L+0.750S-0.5250E+H		k		k	3.074 k
+0.60D+0.60W+0.60H		k	2.310	k	1.844 k
+0.60D-0.60W+0.60H		k	-2.310	k	1.844 k
+0.60D+0.70E+0.60H		k		k	1.844 k
+0.60D-0.70E+0.60H		k		k	1.844 k
D Only		k		k	3.074 k
Lr Only		k		k	k
L Only		k		k	k
S Only		k		k	k
W Only		k	3.850	k	k
-W		k	-3.850	k	k
E Only		k		k	k
E Only * -1.0		k		k	k
H Only		k		k	k

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
+D+H	0.0000 in	0.000 ft	0.117 in	24.000 ft
+D+L+H	0.0000 in	0.000 ft	0.117 in	24.000 ft
+D+Lr+H	0.0000 in	0.000 ft	0.117 in	24.000 ft
+D+S+H	0.0000 in	0.000 ft	0.117 in	24.000 ft
+D+0.750Lr+0.750L+H	0.0000 in	0.000 ft	0.117 in	24.000 ft
+D+0.750L+0.750S+H	0.0000 in	0.000 ft	0.117 in	24.000 ft
+D+0.60W+H	0.0000 in	0.000 ft	1.935 in	24.000 ft
+D-0.60W+H	0.0000 in	0.000 ft	-1.701 in	24.000 ft
+D+0.70E+H	0.0000 in	0.000 ft	0.117 in	24.000 ft
+D-0.70E+H	0.0000 in	0.000 ft	0.117 in	24.000 ft
+D+0.750Lr+0.750L+0.450W+H	0.0000 in	0.000 ft	1.481 in	24.000 ft
+D+0.750Lr+0.750L-0.450W+H	0.0000 in	0.000 ft	-1.246 in	24.000 ft
+D+0.750L+0.750S+0.450W+H	0.0000 in	0.000 ft	1.481 in	24.000 ft
+D+0.750L+0.750S-0.450W+H	0.0000 in	0.000 ft	-1.246 in	24.000 ft
+D+0.750L+0.750S+0.5250E+H	0.0000 in	0.000 ft	0.117 in	24.000 ft
+D+0.750L+0.750S-0.5250E+H	0.0000 in	0.000 ft	0.117 in	24.000 ft
+0.60D+0.60W+0.60H	0.0000 in	0.000 ft	1.888 in	24.000 ft

You can change this area
using the "Settings" menu item
and then using the "Printing &
Title Block" selection.

Project Title:
Engineer:
Project Descr:

Project ID:

9

Printed: 16 FEB 2017, 4:21 PM

Steel Column

File: C:\Users\VID\Desktop\0112-1\17\2017DE-1\THUNDE-1\THUNDE-1.EC8
GENERALC\INC\1983-2017_Bldgs\17.1.16_Ver3.17.1.16

License #: KW:06004221

Licensee: PAUL KOEHLER ENGINEERS

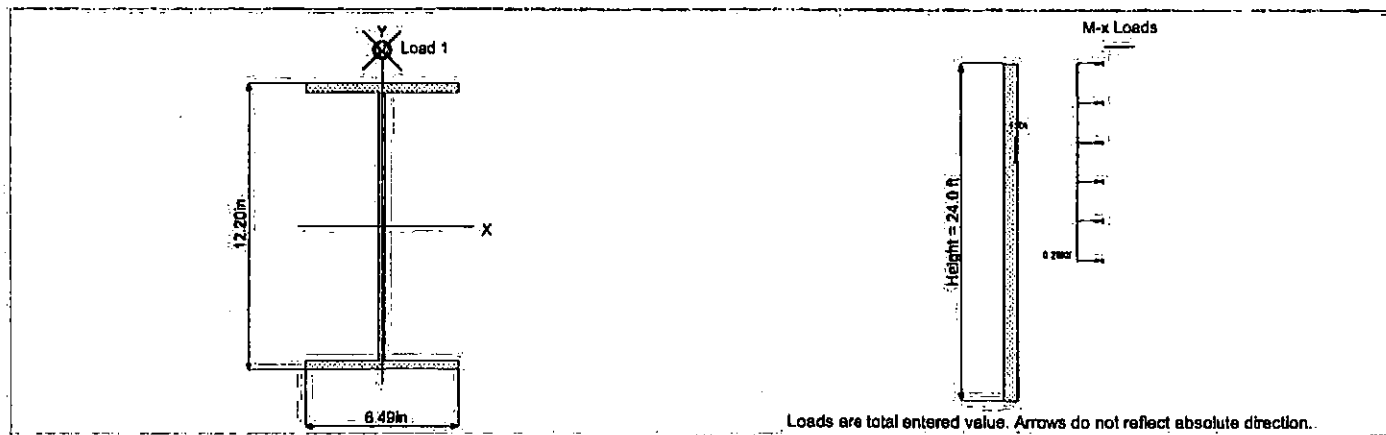
Description: Score Board Support Columns

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
+0.60D-0.60W+0.60H	0.0000 in	0.000 ft	-1.747 in	24.000 ft
+0.60D+0.70E+0.60H	0.0000 in	0.000 ft	0.070 in	24.000 ft
+0.60D-0.70E+0.60H	0.0000 in	0.000 ft	0.070 in	24.000 ft
D Only	0.0000 in	0.000 ft	0.117 in	24.000 ft
Lr Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
L Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
S Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
W Only	0.0000 in	0.000 ft	3.030 in	24.000 ft
-W	0.0000 in	0.000 ft	-3.030 in	24.000 ft
E Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
E Only * -1.0	0.0000 in	0.000 ft	0.000 in	0.000 ft
H Only	0.0000 in	0.000 ft	0.000 in	0.000 ft

Steel Section Properties: W12x26

Depth	=	12.200 in	Ixx	=	204.00 in ⁴	J	=	0.300 in ⁴
Web Thick	=	0.230 in	Sxx	=	33.40 in ³	Cw	=	607.00 in ⁶
Flange Width	=	6.490 in	Rxx	=	5.170 in			
Flange Thick	=	0.380 in	Zx	=	37.200 in ³			
Area	=	7.650 in ²	Iyy	=	17.300 in ⁴			
Weight	=	26.000 plf	Syy	=	5.340 in ³	Wno	=	19.200 in ²
Kdesign	=	0.680 in	Ryy	=	1.510 in	Sw	=	11.800 in ⁴
K1	=	0.750 in	Zy	=	6.170 in ³	Qf	=	7.030 in ³
rts	=	1.750 in	rT	=	0.000 in	Qw	=	18.300 in ³
Ycg	=	0.000 in						



You can change this area
using the "Settings" menu item
and then using the "Printing &
Title Block" selection.

Project Title:
Engineer:
Project Descr:

Project ID:

10

Title Block Line 6

Printed: 16 FEB 2017, 4:27 PM

Pole Footing Embedded in Soil

File: C:\Users\WID\Desktop\01125117\201702-INTHUNDE-INTHUNDE-1.EC8
ENERCALC, INC., 1983-2017, Build 6.17.1.18, Ver 6.17.1.18

License: KW06004224

Licensee: PAUL KOEHLER ENGINEERS

Description: Score Board Column Concrete Base.

Code References

Calculations per IBC 2012 1807.3, CBC 2013, ASCE 7-10
Load Combinations Used: IBC 2015

General Information

Pole Footing Shape: Circular
Pole Footing Diameter: 36.0 in
Calculate Min. Depth for Allowable Pressures
No Lateral Restraint at Ground Surface
Allow Passive: 200.0 pcf
Max Passive: 1,500.0 pcf

Controlling Values

Governing Load Combination: +D+0.60W+H

Lateral Load: 2.606 k
Moment: 42.797 k-ft

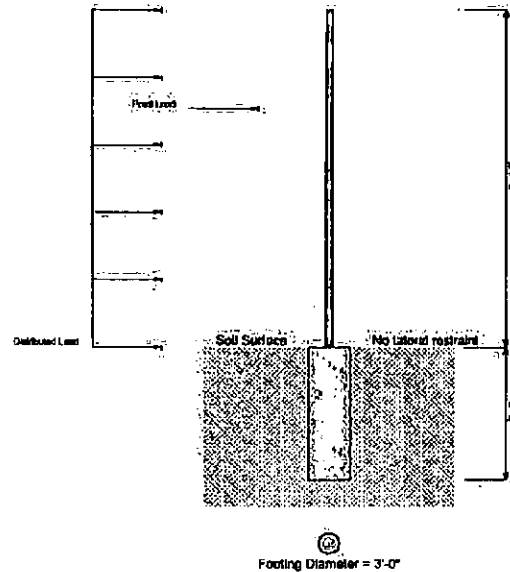
NO Ground Surface Restraint

Pressures at 1/3 Depth

Actual: 626.66 psf
Allowable: 627.52 psf

Minimum Required Depth: 9.50 ft

Footing Base Area: 7.069 ft²
Maximum Soil Pressure: 0.3466 ksf



Applied Loads

Lateral Concentrated Load (k)		Lateral Distributed Loads (klf)		Vertical Load (k)	
D: Dead Load	k		k/ft	2,450	k
Lr: Roof Live	k		k/ft		k
L: Live	k		k/ft		k
S: Snow	k		k/ft		k
W: Wind	3.840 k	0.0210	k/ft		k
E: Earthquake	k		k/ft		k
H: Lateral Earth	k		k/ft		k
Load distance above ground surface	17.0 ft	TOP of Load above ground surface	24.0		
		BOTTOM of Load above ground surface	ft		

Load Combination Results

Load Combination	Forces @ Ground Surface		Required Depth (ft)	Pressure at 1/3 Depth		Soil Increase Factor
	Loads (k)	Moments (ft-k)		Actual (psf)	Allow (psf)	
+D+H	0.000	0.000	0.13	0.0	0.0	1.000
+D+L+H	0.000	0.000	0.13	0.0	0.0	1.000
+D+Lr+H	0.000	0.000	0.13	0.0	0.0	1.000
+D+S+H	0.000	0.000	0.13	0.0	0.0	1.000
+D+0.750Lr+0.750L+H	0.000	0.000	0.13	0.0	0.0	1.000
+D+0.750L+0.750S+H	0.000	0.000	0.13	0.0	0.0	1.000
+D+0.60W+H	2.606	42.797	9.50	626.7	627.5	1.000
+D-0.60W+H	2.606	42.797	9.50	626.7	627.5	1.000
+D+0.70E+H	0.000	0.000	0.13	0.0	0.0	1.000

You can change this area
using the "Settings" menu item
and then using the "Printing &
Title Block" selection.

Project Title:
Engineer:
Project Descr:

Project ID:

01

Printed: 16 FEB 2017, 4:27PM

Pole Footing Embedded in Soil

File = c:\Users\VIDA\Desktop\0112-17\2017DE-1\THUNDE-1\THUNDE-1.EC8

ENERCALC, INC. 1983-2017, Build: 17.1.16, Ver: 17.1.16

Lic #: KW:06004221

Licensee: PAUL KOEHLER ENGINEERS

Description : Score Board Column Concrete Base.

+D-0.70E+H	0.000	0.000	0.13	0.0	0.0	1.000
+D+0.750Lr+0.750L+0.450W+H	1.955	32.098	8.50	562.2	563.6	1.000
+D+0.750Lr+0.750L-0.450W+H	1.955	32.098	8.50	562.2	563.6	1.000
+D+0.750L+0.750S+0.450W+H	1.955	32.098	8.50	562.2	563.6	1.000
+D+0.750L+0.750S-0.450W+H	1.955	32.098	8.50	562.2	563.6	1.000
+D+0.750L+0.750S+0.5250E+H	0.000	0.000	0.13	0.0	0.0	1.000
+D+0.750L+0.750S-0.5250E+H	0.000	0.000	0.13	0.0	0.0	1.000
+0.60D+0.60W+0.60H	2.606	42.797	9.50	626.7	627.5	1.000
+0.60D-0.60W+0.60H	2.606	42.797	9.50	626.7	627.5	1.000
+0.60D+0.70E+0.60H	0.000	0.000	0.13	0.0	0.0	1.000
+0.60D-0.70E+0.60H	0.000	0.000	0.13	0.0	0.0	1.000



PK Associates, LLC

Job Name THUNDERBIRD ACADEMY

SCORE BOARD
Job No. 17060 Sheet No. 12

By VD Date 02/16/17

APPENDIX

FOR REVIEW ONLY

DAKTRONICS MS-918 PRODUCT SPECIFICATIONS



This outdoor LED multisport scoreboard displays period time to 99:59, HOME and GUEST scores to 99 and INNING or PERIOD to nine with included reversible caption panel. Indicators show BALL, STRIKE, OUT, H (hit) and E (error) in baseball mode. When period time is less than one minute, the scoreboard displays time to 1/10 of a second. Scoreboard shown with optional striping and amber PanaView® digits.

		VINYL CAPTIONS (STANDARD)	TNMCs & VINYL CAPTIONS
POWER (120 VAC)*	Red/Amber Digits	120 Watts, 1 Amp	390 Watts, 3.3 Amps
	White Digits	260 Watts, 2.2 Amps	530 Watts, 4.4 Amps
UNCRATED WEIGHT		265 lb (120 kg)	345 lb (156 kg)
DIMENSIONS		5'-0" H x 14'-0" W x 8" D (1.52 m, 4.27 m, 203 mm)	

*Scoreboard requires a dedicated circuit. Models with 240 VAC power at half the indicated amperage are also offered (International Use Only).

DIGITS & INDICATORS

- INNING/PERIOD digit is 15" (381 mm) high. All other digits are 18" (457 mm) high. All indicators are 2" (51 mm) in diameter.
- Select red, amber, or white LED digits and indicators.
- Scoreboard features robust weather-sealed digits (see DD2495646).
- Digits may be dimmed for night viewing.

CAPTIONS

- HOME and GUEST captions are 10" (254 mm) high. All other captions are 8" (203 mm) high.
- Standard captions are vinyl, applied to the display face. INNING and PERIOD captions are on a reversible panel.
- Optional TNMCs are 10.6" (269 mm) high.

DISPLAY COLOR

Choose from 150+ colors (from Martin Senour® paint book) at no additional cost.

CONSTRUCTION

Alcoa aluminum alloy 5052 for excellent corrosion resistance

PRODUCT SAFETY APPROVAL

ETL listed to UL 48, tested to CSA standards, and CE labeled

OPERATING TEMPERATURES

- Display: -22° to 122° Fahrenheit (-30° to 50° Celsius)
- Console: 32° to 130° Fahrenheit (0° to 54° Celsius)

WWW.DAKTRONICS.COM E-MAIL: SALES@DAKTRONICS.COM

201 Daktronics Drive, PO Box 5128, Brookings, SD 57006
Phone: 1-800-325-8766 or 605-692-0200 Fax: 605-697-4746
DD2167408 050616 Page 1 of 1



DAKTRONICS MS-918 PRODUCT SPECIFICATIONS

CONTROL CONSOLES	CONTROL OPTIONS
All Sport® 1600* (see SL-04352) <i>*May be upgraded to All Sport 5000 (see SL-03991)</i>	Wired (standard): One-pair shielded cable of 22 AWG minimum is required. A cover plate with mounted connector and standard 2" x 4" x 2" (51 mm x 102 mm x 51 mm) outlet box is provided. Connector mates with signal cable from control console. Wireless (optional): 2.4 GHz spread spectrum radio features 64 non-interfering channels and 8 broadcast groups (see SL-04370).
RC-100 (see SL-07397)	Optional wireless handheld controller features 900 MHz spread spectrum radio with 15 non-interfering channels and up to 10 hours of operation via internal rechargeable battery.

Note: All Sport 5000 required for Team Name Message Centers.

SEGMENT TIMER MODE

The segment timer mode is ideal for keeping practices on schedule. The horn at the end of a segment allows coaches and athletes to focus on the practice and to listen for the horn when it is time to change drills (see [SL-04004](#)).

TIME OF DAY MODE

This scoreboard features a Time of Day (TOD) mode that allows it to act as a clock when the control console is unplugged or off. Refer to the scoreboard installation manual for instructions on how to enable the Time of Day mode.

MOUNTING

Scoreboard is typically mounted on two vertical beams or poles. Hardware to mount scoreboard on two beams is included; hardware for more beams is at additional cost. Standard mounting uses I-beam clamps. Optional mounting method using angle brackets is also offered; maximum beam width is 12" (305 mm) and maximum beam depth is 22" (559 mm). Refer to attached drawings for more information on mounting methods.

SERVICE ACCESS

Digit panels and electronics are serviced from the front of the scoreboard.

GENERAL INFORMATION

Scoreboard provides scoring capabilities for two teams. 100% solid state electronics are housed in an all aluminum cabinet. Scoreboard is shipped in one section. Scoreboard power is to be provided on a dedicated circuit to prevent loss of game information due to failure of another component on the circuit. Specifications and pricing are subject to change without notice.

ADVERTISING/IDENTIFICATION PANELS

Backlit & Non-Backlit:

- 1'-6" H x 14'-0" W (457 mm, 4.27 m)
- 2'-0" H x 14'-0" W (610 mm, 4.27 m)
- 2'-6" H x 14'-0" W (762 mm, 4.27 m)

For additional non-backlit panel sizes, see [SL-03761](#).

OPTIONS & ACCESSORIES

- Scoreboard border striping
- Multiple caption and striping colors (see [DD2101644](#))
- Team name caption in place of HOME *
- Team names on changeable panels *
- Programmable Team Name Message Centers (see [DD1696958](#))
- Reversible HALF and QTR caption panel
- Horn
- Individual digit protective screens (see [SL-04939](#))
- Protective netting (see [DD2690927](#))
- Optional angle bracket mounting method
- Advertising/identification panels
- Decorative accents
- Electronic message centers and video displays in multiple sizes

* Not available with TNMCs

FOR ADDITIONAL INFORMATION

- Installation Specifications: DWG-1157187 (attached)
- Standard I-beam Mounting: DWG-1052565 (attached)
- Optional Pole Mounting: DWG-1048184 (attached)
- Component Locations: DWG-1074633 (attached)
- Architectural Specifications: See [SL-05167](#)



DAKTRONICS MS-918 PRODUCT SPECIFICATIONS

ALTERNATE CAPTIONS & SCORING MODES

Standard Vinyl Captions
(PERIOD & INNING on included caption panel)

HOME	12:47	GUEST
4	INNING 3	1
BALL	STRIKE	OUT H E

Baseball Mode

HOME	9:36	GUEST
10	PERIOD 2	7
BALL	STRIKE	OUT H E

Lacrosse/Field Hockey Mode

Optional Vinyl Captions on Reversible Panel

HOME	7:23	GUEST
21	QTR 3	14
BALL	STRIKE	OUT H E

Football Mode

HOME	37:58	GUEST
2	HALF 2	1
BALL	STRIKE	OUT H E

Soccer Mode



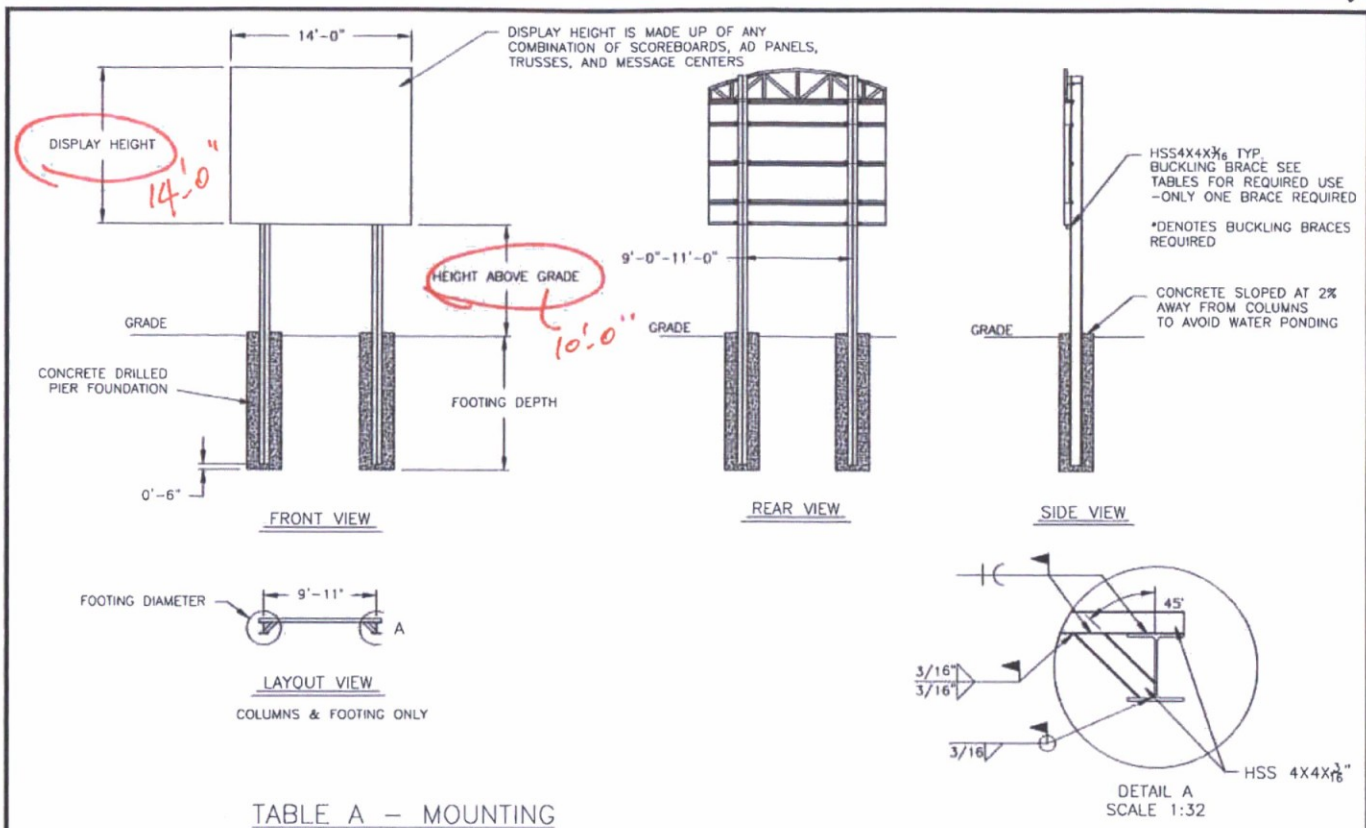


TABLE A - MOUNTING

EXPOSURE B						EXPOSURE C					
HEIGHT ABOVE GRADE = 10'						HEIGHT ABOVE GRADE = 15'					
DISPLAY HEIGHT (FT)		DESIGN WIND VELOCITY				DISPLAY HEIGHT (FT)		DESIGN WIND VELOCITY			
		115 MPH	130 MPH	150 MPH	170 MPH			115 MPH	130 MPH	150 MPH	170 MPH
6	COLUMN FOOTING	W6x15 2.0'x6.5'	W8x21 2.0'x7.0'	W8x21 2.0'x8.0'	W10x22 2.0'x8.5'	6	COLUMN FOOTING	W8x24 2.0'x7.5'	W8x24 2.0'x8.0'	W8x28 2.0'x9.0'	W8x31 2.0'x9.5'
8	COLUMN FOOTING	W10x22 2.0'x7.5'	W8x24 2.0'x8.0'	W8x24 2.0'x9.0'	W8x28 2.0'x10.0'	8	COLUMN FOOTING	W8x31 2.0'x8.5'	W8x31 2.0'x9.0'	W10x33 3.0'x8.5'	W10x39 3.0'x9.5'
10	COLUMN FOOTING	W8x24 2.0'x8.5'	W8x28 2.0'x9.0'	W8x31 2.0'x10.0'	W10x33 3.0'x9.5'	10	COLUMN FOOTING	W8x24* 2.0'x9.0'	W12x26* 2.0'x10.0'	W10x33* 3.0'x9.5'	W16x36* 3.0'x11.0'
12	COLUMN FOOTING	W8x28 2.0'x9.0'	W8x31 2.0'x10.0'	W10x39 3.0'x9.5'	W12x40 3.0'x11.0'	12	COLUMN FOOTING	W12x26* 2.0'x10.0'	W14x30* 3.0'x9.5'	W16x36* 3.0'x11.0'	W14x43* 3.0'x12.0'
14	COLUMN FOOTING	W10x26* 2.0'x10.0'	W10x26* 3.0'x9.5'	W12x30* 3.0'x10.0'	W14x34* 3.0'x11.0'	14	COLUMN FOOTING	W10x30* 3.0'x9.5'	W10x49* 3.0'x10.0'	W16x40* 3.0'x11.0'	W14x48* 3.0'x13.0'

FOOTING DIMENSIONS = DIAMETER X DEPTH
* DENOTES BUCKLING BRACE REQUIRED

- NOTES:
1. FOOTING AND COLUMN SIZES ARE SUGGESTIONS ONLY, PROVIDED TO ASSIST WITH ESTIMATING INSTALLATION COSTS AND ARE NOT INTENDED FOR CONSTRUCTION PURPOSES. THE DESIGN MUST BE CERTIFIED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF THE INSTALLATION BEFORE THEY CAN BE USED FOR FABRICATION OR ERECTION.
 2. INTERNATIONAL BUILDING CODE 2012 USED IN DESIGN OF COLUMNS AND FOOTINGS WITH IMPORTANCE FACTOR=1, Kzt=1.0, Kd=0.85, G=0.85. SEISMIC DESIGN WAS NOT CONSIDERED.
 3. FOOTING DIMENSIONS ARE BASED ON ASSUMED SOIL CLASS 4 (ALLOWABLE LATERAL BEARING PRESSURE OF 150 psi).
 4. STRUCTURAL STEEL IS GRADE A992 (50 ksi) STEEL. CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2500 psi.
 5. THE AVERAGE DISPLAY WEIGHT FOR A LAYOUT CAN NOT EXCEED 8 PSF.
 6. DAKTRONICS INC. IS NOT RESPONSIBLE FOR STRUCTURES DESIGNED AND INSTALLED BY OTHERS.
 7. LOCAL BUILDING OFFICIALS SHOULD BE CONTACTED TO DETERMINE THE WIND SPEED AND EXPOSURE CATEGORY FOR THE PROPOSED SIGN LOCATION. THE EXPOSURE CATEGORY C IS DEFINED AS:
EXPOSURE B - URBAN AND SUBURBAN AREAS, OR OTHER TERRAIN WITH NUMEROUS SPACED OBSTRUCTIONS HAVING THE SIZE OF SINGLE-FAMILY DWELLINGS OR LARGER. THESE CONDITIONS MUST PREVAIL FOR A DISTANCE FROM THE SIGN OF AT LEAST 2,500 FT OR 20 TIMES THE SIGN HEIGHT, WHICHEVER IS GREATER.
EXPOSURE C - OPEN TERRAIN WITH SCATTERED OBSTRUCTIONS HAVING HEIGHTS GENERALLY LESS THAN 30 FT. THIS CATEGORY INCLUDES FLAT OPEN COUNTRY, GRASSLANDS, AND ALL WATER SURFACES IN HURRICANE PRONE REGIONS.
 8. FOR SPECIFIC PRODUCT DETAILS ON WEIGHT, MOUNTING, ETC. REFER TO THE INDIVIDUAL PRODUCT SPECIFICATION SHEETS.

EXPOSURE B					EXPOSURE C				
HEIGHT ABOVE GRADE = 10'					HEIGHT ABOVE GRADE = 15'				
DISPLAY HEIGHT (FT)	DESIGN WIND VELOCITY	115 MPH	140 MPH		DISPLAY HEIGHT (FT)	DESIGN WIND VELOCITY	115 MPH	140 MPH	
6	COLUMN	W8X21	W8X24		6	COLUMN	W12X26	W8X31	
	FOOTING	2.0'x7.5'	2.0'x8.5'			FOOTING	2.0'x8.5'	2.0'x9.5'	
8	COLUMN	W8X24	W8X28		8	COLUMN	W8X31	W10X39	
	FOOTING	2.0'x8.5'	2.0'x10.0'			FOOTING	2.0'x9.5'	3.0'x9.5'	
10	COLUMN	W8X31	W10X33		10	COLUMN	W12X30	W16X36	
	FOOTING	2.0'x9.5'	3.0'x9.5'			FOOTING	3.0'x9.0'	3.0'x11.0'	
12	COLUMN	W10X33	W12X40		12	COLUMN	W14X34	W16X40	
	FOOTING	3.0'x9.0'	3.0'x10.5'			FOOTING	3.0'x9.5'	3.0'x11.0'	
14	COLUMN	W12X26	W14X34		14	COLUMN	W14X38	W16X48	
	FOOTING	3.0'x9.5'	3.0'x11.0'			FOOTING	3.0'x11.0'	3.0'x13.0'	

FOOTING DIMENSIONS = DIAMETER X DEPTH
* DENOTES BUCKLING BRACE REQUIRED

NOTE:
REFER TO NOTE 7 FOR EXPOSURE CATEGORY DEFINITIONS.

SEE PKA VERIFICATION CALCULATIONS.



DAKTRONICS, INC.

BROOKINGS, SD 57006

DO NOT SCALE DRAWING

THE CONCEPTS EXPRESSED AND DETAILS SHOWN ON THIS DRAWING ARE CONFIDENTIAL AND PROPRIETARY. DO NOT REPRODUCE BY ANY MEANS WITHOUT THE EXPRESSED WRITTEN CONSENT OF DAKTRONICS, INC.

COPYRIGHT 2013 DAKTRONICS, INC.

PROJ: OUTDOOR SCOREBOARD INSTALLATION

TITLE: 14' WIDTH SCOREBOARD INSTALLATION SPECS

DESIGN: RSCHWAR

DRAWN: RSCHWAR

DATE: 27 NOV 13

SCALE: 1/16"=1'

SHEET

REV

JOB NO.

REV 02	DATE: 27 OCT 15	UPDATED WIDE FLANGE AND FOUNDATION VALUES	BY: AMP
REV 01	DATE: 23 JUL 14	UPDATED CLAMPS IN REAR AND SIDE VIEW AND ADDED 170 MPH WIND SPEC COLUMN	BY: TJT

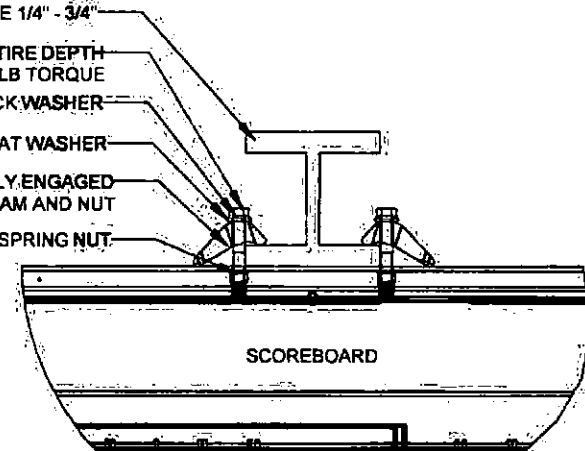
VERTICAL BEAM - FLANGE THICKNESS MUST BE 1/4" - 3/4"
 1/2-13 X 3,000 BOLT - BOLT THREAD MUST ENGAGE ENTIRE DEPTH
 OF SPRING NUT. BOLT MUST BE TIGHTENED TO 40FT-LB TORQUE

1/2" LOCK WASHER

1/2" FLAT WASHER

I-BEAM CLAMP - ASSURE CLAMP IS TIGHTLY ENGAGED
 TO I-BEAM AND NUT

SPRING NUT

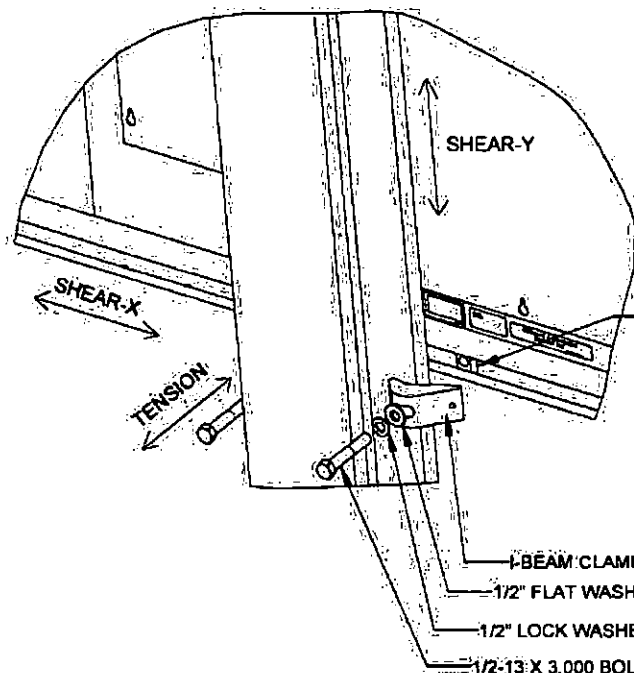


FRONT OF SCOREBOARD

TOP VIEW

CRITICAL

DO NOT USE ANY LUBRICANT
 ON ANY MOUNTING HARDWARE
 OR WARRANTY WILL BE VOIDED



SPRING NUT
 CRITICAL
 MAKE SURE SPRING NUT
 IS TURNED TO VERTICAL
 POSITION INSIDE
 SCOREBOARD CHANNEL

I-BEAM CLAMP
 1/2" FLAT WASHER
 1/2" LOCK WASHER
 1/2-13 X 3,000 BOLT

EXPLODED REAR ISOMETRIC VIEW

STANDARD MOUNTING METHOD

MOUNTING INSTRUCTIONS:

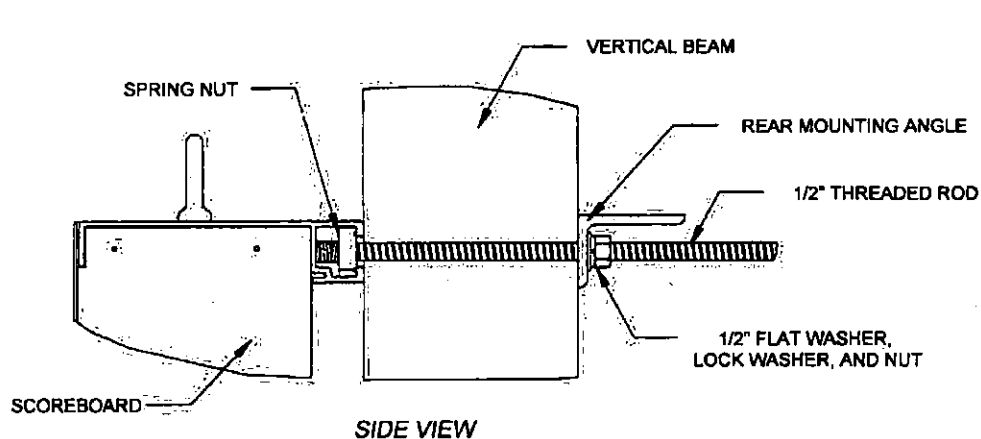
PLACE SPRING NUTS INTO SCOREBOARD
 CHANNEL IN APPROXIMATE LOCATION OF
 VERTICAL BEAMS
 LIFT SCOREBOARD INTO POSITION
 MAKE SURE THE 1/2-13 BOLTS ARE AS
 CLOSE TO THE I-BEAM FLANGES AS POSSIBLE
 WHEN SCOREBOARD IS ADJUSTED TO
 FINAL DESIRED POSITION, TIGHTEN
 BOLTS FIRMLY
 IF FLANGE THICKNESS IS MORE THAN 3/4"
 THICK LONGER BOLTS WILL BE REQUIRED
 AT THE CUSTOMER'S EXPENSE

STRUCTURAL NOTES

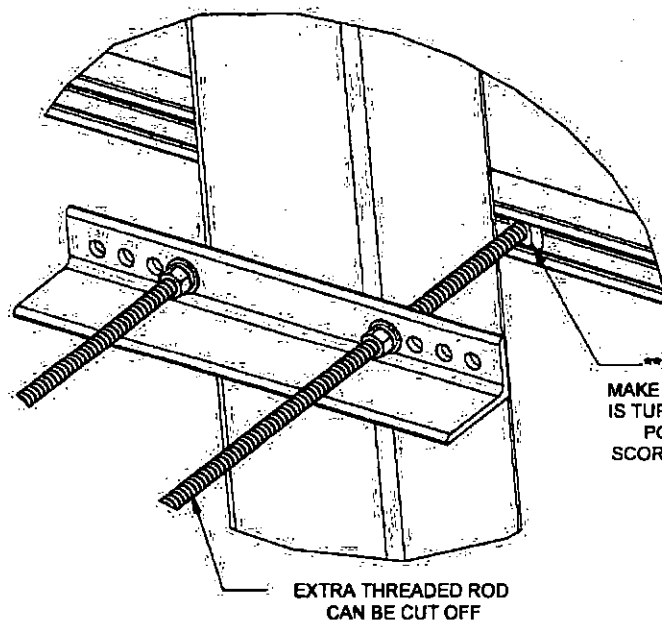
ALLOWABLE CAPACITY PER EACH CLAMP:
 SHEAR = 160 LBS
 TENSION = 2300 LBS

SHEAR AND TENSION LOAD
 DIRECTION ARE AS INDICATED ON
 REAR ISOMETRIC VIEW

05	22 DEC 15	ADDED LUBRICANT WARNING	PJS	
04	06 JAN 14	ADDED ALLOWABLE TENSION AND SHEAR CAPACITY DETAILS	JAVA	
03	23 OCT 13	PER EC-12382: CHANGED BOLT TORQUE FROM 30 FT-LB TO 40 FT-LB	NLM	
02	07 MAR 12	ADDED STANDARD MOUNTING METHOD NOTES	KDD	
01	21 FEB 12	CHANGED ROCKER TO I-BEAM	KDD	
REV	DATE:		BY:	
		THE CONCEPTS EXPRESSED AND DETAILS SHOWN ON THIS DRAWING ARE CONFIDENTIAL AND PROPRIETARY. DO NOT REPRODUCE BY ANY MEANS WITHOUT THE EXPRESS WRITTEN CONSENT OF DAKTRONICS, INC. OR ITS WHOLLY OWNED SUBSIDIARIES. COPYRIGHT 2016 DAKTRONICS, INC. (USA)		
		PROJECT: OUTDOOR SCOREBOARD TITLE: P1647 I-BEAM CLAMP MOUNTING		
DATE:	22-DEC-15	DIM UNITS:	INCHES [MILLIMETERS]	SHEET
SCALE:	1/8	DO NOT SCALE DRAWING		REV
DESIGN:	MCARSRU	JOB NO:	P1647	1 OF 1
DRAWN:	MCARSRU	FUNC - TYPE - SIZE	E-07-A	05
				1052565



SIDE VIEW

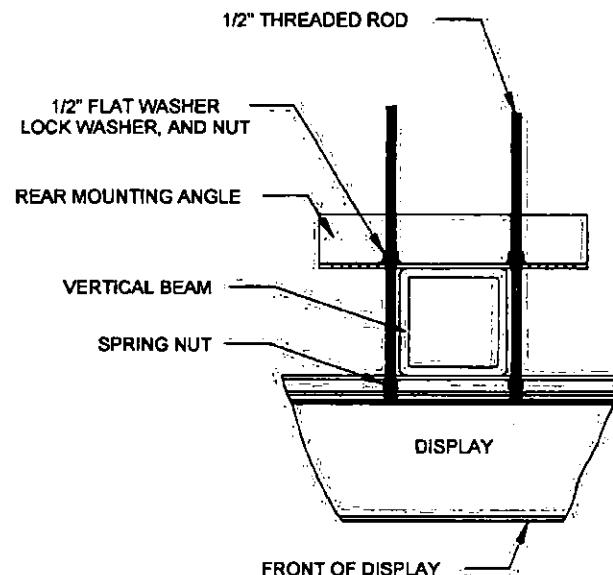


REAR ISOMETRIC VIEW

STRUCTURAL NOTES:
- BOLT TORQUE: 30 FT-LB

NOTES:



THREADED RODS RUN ALONG BOTH SIDES OF BEAM
RODS DO NOT PASS THROUGH THE FLANGES OF THE BEAM
NO DRILLING NECESSARY
MAKE SURE SPRING NUT IS PERPENDICULAR TO CHANNEL
OPENING ON SCOREBOARD



TOP VIEW
SCALE 1/10

*****CRITICAL*****
DO NOT USE ANY LUBRICANT
ON ANY MOUNTING HARDWARE
OR WARRANTY WILL BE VOIDED

04	22 DEC 15	ADDED LUBRICANT WARNING	PJS
03	03 JULY 13	ADDED STRUCTURAL NOTE	TTF
02	20 SEP 12	PER EC-7114; REMOVED CHAMFER FROM 0A-133258	LMG
01	08 OCT 11	REPLACED VERTICAL I-BEAM WITH 8" X 8" SQUARE TUBE	JAVA
REV	DATE:		BY:

 DAKTRONICS	<p>THE CONCEPTS EXPRESSED AND DETAILS SHOWN ON THIS DRAWING ARE CONFIDENTIAL AND PROPRIETARY. DO NOT REPRODUCE BY ANY MEANS WITHOUT THE EXPRESS WRITTEN CONSENT OF DAKTRONICS, INC. OR ITS WHOLLY OWNED SUBSIDIARIES. COPYRIGHT 2018 DAKTRONICS, INC. (USA)</p>	THIRD ANGLE PROJECTION 
PROJECT: OUTDOOR SCOREBOARDS:		
TITLE: P1647: POLE MOUNTING OPTIONS		
DATE: 22-DEC-15	DIM UNITS: INCHES (MILLIMETERS)	SHEET
SCALE: 1/5	DO NOT SCALE DRAWING	1 OF 1
DESIGN: DOPPELT	JOB NO. P1647	FUNC. TYPE - SIZE
DRAWN: DOPPELT		E - 10 - A
1048184		

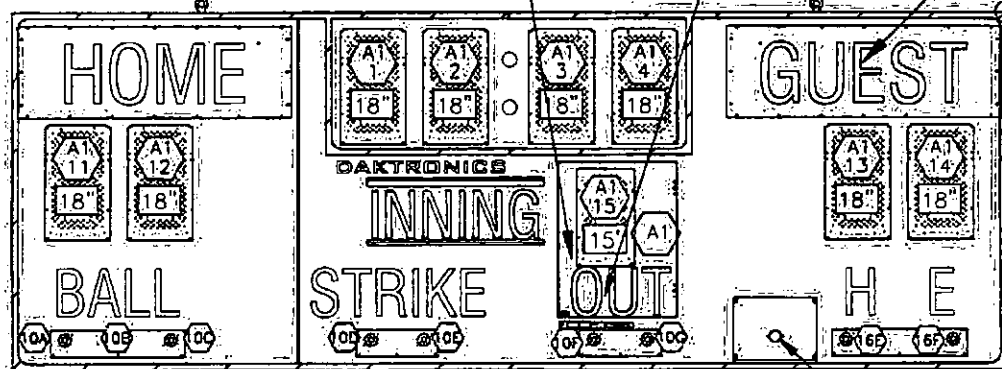
REV: 01 DATE: 27 FEB 15
 PER EC-17119 REMOVED DETAIL A
 ADDED SIGNAL OPTION NOTE
 CHANGED SLAVE AND MASTER DRIVER NAMES
 BY: KMA

MS-918-R/A/W

PRIMARY DRIVER (A1)
 KNOCKOUTS FOR 1/2" CONDUIT
 SIGNAL OPTION ON THIS DRIVER
 (WIRE, FIBER, OR RADIO)

OPTIONAL RADIO

OPTIONAL TNMCS
 8x32-34mm



FRONT VIEW

OPTIONAL HORN

NOTES:

- = LED DRIVER NUMBER & LED DRIVER CONNECTOR WIRED TO THAT DIGIT.
- = DIGIT SIZE
- = SEGMENT DESIGNATION
- = DRIVER NUMBER

DAKTRONICS, INC.
 BROOKINGS, SD 57006

DO NOT SCALE DRAWING

PROD OUTDOOR LED SCOREBOARDS

FILE COMPONENT LOCATION: MS-918-201X-R/A/W-FV-F

DESIGN: KDRAGT

DESIGN: ZRYKHUS

DATE: 7 NOV 11

SCALE: 1=30

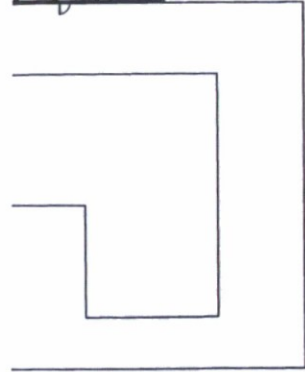
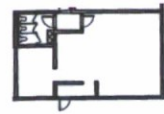
SHEET

REV

DOB INC.

19





APPROVED
CASE# 226-SA-2017
STIPULATION SET
RETAIN FOR RECORDS

08/28/17
DATE

Andrew Chi
APPROVED BY

STREET
CENTERLINE
EXISTING TREES AND
TALL OLEANDER
BUSHES

EXISTING
BASEBALL
FIELD

SCOREBOARD
SIDE
NOT VISIBLE
FROM RESIDENTS
BACK OF
SCOREBOARD
COLOR: GRAY
ALUMINUM

EXISTING 8'
WALL

STREET
CENTERLINE

GRASS
FIELD

210'-0"

EXISTING
TREES

130'-0"

87'-0"

N. Miller Rd.

EXISTING SINGLE FAMILY RESIDENTIAL LOTS
ZONING: R1-35

Sutton Dr

EXISTING SINGLE FAMILY RESIDENTIAL LOTS
ZONING: R1-35

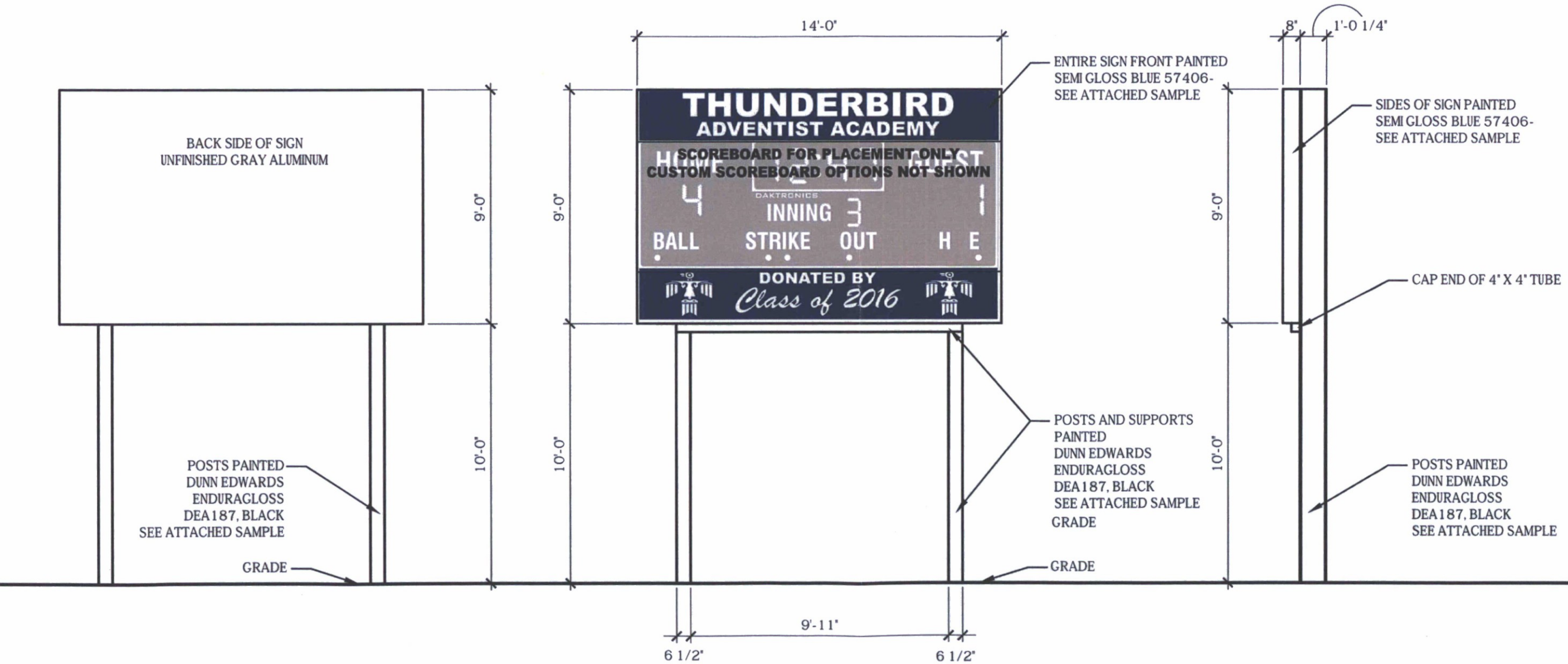
(01) SCOREBOARD SITE LOCATION

<div>LS EN T N E R S</div> <div>TS & PLANNERS</div>	Nelsen Partners, Inc. Austin Scottsdale 15210 N. Scottsdale Rd. #300 Scottsdale, Arizona 85254 t 480.949.6800 nelsonpartners.com		Thunderbird Academy Scoreboard		ASK001
			7410 E. SUTTON DR. Scottsdale, Arizona 85260		SCORE BOARD DRAWING NO./SHEET NO.
	DATE: 07/20/2017		PROJECT NO.: 314035		

APPROVED
CASE# 226-SA-2017
STIPULATION SET
RETAIN FOR RECORDS

08/28/17
DATE

Andrew Chi
APPROVED BY



Thunderbird Academy Scoreboard

LSEN
T N E R S
CONSULTANTS & PLANNERS

Nelsen Partners, Inc.
Austin | Scottsdale
15210 N. Scottsdale Rd, #300
Scottsdale, Arizona 85254
t 480.949.6800
nelsonpartners.com

Sutton Drive
Scottsdale, AZ 85260

PROJECT NO: 314035

DATE: 8/4/2017

Elevations
DRAWING NO./SHEET NO.

GENERAL STRUCTURAL NOTES

APPLY UNLESS NOTED OTHERWISE

BUILDING CODE:

2015 EDITION OF THE INTERNATIONAL BUILDING CODE, WITH CITY OF SCOTTSDALE AMENDMENTS.

WIND:

BASIC WIND SPEED 115 MPH.
WIND IMPORTANCE FACTOR: $I = 1.00$.
BUILDING CATEGORY: II
WIND EXPOSURE: C.

FOUNDATIONS:

FOOTINGS SHALL BEAR ON ASSUMED UNDISTURBED CLAY, SANDY CLAY, SILTY CLAY, CLAYEY SILT AND/OR SANDY SILT. DESIGN SOIL BEARING VALUE = 1,500 PSF.

CONCRETE:

MINIMUM 28 DAY STRENGTH 2,500 PSI EXCEPT AS FOLLOWS: (TYPE II, U.M.O.)

CONCRETE BASES: 2,500 PSI

MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED. MAXIMUM SLUMP $4\frac{1}{2}"$ FOR CONCRETE WITHOUT PLASTICIZER. IF PLASTICIZER IS USED, A HIGHER FINAL SLUMP MAY BE ALLOWED UPON STRUCTURAL ENGINEER'S APPROVAL.

REMBRATE TOPS OF CONCRETE PIER 15 MINUTES AFTER PLACING CONCRETE.

REINFORCING:

ASTM A615 ($F_y = 60$ KSI) DEFORMED BARS FOR ALL BARS. ALL GRADE 60 REINFORCING TO BE WELDED SHALL BE ASTM A706, WELDED WIRE FABRIC PER ASTM A185, WIRE PER ASTM A92. NO TACK WELDING OF REINFORCING BARS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE WITH THE STRUCTURAL ENGINEER. LATEST ACI CODE AND DETAILING MANUAL APPLY. CLEAR CONCRETE COVERAGES AS FOLLOWS:

CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
EXPOSED TO EARTH OR WEATHER: 2"
#6 OR LARGER: 2"
#6 AND SMALLER: 1 1/2"
ALL OTHER PER LATEST EDITION OF ACI 318.

LAP SPLICES IN CONCRETE:

LAP SPLICES, UNLESS NOTED OTHERWISE, SHALL BE CLASS "B" TENSION LAP SPLICES PER LATEST EDITION OF ACI 318. STAGGER SPLICES A MINIMUM OF ONE LAP LENGTH. ALL SPLICE LOCATIONS SUBJECT TO APPROVAL BY THE STRUCTURAL ENGINEER. REINFORCING BAR SPACING GIVEN ARE MAXIMUM ON CENTERS. ALL BARS PER CSI SPECIFICATIONS AND HANDBOOK. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE.

STRUCTURAL STEEL:

ALL STRUCTURAL STEEL SHALL BE ASTM A992 ($F_y = 50$ KSI). ALL CHANNELS, ANGLES, AND PLATES SHALL BE ASTM A36 ($F_y = 36$ KSI). ALL TUBE STEEL SHALL BE ASTM A500 ($F_y = 46$ KSI). ALL BOLTS SHALL BE ASTM A307, UNLESS NOTED OTHERWISE. ALL CONSTRUCTION PER LATEST AISC HANDBOOK. ALL REFERENCE TO HEADED STUDS SHALL BE HIGH STRENGTH HEADED STUDS. ATTACHMENT OF HEADED STUDS SHALL CONFORM TO ALL REQUIREMENTS OF THE LATEST EDITION OF THE "RECOMMENDED PRACTICES FOR STUD WELDING" AND THE "STRUCTURAL WELDING CODE" PUBLISHED BY AWS. ALL BOLTS, ANCHOR BOLTS, ETC. SHALL BE INSTALLED WITH STEEL WASHERS AT SLOTTED HOLES IN STEEL SECTIONS. ALL WELDING SHALL BE PERFORMED BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. CERTIFICATES SHALL BE THOSE ISSUED BY AN ACCEPTED TESTING AGENCY. ALL WELDING PER LATEST AMERICAN WELDING SOCIETY STANDARDS. ALL WELDING DONE BY E70 LOW HYDROGEN RODS UNLESS NOTED OTHERWISE. THESE DRAWINGS DO NOT DISTINGUISH BETWEEN SHOP AND FIELD WELDS; THE CONTRACTOR MAY SHOP WELD OR FIELD WELD AT HIS DISCRETION. SHOP WELDS AND FIELD WELDS SHALL BE SHOWN ON THE SHOP DRAWINGS SUBMITTED FOR REVIEW. ALL FULL (COMPLETE) PENETRATION WELDS SHALL BE TESTED AND CERTIFIED BY AN INDEPENDENT TESTING LABORATORY. WHEN STRUCTURAL STEEL IS FURNISHED TO A SPECIFIED MINIMUM YIELD POINT GREATER THAN 36 KSI, THE ASTM OR OTHER SPECIFICATION DESIGNATION SHALL BE INCLUDED NEAR THE ERECTION MARK ON EACH SHIPPING ASSEMBLY OR IMPORTANT CONSTRUCTION COMPONENT, OVER ANY SHOP COAT OF PAINT, PRIOR TO SHIPMENT FROM THE FABRICATOR'S PLANT.

SHOP DRAWINGS:

SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS IN ADDITION TO ITEMS REQUIRED BY ARCHITECTURAL SPECIFICATIONS.

THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMITTAL. ITEMS NOT IN ACCORDANCE WITH CONTRACT DOCUMENTS SHALL BE FLAGGED UPON HIS REVIEW.

VERIFY ALL DIMENSIONS WITH ARCHITECT AND ALL FINISHED GRADE WITH CIVIL DRAWINGS.

ANY CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM CONTRACT DOCUMENTS SHALL BE CLOUDED BY MANUFACTURER OR FABRICATOR. ANY OF THE AFOREMENTIONED WHICH ARE NOT CLOUDED OR FLAGGED BY SUBMITTING PARTIES, SHALL NOT BE CONSIDERED APPROVED AFTER ENGINEER'S REVIEW, UNLESS NOTED ACCORDINGLY.

THE ENGINEER HAS THE RIGHT TO APPROVE OR DISAPPROVE ANY CHANGES TO CONTRACT DOCUMENTS AT ANYTIME BEFORE OR AFTER SHOP DRAWING REVIEW.

THE SHOP DRAWINGS DO NOT REPLACE THE CONTRACT DOCUMENTS. ITEMS OMITTED OR SHOWN INCORRECTLY AND ARE NOT FLAGGED BY THE STRUCTURAL ENGINEER OR ARCHITECT ARE NOT TO BE CONSIDERED CHANGES TO CONTRACT DOCUMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO MAKE SURE ITEMS ARE CONSTRUCTED TO CONTRACT DOCUMENTS.

THE ADEQUACY OF ENGINEERING DESIGNS AND LAYOUT PERFORMED BY OTHERS RESTS WITH THE DESIGNING OR SUBMITTING AUTHORITY.

REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS. RESPONSIBILITY FOR CORRECTNESS SHALL REST WITH THE CONTRACTOR.

DEFERRED SUBMITTALS: (PER 2015 IBC 107.3.4.1)

FOR THE PURPOSES OF THIS SECTION, DEFERRED SUBMITTALS ARE DEFINED AS THOSE PORTIONS OF THE DESIGN WHICH ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION AND THAT ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED PERIOD.

DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE PROFESSIONAL OR RESPONSIBLE CHARGE FOR REVIEW. THE CONTRACTOR SHALL FORWARD THE REVIEWED DOCUMENTS TO BUILDING OFFICIAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

DEFERRED SUBMITTAL ITEMS:

SCORE BOARD

GENERAL:

ENTIRE CONTRACT DOCUMENTS SHALL BE USED TO BUILD BUILDING. SOME CRITICAL ITEMS REQUIRED BY OTHER DISCIPLINES MAY NOT BE SHOWN ON STRUCTURAL DRAWING.

ITEMS SHOWN BY OTHER DISCIPLINES WITH REFERENCE TO STRUCTURAL DRAWING BUT NOT SHOWN ON THESE STRUCTURAL DOCUMENT SHALL BE CONSIDERED DESIGN BUILD ITEMS. CONTRACTOR SHALL SUBMIT DESIGN BY OTHERS FOR REVIEW.

THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERE TO (NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS).

WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDA.

NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT.

ALL DIMENSIONS SHOWN (INCLUDING ELEVATIONS) ON STRUCTURAL DRAWINGS ARE TO ASSIST CONTRACTOR IN VERIFICATION. SCALING DIMENSIONS FROM DRAWINGS IS NOT PERMITTED. LOCATION OF ALL ITEMS SHALL BE DETERMINED BY DIMENSIONS OR NOTES ONLY; DO NOT USE GRAPHIC APPEARANCE TO ASSUME SPECIFIC LOCATIONS.

CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL AND FINISHED GRADE WITH CIVIL DRAWINGS PRIOR TO START OF CONSTRUCTION. RESOLVE ANY DISCREPANCY WITH THE ARCHITECT.

TYPICAL DETAILS MAY NOT NECESSARILY BE CUT ON PLANS, BUT APPLY UNLESS NOTED OTHERWISE.

WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.

ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF JURISDICTION.

SUPPLIER OF ENGINEERED STRUCTURAL COMPONENTS (L&SCORE BOARD) SHALL BE RESPONSIBLE FOR COMPLETE DESIGN AND SHALL USE ENTIRE CONTRACT DOCUMENTS TO INCLUDE ALL LOADS AND DETAIL REQUIREMENTS FROM ALL DISCIPLINES. SUPPLIER SHALL PROVIDE ADDITIONAL MATERIAL REQUIRED TO MEET ALL THEIR REQUIREMENTS FOR INSTALLATION (L&).

CONC. PSI	CLASS B TENSION SPLICE LENGTHS						COMP. BARS	
	$f'_c = 2,500$ PSI/ $f'_c = 3,000$ PSI		$f'_c = 4,000$ PSI		$f'_c = 5,000$ PSI AND HIGHER		$f'_c = \text{ALL}$	
BAR LOCATION	REGULAR CLASS	TOP CLASS	REGULAR CLASS	TOP CLASS	REGULAR CLASS	TOP CLASS	STD LAP	ENCLOSED W/ SPIRAL TIES
SIZE								
#3	24"	31"	19"	24"	17"	22"	12"	12"
#4	32"	41"	25"	33"	23"	29"	15"	12"
#5	39"	51"	31"	41"	28"	38"	18"	14"
#6	47"	61"	37"	49"	34"	43"	23"	17"
#7	69"	89"	54"	71"	49"	63"	28"	20"
#8	78"	102"	62"	81"	58"	72"	30"	23"
#9	88"	115"	70"	91"	63"	81"	34"	25"
#10	100"	129"	79"	102"	70"	92"	38"	29"
#11	110"	143"	87"	113"	78"	102"	42"	32"

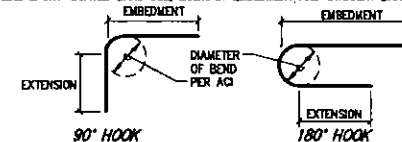
- NOTES:
- TOP BARS ARE ANY HORIZONTAL BARS PLACED SO THAT MORE THAN 12" OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE REINFORCEMENT.
 - UNLESS NOTED OTHERWISE, LAP SPLICES IN CONCRETE BEAMS, SLABS AND WALLS SHALL BE CLASS "B" TENSION SPLICES. CONCRETE COLUMNS SHALL USE COMPRESSION LAP SPLICES.
 - CONTACT STRUCTURAL ENGINEER IF CENTER-TO-CENTER SPACING OF REINFORCING IS LESS THAN OR EQUAL TO 3 BAR DIAMETERS $<3d$ OR $2d$ CLEAR SPACING BETWEEN BARS.
 - WHERE CLEAR COVER $<4d$, MULTIPLY TENSION LAP SPLICE BY 1.5.
 - ALL SPLICES MUST BE FULL CONTACT.
 - WHERE EPOXY COATED BARS USE, MULTIPLY LAP SPLICE BY 1.5.

LAP SCHEDULE FOR REINFORCING STEEL

NO SCALE

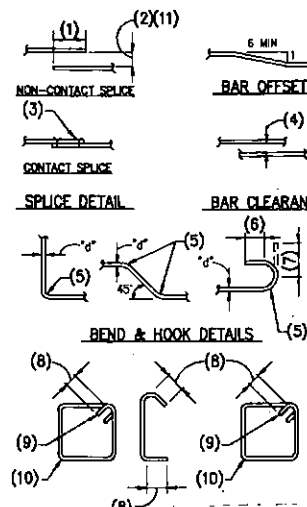
BAR SIZE	HOOKED EMBEDMENT			EXTENSION		STRAIGHT BAR EMBEDMENT		
	3000 PSI CONCRETE	4000 PSI CONCRETE	5000 PSI CONCRETE	90° HOOK	180° HOOK	5000 PSI	4000 PSI	3000 PSI
#3	6	6	6	4.5	2.5	13	14	18
#4	8	7	6	6.0	2.5	17	19	22
#5	10	8	7	7.5	2.5	21	24	27
#6	12	10	9	9.0	3.0	26	28	33
#7	13	12	10	10.5	3.5	37	42	48
#8	15	13	12	12.0	4.0	43	47	55
#9	17	15	13	13.5	4.5	48	54	62
#10	19	17	15	15.2	5.1	54	60	70
#11	22	19	17	16.9	5.6	60	67	77

- NOTES:
- EMBEDMENT LENGTH IS BASED ON 2 1/2" MINIMUM SIDE COVER AND 2" MINIMUM END COVER.
 - CONTACT STRUCTURAL ENGINEER IF CENTER-TO-CENTER SPACING OF REINFORCING IS LESS THAN OR EQUAL TO 3 BAR DIAMETERS $<3d$ OR $2d$ CLEAR SPACING BETWEEN BARS.
 - WHERE CLEAR COVER $<4d$, MULTIPLY EMBEDMENT AND STRAIGHT BAR DEVELOPMENT BY 1.5.
 - WHERE EPOXY-COATED BARS USE, MULTIPLY EMBEDMENT AND STRAIGHT BAR DEVELOPMENT BY 1.5.



DEVELOPMENT LENGTH IN TENSION (INCHES)

NO SCALE



NOTES:

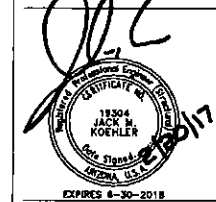
- LAP - SEE G.S.N.
- MAXIMUM 1/5 LAP LENGTH BUT NOT MORE THAN 6".
- WIRE TIES.
- 1d (1" MINIMUM).
- RADIUS=3d FOR BARS NOT OVER #8; 4d FOR #9, #10 AND #11 BARS; 5d FOR #14 AND #18 BARS.
- 4d ((2 1/2" MINIMUM).
- 12d (90 DEGREE HOOK).
- 6d (5" MINIMUM).
- 135 DEGREE BEND.
- BEND AROUND 1 1/2" ϕ PIN FOR #3 BARS. BEND AROUND 2" PIN FOR #4 BARS. BEND AROUND 2 1/2" PIN FOR #5 BARS.
- PRIOR APPROVAL MUST BE GIVEN BY OUR OFFICE TO ALLOW NON-CONTACT SPLICES.
- LAP THE MIN. 6".
- LONGITUDINAL REINFORCEMENT.
- LONGITUDINAL BAR AS OCCURS.
- PROVIDE 135° HOOK AT LONGITUDINAL REINFORCEMENT.
- ROTATE AND ALTERNATE TIE LAP AT DIFFERENT VERTICAL REBAR LOCATION AT EACH TIE.



NELSEN PARTNERS ARCHITECTS & PLANNERS

Nelson Partners, Inc.
Austin | Scottsdale

15210 North Scottsdale Road
Suite #300
Scottsdale, Arizona 85254
t: 480.949.6800
nelsonpartners.com



SDA Academy Scoreboard

7410 E. Suttin Dr.
Scottsdale, Arizona 85260

Date
February 25, 2017

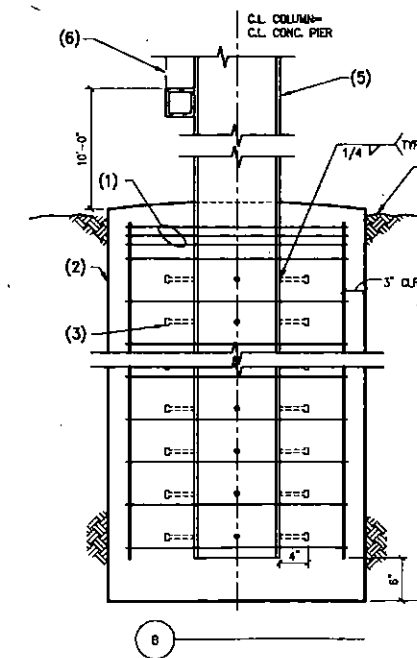
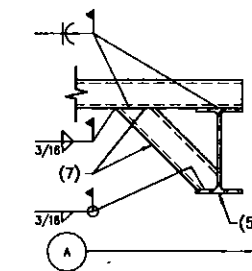
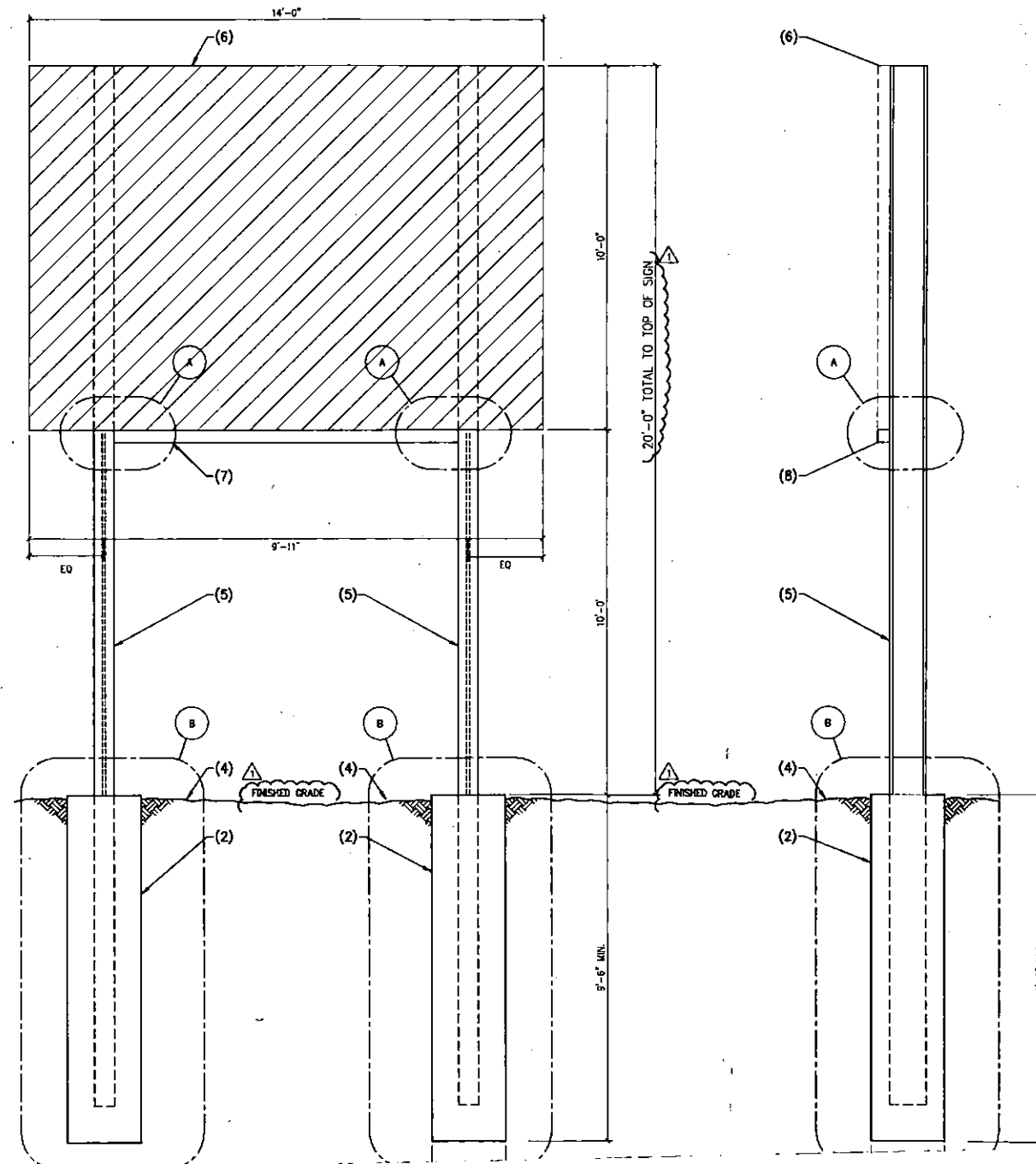
Drawings and written material appearing hereon remain the property of Nelson Partners, Inc. and are not to be reproduced, used, or disclosed without written consent of the architect.

© 2015 NELSEN PARTNERS, INC.

Project No.



- NOTES:**
- 3 TIES AT TOP 8".
 - CONCRETE DRILLED PIER FOUNDATION W/ 12-#5 VERT. AND #3 TIES AT 8" O.C. 1/2" H.S. #3 LONG AT 18" O.C. ON ALL SIDES OF COLUMN - TYP.
 - FINISHED GRADE.
 - W12x26 STEEL COLUMN.
 - SCORE BOARD PANELS AND ATTACHMENT BY OTHERS.
 - HSS4"x4"x3/16" BELOW SCOREBOARD.
 - FINISHED GRADE.



Date
February 25, 2017

City Comments 08/23/17

Drawings and written material prepared herein constitute original and unpublished work of the architect and may not be duplicated, used, or disclosed without written consent of the architect.

© 2015 NELSEN PARTNERS, INC.

Printed On

SDA Academy Scoreboard

7410 E. Sutton Dr.
Scottsdale, Arizona 85260