

PRELIMINARY DRAINAGE REPORT

STORYROCK PHASE 1C



SEPTEMBER 2017

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INTRODUCTION

PROJECT DESCRIPTION

This Preliminary Drainage report has been prepared for the proposed Storyrock Phase 1C residential development. StoryRock Phase 1C (Phase 1C) is part of the StoryRock Master Planned Community (formerly named Cavalliere Ranch), a development consisting of 462-acres of single family residential construction. A Conceptual Master Drainage report was approved October 2014 with the project Zoning Case (13-ZN-2014).

StoryRock Phase 1C is a proposed 120-acre single family residential subdivision consisting of 96 single family residential units. Phase 1C is zoned for R1-18, R1-35, and R1-43 development. All R1-18 areas are proposed to be mass-graded and R1-35, R1-43, and R1-70 areas will require separate single lot grading plans for each lot developed. The proposed site is located within the City of Scottsdale and falls under the City's Environmentally Sensitive Lands Ordinance (ESLO).

PROJECT LOCATION AND DESCRIPTION

StoryRock is located within Section 12 of Township 4 North, Range 5 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. The site is bound to the north by the Happy Valley Road Alignment and to the west by 128th Street. The Pinnacle Peak Road Alignment borders the site to the south. The McDowell Sonoran Preserve borders the site to the east and portions of the site to the north and south. Phase 1C is located in the eastern portion of the site, spanning Alameda Road. (See **Figure 1: Location Map**).

The development is located within one flood zone as shown on Flood Insurance Rate Map (FIRM) panel number 04013C1335L, dated October 16, 2013 (see **Appendix A** for FIRM). The flood zones that pertain to the site are as follows:

"Other Areas" Zone D – "Areas in which flood hazards are undetermined, but possible"

The property is undeveloped natural desert, characterized by braided washes and rock features of varying sizes. Undeveloped desert is also characterized by native desert grasses and brush.

SCOPE OF DRAINAGE REPORT

The approved master drainage report and associated zoning material established the general drainage parameter and criteria for site planning. This report for Phase 1C further establishes drainage parameters and criteria for preliminary design. This report establishes a hydrologic plan for the development of the site as well as preliminary hydraulic analysis for the washes crossing the site.

All drainage criteria presented in this report will conform to the City of Scottsdale Design Standards & Polices Manual (DS&PM).

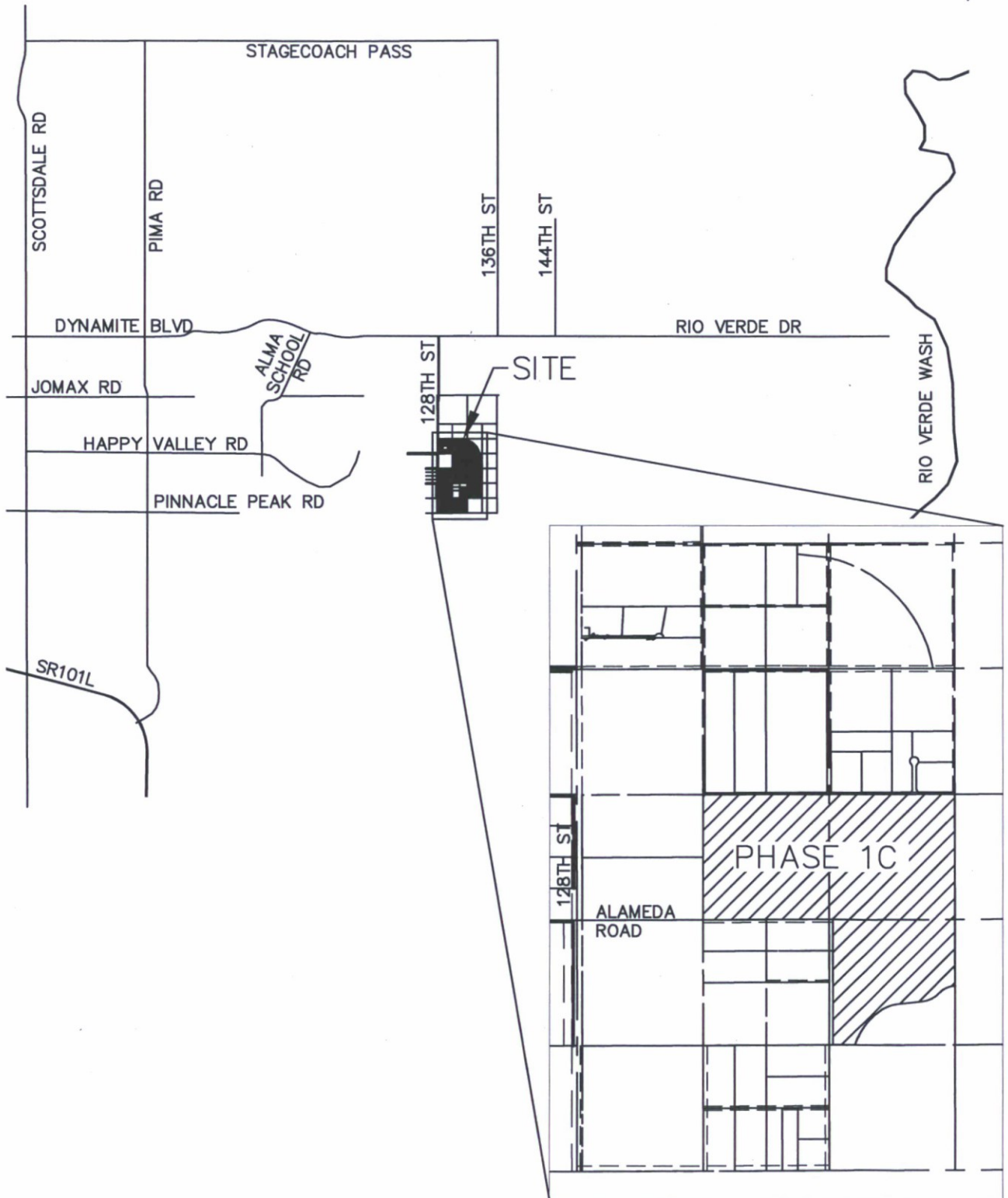


FIGURE 1
VICINITY MAP
STORYROCK



DESCRIPTION OF EXISTING DRAINAGE CONDITIONS AND CHARACTERISTICS

EXISTING SITE CONDITIONS

The site is characterized by many washes and rock features of varying sizes. The on-site washes vary in size and depth, but generally flow from the southwest to northeast or east through the site. Phase 1C is located in the eastern portion of the Storyrock development. Phase 1C is bound by other Storyrock phases to the north, west and southwest, an undeveloped residential parcel to the southwest and the McDowell Sonoran Preserve to the east. Storm water runoff impacts Phase 1C from the west and south, and is conveyed through the site in existing washes. Runoff is discharged into Storyrock Phase 1B to the north, and into the Preserve to the east. Multiple ridgelines run through the site, in the general direction of southwest to northeast.

The site falls within the City of Scottsdale Environmentally Sensitive Lands (ESL) and is subject to the design criteria of the Environmentally Sensitive Lands Ordinance (ELSO). Specifically, the site is categorized as Upper Desert Landform of ESL. Per the DS&PM "The ordinance requires that a percentage of each property be permanently preserved as natural area open space (NAOS) and the specific environmental features, including vegetation, washes, mountain ridges and peaks be protected for inappropriate development".

EXISTING OFF-SITE DRAINAGE CONDITIONS

Off-site flows impact the site from the south, and west. Off-site flows originate from large undeveloped sub-basins. The areas to the west of 128th Street are mostly undeveloped residential properties. The sub-basin to the south and west of Phase 1C are part of other Storyrock Phases. Refer to **Figure 2: Offsite Drainage Map**.

Off-site flows vary from local low flows up to large wash flows over 250 cfs. Most off-site flows will be conveyed through the site by the existing washes. 128th Street from Ranch Gate Road south to the Tom's Thumb trailhead consists of a two-lane paved road with several at-grade drainage crossings. A few locations provide low flow pipe culvert crossings that are undersized to accommodate larger storm events from overtopping the roadway section. No curb exists along the roadway and runoff generated along 128th Street sheet flows in the eastwardly direction through the StoryRock development.

An existing conditions hydrologic model was completed to develop peak discharges for the offsite runoff contributing to the Site. Offsite sub-basins were delineated based on the City of Scottsdale Quarter Section Topography. Significant washes are defined as having a 100-year flow of 50 cfs or more. There are no washes with 100-year peak flows of 750 cfs or greater, which indicates that no Vista Corridors exist within the project area. Significant washes been identified on **Figure 3: Existing Drainage Condition**. Hydrologic results can be found in **Appendix B**.

EXISTING ON-SITE DRAINAGE CONDITIONS

Three significant offsite washes cross Phase 1C and have been identified **Figure 3: Existing Drainage Condition**.

An existing conditions hydrologic model was completed to determine the peak stormwater discharges leaving the site. The existing condition discharges will be compared to the proposed condition discharges in a "pre-vs-post" analysis. The proposed condition discharges must be equal to or below the existing condition.

An existing conditions hydraulic model was completed for the three significant washes, and one minor wash crossing the site. The existing conditions hydraulic analysis was used as the basis for a post-conditions analysis. Hydraulic analysis has been performed on the significant washes to determine the 100-year Base Flood Elevations (BFEs) at specific cross sections. These elevations are used to map the existing 100-year limits of inundation.

Hydrology and hydraulic results can be found in **Appendix B** and **Appendix C** respectively.

PROPOSED PRELIMINARY DRAINAGE PLAN

PROPOSED ON-SITE DRAINAGE PLAN

The proposed Phase 1C development consists of 96 single family residential units. Lots located along the washes will have finished floor elevations a minimum of one foot above the 100-year base flood elevation (BFE). In general, lots will drain to the street system and runoff will be conveyed in the streets and/or in on-site swales and storm drain systems to detention basins or wash crossing locations located throughout the project. Due to the steep and undulating terrain, some lots will require rear or side yard drainage into adjacent washes or drainage swales within the development in order to minimize impacts to environmental features, existing natural area open space, and meet design criteria as required with the Environmentally Sensitive Lands Ordinance for the project. Specific lots that drain via rear or side yard locations within the project have been identified on **Figure 4** and on the preliminary grading and drainage plan in **Appendix E**. Detention basins will detain runoff before discharging into the existing washes and will be sized to meet first flush criteria. Specific areas that discharge into existing washes and are not routed through drainage basins will provide for alternative methods to meet first flush criteria. A further discussion regarding alternative methods to first flush is provided in the "Stormwater Storage Method" section below. The post development flows exiting the site will be attenuated through detention basins to a level equal to or less than pre-development flows. See **Figure 4: Proposed Conditions Drainage Map**.

PROPOSED OFF-SITE DRAINAGE PLAN

Offsite flows impact the site from the south, and west. Flows will be conveyed through the site and will discharge at their historic locations on the east and north side of the Site. In most cases, off-site flows are conveyed within the existing washes.

Associated with the development of Storyrock, 128th Street and Alameda Road roadway infrastructure will be constructed. These roadway improvements will be completed under a separate plan from the on-site improvements, and will include final drainage reports, however, the run-off from the off-site roadways is included within the on-site drainage plans.

128th Street consists of a median divided roadway with curb, and will include multiple culvert crossings to convey flow under the proposed roadway. Alameda Road and a crowned two lane road with curb with portions of median.

The proposed 128th Street Road improvements includes multiple culvert crossings to convey off-site flow under the proposed roadway. In the existing condition, 128th Street roadway flows sheet flow to the east. The proposed 128th Street roadway design consolidates the roadway discharges locations to major wash crossings. Alameda Roadway discharges roadway runoff into the adjacent wash at proposed locations.

Any increased run-off created by the 128th Street roadway improvements. from either the increased impervious area or changes to the drainage patterns are accounted for with the Phase 1A and Phase 2 drainage plans. Runoff from the western portion of Alameda Road, is accounted for in the Phase 2 drainage plan. Runoff from the eastern portion of Alameda, which impacts Phase 1C, is accounted for in the Phase 1C drainage Plan. The pre-vs post analysis for Phase 1C includes the Alameda proposed sub basins and land uses.

All of the significant washes are maintained within their existing wash corridors. A small portion of Wash160 will be modified to avoid the proposed development. The wash modification will provide a continuous natural wash corridor. Of the approximate 5,500 feet of significant washes within Phase 2, it is proposed to modify approximately 100 feet. For extents of the wash modifications see **Figure 4: Proposed Conditions Drainage Map**.

PROPOSED ON-SITE HYDROLOGY

On-site runoff from the proposed development maintains post-development flows at or below pre-development conditions at each of the Phase 1C exit points, for the three design storms (2-year, 10-year, and 100-year). Except for two locations, CP7 & CP15, where the post development flows exceed the existing condition flow by approximately 1-2 cfs and two additional locations, CP8 & CP14, where only the 2-year post development flows exceed the existing condition flow by approximately 1-2 cfs. This is within the level of accuracy of the analysis, and should be considered incidental and in conformance with the design. Furthermore, the downstream condition is undeveloped McDowell Sonoran Preserve and the minor increase does not negatively impact any downstream properties. A summary of pre- and post-development peak discharges is provided in **Table 1**. Multiple detention basins are used to attenuate peak discharge from on-site runoff. A basin summary table has been provided in **Appendix B** indicating basin volumes, maximum depths, orifice sizes, side slopes, peak inflow and outflow rates, drain times, and storage volumes provided for the 2, 10, and 100 year events. Each basin utilizes a bleed-off pipe with orifice plate with the intent to control post-development runoff exiting the development, with a spillway for larger storm events. The total drain time for all basins is less than 36 hours. Detention Basin 160 (DB160) and DB182 are in-line basins and takes advantage of the natural detention and attenuation created by a roadway culvert crossing which allows for minimal disturbance to NAOS in the area. These specific in-line basins experiences depths greater than 3 feet for a very short period during the peak of larger storm events. DB160 and DB182 provides a drain time of 14 and 20 minutes respectively, which helps minimize safety concerns in this location. Furthermore, the basins are located within the private community, setback from pedestrian walkways and a safety rail will be provided at the inlet headwall of the culverts. For in-line basins, the potential for culvert sedimentation build-up is increased. Sedimentation deposit within the culvert should be minimized, however, due to the high flow velocities within the culvert. Additionally, a culvert maintenance program is proposed with the development (see additional information in the "Culvert Sedimentation" section below). Refer to **Appendix B** for the detailed hydrologic model results.

Table 1: Peak Discharge Summary

Concentration Point	Prop. Cond. 2-Year (cfs)	Ex. Cond. 2-Year (cfs)	Prop. Cond. 10-Year (cfs)	Ex. Cond. 10-Year (cfs)	Prop. Cond. 100-Year (cfs)	Ex. Cond. 100-Year (cfs)
CP1	2	2	7	7	16	16
CP2	17	17	52	52	130	130
CP3	3	3	9	9	23	23
CP4	2	2	7	7	16	16
CP5	2	2	7	7	16	16
CP6	35	35	104	104	265	265
CP7	21	20	61	61	154	153
CP8	3	2	6	6	13	15
CP9	10	10	22	32	70	81
CP10	11	11	24	34	83	89
CP11	2	2	4	5	7	11
CP12	5	6	11	18	24	45
CP13	1	1	2	2	5	5

Concentration Point	Prop. Cond. 2-Year (cfs)	Ex. Cond. 2-Year (cfs)	Prop. Cond. 10-Year (cfs)	Ex. Cond. 10-Year (cfs)	Prop. Cond. 100-Year (cfs)	Ex. Cond. 100-Year (cfs)
CP14	5	3	9	9	17	21
CP15	40	39	120	120	308	307

PROPOSED ON-SITE HYDRAULICS

On-site runoff will be conveyed in the local streets, swales, storm drains, and culverts to the detention basins or wash discharge locations. Per the DS&PM all interior streets will be designed to convey the peak discharge from the 10-year storm event at or below the top of curb elevation. Additionally, the streets will convey the 100-year runoff within the proposed tracts and maintain a maximum flow depth of eight inches above the gutter flow line. Catch basins with storm drains or scuppers will capture pavement runoff and outfall to the proposed detention basins. In specific roadway discharge locations areas where detention basins are not provided, alternative methods such as stormceptor structures will be provided to meet first flush criteria (see additional information in the "Stormwater Storage Method" section below). The scupper, catch basins and storm drains will be designed per the DS&PM and FCDMC's Drainage Policies and Standards. Detailed catch basin and street capacity analysis will be completed as part of the final design.

The existing hydraulic model was revised for a proposed hydraulic model to determine the proposed condition 100-year BFE and limits of inundation. Development of the site, including roadway, culverts and lots encroach into the existing BFE. The proposed hydraulic model includes these encroachments and modifications to calculate the proposed BFEs and proposed 100-year limits of inundations. The proposed BFEs at the boundary of the site, both upstream and downstream cannot be higher than the existing condition. **Table 2** provides a summary of the existing and proposed 100-year BFE at the boundary conditions of the site. Refer to **Appendix C** for hydraulic results.

Table 2: Boundary Base Flood Elevation Summary

Wash	HEC-RAS Cross Sections	Ex. Cond BFE	Prop. Cond BFE	Note:
Wash 85	2114	2641.84	2641.77	Site Entrance
Wash 85	933	2600.95	2600.89	Site Exit
Wash 160	241	2545.51	2545.60	Site Exit
Wash 180	139	2549.43	2549.29	Site Exit
Wash 215	1770	2610.51	2610.54	Site Entrance
Wash 215	150	2551.24	2551.27	Site Exit

Roadway culvert crossings of significant washes were designed. The proposed culverts are designed to pass at least the 10-year flow without overtopping. The culverts will pass the 100-year flow with a maximum overtopping of 12-inches. Culverts will include design measures to protect the roadway from erosion during overtopping events. All lots and structures will be accessible by at least one route with a depth of flow no greater than 1 foot during the 100-year event. The proposed culverts are included in the hydraulic analysis, Refer to **Appendix C** for hydraulic results.

For areas that will require future R1-35, R1-43, and R1-70 single lot grading plans, flows will be routed around and match existing drainage discharge locations. A typical lot grading detail has been provided on the preliminary grading and drainage plan in **Appendix E** for reference.

Per section 4-1.407 of the DS&PM development within ESL should minimize the modification of significant washes and maintain these washes in their native locations and conditions. All significant washes within Phase 1C are maintained in their existing corridors. See **Appendix E** for a copy of the preliminary grading plan. The preliminary grading plan shows the HEC-RAS cross sections and BFEs for the proposed conditions. The plan also includes information on the proposed detentions basins, fished floor elevations and culvert sizes.

LOWEST FINISH FLOORS

The finished floor elevations for each lot will have a minimum elevation of one foot above the 100-year base flood elevation (BFE). See **Appendix C** for complete hydraulic results and **Appendix E** for a copy of the preliminary grading plan with BFEs and pad elevations.

SPECIAL CONDITIONS

404 PERMIT/JURISDICTIONAL WASHES

Kimley-Horn has submitted and received approval of Jurisdictional Delineation (JD) for the entire Storyrock development from U.S. Army Corps of Engineers (Corp). Kimley-Horn will also prepare and process an Individual Permit for proposed disturbances.

EROSION SETBACK ANALYSIS

A Level I erosion setback analysis was performed on the significant washes on the site. The analysis followed the requirement in the Arizona Department of Water Resources (ADWR) State Standard Attachment 5-96. Locations along the washes, where roadways or lot wall encroaches into the erosion setback, a form of erosion protection is required. A summary of the erosion setbacks for the significant washes is provided in **Table 3**. The erosion hazard setback and preliminary erosion protection shown on the preliminary grading plan, see **Appendix E**, for reference. The erosion cutoff walls or other form of erosion protection will be designed during the final design.

Setback = $\text{Sqrt}(Q100)$ for straight wash sections, with a minimum setback of 20'

Setback = $2.5 * \text{Sqrt}(Q100)$ for curved wash sections, with a minimum setback of 50'

Table 3 Erosion Setback Summary

	Q100 (cfs)	Erosion Hazard Setback, Straight Reach (Calculated)	Erosion Hazard Setback, Straight Reach (Design)	Erosion Hazard Setback, Curved Reach (Calculated)	Erosion Hazard Setback, Curved Reach (Design)
Wash 85	153	13'	20'	31'	50'
Wash 160	83	9'	20'	23'	50'
Wash 180	24	5'	20'	12'	50'
Wash 215	308	18'	20'	44'	50'

ADEQ WATER QUALITY REQUIREMENTS

Development of the project will impact a large enough area to require a submittal of a Notice of Intent (NOI) to the Arizona Department of Environmental Quality (ADEQ). The NOI will be submitted to ADEQ and an approved NOI certificate with an AZCON number will be provided to the city before approval of any improvement plans.

CULVERT SEDIMENTATION

Sedimentation reduces the hydraulic performance of culverts and can lead to safety, erosion, and maintenance issues. The proposed culverts and storm drains within the project have been designed to minimize sedimentation when possible, as well as providing solutions to reduce the impact of sedimentation. Culverts are designed to match the slope of the existing channel. Additionally, the majority of the culverts are "inlet" control, with flow velocity greater than 10 ft/s. These "self-cleaning" velocities help clear the culverts of sedimentation in larger storm events.

Storm drains which receive natural channel flow are susceptible to sedimentation. Storm drains will be designed with sediment traps prior to the storm drain to capture sediment.

Sedimentation is inevitable given the natural condition of the existing washes traversing the property. To help alleviate the potential concerns that arise from sediment build-up in culverts and storm drains on the project, it is recommended that the HOA implement a culvert and storm drain maintenance program. The scheduled program will inspect and clean the culverts to limit sedimentation and ensure proper operation of the drainage facilities. The program will inspect all culverts and storm drains bi-annually, and after significant storm events.

DATA ANALYSIS METHODS

GENERAL DISCUSSION

A detailed hydrologic model was prepared for the existing and proposed site condition. A hydraulic model was prepared for the significant washes that traverse the site. The sections below provide the hydrology and hydraulic methodology.

HYDROLOGY

The U.S. Army Corps of Engineers HEC-1 hydrologic computer program was used to determine the 2-, 10-, and 100-year peak discharges for off-site and on-site flows. HEC-1 models were prepared for the existing and proposed development conditions. The Drainage Design Management System for Windows (DDMSW) program was used to develop the hydrologic parameters for the on-site drainage areas and off-site drainage areas east of the site. Green and Ampt rainfall loss parameters were estimated using DDMSW, the City of Scottsdale parameters, and the Flood Control District of Maricopa County (FCDMC) Drainage Design Manual – Hydrology (Hydrology Manual). Time of Concentration calculations were calculated using DDMSW. Values that show non-default values or out-of-range results are due to the NMIN parameter selected for the HEC-1 Model. Because of the varying sub-basin sizes, the selected NMIN parameter will not meet the time of concentration requirements specified in the FCDMC Drainage Design Manual - Hydrology for each sub-basin. The HEC-1 models were run with varying NMIN parameters to confirm that the hydrograph shape and peaks were valid. The HEC-1 models were prepared using the Clark Unit Hydrograph. Rainfall depth were estimated for the site from the National Oceanic and Atmospheric Administration Atlas 14 (NOAA14).

Two different soil types were identified for the on-site and off-site sub-basins using the web soil survey from the National Resource Conservation Service (NRCS). A list of the soils found in the watershed is shown below:

- Gran-Wickenburg complex, 1 to 10 percent slopes
- Gran-Wickenburg-Rock outcrop complex, 10 to 65 percent slopes

The majority of the site, with the exception of a small portion of a few offsite sub-basins, falls within the 1 to 10 percent range. A map showing the different soil types, which was developed as part of the approved master drainage report, along with web soil survey results is included within **Appendix B**.

Land use parameters for the HEC-1 models were determined for each of the project zoning types, roadway and natural desert. The initial abstraction (IA) and Vegetation cover parameters are based on matching land use types from Table 4.2 of the County Hydrology Manual. The RTIMP for each zoning case was calculated by taking a sample area of roadway and lots and determining the percent of hydraulically connected area. See **Table 4** below for complete Land Use Parameters. The sample areas and RTIMP calculations are included in **Appendix B**. Land use maps for the existing and proposed development conditions are provided in **Appendix B**.

Table 4 Land Use Parameters

Land Use Code	Description	IA	RTIMP	Vegetation Cover
R1-18	Min Lot Size = 13,500 Sq Ft.	0.30	27	50.0
R1-35	Min Lot Size = 26,250 Sq Ft	0.30	21	50.0
R1-43	Min Lot Size = 32,250 Sq Ft	0.30	17	20.0
Road	24' Roadway, 40' Tract/ROW	0.10	60	75.0
Natural Desert	Natural Desert	0.35	0	25.0

A stage storage and outfall rate calculation spreadsheet was prepared for the proposed detention basins. The state storage volume is based on end-area calculations at 1-foot intervals. The basin discharge rates through the proposed bleed pipes is calculated from Manning and Orifice equations. Overflow for larger storm events are provided in an overflow weir, which will be sized at final design. Stage storage and discharge rate tables are provided in **Appendix B**.

HYDRAULICS

100-year BFEs for the significant washes were established using the U.S. Army Corps of Engineers HEC-RAS (v4.1.1) computer program. Cross sections were cut for the existing washes using the 1' flown aerial topography. The hydraulic models were run using mixed flow regime conditions with the normal depth boundary condition. Manning's 'n' coefficients for the channels was set at 0.035 and values for the overbanks are 0.050. Values were selected from Table 3-1 of the HEC-RAS Reference Manual. See **Appendix C** for Table 3-1. Based on field observations and aerial photography the washes are an undisturbed natural desert with an impervious weed barrier. One flow profile is used in the existing condition model representing the design flow. The proposed condition model uses two flow profiles. The first is a baseline of the design flow, the second is the same base design flow with the development encroachments included in the model.

Culvert crossing of the significant washes were sized using the Federal Highway Administration HY-8 version 7.30 computer program. Culverts were preliminary size to convey at least the 10-year storm through the structure, and convey the 100-year flow with a maximum roadway overtopping of 12-inches.

Refer to **Appendix C** for the results of the hydraulic modeling for the existing and proposed condition. See the attached CD for copy of the HEC-RAS report for the existing and proposed condition.

STORMWATER STORAGE METHOD

The existing property is a part of the ESLO. Based on new City ordinances, a waiver will need to be obtained for any volume less than the 100-year, 2-hour volume. However, there is no waiver fee associated with the volumes that do not result in an increase in downstream runoff. See **Appendix D** for a copy of the waiver. Refer to **Appendix B** for the pre- and post-development hydrologic model results.

Where detention basins are provided within the development to capture runoff generated on-site and discharged from roadway improvements, the basins have been sized to meet first flush storage requirements (See First Flush Summary Table in **Appendix B**). In specific areas within the development, a detention basin is not feasible to meet the first flush criteria. As outlined in the City's Stormwater and Floodplain Management Ordinance, an alternative stormwater control can be provided if the development is located within the upper desert landform and runoff has no additional adverse impact on other properties. The locations where an alternative method is proposed are identified on the preliminary grading and drainage plan (**Appendix E**) and do not adversely impact any adjacent properties. In various areas, a rip-rap spillway and small dissipation basin is proposed as an alternative stormwater control. The rip-rap basins are proposed to be 1-foot deep, and sized to reduce the velocity entering the basin for better capture of sedimentation and potential contaminants that may be present. The basin bottom was calculated using the rip-rap apron dimension requirements as outlined in the Drainage Design Manual for Maricopa County, Table 8.6. The spillways will be designed for a median rip-rap size (D50) of 6 inches to convey flow at 6-inch flow depth for the 2-year, 10-minute design storm. The higher frequency storm events are appropriate when evaluating water quality and represents the first-half inch of rainfall within the street network. Refer to **Appendix C** for the spillway and dissipation basin calculations.

There are other options for stormwater quality that could be considered for this project such as a Stormceptor system or other oil grit separators on the market. If a particular area on the project warrants this type of application in the future, specific stormwater control design measures will be provided for the City's review and approval during final design.

CONCLUSIONS

- Multiple significant washes cross the development. Proposed development will encroach on the washes. Hydraulic models for the existing and proposed conditions were prepared to determine the BFE. The BFE was used to set the finished floor elevations for each lot.
- Significant washes are maintained in their existing corridors.
- Onsite runoff will be conveyed through the local streets and storm drains to the detention basins and wash corridors. Culverts will convey the flow under the new roads. The conveyance facilities will be sized during final design.
- Hydrologic models were prepared for the on-site and off-site areas for the pre- and post-development conditions. Onsite detention basins were sized to ensure that the post-development runoff exiting the site are equal or less than pre-development conditions. Basins are design to drain within 36 hours.
- A Level 1 Erosion Setback analysis was performed for each major wash corridor. Locations where the setback is located within future development will require an erosion protection. The erosion protection will be designed as part of the final design. The Erosion Setback is shown on the Preliminary Grading Plan.
- Where detention basins are provided within the development to capture runoff generated on-site and discharged from roadway improvements, the basins have been sized to meet first flush storage requirements (See First Flush Summary Table in Appendix B). In specific areas within the development, a detention basin is not feasible to meet the first flush criteria. As outlined in the City's Stormwater and Floodplain Management Ordinance, an alternative stormwater control can be provided if the development is located within the upper desert landform and runoff has no additional adverse impact on other properties. In various areas, a rip-rap spillway and small dissipation basin is proposed as an alternative stormwater control.

Appendix A – Flood Insurance Rate Map

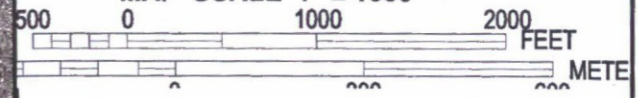
990000 FT

JOINS PANEL 1330

985000 FT



MAP SCALE 1" = 1000'



NFIP

PANEL 1335L

NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP
MARICOPA COUNTY,
ARIZONA
AND INCORPORATED AREAS

PANEL 1335 OF 4425
 (SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
MARICOPA COUNTY	040037	1335	L
SCOTTSDALE, CITY OF	045012	1335	L

MARICOPA COUNTY
UNINCORPORATED AREAS
040037

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.



MAP NUMBER
04013C1335L
MAP REVISED
OCTOBER 16, 2013

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made.

Appendix B – Hydrology

HEC-1 Exhibits (Existing and Proposed Conditions)

- HEC-1 Schematic Map
- Soils Map
- Land Use

DDSMW Output: (Existing and Proposed Conditions)

- Rainfall
- Land use
- Soils
- Storage
- Routing

HEC-1 Output

- Existing Condition
- Proposed Condition

Existing Condition

Flood Control District of Maricopa County
 Drainage Design Management System
 RAINFALL DATA
 Project Reference: STORYROCK PH1A EX

ID	Method	Duration	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
DEFAULT	NOAA14	5 MIN	0.314	0.423	0.504	0.610	0.689	0.768
	NOAA14	10 MIN	0.478	0.643	0.767	0.928	1.049	1.169
	NOAA14	15 MIN	0.593	0.798	0.951	1.151	1.300	1.449
	NOAA14	30 MIN	0.799	1.074	1.280	1.550	1.750	1.951
	NOAA14	1 HOUR	0.988	1.330	1.584	1.918	2.166	2.415
	NOAA14	2 HOUR	1.123	1.489	1.766	2.138	2.418	2.703
	NOAA14	3 HOUR	1.191	1.551	1.837	2.231	2.541	2.861
	NOAA14	6 HOUR	1.419	1.800	2.105	2.519	2.841	3.174
	NOAA14	12 HOUR	1.703	2.140	2.487	2.957	3.318	3.689
	NOAA14	24 HOUR	2.115	2.752	3.265	3.991	4.576	5.194

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters					
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin ID: 01																	
OFF95	0.064	0.74	197.8	197.8	NATURAL	0.058	0.35	0.40	6.00	0.176	Tc (Hrs)	0.545	0.508	0.453	0.399	0.366	0.341
											Vel (f/s)	1.99	2.14	2.40	2.72	2.97	3.18
											R (Hrs)	0.711	0.657	0.579	0.502	0.457	0.422
OFF100	0.011	0.23	253.3	241.8	NATURAL	0.068	0.35	0.40	6.00	0.176	Tc (Hrs)	0.310	0.289*	0.258*	0.227*	0.208*	0.194*
											Vel (f/s)	1.09	1.17	1.31	1.49	1.62	1.74
											R (Hrs)	0.407	0.376	0.331	0.288	0.262	0.241
OFF105	0.085	0.87	275.2	253.3	NATURAL	0.056	0.35	0.40	6.00	0.176	Tc (Hrs)	0.538	0.501	0.447	0.393	0.361	0.336
											Vel (f/s)	2.37	2.55	2.85	3.25	3.53	3.80
											R (Hrs)	0.678	0.627	0.552	0.479	0.436	0.402
OFF110	0.037	0.56	470.3	299.0	NATURAL	0.061	0.35	0.40	6.00	0.176	Tc (Hrs)	0.428	0.399	0.356	0.313	0.288*	0.268*
											Vel (f/s)	1.92	2.06	2.31	2.62	2.85	3.06
											R (Hrs)	0.595	0.550	0.484	0.420	0.382	0.353
OFF115	0.007	0.13	375.0	283.6	NATURAL	0.071	0.35	0.40	6.00	0.176	Tc (Hrs)	0.227*	0.211*	0.189*	0.166*	0.152*	0.142*
											Vel (f/s)	0.84	0.90	1.01	1.15	1.25	1.34
											R (Hrs)	0.236	0.218	0.192	0.167	0.152	0.140
ON135	0.008	0.20	225.6	222.8	NATURAL	0.070	0.35	0.40	6.00	0.176	Tc (Hrs)	0.301	0.281*	0.250*	0.220*	0.202*	0.188*
											Vel (f/s)	0.97	1.04	1.17	1.33	1.45	1.56
											R (Hrs)	0.422	0.391	0.344	0.298	0.272	0.251
ON140	0.017	0.36	195.6	195.6	NATURAL	0.066	0.35	0.40	6.00	0.176	Tc (Hrs)	0.408	0.380	0.339	0.298*	0.274*	0.255*
											Vel (f/s)	1.29	1.39	1.56	1.77	1.93	2.07
											R (Hrs)	0.616	0.570	0.502	0.435	0.396	0.366
ON145	0.014	0.32	214.3	213.3	NATURAL	0.067	0.35	0.40	6.00	0.176	Tc (Hrs)	0.377	0.352	0.314	0.276*	0.254*	0.236*
											Vel (f/s)	1.24	1.33	1.49	1.70	1.85	1.99
											R (Hrs)	0.575	0.531	0.468	0.406	0.369	0.341
ON150	0.007	0.16	250.0	239.9	NATURAL	0.071	0.35	0.40	6.00	0.176	Tc (Hrs)	0.265*	0.247*	0.220*	0.194*	0.178*	0.166*
											Vel (f/s)	0.89	0.95	1.07	1.21	1.32	1.41
											R (Hrs)	0.331	0.306	0.270	0.234	0.213	0.196

* Non default value or value out of range

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: STORYROCK PH1C EX

0/27/20

Land Use Code	Area (sq mi)	Area Initial Loss (IA) (%)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
DESERT	0.0107	100.0	0.35	0	25.0	DRY	Desert
	0.0107	100.0					
DESERT	0.0851	100.0	0.35	0	25.0	DRY	Desert
	0.0851	100.0					
DESERT	0.0369	100.0	0.35	0	25.0	DRY	Desert
	0.0369	100.0					
DESERT	0.0065	100.0	0.35	0	25.0	DRY	Desert
	0.0065	100.0					
DESERT	0.0642	100.0	0.35	0	25.0	DRY	Desert
	0.0642	100.0					
DESERT	0.0082	100.0	0.35	0	25.0	DRY	Desert
	0.0082	100.0					
DESERT	0.0167	100.0	0.35	0	25.0	DRY	Desert
	0.0167	100.0					
DESERT	0.0137	100.0	0.35	0	25.0	DRY	Desert
	0.0137	100.0					
DESERT	0.0067	100.0	0.35	0	25.0	DRY	Desert
	0.0067	100.0					
DESERT	0.0175	100.0	0.35	0	25.0	DRY	Desert
	0.0175	100.0					

* Non default value

(stLuDataCG

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: STORYROCK PH1C EX

0/27/20

Land Use Code	Area (sq mi)	Area Initial Loss (IA) (%)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Sub Basin ID: 01							
DESERT	0.0171	100.0	0.35	0	25.0	DRY	Desert
	0.0171	100.0					
DESERT	0.0302	100.0	0.35	0	25.0	DRY	Desert
	0.0302	100.0					
DESERT	0.0028	100.0	0.35	0	25.0	DRY	Desert
	0.0028	100.0					
DESERT	0.0024	100.0	0.35	0	25.0	DRY	Desert
	0.0024	100.0					
DESERT	0.0071	100.0	0.35	0	25.0	DRY	Desert
	0.0071	100.0					
DESERT	0.0033	100.0	0.35	0	25.0	DRY	Desert
	0.0033	100.0					
DESERT	0.0026	100.0	0.35	0	25.0	DRY	Desert
	0.0026	100.0					
DESERT	0.0018	100.0	0.35	0	25.0	DRY	Desert
	0.0018	100.0					
DESERT	0.0022	100.0	0.35	0	25.0	DRY	Desert
	0.0022	100.0					
DESERT	0.0008	100.0	0.35	0	25.0	DRY	Desert
	0.0008	100.0					

* Non default value

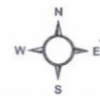
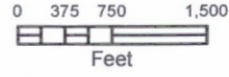
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Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
OFF100	645	61	64561	0.011	100.00	0.150	-	100	
OFF105	645	61	64561	0.085	100.00	0.150	-	100	
OFF110	645	61	64561	0.037	100.00	0.150	-	100	
OFF115	645	61	64561	0.007	100.00	0.150	-	100	
OFF95	645	61	64561	0.064	100.00	0.150	-	100	
ON135	645	61	64561	0.008	100.00	0.150	-	100	
ON140	645	61	64561	0.017	100.00	0.150	-	100	
ON145	645	61	64561	0.014	100.00	0.150	-	100	
ON150	645	61	64561	0.007	100.00	0.150	-	100	
ON155	645	61	64561	0.018	100.00	0.150	-	100	
ON160	645	61	64561	0.037	100.00	0.150	-	100	
ON165	645	61	64561	0.008	100.00	0.150	-	100	
ON170	645	61	64561	0.006	100.00	0.150	-	100	
ON175	645	61	64561	0.005	100.00	0.150	-	100	
ON180	645	61	64561	0.024	100.00	0.150	-	100	
ON185	645	61	64561	0.002	100.00	0.150	-	100	
ON190	645	61	64561	0.013	100.00	0.150	-	100	
ON195	645	61	64561	0.009	100.00	0.150	-	100	
ON200	645	61	64561	0.007	100.00	0.150	-	100	
ON205	645	61	64561	0.019	100.00	0.150	-	100	
ON210	645	61	64561	0.017	100.00	0.150	-	100	
ON215	645	61	64561	0.030	100.00	0.150	-	100	
ON220	645	61	64561	0.003	100.00	0.150	-	100	
ON235	645	61	64561	0.002	100.00	0.150	-	100	
ON240	645	61	64561	0.007	100.00	0.150	-	100	
ON245	645	61	64561	0.003	100.00	0.150	-	100	
ON260	645	61	64561	0.003	100.00	0.150	-	100	
ON265	645	61	64561	0.002	100.00	0.150	-	100	
ON270	645	61	64561	0.002	100.00	0.150	-	100	
ON280	645	61	64561	0.001	100.00	0.150	-	100	



Legend

SUBBASIN BOUNDARY
Soil ID
 64561
 64563

CAVALLIERE EXISTING SOILS MAP		Kimley»Horn	<small>© 2014 KIMLEY-HORN AND ASSOCIATES, INC. 7740 North 18th Street, Suite 300 Phoenix, Arizona 85020 (602) 944-5500</small>
<small>SCALE: 1" = 150'</small>	<small>DESIGNED BY: MAW</small>	<small>NO.</small>	<small>BY DATE</small>
<small>DRAWN BY: DWT</small>	<small>CHECKED BY: MAW</small>	<small>REVISION</small>	<small>APPR.</small>
<small>DATE: AUGUST 2014</small>			
<small>PROJECT NO.: 191089013</small>		<small>DRAWING NAME: Existing_Soils.mxd</small>	

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 ROUTING DATA
 Project Reference: STORYROCK PH1C EX

Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)		1.	2.	3.	4.	5.	6.	7.	8.
NORMAL DEPTH															
Major Basin 01															
RO140A	0.050	0.035	0.050	333.00	0.0270	-	X:	-	27.00	30.00	34.00	34.20	40.00	47.00	82.00
							Y:	3.00	2.00	1.00	-	-	1.00	2.00	3.00
RO140B	0.050	0.035	0.050	626.00	0.0340	-	X:	-	7.00	8.00	11.00	11.20	14.00	16.00	31.00
							Y:	3.00	2.00	1.00	-	-	1.00	2.00	3.00
RO140C	0.050	0.035	0.050	1,210.00	0.0320	-	X:	-	3.00	6.00	11.00	11.20	17.00	23.00	48.00
							Y:	3.00	2.00	1.00	-	-	1.00	2.00	3.00
RO155A	0.050	0.035	0.050	805.00	0.2900	-	X:	-	9.00	18.00	22.00	22.10	27.00	32.00	43.00
							Y:	3.00	2.00	1.00	-	-	1.00	2.00	3.00
RO155B	0.050	0.035	0.050	910.00	0.0350	-	X:	-	16.00	61.00	76.00	90.00	121.00	200.00	226.00
							Y:	2.00	0.50	1.00	-	-	1.00	-	2.00
RO155C	0.050	0.035	0.050	392.00	0.0330	-	X:	-	14.00	22.00	30.00	42.00	54.00	68.00	90.00
							Y:	2.00	0.50	1.00	-	-	1.00	-	2.00
RO160A	0.050	0.035	0.050	860.00	0.0300	-	X:	-	9.00	16.00	19.00	19.10	22.00	25.00	32.00
							Y:	3.00	2.00	1.00	-	-	1.00	2.00	3.00
RO160B	0.050	0.035	0.050	1,090.00	0.0300	-	X:	-	6.00	10.00	14.00	16.00	18.00	22.00	28.00
							Y:	3.00	2.00	1.00	-	-	1.00	2.00	3.00
RO160C	0.050	0.035	0.050	1,952.00	0.0300	-	X:	-	12.00	20.00	29.00	36.00	40.00	50.00	54.00
							Y:	3.00	2.00	1.00	-	-	1.00	2.00	3.00
RO170	0.050	0.035	0.050	585.00	0.0310	-	X:	-	1.00	3.00	4.00	10.00	11.00	15.00	20.00
							Y:	2.00	2.00	2.00	-	-	2.00	2.00	2.00
RO180	0.050	0.035	0.050	2,300.00	0.0340	-	X:	-	16.00	24.00	28.00	29.00	36.00	58.00	95.00
							Y:	3.00	2.00	1.00	-	-	1.00	2.00	3.00

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 ROUTING DATA
 Project Reference: STORYROCK PH1C EX

Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)		1.	2.	3.	4.	5.	6.	7.	8.
RO200A	0.050	0.035	0.050	611.00	0.0360	-	X: Y:	- 3.00	5.00 2.00	12.00 1.00	45.00 -	61.00 -	68.00 1.00	73.00 2.00	78.00 3.00
RO200B	0.050	0.035	0.050	345.00	0.0290	-	X: Y:	- 3.00	6.00 2.00	12.00 1.00	20.00 -	21.00 -	41.00 1.00	47.00 2.00	51.00 3.00
RO205	0.050	0.035	0.050	1,565.00	0.0370	-	X: Y:	- 4.00	6.00 2.00	10.00 1.00	15.00 -	48.00 -	58.00 1.00	64.00 2.00	77.00 4.00
RO210	0.050	0.035	0.050	1,804.00	0.0380	-	X: Y:	- 3.00	2.00 2.00	4.00 1.00	5.00 -	10.00 -	12.00 1.00	14.00 2.00	18.00 3.00
RO215A	0.050	0.035	0.050	975.00	0.0360	-	X: Y:	- 2.00	9.00 0.50	21.00 1.00	25.00 -	32.00 -	36.00 1.00	70.00 -	90.00 2.00
RO215B	0.050	0.035	0.050	800.00	0.0340	-	X: Y:	- 2.00	6.00 0.50	10.00 1.00	13.00 -	19.00 -	21.00 1.00	26.00 -	62.00 2.00
RO215C	0.050	0.035	0.050	445.00	0.0340	-	X: Y:	- 2.00	12.00 0.50	28.00 1.00	36.00 -	48.00 -	71.00 1.00	73.00 -	76.00 2.00
RO215D	0.050	0.035	0.050	364.00	0.0300	-	X: Y:	- 3.00	5.00 2.00	8.00 1.00	11.00 -	11.10 -	14.00 1.00	17.00 2.00	24.00 3.00
RO220	0.050	0.035	0.050	310.00	0.0420	-	X: Y:	- 2.00	8.00 0.50	15.00 1.00	21.00 -	21.10 -	25.00 1.00	30.00 -	36.00 2.00
RO240A	0.050	0.035	0.050	245.00	0.0370	-	X: Y:	- 3.00	5.00 2.00	10.00 1.00	14.00 -	32.00 -	41.00 1.00	48.00 2.00	52.00 3.00
RO240B	0.050	0.035	0.050	465.00	0.0390	-	X: Y:	- 2.00	7.00 0.50	19.00 1.00	49.00 -	65.00 -	70.00 1.00	77.00 -	85.00 2.00
RO260A	0.050	0.035	0.050	225.00	0.0360	-	X: Y:	- 2.00	7.00 1.00	15.00 -	26.00 -	29.00 1.00	34.00 1.00	42.00 0.50	50.00 1.00

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 ROUTING DATA
 Project Reference: STORYROCK PH1C EX

Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)		1.	2.	3.	4.	5.	6.	7.	8.
RO260B	0.050	0.035	0.050	361.00	0.0280	-	X:	-	23.00	44.00	69.00	70.00	85.00	92.00	104.00
							Y:	3.00	2.00	1.00	-	-	1.00	2.00	3.00

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 27OCT16 TIME 18:23:18
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*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1G5, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID STORYROCK PH1C EX - STORYROCK PHASE 1C EXISTING CONDITION
3 ID 2 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: Clark
6 ID Storm: Multiple
7 ID 10/27/2016
*DIAGRAM
8 IT 5 1JAN99 0 2000
9 IO 5
10 IN 15
*
11 JD 1.419 0.0001
12 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
13 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
14 PC 0.962 0.972 0.983 0.991 1.000
15 JD 1.410 0.5000
16 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
17 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
18 PC 0.962 0.972 0.983 0.991 1.000
*
19 KK OFF95 BASIN
20 BA 0.064
21 LG 0.35 0.40 6.00 0.18 0
22 UC 0.545 0.711
23 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
24 UA 100
*
25 KK R0140A ROUTE
26 RS 1 FLOW
27 RC 0.050 0.035 0.050 333 0.0270 0.00
28 RX 0.00 27.00 30.00 34.00 34.20 40.00 47.00 82.00
29 RY 3.00 2.00 1.00 0.00 0.00 1.00 2.00 3.00
*
30 KK OFF100 BASIN
31 BA 0.011
    
```

34	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
35	UA	100									
	*										
36	KK	R0140B	ROUTE								
37	RS	1	FLOW								
38	RC	0.050	0.035	0.050	626	0.0340	0.00				
39	RX	0.00	7.00	8.00	11.00	11.20	14.00	16.00	31.00		
40	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										

1

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

41	KK	C0140A	COMBINE								
42	HC	2									
	*										
43	KK	R0140C	ROUTE								
44	RS	1	FLOW								
45	RC	0.050	0.035	0.050	1210	0.0320	0.00				
46	RX	0.00	3.00	6.00	11.00	11.20	17.00	23.00	48.00		
47	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
48	KK	ON140	BASIN								
49	BA	0.017									
50	LG	0.35	0.40	6.00	0.18	0					
51	UC	0.408	0.616								
52	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
53	UA	100									
	*										

54	KK	C0140B	COMBINE								
55	HC	2									
	*										
56	KK	R0155B	ROUTE								
57	RS	1	FLOW								
58	RC	0.050	0.035	0.050	910	0.0350	0.00				
59	RX	0.00	16.00	61.00	76.00	90.00	121.00	200.00	226.00		
60	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										
61	KK	ON135	BASIN								
62	BA	0.008									
63	LG	0.35	0.40	6.00	0.18	0					
64	UC	0.301	0.422								
65	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
66	UA	100									
	*										

67	KK	R0155A	ROUTE								
68	RS	1	FLOW								
69	RC	0.050	0.035	0.050	805	0.2900	0.00				
70	RX	0.00	9.00	18.00	22.00	22.10	27.00	32.00	43.00		
71	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
72	KK	C0155A	COMBINE								
73	HC	2									
	*										

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

74	KK	R0155C	ROUTE								
75	RS	1	FLOW								
76	RC	0.050	0.035	0.050	392	0.0330	0.00				
77	RX	0.00	14.00	22.00	30.00	42.00	54.00	68.00	90.00		
78	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										
79	KK	ON155	BASIN								
80	BA	0.017									
81	LG	0.35	0.40	6.00	0.18	0					

83	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
84	UA	100									
	*										
85	KK	CO155B	COMBINE								
86	HC	2									
	*										
87	KK	ON145	BASIN								
88	BA	0.014									
89	LG	0.35	0.40	6.00	0.18	0					
90	UC	0.377	0.575								
91	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
92	UA	100									
	*										
93	KK	RO160A	ROUTE								
94	RS	1	FLOW								
95	RC	0.050	0.035	0.050	860	0.0300	0.00				
96	RX	0.00	9.00	16.00	19.00	19.10	22.00	25.00	32.00		
97	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
98	KK	ON150	BASIN								
99	BA	0.007									
100	LG	0.35	0.40	6.00	0.18	0					
101	UC	0.265	0.331								
102	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
103	UA	100									
	*										
104	KK	RO160B	ROUTE								
105	RS	1	FLOW								
106	RC	0.050	0.035	0.050	1090	0.0300	0.00				
107	RX	0.00	6.00	10.00	14.00	16.00	18.00	22.00	28.00		
108	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										

HEC-1 INPUT

PAGE 4

1
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

109	KK	CO160A	COMBINE								
110	HC	2									
	*										
111	KK	RO160C	ROUTE								
112	RS	1	FLOW								
113	RC	0.050	0.035	0.050	1952	0.0300	0.00				
114	RX	0.00	12.00	20.00	29.00	36.00	40.00	50.00	54.00		
115	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
116	KK	ON160	BASIN								
117	BA	0.037									
118	LG	0.35	0.40	6.00	0.18	0					
119	UC	0.520	0.790								
120	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
121	UA	100									
	*										
122	KK	CO160B	COMBINE								
123	HC	2									
	*										
124	KK	RO170	ROUTE								
125	RS	1	FLOW								
126	RC	0.050	0.035	0.050	585	0.0310	0.00				
127	RX	0.00	1.00	3.00	4.00	10.00	11.00	15.00	20.00		
128	RY	2.00	2.00	2.00	0.00	0.00	2.00	2.00	2.00		
	*										
129	KK	ON170	BASIN								
130	BA	0.006									
131	LG	0.35	0.40	6.00	0.18	0					
132	UC	0.258	0.316								
133	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0

185	UC	0.538	0.678								
186	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
187	UA	100									
	*										
188	KK	RO215A	ROUTE								
189	RS	1	FLOW								
190	RC	0.050	0.035	0.050	975	0.0360	0.00				
191	RX	0.00	9.00	21.00	25.00	32.00	36.00	70.00	90.00		
192	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										
193	KK	OFF110	BASIN								
194	BA	0.037									
195	LG	0.35	0.40	6.00	0.18	0					
196	UC	0.428	0.595								
197	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
198	UA	100									
	*										
199	KK	OFF115	BASIN								
200	BA	0.007									
201	LG	0.35	0.40	6.00	0.18	0					
202	UC	0.227	0.236								
203	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
204	UA	100									
	*										
205	KK	RO220	ROUTE								
206	RS	1	FLOW								
207	RC	0.050	0.035	0.050	310	0.0420	0.00				
208	RX	0.00	8.00	15.00	21.00	21.10	25.00	30.00	36.00		
209	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										
210	KK	ON220	BASIN								
211	BA	0.003									
212	LG	0.35	0.40	6.00	0.18	0					
213	UC	0.222	0.326								
214	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
215	UA	100									
	*										
216	KK	CO220	COMBINE								
217	HC	2									
	*										

HEC-1 INPUT

PAGE 7

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

218	KK	RO215D	ROUTE								
219	RS	1	FLOW								
220	RC	0.050	0.035	0.050	364	0.0300	0.00				
221	RX	0.00	5.00	8.00	11.00	11.10	14.00	17.00	24.00		
222	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
223	KK	CO215A	COMBINE								
224	HC	2									
	*										
225	KK	RO215B	ROUTE								
226	RS	1	FLOW								
227	RC	0.050	0.035	0.050	800	0.0340	0.00				
228	RX	0.00	6.00	10.00	13.00	19.00	21.00	26.00	62.00		
229	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										
230	KK	CO215B	COMBINE								
231	HC	2									
	*										
232	KK	RO215C	ROUTE								
233	RS	1	FLOW								
234	RC	0.050	0.035	0.050	445	0.0340	0.00				
235	RX	0.00	12.00	28.00	36.00	48.00	71.00	73.00	76.00		

237 KK ON215 BASIN
 238 BA 0.030
 239 LG 0.35 0.40 6.00 0.18 0
 240 UC 0.349 0.341
 241 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 242 UA 100
 *

243 KK CO215C COMBINE
 244 HC 2
 *

245 KK RO240A ROUTE
 246 RS 1 FLOW
 247 RC 0.050 0.035 0.050 245 0.0370 0.00
 248 RX 0.00 5.00 10.00 14.00 32.00 41.00 48.00 52.00
 249 RY 3.00 2.00 1.00 0.00 0.00 1.00 2.00 3.00
 *

250 KK ON235 BASIN
 251 BA 0.002
 252 LG 0.35 0.40 6.00 0.18 0
 253 UC 0.176 0.222
 254 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 255 UA 100
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

256 KK CO240A COMBINE
 257 HC 2
 *

258 KK RO240B ROUTE
 259 RS 1 FLOW
 260 RC 0.050 0.035 0.050 465 0.0390 0.00
 261 RX 0.00 7.00 19.00 49.00 65.00 70.00 77.00 85.00
 262 RY 2.00 0.50 1.00 0.00 0.00 1.00 0.00 2.00
 *

263 KK ON240 BASIN
 264 BA 0.007
 265 LG 0.35 0.40 6.00 0.18 0
 266 UC 0.276 0.380
 267 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 268 UA 100
 *

269 KK CO240B COMBINE
 270 HC 2
 *

271 KK RO260A ROUTE
 272 RS 1 FLOW
 273 RC 0.050 0.035 0.050 225 0.0360 0.00
 274 RX 0.00 7.00 15.00 26.00 29.00 34.00 42.00 50.00
 275 RY 2.00 1.00 0.00 0.00 1.00 1.00 0.50 1.00
 *

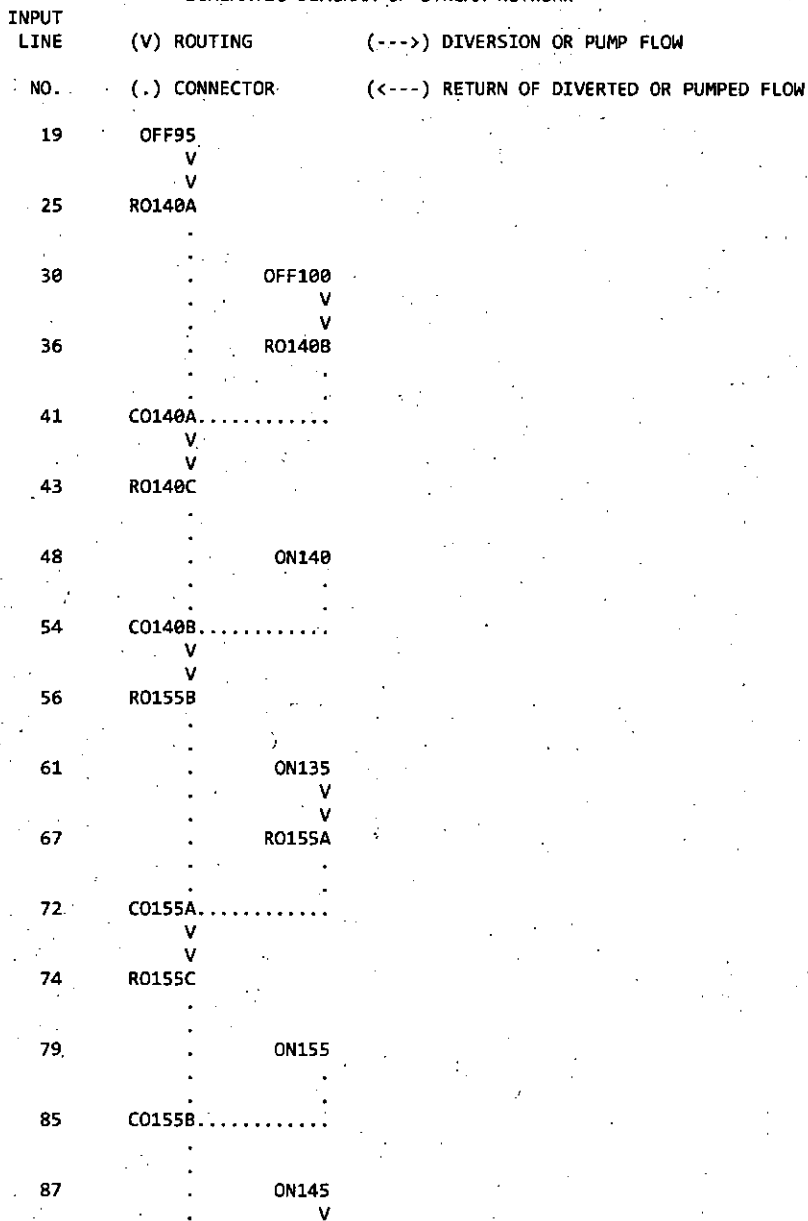
276 KK ON245 BASIN
 277 BA 0.003
 278 LG 0.35 0.40 6.00 0.18 0
 279 UC 0.230 0.363
 280 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 281 UA 100
 *

282 KK RO260B ROUTE
 283 RS 1 FLOW
 284 RC 0.050 0.035 0.050 361 0.0280 0.00
 285 RX 0.00 23.00 44.00 69.00 70.00 85.00 92.00 104.00
 286 RY 3.00 2.00 1.00 0.00 0.00 1.00 2.00 3.00
 *

340	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
341	UA	100									
	*										
342	KK	RO210	ROUTE								
343	RS	1	FLOW								
344	RC	0.050	0.035	0.050	1804	0.0380	0.00				
345	RX	0.00	2.00	4.00	5.00	10.00	12.00	14.00	18.00		
346	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
347	KK	ON210	BASIN								
348	BA	0.017									
349	LG	0.35	0.40	6.00	0.18	0					
350	UC	0.403	0.608								
351	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
352	UA	100									
	*										
353	KK	CO205	COMBINE								
354	HC	4									
	*										
355	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK



98		ON150		
		V		
		V		
104		R0160B		
109	CO160A			
	V			
	V			
111	R0160C			
116		ON160		
122	CO160B			
	V			
	V			
124	R0170			
129		ON170		
135	CO170			
137		ON165		
143		ON175		
149			ON180	
155	COEX1			
157		ON195		
		V		
		V		
163		R0180		
168	CO180			
170		ON185		
176		ON190		
182			OFF105	
			V	
			V	
188			R0215A	
193				OFF110
199				OFF115
				V
				V
205				R0220
210				ON220
216				CO220
				V
				V

223
225
230
232
237
243
245
250
256
258
263
269
271
276
282
287
293
295
300
306
311
317
323
325
330
336

CO215A.....
V
V
RO215B

CO215B.....
V
V
RO215C

ON215

CO215C.....
V
V
RO240A

ON235

CO240A.....
V
V
RO240B

ON240

CO240B.....
V
V
RO250A

ON245
V
V
RO260B

ON260

CO260.....
V
V
RO200A

ON265
V
V
RO200B

ON200

ON270

CO200.....
V
V
RO205

ON205

ON280

342 R0210
 347 ON210
 353 CO205.....

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * JUN 1998 *
 * VERSION 4.1 *
 * RUN DATE 27OCT16 TIME 18:23:18 *

 * U.S. ARMY CORPS OF ENGINEERS *
 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 756-1104 *

Flood Control District of Maricopa County
 STORYROCK PH1C EX - STORYROCK PHASE 1C EXISTING CONDITION
 2 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 10/27/2016

9 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 5 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 2000 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 7JAN99 ENDING DATE
 NDTIME 2235 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.08 HOURS
 TOTAL TIME BASE 166.58 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

11 JD INDEX STORM NO. 1
 STRM 1.42 PRECIPITATION DEPTH
 TRDA 0.00 TRANSPOSITION DRAINAGE AREA

12 PI PRECIPITATION PATTERN
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.03
 0.03 0.03 0.05 0.05 0.05 0.15 0.15 0.15 0.03 0.03
 0.03 0.01 0.01 0.01 0.01 0.01 0.01 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00

15 JD INDEX STORM NO. 2
 STRM 1.41 PRECIPITATION DEPTH
 TRDA 0.50 TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.03
0.03	0.03	0.05	0.05	0.05	0.15	0.15	0.15	0.03	0.03
0.03	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00								

1

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+	ROUTED TO								
	OFF95	12.	4.42	2.	1.	0.	0.06		
+	ROUTED TO								
	RO140A	12.	4.42	2.	1.	0.	0.06		
+	HYDROGRAPH AT								
+	ROUTED TO								
	OFF100	3.	4.25	0.	0.	0.	0.01		
+	ROUTED TO								
	RO140B	3.	4.25	0.	0.	0.	0.01		
+	2 COMBINED AT								
+	ROUTED TO								
	CO140A	15.	4.42	2.	1.	0.	0.08		
+	ROUTED TO								
	RO140C	14.	4.50	2.	1.	0.	0.08		
+	HYDROGRAPH AT								
+	ROUTED TO								
	ON140	4.	4.33	1.	0.	0.	0.02		
+	2 COMBINED AT								
+	ROUTED TO								
	CO140B	17.	4.50	3.	1.	0.	0.09		
+	ROUTED TO								
	RO155B	16.	4.58	3.	1.	0.	0.09		
+	HYDROGRAPH AT								
+	ROUTED TO								
	ON135	2.	4.25	0.	0.	0.	0.01		
+	ROUTED TO								
	RO155A	2.	4.25	0.	0.	0.	0.01		
+	2 COMBINED AT								
+	ROUTED TO								
	CO155A	18.	4.58	3.	1.	0.	0.10		
+	ROUTED TO								
	RO155C	18.	4.58	3.	1.	0.	0.10		
+	HYDROGRAPH AT								
+	ROUTED TO								
	ON155	5.	4.25	1.	0.	0.	0.02		
+	2 COMBINED AT								
+	ROUTED TO								
	CO155B	20.	4.58	4.	1.	0.	0.12		
+	HYDROGRAPH AT								
+	ROUTED TO								
	ON145	3.	4.25	0.	0.	0.	0.01		
+	ROUTED TO								
	RO160A	3.	4.33	0.	0.	0.	0.01		
+	HYDROGRAPH AT								
+	ROUTED TO								
	ON150	2.	4.17	0.	0.	0.	0.01		
+	ROUTED TO								
	RO160B	2.	4.25	0.	0.	0.	0.01		
+	2 COMBINED AT								
+	ROUTED TO								
	CO160A	5.	4.33	1.	0.	0.	0.02		
+	ROUTED TO								
	RO160C	4.	4.50	1.	0.	0.	0.02		

+	2 COMBINED AT	CO1608	10.	4.42	2.	0.	0.	0.06
	ROUTED TO	RO170	10.	4.50	2.	0.	0.	0.06
+	HYDROGRAPH AT	ON170	2.	4.17	0.	0.	0.	0.01
+	2 COMBINED AT	CO170	11.	4.50	2.	1.	0.	0.06
+	HYDROGRAPH AT	ON165	2.	4.25	0.	0.	0.	0.01
+	HYDROGRAPH AT	ON175	2.	4.17	0.	0.	0.	0.00
+	HYDROGRAPH AT	ON180	5.	4.33	1.	0.	0.	0.02
+	4 COMBINED AT	COEX1	18.	4.42	3.	1.	0.	0.10
+	HYDROGRAPH AT	ON195	2.	4.25	0.	0.	0.	0.01
+	ROUTED TO	RO180	2.	4.42	0.	0.	0.	0.01
+	2 COMBINED AT	CO180	20.	4.42	3.	1.	0.	0.11
+	HYDROGRAPH AT	ON185	1.	4.08	0.	0.	0.	0.00
+	HYDROGRAPH AT	ON190	3.	4.25	0.	0.	0.	0.01
+	HYDROGRAPH AT	OFF105	17.	4.42	3.	1.	0.	0.09
+	ROUTED TO	RO215A	16.	4.50	3.	1.	0.	0.09
+	HYDROGRAPH AT	OFF110	8.	4.33	1.	0.	0.	0.04
+	HYDROGRAPH AT	OFF115	3.	4.17	0.	0.	0.	0.01
+	ROUTED TO	RO220	3.	4.17	0.	0.	0.	0.01
+	HYDROGRAPH AT	ON220	1.	4.17	0.	0.	0.	0.00
+	2 COMBINED AT	CO220	4.	4.17	0.	0.	0.	0.01
+	ROUTED TO	RO215D	4.	4.17	0.	0.	0.	0.01
+	2 COMBINED AT	CO215A	12.	4.25	1.	0.	0.	0.05
+	ROUTED TO	RO215B	11.	4.33	1.	0.	0.	0.05
+	2 COMBINED AT	CO215B	26.	4.42	4.	1.	0.	0.13
+	ROUTED TO	RO215C	26.	4.50	4.	1.	0.	0.13
	HYDROGRAPH AT							

185	UC	0.447	0.552									
186	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
187	UA	100										
	*											
188	KK	RO215A	ROUTE									
189	RS	1	FLOW									
190	RC	0.050	0.035	0.050	975	0.0360	0.00					
191	RX	0.00	9.00	21.00	25.00	32.00	36.00	70.00	90.00			
192	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00			
	*											
193	KK	OFF110	BASIN									
194	BA	0.037										
195	LG	0.35	0.40	6.00	0.18	0						
196	UC	0.356	0.484									
197	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
198	UA	100										
	*											
199	KK	OFF115	BASIN									
200	BA	0.007										
201	LG	0.35	0.40	6.00	0.18	0						
202	UC	0.189	0.192									
203	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
204	UA	100										
	*											
205	KK	RO220	ROUTE									
206	RS	1	FLOW									
207	RC	0.050	0.035	0.050	310	0.0420	0.00					
208	RX	0.00	8.00	15.00	21.00	21.10	25.00	30.00	36.00			
209	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00			
	*											
210	KK	ON220	BASIN									
211	BA	0.003										
212	LG	0.35	0.40	6.00	0.18	0						
213	UC	0.184	0.266									
214	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
215	UA	100										
	*											

216	KK	CO220	COMBINE
217	HC	2	
	*		

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

218	KK	RO215D	ROUTE									
219	RS	1	FLOW									
220	RC	0.050	0.035	0.050	364	0.0300	0.00					
221	RX	0.00	5.00	8.00	11.00	11.10	14.00	17.00	24.00			
222	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00			
	*											

223	KK	CO215A	COMBINE
224	HC	2	
	*		

225	KK	RO215B	ROUTE									
226	RS	1	FLOW									
227	RC	0.050	0.035	0.050	800	0.0340	0.00					
228	RX	0.00	6.00	10.00	13.00	19.00	21.00	26.00	62.00			
229	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00			
	*											

230	KK	CO215B	COMBINE
231	HC	2	
	*		

232	KK	RO215C	ROUTE									
233	RS	1	FLOW									
234	RC	0.050	0.035	0.050	445	0.0340	0.00					
235	RX	0.00	12.00	28.00	36.00	48.00	71.00	73.00	76.00			

340	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
341	UA	100									
	*										
342	KK	RO210	ROUTE								
343	RS	1	FLOW								
344	RC	0.050	0.035	0.050	1804	0.0380	0.00				
345	RX	0.00	2.00	4.00	5.00	10.00	12.00	14.00	18.00		
346	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
347	KK	ON210	BASIN								
348	BA	0.017									
349	LG	0.35	0.40	6.00	0.18	0					
350	UC	0.335	0.495								
351	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
352	UA	100									
	*										
353	KK	CO205	COMBINE								
354	HC	4									
	*										
355	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(---->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
19	OFF95	
	V	
	V	
25	RO140A	
	.	
30	OFF100	
	V	
	V	
36	RO140B	
	.	
41	CO140A.....	
	V	
	V	
43	RO140C	
	.	
48	ON140	
	.	
54	CO140B.....	
	V	
	V	
56	RO155B	
	.	
61	ON135	
	V	
	V	
67	RO155A	
	.	
72	CO155A.....	
	V	
	V	
74	RO155C	
	.	
79	ON155	
	.	
85	CO155B.....	
	.	
87	ON145	
	V	

98		ON150		
		V		
		V		
104		RO160B		
109	CO160A.....			
	V			
	V			
111	RO160C			
116		ON160		
122	CO160B.....			
	V			
	V			
124	RO170			
129		ON170		
135	CO170.....			
137		ON165		
143		ON175		
149			ON180	
155	COEX1.....			
157		ON195		
		V		
		V		
163		RO180		
168	CO180.....			
170		ON185		
176		ON190		
182			OFF105	
			V	
			V	
188			RO215A	
193				OFF110
199				OFF115
				V
				V
205				RO220
210				ON220
216				CO220.....
				V
				V

223
225
230
232
237
243
245
250
256
258
263
269
271
276
282
287
293
295
300
306
311
317
323
325
330
336

CO215A
V
V
RO215B
CO215B
V
V
RO215C
ON215
CO215C
V
V
RO240A
ON235
CO240A
V
V
RO240B
ON240
CO240B
V
V
RO260A
ON245
V
V
RO260B
ON260
CO260
V
V
RO200A
ON265
V
V
RO200B
ON200
ON270
CO200
V
V
RO205
ON205
ON280

0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.03
0.03	0.03	0.05	0.05	0.05	0.15	0.15	0.15	0.03	0.03
0.03	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00								

1

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+	ROUTED TO								
	OFF95	36.	4.33	5.	1.	0.	0.06		
+	ROUTED TO								
	RO140A	36.	4.33	5.	1.	0.	0.06		
+	HYDROGRAPH AT								
	OFF100	9.	4.17	1.	0.	0.	0.01		
+	ROUTED TO								
	RO140B	9.	4.17	1.	0.	0.	0.01		
+	2 COMBINED AT								
	CO140A	43.	4.33	6.	1.	0.	0.08		
+	ROUTED TO								
	RO140C	42.	4.33	6.	1.	0.	0.08		
+	HYDROGRAPH AT								
	ON140	11.	4.25	1.	0.	0.	0.02		
+	2 COMBINED AT								
	CO140B	52.	4.33	7.	2.	1.	0.09		
+	ROUTED TO								
	RO155B	49.	4.42	7.	2.	1.	0.09		
+	HYDROGRAPH AT								
	ON135	7.	4.17	1.	0.	0.	0.01		
+	ROUTED TO								
	RO155A	7.	4.17	1.	0.	0.	0.01		
+	2 COMBINED AT								
	CO155A	52.	4.42	8.	2.	1.	0.10		
+	ROUTED TO								
	RO155C	52.	4.42	8.	2.	1.	0.10		
+	HYDROGRAPH AT								
	ON155	13.	4.17	1.	0.	0.	0.02		
+	2 COMBINED AT								
	CO155B	61.	4.42	9.	2.	1.	0.12		
+	HYDROGRAPH AT								
	ON145	9.	4.25	1.	0.	0.	0.01		
+	ROUTED TO								
	RO160A	9.	4.25	1.	0.	0.	0.01		
+	HYDROGRAPH AT								
	ON150	7.	4.17	1.	0.	0.	0.01		
+	ROUTED TO								
	RO160B	6.	4.17	1.	0.	0.	0.01		
+	2 COMBINED AT								
	CO160A	15.	4.25	2.	0.	0.	0.02		
+	ROUTED TO								
	RO160C	12.	4.33	2.	0.	0.	0.02		

+	2 COMBINED AT	C0160B	32.	4.33	4.	1.	0.	0.06
	ROUTED TO							
+		R0170	31.	4.33	4.	1.	0.	0.06
	HYDROGRAPH AT							
+		ON170	6.	4.08	0.	0.	0.	0.01
	2 COMBINED AT							
+		C0170	34.	4.33	5.	1.	0.	0.06
	HYDROGRAPH AT							
+		ON165	6.	4.17	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON175	5.	4.17	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON180	13.	4.25	2.	0.	0.	0.02
	4 COMBINED AT							
+		COEX1	54.	4.33	8.	2.	1.	0.10
	HYDROGRAPH AT							
+		ON195	7.	4.17	1.	0.	0.	0.01
	ROUTED TO							
+		R0180	5.	4.33	1.	0.	0.	0.01
	2 COMBINED AT							
+		C0180	59.	4.33	9.	2.	1.	0.11
	HYDROGRAPH AT							
+		ON185	2.	4.08	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON190	9.	4.25	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		OFF105	49.	4.33	7.	2.	1.	0.09
	ROUTED TO							
+		R0215A	46.	4.42	7.	2.	1.	0.09
	HYDROGRAPH AT							
+		OFF110	24.	4.25	3.	1.	0.	0.04
	HYDROGRAPH AT							
+		OFF115	8.	4.08	1.	0.	0.	0.01
	ROUTED TO							
+		R0220	8.	4.17	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON220	3.	4.08	0.	0.	0.	0.00
	2 COMBINED AT							
+		C0220	11.	4.08	1.	0.	0.	0.01
	ROUTED TO							
+		R0215D	11.	4.17	1.	0.	0.	0.01
	2 COMBINED AT							
+		C0215A	33.	4.25	4.	1.	0.	0.05
	ROUTED TO							
+		R0215B	32.	4.25	4.	1.	0.	0.05
	2 COMBINED AT							
+		C0215B	75.	4.33	10.	3.	1.	0.13
	ROUTED TO							
+		R0215C	74.	4.33	10.	3.	1.	0.13
	HYDROGRAPH AT							

+	2 COMBINED AT	CO215C	93.	4.33	13.	3.	1.	0.16
	ROUTED TO							
+		RO240A	93.	4.33	13.	3.	1.	0.16
	HYDROGRAPH AT							
+		ON235	2.	4.08	0.	0.	0.	0.00
+	2 COMBINED AT	CO240A	94.	4.33	13.	3.	1.	0.16
	ROUTED TO							
+		RO240B	94.	4.33	13.	3.	1.	0.16
	HYDROGRAPH AT							
+		ON240	6.	4.17	1.	0.	0.	0.01
+	2 COMBINED AT	CO240B	97.	4.33	13.	3.	1.	0.17
	ROUTED TO							
+		RO260A	97.	4.33	13.	3.	1.	0.17
	HYDROGRAPH AT							
+		ON245	3.	4.08	0.	0.	0.	0.00
	ROUTED TO							
+		RO260B	3.	4.17	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON260	3.	4.08	0.	0.	0.	0.00
+	3 COMBINED AT	CO260	100.	4.33	14.	3.	1.	0.18
	ROUTED TO							
+		RO200A	99.	4.42	14.	3.	1.	0.18
	HYDROGRAPH AT							
+		ON265	2.	4.08	0.	0.	0.	0.00
	ROUTED TO							
+		RO200B	2.	4.17	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON200	7.	4.08	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON270	2.	4.08	0.	0.	0.	0.00
+	4 COMBINED AT	CO200	104.	4.33	15.	4.	1.	0.19
	ROUTED TO							
+		RO205	102.	4.42	15.	4.	1.	0.19
	HYDROGRAPH AT							
+		ON205	14.	4.25	1.	0.	0.	0.02
	HYDROGRAPH AT							
+		ON280	1.	4.00	0.	0.	0.	0.00
	ROUTED TO							
+		RO210	1.	4.17	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON210	11.	4.25	1.	0.	0.	0.02
+	4 COMBINED AT	CO205	120.	4.42	17.	4.	1.	0.23

83	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
84	UA	100									
	*										
85	KK	CO155B	COMBINE								
86	HC	2									
	*										
87	KK	ON145	BASIN								
88	BA	0.014									
89	LG	0.35	0.40	6.00	0.18	0					
90	UC	0.236	0.341								
91	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
92	UA	100									
	*										
93	KK	RO160A	ROUTE								
94	RS	1	FLOW								
95	RC	0.050	0.035	0.050	860	0.0300	0.00				
96	RX	0.00	9.00	16.00	19.00	19.10	22.00	25.00	32.00		
97	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
98	KK	ON150	BASIN								
99	BA	0.007									
100	LG	0.35	0.40	6.00	0.18	0					
101	UC	0.166	0.196								
102	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
103	UA	100									
	*										
104	KK	RO160B	ROUTE								
105	RS	1	FLOW								
106	RC	0.050	0.035	0.050	1090	0.0300	0.00				
107	RX	0.00	6.00	10.00	14.00	16.00	18.00	22.00	28.00		
108	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										

HEC-1 INPUT

PAGE 4

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

109	KK	CO160A	COMBINE								
110	HC	2									
	*										
111	KK	RO160C	ROUTE								
112	RS	1	FLOW								
113	RC	0.050	0.035	0.050	1952	0.0300	0.00				
114	RX	0.00	12.00	20.00	29.00	36.00	40.00	50.00	54.00		
115	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
116	KK	ON160	BASIN								
117	BA	0.037									
118	LG	0.35	0.40	6.00	0.18	0					
119	UC	0.325	0.468								
120	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
121	UA	100									
	*										
122	KK	CO160B	COMBINE								
123	HC	2									
	*										
124	KK	RO170	ROUTE								
125	RS	1	FLOW								
126	RC	0.050	0.035	0.050	585	0.0310	0.00				
127	RX	0.00	1.00	3.00	4.00	10.00	11.00	15.00	20.00		
128	RY	2.00	2.00	2.00	0.00	0.00	2.00	2.00	2.00		
	*										
129	KK	ON170	BASIN								
130	BA	0.006									
131	LG	0.35	0.40	6.00	0.18	0					
132	UC	0.161	0.187								
133	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0

185	UC	0.336	0.402									
186	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
187	UA	100										
	*											
188	KK	RO215A	ROUTE									
189	RS	1	FLOW									
190	RC	0.050	0.035	0.050	975	0.0360	0.00					
191	RX	0.00	9.00	21.00	25.00	32.00	36.00	70.00	90.00			
192	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00			
	*											
193	KK	OFF110	BASIN									
194	BA	0.037										
195	LG	0.35	0.40	6.00	0.18	0						
196	UC	0.268	0.353									
197	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
198	UA	100										
	*											
199	KK	OFF115	BASIN									
200	BA	0.007										
201	LG	0.35	0.40	6.00	0.18	0						
202	UC	0.142	0.140									
203	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
204	UA	100										
	*											
205	KK	RO220	ROUTE									
206	RS	1	FLOW									
207	RC	0.050	0.035	0.050	310	0.0420	0.00					
208	RX	0.00	8.00	15.00	21.00	21.10	25.00	30.00	36.00			
209	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00			
	*											
210	KK	ON220	BASIN									
211	BA	0.003										
212	LG	0.35	0.40	6.00	0.18	0						
213	UC	0.139	0.194									
214	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
215	UA	100										
	*											
216	KK	CO220	COMBINE									
217	HC	2										
	*											

HEC-1 INPUT

PAGE 7

1

LINE	ID	1	2	3	4	5	6	7	8	9	10
218	KK	RO215D	ROUTE								
219	RS	1	FLOW								
220	RC	0.050	0.035	0.050	364	0.0300	0.00				
221	RX	0.00	5.00	8.00	11.00	11.10	14.00	17.00	24.00		
222	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
223	KK	CO215A	COMBINE								
224	HC	2									
	*										
225	KK	RO215B	ROUTE								
226	RS	1	FLOW								
227	RC	0.050	0.035	0.050	800	0.0340	0.00				
228	RX	0.00	6.00	10.00	13.00	19.00	21.00	26.00	62.00		
229	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										
230	KK	CO215B	COMBINE								
231	HC	2									
	*										
232	KK	RO215C	ROUTE								
233	RS	1	FLOW								
234	RC	0.050	0.035	0.050	445	0.0340	0.00				
235	RX	0.00	12.00	28.00	36.00	48.00	71.00	73.00	76.00		

340	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
341	UA	100									
	*										
342	KK	RO210	ROUTE								
343	RS	1	FLOW								
344	RC	0.050	0.035	0.050	1804	0.0380	0.00				
345	RX	0.00	2.00	4.00	5.00	10.00	12.00	14.00	18.00		
346	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
347	KK	ON210	BASIN								
348	BA	0.017									
349	LG	0.35	0.40	6.00	0.18	0					
350	UC	0.252	0.361								
351	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
352	UA	100									
	*										
353	KK	CO205	COMBINE								
354	HC	4									
	*										
355	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
19	OFF95	
	V	
	V	
25	RO140A	
	.	
30	OFF100	
	V	
	V	
36	RO140B	
	.	
41	CO140A.....	
	V	
	V	
43	RO140C	
	.	
48	ON140	
	.	
54	CO140B.....	
	V	
	V	
56	RO155B	
	.	
61	ON135	
	V	
	V	
67	RO155A	
	.	
72	CO155A.....	
	V	
	V	
74	RO155C	
	.	
79	ON155	
	.	
85	CO155B.....	
	.	
87	ON145	
	V	

98		ON150	
		V	
		V	
104		RO160B	
109	CO160A		
	V		
	V		
111	RO160C		
116		ON160	
122	CO160B		
	V		
	V		
124	RO170		
129		ON170	
135	CO170		
137		ON165	
143		ON175	
149		ON180	
155	COEX1		
157		ON195	
		V	
		V	
163		RO180	
168	CO180		
170		ON185	
176		ON190	
182		OFF105	
		V	
		V	
188		RO215A	
193		OFF110	
199		OFF115	
		V	
		V	
205		RO220	
210			ON220
216		CO220	
		V	
		V	

223
225
230
232
237
243
245
250
256
258
263
269
271
276
282
287
293
295
300
306
311
317
323
325
330
336

C0215A
V
V
RO215B

C0215B
V
V
RO215C

ON215

C0215C
V
V
RO240A

ON235

C0240A
V
V
RO240B

ON240

C0240B
V
V
RO260A

ON245
V
V
RO260B

ON260

C0260
V
V
RO200A

ON265
V
V
RO200B

ON200

ON270

C0200
V
V
RO205

ON205

ON280

0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.03
0.03	0.03	0.05	0.05	0.05	0.15	0.15	0.15	0.03	0.03
0.03	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00								

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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+	ROUTED TO								
	OFF95	89.	4.25	11.	3.	1.	0.06		
+	ROUTED TO								
	RO140A	90.	4.25	11.	3.	1.	0.06		
+	HYDROGRAPH AT								
+	ROUTED TO								
	OFF100	22.	4.08	2.	0.	0.	0.01		
+	ROUTED TO								
	RO140B	21.	4.17	2.	0.	0.	0.01		
+	2 COMBINED AT								
+	ROUTED TO								
	CO140A	107.	4.25	12.	3.	1.	0.08		
+	ROUTED TO								
	RO140C	107.	4.25	12.	3.	1.	0.08		
+	HYDROGRAPH AT								
+	ROUTED TO								
	ON140	26.	4.17	3.	1.	0.	0.02		
+	2 COMBINED AT								
+	ROUTED TO								
	CO140B	130.	4.25	15.	4.	1.	0.09		
+	ROUTED TO								
	RO155B	124.	4.33	15.	4.	1.	0.09		
+	HYDROGRAPH AT								
+	ROUTED TO								
	ON135	16.	4.08	1.	0.	0.	0.01		
+	ROUTED TO								
	RO155A	15.	4.17	1.	0.	0.	0.01		
+	2 COMBINED AT								
+	ROUTED TO								
	CO155A	133.	4.33	17.	4.	1.	0.10		
+	ROUTED TO								
	RO155C	134.	4.33	17.	4.	1.	0.10		
+	HYDROGRAPH AT								
+	ROUTED TO								
	ON155	31.	4.17	3.	1.	0.	0.02		
+	2 COMBINED AT								
+	ROUTED TO								
	CO155B	153.	4.25	19.	5.	2.	0.12		
+	HYDROGRAPH AT								
+	ROUTED TO								
	ON145	23.	4.17	2.	1.	0.	0.01		
+	ROUTED TO								
	RO160A	22.	4.17	2.	1.	0.	0.01		
+	HYDROGRAPH AT								
+	ROUTED TO								
	ON150	16.	4.08	1.	0.	0.	0.01		
+	ROUTED TO								
	RO160B	14.	4.17	1.	0.	0.	0.01		
+	2 COMBINED AT								
+	ROUTED TO								
	CO160A	37.	4.17	4.	1.	0.	0.02		
+	ROUTED TO								
	RO160C	33.	4.25	4.	1.	0.	0.02		

+	2 COMBINED AT	C0160B	81.	4.25	10.	2.	1.	0.06
	ROUTED TO							
+		R0170	81.	4.25	10.	2.	1.	0.06
	HYDROGRAPH AT							
+		ON170	14.	4.08	1.	0.	0.	0.01
	2 COMBINED AT							
+		C0170	89.	4.25	11.	3.	1.	0.06
	HYDROGRAPH AT							
+		ON165	15.	4.08	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON175	11.	4.08	1.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON180	32.	4.17	4.	1.	0.	0.02
	4 COMBINED AT							
+		COEX1	139.	4.17	17.	4.	1.	0.10
	HYDROGRAPH AT							
+		ON195	16.	4.08	2.	0.	0.	0.01
	ROUTED TO							
+		R0180	13.	4.25	2.	0.	0.	0.01
	2 COMBINED AT							
+		C0180	152.	4.25	18.	5.	2.	0.11
	HYDROGRAPH AT							
+		ON185	5.	4.00	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON190	21.	4.17	2.	1.	0.	0.01
	HYDROGRAPH AT							
+		OFF105	122.	4.25	14.	4.	1.	0.09
	ROUTED TO							
+		R0215A	116.	4.25	14.	4.	1.	0.09
	HYDROGRAPH AT							
+		OFF110	58.	4.17	6.	2.	1.	0.04
	HYDROGRAPH AT							
+		OFF115	17.	4.08	1.	0.	0.	0.01
	ROUTED TO							
+		R0220	17.	4.08	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON220	7.	4.08	1.	0.	0.	0.00
	2 COMBINED AT							
+		C0220	24.	4.08	2.	0.	0.	0.01
	ROUTED TO							
+		R0215D	24.	4.08	2.	0.	0.	0.01
	2 COMBINED AT							
+		C0215A	79.	4.17	8.	2.	1.	0.05
	ROUTED TO							
+		R0215B	77.	4.17	8.	2.	1.	0.05
	2 COMBINED AT							
+		C0215B	190.	4.25	22.	5.	2.	0.13
	ROUTED TO							
+		R0215C	189.	4.25	22.	5.	2.	0.13
	HYDROGRAPH AT							

+	2 COMBINED AT	CO215C	233.	4.25	27.	7.	2.	0.16
	ROUTED TO							
+		RO240A	233.	4.25	27.	7.	2.	0.16
	HYDROGRAPH AT							
+		ON235	5.	4.00	0.	0.	0.	0.00
+	2 COMBINED AT	CO240A	235.	4.25	27.	7.	2.	0.16
	ROUTED TO							
+		RO240B	236.	4.25	27.	7.	2.	0.16
	HYDROGRAPH AT							
+		ON240	15.	4.08	1.	0.	0.	0.01
+	2 COMBINED AT	CO240B	245.	4.25	28.	7.	2.	0.17
	ROUTED TO							
+		RO260A	244.	4.25	28.	7.	2.	0.17
	HYDROGRAPH AT							
+		ON245	6.	4.08	1.	0.	0.	0.00
	ROUTED TO							
+		RO260B	6.	4.08	1.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON260	7.	4.08	1.	0.	0.	0.00
+	3 COMBINED AT	CO260	251.	4.25	29.	7.	2.	0.18
	ROUTED TO							
+		RO200A	252.	4.25	29.	7.	2.	0.18
	HYDROGRAPH AT							
+		ON265	4.	4.08	0.	0.	0.	0.00
	ROUTED TO							
+		RO200B	4.	4.08	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON200	16.	4.08	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON270	5.	4.00	0.	0.	0.	0.00
+	4 COMBINED AT	CO200	265.	4.25	31.	8.	3.	0.19
	ROUTED TO							
+		RO205	257.	4.33	31.	8.	3.	0.19
	HYDROGRAPH AT							
+		ON205	33.	4.17	3.	1.	0.	0.02
	HYDROGRAPH AT							
+		ON280	3.	4.00	0.	0.	0.	0.00
	ROUTED TO							
+		RO210	2.	4.08	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON210	27.	4.17	3.	1.	0.	0.02
+	4 COMBINED AT	CO205	307.	4.25	37.	9.	3.	0.23

Proposed Condition

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Project Reference: STORYROCK PH1C PROP

2/20/2017

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Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr	
Major Basin ID: 01																		
OFF95	0.064	0.74	197.8	197.8	NATURAL	0.058	0.35	0.40	6.00	0.176		Tc (Hrs)	0.545	0.508	0.453	0.399	0.366	0.341
												Vel (f/s)	1.99	2.14	2.40	2.72	2.97	3.18
												R (Hrs)	0.711	0.657	0.579	0.502	0.457	0.422
OFF100	0.011	0.23	253.3	241.8	NATURAL	0.068	0.35	0.40	6.00	0.176		Tc (Hrs)	0.310	0.289*	0.258*	0.227*	0.208*	0.194*
												Vel (f/s)	1.09	1.17	1.31	1.49	1.62	1.74
												R (Hrs)	0.407	0.376	0.331	0.288	0.262	0.241
OFF105	0.085	0.87	275.2	253.3	NATURAL	0.056	0.35	0.40	6.00	0.176		Tc (Hrs)	0.538	0.501	0.447	0.393	0.361	0.336
												Vel (f/s)	2.37	2.55	2.85	3.25	3.53	3.80
												R (Hrs)	0.678	0.627	0.552	0.479	0.436	0.402
OFF110	0.037	0.56	470.3	299.0	NATURAL	0.061	0.35	0.40	6.00	0.176		Tc (Hrs)	0.428	0.399	0.356	0.313	0.288*	0.268*
												Vel (f/s)	1.92	2.06	2.31	2.62	2.85	3.06
												R (Hrs)	0.595	0.550	0.484	0.420	0.382	0.353
OFF115	0.007	0.13	375.0	283.6	NATURAL	0.071	0.35	0.40	6.00	0.176		Tc (Hrs)	0.227*	0.211*	0.189*	0.166*	0.152*	0.142*
												Vel (f/s)	0.84	0.90	1.01	1.15	1.25	1.34
												R (Hrs)	0.236	0.218	0.192	0.167	0.152	0.140
ON135	0.008	0.20	225.6	222.8	NATURAL	0.070	0.35	0.40	6.00	0.176		Tc (Hrs)	0.301	0.281*	0.250*	0.220*	0.202*	0.188*
												Vel (f/s)	0.97	1.04	1.17	1.33	1.45	1.56
												R (Hrs)	0.422	0.391	0.344	0.298	0.272	0.251
ON140	0.017	0.36	195.6	195.6	NATURAL	0.064	0.34	0.39	6.00	0.180	4	Tc (Hrs)	0.391	0.366	0.328	0.290*	0.267*	0.249*
												Vel (f/s)	1.35	1.44	1.61	1.82	1.98	2.12
												R (Hrs)	0.588	0.546	0.483	0.422	0.385	0.356
ON145	0.014	0.32	214.3	213.3	NATURAL	0.062	0.31	0.38	6.00	0.188	9	Tc (Hrs)	0.341	0.321	0.290*	0.258*	0.238*	0.223*
												Vel (f/s)	1.38	1.46	1.62	1.82	1.97	2.10
												R (Hrs)	0.514	0.481	0.428	0.376	0.345	0.320
ON150	0.007	0.16	250.0	239.9	NATURAL	0.071	0.35	0.40	6.00	0.176		Tc (Hrs)	0.265*	0.247*	0.220*	0.194*	0.178*	0.166*
												Vel (f/s)	0.89	0.95	1.07	1.21	1.32	1.41
												R (Hrs)	0.331	0.306	0.270	0.234	0.213	0.196

* Non default value or value out of range

(stSubBasCG.rpt)

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS
 Project Reference: STORYROCK PH1C PROP

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr	
Major Basin ID: 01																		
DN155	0.017	0.28	191.3	191.3	NATURAL	0.052	0.33	0.34	6.00	0.192	11	Tc (Hrs)	0.297*	0.280*	0.253*	0.225*	0.208*	0.195 *
												Vel (f/s)	1.38	1.47	1.62	1.83	1.97	2.11
												R (Hrs)	0.354	0.332	0.296	0.261	0.239	0.222
DN160	0.020	0.61	178.5	178.5	NATURAL	0.049	0.33	0.33	6.00	0.197	14	Tc (Hrs)	0.428	0.404	0.366	0.327	0.303	0.284 *
												Vel (f/s)	2.09	2.21	2.44	2.74	2.95	3.15
												R (Hrs)	0.904	0.848	0.760	0.670	0.616	0.573
DN161	0.010	0.32	224.7	222.1	NATURAL	0.063	0.34	0.37	6.00	0.183	5	Tc (Hrs)	0.348	0.326	0.293*	0.259*	0.239*	0.223 *
												Vel (f/s)	1.35	1.44	1.60	1.81	1.96	2.10
												R (Hrs)	0.636	0.592	0.525	0.458	0.419	0.388
DN165	0.008	0.35	183.4	183.4	NATURAL	0.040	0.31	0.27	6.00	0.212	24	Tc (Hrs)	0.274*	0.260*	0.237*	0.214*	0.199*	0.188 *
												Vel (f/s)	1.87	1.97	2.17	2.40	2.58	2.73
												R (Hrs)	0.594	0.562	0.508	0.451	0.418	0.391
DN170	0.005	0.14	204.4	204.2	NATURAL	0.073	0.35	0.40	6.00	0.176		Tc (Hrs)	0.265*	0.247*	0.220*	0.193*	0.178*	0.165 *
												Vel (f/s)	0.77	0.83	0.93	1.06	1.15	1.24
												R (Hrs)	0.360	0.332	0.293	0.254	0.231	0.213
DN175	0.002	0.06	327.6	272.1	NATURAL	0.039	0.30	0.25	6.00	0.167	17	Tc (Hrs)	0.099*	0.094*	0.085*	0.076*	0.071*	0.067 *
												Vel (f/s)	0.89	0.94	1.04	1.16	1.24	1.31
												R (Hrs)	0.103	0.097	0.087	0.077	0.072	0.067
DN180	0.005	0.14	957.7	313.0	NATURAL	0.059	0.25	0.34	6.00	0.209	24	Tc (Hrs)	0.181*	0.172*	0.157*	0.141*	0.132*	0.124 *
												Vel (f/s)	1.13	1.19	1.31	1.46	1.56	1.66
												R (Hrs)	0.235	0.222	0.201	0.179	0.166	0.155
DN179	0.002	0.07	291.7	260.3	NATURAL	0.079	0.35	0.40	6.00	0.176		Tc (Hrs)	0.181*	0.168*	0.150*	0.132*	0.121*	0.113 *
												Vel (f/s)	0.57	0.61	0.68	0.78	0.85	0.91
												R (Hrs)	0.228	0.211	0.186	0.161	0.147	0.135
DN181	0.004	0.13	310.1	266.8	NATURAL	0.047	0.31	0.29	6.00	0.207	20	Tc (Hrs)	0.165*	0.156*	0.142*	0.128*	0.119*	0.112 *
												Vel (f/s)	1.16	1.22	1.34	1.49	1.60	1.70
												R (Hrs)	0.227	0.214	0.193	0.171	0.158	0.148

* Non default value or value out of range

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS
 Project Reference: STORYROCK PH1C PROP

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr	
Major Basin ID: 01																		
ON182	0.009	0.19	203.2	203.0	NATURAL	0.050	0.32	0.32	6.00	0.198	15	Tc (Hrs)	0.230*	0.217*	0.197*	0.176*	0.163*	0.153 *
												Vel (f/s)	1.21	1.28	1.41	1.58	1.71	1.82
												R (Hrs)	0.281	0.264	0.237	0.209	0.192	0.179
ON183	0.007	0.27	200.0	199.9	NATURAL	0.056	0.33	0.34	6.00	0.179	9	Tc (Hrs)	0.299*	0.282*	0.254*	0.226*	0.209*	0.196 *
												Vel (f/s)	1.32	1.40	1.56	1.75	1.89	2.02
												R (Hrs)	0.575	0.539	0.481	0.421	0.386	0.359
ON184	0.009	0.18	165.7	165.7	NATURAL	0.035	0.30	0.25	6.00	0.216	27	Tc (Hrs)	0.186*	0.177*	0.162*	0.146*	0.136*	0.129 *
												Vel (f/s)	1.42	1.49	1.63	1.81	1.94	2.05
												R (Hrs)	0.212	0.201	0.182	0.162	0.151	0.141
ON185	0.002	0.07	309.9	266.8	NATURAL	0.079	0.35	0.40	6.00	0.176		Tc (Hrs)	0.179*	0.167*	0.149*	0.131*	0.121*	0.112 *
												Vel (f/s)	0.57	0.61	0.69	0.78	0.85	0.92
												R (Hrs)	0.226	0.209	0.184	0.160	0.145	0.134
ON190	0.006	0.21	232.2	227.8	NATURAL	0.048	0.32	0.30	6.00	0.203	18	Tc (Hrs)	0.225*	0.213*	0.193*	0.173*	0.161*	0.151 *
												Vel (f/s)	1.37	1.45	1.60	1.78	1.91	2.04
												R (Hrs)	0.374	0.352	0.317	0.280	0.258	0.241
ON191	0.006	0.14	273.4	252.5	NATURAL	0.041	0.31	0.27	6.00	0.182	17	Tc (Hrs)	0.161*	0.153*	0.138*	0.124*	0.115*	0.109 *
												Vel (f/s)	1.28	1.34	1.49	1.66	1.79	1.88
												R (Hrs)	0.187	0.176	0.158	0.139	0.129	0.121
ON195	0.009	0.23	214.9	213.8	NATURAL	0.070	0.35	0.40	6.00	0.176		Tc (Hrs)	0.327	0.305	0.272*	0.239*	0.220*	0.204 *
												Vel (f/s)	1.03	1.11	1.24	1.41	1.53	1.65
												R (Hrs)	0.484	0.448	0.394	0.342	0.311	0.287
ON200	0.006	0.15	328.8	272.4	NATURAL	0.072	0.35	0.40	6.00	0.176		Tc (Hrs)	0.249*	0.232*	0.207*	0.182*	0.167*	0.155 *
												Vel (f/s)	0.88	0.95	1.06	1.21	1.32	1.42
												R (Hrs)	0.320	0.296	0.260	0.226	0.206	0.190
ON210	0.017	0.36	192.7	192.7	NATURAL	0.066	0.35	0.40	6.00	0.176		Tc (Hrs)	0.410	0.382	0.341	0.300	0.275*	0.256 *
												Vel (f/s)	1.29	1.38	1.55	1.76	1.92	2.06
												R (Hrs)	0.620	0.573	0.504	0.438	0.398	0.367

* Non default value or value out of range

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Project Reference: STORYROCK PH1C PROP

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr	
Major Basin ID: 01																		
ON205	0.020	0.33	224.9	222.2	NATURAL	0.062	0.35	0.39	6.00	0.179	2	Tc (Hrs)	0.359	0.335	0.299*	0.264*	0.243*	0.226 *
												Vel (f/s)	1.35	1.44	1.62	1.83	1.99	2.14
												R (Hrs)	0.454	0.421	0.372	0.323	0.295	0.272
ON215	0.030	0.32	251.6	240.8	NATURAL	0.062	0.35	0.40	6.00	0.176		Tc (Hrs)	0.349	0.325	0.290*	0.255*	0.235*	0.218 *
												Vel (f/s)	1.34	1.44	1.62	1.84	2.00	2.15
												R (Hrs)	0.341	0.316	0.278	0.241	0.219	0.202
ON220	0.003	0.11	293.6	261.0	NATURAL	0.076	0.35	0.40	6.00	0.176		Tc (Hrs)	0.222*	0.207*	0.184*	0.162*	0.149*	0.139 *
												Vel (f/s)	0.73	0.78	0.88	1.00	1.08	1.16
												R (Hrs)	0.326	0.302	0.266	0.231	0.210	0.194
ON235	0.002	0.07	369.9	282.5	NATURAL	0.079	0.35	0.40	6.00	0.176		Tc (Hrs)	0.176*	0.164*	0.146*	0.129*	0.118*	0.110 *
												Vel (f/s)	0.58	0.63	0.70	0.80	0.87	0.93
												R (Hrs)	0.222	0.205	0.181	0.157	0.143	0.132
ON240	0.007	0.18	280.2	255.6	NATURAL	0.071	0.35	0.40	6.00	0.176		Tc (Hrs)	0.276*	0.257*	0.229*	0.202*	0.185*	0.172 *
												Vel (f/s)	0.96	1.03	1.15	1.31	1.43	1.53
												R (Hrs)	0.380	0.351	0.309	0.269	0.244	0.225
ON245	0.003	0.12	317.1	269.0	NATURAL	0.076	0.35	0.40	6.00	0.176		Tc (Hrs)	0.230*	0.214*	0.191*	0.168*	0.154*	0.143 *
												Vel (f/s)	0.77	0.82	0.92	1.05	1.14	1.23
												R (Hrs)	0.363	0.336	0.296	0.257	0.234	0.216
ON260	0.003	0.09	219.8	218.0	NATURAL	0.076	0.35	0.40	6.00	0.176		Tc (Hrs)	0.212*	0.198*	0.176*	0.155*	0.143*	0.133 *
												Vel (f/s)	0.62	0.67	0.75	0.85	0.92	0.99
												R (Hrs)	0.265	0.245	0.215	0.187	0.170	0.157
ON265	0.002	0.09	311.1	267.1	NATURAL	0.079	0.35	0.40	6.00	0.176		Tc (Hrs)	0.203*	0.189*	0.169*	0.149*	0.137*	0.127 *
												Vel (f/s)	0.65	0.70	0.78	0.89	0.96	1.04
												R (Hrs)	0.318	0.294	0.259	0.225	0.204	0.189
ON270	0.002	0.08	292.7	260.7	NATURAL	0.079	0.35	0.40	6.00	0.176		Tc (Hrs)	0.193*	0.180*	0.160*	0.141*	0.130*	0.121 *
												Vel (f/s)	0.61	0.65	0.73	0.83	0.90	0.97
												R (Hrs)	0.273	0.253	0.222	0.193	0.176	0.162

* Non default value or value out of range

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Project Reference: STORYROCK PH1C PROP

Area ID	Sub Basin Parameters					Kb	Rainfall Losses					Return Period Parameters					
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area		IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin ID: 01																	
ON280	0.001	0.04	388.9	286.4	NATURAL	0.083	0.35	0.40	6.00	0.176	Tc (Hrs)	0.136*	0.127*	0.113*	0.100*	0.091*	0.085*
											Vel (f/s)	0.43	0.46	0.52	0.59	0.64	0.69
											R (Hrs)	0.158	0.146	0.129	0.112	0.102	0.094

* Non default value or value out of range

Flood Control District of Maricopa County
 Drainage Design Management System
LAND USE
 Project Reference: STORYROCK PH1C PROP

2/20/201

	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
for Basin ID: 01									
00	DESERT	0.0107	100.0	0.35	0	25.0	DRY	0.068	Desert
		0.0107	100.0						
05	DESERT	0.0851	100.0	0.35	0	25.0	DRY	0.056	Desert
		0.0851	100.0						
10	DESERT	0.0369	100.0	0.35	0	25.0	DRY	0.061	Desert
		0.0369	100.0						
15	DESERT	0.0065	100.0	0.35	0	25.0	DRY	0.071	Desert
		0.0065	100.0						
5	DESERT	0.0642	100.0	0.35	0	25.0	DRY	0.058	Desert
		0.0642	100.0						
5	DESERT	0.0082	100.0	0.35	0	25.0	DRY	0.070	Desert
		0.0082	100.0						
0	DESERT	0.0160	94.1	0.35	0	25.0	DRY	0.066	Desert
	ROAD	0.0010	5.9	0.10	60	75.0	NORMAL	0.034	Local Roadway 40' Tract 24' BC
		0.0170	100.0						
5	DESERT	0.0120	85.7	0.35	0	25.0	DRY	0.067	Desert
	ROAD	0.0020	14.3	0.10	60	75.0	NORMAL	0.034	Local Roadway 40' Tract 24' BC
		0.0140	100.0						
0	DESERT	0.0067	100.0	0.35	0	25.0	DRY	0.071	Desert
		0.0067	100.0						
5	DESERT	0.0100	58.8	0.35	0	25.0	DRY	0.066	Desert

* Non default value

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Flood Control District of Maricopa County
 Drainage Design Management System
LAND USE
 Project Reference: STORYROCK PH1C PROP

2/20/201

Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Basin ID: 01								
R1-18	0.0070	41.2	0.30	27	50.0	NORMAL	0.034	Residential 18,000 sq-ft lots
	0.0170	100.0						
DESERT	0.0100	50.0	0.35	0	25.0	DRY	0.065	Desert
R1-18	0.0100	50.0	0.30	27	50.0	NORMAL	0.033	Residential 18,000 sq-ft lots
	0.0200	100.0						
DESERT	0.0090	81.8	0.35	0	25.0	DRY	0.069	Desert
R1-18	0.0020	18.2	0.30	27	50.0	NORMAL	0.035	Residential 18,000 sq-ft lots
	0.0110	100.0						
DESERT	0.0010	12.5	0.35	0	25.0	DRY	0.070	Desert
R1-18	0.0070	87.5	0.30	27	50.0	NORMAL	0.036	Residential 18,000 sq-ft lots
	0.0080	100.0						
DESERT	0.0050	100.0	0.35	0	25.0	DRY	0.073	Desert
	0.0050	100.0						
R1-43	0.0020	100.0	0.30	17	20.0	NORMAL	0.039	Residential 43,000 sq-ft lots
	0.0020	100.0						
DESERT	0.0020	100.0	0.35	0	25.0	DRY	0.079	Desert
	0.0020	100.0						
DESERT	0.0030	60.0	0.35	0	25.0	DRY	0.073	Desert
ROAD	0.0020	40.0	0.10	60	75.0	NORMAL	0.037	Local Roadway 40' Tract 24' BC
	0.0050	100.0						

* Non default value

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Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: STORYROCK PH1C PROP

2/20/201

Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Basin ID: 01								
1 DESERT	0.0010	25.0	0.35	0	25.0	DRY	0.074	Desert
R1-18	0.0030	75.0	0.30	27	50.0	NORMAL	0.037	Residential 18,000 sq-ft lots
	0.0040	100.0						
2 DESERT	0.0040	44.4	0.35	0	25.0	DRY	0.070	Desert
R1-18	0.0050	55.6	0.30	27	50.0	NORMAL	0.035	Residential 18,000 sq-ft lots
	0.0090	100.0						
3 DESERT	0.0040	57.1	0.35	0	25.0	DRY	0.071	Desert
R1-18	0.0010	14.3	0.30	27	50.0	NORMAL	0.036	Residential 18,000 sq-ft lots
R1-43	0.0020	28.6	0.30	17	20.0	NORMAL	0.036	Residential 43,000 sq-ft lots
	0.0070	100.0						
4 R1-18	0.0090	100.0	0.30	27	50.0	NORMAL	0.035	Residential 18,000 sq-ft lots
	0.0090	100.0						
5 DESERT	0.0010	100.0	0.35	0	25.0	DRY	0.079	Desert
	0.0010	100.0						
0 DESERT	0.0020	33.3	0.35	0	25.0	DRY	0.072	Desert
R1-18	0.0040	66.7	0.30	27	50.0	NORMAL	0.036	Residential 18,000 sq-ft lots
	0.0060	100.0						
1 DESERT	0.0010	14.3	0.35	0	25.0	DRY	0.072	Desert
R1-18	0.0020	28.6	0.30	27	50.0	NORMAL	0.036	Residential 18,000 sq-ft lots
R1-43	0.0040	57.1	0.30	17	20.0	NORMAL	0.036	Residential 43,000 sq-ft lots

* Non default value

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Flood Control District of Maricopa County
 Drainage Design Management System
LAND USE
 Project Reference: STORYROCK PH1C PROP

2/20/201

Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Basin ID: 01								
	0.0070	100.0						
05 DESERT	0.0093	100.0	0.35	0	25.0	DRY	0.070	Desert
	0.0093	100.0						
00 DESERT	0.0060	100.0	0.35	0	25.0	DRY	0.072	Desert
	0.0060	100.0						
05 DESERT	0.0185	92.5	0.35	0	25.0	DRY	0.065	Desert
R1-18	0.0015	7.5	0.30	27	50.0	NORMAL	0.033	Residential 18,000 sq-ft lots
	0.0200	100.0						
00 DESERT	0.0170	100.0	0.35	0	25.0	DRY	0.066	Desert
	0.0170	100.0						
5 DESERT	0.0302	100.0	0.35	0	25.0	DRY	0.062	Desert
	0.0302	100.0						
00 DESERT	0.0028	100.0	0.35	0	25.0	DRY	0.076	Desert
	0.0028	100.0						
5 DESERT	0.0024	100.0	0.35	0	25.0	DRY	0.079	Desert
	0.0024	100.0						
00 DESERT	0.0071	100.0	0.35	0	25.0	DRY	0.071	Desert
	0.0071	100.0						
5 DESERT	0.0033	100.0	0.35	0	25.0	DRY	0.076	Desert
	0.0033	100.0						
00 DESERT	0.0026	100.0	0.35	0	25.0	DRY	0.076	Desert

* Non default value

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Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: STORYROCK PH1C PROP

2/20/201

Land Use Code Area (sq mi) Area (%) Initial Loss (tA) Percent Impervious (RTIMP) Vegetation Cover (%) DTHETA Kb Description

for Basin ID: 01

		0.0026	100.0						
265	DESERT	0.0018	100.0	0.35	0	25.0	DRY	0.079	Desert
		0.0018	100.0						
270	DESERT	0.0022	100.0	0.35	0	25.0	DRY	0.079	Desert
		0.0022	100.0						
280	DESERT	0.0010	100.0	0.35	0	25.0	DRY	0.083	Desert
		0.0010	100.0						

* Non default value

(stLuDataCG

Project **Storyrock**

Subject **Land Use Summary Table**

Designed by **ZJH**

Date **2/5/2016**

Project No. **191069020**

Checked by **JMB**

Date **2/5/2016**

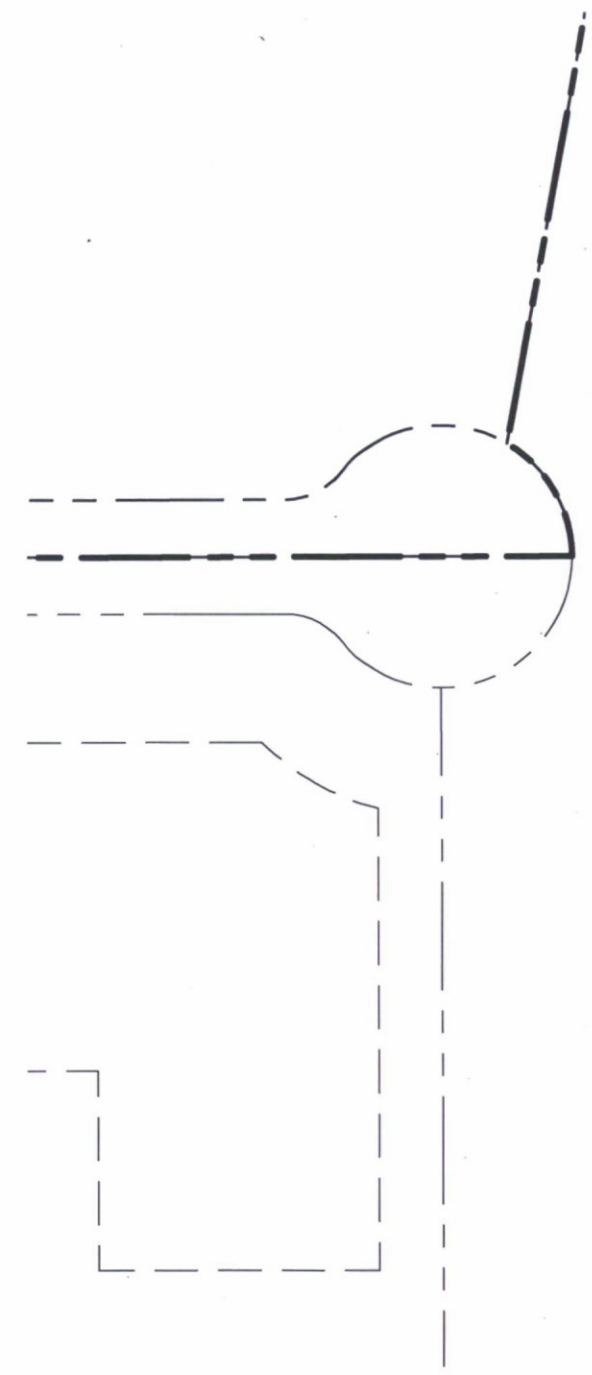
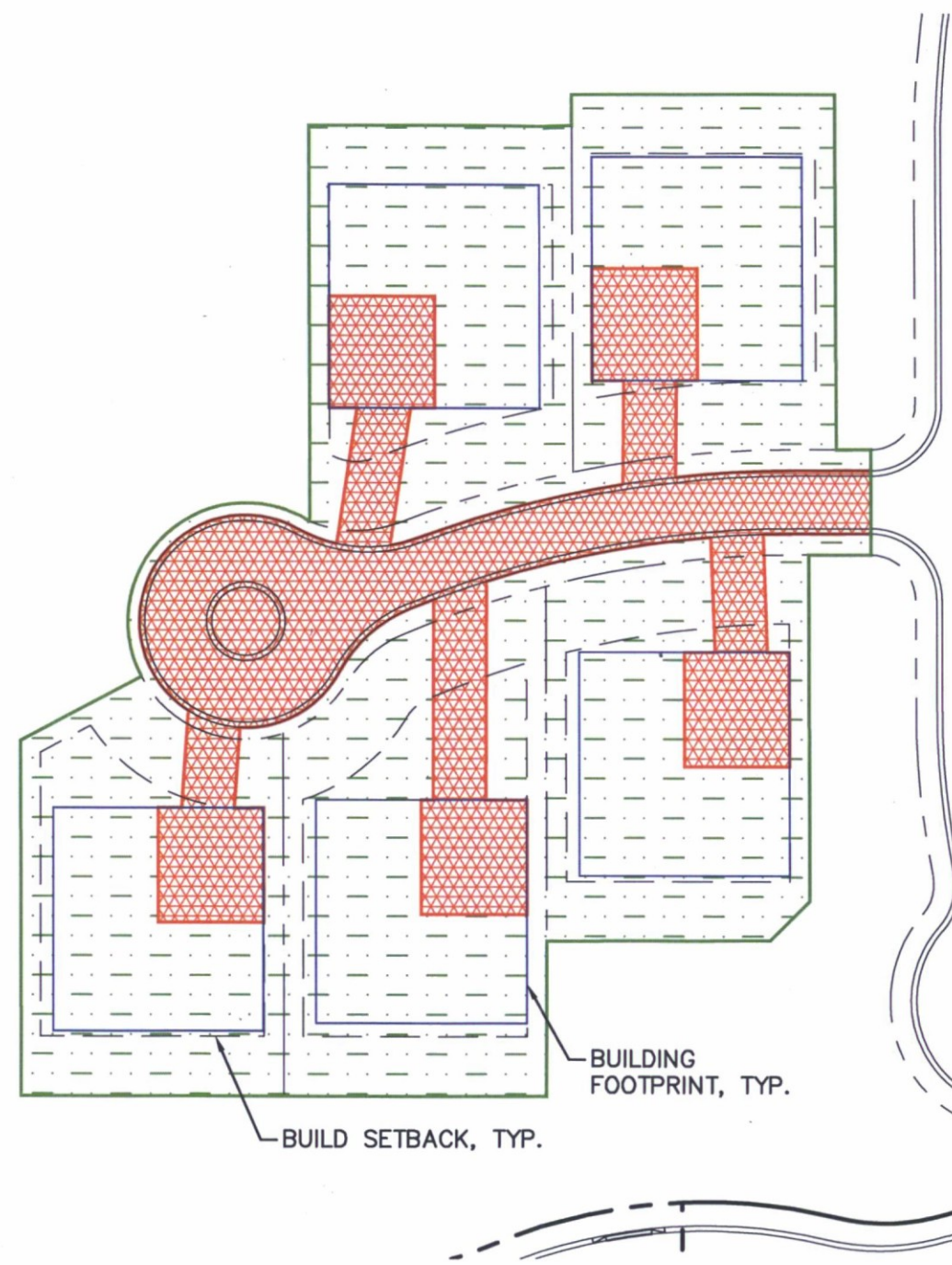
Objective: Land Use Hec-1 Values

Land Use Code	Description	IA	RTIMP	Vegetation Cover
R1-18	Min Lot Size = 13,500 Sq Ft	0.30	27	50.0
R1-35	Min Lot Size = 26,250 Sq Ft	0.30	21	50.0
R1-43	Min Lot Size = 32,250 Sq Ft	0.30	17	20.0
R1-70	Min Lot Size = 52,500 Sq Ft	0.30	14	20.0
Road	24' Roadway, 40' Tract/ROW	0.10	60	75.0
Natural Desert	Natural Desert	0.35	0	25.0



SCALE: 1" = 60'

TOTAL AREA = 88,500 SF
HYDRAULICALLY CONNECTED AREA = 23,800 SF
RTIMP = 23,800/88,500 = 26.9% CALCULATED
= 27.0% DESIGN



BUILD SETBACK, TYP.
BUILDING FOOTPRINT, TYP.

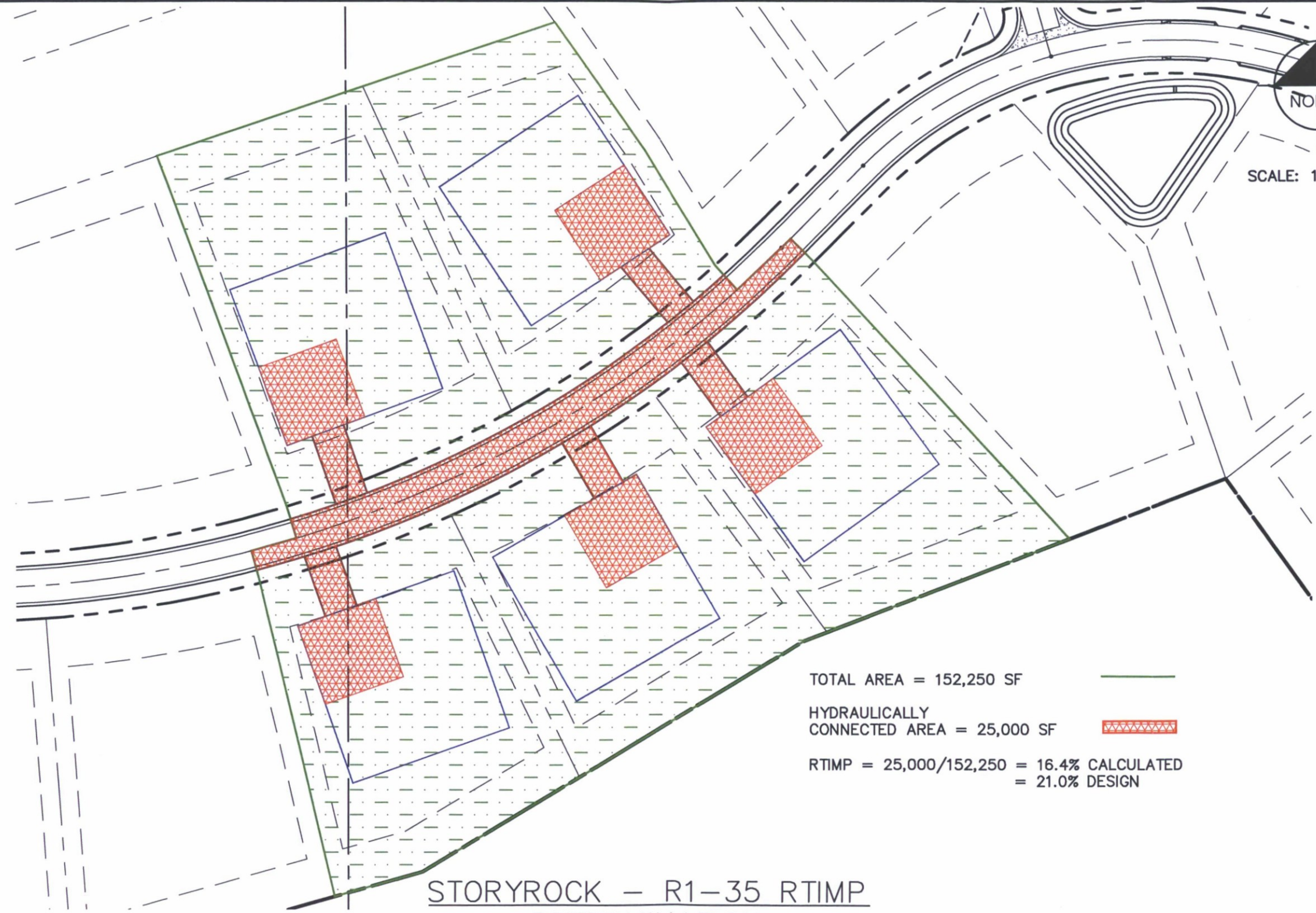
STORYROCK - R1-18 RTIMP DETERMINATION

\\s:\na\civ\17180000 - Storyrock\Drawings\Phase 1A\Storyrock\Work\Storyrock_Trad_L1.dwg, May 02, 2017 10:48:48 AM
PLOT: 17180000 - RTIMP - PHA - 17180000 - 18 - 17180000 - PHA - 17180000





SCALE: 1" = 60'



TOTAL AREA = 152,250 SF
HYDRAULICALLY CONNECTED AREA = 25,000 SF
RTIMP = $25,000 / 152,250 = 16.4\%$ CALCULATED
= 21.0% DESIGN

STORYROCK - R1-35 RTIMP DETERMINATION



\\snp\csl\10000000 - Storyrock\Drawings\Site\RTIMP\Storyrock RTIMP R1-35.dwg Plot Date: 08/20/17 10:48 AM
PLOT SCALE: 1"=60'0" PLOT AREA: 152,250 SF HYDRAULICALLY CONNECTED AREA: 25,000 SF RTIMP: 16.4% CALCULATED, 21.0% DESIGN

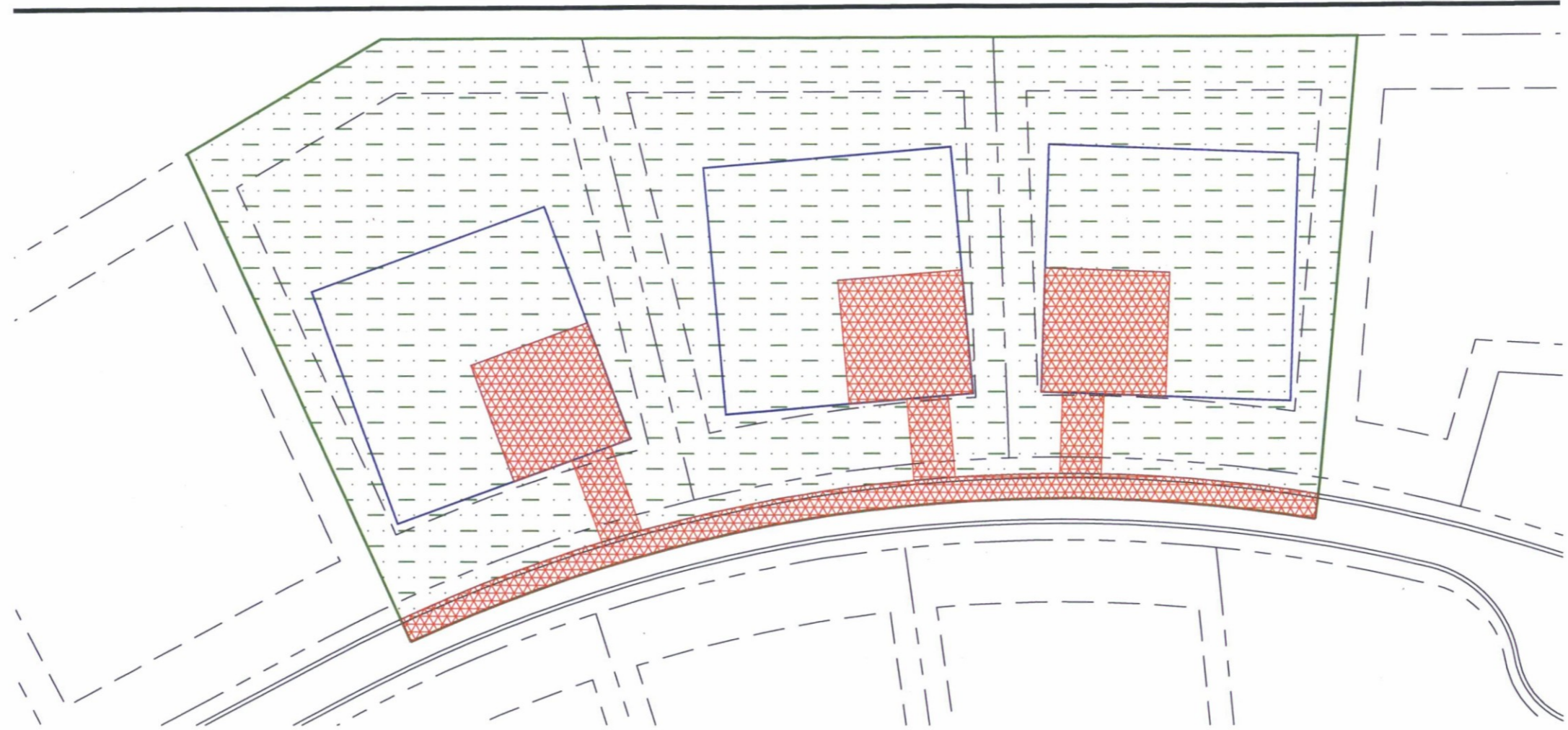


SCALE: 1" = 60'

TOTAL AREA = 123,750 SF

HYDRAULICALLY
CONNECTED AREA = 19,000 SF

RTIMP = 19,000/123,750 = 15.4% CALCULATED
17.0% DESIGN

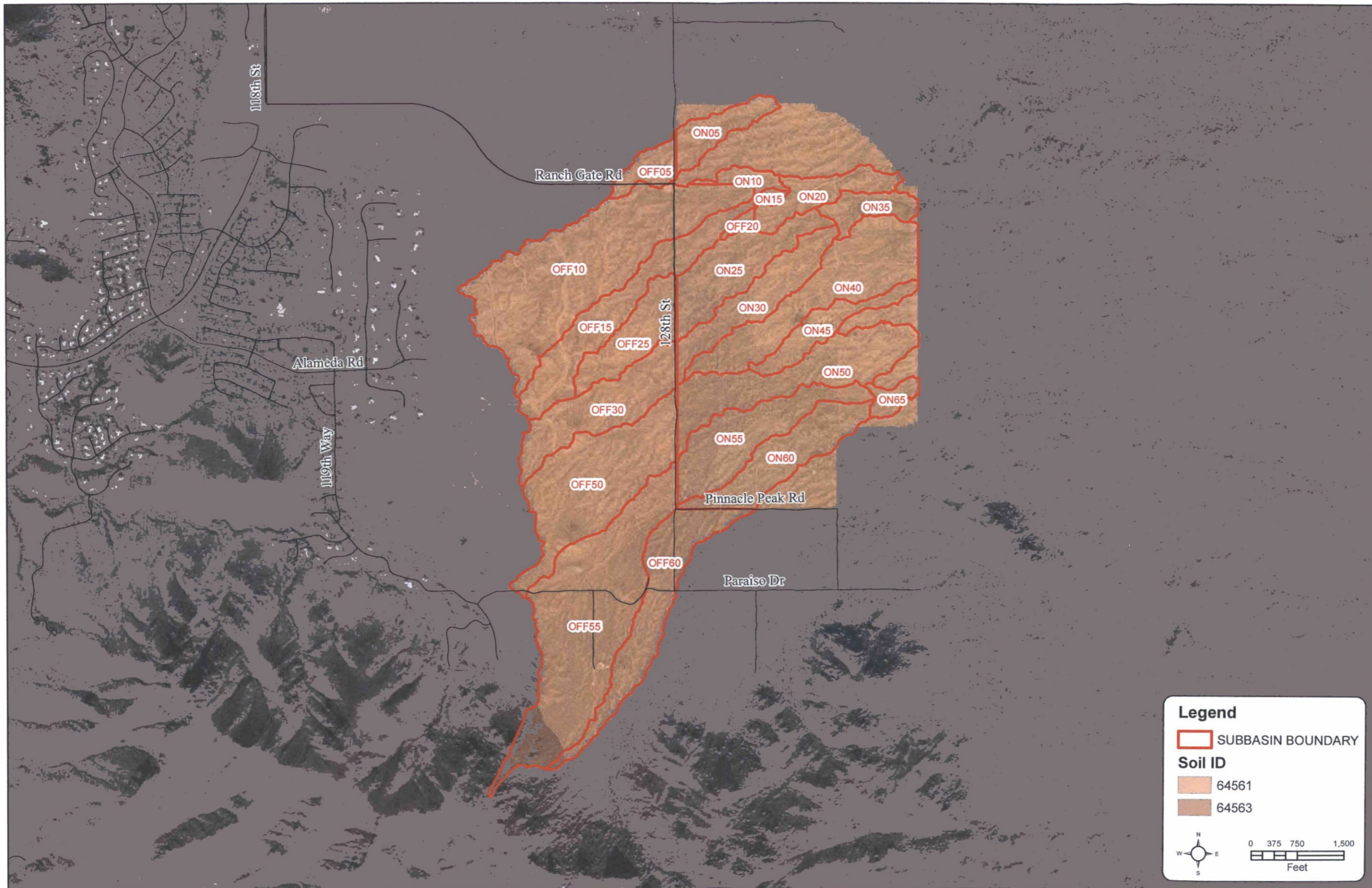


STORYROCK - R1-43 RTIMP
DETERMINATION



KIMLEY HORN & ASSOCIATES, INC. ENGINEERS, ARCHITECTS, PLANNERS, ENVIRONMENTAL SCIENTISTS, AND LANDSCAPE ARCHITECTS
1000 EAST 17TH AVENUE, SUITE 1000, DENVER, COLORADO 80202
TEL: 303.733.1000 FAX: 303.733.1001 WWW.KIMLEYHORN.COM

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
OFF100	645	61	64561	0.011	100.00	0.150	-	100	
OFF105	645	61	64561	0.085	100.00	0.150	-	100	
OFF110	645	61	64561	0.037	100.00	0.150	-	100	
OFF115	645	61	64561	0.007	100.00	0.150	-	100	
OFF95	645	61	64561	0.064	100.00	0.150	-	100	
ON135	645	61	64561	0.008	100.00	0.150	-	100	
ON140	645	61	64561	0.017	100.00	0.150	-	100	
ON145	645	61	64561	0.014	100.00	0.150	-	100	
ON150	645	61	64561	0.007	100.00	0.150	-	100	
ON155	645	61	64561	0.017	100.00	0.150	-	100	
ON160	645	61	64561	0.020	100.00	0.150	-	100	
ON161	645	61	64561	0.010	100.00	0.150	-	100	
ON165	645	61	64561	0.008	100.00	0.150	-	100	
ON170	645	61	64561	0.005	100.00	0.150	-	100	
ON175	645	61	64561	0.002	100.00	0.150	-	100	
ON179	645	61	64561	0.002	100.00	0.150	-	100	
ON180	645	61	64561	0.005	100.00	0.150	-	100	
ON181	645	61	64561	0.004	100.00	0.150	-	100	
ON182	645	61	64561	0.009	100.00	0.150	-	100	
ON183	645	61	64561	0.007	100.00	0.150	-	100	
ON184	645	61	64561	0.009	100.00	0.150	-	100	
ON185	645	61	64561	0.002	100.00	0.150	-	100	
ON190	645	61	64561	0.006	100.00	0.150	-	100	
ON191	645	61	64561	0.006	100.00	0.150	-	100	
ON195	645	61	64561	0.009	100.00	0.150	-	100	
ON200	645	61	64561	0.006	100.00	0.150	-	100	
ON205	645	61	64561	0.020	100.00	0.150	-	100	
ON210	645	61	64561	0.017	100.00	0.150	-	100	
ON215	645	61	64561	0.030	100.00	0.150	-	100	
ON220	645	61	64561	0.003	100.00	0.150	-	100	
ON235	645	61	64561	0.002	100.00	0.150	-	100	
ON240	645	61	64561	0.007	100.00	0.150	-	100	
ON245	645	61	64561	0.003	100.00	0.150	-	100	
ON260	645	61	64561	0.003	100.00	0.150	-	100	
ON265	645	61	64561	0.002	100.00	0.150	-	100	
ON270	645	61	64561	0.002	100.00	0.150	-	100	
ON280	645	61	64561	0.001	100.00	0.150	-	100	



Legend

SUBBASIN BOUNDARY

Soil ID

64561

64563

N
W — E
S

0 375 750 1,500
Feet

<p>Kimley»Horn KIMLEY-HORN AND ASSOCIATES, INC. 7740 North 16th Street, Suite 300 Phoenix, Arizona 85020 (602) 944-5500</p>		NO.	BY	DATE	APPR.
<p>SCALE: 1" = 100'</p> <p>DESIGNED BY: MAW DRAWN BY: DWT CHECKED BY: MAW</p> <p>DATE: AUGUST 2014</p>		<p>PROJECT NO. 191069013</p> <p>DRAWING NAME Existing_Soils.mxd</p>			
<p>CAVALLIERE EXISTING SOILS MAP</p>		<p>REVISION</p>			

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 STORAGE FACILITIES

Storage Basin ID: DB160		<u>1</u>	<u>2</u>	<u>3</u>	<u>4</u>	<u>5</u>	<u>6</u>	<u>7</u>	<u>8</u>	<u>9</u>	<u>10</u>
Spillway Characteristics (SS)											
Spillway Crest Elevation:	-NA- Volume (ac-ft)	-	-	0.1	0.1	0.2	0.3	0.5	0.5	-	-
Spillway Length:	-NA- Discharge (cfs)	0	3	7	9	14	17	20	65	0	0
Discharge Coefficient:	-NA- Elevation (ft)	-	1.0	2.0	3.0	4.0	5.0	6.0	6.2	-	-
Weir Coefficient:	-NA-										
Low-Level Outlet (SL)		<u>11</u>	<u>12</u>	<u>13</u>	<u>14</u>	<u>15</u>	<u>16</u>	<u>17</u>	<u>18</u>	<u>19</u>	<u>20</u>
Centerline Elevation:	-NA- Volume (ac-ft)	-	-	-	-	-	-	-	-	-	-
Cross-Section Area:	-NA- Discharge (cfs)	0	0	0	0	0	0	0	0	0	0
Discharge Coefficient:	-NA- Elevation (ft)	-	-	-	-	-	-	-	-	-	-
Orifice Equation Exponent:	-NA-										
Top of Dam Overflow (ST)		<u>2 Yr</u>	<u>5 Yr</u>	<u>10 Yr</u>	<u>25 Yr</u>	<u>50 Yr</u>	<u>100 Yr</u>				
Elevation Top of Dam:	-NA- Peak Volume (ac-ft)	0.11	0.00	0.33	0.00	0.00	0.45				
Length of Dam:	-NA- Peak Stage (ft)	3.00	0.00	5.33	0.00	0.00	6.19				
Discharge Coefficient:	-NA- Peak Discharge (cfs)	9.00	0.00	18.00	0.00	0.00	62.00				
Weir Coefficient:	-NA-										

Storage Basin ID: DB180		<u>1</u>	<u>2</u>	<u>3</u>	<u>4</u>	<u>5</u>	<u>6</u>	<u>7</u>	<u>8</u>	<u>9</u>	<u>10</u>
Spillway Characteristics (SS)											
Spillway Crest Elevation:	-NA- Volume (ac-ft)	-	0.1	0.1	0.2	0.3	0.3	0.4	0.4	-	-
Spillway Length:	-NA- Discharge (cfs)	0	1	3	4	5	5	6	6	0	0
Discharge Coefficient:	-NA- Elevation (ft)	-	0.5	1.0	1.5	2.0	2.5	3.0	3.0	-	-
Weir Coefficient:	-NA-										
Low-Level Outlet (SL)		<u>11</u>	<u>12</u>	<u>13</u>	<u>14</u>	<u>15</u>	<u>16</u>	<u>17</u>	<u>18</u>	<u>19</u>	<u>20</u>
Centerline Elevation:	-NA- Volume (ac-ft)	-	-	-	-	-	-	-	-	-	-
Cross-Section Area:	-NA- Discharge (cfs)	0	0	0	0	0	0	0	0	0	0
Discharge Coefficient:	-NA- Elevation (ft)	-	-	-	-	-	-	-	-	-	-
Orifice Equation Exponent:	-NA-										
Top of Dam Overflow (ST)		<u>2 Yr</u>	<u>5 Yr</u>	<u>10 Yr</u>	<u>25 Yr</u>	<u>50 Yr</u>	<u>100 Yr</u>				
Elevation Top of Dam:	-NA- Peak Volume (ac-ft)	0.06	0.00	0.14	0.00	0.00	0.26				
Length of Dam:	-NA- Peak Stage (ft)	0.50	0.00	1.00	0.00	0.00	2.00				
Discharge Coefficient:	-NA- Peak Discharge (cfs)	1.00	0.00	3.00	0.00	0.00	5.00				
Weir Coefficient:	-NA-										

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 STORAGE FACILITIES

Storage Basin ID: DB181			<u>1</u>	<u>2</u>	<u>3</u>	<u>4</u>	<u>5</u>	<u>6</u>	<u>7</u>	<u>8</u>	<u>9</u>	<u>10</u>
Spillway Characteristics (SS)												
Spillway Crest Elevation:	-NA-	Volume (ac-ft)		0.1	0.1	0.2	0.3	0.4	0.5			
Spillway Length:	-NA-	Discharge (cfs)	0	1	1	2	2	2	2	0	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	0.5	1.0	1.5	2.0	2.5	3.0	-	-	-
Weir Coefficient:	-NA-											
Low-Level Outlet (SL)			<u>11</u>	<u>12</u>	<u>13</u>	<u>14</u>	<u>15</u>	<u>16</u>	<u>17</u>	<u>18</u>	<u>19</u>	<u>20</u>
Centerline Elevation:	-NA-	Volume (ac-ft)	-	-	-	-	-	-	-	-	-	-
Cross-Section Area:	-NA-	Discharge (cfs)	0	0	0	0	0	0	0	0	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	-	-	-	-	-	-	-	-	-
Orifice Equation Exponent:	-NA-											
Top of Dam Overflow (ST)			<u>2 Yr</u>	<u>5 Yr</u>	<u>10 Yr</u>	<u>25 Yr</u>	<u>50 Yr</u>	<u>100 Yr</u>				
Elevation Top of Dam:	-NA-	Peak Volume (ac-ft)	0.06	0.00	0.06	0.00	0.00	0.20				
Length of Dam:	-NA-	Peak Stage (ft)	0.50	0.00	0.50	0.00	0.00	1.50				
Discharge Coefficient:	-NA-	Peak Discharge (cfs)	1.00	0.00	1.00	0.00	0.00	2.00				
Weir Coefficient:	-NA-											

Storage Basin ID: DB182			<u>1</u>	<u>2</u>	<u>3</u>	<u>4</u>	<u>5</u>	<u>6</u>	<u>7</u>	<u>8</u>	<u>9</u>	<u>10</u>
Spillway Characteristics (SS)												
Spillway Crest Elevation:	-NA-	Volume (ac-ft)		-	-	-	0.1	0.1	0.2	0.3	0.3	
Spillway Length:	-NA-	Discharge (cfs)	0	2	4	7	9	11	13	14	44	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	0.5	1.0	1.5	2.0	2.5	3.0	3.5	4.0	-
Weir Coefficient:	-NA-											
Low-Level Outlet (SL)			<u>11</u>	<u>12</u>	<u>13</u>	<u>14</u>	<u>15</u>	<u>16</u>	<u>17</u>	<u>18</u>	<u>19</u>	<u>20</u>
Centerline Elevation:	-NA-	Volume (ac-ft)	-	-	-	-	-	-	-	-	-	-
Cross-Section Area:	-NA-	Discharge (cfs)	0	0	0	0	0	0	0	0	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	-	-	-	-	-	-	-	-	-
Orifice Equation Exponent:	-NA-											
Top of Dam Overflow (ST)			<u>2 Yr</u>	<u>5 Yr</u>	<u>10 Yr</u>	<u>25 Yr</u>	<u>50 Yr</u>	<u>100 Yr</u>				
Elevation Top of Dam:	-NA-	Peak Volume (ac-ft)	0.04	0.00	0.18	0.00	0.00	0.34				
Length of Dam:	-NA-	Peak Stage (ft)	1.50	0.00	3.00	0.00	0.00	4.00				
Discharge Coefficient:	-NA-	Peak Discharge (cfs)	7.00	0.00	13.00	0.00	0.00	44.00				
Weir Coefficient:	-NA-											

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 STORAGE FACILITIES

Storage Basin ID:		DB184										
Spillway Characteristics (SS)			1	2	3	4	5	6	7	8	9	10
Spillway Crest Elevation:	-NA-	Volume (ac-ft)		0.1	0.2	0.4	0.5	0.6	0.7	0.7		
Spillway Length:	-NA-	Discharge (cfs)	0	0	1	1	1	2	2	10	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	0.5	1.0	1.5	2.0	2.5	3.0	3.0	-	-
Weir Coefficient:	-NA-											
Low-Level Outlet (SL)			11	12	13	14	15	16	17	18	19	20
Centerline Elevation:	-NA-	Volume (ac-ft)	-	-	-	-	-	-	-	-	-	-
Cross-Section Area:	-NA-	Discharge (cfs)	0	0	0	0	0	0	0	0	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	-	-	-	-	-	-	-	-	-
Orifice Equation Exponent:	-NA-											
Top of Dam Overflow (ST)			2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr				
Elevation Top of Dam:	-NA-	Peak Volume (ac-ft)	0.24	0.00	0.24	0.00	0.00	0.72				
Length of Dam:	-NA-	Peak Stage (ft)	1.00	0.00	1.00	0.00	0.00	3.00				
Discharge Coefficient:	-NA-	Peak Discharge (cfs)	1.00	0.00	1.00	0.00	0.00	10.00				
Weir Coefficient:	-NA-											

Project **Storyrock Phase 1C**

Subject **Detention Basin Calculations**

Designed by **ZJH**

Date **2/5/2016**

Project No. **191069020**

Checked by **JMB**

Date **2/5/2016**

Objective: to determine the storage-flow relationship for small detention basins

DB160

Drains in 0.24 hours

Outlet Diameter 1.50 ft Outlet X-Sect Area 1.767 ft²
 Outlet Elevation 0 ft No. of Outlet Barrels 1
 Outlet Pipe Slope 0.005 ft/ft

Elevation [ft]	Surface Storage Area [ft ²]	Surface Storage Area [acre]	Average Area [acre]	Δ Elev [ft]	Δ Vol [ac-ft]	Σ Vol [ac-ft]	Δ Time to Drain [hr]	Q _{pipe} [cfs]	Q _{weir} [cfs]	Total Q _{out} [cfs]
0	233	0.01	0.01	1.0	0.01	0	0.07	0	0	0
1	534	0.01	0.04	1.0	0.04	0.01	0.09	3	0	3
2	2,609	0.06	0.05	1.0	0.05	0.04	0.08	7	0	7
3	1,955	0.04	0.06	1.0	0.06	0.10	0.07	9	0	9
4	3,465	0.08	0.11	1.0	0.11	0.16	0.09	14	0	14
5	6,173	0.14	0.18	1.0	0.18	0.27	0.05	17	0	17
6	9,419	0.22				0.45		20	42	62

Notes:

Q_{pipe} goes from Mannings Eqn to Orifice Eqn when water surface exceeds 1.2*(Outlet Diameter)
 per Linsley et al. *Water Resources Engineering* 4th Edition, pg 652.

Project **Storyrock Phase 1C**
 Subject **Detention Basin Calculations**
 Designed by **ZJH** Date **2/5/2016** Project No. **191069020**
 Checked by **JMB** Date **2/5/2016**

Objective: to determine the storage-flow relationship for small detention basins

DB180 **Drains in 1.83 hours**

Outlet Diameter 1.00 ft Outlet X-Sect Area 0.785 ft²
 Outlet Elevation 0. ft No. of Outlet Barrels 1
 Outlet Pipe Slope 0.005 ft/ft

Elevation [ft]	Surface Storage Area [ft ²]	Surface Storage Area [acre]	Average Area [acre]	Δ Elev [ft]	Δ Vol [ac-ft]	Σ Vol [ac-ft]	Δ Time to Drain [hr]	Q _{pipe} [cfs]	Q _{weir} [cfs]	Total Q _{out} [cfs]
0	4,055	0.09	0.11	1.0	0.11	0	1.02	0	0	0
1	5,153	0.12	0.13	1.0	0.13	0.11	0.45	3	0	3
2	6,372	0.15	0.16	1.0	0.16	0.24	0.37	5	0	5
3	7,711	0.18				0.40		6	0	6

Notes:

Q_{pipe} goes from Mannings Eqn to Orifice Eqn when water surface exceeds 1.2*(Outlet Diameter)
 per Linsley et al. *Water Resources Engineering* 4th Edition, pg 652.

Project **Storyrock Phase 1C**

Subject **Detention Basin Calculations**

Designed by **ZJH**

Date **2/5/2016**

Project No. **191069020**

Checked by **JMB**

Date **2/5/2016**

Objective: to determine the storage-flow relationship for small detention basins

DB181

Drains in 7.06 hours

Outlet Diameter 0.50 ft Outlet X-Sect Area 0.196 ft²
 Outlet Elevation 0 ft No. of Outlet Barrels 1
 Outlet Pipe Slope 0.005 ft/ft

Elevation [ft]	Surface Storage Area [ft ²]	Surface Storage Area [acre]	Average Area [acre]	Δ Elev [ft]	Δ Vol [ac-ft]	Σ Vol [ac-ft]	Δ Time to Drain [hr]	Q _{pipe} [cfs]	Q _{weir} [cfs]	Total Q _{out} [cfs]
0	4,845	0.11	0.12	1.0	0.12	0	3.69	0	0	0
1	6,032	0.14	0.15	1.0	0.15	0.12	1.79	1	0	1
2	7,322	0.17	0.18	1.0	0.18	0.28	1.58	1	0	1
3	8,718	0.20				0.46		2	0	2

Notes:

Q_{pipe} goes from Mannings Eqn to Orifice Eqn when water surface exceeds 1.2*(Outlet Diameter)

per Linsley et al. *Water Resources Engineering* 4th Edition, pg 652.

Project **Storyrock Phase 1C**

Subject **Detention Basin Calculations**

Designed by **ZJH**

Date **2/5/2016**

Project No. **191069020**

Checked by **JMB**

Date **2/5/2016**

Objective: to determine the storage-flow relationship for small detention basins

DB182

Drains in 0.34 hours

Outlet Diameter 1.50 ft Outlet X-Sect Area 1.767 ft²
 Outlet Elevation 0 ft No. of Outlet Barrels 1
 Outlet Pipe Slope 0.005 ft/ft

Elevation	Surface Storage Area	Surface Storage Area	Average Area	Δ Elev	Δ Vol	Σ Vol	Δ Time to Drain	Q _{pipe}	Q _{weir}	Total Q _{out}
[ft]	[ft ²]	[acre]	[acre]	[ft]	[ac-ft]	[ac-ft]	[hr]	[cfs]	[cfs]	[cfs]
0	329	0.01				0		0	0	0
1	1,502	0.03	0.02	1.0	0.02	0.02	0.13	4	0	4
2	3,270	0.08	0.05	1.0	0.05	0.08	0.10	9	0	9
3	5,600	0.13	0.10	1.0	0.10	0.18	0.11	13	0	13
4	8,135	0.19	0.16	1.0	0.16	0.34	0.07	16	28	44

Notes:

Q_{pipe} goes from Mannings Eqn to Orifice Eqn when water surface exceeds 1.2*(Outlet Diameter)
 per Linsley et al. *Water Resources Engineering* 4th Edition, pg 652.

Project **Storyrock Phase 1C**
 Subject **Detention Basin Calculations**
 Designed by **ZJH** Date **2/5/2016** Project No. **191069020**
 Checked by **JMB** Date **2/5/2016**

Objective: to determine the storage-flow relationship for small detention basins

DB184 **Drains in 8.81 hours**

Outlet Diameter 0.50 ft Outlet X-Sect Area 0.196 ft²
 Outlet Elevation 0 ft No. of Outlet Barrels 1
 Outlet Pipe Slope 0.02 ft/ft

Elevation	Surface Storage Area	Surface Storage Area	Average Area	Δ Elev	Δ Vol	Σ Vol	Δ Time to Drain	Q _{pipe}	Q _{weir}	Total Q _{out}
[ft]	[ft ²]	[acre]	[acre]	[ft]	[ac-ft]	[ac-ft]	[hr]	[cfs]	[cfs]	[cfs]
0	7,542	0.17				0		0.0	0	0
1	8,978	0.21	0.19	1.0	0.19	0.19	5.60	1	0	1
2	10,538	0.24	0.22	1.0	0.22	0.41	2.62	1	0	1
3	12,221	0.28	0.26	1.0	0.26	0.67	0.58	2	8	10

Notes:

Q_{pipe} goes from Mannings Eqn to Orifice Eqn when water surface exceeds 1.2*(Outlet Diameter)
 per Linsley et al. *Water Resources Engineering* 4th Edition, pg 652.

Project **Storyrock Phase 1C**
Subject **Basin Summary Table**

Designed by **ZJH**
Checked by **JMB**

Date **2/5/2016**
Date **2/5/2016**

Project No. **191069020**

Objective: Basin Summary Table

Basin ID	Volume (ac-ft)	Max Depth (ft)	Max Side Slope (ft, H:V)	100 Yr Peak Inflow Rate (cfs)	100 Yr Peak Outflow Rate (cfs)	100 Yr Peak Flow Attenuation (cfs)	Orifice Size (ft)	Drain Time (hr)	Type	2 Year Storage Volume (ac-ft)	10 Year Storage Volume (ac-ft)	100 Year Storage Volume (ac-ft)
DB160	0.45	6.00	*	62	62	0	1.50	0.24	In-Line	0.11	0.33	0.45
DB180	0.40	3.00	4:1	12	6	6	1.00	1.83	Standard	0.06	0.14	0.26
DB181	0.46	3.00	4:1	10	2	8	0.50	7.06	Standard	0.06	0.06	0.20
DB182	0.34	4.00	*	44	44	0	1.50	0.34	In-Line	0.04	0.18	0.34
DB184	0.67	3.00	4:1	23	10	13	0.50	8.81	Standard	0.24	0.24	0.72

Project **Storyrock Phase 1C**

Subject **First Flush Summary**

Designed by **ZJH**

Date **2/5/2016**

Project No. **191069020**

Checked by **JMB**

Date **2/5/2016**

Objective: First Flush Summary

First Flush Volume =

$A * C * P / 12$

Contributing Sub Basin	First Flush Method	Contributing Developed Area (sf)	Contributing Developed Area (ac)	First Flush Volume Req. (ac-ft)	Basin Volume Prov. (ac-ft)
ON155	Stormceptor /Alternate First Flush Method	N/A	N/A	N/A	N/A
ON160	Stormceptor /Alternate First Flush Method	58,703	1.35	0.05	0.14
ON161	N/A*	N/A	N/A	N/A	N/A
ON165	Stormceptor /Alternate First Flush Method	40,197	0.92	0.04	-
ON175	N/A*	N/A	N/A	N/A	N/A
ON180	Basin	131,097	3.01	0.12	0.40
ON181	Basin	118,054	2.71	0.11	0.46
ON182	Stormceptor /Alternate First Flush Method	N/A	N/A	N/A	N/A
ON183	N/A*	N/A	N/A	N/A	N/A
ON184	Basin	242,791	5.57	0.22	0.67
ON190	Stormceptor /Alternate First Flush Method	N/A	N/A	N/A	N/A
ON191	Stormceptor /Alternate First Flush Method	N/A	N/A	N/A	N/A

Note* Direct discharge of lot drainage only.

Flood Control District of Maricopa County
Drainage Design Management System
HEC-1 ROUTING DATA
Project Reference: STORYROCK PH1C PROP

Route ID	Shape	Length (ft)	Slope (ft/ft)	Manning's N	Diameter (in)	Height (ft)	Width (ft)	Side Slope (h/v)
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KINEMATIC WAVE

Major Basin 01

RO155A	PIPE	837.00	0.2900	0.013	24	-	-	-
--------	------	--------	--------	-------	----	---	---	---

187	UC	0.181	0.235								
188	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
189	UA	100									

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

190	KK	DB180	STORAGE								
191	KO										
192	RS	1	STOR								
193	SV		0.06	0.14	0.20	0.26	0.34	0.39	0.40		
194	SQ		1.00	3.00	4.00	5.00	5.00	6.00	6.00		
195	SE		0.50	1.00	1.50	2.00	2.50	2.95	3.00		

196	KK	RO182A	ROUTE								
197	RS	1	FLOW								
198	RC	0.050	0.035	0.050	555	0.0340	0.00				
199	RX	0.00	16.00	24.00	28.00	29.00	36.00	58.00	95.00		
200	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		

201	KK	ON179	BASIN								
202	BA	0.002									
203	LG	0.35	0.40	6.00	0.18	0					
204	UC	0.181	0.228								
205	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
206	UA	100									

207	KK	RO182C	ROUTE								
208	RS	1	FLOW								
209	RC	0.050	0.035	0.050	952	0.0340	0.00				
210	RX	0.00	16.00	24.00	28.00	29.00	36.00	58.00	95.00		
211	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		

212	KK	ON195	BASIN								
213	BA	0.009									
214	LG	0.35	0.40	6.00	0.18	0					
215	UC	0.327	0.484								
216	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
217	UA	100									

218	KK	RO182B	ROUTE								
219	RS	1	FLOW								
220	RC	0.050	0.035	0.050	1027	0.0340	0.00				
221	RX	0.00	16.00	24.00	28.00	29.00	36.00	58.00	95.00		
222	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		

223	KK	ON182	BASIN								
224	BA	0.009									
225	LG	0.32	0.32	6.00	0.20	15					
226	UC	0.230	0.281								
227	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
228	UA	100									

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

229	KK	CO182	COMBINE								
230	HC	4									

231	KK	DB182	STORAGE								
232	KO										
233	RS	1	STOR								
234	SV		0.01	0.02	0.04	0.05	0.13	0.18	0.26	0.34	
235	SQ		2.00	4.00	7.00	9.00	11.00	13.00	14.00	44.00	
236	SE		0.50	1.00	1.50	2.00	2.50	3.00	3.50	4.00	

293	KK	RO215A	ROUTE								
294	RS	1	FLOW								
295	RC	0.050	0.035	0.050	975	0.0360	0.00				
296	RX	0.00	9.00	21.00	25.00	32.00	36.00	70.00	90.00		
297	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										
298	KK	OFF110	BASIN								
299	BA	0.037									
300	LG	0.35	0.40	6.00	0.18	0					
301	UC	0.428	0.595								
302	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
303	UA	100									
	*										

HEC-1 INPUT

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

304	KK	OFF115	BASIN								
305	BA	0.007									
306	LG	0.35	0.40	6.00	0.18	0					
307	UC	0.227	0.236								
308	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
309	UA	100									
	*										

310	KK	RO220	ROUTE								
311	RS	1	FLOW								
312	RC	0.050	0.035	0.050	310	0.0420	0.00				
313	RX	0.00	8.00	15.00	21.00	21.10	25.00	30.00	36.00		
314	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										

315	KK	ON220	BASIN								
316	BA	0.003									
317	LG	0.35	0.40	6.00	0.18	0					
318	UC	0.222	0.326								
319	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
320	UA	100									
	*										

321	KK	CO220	COMBINE								
322	HC	2									
	*										

323	KK	RO215D	ROUTE								
324	RS	1	FLOW								
325	RC	0.050	0.035	0.050	364	0.0300	0.00				
326	RX	0.00	5.00	8.00	11.00	11.10	14.00	17.00	24.00		
327	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										

328	KK	CO215A	COMBINE								
329	HC	2									
	*										

330	KK	RO215B	ROUTE								
331	RS	1	FLOW								
332	RC	0.050	0.035	0.050	800	0.0340	0.00				
333	RX	0.00	6.00	10.00	13.00	19.00	21.00	26.00	62.00		
334	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										

335	KK	CO215B	COMBINE								
336	HC	2									
	*										

HEC-1 INPUT

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

337	KK	RO215C	ROUTE								
338	RS	1	FLOW								
339	RC	0.050	0.035	0.050	445	0.0340	0.00				

393	BA	0.003									
394	LG	0.35	0.40	6.00	0.18	0					
395	UC	0.212	0.265								
396	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
397	UA	100									
	*										
398	KK	C0260 COMBINE									
399	HC	3									
	*										
400	KK	RO200A	ROUTE								
401	RS	1	FLOW								
402	RC	0.050	0.035	0.050	611	0.0360	0.00				
403	RX	0.00	5.00	12.00	45.00	61.00	68.00	73.00	78.00		
404	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
405	KK	ON265	BASIN								
406	BA	0.002									
407	LG	0.35	0.40	6.00	0.18	0					
408	UC	0.203	0.318								
409	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
410	UA	100									
	*										

1

HEC-1 INPUT

PAGE 12

LINE	ID	1	2	3	4	5	6	7	8	9	10
411	KK	RO200B	ROUTE								
412	RS	1	FLOW								
413	RC	0.050	0.035	0.050	345	0.0290	0.00				
414	RX	0.00	6.00	12.00	20.00	21.00	41.00	47.00	51.00		
415	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
416	KK	ON200	BASIN								
417	BA	0.006									
418	LG	0.35	0.40	6.00	0.18	0					
419	UC	0.249	0.320								
420	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
421	UA	100									
	*										
422	KK	ON270	BASIN								
423	BA	0.002									
424	LG	0.35	0.40	6.00	0.18	0					
425	UC	0.193	0.273								
426	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
427	UA	100									
	*										
428	KK	C0200 COMBINE									
429	HC	4									
	*										
430	KK	RO205	ROUTE								
431	RS	1	FLOW								
432	RC	0.050	0.035	0.050	1565	0.0370	0.00				
433	RX	0.00	6.00	10.00	15.00	48.00	58.00	64.00	77.00		
434	RY	4.00	2.00	1.00	0.00	0.00	1.00	2.00	4.00		
	*										
435	KK	ON205	BASIN								
436	BA	0.020									
437	LG	0.35	0.39	6.00	0.18	2					
438	UC	0.359	0.454								
439	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
440	UA	100									
	*										
441	KK	ON280	BASIN								
442	BA	0.001									
443	LG	0.35	0.40	6.00	0.18	0					
444	UC	0.136	0.158								
445	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0

1

HEC-1 INPUT

PAGE 13

LINE	ID	1	2	3	4	5	6	7	8	9	10
447	KK	RO210	ROUTE								
448	RS	1	FLOW								
449	RC	0.050	0.035	0.050	1804	0.0382	0.00				
450	RX	0.00	2.00	4.00	5.00	10.00	12.00	14.00	18.00		
451	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
452	KK	ON210	BASIN								
453	BA	0.017									
454	LG	0.35	0.40	6.00	0.18	0					
455	UC	0.410	0.620								
456	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
457	UA	100									
	*										
458	KK	CO205	COMBINE								
459	HC	4									
	*										
460	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
19	OFF95	
	V	
	V	
25	RO140A	
	.	
30	OFF100	
	V	
	V	
36	RO140B	
	.	
41	CO140A.....	
	V	
	V	
43	RO140C	
	.	
48	ON140	
	.	
54	CO140B.....	
	V	
	V	
56	RO155B	
	.	
61	ON135	
	V	
	V	
67	RO155A	
	.	
69	CO155A.....	
	V	
	V	
71	RO155C	
	.	
76	ON155	
	.	
82	CO155B.....	
	.	
84	ON145	

```

90 .      RO160A
.
.
95 .      ON150
.      V
.      V
101 .     RO160B
.
.
106 .     CO160A .....
.      V
.      V
108 .     RO160C
.
.
113 .     ON160
.
.
119 .     CO160B .....
.      V
.      V
121 .     DB160
.      V
.      V
127 .     RO161
.
.
132 .     ON161
.
.
138 .     CO161 .....
.      V
.      V
140 .     RO170
.
.
145 .     ON170
.
.
151 .     ON184
.      V
.      V
157 .     DB184
.      V
.      V
163 .     RO170
.
.
168 .     CO170 .....
.
.
170 .     ON165
.
.
176 .     ON175
.
.
182 .     COEX1 .....
.
.
184 .     ON180
.      V
.      V
190 .     DB180
.      V
.      V
196 .     RO182A
.
.
201 .     ON179
.      V
.      V
207 .     RO182C
.
.
212 .     ON195
.      V

```


223
229
231
237
243
249
251
256
262
268
273
275
281
287
293
298
304
310
315
321
323
328
330
335
337

ON182

CO182.....

V
V

DB182

ON181

V
V

DB181

CO183A.....

V
V

RO183A

ON183

ON190

V
V

RO183B

CO183B.....

ON185

ON191

OFF105

V
V

RO215A

OFF110

OFF115

V
V

RO220

ON220

CO220.....

V
V

RO215D

CO215A.....

V
V

RO215B

CO215B.....

V
V

RO215C

348	CO215C.....		
	V		
	V		
350	RO240A		
		ON235	
355			
361	CO240A.....		
	V		
	V		
363	RO240B		
		ON240	
368			
374	CO240B.....		
	V		
	V		
376	RO260A		
		ON245	
381		V	
		V	
387		RO260B	
392			ON260
398	CO260.....		
	V		
	V		
400	RO200A		
		ON265	
405		V	
		V	
411		RO200B	
416			ON200
422			ON270
428	CO200.....		
	V		
	V		
430	RO205		
		ON205	
435			
441			ON280
			V
447			V
			RO210
452			ON210
458	CO205.....		

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *

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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *

```

* RUN DATE 19FEB17 TIME 11:22:22 *
*

* (916) 756-1104 *
*

Flood Control District of Maricopa County
STORYROCK PHIC PROP - STORYROCK PHASE 1C PROP CONDITION
2 YEAR
6 Hour Storm
Unit Hydrograph: Clark
Storm: Multiple
02/19/2017

9 IO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
NMIN 5 MINUTES IN COMPUTATION INTERVAL
IDATE 1JAN99 STARTING DATE
ITIME 0000 STARTING TIME
NQ 2000 NUMBER OF HYDROGRAPH ORDINATES
NDDATE 7JAN99 ENDING DATE
NDTIME 2235 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.08 HOURS
TOTAL TIME BASE 166.58 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

11 JD INDEX STORM NO. 1
STRM 1.42 PRECIPITATION DEPTH
TRDA 0.00 TRANSPOSITION DRAINAGE AREA

12 PI PRECIPITATION PATTERN
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.03
0.03 0.03 0.05 0.05 0.05 0.15 0.15 0.15 0.03 0.03
0.03 0.01 0.01 0.01 0.01 0.01 0.01 0.00 0.00 0.00
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
0.00 0.00

15 JD INDEX STORM NO. 2
STRM 1.41 PRECIPITATION DEPTH
TRDA 0.50 TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.03
0.03 0.03 0.05 0.05 0.05 0.15 0.15 0.15 0.03 0.03
0.03 0.01 0.01 0.01 0.01 0.01 0.01 0.00 0.00 0.00
0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
0.00 0.00

* *

```

*****
122 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

*** **

```

*****
*          *
157 KK      *  DB184 *  STORAGE
*          *
*****
    
```

```

158 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

*** **

```

*****
*          *
190 KK      *  DB180 *  STORAGE
*          *
*****
    
```

```

191 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

*** **

```

*****
*          *
231 KK      *  DB182 *  STORAGE
*          *
*****
    
```

```

232 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

*** **

```

*****
*          *
243 KK      *  DB181 *  STORAGE
*          *
*****
    
```

```

244 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
-----------	---------	-----------	--------------	---------------------------------	------------	---------------	-------------------

+	HYDROGRAPH AT	OFF95	12.	4.42	2.	1.	0.	0.06
+	ROUTED TO	RO140A	12.	4.42	2.	1.	0.	0.06
+	HYDROGRAPH AT	OFF100	3.	4.25	0.	0.	0.	0.01
+	ROUTED TO	RO140B	3.	4.25	0.	0.	0.	0.01
+	2 COMBINED AT	CO140A	15.	4.42	2.	1.	0.	0.08
+	ROUTED TO	RO140C	14.	4.50	2.	1.	0.	0.08
+	HYDROGRAPH AT	ON140	4.	4.33	1.	0.	0.	0.02
+	2 COMBINED AT	CO140B	18.	4.50	3.	1.	0.	0.09
+	ROUTED TO	RO155B	17.	4.58	3.	1.	0.	0.09
+	HYDROGRAPH AT	ON135	2.	4.25	0.	0.	0.	0.01
+	ROUTED TO	RO155A	2.	4.25	0.	0.	0.	0.01
+	2 COMBINED AT	CO155A	18.	4.58	3.	1.	0.	0.10
+	ROUTED TO	RO155C	18.	4.58	3.	1.	0.	0.10
+	HYDROGRAPH AT	ON155	7.	4.17	1.	0.	0.	0.02
+	2 COMBINED AT	CO155B	21.	4.58	4.	1.	0.	0.12
+	HYDROGRAPH AT	ON145	5.	4.25	1.	0.	0.	0.01
+	ROUTED TO	RO160A	4.	4.33	1.	0.	0.	0.01
+	HYDROGRAPH AT	ON150	2.	4.17	0.	0.	0.	0.01
+	ROUTED TO	RO160B	2.	4.25	0.	0.	0.	0.01
+	2 COMBINED AT	CO160A	7.	4.25	1.	0.	0.	0.02
+	ROUTED TO	RO160C	6.	4.33	1.	0.	0.	0.02
+	HYDROGRAPH AT	ON160	5.	4.33	1.	0.	0.	0.02
+	2 COMBINED AT	CO160B	11.	4.33	2.	0.	0.	0.04
+	ROUTED TO	DB160	9.	4.58	2.	0.	0.	0.04
+	ROUTED TO	RO161	8.	4.67	2.	0.	0.	0.04
+	HYDROGRAPH AT	ON161	3.	4.25	0.	0.	0.	0.01

+		CO161	10.	4.58	2.	1.	0.	0.05
	ROUTED TO							
+		RO170	10.	4.67	2.	1.	0.	0.05
	HYDROGRAPH AT							
+		ON170	2.	4.17	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON184	7.	4.08	1.	0.	0.	0.01
	ROUTED TO							
+		DB184	1.	4.50	0.	0.	0.	0.01
	ROUTED TO							
+		RO170	1.	4.83	0.	0.	0.	0.01
	3 COMBINED AT							
+		CO170	11.	4.58	3.	1.	0.	0.06
	HYDROGRAPH AT							
+		ON165	3.	4.25	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON175	2.	4.00	0.	0.	0.	0.00
	4 COMBINED AT							
+		COEX1	34.	4.58	7.	2.	1.	0.19
	HYDROGRAPH AT							
+		ON180	3.	4.08	0.	0.	0.	0.00
	ROUTED TO							
+		DB180	1.	4.42	0.	0.	0.	0.00
	ROUTED TO							
+		RO182A	1.	4.42	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON179	1.	4.08	0.	0.	0.	0.00
	ROUTED TO							
+		RO182C	1.	4.25	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON195	2.	4.25	0.	0.	0.	0.01
	ROUTED TO							
+		RO182B	2.	4.33	0.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON182	5.	4.17	0.	0.	0.	0.01
	4 COMBINED AT							
+		CO182	8.	4.25	1.	0.	0.	0.02
	ROUTED TO							
+		DB182	7.	4.33	1.	0.	0.	0.02
	HYDROGRAPH AT							
+		ON181	3.	4.08	0.	0.	0.	0.00
	ROUTED TO							
+		DB181	1.	4.42	0.	0.	0.	0.00
	2 COMBINED AT							
+		CO183A	8.	4.33	1.	0.	0.	0.03
	ROUTED TO							
+		RO183A	8.	4.42	1.	0.	0.	0.03
	HYDROGRAPH AT							
+		ON183	2.	4.25	0.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON190	3.	4.17	0.	0.	0.	0.01

+	2 COMBINED AT	CO183B	5.	4.25	1.	0.	0.	0.01
	HYDROGRAPH AT	ON185	1.	4.08	0.	0.	0.	0.00
+	HYDROGRAPH AT	ON191	5.	4.08	0.	0.	0.	0.01
+	HYDROGRAPH AT	OFF105	17.	4.42	3.	1.	0.	0.09
+	ROUTED TO	RO215A	16.	4.50	3.	1.	0.	0.09
+	HYDROGRAPH AT	OFF110	8.	4.33	1.	0.	0.	0.04
+	HYDROGRAPH AT	OFF115	3.	4.17	0.	0.	0.	0.01
+	ROUTED TO	RO220	3.	4.17	0.	0.	0.	0.01
+	HYDROGRAPH AT	ON220	1.	4.17	0.	0.	0.	0.00
+	2 COMBINED AT	CO220	4.	4.17	0.	0.	0.	0.01
+	ROUTED TO	RO215D	4.	4.17	0.	0.	0.	0.01
+	2 COMBINED AT	CO215A	12.	4.25	1.	0.	0.	0.05
+	ROUTED TO	RO215B	11.	4.33	1.	0.	0.	0.05
+	2 COMBINED AT	CO215B	26.	4.42	4.	1.	0.	0.13
+	ROUTED TO	RO215C	26.	4.50	4.	1.	0.	0.13
+	HYDROGRAPH AT	ON215	10.	4.25	1.	0.	0.	0.03
+	2 COMBINED AT	CO215C	32.	4.42	5.	1.	0.	0.16
+	ROUTED TO	RO240A	31.	4.42	5.	1.	0.	0.16
+	HYDROGRAPH AT	ON235	1.	4.08	0.	0.	0.	0.00
+	2 COMBINED AT	CO240A	32.	4.42	5.	1.	0.	0.16
+	ROUTED TO	RO240B	32.	4.50	5.	1.	0.	0.16
+	HYDROGRAPH AT	ON240	2.	4.17	0.	0.	0.	0.01
+	2 COMBINED AT	CO240B	33.	4.50	5.	1.	0.	0.17
+	ROUTED TO	RO260A	33.	4.50	5.	1.	0.	0.17
+	HYDROGRAPH AT	ON245	1.	4.17	0.	0.	0.	0.00
+	ROUTED TO							

+	HYDROGRAPH AT	ON260	1.	4.17	0.	0.	0.	0.00
	3 COMBINED AT	CO260	34.	4.50	6.	1.	0.	0.18
+	ROUTED TO	RO200A	34.	4.50	6.	1.	0.	0.18
+	HYDROGRAPH AT	ON265	1.	4.17	0.	0.	0.	0.00
+	ROUTED TO	RO200B	1.	4.17	0.	0.	0.	0.00
+	HYDROGRAPH AT	ON200	2.	4.17	0.	0.	0.	0.01
+	HYDROGRAPH AT	ON270	1.	4.17	0.	0.	0.	0.00
+	4 COMBINED AT	CO200	35.	4.50	6.	1.	0.	0.19
+	ROUTED TO	RO205	34.	4.58	6.	1.	0.	0.19
+	HYDROGRAPH AT	ON205	6.	4.25	1.	0.	0.	0.02
+	HYDROGRAPH AT	ON280	1.	4.08	0.	0.	0.	0.00
+	ROUTED TO	RO210	0.	4.25	0.	0.	0.	0.00
+	HYDROGRAPH AT	ON210	4.	4.33	1.	0.	0.	0.02
+	4 COMBINED AT	CO205	40.	4.58	7.	2.	1.	0.23
1								

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO COMPUTATION INTERVAL		
							PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)
FOR STORM = 1	STORM AREA (SQ MI) =		0.00						
RO155A	MANE	0.31	2.42	255.02	0.30	5.00	2.42	255.00	0.30

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1274E+00 EXCESS=0.0000E+00 OUTFLOW=0.1274E+00 BASIN STORAGE=0.3292E-18 PERCENT ERROR= 0.0

FOR STORM = 2	STORM AREA (SQ MI) =		0.50						
RO155A	MANE	0.31	2.37	255.19	0.29	5.00	2.37	255.00	0.29

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1248E+00 EXCESS=0.0000E+00 OUTFLOW=0.1248E+00 BASIN STORAGE=0.3293E-18 PERCENT ERROR= 0.0

*** NORMAL END OF HEC-1 ***

34	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
35	UA	100									
	*										
36	KK	RO140B	ROUTE								
37	RS	1	FLOW								
38	RC	0.050	0.035	0.050	626	0.0340	0.00				
39	RX	0.00	7.00	8.00	11.00	11.20	14.00	16.00	31.00		
40	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										

1

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

41	KK	CO140A	COMBINE								
42	HC	2									
	*										
43	KK	RO140C	ROUTE								
44	RS	1	FLOW								
45	RC	0.050	0.035	0.050	1210	0.0320	0.00				
46	RX	0.00	3.00	6.00	11.00	11.20	17.00	23.00	48.00		
47	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
48	KK	ON140	BASIN								
49	BA	0.017									
50	LG	0.34	0.39	6.00	0.18	4					
51	UC	0.328	0.483								
52	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
53	UA	100									
	*										
54	KK	CO140B	COMBINE								
55	HC	2									
	*										
56	KK	RO155B	ROUTE								
57	RS	1	FLOW								
58	RC	0.050	0.035	0.050	910	0.0350	0.00				
59	RX	0.00	16.00	61.00	76.00	90.00	121.00	200.00	226.00		
60	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										
61	KK	ON135	BASIN								
62	BA	0.008									
63	LG	0.35	0.40	6.00	0.18	0					
64	UC	0.250	0.344								
65	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
66	UA	100									
	*										
67	KK	RO155A	ROUTE								
68	RK	837	0.2900	0.013		CIRC	2.000				
	*										
69	KK	CO155A	COMBINE								
70	HC	2									
	*										
71	KK	RO155C	ROUTE								
72	RS	1	FLOW								
73	RC	0.050	0.035	0.050	392	0.0330	0.00				
74	RX	0.00	14.00	22.00	30.00	42.00	54.00	68.00	90.00		
75	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

76	KK	ON155	BASIN								
77	BA	0.017									
78	LG	0.33	0.34	6.00	0.19	11					
79	UC	0.253	0.296								
80	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0

82	KK	CO155B	COMBINE									
83	HC	2										
	*											
84	KK	ON145	BASIN									
85	BA	0.014										
86	LG	0.31	0.38	6.00	0.19	9						
87	UC	0.290	0.428									
88	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
89	UA	100										
	*											
90	KK	RO160A	ROUTE									
91	RS	1	FLOW									
92	RC	0.050	0.035	0.050	581	0.0280	0.00					
93	RX	0.00	6.00	12.00	18.00	22.00	28.00	34.00	40.00			
94	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00			
	*											
95	KK	ON150	BASIN									
96	BA	0.007										
97	LG	0.35	0.40	6.00	0.18	0						
98	UC	0.220	0.270									
99	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
100	UA	100										
	*											
101	KK	RO160B	ROUTE									
102	RS	1	FLOW									
103	RC	0.050	0.035	0.050	800	0.0300	0.00					
104	RX	0.00	6.00	10.00	14.00	16.00	18.00	22.00	28.00			
105	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00			
	*											
106	KK	CO160A	COMBINE									
107	HC	2										
	*											
108	KK	RO160C	ROUTE									
109	RS	1	FLOW									
110	RC	0.050	0.035	0.050	794	0.0300	0.00					
111	RX	0.00	6.00	17.00	22.00	28.00	32.00	36.00	48.00			
112	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00			
	*											
HEC-1 INPUT												
1												
LINE	ID	1	2	3	4	5	6	7	8	9	10	
113	KK	ON160	BASIN									
114	BA	0.020										
115	LG	0.33	0.33	6.00	0.20	14						
116	UC	0.366	0.760									
117	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
118	UA	100										
	*											
119	KK	CO160B	COMBINE									
120	HC	2										
	*											
121	KK	DB160	STORAGE									
122	KO											
123	RS	1	STOR									
124	SV		0.01	0.05	0.11	0.16	0.27	0.45	0.45			
125	SQ		3.00	7.00	9.00	14.00	17.00	20.00	65.00			
126	SE		1.00	2.00	3.00	4.00	5.00	6.00	6.24			
	*											
127	KK	RO161	ROUTE									
128	RS	1	FLOW									
129	RC	0.050	0.035	0.050	1451	0.0312	0.00					
130	RX	0.00	12.00	20.00	29.00	36.00	40.00	50.00	54.00			
131	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00			
	*											

187	UC	0.157	0.201								
188	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
189	UA	100									

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

190	KK	DB180	STORAGE								
191	KO										
192	RS	1	STOR								
193	SV		0.06	0.14	0.20	0.26	0.34	0.39	0.40		
194	SQ		1.00	3.00	4.00	5.00	5.00	6.00	6.00		
195	SE		0.50	1.00	1.50	2.00	2.50	2.95	3.00		

196	KK	RO182A	ROUTE								
197	RS	1	FLOW								
198	RC	0.050	0.035	0.050	555	0.0340	0.00				
199	RX	0.00	16.00	24.00	28.00	29.00	36.00	58.00	95.00		
200	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		

201	KK	ON179	BASIN								
202	BA	0.002									
203	LG	0.35	0.40	6.00	0.18	0					
204	UC	0.150	0.186								
205	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
206	UA	100									

207	KK	RO182C	ROUTE								
208	RS	1	FLOW								
209	RC	0.050	0.035	0.050	952	0.0340	0.00				
210	RX	0.00	16.00	24.00	28.00	29.00	36.00	58.00	95.00		
211	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		

212	KK	ON195	BASIN								
213	BA	0.009									
214	LG	0.35	0.40	6.00	0.18	0					
215	UC	0.272	0.394								
216	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
217	UA	100									

218	KK	RO182B	ROUTE								
219	RS	1	FLOW								
220	RC	0.050	0.035	0.050	1027	0.0340	0.00				
221	RX	0.00	16.00	24.00	28.00	29.00	36.00	58.00	95.00		
222	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		

223	KK	ON182	BASIN								
224	BA	0.009									
225	LG	0.32	0.32	6.00	0.20	15					
226	UC	0.197	0.237								
227	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
228	UA	100									

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

229	KK	CO182	COMBINE								
230	HC	4									

231	KK	DB182	STORAGE								
232	KO										
233	RS	1	STOR								
234	SV		0.01	0.02	0.04	0.05	0.13	0.18	0.26	0.34	
235	SQ		2.00	4.00	7.00	9.00	11.00	13.00	14.00	44.00	
236	SE		0.50	1.00	1.50	2.00	2.50	3.00	3.50	4.00	

238	BA	0.004									
239	LG	0.31	0.29	6.00	0.21	20					
240	UC	0.142	0.193								
241	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
242	UA	100									
	*										
243	KK	DB181	STORAGE								
244	KO										
245	RS	1	STOR								
246	SV		0.06	0.12	0.20	0.28	0.37	0.46			
247	SQ		1.00	1.00	2.00	2.00	2.00	2.00			
248	SE		0.50	1.00	1.50	2.00	2.50	3.00			
	*										
249	KK	CO183A	COMBINE								
250	HC	2									
	*										
251	KK	RO183A	ROUTE								
252	RS	1	FLOW								
253	RC	0.050	0.035	0.050	1262	0.0360	0.00				
254	RX	0.00	10.00	18.00	22.00	29.00	34.00	38.00	43.00		
255	RY	2.00	1.50	1.00	0.00	0.00	1.00	1.50	2.00		
	*										
256	KK	ON183	BASIN								
257	BA	0.007									
258	LG	0.33	0.34	6.00	0.18	9					
259	UC	0.254	0.481								
260	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
261	UA	100									
	*										
262	KK	ON190	BASIN								
263	BA	0.006									
264	LG	0.32	0.30	6.00	0.20	18					
265	UC	0.193	0.317								
266	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
267	UA	100									
	*										

HEC-1 INPUT

PAGE 8

LINE ID1.....2.....3.....4.....5.....6.....7.....8.....9.....10

268	KK	RO183B	ROUTE								
269	RS	1	FLOW								
270	RC	0.050	0.035	0.050	621	0.0360	0.00				
271	RX	0.00	10.00	18.00	22.00	29.00	34.00	38.00	43.00		
272	RY	2.00	1.50	1.00	0.00	0.00	1.00	1.50	2.00		
	*										
273	KK	CO183B	COMBINE								
274	HC	2									
	*										
275	KK	ON185	BASIN								
276	BA	0.002									
277	LG	0.35	0.40	6.00	0.18	0					
278	UC	0.149	0.184								
279	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
280	UA	100									
	*										
281	KK	ON191	BASIN								
282	BA	0.006									
283	LG	0.31	0.27	6.00	0.18	17					
284	UC	0.138	0.158								
285	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
286	UA	100									
	*										
287	KK	OFF105	BASIN								
288	BA	0.085									
289	LG	0.35	0.40	6.00	0.18	0					
290	UC	0.447	0.552								

293	KK	RO215A	ROUTE							
294	RS	1	FLOW							
295	RC	0.050	0.035	0.050	975	0.0360	0.00			
296	RX	0.00	9.00	21.00	25.00	32.00	36.00	70.00	90.00	
297	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00	
	*									

298	KK	OFF110	BASIN							
299	BA	0.037								
300	LG	0.35	0.40	6.00	0.18	0				
301	UC	0.356	0.484							
302	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0
303	UA	100								96.0
	*									

HEC-1 INPUT

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

304	KK	OFF115	BASIN							
305	BA	0.007								
306	LG	0.35	0.40	6.00	0.18	0				
307	UC	0.189	0.192							
308	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0
309	UA	100								96.0
	*									

310	KK	RO220	ROUTE							
311	RS	1	FLOW							
312	RC	0.050	0.035	0.050	310	0.0420	0.00			
313	RX	0.00	8.00	15.00	21.00	21.10	25.00	30.00	36.00	
314	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00	
	*									

315	KK	ON220	BASIN							
316	BA	0.003								
317	LG	0.35	0.40	6.00	0.18	0				
318	UC	0.184	0.266							
319	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0
320	UA	100								96.0
	*									

321	KK	CO220	COMBINE							
322	HC	2								
	*									

323	KK	RO215D	ROUTE							
324	RS	1	FLOW							
325	RC	0.050	0.035	0.050	364	0.0300	0.00			
326	RX	0.00	5.00	8.00	11.00	11.10	14.00	17.00	24.00	
327	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00	
	*									

328	KK	CO215A	COMBINE							
329	HC	2								
	*									

330	KK	RO215B	ROUTE							
331	RS	1	FLOW							
332	RC	0.050	0.035	0.050	800	0.0340	0.00			
333	RX	0.00	6.00	10.00	13.00	19.00	21.00	26.00	62.00	
334	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00	
	*									

335	KK	CO215B	COMBINE							
336	HC	2								
	*									

HEC-1 INPUT

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

337	KK	RO215C	ROUTE							
338	RS	1	FLOW							
339	RC	0.050	0.035	0.050	445	0.0340	0.00			

393	BA	0.003									
394	LG	0.35	0.40	6.00	0.18	0					
395	UC	0.176	0.215								
396	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
397	UA	100									
	*										
398	KK	CO260	COMBINE								
399	HC	3									
	*										
400	KK	RO200A	ROUTE								
401	RS	1	FLOW								
402	RC	0.050	0.035	0.050	611	0.0360	0.00				
403	RX	0.00	5.00	12.00	45.00	61.00	68.00	73.00	78.00		
404	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
405	KK	ON265	BASIN								
406	BA	0.002									
407	LG	0.35	0.40	6.00	0.18	0					
408	UC	0.169	0.259								
409	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
410	UA	100									
	*										

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

411	KK	RO200B	ROUTE								
412	RS	1	FLOW								
413	RC	0.050	0.035	0.050	345	0.0290	0.00				
414	RX	0.00	6.00	12.00	20.00	21.00	41.00	47.00	51.00		
415	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
416	KK	ON200	BASIN								
417	BA	0.006									
418	LG	0.35	0.40	6.00	0.18	0					
419	UC	0.207	0.260								
420	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
421	UA	100									
	*										
422	KK	ON270	BASIN								
423	BA	0.002									
424	LG	0.35	0.40	6.00	0.18	0					
425	UC	0.160	0.222								
426	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
427	UA	100									
	*										
428	KK	CO200	COMBINE								
429	HC	4									
	*										
430	KK	RO205	ROUTE								
431	RS	1	FLOW								
432	RC	0.050	0.035	0.050	1565	0.0370	0.00				
433	RX	0.00	6.00	10.00	15.00	48.00	58.00	64.00	77.00		
434	RY	4.00	2.00	1.00	0.00	0.00	1.00	2.00	4.00		
	*										
435	KK	ON205	BASIN								
436	BA	0.020									
437	LG	0.35	0.39	6.00	0.18	2					
438	UC	0.299	0.372								
439	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
440	UA	100									
	*										
441	KK	ON280	BASIN								
442	BA	0.001									
443	LG	0.35	0.40	6.00	0.18	0					
444	UC	0.113	0.129								
445	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0

1

HEC-1 INPUT

PAGE 13

LINE	ID	1	2	3	4	5	6	7	8	9	10
447	KK	RO210	ROUTE								
448	RS	1	FLOW								
449	RC	0.050	0.035	0.050	1804	0.0382	0.00				
450	RX	0.00	2.00	4.00	5.00	10.00	12.00	14.00	18.00		
451	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
452	KK	ON210	BASIN								
453	BA	0.017									
454	LG	0.35	0.40	6.00	0.18	0					
455	UC	0.341	0.504								
456	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
457	UA	100									
	*										
458	KK	CO205	COMBINE								
459	HC	4									
	*										
460	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(---->) DIVERSION OR PUMP FLOW
	(.) CONNECTOR	(<----) RETURN OF DIVERTED OR PUMPED FLOW
19	OFF95	
	V	
	V	
25	RO140A	
	.	
30	OFF100	
	V	
	V	
36	RO140B	
	.	
41	CO140A.....	
	V	
	V	
43	RO140C	
	.	
48	ON140	
	.	
54	CO140B.....	
	V	
	V	
56	RO155B	
	.	
61	ON135	
	V	
	V	
67	RO155A	
	.	
69	CO155A.....	
	V	
	V	
71	RO155C	
	.	
76	ON155	
	.	
82	CO155B.....	
	.	
84	ON145	

90	RO160A		
95		ON150	
		V	
		V	
101	RO160B		
106	CO160A.....		
	V		
	V		
108	RO160C		
113		ON160	
119	CO160B.....		
	V		
	V		
121	DB160		
	V		
	V		
127	RO161		
132		ON161	
138	CO161.....		
	V		
	V		
140	RO170		
145		ON170	
151			ON184
			V
			V
157			DB184
			V
			V
163			RO170
168	CO170.....		
170		ON165	
176			ON175
182	COEX1.....		
184	ON180		
	V		
	V		
190	DB180		
	V		
	V		
196	RO182A		
201		ON179	
		V	
		V	
207	RO182C		
212			ON195
			V

223
229
231
237
243
249
251
256
262
268
273
275
281
287
293
298
304
310
315
321
323
328
330
335
337

ON182

CO182.....

V
V

DB182

ON181

V
V

DB181

CO183A.....

V
V

RO183A

ON183

ON190

V
V

RO183B

CO183B.....

ON185

ON191

OFF105

V
V

RO215A

OFF110

OFF115

V
V

RO220

ON220

CO220.....

V
V

RO215D

CO215A.....

V
V

RO215B

CO215B.....

V
V

RO215C

348
350
355
361
363
368
374
376
381
387
392
398
400
405
411
416
422
428
430
435
441
447
452
458

```

CO215C.....
  V
  V
RO240A

      ON235

CO240A.....
  V
  V
RO240B

      ON240

CO240B.....
  V
  V
RO260A

      ON245
      V
      V
RO260B

      ON260

CO260.....
  V
  V
RO200A

      ON265
      V
      V
RO200B

      ON200

      ON270

CO200.....
  V
  V
RO205

      ON205

      ON280
      V
      V
RO210

      ON210

CO205.....

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

* RUN DATE 19FEB17 TIME 11:22:29 *
 *

* (916) 756-1104 *
 *

Flood Control District of Maricopa County
 STORYROCK PHIC PROP - STORYROCK PHASE 1C PROP CONDITION
 10 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 02/19/2017

9 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 5 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 2000 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 7JAN99 ENDING DATE
 NDTIME 2235 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.08 HOURS
 TOTAL TIME BASE 166.58 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

11 JD INDEX STORM NO. 1
 STRM 2.11 PRECIPITATION DEPTH
 TRDA 0.00 TRANSPOSITION DRAINAGE AREA

12 PI PRECIPITATION PATTERN
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.03
 0.03 0.03 0.05 0.05 0.05 0.15 0.15 0.15 0.03 0.03 0.03
 0.03 0.01 0.01 0.01 0.01 0.01 0.01 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00

15 JD INDEX STORM NO. 2
 STRM 2.09 PRECIPITATION DEPTH
 TRDA 0.50 TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.03
 0.03 0.03 0.05 0.05 0.05 0.15 0.15 0.15 0.03 0.03 0.03
 0.03 0.01 0.01 0.01 0.01 0.01 0.01 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00

 * *

```

*****
122 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

*** **

```

*****
*          *
157 KK      *  DB184 *  STORAGE
*          *
*****
    
```

```

158 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

*** **

```

*****
*          *
190 KK      *  DB180 *  STORAGE
*          *
*****
    
```

```

191 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

*** **

```

*****
*          *
231 KK      *  DB182 *  STORAGE
*          *
*****
    
```

```

232 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

*** **

```

*****
*          *
243 KK      *  DB181 *  STORAGE
*          *
*****
    
```

```

244 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
-----------	---------	-----------	--------------	---------------------------------	------------	---------------	-------------------

+	HYDROGRAPH AT	OFF95	36.	4.33	5.	1.	0.	0.06
	ROUTED TO	RO140A	36.	4.33	5.	1.	0.	0.06
+	HYDROGRAPH AT	OFF100	9.	4.17	1.	0.	0.	0.01
+	ROUTED TO	RO140B	9.	4.17	1.	0.	0.	0.01
+	2 COMBINED AT	CO140A	43.	4.33	6.	1.	0.	0.08
+	ROUTED TO	RO140C	42.	4.33	6.	1.	0.	0.08
+	HYDROGRAPH AT	ON140	12.	4.25	1.	0.	0.	0.02
+	2 COMBINED AT	CO140B	52.	4.33	7.	2.	1.	0.09
+	ROUTED TO	RO155B	49.	4.42	7.	2.	1.	0.09
+	HYDROGRAPH AT	ON135	7.	4.17	1.	0.	0.	0.01
+	ROUTED TO	RO155A	7.	4.17	1.	0.	0.	0.01
+	2 COMBINED AT	CO155A	53.	4.42	8.	2.	1.	0.10
+	ROUTED TO	RO155C	52.	4.42	8.	2.	1.	0.10
+	HYDROGRAPH AT	ON155	17.	4.17	2.	0.	0.	0.02
+	2 COMBINED AT	CO155B	61.	4.42	9.	2.	1.	0.12
+	HYDROGRAPH AT	ON145	11.	4.17	1.	0.	0.	0.01
+	ROUTED TO	RO160A	11.	4.25	1.	0.	0.	0.01
+	HYDROGRAPH AT	ON150	7.	4.17	1.	0.	0.	0.01
+	ROUTED TO	RO160B	7.	4.17	1.	0.	0.	0.01
+	2 COMBINED AT	CO160A	17.	4.25	2.	0.	0.	0.02
+	ROUTED TO	RO160C	17.	4.25	2.	0.	0.	0.02
+	HYDROGRAPH AT	ON160	11.	4.25	2.	0.	0.	0.02
+	2 COMBINED AT	CO160B	27.	4.25	4.	1.	0.	0.04
+	ROUTED TO	DB160	18.	4.50	4.	1.	0.	0.04
+	ROUTED TO	RO161	18.	4.67	4.	1.	0.	0.04
+	HYDROGRAPH AT	ON161	7.	4.25	1.	0.	0.	0.01

+	2 COMBINED AT	CO183B	11.	4.17	1.	0.	0.	0.01
+	HYDROGRAPH AT	ON185	2.	4.08	0.	0.	0.	0.00
+	HYDROGRAPH AT	ON191	9.	4.08	1.	0.	0.	0.01
+	HYDROGRAPH AT	OFF105	49.	4.33	7.	2.	1.	0.09
+	ROUTED TO	RO215A	46.	4.42	7.	2.	1.	0.09
+	HYDROGRAPH AT	OFF110	24.	4.25	3.	1.	0.	0.04
+	HYDROGRAPH AT	OFF115	8.	4.08	1.	0.	0.	0.01
+	ROUTED TO	RO220	8.	4.17	1.	0.	0.	0.01
+	HYDROGRAPH AT	ON220	3.	4.08	0.	0.	0.	0.00
+	2 COMBINED AT	CO220	11.	4.08	1.	0.	0.	0.01
+	ROUTED TO	RO215D	11.	4.17	1.	0.	0.	0.01
+	2 COMBINED AT	CO215A	33.	4.25	4.	1.	0.	0.05
+	ROUTED TO	RO215B	32.	4.25	4.	1.	0.	0.05
+	2 COMBINED AT	CO215B	75.	4.33	10.	3.	1.	0.13
+	ROUTED TO	RO215C	74.	4.33	10.	3.	1.	0.13
+	HYDROGRAPH AT	ON215	28.	4.17	2.	1.	0.	0.03
+	2 COMBINED AT	CO215C	93.	4.33	13.	3.	1.	0.16
+	ROUTED TO	RO240A	93.	4.33	13.	3.	1.	0.16
+	HYDROGRAPH AT	ON235	2.	4.08	0.	0.	0.	0.00
+	2 COMBINED AT	CO240A	94.	4.33	13.	3.	1.	0.16
+	ROUTED TO	RO240B	94.	4.33	13.	3.	1.	0.16
+	HYDROGRAPH AT	ON240	6.	4.17	1.	0.	0.	0.01
+	2 COMBINED AT	CO240B	97.	4.33	13.	3.	1.	0.17
+	ROUTED TO	RO260A	97.	4.33	13.	3.	1.	0.17
+	HYDROGRAPH AT	ON245	3.	4.08	0.	0.	0.	0.00
+	ROUTED TO							

+	HYDROGRAPH AT	ON260	3.	4.08	0.	0.	0.	0.00
	3 COMBINED AT							
+		CO260	100.	4.33	14.	3.	1.	0.18
	ROUTED TO							
+		RO200A	99.	4.42	14.	3.	1.	0.18
	HYDROGRAPH AT							
+		ON265	2.	4.08	0.	0.	0.	0.00
	ROUTED TO							
+		RO200B	2.	4.17	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON200	6.	4.08	0.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON270	2.	4.08	0.	0.	0.	0.00
	4 COMBINED AT							
+		CO200	104.	4.33	14.	4.	1.	0.19
	ROUTED TO							
+		RO205	102.	4.42	14.	4.	1.	0.19
	HYDROGRAPH AT							
+		ON205	16.	4.17	2.	0.	0.	0.02
	HYDROGRAPH AT							
+		ON280	1.	4.00	0.	0.	0.	0.00
	ROUTED TO							
+		RO210	1.	4.17	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON210	11.	4.25	1.	0.	0.	0.02
	4 COMBINED AT							
+		CO205	120.	4.42	17.	4.	1.	0.23
1								

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION INTERVAL		
							PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1	STORM AREA (SQ MI) =		0.00						
RO155A	MANE	0.17	6.74	250.24	0.73	5.00	6.71	250.00	0.73

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3094E+00 EXCESS=0.0000E+00 OUTFLOW=0.3094E+00 BASIN STORAGE=0.3251E-18 PERCENT ERROR= 0.0

FOR STORM = 2	STORM AREA (SQ MI) =		0.50						
RO155A	MANE	0.15	6.68	250.29	0.72	5.00	6.64	250.00	0.72

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3059E+00 EXCESS=0.0000E+00 OUTFLOW=0.3059E+00 BASIN STORAGE=0.3257E-18 PERCENT ERROR= 0.0

*** NORMAL END OF HEC-1 ***

34	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
35	UA	100									
	*										
36	KK	RO140B	ROUTE								
37	RS	1	FLOW								
38	RC	0.050	0.035	0.050	626	0.0340	0.00				
39	RX	0.00	7.00	8.00	11.00	11.20	14.00	16.00	31.00		
40	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

41	KK	CO140A	COMBINE								
42	HC	2									
	*										
43	KK	RO140C	ROUTE								
44	RS	1	FLOW								
45	RC	0.050	0.035	0.050	1210	0.0320	0.00				
46	RX	0.00	3.00	6.00	11.00	11.20	17.00	23.00	48.00		
47	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										

48	KK	ON140	BASIN								
49	BA	0.017									
50	LG	0.34	0.39	6.00	0.18	4					
51	UC	0.249	0.356								
52	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
53	UA	100									
	*										

54	KK	CO140B	COMBINE								
55	HC	2									
	*										
56	KK	RO155B	ROUTE								
57	RS	1	FLOW								
58	RC	0.050	0.035	0.050	910	0.0350	0.00				
59	RX	0.00	16.00	61.00	76.00	90.00	121.00	200.00	226.00		
60	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										

61	KK	ON135	BASIN								
62	BA	0.008									
63	LG	0.35	0.40	6.00	0.18	0					
64	UC	0.188	0.251								
65	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
66	UA	100									
	*										

67	KK	RO155A	ROUTE								
68	RK	837	0.2900	0.013		CIRC	2.000				
	*										

69	KK	CO155A	COMBINE								
70	HC	2									
	*										

71	KK	RO155C	ROUTE								
72	RS	1	FLOW								
73	RC	0.050	0.035	0.050	392	0.0330	0.00				
74	RX	0.00	14.00	22.00	30.00	42.00	54.00	68.00	90.00		
75	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		
	*										

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

*76	KK	ON155	BASIN								
77	BA	0.017									
78	LG	0.33	0.34	6.00	0.19	11					
79	UC	0.195	0.222								
80	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0

82	KK	CO155B	COMBINE										
83	HC	2											
	*												
84	KK	ON145	BASIN										
85	BA	0.014											
86	LG	0.31	0.38	6.00	0.19	9							
87	UC	0.223	0.320										
88	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0		
89	UA	100											
	*												
90	KK	RO160A	ROUTE										
91	RS	1	FLOW										
92	RC	0.050	0.035	0.050	581	0.0280	0.00						
93	RX	0.00	6.00	12.00	18.00	22.00	28.00	34.00	40.00				
94	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00				
	*												
95	KK	ON150	BASIN										
96	BA	0.007											
97	LG	0.35	0.40	6.00	0.18	0							
98	UC	0.166	0.196										
99	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0		
100	UA	100											
	*												
101	KK	RO160B	ROUTE										
102	RS	1	FLOW										
103	RC	0.050	0.035	0.050	800	0.0300	0.00						
104	RX	0.00	6.00	10.00	14.00	16.00	18.00	22.00	28.00				
105	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00				
	*												
106	KK	CO160A	COMBINE										
107	HC	2											
	*												
108	KK	RO160C	ROUTE										
109	RS	1	FLOW										
110	RC	0.050	0.035	0.050	794	0.0300	0.00						
111	RX	0.00	6.00	17.00	22.00	28.00	32.00	36.00	48.00				
112	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00				
	*												
					HEC-1 INPUT								
113	KK	ON160	BASIN										
114	BA	0.020											
115	LG	0.33	0.33	6.00	0.20	14							
116	UC	0.284	0.573										
117	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0		
118	UA	100											
	*												
119	KK	CO160B	COMBINE										
120	HC	2											
	*												
121	KK	DB160	STORAGE										
122	KO												
123	RS	1	STOR										
124	SV		0.01	0.05	0.11	0.16	0.27	0.45	0.45				
125	SQ		3.00	7.00	9.00	14.00	17.00	20.00	65.00				
126	SE		1.00	2.00	3.00	4.00	5.00	6.00	6.24				
	*												
127	KK	RO161	ROUTE										
128	RS	1	FLOW										
129	RC	0.050	0.035	0.050	1451	0.0312	0.00						
130	RX	0.00	12.00	20.00	29.00	36.00	40.00	50.00	54.00				
131	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00				
	*												

1

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

187	UC	0.124	0.155								
188	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
189	UA	100									

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

190	KK	DB180	STORAGE								
191	KO										
192	RS	1	STOR								
193	SV		0.06	0.14	0.20	0.26	0.34	0.39	0.40		
194	SQ		1.00	3.00	4.00	5.00	5.00	6.00	6.00		
195	SE		0.50	1.00	1.50	2.00	2.50	2.95	3.00		

196	KK	RO182A	ROUTE								
197	RS	1	FLOW								
198	RC	0.050	0.035	0.050	555	0.0340	0.00				
199	RX	0.00	16.00	24.00	28.00	29.00	36.00	58.00	95.00		
200	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		

201	KK	ON179	BASIN								
202	BA	0.002									
203	LG	0.35	0.40	6.00	0.18	0					
204	UC	0.113	0.135								
205	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
206	UA	100									

207	KK	RO182C	ROUTE								
208	RS	1	FLOW								
209	RC	0.050	0.035	0.050	952	0.0340	0.00				
210	RX	0.00	16.00	24.00	28.00	29.00	36.00	58.00	95.00		
211	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		

212	KK	ON195	BASIN								
213	BA	0.009									
214	LG	0.35	0.40	6.00	0.18	0					
215	UC	0.204	0.287								
216	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
217	UA	100									

218	KK	RO182B	ROUTE								
219	RS	1	FLOW								
220	RC	0.050	0.035	0.050	1027	0.0340	0.00				
221	RX	0.00	16.00	24.00	28.00	29.00	36.00	58.00	95.00		
222	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		

223	KK	ON182	BASIN								
224	BA	0.009									
225	LG	0.32	0.32	6.00	0.20	15					
226	UC	0.153	0.179								
227	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
228	UA	100									

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

229	KK	CO182	COMBINE								
230	HC	4									

231	KK	DB182	STORAGE								
232	KO										
233	RS	1	STOR								
234	SV		0.01	0.02	0.04	0.05	0.13	0.18	0.26	0.34	
235	SQ		2.00	4.00	7.00	9.00	11.00	13.00	14.00	44.00	
236	SE		0.50	1.00	1.50	2.00	2.50	3.00	3.50	4.00	

*

293	KK	RO215A	ROUTE								
294	RS	1	FLOW								
295	RC	0.050	0.035	0.050	975	0.0360	0.00				
296	RX	0.00	9.00	21.00	25.00	32.00	36.00	70.00	90.00		
297	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		

*

298	KK	OFF110	BASIN								
299	BA	0.037									
300	LG	0.35	0.40	6.00	0.18	0					
301	UC	0.268	0.353								
302	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
303	UA	100									

*

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

304	KK	OFF115	BASIN								
305	BA	0.007									
306	LG	0.35	0.40	6.00	0.18	0					
307	UC	0.142	0.140								
308	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
309	UA	100									

*

310	KK	RO220	ROUTE								
311	RS	1	FLOW								
312	RC	0.050	0.035	0.050	310	0.0420	0.00				
313	RX	0.00	8.00	15.00	21.00	21.10	25.00	30.00	36.00		
314	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		

*

315	KK	ON220	BASIN								
316	BA	0.003									
317	LG	0.35	0.40	6.00	0.18	0					
318	UC	0.139	0.194								
319	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
320	UA	100									

*

321	KK	CO220	COMBINE								
322	HC	2									

*

323	KK	RO215D	ROUTE								
324	RS	1	FLOW								
325	RC	0.050	0.035	0.050	364	0.0300	0.00				
326	RX	0.00	5.00	8.00	11.00	11.10	14.00	17.00	24.00		
327	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		

*

328	KK	CO215A	COMBINE								
329	HC	2									

*

330	KK	RO215B	ROUTE								
331	RS	1	FLOW								
332	RC	0.050	0.035	0.050	800	0.0340	0.00				
333	RX	0.00	6.00	10.00	13.00	19.00	21.00	26.00	62.00		
334	RY	2.00	0.50	1.00	0.00	0.00	1.00	0.00	2.00		

*

335	KK	CO215B	COMBINE								
336	HC	2									

*

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

337	KK	RO215C	ROUTE								
338	RS	1	FLOW								
339	RC	0.050	0.035	0.050	445	0.0340	0.00				

393	BA	0.003									
394	LG	0.35	0.40	6.00	0.18	0					
395	UC	0.133	0.157								
396	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
397	UA	100									
	*										
398	KK	CO260	COMBINE								
399	HC	3									
	*										
400	KK	RO200A	ROUTE								
401	RS	1	FLOW								
402	RC	0.050	0.035	0.050	611	0.0360	0.00				
403	RX	0.00	5.00	12.00	45.00	61.00	68.00	73.00	78.00		
404	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
405	KK	ON265	BASIN								
406	BA	0.002									
407	LG	0.35	0.40	6.00	0.18	0					
408	UC	0.127	0.189								
409	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
410	UA	100									
	*										

HEC-1 INPUT

PAGE 12

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

411	KK	RO200B	ROUTE								
412	RS	1	FLOW								
413	RC	0.050	0.035	0.050	345	0.0290	0.00				
414	RX	0.00	6.00	12.00	20.00	21.00	41.00	47.00	51.00		
415	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
416	KK	ON200	BASIN								
417	BA	0.006									
418	LG	0.35	0.40	6.00	0.18	0					
419	UC	0.155	0.190								
420	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
421	UA	100									
	*										
422	KK	ON270	BASIN								
423	BA	0.002									
424	LG	0.35	0.40	6.00	0.18	0					
425	UC	0.121	0.162								
426	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
427	UA	100									
	*										
428	KK	CO200	COMBINE								
429	HC	4									
	*										
430	KK	RO205	ROUTE								
431	RS	1	FLOW								
432	RC	0.050	0.035	0.050	1565	0.0370	0.00				
433	RX	0.00	6.00	10.00	15.00	48.00	58.00	64.00	77.00		
434	RY	4.00	2.00	1.00	0.00	0.00	1.00	2.00	4.00		
	*										
435	KK	ON205	BASIN								
436	BA	0.020									
437	LG	0.35	0.39	6.00	0.18	2					
438	UC	0.226	0.272								
439	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
440	UA	100									
	*										
441	KK	ON280	BASIN								
442	BA	0.001									
443	LG	0.35	0.40	6.00	0.18	0					
444	UC	0.085	0.094								
445	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0

1

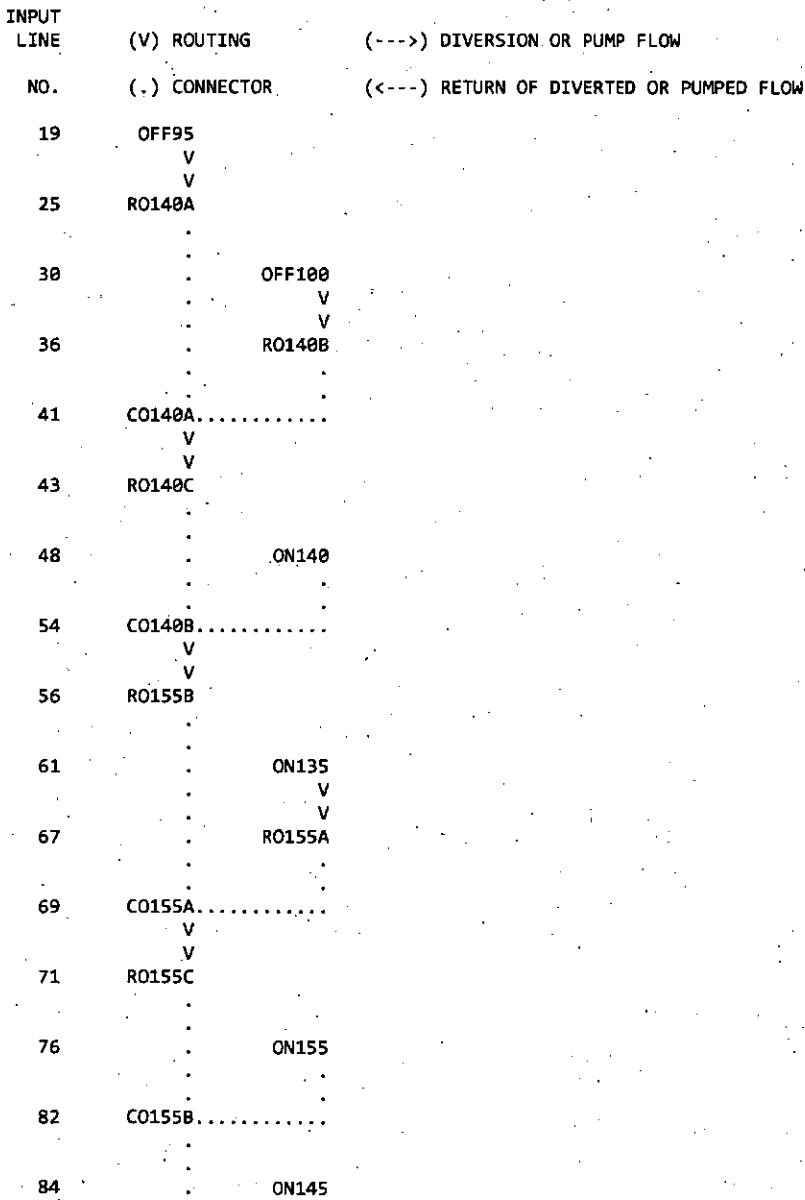
HEC-1 INPUT

PAGE 13

LINE	ID	1	2	3	4	5	6	7	8	9	10
447	KK	R0210	ROUTE								
448	RS	1	FLOW								
449	RC	0.050	0.035	0.050	1804	0.0382	0.00				
450	RX	0.00	2.00	4.00	5.00	10.00	12.00	14.00	18.00		
451	RY	3.00	2.00	1.00	0.00	0.00	1.00	2.00	3.00		
	*										
452	KK	ON210	BASIN								
453	BA	0.017									
454	LG	0.35	0.40	6.00	0.18	0					
455	UC	0.256	0.367								
456	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
457	UA	100									
	*										
458	KK	CO205	COMBINE								
459	HC	4									
	*										
460	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK



90	R0160A		
95		ON150	
		V	
		V	
101		R0160B	
106	CO160A.....		
	V		
	V		
108	R0160C		
113		ON160	
119	CO160B.....		
	V		
	V		
121	DB160		
	V		
	V		
127	R0161		
132		ON161	
138	CO161.....		
	V		
	V		
140	R0170		
145		ON170	
151		ON184	
		V	
		V	
157		DB184	
		V	
		V	
163		R0170	
168	CO170.....		
170		ON165	
176		ON175	
182	COEX1.....		
184	ON180		
	V		
	V		
190	DB180		
	V		
	V		
196	R0182A		
201		ON179	
		V	
		V	
207		R0182C	
212		ON195	
		V	

223			ON182		
229	CO182			
	V				
	V				
231	DB182				
237		ON181			
		V			
		V			
243		DB181			
249	CO183A			
	V				
	V				
251	RO183A				
256		ON183			
262			ON190		
			V		
			V		
268			RO183B		
273	CO183B			
275		ON185			
281			ON191		
287				OFF105	
				V	
				V	
293				RO215A	
298					OFF110
304					OFF115
					V
					V
310					RO220
315					
					ON220
321					CO220
				
					V
					V
323					RO215D
328				CO215A
				V	
				V	
330				RO215B	
335				CO215B
				V	
				V	
337				RO215C	

```

348      . . . . . CO215C .....
          . . . . .   V
          . . . . .   V
350      . . . . . RO240A
          . . . . .
          . . . . . ON235
          . . . . .
361      . . . . . CO240A .....
          . . . . .   V
          . . . . .   V
363      . . . . . RO240B
          . . . . .
          . . . . . ON240
          . . . . .
374      . . . . . CO240B .....
          . . . . .   V
          . . . . .   V
376      . . . . . RO260A
          . . . . .
          . . . . . ON245
          . . . . .   V
381      . . . . .   V
          . . . . .   V
387      . . . . . RO260B
          . . . . .
          . . . . . ON260
          . . . . .
392      . . . . .
          . . . . .
          . . . . . CO260 .....
          . . . . .   V
          . . . . .   V
400      . . . . . RO200A
          . . . . .
          . . . . . ON265
          . . . . .   V
405      . . . . .   V
          . . . . .   V
411      . . . . . RO200B
          . . . . .
          . . . . . ON200
          . . . . .
416      . . . . .
          . . . . .
          . . . . . ON270
          . . . . .
422      . . . . .
          . . . . .
          . . . . . CO200 .....
          . . . . .   V
          . . . . .   V
430      . . . . . RO205
          . . . . .
          . . . . . ON205
          . . . . .
435      . . . . .
          . . . . .
          . . . . . ON280
          . . . . .   V
441      . . . . .   V
          . . . . .   V
447      . . . . . RO210
          . . . . .
          . . . . . ON210
          . . . . .
452      . . . . .
          . . . . .
          . . . . . CO205 .....
          . . . . .
          . . . . .
458      . . . . .
          . . . . .
          . . . . .

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
*

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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
*

```

* RUN DATE 19FEB17 TIME 11:22:34 *

(916) 756-1104

Flood Control District of Maricopa County
 STORYROCK PHIC PROP - STORYROCK PHASE 1C PROP CONDITION
 100 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 02/19/2017

9 IO OUTPUT CONTROL VARIABLES

IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA

NMIN 5 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 2000 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 7JAN99 ENDING DATE
 NDTIME 2235 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.08 HOURS
 TOTAL TIME BASE 166.58 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

11 JD INDEX STORM NO. 1

STRM 3.17 PRECIPITATION DEPTH
 TRDA 0.00 TRANSPOSITION DRAINAGE AREA

12 PI PRECIPITATION PATTERN

0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.03
0.03	0.03	0.05	0.05	0.05	0.15	0.15	0.15	0.15	0.03	0.03
0.03	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00									

15 JD INDEX STORM NO. 2

STRM 3.15 PRECIPITATION DEPTH
 TRDA 0.50 TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN

0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.03
0.03	0.03	0.05	0.05	0.05	0.15	0.15	0.15	0.15	0.03	0.03
0.03	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00									

*** **

```
*****
122 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
```

*** **

```
*****
157 KK      *      DB184 *      STORAGE
            *      *
            *****
```

```
158 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
```

*** **

```
*****
190 KK      *      DB180 *      STORAGE
            *      *
            *****
```

```
191 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
```

*** **

```
*****
231 KK      *      DB182 *      STORAGE
            *      *
            *****
```

```
232 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
```

WARNING --- ROUTED OUTFLOW (44.) IS GREATER THAN MAXIMUM OUTFLOW (44.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW (44.) IS GREATER THAN MAXIMUM OUTFLOW (44.) IN STORAGE-OUTFLOW TABLE

*** **

```
*****
243 KK      *      DB181 *      STORAGE
            *      *
            *****
```

```
244 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
```


+	HYDROGRAPH AT	ON161	16.	4.17	2.	0.	0.	0.01
	2 COMBINED AT							
+		CO161	70.	4.25	9.	2.	1.	0.05
	ROUTED TO							
+		RO170	69.	4.33	9.	2.	1.	0.05
	HYDROGRAPH AT							
+		ON170	11.	4.08	1.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON184	23.	4.00	2.	0.	0.	0.01
	ROUTED TO							
+		DB184	10.	4.25	1.	0.	0.	0.01
	ROUTED TO							
+		RO170	9.	4.33	1.	0.	0.	0.01
	3 COMBINED AT							
+		CO170	83.	4.33	11.	3.	1.	0.06
	HYDROGRAPH AT							
+		ON165	13.	4.17	2.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON175	7.	4.00	0.	0.	0.	0.00
	4 COMBINED AT							
+		COEX1	245.	4.25	33.	8.	3.	0.19
	HYDROGRAPH AT							
+		ON180	12.	4.00	1.	0.	0.	0.00
	ROUTED TO							
+		DB180	5.	4.25	1.	0.	0.	0.00
	ROUTED TO							
+		RO182A	5.	4.33	1.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON179	5.	4.00	0.	0.	0.	0.00
	ROUTED TO							
+		RO182C	5.	4.08	0.	0.	0.	0.00
	HYDROGRAPH AT							
+		ON195	16.	4.08	2.	0.	0.	0.01
	ROUTED TO							
+		RO182B	16.	4.17	2.	0.	0.	0.01
	HYDROGRAPH AT							
+		ON182	22.	4.08	2.	0.	0.	0.01
	4 COMBINED AT							
+		CO182	43.	4.08	5.	1.	0.	0.02
	ROUTED TO							
+		DB182	44.	4.17	5.	1.	0.	0.02
	HYDROGRAPH AT							
+		ON181	10.	4.00	1.	0.	0.	0.00
	ROUTED TO							
+		DB181	2.	4.08	1.	0.	0.	0.00
	2 COMBINED AT							
+		CO183A	46.	4.17	5.	1.	0.	0.03
	ROUTED TO							
+		RO183A	43.	4.25	5.	1.	0.	0.03
	HYDROGRAPH AT							
+		ON183	12.	4.17	1.	0.	0.	0.01

+	ROUTED TO	RO260B	6.	4.08	1.	0.	0.	0.00
+	HYDROGRAPH AT	ON260	7.	4.08	1.	0.	0.	0.00
+	3 COMBINED AT	CO260	251.	4.25	29.	7.	2.	0.18
+	ROUTED TO	RO200A	252.	4.25	29.	7.	2.	0.18
+	HYDROGRAPH AT	ON265	4.	4.08	0.	0.	0.	0.00
+	ROUTED TO	RO200B	4.	4.08	0.	0.	0.	0.00
+	HYDROGRAPH AT	ON200	13.	4.08	1.	0.	0.	0.01
+	HYDROGRAPH AT	ON270	5.	4.00	0.	0.	0.	0.00
+	4 COMBINED AT	CO200	264.	4.25	31.	8.	3.	0.19
+	ROUTED TO	RO205	256.	4.33	31.	8.	3.	0.19
+	HYDROGRAPH AT	ON205	36.	4.17	3.	1.	0.	0.02
+	HYDROGRAPH AT	ON280	3.	4.00	0.	0.	0.	0.00
+	ROUTED TO	RO210	2.	4.08	0.	0.	0.	0.00
+	HYDROGRAPH AT	ON210	26.	4.17	3.	1.	0.	0.02
+	4 COMBINED AT	CO205	308.	4.25	38.	9.	3.	0.23

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INSTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO COMPUTATION INTERVAL		
							PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)
FOR STORM = 1	STORM AREA (SQ MI) =			0.50					
RO155A	MANE	0.21	15.60	245.37	1.56	5.00	15.47	245.00	1.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6662E+00 EXCESS=0.0000E+00 OUTFLOW=0.6662E+00 BASIN STORAGE=0.3231E-18 PERCENT ERROR= 0.0

FOR STORM = 2	STORM AREA (SQ MI) =			0.50					
RO155A	MANE	0.22	15.47	245.44	1.55	5.00	15.35	245.00	1.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6596E+00 EXCESS=0.0000E+00 OUTFLOW=0.6595E+00 BASIN STORAGE=0.3235E-18 PERCENT ERROR= 0.0

Appendix C – Hydraulics

HEC-RAS Output (Existing and Proposed Conditions)

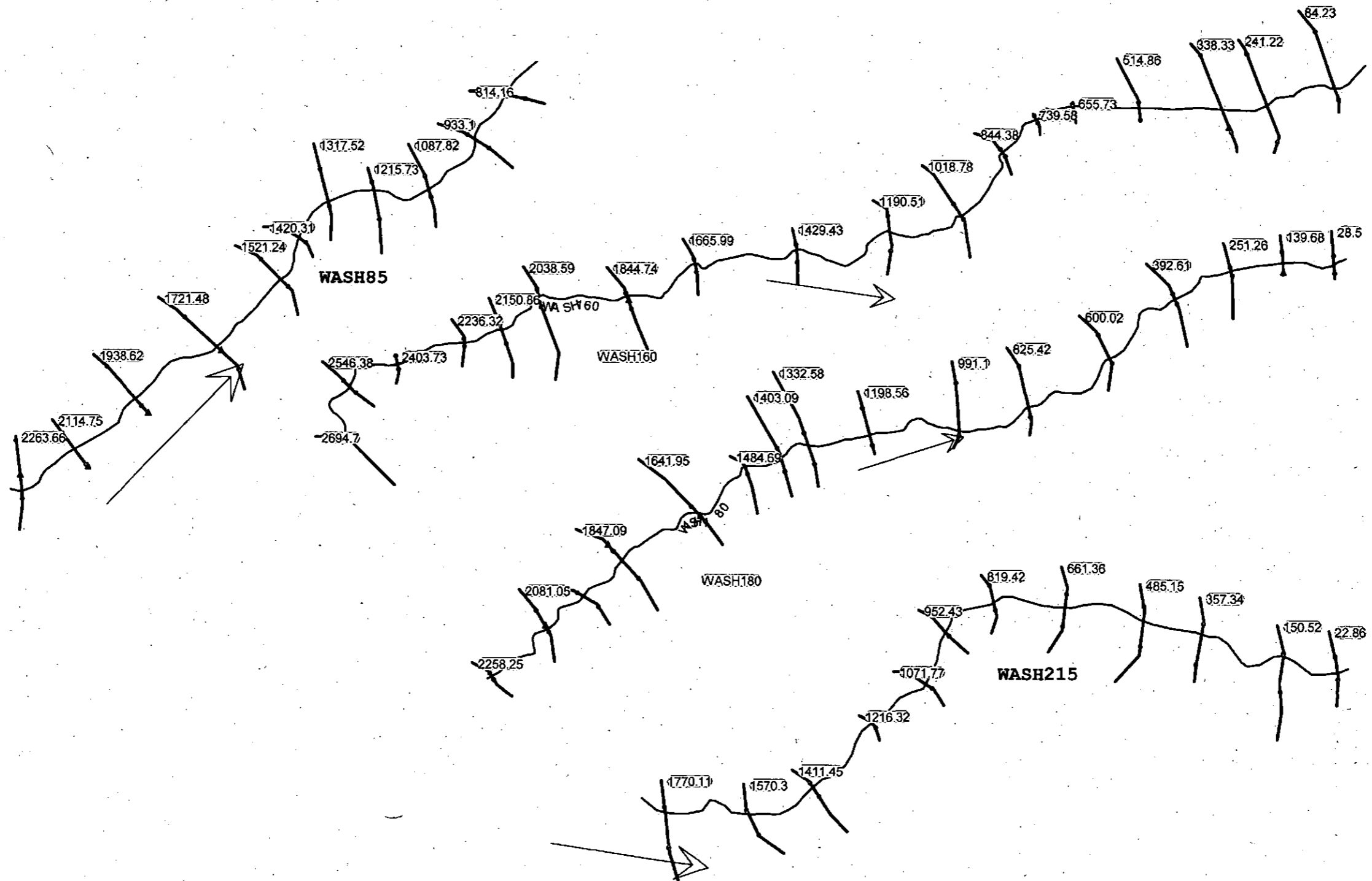
- Schematic Geometry
- Cross Sections
- Profiles
- Summary Table

HY-8 Output:

- Preliminary Culvert Calculations

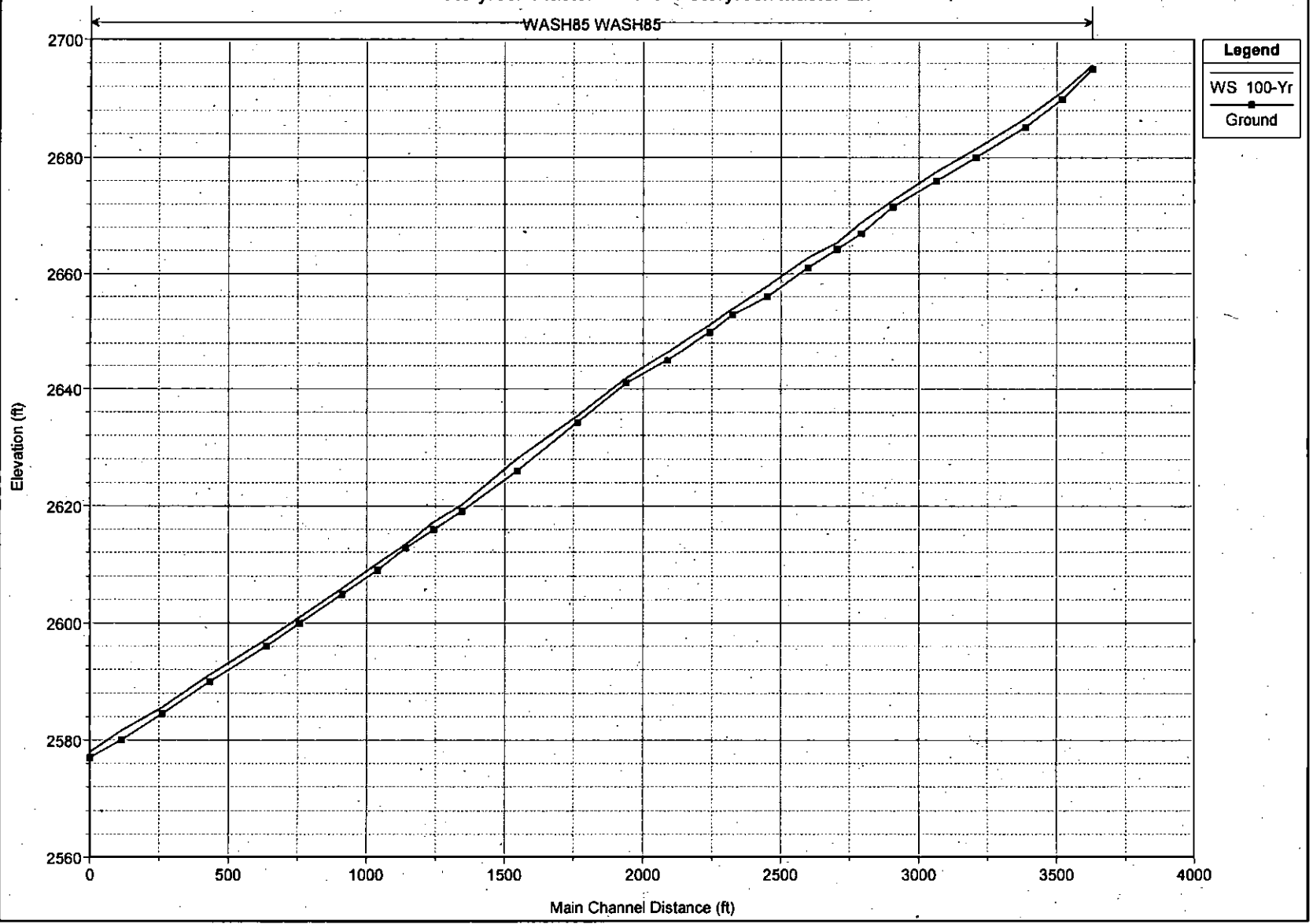
First Flush Spillway/Dissipation Basin Design:

HEC-RAS Existing Condition



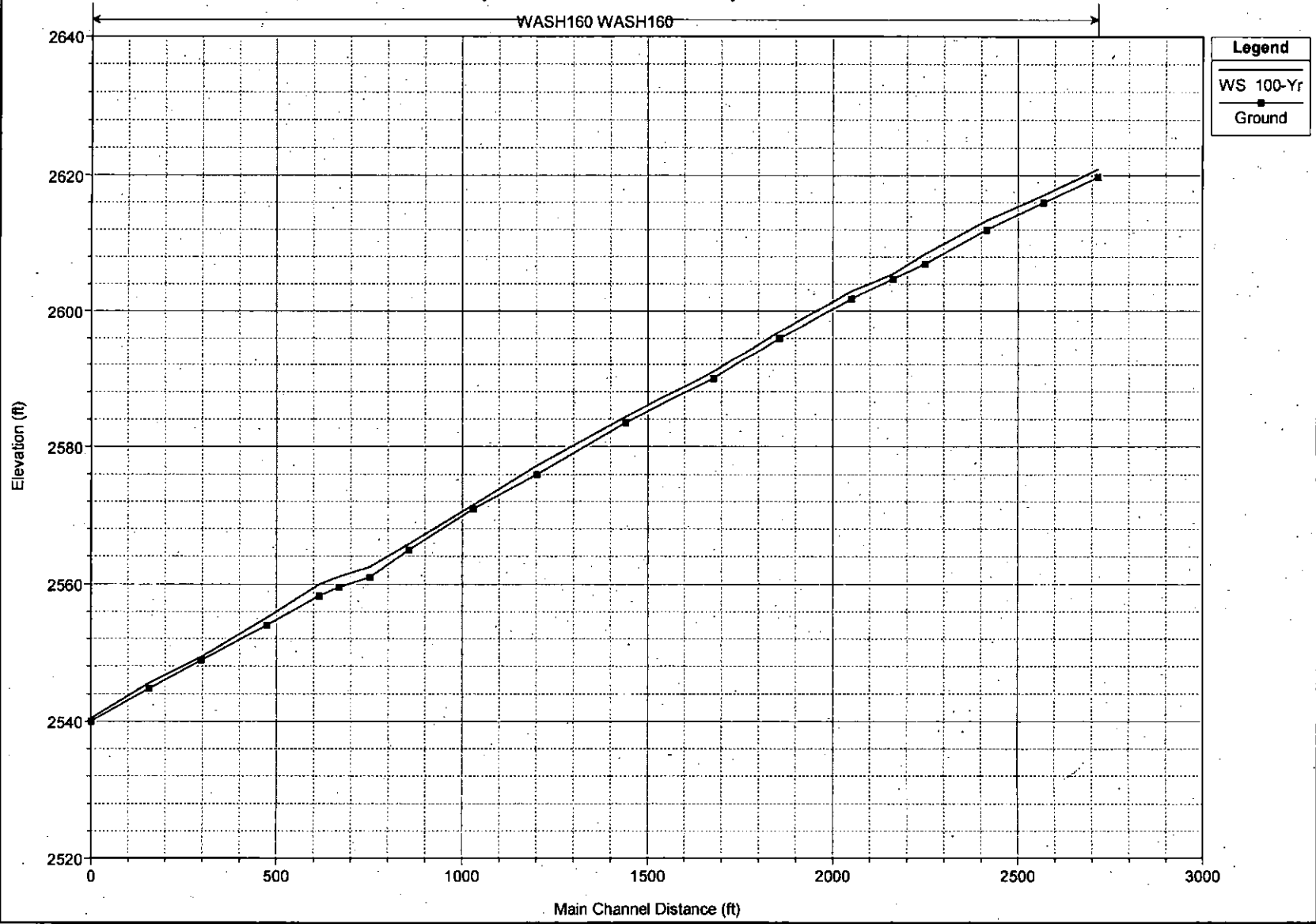
Storyrock Master Plan: Storyrock Master Ex

WASH85 WASH85



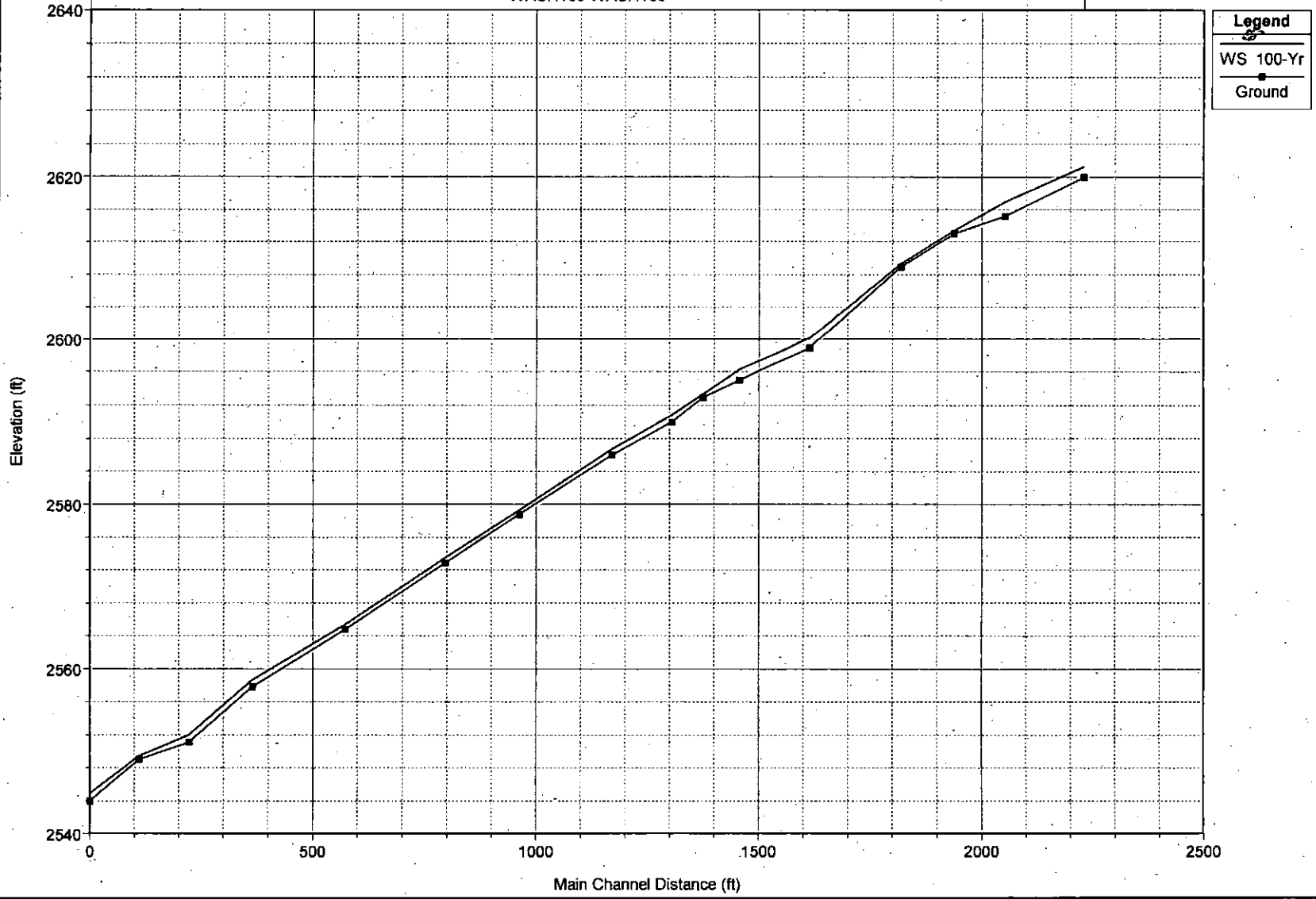
Storyrock Master Plan: Storyrock Master Ex

WASH160 WASH160



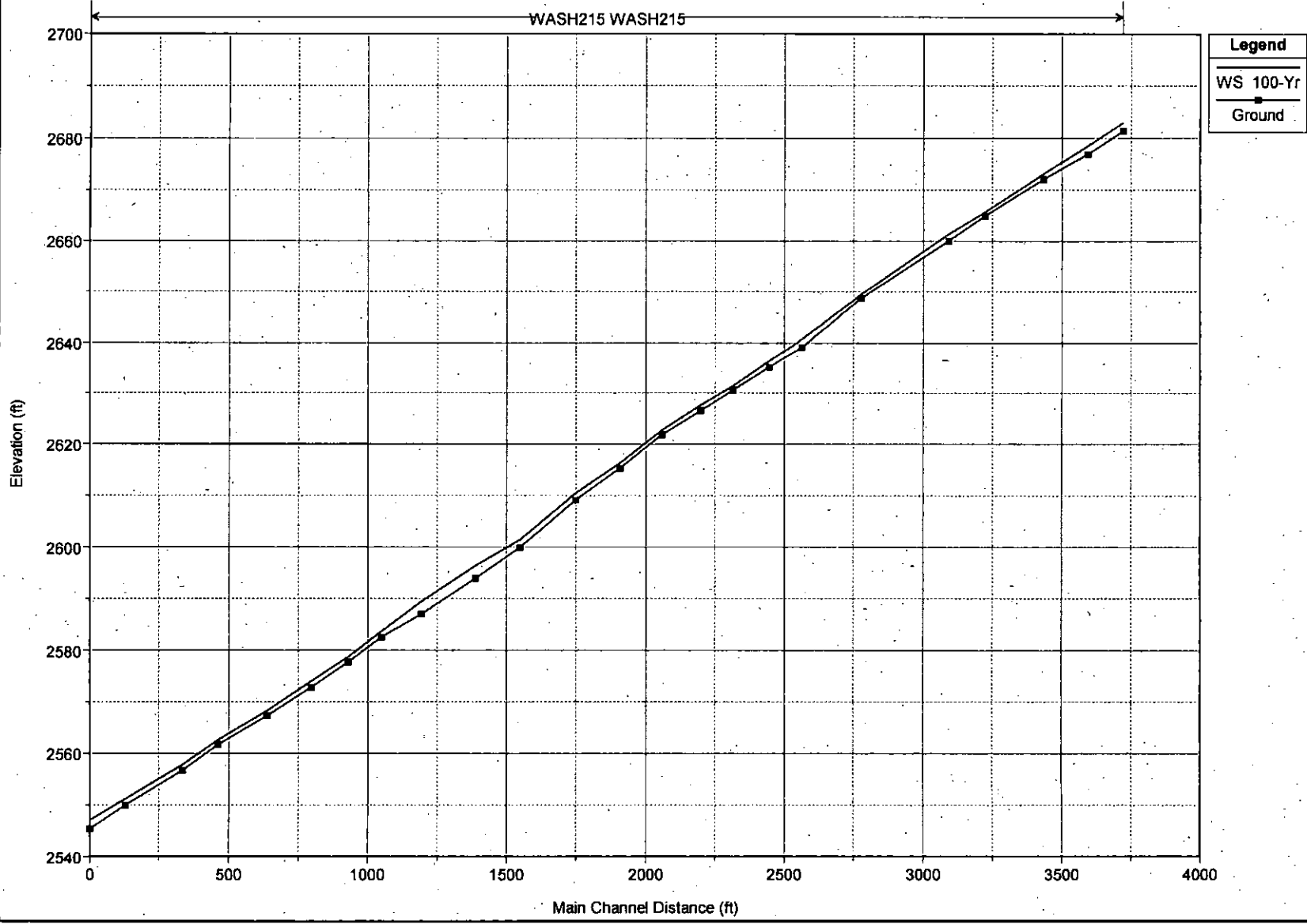
Storyrock Master Plan: Storyrock Master Ex

WASH180 WASH180

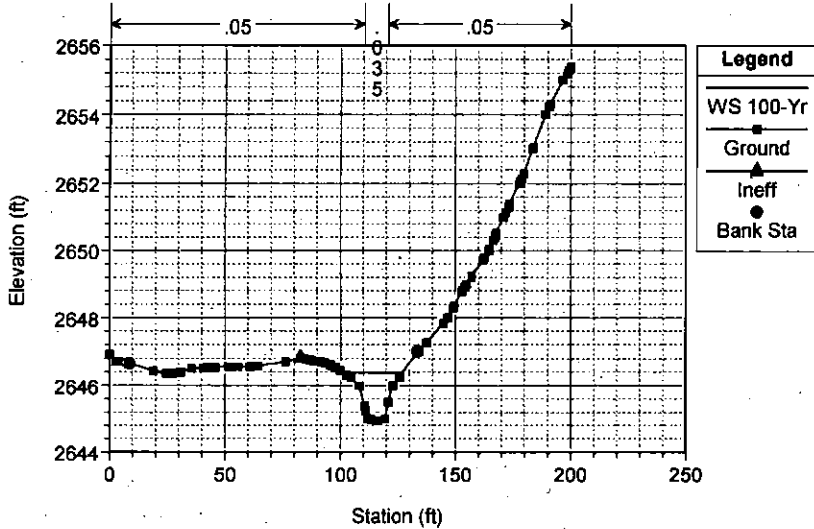


Storyrock Master. Plan: Storyrock Master Ex

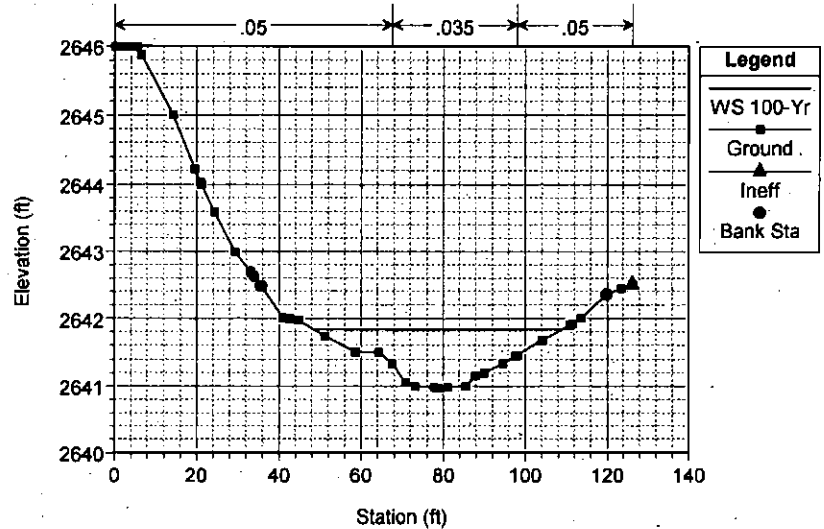
WASH215 WASH215



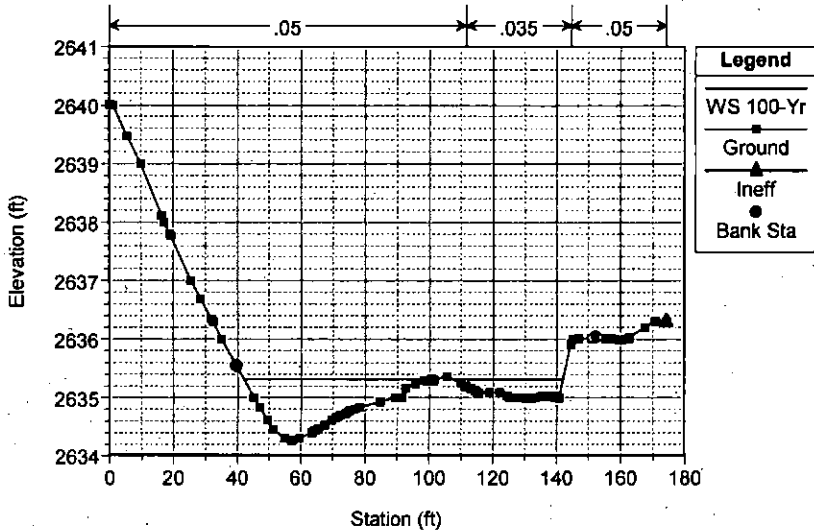
Storyrock Master Plan: Storyrock Master Ex
 River = WASH85 Reach = WASH85 RS = 2263.66



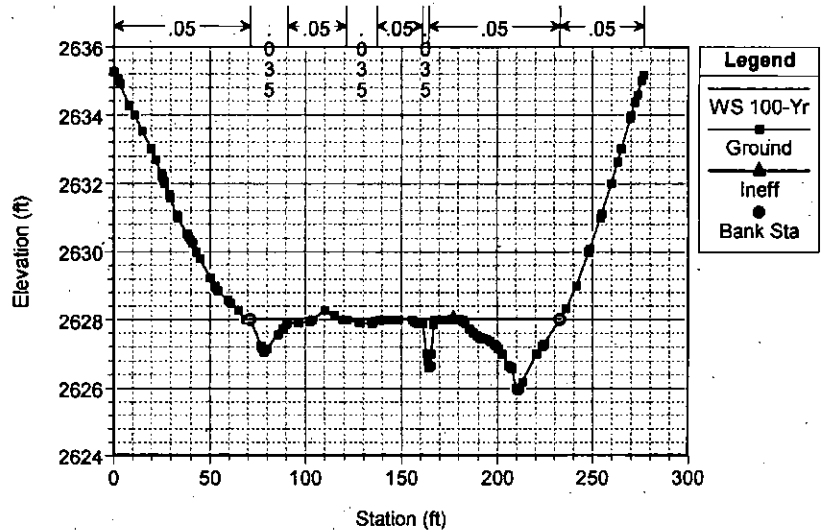
Storyrock Master Plan: Storyrock Master Ex
 River = WASH85 Reach = WASH85 RS = 2114.75

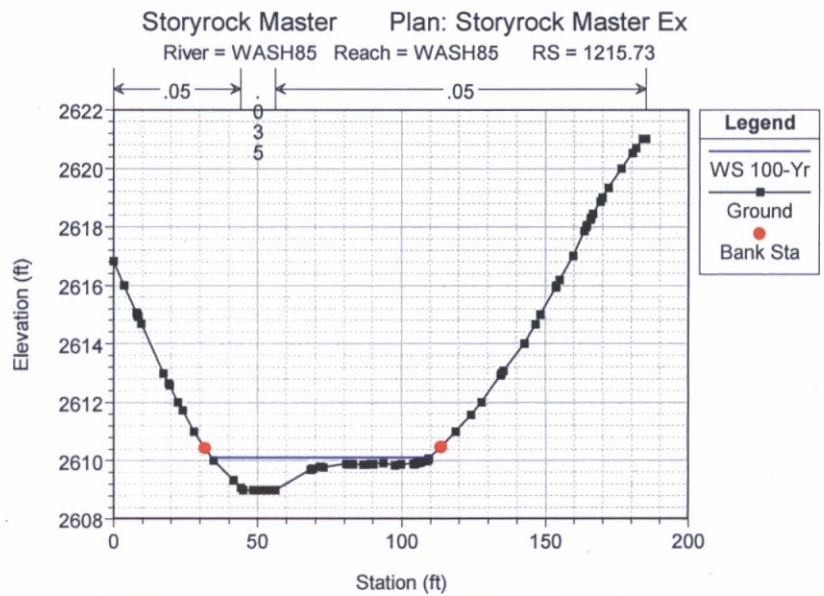
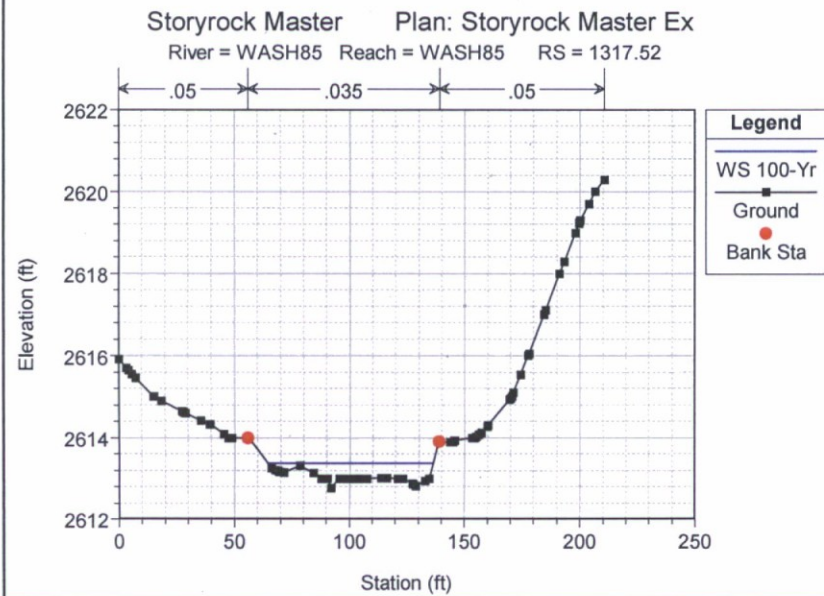
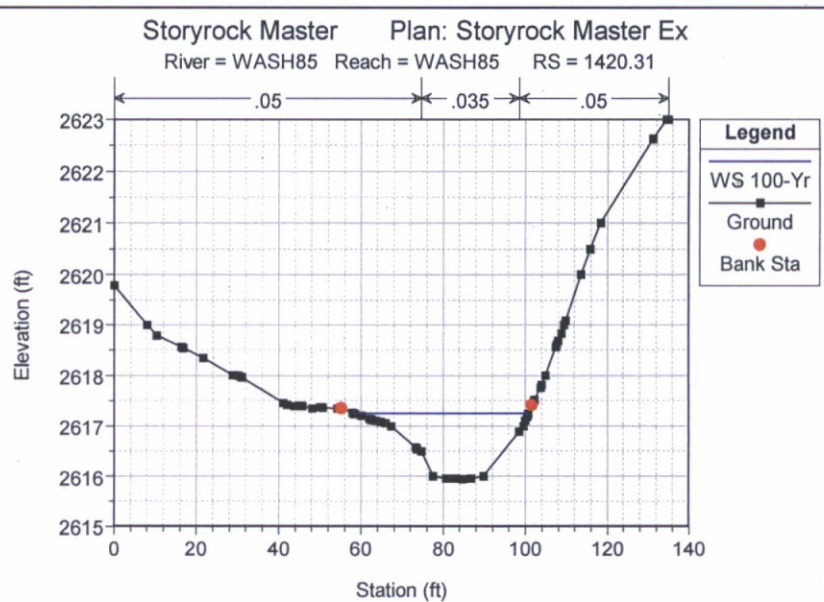
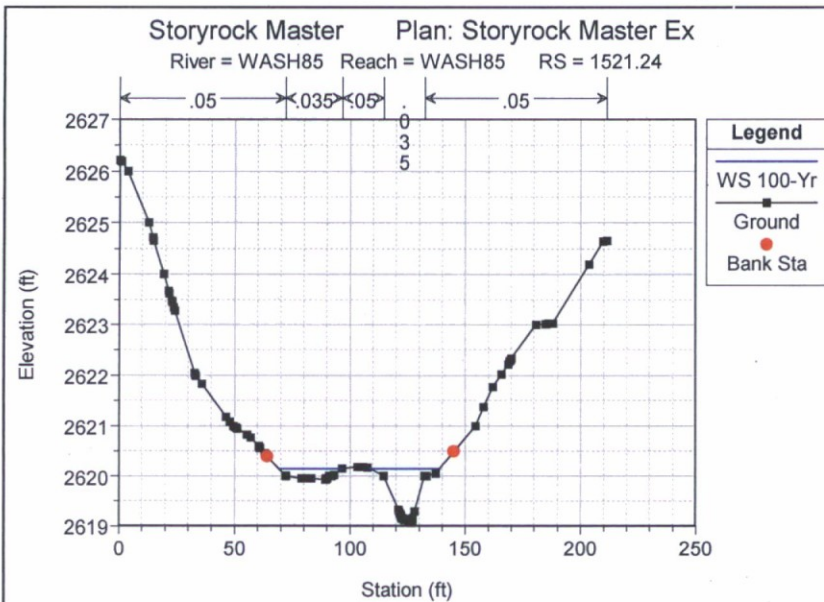


Storyrock Master Plan: Storyrock Master Ex
 River = WASH85 Reach = WASH85 RS = 1938.62

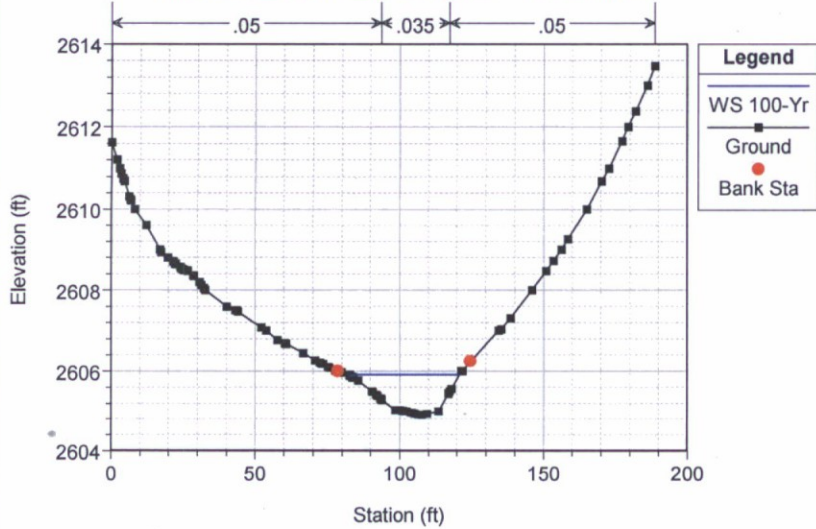


Storyrock Master Plan: Storyrock Master Ex
 River = WASH85 Reach = WASH85 RS = 1721.48

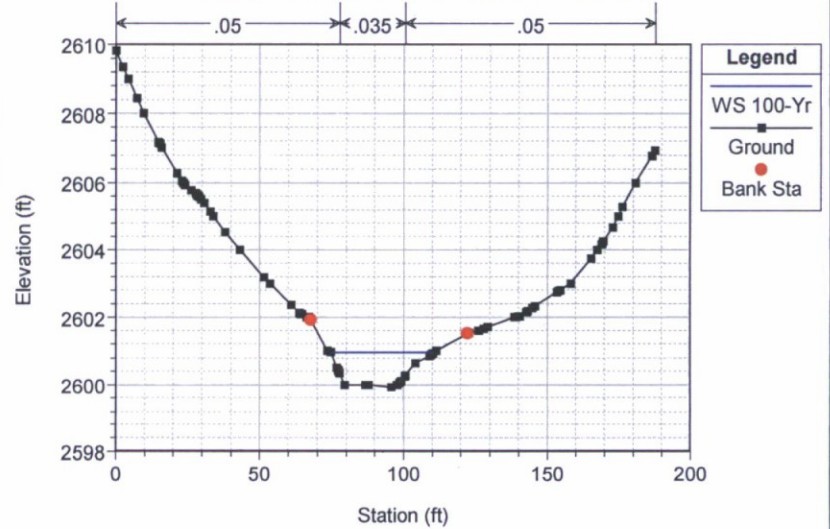




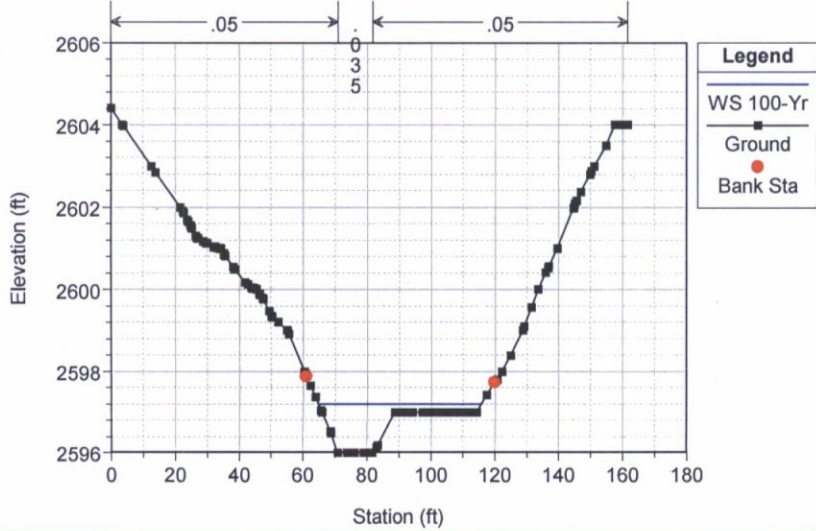
Storyrock Master Plan: Storyrock Master Ex
 River = WASH85 Reach = WASH85 RS = 1087.82



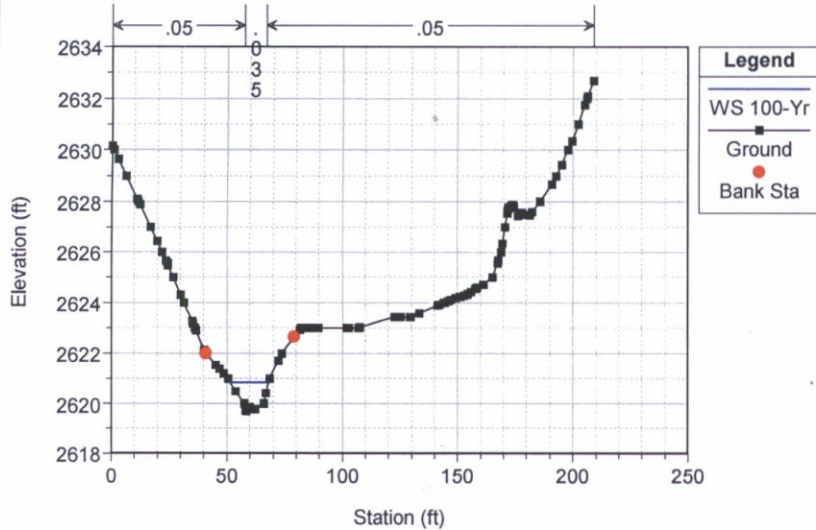
Storyrock Master Plan: Storyrock Master Ex
 River = WASH85 Reach = WASH85 RS = 933.1



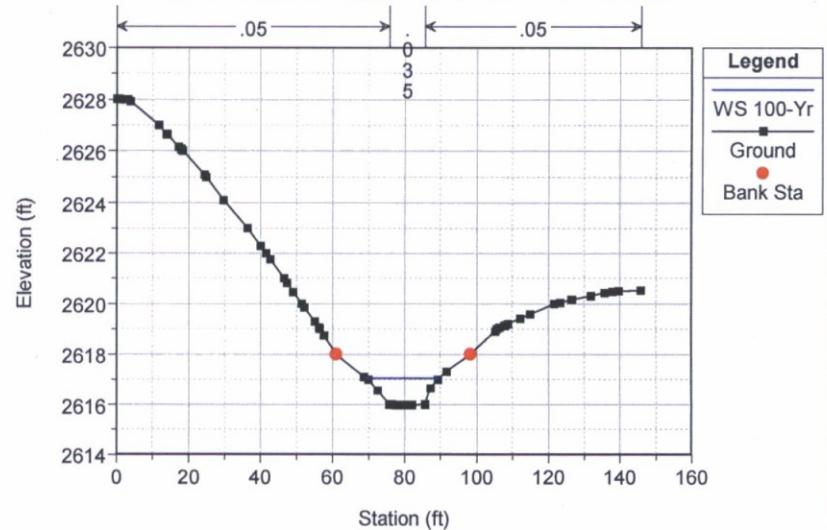
Storyrock Master Plan: Storyrock Master Ex
 River = WASH85 Reach = WASH85 RS = 814.16



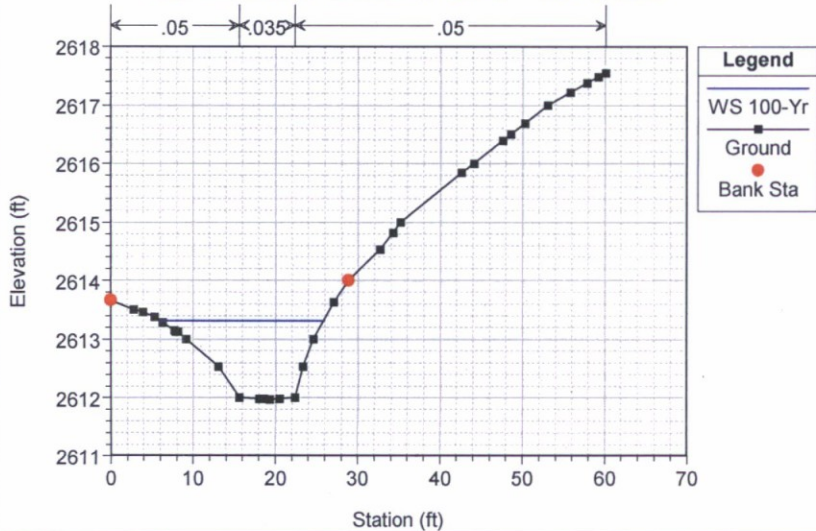
Storyrock Master Plan: Storyrock Master Ex
 River = WASH160 Reach = WASH160 RS = 2694.7



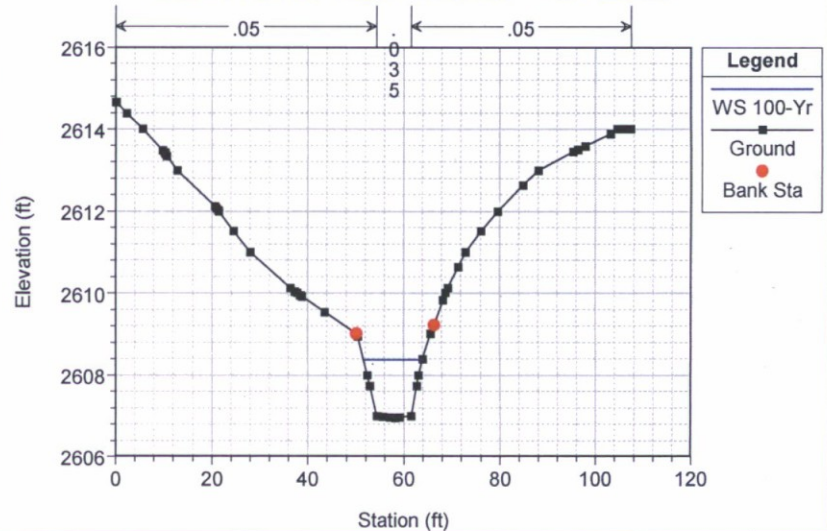
Storyrock Master Plan: Storyrock Master Ex
 River = WASH160 Reach = WASH160 RS = 2546.38

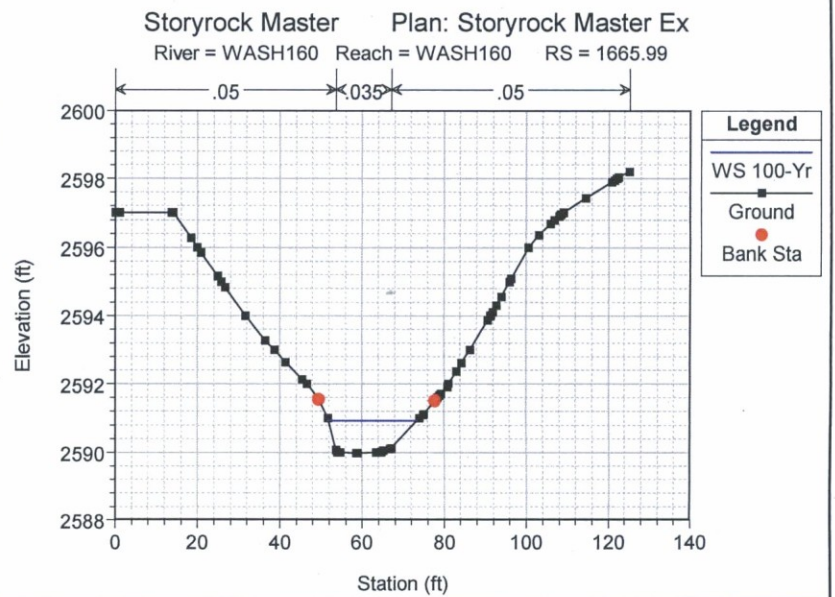
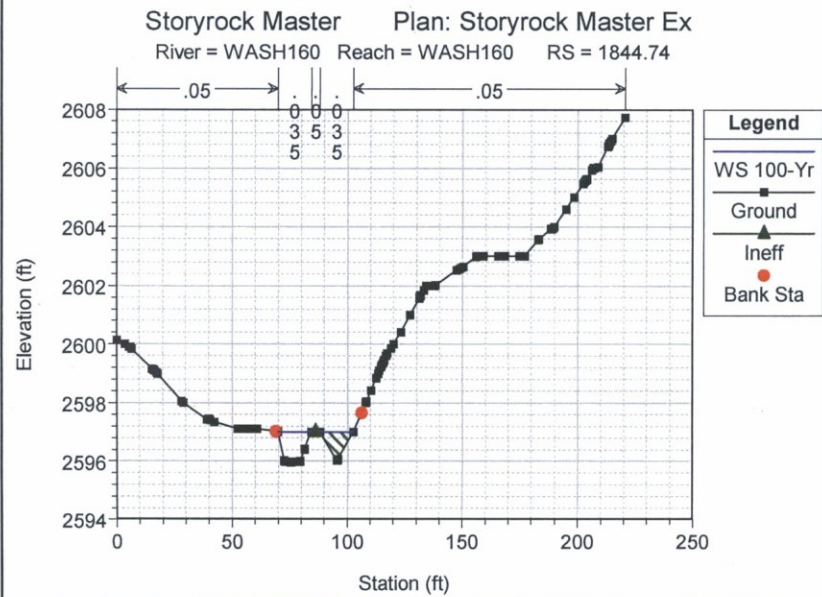
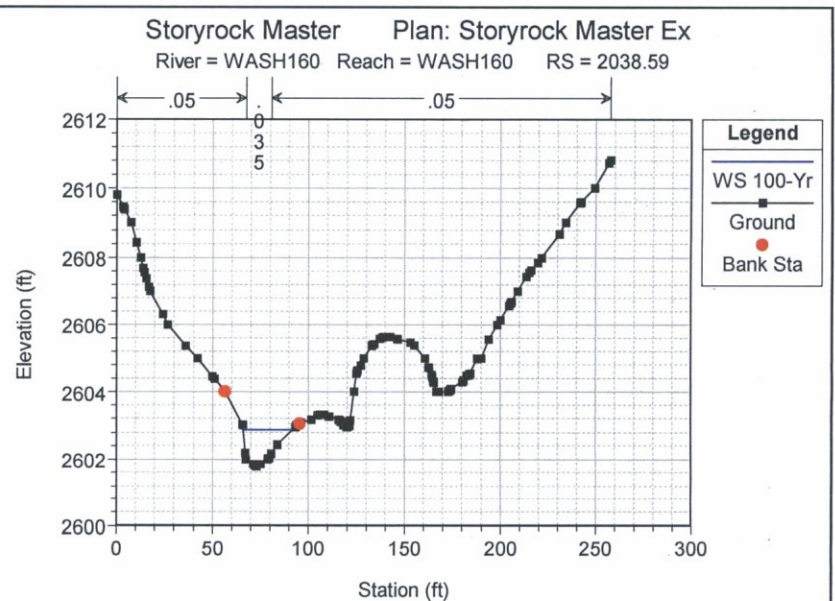
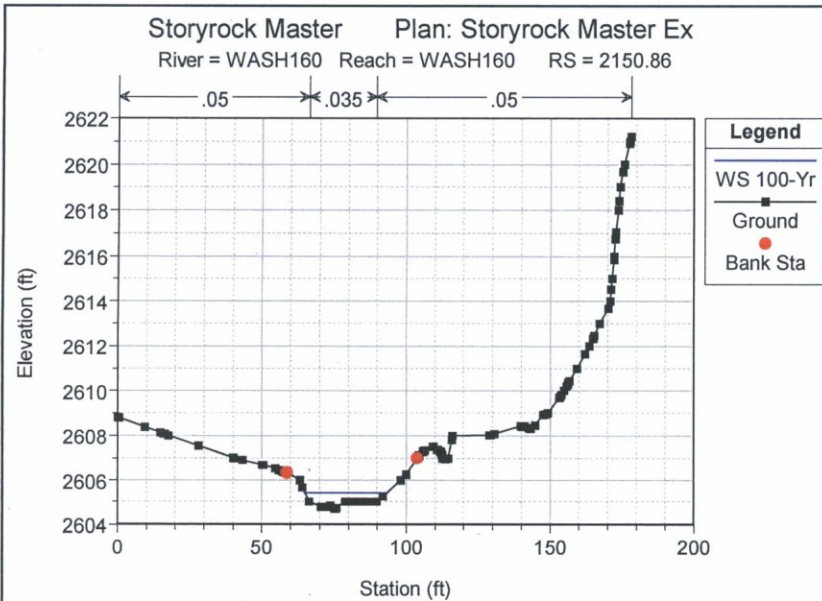


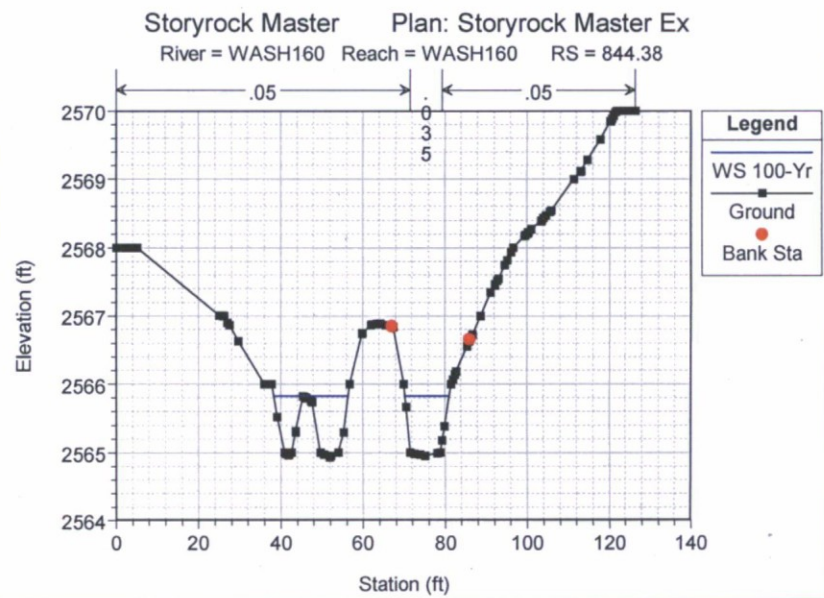
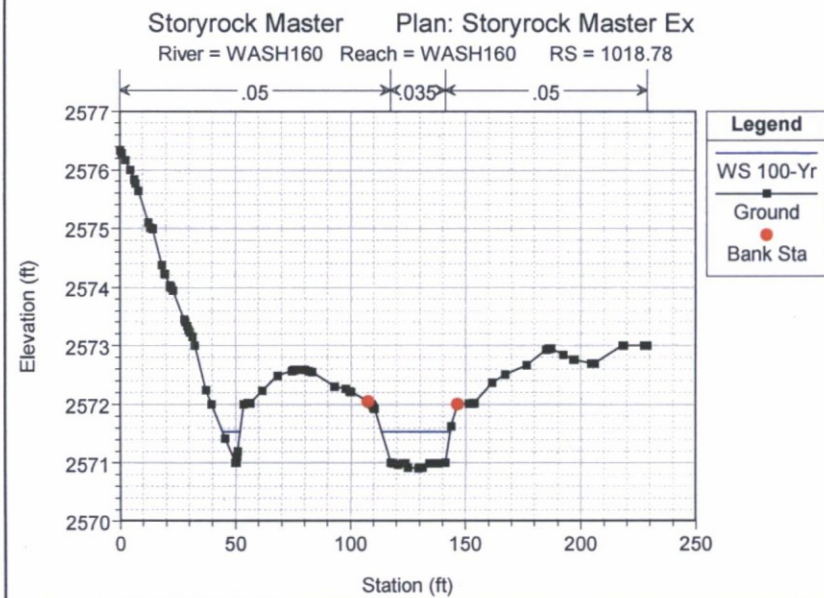
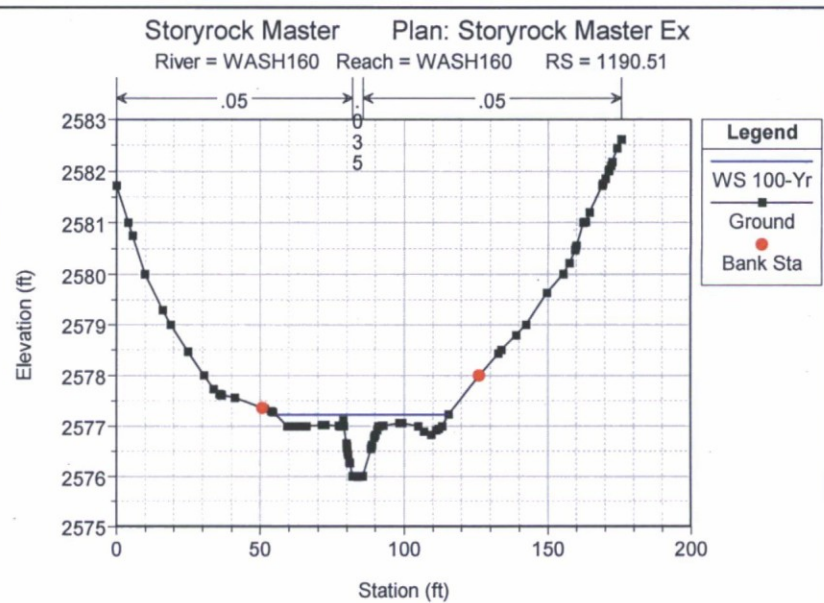
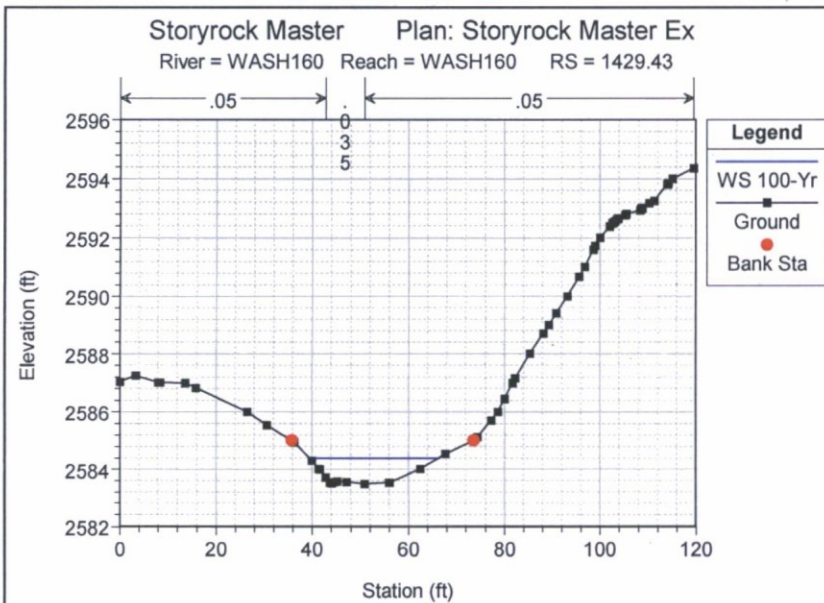
Storyrock Master Plan: Storyrock Master Ex
 River = WASH160 Reach = WASH160 RS = 2403.73

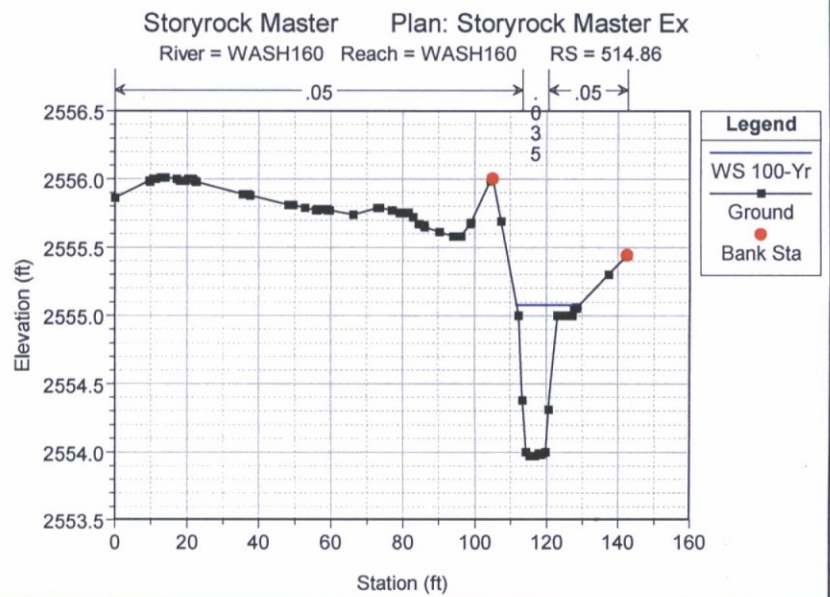
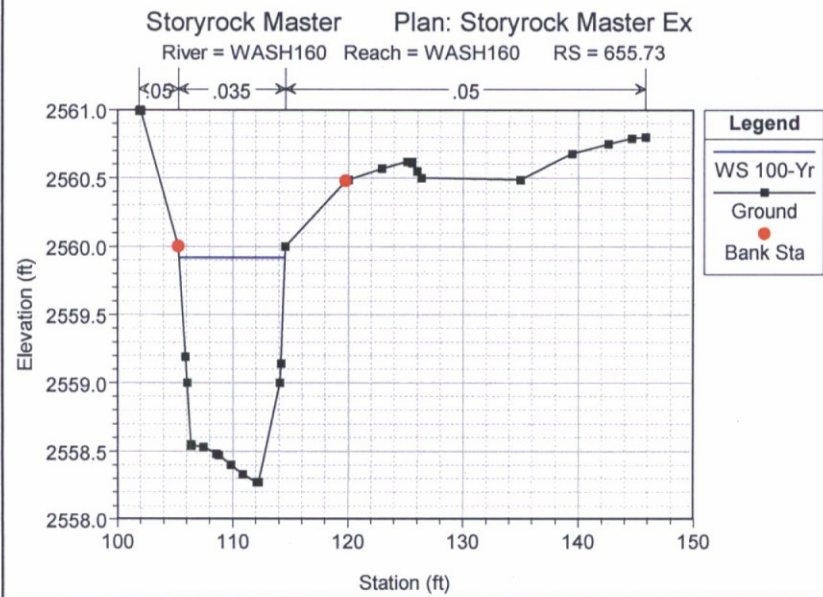
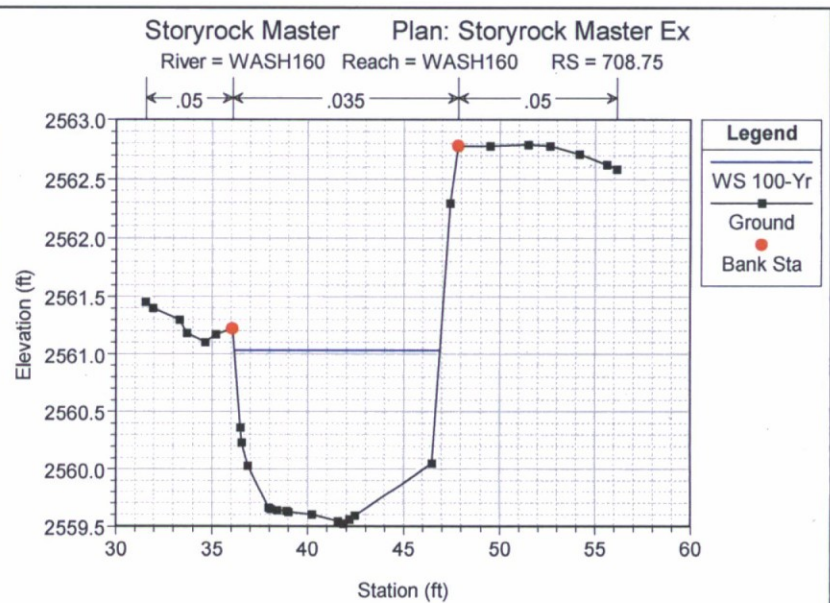
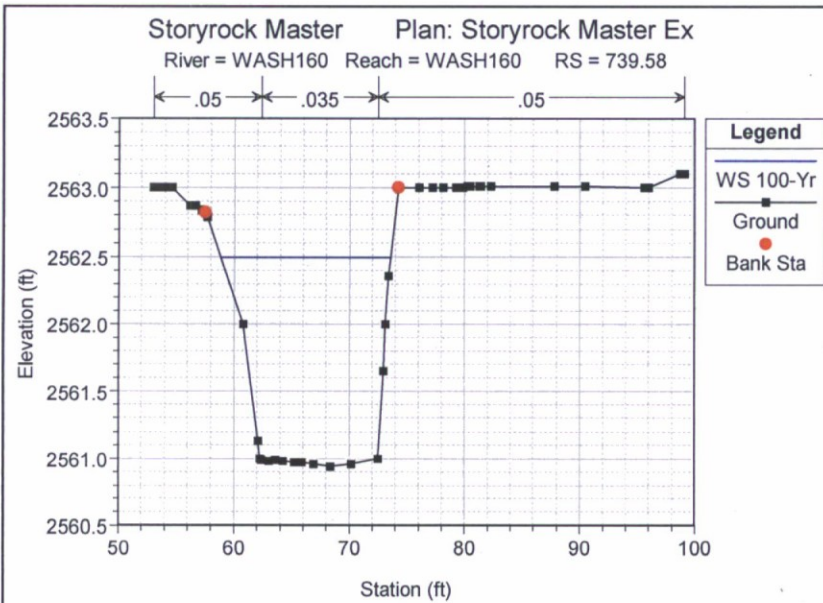


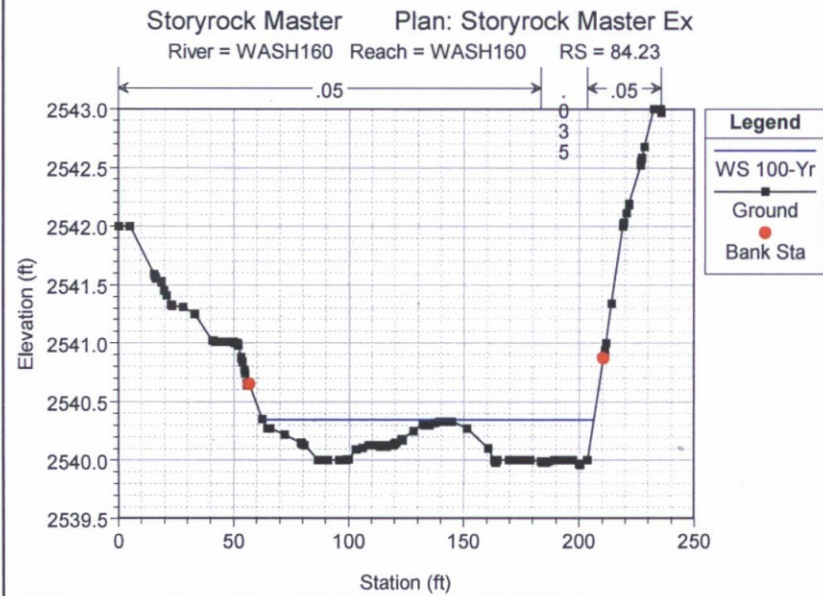
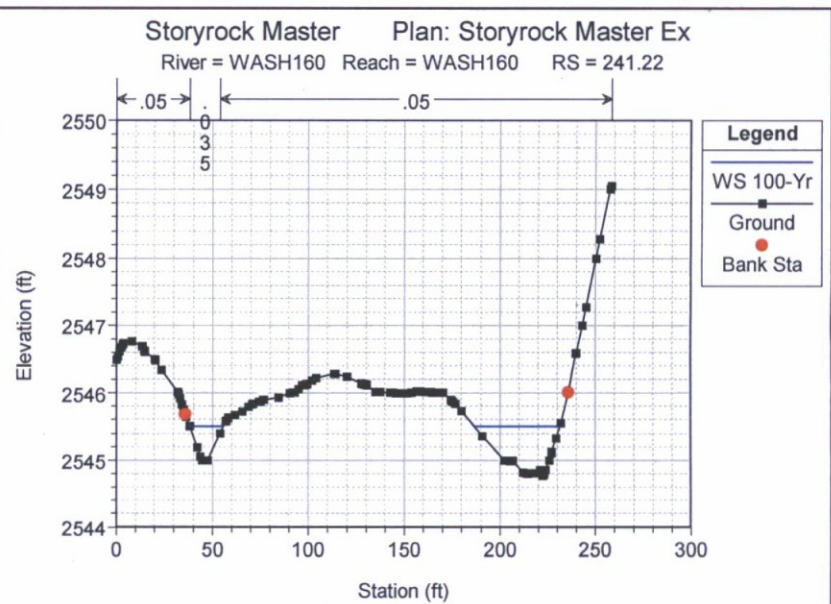
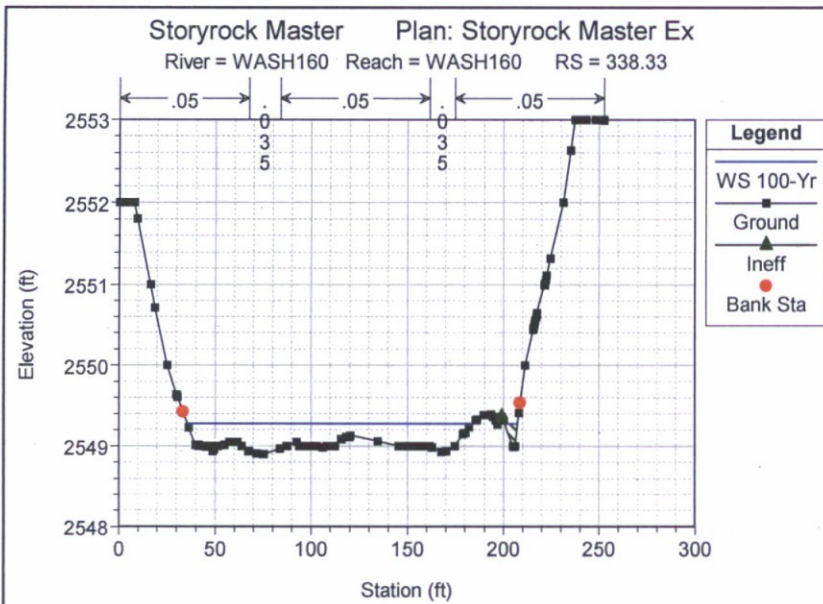
Storyrock Master Plan: Storyrock Master Ex
 River = WASH160 Reach = WASH160 RS = 2236.32



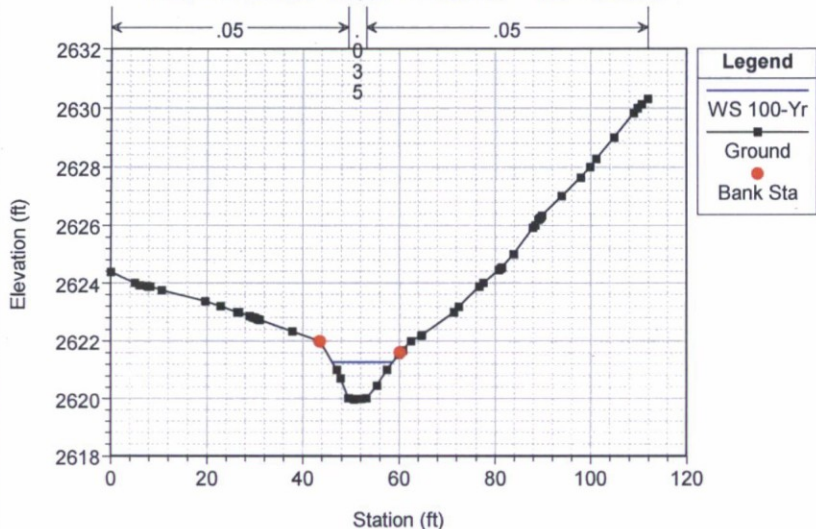




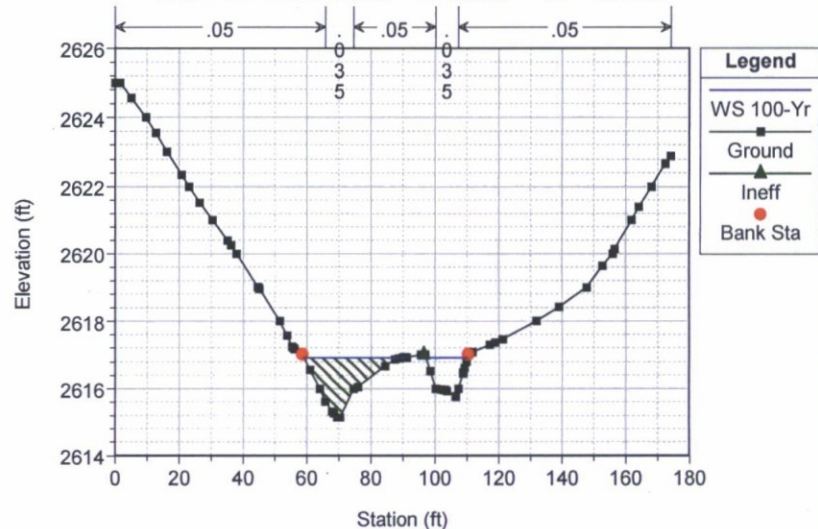




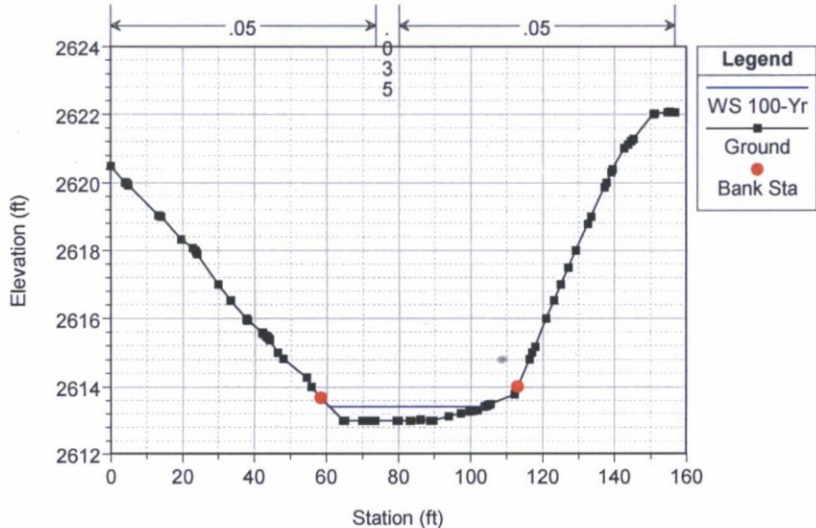
Storyrock Master Plan: Storyrock Master Ex
 River = WASH180 Reach = WASH180 RS = 2258.25



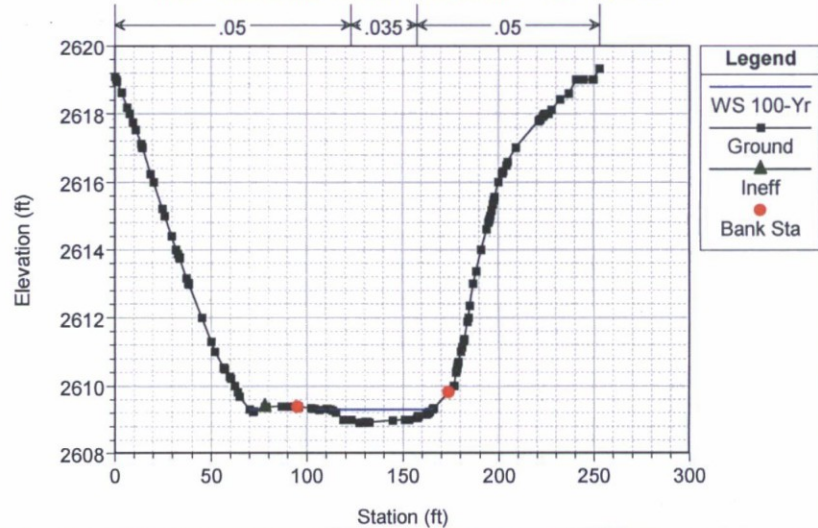
Storyrock Master Plan: Storyrock Master Ex
 River = WASH180 Reach = WASH180 RS = 2081.05



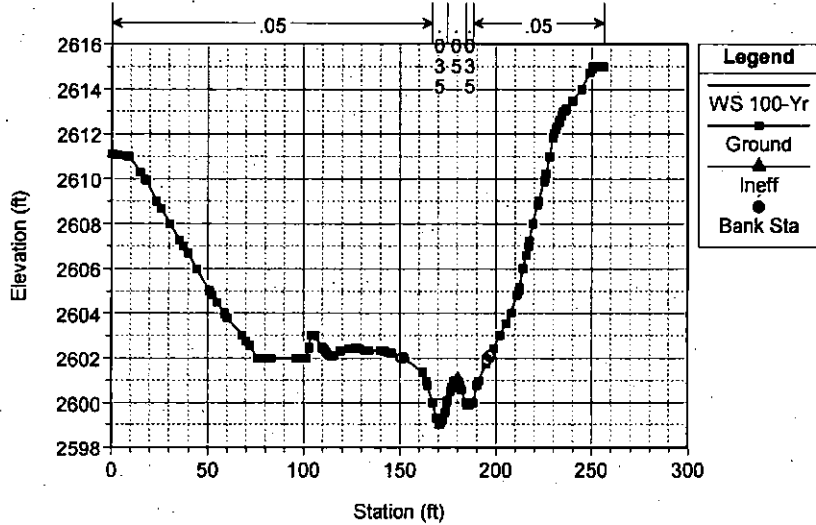
Storyrock Master Plan: Storyrock Master Ex
 River = WASH180 Reach = WASH180 RS = 1966.66



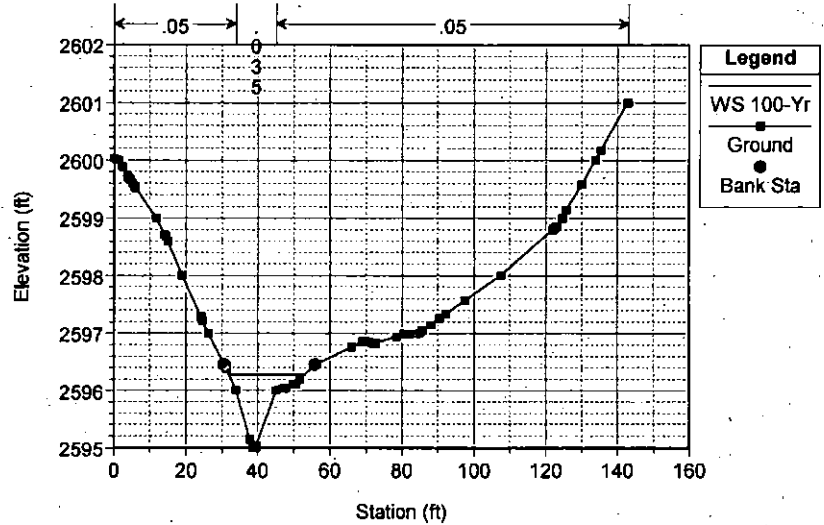
Storyrock Master Plan: Storyrock Master Ex
 River = WASH180 Reach = WASH180 RS = 1847.09



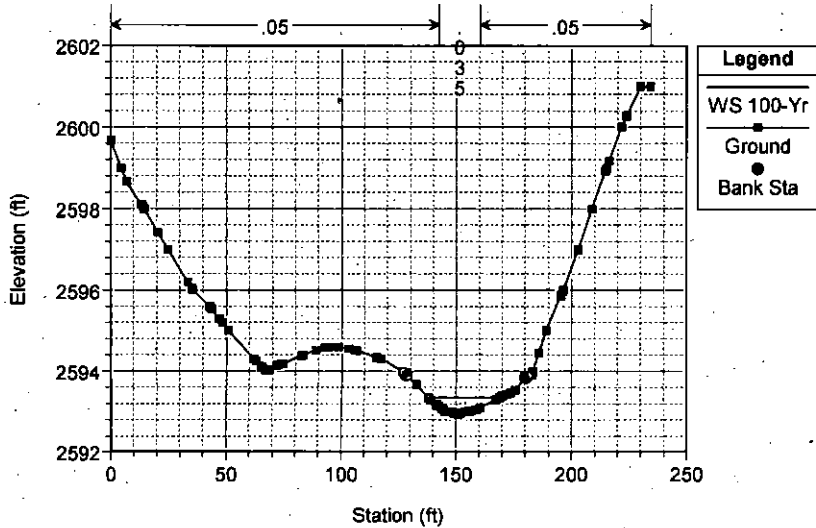
Storyrock Master Plan: Storyrock Master Ex
 River = WASH180 Reach = WASH180 RS = 1641.95



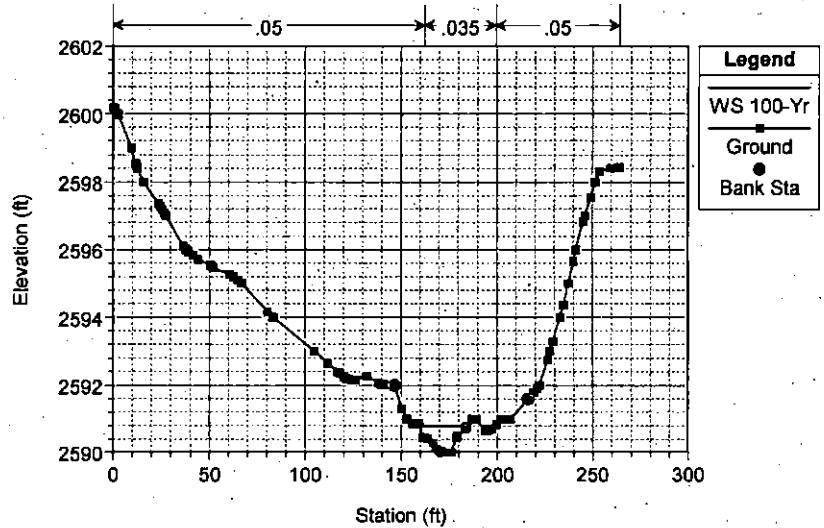
Storyrock Master Plan: Storyrock Master Ex
 River = WASH180 Reach = WASH180 RS = 1484.69

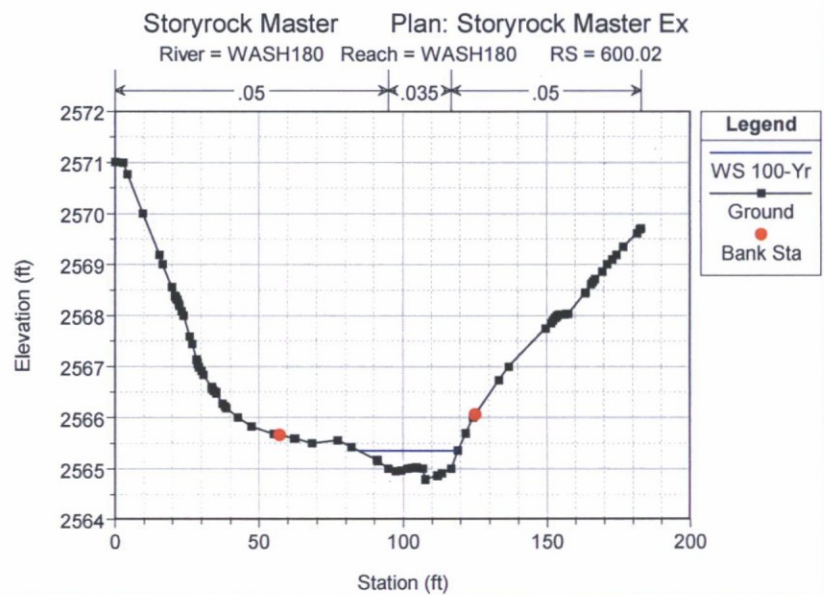
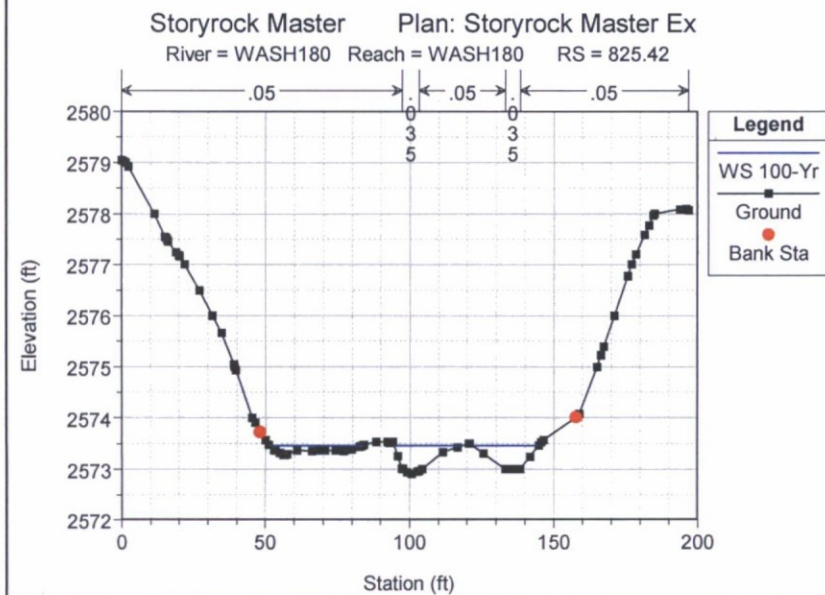
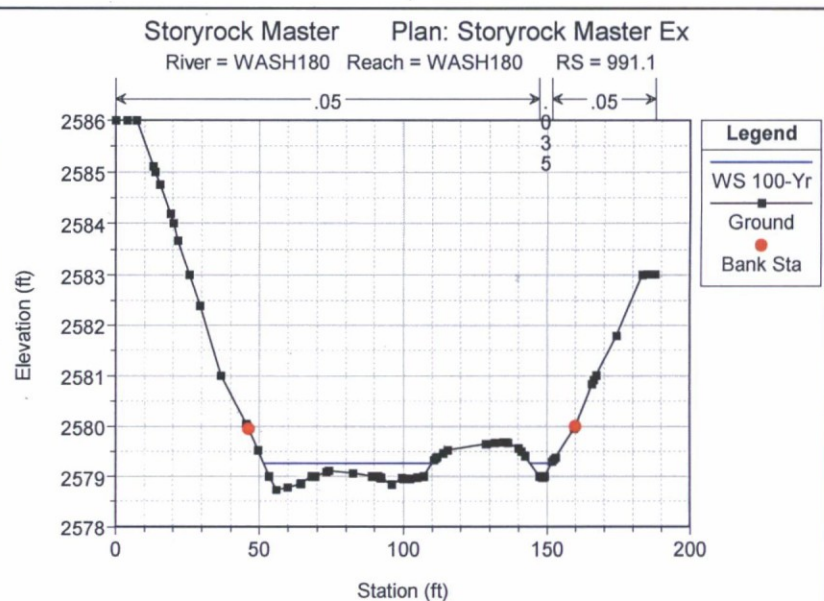
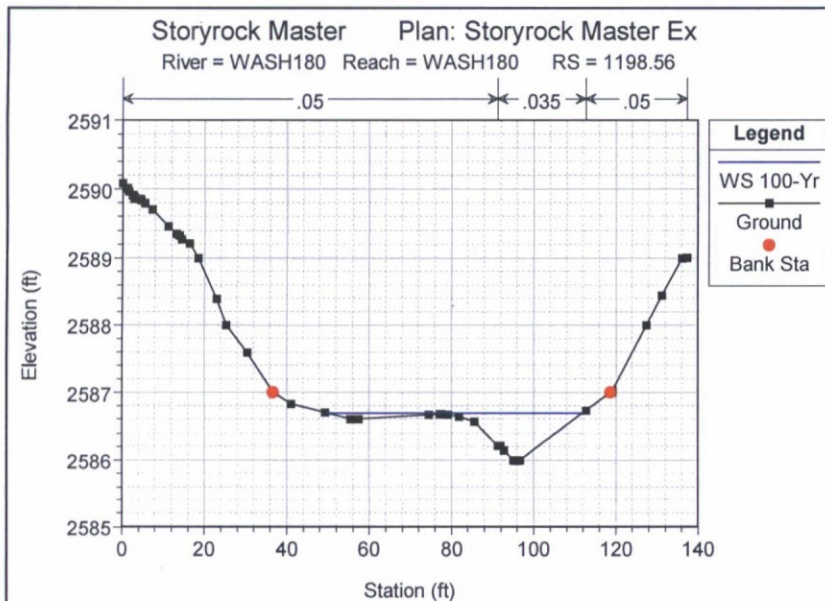


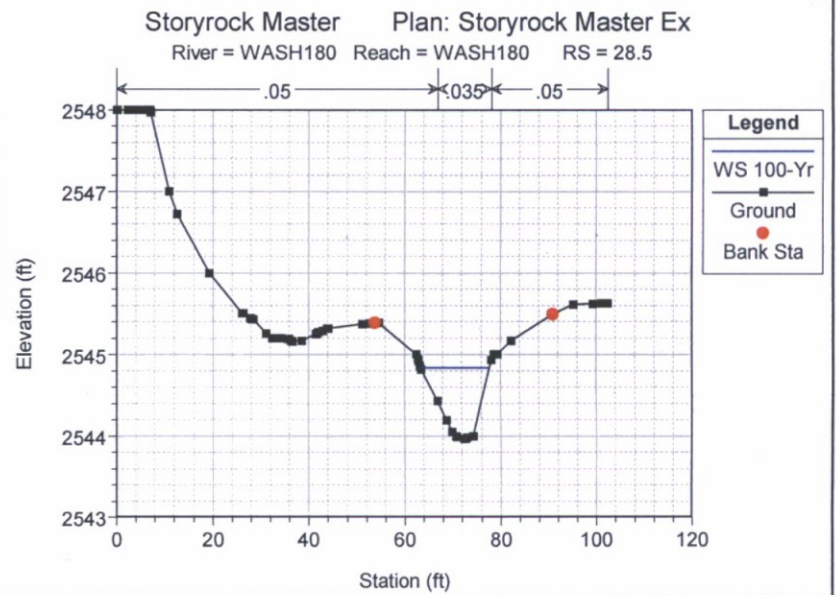
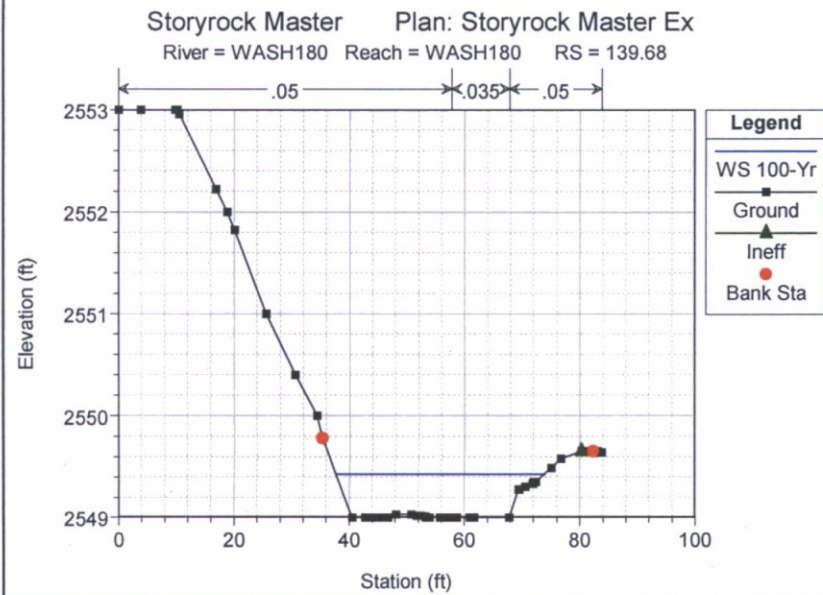
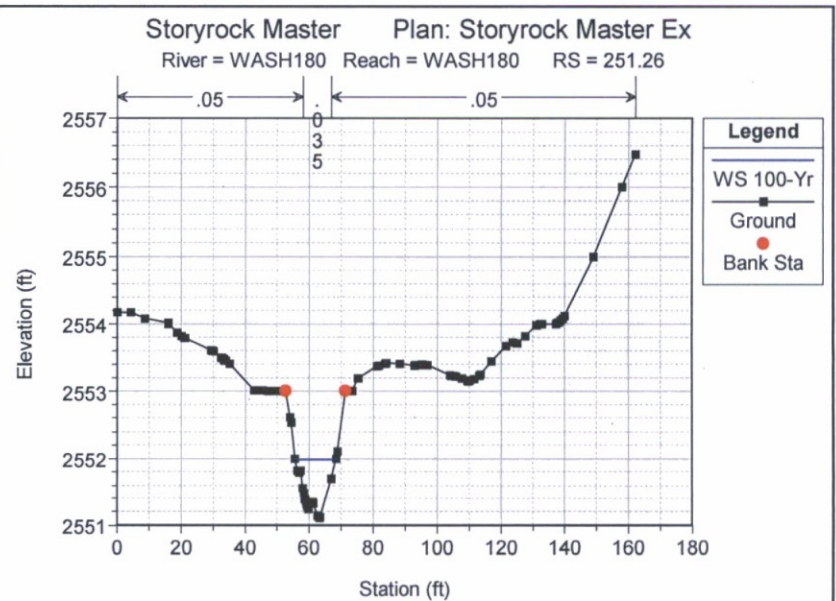
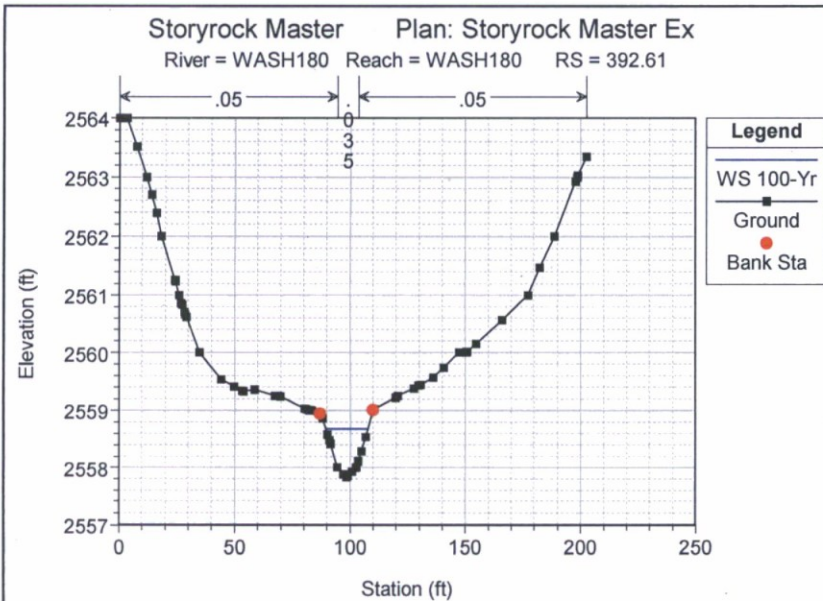
Storyrock Master Plan: Storyrock Master Ex
 River = WASH180 Reach = WASH180 RS = 1403.09



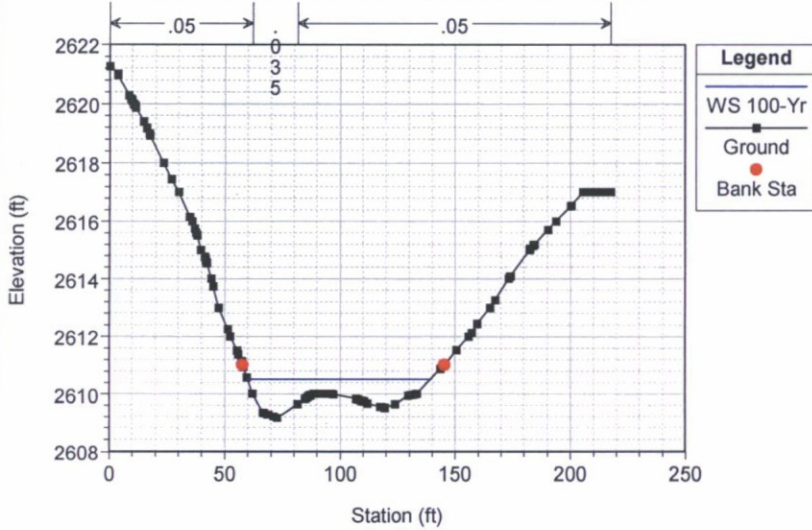
Storyrock Master Plan: Storyrock Master Ex
 River = WASH180 Reach = WASH180 RS = 1332.58



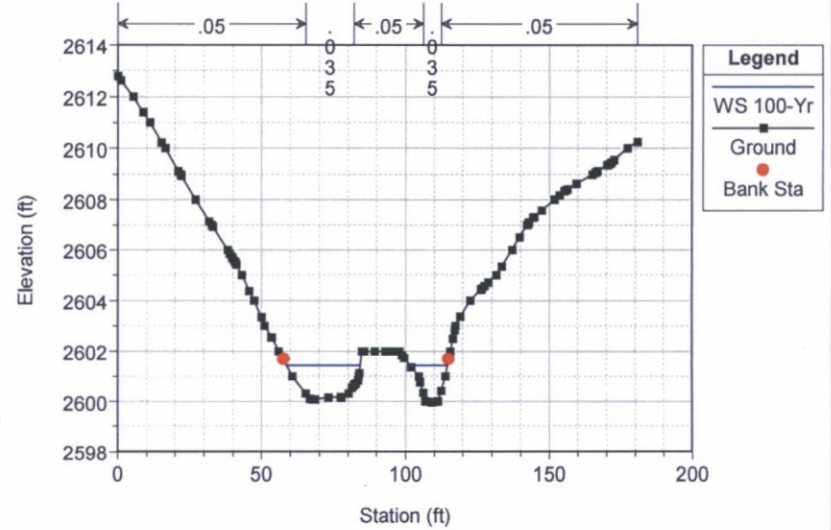




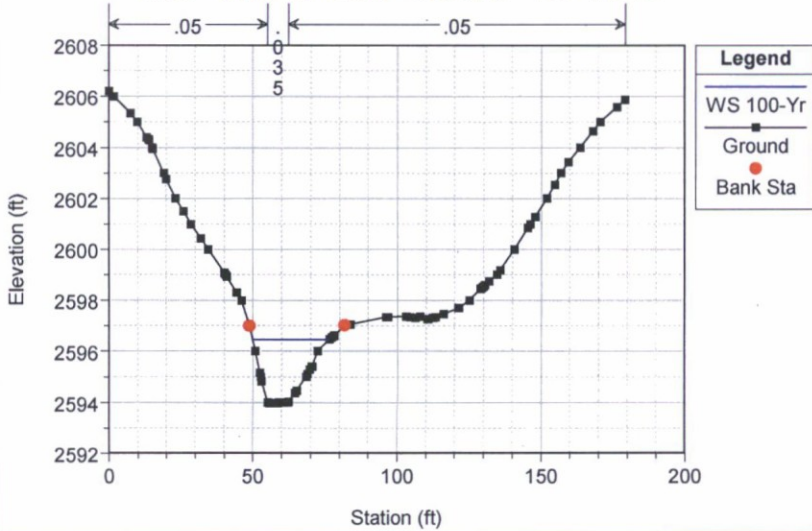
Storyrock Master Plan: Storyrock Master Ex
 River = WASH215 Reach = WASH215 RS = 1770.11



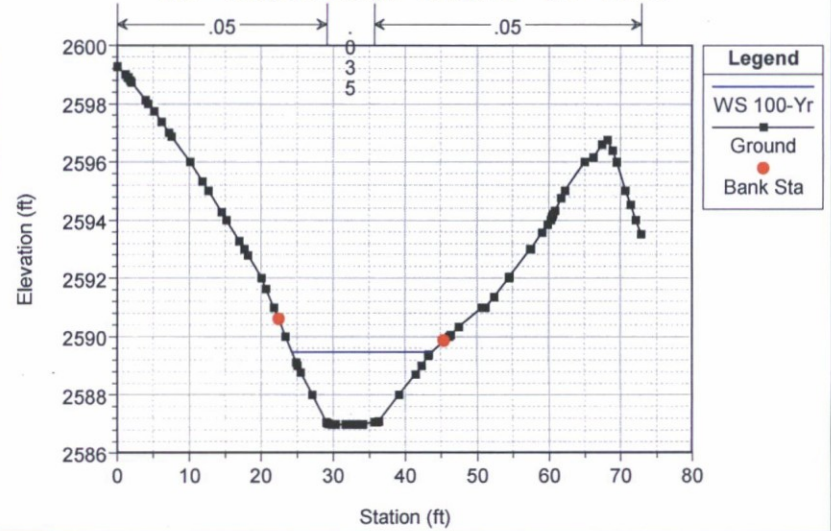
Storyrock Master Plan: Storyrock Master Ex
 River = WASH215 Reach = WASH215 RS = 1570.3



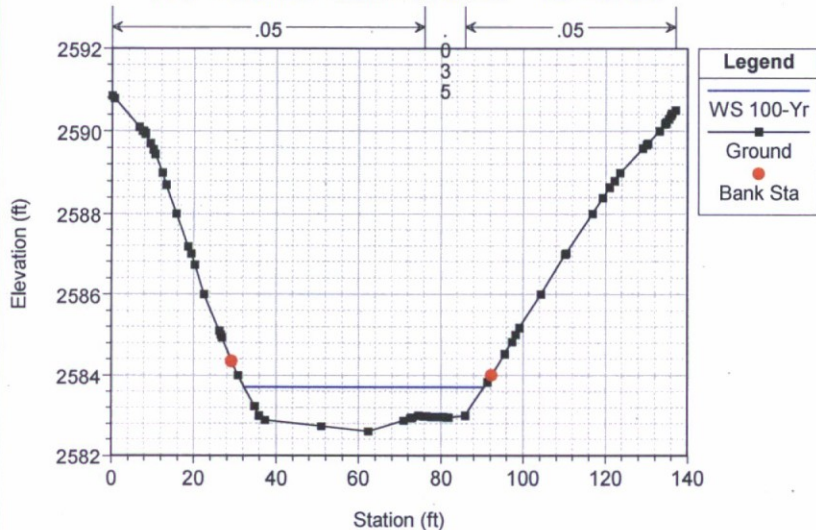
Storyrock Master Plan: Storyrock Master Ex
 River = WASH215 Reach = WASH215 RS = 1411.45



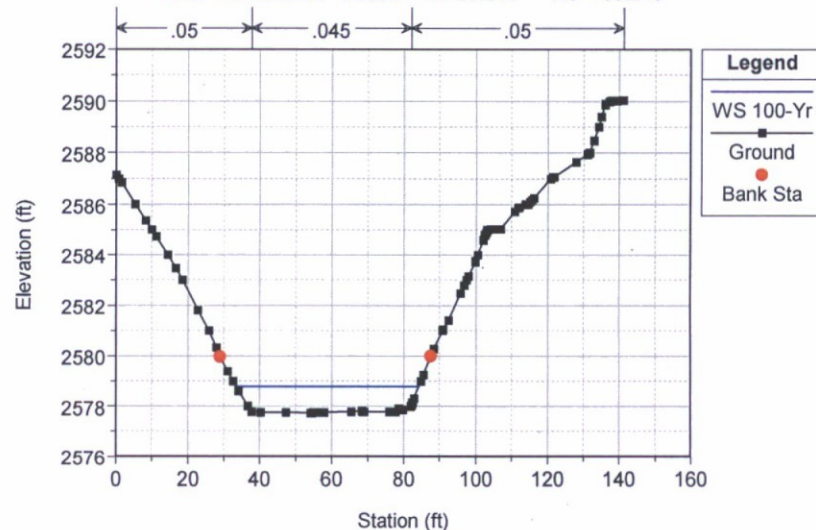
Storyrock Master Plan: Storyrock Master Ex
 River = WASH215 Reach = WASH215 RS = 1216.32



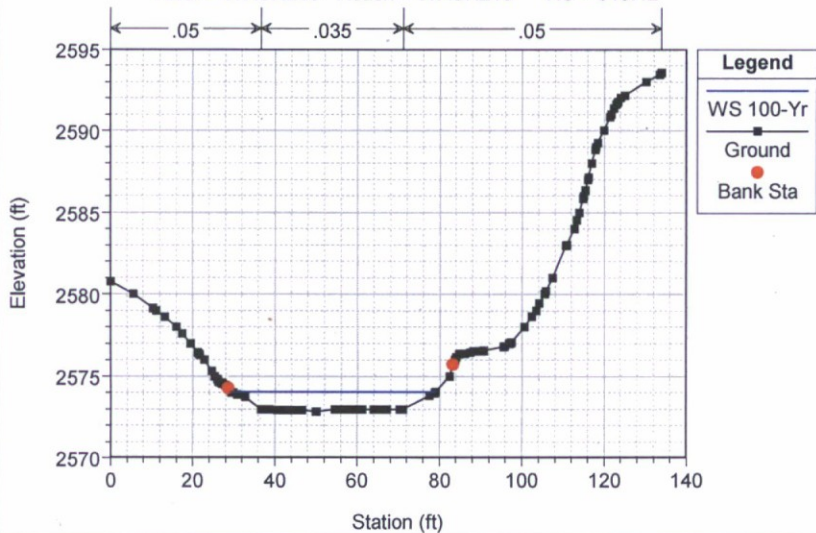
Storyrock Master Plan: Storyrock Master Ex
 River = WASH215 Reach = WASH215 RS = 1071.77



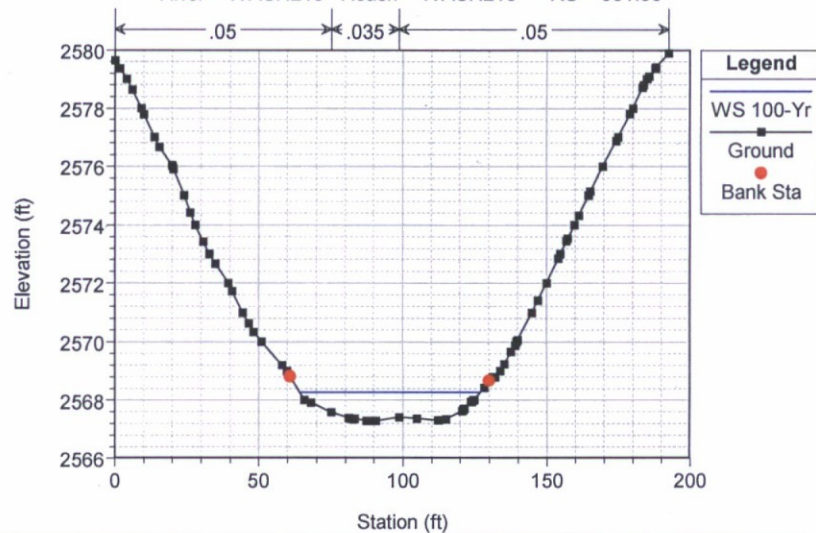
Storyrock Master Plan: Storyrock Master Ex
 River = WASH215 Reach = WASH215 RS = 952.43

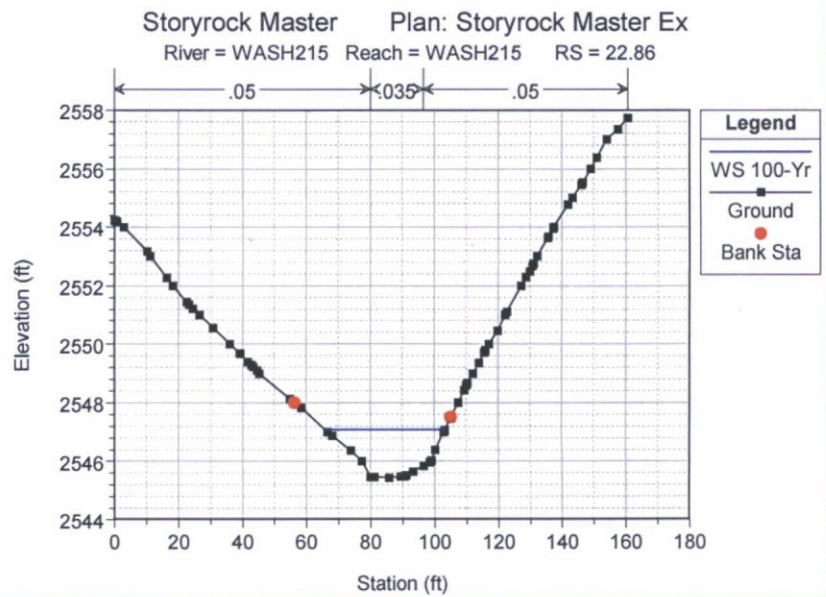
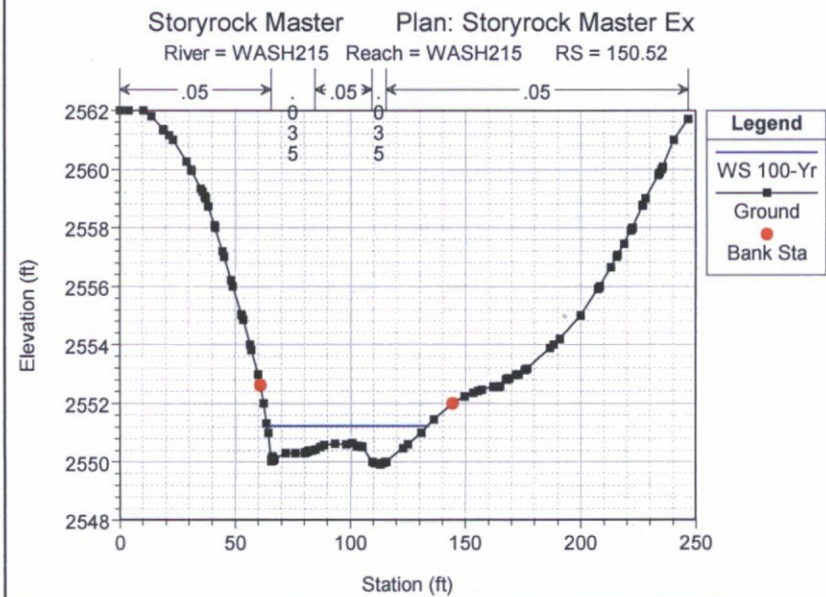
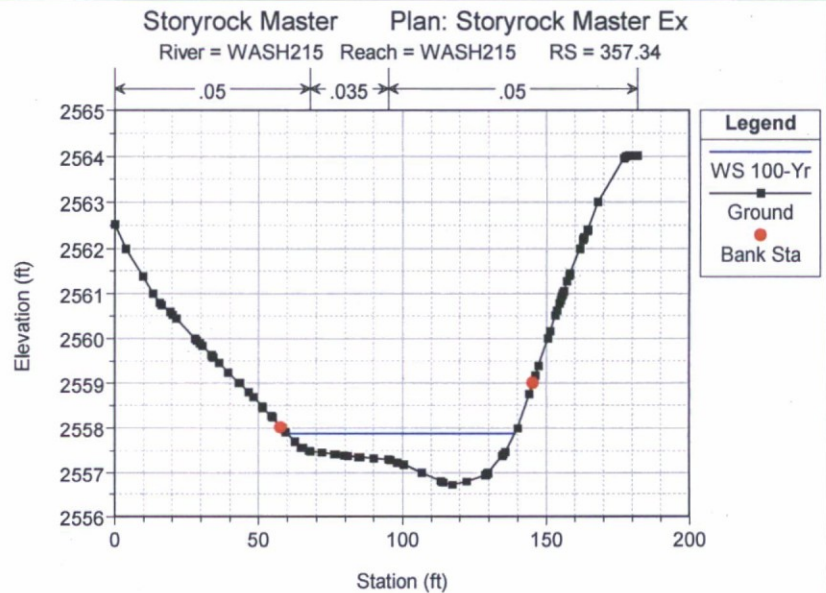
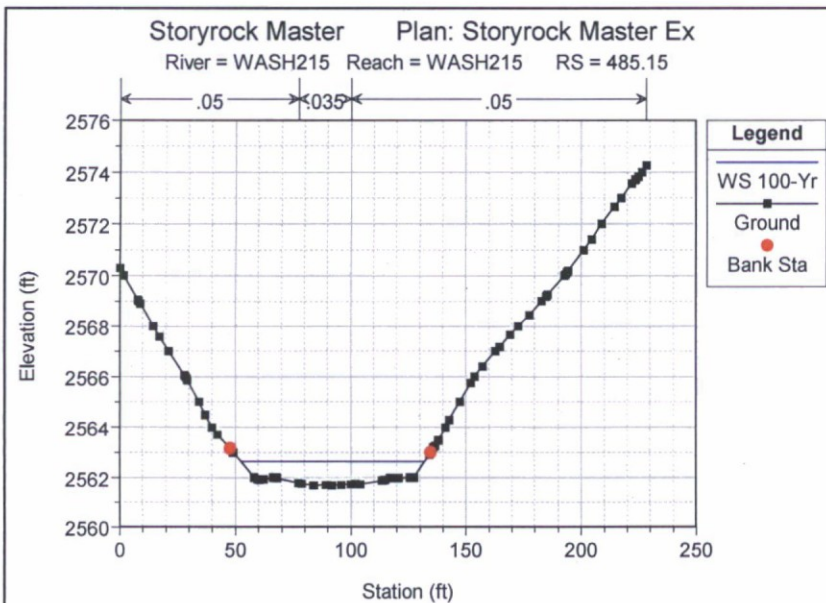


Storyrock Master Plan: Storyrock Master Ex
 River = WASH215 Reach = WASH215 RS = 819.42



Storyrock Master Plan: Storyrock Master Ex
 River = WASH215 Reach = WASH215 RS = 661.36





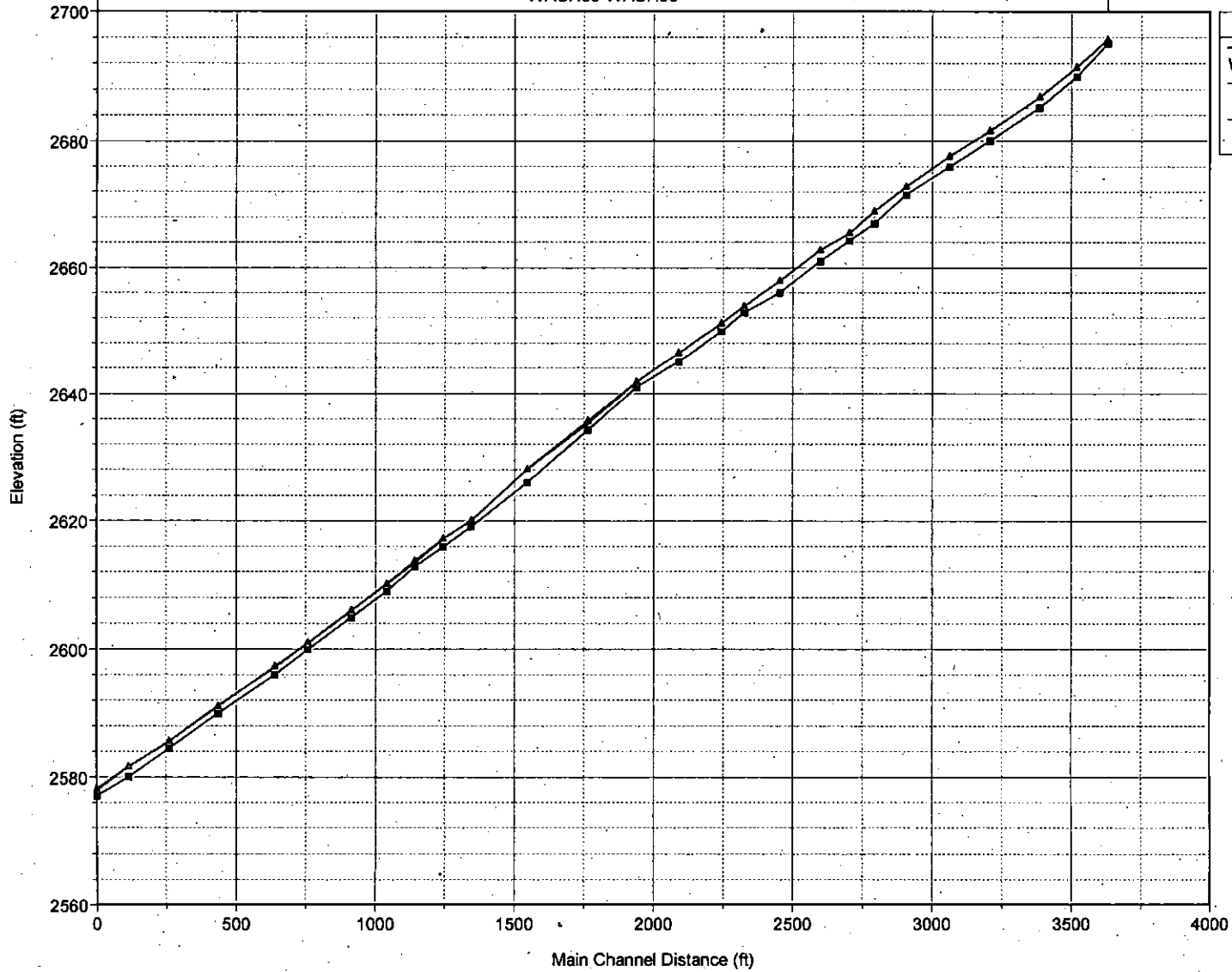
SITE BOUNDARY CROSS SECTION

Profile	Q Total	Min Ch Elevation	W.S. Elevation	Crit W.S.	E.G. Elevation	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft ²)	(sq ft)	(ft)	(ft)
WASH85	2263.66	130.00	2644.94	2646.39	2646.62	2647.13	0.027938	6.92	18.77	34.36
WASH85	1087.82	156.00	2604.91	2605.90	2606.10	2606.56	0.037294	6.55	22.82	37.92
WASH85	1215.73	156.00	2609.00	2610.11	2610.12	2610.37	0.024461	4.11	37.99	75.86
WASH85	1317.52	156.00	2613.77	2613.77	2613.77	2614.07	0.111026	6.69	23.33	71.72
WASH85	1420.31	156.00	2615.94	2617.25	2617.27	2617.64	0.016565	5.02	31.08	42.79
WASH85	1521.24	156.00	2619.06	2620.44	2620.44	2621.31	0.106785	8.64	18.05	58.86
WASH85	1721.48	156.00	2625.94	2628.04	2628.04	2628.13	0.016134	2.36	66.09	151.35
WASH85	1936.62	156.00	2634.26	2635.31	2635.33	2635.55	0.045016	3.91	39.90	94.34
WASH85	2114.75	156.00	2641.97	2641.97	2641.97	2642.29	0.035912	5.41	28.85	60.34
WASH85	2263.66	130.00	2644.94	2646.39	2646.62	2647.13	0.027938	6.92	18.77	34.36
WASH85	933.1	100-Yr	2599.93	2600.95	2601.11	2601.54	0.028280	6.15	25.37	35.68
WASH85	814.16	156.00	2596.00	2597.20	2597.33	2597.73	0.036231	6.88	26.51	50.93
WASH85	2694.7	79.00	2619.97	2620.98	2621.03	2621.52	0.035097	6.56	12.04	16.71
WASH85	2546.38	79.00	2615.04	2617.04	2617.08	2617.46	0.021204	5.20	15.19	20.13
WASH85	2409.73	79.00	2611.97	2613.32	2613.28	2613.69	0.029571	4.87	16.22	19.88
WASH85	2236.32	79.00	2608.69	2608.37	2608.92	2609.92	0.027301	5.94	13.31	12.77
WASH85	2150.86	79.00	2604.69	2605.45	2605.59	2605.95	0.044906	5.68	13.92	28.66
WASH85	2038.59	79.00	2601.77	2602.88	2602.88	2603.21	0.018211	4.66	16.97	25.62
WASH85	1844.74	79.00	2595.97	2597.00	2597.02	2597.84	0.046021	7.37	10.72	28.81
WASH85	1665.99	79.00	2589.97	2590.93	2590.93	2591.31	0.026893	4.94	16.00	21.79
WASH85	1429.43	79.00	2583.51	2584.40	2584.40	2584.72	0.028489	4.58	17.24	27.08
WASH85	1190.51	79.00	2575.99	2577.23	2577.26	2577.45	0.032863	3.76	21.02	60.29
WASH85	1018.78	79.00	2570.91	2571.53	2571.60	2571.89	0.032270	4.90	17.30	37.54
WASH85	844.38	79.00	2564.95	2565.88	2565.88	2566.19	0.036761	5.56	18.26	29.04
WASH85	739.58	79.00	2560.94	2562.16	2562.16	2562.78	0.011687	4.33	17.50	14.72
WASH85	708.75	79.00	2558.27	2561.03	2560.97	2561.58	0.017129	5.94	13.30	10.72
WASH85	655.73	79.00	2558.92	2559.92	2559.92	2560.59	0.020115	6.56	12.04	17.62
WASH85	514.86	79.00	2553.97	2555.08	2555.43	2556.17	0.052905	8.37	9.44	1.01
WASH85	338.33	79.00	2549.90	2549.90	2549.90	2549.90	0.023075	2.09	37.76	154.84
WASH85	241.22	79.00	2544.77	2545.51	2545.45	2545.66	0.030301	3.08	25.64	62.80
WASH85	84.23	79.00	2539.96	2540.35	2540.33	2540.44	0.036395	2.43	32.49	144.05
WASH85	2258.25	45.00	2619.97	2621.16	2621.16	2621.57	0.024636	4.36	10.32	12.57
WASH85	2081.05	45.00	2615.14	2616.93	2616.87	2617.24	0.024075	4.51	9.98	44.58
WASH85	1966.66	45.00	2613.00	2613.41	2613.41	2613.57	0.043671	3.24	15.08	62.79
WASH85	1847.09	45.00	2608.91	2609.31	2609.32	2609.45	0.027865	2.98	16.08	62.79
WASH85	1641.95	45.00	2599.00	2600.18	2600.49	2601.07	0.063967	7.57	5.94	13.78
WASH85	1484.69	100-Yr	2596.00	2596.28	2596.55	2596.55	0.016247	4.16	10.83	21.03
WASH85	1403.09	45.00	2592.93	2593.36	2593.52	2593.86	0.096248	5.63	7.99	31.51
WASH85	1323.58	45.00	2590.80	2590.80	2591.00	2591.00	0.024399	3.60	12.50	31.24
WASH85	1198.56	100-Yr	2585.99	2586.70	2586.75	2586.90	0.039314	3.62	12.43	62.35
WASH85	991.1	100-Yr	2578.74	2579.26	2579.23	2579.36	0.037922	2.48	18.11	66.24
WASH85	825.42	45.00	2572.91	2573.47	2573.47	2573.58	0.032209	2.70	16.66	79.22
WASH85	600.02	45.00	2564.79	2565.35	2565.42	2565.61	0.038887	4.07	11.04	34.50
WASH85	392.61	45.00	2557.84	2558.68	2558.73	2559.02	0.026429	4.65	9.68	18.41
WASH85	251.26	100-Yr	2551.11	2551.99	2552.22	2552.74	0.087712	6.94	6.48	12.72
WASH85	251.26	100-Yr	2551.11	2551.99	2552.22	2552.74	0.087712	6.94	6.48	12.72
WASH85	28.5	45.00	2544.97	2545.83	2545.00	2545.37	0.040087	5.87	7.67	14.40
WASH85	1770.11	100-Yr	2609.17	2610.51	2610.51	2610.89	0.036125	4.96	58.84	79.39
WASH85	1570.3	100-Yr	2599.96	2601.43	2601.68	2602.37	0.050929	7.78	37.55	38.66
WASH85	1411.45	100-Yr	2593.98	2596.45	2597.24	2597.24	0.028426	7.13	40.96	26.39
WASH85	1216.32	292.00	2587.00	2589.48	2589.74	2590.71	0.039572	8.90	32.81	19.55
WASH85	952.43	100-Yr	2577.74	2578.81	2578.86	2579.37	0.036326	5.99	48.75	51.04
WASH85	924.43	100-Yr	2574.82	2574.06	2574.17	2574.70	0.033880	6.43	45.44	49.79
WASH85	661.36	292.00	2567.30	2568.28	2568.40	2568.87	0.040164	6.14	47.55	62.87
WASH85	485.15	292.00	2561.69	2562.65	2562.65	2563.03	0.029014	4.94	59.07	79.63
WASH85	357.34	100-Yr	2556.72	2557.88	2557.97	2558.36	0.047011	5.61	52.07	79.15
WASH85	150.52	100-Yr	2549.92	2551.24	2551.24	2551.66	0.033100	5.18	56.32	69.55
WASH85	222.86	100-Yr	2545.43	2547.08	2547.26	2547.89	0.026135	7.23	40.39	37.62

HEC-RAS Proposed Condition

Storyrock Master Plan: Storyrock Master Encroach

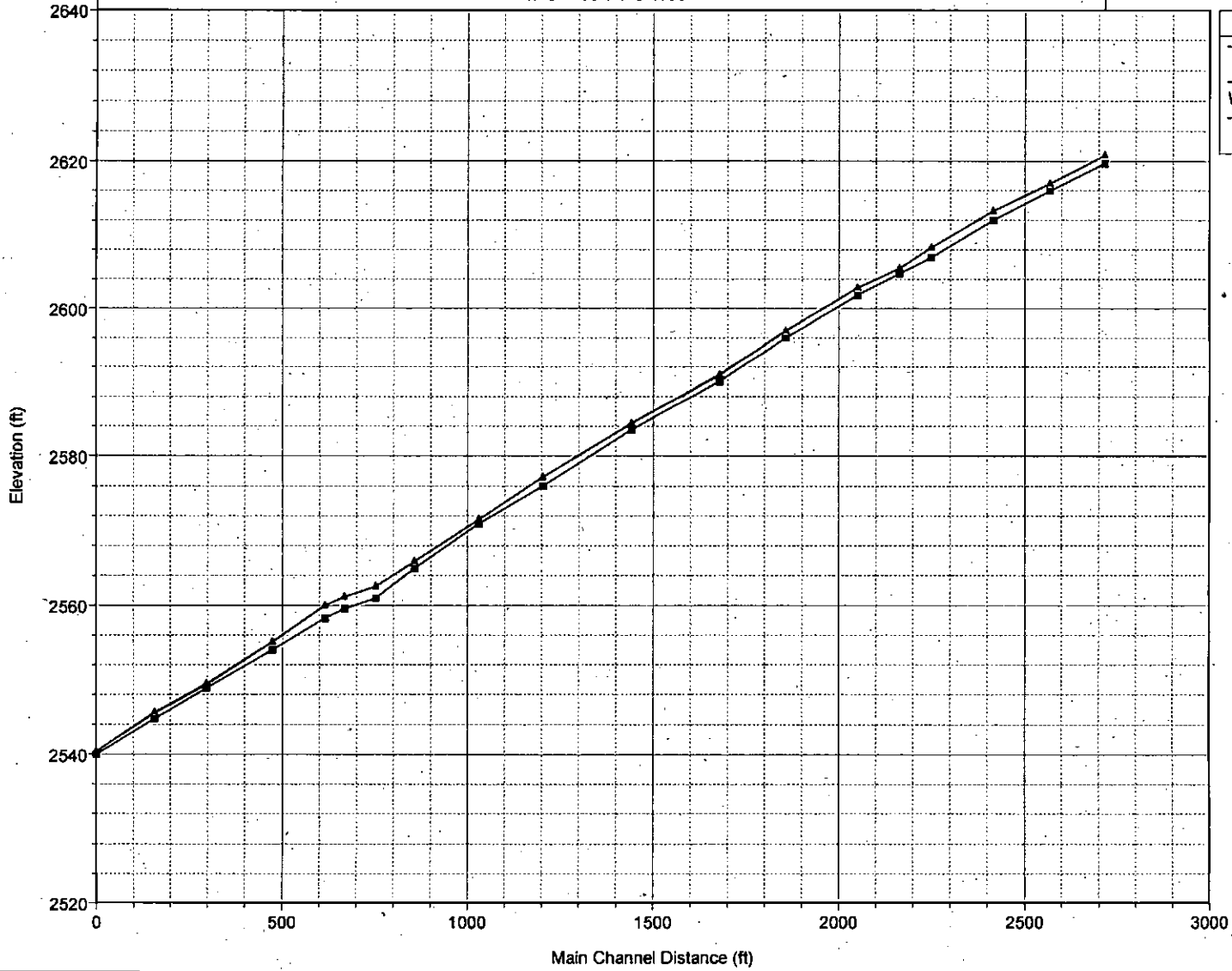
WASH85 WASH85



Legend	
WS 100-Yr_Encroache	▲
WS 100-Yr	■
Ground	●

Storyrock Master Plan: Storyrock Master Encroach

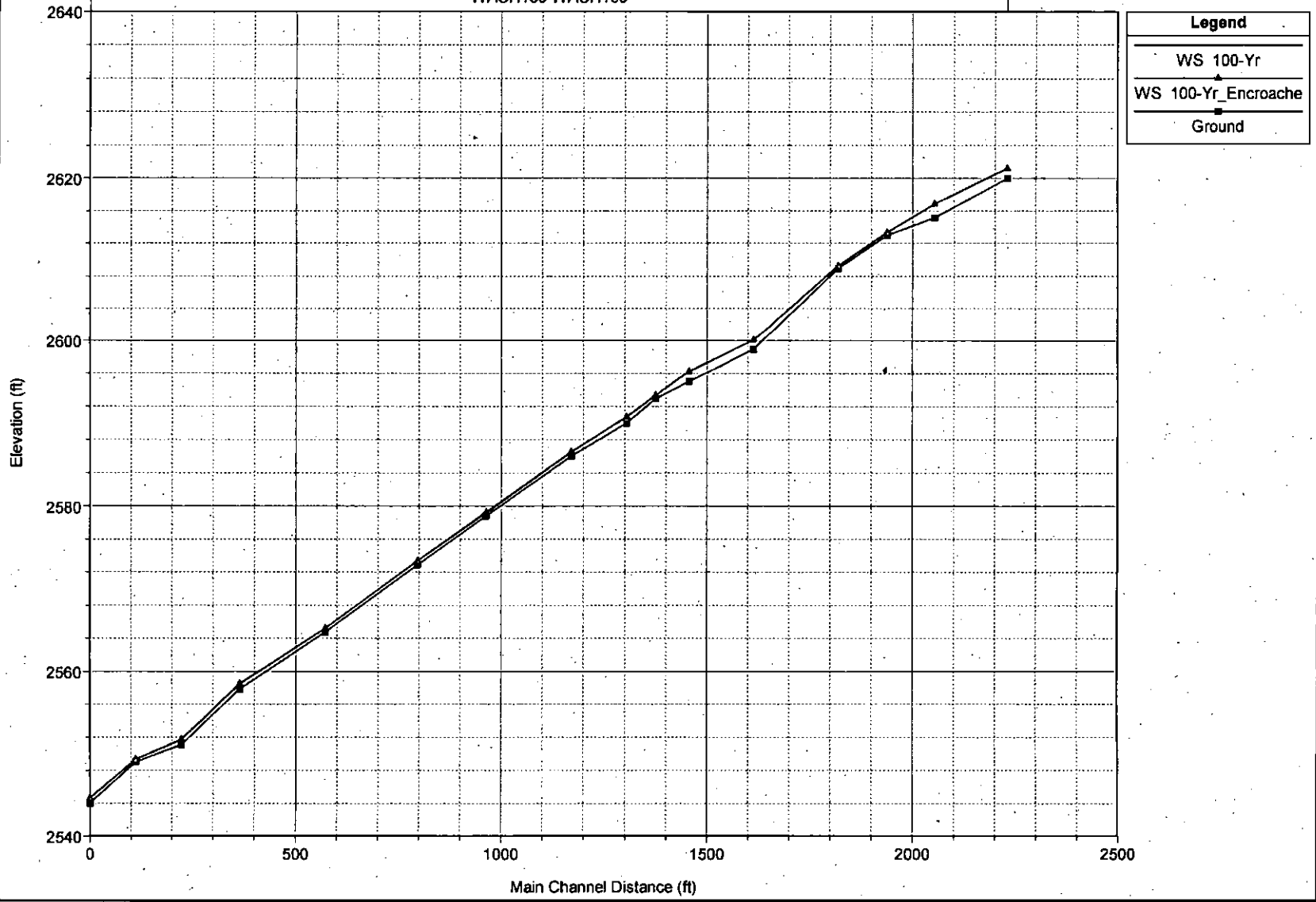
WASH160 WASH160



Legend	
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WS 100-Yr_Encroache	■
Ground	●

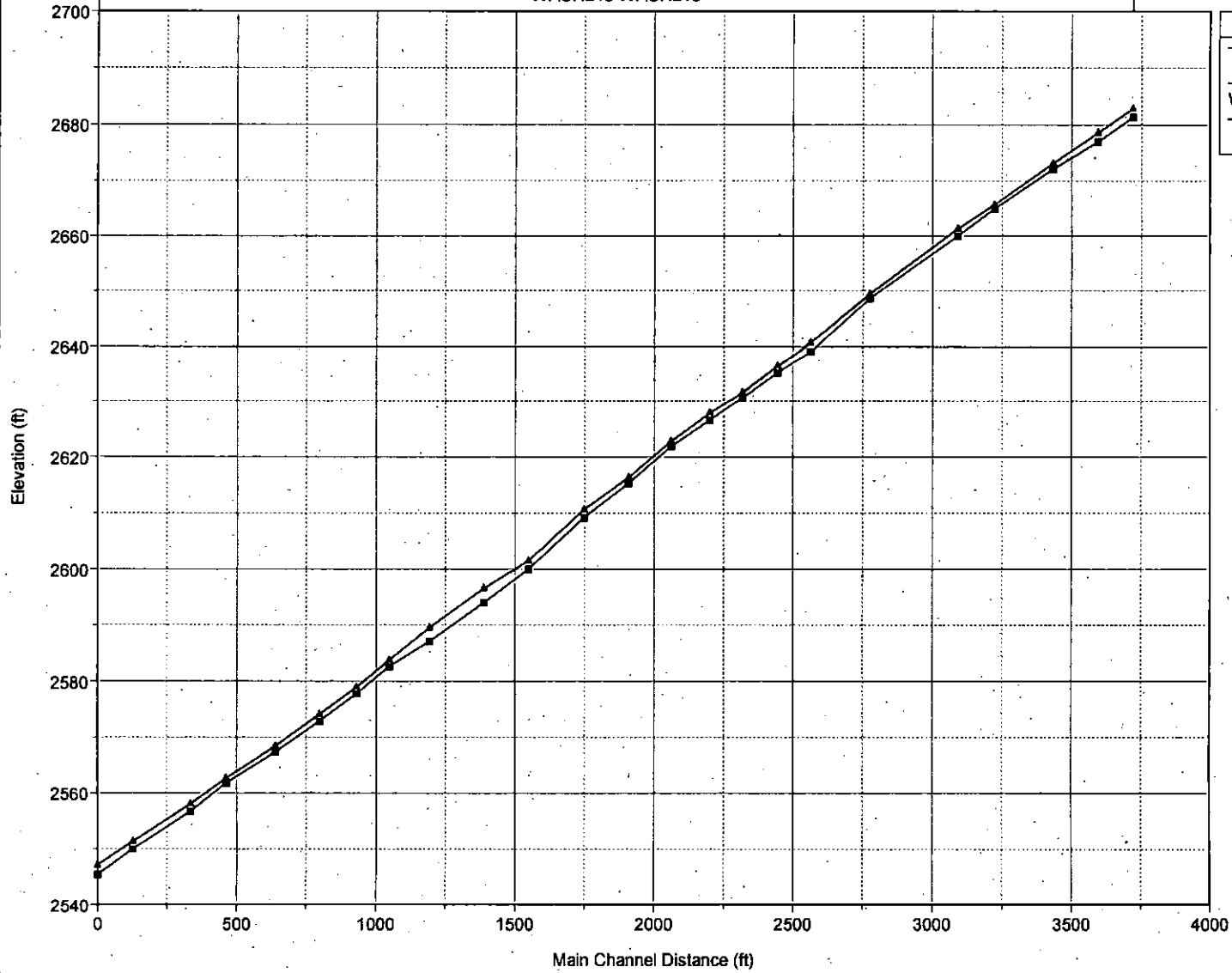
Storyrock Master Plan: Storyrock Master Encroach

WASH180 WASH180

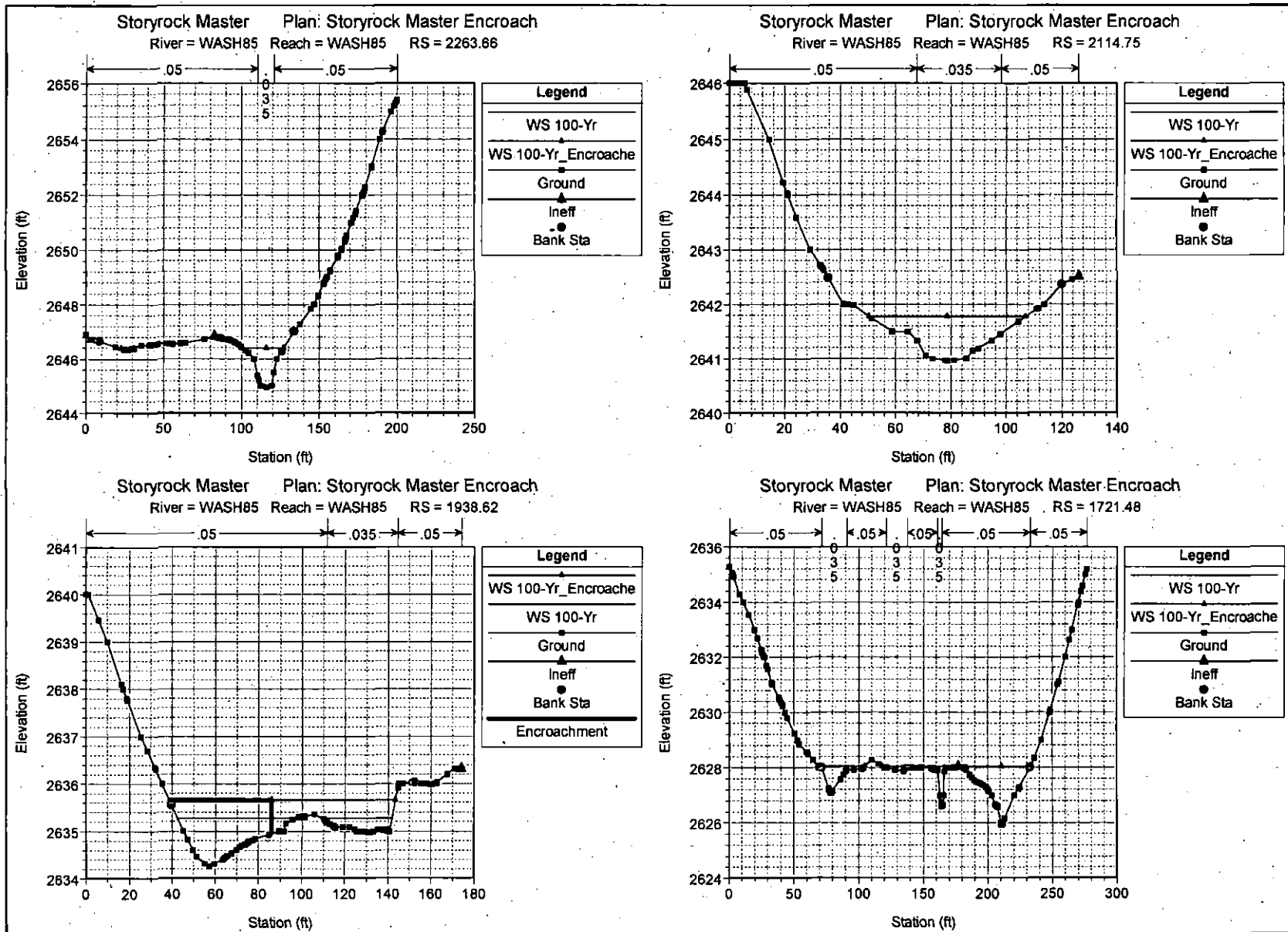


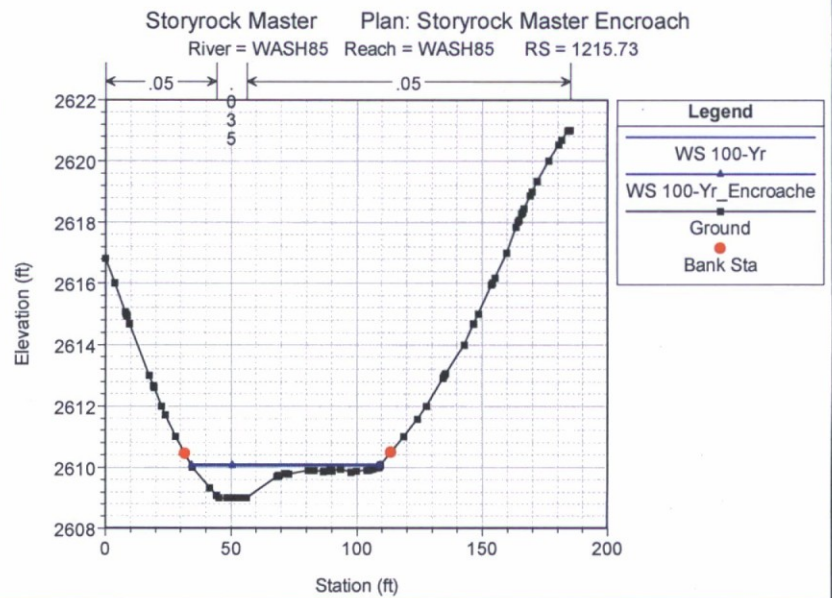
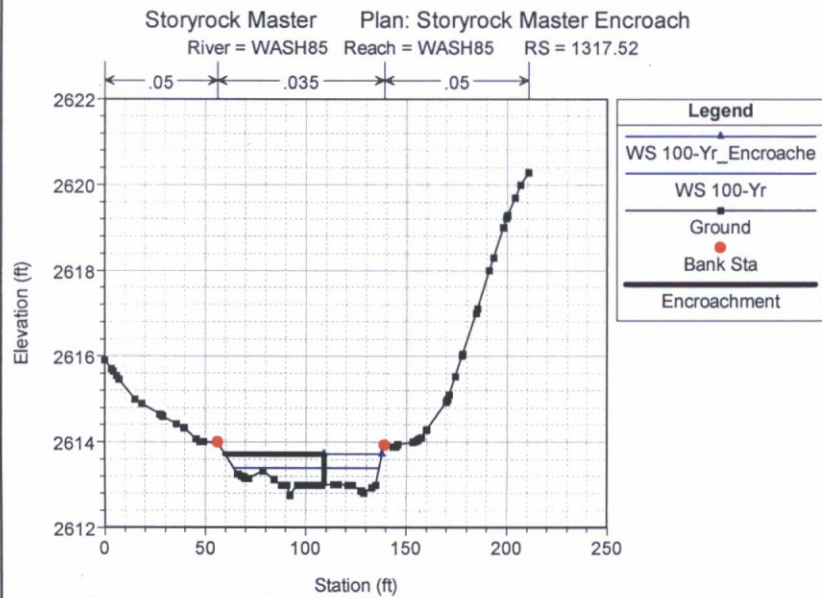
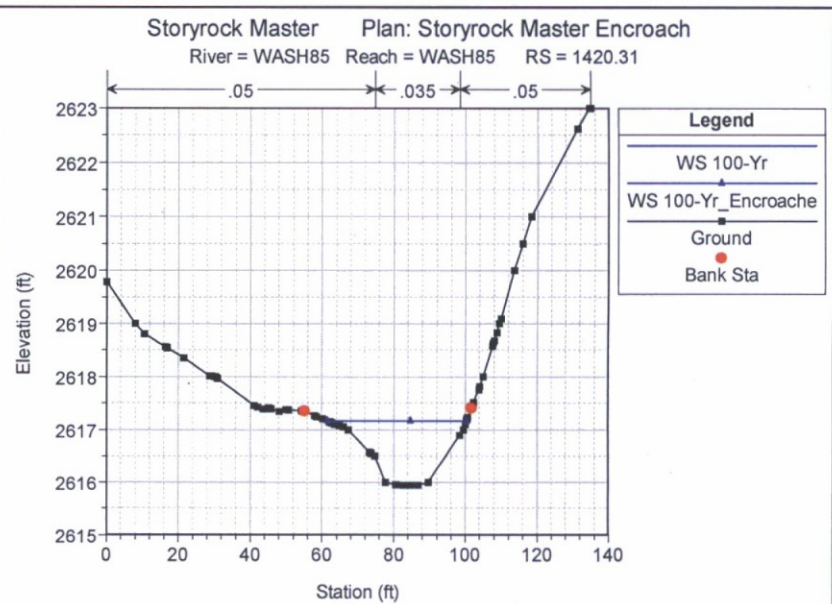
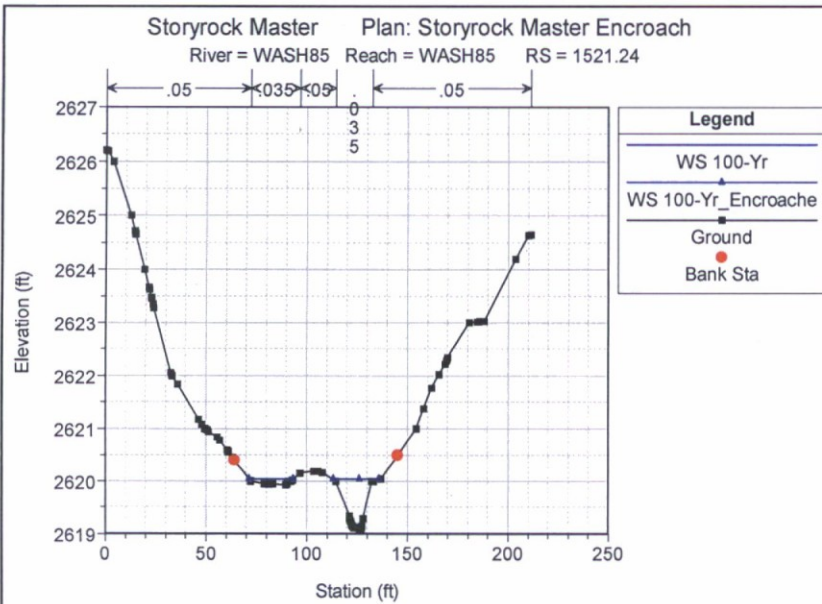
Storyrock Master Plan: Storyrock Master Encroach

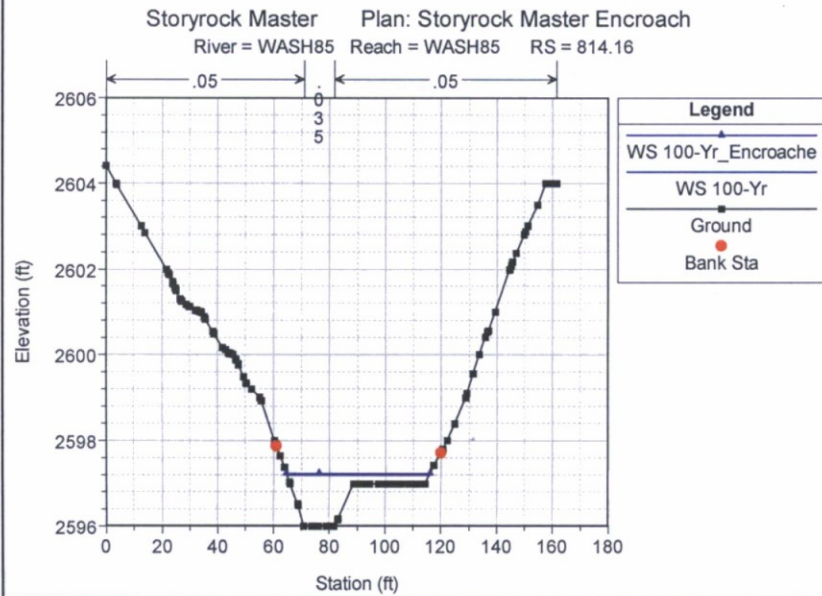
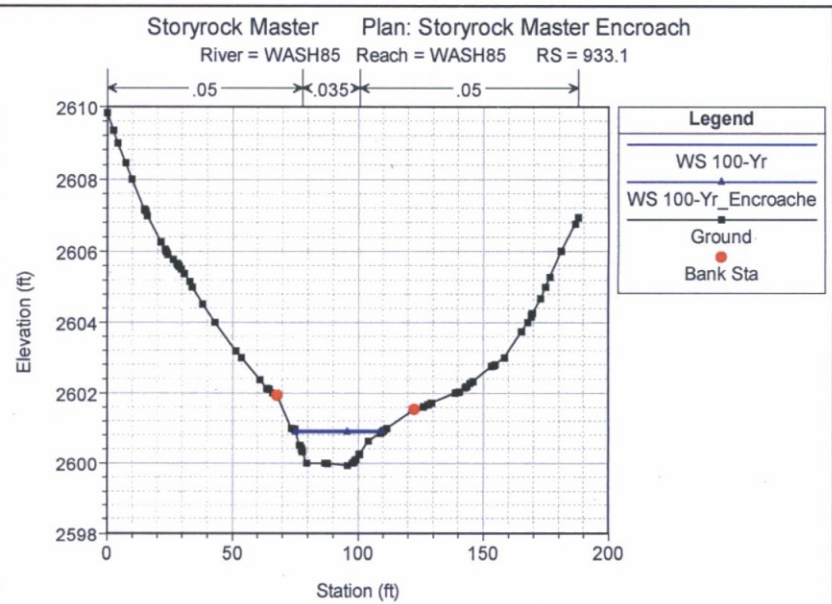
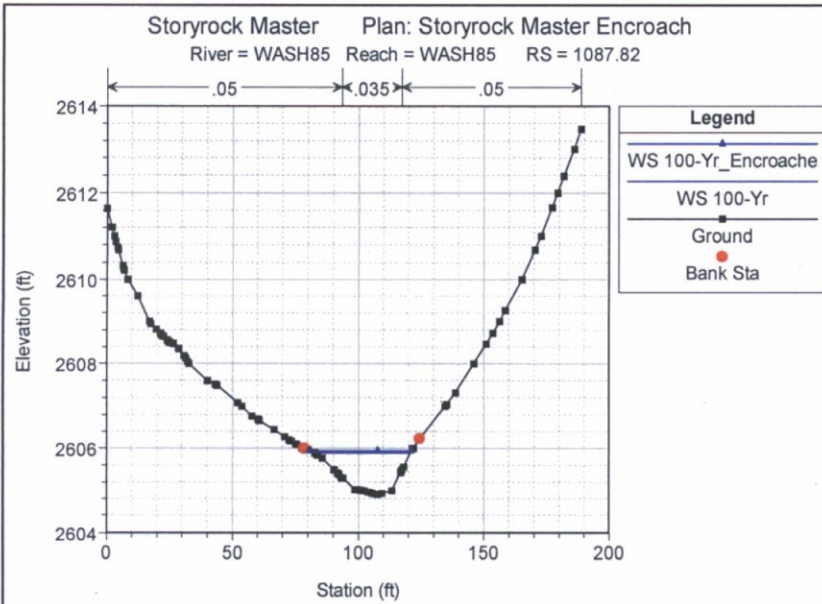
WASH215 WASH215

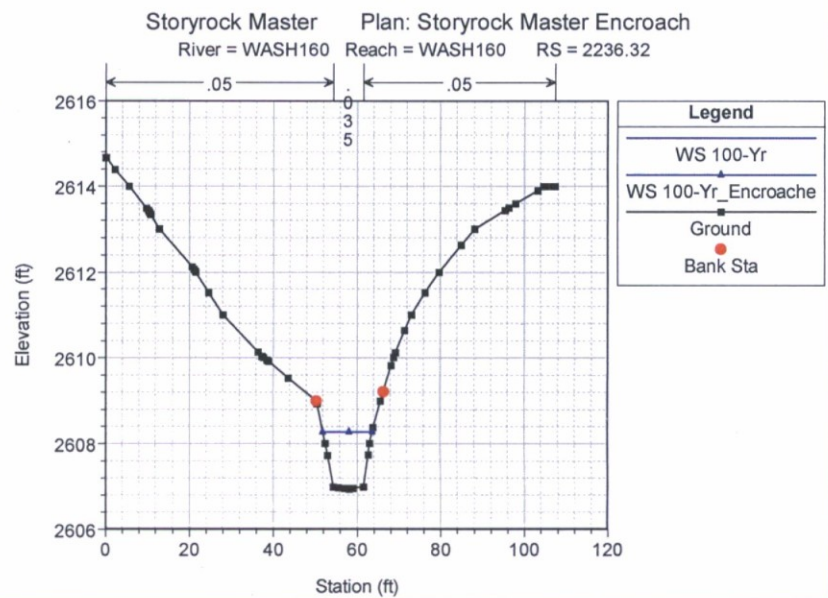
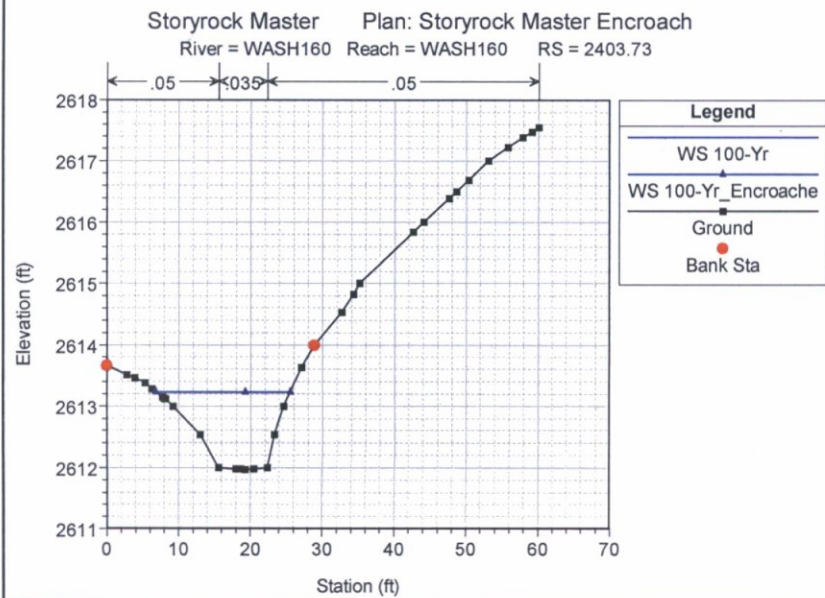
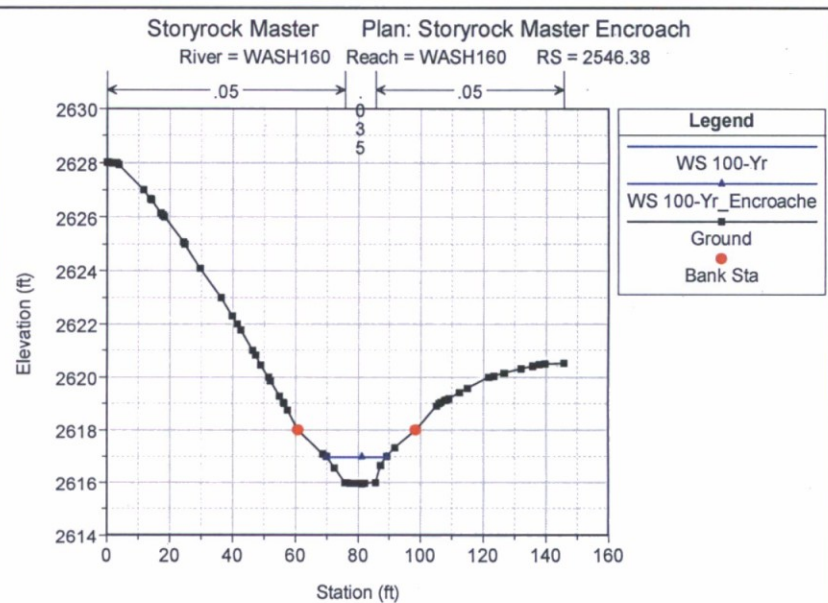
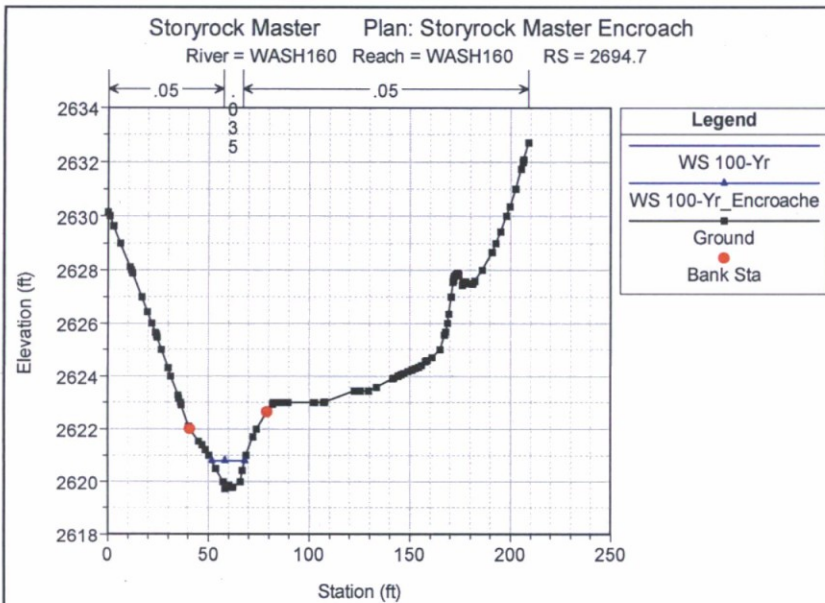


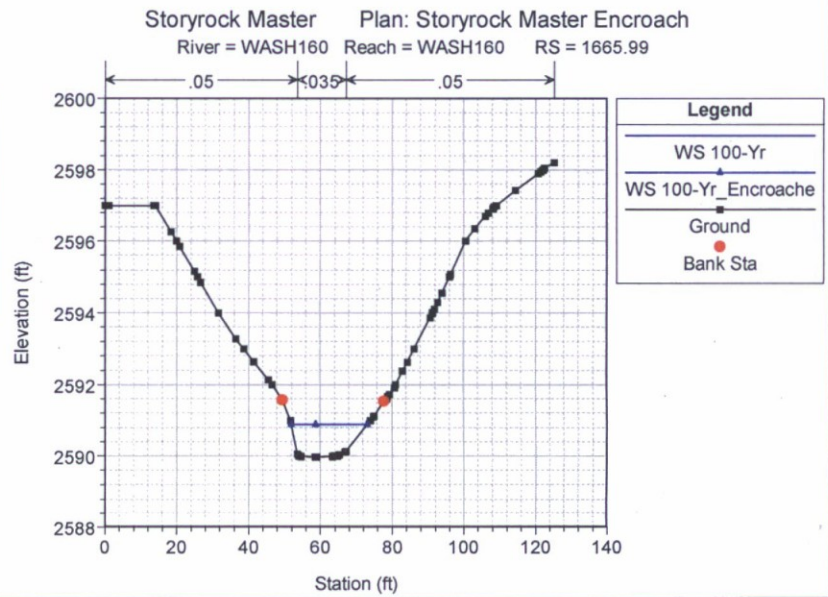
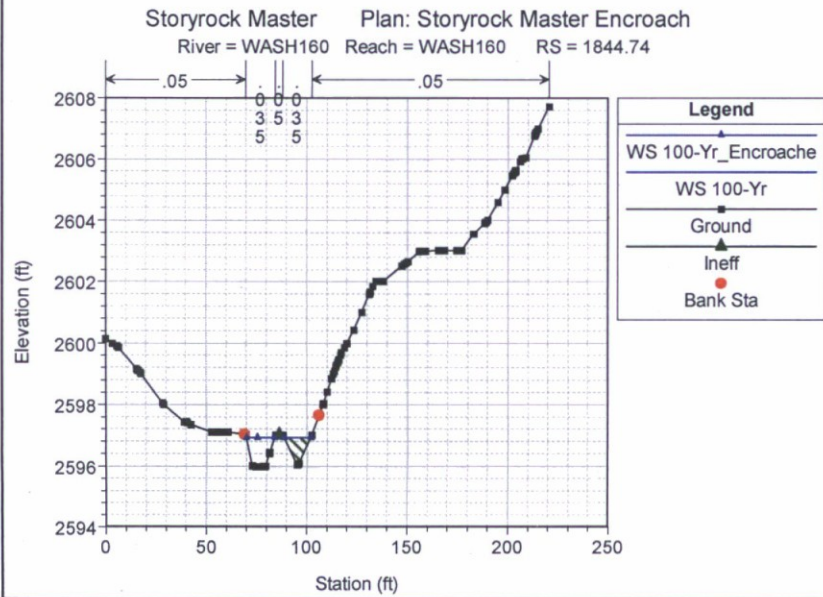
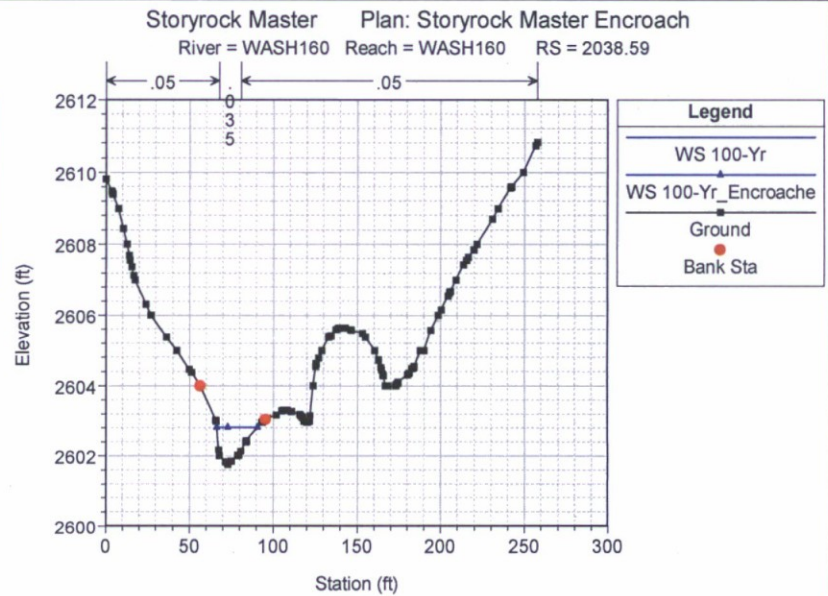
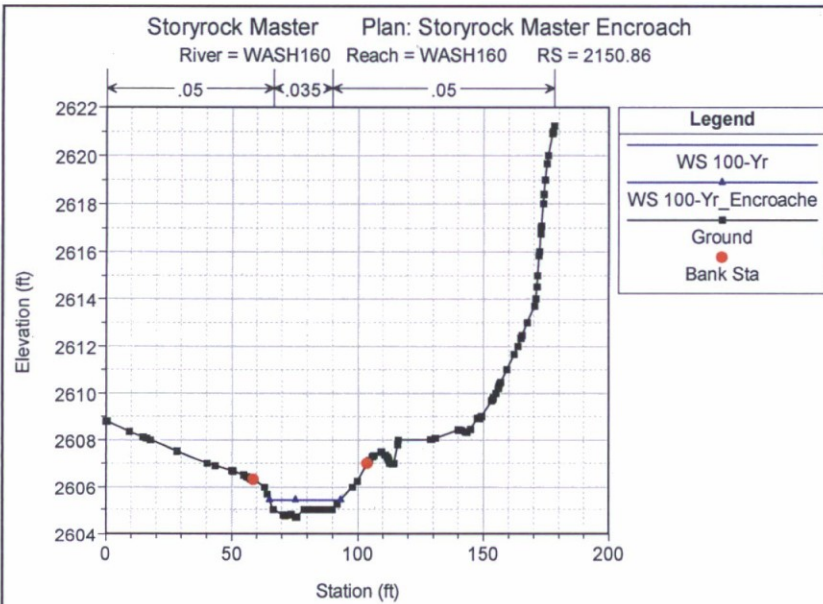
Legend	
WS 100-Yr	▲
WS 100-Yr_Encroache	■
Ground	■

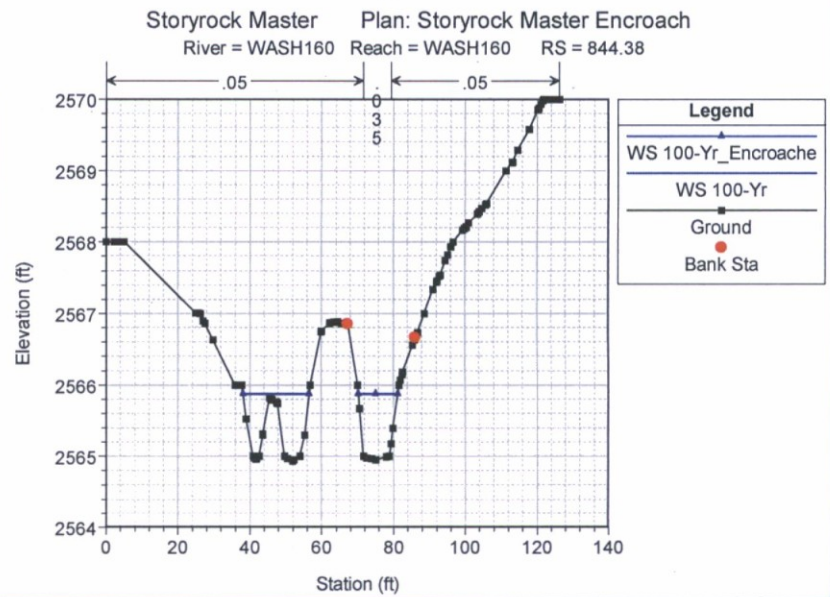
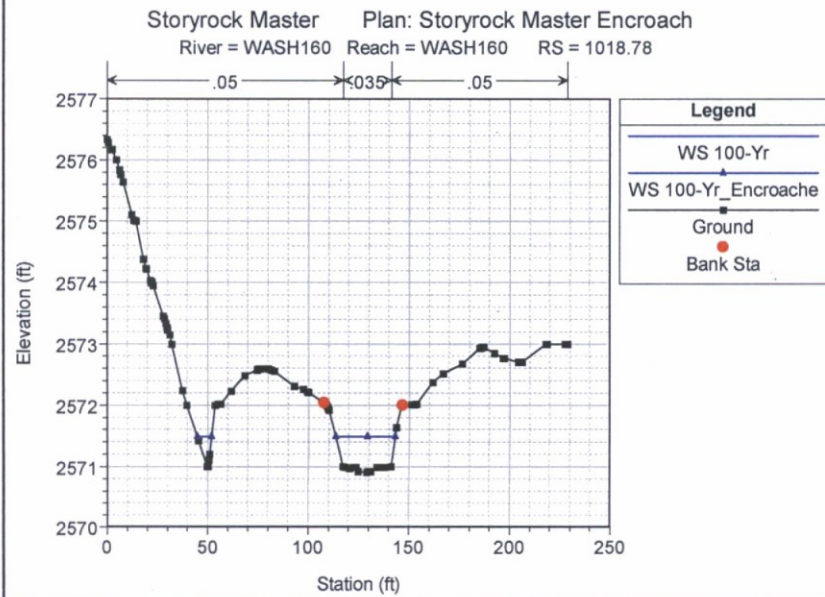
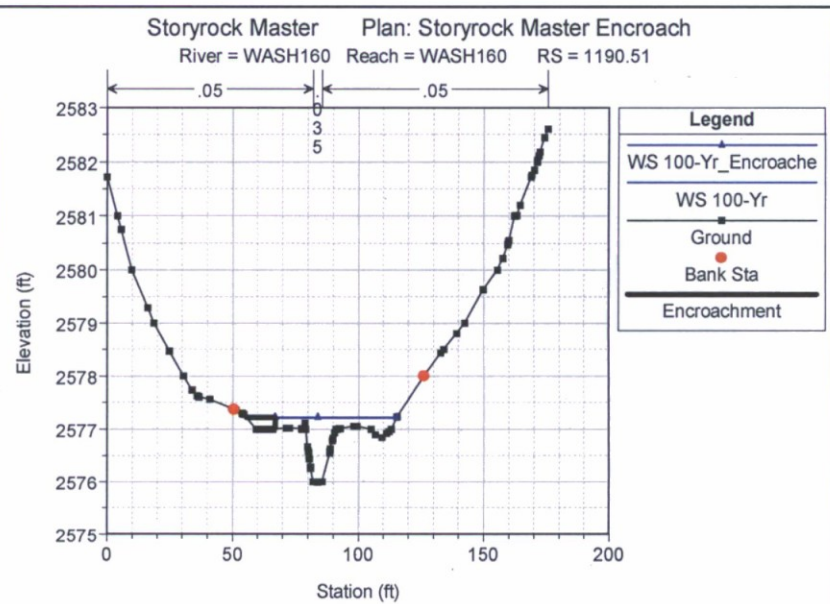
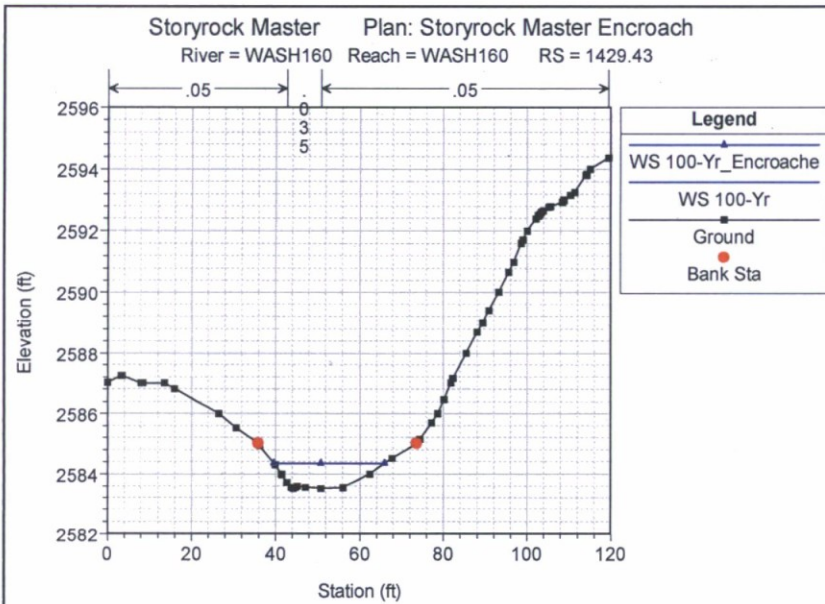


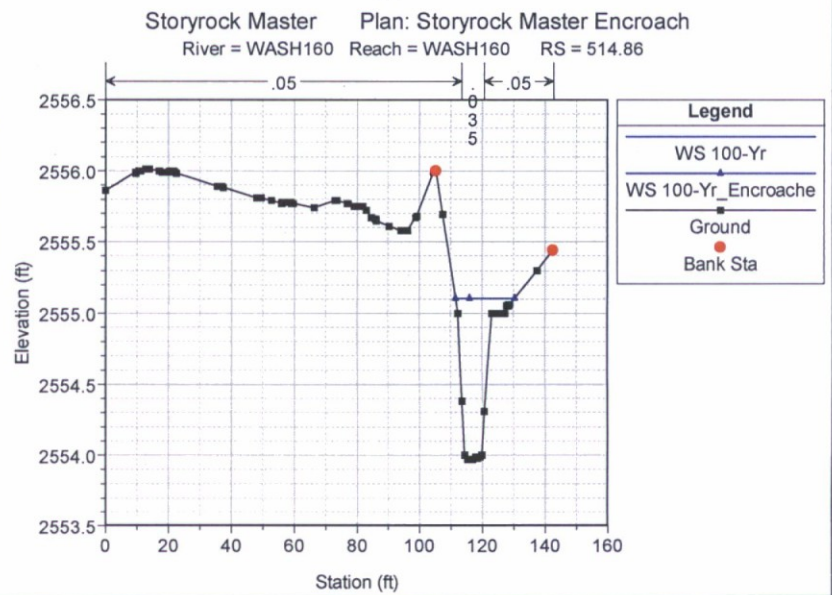
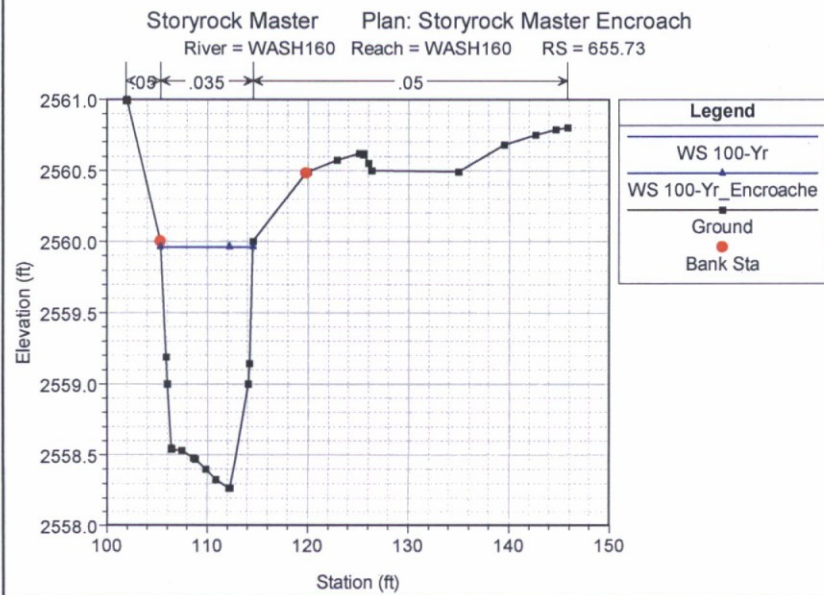
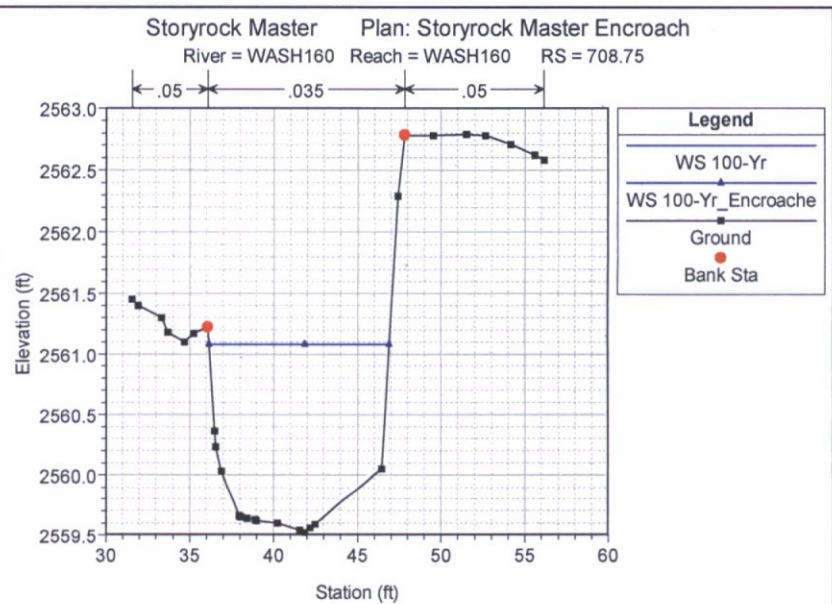
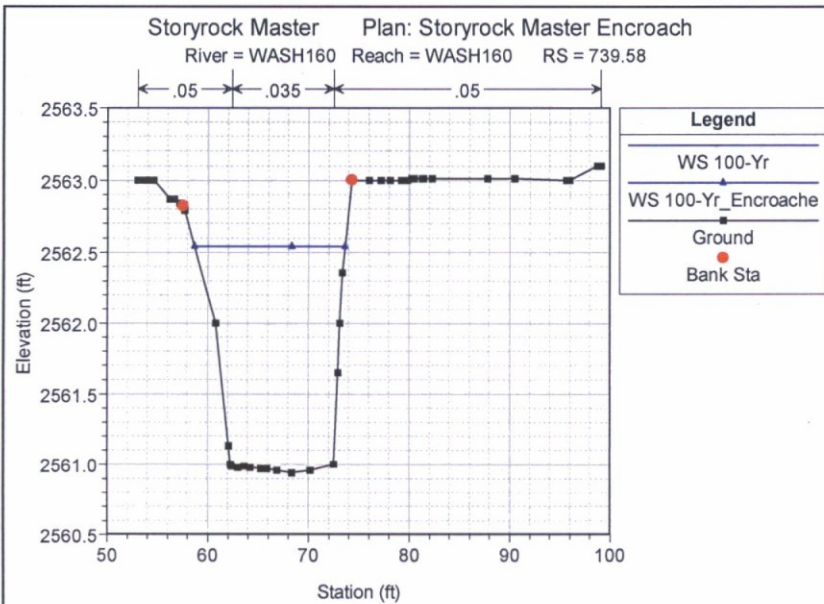


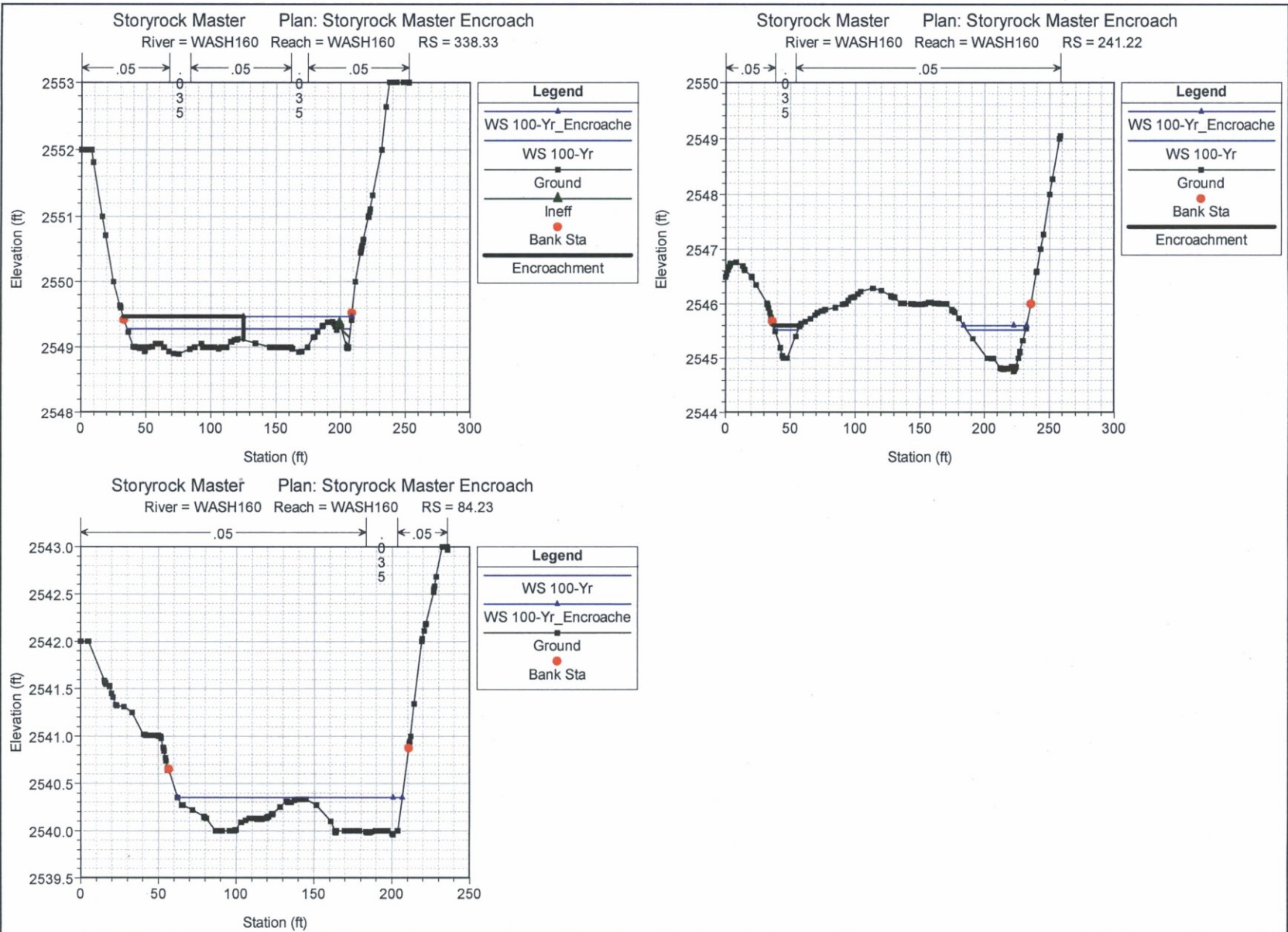


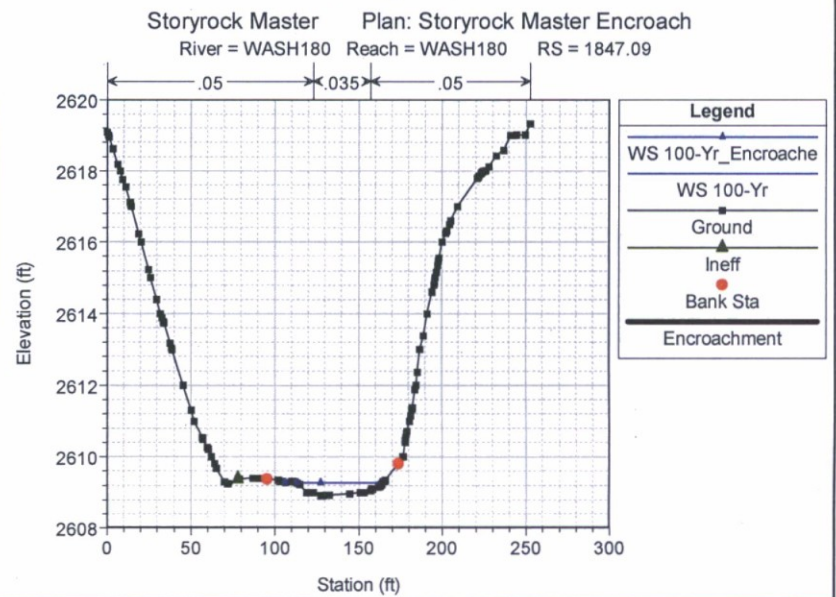
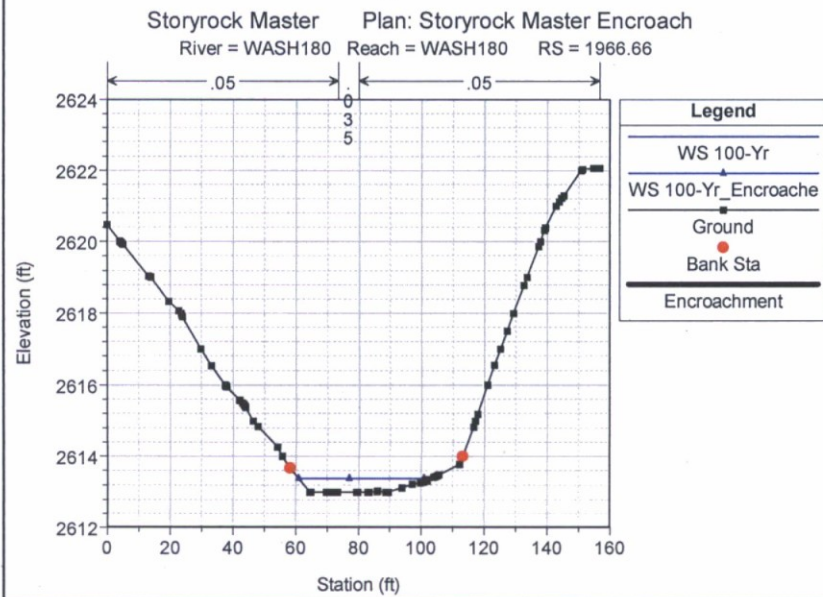
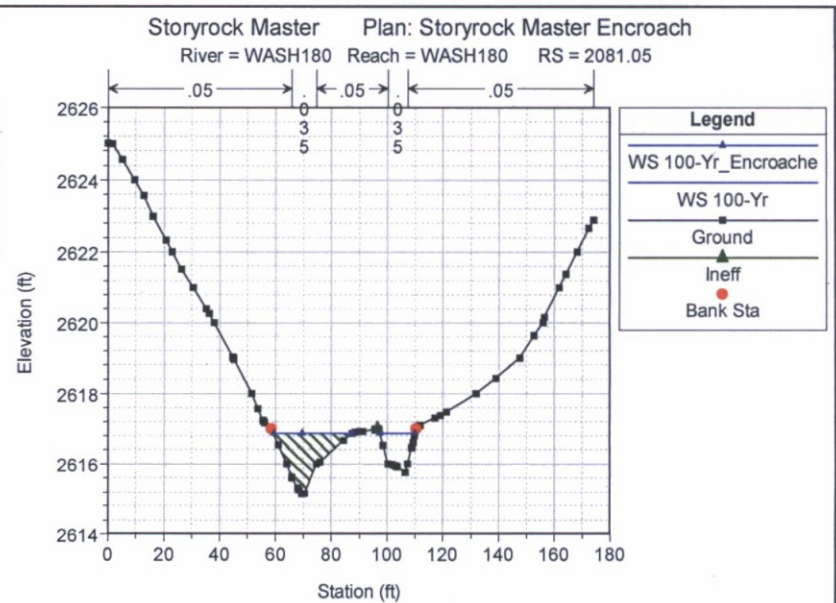
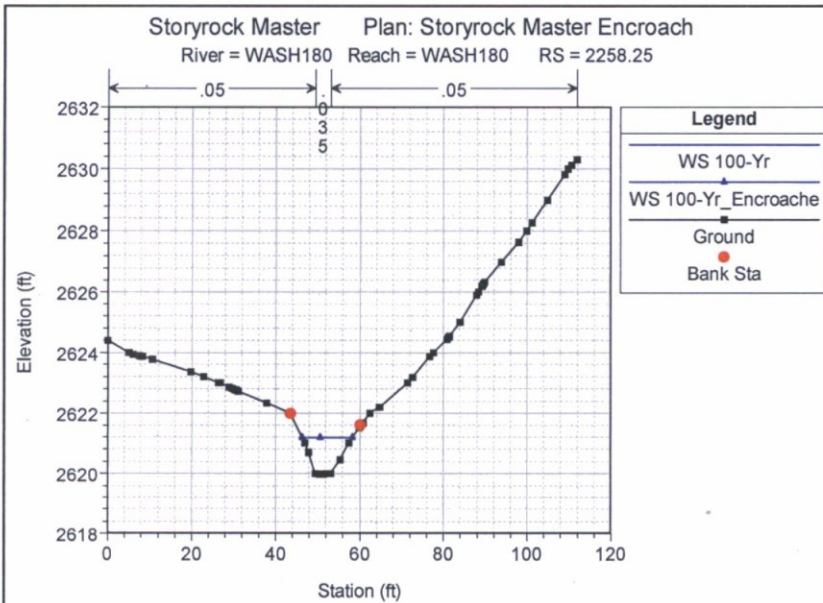


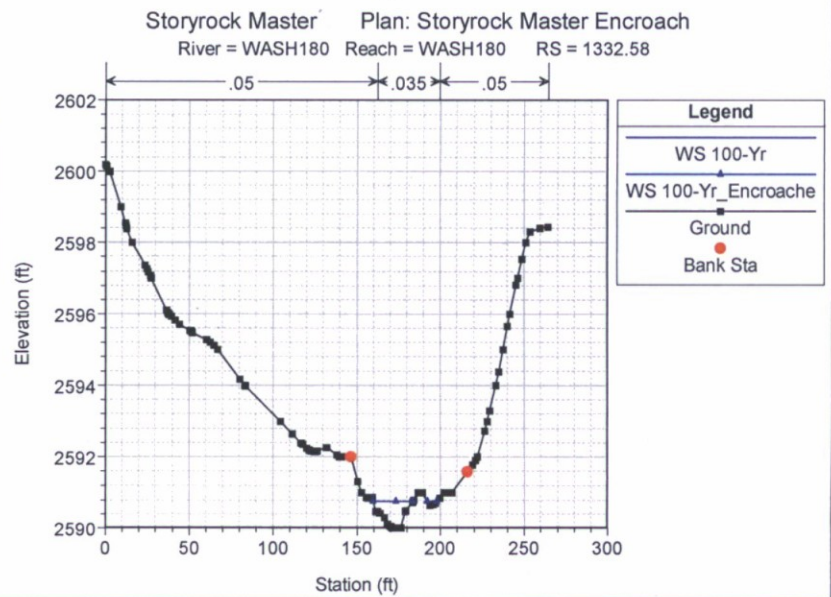
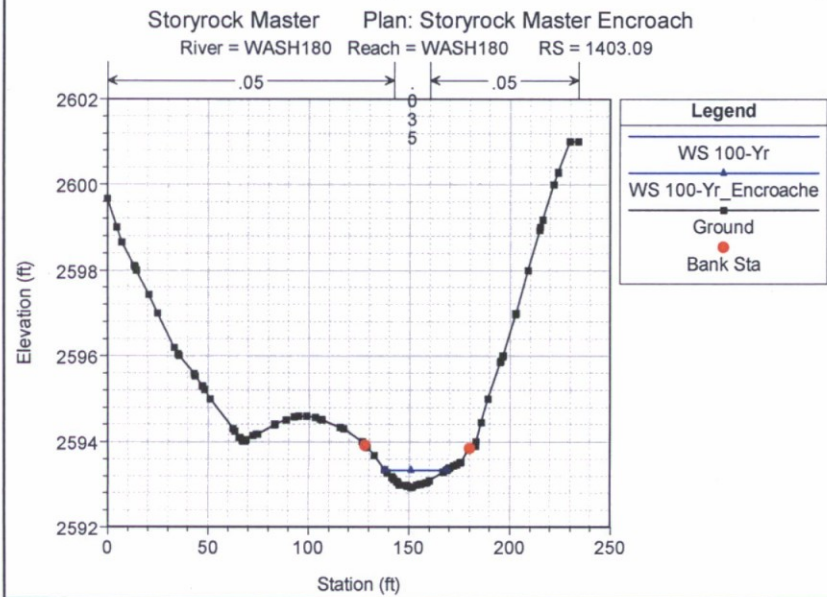
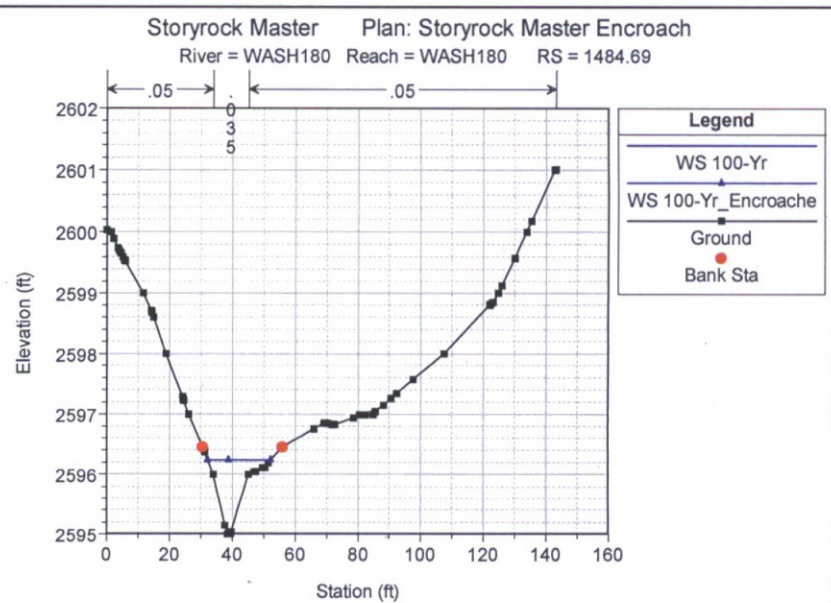
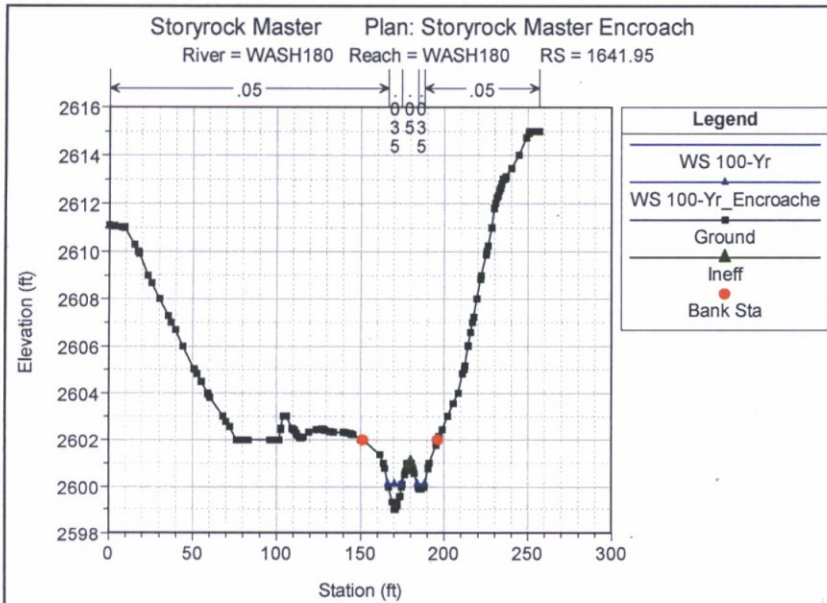


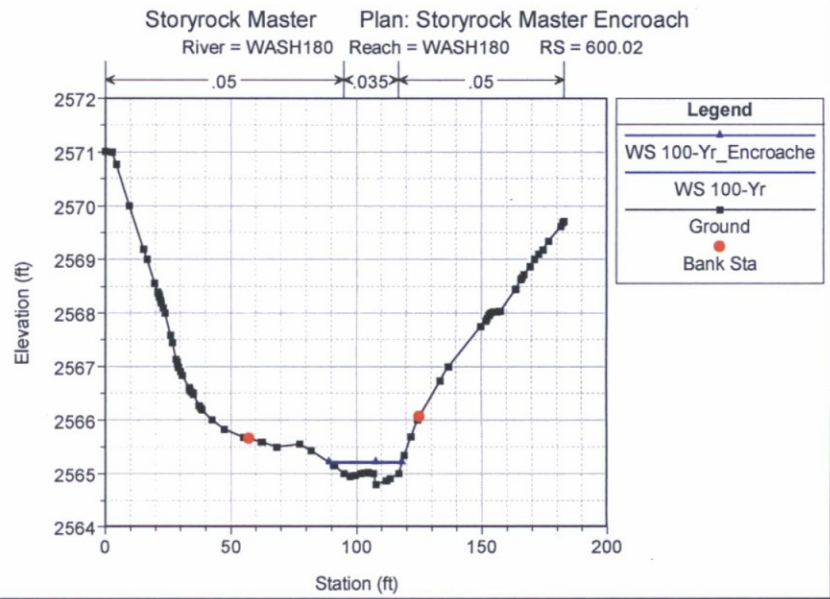
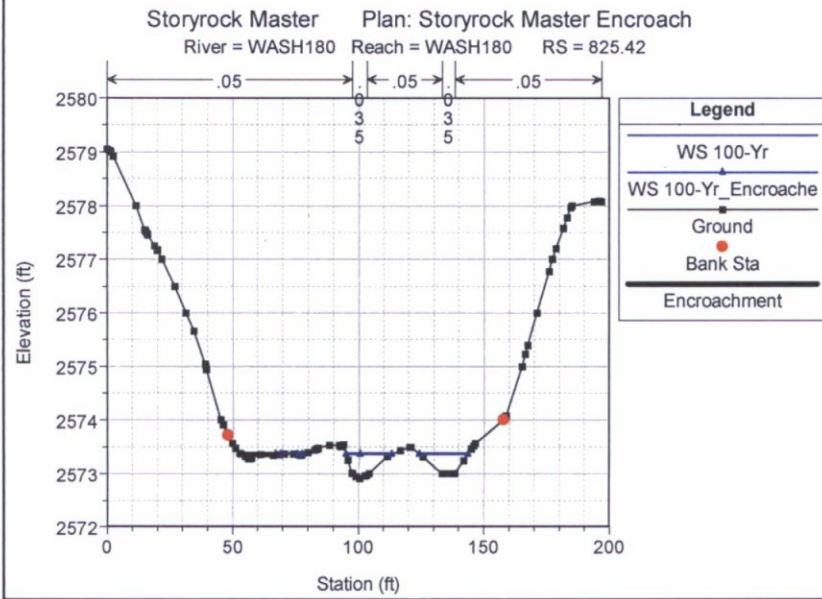
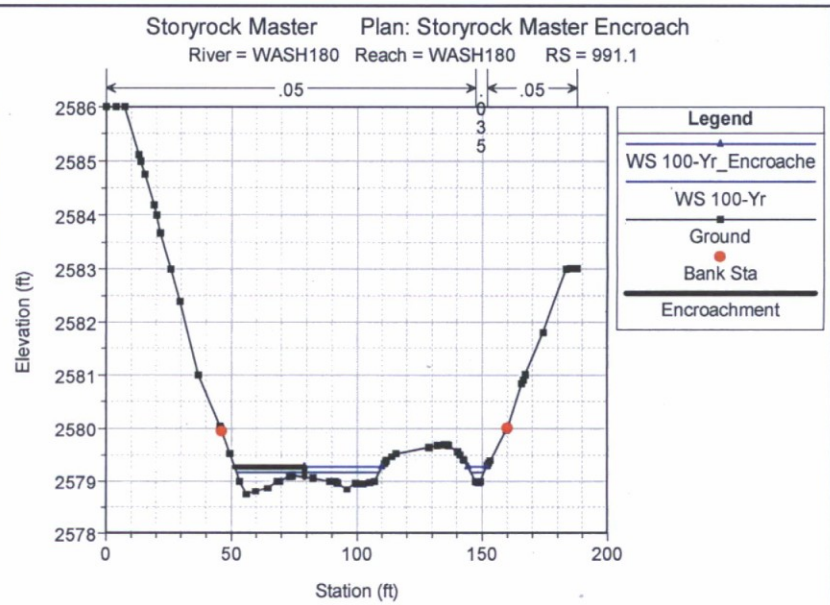
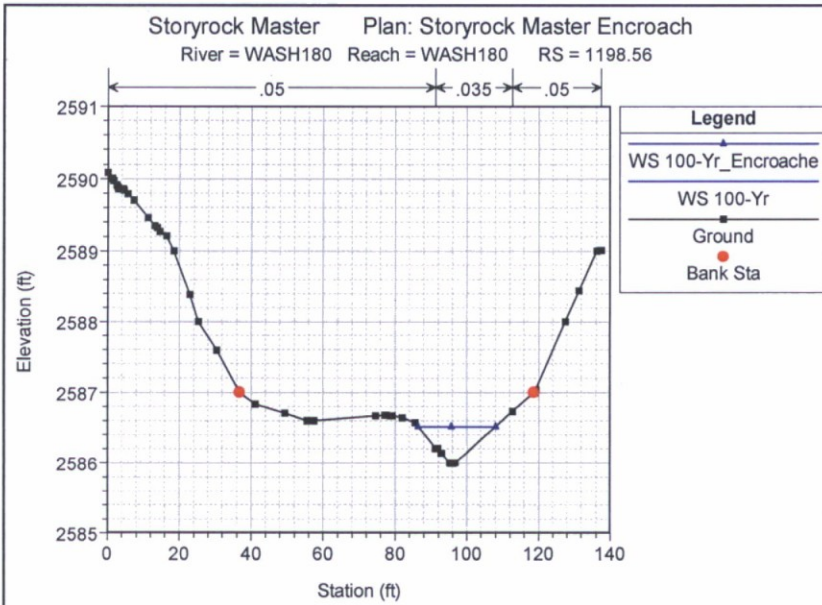


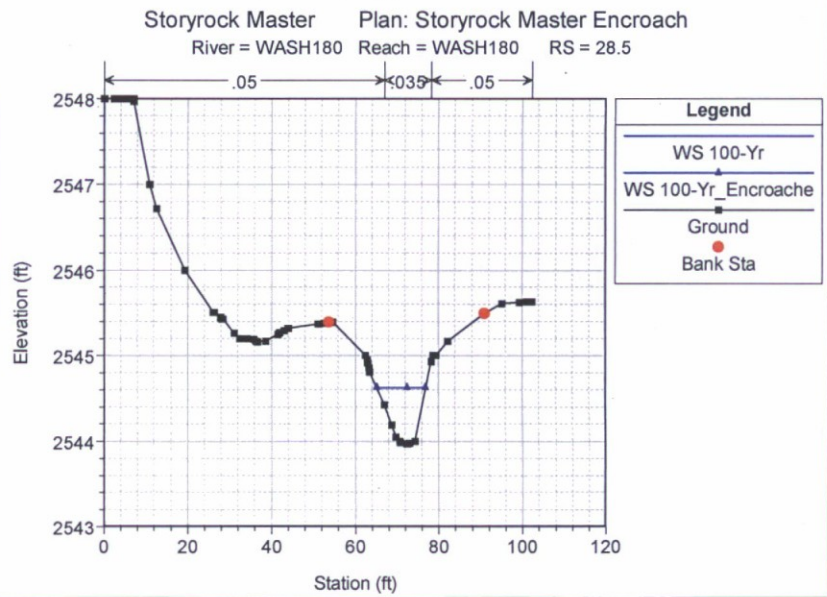
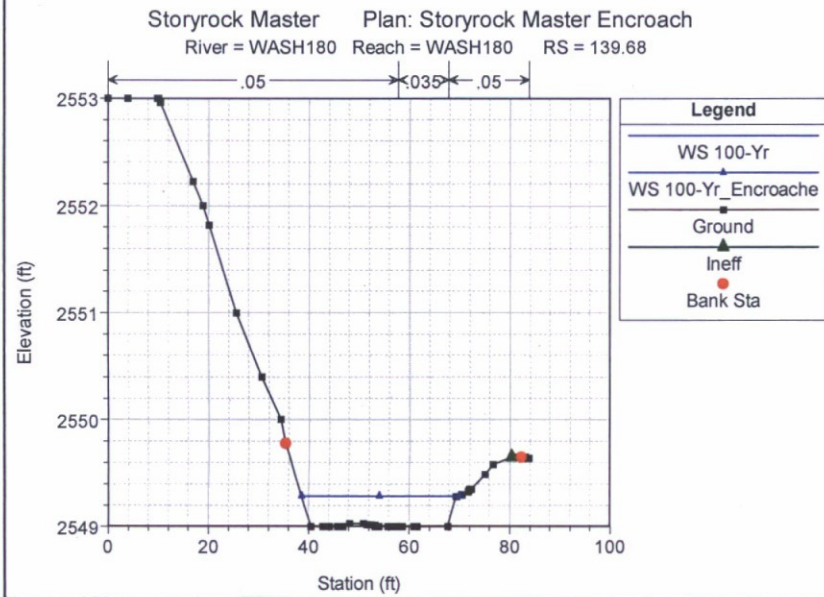
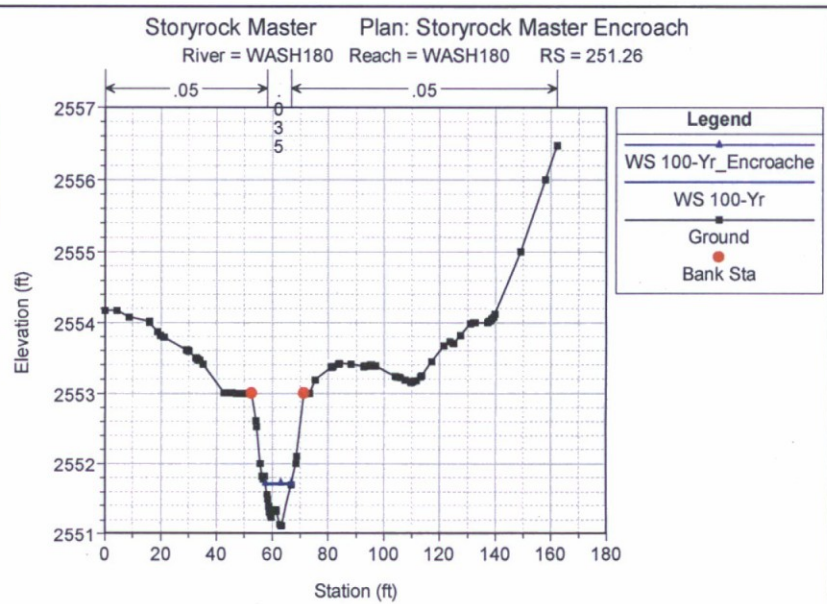
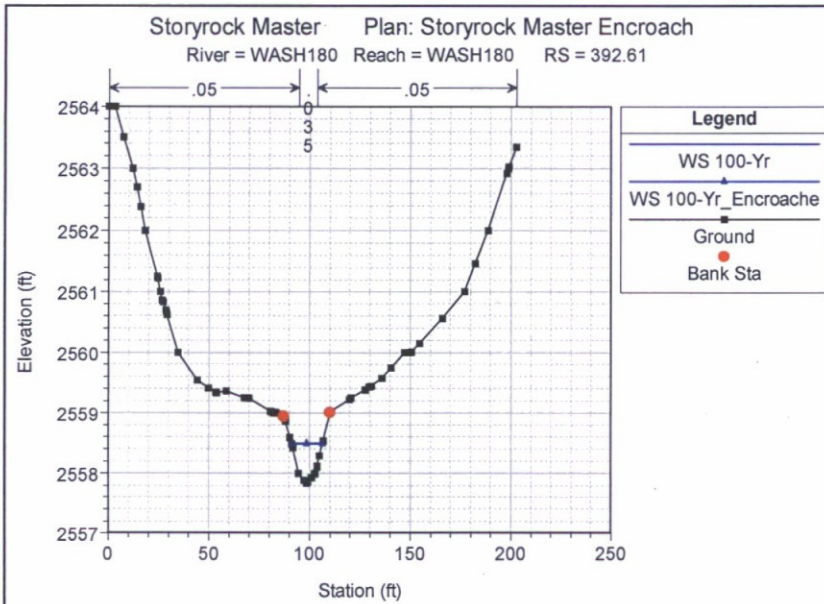


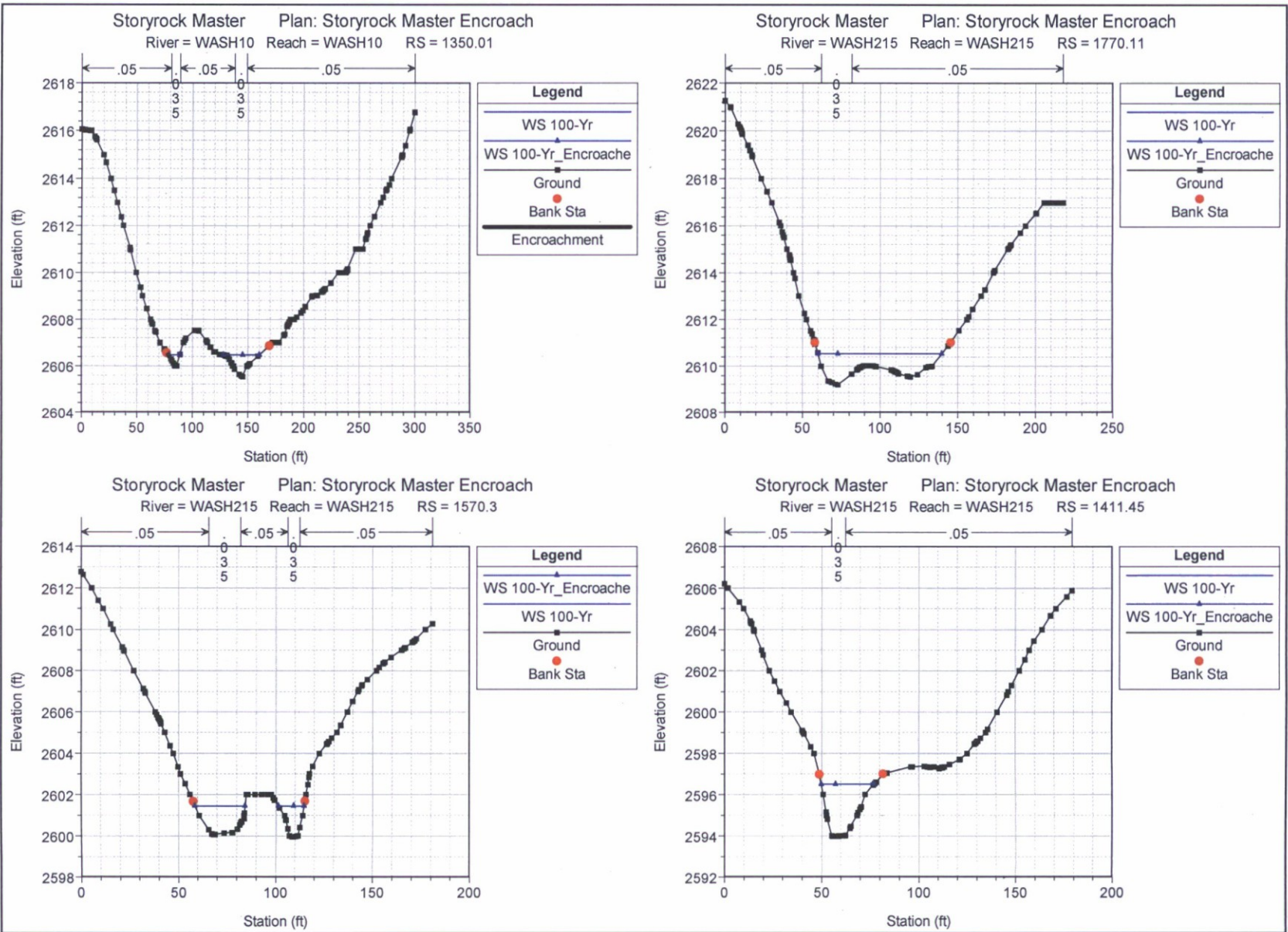


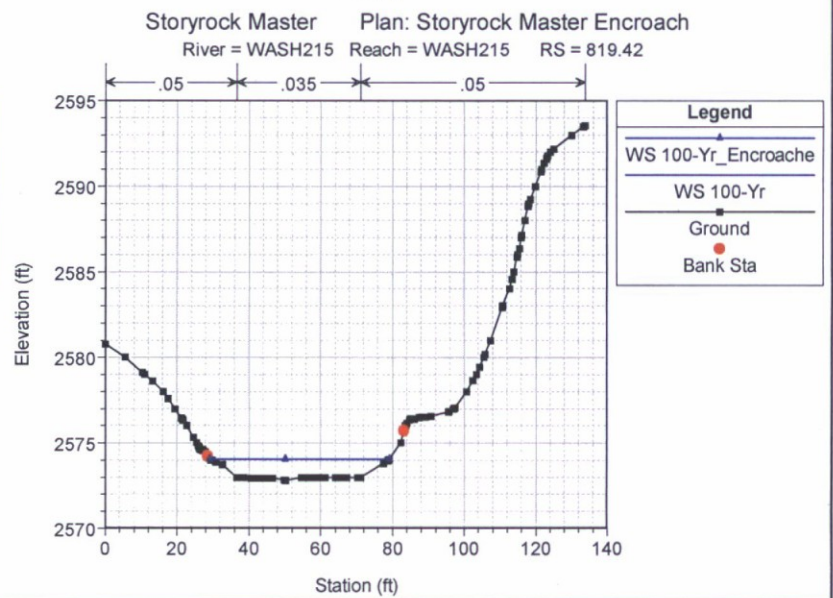
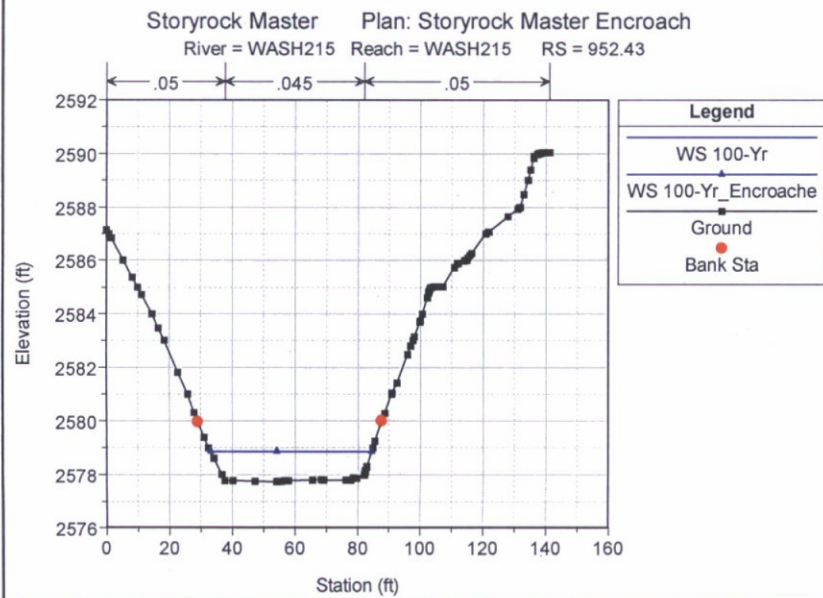
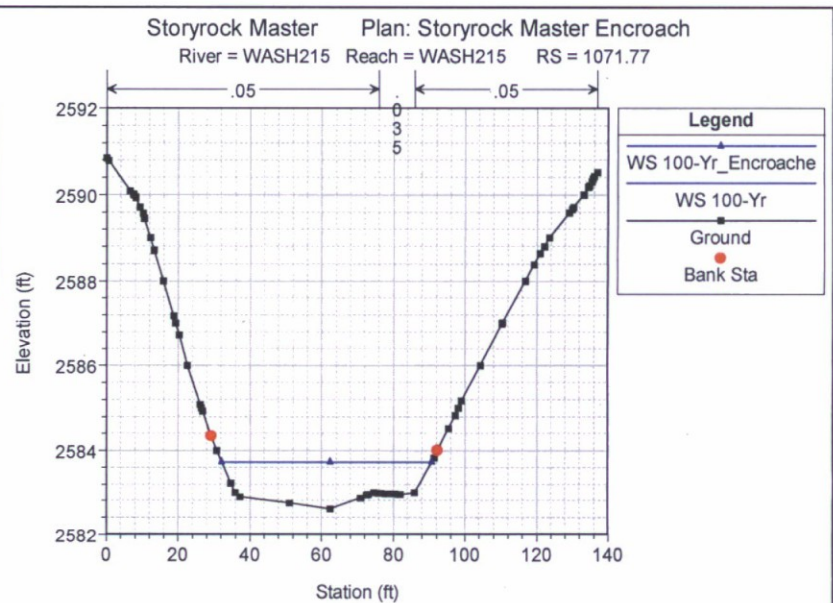
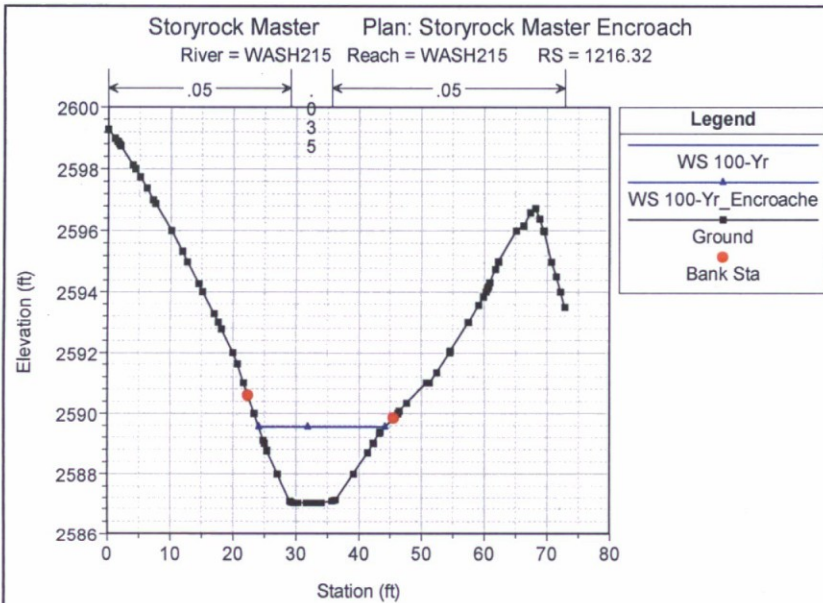


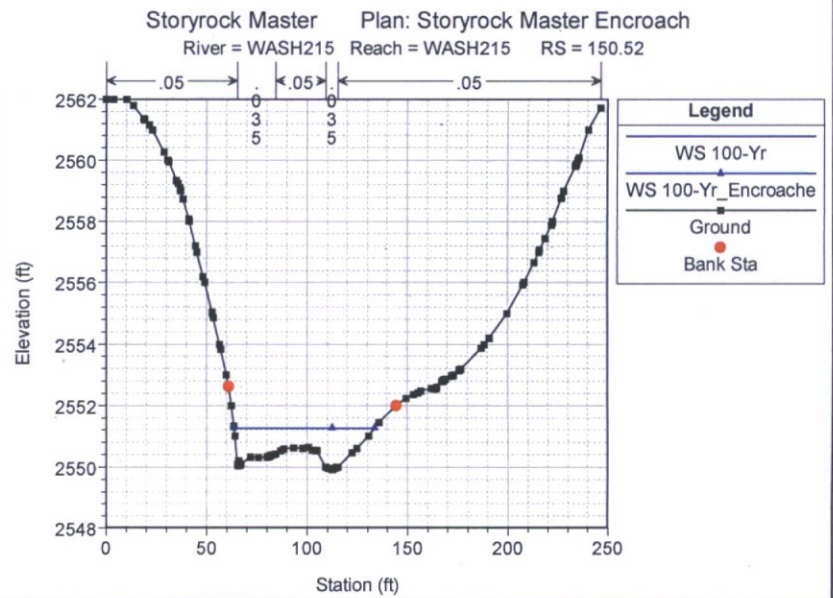
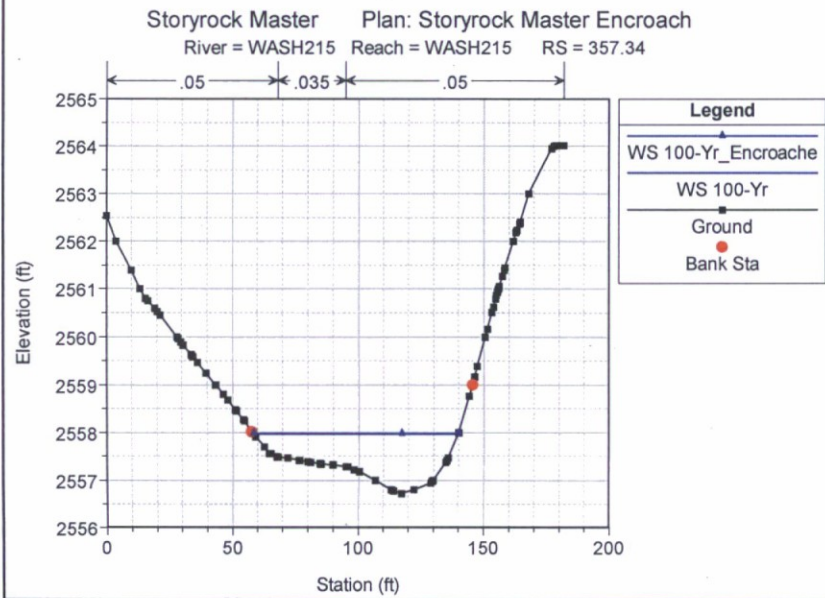
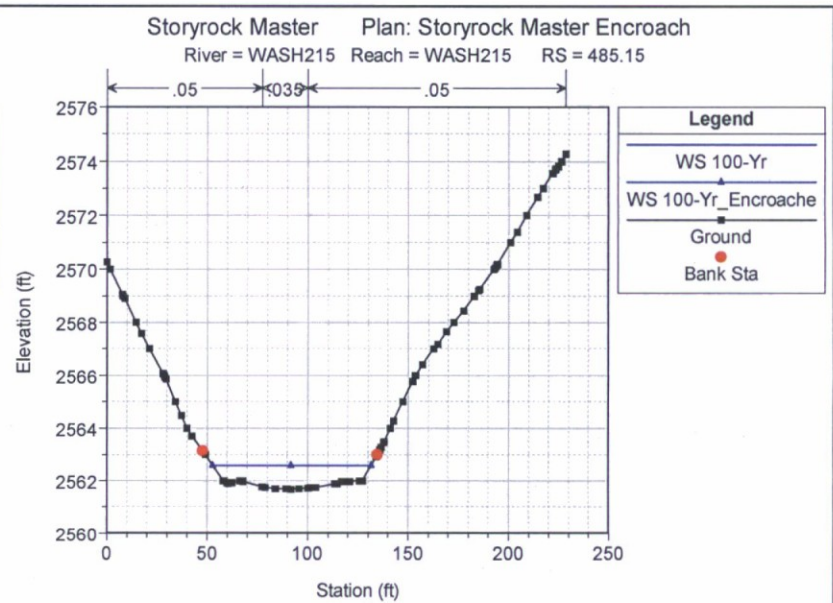
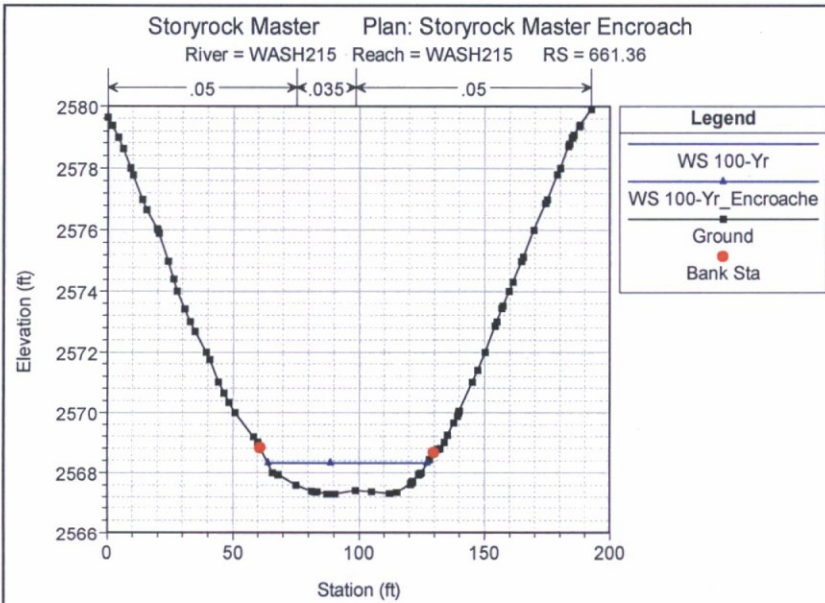






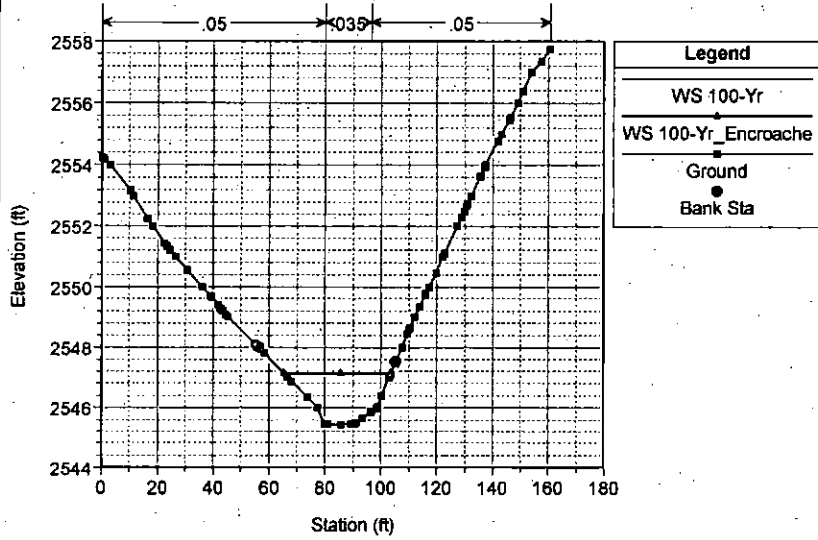






Storyrock Master Plan: Storyrock Master Encroach

River = WASH215 Reach = WASH215 RS = 22.86



HEC-RAS Plan: Encroach Locations: User Defined

River	Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # CH
WASH85	WASH85	2263.66	100-Yr	130.00	2644.94	2646.39	2646.62	2647.13	0.027938	6.92	18.77	34.36	1.44
WASH85	WASH85	2263.66	100-Yr Encroache	130.00	2644.94	2646.39	2646.62	2647.13	0.027938	6.92	18.77	34.36	1.44
WASH85	WASH85	2114.75	100-Yr	133.00	2640.97	2641.77	2641.90	2642.21	0.037910	5.33	24.93	56.52	1.42
WASH85	WASH85	2114.75	100-Yr Encroache	133.00	2640.97	2641.77	2641.90	2642.21	0.037910	5.33	24.93	56.52	1.42
WASH85	WASH85	1938.62	100-Yr	133.00	2634.26	2635.27	2635.27	2635.48	0.039503	3.66	36.32	87.71	1.00
WASH85	WASH85	1938.62	100-Yr Encroache	133.00	2634.95	2635.65	2635.68	2635.98	0.032925	4.48	29.70	57.39	1.10
WASH85	WASH85	1721.48	100-Yr	133.00	2625.94	2628.04	2628.04	2628.10	0.011727	2.02	66.09	151.35	0.53
WASH85	WASH85	1721.48	100-Yr Encroache	133.00	2625.94	2628.04	2628.04	2628.10	0.011727	2.02	66.09	151.35	0.53
WASH85	WASH85	1521.24	100-Yr	133.00	2619.06	2620.04	2620.39	2621.96	0.177883	11.14	11.94	44.61	3.79
WASH85	WASH85	1521.24	100-Yr Encroache	133.00	2619.06	2620.04	2620.39	2621.96	0.177883	11.14	11.94	44.61	3.79
WASH85	WASH85	1420.31	100-Yr	133.00	2615.94	2617.16	2617.17	2617.53	0.016802	4.88	27.23	38.78	1.03
WASH85	WASH85	1420.31	100-Yr Encroache	133.00	2615.94	2617.16	2617.17	2617.53	0.016802	4.88	27.23	38.78	1.03
WASH85	WASH85	1317.52	100-Yr	153.00	2612.77	2613.39	2613.57	2613.98	0.087055	6.15	24.86	72.09	1.85
WASH85	WASH85	1317.52	100-Yr Encroache	153.00	2612.82	2613.72	2614.01	2614.59	0.050705	7.49	20.43	28.66	1.56
WASH85	WASH85	1215.73	100-Yr	153.00	2609.00	2610.10	2610.10	2610.38	0.024562	4.10	37.35	75.81	1.03
WASH85	WASH85	1215.73	100-Yr Encroache	153.00	2609.00	2610.05	2610.10	2610.38	0.032038	4.57	33.50	74.96	1.20
WASH85	WASH85	1087.82	100-Yr	153.00	2604.91	2605.90	2606.09	2606.54	0.036256	6.45	23.72	37.87	1.44
WASH85	WASH85	1087.82	100-Yr Encroache	153.00	2604.91	2605.95	2606.09	2606.49	0.028652	5.89	25.97	40.48	1.30
WASH85	WASH85	933.1	100-Yr	153.00	2599.93	2600.94	2601.10	2601.53	0.028952	6.17	24.80	35.35	1.30
WASH85	WASH85	933.1	100-Yr Encroache	153.00	2599.93	2600.89	2601.09	2601.57	0.035246	6.63	23.09	34.43	1.43
WASH85	WASH85	814.16	100-Yr	153.00	2596.00	2597.19	2597.33	2597.72	0.035255	5.80	26.38	50.90	1.42
WASH85	WASH85	814.16	100-Yr Encroache	153.00	2596.00	2597.24	2597.33	2597.68	0.029494	5.36	28.54	51.44	1.27
WASH160	WASH160	2694.7	100-Yr	70.00	2619.72	2620.79	2620.95	2621.41	0.035115	6.35	11.03	16.14	1.35
WASH160	WASH160	2694.7	100-Yr Encroache	70.00	2619.72	2620.79	2620.95	2621.41	0.035115	6.35	11.03	16.14	1.35
WASH160	WASH160	2546.38	100-Yr	70.00	2615.97	2616.97	2617.00	2617.37	0.021139	5.05	13.86	18.94	1.04
WASH160	WASH160	2546.38	100-Yr Encroache	70.00	2615.97	2616.97	2616.99	2617.37	0.021158	5.05	13.86	18.94	1.04
WASH160	WASH160	2403.73	100-Yr	70.00	2611.97	2613.23	2613.20	2613.59	0.029929	4.79	14.61	18.69	0.96
WASH160	WASH160	2403.73	100-Yr Encroache	70.00	2611.97	2613.23	2613.20	2613.59	0.029929	4.79	14.61	18.69	0.96
WASH160	WASH160	2236.32	100-Yr	70.00	2606.95	2608.27	2608.27	2608.79	0.027353	5.77	12.14	11.84	1.00
WASH160	WASH160	2236.32	100-Yr Encroache	70.00	2606.95	2608.27	2608.27	2608.79	0.027353	5.77	12.14	11.84	1.00
WASH160	WASH160	2150.86	100-Yr	70.00	2604.69	2605.42	2605.54	2605.87	0.042990	5.36	13.05	28.30	1.39
WASH160	WASH160	2150.86	100-Yr Encroache	70.00	2604.69	2605.42	2605.54	2605.87	0.043131	5.37	13.03	28.30	1.39
WASH160	WASH160	2038.59	100-Yr	70.00	2601.77	2602.81	2602.81	2603.14	0.018448	4.55	15.38	24.42	1.01
WASH160	WASH160	2038.59	100-Yr Encroache	70.00	2601.77	2602.81	2602.81	2603.14	0.018467	4.55	15.38	24.41	1.01
WASH160	WASH160	1844.74	100-Yr	70.00	2595.97	2596.93	2597.02	2597.72	0.046114	7.13	9.82	27.41	1.50
WASH160	WASH160	1844.74	100-Yr Encroache	70.00	2595.97	2596.94	2597.02	2597.72	0.046024	7.12	9.83	27.42	1.50
WASH160	WASH160	1665.99	100-Yr	70.00	2589.97	2590.88	2590.88	2591.22	0.026933	4.68	14.96	21.30	0.98
WASH160	WASH160	1665.99	100-Yr Encroache	70.00	2589.97	2590.88	2590.87	2591.22	0.027163	4.69	14.92	21.28	0.99
WASH160	WASH160	1429.43	100-Yr	70.00	2583.51	2584.34	2584.34	2584.65	0.028594	4.43	15.80	26.23	1.01
WASH160	WASH160	1429.43	100-Yr Encroache	70.00	2583.51	2584.34	2584.34	2584.65	0.028366	4.42	15.85	26.26	1.00
WASH160	WASH160	1190.51	100-Yr	70.00	2575.99	2577.20	2577.22	2577.40	0.031717	3.64	19.21	59.40	1.13
WASH160	WASH160	1190.51	100-Yr Encroache	70.00	2575.99	2577.21	2577.25	2577.45	0.031463	3.89	17.97	48.32	1.13
WASH160	WASH160	1018.78	100-Yr	70.00	2570.91	2571.49	2571.56	2571.82	0.033633	4.76	15.69	36.45	1.23
WASH160	WASH160	1018.78	100-Yr Encroache	70.00	2570.91	2571.48	2571.56	2571.83	0.034422	4.80	15.57	36.36	1.24
WASH160	WASH160	844.38	100-Yr	83.00	2564.95	2565.87	2565.91	2566.22	0.034366	5.47	18.54	29.36	1.12
WASH160	WASH160	844.38	100-Yr Encroache	83.00	2564.95	2565.87	2565.91	2566.22	0.033723	5.43	18.66	29.40	1.11
WASH160	WASH160	739.58	100-Yr	83.00	2560.94	2562.54	2562.19	2562.84	0.011687	4.37	18.99	14.99	0.68
WASH160	WASH160	739.58	100-Yr Encroache	83.00	2560.94	2562.54	2562.19	2562.84	0.011687	4.37	18.99	14.99	0.68
WASH160	WASH160	708.75	100-Yr	83.00	2559.52	2561.08	2561.02	2561.64	0.016944	6.02	13.79	10.76	0.94
WASH160	WASH160	708.75	100-Yr Encroache	83.00	2559.52	2561.08	2561.02	2561.64	0.016944	6.02	13.79	10.76	0.94
WASH160	WASH160	655.73	100-Yr	83.00	2558.27	2559.96	2559.96	2560.65	0.020068	6.66	12.46	9.19	1.01
WASH160	WASH160	655.73	100-Yr Encroache	83.00	2558.27	2559.96	2559.96	2560.65	0.020068	6.66	12.46	9.19	1.01
WASH160	WASH160	514.86	100-Yr	83.00	2553.97	2555.10	2555.45	2556.20	0.053459	8.40	9.88	18.67	2.04
WASH160	WASH160	514.86	100-Yr Encroache	83.00	2553.97	2555.10	2555.45	2556.20	0.053459	8.40	9.88	18.67	2.04
WASH160	WASH160	338.33	100-Yr	83.00	2548.90	2549.28	2549.23	2549.35	0.022937	2.12	39.06	155.95	0.73
WASH160	WASH160	338.33	100-Yr Encroache	83.00	2548.93	2549.47	2549.42	2549.59	0.023742	2.73	30.39	83.54	0.80
WASH160	WASH160	241.22	100-Yr	83.00	2544.77	2545.52	2545.47	2545.67	0.030227	3.12	26.57	63.81	0.85
WASH160	WASH160	241.22	100-Yr Encroache	83.00	2544.77	2545.60	2545.53	2545.78	0.031461	3.36	24.72	48.59	0.83

HEC-RAS Plan Encroach Locations: User Defined (Continued)

River	Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Cntrl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Cfl
WASH180	WASH180	84.23	100-Yr	83.00	2539.96	2540.35	2540.33	2540.45	0.036432	2.47	33.55	144.28	0.90
WASH180	WASH180	84.23	100-Yr Encroache	83.00	2539.96	2540.35	2540.33	2540.45	0.036432	2.47	33.55	144.28	0.90
WASH180	WASH180	2258.25	100-Yr	40.00	2619.97	2621.19	2621.09	2621.48	0.024788	4.27	9.37	11.94	0.85
WASH180	WASH180	2258.25	100-Yr Encroache	40.00	2619.97	2621.19	2621.09	2621.48	0.025339	4.31	9.29	11.89	0.86
WASH180	WASH180	2081.05	100-Yr	40.00	2615.14	2616.86	2616.81	2617.16	0.023915	4.36	9.17	40.50	0.90
WASH180	WASH180	2081.05	100-Yr Encroache	40.00	2615.14	2616.87	2616.81	2617.16	0.023421	4.33	9.24	40.64	0.89
WASH180	WASH180	1966.66	100-Yr	40.00	2613.00	2613.39	2613.38	2613.53	0.042456	3.08	12.97	42.22	0.98
WASH180	WASH180	1966.66	100-Yr Encroache	40.00	2613.00	2613.38	2613.38	2613.54	0.043787	3.17	12.61	39.93	0.99
WASH180	WASH180	1847.09	100-Yr	40.00	2608.91	2609.28	2609.28	2609.41	0.028480	2.94	13.62	54.73	1.01
WASH180	WASH180	1847.09	100-Yr Encroache	40.00	2608.91	2609.28	2609.28	2609.42	0.027832	2.93	13.66	50.52	0.99
WASH180	WASH180	1641.95	100-Yr	41.00	2599.00	2600.13	2600.42	2600.99	0.062179	7.42	5.52	13.09	1.62
WASH180	WASH180	1641.95	100-Yr Encroache	41.00	2599.00	2600.12	2600.42	2601.00	0.063985	7.53	5.45	12.97	1.64
WASH180	WASH180	1484.69	100-Yr	41.00	2595.00	2596.24	2596.24	2596.50	0.016247	4.10	10.01	20.08	1.02
WASH180	WASH180	1484.69	100-Yr Encroache	41.00	2595.00	2596.24	2596.24	2596.50	0.016247	4.10	10.01	20.08	1.02
WASH180	WASH180	1403.09	100-Yr	41.00	2592.93	2593.35	2593.50	2593.81	0.095826	5.49	7.47	30.91	1.97
WASH180	WASH180	1403.09	100-Yr Encroache	41.00	2592.93	2593.35	2593.50	2593.81	0.095826	5.49	7.47	30.91	1.97
WASH180	WASH180	1332.58	100-Yr	41.00	2590.00	2590.76	2590.76	2590.96	0.024678	3.56	11.50	29.52	1.01
WASH180	WASH180	1332.58	100-Yr Encroache	41.00	2590.00	2590.76	2590.76	2590.96	0.025119	3.59	11.42	29.37	1.02
WASH180	WASH180	1198.56	100-Yr	24.00	2585.99	2586.51	2586.56	2586.77	0.050539	4.07	5.90	21.43	1.37
WASH180	WASH180	1198.56	100-Yr Encroache	24.00	2585.99	2586.51	2586.53	2586.76	0.048492	4.00	5.99	21.60	1.34
WASH180	WASH180	991.1	100-Yr	24.00	2578.74	2579.15	2579.15	2579.23	0.055257	2.22	10.80	61.68	0.94
WASH180	WASH180	991.1	100-Yr Encroache	24.00	2578.84	2579.26	2579.24	2579.36	0.046440	2.57	9.33	38.43	0.92
WASH180	WASH180	825.42	100-Yr	24.00	2572.91	2573.38	2573.38	2573.46	0.023844	2.31	10.41	64.28	1.01
WASH180	WASH180	825.42	100-Yr Encroache	24.00	2572.91	2573.36	2573.36	2573.47	0.028132	2.62	9.15	40.99	0.98
WASH180	WASH180	600.02	100-Yr	24.00	2564.79	2565.20	2565.27	2565.43	0.058579	3.87	6.20	28.30	1.46
WASH180	WASH180	600.02	100-Yr Encroache	24.00	2564.79	2565.22	2565.27	2565.42	0.046777	3.59	6.69	28.99	1.32
WASH180	WASH180	392.61	100-Yr	24.00	2557.84	2558.49	2558.49	2558.71	0.022101	3.71	6.47	15.42	1.01
WASH180	WASH180	392.61	100-Yr Encroache	24.00	2557.84	2558.48	2558.49	2558.71	0.023712	3.81	6.31	15.25	1.04
WASH180	WASH180	251.26	100-Yr	24.00	2551.11	2551.71	2551.96	2552.52	0.120751	7.22	3.32	9.21	2.12
WASH180	WASH180	251.26	100-Yr Encroache	24.00	2551.11	2551.72	2551.96	2552.46	0.106535	6.89	3.49	9.36	1.99
WASH180	WASH180	139.68	100-Yr	24.00	2549.00	2549.29	2549.29	2549.42	0.042070	2.93	8.19	31.25	1.01
WASH180	WASH180	139.68	100-Yr Encroache	24.00	2549.00	2549.29	2549.29	2549.42	0.042070	2.93	8.19	31.25	1.01
WASH180	WASH180	28.5	100-Yr	24.00	2543.97	2544.63	2544.72	2544.99	0.037318	4.86	4.94	11.71	1.32
WASH180	WASH180	28.5	100-Yr Encroache	24.00	2543.97	2544.63	2544.72	2544.99	0.037318	4.86	4.94	11.71	1.32
WASH215	WASH215	1770.11	100-Yr	308.00	2609.17	2610.54	2610.54	2610.93	0.036080	5.06	60.92	79.80	1.02
WASH215	WASH215	1770.11	100-Yr Encroache	308.00	2609.17	2610.54	2610.54	2610.93	0.036080	5.06	60.92	79.80	1.02
WASH215	WASH215	1570.3	100-Yr	308.00	2599.96	2601.46	2601.73	2602.43	0.051031	7.89	39.03	39.18	1.39
WASH215	WASH215	1570.3	100-Yr Encroache	308.00	2599.96	2601.47	2601.73	2602.43	0.050695	7.87	39.13	39.21	1.39
WASH215	WASH215	1411.45	100-Yr	308.00	2593.98	2596.53	2596.53	2597.32	0.028193	7.15	43.10	27.37	1.00
WASH215	WASH215	1411.45	100-Yr Encroache	308.00	2593.98	2596.53	2596.53	2597.32	0.028193	7.15	43.10	27.37	1.00
WASH215	WASH215	1216.32	100-Yr	308.00	2587.00	2589.55	2589.81	2590.81	0.039592	9.00	34.24	20.00	1.21
WASH215	WASH215	1216.32	100-Yr Encroache	308.00	2587.00	2589.55	2589.81	2590.81	0.039691	9.00	34.20	19.99	1.21
WASH215	WASH215	1071.77	100-Yr	308.00	2582.59	2583.74	2583.85	2584.34	0.047754	6.25	49.29	58.57	1.20
WASH215	WASH215	1071.77	100-Yr Encroache	308.00	2582.59	2583.74	2583.85	2584.34	0.047624	6.24	49.33	58.58	1.20
WASH215	WASH215	952.43	100-Yr	308.00	2577.74	2578.85	2578.90	2579.43	0.035759	6.07	50.70	51.30	1.08
WASH215	WASH215	952.43	100-Yr Encroache	308.00	2577.74	2578.85	2578.90	2579.43	0.035841	6.08	50.67	51.30	1.08
WASH215	WASH215	819.42	100-Yr	308.00	2572.82	2574.09	2574.21	2574.76	0.034315	6.58	46.84	49.98	1.20
WASH215	WASH215	819.42	100-Yr Encroache	308.00	2572.82	2574.09	2574.21	2574.76	0.034260	6.57	46.87	49.98	1.20
WASH215	WASH215	661.36	100-Yr	308.00	2567.30	2568.31	2568.44	2568.92	0.039719	6.23	49.45	63.29	1.24
WASH215	WASH215	661.36	100-Yr Encroache	308.00	2567.30	2568.31	2568.44	2568.92	0.039757	6.23	49.43	63.28	1.24
WASH215	WASH215	485.15	100-Yr	308.00	2561.69	2562.59	2562.68	2563.08	0.040321	5.61	54.91	78.79	1.18
WASH215	WASH215	485.15	100-Yr Encroache	308.00	2561.69	2562.59	2562.68	2563.08	0.040798	5.63	54.70	78.74	1.19
WASH215	WASH215	357.34	100-Yr	308.00	2556.72	2557.98	2557.99	2558.38	0.033396	5.11	60.26	81.34	1.05
WASH215	WASH215	357.34	100-Yr Encroache	308.00	2556.72	2557.98	2557.99	2558.38	0.033026	5.09	60.48	81.40	1.04
WASH215	WASH215	150.52	100-Yr	308.00	2549.92	2551.27	2551.27	2551.70	0.032599	5.26	58.60	70.29	1.01
WASH215	WASH215	150.52	100-Yr Encroache	308.00	2549.92	2551.27	2551.27	2551.70	0.032599	5.26	58.60	70.29	1.01
WASH215	WASH215	22.86	100-Yr	308.00	2545.43	2547.13	2547.31	2547.96	0.028015	7.31	42.12	38.29	1.23
WASH215	WASH215	22.86	100-Yr Encroache	308.00	2545.43	2547.13	2547.31	2547.96	0.028061	7.32	42.09	38.28	1.23

○ SITE BOUNDARY CROSS SECTION

HY-8 Culvert Calculation Output

HY-8 Culvert Analysis Report

Culvert: ON-155

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 60 cfs

Design Flow: 153 cfs

Maximum Flow: 153 cfs

Table 1 - Summary of Culvert Flows at Crossing: ON 155

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
2620.00	60.00	60.00	0.00	1
2620.17	69.30	69.30	0.00	1
2620.33	78.60	78.60	0.00	1
2620.49	87.90	87.90	0.00	1
2620.65	97.20	97.20	0.00	1
2620.80	106.50	106.50	0.00	1
2620.96	115.80	115.80	0.00	1
2621.12	125.10	125.10	0.00	1
2621.29	134.40	134.40	0.00	1
2621.47	143.70	143.70	0.00	1
2621.65	153.00	153.00	0.00	1
2622.00	169.17	169.17	0.00	Overtopping

Rating Curve Plot for Crossing: ON 155

Total Rating Curve

Crossing: ON 155

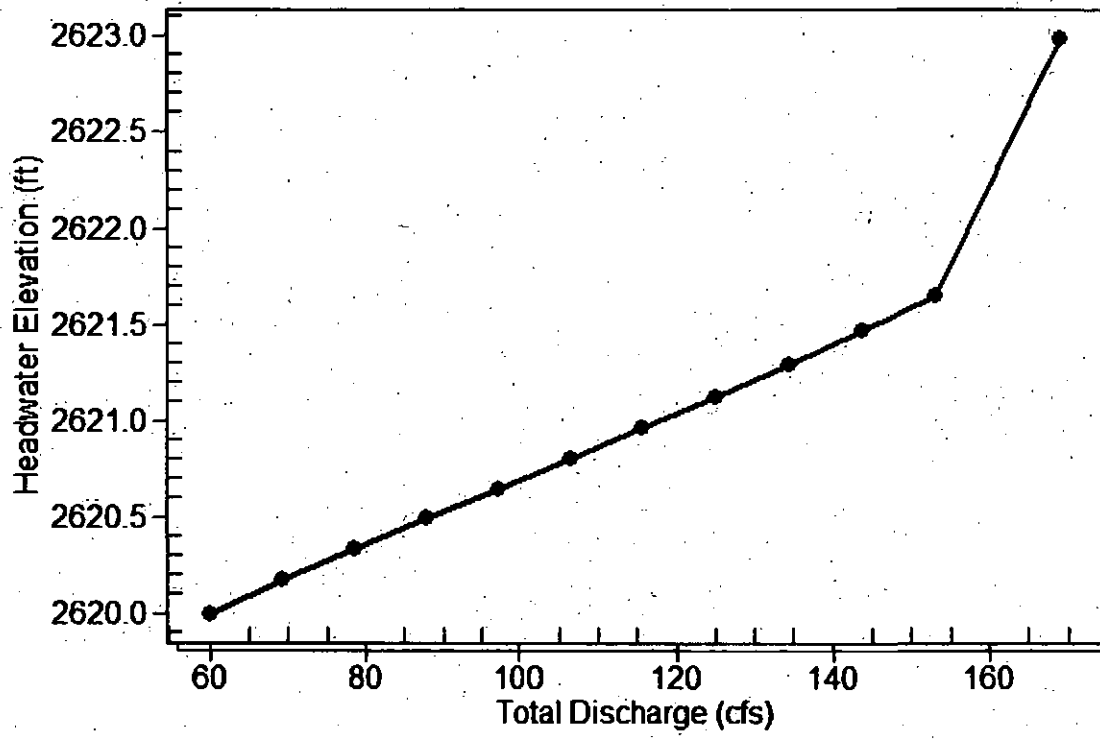


Table 2 - Culvert Summary Table: Culvert 1

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
60.00	60.00	2620.00	1.998	0.615	1-S2n	0.846	1.436	0.971	1.000	10.059	0.000
69.30	69.30	2620.17	2.171	0.786	1-S2n	0.907	1.546	1.055	1.000	10.399	0.000
78.60	78.60	2620.33	2.334	0.956	1-S2n	0.969	1.648	1.137	1.000	10.694	0.000
87.90	87.90	2620.49	2.491	1.134	1-S2n	1.030	1.748	1.215	1.000	10.906	0.000
97.20	97.20	2620.65	2.646	1.316	1-S2n	1.092	1.844	1.289	1.000	11.154	0.000
106.50	106.50	2620.80	2.800	1.499	1-S2n	1.144	1.933	1.362	1.000	11.371	0.000
115.80	115.80	2620.96	2.958	1.686	1-S2n	1.196	2.017	1.432	1.000	11.587	0.000
125.10	125.10	2621.12	3.120	1.882	5-S2n	1.249	2.101	1.499	1.000	11.805	0.000
134.40	134.40	2621.29	3.289	2.079	5-S2n	1.301	2.178	1.567	1.000	11.994	0.000
143.70	143.70	2621.47	3.466	2.656	5-S2n	1.353	2.251	1.632	1.000	12.185	0.000
153.00	153.00	2621.65	3.652	2.828	5-S2n	1.402	2.320	1.696	1.000	12.384	0.000

Straight Culvert

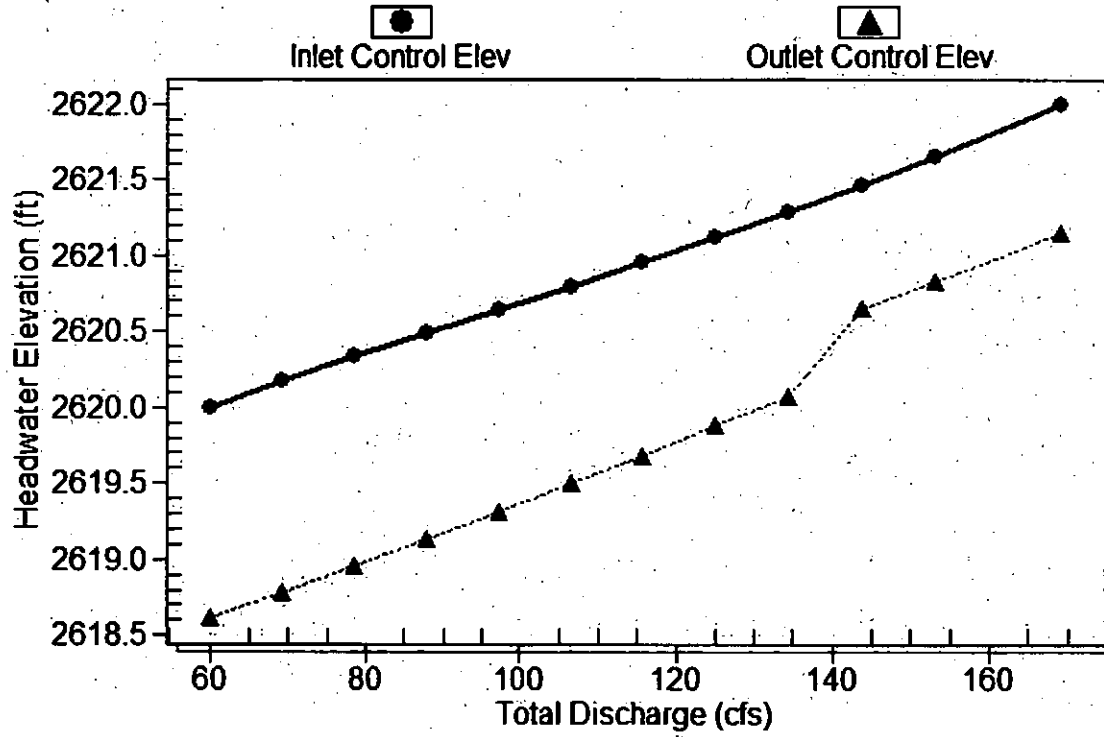
Inlet Elevation (invert): 2618.00 ft, Outlet Elevation (invert): 2617.00 ft

Culvert Length: 40.01 ft, Culvert Slope: 0.0250

Culvert Performance Curve Plot: Culvert 1

Performance Curve

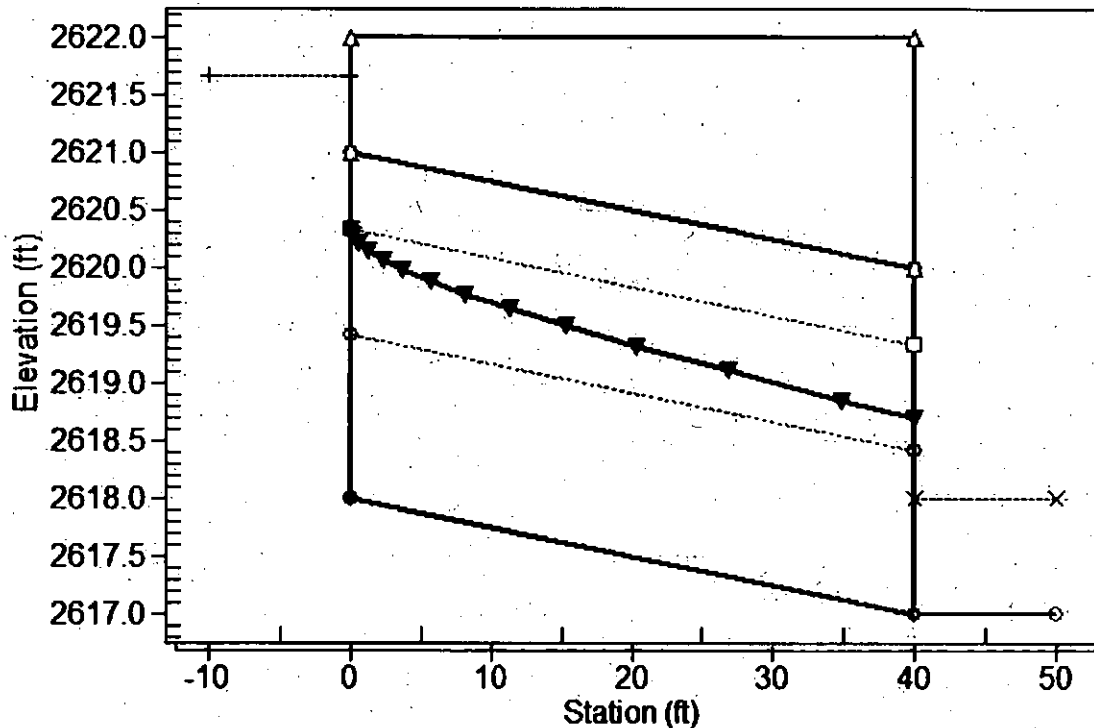
Culvert: Culvert 1



Water Surface Profile Plot for Culvert: Culvert 1

Crossing - ON 155, Design Discharge - 153.0 cfs

Culvert - Culvert 1, Culvert Discharge - 153.0 cfs



Site Data - Culvert 1

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 2618.00 ft

Outlet Station: 40.00 ft

Outlet Elevation: 2617.00 ft

Number of Barrels: 3

Culvert Data Summary - Culvert 1

Barrel Shape: Circular

Barrel Diameter: 3.00 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Grooved End in Headwall

Inlet Depression: NONE

Table 3 - Downstream Channel Rating Curve (Crossing: ON 155)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)
60.00	2618.00	1.00
69.30	2618.00	1.00
78.60	2618.00	1.00
87.90	2618.00	1.00
97.20	2618.00	1.00
106.50	2618.00	1.00
115.80	2618.00	1.00
125.10	2618.00	1.00
134.40	2618.00	1.00
143.70	2618.00	1.00
153.00	2618.00	1.00

Tailwater Channel Data - ON 155

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 2618.00 ft

Roadway Data for Crossing: ON 155

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 90.00 ft

Crest Elevation: 2622.00 ft

Roadway Surface: Paved

Roadway Top Width: 40.00 ft

Culvert: ON-160

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 23 cfs

Maximum Flow: 63 cfs

Table 4 - Summary of Culvert Flows at Crossing: ON160

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
2607.00	0.00	0.00	0.00	1
2608.48	6.30	6.30	0.00	1
2609.97	12.60	12.60	0.00	1
2612.57	18.90	18.90	0.00	1
2613.05	23.00	19.81	3.02	20
2613.12	31.50	19.93	11.46	5
2613.15	37.80	20.00	17.71	4
2613.19	44.10	20.06	23.81	3
2613.22	50.40	20.12	30.13	3
2613.25	56.70	20.17	36.45	3
2613.27	63.00	20.22	42.74	3
2613.00	19.72	19.72	0.00	Overtopping

Rating Curve Plot for Crossing: ON160

Total Rating Curve

Crossing: ON160

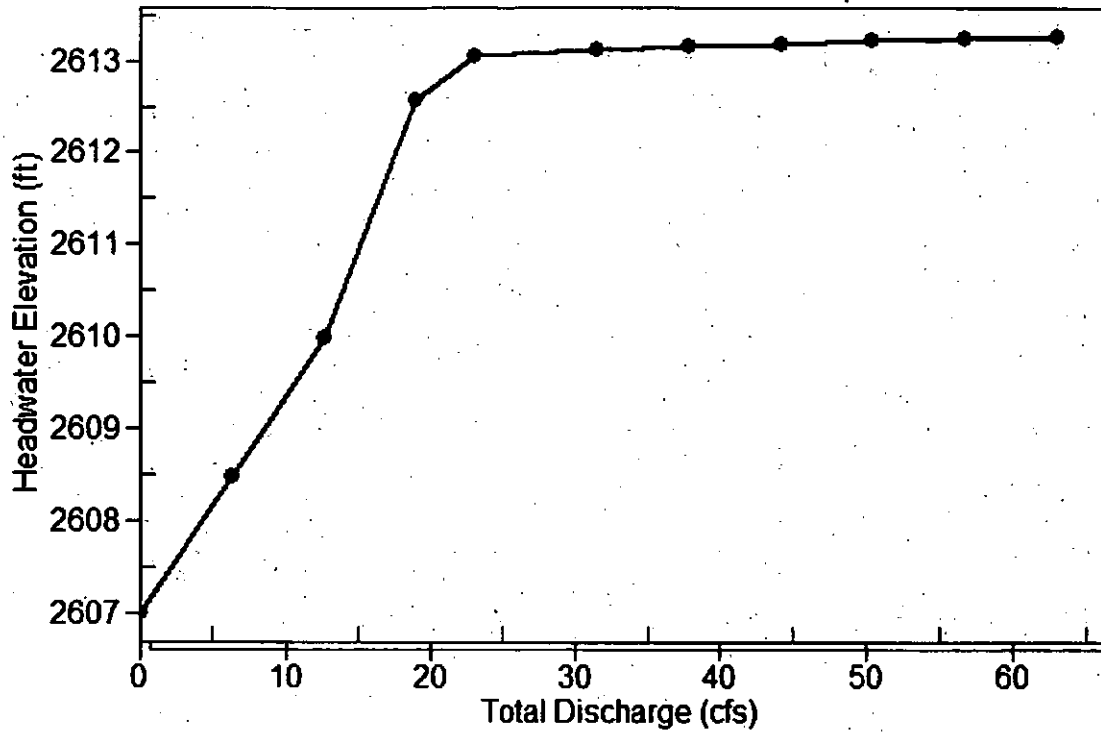


Table 5 - Culvert Summary Table: Culvert 1

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.00	0.00	2607.00	0.000	0.000	0-NF	0.000	0.000	1.000	1.000	0.000	0.000
6.30	6.30	2608.48	1.479	0.0*	1-S2n	0.505	0.965	0.554	1.000	10.623	0.000
12.60	12.60	2609.97	2.974	1.091	5-S2n	0.744	1.337	0.837	1.000	12.433	0.000
18.90	18.90	2612.57	5.574	3.239	5-S2n	0.962	1.453	1.085	1.000	13.814	0.000
23.00	19.81	2613.05	6.047	3.632	5-S2n	0.995	1.500	1.168	1.000	13.463	0.000
31.50	19.93	2613.12	6.115	3.685	5-S2n	1.000	1.500	1.171	1.000	13.497	0.000
37.80	20.00	2613.15	6.153	3.715	5-S2n	1.002	1.500	1.173	1.000	13.516	0.000
44.10	20.06	2613.19	6.186	3.740	5-S2n	1.005	1.500	1.126	1.000	14.076	0.000
50.40	20.12	2613.22	6.217	3.765	5-S2n	1.007	1.500	1.155	1.000	13.809	0.000
56.70	20.17	2613.25	6.247	3.787	5-S2n	1.009	1.500	1.198	1.000	13.347	0.000
63.00	20.22	2613.27	6.274	3.809	5-S2n	1.010	1.500	1.010	1.000	15.993	0.000

* Full Flow Headwater elevation is below inlet invert.

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Straight Culvert

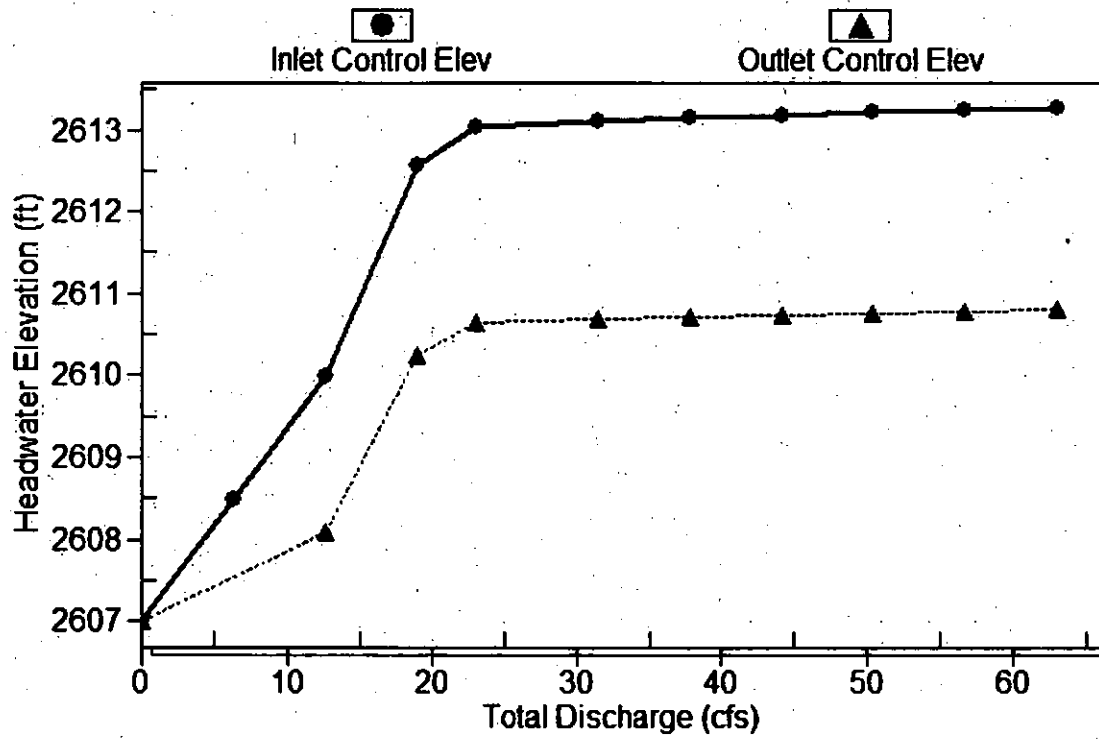
Inlet Elevation (invert): 2607.00 ft, Outlet Elevation (invert): 2605.00 ft

Culvert Length: 40.05 ft, Culvert Slope: 0.0500
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Culvert Performance Curve Plot: Culvert 1

Performance Curve

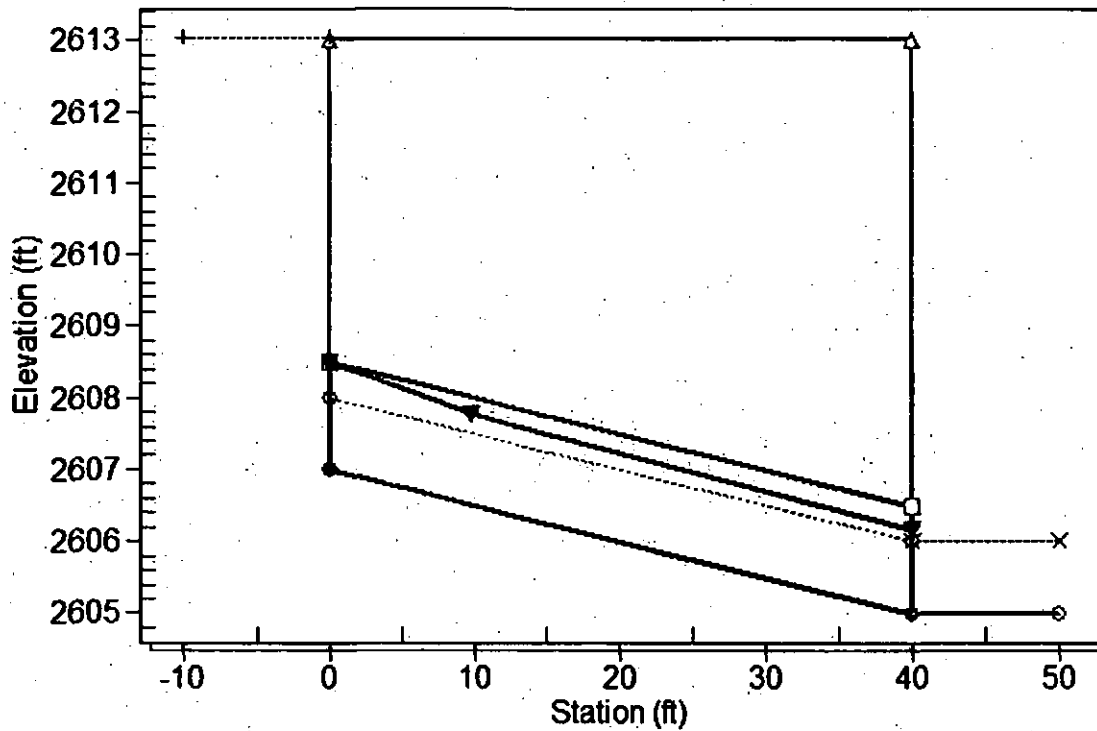
Culvert: Culvert 1



Water Surface Profile Plot for Culvert: Culvert 1

Crossing - ON160, Design Discharge - 23.0 cfs

Culvert - Culvert 1, Culvert Discharge - 19.8 cfs



Site Data - Culvert 1

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 2607.00 ft

Outlet Station: 40.00 ft

Outlet Elevation: 2605.00 ft

Number of Barrels: 1

Culvert Data Summary - Culvert 1

Barrel Shape: Circular

Barrel Diameter: 1.50 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: NONE

Table 6 - Downstream Channel Rating Curve (Crossing: ON160)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)
0.00	2606.00	1.00
6.30	2606.00	1.00
12.60	2606.00	1.00
18.90	2606.00	1.00
23.00	2606.00	1.00
31.50	2606.00	1.00
37.80	2606.00	1.00
44.10	2606.00	1.00
50.40	2606.00	1.00
56.70	2606.00	1.00
63.00	2606.00	1.00

Tailwater Channel Data - ON160

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 2606.00 ft

Roadway Data for Crossing: ON160

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 100.00 ft

Crest Elevation: 2613.00 ft

Roadway Surface: Paved

Roadway Top Width: 40.00 ft

Culvert: ON-182

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 18 cfs

Maximum Flow: 39 cfs

Table 7 - Summary of Culvert Flows at Crossing: ON182

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
2593.00	0.00	0.00	0.00	1
2594.06	3.90	3.90	0.00	1
2594.73	7.80	7.80	0.00	1
2595.68	11.70	11.70	0.00	1
2597.00	15.60	15.49	0.00	75
2597.06	18.00	15.63	2.30	7
2597.12	23.40	15.80	7.55	5
2597.16	27.30	15.89	11.36	4
2597.19	31.20	15.97	15.10	3
2597.22	35.10	16.05	18.97	3
2597.25	39.00	16.12	22.84	3
2597.00	15.49	15.49	0.00	Overtopping

Rating Curve Plot for Crossing: ON182

Total Rating Curve

Crossing: ON182

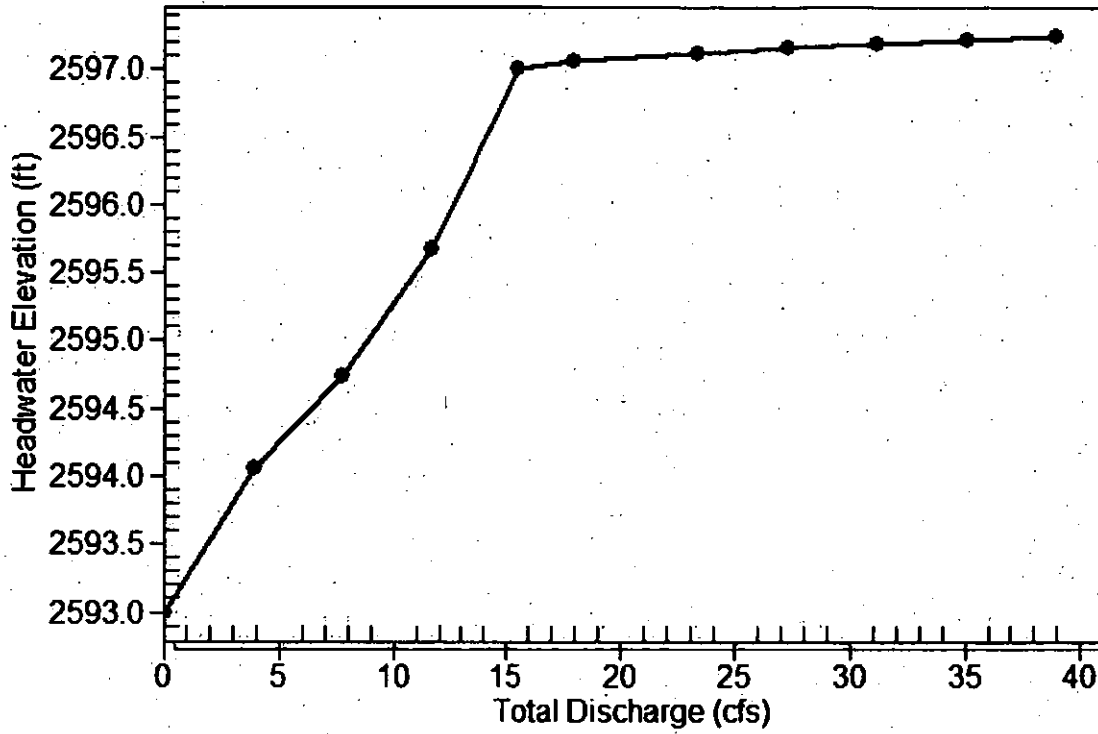


Table 8 - Culvert Summary Table: Culvert 1

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.00	0.00	2593.00	0.000	0.000	0-NF	0.000	0.000	1.000	1.000	0.000	0.000
3.90	3.90	2594.06	1.060	0.0*	1-S2n	0.352	0.755	0.375	1.000	11.209	0.000
7.80	7.80	2594.73	1.731	0.0*	5-S2n	0.508	1.078	0.565	1.000	12.835	0.000
11.70	11.70	2595.68	2.677	0.0*	5-S2n	0.634	1.301	0.720	1.000	13.970	0.000
15.60	15.49	2597.00	3.999	0.985	5-S2n	0.746	1.412	0.857	1.000	14.845	0.000
18.00	15.63	2597.06	4.056	1.032	5-S2n	0.750	1.414	0.861	1.000	14.892	0.000
23.40	15.80	2597.12	4.122	1.092	5-S2n	0.754	1.425	0.868	1.000	14.905	0.000
27.30	15.89	2597.16	4.159	1.120	5-S2n	0.757	1.419	0.869	1.000	14.984	0.000
31.20	15.97	2597.19	4.193	1.148	5-S2n	0.759	1.420	0.872	1.000	14.997	0.000
35.10	16.05	2597.22	4.225	1.179	5-S2n	0.761	1.429	0.875	1.000	14.997	0.000
39.00	16.12	2597.25	4.254	1.199	5-S2n	0.763	1.423	0.878	1.000	15.018	0.000

* Full Flow Headwater elevation is below inlet invert.

Straight Culvert

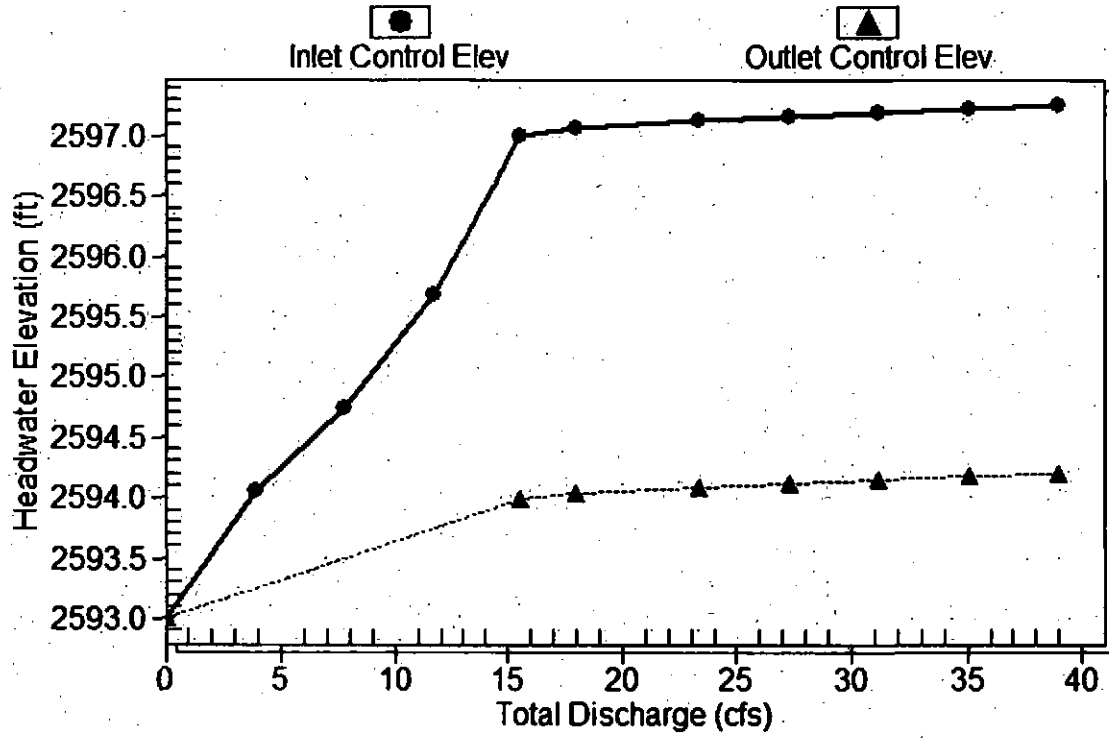
Inlet Elevation (invert): 2593.00 ft, Outlet Elevation (invert): 2590.00 ft

Culvert Length: 40.11 ft, Culvert Slope: 0.0750

Culvert Performance Curve Plot: Culvert 1

Performance Curve

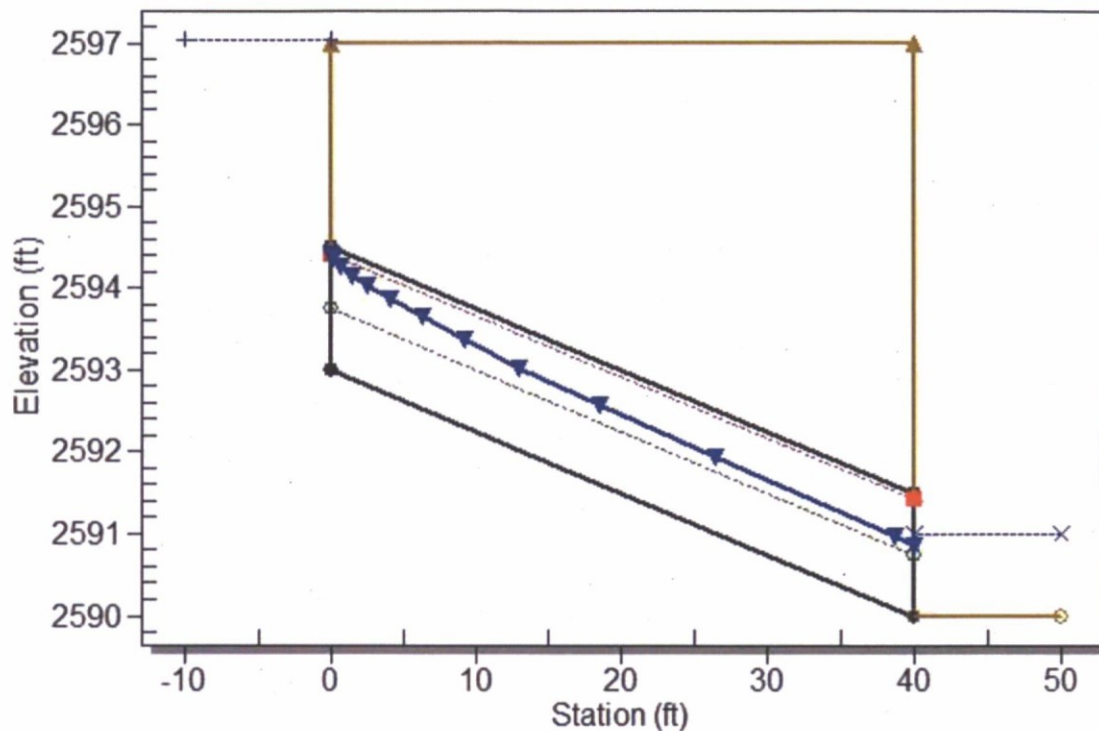
Culvert: Culvert 1



Water Surface Profile Plot for Culvert: Culvert 1

Crossing - ON182, Design Discharge - 18.0 cfs

Culvert - Culvert 1, Culvert Discharge - 15.6 cfs



Site Data - Culvert 1

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 2593.00 ft

Outlet Station: 40.00 ft

Outlet Elevation: 2590.00 ft

Number of Barrels: 1

Culvert Data Summary - Culvert 1

Barrel Shape: Circular

Barrel Diameter: 1.50 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: NONE

Table 9 - Downstream Channel Rating Curve (Crossing: ON182)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)
0.00	2591.00	1.00
3.90	2591.00	1.00
7.80	2591.00	1.00
11.70	2591.00	1.00
15.60	2591.00	1.00
18.00	2591.00	1.00
23.40	2591.00	1.00
27.30	2591.00	1.00
31.20	2591.00	1.00
35.10	2591.00	1.00
39.00	2591.00	1.00

Tailwater Channel Data - ON182

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 2591.00 ft

Roadway Data for Crossing: ON182

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 60.00 ft

Crest Elevation: 2597.00 ft

Roadway Surface: Paved

Roadway Top Width: 40.00 ft

Culvert: ON-183

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 0 cfs

Design Flow: 18 cfs

Maximum Flow: 39 cfs

Table 10 - Summary of Culvert Flows at Crossing: ON183

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert 1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
2556.00	0.00	0.00	0.00	1
2557.10	3.90	3.90	0.00	1
2557.77	7.80	7.80	0.00	1
2558.07	11.70	9.19	2.45	12
2558.12	15.60	9.42	6.12	5
2558.15	18.00	9.54	8.41	4
2558.20	23.40	9.77	13.58	4
2558.24	27.30	9.92	17.29	3
2558.27	31.20	10.05	21.09	3
2558.30	35.10	10.18	24.89	3
2558.33	39.00	10.29	28.69	3
2558.00	8.89	8.89	0.00	Overtopping

Total Rating Curve

Crossing: ON183

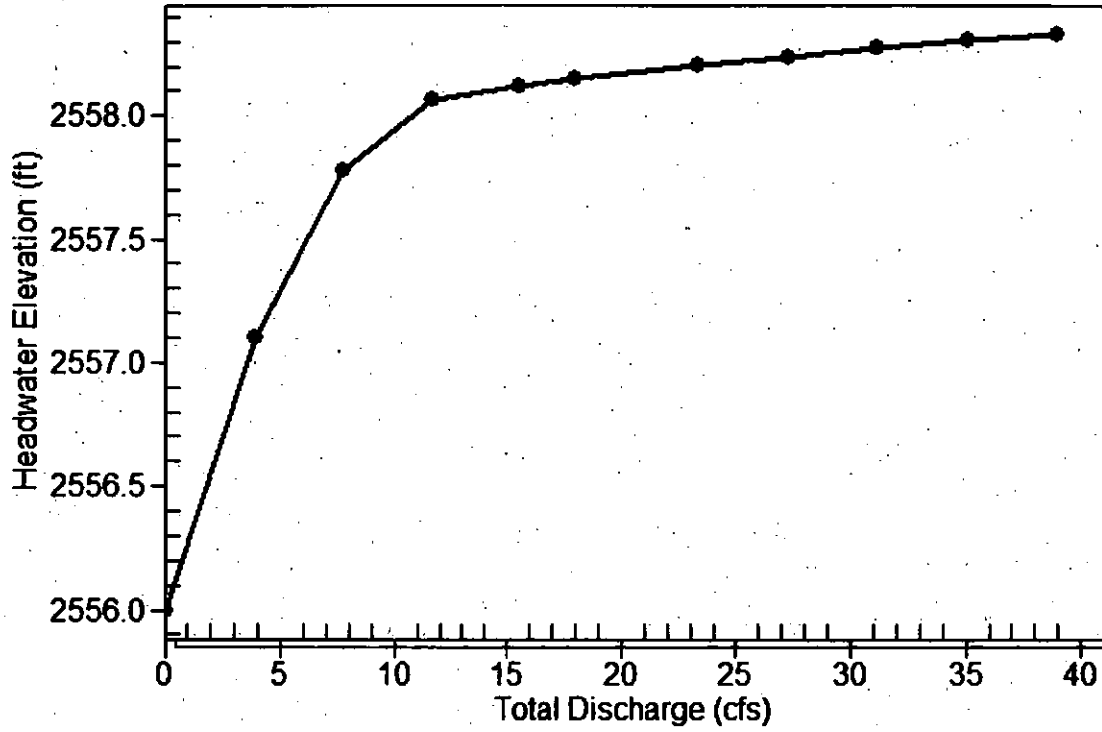


Table 11 - Culvert Summary Table: Culvert 1

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.00	0.00	2556.00	0.000	0.000	0-NF	0.000	0.000	1.000	1.000	0.000	0.000
3.90	3.90	2557.10	1.104	0.184	1-S2n	0.525	0.755	0.536	1.000	6.874	0.000
7.80	7.80	2557.77	1.775	0.812	5-S2n	0.776	1.078	0.799	1.000	8.146	0.000
11.70	9.19	2558.07	2.065	1.352	5-S2n	0.859	1.167	0.885	1.000	8.470	0.000
15.60	9.42	2558.12	2.120	1.414	5-S2n	0.873	1.185	0.900	1.000	8.519	0.000
18.00	9.54	2558.15	2.148	1.445	5-S2n	0.880	1.192	0.907	1.000	8.544	0.000
23.40	9.77	2558.20	2.203	1.505	5-S2n	0.894	1.205	0.921	1.000	8.590	0.000
27.30	9.92	2558.24	2.238	1.544	5-S2n	0.902	1.213	0.930	1.000	8.618	0.000
31.20	10.05	2558.27	2.271	1.580	5-S2n	0.910	1.221	0.939	1.000	8.643	0.000
35.10	10.18	2558.30	2.302	1.613	5-S2n	0.918	1.227	0.946	1.000	8.665	0.000
39.00	10.29	2558.33	2.332	1.645	5-S2n	0.924	1.234	0.954	1.000	8.685	0.000

 Straight Culvert

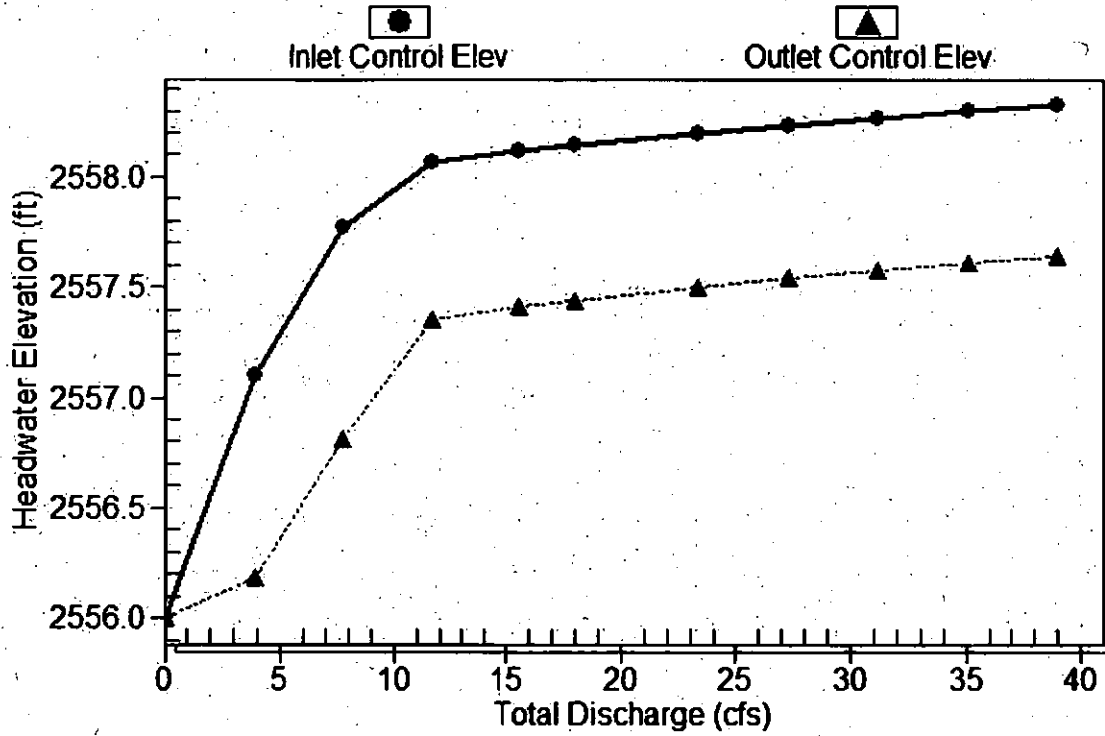
Inlet Elevation (invert): 2556.00 ft, Outlet Elevation (invert): 2555.00 ft

Culvert Length: 60.01 ft, Culvert Slope: 0.0167

Culvert Performance Curve Plot: Culvert 1

Performance Curve

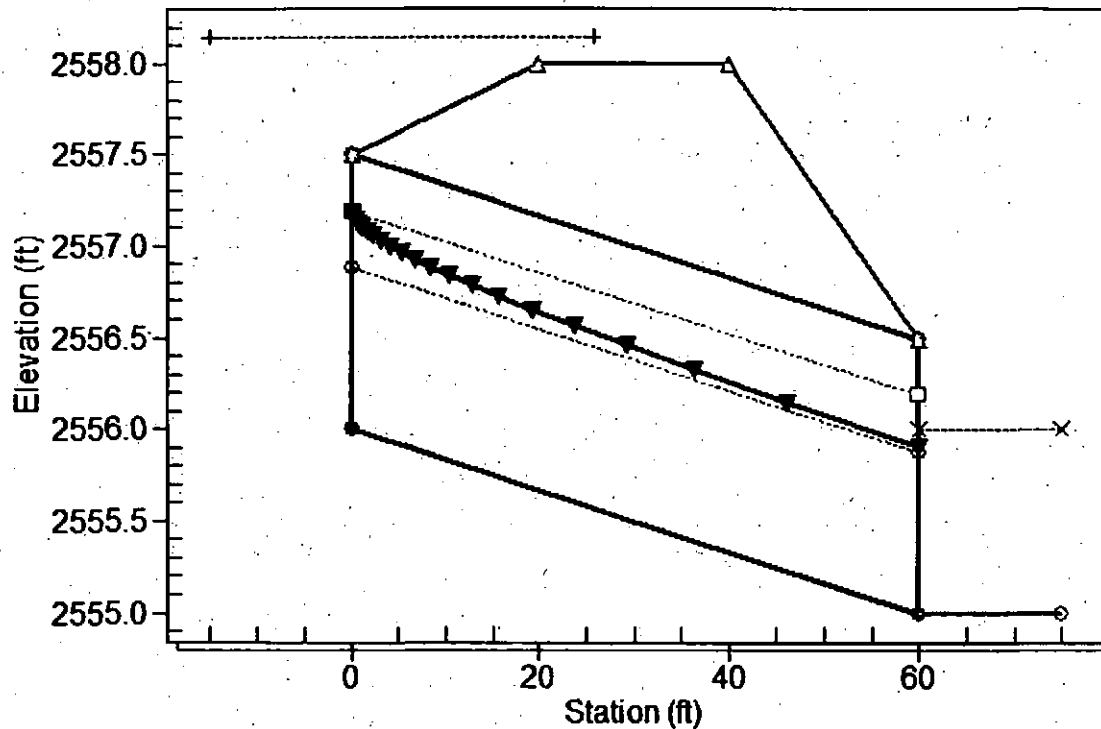
Culvert: Culvert 1



Water Surface Profile Plot for Culvert: Culvert 1

Crossing - ON183, Design Discharge - 18.0 cfs

Culvert - Culvert 1, Culvert Discharge - 9.5 cfs



Site Data - Culvert 1

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 2556.00 ft

Outlet Station: 60.00 ft

Outlet Elevation: 2555.00 ft

Number of Barrels: 1

Culvert Data Summary - Culvert 1

Barrel Shape: Circular

Barrel Diameter: 1.50 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: NONE

Table 12 - Downstream Channel Rating Curve (Crossing: ON183)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)
0.00	2556.00	1.00
3.90	2556.00	1.00
7.80	2556.00	1.00
11.70	2556.00	1.00
15.60	2556.00	1.00
18.00	2556.00	1.00
23.40	2556.00	1.00
27.30	2556.00	1.00
31.20	2556.00	1.00
35.10	2556.00	1.00
39.00	2556.00	1.00

Tailwater Channel Data - ON183

Tailwater Channel Option: Enter Constant Tailwater Elevation

Constant Tailwater Elevation: 2556.00 ft

Roadway Data for Crossing: ON183

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft

Crest Elevation: 2558.00 ft

Roadway Surface: Paved

Roadway Top Width: 20.00 ft

First Flush Spillway/Dissipation Basin Design

Project **Storyrock**

Subject **First Flush Spillway/Dissipation Basin Design**

Designed by **ZJH**

Date **2/5/2016**

Project No. **191069020**

Checked by **JMB**

Date **2/5/2016**

Objective: Design First Flush Spillway & Dissipation Basin for Typical Area

First Flush Equivalent Design Storm: 2 Year

$Q_2 = 2-8$ cfs

Spillway Design:

Top Width 8 Feet
Side Slopes 4:1 H:V
Flow Depth
(2 year Design Storm) 0.5 Feet
Capacity⁽¹⁾
(2 year Design Storm) 5 CFS

Dissipation Basin Design:

V = Spillway Velocity⁽¹⁾ 5.25 ft/s
D = Equivalent Opening Width 4 Feet
Riprap $D_{50} = 0.0191 * V^2 * (0.61)^{(2)}$ 6 Inches
Basin Length = $4xD^{(3)}$ 16 Feet

Notes:

- (1) Refer to Attached Flowmaster Output for Spillway Hydraulic Design
- (2) Per Drainage Design Manual - Hydraulics Equation 6.36, Specific Weight of Stone = 165 lb/ft³
- (3) Per Drainage Design Manual - Hydraulics Table 8.6

Worksheet for First Flush Spillway

Project Description

Friction Method Manning Formula
Solve For Discharge

Input Data

Roughness Coefficient	0.055	
Channel Slope	0.25000	ft/ft
Normal Depth	0.50	ft
Left Side Slope	4.00	ft/ft (H:V)
Right Side Slope	4.00	ft/ft (H:V)
Bottom Width	0.00	ft

Results

Discharge	5.25	ft ³ /s
Flow Area	1.00	ft ²
Wetted Perimeter	4.12	ft
Hydraulic Radius	0.24	ft
Top Width	4.00	ft
Critical Depth	0.64	ft
Critical Slope	0.06711	ft/ft
Velocity	5.25	ft/s
Velocity Head	0.43	ft
Specific Energy	0.93	ft
Froude Number	1.85	
Flow Type	Supercritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.50	ft
Critical Depth	0.64	ft
Channel Slope	0.25000	ft/ft

Appendix D – Stormwater Storage Waiver



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA - - ZN - - UP - - DR - - PP - PC#

The applicant/developer must complete and submit this form to the city for processing and obtain approval of waiver request **before submitting improvement plans**. Denial of the waiver may require the developer to submit a revised site plan to the Development Review Board.

Date 10/26/2016 Project Name Storyrock
 Project Location 128th Street and Ranch Gate Road
 Applicant Contact Jason Burm, PE Company Name Kimley-Horn and Associates
 Phone 480-207-2667 Fax _____ E-mail jason.burm@kimley-horn.com
 Address 1855 W. Baseline Road, Suite 200 Mesa, AZ 85202

Waiver Criteria

A project must meet at least one of three criteria listed below for the city to consider waiving some or all required stormwater storage. **However, regardless of the criteria, a waiver will only be granted if the applicant can demonstrate that the effect of a waiver will not increase the potential for flooding on any property.** Check the applicable box and provide a signed engineering report and supporting engineering analysis that demonstrate the project meets the criteria and that the effect of a waiver will not increase the potential for flooding on any property.

If the runoff for the project has been included in a storage facility at another location, the applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility.

- 1. The development is adjacent to a conveyance facility that an engineering analysis shows is designed and constructed to handle the additional runoff from the site as a result of development.
- 2. The development is on a parcel less than one-half acre in size.
- 3. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO).

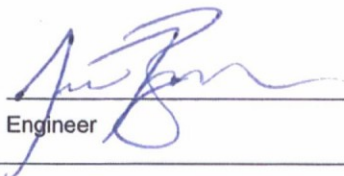
For a full storage waiver, a conflict with ESLO is limited to:

- Property located in the hillside landform as defined in the city Zoning Ordinance
- Property in the upper desert landform that has a land slope steeper than 5% as defined in the city Zoning Ordinance
- Property within the ESL zoning overlay district where the only viable location for a stormwater storage basin requires blasting

This full waiver only applies to those portions of property meeting one of these three requirements.

Partial waivers are available for projects or portions of properties within the Environmentally Sensitive Lands Zoning Overlay District, not meeting any of the three full waiver criteria above, if post-development peak discharge rates do not exceed pre-development conditions, based on the 10- and 100-year storm events.

By signing below, I certify that the stated project meets the waiver criteria selected above as demonstrated by the attached documentation.



 Engineer

10.27.16

 Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA -

- ZN -

- UP -

- DR -

- PP -

PC#

CITY STAFF TO COMPLETE THIS PAGE

Project Name _____

Check Appropriate Boxes:

Meets waiver criteria (specify): 1 2 3

Recommend approve waiver.

Recommend deny waiver:

None of waiver criteria met.

Downstream conditions prohibit waiver of any storage.

Other:

Explain: _____

Return waiver request:

Insufficient data provided.

Other: _____

Explain: _____

Recommended Conditions of Waiver:

All storage requirements waived.

Post-development peak discharge rates do not exceed pre-development conditions.

Other:

Explain: _____

Waiver approved per above conditions.

Waiver denied.

Floodplain Administrator or Designee

Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA - - ZN - - UP - - DR - - PP - PC#

In-Lieu Fee and In-Kind Contributions

In-lieu fees are only applicable to projects where post-development peak discharge rates exceed pre-development levels, based on the 10- and 100-year storm events. If the city grants a waiver, the developer is required to calculate and contribute an in-lieu fee based on what it would cost the city to provide a storage basin, sized as described below, including costs such as land acquisition, construction, landscaping, design, construction management, and maintenance over a 75-year design life. The fee for this cost is \$1.87 per cubic foot of stormwater storage for a virtual storage basin designed to mitigate the increase in runoff associated with the 100-year/2-hour storm event. The applicant may submit site-specific in-lieu fee calculations subject to the Floodplain Administrator's approval.

The Floodplain Administrator considers in-kind contributions on a case-by-case basis. An in-kind contribution can serve as part of or instead of the calculated in-lieu fee. In-kind contributions must be stormwater related and must constitute a public benefit. In-lieu fees and in-kind contributions are subject to the approval of the Floodplain Administrator or designee.

Project Name STARBUCK

The waived stormwater storage volume is calculated using a simplified approach as follows:

V = ΔCRA; where

V = stormwater storage volume required, in cubic feet,

ΔC = increase in weighted average runoff coefficient over disturbed area ($C_{post} - C_{pre}$),

R = 100-year/2-hour precipitation depth, in feet (DSPM, Appendix 4-1D, page 11), and

A = area of disturbed ground, in square feet

Furthermore,

$V_w = V - V_p$; where

V_w = volume waived,

V = volume required, and

V_p = volume provided

R = _____

ΔC = _____

A = _____

V = _____

V_p = _____

V_w = _____

An in-lieu fee will be paid, based on the following calculations and supporting documentation:

In-lieu fee (\$) = V_w (cu. ft.) x \$1.87 per cubic foot = _____

An in-kind contribution will be made, as follows:

No in-lieu fee is required. Reason:

Approved by:

Floodplain Administrator or Designee

Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781

Appendix E – Preliminary Grading Plan