

PRELIMINARY DRAINAGE REPORT

Asteria Highlands

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ASTERIA HIGHLANDS

DECEMBER 2017

Prepared By:



Expires 06/30/19

Kimley»»Horn

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INTRODUCTION

SITE LOCATION

This Preliminary Drainage report has been prepared for the proposed Asteria Highlands development (Development). The Development is bound to the east by 128th Street, to the west by a proposed development (Sereno Canyon), and to the north and south by an undeveloped residential property. The Story Rock development is on the east side of 128th Street. The Development is located within Section 11 of Township 4 North, Range 5 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. Refer to **Figure 1A** and **Figure 1B** for the Location Map and Aerial Photo Map, respectively.

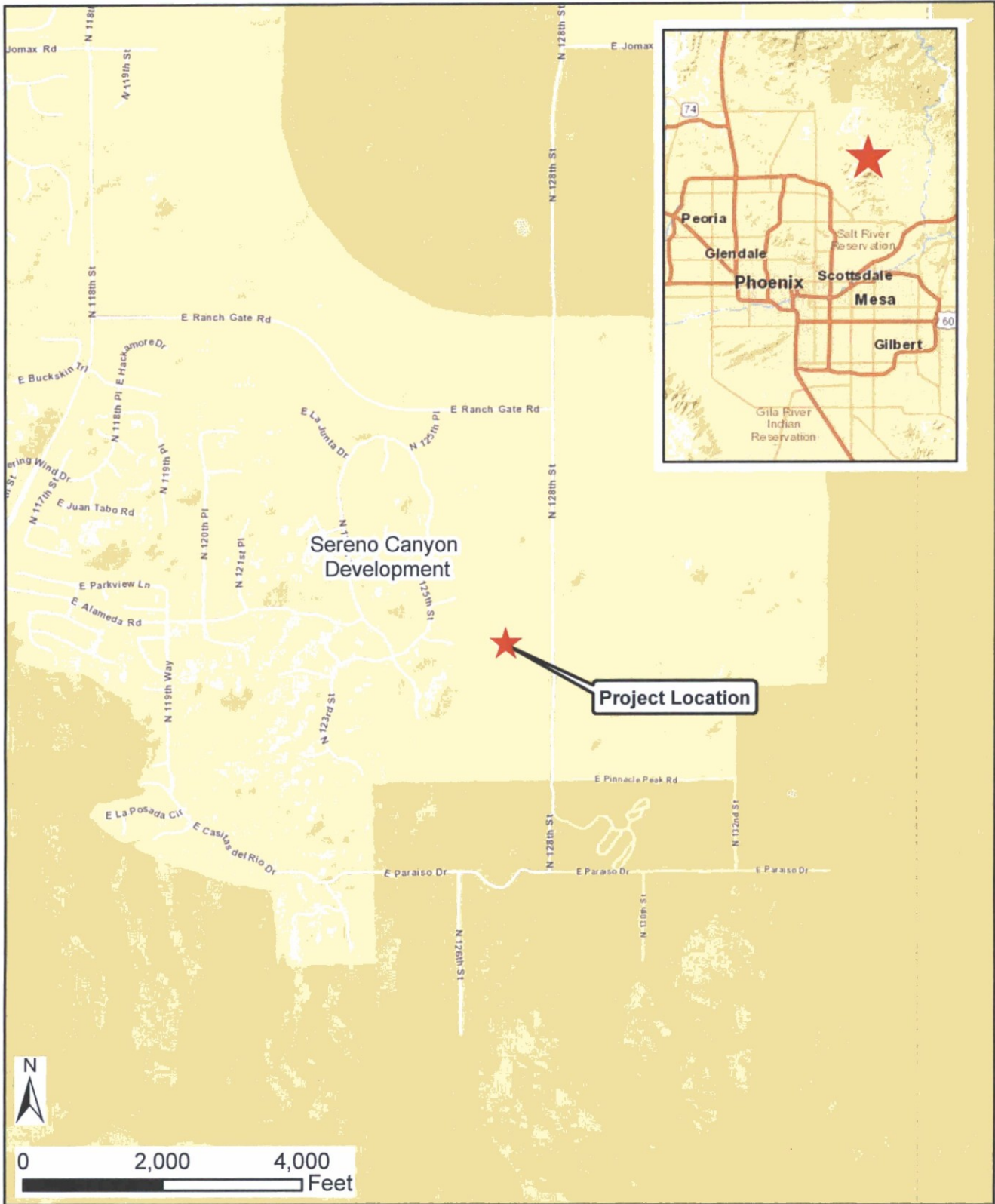
PROJECT SIZE AND TYPE

The Development is a proposed 40-acre custom-lot residential subdivision. The proposed development consists of 31 residential units. The majority of the Development is zoned R1-35 ESL. Lots one (1) through four (4) are zoned R1-70. The proposed site is located within the City of Scottsdale (City) and falls under the City's Environmentally Sensitive Lands Ordinance (ESLO).

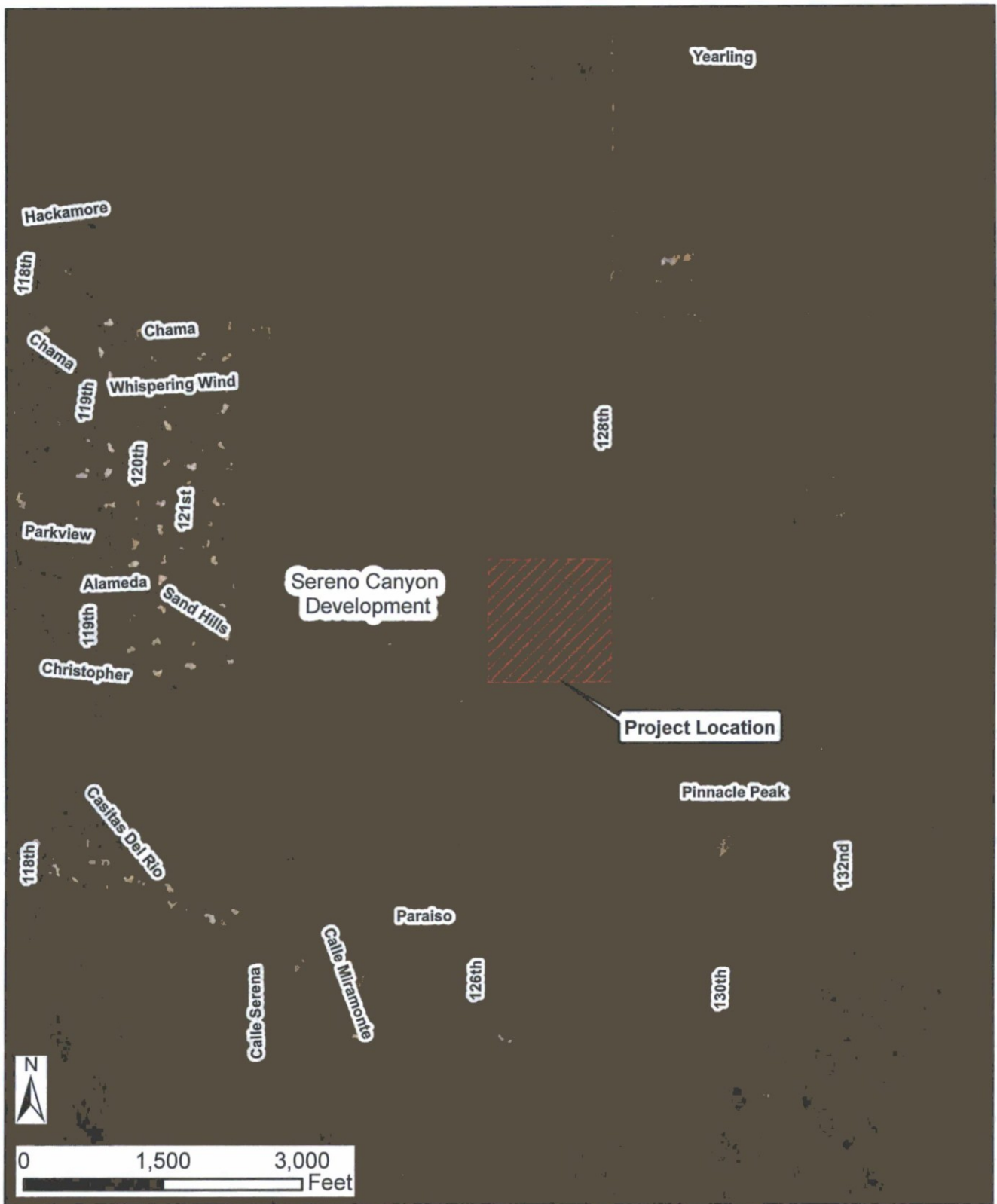
PURPOSE AND OBJECTIVES


This report establishes drainage parameters and criteria for the Development. This report establishes a general hydrologic and hydraulic plan for the development of the site.

- Demonstrate compliance with the City's Design Standards & Polices Manual (DSPM)
- Quantify offsite runoff from being conveyed through the existing property
- Establish drainage parameters and criteria for design.
- Provide a hydraulic analysis for the Development.



 Expect More. Experience Better.	Asteria Highlands	Scottsdale, AZ
	Figure 1A. Location Map	



 Expect More. Experience Better.	Asteria Highlands	Scottsdale, AZ
	Figure 1B. Aerial Photo Map	

DESCRIPTION OF EXISTING DRAINAGE CONDITIONS AND CHARACTERISTICS

EXISTING ONSITE CONDITIONS

The Development consists of undeveloped natural desert. The Development slopes from southwest to northeast with an average slope of 3.9%. Three significant washes cross the site. Significant washes are defined as having a 100-year peak discharge of 50 cubic feet per second (cfs) or more. All three significant washes flow from southwest to northeast, discharging onto 128th Street. Refer to **Figure 2** for the Existing Conditions Map.

EXISTING OFFSITE DRAINAGE CONDITIONS

Offsite runoff originates southwest of the development from large, undeveloped sub-basins. The offsite basins are part of the McDowell Sonoran Preserve. Offsite runoff is conveyed through the site by three (3) significant washes. Refer to **Figure 2**.

There are no washes with 100-year peak discharges of 750 cfs or greater, which indicates that no Vista Corridors exist within the project area. Existing conditions hydrologic results can be found in **Appendix A**.

CONTEXT RELATIVE TO ADJACENT PROJECTS AND IMPROVEMENTS

Runoff discharges east to 128th Street and north to the adjacent undeveloped parcel. Runoff crossing 128th Street will be conveyed through the Story Rock development. The Story Rock development is currently under final design. The Story Rock development will be constructing a portion of the 128th Street improvements including several culvert crossings. The culvert crossings will be conveying runoff from the Development and are included in the hydraulic analysis completed for this report.

FLOOD HAZARD ZONES ON PROPERTY, FIRM MAPS

The development is located within one flood zone as shown on Flood Insurance Rate Map (FIRM) panel number 04013C1335M dated November 4, 2015. Panel 04013C1335M is not printed, because no special flood hazard areas exist in its area. The Development is in a Zone D flood zone. Zone D flood zones are classified as "Areas in which flood hazards are undetermined, but possible."

PROPOSED DRAINAGE PLAN

PROPOSED ONSITE DRAINAGE PLAN

The Development consists of 31 single-family units. Runoff will be conveyed in the streets to the northeast corner of the site. Runoff will be collected in a storm drain system and discharged to detention basins.

The detention basin at the northeast corner of the site, SON05, will be used to collect onsite runoff from a storm drain system. This basin will have an 18-inch bleed off pipe with a 12-inch orifice plate to discharge runoff to a northern exit point. The detention basin will overtop during the 100-year storm event. The overflow will be conveyed towards the existing conditions discharge point from the site.

Onsite runoff in the southern portion of the Development will be collected in detention basin SON18 that will have two 18-inch outlet pipes. The runoff reaching this detention basin is being routed to two separate exit points on the site to maintain pre-development conditions. In the existing condition, two subbasins with separate exit points make up the proposed subbasin ON18. Both outlet pipes will have 12-inch orifice plates. One will discharge to CP15 and the other to CP20.

The post development peak discharges exiting the site are equal to or less than pre-development peak discharges, except for one location: CP10 in the 2-year and 10-year storm events. The post-development discharge exceeds the pre-development discharges by one (1) cfs in both cases, but meets the 100-year pre-development discharge. Four (4) culverts will convey runoff across 128th Street to the Story Rock development. The roadway and culvert are being designed with the Story Rock development.

Lots located adjacent to the significant wash will have finished floor elevations a minimum of one foot above the 100-year base flood elevation (BFE). Refer to Figure 4 for the Proposed Conditions Drainage Map.

PROPOSED ONSITE HYDROLOGY

Onsite runoff maintains post-development peak discharges at or below pre-development conditions at exit points for the Development, except for CP10 in the 2-year and 10-year storm events. A normal depth calculation was done for the pre- and post-project conditions for the wash downstream of the exit point to determine the impacts. The results show that the rise in WSEL for CP10 is 0.01 feet in both the 2-year and 10-year storm events. The velocity increases for CP10 are 0.07 fps and 0.03 fps respectively. See **Appendix D** for normal depth calculations. These changes in WSEL and velocity are negligible, and downstream drainage structures are designed for the 10-year storm event.

Onsite runoff is routed to detention basins to attenuate runoff. A summary of the pre- and post-development peak discharges is provided in **Table 1**. Refer to **Appendix A** and **Appendix B** for the detailed hydrologic model results.

Table 1. Peak Discharge Summary

HEC-1 Combination Point (Pre/Post)	Pre Q ₂ [cfs]	Post Q ₂ [cfs]	Pre Q ₁₀ [cfs]	Post Q ₁₀ [cfs]	Pre Q ₁₀₀ [cfs]	Post Q ₁₀₀ [cfs]
ON07/ON07	2	1	5	3	9	6
CP05/SON05	5	2	14	5	31	19
CP10/CP10	14	15	40	41	99	98
ON12/ON12	1	1	3	3	7	6
ON15/CP15	3	3	9	8	20	15
CP20/CP20	20	19	56	55	138	134
CP25/CP25	9	9	25	24	62	60

PROPOSED ONSITE HYDRAULICS

Onsite runoff will be conveyed in the local streets to two storm drain systems. One is located at the northeast corner of the site. The other is located at the southeast corner. Per the DSPM, all interior streets will be designed to convey the peak discharge from the 10-year storm event at or below the top of curb elevation. Additionally, the streets will convey the 100-year runoff within the proposed right-of-way tracts and maintain a maximum flow depth of eight (8) inches above the gutter flow line. Detailed street capacity analysis will be completed as part of the final design. Storm drain systems will convey runoff from the streets to the two retention basins. Storm drain analysis will be completed with final design.

The significant wash in the southeast corner of the site in subbasin ON25 (refer to **Figure 2**) will be unaffected by the improvements. The wash running through subbasin ON10 (refer to **Figure 2**) will be routed through a 2-24" culvert under the proposed roadway (refer to **Figure 4**), with no overtopping in the 10-year storm and less than three (3) inches of overtopping in the 100-year event. Runoff through the significant wash in subbasin ON20 (refer to **Figure 2**) will be conveyed under the proposed driveway with an eight-foot (8') span bridge (refer to **Figure 4**). The driveway will have a dry crossing in the 100-year event. The culverts conveying runoff across 128th Street were confirmed in the Story Rock design to convey both the 10- and 100-year events without overtopping the road.

Lateral erosion setbacks were determined for the significant washes. The minimum setbacks of 20 feet for straight channel reaches and 50 feet for channels with obvious curvature required per the Arizona Department of Water Resources State Standard for Watercourse System Sediment Balance (SS5-96) were used. To mitigate lateral erosion setbacks and scour, gabion basket walls will be placed between the wash and the proposed lots. The gabion basket walls or other form of erosion protection will be designed during the final design. Erosion setbacks are depicted in **Figure 4**.

The two impacted significant washes were analyzed to determine the BFE of the washes. The pad elevations for each lot will be set eight inches above the adjacent BFE, ensuring the lowest finished floor will be at least one foot above the BFE per the DSPM. Inundation limits are depicted in **Figure 4**.

Pre-development versus post-development BFE's were analyzed; refer to **Table 2** for a summary. Where the significant wash in subbasin ON10 (refer to **Figure 3**) exits the Development, the post-development BFE is lower than the pre-development. The significant wash in subbasin ON20 (refer to **Figure 3**) has a BFE increase of less than 0.1 feet. The increase in BFE can be attributed to instability in the hydraulic model. Such instability is common with natural washes. The minor increase in BFE will not impact downstream structures. Refer to **Appendix D** for detailed results.

Table 2. Wash BFE Summary

XS	Wash	Pre Q ₁₀₀ [cfs]	Post Q ₁₀₀ [cfs]	Pre BFE [ft]	Post BFE [ft]	Pre Velocity [fps]	Post Velocity [fps]
2	ON10	99	98	2693.05	2691.93	4.3	4.8
11	ON20	138	134	2711.03	2711.08	5.4	5.8

DATA ANALYSIS METHODS

HYDROLOGY

The U.S. Army Corps of Engineers HEC-1 hydrologic computer program was used to determine the 2-year, 10-year, and 100-year peak discharges for offsite and onsite subbasins. The hydrologic model prepared for this report uses rainfall depths from National Oceanic and Atmospheric Administration Atlas 14 (NOAA 14). The watershed is within NOAA 14 Map Index 64, Cell Numbers 460, 461, 500, 501, 540, and 541. The six-hour storm duration was used for this project due to the size of the contributing watershed. Refer to **Appendix A**. The Drainage Design Management System for Windows (DDMSW) program was used to develop the hydrologic parameters for the onsite drainage areas and offsite drainage areas east of the Development. The site is located southwest of the Story Rock Phase 2 development project. Their offsite subbasin delineations were used as a basis and divided into onsite and offsite subbasins for this project. Due to the re-divided subbasins and new routes for this project, the peak offsite discharges from Story Rock Phase 2 do not match the corresponding discharges leaving our project site. No hydrologic parameters, such as land use or soil, were modified.

The default NMIN value was changed in DDMSW from 5 to 3 based on the FCDMC Drainage Design Manual – Hydrology manual. The lower value was used for the shorter time of concentrations. The “Non-default value or value out of range” note in the proposed conditions parameters in **Appendix B** is because of the varying subbasin sizes and flowpaths. The selected NMIN parameter will not meet the time of concentration requirements specified in the FCDMC Drainage Design Manual – Hydrology due to the varying subbasin sizes. The HEC-1 models were run with varying NMIN parameters to confirm that the hydrograph shape and peaks were valid. Green and Ampt rainfall loss parameters were estimated using DDMSW, DS&PM and FCDMC Drainage Design Manual – Hydrology, see **Table 3**. No changes were made to the default hydrologic parameters. The Clark unit hydrograph was used. Offsite subbasins were delineated using the City Quarter Section Topography. Existing land use types for the HEC-1 models were based on aerial photography. The existing land use is undeveloped desert. The post-development condition land use is being re-zoned to R1-35 and R1-70. A sensitivity analysis was completed with the Story Rock project to confirm the RTIMP values for the developed condition. The analysis for zone R1-35 is included in **Appendix A**. Land use maps for the existing and proposed development conditions are provided in **Appendix A** and **Appendix B**. **Table 3** is a summary of the land use parameters used in the hydrologic model.

Table 3. Green & Ampt Parameters

FCDMC Land Use Code	Land Use Category	COS Land Use	IA [in]	R _{imp} [%]	Cover [%]	O _{theta} [cfs]	K _b [cfs]
120	Estate Residential	R1-70	0.30	5	30	Normal	Min
130	Large Lot Residential	R1-35	0.30	15	50	Normal	Min
2002	Pavement/Rooftops	Paved Streets	0.05	95	0	Dry	Min
Desert	Desert Landscaping	Undisturbed Desert	0.35	0	25	Dry	Low

One soil type was identified for the onsite and offsite subbasins using the web soil survey from the National Resource Conservation Service (NRCS). Maps showing the soil type is shown in **Appendix A** and **Appendix B**. A list of the soils found in the watershed is shown below:

- Gran-Wickenburg complex, 1 to 10 percent slopes (64561)

Normal depth method was used for routes through existing channels. Eight point cross sections are provided.

HYDRAULICS

The two (2) impacted onsite significant washes were modeled using HEC-RAS version 5.0. Cross sections were cut using project topography. Manning's n values for the channel and overbanks (0.035 and 0.05, respectively) were selected to match the HEC-1 hydrologic model for the Story Rock development. Runoff in the washes was updated to reflect the results of the post-development condition. The HEC-RAS model was also used to size the onsite culvert and bridge. For the culverts, an entrance loss coefficient of 0.5 for concrete pipes with square cut ends was used. An exit loss coefficient of 1.0 was used for a sudden expansion of flow. For the remaining cross sections, contraction and expansion coefficients of 0.1 and 0.3 were used, respectively. Ineffective flow areas were set in cross-sections where either smaller washes joined the significant wash or where a culvert or bridge constricts flow. Refer to **Figure 4** for cross-section locations and ineffective flow areas. Refer to **Appendix D** for the HEC-RAS model results.

Lateral erosion setbacks were determined based on the Arizona Department of Water Resources (ADWR) State Standard 5-96 (SS5-96) Level I Analysis. The calculated setbacks were less than the minimum requirement, therefore the minimum setbacks were used. Refer to **Figure 4** for the Proposed Conditions Drainage Map.

STORMWATER STORAGE METHOD

The existing property is a part of the ESLO. Based on new City ordinances, a waiver will need to be obtained for any volume less than the 100-year, 2-hour volume. However, there is no waiver fee associated with the volumes that do not result in an increase in downstream runoff. See **Appendix C** for a copy of the waiver.

Detention basins are being used to meet pre- vs. post- conditions. The detention basins are also used to treat the first flush.

CONCLUSIONS

- Hydrologic models were prepared for the onsite and offsite areas for the pre- and post-development conditions. The Development will not increase post-development peak discharges exiting the site from the pre-development conditions, except for negligible increases at one (1) discharge point in the 2-year and 10-year storm events. Detention basins with orifice plates are used to attenuate onsite runoff. The detention basins will treat the first flush.
- Three significant washes cross the Development. One will be unaffected by the improvements. The other two will be routed through a culvert and a bridge under the proposed roadway. The improved washes will exit the Development at their existing locations. Lateral erosion calculations were performed to determine where erosion protection will be needed. Gabion walls will be designed to protect lots from lateral erosion and scour.
- Culverts crossing 128th Street were designed with the Story Rock development. They will convey the 100-year storm event without overtopping 128th Street.
- Finished floor elevations will be set at a minimum of one foot above the adjacent wash BFE.

REFERENCES

Arizona Department of Water Resources, *State Standard for Watercourse System Sediment Balance*, September 1996.

City of Scottsdale, *Design Standards and Policies Manual*, January 2010.

Flood Control District of Maricopa County, *Pinnacle Peak West Area Drainage Master Plan*, 2013.

Federal Emergency Management Agency, Flood Insurance Rate Map Panel No04013C1335M, dated November 2015.

Flood Control District of Maricopa County, *Drainage Design Manual – Hydrology*, updated August 15, 2013.

City of Scottsdale Topography Quarter Section Maps.

Appendix A Existing Conditions Hydrology

- *NOAA 14 Rainfall*
- *Existing Conditions Soils*
- *Existing Conditions Land Use*
- *Subbasin Parameters*
- *Time of Concentration Map*
- *Routing Reaches*
- *HEC-1 Results*

Appendix A Existing Conditions Hydrology

- *NOAA 14 Rainfall*

Flood Control District of Maricopa County
 Drainage Design Management System
 RAINFALL DATA
 Project Reference: ASTERIA EX

ID	Method	Duration	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
DEFAULT	NOAA14	5 MIN	0.315	0.424	0.505	0.611	0.690	0.769
	NOAA14	10 MIN	0.480	0.645	0.769	0.930	1.050	1.170
	NOAA14	15 MIN	0.595	0.800	0.953	1.153	1.302	1.451
	NOAA14	30 MIN	0.801	1.077	1.283	1.553	1.753	1.954
	NOAA14	1 HOUR	0.991	1.333	1.588	1.922	2.170	2.418
	NOAA14	2 HOUR	1.126	1.492	1.770	2.142	2.421	2.707
	NOAA14	3 HOUR	1.194	1.555	1.841	2.236	2.548	2.867
	NOAA14	6 HOUR	1.422	1.804	2.110	2.525	2.847	3.180
	NOAA14	12 HOUR	1.707	2.146	2.493	2.964	3.326	3.698
	NOAA14	24 HOUR	2.118	2.755	3.269	3.996	4.581	5.199

Map Index 64, Cell Number 460, 461, 500, 501, 540, and 541

Appendix A Existing Conditions Hydrology

- *Existing Conditions Soils*

Flood Control District of Maricopa County
 Drainage Design Management System
 SOILS

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
OFF05	645	61	64561	0.005	100.00	0.150	-	100	
OFF10	645	61	64561	0.028	100.00	0.150	-	100	
OFF11	645	61	64561	0.004	100.00	0.150	-	100	
OFF20	645	61	64561	0.077	100.00	0.150	-	100	
OFF25	645	61	64561	0.034	100.00	0.150	-	100	
ON05	645	61	64561	0.010	100.00	0.150	-	100	
ON07	645	61	64561	0.003	100.00	0.150	-	100	
ON10	645	61	64561	0.027	100.00	0.150	-	100	
ON12	645	61	64561	0.003	100.00	0.150	-	100	
ON15	645	61	64561	0.010	100.00	0.150	-	100	
ON20	645	61	64561	0.007	100.00	0.150	-	100	
ON25	645	61	64561	0.002	100.00	0.150	-	100	

Appendix A Existing Conditions Hydrology

- *Existing Conditions Land Use*

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: ASTERIA EX

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
OFF05	DESERT	0.0052	100.0	0.35	0	25.0	DRY	0.073	Desert
		0.0052	100.0						
OFF10	DESERT	0.0284	100.0	0.35	0	25.0	DRY	0.063	Desert
		0.0284	100.0						
OFF11	DESERT	0.0038	100.0	0.35	0	25.0	DRY	0.074	Desert
		0.0038	100.0						
OFF20	DESERT	0.0774	100.0	0.35	0	25.0	DRY	0.057	Desert
		0.0774	100.0						
OFF25	DESERT	0.0338	100.0	0.35	0	25.0	DRY	0.062	Desert
		0.0338	100.0						
ON05	DESERT	0.0102	100.0	0.35	0	25.0	DRY	0.069	Desert
		0.0102	100.0						
ON07	DESERT	0.0030	100.0	0.35	0	25.0	DRY	0.076	Desert
		0.0030	100.0						
ON10	DESERT	0.0271	100.0	0.35	0	25.0	DRY	0.063	Desert
		0.0271	100.0						
ON12	DESERT	0.0033	100.0	0.35	0	25.0	DRY	0.076	Desert
		0.0033	100.0						
ON15	DESERT	0.0097	100.0	0.35	0	25.0	DRY	0.069	Desert
		0.0097	100.0						
ON20	DESERT	0.0072	100.0	0.35	0	25.0	DRY	0.071	Desert

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: ASTERIA EX

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
		<u>0.0072</u>	<u>100.0</u>						
ON25	DESERT	0.0017	100.0	0.35	0	25.0	DRY	0.079	Desert
		<u>0.0017</u>	<u>100.0</u>						

* Non default value

Appendix A Existing Conditions Hydrology

- *Subbasin Parameters*

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters					
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin ID: 01																	
OFF05	0.005	0.08	346.2	277.0	NATURAL	0.073	0.35	0.40	6.00	0.176	Tc (Hrs)	0.182	0.170	0.151*	0.133*	0.122*	0.113*
											Vel (f/s)	0.64	0.69	0.78	0.88	0.96	1.04
											R (Hrs)	0.152	0.140	0.123	0.107	0.097	0.090
OFF11	0.004	0.07	876.7	313.0	Natural	0.074	0.35	0.40	6.00	0.176	Tc (Hrs)	0.165*	0.154*	0.137*	0.120*	0.111*	0.103*
											Vel (f/s)	0.62	0.67	0.75	0.86	0.92	1.00
											R (Hrs)	0.139	0.128	0.113	0.098	0.089	0.082
OFF10	0.028	0.40	225.6	222.8	NATURAL	0.063	0.35	0.40	6.00	0.176	Tc (Hrs)	0.403	0.376	0.335	0.294	0.270	0.251
											Vel (f/s)	1.46	1.56	1.75	2.00	2.17	2.34
											R (Hrs)	0.498	0.460	0.405	0.351	0.319	0.295
ON07	0.003	0.06	309.1	266.5	Natural	0.076	0.35	0.40	6.00	0.176	Tc (Hrs)	0.163*	0.152*	0.135*	0.119*	0.109*	0.102*
											Vel (f/s)	0.54	0.58	0.65	0.74	0.81	0.86
											R (Hrs)	0.143	0.132	0.116	0.100	0.091	0.084
OFF20	0.077	0.72	295.8	261.9	NATURAL	0.057	0.35	0.40	6.00	0.176	Tc (Hrs)	0.489	0.455	0.406	0.356	0.327	0.305
											Vel (f/s)	2.16	2.32	2.60	2.97	3.23	3.46
											R (Hrs)	0.554	0.512	0.451	0.390	0.355	0.328
ON12	0.003	0.11	321.1	270.2	Natural	0.076	0.35	0.40	6.00	0.176	Tc (Hrs)	0.220	0.205	0.182	0.160*	0.147*	0.137*
											Vel (f/s)	0.73	0.79	0.89	1.01	1.10	1.18
											R (Hrs)	0.322	0.298	0.262	0.227	0.207	0.191
OFF25	0.034	0.50	504.0	303.0	NATURAL	0.062	0.35	0.40	6.00	0.176	Tc (Hrs)	0.407	0.379	0.338	0.296	0.272	0.253
											Vel (f/s)	1.80	1.93	2.17	2.48	2.70	2.90
											R (Hrs)	0.538	0.497	0.438	0.379	0.345	0.318
ON05	0.010	0.24	172.3	172.3	NATURAL	0.069	0.35	0.40	6.00	0.176	Tc (Hrs)	0.355	0.330	0.295	0.259	0.238	0.221
											Vel (f/s)	0.99	1.07	1.19	1.36	1.48	1.59
											R (Hrs)	0.516	0.477	0.420	0.363	0.331	0.305
ON10	0.027	0.41	204.4	204.2	NATURAL	0.063	0.35	0.40	6.00	0.176	Tc (Hrs)	0.420	0.391	0.348	0.306	0.281	0.262
											Vel (f/s)	1.43	1.54	1.73	1.97	2.14	2.30
											R (Hrs)	0.542	0.501	0.441	0.382	0.347	0.321

* Non default value or value out of range

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters					
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin ID: 01																	
ON15	0.010	0.22	242.0	234.7	NATURAL	0.069	0.35	0.40	6.00	0.176	Tc (Hrs)	0.309	0.287	0.256	0.225	0.207	0.192
											Vel (f/s)	1.04	1.12	1.26	1.43	1.56	1.68
											R (Hrs)	0.412	0.381	0.336	0.290	0.264	0.244
ON20	0.007	0.24	233.1	228.5	NATURAL	0.071	0.35	0.40	6.00	0.176	Tc (Hrs)	0.330	0.307	0.274	0.240	0.221	0.206
											Vel (f/s)	1.07	1.15	1.28	1.47	1.59	1.71
											R (Hrs)	0.583	0.539	0.475	0.411	0.374	0.345
ON25	0.002	0.07	328.6	272.4	NATURAL	0.079	0.35	0.40	6.00	0.176	Tc (Hrs)	0.178	0.166*	0.148*	0.130*	0.119*	0.111*
											Vel (f/s)	0.58	0.62	0.69	0.79	0.86	0.92
											R (Hrs)	0.225	0.208	0.183	0.158	0.144	0.133

* Non default value or value out of range

Appendix A Existing Conditions Hydrology

- *Time of Concentration Map*

Appendix A Existing Conditions Hydrology

- *Routing Reaches*

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 ROUTING DATA
 Project Reference: ASTERIA EX

Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)	1.	2.	3.	4.	5.	6.	7.	8.
NORMAL DEPTH														
Major Basin 01														
RON05	0.035	0.035	0.035	1,253.80	0.0327	-	X: 9.60	73.10	89.30	91.70	92.70	98.60	120.20	226.30
							Y: 2,741.00	2,731.94	2,731.00	2,730.00	2,730.00	2,733.00	2,736.00	2,753.00
RON10A	0.035	0.035	0.035	1,763.40	0.0366	-	X: 3.20	62.30	106.10	114.70	118.30	140.60	211.30	287.50
							Y: 2,752.00	2,740.99	2,738.24	2,736.07	2,737.30	2,740.34	2,743.12	2,754.09
RON10B	0.035	0.035	0.035	1,763.40	0.0366	-	X: 3.20	62.30	106.10	114.70	118.30	140.60	211.30	287.50
							Y: 2,752.00	2,740.99	2,738.24	2,736.07	2,737.30	2,740.34	2,743.12	2,754.09
RON20	0.035	0.035	0.035	767.00	0.0339	-	X: 50.60	70.30	78.60	88.30	94.50	100.00	113.70	161.00
							Y: 2,741.42	2,740.89	2,740.01	2,738.78	2,738.91	2,740.99	2,743.04	2,748.80
RON25	0.035	0.035	0.035	219.00	0.0411	-	X: -	33.10	62.10	77.00	81.70	84.60	140.40	158.60
							Y: 2,739.49	2,736.34	2,734.18	2,733.23	2,733.44	2,733.82	2,737.85	2,739.50

Appendix A Existing Conditions Hydrology

- *HEC-1 Results*

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 31MAY17 TIME 18:27:00 *
*****
    
```

Existing 2-year

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX
    
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION

KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Flood Control District of Maricopa County									
2	ID	ASTERIA EX - ASTERIA EX									
3	ID	2 YEAR									
4	ID	6 Hour Storm									
5	ID	Unit Hydrograph: Clark									
6	ID	Storm: Multiple									
7	ID	05/04/2017									
	*DIAGRAM										
8	IT	3	1JAN99	0	2000						
9	IO	5									
10	IN	15									
	*										
11	JD	1.422	0.0001								
12	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
13	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
14	PC	0.962	0.972	0.983	0.991	1.000					
15	JD	1.413	0.5000								
16	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
17	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
18	PC	0.962	0.972	0.983	0.991	1.000					
	*										
19	KK	ON07	BASIN								
20	BA	0.003									
21	LG	0.35	0.40	6.00	0.18	0					
22	UC	0.163	0.143								
23	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
24	UA	100									
	*										
25	KK	OFF05	BASIN								
26	BA	0.005									
27	LG	0.35	0.40	6.00	0.18	0					
28	UC	0.182	0.152								
29	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
30	UA	100									
	*										
31	KK	RON05	ROUTE								
32	RS	1	FLOW								
33	RC	0.035	0.035	0.035	1254	0.0327	0.00				
34	RX	9.60	73.10	89.30	91.70	92.70	98.60	120.20	226.30		
35	RY	2741.0	2731.94	2731.00	2730.00	2730.00	2733.00	2736.00	2753.00		
	*										
36	KK	ON05	BASIN								
37	BA	0.010									
38	LG	0.35	0.40	6.00	0.18	0					
39	UC	0.355	0.516								
40	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
41	UA	100									
	*										

LINE	ID	1	2	3	4	5	6	7	8	9	10
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42	KK	CP05 COMBINE											
43	HC	2											
	*												
44	KK	OFF10 BASIN											
45	BA	0.028											
46	LG	0.35 0.40	6.00	0.18	0								
47	UC	0.403 0.498											
48	UA	0 3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
49	UA	100											
	*												
50	KK	RON10A ROUTE											
51	RS	1 FLOW											
52	RC	0.035 0.035	0.035	1763	0.0366	0.00							
53	RX	3.20 62.30	106.10	114.70	118.30	140.60	211.30	287.50					
54	RY	2752.0 2740.99	2738.24	2736.07	2737.30	2740.34	2743.12	2754.09					
	*												
55	KK	OFF11 BASIN											
56	BA	0.0038											
57	LG	0.35 0.40	6.00	0.18	0								
58	UC	0.165 0.139											
59	UA	0 3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
60	UA	100											
	*												
61	KK	RON10B ROUTE											
62	RS	1 FLOW											
63	RC	0.035 0.035	0.035	1763	0.0366	0.00							
64	RX	3.20 62.30	106.10	114.70	118.30	140.60	211.30	287.50					
65	RY	2752.0 2740.99	2738.24	2736.07	2737.30	2740.34	2743.12	2754.09					
	*												
66	KK	ON10 BASIN											
67	BA	0.027											
68	LG	0.35 0.40	6.00	0.18	0								
69	UC	0.420 0.542											
70	UA	0 3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
71	UA	100											
	*												
72	KK	CP10 COMBINE											
73	HC	3											
	*												
74	KK	OFF20 BASIN											
75	BA	0.077											
76	LG	0.35 0.40	6.00	0.18	0								
77	UC	0.489 0.554											
78	UA	0 3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
79	UA	100											
	*												
			HEC-1 INPUT										
			PAGE 3										
80	KK	RON20 ROUTE											
81	RS	1 FLOW											
82	RC	0.035 0.035	0.035	767	0.0339	0.00							
83	RX	50.60 70.30	78.60	88.30	94.50	100.00	113.70	161.00					
84	RY	2741.4 2740.89	2740.01	2738.78	2738.91	2740.99	2743.04	2748.80					
	*												
85	KK	ON20 BASIN											
86	BA	0.007											
87	LG	0.35 0.40	6.00	0.18	0								
88	UC	0.330 0.583											
89	UA	0 3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
90	UA	100											
	*												
91	KK	CP20 COMBINE											
92	HC	2											
	*												
93	KK	OFF25 BASIN											
94	BA	0.034											
95	LG	0.35 0.40	6.00	0.18	0								
96	UC	0.407 0.538											
97	UA	0 3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
98	UA	100											
	*												
99	KK	RON25 ROUTE											
100	RS	1 FLOW											
101	RC	0.035 0.035	0.035	219	0.0411	0.00							
102	RX	0.00 33.10	62.10	77.00	81.70	84.60	140.40	158.60					
103	RY	2739.5 2736.34	2734.18	2733.23	2733.44	2733.82	2737.85	2739.50					
	*												

104	KK	ON25	BASIN									
105	BA	0.002										
106	LG	0.35	0.40	6.00	0.18	0						
107	UC	0.178	0.225									
108	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
109	UA	100										
	*											

110	KK	CP25	COMBINE									
111	HC	2										
	*											

112	KK	ON12	BASIN									
113	BA	0.003										
114	LG	0.35	0.40	6.00	0.18	0						
115	UC	0.220	0.322									
116	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
117	UA	100										
	*											

1 HEC-1 INPUT PAGE 4

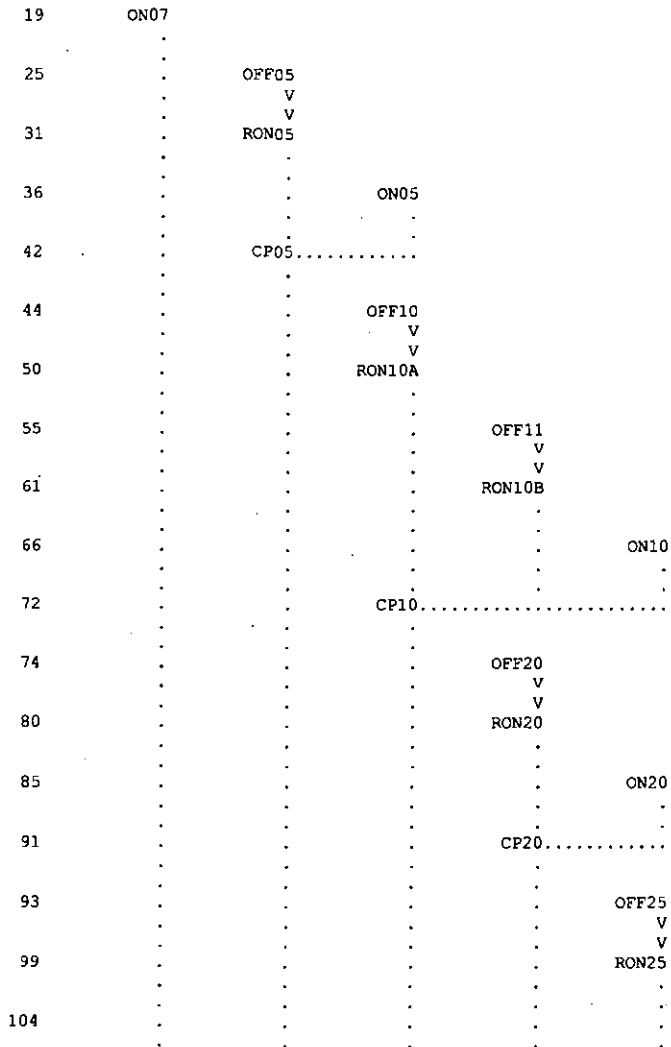
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

118	KK	ON15	BASIN									
119	BA	0.010										
120	LG	0.35	0.40	6.00	0.18	0						
121	UC	0.309	0.412									
122	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
123	UA	100										
	*											

124 ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW



110 CP25

 112 ON12

 118 ON15

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * JUN 1998 *
 * VERSION 4.1 *
 * RUN DATE 31MAY17 TIME 18:27:00 *

 * U.S. ARMY CORPS OF ENGINEERS *
 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 756-1104 *

Flood Control District of Maricopa County
 ASTERIA EX - ASTERIA EX
 2 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 05/04/2017

9 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 3 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 2000 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 5JAN99 ENDING DATE
 NDTIME 0357 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .05 HOURS
 TOTAL TIME BASE 99.95 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

11 JD INDEX STORM NO. 1
 STRM 1.42 PRECIPITATION DEPTH
 TRDA .00 TRANSPOSITION DRAINAGE AREA

12 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
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 .03 .03 .03 .03 .03 .09 .09 .09 .09 .09
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15 JD INDEX STORM NO. 2
 STRM 1.41 PRECIPITATION DEPTH
 TRDA .50 TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
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1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+									
+	HYDROGRAPH AT								
	ON07	2.	4.10	0.	0.	0.	.00		
+	HYDROGRAPH AT								
	OFF05	3.	4.10	0.	0.	0.	.00		
+	ROUTED TO								
	RON05	3.	4.15	0.	0.	0.	.00		
+	HYDROGRAPH AT								
	ON05	3.	4.25	0.	0.	0.	.01		
+	2 COMBINED AT								
	CP05	5.	4.20	0.	0.	0.	.01		
+	HYDROGRAPH AT								
	OFF10	8.	4.30	1.	0.	0.	.03		
+	ROUTED TO								
	RON10A	7.	4.40	1.	0.	0.	.03		
+	HYDROGRAPH AT								
	OFF11	2.	4.10	0.	0.	0.	.00		
+	ROUTED TO								
	RON10B	2.	4.15	0.	0.	0.	.00		
+	HYDROGRAPH AT								
	ON10	7.	4.30	1.	0.	0.	.03		
+	3 COMBINED AT								
	CP10	14.	4.35	2.	0.	0.	.06		
+	HYDROGRAPH AT								
	OFF20	19.	4.35	2.	1.	0.	.08		
+	ROUTED TO								
	RON20	18.	4.40	2.	1.	0.	.08		
+	HYDROGRAPH AT								
	ON20	2.	4.25	0.	0.	0.	.01		
+	2 COMBINED AT								
	CP20	20.	4.40	3.	1.	0.	.08		
+	HYDROGRAPH AT								
	OFF25	9.	4.30	1.	0.	0.	.03		
+	ROUTED TO								
	RON25	9.	4.30	1.	0.	0.	.03		
+	HYDROGRAPH AT								
	ON25	1.	4.10	0.	0.	0.	.00		
+	2 COMBINED AT								
	CP25	9.	4.30	1.	0.	0.	.04		
+	HYDROGRAPH AT								
	ON12	1.	4.15	0.	0.	0.	.00		
+	HYDROGRAPH AT								
	ON15	3.	4.20	0.	0.	0.	.01		

*** NORMAL END OF HEC-1 ***

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   JUN 1998                   *
*   VERSION 4.1                 *
* RUN DATE 31MAY17 TIME 18:29:31 *
*****
    
```

Existing 10-year

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET            *
* DAVIS, CALIFORNIA 95616     *
* (916) 756-1104              *
*****
    
```

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X   X   XXXXXXXX   XXXXX   X
X   X   X           X   X   XX
X   X   X           X       X
XXXXXXX XXXX   X       XXXXX X
X   X   X           X       X
X   X   X           X   X   X
X   X   XXXXXXXX   XXXXX   XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Flood Control District of Maricopa County									
2	ID	ASTERIA EX - ASTERIA EX									
3	ID	10 YEAR									
4	ID	6 Hour Storm									
5	ID	Unit Hydrograph: Clark									
6	ID	Storm: Multiple									
7	ID	05/04/2017									
	*DIAGRAM										
8	IT	3	1JAN99	0	2000						
9	IO	5									
10	IN	15									
	*										
11	JD	2.110	0.0001								
12	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
13	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
14	PC	0.962	0.972	0.983	0.991	1.000					
15	JD	2.097	0.5000								
16	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
17	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
18	PC	0.962	0.972	0.983	0.991	1.000					
	*										
19	KK	ON07	BASIN								
20	BA	0.003									
21	LG	0.35	0.40	6.00	0.18	0					
22	UC	0.135	0.116								
23	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
24	UA	100									
	*										
25	KK	OFF05	BASIN								
26	BA	0.005									
27	LG	0.35	0.40	6.00	0.18	0					
28	UC	0.151	0.123								
29	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
30	UA	100									
	*										
31	KK	RON05	ROUTE								
32	RS	1	FLOW								
33	RC	0.035	0.035	0.035	1254	0.0327	0.00				
34	RX	9.60	73.10	89.30	91.70	92.70	98.60	120.20	226.30		
35	RY	2741.0	2731.94	2731.00	2730.00	2730.00	2733.00	2736.00	2753.00		
	*										
36	KK	ON05	BASIN								
37	BA	0.010									
38	LG	0.35	0.40	6.00	0.18	0					
39	UC	0.295	0.420								
40	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
41	UA	100									
	*										

1 HEC-1 INPUT PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
------	----	---	---	---	---	---	---	---	---	---	----

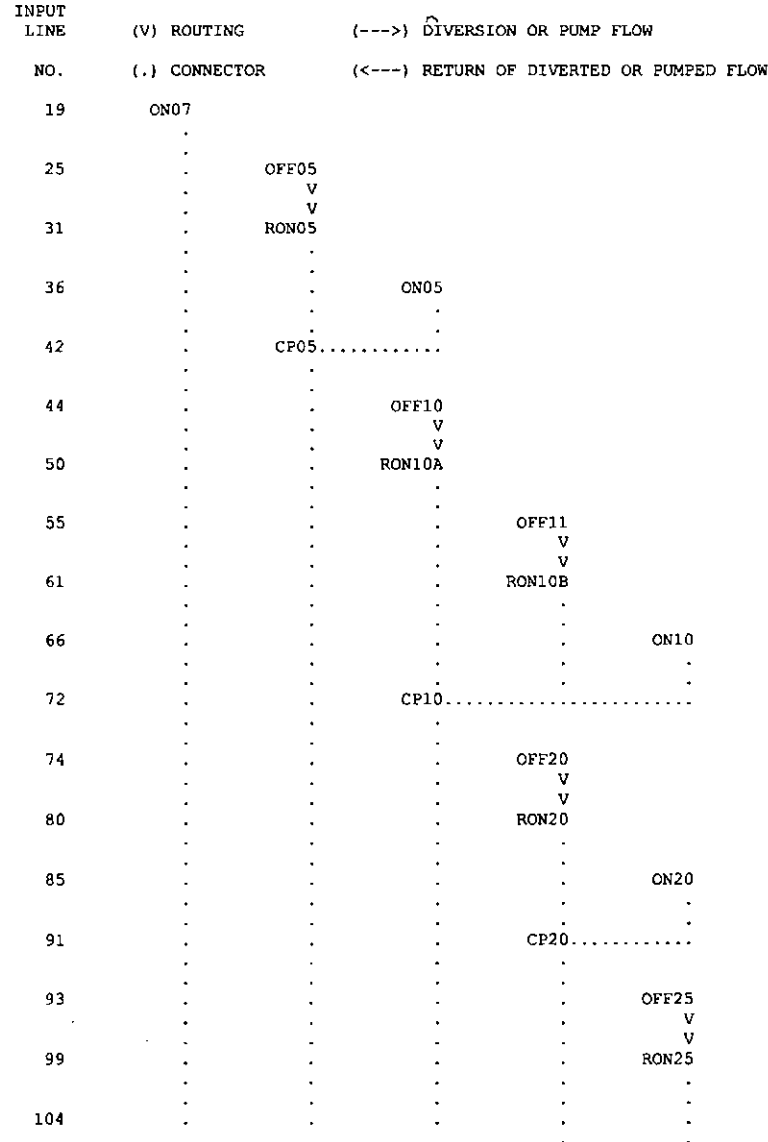
104	KK	ON25	BASIN									
105	BA	0.002										
106	LG	0.35	0.40	6.00	0.18	0						
107	UC	0.148	0.183									
108	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
109	UA	100										
	*											
110	KK	CP25	COMBINE									
111	HC	2										
	*											
112	KK	ON12	BASIN									
113	BA	0.003										
114	LG	0.35	0.40	6.00	0.18	0						
115	UC	0.182	0.262									
116	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
117	UA	100										
	*											

1 HEC-1 INPUT PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

118	KK	ON15	BASIN									
119	BA	0.010										
120	LG	0.35	0.40	6.00	0.18	0						
121	UC	0.256	0.336									
122	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
123	UA	100										
	*											
124	ZZ											

1 SCHEMATIC DIAGRAM OF STREAM NETWORK



110 CP25
 112 ON12
 118 ON15

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * JUN 1998 *
 * VERSION 4.1 *
 * RUN DATE 31MAY17 TIME 18:29:31 *

 * U.S. ARMY CORPS OF ENGINEERS *
 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 756-1104 *

Flood Control District of Maricopa County
 ASTERIA EX - ASTERIA EX
 10 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 05/04/2017

9 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 3 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 2000 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 5JAN99 ENDING DATE
 NDTIME 0357 ENDING TIME
 ICENT 19 CENTURY MARK
 COMPUTATION INTERVAL .05 HOURS
 TOTAL TIME BASE 99.95 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

11 JD INDEX STORM NO. 1
 STRM 2.11 PRECIPITATION DEPTH
 TRDA .00 TRANSPOSITION DRAINAGE AREA

12 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .03 .03 .03 .03 .03 .09 .09 .09 .09 .09 .09
 .02 .02 .02 .02 .02 .00 .00 .00 .00 .00 .00
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 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00

15 JD INDEX STORM NO. 2
 STRM 2.10 PRECIPITATION DEPTH
 TRDA .50 TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
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 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .03 .03 .03 .03 .03 .09 .09 .09 .09 .09 .09
 .02 .02 .02 .02 .02 .00 .00 .00 .00 .00 .00
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1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	ON07	5.	4.05	0.	0.	0.	.00		
HYDROGRAPH AT	OFF05	8.	4.05	0.	0.	0.	.00		
ROUTED TO	RON05	7.	4.10	0.	0.	0.	.00		
HYDROGRAPH AT	ON05	8.	4.20	1.	0.	0.	.01		
2 COMBINED AT	CP05	14.	4.15	1.	0.	0.	.01		
HYDROGRAPH AT	OFF10	21.	4.20	2.	1.	0.	.03		
ROUTED TO	RON10A	19.	4.30	2.	1.	0.	.03		
HYDROGRAPH AT	OFF11	6.	4.05	0.	0.	0.	.00		
ROUTED TO	RON10B	5.	4.10	0.	0.	0.	.00		
HYDROGRAPH AT	ON10	19.	4.25	2.	1.	0.	.03		
3 COMBINED AT	CP10	40.	4.25	5.	1.	0.	.06		
HYDROGRAPH AT	OFF20	52.	4.30	6.	1.	0.	.08		
ROUTED TO	RON20	52.	4.30	6.	1.	0.	.08		
HYDROGRAPH AT	ON20	5.	4.20	1.	0.	0.	.01		
2 COMBINED AT	CP20	56.	4.30	7.	2.	1.	.08		
HYDROGRAPH AT	OFF25	24.	4.25	3.	1.	0.	.03		
ROUTED TO	RON25	24.	4.25	3.	1.	0.	.03		
HYDROGRAPH AT	ON25	3.	4.05	0.	0.	0.	.00		
2 COMBINED AT	CP25	25.	4.25	3.	1.	0.	.04		
HYDROGRAPH AT	ON12	3.	4.10	0.	0.	0.	.00		
HYDROGRAPH AT	ON15	9.	4.15	1.	0.	0.	.01		

*** NORMAL END OF HEC-1 ***


```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 31MAY17 TIME 18:29:39 *
*****
    
```

Existing 100-year

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX
    
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Flood Control District of Maricopa County									
2	ID	ASTERIA EX - ASTERIA EX									
3	ID	100 YEAR									
4	ID	6 Hour Storm									
5	ID	Unit Hydrograph: Clark									
6	ID	Storm: Multiple									
7	ID	05/04/2017									
	*DIAGRAM										
8	IT	3	1JAN99	0	2000						
9	IO	5									
10	IN	15									
	*										
11	JD	3.180	0.0001								
12	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
13	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
14	PC	0.962	0.972	0.983	0.991	1.000					
15	JD	3.161	0.5000								
16	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
17	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
18	PC	0.962	0.972	0.983	0.991	1.000					
	*										
19	KK	ON07	BASIN								
20	BA	0.003									
21	LG	0.35	0.40	6.00	0.18	0					
22	UC	0.102	0.084								
23	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
24	UA	100									
	*										
25	KK	OFF05	BASIN								
26	BA	0.005									
27	LG	0.35	0.40	6.00	0.18	0					
28	UC	0.113	0.090								
29	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
30	UA	100									
	*										
31	KK	RON05	ROUTE								
32	RS	1	FLOW								
33	RC	0.035	0.035	0.035	1254	0.0327	0.00				
34	RK	9.60	73.10	89.30	91.70	92.70	98.60	120.20	226.30		
35	RY	2741.0	2731.94	2731.00	2730.00	2730.00	2733.00	2736.00	2753.00		
	*										
36	KK	ON05	BASIN								
37	BA	0.010									
38	LG	0.35	0.40	6.00	0.18	0					
39	UC	0.221	0.305								
40	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
41	UA	100									
	*										

LINE	ID	1	2	3	4	5	6	7	8	9	10
------	----	---	---	---	---	---	---	---	---	---	----

42 KK CP05 COMBINE
 43 HC 2
 *

44 KK OFF10 BASIN
 45 BA 0.028
 46 LG 0.35 0.40 6.00 0.18 0
 47 UC 0.251 0.295
 48 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 49 UA 100
 *

50 KK RON10A ROUTE
 51 RS 1 FLOW
 52 RC 0.035 0.035 0.035 1763 0.0366 0.00
 53 RX 3.20 62.30 106.10 114.70 118.30 140.60 211.30 287.50
 54 RY 2752.0 2740.99 2738.24 2736.07 2737.30 2740.34 2743.12 2754.09
 *

55 KK OFF11 BASIN
 56 BA 0.0038
 57 LG 0.35 0.40 6.00 0.18 0
 58 UC 0.103 0.082
 59 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 60 UA 100
 *

61 KK RON10B ROUTE
 62 RS 1 FLOW
 63 RC 0.035 0.035 0.035 1763 0.0366 0.00
 64 RX 3.20 62.30 106.10 114.70 118.30 140.60 211.30 287.50
 65 RY 2752.0 2740.99 2738.24 2736.07 2737.30 2740.34 2743.12 2754.09
 *

66 KK ON10 BASIN
 67 BA 0.027
 68 LG 0.35 0.40 6.00 0.18 0
 69 UC 0.262 0.321
 70 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 71 UA 100
 *

72 KK CP10 COMBINE
 73 HC 3
 *

74 KK OFF20 BASIN
 75 BA 0.077
 76 LG 0.35 0.40 6.00 0.18 0
 77 UC 0.305 0.328
 78 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 79 UA 100
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

80 KK RON20 ROUTE
 81 RS 1 FLOW
 82 RC 0.035 0.035 0.035 767 0.0339 0.00
 83 RX 50.60 70.30 78.60 88.30 94.50 100.00 113.70 161.00
 84 RY 2741.4 2740.89 2740.01 2738.78 2738.91 2740.99 2743.04 2748.80
 *

85 KK ON20 BASIN
 86 BA 0.007
 87 LG 0.35 0.40 6.00 0.18 0
 88 UC 0.206 0.345
 89 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 90 UA 100
 *

91 KK CP20 COMBINE
 92 HC 2
 *

93 KK OFF25 BASIN
 94 BA 0.034
 95 LG 0.35 0.40 6.00 0.18 0
 96 UC 0.253 0.318
 97 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 98 UA 100
 *

99 KK RON25 ROUTE
 100 RS 1 FLOW
 101 RC 0.035 0.035 0.035 219 0.0411 0.00
 102 RX 0.00 33.10 62.10 77.00 81.70 84.60 140.40 158.60
 103 RY 2739.5 2736.34 2734.18 2733.23 2733.44 2733.82 2737.85 2739.50
 *

104	KK	ON25	BASIN									
105	BA	0.002										
106	LG	0.35	0.40	6.00	0.18	0						
107	UC	0.111	0.133									
108	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
109	UA	100										
	*											

110	KK	CP25	COMBINE									
111	HC	2										
	*											

112	KK	ON12	BASIN									
113	BA	0.003										
114	LG	0.35	0.40	6.00	0.18	0						
115	UC	0.137	0.191									
116	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
117	UA	100										
	*											

HEC-1 INPUT

PAGE 4

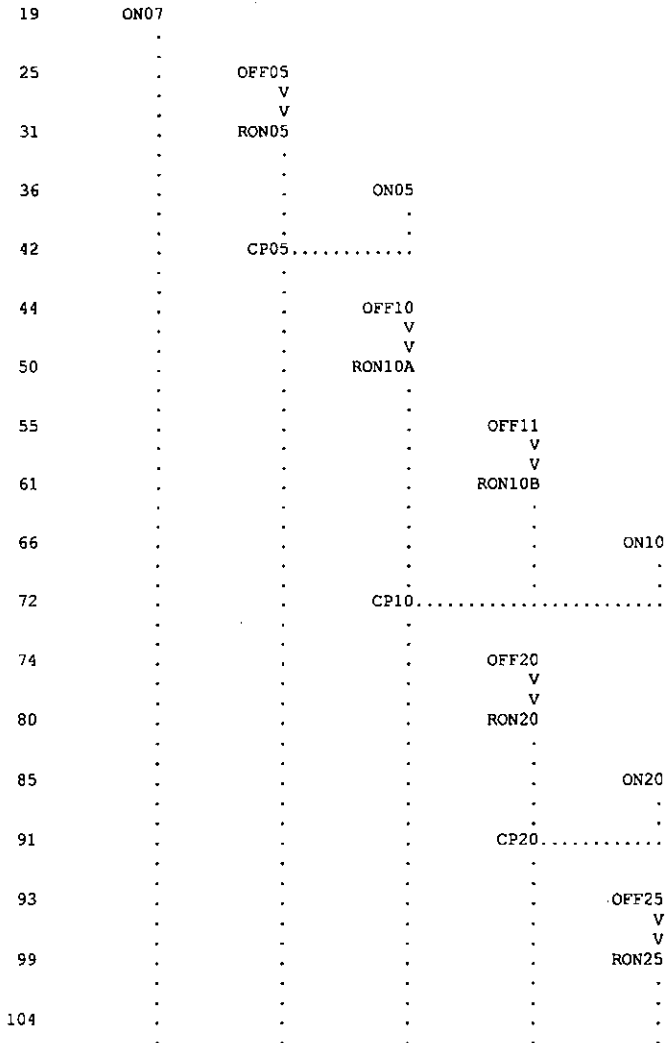
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

118	KK	ON15	BASIN									
119	BA	0.010										
120	LG	0.35	0.40	6.00	0.18	0						
121	UC	0.192	0.244									
122	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
123	UA	100										
	*											

124 22

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW



110 CP25.....
 112 ON12
 118 ON15

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 31MAY17 TIME 18:29:39 *
*****
    
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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

Flood Control District of Maricopa County
 ASTERIA EX - ASTERIA EX
 100 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 05/04/2017

9 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 3 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 2000 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 5JAN99 ENDING DATE
 NDTIME 0357 ENDING TIME
 ICENT 19 CENTURY MARK
 COMPUTATION INTERVAL .05 HOURS
 TOTAL TIME BASE 99.95 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

11 JD INDEX STORM NO. 1
 STRM 3.18 PRECIPITATION DEPTH
 TRDA .00 TRANSPOSITION DRAINAGE AREA

12 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
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15 JD INDEX STORM NO. 2
 STRM 3.16 PRECIPITATION DEPTH
 TRDA .50 TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
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1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+									
+	HYDROGRAPH AT								
	ON07	9.	4.05	1.	0.	0.	.00		
+	HYDROGRAPH AT								
	OFF05	15.	4.05	1.	0.	0.	.00		
+	ROUTED TO								
	RON05	13.	4.05	1.	0.	0.	.00		
+	HYDROGRAPH AT								
	ON05	18.	4.15	2.	0.	0.	.01		
+	2 COMBINED AT								
	CP05	31.	4.10	3.	1.	0.	.01		
+	HYDROGRAPH AT								
	OFF10	51.	4.15	5.	1.	0.	.03		
+	ROUTED TO								
	RON10A	47.	4.20	5.	1.	0.	.03		
+	HYDROGRAPH AT								
	OFF11	11.	4.05	1.	0.	0.	.00		
+	ROUTED TO								
	RON10B	10.	4.10	1.	0.	0.	.00		
+	HYDROGRAPH AT								
	ON10	46.	4.15	5.	1.	0.	.03		
+	3 COMBINED AT								
	CP10	99.	4.20	10.	2.	1.	.06		
+	HYDROGRAPH AT								
	OFF20	129.	4.20	13.	3.	1.	.08		
+	ROUTED TO								
	RON20	127.	4.20	13.	3.	1.	.08		
+	HYDROGRAPH AT								
	ON20	12.	4.15	1.	0.	0.	.01		
+	2 COMBINED AT								
	CP20	138.	4.20	14.	4.	1.	.08		
+	HYDROGRAPH AT								
	OFF25	59.	4.15	6.	1.	0.	.03		
+	ROUTED TO								
	RON25	58.	4.15	6.	1.	0.	.03		
+	HYDROGRAPH AT								
	ON25	5.	4.05	0.	0.	0.	.00		
+	2 COMBINED AT								
	CP25	62.	4.15	6.	2.	1.	.04		
+	HYDROGRAPH AT								
	ON12	7.	4.05	1.	0.	0.	.00		
+	HYDROGRAPH AT								
	ON15	20.	4.10	2.	0.	0.	.01		

*** NORMAL END OF HEC-1 ***

Appendix B Proposed Conditions Hydrology

- *NOAA 14 Rainfall*
- *Proposed Conditions Soils*
- *Proposed Conditions Land Use*
- *Subbasin Parameters*
- *Time of Concentration Map*
- *Routing Reaches*
- *Proposed Storage*
- *Proposed Diversions*
- *HEC-1 Results*

Appendix B Proposed Conditions Hydrology

- *NOAA 14 Rainfall*

Flood Control District of Maricopa County
 Drainage Design Management System
 RAINFALL DATA
 Project Reference: ASTERIA PROP

ID	Method	Duration	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
DEFAULT	NOAA14	5 MIN	0.315	0.424	0.505	0.611	0.690	0.769
	NOAA14	10 MIN	0.480	0.645	0.769	0.930	1.050	1.170
	NOAA14	15 MIN	0.595	0.800	0.953	1.153	1.302	1.451
	NOAA14	30 MIN	0.801	1.077	1.283	1.553	1.753	1.954
	NOAA14	1 HOUR	0.991	1.333	1.588	1.922	2.170	2.418
	NOAA14	2 HOUR	1.126	1.492	1.770	2.142	2.421	2.707
	NOAA14	3 HOUR	1.194	1.555	1.841	2.236	2.548	2.867
	NOAA14	6 HOUR	1.422	1.804	2.110	2.525	2.847	3.180
	NOAA14	12 HOUR	1.707	2.146	2.493	2.964	3.326	3.698
	NOAA14	24 HOUR	2.118	2.755	3.269	3.996	4.581	5.199

Map Index 64, Cell Number 460, 461, 500, 501, 540, and 541

Appendix B Proposed Conditions Hydrology

- *Proposed Conditions Soils*

Flood Control District of Maricopa County
 Drainage Design Management System
 SOILS

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
OFF05	645	61	64561	0.005	100.00	0.150	-	100	
OFF10	645	61	64561	0.028	100.00	0.150	-	100	
OFF11	645	61	64561	0.004	100.00	0.150	-	100	
OFF20	645	61	64561	0.077	100.00	0.150	-	100	
OFF25	645	61	64561	0.034	100.00	0.150	-	100	
ON05	645	61	64561	0.017	100.00	0.150	-	100	
ON07	645	61	64561	0.002	100.00	0.150	-	100	
ON08	645	61	64561	0.001	100.00	0.150	-	100	
ON09	645	61	64561	0.001	100.00	0.150	-	100	
ON10	645	61	64561	0.007	100.00	0.150	-	100	
ON11	645	61	64561	0.001	100.00	0.150	-	100	
ON12	645	61	64561	0.002	100.00	0.150	-	100	
ON13	645	61	64561	0.006	100.00	0.150	-	100	
ON15	645	61	64561	0.005	100.00	0.150	-	100	
ON18	645	61	64561	0.013	100.00	0.150	-	100	
ON20	645	61	64561	0.003	100.00	0.150	-	100	
ON22	645	61	64561	0.003	100.00	0.150	-	100	
ON25	645	61	64561	0.001	100.00	0.150	-	100	

Appendix B Proposed Conditions Hydrology

- *Proposed Conditions Land Use*

Flood Control District of Maricopa County
 Drainage Design Management System
LAND USE
 Project Reference: ASTERIA PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
OFF05	DESERT	0.0052	100.0	0.35	0	25.0	DRY	0.073	Desert
		0.0052	100.0						
OFF10	DESERT	0.0284	100.0	0.35	0	25.0	DRY	0.063	Desert
		0.0284	100.0						
OFF11	DESERT	0.0038	100.0	0.35	0	25.0	DRY	0.074	Desert
		0.0038	100.0						
OFF20	DESERT	0.0774	100.0	0.35	0	25.0	DRY	0.057	Desert
		0.0774	100.0						
OFF25	DESERT	0.0338	100.0	0.35	0	25.0	DRY	0.062	Desert
		0.0338	100.0						
ON05	120	0.0035	21.2	0.30	5	30.0	NORMAL	0.034	Estate Residential (1/5 du per acre to 1 du per acre)
	130	0.0078	47.3	0.30	15	50.0	NORMAL	0.034	Large Lot Residential - Single Family (1 du per acre to 2 du
	2002	0.0013	7.9	0.05	95	0.0	DRY	0.034	Pavement and Rooftops
	DESERT	0.0039	23.6	0.35	0	25.0	DRY	0.066	Desert
		0.0165	100.0						
ON07	120	0.0004	25.0	0.30	5	30.0	NORMAL	0.039	Estate Residential (1/5 du per acre to 1 du per acre)
	DESERT	0.0012	75.0	0.35	0	25.0	DRY	0.079	Desert
		0.0016	100.0						
ON08	DESERT	0.0014	100.0	0.35	0	25.0	DRY	0.083	Desert
		0.0014	100.0						
ON09	DESERT	0.0013	100.0	0.35	0	25.0	DRY	0.083	Desert

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
LAND USE
 Project Reference: ASTERIA PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
		0.0013	100.0						
ON10	2002	0.0001	1.4	0.05	95	0.0	DRY	0.036	Pavement and Rooftops
	DESERT	0.0071	98.6	0.35	0	25.0	DRY	0.071	Desert
		0.0072	100.0						
ON11	DESERT	0.0013	100.0	0.35	0	25.0	DRY	0.083	Desert
		0.0013	100.0						
ON12	130	0.0008	34.8	0.30	15	50.0	NORMAL	0.039	Large Lot Residential - Single Family (1 du per acre to 2 du
	2002	0.0002	8.7	0.05	95	0.0	DRY	0.039	Pavement and Rooftops
	DESERT	0.0013	56.5	0.35	0	25.0	DRY	0.079	Desert
		0.0023	100.0						
ON13	130	0.0052	82.5	0.30	15	50.0	NORMAL	0.036	Large Lot Residential - Single Family (1 du per acre to 2 du
	2002	0.0007	11.1	0.05	95	0.0	DRY	0.036	Pavement and Rooftops
	DESERT	0.0004	6.3	0.35	0	25.0	DRY	0.072	Desert
		0.0063	99.9						
ON15	130	0.0009	19.1	0.30	15	50.0	NORMAL	0.037	Large Lot Residential - Single Family (1 du per acre to 2 du
	2002	0.0008	17.0	0.05	95	0.0	DRY	0.037	Pavement and Rooftops
	DESERT	0.0030	63.8	0.35	0	25.0	DRY	0.073	Desert
		0.0047	99.9						
ON18	130	0.0118	91.5	0.30	15	50.0	NORMAL	0.034	Large Lot Residential - Single Family (1 du per acre to 2 du
	2002	0.0008	6.2	0.05	95	0.0	DRY	0.034	Pavement and Rooftops

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: ASTERIA PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
ON18	DESERT	0.0003	2.3	0.35	0	25.0	DRY	0.067	Desert
		0.0129	100.0						
ON20	130	0.0010	38.5	0.30	15	50.0	NORMAL	0.038	Large Lot Residential - Single Family (1 du per acre to 2 du
	2002	0.0004	15.4	0.05	95	0.0	DRY	0.038	Pavement and Rooftops
	DESERT	0.0012	46.2	0.35	0	25.0	DRY	0.076	Desert
		0.0026	100.1						
ON22	DESERT	0.0030	100.0	0.35	0	25.0	DRY	0.076	Desert
		0.0030	100.0						
ON25	2002	0.0003	20.0	0.05	95	0.0	DRY	0.041	Pavement and Rooftops
	DESERT	0.0012	80.0	0.35	0	25.0	DRY	0.083	Desert
		0.0015	100.0						

* Non default value

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Project Reference: ASTERIA PROP

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr	
Major Basin ID: 01																		
ON07	0.002	0.05	320.8	270.1	Urban	0.069	0.34	0.36	6.00	0.177	1	Tc (Hrs)	0.137*	0.129*	0.115*	0.101*	0.093*	0.087*
												Vel (f/s)	0.54	0.57	0.64	0.73	0.79	0.84
												R (Hrs)	0.129	0.119	0.106	0.091	0.083	0.077
ON08	0.001	0.07	235.3	230.1	Urban	0.083	0.35	0.40	6.00	0.176		Tc (Hrs)	0.193	0.180	0.160*	0.141*	0.129*	0.120*
												Vel (f/s)	0.53	0.57	0.64	0.73	0.80	0.86
												R (Hrs)	0.364	0.336	0.296	0.256	0.233	0.215
ON10	0.007	0.26	220.1	218.3	Urban	0.071	0.35	0.40	6.00	0.174	1	Tc (Hrs)	0.345	0.322	0.288	0.253	0.232	0.216
												Vel (f/s)	1.11	1.18	1.32	1.51	1.64	1.77
												R (Hrs)	0.654	0.606	0.534	0.463	0.422	0.390
ON12	0.002	0.07	342.5	276.1	Urban	0.061	0.31	0.35	6.00	0.186	13	Tc (Hrs)	0.142*	0.134*	0.121*	0.108*	0.100*	0.094*
												Vel (f/s)	0.72	0.77	0.85	0.95	1.03	1.09
												R (Hrs)	0.174	0.164	0.146	0.129	0.118	0.110
ON15	0.005	0.11	265.5	248.6	Urban	0.060	0.29	0.37	6.00	0.176	19	Tc (Hrs)	0.176	0.167	0.152*	0.136*	0.127*	0.119*
												Vel (f/s)	0.92	0.97	1.06	1.19	1.27	1.36
												R (Hrs)	0.189	0.178	0.160	0.142	0.131	0.122
ON18	0.013	0.19	127.0	127.0	Urban	0.035	0.29	0.26	6.00	0.210	20	Tc (Hrs)	0.213	0.202	0.184	0.164*	0.153*	0.145*
												Vel (f/s)	1.31	1.38	1.51	1.70	1.82	1.92
												R (Hrs)	0.209	0.197	0.178	0.157	0.145	0.136
ON20	0.003	0.08	329.1	272.5	Urban	0.056	0.28	0.34	6.00	0.185	20	Tc (Hrs)	0.140*	0.133*	0.121*	0.108*	0.101*	0.095*
												Vel (f/s)	0.84	0.88	0.97	1.09	1.16	1.24
												R (Hrs)	0.152	0.143	0.129	0.114	0.105	0.099
ON22	0.003	0.26	180.4	180.4	Urban	0.076	0.35	0.40	6.00	0.176		Tc (Hrs)	0.383	0.356	0.318	0.279	0.256	0.239
												Vel (f/s)	1.00	1.07	1.20	1.37	1.49	1.60
												R (Hrs)	1.189	1.099	0.968	0.837	0.762	0.704
ON25	0.001	0.07	328.6	272.4	Urban	0.074	0.29	0.40	6.00	0.167	19	Tc (Hrs)	0.153*	0.145*	0.132*	0.118*	0.110*	0.103*
												Vel (f/s)	0.67	0.71	0.78	0.87	0.93	1.00
												R (Hrs)	0.281	0.265	0.238	0.210	0.194	0.182

* Non default value or value out of range

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Project Reference: ASTERIA PROP

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters				
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr

* Non default value or value out of range

Appendix B Proposed Conditions Hydrology

- *Time of Concentration Map*

Appendix B Proposed Conditions Hydrology

- *Routing Reaches*

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 ROUTING DATA
 Project Reference: ASTERIA PROP

Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)	1.	2.	3.	4.	5.	6.	7.	8.
NORMAL DEPTH														
Major Basin 01														
RON08	0.035	0.035	0.035	284.80	0.0562	-	X: 10.90	45.60	57.90	60.00	62.80	80.90	106.80	119.50
							Y: 2,749.71	2,745.71	2,744.30	2,743.47	2,744.86	2,747.52	2,749.05	2,750.64
RON09	0.035	0.035	0.035	361.10	0.0111	-	X: -	30.80	47.40	61.80	64.90	69.00	81.20	88.10
							Y: 2,753.60	2,749.73	2,748.26	2,746.45	2,747.49	2,749.35	2,752.43	2,753.75
RON10A	0.035	0.035	0.035	1,364.30	0.0374	-	X: 23.50	54.60	78.90	83.60	87.30	92.40	111.30	161.10
							Y: 2,742.00	2,733.67	2,731.88	2,730.29	2,732.80	2,733.37	2,735.66	2,741.50
RON10B	0.035	0.035	0.035	1,599.00	0.0375	-	X: 23.50	54.60	78.90	83.60	87.30	92.40	111.30	161.10
							Y: 2,742.00	2,733.67	2,731.88	2,730.29	2,732.80	2,733.37	2,735.66	2,741.50
RON10C	0.035	0.035	0.035	1,353.80	0.0332	-	X: 12.60	28.00	47.90	52.40	55.50	67.80	76.30	97.20
							Y: 2,720.98	2,718.87	2,717.34	2,716.24	2,717.19	2,718.00	2,718.06	2,719.79
RON10D	0.035	0.035	0.035	533.00	0.0488	-	X: 27.90	27.90	44.00	63.60	66.50	74.50	94.90	125.60
							Y: 2,715.00	2,711.32	2,707.83	2,707.15	2,708.37	2,708.33	2,709.32	2,717.00
RON11	0.035	0.035	0.035	328.70	0.0243	-	X: 35.90	50.00	57.80	62.40	64.10	65.90	69.50	74.20
							Y: 2,761.42	2,759.24	2,758.33	2,757.03	2,757.56	2,758.16	2,758.70	2,759.53
RON15	0.035	0.035	0.035	482.60	0.0518	-	X: -	34.00	36.80	42.90	43.00	45.70	50.00	83.80
							Y: 2,722.15	2,717.32	2,717.22	2,715.48	2,715.48	2,716.96	2,719.25	2,728.77
RON20	0.035	0.035	0.035	226.00	0.0354	-	X: -	67.00	78.70	95.90	103.60	109.20	130.20	221.40
							Y: 2,739.00	2,729.44	2,728.38	2,725.60	2,725.84	2,727.71	2,727.42	2,738.58
RON22A	0.035	0.035	0.035	380.80	0.0315	-	X: -	60.50	72.00	88.30	94.50	100.00	113.70	161.00
							Y: 2,741.78	2,740.72	2,740.72	2,738.78	2,738.91	2,740.99	2,743.04	2,748.80
RON25	0.035	0.035	0.035	219.00	0.0411	-	X: -	33.10	62.10	77.00	81.70	84.60	140.40	158.60
							Y: 2,739.49	2,736.34	2,734.18	2,733.23	2,733.44	2,733.82	2,737.85	2,739.50

Flood Control District of Maricopa County
Drainage Design Management System
HEC-1 ROUTING DATA
Project Reference: ASTERIA PROP

Page 2

2/13/2017

Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)	1.	2.	3.	4.	5.	6.	7.	8.
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Appendix B Proposed Conditions Hydrology

- *Proposed Storage*

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 STORAGE FACILITIES

Storage Basin ID:		SON05										
Spillway Characteristics (SS)			<u>1</u>	<u>2</u>	<u>3</u>	<u>4</u>	<u>5</u>	<u>6</u>	<u>7</u>	<u>8</u>	<u>9</u>	<u>10</u>
Spillway Crest Elevation:	-NA-	Volume (ac-ft)		0.1	0.2	0.3	0.4	0.6	0.8	0.8	0.9	1.0
Spillway Length:	-NA-	Discharge (cfs)	0	0	1	3	4	5	5	9	16	24
Discharge Coefficient:	-NA-	Elevation (ft)	2,710.0	2,710.5	2,711.0	2,711.5	2,712.0	2,712.5	2,713.0	2,713.3	2,713.5	2,713.8
Weir Coefficient:	-NA-											
Low-Level Outlet (SL)			<u>11</u>	<u>12</u>	<u>13</u>	<u>14</u>	<u>15</u>	<u>16</u>	<u>17</u>	<u>18</u>	<u>19</u>	<u>20</u>
Centerline Elevation:	-NA-	Volume (ac-ft)	1.1	-	-	-	-	-	-	-	-	-
Cross-Section Area:	-NA-	Discharge (cfs)	34	0	0	0	0	0	0	0	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	2,714.0	-	-	-	-	-	-	-	-	-
Orifice Equation Exponent:	-NA-											
Top of Dam Overflow (ST)			<u>2 Yr</u>	<u>5 Yr</u>	<u>10 Yr</u>	<u>25 Yr</u>	<u>50 Yr</u>	<u>100 Yr</u>				
Elevation Top of Dam:	-NA-	Peak Volume (ac-ft)	0.00	0.00	0.00	0.00	0.00	0.00				
Length of Dam:	-NA-	Peak Stage (ft)	0.00	0.00	0.00	0.00	0.00	0.00				
Discharge Coefficient:	-NA-	Peak Discharge (cfs)	0	0	0	0	0	0				
Weir Coefficient:	-NA-											

Storage Basin ID:		SON18										
Spillway Characteristics (SS)			<u>1</u>	<u>2</u>	<u>3</u>	<u>4</u>	<u>5</u>	<u>6</u>	<u>7</u>	<u>8</u>	<u>9</u>	<u>10</u>
Spillway Crest Elevation:	-NA-	Volume (ac-ft)		0.1	0.2	0.4	0.6	0.7	0.9	1.1	1.3	1.6
Spillway Length:	-NA-	Discharge (cfs)	0	0	1	3	4	5	5	6	7	7
Discharge Coefficient:	-NA-	Elevation (ft)	2,732.6	2,733.0	2,733.5	2,734.0	2,734.5	2,735.0	2,735.5	2,736.0	2,736.5	2,737.0
Weir Coefficient:	-NA-											
Low-Level Outlet (SL)			<u>11</u>	<u>12</u>	<u>13</u>	<u>14</u>	<u>15</u>	<u>16</u>	<u>17</u>	<u>18</u>	<u>19</u>	<u>20</u>
Centerline Elevation:	-NA-	Volume (ac-ft)	1.7	1.8	1.9	2.1	-	-	-	-	-	-
Cross-Section Area:	-NA-	Discharge (cfs)	21	46	78	116	0	0	0	0	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	2,737.3	2,737.5	2,737.8	2,738.0	-	-	-	-	-	-
Orifice Equation Exponent:	-NA-											
Top of Dam Overflow (ST)			<u>2 Yr</u>	<u>5 Yr</u>	<u>10 Yr</u>	<u>25 Yr</u>	<u>50 Yr</u>	<u>100 Yr</u>				
Elevation Top of Dam:	-NA-	Peak Volume (ac-ft)	0.00	0.00	0.00	0.00	0.00	0.00				
Length of Dam:	-NA-	Peak Stage (ft)	0.00	0.00	0.00	0.00	0.00	0.00				
Discharge Coefficient:	-NA-	Peak Discharge (cfs)	0	0	0	0	0	0				
Weir Coefficient:	-NA-											

Kimley»»Horn

Project **Asteria Highlands**
 Subject **Detention Basin Calculations**
 Designed by **DWT** Date **5/31/2017** Project No. **291102002**
 Checked by _____ Date _____

Objective: to determine the storage-flow relationship for small detention basins

SON05

Drains in 2.76 hours

Outlet Diameter	1.00 ft	Outlet X-Sect Area	0.785 ft ²
Outlet Elevation	2710.5 ft	No. of Outlet Barrels	1
Weir Length	10 ft	Outlet Pipe Slope	0.005 ft/ft
Weir Elevation	2713 ft	Weir Coefficient	2.7

Elevation [ft]	Surface Storage Area [ft ²]	Surface Storage Area [acre]	Average Area [acre]	Δ Elev [ft]	Δ Vol [ac-ft]	Σ Vol [ac-ft]	Δ Time to Drain [hr]	Q _{pipe} [cfs]	Q _{weir} [cfs]	Total Q _{out} [cfs]
2710	7,548	0.17	0.19	0.5	0.09	0	0.00	0.00	0.00	0
2710.5	8,590	0.20	0.21	0.5	0.10	0.09	2.01	0.00	0.00	0
2711	9,635	0.22	0.23	0.5	0.12	0.20	0.75	1.26	0.00	1
2711.5	10,794	0.25	0.26	0.5	0.13	0.31	0.50	2.52	0.00	3
2712	11,936	0.27	0.29	0.5	0.14	0.44	0.41	3.78	0.00	4
2712.5	13,115	0.30	0.32	0.5	0.16	0.59	0.38	4.63	0.00	5
2713	14,426	0.33	0.34	0.3	0.08	0.75	0.14	5.35	0.00	5
2713.25	15,047	0.35	0.35	0.3	0.09	0.83	0.09	5.67	3.38	9
2713.5	15,675	0.36	0.37	0.3	0.09	0.92	0.06	5.98	9.55	16
2713.75	16,312	0.37	0.38	0.3	0.10	1.01	0.04	6.27	17.54	24
2714	16,957	0.39				1.11		6.55	27.00	34

Notes:

Q_{pipe} goes from Mannings Eqn to Orifice Eqn when water surface exceeds 1.2*(Outlet Diameter)
 per Linsley et al. *Water Resources Engineering* 4th Edition, pg 652.

Project *Asterla Highlands*
 Subject *Detention Basin Calculations*
 Designed by **DWT** Date 5/31/2017 Project No. 291102002
 Checked by _____ Date _____

Objective: to determine the storage-flow relationship for small detention basins

SON18

Drains in 3.64 hours

Outlet Diameter 1.00 ft Outlet X-Sect Area 0.785 ft²
 Outlet Elevation 2733 ft No. of Outlet Barrels 1
 Weir Length 40 ft Outlet Pipe Slope 0.005 ft/ft
 Weir Elevation 2737 ft Weir Coefficient 2.7

Elevation	Surface Storage Area	Surface Storage Area	Average Area	Δ Elev	Δ Vol	Σ Vol	Δ Time to Drain	Q _{pipe}	Q _{weir}	Total Q _{out}
[ft]	[ft ²]	[acre]	[acre]	[ft]	[ac-ft]	[ac-ft]	[hr]	[cfs]	[cfs]	[cfs]
2732.6	10,878	0.25	0.26	0.4	0.10	0	0.00	0.00	0.00	0
2733	11,627	0.27	0.28	0.5	0.14	0.10	2.67	0.00	0.00	0
2733.5	12,622	0.29	0.30	0.5	0.15	0.24	0.97	1.26	0.00	1
2734	13,648	0.31	0.33	0.5	0.16	0.39	0.62	2.52	0.00	3
2734.5	14,694	0.34	0.35	0.5	0.17	0.56	0.50	3.78	0.00	4
2735	15,763	0.36	0.37	0.5	0.19	0.73	0.45	4.63	0.00	5
2735.5	16,862	0.39	0.40	0.5	0.20	0.92	0.43	5.35	0.00	5
2736	17,966	0.41	0.43	0.5	0.21	1.12	0.41	5.98	0.00	6
2736.5	19,116	0.44	0.45	0.5	0.23	1.33	0.40	6.55	0.00	7
2737	20,262	0.47	0.47	0.3	0.12	1.56	0.10	7.07	0.00	7
2737.25	20,860	0.48	0.49	0.3	0.12	1.67	0.04	7.32	13.50	21
2737.5	21,466	0.49	0.50	0.3	0.12	1.80	0.02	7.56	38.18	46
2737.75	22,078	0.51	0.51	0.3	0.13	1.92	0.02	7.80	70.15	78
2738	22,697	0.52				2.05		8.02	108.00	116

Notes:
 Q_{pipe} goes from Mannings Eqn to Crifice Eqn when water surface exceeds 1.2*(Outlet Diameter)
 per Linsley et al. *Water Resources Engineering* 4th Edition, pg 652.

Appendix B Proposed Conditions Hydrology

- *Proposed Diversions*

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 DIVERSIONS
 Project Reference: ASTERIA PROP

Diversion ID/ DT Card ID	Maximum Volume (ac-ft)	Maximum Diversion (cfs)	1.	2.	3.	4.	5.	6.	7.	8.	9.	10.
DON18	0.30	100.0										
		Inflow (cfs)		1	3	4	5	6	7	21	46	78
		Diversion (cfs)		1	2	2	3	3	4	11	23	39

Appendix B Proposed Conditions Hydrology

- *HEC-1 Results*

 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * JUN 1998 *
 * VERSION 4.1 *
 * RUN DATE 05DEC17 TIME 20:16:59 *

Proposed 2-year

 * U.S. ARMY CORPS OF ENGINEERS *
 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 756-1104 *

```

X   X   XXXXXXX   XXXXX   X
X   X   X   X   X   X   IX
X   X   X   X   X   X
XXXXXXXX XXXX   X   XXXXX   X
X   X   X   X   X   X
X   X   X   X   X   X
X   X   XXXXXXX   XXXXX   XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Flood Control District of Maricopa County									
2	ID	ASTERIA PROP - ASTERIA PROP									
3	ID	2 YEAR									
4	ID	6 Hour Storm									
5	ID	Unit Hydrograph: Clark									
6	ID	Storm: Multiple									
7	ID	06/01/2017									
	*DIAGRAM										
8	IT	3	1JAN99	0	400						
9	IO	5									
10	IN	15									
	*										
11	JD	1.422	0.0001								
12	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
13	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
14	PC	0.962	0.972	0.983	0.991	1.000					
15	JD	1.413	0.5000								
16	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
17	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
18	PC	0.962	0.972	0.983	0.991	1.000					
	*										
19	KK	ON07	BASIN								
20	BA	0.002									
21	LG	0.34	0.36	6.00	0.18	1					
22	UC	0.137	0.129								
23	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
24	UA	100									
	*										
25	KK	ON05	BASIN								
26	BA	0.017									
27	LG	0.29	0.30	6.00	0.19	16					
28	UC	0.276	0.354								
29	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
30	UA	100									
	*										
31	KK	SON05	STORAGE								
32	KO	3	3								
33	RS	1	STOR								
34	SV	0.0	0.20	0.31	0.44	0.59	0.75	0.83	0.92	1.01	1.11
35	SE	2710.0	2711.00	2711.50	2712.00	2712.50	2713.00	2713.25	2713.50	2713.75	2714.00
36	SQ	0.0	1.0	3.0	4.0	5.0	5.0	9.0	16.0	24.0	34.0
	*										
37	KK	OFF05	BASIN								
38	BA	0.005									
39	LG	0.35	0.40	6.00	0.18	0					
40	UC	0.182	0.152								
41	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
42	UA	100									
	*										

1 HEC-1 INPUT PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
43	KK	RON08	ROUTE								
44	RS	1	FLOW								
45	RC	0.035	0.035	0.035	285	0.0562	0.00				
46	RX	10.90	45.60	57.90	60.00	62.80	80.90	106.80	119.50		
47	RY	2749.7	2745.71	2744.30	2743.47	2744.86	2747.52	2749.05	2750.64		
	*										

48	KK	ON08	BASIN										
49	BA	0.001											
50	LG	0.35	0.40	6.00	0.18	0							
51	UC	0.193	0.364										
52	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
53	UA	100											
	*												
54	KK	CP08	COMBINE										
55	HC	2											
	*												
56	KK	RON10C	ROUTE										
57	RS	1	FLOW										
58	RC	0.035	0.035	0.035	1354	0.0332	0.00						
59	RX	12.60	28.00	47.90	52.40	55.50	67.80	76.30	97.20				
60	RY	2721.0	2718.87	2717.34	2716.24	2717.19	2718.00	2718.06	2719.79				
	*												
61	KK	OFF10	BASIN										
62	BA	0.028											
63	LG	0.35	0.40	6.00	0.18	0							
64	UC	0.403	0.498										
65	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
66	UA	100											
	*												
67	KK	RON09	ROUTE										
68	RS	1	FLOW										
69	RC	0.035	0.035	0.035	361	0.0111	0.00						
70	RX	0.00	30.80	47.40	61.80	64.90	69.00	81.20	88.10				
71	RY	2753.6	2749.73	2748.26	2746.45	2747.49	2749.35	2752.43	2753.75				
	*												
72	KK	ON09	BASIN										
73	BA	0.001											
74	LG	0.35	0.40	6.00	0.18	0							
75	UC	0.195	0.409										
76	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
77	UA	100											
	*												

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

78	KK	CP09	COMBINE										
79	HC	2											
	*												
80	KK	RON10A	ROUTE										
81	RS	1	FLOW										
82	RC	0.035	0.035	0.035	1364	0.0374	0.00						
83	RX	23.50	54.60	78.90	83.60	87.30	92.40	111.30	161.10				
84	RY	2742.0	2733.67	2731.88	2730.29	2732.80	2733.37	2735.66	2741.50				
	*												
85	KK	OFF11	BASIN										
86	BA	0.0038											
87	LG	0.35	0.40	6.00	0.18	0							
88	UC	0.165	0.139										
89	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
90	UA	100											
	*												
91	KK	RON11	ROUTE										
92	RS	1	FLOW										
93	RC	0.035	0.035	0.035	329	0.0243	0.00						
94	RX	35.90	50.00	57.80	62.40	64.10	65.90	69.50	74.20				
95	RY	2761.4	2759.24	2758.33	2757.03	2757.56	2758.16	2758.70	2759.53				
	*												
96	KK	ON11	BASIN										
97	BA	0.001											
98	LG	0.35	0.40	6.00	0.18	0							
99	UC	0.212	0.493										
100	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
101	UA	100											
	*												
102	KK	CP11	COMBINE										
103	HC	2											
	*												
104	KK	RON10B	ROUTE										
105	RS	1	FLOW										
106	RC	0.035	0.035	0.035	1599	0.0375	0.00						
107	RX	23.50	54.60	78.90	83.60	87.30	92.40	111.30	161.10				
108	RY	2742.0	2733.67	2731.88	2730.29	2732.80	2733.37	2735.66	2741.50				
	*												
109	KK	ON13	BASIN										
110	BA	0.006											
111	LG	0.28	0.28	6.00	0.20	23							
112	UC	0.167	0.171										
113	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
114	UA	100											
	*												

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

186 RS 1 FLOW
187 RC 0.035 0.035 0.035 219 0.0411 0.00
188 RX 0.00 33.10 62.10 77.00 81.70 84.60 140.40 158.60
189 RY 2739.5 2736.34 2734.18 2733.23 2733.44 2733.82 2737.85 2739.50

1

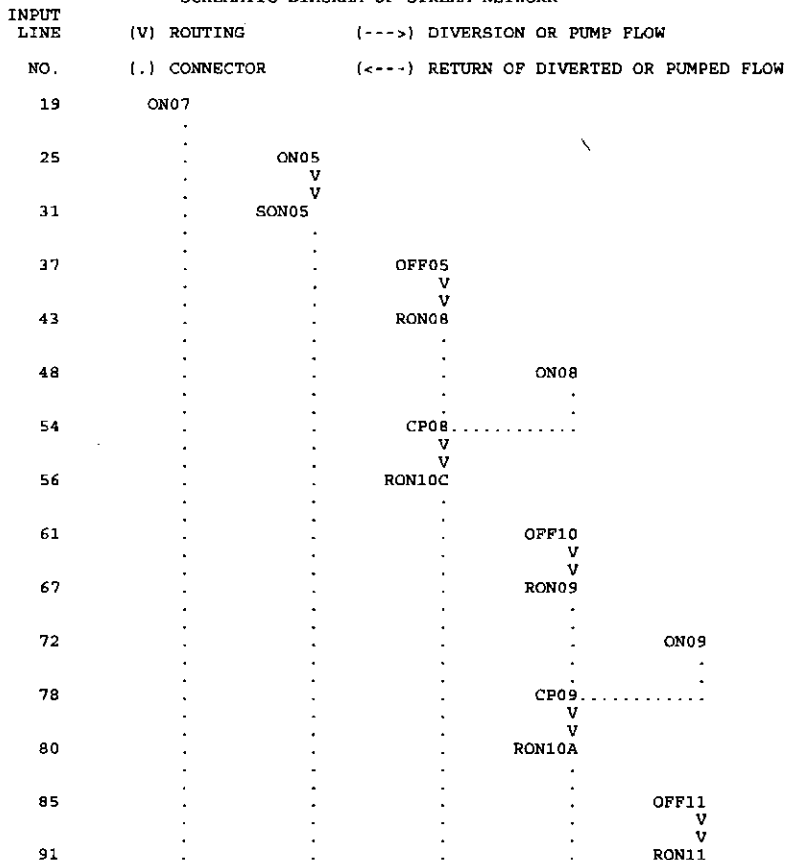
HEC-1 INPUT

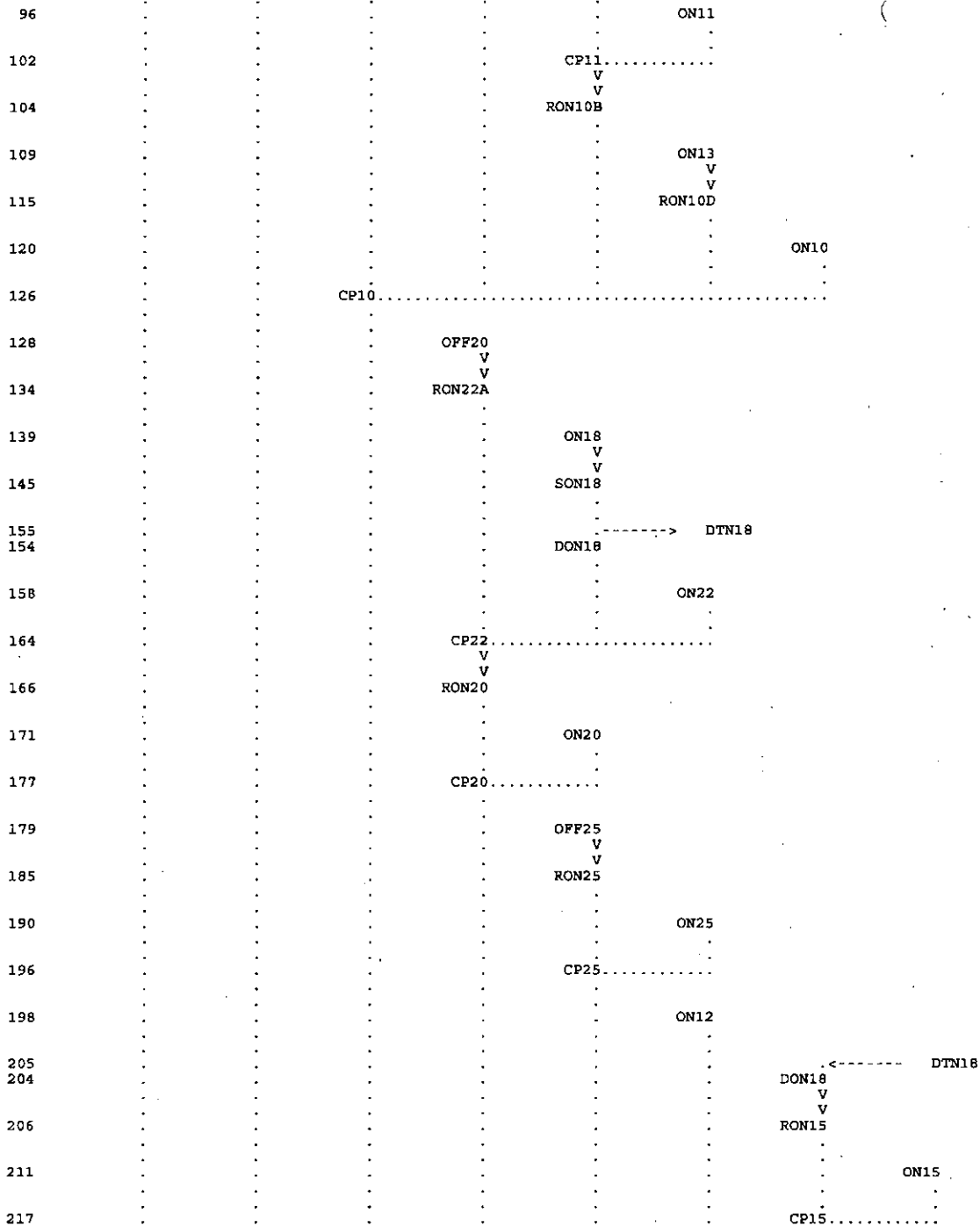
PAGE 6

LINE	ID	1	2	3	4	5	6	7	8	9	10
190	KK	ON25	BASIN								
191	BA	0.001									
192	LG	0.29	0.40	6.00	0.17	19					
193	UC	0.153	0.281								
194	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
195	UA	100									
196	KK	CP25	COMBINE								
197	HC	2									
198	KK	ON12	BASIN								
199	BA	0.002									
200	LG	0.31	0.35	6.00	0.19	13					
201	UC	0.142	0.174								
202	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
203	UA	100									
204	KK	DON18	RETRIEVE								
205	DR	DTN18									
206	KK	RON15	ROUTE								
207	RS	1	FLOW								
208	RC	0.035	0.035	0.035	483	0.0518	0.00				
209	RX	0.00	34.00	36.80	42.90	43.00	45.70	50.00	83.90		
210	RY	2722.2	2717.32	2717.22	2715.48	2715.48	2716.96	2719.25	2728.77		
211	KK	ON15	BASIN								
212	BA	0.0047									
213	LG	0.29	0.37	6.00	0.18	19					
214	UC	0.176	0.189								
215	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
216	UA	100									
217	KK	CP15	COMBINE								
218	HC	2									
219	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK





(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 05DEC17 TIME 20:16:59
*
*****
    
```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
    
```

Flood Control District of Maricopa County
 ASTERIA PROP - ASTERIA PROP
 2 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 06/01/2017

9 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 3 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 400 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 1JAN99 ENDING DATE
 NDTIME 1957 ENDING TIME
 ICENT 19 CENTURY MARK
 COMPUTATION INTERVAL .05 HOURS
 TOTAL TIME BASE 19.95 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

11 JD INDEX STORM NO. 1
 STRM 1.42 PRECIPITATION DEPTH
 TRDA .00 TRANSPOSITION DRAINAGE AREA

12 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .03 .03 .03 .03 .03 .09 .09 .09 .09
 .02 .02 .02 .02 .02 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00

15 JD INDEX STORM NO. 2
 STRM 1.41 PRECIPITATION DEPTH
 TRDA .50 TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
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 .03 .03 .03 .03 .03 .09 .09 .09 .09
 .02 .02 .02 .02 .02 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00

*** **

31 KK *****
 * *
 * SON05 * STORAGE
 * *

32 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 3 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

33 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC .00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

34 SV STORAGE .0 .2 .3 .4 .6 .8 .8 .9 1.0 1.1

35 SE ELEVATION 2710.00 2711.00 2711.50 2712.00 2712.50 2713.00 2713.25 2713.50 2713.75 2714.00

36 SQ DISCHARGE 0. 1. 3. 4. 5. 5. 9. 16. 24. 34.

*** *** *** *** ***

HYDROGRAPH AT STATION SON05
 TRANSPOSITION AREA .0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	19.95-HR
3.	4.60	1.	0.	0.	0.	
		(INCHES)	.485	.541	.541	.541
		(AC-FT)	0.	0.	0.	0.

PEAK STORAGE + (AC-FT)	TIME (HR)	(CFS)	MAXIMUM AVERAGE STORAGE			
			6-HR	24-HR	72-HR	19.95-HR
0.	4.60	0.	0.	0.	0.	

PEAK STAGE + (FEET)	TIME (HR)	(CFS)	MAXIMUM AVERAGE STAGE			
			6-HR	24-HR	72-HR	19.95-HR
2711.43	4.60	2710.69	2710.24	2710.24	2710.24	

CUMULATIVE AREA = .02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SON05
 TRANSPOSITION AREA .5 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	19.95-HR
3.	4.60	1.	0.	0.	0.	
		(INCHES)	.479	.535	.535	.535
		(AC-FT)	0.	0.	0.	0.

PEAK STORAGE + (AC-FT)	TIME (HR)	(CFS)	MAXIMUM AVERAGE STORAGE			
			6-HR	24-HR	72-HR	19.95-HR
0.	4.60	0.	0.	0.	0.	

PEAK STAGE + (FEET)	TIME (HR)	(CFS)	MAXIMUM AVERAGE STAGE			
			6-HR	24-HR	72-HR	19.95-HR
2711.42	4.60	2710.69	2710.24	2710.24	2710.24	

CUMULATIVE AREA = .02 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT SON05

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	19.95-HR
3.	4.60	1.	0.	0.	0.	
		(INCHES)	.481	.537	.537	.537
		(AC-FT)	0.	0.	0.	0.

CUMULATIVE AREA = .02 SQ MI

*** **

 * *
 145 KK * SON18 * STORAGE
 * *

146 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 3 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

147 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC .00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

148 SV	STORAGE	.0	.1	.2	.4	.6	.7	.9	1.1	1.3	1.6
		1.7	1.8	1.9	2.0						

150 SE	ELEVATION	2732.60	2733.00	2733.50	2734.00	2734.50	2735.00	2735.50	2736.00	2736.50	2737.00
		2737.30	2737.50	2737.75	2738.00						

152 SQ	DISCHARGE	0.	0.	1.	3.	4.	5.	5.	6.	7.	7.
		21.	46.	78.	116.						

*** *** *** *** ***

HYDROGRAPH AT STATION SON18
 TRANSPOSITION AREA .0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	19.95-HR
+ (CFS)	(HR)	(CFS)				
+ 2.	4.50		1.	0.	0.	0.
		(INCHES)	.427	.442	.442	.442
		(AC-FT)	0.	0.	0.	0.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
+ (AC-FT)	(HR)		6-HR	24-HR	72-HR	19.95-HR
+ 0.	4.45		0.	0.	0.	0.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
+ (FEET)	(HR)		6-HR	24-HR	72-HR	19.95-HR
+ 2733.72	4.50		2733.27	2733.02	2733.02	2733.02
CUMULATIVE AREA =			.01 SQ MI			

*** **

HYDROGRAPH AT STATION SON18
 TRANSPORTATION AREA .5 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	19.95-HR
+ (CFS)	(HR)	(CFS)				
+ 2.	4.50		1.	0.	0.	0.
		(INCHES)	.421	.435	.435	.435
		(AC-FT)	0.	0.	0.	0.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
+ (AC-FT)	(HR)		6-HR	24-HR	72-HR	19.95-HR
+ 0.	4.45		0.	0.	0.	0.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
+ (FEET)	(HR)		6-HR	24-HR	72-HR	19.95-HR
+ 2733.72	4.50		2733.27	2733.01	2733.01	2733.01
CUMULATIVE AREA =			.01 SQ MI			

*** **

INTERPOLATED HYDROGRAPH AT SON18

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	19.95-HR
+ (CFS)	(HR)	(CFS)				
+ 2.	4.50		1.	0.	0.	0.
		(INCHES)	.424	.438	.438	.438
		(AC-FT)	0.	0.	0.	0.
CUMULATIVE AREA =			.01 SQ MI			

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+ HYDROGRAPH AT	ON07	1.	4.05	0.	0.	0.	.00		
+ HYDROGRAPH AT	ON05	9.	4.10	1.	0.	0.	.02		
+ ROUTED TO	SON05	3.	4.60	1.	0.	0.	.02		
+ HYDROGRAPH AT	OFF05	3.	4.05	0.	0.	0.	.00		
+ ROUTED TO	RON08	3.	4.10	0.	0.	0.	.00		
+ HYDROGRAPH AT	ON08	0.	4.10	0.	0.	0.	.00		
+ 2 COMBINED AT	CP08	3.	4.10	0.	0.	0.	.01		
+ ROUTED TO	RON10C	3.	4.15	0.	0.	0.	.01		
+ HYDROGRAPH AT	OFF10	7.	4.20	1.	0.	0.	.03		
+ ROUTED TO	RON09	7.	4.25	1.	0.	0.	.03		
+ HYDROGRAPH AT	ON09	0.	4.10	0.	0.	0.	.00		

+	2 COMBINED AT	CP09	7.	4.25	1.	0.	0.	.03
	ROUTED TO	RON10A	7.	4.30	1.	0.	0.	.03
+	HYDROGRAPH AT	OFF11	2.	4.05	0.	0.	0.	.00
+	ROUTED TO	RON11	2.	4.10	0.	0.	0.	.00
+	HYDROGRAPH AT	ON11	0.	4.10	0.	0.	0.	.00
+	2 COMBINED AT	CP11	3.	4.10	0.	0.	0.	.00
+	ROUTED TO	RON10B	2.	4.20	0.	0.	0.	.00
+	HYDROGRAPH AT	ON13	5.	4.05	0.	0.	0.	.01
+	ROUTED TO	RON10D	5.	4.10	0.	0.	0.	.01
+	HYDROGRAPH AT	ON10	2.	4.20	0.	0.	0.	.01
+	5 COMBINED AT	CP10	15.	4.20	2.	1.	1.	.05
+	HYDROGRAPH AT	OFF20	17.	4.25	2.	1.	1.	.08
+	ROUTED TO	RON22A	17.	4.25	2.	1.	1.	.08
+	HYDROGRAPH AT	ON18	9.	4.05	1.	0.	0.	.01
+	ROUTED TO	SON18	2.	4.50	1.	0.	0.	.01
+	DIVERSION TO	DTN18	1.	4.50	0.	0.	0.	.01
+	HYDROGRAPH AT	DON18	1.	4.50	0.	0.	0.	.01
+	HYDROGRAPH AT	ON22	0.	4.25	0.	0.	0.	.00
+	3 COMBINED AT	CP22	18.	4.25	3.	1.	1.	.09
+	ROUTED TO	RON20	18.	4.30	3.	1.	1.	.09
+	HYDROGRAPH AT	ON20	2.	4.05	0.	0.	0.	.00
+	2 COMBINED AT	CP20	19.	4.25	3.	1.	1.	.10
+	HYDROGRAPH AT	OFF25	8.	4.20	1.	0.	0.	.03
+	ROUTED TO	RON25	8.	4.20	1.	0.	0.	.03
+	HYDROGRAPH AT	ON25	1.	4.05	0.	0.	0.	.00
+	2 COMBINED AT	CP25	9.	4.20	1.	0.	0.	.04
+	HYDROGRAPH AT	ON12	1.	4.05	0.	0.	0.	.00
+	HYDROGRAPH AT	DON18	1.	4.50	0.	0.	0.	.01
+	ROUTED TO	RON15	1.	4.50	0.	0.	0.	.01
+	HYDROGRAPH AT	ON15	3.	4.05	0.	0.	0.	.00
+	2 COMBINED AT	CP15	4.	4.05	1.	0.	0.	.00

*** NORMAL END OF HEC-1 ***

48	KK	ON08	BASIN										
49	BA	0.001											
50	LG	0.35	0.40	6.00	0.18	0							
51	UC	0.160	0.296										
52	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
53	UA	100											
	*												
54	KK	CP08	COMBINE										
55	HC	2											
	*												
56	KK	RON10C	ROUTE										
57	RS	1	FLOW										
58	RC	0.035	0.035	0.035	1354	0.0332	0.00						
59	RX	12.60	28.00	47.90	52.40	55.50	67.80	76.30	97.20				
60	RY	2721.0	2718.87	2717.34	2716.24	2717.19	2718.00	2718.06	2719.79				
	*												
61	KK	OFF10	BASIN										
62	BA	0.028											
63	LG	0.35	0.40	6.00	0.18	0							
64	UC	0.335	0.405										
65	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
66	UA	100											
	*												
67	KK	RON09	ROUTE										
68	RS	1	FLOW										
69	RC	0.035	0.035	0.035	361	0.0111	0.00						
70	RX	0.00	30.80	47.40	61.80	64.90	69.00	81.20	88.10				
71	RY	2753.6	2749.73	2748.26	2746.45	2747.49	2749.35	2752.43	2753.75				
	*												
72	KK	ON09	BASIN										
73	BA	0.001											
74	LG	0.35	0.40	6.00	0.18	0							
75	UC	0.162	0.333										
76	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
77	UA	100											
	*												

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

78	KK	CP09	COMBINE										
79	HC	2											
	*												
80	KK	RON10A	ROUTE										
81	RS	1	FLOW										
82	RC	0.035	0.035	0.035	1364	0.0374	0.00						
83	RX	23.50	54.60	78.90	83.60	87.30	92.40	111.30	161.10				
84	RY	2742.0	2733.67	2731.88	2730.29	2732.80	2733.37	2735.66	2741.50				
	*												
85	KK	OFF11	BASIN										
86	BA	0.0038											
87	LG	0.35	0.40	6.00	0.18	0							
88	UC	0.137	0.113										
89	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
90	UA	100											
	*												
91	KK	RON11	ROUTE										
92	RS	1	FLOW										
93	RC	0.035	0.035	0.035	329	0.0243	0.00						
94	RX	35.90	50.00	57.80	62.40	64.10	65.90	69.50	74.20				
95	RY	2761.4	2759.24	2758.33	2757.03	2757.56	2758.16	2758.70	2759.53				
	*												
96	KK	ON11	BASIN										
97	BA	0.001											
98	LG	0.35	0.40	6.00	0.18	0							
99	UC	0.176	0.401										
100	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
101	UA	100											
	*												
102	KK	CP11	COMBINE										
103	HC	2											
	*												
104	KK	RON10B	ROUTE										
105	RS	1	FLOW										
106	RC	0.035	0.035	0.035	1599	0.0375	0.00						
107	RX	23.50	54.60	78.90	83.60	87.30	92.40	111.30	161.10				
108	RY	2742.0	2733.67	2731.88	2730.29	2732.80	2733.37	2735.66	2741.50				
	*												
109	KK	ON13	BASIN										
110	BA	0.006											
111	LG	0.28	0.28	6.00	0.20	23							
112	UC	0.145	0.146										
113	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
114	UA	100											
	*												

HEC-1 INPUT

PAGE 4

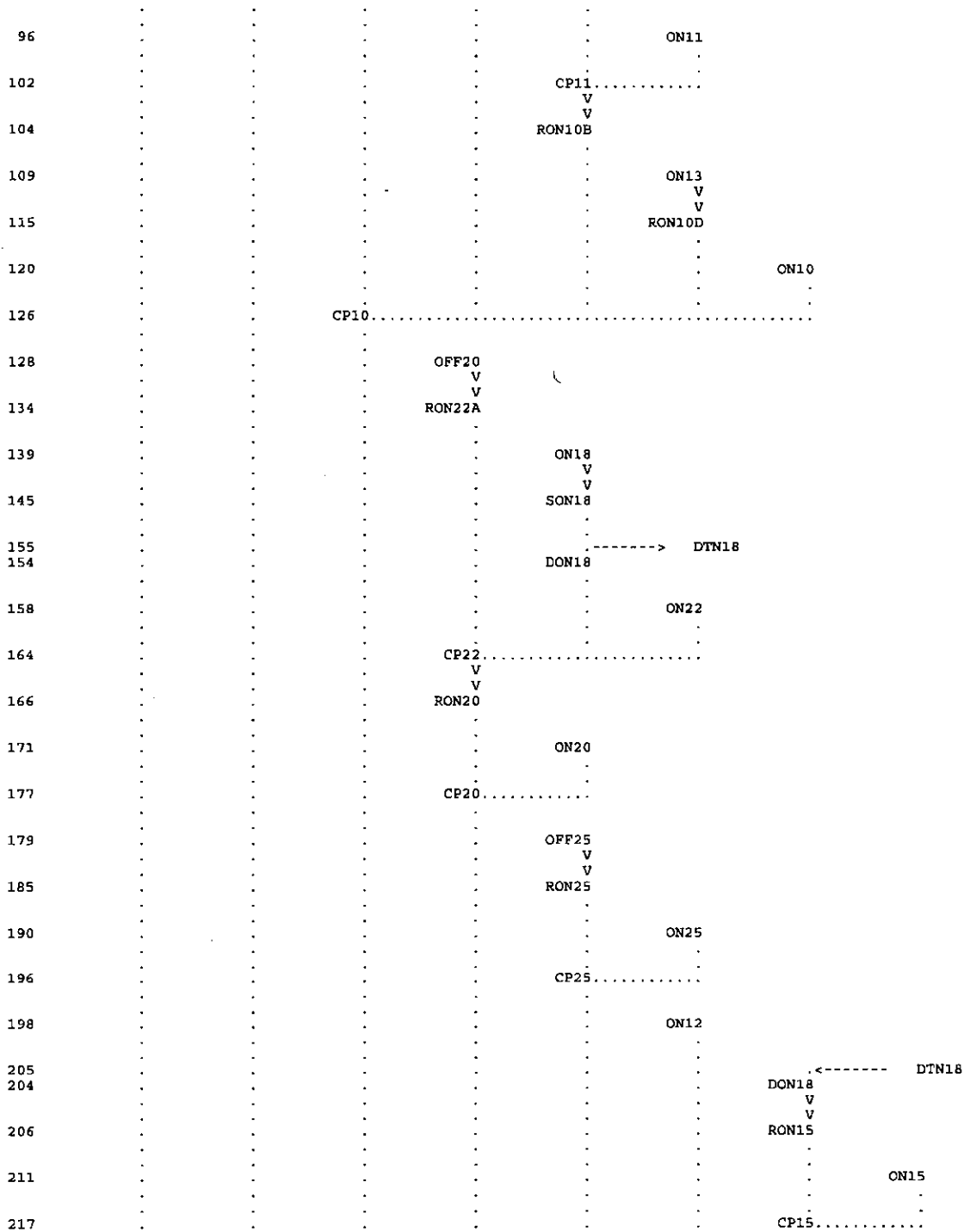
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

115	KK	RON10D	ROUTE																		
116	RS	1	FLOW																		
117	RC	0.035	0.035	0.035	533	0.0488	0.00														
118	RX	27.90	27.90	44.00	63.60	66.50	74.50	94.90	125.60												
119	RY	2715.0	2711.32	2707.83	2707.15	2708.37	2708.33	2709.32	2717.00												
	*																				
120	KK	ON10	BASIN																		
121	BA	0.007																			
122	LG	0.35	0.40	6.00	0.17	1															
123	UC	0.288	0.534																		
124	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0										
125	UA	100																			
	*																				
126	KK	CP10	COMBINE																		
127	HC	5																			
	*																				
128	KK	OFF20	BASIN																		
129	BA	0.077																			
130	LG	0.35	0.40	6.00	0.18	0															
131	UC	0.406	0.451																		
132	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0										
133	UA	100																			
	*																				
134	KK	RON22A	ROUTE																		
135	RS	1	FLOW																		
136	RC	0.035	0.035	0.035	381	0.0315	0.00														
137	RX	0.00	60.50	72.00	88.30	94.50	100.00	113.70	161.00												
138	RY	2741.8	2740.72	2740.72	2738.78	2738.91	2740.99	2743.04	2748.80												
	*																				
139	KK	ON18	BASIN																		
140	BA	0.013																			
141	LG	0.29	0.26	6.00	0.21	20															
142	UC	0.184	0.178																		
143	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0										
144	UA	100																			
	*																				
145	KK	SON18	STORAGE																		
146	KO	3	3																		
147	RS	1	STOR																		
148	SV		0.10	0.24	0.39	0.56	0.73	0.92	1.12	1.33	1.56										
149	SV	1.67	1.80	1.92	2.05																
150	SE	2732.6	2733.00	2733.50	2734.00	2734.50	2735.00	2735.50	2736.00	2736.50	2737.00										
151	SE	2737.3	2737.50	2737.75	2738.00																
152	SQ	0.0	0.0	1.0	3.0	4.0	5.0	5.0	6.0	7.0	7.0										
153	SQ	21.0	46.0	78.0	116.0																
	*																				

HEC-1 INPUT

PAGE 5

LINE	ID	1	2	3	4	5	6	7	8	9	10
154	KK	DON18	DIVERT								
155	DT	DTN18	0.30	100.0							
156	DI	0.0	1.0	3.0	4.0	5.0	6.0	7.0	21.0	46.0	78.0
157	DQ	0.0	0.5	1.5	2.0	2.5	3.0	3.5	10.5	23.0	39.0
	*										
158	KK	ON22	BASIN								
159	BA	0.003									
160	LG	0.35	0.40	6.00	0.18	0					
161	UC	0.318	0.968								
162	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
163	UA	100									
	*										
164	KK	CP22	COMBINE								
165	HC	3									
	*										
166	KK	RON20	ROUTE								
167	RS	1	FLOW								
168	RC	0.035	0.035	0.035	226	0.0354	0.00				
169	RX	0.00	67.00	78.70	95.90	103.60	109.20	130.20	221.40		
170	RY	2739.0	2729.44	2728.38	2725.60	2725.84	2727.71	2727.42	2738.58		
	*										
171	KK	ON20	BASIN								
172	BA	0.003									
173	LG	0.28	0.34	6.00	0.19	20					
174	UC	0.121	0.129								
175	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
176	UA	100									
	*										
177	KK	CP20	COMBINE								
178	HC	2									
	*										
179	KK	OFF25	BASIN								
180	BA	0.034									
181	LG	0.35	0.40	6.00	0.18	0					
182	UC	0.338	0.438								
183	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
184	UA	100									
	*										
185	KK	RON25	ROUTE								



(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   JUN 1998 *
*   VERSION 4.1 *
* RUN DATE 05DEC17 TIME 20:17:07 *
*****
    
```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

Flood Control District of Maricopa County
 ASTERIA PROP - ASTERIA PROP
 10 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 06/01/2017

9 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 3 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 400 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 1JAN99 ENDING DATE
 NDTIME 1957 ENDING TIME
 ICENT 19 CENTURY MARK
 COMPUTATION INTERVAL .05 HOURS
 TOTAL TIME BASE 19.95 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

11 JD INDEX STORM NO. 1
 STRM 2.11 PRECIPITATION DEPTH
 TRDA .00 TRANSPOSITION DRAINAGE AREA

12 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .03 .03 .03 .03 .03 .09 .09 .09 .09 .09
 .02 .02 .02 .02 .02 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00

15 JD INDEX STORM NO. 2
 STRM 2.10 PRECIPITATION DEPTH
 TRDA .50 TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .03 .03 .03 .03 .03 .09 .09 .09 .09 .09
 .02 .02 .02 .02 .02 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00

*** **

31 KK *****
 * SON05 * STORAGE
 * *****

32 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 3 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

33 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC .00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

34 SV STORAGE .0 .2 .3 .4 .6 .8 .8 .9 1.0 1.1
 35 SE ELEVATION 2710.00 2711.00 2711.50 2712.00 2712.50 2713.00 2713.25 2713.50 2713.75 2714.00
 36 SQ DISCHARGE 0. 1. 3. 4. 5. 5. 9. 16. 24. 34.

*** **

HYDROGRAPH AT STATION SON05
 TRANSPOSITION AREA .0 SQ MI

+	2 COMBINED AT	CP09	21.	4.15	2.	1.	1.	.03
	ROUTED TO	RON10A	20.	4.25	2.	1.	1.	.03
+	HYDROGRAPH AT	OFF11	6.	4.00	0.	0.	0.	.00
	ROUTED TO	RON11	6.	4.05	0.	0.	0.	.00
+	HYDROGRAPH AT	ON11	1.	4.10	0.	0.	0.	.00
+	2 COMBINED AT	CP11	7.	4.05	0.	0.	0.	.00
	ROUTED TO	RON10B	6.	4.10	0.	0.	0.	.00
+	HYDROGRAPH AT	ON13	10.	4.05	1.	0.	0.	.01
	ROUTED TO	RON10D	9.	4.05	1.	0.	0.	.01
+	HYDROGRAPH AT	ON10	4.	4.15	1.	0.	0.	.01
+	5 COMBINED AT	CP10	41.	4.15	4.	1.	1.	.05
	HYDROGRAPH AT	OFF20	50.	4.20	6.	2.	2.	.08
+	ROUTED TO	RON22A	50.	4.20	6.	2.	2.	.08
	HYDROGRAPH AT	ON18	19.	4.05	1.	0.	0.	.01
+	ROUTED TO	SON18	4.	4.40	1.	0.	0.	.01
	DIVERSION TO	DTN18	2.	4.40	1.	0.	0.	.01
+	HYDROGRAPH AT	DON18	2.	4.40	1.	0.	0.	.01
	HYDROGRAPH AT	ON22	1.	4.20	0.	0.	0.	.00
+	3 COMBINED AT	CP22	53.	4.20	7.	2.	2.	.09
	ROUTED TO	RON20	53.	4.20	7.	2.	2.	.09
+	HYDROGRAPH AT	ON20	5.	4.00	0.	0.	0.	.00
+	2 COMBINED AT	CP20	55.	4.20	7.	2.	2.	.10
	HYDROGRAPH AT	OFF25	24.	4.15	3.	1.	1.	.03
+	ROUTED TO	RON25	23.	4.15	3.	1.	1.	.03
	HYDROGRAPH AT	ON25	1.	4.05	0.	0.	0.	.00
+	2 COMBINED AT	CP25	24.	4.15	3.	1.	1.	.04
	HYDROGRAPH AT	ON12	3.	4.00	0.	0.	0.	.00
+	HYDROGRAPH AT	DON18	2.	4.40	1.	0.	0.	.01
	ROUTED TO	RON15	2.	4.45	1.	0.	0.	.01
+	HYDROGRAPH AT	ON15	7.	4.05	1.	0.	0.	.00
+	2 COMBINED AT	CP15	8.	4.05	1.	0.	0.	.00

*** NORMAL END OF HEC-1 ***

Proposed 100-year

 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * JUN 1998 *
 * VERSION 4.1 *
 * RUN DATE 05DEC17 TIME 20:17:16 *

 * U.S. ARMY CORPS OF ENGINEERS *
 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 756-1104 *

```

X   X   XXXXXXX   XXXXX   X
X   X   X   X   X   X   X
X   X   X   X   X   X   X
XXXXXXXX XXXX   X   XXXXX   X
X   X   X   X   X   X   X
X   X   X   X   X   X   X
X   X   XXXXXXX   XXXXX   XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GRBEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Flood Control District of Maricopa County									
2	ID	ASTERIA PROP - ASTERIA PROP									
3	ID	100 YEAR									
4	ID	6 Hour Storm									
5	ID	Unit Hydrograph: Clark									
6	ID	Storm: Multiple									
7	ID	06/01/2017									
	*DIAGRAM										
8	IT	3	1JAN99	0	400						
9	IO	5									
10	IN	15									
	*										
11	JD	3.180	0.0001								
12	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
13	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
14	PC	0.962	0.972	0.983	0.991	1.000					
15	JD	3.161	0.5000								
16	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
17	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
18	PC	0.962	0.972	0.983	0.991	1.000					
	*										
19	KK	ON07	BASIN								
20	BA	0.002									
21	LG	0.34	0.36	6.00	0.18	1					
22	UC	0.087	0.077								
23	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
24	UA	100									
	*										
25	KK	ON05	BASIN								
26	BA	0.017									
27	LG	0.29	0.30	6.00	0.19	16					
28	UC	0.185	0.227								
29	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
30	UA	100									
	*										
31	KK	SON05	STORAGE								
32	KO	3	3								
33	RS	1	STOR								
34	SV	0.0	0.20	0.31	0.44	0.59	0.75	0.83	0.92	1.01	1.11
35	SE	2710.0	2711.00	2711.50	2712.00	2712.50	2713.00	2713.25	2713.50	2713.75	2714.00
36	SQ	0.0	1.0	3.0	4.0	5.0	5.0	9.0	16.0	24.0	34.0
	*										
37	KK	OFF05	BASIN								
38	BA	0.005									
39	LG	0.35	0.40	6.00	0.18	0					
40	UC	0.113	0.090								
41	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
42	UA	100									
	*										

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
43	KK	RON08	ROUTE								
44	RS	1	FLOW								
45	RC	0.035	0.035	0.035	285	0.0562	0.00				
46	RX	10.90	45.60	57.90	60.00	62.80	80.90	106.80	119.50		
47	RY	2749.7	2745.71	2744.30	2743.47	2744.86	2747.52	2749.05	2750.64		
	*										

48	KK	ON08	BASIN											
49	BA	0.001												
50	LG	0.35	0.40	6.00	0.18	0								
51	UC	0.120	0.215											
52	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
53	UA	100												
	*													
54	KK	CP08	COMBINE											
55	HC	2												
	*													
56	KK	RON10C	ROUTE											
57	RS	1	FLOW											
58	RC	0.035	0.035	0.035	1354	0.0332	0.00							
59	RX	12.60	28.00	47.90	52.40	55.50	67.80	76.30	97.20					
60	RY	2721.0	2718.87	2717.34	2716.24	2717.19	2718.00	2718.06	2719.79					
	*													
61	KK	OFF10	BASIN											
62	BA	0.028												
63	LG	0.35	0.40	6.00	0.18	0								
64	UC	0.251	0.295											
65	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
66	UA	100												
	*													
67	KK	RON09	ROUTE											
68	RS	1	FLOW											
69	RC	0.035	0.035	0.035	361	0.0111	0.00							
70	RX	0.00	30.80	47.40	61.80	64.90	69.00	81.20	88.10					
71	RY	2753.6	2749.73	2748.26	2746.45	2747.49	2749.35	2752.43	2753.75					
	*													
72	KK	ON09	BASIN											
73	BA	0.001												
74	LG	0.35	0.40	6.00	0.18	0								
75	UC	0.121	0.242											
76	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
77	UA	100												
	*													

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

78	KK	CP09	COMBINE											
79	HC	2												
	*													
80	KK	RON10A	ROUTE											
81	RS	1	FLOW											
82	RC	0.035	0.035	0.035	1364	0.0374	0.00							
83	RX	23.50	54.60	78.90	83.60	87.30	92.40	111.30	161.10					
84	RY	2742.0	2733.67	2731.88	2730.29	2732.80	2733.37	2735.66	2741.50					
	*													
85	KK	OFF11	BASIN											
86	BA	0.0038												
87	LG	0.35	0.40	6.00	0.18	0								
88	UC	0.103	0.082											
89	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
90	UA	100												
	*													
91	KK	RON11	ROUTE											
92	RS	1	FLOW											
93	RC	0.035	0.035	0.035	329	0.0243	0.00							
94	RX	35.90	50.00	57.80	62.40	64.10	65.90	69.50	74.20					
95	RY	2761.4	2759.24	2758.33	2757.03	2757.56	2758.16	2758.70	2759.53					
	*													
96	KK	ON11	BASIN											
97	BA	0.001												
98	LG	0.35	0.40	6.00	0.18	0								
99	UC	0.132	0.292											
100	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
101	UA	100												
	*													
102	KK	CP11	COMBINE											
103	HC	2												
	*													
104	KK	RON10B	ROUTE											
105	RS	1	FLOW											
106	RC	0.035	0.035	0.035	1599	0.0375	0.00							
107	RX	23.50	54.60	78.90	83.60	87.30	92.40	111.30	161.10					
108	RY	2742.0	2733.67	2731.88	2730.29	2732.80	2733.37	2735.66	2741.50					
	*													
109	KK	ON13	BASIN											
110	BA	0.006												
111	LG	0.28	0.28	6.00	0.20	23								
112	UC	0.114	0.113											
113	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
114	UA	100												
	*													

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

115	KK	RON10D	ROUTE																			
116	RS	1	FLOW																			
117	RC	0.035	0.035	0.035	533	0.0488	0.00															
118	RX	27.90	27.90	44.00	63.60	66.50	74.50	94.90	125.60													
119	RY	2715.0	2711.32	2707.83	2707.15	2708.37	2708.33	2709.32	2717.00													
*																						
120	KK	ON10	BASIN																			
121	BA	0.007																				
122	LG	0.35	0.40	6.00	0.17	1																
123	UC	0.216	0.390																			
124	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0											
125	UA	100																				
*																						
126	KK	CP10	COMBINE																			
127	HC	5																				
*																						
128	KK	OFF20	BASIN																			
129	BA	0.077																				
130	LG	0.35	0.40	6.00	0.18	0																
131	UC	0.305	0.328																			
132	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0											
133	UA	100																				
*																						
134	KK	RON22A	ROUTE																			
135	RS	1	FLOW																			
136	RC	0.035	0.035	0.035	381	0.0315	0.00															
137	RX	0.00	60.50	72.00	88.30	94.50	100.00	113.70	161.00													
138	RY	2741.8	2740.72	2740.72	2738.78	2738.91	2740.99	2743.04	2748.80													
*																						
139	KK	ON18	BASIN																			
140	BA	0.013																				
141	LG	0.29	0.26	6.00	0.21	20																
142	UC	0.145	0.136																			
143	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0											
144	UA	100																				
*																						
145	KK	SON18	STORAGE																			
146	KC	3	3																			
147	RS	1	STOR																			
148	SV		0.10	0.24	0.39	0.56	0.73	0.92	1.12	1.33	1.56											
149	SV	1.67	1.80	1.92	2.05																	
150	SE	2732.6	2733.00	2733.50	2734.00	2734.50	2735.00	2735.50	2736.00	2736.50	2737.00											
151	SE	2737.3	2737.50	2737.75	2738.00																	
152	SQ	0.0	0.0	1.0	3.0	4.0	5.0	5.0	6.0	7.0	7.0											
153	SQ	21.0	46.0	78.0	116.0																	
*																						
HEC-1 INPUT																						
LINE	ID1.....2.....3.....4.....5.....6.....7.....8.....9.....10																				
154	KK	DON18	DIVERT																			
155	DT	DTN18	0.30	100.0																		
156	DI	0.0	1.0	3.0	4.0	5.0	6.0	7.0	21.0	46.0	78.0											
157	DQ	0.0	0.5	1.5	2.0	2.5	3.0	3.5	10.5	23.0	39.0											
*																						
158	KK	ON22	BASIN																			
159	BA	0.003																				
160	LG	0.35	0.40	6.00	0.18	0																
161	UC	0.239	0.704																			
162	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0											
163	UA	100																				
*																						
164	KK	CP22	COMBINE																			
165	HC	3																				
*																						
166	KK	RON20	ROUTE																			
167	RS	1	FLOW																			
168	RC	0.035	0.035	0.035	226	0.0354	0.00															
169	RX	0.00	67.00	78.70	95.90	103.60	109.20	130.20	221.40													
170	RY	2739.0	2729.44	2728.38	2725.60	2725.84	2727.71	2727.42	2738.58													
*																						
171	KK	ON20	BASIN																			
172	BA	0.003																				
173	LG	0.28	0.34	6.00	0.19	20																
174	UC	0.095	0.099																			
175	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0											
176	UA	100																				
*																						
177	KK	CP20	COMBINE																			
178	HC	2																				
*																						
179	KK	OFF25	BASIN																			
180	BA	0.034																				
181	LG	0.35	0.40	6.00	0.18	0																
182	UC	0.253	0.318																			
183	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0											
184	UA	100																				
*																						
185	KK	RON25	ROUTE																			

1

PAGE 5

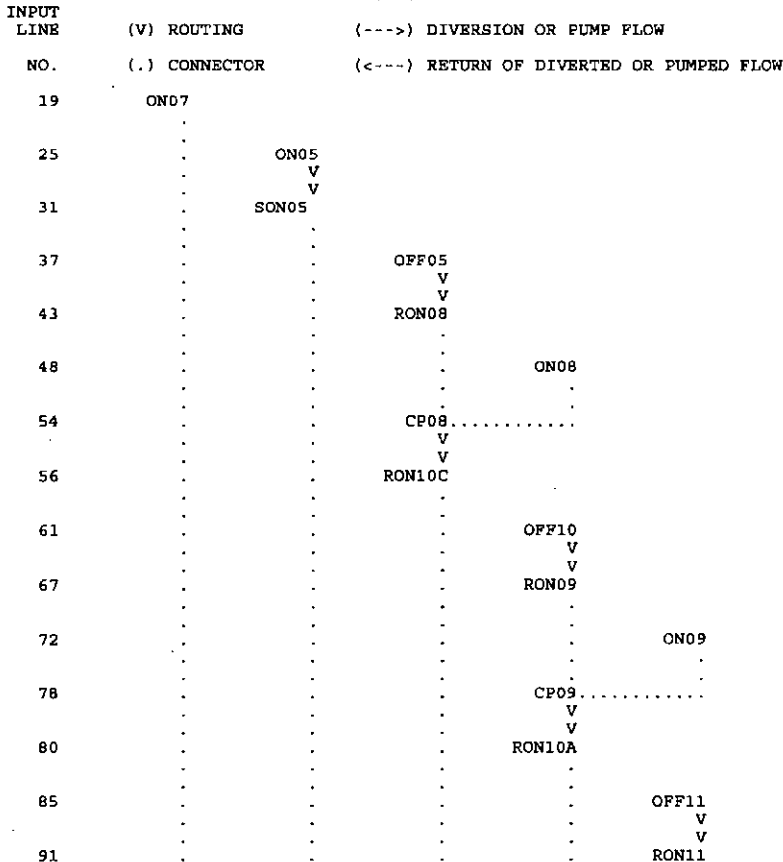
186	RS	1	FLOW								
187	RC	0.035	0.035	0.035	219	0.0411	0.00				
188	RX	0.00	33.10	62.10	77.00	81.70	84.60	140.40	158.60		
189	RY	2739.5	2736.34	2734.18	2733.23	2733.44	2733.82	2737.85	2739.50		

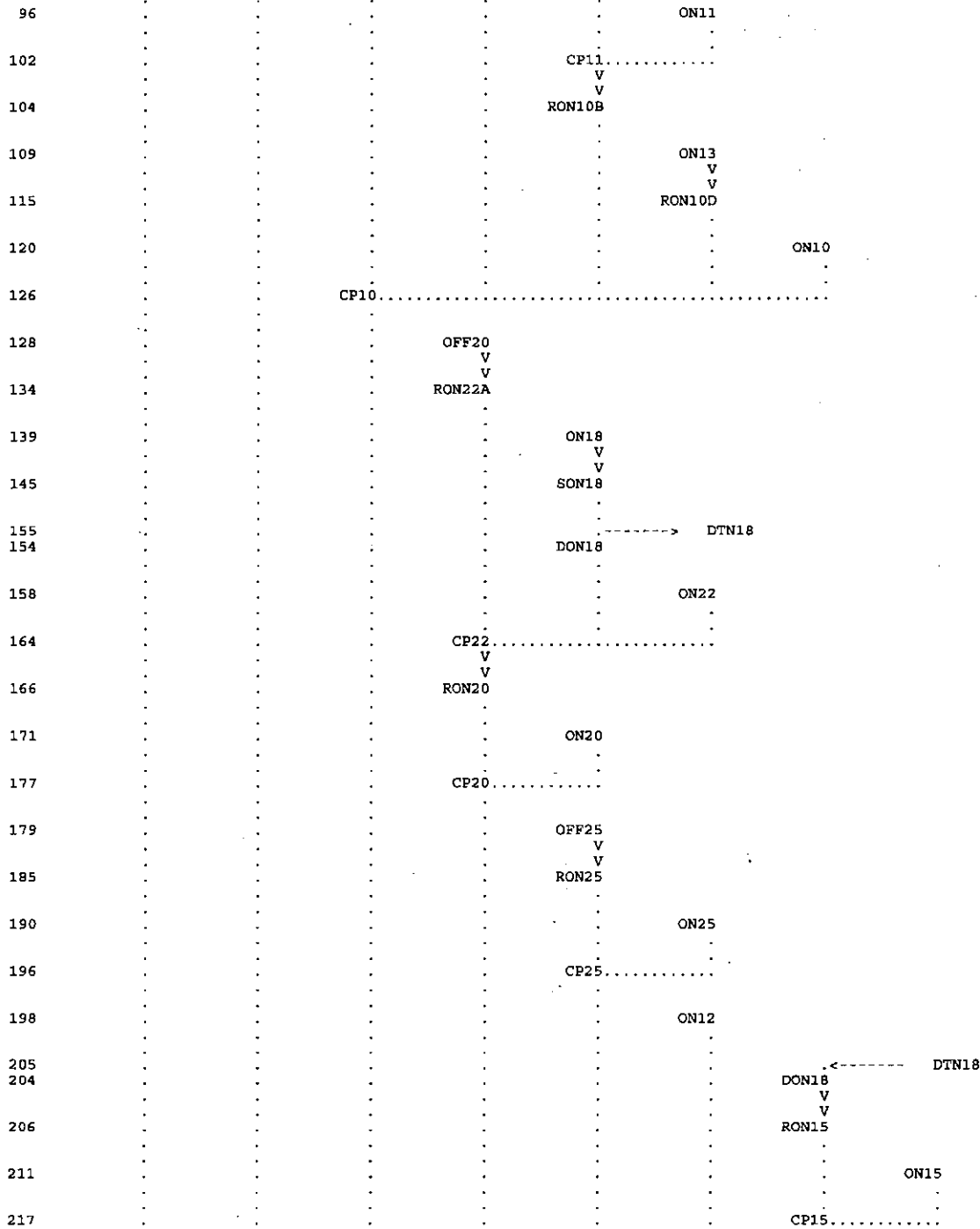
HEC-1 INPUT

PAGE 6

LINE	ID	1	2	3	4	5	6	7	8	9	10
190	KK	ON25	BASIN								
191	BA	0.001									
192	LG	0.29	0.40	6.00	0.17	19					
193	UC	0.103	0.182								
194	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
195	UA	100									
	*										
196	KK	CP25	COMBINE								
197	HC	2									
	*										
198	KK	ON12	BASIN								
199	BA	0.002									
200	LG	0.31	0.35	6.00	0.19	13					
201	UC	0.094	0.110								
202	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
203	UA	100									
	*										
204	KK	DN18	RETRIEVE								
205	DR	DTN18									
	*										
206	KK	RON15	ROUTE								
207	RS	1	FLOW								
208	RC	0.035	0.035	0.035	483	0.0518	0.00				
209	RX	0.00	34.00	36.80	42.90	43.00	45.70	50.00	83.90		
210	RY	2722.2	2717.32	2717.22	2715.48	2715.48	2716.96	2719.25	2728.77		
	*										
211	KK	ON15	BASIN								
212	BA	0.0047									
213	LG	0.29	0.37	6.00	0.18	19					
214	UC	0.119	0.122								
215	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
216	UA	100									
	*										
217	KK	CP15	COMBINE								
218	HC	2									
	*										
219	ZZ										

SCHEMATIC DIAGRAM OF STREAM NETWORK





(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 05DEC17 TIME 20:17:16 *
*****
    
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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

Flood Control District of Maricopa County
 ASTERIA PROP - ASTERIA PROP
 100 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 06/01/2017

9 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 3 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 400 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 1JAN99 ENDING DATE
 NDTIME 1957 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .05 HOURS
 TOTAL TIME BASE 19.95 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

11 JD INDEX STORM NO. 1
 STRM 3.18 PRECIPITATION DEPTH
 TRDA .00 TRANSPOSITION DRAINAGE AREA

12	PI	PRECIPITATION PATTERN											
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.03	.03	.03	.03	.03	.09	.09	.09	.09	.09	.09	.09
		.02	.02	.02	.02	.02	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

15 JD INDEX STORM NO. 2
 STRM 3.16 PRECIPITATION DEPTH
 TRDA .50 TRANSPOSITION DRAINAGE AREA

16	PI	PRECIPITATION PATTERN											
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.03	.03	.03	.03	.03	.09	.09	.09	.09	.09	.09	.09
		.02	.02	.02	.02	.02	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

*** **

```

*****
*                                     *
31 KK * SON05 * STORAGE
*                                     *
*****
    
```

32 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 3 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

33 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC .00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

34	SV	STORAGE	.0	.2	.3	.4	.6	.8	.8	.9	1.0	1.1
35	SE	ELEVATION	2710.00	2711.00	2711.50	2712.00	2712.50	2713.00	2713.25	2713.50	2713.75	2714.00
36	SQ	DISCHARGE	0.	1.	3.	4.	5.	5.	9.	16.	24.	34.

*** **

HYDROGRAPH AT STATION SON05
 TRANSPOSITION AREA .0 SQ MI

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24-HR	72-HR	19.95-HR
+ (CFS)	(HR)						
		(CFS)	3.	1.	1.	1.	1.
		(INCHES)	1.804	1.907	1.907	1.907	1.907
		(AC-FT)	2.	2.	2.	2.	2.
PEAK STORAGE	TIME		6-HR	MAXIMUM AVERAGE STORAGE	24-HR	72-HR	19.95-HR
+ (AC-FT)	(HR)						
			0.	0.	0.	0.	0.
PEAK STAGE	TIME		6-HR	MAXIMUM AVERAGE STAGE	24-HR	72-HR	19.95-HR
+ (FEET)	(HR)						
			2711.48	2710.50	2710.50	2710.50	2710.50
CUMULATIVE AREA =			.02 SQ MI				

*** **

HYDROGRAPH AT STATION SON05
 TRANSPOSITION AREA .5 SQ MI

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24-HR	72-HR	19.95-HR
+ (CFS)	(HR)						
		(CFS)	3.	1.	1.	1.	1.
		(INCHES)	1.788	1.891	1.891	1.891	1.891
		(AC-FT)	2.	2.	2.	2.	2.
PEAK STORAGE	TIME		6-HR	MAXIMUM AVERAGE STORAGE	24-HR	72-HR	19.95-HR
+ (AC-FT)	(HR)						
			0.	0.	0.	0.	0.
PEAK STAGE	TIME		6-HR	MAXIMUM AVERAGE STAGE	24-HR	72-HR	19.95-HR
+ (FEET)	(HR)						
			2711.47	2710.50	2710.50	2710.50	2710.50
CUMULATIVE AREA =			.02 SQ MI				

*** **

INTERPOLATED HYDROGRAPH AT SON05

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24-HR	72-HR	19.95-HR
+ (CFS)	(HR)						
		(CFS)	3.	1.	1.	1.	1.
		(INCHES)	1.795	1.897	1.897	1.897	1.897
		(AC-FT)	2.	2.	2.	2.	2.
CUMULATIVE AREA =			.02 SQ MI				

145 KK *****
 * SON18 * STORAGE
 * *

146 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 3 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

147 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC .00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

148 SV	STORAGE	.0	.1	.2	.4	.6	.7	.9	1.1	1.3	1.6
		1.7	1.8	1.9	2.0						
150 SE	ELEVATION	2732.60	2733.00	2733.50	2734.00	2734.50	2735.00	2735.50	2736.00	2736.50	2737.00
		2737.30	2737.50	2737.75	2738.00						
152 SQ	DISCHARGE	0.	0.	1.	3.	4.	5.	5.	6.	7.	7.
		21.	46.	78.	116.						

*** **

HYDROGRAPH AT STATION SON18
 TRANSPOSITION AREA .0 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	19.95-HR
6.	4.40	2.	1.	1.	1.	
		(INCHES)	1.770	1.824	1.824	1.824
		(AC-FT)	1.	1.	1.	1.
PEAK STORAGE		TIME		MAXIMUM AVERAGE STORAGE		
+ (AC-FT)	(HR)	6-HR	24-HR	72-HR	19.95-HR	
1.	4.40	0.	0.	0.	0.	
PEAK STAGE		TIME		MAXIMUM AVERAGE STAGE		
+ (FEET)	(HR)	6-HR	24-HR	72-HR	19.95-HR	
2735.84	4.40	2734.07	2733.28	2733.28	2733.28	
CUMULATIVE AREA =			.01 SQ MI			

*** **

HYDROGRAPH AT STATION SON18
 TRANSPOSITION AREA .5 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	19.95-HR
6.	4.40	2.	1.	1.	1.	
		(INCHES)	1.755	1.809	1.809	1.809
		(AC-FT)	1.	1.	1.	1.
PEAK STORAGE		TIME		MAXIMUM AVERAGE STORAGE		
+ (AC-FT)	(HR)	6-HR	24-HR	72-HR	19.95-HR	
1.	4.40	0.	0.	0.	0.	
PEAK STAGE		TIME		MAXIMUM AVERAGE STAGE		
+ (FEET)	(HR)	6-HR	24-HR	72-HR	19.95-HR	
2735.82	4.40	2734.06	2733.28	2733.28	2733.28	
CUMULATIVE AREA =			.01 SQ MI			

*** **

INTERPOLATED HYDROGRAPH AT SON18

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	19.95-HR
6.	4.40	2.	1.	1.	1.	
		(INCHES)	1.762	1.815	1.815	1.815
		(AC-FT)	1.	1.	1.	1.
CUMULATIVE AREA =			.01 SQ MI			

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+ HYDROGRAPH AT	ON07	6.	4.00	0.	0.	0.	.00		
+ HYDROGRAPH AT	ON05	38.	4.05	3.	1.	1.	.02		
+ ROUTED TO	SON05	19.	4.30	3.	1.	1.	.02		
+ HYDROGRAPH AT	OFF05	15.	4.00	1.	0.	0.	.00		
+ ROUTED TO	RON08	15.	4.00	1.	0.	0.	.00		
+ HYDROGRAPH AT	ON08	2.	4.05	0.	0.	0.	.00		
+ 2 COMBINED AT	CP08	17.	4.00	1.	0.	0.	.01		
+ ROUTED TO	RON10C	15.	4.05	1.	0.	0.	.01		
+ HYDROGRAPH AT	OFF10	50.	4.10	5.	1.	1.	.03		
+ ROUTED TO	RON09	49.	4.10	5.	1.	1.	.03		
+ HYDROGRAPH AT	ON09	2.	4.05	0.	0.	0.	.00		

+	2 COMBINED AT	CP09	51.	4.10	5.	1.	1.	.03
	ROUTED TO	RON10A	49.	4.15	5.	1.	1.	.03
	HYDROGRAPH AT	OFF11	12.	4.00	1.	0.	0.	.00
	ROUTED TO	RON11	11.	4.00	1.	0.	0.	.00
	HYDROGRAPH AT	ON11	2.	4.05	0.	0.	0.	.00
	2 COMBINED AT	CP11	13.	4.00	1.	0.	0.	.00
	ROUTED TO	RON10B	12.	4.05	1.	0.	0.	.00
	HYDROGRAPH AT	ON13	18.	4.00	1.	0.	0.	.01
	ROUTED TO	RON10D	17.	4.05	1.	0.	0.	.01
	HYDROGRAPH AT	ON10	11.	4.10	1.	0.	0.	.01
	5 COMBINED AT	CP10	98.	4.10	9.	3.	3.	.05
	HYDROGRAPH AT	OFF20	125.	4.10	13.	4.	4.	.08
	ROUTED TO	RON22A	124.	4.15	13.	4.	4.	.08
	HYDROGRAPH AT	ON18	35.	4.00	3.	1.	1.	.01
	ROUTED TO	SON18	6.	4.40	2.	1.	1.	.01
	DIVERSION TO	DTN18	3.	4.40	1.	0.	0.	.01
	HYDROGRAPH AT	DON18	5.	5.35	2.	1.	1.	.01
	HYDROGRAPH AT	ON22	3.	4.10	1.	0.	0.	.00
	3 COMBINED AT	CP22	130.	4.15	15.	5.	5.	.09
	ROUTED TO	RON20	131.	4.15	15.	5.	5.	.09
	HYDROGRAPH AT	ON20	9.	4.00	1.	0.	0.	.00
	2 COMBINED AT	CP20	134.	4.15	16.	5.	5.	.10
	HYDROGRAPH AT	OFF25	58.	4.10	6.	2.	2.	.03
	ROUTED TO	RON25	58.	4.10	6.	2.	2.	.03
	HYDROGRAPH AT	ON25	3.	4.00	0.	0.	0.	.00
	2 COMBINED AT	CP25	60.	4.10	6.	2.	2.	.04
	HYDROGRAPH AT	ON12	6.	4.00	0.	0.	0.	.00
	HYDROGRAPH AT	DON18	3.	4.40	1.	0.	0.	.01
	ROUTED TO	RON15	3.	4.40	1.	0.	0.	.01
	HYDROGRAPH AT	ON15	13.	4.00	1.	0.	0.	.00
	2 COMBINED AT	CP15	15.	4.00	2.	0.	0.	.00

*** NORMAL END OF HEC-1 ***

Appendix C Stormwater Waiver



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA - - ZN - - UP - - DR - - PP - PC#

The applicant/developer must complete and submit this form to the city for processing and obtain approval of waiver request **before submitting improvement plans**. Denial of the waiver may require the developer to submit a revised site plan to the Development Review Board.

Date 2/17/2017 Project Name Asteria Highlands
 Project Location East of 128th Street, North of Pinnacle Peak Road alignment.
 Applicant Contact Zach Schmidt, P.E., CFM Company Name Kimley-Horn
 Phone 602-906-1116 Fax _____ E-mail zach.schmidt@kimley-horn.com
 Address 7740 N. 16th Street, Suite 300, Phoenix, AZ 85020

Waiver Criteria

A project must meet at least one of three criteria listed below for the city to consider waiving some or all required stormwater storage. **However, regardless of the criteria, a waiver will only be granted if the applicant can demonstrate that the effect of a waiver will not increase the potential for flooding on any property.** Check the applicable box and provide a signed engineering report and supporting engineering analysis that demonstrate the project meets the criteria and that the effect of a waiver will not increase the potential for flooding on any property.

If the runoff for the project has been included in a storage facility at another location, the applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility.

- 1. The development is adjacent to a conveyance facility that an engineering analysis shows is designed and constructed to handle the additional runoff from the site as a result of development.
- 2. The development is on a parcel less than one-half acre in size.
- 3. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO).

For a full storage waiver, a conflict with ESLO is limited to:

- Property located in the hillside landform as defined in the city Zoning Ordinance
- Property in the upper desert landform that has a land slope steeper than 5% as defined in the city Zoning Ordinance
- Property within the ESL zoning overlay district where the only viable location for a stormwater storage basin requires blasting

This full waiver only applies to those portions of property meeting one of these three requirements.

Partial waivers are available for projects or portions of properties within the Environmentally Sensitive Lands Zoning Overlay District, not meeting any of the three full waiver criteria above, if post-development peak discharge rates do not exceed pre-development conditions, based on the 10- and 100-year storm events.

By signing below, I certify that the stated project meets the waiver criteria selected above as demonstrated by the attached documentation.



 Engineer

12/15/2017

 Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA -

- ZN -

- UP -

- DR -

6

- PP - 2016

PC#

CITY STAFF TO COMPLETE THIS PAGE

Project Name _____

Check Appropriate Boxes:

Meets waiver criteria (specify): 1 2 3

Recommend approve waiver.

Recommend deny waiver:

None of waiver criteria met.

Downstream conditions prohibit waiver of any storage.

Other:

Explain: _____

Return waiver request:

Insufficient data provided.

Other: _____

Explain: _____

Recommended Conditions of Waiver:

All storage requirements waived.

Post-development peak discharge rates do not exceed pre-development conditions.

Other:

Explain: _____

Waiver approved per above conditions.

Waiver denied.

Floodplain Administrator or Designee

Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA - - ZN - - UP - - DR - 6 - PP - 2016 PC#

In-Lieu Fee and In-Kind Contributions

In-lieu fees are only applicable to projects where post-development peak discharge rates exceed pre-development levels, based on the 10- and 100-year storm events. If the city grants a waiver, the developer is required to calculate and contribute an in-lieu fee based on what it would cost the city to provide a storage basin, sized as described below, including costs such as land acquisition, construction, landscaping, design, construction management, and maintenance over a 75-year design life. The fee for this cost is \$1.87 per cubic foot of stormwater storage for a virtual storage basin designed to mitigate the increase in runoff associated with the 100-year/2-hour storm event. The applicant may submit site-specific in-lieu fee calculations subject to the Floodplain Administrator's approval.

The Floodplain Administrator considers in-kind contributions on a case-by-case basis. An in-kind contribution can serve as part of or instead of the calculated in-lieu fee. In-kind contributions must be stormwater related and must constitute a public benefit. In-lieu fees and in-kind contributions are subject to the approval of the Floodplain Administrator or designee.

Project Name _____

The waived stormwater storage volume is calculated using a simplified approach as follows:

V = ΔCRA; where

V = stormwater storage volume required, in cubic feet,

ΔC = increase in weighted average runoff coefficient over disturbed area ($C_{post} - C_{pre}$),

R = 100-year/2-hour precipitation depth, in feet (DSPM, Appendix 4-1D, page 11), and

A = area of disturbed ground, in square feet

Furthermore,

R = _____

ΔC = _____

A = _____

$V_w = V - V_p$; where

V = _____

V_w = volume waived,

V_p = _____

V = volume required, and

V_w = _____

V_p = volume provided

An in-lieu fee will be paid, based on the following calculations and supporting documentation:

In-lieu fee (\$) = V_w (cu. ft.) x \$1.87 per cubic foot = _____

An in-kind contribution will be made, as follows:

No in-lieu fee is required. Reason:

Approved by:

Floodplain Administrator or Designee

Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 ♦ Phone: 480-312-2500 ♦ Fax: 480-312-7781

Appendix D Hydraulic Analysis

- *CP10 Normal Depth Analysis*
- *HEC-RAS Results*

Appendix D Hydraulic Analysis

- *CP10 Normal Depth Analysis*

Worksheet for Pre CP10 2-year Downstream XS

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Channel Slope 0.02850 ft/ft
Discharge 14.00 ft³/s

Section Definitions

Station (ft)	Elevation (ft)
0+00	2705.53
0+10	2704.39
0+58	2695.63
0+67	2695.47
0+73	2695.71
0+79	2696.28
0+99	2696.89
1+62	2701.34
1+86	2703.56

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 2705.53)	(1+86, 2703.56)	0.035

Options

Current Roughness Weighted Method Pavlovskii's Method
Open Channel Weighting Method Pavlovskii's Method
Closed Channel Weighting Method Pavlovskii's Method

Results

Normal Depth 0.39 ft
Elevation Range 2695.47 to 2705.53 ft
Flow Area 4.79 ft²
Wetted Perimeter 18.39 ft

Worksheet for Pre CP10 2-year Downstream XS

Results

Hydraulic Radius	0.26	ft
Top Width	18.36	ft
Normal Depth	0.39	ft
Critical Depth	0.39	ft
Critical Slope	0.02795	ft/ft
Velocity	2.92	ft/s
Velocity Head	0.13	ft
Specific Energy	0.52	ft
Froude Number	1.01	
Flow Type	Supercritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.39	ft
Critical Depth	0.39	ft
Channel Slope	0.02850	ft/ft
Critical Slope	0.02795	ft/ft

Cross Section for Pre CP10 2-year Downstream XS

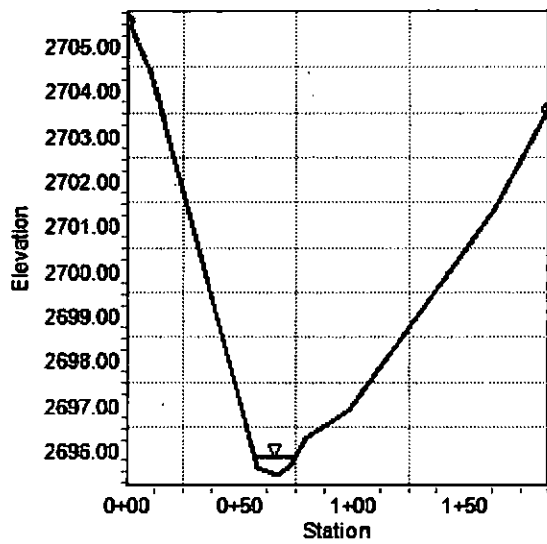
Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Channel Slope	0.02850	ft/ft
Normal Depth	0.39	ft
Discharge	14.00	ft ³ /s

Cross Section Image



Worksheet for Post CP10 2-year Downstream XS

Project Description

Friction Method Manning Formula
 Solve For Normal Depth

Input Data

Channel Slope 0.02850 ft/ft
 Discharge 15.00 ft³/s
 Section Definitions

Station (ft)	Elevation (ft)
--------------	----------------

0+00	2705.53
0+10	2704.39
0+58	2695.63
0+67	2695.47
0+73	2695.71
0+79	2696.28
0+99	2696.89
1+62	2701.34
1+86	2703.56

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
---------------	----------------	-----------------------

(0+00, 2705.53)	(1+86, 2703.56)	0.035
-----------------	-----------------	-------

Options

Current Roughness weighted Method Pavlovskii's Method
 Open Channel Weighting Method Pavlovskii's Method
 Closed Channel Weighting Method Pavlovskii's Method

Results

Normal Depth 0.40 ft
 Elevation Range 2695.47 to 2705.53 ft
 Flow Area 5.01 ft²
 Wetted Perimeter 18.59 ft

Worksheet for Post CP10 2-year Downstream XS

Results

Hydraulic Radius	0.27	ft
Top Width	18.55	ft
Normal Depth	0.40	ft
Critical Depth	0.40	ft
Critical Slope	0.02761	ft/ft
Velocity	2.99	ft/s
Velocity Head	0.14	ft
Specific Energy	0.54	ft
Froude Number	1.01	
Flow Type	Supercritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.40	ft
Critical Depth	0.40	ft
Channel Slope	0.02850	ft/ft
Critical Slope	0.02761	ft/ft

Cross Section for Post CP10 2-year Downstream XS

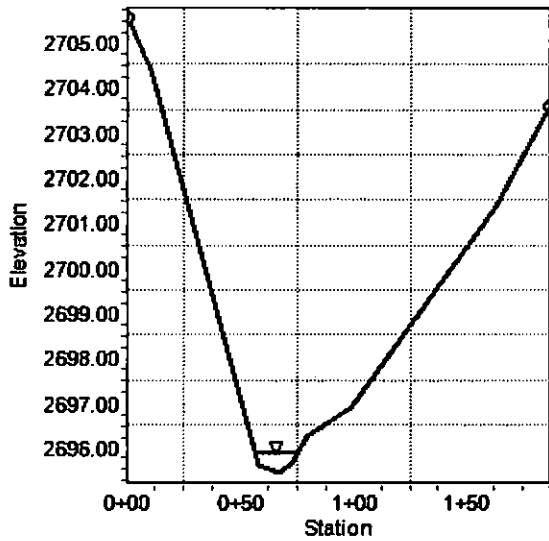
Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Channel Slope 0.02850 ft/ft
Normal Depth 0.40 ft
Discharge 15.00 ft³/s

Cross Section Image



Worksheet for Pre CP10 10-year Downstream XS

Project Description

Friction Method Manning Formula
 Solve For Normal Depth

Input Data

Channel Slope 0.02850 ft/ft
 Discharge 40.00 ft³/s

Section Definitions

Station (ft)	Elevation (ft)
--------------	----------------

0+00	2705.53
0+10	2704.39
0+58	2695.63
0+67	2695.47
0+73	2695.71
0+79	2696.28
0+99	2696.89
1+62	2701.34
1+86	2703.56

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 2705.53)	(1+86, 2703.56)	0.035

Options

Current Roughness weighted Method Pavlovskii's Method
 Open Channel Weighting Method Pavlovskii's Method
 Closed Channel Weighting Method Pavlovskii's Method

Results

Normal Depth 0.63 ft
 Elevation Range 2695.47 to 2705.53 ft
 Flow Area 9.72 ft²
 Wetted Perimeter 22.32 ft

Worksheet for Pre CP10 10-year Downstream XS

Results

Hydraulic Radius	0.44	ft
Top Width	22.25	ft
Normal Depth	0.63	ft
Critical Depth	0.66	ft
Critical Slope	0.02325	ft/ft
Velocity	4.12	ft/s
Velocity Head	0.26	ft
Specific Energy	0.89	ft
Froude Number	1.10	
Flow Type	Supercritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.63	ft
Critical Depth	0.66	ft
Channel Slope	0.02850	ft/ft
Critical Slope	0.02325	ft/ft

Cross Section for Pre CP10 10-year Downstream XS

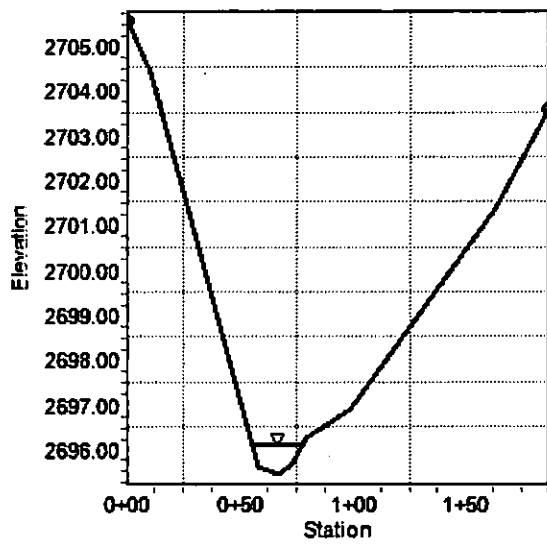
Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Channel Slope 0.02850 ft/ft
Normal Depth 0.63 ft
Discharge 40.00 ft³/s

Cross Section Image



Worksheet for Post CP10 10-year Downstream XS

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Channel Slope 0.02850 ft/ft
Discharge 41.00 ft³/s
Section Definitions

Station (ft)	Elevation (ft)
--------------	----------------

0+00	2705.53
0+10	2704.39
0+58	2695.63
0+67	2695.47
0+73	2695.71
0+79	2696.28
0+99	2696.89
1+62	2701.34
1+86	2703.56

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
---------------	----------------	-----------------------

(0+00, 2705.53)	(1+86, 2703.56)	0.035
-----------------	-----------------	-------

Options

Current Roughness weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	0.64 ft
Elevation Range	2695.47 to 2705.53 ft
Flow Area	9.88 ft ²
Wetted Perimeter	22.44 ft

Worksheet for Post CP10 10-year Downstream XS

Results

Hydraulic Radius	0.44	ft
Top Width	22.37	ft
Normal Depth	0.64	ft
Critical Depth	0.67	ft
Critical Slope	0.02316	ft/ft
Velocity	4.15	ft/s
Velocity Head	0.27	ft
Specific Energy	0.90	ft
Froude Number	1.10	
Flow Type	Supercritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.64	ft
Critical Depth	0.67	ft
Channel Slope	0.02850	ft/ft
Critical Slope	0.02316	ft/ft

Cross Section for Post CP10 10-year Downstream XS

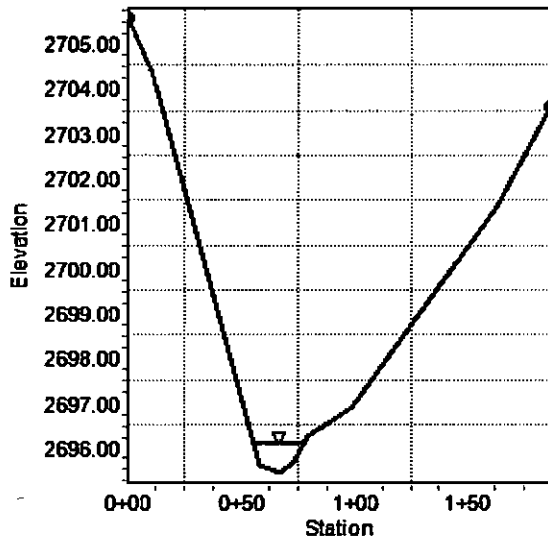
Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Channel Slope 0.02850 ft/ft
Normal Depth 0.64 ft
Discharge 41.00 ft³/s

Cross Section Image



Appendix D Hydraulic Analysis

- *HEC-RAS Results*

Existing Conditions Model

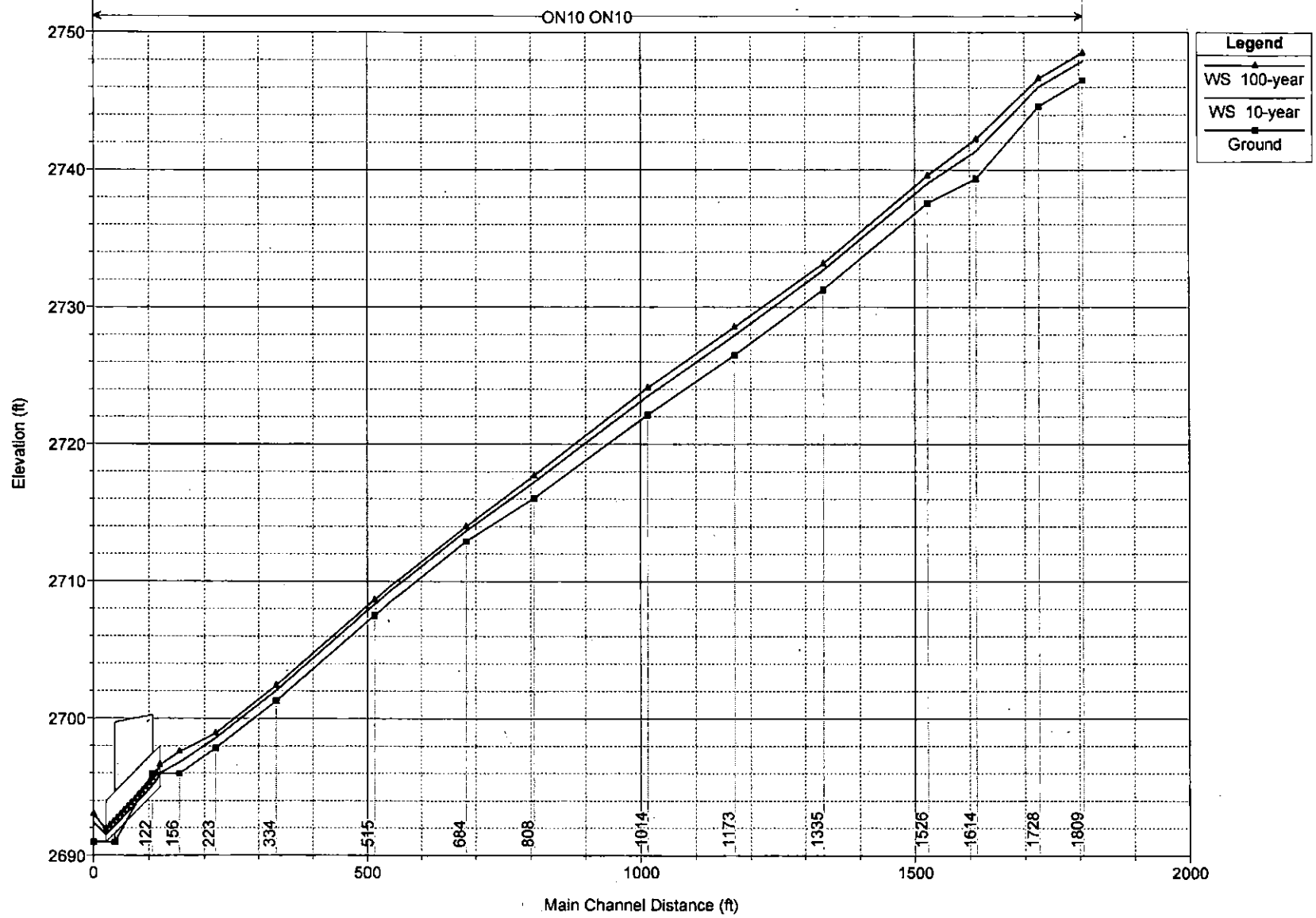
HEC-RAS Plan: EX

River	Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
ON20	ON20	986	10-year	56.00	2742.07	2742.71	2742.71	2742.89	0.021631	3.54	17.05	53.10	0.95
ON20	ON20	986	100-year	138.00	2742.07	2743.03	2743.03	2743.29	0.015771	4.39	39.40	81.17	0.90
ON20	ON20	844	10-year	56.00	2736.57	2737.64	2737.64	2738.01	0.020927	4.88	11.48	15.84	1.01
ON20	ON20	844	100-year	138.00	2736.57	2738.27	2738.27	2738.80	0.018428	5.83	23.68	22.66	1.00
ON20	ON20	713	10-year	56.00	2732.61	2733.77	2733.77	2734.12	0.020772	4.76	11.77	16.96	1.01
ON20	ON20	713	100-year	138.00	2732.61	2734.37	2734.37	2734.91	0.018032	5.90	23.41	22.83	1.01
ON20	ON20	685	10-year	56.00	2731.86	2733.09	2733.09	2733.41	0.017755	4.59	12.86	23.85	0.94
ON20	ON20	685	100-year	138.00	2731.86	2733.67	2733.67	2734.08	0.011832	5.52	32.40	44.48	0.85
ON20	ON20	639	10-year	56.00	2730.35	2731.92	2731.92	2732.31	0.019801	5.05	11.30	16.83	0.99
ON20	ON20	639	100-year	138.00	2730.35	2732.63	2732.63	2733.08	0.011498	5.72	31.07	39.99	0.83
ON20	ON20	596	10-year	56.00	2728.80	2730.21	2730.21	2730.52	0.023852	4.45	12.59	21.58	1.03
ON20	ON20	596	100-year	138.00	2728.80	2730.73	2730.73	2731.17	0.021359	5.33	25.90	30.97	1.03
ON20	ON20	554	10-year	56.00	2727.58	2728.73	2728.73	2729.11	0.021039	4.95	11.32	15.31	1.01
ON20	ON20	554	100-year	138.00	2727.58	2729.40	2729.40	2729.95	0.018133	5.98	23.09	20.95	1.00
ON20	ON20	469	10-year	56.00	2725.16	2726.47	2726.47	2726.91	0.018991	5.31	10.54	21.14	0.99
ON20	ON20	469	100-year	138.00	2725.16	2727.30	2727.30	2727.49	0.008841	3.54	40.30	53.53	0.67
ON20	ON20	385	10-year	56.00	2721.51	2723.11	2722.71	2723.24	0.005768	2.91	19.26	23.38	0.55
ON20	ON20	385	100-year	138.00	2721.51	2724.82	2723.30	2724.90	0.000861	2.20	63.65	39.11	0.25
ON20	ON20	371		Culvert									
ON20	ON20	163	10-year	56.00	2713.77	2714.61	2714.61	2714.97	0.022482	4.85	11.54	15.97	1.01
ON20	ON20	163	100-year	138.00	2713.77	2715.21	2715.21	2715.87	0.019603	6.50	21.23	16.00	0.99
ON20	ON20	11	10-year	56.00	2709.40	2710.61	2710.61	2710.86	0.016286	4.25	16.39	49.55	0.90
ON20	ON20	11	100-year	138.00	2709.40	2711.03	2711.03	2711.38	0.013986	5.40	36.09	69.75	0.90
ON10	ON10	1809	10-year	40.00	2746.51	2747.92	2747.92	2748.27	0.021896	4.73	8.46	12.58	1.02
ON10	ON10	1809	100-year	99.00	2746.51	2748.53	2748.53	2749.00	0.019424	5.49	18.02	19.57	1.01
ON10	ON10	1728	10-year	40.00	2744.63	2746.05	2746.05	2746.40	0.021584	4.76	8.40	12.23	1.01
ON10	ON10	1728	100-year	99.00	2744.63	2746.65	2746.65	2747.13	0.019120	5.56	17.82	18.89	1.01

HEC-RAS Plan: EX (Continued)

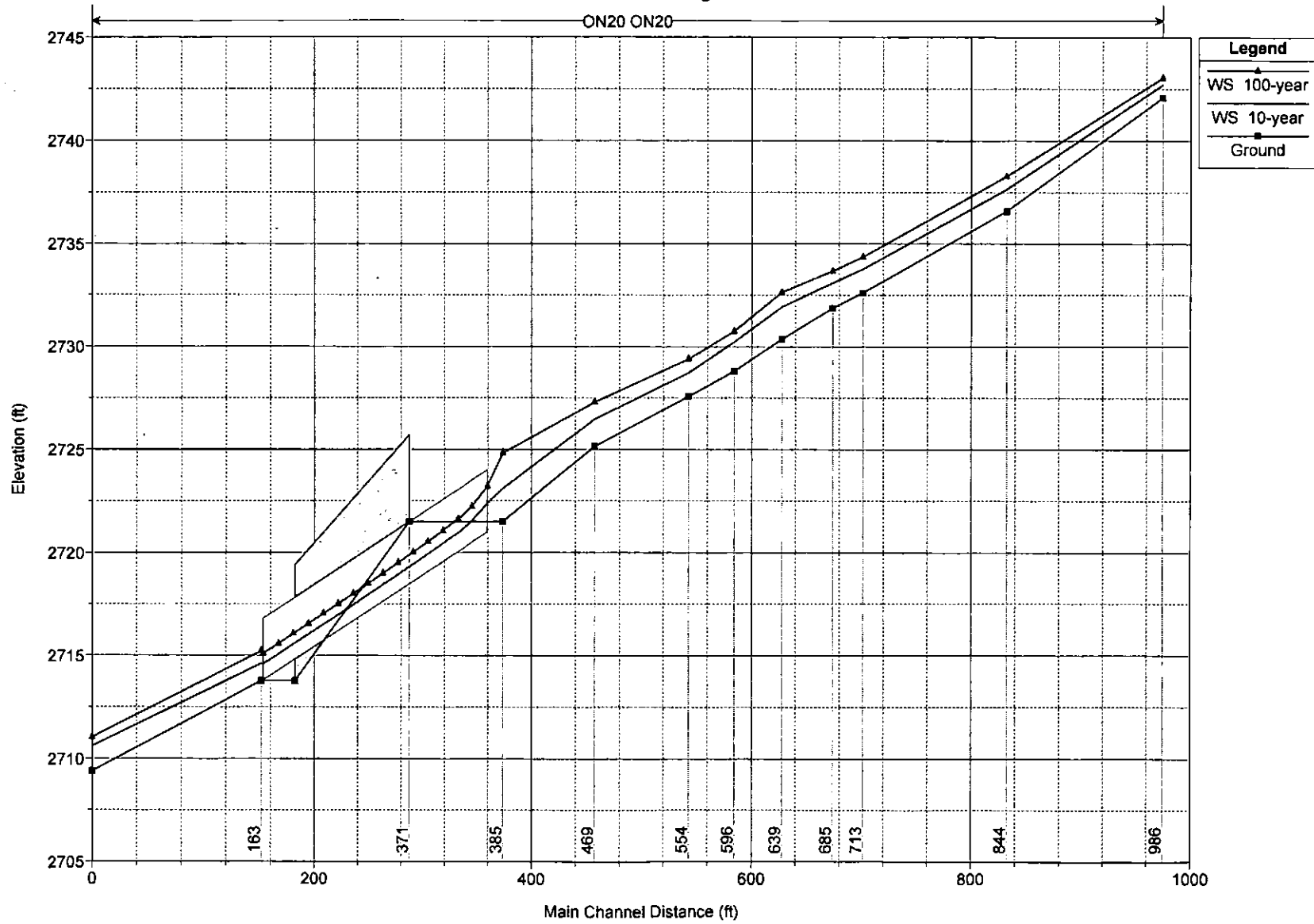
River	Reach	River Sta	Profile	Q Total (cfs)	Min Chl El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
ON10	ON10	1614	10-year	40.00	2739.36	2741.37	2741.37	2741.88	0.022485	5.76	6.94	6.93	1.01
ON10	ON10	1614	100-year	99.00	2739.36	2742.24	2742.24	2742.97	0.019884	6.84	14.47	10.25	1.01
ON10	ON10	1526	10-year	40.00	2737.56	2739.01	2739.01	2738.32	0.022877	4.50	8.89	14.56	1.02
ON10	ON10	1526	100-year	99.00	2737.56	2739.57	2739.57	2739.98	0.019287	5.17	19.21	23.58	0.99
ON10	ON10	1335	10-year	40.00	2731.26	2732.68	2732.68	2732.97	0.023386	4.29	9.32	16.68	1.01
ON10	ON10	1335	100-year	99.00	2731.26	2733.15	2733.15	2733.63	0.018810	5.57	18.09	21.23	0.99
ON10	ON10	1173	10-year	40.00	2726.51	2727.95	2727.95	2728.29	0.021654	4.67	8.56	14.26	1.00
ON10	ON10	1173	100-year	99.00	2726.51	2728.55	2728.55	2729.04	0.018670	5.65	17.53	24.39	0.99
ON10	ON10	1014	10-year	40.00	2722.13	2723.50	2723.50	2723.81	0.020471	4.65	9.43	16.27	0.99
ON10	ON10	1014	100-year	99.00	2722.13	2724.11	2724.11	2724.44	0.010863	5.15	28.07	46.31	0.80
ON10	ON10	808	10-year	40.00	2716.06	2717.23	2717.23	2717.50	0.022602	4.17	9.71	20.49	1.01
ON10	ON10	808	100-year	99.00	2716.06	2717.70	2717.70	2718.09	0.014895	5.15	21.91	31.59	0.91
ON10	ON10	684	10-year	40.00	2712.89	2713.65	2713.65	2713.65	0.026110	3.55	11.31	30.97	1.02
ON10	ON10	684	100-year	99.00	2712.89	2713.97	2713.97	2714.29	0.021003	4.56	22.21	37.02	1.00
ON10	ON10	515	10-year	40.00	2707.50	2708.30	2708.30	2708.53	0.023235	3.86	10.55	26.84	1.00
ON10	ON10	515	100-year	99.00	2707.50	2708.65	2708.65	2708.99	0.019944	4.72	21.64	40.44	1.03
ON10	ON10	334	10-year	40.00	2701.29	2702.04	2702.04	2702.26	0.025324	3.73	10.73	25.80	1.02
ON10	ON10	334	100-year	99.00	2701.29	2702.40	2702.40	2702.77	0.021147	4.87	20.33	28.48	1.02
ON10	ON10	223	10-year	40.00	2697.84	2698.59	2698.59	2698.79	0.026412	3.56	11.23	30.09	1.03
ON10	ON10	223	100-year	99.00	2697.84	2698.92	2698.92	2699.24	0.021622	4.56	21.69	34.45	1.01
ON10	ON10	156	10-year	40.00	2695.99	2696.79	2696.79	2696.99	0.018093	3.69	12.04	39.12	0.80
ON10	ON10	156	100-year	99.00	2695.99	2697.57	2697.04	2697.63	0.002250	2.21	54.75	62.33	0.36
ON10	ON10	122		Culvert									
ON10	ON10	2	10-year	40.00	2691.00	2692.40	2692.18	2692.56	0.008015	3.43	12.84	88.31	0.65
ON10	ON10	2	100-year	99.00	2691.00	2693.05	2692.71	2693.32	0.008005	4.34	24.36	99.49	0.68

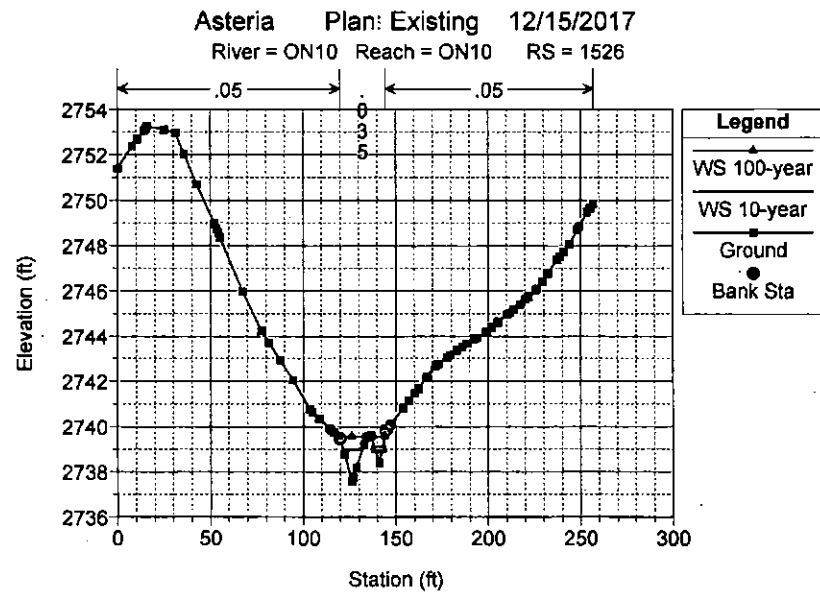
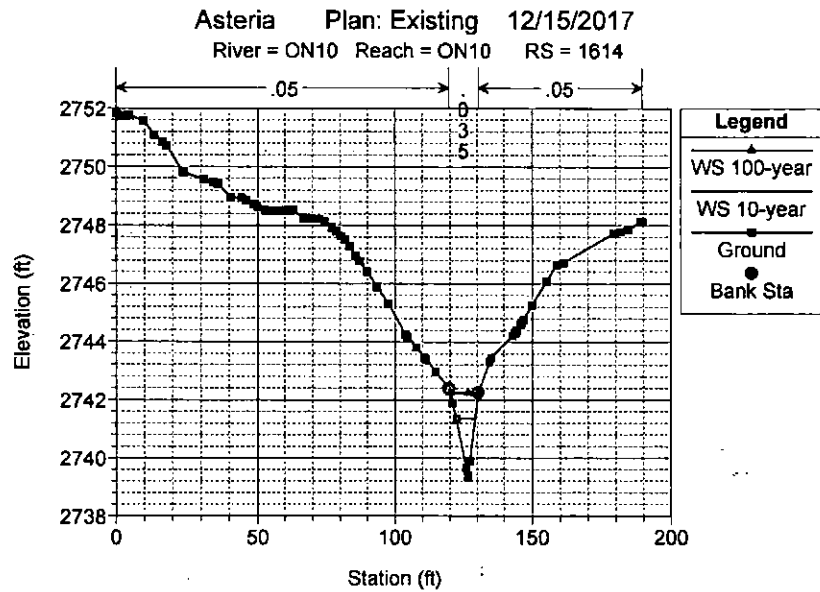
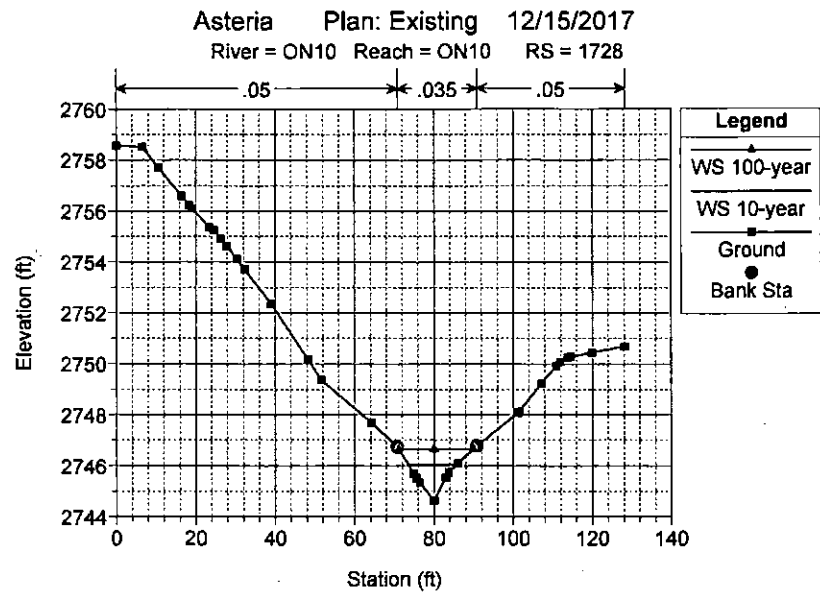
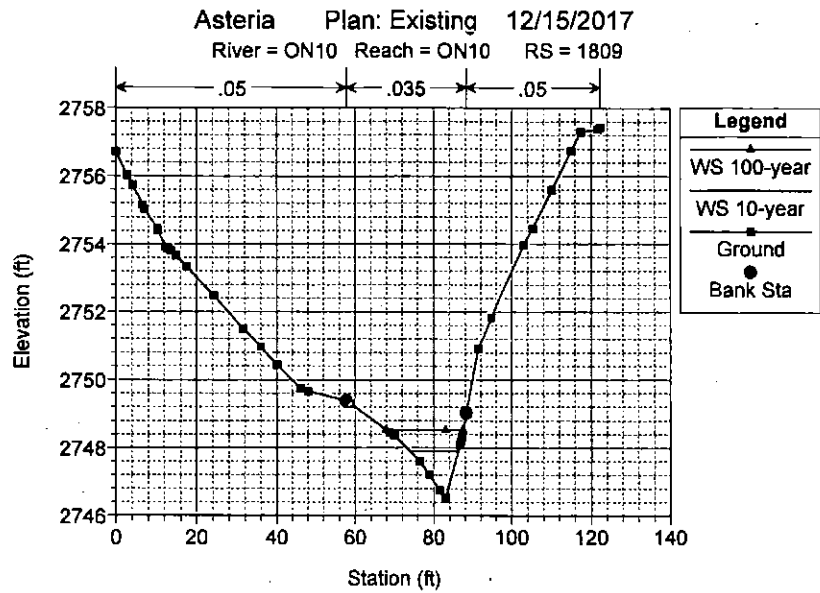
Asteria Plan: Existing 12/14/2017

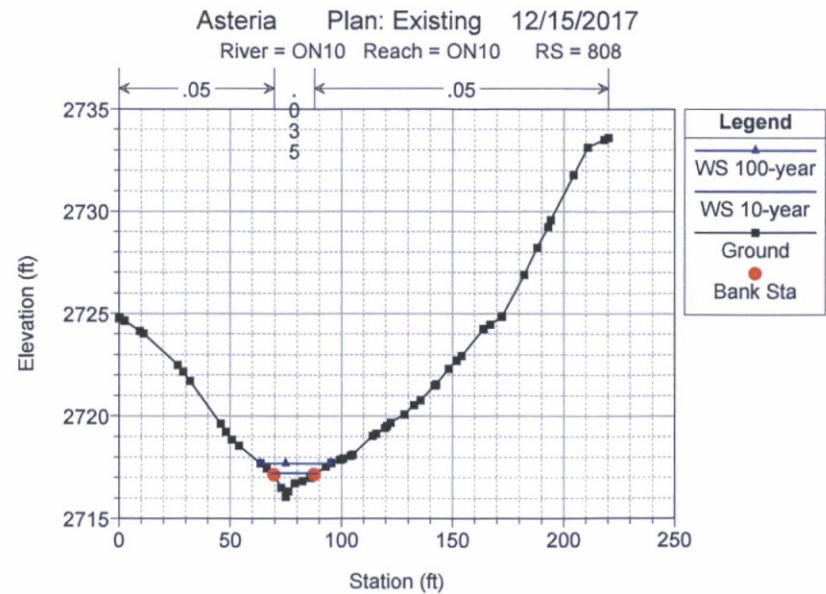
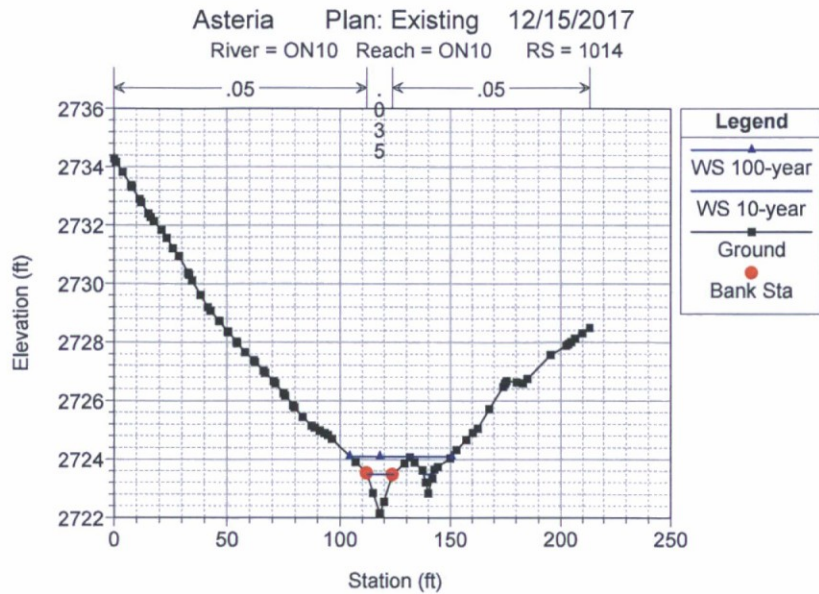
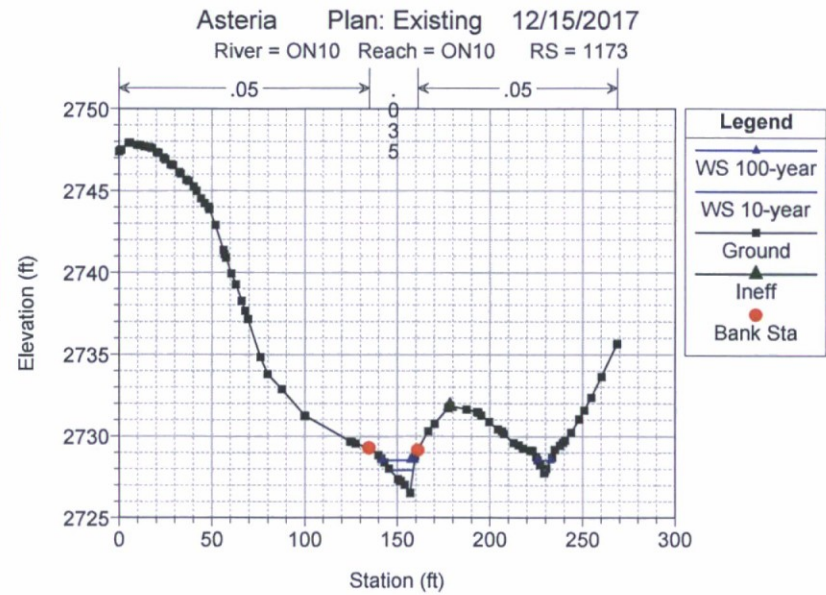
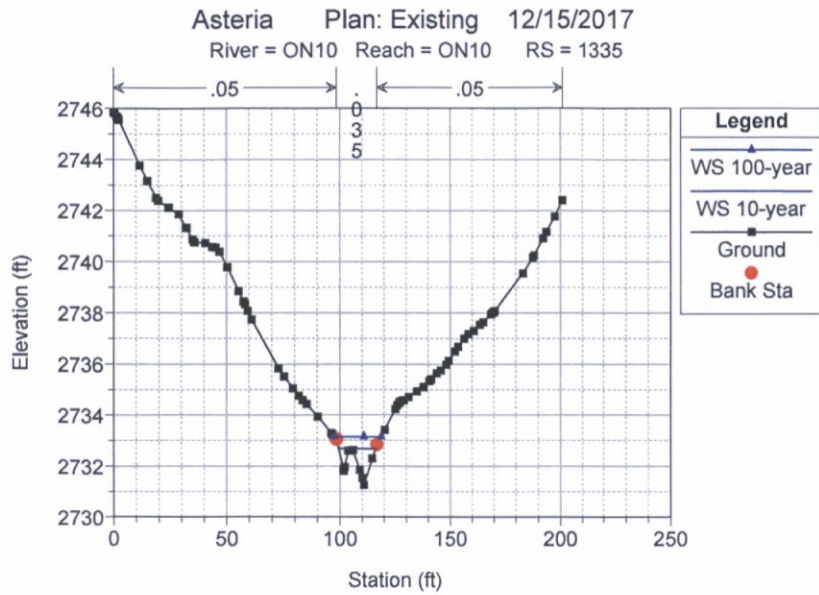


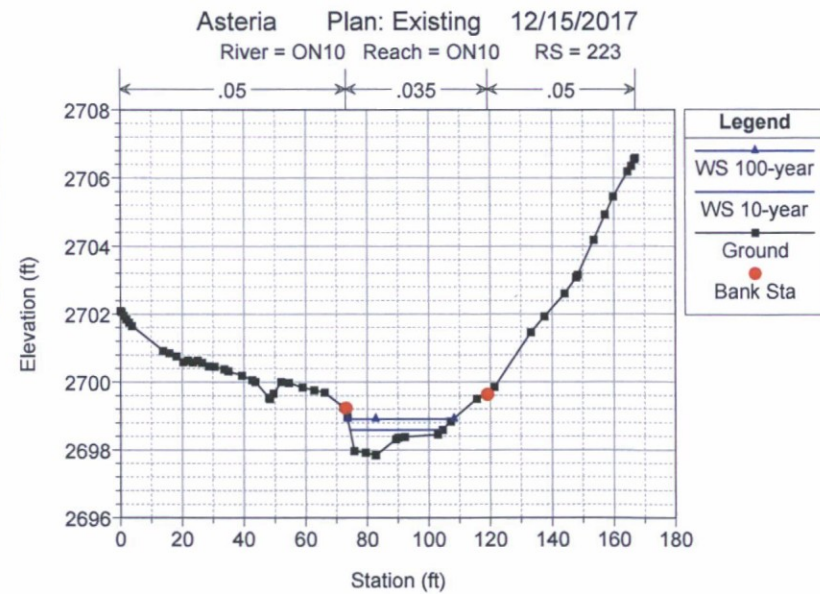
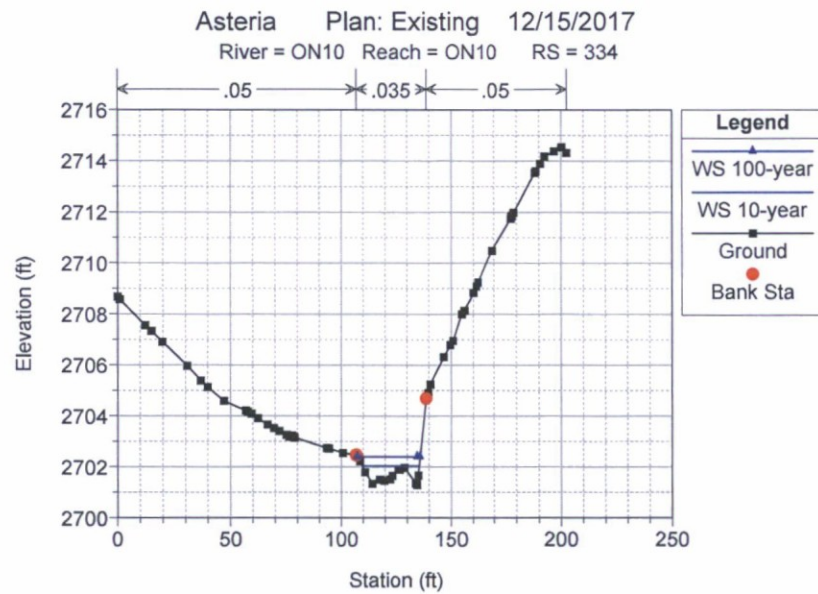
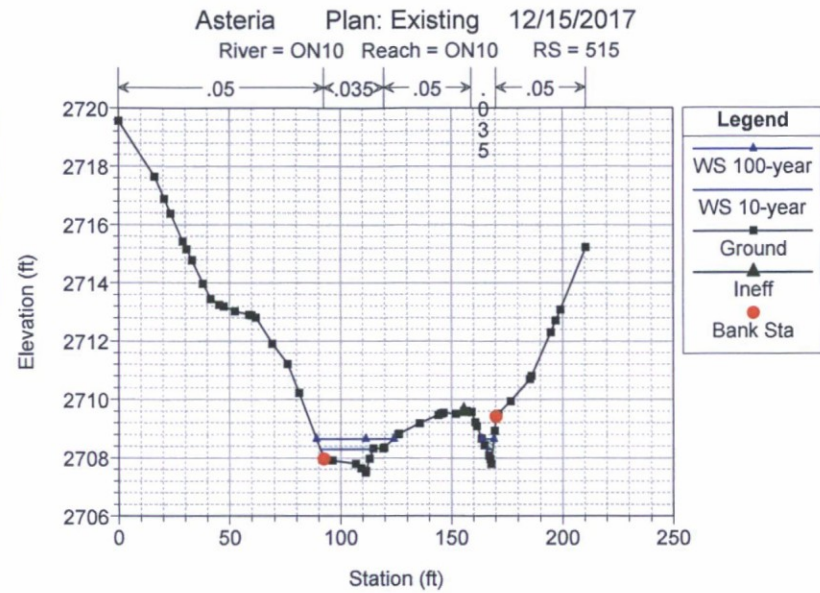
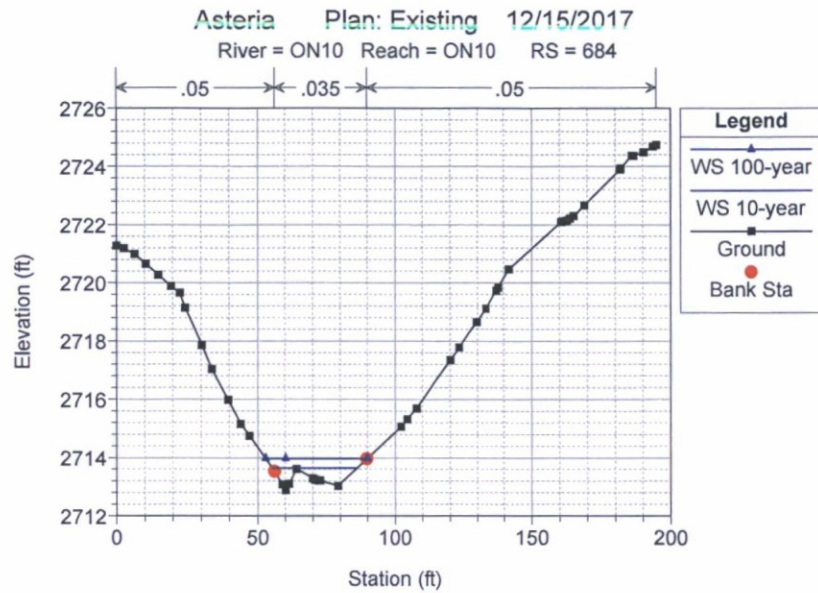
Asteria Plan: Existing 12/14/2017

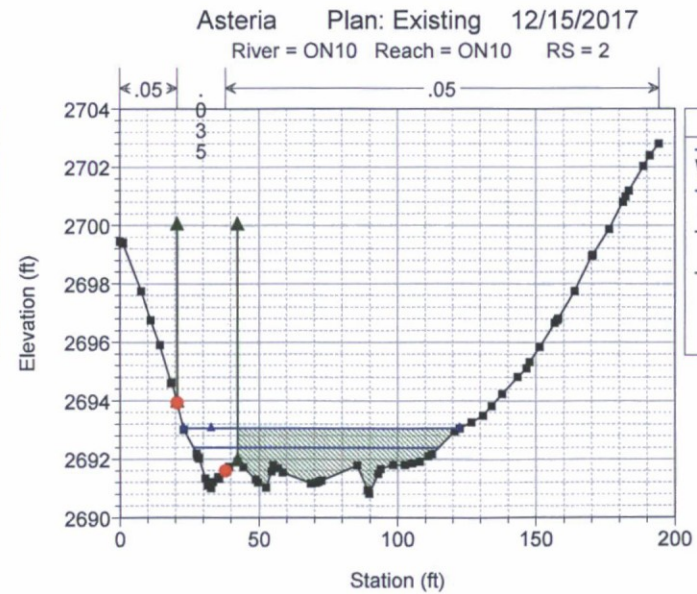
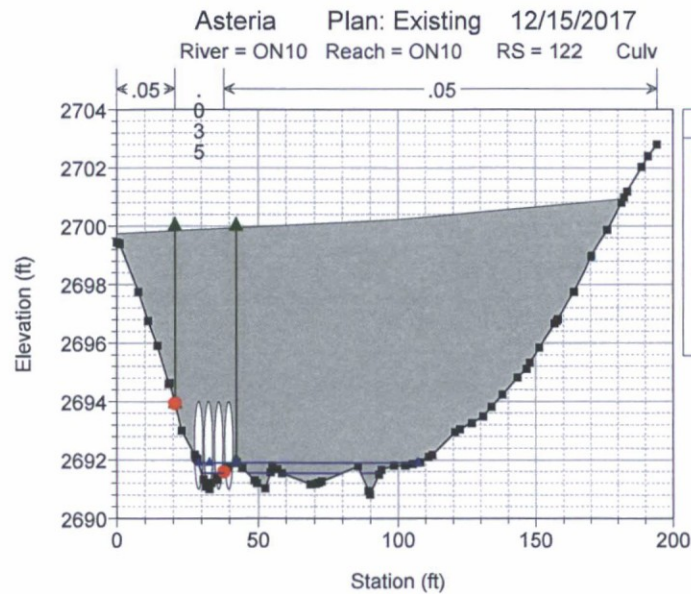
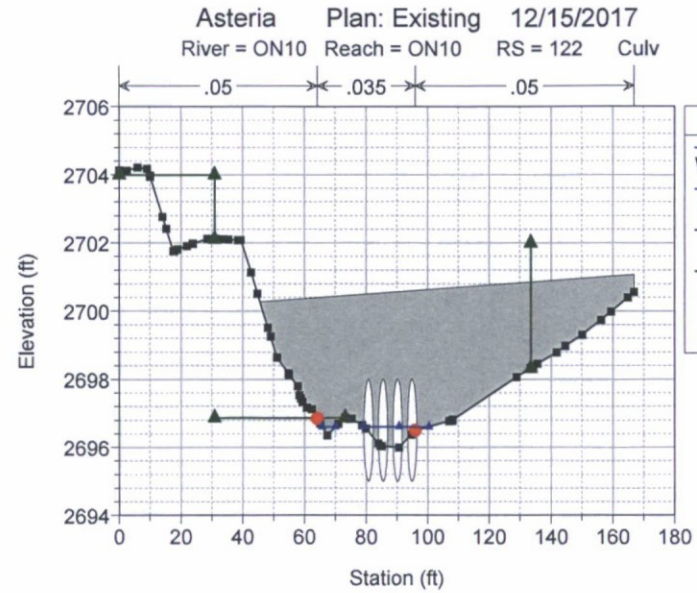
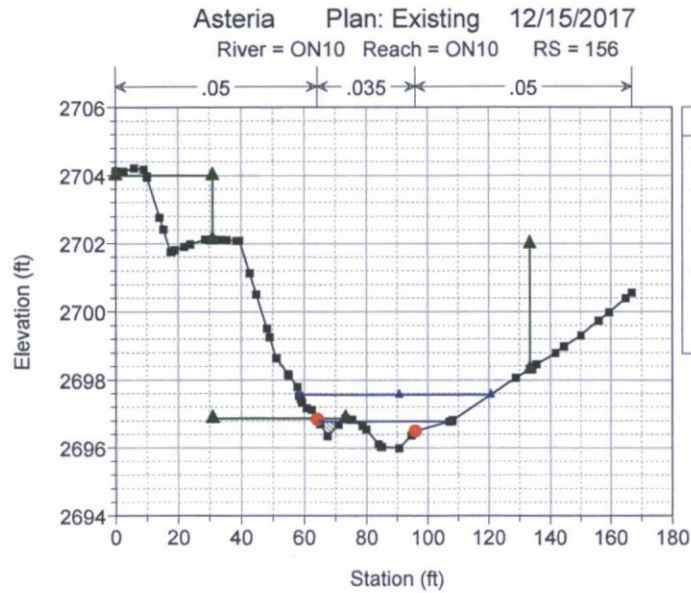
ON20 ON20



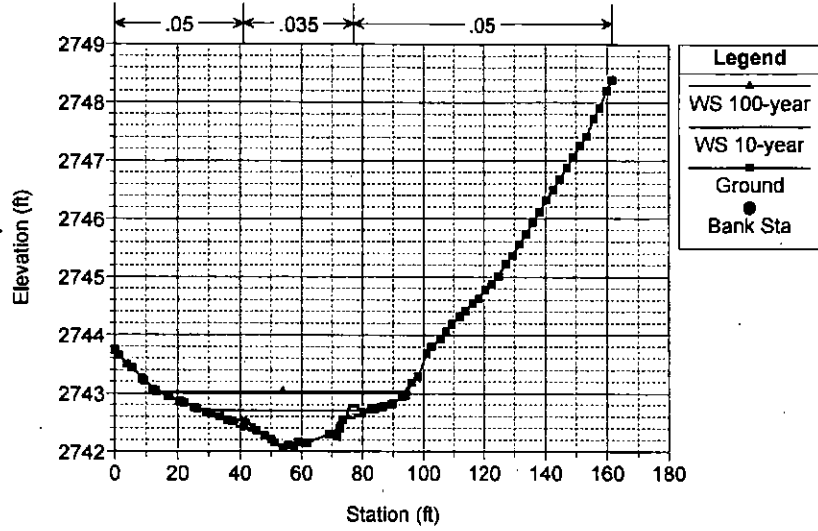




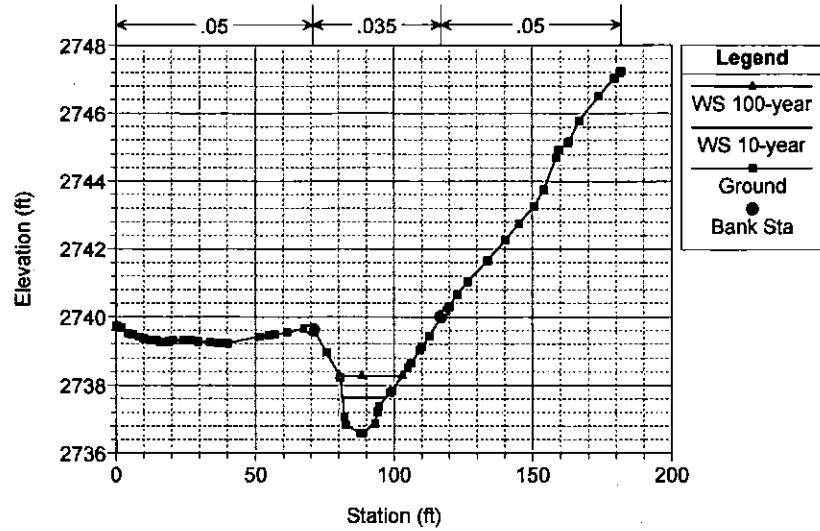




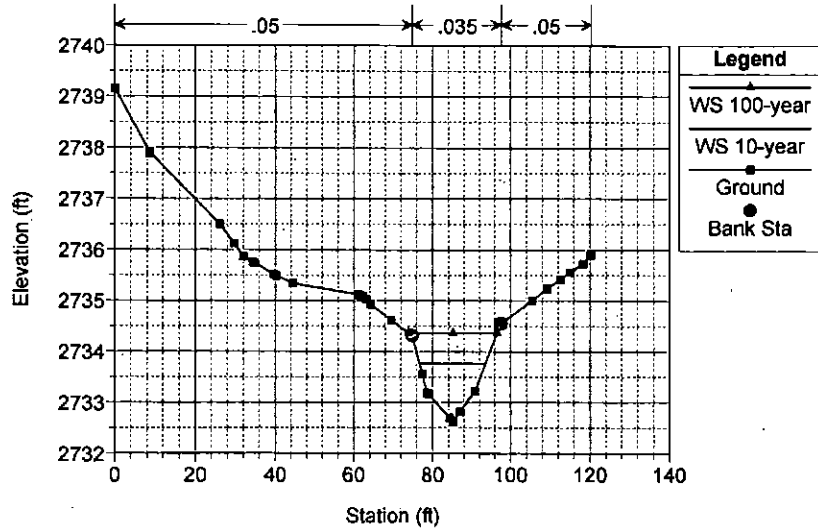
Asteria Plan: Existing 12/15/2017
 River = ON20 Reach = ON20 RS = 986



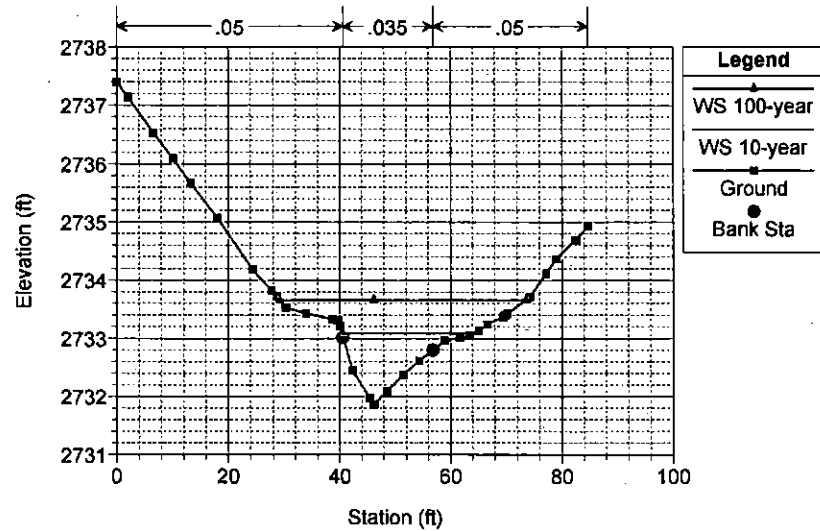
Asteria Plan: Existing 12/15/2017
 River = ON20 Reach = ON20 RS = 844

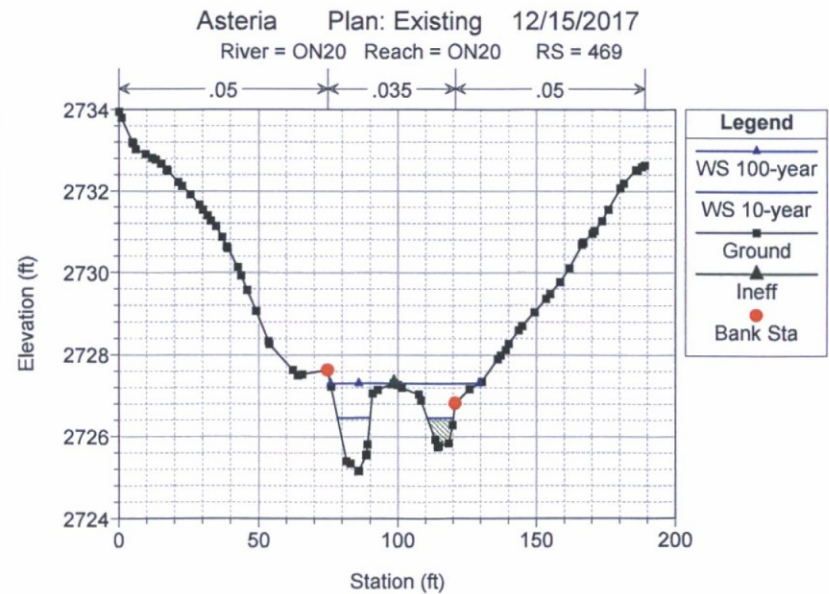
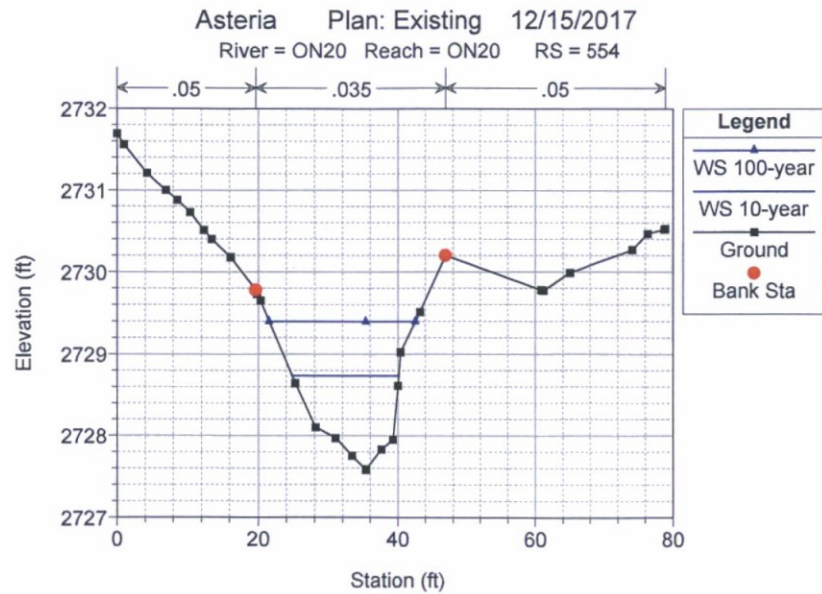
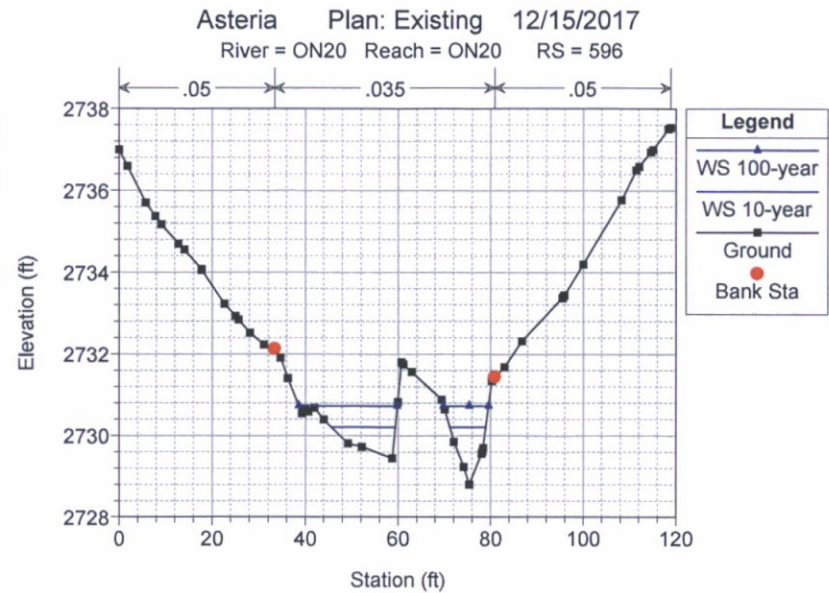
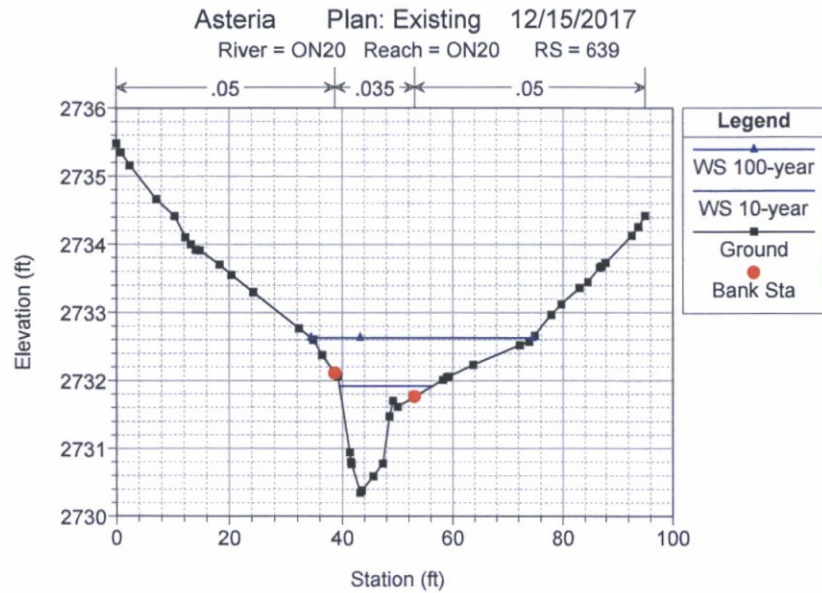


Asteria Plan: Existing 12/15/2017
 River = ON20 Reach = ON20 RS = 713

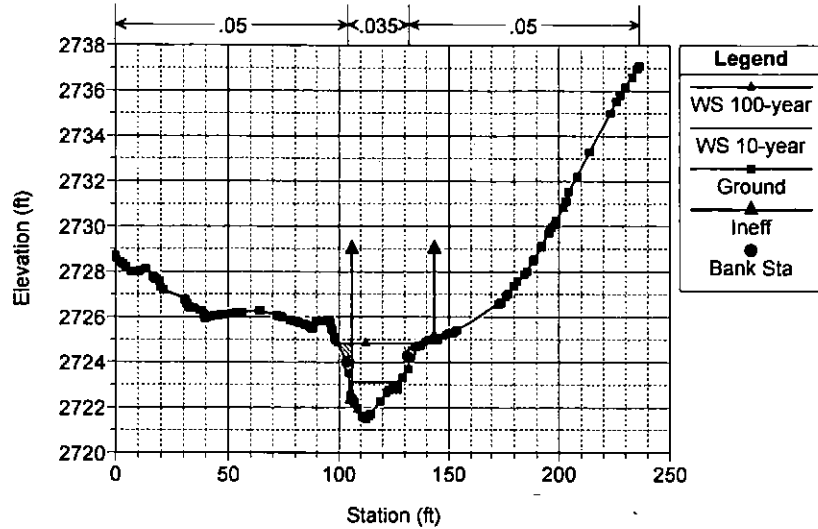


Asteria Plan: Existing 12/15/2017
 River = ON20 Reach = ON20 RS = 685

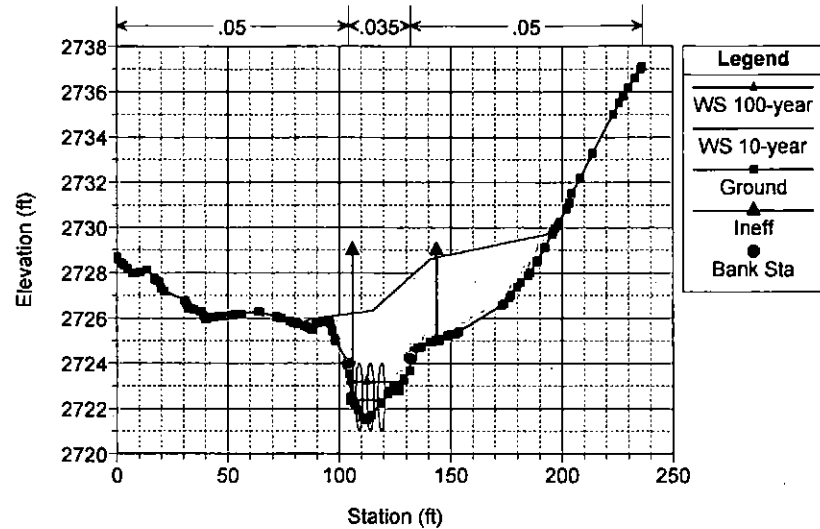




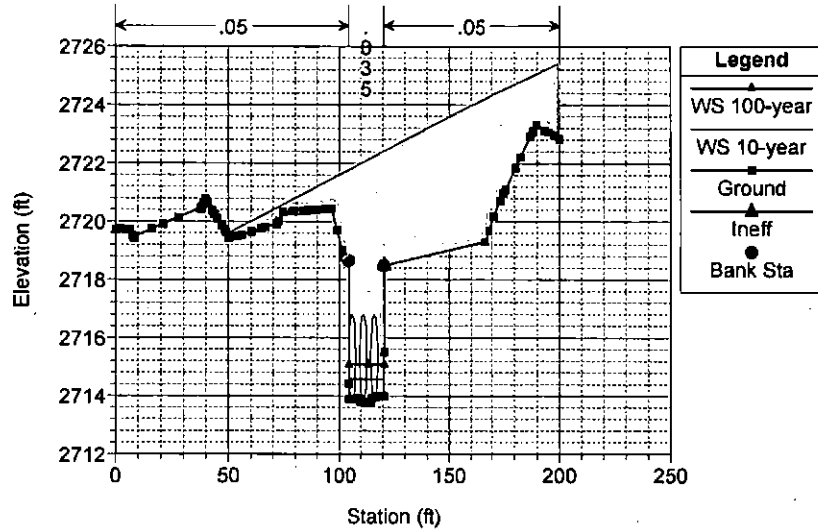
Asteria Plan: Existing 12/15/2017
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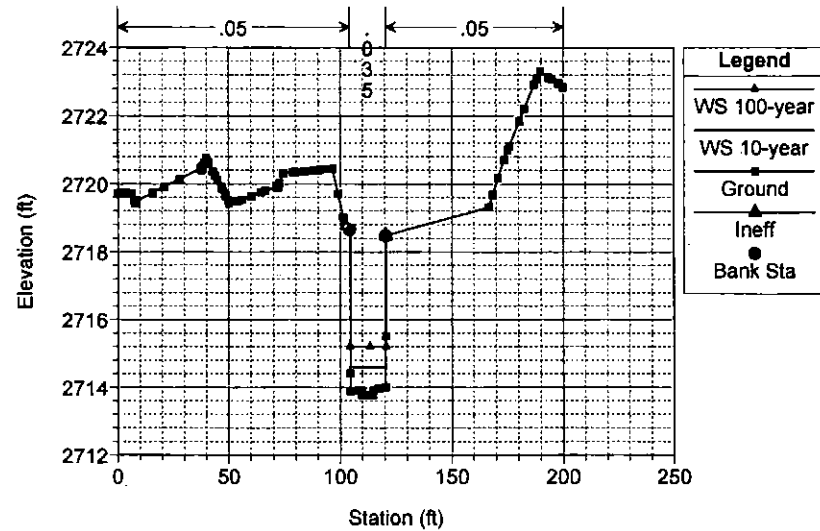
Asteria Plan: Existing 12/15/2017
 River = ON20 Reach = ON20 RS = 371 Culv



Asteria Plan: Existing 12/15/2017
 River = ON20 Reach = ON20 RS = 371 Culv

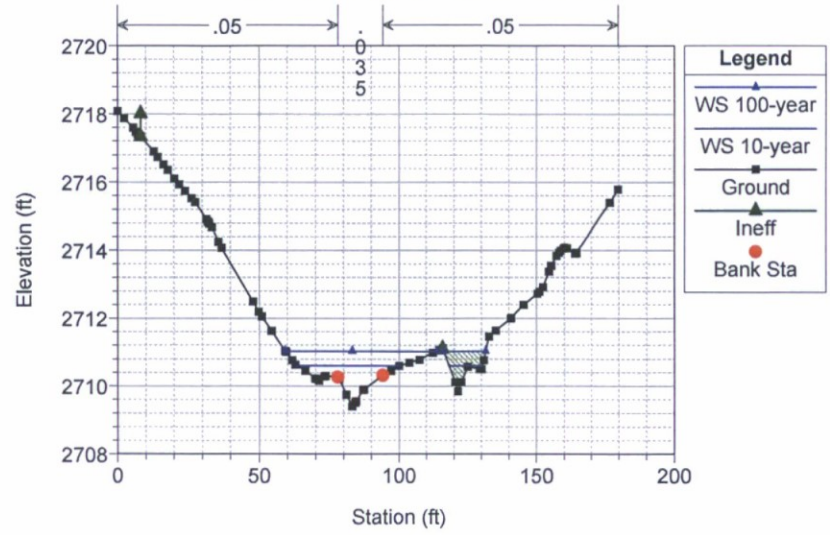


Asteria Plan: Existing 12/15/2017
 River = ON20 Reach = ON20 RS = 163



Asteria Plan: Existing 12/15/2017

River = ON20 Reach = ON20 RS = 11



Proposed Conditions Model

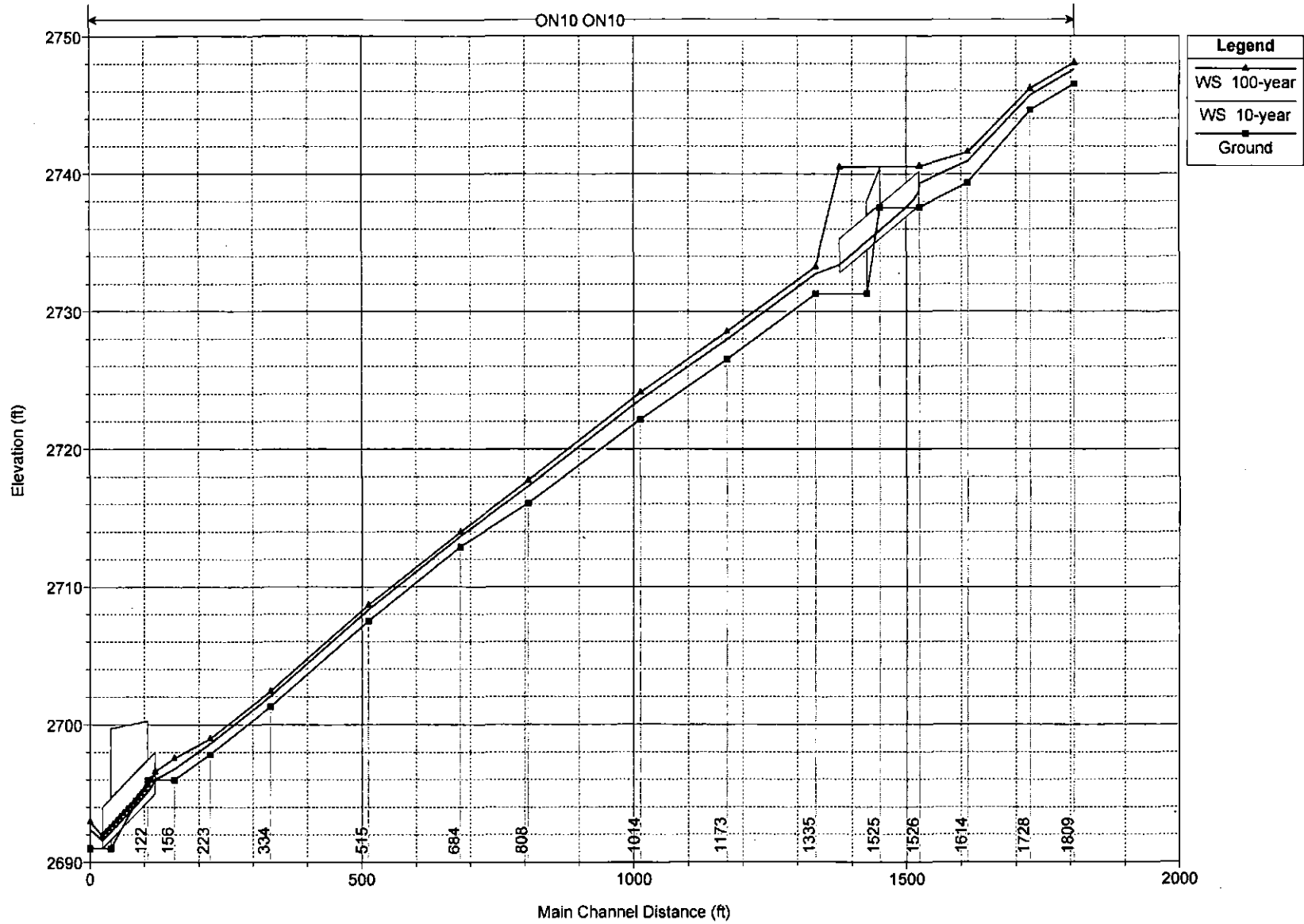
HEC-RAS Plan: PR

River	Reach	River Sta	Profile	Q Total (cfs)	Min Ch Fl (ft)	W/S Elev (ft)	Crit W.S. (ft)	E.C. Elev (ft)	E.C. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
ON20	ON20	986	10-year	53.00	2742.07	2742.69	2742.69	2742.92	0.022820	4.12	16.28	50.78	1.01
ON20	ON20	986	100-year	130.00	2742.07	2743.05	2743.05	2743.30	0.014619	4.71	41.48	82.22	0.89
ON20	ON20	844	10-year	53.00	2736.57	2737.62	2737.62	2737.97	0.020312	4.75	11.15	15.65	0.99
ON20	ON20	844	100-year	130.00	2736.57	2738.23	2738.23	2738.79	0.015515	6.05	22.69	21.99	0.95
ON20	ON20	713	10-year	53.00	2732.61	2733.76	2733.76	2734.14	0.017747	5.08	11.52	16.84	0.97
ON20	ON20	713	100-year	130.00	2732.61	2734.33	2734.33	2735.00	0.016604	6.93	22.64	22.23	1.02
ON20	ON20	685	10-year	53.00	2731.86	2733.06	2733.06	2733.38	0.018063	4.52	12.25	23.25	0.94
ON20	ON20	685	100-year	130.00	2731.86	2733.63	2733.63	2734.03	0.011685	5.39	31.15	45.88	0.84
ON20	ON20	639	10-year	55.00	2730.35	2731.92	2731.92	2732.30	0.019251	4.97	11.27	16.79	0.97
ON20	ON20	639	100-year	134.00	2730.35	2732.61	2732.61	2733.05	0.011386	5.65	30.40	39.55	0.83
ON20	ON20	596	10-year	55.00	2728.80	2731.70	2730.23	2731.71	0.000255	0.98	62.27	46.79	0.13
ON20	ON20	596	100-year	134.00	2728.80	2732.29	2730.74	2732.32	0.000542	1.71	92.45	56.33	0.19
ON20	ON20	591		Bridge									
ON20	ON20	554	10-year	55.00	2727.58	2728.75	2728.75	2729.18	0.019473	5.38	10.38	15.45	1.00
ON20	ON20	554	100-year	134.00	2727.58	2729.50	2729.50	2730.18	0.014485	6.66	20.18	24.26	1.01
ON20	ON20	469	10-year	55.00	2725.16	2726.46	2726.46	2726.89	0.019069	5.29	10.39	20.99	0.99
ON20	ON20	469	100-year	134.00	2725.16	2727.22	2727.22	2727.88	0.016174	6.50	20.95	47.26	0.97
ON20	ON20	385	10-year	55.00	2721.51	2723.07	2722.70	2723.21	0.005444	3.03	18.52	23.23	0.55
ON20	ON20	385	100-year	134.00	2721.51	2724.74	2723.29	2724.82	0.000862	2.36	61.00	36.06	0.26
ON20	ON20	371		Culvert									
ON20	ON20	163	10-year	55.00	2713.77	2714.61	2714.61	2714.96	0.021103	4.87	11.58	15.97	1.00
ON20	ON20	163	100-year	134.00	2713.77	2715.18	2715.18	2715.84	0.019293	6.65	20.77	16.00	1.02
ON20	ON20	11	10-year	55.00	2709.40	2710.66	2710.66	2710.91	0.013519	4.62	18.17	52.15	0.85
ON20	ON20	11	100-year	134.00	2709.40	2711.08	2711.08	2711.41	0.012882	5.83	38.92	72.06	0.89
ON10	ON10	1809	10-year	21.00	2746.51	2747.59	2747.59	2747.88	0.024235	4.31	4.87	9.09	1.03
ON10	ON10	1809	100-year	51.00	2746.51	2748.07	2748.07	2748.45	0.019810	5.00	10.44	14.14	0.99

HEC-RAS Plan: PR (Continued)

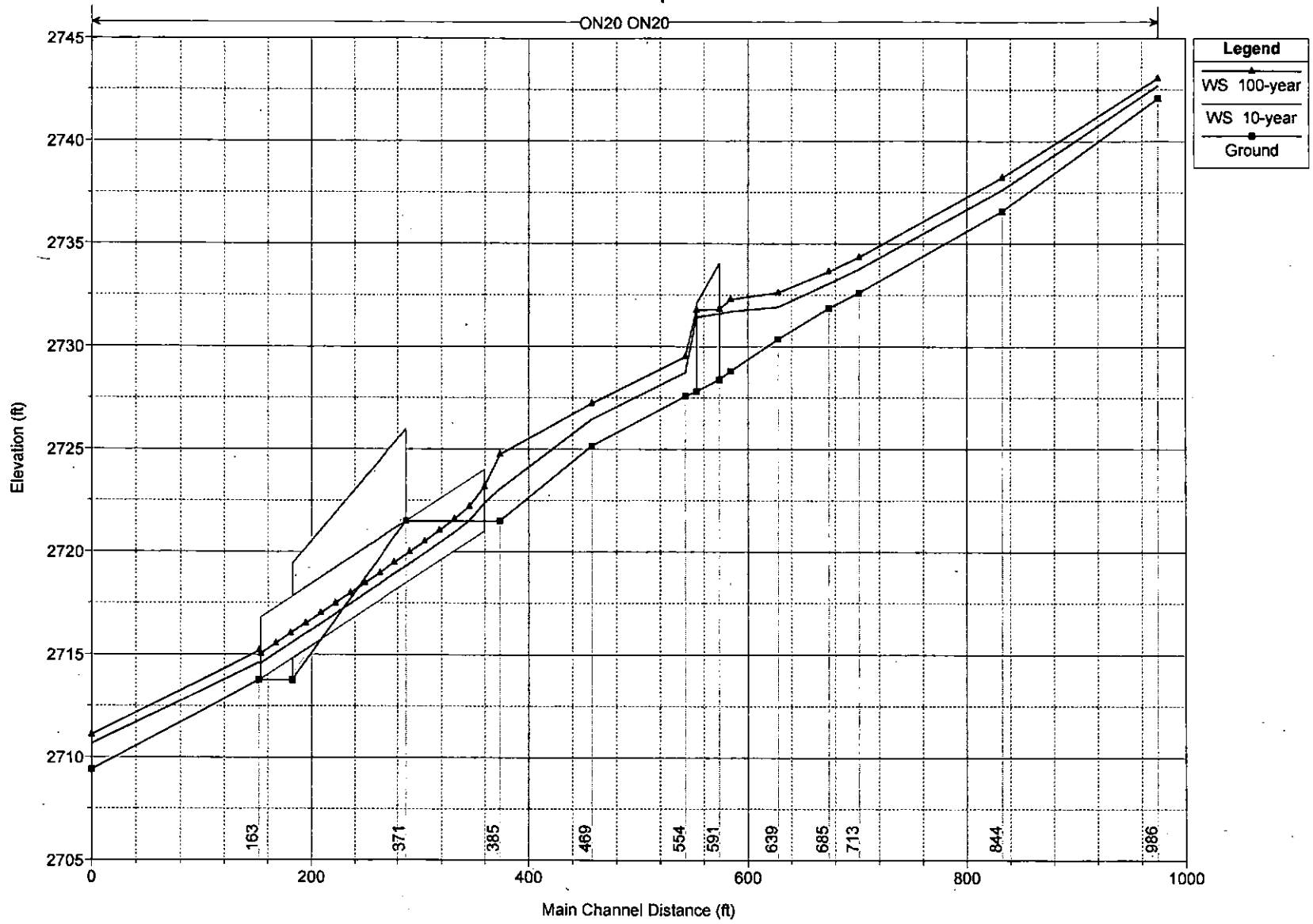
River	Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
ON10	ON10	2	10-year	41.00	2691.00	2692.36	2692.14	2692.54	0.008003	3.40	12.31	87.77	0.65
ON10	ON10	2	100-year	98.00	2691.00	2692.93	2692.67	2693.27	0.007999	4.76	22.12	96.96	0.70

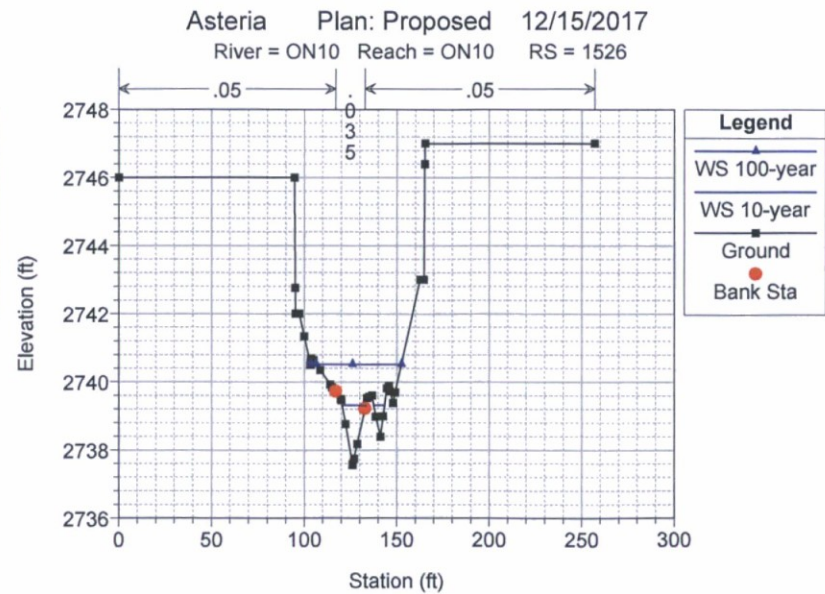
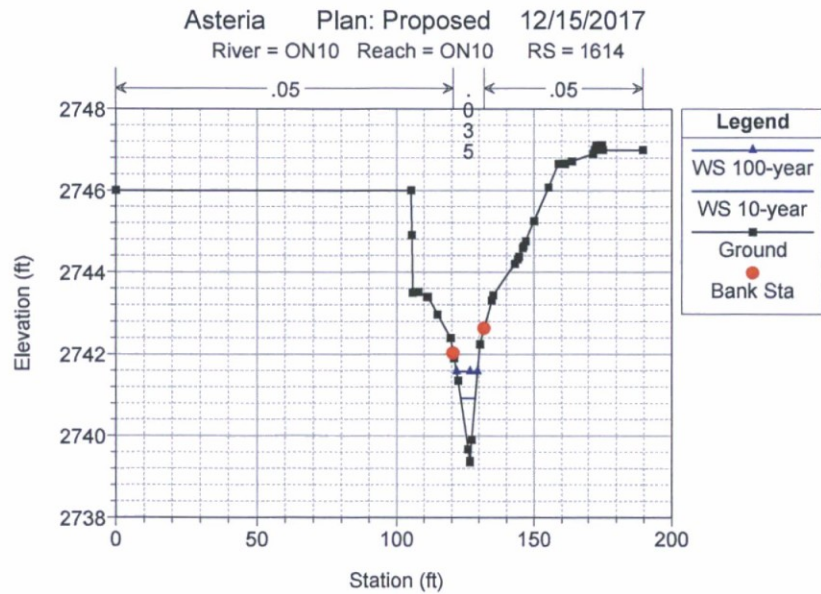
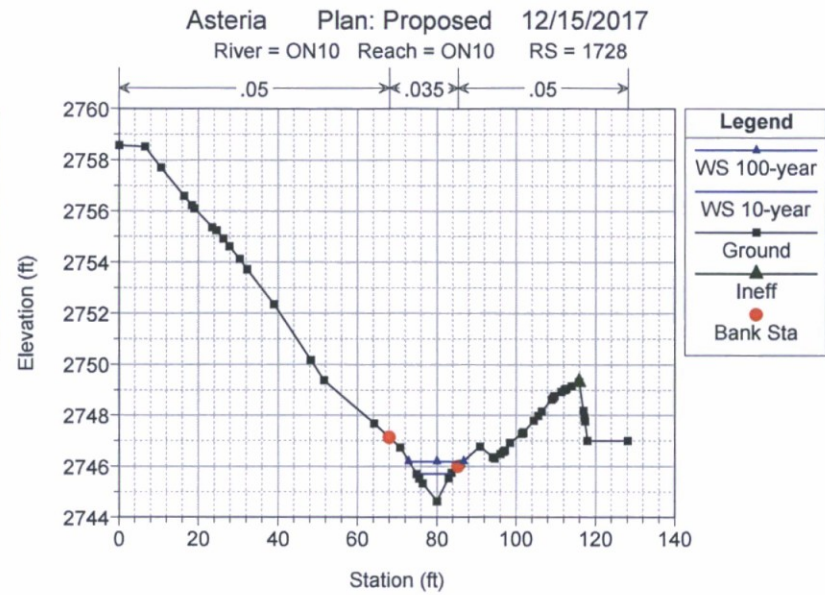
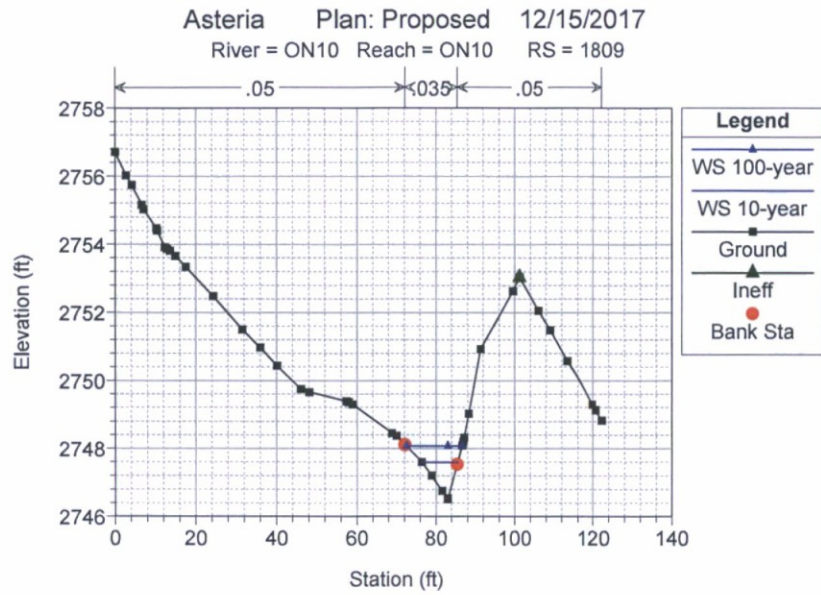
Asteria Plan: Proposed 12/15/2017

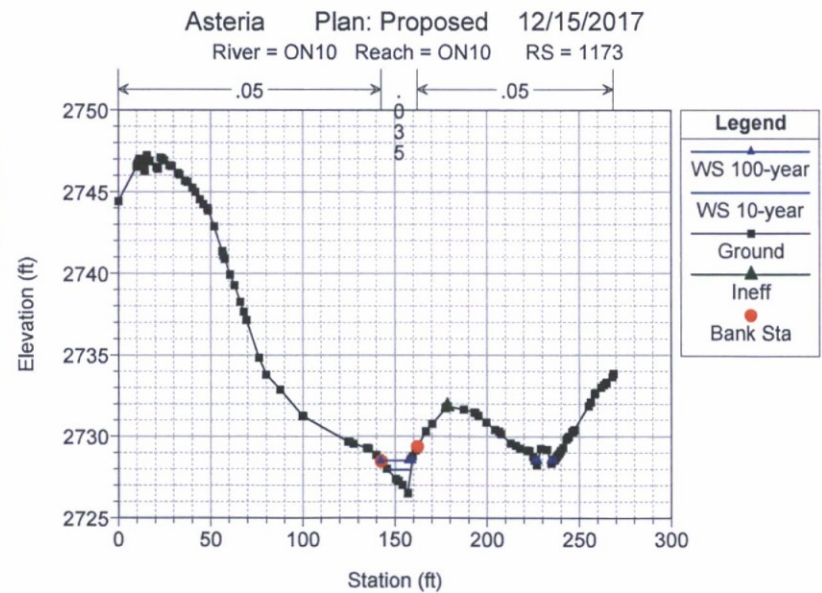
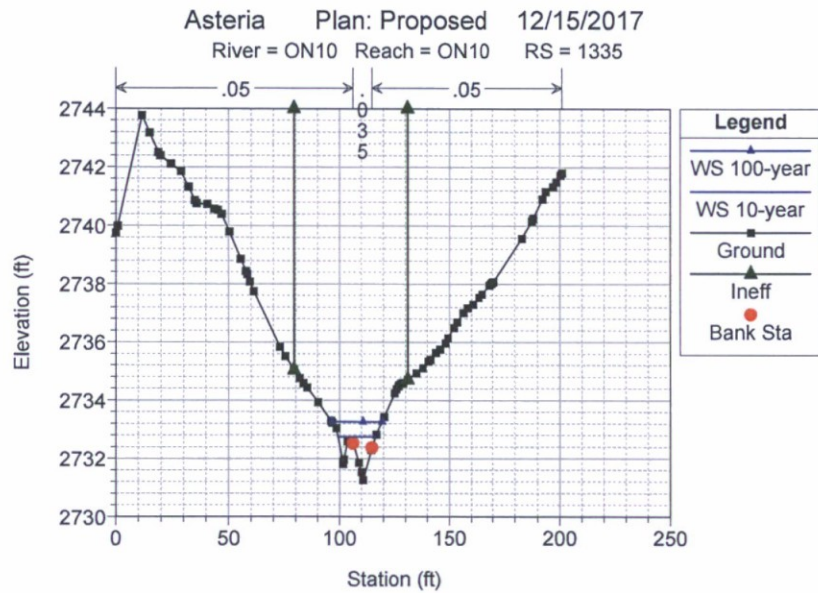
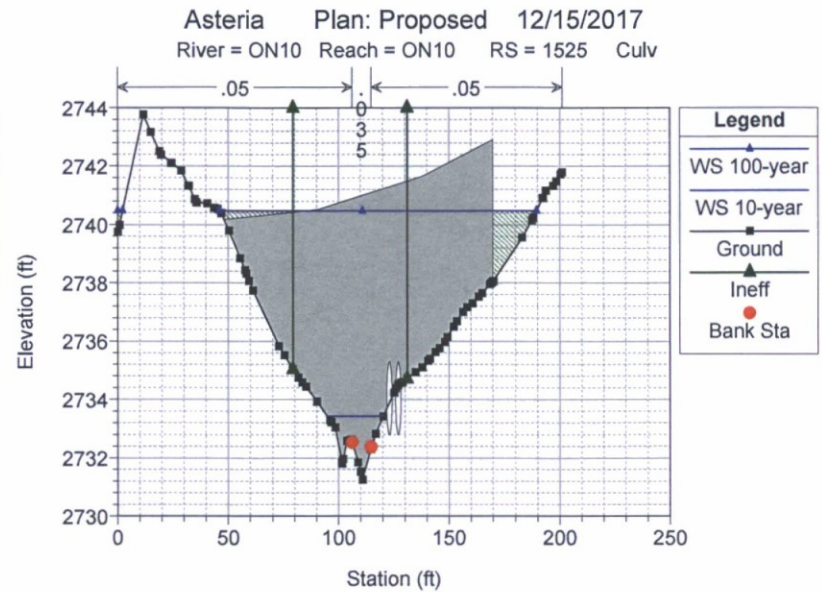
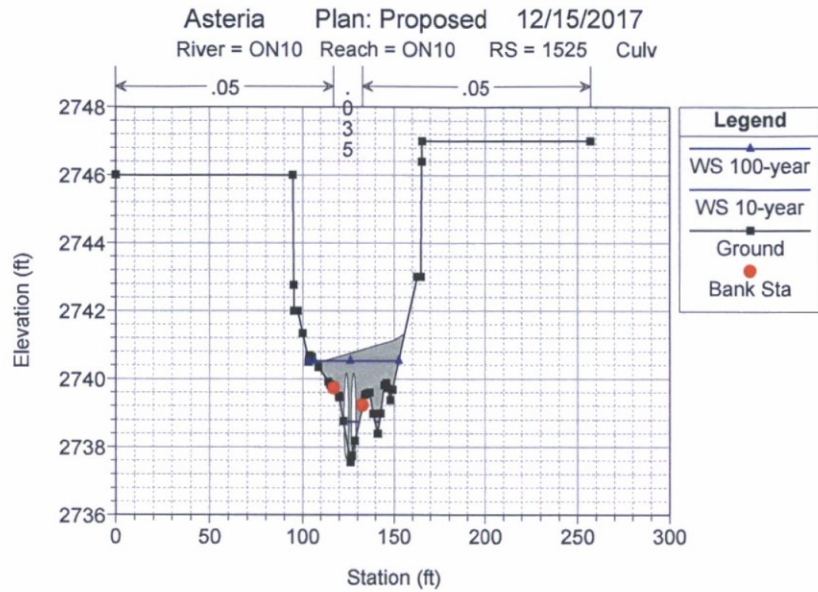


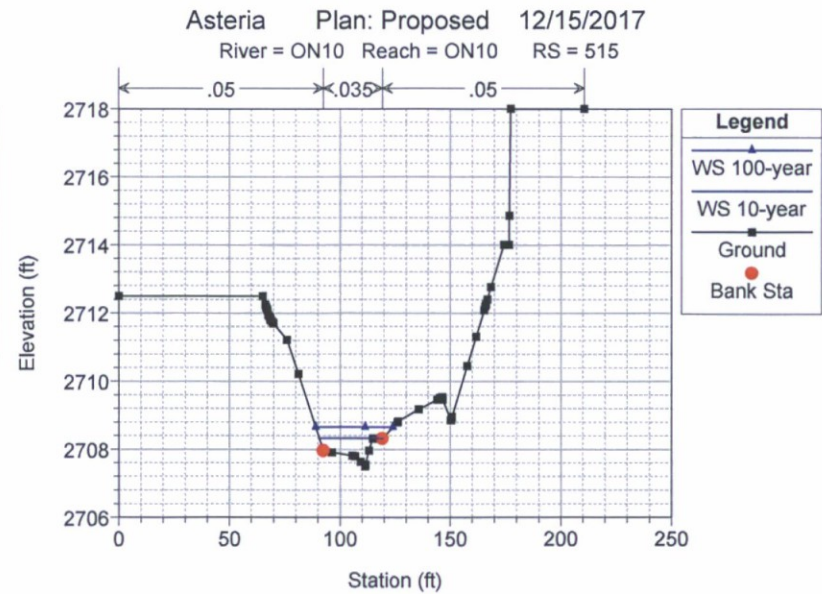
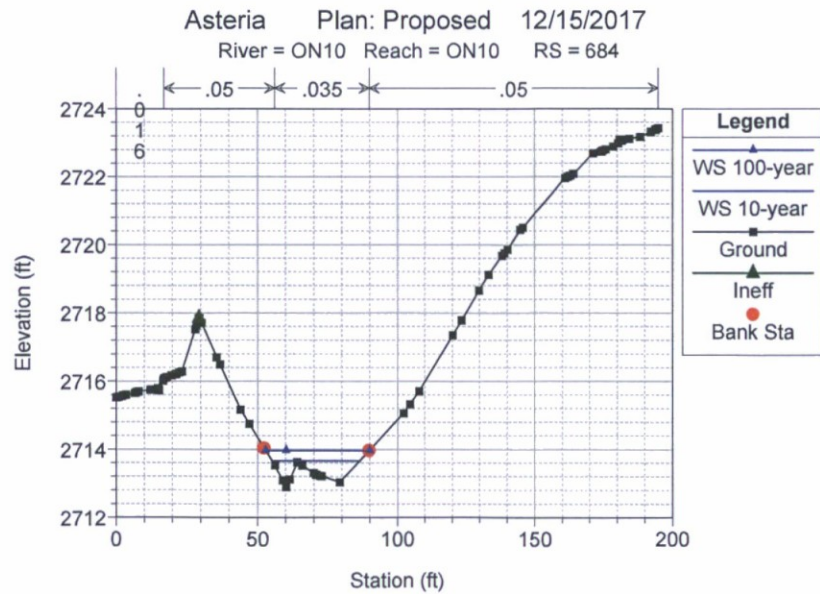
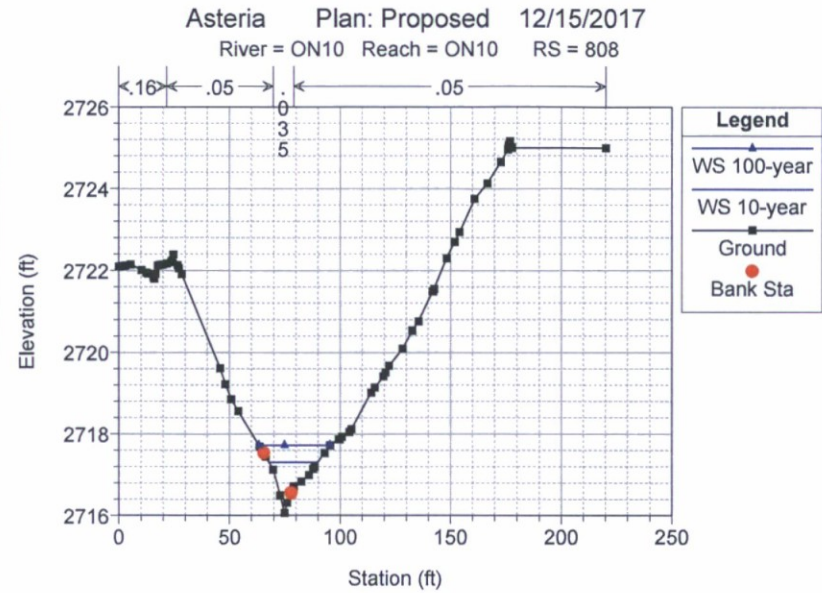
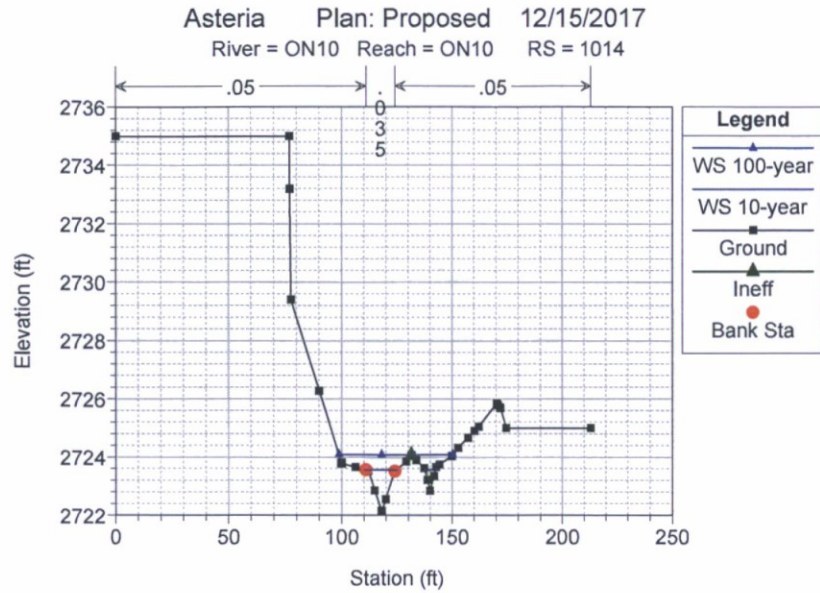
Asteria Plan: Proposed 12/15/2017

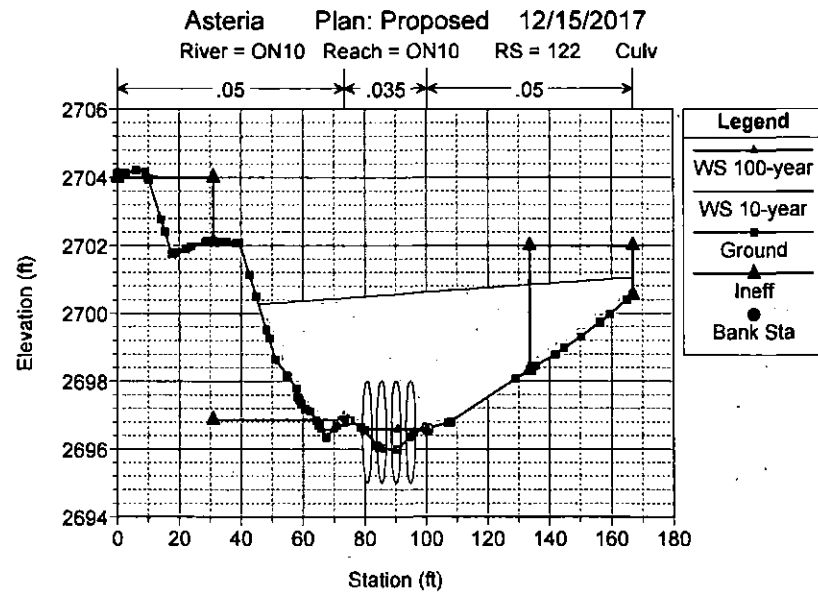
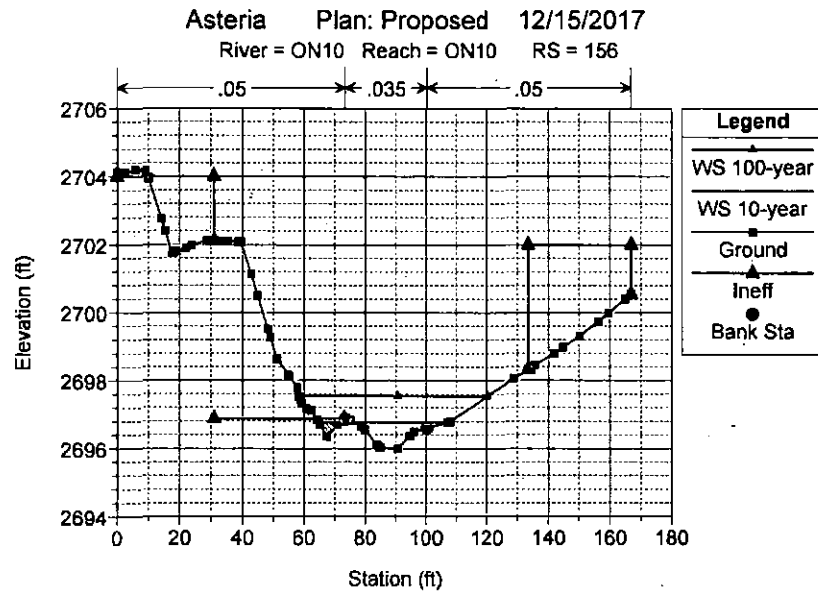
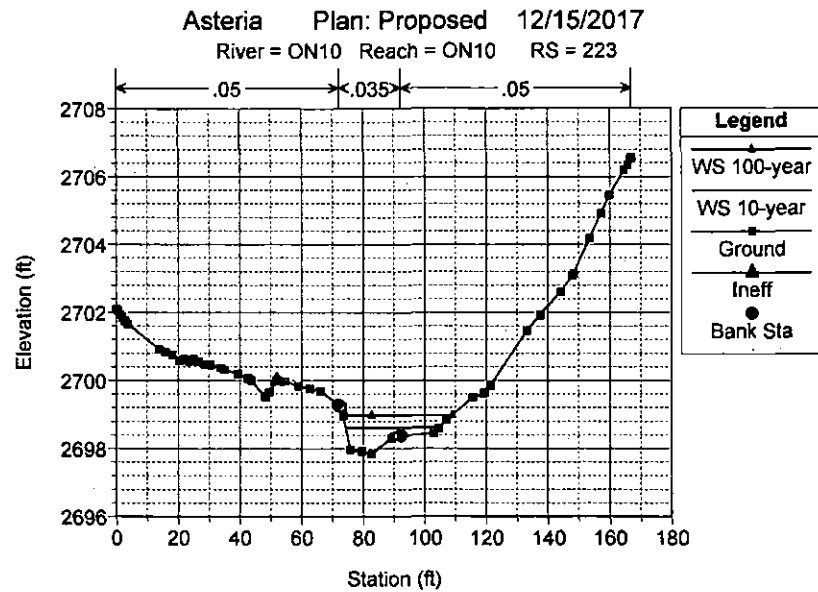
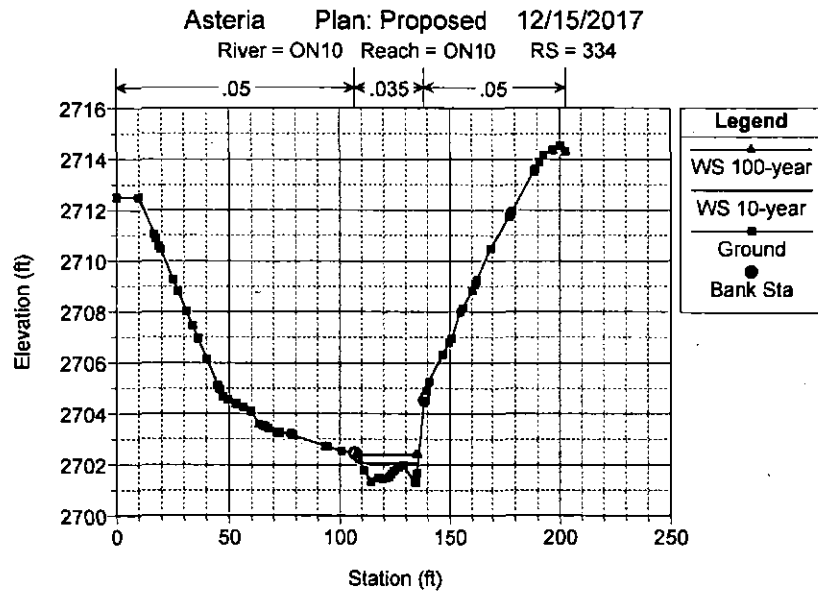
ON20 ON20

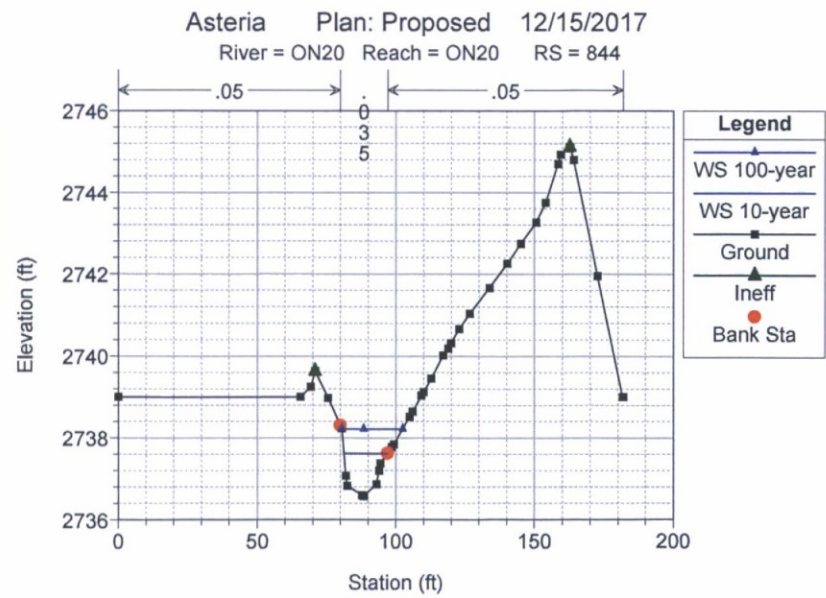
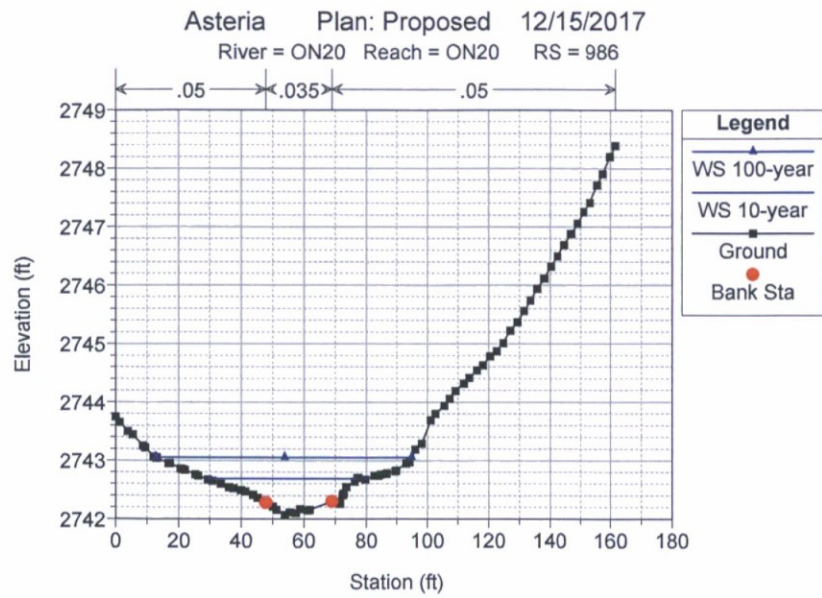
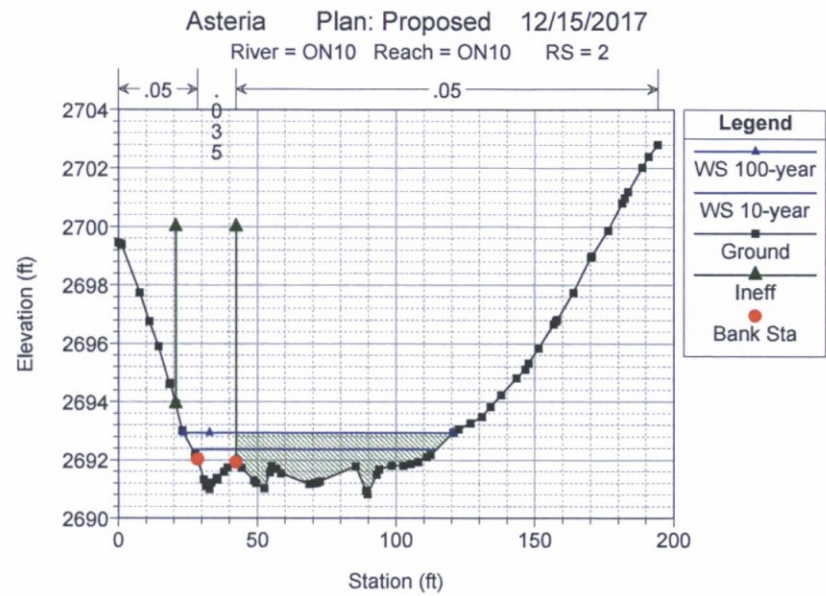
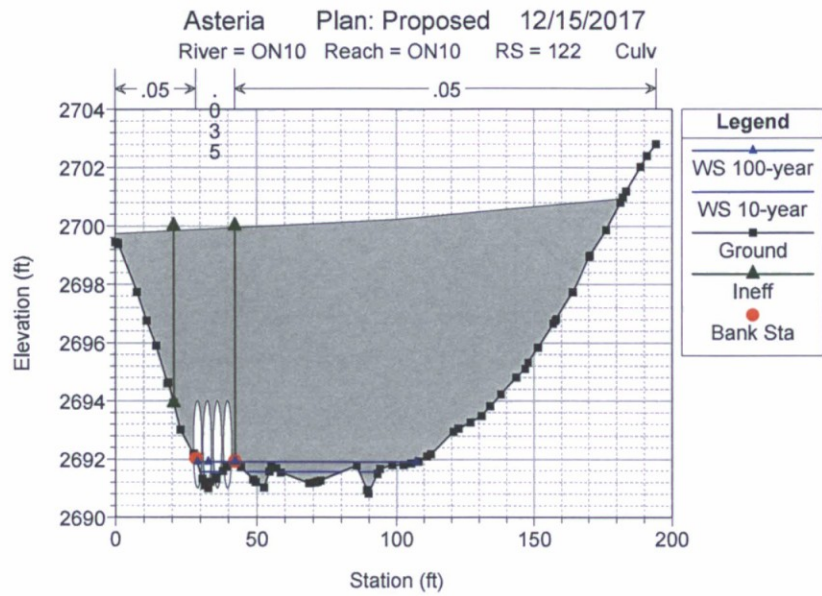


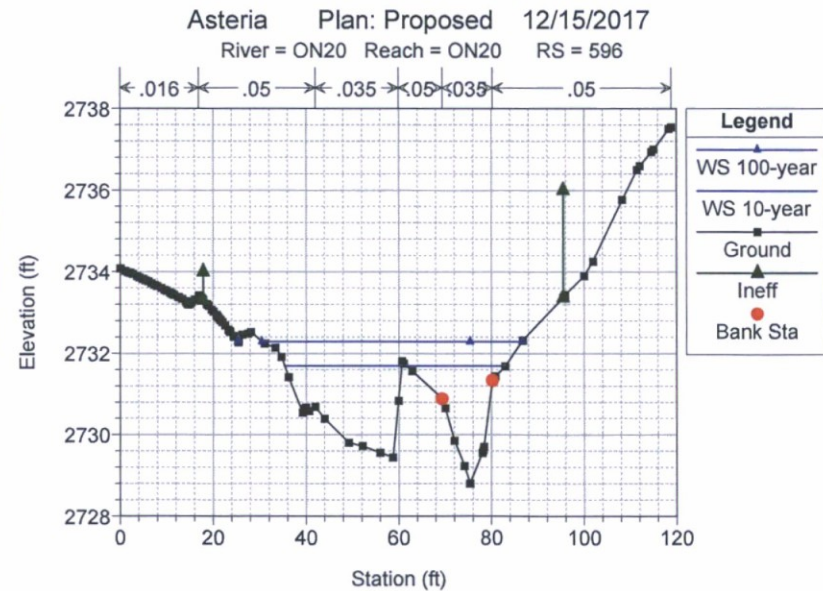
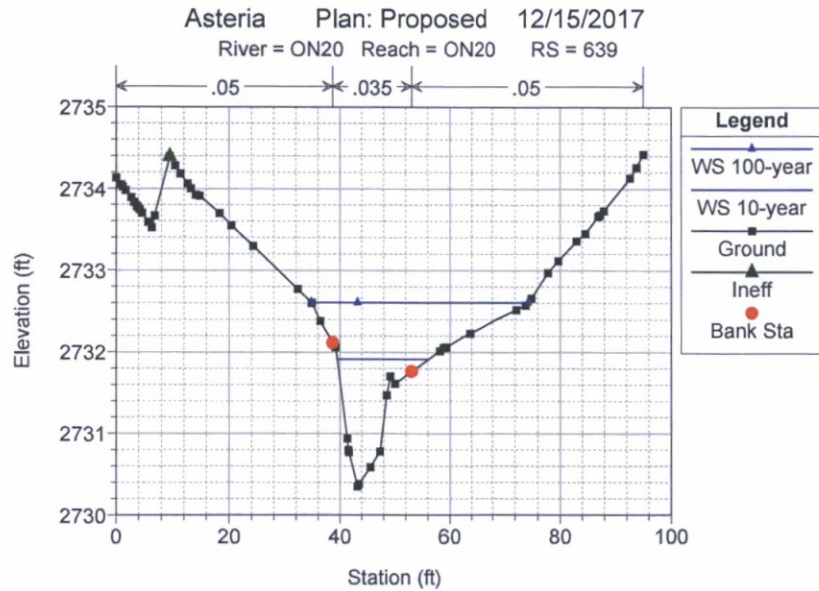
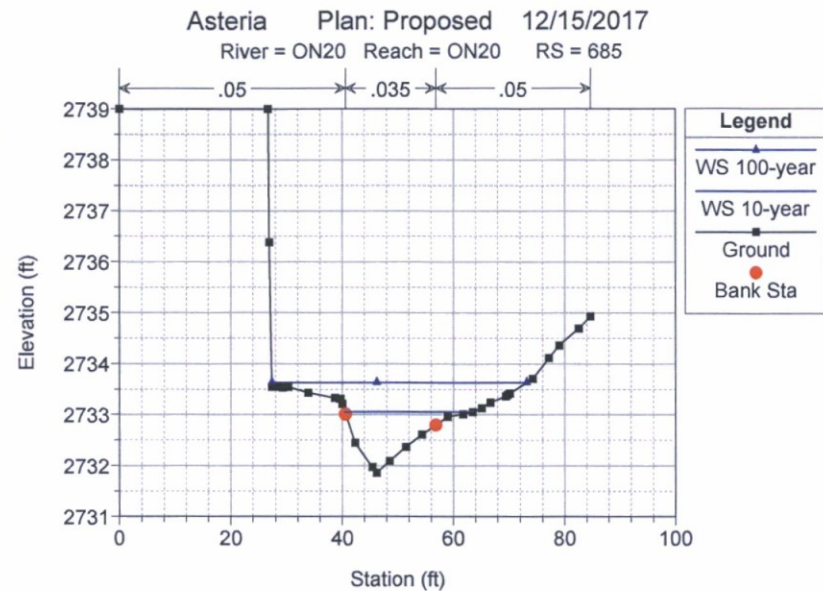
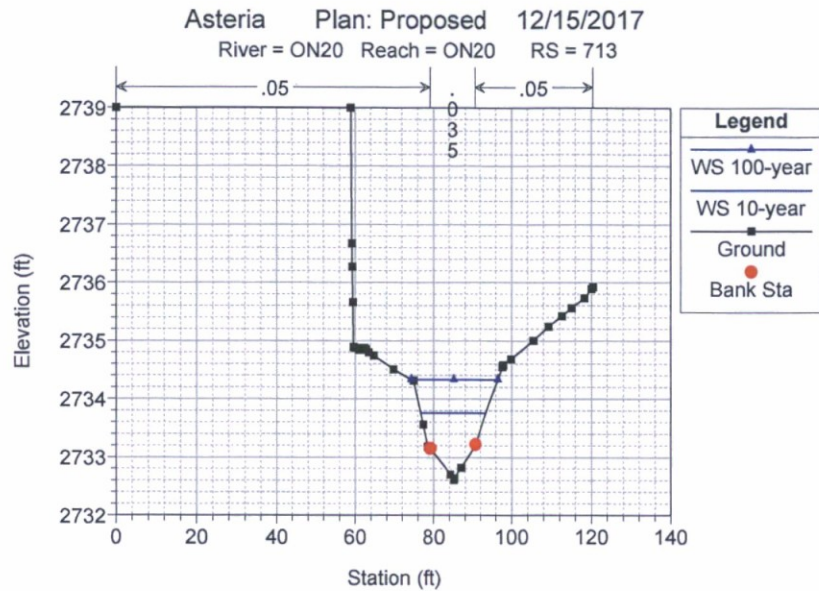


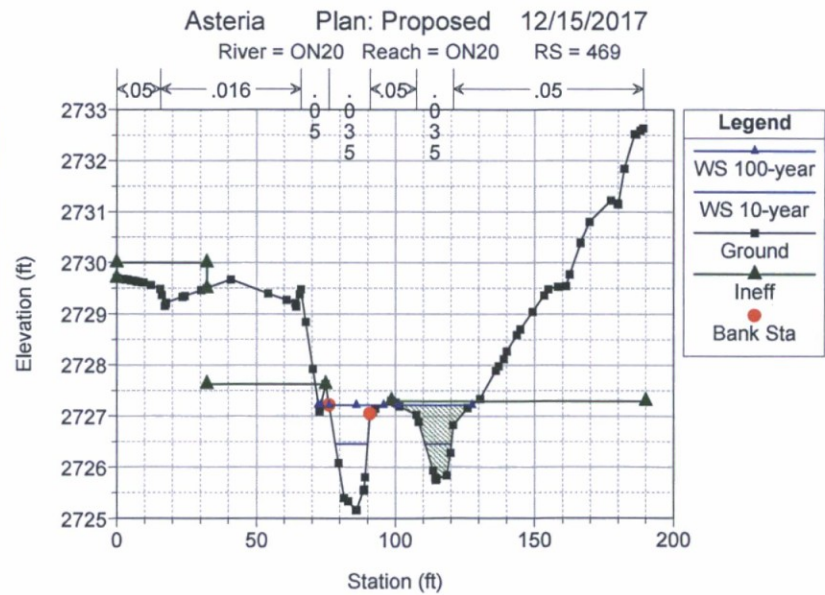
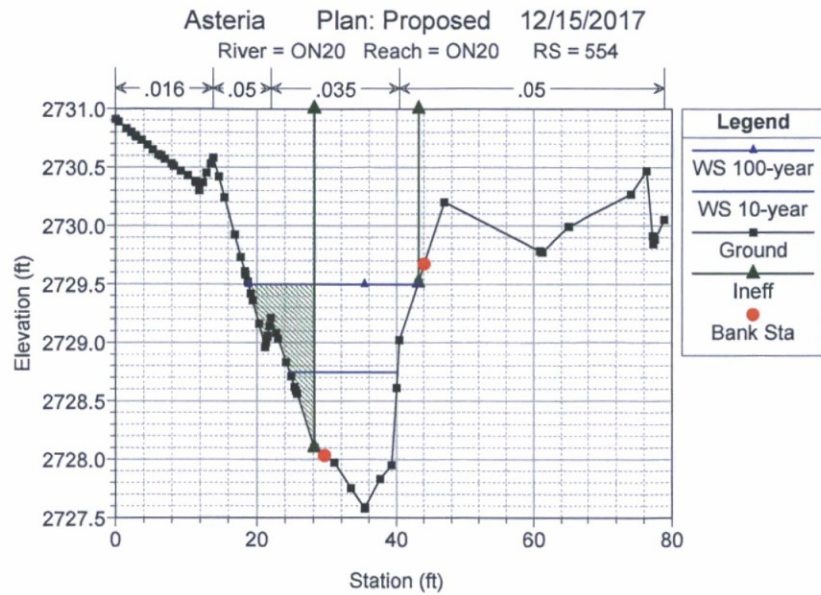
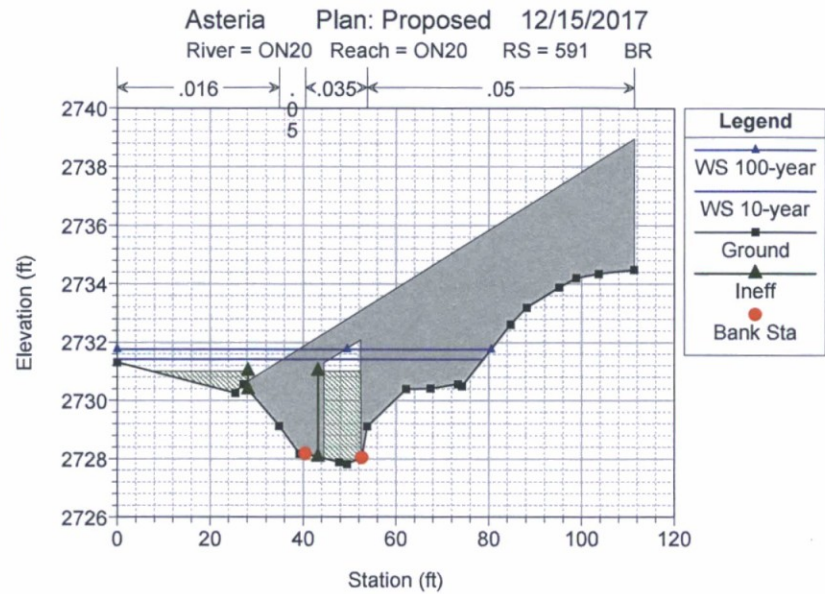
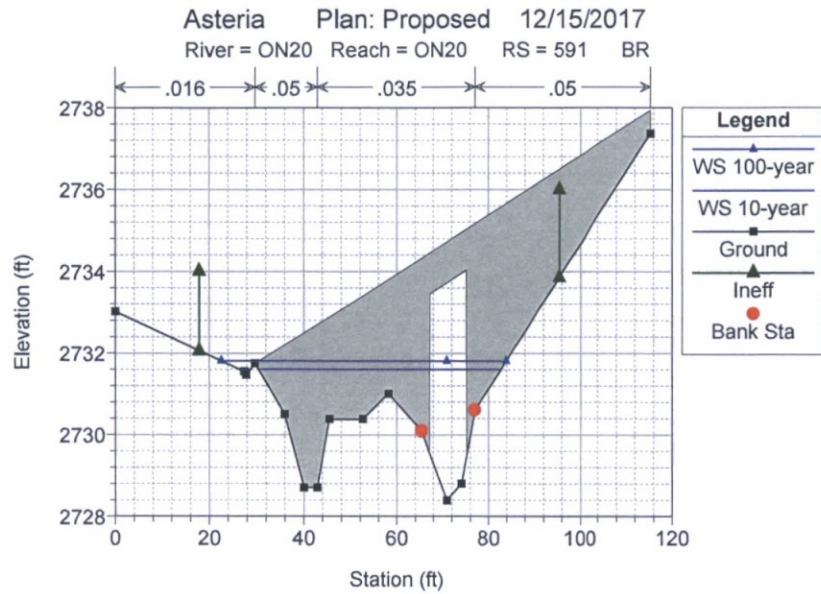


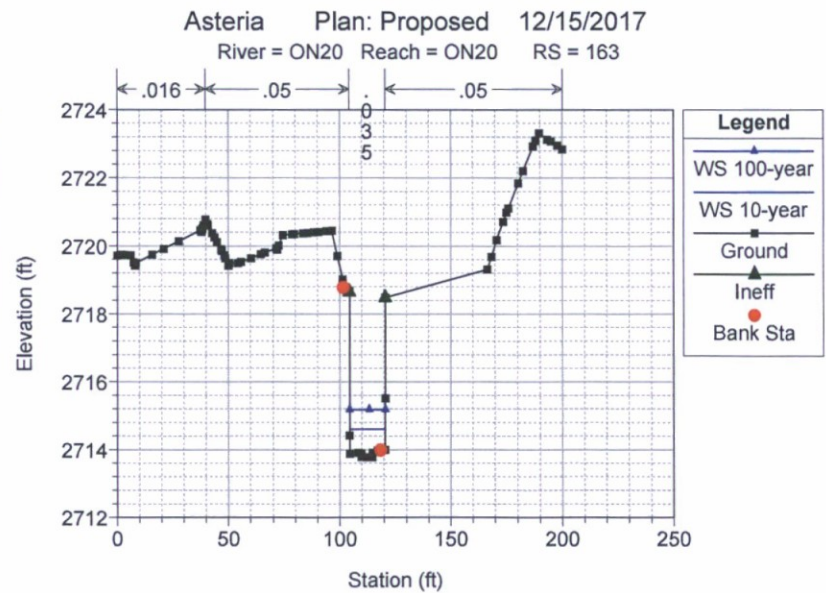
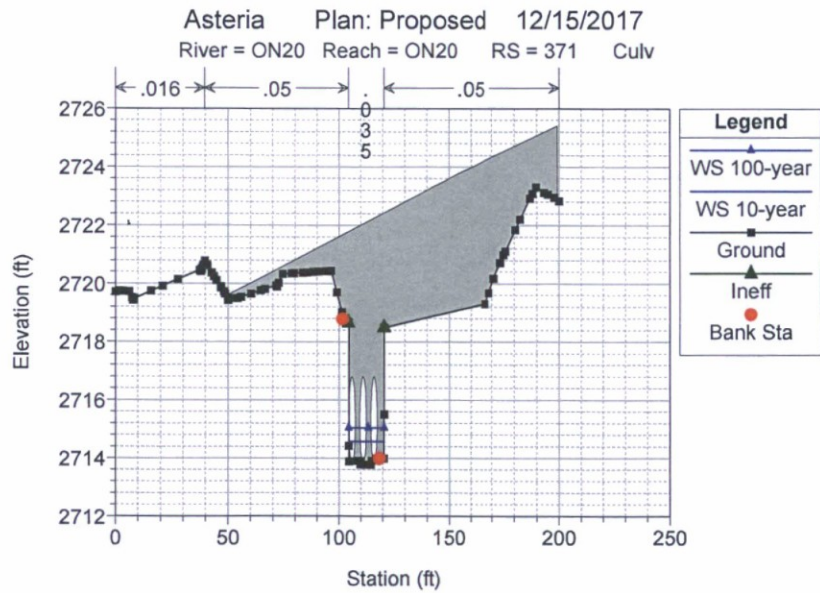
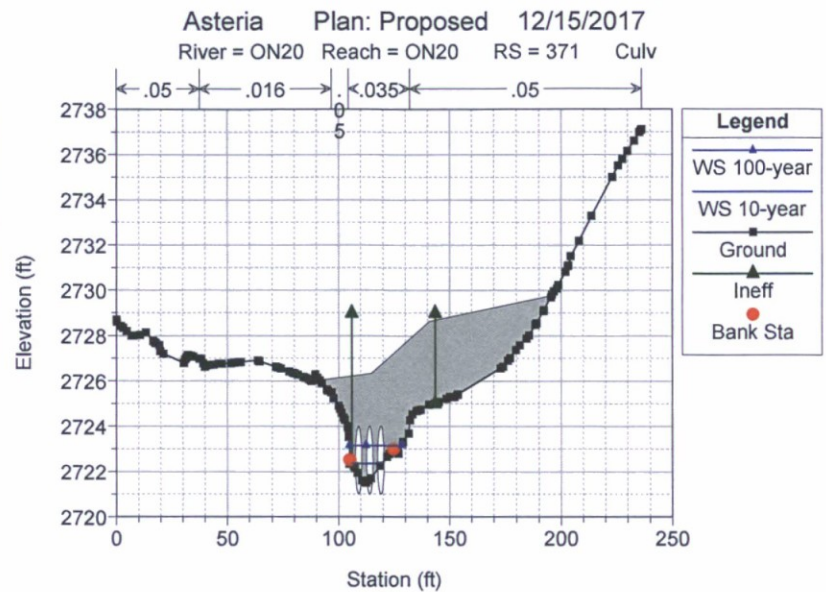
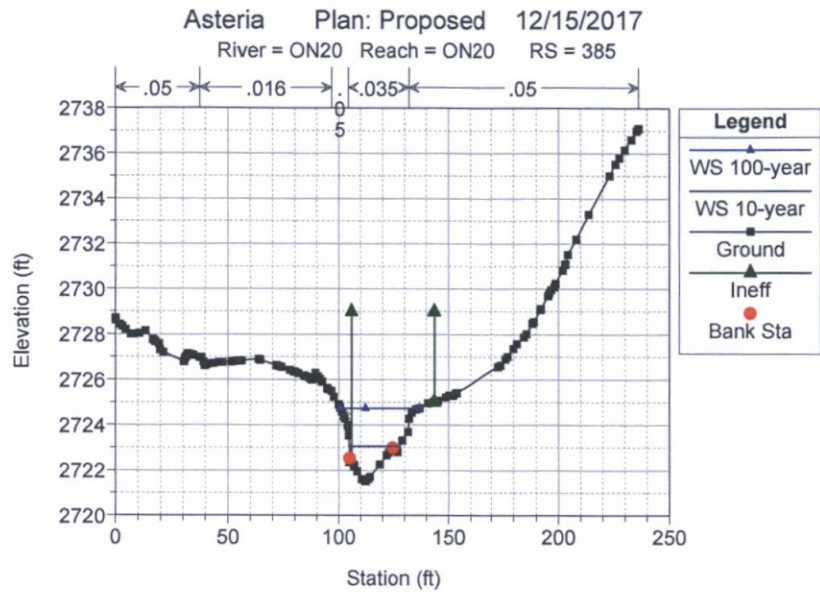












Asteria Plan: Proposed 12/15/2017

River = ON20 Reach = ON20 RS = 11

