

**Drainage Reports**

**Abbreviated Water & Sewer Need Reports**

**Water Study**

**Wastewater Study**

**Stormwater Waiver Application**

**PRELIMINARY DRAINAGE REPORT  
FOR  
DESERT MOUNTAIN PARCEL 19**

June 16, 2016  
WP# 164434

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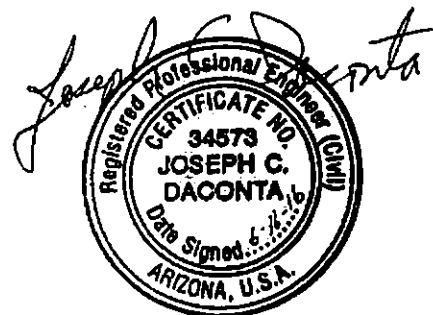
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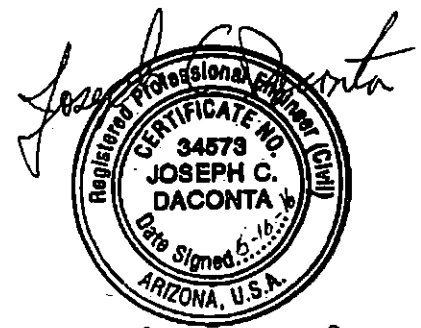
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## APPENDICES

- Appendix A            Existing Condition Hydrologic Calculations
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  - 2-year HEC-1 Model
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## EXHIBITS

- Exhibit 1            Vicinity Map
- Exhibit 2            ESL Classification Map
- Exhibit 3            Flood Insurance Rate Map (FIRM)
- Exhibit 4            Soils Classification Map
- Exhibit 5            Aerial Map
- Exhibit 6            Developed Conditions Land Use Map
- Exhibit 7            Existing Conditions Sub-Basin HEC-1 Map
- Exhibit 8            Developed Conditions Sub-Basin HEC-1 Map
- Exhibit 9            Existing Conditions Hydraulics Map
- Exhibit 10            Developed Conditions Hydraulics Map
- Exhibit 11            Preliminary Grading Plan



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## 1.0 INTRODUCTION

### 1.1 Project Description

Desert Mountain Parcel 19 (DM 19), herein referred to as the “Site,” is an approximate 92-acre parcel of land located in north Scottsdale and is proposed to be rezoned to R4 zoning and OS (Open Space). The Site is located in Section 31, Township 6 North, Range 5 East, of the Gila and Salt River Base and Meridian. The Site is currently an undeveloped parcel, bound to the west by Pima Road and the Carefree Fore More Development (located within the Town of Carefree), to the east by the Desert Mountain Club Golf Course, to the north by the Gambel Quail Preserve 2 Development (both located in the City of Scottsdale), and to the south and southeast by the Velvet Shadows 3 Development and two churches (Christ the Lord Lutheran Church and Our Lady of Joy Roman Catholic Church; all within the Town of Carefree). The Site was originally part of the Desert Mountain Master Development Plan. Exhibit 1 – *Vicinity Map* shows the general location of the project and surrounding areas.

### 1.2 Type of Report

This Report is being prepared as a Preliminary Drainage Report for the DM 19 rezoning submittal to the City of Scottsdale. At this time, only preliminary grading, roadways and a general land use plan has been completed for the Site.

### 1.3 Purpose

The Preliminary Drainage Report for DM 19 has been prepared to meet the drainage design requirements outlined in Chapter 4 of the *City of Scottsdale Design Standards and Policies Manual (DS&PM)*, the *Flood Control District of Maricopa County (FCDMC), Drainage Design Manual for Maricopa County, Arizona: Volume I – Hydrology*, and the *Flood Control District of Maricopa County, Drainage Design Manual for Maricopa County, Arizona: Volume II – Hydraulics*. This report presents the results of the hydrological and hydraulic modeling of the Site’s proposed preliminary drainage systems.

The main purpose of this Report is to illustrate the following:

- Compliance with the City of Scottsdale's Floodplain Ordinance stormwater storage requirements for the property subject to the Environmentally Sensitive Land Ordinance (ESLO). The Site will provide the first flush stormwater storage for the first 0.5-inch of runoff and/or first flush treatment for the property's improved areas. A Request for a Stormwater Storage Waiver is anticipated to be submitted to the City of Scottsdale.
- Reduction of post-development flows to at or below the pre-development flows for 2-year, 10-year, and 100-year, 6-hour storm events using the *U.S. Army Corps of Engineers, HEC-1, Flood Hydrograph Package*. Hence, it is anticipated that the downstream properties shall be provided with similar or potentially better flood protection than the pre-development conditions.
- Maintaining the two main water courses traversing through the Site in their natural locations (the Galloway Wash and Wash A).
- Delineation of the 100-year water surface inundation extents, along with determination of the 100-year water surface elevations for the two main drainage courses using the *U.S. Army Corps of Engineers, HEC-RAS (Version 4.1.0)*.
- Delineation of the 100-year water surface inundation extents for all washes determined to have peak flows equal to or greater than 50 cfs using hydraulic analysis software such as AutoCad Civil 3D *Hydroflow (Version 10.5)*.
- Hydraulic analyses of preliminary drainage structures and wash conveyance corridors.

## 2.0 EXISTING ON-SITE DRAINAGE CONDITIONS AND CHARACTERISTICS

### 2.1 On-Site Drainage

The Site lies in the northern planning section of the City of Scottsdale. The elevations range from 2,612 in the middle of the Site, to 2,650 feet in the east and 2,587 feet in the west. Based on the existing topography, the Site drains primarily from east to west with an approximate average slope of 2%. There are two primary outflow concentration points, one located at the northwest corner and one at the center of the western property boundary. There are several additional minor outflow concentration points along the southern and western property boundary.

Vegetation is typical Sonoran Desert type with creosote bush, jumping cholla, saguaro cacti, palo verde, ironwood, and mesquite trees. The Site lies within the areas identified as Environmentally Sensitive Lands (ESL) by the City of Scottsdale. The Site is further classified as 'Upper Desert' within the ESL areas as shown on Exhibit 2 – *ESL Classification Map*.

The USDA Natural Resources Conservation Service (NRCS), formerly known as the Soil Conservation Service (SCS), provides soils data for all of Maricopa County. This data is reproduced on Exhibit 4 – *Soils Classification Map*. Soils data is the basis of the rainfall loss parameters, as discussed in Section 5.2.3.

There is one wash, the Galloway Wash, that traverses the Site and is categorized as a 'Vista Corridor' or 'Major Wash', as the calculated 100-year, 6-hour flow is greater than 750 cfs. There is also one other significant watercourse traversing the Site that has a 100-year, 6-hour flow greater than 400 cfs and is referred to as "Wash A". The Galloway Wash and Wash A are identified on Exhibit 9 – *Existing Conditions Hydraulics Map*. Wash A was determined to have a 100-year flow of approximately 470 cfs. Both washes will be maintained in their natural locations and will not be re-aligned.

### 2.2 Existing On-Site Drainage Network

Existing on-site drainage sub-basin boundaries were identified using aerial mapped 1-foot contours; refer to Exhibit 7 – *Existing Conditions Sub-Basin HEC-1 Map*.

The on-site watersheds primarily drain east to west or southwest with only one wash exiting the property along the southern boundary with a 100-year peak flow greater than 50 cfs. This wash appears to enter the Velvet Shadows 3 Development and continues west combining with some additional runoff from the Site before flowing through the Our Lady of Joy Roman Catholic Parish Carefree church parking lot and ultimately over Pima Road by an at-grade drainage crossing. The flow exiting the Site has an estimated existing 100-year peak flow of 65 cfs.

Based on the results of the wash hydraulic analysis, a delineation of this wash's existing condition 100-year, 6-hour floodplain limits has been illustrated on Exhibit 9 – *Existing Conditions Hydraulic Map*. Pre-development sub-critical flow water surface elevations for each cross-section are included on Exhibit 9. The hydrologic and hydraulic analysis procedures are discussed in Section 5.0. It is anticipated that this wash will be modified and an Application for Wash Modification will be submitted to the City of Scottsdale.

### **2.3 Off-Site Watersheds**

The off-site drainage areas impacting the Site lay to the east and are in the northern planning section of the City of Scottsdale. The off-site watersheds contain primarily large rural lot single family residential subdivisions and are part of the Desert Mountain Master Development Plan community. These drainage areas are identified on Exhibit 7. The off-site areas are also classified as 'Upper Desert' ESL landform areas by the City of Scottsdale.

### **2.4 Existing Off-Site Drainage Network**

The *Floodplain Delineation Study of Andora Hills & Galloway Washes Technical Data Notebook* (TDN), the *Master Drainage Report for Desert Mountain Parcel C Offsite Drainage Map*, the *Master Drainage Report for Desert Mountain Development Master Development Plan* exhibit and the City of Scottsdale 2-foot contour interval topographic Quarter Section Maps were used to identify and confirm the off-site drainage areas impacting the Site. Refer to Exhibit 7 for the limits of the drainage areas and concentration points.

Off-site flows from the east enter the Site's eastern property boundary at five locations. Four of the concentration points are washes with 100-year peak discharges greater than

50 cfs and one location has a discharge of only 3 cfs. Starting at the northeast property boundary and continuing south, the washes' approximate 100-year peak flows were determined to be: Wash A's flow is 468 cfs, two unnamed washes' flows are 72 cfs and 111 cfs, and the Galloway Wash is 1,111 cfs.

Based on the results of the HEC-RAS and hydraulic wash analyses, a delineation of the existing condition 100-year, 6-hour floodplain limits for the Galloway Wash, Wash A and the two unnamed washes have been illustrated on Exhibit 9 – *Exist Condition Hydraulics Map*. Pre-development subcritical flow water surface elevations for each cross-section are included on Exhibit 9. The hydrologic and hydraulic analysis procedures are discussed in Section 5.0.

## **2.5 Existing Drainage Relative to Adjacent Projects**

Existing washes exit the Site as concentrated flows at various locations along the western and southern property boundary. As Wash A exits the Site it immediately combines with additional flow from the north and is conveyed over Pima Road by an at-grade drainage crossing. The Galloway Wash also is conveyed over Pima Road by an at-grade drainage crossing and the preliminary hydraulic results determined that the depth of flow over Pima Road is greater than one foot. As detailed within the *Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN, when the flow leaves Pima Road it splits around the existing single-family residence before recombining downstream and continuing to the west.

There are an additional six concentrated flow exit locations along the western property boundary and three concentrated flow exit locations along the southern property boundary. These 100-year peak flows vary from a maximum discharge of 67 cfs to only 4 cfs. Refer to Exhibit 7 for the exiting concentration point locations.

There is also a small piece of the Site that is not within the City of Scottsdale and is located at the southeast corner of the property. This portion of the Site is located within the Town of Carefree and also has two existing concentration points that exit the Site to the west.

## 2.6 FEMA Regulated Flood Zones

The Site is located within the Flood Insurance Rate Map (FIRM) for Maricopa County, Arizona and Incorporated Areas, Panel Number 04013C0884L, effective date October 16, 2013. The FIRM, published by the Federal Emergency Management Agency (FEMA), indicates that the Site is located within Special Flood Hazard Areas (SFHAs) Zone AE, Other Flood Areas Zone "X" (Shaded) and Other Areas Zone "X".

Zone "X" is defined by FEMA as follows:

*"Areas determined to be outside 500-year floodplain."*

Zone "X (Shaded)" is defined by FEMA as follows:

*"Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance floods."*

Special Flood Hazard Areas (SFHAs) Subject To Inundation By the 1% Annual Chance Flood is defined as follows:

*"The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Areas is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE AH, AO, AR, A99, V and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood."*

Zone "AE" is defined as: *"Base Flood Elevations determined"*.

The approximate location of the Site relative to the FEMA FIRM panel is illustrated on Exhibit 3- *Flood Insurance Rate Map (FIRM)*.

The proposed Site's development will not alter the effective FEMA SFHA floodplain and/or floodway and will not change the effective FEMA SFHA base flood elevations. This will be documented in more detail as the Site progresses into the pre-plat phase of development.

### **3.0 PROPOSED DRAINAGE PLAN**

#### **3.1 General Description of Proposed Drainage System**

The Site is being rezoned to approximately 44-acres of R4 zoning and approximately 48-acres of Open Space zoning. Preliminary site grading is shown on Exhibit 11 – *Preliminary Grading Plan*. The proposed drainage system is being designed to allow existing drainage patterns to be maintained in their natural location and condition where possible. Where the proposed development will disturb existing washes with 100-year peak discharges equal to or greater than 50 cfs, the washes will be modified to re-direct flow around and/or through the development to maintain historical flow patterns.

The off-site flow of the Galloway Wash will be conveyed through the Site with as little disturbance to the natural wash corridor as possible. The Galloway Wash will have two proposed 3-barrel, 10-foot wide by 4-foot high box culvert roadway drainage crossings to pass the 100-year discharge under the streets. One crossing is located near the eastern property boundary and one crossing is located within the middle of the Site. There is no anticipated disturbance to the Galloway Wash natural wash bottom between these proposed drainage crossings. Downstream of the middle box culvert roadway crossing to approximately 200-feet upstream of the western property boundary, the Galloway Wash will have a relatively small pilot channel (20-foot wide by 1-foot deep) to offset some potential open space encroachment grading proposed within the existing FEMA SHFA. Any grading proposed within the existing FEMA 100-year floodplain will require an FCDMC floodplain use permit.

The off-site flow of Wash A will be conveyed through the Site. The post-development design is not anticipated to disturb the existing natural wash corridor. In addition, any post-development adjacent to the wash is being designed to drain away from Wash A. Therefore, there is no foreseen modification to the wash and/or change to the existing 100-year peak flow of Wash A due to development.

#### **3.2 Future Conditions**

Currently, the upstream off-site watersheds located within Desert Mountain have been fully developed. The Desert Mountain development land uses were obtained from the

*Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN and were duplicated in the HEC-1 hydrologic models.

The drainage plan for the Site proposes to maintain the 2-year, 10-year, and 100-year, 6-hour storm event post-development peak discharges at or below the pre-development flows. In addition, the two significant drainage courses, Wash A and the Galloway Wash, will be maintained in their historic locations at the property boundary. No adverse impact is anticipated on the adjacent properties downstream of the project Site for the 2-year, 10-year, and 100-year, 6-hour storm events.

### **3.3 Stormwater Storage Requirements**

According to the City of Scottsdale's Floodplain Ordinance for property located within the ESL, all runoff generated from the developed portion of the Site must be managed, and the peak discharges from the Site reduced to at least pre-development values during the 100-year, 10-year and 2-year storm frequencies. Proposed retention and/or detention basins will be strategically located along several exiting flow locations and along the Galloway Wash which will attenuate the post-development peak discharges to at or below the pre-development values. Stormwater storage basins will also be designed to intercept flow from upstream proposed developed areas to retain the post-development required first flush retention volume and/or flow will be treated prior to exiting into the adjacent washes. 18-inch bleed-off pipes with removable 6-inch orifice plates are proposed for the dissipation of the retained storm water.

Table 3.3 below summarizes the required first flush stormwater storage for the Site and Exhibit 8 and Exhibit 11 detail the on-site drainage areas and preliminary first flush basin locations, respectively.

**Table 3.3**  
**Required First Flush Stormwater Storage**

<b>Drainage Area</b>	<b>Area (SF)</b>	<b>Area (ac)</b>	<b>Weighted Runoff Coefficient</b>	<b>First Flush Volume (ac-ft)</b>	<b>Required Pre vs Post Volume (ac-ft)</b>
B5	165024	3.79	1.00	0.16	0.42
B6	239591	5.50	1.00	0.23	N/A
B8	300366	6.90	1.00	0.29	N/A
B9	217378	4.99	1.00	0.21	N/A
B10	335363	7.70	1.00	0.32	N/A
B11	269364	6.18	1.00	0.26	N/A
B13	80916	1.86	1.00	0.08	N/A
B14	1072718	24.63	1.00	1.03	N/A
B14A	115650	2.65	1.00	0.11	N/A
C1	636403	14.61	1.00	0.61	N/A

**3.4 Pre- and Post-Runoff Characteristics**

The U.S. Army Corps of Engineers' HEC-1 computer analysis program was used for hydrologic modeling, including routing of flow through storage basins and combining hydrographs. The HEC-1 model was also used to compare the pre- and post-development runoff at concentration points exiting the property. Runoff for each drainage sub-basin was computed and if required, hydrographs were then combined. Drainage basins were further divided into sub-basins to simulate the developed conditions. The parameters were selected per the guidelines provided in the *DS&PM*. The parameters selected and the inputs for the HEC-1 models are discussed in Section 5.1.

The HEC-1 input data and output files for the existing conditions are included in Appendix A. The developed conditions data is included in Appendix B. The data analysis procedures are discussed in Section 5.0.

Table 3.4-1 below provides the comparative peak flows for the pre- vs. post-developed conditions for 100-year, 10-year, and 2-year, 6-hour events at concentration point where flow leaves the property. For the location of these concentration points and corresponding 100-year, 6-hour flow values, refer to Exhibit 7 and Exhibit 8.

**Table 3.4-1  
Pre – vs. Post-Peak Flow Analysis**

HEC-1 ID PRE/POST	100-yr, 6- hr Existing Condition Peak Flow (cfs)	100-yr, 6- hr Post- Dev. Condition Peak Flow (cfs)	100-yr, 6- hr Difference in Peak Flow (cfs)	10-yr, 6- hr Existing Condition Peak Flow (cfs)	10-yr, 6- hr Post- Dev. Condition Peak Flow (cfs)	10-yr, 6- hr Difference in Peak Flow (cfs)	2-yr, 6-hr Existing Condition Peak Flow (cfs)	2-yr, 6-hr Post-Dev. Condition Peak Flow (cfs)	2-yr, 6-hr Difference in Peak Flow (cfs)
CP-A2/CP-A2	469	467	2	261	260	1	128	128	0
B1/DVB11	35	30	5	19	15	4	8	0	8
C1/DTC11	26	5	21	14	5	9	6	5	1
CP-E5/CPGAL	1124	1025	99	549	527	22	230	222	8
F1/DTGF	4	3	1	2	1	1	1	1	0
G1/DTC-H	23	15	8	13	8	5	6	0	6
H1/DV-B5	17	17	0	10	0	10	5	0	5
I1	20			11			5		
J1	10			6			3		
K1	65			36	29		17		
I1,J1,K1/DTC- K	95	50	45	53	18	35	25	1	24
L1	10	10	0	6	6	0	3	3	0
M1	3	3	0	2	2	0	1	1	0

### 3.5 Proposed Drainage Structures

#### 3.5.1 On-site First Flush Storage Basins

On-site first flush storage basins will be used to capture the first 0.5-inch of runoff from the post-development disturbed areas. The design of the basins will be at a maximum depth of 3-feet and have 4:1 side slopes. 18-inch bleed-off pipes with 6-inch orifice plates will be used to drain the stormwater storage within a 36-hour period. Outlet weirs will be incorporated into the basin design for overflow conveyance. When the capacity of the basin is reached, the runoff will overtop the weir and be released at historic flow locations. Erosion protection will be incorporated within the design of the outlets.

#### 3.5.2 Off-Site & On-Site Galloway Wash Detention Basins

As detailed on Exhibit 8, there are two off-site off-line detention basins (Basin 3 and Basin 4) and one on-site off-line detention basin (Basin 1) proposed along the Galloway Wash. The two off-site off-line detention basins will require improved wash channel sections and erosion protected weir inlets to remove a

portion of the 100-year peak flow from the wash. The runoff will enter the detention basin and pond a maximum depth of 4-feet during the 100-year event and have a minimum of an 18-inch outflow pipe that will drain the basin within a few hours. The proposed detention basins will have storage volume capacities of approximately 2.0 acre-feet (Basin 3) and 1.4 acre-feet (Basin 4). Contractual agreements are being prepared with the upstream land owners to allow for the construction and maintenance of these off-line detention basins.

One on-site off-line detention basin will be located downstream of the Galloway Wash middle box culvert crossing and will receive flow from an 18-inch pipe located upstream of the roadway crossing. This detention basin will collect some of the low-flow stormwater runoff to reduce the 2-year and 10-year peak flow of the Galloway Wash. There will be a weir outlet from the detention basin that will drain excess flow back into the Galloway Wash. The proposed detention basin will have approximately 1.1 acre-feet of storage volume at a maximum ponding depth of 4-feet during the 100-year runoff event. The bottom 1-foot will be used for on-site first flush storage. The basin will have an 18-inch outflow pipe with a 6-inch orifice plate that will also drain back into the Galloway Wash. It is anticipated that the basin will drain within a few hours.

The three off-line detention basins were included in the post-development HEC-1 model. The hydrology results can be found within Appendix B.

In several critical locations along the wash, bank protection is necessary to protect proposed off-line detention basins. The bank protection is planned to be aesthetically pleasing and may be either integrally colored soil cement and/or reinforced gunnite/concrete, seated grouted rock riprap and/or gabion baskets/mattresses. Bank protection will be designed to extend to a maximum calculated scour depth of 4-5 feet below the wash bottom. Refer to Exhibit 10 for bank protection locations and Appendix E for scour depth calculations.

### **3.5.3 Roadway Crossing Structures**

There are currently two proposed on-site roadway drainage crossing structures that have been designed to convey the anticipated post-development 100-year

flow of the Galloway Wash. The flows were calculated at the box culvert locations using the results of the post-development HEC-1 model analysis. Refer to Appendix B for the flow calculations.

Both crossing locations will be designed with a 3-barrel, 10-foot wide by 4-foot high box culvert roadway drainage crossing. The roadway crossings were analyzed within the wash's hydraulic analysis using the U.S. Army Corps of Engineers *HEC-RAS (Version 4.1.0)*, such that the estimated upstream 100-year peak flow is contained within the wash and no overtopping of the roadway is anticipated. The box culverts will be designed with drop inlets and a longitudinal slope such that sediment deposition is not anticipated within the culverts.

Riprap and/or erosion protection will be designed at the inlets and outlets of the box culverts to control velocities and prevent erosion. In several critical locations along the wash, bank protection is necessary to protect the proposed roadway improvements. The bank protection shall extend to a maximum calculated scour depth of 4-5 feet below the wash bottom. Refer to Exhibit 10 for bank protection locations and Appendix E for scour depth calculations.

Although the land use is still in a preliminary stage, there are additional locations within the Site that are anticipated to require culverts to pass the flow under roadways and/or to convey flow from the eastern property boundary to the Galloway Wash. These culverts will be designed using AutoCad Civil 3D *Hydroflow (Version 10.5)*, Bentley's *CulvertMaster<sup>®</sup> v3.1*, Copyright<sup>©</sup> 1995-2007, and/or the Federal Highway Administration HY-8 v7.3, 2013 program to determine the pipe sizes, headwater elevations and pipe velocities.

### **3.5.4 On-site Roadway Drainage Structures**

#### **3.5.4.1 Street Flow**

The Rational Method will be used to calculate the storm water runoff generated from the roadways and any on-site adjacent developments that drain into the residential roadways. The 100-year and 10-year flow will be calculated for the street flow based on the FCDMC Rational Method, as discussed in Section 5.1.3.

#### **3.5.4.2 Street Capacity Hydraulics**

There are no anticipated arterial, major and/or minor collector roadways proposed within the development. For all local interior streets, the street conveyance capacity will be calculated using Manning's Equation. The streets will be designed such that the 10-year peak flow is contained within the street curbs and the 100-year peak flow does not exceed a maximum depth of 8-inches. A Manning's "n" value of 0.015 will be used for the standard street cross-section. Roadway capacities will be calculated based on 4-inch and/or 6-inch roll curb (where necessary). The drainage for local residential roadways will be designed consistent with the City of Scottsdale Ordinance 37-42(4) for the allowable depth of water on the street when the street is being utilized as a water carrier. The methodology developed by the Federal Highway Administration will be used to calculate, in spreadsheet format, the allowable street cross-section capacities.

#### **3.5.4.3 Curb Openings**

Catch basins, scuppers and/or depressed curb openings will be designed to convey runoff from the streets such that street flow depths do not exceed 8-inches for the 100-year flow, and the 10-year flow is contained within the curbs. On-site storm water runoff generated by the residential roadways is planned to exit the street via on-grade and/or low point sump curb openings. Flow will then be directed within storm drain, roadside drainage swales and/or along natural grade to the closest receiving wash, retention basin, and/or culvert inlet. All scupper and catch basin curb openings will be designed to include a 0.80 reduction factor.

## 4.0 SPECIAL CONDITIONS

### 4.1 Section 404 Washes

To the best of our knowledge, the US Army Corps of Engineers (Corps) Section 404 of the Clean Water Act (CWA) Jurisdictional Waters of the United States has not yet been determined within the Site. Potential CWA Section 404 Jurisdictional Washes are under investigation and an approved jurisdictional determination submittal inclusive of a significant nexus analysis is being prepared for the Site by Del Sol Group to identify if any of the washes may be deemed jurisdictional. It is understood that an approved jurisdictional delineation of the Waters of the United States is required for the Site.

## 5.0 DATA ANALYSIS METHODS

### 5.1 Hydrologic Method Description

This section documents the engineering procedures and methodologies used to generate the existing and developed condition hydrologic models for the Site. The results of the hydrologic models were used in the conceptual design of drainage facilities and to assure compliance with current drainage design standards.

Precipitation was input by use of the FCDMC 6-hour local storm. Key rainfall statistics were obtained from the *NOAA Atlas 14*, Arizona. The FCDMC DDMSW program was used to develop the necessary point rainfall depth-duration-frequency statistics. Table 5.1 below provides a summary of the point rainfall depth-duration-frequency data. Rainfall losses were calculated by use of the Green and Ampt infiltration equation with an allowance for surface retention loss. Synthetic unit hydrographs for each sub-basin were developed using the Phoenix Desert/Rangeland unit hydrograph as was used within the approved *Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN. Existing and proposed discharges for the 100-year, 10-year and 2-year, 6-hour storm events were modeled using HEC-1 at various concentration points as shown on Exhibit 7 and Exhibit 8, respectively. The HEC-1 results are provided within Appendix A and Appendix B for the existing and proposed condition watersheds, respectively.

**Table 5.1**  
**Point Rainfall Depth-Duration-Frequency Data**

Frequency→	2-year	5-year	10-year	25-year	50-year	100-year
Duration↓	Rainfall (in)					
5 min	0.33	0.44	0.53	0.64	0.73	0.82
10 min	0.50	0.67	0.80	0.98	1.11	1.24
15 min	0.62	0.83	1.00	1.21	1.37	1.54
30 min	0.83	1.12	1.34	1.63	1.85	2.07
1 hour	1.03	1.39	1.66	2.02	2.29	2.56
2 hour	1.19	1.57	1.87	2.27	2.58	2.90
3 hour	1.26	1.64	1.94	2.36	2.70	3.04
6 hour	1.48	1.87	2.18	2.62	2.96	3.31
12 hour	1.78	2.23	2.59	3.08	3.45	3.84
24 hour	2.08	2.73	3.27	4.02	4.65	5.32

## 5.2 Parameter Estimation

The physical parameters of the sub-basins modeled by HEC-1 were estimated by the procedures in the *FCDMC Hydrology Manual*. The information and procedures used to estimate the aforementioned parameters are contained in the following sections. Parameter values are summarized for each sub-basin within the *FCDMC Drainage Design Management System for Windows* (DDMSW) software output located in Appendices A and Appendix B, for the existing and proposed conditions, respectively.

### 5.2.1 Drainage Area

For the existing conditions, the sub-basin drainage areas were determined for use in the HEC-1 model and are shown on Exhibit 7. For the proposed condition, the sub-basin drainage areas were determined for use in the HEC-1 model and are shown on Exhibit 8.

### 5.2.2 Precipitation

Due to the approximate one square mile size of the largest contributing watershed of the Galloway Wash, the 100-year, 6-hour storm frequency was used for the hydrology analysis. Rainfall distributions based on watershed area are furnished by the FCDMC. The contributing watershed area and corresponding precipitation pattern were determined and input into the HEC-1 model using the JD and PC record option.

Point precipitation values used in this study were derived from the isopluvial maps in the *FCDMC Hydrology Manual* which, in turn, were derived from the *NOAA Atlas XIV, Volume III*. The 100-year, 6-hour point rainfall depth used for the Site is 3.31 inches.

### 5.2.3 Soil Data

A description of the soils in the watershed is contained within the *NRCS Soil Survey of Aguila-Carefree Area, Parts of Maricopa and Pinal Counties, Arizona*. Based on the NRCS surveys, the Site's watersheds lie mainly within several soil map units: Unit 93, 96, 34 and for the Galloway Wash Unit 6, with the upstream watersheds consisting of primarily Unit 33. According to the soil survey, the soil

surface is primarily gravely clay and sandy loam. Exhibit 4 – *Soils Classification Map* depicts the contributing watersheds and soil designations.

#### 5.2.4 Rainfall Losses

Rainfall losses were estimated using the Green and Ampt infiltration equation. The procedures used are described in the following paragraphs and were utilized for both existing and proposed condition HEC-1 modeling.

The composite (unadjusted) XKSAT parameter was calculated in the *FCDMC DDMSW* program using the log-average method for each sub-basin. This was accomplished by multiplying the total area of each soil map unit in the sub-basin by the common logarithm of the associated XKSAT value. The resultant products were then totaled and the sum was divided by the total area of the sub-basin. The result is the composite log-average bare ground XKSAT parameter. The log-average XKSAT parameter was then adjusted for the effects of vegetation cover using data from Figure 4.4 of the *FCDMC Hydrology Manual*. The volumetric soil moisture deficit at the start of rainfall (DTHETA) and wetting front capillary suction (PSIF) parameters are directly related to the composite bare ground hydraulic conductivity (XKSAT) by Figure 4.3 in the *FCDMC Hydrology Manual*.

The DTHETA parameters were read from lookup tables within the DDMSW program using the unadjusted XKSAT value calculated as described above. Two (2) DTHETA conditions are possible, dry and normal. The "dry" condition was used for all areas in the existing condition model for undeveloped desert areas. The "normal" condition was used for all other land uses occurring within the project watershed for the proposed condition model. DTHETA values were read from the lookup tables corresponding to the unadjusted XKSAT value, and were averaged by land use area-weighting within the DDMSW program. The value of PSIF was also read from lookup tables based on the unadjusted XKSAT value.

Initial abstraction (IA) and percent impervious (RTIMP) values correlate to soil types and land use. The following section further discusses the hydrologic significance of the IA and RTIMP parameters.

### 5.2.5 Land Use Characteristics

Land use characteristics upstream of the study area were obtained from the *Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN and were verified from aerial photographs. Surface characteristics affecting the hydrology include terrain (land use classification), the proportion of impervious surfaces, and vegetative cover density. *DDMSW* values for initial abstraction (IA), percent impervious (RTIMP) were obtained from the FCDMC Hydrology Manual. The *Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN was used to estimate the hydraulic efficiency (Kn) for the *Rural Density Residential* land use parameters. Assigned values for all parameters are shown in Appendices A and Appendix B for existing and proposed land use conditions, respectively.

### 5.2.6 Unit Hydrographs

To be consistent with the approved *Floodplain Delineation Study of Andora Hills & Galloway Washes* TDN, the *FCDMC Hydrology Manual* Phoenix Desert/Rangeland unit hydrograph was used for watersheds upstream of the project Site and for the undeveloped on-site areas. The Phoenix Valley S-graph unit hydrograph was used for the post-development on-site conditions. Separate unit hydrographs are generated for each sub-basin by the use of the *DDMSW* program. This program calculates the basin lag time. Assigned values for all parameters are shown within the printouts of the *FCDMC DDMSW* located within Appendix A and Appendix B for the existing and proposed conditions, respectively.

### 5.2.7 Computation Time Interval

The computation time interval (NMIN) used in the HEC-1 models was based on guidelines in the *FCDMC Hydrology Manual*, which recommends an NMIN value of  $0.15 * T_c$ . Due to the small post-development on-site watershed areas, a minimum NMIN value of 1 minute was used and a 15-minute hydrograph time interval was used for the entire study area. For comparison purposes, a 1-minute NMIN value and a 15-minute hydrograph time interval was also used for the existing condition model.

### 5.2.8 Routing Parameters

Routing of sub-basin hydrographs in the study area will be performed utilizing the normal depth/storage channel routing option of HEC-1.

## 5.2 Rational Method

The Rational Method will be used to compute peak discharges to size on-site culverts with watersheds less than or equal to 160 acres. Parameters necessary for this procedure are the measurement of drainage sub-basin areas, runoff coefficient (“C” values), and calculation of rainfall intensity. Runoff coefficients will be calculated using the values based on Figure 4-5 “Runoff Coefficients for Use with Rational Method” in the *DS&PM*.

## 5.3 Storm Water Storage

Based on the City of Scottsdale’s Drainage Ordinance stormwater storage requirements, on-site first flush storm water storage is proposed to be provided for the first 0.5-inch of runoff. The Rational Method is used to estimate the first flush stormwater storage volumes. The required volume is based on the areas of the proposed disturbances within the development. The equations used to calculate the required and provided retention volumes are presented below.

### 5.3.1 Required Retention Volume

Retention volume required for the 100-year, 2-hour event is:

$$V_{REQUIRED} = C * \left( \frac{P}{12} \right) * A$$

Where:

- V is the required retention volume in acre-feet.
- C is the weighted “C” coefficient.
- P is the precipitation in inches for the 100-year, 2-hour rainfall; 2.70- inches for the proposed developed areas.
- A is the drainage area in acres.

### 5.3.2 Provided Retention Volume

Retention volume provided for the 100-year, 2-hour event is:

$$V_{PROVIDED} = \left[ \left( \frac{A_1 + A_2}{2} \right) * (ELEV_2 - ELEV_1) \right]$$

Where:

- V is the provided retention volume in acre-feet.
- A is the contour area in acres.
- ELEV is the contour elevation.

Section 3.3 of this Report describes the preliminary storm water storage proposed basin locations. Assumptions that were made when determining provided volume were maximum side slopes of 4:1. Refer to stormwater storage basin volume calculations in Appendix C.

## **5.4 Hydraulic Procedures**

### **5.4.1 Hydraulic Analysis of Open Channels**

Due to the fact that the Galloway Wash and Wash A have significant 100-year peak flows, the U.S. Army Corps of Engineers' HEC-RAS computer program was used for the hydraulic analysis of both washes. Washes that were determined to be less than a 100-year peak flow of 150 cfs will use hydraulic analysis software such as AutoCad Civil 3D *Hydroflow* (Version 10.5) to determine their existing wash hydraulic conditions.

### **5.4.2 HEC-RAS Errors, Warnings, and Notes**

The HEC-RAS cross sections were placed such that significant variations in the channel cross-sectional geometry are adequately represented. Due to the relatively steep slopes on the Site, it was not feasible to put enough cross-sections such that the difference in energy grade elevations is less than 1-foot between cross sections. The HEC-RAS computer program gives a warning message for a difference in energy grade elevation of greater than 1-foot between cross sections. Hence, a wash was analyzed at an elevation difference of 1-foot to demonstrate the impact of additional cross sections on the HEC-RAS analysis. It was found that increasing the number of cross-sections did not impact the flow depths, as long as adequate cross-sections representative of the geometry of the channel are included.

HEC-RAS gives multiple warning messages when the energy equation could not be balanced, resulting in the program using the critical depth for the water

surface elevation. This is due to the fact that a sub-critical flow regime was selected for the steady flow computations on a site that is relatively steep. The on-site washes are flowing super-critical, but a sub-critical flow regime was chosen because it results in the highest water surface elevations.

Since the proposed culverts generally follow the same slope as the washes, the flow in the culverts is also flowing super-critical, as noted by HEC-RAS with a note for each culvert within the model. These warnings and notes are expected when running sub-critical flow regimes for a steady flow analysis on steep sites.

HEC-RAS gives a warning message when the upstream conveyance ratio divided by the downstream conveyance ratio is less than 0.7 or greater than 1.4. Again, this is a result of the steep slopes on the Site, and it was not feasible to put enough cross-sections such that the difference in the conveyance ratio is less than 0.7 or greater than 1.4.

There are a number of warning messages generated by the HEC-RAS computer program for the hydraulic analysis of the project. These warnings do not affect the accuracy of the results and are intended to alert the user of any conditions outside of the expected norm. These warning messages, and notes for both the Developed Conditions and the Existing Conditions models were ignored, and the model was determined to be acceptable.

#### **5.4.3 Hydraulic Analysis of Culverts**

For analysis of anticipated additional culvert roadway crossings, the computer programs AutoCad Civil 3D *Hydroflow* (Version 10.5), CulvertMaster and/or HY-8 will be used for final design. Site characteristics and flow are entered into the program and the resulting pipe sizes, flow regime, headwater and tailwater values are calculated.

## 6.0 DRAINAGE PLAN REQUIREMENTS

### 6.1 Drainage System Requirements

The Site is being rezoned to approximately 44-acres of R4 zoning and approximately 48-acres of Open Space (OS) zoning. There are specific drainage system requirements in order for the proposed drainage design to be approved for the Site by the City of Scottsdale and are as follows:

1. According to the City of Scottsdale's Floodplain Ordinance for property located within the ESL, storm water storage may be waived under certain conditions and peak discharges from the Site reduced to at least pre-development values during the 100-year, 10-year and 2-year storm frequencies. The development must obtain a Stormwater Storage Waiver from the City of Scottsdale. In addition to the Stormwater Storage Waiver, the Site must either provide first-flush treatment and/or first-flush stormwater storage for the first 0.5-inch of runoff from the property's developed areas.
2. The proposed drainage system is being designed to allow existing drainage patterns to be maintained in their natural condition and location where possible. When the proposed development will disturb existing washes with peak 100-year discharges of 50 cfs or more, the washes will be modified to re-direct flow around and/or through the development to maintain historical flow patterns. The development must obtain a Wash Modification approval from the City of Scottsdale.
3. Any proposed development encroachment into the FEMA FIRM SFHAs must obtain an FCDMC Floodplain Use Permit.
4. The determination of the CWA Section 404 Jurisdictional Washes for the Site must be approved by the U.S. Army Corps.
5. Before any construction activities that will disturb one or more acres begin, these activities must be authorized by ADEQ under the Arizona Pollutant Discharge Elimination System (AZPDES) Construction General Permit. The City of Scottsdale also requires evidence of compliance before issuing development permits.

## **6.2 Easement Requirements**

Where flows from the 100-year storm event are greater than 50 cfs, drainage easements will be required around the limits of the 100-year floodplain inundation. In addition, drainage easements will also be dedicated around the limits of the 100-year ponding for the retention and detention basins.

## **6.3 Roadway Crossing Requirements**

In all cases, the depth of flow over streets will be in accordance with the City of Scottsdale Floodplain Ordinance and Design Standards & Policies Manual (2010).

## **6.4 Lowest Floor Elevations**

Lowest floor (LF) elevations are to be a minimum of 12-inches above the highest adjacent 100-year water surface elevation and 14-inches above the low-site outfall. Lowest floor elevations on the grading and drainage plans for residential units reflect slab on grade conditions and cannot be lowered without agency approval in locations where 'Special Flood Hazard Areas' exist. In non-flood hazard locations, to ensure that adequate residential lot drainage can be achieved, a professional engineer should be consulted if the lowest floor elevation for the slab is proposed to be lowered, or if a basement is to be constructed.

## **6.5 Maintenance**

Ongoing maintenance of the designed or recommended drainage systems is required to preserve the design integrity and purpose of the drainage system. Failure to provide maintenance can prevent the drainage system from performing to its intended design purpose and can result in reduced performance. Maintenance within the public right-of-way is the responsibility of the governing municipality. However, it is the responsibility of private developers, homeowner associations, etc. for facilities on private property within drainage easements and includes private streets. A regular maintenance program is required so that drainage systems perform to the level of protection or service as presented in this report and the project's plans and specifications.

Regular maintenance must be performed on detention/retention basins that are designed with sediment pools and/or are susceptible to wash sediment loads. Observation is required annually and after major storm events to monitor basin sediment load. Basins should be maintained and cleaned out in order for the drainage system to function properly.

#### **6.6 Bank Protection**

Scour protection shall be provided at all locations where the wash banks are being modified, where development is encroaching within the wash's erosion hazard setback limits and where it is necessary to protect proposed retention/detention basins. The bank protection considered may be one of a variety of choices: integrally colored soil cement and/or reinforced shotcrete/concrete, seated grouted rock riprap and/or gabion baskets. Bank protection will be designed to extend to a maximum calculated scour depth of 4-5 feet below the wash bottom. Refer to Exhibit 10 – *Developed Condition Hydraulics Map* for bank protection locations, and Appendix E for scour depth calculations.

#### **6.7 Erosion Protection**

Culverts that convey flow beneath roadways are to incorporate erosion protection at both the inlet and the outlet of the structures to dissipate energy and provide flow line scour protection. Detention and/or retention basins that utilize weir inlet and/or outlet structures will require erosion protection to prevent scour when flows overtops the weir. Bleed-off pipes will also incorporate riprap protection at pipe outlets.

## 7.0 CONCLUSIONS

1. The Site is located within the Flood Insurance Rate Map (FIRM) for Maricopa County, Arizona and Incorporated Areas, Panel Number 04013C0884L, effective date October 16, 2013 and is located within Special Flood Hazard Areas (SFHAs) Zone AE, Other Flood Areas Zone "X" (Shaded) and Other Areas Zone "X".
2. The proposed Site development will not alter the effective FEMA SFHA floodplain and/or floodway and will not change the effective FEMA SFHA base flood elevations. This will be documented in more detail as the Site progresses into the pre-plat phase of development.
3. The Galloway Wash and Wash 'A' will be maintained in their natural location and condition where possible and will not be re-aligned.
4. According to the City of Scottsdale's Floodplain Ordinance the Site is located within the ESL and will apply for a stormwater storage waiver
5. Peak discharges from the Site will be reduced to at least pre-development values during the 100-year, 10-year and 2-year storm frequencies.
6. The Site will must provide either first flush stormwater storage for the first 0.5-inch of runoff and/or first flush treatment for the property's developed areas.
7. The proposed drainage system is being designed to allow existing drainage patterns to be maintained in their natural location and condition where possible. Where the proposed development will disturb existing washes with peak 100-year discharges of 50 cfs or more, the washes will be modified to re-direct flow around and/or through the development to maintain historical flow patterns. The development will obtain a Wash Modification approval from the City of Scottsdale.
8. Any proposed development encroachment into the FEMA FIRM SFHAs must obtain an FCDMC Floodplain Use Permit.
9. The determination of the CWA Section 404 Jurisdictional Washes for the Site must be approved by the U.S. Army Corps.
10. Before any construction activities that will disturb one or more acres begin, these activities must be authorized by ADEQ under the Arizona Pollutant Discharge Elimination System (AZPDES) Construction General Permit. The City of Scottsdale also requires evidence of compliance before issuing development permits.

11. The design of hydraulic structures is based on generally accepted engineering practices and in accordance with City of Scottsdale's requirements.
12. The drainage for the local residential roadways will be designed consistent with City of Scottsdale Ordinance 37-42(4) for the allowable depth of water on the street when the street is being utilized as a water carrier.
13. Ongoing maintenance is required for all drainage systems in order to assure design performance. Regular maintenance must be performed on detention/retention basins that are designed with sediment pools and/or are susceptible to wash sediment loads. Observation is required annually and after major storm events to monitor basin sediment load. Basins should be maintained and cleaned out in order for the drainage system to function properly.

8.0 WARNING & DISCLAIMER OF LIABILITY

Per the requirements outlined in Chapter 4 of the *City of Scottsdale Design Standards and Policies Manual (DS&PM)*, each drainage report must include a completed 'Warning and Disclaimer of Liability' as provided within the *DS&PM Appendix 4-1C*. As such, below is a City of Scottsdale 'Warning and Disclaimer of Liability' that will be completed as the Site progress into the pre-plat phase of development.



Appendix 4-1C  
**WARNING & DISCLAIMER OF LIABILITY**

The Drainage and Floodplain Regulations and Ordinances of the City of Scottsdale are intended to "minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding caused by the surface runoff of rainfall" (Scottsdale Revised Code §37-16).

As defined in S.R.C. §37-17, a flood plain or "Special flood hazard area means an area having flood and/or flood related erosion hazards as shown on a FHBM or FIRM as zone A, AO, A1-30, AE, A99, AH, or E, and those areas identified as such by the floodplain administrator, delineated in accordance with subsection 37-18(b) and adopted by the floodplain board." It is possible that a property could be inundated by greater frequency flood events or by a flood greater in magnitude than a 100-year flood. Additionally, much of the Scottsdale area is a dynamic flood area; that is, the floodplains may shift from one location to another, over time, due to natural processes.

**WARNING AND DISCLAIMER OF LIABILITY PURSUANT TO S.R.C §37-22**

"The degree of flood protection provided by the requirements in this article is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by man-made or natural causes. This article (Chapter 37, Article II) shall not create liability on the part of the city, any officer or employee thereof, or the federal government for any flood damages that result from reliance on this article or any administrative decision lawfully made thereunder."

Compliance with Drainage and Floodplain Regulations and Ordinances does not insure complete protection from flooding. The Floodplain Regulations and Ordinances meet established local and federal standards for floodplain management, but neither this review nor the Regulations and Ordinances take into account such flood related problems as natural erosion, streambed meander or man-made obstructions and diversions, all of which may have an adverse affect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations.

I have read and understand the above. If I am an agent for an owner I have made the owner aware of and explained this disclaimer.

Plan Check No.                      Owner or Agent                      Date

## **APPENDIX A**

### **Existing Condition Hydrologic Calculations**

**DDMSW Output Data**

**2-year HEC-1 Model**

**10-year HEC-1 Model**

**100-year HEC-1 Model**

**DDMSW Output Data**

Flood Control District of Maricopa County  
 Drainage Design Management System  
 LAND USE  
 Project Reference: DM19 EX

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
<b>Major Basin ID: 01</b>									
A1	901	0.0181	9.2	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.1792	90.8	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		<b>0.1973</b>	<b>100.0</b>						
A2	900	0.0146	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0146</b>	<b>100.0</b>						
B1	900	0.0115	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0115</b>	<b>100.0</b>						
C1	900	0.0084	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0084</b>	<b>100.0</b>						
D1	900	0.0024	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0024</b>	<b>100.0</b>						
E1	900	0.0018	8.3	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.0045	20.7	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.0154	71.0	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		<b>0.0217</b>	<b>100.0</b>						
E2	900	0.0012	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0012</b>	<b>100.0</b>						
E3	900	0.0038	10.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.0057	15.0	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.0285	75.0	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		<b>0.0380</b>	<b>100.0</b>						

\* Non default value

Flood Control District of Maricopa County  
 Drainage Design Management System  
 LAND USE  
 Project Reference: DM19 EX

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
<b>Major Basin ID: 01</b>									
		<b>0.0200</b>	<b>100.0</b>						
L1	900	0.0027	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0027</b>	<b>100.0</b>						
M1	900	0.0015	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0015</b>	<b>100.0</b>						

\* Non default value

Flood Control District of Maricopa County  
 Drainage Design Management System  
 SUB BASINS

Project Reference: DM19 EX

Area ID	Sub Basin Parameters								Rainfall Losses				
	Area (sq mi)	Length (mi)	Slope (ft/mi)	S-Graph	Lca (mi)	Lag (min)	Velocity (f/s)	Kn	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)
<b>Major Basin ID: 01</b>													
A1	0.197	0.95	134.1	DESERT/RANGE	0.51	16.80	4.96	0.039	0.29	0.25	5.34	0.258	5
A2	0.015	0.19	118.3	DESERT/RANGE	0.10	5.10	3.20	0.040	0.35	0.37	5.24	0.250	
B1	0.011	0.22	209.1	DESERT/RANGE	0.10	4.90	3.96	0.040	0.35	0.35	4.33	0.401	
C1	0.008	0.21	216.3	DESERT/RANGE	0.09	4.60	4.00	0.040	0.35	0.35	4.33	0.403	
D1	0.002	0.09	252.9	DESERT/RANGE	0.04	2.30	3.27	0.040	0.35	0.35	4.33	0.403	
E1	0.022	0.31	184.7	DESERT/RANGE	0.15	6.40	4.35	0.038	0.28	0.26	5.05	0.306	4
E2	0.001	0.05	297.9	DESERT/RANGE	0.02	1.40	3.00	0.040	0.35	0.36	5.05	0.281	
E3	0.038	0.57	146.3	DESERT/RANGE	0.25	10.40	4.85	0.039	0.29	0.26	5.05	0.299	4
E4	0.921	3.33	259.5	DESERT/RANGE	1.69	36.70	7.99	0.038	0.28	0.27	5.24	0.281	6
E5	0.063	0.39	123.7	DESERT/RANGE	0.23	9.20	3.71	0.040	0.35	0.36	5.14	0.272	
F1	0.001	0.05	215.7	DESERT/RANGE	0.02	1.50	2.96	0.040	0.35	0.30	7.94	0.085	
G1	0.007	0.24	167.4	DESERT/RANGE	0.09	5.10	4.15	0.040	0.35	0.36	5.14	0.264	
H1	0.005	0.17	189.3	DESERT/RANGE	0.07	3.90	3.77	0.040	0.35	0.36	5.05	0.281	
I1	0.006	0.23	191.1	DESERT/RANGE	0.11	5.20	3.80	0.040	0.35	0.36	5.05	0.281	
J1	0.003	0.16	206.5	DESERT/RANGE	0.07	3.80	3.64	0.040	0.35	0.36	5.14	0.262	
K1	0.020	0.23	171.7	DESERT/RANGE	0.15	6.10	3.38	0.040	0.35	0.37	5.24	0.257	
L1	0.003	0.12	243.7	DESERT/RANGE	0.05	2.90	3.62	0.040	0.35	0.36	5.05	0.281	
M1	0.001	0.10	204.1	DESERT/RANGE	0.04	2.60	3.38	0.040	0.35	0.36	5.05	0.281	

\* Non default value

**2-year HEC-1 Model**

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 15JUN16 TIME 20:29:19 *
*****

```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

```

X X XXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXX XXXXX XXXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 EX - Desert Mountain 19 Existing Condition
3 ID 2 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19EX2.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 1.475 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 1.466 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 1.438 2.8
21 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
22 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
23 PC 0.950 0.963 0.975 0.988 1.000
*
24 KK A1 BASIN
25 BA 0.197
26 LG 0.29 0.25 5.34 0.26 5
27 UI 0 39 39 39 50 114 146 182 223 254
28 UI 279 323 344 355 374 378 378 369 357 338
29 UI 302 275 245 222 200 181 164 147 133 120
30 UI 108 94 88 75 74 62 61 53 42 42
31 UI 42 31 27 27 27 27 25 10 10 10
32 UI 10 10 10 10 10 10 10 10 10 10
33 UI 10 10 0 0 0 0 0 0 0 0
*
34 KK R-A1 ROUTE
35 RS 4 FLOW
36 RC 0.060 0.040 0.060 980 0.0224 19.00
37 RX 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
38 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*
39 KK A2 BASIN
40 BA 0.015
41 LG 0.35 0.37 5.24 0.25 0
42 UI 0 10 31 61 85 95 85 63 45 32
43 UI 22 16 11 8 6 2 2 2 2 0
44 UI 0 0 0 0 0 0 0 0 0 0
45 UI 0 0 0 0 0 0 0 0 0 0
46 UI 0 0 0 0 0 0 0 0 0 0
*

```

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
47 KK CP-A2 COMBINE
48 HC 2 .212
*
49 KK B1 BASIN
50 BA 0.011
51 LG 0.35 0.35 4.33 0.40 0

```

52	UI	0	8	25	49	67	72	62	44	31	21
53	UI	15	10	7	5	3	2	2	2	2	0
54	UI	0	0	0	0	0	0	0	0	0	0
55	UI	0	0	0	0	0	0	0	0	0	0
56	UI	0	0	0	0	0	0	0	0	0	0
*											
57	KK	C1	BASIN								
58	BA	0.008									
59	LG	0.35	0.35	4.33	0.40	0					
60	UI	0	6	22	41	54	55	43	29	20	13
61	UI	10	6	4	3	1	1	1	0	0	0
62	UI	0	0	0	0	0	0	0	0	0	0
63	UI	0	0	0	0	0	0	0	0	0	0
64	UI	0	0	0	0	0	0	0	0	0	0
*											
65	KK	D1	BASIN								
66	BA	0.002									
67	LG	0.35	0.35	4.33	0.40	0					
68	UI	0	7	24	24	12	6	3	1	1	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
71	UI	0	0	0	0	0	0	0	0	0	0
72	UI	0	0	0	0	0	0	0	0	0	0
*											
73	KK	CLEAR	COMBINE								
74	HC	3									
*											
75	KK	E1	BASIN								
76	BA	0.022									
77	LG	0.28	0.26	5.05	0.31	4					
78	UI	0	12	23	55	81	101	111	106	88	66
79	UI	51	39	29	23	17	12	9	8	5	3
80	UI	3	3	3	3	0	0	0	0	0	0
81	UI	0	0	0	0	0	0	0	0	0	0
82	UI	0	0	0	0	0	0	0	0	0	0
*											

1

## HEC-1 INPUT

PAGE 3

LINE ID ..... 1 ..... 2 ..... 3 ..... 4 ..... 5 ..... 6 ..... 7 ..... 8 ..... 9 ..... 10

83	KK	E2	BASIN								
84	BA	0.001									
85	LG	0.35	0.36	5.05	0.28	0					
86	UI	0	10	19	7	2	1	0	0	0	0
87	UI	0	0	0	0	0	0	0	0	0	0
88	UI	0	0	0	0	0	0	0	0	0	0
89	UI	0	0	0	0	0	0	0	0	0	0
90	UI	0	0	0	0	0	0	0	0	0	0
*											
91	KK	E3	BASIN								
92	BA	0.038									
93	LG	0.29	0.26	5.05	0.30	4					
94	UI	0	12	12	26	47	66	83	99	110	117
95	UI	118	113	105	88	74	63	54	45	38	32
96	UI	27	23	19	17	13	13	8	8	8	5
97	UI	3	3	3	3	3	3	3	3	3	0
98	UI	0	0	0	0	0	0	0	0	0	0
*											
99	KK	E4	BASIN								
100	BA	0.921									
101	LG	0.28	0.27	5.24	0.28	6					
102	UI	0	84	84	84	84	84	84	84	84	173
103	UI	243	243	297	329	364	405	445	470	511	545
104	UI	580	602	598	694	695	721	747	752	771	764
105	UI	810	810	810	810	810	810	803	777	766	760
106	UI	748	720	694	650	614	589	589	531	511	494
107	UI	463	445	432	399	393	367	360	329	329	299
108	UI	299	274	259	259	235	231	231	198	189	189
109	UI	188	159	159	159	143	130	130	130	130	130
110	UI	128	90	90	90	90	90	90	90	87	58
111	UI	58	58	58	58	58	58	58	58	58	58
112	UI	58	30	21	21	21	21	21	21	21	21
113	UI	21	21	21	21	21	21	21	21	21	21
114	UI	21	21	21	21	21	21	21	21	21	21
115	UI	21	21	21	21	21	0	0	0	0	0
*											
116	KK	CP-E4	COMBINE								
117	HC	4	.982								
*											
118	KK	R-CPE4	ROUTE								
119	RS	1	FLOW								
120	RC	0.060	0.040	0.060	2050	0.0234	2635.00				
121	RK	0.00	33.00	50.00	130.00	345.00	390.00	447.00	530.00		
122	RY	34.00	25.00	18.00	19.00	18.00	20.00	26.00	32.00		
*											

1

## HEC-1 INPUT

PAGE 4

LINE ID ..... 1 ..... 2 ..... 3 ..... 4 ..... 5 ..... 6 ..... 7 ..... 8 ..... 9 ..... 10

123	KK	E5	BASIN								
124	BA	0.063									
125	LG	0.35	0.36	5.14	0.27	0					
126	UI	0	23	23	65	105	143	176	204	217	221
127	UI	209	184	152	126	105	86	71	58	48	41



SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW	(<---) RETURN OF DIVERTED OR PUMPED FLOW
24	A1 V		
34	R-A1 V		
39	A2		
47	CP-A2		
49	B1		
57	C1		
65	D1		
73	CLEAR		
75	E1		
83	E2		
91	E3		
99	E4		
116	CP-E4 V		
118	R-CPE4 V		
123	E5		
131	CP-E5		
133	F1		
141	G1		
149	H1		
157	I1		
165	CLR2		
167	J1		
175	K1		
183	L1		
191	M1		

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION





RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	131.	4.15	9.	2.	2.	.20		
ROUTED TO	R-A1	128.	4.22	9.	2.	2.	.20		
HYDROGRAPH AT	A2	13.	4.03	0.	0.	0.	.01		
2 COMBINED AT	CP-A2	128.	4.22	10.	2.	2.	.21		
HYDROGRAPH AT	B1	8.	4.03	0.	0.	0.	.01		
HYDROGRAPH AT	C1	6.	4.03	0.	0.	0.	.01		
HYDROGRAPH AT	D1	2.	4.02	0.	0.	0.	.00		
3 COMBINED AT	CLEAR	16.	4.03	0.	0.	0.	.02		
HYDROGRAPH AT	E1	21.	4.03	1.	0.	0.	.02		
HYDROGRAPH AT	E2	1.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	E3	31.	4.08	2.	0.	0.	.04		
HYDROGRAPH AT	E4	236.	4.43	33.	8.	6.	.92		
4 COMBINED AT	CP-E4	230.	4.43	34.	9.	6.	.98		
ROUTED TO	R-CPE4	230.	4.43	34.	9.	6.	.98		
HYDROGRAPH AT	E5	44.	4.07	2.	0.	0.	.06		
2 COMBINED AT	CP-E5	230.	4.43	35.	9.	6.	1.04		
HYDROGRAPH AT	F1	1.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	G1	6.	4.03	0.	0.	0.	.01		
HYDROGRAPH AT	H1	5.	4.03	0.	0.	0.	.00		
HYDROGRAPH AT	I1	5.	4.03	0.	0.	0.	.01		
4 COMBINED AT	CLR2	17.	4.02	1.	0.	0.	.02		
HYDROGRAPH AT	J1	3.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	K1	17.	4.05	1.	0.	0.	.02		
HYDROGRAPH AT	L1	3.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	M1	1.	4.02	0.	0.	0.	.00		

\*\*\* NORMAL END OF HEC-1 \*\*\*

**10-year HEC-1 Model**

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 15JUN16 TIME 20:29:28 *
*****

```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 EX - Desert Mountain 19 Existing Condition
3 ID 10 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19EX10.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 2.182 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 2.169 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 2.127 2.8
21 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
22 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
23 PC 0.950 0.963 0.975 0.988 1.000
*
24 KK A1 BASIN
25 BA 0.197
26 LG 0.29 0.25 5.34 0.26 5
27 UI 0 39 39 39 50 114 146 182 223 254
28 UI 279 323 344 355 374 378 378 369 357 338
29 UI 302 275 245 222 200 181 164 147 133 120
30 UI 108 94 88 75 74 62 61 53 42 42
31 UI 42 31 27 27 27 27 25 10 10 10
32 UI 10 10 10 10 10 10 10 10 10 10
33 UI 10 10 0 0 0 0 0 0 0 0
*
34 KK R-A1 ROUTE
35 RS 3 FLOW
36 RC 0.060 0.040 0.060 980 0.0224 19.00
37 RK 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
38 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*
39 KK A2 BASIN
40 BA 0.015
41 LG 0.35 0.37 5.24 0.25 0
42 UI 0 10 31 61 85 95 85 63 45 32
43 UI 22 16 11 8 6 2 2 2 2 0
44 UI 0 0 0 0 0 0 0 0 0 0
45 UI 0 0 0 0 0 0 0 0 0 0
46 UI 0 0 0 0 0 0 0 0 0 0
*

```

1

HEC-1 INPUT

PAGE 2

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
47 KK CP-A2 COMBINE
48 HC 2 .212
*
49 KK B1 BASIN
50 BA 0.011
51 LG 0.35 0.35 4.33 0.40 0

```

52	UI	0	8	25	49	67	72	62	44	31	21
53	UI	15	10	7	5	3	2	2	2	2	0
54	UI	0	0	0	0	0	0	0	0	0	0
55	UI	0	0	0	0	0	0	0	0	0	0
56	UI	0	0	0	0	0	0	0	0	0	0
	*										
57	KK	C1	BASIN								
58	BA	0.008									
59	LG	0.35	0.35	4.33	0.40	0					
60	UI	0	6	22	41	54	55	43	29	20	13
61	UI	10	6	4	3	1	1	1	0	0	0
62	UI	0	0	0	0	0	0	0	0	0	0
63	UI	0	0	0	0	0	0	0	0	0	0
64	UI	0	0	0	0	0	0	0	0	0	0
	*										
65	KK	D1	BASIN								
66	BA	0.002									
67	LG	0.35	0.35	4.33	0.40	0					
68	UI	0	7	24	24	12	6	3	1	1	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
71	UI	0	0	0	0	0	0	0	0	0	0
72	UI	0	0	0	0	0	0	0	0	0	0
	*										
73	KK	CLEAR	COMBINE								
74	HC	3									
	*										
75	KK	E1	BASIN								
76	BA	0.022									
77	LG	0.28	0.26	5.05	0.31	4					
78	UI	0	12	23	55	81	101	111	106	88	66
79	UI	51	39	29	23	17	12	9	8	5	3
80	UI	3	3	3	3	0	0	0	0	0	0
81	UI	0	0	0	0	0	0	0	0	0	0
82	UI	0	0	0	0	0	0	0	0	0	0
	*										

## HEC-1 INPUT

PAGE 3

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

83	KK	E2	BASIN								
84	BA	0.001									
85	LG	0.35	0.36	5.05	0.28	0					
86	UI	0	10	19	7	2	1	0	0	0	0
87	UI	0	0	0	0	0	0	0	0	0	0
88	UI	0	0	0	0	0	0	0	0	0	0
89	UI	0	0	0	0	0	0	0	0	0	0
90	UI	0	0	0	0	0	0	0	0	0	0
	*										
91	KK	E3	BASIN								
92	BA	0.038									
93	LG	0.29	0.26	5.05	0.30	4					
94	UI	0	12	12	26	47	66	83	99	110	117
95	UI	118	113	105	88	74	63	54	45	38	32
96	UI	27	23	19	17	13	13	8	8	8	5
97	UI	3	3	3	3	3	3	3	3	3	0
98	UI	0	0	0	0	0	0	0	0	0	0
	*										
99	KK	E4	BASIN								
100	BA	0.921									
101	LG	0.28	0.27	5.24	0.28	6					
102	UI	0	84	84	84	84	84	84	84	84	173
103	UI	243	243	297	329	364	405	445	470	511	545
104	UI	580	602	598	694	695	721	747	752	771	764
105	UI	810	810	810	810	810	810	803	777	766	760
106	UI	748	720	694	650	614	589	589	531	511	494
107	UI	463	445	432	399	393	367	360	329	329	299
108	UI	299	274	259	259	235	231	231	198	189	189
109	UI	188	159	159	159	159	143	130	130	130	130
110	UI	128	90	90	90	90	90	90	90	87	58
111	UI	58	58	58	58	58	58	58	58	58	58
112	UI	58	30	21	21	21	21	21	21	21	21
113	UI	21	21	21	21	21	21	21	21	21	21
114	UI	21	21	21	21	21	21	21	21	21	21
115	UI	21	21	21	21	21	0	0	0	0	0
	*										
116	KK	CP-E4	COMBINE								
117	HC	4	.982								
	*										
118	KK	R-CPE4	ROUTE								
119	RS	1	FLOW								
120	RC	0.060	0.040	0.060	2050	0.0234	2635.00				
121	RX	0.00	33.00	50.00	130.00	345.00	390.00	447.00	530.00		
122	RY	34.00	25.00	18.00	18.00	18.00	20.00	26.00	32.00		
	*										

## HEC-1 INPUT

PAGE 4

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

123	KK	E5	BASIN								
124	BA	0.063									
125	LG	0.35	0.36	5.14	0.27	0					
126	UI	0	23	23	65	105	143	176	204	217	221
127	UI	209	184	152	126	105	86	71	58	48	41



SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
24	A1 V	
34	R-A1 V	
39	A2	
47	CP-A2	
49	B1	
57	C1	
65	D1	
73	CLEAR	
75	E1	
83	E2	
91	E3	
99	E4	
116	CP-E4	
118	R-CPE4	
123	E5	
131	CP-E5	
133	F1	
141	G1	
149	H1	
157	I1	
165	CLR2	
167	J1	
175	K1	
183	L1	
191	M1	

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION





RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	262.	4.13	19.	5.	3.	.20		
ROUTED TO	R-A1	258.	4.18	19.	5.	3.	.20		
HYDROGRAPH AT	A2	28.	4.02	1.	0.	0.	.01		
2 COMBINED AT	CP-A2	261.	4.18	20.	5.	4.	.21		
HYDROGRAPH AT	B1	19.	4.02	1.	0.	0.	.01		
HYDROGRAPH AT	C1	14.	4.02	1.	0.	0.	.01		
HYDROGRAPH AT	D1	4.	4.02	0.	0.	0.	.00		
3 COMBINED AT	CLEAR	36.	4.02	1.	0.	0.	.02		
HYDROGRAPH AT	E1	42.	4.02	2.	0.	0.	.02		
HYDROGRAPH AT	E2	2.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	E3	63.	4.07	3.	1.	1.	.04		
HYDROGRAPH AT	E4	553.	4.43	78.	19.	14.	.92		
4 COMBINED AT	CP-E4	549.	4.42	82.	20.	15.	.98		
ROUTED TO	R-CPE4	549.	4.43	82.	20.	15.	.98		
HYDROGRAPH AT	E5	102.	4.05	5.	1.	1.	.06		
2 COMBINED AT	CP-E5	549.	4.42	85.	21.	15.	1.04		
HYDROGRAPH AT	F1	2.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	G1	13.	4.02	1.	0.	0.	.01		
HYDROGRAPH AT	H1	10.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	I1	11.	4.02	0.	0.	0.	.01		
4 COMBINED AT	CLR2	36.	4.02	2.	0.	0.	.02		
HYDROGRAPH AT	J1	6.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	K1	36.	4.03	2.	0.	0.	.02		
HYDROGRAPH AT	L1	6.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	M1	2.	4.02	0.	0.	0.	.00		

\*\*\* NORMAL END OF HEC-1 \*\*\*

## **100-year HEC-1 Model**

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 15JUN16 TIME 20:29:36
*
*****

```

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

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X . X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 EX - Desert Mountain 19 Existing Condition
3 ID 100 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19EX100.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 3.313 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 3.293 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 3.230 2.8
21 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
22 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
23 PC 0.950 0.963 0.975 0.988 1.000
*
24 KK A1 BASIN
25 BA 0.197
26 LG 0.29 0.25 5.34 0.26 5
27 UI 0 39 39 39 50 114 146 182 223 254
28 UI 279 323 344 355 374 378 378 369 357 338
29 UI 302 275 245 222 200 181 164 147 133 120
30 UI 108 94 88 75 74 62 61 53 42 42
31 UI 42 31 27 27 27 27 25 10 10 10
32 UI 10 10 10 10 10 10 10 10 10 10
33 UI 10 10 0 0 0 0 0 0 0 0
*
34 KK R-A1 ROUTE
35 RS 3 FLOW
36 RC 0.060 0.040 0.060 980 0.0224 19.00
37 RK 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
38 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*
39 KK A2 BASIN
40 BA 0.015
41 LG 0.35 0.37 5.24 0.25 0
42 UI 0 10 31 61 85 95 85 63 45 32
43 UI 22 16 11 8 6 2 2 2 2 0
44 UI 0 0 0 0 0 0 0 0 0 0
45 UI 0 0 0 0 0 0 0 0 0 0
46 UI 0 0 0 0 0 0 0 0 0 0
*

```

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
47 KK CP-A2 COMBINE
48 HC 2
*
49 KK B1 BASIN
50 BA 0.011
51 LG 0.35 0.35 4.33 0.40 0

```

52	UI	0	8	25	49	67	72	62	44	31	21
53	UI	15	10	7	5	3	2	2	2	2	0
54	UI	0	0	0	0	0	0	0	0	0	0
55	UI	0	0	0	0	0	0	0	0	0	0
56	UI	0	0	0	0	0	0	0	0	0	0
*											
57	KK	C1	BASIN								
58	BA	0.008									
59	LG	0.35	0.35	4.33	0.40	0					
60	UI	0	6	22	41	54	55	43	29	20	13
61	UI	10	6	4	3	1	1	1	0	0	0
62	UI	0	0	0	0	0	0	0	0	0	0
63	UI	0	0	0	0	0	0	0	0	0	0
64	UI	0	0	0	0	0	0	0	0	0	0
*											
65	KK	D1	BASIN								
66	BA	0.002									
67	LG	0.35	0.35	4.33	0.40	0					
68	UI	0	7	24	24	12	6	3	1	1	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
71	UI	0	0	0	0	0	0	0	0	0	0
72	UI	0	0	0	0	0	0	0	0	0	0
*											
73	KK	CLEAR COMBINE									
74	HC	3									
*											
75	KK	E1	BASIN								
76	BA	0.022									
77	LG	0.28	0.26	5.05	0.31	4					
78	UI	0	12	23	55	81	101	111	106	88	66
79	UI	51	39	29	23	17	12	9	8	5	3
80	UI	3	3	3	3	0	0	0	0	0	0
81	UI	0	0	0	0	0	0	0	0	0	0
82	UI	0	0	0	0	0	0	0	0	0	0
*											

## HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

83	KK	E2	BASIN								
84	BA	0.001									
85	LG	0.35	0.36	5.05	0.28	0					
86	UI	0	10	19	7	2	1	0	0	0	0
87	UI	0	0	0	0	0	0	0	0	0	0
88	UI	0	0	0	0	0	0	0	0	0	0
89	UI	0	0	0	0	0	0	0	0	0	0
90	UI	0	0	0	0	0	0	0	0	0	0
*											
91	KK	E3	BASIN								
92	BA	0.038									
93	LG	0.29	0.26	5.05	0.30	4					
94	UI	0	12	12	26	47	66	83	99	110	117
95	UI	118	113	105	88	74	63	54	45	38	32
96	UI	27	23	19	17	13	13	8	8	8	5
97	UI	3	3	3	3	3	3	3	3	3	0
98	UI	0	0	0	0	0	0	0	0	0	0
*											
99	KK	E4	BASIN								
100	BA	0.921									
101	LG	0.28	0.27	5.24	0.28	6					
102	UI	0	84	84	84	84	84	84	84	84	173
103	UI	243	243	297	329	364	405	445	470	511	545
104	UI	580	602	598	694	695	721	747	752	771	764
105	UI	810	810	810	810	810	810	803	777	766	760
106	UI	748	720	694	650	614	589	589	531	511	494
107	UI	463	445	432	399	393	367	360	329	329	299
108	UI	299	274	259	259	235	231	231	198	189	189
109	UI	188	159	159	159	159	143	130	130	130	130
110	UI	128	90	90	90	90	90	90	90	87	58
111	UI	58	58	58	58	58	58	58	58	58	58
112	UI	58	30	21	21	21	21	21	21	21	21
113	UI	21	21	21	21	21	21	21	21	21	21
114	UI	21	21	21	21	21	21	21	21	21	21
115	UI	21	21	21	21	21	0	0	0	0	0
*											
116	KK	CP-E4	COMBINE								
117	HC	4 .982									
*											
118	KK	R-CPE4	ROUTE								
119	RS	FLOW									
120	RC	0.060	0.040	0.060	2050	0.0234	2635.00				
121	RX	0.00	33.00	50.00	130.00	345.00	390.00	447.00	530.00		
122	RY	34.00	25.00	18.00	19.00	18.00	20.00	26.00	32.00		
*											

## HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

123	KK	E5	BASIN								
124	BA	0.063									
125	LG	0.35	0.36	5.14	0.27	0					
126	UI	0	23	23	65	105	143	176	204	217	221
127	UI	209	184	152	126	105	86	71	58	48	41



SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
24	A1	
	V	
	V	
34	R-A1	
	.	
39	A2	
	.	
47	CP-A2	
	.	
49	B1	
	.	
57	C1	
	.	
65	D1	
	.	
73	CLEAR	
	.	
75	E1	
	.	
83	E2	
	.	
91	E3	
	.	
99	E4	
	.	
116	CP-E4	
	V	
	V	
118	R-CPE4	
	.	
123	E5	
	.	
131	CP-E5	
	.	
133	F1	
	.	
141	G1	
	.	
149	H1	
	.	
157	I1	
	.	
165	CLR2	
	.	
167	J1	
	.	
175	K1	
	.	
183	L1	
	.	
191		M1

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION





RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+		A1	468.	4.13	38.	10.	7.	.20	
+	ROUTED TO								
+		R-A1	462.	4.18	38.	10.	7.	.20	
+	HYDROGRAPH AT								
+		A2	50.	4.02	3.	1.	0.	.01	
+	2 COMBINED AT								
+		CP-A2	469.	4.17	41.	10.	7.	.21	
+	HYDROGRAPH AT								
+		B1	35.	4.02	2.	0.	0.	.01	
+	HYDROGRAPH AT								
+		C1	26.	4.02	1.	0.	0.	.01	
+	HYDROGRAPH AT								
+		D1	7.	4.02	0.	0.	0.	.00	
+	3 COMBINED AT								
+		CLEAR	67.	4.02	3.	1.	1.	.02	
+	HYDROGRAPH AT								
+		E1	72.	4.02	4.	1.	1.	.02	
+	HYDROGRAPH AT								
+		E2	3.	4.02	0.	0.	0.	.00	
+	HYDROGRAPH AT								
+		E3	111.	4.07	7.	2.	1.	.04	
+	HYDROGRAPH AT								
+		E4	1111.	4.42	165.	41.	30.	.92	
+	4 COMBINED AT								
+		CP-E4	1115.	4.40	174.	44.	31.	.98	
+	ROUTED TO								
+		R-CPE4	1116.	4.40	174.	44.	31.	.98	
+	HYDROGRAPH AT								
+		E5	186.	4.05	11.	3.	2.	.06	
+	2 COMBINED AT								
+		CP-E5	1124.	4.38	183.	46.	33.	1.04	
+	HYDROGRAPH AT								
+		F1	4.	4.02	0.	0.	0.	.00	
+	HYDROGRAPH AT								
+		G1	23.	4.02	1.	0.	0.	.01	
+	HYDROGRAPH AT								
+		H1	17.	4.02	1.	0.	0.	.00	
+	HYDROGRAPH AT								
+		I1	20.	4.02	1.	0.	0.	.01	
+	4 COMBINED AT								
+		CLR2	63.	4.02	3.	1.	1.	.02	
+	HYDROGRAPH AT								
+		J1	10.	4.02	1.	0.	0.	.00	
+	HYDROGRAPH AT								
+		K1	65.	4.02	3.	1.	1.	.02	
+	HYDROGRAPH AT								
+		L1	10.	4.02	1.	0.	0.	.00	
+	HYDROGRAPH AT								
+		M1	3.	4.02	0.	0.	0.	.00	

\*\*\* NORMAL END OF HEC-1 \*\*\*

## **APPENDIX B**

### **Developed Condition Hydrologic Calculations**

**DDMSW Output Data**

**2-year HEC-1 Model**

**10-year HEC-1 Model**

**100-year HEC-1 Model**

## **DDMSW Output Data**

Flood Control District of Maricopa County  
 Drainage Design Management System  
 SOILS

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
<b>Major Basin ID: 01</b>									
A1	645	33	64533	0.085	43.00	0.230	-	100	
	645	40	64540	0.083	42.20	0.170	-	100	
	645	93	64593	0.023	11.50	0.330	-	100	
	645	96	64596	0.007	3.30	0.070	-	100	
A2	645	93	64593	0.010	69.20	0.330	-	100	
	645	96	64596	0.005	30.80	0.070	-	100	
B1	645	33	64533	0.022	100.00	0.230	-	100	
B10	645	93	64593	0.012	100.00	0.330	-	100	
B11	645	93	64593	0.009	92.70	0.330	-	100	
	645	34	64534	0.001	7.30	0.230	-	100	
B12	645	6	6456	0.002	49.00	0.620	-	100	
	645	93	64593	0.003	51.00	0.330	-	100	
B13	645	34	64534	0.000	3.40	0.230	-	100	
	645	96	64596	0.003	86.20	0.070	-	100	
	645	6	6456	0.000	10.30	0.620	-	100	
B14	645	6	6456	0.009	22.90	0.620	-	100	
	645	96	64596	0.016	41.60	0.070	-	100	
	645	34	64534	0.011	29.60	0.230	-	100	
	645	33	64533	0.002	5.20	0.230	-	100	
	645	93	64593	0.000	0.80	0.330	-	100	
B14A	645	96	64596	0.003	69.00	0.070	-	100	
	645	6	6456	0.001	31.00	0.620	-	100	
B2	645	33	64533	0.001	100.00	0.230	-	100	
B3	645	33	64533	0.038	100.00	0.230	-	100	
B4	645	6	6456	0.043	4.60	0.620	-	100	
	645	33	64533	0.712	77.30	0.230	-	100	
	645	34	64534	0.004	0.50	0.230	-	100	
	645	40	64540	0.014	1.50	0.170	-	100	
	645	63	64563	0.079	8.60	0.140	25.00	100	
	645	96	64596	0.069	7.50	0.070	-	100	
B5	645	34	64534	0.006	100.00	0.230	-	100	
B6	645	6	6456	0.002	20.90	0.620	-	100	
	645	33	64533	0.005	54.70	0.230	-	100	
	645	93	64593	0.002	24.40	0.330	-	100	
B6W	645	93	64593	0.000	3.30	0.330	-	100	
	645	33	64533	0.000	3.30	0.230	-	100	
	645	6	6456	0.003	93.30	0.620	-	100	
B8	645	33	64533	0.008	75.90	0.230	-	100	
	645	93	64593	0.002	22.20	0.330	-	100	
	645	6	6456	0.000	1.90	0.620	-	100	
B9	645	93	64593	0.008	100.00	0.330	-	100	
C1	645	33	64533	0.001	3.10	0.230	-	100	
	645	34	64534	0.022	94.70	0.230	-	100	
	645	96	64596	0.001	2.20	0.070	-	100	
L1	645	33	64533	0.003	100.00	0.230	-	100	
M1	645	33	64533	0.002	100.00	0.230	-	100	

Flood Control District of Maricopa County  
 Drainage Design Management System  
**LAND USE**  
 Project Reference: DM19 PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
<b>Major Basin ID: 01</b>									
A1	901	0.0181	9.2	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.1792	90.8	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		<b>0.1973</b>	<b>100.0</b>						
A2	900	0.0110	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0110</b>	<b>100.0</b>						
B1	900	0.0018	8.3	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.0045	20.7	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.0154	71.0	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		<b>0.0217</b>	<b>100.0</b>						
B10	110	0.0010	8.3	0.30	5	30.0	NORMAL	0.020	Rural Residential (<= 1/5 du per acre)
	150	0.0010	8.3	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	901	0.0100	83.3	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		<b>0.0120</b>	<b>99.9</b>						
B11	150	0.0080	80.0	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0020	20.0	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avai
		<b>0.0100</b>	<b>100.0</b>						
B12	730	0.0050	100.0	0.10	0	90.0	NORMAL	0.030	Passive Open Space (Includes mountain preserves and washes)
		<b>0.0050</b>	<b>100.0</b>						
B13	150	0.0004	13.3	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0015	50.0	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avai
	901	0.0011	36.7	0.20	0	75.0	NORMAL	0.030	Recreational Open Space

\* Non default value

Flood Control District of Maricopa County  
 Drainage Design Management System  
**LAND USE**  
 Project Reference: DM19 PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
<b>Major Basin ID: 01</b>									
		<b>0.0030</b>	<b>100.0</b>						
B14	150	0.0070	17.9	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	230	0.0040	10.3	0.10	80	30.0	NORMAL	0.020	Community Commercial (100,000 to 500,000 sq. ft.)
	600	0.0040	10.3	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
	901	0.0240	61.5	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		<b>0.0390</b>	<b>100.0</b>						
B14A	150	0.0040	80.0	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0010	20.0	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
		<b>0.0050</b>	<b>100.0</b>						
B2	900	0.0012	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0012</b>	<b>100.0</b>						
B3	900	0.0038	10.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.0057	15.0	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.0285	75.0	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)
		<b>0.0380</b>	<b>100.0</b>						
B4	130	0.0074	0.8	0.18	15	35.0	NORMAL	0.040	Large Lot Residential - Single Family (1 du per acre to 2 du
	150	0.0250	2.7	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	220	0.0067	0.7	0.07	80	10.0	NORMAL	0.020	Neighborhood Retail Center
	900	0.1237	13.5	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.2047	22.3	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
	902	0.5446	59.3	0.30	5	30.0	NORMAL	0.040	Rural (1 dwelling unit per acre or less)

\* Non default value

Flood Control District of Maricopa County  
 Drainage Design Management System  
**LAND USE**  
 Project Reference: DM19 PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
<b>Major Basin ID: 01</b>									
B4	903	0.0066	0.7	0.20	0	35.0	NORMAL	0.050	Dedicated Open Space
		<b>0.9187</b>	<b>100.0</b>						
B5	900	0.0010	16.7	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
	901	0.0050	83.3	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		<b>0.0060</b>	<b>100.0</b>						
B6	600	0.0003	3.4	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
	730	0.0001	1.1	0.10	0	90.0	NORMAL	0.030	Passive Open Space (Includes mountain preserves and washes)
	901	0.0083	95.4	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		<b>0.0087</b>	<b>99.9</b>						
B6W	600	0.0002	6.5	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
	730	0.0019	61.3	0.10	0	90.0	NORMAL	0.030	Passive Open Space (Includes mountain preserves and washes)
	901	0.0010	32.3	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		<b>0.0031</b>	<b>100.1</b>						
B8	150	0.0090	81.8	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0020	18.2	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
		<b>0.0110</b>	<b>100.0</b>						
B9	150	0.0060	75.0	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0020	25.0	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail
		<b>0.0080</b>	<b>100.0</b>						
C1	150	0.0060	25.0	0.15	25	30.0	NORMAL	0.040	Small Lot Residential - (2-5 dwelling units per acre)
	600	0.0020	8.3	0.05	95	0.0	DRY	0.015	General Transportation (Transportation where no detail avail

\* Non default value

Flood Control District of Maricopa County  
 Drainage Design Management System  
 LAND USE  
 Project Reference: DM19 PROP

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kn	Description
<b>Major Basin ID: 01</b>									
C1	901	0.0160	66.7	0.20	0	75.0	NORMAL	0.030	Recreational Open Space
		<b>0.0240</b>	<b>100.0</b>						
L1	900	0.0027	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0027</b>	<b>100.0</b>						
M1	900	0.0015	100.0	0.35	0	30.0	DRY	0.040	Vacant (Existing land use database only)
		<b>0.0015</b>	<b>100.0</b>						

\* Non default value

Flood Control District of Maricopa County  
 Drainage Design Management System  
 SUB BASINS

Project Reference: DM19 PROP

Area ID	Sub Basin Parameters								Rainfall Losses				
	Area (sq mi)	Length (mi)	Slope (ft/mi)	S-Graph	Lca (mi)	Lag (min)	Velocity (f/s)	Kn	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)
<b>Major Basin ID: 01</b>													
B14A	0.004	0.04	394.7	VALLEY	0.02	1.10	3.17	0.035	0.13	0.26	6.16	0.160	39
A1	0.197	0.95	134.1	DESERT/RANGE	0.51	16.80	4.96	0.039	0.29	0.25	5.34	0.258	5
B6W	0.003	0.17	126.5	VALLEY	0.08	3.20	4.53	0.029	0.13	0.28	3.37	1.041	6
A2	0.011	0.19	118.3	DESERT/RANGE	0.10	5.10	3.20	0.040	0.35	0.37	5.24	0.250	
B5	0.006	0.11	136.4	VALLEY	0.06	2.70	3.60	0.032	0.23	0.27	5.05	0.377	
B6	0.009	0.11	133.9	VALLEY	0.06	2.50	4.01	0.029	0.19	0.25	4.45	0.524	3
B8	0.010	0.18	61.5	VALLEY	0.09	4.80	3.28	0.035	0.13	0.27	4.87	0.295	38
B1	0.022	0.31	184.7	DESERT/RANGE	0.15	6.40	4.35	0.038	0.28	0.26	5.05	0.306	4
B9	0.008	0.08	144.6	VALLEY	0.04	2.20	3.36	0.034	0.13	0.28	4.33	0.376	43
B2	0.001	0.05	297.9	DESERT/RANGE	0.02	1.40	3.00	0.040	0.35	0.36	5.05	0.281	
B10	0.012	0.22	113.6	VALLEY	0.11	4.30	4.53	0.030	0.20	0.25	4.33	0.541	2
B3	0.038	0.57	146.3	DESERT/RANGE	0.25	10.40	4.85	0.039	0.29	0.26	5.05	0.299	4
B11	0.010	0.18	107.3	VALLEY	0.09	4.30	3.62	0.035	0.13	0.27	4.39	0.374	39
B4	0.921	3.33	259.5	DESERT/RANGE	1.69	36.70	7.99	0.038	0.28	0.27	5.24	0.281	6
B12	0.005	0.13	150.4	DESERT/RANGE	0.07	2.80	4.15	0.030	0.10	0.25	3.79	0.851	
B13	0.003	0.05	408.2	VALLEY	0.02	.80	5.44	0.024	0.12	0.24	7.27	0.114	51
B14	0.038	0.33	121.2	VALLEY	0.16	5.50	5.29	0.029	0.17	0.26	5.58	0.266	23
C1	0.023	0.30	82.5	VALLEY	0.15	6.00	4.47	0.031	0.18	0.26	5.05	0.344	14
L1	0.003	0.12	243.7	DESERT/RANGE	0.05	2.90	3.62	0.040	0.35	0.36	5.05	0.281	
M1	0.001	0.10	204.1	DESERT/RANGE	0.04	2.60	3.38	0.040	0.35	0.36	5.05	0.281	

\* Non default value

**2-year HEC-1 Model**

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 15JUN16 TIME 20:59:37 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 PROP - Desert Mountain 19 Post Online Det Basins 1st Flush
3 ID 2 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19FT2.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 1.475 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 1.466 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 1.456 1.0
21 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
22 PC 0.087 0.099 0.119 0.148 0.230 0.407 0.778 0.881 0.919 0.945
23 PC 0.957 0.968 0.980 0.990 1.000
24 JD 1.438 2.8
25 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
26 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
27 PC 0.950 0.963 0.975 0.988 1.000
*
28 KK A1 BASIN
29 BA 0.197
30 LG 0.29 0.25 5.34 0.26 5
31 UI 0 39 39 50 114 146 182 223 254
32 UI 279 323 344 355 374 378 378 369 357 338
33 UI 302 275 245 222 200 181 164 147 133 120
34 UI 108 94 88 75 74 62 61 53 42 42
35 UI 42 31 27 27 27 25 10 10 10
36 UI 10 10 10 10 10 10 10 10 10 10
37 UI 10 10 0 0 0 0 0 0 0 0
*
38 KK R-A1 ROUTE
39 RS 4 FLOW
40 RC 0.060 0.040 0.060 980 0.0224 19.00
41 RX 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
42 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*

```

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
43 KK A2 BASIN
44 BA 0.011
45 LG 0.35 0.37 5.24 0.25 0
46 UI 0 7 22 45 62 70 63 46 33 23
47 UI 16 12 8 6 5 2 2 2 2 0
48 UI 0 0 0 0 0 0 0 0 0 0
49 UI 0 0 0 0 0 0 0 0 0 0
50 UI 0 0 0 0 0 0 0 0 0 0
*
51 KK CP-A2 COMBINE
52 HC 2
*

```

53	KK	B1	BASIN								
54	BA	0.022									
55	LG	0.28	0.26	5.05	0.31	4					
56	UI	0	12	23	55	81	101	111	106	88	66
57	UI	51	39	29	23	17	12	9	8	5	3
58	UI	3	3	3	3	0	0	0	0	0	0
59	UI	0	0	0	0	0	0	0	0	0	0
60	UI	0	0	0	0	0	0	0	0	0	0
	*										

61	KK	R-B1	ROUTE								
62	KK	500	0.0200	0.013		CIRC	3.500				
	*										

63	KK	B2	BASIN								
64	BA	0.001									
65	LG	0.35	0.36	5.05	0.28	0					
66	UI	0	10	19	7	2	1	0	0	0	0
67	UI	0	0	0	0	0	0	0	0	0	0
68	UI	0	0	0	0	0	0	0	0	0	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
	*										

71	KK	B3	BASIN								
72	BA	0.038									
73	LG	0.29	0.26	5.05	0.30	4					
74	UI	0	12	12	26	47	66	83	99	110	117
75	UI	118	113	105	88	74	63	54	45	38	32
76	UI	27	23	19	17	13	13	8	8	8	5
77	UI	3	3	3	3	3	3	3	3	3	0
78	UI	0	0	0	0	0	0	0	0	0	0
	*										

79	KK	CP-B3	COMBINE								
80	HC	2									
	*										

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

81	KK	R-CPB3	ROUTE								
82	RS	2	FLOW								
83	RC	0.060	0.040	0.060	300	0.0100	10.00				
84	RK	0.00	7.50	15.00	18.00	21.00	23.00	30.50	38.00		
85	RY	10.00	7.50	5.00	5.00	5.00	5.00	7.50	10.00		
	*										

86	KK	B4	BASIN								
87	BA	0.921									
88	LG	0.28	0.27	5.24	0.28	6					
89	UI	0	84	84	84	84	84	84	84	84	173
90	UI	243	243	297	329	364	405	445	470	511	545
91	UI	580	602	598	694	695	721	747	752	771	764
92	UI	810	810	810	810	810	810	803	777	766	760
93	UI	748	720	694	650	614	589	589	531	511	494
94	UI	463	445	432	399	393	367	360	329	329	299
95	UI	299	274	259	259	235	231	231	198	189	189
96	UI	188	159	159	159	159	143	130	130	130	130
97	UI	128	90	90	90	90	90	90	90	87	58
98	UI	58	58	58	58	58	58	58	58	58	58
99	UI	58	30	21	21	21	21	21	21	21	21
100	UI	21	21	21	21	21	21	21	21	21	21
101	UI	21	21	21	21	21	21	21	21	21	21
102	UI	21	21	21	21	21	0	0	0	0	0
	*										

103	KK	DVOC	DIVERT								
104	DT	DTOC	0.00	1050.0							
105	DI	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
106	DQ	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

107	KK	DETOC	STORAGE								
108	KO										
109	RS	1	STOR								
110	SV		0.26	0.59	0.99	1.48					
111	SQ		4.00	9.00	12.00	15.00					
112	SE		1.00	2.00	3.00	4.00					
	*										

113	KK	DVOC	RETRIEVE								
114	DR		DTOC								
	*										

115	KK	B4C	COMBINE								
116	HC	2									
	*										

117	KK	DVOD	DIVERT								
118	DT	DTOC	0.00	975.0							
119	DI	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
120	DQ	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

121	KK	DETOC	STORAGE								
122	KO										
123	RS	1	STOR								
124	SV		0.42	0.89	1.42	2.01					

125 SQ 4.00 9.00 12.00 15.00  
 126 SE 1.00 2.00 3.00 4.00  
 \*  
 127 KK DVODRETRIEVE  
 128 DR DTOD  
 \*  
 129 KK B4D COMBINE  
 130 HC 2  
 \*  
 131 KK CP-B4 COMBINE  
 132 HC 2  
 \*  
 133 KK R-CPB4 ROUTE  
 134 RS 1 FLOW  
 135 RC 0.060 0.040 0.060 300 0.0300 2631.00  
 136 RX 0.00 18.90 43.80 54.80 81.50 91.60 120.00 160.00  
 137 RY 2635.0 2630.00 2629.00 2625.00 2625.00 2629.00 2630.00 2631.00  
 \*

138 KK CP-B5 COMBINE  
 139 HC 2  
 \*  
 140 KK R-CPB5 ROUTE  
 141 RS 2 FLOW  
 142 RC 0.060 0.040 0.060 650 0.0300 2631.00  
 143 RX 0.00 18.90 43.80 54.80 81.50 91.60 120.00 160.00  
 144 RY 2635.0 2630.00 2629.00 2625.00 2625.00 2629.00 2630.00 2631.00  
 \*

145 KK B6 BASIN  
 146 BA 0.009  
 147 LG 0.19 0.25 4.45 0.52 3  
 148 UI 0 26 78 129 74 27 9 4 0 0  
 149 UI 0 0 0 0 0 0 0 0 0 0  
 150 UI 0 0 0 0 0 0 0 0 0 0  
 151 UI 0 0 0 0 0 0 0 0 0 0  
 152 UI 0 0 0 0 0 0 0 0 0 0  
 \*

153 KK B8 BASIN  
 154 BA 0.010  
 155 LG 0.13 0.27 4.87 0.30 38  
 156 UI 0 7 25 39 55 83 63 46 31 15  
 157 UI 10 6 2 2 2 0 0 0 0 0  
 158 UI 0 0 0 0 0 0 0 0 0 0  
 HEC-1 INPUT

1  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10  
 159 UI 0 0 0 0 0 0 0 0 0 0  
 160 UI 0 0 0 0 0 0 0 0 0 0  
 \*

161 KK CP68 COMBINE  
 162 HC 2  
 \*  
 163 KK DET68 STORAGE  
 164 KO  
 165 RS 1 STOR  
 166 SV 0.09 0.21 0.37 0.58  
 167 SQ 1.00 1.00 2.00 2.00  
 168 SE 2608.0 2609.00 2610.00 2611.00 2612.00  
 169 SS 2612.0 20.00 2.80 1.50  
 \*

170 KK B6W BASIN  
 171 BA 0.003  
 172 LG 0.13 0.28 3.37 1.04 6  
 173 UI 0 5 16 29 32 19 9 4 1 1  
 174 UI 0 0 0 0 0 0 0 0 0 0  
 175 UI 0 0 0 0 0 0 0 0 0 0  
 176 UI 0 0 0 0 0 0 0 0 0 0  
 177 UI 0 0 0 0 0 0 0 0 0 0  
 \*

178 KK CP-6W COMBINE  
 179 HC 3  
 \*  
 180 KK DV-B7 DIVERT  
 181 DT DT-B7 0.00 0.0  
 182 DI 0.0 150.0 300.0 450.0 600.0 750.0 900.0 1050.0 1200.0 0.0  
 183 DQ 0.0 4.0 9.0 12.0 15.0 18.0 20.0 22.0 24.0 0.0  
 \*

184 KK DV-B7RETRIEVE  
 185 DR DT-B7  
 \*  
 186 KK B14A BASIN  
 187 BA 0.004  
 188 LG 0.13 0.26 6.16 0.16 39  
 189 UI 0 62 83 9 0 0 0 0 0 0  
 190 UI 0 0 0 0 0 0 0 0 0 0  
 191 UI 0 0 0 0 0 0 0 0 0 0  
 192 UI 0 0 0 0 0 0 0 0 0 0  
 193 UI 0 0 0 0 0 0 0 0 0 0  
 \*

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

194 KK CP-V7 COMBINE  
 195 HC 2  
 \*

196 KK DETEV7 STORAGE  
 197 KO  
 198 RS 1 STOR  
 199 SV 0.22 0.47 0.77 1.10  
 200 SQ 1.00 1.00 2.00 50.00  
 201 SE 2606.0 2607.00 2608.00 2609.00 2610.00  
 \*

202 KK B9 BASIN  
 203 BA 0.008  
 204 LG 0.13 0.28 4.33 0.38 43  
 205 UI 0 31 93 116 50 15 4 0 0 0  
 206 UI 0 0 0 0 0 0 0 0 0 0  
 207 UI 0 0 0 0 0 0 0 0 0 0  
 208 UI 0 0 0 0 0 0 0 0 0 0  
 209 UI 0 0 0 0 0 0 0 0 0 0  
 \*

210 KK DV-B9 DIVERT  
 211 DT DT-B9 0.21 0.0  
 212 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 213 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

214 KK CP-B9 COMBINE  
 215 HC 3  
 \*

216 KK R-CPB9 ROUTE  
 217 RS 1 FLOW  
 218 RC 0.060 0.040 0.060 750 0.0300 2605.00  
 219 RX 0.00 55.30 123.80 129.00 141.80 146.00 155.20 188.60  
 220 RY 2608.0 2600.00 2598.00 2596.00 2596.00 2597.80 2598.00 2605.00  
 \*

221 KK B10 BASIN  
 222 BA 0.012  
 223 LG 0.20 0.25 4.33 0.54 2  
 224 UI 0 10 39 58 90 102 69 47 23 13  
 225 UI 7 3 3 0 0 0 0 0 0 0  
 226 UI 0 0 0 0 0 0 0 0 0 0  
 227 UI 0 0 0 0 0 0 0 0 0 0  
 228 UI 0 0 0 0 0 0 0 0 0 0  
 \*

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

229 KK DV-B10 DIVERT  
 230 DT DT-B10 0.32 0.0  
 231 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 232 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

233 KK B12 BASIN  
 234 BA 0.005  
 235 LG 0.10 0.25 3.79 0.85 0  
 236 UI 0 11 41 56 40 22 12 6 3 1  
 237 UI 1 0 0 0 0 0 0 0 0 0  
 238 UI 0 0 0 0 0 0 0 0 0 0  
 239 UI 0 0 0 0 0 0 0 0 0 0  
 240 UI 0 0 0 0 0 0 0 0 0 0  
 \*

241 KK B14 BASIN  
 242 BA 0.038  
 243 LG 0.17 0.26 5.58 0.27 23  
 244 UI 0 23 67 119 155 222 274 198 149 108  
 245 UI 58 39 24 12 7 7 7 0 0 0  
 246 UI 0 0 0 0 0 0 0 0 0 0  
 247 UI 0 0 0 0 0 0 0 0 0 0  
 248 UI 0 0 0 0 0 0 0 0 0 0  
 \*

249 KK DV-B14 DIVERT  
 250 DT DT-B14 1.03 0.0  
 251 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 252 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

253 KK CP-B14 COMBINE  
 254 HC 4  
 \*

255 KK C1 BASIN  
 256 BA 0.023  
 257 LG 0.18 0.26 5.05 0.34 14  
 258 UI 0 13 33 61 79 104 155 128 99 76  
 259 UI 55 29 22 13 8 4 4 4 0 0  
 260 UI 0 0 0 0 0 0 0 0 0 0  
 261 UI 0 0 0 0 0 0 0 0 0 0  
 262 UI 0 0 0 0 0 0 0 0 0 0  
 \*

263 KK DV-C1 DIVERT  
 264 DT DT-C1 0.61 0.0

265 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 266 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

267 KK DVC-K DIVERT  
 268 DT DTC-K 0.00 1  
 269 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 270 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

271 KK DVC-H DIVERT  
 272 DT DTC-H 0.00 4  
 273 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 274 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

275 KK B13 BASIN  
 276 BA 0.003  
 277 LG 0.12 0.24 7.27 0.11 51  
 278 UI 0 82 33 0 0 0 0 0 0  
 279 UI 0 0 0 0 0 0 0 0 0  
 280 UI 0 0 0 0 0 0 0 0 0  
 281 UI 0 0 0 0 0 0 0 0 0  
 282 UI 0 0 0 0 0 0 0 0 0  
 \*

283 KK DV-B13 DIVERT  
 284 DT DT-B13 0.08 0.0  
 285 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 286 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

287 KK CPB13 COMBINE  
 288 HC 2  
 \*

289 KK DVGF DIVERT  
 290 DT DVGF 0.00 1  
 291 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 292 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

293 KK R-B13 ROUTE  
 294 RK 450 0.0090 0.013 CIRC 4.000  
 \*

295 KK CPGAL COMBINE  
 296 HC 2  
 \*

297 KK 85 BASIN  
 298 BA 0.006  
 299 LG 0.23 0.27 5.05 0.38 0  
 300 UI 0 15 45 80 54 25 10 2 2 0  
 301 UI 0 0 0 0 0 0 0 0 0 0  
 302 UI 0 0 0 0 0 0 0 0 0 0  
 303 UI 0 0 0 0 0 0 0 0 0 0  
 304 UI 0 0 0 0 0 0 0 0 0 0  
 \*

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

305 KK DV-B5 DIVERT  
 306 DT DT-B5 0.55 0.0  
 307 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 308 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

309 KK B11 BASIN  
 310 BA 0.010  
 311 LG 0.13 0.27 4.39 0.37 39  
 312 UI 0 8 32 48 75 85 57 39 19 11  
 313 UI 6 2 2 0 0 0 0 0 0 0  
 314 UI 0 0 0 0 0 0 0 0 0 0  
 315 UI 0 0 0 0 0 0 0 0 0 0  
 316 UI 0 0 0 0 0 0 0 0 0 0  
 \*

317 KK DV-C11 DIVERT  
 318 DT DT-C11 0.0 5.0  
 319 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 320 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

321 KK DV-B11 DIVERT  
 322 DT DT-B11 0.26 0.0  
 323 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 324 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

325 KK L1 BASIN  
 326 BA 0.003  
 327 LG 0.35 0.36 5.05 0.28 0  
 328 UI 0 6 23 33 25 14 7 4 2 1  
 329 UI 1 0 0 0 0 0 0 0 0 0  
 330 UI 0 0 0 0 0 0 0 0 0 0  
 331 UI 0 0 0 0 0 0 0 0 0 0  
 332 UI 0 0 0 0 0 0 0 0 0 0  
 \*

## SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW	(<---) RETURN OF DIVERTED OR PUMPED FLOW
NO.	(.) CONNECTOR		
28	A1		
	V		
38	R-A1		
43	A2		
51	CP-A2		
53	B1		
	V		
61	R-B1		
63	B2		
71	B3		
79	CP-B3		
	V		
81	R-CPB3		
86	B4		
104		-----> DTOC	
103	DVOC		
	V		
107	DETOC		
114			<----- DTOC
113	DVOC		
115	B4C		
118		-----> DTOD	
117	DVOD		
	V		
121	DETOD		
128			<----- DTOD
127	DVOD		
129	B4D		
131	CP-B4		
	V		
133	R-CPB4		
138	CP-B5		
	V		
140	R-CPB5		
145	B6		
153	B8		
161	CP68		
	V		
163	DET68		
170	B6W		
178	CP-6W		
181		-----> DT-B7	
180	DV-B7		
185			<----- DT-B7
184	DV-B7		

```

186 . . . . . B14A
. . . . .
194 . . . . . CP-V7
. . . . . V
. . . . . V
196 . . . . . DETEV7
. . . . .
202 . . . . . B9
. . . . .
211 . . . . . -----> DT-B9
210 . . . . . DV-B9
. . . . .
214 . . . . . CP-B9
. . . . . V
. . . . . V
216 . . . . . R-CPB9
. . . . .
221 . . . . . B10
. . . . .
230 . . . . . -----> DT-B10
229 . . . . . DV-B10
. . . . .
233 . . . . . B12
. . . . .
241 . . . . . B14
. . . . .
250 . . . . . -----> DT-B14
249 . . . . . DV-B14
. . . . .
253 . . . . . CP-B14
. . . . .
255 . . . . . C1
. . . . .
264 . . . . . -----> DT-C1
263 . . . . . DV-C1
. . . . .
268 . . . . . -----> DTC-K
267 . . . . . DVC-K
. . . . .
272 . . . . . -----> DTC-H
271 . . . . . DVC-H
. . . . .
275 . . . . . B13
. . . . .
284 . . . . . -----> DT-B13
283 . . . . . DV-B13
. . . . .
287 . . . . . CPB13
. . . . .
290 . . . . . -----> DVGf
289 . . . . . DVGf
. . . . . V
. . . . . V
293 . . . . . R-B13
. . . . .
295 . . . . . CPGAL
. . . . .
297 . . . . . B5
. . . . .
306 . . . . . -----> DT-B5
305 . . . . . DV-B5
. . . . .
309 . . . . . B11
. . . . .
318 . . . . . -----> DT-C11
317 . . . . . DV-C11
. . . . .
322 . . . . . -----> DT-B11
321 . . . . . DV-B11
. . . . .
325 . . . . . L1
. . . . .
333 . . . . .

```

M1

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION





.00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
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.00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00

\*\*\* \*\*

```
*****
*          *
107 KK * DETOC * STORAGE
*          *
*****
```

```
108 KO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0. HYDROGRAPH PLOT SCALE
```

\*\*\* \*\*

```
*****
*          *
121 KK * DETOD * STORAGE
*          *
*****
```

```
122 KO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0. HYDROGRAPH PLOT SCALE
```

\*\*\* \*\*

```
*****
*          *
163 KK * DET68 * STORAGE
*          *
*****
```

```
164 KO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0. HYDROGRAPH PLOT SCALE
```

\*\*\* \*\*

```
*****
*          *
196 KK * DETBV7 * STORAGE
*          *
*****
```

```
197 KO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0. HYDROGRAPH PLOT SCALE
```

RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	131.	4.15	9.	2.	2.	.20		
ROUTED TO	R-A1	128.	4.22	9.	2.	2.	.20		
HYDROGRAPH AT	A2	10.	4.03	0.	0.	0.	.01		
2 COMBINED AT	CP-A2	128.	4.22	10.	2.	2.	.21		
HYDROGRAPH AT	B1	21.	4.03	1.	0.	0.	.02		
ROUTED TO	R-B1	21.	4.05	1.	0.	0.	.02		
HYDROGRAPH AT	B2	1.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	B3	31.	4.08	2.	0.	0.	.04		
2 COMBINED AT	CP-B3	31.	4.07	2.	0.	0.	.04		
ROUTED TO	R-CPB3	31.	4.10	2.	0.	0.	.04		
HYDROGRAPH AT	B4	236.	4.43	33.	8.	6.	.92		
DIVERSION TO	DTOC	236.	4.43	33.	8.	6.	.92		
HYDROGRAPH AT	DVOC	0.	.00	0.	0.	0.	.92		
ROUTED TO	DETOC	0.	.00	0.	0.	0.	.92		
HYDROGRAPH AT	DVOC	236.	4.43	33.	8.	6.	.92		
2 COMBINED AT	B4C	236.	4.43	33.	8.	6.	.92		
DIVERSION TO	DTOD	236.	4.43	33.	8.	6.	.92		
HYDROGRAPH AT	DVOD	0.	.00	0.	0.	0.	.92		
ROUTED TO	DETOD	0.	.00	0.	0.	0.	.92		
HYDROGRAPH AT	DVOD	236.	4.43	33.	8.	6.	.92		
2 COMBINED AT	B4D	236.	4.43	33.	8.	6.	.92		
2 COMBINED AT	CP-B4	234.	4.43	34.	8.	6.	.96		
ROUTED TO	R-CPB4	233.	4.43	34.	8.	6.	.96		
2 COMBINED AT	CP-B5	231.	4.43	34.	9.	6.	.98		
ROUTED TO	R-CPB5	230.	4.47	34.	9.	6.	.98		
HYDROGRAPH AT	B6	8.	4.02	0.	0.	0.	.01		
HYDROGRAPH AT	B8	13.	4.02	1.	0.	0.	.01		
2 COMBINED AT	CP68	21.	4.02	1.	0.	0.	.02		
ROUTED TO	DET68	2.	4.03	1.	0.	0.	.02		
HYDROGRAPH AT	B6W	1.	4.03	0.	0.	0.	.00		
3 COMBINED AT	CP-6W	230.	4.47	35.	9.	6.	1.00		
DIVERSION TO	DT-B7	7.	4.47	1.	0.	0.	1.00		

+	HYDROGRAPH AT	DV-B7	223.	4.47	34.	8.	6.	1.00
+	HYDROGRAPH AT	DV-B7	7.	4.47	1.	0.	0.	1.00
+	HYDROGRAPH AT	B14A	6.	4.02	0.	0.	0.	.00
+	2 COMBINED AT	CP-V7	10.	4.45	2.	0.	0.	.00
+	ROUTED TO	DETBV7	2.	5.22	1.	0.	0.	.00
+	HYDROGRAPH AT	B9	11.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-B9	10.	3.92	0.	0.	0.	.01
+	HYDROGRAPH AT	DV-B9	11.	4.02	0.	0.	0.	.01
+	3 COMBINED AT	CP-B9	223.	4.47	34.	9.	6.	1.02
+	ROUTED TO	R-CPB9	222.	4.48	34.	9.	6.	1.02
+	HYDROGRAPH AT	B10	10.	4.03	0.	0.	0.	.01
+	DIVERSION TO	DT-B10	10.	4.03	0.	0.	0.	.01
+	HYDROGRAPH AT	DV-B10	0.	.00	0.	0.	0.	.01
+	HYDROGRAPH AT	B12	3.	4.02	0.	0.	0.	.00
+	HYDROGRAPH AT	B14	47.	4.02	3.	1.	0.	.04
+	DIVERSION TO	DT-B14	47.	4.02	2.	1.	0.	.04
+	HYDROGRAPH AT	DV-B14	37.	4.08	1.	0.	0.	.04
+	4 COMBINED AT	CP-B14	222.	4.48	34.	9.	6.	1.07
+	HYDROGRAPH AT	C1	25.	4.03	1.	0.	0.	.02
+	DIVERSION TO	DT-C1	25.	4.03	1.	0.	0.	.02
+	HYDROGRAPH AT	DV-C1	1.	4.30	0.	0.	0.	.02
+	DIVERSION TO	DTC-K	1.	4.30	0.	0.	0.	.02
+	HYDROGRAPH AT	DVC-K	0.	4.23	0.	0.	0.	.02
+	DIVERSION TO	DTC-H	0.	4.23	0.	0.	0.	.02
+	HYDROGRAPH AT	DVC-H	0.	.00	0.	0.	0.	.02
+	HYDROGRAPH AT	B13	5.	4.02	0.	0.	0.	.00
+	DIVERSION TO	DT-B13	5.	3.85	0.	0.	0.	.00
+	HYDROGRAPH AT	DV-B13	5.	4.02	0.	0.	0.	.00
+	2 COMBINED AT	CPB13	5.	4.02	0.	0.	0.	.03
+	DIVERSION TO	DVGF	1.	3.87	0.	0.	0.	.03
+	HYDROGRAPH AT	DVGF	4.	4.02	0.	0.	0.	.03
+	ROUTED TO	R-B13	4.	4.00	0.	0.	0.	.03
+	2 COMBINED AT	CPGAL	222.	4.48	34.	9.	6.	1.10
+	HYDROGRAPH AT	B5	6.	4.02	0.	0.	0.	.01
+	DIVERSION TO	DT-B5	6.	4.02	0.	0.	0.	.01

+	HYDROGRAPH AT	DV-B5	0.	.00	0.	0.	0.	.01
+	HYDROGRAPH AT	B11	13.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-C11	5.	3.83	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-C11	8.	4.02	0.	0.	0.	.01
+	DIVERSION TO	DT-B11	8.	4.02	0.	0.	0.	.01
+	HYDROGRAPH AT	DV-B11	0.	.00	0.	0.	0.	.01
+	HYDROGRAPH AT	L1	3.	4.02	0.	0.	0.	.00
+	HYDROGRAPH AT	M1	1.	4.02	0.	0.	0.	.00

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO		VOLUME (IN)
							COMPUTATION PEAK (CFS)	INTERVAL TIME TO PEAK (MIN)	
FOR STORM = 1 STORM AREA (SQ MI) = .00									
R-B1	MANE	.31	21.32	242.49	.40	1.00	21.29	243.00	.40
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4734E+00 EXCESS= .0000E+00 OUTFLOW= .4734E+00 BASIN STORAGE= .3981E-14 PERCENT ERROR= .0									
FOR STORM = 2 STORM AREA (SQ MI) = .50									
R-B1	MANE	.31	21.05	242.51	.40	1.00	21.01	243.00	.40
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4667E+00 EXCESS= .0000E+00 OUTFLOW= .4667E+00 BASIN STORAGE= .3958E-14 PERCENT ERROR= .0									
FOR STORM = 3 STORM AREA (SQ MI) = 1.00									
R-B1	MANE	.32	14.69	242.84	.29	1.00	14.68	243.00	.29
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3390E+00 EXCESS= .0000E+00 OUTFLOW= .3390E+00 BASIN STORAGE= .3900E-14 PERCENT ERROR= .0									
FOR STORM = 4 STORM AREA (SQ MI) = 2.80									
R-B1	MANE	.31	5.30	243.35	.13	1.00	5.26	243.00	.13
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1480E+00 EXCESS= .0000E+00 OUTFLOW= .1480E+00 BASIN STORAGE= .3907E-14 PERCENT ERROR= .0									
FOR STORM = 1 STORM AREA (SQ MI) = .00									
R-B13	MANE	.41	3.71	240.86	.04	1.00	3.71	240.00	.04
CONTINUITY SUMMARY (AC-FT) - INFLOW= .5032E-01 EXCESS= .0000E+00 OUTFLOW= .5052E-01 BASIN STORAGE= .7473E-14 PERCENT ERROR= -.4									
FOR STORM = 2 STORM AREA (SQ MI) = .50									
R-B13	MANE	.42	3.68	240.73	.04	1.00	3.67	240.00	.04
CONTINUITY SUMMARY (AC-FT) - INFLOW= .4921E-01 EXCESS= .0000E+00 OUTFLOW= .4924E-01 BASIN STORAGE= .7400E-14 PERCENT ERROR= -.1									
FOR STORM = 3 STORM AREA (SQ MI) = 1.00									
R-B13	MANE	.38	2.69	240.74	.03	1.00	2.69	241.00	.03
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3451E-01 EXCESS= .0000E+00 OUTFLOW= .3471E-01 BASIN STORAGE= .7444E-14 PERCENT ERROR= -.6									
FOR STORM = 4 STORM AREA (SQ MI) = 2.80									
R-B13	MANE	.47	1.24	240.99	.01	1.00	1.24	241.00	.01
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1741E-01 EXCESS= .0000E+00 OUTFLOW= .1761E-01 BASIN STORAGE= .7703E-14 PERCENT ERROR= -1.1									

\*\*\* NORMAL END OF HEC-1 \*\*\*

## **10-year HEC-1 Model**

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 15JUN16 TIME 20:59:46
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

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X X XXXXXXX XXXXX X
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 PROP - Desert Mountain 19 Post Online Det Basins 1st Flush
3 ID 10 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19FT10.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 2.182 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 2.169 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 2.154 1.0
21 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.075
22 PC 0.087 0.099 0.119 0.148 0.230 0.407 0.778 0.881 0.919 0.945
23 PC 0.957 0.968 0.980 0.990 1.000
24 JD 2.127 2.8
25 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
26 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
27 PC 0.950 0.963 0.975 0.988 1.000
*
28 KK A1 BASIN
29 BA 0.197
30 LG 0.29 0.25 5.34 0.26 5
31 UI 0 39 39 50 114 146 182 223 254
32 UI 279 323 344 355 374 378 378 369 357 338
33 UI 302 275 245 222 200 181 164 147 133 120
34 UI 108 94 88 75 74 62 61 53 42 42
35 UI 42 31 27 27 27 25 10 10 10
36 UI 10 10 10 10 10 10 10 10 10 10
37 UI 10 10 0 0 0 0 0 0 0 0
*
38 KK R-A1 ROUTE
39 RS 3 FLOW
40 RC 0.060 0.040 0.060 980 0.0224 19.00
41 RK 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
42 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*

```

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
43 KK A2 BASIN
44 BA 0.011
45 LG 0.35 0.37 5.24 0.25 0
46 UI 0 7 22 45 62 70 63 46 33 23
47 UI 16 12 8 6 5 2 2 2 2 0
48 UI 0 0 0 0 0 0 0 0 0 0
49 UI 0 0 0 0 0 0 0 0 0 0
50 UI 0 0 0 0 0 0 0 0 0 0
*
51 KK CP-A2 COMBINE
52 HC 2
*

```

53	KK	B1	BASIN								
54	BA	0.022									
55	LG	0.28	0.26	5.05	0.31	4					
56	UI	0	12	23	55	81	101	111	106	88	56
57	UI	51	39	29	23	17	12	9	8	5	3
58	UI	3	3	3	3	0	0	0	0	0	0
59	UI	0	0	0	0	0	0	0	0	0	0
60	UI	0	0	0	0	0	0	0	0	0	0
	*										

61	KK	R-B1	ROUTE								
62	RK	500	0.0200	0.013		CIRC	3.500				
	*										

63	KK	B2	BASIN								
64	BA	0.001									
65	LG	0.35	0.36	5.05	0.28	0					
66	UI	0	10	19	7	2	1	0	0	0	0
67	UI	0	0	0	0	0	0	0	0	0	0
68	UI	0	0	0	0	0	0	0	0	0	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
	*										

71	KK	B3	BASIN								
72	BA	0.038									
73	LG	0.29	0.26	5.05	0.30	4					
74	UI	0	12	12	26	47	66	83	99	110	117
75	UI	118	113	105	88	74	63	54	45	38	32
76	UI	27	23	19	17	13	13	8	8	8	5
77	UI	3	3	3	3	3	3	3	3	3	0
78	UI	0	0	0	0	0	0	0	0	0	0
	*										

79	KK	CP-B3	COMBINE								
80	HC	2									
	*										

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

81	KK	R-CPB3	ROUTE								
82	RS	1	FLOW								
83	RC	0.060	0.040	0.060	300	0.0100	10.00				
84	RX	0.00	7.50	15.00	18.00	21.00	23.00	30.50	38.00		
85	RY	10.00	7.50	5.00	5.00	5.00	5.00	7.50	10.00		
	*										

86	KK	B4	BASIN								
87	BA	0.921									
88	LG	0.28	0.27	5.24	0.28	6					
89	UI	0	84	84	84	84	84	84	84	84	173
90	UI	243	243	297	329	364	405	445	470	511	545
91	UI	580	602	598	694	695	721	747	752	771	764
92	UI	810	810	810	810	810	810	803	777	766	760
93	UI	748	720	694	650	614	589	589	531	511	494
94	UI	463	445	432	399	393	367	360	329	329	299
95	UI	299	274	259	259	235	231	231	198	189	189
96	UI	188	159	159	159	159	143	130	130	130	130
97	UI	128	90	90	90	90	90	90	90	87	58
98	UI	58	58	58	58	58	58	58	58	58	58
99	UI	58	30	21	21	21	21	21	21	21	21
100	UI	21	21	21	21	21	21	21	21	21	21
101	UI	21	21	21	21	21	21	21	21	21	21
102	UI	21	21	21	21	21	0	0	0	0	0
	*										

103	KK	DVOC	DIVERT								
104	DT	DTOC	0.00	1050.0							
105	DI	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
106	DQ	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

107	KK	DETOC	STORAGE								
108	KO										
109	RS	1	STOR								
110	SV		0.26	0.59	0.99	1.48					
111	SQ		4.00	9.00	12.00	15.00					
112	SE		1.00	2.00	3.00	4.00					
	*										

113	KK	DVOC	RETRIEVE								
114	DR	DTOC									
	*										

115	KK	B4C	COMBINE								
116	HC	2									
	*										

117	KK	DVOD	DIVERT								
118	DT	DTOD	0.00	975.0							
119	DI	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
120	DQ	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

121	KK	DETOC	STORAGE								
122	KO										
123	RS	1	STOR								
124	SV		0.42	0.89	1.42	2.01					

125	SQ		4.00	9.00	12.00	15.00				
126	SE		1.00	2.00	3.00	4.00				
	*									
127	KK	DVODRETRIEVE								
128	DR	DTOD								
	*									
129	KK	B4D COMBINE								
130	HC	2								
	*									
131	KK	CP-B4 COMBINE								
132	HC	2								
	*									
133	KK	R-CPB4 ROUTE								
134	RS	1 FLOW								
135	RC	0.060 0.040 0.060 300 0.0300 2631.00								
136	RX	0.00 18.90 43.80 54.80 81.50 91.60 120.00 160.00								
137	RY	2635.0 2630.00 2629.00 2625.00 2625.00 2629.00 2630.00 2631.00								
	*									

138	KK	CP-B5 COMBINE								
139	HC	2								
	*									
140	KK	R-CPB5 ROUTE								
141	RS	1 FLOW								
142	RC	0.060 0.040 0.060 650 0.0300 2631.00								
143	RX	0.00 18.90 43.80 54.80 81.50 91.60 120.00 160.00								
144	RY	2635.0 2630.00 2629.00 2625.00 2625.00 2629.00 2630.00 2631.00								
	*									

145	KK	B6 BASIN								
146	BA	0.009								
147	LG	0.19 0.25 4.45 0.52 3								
148	UI	0 26 78 129 74 27 9 4 0 0								
149	UI	0 0 0 0 0 0 0 0 0 0								
150	UI	0 0 0 0 0 0 0 0 0 0								
151	UI	0 0 0 0 0 0 0 0 0 0								
152	UI	0 0 0 0 0 0 0 0 0 0								
	*									

153	KK	B8 BASIN								
154	BA	0.010								
155	LG	0.13 0.27 4.87 0.30 38								
156	UI	0 7 25 39 55 83 63 46 31 15								
157	UI	10 6 2 2 2 0 0 0 0 0								
158	UI	0 0 0 0 0 0 0 0 0 0								
	*									

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
159	UI	0	0	0	0	0	0	0	0	0	0
160	UI	0	0	0	0	0	0	0	0	0	0
	*										

161	KK	CP68 COMBINE									
162	HC	2									
	*										
163	KK	DET68 STORAGE									
164	KO										
165	RS	1 STOR									
166	SV	0.09 0.21 0.37 0.58									
167	SQ	1.00 1.00 2.00 2.00									
168	SE	2608.0 2609.00 2610.00 2611.00 2612.00									
169	SS	2612.0 20.00 2.80 1.50									
	*										

170	KK	B6W BASIN									
171	BA	0.003									
172	LG	0.13 0.28 3.37 1.04 6									
173	UI	0 5 16 29 32 19 9 4 1 1									
174	UI	0 0 0 0 0 0 0 0 0 0									
175	UI	0 0 0 0 0 0 0 0 0 0									
176	UI	0 0 0 0 0 0 0 0 0 0									
177	UI	0 0 0 0 0 0 0 0 0 0									
	*										

178	KK	CP-6W COMBINE									
179	HC	3									
	*										
180	KK	DV-B7 DIVERT									
181	DT	DT-B7 0.00 0.0									
182	DI	0.0 150.0 300.0 450.0 600.0 750.0 900.0 1050.0 1200.0 0.0									
183	DQ	0.0 4.0 9.0 12.0 15.0 18.0 20.0 22.0 24.0 0.0									
	*										

184	KK	DV-B7RETRIEVE									
185	DR	DT-B7									
	*										

186	KK	B14A BASIN									
187	BA	0.004									
188	LG	0.13 0.26 6.16 0.16 39									
189	UI	0 62 83 9 0 0 0 0 0 0									
190	UI	0 0 0 0 0 0 0 0 0 0									
191	UI	0 0 0 0 0 0 0 0 0 0									
192	UI	0 0 0 0 0 0 0 0 0 0									
193	UI	0 0 0 0 0 0 0 0 0 0									
	*										

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

194 KK CP-V7 COMBINE  
 195 HC 2  
 \*

196 KK DETBV7 STORAGE  
 197 KO  
 198 RS 1 STOR  
 199 SV 0.22 0.47 0.77 1.10  
 200 SQ 1.00 1.00 2.00 50.00  
 201 SE 2606.0 2607.00 2608.00 2609.00 2610.00  
 \*

202 KK B9 BASIN  
 203 BA 0.008  
 204 LG 0.13 0.28 4.33 0.38 43  
 205 UI 0 31 93 116 50 15 4 0 0 0  
 206 UI 0 0 0 0 0 0 0 0 0 0  
 207 UI 0 0 0 0 0 0 0 0 0 0  
 208 UI 0 0 0 0 0 0 0 0 0 0  
 209 UI 0 0 0 0 0 0 0 0 0 0  
 \*

210 KK DV-B9 DIVERT  
 211 DT DT-B9 0.21 0.0  
 212 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 213 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

214 KK CP-B9 COMBINE  
 215 HC 3  
 \*

216 KK R-CPB9 ROUTE  
 217 RS 2 FLOW  
 218 RC 0.060 0.040 0.060 750 0.0300 2605.00  
 219 RX 0.00 55.30 123.80 129.00 141.80 146.00 155.20 188.60  
 220 RY 2608.0 2600.00 2598.00 2596.00 2596.00 2597.80 2598.00 2605.00  
 \*

221 KK B10 BASIN  
 222 BA 0.012  
 223 LG 0.20 0.25 4.33 0.54 2  
 224 UI 0 10 39 58 90 102 69 47 23 13  
 225 UI 7 3 3 0 0 0 0 0 0 0  
 226 UI 0 0 0 0 0 0 0 0 0 0  
 227 UI 0 0 0 0 0 0 0 0 0 0  
 228 UI 0 0 0 0 0 0 0 0 0 0  
 \*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

229 KK DV-B10 DIVERT  
 230 DT DT-B10 0.32 0.0  
 231 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 232 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

233 KK B12 BASIN  
 234 BA 0.005  
 235 LG 0.10 0.25 3.79 0.85 0  
 236 UI 0 11 41 56 40 22 12 6 3 1  
 237 UI 1 0 0 0 0 0 0 0 0 0  
 238 UI 0 0 0 0 0 0 0 0 0 0  
 239 UI 0 0 0 0 0 0 0 0 0 0  
 240 UI 0 0 0 0 0 0 0 0 0 0  
 \*

241 KK B14 BASIN  
 242 BA 0.038  
 243 LG 0.17 0.26 5.58 0.27 23  
 244 UI 0 23 67 119 155 222 274 198 149 108  
 245 UI 58 39 24 12 7 7 7 0 0 0  
 246 UI 0 0 0 0 0 0 0 0 0 0  
 247 UI 0 0 0 0 0 0 0 0 0 0  
 248 UI 0 0 0 0 0 0 0 0 0 0  
 \*

249 KK DV-B14 DIVERT  
 250 DT DT-B14 1.03 0.0  
 251 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 252 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

253 KK CP-B14 COMBINE  
 254 HC 4  
 \*

255 KK C1 BASIN  
 256 BA 0.023  
 257 LG 0.18 0.26 5.05 0.34 14  
 258 UI 0 13 33 61 79 104 155 128 99 76  
 259 UI 55 29 22 13 8 4 4 4 0 0  
 260 UI 0 0 0 0 0 0 0 0 0 0  
 261 UI 0 0 0 0 0 0 0 0 0 0  
 262 UI 0 0 0 0 0 0 0 0 0 0  
 \*

263 KK DV-C1 DIVERT  
 264 DT DT-C1 0.61 0.0

265 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 266 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

267 KK DVC-K DIVERT  
 268 DT DTC-K 0.00 18  
 269 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 270 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

271 KK DVC-H DIVERT  
 272 DT DTC-H 0.00 8  
 273 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 274 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

275 KK B13 BASIN  
 276 BA 0.003  
 277 LG 0.12 0.24 7.27 0.11 51  
 278 UI 0 82 33 0 0 0 0 0 0 0  
 279 UI 0 0 0 0 0 0 0 0 0 0 0  
 280 UI 0 0 0 0 0 0 0 0 0 0 0  
 281 UI 0 0 0 0 0 0 0 0 0 0 0  
 282 UI 0 0 0 0 0 0 0 0 0 0 0  
 \*

283 KK DV-B13 DIVERT  
 284 DT DT-B13 0.08 0.0  
 285 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 286 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

287 KK CPB13 COMBINE  
 288 HC 2  
 \*

289 KK DVGF DIVERT  
 290 DT DTGF 0.00 1  
 291 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 292 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

293 KK R-B13 ROUTE  
 294 RK 450 0.0090 0.013 CIRC 4.000  
 \*

295 KK CPGAL COMBINE  
 296 HC 2  
 \*

297 KK B5 BASIN  
 298 BA 0.006  
 299 LG 0.23 0.27 5.05 0.38 0  
 300 UI 0 15 45 80 54 25 10 2 2 0  
 301 UI 0 0 0 0 0 0 0 0 0 0 0  
 302 UI 0 0 0 0 0 0 0 0 0 0 0  
 303 UI 0 0 0 0 0 0 0 0 0 0 0  
 304 UI 0 0 0 0 0 0 0 0 0 0 0  
 \*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

305 KK DV-B5 DIVERT  
 306 DT DT-B5 0.42 0.0  
 307 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 308 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

309 KK B11 BASIN  
 310 BA 0.010  
 311 LG 0.13 0.27 4.39 0.37 39  
 312 UI 0 8 32 48 75 85 57 39 19 11  
 313 UI 6 2 2 0 0 0 0 0 0 0  
 314 UI 0 0 0 0 0 0 0 0 0 0 0  
 315 UI 0 0 0 0 0 0 0 0 0 0 0  
 316 UI 0 0 0 0 0 0 0 0 0 0 0  
 \*

317 KK DV-C11 DIVERT  
 318 DT DT-C11 0.0 5.0  
 319 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 320 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

321 KK DV-B11 DIVERT  
 322 DT DV-B11 0.26 0.0  
 323 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 324 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

325 KK L1 BASIN  
 326 BA 0.003  
 327 LG 0.35 0.36 5.05 0.28 0  
 328 UI 0 6 23 33 25 14 7 4 2 1  
 329 UI 1 0 0 0 0 0 0 0 0 0 0  
 330 UI 0 0 0 0 0 0 0 0 0 0 0  
 331 UI 0 0 0 0 0 0 0 0 0 0 0  
 332 UI 0 0 0 0 0 0 0 0 0 0 0  
 \*



## SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
28	A1 V	
38	R-A1 V	
43	A2	
51	CP-A2	
53	B1 V	
61	R-B1 V	
63	B2	
71	B3	
79	CP-B3	
81	R-CPB3 V	
86	B4	
104		DTOC
103	DVOC	
107	DETOC	
114		DTOC
113	DVOC	
115	B4C	
118		DTOD
117	DVOD	
121	DETOD	
128		DTOD
127	DVOD	
129	B4D	
131	CP-B4	
133	R-CPB4 V	
138	CP-B5	
140	R-CPB5 V	
145	B6	
153	B8	
161	CP68	
163	DET68 V	
170	B6W	
178	CP-6W	
181		DT-B7
180	DV-B7	
185		DT-B7
184	DV-B7	

```

186 . . . . . B14A
. . . . .
194 . . . . . CP-V7
. . . . . V
. . . . . V
196 . . . . . DETEV7
. . . . .
202 . . . . . B9
. . . . .
211 . . . . . -----> DT-B9
210 . . . . . DV-B9
. . . . .
214 . . . . . CP-B9
. . . . . V
. . . . . V
216 . . . . . R-CPB9
. . . . .
221 . . . . . B10
. . . . .
230 . . . . . -----> DT-B10
229 . . . . . DV-B10
. . . . .
233 . . . . . B12
. . . . .
241 . . . . . B14
. . . . .
250 . . . . . -----> DT-B14
249 . . . . . DV-B14
. . . . .
253 . . . . . CP-B14
. . . . .
255 . . . . . C1
. . . . .
264 . . . . . -----> DT-C1
263 . . . . . DV-C1
. . . . .
268 . . . . . -----> DTC-K
267 . . . . . DVC-K
. . . . .
272 . . . . . -----> DTC-H
271 . . . . . DVC-H
. . . . .
275 . . . . . B13
. . . . .
284 . . . . . -----> DT-B13
283 . . . . . DV-B13
. . . . .
287 . . . . . CPB13
. . . . .
290 . . . . . -----> DTGF
289 . . . . . DVGf
. . . . . V
. . . . . V
293 . . . . . R-B13
. . . . .
295 . . . . . CPGAL
. . . . .
297 . . . . . B5
. . . . .
306 . . . . . -----> DT-B5
305 . . . . . DV-B5
. . . . .
309 . . . . . B11
. . . . .
318 . . . . . -----> DT-C11
317 . . . . . DV-C11
. . . . .
322 . . . . . -----> DV-B11
321 . . . . . DV-B11
. . . . .
325 . . . . . L1
. . . . .
333 . . . . .

```

M1

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION





.00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
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\*\*\* \*\*

\*\*\*\*\*  
\* \*  
107 KK \* DETOC \* STORAGE  
\* \*  
\*\*\*\*\*

108 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
121 KK \* DETOD \* STORAGE  
\* \*  
\*\*\*\*\*

122 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
163 KK \* DET68 \* STORAGE  
\* \*  
\*\*\*\*\*

164 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
196 KK \* DETEV7 \* STORAGE  
\* \*  
\*\*\*\*\*

197 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	252.	4.13	19.	5.	3.	.20		
ROUTED TO	R-A1	258.	4.18	19.	5.	3.	.20		
HYDROGRAPH AT	A2	21.	4.02	1.	0.	0.	.01		
2 COMBINED AT	CP-A2	260.	4.18	20.	5.	4.	.21		
HYDROGRAPH AT	B1	42.	4.02	2.	0.	0.	.02		
ROUTED TO	R-B1	42.	4.03	2.	0.	0.	.02		
HYDROGRAPH AT	B2	2.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	B3	63.	4.07	3.	1.	1.	.04		
2 COMBINED AT	CP-B3	64.	4.07	3.	1.	1.	.04		
ROUTED TO	R-CPB3	63.	4.08	3.	1.	1.	.04		
HYDROGRAPH AT	B4	546.	4.43	76.	19.	14.	.92		
DIVERSION TO	DTOC	546.	4.43	76.	19.	14.	.92		
HYDROGRAPH AT	DVOC	0.	.00	0.	0.	0.	.92		
ROUTED TO	DETOC	0.	.00	0.	0.	0.	.92		
HYDROGRAPH AT	DVOC	546.	4.43	76.	19.	14.	.92		
2 COMBINED AT	B4C	546.	4.43	76.	19.	14.	.92		
DIVERSION TO	DTOD	546.	4.43	76.	19.	14.	.92		
HYDROGRAPH AT	DVOD	0.	.00	0.	0.	0.	.92		
ROUTED TO	DETOD	0.	.00	0.	0.	0.	.92		
HYDROGRAPH AT	DVOD	546.	4.43	76.	19.	14.	.92		
2 COMBINED AT	B4D	546.	4.43	76.	19.	14.	.92		
2 COMBINED AT	CP-B4	546.	4.42	79.	20.	14.	.96		
ROUTED TO	R-CPB4	546.	4.43	79.	20.	14.	.96		
2 COMBINED AT	CP-B5	543.	4.43	80.	20.	14.	.98		
ROUTED TO	R-CPB5	543.	4.43	80.	20.	14.	.98		
HYDROGRAPH AT	B6	16.	4.02	1.	0.	0.	.01		
HYDROGRAPH AT	B8	22.	4.02	1.	0.	0.	.01		
2 COMBINED AT	CP68	39.	4.02	2.	1.	0.	.02		
ROUTED TO	DET68	2.	3.92	2.	1.	0.	.02		
HYDROGRAPH AT	B6W	4.	4.02	0.	0.	0.	.00		
3 COMBINED AT	CP-6W	543.	4.43	81.	20.	15.	1.00		
DIVERSION TO	DT-B7	14.	4.43	2.	1.	0.	1.00		

+	HYDROGRAPH AT	DV-B7	527.	4.43	79.	20.	14.	1.00
+	HYDROGRAPH AT	DV-B7	14.	4.43	2.	1.	0.	1.00
+	HYDROGRAPH AT	B14A	9.	4.02	1.	0.	0.	.00
+	2 COMBINED AT	CP-V7	16.	4.02	3.	1.	1.	.00
+	ROUTED TO	DETEV7	13.	4.65	2.	1.	1.	.00
+	HYDROGRAPH AT	B9	18.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-B9	16.	3.83	0.	0.	0.	.01
+	HYDROGRAPH AT	DV-B9	18.	4.02	1.	0.	0.	.01
+	3 COMBINED AT	CP-B9	528.	4.43	81.	21.	15.	1.02
+	ROUTED TO	R-CPB9	527.	4.47	81.	21.	15.	1.02
+	HYDROGRAPH AT	B10	21.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-B10	21.	4.00	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-B10	21.	4.03	0.	0.	0.	.01
+	HYDROGRAPH AT	B12	8.	4.02	0.	0.	0.	.00
+	HYDROGRAPH AT	B14	82.	4.02	5.	1.	1.	.04
+	DIVERSION TO	DT-B14	77.	3.93	2.	1.	0.	.04
+	HYDROGRAPH AT	DV-B14	82.	4.02	3.	1.	0.	.04
+	4 COMBINED AT	CP-B14	527.	4.47	83.	21.	15.	1.07
+	HYDROGRAPH AT	C1	47.	4.02	2.	1.	0.	.02
+	DIVERSION TO	DT-C1	45.	3.97	1.	0.	0.	.02
+	HYDROGRAPH AT	DV-C1	47.	4.02	1.	0.	0.	.02
+	DIVERSION TO	DTC-K	18.	4.00	1.	0.	0.	.02
+	HYDROGRAPH AT	DVC-K	29.	4.02	0.	0.	0.	.02
+	DIVERSION TO	DTC-H	8.	4.00	0.	0.	0.	.02
+	HYDROGRAPH AT	DVC-H	21.	4.02	0.	0.	0.	.02
+	HYDROGRAPH AT	B13	7.	4.02	1.	0.	0.	.00
+	DIVERSION TO	DT-B13	2.	3.75	0.	0.	0.	.00
+	HYDROGRAPH AT	DV-B13	7.	4.02	0.	0.	0.	.00
+	2 COMBINED AT	CPB13	28.	4.02	1.	0.	0.	.03
+	DIVERSION TO	DTGF	1.	3.78	0.	0.	0.	.03
+	HYDROGRAPH AT	DVGF	27.	4.02	1.	0.	0.	.03
+	ROUTED TO	R-B13	27.	4.02	1.	0.	0.	.03
+	2 COMBINED AT	CPGAL	527.	4.47	83.	21.	15.	1.10
+	HYDROGRAPH AT	B5	11.	4.02	0.	0.	0.	.01
+	DIVERSION TO	DT-B5	11.	4.02	0.	0.	0.	.01

+	HYDROGRAPH AT	DV-B5	0.	.00	0.	0.	0.	.01
+	HYDROGRAPH AT	B11	22.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-C11	5.	3.80	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-C11	17.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DV-B11	17.	4.02	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-B11	15.	4.05	0.	0.	0.	.01
+	HYDROGRAPH AT	L1	6.	4.02	0.	0.	0.	.00
+	HYDROGRAPH AT	M1	2.	4.02	0.	0.	0.	.00

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO COMPUTATION INTERVAL		VOLUME (IN)
							PEAK (CFS)	TIME TO PEAK (MIN)	
FOR STORM = 1	STORM AREA (SQ MI) =			.00					
R-B1	MANE	.17	42.15	241.37	.84	1.00	42.13	242.00	.84
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9822E+00 EXCESS= .0000E+00 OUTFLOW= .9825E+00 BASIN STORAGE= .3951E-14 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =			.50					
R-B1	MANE	.18	41.78	241.51	.83	1.00	41.77	242.00	.83
CONTINUITY SUMMARY (AC-FT) - INFLOW= .9720E+00 EXCESS= .0000E+00 OUTFLOW= .9723E+00 BASIN STORAGE= .3979E-14 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =			1.00					
R-B1	MANE	.17	31.78	241.47	.70	1.00	31.77	242.00	.70
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8268E+00 EXCESS= .0000E+00 OUTFLOW= .8269E+00 BASIN STORAGE= .3940E-14 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =			2.80					
R-B1	MANE	.27	16.95	242.35	.57	1.00	16.94	242.00	.57
CONTINUITY SUMMARY (AC-FT) - INFLOW= .6687E+00 EXCESS= .0000E+00 OUTFLOW= .6687E+00 BASIN STORAGE= .3984E-14 PERCENT ERROR= .0									
FOR STORM = 1	STORM AREA (SQ MI) =			.00					
R-B13	MANE	.30	26.83	241.14	.21	1.00	26.82	241.00	.21
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2869E+00 EXCESS= .0000E+00 OUTFLOW= .2870E+00 BASIN STORAGE= .7598E-14 PERCENT ERROR= -.1									
FOR STORM = 2	STORM AREA (SQ MI) =			.50					
R-B13	MANE	.37	26.43	241.00	.20	1.00	26.43	241.00	.20
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2821E+00 EXCESS= .0000E+00 OUTFLOW= .2826E+00 BASIN STORAGE= .7502E-14 PERCENT ERROR= -.2									
FOR STORM = 3	STORM AREA (SQ MI) =			1.00					
R-B13	MANE	.34	13.15	241.76	.11	1.00	12.37	242.00	.11
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1582E+00 EXCESS= .0000E+00 OUTFLOW= .1584E+00 BASIN STORAGE= .7731E-14 PERCENT ERROR= -.2									
FOR STORM = 4	STORM AREA (SQ MI) =			2.80					
R-B13	MANE	.44	2.57	240.86	.06	1.00	2.57	240.00	.06
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8541E-01 EXCESS= .0000E+00 OUTFLOW= .8564E-01 BASIN STORAGE= .7698E-14 PERCENT ERROR= -.3									

\*\*\* NORMAL END OF HEC-1 \*\*\*

## **100-year HEC-1 Model**

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 15JUN16 TIME 20:59:52
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

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X X X X X X
X X X X X X
X X XXXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID DM19 PROP - Desert Mountain 19 Post Online Det Basins 1st Flush
3 ID 100 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: S-Graph
6 ID Storm: Multiple
7 ID 06/14/2016
8 ID WOOD/PATEL FILE NAME: DM19FT100.DAT
*DIAGRAM
9 IT 1 1JAN99 0 2000
10 IO 5
11 IN 15
*
12 JD 3.313 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
14 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
15 PC 0.962 0.972 0.983 0.991 1.000
16 JD 3.293 0.5000
17 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
18 PC 0.087 0.099 0.118 0.138 0.216 0.377 0.834 0.911 0.931 0.950
19 PC 0.962 0.972 0.983 0.991 1.000
20 JD 3.270 1.0
21 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074
22 PC 0.087 0.099 0.119 0.148 0.230 0.407 0.778 0.881 0.919 0.945
23 PC 0.957 0.968 0.980 0.990 1.000
24 JD 3.230 2.8
25 PC 0.000 0.009 0.016 0.025 0.034 0.042 0.051 0.059 0.067 0.076
26 PC 0.087 0.100 0.120 0.163 0.252 0.451 0.694 0.837 0.900 0.938
27 PC 0.950 0.963 0.975 0.988 1.000
*
28 KK A1 BASIN
29 BA 0.197
30 LG 0.29 0.25 5.34 0.26 5
31 UI 0 39 39 39 50 114 146 182 223 254
32 UI 279 323 344 355 374 378 378 369 357 338
33 UI 302 275 245 222 200 181 164 147 133 120
34 UI 108 94 88 75 74 62 61 53 42 42
35 UI 42 31 27 27 27 27 25 10 10 10
36 UI 10 10 10 10 10 10 10 10 10 10
37 UI 10 10 0 0 0 0 0 0 0 0
*
38 KK R-A1 ROUTE
39 RE 3 FLOW
40 RC 0.060 0.040 0.060 980 0.0224 19.00
41 RX 0.00 42.00 73.00 106.00 196.00 225.00 251.00 295.00
42 RY 20.00 15.00 10.00 7.00 6.00 5.00 13.00 19.00
*

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
43 KK A2 BASIN
44 BA 0.011
45 LG 0.35 0.37 5.24 0.25 0
46 UI 0 7 22 45 62 70 63 46 33 23
47 UI 16 12 8 6 5 2 2 2 2 0
48 UI 0 0 0 0 0 0 0 0 0 0
49 UI 0 0 0 0 0 0 0 0 0 0
50 UI 0 0 0 0 0 0 0 0 0 0
*
51 KK CP-A2 COMBINE
52 HC 2
*

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53	KK	B1	BASIN								
54	BA	0.022									
55	LG	0.28	0.26	5.05	0.31	4					
56	UI	0	12	23	55	81	101	111	106	88	66
57	UI	51	39	29	23	17	12	9	8	5	3
58	UI	3	3	3	3	0	0	0	0	0	0
59	UI	0	0	0	0	0	0	0	0	0	0
60	UI	0	0	0	0	0	0	0	0	0	0
	*										

61	KK	R-B1	ROUTE								
62	RK	500	0.0200	0.013		CIRC	3.500				
	*										

63	KK	B2	BASIN								
64	BA	0.001									
65	LG	0.35	0.36	5.05	0.28	0					
66	UI	0	10	19	7	2	1	0	0	0	0
67	UI	0	0	0	0	0	0	0	0	0	0
68	UI	0	0	0	0	0	0	0	0	0	0
69	UI	0	0	0	0	0	0	0	0	0	0
70	UI	0	0	0	0	0	0	0	0	0	0
	*										

71	KK	B3	BASIN								
72	BA	0.038									
73	LG	0.29	0.26	5.05	0.30	4					
74	UI	0	12	12	26	47	66	83	99	110	117
75	UI	118	113	105	88	74	63	54	45	38	32
76	UI	27	23	19	17	13	13	8	8	8	5
77	UI	3	3	3	3	3	3	3	3	3	0
78	UI	0	0	0	0	0	0	0	0	0	0
	*										

79	KK	CP-B3	COMBINE								
80	HC	2									
	*										

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

81	KK	R-CPB3	ROUTE								
82	RS	1	FLOW								
83	RC	0.060	0.040	0.060	300	0.0100	10.00				
84	RK	0.00	7.50	15.00	18.00	21.00	23.00	30.50	38.00		
85	RY	10.00	7.50	5.00	5.00	5.00	5.00	7.50	10.00		
	*										

86	KK	B4	BASIN								
87	BA	0.921									
88	LG	0.28	0.27	5.24	0.28	6					
89	UI	0	84	84	84	84	84	84	84	84	173
90	UI	243	243	297	329	364	405	445	470	511	545
91	UI	580	602	598	694	695	721	747	752	771	764
92	UI	810	810	810	810	810	810	803	777	766	760
93	UI	748	720	694	650	614	589	589	531	511	494
94	UI	463	445	432	399	393	367	360	329	329	299
95	UI	299	274	259	259	235	231	231	198	189	189
96	UI	188	159	159	159	159	143	130	130	130	130
97	UI	128	90	90	90	90	90	90	90	87	58
98	UI	58	58	58	58	58	58	58	58	58	58
99	UI	58	30	21	21	21	21	21	21	21	21
100	UI	21	21	21	21	21	21	21	21	21	21
101	UI	21	21	21	21	21	21	21	21	21	21
102	UI	21	21	21	21	21	0	0	0	0	0
	*										

103	KK	DVOC	DIVERT								
104	DT	DTOC	0.00	1050.0							
105	DI	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
106	DQ	0.0	2000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

107	KK	DETOC	STORAGE								
108	KO										
109	RS	1	STOR								
110	SV		0.26	0.59	0.99	1.48					
111	SQ		4.00	9.00	12.00	15.00					
112	SE		1.00	2.00	3.00	4.00					
	*										

113	KK	DVOC	RETRIEVE								
114	DR	DTOC									
	*										

115	KK	B4C	COMBINE								
116	HC	2									
	*										

117	KK	DVOD	DIVERT								
118	DT	DTOD	0.00	975.0							
119	DI	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
120	DQ	0.0	20000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	*										

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

121	KK	DETOC	STORAGE								
122	KO										
123	RS	1	STOR								
124	SV		0.42	0.89	1.42	2.01					

125	SQ		4.00	9.00	12.00	15.00					
126	SE		1.00	2.00	3.00	4.00					
	*										
127	KK	DVODRETRIEVE									
128	DR	DTOD									
	*										
129	KK	B4D COMBINE									
130	HC	2									
	*										
131	KK	CP-B4 COMBINE									
132	HC	2									
	*										
133	KK	R-CPB4 ROUTE									
134	RS	1 FLOW									
135	RC	0.060 0.040 0.060 300 0.0300 2631.00									
136	RX	0.00 18.90 43.80 54.80 81.50 91.60 120.00 160.00									
137	RY	2635.0 2630.00 2629.00 2625.00 2625.00 2629.00 2630.00 2631.00									
	*										
138	KK	CP-B5 COMBINE									
139	HC	2									
	*										
140	KK	R-CPB5 ROUTE									
141	RS	1 FLOW									
142	RC	0.060 0.040 0.060 650 0.0300 2631.00									
143	RX	0.00 18.90 43.80 54.80 81.50 91.60 120.00 160.00									
144	RY	2635.0 2630.00 2629.00 2625.00 2625.00 2629.00 2630.00 2631.00									
	*										
145	KK	B6 BASIN									
146	BA	0.009									
147	LG	0.19 0.25 4.45 0.52 3									
148	UI	0 26 78 129 74 27 9 4 0 0									
149	UI	0 0 0 0 0 0 0 0 0 0									
150	UI	0 0 0 0 0 0 0 0 0 0									
151	UI	0 0 0 0 0 0 0 0 0 0									
152	UI	0 0 0 0 0 0 0 0 0 0									
	*										
153	KK	B8 BASIN									
154	BA	0.010									
155	LG	0.13 0.27 4.87 0.30 38									
156	UI	0 7 25 39 55 83 63 46 31 15									
157	UI	10 6 2 2 2 0 0 0 0 0									
158	UI	0 0 0 0 0 0 0 0 0 0									
	*										

HEC-1 INPUT

PAGE 5

LINE	ID	1	2	3	4	5	6	7	8	9	10
159	UI	0	0	0	0	0	0	0	0	0	0
160	UI	0	0	0	0	0	0	0	0	0	0
	*										
161	KK	CP68 COMBINE									
162	HC	2									
	*										
163	KK	DET68 STORAGE									
164	KO										
165	RS	1 STOR									
166	SV	0.09 0.21 0.37 0.58									
167	SQ	1.00 1.00 2.00 2.00									
168	SE	2608.0 2609.00 2610.00 2611.00 2612.00									
169	SS	2612.0 20.00 2.80 1.50									
	*										
170	KK	B6W BASIN									
171	BA	0.003									
172	LG	0.13 0.28 3.37 1.04 6									
173	UI	0 5 16 29 32 19 9 4 1 1									
174	UI	0 0 0 0 0 0 0 0 0 0									
175	UI	0 0 0 0 0 0 0 0 0 0									
176	UI	0 0 0 0 0 0 0 0 0 0									
177	UI	0 0 0 0 0 0 0 0 0 0									
	*										
178	KK	CP-6W COMBINE									
179	HC	3									
	*										
180	KK	DV-B7 DIVERT									
181	DT	DT-B7 0.00 0.0									
182	DI	0.0 150.0 300.0 450.0 600.0 750.0 900.0 1050.0 1200.0 0.0									
183	DQ	0.0 4.0 9.0 12.0 15.0 18.0 20.0 22.0 24.0 0.0									
	*										
184	KK	DV-B7RETRIEVE									
185	DR	DT-B7									
	*										
186	KK	B14A BASIN									
187	BA	0.004									
188	LG	0.13 0.26 6.16 0.16 39									
189	UI	0 62 83 9 0 0 0 0 0 0									
190	UI	0 0 0 0 0 0 0 0 0 0									
191	UI	0 0 0 0 0 0 0 0 0 0									
192	UI	0 0 0 0 0 0 0 0 0 0									
193	UI	0 0 0 0 0 0 0 0 0 0									
	*										

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

194 KK CP-V7 COMBINE  
 195 HC 2  
 \*

196 KK DETV7 STORAGE  
 197 KO  
 198 RS 1 STOR  
 199 SV 0.22 0.47 0.77 1.10  
 200 SQ 1.00 1.00 2.00 50.00  
 201 SE 2606.0 2607.00 2608.00 2609.00 2610.00  
 \*

202 KK B9 BASIN  
 203 BA 0.008  
 204 LG 0.13 0.28 4.33 0.38 43  
 205 UI 0 31 93 116 50 15 4 0 0 0  
 206 UI 0 0 0 0 0 0 0 0 0 0  
 207 UI 0 0 0 0 0 0 0 0 0 0  
 208 UI 0 0 0 0 0 0 0 0 0 0  
 209 UI 0 0 0 0 0 0 0 0 0 0  
 \*

210 KK DV-B9 DIVERT  
 211 DT DT-B9 0.21 0.0  
 212 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 213 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

214 KK CP-B9 COMBINE  
 215 HC 3  
 \*

216 KK R-CPB9 ROUTE  
 217 RS 1 FLOW  
 218 RC 0.060 0.040 0.060 750 0.0300 2605.00  
 219 RK 0.00 55.30 123.80 129.00 141.80 146.00 155.20 188.60  
 220 RY 2608.0 2600.00 2598.00 2596.00 2596.00 2597.80 2598.00 2605.00  
 \*

221 KK B10 BASIN  
 222 BA 0.012  
 223 LG 0.20 0.25 4.33 0.54 2  
 224 UI 0 10 39 58 90 102 69 47 23 13  
 225 UI 7 3 3 0 0 0 0 0 0 0  
 226 UI 0 0 0 0 0 0 0 0 0 0  
 227 UI 0 0 0 0 0 0 0 0 0 0  
 228 UI 0 0 0 0 0 0 0 0 0 0  
 \*

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

229 KK DT-B10 DIVERT  
 230 DT DT-B10 0.32 0.0  
 231 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 232 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

233 KK B12 BASIN  
 234 BA 0.005  
 235 LG 0.10 0.25 3.79 0.85 0  
 236 UI 0 11 41 56 40 22 12 6 3 1  
 237 UI 1 0 0 0 0 0 0 0 0 0  
 238 UI 0 0 0 0 0 0 0 0 0 0  
 239 UI 0 0 0 0 0 0 0 0 0 0  
 240 UI 0 0 0 0 0 0 0 0 0 0  
 \*

241 KK B14 BASIN  
 242 BA 0.038  
 243 LG 0.17 0.26 5.58 0.27 23  
 244 UI 0 23 67 119 155 222 274 198 149 108  
 245 UI 58 39 24 12 7 7 7 0 0 0  
 246 UI 0 0 0 0 0 0 0 0 0 0  
 247 UI 0 0 0 0 0 0 0 0 0 0  
 248 UI 0 0 0 0 0 0 0 0 0 0  
 \*

249 KK DV-B14 DIVERT  
 250 DT DT-B14 1.03 0.0  
 251 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 252 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

253 KK CP-B14 COMBINE  
 254 HC 4  
 \*

255 KK C1 BASIN  
 256 BA 0.023  
 257 LG 0.18 0.26 5.05 0.34 14  
 258 UI 0 13 33 61 79 104 155 128 99 76  
 259 UI 55 29 22 13 8 4 4 4 0 0  
 260 UI 0 0 0 0 0 0 0 0 0 0  
 261 UI 0 0 0 0 0 0 0 0 0 0  
 262 UI 0 0 0 0 0 0 0 0 0 0  
 \*

263 KK DV-C1 DIVERT  
 264 DT DT-C1 0.61 0.0

265 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 266 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

267 KK DVC-K DIVERT  
 268 DT DTC-K 0.00 50  
 269 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 270 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

271 KK DVC-H DIVERT  
 272 DT DTC-H 0.00 15  
 273 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 274 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

275 KK B13 BASIN  
 276 BA 0.003  
 277 LG 0.12 0.24 7.27 0.11 51  
 278 UI 0 82 33 0 0 0 0 0 0  
 279 UI 0 0 0 0 0 0 0 0 0 0 0  
 280 UI 0 0 0 0 0 0 0 0 0 0 0  
 281 UI 0 0 0 0 0 0 0 0 0 0 0  
 282 UI 0 0 0 0 0 0 0 0 0 0 0  
 \*

283 KK DV-B13 DIVERT  
 284 DT DT-B13 0.08 0.0  
 285 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 286 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

287 KK CPB13 COMBINE  
 288 HC 2  
 \*

289 KK DVGF DIVERT  
 290 DT DTGF 0.00 3  
 291 DI 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 292 DQ 0.0 1000 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

293 KK R-B13 ROUTE  
 294 RK 450 0.0090 0.013 CIRC 4.000  
 \*

295 KK CPGAL COMBINE  
 296 HC 2  
 \*

297 KK B5 BASIN  
 298 BA 0.006  
 299 LG 0.23 0.27 5.05 0.38 0  
 300 UI 0 15 45 80 54 25 10 2 2 0  
 301 UI 0 0 0 0 0 0 0 0 0 0 0  
 302 UI 0 0 0 0 0 0 0 0 0 0 0  
 303 UI 0 0 0 0 0 0 0 0 0 0 0  
 304 UI 0 0 0 0 0 0 0 0 0 0 0  
 \*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

305 KK DV-B5 DIVERT  
 306 DT DT-B5 0.42 0.0  
 307 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 308 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

309 KK B11 BASIN  
 310 BA 0.010  
 311 LG 0.13 0.27 4.39 0.37 39  
 312 UI 0 8 32 48 75 85 57 39 19 11  
 313 UI 6 2 2 0 0 0 0 0 0 0  
 314 UI 0 0 0 0 0 0 0 0 0 0  
 315 UI 0 0 0 0 0 0 0 0 0 0  
 316 UI 0 0 0 0 0 0 0 0 0 0  
 \*

317 KK DV-C11 DIVERT  
 318 DT DT-C11 0.0 5.0  
 319 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 320 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

321 KK DV-B11 DIVERT  
 322 DT DT-B11 0.26 0.0  
 323 DI 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 324 DQ 0.0 1000.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 \*

325 KK L1 BASIN  
 326 BA 0.003  
 327 LG 0.35 0.36 5.05 0.28 0  
 328 UI 0 6 23 33 25 14 7 4 2 1  
 329 UI 1 0 0 0 0 0 0 0 0 0  
 330 UI 0 0 0 0 0 0 0 0 0 0  
 331 UI 0 0 0 0 0 0 0 0 0 0  
 332 UI 0 0 0 0 0 0 0 0 0 0  
 \*



SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
28	A1 V	
38	R-A1 V	
43	A2	
51	CP-A2	
53	B1 V	
61	R-B1 V	
63	B2	
71	B3	
79	CP-B3	
81	R-CPB3 V	
86	B4	
104		DTOC
103	DVOC	
107	DETOC	
114		DTOC
113	DVOC	
115	B4C	
118		DTOD
117	DVOD	
121	DETOC	
128		DTOD
127	DVOD	
129	B4D	
131	CP-B4	
133	R-CPB4 V	
138	CP-B5	
140	R-CPB5 V	
145	B6	
153	B8	
161	CP68	
163	DET68 V	
170	B6W	
178	CP-6W	
181		DT-B7
180	DV-B7	
185		DT-B7
184	DV-B7	

```

186 . . . . . B14A
. . . . .
194 . . . . . CP-V7
. . . . . V
. . . . . V
196 . . . . . DETBV7
. . . . .
202 . . . . . B9
. . . . .
211 . . . . . -----> DT-B9
210 . . . . . DV-B9
. . . . .
214 . . . . . CP-B9
. . . . . V
. . . . . V
216 . . . . . R-CPB9
. . . . .
221 . . . . . B10
. . . . .
230 . . . . . -----> DT-B10
229 . . . . . DT-B10
. . . . .
233 . . . . . B12
. . . . .
241 . . . . . B14
. . . . .
250 . . . . . -----> DT-B14
249 . . . . . DV-B14
. . . . .
253 . . . . . CP-B14
. . . . .
255 . . . . . C1
. . . . .
264 . . . . . -----> DT-C1
263 . . . . . DV-C1
. . . . .
268 . . . . . -----> DTC-K
267 . . . . . DVC-K
. . . . .
272 . . . . . -----> DTC-H
271 . . . . . DVC-H
. . . . .
275 . . . . . B13
. . . . .
284 . . . . . -----> DT-B13
283 . . . . . DV-B13
. . . . .
287 . . . . . CPB13
. . . . .
290 . . . . . -----> DTGF
289 . . . . . DVGF
. . . . . V
. . . . . V
293 . . . . . R-B13
. . . . .
295 . . . . . CPGAL
. . . . .
297 . . . . . B5
. . . . .
306 . . . . . -----> DT-B5
305 . . . . . DV-B5
. . . . .
309 . . . . . B11
. . . . .
318 . . . . . -----> DT-C11
317 . . . . . DV-C11
. . . . .
322 . . . . . -----> DT-B11
321 . . . . . DV-B11
. . . . .
325 . . . . . L1
. . . . .
333 . . . . .

```

M1

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION





.00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
.00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
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\*\*\*\*\*  
\* \*  
107 KK \* DETOC \* STORAGE  
\* \*  
\*\*\*\*\*

108 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

\*\*\*\*\*

\*\*\*\*\*  
\* \*  
121 KK \* DETOD \* STORAGE  
\* \*  
\*\*\*\*\*

122 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

\*\*\*\*\*

\*\*\*\*\*  
\* \*  
163 KK \* DET68 \* STORAGE  
\* \*  
\*\*\*\*\*

164 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

\*\*\*\*\*

\*\*\*\*\*  
\* \*  
196 KK \* DETEV7 \* STORAGE  
\* \*  
\*\*\*\*\*

197 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	A1	468.	4.13	38.	10.	7.	.20		
ROUTED TO	R-A1	462.	4.18	38.	10.	7.	.20		
HYDROGRAPH AT	A2	36.	4.02	2.	0.	0.	.01		
2 COMBINED AT	CP-A2	467.	4.18	40.	10.	7.	.21		
HYDROGRAPH AT	B1	72.	4.02	4.	1.	1.	.02		
ROUTED TO	R-B1	72.	4.03	4.	1.	1.	.02		
HYDROGRAPH AT	B2	3.	4.02	0.	0.	0.	.00		
HYDROGRAPH AT	B3	111.	4.07	7.	2.	1.	.04		
2 COMBINED AT	CP-B3	111.	4.05	7.	2.	1.	.04		
ROUTED TO	R-CPB3	111.	4.07	7.	2.	1.	.04		
HYDROGRAPH AT	B4	1110.	4.42	163.	41.	29.	.92		
DIVERSION TO	DTOC	1050.	4.33	161.	40.	29.	.92		
HYDROGRAPH AT	DVOC	60.	4.42	2.	0.	0.	.92		
ROUTED TO	DETOC	8.	4.52	2.	0.	0.	.92		
HYDROGRAPH AT	DVOC	1050.	4.33	161.	40.	29.	.92		
2 COMBINED AT	B4C	1058.	4.50	163.	41.	29.	.92		
DIVERSION TO	DTOD	975.	4.28	159.	40.	29.	.92		
HYDROGRAPH AT	DVOD	83.	4.50	3.	1.	1.	.92		
ROUTED TO	DETOD	12.	4.58	3.	1.	1.	.92		
HYDROGRAPH AT	DVOD	975.	4.28	159.	40.	29.	.92		
2 COMBINED AT	B4D	987.	4.57	162.	41.	29.	.92		
2 COMBINED AT	CP-B4	1012.	4.27	168.	42.	30.	.96		
ROUTED TO	R-CPB4	1013.	4.28	168.	42.	30.	.96		
2 COMBINED AT	CP-B5	1024.	4.28	172.	43.	31.	.98		
ROUTED TO	R-CPB5	1021.	4.30	172.	43.	31.	.98		
HYDROGRAPH AT	B6	29.	4.02	1.	0.	0.	.01		
HYDROGRAPH AT	B8	36.	4.02	2.	1.	0.	.01		
2 COMBINED AT	CP68	65.	4.02	4.	1.	1.	.02		
ROUTED TO	DET68	2.	3.83	2.	1.	1.	.02		
HYDROGRAPH AT	B6W	9.	4.02	0.	0.	0.	.00		
3 COMBINED AT	CP-6W	1022.	4.30	173.	44.	32.	1.00		
DIVERSION TO	DT-B7	22.	4.30	4.	1.	1.	1.00		

+	HYDROGRAPH AT	DV-B7	1001.	4.30	169.	43.	31.	1.00
+	HYDROGRAPH AT	DV-B7	22.	4.30	4.	1.	1.	1.00
+	HYDROGRAPH AT	B14A	15.	4.02	1.	0.	0.	.00
+	2 COMBINED AT	CP-V7	29.	4.02	5.	1.	1.	.00
+	ROUTED TO	DETEV7	22.	4.53	5.	1.	1.	.00
+	HYDROGRAPH AT	B9	29.	4.02	2.	1.	0.	.01
+	DIVERSION TO	DT-B9	8.	3.67	0.	0.	0.	.01
+	HYDROGRAPH AT	DV-B9	29.	4.02	2.	0.	0.	.01
+	3 COMBINED AT	CP-B9	1022.	4.30	174.	45.	32.	1.02
+	ROUTED TO	R-CPB9	1016.	4.33	174.	45.	32.	1.02
+	HYDROGRAPH AT	B10	38.	4.02	2.	0.	0.	.01
+	DIVERSION TO	DT-B10	35.	3.88	1.	0.	0.	.01
+	HYDROGRAPH AT	DT-B10	38.	4.02	1.	0.	0.	.01
+	HYDROGRAPH AT	B12	15.	4.02	1.	0.	0.	.00
+	HYDROGRAPH AT	B14	134.	4.02	9.	2.	2.	.04
+	DIVERSION TO	DT-B14	50.	3.82	2.	1.	0.	.04
+	HYDROGRAPH AT	DV-B14	134.	4.02	6.	2.	1.	.04
+	4 COMBINED AT	CP-B14	1027.	4.32	181.	46.	33.	1.07
+	HYDROGRAPH AT	C1	78.	4.02	5.	1.	1.	.02
+	DIVERSION TO	DT-C1	48.	3.87	1.	0.	0.	.02
+	HYDROGRAPH AT	DV-C1	78.	4.02	3.	1.	1.	.02
+	DIVERSION TO	DTC-K	50.	3.88	3.	1.	0.	.02
+	HYDROGRAPH AT	DVC-K	28.	4.02	1.	0.	0.	.02
+	DIVERSION TO	DTC-H	15.	3.92	1.	0.	0.	.02
+	HYDROGRAPH AT	DVC-H	13.	4.02	0.	0.	0.	.02
+	HYDROGRAPH AT	B13	11.	4.02	1.	0.	0.	.00
+	DIVERSION TO	DT-B13	4.	3.55	0.	0.	0.	.00
+	HYDROGRAPH AT	DV-B13	11.	4.02	1.	0.	0.	.00
+	2 COMBINED AT	CPB13	24.	4.02	1.	0.	0.	.03
+	DIVERSION TO	DTGF	3.	3.58	0.	0.	0.	.03
+	HYDROGRAPH AT	DVGF	21.	4.02	1.	0.	0.	.03
+	ROUTED TO	R-B13	21.	4.02	1.	0.	0.	.03
+	2 COMBINED AT	CPGAL	1025.	4.32	181.	46.	33.	1.10
+	HYDROGRAPH AT	B5	20.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-B5	20.	4.02	1.	0.	0.	.01

+	HYDROGRAPH AT	DV-B5	17.	4.03	0.	0.	0.	.01
+	HYDROGRAPH AT	B11	35.	4.02	2.	1.	0.	.01
+	DIVERSION TO	DT-C11	5.	3.58	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-C11	30.	4.02	1.	0.	0.	.01
+	DIVERSION TO	DT-B11	28.	3.90	1.	0.	0.	.01
+	HYDROGRAPH AT	DV-B11	30.	4.02	1.	0.	0.	.01
+	HYDROGRAPH AT	L1	10.	4.02	1.	0.	0.	.00
+	HYDROGRAPH AT	M1	3.	4.02	0.	0.	0.	.00

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

I STAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	DT (MIN)	INTERPOLATED TO		VOLUME (IN)
							COMPUTATION PEAK (CFS)	INTERVAL TIME TO PEAK (MIN)	
FOR STORM = 1	STORM AREA (SQ MI) =			.00					
R-B1	MANE	.30	72.25	241.40	1.72	1.00	72.12	242.00	1.72
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2021E+01 EXCESS= .0000E+00 OUTFLOW= .2021E+01 BASIN STORAGE= .3970E-14 PERCENT ERROR= .0									
FOR STORM = 2	STORM AREA (SQ MI) =			.50					
R-B1	MANE	.18	71.78	241.39	1.71	1.00	71.60	242.00	1.71
CONTINUITY SUMMARY (AC-FT) - INFLOW= .2002E+01 EXCESS= .0000E+00 OUTFLOW= .2003E+01 BASIN STORAGE= .3981E-14 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =			1.00					
R-B1	MANE	.17	56.32	241.36	1.57	1.00	56.19	242.00	1.57
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1841E+01 EXCESS= .0000E+00 OUTFLOW= .1842E+01 BASIN STORAGE= .3970E-14 PERCENT ERROR= .0									
FOR STORM = 4	STORM AREA (SQ MI) =			2.80					
R-B1	MANE	.17	33.58	241.35	1.40	1.00	33.53	242.00	1.40
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1647E+01 EXCESS= .0000E+00 OUTFLOW= .1647E+01 BASIN STORAGE= .3963E-14 PERCENT ERROR= .0									
FOR STORM = 1	STORM AREA (SQ MI) =			.00					
R-B13	MANE	.35	21.50	240.86	.23	1.00	21.47	241.00	.23
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3152E+00 EXCESS= .0000E+00 OUTFLOW= .3155E+00 BASIN STORAGE= .7448E-14 PERCENT ERROR= -.1									
FOR STORM = 2	STORM AREA (SQ MI) =			.50					
R-B13	MANE	.39	20.87	240.84	.22	1.00	20.79	241.00	.22
CONTINUITY SUMMARY (AC-FT) - INFLOW= .3053E+00 EXCESS= .0000E+00 OUTFLOW= .3054E+00 BASIN STORAGE= .7516E-14 PERCENT ERROR= .0									
FOR STORM = 3	STORM AREA (SQ MI) =			1.00					
R-B13	MANE	.31	5.94	240.94	.10	1.00	5.94	241.00	.10
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1403E+00 EXCESS= .0000E+00 OUTFLOW= .1408E+00 BASIN STORAGE= .7520E-14 PERCENT ERROR= -.4									
FOR STORM = 4	STORM AREA (SQ MI) =			2.80					
R-B13	MANE	.45	2.67	240.59	.06	1.00	2.67	240.00	.06
CONTINUITY SUMMARY (AC-FT) - INFLOW= .8898E-01 EXCESS= .0000E+00 OUTFLOW= .8918E-01 BASIN STORAGE= .7557E-14 PERCENT ERROR= -.2									

\*\*\* NORMAL END OF HEC-1 \*\*\*

## **APPENDIX C**

### **Site Retention Calculations**

#### **Required First Flush Storm Water Calculations**

#### **Provided Storm Water Calculations**

**Required First Flush Storm Water Calculations**

# WOOD/PATEL

CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

## First Flush Volume Required

**Description:** Desert Mountain Parcel 19  
**Date:** 05/06/16  
**Location:** City of Scottsdale  
**Reference:** City of Scottsdale, *Design Standards and Policies Manual Chapter 4 Grading & Drainage*, January 2010.

**Known Values:** First Flush  
0.5 inches

**Calc. Values:**  $V=(P/12)*A*C$

V = Retention Volume Required  
C = Runoff Coefficient  
P = Precipitation amount=100-year 2 hour rainfall  
A = Area of Watershed Contributing

Drainage Area	Area (SF)	Area (ac)	Weighted Runoff Coefficient	First Flush Volume (ac-ft)	Required Pre vs Post Volume (ac ft)
B5	165024	3.79	1.00	0.16	0.42
B6	322261	5.50	1.00	0.23	N/A
B8	300366	6.90	1.00	0.29	N/A
B9	217378	4.99	1.00	0.21	N/A
B10	335363	7.70	1.00	0.32	N/A
B11	269364	6.18	1.00	0.26	N/A
B13	80916	1.86	1.00	0.08	N/A
B14	1072718	24.63	1.00	1.03	N/A
B14A	115650	2.65	1.00	0.11	N/A
C1	636403	14.61	1.00	0.61	N/A
<b>TOTAL =</b>				<b>3.28</b>	

## **Provided Storm Water Calculations**

**Off-Site Detention Basin Capacity**

Description: Proposed Detention Basin Capacities  
 Project: **Desert Mountain Parcel 19**  
 Reference: *Drainage Design Manual for Maricopa County, Arizona - Hydrology*  
 City of Scottsdale, *Design Standards and Policies Manual*

Basin IDs	Bottom Contour Area	Top Contour	Bottom Elevation	Top Elevation	Volume Provided	Volume Provided
<b>Basin 3</b>	17,184	19,439	0.0	1	<b>0.42</b>	<b>2.01</b>
	19,439	21,796	1.0	2	<b>0.47</b>	
	21,796	24,252	2.0	3	<b>0.53</b>	
	24,252	26,810.00	3.0	4	<b>0.59</b>	

Basin IDs	Bottom Contour Area	Top Contour	Bottom Elevation	Top Elevation	Volume Provided	Volume Provided
<b>Basin 4</b>	10,798	13,254	2680.0	2681.0	<b>0.28</b>	<b>1.39</b>
	13,254	15,028	2681.0	2682.0	<b>0.32</b>	
	15,028	17,134	2682.0	2683.0	<b>0.37</b>	
	17,134	19,270	2683.0	2684.0	<b>0.42</b>	
	19,270	22,350	2684.0	2684.0	<b>0.00</b>	

**On-Site Detention Basin Capacity**

Description: Proposed Detention Basin Capacities  
 Project: **Desert Mountain Parcel 19**  
 Reference: *Drainage Design Manual for Maricopa County, Arizona - Hydrology*  
*City of Scottsdale, Design Standards and Policies Manual*

Basin IDs	Bottom Contour Area	Top Contour	Bottom Elevation	Top Elevation	Volume Provided	Volume Provided
<b>Basin 1</b>	8,584	10,210	2,606.0	2,607	<b>0.22</b>	<b>1.10</b>
	10,210	11,949	2,607.0	2,608	<b>0.25</b>	
	11,949	13,800	2,608.0	2,609	<b>0.30</b>	
	13,800	15,757	2,609.0	2,610	<b>0.34</b>	

Basin IDs	Bottom Contour Area	Top Contour	Bottom Elevation	Top Elevation	Volume Provided	Volume Provided
<b>Basin 2</b>	3,232	4,553	2,608.0	2,609	<b>0.09</b>	<b>0.58</b>
	4,553	6,056	2,609.0	2,610	<b>0.12</b>	
	6,056	7,964	2,610.0	2,611	<b>0.16</b>	
	7,964	10,184	2,611.0	2,612	<b>0.21</b>	

**APPENDIX D**

**Hydraulic Calculations**

**HEC-RAS Output Files Existing & Proposed Conditions**

**Scour Calculations**

**Erosion Hazard Setback Calculations**

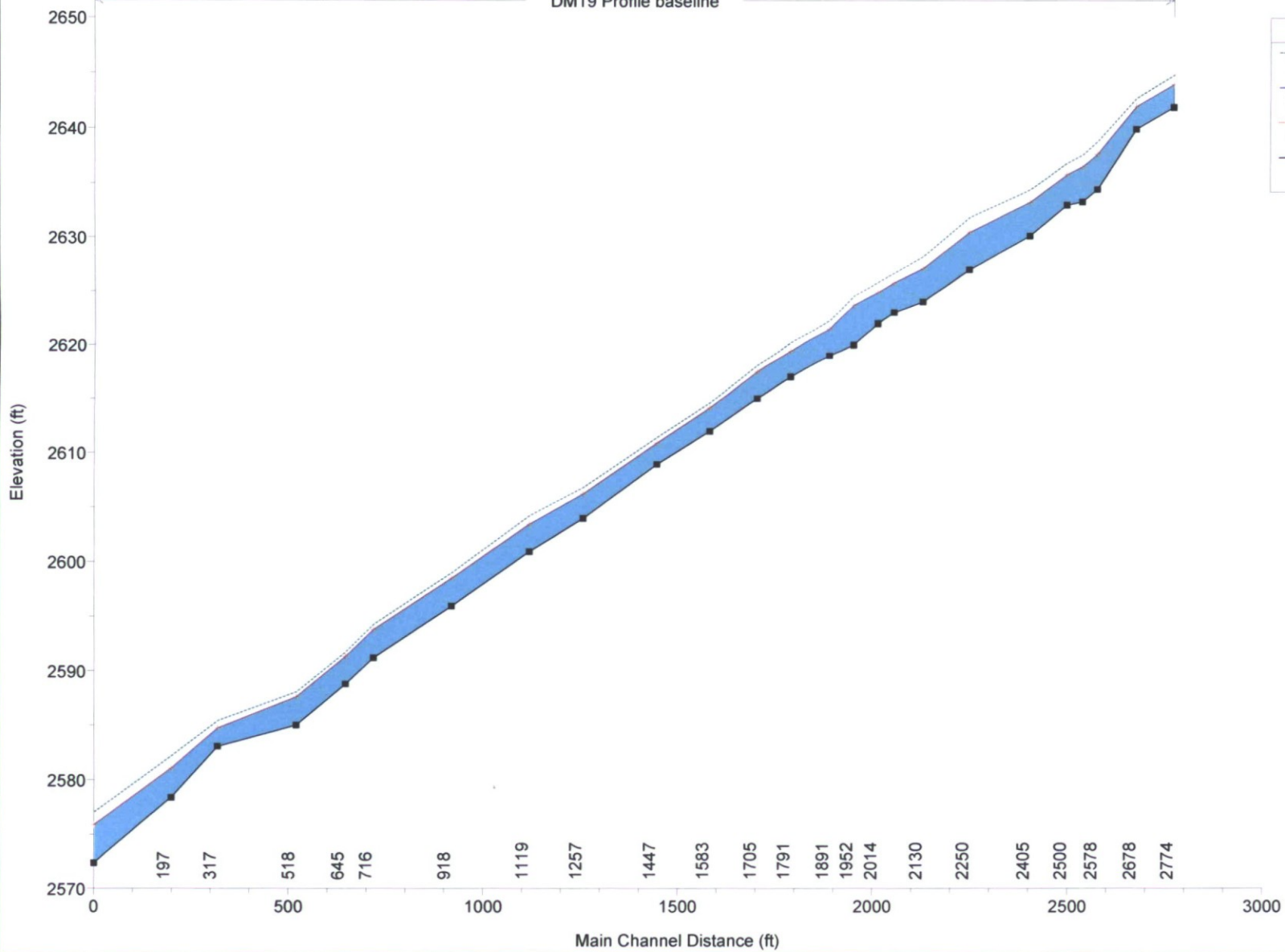
**HEC-RAS Output Files Existing & Proposed Conditions**

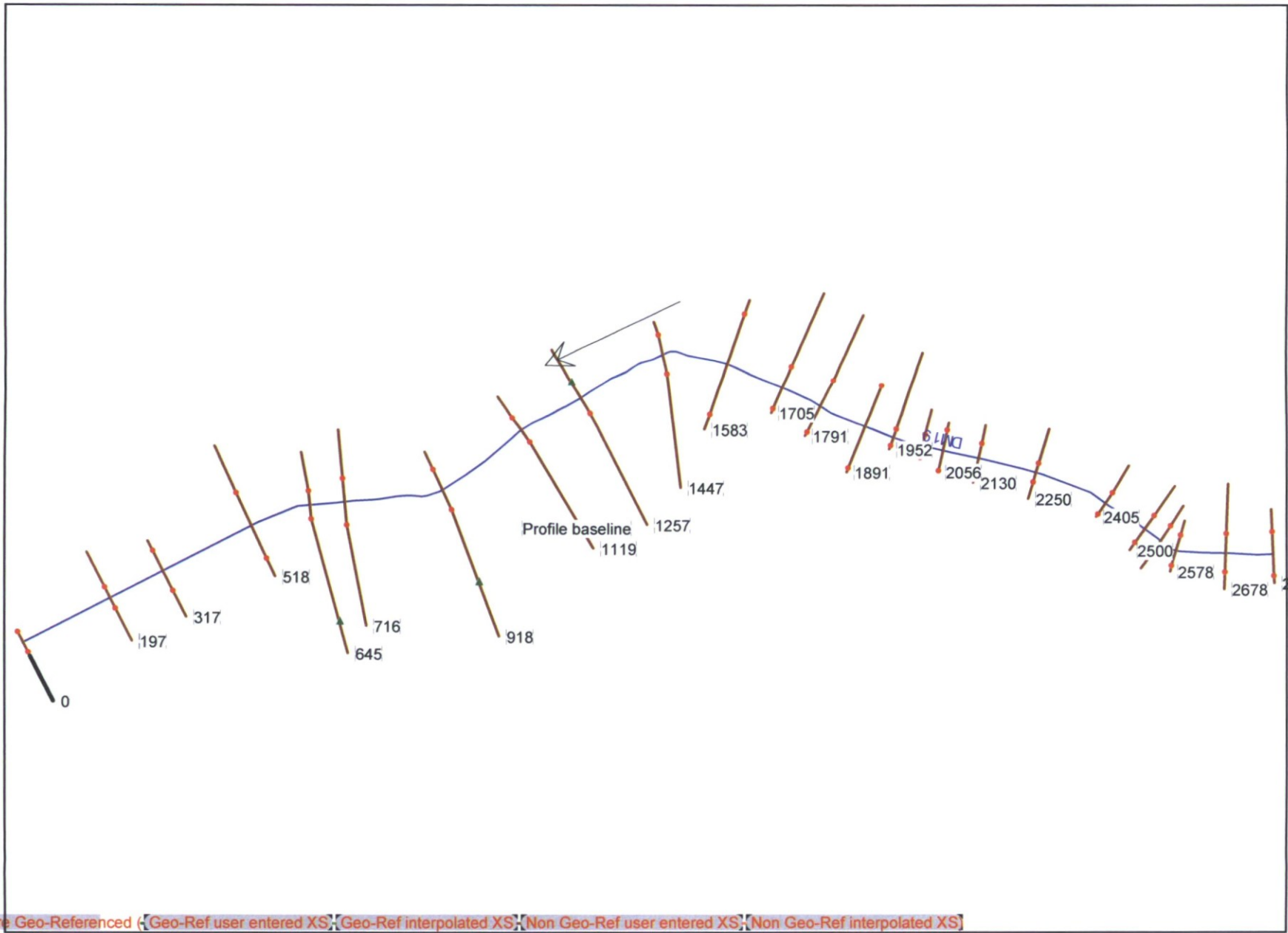
**EXISTING**

**GALLOWAY WASH**

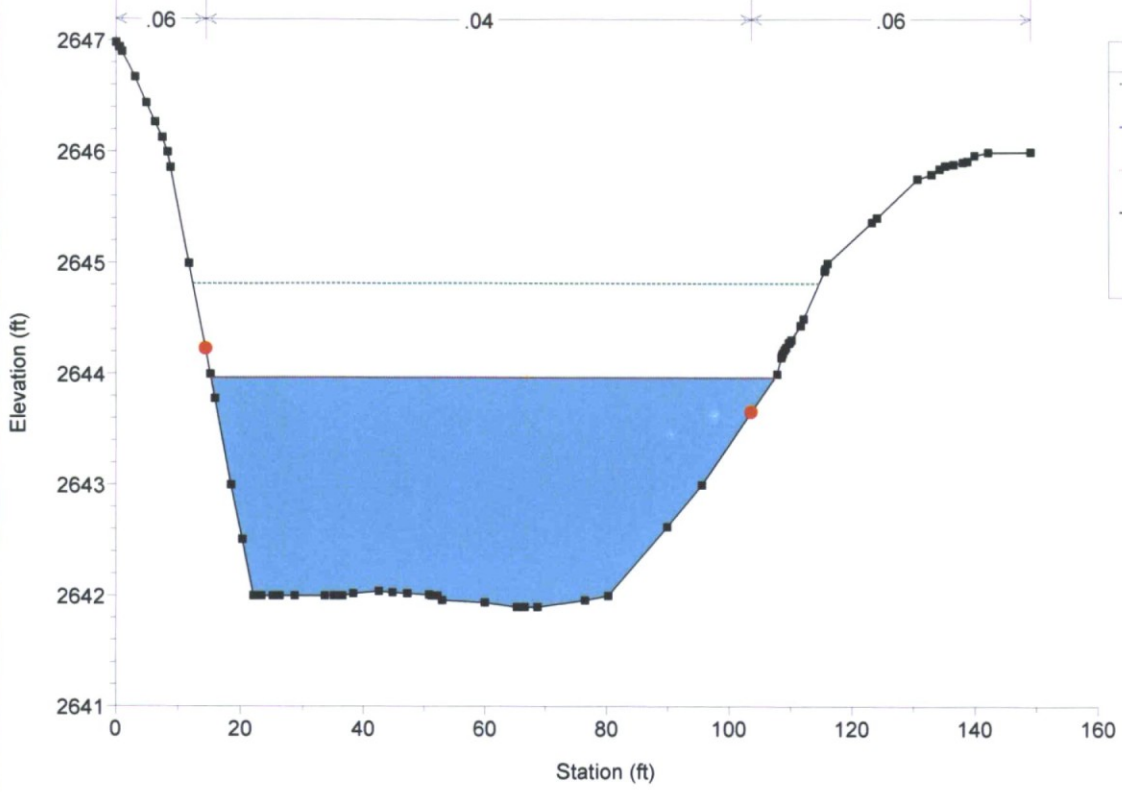
DM19 Existing Plan: Plan 01 6/15/2016

DM19 Profile baseline

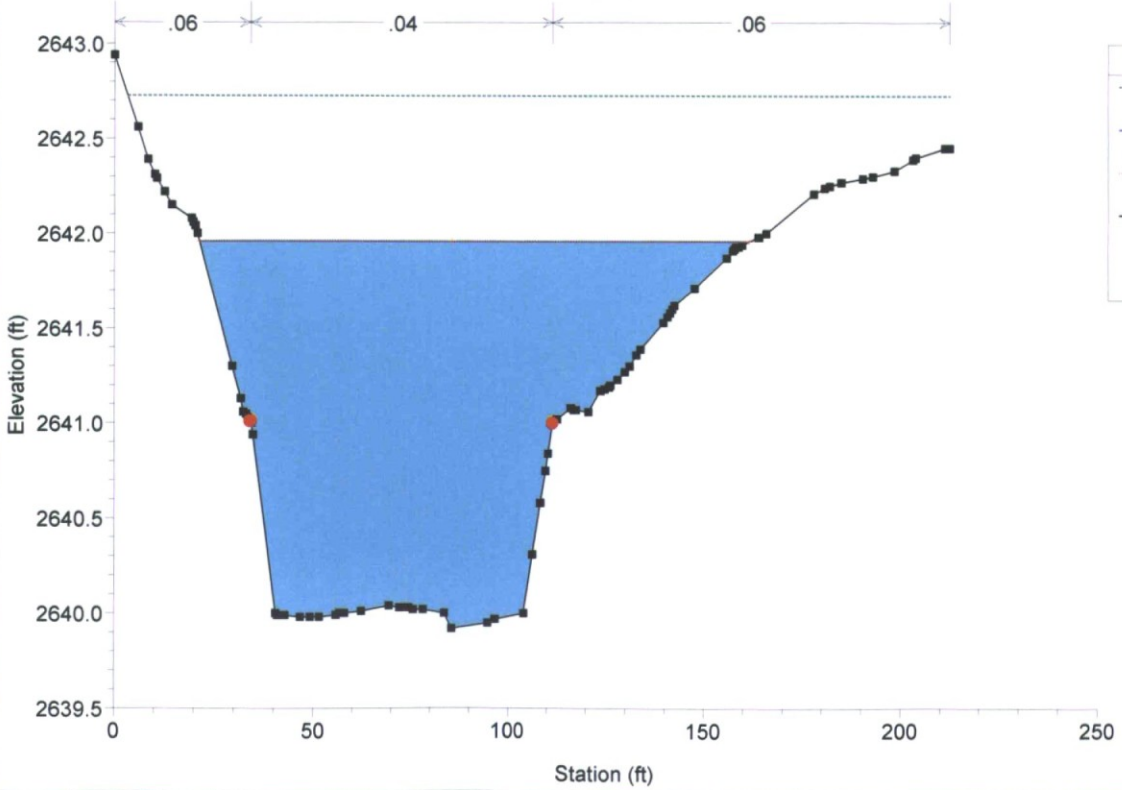




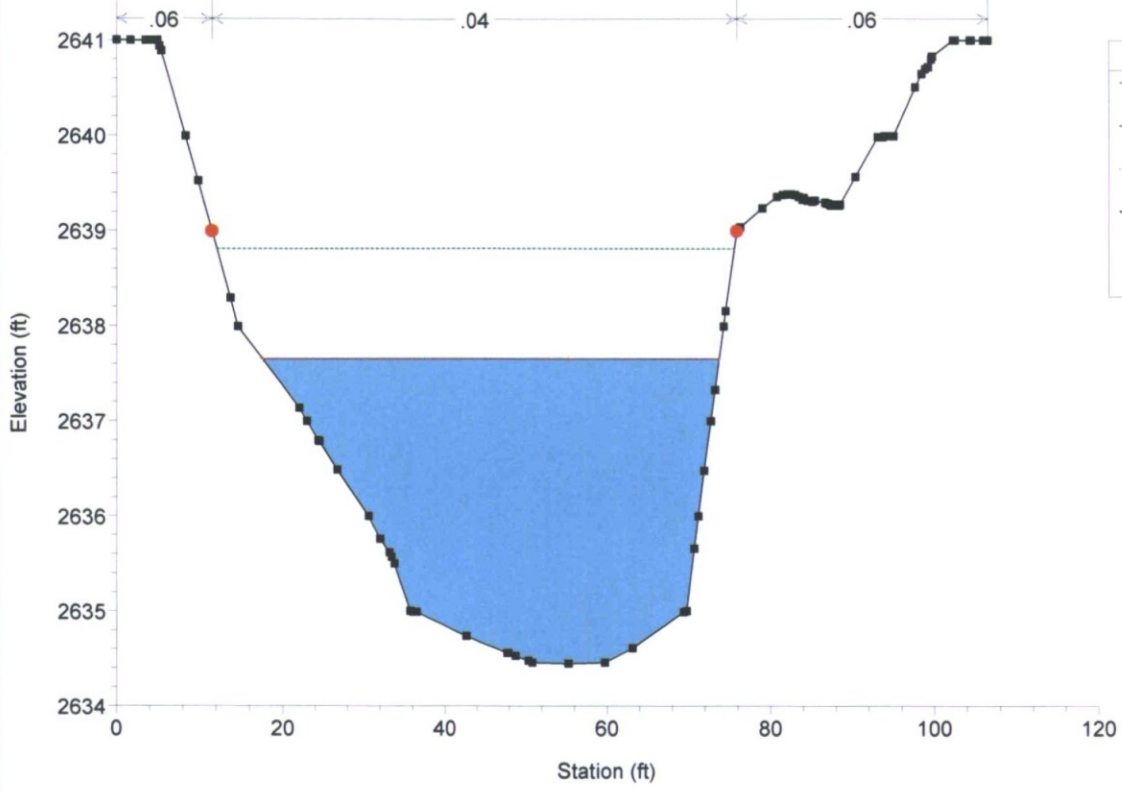
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RS = 2774



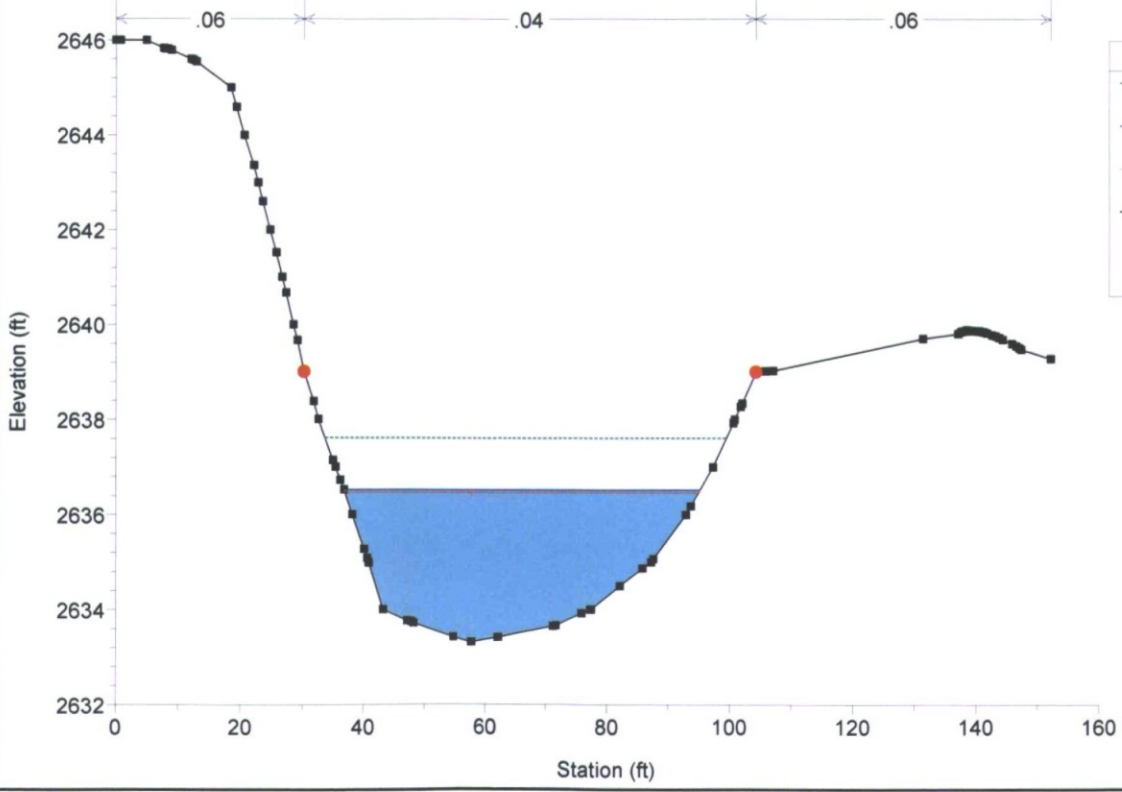
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RS = 2678



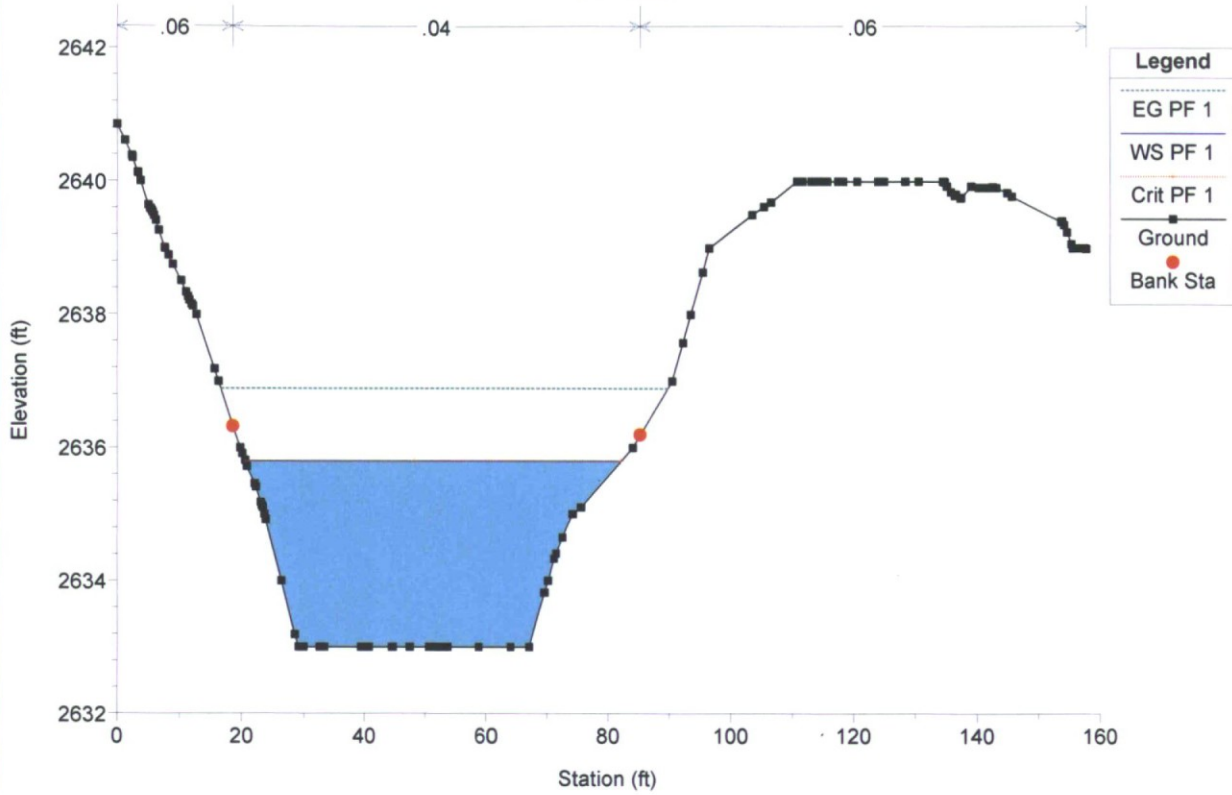
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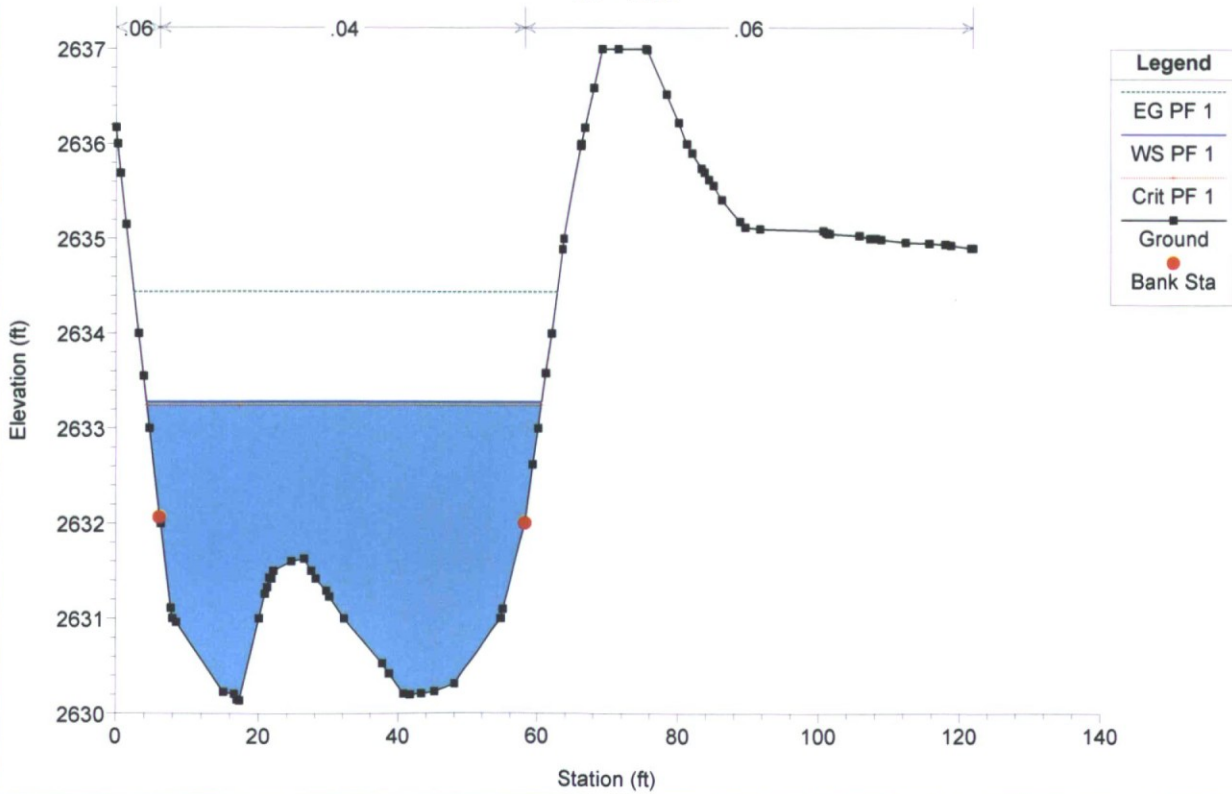
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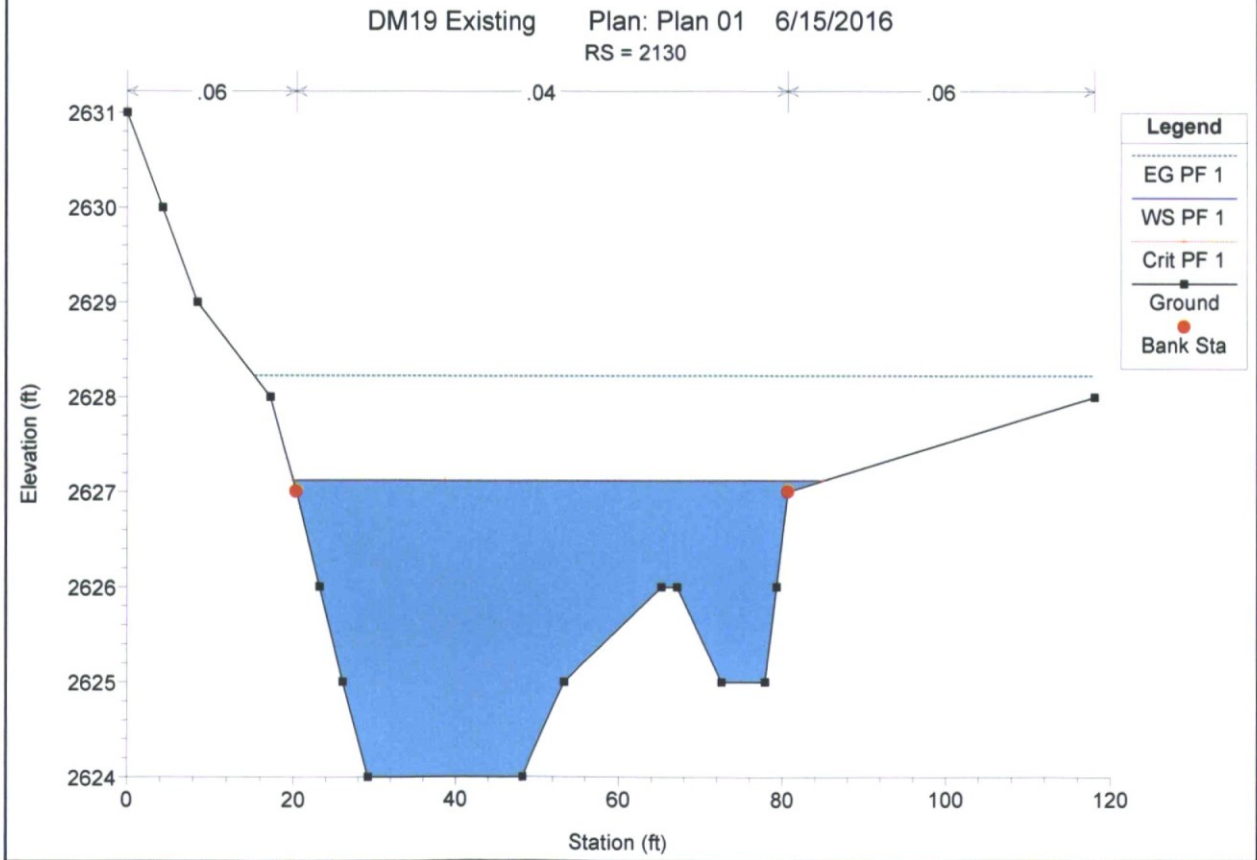
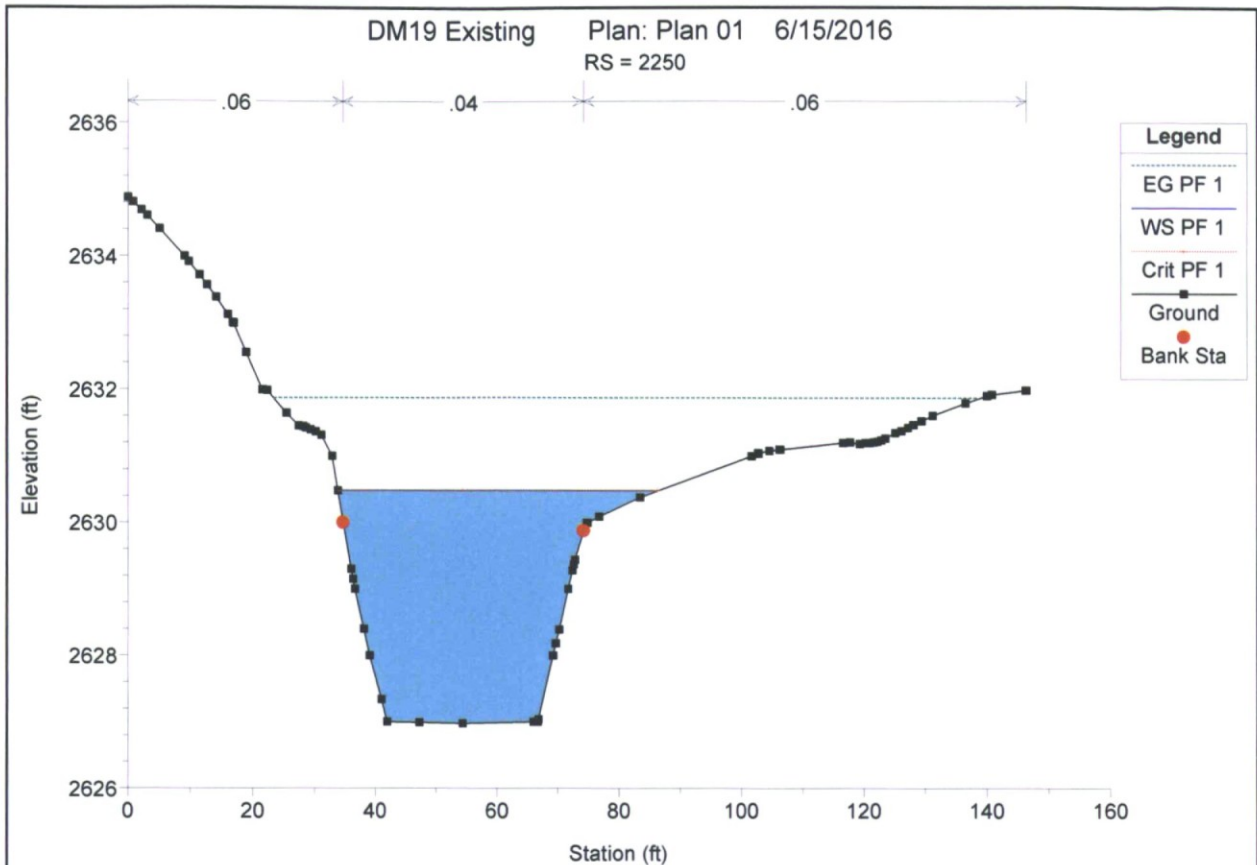


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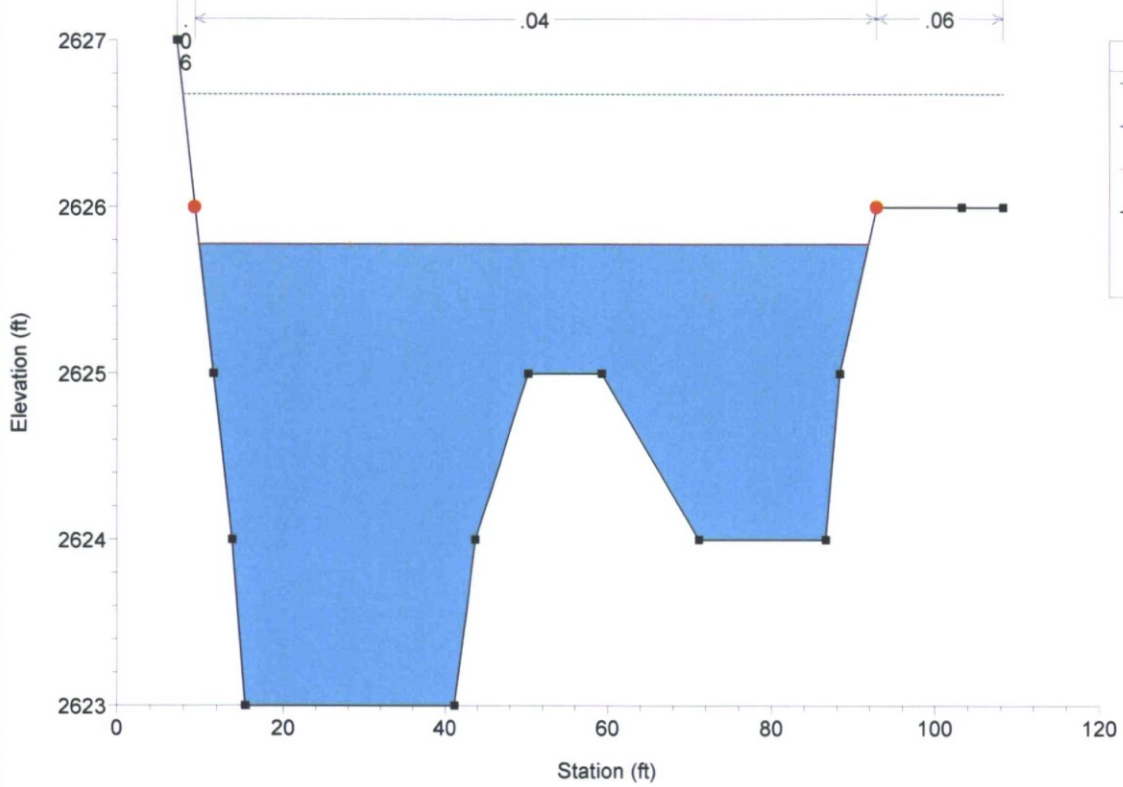


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RS = 2405

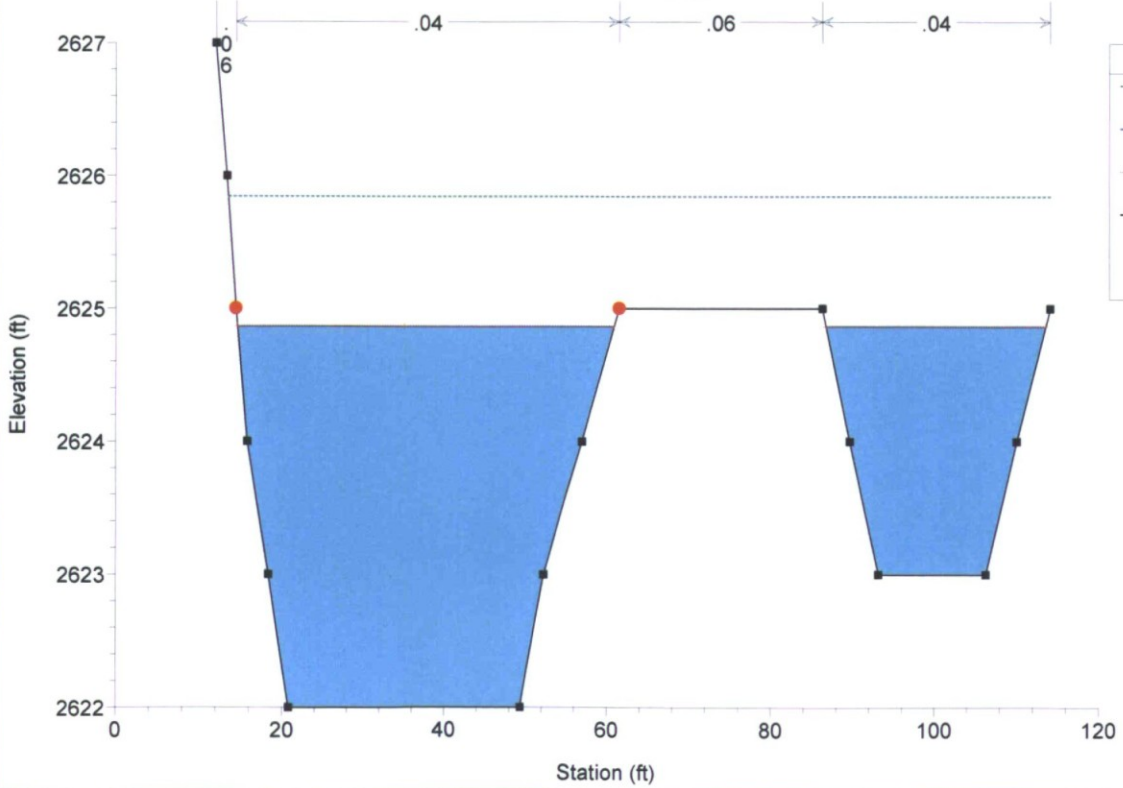




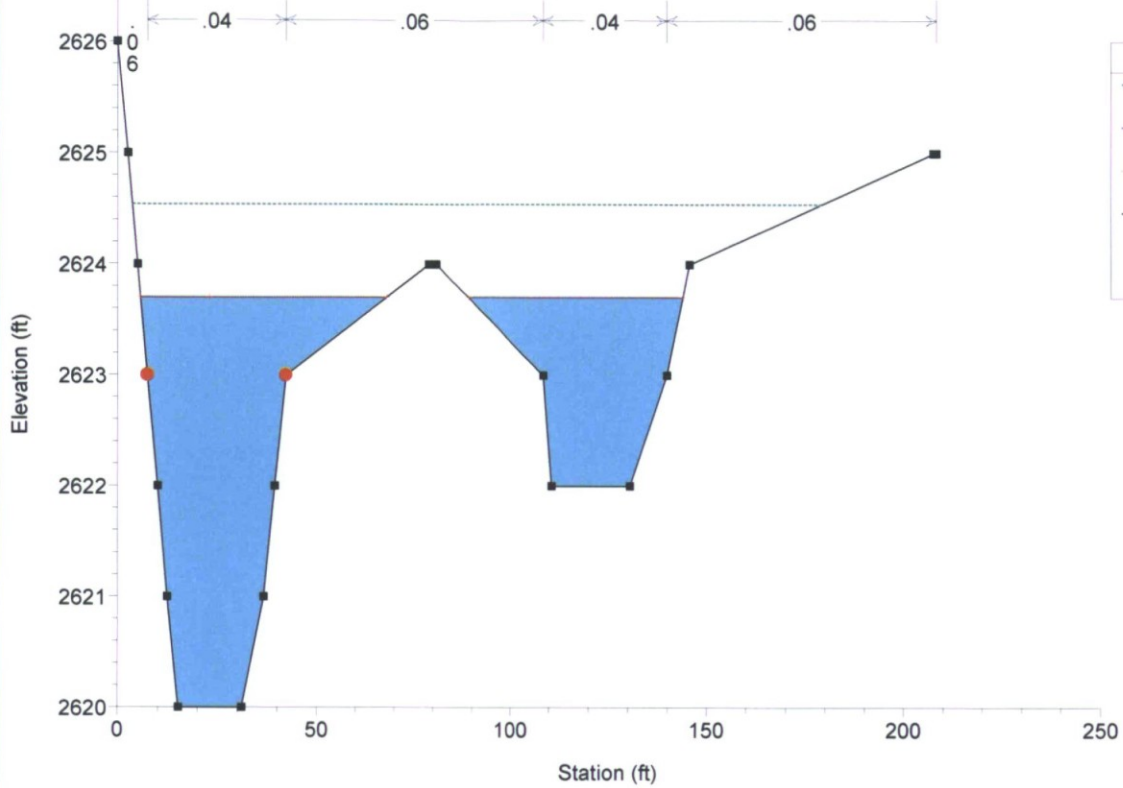
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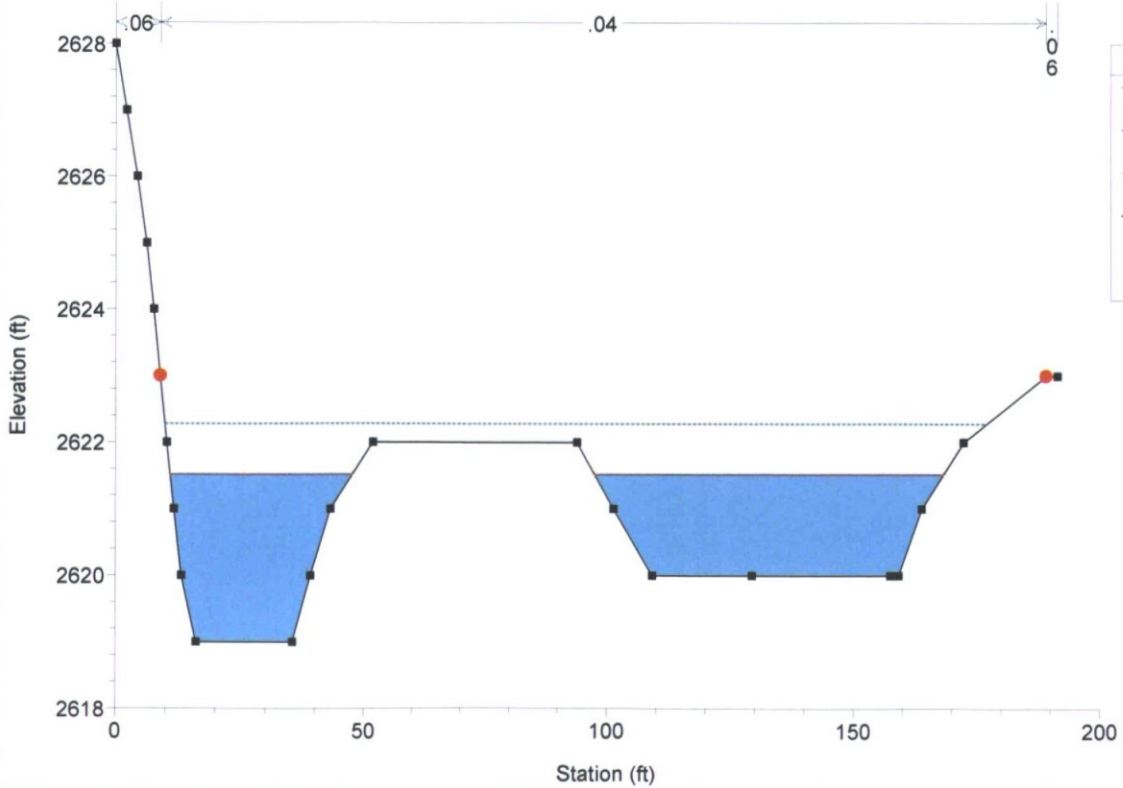
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RS = 2014 cs 60



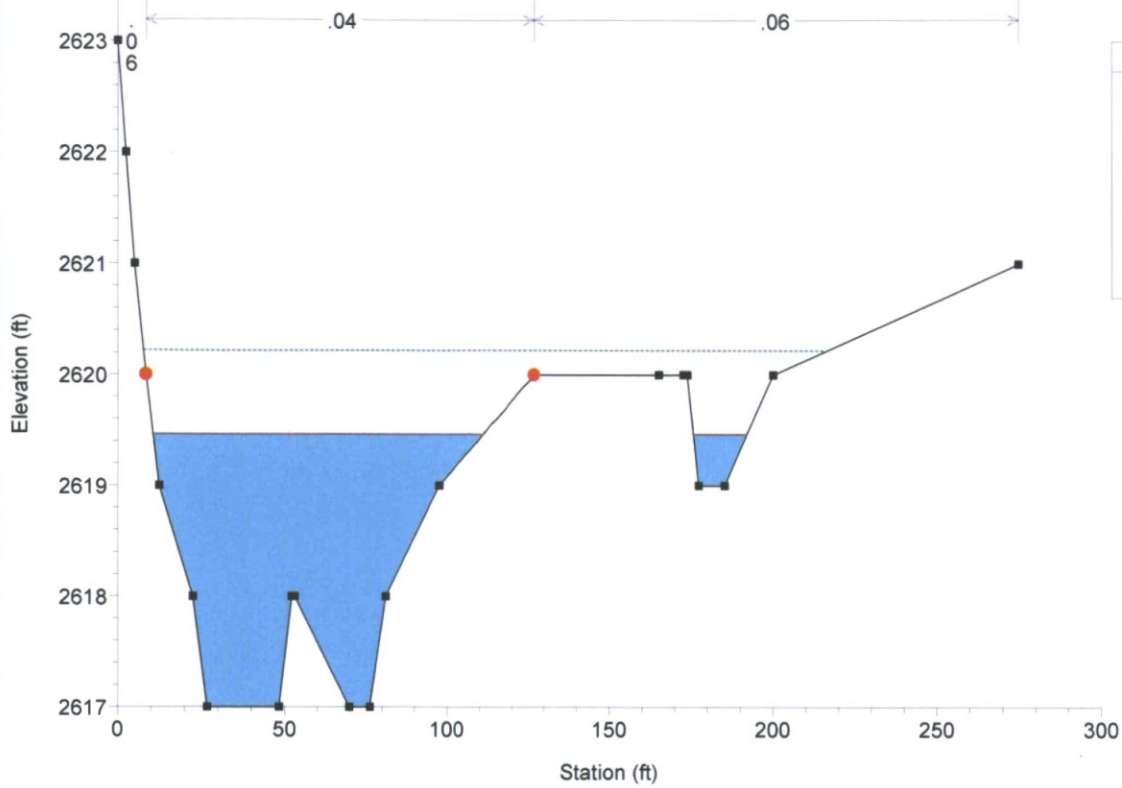
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RS = 1952



DM19 Existing Plan: Plan 01 6/15/2016  
RS = 1891

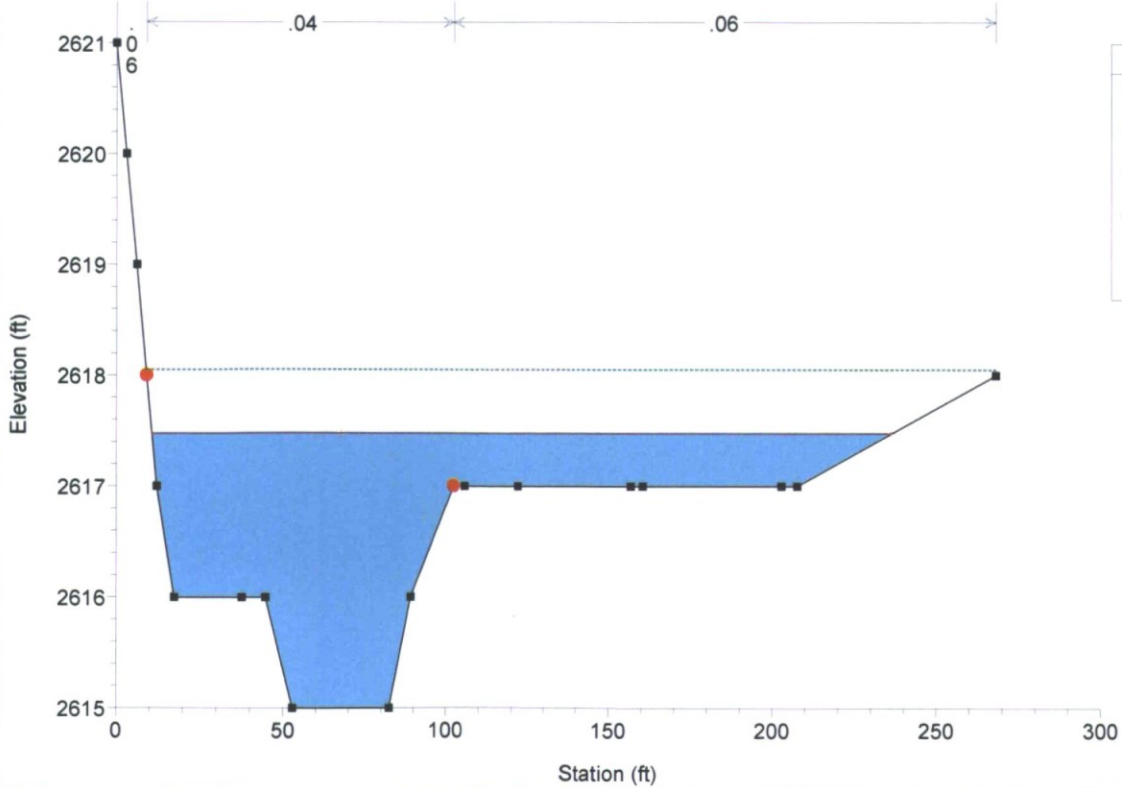


DM19 Existing Plan: Plan 01 6/15/2016  
RS = 1791

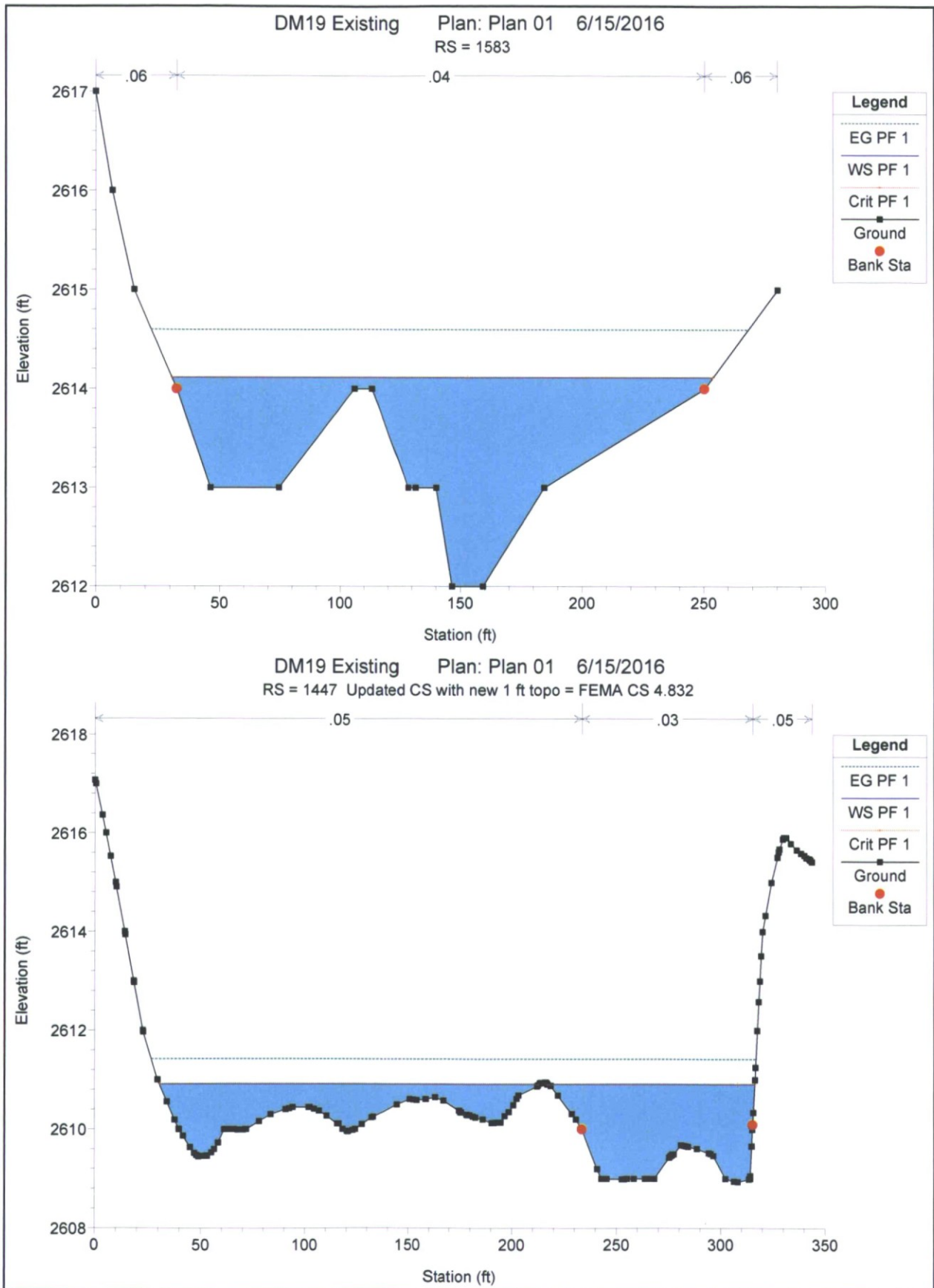


Legend	
—	EG PF 1
—	WS PF 1
—	Crit PF 1
■	Ground
●	Bank Sta

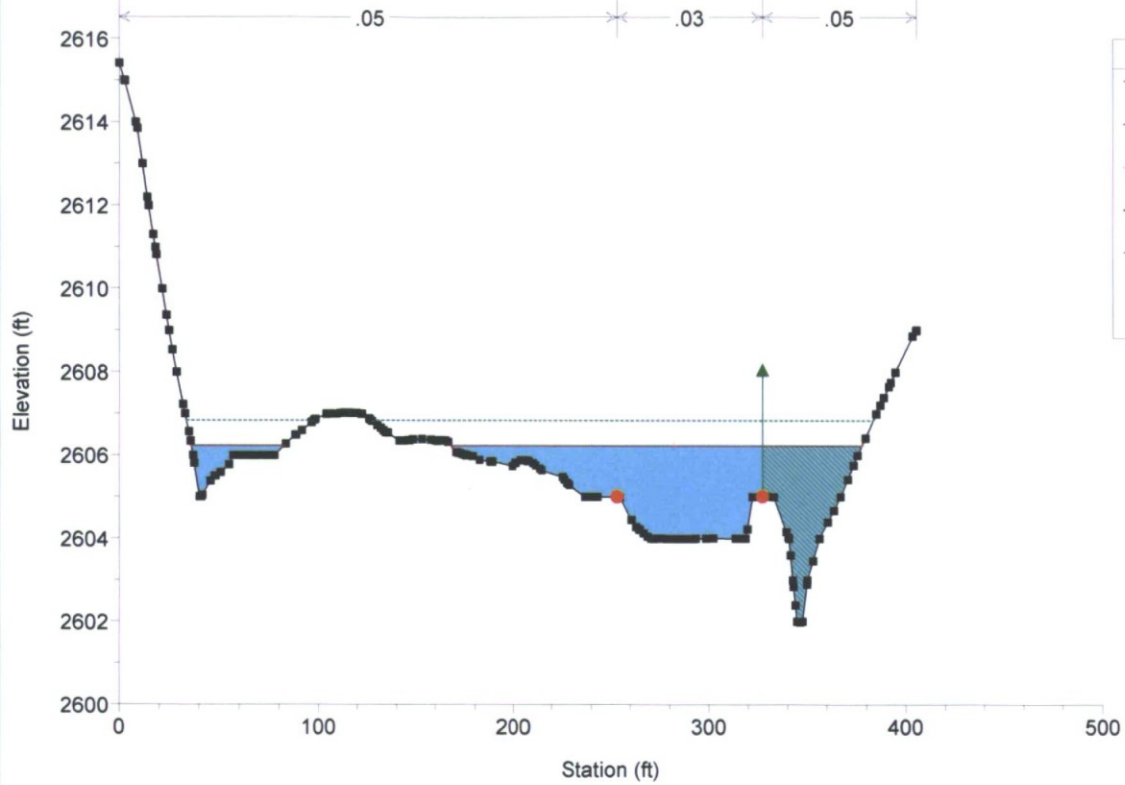
DM19 Existing Plan: Plan 01 6/15/2016  
RS = 1705



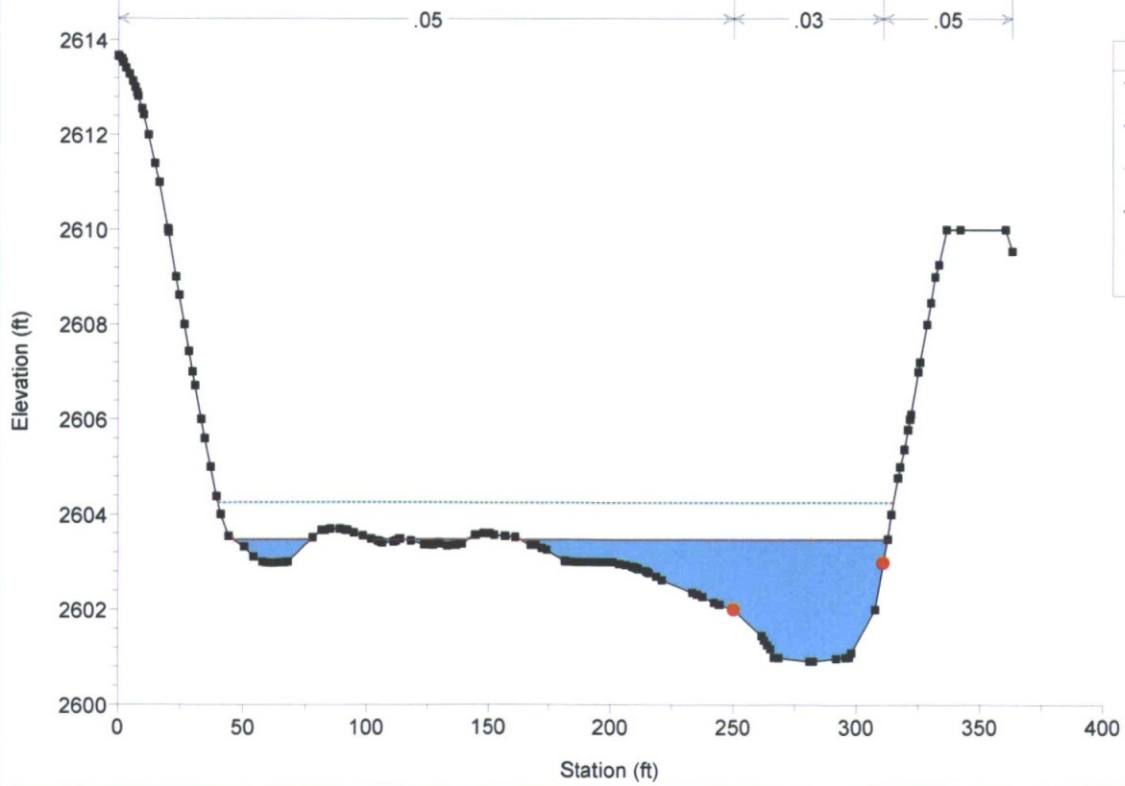
Legend	
—	EG PF 1
—	WS PF 1
—	Crit PF 1
■	Ground
●	Bank Sta

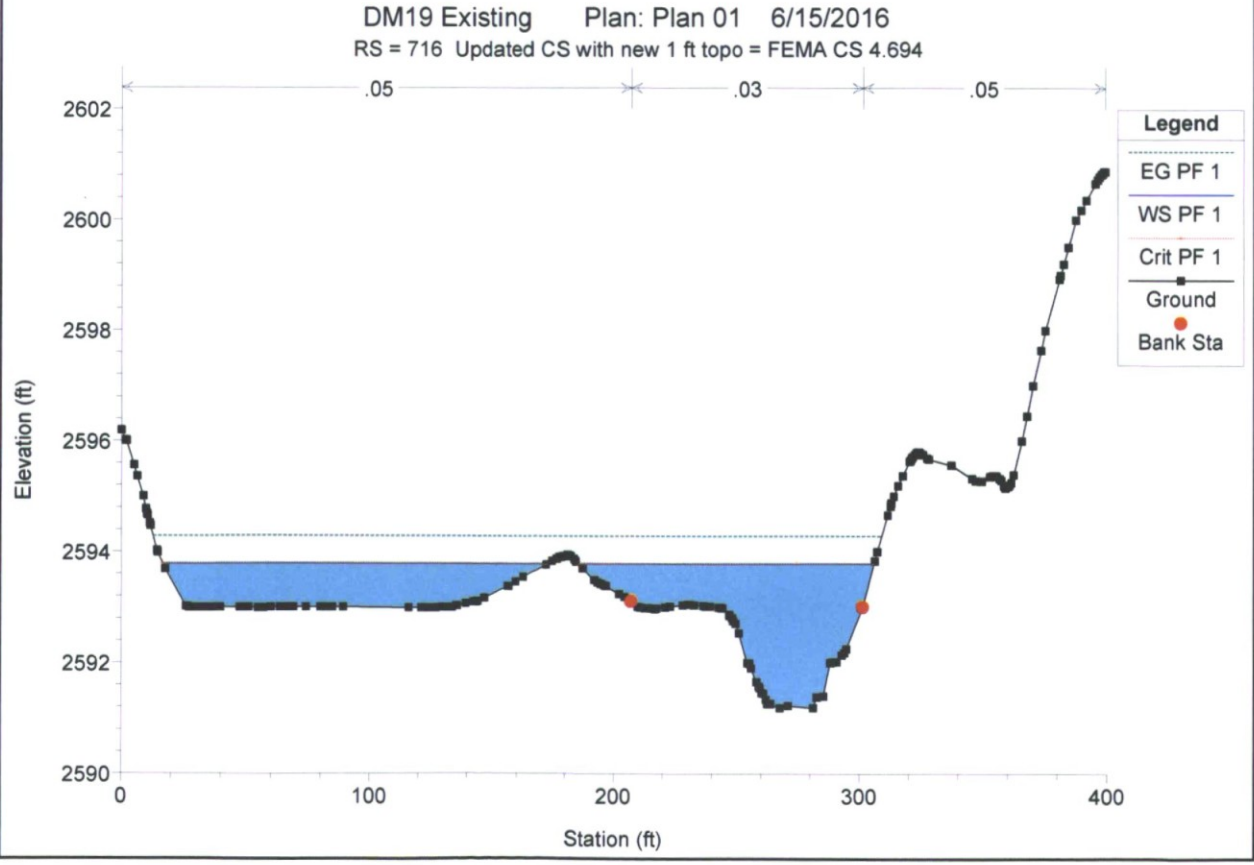
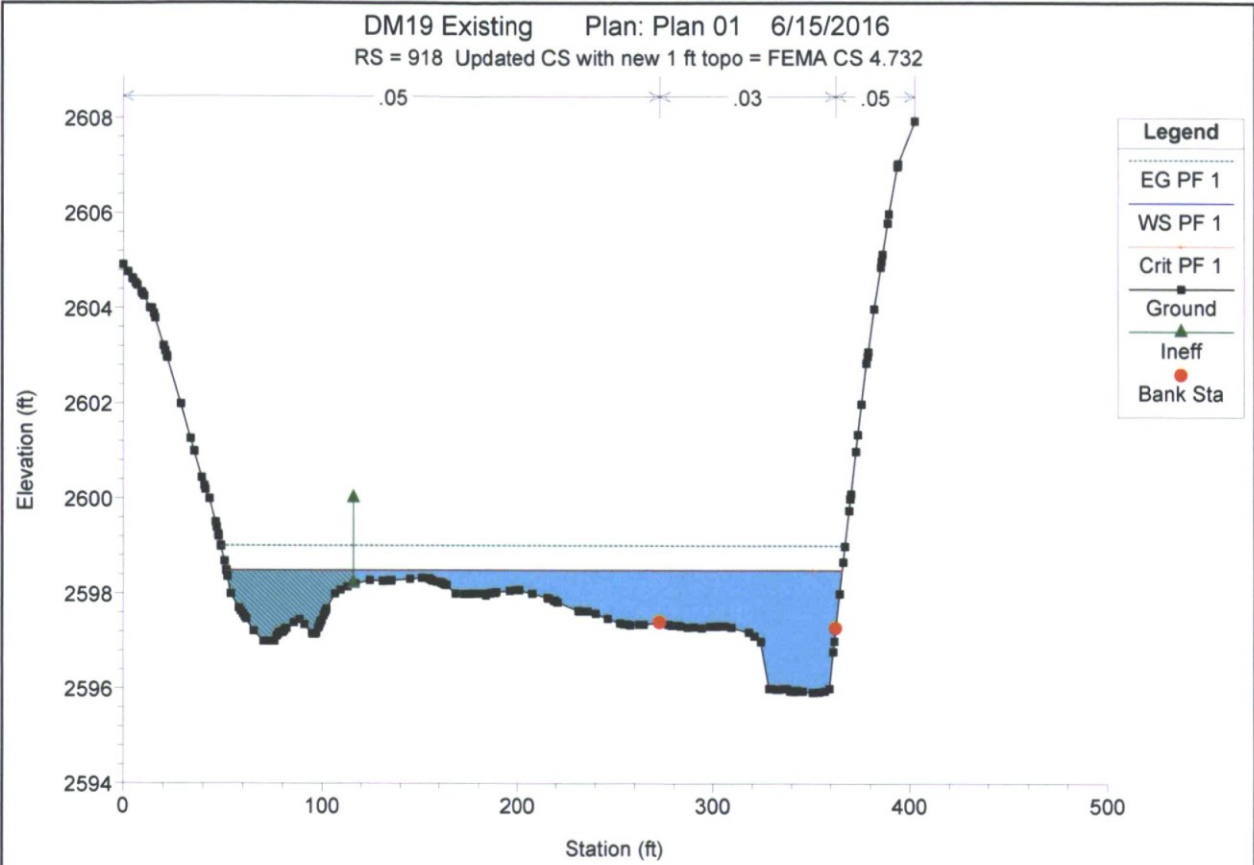


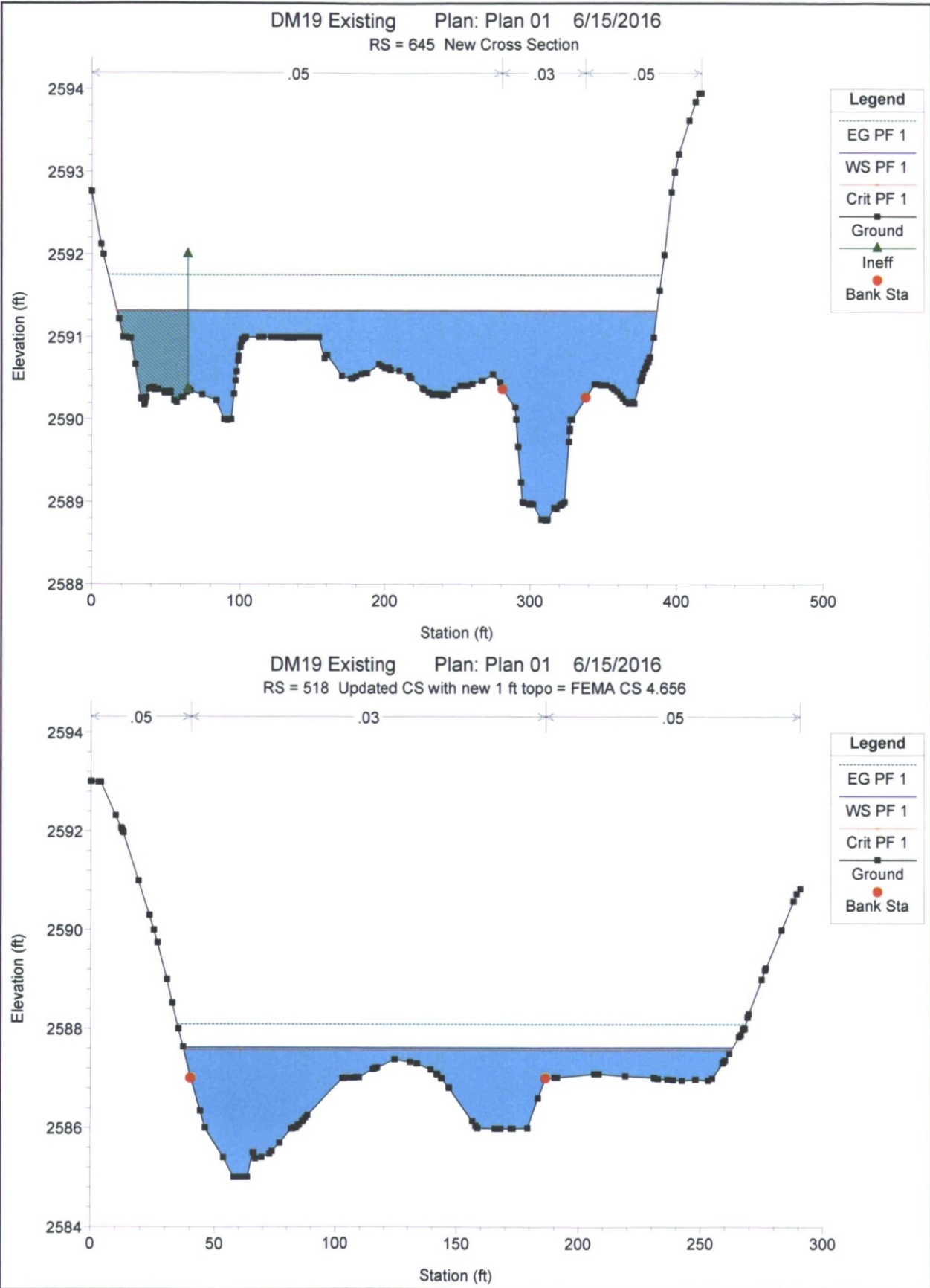
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RS = 1257 Updated CS with new 1 ft topo = FEMA CS 4.796

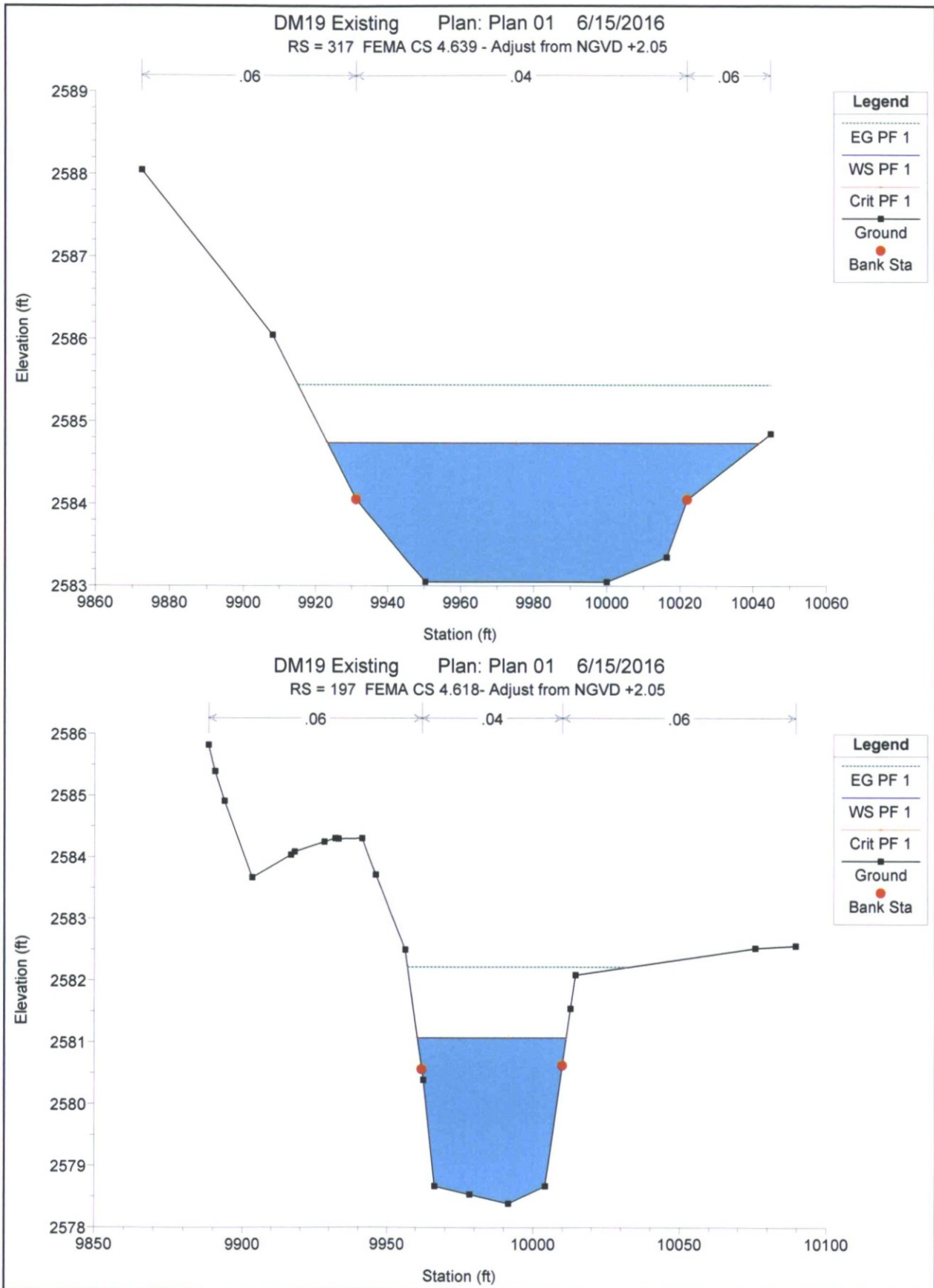


DM19 Existing Plan: Plan 01 6/15/2016  
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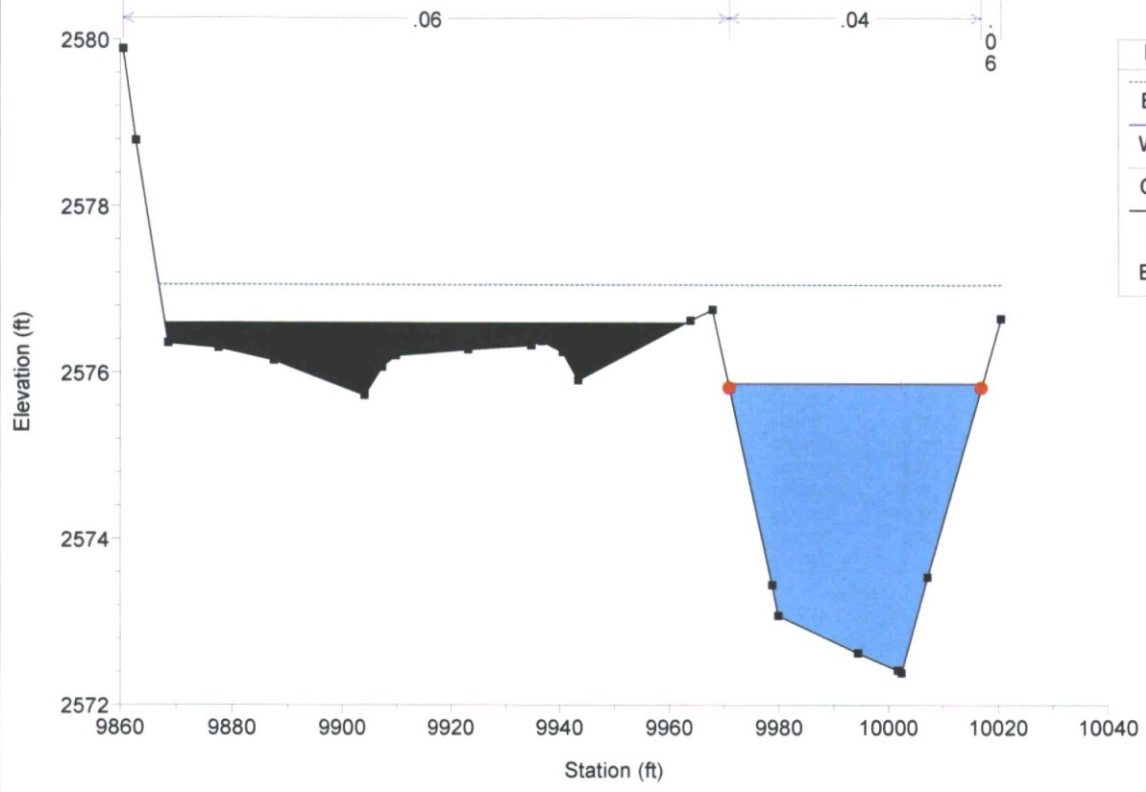








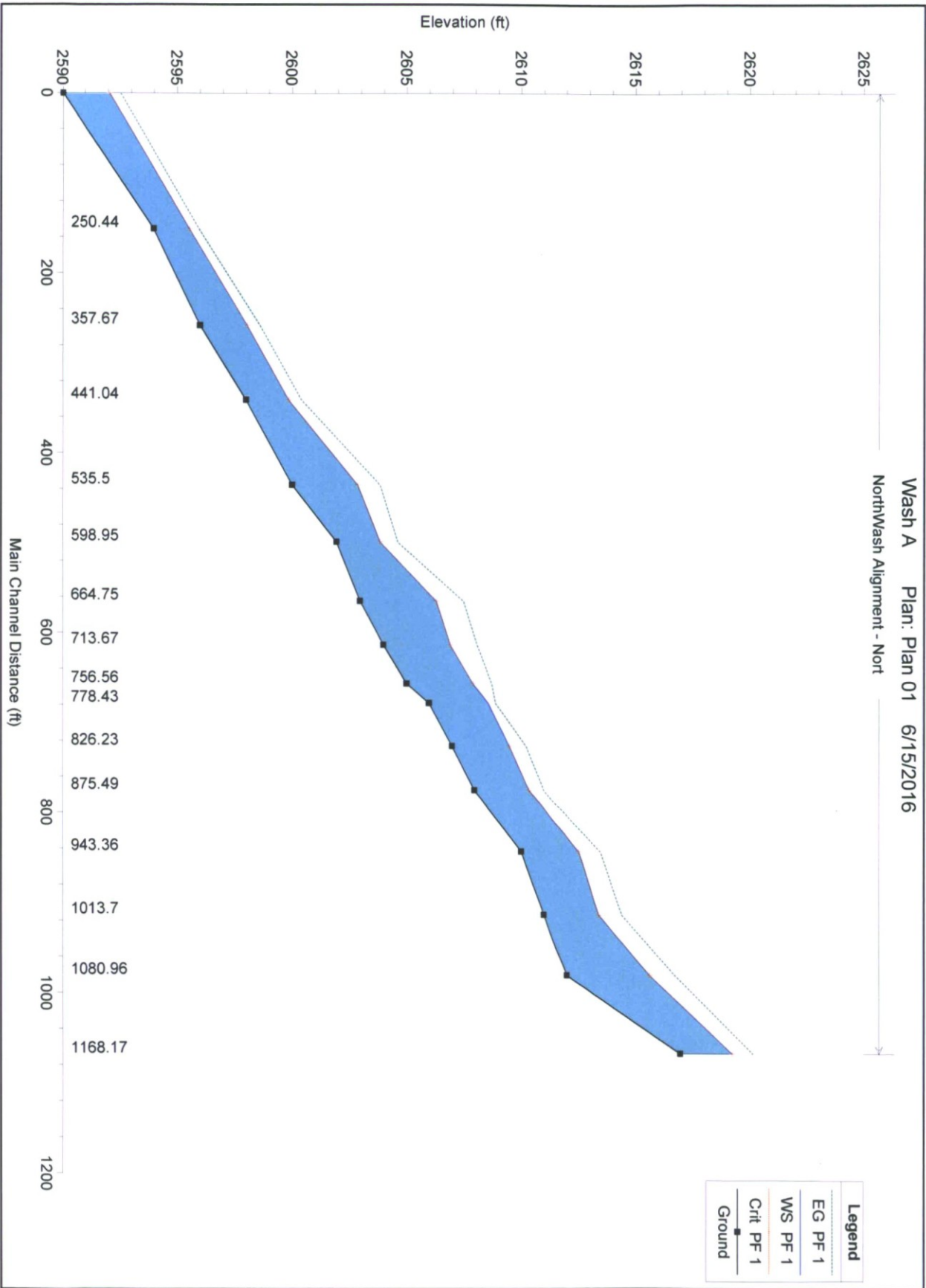
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RS = 0 FEMA CS 4.581- Adjust from NGVD +2.05



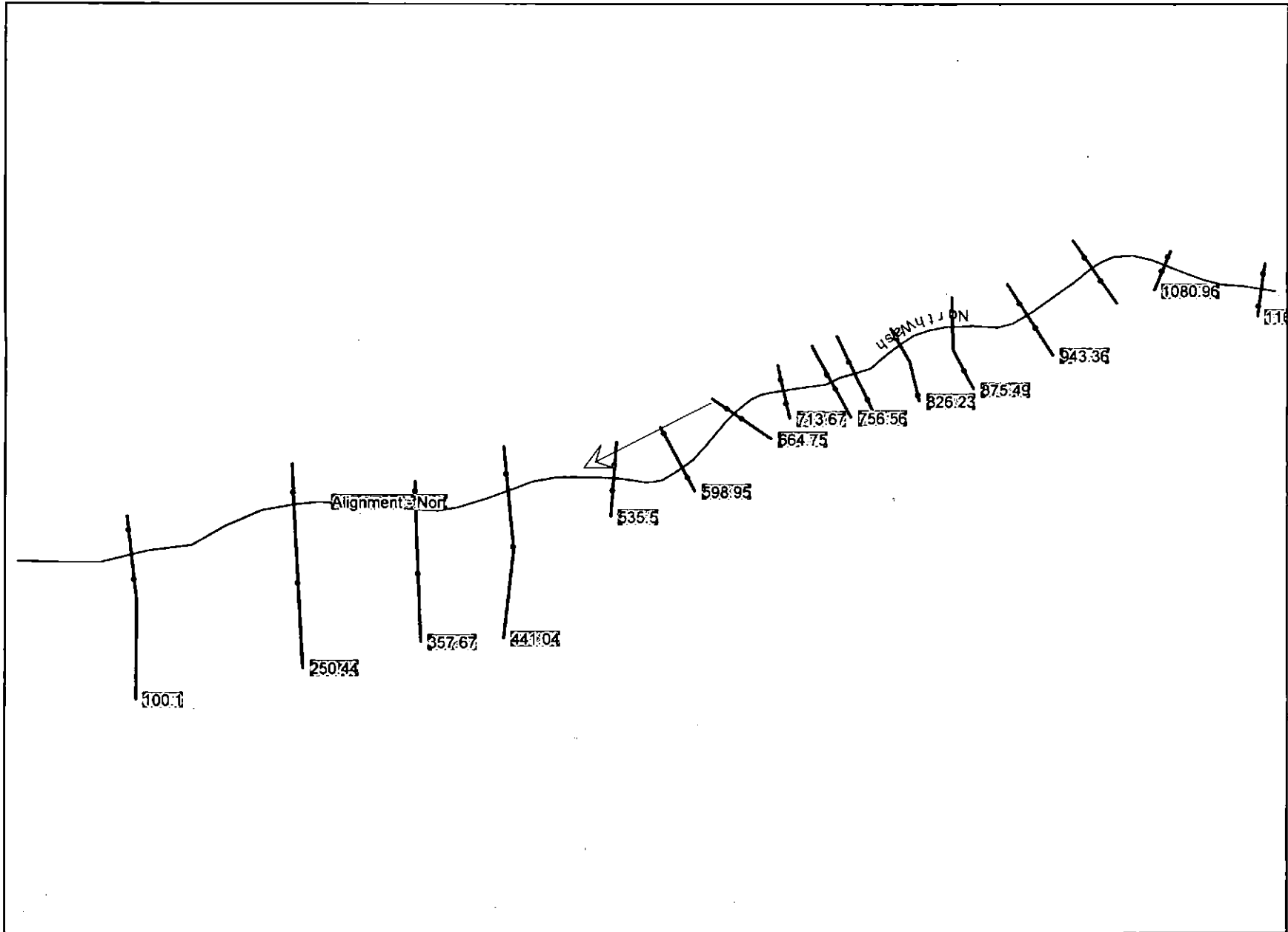
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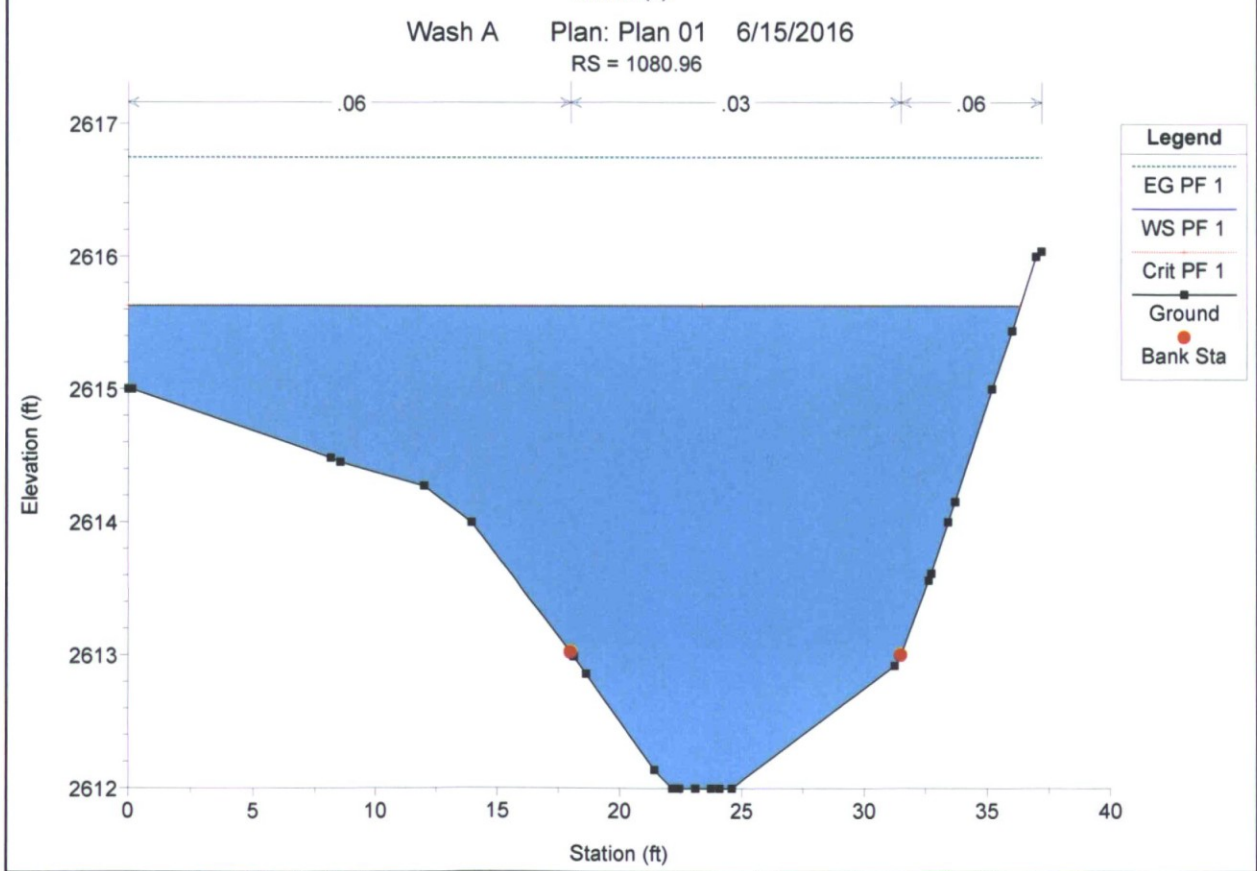
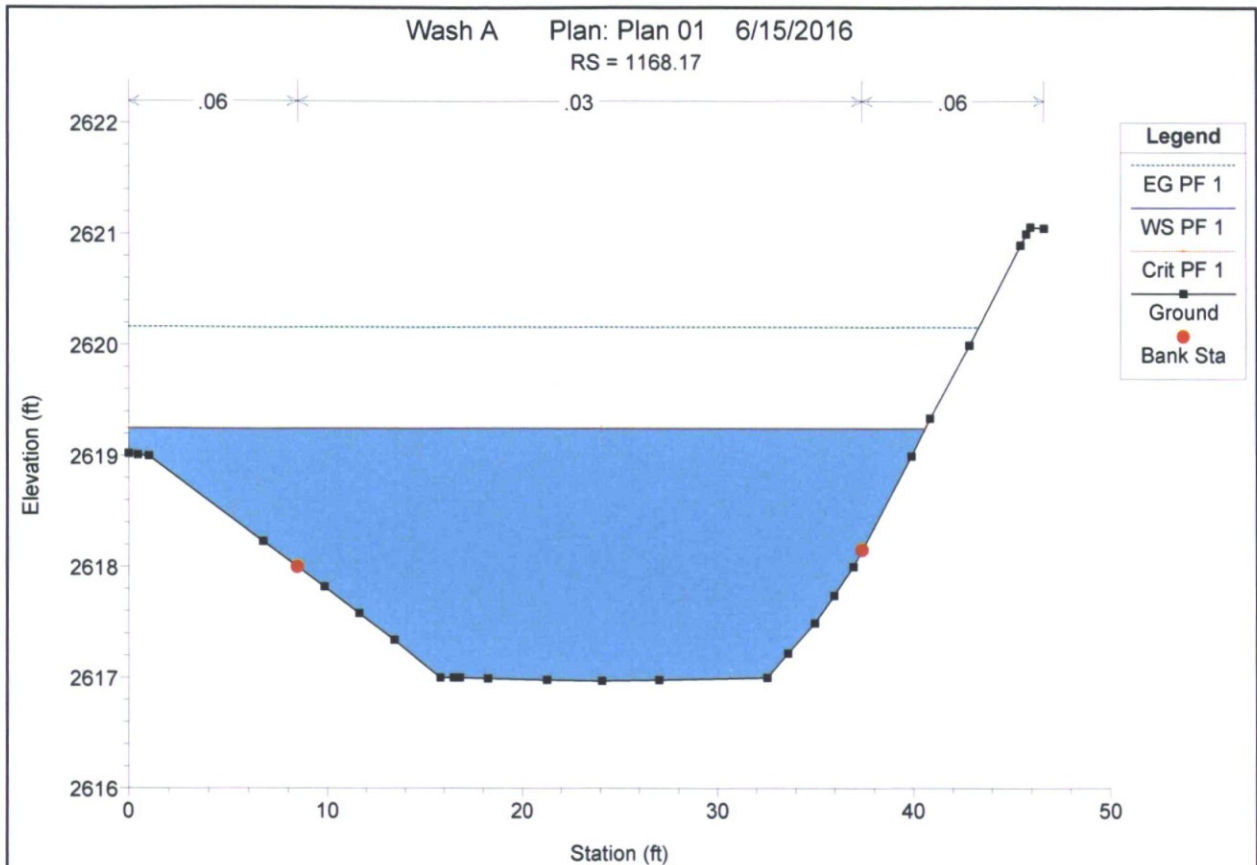
**WASH A**

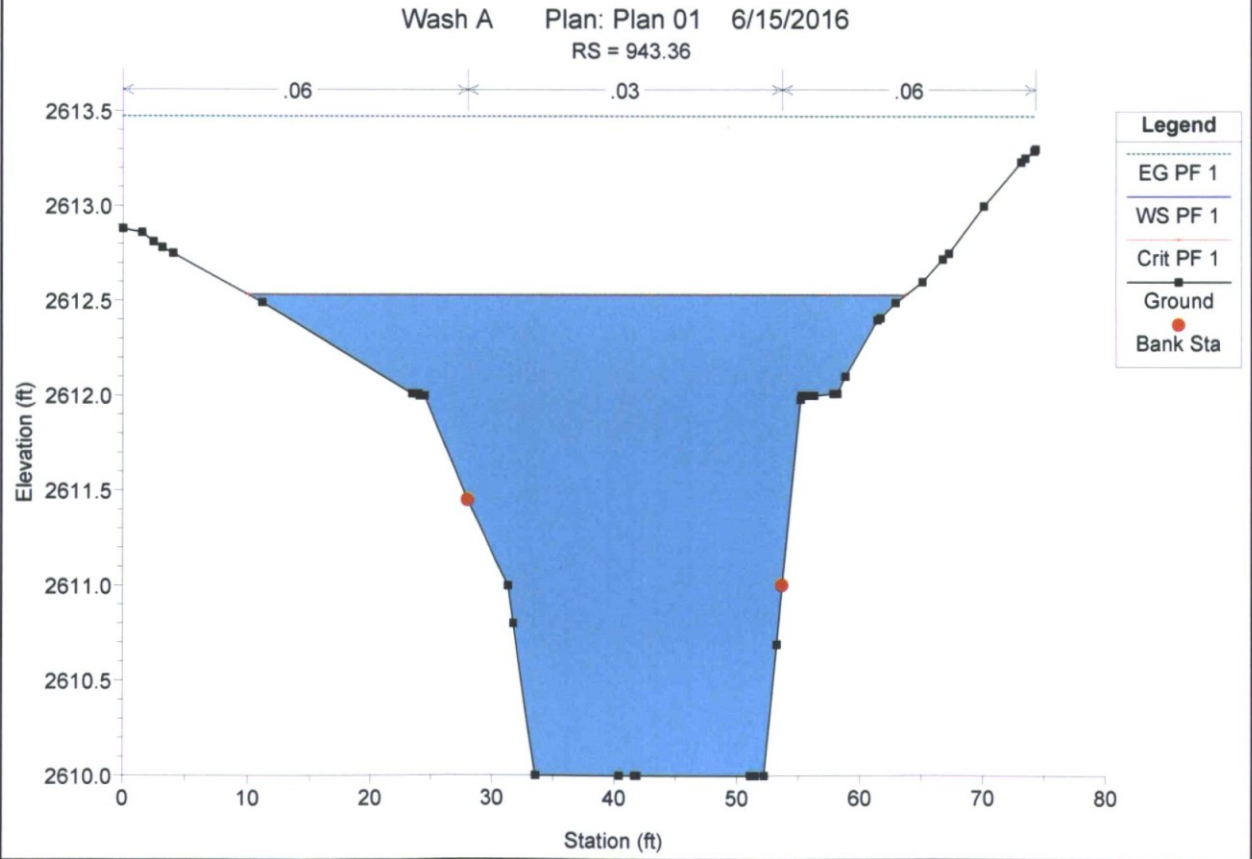
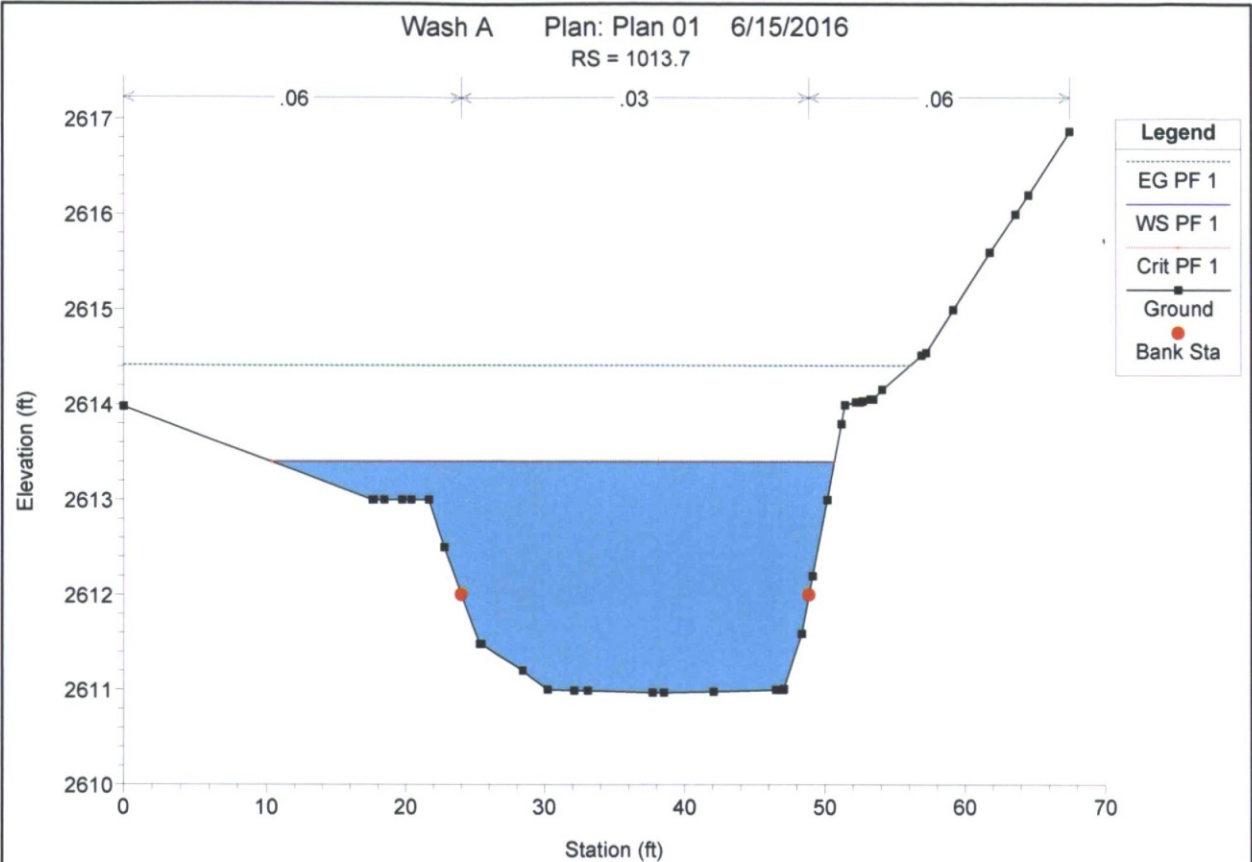
Wash A Plan: Plan 01 6/15/2016  
 NorthWash Alignment - Nort

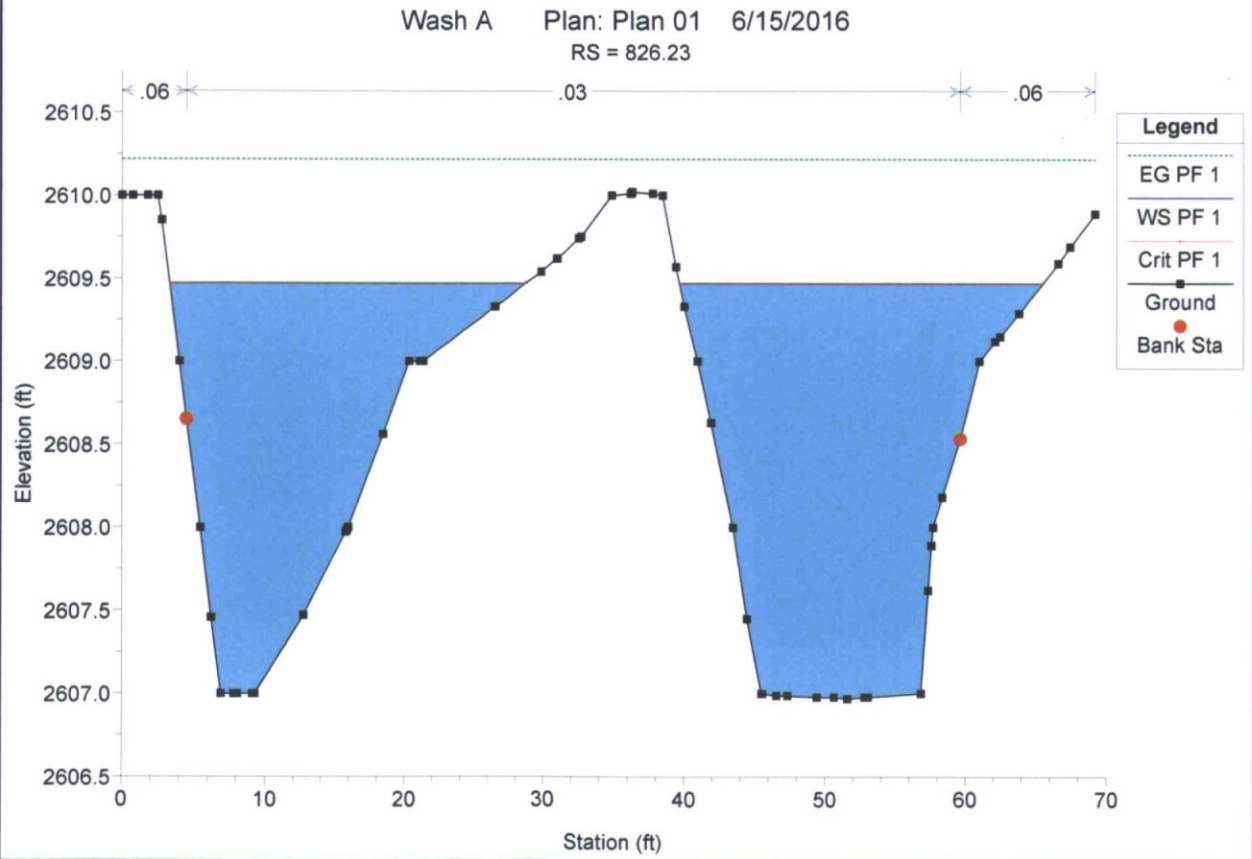
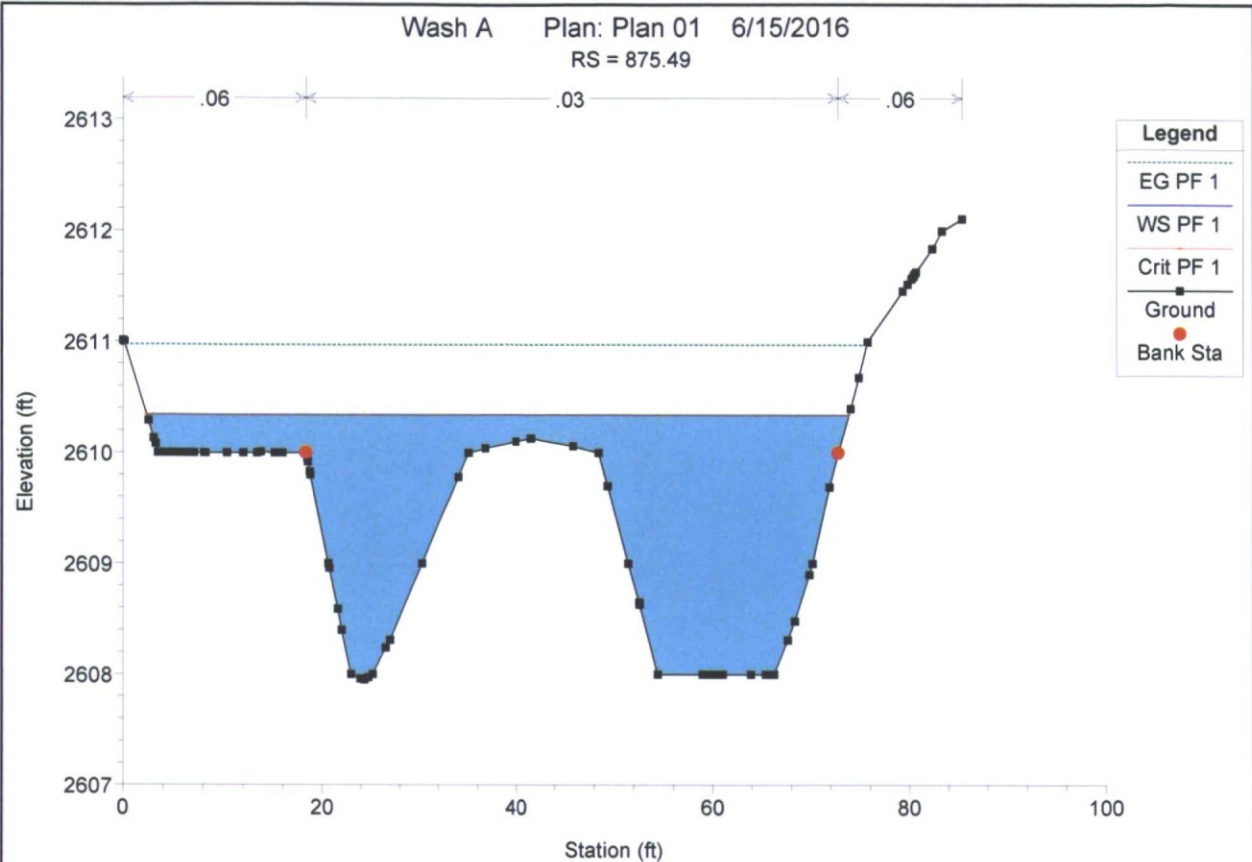


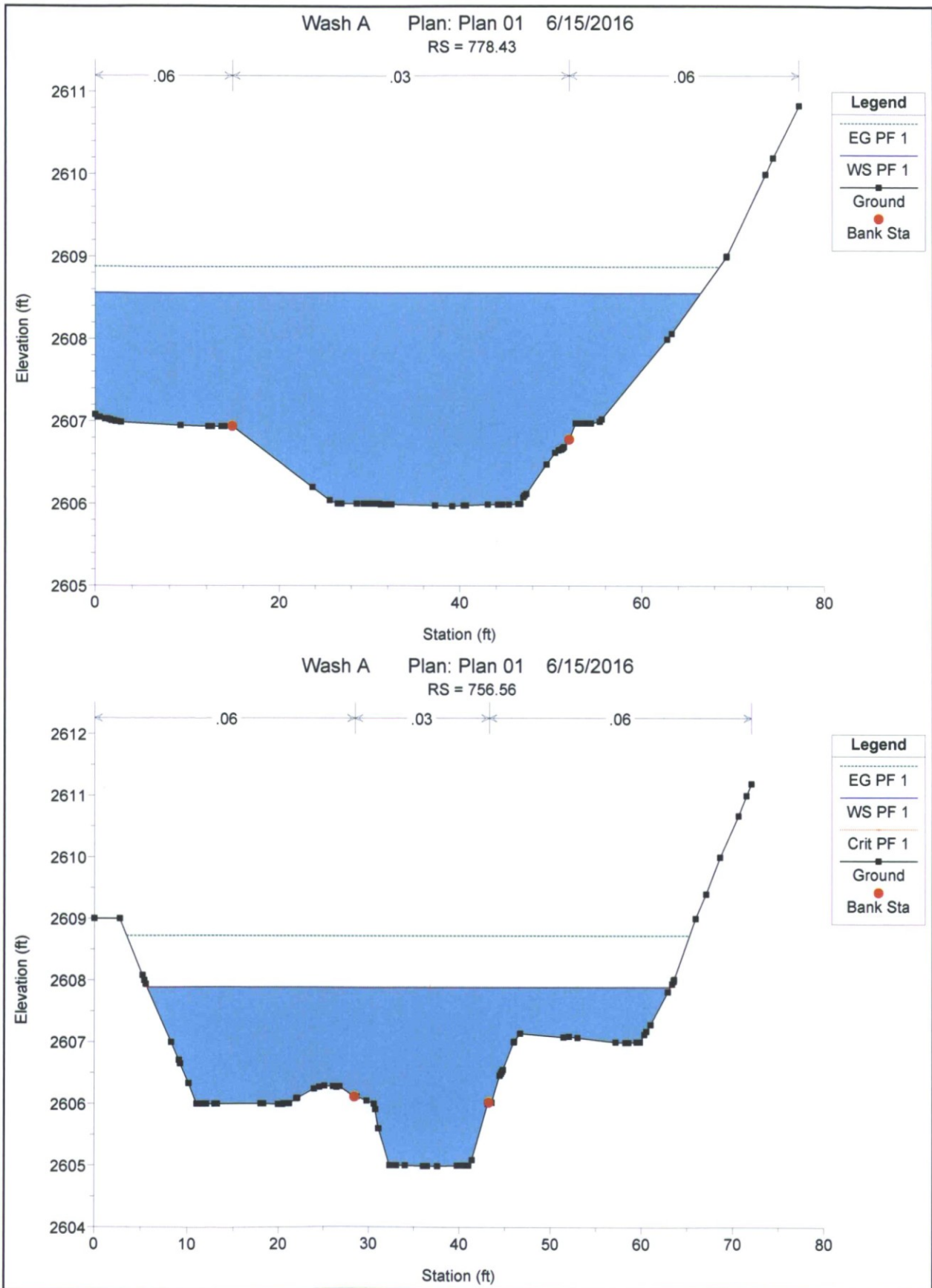
Legend	
EG PF 1	(Dashed line)
WS PF 1	(Solid line)
Crit PF 1	(Solid line with square markers)
Ground	(Dotted line)

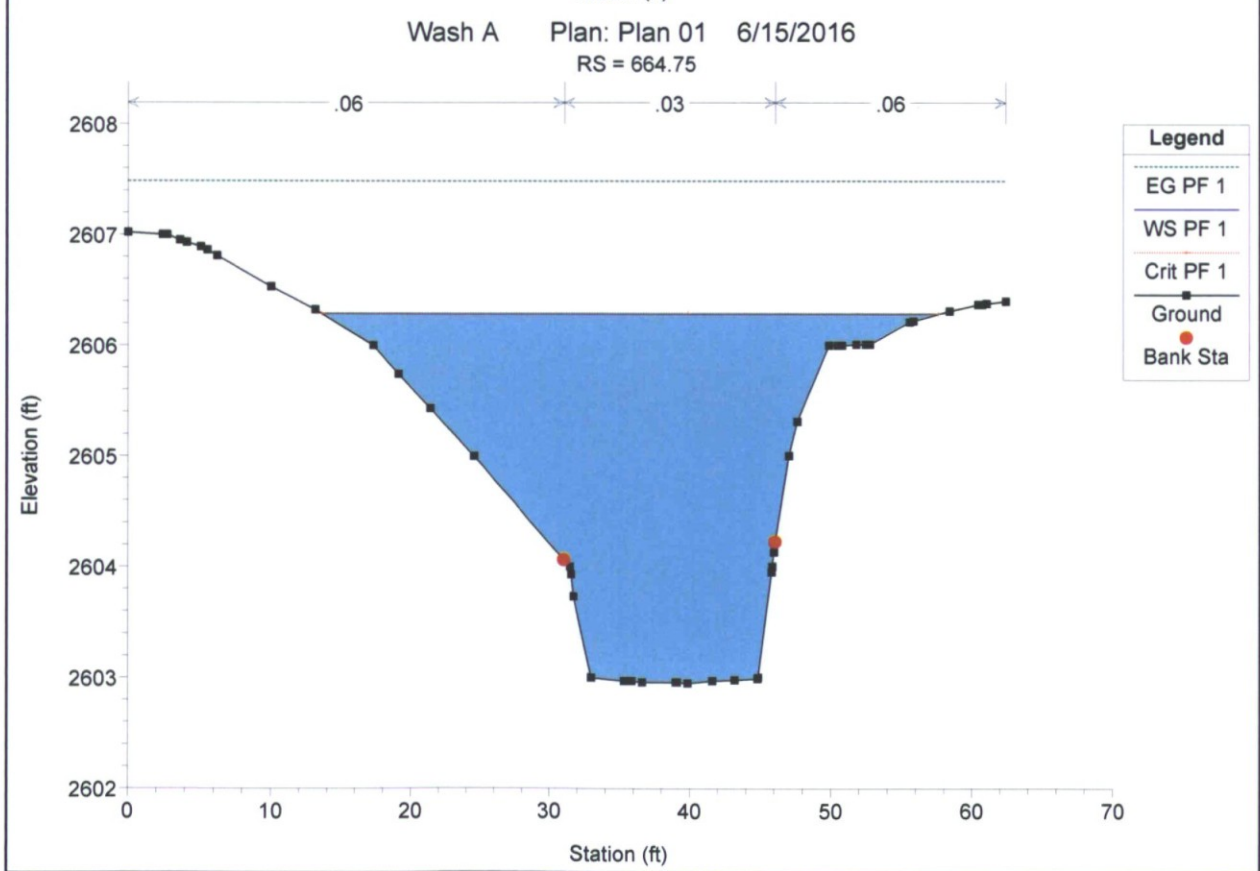
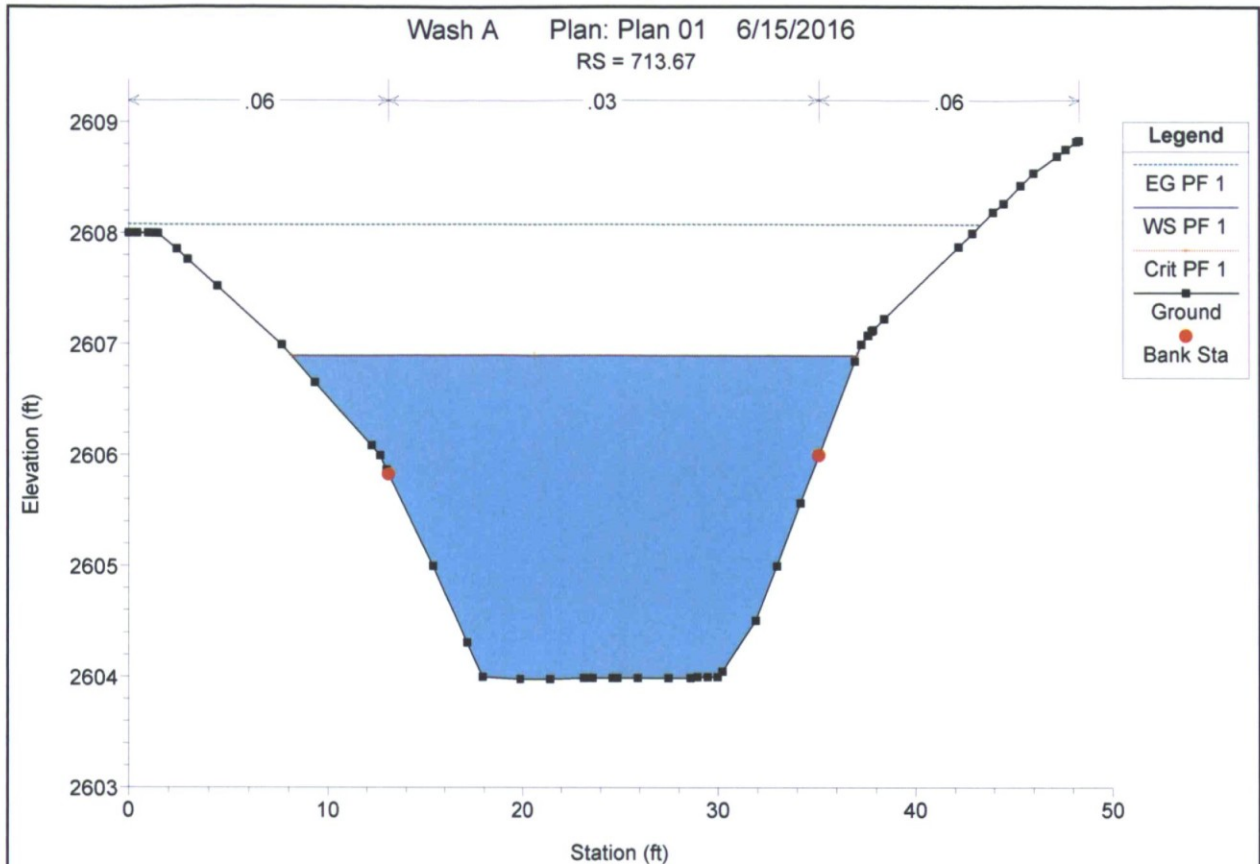


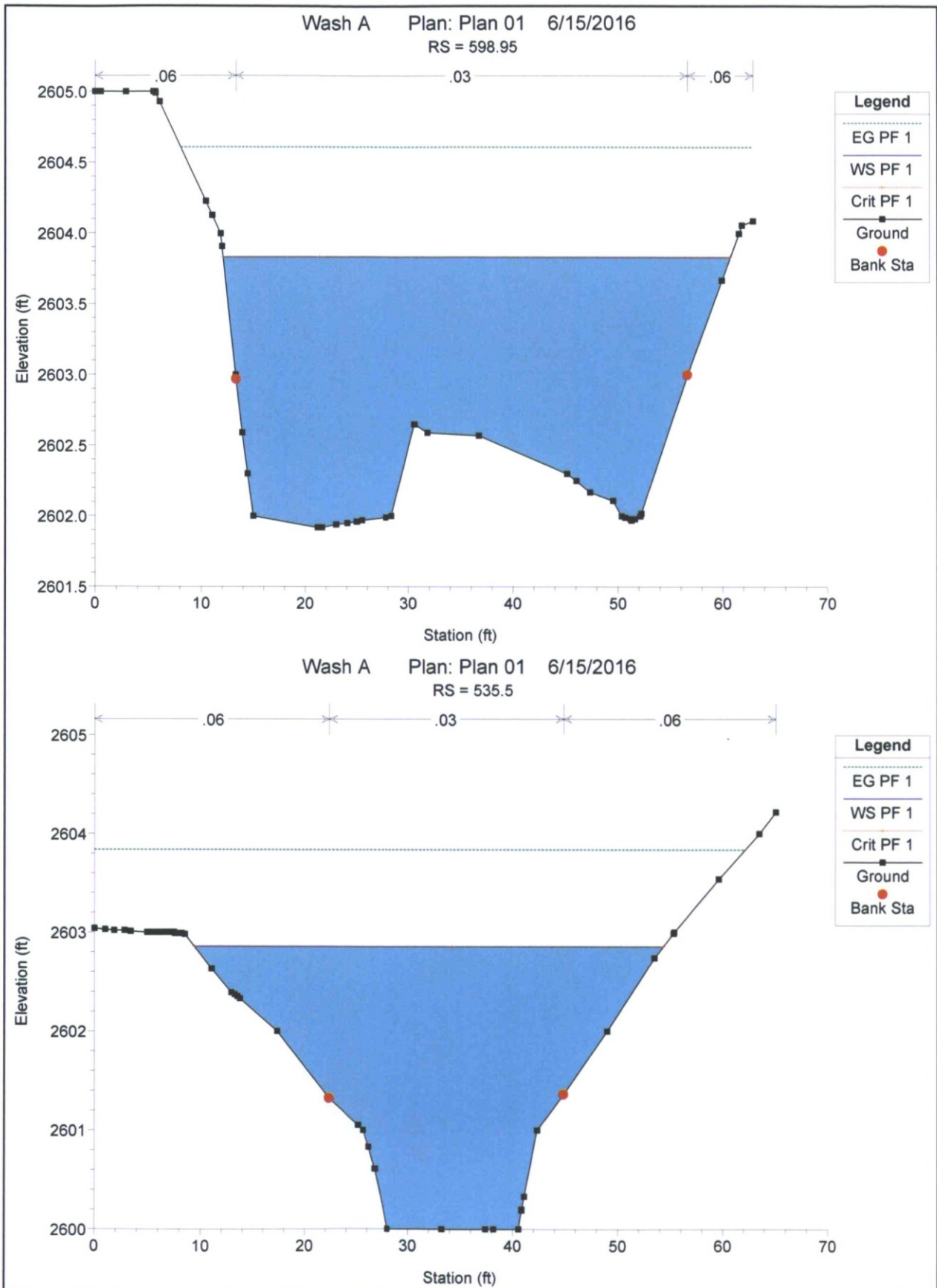


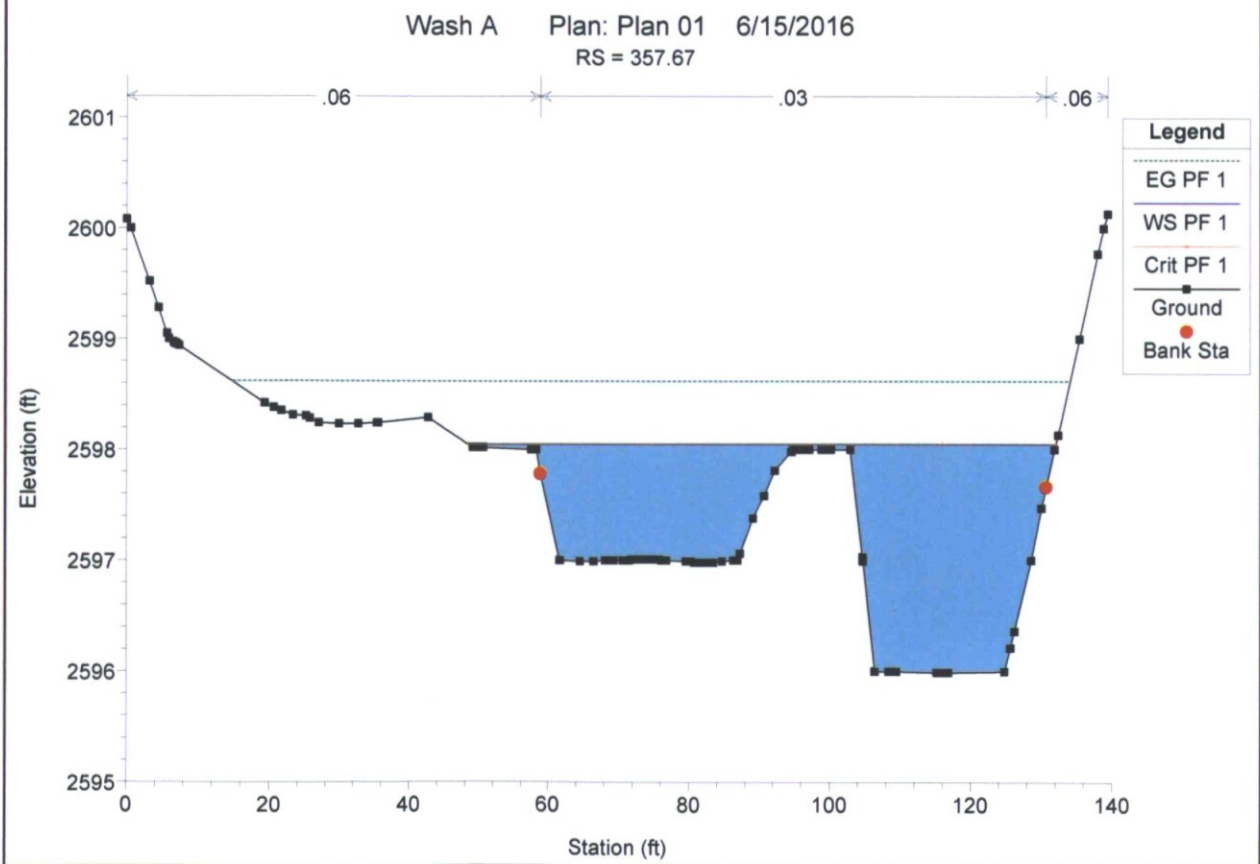
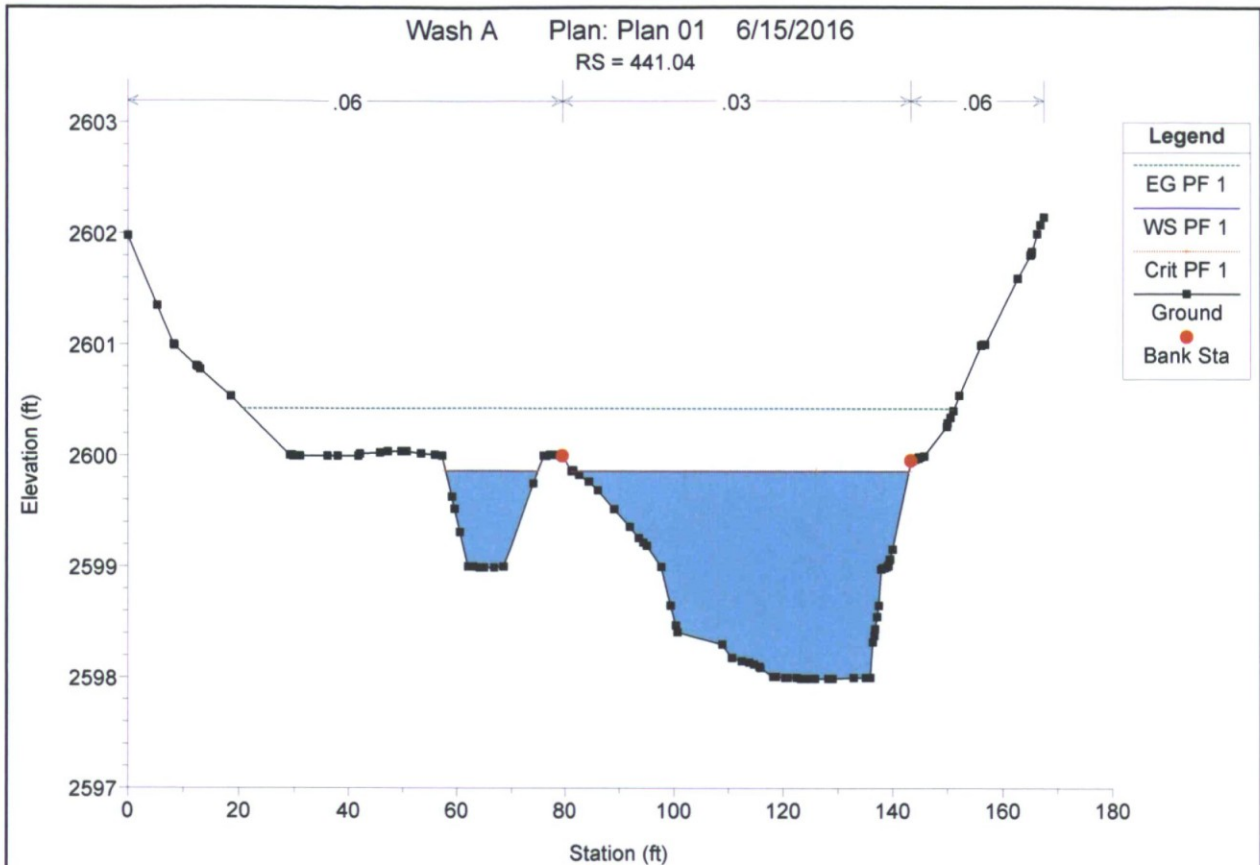


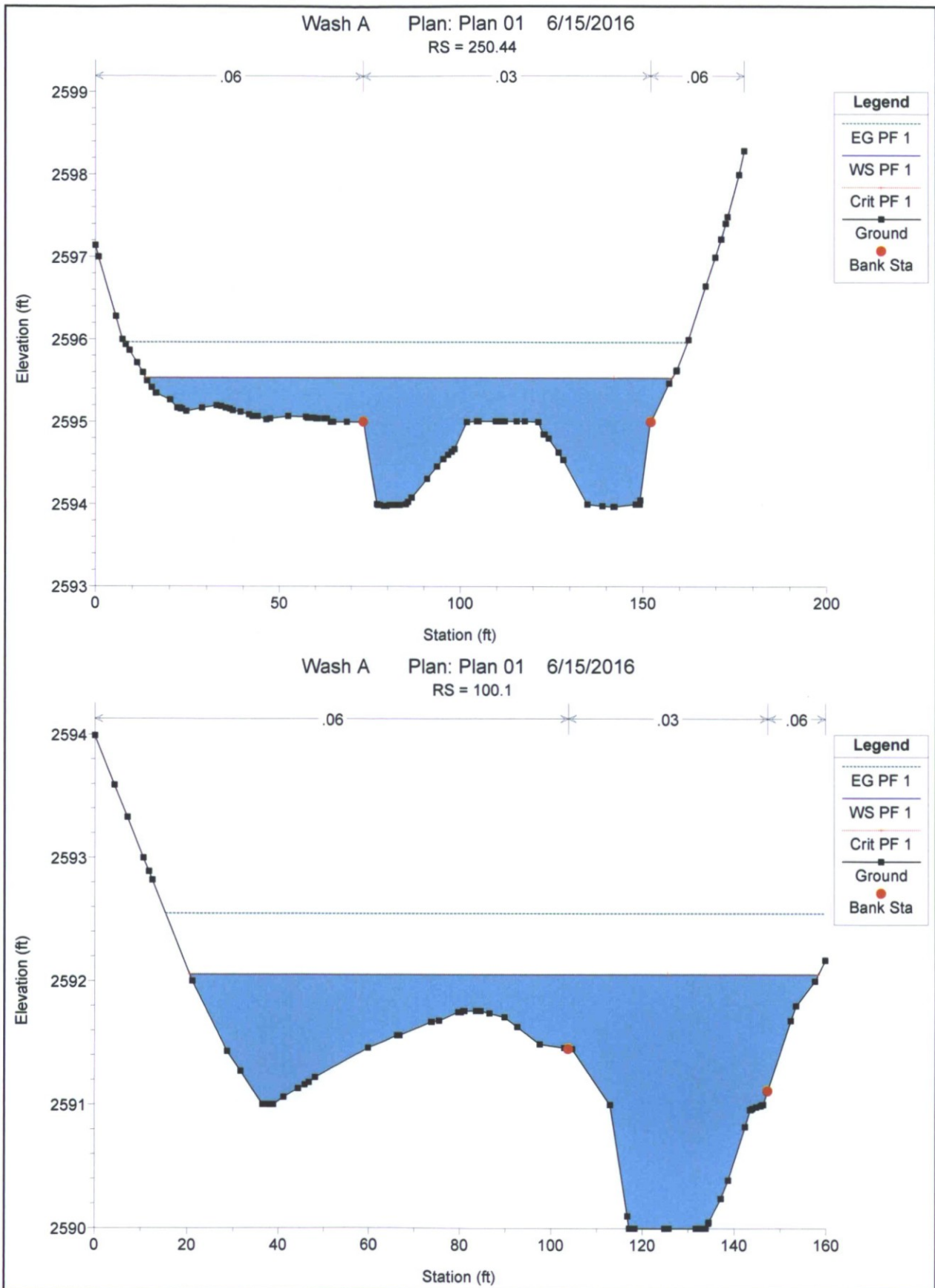












HEC-RAS Plan: Plan 01 River: NorthWash Reach: Alignment - Nort Profile: PF 1

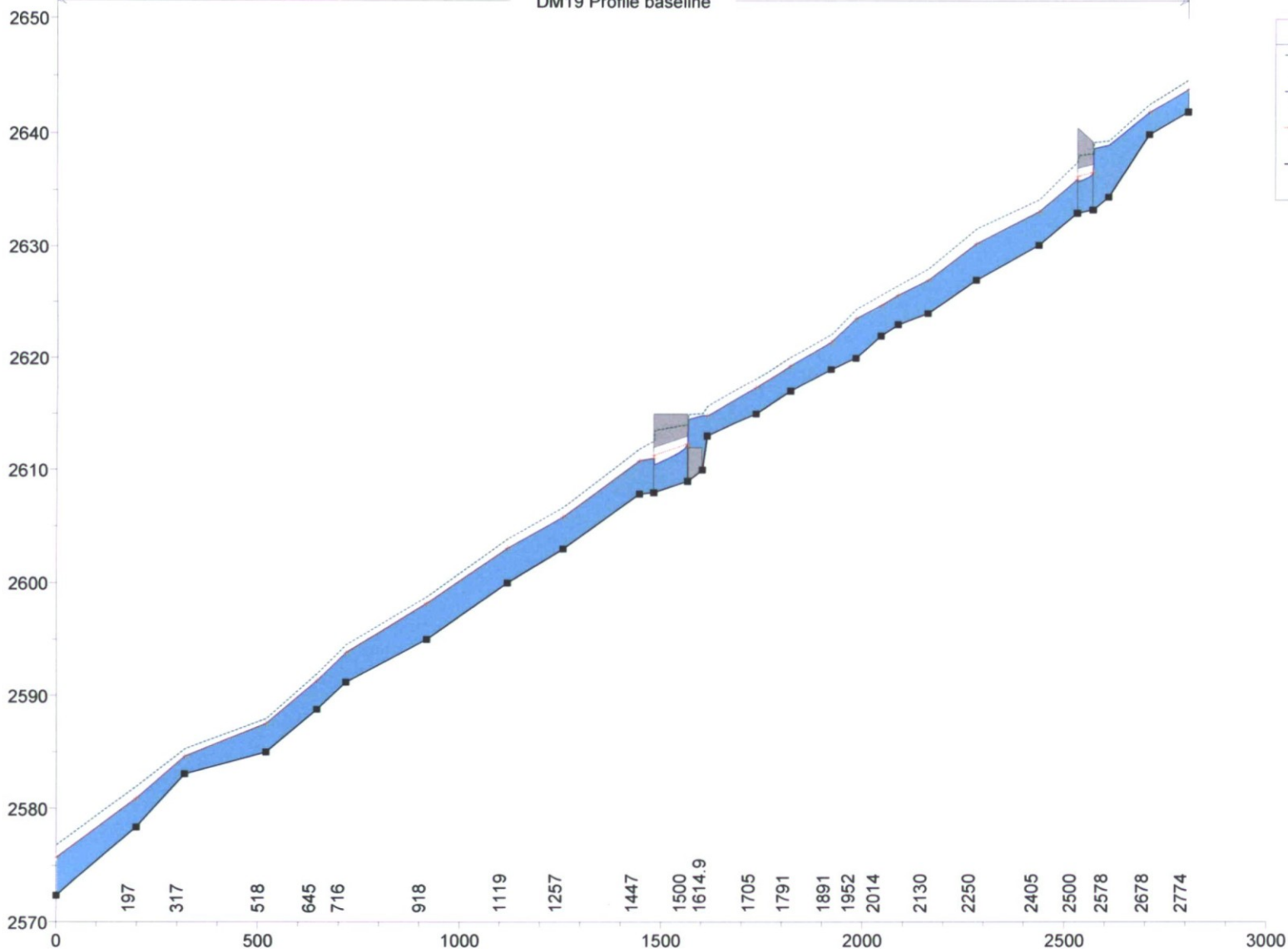
Reach	River Sta	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Alignment - Nort	1168.17	472.00	2616.97	2619.25	2619.25	2620.16	0.009615	7.77	66.58	40.60	0.96
Alignment - Nort	1080.96	472.00	2612.00	2615.63	2615.63	2616.74	0.007289	9.15	73.61	36.33	0.90
Alignment - Nort	1013.7	472.00	2610.97	2613.40	2613.40	2614.41	0.009090	8.13	63.57	40.35	0.95
Alignment - Nort	943.36	480.00	2610.00	2612.53	2612.53	2613.47	0.008585	7.89	70.29	53.67	0.92
Alignment - Nort	875.49	480.00	2607.95	2610.34	2610.34	2610.97	0.011524	6.41	79.21	71.47	0.97
Alignment - Nort	826.23	480.00	2606.97	2609.47	2609.47	2610.22	0.011584	6.96	71.02	51.19	0.98
Alignment - Nort	778.43	480.00	2605.97	2608.56		2608.88	0.002981	4.79	125.60	66.45	0.55
Alignment - Nort	756.56	480.00	2604.99	2607.89	2607.89	2608.72	0.008770	8.59	91.77	57.49	0.94
Alignment - Nort	713.67	485.00	2603.98	2606.90	2606.90	2608.08	0.009615	8.74	58.25	28.88	0.98
Alignment - Nort	664.75	485.00	2602.95	2606.28	2606.28	2607.48	0.007889	9.19	70.56	44.05	0.91
Alignment - Nort	598.95	485.00	2601.92	2603.83	2603.83	2604.61	0.011333	7.08	70.34	48.65	0.99
Alignment - Nort	535.5	485.00	2600.00	2602.85	2602.85	2603.84	0.008559	8.21	72.00	44.88	0.92
Alignment - Nort	441.04	490.00	2597.99	2599.86	2599.86	2600.43	0.011514	6.13	87.05	78.67	0.97
Alignment - Nort	357.67	490.00	2595.99	2598.05	2598.05	2598.61	0.012877	6.04	81.71	83.58	1.00
Alignment - Nort	250.44	495.00	2593.97	2595.54	2595.54	2595.97	0.011294	5.46	110.14	144.51	0.93
Alignment - Nort	100.1	495.00	2590.00	2592.05	2592.05	2592.55	0.008482	6.12	119.82	137.73	0.86

**POST-DEVELOPMENT**

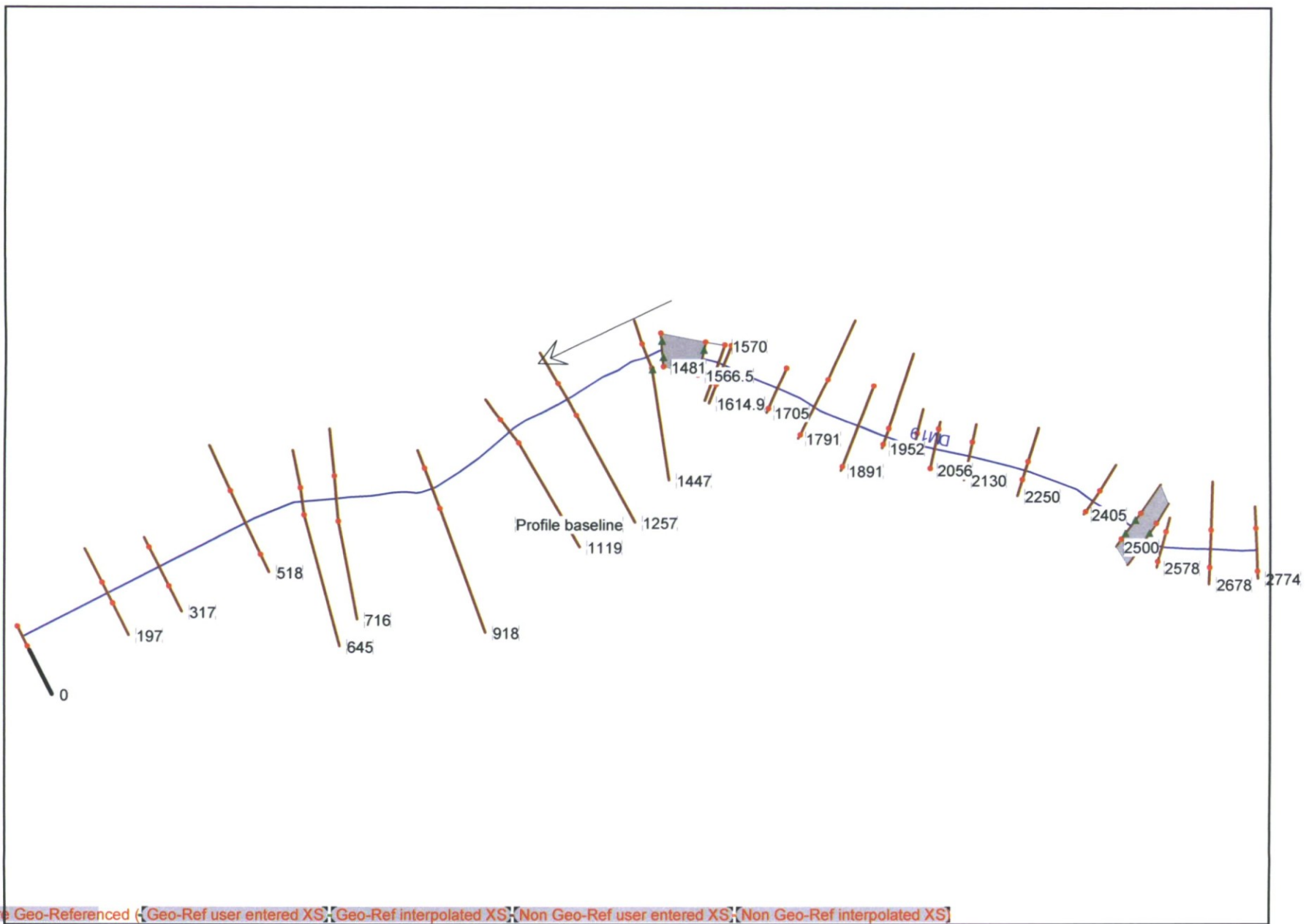
**GALLOWAY WASH**

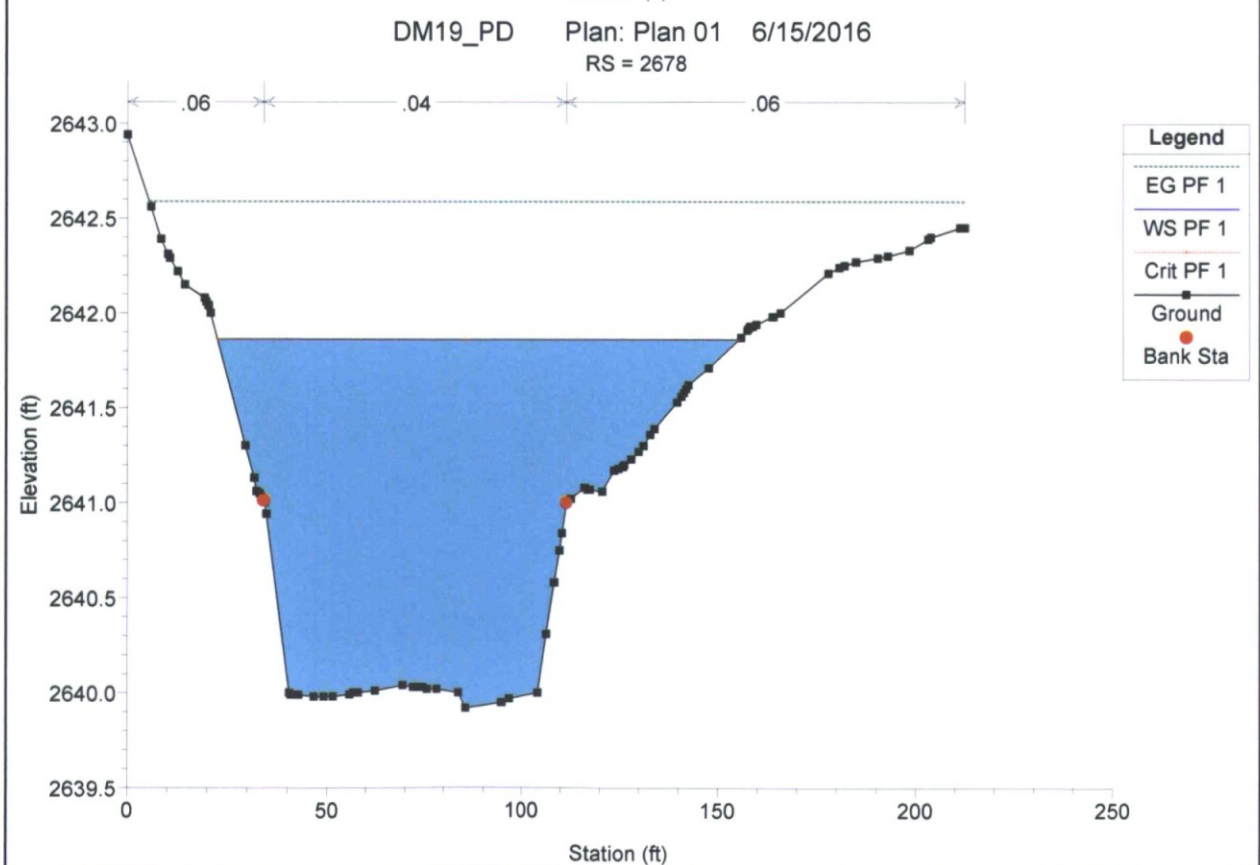
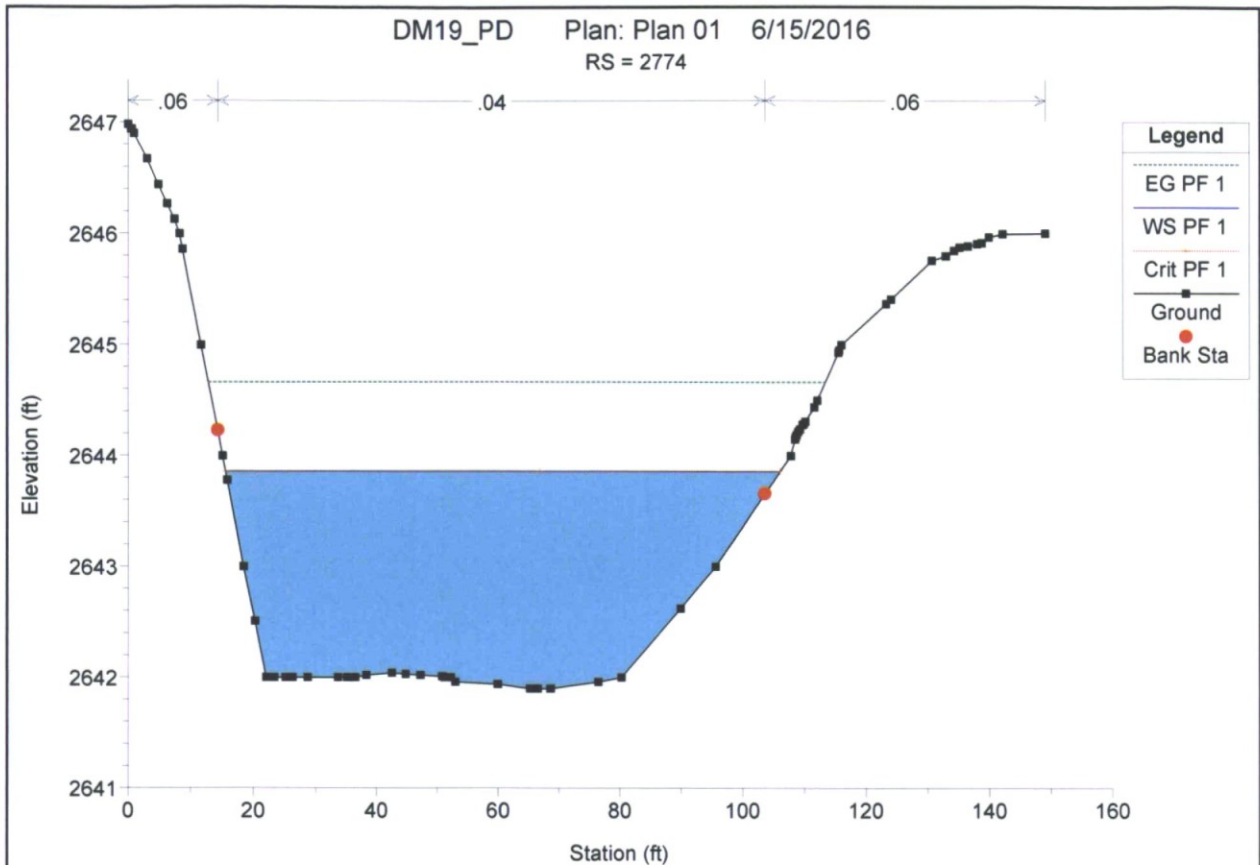
DM19 Profile baseline

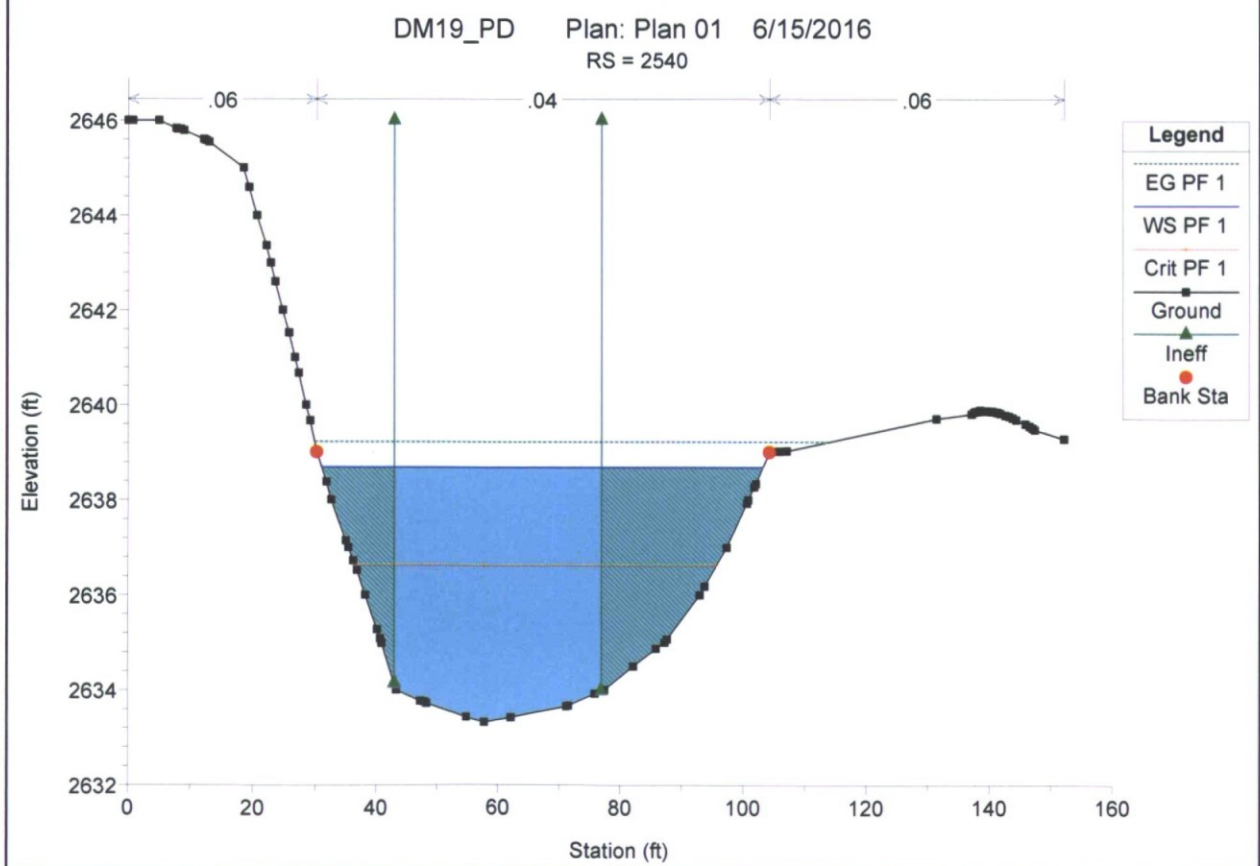
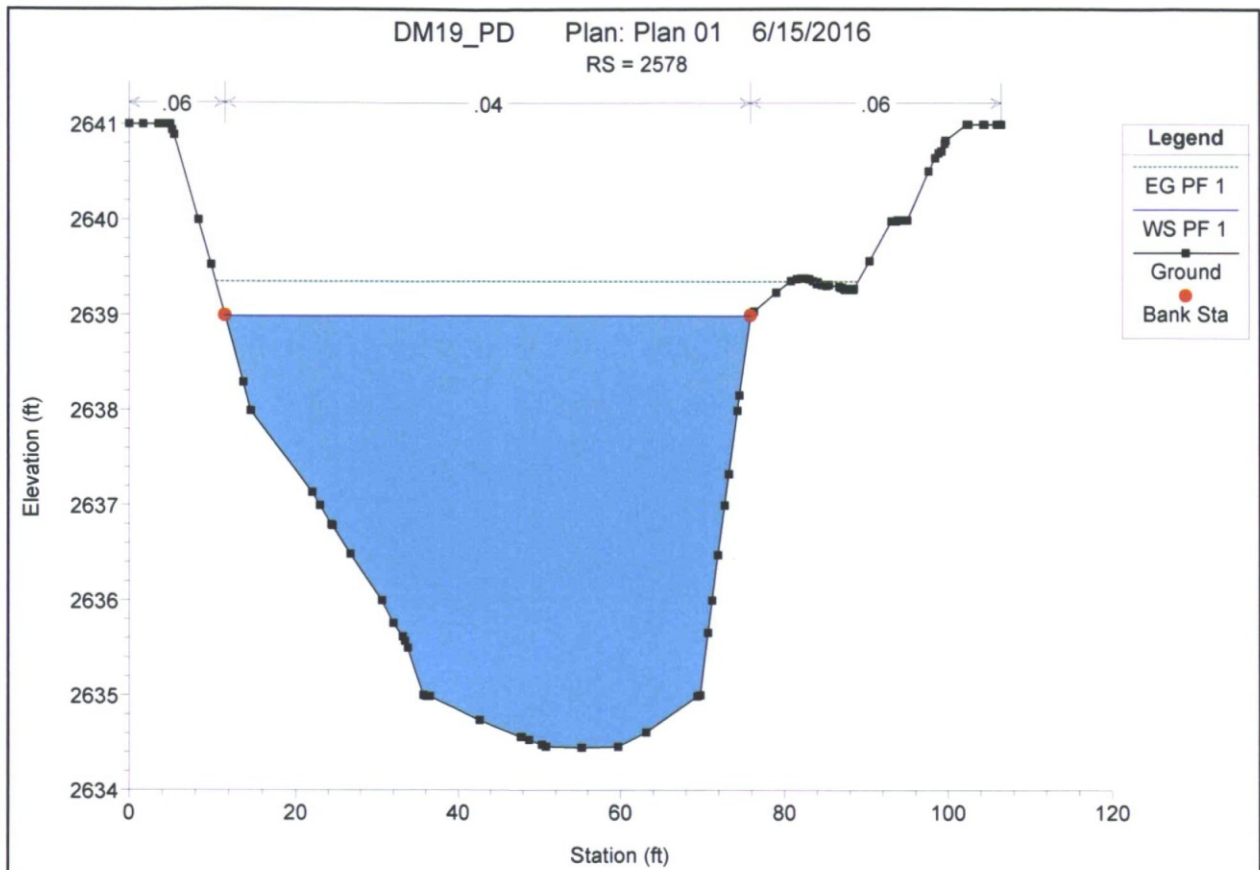
Elevation (ft)



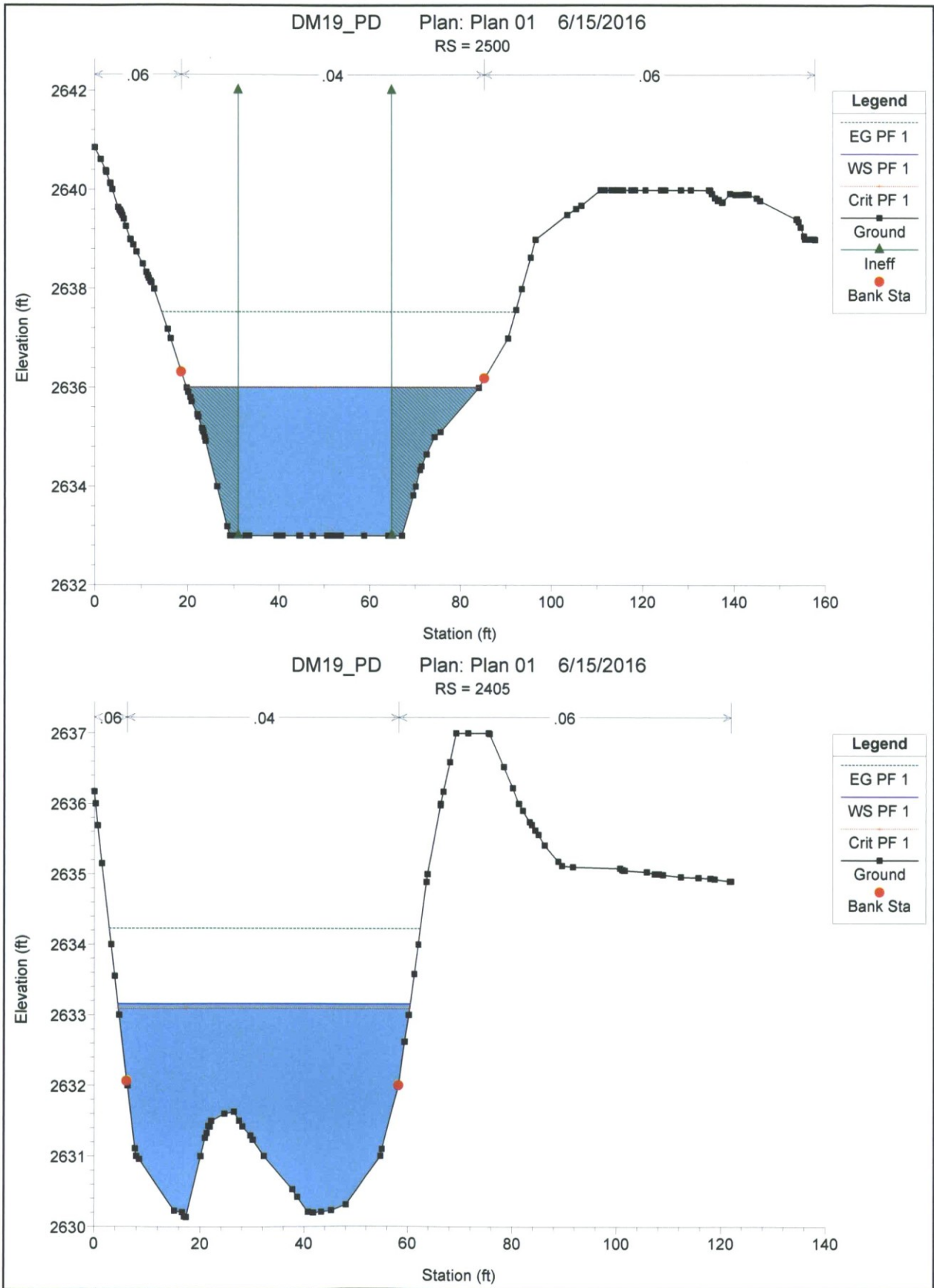
Legend	
EG PF 1	(Dotted line)
WS PF 1	(Solid blue line)
Crit PF 1	(Dashed line)
Ground	(Solid black line with square markers)

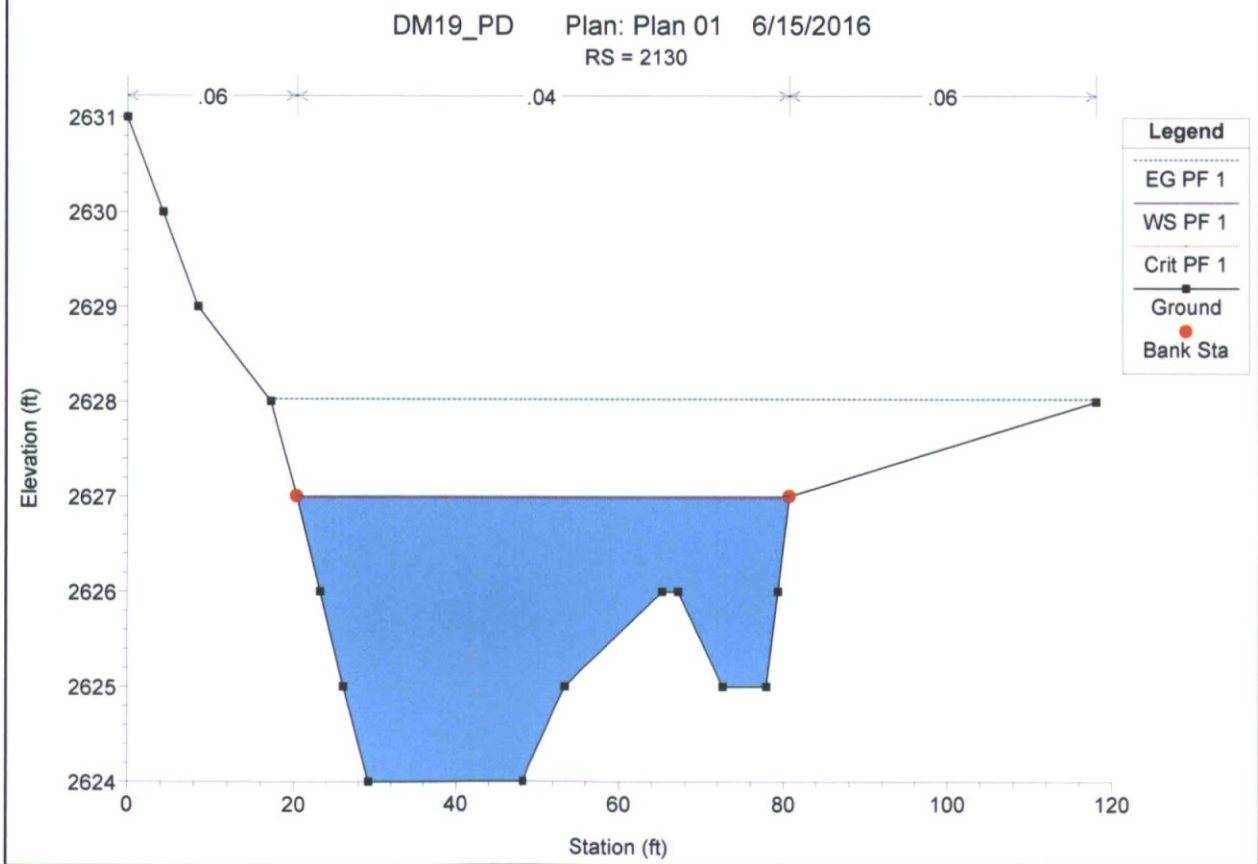
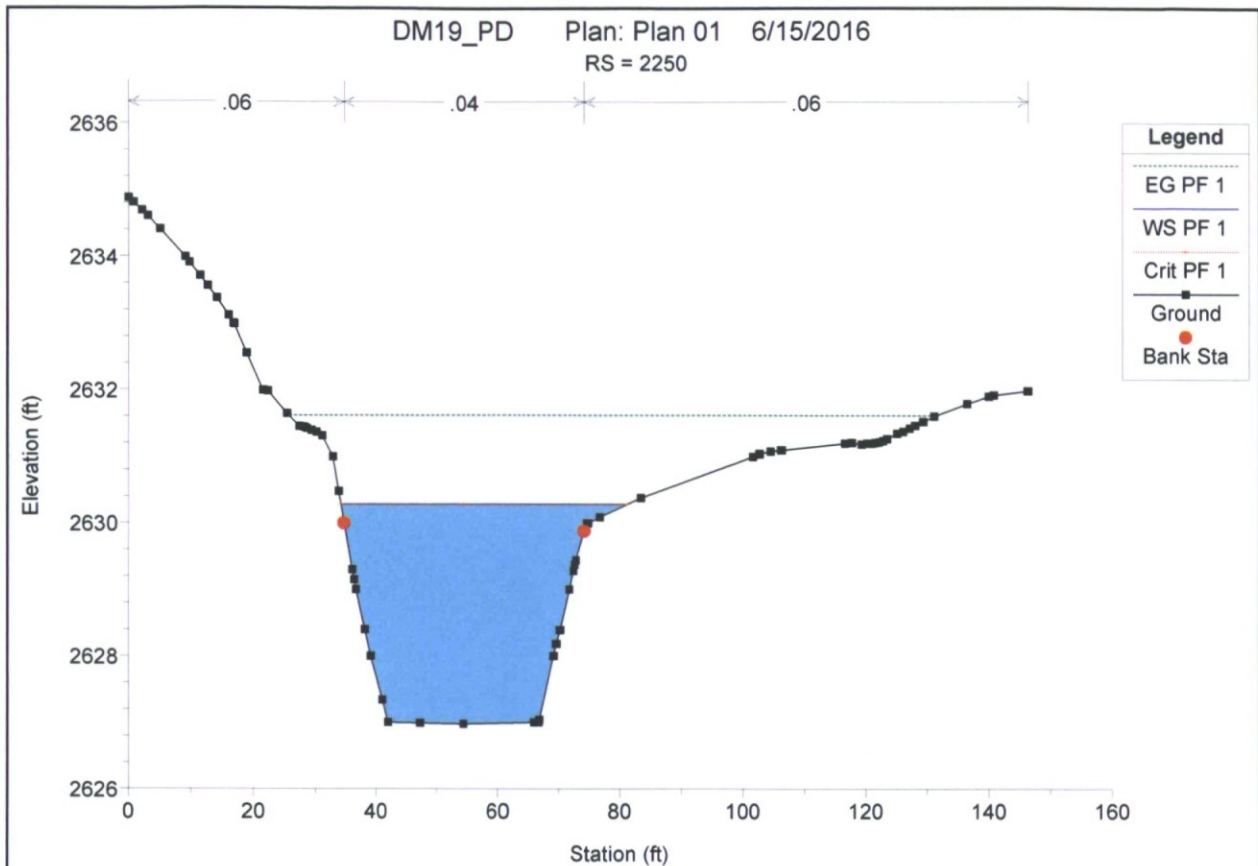


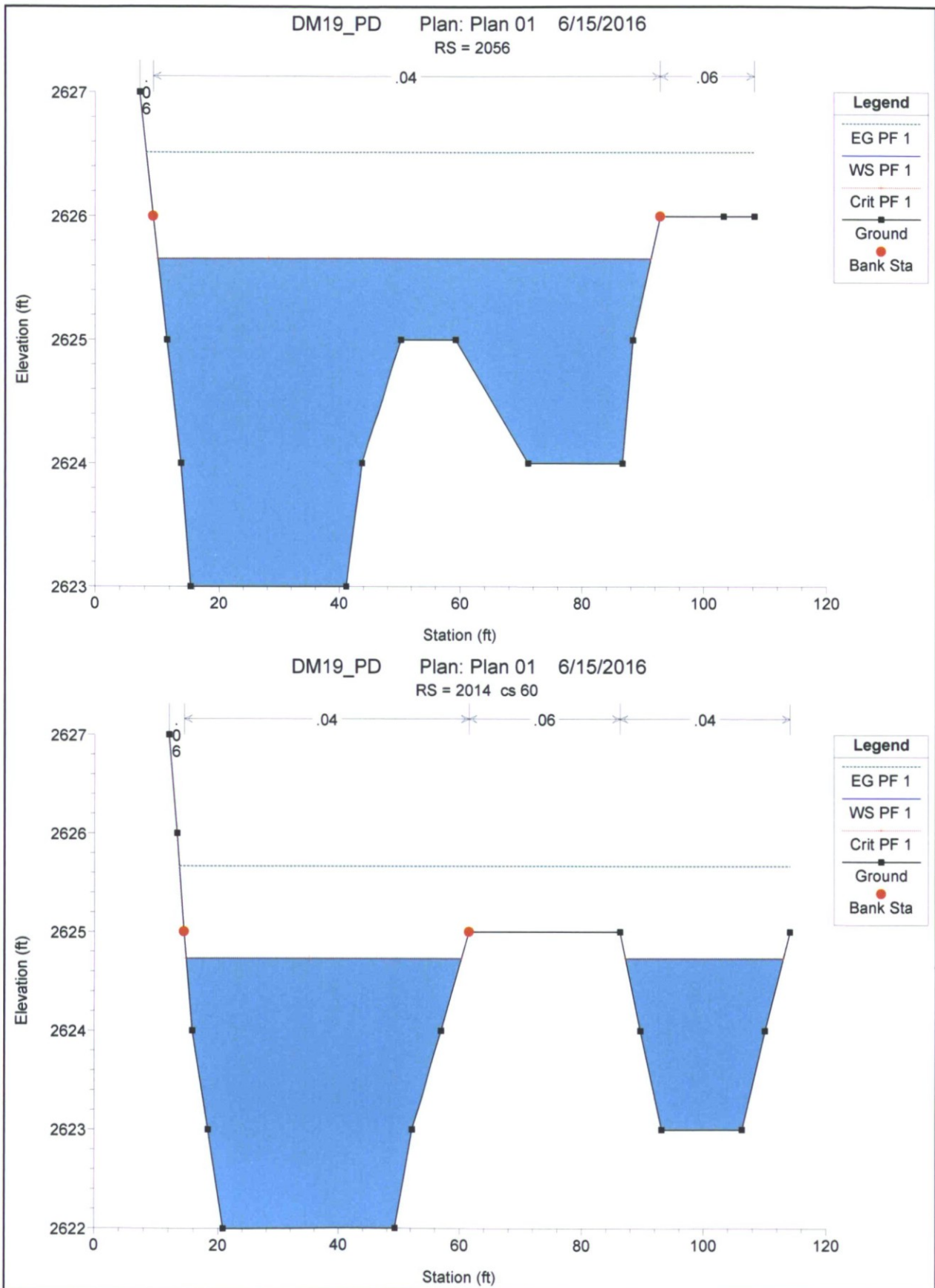


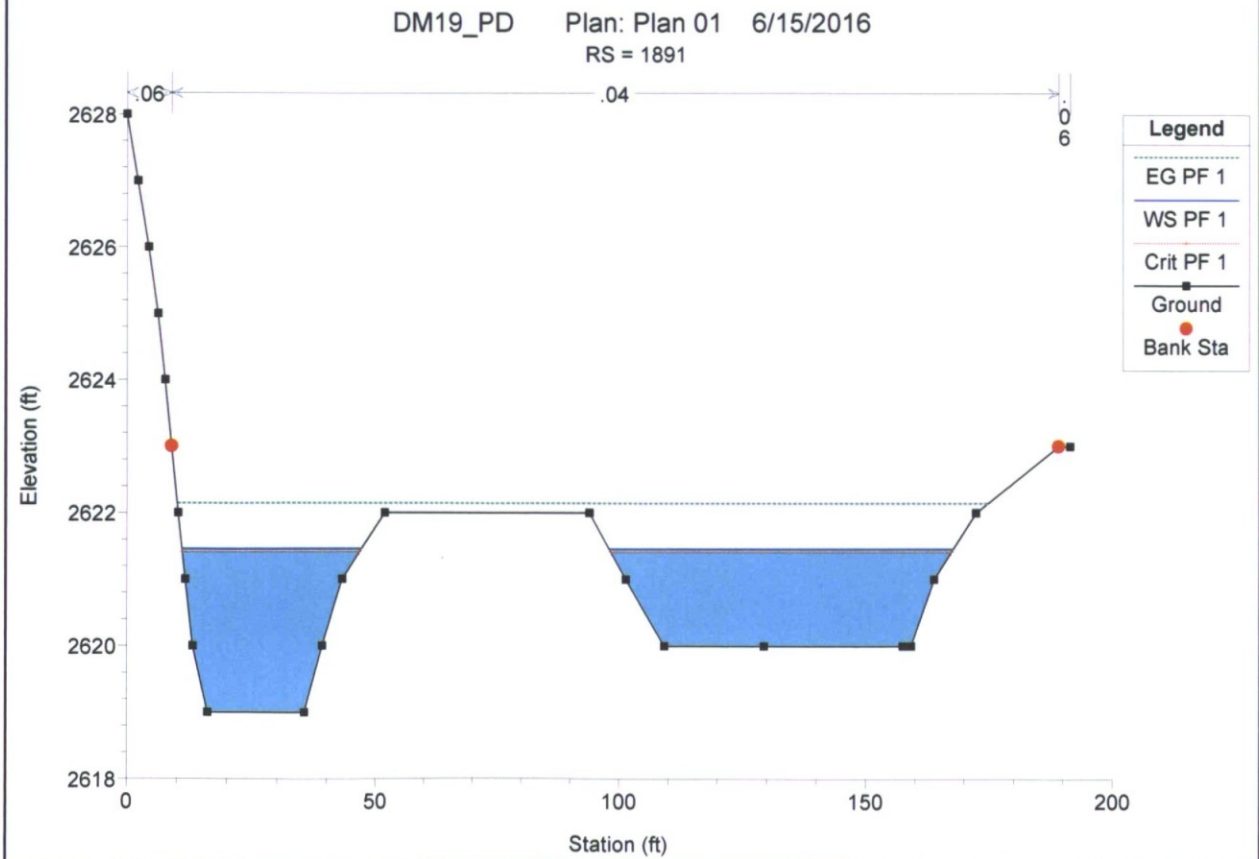
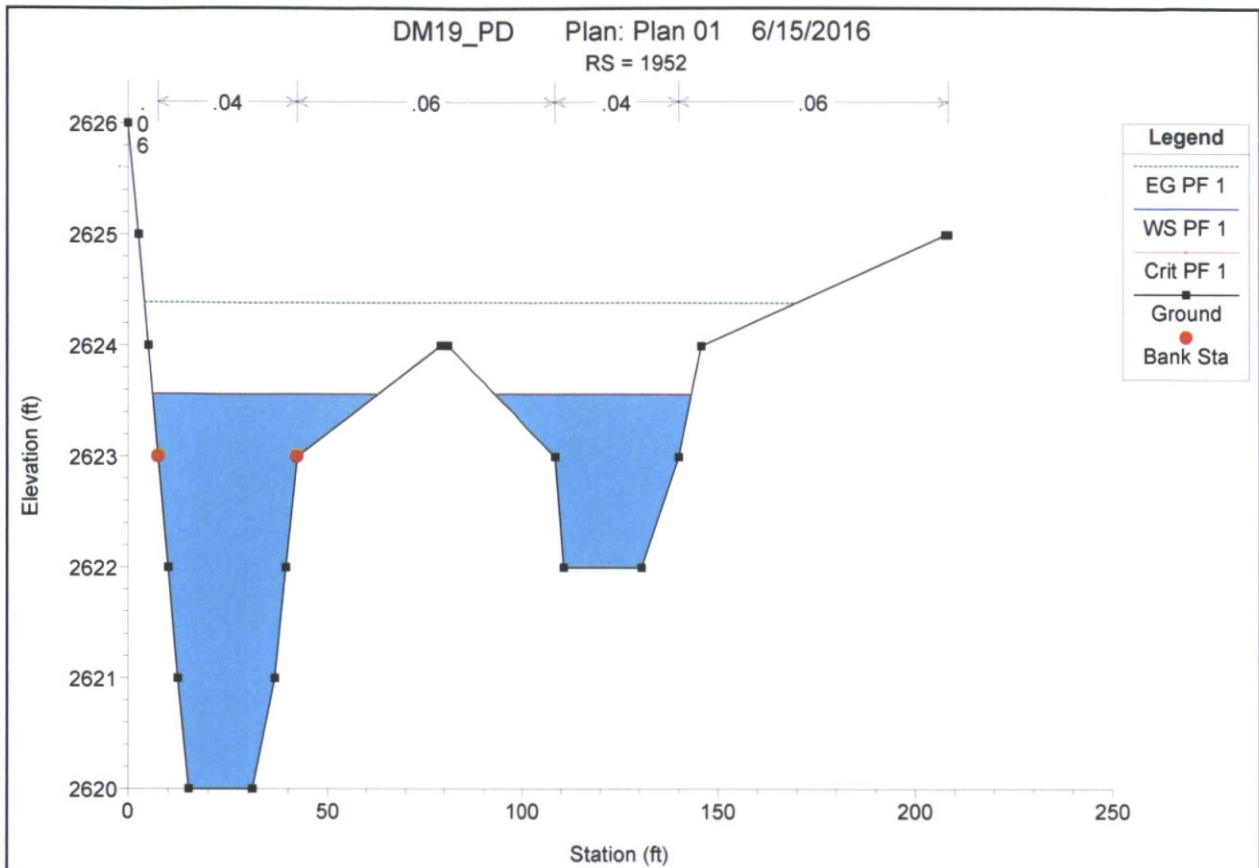




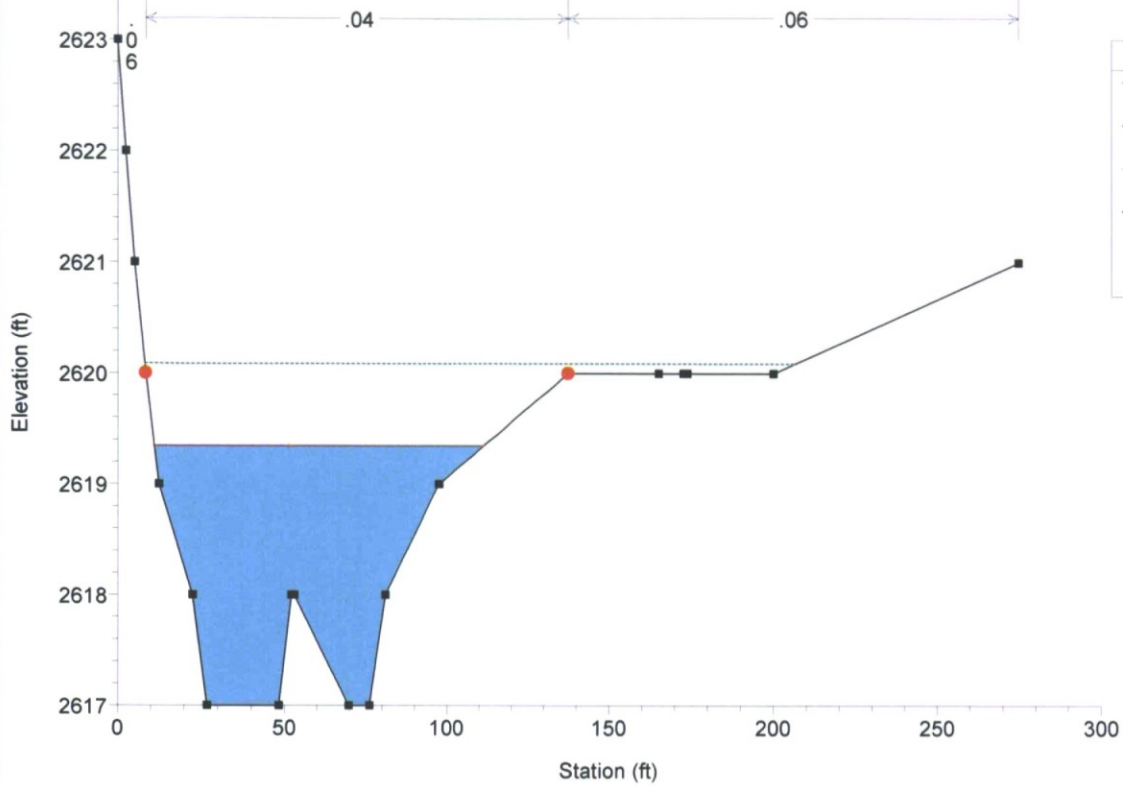




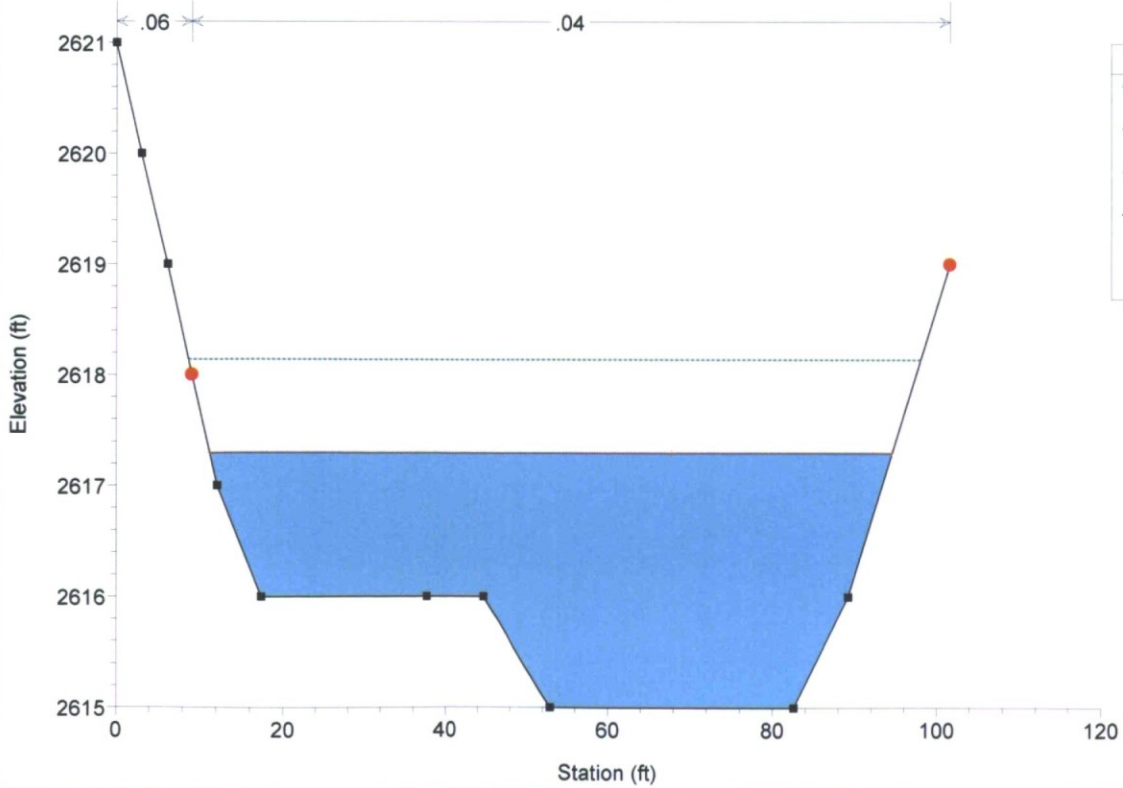


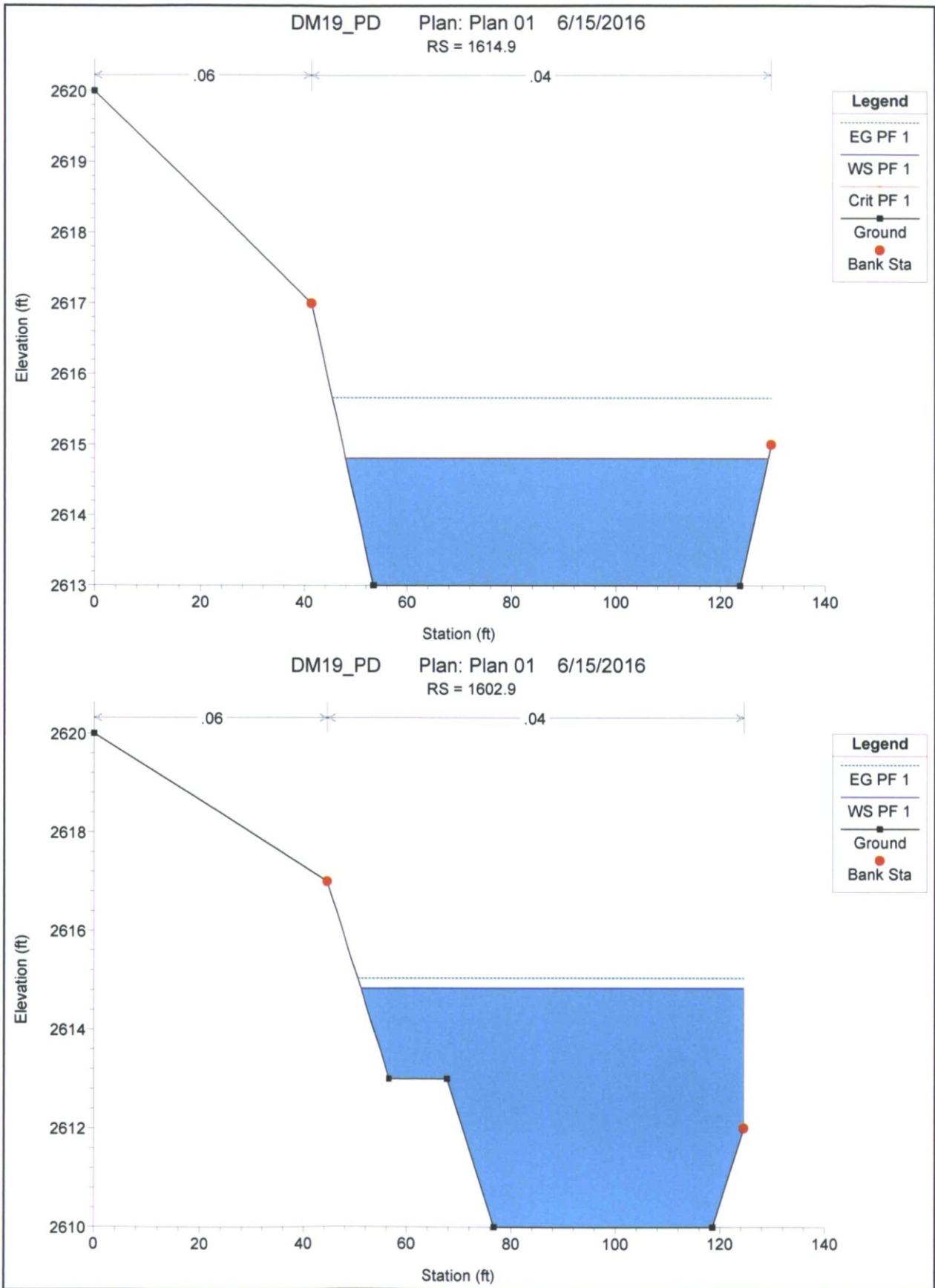


DM19\_PD Plan: Plan 01 6/15/2016  
RS = 1791

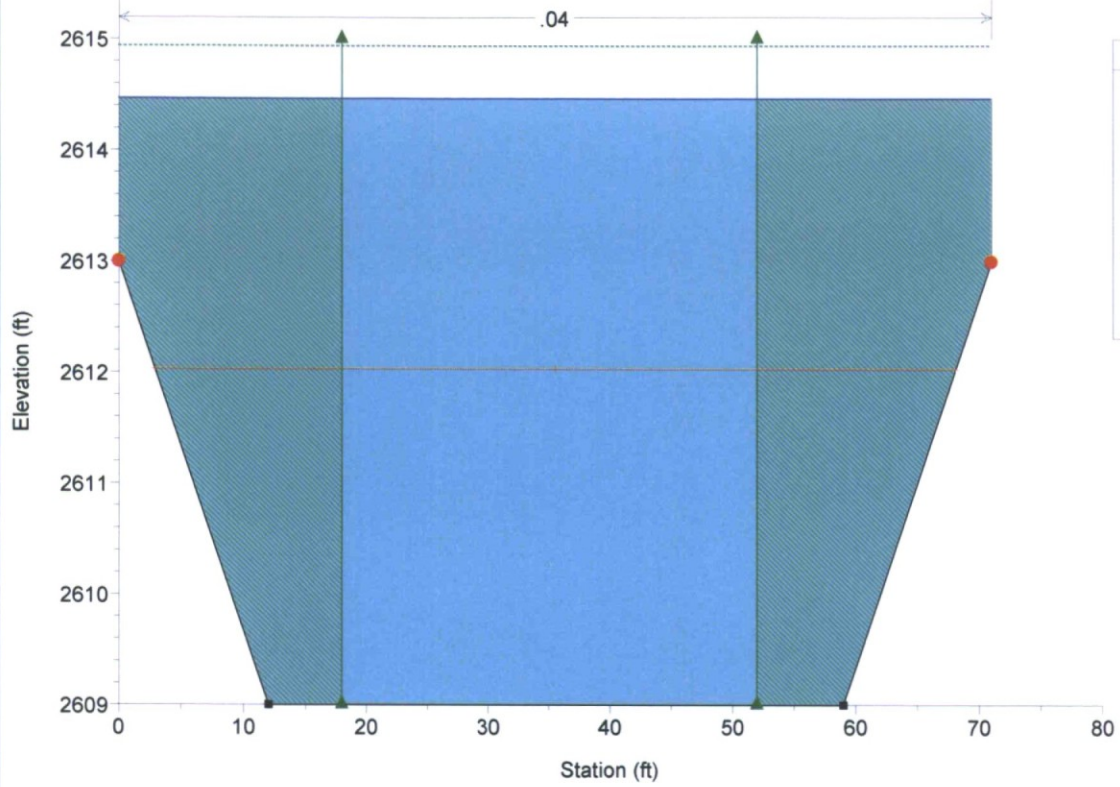


DM19\_PD Plan: Plan 01 6/15/2016  
RS = 1705

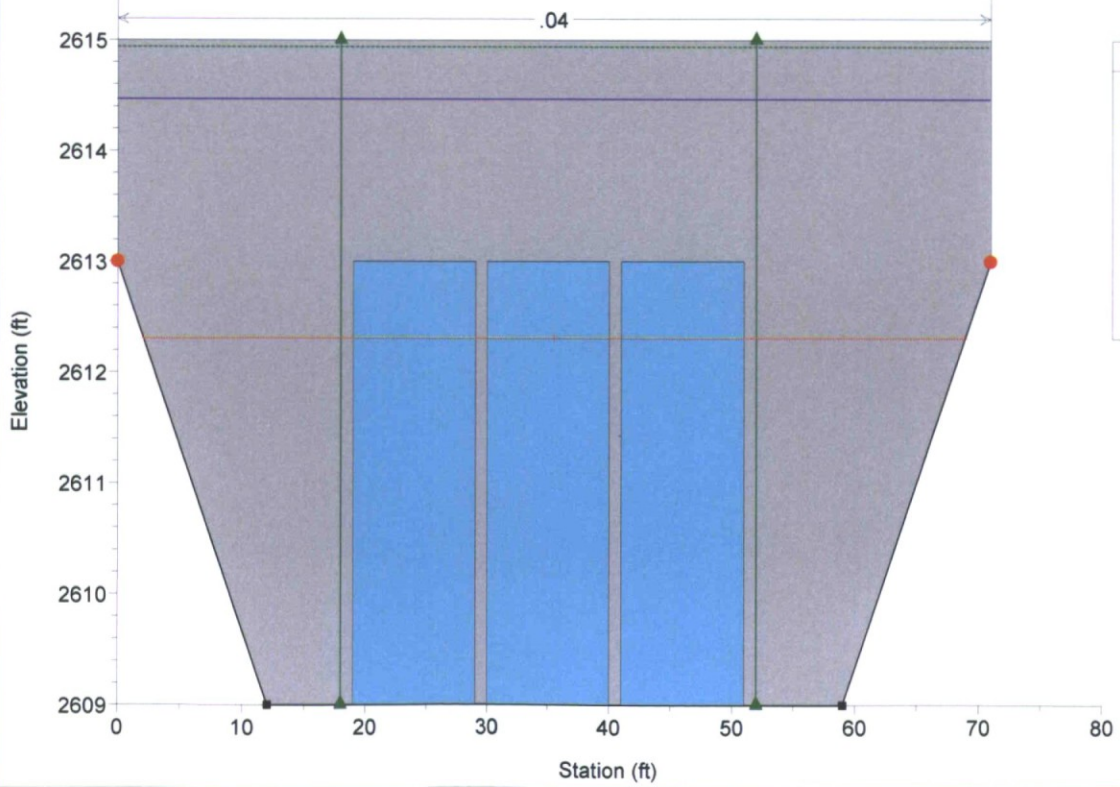




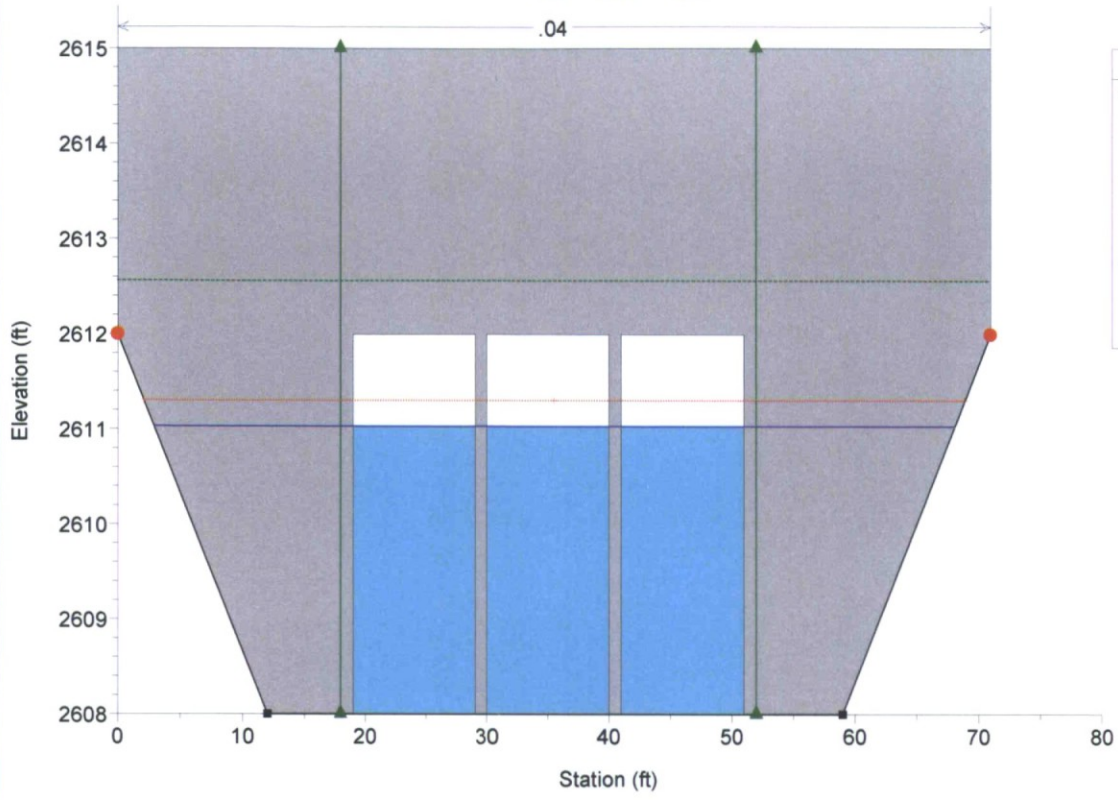
DM19\_PD Plan: Plan 01 6/15/2016  
RS = 1566.5



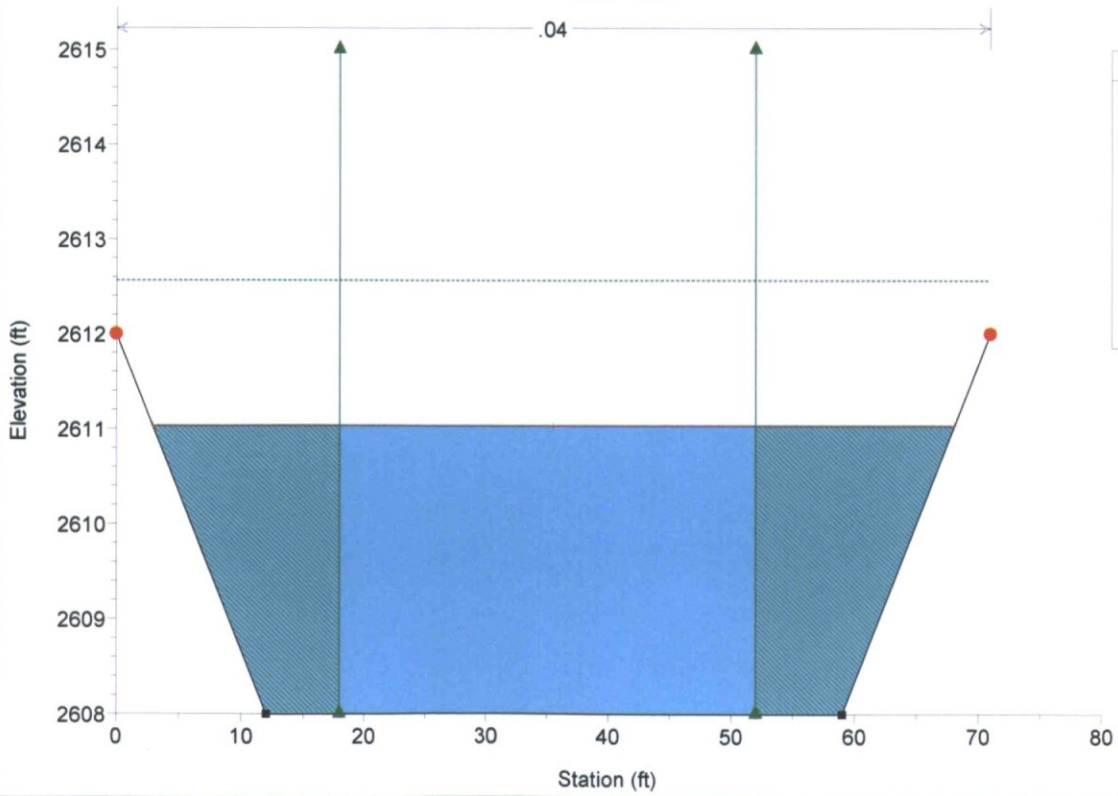
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RS = 1500 Culv



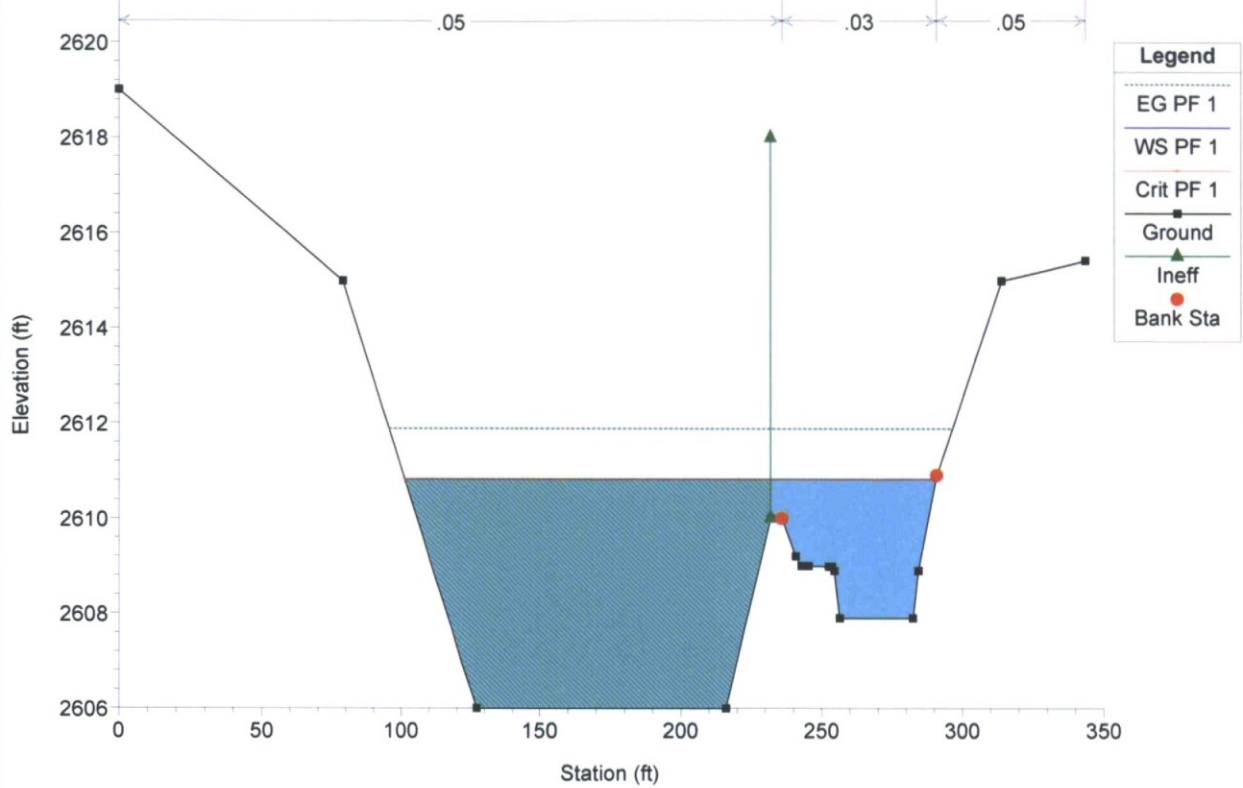
DM19\_PD Plan: Plan 01 6/15/2016  
RS = 1500 Culv



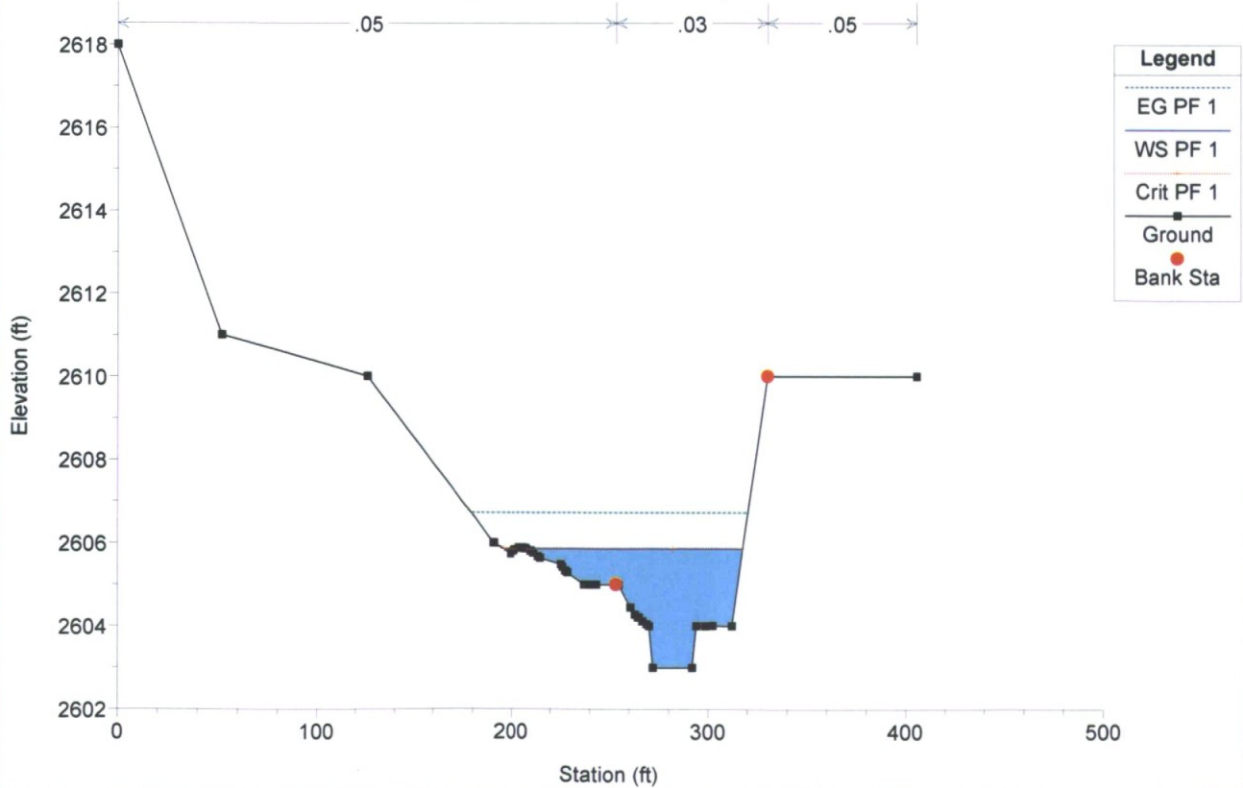
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RS = 1481



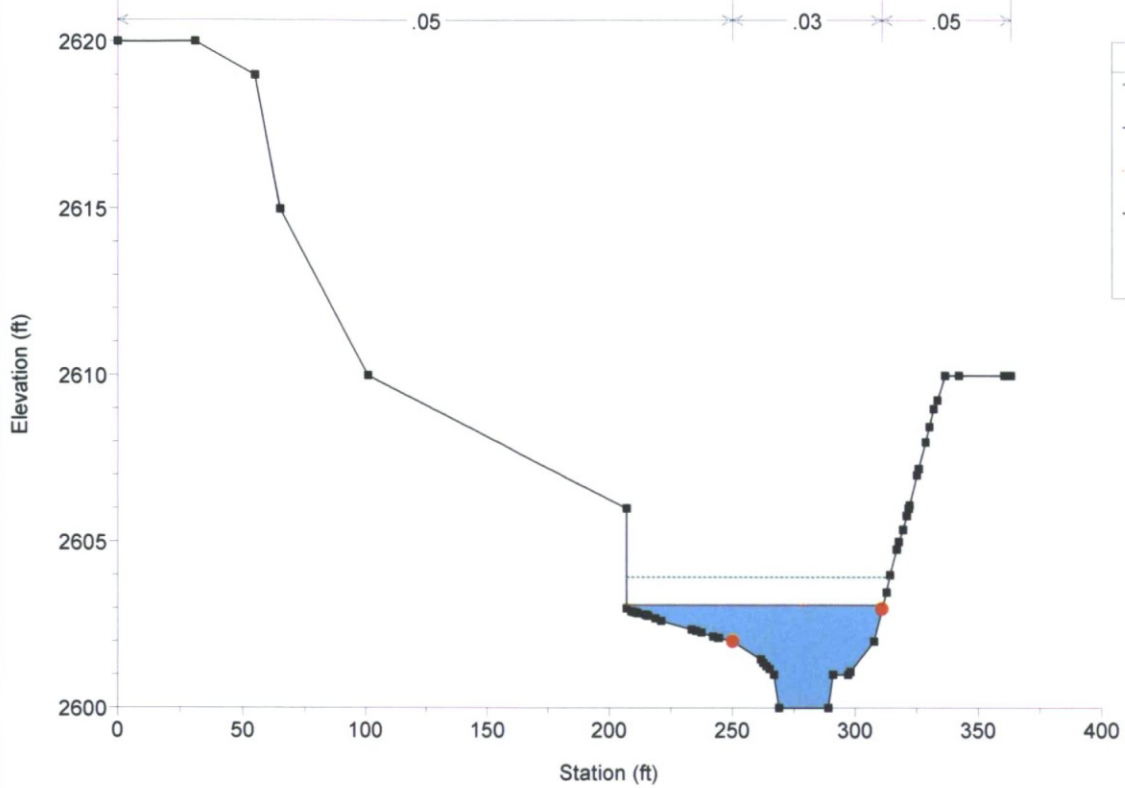
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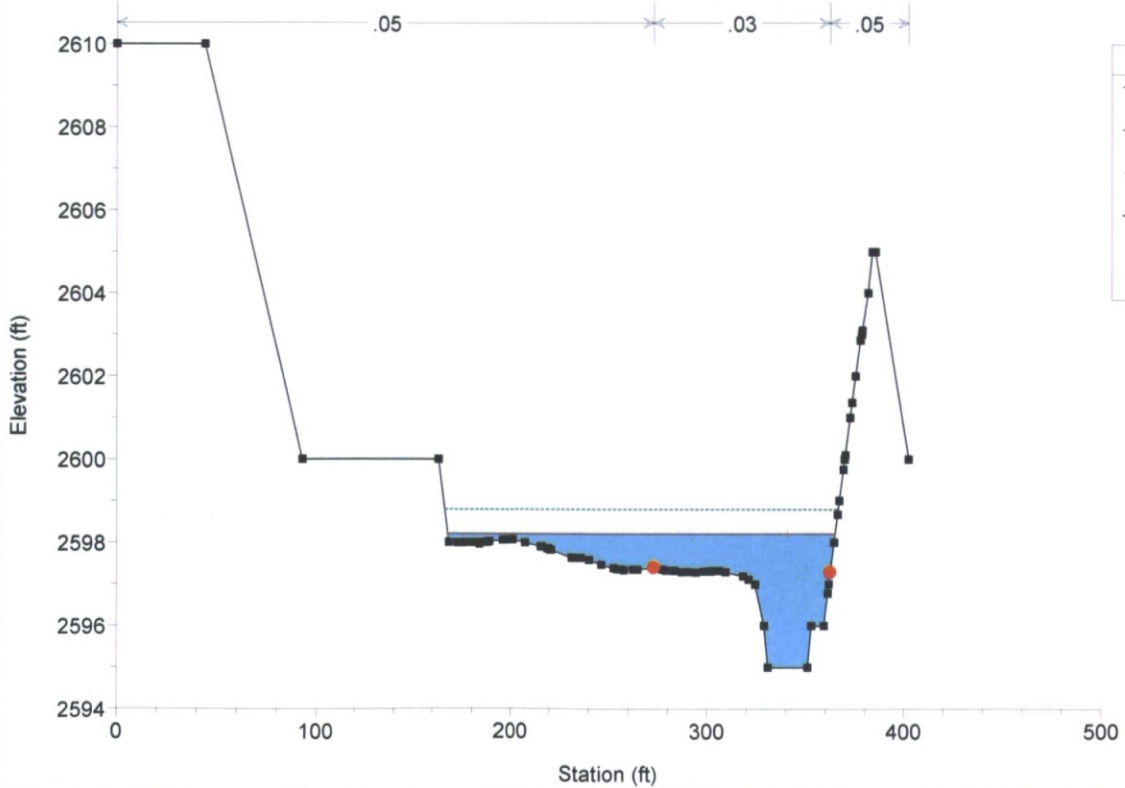
DM19\_PD Plan: Plan 01 6/15/2016  
 RS = 1257 Updated CS with new 1 ft topo = FEMA CS 4.796



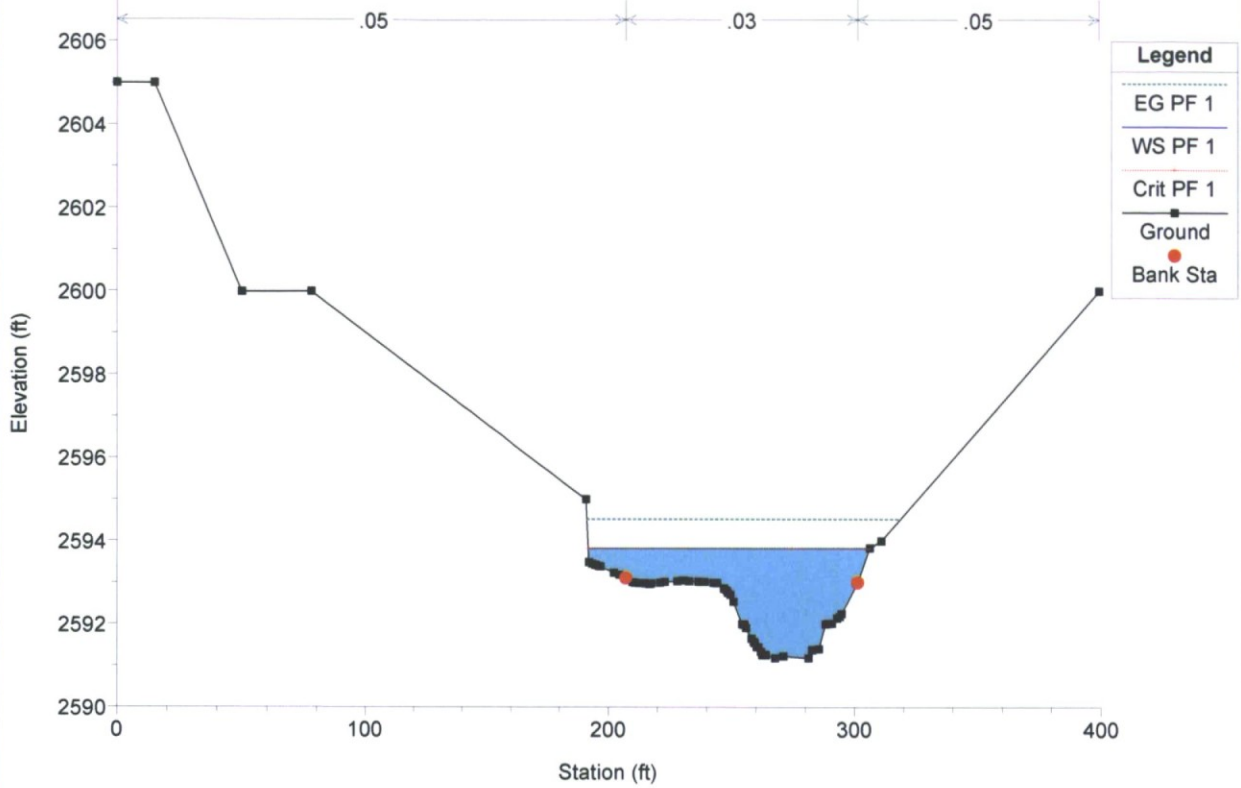
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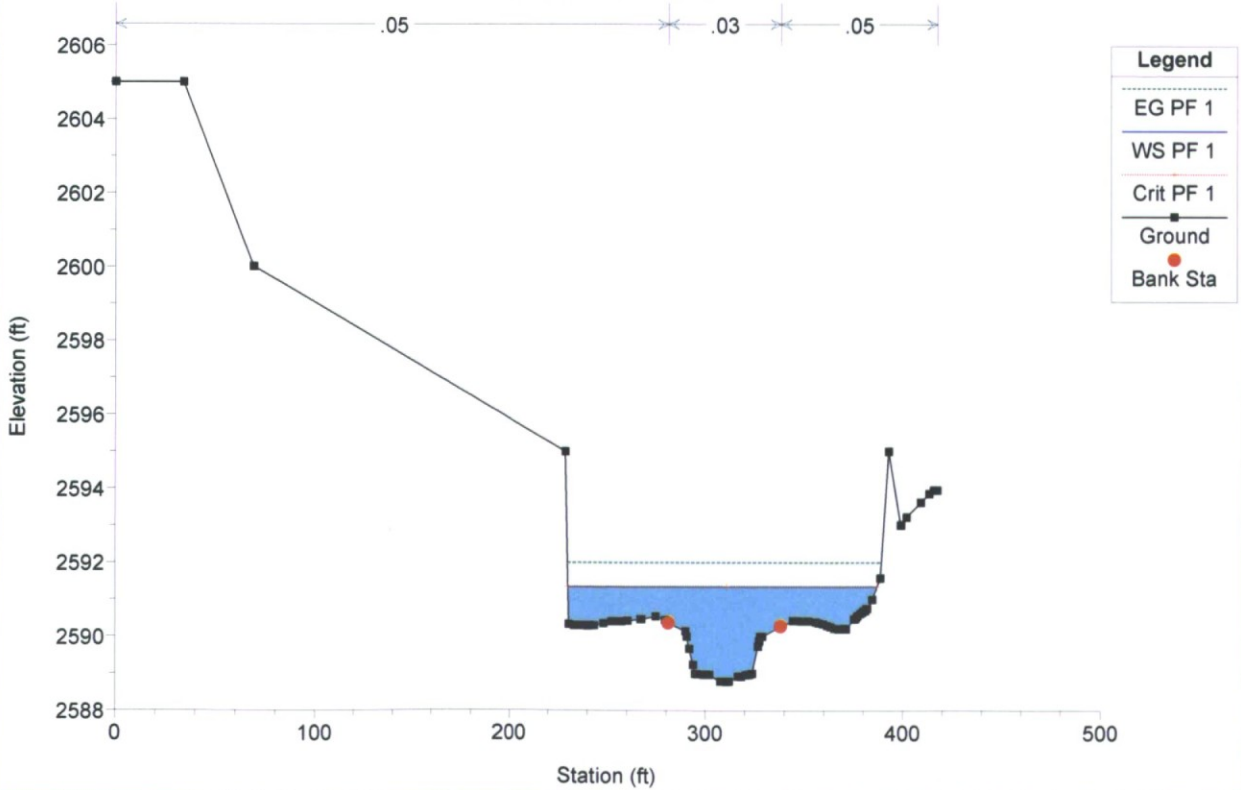
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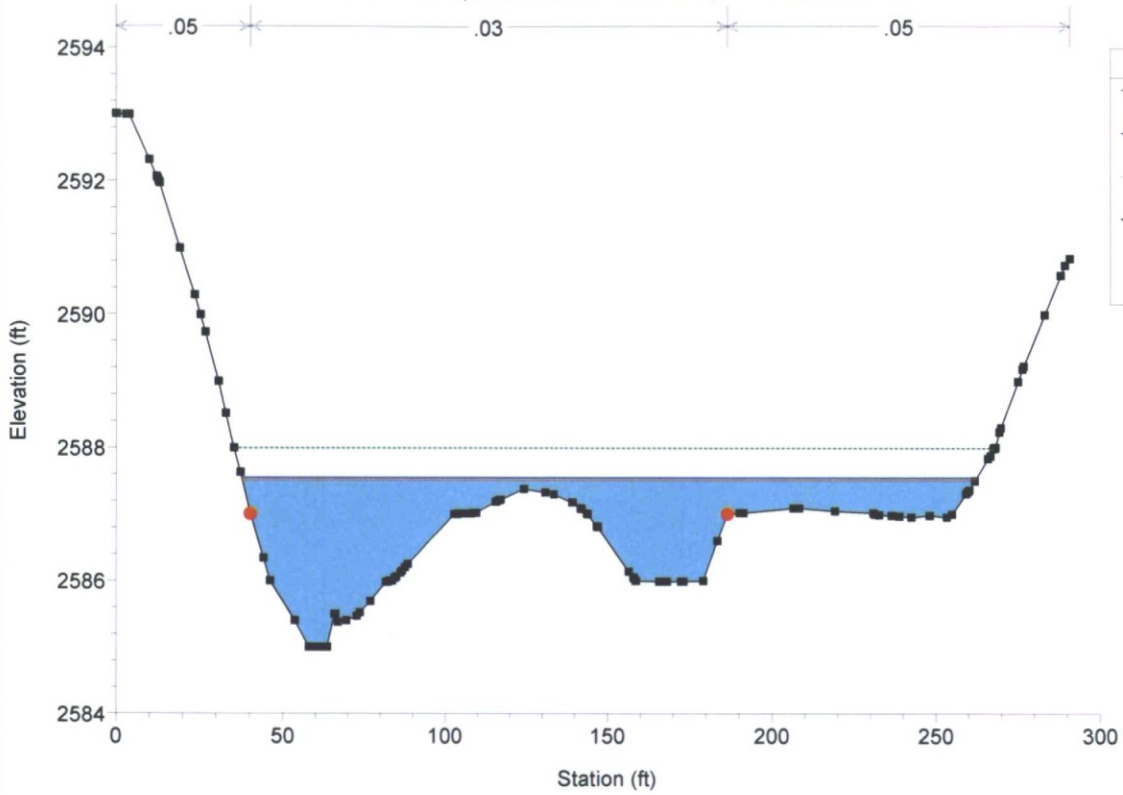
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DM19\_PD Plan: Plan 01 6/15/2016  
 RS = 645 New Cross Section

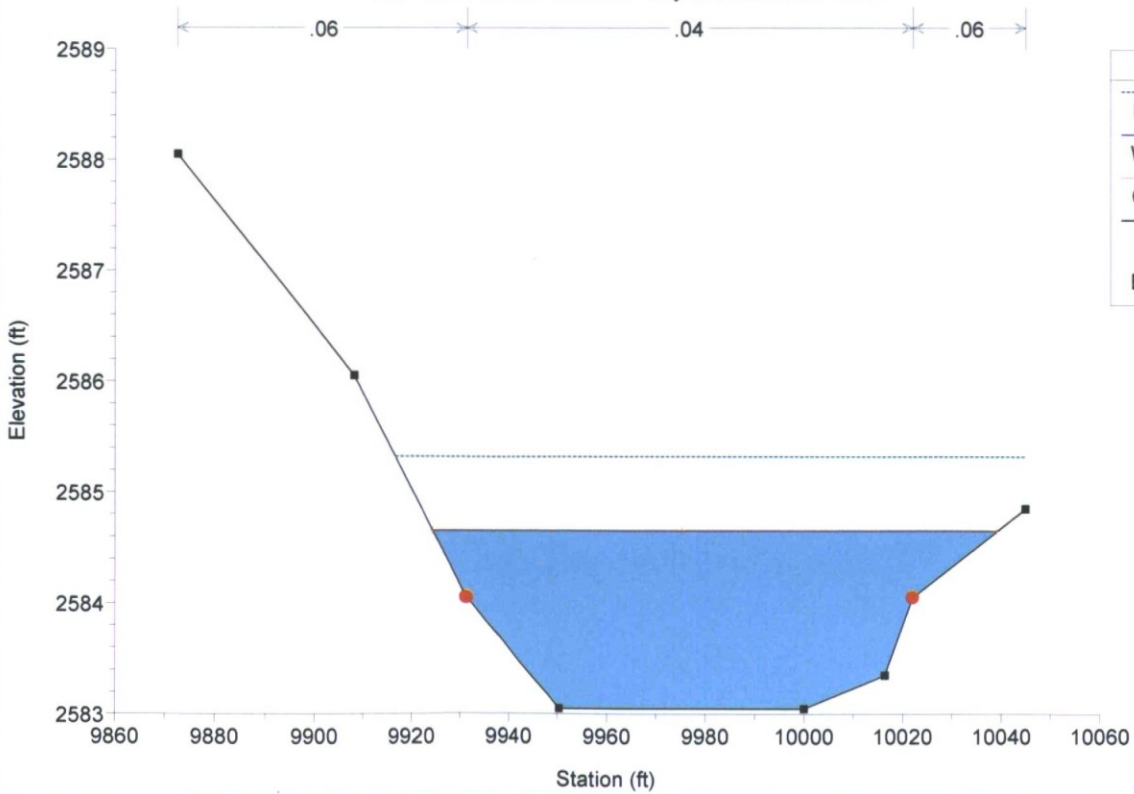


DM19\_PD Plan: Plan 01 6/15/2016  
 RS = 518 Updated CS with new 1 ft topo = FEMA CS 4.656

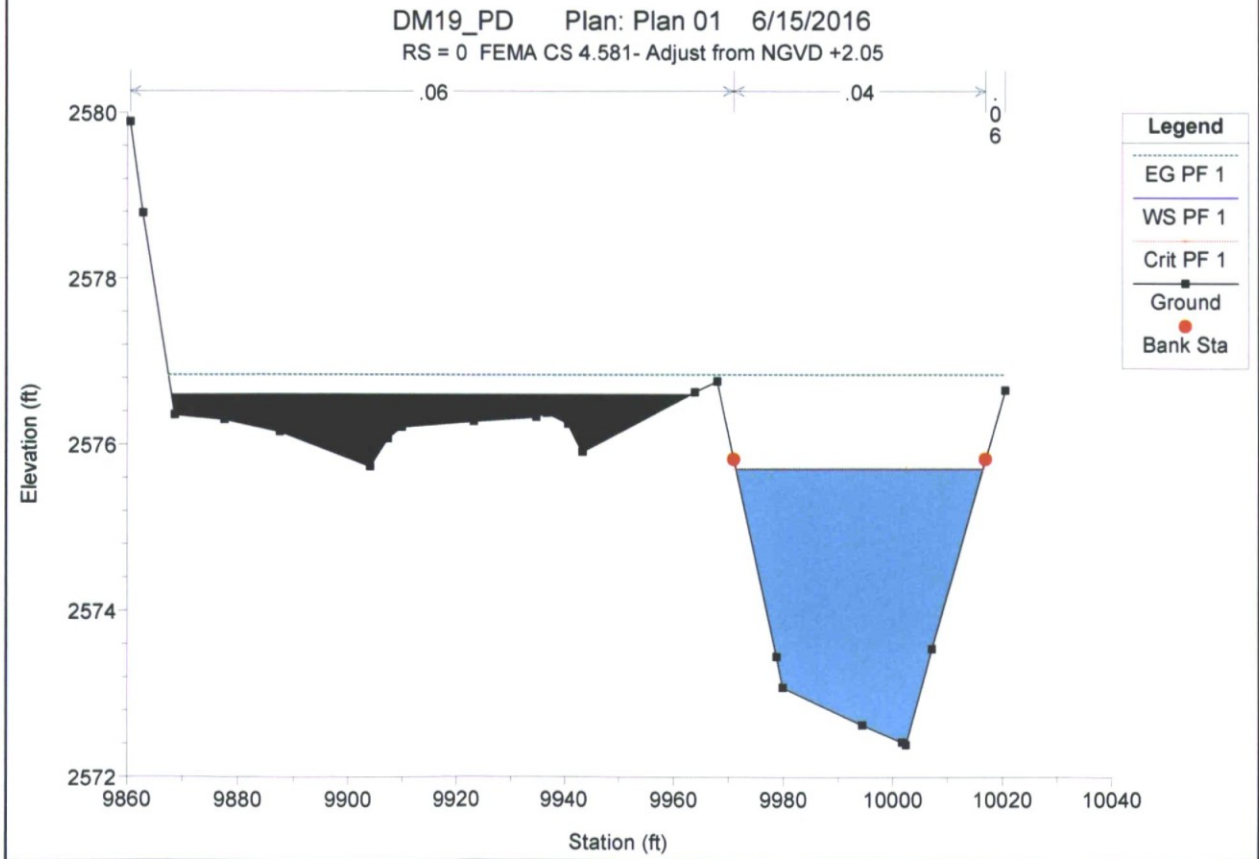
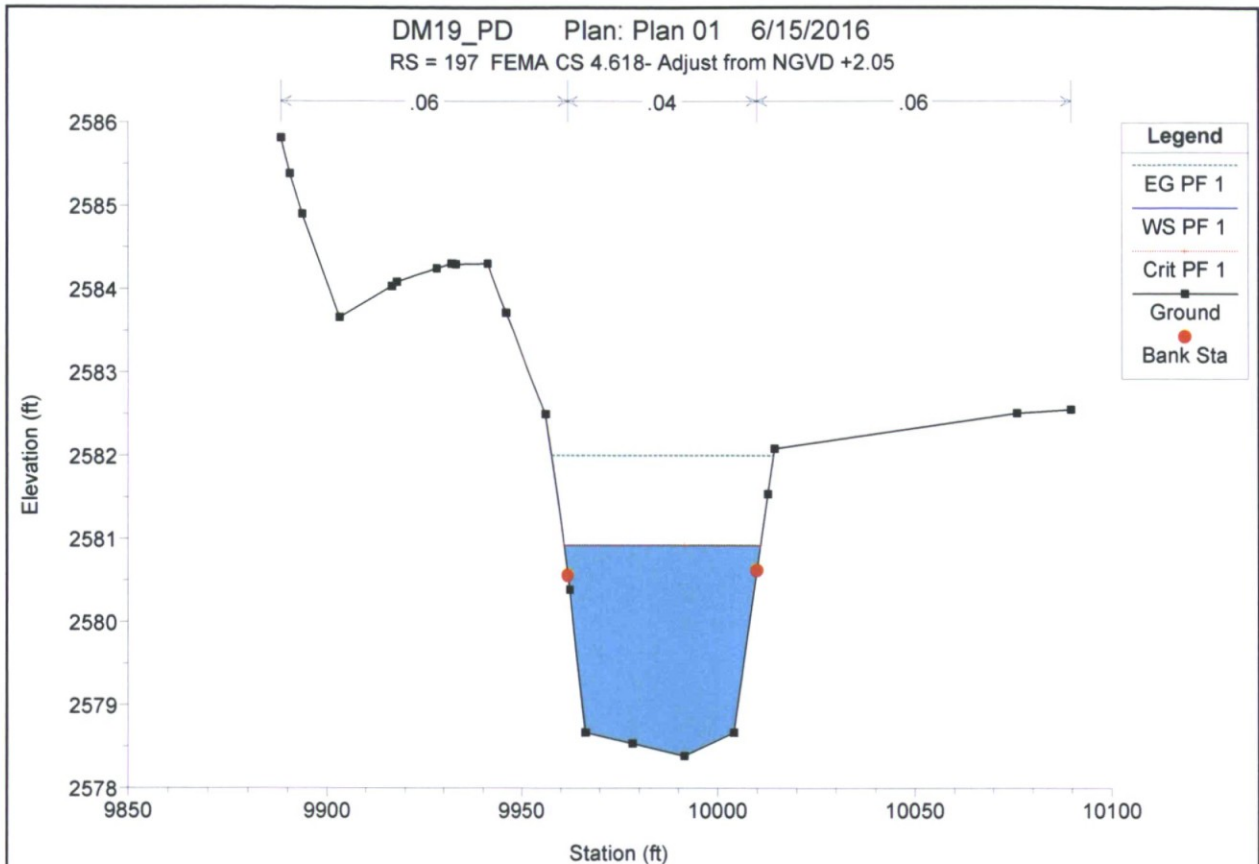


Legend	
EG PF 1	
WS PF 1	
Crit PF 1	
Ground	
Bank Sta	

DM19\_PD Plan: Plan 01 6/15/2016  
 RS = 317 FEMA CS 4.639 - Adjust from NGVD +2.05



Legend	
EG PF 1	
WS PF 1	
Crit PF 1	
Ground	
Bank Sta	



HEC-RAS Plan: Plan 01 River: DM19 Reach: Profile baseline Profile: PF 1

Reach	River Sta	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Profile baseline	2774	1012.00	2641.90	2643.86	2643.86	2644.66	0.020167	7.19	140.92	90.46	1.00
Profile baseline	2678	1012.00	2639.92	2641.86	2641.86	2642.59	0.016480	6.99	163.50	132.82	0.92
Profile baseline	2578	1012.00	2634.45	2638.99		2639.35	0.003594	4.82	209.95	64.34	0.47
Profile baseline	2540	1012.00	2633.32	2638.68	2636.62	2639.21	0.002881	5.88	172.24	72.12	0.46
Profile baseline	2520	Culvert									
Profile baseline	2500	1012.00	2633.00	2636.01	2636.01	2637.53	0.016380	9.90	102.20	64.35	1.01
Profile baseline	2405	1012.00	2630.14	2633.15	2633.09	2634.22	0.016536	8.31	123.48	55.97	0.96
Profile baseline	2250	1012.00	2626.98	2630.28	2630.28	2631.62	0.016755	9.27	110.19	46.98	0.98
Profile baseline	2130	1012.00	2624.00	2626.99	2626.98	2628.02	0.018886	8.14	124.27	60.32	1.00
Profile baseline	2056	1012.00	2623.00	2625.66	2625.66	2626.51	0.020458	7.42	136.31	81.23	1.01
Profile baseline	2014	1012.00	2622.00	2624.73	2624.73	2625.66	0.017239	8.15	134.02	71.27	0.97
Profile baseline	1952	1012.00	2620.00	2623.56	2623.56	2624.38	0.011822	7.94	153.49	107.14	0.83
Profile baseline	1891	1012.00	2619.00	2621.45	2621.41	2622.13	0.019903	6.64	152.39	105.72	0.97
Profile baseline	1791	1012.00	2617.00	2619.35	2619.35	2620.09	0.020971	6.90	146.65	100.43	1.01
Profile baseline	1705	1012.00	2615.00	2617.29	2617.29	2618.13	0.020197	7.35	137.63	83.30	1.01
Profile baseline	1614.9	1012.00	2613.00	2614.81	2614.81	2615.66	0.019987	7.40	136.75	81.17	1.00
Profile baseline	1602.9	1012.00	2610.00	2614.83		2615.03	0.001702	3.61	280.14	73.48	0.33
Profile baseline	1570	Lat Struct									
Profile baseline	1566.5	1022.00	2609.00	2614.46	2612.03	2614.93	0.002280	5.50	185.74	71.00	0.41
Profile baseline	1500	Culvert									
Profile baseline	1481	1022.00	2608.00	2611.03	2611.03	2612.56	0.016291	9.93	102.97	65.17	1.01
Profile baseline	1447	1022.00	2607.90	2610.82	2610.82	2611.88	0.009809	8.30	125.44	189.41	0.98
Profile baseline	1257	1022.00	2603.00	2605.85	2605.85	2606.71	0.009504	7.59	153.46	115.91	0.95
Profile baseline	1119	1022.00	2600.00	2603.09	2603.09	2603.92	0.008530	7.50	155.99	104.42	0.91
Profile baseline	918	1022.00	2595.00	2598.20	2598.20	2598.78	0.008361	6.33	197.76	197.40	0.86
Profile baseline	716	1022.00	2591.19	2593.82	2593.82	2594.52	0.011845	6.70	153.95	114.34	1.00
Profile baseline	645	1022.00	2588.78	2591.34	2591.34	2591.98	0.008972	7.19	199.69	156.65	0.92
Profile baseline	518	1022.00	2585.00	2587.54	2587.51	2587.99	0.009998	5.52	210.58	224.44	0.90
Profile baseline	317	861.00	2583.05	2584.65	2584.65	2585.31	0.019510	6.57	136.54	114.75	0.97
Profile baseline	197	861.00	2578.39	2580.92	2580.92	2582.00	0.018503	8.34	103.55	49.95	1.00
Profile baseline	0	861.00	2572.39	2575.70	2575.70	2576.83	0.018487	8.54	100.84	45.09	1.01

**Scour Calculations**

**GALLOWAY WASH**

## Level 1 Analysis of Stream Degradation

**Reference:** ADWR, Flood Warning and Dam Safety Section, 1996.  
State Standard 5-96: "Watercourse System Sediment Balance - Guideline 2:  
Channel Degradation Estimation for Alluvial Channels in Arizona"

**Assumptions:** Channel reaches without major disturbances, such as dams and bridges.

**Equations:**

$$D_s = D_{gs} + D_{lts}$$

where:

$D_s$  = total scour depth, in feet;

$D_{gs}$  = general degradation, in feet;

$D_{lts}$  = long term degradation, in feet;

For straight channel reaches:

$$D_{gs} = 0.157 * Q_{100}^{0.4}$$

For channel reaches with curvature:

$$D_{gs} = 0.219 * Q_{100}^{0.4}$$

Long term degradation:

$$D_{lts} = 0.02 * Q_{100}^{0.6}$$

**Project Name:** DM 19

**Location:** Galloway Wash

**Input Data:**

$$Q_{100} = 1025 \text{ cfs}$$

$D_s = 3.79 \text{ ft}$  for straight channel

$D_s = 4.79 \text{ ft}$  otherwise

**Recommended Scour Depth = 5.0 ft**

Note: the minimum total scour depth,  $D_s$ , shall be 3 feet.

**WASH A**

## Level 1 Analysis of Stream Degradation

**Reference:** ADWR, Flood Warning and Dam Safety Section, 1996.  
State Standard 5-96: "Watercourse System Sediment Balance - Guideline 2:  
Channel Degradation Estimation for Alluvial Channels in Arizona"

**Assumptions:** Channel reaches without major disturbances, such as dams and bridges.

**Equations:**

$$D_s = D_{gs} + D_{lts}$$

where:

$D_s$  = total scour depth, in feet;

$D_{gs}$  = general degradation, in feet;

$D_{lts}$  = long term degradation, in feet;

For straight channel reaches:

$$D_{gs} = 0.157 * Q_{100}^{0.4}$$

For channel reaches with curvature:

$$D_{gs} = 0.219 * Q_{100}^{0.4}$$

Long term degradation:

$$D_{lts} = 0.02 * Q_{100}^{0.6}$$

**Project Name:** DM 19

**Location:** Wash A

**Input Data:**

$$Q_{100} = 468 \text{ cfs}$$

$D_s = 2.64 \text{ ft}$  for straight channel

$D_s = 3.36 \text{ ft}$  otherwise

**Recommended Scour Depth = 4.0 ft**

Note: the minimum total scour depth,  $D_s$ , shall be 3 feet.

## WOOD/PATEL

CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Desert Mountain Parcel 19*  
Location: *Phoenix, Maricopa County Arizona*  
Date: *16-Jun-16*  
References: *Storm Water Policies & Standards, City of Phoenix (12/2013)*

### ADWR Erosion Hazard Setback Equations State Standard for Watercourse System Sediment Balance SSA 5-96, LMSA-5, September 1996

For straight channel reaches or reaches with minor curvature  $setback = 1.0(Q100)^{0.5}$

For channels with obvious curvature or channel bend  $setback = 2.5(Q100)^{0.5}$

The setback allowance is to be measured outward from the 100-year floodway  
Or the top of the channel bank, whichever is greater

### EROSION HAZARD SETBACK CALCULATIONS

Wash Name	Q100 Discharge (cfs)	ADWR Erosion Hazard Setback (EHS) (ft)
<b>Galloway Wash</b>		
Off-Site Wash	1025	80.0
(bend)		
Off-Site Wash	1025	32.0

Note:  
The minimum EHS is 20' for Straight Channels

# WOOD/PATEL

CIVIL ENGINEERS \* HYDROLOGISTS \* LAND SURVEYORS \* CONSTRUCTION MANAGERS

Project: *Desert Mountain Parcel 19*  
Location: *Phoenix, Maricopa County Arizona*  
Date: *16-Jun-16*  
References: *Storm Water Policies & Standards, City of Phoenix (12/2013)*

## ADWR Erosion Hazard Setback Equations State Standard for Watercourse System Sediment Balance SSA 5-96, LMSA-5, September 1996

For straight channel reaches or reaches with minor curvature  $setback = 1.0(Q100)^{0.5}$

For channels with obvious curvature or channel bend  $setback = 2.5(Q100)^{0.5}$

The setback allowance is to be measured outward from the 100-year floodway  
Or the top of the channel bank, whichever is greater

## EROSION HAZARD SETBACK CALCULATIONS

Wash Name	Q100 Discharge (cfs)	ADWR Erosion Hazard Setback (EHS) (ft)
<b>Wash A</b>		
Off-Site Wash (bend)	468	54
Off-Site Wash	468	22

Note:  
The minimum EHS is 20' for Straight Channels

## **APPENDIX E**

### **Electronic Versions:**

**PDF of DM 19 Preliminary Drainage Report**

**DDMSW DM 19 Existing & Proposed ZIP files**

**Existing & Proposed 2-year, 10-year & 100-year HEC-1 Files**

**Existing Wash A & Existing & Proposed Galloway Wash HEC-RAS Files**

**EXHIBIT 1**

**Vicinity Map**



**VICINITY MAP**

N.T.S.



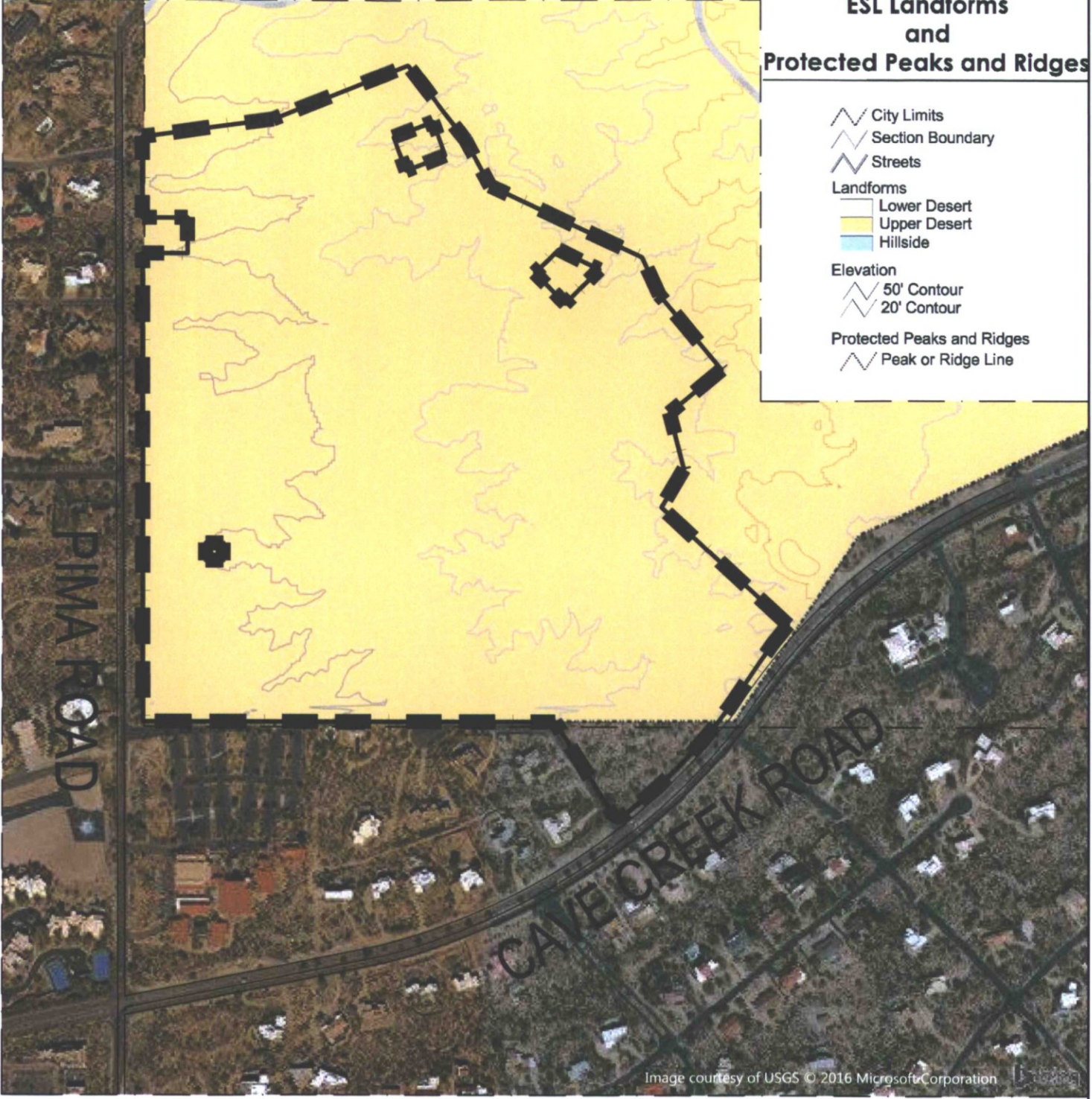
<p><b>WOOD/PATEL</b> MISSION: CLIENT SERVICE™ (602) 335-8500 WWW.WOODPATEL.COM PHOENIX - MESA - TUCSON</p>	<p><b>DESERT MOUNTAIN 19</b></p>		
	<p><b>EXHIBIT 1 VICINITY MAP</b></p>		
	<p>DATE: 06/15/2016</p>	<p>SCALE: N.T.S.</p>	<p>SHEET 1 OF 1</p>
	<p>JOB NO.: 164434</p>	<p>DESIGN: RMH DRAWN: RMH</p>	

**EXHIBIT 2**

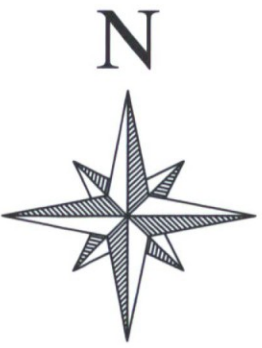
**ESL Classification Map**

**ESL Landforms  
and  
Protected Peaks and Ridges**

-  City Limits
-  Section Boundary
-  Streets
- Landforms**
-  Lower Desert
-  Upper Desert
-  Hillside
- Elevation**
-  50' Contour
-  20' Contour
- Protected Peaks and Ridges**
-  Peak or Ridge Line



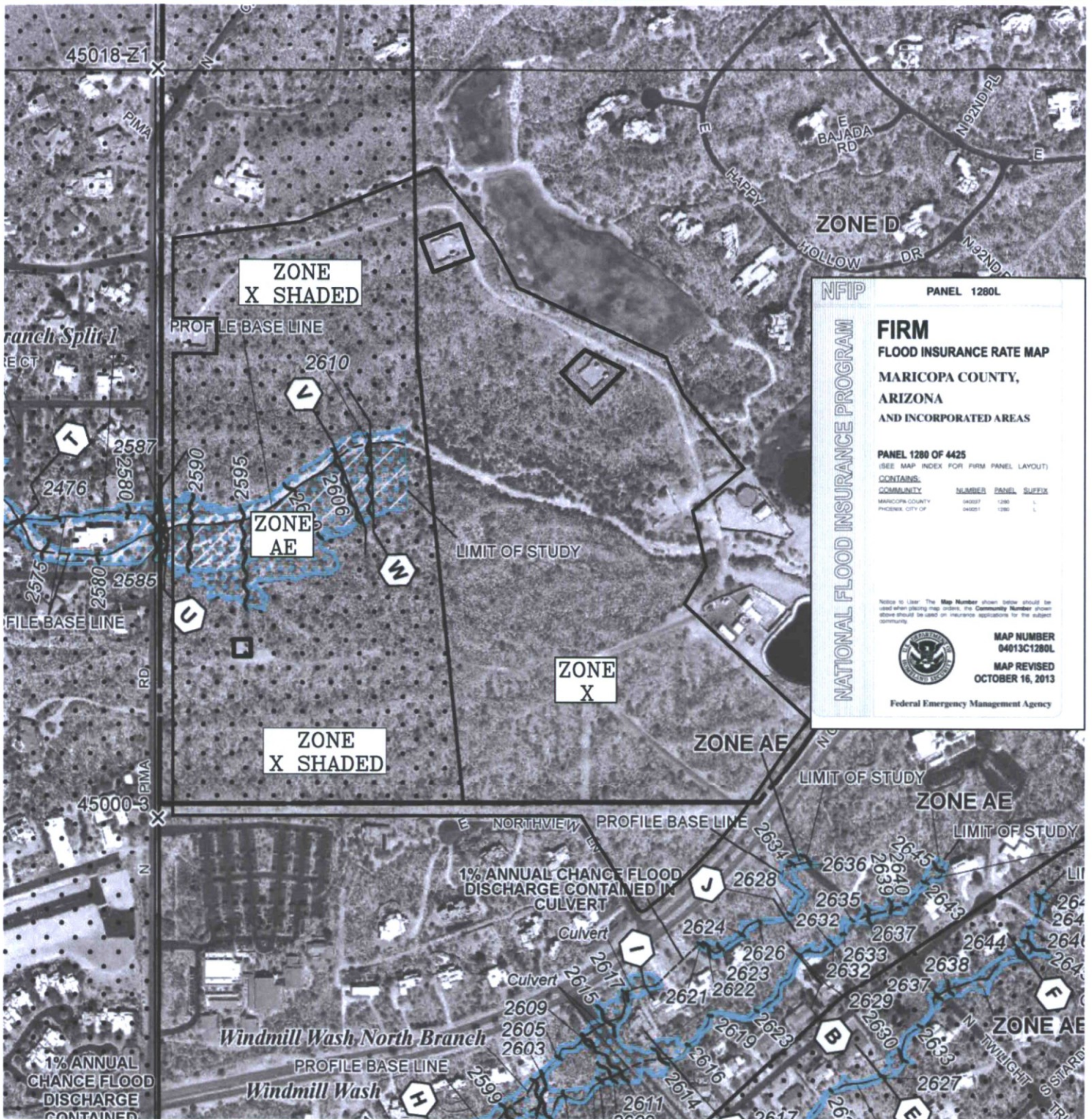
**VICINITY MAP**  
N.T.S.



<p><b>WOOD/PATEL</b> MISSION: CLIENT SERVICE™ (602) 335-8500 WWW.WOODPATEL.COM PHOENIX - MESA - TUCSON</p>	<p><b>DESERT MOUNTAIN 19</b></p>		<p>SHEET 1 OF 1</p>
	<p>EXHIBIT 2 ESL Classification Map</p>		
	DATE: 06/09/2016	SCALE: N.T.S.	
	JOB NO.: 164434	DESIGN: SES DRAWN: SES	

**EXHIBIT 3**

**Flood Insurance Rate Map (FIRM)**



**NFIP** PANEL 1280L

**FIRM**  
**FLOOD INSURANCE RATE MAP**  
**MARICOPA COUNTY,**  
**ARIZONA**  
**AND INCORPORATED AREAS**

PANEL 1280 OF 4425  
 (SEE MAP INDEX FOR FIRM PANEL LAYOUT)

**CONTAINS:**  
 COMMUNITY: MARICOPA COUNTY  
 FEDERAL EMERGENCY MANAGEMENT AGENCY

COMMUNITY	NUMBER	PANEL	SUFFIX
MARICOPA COUNTY	04037	1280	L
PHOENIX CITY OF	04037	1280	L

Notice to User: The Map Number shown below should be used when obtaining map orders. The Community Number shown above should be used on insurance applications for the subject community.

**MAP NUMBER**  
04013C1280L

**MAP REVISED**  
OCTOBER 16, 2013

Federal Emergency Management Agency

N



Horz. 1 in. = 500 ft.

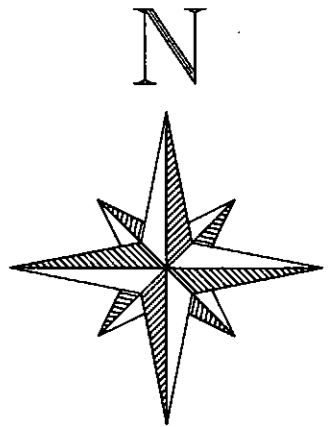
**WOOD/PATEL**  
 MISSION: CLIENT SERVICE\*  
 (602) 335-8500  
 WWW.WOODPATEL.COM

<b>DESERT MOUNTAIN 19</b>			
<b>EXHIBIT 3</b>			
<b>FLOOD INSURANCE RATE MAP</b>			
DATE	SCALE	SHEET	
6/15/2016	1" = 500'	1 OF 1	
JOB NO.	DESIGN	RMH	CHECK JCD
164434	DRAWN	RMH	RFI #

fao.dh 1-FEN 1s4443 1ae/PI 1Reoc 1rolex 1201E

**EXHIBIT 4**

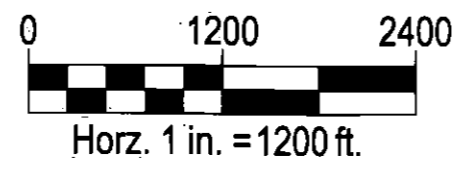
**Soils Classification Map**



N:\2016\164434\Project Support\Reports\Drainage\DRR\Exhibits\4434-Exhibit 4- Soils Classification Map.dwg

Image courtesy of USGS © 2016 Microsoft Corporation

LEGEND	
	NRCS MAP UNIT LEGEND
	SUB BASIN ID
	SOIL BOUNDARY
	SUB BASIN BOUNDARY



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OR RECORDING**

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DESERT MOUNTAIN 19		
EXHIBIT 4 SOILS CLASSIFICATION MAP		
DATE 06/09/2016	SCALE 1" = 1200'	SHEET 1 OF 1
JOB NO. 164434	DESIGN JCD DRAWN SES	CHECK ??? RFI #

**EXHIBIT 5**

**Aerial Map**



Image courtesy of USGS © 2016 Microsoft Corporation

N



Horz. 1 in. = 500 ft.

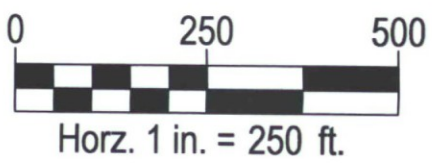
<b>WOOD/PATEL</b> MISSION: CLIENT SERVICE™ (602) 335-8500 WWW.WOODPATEL.COM PHOENIX - MESA - TUCSON	<b>DESERT MOUNTAIN 19</b>	
	<b>EXHIBIT 5 AERIAL MAP</b>	
	DATE: 06/15/2016	SCALE: 1" = 500'
JOB NO.: 164434	DESIGN: RMH	
	DRAWN: RMH	

**EXHIBIT 6**

**Developed Conditions Land Use Map**



N:\2016\164434\Project Support\Reports\Drainage\PDRE\Exhibits\4434-Exhibit 6-Developed Conditions Land Use Map.dwg



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**DESERT MOUNTAIN 19**

**EXHIBIT 6  
 POST DEVELOPMENT LAND USE  
 MAP**

DATE 6/15/2016	SCALE 1" = 500'	SHEET 1 OF 1	
JOB NO. 164434	DESIGN DRAWN	SES SES	CHECK RFI #
		JCD	

**EXHIBIT 7**

**Existing Conditions Sub-Basin HEC-1 Map**






**EXHIBIT 8**


**Developed Conditions Sub-Basin HEC-1 Map**



**LEGEND**

-  SUB-BASIN
-  CHANNEL ROUTING
-  STORAGE ROUTING

-  HYDROGRAPH COMBINING
-  FLOW DIVERSION

-  DRAINAGE AREA BOUNDARY



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**DESERT MOUNTAIN 19**

**EXHIBIT 8**  
**DEVELOPED CONDITIONS**  
**SUB-BASIN HEC-1 MAP**

DATE: 06-16-2016	SCALE: 1" = 250'	SHEET 1 OF 1
JOB NO.: 164434	DESIGN: SES	
	DRAWN: SES	

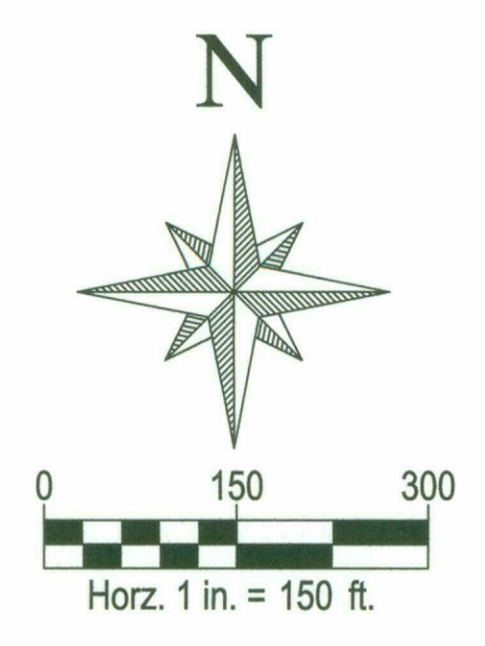
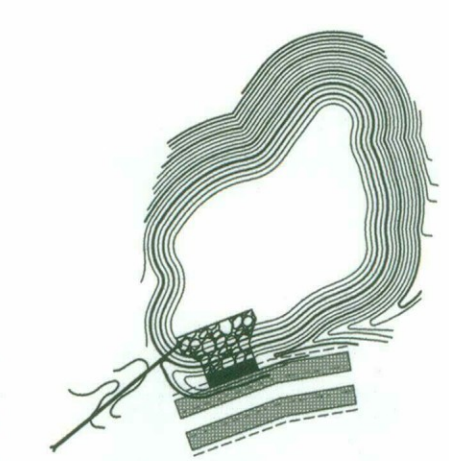
**EXHIBIT 9**

**Existing Conditions Hydraulics Map**



**EXHIBIT 10**

**Developed Conditions Hydraulics Map**

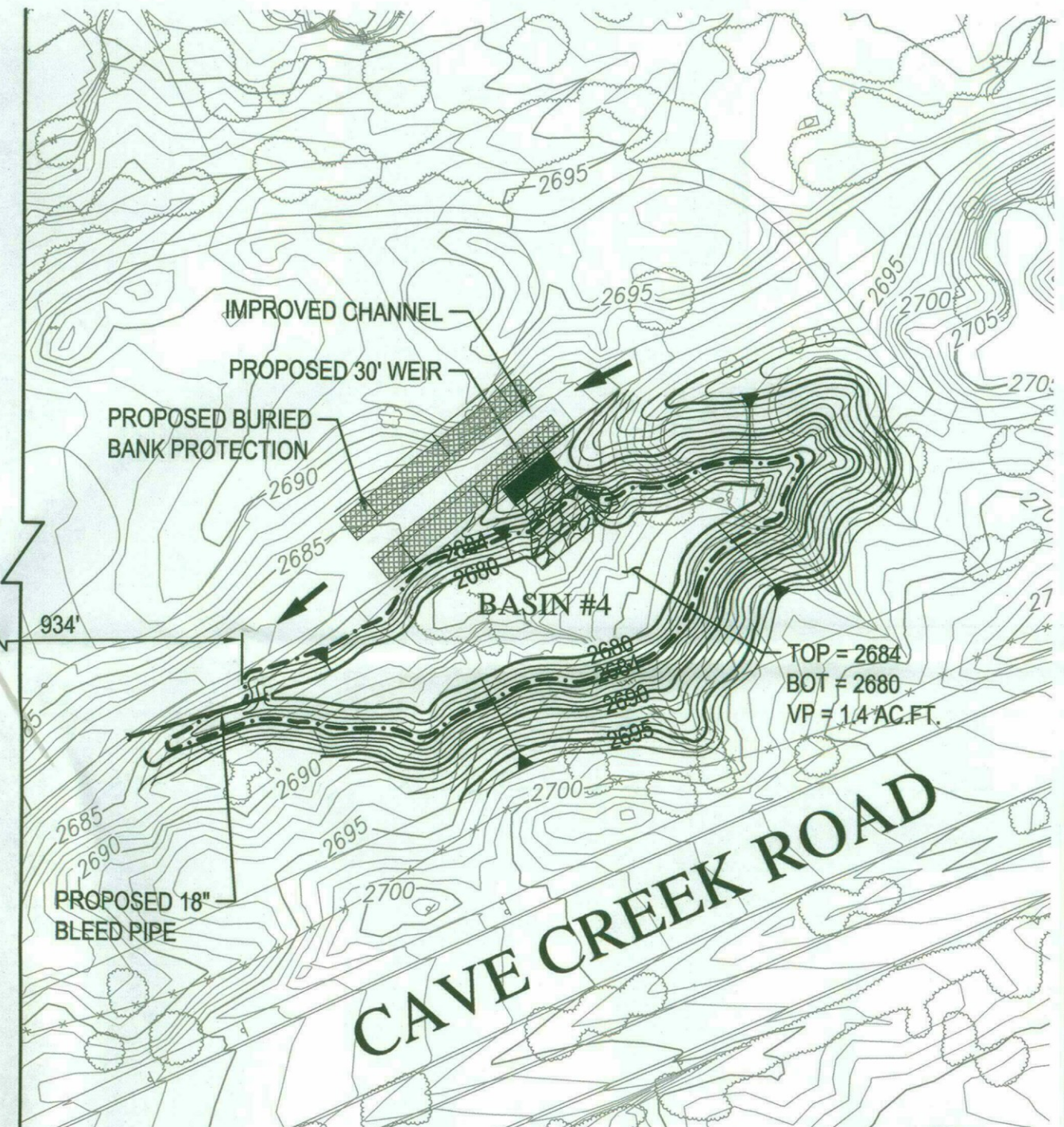
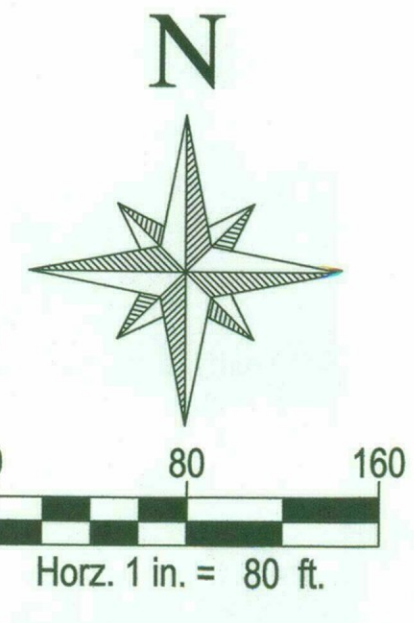


N:\2016\164434\Project\Report\Drawings\DESERT MOUNTAIN 19 DEVELOPED CONDITIONS HYDRAULICS MAP.dwg  
 15/06/2016 10:00:00 AM  
 JCD

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	DATE: 06-16-2016	SCALE: 1" = 150'	SHEET 1 OF 1	
	JOB NO.: 164434	DESIGN: JCD DRAWN: JCD		

**EXHIBIT 11**

**Preliminary Grading Plan**



LEGEND	
	OPEN SPACE BOUNDARY
	FEMA FLOODPLAIN
	FEMA FLOODWAY
	PRELIMINARY 100-YEAR FLOODPLAIN
	RETENTION/DETENTION BASINS
	DRAIN FLOW
	BANK PROTECTION
	WALL
	CULVERT W/ HEADWALL
	SURFACE FLOW
	EROSION ROCK
	EXISTING Q <sub>100</sub>
	POST DEVELOPMENT Q <sub>100</sub>

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**DESERT MOUNTAIN PARCEL 19**

**PRELIMINARY GRADING PLAN**

DATE 6/16/16	SCALE 1" = 80'	SHEET 1 OF 1
JOB NO. 164434	DESIGN DC	CHECK DC
	DRAWN CD	RFI #