

Drainage Reports
Abbreviated Water & Sewer Need Reports
Water Study
Wastewater Study
Stormwater Waiver Application

WATER BASIS OF DESIGN

Estates at Hayden

Prepared for:

Blandford Homes
3321 East Baseline Road
Gilbert, AZ 85234

Prepared by:

Kimley-Horn and Associates, Inc.
7740 N. 16th Street, Suite 300
Phoenix, Arizona 85020

291109000
March 2018
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5-ZN-2018
04/03/18





Final Water Basis of Design

ESTATES AT HAYDEN

MARCH 2018

Prepared By:

Kimley»»Horn

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- Appendix B – Proposed Water System Layout
- Appendix C – WaterCAD Analysis Results
- Appendix D – Fire Flow Test Results

INTRODUCTION

INTENT

The purpose of this water report is to support the water system for the proposed Estates at Hayden residential development located on North Hayden Road between East Black Mountain Road and East Westland Road in Scottsdale, Arizona. This report presents the basis of design criteria that will be used for the engineering design of the proposed development utilizing current water design standards and guidelines set forth by the City of Scottsdale, Arizona.

PROJECT DESCRIPTION

Estates at Hayden is located within Section 12, Township 5 North, Range 4 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. The development is bound to the west by North Hayden Road, Sunflower Estates development and Black Mountain Road to the north, North 81st Street to the east, and Westland Estates development to the south. See **Appendix A: Site Location Map**

The 2.9 acre site is a single family development, consisting of 8 single family residential units. Currently, the property is composed of undeveloped desert rangeland. The site is currently zoned as R1-43 and is going to be rezoned as R1-43 PAD.

DISTRIBUTION SYSTEM DESCRIPTION

EXISTING DISTRIBUTION SYSTEM

The site is surrounded by existing single family residential development. Per the City of Scottsdale Quarter Section Map 58-47 there is an existing 12-inch waterline (material unknown) in Hayden Road directly west of the site and a 6-inch DIP waterline in 81st Street directly east of the site. At the southeast corner of the site there exists two water meters from the 6-inch DIP waterline in 81st Street.

According to Figure 6.1-3 of the City of Scottsdale Design Standards and Policies Manual (DS&PM), the site is located in Pressure Zone 10 with existing ground elevation ranging from 2310 feet in the west to 2440 feet in the east.

PROPOSED DISTRIBUTION SYSTEM

The proposed Estates at Hayden site is in pressure zone 10. The water system will tap into the existing 12-inch City of Scottsdale waterline in Hayden Road. The proposed on-site distribution system will service Lots 1-6 and will consist of an 8-inch Class 350 DIP water line that will provide potable water and fire protection. Lots 7 & 8 along the south of the site will be serviced by the existing water meters located at the southeast corner of the site. Refer to **Appendix B** for the Proposed Water System Layout Exhibit.

BASIS OF DESIGN

DESIGN METHODOLOGY

The WaterCAD v8i water system modeling software distributed by Haestad Methods, Inc. was used to model the proposed water network. The existing water meters servicing Lots 7 & 8 are not analyzed with this report. A fire flow test was performed to determine the residual and static pressures of the existing system. The fire flow test was performed on existing hydrants along N. Hayden Road to the west of the site. See **Appendix D** for complete fire flow test results.

According to Section 6-1.406 of the DSPM, distribution systems shall be designed with a minimum residual pressure of 50 psi and a maximum static pressure of 120 psi. For fire flow scenarios, a minimum design pressure of 20 psi at Finished Floor Elevation is required.

WATER SYSTEM ANALYSIS

The proposed water distribution system for the project is modeled under 4 design scenarios. Average Day, Max Day, Peak Hour and Max Day plus Fire Flows scenarios are modeled. Average Day Demands are based on Figure 6.1-2 in the DS&PM, with peaking factors per section 6-1.404. A fire flow of 1,000 gpm per section 6-1.501 of the DS&PM was used. See **Table 1** below for a summary of water demands.

Table 1 Water Demands

Land Use	Dwelling units (du)	Average Daily Demand (gpm/du)	Average Daily Flow (gpd)	Average Day Flow (gpm)	Max Day Flow (gpm)	Peak Hour Flow (gpm)
2-2.9 du/ac	6	0.66	5,702.4	3.96	7.92	13.86

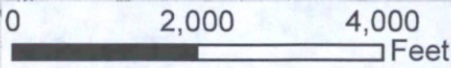
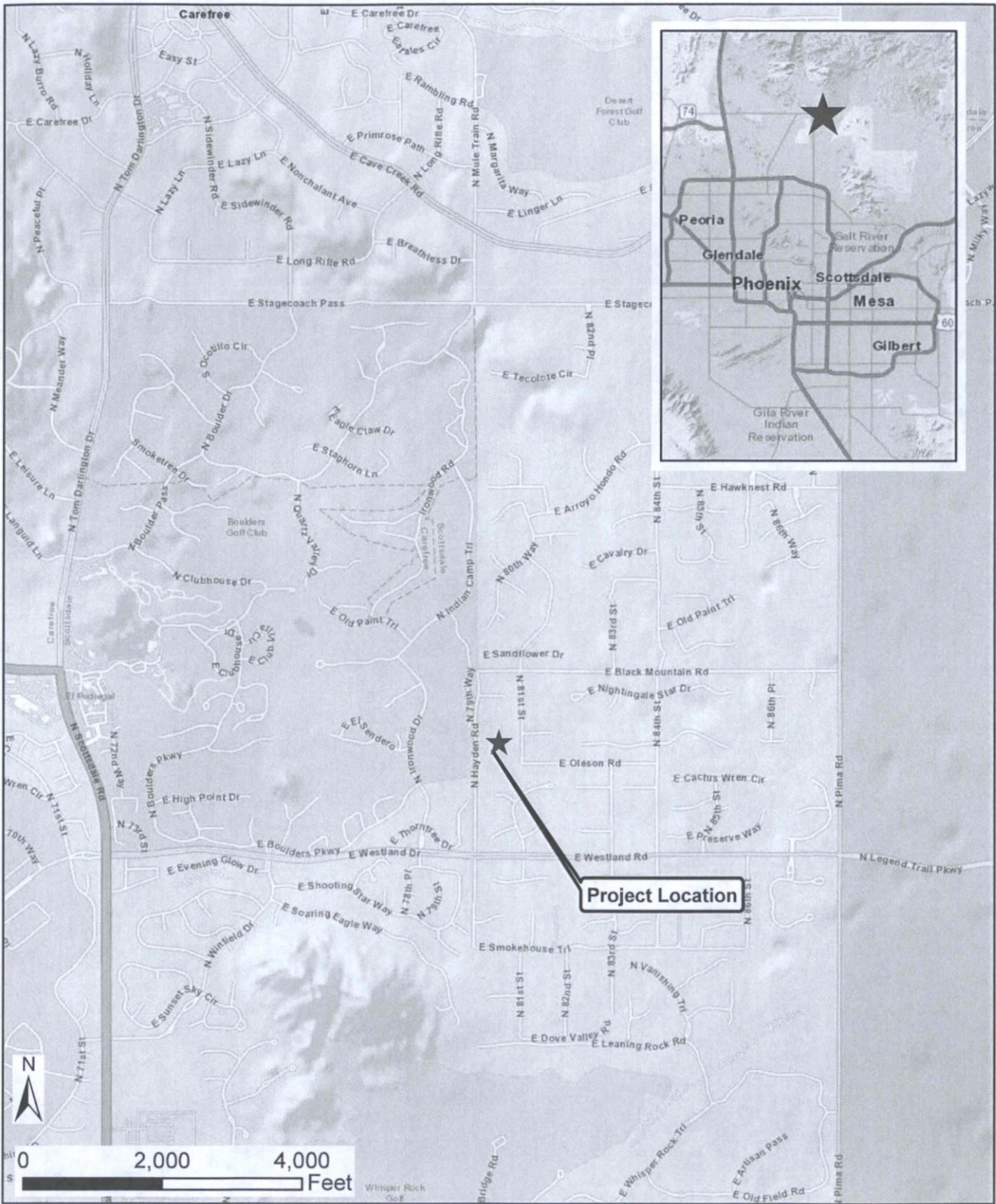
Average Day, Max Day, and Peak Hour Demands are applied at hydraulic model nodes based on number of adjacent proposed units. Fire flow demands are applied to all junctions within the project boundary.

RESULTS

Based on the fire flow tests performed and the results of the WaterCAD analysis, the proposed water system is capable of providing the required domestic flows at pressures ranging from 62.6 psi to 59.1 psi in the average day, max day, and peak hour scenarios. The fire flow pressures meet the minimum requirement of 20 psi, ranging from 55.4 psi to 50.6 psi.

Refer to **Appendix C** for the WaterCAD results. The WaterCAD model includes a pump curve as an approximation of the existing water distribution system.

Appendix A – Site Location Map



Kimley»Horn
Expect More. Experience Better.

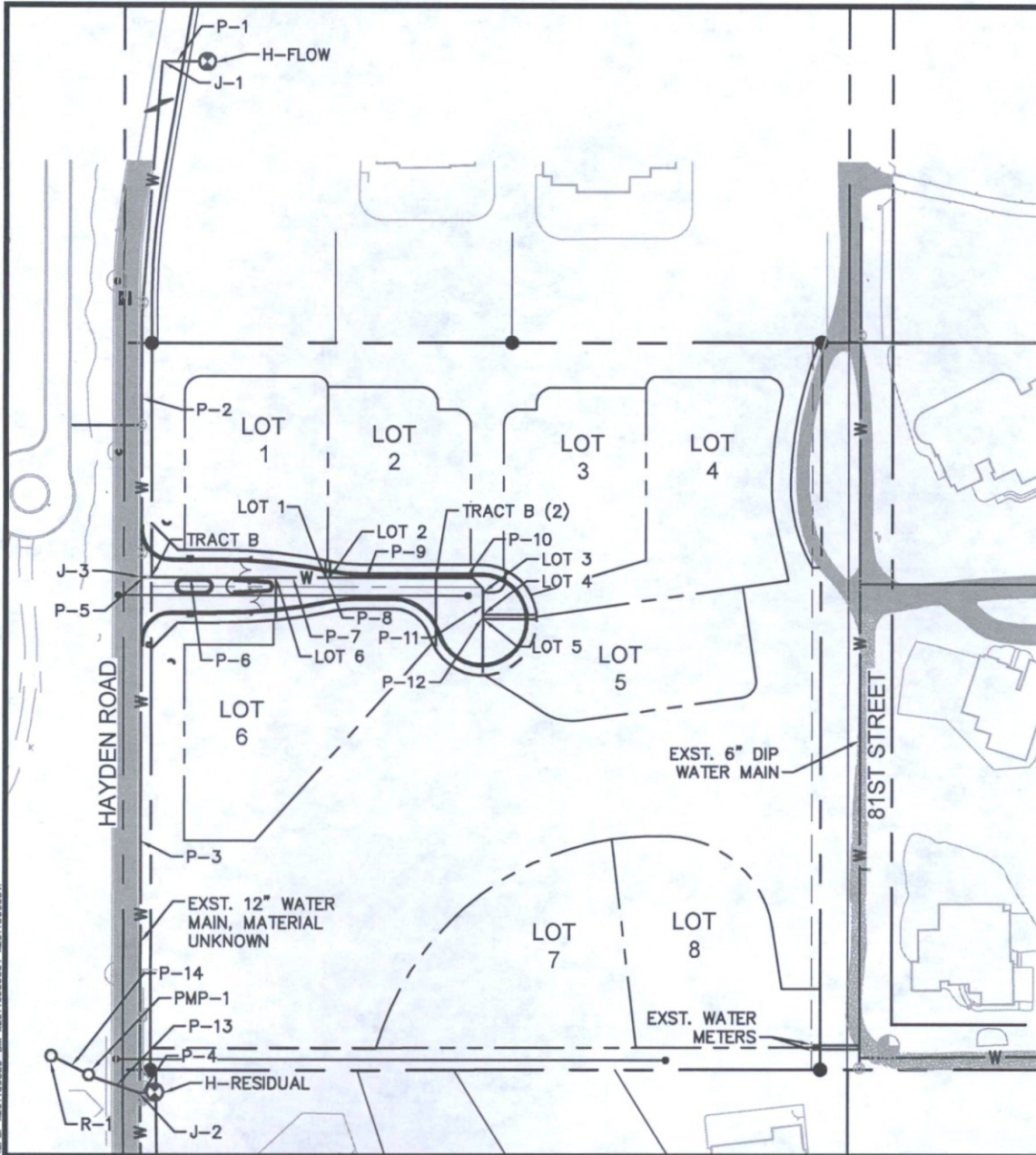
Estates At Hayden

Scottsdale, AZ

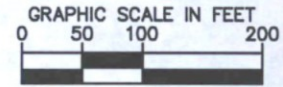
Site Location Map

Appendix B – Proposed Water System Layout

K:\WORK_CAD\2011\09005 - Estates at Hayden\09005\Exhibits\0916-03-01 WaterCAD Exhibit.dwg Mar 02, 2016 CorrettLFrame
XREFS: 291109005-03 291109005-01 291109005-02



— W — PROPOSED WATER LINE
— W — EXISTING WATER LINE



ESTATES AT HAYDEN
WATER LAYOUT

Kimley»Horn

Appendix C – WaterCAD Analysis Results

Model Pump Curve

Max Day Plus Fire Flow

Average Day

Max Day

Peak Hour

Pump Definition Detailed Report: 2018-03-02 Fire Flow Test

Element Details			
ID	61	Notes	
Label	2018-03-02 Fire Flow Test		
Pump Definition Type			
Pump Definition Type	Standard (3 Point)	Design Head	110.65 ft
Shutoff Flow	0.00 gpm	Maximum Operating Flow	3,171.00 gpm
Shutoff Head	147.61 ft	Maximum Operating Head	46.20 ft
Design Flow	1,839.00 gpm		
Pump Efficiency Type			
Pump Efficiency Type	Best Efficiency Point	Motor Efficiency	100.0 %
BEP Efficiency	100.0 %	Is Variable Speed Drive?	False
BEP Flow	0.00 gpm		
Transient (Physical)			
Inertia (Pump and Motor)	0.000 lb·ft ²	Specific Speed	SI=25, US=1280
Speed (Full)	0 rpm	Reverse Spin Allowed?	True

Pump Definition Detailed Report: 2018-03-02 Fire Flow Test



Active Scenario: Max Day + Fire Flow
Fire Flow Node FlexTable: Fire Flow Report

Label	Fire Flow (Needed) (gpm)	Fire Flow (Available) (gpm)	Satisfies Fire Flow Constraints?	Flow (Total Needed) (gpm)	Pressure (Calculated Residual @ Total Flow Needed) (psi)	Flow (Total Available) (gpm)	Pressure (Calculated System Lower Limit) (psi)
H-Flow	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
H-Residual	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
J-1	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
J-2	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
J-3	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
Lot 1	1,000.00	2,521.09	True	1,001.32	55.4	2,522.41	20.0
Lot 2	1,000.00	2,514.92	True	1,001.32	55.0	2,516.24	20.0
Lot 3	1,000.00	2,317.00	True	1,001.32	51.7	2,318.32	20.0
Lot 4	1,000.00	2,293.76	True	1,001.32	50.6	2,295.08	20.0
Lot 5	1,000.00	2,288.84	True	1,001.32	50.6	2,290.16	20.2
Lot 6	1,000.00	2,592.81	True	1,001.32	54.9	2,594.13	20.0
Tract B	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
Tract B (2)	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)

Active Scenario: Average Day
FlexTable: Junction Table

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
J-1	2,403.00	0.00	2,543.61	60.8
J-2	2,396.00	0.00	2,543.61	63.9
J-3	2,398.00	0.00	2,543.61	63.0
Lot 1	2,399.00	0.66	2,543.61	62.6
Lot 2	2,400.00	0.66	2,543.61	62.1
Lot 3	2,405.00	0.66	2,543.61	60.0
Lot 4	2,407.00	0.66	2,543.61	59.1
Lot 5	2,407.00	0.66	2,543.61	59.1
Lot 6	2,401.00	0.66	2,543.61	61.7
Tract B	2,398.00	0.00	2,543.61	63.0
Tract B (2)	2,401.00	0.00	2,543.61	61.7

Active Scenario: Average Day

FlexTable: Pipe Table

Label	Material	Length (ft)	Diameter (in)	Hazen-Williams C	Flow (gpm)	Velocity (ft/s)
P-1	Ductile Iron	13	6.0	130.0	0.00	0.00
P-2	Ductile Iron	557	12.0	130.0	0.00	0.00
P-3	Ductile Iron	468	12.0	130.0	-3.96	0.01
P-4	Ductile Iron	8	6.0	130.0	0.00	0.00
P-5	Ductile Iron	9	8.0	130.0	3.96	0.03
P-6	Ductile Iron	113	8.0	130.0	3.96	0.03
P-7	Ductile Iron	45	8.0	130.0	3.30	0.02
P-8	Ductile Iron	4	8.0	130.0	2.64	0.02
P-9	Ductile Iron	96	8.0	130.0	1.98	0.01
P-10	Ductile Iron	49	8.0	130.0	1.98	0.01
P-11	Ductile Iron	20	8.0	130.0	1.32	0.01
P-12	Ductile Iron	4	8.0	130.0	0.66	0.00
P-13	Ductile Iron	1	100.0	130.0	3.97	0.00
P-14	Ductile Iron	1	100.0	130.0	3.97	0.00

Active Scenario: Max Day
FlexTable: Junction Table

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
J-1	2,403.00	0.00	2,543.61	60.8
J-2	2,396.00	0.00	2,543.61	63.9
J-3	2,398.00	0.00	2,543.61	63.0
Lot 1	2,399.00	1.32	2,543.61	62.6
Lot 2	2,400.00	1.32	2,543.61	62.1
Lot 3	2,405.00	1.32	2,543.61	60.0
Lot 4	2,407.00	1.32	2,543.61	59.1
Lot 5	2,407.00	1.32	2,543.61	59.1
Lot 6	2,401.00	1.32	2,543.61	61.7
Tract B	2,398.00	1.32	2,543.61	63.0
Tract B (2)	2,401.00	0.00	2,543.61	61.7

Active Scenario: Max Day

FlexTable: Pipe Table

Label	Material	Length (ft)	Diameter (in)	Hazen-Williams C	Flow (gpm)	Velocity (ft/s)
P-1	Ductile Iron	13	6.0	130.0	0.00	0.00
P-2	Ductile Iron	557	12.0	130.0	0.00	0.00
P-3	Ductile Iron	468	12.0	130.0	-9.24	0.03
P-4	Ductile Iron	8	6.0	130.0	0.00	0.00
P-5	Ductile Iron	9	8.0	130.0	9.24	0.06
P-6	Ductile Iron	113	8.0	130.0	7.92	0.05
P-7	Ductile Iron	45	8.0	130.0	6.60	0.04
P-8	Ductile Iron	4	8.0	130.0	5.28	0.03
P-9	Ductile Iron	96	8.0	130.0	3.96	0.03
P-10	Ductile Iron	49	8.0	130.0	3.96	0.03
P-11	Ductile Iron	20	8.0	130.0	2.64	0.02
P-12	Ductile Iron	4	8.0	130.0	1.32	0.01
P-13	Ductile Iron	1	100.0	130.0	9.25	0.00
P-14	Ductile Iron	1	100.0	130.0	9.25	0.00

Active Scenario: Peak Hour

FlexTable: Junction Table

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
J-1	2,403.00	0.00	2,543.61	60.8
J-2	2,396.00	0.00	2,543.61	63.9
J-3	2,398.00	0.00	2,543.61	63.0
Lot 1	2,399.00	2.31	2,543.60	62.6
Lot 2	2,400.00	2.31	2,543.60	62.1
Lot 3	2,405.00	2.31	2,543.60	60.0
Lot 4	2,407.00	2.31	2,543.60	59.1
Lot 5	2,407.00	2.31	2,543.60	59.1
Lot 6	2,401.00	2.31	2,543.60	61.7
Tract B	2,398.00	0.00	2,543.61	63.0
Tract B (2)	2,401.00	0.00	2,543.60	61.7

Active Scenario: Peak Hour

FlexTable: Pipe Table

Label	Material	Length (ft)	Diameter (in)	Hazen-Williams C	Flow (gpm)	Velocity (ft/s)
P-1	Ductile Iron	13	6.0	130.0	0.00	0.00
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P-3	Ductile Iron	468	12.0	130.0	-13.86	0.04
P-4	Ductile Iron	8	6.0	130.0	0.00	0.00
P-5	Ductile Iron	9	8.0	130.0	13.86	0.09
P-6	Ductile Iron	113	8.0	130.0	13.86	0.09
P-7	Ductile Iron	45	8.0	130.0	11.55	0.07
P-8	Ductile Iron	4	8.0	130.0	9.24	0.06
P-9	Ductile Iron	96	8.0	130.0	6.93	0.04
P-10	Ductile Iron	49	8.0	130.0	6.93	0.04
P-11	Ductile Iron	20	8.0	130.0	4.62	0.03
P-12	Ductile Iron	4	8.0	130.0	2.31	0.01
P-13	Ductile Iron	1	100.0	130.0	13.87	0.00
P-14	Ductile Iron	1	100.0	130.0	13.87	0.00

Appendix D – Fire Flow Test Results



Flow Test Summary

Project Name: EJFT 18040
Project Address: 34303 N Hayden Rd, Scottsdale, AZ 85266
Date of Flow Test: 2018-03-02
Time of Flow Test: 7:44 AM
Data Reliable Until: 2018-09-02
Conducted By: Cesar Reyna & Eder Cueva (EJ Flow Tests) 602.999.7637
Witnessed By: Jim Demarbiex (City of Scottsdale) 602.541.0586
City Forces Contacted: City of Scottsdale (602.541.0586)
Permit Number: C54832

Raw Flow Test Data

Static Pressure: 71.0 PSI
Residual Pressure: 55.0 PSI
Flowing GPM: 1,839
GPM @ 20 PSI: 3,439

Data with a 10 % Safety Factor

Static Pressure: 63.9 PSI
Residual Pressure: 47.9 PSI
Flowing GPM: 1,839
GPM @ 20 PSI: 3,171

Hydrant F₁

Pitot Pressure (1): 30 PSI
Coefficient of Discharge (1): 0.9
Hydrant Orifice Diameter (1): 2.5 inches
Pitot Pressure (2): 30 PSI
Coefficient of Discharge (2): 0.9
Hydrant Orifice Diameter (2): 2.5 inches



- Project Site
- Static-Residual Hydrant
- Flow Hydrant

Main Size
12 inches

Distance Between F₁ and R
1303 ft (measured linearly)

Static-Residual Elevation
2396 ft (above sea level)

Flow Hydrant (F₁) Elevation
2403 ft (above sea level)

Elevation & distance values are approximate

EJ Flow Tests, LLC

21505 North 78th Ave. | Suite 130 | Peoria, Arizona 85382 | (602) 999-7637 | www.ejengineering.com
John L. Echeverri | NICET Level IV 078493 SME | C-16 FP Contractor ROC 271705 AZ | NFPA CFPS 1915

Static-Residual Hydrant



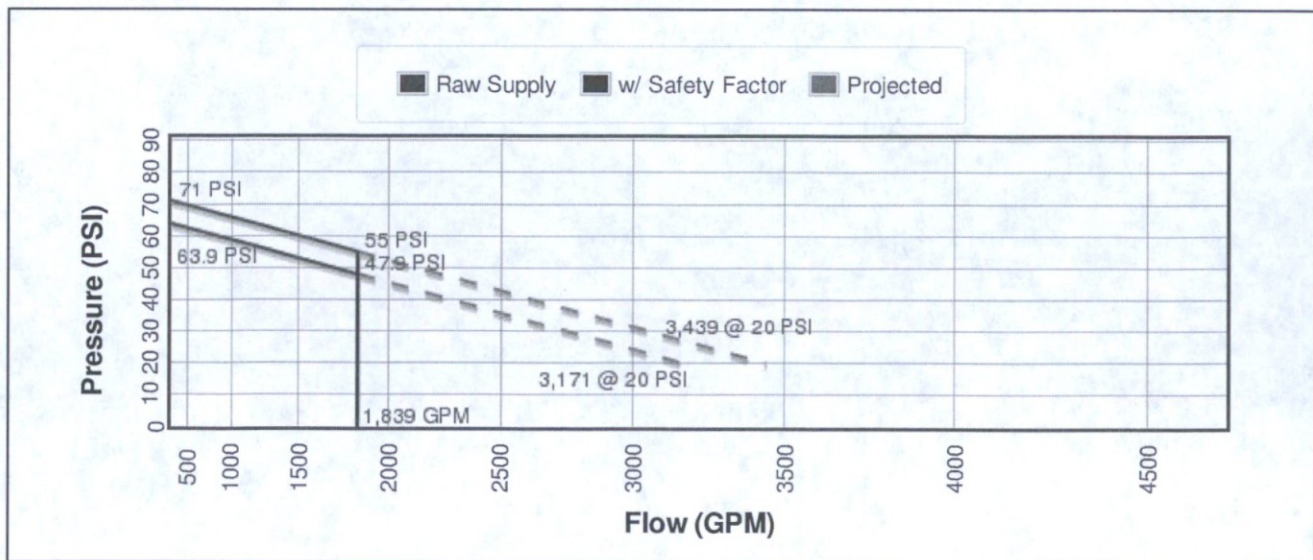
Flow Hydrant (only hydrant F1 shown for clarity)



Approximate Project Site



Water Supply Curve N^{1.85} Graph



WATER BASIS OF DESIGN

Estates at Hayden

REVISE & RESUBMIT

City of Scottsdale
Water Resources Administration
9379 E. San Salvador
Scottsdale, AZ 85258

Rahman . 04/23/2018.

Prepared for:


Blandford Homes
3321 East Baseline Road
Gilbert, AZ 85234

Prepared by:

Kimley-Horn and Associates, Inc.
7740 N. 16th Street, Suite 300
Phoenix, Arizona 85020



ETG 3/21/18



Final Water Basis of Design

ESTATES AT HAYDEN

MARCH 2018

Prepared By:

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According to Figure 6.1-3 of the City of Scottsdale Design Standards and Policies Manual (DS&PM), the site is located in Pressure Zone 10 with existing ground elevation ranging from 2310 feet in the west to 2440 feet in the east.

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** Co-ordinate w/ Fire Dept. if a Fire Hydrant⁵ Required @ within the Sub-division.*

*City map has no water line
Per Water Division*

BASIS OF DESIGN

DESIGN METHODOLOGY

The WaterCAD v8i water system modeling software distributed by Haestad Methods, Inc. was used to model the proposed water network. The existing water meters servicing Lots 7 & 8 are not analyzed with this report. A fire flow test was performed to determine the residual and static pressures of the existing system. The fire flow test was performed on existing hydrants along N. Hayden Road to the west of the site. See **Appendix D** for complete fire flow test results.

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Average Day, Max Day, and Peak Hour Demands are applied at hydraulic model nodes based on number of adjacent proposed units. Fire flow demands are applied to all junctions within the project boundary.

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Based on the fire flow tests performed and the results of the WaterCAD analysis, the proposed water system is capable of providing the required domestic flows at pressures ranging from 62.6 psi to 59.1 psi in the average day, max day, and peak hour scenarios. The fire flow pressures meet the minimum requirement of 20 psi, ranging from 55.4 psi to 50.6 psi.

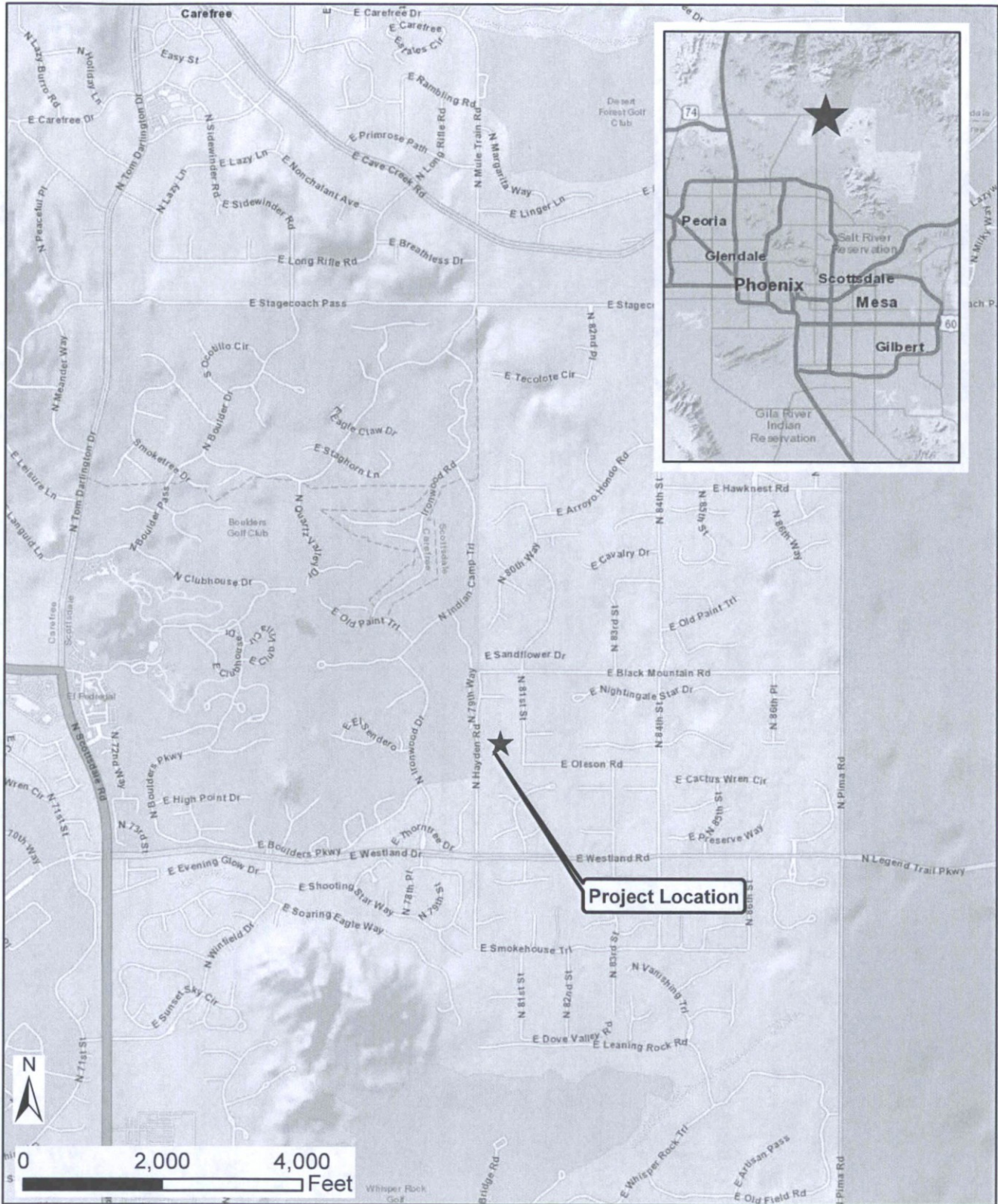
Refer to **Appendix C** for the WaterCAD results. The WaterCAD model includes a pump curve as an approximation of the existing water distribution system.

Analysis Required

30 psi

Include separate Table For Lots 7 & 8 & Show Modeling by including 6" Line @ 81st St.

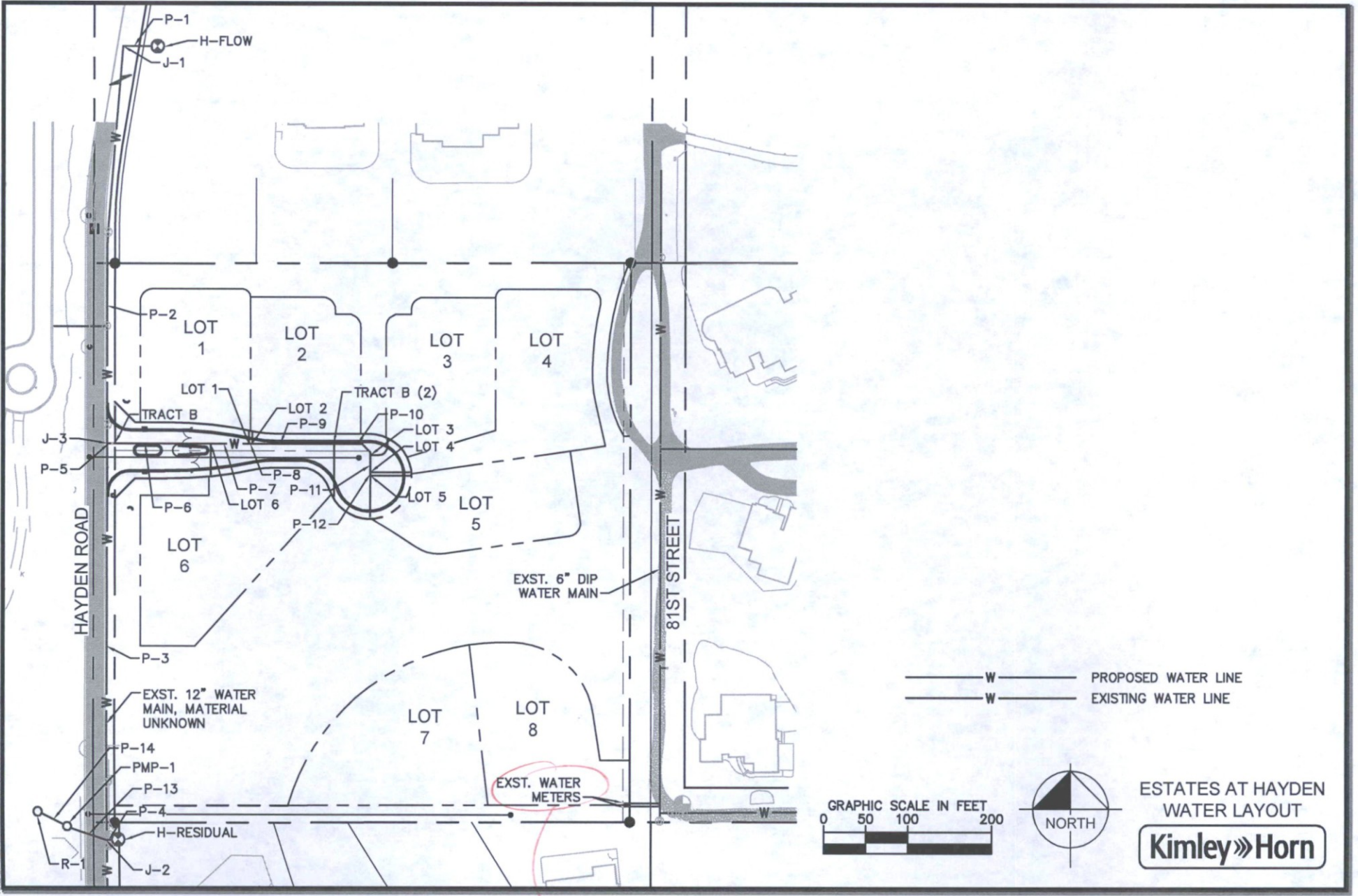
Appendix A – Site Location Map



<p>Kimley»Horn Expect More. Experience Better.</p>	Estates At Hayden	Scottsdale, AZ
	Site Location Map	

Appendix B – Proposed Water System Layout

K:\P\K_C\N\20110909 - Estates at Hayden\GIS\Exhibits\2018-03-01 WaterCAD Exhibit.dwg Mar 02, 2018 Corrett:Frame
XREFS: 2591102001-BA 2591102003-UT 2591102002



Include in the Model ← City Map does not show any water Meters here.?

Appendix C – WaterCAD Analysis Results

Model Pump Curve

Max Day Plus Fire Flow

Average Day

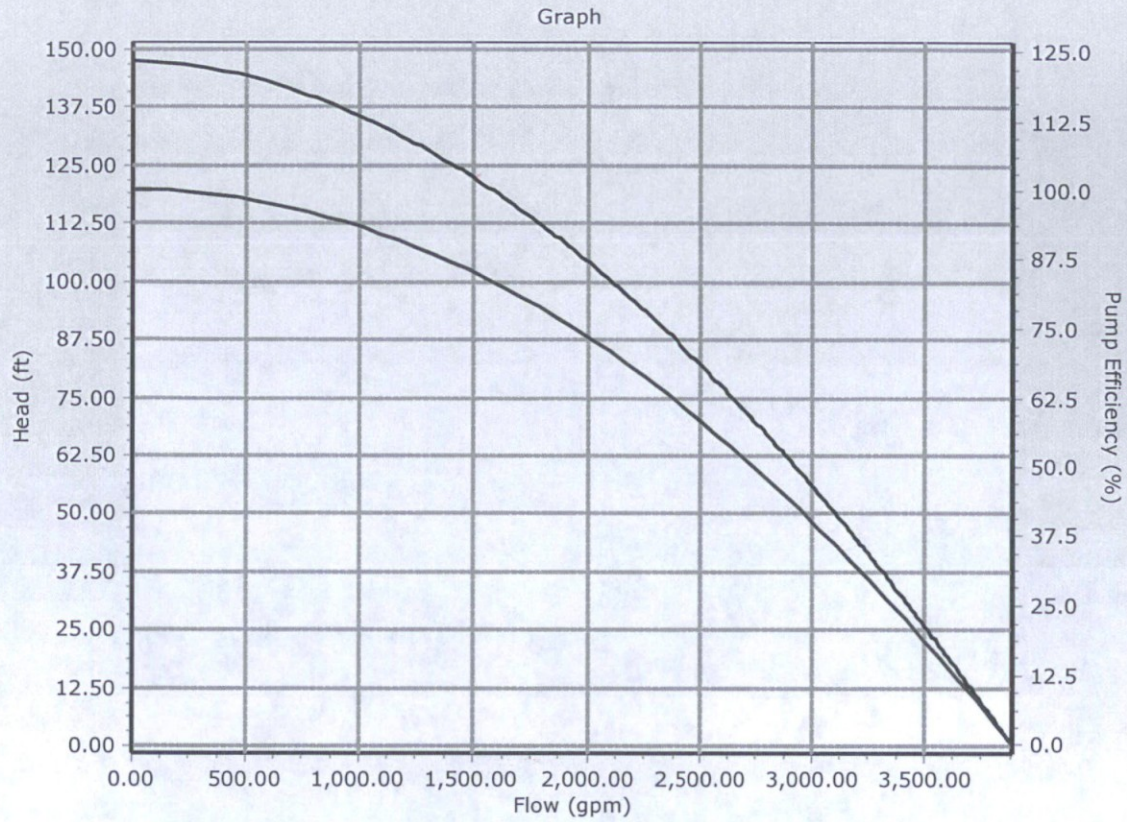
Max Day

Peak Hour

Pump Definition Detailed Report: 2018-03-02 Fire Flow Test

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ID	61	Notes	
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Pump Definition Type	Standard (3 Point)	Design Head	110.65 ft
Shutoff Flow	0.00 gpm	Maximum Operating Flow	3,171.00 gpm
Shutoff Head	147.61 ft	Maximum Operating Head	46.20 ft
Design Flow	1,839.00 gpm		
Pump Efficiency Type			
Pump Efficiency Type	Best Efficiency Point	Motor Efficiency	100.0 %
BEP Efficiency	100.0 %	Is Variable Speed Drive?	False
BEP Flow	0.00 gpm		
Transient (Physical)			
Inertia (Pump and Motor)	0.000 lb·ft ²	Specific Speed	SI=25, US=1280
Speed (Full)	0 rpm	Reverse Spin Allowed?	True

Pump Definition Detailed Report: 2018-03-02 Fire Flow Test



Active Scenario: Max Day + Fire Flow
Fire Flow Node FlexTable: Fire Flow Report

Label	Fire Flow (Needed) (gpm)	Fire Flow (Available) (gpm)	Satisfies Fire Flow Constraints?	Flow (Total Needed) (gpm)	Pressure (Calculated Residual @ Total Flow Needed) (psi)	Flow (Total Available) (gpm)	Pressure (Calculated System Lower Limit) (psi)
H-Flow	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
H-Residual	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
J-1	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
J-2	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
J-3	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
Lot 1	1,000.00	2,521.09	True	1,001.32	55.4	2,522.41	20.0
Lot 2	1,000.00	2,514.92	True	1,001.32	55.0	2,516.24	20.0
Lot 3	1,000.00	2,317.00	True	1,001.32	51.7	2,318.32	20.0
Lot 4	1,000.00	2,293.76	True	1,001.32	50.6	2,295.08	20.0
Lot 5	1,000.00	2,288.84	True	1,001.32	50.6	2,290.16	20.2
Lot 6	1,000.00	2,592.81	True	1,001.32	54.9	2,594.13	20.0
Tract B	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)
Tract B (2)	1,000.00	(N/A)	False	(N/A)	(N/A)	(N/A)	(N/A)



Rev Pump Curve, Head @

Active Scenario: Average Day
FlexTable: Junction Table

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
J-1	2,403.00	0.00	2,543.61	60.8
J-2	2,396.00	0.00	2,543.61	63.9
J-3	2,398.00	0.00	2,543.61	63.0
Lot 1	2,399.00	0.66	2,543.61	62.6
Lot 2	2,400.00	0.66	2,543.61	62.1
Lot 3	2,405.00	0.66	2,543.61	60.0
Lot 4	2,407.00	0.66	2,543.61	59.1
Lot 5	2,407.00	0.66	2,543.61	59.1
Lot 6	2,401.00	0.66	2,543.61	61.7
Tract B	2,398.00	0.00	2,543.61	63.0
Tract B (2)	2,401.00	0.00	2,543.61	61.7


Active Scenario: Average Day

FlexTable: Pipe Table

Label	Material	Length (ft)	Diameter (in)	Hazen-Williams C	Flow (gpm)	Velocity (ft/s)
P-1	Ductile Iron	13	6.0	130.0	0.00	0.00
P-2	Ductile Iron	557	12.0	130.0	0.00	0.00
P-3	Ductile Iron	468	12.0	130.0	-3.96	0.01
P-4	Ductile Iron	8	6.0	130.0	0.00	0.00
P-5	Ductile Iron	9	8.0	130.0	3.96	0.03
P-6	Ductile Iron	113	8.0	130.0	3.96	0.03
P-7	Ductile Iron	45	8.0	130.0	3.30	0.02
P-8	Ductile Iron	4	8.0	130.0	2.64	0.02
P-9	Ductile Iron	96	8.0	130.0	1.98	0.01
P-10	Ductile Iron	49	8.0	130.0	1.98	0.01
P-11	Ductile Iron	20	8.0	130.0	1.32	0.01
P-12	Ductile Iron	4	8.0	130.0	0.66	0.00
P-13	Ductile Iron	1	100.0	130.0	3.97	0.00
P-14	Ductile Iron	1	100.0	130.0	3.97	0.00

Active Scenario: Max Day
FlexTable: Junction Table

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
J-1	2,403.00	0.00	2,543.61	60.8
J-2	2,396.00	0.00	2,543.61	63.9
J-3	2,398.00	0.00	2,543.61	63.0
Lot 1	2,399.00	1.32	2,543.61	62.6
Lot 2	2,400.00	1.32	2,543.61	62.1
Lot 3	2,405.00	1.32	2,543.61	60.0
Lot 4	2,407.00	1.32	2,543.61	59.1
Lot 5	2,407.00	1.32	2,543.61	59.1
Lot 6	2,401.00	1.32	2,543.61	61.7
Tract B	2,398.00	1.32	2,543.61	63.0
Tract B (2)	2,401.00	0.00	2,543.61	61.7

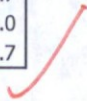


Active Scenario: Max Day
FlexTable: Pipe Table

Label	Material	Length (ft)	Diameter (in)	Hazen-Williams C	Flow (gpm)	Velocity (ft/s)
P-1	Ductile Iron	13	6.0	130.0	0.00	0.00
P-2	Ductile Iron	557	12.0	130.0	0.00	0.00
P-3	Ductile Iron	468	12.0	130.0	-9.24	0.03
P-4	Ductile Iron	8	6.0	130.0	0.00	0.00
P-5	Ductile Iron	9	8.0	130.0	9.24	0.06
P-6	Ductile Iron	113	8.0	130.0	7.92	0.05
P-7	Ductile Iron	45	8.0	130.0	6.60	0.04
P-8	Ductile Iron	4	8.0	130.0	5.28	0.03
P-9	Ductile Iron	96	8.0	130.0	3.96	0.03
P-10	Ductile Iron	49	8.0	130.0	3.96	0.03
P-11	Ductile Iron	20	8.0	130.0	2.64	0.02
P-12	Ductile Iron	4	8.0	130.0	1.32	0.01
P-13	Ductile Iron	1	100.0	130.0	9.25	0.00
P-14	Ductile Iron	1	100.0	130.0	9.25	0.00

Active Scenario: Peak Hour
FlexTable: Junction Table

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
J-1	2,403.00	0.00	2,543.61	60.8
J-2	2,396.00	0.00	2,543.61	63.9
J-3	2,398.00	0.00	2,543.61	63.0
Lot 1	2,399.00	2.31	2,543.60	62.6
Lot 2	2,400.00	2.31	2,543.60	62.1
Lot 3	2,405.00	2.31	2,543.60	60.0
Lot 4	2,407.00	2.31	2,543.60	59.1
Lot 5	2,407.00	2.31	2,543.60	59.1
Lot 6	2,401.00	2.31	2,543.60	61.7
Tract B	2,398.00	0.00	2,543.61	63.0
Tract B (2)	2,401.00	0.00	2,543.60	61.7



Active Scenario: Peak Hour

FlexTable: Pipe Table

Label	Material	Length (ft)	Diameter (in)	Hazen-Williams C	Flow (gpm)	Velocity (ft/s)
P-1	Ductile Iron	13	6.0	130.0	0.00	0.00
P-2	Ductile Iron	557	12.0	130.0	0.00	0.00
P-3	Ductile Iron	468	12.0	130.0	-13.86	0.04
P-4	Ductile Iron	8	6.0	130.0	0.00	0.00
P-5	Ductile Iron	9	8.0	130.0	13.86	0.09
P-6	Ductile Iron	113	8.0	130.0	13.86	0.09
P-7	Ductile Iron	45	8.0	130.0	11.55	0.07
P-8	Ductile Iron	4	8.0	130.0	9.24	0.06
P-9	Ductile Iron	96	8.0	130.0	6.93	0.04
P-10	Ductile Iron	49	8.0	130.0	6.93	0.04
P-11	Ductile Iron	20	8.0	130.0	4.62	0.03
P-12	Ductile Iron	4	8.0	130.0	2.31	0.01
P-13	Ductile Iron	1	100.0	130.0	13.87	0.00
P-14	Ductile Iron	1	100.0	130.0	13.87	0.00

Appendix D – Fire Flow Test Results



Flow Test Summary

Project Name: EJFT 18040
Project Address: 34303 N Hayden Rd, Scottsdale, AZ 85266
Date of Flow Test: 2018-03-02
Time of Flow Test: 7:44 AM
Data Reliable Until: 2018-09-02
Conducted By: Cesar Reyna & Eder Cueva (EJ Flow Tests) 602.999.7637
Witnessed By: Jim Demarbiex (City of Scottsdale) 602.541.0586
City Forces Contacted: City of Scottsdale (602.541.0586)
Permit Number: C54832

Raw Flow Test Data

Static Pressure: 71.0 PSI
Residual Pressure: 55.0 PSI
Flowing GPM: 1,839
GPM @ 20 PSI: 3,439

Data with a 10 % Safety Factor

Static Pressure: 63.9 PSI
Residual Pressure: 47.9 PSI
Flowing GPM: 1,839
GPM @ 20 PSI: 3,171

Hydrant F₁

Pitot Pressure (1): 30 PSI
Coefficient of Discharge (1): 0.9
Hydrant Orifice Diameter (1): 2.5 inches
Pitot Pressure (2): 30 PSI
Coefficient of Discharge (2): 0.9
Hydrant Orifice Diameter (2): 2.5 inches



- Project Site
- Static-Residual Hydrant
- Flow Hydrant

Main Size
12 inches

Distance Between F₁ and R
1303 ft (measured linearly)

Static-Residual Elevation
2396 ft (above sea level)

Flow Hydrant (F₁) Elevation
2403 ft (above sea level)

Elevation & distance values are approximate

E·J | Flow Test Summary

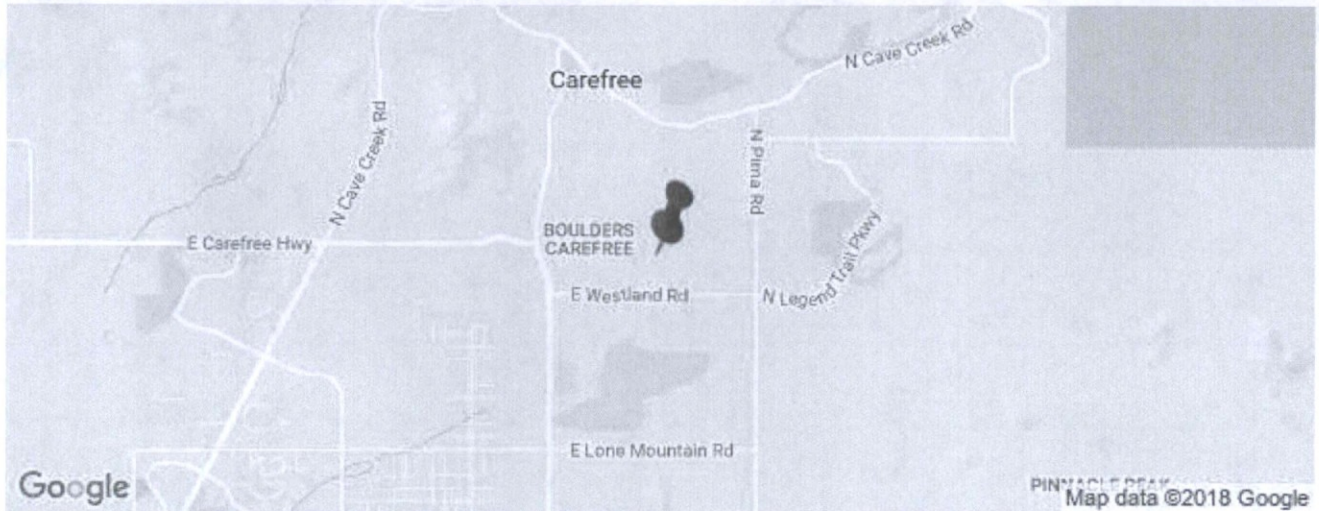
Static-Residual Hydrant



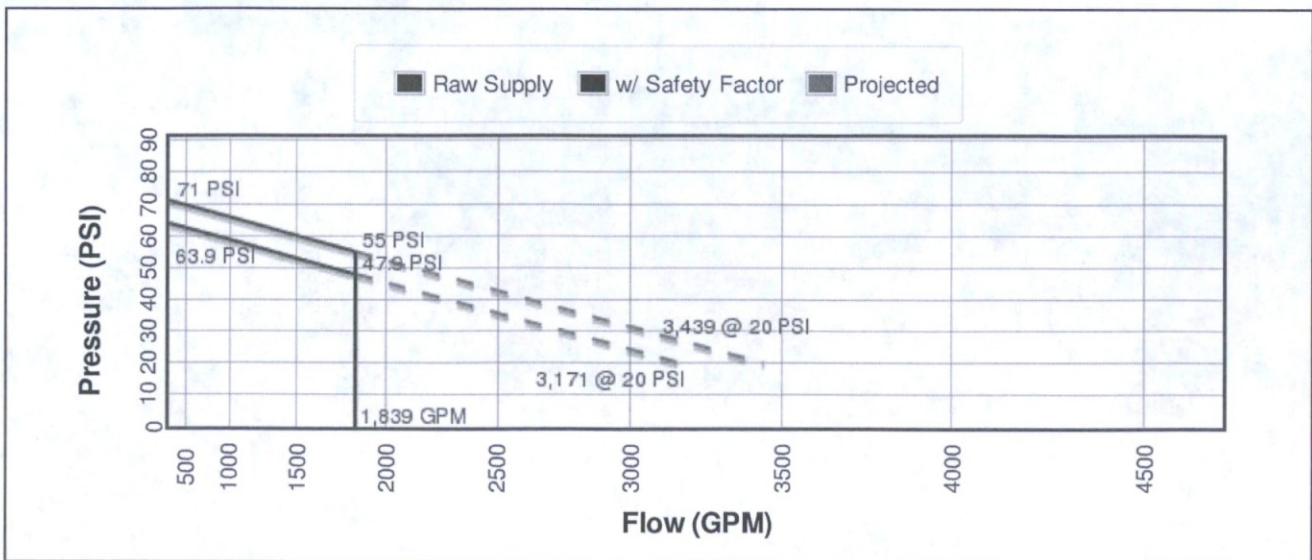
Flow Hydrant (only hydrant F1 shown for clarity)



Approximate Project Site



Water Supply Curve N^{1.85} Graph



SEWER BASIS OF DESIGN

Estates at Hayden

Prepared for:

Blandford Homes
3321 E. Baseline Rd.
Gilbert, Arizona 85234

Prepared by:

Kimley-Horn and Associates, Inc.
7740 N. 16th Street, Suite 300
Phoenix, Arizona 85020



Exp 3/31/18



Final Sewer Basis of Design

ESTATES AT HAYDEN

MARCH 2018

Prepared By:

Kimley»»Horn

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Appendices

- Appendix A – Site Location Map
- Appendix B – Proposed Sewer System Layout
- Appendix C – Flowmaster Calculations

INTRODUCTION

INTENT

The purpose of this sewer report is to support the sanitary sewer system for the proposed Estates at Hayden residential development located on North Hayden Road between East Black Mountain Road and East Westland Road in Scottsdale, Arizona. This report presents the basis of design criteria that will be used for the engineering design of the proposed development utilizing current sewer design standards and guidelines set forth by the City of Scottsdale, Arizona.

PROJECT DESCRIPTION

Estates at Hayden is located within Section 12, Township 5 North, Range 4 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. The Development is bound to the west by North Hayden Road, Sunflower Estates development and Black Mountain Road to the north, North 81st Street to the east, and Westland Estates development to the south. See **Appendix A: Site Location Map**

The 2.9 acre site is a single family development, consisting of 8 single family residential units. Currently, the property is composed of undeveloped desert rangeland. The site is currently zoned as R1-43 and is going to be rezoned as R1-43 PAD.

DISTRIBUTION SYSTEM DESCRIPTION

EXISTING COLLECTION SYSTEM

The site is surrounded by existing single family residential developments. Per the City of Scottsdale Quarter Section Map 58-47 there is an existing 8-inch PVC sewer line in North Hayden Road that flows south to a 10-inch PVC main in Westland Road.

PROPOSED COLLECTION SYSTEM

The wastewater flows generated by Lots 1-6 will be collected and conveyed through a new 8-inch PVC sewer line to the existing 8-inch PVC sewer main in Hayden Road. This proposed sewer line will connect to the existing Hayden Road sewer line near the midpoint of the west property line. A second 8-inch PVC sewer line will be installed along the south property line and collect the wastewater flow generated by Lots 7 & 8. This proposed sewer line will connect to existing 8-inch PVC sewer line in Hayden Road near the southwest corner of the site. Refer to **Appendix B** for the Proposed Sewer System Layout Exhibit.

BASIS OF DESIGN

DESIGN METHODOLOGY

Average Day Demand design flows are calculated based on design criteria detailed within the City of Scottsdale Design Standards and Polices Manual (DS&PM). Per DS&PM Chapter 7, a design flow of 100 gallons per capita per day (gpcpd) shall be used. The DS&PM also requires a peaking factor of 4.0 and a residential density of 2.5 persons per dwelling unit.

WASTEWATER SYSTEM ANALYSIS

See **Table 1** below for a summary of sewer demands.

Table 1 Sewer Demands - Lots 1-6

Land Use	Dwelling units (du)	Density (persons/ du)	Population (persons)	Average Day Demand (gpd)	Peak Flow (gpd)	Peak Flow (gpm)
<2 du/ac	6	2.5	15	1,500	6,000	4.17

Table 2 Sewer Demands - Lots 7 & 8

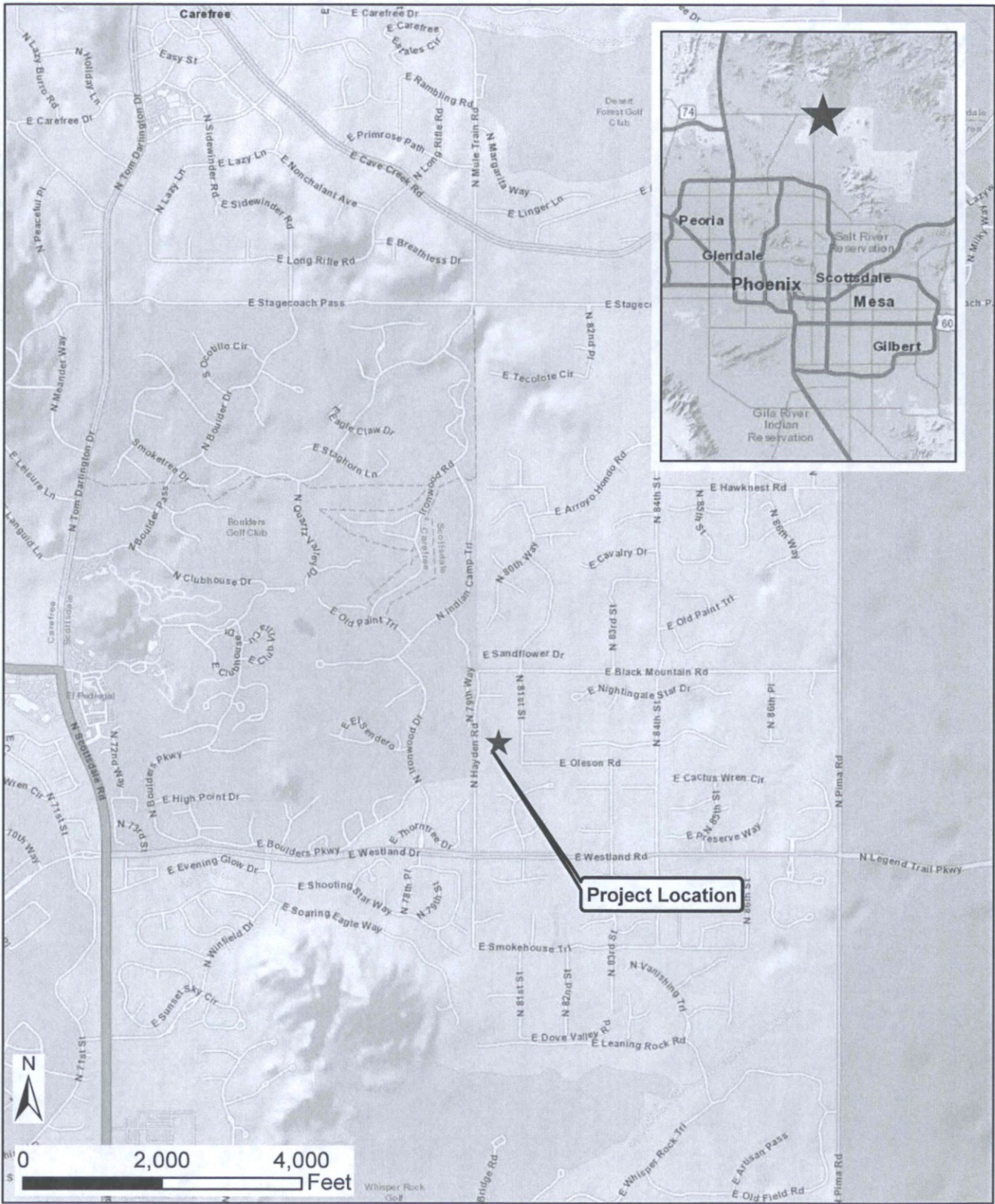
Land Use	Dwelling units (du)	Density (persons/ du)	Population (persons)	Average Day Demand (gpd)	Peak Flow (gpd)	Peak Flow (gpm)
<2 du/ac	2	2.5	5	500	1,000	0.69

Per the DS&PM proposed sewer lines shall be designed to achieve a full flow velocity of between 2.5 and 10 feet per second and maintain a maximum d/D ratio of 0.65 when calculated with a Manning's "n" value of 0.013. An 8-inch sewer will satisfy these requirements using a minimum slope of 0.0052 ft/ft (0.52%) and a maximum slope of 0.0833 ft/ft (8.33%). See **Appendix C** for pipe slope calculations.

RESULTS

To determine the capacity of the proposed wastewater collection system, the peak design flow was analyzed within the minimum design pipe slope for the 8-inch PVC sewer line servicing Lots 1-6. At the minimum design slope of 0.0052 ft/ft an 8-inch line has the capacity to convey approximately 391.09 gallons per minute. The proposed peak design flow of 4.17 gallons per min has a normal depth of 0.52 feet or a d/D ratio of 0.065 at the minimum design slope. See **Appendix C – Flowmaster Calculations** for pipe capacity calculations.

Appendix A – Site Location Map



Kimley»Horn
 Expect More. Experience Better.

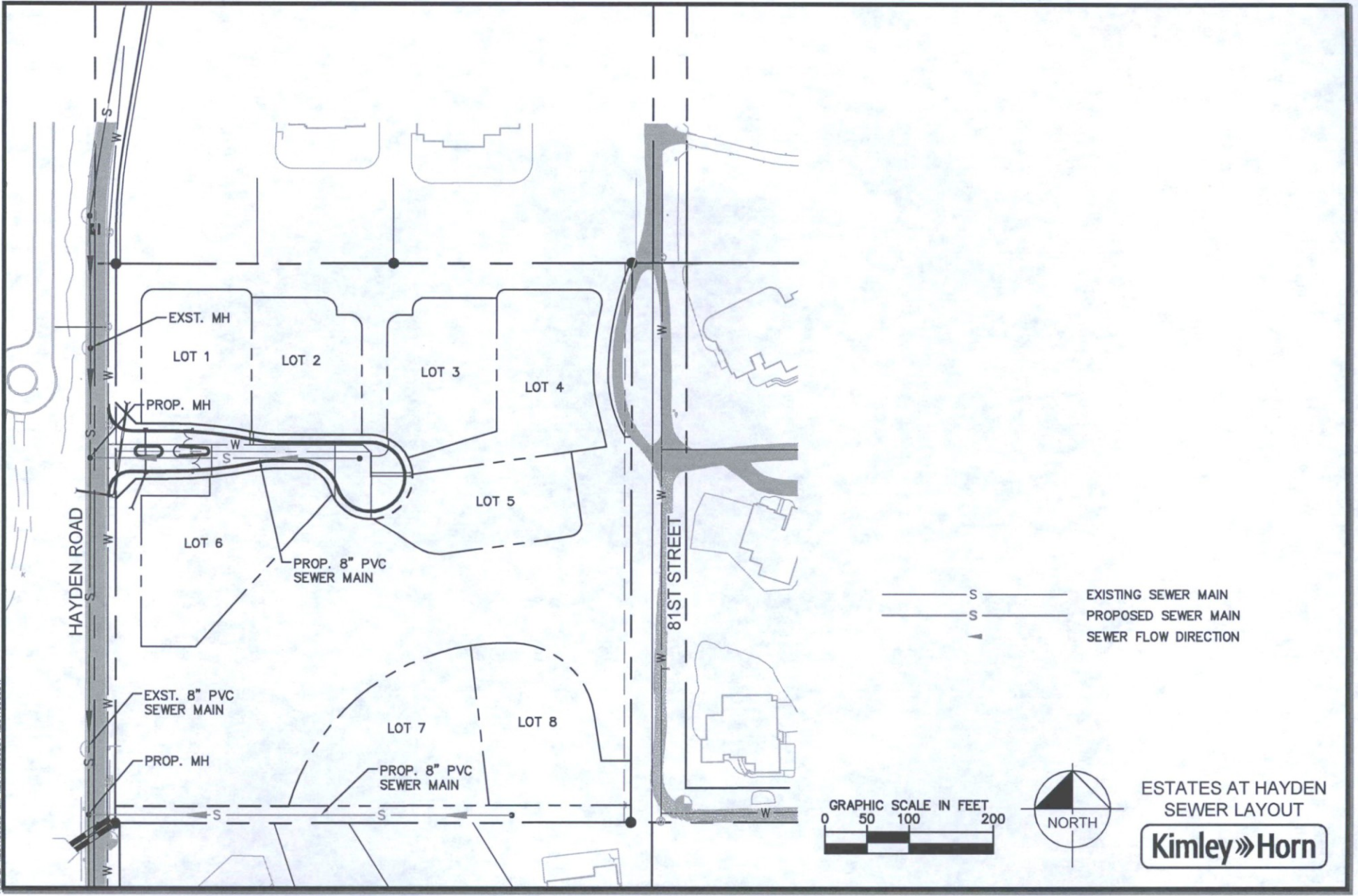
Estates At Hayden

Scottsdale, AZ

Site Location Map

Appendix B – Proposed Sewer System Layout

K:\P\K_C\A\201109000 - Estates at Hayden\CAD\Exhibits\2016-03-01 Sewer Layout Exhibit.dwg Mar 01, 2016 Garrett\Fram



— S — EXISTING SEWER MAIN
- - S - - PROPOSED SEWER MAIN
▲ SEWER FLOW DIRECTION

GRAPHIC SCALE IN FEET
0 50 100 200



ESTATES AT HAYDEN
SEWER LAYOUT



Appendix C – Flowmaster Calculations

Worksheet for 8" Sewer Minimum Slope Capacity

Project Description

Friction Method Manning Formula
Solve For Full Flow Capacity

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00520	ft/ft
Normal Depth	8.00	in
Diameter	8.00	in
Discharge	391.09	gal/min

Results

Discharge	391.09	gal/min
Normal Depth	8.00	in
Flow Area	0.35	ft ²
Wetted Perimeter	2.09	ft
Hydraulic Radius	2.00	in
Top Width	0.00	ft
Critical Depth	0.44	ft
Percent Full	100.0	%
Critical Slope	0.00857	ft/ft
Velocity	2.50	ft/s
Velocity Head	0.10	ft
Specific Energy	0.76	ft
Froude Number	0.00	
Maximum Discharge	0.94	ft ³ /s
Discharge Full	0.87	ft ³ /s
Slope Full	0.00520	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.00	in
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	in
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%

Worksheet for 8" Sewer Minimum Slope Capacity

GVF Output Data

Normal Depth Over Rise	100.00	%
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	8.00	in
Critical Depth	0.44	ft
Channel Slope	0.00520	ft/ft
Critical Slope	0.00857	ft/ft

Worksheet for 8" Sewer Maximum Slope Capacity

Project Description

Friction Method Manning Formula
Solve For Full Flow Capacity

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.08330	ft/ft
Normal Depth	8.00	in
Diameter	8.00	in
Discharge	1565.30	gal/min

Results

Discharge	1565.30	gal/min
Normal Depth	8.00	in
Flow Area	0.35	ft ²
Wetted Perimeter	2.09	ft
Hydraulic Radius	2.00	in
Top Width	0.00	ft
Critical Depth	0.66	ft
Percent Full	100.0	%
Critical Slope	0.07763	ft/ft
Velocity	9.99	ft/s
Velocity Head	1.55	ft
Specific Energy	2.22	ft
Froude Number	0.00	
Maximum Discharge	3.75	ft ³ /s
Discharge Full	3.49	ft ³ /s
Slope Full	0.08330	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.00	in
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	in
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%

Worksheet for 8" Sewer Maximum Slope Capacity

GVF Output Data

Normal Depth Over Rise	100.00	%
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	8.00	in
Critical Depth	0.66	ft
Channel Slope	0.08330	ft/ft
Critical Slope	0.07763	ft/ft

Worksheet for 8" Sewer Normal Depth at Design Flow

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Roughness Coefficient	0.010	
Channel Slope	0.00520	ft/ft
Diameter	8.00	in
Discharge	4.17	gal/min

Results

Normal Depth	0.52	in
Flow Area	0.01	ft ²
Wetted Perimeter	0.34	ft
Hydraulic Radius	0.33	in
Top Width	0.33	ft
Critical Depth	0.04	ft
Percent Full	6.5	%
Critical Slope	0.00500	ft/ft
Velocity	0.97	ft/s
Velocity Head	0.01	ft
Specific Energy	0.06	ft
Froude Number	1.01	
Maximum Discharge	1.22	ft ³ /s
Discharge Full	1.13	ft ³ /s
Slope Full	0.00000	ft/ft
Flow Type	SuperCritical	

GVF Input Data

Downstream Depth	0.00	in
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	in
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	6.46	%
Downstream Velocity	Infinity	ft/s

Worksheet for 8" Sewer Normal Depth at Design Flow

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.52	in
Critical Depth	0.04	ft
Channel Slope	0.00520	ft/ft
Critical Slope	0.00500	ft/ft

SEWER BASIS OF DESIGN

Estates at Hayden

REVISE & RESUBMIT

City of Scottsdale
Water Resources Administration
9379 E. San Salvador
Scottsdale, AZ 85258

Rehman. 04/23/2018.

Prepared for:

Blandford Homes
3321 E. Baseline Rd.
Gilbert, Arizona 85234

Prepared by:

Kimley-Horn and Associates, Inc.
7740 N. 16th Street, Suite 300
Phoenix, Arizona 85020



ET 3/31/18



Final Sewer Basis of Design

ESTATES AT HAYDEN

MARCH 2018

Prepared By:

Kimley»»Horn

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WASTEWATER SYSTEM ANALYSIS

See **Table 1** below for a summary of sewer demands.

See Comment Below. Include offsite sewer from 81st St.

Table 1 Sewer Demands - Lots 1-6

Land Use	Dwelling units (du)	Density (persons/ du)	Population (persons)	Average Day Demand (gpd)	Peak Flow (gpd)	Peak Flow (gpm)
<2 du/ac	6	2.5	15	1,500	6,000	4.17

Table 2 Sewer Demands - Lots 7 & 8

Land Use	Dwelling units (du)	Density (persons/ du)	Population (persons)	Average Day Demand (gpd)	Peak Flow (gpd)	Peak Flow (gpm)
<2 du/ac	2	2.5	5	500	1,000	0.69

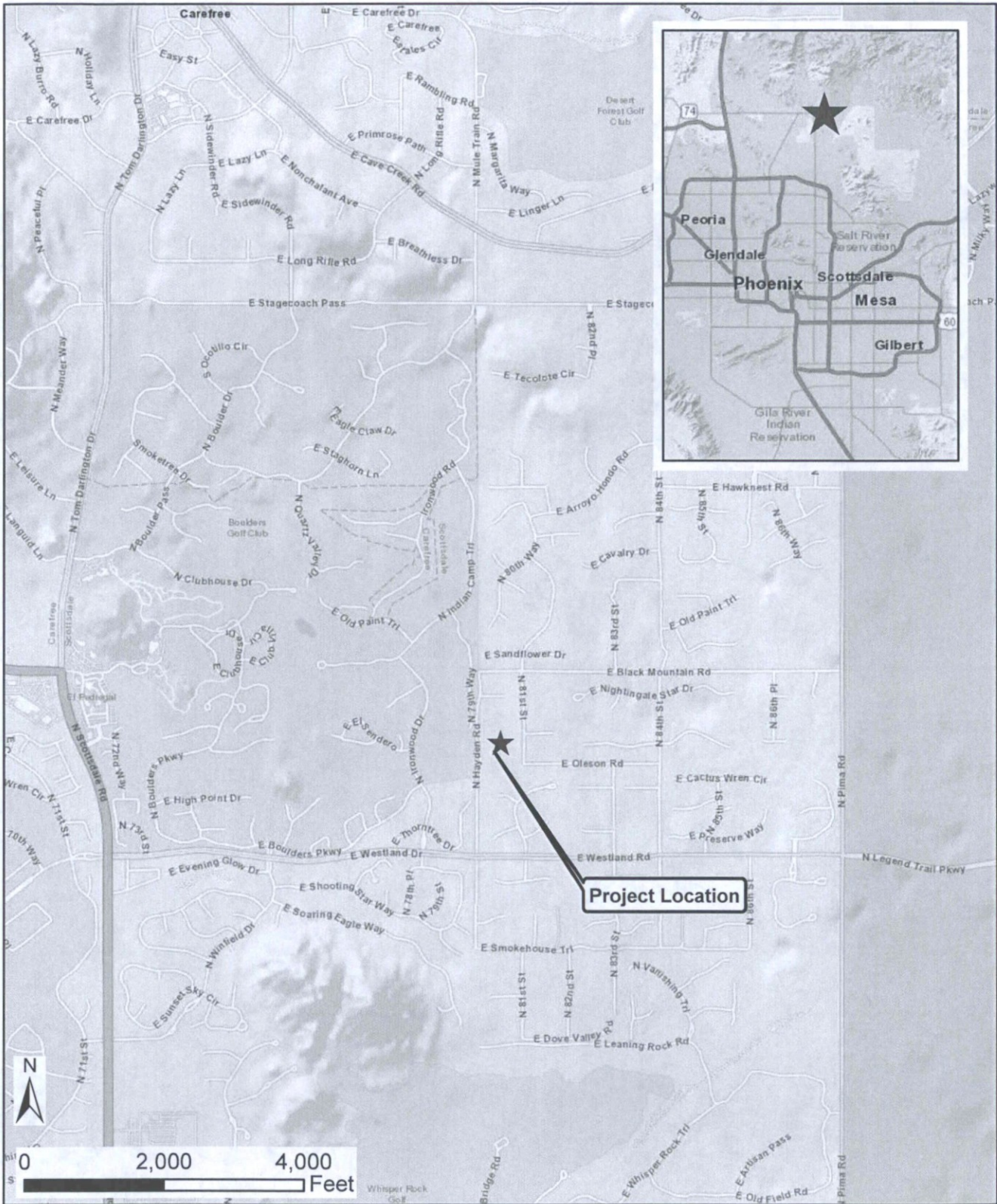
Per the DS&PM proposed sewer lines shall be designed to achieve a full flow velocity of between 2.5 and 10 feet per second and maintain a maximum d/D ratio of 0.65 when calculated with a Manning's "n" value of 0.013. An 8-inch sewer will satisfy these requirements using a minimum slope of 0.0052 ft/ft (0.52%) and a maximum slope of 0.0833 ft/ft (8.33%). See **Appendix C** for pipe slope calculations. ✓

RESULTS

To determine the capacity of the proposed wastewater collection system, the peak design flow was analyzed within the minimum design pipe slope for the 8-inch PVC sewer line servicing Lots 1-6. At the minimum design slope of 0.0052 ft/ft an 8-inch line has the capacity to convey approximately 391.09 gallons per minute. The proposed peak design flow of 4.17 gallons per min has a normal depth of 0.52 feet or a d/D ratio of 0.065 at the minimum design slope. See **Appendix C – Flowmaster Calculations** for pipe capacity calculations. ✓

See Comment on Appendix B - Lay out Regarding Sewer line Extension Along Property Frontages. Expand Capacity Analysis to include Sewer generated from 81st St. Alignment (off-site sewer).

Appendix A – Site Location Map



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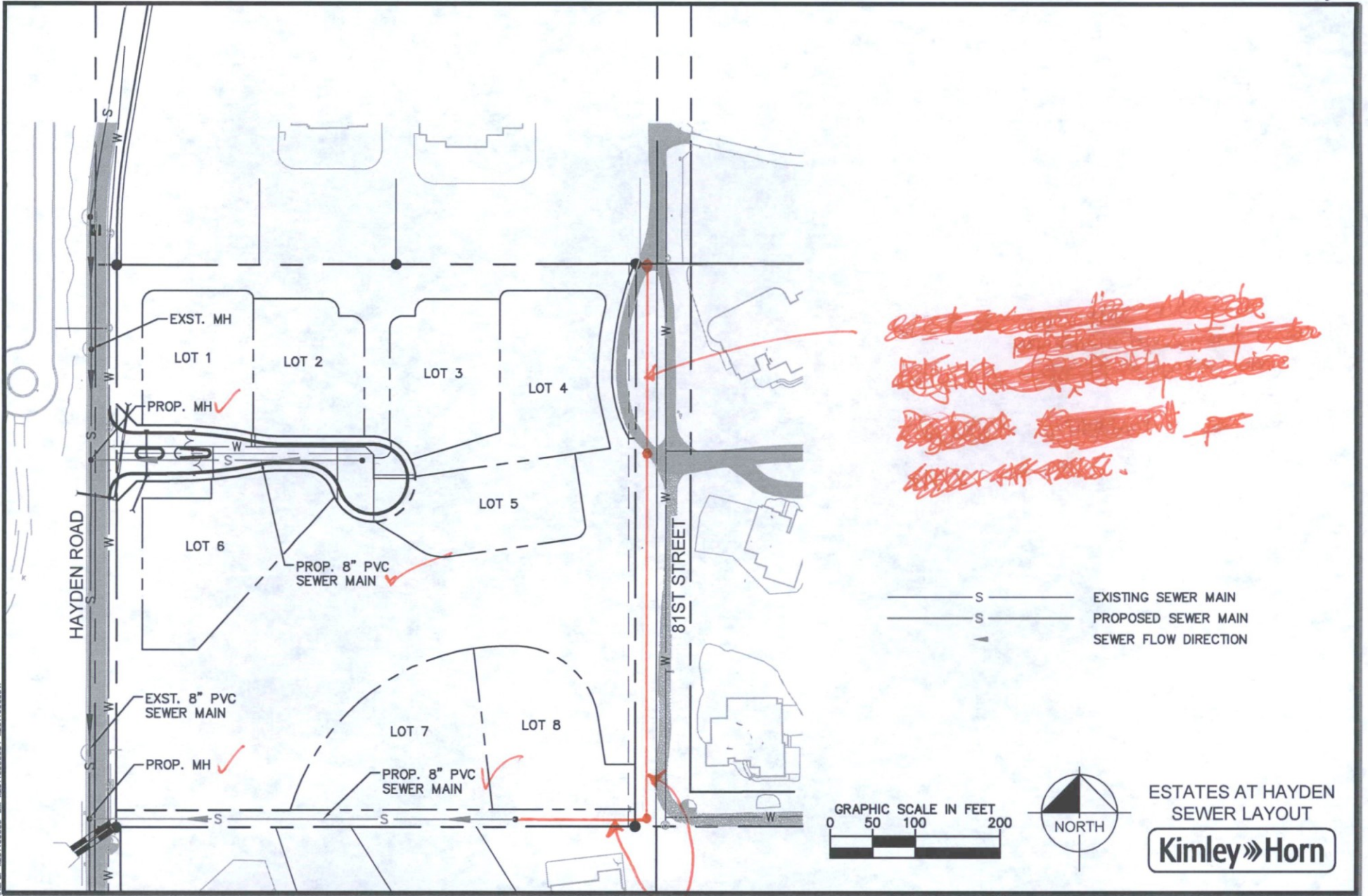
Estates At Hayden

Scottsdale, AZ

Site Location Map

Appendix B – Proposed Sewer System Layout

K:\PRK_C2\15010000 - Estates at Hayden\CD\Exhibits\2016-03-01 Sewer Layout Exhibit.dwg Mar 01, 2016 Garrett.Fram



~~Install/Extend sewer line along property frontages (sewer easement + 81st St) per DSPM, Section 7-1.400.~~

~~Install/Extend sewer line along property frontages (sewer easement + 81st St) per DSPM, Section 7-1.400.~~

~~Install/Extend sewer line along property frontages (sewer easement + 81st St) per DSPM, Section 7-1.400.~~

~~Install/Extend sewer line along property frontages (sewer easement + 81st St) per DSPM, Section 7-1.400.~~

Install/Extend sewer line Along Property Frontages (sewer Easement + 81st St) Per DSPM, Section 7-1.400.

Appendix C – Flowmaster Calculations

Worksheet for 8" Sewer Minimum Slope Capacity

Project Description

Friction Method Manning Formula
Solve For Full Flow Capacity

Input Data

Roughness Coefficient 0.013
Channel Slope 0.00520 ft/ft
Normal Depth 8.00 in
Diameter 8.00 in
Discharge 391.09 gal/min

Results

Discharge 391.09 gal/min
Normal Depth 8.00 in
Flow Area 0.35 ft²
Wetted Perimeter 2.09 ft
Hydraulic Radius 2.00 in
Top Width 0.00 ft
Critical Depth 0.44 ft
Percent Full 100.0 %
Critical Slope 0.00857 ft/ft
Velocity 2.50 ft/s
Velocity Head 0.10 ft
Specific Energy 0.76 ft
Froude Number 0.00
Maximum Discharge 0.94 ft³/s
Discharge Full 0.87 ft³/s
Slope Full 0.00520 ft/ft
Flow Type SubCritical

GVF Input Data

Downstream Depth 0.00 in
Length 0.00 ft
Number Of Steps 0

GVF Output Data

Upstream Depth 0.00 in
Profile Description
Profile Headloss 0.00 ft
Average End Depth Over Rise 0.00 %

Worksheet for 8" Sewer Minimum Slope Capacity

GVF Output Data

Normal Depth Over Rise	100.00	%
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	8.00	in
Critical Depth	0.44	ft
Channel Slope	0.00520	ft/ft
Critical Slope	0.00857	ft/ft

Worksheet for 8" Sewer Maximum Slope Capacity

Project Description

Friction Method Manning Formula
Solve For Full Flow Capacity

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.08330	ft/ft
Normal Depth	8.00	in
Diameter	8.00	in
Discharge	1565.30	gal/min

Results

Discharge	1565.30	gal/min
Normal Depth	8.00	in
Flow Area	0.35	ft ²
Wetted Perimeter	2.09	ft
Hydraulic Radius	2.00	in
Top Width	0.00	ft
Critical Depth	0.66	ft
Percent Full	100.0	%
Critical Slope	0.07763	ft/ft
Velocity	9.99	ft/s
Velocity Head	1.55	ft
Specific Energy	2.22	ft
Froude Number	0.00	
Maximum Discharge	3.75	ft ³ /s
Discharge Full	3.49	ft ³ /s
Slope Full	0.08330	ft/ft
Flow Type	SubCritical	

*Per DSPM, Section
7-1.404, Ultimate
Peak flow Condition
shall be evaluated
@ $d/D = 0.65$.*

GVF Input Data

Downstream Depth	0.00	in
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	in
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%

Worksheet for 8" Sewer Maximum Slope Capacity

GVF Output Data

Normal Depth Over Rise	100.00	%
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	8.00	in
Critical Depth	0.66	ft
Channel Slope	0.08330	ft/ft
Critical Slope	0.07763	ft/ft

Worksheet for 8" Sewer Normal Depth at Design Flow

Project Description

Friction Method Manning Formula
Solve For Normal Depth

Input Data

Roughness Coefficient 0.010
Channel Slope 0.00520 ft/ft
Diameter 8.00 in
Discharge 4.17 gal/min

Results

Normal Depth 0.52 in
Flow Area 0.01 ft²
Wetted Perimeter 0.34 ft
Hydraulic Radius 0.33 in
Top Width 0.33 ft
Critical Depth 0.04 ft
Percent Full 6.5 %
Critical Slope 0.00500 ft/ft
Velocity 0.97 ft/s
Velocity Head 0.01 ft
Specific Energy 0.06 ft
Froude Number 1.01
Maximum Discharge 1.22 ft³/s
Discharge Full 1.13 ft³/s
Slope Full 0.00000 ft/ft
Flow Type SuperCritical

GVF Input Data

Downstream Depth 0.00 in
Length 0.00 ft
Number Of Steps 0

GVF Output Data

Upstream Depth 0.00 in
Profile Description
Profile Headloss 0.00 ft
Average End Depth Over Rise 0.00 %
Normal Depth Over Rise 6.46 %
Downstream Velocity Infinity ft/s

Worksheet for 8" Sewer Normal Depth at Design Flow

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.52	in
Critical Depth	0.04	ft
Channel Slope	0.00520	ft/ft
Critical Slope	0.00500	ft/ft

ZONING DRAINAGE REPORT

Estates at Hayden

Prepared for:

Blandford Homes

Submitted to:

City of Scottsdale

7447 E. Indian School Road
Scottsdale, Arizona 85251

Prepared by:

Kimley-Horn

7740 N. 16th Street
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Stormwater Review By:
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Phone 480-312-2563 Fax 480-312-7731
e-mail: mrahman@ScottsdaleAZ.gov
Review Cycle #1 Date 4/8/18

Kimley»»Horn

291109000
March 2018

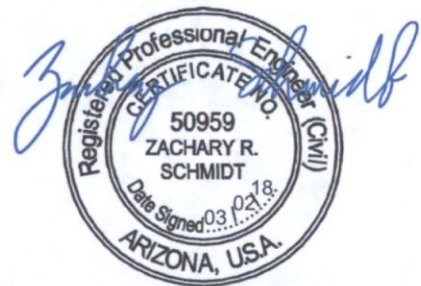
5-ZN-2018
04/03/18

ZONING DRAINAGE REPORT

ESTATES AT HAYDEN

MARCH 2018

Prepared By:



Expires 06/30/19

Kimley»»Horn

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Appendices

- Appendix A – Existing Conditions Hydrology
- Appendix B – Proposed Conditions Hydrology
- Appendix C – Upper Boulders Wash Hydraulics
- Appendix D – Stormwater Storage
- Appendix E. Disclaimer and Liability

Figures

Figure 1A: Location Map

Figure 1B: Aerial Map

Figure 2: FIRMette Panel 04013C0895L

Figure 3. Existing Conditions Hydrology

Figure 4. Proposed Conditions Hydrology

Figure 5. Proposed Hydraulics Map

INTRODUCTION

PURPOSE

This Zoning Drainage Report for Estates at Hayden (Site) has been prepared to meet the drainage requirements outlined in Chapter 4 of the City of Scottsdale Design Standards and Policies Manual (DS&PM). The Site is currently zoned R1-43 and will be re-zoned to R1-43 PAD.

The main purposes of this report are the following:


- Illustrate compliance with the *DS&PM*.
- Establish drainage parameters and criteria for planning and zoning.
- Provide a preliminary hydrologic analysis for the development of the Site.
- Provide a hydraulic analysis for the impact of the Site on the existing floodplain and floodway for Upper Boulders Wash.

PROJECT LOCATION AND DESCRIPTION

The Site is bounded on the west by North Hayden Road, north by the Sunflower Estates development and Black Mountain Road, east by North 81st Street and south by the Westland Estates development. It lies within a portion of Section 12, Township 5 North, Range 4 East of the Gila and Salt River Base and Meridian. The Site is located within the City of Scottsdale (City) and falls under the City's Environmentally Sensitive Lands Overlay (ESLO). The Site consists of eight (8) lots in the nine-acre (9-acre) site. It is currently zoned R1-43. See **Figure 1A** and **Figure 1B** for the Location and Aerial Maps.

The Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) for Maricopa County, Arizona and incorporated areas, Panel Number 04013C0895L, effective date October 16, 2013, indicates that the Upper Boulders Wash runs through the Site. Upper Boulders Wash is a defined Zone AE floodplain with regulatory floodway. The rest of the site is Zone X. The FIRM Panel is included as **Figure 2**.



 <p>Expect More. Experience Better.</p>	Estates At Hayden	Scottsdale, AZ
	Figure 1A. Location Map	



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Estates At Hayden

Scottsdale, AZ

Figure 1B. Aerial Photo Map

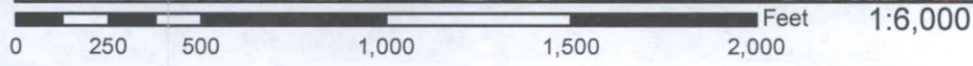
National Flood Hazard Layer FIRMeTte



FEMA

33°48'2.16"N

111°54'45.03"W



Source: Esri, DigitalGlobe, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth
		Regulatory Floodway Zone AE, AO, AH, VE, AR
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes, Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		Area of Minimal Flood Hazard Zone X
		Effective LOMRs
		Area of Undetermined Flood Hazard Zone D
GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		Cross Sections with 1% Annual Chance Water Surface Elevation
		Cross Sections with 1% Annual Chance Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
MAP PANELS		Digital Data Available
		No Digital Data Available
		Unmapped

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The base map shown complies with FEMA's base map accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 2/27/2018 at 12:02:12 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: base map imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



111°54'45.03"W

DESCRIPTION OF EXISTING DRAINAGE CONDITIONS AND CHARACTERISTICS

EXISTING OFFSITE DRAINAGE CONDITIONS

The Site is located within the North Scottsdale Floodplain Delineation Study (NSFDS). Floodplain limits and Base Flood Elevations (BFE) for Upper Boulders Wash were established as part of the NSFDS. Upper Boulders Wash flows generally from northeast to the southwest through the Site. Upper Boulders Wash crosses under Hayden Road in a two (2) barrel 6-ft x 4-ft box culvert. Additionally, a small wash enters at the northeast corner of the Site. The wash passes through the Site to an existing 24-inch culvert under Hayden Road. Refer to **Figure 3** for the Existing Conditions Map.

EXISTING ONSITE DRAINAGE CONDITIONS

The Site is undeveloped. A majority of the site drains towards Upper Boulders Wash. The north portion of the Site drains towards the 24-inch culvert crossing Hayden Road. No additional significant washes or Vista Corridors cross the Site. Refer to **Figure 3** for the Existing Conditions Map.

CONTEXT RELATIVE TO ADJACENT PROJECTS AND IMPROVEMENTS

Runoff in Upper Boulders Wash flows over 81st Street onto the Site. After exiting the Site through a culvert under Hayden Road, Upper Boulders Wash continues through the crossing at the Boulders subdivision to the west.

PROPOSED DRAINAGE PLAN

PROPOSED ONSITE DRAINAGE PLAN

Upper Boulders Wash will not be impacted with the project. The wash entering the Site at the northeast corner will be re-routed along the north side of the site where it will connect through a small storm drain to the existing 24-inch culvert crossing Hayden Road similar to existing conditions.

Proposed onsite sub-basin boundaries were delineated from the proposed overall grading concept. The onsite lots are directed to drain towards the streets and in some cases toward Upper Boulders Wash. The runoff is conveyed in the street to a new storm drain system into a detention basin (D1). The storm drain system will be designed as part of final design. Detention basin D1 has a six (6) inch orifice opening located six (6) inches above the basin bottom. The outlet discharges into Upper Boulders Wash. The detention basin is designed with an overflow weir six (6) inches below the top of the basin. The orifice and overflow weirs are being used to control post-development peak discharges. Refer to **Figure 4** and **Figure 5** for the proposed hydrology and hydraulics maps.

PROPOSED ONSITE HYDROLOGY

The development of the Site minimizes increases in post-development runoff to pre-development conditions where runoff exits the Site for the three design storms (2-, 10- and 100-year). Due to the size and timing of the watershed for Upper Boulders Wash it is not achievable to reduce post-development peak discharge to a level at or below the pre-development peak discharges. However, increases are negligible. Refer to **Table 1** for a summary of pre- and post-development discharges. Detention basin D1 is used to attenuate peak discharges from onsite runoff. Refer to **Figure 4** for the Proposed Hydrology Map. Refer to **Appendix B** for detailed hydrologic calculations and results.

Table 1. Pre- Versus Post-Development Runoff

HEC-1 Name		Q ₂ [cfs]		Q ₁₀ [cfs]		Q ₁₀₀ [cfs]	
Pre	Post	Pre	Post	Pre	Post	Pre	Post
CPON05	RON05	8	6	19	14	39	29
CPON15	CPON15	428	428	877	879	1632	1634
ON10	N/A	1	0	1	0	3	0

PROPOSED ONSITE HYDRAULICS

Onsite runoff will be conveyed in local streets and storm drains to the detention basins. Per the DS&PM, all interior streets will be designed to convey the peak discharge from the 10-year storm event at or below the top of curb elevation. Additionally, the streets will convey the 100-year runoff within the proposed tracts and maintain a maximum flow depth of eight inches above the gutter flow line. Catch basins will capture pavement runoff and outfall to the proposed detention basin. Catch basins, curb cuts, and storm drains will be designed per the DS&PM and Flood Control District of Maricopa County (FCDMC) Drainage Policies and Standards as part of the final design.

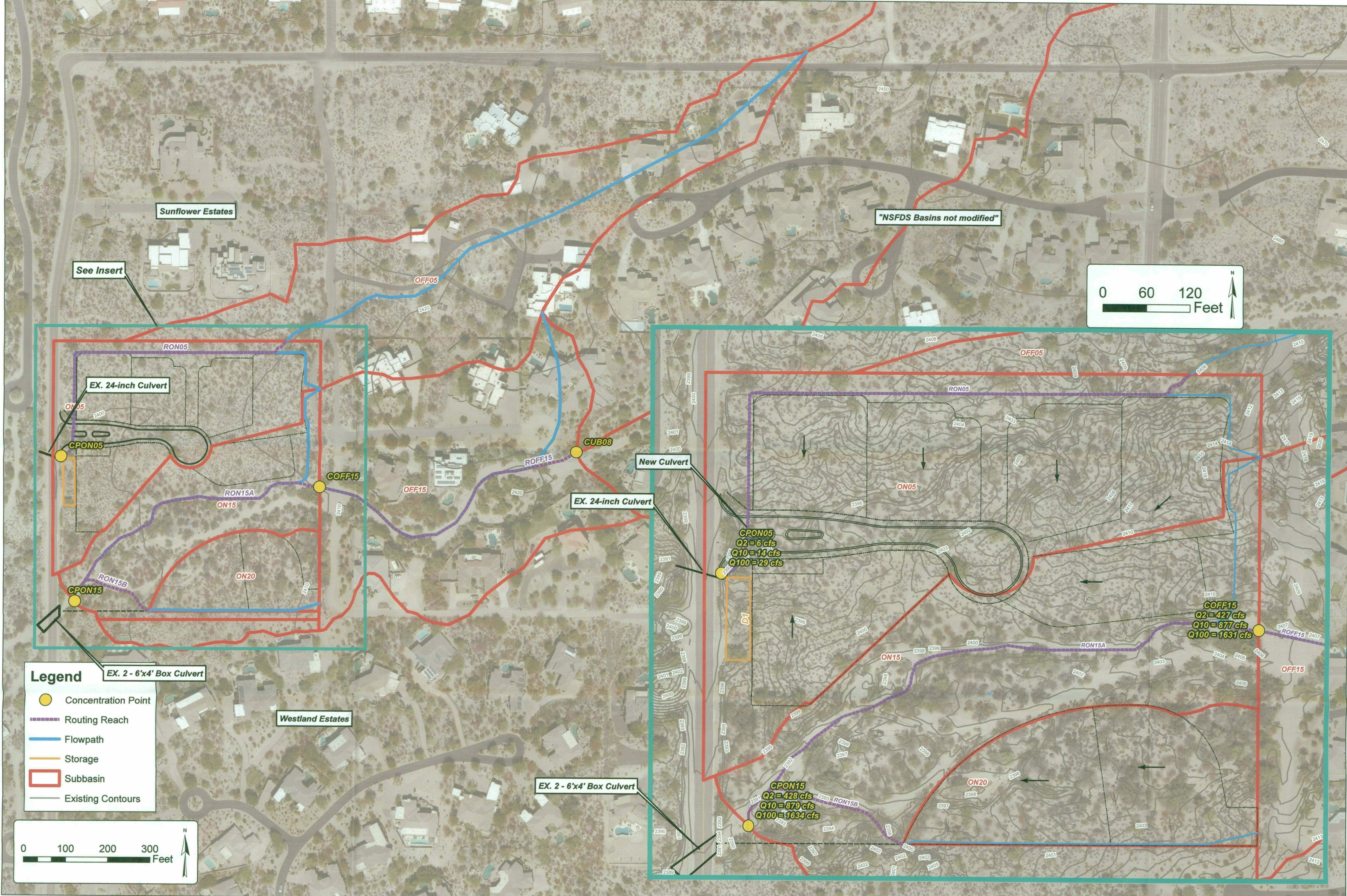
PROPOSED OFFSITE DRAINAGE PLAN

The Upper Boulders Wash floodplain will be revised with the Site development. A Conditional Letter of Map Revisions (CLOMR) application that meets FEMA requirements will be prepared as a separate report. The CLOMR application will be prepared with the Preliminary Drainage Report. The hydraulic analysis required with the CLOMR has been completed for this project. The Site will be developed to match water surface elevations and velocities at the property boundaries. Adjacent lots will be set at least one foot above the base flood elevations (BFE) from the post-project model per the DS&PM. The box culvert under Hayden Road will not be impacted with the Site. Refer to **Figure 5** for proposed floodplain mapping. Refer to **Appendix C** for wash hydraulics.

The wash entering the northeast portion of the Site will be re-routed around the north part of the Site in a new channel. The channel will discharge into a new small storm drain that will discharge into the existing 24-inch culvert crossing Hayden Road. This drainage pattern is similar to the existing condition. Hydraulic analysis for the storm drain and new channel will be completed with final design.

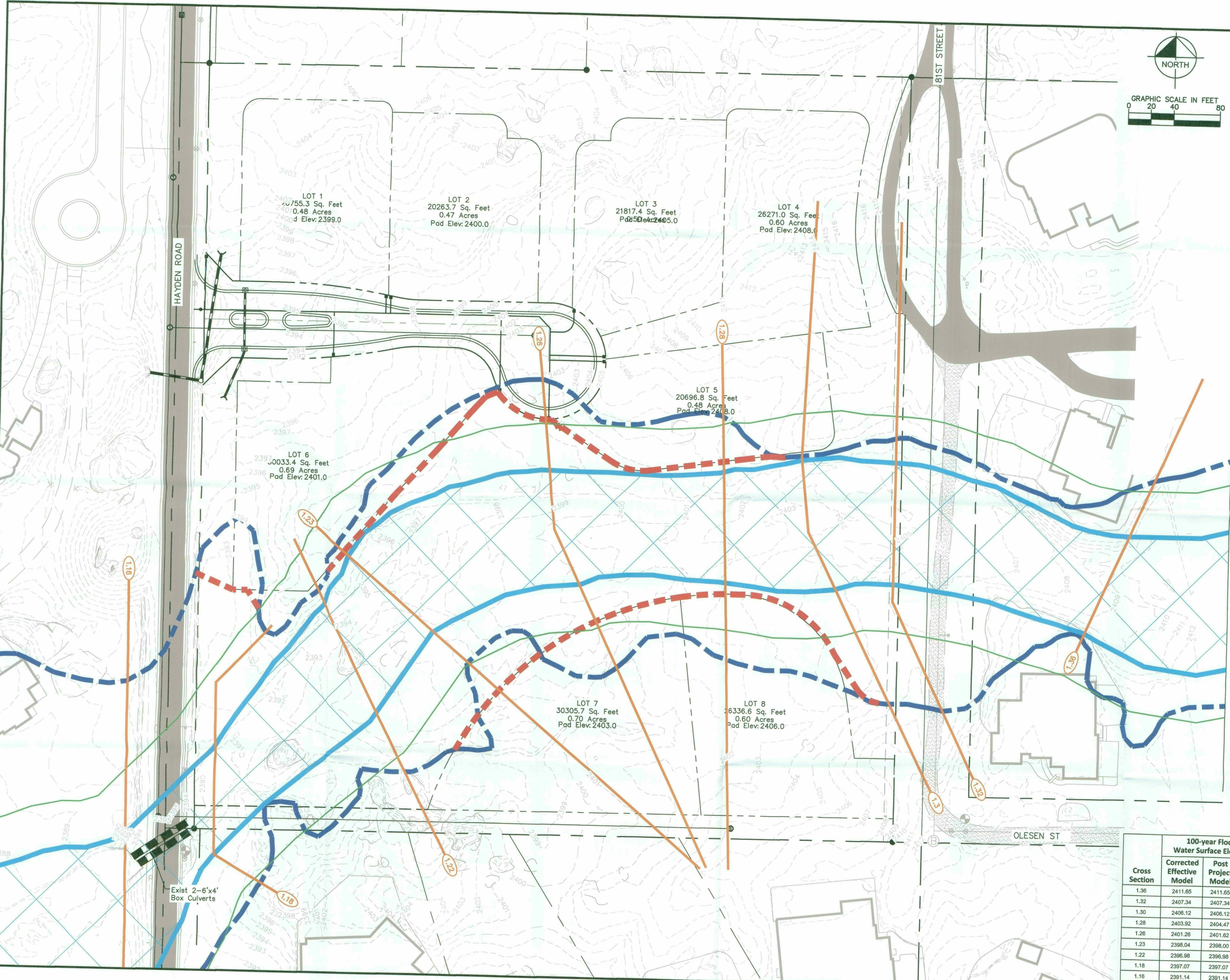
PROPOSED PROJECT PHASING

The project will be constructed in one phase.



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		SCALE(V)	N/A
		DESIGNED BY	ZRS
		DRAWN BY	BAH
CHECKED BY	ZRS	DATE:	03/20/18
<p>CITY OF SCOTTSDALE ESTATES AT HAYDEN PROPOSED CONDITIONS FIGURE 4</p>		PROJECT NO.	291109000
		DRAWING NAME	FIGURE 4

K:\PHX_Civil\29109000 - Estates at Hayden\Drawings\Hydraulics.dwg Mar 01, 2018 caroline.egg
 XREFS: 291109000-FRM 291109000V 291109000V 291109000V
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HAYDEN ROAD

81ST STREET

OLESEN ST

Exist 2'-6"x4' Box Culverts

LOT 1
20755.3 Sq. Feet
0.48 Acres
Pad Elev: 2399.0

LOT 2
20263.7 Sq. Feet
0.47 Acres
Pad Elev: 2400.0

LOT 3
21817.4 Sq. Feet
0.50 Acres
Pad Elev: 2405.0

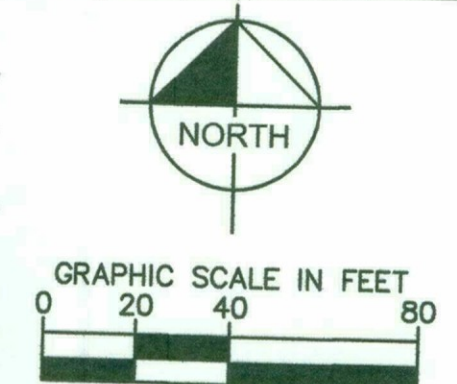
LOT 4
26271.0 Sq. Feet
0.60 Acres
Pad Elev: 2408.0

LOT 5
20696.8 Sq. Feet
0.48 Acres
Pad Elev: 2403.0

LOT 6
30033.4 Sq. Feet
0.69 Acres
Pad Elev: 2401.0

LOT 7
30305.7 Sq. Feet
0.70 Acres
Pad Elev: 2403.0

LOT 8
26336.6 Sq. Feet
0.60 Acres
Pad Elev: 2406.0



- LEGEND**
- — — — EXISTING FLOODPLAIN
 - — — — PROPOSED FLOODPLAIN
 - — — — EROSION SETBACK
 - — — — REGULATORY FLOODWAY
 - 1.36 HEC-RAS CROSS SECTION

Cross Section	100-year Floodplain Water Surface Elevation [ft]			100-year Floodway Water Surface Elevation [ft]		
	Corrected Effective Model	Post Project Model	Change	Corrected Effective Model	Post Project Model	Change
1.36	2411.65	2411.65	0.00	2411.57	2411.57	0.00
1.32	2407.34	2407.34	0.00	2407.32	2407.32	0.00
1.30	2406.12	2406.12	0.00	2406.12	2406.12	0.00
1.28	2403.92	2404.47	0.55	2404.55	2404.55	0.00
1.26	2401.26	2401.62	0.36	2402.00	2402.00	0.00
1.23	2398.04	2398.00	-0.04	2399.06	2399.06	0.00
1.22	2396.98	2396.98	0.00	2397.67	2397.67	0.00
1.18	2397.07	2397.07	0.00	2397.79	2397.79	0.00
1.16	2391.14	2391.14	0.00	2391.14	2391.14	0.00

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SCALE (H): 1"=40' SCALE (V): NONE	DESIGNED BY: CEO DRAWN BY: CEO CHECKED BY: ZRS	DATE: 3/2018
CITY OF SCOTTSDALE ESTATES AT HAYDEN PROPOSED HYDRAULICS WORKMAP FIGURE 5		
PROJECT NO. 291109000	DRAWING NAME HYDRAULICS	
1 OF 1	NO.	REVISION
		DATE

SPECIAL CONDITIONS

ADEQ REQUIREMENTS

A stormwater management plan will be prepared to meet Arizona Department of Environmental Quality (ADEQ) requirements. The stormwater management plan is prepared under the Arizona Pollutant Discharge Elimination System (AZPDES) program. The stormwater management plan is a part of a separate document and is not included with this report.

DATA ANALYSIS METHODS

HYDROLOGY

The U.S. Army Corps of Engineers HEC-1 hydrologic computer program was used to determine the 2-, 10, and 100-year peak discharges for offsite and onsite flows. Models were prepared for both the pre- and post-development conditions. The NSFDS HEC-1 model from May 2002 was used as a basis for the hydrology. This model uses National Oceanic Atmospheric Administration Atlas 2 (NOAA2) rainfall which has since been replaced by NOAA Atlas 14 rainfall depths. The pre- and post-development conditions were modeled using the NOAA 2 rainfall depths from the NSFDS. The NOAA 2 rainfall depths were compared to the NOAA 14 rainfall depths and were determined to be higher. Using the higher NOAA 2 rainfall values in the hydrologic model is more conservative and will reflect similar results of the NSFDS and the effective hydrology used to map the Upper Boulders Wash floodplain. Existing parameters from the NSFDS were not changed for the hydrologic model. The Drainage Design Management System for Windows (DDMSW) program was used to develop the hydrologic parameters for the onsite drainage areas. The onsite sub-basins were added to the NSFDS hydrologic model.

The “Non-default value or value out of range” note in the proposed conditions parameters in Appendix B is because of the varying sub-basin sizes and flowpaths. Additionally, input parameters from NSFDS were input into DDMSW. The selected NMIN parameter will not meet the time of concentration requirements specified in the FCDMC Drainage Design Manual – Hydrology for the varying sub-basin sizes.

Green and Ampt rainfall loss parameters for the onsite basins were estimated using DDMSW, the City parameters, and the FCDMC Drainage Design Manual – Hydrology. The Clark Unit hydrograph was used. The existing onsite land use is undeveloped desert. No changes were made to the existing offsite land use types. **Table 2** is a summary of the land use parameters used for the onsite hydrologic model. The values shown were taken from the FCDMC Drainage Policies and Standards. Proposed land use maps are included in **Appendix B**

Table 2. Land Use Parameters

Land Use Code	Land Use Category	IA [in]	R _{ump} [%]	Cover [%]	D _{theta} [cfs]	K _b [cfs]
120	Large Lot Residential – Single Family	0.30	5	30	Normal	Min
730	Passive Open Space	0.10	0	90	Normal	Max

Three soil types were identified for the onsite and offsite subbasins using the web soil survey from the National Resource Conservation Service (NRCS). Maps showing the soil type is shown in **Appendix A** and **Appendix B**.

The normal depth method was used for routes through the existing washes. Eight point cross sections are provided. The proposed model includes routes through the detention basins. Refer to **Appendix A** and **Appendix B**.

HYDRAULICS

The effective hydraulic model for Upper Boulders Wash from the NSFDS was done in HEC-RAS version 3.0. The hydraulic analysis for this report was completed in HEC-RAS 5.0. Differences in results between

the two versions are due to changes in culvert hydraulic modeling. Duplicate effective, corrected effective and post-project conditions model have been developed. The existing culvert under Hayden Road is included in all models. Additional cross sections and more detailed topography on the Site were included in the corrected effective model. Manning's n values for the channel and overbanks were maintained from the effective hydraulic model and not changed. The post-project conditions model includes grading changes associated with the Site. **Table 3** is a comparison of results between the duplicate effective and corrected effective models. Differences are due to more detailed topography on the Site. **Table 4** is a comparison of results between corrected effective and post-project models. Increases in the floodplain are on the Site. Refer to **Appendix C** for results from the HEC-RAS model. Refer to **Figure 5** for the proposed hydraulics map.

Table 3. Duplicate vs Corrected

Cross Section	Floodplain Duplicate Effective WSEL	Floodplain Corrected Effective WSEL	Difference [ft]	Floodway Duplicate Effective WSEL	Floodway Duplicate Effective WSEL	Difference [ft]
1.41	2417.57	2417.57	0.00	2418.05	2418.05	0.00
1.36	2411.65	2411.65	0.00	2411.57	2411.57	0.00
1.32	2407.77	2407.34	-0.43	2407.45	2407.32	-0.13
1.30	2405.83	2406.12	0.29	2405.86	2406.12	0.26
1.28	2403.58	2403.92	0.34	2404.15	2404.55	0.40
1.26	2401.33	2401.26	-0.07	2401.25	2402.00	0.75
1.23	2397.96	2398.04	0.08	2397.96	2399.06	1.10
1.22	2397.78	2396.98	-0.80	2397.95	2397.67	-0.28
1.18	2397.07	2397.07	0.00	2397.79	2397.79	0.00
1.16	2391.14	2391.14	0.00	2391.14	2391.14	0.00
1.14	2387.89	2387.89	0.00	2388.36	2388.36	0.00
1.05	2379.57	2379.57	0.00	2380.44	2380.44	0.00

Table 4. Corrected vs Post-Project

Cross Section	Floodplain Corrected Effective WSEL	Floodplain Post-Project WSEL	Difference [ft]	Floodway Corrected Effective WSEL	Floodway Post-Project WSEL	Difference [ft]
1.41	2417.57	2417.57	0.00	2418.05	2418.05	0.00
1.36	2411.65	2411.65	0.00	2411.57	2411.57	0.00
1.32	2407.34	2407.34	0.00	2407.32	2407.32	0.00
1.30	2406.12	2406.12	0.00	2406.12	2406.12	0.00
1.28	2403.92	2404.47	0.55	2404.55	2404.55	0.00
1.26	2401.26	2401.62	0.36	2402.00	2402.00	0.00
1.23	2398.04	2398.00	-0.04	2399.06	2399.06	0.00
1.22	2396.98	2396.98	0.00	2397.67	2397.67	0.00
1.18	2397.07	2397.07	0.00	2397.79	2397.79	0.00
1.16	2391.14	2391.14	0.00	2391.14	2391.14	0.00
1.14	2387.89	2387.89	0.00	2388.36	2388.36	0.00
1.05	2379.57	2379.57	0.00	2380.44	2380.44	0.00

Lateral erosion setbacks were determined based on the Arizona Department of Water Resources (ADWR) State Standard 5-96 (SS5-96) Level I Analysis. See **Table 5** for Erosion Setbacks. Scour walls will be used to revise the erosion setbacks along Upper Boulders Wash. See **Appendix C** for erosion setback calculations.

Table 5. Erosion Setbacks

Location	Q ₁₀₀ [cfs]	Calculated setback [ft]	Minimum Setback [ft]	Used Setback [ft]
Upper Boulders Wash	1628	40	20	40

PROPOSED ONSITE STORMWATER STORAGE

The existing property is a part of the ESLO. Based on a new City ordinances, a waiver will be obtained for any volume less than the 100-year, 2-hour volume. However, there is no waiver fee associated with the volumes that do not result in an increase in downstream runoff. See **Appendix E** for the stormwater waiver.

The outlet for the detention basin is elevated six (6) inches above the bottom of the basin. The volume below the outlet will treat the first flush runoff.

CONCLUSIONS

- Hydrologic models were prepared for the onsite and offsite runoff for the pre- and post-development conditions. Increases to the post-development peak discharges from the pre-development peak discharges existing the Site into Upper Boulders Wash are negligible. A detention basin is used to attenuate onsite runoff. The detention basin will also treat the first flush runoff below the opening invert.
- Upper Boulders Wash is a FEMA-delineated wash running through the Site. Pre- and post-development hydraulic models were prepared to ensure no impact to adjacent properties. Water surface elevations and post-development floodplain boundary were determined to establish lot pad elevations. A FEMA CLOMR application will be completed as part of the Preliminary Drainage Report. Lateral erosion setbacks have been set accordingly. The existing culvert at Hayden Road will not be impacted by the development.
- Onsite runoff will be conveyed in the streets to a small storm drain system that discharges into the detention basin. The storm drain system will be sized with the Final Design.
- Adjacent properties will be unaffected by the proposed development.

REFERENCES

- Arizona Department of Water Resources, *State Standard for Watercourse System Sediment Balance*, September 1996.
- Bentley Systems, Inc., *FlowMaster*, V8i, 2009.
- City of Scottsdale, *Design Standards and Policies Manual*, January 2010.
- Flood Control District of Maricopa County, *Drainage Design Manual for Maricopa County, Arizona – Hydrology*, revised 2013.
- Flood Control District of Maricopa County, *Drainage Design Manual for Maricopa County, Arizona – Hydraulics*, revised 2013.
- Flood Control District of Maricopa County, *Drainage Policies and Standards for Maricopa County, Arizona*, revised 2016.
- Flood Control District of Maricopa County, *North Scottsdale Floodplain Delineation Study*, April 2005.

Appendix A – Existing Conditions Hydrology

NSFDS Report

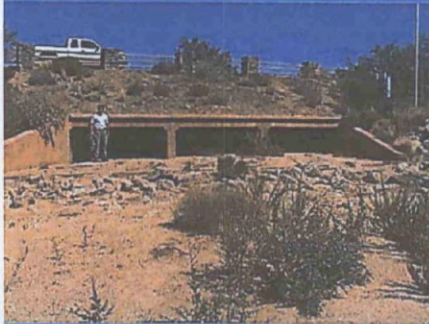
Existing Conditions Soils

Existing Conditions Land Use

Subbasin Hydrologic Parameters

Routing Reaches

HEC-1 Results



NORTH SCOTTSDALE FLOODPLAIN DELINEATION STUDY

Report and Technical Data Notebook

MAIN REPORT
VOLUME 3 of 4
FCD CONTRACT
NO. 2003C008

Submitted April 2005

PREPARED FOR:

Flood Control District of Maricopa County
2801 W. Durango Street
Phoenix, AZ 85009
(602) 506-1501

A FEMA Cooperative Technical Partner

IN CONJUNCTION WITH:

City of Scottsdale
3939 N. Drinkwater Boulevard
Scottsdale, AZ 85251

and

City of Carefree
100 Easy St.
Carefree, AZ 85377

PREPARED BY:

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6225 North 24th Street, Suite 200
Phoenix, AZ 85016



DEI Professional Services, LLC
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Soil Data

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Sub Basin ID	Soil Survey	Map Unit	Area	Area Pct (%)	XKSAT	Rock Outcrop (%)	Effective (%)
Major Basin 01							
SCP-14	Aguila/Carefree	33	0.003	5.3	0.23		
	Aguila/Carefree	61	0.043	69.0	0.15		
	Aguila/Carefree	96	0.013	20.3	0.07		
	Aguila/Carefree	33	0.003	5.3	0.23		
SCP-13	Aguila/Carefree	33	0.001	0.8	0.23		
	Aguila/Carefree	61	0.066	82.8	0.15		
	Aguila/Carefree	96	0.013	16.5	0.07		
SCP-12	Aguila/Carefree	33	0.015	51.6	0.23		
	Aguila/Carefree	96	0.014	48.4	0.07		
SCP-11	Aguila/Carefree	33	0.026	100.0	0.23		
SCP-10	Aguila/Carefree	33	0.068	61.5	0.23		
	Aguila/Carefree	61	0.014	12.3	0.15		
	Aguila/Carefree	96	0.029	26.2	0.07		
SCP-09	Aguila/Carefree	33	0.102	95.0	0.23		
	Aguila/Carefree	96	0.005	5.0	0.07		
SCP-08	Aguila/Carefree	6	0.000	0.1	0.40 *		
	Aguila/Carefree	33	0.127	99.9	0.23		
SCP-06	Aguila/Carefree	6	0.025	24.7	0.40 *		
	Aguila/Carefree	33	0.076	75.3	0.23		
SCP-03	Aguila/Carefree	6	0.018	23.4	0.40 *		
	Aguila/Carefree	33	0.032	42.7	0.23		
	Aguila/Carefree	93	0.026	33.9	0.33		
SCP-01	Aguila/Carefree	6	0.074	22.7	0.40 *		
	Aguila/Carefree	33	0.096	29.7	0.23		
	Aguila/Carefree	61	0.124	38.3	0.15		
	Aguila/Carefree	63	0.014	4.4	0.14	25.0	100
	Aguila/Carefree	93	0.016	4.9	0.33		
UB-12	Aguila/Carefree	33	0.059	100.0	0.23		
UB-11	Aguila/Carefree	33	0.201	97.5	0.23		
	Aguila/Carefree	96	0.005	2.5	0.07		
UB-10	Aguila/Carefree	33	0.090	96.2	0.23		
	Aguila/Carefree	96	0.004	3.8	0.07		
UB-09	Aguila/Carefree	33	0.159	46.5	0.23		
	Aguila/Carefree	61	0.054	15.8	0.15		
	Aguila/Carefree	96	0.129	37.7	0.07		
UB-07	Aguila/Carefree	33	0.012	6.3	0.23		
	Aguila/Carefree	61	0.179	93.5	0.15		
	Aguila/Carefree	96	0.000	0.2	0.07		
UB-04	Aguila/Carefree	6	0.001	0.2	0.40 *		
	Aguila/Carefree	33	0.018	5.1	0.23		
	Aguila/Carefree	61	0.260	74.2	0.15		

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Soil Data

Sub Basin ID	Soil Survey	Map Unit	Area	Area Pct (%)	XKSAT	Rock Outcrop (%)	Effective (%)
UB-04	Aguila/Carefree	63	0.072	20.6	0.14	25.0	100
UB-03	Aguila/Carefree	6	0.001	2.9	0.40 *		
	Aguila/Carefree	33	0.020	73.9	0.23		
	Aguila/Carefree	61	0.006	23.2	0.15		
UB-02	Aguila/Carefree	33	0.038	75.8	0.23		
	Aguila/Carefree	63	0.012	24.2	0.14	25.0	100
UB-01	Aguila/Carefree	6	0.022	41.5	0.40 *		
	Aguila/Carefree	33	0.031	58.5	0.23		
F6C-6	Aguila/Carefree	6	0.010	1.1	0.40 *		
	Aguila/Carefree	33	0.566	63.8	0.23		
	Aguila/Carefree	61	0.231	26.0	0.15		
	Aguila/Carefree	96	0.081	9.1	0.07		
F6C-5	Aguila/Carefree	33	0.137	100.0	0.23		
F6C-4	Aguila/Carefree	33	0.025	100.0	0.23		
F6C-3	Aguila/Carefree	33	0.044	94.8	0.23		
	Aguila/Carefree	96	0.002	5.2	0.07		
F6C-2	Aguila/Carefree	6	0.002	2.5	0.40 *		
	Aguila/Carefree	33	0.025	28.0	0.23		
	Aguila/Carefree	61	0.041	46.4	0.15		
	Aguila/Carefree	96	0.020	23.2	0.07		
F6AN-8	Aguila/Carefree	6	0.016	6.8	0.40 *		
	Aguila/Carefree	61	0.219	93.2	0.15		
F6AN-7	Aguila/Carefree	6	0.003	1.1	0.40 *		
	Aguila/Carefree	33	0.180	63.2	0.23		
	Aguila/Carefree	61	0.102	35.7	0.15		
F6AN-6	Aguila/Carefree	6	0.063	58.7	0.40 *		
	Aguila/Carefree	33	0.034	31.6	0.23		
	Aguila/Carefree	61	0.011	9.8	0.15		
F6AN-5	Aguila/Carefree	6	0.031	29.7	0.40 *		
	Aguila/Carefree	33	0.073	70.3	0.23		
F6AN-4	Aguila/Carefree	33	0.244	97.4	0.23		
	Aguila/Carefree	61	0.002	0.8	0.15		
	Aguila/Carefree	96	0.005	1.8	0.07		
F6AN-3	Aguila/Carefree	6	0.001	0.8	0.40 *		
	Aguila/Carefree	33	0.113	82.1	0.23		
	Aguila/Carefree	96	0.024	17.1	0.07		
F6AN-2	Aguila/Carefree	33	0.136	90.8	0.23		
	Aguila/Carefree	61	0.004	2.9	0.15		
	Aguila/Carefree	96	0.009	6.1	0.07		
	Aguila/Carefree	34	0.000	0.2	0.10 *		
F6AN-1	Aguila/Carefree	26	0.001	0.2	0.01		

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 Soil Data

Sub Basin ID	Soil Survey	Map Unit	Area	Area Pct (%)	XKSAT	Rock Outcrop (%)	Effective (%)
UF5-2	Aguilá/Carefree	61	0.415	66.5	0.15		
	Aguilá/Carefree	72	0.029	4.6	0.09	30.0	100
	Aguilá/Carefree	93	0.052	8.4	0.33		
	Aguilá/Carefree	96	0.001	0.1	0.07		
UF5-1	Aguilá/Carefree	6	0.022	26.7	0.40 *		
	Aguilá/Carefree	33	0.004	5.0	0.23		
	Aguilá/Carefree	61	0.057	68.2	0.15		
SCP-5C	Aguilá/Carefree	33	0.150	98.3	0.23		
	Aguilá/Carefree	96	0.003	1.7	0.07		
SCP-5B	Aguilá/Carefree	33	0.027	100.0	0.23		
SCP-5A	Aguilá/Carefree	6	0.001	1.0	0.40 *		
	Aguilá/Carefree	33	0.087	95.4	0.23		
	Aguilá/Carefree	93	0.003	3.6	0.33		
SCP-4B	Aguilá/Carefree	33	0.140	100.0	0.23		
SCP-4A	Aguilá/Carefree	6	0.034	45.7	0.40 *		
	Aguilá/Carefree	33	0.041	54.3	0.23		
SCP-2B	Aguilá/Carefree	6	0.011	20.1	0.40 *		
	Aguilá/Carefree	33	0.032	59.5	0.23		
	Aguilá/Carefree	93	0.011	20.4	0.33		
SCP-2A	Aguilá/Carefree	6	0.021	39.5	0.40 *		
	Aguilá/Carefree	33	0.028	51.8	0.23		
	Aguilá/Carefree	61	0.001	2.4	0.15		
	Aguilá/Carefree	93	0.003	6.3	0.33		
UB-08C	Aguilá/Carefree	33	0.009	97.9	0.23		
	Aguilá/Carefree	96	0.000	2.1	0.07		
UB-08B	Aguilá/Carefree	33	0.010	29.4	0.23		
	Aguilá/Carefree	96	0.024	70.6	0.07		
UB-08A	Aguilá/Carefree	33	0.074	61.3	0.23		
	Aguilá/Carefree	61	0.047	38.7	0.15		
UB-06C	Aguilá/Carefree	61	0.037	100.0	0.15		
UB-06B	Aguilá/Carefree	61	0.053	100.0	0.15		
UB-06A	Aguilá/Carefree	33	0.005	12.8	0.23		
	Aguilá/Carefree	61	0.037	87.2	0.15		
UB-05B	Aguilá/Carefree	61	0.022	100.0	0.15		
UB-05A	Aguilá/Carefree	61	0.089	100.0	0.15		
F6C-1D	Aguilá/Carefree	6	0.001	3.0	0.40 *		
	Aguilá/Carefree	61	0.038	97.0	0.15		
F6C-1C	Aguilá/Carefree	6	0.018	23.8	0.40 *		
	Aguilá/Carefree	61	0.059	76.2	0.15		
F6C-1B	Aguilá/Carefree	33	0.087	44.4	0.23		
	Aguilá/Carefree	61	0.109	55.6	0.15		

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 Sub Basin Data

Basin No.	Storms Multiple	Duration: 6 Hour	Loss Method: Green Ampt	Unit Hydrograph: Clark														
Sub Basin ID	Sub Basin Parameters					Rainfall Losses					Return Period (Years)							
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2	5	10	25	50	100	
												R (hrs)	0.47	0.37	0.33	0.30	0.28	0.27
SCP-03	0.08	0.71	114.2	114.2	Urban	0.032	0.30	0.25	4.50	0.37	17	Tc (hrs)	0.31	0.25	0.23	0.21	0.20	0.19
												Vel (f/s)	3.33	4.17	4.55	4.89	5.21	5.42
												R (hrs)	0.34	0.26	0.24	0.22	0.21	0.20
SCP-01	0.32	2.18	112.0	112.0	Urban	0.031	0.31	0.25	5.10	0.29	20	Tc (hrs)	0.69	0.55	0.48	0.42	0.38	0.36
												Vel (f/s)	4.65	5.86	6.68	7.67	8.35	8.93
												R (hrs)	0.87	0.67	0.58	0.50	0.45	0.42
UB-12	0.06	0.76	119.6	119.6	Natural	0.032	0.30	0.25	5.00	0.32	26	Tc (hrs)	0.30	0.25	0.23	0.21	0.20	0.19
												Vel (f/s)	3.67	4.46	4.87	5.23	5.57	5.81
												R (hrs)	0.40	0.32	0.29	0.27	0.25	0.24
UB-11	0.21	1.36	114.4	114.4	Natural	0.029	0.27	0.25	5.10	0.31	35	Tc (hrs)	0.43	0.35	0.31	0.28	0.26	0.25
												Vel (f/s)	4.69	5.76	6.37	7.15	7.58	8.11
												R (hrs)	0.45	0.36	0.32	0.28	0.26	0.25
UB-10	0.09	1.11	110.3	110.3	Natural	0.032	0.28	0.25	5.10	0.29	28	Tc (hrs)	0.41	0.33	0.30	0.27	0.25	0.24
												Vel (f/s)	3.99	4.95	5.50	6.10	6.51	6.84
												R (hrs)	0.57	0.45	0.40	0.36	0.33	0.31
UB-09	0.34	1.66	105.5	105.5	Urban	0.028	0.28	0.23	6.20	0.20	32	Tc (hrs)	0.48	0.39	0.35	0.32	0.30	0.28
												Vel (f/s)	5.08	6.21	6.88	7.68	8.23	8.73
												R (hrs)	0.45	0.36	0.32	0.29	0.27	0.25
UB-07	0.19	1.34	104.9	104.9	Natural	0.029	0.30	0.25	6.00	0.17	15	Tc (hrs)	0.44	0.35	0.32	0.28	0.27	0.25
												Vel (f/s)	4.45	5.55	6.20	6.94	7.36	7.74
												R (hrs)	0.49	0.38	0.34	0.30	0.28	0.26
UB-04	0.35	1.82	94.9	94.9	Urban	0.028	0.26	0.28	6.00	0.17	23	Tc (hrs)	0.55	0.45	0.40	0.35	0.33	0.31
												Vel (f/s)	4.82	5.99	6.67	7.54	8.02	8.53
												R (hrs)	0.56	0.44	0.39	0.34	0.32	0.30

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Sub Basin Data

Basin NS	Storms/Multiple	Duration	Loss Method	Green Ampt	Unit Hydrograph	Clark														
Sub Basin Parameters							Rainfall Losses					Return Period (Years)								
Sub Basin ID	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	Tc (hrs)	Vel (f/s)	R (hrs)	2	5	10	25	50	100
UB-03	0.03	0.37	106.0	106.0	Urban	0.034	0.18	0.30	5.20	0.26	23	Tc (hrs)	0.21	0.18	0.17	0.16	0.15	0.14		
												Vel (f/s)	2.61	2.97	3.17	3.43	3.62	3.82		
												R (hrs)	0.23	0.20	0.18	0.17	0.16	0.15		
UB-02	0.05	0.63	445.2	299.0	Urban	0.033	0.24	0.27	5.30	0.27	51	Tc (hrs)	0.19	0.17	0.15	0.15	0.14	0.13		
												Vel (f/s)	4.91	5.53	6.00	6.33	6.70	6.95		
												R (hrs)	0.22	0.19	0.18	0.17	0.16	0.15		
UB-01	0.05	0.60	100.8	100.8	Urban	0.033	0.20	0.27	4.55	0.39	47	Tc (hrs)	0.27	0.23	0.22	0.20	0.19	0.18		
												Vel (f/s)	3.30	3.84	4.06	4.40	4.58	4.81		
												R (hrs)	0.30	0.25	0.24	0.22	0.21	0.20		
F6C-6	0.89	3.88	162.0	162.0	Urban	0.027	0.33	0.25	5.40	0.25	14	Tc (hrs)	1.50	0.70	0.60	0.52	0.48	0.45		
												Vel (f/s)	3.67	8.08	9.42	10.92	11.98	12.76		
												R (hrs)	1.91	0.79	0.67	0.57	0.51	0.48		
F6C-5	0.14	1.71	117.5	117.5	Urban	0.031	0.32	0.25	5.00	0.33	22	Tc (hrs)	0.57	0.45	0.40	0.35	0.32	0.30		
												Vel (f/s)	4.42	5.57	6.33	7.25	7.81	8.36		
												R (hrs)	0.94	0.73	0.63	0.54	0.50	0.46		
F6C-4	0.03	0.48	137.8	137.8	Urban	0.036	0.31	0.25	5.00	0.33	23	Tc (hrs)	0.24	0.20	0.19	0.17	0.16	0.16		
												Vel (f/s)	2.96	3.52	3.74	4.12	4.32	4.46		
												R (hrs)	0.34	0.28	0.26	0.24	0.22	0.22		
F6C-3	0.05	0.63	64.8	64.8	Urban	0.034	0.33	0.25	5.10	0.33	24	Tc (hrs)	0.38	0.30	0.27	0.24	0.23	0.22		
												Vel (f/s)	2.44	3.08	3.46	3.82	4.03	4.18		
												R (hrs)	0.50	0.39	0.34	0.31	0.29	0.28		
F6C-2	0.09	0.83	108.2	108.2	Natural	0.031	0.29	0.25	6.00	0.21	29	Tc (hrs)	0.31	0.26	0.24	0.23	0.21	0.20		
												Vel (f/s)	3.89	4.72	5.03	5.41	5.72	5.97		
												R (hrs)	0.35	0.28	0.26	0.24	0.23	0.22		
F6AN-8	0.24	1.12	180.7	180.7	Natural	0.026	0.30	0.25	5.80	0.18	6	Tc (hrs)	0.28	0.24	0.22	0.20	0.19	0.18		
												Vel (f/s)	5.89	6.90	7.43	8.05	8.56	8.98		

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01131.12 - NORTH SCOTTSDALE DELINEATION STUDY
Sub Basin Data

Basin: NS Storms: Multiple Duration: 6 Hour Loss Method: Green Ampt Unit Hydrograph: Clark																		
Sub Basin ID	Sub Basin Parameters						Rainfall Losses					Return Period (Years)						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2	5	10	25	50	100	
SCP-2A	0.05	0.77	137.1	137.1	Urban	0.032	0.30	0.25	4.55	0.32	14	Tc (hrs)	0.30	0.24	0.23	0.21	0.20	0.19
												Vel (f/s)	3.76	4.67	5.02	5.43	5.76	6.01
												R (hrs)	0.42	0.33	0.30	0.28	0.26	0.25
UB-08C	0.01	0.24	118.6	118.6	Natural	0.035	0.25	0.25	5.00	0.32	36	Tc (hrs)	0.16	0.14	0.13	0.13	0.12	0.11
												Vel (f/s)	2.16	2.48	2.65	2.82	3.01	3.12
												R (hrs)	0.22	0.19	0.17	0.16	0.15	0.14
UB-08B	0.03	0.42	110.8	110.8	Natural	0.032	0.25	0.15	7.00	0.14	36	Tc (hrs)	0.20	0.18	0.17	0.16	0.15	0.14
												Vel (f/s)	3.08	3.44	3.69	3.90	4.11	4.34
												R (hrs)	0.22	0.19	0.18	0.17	0.16	0.15
UB-08A	0.12	1.34	93.6	93.6	Urban	0.031	0.30	0.25	5.30	0.24	14	Tc (hrs)	0.52	0.41	0.36	0.32	0.30	0.28
												Vel (f/s)	3.80	4.82	5.49	6.20	6.64	7.04
												R (hrs)	0.75	0.58	0.50	0.44	0.40	0.38
UB-06C	0.04	0.53	119.5	119.5	Urban	0.031	0.30	0.25	6.00	0.17	17	Tc (hrs)	0.23	0.20	0.19	0.17	0.16	0.16
												Vel (f/s)	3.45	3.89	4.13	4.55	4.77	4.92
												R (hrs)	0.28	0.24	0.23	0.21	0.19	0.19
UB-06B	0.05	0.90	115.1	115.1	Urban	0.032	0.30	0.25	6.00	0.17	18	Tc (hrs)	0.34	0.27	0.25	0.23	0.22	0.21
												Vel (f/s)	3.91	4.87	5.28	5.67	5.97	6.20
												R (hrs)	0.54	0.43	0.39	0.36	0.34	0.33
UB-06A	0.04	0.56	108.1	108.1	Urban	0.034	0.24	0.28	5.80	0.20	23	Tc (hrs)	0.25	0.22	0.21	0.19	0.18	0.18
												Vel (f/s)	3.23	3.72	3.95	4.28	4.49	4.69
												R (hrs)	0.31	0.27	0.25	0.23	0.22	0.21
UB-05B	0.02	0.33	116.6	116.6	Urban	0.033	0.30	0.25	6.00	0.17	17	Tc (hrs)	0.19	0.16	0.15	0.14	0.13	0.13
												Vel (f/s)	2.57	2.97	3.14	3.41	3.64	3.75
												R (hrs)	0.21	0.18	0.17	0.15	0.14	0.14
UB-05A	0.09	0.93	116.8	116.8	Urban	0.033	0.29	0.25	6.00	0.19	24	Tc (hrs)	0.35	0.28	0.26	0.24	0.23	0.22
												Vel (f/s)	3.90	4.82	5.29	5.73	6.06	6.29

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 01131.12 - NORTH SCOTTSDALE DELINEATION STUDY
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Basin	Reach ID	RS Card	RC Card					RX and RY Cards									
		NSTPS	ANL	ANCH	ANR	RLNTH (ft)	SEL (ft/ft)	ELMAX	1	2	LB	4	5	RB	7	8	
N S	RSCP14	1	0.064	0.044	0.064	1086	0.0322		Sta	812.0	844.1	955.0	977.5	1000.0	1009.0	1017.6	1031.5
									Elev	957.3	957.0	956.0	955.0	954.0	956.0	958.0	960.0
N S	RSCP12	4	0.064	0.044	0.064	3465	0.0268		Sta	870.5	892.9	951.4	993.0	1049.1	1067.0	1124.7	1164.5
									Elev	886.0	884.0	882.0	880.0	880.0	882.0	884.0	886.0
N S	RSCP10	5	0.064	0.044	0.064	4424	0.0255		Sta	814.2	853.2	899.7	949.5	1052.9	1130.7	1232.4	1257.1
									Elev	820.0	815.3	816.0	812.0	812.0	814.0	818.0	820.0
N S	RSCP09	3	0.064	0.044	0.064	2968	0.0230		Sta	865.1	910.6	932.1	961.1	1066.2	1091.4	1109.0	1125.3
									Elev	724.0	722.0	718.0	714.0	714.0	718.0	722.0	724.0
N S	RSCP08	3	0.064	0.044	0.064	2847	0.0204		Sta	850.9	901.3	954.4	979.4	1000.0	1062.3	1114.0	1201.2
									Elev	668.0	666.0	664.0	662.0	660.0	662.0	664.0	666.0
N S	RSCP07	4	0.056	0.038	0.076	3297	0.0179		Sta	857.9	951.0	966.1	972.2	1006.8	1087.4	1125.8	1266.4
									Elev	578.0	574.0	572.0	568.0	568.0	571.5	569.3	578.0
N S	RSCP06	4	0.064	0.044	0.064	3157	0.0196		Sta	627.2	683.7	977.3	987.0	1012.1	1029.7	1045.2	1079.6
									Elev	538.0	534.0	536.0	530.0	530.0	534.0	536.0	538.0
N S	RSCP04	2	0.056	0.038	0.086	1499	0.0180		Sta	850.6	870.5	971.1	987.9	1027.2	1050.7	1218.9	1240.9
									Elev	492.0	490.0	486.0	484.0	484.0	486.0	488.0	492.0
N S	RSCP03	2	0.061	0.038	0.056	1378	0.0181		Sta	966.2	984.0	990.6	997.6	1003.9	1019.0	1048.1	1065.0
									Elev	447.7	446.0	444.0	442.0	442.0	444.0	446.0	448.0
N S	RSCP02	7	0.056	0.038	0.056	5949	0.0212		Sta	623.1	687.8	904.9	959.7	1000.0	1016.9	1082.0	1122.3
									Elev	290.0	286.8	289.6	286.0	284.0	286.0	288.0	290.0
N S	RUB12	6	0.056	0.044	0.056	5388	0.0182		Sta	849.6	861.9	926.8	975.3	1017.2	1046.5	1060.3	1074.7
									Elev	650.0	648.0	648.0	642.0	642.0	646.0	648.0	650.0
N S	RUB10	4	0.056	0.044	0.056	3907	0.0179		Sta	810.4	844.8	959.8	972.8	1005.9	1035.5	1109.8	1164.3
									Elev	564.0	562.0	560.0	556.0	556.0	560.0	560.0	562.0
N S	RUB09	5	0.061	0.038	0.061	4450	0.0187		Sta	865.9	913.3	986.7	990.5	1009.0	1014.7	1037.1	1056.8
									Elev	492.0	489.0	488.0	486.0	486.0	488.0	490.0	492.0

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 Routing Data - Normal Depth

Basin	Reach ID	RS Card	RC Card					RX and RY Cards								
		NSTPS	ANL	ANCH	ANR	RLNTH (ft)	SEL (ft/ft)	ELMAX	1	2	LB	4	5	RB	7	8
N S	RUB07	2	0.061	0.038	0.061	1441	0.0187	Sta	802.8	896.1	939.4	977.6	1023.6	1036.2	1071.8	1144.8
								Elev	420.0	415.1	416.5	416.0	416.0	418.0	416.4	420.0
N S	RUB06	2	0.076	0.038	0.076	1436	0.0202	Sta	924.2	953.8	984.0	988.7	1020.6	1078.2	1087.4	1126.5
								Elev	336.0	334.0	332.0	330.0	330.0	332.0	334.0	336.0
N S	RUB05	2	0.076	0.038	0.076	1527	0.0216	Sta	924.2	953.8	984.0	988.7	1020.6	1078.2	1087.4	1126.5
								Elev	336.0	334.0	332.0	330.0	330.0	332.0	334.0	336.0
N S	RUB03	3	0.076	0.038	0.076	2425	0.0177	Sta	785.3	899.0	967.3	981.0	1004.3	1011.0	1258.0	1351.4
								Elev	312.0	306.6	308.0	306.0	306.0	308.0	310.0	312.0
N S	RF6C6	2	0.050	0.035	0.050	1780	0.0185	Sta	864.4	924.1	983.8	985.5	1015.1	1025.5	1034.0	1042.4
								Elev	626.0	625.0	624.0	622.0	622.0	624.0	625.0	626.0
N S	RF6C4	3	0.050	0.035	0.050	2707	0.0070	Sta	697.3	735.7	986.9	990.6	1012.9	1106.1	1185.2	1250.7
								Elev	596.0	592.0	592.0	590.0	590.0	592.0	594.0	596.0
N S	RF6C3	4	0.058	0.038	0.058	3172	0.0284	Sta	731.4	760.4	943.4	993.9	1019.8	1057.8	1074.8	1160.5
								Elev	552.0	550.0	548.0	548.0	548.0	550.0	550.0	552.0
N S	RF6C2	3	0.058	0.038	0.058	2486	0.0193	Sta	963.5	971.8	977.4	981.8	1009.3	1024.9	1079.5	1115.0
								Elev	464.0	462.0	460.0	458.0	458.0	460.0	462.0	462.4
N S	RF6AN8	8	0.056	0.039	0.056	6862	0.0331	Sta	942.1	950.5	958.9	988.7	1010.7	1028.3	1056.8	1085.2
								Elev	950.0	949.0	948.0	946.0	946.0	948.0	949.0	950.0
N S	RF6AN6	5	0.056	0.038	0.053	4132	0.0225	Sta	962.5	970.6	978.4	991.0	1006.8	1227.5	1256.1	1279.4
								Elev	806.0	804.0	802.0	800.0	800.0	802.0	804.0	806.0
N S	RF6AN5	3	0.056	0.038	0.053	3137	0.0217	Sta	978.0	983.5	989.4	1007.0	1029.9	1143.3	1284.4	1331.4
								Elev	732.0	730.0	728.0	728.0	729.3	729.0	726.7	732.0
N S	RF6AN3	1	0.056	0.038	0.053	876	0.0171	Sta	978.9	986.9	991.1	995.6	1017.5	1098.7	1195.1	1228.0
								Elev	690.0	688.0	686.0	684.0	684.0	686.0	688.0	690.0
N S	RF6AN2	27	0.056	0.038	0.053	7999	0.0176	Sta	897.4	913.8	927.5	971.8	1023.5	1071.1	1152.0	1224.7
								Elev	685.0	656.0	654.0	652.0	652.0	654.0	652.7	658.0

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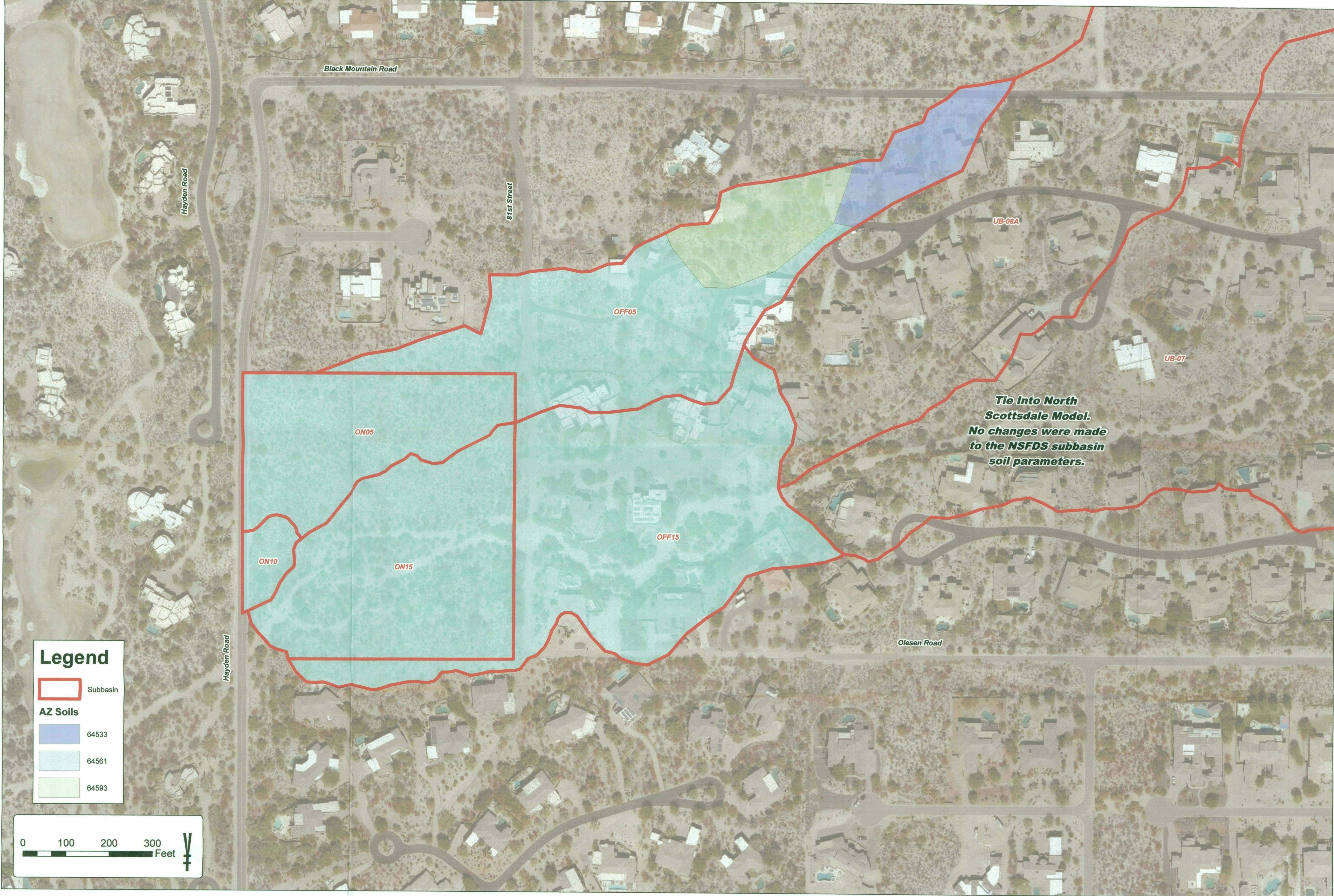
Basin	Reach ID	RS Card	RC Card					RX and RY Cards									
		NSTPS	ANL	ANCH	ANR	RLNTH (ft)	SEL (ft/ft)	ELMAX	1	2	LB	4	5	RB	7	8	
N S	RF6AN1	3	0.063	0.038	0.063	2859	0.0161		Sta	879.7	894.2	947.2	979.7	1018.2	1032.6	1044.2	1055.2
									Elev	540.0	538.0	536.0	534.0	534.0	536.0	538.0	540.0
N S	RF6AS5	9	0.064	0.044	0.064	8083	0.0192		Sta	877.0	887.9	915.7	986.3	1151.0	1210.3	1238.3	1261.5
									Elev	780.0	778.0	776.0	774.0	774.0	776.0	778.0	780.0
N S	RF6AS3	3	0.064	0.044	0.064	2438	0.0160		Sta	830.7	866.0	931.0	995.0	1017.3	1026.5	1076.2	1104.9
									Elev	652.0	650.0	648.0	646.0	646.0	648.0	650.0	652.0
N S	RF6AS2	7	0.064	0.044	0.064	5914	0.0167		Sta	850.1	874.8	889.0	953.7	1000.0	1016.4	1036.5	1067.1
									Elev	602.0	600.0	598.0	596.4	595.5	596.0	598.0	602.0
N S	RRHW1	9	0.068	0.043	0.068	8155	0.0180		Sta	766.3	889.6	954.5	973.4	1000.0	1007.8	1108.1	1188.8
									Elev	736.0	730.4	730.8	730.0	728.7	730.0	732.0	736.0
N S	RUF55	9	0.068	0.043	0.068	8318	0.0174		Sta	766.3	889.6	954.5	973.4	1000.0	1007.8	1108.1	1188.8
									Elev	736.0	730.4	730.8	730.0	728.7	730.0	732.0	736.0
N S	RUF54	13	0.071	0.038	0.071	11314	0.0175		Sta	931.9	946.9	961.7	982.9	1012.6	1048.2	1068.2	1079.1
									Elev	518.0	516.0	514.0	512.0	512.0	516.6	516.0	518.0
N S	RUF52	2	0.071	0.038	0.071	2113	0.0170		Sta	904.8	916.9	925.7	945.0	1017.8	1042.5	1074.4	1089.9
									Elev	420.0	418.0	416.0	414.0	414.0	416.0	418.0	420.0
N S	RSCP5C	3	0.064	0.044	0.064	2696	0.0185		Sta	903.8	943.5	972.7	996.7	1005.3	1068.4	1099.0	1112.6
									Elev	542.0	540.0	538.0	536.0	536.0	538.0	540.0	542.0
N S	RSCP5B	3	0.064	0.044	0.064	2870	0.0209		Sta	788.3	815.7	850.1	990.6	1008.8	1019.8	1037.3	1075.2
									Elev	522.0	520.0	518.0	516.0	516.0	518.0	520.0	522.0
N S	RSCP4B	4	0.064	0.044	0.064	3614	0.0183		Sta	627.2	683.7	977.3	987.0	1012.1	1029.7	1045.2	1079.6
									Elev	538.0	534.0	536.0	530.0	530.0	534.0	536.0	538.0
N S	RSCP2B	2	0.061	0.038	0.056	1827	0.0230		Sta	930.6	951.8	959.8	985.0	1023.8	1033.5	1198.5	1217.1
									Elev	414.0	412.0	410.0	408.0	408.0	410.0	412.0	412.7
N S	RUB08C	7	0.061	0.038	0.061	5952	0.0190		Sta	975.3	981.7	986.9	991.0	1009.2	1073.1	1118.9	1137.7
									Elev	498.0	496.0	494.0	492.0	492.0	494.0	496.0	498.0

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 Routing Data - Normal Depth

Basin	Reach ID	RS Card	RC Card					RX and RY Cards								
		NSTPS	ANL	ANCH	ANR	RLNTH (ft)	SEL (ft/ft)	ELMAX	1	2	LB	4	5	RB	7	8
N S	RUB08B	7	0.061	0.038	0.061	6656	0.0173	Sta	975.3	981.7	986.9	991.0	1009.2	1073.1	1118.9	1137.7
								Elev	498.0	496.0	494.0	492.0	492.0	494.0	496.0	498.0
N S	RUB05B	3	0.076	0.038	0.076	2325	0.0206	Sta	874.5	885.3	983.5	990.7	1012.0	1019.9	1042.6	1086.1
								Elev	370.1	370.0	368.0	366.0	366.0	368.0	369.0	369.5
N S	RUB06C	2	0.076	0.038	0.076	2022	0.0183	Sta	921.6	939.8	960.8	993.3	1002.3	1033.8	1053.5	1122.0
								Elev	366.0	364.0	362.0	360.0	360.0	362.0	364.0	366.0
N S	RUB06B	2	0.076	0.038	0.076	1719	0.0180	Sta	921.6	939.8	960.8	993.3	1002.3	1033.8	1053.5	1122.0
								Elev	366.0	364.0	362.0	360.0	360.0	362.0	364.0	366.0
N S	RF6C1B	2	0.058	0.038	0.058	1607	0.0162	Sta	963.5	971.8	977.4	981.8	1009.3	1024.9	1079.5	1115.0
								Elev	464.0	462.0	460.0	458.0	458.0	460.0	462.0	462.4
N S	RF6C1C	5	0.063	0.039	0.063	4158	0.0180	Sta	909.7	984.0	987.5	990.5	1009.6	1023.1	1039.2	1182.5
								Elev	446.0	444.0	442.0	440.0	440.0	442.0	444.0	445.2
N S	RF6C1D	4	0.063	0.039	0.063	3718	0.0169	Sta	717.1	850.7	987.4	994.7	1006.6	1013.0	1206.6	1234.5
								Elev	426.0	424.0	422.0	420.0	420.0	422.0	424.0	425.3
N S	RSCP7B	3	0.064	0.044	0.064	2275	0.0224	Sta	947.0	967.3	977.1	990.7	1013.1	1024.4	1047.5	1074.1
								Elev	646.0	644.0	642.0	640.0	640.0	642.0	644.0	646.0
N S	RSCP7C	1	0.064	0.044	0.064	821	0.0207	Sta	827.0	845.3	898.0	993.5	1008.9	1024.0	1042.1	1057.6
								Elev	640.0	638.0	636.0	634.0	634.0	636.0	638.0	640.0
N S	RSCP7D	1	0.064	0.044	0.064	217	0.0276	Sta	956.3	969.1	987.2	991.1	1012.9	1019.5	1030.2	1044.8
								Elev	620.0	618.0	616.0	614.0	614.0	616.0	618.0	619.0

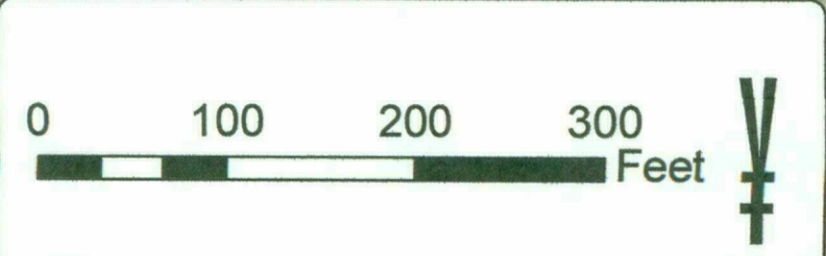
Flood Control District of Maricopa County
 Drainage Design Management System
 SOILS

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
OFF05	645	33	64533	0.002	16.10	0.230	-	100	
	645	61	64561	0.008	64.50	0.150	-	100	
	645	93	64593	0.002	19.40	0.330	-	100	
OFF15	645	61	64561	0.013	100.00	0.150	-	100	
ON05	645	61	64561	0.006	100.00	0.150	-	100	
ON10	645	61	64561	0.001	100.00	0.150	-	100	
ON15	645	61	64561	0.008	100.00	0.150	-	100	



Legend

- Subbasin
- AZ Soils**
- 64533
- 64561
- 64593



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<p>CITY OF SCOTTSDALE ESTATES AT HAYDEN EXISTING SOILS</p>	<p>PROJECT NO. 291109000</p> <p>DRAWING NAME EX SOILS</p>

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: ESTATES HAYDEN EX

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
OFF05	120	0.0124	100.0	0.30	5	30.0	NORMAL	0.034	Estate Residential (1/5 du per acre to 1 du per acre)
		0.0124	100.0						
OFF15	120	0.0125	93.3	0.30	5	30.0	NORMAL	0.034	Estate Residential (1/5 du per acre to 1 du per acre)
	730	0.0009	6.7	0.10	0	90.0	NORMAL	0.172	Passive Open Space (Includes mountain preserves and washes)
		0.0134	100.0						
ON05	730	0.0057	100.0	0.10	0	90.0	NORMAL	0.182	Passive Open Space (Includes mountain preserves and washes)
		0.0057	100.0						
ON10	730	0.0007	100.0	0.10	0	90.0	NORMAL	0.206	Passive Open Space (Includes mountain preserves and washes)
		0.0007	100.0						
ON15	730	0.0084	100.0	0.10	0	90.0	NORMAL	0.179	Passive Open Space (Includes mountain preserves and washes)
		0.0084	100.0						

* Non default value

184	KM	AT PIMA ROAD FLOW SPLIT.									
185	HC	2									
	*										
186	KK	SUB09									
187	RS	1	FLOW	1							
188	SA	0	0.01								
189	SE	2502	2520								
190	SQ	0	10000								
191	SE	2502	2520								
	*										
192	KK	DCUB09									
193	DR	DCUB09									
194	KK	DCUB09									
195	KM	COMBINE DCUB09, AND SUB09									
196	KM	FLOW SPLIT WEST OF PIMA ROAD FROM FAN 6C TO UPPER BOULDERS									
197	HC	2									
	*										
198	KK	RUB09	ROUTE	REACH							
199	RS	5	STOR	-1							
200	RC	0.061	0.038	0.061	4450	0.0187	0.00				
201	RX	865.9	913.3	986.7	990.5	1009.0	1014.7	1037.1	1056.8		
202	RY	492.0	489.0	488.0	486.0	486.0	488.0	490.0	492.0		
	*										
203	KK	UB-07	BASIN								
204	BA	0.191									
205	LG	0.30	0.15	8.80	0.06	16					
206	UC	0.440	0.490								
207	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
208	UA	100									
	*										

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

209	KK	CUB07									
210	KM	COMBINE UB-07, AND RUB09									
211	KM	CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.									
212	HC	2									
	*										
213	KK	UB-08C	BASIN								
214	BA	0.010									
215	LG	0.25	0.25	4.00	0.55	55					
216	UC	0.160	0.220								
217	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
218	UA	100									
	*										
219	KK	RUB08C	ROUTE	REACH							
220	RS	7	STOR	-1							
221	RC	0.061	0.038	0.061	5952	0.0190	0.00				
222	RX	975.3	981.7	986.9	991.0	1009.2	1073.1	1118.9	1137.7		
223	RY	498.0	496.0	494.0	492.0	492.0	494.0	496.0	498.0		
	*										
224	KK	UB-08B	BASIN								
225	BA	0.033									
226	LG	0.25	0.17	6.80	0.15	42					
227	UC	0.200	0.220								
228	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
229	UA	100									
	*										
230	KK	RUB08B	ROUTE	REACH							
231	RS	7	STOR	-1							
232	RC	0.061	0.038	0.061	6656	0.0173	0.00				
233	RX	975.3	981.7	986.9	991.0	1009.2	1073.1	1118.9	1137.7		
234	RY	498.0	496.0	494.0	492.0	492.0	494.0	496.0	498.0		
	*										
235	KK	UB-08A	BASIN								
236	BA	0.121									
237	LG	0.30	0.25	5.70	0.21	26					
238	UC	0.520	0.750								
239	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
240	UA	100									
	*										
241	KK	CUB08									
242	KM	COMBINE UB-08A, RUB08B, RUB08C, AND CUB07									
243	KM	CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.									
244	HC	4									
	*										

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

245	KK	RUB07	ROUTE	REACH						
246	RS	2	STOR	-1						

54	JD	1.600	0.0001								
55	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
56	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
57	PC	0.962	0.972	0.983	0.991	1.000					
58	JD	1.590	0.5000								
59	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
60	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
61	PC	0.962	0.972	0.983	0.991	1.000					
62	JD	1.560	2.8								
63	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076
64	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938
65	PC	0.950	0.963	0.975	0.988	1.000					

* END STAGE COACH PASS AND BEGIN FAN 6C

66	KK	F6C-6	BASIN								
67	BA	0.888									
68	LG	0.33	0.25	5.40	0.25	27					
69	UC	1.500	1.910								
70	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
71	UA	100									

72	KK	RF6C6	ROUTE	REACH							
73	RS	2	STOR	-1							
74	RC	0.050	0.035	0.050	1780	0.0185	0.00				
75	RX	864.4	924.1	983.8	985.5	1015.1	1025.5	1034.0	1042.4		
76	RY	626.0	625.0	624.0	622.0	622.0	624.0	625.0	626.0		

77	KK	F6C-4	BASIN								
78	BA	0.025									
79	LG	0.31	0.25	3.95	0.57	43					
80	UC	0.240	0.340								
81	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
82	UA	100									

83 KK CF6C4
 84 KM COMBINE F6C-4, AND RF6C6
 85 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 86 HC 2

87	KK	F6C-5	BASIN								
88	BA	0.137									
89	LG	0.32	0.25	3.95	0.58	42					
90	UC	0.570	0.940								
91	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
92	UA	100									

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

93 KK CF6C5
 94 KM COMBINE CF6C4, AND F6C-5
 95 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 96 HC 2

97	KK	RF6C4	ROUTE	REACH							
98	RS	3	STOR	-1							
99	RC	0.050	0.035	0.050	2707	0.0070	0.00				
100	RX	697.3	735.7	986.9	990.6	1012.9	1106.1	1185.2	1250.7		
101	RY	596.0	592.0	592.0	590.0	590.0	592.0	594.0	596.0		

102	KK	F6C-3	BASIN								
103	BA	0.046									
104	LG	0.33	0.25	4.20	0.53	43					
105	UC	0.380	0.500								
106	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
107	UA	100									

108 KK CF6C3
 109 KM COMBINE F6C-3, AND RF6C4
 110 KM EXITING LEGEND TRAIL ALONG WEST BOUNDARY.
 111 HC 2

112	KK	RF6C3	ROUTE	REACH							
113	RS	4	STOR	-1							
114	RC	0.058	0.038	0.058	3172	0.0284	0.00				
115	RX	731.4	760.4	943.4	993.9	1019.8	1057.8	1074.8	1160.5		
116	RY	552.0	550.0	548.0	548.0	548.0	550.0	550.0	552.0		

117	KK	F6C-2	BASIN								
118	BA	0.088									
119	LG	0.29	0.15	7.30	0.13	34					
120	UC	0.310	0.350								

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 26FEB18 TIME 09:16:20
*
*****
    
```

Existing 2-year Storm

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
    
```

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX
    
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

```

1
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID ESTATES AT HAYDEN EX - Estates at Hayden EX
3 ID 2 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: Clark
6 ID Storm: Multiple
7 ID 08/01/2017
8 ID
9 ID NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
10 ID MAY 2002
11 ID
12 ID FILE NAME: NS.DAT
13 ID THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
14 ID THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.
15 ID
16 ID 2-YEAR, 6-HOUR FREQUENCY
17 ID
18 ID METHODOLOGY:
19 ID THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
20 ID VERSION 4.1
21 ID MULTIPLE STORMS
22 ID CLARK UNIT HYDROGRAPH
23 ID GREEN AMPT LOSS METHOD
24 ID NORMAL DEPTH BASIN AND REACH ROUTING METHOD
25 ID
26 ID LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS
27 ID
28 ID PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
29 ID UPPER BOULDERS WASH (UB)
30 ID FAN 6C (F6C)
31 ID FAN 6A NORTH (F6AN)
32 ID FAN 6A SOULTH (F6AS)
33 ID FAN 6A (F6A)
34 ID UPPER FAN 5 (UF5)
35 ID
36 ID PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
37 ID NUMBER. I.E. STAGE COACH PASS SUB-BASIN 14 IS DESIGNATED AS SCP-14.
38 ID
39 ID PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
40 ID CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOSEN BY THE SUB
41 ID BASIN THE CONCENTRATION POINT IS LOCATED IN. I.E. CONCENTRATION POINT IN
42 ID SUB-BASIN 06 OF UPPER BOULDERS WASH IS DESIGNATED AS CUB06.
43 ID
44 ID PROJECT ROUTES ARE DESIGNATED AS R FOLLOWED BY SUB-BASIN NAME AND ROUTE
45 ID NUMBER. ROUTE NUMBER IS CHOSEN BY THE SUB-BASIN IN WHICH THE ROUTE BEGINS.
46 ID IE A ROUTE FROM SUB-BASIN 4 IN FAN 6C IS DESIGNATED AS RF6C4.
47 ID
48 ID
49 ID
50 ID
51 ID *DIAGRAM
52 ID IT 1 2000
53 ID IO 5
54 ID IN 15
55 ID *
    
```

HEC-1 INPUT

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1
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
    
```

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 ROUTING DATA
 Project Reference: ESTATES HAYDEN EX

Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)	1.	2.	3.	4.	5.	6.	7.	8.
NORMAL DEPTH														
Major Basin 01														
RON05	0.076	0.038	0.076	698.00	0.0186	-	X: 5.60	123.40	137.60	141.90	143.40	147.10	161.50	192.90
							Y: 2,412.22	2,403.48	2,402.24	2,400.40	2,400.46	2,401.89	2,402.86	2,404.84
RON15	0.076	0.038	0.073	722.00	0.0194	-	X: 7.20	66.20	275.00	300.30	349.70	364.10	401.50	491.90
							Y: 2,409.21	2,404.24	2,402.39	2,401.27	2,401.28	2,402.98	2,403.48	2,410.00

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr	
Major Basin ID: 01																		
OFF05	0.012	0.27	98.5	98.5	NATURAL	0.034	0.30	0.25	5.46	0.229	5	Tc (Hrs)	0.288	0.272	0.226	0.204	0.184	0.178
												Vel (f/s)	1.38	1.46	1.75	1.94	2.15	2.22
												R (Hrs)	0.406	0.380	0.309	0.277	0.246	0.238
OFF15	0.013	0.19	230.4	226.5	NATURAL	0.044	0.29	0.25	6.00	0.191	5	Tc (Hrs)	0.210	0.198	0.164*	0.149*	0.135*	0.131*
												Vel (f/s)	1.33	1.41	1.70	1.87	2.06	2.13
												R (Hrs)	0.206	0.193	0.157	0.141	0.126	0.122
ON05	0.006	0.16	144.7	144.7	NATURAL	0.182	0.10	0.25	6.00	0.284		Tc (Hrs)	0.483	0.454	0.376	0.340	0.306	0.296
												Vel (f/s)	0.49	0.52	0.62	0.69	0.77	0.79
												R (Hrs)	0.703	0.656	0.533	0.476	0.423	0.408
ON10	0.001	0.04	200.0	199.9	NATURAL	0.206	0.10	0.25	6.00	0.284		Tc (Hrs)	0.233	0.219	0.182	0.164*	0.147*	0.143*
												Vel (f/s)	0.25	0.27	0.32	0.36	0.40	0.41
												R (Hrs)	0.287	0.267	0.218	0.194	0.173	0.166
ON15	0.008	0.15	162.3	162.3	NATURAL	0.179	0.10	0.25	6.00	0.284		Tc (Hrs)	0.447	0.420	0.349	0.315	0.283	0.274
												Vel (f/s)	0.49	0.52	0.63	0.70	0.78	0.80
												R (Hrs)	0.520	0.486	0.395	0.353	0.313	0.302

* Non default value or value out of range


```
87      .      F6C-5
      .
93      CF6C5 .....
      V
      V
97      RF6C4
      .
102     .      F6C-3
      .
108     CF6C3 .....
      V
      V
112     RF6C3
      .
117     .      F6C-2
      .
123     CF6C2 .....
      V
      V
127     SF6C2
      .
137     -----> DCUB09
133     DCF6C2
      .
140     .      UB-12
      .      V
      .      V
146     .      RUB12
      .
151     .      .      UB-10
      .
157     .      CUB10 .....
      .
161     .      .      UB-11
      .
167     .      CUB11 .....
      .      V
171     .      RUB10
      .
176     .      .      UB-09
      .
182     .      CUB09 .....
      .      V
186     .      SUB09
      .
193     .      .      <----- DCUB09
192     .      DCUB09
      .
194     .      DCUB09 .....
      .      V
198     .      RUB09
      .
203     .      .      UB-07
      .
209     .      CUB07 .....
      .
213     .      .      UB-08C
      .      V
219     .      RUB08C
      .
224     .      .      .      UB-08B
      .      .      V
230     .      .      RUB08B
      .
235     .      .      .      .      UB-08A
      .
241     .      CUB08 .....
      .      V
245     .      RUB07
```

```
250 . . . . . OFF15
    . . . . .
256 . . . . . COFF15.....
    . . . . . V
    . . . . . V
258 . . . . . RON15
    . . . . .
263 . . . . . ON15
    . . . . .
269 . . . . . CPON15.....
    . . . . .
271 . . . . . OFF05
    . . . . . V
    . . . . . V
277 . . . . . RON05
    . . . . .
282 . . . . . ON05
    . . . . .
288 . . . . . CPON05.....
    . . . . .
290 . . . . . ON10
```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```
1*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 26FEB18 TIME 09:16:20 *
*****
```

```
*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
```

Flood Control District of Maricopa County
ESTATES AT HAYDEN EX - Estates at Hayden EX
2 YEAR
6 Hour Storm
Unit Hydrograph: Clark
Storm: Multiple
08/01/2017

NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
MAY 2002

FILE NAME: NS.DAT
THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.

2-YEAR, 6-HOUR FREQUENCY

METHODOLOGY:
THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
VERSION 4.1
MULTIPLE STORMS
CLARK UNIT HYDROGRAPH
GREEN AMPT LOSS METHOD
NORMAL DEPTH BASIN AND REACH ROUTING METHOD

LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS

PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
UPPER BOULDERS WASH (UB)
FAN 6C (F6C)
FAN 6A NORTH (F6AN)
FAN 6A SOULTH (F6AS)
FAN 6A (F6A)
UPPER FAN 5 (UF5)

PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
NUMBER. I.E. STAGE COACH PASS SUB-BASIN 14 IS DESIGNATED AS SCP-14.

PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOSEN BY THE SUB
BASIN THE CONCENTRATION POINT IS LOCATED IN. I.E. CONCENTRATION POINT IN
SUB-BASIN 06 OF UPPER BOULDERS WASH IS DESIGNATED AS CUB06.

PROJECT ROUTES ARE DESIGNATED AS R FOLLOWED BY SUB-BASIN NAME AND ROUTE
NUMBER. ROUTE NUMBER IS CHOSEN BY THE SUB-BASIN IN WHICH THE ROUTE BEGINS.
IE A ROUTE FROM SUB-BASIN 4 IN FAN 6C IS DESIGNATED AS RF6C4.

+		F6C-2	74.	4.20	9.	2.	2.	.09
+	2 COMBINED AT	CF6C2	155.	5.17	77.	20.	15.	1.18
+	ROUTED TO	SF6C2	155.	5.17	77.	20.	15.	1.18
+	DIVERSION TO	DCUB09	0.	.00	0.	0.	0.	1.18
+	HYDROGRAPH AT	DCF6C2	155.	5.17	77.	20.	15.	1.18
+	HYDROGRAPH AT	UB-12	38.	4.20	6.	1.	1.	.06
+	ROUTED TO	RUB12	26.	4.83	6.	1.	1.	.06
+	HYDROGRAPH AT	UB-10	51.	4.28	9.	2.	2.	.09
+	2 COMBINED AT	CUB10	59.	4.32	15.	4.	3.	.15
+	HYDROGRAPH AT	UB-11	133.	4.28	22.	6.	4.	.21
+	2 COMBINED AT	CUB11	191.	4.28	37.	9.	7.	.36
+	ROUTED TO	RUB10	178.	4.48	37.	9.	7.	.36
+	HYDROGRAPH AT	UB-09	213.	4.20	35.	9.	6.	.34
+	2 COMBINED AT	CUB09	322.	4.40	69.	17.	13.	.70
+	ROUTED TO	SUB09	322.	4.40	69.	17.	13.	.70
+	HYDROGRAPH AT	DCUB09	0.	.00	0.	0.	0.	1.18
+	2 COMBINED AT	DCUB09	322.	4.40	69.	17.	13.	.70
+	ROUTED TO	RUB09	316.	4.53	69.	17.	13.	.70
+	HYDROGRAPH AT	UB-07	128.	4.30	18.	5.	3.	.19
+	2 COMBINED AT	CUB07	393.	4.48	85.	22.	16.	.89
+	HYDROGRAPH AT	UB-08C	10.	4.08	1.	0.	0.	.01
+	ROUTED TO	RUB08C	6.	4.80	1.	0.	0.	.01
+	HYDROGRAPH AT	UB-08B	37.	4.12	4.	1.	1.	.03
+	ROUTED TO	RUB08B	20.	4.70	4.	1.	1.	.03
+	HYDROGRAPH AT	UB-08A	47.	4.27	10.	3.	2.	.12
+	4 COMBINED AT	CUB08	428.	4.50	97.	25.	18.	1.06
+	ROUTED TO	RUB07	427.	4.53	97.	25.	18.	1.06
+	HYDROGRAPH AT	OFF15	11.	4.12	1.	0.	0.	.01
+	2 COMBINED AT	COFF15	428.	4.53	97.	25.	18.	1.07
+	ROUTED TO	RON15	427.	4.55	97.	25.	18.	1.07
+	HYDROGRAPH AT	ON15	3.	4.30	0.	0.	0.	.01
+	2 COMBINED AT	CPON15	428.	4.55	97.	25.	18.	1.08
+	HYDROGRAPH AT							

+		OFF05	7.	4.18	1.	0.	0.	.01
	ROUTED TO							
+		RON05	6.	4.23	1.	0.	0.	.01
	HYDROGRAPH AT							
+		ON05	2.	4.33	0.	0.	0.	.01
	2 COMBINED AT							
+		CPON05	8.	4.27	1.	0.	0.	.02
	HYDROGRAPH AT							
+		ON10	1.	4.15	0.	0.	0.	.00

*** NORMAL END OF HEC-1 ***

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 26FEB18 TIME 09:16:36 *
*****
    
```

Existing 10-year Storm

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID ESTATES AT HAYDEN EX - Estates at Hayden EX
3 ID 10 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: Clark
6 ID Storm: Multiple
7 ID 08/01/2017
8 ID
9 ID NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
10 ID MAY 2002
11 ID
12 ID FILE NAME: NS.DAT
13 ID THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
14 ID THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.
15 ID
16 ID 10-YEAR, 6-HOUR FREQUENCY
17 ID
18 ID METHODOLOGY:
19 ID THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
20 ID VERSION 4.1
21 ID MULTIPLE STORMS
22 ID CLARK UNIT HYDROGRAPH
23 ID GREEN AMPT LOSS METHOD
24 ID NORMAL DEPTH BASIN AND REACH ROUTING METHOD
25 ID
26 ID LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS
27 ID
28 ID PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
29 ID UPPER BOULDERS WASH (UB)
30 ID FAN 6C (F6C)
31 ID FAN 6A NORTH (F6AN)
32 ID FAN 6A SOULTH (F6AS)
33 ID FAN 6A (F6A)
34 ID UPPER FAN 5 (UF5)
35 ID
36 ID PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
37 ID NUMBER. I.E. STAGE COACH PASS SUB-BASIN 14 IS DESIGNATED AS SCP-14.
38 ID
39 ID PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
40 ID CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOSEN BY THE SUB
41 ID BASIN THE CONCENTRATION POINT IS LOCATED IN. I.E. CONCENTRATION POINT IN
42 ID SUB-BASIN 06 OF UPPER BOULDERS WASH IS DESIGNATED AS CUB06.
43 ID
44 ID PROJECT ROUTES ARE DESIGNATED AS R FOLLOWED BY SUB-BASIN NAME AND ROUTE
45 ID NUMBER. ROUTE NUMBER IS CHOSEN BY THE SUB-BASIN IN WHICH THE ROUTE BEGINS.
46 ID IE A ROUTE FROM SUB-BASIN 4 IN FAN 6C IS DESIGNATED AS RF6C4.
47 ID
48 ID
49 ID
50 ID
51 ID *DIAGRAM
52 ID IT 1 2000
53 ID IO 5
54 ID IN 15
55 ID *
    
```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

54	JD	2.370	0.0001								
55	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
56	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
57	PC	0.962	0.972	0.983	0.991	1.000					
58	JD	2.356	0.5000								
59	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
60	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
61	PC	0.962	0.972	0.983	0.991	1.000					
62	JD	2.311	2.8								
63	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076
64	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938
65	PC	0.950	0.963	0.975	0.988	1.000					

* END STAGE COACH PASS AND BEGIN FAN 6C

66	KK	F6C-6	BASIN								
67	BA	0.888									
68	LG	0.33	0.25	5.40	0.25	27					
69	UC	0.600	0.670								
70	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
71	UA	100									

72	KK	RF6C6	ROUTE	REACH							
73	RS	2	STOR	-1							
74	RC	0.050	0.035	0.050	1780	0.0185	0.00				
75	RX	864.4	924.1	983.8	985.5	1015.1	1025.5	1034.0	1042.4		
76	RY	626.0	625.0	624.0	622.0	622.0	624.0	625.0	626.0		

77	KK	F6C-4	BASIN								
78	BA	0.025									
79	LG	0.31	0.25	3.95	0.57	43					
80	UC	0.190	0.260								
81	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
82	UA	100									

83 KK CF6C4
 84 KM COMBINE F6C-4, AND RF6C6
 85 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 86 HC 2

87	KK	F6C-5	BASIN								
88	BA	0.137									
89	LG	0.32	0.25	3.95	0.58	42					
90	UC	0.400	0.630								
91	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
92	UA	100									

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

93 KK CF6C5
 94 KM COMBINE CF6C4, AND F6C-5
 95 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 96 HC 2

97	KK	RF6C4	ROUTE	REACH							
98	RS	3	STOR	-1							
99	RC	0.050	0.035	0.050	2707	0.0070	0.00				
100	RX	697.3	735.7	986.9	990.6	1012.9	1106.1	1185.2	1250.7		
101	RY	596.0	592.0	592.0	590.0	590.0	592.0	594.0	596.0		

102	KK	F6C-3	BASIN								
103	BA	0.046									
104	LG	0.33	0.25	4.20	0.53	43					
105	UC	0.270	0.340								
106	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
107	UA	100									

108 KK CF6C3
 109 KM COMBINE F6C-3, AND RF6C4
 110 KM EXITING LENGEN TRAIL ALONG WEST BOUNDARY.
 111 HC 2

112	KK	RF6C3	ROUTE	REACH							
113	RS	4	STOR	-1							
114	RC	0.058	0.038	0.058	3172	0.0284	0.00				
115	RX	731.4	760.4	943.4	993.9	1019.8	1057.8	1074.8	1160.5		
116	RY	552.0	550.0	548.0	548.0	548.0	550.0	550.0	552.0		

117	KK	F6C-2	BASIN								
118	BA	0.088									
119	LG	0.29	0.15	7.30	0.13	34					
120	UC	0.240	0.260								

184	KM	AT PIMA ROAD FLOW SPLIT.									
185	HC	2									
	*										
186	KK	SUB09									
187	RS	1	FLOW	1							
188	SA	0	0.01								
189	SE	2502	2520								
190	SQ	0	10000								
191	SE	2502	2520								
	*										
192	KK	DCUB09									
193	DR	DCUB09									
194	KK	DCUB09									
195	KM	COMBINE DCUB09, AND SUB09									
196	KM	FLOW SPLIT WEST OF PIMA ROAD FROM FAN 6C TO UPPER BOULDERS									
197	HC	2									
	*										
198	KK	RUB09	ROUTE	REACH							
199	RS	5	STOR	-1							
200	RC	0.061	0.038	0.061	4450	0.0187	0.00				
201	RX	865.9	913.3	986.7	990.5	1009.0	1014.7	1037.1	1056.8		
202	RY	492.0	489.0	488.0	486.0	486.0	488.0	490.0	492.0		
	*										
203	KK	UB-07	BASIN								
204	BA	0.191									
205	LG	0.30	0.15	8.80	0.06	16					
206	UC	0.320	0.340								
207	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0 96.0	
208	UA	100									
	*										

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

209	KK	CUB07									
210	KM	COMBINE UB-07, AND RUB09									
211	KM	CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.									
212	HC	2									
	*										
213	KK	UB-08C	BASIN								
214	BA	0.010									
215	LG	0.25	0.25	4.00	0.55	55					
216	UC	0.130	0.170								
217	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0 96.0	
218	UA	100									
	*										
219	KK	RUB08C	ROUTE	REACH							
220	RS	7	STOR	-1							
221	RC	0.061	0.038	0.061	5952	0.0190	0.00				
222	RX	975.3	981.7	986.9	991.0	1009.2	1073.1	1118.9	1137.7		
223	RY	498.0	496.0	494.0	492.0	492.0	494.0	496.0	498.0		
	*										
224	KK	UB-08B	BASIN								
225	BA	0.033									
226	LG	0.25	0.17	6.80	0.15	42					
227	UC	0.170	0.180								
228	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0 96.0	
229	UA	100									
	*										
230	KK	RUB08B	ROUTE	REACH							
231	RS	7	STOR	-1							
232	RC	0.061	0.038	0.061	6656	0.0173	0.00				
233	RX	975.3	981.7	986.9	991.0	1009.2	1073.1	1118.9	1137.7		
234	RY	498.0	496.0	494.0	492.0	492.0	494.0	496.0	498.0		
	*										
235	KK	UB-08A	BASIN								
236	BA	0.121									
237	LG	0.30	0.25	5.70	0.21	26					
238	UC	0.360	0.500								
239	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0 97.0	
240	UA	100									
	*										
241	KK	CUB08									
242	KM	COMBINE UB-08A, RUB08B, RUB08C, AND CUB07									
243	KM	CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.									
244	HC	4									
	*										

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

245	KK	RUB07	ROUTE	REACH						
246	RS	2	STOR	-1						

247	RC	0.061	0.038	0.061	728	0.0247	0.00							
248	RX	802.8	896.1	939.4	977.6	1023.6	1036.2	1071.8	1144.8					
249	RY	420.0	415.1	416.5	416.0	416.0	418.0	416.4	420.0					
*														
250	KK	OFF15	BASIN											
251	BA	0.012												
252	LG	0.30	0.25	6.00	0.18	5								
253	UC	0.143	0.141											
254	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
255	UA	100												
*														
256	KK	COFF15												
257	HC	2												
*														
258	KK	RON15	ROUTE											
259	RS	1	FLOW											
260	RC	0.076	0.038	0.073	722	0.0194	0.00							
261	RX	7.20	66.20	275.00	300.30	349.70	364.10	401.50	491.90					
262	RY	2409.2	2404.24	2402.39	2401.27	2401.28	2402.98	2403.48	2410.00					
*														
263	KK	ON15	BASIN											
264	BA	0.009												
265	LG	0.10	0.25	6.00	0.28	0								
266	UC	0.347	0.367											
267	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
268	UA	100												
*														
269	KK	CPON15	COMBINE											
270	HC	2												
*														
271	KK	OFF05	BASIN											
272	BA	0.012												
273	LG	0.30	0.25	5.46	0.23	5								
274	UC	0.226	0.309											
275	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
276	UA	100												
*														
277	KK	RON05	ROUTE											
278	RS	1	FLOW											
279	RC	0.076	0.038	0.076	698	0.0186	0.00							
280	RX	5.60	123.40	137.60	141.90	143.40	147.10	161.50	192.90					
281	RY	2412.2	2403.48	2402.24	2400.40	2400.46	2401.89	2402.86	2404.84					
*														

1

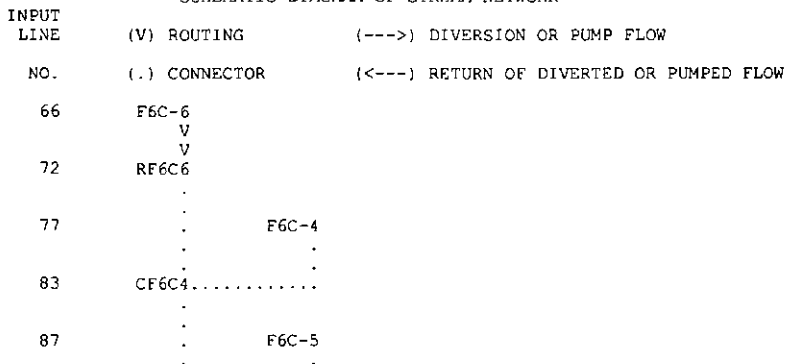
HEC-1 INPUT

PAGE 8

LINE	ID	1	2	3	4	5	6	7	8	9	10
282	KK	ON05	BASIN								
283	BA	0.006									
284	LG	0.10	0.25	6.00	0.28	0					
285	UC	0.376	0.533								
286	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
287	UA	100									
*											
288	KK	CPON05	COMBINE								
289	HC	2									
*											
290	KK	ON10	BASIN								
291	BA	0.001									
292	LG	0.10	0.25	6.00	0.28	0					
293	UC	0.182	0.218								
294	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
295	UA	100									
*											
296	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK



```
93   CF6C5 .....  
     V  
     V  
97   RF6C4  
     .  
102          F6C-3  
     .  
108   CF6C3 .....  
     V  
     V  
112   RF6C3  
     .  
117          F6C-2  
     .  
123   CF6C2 .....  
     V  
     V  
127   SF6C2  
     .  
137   -----> DCUB09  
133   DCF6C2  
     .  
140          UB-12  
     V  
     V  
146          RUB12  
     .  
151          .          UB-10  
     .  
157          CUB10 .....  
     .  
161          .          UB-11  
     .  
167          CUB11 .....  
     V  
     V  
171          RUB10  
     .  
176          .          UB-09  
     .  
182          CUB09 .....  
     V  
     V  
186          SUB09  
     .  
193          .          DCUB09 <----- DCUB09  
192          DCUB09  
     .  
194          DCUB09 .....  
     V  
     V  
198          RUB09  
     .  
203          .          UB-07  
     .  
209          CUB07 .....  
     .  
213          .          UB-08C  
     V  
     V  
219          .          RUB08C  
     .  
224          .          .          UB-08B  
     V  
     V  
230          .          .          RUB08B  
     .  
235          .          .          .          UB-08A  
     .  
241          CUB08 .....  
     V  
     V  
245          RUB07  
     .
```

```

250      .      .      OFF15
      .      .      .
256      .      COFF15.....
      .      V
      .      V
258      .      RON15
      .      .
263      .      .      ON15
      .      .
269      .      CPON15.....
      .      .
271      .      .      OFF05
      .      .      V
      .      .      V
277      .      .      RON05
      .      .
282      .      .      .      ON05
      .      .      .
288      .      .      CPON05.....
      .      .
290      .      .      .      ON10
    
```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   JUN 1998                   *
*   VERSION 4.1                 *
* RUN DATE 26FEB18 TIME 09:16:36 *
*****
    
```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
*   609 SECOND STREET          *
* DAVIS, CALIFORNIA 95616      *
* (916) 756-1104              *
*****
    
```

Flood Control District of Maricopa County
 ESTATES AT HAYDEN EX - Estates at Hayden EX
 10 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 08/01/2017

NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
 MAY 2002

FILE NAME: NS.DAT
 THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
 THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.

10-YEAR, 6-HOUR FREQUENCY

METHODOLOGY:
 THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
 VERSION 4.1
 MULTIPLE STORMS
 CLARK UNIT HYDROGRAPH
 GREEN AMPT LOSS METHOD
 NORMAL DEPTH BASIN AND REACH ROUTING METHOD

LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS

PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
 UPPER BOULDERS WASH (UB)
 FAN 6C (F6C)
 FAN 6A NORTH (F6AN)
 FAN 6A SOULTH (F6AS)
 FAN 6A (F6A)
 UPPER FAN 5 (UF5)

PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
 NUMBER. I.E. STAGE COACH PASS SUB-BASTN 14 IS DESIGNATED AS SCP-14.

PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
 CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOOSEN BY THE SUB
 BASIN THE CONCENTRATION POINT IS LOCATED IN. I.E. CONCENTRATION POINT IN
 SUB-BASIN 06 OF UPPER BOULDERS WASH IS DESIGNATED AS CUB06.

PROJECT ROUTES ARE DESIGNATED AS R FOLLOWED BY SUB-BASIN NAME AND ROUTE
 NUMBER. ROUTE NUMBER IS CHOOSEN BY THE SUB-BASIN IN WHICH THE ROUTE BEGINS.
 IE A ROUTE FROM SUB-BASIN 4 IN FAN 6C IS DESIGNATED AS RF6C4.

+	2 COMBINED AT	CF6C2	636.	4.67	158.	40.	29.	1.18
	ROUTED TO	SF6C2	636.	4.67	158.	40.	29.	1.18
+	DIVERSION TO	DCUB09	0.	.00	0.	0.	0.	1.18
+	HYDROGRAPH AT	DCF6C2	636.	4.67	158.	40.	29.	1.18
+	HYDROGRAPH AT	UB-12	81.	4.13	9.	2.	2.	.06
+	ROUTED TO	RUB12	53.	4.57	9.	2.	2.	.06
+	HYDROGRAPH AT	UB-10	109.	4.20	15.	4.	3.	.09
+	2 COMBINED AT	CUB10	121.	4.20	25.	6.	4.	.15
+	HYDROGRAPH AT	UB-11	275.	4.20	36.	9.	6.	.21
+	2 COMBINED AT	CUB11	396.	4.20	60.	15.	11.	.36
+	ROUTED TO	RUB10	367.	4.35	60.	15.	11.	.36
+	HYDROGRAPH AT	UB-09	458.	4.13	58.	15.	11.	.34
+	2 COMBINED AT	CUB09	673.	4.27	115.	29.	21.	.70
+	ROUTED TO	SUB09	673.	4.27	115.	29.	21.	.70
+	HYDROGRAPH AT	DCUB09	0.	.00	0.	0.	0.	1.18
+	2 COMBINED AT	DCUB09	673.	4.27	115.	29.	21.	.70
+	ROUTED TO	RUB09	646.	4.45	115.	29.	21.	.70
+	HYDROGRAPH AT	UB-07	273.	4.20	32.	8.	6.	.19
+	2 COMBINED AT	CUB07	789.	4.40	144.	36.	26.	.89
+	HYDROGRAPH AT	UB-08C	19.	4.07	2.	0.	0.	.01
+	ROUTED TO	RUB08C	10.	4.75	2.	0.	0.	.01
+	HYDROGRAPH AT	UB-08B	65.	4.08	6.	2.	1.	.03
+	ROUTED TO	RUB08B	41.	4.57	6.	2.	1.	.03
+	HYDROGRAPH AT	UB-08A	116.	4.15	18.	4.	3.	.12
+	4 COMBINED AT	CUB08	876.	4.42	167.	42.	30.	1.06
+	ROUTED TO	RUB07	875.	4.43	167.	42.	30.	1.06
+	HYDROGRAPH AT	OFF15	23.	4.03	1.	0.	0.	.01
+	2 COMBINED AT	COFF15	876.	4.43	168.	42.	31.	1.07
+	ROUTED TO	RON15	874.	4.47	168.	42.	31.	1.07
+	HYDROGRAPH AT	ON15	9.	4.13	1.	0.	0.	.01
+	2 COMBINED AT	CPON15	877.	4.45	169.	43.	31.	1.08
+	HYDROGRAPH AT	OFF05	15.	4.08	1.	0.	0.	.01

+	ROUTED TO	RON05	14.	4.12	1.	0.	0.	.01
+	HYDROGRAPH AT	ON05	5.	4.15	1.	0.	0.	.01
+	2 COMBINED AT	CPON05	19.	4.13	2.	1.	0.	.02
+	HYDROGRAPH AT	ON10	1.	4.05	0.	0.	0.	.00

*** NORMAL END OF HEC-1 ***

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 26FEB18 TIME 09:16:49
*
*****
    
```

Existing 100-year Storm

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
    
```

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX
    
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID ESTATES AT HAYDEN EX - Estates at Hayden EX
3 ID 100 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: Clark
6 ID Storm: Multiple
7 ID 08/01/2017
8 ID
9 ID NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
10 ID MAY 2002
11 ID
12 ID FILE NAME: NS.DAT
13 ID THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
14 ID THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.
15 ID
16 ID 100-YEAR, 6-HOUR FREQUENCY
17 ID
18 ID METHODOLOGY:
19 ID THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
20 ID VERSION 4.1
21 ID MULTIPLE STORMS
22 ID CLARK UNIT HYDROGRAPH
23 ID GREEN AMPT LOSS METHOD
24 ID NORMAL DEPTH BASIN AND REACH ROUTING METHOD
25 ID
26 ID LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS
27 ID
28 ID PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
29 ID UPPER BOULDERS WASH (UB)
30 ID FAN 6C (F6C)
31 ID FAN 6A NORTH (F6AN)
32 ID FAN 6A SOULTH (F6AS)
33 ID FAN 6A (F6A)
34 ID UPPER FAN 5 (UF5)
35 ID
36 ID PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
37 ID NUMBER. I.E. STAGE COACH PASS SUB-BASIN 14 IS DESIGNATED AS SCP-14.
38 ID
39 ID PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
40 ID CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOSEN BY THE SUB
41 ID BASIN THE CONCENTRATION POINT IS LOCATED IN. I.E. CONCENTRATION POINT IN
42 ID SUB-BASIN 06 OF UPPER BOULDERS WASH IS DESIGNATED AS CUB06.
43 ID
44 ID PROJECT ROUTES ARE DESIGNATED AS R FOLLOWED BY SUB-BASIN NAME AND ROUTE
45 ID NUMBER. ROUTE NUMBER IS CHOSEN BY THE SUB-BASIN IN WHICH THE ROUTE BEGINS.
46 ID IE A ROUTE FROM SUB-BASIN 4 IN FAN 6C IS DESIGNATED AS RF6C4.
47 ID
48 ID
49 ID
50 ID
51 ID *DIAGRAM
52 ID IT 1 2000
53 ID IO 5
54 ID IN 15
55 ID *
    
```

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
    
```

54	JD	3.50	0.01								
55	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
56	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
57	PC	0.962	0.972	0.983	0.991	1.000					
58	JD	3.477	0.50								
59	JD	3.412	2.80								
60	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076
61	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938
62	PC	0.950	0.963	0.975	0.988	1.000					
63	JD	3.227	16.0								
64	PC	0.000	0.015	0.020	0.030	0.048	0.063	0.076	0.090	0.105	0.119
65	PC	0.135	0.152	0.175	0.222	0.304	0.472	0.670	0.796	0.868	0.912
66	PC	0.946	0.960	0.973	0.987	1.000					
67	JD	2.842	90.0								
68	PC	0.000	0.021	0.035	0.051	0.071	0.087	0.105	0.125	0.143	0.160
69	PC	0.179	0.201	0.232	0.281	0.364	0.500	0.658	0.773	0.841	0.888
70	PC	0.927	0.945	0.964	0.982	1.000					
71	JD	1.995	500.0								
72	PC	0.000	0.024	0.043	0.059	0.078	0.098	0.119	0.141	0.162	0.186
73	PC	0.212	0.239	0.271	0.321	0.408	0.515	0.627	0.735	0.814	0.864
74	PC	0.907	0.930	0.954	0.977	1.000					

*DIAGRAM
 * END STAGE COACH PASS AND BEGIN FAN 6C
 *

75	KK	F6C-6	BASIN								
76	BA	0.888									
77	JG	0.33	0.25	5.40	0.25	27					
78	UC	0.438	0.469								
79	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
80	UA	100									

81	KK	RF6C6	ROUTE	REACH							
82	RS	2	STOR	-1							
83	RC	0.050	0.035	0.050	1780	0.0185	0.00				
84	RX	864.4	924.1	983.8	985.5	1015.1	1025.5	1034.0	1042.4		
85	RY	626.0	625.0	624.0	622.0	622.0	624.0	625.0	626.0		

86	KK	F6C-4	BASIN								
87	BA	0.025									
88	LG	0.31	0.25	3.95	0.57	43					
89	UC	0.158	0.217								
90	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
91	UA	100									

92 KK CF6C4
 93 KM COMBINE F6C-4, AND RF6C6
 94 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 95 HC 2
 *

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
96	KK	F6C-5	BASIN								
97	BA	0.137									
98	LG	0.32	0.25	3.95	0.58	42					
99	UC	0.300	0.464								
100	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
101	UA	100									
102	KK	CF6C5									
103	KM	COMBINE CF6C4, AND F6C-5									
104	KM	CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.									
105	HC	2									
106	KK	RF6C4	ROUTE	REACH							
107	RS	3	STOR	-1							
108	RC	0.050	0.035	0.050	2707	0.0070	0.00				
109	RX	697.3	735.7	986.9	990.6	1012.9	1106.1	1185.2	1250.7		
110	RY	596.0	592.0	592.0	590.0	590.0	592.0	594.0	596.0		
111	KK	F6C-3	BASIN								
112	BA	0.046									
113	LG	0.33	0.25	4.20	0.53	43					
114	UC	0.221	0.278								
115	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
116	UA	100									
117	KK	CF6C3									
118	KM	COMBINE F6C-3, AND RF6C4									
119	KM	EXITING LENGEN TRAIL ALONG WEST BOUNDARY.									
120	HC	2									
121	KK	RF6C3	ROUTE	REACH							
122	RS	4	STOR	-1							

186 BA 0.342
 187 LG 0.28 0.23 6.20 0.20 41
 188 UC 0.279 0.248
 189 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 190 UA 100
 *

191 KK CUB09
 192 KM COMBINE UB-09, AND RUB10
 193 KM AT PIMA ROAD FLOW SPLIT.
 194 HC 2
 *

195 KK SUB09
 196 RS 1 FLOW 1
 197 SA 0 0.01
 198 SE 2502 2520
 199 SQ 0 10000
 200 SE 2502 2520
 *

201 KK DCUB09
 202 DR DCUB09

203 KK DCUB09
 204 KM COMBINE DCUB09, AND SUB09
 205 KM FLOW SPLIT WEST OF PIMA ROAD FROM FAN 6C TO UPPER BOULDERS
 206 HC 2
 *

HEC-1 INPUT

PAGE 6

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

207 KK RUB09 ROUTE REACH
 208 RS 5 STOR -1
 209 RC 0.061 0.038 0.061 4450 0.0187 0.00
 210 RX 865.9 913.3 986.7 990.5 1009.0 1014.7 1037.1 1056.8
 211 RY 492.0 489.0 488.0 486.0 486.0 488.0 490.0 492.0
 *

212 KK UB-07 BASIN
 213 BA 0.191
 214 LG 0.30 0.15 8.80 0.06 16
 215 UC 0.250 0.257
 216 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 217 UA 100
 *

218 KK CUB07
 219 KM COMBINE UB-07, AND RUB09
 220 KM CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.
 221 HC 2
 *

222 KK UB-08C BASIN
 223 BA 0.010
 224 LG 0.25 0.25 4.00 0.55 55
 225 UC 0.113 0.143
 226 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 227 UA 100
 *

228 KK RUB08C ROUTE REACH
 229 RS 7 STOR -1
 230 RC 0.061 0.038 0.061 5952 0.0190 0.00
 231 RX 975.3 981.7 986.9 991.0 1009.2 1073.1 1118.9 1137.7
 232 RY 498.0 496.0 494.0 492.0 492.0 494.0 496.0 498.0
 *

233 KK UB-08B BASIN
 234 BA 0.033
 235 LG 0.25 0.17 6.80 0.15 42
 236 UC 0.142 0.147
 237 UA 0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0
 238 UA 100
 *

239 KK RUB08B ROUTE REACH
 240 RS 7 STOR -1
 241 RC 0.061 0.038 0.061 6656 0.0173 0.00
 242 RX 975.3 981.7 986.9 991.0 1009.2 1073.1 1118.9 1137.7
 243 RY 498.0 496.0 494.0 492.0 492.0 494.0 496.0 498.0
 *

HEC-1 INPUT

PAGE 7

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

244 KK UB-08A BASIN
 245 BA 0.121
 246 LG 0.30 0.25 5.70 0.21 26
 247 UC 0.275 0.371
 248 UA 0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
 249 UA 100
 *

250	KK	CUB08										
251	KM	COMBINE UB-08A, RUB08B, RUB08C, AND CUB07										
252	KM	CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.										
253	HC	4										
	*											
254	KK	RUB07	ROUTE	REACH								
255	RS	2	STOR	-1								
256	RC	0.061	0.038	0.061	728	0.0247	0.00					
257	RX	802.8	896.1	939.4	977.6	1023.6	1036.2	1071.8	1144.8			
258	RY	420.0	415.1	416.5	416.0	416.0	418.0	416.4	420.0			
	*											
	*	Start of Estates at Hayden										
	*											
259	KK	OFF15	BASIN									
260	BA	0.013										
261	LG	0.29	0.25	6.00	0.19	5						
262	UC	0.131	0.122									
263	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
264	UA	100										
	*											
265	KK	COFF15										
266	HC	2										
	*											
267	KK	RON15	ROUTE									
268	RS	1	FLOW									
269	RC	0.076	0.038	0.073	722	0.0194	0.00					
270	RX	7.20	66.20	275.00	300.30	349.70	364.10	401.50	491.90			
271	RY	2409.2	2404.24	2402.39	2401.27	2401.28	2402.98	2403.48	2410.00			
	*											
272	KK	ON15	BASIN									
273	BA	0.008										
274	LG	0.10	0.25	6.00	0.28	0						
275	UC	0.274	0.302									
276	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
277	UA	100										
	*											

HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

278	KK	CPON15	COMBINE									
279	HC	2										
	*											
280	KK	OFF05	BASIN									
281	BA	0.012										
282	LG	0.30	0.25	5.46	0.23	5						
283	UC	0.178	0.238									
284	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
285	UA	100										
	*											
286	KK	RON05	ROUTE									
287	RS	1	FLOW									
288	RC	0.076	0.038	0.076	698	0.0186	0.00					
289	RX	5.60	123.40	137.60	141.90	143.40	147.10	161.50	192.90			
290	RY	2412.2	2403.48	2402.24	2400.40	2400.46	2401.89	2402.86	2404.84			
	*											
291	KK	ON05	BASIN									
292	BA	0.006										
293	LG	0.10	0.25	6.00	0.28	0						
294	UC	0.296	0.408									
295	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
296	UA	100										
	*											
297	KK	CPON05	COMBINE									
298	HC	2										
	*											
299	KK	ON10	BASIN									
300	BA	0.001										
301	LG	0.10	0.25	6.00	0.28	0						
302	UC	0.143	0.166									
303	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
304	UA	100										
	*											
305	ZZ											

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW
 75 F6C-6
 V
 V

81	RF6C6		
86		F6C-4	
92	CF6C4		
96		F6C-5	
102	CF6C5		
	V		
106	RF6C4		
111		F6C-3	
117	CF6C3		
	V		
121	RF6C3		
126		F6C-2	
132	CF6C2		
	V		
136	SF6C2		
146			DCUB09
142	DCF6C2		
149		UB-12	
		V	
		V	
155		RUB12	
160			UB-10
166		CUB10	
170			UB-11
176		CUB11	
		V	
		V	
180		RUB10	
185			UB-09
191		CUB09	
		V	
		V	
195		SUB09	
202			DCUB09
201		DCUB09	
203		DCUB09	
		V	
		V	
207		RUB09	
212			UB-07
218		CUB07	
222			UB-08C
		V	
		V	
228		RUB08C	
233			UB-08B
		V	
		V	
239			RUB08B

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+		F6C-6	1251.	4.18	210.	53.	38.	.89	
+	ROUTED TO								
+		RF6C6	1236.	4.25	210.	53.	38.	.89	
+	HYDROGRAPH AT								
+		F6C-4	65.	4.05	6.	2.	1.	.03	
+	2 COMBINED AT								
+		CF6C4	1261.	4.25	215.	54.	39.	.91	
+	HYDROGRAPH AT								
+		F6C-5	222.	4.12	34.	9.	6.	.14	
+	2 COMBINED AT								
+		CF6C5	1399.	4.23	246.	62.	45.	1.05	
+	ROUTED TO								
+		RF6C4	1317.	4.43	246.	62.	45.	1.05	
+	HYDROGRAPH AT								
+		F6C-3	103.	4.07	12.	3.	2.	.05	
+	2 COMBINED AT								
+		CF6C3	1346.	4.43	256.	64.	46.	1.10	
+	ROUTED TO								
+		RF6C3	1324.	4.55	256.	64.	46.	1.10	
+	HYDROGRAPH AT								
+		F6C-2	248.	4.12	25.	6.	5.	.09	
+	2 COMBINED AT								
+		CF6C2	1369.	4.53	278.	70.	51.	1.18	
+	ROUTED TO								
+		SF6C2	1369.	4.53	278.	70.	51.	1.18	
+	DIVERSION TO								
+		DCUB09	24.	4.53	1.	0.	0.	1.18	
+	HYDROGRAPH AT								
+		DCF6C2	1345.	4.53	277.	70.	50.	1.18	
+	HYDROGRAPH AT								
+		UB-12	146.	4.12	15.	4.	3.	.06	
+	ROUTED TO								
+		RUB12	112.	4.42	15.	4.	3.	.06	
+	HYDROGRAPH AT								
+		UB-10	206.	4.15	25.	6.	5.	.09	
+	2 COMBINED AT								
+		CUB10	238.	4.32	40.	10.	7.	.15	
+	HYDROGRAPH AT								
+		UB-11	508.	4.15	57.	14.	10.	.21	
+	2 COMBINED AT								
+		CUB11	735.	4.15	97.	24.	18.	.36	
+	ROUTED TO								
+		RUB10	691.	4.27	97.	24.	18.	.36	
+	HYDROGRAPH AT								
+		UB-09	848.	4.10	94.	24.	17.	.34	
+	2 COMBINED AT								
+		CUB09	1270.	4.18	188.	47.	34.	.70	
+	ROUTED TO								
+		SUB09	1270.	4.18	188.	47.	34.	.70	
+	HYDROGRAPH AT								
+		DCUB09	24.	4.53	1.	0.	0.	1.18	
+	2 COMBINED AT								
+		DCUB09	1270.	4.18	190.	48.	34.	.70	
+	ROUTED TO								
+		RUB09	1217.	4.35	190.	48.	34.	.70	
+	HYDROGRAPH AT								
+		UB-07	496.	4.15	53.	13.	10.	.19	

+	2 COMBINED AT	CUB07	1459.	4.32	239.	60.	43.	.89
	HYDROGRAPH AT	UB-08C	32.	4.05	3.	1.	1.	.01
+	ROUTED TO	RUB08C	16.	4.60	3.	1.	1.	.01
	HYDROGRAPH AT	UB-08B	108.	4.07	10.	2.	2.	.03
+	ROUTED TO	RUB08B	75.	4.43	10.	2.	2.	.03
	HYDROGRAPH AT	UB-08A	238.	4.10	30.	8.	5.	.12
+	4 COMBINED AT	CUB08	1629.	4.32	278.	70.	51.	1.06
	ROUTED TO	RUB07	1626.	4.35	278.	70.	51.	1.06
	HYDROGRAPH AT	OFF15	42.	4.05	3.	1.	1.	.01
+	2 COMBINED AT	COFF15	1632.	4.33	281.	71.	51.	1.07
	ROUTED TO	RON15	1625.	4.37	281.	71.	51.	1.07
	HYDROGRAPH AT	ON15	17.	4.17	2.	0.	0.	.01
+	2 COMBINED AT	CPON15	1632.	4.37	282.	71.	51.	1.08
	HYDROGRAPH AT	OFF05	30.	4.10	3.	1.	0.	.01
	ROUTED TO	RON05	29.	4.13	3.	1.	0.	.01
	HYDROGRAPH AT	ON05	10.	4.18	1.	0.	0.	.01
+	2 COMBINED AT	CPON05	39.	4.13	4.	1.	1.	.02
	HYDROGRAPH AT	ON10	3.	4.07	0.	0.	0.	.00

*** NORMAL END OF HEC-1 ***

Appendix B – Proposed Conditions Hydrology

Proposed Conditions Soils

Proposed Conditions Land Use

Subbasin Hydrologic Parameters

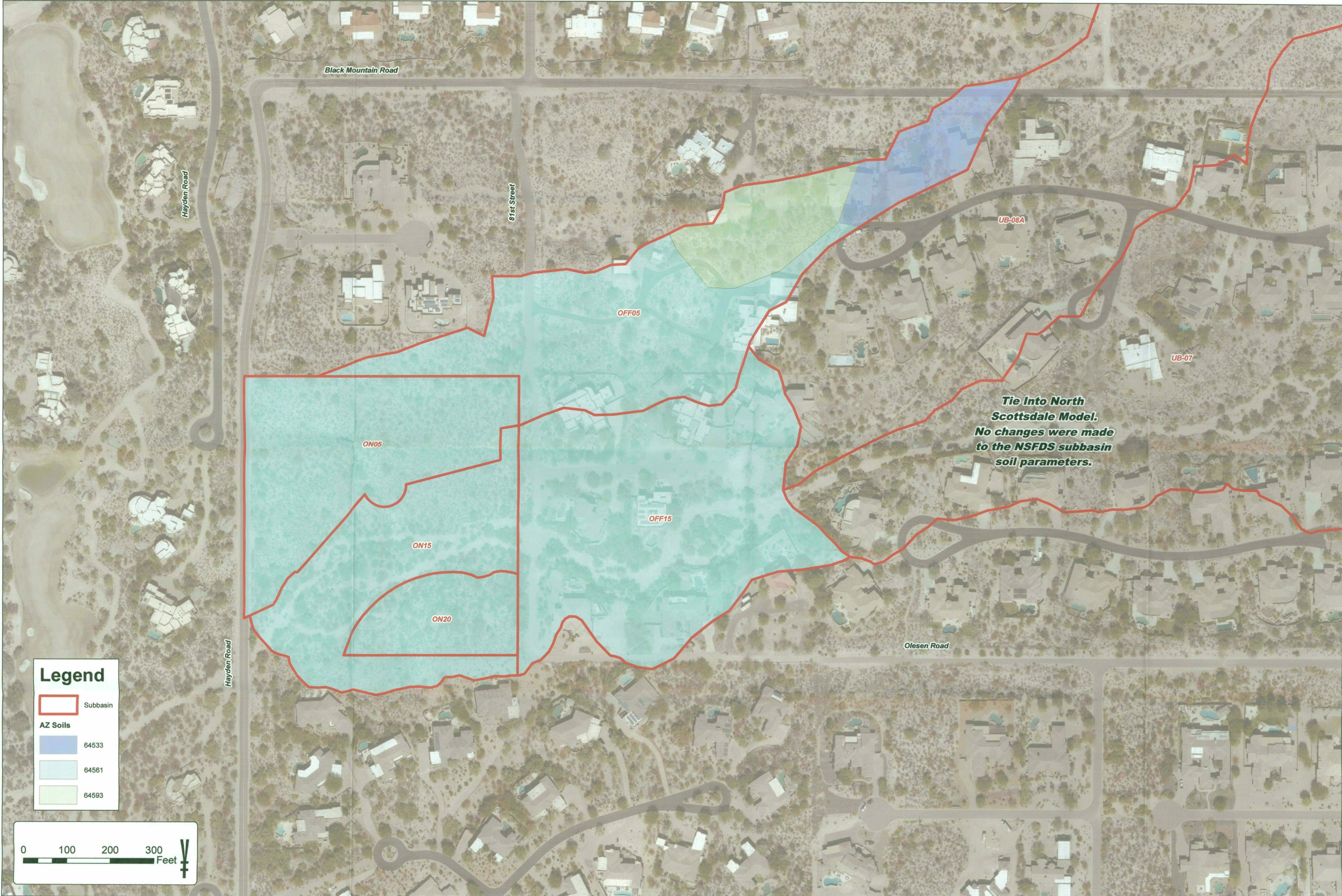
Routing Reaches

Storage Facilities

HEC-1 Results

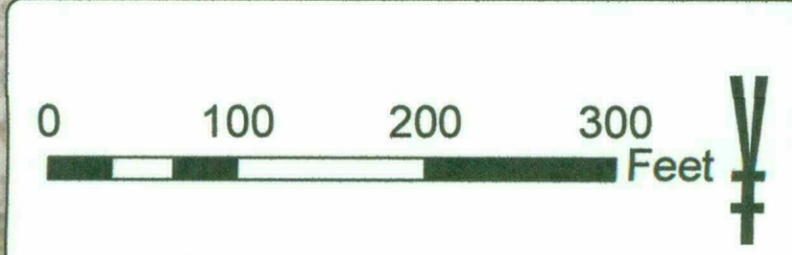
Flood Control District of Maricopa County
 Drainage Design Management System
 SOILS

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
OFF05			64533	0.002	16.10	0.230	- *	100	
			64561	0.008	64.50	0.150	- *	100	
			64593	0.002	19.40	0.330	- *	100	
OFF15			64561	0.013	100.00	0.150	- *	100	
ON05			64561	0.007	100.00	0.150	- *	100	
ON15			64561	0.006	100.00	0.150	- *	100	
ON20			64561	0.002	100.00	0.150	- *	100	



Legend

- Subbasin
- AZ Soils**
- 64533
- 64561
- 64593

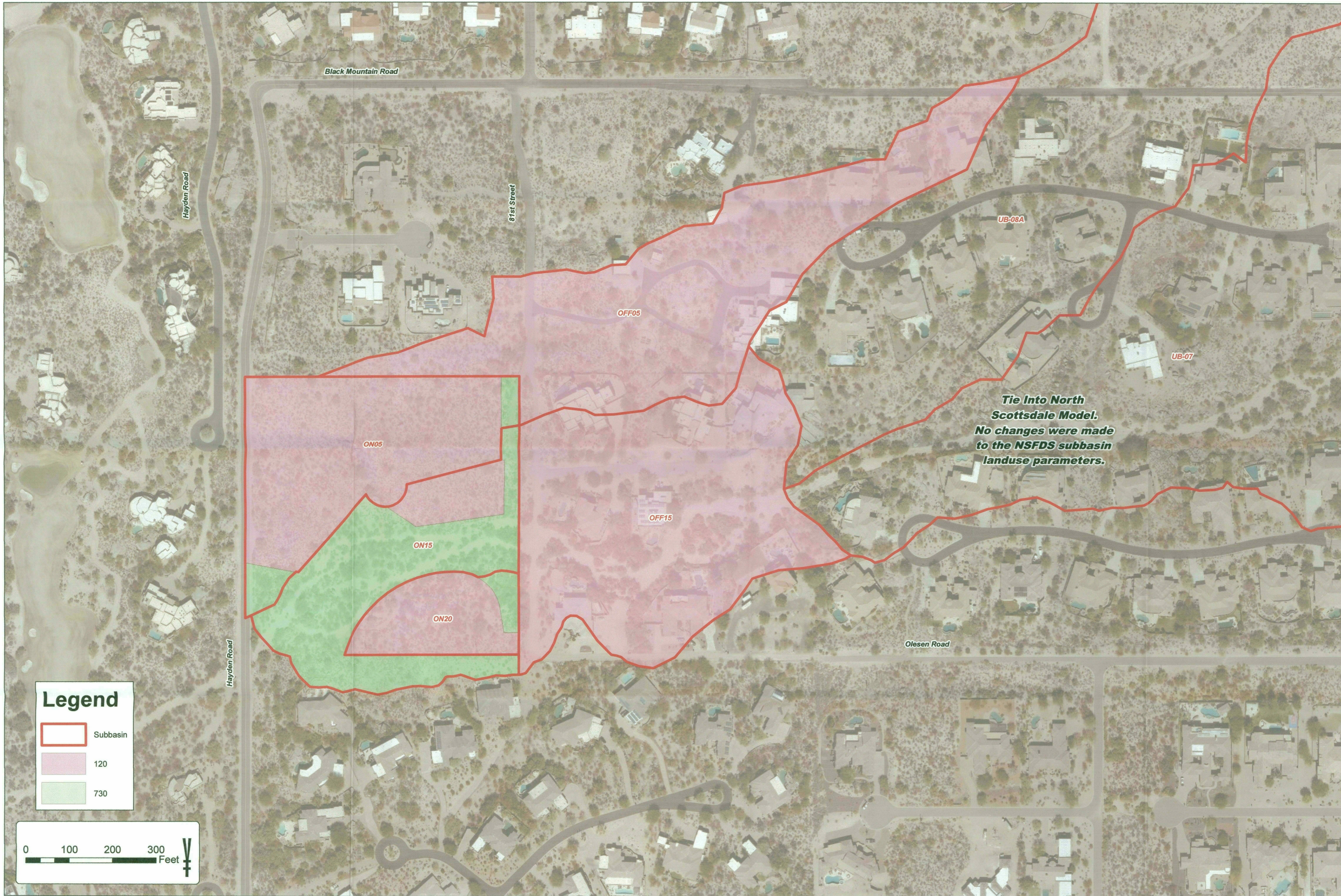


<p>Kimley & Horn</p> <p>© 2018 KIMLEY-HORN AND ASSOCIATES, INC. Engineering, Planning and Environmental Consultants 7740 North 16th Street, Suite 300 Phoenix, Arizona 85020 (602) 944-5500</p>	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>SCALE(H): 1" = 100'</td> <td>SCALE(V): N/A</td> </tr> <tr> <td>DESIGNED BY: ZRS</td> <td>DRAWN BY: BAH</td> </tr> <tr> <td>CHECKED BY: ZRS</td> <td>DATE: 03/2018</td> </tr> </table>	SCALE(H): 1" = 100'	SCALE(V): N/A	DESIGNED BY: ZRS	DRAWN BY: BAH	CHECKED BY: ZRS	DATE: 03/2018
SCALE(H): 1" = 100'	SCALE(V): N/A						
DESIGNED BY: ZRS	DRAWN BY: BAH						
CHECKED BY: ZRS	DATE: 03/2018						
<p>CITY OF SCOTTSDALE ESTATES AT HAYDEN PROPOSED SOILS</p>	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td>PROJECT NO. 291109000</td> </tr> <tr> <td>DRAWING NAME PR SOILS</td> </tr> </table>	PROJECT NO. 291109000	DRAWING NAME PR SOILS				
PROJECT NO. 291109000							
DRAWING NAME PR SOILS							

Flood Control District of Maricopa County
 Drainage Design Management System
 LAND USE
 Project Reference: ESTATES AT HAYDEN PR

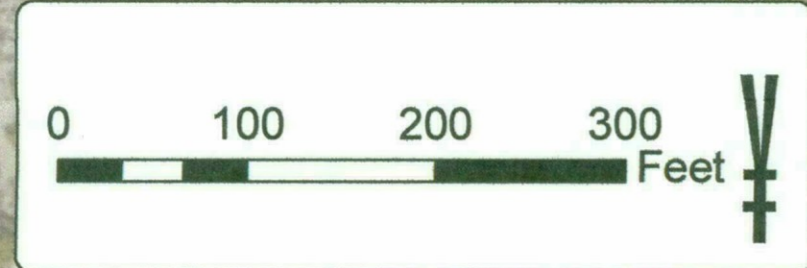
Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
F05	120	0.0124	100.0	0.30 *	5 *	30.0 *	NORMAL *	0.034	
		0.0124	100.0						
F15	120	0.0125	100.0	0.30 *	5 *	30.0 *	NORMAL *	0.034	
		0.0125	100.0						
05	120	0.0067	93.1	0.30 *	5 *	30.0 *	NORMAL *	0.036	
	730	0.0005	6.9	0.10 *	0 *	90.0 *	NORMAL *	0.180	
		0.0072	100.0						
15	120	0.0009	14.5	0.30 *	5 *	30.0 *	NORMAL *	0.036	
	730	0.0053	85.5	0.10 *	0 *	90.0 *	NORMAL *	0.182	
		0.0062	100.0						
20	120	0.0020	87.0	0.30 *	5 *	30.0 *	NORMAL *	0.039	
	730	0.0003	13.0	0.10 *	0 *	9.0 *	NORMAL *	0.197	
		0.0023	100.0						

* Non default value



Legend

- Subbasin
- 120
- 730



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 Engineering, Planning and Environmental Consultants
 7740 North 16th Street, Suite 300
 Phoenix, Arizona 85020 (602) 944-5900

SCALE(H):1"=	100
SCALE(V):	N/A
DESIGNED BY:	ZRS
DRAWN BY:	BAH
CHECKED BY:	ZRS
DATE:	03/2018

CITY OF SCOTTSDALE
 ESTATES AT HAYDEN
 PROPOSED LANDUSE

PROJECT NO.	291109000
DRAWING NAME	PR LANDUSE

Flood Control District of Maricopa County
 Drainage Design Management System
 SUB BASINS

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr	
Major Basin ID: 01																		
OFF05	0.012	0.27	98.5	98.5	NATURAL	0.034	0.30	0.25	5.46	0.229	5	Tc (Hrs)	0.288	0.272	0.226	0.204	0.178	0.178
												Vel (f/s)	1.38	1.46	1.75	1.94	2.15	2.22
												R (Hrs)	0.406	0.380	0.309	0.277	0.246	0.238
OFF15	0.012	0.19	230.4	226.5	NATURAL	0.034	0.30	0.25	6.00	0.183	5	Tc (Hrs)	0.183	0.173	0.143*	0.130*	0.118*	0.114*
												Vel (f/s)	1.52	1.61	1.95	2.14	2.36	2.44
												R (Hrs)	0.185	0.173	0.141	0.127	0.113	0.109
ON05	0.007	0.17	138.7	138.7	URBAN	0.046	0.29	0.25	6.00	0.191	5	Tc (Hrs)	0.237	0.223	0.185	0.168	0.152*	0.147*
												Vel (f/s)	1.05	1.12	1.35	1.48	1.64	1.70
												R (Hrs)	0.306	0.287	0.233	0.209	0.187	0.181
ON15	0.006	0.18	142.0	142.0	NATURAL	0.161	0.13	0.25	6.00	0.269	1	Tc (Hrs)	0.479	0.451	0.374	0.338	0.304	0.294
												Vel (f/s)	0.55	0.59	0.71	0.78	0.87	0.90
												R (Hrs)	0.766	0.715	0.582	0.520	0.463	0.446
ON20	0.002	0.08	155.8	155.8	URBAN	0.060	0.27	0.25	6.00	0.179	4	Tc (Hrs)	0.178	0.168	0.139*	0.127*	0.115*	0.111*
												Vel (f/s)	0.66	0.70	0.84	0.92	1.02	1.06
												R (Hrs)	0.250	0.234	0.190	0.171	0.153	0.148

* Non default value or value out of range

Flood Control District of Maricopa County
 Drainage Design Management System
 HEC-1 ROUTING DATA
 Project Reference: ESTATES AT HAYDEN PR

Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)	1.	2.	3.	4.	5.	6.	7.	8.
NORMAL DEPTH														
Major Basin 01														
RON05	0.076	0.038	0.076	698.00	0.0186	-	X: 5.60	123.40	137.60	141.90	143.40	147.10	161.50	192.90
							Y: 2,412.22	2,403.48	2,402.24	2,400.40	2,400.46	2,401.89	2,402.86	2,404.84
RON15	0.076	0.038	0.073	722.00	0.0194	-	X: 7.20	66.20	275.00	300.30	349.70	364.10	401.50	491.90
							Y: 2,409.21	2,404.24	2,402.39	2,401.27	2,401.28	2,402.98	2,403.48	2,410.00

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 01MAR18 TIME 09:12:18
*
*****
    
```

Proposed 2-year Storm

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
    
```

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID ESTATES AT HAYDEN PR - Estates at Hayden PR
3 ID 2 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: Clark
6 ID Storm: Multiple
7 ID 08/01/2017
8 ID
9 ID NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
10 ID MAY 2002
11 ID
12 ID FILE NAME: NS.DAT
13 ID THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
14 ID THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.
15 ID
16 ID 2-YEAR, 6-HOUR FREQUENCY
17 ID
18 ID METHODOLOGY:
19 ID THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
20 ID VERSION 4.1
21 ID MULTIPLE STORMS
22 ID CLARK UNIT HYDROGRAPH
23 ID GREEN AMPT LOSS METHOD
24 ID NORMAL DEPTH BASIN AND REACH ROUTING METHOD
25 ID
26 ID LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS
27 ID
28 ID PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
29 ID UPPER BOULDERS WASH (UB)
30 ID FAN 6C (F6C)
31 ID FAN 6A NORTH (F6AN)
32 ID FAN 6A SOULTH (F6AS)
33 ID FAN 6A (F6A)
34 ID UPPER FAN 5 (UF5)
35 ID
36 ID PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
37 ID NUMBER. I.E. STAGE COACH PASS SUB-BASIN 14 IS DESIGNATED AS SCP-14.
38 ID
39 ID PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
40 ID CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOSEN BY THE SUB
41 ID BASIN THE CONCENTRATION POINT IS LOCATED IN. I.E. CONCENTRATION POINT IN
42 ID SUB-BASIN 06 OF UPPER BOULDERS WASH IS DESIGNATED AS CUB06.
43 ID
44 ID PROJECT ROUTES ARE DESIGNATED AS R FOLLOWED BY SUB-BASIN NAME AND ROUTE
45 ID NUMBER. ROUTE NUMBER IS CHOSEN BY THE SUB-BASIN IN WHICH THE ROUTE BEGINS.
46 ID IE A ROUTE FROM SUB-BASIN 4 IN FAN 6C IS DESIGNATED AS RF6C4.
47 ID
48 ID
49 ID
50 ID
51 ID *DIAGRAM
52 ID IT 1 2000
53 ID IO 5
54 ID IN 15
55 ID *
    
```

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
    
```

54	JD	1.600	0.0001								
55	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
56	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
57	PC	0.962	0.972	0.983	0.991	1.000					
58	JD	1.590	0.5000								
59	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
60	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
61	PC	0.962	0.972	0.983	0.991	1.000					
62	JD	1.560	2.8								
63	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076
64	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938
65	PC	0.950	0.963	0.975	0.988	1.000					

* END STAGE COACH PASS AND BEGIN FAN 6C
 *

66	KK	F6C-6	BASIN								
67	BA	0.888									
68	LG	0.33	0.25	5.40	0.25	27					
69	UC	1.500	1.910								
70	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
71	UA	100									

72	KK	RF6C6	ROUTE	REACH							
73	RS	2	STOR	-1							
74	RC	0.050	0.035	0.050	1780	0.0185	0.00				
75	RX	864.4	924.1	983.8	985.5	1015.1	1025.5	1034.0	1042.4		
76	RY	626.0	625.0	624.0	622.0	622.0	624.0	625.0	626.0		

77	KK	F6C-4	BASIN								
78	BA	0.025									
79	LG	0.31	0.25	3.95	0.57	43					
80	UC	0.240	0.340								
81	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
82	UA	100									

83 KK CF6C4
 84 KM COMBINE F6C-4, AND RF6C6
 85 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 86 HC 2
 *

87	KK	F6C-5	BASIN								
88	BA	0.137									
89	LG	0.32	0.25	3.95	0.58	42					
90	UC	0.570	0.940								
91	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
92	UA	100									

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

93 KK CF6C5
 94 KM COMBINE CF6C4, AND F6C-5
 95 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 96 HC 2
 *

97	KK	RF6C4	ROUTE	REACH							
98	RS	3	STOR	-1							
99	RC	0.050	0.035	0.050	2707	0.0070	0.00				
100	RX	697.3	735.7	986.9	990.6	1012.9	1106.1	1185.2	1250.7		
101	RY	596.0	592.0	592.0	590.0	590.0	592.0	594.0	596.0		

102	KK	F6C-3	BASIN								
103	BA	0.046									
104	LG	0.33	0.25	4.20	0.53	43					
105	UC	0.380	0.500								
106	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
107	UA	100									

108 KK CF6C3
 109 KM COMBINE F6C-3, AND RF6C4
 110 KM EXITING LEGEND TRAIL ALONG WEST BOUNDARY.
 111 HC 2
 *

112	KK	RF6C3	ROUTE	REACH							
113	RS	4	STOR	-1							
114	RC	0.058	0.038	0.058	3172	0.0284	0.00				
115	RX	731.4	760.4	943.4	993.9	1019.8	1057.8	1074.8	1160.5		
116	RY	552.0	550.0	548.0	548.0	548.0	550.0	550.0	552.0		

117	KK	F6C-2	BASIN								
118	BA	0.088									
119	LG	0.29	0.15	7.30	0.13	34					
120	UC	0.310	0.350								

```

121    UA      0    3.0    5.0    8.0    12.0    20.0    43.0    75.0    90.0    96.0
122    UA     100
      *

123    KK    CF6C2
124    KM    COMBINE F6C-2, AND RF6C3
125    KM    AT PIMA ROAD FLOW SPLIT.
126    HC      2
      *

127    KK    SF6C2
128    RS      1    FLOW      1
129    SA      0    0.01
130    SE    2502    2512
131    SQ      0    10000
132    SE    2502    2512
      *
    
```

1

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

133    KK    DCF6C2
134    KM    TOTAL FLOW=DI
135    KM    CONTINUING FLOW=DCF6C2 (DI-DQ)
136    KM    DIVERTED FLOW=DCUB09 (DQ)
137    DT    DCUB09
138    DI      0    248    969    1174    1321    1369    1517
139    DQ      0      0      0      7      18      28      37
      * END FAN 6C AND BEGIN UPPER BOULDERS WASH
      *

140    KK    UB-12    BASIN
141    BA    0.059
142    LG    0.30    0.25    3.95    0.56    46
143    UC    0.300    0.400
144    UA      0    3.0    5.0    8.0    12.0    20.0    43.0    75.0    90.0    96.0
145    UA     100
      *
    
```

```

146    KK    RUB12    ROUTE    REACH
147    RS      6      STOR     -1
148    RC    0.056    0.044    0.056    5388    0.0182    0.00
149    RX    849.6    861.9    926.8    975.3    1017.2    1046.5    1060.3    1074.7
150    RY    650.0    648.0    648.0    642.0    642.0    646.0    648.0    650.0
      *
    
```

```

151    KK    UB-10    BASIN
152    BA    0.094
153    LG    0.28    0.25    4.15    0.48    47
154    UC    0.410    0.570
155    UA      0    3.0    5.0    8.0    12.0    20.0    43.0    75.0    90.0    96.0
156    UA     100
      *
    
```

```

157    KK    CUB10
158    KM    COMBINE UB-10, AND RUB12
159    KM    EXITING LEGEND TRAIL ON NORTHWEST CORNER.
160    HC      2
      *
    
```

```

161    KK    UB-11    BASIN
162    BA    0.206
163    LG    0.27    0.25    4.10    0.54    54
164    UC    0.430    0.450
165    UA      0    3.0    5.0    8.0    12.0    20.0    43.0    75.0    90.0    96.0
166    UA     100
      *
    
```

```

167    KK    CUB11
168    KM    COMBINE CUB10, AND UB-11
169    KM    EXITING LEGEND TRAIL ON NORTHWEST CORNER.
170    HC      2
      *
    
```

1

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

171    KK    RUB10    ROUTE    REACH
172    RS      4      STOR     -1
173    RC    0.056    0.044    0.056    3907    0.0179    0.00
174    RX    810.4    844.8    959.8    972.8    1005.9    1035.5    1109.8    1164.3
175    RY    564.0    562.0    560.0    556.0    556.0    560.0    560.0    562.0
      *
    
```

```

176    KK    UB-09    BASIN
177    BA    0.342
178    LG    0.28    0.23    6.20    0.20    41
179    UC    0.480    0.450
180    UA      0    5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
181    UA     100
      *
    
```

```

182    KK    CUB09
183    KM    COMBINE UB-09, AND RUB10
    
```

```

184      KM  AT PIMA ROAD FLOW SPLIT.
185      HC      2
      *

186      KK  SUB09
187      RS      1  FLOW      1
188      SA      0  0.01
189      SE      2502  2520
190      SQ      0  10000
191      SE      2502  2520
      *

192      KK  DCUB09
193      DR  DCUB09

194      KK  DCUB09
195      KM  COMBINE DCUB09, AND SUB09
196      KM  FLOW SPLIT WEST OF PIMA ROAD FROM FAN 6C TO UPPER BOULDERS
197      HC      2
      *

198      KK  RUB09  ROUTE  REACH
199      RS      5  STOR    -1
200      RC  0.061  0.038  0.061  4450  0.0187  0.00
201      RX  865.9  913.3  986.7  990.5  1009.0  1014.7  1037.1  1056.8
202      RY  492.0  489.0  488.0  486.0  486.0  488.0  490.0  492.0
      *

203      KK  UB-07  BASIN
204      BA  0.191
205      LG  0.30  0.15  8.80  0.06  16
206      UC  0.440  0.490
207      UA  0  3.0  5.0  8.0  12.0  20.0  43.0  75.0  90.0  96.0
208      UA  100
      *
    
```

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

209      KK  CUB07
210      KM  COMBINE UB-07, AND RUB09
211      KM  CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.
212      HC      2
      *

213      KK  UB-08C  BASIN
214      BA  0.010
215      LG  0.25  0.25  4.00  0.55  55
216      UC  0.160  0.220
217      UA  0  3.0  5.0  8.0  12.0  20.0  43.0  75.0  90.0  96.0
218      UA  100
      *

219      KK  RUB08C  ROUTE  REACH
220      RS      7  STOR    -1
221      RC  0.061  0.038  0.061  5952  0.0190  0.00
222      RX  975.3  981.7  986.9  991.0  1009.2  1073.1  1118.9  1137.7
223      RY  498.0  496.0  494.0  492.0  492.0  494.0  496.0  498.0
      *

224      KK  UB-08B  BASIN
225      BA  0.033
226      LG  0.25  0.17  6.80  0.15  42
227      UC  0.200  0.220
228      UA  0  3.0  5.0  8.0  12.0  20.0  43.0  75.0  90.0  96.0
229      UA  100
      *

230      KK  RUB08B  ROUTE  REACH
231      RS      7  STOR    -1
232      RC  0.061  0.038  0.061  6656  0.0173  0.00
233      RX  975.3  981.7  986.9  991.0  1009.2  1073.1  1118.9  1137.7
234      RY  498.0  496.0  494.0  492.0  492.0  494.0  496.0  498.0
      *

235      KK  UB-08A  BASIN
236      BA  0.121
237      LG  0.30  0.25  5.70  0.21  26
238      UC  0.520  0.750
239      UA  0  5.0  16.0  30.0  65.0  77.0  84.0  90.0  94.0  97.0
240      UA  100
      *

241      KK  CUB08
242      KM  COMBINE UB-08A, RUB08B, RUB08C, AND CUB07
243      KM  CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.
244      HC      4
      *
    
```

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

245      KK  RUB07  ROUTE  REACH
246      RS      2  STOR    -1
    
```

247	RC	0.061	0.038	0.061	728	0.0247	0.00							
248	RX	802.8	896.1	939.4	977.6	1023.6	1036.2	1071.8	1144.8					
249	RY	420.0	415.1	416.5	416.0	416.0	418.0	416.4	420.0					
	*													
250	KK	OFF15	BASIN											
251	BA	0.012												
252	LG	0.30	0.25	6.00	0.18	5								
253	UC	0.183	0.185											
254	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
255	UA	100												
	*													
256	KK	COFF15												
257	HC	2												
	*													
258	KK	RON15	ROUTE											
259	RS	1	FLOW											
260	RC	0.076	0.038	0.073	722	0.0194	0.00							
261	RX	7.20	66.20	275.00	300.30	349.70	364.10	401.50	491.90					
262	RY	2409.2	2404.24	2402.39	2401.27	2401.28	2402.98	2403.48	2410.00					
	*													
263	KK	ON15	BASIN											
264	BA	0.006												
265	LG	0.13	0.25	6.00	0.27	1								
266	UC	0.479	0.766											
267	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
268	UA	100												
	*													
269	KK	ON05	BASIN											
270	BA	0.007												
271	LG	0.29	0.25	6.00	0.19	5								
272	UC	0.237	0.306											
273	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
274	UA	100												
	*													
275	KK	D1	STORAGE											
276	RS	1	STOR											
277	SV	0.03	0.07	0.09	0.10	0.12	0.14	0.16	0.18					
278	SE	2395.0	2396.00	2396.75	2397.00	2397.25	2397.50	2397.75	2398.00	2398.25				
279	SQ	0	0	1	10	28	50	76	106	139				
	*													

HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

280	KK	ON20	BASIN											
281	BA	0.002												
282	LG	0.27	0.25	6.00	0.18	4								
283	UC	0.178	0.250											
284	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
285	UA	100												
	*													
286	KK	CPON15	COMBINE											
287	HC	4												
	*													
288	KK	OFF05	BASIN											
289	BA	0.012												
290	LG	0.30	0.25	5.46	0.23	5								
291	UC	0.288	0.406											
292	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
293	UA	100												
	*													
294	KK	RON05	ROUTE											
295	RS	1	FLOW											
296	RC	0.076	0.038	0.076	698	0.0186	0.00							
297	RX	5.60	123.40	137.60	141.90	143.40	147.10	161.50	192.90					
298	RY	2412.2	2403.48	2402.24	2400.40	2400.46	2401.89	2402.86	2404.84					
	*													
299	ZZ													

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
66	F6C-6	
	V	
	V	
72	RF6C6	
	.	
	.	
77	F6C-4	
	.	
	.	
83	CF6C4.....	
	.	

```
87      .          F6C-5
      .
93      CF6C5.....
      V
      V
97      RF6C4
      .
102     .          F6C-3
      .
108     CF6C3.....
      V
      V
112     RF6C3
      .
117     .          F6C-2
      .
123     CF6C2.....
      V
      V
127     SF6C2
      .
137     -----> DCUB09
133     DCF6C2
      .
140     .          UB-12
      .          V
      .          V
146     .          RUB12
      .
151     .          UB-10
      .
157     .          CUB10.....
      .
161     .          UB-11
      .
167     .          CUB11.....
      .          V
      .          V
171     .          RUB10
      .
176     .          UB-09
      .
182     .          CUB09.....
      .          V
      .          V
186     .          SUB09
      .
193     .          .          <----- DCUB09
192     .          DCUB09
      .
194     .          DCUB09.....
      .          V
      .          V
198     .          RUB09
      .
203     .          UB-07
      .
209     .          CUB07.....
      .
213     .          UB-08C
      .          V
      .          V
219     .          RUB08C
      .
224     .          .          UB-08B
      .          .          V
      .          .          V
230     .          .          RUB08B
      .
235     .          .          .          UB-08A
      .
241     .          CUB08.....
      .          V
      .          V
```

```

245      .      RUB07
      .      .
250      .      .      OFF15
      .      .      .
256      .      COFF15.....
      .      V
      .      V
258      .      RON15
      .      .
263      .      .      ON15
      .      .      .
269      .      .      .      ON05
      .      .      .      V
275      .      .      .      V
      .      .      .      D1
      .      .      .      .
280      .      .      .      .      ON20
      .      .      .      .      .
286      .      .      .      .      .
288      .      .      .      .      .
294      .      .      .      .      .
    
```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 01MAR18 TIME 09:12:18 *
*****
    
```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

Flood Control District of Maricopa County
 ESTATES AT HAYDEN PR - Estates at Hayden PR
 2 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 08/01/2017

NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
 MAY 2002

FILE NAME: NS.DAT
 THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
 THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.

2-YEAR, 6-HOUR FREQUENCY

METHODOLOGY:
 THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
 VERSION 4.1
 MULTIPLE STORMS
 CLARK UNIT HYDROGRAPH
 GREEN AMPT LOSS METHOD
 NORMAL DEPTH BASIN AND REACH ROUTING METHOD

LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS

PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
 UPPER BOULDERS WASH (UB)
 FAN 6C (F6C)
 FAN 6A NORTH (F6AN)
 FAN 6A SOULTH (F6AS)
 FAN 6A (F6A)
 UPPER FAN 5 (UF5)

PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
 NUMBER. I.E. STAGE COACH PASS SUB-BASIN 14 IS DESIGNATED AS SCP-14.

PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
 CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOSEN BY THE SUB
 BASIN THE CONCENTRATION POINT IS LOCATED IN. I.E. CONCENTRATION POINT IN
 SUB-BASIN 06 OF UPPER BOULDERS WASH IS DESIGNATED AS CUB06.

PROJECT ROUTES ARE DESIGNATED AS R FOLLOWED BY SUB-BASIN NAME AND ROUTE
 NUMBER. ROUTE NUMBER IS CHOSEN BY THE SUB-BASIN IN WHICH THE ROUTE BEGINS.
 IE A ROUTE FROM SUB-BASIN 4 IN FAN 6C IS DESIGNATED AS RF6C4.

+	HYDROGRAPH AT	F6C-2	74.	4.20	9.	2.	2.	.09
+	2 COMBINED AT	CF6C2	155.	5.17	77.	20.	15.	1.18
+	ROUTED TO	SF6C2	155.	5.17	77.	20.	15.	1.18
+	DIVERSION TO	DCUB09	0.	.00	0.	0.	0.	1.18
+	HYDROGRAPH AT	DCF6C2	155.	5.17	77.	20.	15.	1.18
+	HYDROGRAPH AT	UB-12	38.	4.20	6.	1.	1.	.06
+	ROUTED TO	RUB12	26.	4.83	6.	1.	1.	.06
+	HYDROGRAPH AT	UB-10	51.	4.28	9.	2.	2.	.09
+	2 COMBINED AT	CUB10	59.	4.32	15.	4.	3.	.15
+	HYDROGRAPH AT	UB-11	133.	4.28	22.	6.	4.	.21
+	2 COMBINED AT	CUB11	191.	4.28	37.	9.	7.	.36
+	ROUTED TO	RUB10	178.	4.48	37.	9.	7.	.36
+	HYDROGRAPH AT	UB-09	213.	4.20	35.	9.	6.	.34
+	2 COMBINED AT	CUB09	322.	4.40	69.	17.	13.	.70
+	ROUTED TO	SUB09	322.	4.40	69.	17.	13.	.70
+	HYDROGRAPH AT	DCUB09	0.	.00	0.	0.	0.	1.18
+	2 COMBINED AT	DCUB09	322.	4.40	69.	17.	13.	.70
+	ROUTED TO	RUB09	316.	4.53	69.	17.	13.	.70
+	HYDROGRAPH AT	UB-07	128.	4.30	18.	5.	3.	.19
+	2 COMBINED AT	CUB07	393.	4.48	85.	22.	16.	.89
+	HYDROGRAPH AT	UB-08C	10.	4.08	1.	0.	0.	.01
+	ROUTED TO	RUB08C	6.	4.80	1.	0.	0.	.01
+	HYDROGRAPH AT	UB-08B	37.	4.12	4.	1.	1.	.03
+	ROUTED TO	RUB08B	20.	4.70	4.	1.	1.	.03
+	HYDROGRAPH AT	UB-08A	47.	4.27	10.	3.	2.	.12
+	4 COMBINED AT	CUB08	428.	4.50	97.	25.	18.	1.06
+	ROUTED TO	RUB07	427.	4.53	97.	25.	18.	1.06
+	HYDROGRAPH AT	OFF15	11.	4.10	1.	0.	0.	.01
+	2 COMBINED AT	COFF15	427.	4.53	97.	25.	18.	1.07
+	ROUTED TO	RON15	426.	4.55	97.	25.	18.	1.07
+	HYDROGRAPH AT	ON15	2.	4.33	0.	0.	0.	.01
+	HYDROGRAPH AT	ON05	5.	4.08	0.	0.	0.	.01

+	ROUTED TO	D1	4.	4.18	0.	0.	0.	.01
+	HYDROGRAPH AT	ON20	2.	4.07	0.	0.	0.	.00
+	4 COMBINED AT	CPON15	428.	4.55	97.	25.	18.	1.08
+	HYDROGRAPH AT	OFF05	7.	4.18	1.	0.	0.	.01
+	ROUTED TO	RON05	6.	4.23	1.	0.	0.	.01

*** NORMAL END OF HEC-1 ***

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 01MAR18 TIME 09:12:30
*
*****
    
```

Proposed 10-year Storm

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
    
```

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID ESTATES AT HAYDEN PR - Estates at Hayden PR
3 ID 10 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: Clark
6 ID Storm: Multiple
7 ID 08/01/2017
8 ID
9 ID NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
10 ID MAY 2002
11 ID
12 ID FILE NAME: NS.DAT
13 ID THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
14 ID THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.
15 ID
16 ID 10-YEAR, 6-HOUR FREQUENCY
17 ID
18 ID METHODOLOGY:
19 ID THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
20 ID VERSION 4.1
21 ID MULTIPLE STORMS
22 ID CLARK UNIT HYDROGRAPH
23 ID GREEN AMPT LOSS METHOD
24 ID NORMAL DEPTH BASIN AND REACH ROUTING METHOD
25 ID
26 ID LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS
27 ID
28 ID PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
29 ID UPPER BOULDERS WASH (UB)
30 ID FAN 6C (F6C)
31 ID FAN 6A NORTH (F6AN)
32 ID FAN 6A SOULTH (F6AS)
33 ID FAN 6A (F6A)
34 ID UPPER FAN 5 (UF5)
35 ID
36 ID PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
37 ID NUMBER. I.E. STAGE COACH PASS SUB-BASIN 14 IS DESIGNATED AS SCP-14.
38 ID
39 ID PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
40 ID CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOSEN BY THE SUB
41 ID BASIN THE CONCENTRATION POINT IS LOCATED IN. I.E. CONCENTRATION POINT IN
42 ID SUB-BASIN 06 OF UPPER BOULDERS WASH IS DESIGNATED AS CUB06.
43 ID
44 ID PROJECT ROUTES ARE DESIGNATED AS R FOLLOWED BY SUB-BASIN NAME AND ROUTE
45 ID NUMBER. ROUTE NUMBER IS CHOSEN BY THE SUB-BASIN IN WHICH THE ROUTE BEGINS.
46 ID IE A ROUTE FROM SUB-BASIN 4 IN FAN 6C IS DESIGNATED AS RF6C4.
47 ID
48 ID
49 ID
50 ID
51 ID *DIAGRAM
52 IT 1 2000
53 IO 5
IN 15
*
    
```

1 HEC-1 INPUT PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

54	JD	2.370	0.0001								
55	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
56	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
57	PC	0.962	0.972	0.983	0.991	1.000					
58	JD	2.356	0.5000								
59	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
60	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
61	PC	0.962	0.972	0.983	0.991	1.000					
62	JD	2.311	2.8								
63	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076
64	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938
65	PC	0.950	0.963	0.975	0.988	1.000					

* END STAGE COACH PASS AND BEGIN FAN 6C

66	KK	F6C-6	BASIN								
67	BA	0.888									
68	LG	0.33	0.25	5.40	0.25	27					
69	UC	0.600	0.670								
70	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
71	UA	100									

72	KK	RF6C6	ROUTE	REACH							
73	RS	2	STOR	-1							
74	RC	0.050	0.035	0.050	1780	0.0185	0.00				
75	RX	864.4	924.1	983.8	985.5	1015.1	1025.5	1034.0	1042.4		
76	RY	626.0	625.0	624.0	622.0	622.0	624.0	625.0	626.0		

77	KK	F6C-4	BASIN								
78	BA	0.025									
79	LG	0.31	0.25	3.95	0.57	43					
80	UC	0.190	0.260								
81	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
82	UA	100									

83 KK CF6C4
 84 KM COMBINE F6C-4, AND RF6C6
 85 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 86 HC 2

87	KK	F6C-5	BASIN								
88	BA	0.137									
89	LG	0.32	0.25	3.95	0.58	42					
90	UC	0.400	0.630								
91	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
92	UA	100									

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

93 KK CF6C5
 94 KM COMBINE CF6C4, AND F6C-5
 95 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 96 HC 2

97	KK	RF6C4	ROUTE	REACH							
98	RS	3	STOR	-1							
99	RC	0.050	0.035	0.050	2707	0.0070	0.00				
100	RX	697.3	735.7	986.9	990.6	1012.9	1106.1	1185.2	1250.7		
101	RY	596.0	592.0	592.0	590.0	590.0	592.0	594.0	596.0		

102	KK	F6C-3	BASIN								
103	BA	0.046									
104	LG	0.33	0.25	4.20	0.53	43					
105	UC	0.270	0.340								
106	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
107	UA	100									

108 KK CF6C3
 109 KM COMBINE F6C-3, AND RF6C4
 110 KM EXITING LEGEND TRAIL ALONG WEST BOUNDARY.
 111 HC 2

112	KK	RF6C3	ROUTE	REACH							
113	RS	4	STOR	-1							
114	RC	0.058	0.038	0.058	3172	0.0284	0.00				
115	RX	731.4	760.4	943.4	993.9	1019.8	1057.8	1074.8	1160.5		
116	RY	552.0	550.0	548.0	548.0	548.0	550.0	550.0	552.0		

117	KK	F6C-2	BASIN								
118	BA	0.088									
119	LG	0.29	0.15	7.30	0.13	34					
120	UC	0.240	0.260								

184	KM	AT PIMA ROAD FLOW SPLIT.									
185	HC	2									
	*										
186	KK	SUB09									
187	RS	1	FLOW	1							
188	SA	0	0.01								
189	SE	2502	2520								
190	SQ	0	10000								
191	SE	2502	2520								
	*										
192	KK	DCUB09									
193	DR	DCUB09									
	*										
194	KK	DCUB09									
195	KM	COMBINE DCUB09, AND SUB09									
196	KM	FLOW SPLIT WEST OF PIMA ROAD FROM FAN 6C TO UPPER BOULDERS									
197	HC	2									
	*										
198	KK	RUB09	ROUTE	REACH							
199	RS	5	STOR	-1							
200	RC	0.061	0.038	0.061	4450	0.0187	0.00				
201	RX	865.9	913.3	986.7	990.5	1009.0	1014.7	1037.1	1056.8		
202	RY	492.0	489.0	488.0	486.0	486.0	488.0	490.0	492.0		
	*										
203	KK	UB-07	BASIN								
204	BA	0.191									
205	LG	0.30	0.15	8.80	0.06	16					
206	UC	0.320	0.340								
207	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	96.0	
208	UA	100									
	*										

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

209	KK	CUB07									
210	KM	COMBINE UB-07, AND RUB09									
211	KM	CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.									
212	HC	2									
	*										
213	KK	UB-08C	BASIN								
214	BA	0.010									
215	LG	0.25	0.25	4.00	0.55	55					
216	UC	0.130	0.170								
217	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	96.0	
218	UA	100									
	*										
219	KK	RUB08C	ROUTE	REACH							
220	RS	7	STOR	-1							
221	RC	0.061	0.038	0.061	5952	0.0190	0.00				
222	RX	975.3	981.7	986.9	991.0	1009.2	1073.1	1118.9	1137.7		
223	RY	498.0	496.0	494.0	492.0	492.0	494.0	496.0	498.0		
	*										
224	KK	UB-08B	BASIN								
225	BA	0.033									
226	LG	0.25	0.17	6.80	0.15	42					
227	UC	0.170	0.180								
228	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	96.0	
229	UA	100									
	*										
230	KK	RUB08B	ROUTE	REACH							
231	RS	7	STOR	-1							
232	RC	0.061	0.038	0.061	6656	0.0173	0.00				
233	RX	975.3	981.7	986.9	991.0	1009.2	1073.1	1118.9	1137.7		
234	RY	498.0	496.0	494.0	492.0	492.0	494.0	496.0	498.0		
	*										
235	KK	UB-08A	BASIN								
236	BA	0.121									
237	LG	0.30	0.25	5.70	0.21	26					
238	UC	0.360	0.500								
239	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	
240	UA	100									
	*										
241	KK	CUB08									
242	KM	COMBINE UB-08A, RUB08B, RUB08C, AND CUB07									
243	KM	CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.									
244	HC	4									
	*										

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

245	KK	RUB07	ROUTE	REACH						
246	RS	2	STOR	-1						

247	RC	0.061	0.038	0.061	728	0.0247	0.00						
248	RX	802.8	896.1	939.4	977.6	1023.6	1036.2	1071.8	1144.8				
249	RY	420.0	415.1	416.5	416.0	416.0	418.0	416.4	420.0				
	*												
250	KK	OFF15	BASIN										
251	BA	0.012											
252	LG	0.30	0.25	6.00	0.18	5							
253	UC	0.143	0.141										
254	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0		
255	UA	100											
	*												
256	KK	COFF15											
257	HC	2											
	*												
258	KK	RON15	ROUTE										
259	RS	1	FLOW										
260	RC	0.076	0.038	0.073	722	0.0194	0.00						
261	RX	7.20	66.20	275.00	300.30	349.70	364.10	401.50	491.90				
262	RY	2409.2	2404.24	2402.39	2401.27	2401.28	2402.98	2403.48	2410.00				
	*												
263	KK	ON15	BASIN										
264	BA	0.006											
265	LG	0.13	0.25	6.00	0.27	1							
266	UC	0.374	0.582										
267	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0		
268	UA	100											
	*												
269	KK	ON05	BASIN										
270	BA	0.007											
271	LG	0.29	0.25	6.00	0.19	5							
272	UC	0.185	0.233										
273	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0		
274	UA	100											
	*												
275	KK	D1 STORAGE											
276	RS	1	STOR										
277	SV	0.03	0.07	0.09	0.10	0.12	0.14	0.16	0.18				
278	SE	2395.0	2396.00	2396.75	2397.00	2397.25	2397.50	2397.75	2398.00	2398.25			
279	SQ	0	0	1	10	28	50	76	106	139			
	*												

HEC-1 INPUT

PAGE 8

LINE	ID	1	2	3	4	5	6	7	8	9	10
280	KK	ON20	BASIN								
281	BA	0.002									
282	LG	0.27	0.25	6.00	0.18	4					
283	UC	0.139	0.190								
284	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
285	UA	100									
	*										
286	KK	CPON15	COMBINE								
287	HC	4									
	*										
288	KK	OFF05	BASIN								
289	BA	0.012									
290	LG	0.30	0.25	5.46	0.23	5					
291	UC	0.226	0.309								
292	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
293	UA	100									
	*										
294	KK	RON05	ROUTE								
295	RS	1	FLOW								
296	RC	0.076	0.038	0.076	698	0.0186	0.00				
297	RX	5.60	123.40	137.60	141.90	143.40	147.10	161.50	192.90		
298	RY	2412.2	2403.48	2402.24	2400.40	2400.46	2401.89	2402.86	2404.84		
	*										
299	ZZ										

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
66	F6C-6	
	V	
	V	
72	RF6C6	
	.	
	.	
77	F6C-4	
	.	
	.	
83	CF6C4.....	

```
87      .          F6C-5
      .
93      CF6C5.....
      V
      V
97      RF6C4
      .
102     .          F6C-3
      .
108     CF6C3.....
      V
      V
112     RF6C3
      .
117     .          F6C-2
      .
123     CF6C2.....
      V
      V
127     SF6C2
      .
137     -----> DCUB09
133     DCF6C2
      .
140     .          UB-12
      .          V
      .          V
146     .          RUB12
      .
151     .          .          UB-10
      .          .
157     .          CUB10.....
      .
161     .          .          UB-11
      .          .
167     .          CUB11.....
      .          V
171     .          RUB10
      .
176     .          .          UB-09
      .          .
182     .          CUB09.....
      .          V
186     .          SUB09
      .          V
193     .          .          <----- DCUB09
192     .          DCUB09
      .
194     .          DCUB09.....
      .          V
198     .          RUB09
      .
203     .          .          UB-07
      .          .
209     .          CUB07.....
      .
213     .          .          UB-08C
      .          .          V
219     .          RUB08C
      .          .          V
224     .          .          .          UB-08B
      .          .          .          V
230     .          .          RUB08B
      .          .          .
235     .          .          .          .          UB-08A
      .          .          .          .
241     .          CUB08.....
      .          V
      .          V
```

```

245      .      RUB07
      .
250      .      .      OFF15
      .      .
256      .      COFF15.....
      .      V
258      .      V
      .      RON15
      .
263      .      .      ON15
      .      .
269      .      .      .      ON05
      .      .      .      V
275      .      .      .      V
      .      .      .      D1
      .      .
280      .      .      .      .      ON20
      .      .      .
286      .      CPON15.....
      .
288      .      .      OFF05
      .      .      V
294      .      .      .      V
      .      .      RON05
    
```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   JUN 1998                   *
*   VERSION 4.1                 *
* RUN DATE 01MAR18 TIME 09:12:30 *
*****
    
```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET             *
* DAVIS, CALIFORNIA 95616      *
* (916) 756-1104               *
*****
    
```

Flood Control District of Maricopa County
 ESTATES AT HAYDEN PR - Estates at Hayden PR
 10 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 08/01/2017

NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
 MAY 2002

FILE NAME: NS.DAT
 THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
 THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.

10-YEAR, 6-HOUR FREQUENCY

METHODOLOGY:
 THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
 VERSION 4.1
 MULTIPLE STORMS
 CLARK UNIT HYDROGRAPH
 GREEN AMPT LOSS METHOD
 NORMAL DEPTH BASIN AND REACH ROUTING METHOD

LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS

PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
 UPPER BOULDERS WASH (UB)
 FAN 6C (F6C)
 FAN 6A NORTH (F6AN)
 FAN 6A SOULTH (F6AS)
 FAN 6A (F6A)
 UPPER FAN 5 (UF5)

PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
 NUMBER. I.E. STAGE COACH PASS SUB-BASIN 14 IS DESIGNATED AS SCP-14.

PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
 CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOSEN BY THE SUB
 BASIN THE CONCENTRATION POINT IS LOCATED IN. I.E. CONCENTRATION POINT IN
 SUB-BASIN 06 OF UPPER BOULDERS WASH IS DESIGNATED AS CUB06.

PROJECT ROUTES ARE DESIGNATED AS R FOLLOWED BY SUB-BASIN NAME AND ROUTE
 NUMBER. ROUTE NUMBER IS CHOSEN BY THE SUB-BASIN IN WHICH THE ROUTE BEGINS.
 IE A ROUTE FROM SUB-BASIN 4 IN FAN 6C IS DESIGNATED AS RF6C4.

+	HYDROGRAPH AT	F6C-2	145.	4.13	15.	4.	3.	.09
+	2 COMBINED AT	CF6C2	636.	4.67	158.	40.	29.	1.18
+	ROUTED TO	SF6C2	636.	4.67	158.	40.	29.	1.18
+	DIVERSION TO	DCUB09	0.	.00	0.	0.	0.	1.18
+	HYDROGRAPH AT	DCF6C2	636.	4.67	158.	40.	29.	1.18
+	HYDROGRAPH AT	UB-12	81.	4.13	9.	2.	2.	.06
+	ROUTED TO	RUB12	53.	4.57	9.	2.	2.	.06
+	HYDROGRAPH AT	UB-10	109.	4.20	15.	4.	3.	.09
+	2 COMBINED AT	CUB10	121.	4.20	25.	6.	4.	.15
+	HYDROGRAPH AT	UB-11	275.	4.20	36.	9.	6.	.21
+	2 COMBINED AT	CUB11	396.	4.20	60.	15.	11.	.36
+	ROUTED TO	RUB10	367.	4.35	60.	15.	11.	.36
+	HYDROGRAPH AT	UB-09	458.	4.13	58.	15.	11.	.34
+	2 COMBINED AT	CUB09	673.	4.27	115.	29.	21.	.70
+	ROUTED TO	SUB09	673.	4.27	115.	29.	21.	.70
+	HYDROGRAPH AT	DCUB09	0.	.00	0.	0.	0.	1.18
+	2 COMBINED AT	DCUB09	673.	4.27	115.	29.	21.	.70
+	ROUTED TO	RUB09	646.	4.45	115.	29.	21.	.70
+	HYDROGRAPH AT	UB-07	273.	4.20	32.	8.	6.	.19
+	2 COMBINED AT	CUB07	789.	4.40	144.	36.	26.	.89
+	HYDROGRAPH AT	UB-08C	19.	4.07	2.	0.	0.	.01
+	ROUTED TO	RUB08C	10.	4.75	2.	0.	0.	.01
+	HYDROGRAPH AT	UB-08B	65.	4.08	6.	2.	1.	.03
+	ROUTED TO	RUB08B	41.	4.57	6.	2.	1.	.03
+	HYDROGRAPH AT	UB-08A	116.	4.15	18.	4.	3.	.12
+	4 COMBINED AT	CUB08	876.	4.42	167.	42.	30.	1.06
+	ROUTED TO	RUB07	875.	4.43	167.	42.	30.	1.06
+	HYDROGRAPH AT	OFF15	23.	4.07	1.	0.	0.	.01
+	2 COMBINED AT	COFF15	877.	4.43	168.	42.	31.	1.07
+	ROUTED TO	RON15	875.	4.47	168.	42.	31.	1.07
+	HYDROGRAPH AT	ON15	5.	4.25	1.	0.	0.	.01
+	HYDROGRAPH AT	ON05	11.	4.05	1.	0.	0.	.01

+	ROUTED TO	D1	11.	4.07	1.	0.	0.	.01
+	HYDROGRAPH AT	ON20	3.	4.03	0.	0.	0.	.00
+	4 COMBINED AT	CPON15	879.	4.45	169.	43.	31.	1.08
+	HYDROGRAPH AT	OFF05	15.	4.13	1.	0.	0.	.01
+	ROUTED TO	RON05	14.	4.17	1.	0.	0.	.01

*** NORMAL END OF HEC-1 ***

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 01MAR18 TIME 09:12:43
*
*****
    
```

Proposed 100-year Storm

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
    
```

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID ESTATES AT HAYDEN PR - Estates at Hayden PR
3 ID 100 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: Clark
6 ID Storm: Multiple
7 ID 08/01/2017
8 ID
9 ID NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
10 ID MAY 2002
11 ID
12 ID FILE NAME: NS.DAT
13 ID THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
14 ID THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.
15 ID
16 ID 100-YEAR, 6-HOUR FREQUENCY
17 ID
18 ID METHODOLOGY:
19 ID THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
20 ID VERSION 4.1
21 ID MULTIPLE STORMS
22 ID CLARK UNIT HYDROGRAPH
23 ID GREEN AMPT LOSS METHOD
24 ID NORMAL DEPTH BASIN AND REACH ROUTING METHOD
25 ID
26 ID LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS
27 ID
28 ID PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
29 ID UPPER BOULDERS WASH (UB)
30 ID FAN 6C (F6C)
31 ID FAN 6A NORTH (F6AN)
32 ID FAN 6A SOULTH (F6AS)
33 ID FAN 6A (F6A)
34 ID UPPER FAN 5 (UF5)
35 ID
36 ID PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
37 ID NUMBER. I.E. STAGE COACH PASS SUB-BASIN 14 IS DESIGNATED AS SCP-14.
38 ID
39 ID PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
40 ID CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOSEN BY THE SUB
41 ID BASIN THE CONCENTRATION POINT IS LOCATED IN. I.E. CONCENTRATION POINT IN
42 ID SUB-BASIN 06 OF UPPER BOULDERS WASH IS DESIGNATED AS CUB06.
43 ID
44 ID PROJECT ROUTES ARE DESIGNATED AS R FOLLOWED BY SUB-BASIN NAME AND ROUTE
45 ID NUMBER. ROUTE NUMBER IS CHOSEN BY THE SUB-BASIN IN WHICH THE ROUTE BEGINS.
46 ID IE A ROUTE FROM SUB-BASIN 4 IN FAN 6C IS DESIGNATED AS RF6C4.
47 ID
48 ID
49 ID
50 ID
51 *DIAGRAM IT 1 2000
52 IO 5
53 IN 15
*
    
```

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
    
```

54	JD	3.50	0.01								
55	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074
56	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950
57	PC	0.962	0.972	0.983	0.991	1.000					
58	JD	3.477	0.50								
59	JD	3.412	2.80								
60	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076
61	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938
62	PC	0.950	0.963	0.975	0.988	1.000					
63	JD	3.227	16.0								
64	PC	0.000	0.015	0.020	0.030	0.048	0.063	0.076	0.090	0.105	0.119
65	PC	0.135	0.152	0.175	0.222	0.304	0.472	0.670	0.796	0.868	0.912
66	PC	0.946	0.960	0.973	0.987	1.000					
67	JD	2.842	90.0								
68	PC	0.000	0.021	0.035	0.051	0.071	0.087	0.105	0.125	0.143	0.160
69	PC	0.179	0.201	0.232	0.281	0.364	0.500	0.658	0.773	0.841	0.888
70	PC	0.927	0.945	0.964	0.982	1.000					
71	JD	1.995	500.0								
72	PC	0.000	0.024	0.043	0.059	0.078	0.098	0.119	0.141	0.162	0.186
73	PC	0.212	0.239	0.271	0.321	0.408	0.515	0.627	0.735	0.814	0.864
74	PC	0.907	0.930	0.954	0.977	1.000					

*DIAGRAM
 * END STAGE COACH PASS AND BEGIN FAN 6C
 *

75	KK	F6C-6	BASIN								
76	BA	0.888									
77	LG	0.33	0.25	5.40	0.25	27					
78	UC	0.438	0.469								
79	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
80	UA	100									

81	KK	RF6C6	ROUTE	REACH							
82	RS	2	STOR	-1							
83	RC	0.050	0.035	0.050	1780	0.0185	0.00				
84	RX	864.4	924.1	983.8	985.5	1015.1	1025.5	1034.0	1042.4		
85	RY	626.0	625.0	624.0	622.0	622.0	624.0	625.0	626.0		

86	KK	F6C-4	BASIN								
87	BA	0.025									
88	LG	0.31	0.25	3.95	0.57	43					
89	UC	0.158	0.217								
90	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
91	UA	100									

92 KK CF6C4
 93 KM COMBINE F6C-4, AND RF6C6
 94 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 95 HC 2
 *

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

96	KK	F6C-5	BASIN								
97	BA	0.137									
98	LG	0.32	0.25	3.95	0.58	42					
99	UC	0.300	0.464								
100	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
101	UA	100									

102 KK CF6C5
 103 KM COMBINE CF6C4, AND F6C-5
 104 KM CONFLUENCE WITHIN LENGEN TRAIL LOCATED IN SECTION 6 OF T5NR5E.
 105 HC 2
 *

106	KK	RF6C4	ROUTE	REACH							
107	RS	3	STOR	-1							
108	RC	0.050	0.035	0.050	2707	0.0070	0.00				
109	RX	697.3	735.7	986.9	990.6	1012.9	1106.1	1185.2	1250.7		
110	RY	596.0	592.0	592.0	590.0	590.0	592.0	594.0	596.0		

111	KK	F6C-3	BASIN								
112	BA	0.046									
113	LG	0.33	0.25	4.20	0.53	43					
114	UC	0.221	0.278								
115	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
116	UA	100									

117 KK CF6C3
 118 KM COMBINE F6C-3, AND RF6C4
 119 KM EXITING LENGEN TRAIL ALONG WEST BOUNDARY.
 120 HC 2
 *

121	KK	RF6C3	ROUTE	REACH							
122	RS	4	STOR	-1							

250	KK	CUB08												
251	KM	COMBINE UB-08A, RUB08B, RUB08C, AND CUB07												
252	KM	CONFLUENCE POINT SOUTH OF BLACK MOUNTAIN ROAD AND EAST OF HAYDEN ROAD.												
253	HC	4												
	*													
254	KK	RUB07	ROUTE	REACH										
255	RS	2	STOR	-1										
256	RC	0.061	0.038	0.061	728	0.0247	0.00							
257	RX	802.8	896.1	939.4	977.6	1023.6	1036.2	1071.8	1144.8					
258	RY	420.0	415.1	416.5	416.0	416.0	418.0	416.4	420.0					
	*													
259	KK	OFF15	BASIN											
260	BA	0.012												
261	LG	0.30	0.25	6.00	0.18	5								
262	UC	0.114	0.109											
263	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
264	UA	100												
	*													
265	KK	COFF15												
266	HC	2												
	*													
267	KK	RON15	ROUTE											
268	RS	1	FLOW											
269	RC	0.076	0.038	0.073	722	0.0194	0.00							
270	RX	7.20	66.20	275.00	300.30	349.70	364.10	401.50	491.90					
271	RY	2409.2	2404.24	2402.39	2401.27	2401.28	2402.98	2403.48	2410.00					
	*													
272	KK	ON15	BASIN											
273	BA	0.006												
274	LG	0.13	0.25	6.00	0.27	1								
275	UC	0.294	0.446											
276	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
277	UA	100												
	*													
278	KK	ON05	BASIN											
279	BA	0.007												
280	LG	0.29	0.25	6.00	0.19	5								
281	UC	0.147	0.181											
282	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
283	UA	100												
	*													

HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

284	KK	D1	STORAGE											
285	RS	1	STOR											
286	SV		0.03	0.07	0.09	0.10	0.12	0.14	0.16	0.18				
287	SE	2395.0	2396.00	2396.75	2397.00	2397.25	2397.50	2397.75	2398.00	2398.25				
288	SQ	0	0	1	10	28	50	76	106	139				
	*													
289	KK	ON20	BASIN											
290	BA	0.002												
291	LG	0.27	0.25	6.00	0.18	4								
292	UC	0.111	0.148											
293	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0			
294	UA	100												
	*													
295	KK	CPON15	COMBINE											
296	HC	4												
	*													
297	KK	OFF05	BASIN											
298	BA	0.012												
299	LG	0.30	0.25	5.46	0.23	5								
300	UC	0.178	0.238											
301	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0			
302	UA	100												
	*													
303	KK	RON05	ROUTE											
304	RS	1	FLOW											
305	RC	0.076	0.038	0.076	698	0.0186	0.00							
306	RX	5.60	123.40	137.60	141.90	143.40	147.10	161.50	192.90					
307	RY	2412.2	2403.48	2402.24	2400.40	2400.46	2401.89	2402.86	2404.84					
	*													
308	ZZ													

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```
81      V
      RF6C6
      .
86      .          F6C-4
      .
92      CF6C4.....
      .
96      .          F6C-5
      .
102     CF6C5.....
      V
      V
106     RF6C4
      .
111     .          F6C-3
      .
117     CF6C3.....
      V
      V
121     RF6C3
      .
126     .          F6C-2
      .
132     CF6C2.....
      V
      V
136     SF6C2
      .
146     -----> DCUB09
142     DCF6C2
      .
149     .          UB-12
      .          V
155     .          RUB12
      .          V
160     .          .          UB-10
      .          .
166     .          CUB10.....
      .          .
170     .          .          UB-11
      .          .
176     .          CUB11.....
      .          V
180     .          RUB10
      .          V
185     .          .          UB-09
      .          .
191     .          CUB09.....
      .          V
195     .          SUB09
      .          V
202     .          .          DCUB09 <----- DCUB09
201     .          DCUB09
      .          .
203     .          DCUB09.....
      .          V
207     .          RUB09
      .          V
212     .          .          UB-07
      .          .
218     .          CUB07.....
      .          .
222     .          .          UB-08C
      .          .          V
228     .          .          RUB08C
      .          .          V
233     .          .          UB-08B
      .          .          V
      .          .          V
```

```

239      .      .      .      RUB08B
      .      .      .      .
244      .      .      .      .      UB-08A
      .      .      .      .
250      .      CUB08.....
      .      V
      .      V
254      .      RUB07
      .      .
259      .      .      OFF15
      .      .
265      .      COFF15.....
      .      V
      .      V
267      .      RON15
      .      .
272      .      .      ON15
      .      .
278      .      .      .      ON05
      .      .      V
      .      .      V
284      .      .      .      D1
      .      .      .
289      .      .      .      .      ON20
      .      .      .      .
295      .      CPON15.....
      .      .
297      .      .      OFF05
      .      .      V
      .      .      V
303      .      .      RON05
    
```

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 01MAR18 TIME 09:12:43 *
*****
    
```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
    
```

Flood Control District of Maricopa County
 ESTATES AT HAYDEN PR - Estates at Hayden PR
 100 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 08/01/2017

NORTH SCOTTSDALE 100-YR FLOODPLAIN DELINEATION STUDY
 MAY 2002

FILE NAME: NS.DAT
 THIS MODEL REPRESENTS THE FUTURE LAND USE CONDITIONS OF THE WATERSHEDS.
 THE TOTAL DRAINAGE AREA FOR THE ENTIRE STUDY IS 15.2 SQUARE MILES.

100-YEAR, 6-HOUR FREQUENCY

METHODOLOGY:
 THE US CORPS OF ENGINEERS FLOOD HYROLOGY MODEL HEC-1, DATED JUNE 1998,
 VERSION 4.1
 MULTIPLE STORMS
 CLARK UNIT HYDROGRAPH
 GREEN AMPT LOSS METHOD
 NORMAL DEPTH BASIN AND REACH ROUTING METHOD

LAND USES PER CITY OF SCOTTSDALE PROVIDED GIS

PROJECT WASHES INCLUDE: STAGE COACH PASS (SCP)
 UPPER BOULDERS WASH (UB)
 FAN 6C (F6C)
 FAN 6A NORTH (F6AN)
 FAN 6A SOULTH (F6AS)
 FAN 6A (F6A)
 UPPER FAN 5 (UF5)

PROJECT SUB-BASINS DESIGNATED AS SUB-BASIN NAME ABBREVIATION FOLLOWED BY BASIN
 NUMBER. I.E. STAGE COACH PASS SUB-BASIN 14 IS DESIGNATED AS SCP-14.

PROJECT CONCENTRATION POINTS DESIGNATED AS C FOLLOWED BY SUB-BASIN NAME AND
 CONCENTRATION POINT NUMBER. CONCENTRATION POINT NUMBER IS CHOSEN BY THE SUB

WARNING EXCESS AT PONDING LESS THAN ZERO FOR PERIOD. EXCESS SET TO ZERO

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

+	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	F6C-6	1251.	4.18	210.	53.	38.	.89		
+	ROUTED TO	RF6C6	1236.	4.25	210.	53.	38.	.89		
+	HYDROGRAPH AT	F6C-4	65.	4.05	6.	2.	1.	.03		
+	2 COMBINED AT	CF6C4	1261.	4.25	215.	54.	39.	.91		
+	HYDROGRAPH AT	F6C-5	222.	4.12	34.	9.	6.	.14		
+	2 COMBINED AT	CF6C5	1399.	4.23	246.	62.	45.	1.05		
+	ROUTED TO	RF6C4	1317.	4.43	246.	62.	45.	1.05		
+	HYDROGRAPH AT	F6C-3	103.	4.07	12.	3.	2.	.05		
+	2 COMBINED AT	CF6C3	1346.	4.43	256.	64.	46.	1.10		
+	ROUTED TO	RF6C3	1324.	4.55	256.	64.	46.	1.10		
+	HYDROGRAPH AT	F6C-2	248.	4.12	25.	6.	5.	.09		
+	2 COMBINED AT	CF6C2	1369.	4.53	278.	70.	51.	1.18		
+	ROUTED TO	SF6C2	1369.	4.53	278.	70.	51.	1.18		
+	DIVERSION TO	DCUB09	24.	4.53	1.	0.	0.	1.18		
+	HYDROGRAPH AT	DCF6C2	1345.	4.53	277.	70.	50.	1.18		
+	HYDROGRAPH AT	UB-12	146.	4.12	15.	4.	3.	.06		
+	ROUTED TO	RUB12	112.	4.42	15.	4.	3.	.06		
+	HYDROGRAPH AT	UB-10	206.	4.15	25.	6.	5.	.09		
+	2 COMBINED AT	CUB10	238.	4.32	40.	10.	7.	.15		
+	HYDROGRAPH AT	UB-11	508.	4.15	57.	14.	10.	.21		
+	2 COMBINED AT	CUB11	735.	4.15	97.	24.	18.	.36		
+	ROUTED TO	RUB10	691.	4.27	97.	24.	18.	.36		
+	HYDROGRAPH AT	UB-09	848.	4.10	94.	24.	17.	.34		
+	2 COMBINED AT	CUB09	1270.	4.18	188.	47.	34.	.70		
+	ROUTED TO	SUB09	1270.	4.18	188.	47.	34.	.70		
+	HYDROGRAPH AT	DCUB09	24.	4.53	1.	0.	0.	1.18		
+	2 COMBINED AT	DCUB09	1270.	4.18	190.	48.	34.	.70		
+	ROUTED TO	RUB09	1217.	4.35	190.	48.	34.	.70		
+	HYDROGRAPH AT	UB-07	496.	4.15	53.	13.	10.	.19		

+	2 COMBINED AT	CUB07	1459.	4.32	239.	60.	43.	.89
	HYDROGRAPH AT	UB-08C	32.	4.05	3.	1.	1.	.01
	ROUTED TO	RUB08C	16.	4.60	3.	1.	1.	.01
+	HYDROGRAPH AT	UB-08B	108.	4.07	10.	2.	2.	.03
	ROUTED TO	RUB08B	75.	4.43	10.	2.	2.	.03
+	HYDROGRAPH AT	UB-08A	238.	4.10	30.	8.	5.	.12
+	4 COMBINED AT	CUB08	1629.	4.32	278.	70.	51.	1.06
	ROUTED TO	RUB07	1626.	4.35	278.	70.	51.	1.06
+	HYDROGRAPH AT	OFF15	40.	4.05	3.	1.	0.	.01
+	2 COMBINED AT	COFF15	1631.	4.33	281.	71.	51.	1.07
	ROUTED TO	RON15	1624.	4.37	280.	71.	51.	1.07
+	HYDROGRAPH AT	ON15	10.	4.18	1.	0.	0.	.01
+	HYDROGRAPH AT	ON05	20.	4.03	2.	0.	0.	.01
	ROUTED TO	D1	20.	4.05	1.	0.	0.	.01
+	HYDROGRAPH AT	ON20	6.	4.02	0.	0.	0.	.00
+	4 COMBINED AT	CPON15	1634.	4.37	283.	71.	51.	1.08
+	HYDROGRAPH AT	OFF05	30.	4.10	3.	1.	0.	.01
+	ROUTED TO	RON05	29.	4.13	3.	1.	0.	.01

*** NORMAL END OF HEC-1 ***

Appendix C – Upper Boulders Wash Hydraulics

HEC-RAS Results

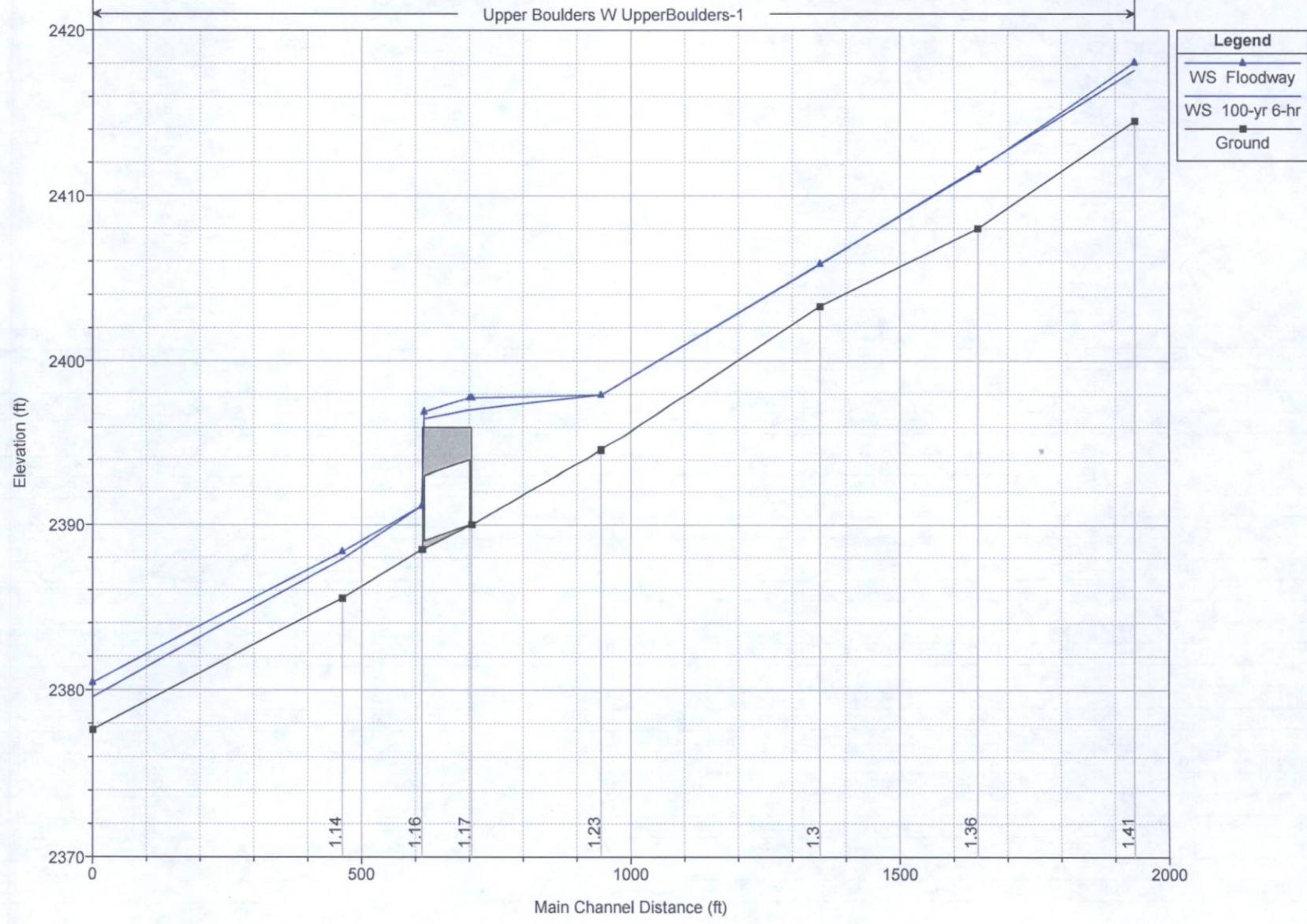
Lateral Erosion Setbacks

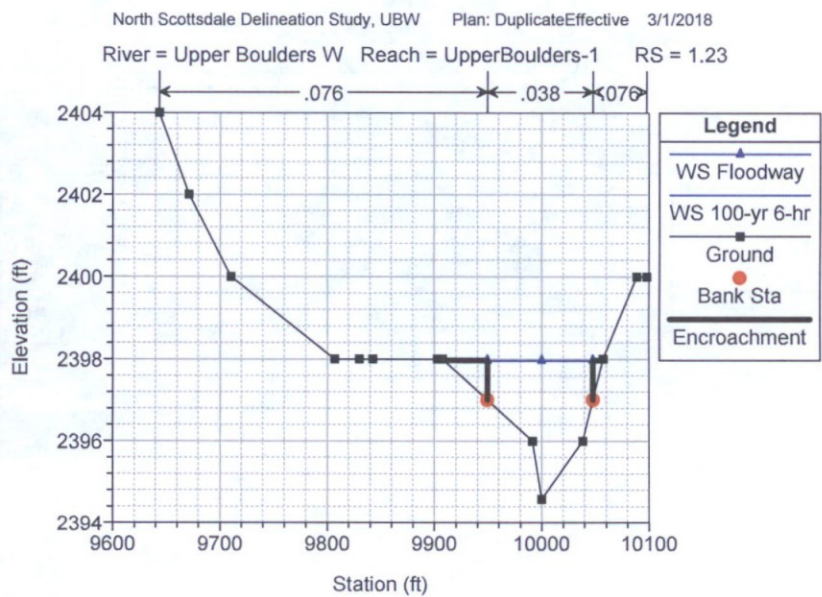
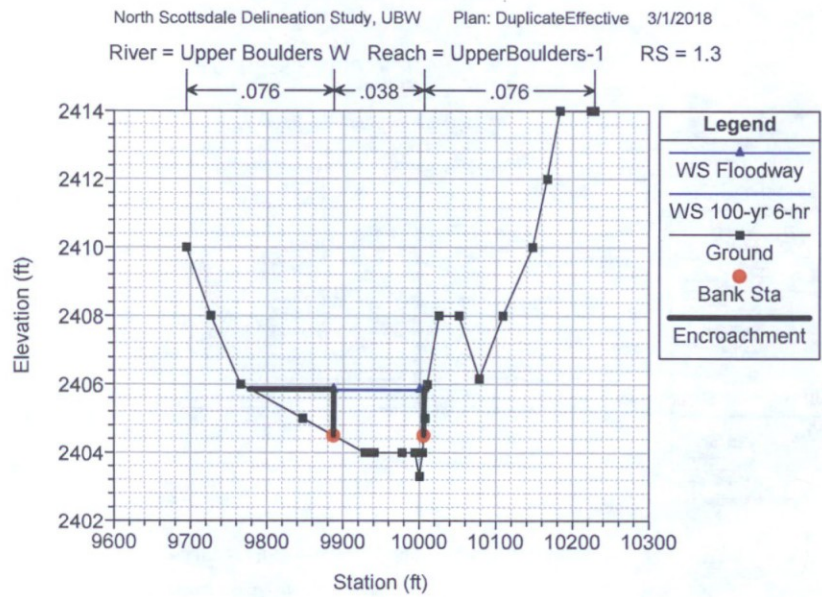
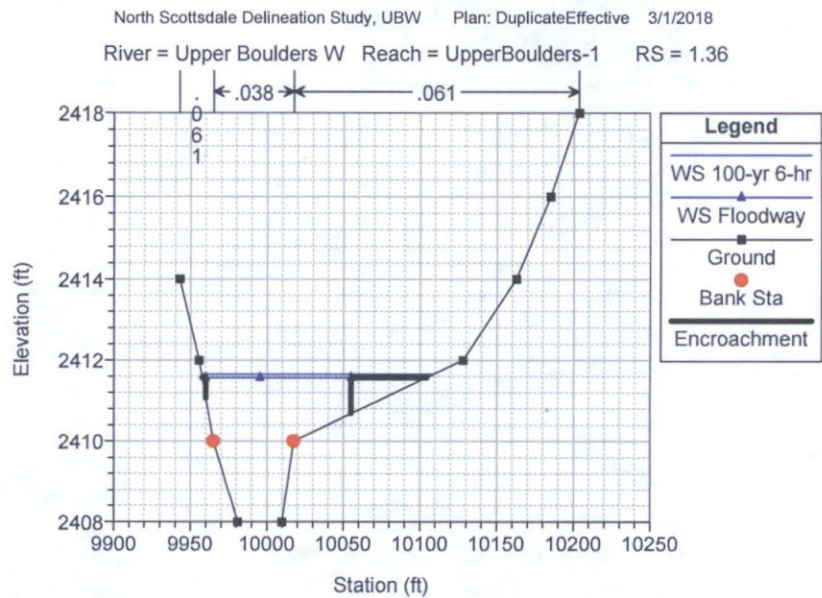
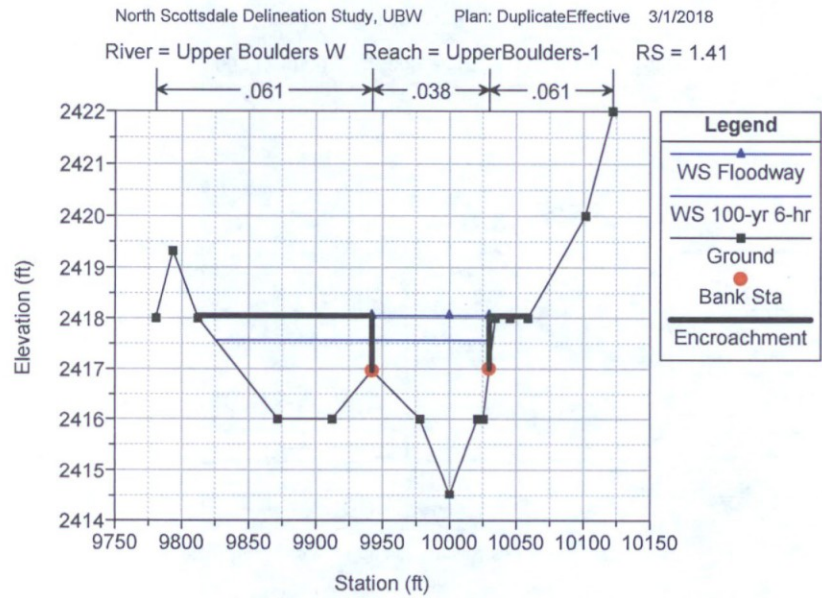
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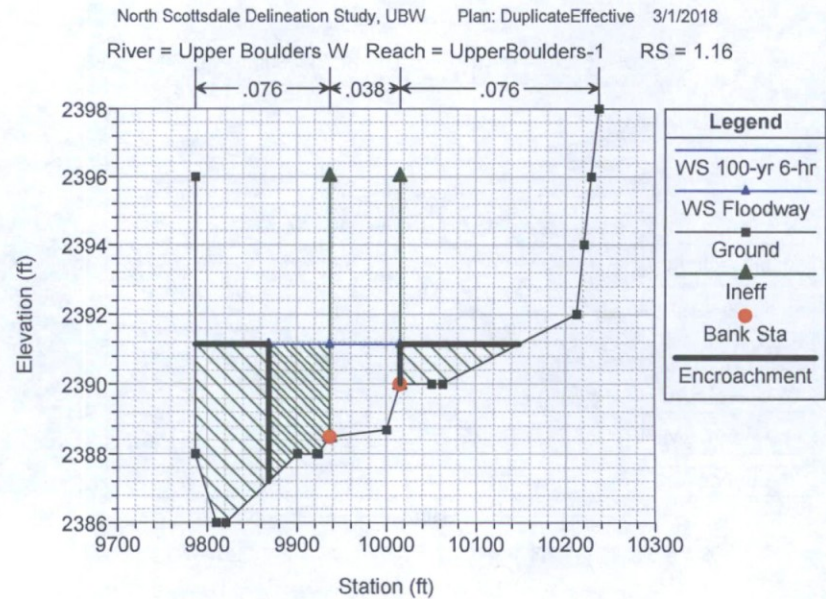
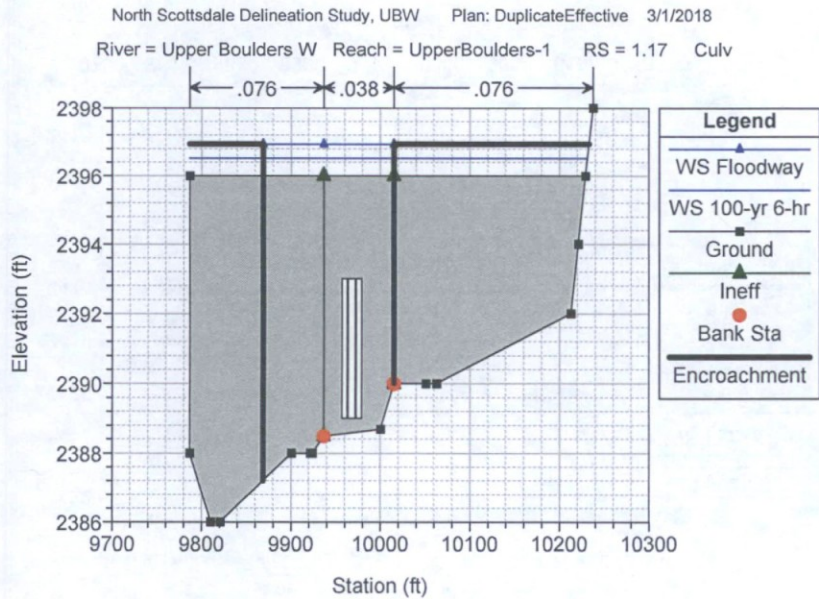
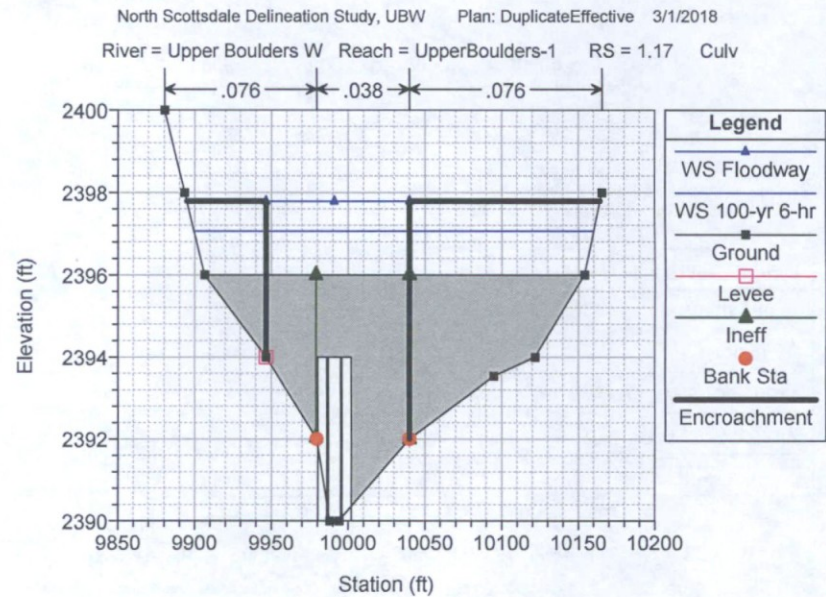
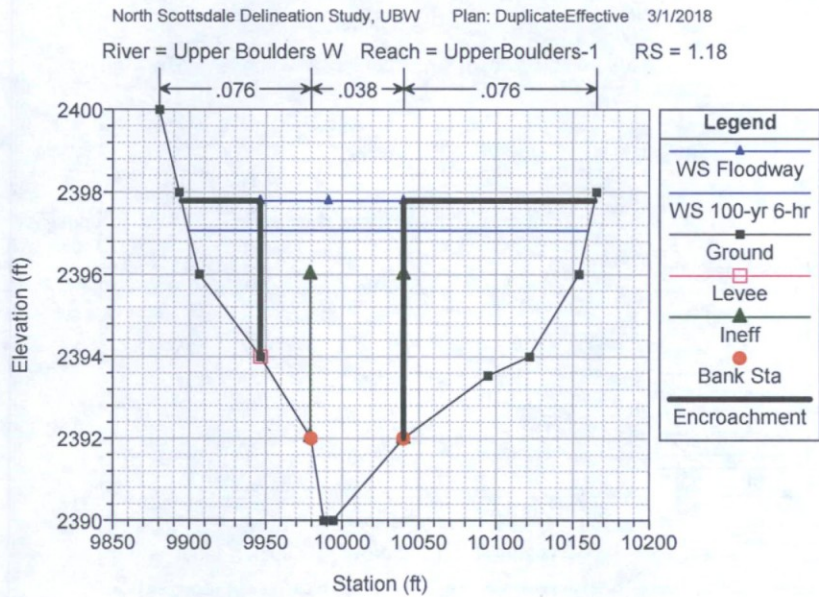
HEC-RAS Plan: DupEff River: Upper Boulders W Reach: UpperBoulders-1

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
UpperBoulders-1	1.41	100-yr 6-hr	1628.00	2414.52	2417.57	2417.57	2418.27	0.018862	7.66	282.92	207.33	1.03
UpperBoulders-1	1.41	Floodway	1628.00	2414.52	2418.05	2418.05	2419.17	0.017159	8.49	191.83	87.59	1.01
UpperBoulders-1	1.36	100-yr 6-hr	1628.00	2408.00	2411.65	2411.65	2412.68	0.010338	8.60	250.38	151.68	0.85
UpperBoulders-1	1.36	Floodway	1628.00	2408.00	2411.57	2411.57	2412.72	0.011729	9.00	215.36	95.00	0.90
UpperBoulders-1	1.3	100-yr 6-hr	1628.00	2403.30	2405.83	2405.83	2406.56	0.015708	7.15	282.35	229.62	0.95
UpperBoulders-1	1.3	Floodway	1628.00	2403.30	2405.86	2405.86	2406.77	0.018233	7.68	211.97	117.92	1.01
UpperBoulders-1	1.23	100-yr 6-hr	1628.00	2394.57	2397.96	2397.96	2398.92	0.016124	7.96	223.66	147.74	0.98
UpperBoulders-1	1.23	Floodway	1628.00	2394.57	2397.96	2397.96	2398.99	0.017159	8.12	200.44	98.25	1.00
UpperBoulders-1	1.18	100-yr 6-hr	1628.00	2390.00	2397.07	2393.74	2397.16	0.000479	2.87	988.23	260.56	0.20
UpperBoulders-1	1.18	Floodway	1628.00	2390.00	2397.79	2393.72	2397.95	0.000661	3.41	573.28	93.27	0.23
UpperBoulders-1	1.17		Culvert									
UpperBoulders-1	1.16	100-yr 6-hr	1667.00	2388.49	2391.14	2391.14	2392.36	0.015836	8.83	188.80	361.75	1.00
UpperBoulders-1	1.16	Floodway	1667.00	2388.49	2391.14	2391.14	2392.36	0.016144	8.83	188.80	146.23	1.00
UpperBoulders-1	1.14	100-yr 6-hr	1667.00	2385.50	2387.89	2387.89	2388.43	0.019929	6.85	364.28	384.35	1.03
UpperBoulders-1	1.14	Floodway	1667.00	2385.50	2388.36	2388.33	2389.24	0.016647	7.53	221.32	119.46	0.98
UpperBoulders-1	1.05	100-yr 6-hr	1667.00	2377.62	2379.57	2379.57	2379.71	0.008135	3.97	611.18	436.34	0.64
UpperBoulders-1	1.05	Floodway	1667.00	2377.62	2380.44	2380.44	2381.45	0.016817	8.07	206.49	100.03	0.99

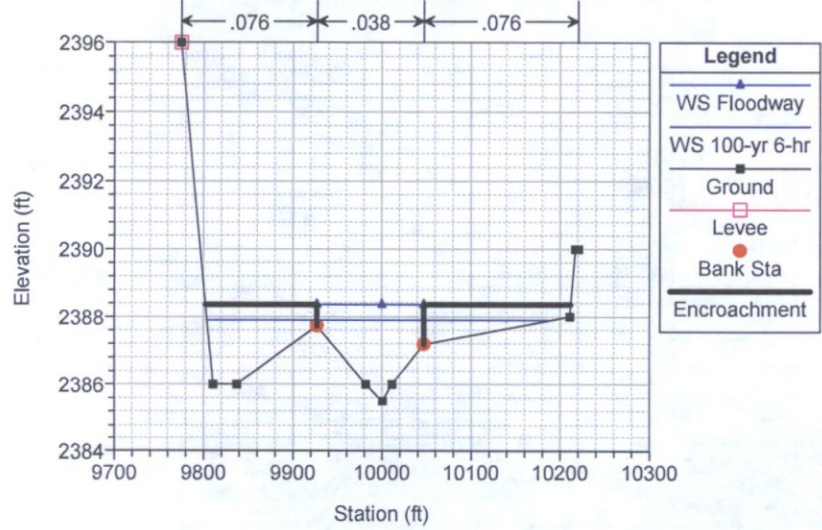
North Scottsdale Delineation Study, UBW Plan: DuplicateEffective 3/1/2018



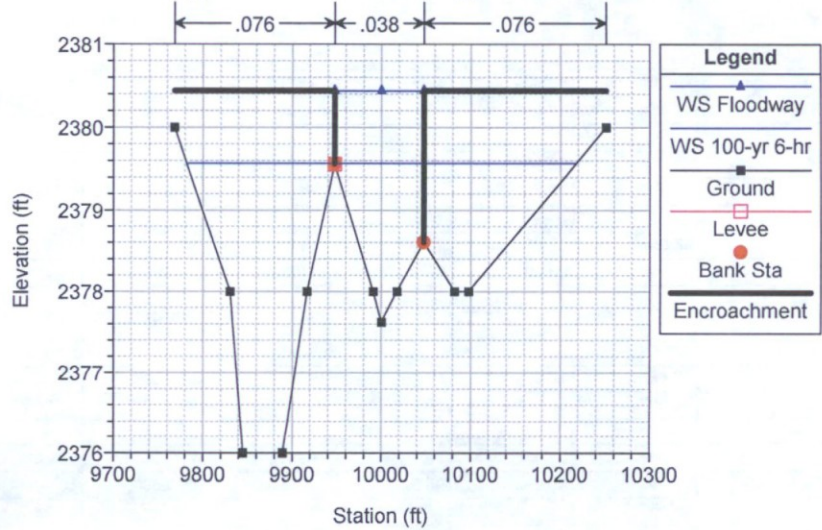




North Scottsdale Delineation Study, UBW Plan: DuplicateEffective 3/1/2018
 River = Upper Boulders W Reach = UpperBoulders-1 RS = 1.14



North Scottsdale Delineation Study, UBW Plan: DuplicateEffective 3/1/2018
 River = Upper Boulders W Reach = UpperBoulders-1 RS = 1.05

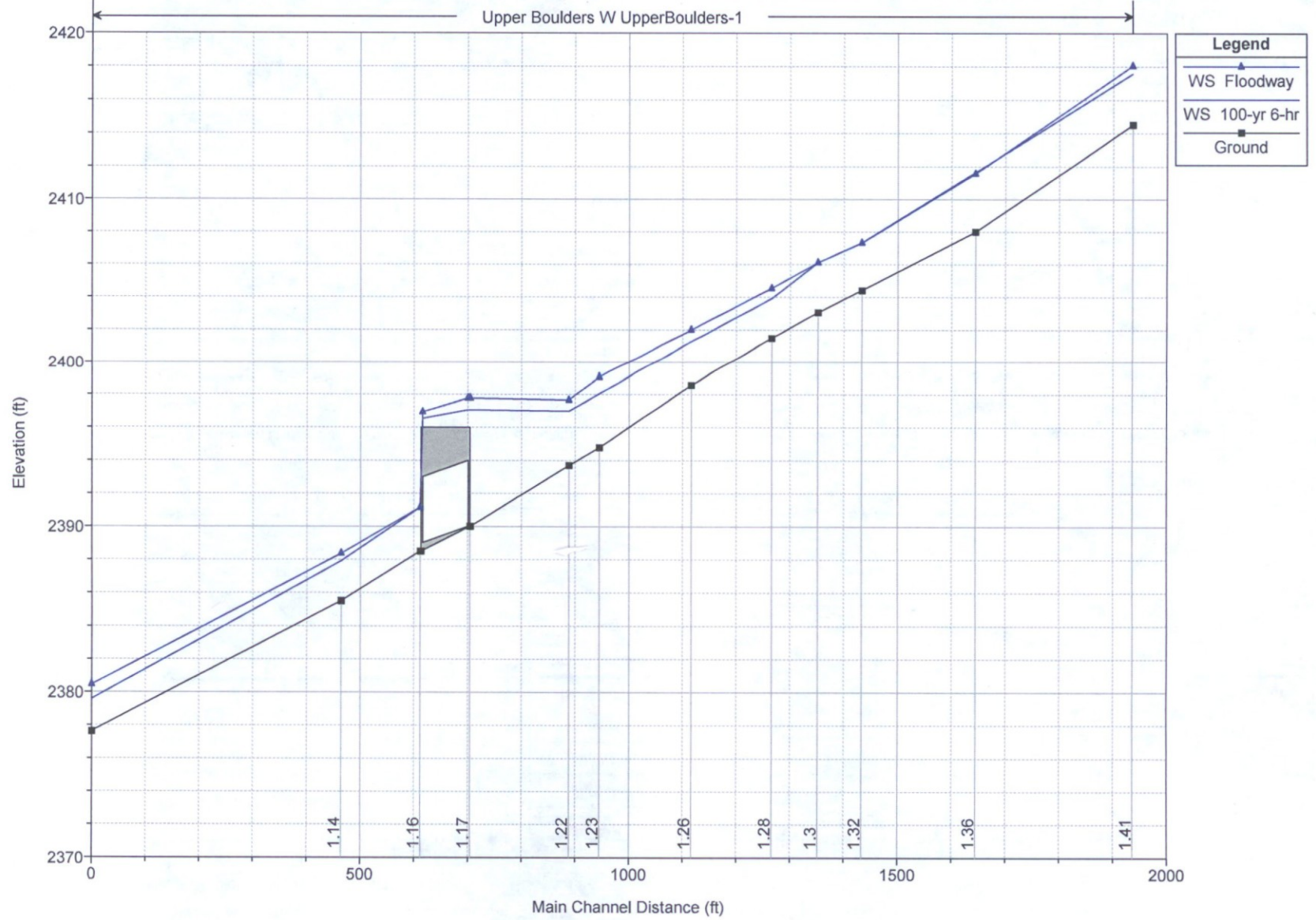


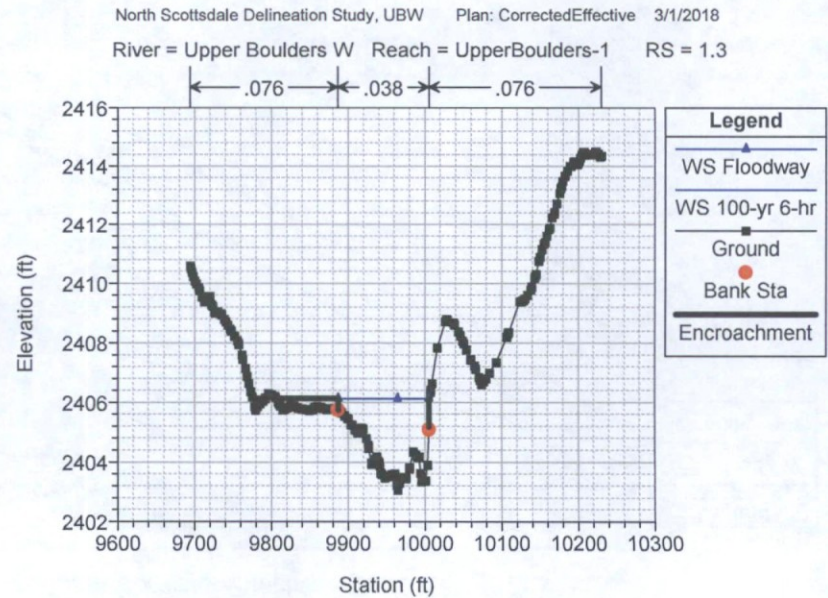
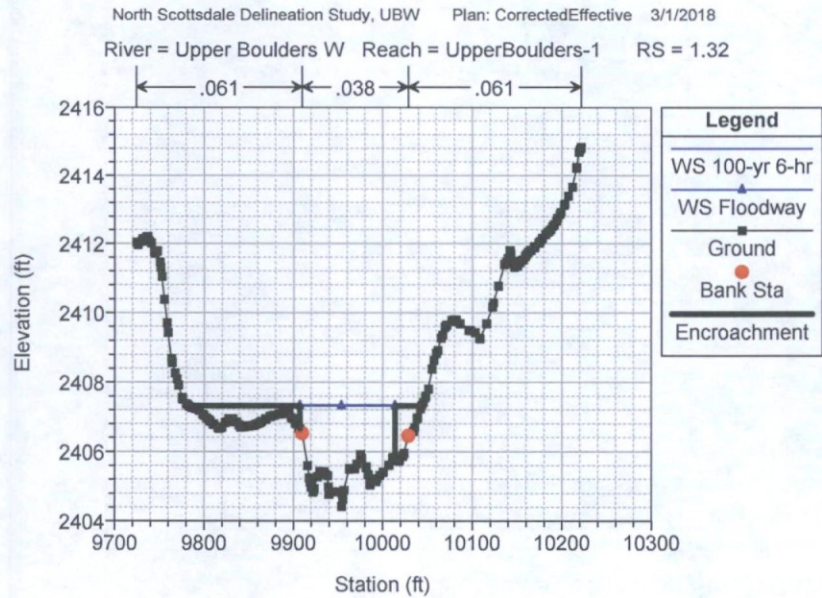
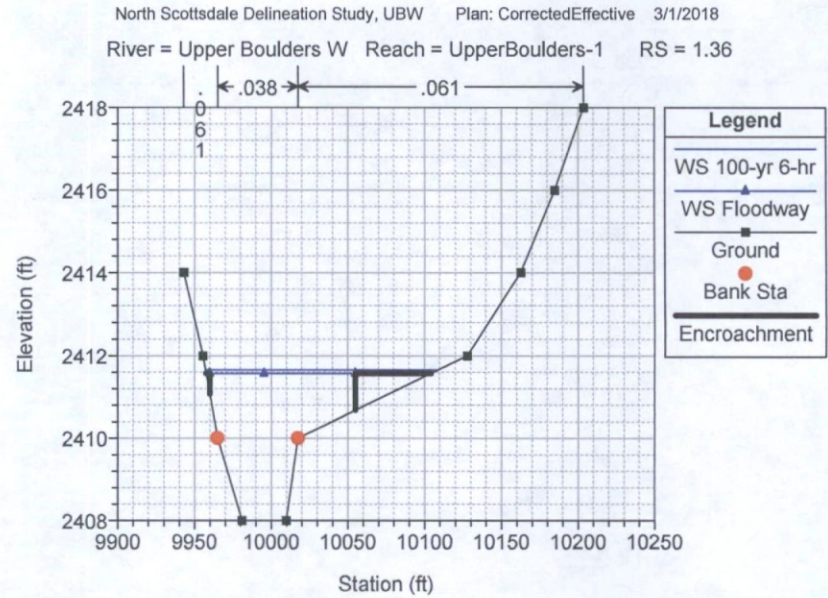
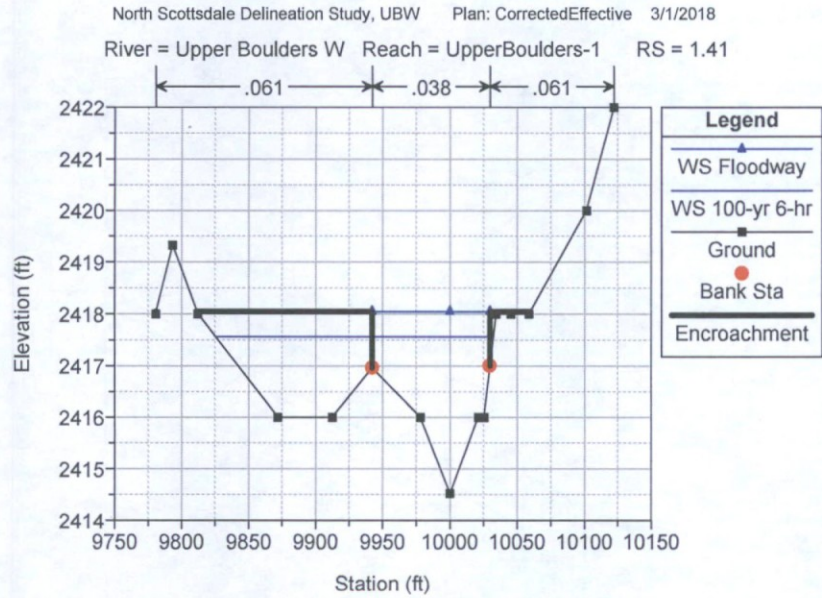
Corrected Effective Model

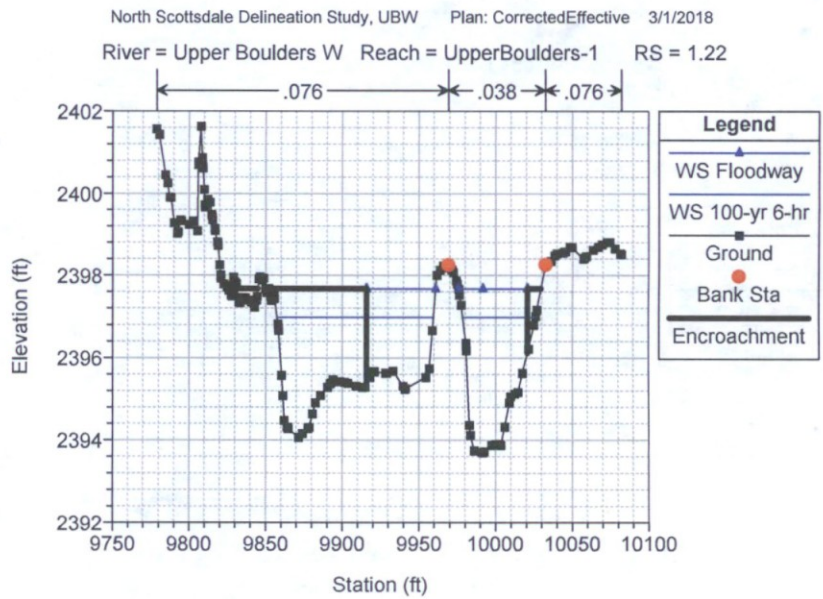
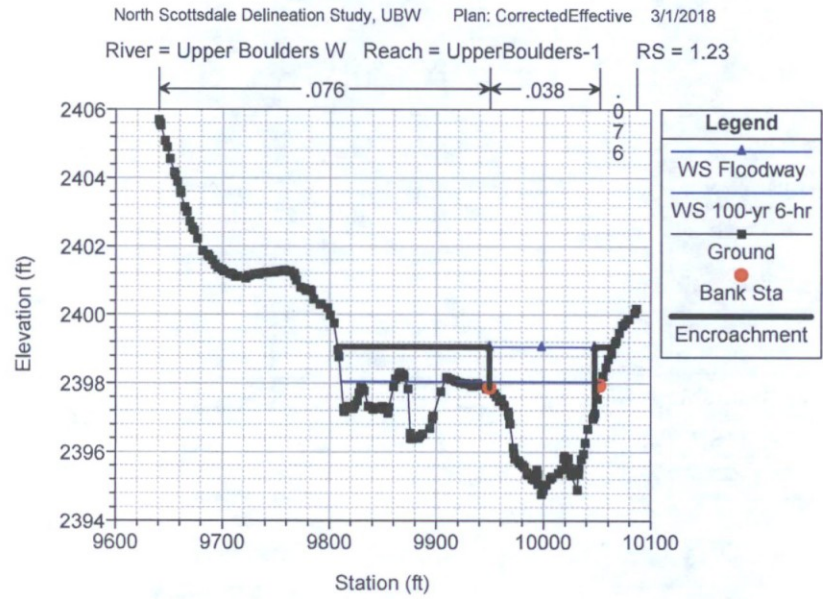
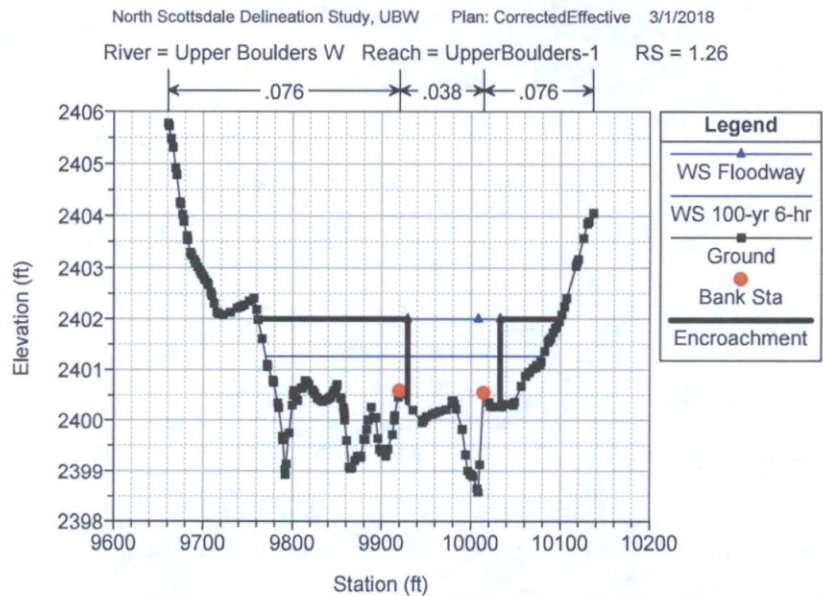
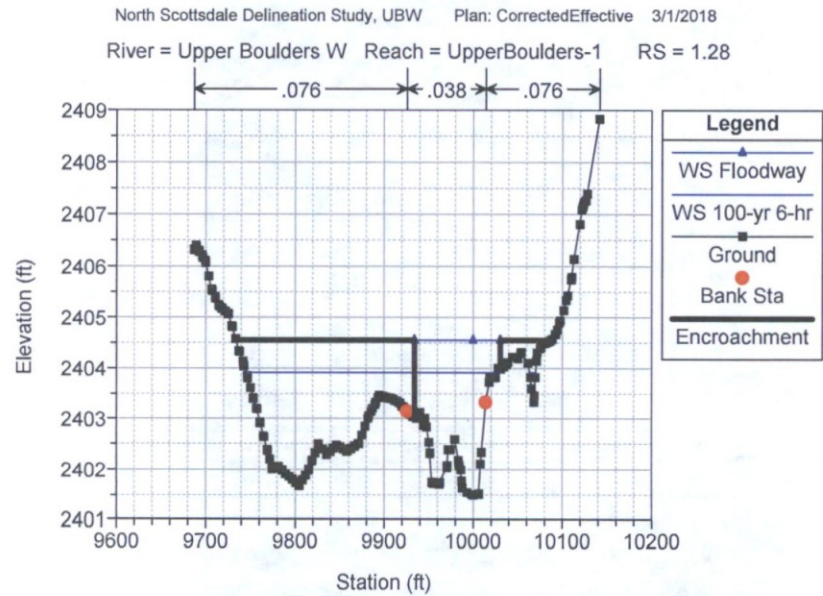
HEC-RAS Plan: CorrEff River: Upper Boulders W Reach: UpperBoulders-1

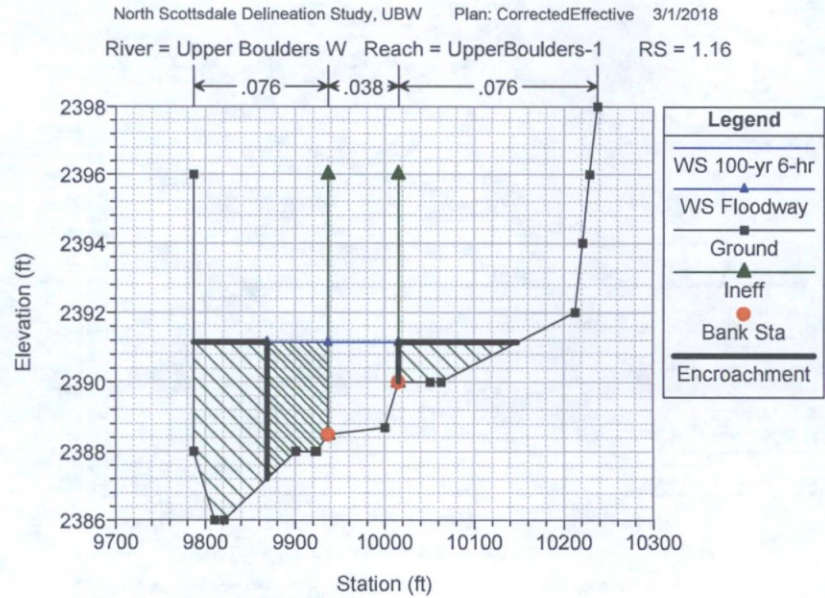
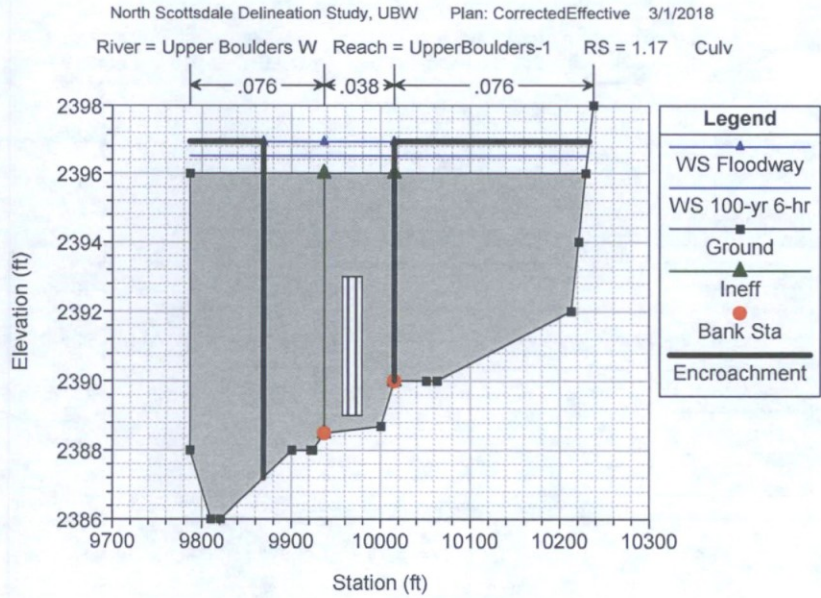
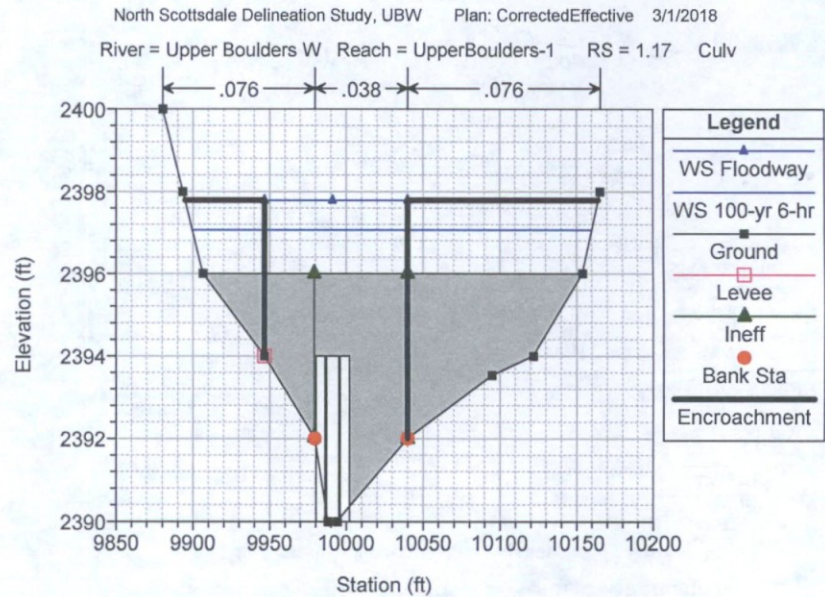
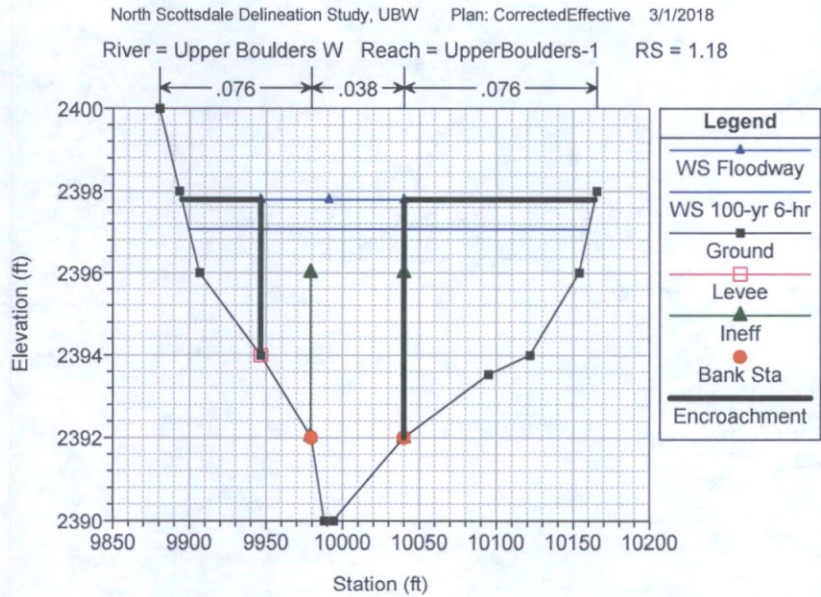
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
UpperBoulders-1	1.41	100-yr 6-hr	1628.00	2414.52	2417.57	2417.57	2418.27	0.018862	7.66	282.92	207.33	1.03
UpperBoulders-1	1.41	Floodway	1628.00	2414.52	2418.05	2418.05	2419.17	0.017159	8.49	191.83	87.59	1.01
UpperBoulders-1	1.36	100-yr 6-hr	1628.00	2408.00	2411.65	2411.65	2412.68	0.010338	8.60	250.38	151.68	0.85
UpperBoulders-1	1.36	Floodway	1628.00	2408.00	2411.57	2411.57	2412.72	0.011729	9.00	215.36	95.00	0.90
UpperBoulders-1	1.32	100-yr 6-hr	1628.00	2404.42	2407.34	2407.30	2407.99	0.011933	6.64	293.33	261.59	0.84
UpperBoulders-1	1.32	Floodway	1628.00	2404.42	2407.32	2407.28	2408.27	0.016456	7.86	208.68	106.27	0.98
UpperBoulders-1	1.3	100-yr 6-hr	1628.00	2403.07	2406.12	2406.12	2406.90	0.014651	7.17	251.15	216.91	0.92
UpperBoulders-1	1.3	Floodway	1628.00	2403.07	2406.12	2406.04	2406.95	0.015032	7.30	223.31	117.92	0.93
UpperBoulders-1	1.28	100-yr 6-hr	1628.00	2401.48	2403.91		2404.36	0.014585	6.60	384.64	287.49	0.90
UpperBoulders-1	1.28	Floodway	1628.00	2401.48	2404.55	2404.55	2405.63	0.015050	8.43	202.82	96.40	0.96
UpperBoulders-1	1.26	100-yr 6-hr	1628.00	2398.58	2401.26	2401.26	2401.82	0.024684	7.41	345.23	310.55	1.13
UpperBoulders-1	1.26	Floodway	1628.00	2398.58	2402.00	2402.00	2403.04	0.017658	8.44	211.46	103.76	1.02
UpperBoulders-1	1.23	100-yr 6-hr	1628.00	2394.78	2398.04	2398.01	2398.79	0.013658	7.22	277.74	220.21	0.90
UpperBoulders-1	1.23	Floodway	1628.00	2394.78	2399.06		2399.51	0.004367	5.38	302.75	98.25	0.54
UpperBoulders-1	1.22	100-yr 6-hr	1628.00	2393.69	2396.98	2396.97	2397.80	0.018862	8.95	283.03	149.51	1.06
UpperBoulders-1	1.22	Floodway	1628.00	2393.69	2397.67	2397.67	2398.85	0.015551	9.63	225.24	90.05	0.99
UpperBoulders-1	1.18	100-yr 6-hr	1628.00	2390.00	2397.07	2393.74	2397.16	0.000479	2.87	988.23	260.56	0.20
UpperBoulders-1	1.18	Floodway	1628.00	2390.00	2397.79	2393.72	2397.95	0.000661	3.41	573.28	93.27	0.23
UpperBoulders-1	1.17		Culvert									
UpperBoulders-1	1.16	100-yr 6-hr	1667.00	2388.49	2391.14	2391.14	2392.36	0.015836	8.83	188.80	361.75	1.00
UpperBoulders-1	1.16	Floodway	1667.00	2388.49	2391.14	2391.14	2392.36	0.016144	8.83	188.80	146.23	1.00
UpperBoulders-1	1.14	100-yr 6-hr	1667.00	2385.50	2387.89	2387.89	2388.43	0.019929	6.85	364.28	384.35	1.03
UpperBoulders-1	1.14	Floodway	1667.00	2385.50	2388.36	2388.33	2389.24	0.016647	7.53	221.32	119.46	0.98
UpperBoulders-1	1.05	100-yr 6-hr	1667.00	2377.62	2379.57	2379.57	2379.71	0.008135	3.97	611.18	436.34	0.64
UpperBoulders-1	1.05	Floodway	1667.00	2377.62	2380.44	2380.44	2381.45	0.016817	8.07	206.49	100.03	0.99

North Scottsdale Delineation Study, UBW Plan: CorrectedEffective 3/1/2018

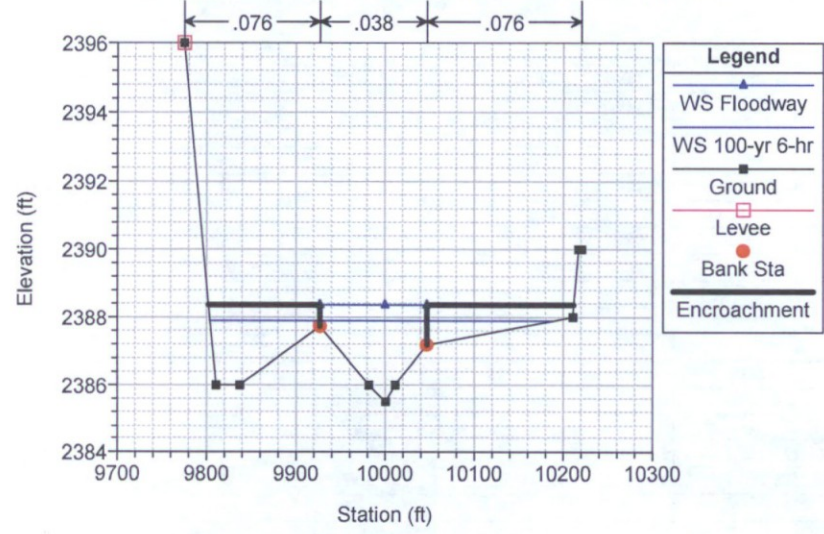




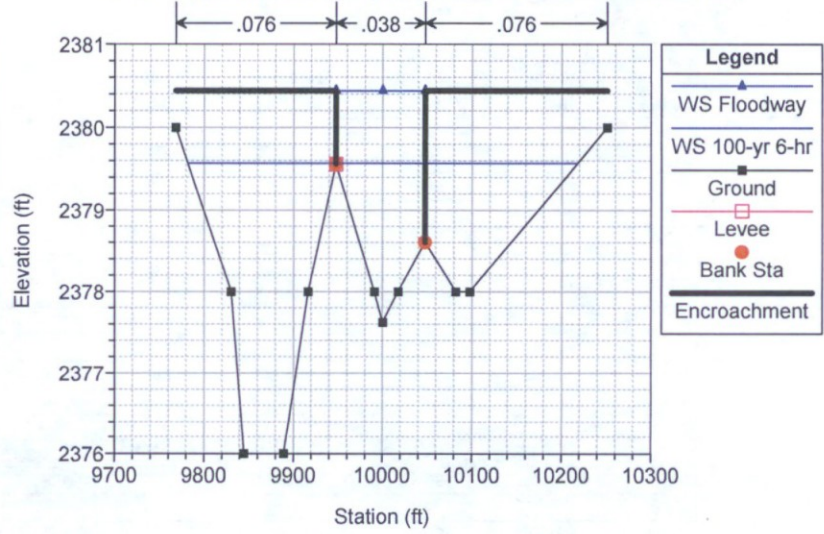




North Scottsdale Delineation Study, UBW Plan: CorrectedEffective 3/1/2018
 River = Upper Boulders W Reach = UpperBoulders-1 RS = 1.14



North Scottsdale Delineation Study, UBW Plan: CorrectedEffective 3/1/2018
 River = Upper Boulders W Reach = UpperBoulders-1 RS = 1.05

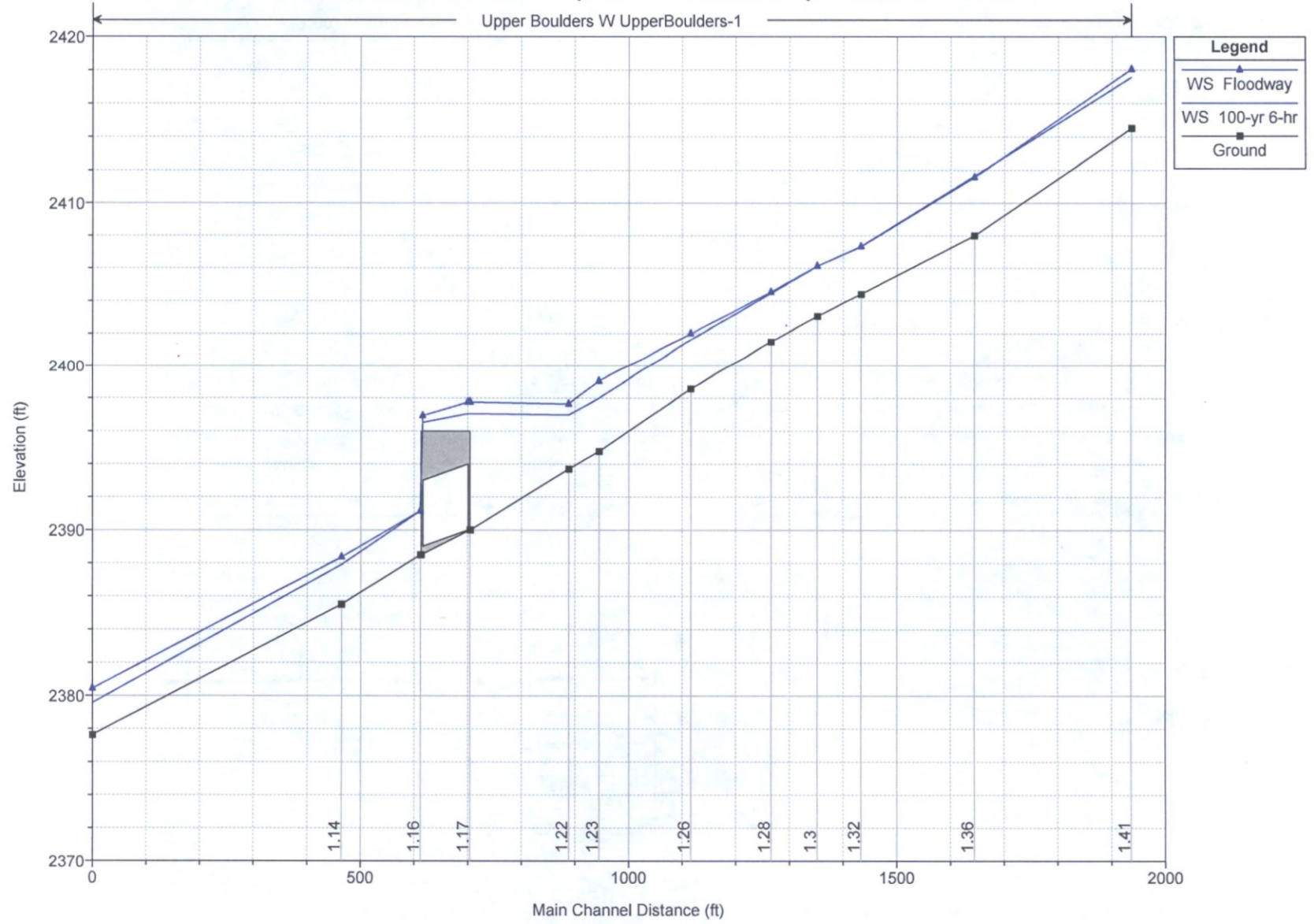


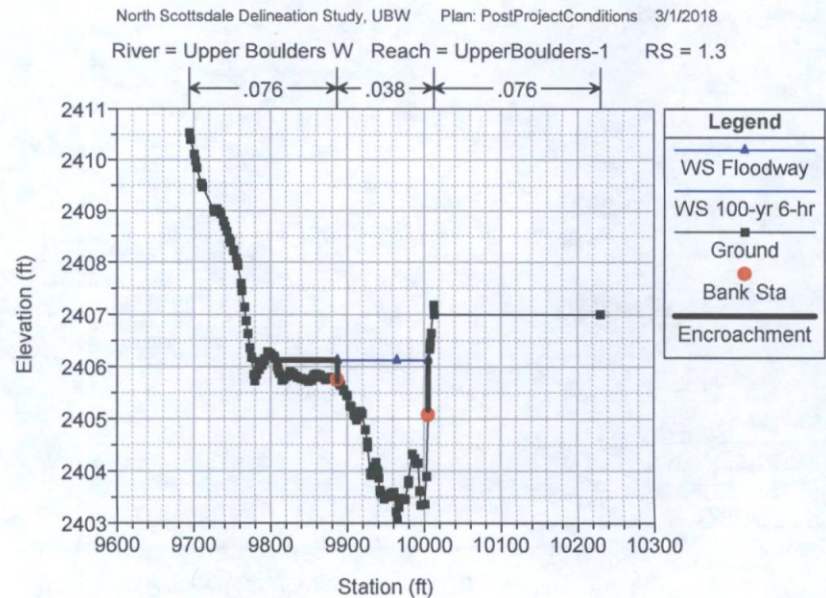
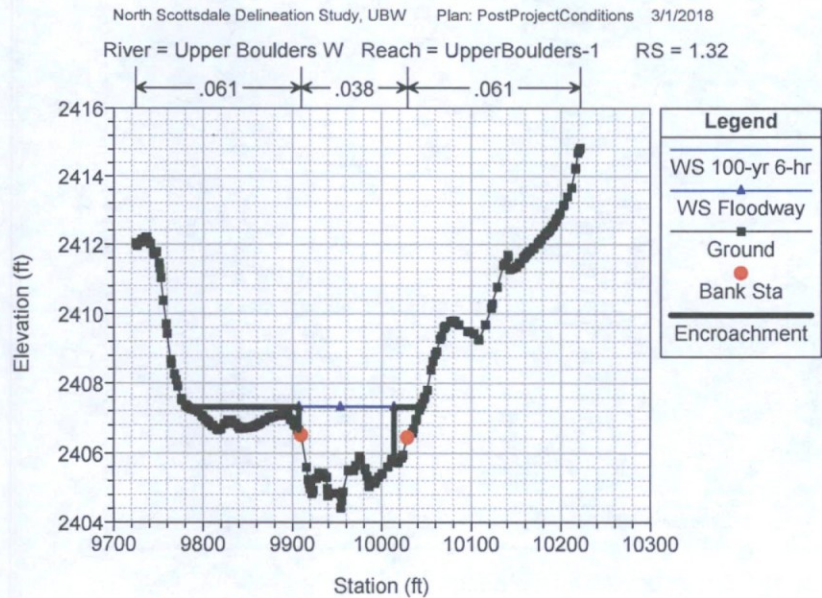
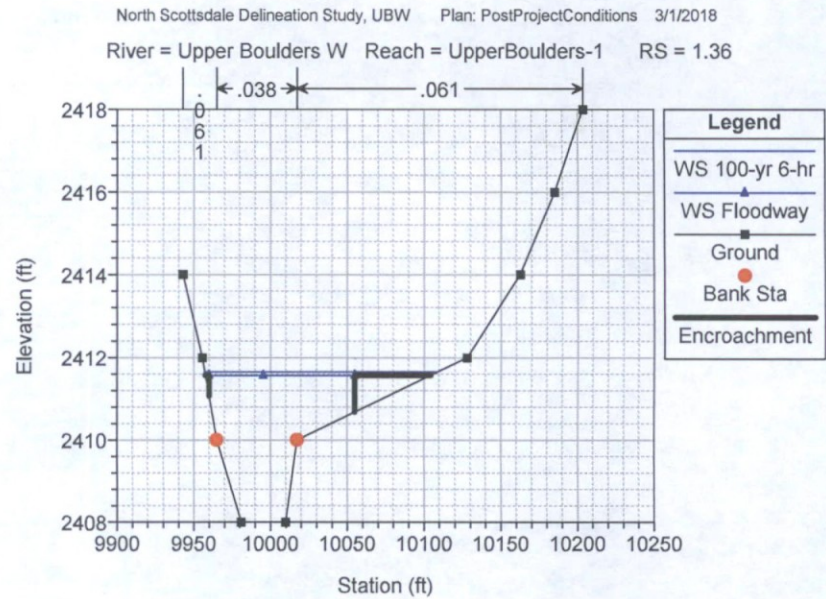
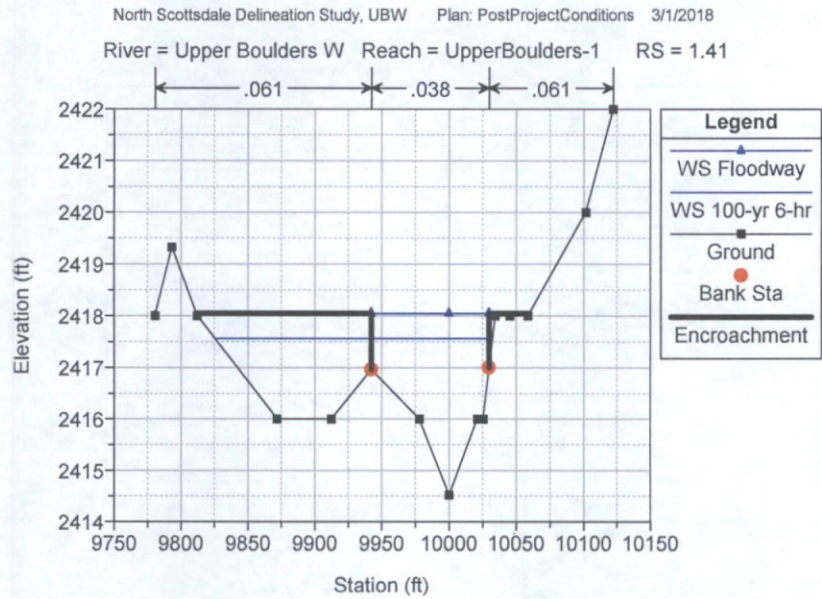
Post-Project Model

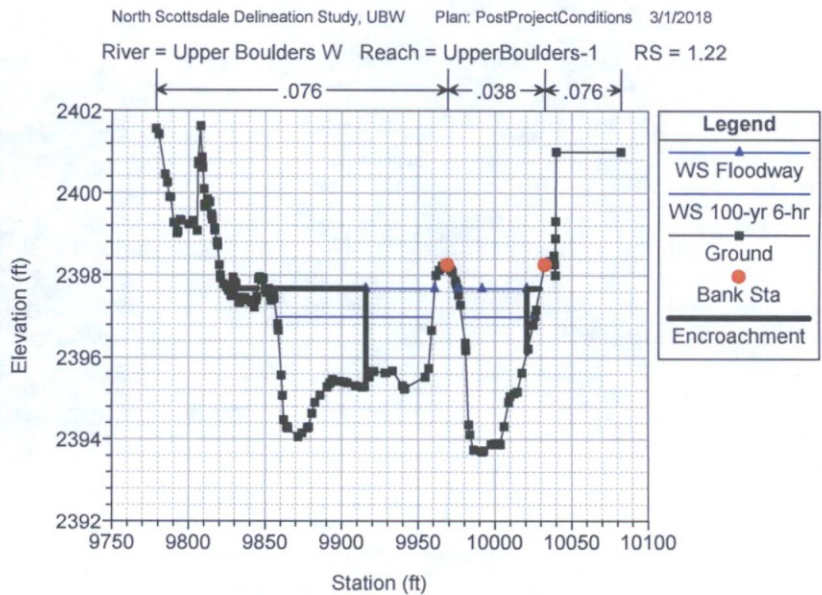
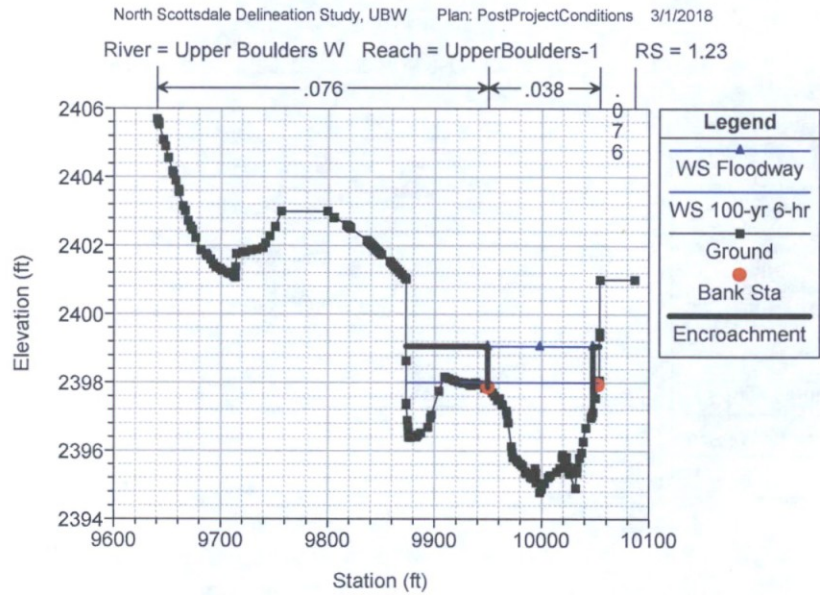
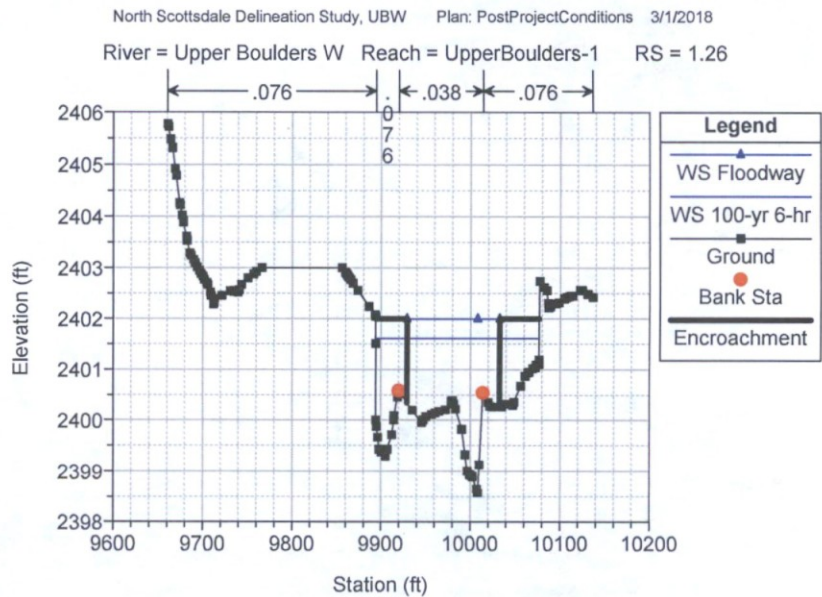
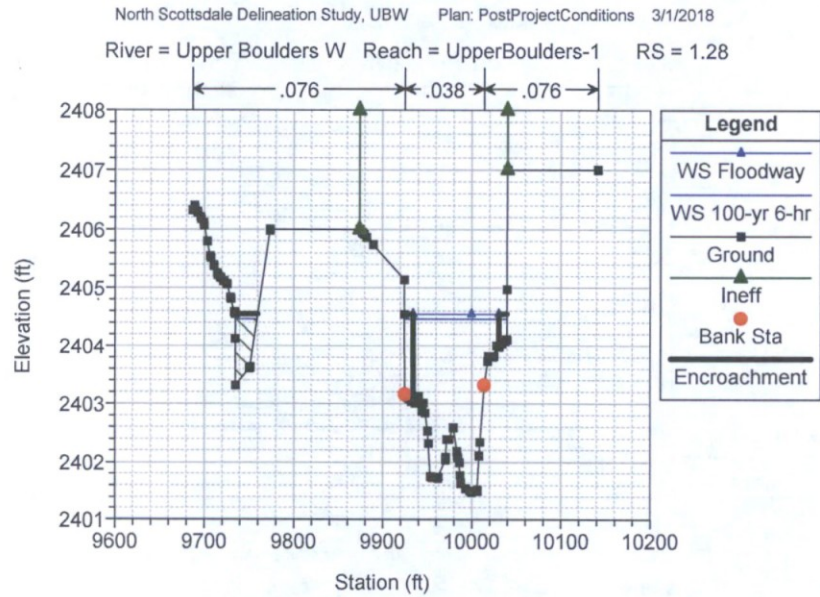
HEC-RAS Plan: PPM River: Upper Boulders W Reach: UpperBoulders-1

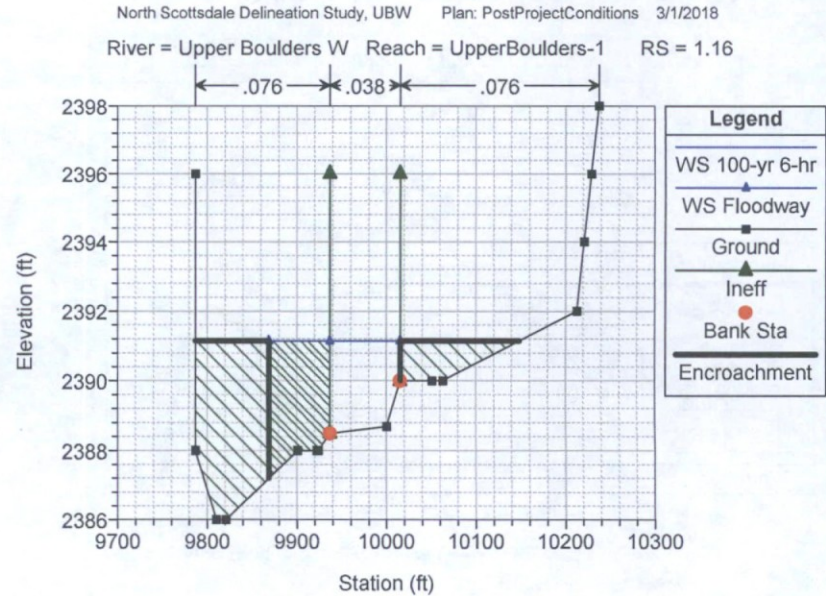
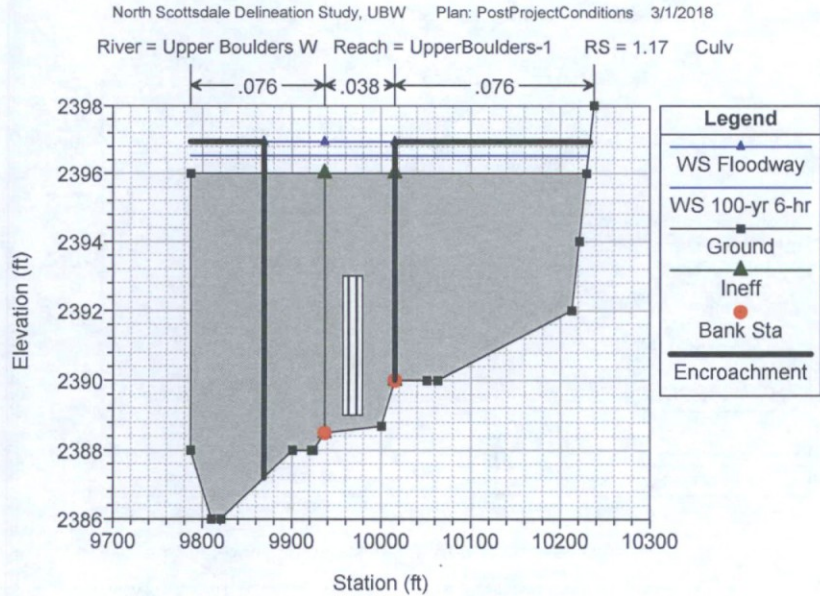
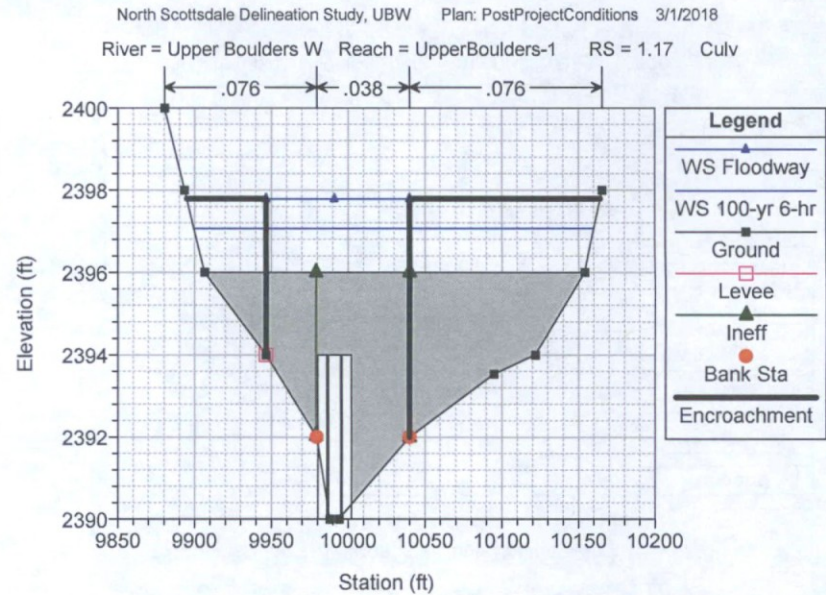
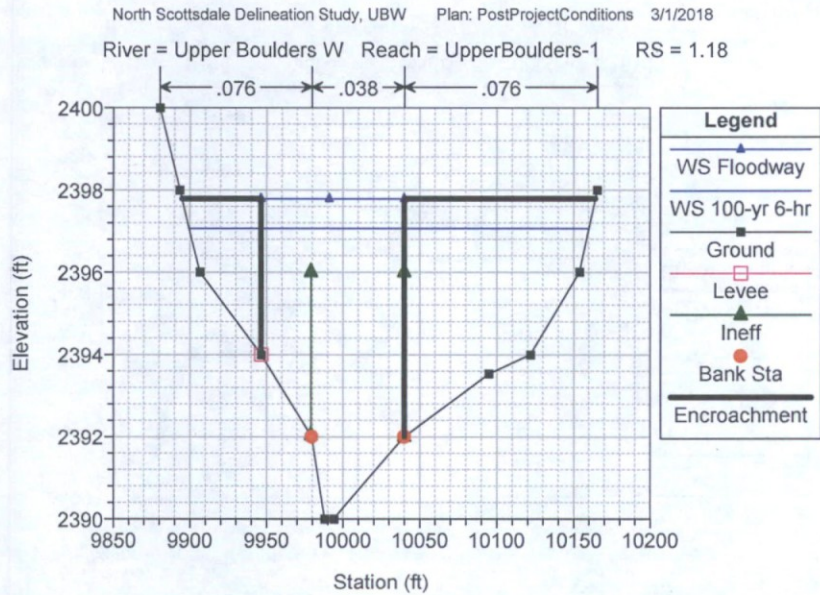
Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
UpperBoulders-1	1.41	100-yr 6-hr	1628.00	2414.52	2417.57	2417.57	2418.27	0.018862	7.66	282.92	207.33	1.03
UpperBoulders-1	1.41	Floodway	1628.00	2414.52	2418.05	2418.05	2419.17	0.017159	8.49	191.83	87.59	1.01
UpperBoulders-1	1.36	100-yr 6-hr	1628.00	2408.00	2411.65	2411.65	2412.68	0.010338	8.60	250.38	151.68	0.85
UpperBoulders-1	1.36	Floodway	1628.00	2408.00	2411.57	2411.57	2412.72	0.011729	9.00	215.36	95.00	0.90
UpperBoulders-1	1.32	100-yr 6-hr	1628.00	2404.42	2407.34	2407.30	2407.99	0.012010	6.65	292.50	261.42	0.84
UpperBoulders-1	1.32	Floodway	1628.00	2404.42	2407.32	2407.28	2408.27	0.016456	7.86	208.68	106.27	0.98
UpperBoulders-1	1.3	100-yr 6-hr	1628.00	2403.07	2406.12	2406.12	2406.90	0.014601	7.16	251.36	216.98	0.92
UpperBoulders-1	1.3	Floodway	1628.00	2403.07	2406.12	2406.03	2406.95	0.015024	7.29	223.34	117.92	0.93
UpperBoulders-1	1.28	100-yr 6-hr	1628.00	2401.48	2404.47	2404.47	2405.48	0.015195	8.15	211.95	139.73	0.97
UpperBoulders-1	1.28	Floodway	1628.00	2401.48	2404.55	2404.55	2405.63	0.015050	8.43	202.82	96.40	0.96
UpperBoulders-1	1.26	100-yr 6-hr	1628.00	2398.58	2401.62	2401.62	2402.37	0.019823	7.76	274.35	182.72	1.06
UpperBoulders-1	1.26	Floodway	1628.00	2398.58	2402.00	2402.00	2403.04	0.017658	8.44	211.46	103.76	1.02
UpperBoulders-1	1.23	100-yr 6-hr	1628.00	2394.78	2398.00	2397.99	2398.85	0.015546	7.60	240.40	161.74	0.96
UpperBoulders-1	1.23	Floodway	1628.00	2394.78	2399.06		2399.51	0.004367	5.38	302.75	98.25	0.54
UpperBoulders-1	1.22	100-yr 6-hr	1628.00	2393.69	2396.98	2396.97	2397.80	0.018869	8.95	282.99	149.50	1.06
UpperBoulders-1	1.22	Floodway	1628.00	2393.69	2397.67	2397.67	2398.85	0.015551	9.63	225.24	90.05	0.99
UpperBoulders-1	1.18	100-yr 6-hr	1628.00	2390.00	2397.07	2393.74	2397.16	0.000479	2.87	988.23	260.56	0.20
UpperBoulders-1	1.18	Floodway	1628.00	2390.00	2397.79	2393.72	2397.95	0.000661	3.41	573.28	93.27	0.23
UpperBoulders-1	1.17		Culvert									
UpperBoulders-1	1.16	100-yr 6-hr	1667.00	2388.49	2391.14	2391.14	2392.36	0.015836	8.83	188.80	361.75	1.00
UpperBoulders-1	1.16	Floodway	1667.00	2388.49	2391.14	2391.14	2392.36	0.016144	8.83	188.80	146.23	1.00
UpperBoulders-1	1.14	100-yr 6-hr	1667.00	2385.50	2387.89	2387.89	2388.43	0.019929	6.85	364.28	384.35	1.03
UpperBoulders-1	1.14	Floodway	1667.00	2385.50	2388.36	2388.33	2389.24	0.016647	7.53	221.32	119.46	0.98
UpperBoulders-1	1.05	100-yr 6-hr	1667.00	2377.62	2379.57	2379.57	2379.71	0.008135	3.97	611.18	436.34	0.64
UpperBoulders-1	1.05	Floodway	1667.00	2377.62	2380.44	2380.44	2381.45	0.016817	8.07	206.49	100.03	0.99

North Scottsdale Delineation Study, UBW Plan: PostProjectConditions 3/1/2018



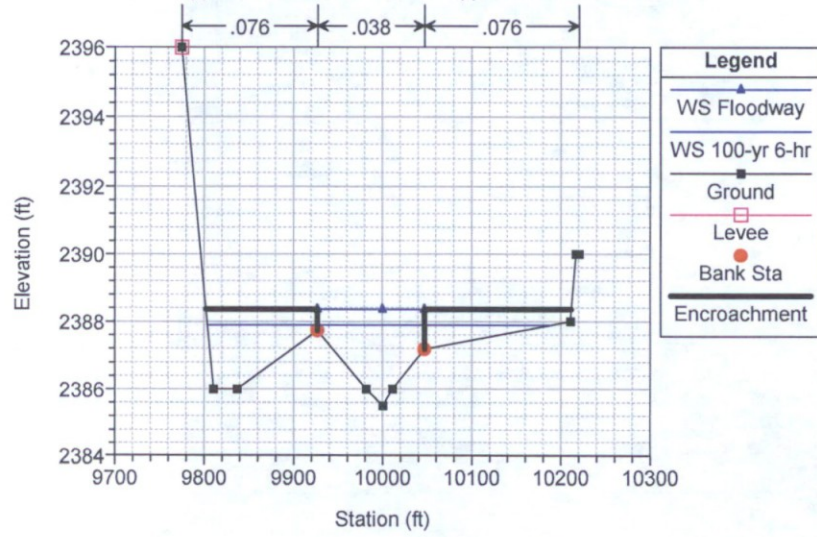






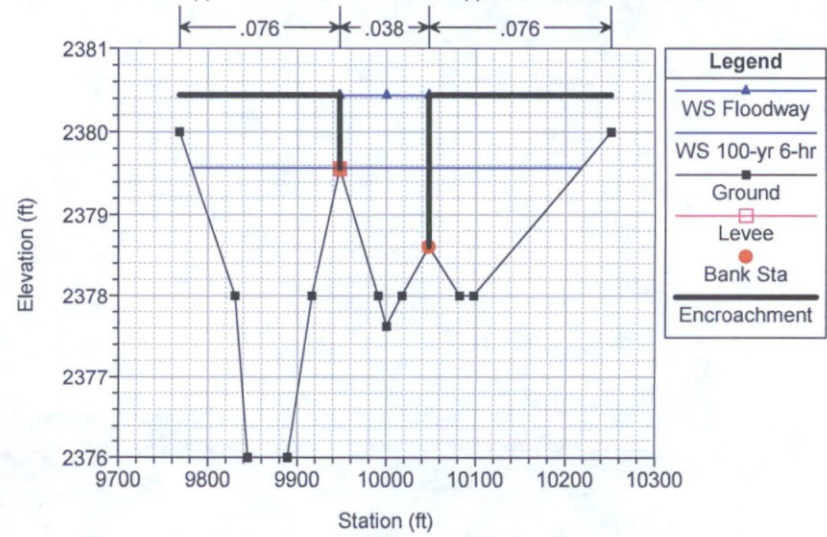
North Scottsdale Delineation Study, UBW Plan: PostProjectConditions 3/1/2018

River = Upper Boulders W Reach = UpperBoulders-1 RS = 1.14



North Scottsdale Delineation Study, UBW Plan: PostProjectConditions 3/1/2018

River = Upper Boulders W Reach = UpperBoulders-1 RS = 1.05



Upper Boulders Wash

$$Q_{100} \text{ (from FIS)} = 1628 \text{ cfs}$$

$$\text{Setback} = 1.0 * Q_{100}^{0.5}$$

$$\text{Setback} = 1628^{0.5} = \boxed{40 \text{ ft}}$$

Appendix D – Stormwater Storage



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA - - ZN - - UP - - DR - - PP - PC#

The applicant/developer must complete and submit this form to the city for processing and obtain approval of waiver request **before submitting improvement plans**. Denial of the waiver may require the developer to submit a revised site plan to the Development Review Board.

Date 3/1/2018 Project Name Estates at Hayden
 Project Location South of East Black Mountain Road, east of North Hayden Road, west of North 81st Street, north of East Westland Road
 Applicant Contact Zach Schmidt, P.E., CFM Company Name Kimley-Horn
 Phone 602-906-1116 Fax _____ E-mail zach.schmidt@kimley-horn.com
 Address 7740 N. 16th Street, Suite 300, Phoenix, AZ 85020

Waiver Criteria

A project must meet at least one of three criteria listed below for the city to consider waiving some or all required stormwater storage. **However, regardless of the criteria, a waiver will only be granted if the applicant can demonstrate that the effect of a waiver will not increase the potential for flooding on any property.** Check the applicable box and provide a signed engineering report and supporting engineering analysis that demonstrate the project meets the criteria and that the effect of a waiver will not increase the potential for flooding on any property.

If the runoff for the project has been included in a storage facility at another location, the applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility.

- 1. The development is adjacent to a conveyance facility that an engineering analysis shows is designed and constructed to handle the additional runoff from the site as a result of development.
- 2. The development is on a parcel less than one-half acre in size.
- 3. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO).

For a full storage waiver, a conflict with ESLO is limited to:

- Property located in the hillside landform as defined in the city Zoning Ordinance
- Property in the upper desert landform that has a land slope steeper than 5% as defined in the city Zoning Ordinance
- Property within the ESL zoning overlay district where the only viable location for a stormwater storage basin requires blasting

This full waiver only applies to those portions of property meeting one of these three requirements.

Partial waivers are available for projects or portions of properties within the Environmentally Sensitive Lands Zoning Overlay District, not meeting any of the three full waiver criteria above, if post-development peak discharge rates do not exceed pre-development conditions, based on the 10- and 100-year storm events.

By signing below, I certify that the stated project meets the waiver criteria selected above as demonstrated by the attached documentation.

Engineer

Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 ♦ Phone: 480-312-2500 ♦ Fax: 480-312-7781



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

____ - PA - ____ - ZN - ____ - UP - ____ - DR - ____ - PP - ____ PC# _____

CITY STAFF TO COMPLETE THIS PAGE

Project Name _____

Check Appropriate Boxes:

Meets waiver criteria (specify): 1 2 3

Recommend approve waiver.

Recommend deny waiver:

None of waiver criteria met.

Downstream conditions prohibit waiver of any storage.

Other:

Explain: _____

Return waiver request:

Insufficient data provided.

Other: _____

Explain: _____

Recommended Conditions of Waiver:

All storage requirements waived.

Post-development peak discharge rates do not exceed pre-development conditions.

Other:

Explain: _____

Waiver approved per above conditions.

Waiver denied.

Floodplain Administrator or Designee

Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781



Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA -

- ZN -

- UP -

- DR -

- PP -

PC#

In-Lieu Fee and In-Kind Contributions

In-lieu fees are only applicable to projects where post-development peak discharge rates exceed pre-development levels, based on the 10- and 100-year storm events. If the city grants a waiver, the developer is required to calculate and contribute an in-lieu fee based on what it would cost the city to provide a storage basin, sized as described below, including costs such as land acquisition, construction, landscaping, design, construction management, and maintenance over a 75-year design life. The fee for this cost is \$1.87 per cubic foot of stormwater storage for a virtual storage basin designed to mitigate the increase in runoff associated with the 100-year/2-hour storm event. The applicant may submit site-specific in-lieu fee calculations subject to the Floodplain Administrator's approval.

The Floodplain Administrator considers in-kind contributions on a case-by-case basis. An in-kind contribution can serve as part of or instead of the calculated in-lieu fee. In-kind contributions must be stormwater related and must constitute a public benefit. In-lieu fees and in-kind contributions are subject to the approval of the Floodplain Administrator or designee.

Project Name Estates at Hayden

The waived stormwater storage volume is calculated using a simplified approach as follows:

V = ΔCRA; where

V = stormwater storage volume required, in cubic feet,

ΔC = increase in weighted average runoff coefficient over disturbed area ($C_{post} - C_{pre}$),

R = 100-year/2-hour precipitation depth, in feet (DSPM, Appendix 4-1D, page 11), and

A = area of disturbed ground, in square feet

Furthermore,

$V_w = V - V_p$; where

V_w = volume waived,

V = volume required, and

V_p = volume provided

R = 0.23

ΔC = 0.10

A = 439109

V = 9990

V_p = 3705

V_w = 6285

An in-lieu fee will be paid, based on the following calculations and supporting documentation:
In-lieu fee (\$) = V_w (cu. ft.) x \$1.87 per cubic foot = \$11,753

An in-kind contribution will be made, as follows:

No in-lieu fee is required. Reason:

Approved by:

Zachary Schmidt

Floodplain Administrator or Designee

03/02/18

Date

Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781

Appendix E. Disclaimer and Liability



WARNING & DISCLAIMER OF LIABILITY

The Drainage and Floodplain Regulations and Ordinances of the City of Scottsdale are intended to "minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding caused by the surface runoff of rainfall" (Scottsdale Revised Code §37-16).

As defined in S.R.C. §37-17, a flood plain or "*Special flood hazard* area means an area having flood and/or flood related erosion hazards as shown on a FHBM or FIRM as zone A, AO, A1-30, AE, A99, AH, or E, and those areas identified as such by the floodplain administrator, delineated in accordance with subsection 37-18(b) and adopted by the floodplain board." It is possible that a property could be inundated by greater frequency flood events or by a flood greater in magnitude than a 100-year flood. Additionally, much of the Scottsdale area is a dynamic flood area; that is, the floodplains may shift from one location to another, over time, due to natural processes.

WARNING AND DISCLAIMER OF LIABILITY PURSUANT TO S.R.C §37-22

"The degree of flood protection provided by the requirements in this article is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by man-made or natural causes. This article (Chapter 37, Article II) shall not create liability on the part of the city, any officer or employee thereof, or the federal government for any flood damages that result from reliance on this article or any administrative decision lawfully made thereunder."

Compliance with Drainage and Floodplain Regulations and Ordinances does not insure complete protection from flooding. The Floodplain Regulations and Ordinances meet established local and federal standards for floodplain management, but neither this review nor the Regulations and Ordinances take into account such flood related problems as natural erosion, streambed meander or man-made obstructions and diversions, all of which may have an adverse affect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations.

I have read and understand the above. If I am an agent for an owner I have made the owner aware of and explained this disclaimer.

	<i>Zimberg Schmidt</i>	03/02/18	
Plan Check No.	Owner or Agent	Date	