

Abbreviated Water and Sewer Needs

SEWER BASIS OF DESIGN

Proposed Whataburger 7134 E Thomas Road Scottsdale, AZ 85251

FINAL Basis of Design Report

□ APPROVED

☑ APPROVED AS NOTED

☐ REVISE AND RESUBMIT



Disclaimer: If approved; the approval is granted under the condition that the final construction documents submitted for city review will match the information herein. Any subsequent changes in the water or sewer design that materially impact design criteria or standards will require re-analysis, re-submittal, and approval of a revised basis of design report prior to the plan review submission.; this approval is not a guarantee of construction document acceptance. For questions or clarifications contact the Water Resources Planning and Engineering Department at 480-312-5685.

BY Idillon

DATE 4/21/2020

Prepared for:

Whataburger 300 Concord Plaza Drive San Antonio, Texas 78216 Address comments below on submitted plans:

- 1) MAG 440-3 sewer service line requires cleanout to be in ROW.
- 2) 4X 90 degree bends shown on service line.
 Replace each 90 degree elbow on service lines with 2X45 degreee bends followed by cleanout.
 3) No location or demolition shown of existing sewer service line. Existing line needs to be removed and permanently plugged at property

Prepared by:

Kimley-Horn and Associates, Inc. 1001 West Southern Avenue Mesa, Arizona 85210

291394000 March 2020 Copyright © 2020, Kimley-Horn and Associates, Inc.

Final Sewer Basis of Design

WHATABURGER SCOTTSDALE



MARCH 2020

Prepared By:



Contents

troduction	. 1
Intent	. 1
Project Description	. 1
stribution System Description	. 1
Existing collection System	. 1
Proposed Collection System	. 1
asis of Design	. 1
Design Methodology	. 1
Wastewater System Anaylsis and Results	. 2

Tables

Table 1 Sewer Demands......2

Appendices

Appendix A – Site Location Map

Appendix B – Proposed Sewer System Layout

Appendix C – Flowmaster Calculations

INTRODUCTION

INTENT

The purpose of this sewer report is to support the sanitary sewer system for the proposed Whataburger restaurant development located in the northeast corner of Thomas Road to and 71st Street in Scottsdale, Arizona. This report presents the basis of design criteria that will be used for the engineering design of the proposed development utilizing current sewer design standards and guidelines set forth by the City of Scottsdale, Arizona.

PROJECT DESCRIPTION

Whataburger Scottsdale is located within Section 27 of Township 2 North, Range 4 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. The site is bound to the south by Thomas Road and to the west by 71st Street. Existing commercial developments surround the site. See **Appendix A: Site Location Map**

Whataburger Scottsdale is a proposed 1.41-acre commercial development. The existing zoning of the project is C-3.

DISTRIBUTION SYSTEM DESCRIPTION

EXISTING COLLECTION SYSTEM

The site is surrounded by existing commercial development. Per the City of Scottsdale Quarter Section Map 15-44 there is an existing 8-inch VCP sewer line in Thomas Road directly south of the site, and an 8-inch PVC sewer line in 71st Street west of the site.

PROPOSED COLLECTION SYSTEM

The proposed Whataburger gravity sewer system will connect via a new wye connection tap to the existing 8-inch PVC main in 71st Street on the west side of the site. The proposed on-site collection system will consist of approximately 226-feet of 6-inch gravity sewer line that will collect and convey wastewater flows generated by the site. Refer to **Appendix B** for the Proposed Sewer System Layout Exhibit.

BASIS OF DESIGN

DESIGN METHODOLOGY

Average Day Demand design flows are calculated based on design criteria detailed within the City of Scottsdale Design Standards and Polices Manual (DS&PM). Per DS&PM Chapter 7, a design flow of 1.2 gallons per day per SF of restaurant shall be used. The DS&PM also requires a peaking factor of 6.0. Water System Analysis. See **Table 1** below for a summary of sewer demands.

Table 1 Sewer Demands

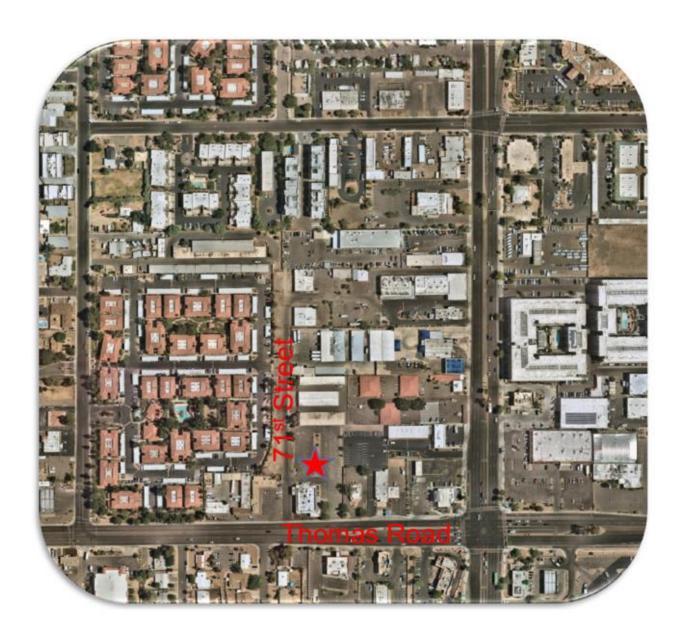
Land Use	Demand	Building SF	Average Day Demand (gpd)	Peaking Factor	Peak Flow (gpd)	Peak Flow (gpm)
Restaurant	1.2 gpd per SF	3,583 SF	4,300	6	25,800	17.92

Per the DS&PM proposed sewer lines were designed to achieve a full flow velocity of between 2.5 and 10 feet per second and maintain a maximum d/D ration 0.65 when calculated with a Manning's "n" value of 0.013. To satisfy these requirements the proposed private 6-inch sewer lateral will be designed with a minimum slope of 0.0188 ft/ft 1.88%). See **Appendix C** for pipe slope calculations.

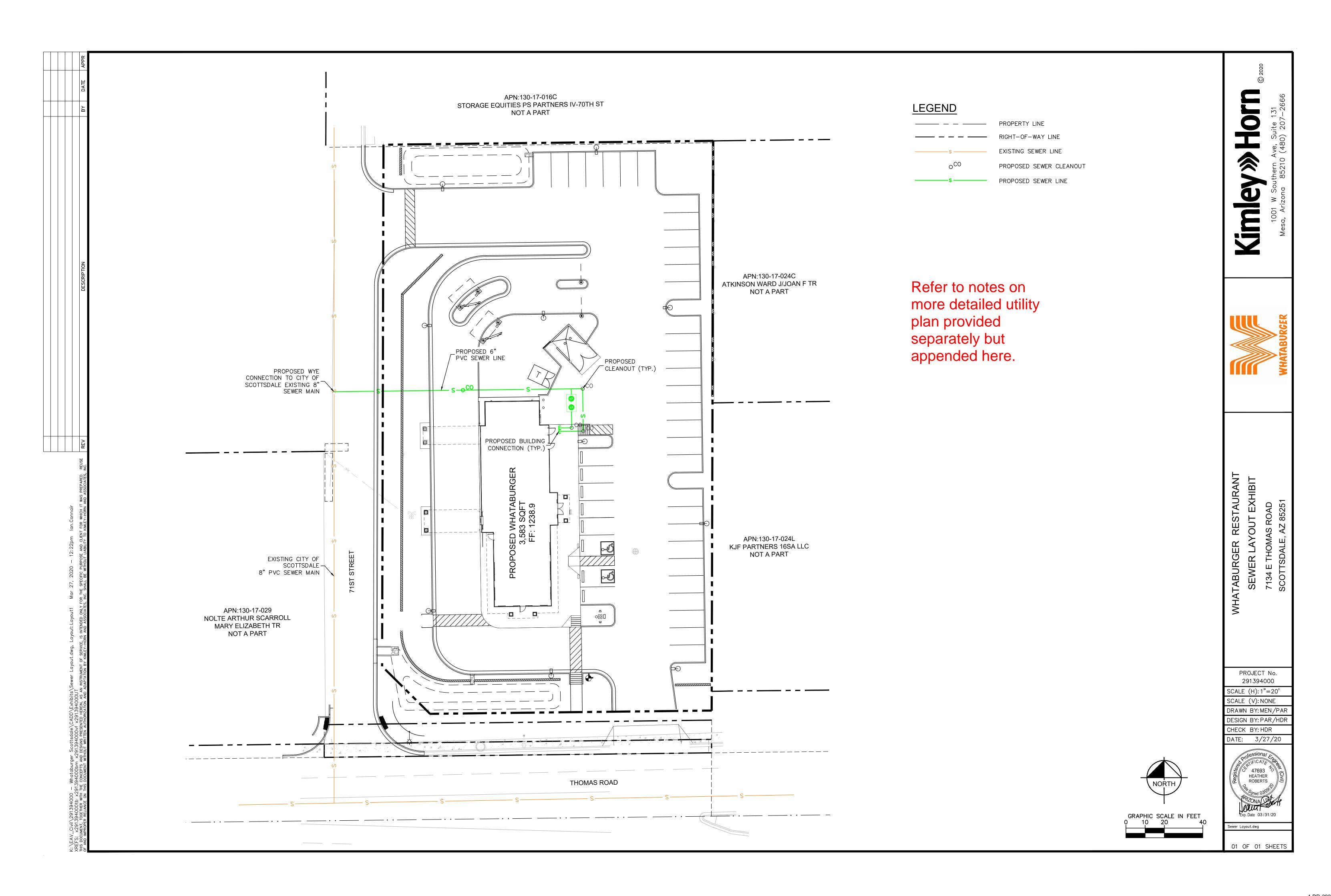
WASTEWATER SYSTEM ANAYLSIS AND RESULTS

To determine the capacity of the proposed wastewater collection system, the peak design flow was analyzed within the minimum design pipe slope. At the minimum design slope of 0.0188 ft/ft a 6-inch line has the capacity to convey approximately 497,000 gallons per day. A 6-inch line at the design slope can convey the proposed peak design flow of 25,800 gallons per day at a normal depth of 0.9' or a d/D ratio of 0.15, at a velocity of 2.06 ft/s. **See Appendix C – Flowmaster Calculations** for pipe capacity calculations.

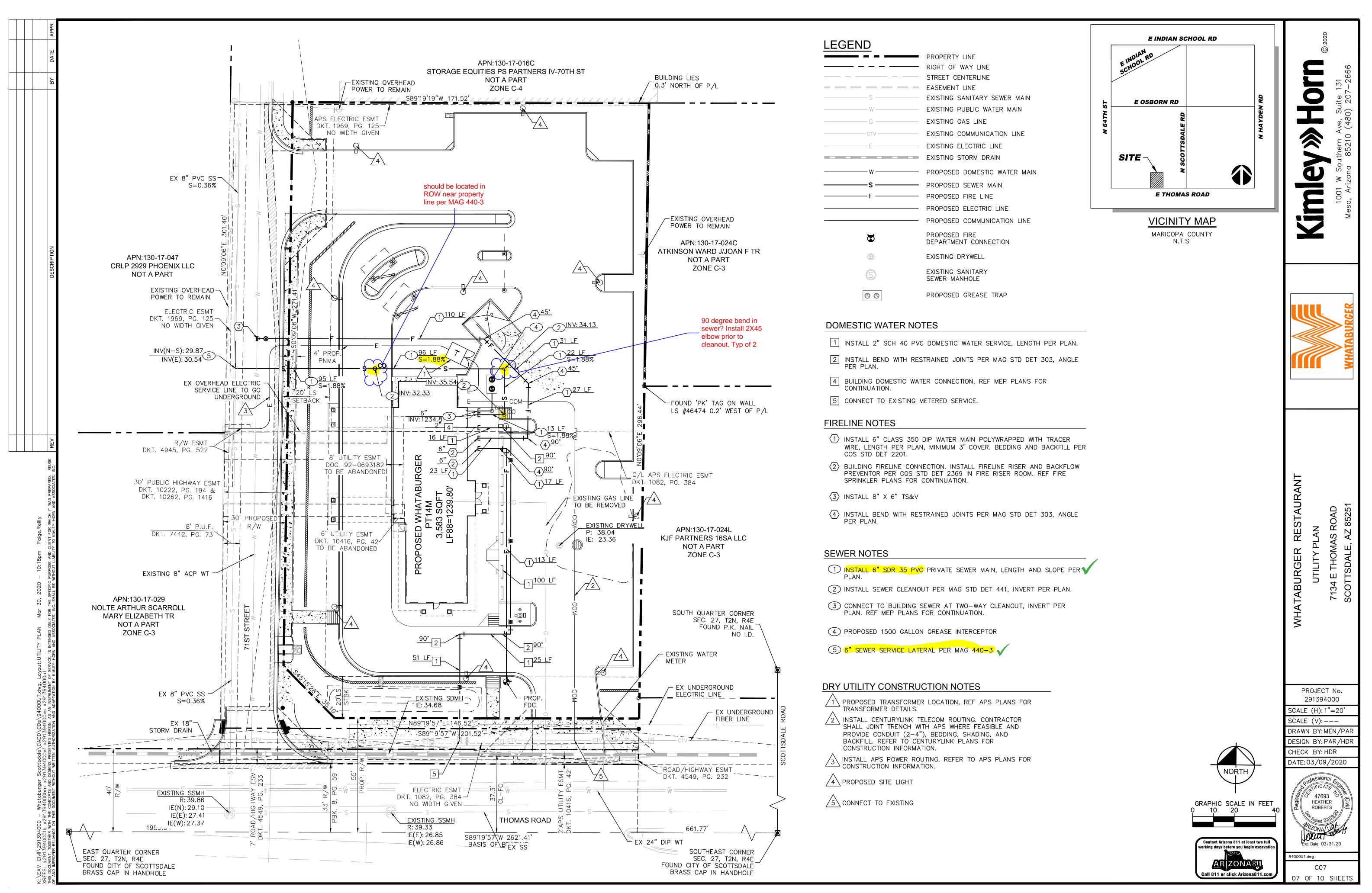
Appendix A – Site Location Map



Appendix B – Proposed Sewer System Layout



Sheet added by LDillon, submitted at same time as this report as separate case file as 4_DR_2020_V2_Utility Plan.pdf



Appendix C – Flowmaster Calculations

Worksheet for 6-inch Capacity

Project Description		1 7
1 Tojout Dosonption	Monnie	
Friction Method	Manning Formula	
	Full Flow	
Solve For	Capacity	
Input Data		
Roughness Coefficient	0.013	
Channel Slope	0.0188 ft/ft	
Normal Depth	6.0 in	
Diameter	6.0 in	
Discharge	497,217 gal/day	
Results		
Discharge	497,217 gal/day	
Normal Depth	6.0 in	
Flow Area	0.2 ft ²	
Wetted Perimeter	1.6 ft	
Hydraulic Radius	1.5 in	
Top Width	0.00 ft	
Critical Depth	5.3 in	
Percent Full	100.0 %	
Critical Slope	0.0170 ft/ft	
Velocity	3.92 ft/s	
Velocity Head	0.24 ft	
Specific Energy	0.74 ft	
Froude Number	(N/A)	
Maximum Discharge	534,859 gal/day	
Discharge Full	497,217 gal/day	
Slope Full	0.0188 ft/ft	
Flow Type	Supercritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description	N/A	
Profile Headloss	0.00 ft	
Average End Depth Over Rise	0.0 %	
Normal Depth Over Rise	100.0 %	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	6.0 in	
Critical Depth	5.3 in	
Critical Depth Channel Slope	5.3 in 0.0188 ft/ft	

Sewer Capacity.fm8 3/26/2020

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 FlowMaster [10.02.00.01] Page 1 of 1

Worksheet for 6-inch Design

Project Description	Worksneet	
	Monning	
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.013	
Channel Slope	0.0188 ft/ft	
Diameter	6.0 in	
Discharge	25,800 gal/day	
Results		
Normal Depth	0.9 in	
Flow Area	0.0 ft ²	
Wetted Perimeter	0.4 ft	
Hydraulic Radius	0.6 in	
Top Width	0.36 ft	
Critical Depth	1.2 in	
Percent Full	15.5 %	
Critical Slope	0.0073 ft/ft	
Velocity	2.06 ft/s	
Velocity Head	0.07 ft	
Specific Energy	0.14 ft	
Froude Number	1.574	
Maximum Discharge	534,859 gal/day	
Discharge Full	497,217 gal/day	
Slope Full	0.0001 ft/ft	
Flow Type	Supercritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description	N/A	
Profile Headloss	0.00 ft	
Average End Depth Over Rise	0.0 %	
Normal Depth Over Rise	15.5 %	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	0.9 in	
Critical Depth	1.2 in	
Channel Slope	0.0188 ft/ft	
Critical Slope	0.0073 ft/ft	

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 FlowMaster [10.02.00.01] Page 1 of 1