



Drainage Reports

PRELIMINARY DRAINAGE REPORT

Solitude

Southeast of Happy Valley Road and Pima Road
Scottsdale, Arizona

Plan #	
Case #	5-PP-2020
Q-S #	
<input checked="" type="checkbox"/> Accepted	
<input type="checkbox"/> Corrections	
N. Baronas	1/12/2021
Reviewed By	Date

Prepared for:

Sonora West Development INC.
8937 East Bell Road
Suite 100
Scottsdale, Arizona 85260



Prepared by:

Kimley-Horn and Associates
1001 West Southern Avenue, Suite 131
Mesa, Arizona 85210
291203002
December 2020

PRELIMINARY DRAINAGE REPORT

SOLITUDE
SOUTHEAST OF HAPPY VALLEY ROAD AND
PIMA ROAD
SCOTTSDALE, ARIZONA

DECEMBER 2020

Prepared By:

Kimley»Horn

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INTRODUCTION

PROJECT DESCRIPTION

This Preliminary Drainage report has been prepared for the proposed Solitude residential development. Solitude is a proposed 20-acre single family residential subdivision consisting of 17 single family residential units. The development will be combined with the adjacent 20-acre subdivision previously called HV91 which is currently under construction. The combined development is intended to be called Solitude would consist of 34 lots on 40-acres.

The zoning case (24-ZN-2017) and preliminary plat (5-PP-2018) for HV91 were approved in 2018. Infrastructure plans for the roadway and drainage facilities (310-19) were approved in 2019 and are currently under construction.

The proposed zoning for the project is R1-43 ESL. The proposed site is located within the City of Scottsdale and falls under the City's Environmentally Sensitive Lands Ordinance (ESLO).

PROJECT LOCATION AND DESCRIPTION

Solitude is located within Section 7 of Township 4 North, Range 5 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. The development is bound to the west by the existing HV91 development, to the east by the 92st Street to the north by Happy Valley Road, and to the south by an undeveloped parcel of land. (See **Figure 1: Vicinity Map**).

The development is located within one flood zone as shown on Flood Insurance Rate Map (FIRM) panel number 04013C1310L, dated October 16, 2013 (see **Appendix A** for FIRM). The flood zones that pertain to the site are as follows:

“Other Flood Areas” Zone X – “Area of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; areas protected by levees from 1% annual chance flood”

The property is undeveloped natural desert, characterized by braided washes and rock features of varying sizes. Undeveloped desert is also characterized by native desert grasses and brush.

SCOPE OF DRAINAGE REPORT

The HV91 project is permitted and under construction. This report specifically covers the new 20 acres of development for the Solitude project.

This drainage report established the general drainage parameter and criteria for preliminary design. This report provides a hydrologic plan for the development of the site as well as preliminary hydraulic analysis for the washes crossing the site.

All drainage criteria presented in this report will conform to the City of Scottsdale Design Standards & Polices Manual (DS&PM).

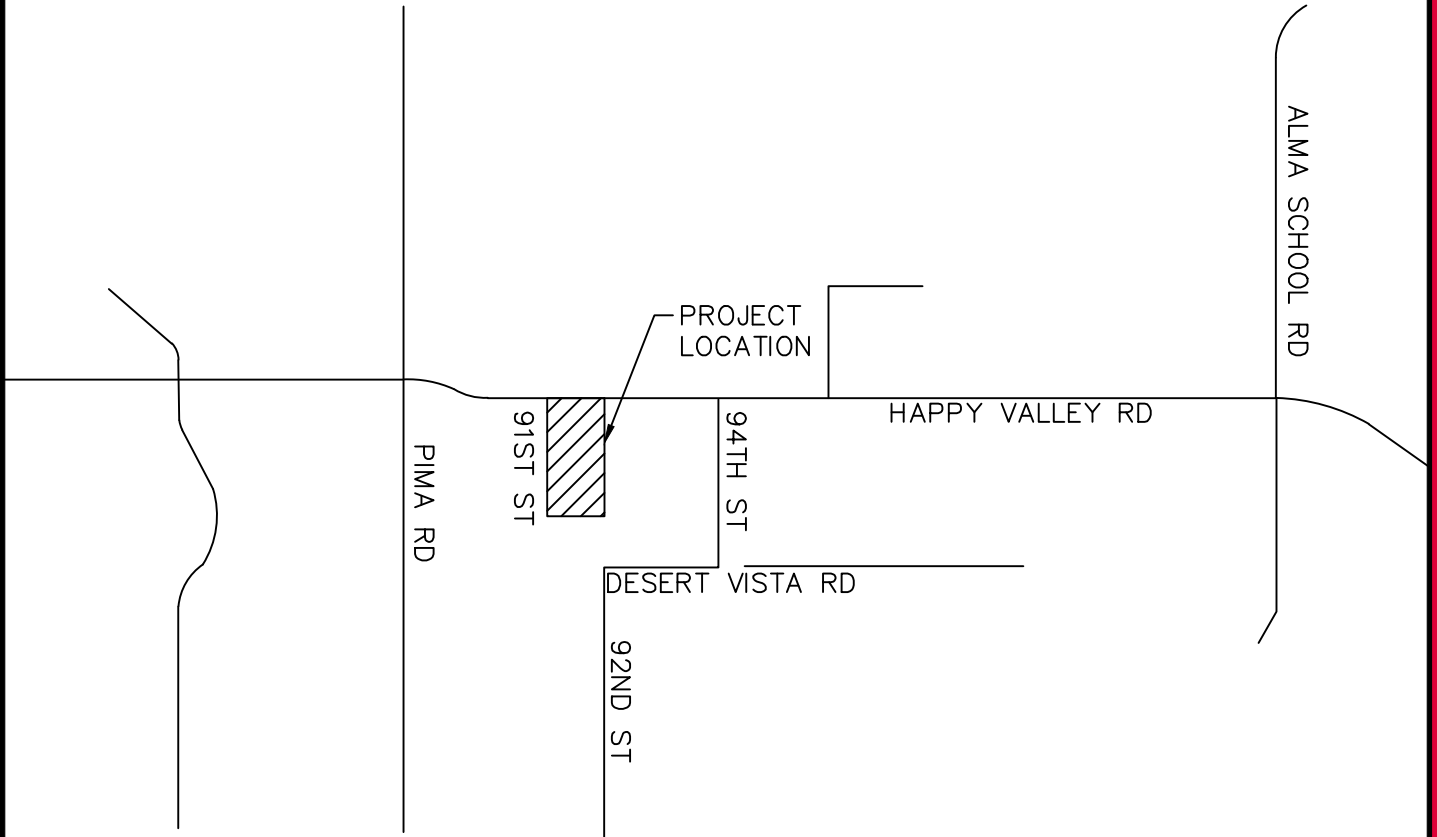


FIGURE 1
VICINITY MAP



DESCRIPTION OF EXISTING DRAINAGE CONDITIONS AND CHARACTERISTICS

EXISTING SITE CONDITIONS

The site is characterized by many washes of varying sizes. The on-site washes vary in size and depth, but generally flow from the northeast to southwest through the site. Storm water runoff impacts the site from the east and north, and is conveyed through the site in existing washes.

The site falls within the City of Scottsdale Environmentally Sensitive Lands (ESL) and is subject to the design criteria of the Environmentally Sensitive Lands Ordinance (ELSO). Specifically, the site is categorized as Upper Desert Landform of ESL. Per the DS&PM “The ordinance requires that a percentage of each property be permanently preserved as natural area open space (NAOS) and the specific environmental features, including vegetation, washes, mountain ridges and peaks be protected for inappropriate development”.

EXISTING OFF-SITE DRAINAGE CONDITIONS

Off-site flows impact the site from the east, and north. Off-site sub-basins north of Happy Valley generally consist of low density residential sub-divisions with some golf course and resort uses. South of Happy Valley Road the sub-basins consist of very low density residential and undeveloped desert uses. The Happy Valley Road section varies between a 2-lane road with shoulder, and a 2-lane road with a median and curb and gutter. Along the north project frontage Happy Valley Road consists of a median divided roadway with curb and gutter.

This report utilized the offsite drainage analysis established with the HV91 drainage report. The existing condition HEC-1 hydraulic model was updated to provide new concentrations points at the eastern boundary of the project along the 92nd Street alignment.

Refer to **Figure 2: Offsite Drainage Map**.

Significant washes are defined as having a 100-year flow of 50 cfs or more. There are no washes with 100-year peak flows of 750 cfs or greater, which indicates that no Vista Corridors exist within the project area. Significant washes have been identified on **Figure 3: Existing Drainage Condition**. Hydrologic results can be found in **Appendix B**.

EXISTING ON-SITE DRAINAGE CONDITIONS

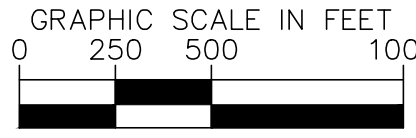
Multiple washes of various sizes and depth cross the site from northeast to southwest. Two of the washes, located at the southeast corner and the north portion of the site, are identified as significant washes with a flow over 50 cfs. Refer to **Figure 3: Existing Drainage Condition**.

An existing conditions hydrologic model was completed to determine the peak stormwater discharges leaving the site. The existing condition discharges will be compared to the proposed condition discharges in a “pre-vs-post” analysis. The concentration points of this pre-vs-post analysis are located on the boundary the eastern boundary of the original HV91 project. While the intent is for Solitude to be one community, the

initial portion of HV91 is under construction, and the drainage facilities have been sized to accommodate the existing flows.

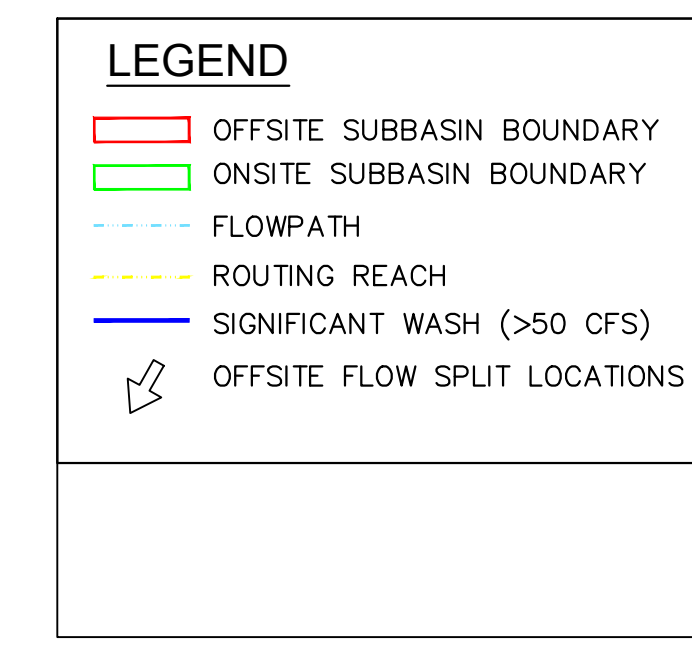
Two significant washes have been identified with Wash A just south of Happy Valley Road, and Wash C located on the southernmost portion of the site. Hydraulic analysis was performed at key cross sections on the significant washes to determine the base flood elevation (BFE).

Hydrology and hydraulic results can be found in **Appendix B** and **Appendix C** respectively.



GRAPHIC SCALE IN FEET

A horizontal scale bar with tick marks at 0, 250, 500, and 1000 feet. The bar is divided into four equal segments, each representing 250 feet. The segments are colored in a repeating pattern: white, black, white, and black.

[illegible]

Kimley»»Horn
© 2019 KIMLEY-HORN AND ASSOCIATES, INC.
7740 North 16th Street, Suite 300
Phoenix, Arizona 85020 (602) 944-5500

SCALE (H): 1"=500'
SCALE (V): NONE
DESIGNED BY: ZJH
DRAWN BY: ZJH
CHECKED BY: JMB
DATE: SEP 2019

PROJECT NO.	291203001
DRAWING NAME	

PROPOSED PRELIMINARY DRAINAGE PLAN

PROPOSED ON-SITE DRAINAGE PLAN

The proposed solitude development consists of an additional 17 single family residential units. Lots located along the washes will have finished floor elevations a minimum of one foot above the 100-year base flood elevation (BFE). In general, lots on the high side of the road (east side) will drain to the street system and runoff will be conveyed in the streets to proposed detention basins. Due to the steep and undulating terrain, lots on the low side of the road will rear or side yard drain into adjacent washes or drainage swales within the development in order to minimize impacts to environmental features, existing natural area open space, and meet design criteria as required with the Environmentally Sensitive Lands Ordinance for the project. Detention basins will be located adjacent to the roadway tract and outside of on-lot building envelopes within designated drainage easements and detain runoff before discharging into the existing washes and will be sized to meet first flush criteria. Post development flows exiting the site will be attenuated through detention basins to a level equal to or less than pre-development flows. See **Figure 4: Proposed Conditions Drainage Map**.

The proposed development is R1-43 ESL zoning. Per this zoning, the development will be constructed in two phases. The first phase consists of the roadway, utilities and infrastructure need to serve the development, include the drainage infrastructure such as the detention basins, channels and culverts. The second phase will be the individual lots and homes. Each lot will require an individual single lot grading plan.

PROPOSED OFF-SITE DRAINAGE PLAN

Off-site flows impact the site from the north and east. Flows will be conveyed through the site and will discharge at their historic locations on the west and south side of the Site. In most cases, off-site flows are conveyed within the existing washes. In some locations, a proposed channel is used to convey the flows through the development. The proposed drainage channels will be designed to maintain a natural appearance as much as possible. Wash C is maintained in the existing natural drainage corridor. Wash A impacts the proposed roadway and must be conveyed in a proposed channel. The proposed channel will connect with the existing channel and culvert designed with HV91. A wash modification will be submitted for the relocation of Wash A.

PROPOSED ON-SITE HYDROLOGY

On-site runoff from the proposed development maintains post-development flows at or below pre-development conditions at each of the concentration points into HV91, for the three design storms (2-year, 10-year, and 100-year). With the exception of one point, CP40, where the 100-year flow is increased by 1 cfs. This is within the level of accuracy of the analysis, and should be considered incidental and in conformance with the design. A summary of pre- and post-development peak discharges is provided in **Table 1**. Multiple detention basins are used to attenuate peak discharge from on-site runoff. Each basin utilizes a bleed-off pipe with orifice plate with the intent to control post-development runoff exiting the development, with a spillway for larger storm events. The total drain time for all basins is less than 36 hours. Refer to **Appendix B** for the detailed hydrologic model results.

Table 1: Peak Discharge Summary

Concentration Point	Prop. Cond. 2-Year (cfs)	Ex. Cond. 2-Year (cfs)	Prop. Cond. 10-Year (cfs)	Ex. Cond. 10-Year (cfs)	Prop. Cond. 100-Year (cfs)	Ex. Cond. 100-Year (cfs)
CP10	30	30	72	72	168	172
CP15	1	1	1	1	2	3
CP20	1	1	1	1	2	3
CP25	1	1	1	2	3	5
CP30	7	7	17	17	38	40
CP35	3	4	7	7	14	15
CP40	2	3	4	6	7	14
CP45	29	29	67	67	156*	155

PROPOSED ON-SITE HYDRAULICS

On-site runoff will be conveyed in the local streets, swales, storm drains, and culverts to the detention basins. Per the DS&PM, all interior streets will be designed to convey the peak discharge from the 10-year storm event at or below the top of curb elevation. Additionally, the streets will convey the 100-year runoff within the proposed tracts and maintain a maximum flow depth of eight inches above the gutter flow line. Catch basins with storm drains or scuppers will capture pavement runoff and outfall to the proposed detention basins. The scupper, catch basins and storm drains will be designed per the DS&PM and FCDMC's Drainage Policies and Standards. Detailed catch basin and street capacity analysis is beyond the scope of this preliminary drainage report and will be completed as part of the final design.

The existing hydraulic cross sections were revised for the proposed condition to determine the 100-year BFE through the property. Development of the site, including roadway, culverts and lots encroach into the existing BFE. The proposed hydraulic model includes these encroachments and modifications to calculate the proposed BFE, which are used to set the adjacent building pads.

Preliminary roadway culverts were designed. The proposed culverts are designed to convey the 100-year storm within the culvert without any roadway overtopping. The two proposed drainage channels were preliminarily designed.

Refer to **Appendix B** for preliminary hydraulic calculations.

Minor flows impact the proposed lots. These flows will be routed around the lots in smaller drainage swales, and at times require culvert driveway crossings. These lot drainage facilities will be included on the single lot grading plans and are not covered in the scope of this report.

SPECIAL CONDITIONS

404 PERMIT/JURISDICTIONAL WASHES

An Approved Jurisdictional Delineation stating no potential water of the US existing on the site was approved with the Army Corp of Engineers (File No. SPL-2019-00519) dated May 29, 2019. No additional analysis or approval is required for the site. **Refer to Appendix D**

EROSION SETBACK ANALYSIS

Erosion Setback Analysis will be performed on the significant washes at the time of preliminary plat and preliminary drainage report. Lots or improvements which encroach into the Erosion Hazard Setback will require a form of erosion protection such as concrete scour wall or launchable rip-rap. The standard minimum 20 foot setback is shown on the Grading and Drainage Plan.

ADEQ WATER QUALITY REQUIREMENTS

Development of the project will impact a large enough area to require a submittal of a Notice of Intent (NOI) to the Arizona Department of Environmental Quality (ADEQ). The NOI will be submitted to ADEQ and an approved NOI certificate with an AZCON number will be provided to the city before approval of any improvement plans.

CULVERT SEDIMENTATION

Sedimentation reduces the hydraulic performance of culverts and can lead to safety, erosion, and maintenance issues. The proposed culverts and storm drains within the project have been designed to minimize sedimentation when possible, as well as providing solutions to reduce the impact of sedimentation. Culverts are designed to match the slope of the existing channel. Additionally, the majority of the culverts are “inlet” control, with flow velocity greater than 10 ft/s. These “self-cleaning” velocities help clear the culverts of sedimentation in larger storm events.

DATA ANALYSIS METHODS

GENERAL DISCUSSION

A detailed hydrologic model was prepared for the existing and proposed site condition. Hydraulic cross sections were prepared for the significant washes that traverse the site. The sections below provide the hydrology and hydraulic methodology.

HYDROLOGY

The U.S. Army Corps of Engineers HEC-1 hydrologic computer program was used to determine the 2-, 10-, and 100-year peak discharges for off-site and on-site flows. HEC-1 models were prepared for the existing and proposed development conditions. The Drainage Design Management System for Windows (DDMSW) program was used to develop the hydrologic parameters for the on-site drainage areas and off-site drainage areas west of the site. Green and Ampt rainfall loss parameters were estimated using DDMSW, the City of Scottsdale parameters, and the Flood Control District of Maricopa County (FCDMC) Drainage Design Manual – Hydrology (Hydrology Manual). Time of Concentration calculations were calculated using DDMSW. Values that show non-default values or out-of-range results are due to the NMN parameter selected for the HEC-1 Model. Because of the varying sub-basin sizes, the selected NMN parameter will not meet the time of concentration requirements specified in the FCDMC Drainage Design Manual - Hydrology for each sub-basin. The HEC-1 models were run with varying NMN parameters to confirm that the hydrograph shape and peaks were valid. The HEC-1 models were prepared using the Clark Unit Hydrograph. Rainfall depth were estimated for the site from the National Oceanic and Atmospheric Administration Atlas 14 (NOAA14).

Six different soil types were identified for the on-site and off-site sub-basins using the web soil survey from the National Resource Conservation Service (NRCS). Web soil survey results are included within **Appendix B**.

Land Use parameters were established for the existing and proposed HEC-1 models. The FCDMC standard land use parameters were used as a basis for the parameters, as well as other recent project is the area. Vegetation cover was set based on a review of aerial photography of the of existing drainage area, taking a typical coverage of a specific land use. A conservative value of 0 percent was used for mountain terrain (UND). Vegetation cover of developed land varies from property to property, a conservative average was used for the overall land use. Vegetation cover is for pervious areas only. RTIMP represents the effective impervious area which is hydraulically connected. RTIMP values for developed land used are based on conservative estimates of connected impervious areas on a typical lot and roadway within the specific land use. Land uses of the existing off-site developments is based on the zoning of the properties and review of existing development as available from aerial photography. See **Table 2** below for complete Land Use Parameters. RTIMP calculations for R1-18 and R1-43 are provided in **Appendix B**. Land use maps for the existing and proposed development conditions are provided in **Appendix B**.

Table 2 Land Use Parameters

Land Use Code	Description	IA	RTIMP	Vegetation Cover
R1-18	Min Lot Size = 13,500 Sq Ft	0.30	27	50.0
R1-43	Min Lot Size = 32,250 Sq Ft	0.30	17	20.0
GOLF	Golf Course	0.44	0	60.0
DESERT	Natural Desert	0.15	0	25.0
UND	Natural Desert S>10%	0.25	0	0

A stage storage and outfall rate calculation spreadsheet was prepared for the proposed detention basins. The stage storage volume is based on end-area calculations at 1-foot intervals. The basin discharge rates through the proposed bleed pipes are calculated from Manning and Orifice equations. Overflow for larger storm events are provided in an overflow weir, which will be sized at final design. Individual basin stage storage and discharge rate tables are provided in **Appendix B**.

HYDRAULICS

100-year BFEs for the significant washes were established using the Bentley Flowmaster V8i computer program. Cross sections were cut for the existing washes at key locations. The cross sections were updated in the proposed condition if they are encroached by a proposed lot. Manning n-value for the channels we estimated from *Chapter 7* of the FCDMC Hydraulics Manual.

Culvert crossing of the significant washes were sized using the Federal Highway Administration HY-8 version 7.30 computer program. Culverts were preliminarily sized to convey the 100-year storm through the structure.

Refer to **Appendix C** for the results of the hydraulic modeling for the existing and proposed condition.

STORMWATER STORAGE METHOD

Stormwater run-off generated by the site is routed to proposed detention basins, located adjacent to the roadway tract and outside of building envelopes on-lot for the development. These detention basins will be located within a designated drainage easement and will attenuate the peak flows leaving the property. Additionally, the basins have been sized to meet first flush requirements of the first half inch of run-off. The existing property is a part of the ESLO. Refer to **Appendix B** for the pre- and post-development hydrologic model results.

CONCLUSIONS

- Hydrologic models were prepared for the on-site and off-site areas for the pre- and post-development conditions. On-site detention basins were sized to ensure that the post-development runoff exiting the site are equal or less than pre-development conditions. Additionally, detention basins are sized to meet first flush requirements. Basins are designed to drain within 36 hours and are located within designated drainage easements on-lot and outside of building envelopes proposed with the development.
- Multiple significant washes cross the development. The proposed development will encroach on the washes. Hydraulic models for the existing and proposed conditions were prepared to determine the BFE. The BFE was used to set the finished floor elevations for each lot.
- Significant washes are maintained in their existing corridors or modified with a wash modification application.
- On-site runoff will be conveyed through the local streets and storm drains to the detention basins and wash corridors. Culverts will convey the flow under the new roads. The conveyance facilities will be sized during final design.

REFERENCES

City of Scottsdale, *Design Standards and Policies Manual*, January 2018.

City of Scottsdale, Stormwater and Floodplain Management Ordinance, Chapter 37, July 2016.

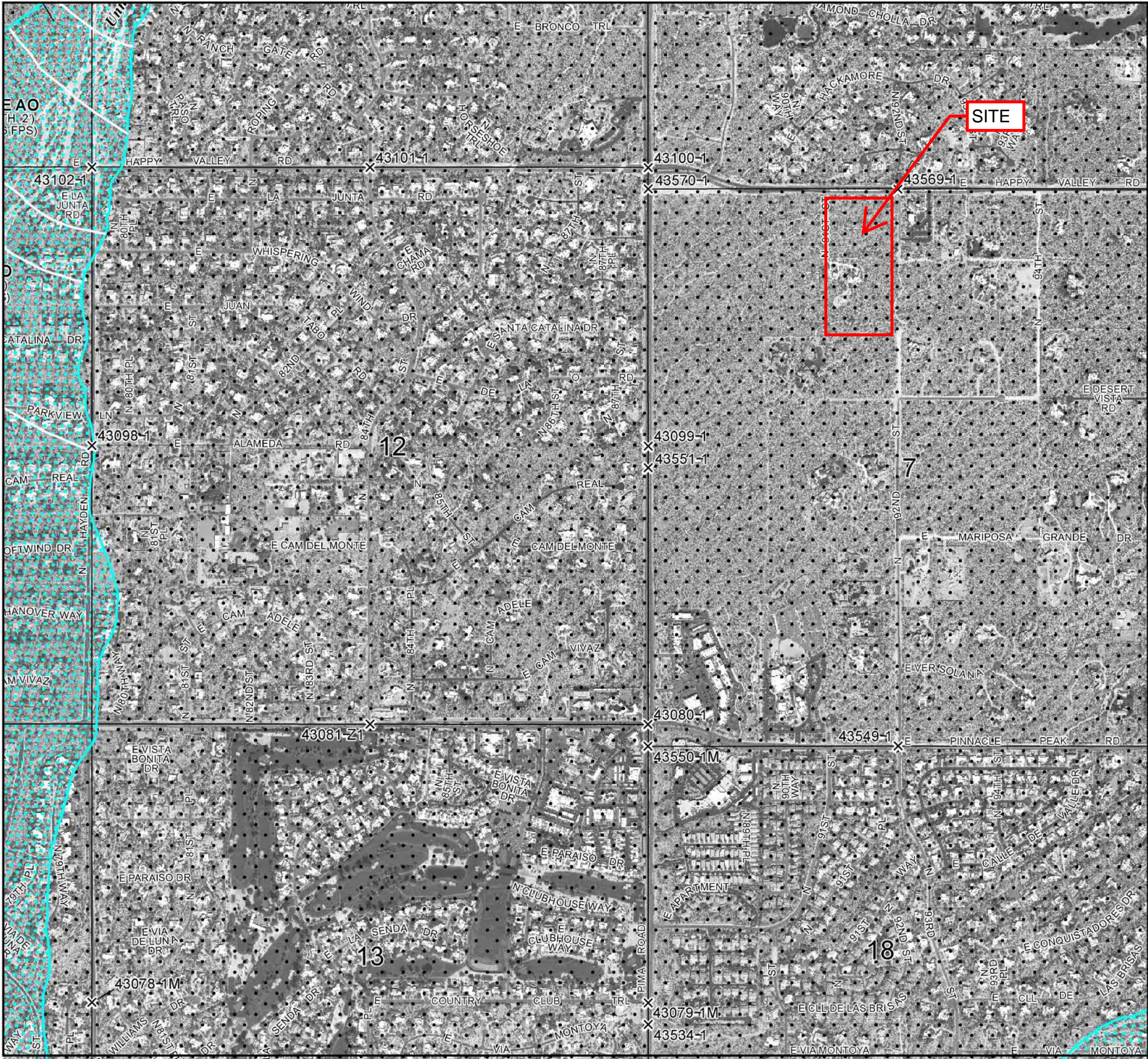
Federal Emergency Management Agency, Flood Insurance Rate Map Panel No04013C1331M, dated November 4, 2015

Flood Control District of Maricopa County, Drainage Design Manual – Hydrology, updated August 15, 2013.

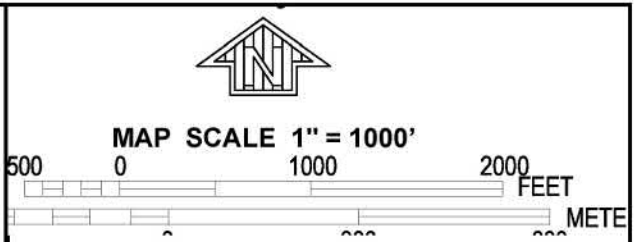
City of Scottsdale Topography Quarter Section Maps.

Final Drainage Report for HV91 dated 05/14/2019, prepared by Kimley-Horn and Associates.

Appendix A – Flood Insurance Rate Map



37 31 000m N
37 30 000m N
37 29 000m N



NFIP

NATIONAL FLOOD INSURANCE PROGRAM


PANEL 1310L

FIRM
FLOOD INSURANCE RATE MAP
MARICOPA COUNTY,
ARIZONA
AND INCORPORATED AREAS

PANEL 1310 OF 4425
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
MARICOPA COUNTY	040037	1310	L
PHOENIX, CITY OF	040051	1310	L
SCOTTSDALE, CITY OF	045012	1310	L



MAP NUMBER
04013C1310L
MAP REVISED
OCTOBER 16, 2013
Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov

Appendix B – Hydrology

HEC-1 Exhibits (Existing and Proposed Conditions)

- HEC-1 Schematic Map
- Soils Map
- Land Use

DDSMW Output: (Existing and Proposed Conditions)

- Rainfall
- Land use
- Soils
- Storage
- Routing

HEC-1 Output

- Existing Condition
- Proposed Condition

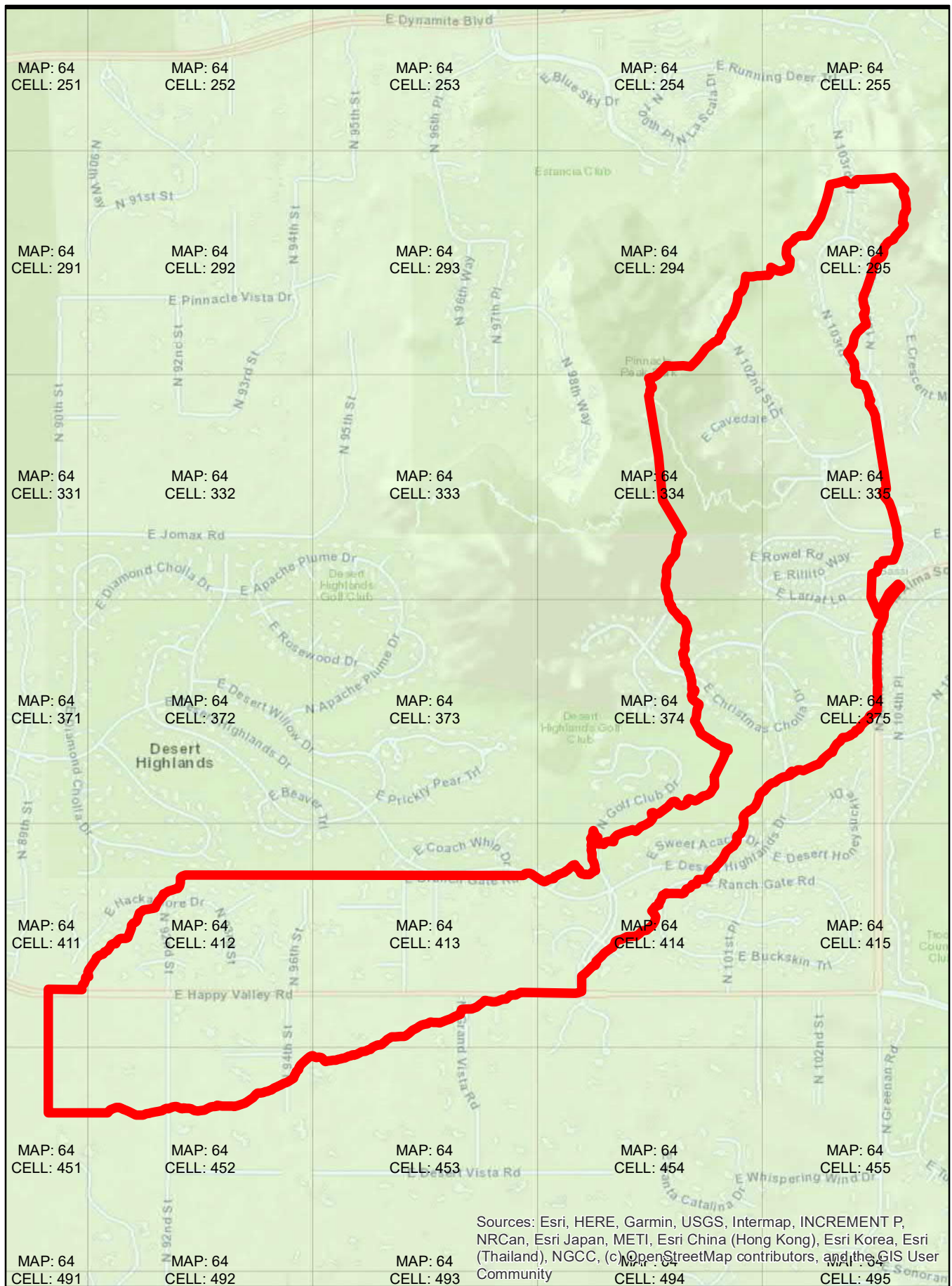
Existing Condition

City of Scottsdale
Drainage Design Management System
RAINFALL DATA
Project Reference: SOLITUDE EX

Page 1

9/25/2019

ID	Method	Duration	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
DEFAULT	NOAA14	5 MIN	0.295	0.398	0.476	0.579	0.657	0.735
	NOAA14	10 MIN	0.450	0.606	0.724	0.881	0.999	1.119
	NOAA14	15 MIN	0.558	0.751	0.898	1.092	1.239	1.387
	NOAA14	30 MIN	0.751	1.012	1.209	1.471	1.669	1.868
	NOAA14	1 HOUR	0.929	1.252	1.496	1.820	2.065	2.312
	NOAA14	2 HOUR	1.067	1.416	1.683	2.044	2.317	2.598
	NOAA14	3 HOUR	1.137	1.481	1.756	2.137	2.438	2.750
	NOAA14	6 HOUR	1.342	1.705	1.995	2.392	2.703	3.024
	NOAA14	12 HOUR	1.587	1.995	2.319	2.758	3.094	3.441
	NOAA14	24 HOUR	1.919	2.497	2.963	3.622	4.153	4.715



City of Scottsdale
Drainage Design Management System
SUB BASINS

Project Reference: SOLITUDE EX

Page 1

7/29/2020

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)		2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin ID: 01																		
OFF10	0.900	3.21	245.5	237.0	Natural	0.034	0.30	0.28	5.58	0.231	21	Tc (Hrs)	0.766*	0.749*	0.671*	0.594*	0.550*	0.515 *
												Vel (f/s)	6.15	6.29	7.02	7.93	8.56	9.14
												R (Hrs)	0.743	0.724	0.642	0.560	0.514	0.478
OFF15	0.003	0.12	176.5	176.5	Natural	0.076	0.15	0.37	6.54	0.140		Tc (Hrs)	0.238	0.233	0.209	0.185	0.171	0.161
												Vel (f/s)	0.74	0.76	0.84	0.95	1.03	1.09
												R (Hrs)	0.379	0.369	0.327	0.285	0.262	0.244
OFF20	0.008	0.21	184.0	184.0	Natural	0.070	0.15	0.37	6.54	0.140		Tc (Hrs)	0.298	0.291	0.261	0.231	0.214	0.201
												Vel (f/s)	1.03	1.06	1.18	1.33	1.44	1.53
												R (Hrs)	0.435	0.423	0.375	0.327	0.301	0.281
OFF30	0.036	0.71	204.2	204.0	Natural	0.054	0.19	0.32	6.54	0.138	4	Tc (Hrs)	0.453*	0.443*	0.398	0.354	0.329	0.309
												Vel (f/s)	2.30	2.35	2.62	2.94	3.17	3.37
												R (Hrs)	0.778	0.758	0.672	0.590	0.545	0.509
OFF35	0.031	0.80	147.5	147.5	Natural	0.052	0.20	0.31	6.54	0.138	5	Tc (Hrs)	0.519*	0.508*	0.456*	0.406	0.378	0.355
												Vel (f/s)	2.26	2.31	2.57	2.89	3.10	3.31
												R (Hrs)	1.084	1.056	0.937	0.824	0.761	0.711
OFF40	0.004	0.15	179.3	179.3	Natural	0.037	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.166	0.163	0.147	0.133	0.125	0.118
												Vel (f/s)	1.33	1.35	1.50	1.65	1.76	1.86
												R (Hrs)	0.258	0.251	0.226	0.201	0.187	0.176
OFF45	0.081	0.95	194.1	194.1	Natural	0.037	0.26	0.29	5.46	0.212	12	Tc (Hrs)	0.442*	0.432*	0.392	0.349	0.324	0.303
												Vel (f/s)	3.15	3.23	3.55	3.99	4.30	4.60
												R (Hrs)	0.601	0.586	0.526	0.463	0.425	0.396
OFF50	0.001	0.08	187.5	187.5	Natural	0.041	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.126	0.124	0.112	0.101	0.095*	0.090 *
												Vel (f/s)	0.93	0.95	1.05	1.16	1.24	1.30
												R (Hrs)	0.253	0.247	0.222	0.198	0.184	0.173
ON10	0.011	0.15	145.7	145.7	Natural	0.068	0.15	0.37	6.54	0.140		Tc (Hrs)	0.267	0.261	0.234	0.207	0.192	0.180
												Vel (f/s)	0.82	0.84	0.94	1.06	1.15	1.22
												R (Hrs)	0.245	0.238	0.211	0.184	0.170	0.158

* Non default value or value out of range

(stSubBasCG.rpt)

City of Scottsdale
Drainage Design Management System
SUB BASINS
Project Reference: SOLITUDE EX

Page 2

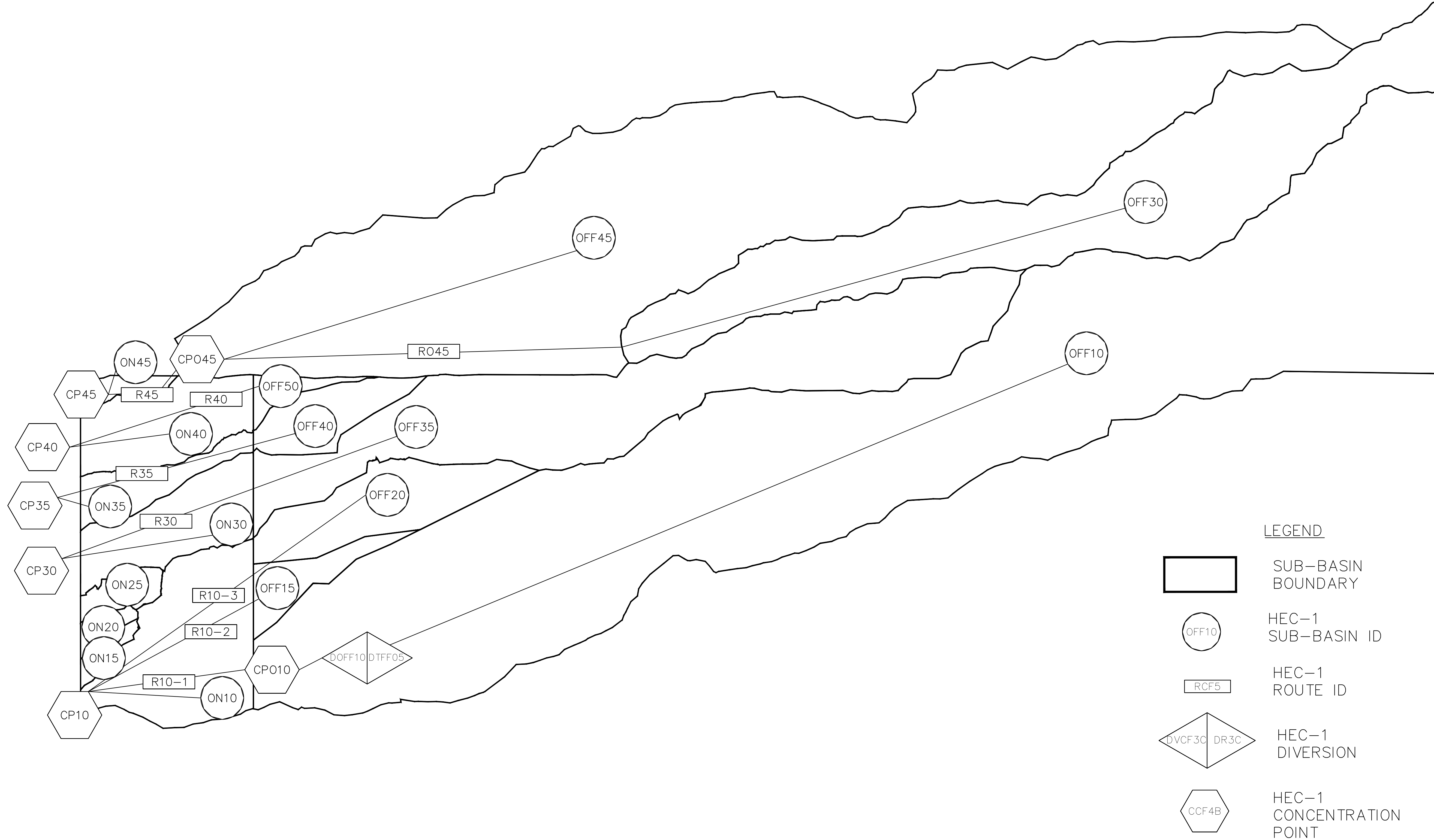
7/29/2020

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr	
Major Basin ID: 01																		
ON15	0.001	0.05	434.0	294.1	Natural	0.083	0.15	0.37	6.54	0.140	Tc (Hrs)	0.138	0.134	0.120	0.106	0.099*	0.093 *	
											Vel (f/s)	0.53	0.55	0.61	0.69	0.74	0.79	
											R (Hrs)	0.191	0.186	0.165	0.144	0.132	0.123	
ON20	0.001	0.05	434.8	294.2	NATURAL	0.083	0.15	0.37	6.54	0.140	Tc (Hrs)	0.138	0.134	0.120	0.106	0.099*	0.093 *	
											Vel (f/s)	0.53	0.55	0.61	0.69	0.74	0.79	
											R (Hrs)	0.191	0.186	0.165	0.144	0.132	0.123	
ON25	0.002	0.07	219.2	217.5	NATURAL	0.079	0.15	0.37	6.54	0.140	Tc (Hrs)	0.174	0.170	0.152	0.135	0.125	0.117	
											Vel (f/s)	0.59	0.60	0.68	0.76	0.82	0.88	
											R (Hrs)	0.219	0.213	0.189	0.165	0.152	0.141	
ON30	0.006	0.16	147.4	147.4	Natural	0.072	0.15	0.37	6.54	0.140	Tc (Hrs)	0.283	0.276	0.248	0.219	0.203	0.191	
											Vel (f/s)	0.83	0.85	0.95	1.07	1.16	1.23	
											R (Hrs)	0.389	0.378	0.335	0.292	0.269	0.251	
ON35	0.003	0.15	138.2	138.2	Natural	0.076	0.15	0.37	6.54	0.140	Tc (Hrs)	0.288	0.281	0.252	0.223	0.207	0.194	
											Vel (f/s)	0.76	0.78	0.87	0.99	1.06	1.13	
											R (Hrs)	0.558	0.543	0.481	0.420	0.386	0.360	
ON40	0.006	0.15	140.0	140.0	NATURAL	0.072	0.15	0.39	5.85	0.185	Tc (Hrs)	0.289	0.282	0.253	0.223	0.206	0.192	
											Vel (f/s)	0.76	0.78	0.87	0.99	1.07	1.15	
											R (Hrs)	0.378	0.367	0.326	0.283	0.259	0.240	
ON45	0.001	0.08	171.1	171.1	NATURAL	0.083	0.15	0.35	3.29	0.725	Tc (Hrs)	0.274	0.262	0.222	0.193	0.177	0.164	
											Vel (f/s)	0.43	0.45	0.53	0.61	0.66	0.72	
											R (Hrs)	0.598	0.568	0.474	0.405	0.368	0.339	

* Non default value or value out of range

(stSubBasCG.rpt)

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SOLITUDE		EXISTING CONDITION		HEC-1 MAP		SCOTTSDALE, ARIZONA	
SCALE (H): 1"=200'		SCALE (V): NONE		DESIGNED BY: ZJH		DRAWN BY: ZJH	
CHECKED BY: JMB		DATE: SEP 2019		PROJECT NO.		DRAWING NAME	
1		OF		1		1	
NO.		REVISION		BY		DATE	
APPR.							

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City of Scottsdale
Drainage Design Management System
LAND USE
Project Reference: SOLITUDE EX

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9/25/2019

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
OFF10	DESERT	0.0740	8.2	0.15	0	25.0	DRY	0.042	Desert
	GOLF	0.1020	11.3	0.44	0	60.0	NORMAL	0.023	Golf Course
	R1-18	0.5110	56.8	0.30	27	50.0	NORMAL	0.023	Residential 18,000 sq-ft lots
	R1-43	0.0650	7.2	0.30	17	20.0	NORMAL	0.023	Residential 43,000 sq-ft lots
	UND	0.1480	16.4	0.25	0	0.0	DRY	0.081	Undisturbed natural desert or desert landscaping (no impervi
		0.9000	99.9						
OFF15	DESERT	0.0020	100.0	0.15	0	25.0	DRY	0.076	Desert
		0.0020	100.0						
OFF20	DESERT	0.0080	100.0	0.15	0	25.0	DRY	0.070	Desert
		0.0080	100.0						
OFF30	DESERT	0.0260	74.3	0.15	0	25.0	DRY	0.061	Desert
	R1-43	0.0090	25.7	0.30	17	20.0	NORMAL	0.031	Residential 43,000 sq-ft lots
		0.0350	100.0						
OFF35	DESERT	0.0210	67.7	0.15	0	25.0	DRY	0.062	Desert
	R1-43	0.0100	32.3	0.30	17	20.0	NORMAL	0.032	Residential 43,000 sq-ft lots
		0.0310	100.0						
OFF40	R1-43	0.0040	100.0	0.30	17	20.0	NORMAL	0.037	Residential 43,000 sq-ft lots
		0.0040	100.0						
OFF45	DESERT	0.0220	27.2	0.15	0	25.0	DRY	0.056	Desert
	R1-43	0.0590	72.8	0.30	17	20.0	NORMAL	0.029	Residential 43,000 sq-ft lots

* Non default value

(stLuDataCG.rpt)

City of Scottsdale
Drainage Design Management System
LAND USE
Project Reference: SOLITUDE EX

Page 2

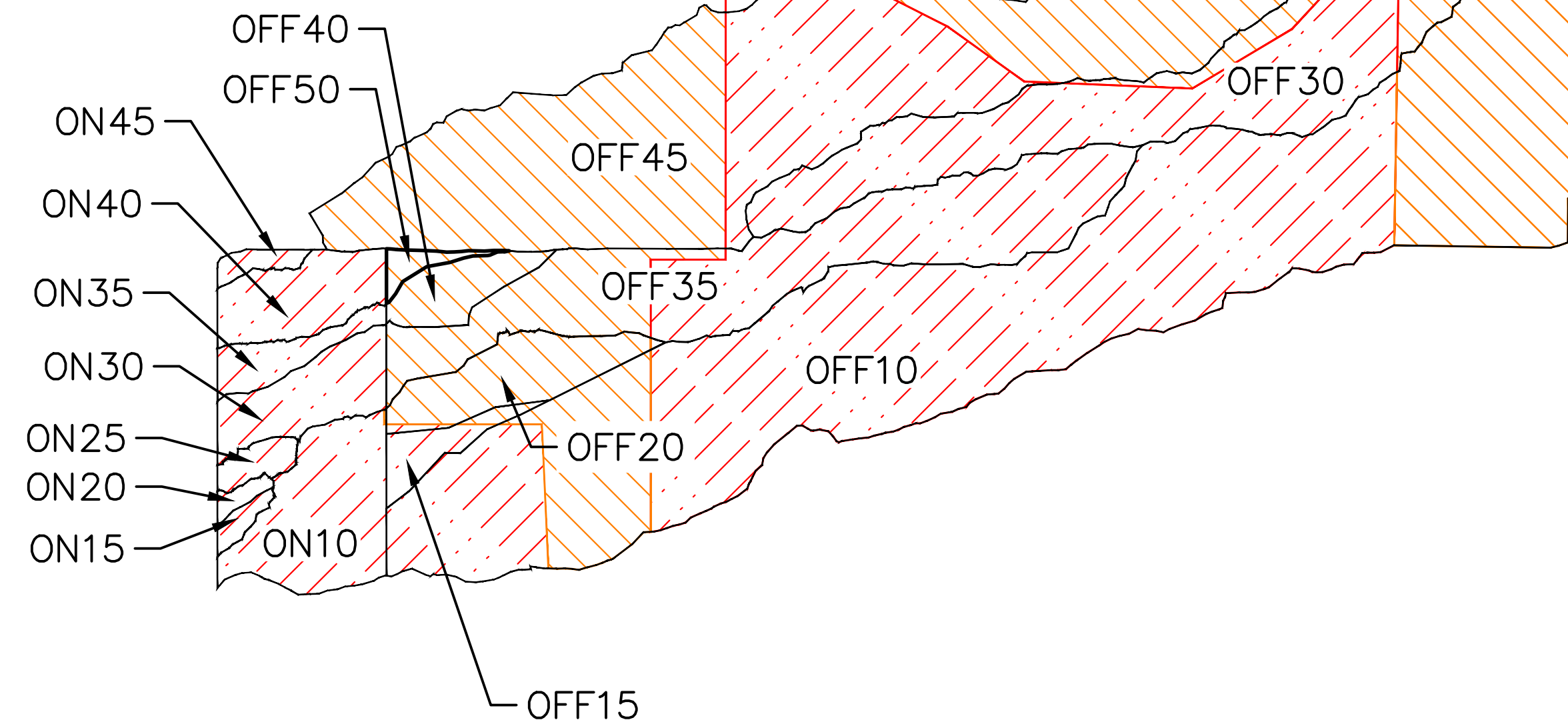
9/25/2019

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
		0.0810	100.0						
OFF50	R1-43	0.0010	100.0	0.30	17	20.0	NORMAL	0.041	Residential 43,000 sq-ft lots
		0.0010	100.0						
ON10	DESERT	0.0110	100.0	0.15	0	25.0	DRY	0.068	Desert
		0.0110	100.0						
ON15	DESERT	0.0010	100.0	0.15	0	25.0	DRY	0.083	Desert
		0.0010	100.0						
ON20	DESERT	0.0010	100.0	0.15	0	25.0	DRY	0.083	Desert
		0.0010	100.0						
ON25	DESERT	0.0010	100.0	0.15	0	25.0	DRY	0.083	Desert
		0.0010	100.0						
ON30	DESERT	0.0060	100.0	0.15	0	25.0	DRY	0.072	Desert
		0.0060	100.0						
ON35	DESERT	0.0030	100.0	0.15	0	25.0	DRY	0.076	Desert
		0.0030	100.0						
ON40	DESERT	0.0060	100.0	0.15	0	25.0	DRY	0.072	Desert
		0.0060	100.0						
ON45	DESERT	0.0010	100.0	0.15	0	25.0	DRY	0.083	Desert
		0.0010	100.0						

* Non default value

(stLuDataCG.rpt)

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XREFS: x03000VVS x03000OBM-PP x03000TP xContours oContours-EG-BAH oContours-EG-MAW REVIEW



LEGEND

OFF10 SUB-BASIN ID


LAND USE



DESERT



R1-43 43,000 SF/LOT



R1-18 18,000 SF/LOT

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GOLF

UND

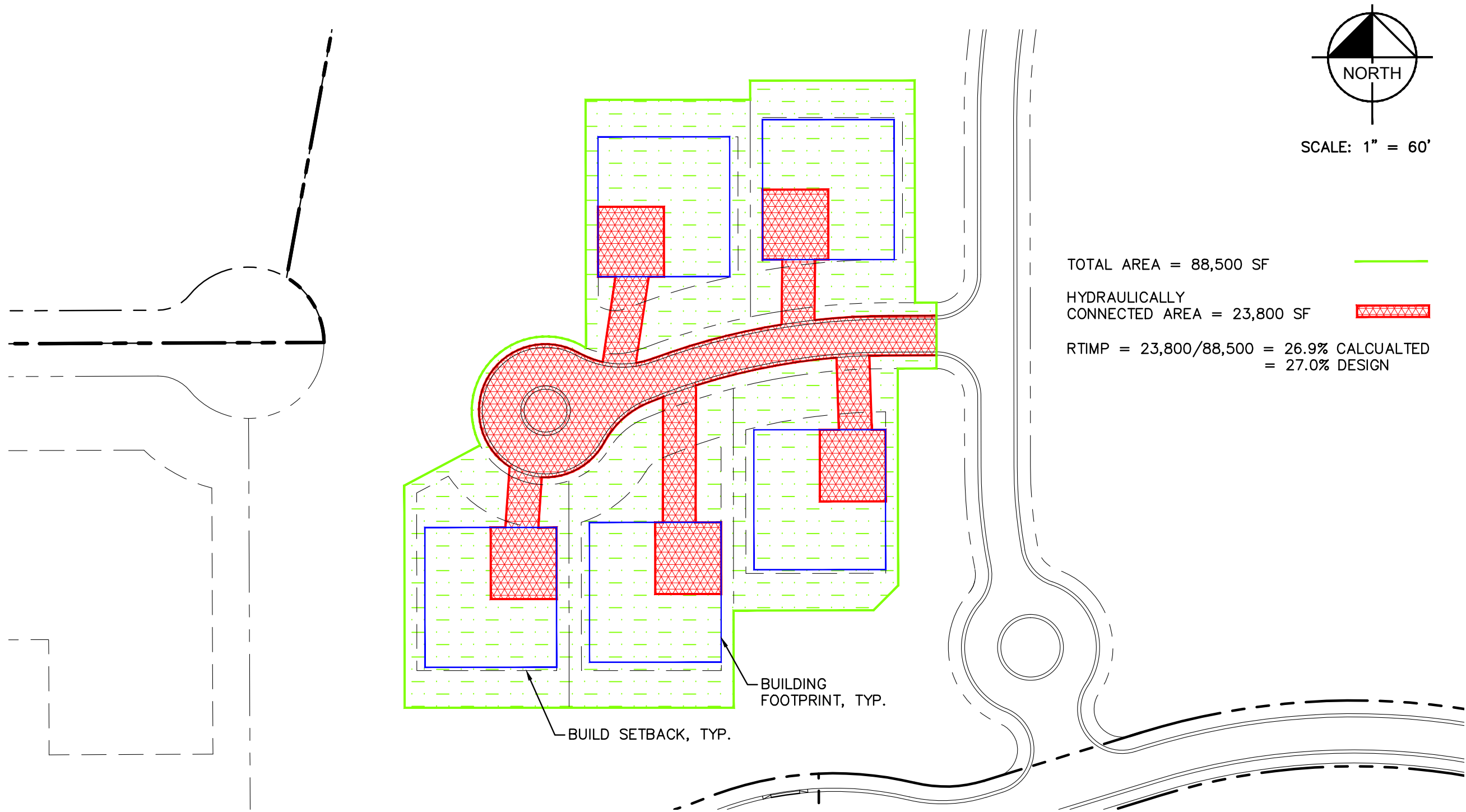
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Phoenix, Arizona 85020 (602) 944-5500

SCALE (H): 1"=500'
SCALE (V): NONE
DESIGNED BY: ZJH
DRAWN BY: ZJH
CHECKED BY: JMN
DATE: SEP 2019

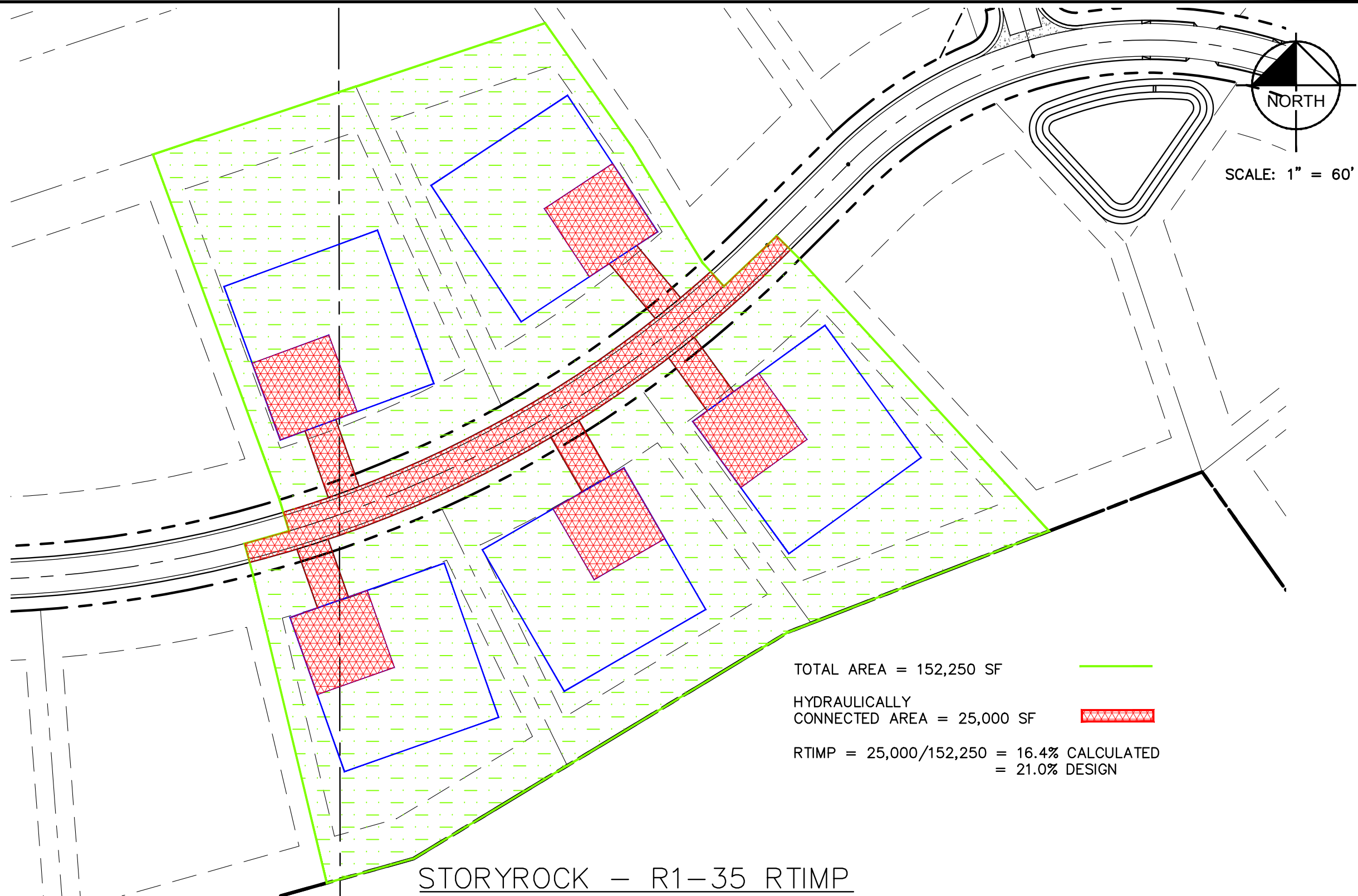
**SOLITUDE
EXISTING CONDITION
HEC 1 - LAND USE MAP
SCOTTSDALE, ARIZONA**

PROJECT NO.	291203001
DRAWING NAME	



STORYROCK — R1-18 RTIMP
DETERMINATION

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XREFS: *88002AR *88002AR-PHA *88002SM-128 *88002SM-PHA *88002SM-PHA-RF *88002AP



STORYROCK — R1-35 RTIMP
DETERMINATION

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City of Scottsdale
Drainage Design Management System
SOILS

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Project Reference: SOLITUDE EX

9/25/2019

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
OFF10	645	33	64533	0.165	18.30	0.230	-	100	
	645	61	64561	0.135	15.00	0.150	-	100	
	645	93	64593	0.168	18.60	0.330	-	100	
	645	121	645121	0.259	28.70	0.120	-	100	
	645	63	64563	0.174	19.30	0.140	25.00	100	
OFF15	645	121	645121	0.003	100.00	0.120	-	100	
OFF20	645	121	645121	0.008	100.00	0.120	-	100	
OFF30	645	121	645121	0.036	100.00	0.120	-	100	
OFF35	645	121	645121	0.031	100.00	0.120	-	100	
OFF40	645	121	645121	0.004	100.00	0.120	-	100	
OFF45	645	121	645121	0.059	72.80	0.120	-	100	
	645	6	6456	0.022	27.20	0.620	-	100	
OFF50	645	121	645121	0.001	100.00	0.120	-	100	
ON10	645	121	645121	0.011	100.00	0.120	-	100	
ON15	645	121	645121	0.001	100.00	0.120	-	100	
ON20	645	121	645121	0.001	100.00	0.120	-	100	
ON25	645	121	645121	0.001	100.00	0.120	-	100	
ON30	645	121	645121	0.006	100.00	0.120	-	100	
ON35	645	121	645121	0.003	100.00	0.120	-	100	
ON40	645	121	645121	0.005	83.30	0.120	-	100	
	645	6	6456	0.001	16.70	0.620	-	100	
ON45	645	6	6456	0.001	100.00	0.620	-	100	

* Non default value

5-SP-2020_V2
12/29/20

City of Scottsdale
Drainage Design Management System
HEC-1 ROUTING DATA
Project Reference: SOLITUDE EX

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Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)		1.	2.	3.	4.	5.	6.	7.	8.
NORMAL DEPTH															
Major Basin 01															
R10-1	0.045	0.045	0.045	800.00	0.0275	-	X:	-	30.00	60.00	65.00	90.00	95.00	110.00	130.00
							Y:	10.20	10.10	10.00	9.00	9.00	10.00	10.10	10.20
R10-2	0.045	0.045	0.045	930.00	0.0300	-	X:	-	10.00	38.00	40.00	42.00	50.00	60.00	70.00
							Y:	10.00	9.90	9.80	9.00	9.80	9.90	10.00	10.00
R10-3	0.045	0.045	0.045	1,000.00	0.0350	-	X:	-	20.00	23.00	26.00	30.00	40.00	50.00	60.00
							Y:	10.20	10.00	10.00	8.00	10.00	10.10	10.20	10.30
R30	0.045	0.045	0.045	825.00	0.0250	-	X:	-	10.00	20.00	23.00	29.00	33.00	40.00	50.00
							Y:	10.20	10.10	10.00	7.00	7.00	10.00	10.10	10.20
R35	0.045	0.045	0.045	805.00	0.0220	-	X:	-	30.00	35.00	38.00	38.50	41.00	45.00	50.00
							Y:	10.10	10.00	9.90	8.70	9.00	9.90	10.00	10.10
R40	0.045	0.045	0.045	780.00	0.0250	-	X:	-	30.00	48.00	52.00	58.00	60.00	70.00	80.00
							Y:	10.10	10.00	9.90	9.20	9.20	9.90	10.00	10.10
R45	0.045	0.045	0.045	395.00	0.0250	-	X:	-	15.00	20.00	25.00	30.00	35.00	40.00	45.00
							Y:	10.10	10.00	9.00	7.50	8.00	8.50	9.00	9.20
RO45	0.045	0.045	0.045	1,773.00	0.0300	-	X:	610.00	620.00	637.00	650.00	663.00	679.00	689.00	697.00
							Y:	139.00	138.70	138.30	138.50	138.80	139.00	138.90	138.90

City of Scottsdale
Drainage Design Management System
HEC-1 DIVERSIONS
Project Reference: SOLITUDE EX

Page 1

9/25/2019

Diversion ID/ DT Card ID	Maximum Volume (ac-ft)	Maximum Diversion (cfs)	1.	2.	3.	4.	5.	6.	7.	8.	9.	10.
DOFF10		Inflow (cfs)		100	200	500	1,000	2,000	4,000	10,000	20,000	50,000
		Diversion (cfs)		85	170	425	850	1,700	3,400	8,500	17,000	42,500

(stHec1Di.rpt)

5-PP-2020_V2
12/29/20

```
1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 29JUL20 TIME 10:49:42
*
*****
```

```
*****
*
* U.S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
```

```

X   X XXXXXXX XXXXX      X
X   X X      X      X    XX
X   X X      X      X    X
XXXXXX XXXX  X      XXXXX X
X   X X      X      X    X
X   X X      X      X    X
X   X XXXXXXX XXXXX      XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE
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NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10						
1	ID	City of Scottsdale															
2	ID	SOLITUDE EX - Solitude Existing Conditions Hydrology															
3	ID	2 YEAR															
4	ID	6 Hour Storm															
5	ID	Unit Hydrograph: Clark															
6	ID	Storm: Multiple															
7	ID	07/29/2020															
8	*DIAGRAM																
9	IT	2	1JAN99	0	2000												
10	IO	5															
11	IN	15															
12	*																
13	JD	1.342	0.0001														
14	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
15	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
16	PC	0.962	0.972	0.983	0.991	1.000											
17	JD	1.334	0.5000														
18	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
19	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
20	PC	0.962	0.972	0.983	0.991	1.000											
21	JD	1.308	2.8														
22	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076						
23	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938						
24	PC	0.950	0.963	0.975	0.988	1.000											
25	*																
26	KK	OFF10	BASIN														
27	BA	0.900															
28	LG	0.30	0.28	5.58	0.23	21											
29	UC	0.766	0.743														
30	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0						
31	UA	100															
32	*																
33	KK	DOFF10	DIVERT														
34	DT	DTFF10	0.0	0.0													
35	DI	0.0	100.0	200.0	500.0	1000.0	2000.0	4000.0	10000.0	20000.0	50000.0						

32	DQ	0.0	85.0	170.0	425.0	850.0	1700.0	3400.0	8500.0	17000.0	42500.0
	*										
33	KK	R10-1	ROUTE								
34	RS	1	FLOW								
35	RC	0.045	0.045	0.045	800	0.0275	0.00				
36	RX	0.00	30.00	60.00	65.00	90.00	95.00	110.00	130.00		
37	RY	10.20	10.10	10.00	9.00	9.00	10.00	10.10	10.20		
	*										
38	KK	OFF15	BASIN								
39	BA	0.003									
40	LG	0.15	0.37	6.54	0.14	0					
41	UC	0.238	0.379								
42	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
43	UA	100									
	*										

1

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44	KK	R10-2	ROUTE								
45	RS	1	FLOW								
46	RC	0.045	0.045	0.045	930	0.0300	0.00				
47	RX	0.00	10.00	38.00	40.00	42.00	50.00	60.00	70.00		
48	RY	10.00	9.90	9.80	9.00	9.80	9.90	10.00	10.00		
	*										
49	KK	OFF20	BASIN								
50	BA	0.008									
51	LG	0.15	0.37	6.54	0.14	0					
52	UC	0.298	0.435								
53	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
54	UA	100									
	*										
55	KK	R10-3	ROUTE								
56	RS	1	FLOW								
57	RC	0.045	0.045	0.045	1000	0.0350	0.00				
58	RX	0.00	20.00	23.00	26.00	30.00	40.00	50.00	60.00		
59	RY	10.20	10.00	10.00	8.00	10.00	10.10	10.20	10.30		
	*										
60	KK	ON10	BASIN								
61	BA	0.011									
62	LG	0.15	0.37	6.54	0.14	0					
63	UC	0.267	0.245								
64	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
65	UA	100									
	*										
66	KK	CP10	COMBINE								
67	HC	4									
	*										
68	KK	ON15	BASIN								
69	BA	0.001									
70	LG	0.15	0.37	6.54	0.14	0					
71	UC	0.138	0.191								
72	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
73	UA	100									
	*										
74	KK	ON20	BASIN								
75	BA	0.001									
76	LG	0.15	0.37	6.54	0.14	0					
77	UC	0.138	0.191								
78	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
79	UA	100									
	*										

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

80	KK	ON25	BASIN									
81	BA	0.002										
82	LG	0.15	0.37	6.54	0.14	0						
83	UC	0.174	0.219									
84	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
85	UA	100										
	*											
86	KK	OFF35	BASIN									
87	BA	0.031										
88	LG	0.20	0.31	6.54	0.14	5						
89	UC	0.519	1.084									
90	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
91	UA	100										
	*											
92	KK	R30	ROUTE									
93	RS	1	FLOW									
94	RC	0.045	0.045	0.045	825	0.0250	0.00					
95	RX	0.00	10.00	20.00	23.00	29.00	33.00	40.00	50.00			
96	RY	10.20	10.10	10.00	7.00	7.00	10.00	10.10	10.20			
	*											
97	KK	ON30	BASIN									
98	BA	0.006										
99	LG	0.15	0.37	6.54	0.14	0						
100	UC	0.283	0.389									
101	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
102	UA	100										
	*											
103	KK	CP30	COMBINE									
104	HC	2										
	*											
105	KK	OFF40	BASIN									
106	BA	0.004										
107	LG	0.30	0.19	6.54	0.13	17						
108	UC	0.166	0.258									
109	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
110	UA	100										
	*											
111	KK	R35	ROUTE									
112	RS	1	FLOW									
113	RC	0.045	0.045	0.045	805	0.0220	0.00					
114	RX	0.00	30.00	35.00	38.00	38.50	41.00	45.00	50.00			
115	RY	10.10	10.00	9.90	8.70	9.00	9.90	10.00	10.10			
	*											

1

HEC-1 INPUT

PAGE 4

LINE	ID	1	2	3	4	5	6	7	8	9	10	
116	KK	ON35	BASIN									
117	BA	0.003										
118	LG	0.15	0.37	6.54	0.14	0						
119	UC	0.288	0.558									
120	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
121	UA	100										
	*											
122	KK	CP35	COMBINE									
123	HC	2										
	*											
124	KK	OFF50	BASIN									
125	BA	0.001										
126	LG	0.30	0.19	6.54	0.13	17						
127	UC	0.126	0.253									
128	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
129	UA	100										
	*											

130	KK	R40	ROUTE								
131	RS	1	FLOW								
132	RC	0.045	0.045	0.045	780	0.0250	0.00				
133	RX	0.00	30.00	48.00	52.00	58.00	60.00	70.00	80.00		
134	RY	10.10	10.00	9.90	9.20	9.20	9.90	10.00	10.10		
	*										
135	KK	ON40	BASIN								
136	BA	0.006									
137	LG	0.15	0.39	5.85	0.19	0					
138	UC	0.289	0.378								
139	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
140	UA	100									
	*										
141	KK	CP40	COMBINE								
142	HC	2									
	*										
143	KK	OFF30	BASIN								
144	BA	0.036									
145	LG	0.19	0.32	6.54	0.14	4					
146	UC	0.453	0.778								
147	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
148	UA	100									
	*										
149	KK	R045	ROUTE								
150	RS	1	FLOW								
151	RC	0.045	0.045	0.045	1773	0.0300	0.00				
152	RX	610.00	620.00	637.00	650.00	663.00	679.00	689.00	697.00		
153	RY	139.00	138.70	138.30	138.50	138.80	139.00	138.90	138.90		
	*										

1

HEC-1 INPUT

PAGE 5

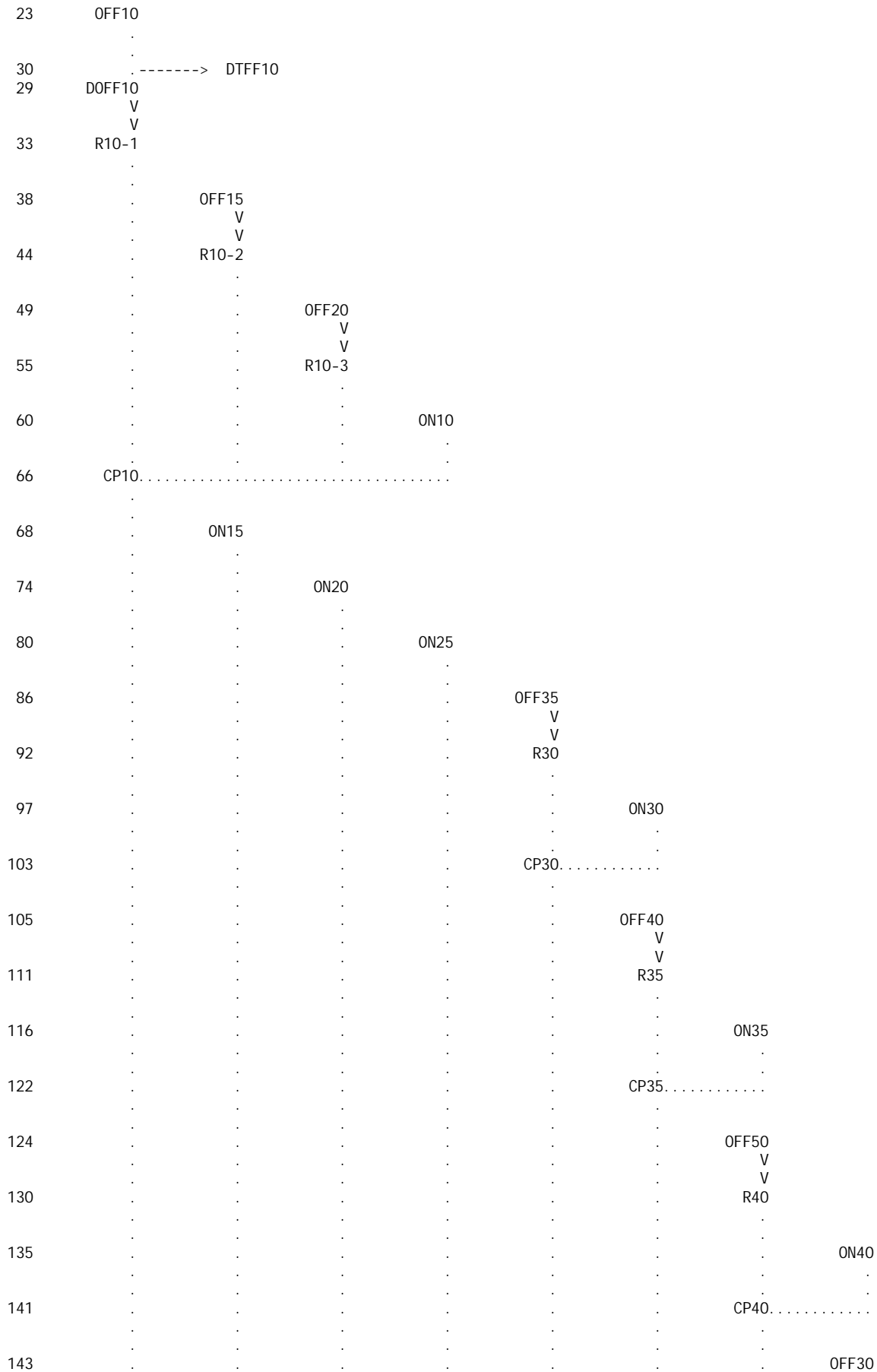
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

154	KK	OFF45	BASIN								
155	BA	0.081									
156	LG	0.26	0.29	5.46	0.21	12					
157	UC	0.442	0.601								
158	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
159	UA	100									
	*										
160	KK	CP045	COMBINE								
161	HC	2									
	*										
162	KK	R45	ROUTE								
163	RS	1	FLOW								
164	RC	0.045	0.045	0.045	395	0.0250	0.00				
165	RX	0.00	15.00	20.00	25.00	30.00	35.00	40.00	45.00		
166	RY	10.10	10.00	9.00	7.50	8.00	8.50	9.00	9.20		
	*										
167	KK	ON45	BASIN								
168	BA	0.001									
169	LG	0.15	0.35	3.29	0.73	0					
170	UC	0.274	0.598								
171	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
172	UA	100									
	*										
173	KK	CP45	COMBINE								
174	HC	2									
	*										
175	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW



	V
	V
149	R045

154	OFF45

160	CP045.....
	V
	V
162	R45

167	ON45

173	CP45.....

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 29JUL20 TIME 10:49:42
*
*****
    
```

```

*****
*
* U. S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
    
```

City of Scottsdale
 SOLITUDE EX - Solitude Existing Conditions Hydrology
 2 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 07/29/2020

```

9 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5 PRINT CONTROL
          IPLOT      0 PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMN        2 MINUTES IN COMPUTATION INTERVAL
          IDATE      1JAN99 STARTING DATE
          ITIME      0000 STARTING TIME
          NQ         2000 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     3JAN99 ENDING DATE
          NDTIME     1838 ENDING TIME
          ICENT      19 CENTURY MARK

          COMPUTATION INTERVAL .03 HOURS
          TOTAL TIME BASE 66.63 HOURS
    
```

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

```

11 JD      INDEX STORM NO. 1
          STRM      1.34 PRECIPITATION DEPTH
          TRDA      .00 TRANSPOSITION DRAINAGE AREA
    
```

```

12 PI      PRECIPITATION PATTERN
          .00 .00 .00 .00 .00 .00 .00 .00 .00
          .00 .00 .00 .00 .00 .00 .00 .00 .00
    
```

[illegible]

15 JD	INDEX STORM NO. 2	
	STRM	1.33 PRECIPITATION DEPTH
	TRDA	.50 TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN

[illegible]

19 JD	INDEX	STORM NO. 3	
	STRM	1. 31	PRECIPITATION DEPTH
	TRDA	2. 80	TRANSPPOSITION DRAINAGE AREA

20 PI PRECIPITATION PATTERN

[illegible]

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

+	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	OFF10	186.	4.57	42.	11.	4.	.90		

+	DIVERSION TO	DTFF10	158.	4.57	36.	9.	3.	.90
+	HYDROGRAPH AT	DOFF10	28.	4.57	6.	2.	1.	.90
+	ROUTED TO	R10-1	27.	4.63	6.	2.	1.	.90
+	HYDROGRAPH AT	OFF15	1.	4.17	0.	0.	0.	.00
+	ROUTED TO	R10-2	1.	4.27	0.	0.	0.	.00
+	HYDROGRAPH AT	OFF20	3.	4.20	0.	0.	0.	.01
+	ROUTED TO	R10-3	3.	4.27	0.	0.	0.	.01
+	HYDROGRAPH AT	ON10	6.	4.17	0.	0.	0.	.01
+	4 COMBINED AT	CP10	30.	4.60	7.	2.	1.	.92
+	HYDROGRAPH AT	ON15	1.	4.07	0.	0.	0.	.00
+	HYDROGRAPH AT	ON20	1.	4.07	0.	0.	0.	.00
+	HYDROGRAPH AT	ON25	1.	4.10	0.	0.	0.	.00
+	HYDROGRAPH AT	OFF35	6.	4.40	1.	0.	0.	.03
+	ROUTED TO	R30	6.	4.47	1.	0.	0.	.03
+	HYDROGRAPH AT	ON30	3.	4.20	0.	0.	0.	.01
+	2 COMBINED AT	CP30	7.	4.43	2.	0.	0.	.04
+	HYDROGRAPH AT	OFF40	3.	4.10	0.	0.	0.	.00
+	ROUTED TO	R35	3.	4.17	0.	0.	0.	.00
+	HYDROGRAPH AT	ON35	1.	4.20	0.	0.	0.	.00
+	2 COMBINED AT	CP35	4.	4.17	0.	0.	0.	.01
+	HYDROGRAPH AT	OFF50	1.	4.07	0.	0.	0.	.00
+	ROUTED TO	R40	1.	4.20	0.	0.	0.	.00
+	HYDROGRAPH AT	ON40	2.	4.20	0.	0.	0.	.01
+	2 COMBINED AT	CP40	3.	4.20	0.	0.	0.	.01
+	HYDROGRAPH AT	OFF30	9.	4.33	2.	0.	0.	.04

+	ROUTED TO	R045	7.	4.60	2.	0.	0.	.04
+	HYDROGRAPH AT	OFF45	25.	4.30	4.	1.	0.	.08
+	2 COMBINED AT	CP045	29.	4.33	5.	1.	1.	.12
+	ROUTED TO	R45	29.	4.37	5.	1.	1.	.12
+	HYDROGRAPH AT	ON45	0.	4.20	0.	0.	0.	.00
+	2 COMBINED AT	CP45	29.	4.37	6.	1.	1.	.12

*** NORMAL END OF HEC-1 ***

```
1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 29JUL20 TIME 10:49:59
*
*****
```

```
*****
*
* U.S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
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X   X   XXXXXX   XXXXX   X
X   X   X       X   X   XX
X   X   X       X       X
XXXXXX XXXX   X       XXXXX X
X   X   X       X       X
X   X   X       X   X   X
X   X   XXXXXX   XXXXX   XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10						
1	ID	City of Scottsdale															
2	ID	SOLITUDE EX - Solitude Existing Conditions Hydrology															
3	ID	10 YEAR															
4	ID	6 Hour Storm															
5	ID	Unit Hydrograph: Clark															
6	ID	Storm: Multiple															
7	ID	07/29/2020															
8	*DIAGRAM																
9	IT	2	1JAN99	0	2000												
10	IO	5															
11	IN	15															
12	JD	1.995	0.0001														
13	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
14	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
15	PC	0.962	0.972	0.983	0.991	1.000											
16	JD	1.983	0.5000														
17	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
18	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
19	PC	0.962	0.972	0.983	0.991	1.000											
20	JD	1.945	2.8														
21	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076						
22	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938						
23	PC	0.950	0.963	0.975	0.988	1.000											
24	*DIAGRAM																
25	KK	OFF10	BASIN														
26	BA	0.900															
27	LG	0.30	0.28	5.58	0.23	21											
28	UC	0.671	0.642														
29	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0						
30	UA	100															
31	*DIAGRAM																
32	KK	DOFF10	DIVERT														
33	DT	DTFF10	0.0	0.0													
34	DI	0.0	100.0	200.0	500.0	1000.0	2000.0	4000.0	10000.0	20000.0	50000.0						

32	DQ	0.0	85.0	170.0	425.0	850.0	1700.0	3400.0	8500.0	17000.0	42500.0
	*										
33	KK	R10-1	ROUTE								
34	RS	1	FLOW								
35	RC	0.045	0.045	0.045	800	0.0275	0.00				
36	RX	0.00	30.00	60.00	65.00	90.00	95.00	110.00	130.00		
37	RY	10.20	10.10	10.00	9.00	9.00	10.00	10.10	10.20		
	*										
38	KK	OFF15	BASIN								
39	BA	0.003									
40	LG	0.15	0.37	6.54	0.14	0					
41	UC	0.209	0.327								
42	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
43	UA	100									
	*										

1

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44	KK	R10-2	ROUTE								
45	RS	1	FLOW								
46	RC	0.045	0.045	0.045	930	0.0300	0.00				
47	RX	0.00	10.00	38.00	40.00	42.00	50.00	60.00	70.00		
48	RY	10.00	9.90	9.80	9.00	9.80	9.90	10.00	10.00		
	*										
49	KK	OFF20	BASIN								
50	BA	0.008									
51	LG	0.15	0.37	6.54	0.14	0					
52	UC	0.261	0.375								
53	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
54	UA	100									
	*										
55	KK	R10-3	ROUTE								
56	RS	1	FLOW								
57	RC	0.045	0.045	0.045	1000	0.0350	0.00				
58	RX	0.00	20.00	23.00	26.00	30.00	40.00	50.00	60.00		
59	RY	10.20	10.00	10.00	8.00	10.00	10.10	10.20	10.30		
	*										
60	KK	ON10	BASIN								
61	BA	0.011									
62	LG	0.15	0.37	6.54	0.14	0					
63	UC	0.234	0.211								
64	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
65	UA	100									
	*										
66	KK	CP10	COMBINE								
67	HC	4									
	*										
68	KK	ON15	BASIN								
69	BA	0.001									
70	LG	0.15	0.37	6.54	0.14	0					
71	UC	0.120	0.165								
72	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
73	UA	100									
	*										
74	KK	ON20	BASIN								
75	BA	0.001									
76	LG	0.15	0.37	6.54	0.14	0					
77	UC	0.120	0.165								
78	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
79	UA	100									
	*										

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

80	KK	ON25	BASIN									
81	BA	0.002										
82	LG	0.15	0.37	6.54	0.14	0						
83	UC	0.152	0.189									
84	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
85	UA	100										
	*											
86	KK	OFF35	BASIN									
87	BA	0.031										
88	LG	0.20	0.31	6.54	0.14	5						
89	UC	0.456	0.937									
90	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
91	UA	100										
	*											
92	KK	R30	ROUTE									
93	RS	1	FLOW									
94	RC	0.045	0.045	0.045	825	0.0250	0.00					
95	RX	0.00	10.00	20.00	23.00	29.00	33.00	40.00	50.00			
96	RY	10.20	10.10	10.00	7.00	7.00	10.00	10.10	10.20			
	*											
97	KK	ON30	BASIN									
98	BA	0.006										
99	LG	0.15	0.37	6.54	0.14	0						
100	UC	0.248	0.335									
101	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
102	UA	100										
	*											
103	KK	CP30	COMBINE									
104	HC	2										
	*											
105	KK	OFF40	BASIN									
106	BA	0.004										
107	LG	0.30	0.19	6.54	0.13	17						
108	UC	0.147	0.226									
109	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
110	UA	100										
	*											
111	KK	R35	ROUTE									
112	RS	1	FLOW									
113	RC	0.045	0.045	0.045	805	0.0220	0.00					
114	RX	0.00	30.00	35.00	38.00	38.50	41.00	45.00	50.00			
115	RY	10.10	10.00	9.90	8.70	9.00	9.90	10.00	10.10			
	*											

1

HEC-1 INPUT

PAGE 4

LINE	ID	1	2	3	4	5	6	7	8	9	10	
116	KK	ON35	BASIN									
117	BA	0.003										
118	LG	0.15	0.37	6.54	0.14	0						
119	UC	0.252	0.481									
120	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
121	UA	100										
	*											
122	KK	CP35	COMBINE									
123	HC	2										
	*											
124	KK	OFF50	BASIN									
125	BA	0.001										
126	LG	0.30	0.19	6.54	0.13	17						
127	UC	0.112	0.222									
128	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
129	UA	100										
	*											

130	KK	R40	ROUTE								
131	RS	1	FLOW								
132	RC	0.045	0.045	0.045	780	0.0250	0.00				
133	RX	0.00	30.00	48.00	52.00	58.00	60.00	70.00	80.00		
134	RY	10.10	10.00	9.90	9.20	9.20	9.90	10.00	10.10		
	*										
135	KK	ON40	BASIN								
136	BA	0.006									
137	LG	0.15	0.39	5.85	0.19	0					
138	UC	0.253	0.326								
139	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
140	UA	100									
	*										
141	KK	CP40	COMBINE								
142	HC	2									
	*										
143	KK	OFF30	BASIN								
144	BA	0.036									
145	LG	0.19	0.32	6.54	0.14	4					
146	UC	0.398	0.672								
147	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
148	UA	100									
	*										
149	KK	R045	ROUTE								
150	RS	1	FLOW								
151	RC	0.045	0.045	0.045	1773	0.0300	0.00				
152	RX	610.00	620.00	637.00	650.00	663.00	679.00	689.00	697.00		
153	RY	139.00	138.70	138.30	138.50	138.80	139.00	138.90	138.90		
	*										

1

HEC-1 INPUT

PAGE 5

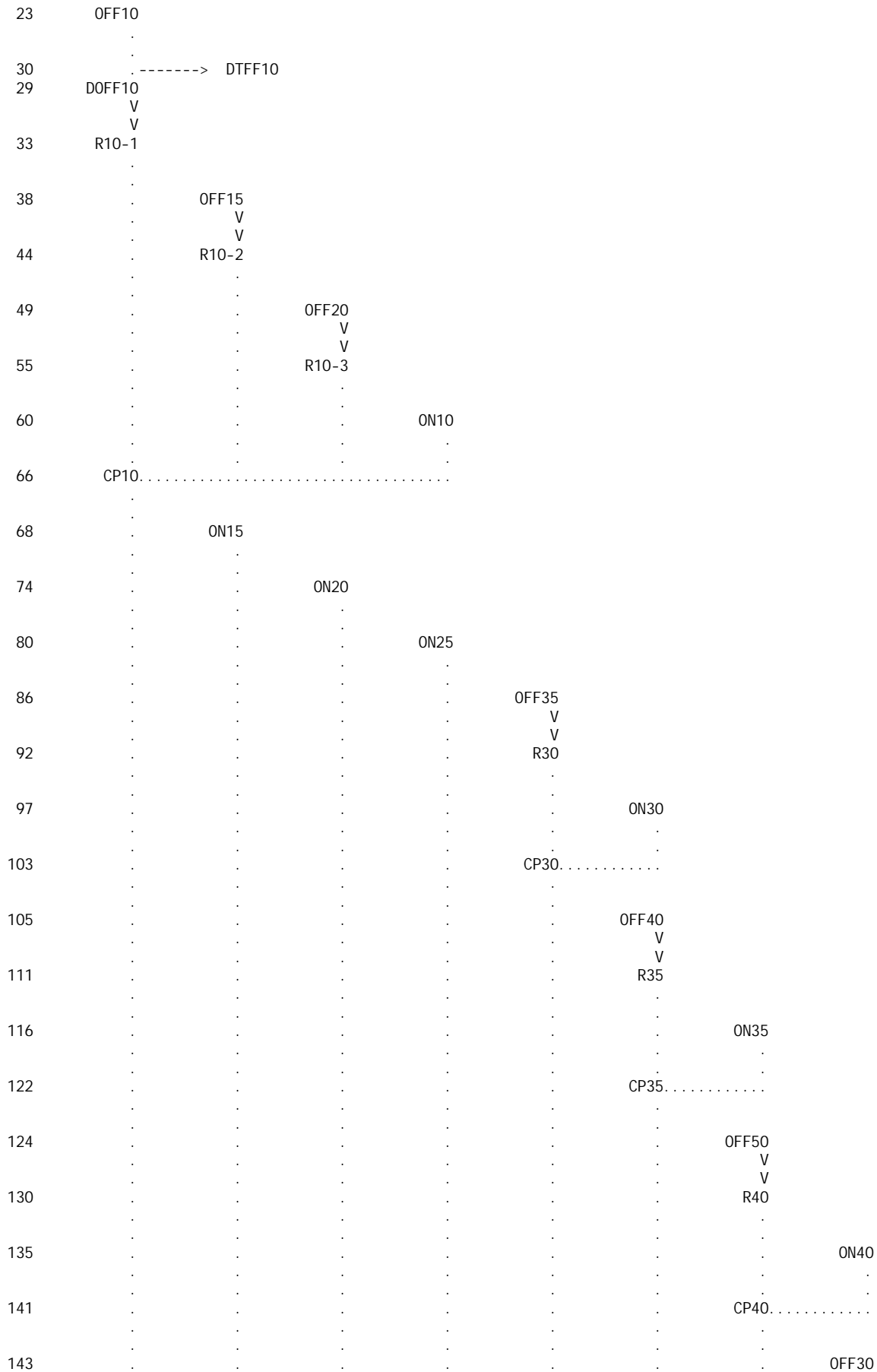
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

154	KK	OFF45	BASIN								
155	BA	0.081									
156	LG	0.26	0.29	5.46	0.21	12					
157	UC	0.392	0.526								
158	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
159	UA	100									
	*										
160	KK	CP045	COMBINE								
161	HC	2									
	*										
162	KK	R45	ROUTE								
163	RS	1	FLOW								
164	RC	0.045	0.045	0.045	395	0.0250	0.00				
165	RX	0.00	15.00	20.00	25.00	30.00	35.00	40.00	45.00		
166	RY	10.10	10.00	9.00	7.50	8.00	8.50	9.00	9.20		
	*										
167	KK	ON45	BASIN								
168	BA	0.001									
169	LG	0.15	0.35	3.29	0.73	0					
170	UC	0.222	0.474								
171	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
172	UA	100									
	*										
173	KK	CP45	COMBINE								
174	HC	2									
	*										
175	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW



	V
	V
149	R045

154	OFF45

160	CP045.....
	V
	V
162	R45

167	ON45

173	CP45.....

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 29JUL20 TIME 10:49:59
*
*****
    
```

```

*****
*
* U. S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
    
```

City of Scottsdale
 SOLITUDE EX - Solitude Existing Conditions Hydrology
 10 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 07/29/2020

```

9 10      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN       2  MINUTES IN COMPUTATION INTERVAL
          IDATE      1JAN99  STARTING DATE
          ITIME      0000  STARTING TIME
          NQ         2000  NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     3JAN99  ENDING DATE
          NDTIME     1838  ENDING TIME
          ICENT      19  CENTURY MARK

          COMPUTATION INTERVAL .03 HOURS
          TOTAL TIME BASE 66.63 HOURS
    
```

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

```

11 JD      INDEX STORM NO. 1
          STRM      2.00  PRECIPITATION DEPTH
          TRDA      .00  TRANSPOSITION DRAINAGE AREA
    
```

```

12 PI      PRECIPITATION PATTERN
          .00      .00      .00      .00      .00      .00      .00      .00      .00
          .00      .00      .00      .00      .00      .00      .00      .00      .00
    
```


[illegible]

15 JD	INDEX STORM NO. 2		
	STRM	1.98	PRECIPITATION DEPTH
	TRDA	.50	TRANSPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN

[illegible]

19 JD	INDEX	STORM NO. 3	
	STRM	1.95	PRECIPITATION DEPTH
	TRDA	2.80	TRANSPOSITION DRAINAGE AREA

20 PI PRECIPITATION PATTERN

[illegible]

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

+	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	OFF10	439.	4. 47	85.	22.	8.	. 90		

+	DIVERSION TO	DTFF10	374.	4.47	72.	18.	7.	.90
+	HYDROGRAPH AT	DOFF10	66.	4.47	13.	3.	1.	.90
+	ROUTED TO	R10-1	65.	4.53	13.	3.	1.	.90
+	HYDROGRAPH AT	OFF15	3.	4.13	0.	0.	0.	.00
+	ROUTED TO	R10-2	3.	4.20	0.	0.	0.	.00
+	HYDROGRAPH AT	OFF20	7.	4.17	1.	0.	0.	.01
+	ROUTED TO	R10-3	7.	4.23	1.	0.	0.	.01
+	HYDROGRAPH AT	ON10	14.	4.13	1.	0.	0.	.01
+	4 COMBINED AT	CP10	72.	4.50	14.	4.	1.	.92
+	HYDROGRAPH AT	ON15	1.	4.07	0.	0.	0.	.00
+	HYDROGRAPH AT	ON20	1.	4.07	0.	0.	0.	.00
+	HYDROGRAPH AT	ON25	3.	4.07	0.	0.	0.	.00
+	HYDROGRAPH AT	OFF35	14.	4.33	3.	1.	0.	.03
+	ROUTED TO	R30	14.	4.40	3.	1.	0.	.03
+	HYDROGRAPH AT	ON30	6.	4.17	1.	0.	0.	.01
+	2 COMBINED AT	CP30	17.	4.37	3.	1.	0.	.04
+	HYDROGRAPH AT	OFF40	6.	4.07	0.	0.	0.	.00
+	ROUTED TO	R35	5.	4.13	0.	0.	0.	.00
+	HYDROGRAPH AT	ON35	2.	4.17	0.	0.	0.	.00
+	2 COMBINED AT	CP35	7.	4.13	1.	0.	0.	.01
+	HYDROGRAPH AT	OFF50	1.	4.07	0.	0.	0.	.00
+	ROUTED TO	R40	1.	4.13	0.	0.	0.	.00
+	HYDROGRAPH AT	ON40	5.	4.17	0.	0.	0.	.01
+	2 COMBINED AT	CP40	6.	4.17	1.	0.	0.	.01
+	HYDROGRAPH AT	OFF30	21.	4.27	3.	1.	0.	.04

+	ROUTED TO	R045	17.	4.50	3.	1.	0.	.04
+	HYDROGRAPH AT	OFF45	55.	4.27	8.	2.	1.	.08
+	2 COMBINED AT	CP045	67.	4.30	11.	3.	1.	.12
+	ROUTED TO	R45	66.	4.30	11.	3.	1.	.12
+	HYDROGRAPH AT	ON45	0.	4.17	0.	0.	0.	.00
+	2 COMBINED AT	CP45	67.	4.30	11.	3.	1.	.12

*** NORMAL END OF HEC-1 ***

```
1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 29JUL20 TIME 10:50:17
*
*****
```

```
*****
*
* U.S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
```

```

X   X XXXXXXX XXXXX      X
X   X X      X      X    XX
X   X X      X      X    X
XXXXXX XXXX  X      XXXXX X
X   X X      X      X    X
X   X X      X      X    X
X   X XXXXXXX XXXXX      XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10						
1	ID	City of Scottsdale															
2	ID	SOLITUDE EX - Solitude Existing Conditions Hydrology															
3	ID	100 YEAR															
4	ID	6 Hour Storm															
5	ID	Unit Hydrograph: Clark															
6	ID	Storm: Multiple															
7	ID	07/29/2020															
8	*DIAGRAM																
9	IT	2	1JAN99	0	2000												
10	IO	5															
11	IN	15															
12	JD	3.024	0.0001														
13	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
14	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
15	PC	0.962	0.972	0.983	0.991	1.000											
16	JD	3.006	0.5000														
17	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
18	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
19	PC	0.962	0.972	0.983	0.991	1.000											
20	JD	2.948	2.8														
21	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076						
22	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938						
23	PC	0.950	0.963	0.975	0.988	1.000											
24	KK	OFF10	BASIN														
25	BA	0.900															
26	LG	0.30	0.28	5.58	0.23	21											
27	UC	0.515	0.478														
28	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0						
29	UA	100															
30	KK	DOFF10	DIVERT														
31	DT	DTFF10	0.0	0.0													
32	DI	0.0	100.0	200.0	500.0	1000.0	2000.0	4000.0	10000.0	20000.0	50000.0						

32	DQ	0.0	85.0	170.0	425.0	850.0	1700.0	3400.0	8500.0	17000.0	42500.0
	*										
33	KK	R10-1	ROUTE								
34	RS	1	FLOW								
35	RC	0.045	0.045	0.045	800	0.0275	0.00				
36	RX	0.00	30.00	60.00	65.00	90.00	95.00	110.00	130.00		
37	RY	10.20	10.10	10.00	9.00	9.00	10.00	10.10	10.20		
	*										
38	KK	OFF15	BASIN								
39	BA	0.003									
40	LG	0.15	0.37	6.54	0.14	0					
41	UC	0.161	0.244								
42	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
43	UA	100									
	*										

1

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44	KK	R10-2	ROUTE								
45	RS	1	FLOW								
46	RC	0.045	0.045	0.045	930	0.0300	0.00				
47	RX	0.00	10.00	38.00	40.00	42.00	50.00	60.00	70.00		
48	RY	10.00	9.90	9.80	9.00	9.80	9.90	10.00	10.00		
	*										
49	KK	OFF20	BASIN								
50	BA	0.008									
51	LG	0.15	0.37	6.54	0.14	0					
52	UC	0.201	0.281								
53	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
54	UA	100									
	*										
55	KK	R10-3	ROUTE								
56	RS	1	FLOW								
57	RC	0.045	0.045	0.045	1000	0.0350	0.00				
58	RX	0.00	20.00	23.00	26.00	30.00	40.00	50.00	60.00		
59	RY	10.20	10.00	10.00	8.00	10.00	10.10	10.20	10.30		
	*										
60	KK	ON10	BASIN								
61	BA	0.011									
62	LG	0.15	0.37	6.54	0.14	0					
63	UC	0.180	0.158								
64	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
65	UA	100									
	*										
66	KK	CP10	COMBINE								
67	HC	4									
	*										
68	KK	ON15	BASIN								
69	BA	0.001									
70	LG	0.15	0.37	6.54	0.14	0					
71	UC	0.093	0.123								
72	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
73	UA	100									
	*										
74	KK	ON20	BASIN								
75	BA	0.001									
76	LG	0.15	0.37	6.54	0.14	0					
77	UC	0.093	0.123								
78	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
79	UA	100									
	*										

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

80	KK	ON25	BASIN									
81	BA	0.002										
82	LG	0.15	0.37	6.54	0.14	0						
83	UC	0.117	0.141									
84	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
85	UA	100										
	*											
86	KK	OFF35	BASIN									
87	BA	0.031										
88	LG	0.20	0.31	6.54	0.14	5						
89	UC	0.355	0.711									
90	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
91	UA	100										
	*											
92	KK	R30	ROUTE									
93	RS	1	FLOW									
94	RC	0.045	0.045	0.045	825	0.0250	0.00					
95	RX	0.00	10.00	20.00	23.00	29.00	33.00	40.00	50.00			
96	RY	10.20	10.10	10.00	7.00	7.00	10.00	10.10	10.20			
	*											
97	KK	ON30	BASIN									
98	BA	0.006										
99	LG	0.15	0.37	6.54	0.14	0						
100	UC	0.191	0.251									
101	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
102	UA	100										
	*											
103	KK	CP30	COMBINE									
104	HC	2										
	*											
105	KK	OFF40	BASIN									
106	BA	0.004										
107	LG	0.30	0.19	6.54	0.13	17						
108	UC	0.118	0.176									
109	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
110	UA	100										
	*											
111	KK	R35	ROUTE									
112	RS	1	FLOW									
113	RC	0.045	0.045	0.045	805	0.0220	0.00					
114	RX	0.00	30.00	35.00	38.00	38.50	41.00	45.00	50.00			
115	RY	10.10	10.00	9.90	8.70	9.00	9.90	10.00	10.10			
	*											

1

HEC-1 INPUT

PAGE 4

LINE	ID	1	2	3	4	5	6	7	8	9	10	
116	KK	ON35	BASIN									
117	BA	0.003										
118	LG	0.15	0.37	6.54	0.14	0						
119	UC	0.194	0.360									
120	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
121	UA	100										
	*											
122	KK	CP35	COMBINE									
123	HC	2										
	*											
124	KK	OFF50	BASIN									
125	BA	0.001										
126	LG	0.30	0.19	6.54	0.13	17						
127	UC	0.090	0.173									
128	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
129	UA	100										
	*											

130	KK	R40	ROUTE								
131	RS	1	FLOW								
132	RC	0.045	0.045	0.045	780	0.0250	0.00				
133	RX	0.00	30.00	48.00	52.00	58.00	60.00	70.00	80.00		
134	RY	10.10	10.00	9.90	9.20	9.20	9.90	10.00	10.10		
	*										
135	KK	ON40	BASIN								
136	BA	0.006									
137	LG	0.15	0.39	5.85	0.19	0					
138	UC	0.192	0.240								
139	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
140	UA	100									
	*										
141	KK	CP40	COMBINE								
142	HC	2									
	*										
143	KK	OFF30	BASIN								
144	BA	0.036									
145	LG	0.19	0.32	6.54	0.14	4					
146	UC	0.309	0.509								
147	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
148	UA	100									
	*										
149	KK	R045	ROUTE								
150	RS	1	FLOW								
151	RC	0.045	0.045	0.045	1773	0.0300	0.00				
152	RX	610.00	620.00	637.00	650.00	663.00	679.00	689.00	697.00		
153	RY	139.00	138.70	138.30	138.50	138.80	139.00	138.90	138.90		
	*										

1

HEC-1 INPUT

PAGE 5

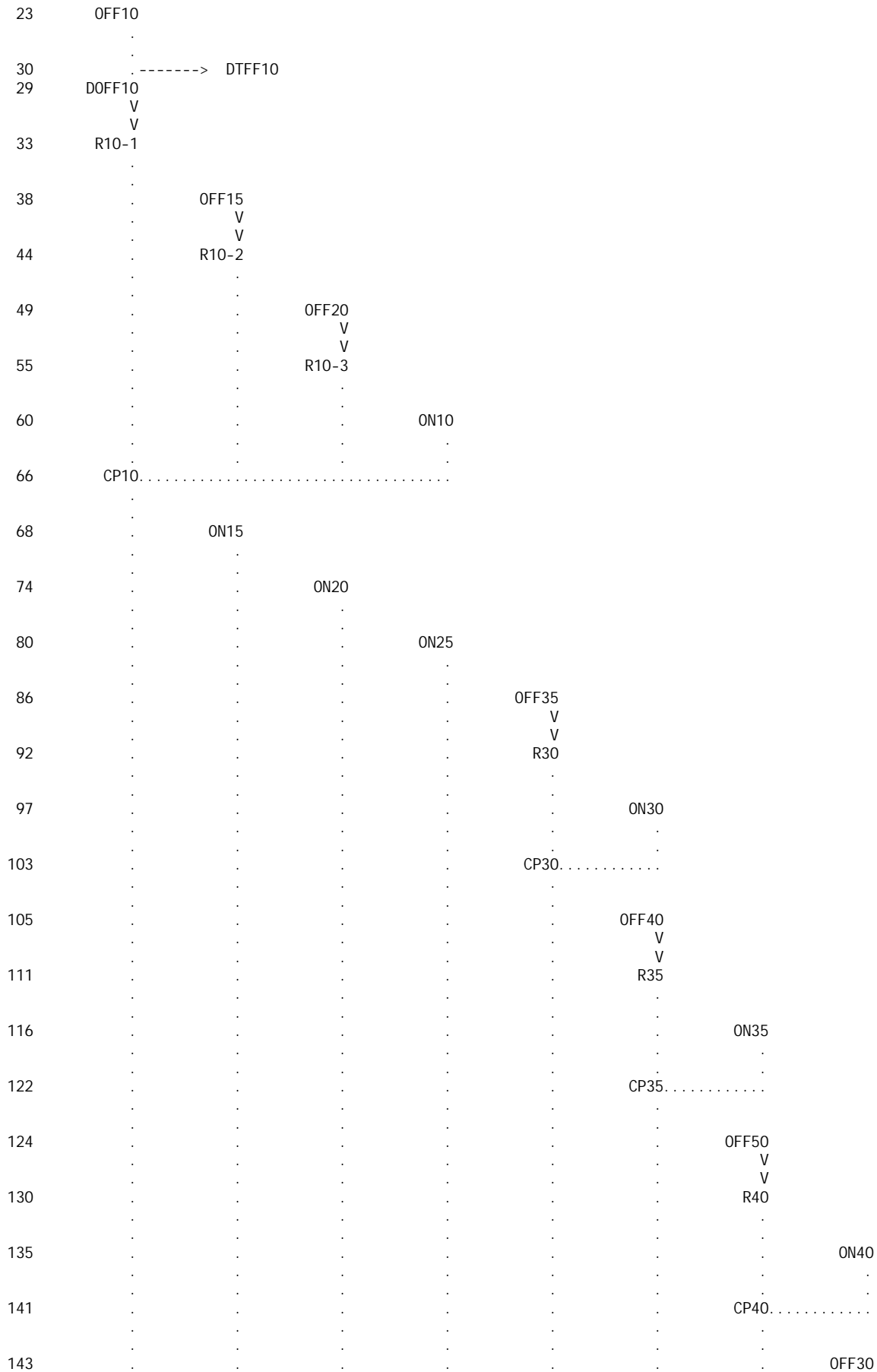
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

154	KK	OFF45	BASIN								
155	BA	0.081									
156	LG	0.26	0.29	5.46	0.21	12					
157	UC	0.303	0.396								
158	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
159	UA	100									
	*										
160	KK	CP045	COMBINE								
161	HC	2									
	*										
162	KK	R45	ROUTE								
163	RS	1	FLOW								
164	RC	0.045	0.045	0.045	395	0.0250	0.00				
165	RX	0.00	15.00	20.00	25.00	30.00	35.00	40.00	45.00		
166	RY	10.10	10.00	9.00	7.50	8.00	8.50	9.00	9.20		
	*										
167	KK	ON45	BASIN								
168	BA	0.001									
169	LG	0.15	0.35	3.29	0.73	0					
170	UC	0.164	0.339								
171	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
172	UA	100									
	*										
173	KK	CP45	COMBINE								
174	HC	2									
	*										
175	ZZ										

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW



	V
	V
149	R045

154	OFF45

160	CP045.....
	V
	V
162	R45

167	ON45

173	CP45.....

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 29JUL20 TIME 10:50:17
*
*****
    
```

```

*****
*
* U. S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
    
```

City of Scottsdale
 SOLITUDE EX - Solitude Existing Conditions Hydrology
 100 YEAR
 6 Hour Storm
 Unit Hydrograph: Clark
 Storm: Multiple
 07/29/2020

```

9 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5 PRINT CONTROL
          IPLOT      0 PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMN        2 MINUTES IN COMPUTATION INTERVAL
          IDATE      1JAN99 STARTING DATE
          ITIME      0000 STARTING TIME
          NQ         2000 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     3JAN99 ENDING DATE
          NDTIME     1838 ENDING TIME
          ICENT      19 CENTURY MARK

          COMPUTATION INTERVAL .03 HOURS
          TOTAL TIME BASE 66.63 HOURS
    
```

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

```

11 JD      INDEX STORM NO. 1
          STRM      3.02 PRECIPITATION DEPTH
          TRDA      .00 TRANSPOSITION DRAINAGE AREA
    
```

```

12 PI      PRECIPITATION PATTERN
          .00 .00 .00 .00 .00 .00 .00 .00 .00
          .00 .00 .00 .00 .00 .00 .00 .00 .00
    
```

[illegible]

15 JD	INDEX STORM NO. 2		
	STRM	3.01	PRECIPITATION DEPTH
	TRDA	.50	TRANSPPOSITION DRAINAGE AREA

16 PI PRECIPITATION PATTERN

[illegible]

19 JD	INDEX STORM NO. 3	
	STRM	2.95 PRECIPITATION DEPTH
	TRDA	2.80 TRANSPOSITION DRAINAGE AREA

20 PI PRECIPITATION PATTERN

[illegible]

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+					6-HOUR	24-HOUR	72-HOUR			
	HYDROGRAPH AT									
+		OFF10	1028.	4. 33	166.	42.	15.	. 90		

+	DIVERSION TO	DTFF10	874.	4.33	141.	35.	13.	.90
+	HYDROGRAPH AT	DOFF10	154.	4.33	25.	6.	2.	.90
+	ROUTED TO	R10-1	152.	4.40	25.	6.	2.	.90
+	HYDROGRAPH AT	OFF15	6.	4.10	1.	0.	0.	.00
+	ROUTED TO	R10-2	4.	4.40	1.	0.	0.	.00
+	HYDROGRAPH AT	OFF20	15.	4.13	1.	0.	0.	.01
+	ROUTED TO	R10-3	15.	4.17	1.	0.	0.	.01
+	HYDROGRAPH AT	ON10	27.	4.10	2.	0.	0.	.01
+	4 COMBINED AT	CP10	172.	4.37	28.	7.	3.	.92
+	HYDROGRAPH AT	ON15	3.	4.03	0.	0.	0.	.00
+	HYDROGRAPH AT	ON20	3.	4.03	0.	0.	0.	.00
+	HYDROGRAPH AT	ON25	5.	4.03	0.	0.	0.	.00
+	HYDROGRAPH AT	OFF35	33.	4.27	6.	1.	1.	.03
+	ROUTED TO	R30	32.	4.30	6.	1.	1.	.03
+	HYDROGRAPH AT	ON30	12.	4.10	1.	0.	0.	.01
+	2 COMBINED AT	CP30	40.	4.27	7.	2.	1.	.04
+	HYDROGRAPH AT	OFF40	10.	4.07	1.	0.	0.	.00
+	ROUTED TO	R35	10.	4.10	1.	0.	0.	.00
+	HYDROGRAPH AT	ON35	5.	4.13	1.	0.	0.	.00
+	2 COMBINED AT	CP35	15.	4.10	1.	0.	0.	.01
+	HYDROGRAPH AT	OFF50	3.	4.03	0.	0.	0.	.00
+	ROUTED TO	R40	2.	4.10	0.	0.	0.	.00
+	HYDROGRAPH AT	ON40	12.	4.10	1.	0.	0.	.01
+	2 COMBINED AT	CP40	14.	4.10	1.	0.	0.	.01
+	HYDROGRAPH AT	OFF30	48.	4.20	7.	2.	1.	.04

+	ROUTED TO	R045	39.	4.37	7.	2.	1.	.04
+	HYDROGRAPH AT	OFF45	124.	4.20	15.	4.	1.	.08
+	2 COMBINED AT	CP045	154.	4.20	21.	5.	2.	.12
+	ROUTED TO	R45	154.	4.23	21.	5.	2.	.12
+	HYDROGRAPH AT	ON45	1.	4.10	0.	0.	0.	.00
+	2 COMBINED AT	CP45	155.	4.23	22.	5.	2.	.12

*** NORMAL END OF HEC-1 ***

Proposed Condition

City of Scottsdale
Drainage Design Management System
SUB BASINS

Project Reference: SOLITUDE PROP

Page 1

7/29/2020

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)		2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin ID: 01																		
OFF10	0.900	3.21	245.5	237.0	Natural	0.034	0.30	0.28	5.58	0.231	21	Tc (Hrs)	0.766*	0.749*	0.671*	0.594*	0.550*	0.515 *
												Vel (f/s)	6.15	6.29	7.02	7.93	8.56	9.14
												R (Hrs)	0.743	0.724	0.642	0.560	0.514	0.478
OFF15	0.003	0.12	176.5	176.5	Natural	0.076	0.15	0.37	6.54	0.140		Tc (Hrs)	0.238	0.233	0.209	0.185	0.171	0.161
												Vel (f/s)	0.74	0.76	0.84	0.95	1.03	1.09
												R (Hrs)	0.379	0.369	0.327	0.285	0.262	0.244
OFF20	0.008	0.21	184.0	184.0	Natural	0.070	0.15	0.37	6.54	0.140		Tc (Hrs)	0.298	0.291	0.261	0.231	0.214	0.201
												Vel (f/s)	1.03	1.06	1.18	1.33	1.44	1.53
												R (Hrs)	0.435	0.423	0.375	0.327	0.301	0.281
OFF30	0.036	0.71	204.2	204.0	Natural	0.054	0.19	0.32	6.54	0.138	4	Tc (Hrs)	0.453*	0.443*	0.398	0.354	0.329	0.309
												Vel (f/s)	2.30	2.35	2.62	2.94	3.17	3.37
												R (Hrs)	0.778	0.758	0.672	0.590	0.545	0.509
OFF35	0.031	0.80	147.5	147.5	Natural	0.052	0.20	0.31	6.54	0.138	5	Tc (Hrs)	0.519*	0.508*	0.456*	0.406	0.378	0.355
												Vel (f/s)	2.26	2.31	2.57	2.89	3.10	3.31
												R (Hrs)	1.084	1.056	0.937	0.824	0.761	0.711
OFF40	0.004	0.15	179.3	179.3	Natural	0.037	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.166	0.163	0.147	0.133	0.125	0.118
												Vel (f/s)	1.33	1.35	1.50	1.65	1.76	1.86
												R (Hrs)	0.258	0.251	0.226	0.201	0.187	0.176
OFF45	0.081	0.95	194.1	194.1	Natural	0.037	0.26	0.29	5.46	0.212	12	Tc (Hrs)	0.442*	0.432*	0.392	0.349	0.324	0.303
												Vel (f/s)	3.15	3.23	3.55	3.99	4.30	4.60
												R (Hrs)	0.601	0.586	0.526	0.463	0.425	0.396
OFF50	0.001	0.08	187.5	187.5	Natural	0.041	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.126	0.124	0.112	0.101	0.095*	0.090 *
												Vel (f/s)	0.93	0.95	1.05	1.16	1.24	1.30
												R (Hrs)	0.253	0.247	0.222	0.198	0.184	0.173
ON10	0.003	0.15	145.7	145.7	Natural	0.076	0.15	0.37	6.54	0.140		Tc (Hrs)	0.283	0.276	0.248	0.219	0.203	0.191
												Vel (f/s)	0.78	0.80	0.89	1.00	1.08	1.15
												R (Hrs)	0.548	0.533	0.472	0.412	0.379	0.353

* Non default value or value out of range

(stSubBasCG.rpt)

City of Scottsdale
Drainage Design Management System
SUB BASINS

Project Reference: SOLITUDE PROP

Page 2

7/29/2020

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)		2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin ID: 01																		
ON11	0.004	0.11	87.7	87.7	NATURAL	0.037	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.178	0.174	0.158	0.142	0.133	0.126
												Vel (f/s)	0.91	0.93	1.02	1.14	1.21	1.28
												R (Hrs)	0.217	0.211	0.189	0.169	0.157	0.148
ON15	0.001	0.07	76.9	76.9	Natural	0.041	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.156	0.152	0.138	0.125	0.117	0.110
												Vel (f/s)	0.66	0.68	0.74	0.82	0.88	0.93
												R (Hrs)	0.287	0.280	0.251	0.224	0.209	0.196
ON20	0.001	0.08	66.7	66.7	Natural	0.041	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.174	0.170	0.154	0.139	0.131	0.123
												Vel (f/s)	0.67	0.69	0.76	0.84	0.90	0.95
												R (Hrs)	0.362	0.352	0.316	0.282	0.263	0.247
ON25	0.001	0.07	121.2	121.2	NATURAL	0.041	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.135	0.132	0.120	0.108	0.102	0.096 *
												Vel (f/s)	0.76	0.78	0.86	0.95	1.01	1.07
												R (Hrs)	0.246	0.239	0.215	0.192	0.178	0.168
ON26	0.008	0.18	44.4	44.4	NATURAL	0.036	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.277	0.271	0.246	0.222	0.208	0.196
												Vel (f/s)	0.95	0.97	1.07	1.19	1.27	1.35
												R (Hrs)	0.354	0.345	0.309	0.276	0.257	0.241
ON27	0.001	0.08	177.2	177.2	NATURAL	0.083	0.15	0.37	6.54	0.140		Tc (Hrs)	0.204	0.199	0.178	0.158	0.146	0.137
												Vel (f/s)	0.58	0.59	0.66	0.74	0.80	0.86
												R (Hrs)	0.430	0.419	0.371	0.323	0.298	0.277
ON28	0.001	0.10	168.4	168.4	NATURAL	0.083	0.15	0.37	6.54	0.140		Tc (Hrs)	0.231	0.226	0.202	0.179	0.166	0.156
												Vel (f/s)	0.63	0.65	0.73	0.82	0.88	0.94
												R (Hrs)	0.592	0.576	0.511	0.445	0.410	0.382
ON30	0.002	0.10	40.4	40.4	Natural	0.039	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.222	0.217	0.196	0.177	0.166	0.157
												Vel (f/s)	0.66	0.68	0.75	0.83	0.88	0.93
												R (Hrs)	0.380	0.371	0.333	0.297	0.276	0.260
ON35	0.003	0.08	105.3	105.3	Natural	0.038	0.30	0.19	6.54	0.133	17	Tc (Hrs)	0.145	0.142	0.129	0.116	0.109	0.103
												Vel (f/s)	0.81	0.83	0.91	1.01	1.08	1.14
												R (Hrs)	0.158	0.154	0.138	0.123	0.115	0.108

* Non default value or value out of range

(stSubBasCG.rpt)

City of Scottsdale
Drainage Design Management System
SUB BASINS

Project Reference: SOLITUDE PROP

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7/29/2020

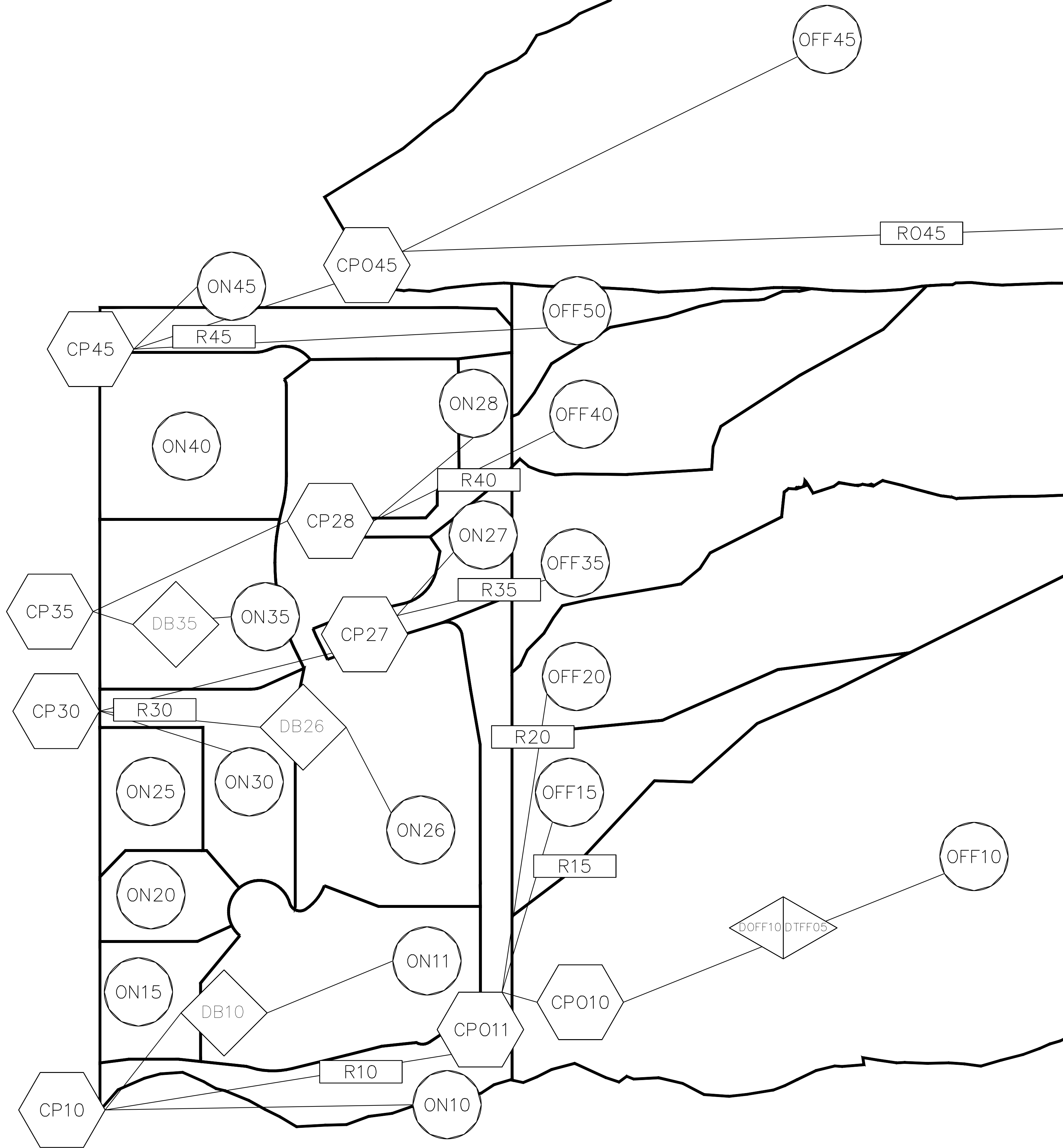
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	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)		2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin ID: 01																		
ON40	0.003	0.10	84.2	84.2	NATURAL	0.038	0.30	0.25	4.72	0.303	17	Tc (Hrs)	0.189	0.184	0.168	0.151	0.140	0.131
												Vel (f/s)	0.78	0.80	0.87	0.97	1.05	1.12
												R (Hrs)	0.252	0.246	0.221	0.197	0.181	0.168
ON45	0.001	0.12	146.3	146.3	NATURAL	0.083	0.15	0.35	4.72	0.319		Tc (Hrs)	0.290	0.282	0.252	0.223	0.205	0.190
												Vel (f/s)	0.61	0.62	0.70	0.79	0.86	0.93
												R (Hrs)	0.879	0.853	0.753	0.659	0.600	0.552

* Non default value or value out of range

(stSubBasCG.rpt)

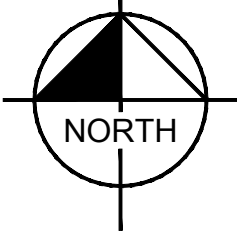
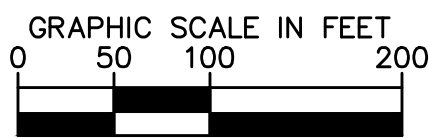
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12/29/20

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XREFS: - MUST TOGETHER WITH THE CONCEPTS AND DESIGN PRESENTED HEREON, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE
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- LEGEND**
- SUB-BASIN BOUNDARY
 - HEC-1 SUB-BASIN ID
 - HEC-1 ROUTE ID
 - HEC-1 DIVERSION
 - HEC-1 CONCENTRATION POINT
 - HEC-1 STORAGE

OFF-SITE
SEE EXISTING
CONDITION
HEC-1 MAP



PROJECT NO. 291203001		REVISION		NO.		BY DATE		APPR.	
DRAWING NAME									
1 OF 1									

SOLITUDE

PROPOSED CONDITION

HEC-1 MAP

SCOTTSDALE, ARIZONA

Kimley»Horn

© 2019 KIMLEY-HORN AND ASSOCIATES, INC.
7740 North 16th Street, Suite 300
Phoenix, Arizona 85020 (602) 944-5500

SCALE (H): 1"=100'

SCALE (V): NONE

DESIGNED BY: ZJH

DRAWN BY: ZJH

CHECKED BY: JMB

DATE: SEP 2019

City of Scottsdale
Drainage Design Management System
LAND USE
Project Reference: SOLITUDE PROP

Page 1

7/29/2020

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
OFF10	DESERT	0.0740	8.2	0.15	0	25.0	DRY	0.042	Desert
	GOLF	0.1020	11.3	0.44	0	60.0	NORMAL	0.023	Golf Course
	R1-18	0.5110	56.8	0.30	27	50.0	NORMAL	0.023	Residential 18,000 sq-ft lots
	R1-43	0.0650	7.2	0.30	17	20.0	NORMAL	0.023	Residential 43,000 sq-ft lots
	UND	0.1480	16.4	0.25	0	0.0	DRY	0.081	Undisturbed natural desert or desert landscaping (no impervi
		0.9000	99.9						
OFF15	DESERT	0.0020	100.0	0.15	0	25.0	DRY	0.076	Desert
		0.0020	100.0						
OFF20	DESERT	0.0080	100.0	0.15	0	25.0	DRY	0.070	Desert
		0.0080	100.0						
OFF30	DESERT	0.0260	74.3	0.15	0	25.0	DRY	0.061	Desert
	R1-43	0.0090	25.7	0.30	17	20.0	NORMAL	0.031	Residential 43,000 sq-ft lots
		0.0350	100.0						
OFF35	DESERT	0.0210	67.7	0.15	0	25.0	DRY	0.062	Desert
	R1-43	0.0100	32.3	0.30	17	20.0	NORMAL	0.032	Residential 43,000 sq-ft lots
		0.0310	100.0						
OFF40	R1-43	0.0040	100.0	0.30	17	20.0	NORMAL	0.037	Residential 43,000 sq-ft lots
		0.0040	100.0						
OFF45	DESERT	0.0220	27.2	0.15	0	25.0	DRY	0.056	Desert
	R1-43	0.0590	72.8	0.30	17	20.0	NORMAL	0.029	Residential 43,000 sq-ft lots

* Non default value

(stLuDataCG.rpt)

City of Scottsdale
Drainage Design Management System
LAND USE
Project Reference: SOLITUDE PROP

Page 2

7/29/2020

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
		0.0810	100.0						
OFF50	R1-43	0.0010	100.0	0.30	17	20.0	NORMAL	0.041	Residential 43,000 sq-ft lots
		0.0010	100.0						
ON10	DESERT	0.0030	100.0	0.15	0	25.0	DRY	0.076	Desert
		0.0030	100.0						
ON11	R1-43	0.0040	100.0	0.30	17	20.0	NORMAL	0.037	Residential 43,000 sq-ft lots
		0.0040	100.0						
ON15	R1-43	0.0010	100.0	0.30	17	20.0	NORMAL	0.041	Residential 43,000 sq-ft lots
		0.0010	100.0						
ON20	R1-43	0.0010	100.0	0.30	17	20.0	NORMAL	0.041	Residential 43,000 sq-ft lots
		0.0010	100.0						
ON25	R1-43	0.0010	100.0	0.30	17	20.0	NORMAL	0.041	Residential 43,000 sq-ft lots
		0.0010	100.0						
ON26	R1-43	0.0080	100.0	0.30	17	20.0	NORMAL	0.036	Residential 43,000 sq-ft lots
		0.0080	100.0						
ON27	DESERT	0.0010	100.0	0.15	0	25.0	DRY	0.083	Desert
		0.0010	100.0						
ON28	DESERT	0.0010	100.0	0.15	0	25.0	DRY	0.083	Desert
		0.0010	100.0						
ON30	R1-43	0.0020	100.0	0.30	17	20.0	NORMAL	0.039	Residential 43,000 sq-ft lots

* Non default value

(stLuDataCG.rpt)

City of Scottsdale
Drainage Design Management System
LAND USE
Project Reference: SOLITUDE PROP

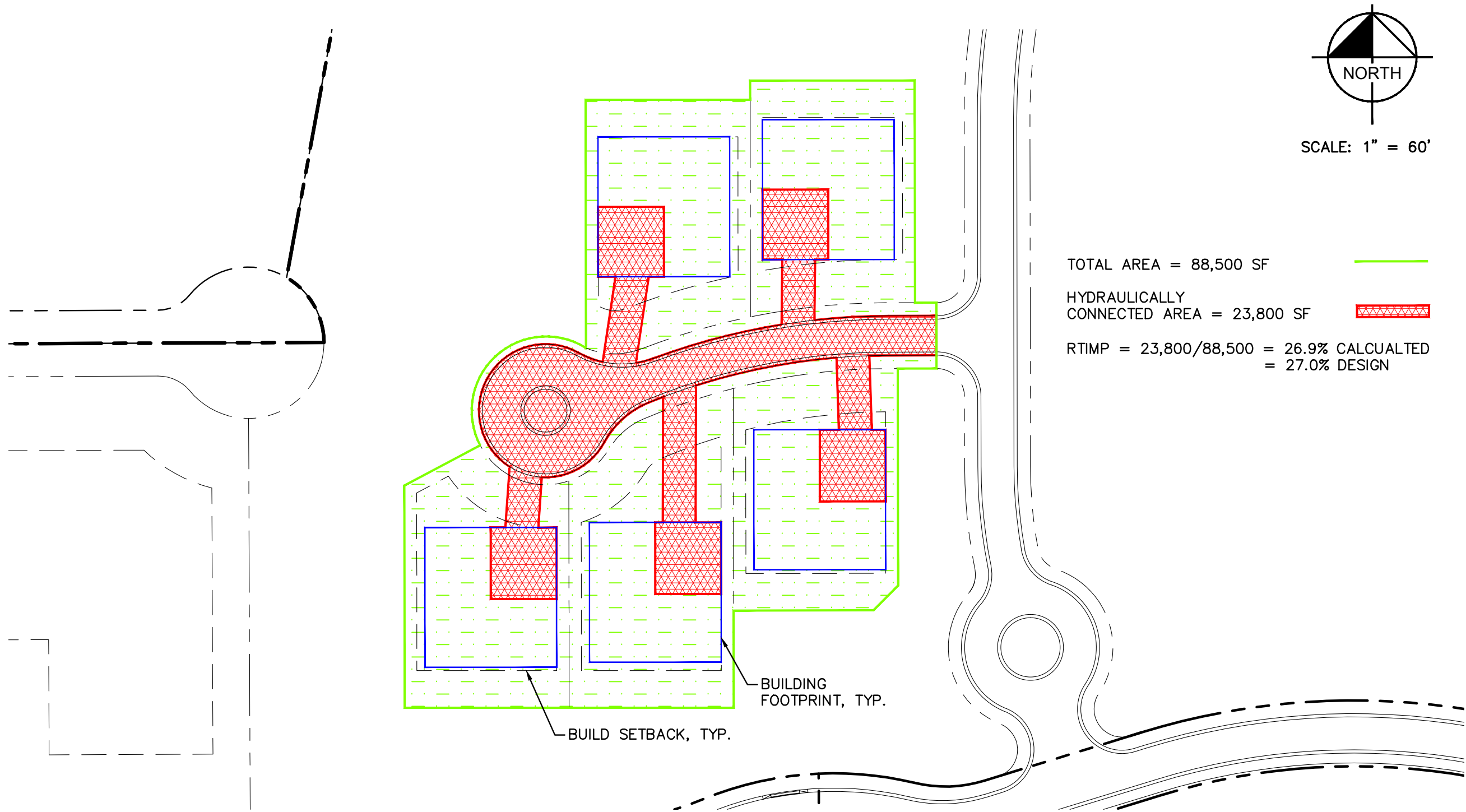
Page 3

7/29/2020

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
		0.0020	100.0						
ON35	R1-43	0.0030	100.0	0.30	17	20.0	NORMAL	0.038	Residential 43,000 sq-ft lots
		0.0030	100.0						
ON40	R1-43	0.0030	100.0	0.30	17	20.0	NORMAL	0.038	Residential 43,000 sq-ft lots
		0.0030	100.0						
ON45	DESERT	0.0010	100.0	0.15	0	25.0	DRY	0.083	Desert
		0.0010	100.0						

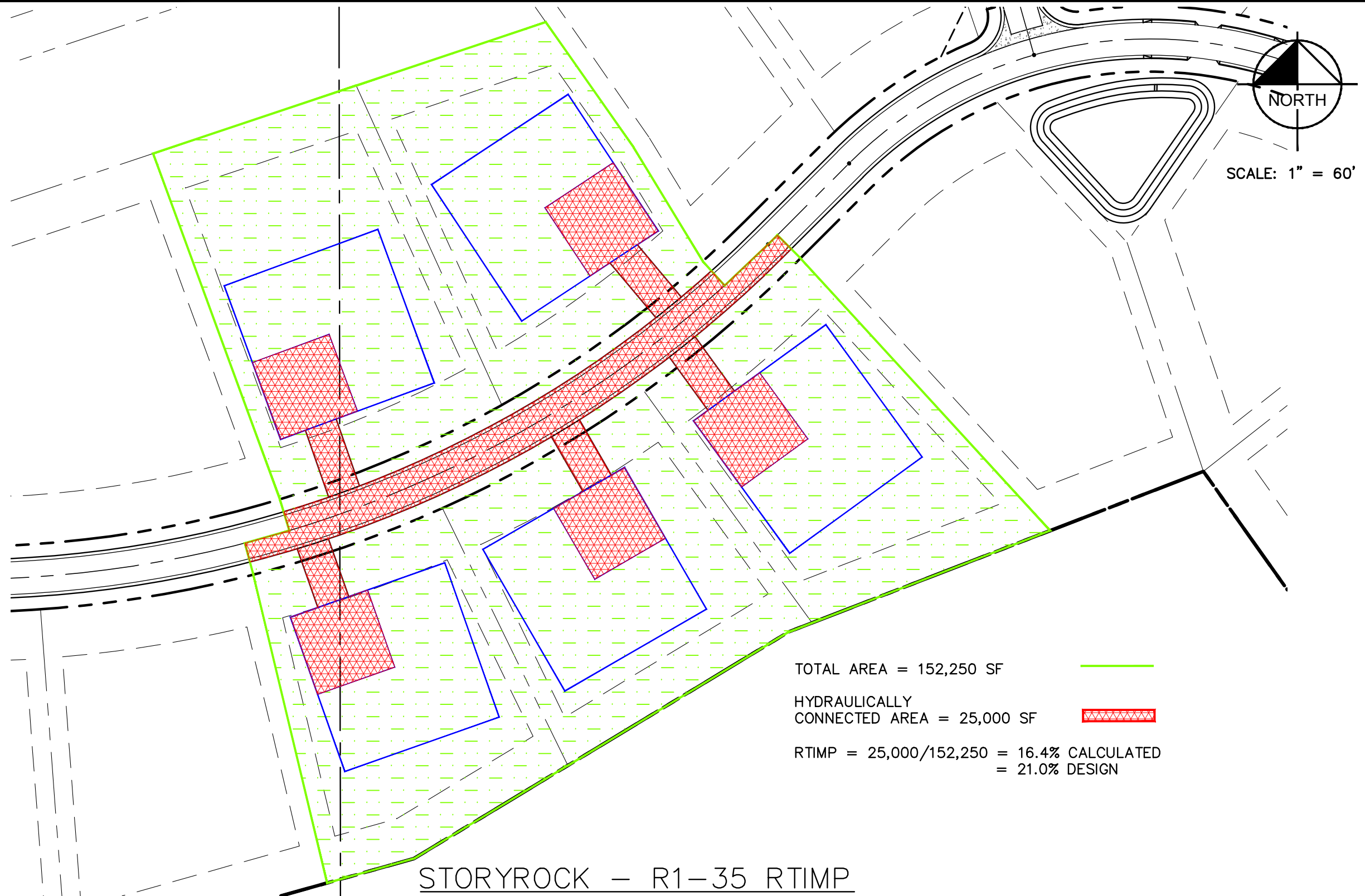
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(stLuDataCG.rpt)



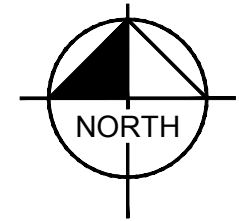
STORYROCK — R1-18 RTIMP
DETERMINATION

K:\EAV_Civil\10108002 - Storyrock\Drawings\Phase 1A\Figures\Working\Storyrock_Test_Li.dwg May 02, 2017 zsh:hl
XREFS: *88002AR *88002AR-PHA *88002AR-128 *88002AR-PHA *88002AR-PHA-Ref *88002AR



STORYROCK — R1-35 RTIMP
DETERMINATION

Kimley»Horn



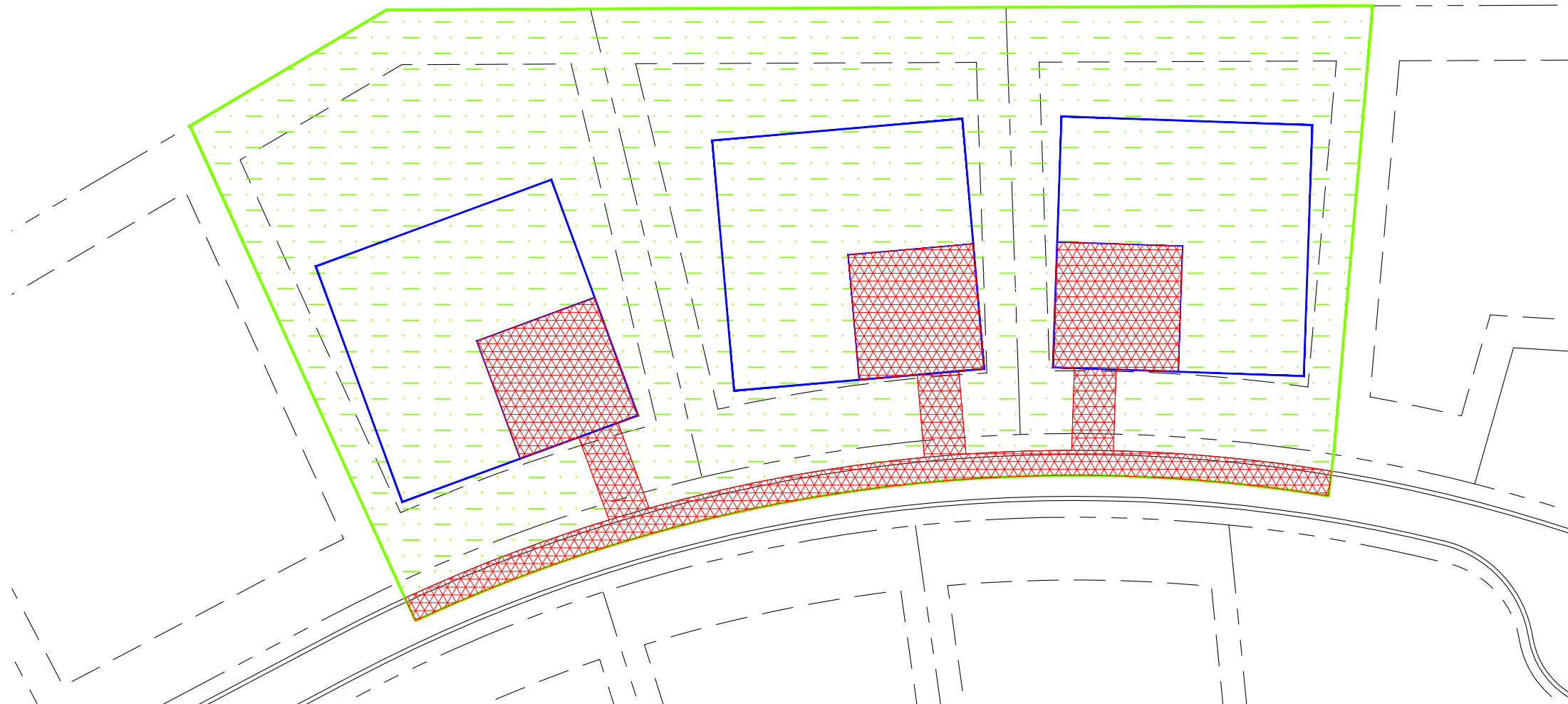
SCALE: 1" = 60'

TOTAL AREA = 123,750 SF

HYDRAULICALLY
CONNECTED AREA = 19,000 SF



RTIMP = $19,000 / 123,750 = 15.4\%$ CALCULATED
17.0% DESIGN



STORYROCK — R1-43 RTIMP
DETERMINATION

Kimley»Horn

City of Scottsdale
Drainage Design Management System
SOILS

Page 1

Project Reference: SOLITUDE PROP

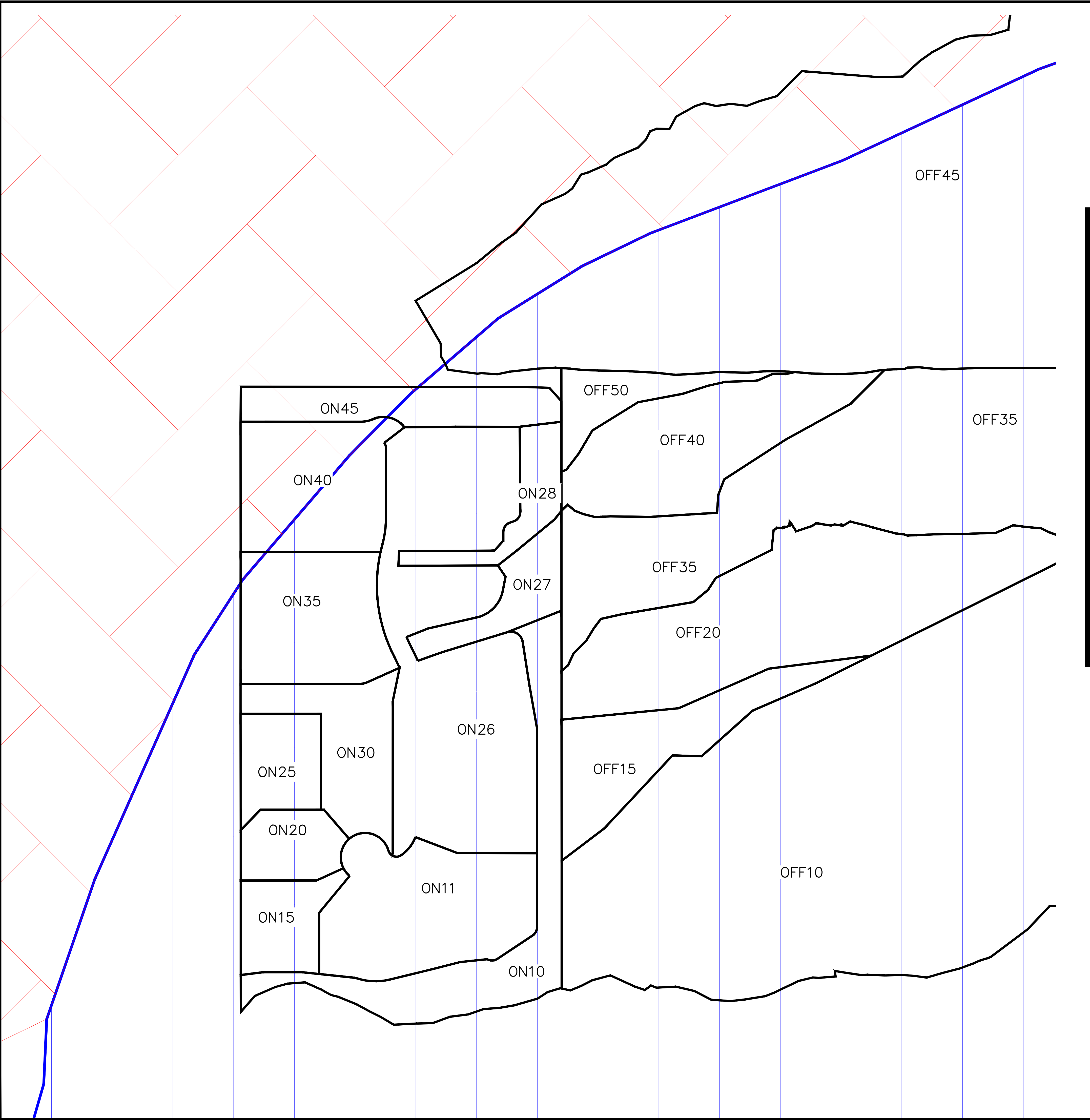
7/29/2020

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
OFF10	645	33	64533	0.165	18.30	0.230	-	100	
	645	61	64561	0.135	15.00	0.150	-	100	
	645	93	64593	0.168	18.60	0.330	-	100	
	645	121	645121	0.259	28.70	0.120	-	100	
	645	63	64563	0.174	19.30	0.140	25.00	100	
OFF15	645	121	645121	0.003	100.00	0.120	-	100	
OFF20	645	121	645121	0.008	100.00	0.120	-	100	
OFF30	645	121	645121	0.036	100.00	0.120	-	100	
OFF35	645	121	645121	0.031	100.00	0.120	-	100	
OFF40	645	121	645121	0.004	100.00	0.120	-	100	
OFF45	645	121	645121	0.059	72.80	0.120	-	100	
	645	6	6456	0.022	27.20	0.620	-	100	
OFF50	645	121	645121	0.001	100.00	0.120	-	100	
ON10	645	121	645121	0.003	100.00	0.120	-	100	
ON11	645	121	645121	0.004	100.00	0.120	-	100	
ON15	645	121	645121	0.001	100.00	0.120	-	100	
ON20	645	121	645121	0.001	100.00	0.120	-	100	
ON25	645	121	645121	0.001	100.00	0.120	-	100	
ON26	645	121	645121	0.008	100.00	0.120	-	100	
ON27	645	121	645121	0.001	100.00	0.120	-	100	
ON28	645	121	645121	0.001	100.00	0.120	-	100	
ON30	645	121	645121	0.002	100.00	0.120	-	100	
ON35	645	121	645121	0.003	100.00	0.120	-	100	
ON40	645	6	6456	0.001	50.00	0.620	-	100	
	645	121	645121	0.001	50.00	0.120	-	100	
ON45	645	6	6456	0.001	50.00	0.620	-	100	
	645	121	645121	0.001	50.00	0.120	-	100	

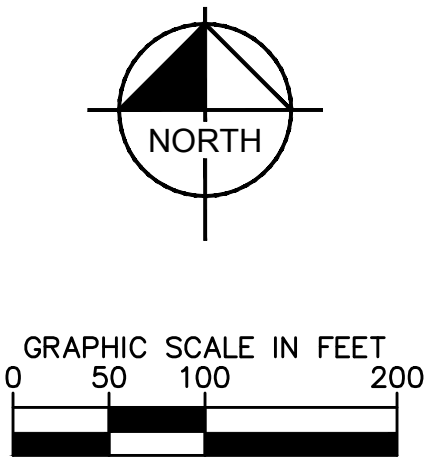
* Non default value

5-SEP-2020_V2
12/29/20



K:\EAV_Civil\291203001 - HV92\Drainage\Preliminary\Figures\Prop Soils.dwg Sep 25, 2019 zoch.hill
XREFS: MUST TOGETHER WITH THE CONCEPTS AND DESIGN PRESENTED HEREON, AS AN INSTRUMENT OF SERVICE, IS INTENDED ONLY FOR THE SPECIFIC PURPOSE AND CLIENT FOR WHICH IT WAS PREPARED. REUSE OF AND IMPROPER RELIANCE ON THIS DOCUMENT WITHOUT WRITTEN AUTHORIZATION AND ADAPTATION BY KIMLEY-HORN AND ASSOCIATES, INC. SHALL BE WITHOUT LIABILITY TO KIMLEY-HORN AND ASSOCIATES, INC.



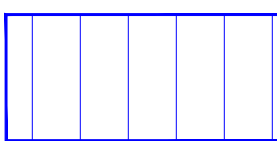
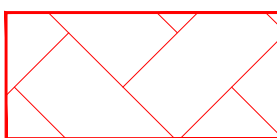
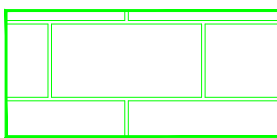
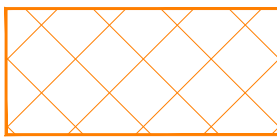

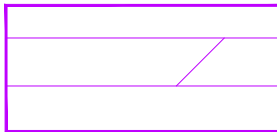
OFF-SITE
SEE EXISTING
CONDITION
SOILS MAP



LEGEND

-  SUB-BASIN BOUNDARY
-  SUB-BASIN ID

SOIL TYPES

-  121
-  6
-  93
-  63
-  33
-  61

SOLITUDE		PROJECT NO. 291203001	
PROPOSED CONDITION		DRAWING NAME	
HEC 1 - SOILS MAP		1 OF 1	
SCOTTSDALE, ARIZONA		12/29/20	
SCALE (H): 1"=100'		5-PP-2020_V2	
SCALE (V): NONE		12/29/20	
DESIGNED BY: ZJH		12/29/20	
DRAWN BY: ZJH		12/29/20	
CHECKED BY: JMB		12/29/20	
DATE: SEP 2019		12/29/20	
© 2019 KIMLEY-HORN AND ASSOCIATES, INC.		12/29/20	
7740 North 16th Street, Suite 300		12/29/20	
Phoenix, Arizona 85020 (602) 944-5500		12/29/20	
Kimley»Horn		12/29/20	
REVISION		12/29/20	
NO.		12/29/20	
BY DATE		12/29/20	
APPR.		12/29/20	

City of Scottsdale
Drainage Design Management System
HEC-1 STORAGE FACILITIES

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Storage Basin ID:		DB10										
Spillway Characteristics (SS)			<u>1</u>	<u>2</u>	<u>3</u>	<u>4</u>	<u>5</u>	<u>6</u>	<u>7</u>	<u>8</u>	<u>9</u>	<u>10</u>
Spillway Crest Elevation:	-NA-	Volume (ac-ft)		-	-	0.1	0.1	0.1	0.2	0.2		
Spillway Length:	-NA-	Discharge (cfs)	0	0	0	1	1	1	1	15	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	0.5	1.0	1.5	2.0	2.5	3.0	3.0	-	-
Weir Coefficient:	-NA-											
Low-Level Outlet (SL)			<u>11</u>	<u>12</u>	<u>13</u>	<u>14</u>	<u>15</u>	<u>16</u>	<u>17</u>	<u>18</u>	<u>19</u>	<u>20</u>
Centerline Elevation:	-NA-	Volume (ac-ft)	-	-	-	-	-	-	-	-	-	-
Cross-Section Area:	-NA-	Discharge (cfs)	0	0	0	0	0	0	0	0	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	-	-	-	-	-	-	-	-	-
Orifice Equation Exponent:	-NA-											
Top of Dam Overflow (ST)			<u>2 Yr</u>	<u>5 Yr</u>	<u>10 Yr</u>	<u>25 Yr</u>	<u>50 Yr</u>	<u>100 Yr</u>				
Elevation Top of Dam:	-NA-	Peak Volume (ac-ft)	0.07	0.00	0.07	0.00	0.00	0.19				
Length of Dam:	-NA-	Peak Stage (ft)	1.50	0.00	1.50	0.00	0.00	3.00				
Discharge Coefficient:	-NA-	Peak Discharge (cfs)	1.00	0.00	1.00	0.00	0.00	11.00				
Weir Coefficient:	-NA-											
Storage Basin ID:		DB26										
Spillway Characteristics (SS)			<u>1</u>	<u>2</u>	<u>3</u>	<u>4</u>	<u>5</u>	<u>6</u>	<u>7</u>	<u>8</u>	<u>9</u>	<u>10</u>
Spillway Crest Elevation:	-NA-	Volume (ac-ft)		0.1	0.2	0.3	0.4	0.6	0.7	0.7		
Spillway Length:	-NA-	Discharge (cfs)	0	0	0	1	1	2	2	2	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	0.5	1.0	1.5	2.0	2.5	2.9	3.0	-	-
Weir Coefficient:	-NA-											
Low-Level Outlet (SL)			<u>11</u>	<u>12</u>	<u>13</u>	<u>14</u>	<u>15</u>	<u>16</u>	<u>17</u>	<u>18</u>	<u>19</u>	<u>20</u>
Centerline Elevation:	-NA-	Volume (ac-ft)	-	-	-	-	-	-	-	-	-	-
Cross-Section Area:	-NA-	Discharge (cfs)	0	0	0	0	0	0	0	0	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	-	-	-	-	-	-	-	-	-
Orifice Equation Exponent:	-NA-											
Top of Dam Overflow (ST)			<u>2 Yr</u>	<u>5 Yr</u>	<u>10 Yr</u>	<u>25 Yr</u>	<u>50 Yr</u>	<u>100 Yr</u>				
Elevation Top of Dam:	-NA-	Peak Volume (ac-ft)	0.25	0.00	0.30	0.00	0.00	0.56				
Length of Dam:	-NA-	Peak Stage (ft)	1.31	0.00	1.50	0.00	0.00	2.50				
Discharge Coefficient:	-NA-	Peak Discharge (cfs)	0.00	0.00	1.00	0.00	0.00	2.00				
Weir Coefficient:	-NA-											

City of Scottsdale
Drainage Design Management System
HEC-1 STORAGE FACILITIES

Page 2

7/29/2020

Storage Basin ID:		DB35										
Spillway Characteristics (SS)			<u>1</u>	<u>2</u>	<u>3</u>	<u>4</u>	<u>5</u>	<u>6</u>	<u>7</u>	<u>8</u>	<u>9</u>	<u>10</u>
Spillway Crest Elevation:	-NA-	Volume (ac-ft)		-	0.1	0.1	0.1	0.2	0.2	0.2		
Spillway Length:	-NA-	Discharge (cfs)	0	0	0	1	1	2	2	2	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	0.5	1.0	1.5	2.0	2.5	2.9	3.0	-	-
Weir Coefficient:	-NA-											
Low-Level Outlet (SL)			<u>11</u>	<u>12</u>	<u>13</u>	<u>14</u>	<u>15</u>	<u>16</u>	<u>17</u>	<u>18</u>	<u>19</u>	<u>20</u>
Centerline Elevation:	-NA-	Volume (ac-ft)	-	-	-	-	-	-	-	-	-	-
Cross-Section Area:	-NA-	Discharge (cfs)	0	0	0	0	0	0	0	0	0	0
Discharge Coefficient:	-NA-	Elevation (ft)	-	-	-	-	-	-	-	-	-	-
Orifice Equation Exponent:	-NA-											
Top of Dam Overflow (ST)			<u>2 Yr</u>	<u>5 Yr</u>	<u>10 Yr</u>	<u>25 Yr</u>	<u>50 Yr</u>	<u>100 Yr</u>				
Elevation Top of Dam:	-NA-	Peak Volume (ac-ft)	0.10	0.00	0.10	0.00	0.00	0.18				
Length of Dam:	-NA-	Peak Stage (ft)	1.50	0.00	1.50	0.00	0.00	2.50				
Discharge Coefficient:	-NA-	Peak Discharge (cfs)	1.00	0.00	1.00	0.00	0.00	2.00				
Weir Coefficient:	-NA-											

Project **Solitude**
 Subject **Detention Basin Calculations**
 Designed by **ZJH** Date 9/24/2019 Project No. 291203001
 Checked by **JMB** Date 9/24/2019

Objective: to determine the storage-flow relationship for small detention basins

DB10

Drains in 4.31 hours

Outlet Diameter 0.50 ft Outlet X-Sect Area 0.196 ft²
 Outlet Elevation 0.5 ft No. of Outlet Barrels 1
 Outlet Pipe Slope 0.005 ft/ft

Elevation	Surface Storage Area	Surface Storage Area	Average Area	Δ Elev	Δ Vol	Σ Vol	Δ Time to Drain	Q _{pipe}	Q _{weir}	Total Q _{out}
[ft]	[ft ²]	[acre]	[acre]	[ft]	[ac-ft]	[ac-ft]	[hr]	[cfs]	[cfs]	[cfs]
0	1,200	0.03	0.04	1.0	0.04	0	2.38	0	0	0
1	2,200	0.05	0.06	1.0	0.06	0.04	1.05	0	0	0
2	3,300	0.08	0.09	1.0	0.09	0.10	0.88	1	0	1
3	4,500	0.10				0.19		1	0	1

Notes:

Q_{pipe} goes from Mannings Eqn to Orifice Eqn when water surface exceeds 1.2*(Outlet Diameter)
 per Linsley et al. *Water Resources Engineering* 4th Edition, pg 652.

Project **Solitude**
 Subject **Detention Basin Calculations**
 Designed by **ZJH** Date 9/24/2019 Project No. 291203001
 Checked by **JMB** Date 9/24/2019

Objective: to determine the storage-flow relationship for small detention basins

DB26

Drains in 9.42 hours

Outlet Diameter 0.66 ft Outlet X-Sect Area 0.342 ft²
 Outlet Elevation 0.5 ft No. of Outlet Barrels 1
 Outlet Pipe Slope 0.005 ft/ft

Elevation	Surface Storage Area	Surface Storage Area	Average Area	Δ Elev	Δ Vol	Σ Vol	Δ Time to Drain	Q _{pipe}	Q _{weir}	Total Q _{out}
[ft]	[ft ²]	[acre]	[acre]	[ft]	[ac-ft]	[ac-ft]	[hr]	[cfs]	[cfs]	[cfs]
0	6,319	0.15	0.17	1.0	0.17	0	5.50	0	0	0
1	8,874	0.20	0.23	1.0	0.23	0.17	2.23	1	0	1
2	11,550	0.27	0.29	1.0	0.29	0.41	1.69	2	0	2
3	14,105	0.32				0.70		2	0	2

Notes:

Q_{pipe} goes from Mannings Eqn to Orifice Eqn when water surface exceeds 1.2*(Outlet Diameter)
 per Linsley et al. *Water Resources Engineering* 4th Edition, pg 652.

Project **Solitude**
 Subject **Detention Basin Calculations**
 Designed by **ZJH** Date 9/24/2019 Project No. 291203001
 Checked by **JMB** Date 9/24/2019

Objective: to determine the storage-flow relationship for small detention basins

DB35

Drains in 2.92 hours

Outlet Diameter 0.66 ft Outlet X-Sect Area 0.342 ft²
 Outlet Elevation 0.5 ft No. of Outlet Barrels 1
 Outlet Pipe Slope 0.005 ft/ft

Elevation	Surface Storage Area	Surface Storage Area	Average Area	Δ Elev	Δ Vol	Σ Vol	Δ Time to Drain	Q _{pipe}	Q _{weir}	Total Q _{out}
[ft]	[ft ²]	[acre]	[acre]	[ft]	[ac-ft]	[ac-ft]	[hr]	[cfs]	[cfs]	[cfs]
0	1,940	0.04	0.05	1.0	0.05	0	1.70	0	0	0
1	2,750	0.06	0.07	1.0	0.07	0.05	0.69	1	0	1
2	3,580	0.08	0.09	1.0	0.09	0.13	0.54	2	0	2
3	4,550	0.10				0.22		2	0	2

Notes:

Q_{pipe} goes from Mannings Eqn to Orifice Eqn when water surface exceeds 1.2*(Outlet Diameter)
 per Linsley et al. *Water Resources Engineering* 4th Edition, pg 652.

City of Scottsdale
Drainage Design Management System
HEC-1 ROUTING DATA
Project Reference: SOLITUDE PROP

Page 1

9/25/2019

Route ID	LOB N	Chan N	ROB N	Length (ft)	Slope (ft/ft)	Max Elev (ft)		1.	2.	3.	4.	5.	6.	7.	8.
NORMAL DEPTH															
Major Basin 01															
R10	0.045	0.045	0.045	800.00	0.0275	-	X:	-	30.00	60.00	65.00	90.00	95.00	110.00	130.00
							Y:	10.20	10.10	10.00	9.00	9.00	10.00	10.10	10.20
R15	0.045	0.045	0.045	370.00	0.0200	-	X:	-	2.00	8.00	10.00	12.00	16.00	20.00	30.00
							Y:	10.00	10.00	10.00	9.00	9.00	10.00	10.00	10.00
R20	0.045	0.045	0.045	500.00	0.0200	-	X:	-	2.00	8.00	10.00	12.00	16.00	20.00	30.00
							Y:	10.00	10.00	10.00	9.00	9.00	10.00	10.00	10.00
R30	0.045	0.045	0.045	300.00	0.0250	-	X:	-	10.00	20.00	26.00	30.00	34.00	40.00	50.00
							Y:	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20
R35	0.045	0.045	0.045	400.00	0.0250	-	X:	-	10.00	20.00	26.00	30.00	34.00	40.00	50.00
							Y:	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20
R40	0.045	0.045	0.045	500.00	0.0250	-	X:	-	10.00	20.00	26.00	30.00	34.00	40.00	50.00
							Y:	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20
R45	0.045	0.045	0.045	650.00	0.0250	-	X:	-	2.00	10.00	18.00	24.00	32.00	40.00	45.00
							Y:	10.20	10.10	10.00	7.50	7.50	10.00	10.10	10.20
RO45	0.045	0.045	0.045	1,773.00	0.0300	-	X:	610.00	620.00	637.00	650.00	663.00	679.00	689.00	697.00
							Y:	139.00	138.70	138.30	138.50	138.80	139.00	138.90	138.90

City of Scottsdale
Drainage Design Management System
HEC-1 DIVERSIONS
Project Reference: SOLITUDE PROP

Page 1

9/25/2019

Diversion ID/ DT Card ID	Maximum Volume (ac-ft)	Maximum Diversion (cfs)	1.	2.	3.	4.	5.	6.	7.	8.	9.	10.
DOFF10		Inflow (cfs)		100	200	500	1,000	2,000	4,000	10,000	20,000	50,000
		Diversion (cfs)		85	170	425	850	1,700	3,400	8,500	17,000	42,500

```
1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 29JUL20 TIME 10:50:46
*
*****
```

```
*****
*
* U.S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
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X   X   XXXXXX   XXXXX   X
X   X   X       X   X   XX
X   X   X       X       X
XXXXXX XXXX   X       XXXXX X
X   X   X       X       X
X   X   X       X   X   X
X   X   XXXXXX   XXXXX   XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10						
1	ID	City of Scottsdale															
2	ID	SOLITUDE PROP - Solitude Proposed Conditions Hydrology															
3	ID	2 YEAR															
4	ID	6 Hour Storm															
5	ID	Unit Hydrograph: Clark															
6	ID	Storm: Multiple															
7	ID	07/29/2020															
8	*DIAGRAM																
9	IT	2	1JAN99	0	2000												
10	IO	5															
11	IN	15															
12	*																
13	JD	1.342	0.0001														
14	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
15	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
16	PC	0.962	0.972	0.983	0.991	1.000											
17	JD	1.334	0.5000														
18	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
19	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
20	PC	0.962	0.972	0.983	0.991	1.000											
21	JD	1.308	2.8														
22	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076						
23	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938						
24	PC	0.950	0.963	0.975	0.988	1.000											
25	*																
26	KK	OFF10	BASIN														
27	BA	0.900															
28	LG	0.30	0.28	5.58	0.23	21											
29	UC	0.766	0.743														
30	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0						
31	UA	100															
32	*																
33	KK	DOFF10	DIVERT														
34	DT	DTFF10	0.0	0.0													
35	DI	0.0	100.0	200.0	500.0	1000.0	2000.0	4000.0	10000.0	20000.0	50000.0						

32	DQ	0.0	85.0	170.0	425.0	850.0	1700.0	3400.0	8500.0	17000.0	42500.0
	*										
33	KK	OFF15	BASIN								
34	BA	0.003									
35	LG	0.15	0.37	6.54	0.14	0					
36	UC	0.238	0.379								
37	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
38	UA	100									
	*										
39	KK	R15	ROUTE								
40	RS	1	FLOW								
41	RC	0.045	0.045	0.045	370	0.0200	0.00				
42	RX	0.00	2.00	8.00	10.00	12.00	16.00	20.00	30.00		
43	RY	10.00	10.00	10.00	9.00	9.00	10.00	10.00	10.00		
	*										

1

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44	KK	OFF20	BASIN								
45	BA	0.008									
46	LG	0.15	0.37	6.54	0.14	0					
47	UC	0.298	0.435								
48	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
49	UA	100									
	*										
50	KK	R20	ROUTE								
51	RS	1	FLOW								
52	RC	0.045	0.045	0.045	500	0.0200	0.00				
53	RX	0.00	2.00	8.00	10.00	12.00	16.00	20.00	30.00		
54	RY	10.00	10.00	10.00	9.00	9.00	10.00	10.00	10.00		
	*										
55	KK	CP011	COMBINE								
56	HC	3									
	*										
57	KK	R10	ROUTE								
58	RS	1	FLOW								
59	RC	0.045	0.045	0.045	800	0.0275	0.00				
60	RX	0.00	30.00	60.00	65.00	90.00	95.00	110.00	130.00		
61	RY	10.20	10.10	10.00	9.00	9.00	10.00	10.10	10.20		
	*										
62	KK	ON11	BASIN								
63	BA	0.004									
64	LG	0.30	0.19	6.54	0.13	17					
65	UC	0.178	0.217								
66	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
67	UA	100									
	*										
68	KK	DB10	STORAGE								
69	KO										
70	RS	1	STOR								
71	SV		0.02	0.04	0.07	0.10	0.05	0.19	0.19		
72	SQ				1.00	1.00	1.00	1.00	15.00		
73	SE		0.50	1.00	1.50	2.00	2.50	2.95	3.00		
	*										
74	KK	ON10	BASIN								
75	BA	0.003									
76	LG	0.15	0.37	6.54	0.14	0					
77	UC	0.283	0.548								
78	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
79	UA	100									
	*										

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

80	KK	CP10	COMBINE									
81	HC	3										
	*											
82	KK	ON15	BASIN									
83	BA	0.001										
84	LG	0.30	0.19	6.54	0.13	17						
85	UC	0.156	0.287									
86	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
87	UA	100										
	*											
88	KK	ON20	BASIN									
89	BA	0.001										
90	LG	0.30	0.19	6.54	0.13	17						
91	UC	0.174	0.362									
92	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
93	UA	100										
	*											
94	KK	ON25	BASIN									
95	BA	0.001										
96	LG	0.30	0.19	6.54	0.13	17						
97	UC	0.135	0.246									
98	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
99	UA	100										
	*											
100	KK	OFF35	BASIN									
101	BA	0.031										
102	LG	0.20	0.31	6.54	0.14	5						
103	UC	0.519	1.084									
104	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
105	UA	100										
	*											
106	KK	R35	ROUTE									
107	RS	1	FLOW									
108	RC	0.045	0.045	0.045	400	0.0250	0.00					
109	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00			
110	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20			
	*											
111	KK	ON27	BASIN									
112	BA	0.001										
113	LG	0.15	0.37	6.54	0.14	0						
114	UC	0.204	0.430									
115	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
116	UA	100										
	*											

1

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

117	KK	CP27	COMBINE									
118	HC	2										
	*											
119	KK	R30	ROUTE									
120	RS	1	FLOW									
121	RC	0.045	0.045	0.045	300	0.0250	0.00					
122	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00			
123	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20			
	*											
124	KK	ON26	BASIN									
125	BA	0.008										
126	LG	0.30	0.19	6.54	0.13	17						
127	UC	0.277	0.354									
128	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
129	UA	100										
	*											

130	KK	DB26	STORAGE								
131	KO										
132	RS	1	STOR								
133	SV		0.10	0.17	0.30	0.41	0.56	0.70	0.71		
134	SQ				1.00	1.00	2.00	2.00	2.00		
135	SE		0.50	1.00	1.50	2.00	2.50	2.90	3.00		
	*										
136	KK	R30	ROUTE								
137	RS	1	FLOW								
138	RC	0.045	0.045	0.045	300	0.0250	0.00				
139	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00		
140	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20		
	*										
141	KK	ON30	BASIN								
142	BA	0.002									
143	LG	0.30	0.19	6.54	0.13	17					
144	UC	0.222	0.380								
145	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
146	UA	100									
	*										
147	KK	CP30	COMBINE								
148	HC	3									
	*										
149	KK	X1	COMBINE								
150	HC	5									
	*										

1

HEC-1 INPUT

PAGE 5

LINE	ID	1	2	3	4	5	6	7	8	9	10
151	KK	OFF40	BASIN								
152	BA	0.004									
153	LG	0.30	0.19	6.54	0.13	17					
154	UC	0.166	0.258								
155	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
156	UA	100									
	*										
157	KK	R40	ROUTE								
158	RS	1	FLOW								
159	RC	0.045	0.045	0.045	500	0.0250	0.00				
160	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00		
161	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20		
	*										
162	KK	ON28	BASIN								
163	BA	0.001									
164	LG	0.15	0.37	6.54	0.14	0					
165	UC	0.231	0.592								
166	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
167	UA	100									
	*										
168	KK	CP28	COMBINE								
169	HC	2									
	*										
170	KK	ON35	BASIN								
171	BA	0.003									
172	LG	0.30	0.19	6.54	0.13	17					
173	UC	0.145	0.158								
174	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
175	UA	100									
	*										
176	KK	DB35	STORAGE								
177	KO										
178	RS	1	STOR								
179	SV		0.03	0.05	0.10	0.13	0.18	0.22	0.22		
180	SQ				1.00	1.00	2.00	2.00	2.00		

181	SE		0.50	1.00	1.50	2.00	2.50	2.90	3.00		
	*										
182	KK	C035	COMBINE								
183	HC	2									
	*										
184	KK	ON40	BASIN								
185	BA	0.003									
186	LG	0.30	0.25	4.72	0.30	17					
187	UC	0.189	0.252								
188	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
189	UA	100									
	*										

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

190	KK	OFF30	BASIN								
191	BA	0.036									
192	LG	0.19	0.32	6.54	0.14	4					
193	UC	0.453	0.778								
194	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
195	UA	100									
	*										

196	KK	R045	ROUTE								
197	RS	1	FLOW								
198	RC	0.045	0.045	0.045	1773	0.0300	0.00				
199	RX	610.00	620.00	637.00	650.00	663.00	679.00	689.00	697.00		
200	RY	139.00	138.70	138.30	138.50	138.80	139.00	138.90	138.90		
	*										

201	KK	OFF45	BASIN								
202	BA	0.081									
203	LG	0.26	0.29	5.46	0.21	12					
204	UC	0.442	0.601								
205	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
206	UA	100									
	*										

207	KK	CP045	COMBINE								
208	HC	2									
	*										

209	KK	R45	ROUTE								
210	RS	1	FLOW								
211	RC	0.045	0.045	0.045	650	0.0250	0.00				
212	RX	0.00	2.00	10.00	18.00	24.00	32.00	40.00	45.00		
213	RY	10.20	10.10	10.00	7.50	7.50	10.00	10.10	10.20		
	*										

214	KK	OFF50	BASIN								
215	BA	0.001									
216	LG	0.30	0.19	6.54	0.13	17					
217	UC	0.126	0.253								
218	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
219	UA	100									
	*										

220	KK	R45	ROUTE								
221	RS	1	FLOW								
222	RC	0.045	0.045	0.045	650	0.0250	0.00				
223	RX	0.00	2.00	10.00	18.00	24.00	32.00	40.00	45.00		
224	RY	10.20	10.10	10.00	7.50	7.50	10.00	10.10	10.20		
	*										

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

225	KK	ON45	BASIN								
226	BA	0.001									
227	LG	0.15	0.35	4.72	0.32	0					

228	UC	0.290	0.879									
229	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
230	UA	100										
	*											
231	KK	CP45	COMBINE									
232	HC	3										
	*											
233	ZZ											

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
LINE

(V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

23  OFF10
    .
    .
30  .-----> DTFF10
29  D0FF10
    .
    .
33  .      OFF15
    .      V
    .      V
39  .      R15
    .      .
    .      .
44  .      .      OFF20
    .      .      V
    .      .      V
50  .      .      R20
    .      .
    .      .
55  CP011.....
    V
    V
57  R10
    .
    .
62  .      ON11
    .      V
    .      V
68  .      DB10
    .      .
    .      .
74  .      .      ON10
    .      .
    .      .
80  CP10.....
    .
    .
82  .      ON15
    .      .
    .      .
88  .      .      ON20
    .      .
    .      .
94  .      .      .      ON25
    .      .      .
    .      .      .
100 .      .      .      .      OFF35
    .      .      .      .      V
    .      .      .      .      V
106 .      .      .      .      R35
    .      .      .      .
    .      .      .      .
111 .      .      .      .      .      ON27
    .      .      .      .
    .      .      .      .
117 .      .      .      .      CP27.....
    .      .      .      .      V
    .      .      .      .      V
119 .      .      .      .      R30
    .      .      .      .
    
```


124	ON26
	V
	V
130	DB26
	V
	V
136	R30

141	ON30

147	CP30.....	.

149	X1.....

151	.	OFF40
	.	V
	.	V
157	.	R40

162	.	.	ON28	.	.	.

168	.	CP28.....

170	.	.	ON35	.	.	.
	.	.	V	.	.	.
	.	.	V	.	.	.
176	.	.	DB35	.	.	.

182	.	C035.....

184	.	.	ON40	.	.	.

190	.	.	.	OFF30	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
196	.	.	.	R045	.	.

201	OFF45	.

207	.	.	.	CP045.....	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
209	.	.	.	R45	.	.

214	OFF50	.
	V	.
	V	.
220	R45	.

225	ON45

231	CP45.....	.

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *

*
* U. S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT

* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*

[illegible]

.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.01	.01	.01	.01	.01	.02	.02	.02	.02	.02
.02	.02	.04	.06	.06	.06	.06	.06	.06	.06
.01	.01	.01	.01	.01	.01	.01	.01	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

19 JD INDEX STORM NO. 3
STRM 1.31 PRECIPITATION DEPTH
TRDA 2.80 TRANSPOSITION DRAINAGE AREA

20 PI PRECIPITATION PATTERN

.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.01	.01	.01	.01	.01	.01	.01	.01	.01	.01
.01	.01	.01	.01	.01	.03	.03	.03	.03	.03
.03	.03	.03	.03	.03	.03	.03	.03	.03	.03
.02	.02	.02	.02	.02	.02	.02	.01	.01	.01
.01	.01	.01	.01	.01	.01	.01	.01	.01	.01
.01	.01	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

*** **

68 KK DB10 STORAGE

69 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

130 KK DB26 STORAGE

131 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

```

*****
*
176 KK * DB35 * STORAGE
*
*****
    
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177 K0 OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+										
+	HYDROGRAPH AT	OFF10	186.	4.57	42.	11.	4.	.90		
+	DIVERSION TO	DTFF10	158.	4.57	36.	9.	3.	.90		
+	HYDROGRAPH AT	DOFF10	28.	4.57	6.	2.	1.	.90		
+	HYDROGRAPH AT	OFF15	1.	4.17	0.	0.	0.	.00		
+	ROUTED TO	R15	1.	4.20	0.	0.	0.	.00		
+	HYDROGRAPH AT	OFF20	3.	4.20	0.	0.	0.	.01		
+	ROUTED TO	R20	3.	4.27	0.	0.	0.	.01		
+	3 COMBINED AT	CP011	30.	4.53	7.	2.	1.	.91		
+	ROUTED TO	R10	29.	4.60	7.	2.	1.	.91		
+	HYDROGRAPH AT	ON11	3.	4.10	0.	0.	0.	.00		
+	ROUTED TO	DB10	1.	4.20	0.	0.	0.	.00		
+	HYDROGRAPH AT	ON10	1.	4.20	0.	0.	0.	.00		
+	3 COMBINED AT	CP10	30.	4.60	7.	2.	1.	.92		
+	HYDROGRAPH AT	ON15	1.	4.10	0.	0.	0.	.00		
+	HYDROGRAPH AT	ON20	1.	4.10	0.	0.	0.	.00		
+	HYDROGRAPH AT	ON25	1.	4.07	0.	0.	0.	.00		
+	HYDROGRAPH AT	OFF35	6.	4.40	1.	0.	0.	.03		
+	ROUTED TO	R35	6.	4.43	1.	0.	0.	.03		

+	HYDROGRAPH AT	ON27	0.	4.13	0.	0.	0.	.00
+	2 COMBINED AT	CP27	6.	4.43	1.	0.	0.	.03
+	ROUTED TO	R30	6.	4.43	1.	0.	0.	.03
+	HYDROGRAPH AT	ON26	5.	4.17	1.	0.	0.	.01
+	ROUTED TO	DB26	0.	5.13	0.	0.	0.	.01
+	ROUTED TO	R30	0.	5.20	0.	0.	0.	.01
+	HYDROGRAPH AT	ON30	1.	4.13	0.	0.	0.	.00
+	3 COMBINED AT	CP30	7.	4.43	2.	0.	0.	.04
+	5 COMBINED AT	X1	35.	4.60	8.	2.	1.	.96
+	HYDROGRAPH AT	OFF40	3.	4.10	0.	0.	0.	.00
+	ROUTED TO	R40	3.	4.13	0.	0.	0.	.00
+	HYDROGRAPH AT	ON28	0.	4.17	0.	0.	0.	.00
+	2 COMBINED AT	CP28	3.	4.13	0.	0.	0.	.01
+	HYDROGRAPH AT	ON35	3.	4.07	0.	0.	0.	.00
+	ROUTED TO	DB35	1.	4.40	0.	0.	0.	.00
+	2 COMBINED AT	C035	3.	4.17	0.	0.	0.	.01
+	HYDROGRAPH AT	ON40	2.	4.10	0.	0.	0.	.00
+	HYDROGRAPH AT	OFF30	9.	4.33	2.	0.	0.	.04
+	ROUTED TO	R045	7.	4.60	2.	0.	0.	.04
+	HYDROGRAPH AT	OFF45	25.	4.30	4.	1.	0.	.08
+	2 COMBINED AT	CP045	29.	4.33	5.	1.	1.	.12
+	ROUTED TO	R45	29.	4.40	5.	1.	1.	.12
+	HYDROGRAPH AT	OFF50	1.	4.07	0.	0.	0.	.00
+	ROUTED TO	R45	1.	4.17	0.	0.	0.	.00
+	HYDROGRAPH AT	ON45	0.	4.20	0.	0.	0.	.00

+	3 COMBINED AT						
	CP45	29.	4.40	6.	1.	1.	.12

*** NORMAL END OF HEC-1 ***

```
1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 29JUL20 TIME 10:51:04
*
*****
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*****
*
* U.S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
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X   X   XXXXXX   XXXXX   X
X   X   X       X       XX
X   X   X       X       X
XXXXXX XXXX   X       XXXXX X
X   X   X       X       X
X   X   X       X       X
X   X   XXXXXX   XXXXX   XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10						
1	ID	City of Scottsdale															
2	ID	SOLITUDE PROP - Solitude Proposed Conditions Hydrology															
3	ID	10 YEAR															
4	ID	6 Hour Storm															
5	ID	Unit Hydrograph: Clark															
6	ID	Storm: Multiple															
7	ID	07/29/2020															
8	*DIAGRAM																
9	IT	2	1JAN99	0	2000												
10	IO	5															
11	IN	15															
12	*																
13	JD	1.995	0.0001														
14	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
15	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
16	PC	0.962	0.972	0.983	0.991	1.000											
17	JD	1.983	0.5000														
18	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
19	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
20	PC	0.962	0.972	0.983	0.991	1.000											
21	JD	1.945	2.8														
22	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076						
23	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938						
24	PC	0.950	0.963	0.975	0.988	1.000											
25	*																
26	KK	OFF10	BASIN														
27	BA	0.900															
28	LG	0.30	0.28	5.58	0.23	21											
29	UC	0.671	0.642														
30	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0						
31	UA	100															
32	*																
33	KK	DOFF10	DIVERT														
34	DT	DTFF10	0.0	0.0													
35	DI	0.0	100.0	200.0	500.0	1000.0	2000.0	4000.0	10000.0	20000.0	50000.0						

32	DQ	0.0	85.0	170.0	425.0	850.0	1700.0	3400.0	8500.0	17000.0	42500.0
	*										
33	KK	OFF15	BASIN								
34	BA	0.003									
35	LG	0.15	0.37	6.54	0.14	0					
36	UC	0.209	0.327								
37	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
38	UA	100									
	*										
39	KK	R15	ROUTE								
40	RS	1	FLOW								
41	RC	0.045	0.045	0.045	370	0.0200	0.00				
42	RX	0.00	2.00	8.00	10.00	12.00	16.00	20.00	30.00		
43	RY	10.00	10.00	10.00	9.00	9.00	10.00	10.00	10.00		
	*										

1

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44	KK	OFF20	BASIN								
45	BA	0.008									
46	LG	0.15	0.37	6.54	0.14	0					
47	UC	0.261	0.375								
48	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
49	UA	100									
	*										
50	KK	R20	ROUTE								
51	RS	1	FLOW								
52	RC	0.045	0.045	0.045	500	0.0200	0.00				
53	RX	0.00	2.00	8.00	10.00	12.00	16.00	20.00	30.00		
54	RY	10.00	10.00	10.00	9.00	9.00	10.00	10.00	10.00		
	*										
55	KK	CP011	COMBINE								
56	HC	3									
	*										
57	KK	R10	ROUTE								
58	RS	1	FLOW								
59	RC	0.045	0.045	0.045	800	0.0275	0.00				
60	RX	0.00	30.00	60.00	65.00	90.00	95.00	110.00	130.00		
61	RY	10.20	10.10	10.00	9.00	9.00	10.00	10.10	10.20		
	*										
62	KK	ON11	BASIN								
63	BA	0.004									
64	LG	0.30	0.19	6.54	0.13	17					
65	UC	0.158	0.189								
66	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
67	UA	100									
	*										
68	KK	DB10	STORAGE								
69	KO										
70	RS	1	STOR								
71	SV		0.02	0.04	0.07	0.10	0.05	0.19	0.19		
72	SQ				1.00	1.00	1.00	1.00	15.00		
73	SE		0.50	1.00	1.50	2.00	2.50	2.95	3.00		
	*										
74	KK	ON10	BASIN								
75	BA	0.003									
76	LG	0.15	0.37	6.54	0.14	0					
77	UC	0.248	0.472								
78	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
79	UA	100									
	*										

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

80	KK	CP10	COMBINE									
81	HC	3										
	*											
82	KK	ON15	BASIN									
83	BA	0.001										
84	LG	0.30	0.19	6.54	0.13	17						
85	UC	0.138	0.251									
86	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
87	UA	100										
	*											
88	KK	ON20	BASIN									
89	BA	0.001										
90	LG	0.30	0.19	6.54	0.13	17						
91	UC	0.154	0.316									
92	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
93	UA	100										
	*											
94	KK	ON25	BASIN									
95	BA	0.001										
96	LG	0.30	0.19	6.54	0.13	17						
97	UC	0.120	0.215									
98	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
99	UA	100										
	*											
100	KK	OFF35	BASIN									
101	BA	0.031										
102	LG	0.20	0.31	6.54	0.14	5						
103	UC	0.456	0.937									
104	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
105	UA	100										
	*											
106	KK	R35	ROUTE									
107	RS	1	FLOW									
108	RC	0.045	0.045	0.045	400	0.0250	0.00					
109	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00			
110	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20			
	*											
111	KK	ON27	BASIN									
112	BA	0.001										
113	LG	0.15	0.37	6.54	0.14	0						
114	UC	0.178	0.371									
115	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
116	UA	100										
	*											

1

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

117	KK	CP27	COMBINE									
118	HC	2										
	*											
119	KK	R30	ROUTE									
120	RS	1	FLOW									
121	RC	0.045	0.045	0.045	300	0.0250	0.00					
122	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00			
123	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20			
	*											
124	KK	ON26	BASIN									
125	BA	0.008										
126	LG	0.30	0.19	6.54	0.13	17						
127	UC	0.246	0.309									
128	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
129	UA	100										
	*											

130	KK	DB26	STORAGE								
131	KO										
132	RS	1	STOR								
133	SV		0.10	0.17	0.30	0.41	0.56	0.70	0.71		
134	SQ				1.00	1.00	2.00	2.00	2.00		
135	SE		0.50	1.00	1.50	2.00	2.50	2.90	3.00		
	*										
136	KK	R30	ROUTE								
137	RS	1	FLOW								
138	RC	0.045	0.045	0.045	300	0.0250	0.00				
139	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00		
140	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20		
	*										
141	KK	ON30	BASIN								
142	BA	0.002									
143	LG	0.30	0.19	6.54	0.13	17					
144	UC	0.196	0.333								
145	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
146	UA	100									
	*										
147	KK	CP30	COMBINE								
148	HC	3									
	*										
149	KK	X1	COMBINE								
150	HC	5									
	*										

1

HEC-1 INPUT

PAGE 5

LINE	ID	1	2	3	4	5	6	7	8	9	10
151	KK	OFF40	BASIN								
152	BA	0.004									
153	LG	0.30	0.19	6.54	0.13	17					
154	UC	0.147	0.226								
155	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
156	UA	100									
	*										
157	KK	R40	ROUTE								
158	RS	1	FLOW								
159	RC	0.045	0.045	0.045	500	0.0250	0.00				
160	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00		
161	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20		
	*										
162	KK	ON28	BASIN								
163	BA	0.001									
164	LG	0.15	0.37	6.54	0.14	0					
165	UC	0.202	0.511								
166	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
167	UA	100									
	*										
168	KK	CP28	COMBINE								
169	HC	2									
	*										
170	KK	ON35	BASIN								
171	BA	0.003									
172	LG	0.30	0.19	6.54	0.13	17					
173	UC	0.129	0.138								
174	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
175	UA	100									
	*										
176	KK	DB35	STORAGE								
177	KO										
178	RS	1	STOR								
179	SV		0.03	0.05	0.10	0.13	0.18	0.22	0.22		
180	SQ				1.00	1.00	2.00	2.00	2.00		

181	SE		0.50	1.00	1.50	2.00	2.50	2.90	3.00		
	*										
182	KK	C035	COMBINE								
183	HC	2									
	*										
184	KK	ON40	BASIN								
185	BA	0.003									
186	LG	0.30	0.25	4.72	0.30	17					
187	UC	0.168	0.221								
188	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
189	UA	100									
	*										

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HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

190	KK	OFF30	BASIN								
191	BA	0.036									
192	LG	0.19	0.32	6.54	0.14	4					
193	UC	0.398	0.672								
194	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
195	UA	100									
	*										

196	KK	R045	ROUTE								
197	RS	1	FLOW								
198	RC	0.045	0.045	0.045	1773	0.0300	0.00				
199	RX	610.00	620.00	637.00	650.00	663.00	679.00	689.00	697.00		
200	RY	139.00	138.70	138.30	138.50	138.80	139.00	138.90	138.90		
	*										

201	KK	OFF45	BASIN								
202	BA	0.081									
203	LG	0.26	0.29	5.46	0.21	12					
204	UC	0.392	0.526								
205	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
206	UA	100									
	*										

207	KK	CP045	COMBINE								
208	HC	2									
	*										

209	KK	R45	ROUTE								
210	RS	1	FLOW								
211	RC	0.045	0.045	0.045	650	0.0250	0.00				
212	RX	0.00	2.00	10.00	18.00	24.00	32.00	40.00	45.00		
213	RY	10.20	10.10	10.00	7.50	7.50	10.00	10.10	10.20		
	*										

214	KK	OFF50	BASIN								
215	BA	0.001									
216	LG	0.30	0.19	6.54	0.13	17					
217	UC	0.112	0.222								
218	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
219	UA	100									
	*										

220	KK	R45	ROUTE								
221	RS	1	FLOW								
222	RC	0.045	0.045	0.045	650	0.0250	0.00				
223	RX	0.00	2.00	10.00	18.00	24.00	32.00	40.00	45.00		
224	RY	10.20	10.10	10.00	7.50	7.50	10.00	10.10	10.20		
	*										

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

225	KK	ON45	BASIN								
226	BA	0.001									
227	LG	0.15	0.35	4.72	0.32	0					

228	UC	0.252	0.753									
229	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
230	UA	100										
	*											
231	KK	CP45	COMBINE									
232	HC	3										
	*											
233	ZZ											

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
LINE

(V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

23  OFF10
    .
    .
30  .-----> DTFF10
29  DOFF10
    .
    .
33  .      OFF15
    .      V
    .      V
39  .      R15
    .      .
    .      .
44  .      .      OFF20
    .      .      V
    .      .      V
50  .      .      R20
    .      .
    .      .
55  CP011.....
    V
    V
57  R10
    .
    .
62  .      ON11
    .      V
    .      V
68  .      DB10
    .      .
    .      .
74  .      .      ON10
    .      .
    .      .
80  CP10.....
    .
    .
82  .      ON15
    .      .
    .      .
88  .      .      ON20
    .      .
    .      .
94  .      .      .      ON25
    .      .      .
    .      .      .
100 .      .      .      .      OFF35
    .      .      .      .      V
    .      .      .      .      V
106 .      .      .      .      R35
    .      .      .      .
    .      .      .      .
111 .      .      .      .      .      ON27
    .      .      .      .
    .      .      .      .
117 .      .      .      .      CP27.....
    .      .      .      .      V
    .      .      .      .      V
119 .      .      .      .      R30
    .      .      .      .
    
```

124	ON26
	V
	V
130	DB26
	V
	V
136	R30

141	ON30

147	CP30.....	.

149	X1.....

151	.	OFF40
	.	V
	.	V
157	.	R40

162	.	.	ON28	.	.	.

168	.	CP28.....

170	.	.	ON35	.	.	.
	.	.	V	.	.	.
	.	.	V	.	.	.
176	.	.	DB35	.	.	.

182	.	C035.....

184	.	.	ON40	.	.	.

190	.	.	.	OFF30	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
196	.	.	.	R045	.	.

201	OFF45	.

207	.	.	.	CP045.....	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
209	.	.	.	R45	.	.

214	OFF50	.
	V	.
	V	.
220	R45	.

225	ON45

231	.	.	.	CP45.....	.	.

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *

*
* U. S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT

* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*

[illegible]

.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.01	.01	.01	.01	.01	.02	.02	.02	.02	.02
.02	.02	.04	.06	.06	.06	.06	.06	.06	.06
.01	.01	.01	.01	.01	.01	.01	.01	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

19 JD INDEX STORM NO. 3
STRM 1.95 PRECIPITATION DEPTH
TRDA 2.80 TRANSPOSITION DRAINAGE AREA

20 PI PRECIPITATION PATTERN

.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.01	.01	.01	.01	.01	.01	.01	.01	.01	.01
.01	.01	.01	.01	.01	.01	.03	.03	.03	.03
.03	.03	.03	.03	.03	.03	.03	.03	.03	.03
.02	.02	.02	.02	.02	.02	.02	.02	.01	.01
.01	.01	.01	.01	.01	.01	.01	.01	.01	.01
.01	.01	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

*** **

68 KK DB10 STORAGE

69 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

130 KK DB26 STORAGE

131 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

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*****
*
176 KK  *      DB35  *      STORAGE
*
*****
    
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177 KO      OUTPUT CONTROL VARIABLES
            IPRNT      5 PRINT CONTROL
            IPLOT      0 PLOT CONTROL
            QSCAL      0. HYDROGRAPH PLOT SCALE
    
```

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+										
+	HYDROGRAPH AT	OFF10	439.	4.47	85.	22.	8.	.90		
+	DIVERSION TO	DTFF10	374.	4.47	72.	18.	7.	.90		
+	HYDROGRAPH AT	DOFF10	66.	4.47	13.	3.	1.	.90		
+	HYDROGRAPH AT	OFF15	3.	4.13	0.	0.	0.	.00		
+	ROUTED TO	R15	3.	4.17	0.	0.	0.	.00		
+	HYDROGRAPH AT	OFF20	7.	4.17	1.	0.	0.	.01		
+	ROUTED TO	R20	7.	4.20	1.	0.	0.	.01		
+	3 COMBINED AT	CP011	71.	4.47	14.	3.	1.	.91		
+	ROUTED TO	R10	70.	4.50	14.	3.	1.	.91		
+	HYDROGRAPH AT	ON11	6.	4.07	0.	0.	0.	.00		
+	ROUTED TO	DB10	1.	4.03	0.	0.	0.	.00		
+	HYDROGRAPH AT	ON10	2.	4.17	0.	0.	0.	.00		
+	3 COMBINED AT	CP10	72.	4.50	14.	4.	1.	.92		
+	HYDROGRAPH AT	ON15	1.	4.07	0.	0.	0.	.00		
+	HYDROGRAPH AT	ON20	1.	4.10	0.	0.	0.	.00		
+	HYDROGRAPH AT	ON25	1.	4.07	0.	0.	0.	.00		
+	HYDROGRAPH AT	OFF35	14.	4.33	3.	1.	0.	.03		
+	ROUTED TO	R35	14.	4.37	3.	1.	0.	.03		

+	HYDROGRAPH AT	ON27	1.	4.10	0.	0.	0.	.00
+	2 COMBINED AT	CP27	15.	4.37	3.	1.	0.	.03
+	ROUTED TO	R30	15.	4.37	3.	1.	0.	.03
+	HYDROGRAPH AT	ON26	9.	4.17	1.	0.	0.	.01
+	ROUTED TO	DB26	1.	4.37	1.	0.	0.	.01
+	ROUTED TO	R30	1.	4.53	1.	0.	0.	.01
+	HYDROGRAPH AT	ON30	2.	4.13	0.	0.	0.	.00
+	3 COMBINED AT	CP30	17.	4.37	4.	1.	0.	.04
+	5 COMBINED AT	X1	86.	4.50	18.	5.	2.	.96
+	HYDROGRAPH AT	OFF40	6.	4.07	0.	0.	0.	.00
+	ROUTED TO	R40	5.	4.10	0.	0.	0.	.00
+	HYDROGRAPH AT	ON28	1.	4.13	0.	0.	0.	.00
+	2 COMBINED AT	CP28	6.	4.10	1.	0.	0.	.01
+	HYDROGRAPH AT	ON35	5.	4.07	0.	0.	0.	.00
+	ROUTED TO	DB35	1.	4.37	0.	0.	0.	.00
+	2 COMBINED AT	C035	7.	4.10	1.	0.	0.	.01
+	HYDROGRAPH AT	ON40	4.	4.10	0.	0.	0.	.00
+	HYDROGRAPH AT	OFF30	21.	4.27	3.	1.	0.	.04
+	ROUTED TO	R045	17.	4.50	3.	1.	0.	.04
+	HYDROGRAPH AT	OFF45	55.	4.27	8.	2.	1.	.08
+	2 COMBINED AT	CP045	67.	4.30	11.	3.	1.	.12
+	ROUTED TO	R45	66.	4.33	11.	3.	1.	.12
+	HYDROGRAPH AT	OFF50	1.	4.07	0.	0.	0.	.00
+	ROUTED TO	R45	1.	4.13	0.	0.	0.	.00
+	HYDROGRAPH AT	ON45	0.	4.17	0.	0.	0.	.00

3 COMBINED AT							
+	CP45	67.	4.33	11.	3.	1.	.12

*** NORMAL END OF HEC-1 ***


```
1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   JUN 1998
*   VERSION 4.1
*
* RUN DATE 29JUL20 TIME 10:51:53
*
*****
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*****
*
* U.S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
```

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X   X XXXXXXX XXXXX      X
X   X X      X      X    XX
X   X X      X      X    X
XXXXXX XXXX  X      XXXXX X
X   X X      X      X    X
X   X X      X      X    X
X   X XXXXXXX XXXXX      XXX
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10						
1	ID	City of Scottsdale															
2	ID	SOLITUDE PROP - Solitude Proposed Conditions Hydrology															
3	ID	100 YEAR															
4	ID	6 Hour Storm															
5	ID	Unit Hydrograph: Clark															
6	ID	Storm: Multiple															
7	ID	07/29/2020															
8	*DIAGRAM																
9	IT	2	1JAN99	0	2000												
10	IO	5															
11	IN	15															
12	JD	3.024	0.0001														
13	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
14	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
15	PC	0.962	0.972	0.983	0.991	1.000											
16	JD	3.006	0.5000														
17	PC	0.000	0.008	0.016	0.025	0.033	0.041	0.050	0.058	0.066	0.074						
18	PC	0.087	0.099	0.118	0.138	0.216	0.377	0.834	0.911	0.931	0.950						
19	PC	0.962	0.972	0.983	0.991	1.000											
20	JD	2.948	2.8														
21	PC	0.000	0.009	0.016	0.025	0.034	0.042	0.051	0.059	0.067	0.076						
22	PC	0.087	0.100	0.120	0.163	0.252	0.451	0.694	0.837	0.900	0.938						
23	PC	0.950	0.963	0.975	0.988	1.000											
24	*DIAGRAM																
25	KK	OFF10	BASIN														
26	BA	0.900															
27	LG	0.30	0.28	5.58	0.23	21											
28	UC	0.515	0.478														
29	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0						
30	UA	100															
31	*DIAGRAM																
32	KK	DOFF10	DIVERT														
33	DT	DTFF10	0.0	0.0													
34	DI	0.0	100.0	200.0	500.0	1000.0	2000.0	4000.0	10000.0	20000.0	50000.0						

32	DQ	0.0	85.0	170.0	425.0	850.0	1700.0	3400.0	8500.0	17000.0	42500.0
	*										
33	KK	OFF15	BASIN								
34	BA	0.003									
35	LG	0.15	0.37	6.54	0.14	0					
36	UC	0.161	0.244								
37	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
38	UA	100									
	*										
39	KK	R15	ROUTE								
40	RS	1	FLOW								
41	RC	0.045	0.045	0.045	370	0.0200	0.00				
42	RX	0.00	2.00	8.00	10.00	12.00	16.00	20.00	30.00		
43	RY	10.00	10.00	10.00	9.00	9.00	10.00	10.00	10.00		
	*										

1

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44	KK	OFF20	BASIN								
45	BA	0.008									
46	LG	0.15	0.37	6.54	0.14	0					
47	UC	0.201	0.281								
48	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
49	UA	100									
	*										
50	KK	R20	ROUTE								
51	RS	1	FLOW								
52	RC	0.045	0.045	0.045	500	0.0200	0.00				
53	RX	0.00	2.00	8.00	10.00	12.00	16.00	20.00	30.00		
54	RY	10.00	10.00	10.00	9.00	9.00	10.00	10.00	10.00		
	*										
55	KK	CP011	COMBINE								
56	HC	3									
	*										
57	KK	R10	ROUTE								
58	RS	1	FLOW								
59	RC	0.045	0.045	0.045	800	0.0275	0.00				
60	RX	0.00	30.00	60.00	65.00	90.00	95.00	110.00	130.00		
61	RY	10.20	10.10	10.00	9.00	9.00	10.00	10.10	10.20		
	*										
62	KK	ON11	BASIN								
63	BA	0.004									
64	LG	0.30	0.19	6.54	0.13	17					
65	UC	0.126	0.148								
66	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
67	UA	100									
	*										
68	KK	DB10	STORAGE								
69	KO										
70	RS	1	STOR								
71	SV		0.02	0.04	0.07	0.10	0.05	0.19	0.19		
72	SQ				1.00	1.00	1.00	1.00	15.00		
73	SE		0.50	1.00	1.50	2.00	2.50	2.95	3.00		
	*										
74	KK	ON10	BASIN								
75	BA	0.003									
76	LG	0.15	0.37	6.54	0.14	0					
77	UC	0.191	0.353								
78	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
79	UA	100									
	*										

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

80	KK	CP10	COMBINE									
81	HC	3										
	*											
82	KK	ON15	BASIN									
83	BA	0.001										
84	LG	0.30	0.19	6.54	0.13	17						
85	UC	0.110	0.196									
86	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
87	UA	100										
	*											
88	KK	ON20	BASIN									
89	BA	0.001										
90	LG	0.30	0.19	6.54	0.13	17						
91	UC	0.123	0.247									
92	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
93	UA	100										
	*											
94	KK	ON25	BASIN									
95	BA	0.001										
96	LG	0.30	0.19	6.54	0.13	17						
97	UC	0.096	0.168									
98	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
99	UA	100										
	*											
100	KK	OFF35	BASIN									
101	BA	0.031										
102	LG	0.20	0.31	6.54	0.14	5						
103	UC	0.355	0.711									
104	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
105	UA	100										
	*											
106	KK	R35	ROUTE									
107	RS	1	FLOW									
108	RC	0.045	0.045	0.045	400	0.0250	0.00					
109	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00			
110	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20			
	*											
111	KK	ON27	BASIN									
112	BA	0.001										
113	LG	0.15	0.37	6.54	0.14	0						
114	UC	0.137	0.277									
115	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
116	UA	100										
	*											

1

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

117	KK	CP27	COMBINE									
118	HC	2										
	*											
119	KK	R30	ROUTE									
120	RS	1	FLOW									
121	RC	0.045	0.045	0.045	300	0.0250	0.00					
122	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00			
123	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20			
	*											
124	KK	ON26	BASIN									
125	BA	0.008										
126	LG	0.30	0.19	6.54	0.13	17						
127	UC	0.196	0.241									
128	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
129	UA	100										
	*											

130	KK	DB26	STORAGE								
131	KO										
132	RS	1	STOR								
133	SV		0.10	0.17	0.30	0.41	0.56	0.70	0.71		
134	SQ				1.00	1.00	2.00	2.00	2.00		
135	SE		0.50	1.00	1.50	2.00	2.50	2.90	3.00		
	*										
136	KK	R30	ROUTE								
137	RS	1	FLOW								
138	RC	0.045	0.045	0.045	300	0.0250	0.00				
139	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00		
140	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20		
	*										
141	KK	ON30	BASIN								
142	BA	0.002									
143	LG	0.30	0.19	6.54	0.13	17					
144	UC	0.157	0.260								
145	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
146	UA	100									
	*										
147	KK	CP30	COMBINE								
148	HC	3									
	*										
149	KK	X1	COMBINE								
150	HC	5									
	*										

1

HEC-1 INPUT

PAGE 5

LINE	ID	1	2	3	4	5	6	7	8	9	10
151	KK	OFF40	BASIN								
152	BA	0.004									
153	LG	0.30	0.19	6.54	0.13	17					
154	UC	0.118	0.176								
155	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
156	UA	100									
	*										
157	KK	R40	ROUTE								
158	RS	1	FLOW								
159	RC	0.045	0.045	0.045	500	0.0250	0.00				
160	RX	0.00	10.00	20.00	26.00	30.00	34.00	40.00	50.00		
161	RY	10.20	10.10	10.00	8.00	8.00	10.00	10.10	10.20		
	*										
162	KK	ON28	BASIN								
163	BA	0.001									
164	LG	0.15	0.37	6.54	0.14	0					
165	UC	0.156	0.382								
166	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
167	UA	100									
	*										
168	KK	CP28	COMBINE								
169	HC	2									
	*										
170	KK	ON35	BASIN								
171	BA	0.003									
172	LG	0.30	0.19	6.54	0.13	17					
173	UC	0.103	0.108								
174	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
175	UA	100									
	*										
176	KK	DB35	STORAGE								
177	KO										
178	RS	1	STOR								
179	SV		0.03	0.05	0.10	0.13	0.18	0.22	0.22		
180	SQ				1.00	1.00	2.00	2.00	2.00		

181	SE		0.50	1.00	1.50	2.00	2.50	2.90	3.00		
	*										
182	KK	C035	COMBINE								
183	HC	2									
	*										
184	KK	ON40	BASIN								
185	BA	0.003									
186	LG	0.30	0.25	4.72	0.30	17					
187	UC	0.131	0.168								
188	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
189	UA	100									
	*										

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

190	KK	OFF30	BASIN								
191	BA	0.036									
192	LG	0.19	0.32	6.54	0.14	4					
193	UC	0.309	0.509								
194	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
195	UA	100									
	*										

196	KK	R045	ROUTE								
197	RS	1	FLOW								
198	RC	0.045	0.045	0.045	1773	0.0300	0.00				
199	RX	610.00	620.00	637.00	650.00	663.00	679.00	689.00	697.00		
200	RY	139.00	138.70	138.30	138.50	138.80	139.00	138.90	138.90		
	*										

201	KK	OFF45	BASIN								
202	BA	0.081									
203	LG	0.26	0.29	5.46	0.21	12					
204	UC	0.303	0.396								
205	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
206	UA	100									
	*										

207	KK	CP045	COMBINE								
208	HC	2									
	*										

209	KK	R45	ROUTE								
210	RS	1	FLOW								
211	RC	0.045	0.045	0.045	650	0.0250	0.00				
212	RX	0.00	2.00	10.00	18.00	24.00	32.00	40.00	45.00		
213	RY	10.20	10.10	10.00	7.50	7.50	10.00	10.10	10.20		
	*										

214	KK	OFF50	BASIN								
215	BA	0.001									
216	LG	0.30	0.19	6.54	0.13	17					
217	UC	0.090	0.173								
218	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
219	UA	100									
	*										

220	KK	R45	ROUTE								
221	RS	1	FLOW								
222	RC	0.045	0.045	0.045	650	0.0250	0.00				
223	RX	0.00	2.00	10.00	18.00	24.00	32.00	40.00	45.00		
224	RY	10.20	10.10	10.00	7.50	7.50	10.00	10.10	10.20		
	*										

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

225	KK	ON45	BASIN								
226	BA	0.001									
227	LG	0.15	0.35	4.72	0.32	0					

228	UC	0.190	0.552									
229	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0	
230	UA	100										
	*											
231	KK	CP45	COMBINE									
232	HC	3										
	*											
233	ZZ											

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
LINE

(V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

23  OFF10
    .
    .
30  .-----> DTFF10
29  DOFF10
    .
    .
33  .      OFF15
    .      V
    .      V
39  .      R15
    .      .
    .      .
44  .      .      OFF20
    .      .      V
    .      .      V
50  .      .      R20
    .      .
    .      .
55  CP011.....
    V
    V
57  R10
    .
    .
62  .      ON11
    .      V
    .      V
68  .      DB10
    .      .
    .      .
74  .      .      ON10
    .      .
    .      .
80  CP10.....
    .
    .
82  .      ON15
    .      .
    .      .
88  .      .      ON20
    .      .
    .      .
94  .      .      .      ON25
    .      .      .
    .      .      .
100 .      .      .      .      OFF35
    .      .      .      .      V
    .      .      .      .      V
106 .      .      .      .      R35
    .      .      .      .
    .      .      .      .
111 .      .      .      .      .      ON27
    .      .      .      .      .
    .      .      .      .      .
117 .      .      .      .      CP27.....
    .      .      .      .      V
    .      .      .      .      V
119 .      .      .      .      R30
    .      .      .      .
    
```

124	ON26
	V
	V
130	DB26
	V
	V
136	R30

141	ON30

147	CP30.....	.

149	X1.....

151	.	OFF40
	.	V
	.	V
157	.	R40

162	.	.	ON28	.	.	.

168	.	CP28.....

170	.	.	ON35	.	.	.
	.	.	V	.	.	.
	.	.	V	.	.	.
176	.	.	DB35	.	.	.

182	.	C035.....

184	.	.	ON40	.	.	.

190	.	.	.	OFF30	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
196	.	.	.	R045	.	.

201	OFF45	.

207	.	.	.	CP045.....	.	.
	.	.	.	V	.	.
	.	.	.	V	.	.
209	.	.	.	R45	.	.

214	OFF50	.
	V	.
	V	.
220	R45	.

225	ON45

231	.	.	.	CP45.....	.	.

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *

*
* U. S. ARMY CORPS OF ENGINEER
* HYDROLOGIC ENGINEERING CENT

* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*

[illegible]

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.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
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.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.01	.01	.01	.01	.01	.02	.02	.02	.02	.02
.02	.02	.04	.06	.06	.06	.06	.06	.06	.06
.01	.01	.01	.01	.01	.01	.01	.01	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
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.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
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19 JD INDEX STORM NO. 3
STRM 2.95 PRECIPITATION DEPTH
TRDA 2.80 TRANSPOSITION DRAINAGE AREA

20 PI PRECIPITATION PATTERN

.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
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.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.01	.01	.01	.01	.01	.01	.01	.01	.01	.01
.01	.01	.01	.01	.01	.03	.03	.03	.03	.03
.03	.03	.03	.03	.03	.03	.03	.03	.03	.03
.02	.02	.02	.02	.02	.02	.02	.01	.01	.01
.01	.01	.01	.01	.01	.01	.01	.01	.01	.01
.01	.01	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

*** **

68 KK DB10 STORAGE

69 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

130 KK DB26 STORAGE

131 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

```
*****
*
176 KK      *      DB35      *      STORAGE
*
*****
```

```
177 K0      OUTPUT CONTROL VARIABLES
            IPRNT      5  PRINT CONTROL
            IPLOT      0  PLOT CONTROL
            QSCAL      0.  HYDROGRAPH PLOT SCALE
```


Appendix C – Hydraulics

Flowmaster Cross Sections (Existing and Proposed Conditions)

- Flowmaster Output
- Cross Sections

HY-8 Output:

- Preliminary Culvert Calculations

HY-8 Culvert Calculation Output

HY-8 Culvert Analysis Report

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 29 cfs

Design Flow: 68 cfs

Maximum Flow: 158 cfs

Table 1 - Summary of Culvert Flows at Crossing: Culvert A

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert A Discharge (cfs)	Roadway Discharge (cfs)	Iterations
102.33	29.00	29.00	0.00	1
102.64	41.90	41.90	0.00	1
102.93	54.80	54.80	0.00	1
103.19	67.70	67.70	0.00	1
103.19	68.00	68.00	0.00	1
103.65	93.50	93.50	0.00	1
103.88	106.40	106.40	0.00	1
104.12	119.30	119.30	0.00	1
104.38	132.20	132.20	0.00	1
104.66	145.10	145.10	0.00	1
104.96	158.00	158.00	0.00	1
105.00	159.42	159.42	0.00	Overtopping

Rating Curve Plot for Crossing: Culvert A

Table 2 - Culvert Summary Table: Culvert A

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
29.00	29.00	102.33	1.327	0.029	1-S2n	0.682	0.980	0.714	0.495	7.233	4.187
41.90	41.90	102.64	1.641	0.290	1-S2n	0.821	1.187	0.869	0.621	7.938	4.819
54.80	54.80	102.93	1.934	0.544	1-S2n	0.944	1.369	1.007	0.734	8.470	5.333
67.70	67.70	103.19	2.189	0.796	1-S2n	1.055	1.528	1.134	0.838	8.902	5.773
68.00	68.00	103.19	2.195	0.801	1-S2n	1.058	1.531	1.137	0.840	8.911	5.783
93.50	93.50	103.65	2.649	1.318	1-S2n	1.258	1.808	1.367	1.026	9.609	6.507
106.40	106.40	103.88	2.877	1.593	1-S2n	1.354	1.932	1.476	1.114	9.911	6.822
119.30	119.30	104.12	3.118	1.882	5-S2n	1.446	2.051	1.582	1.198	10.180	7.112
132.20	132.20	104.38	3.376	2.180	5-S2n	1.537	2.160	1.684	1.279	10.448	7.381
145.10	145.10	104.66	3.658	2.860	5-S2n	1.628	2.262	1.784	1.358	10.702	7.632
158.00	158.00	104.96	3.964	3.135	5-S2n	1.718	2.356	1.882	1.434	10.943	7.868

Straight Culvert

Inlet Elevation (invert): 101.00 ft, Outlet Elevation (invert): 100.00 ft

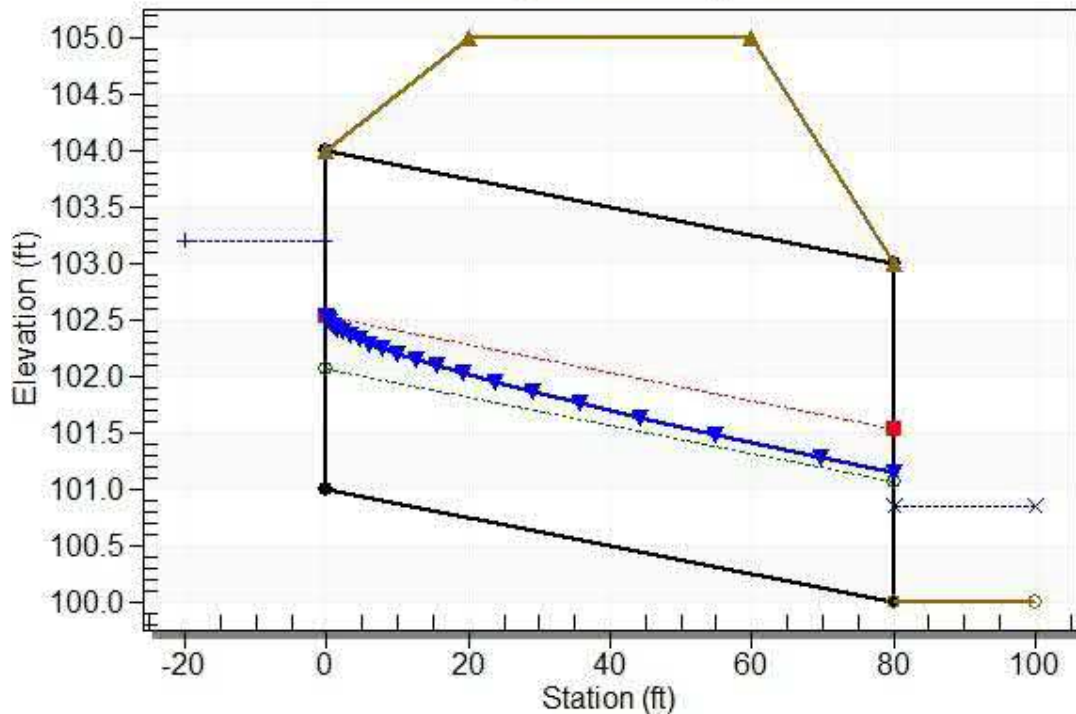
Culvert Length: 80.01 ft, Culvert Slope: 0.0125

Culvert Performance Curve Plot: Culvert A

Water Surface Profile Plot for Culvert: Culvert A

Crossing - Culvert A, Design Discharge - 68.0 cfs

Culvert - Culvert A, Culvert Discharge - 68.0 cfs



Site Data - Culvert A

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 101.00 ft

Outlet Station: 80.00 ft

Outlet Elevation: 100.00 ft

Number of Barrels: 3

Culvert Data Summary - Culvert A

Barrel Shape: Circular

Barrel Diameter: 3.00 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Beveled Edge (1:1)

Inlet Depression: None

Table 3 - Downstream Channel Rating Curve (Crossing: Culvert A)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
29.00	100.49	0.49	4.19	0.62	1.05
41.90	100.62	0.62	4.82	0.78	1.08
54.80	100.73	0.73	5.33	0.92	1.10
67.70	100.84	0.84	5.77	1.05	1.11
68.00	100.84	0.84	5.78	1.05	1.11
93.50	101.03	1.03	6.51	1.28	1.13
106.40	101.11	1.11	6.82	1.39	1.14
119.30	101.20	1.20	7.11	1.50	1.15
132.20	101.28	1.28	7.38	1.60	1.15
145.10	101.36	1.36	7.63	1.69	1.15
158.00	101.43	1.43	7.87	1.79	1.16

Tailwater Channel Data - Culvert A

Tailwater Channel Option: Rectangular Channel

Bottom Width: 14.00 ft

Channel Slope: 0.0200

Channel Manning's n: 0.0300

Channel Invert Elevation: 100.00 ft

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 3 cfs

Design Flow: 6 cfs

Maximum Flow: 12 cfs

Table 4 - Summary of Culvert Flows at Crossing: Culvert B

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert B Discharge (cfs)	Roadway Discharge (cfs)	Iterations
111.81	3.00	3.00	0.00	1
111.93	3.90	3.90	0.00	1
112.05	4.80	4.80	0.00	1
112.17	5.70	5.70	0.00	1
112.20	6.00	6.00	0.00	1
112.38	7.50	7.50	0.00	1
112.47	8.40	8.40	0.00	1
112.56	9.30	9.30	0.00	1
112.65	10.20	10.20	0.00	1
112.74	11.10	11.10	0.00	1
112.82	12.00	12.00	0.00	1
115.00	27.32	27.32	0.00	Overtopping

Rating Curve Plot for Crossing: Culvert B

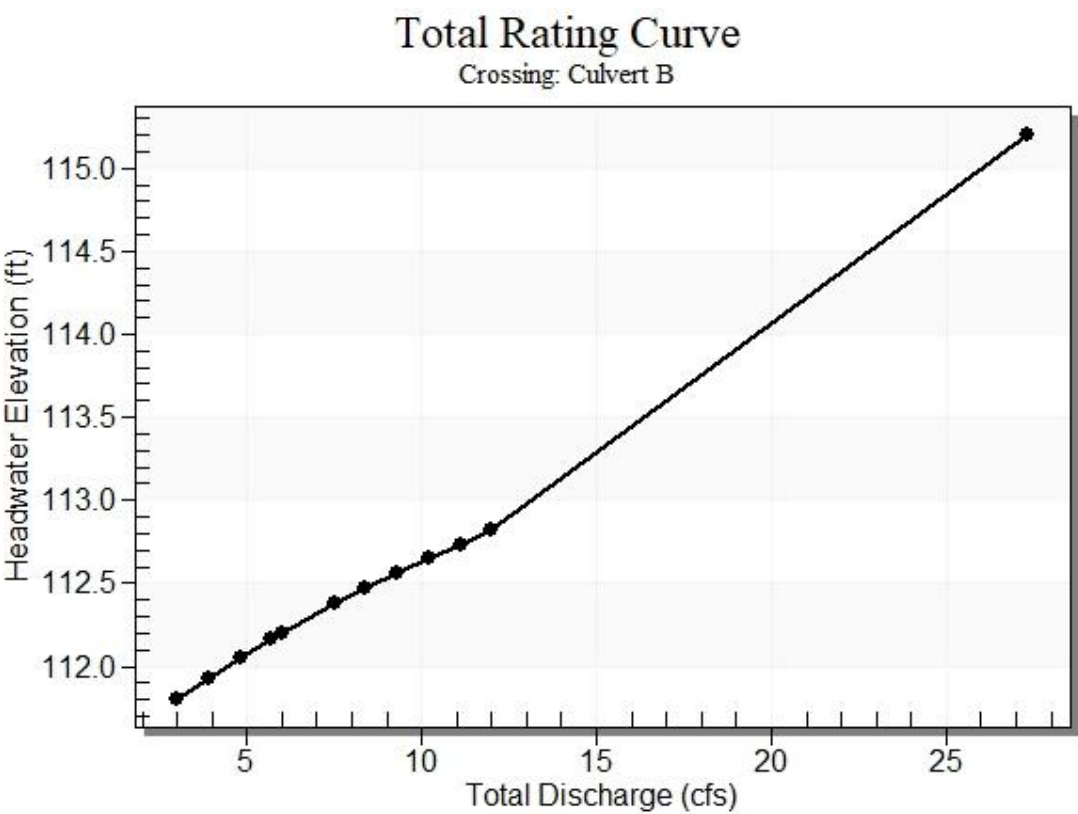


Table 5 - Culvert Summary Table: Culvert B

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
3.00	3.00	111.81	0.808	0.0*	1-S2n	0.374	0.601	0.394	0.428	6.633	3.506
3.90	3.90	111.93	0.929	0.0*	1-S2n	0.427	0.688	0.453	0.512	7.071	3.806
4.80	4.80	112.05	1.047	0.0*	1-S2n	0.473	0.765	0.507	0.592	7.425	4.051
5.70	5.70	112.17	1.167	0.0*	1-S2n	0.517	0.841	0.556	0.669	7.729	4.258
6.00	6.00	112.20	1.204	0.0*	1-S2n	0.530	0.865	0.573	0.694	7.783	4.321
7.50	7.50	112.38	1.376	0.121	1-S2n	0.595	0.974	0.649	0.816	8.203	4.595
8.40	8.40	112.47	1.471	0.217	1-S2n	0.631	1.032	0.693	0.887	8.394	4.735
9.30	9.30	112.56	1.561	0.313	1-S2n	0.666	1.086	0.735	0.957	8.577	4.860
10.20	10.20	112.65	1.649	0.413	1-S2n	0.699	1.141	0.775	1.025	8.755	4.975
11.10	11.10	112.74	1.735	0.514	1-S2n	0.731	1.191	0.815	1.093	8.925	5.079
12.00	12.00	112.82	1.822	0.616	1-S2n	0.763	1.239	0.853	1.160	9.074	5.174

* Full Flow Headwater elevation is below inlet invert.

Straight Culvert

Inlet Elevation (invert): 111.00 ft, Outlet Elevation (invert): 110.00 ft

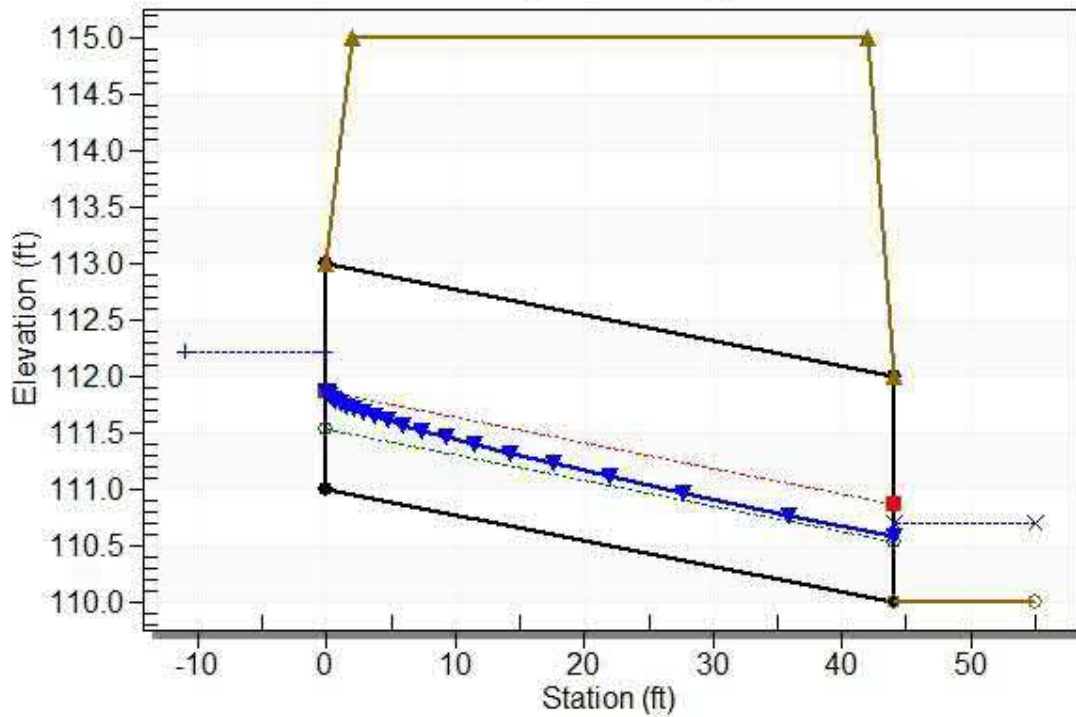
Culvert Length: 44.01 ft, Culvert Slope: 0.0227

Culvert Performance Curve Plot: Culvert B

Water Surface Profile Plot for Culvert: Culvert B

Crossing - Culvert B, Design Discharge - 6.0 cfs

Culvert - Culvert B, Culvert Discharge - 6.0 cfs



Site Data - Culvert B

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 111.00 ft

Outlet Station: 44.00 ft

Outlet Elevation: 110.00 ft

Number of Barrels: 1

Culvert Data Summary - Culvert B

Barrel Shape: Circular

Barrel Diameter: 2.00 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Beveled Edge (1:1)

Inlet Depression: None

Table 6 - Downstream Channel Rating Curve (Crossing: Culvert B)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
3.00	110.43	0.43	3.51	0.67	0.94
3.90	110.51	0.51	3.81	0.80	0.94
4.80	110.59	0.59	4.05	0.92	0.93
5.70	110.67	0.67	4.26	1.04	0.92
6.00	110.69	0.69	4.32	1.08	0.91
7.50	110.82	0.82	4.60	1.27	0.90
8.40	110.89	0.89	4.74	1.38	0.89
9.30	110.96	0.96	4.86	1.49	0.88
10.20	111.03	1.03	4.97	1.60	0.87
11.10	111.09	1.09	5.08	1.70	0.86
12.00	111.16	1.16	5.17	1.81	0.85

Tailwater Channel Data - Culvert B

Tailwater Channel Option: Rectangular Channel

Bottom Width: 2.00 ft

Channel Slope: 0.0250

Channel Manning's n: 0.0300

Channel Invert Elevation: 110.00 ft

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 6 cfs

Design Flow: 15 cfs

Maximum Flow: 34 cfs

Table 7 - Summary of Culvert Flows at Crossing: Culvert C

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert C Discharge (cfs)	Roadway Discharge (cfs)	Iterations
110.02	6.00	6.00	0.00	1
110.25	8.80	8.80	0.00	1
110.45	11.60	11.60	0.00	1
110.65	14.40	14.40	0.00	1
110.69	15.00	15.00	0.00	1
111.02	20.00	20.00	0.00	1
111.18	22.80	22.80	0.00	1
111.33	25.60	25.60	0.00	1
111.48	28.40	28.40	0.00	1
111.63	31.20	31.20	0.00	1
111.78	34.00	34.00	0.00	1
112.00	38.08	38.08	0.00	Overtopping

Rating Curve Plot for Crossing: Culvert C

Table 8 - Culvert Summary Table: Culvert C

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
6.00	6.00	110.02	1.019	0.0*	1-S2n	0.444	0.766	0.464	0.251	8.317	2.991
8.80	8.80	110.25	1.247	0.0*	1-S2n	0.537	0.932	0.559	0.318	9.348	3.464
11.60	11.60	110.45	1.446	0.0*	1-S2n	0.615	1.075	0.651	0.377	9.913	3.848
14.40	14.40	110.65	1.651	0.0*	1-S2n	0.685	1.204	0.726	0.431	10.529	4.175
15.00	15.00	110.69	1.694	0.0*	1-S2n	0.699	1.231	0.732	0.442	10.844	4.239
20.00	20.00	111.02	2.018	0.0*	1-S2n	0.808	1.436	0.867	0.530	11.398	4.720
22.80	22.80	111.18	2.180	0.0*	1-S2n	0.864	1.536	0.930	0.575	11.792	4.954
25.60	25.60	111.33	2.333	0.0*	1-S2n	0.918	1.629	0.988	0.619	12.175	5.169
28.40	28.40	111.48	2.481	0.0*	1-S2n	0.969	1.721	1.052	0.661	12.401	5.368
31.20	31.20	111.63	2.628	0.0*	1-S2n	1.019	1.809	1.106	0.702	12.736	5.555
34.00	34.00	111.78	2.776	0.0*	1-S2n	1.066	1.890	1.166	0.742	12.929	5.729

* Full Flow Headwater elevation is below inlet invert.

Straight Culvert

Inlet Elevation (invert): 109.00 ft, Outlet Elevation (invert): 106.00 ft

Culvert Length: 110.04 ft, Culvert Slope: 0.0273

Culvert Performance Curve Plot: Culvert C

Water Surface Profile Plot for Culvert: Culvert C

Site Data - Culvert C

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 109.00 ft

Outlet Station: 110.00 ft

Outlet Elevation: 106.00 ft

Number of Barrels: 1

Culvert Data Summary - Culvert C

Barrel Shape: Circular

Barrel Diameter: 3.00 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0120

Culvert Type: Straight

Inlet Configuration: Beveled Edge (1:1)

Inlet Depression: None

Table 9 - Downstream Channel Rating Curve (Crossing: Culvert C)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
6.00	106.25	0.25	2.99	0.39	1.05
8.80	106.32	0.32	3.46	0.50	1.08
11.60	106.38	0.38	3.85	0.59	1.10
14.40	106.43	0.43	4.17	0.67	1.12
15.00	106.44	0.44	4.24	0.69	1.12
20.00	106.53	0.53	4.72	0.83	1.14
22.80	106.58	0.58	4.95	0.90	1.15
25.60	106.62	0.62	5.17	0.97	1.16
28.40	106.66	0.66	5.37	1.03	1.16
31.20	106.70	0.70	5.55	1.10	1.17
34.00	106.74	0.74	5.73	1.16	1.17

Tailwater Channel Data - Culvert C

Tailwater Channel Option: Rectangular Channel

Bottom Width: 8.00 ft

Channel Slope: 0.0250

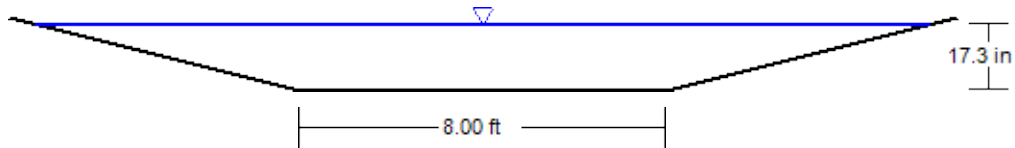
Channel Manning's n: 0.0300

Channel Invert Elevation: 106.00 ft

Wash Cross Sections Existing Condition

Cross Section for Chnl A

Project Description	
Friction Method	Manning
Solve For	Formula
	Normal Depth
Input Data	
Roughness Coefficient	0.040
Channel Slope	0.040 ft/ft
Normal Depth	17.3 in
Left Side Slope	4.000 H:V
Right Side Slope	4.000 H:V
Bottom Width	8.00 ft
Discharge	148.00 cfs



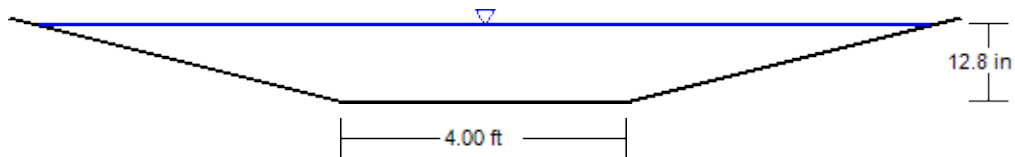
V: 1
H: 1

Worksheet for Chnl A

Project Description	
Friction Method	Manning
Solve For	Formula
	Normal Depth
Input Data	
Roughness Coefficient	0.040
Channel Slope	0.040 ft/ft
Left Side Slope	4.000 H:V
Right Side Slope	4.000 H:V
Bottom Width	8.00 ft
Discharge	148.00 cfs
Results	
Normal Depth	17.3 in
Flow Area	19.9 ft ²
Wetted Perimeter	19.9 ft
Hydraulic Radius	12.0 in
Top Width	19.56 ft
Critical Depth	20.0 in
Critical Slope	0.023 ft/ft
Velocity	7.43 ft/s
Velocity Head	0.86 ft
Specific Energy	2.30 ft
Froude Number	1.298
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	17.3 in
Critical Depth	20.0 in
Channel Slope	0.040 ft/ft
Critical Slope	0.023 ft/ft

Cross Section for Chnl C

Project Description	
Friction Method	Manning
Solve For	Formula
	Normal Depth
Input Data	
Roughness Coefficient	0.040
Channel Slope	0.030 ft/ft
Normal Depth	12.8 in
Left Side Slope	4.000 H:V
Right Side Slope	4.000 H:V
Bottom Width	4.00 ft
Discharge	44.00 cfs



Worksheet for Chnl C

Project Description	
Friction Method	Manning
Solve For	Formula Normal Depth
Input Data	
Roughness Coefficient	0.040
Channel Slope	0.030 ft/ft
Left Side Slope	4.000 H:V
Right Side Slope	4.000 H:V
Bottom Width	4.00 ft
Discharge	44.00 cfs
Results	
Normal Depth	12.8 in
Flow Area	8.8 ft ²
Wetted Perimeter	12.8 ft
Hydraulic Radius	8.2 in
Top Width	12.51 ft
Critical Depth	13.1 in
Critical Slope	0.027 ft/ft
Velocity	5.01 ft/s
Velocity Head	0.39 ft
Specific Energy	1.45 ft
Froude Number	1.055
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	12.8 in
Critical Depth	13.1 in
Channel Slope	0.030 ft/ft
Critical Slope	0.027 ft/ft

Worksheet for EX A1

Project Description

Friction Method	Manning
	Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.022 ft/ft
Discharge	155.00 cfs

Section Definitions

Station (ft)	Elevation (ft)
0+00	9.50
0+24	8.00
0+32	6.50
0+40	6.50
0+54	7.00
0+72	8.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 9.50)	(0+72, 8.00)	0.030

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	13.1 in
Elevation Range	6.5 to 9.5 ft
Flow Area	26.9 ft ²
Wetted Perimeter	38.6 ft
Hydraulic Radius	8.4 in
Top Width	38.49 ft
Normal Depth	13.1 in
Critical Depth	14.4 in
Critical Slope	0.014 ft/ft
Velocity	5.77 ft/s
Velocity Head	0.52 ft
Specific Energy	1.61 ft
Froude Number	1.217
Flow Type	Supercritical

GVF Input Data

Worksheet for EX A1

GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	13.1 in
Critical Depth	14.4 in
Channel Slope	0.022 ft/ft
Critical Slope	0.014 ft/ft

Worksheet for EX C1

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth
Input Data	
Channel Slope	0.035 ft/ft
Discharge	156.00 cfs

Section Definitions

Station (ft)	Elevation (ft)
0+00	2.00
0+50	1.80
0+52	1.00
0+80	1.00
0+85	2.00
0+90	3.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 2.00)	(0+90, 3.00)	0.030

Options	
Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results	
Normal Depth	8.7 in
Elevation Range	1.0 to 3.0 ft
Flow Area	22.2 ft ²
Wetted Perimeter	33.6 ft
Hydraulic Radius	7.9 in
Top Width	33.42 ft
Normal Depth	8.7 in
Critical Depth	12.5 in
Critical Slope	0.017 ft/ft
Velocity	7.03 ft/s
Velocity Head	0.77 ft
Specific Energy	1.49 ft
Froude Number	1.520
Flow Type	Supercritical

GVF Input Data

Worksheet for EX C1

GVF Input Data

Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.0 in
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	8.7 in
Critical Depth	12.5 in
Channel Slope	0.035 ft/ft
Critical Slope	0.017 ft/ft

Worksheet for EX C2

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.035 ft/ft
Discharge	156.00 cfs

Section Definitions

Station (ft)	Elevation (ft)
0+00	91.50
0+20	90.50
0+90	90.50
1+20	91.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 91.50)	(1+20, 91.00)	0.030

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	4.8 in
Elevation Range	90.5 to 91.5 ft
Flow Area	34.6 ft ²
Wetted Perimeter	102.2 ft
Hydraulic Radius	4.1 in
Top Width	102.18 ft
Normal Depth	4.8 in
Critical Depth	5.8 in
Critical Slope	0.018 ft/ft
Velocity	4.50 ft/s
Velocity Head	0.32 ft
Specific Energy	0.72 ft
Froude Number	1.364
Flow Type	Supercritical

GVF Input Data

Downstream Depth	0.0 in
------------------	--------

Worksheet for EX C2

GVF Input Data	
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	4.8 in
Critical Depth	5.8 in
Channel Slope	0.035 ft/ft
Critical Slope	0.018 ft/ft

Wash Cross Sections Proposed Condition

Worksheet for PROP A1

Project Description	
Friction Method	Manning
Solve For	Formula
	Normal Depth
Input Data	
Roughness Coefficient	0.040
Channel Slope	0.040 ft/ft
Left Side Slope	4.000 H:V
Right Side Slope	4.000 H:V
Bottom Width	8.00 ft
Discharge	155.00 cfs
Results	
Normal Depth	17.8 in
Flow Area	20.6 ft ²
Wetted Perimeter	20.2 ft
Hydraulic Radius	12.2 in
Top Width	19.84 ft
Critical Depth	20.5 in
Critical Slope	0.023 ft/ft
Velocity	7.53 ft/s
Velocity Head	0.88 ft
Specific Energy	2.36 ft
Froude Number	1.302
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	17.8 in
Critical Depth	20.5 in
Channel Slope	0.040 ft/ft
Critical Slope	0.023 ft/ft

Worksheet for PROP C1

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth
Input Data	
Channel Slope	0.035 ft/ft
Discharge	156.00 cfs

Section Definitions

Station (ft)	Elevation (ft)
0+00	4.00
0+20	4.00
0+20	2.00
0+50	1.80
0+52	1.00
0+80	1.00
0+85	2.00
0+90	3.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 4.00)	(0+90, 3.00)	0.030

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	8.7 in
Elevation Range	1.0 to 4.0 ft
Flow Area	22.2 ft ²
Wetted Perimeter	33.6 ft
Hydraulic Radius	7.9 in
Top Width	33.42 ft
Normal Depth	8.7 in
Critical Depth	12.4 in
Critical Slope	0.016 ft/ft
Velocity	7.03 ft/s
Velocity Head	0.77 ft
Specific Energy	1.49 ft
Froude Number	1.520
Flow Type	Supercritical

Worksheet for PROP C1

GVF Input Data

Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.0 in
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	8.7 in
Critical Depth	12.4 in
Channel Slope	0.035 ft/ft
Critical Slope	0.016 ft/ft

Worksheet for PROP C2

Project Description	
Friction Method	Manning
Solve For	Formula
	Normal Depth
Input Data	
Channel Slope	0.035 ft/ft
Discharge	156.00 cfs

Section Definitions

Station (ft)	Elevation (ft)
0+00	95.00
0+10	95.00
0+11	91.50
0+20	90.50
0+90	90.50
1+20	91.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 95.00)	(1+20, 91.00)	0.030

Options	
Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results	
Normal Depth	4.9 in
Elevation Range	90.5 to 95.0 ft
Flow Area	34.1 ft ²
Wetted Perimeter	98.0 ft
Hydraulic Radius	4.2 in
Top Width	97.98 ft
Normal Depth	4.9 in
Critical Depth	5.9 in
Critical Slope	0.018 ft/ft
Velocity	4.58 ft/s
Velocity Head	0.33 ft
Specific Energy	0.73 ft
Froude Number	1.370
Flow Type	Supercritical

Worksheet for PROP C2

GVF Input Data

Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

GVF Output Data

Upstream Depth	0.0 in
Profile Description	
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	4.9 in
Critical Depth	5.9 in
Channel Slope	0.035 ft/ft
Critical Slope	0.018 ft/ft

Appendix D – Army Corp Jurisdiction Delineation



DEPARTMENT OF THE ARMY
U.S. ARMY CORPS OF ENGINEERS, LOS ANGELES DISTRICT
3636 N CENTRAL AVENUE, SUITE 900
PHOENIX, AZ 85012-1939

June 26, 2019

SUBJECT: Determination of Need for Department of the Army Permit

Gideon H. Zeidler
Parolo, LLC
7775 E Fledgling Drive
Scottsdale, Arizona 85255

Dear Mr. Zeidler:

I am responding to your request (File No. SPL-2019-00519) dated May 29, 2019, for clarification whether a Department of the Army Permit is required concerning the proposed development of your property, located within 40 acres of land south of Happy Valley Road and west of 92nd Street, in a portion of Section 7, Township 4 North, Range 5 East (33.710817°N, -111.886054°W), City of Scottsdale, Maricopa County, Arizona.

The Corps' evaluation process for determining if you need a permit is based on whether or not the proposed project is located within or contains a water of the United States, and whether or not the proposed project includes an activity potentially regulated under Section 10 of the Rivers and Harbors Act or Section 404 of the Clean Water Act. If both conditions are met, a permit would be required.

Based on the attached dry land approved jurisdictional determination dated June 24, 2019, it appears that your project site (described above) does not contain waters of the United States pursuant to 33 CFR Part 325.9. Although I have determined the proposed project does not require a permit under Section 404 of the Clean Water Act pursuant to 33 CFR Part 323.4, your proposed project may be regulated under other Federal, State, and local laws.

If you have any questions, please contact Ann Palaruan at (602) 230-6955 or via e-mail at Cynthia.A.Palaruan@usace.army.mil. Thank you for participating in the Regulatory Program. Please help me to evaluate and improve the regulatory experience for others by completing the customer survey form at http://corpsmapu.usace.army.mil/cm_apex/f?p=regulatory_survey.

Sincerely,

A handwritten signature in blue ink that reads "Sallie Diebolt".

Sallie Diebolt
Chief, Arizona Branch
Regulatory Division

DRY LAND APPROVED JURISDICTIONAL DETERMINATION FORM¹
U.S. Army Corps of Engineers

This form should be completed by following the instructions provided in Section IV of the JD Form Instructional Guidebook.

SECTION I: BACKGROUND INFORMATION

A. REPORT COMPLETION DATE FOR APPROVED JURISDICTIONAL DETERMINATION (JD): 6/24/2019

B. DISTRICT OFFICE AND FILE NUMBER: Los Angeles District, SPL-2019-00519

C. PROJECT LOCATION AND BACKGROUND INFORMATION:

State: Arizona County: Maricopa

Coordinates of site (lat/long in degree decimal format): Lat. 33°710817N, Long. -111.886054W°

Name of nearest waterbody: Cave Creek

- ☒ Check if map/diagram of review area is available upon request.
- ☐ Check if other sites (e.g., offsite mitigation sites, disposal sites, etc...) are associated with this action and are recorded on a different JD form.

D. REVIEW PERFORMED FOR SITE EVALUATION (CHECK ALL THAT APPLY):

- ☒ Office (Desk) Determination. Date: 6/24/2019
- ☐ Field Determination. Date(s):

SECTION II: SUMMARY OF FINDINGS

A. RHA SECTION 10 DETERMINATION OF JURISDICTION.

There **are no** "navigable waters of the U.S." within Rivers and Harbors Act (RHA) jurisdiction (as defined by 33 CFR part 329) in the review area.

B. CWA SECTION 404 DETERMINATION OF JURISDICTION.

There **are no** "waters of the U.S." within Clean Water Act (CWA) jurisdiction (as defined by 33 CFR part 328) in the review area.

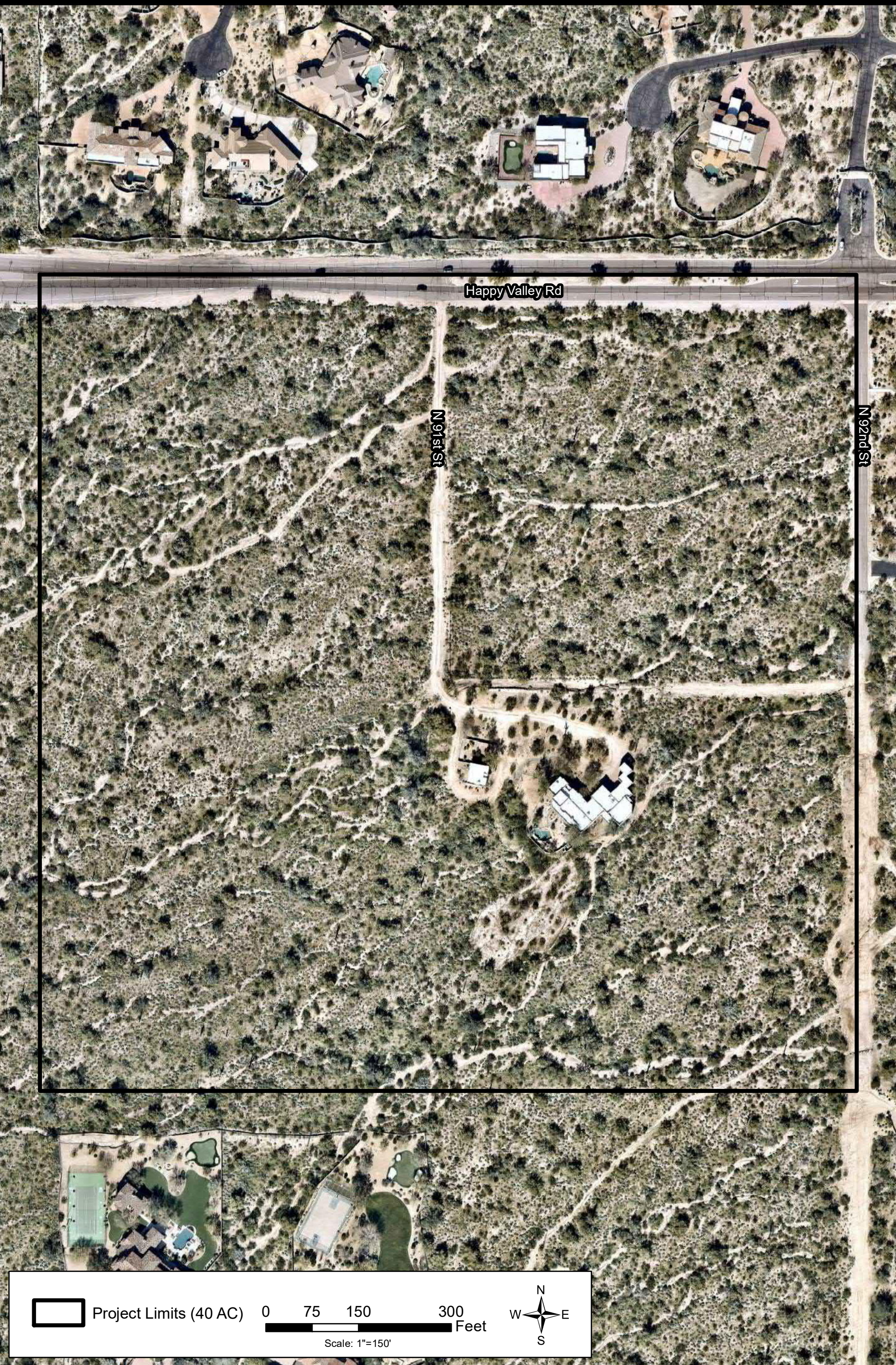
SECTION III: DATA SOURCES.


A. SUPPORTING DATA. Data reviewed for JD (check all that apply - checked items shall be included in case file and, where checked and requested, appropriately reference sources below):

- ☒ Maps, plans, plots or plat submitted by or on behalf of the applicant/consultant:
Data sheets prepared/submitted by or on behalf of the applicant/consultant.
Office concurs with data sheets/delineation report.
Office does not concur with data sheets/delineation report.
Data sheets prepared by the Corps:
U.S. Geological Survey Hydrologic Atlas:
USGS NHD data.
USGS 8 and 12 digit HUC maps.
- ☒ U.S. Geological Survey map(s). Quad name: Curry's Corner, Ariz.
USDA Natural Resources Conservation Service Soil Survey. Citation: Aguila-Carefree Area, Arizona, Parts of Maricopa and Pinal Counties
National wetlands inventory map(s). Cite name:
State/Local wetland inventory map(s):
FEMA/FIRM maps:
100-year Floodplain Elevation is:
- ☒ Photographs: ☒ Aerial (Name & Date): Goggle Map Imagery, Yr2019
☒ or ☒ (Name & Date): Field Photos: 2/2018
- ☐ Applicable/supporting case law:

B. REQUIRED ADDITIONAL COMMENTS TO SUPPORT JD. EXPLAIN RATIONALE FOR DETERMINATION THAT THE REVIEW AREA ONLY INCLUDES DRY LAND: This property lacks physical indicators of a 404 Ordinary Highwater Mark. No Clean Water Act Section 404 jurisdiction is evident on this property.

¹ This form is for use only in recording approved JDs involving dry land. It extracts the relevant elements of the longer approved JD form in use since 2007 for aquatic areas and adds no new fields.



 Expect More. Experience Better.	HV91	Scottsdale, Maricopa County, Arizona
	Project Aerial	

FINAL SEWER BASIS OF DESIGN REPORT

Solitude

Southeast of Happy Valley Road and Pima Road
Scottsdale, Arizona

FINAL Basis of Design Report

☒ APPROVED

☐ APPROVED AS NOTED

☐ REVISE AND RESUBMIT



Disclaimer: If approved; the approval is granted under the condition that the final construction documents submitted for city review will match the information herein. Any subsequent changes in the water or sewer design that materially impact design criteria or standards will require re-analysis, re-submittal, and approval of a revised basis of design report prior to the plan review submission.; this approval is not a guarantee of construction document acceptance. For questions or clarifications contact the Water Resources Planning and Engineering Department at 480-312-5685.

BY scan

DATE 10/19/2020

Prepared for:

Sonora West Development INC.
8937 East Bell Road
Suite 100
Scottsdale, Arizona 85260



Prepared by:

Kimley-Horn and Associates
1001 West Southern Avenue, Suite 131
Mesa, Arizona 85210
291203001
May 2020



FINAL SEWER BASIS OF DESIGN REPORT

SOLITUDE
SOUTHEAST OF HAPPY VALLEY ROAD AND PIMA ROAD
SCOTTSDALE, ARIZONA

MAY 2020

Prepared By:

Kimley»Horn

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Site Location.....	2
Project Size and Type	2
Purpose and Objectives	2
Collection system description	4
Existing collection system	4
Proposed Collection System	4
Basis of Design	6
Design Criteria	6
Wastewater system analysis	7
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Appendices

Appendix A – Flowmaster Calculations

Appendix B - Pump Selection

Appendix C - Lift Station Report

INTRODUCTION

SITE LOCATION

This Final Sewer Basis of Design Report (SewerBOD) has been prepared for the proposed Solitude single family development located southeast of Happy Valley Road and Pima Road in Scottsdale, Arizona (development). The development is bound to the West by the 91st Street alignment, to the north by Happy Valley Road, to the south by undeveloped land, and to the west by the 92nd Street alignment. The development is located within Section 7 of Township 4 North, Range 5 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. Refer to **Figure 1** for the Vicinity Map.

PROJECT SIZE AND TYPE

The development is a proposed 17-unit single family residential subdivision. The proposed buildings are one-story units. The development is approximately 20 acres.

PURPOSE AND OBJECTIVES

This report presents the basis of design criteria that will be used for engineering design of the proposed development. This report establishes a sewer solution for the development of the site.

- Demonstrate compliance with the City's Design Standards & Policies Manual (DSPM).
- Identify a final sewer system layout for the proposed development.
- Determination of the sewer demand generated by the development.
- Analysis of the capacity of the development's gravity sewer system.
- Proposed lift station location and force main sizing.

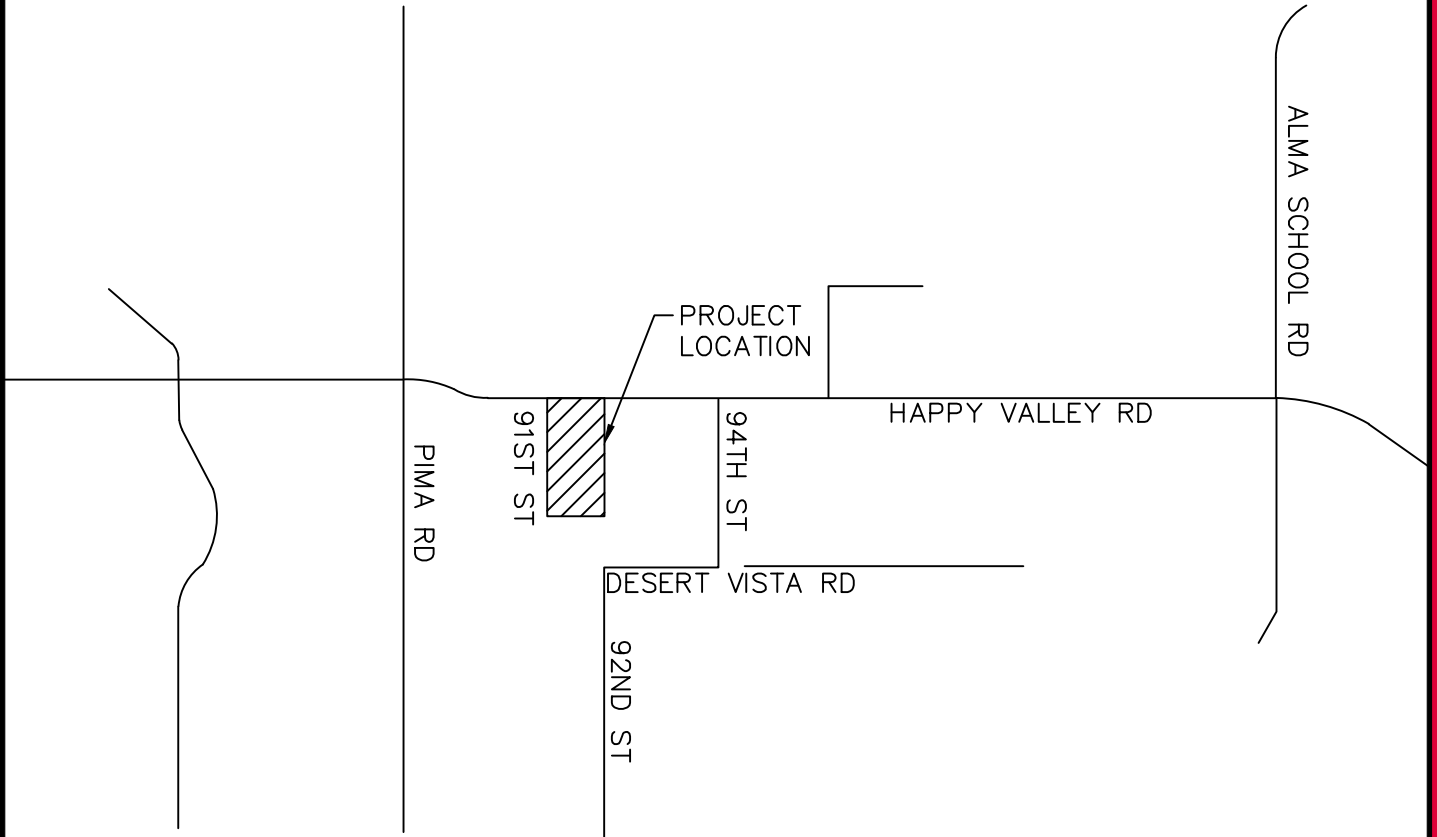


FIGURE 1
VICINITY MAP



COLLECTION SYSTEM DESCRIPTION

EXISTING COLLECTION SYSTEM

The existing site is undeveloped natural desert and one existing residential lot. The site slopes consistently in the southwesterly direction across the site. Existing grade elevations on the site range from approximately 2125-2080. Based on a review of the City Quarter Section Maps, there is existing 8-inch sewer in Happy Valley Road routing flow west to Pima Road. Existing invert elevations of the 8-inch sewer in Happy Valley Road along the development frontage appear to range from 2096-2120+/- according to the City Quarter Section Maps. Additionally, there is an 8-inch sewer in the 92nd Street which appears to be a dry line and run south from the Whispering Wind alignment. The existing home is served by a septic system.

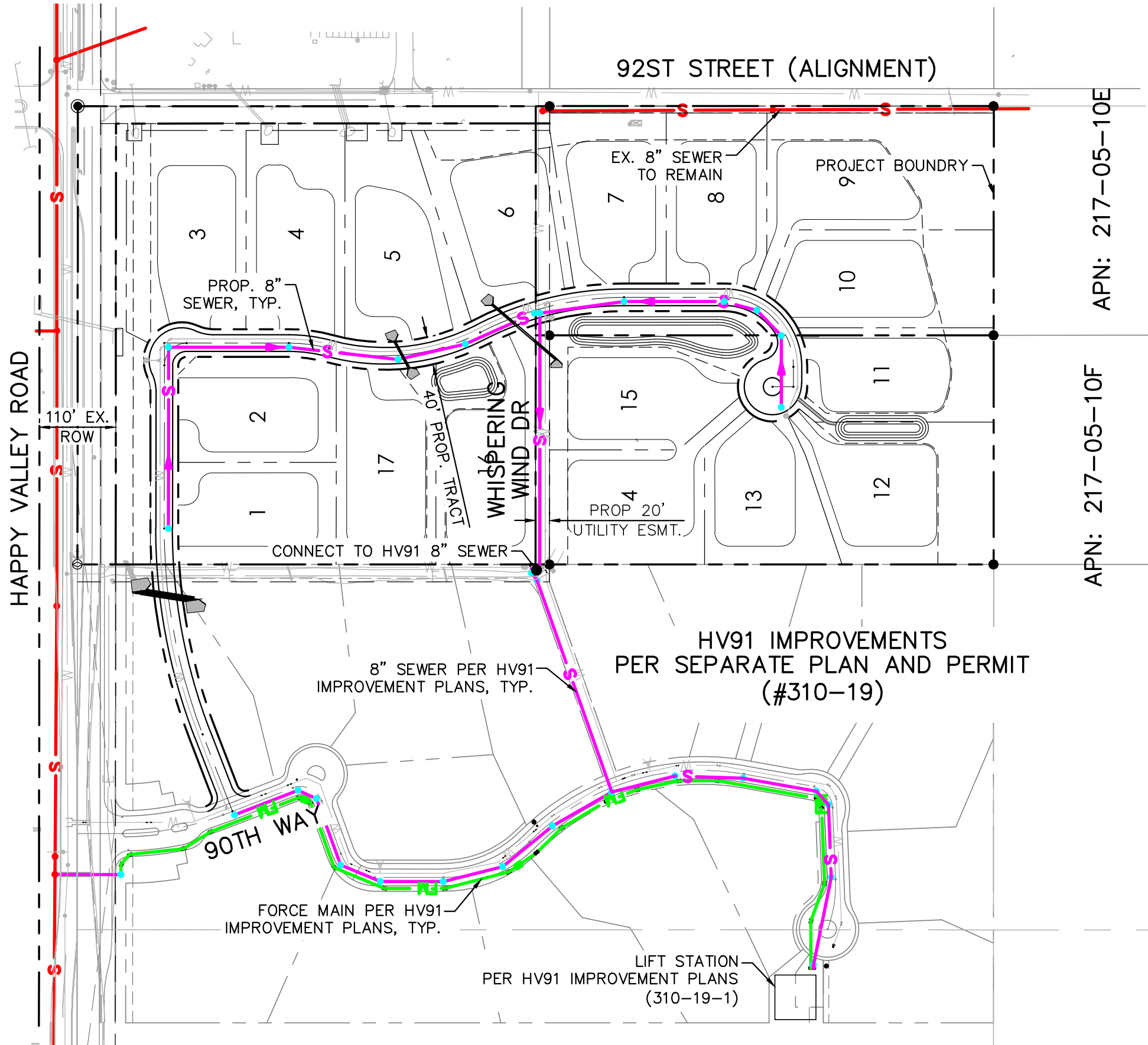
PROPOSED COLLECTION SYSTEM

On the western boarder of the proposed development is the HV91 development (CoS Plan Check #310-19). As a part of the HV91 project a lift station with a force main line connecting to the existing 8-inch sewer line in Happy Valley Road has been approved. The Lift station was anticipated to accommodate the flows from HV91 and future development. The wet well and forcemain can accommodate the additional flow. A larger pump will be installed to handle the increased flow. As a part of HV91 an 8-inch sewer stub will be installed at the western boundary of the proposed development, in the Whispering Wind alignment. A biofilter system will be designed and installed for the outfall of the force main system at Happy Valley Road.

The proposed development consists of 17 single family residential units. This development will be served by an 8-inch public sewer system within the private roadway tract and will connect to the 8-inch sewer stub installed with the HV91 system. From there flows will connect to the lift station described above. Refer to **Figure 2** for the proposed sewer layout.

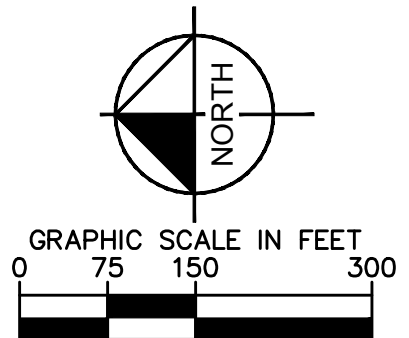
In addition to the installation of the proposed sewer system, the existing septic system will be removed from the site.

K:\EAV_CHA\391903001 - HV91 CADD\Exhibits\A Sewer.sdw.dwg Sep 28, 2019 dan.recker
XREFS: x30001bm-rp x30001va x30000le x30000ut x30000m
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LEGEND

- EX. SEWER LINE
- PROP. SEWER LINE
- FORCE MAIN PER HV91 IMPROVEMENT PLANS
- PROP. SEWER MANHOLE



Kimley»Horn

C 2019 KIMLEY-HORN AND ASSOCIATES, INC.
1001 West Southern Avenue, Suite 131
Mesa, Arizona 85210 (480) 207-2666

SCALE (H): 1"=150'
SCALE (V): NONE
DESIGNED BY: TAF
DRAWN BY: DAK
CHECKED BY: JMB
DATE: SEP 2019

SOLITUDE
FINAL SEWER BOD
FIGURE 2 - SEWER SYSTEM LAYOUT
SCOTTSDALE, ARIZONA

PROJECT NO.
291903001
DRAWING NAME

1 OF 1

BASIS OF DESIGN

DESIGN CRITERIA

The design criteria for the development is based on the City of Scottsdale Design Standards and Policies Manual (DS&PM). Average daily demands and peaking factors for the various building uses were used to determine the existing and proposed peak flows generated on site. See **Table 1** below for a summary of the design criteria used.

Table 1. Wastewater Design Criteria

WASTEWATER DESIGN CRITERIA			
Wastewater Demands			
Land Use	Average Daily Flow (gpd)		Peaking Factor
Residential	250	Per Unit	4
Wastewater Design Criteria			
Minimum Pipe Slope			
8-inch	0.52	%	
Full Flow Velocities			
Minimum	2.5	fps	
Maximum	10	fps	
Manning's Roughness Coefficient (n)	0.013		
Design d/D	0.65		

The proposed site generates a peak flow of approximately 17,000 gpd or 12 gpm. Additionally, a peak demand of 35gpm is added to the lift station flow to account for swimming pool drainage. The previously approved lift station will serve flow from this development and HV91 for a total peak flow of approximately 59 gpm. A new pump has been selected to handle this flow, see **Appendix B**. See **Table 2** below for a summary of the proposed flows generated with the proposed development. See **Appendix C** for the HV91 Lift Station Report.

Table 2. Wastewater Demand Calculations

Wastewater Demand Calculations						
Use	Units	Demand (gpd)	Average Daily Demand (gpd)	Peaking Factor	Peak Flow (gpd)	Peak Flow (gpm)
Solitude	17	250	4,250	4.0	17,000	12
HV91	17	250	4,250	4.0	17,000	12
Pool Drainage	-	-	-	-	50,400	35
Total to Lift Station					84,400	59

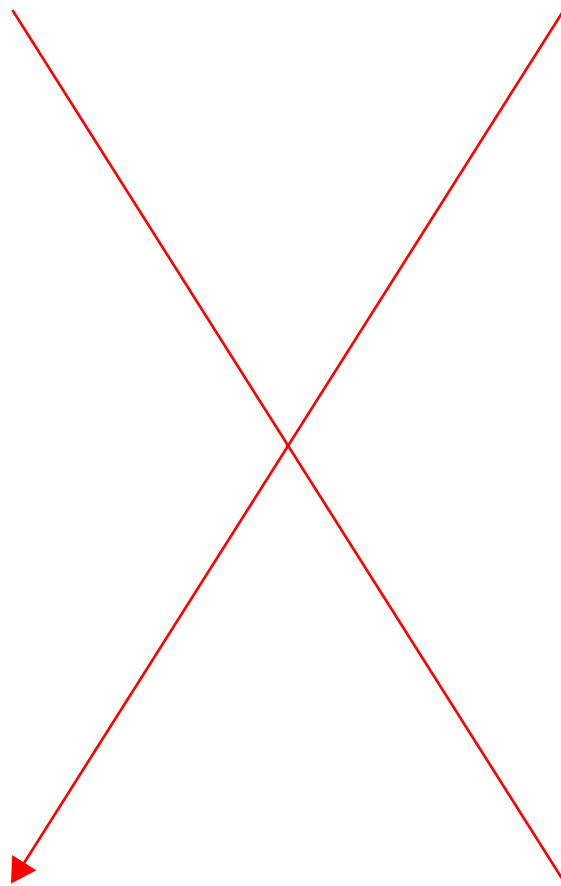
WASTEWATER SYSTEM ANALYSIS

To determine the capacity of the proposed wastewater collection system, the peak design flow was analyzed using the minimum design pipe slope. At the minimum design slope of 0.0052 ft/ft, an 8-inch line has the capacity to convey approximately 391 gpm. An 8-inch line at the minimum design slope can convey the proposed peak design flow from Solitude of 12 gpm at a normal depth of 0.08 or a d/D ratio of 0.13, at a velocity of 1.13 ft/s. An 8-inch line at the minimum design slope can convey the proposed combined peak design flow to the lift station of 59 gpm at a normal depth of 0.167 or a d/D ratio of 0.25, at a velocity of 1.74 ft/s.

The existing 2-inch forcemain conveys the pumped flow of 59 gpm at a velocity of 6 ft/s. See **Appendix A** for Flowmaster results.

CONCLUSION

The proposed development for Solitude results in a generated wastewater peak flow of 17,000 gallons per day. The proposed wastewater flow will be conveyed through a gravity sanitary sewer to connect with the HV91 sewer system. From there the combined peak flow of 84,400 gallons per day to a proposed lift station at the southwest corner of the adjacent property. From there, flow will be pumped back to the existing 8-inch gravity sewer in Happy Valley Road. Previous discussions with City staff has indicated that the existing gravity line in Happy Valley Road has capacity to serve the proposed development flows identified in this report. The scope of this BOD does not include analysis of the existing Happy Valley Road sewer system.



Appendix A – Flowmaster Calculations

WASTEWATER SYSTEM ANALYSIS

To determine the capacity of the proposed wastewater collection system, the peak design flow was analyzed using the minimum design pipe slope. At the minimum design slope of 0.0052 ft/ft, an 8-inch line has the capacity to convey approximately 391 gpm. An 8-inch line at the minimum design slope can convey the proposed peak design flow from Solitude of 12 gpm at a normal depth of 0.08 or a d/D ratio of 0.13, at a velocity of 1.13 ft/s. An 8-inch line at the minimum design slope can convey the proposed combined peak design flow to the lift station of 59 gpm at a normal depth of 0.167 or a d/D ratio of 0.25, at a velocity of 1.74 ft/s.

The existing 2-inch forcemain conveys the pumped flow of 59 gpm at a velocity of 6 ft/s. See Appendix A for Flowmaster results.

CONCLUSION

The proposed development for Solitude results in a generated wastewater peak flow of 17,000 gallons per day. The proposed wastewater flow will be conveyed through a gravity sanitary sewer to connect with the HV91 sewer system. From there the combined peak flow of 84,400 gallons per day to a proposed lift station at the southwest corner of the adjacent property. From there, flow will be pumped back to the existing 8-inch gravity sewer in Happy Valley Road. Previous discussions with City staff has indicated that the existing gravity line in Happy Valley Road has capacity to serve the proposed development flows identified in this report. The scope of this BOD does not include analysis of the existing Happy Valley Road sewer system.

Appendix A – Flowmaster Calculations

Worksheet for 8" Sewer (min. slope)

Project Description	
Friction Method	Manning Formula
Solve For	Full Flow Capacity
Input Data	
Roughness Coefficient	0.013
Channel Slope	0.005 ft/ft
Normal Depth	8.0 in
Diameter	8.0 in
Discharge	0.87 cfs
Results	
Discharge	0.87 cfs
Normal Depth	8.0 in
Flow Area	0.3 ft ²
Wetted Perimeter	2.1 ft
Hydraulic Radius	2.0 in
Top Width	0.00 ft
Critical Depth	5.3 in
Percent Full	100.0 %
Critical Slope	0.009 ft/ft
Velocity	2.50 ft/s
Velocity Head	0.10 ft
Specific Energy	0.76 ft
Froude Number	(N/A)
Maximum Discharge	0.94 cfs
Discharge Full	0.87 cfs
Slope Full	0.005 ft/ft
Flow Type	SubCritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	
Profile Headloss	0.00 ft
Average End Depth Over Rise	0.0 %
Normal Depth Over Rise	100.0 %
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	8.0 in
Critical Depth	5.3 in
Channel Slope	0.005 ft/ft
Critical Slope	0.009 ft/ft

Worksheet for 8" Sewer (Design)

Project Description	
Friction Method	Manning
	Formula
Solve For	Normal Depth
Input Data	
Roughness Coefficient	0.013
Channel Slope	0.005 ft/ft
Diameter	8.0 in
Discharge	0.03 cfs
Results	
Normal Depth	1.0 in
Flow Area	0.0 ft ²
Wetted Perimeter	0.5 ft
Hydraulic Radius	0.6 in
Top Width	0.43 ft
Critical Depth	0.9 in
Percent Full	12.0 %
Critical Slope	0.007 ft/ft
Velocity	1.13 ft/s
Velocity Head	0.02 ft
Specific Energy	0.10 ft
Froude Number	0.849
Maximum Discharge	0.94 cfs
Discharge Full	0.87 cfs
Slope Full	0.000 ft/ft
Flow Type	Subcritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	
Profile Headloss	0.00 ft
Average End Depth Over Rise	0.0 %
Normal Depth Over Rise	12.0 %
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	1.0 in
Critical Depth	0.9 in
Channel Slope	0.005 ft/ft
Critical Slope	0.007 ft/ft

Worksheet for 8" Sewer (Combined Design)

Project Description	
Friction Method	Manning
	Formula
Solve For	Normal Depth
Input Data	
Roughness Coefficient	0.013
Channel Slope	0.005 ft/ft
Diameter	8.0 in
Discharge	53.00 gal/min
Results	
Normal Depth	2.0 in
Flow Area	0.1 ft ²
Wetted Perimeter	0.7 ft
Hydraulic Radius	1.2 in
Top Width	0.58 ft
Critical Depth	1.9 in
Percent Full	24.9 %
Critical Slope	0.007 ft/ft
Velocity	1.74 ft/s
Velocity Head	0.05 ft
Specific Energy	0.21 ft
Froude Number	0.897
Maximum Discharge	420.70 gal/min
Discharge Full	391.09 gal/min
Slope Full	0.000 ft/ft
Flow Type	Subcritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	
Profile Headloss	0.00 ft
Average End Depth Over Rise	0.0 %
Normal Depth Over Rise	12.0 %
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	2.0 in
Critical Depth	1.9 in
Channel Slope	0.005 ft/ft
Critical Slope	0.007 ft/ft

Worksheet for Force main

Project Description	
Friction Method	Manning
Solve For	Formula Pressure at 1
Input Data	
Pressure 2	0 psi
Elevation 1	2,070.00 ft
Elevation 2	2,105.00 ft
Length	1,200.0 ft
Roughness Coefficient	0.010
Diameter	2.0 in
Discharge	59.00 gal/min
Results	
Pressure 1	74 psi
Headloss	136.60 ft
Energy Grade 1	2,242.16 ft
Energy Grade 2	2,105.56 ft
Hydraulic Grade 1	2,241.60 ft
Hydraulic Grade 2	2,105.00 ft
Flow Area	0.0 ft ²
Wetted Perimeter	0.5 ft
Velocity	6.03 ft/s
Velocity Head	0.56 ft
Friction Slope	0.114 ft/ft

Appendix B – Pump Selection

MP 3127 HT 3~ 262

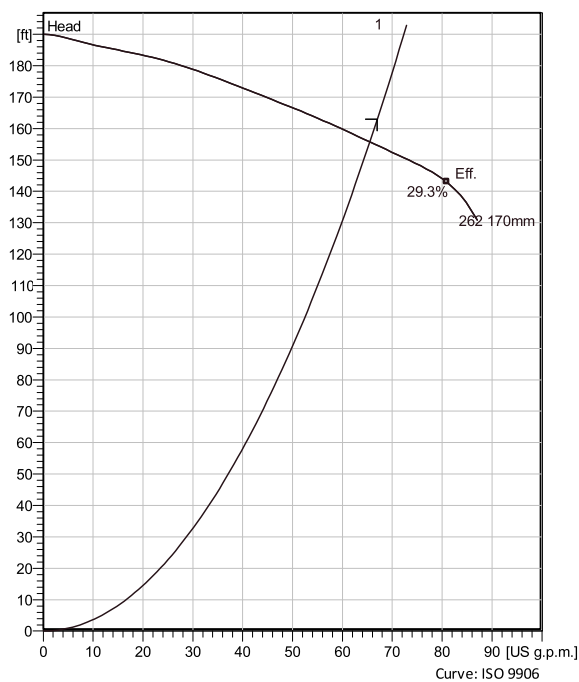
Semi-open multi-channel impellers with integral grinder cutter in single volute casing for liquids containing solids and fibres.



Technical specification



Curves according to: Water, pure [100%], 39.2 °F, 62.42 lb/ft³, 1.6891E-5 ft²/s



Configuration

Motor number M3127.170 21-11-2AL-W 11hp	Installation type P - Semi permanent, Wet
Impeller diameter 170 mm	Discharge diameter 1 15/16 inch

Pump information

Impeller diameter 170 mm
Discharge diameter 1 15/16 inch
Inlet diameter 50 mm
Maximum operating speed 3495 rpm
Number of blades 6

Max. operating temperature
40 °F

Materials

Impeller Grey cast iron
Stator housing material Grey cast iron

Project
Block

0

Created by
Created on

3/4/2020

Last update

MP 3127 HT 3~ 262

Technical specification



Motor - General

Motor number M3127.170 21-11-2AL-W 11hp	Phases 3~	Rated speed 3495 rpm	Rated power 11 hp
Approval No	Number of poles 2	Rated current 13 A	Stator variant 12
Frequency 60 Hz	Rated voltage 460 V	Insulation class H	Type of Duty S1
Version code 170			

Motor - Technical

Power factor - 1/1 Load 0.92	Motor efficiency - 1/1 Load 87.6 %	Total moment of inertia 0.285 lb ft ²	Starts per hour max. 30
Power factor - 3/4 Load 0.90	Motor efficiency - 3/4 Load 88.4 %	Starting current, direct starting 110 A	
Power factor - 1/2 Load 0.85	Motor efficiency - 1/2 Load 87.7 %	Starting current, star-delta 36.7 A	

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Last update

MP 3127 HT 3~ 262

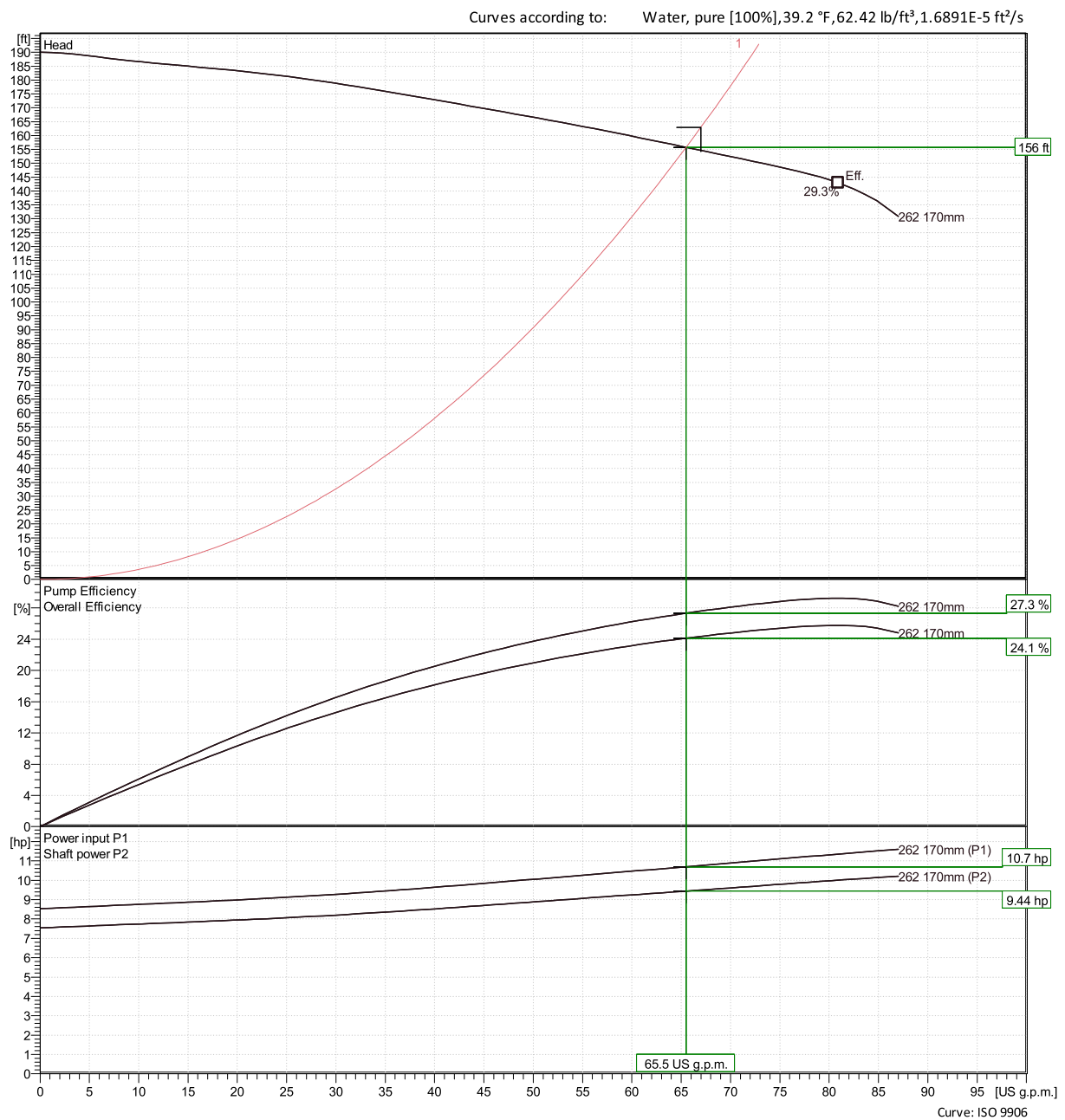
Performance curve



Duty point

Flow
65.5 US g.p.m.

Head
156 ft



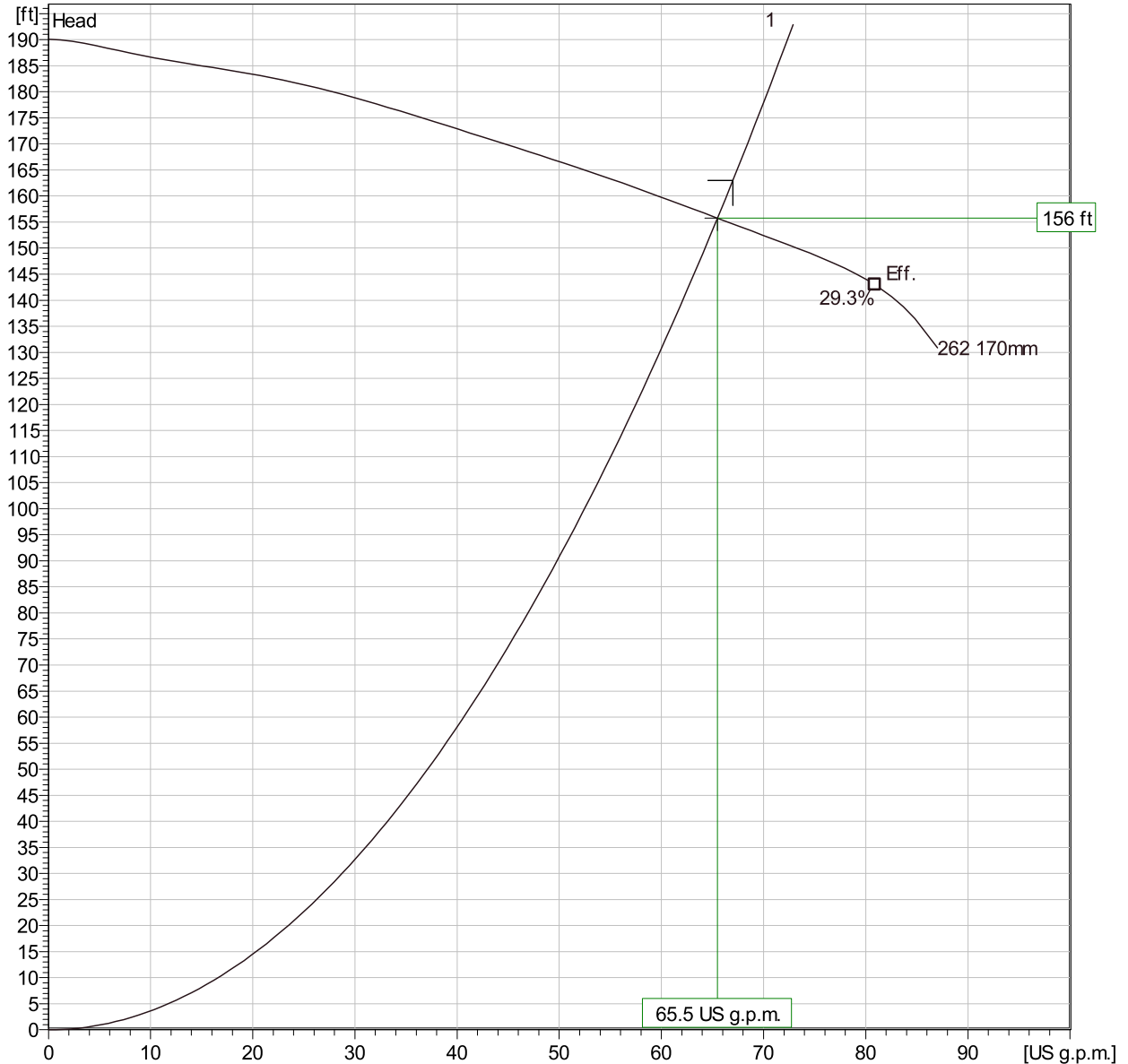
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MP 3127 HT 3~ 262

Duty Analysis



Curves according to: Water, pure [100%], 39.2 °F, 62.42 lb/ft³, 1.6891E-5 ft²/s



Operating characteristics

Pumps/Systems	Flow	Head	Shaft power	Flow	Head	Shaft power	Hydr.eff.	Specific energy	NPSHr
1	65.5 US g.p.m.	156 ft	9.44 hp	65.5 US g.p.m.	156 ft	9.44 hp	27.3 %	2030 kWh/US M	

Project

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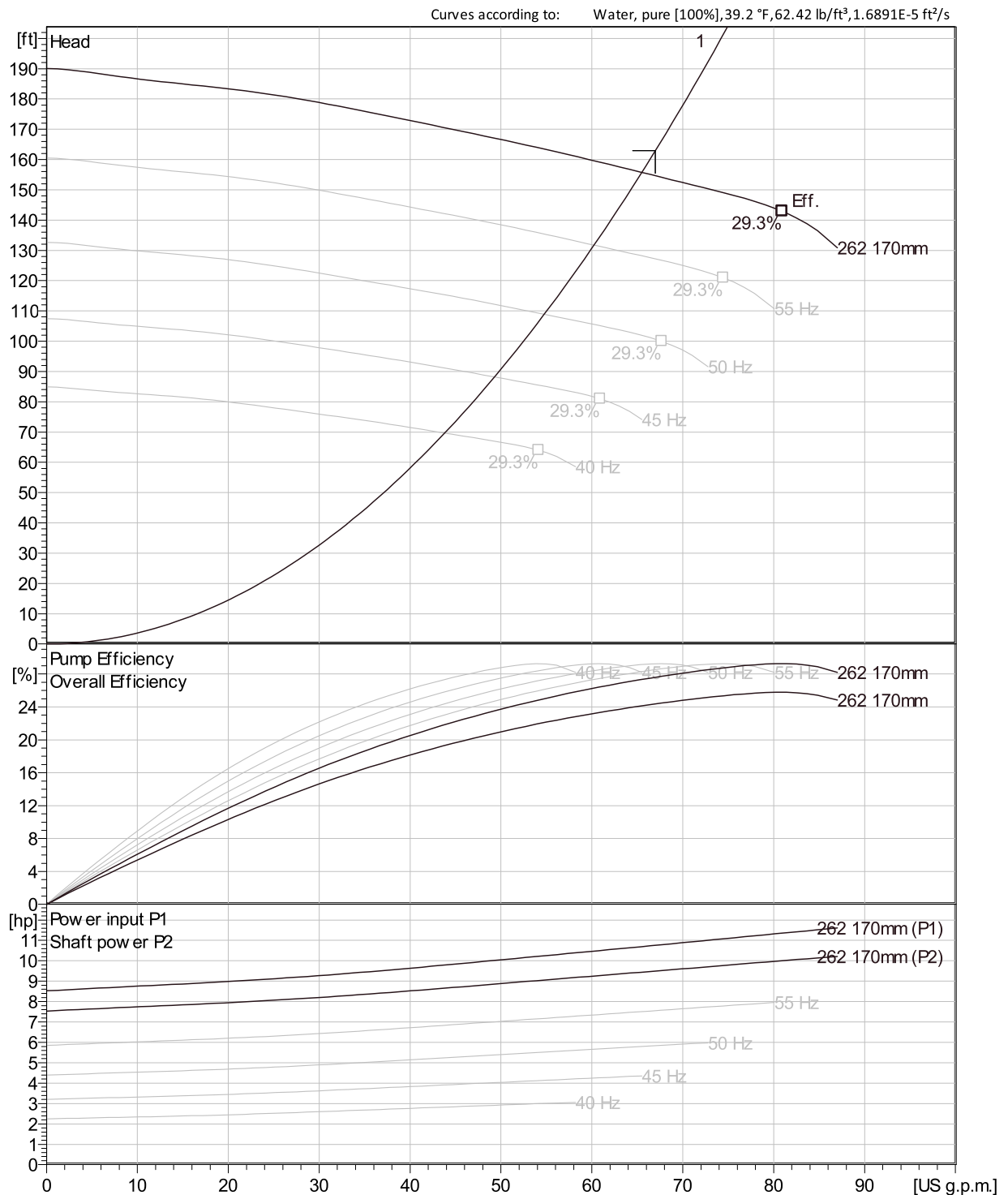
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MP 3127 HT 3~ 262

VFD Curve



Curve: ISO 9906

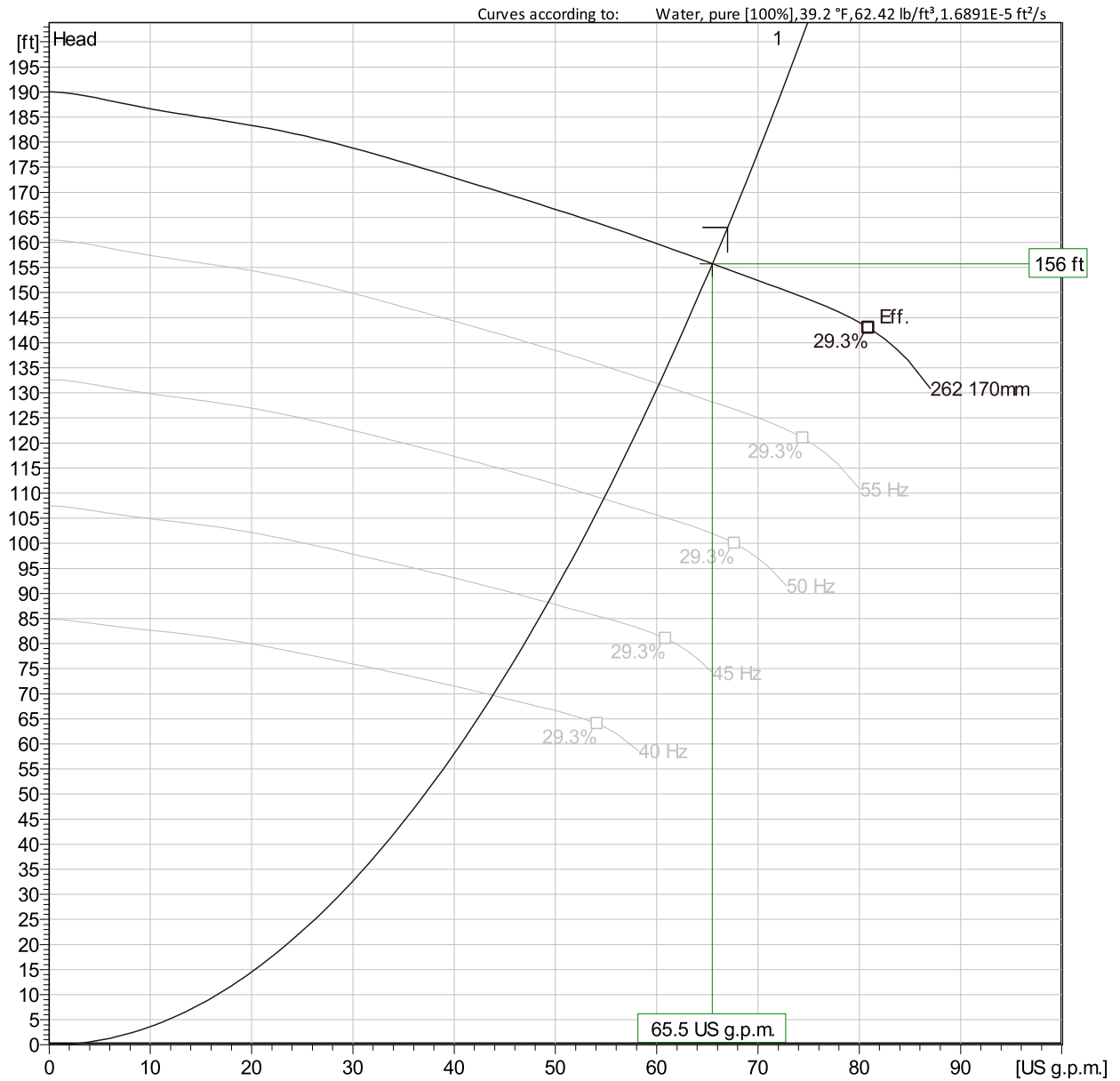
Project
Block 0

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MP 3127 HT 3~ 262

VFD Analysis



Curve: ISO 9906

Operating Characteristics

Pumps/Syste s	Frequency	Flow	Head	Shaft power	Flow	Head	Shaft power	Hydr.eff.	Specific Energy	NPSHr
1	59.9 Hz	65.5 US g.p.m	156 ft	9.44 hp	65.5 US g.p.m	156 ft	9.44 hp	27.3 %	2030 kWh/US M	
1	55 Hz	60.2 US g.p.m	132 ft	7.35 hp	60.2 US g.p.m	132 ft	7.35 hp	27.3 %	1720 kWh/US M	
1	50 Hz	54.8 US g.p.m	109 ft	5.52 hp	54.8 US g.p.m	109 ft	5.52 hp	27.3 %	1430 kWh/US M	
1	45 Hz	49.3 US g.p.m	88.2 ft	4.02 hp	49.3 US g.p.m	88.2 ft	4.02 hp	27.3 %	1180 kWh/US M	
1	40 Hz	43.8 US g.p.m	69.7 ft	2.83 hp	43.8 US g.p.m	69.7 ft	2.83 hp	27.3 %	965 kWh/US M	

Project

Block 0

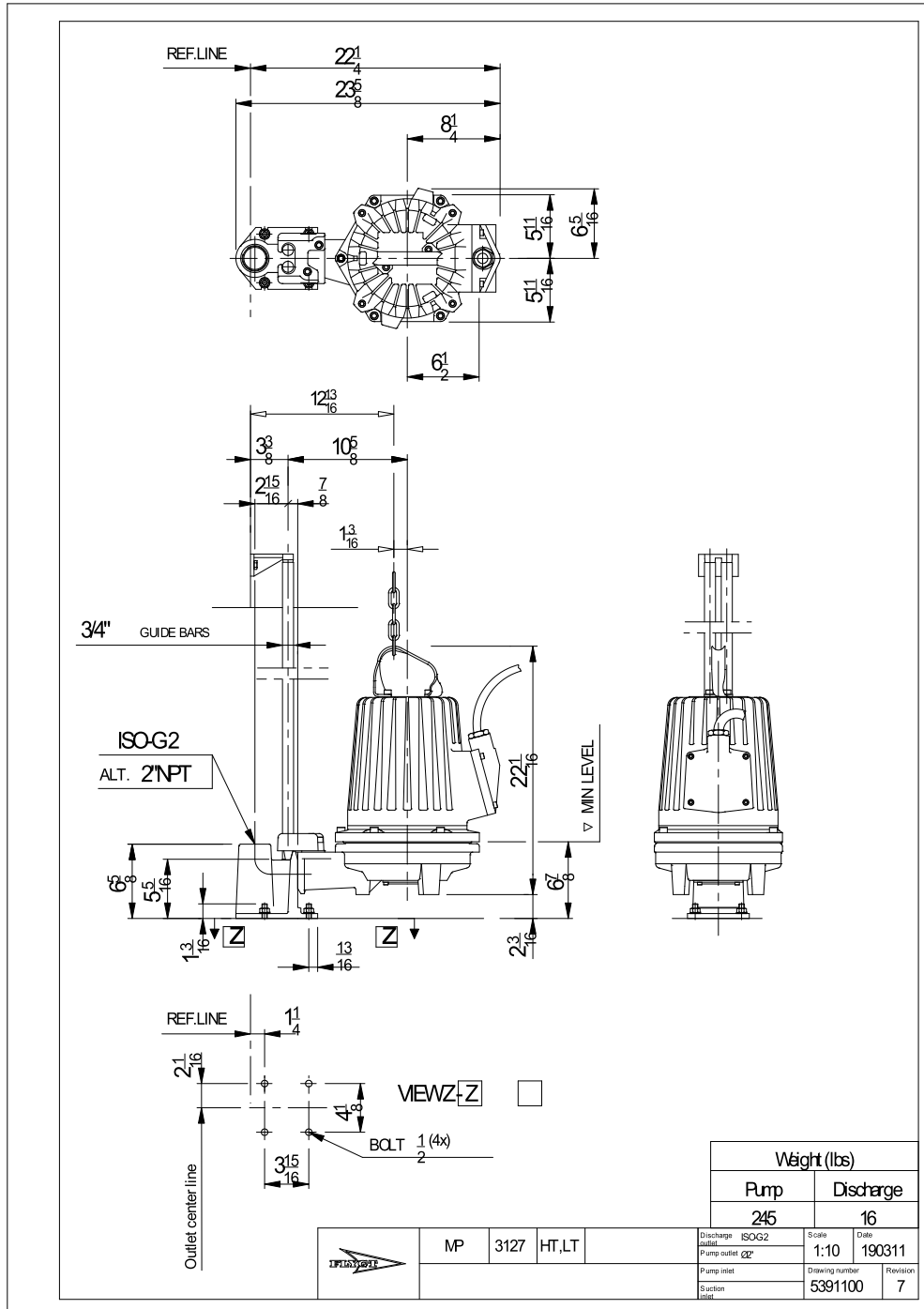
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Created on 3/4/2020

Last update

MP 3127 HT 3~ 262

Dimensional Drawing



Project
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Created by
Created on 3/4/2020

Last update

Appendix C – Lift Station Report

FINAL LIFT STATION DESIGN REPORT

HV91

Southeast of Happy Valley Road and Pima Road
Scottsdale, Arizona

Prepared for:

Parolo, LLC
7775 E. Fledgling Drive
Scottsdale, Arizona 85255

Prepared by:

Kimley-Horn and Associates
1001 West Southern Avenue, Suite 131
Mesa, Arizona 85210
291203000
January 2018

FINAL LIFT STATION DESIGN REPORT

HV91

SOUTHEAST OF HAPPY VALLEY ROAD AND PIMA ROAD
SCOTTSDALE, ARIZONA

JANUARY 2018



Prepared By:

Kimley»Horn

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Appendix B – Forcemain Calculation	
Appendix C – Odor Control	
Appendix D – OneLift Wet Well Alternative Product Information	

INTRODUCTION

SITE LOCATION

The purpose of this report is to provide preliminary design analysis for the construction of a new lift station serving HV91, a single-family development located southeast of Happy Valley Road and Pima Road in Scottsdale, Arizona (development). The purpose of this report is to provide for review and comment a preliminary design for the lift station layout, pumping alternatives, and associated pumping and force main options. The development is located within Section 7 of Township 4 North, Range 5 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. Refer to **Figure 1** for the Vicinity Map.

PROJECT SIZE AND TYPE

The development is a proposed 17-unit single family residential subdivision. The proposed buildings are one-story units. The development is approximately 20 acres.

PURPOSE AND OBJECTIVES

This report presents the basis of design criteria that will be used for engineering design of the proposed public lift station.

- Demonstrate compliance with the City's Design Standards & Policies Manual (DSPM) & Lift Station Design Criteria.
- Identify lift station site and preliminary site layout.
- Determine preliminary design for lift station equipment including: pump, wet-well, and forcemain.

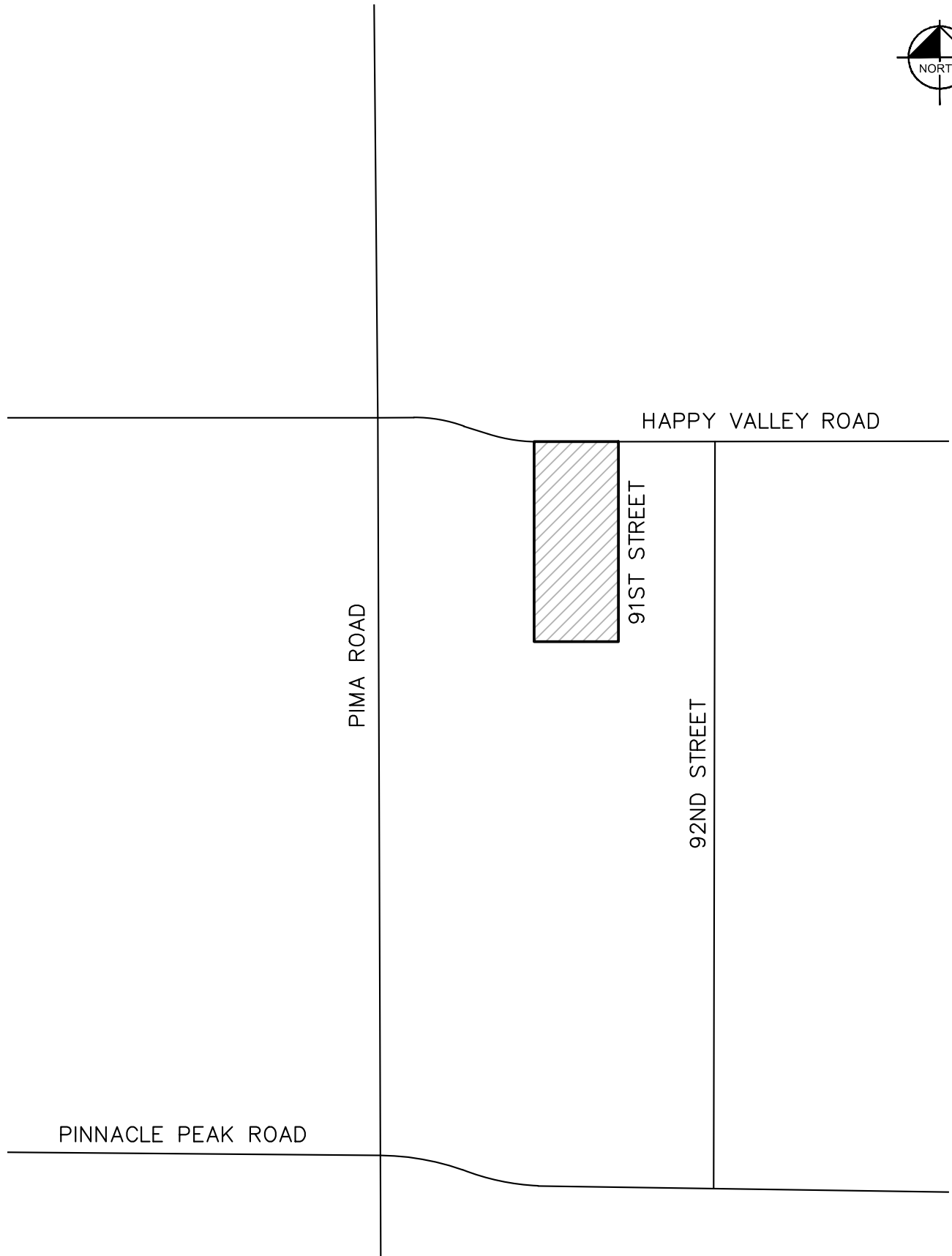


FIGURE 1
VICINITY MAP



DESIGN CRITERIA

COLLECTION SYSTEM

The Preliminary SewerBOD for HV91 establishes the design criteria for the gravity sewer collection system, per City of Scottsdale Design Standards and Policies Manual (DS&PM).

LIFT STATIONS

In selecting a site for the sewage lift station, considerations included accessibility, drainage patterns, visual impact, function and design constraints. The station's equipment must be protected from damage and remain operable during a 100-year flood plain. The proposed site is located outside the 100-year flood plain. Unless otherwise agreed to in writing by the City's rights-of-way agent, the tract or lot dedicated to the City will be conveyed by a general warranty deed and accompanied by a title policy in favor of the City, both to the satisfaction of the City.

Arizona Administrative Code, Title 18, Chapter 9, "Water Pollution Control," contains minimum requirements for a wastewater lift station. At a minimum, telemetry, dual pumps, backup power supply, three-phase power, provisions for future odor control, and perimeter walls will be required. The site will also be large enough to contain all the equipment and service equipment for repairs. Additionally, the lift station design will confirm to City of Scottsdale DS&PM and Lift Station Design Criteria.

A final design report prepared by a registered professional engineer, licensed in the State of Arizona, must accompany all pump station design drawings and specifications submitted to the City for review.

FORCEMAIN

City of Scottsdale staff has indicated that force mains smaller than 4 inches will require a parallel force main with interconnecting valves.

The flow velocity in the force main must be between 3 and 6 feet per second (fps).

All pipe material used in design of the force mains must have established ASTM, ANSI, AWWA and NSF standards of manufacture or seals of approval and shall be designated as pressure sanitary sewer pipe. Force mains must be identified as such with marking tape 1 foot above the pipe. All ductile iron force mains shall be lined.

Air release valves designed for sewage must be provided on force mains at all peaks.

Two-way cleanouts shall be provided every 1,300 feet apart or 1-way cleanouts every 650 feet. Single cleanouts must be provided at all horizontal bends oriented in line with the downstream pipe.

Where a force main crosses a water main or transmission line, protection must be provided as per ADEQ Engineering Bulletin No. 10 and the Arizona Administrative Code, Title 18, Chapter 9, "Water Pollution

Control.” At a minimum, the force main should be constructed of ductile iron pipe for a distance of 10 feet on each side of the water line.

See COS Standard Detail No. 2402 for details regarding discharge into a manhole from a force main.

The minimum separation between the force mains and water lines should be 2 feet wall-to-wall vertically and 6 feet horizontally under all conditions. Where a force main crosses above or less than 6 feet below a water line, the force main shall be encased in at least 6 inches of concrete for 10 feet on either side of the water line. Fittings should not fall within the encasement.

The engineer must evaluate the potential for odor to develop from a force main downstream of the receiving manhole. One-way valves on building service lines shall be specified where there is potential for gasses to strip from the waste stream. The valves should be located at or near the building

FINAL LIFT STATION DESIGN

SITE LAYOUT

The proposed lift station is located at the southwest end of the site, adjacent to the cul-de-sac of the private roadway tract. The lift station is located on a parcel of land approximately 0.16 acres in size. The land for the proposed lift station will be deeded to the City of Scottsdale as part of the final plat for the development. The site will consist of a six-foot diameter wet well, valve vault, meter vault, electrical control pad, transformer, concrete pad for a future chemical feed system, and a gas-powered generator. Refer to **Figure 2** for Final Lift Station Layout.

LIFT STATION DESIGN

Final pump design criteria has been developed for the proposed lift station. Pumps are required to convey the peak design flow rate at the total dynamic head calculated. In order to mitigate low design flow rates, the City has accepted the use of supplementing demands with potable water that can be entered into the gravity sewer system upstream of the proposed lift station. The proposed lift station will need to meet the following calculated pump requirements:

Lift station Design Summary					
Site	Gravity Collection Peak Flow (gpm)	Swimming pool Drainage	Total Lift Station Pumped Flow (gpm)	Static Head (ft)	Total Dynamic Head (ft)
		(gpm)			
Lift Station	17	35	47	35	101

*Assumes a Hazen-Williams C value of 130

A “xylem FLYGT model MP 3102 HT” pump has been preliminarily selected to convey the peak design flow. Refer to **Appendix A** for pump information.

Final wet well sizes have been calculated for the proposed lift stations. Wet well volume calculations are based on the following criteria and assumptions:

- Minimum flow to lift station = 0 gpm
- Minimum pump running time = 2 minutes
- Minimum pump cycle time = 10 minutes
- Wet well diameter = 5 feet

Based on the above criteria wet well volumes are as follows:

	Lift Station
Wet Well Volume (ft ³)	40
Operational Depth (ft)	0.81

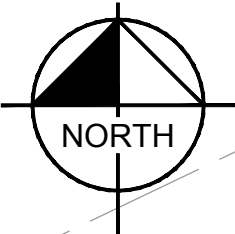
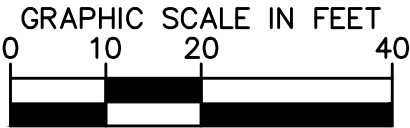
FORCEMAIN DESIGN

The proposed forcemain is designed to convey the pumped flow within the accepted velocity range per the City Design Criteria. A 2-inch PVC pipe was selected to convey the pumped flow. The proposed pumped flow of 47gpm is conveyed through a 2-inch force main line with a velocity of 4.8 ft/s. **See Appendix B – Forcemain Calculations.**

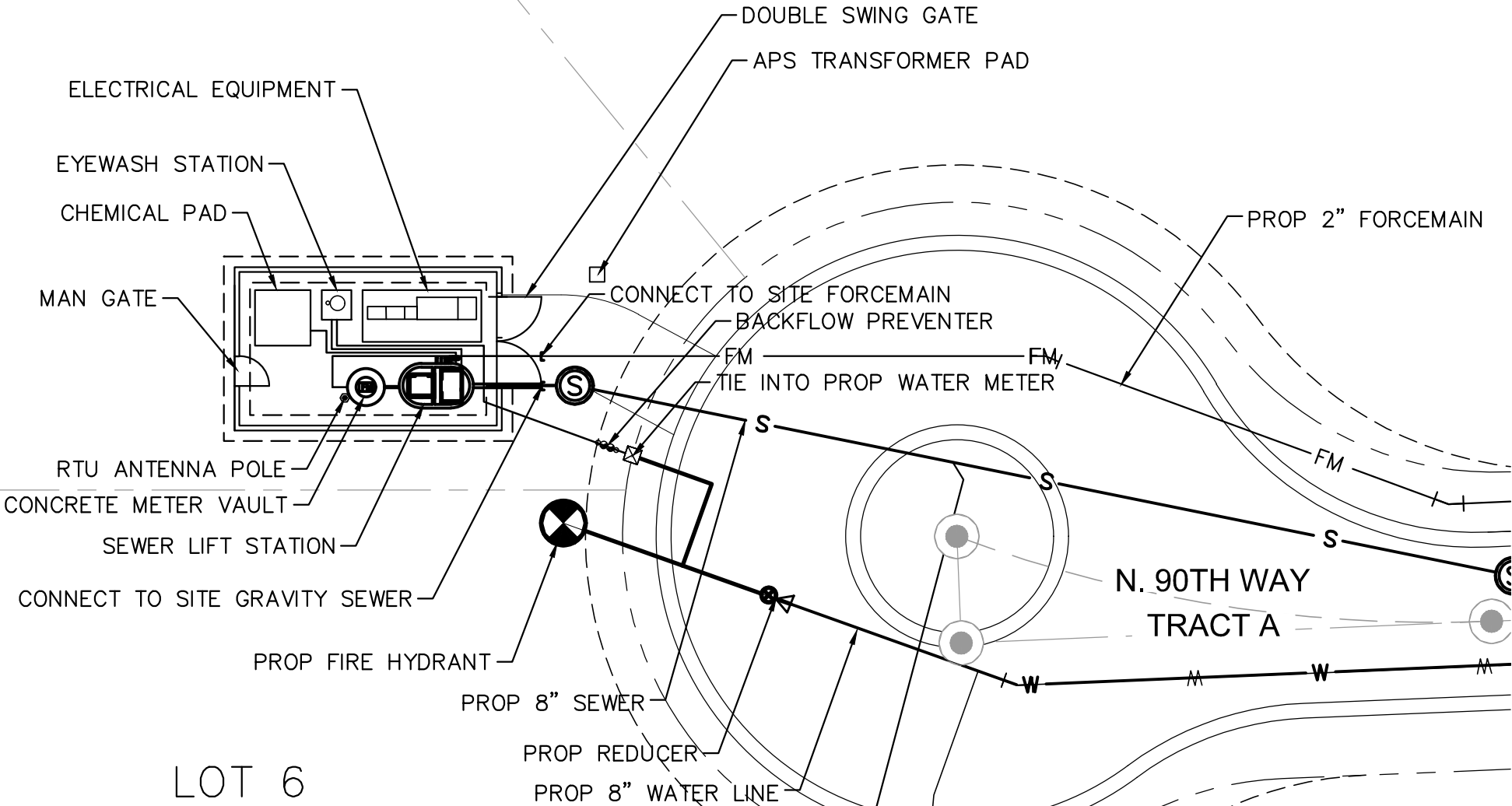
K:\EAV_GW\291203000 - HV91\Reports Lift Station Report\Final\Figures\HV91 Lift Station Layout_Final.dwg Jan 18, 2019 Isabel Garcia
XREFS: x030006M-FP x030000TP x030000VS x030000VS x030000VS x030000VS x030000VS x030000VS x030000VS x030000VS x030000VS x030000VS
IF ANY PART OF THIS DRAWING IS TO BE USED FOR ANY OTHER PROJECT, THE USER MUST OBTAIN WRITTEN AUTHORIZATION FROM KIMLEY-HORN AND ASSOCIATES, INC. NO PART OF THIS DRAWING SHALL BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, WITHOUT WRITTEN AUTHORIZATION FROM KIMLEY-HORN AND ASSOCIATES, INC.

PROJECT PROPERTY BOUNDARY

LOT 4



LOT 5



LOT 6

		NO.	REVISION	DATE
HV91		SCALE (H): 1"=20'	SCALE (V): NONE	
FINAL LIFT STATION REPORT		DESIGNED BY: ZJH	CHECKED BY: JMB	DATE: JAN 2019
LIFT STATION LAYOUT				
SCOTTSDALE, ARIZONA				
PROJECT NO. 291203000				
DRAWING NAME				

SITEWORK

SITE OPTIONS

The proposed development will consist of high end residential homes. The development is conscious of the potential negative aspects associated with a lift station (e.g. how the site looks, controlling odors, noise, etc.). This report presents options to reduce these hindrances and shield the properties from the proposed lift station. Options for the site development of the lift station are as follows:

- Develop a decorative wall high enough to shield immediate neighbors from both views of the lift station, as well as prevent excess noise. Wall articulation that matches the theme of the neighborhood would help maintain an attractive look to the neighborhood.
- Decorative sun shades could be utilized to shield the site from above. Shades could be constructed for both the individual pieces of equipment, as well as for the whole site.
- Landscaping could be utilized, including large trees and native vegetation, and/or well placed earth with retaining walls to give a more natural look and obstruct views of the station equipment.
- Depending on the topography of the area, parts of the lift station can be constructed into sunk areas with retaining walls, creating a larger difference between the top of the walls and the top of the equipment
- The developers could look at constructing a house or architectural building matching area homes.

SITE WALL

Per Section 7-1.205 of the *City of Scottsdale Design Standards & Policies Manual*, a perimeter wall will be required to be constructed around the site, but maintaining enough room inside of the site that all equipment and service equipment will be easily accessible for repair. As such, a 10-foot block wall is proposed around the site, with gate access located on the northwest corner of the site. As discussed in the previous section, there are several different options available to improve the aesthetics of the wall.

ODOR CONTROL

~~Per discussions with the City, odor control is not typically installed at new lift station sites.~~ However, not correct provisions for odor control chemical addition shall be provided at the lift station site in case the City deems it necessary to have installed. Installation will include a concrete pad for a future chemical storage tank, as well as electrical hook ups for future installation.

The City will require an odor control system at the force main outfall into the gravity system. Various options for outfall odor control include:

- Installing a sealed manhole at the sewer outfall with a 'blower' to send the air through a filter that absorbs the H₂S. See **Appendix C – Odor Control** for examples of a Hartzell Blower, as well as both a Vapex and Ecoair filter.

- Installing a chemical feed at the lift station to help treat the sewage for H₂S. The bioxide chemical used in this process is non-toxic, which means secondary containment on-site is unnecessary and chemical refilling procedures are greatly simplified. The chemical would be added before the sewage enters the force main, allowing the chemical to work as it works its way towards the outfall.

GENERATOR

The site power will be supplemented with a standby generator. Similar sites (using combined motor Hp under 100Hp) utilize generators in size from 60KVA to 150KVA. This site will utilize a 60KVA generator that is switched via ATS in an emergency condition.

CONTROLS

Per Scottsdale Sewer Lift Station Design Criteria (Revision 10/15/15) the lift station will include controls of the station pumps and control its overall operation. City design standards will dictate flow sensing, telemetry, alarm systems and safety precautions, and associated hardware to ensure reliable communication with existing radio systems. Overall functionality and sequence of lift station's operations will be confirmed with City personnel in cases of specific operations for this lift station.

SITE LIGHTING

Perimeter lighting will be installed per applicable City of Scottsdale standards. We will first design lighting in accord with lift station design requirements. In absence of specific lighting requirements for lift stations, IES (Illuminating Engineering Society) suggestions will be supplemented. Site lighting will be placed in locations that maintenance personnel agree with, and will be controlled using a hierarchy that is dictated by site management. We will conduct a basic calculation (AGI32 or approved software) to determine light levels and provide verification of fixture number and positioning.

PUMP ENCLOSURE

Various options exist for the wet well on the Lift Station development site, which will vary based on the required volume and maximum depth of the wet well. As stated in Section - Lift Station Design, the size of the wet well will be 6' diameter with approximately a 3' operational depth. Additional vendor information regarding the layout of the wet well and associated piping can be found in **Appendix D**.

An additional option for the wet well construction would be a OneLift wet well, which is pre-constructed to include pumps, valve box, and water meter all in the construction of the wet well itself. This particular wet well would have a smaller foot print, which would assist in minimizing the size of the overall lift station site. Example plans of the OneLift wet well can be found in **Appendix E**.

Job Number	291203000
Job Description	HV91
Date	1/18/2019
Designed By	ZJH
Checked By	REL
Pump Type	MP3102 HT3-267

HAZEN-WILLIAMS EQUATION FOR TDH CALCULATIONS

System Elevations:

Pumps Off Elevation	2070.00	ft
Flow Line of Force Main at High Point	2105.00	ft
Calculated Static Head	$\Delta Z = 35.00$	ft

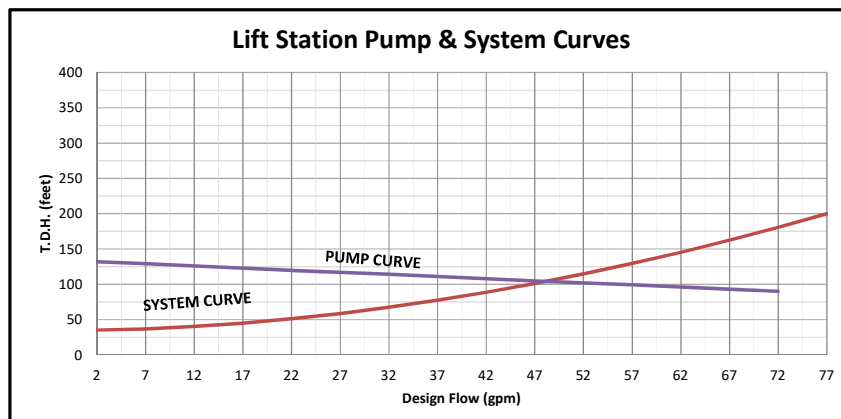
Hazen-Williams Parameters:

Dia. of Discharge Piping	D = 2	inches
Length of Discharge Piping	L = 10	ft
Dia. of Force Main	D = 2.000	inches
Length of Force Main	L = 1200	ft
Hazen Williams Coefficients	C = 130	
Minor Losses - Sum of Coefficients for Discharge Piping	$K_D = 1$	taken from Minor Losses tab
Minor Losses - Sum of Coefficients for Force Main	$K_{FM} = 6.87$	taken from Minor Losses tab

Flow Interval for Tables **5** gpm

C = 130 for Discharge Piping, C = 130 for Proposed Force Main

Flow (GPM)	Friction Head (Discharge Piping) (ft.)	Minor Losses (Discharge Piping) (ft.)	Friction Head (Force Main) (ft.)	Minor Losses (Force Main) (ft.)	T.D.H. (ft.)	Pressure (PSI)	Force Main Flow Velocity (fps)	Pump Curve (ft.)
2	0.00	0.00	0.19	0.00	35.19	15.23	0.20	135
7	0.02	0.01	1.93	0.05	36.95	16.00	0.71	132
12	0.04	0.02	5.22	0.16	40.28	17.44	1.23	129
17	0.08	0.05	9.94	0.32	45.07	19.51	1.74	126
22	0.13	0.08	16.01	0.54	51.23	22.18	2.25	123
27	0.19	0.12	23.39	0.81	58.70	25.41	2.76	120
32	0.27	0.17	32.03	1.14	67.46	29.20	3.27	117
37	0.35	0.22	41.90	1.52	77.47	33.54	3.78	114
42	0.44	0.29	52.97	1.96	88.70	38.40	4.29	111
47	0.54	0.36	65.22	2.46	101.12	43.78	4.80	108
52	0.66	0.44	78.64	3.01	114.73	49.67	5.31	105
57	0.78	0.53	93.19	3.62	129.50	56.06	5.82	102
62	0.91	0.62	108.88	4.28	145.41	62.95	6.33	99
67	1.05	0.73	125.68	5.00	162.45	70.33	6.84	96
72	1.20	0.84	143.58	5.77	180.61	78.19	7.35	93
77	1.35	0.96	162.57	6.60	199.88	86.53	7.86	90



Approximate Capacity at C = 130: **47 gpm**

Worksheet for Force main

Project Description

Friction Method	Manning Formula
Solve For	Pressure at 1

Input Data

Pressure 2	0.00	psi
Elevation 1	2070.00	ft
Elevation 2	2105.00	ft
Length	1200.00	ft
Roughness Coefficient	0.010	
Diameter	2.00	in
Discharge	47.00	gpm

Results

Pressure 1	52.75	psi
Headloss	86.68	ft
Energy Grade 1	2192.04	ft
Energy Grade 2	2105.36	ft
Hydraulic Grade 1	2191.68	ft
Hydraulic Grade 2	2105.00	ft
Flow Area	0.02	ft ²
Wetted Perimeter	0.52	ft
Velocity	4.80	ft/s
Velocity Head	0.36	ft
Friction Slope	0.07223	ft/ft

Fiberglass Backward Curved Centrifugal Fans

Type FA

Series 41

Series 41P



HARTZELL®

Hartzell Fan, Inc., Piqua, Ohio 45356
www.hartzellfan.com

5-PP-2020
Bulletin A-160
9/28/2020

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Certified Ratings for Air and Sound

Hartzell Fan, Inc. certifies that the Series 41, Fiberglass Backward Curved Centrifugal Fans, Type FA shown on pages 7–11 and 14–21, and Series 41P, Fiberglass Backward Curved Centrifugal Fan, Packaged, shown on pages 12–20, are licensed to bear the AMCA Seal for Air and Sound Performance. The ratings shown are based on tests and procedures performed in accordance with AMCA Standard 211 and AMCA Standard 311 and comply with the requirements of the AMCA Certified Ratings Program.

Sound Performance data is available upon request. Please contact the factory and ask for Engineering Publication #SD-160.

Hartzell Model Code Explanation

Hartzell Model Code

Type_____	A	4	1	–	9	–	3	3	1	FA	100	FG	OP	M	3
A – Production Item															
S – Stock Item															
Q – Special Quote															
Product Series_____															
Arrangement (centrifugals only)_____															
Size (nominal wheel diameter, inches)_____															
Class_____															
Wheel Code_____															
Wheel Width (entries represent percents)_____															
Material of Construction_____															
Motor Enclosure_____															
Motor Horsepower_____															
Motor RPM/Phase_____															

Motor RPM/Phase Code	
3 Phase	1 Phase
2 = 3450	B = 3450
3 = 1750	C = 1750
4 = 1140	D = 1140
5 = 870	E = 870
6 = 690	F = 690
7 = 575	G = 575

Motor Horsepower

Horsepower	1/4	1/3	1/2	3/4	1	1 1/2	2	3	5	7 1/2	10	15	20	25	30	40	50	60	75	100	125	150	200
Code Letter	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T	U	V	W	X	Y	Z

Example:

Assume a needed performance of 12,000 CFM at 5" SP, standard air. Reading the 33" rating table for 100% width on page 17, we find a fan RPM of 1,168 and brake horsepower (BHP) of 12.3. Required motor horsepower is 15. The model code can be constructed as follows: Type will be a production item (code A), product series for the Fiberglass Backward Curved Fans is 41, arrangement is 9 (code 9), size of the wheel is 33", class of construction is I (code 1), wheel code for this item

is FA, wheel width is 100% (code 100), material of construction is fiberglass (code FG), motor enclosure is open protected drip-proof (code OP), motor horsepower is 15 (code O), and motor RPM/phase is 1750 (code 3).

Note: All other informational fields must be filled with hyphens/dashes (-) if they are not applicable to the fan being considered.

This bulletin lists Hartzell's line of Fiberglass Backward Curved Centrifugal Fans, Type FA and accessories. More than 70 Hartzell offices can provide specific performance and installation data to meet your requirements. Call your Hartzell representative for assistance. Visit our website (www.hartzellfan.com) or call toll-free (1-800-336-3267) for the name of your Hartzell representative.



General Fiberglass Construction Features

A variety of corrosion problems plague industry today. Fans and blowers made of coated steel or metals such as stainless and monel can handle some problem areas. Please refer to the corrosion resistance table on page 5 of this bulletin. Fiberglass centrifugal blowers can be used in most applications where corrosive elements exist in fume and vapor form. The resistance to corrosive elements is a major advantage, but the physical properties of fiberglass equipment offer these additional advantages:

- Fiberglass equipment is corrosion resistant.
- Fiberglass equipment weighs 25% less than comparable equipment made of carbon steel.
- Fiberglass has an extremely high strength-to-weight ratio, stronger than steel on a per-pound basis.
- Dimensional stability of fiberglass is excellent.
- Fiberglass air moving equipment will not become brittle at low temperatures and at 0°F the laminated fiberglass will be stronger than at room temperature.

Hartzell Fan, Inc. conforms to ASTM D4167-97, Standard Specification for fiber-reinforced plastic fans and blowers, when optional surfacing veil, electrical grounding, and dynamic balancing to ASTM D4167-97 levels, are added to the fan.

The following are standard Hartzell fiberglass construction features:

- Corrosion resistant polyester resin, having a Class I flame spread rate of 25 or less is used for all housings. Vinylester resin having a Class I flame spread rate of 25 or less is used for all wheels.
- All structural parts in the airstream are fiberglass and resin. All fiberglass surfaces are protected with a minimum 10-mil thickness of chemical, flame, and ultraviolet resistant resin.
- Shafts are turned, ground, polished, and keyed at both ends with a fiberglass sleeve in the airstream. Shafts are sized to operate well below critical speed. 304 or 316 Stainless steel or monel shafting is available as an option at extra cost.
- Internal hardware (airstream) is Type 304 stainless steel. All internal hardware (airstream) is encapsulated. All external hardware (out of airstream) is zinc plated as standard. Where metal is subject to attack by the corrosive elements being handled, all metal parts can be resin-coated after assembly.
- A fiberglass and neoprene shaft seal is placed where the shaft leaves the housing along with a neoprene shaft slinger between the seal and wheel on belt drive units (seal is not gas tight).



Series 41P



Series 41

Fiberglass Centrifugal Fans

- Bearings on belt drive units are heavy duty, deep row radial ball or double row spherical roller type self-aligning and shielded in cast iron housings. Long inner races ensure even load distribution, providing a high radial and thrust load capacity. Bearings are relubricable for continuous service with lubrication tubes extended to the exterior of fan base as necessary.
- V-Belt Drives are oversized for long life and continuous duty as standard. Fixed pitch or variable pitch drives are available upon request. Belts are oil, heat, and static resistant type.

Type FA Wheel Features

The Type FA wheel is unique in the fan and blower industry. It is available in diameters from 12" to 60" in both clockwise and counter-clockwise rotations. The wheel is airfoil design and solid fiberglass die formed and coated with Dow Derakane 510-A corrosion resistant vinylester resin. The manufactured wheel is a single piece, removed from the pattern whole. This ensures each wheel is aerodynamically identical and provides reliable repeatable performance without the variability of hand made and taped components. The design is the result of a substantial investment in research, development, tooling, and manufacturing methods by Hartzell Fan, Inc.

The type FA wheel is highly efficient, with tapered inlet side and airfoil blades. It has non-overloading horsepower characteristic curve. When used in conjunction with a precision inlet cone it **efficiently moves large volumes of air at high pressures with low noise characteristics at low RPM.**

The fiberglass resin has a Class I flame spread rate of 25 or less. The wheel is electronically statically and dynamically balanced to the requirements of Fan Application Category BV-3 of AMCA ANSI Std. 204-96 and receives an Operational Test and Inspection before shipment. Special constructions are available for abrasive environments or extremely corrosive environments.



Type FA Wheel



Hartzell Selection Guide

The Hartzell Fiberglass Backward Curved Centrifugal Fan performances on the following pages are based on standard air conditions (sea level, 70°F, and 29.92 inches barometric pressure). Performance data does not include drive losses on belt drive units.

How to use Performance Tables

1. Select a model for a given air delivery and pressure by looking up the required flow vertically along the left column of the performance table and moving to the required pressure. The model is identified with each table.

2. Note the required RPM and BHP. Refer to page 2 Hartzell Model Code Explanation for additional details.

3. If non-standard temperature or altitude is involved, correct to standard air density (see Temperature/Altitude Applications).

When placing your order, be sure to specify the Hartzell Model Code. Be sure to include fan model, performance requirements, operating temperature, motor data (enclosure, voltage, mounting position, etc.), and a list of required accessory items. (See pages 22 and 23.) For selection assistance and additional data contact your local Hartzell Sales Representative for assistance.

Temperature/Altitude Applications

When a fan operates in ambient conditions, generally it is handling standard air at 70°F, 29.92" barometric pressure, weighing 0.075-lbs./cu. ft. For an application where the fan operates at other than ambient conditions (temperature, altitude, or both), correction factors must be applied to the selection of the fan. In addition, the standard construction of the fan must be modified.

Correction factors for temperatures and altitudes are provided in Table 1. When a fan operates at other than ambient conditions,

the correction factors in Table 1 will be required to correct static pressure and horsepower.

Table 2 shows the maximum safe operating speeds for each size fan wheel. At high temperatures, these maximum safe operating speeds should be derated.

Table 3 provides maximum safe speed correction factors by temperature and material construction. An example on the use of these tables appears at the bottom of this page.

Table 1 Altitude/Temperature Correction Factors

Temp.* (°F)	-50	-25	0	25	50	70	100	125	150	175	200	250
Factor	0.77	0.82	0.87	0.91	0.96	1.00	1.06	1.10	1.15	1.20	1.25	1.34

Alt.** (Ft.)	0	1,000	2,000	3,000	4,000	5,000	6,000	7,000	8,000	9,000	10,000
Factor	1.00	1.04	1.08	1.12	1.16	1.20	1.25	1.30	1.35	1.40	1.46

Above table has inverted values. Actual density is the reciprocal of the above values.

*At sea level. **At 70°F.

For corrections involving both temperature and altitude, correction factors should be multiplied.

Example: 150°F at 7000 ft.: Temperature factor 1.15 x altitude factor 1.30 = 1.50 combined correction factor.

Table 2 Maximum Safe Speeds @70°F

Fan Size	100% Width	66% Width
12	4,520	5,320
15	3,600	4,340
18	2,990	3,610
22	2,440	2,950
24	2,240	2,710
27	2,000	2,410
30	1,840	2,220
33	1,670	2,020
36	1,530	1,850
40	1,370	1,660
44	1,240	1,500
49	1,130	1,360
54	1,020	1,230
60	920	1,110

Use of Correction Factors and Tables

First select size, RPM and BHP of the blower needed.

If temperature or altitude is involved, correct to standard air.

Example: Assume the required performance to be 12,000 CFM at 4.62" SP, 175°F and 2000 feet altitude.

1. Temperature factor 1.20 x altitude factor 1.08 = 1.30 combined correction factor.
2. Correct SP to standard 4.62" SP x 1.30 = 6" SP for 70°F at sea level.
3. A Series 41, size 33" class II 66% width belt drive backward curved centrifugal, selected from the rating tables (page 17) for the new condition shows 12,000 CFM at 6" SP, 1,398 RPM and 15.5 BHP.
4. Correct the horsepower and static pressure in item 3 to non-standard performance by dividing by factor: 6" SP divided by 1.30 = 4.62" SP; 15.5 BHP divided by 1.30 = 11.9 BHP.
5. Check the maximum safe speed. Maximum speed at 70°F for fan size 33" 66% width, 2,020 RPM. Using the maximum safe speed factor table for fiberglass construction yields a safe speed factor of .95. The maximum safe speed is 2,020 x .95 = 1,919 RPM; thus operation at 1,438 RPM at 175°F is satisfactory.
6. Final performance of the unit at the assumed conditions: 12,000 CFM at 4.62" SP, 1,398 RPM, 11.9 BHP at 175°F and 2000 feet altitude.
7. Size motor for cold startups and use a special high altitude motor if altitude exceeds 3300 feet.

Table 3 Maximum Safe Speed Correction Factors*

Temp. (°F)	0	70	100	150	175	200	225	250
FRP	1.00	1.00	1.00	0.98	0.95	0.91	0.82	0.70

* To correct maximum safe operating speeds (Table 2) for high temperatures, multiply those speeds by correction factors from Table 3.



Corrosion Resistance Guide

Temperature values shown are for immersion or condensate contact applications. Where temperature values are shown, resin is suitable for hood and duct type applications for the full operating temperature range of the product. See product specifications for materials of construction and maximum operating temperature limits.

Environment	Hetron 693 Ashland F.	6694 Reichold F.	510A Dow F.	Environment	Hetron 693 Ashland F.	6694 Reichold F.	510A Dow F.	Environment	Hetron 693 Ashland F.	6694 Reichold F.	510A Dow F.
ACIDS				ALKALIES (Synthetic Veil)				SALTS (cont'd.)			
Acetic to 10%	180	200	210	Ammonium Bicarbonate to 50%	140	\$170	160	Sodium Ferricyanide	220	220	210
Acetic to 50%	90	160	180	Ammonium Carbonate	120	\$140	150	Sodium Fluoride	—	\$180	\$180
Acetic to 100%	—	NR	NR	Ammonium Hydroxide to 5%	\$90	\$180	\$180	Sodium Nitrate	220	220	210
Acrylic to 25%	—	100	100	Ammonium Hydroxide to 10%	\$90	\$170	\$150	Sodium Nitrite	—	220	NR
Benzene Sulfonic to 25%	180	210	150	Ammonium Hydroxide to 29%	NR	\$100	\$100	Sodium Silicate PH less than 1	160	210	NR
Benzene Sulfonic 25% up	90	210	NR	Barium Carbonate	180	\$240	210	Sodium Sulfate	180	240	210
Benzoic	250	220	210	Barium Hydroxide to 10%	—	\$170	150	Sodium Sulfite	—	220	210
Boric	180	220	210	Calcium Hydroxide to 15%	160	\$210	\$180	Stannic Chloride	*180	*220	*210
Butyric to 50%	150	150	210	Magnesium Carbonate	160	\$210	180	Stannous Chloride	*200	*220	*210
Butyric 50% up	—	100	80	Potassium Bicarbonate to 10%	90	\$170	\$150	Zinc Chloride	200	*220	*210
Carbonic	160	220	NR	Potassium Carbonate to 10%	90	\$180	\$150	Zinc Nitrate	180	220	210
Chloroacetic to 25%	NR	*180	*150	Potassium Hydroxide to 25%	NR	\$120	\$150	Zinc Sulfite	150	220	NR
Chloroacetic 25% to 50%	NR	*150	*120	Sodium Bicarbonate to 10%	140	\$210	\$180				
Chromic to 5%	100	110	150	Sodium Carbonate to 35%	90	\$180	\$180	SOLVENTS			
Chromic to 10% to 20%	—	NR	150	Sodium Hydroxide to 10%	NR	\$160	\$180	Acetone to 10%	NR	180	180
Citic	*200	*220	*210	Sodium Hydroxide to 25%	NR	\$160	\$180	Benzene	90	80	NR
Fluoboric	*\$90	*\$220	*\$210	Sodium Sulfide	90	\$220	\$210	Carbon Disulfide	NR	NR	NR
Glucosilicic up to 10%	\$100	\$150	\$180	Trisodium Phosphate to 50%	—	\$175	210	Carbon Tetrachloride	90 VAPOR	110	150
Formic up to 10%	200	150	180					Chlorobenzene	NR	NR	NR
Gluconic to 50%	120	180	180	SALTS				Ethyl Acetate	NR	NR	NR
Hydrobromic to 25%	*160	*170	*180	Aluminum Chloride	*120	*240	*210	Ethyl Chloride	90 VAPOR	NR	NR
Hydrochloric to 15%	*230	*210	*180	Aluminum Potassium Sulfate	160	240	210	Ethylene Dibromide	NR	NR	NR
Hydrocyanic to 10%	200	170	210	Aluminum Sulfate	250	240	210	Ethylene Glycol	250	220	210
Hydrofluoric to 10%	***\$100	***\$150	***\$150	Ammonium Chloride	*200	*220	*210	n-Heptane	120	210	210
Hydrofluorsilicic up to 10%	*\$100	*\$150	*\$180	Ammonium Nitrate	200	220	220	Hexane	—	150	160
Hypochlorous to 20%	90	110	NR	Ammonium Persulfate	150	200	180	Methyl Ethyl Ketone to 10%	NR	80	NR
Lactic	*200	*220	*210	Ammonium Persulfate, saturate	150	NR	NR	Naphtha	200	210	180
Maleic	170	210	210	Ammonium Sulfate	200	220	220	Naphthalene	130	220	210
Nitric to 5%	200	170	150	Aniline Sulfate to 25%	150	220	210	Tetrachloroethylene	NR	100	80
Nitric 5% to 20%	—	140	120	Aniline Sulfate, saturated	150	220	NR	Toluene	90	NR	80
Oleic	200	220	210	Barium Chloride	200	240	210	Xylene	90	80	80
Oxalic	*220	*220	*210	Barium Sulfide	NR	\$210	180	BLEACHES			
Perchloric to 10%	H&D	**150	**150	Calcium Chlorate	180	220	220	Calcium Chlorate	180	220	220
Phosphoric	*220	*\$210	*\$210	Calcium Chloride	250	240	220	Calcium Hypochlorite	100	NR	\$160
Phosphoric, super	—	*\$210	*\$210	Calcium Sulfate	*200	*240	*210	Chlorine Dioxide up to 15%	—	160	*200
Phthalic Anhydride	*150	*210	*210	Copper Chloride	*250	*220	*220	Chlorine Water	*125	*210	*200
Picric to 10%	100	170	NR	Copper Cyanide	90	\$220	210	Hydrogen Peroxide to 30%	120	100	150
Silicic	—	220	NR	Copper Fluoride	NR	\$170	NR	Sodium Chlorate	90	210	210
Stearic	200	220	210	Copper Sulfate	250	240	210	Sodium Hypochlorite to 15%	NR	125	\$180
Sulfamic to 25%	160	150	NR	Ferric Chloride	*250	*220	*210				
Sulfuric to 25%	*200	*220	*210	Ferric Nitrate	170	220	210	OTHERS			
Sulfuric to 50%	*200	*200	*180	Ferric Sulfate	200	220	210	Alum. Chlorohydroxide to 50%	—	220	210
Sulfuric to 70%	*150	*180	*100	Ferrous Chloride	*220	*220	*210	Ammonium Phosphate	150	210	210
Sulfuric to 80%	NR	80	NR	Ferrous Nitrate	160	220	210	Aqua Rega	NR	*80	NR
Sulfurous to 10%	90	110	120	Ferrous Sulfate	220	220	210	Detergents	120	170	150
Tannic	200	220	210	Lead Acetate	160	220	210	Glycerine	200	220	210
Tartaric	220	220	210	Magnesium Chloride	220	240	210	Kerosene	120	210	180
Trichloroacetic to 50%	*90	*220	*200	Magnesium Hydroxide	—	\$210	210	Photographic Solutions	—	80	NR
				Magnesium Sulfate	200	210	210	Perchloroethylene	NR	100	80
ALCOHOLS				Mercuric Chloride	*210	*220	*210	Sodium Tetraborate	180	\$210	180
Amyl	200	210	120	Mercurous Chloride	210	220	210	Sodium Tripolyphosphate	125	210	210
Benzyl	NR	100	NR	Nickel Chloride	220	220	210	Sodium Xylene Sulfonate	—	170	160
Butyl	190	150	120	Nickel Nitrate	220	220	210	Sorbitol Solutions	180	220	160
Ethyl	90	120	80	Nickel Sulfate	220	220	210	Urea	90	170	150
Methyl	90	80	NR	Potassium Chloride	200	240	210	Urea-Ammonium-Nitrate	—	120	120
				Potassium Dichromate	200	220	210	Fertilizer Fumes	100	120	150
GASES AND VAPORS				Potassium Ferricyanide	200	220	210	Shell-D-D	NR	100	NR
Ammonia, Dry	90	170	100	Potassium Nitrate	200	220	210	Steam Vapor	180	210	180
Ammonia, Wet	90	NR	NR	Potassium Permanganate	150	210	210				
Bromine, Wet	90	*100	NR	Potassium Persulfate	90	220	210				
Carbon Dioxide	250	250	250	Potassium Sulfate	200	240	210				
Carbon Monoxide	200	250	250	Silver Nitrate	200	220	210				
Chlorine, Dry	*200	*210	NR	Sodium Acetate	150	220	210				
Fluorine	—	NR	80	Sodium Bisulfate	200	220	210				
Hydrogen Fluoride, Vapor	*90	*\$180	*\$180	Sodium Chloride	200	240	180				
Hydrogen Sulfide to 5%	250	240	180	Sodium Chlorite to 10%	175	170	150				
Sulfur Dioxide, Dry	200	250	210	Sodium Cyanide	100	220	210				
Sulfur Dioxide, Wet	200	250	210	Sodium Dichromate	160	220	210				
Sulfur Trioxide, Wet	—	220	210								

Reference
C.R.G.1.1

NOTES: NR = Not Recommended S = Synthetic surfacing veil or mat required. Contact factory. "—" = No test data available

- * Special shaft and hardware required, contact factory.
 - ** Special design considerations required (explosive environment), contact factory.
 - *** Do not use HartKoate. Special shaft and hardware required, contact factory.
- For environments not shown, or when temperatures exceed the maximum listed, contact factory.
Hydrocarbon fuel environments may require static grounding, contact factory.
- Do not use HartKoate (Alum. Oxide) with Hydrofluoric acid.



Series 41 Backward Curved Centrifugal Fan, Type FA

Series 41 Hartzell Fiberglass Backward Curved Centrifugal Fans offers **non-overloading, high efficiency, low noise**, and economy for corrosive atmospheres. This fan is unique in the fan and blower industry. The fan incorporates the proven, highly efficient, backward curved, airfoil-bladed, solid fiberglass, Type FA wheel in a solid fiberglass housing. This design incorporates the airfoil centrifugal wheel, centrifugal fan housing, and inlet cone to produce a compact, highly efficient unit with low noise characteristics.



Series 41



Hartzell Fan, Inc. certifies that the Series 41, Fiberglass Backward Curved Centrifugal Fans, Type FA, shown herein are licensed to bear the AMCA seal for air and sound performance. The ratings shown are based on tests and procedures performed in accordance with AMCA Publication 211 and Publication 311 and comply with the requirements of the AMCA Certified Ratings Program.

Sound Performance data is available upon request. Please contact the factory and ask for Engineering Publication #SD-160.



Type FA Wheel

- **Applications** – Developed for compatible corrosive applications where it is advantageous to have fiberglass materials and have the motor out of the airstream with the versatility of a belt drive fan.
- **Performance** – Type FA fiberglass airfoil wheel with inlet cone and aerodynamically designed housing produces from **800 CFM to 90,000 CFM at pressures from free delivery to 14" W.G.** at high efficiencies with non-overloading horsepower, low noise, and low RPM. Maximum temperature capability is 250°F.

Features




- **Sizes** – 12", 15", 18", 22", 24", 27", 30", 33", 36", 40", 44", 49", 54", and 60" wheel diameters. Available in Class I and II in 100% width and Class I, II, and III in 66% width. Available in Belt Drive Arrangements #1, #9, and #10, Direct Drive Arr. #4 and Direct Coupled Arr. #8. Contact Factory for Arr. #8 dimensions and for other arrangements.
- **FRP Materials** – Solid fiberglass wheel molded with Dow Derakane 510-A corrosion resistant vinylester resin having a Class I flame spread rate of 25 or less. The housing and other standard FRP components are constructed of fiberglass and Ashland Hertron 693 corrosive resistant polyester resin having a Class I flame spread rate of 25 or less. No metal parts are exposed in the airstream. See Corrosion Resistance Guide on page 5 for resin characteristics. Other resins are available.
- **Type FA Wheel** – **High efficiency, airfoil** design with **one-piece, solid fiberglass**, construction. Tapered inlet side design efficiently moves large volumes of air at high pressures. Wheel has non-overloading horsepower characteristic curve.
- **Rotation and Discharge Positions** – Available in both clockwise and counter-clockwise rotations and in all standard discharge positions. Housing discharge position can be changed on fan sizes 12" through 36". Larger size housings are non-rotatable.
- **Easy Installation and Maintenance** – Motor, drives, and bearings are readily accessible for ease in wiring, installation, adjustment, and lubrication.
- **Shafts** – Shafts are turned ground and polished, keyed at both ends with fiberglass sleeve in the airstream and sized to operate well below critical speed.
- **Bearings** – Bearings are heavy duty, self-aligning, ball or roller type, in cast iron pillow block housings, selected for minimum L-50 Life of 250,000 hours, and include extended lubrication fittings as standard.
- **Standard Shaft Seal** – A fiberglass and neoprene shaft seal is placed where the shaft leaves the housing along with a neoprene shaft slinger between the seal and wheel. Seal is not gas tight.
- **Hardware** – Airstream hardware is Type 304 stainless steel and encapsulated.
- **Motor Out of the Airstream** – Exterior mounting of Drip-Proof Protected motor on an adjustable motor slide base in belt drive models is standard. Motors can be furnished as TEFC, Mill and Chemical Duty, or to specifications upon request. Motor HP and frame size limits are identified in Dimensions and Material Specifications table.
- **Drives (Belt Drive Fans)** – V-Belt Drives are oversized for long life and continuous duty and are fixed pitch as standard option. Variable pitch drives are available upon request. Belts are oil, heat, and static resistant type.
- **Balancing** – The fan is electronically statically and dynamically balanced to the requirements of Fan Application Category BV-3 of AMCA ANSI Std. 204-96. All fans receive an inspection prior to shipment and, whenever possible, an operational test.
- **Flanged Duct Connections** – Outlet flange is standard, inlet flange is optional. Flange bolt holes are optional.
- **Bases** – Heavy gauge, welded, hot rolled steel with epoxy coating are standard.
- **Options and Accessories** – See pages 22 and 23.
- **Spark Resistant Construction and Protective Coatings** – Spark resistant construction for fiberglass equipment is optional, and for abrasive environments or extremely corrosive environments, special construction is available, see page 23.



Hartzell Centrifugal Fan Classifications

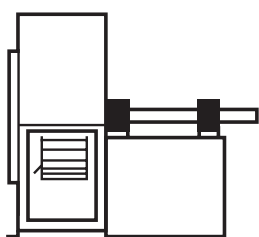
Hartzell Series 41 Fiberglass Backward Curved Centrifugal Fans, Type FA, 100% width, are designed and classified to perform within the centrifugal fan classification parameters established by AMCA Standard No. 2408; AMCA Publication 99. Hartzell Series 41 Fiberglass Backward Curved Centrifugal Fans, Type FA, 100%

width are available in Class I and II construction. Hartzell Series 41 in 66% width are available in Class I, II, and III construction. Series 41P are available in Class I construction only. See performance tables for specific ratings. These parameters are explained in the following table.

FAN CLASS	PERFORMANCE RANGE*	TABLE SHADING
I	5" @ 2300 FPM To 2½" @ 3200 FPM	
II	8½" @ 3000 FPM To 4¼" @ 4175 FPM	
III	13½" @ 3780 FPM To 6¾" @ 5260 FPM	

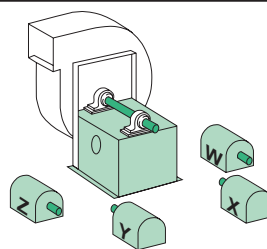
* At standard air conditions (70°F, 29.92 in. HG barometric pressure, .075 lbs./ft.3). Static pressure shown in inches of water; outlet velocity shown in feet per minute. Performance Ranges apply only to 100% width construction.

Centrifugal Fan Arrangements



Arrangement 1

Unit furnished with shaft and bearings, less motor and drive. Designed to be driven by a separately mounted motor. Impeller is overhung – ~~two bearings on base~~. Temperature limitations: 250°F.

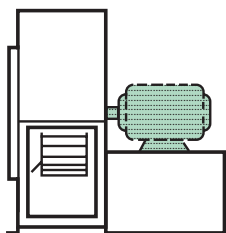


Motor Position Designation

Motor position designation is necessary when ordering the following for Arrangement 1 fans –

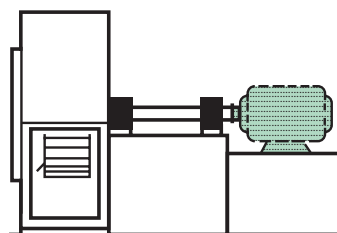
- 1 – V Belt Drive.
- 2 – Vibration Bases.
- 3 – Belt Guards.

Note: Location of motor is determined by facing the drive side of the fan and designating the motor position by letters W, X, Y, or Z. Consider discharge location and height when specifying.



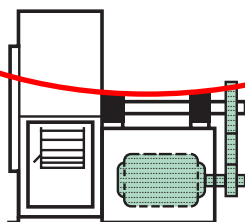
Arrangement 4

Direct drive packaged unit, wheel is overhung and attached to the shaft of the electric motor. No bearings on fan. Temperature limitations: 200°F.



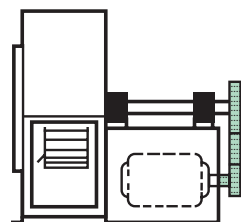
Arrangement 8

Direct coupled configuration with motor mounted to common fan base. Impeller is overhung and supported by two bearings on fan base. Temperature Limitations: 250°F.



Arrangement 9

Belt drive configuration with motor mounted on outside of bearing base support. Packaged unit, wheel is overhung, slide rail motor base permits easy adjustment of belt tension. Available on either left or right hand side of base (when facing drive end of shaft). Temperature limitations: 250°F.



Arrangement 10

Belt drive configuration with motor mounted inside base. Packaged unit, wheel is overhung. Temperature limitations: 250°F.

Adapted from AMCA Standard 99-2404-03, *Drive Arrangements for Centrifugal Fans*, and AMCA Standard 99-2407-03, *Motor Positions for Belt or Chain Drive Centrifugal Fans*, with written permission from Air Movement and Control Association International, Inc.



Material Specifications/Weights

Series 41

Class	Fan Size	Flanges				Shaft & Bearings		FA Type Wheel WR ² (Lbs.-Ft. ²)	Motor Frames			Installation Weights (Lbs. Less Motor)	
		Inlet		Outlet					Minimum Arr. #4	Maximum Arr. #4	Maximum Arr. #9 & #10	Arr. #4	Arr. #9 & #10
		Thickness	Holes	Thickness	Holes 100% & 66%	Size	Type						
I	12	1/8	7/16 X 8	1/4	7/16 X 10	1 ³ /16	P3U219	1.6	56	184T	182T	160	193
	15	3/16	7/16 X 8	1/4	7/16 X 14	1 ³ /16	P3U219	4.7	143T	215T	184T	235	230
	18	3/16	7/16 X 8	1/4	7/16 X 14	1 ⁷ /16	P3U223	11	143T	256T	213T	350	355
	22	1/4	7/16 X 8	1/4	7/16 X 18	1 ⁷ /16	P3U223	29	182T	286T	215T	490	490
	24	1/4	7/16 X 8	1/4	7/16 X 18	1 ⁷ /16	P3U223	44	182T	286T	254T	580	605
	27	5/16	7/16 X 8	3/8	7/16 X 18	2 ³ /16	P3U235	78	182T	286T	254T	660	770
	30	5/16	7/16 X 8	3/8	7/16 X 18	2 ⁷ /16	P3U239	119	213T	326T	256T	935	975
	33	5/16	7/16 X 8	3/8	7/16 X 22	2 ⁷ /16	P3U239	160	254T	365T	284T	1145	1185
	36	5/16	7/16 X 8	3/8	7/16 X 22	2 ¹¹ /16	P3U243	251	—	—	286T	—	1550
	40	5/16	7/16 X 8	1/2	7/16 X 26	2 ¹⁵ /16	P3U247	423	—	—	324T	—	2015
	44	3/8	7/16 X 8	1/2	7/16 X 30	2 ¹⁵ /16	P3U247	717	—	—	324T	—	2515
	49	3/8	9/16 X 16	1/2	7/16 X 34	2 ¹⁵ /16	P3U247	1180	—	—	326T	—	2940
	54	7/16	9/16 X 16	1/2	7/16 X 34	2 ¹⁵ /16	PB22447	1810	—	—	364T	—	3340
	60	7/16	9/16 X 16	1/2	7/16 X 38	2 ¹⁵ /16	PB22447	2875	—	—	365T	—	3670
II	12	1/8	7/16 X 8	1/4	7/16 X 10	1 ⁷ /16	P3U223	1.6	56	184T	184T	160	202
	15	3/16	7/16 X 8	1/4	7/16 X 14	1 ⁷ /16	P3U223	4.7	143T	215T	215T	235	235
	18	3/16	7/16 X 8	1/4	7/16 X 14	1 ¹¹ /16	P3U227	11	143T	256T	256T	350	355
	22	1/4	7/16 X 8	1/4	7/16 X 18	1 ¹¹ /16	PB22427	29	182T	286T	256T*	490	505
	24	1/4	7/16 X 8	1/4	7/16 X 18	1 ¹¹ /16	PB22427	44	182T	286T	286T*	580	625
	27	5/16	9/16 X 8	3/8	7/16 X 18	2 ³ /16	PB22435	78	182T	286T	286T*	660	800
	30	5/16	9/16 X 8	3/8	7/16 X 18	2 ⁷ /16	PB22439	119	213T	326T	286T*	935	995
	33	5/16	9/16 X 8	3/8	7/16 X 22	2 ⁷ /16	PB22439	160	254T	365T	326T*	1145	1195
	36	5/16	9/16 X 8	3/8	7/16 X 22	2 ¹¹ /16	PB22443	251	—	—	326T*	—	1620
	40	5/16	9/16 X 8	1/2	7/16 X 26	2 ¹⁵ /16	PB22447	423	—	—	365T*	—	2060
	44	3/8	9/16 X 8	1/2	7/16 X 30	2 ¹⁵ /16	PB22447	717	—	—	365T*	—	2560
	49	3/8	1 ¹ /16 X 16	1/2	7/16 X 34	2 ¹⁵ /16	PB22447	1180	—	—	405T*	—	3040
	54	7/16	1 ¹ /16 X 16	1/2	7/16 X 34	2 ¹⁵ /16	PB22447	1810	—	—	405T*	—	3480
	60	7/16	1 ¹ /16 X 16	1/2	7/16 X 38	2 ¹⁵ /16	PB22447	2875	—	—	405T*	—	3670
III	12	1/8	9/16 X 8	1/4	7/16 X 10	1 ¹¹ /16	P3U227	1.6	56	184T	184T	160	213
	15	3/16	9/16 X 8	1/4	7/16 X 14	1 ¹¹ /16	P3U227	4.7	143T	215T	215T*	235	250
	18	3/16	9/16 X 8	1/4	7/16 X 14	1 ¹⁵ /16	P3U231	11	143T	256T	256T*	350	375
	22	1/4	9/16 X 8	1/4	7/16 X 18	1 ¹⁵ /16	PB22431	29	182T	286T	256T*	490	525
	24	1/4	9/16 X 8	1/4	7/16 X 18	1 ¹⁵ /16	PB22431	44	182T	286T	286T*	580	635
	27	5/16	9/16 X 16	3/8	7/16 X 18	2 ³ /16	PB22435	78	182T	286T	286T*	660	820
	30	5/16	9/16 X 16	3/8	7/16 X 18	2 ⁷ /16	PB22439	119	213T	326T	286T*	935	1040
	33	5/16	9/16 X 16	3/8	7/16 X 22	2 ⁷ /16	PB22439	160	254T	365T	326T*	1145	1210
	36	5/16	9/16 X 16	3/8	7/16 X 22	2 ¹¹ /16	PB22443	251	—	—	326T*	—	1630
	40	5/16	9/16 X 16	1/2	7/16 X 26	2 ¹⁵ /16	PB22447	423	—	—	365T*	—	2080
	44	3/8	9/16 X 16	1/2	7/16 X 30	2 ¹⁵ /16	PB22447	717	—	—	365T*	—	2580
	49	3/8	1 ¹ /16 X 16	1/2	7/16 X 34	2 ¹⁵ /16	PB22447	1180	—	—	405T*	—	3110
	54	7/16	1 ¹ /16 X 16	1/2	7/16 X 34	2 ¹⁵ /16	PB22447	1810	—	—	405T*	—	3500
	60	7/16	1 ¹ /16 X 16	1/2	7/16 X 38	2 ¹⁵ /16	PB22447	2875	—	—	405T*	—	3800

* Motor Frames exceeding these values must be Arrangement 9M, Arrangement 1, or Arrangement 8.

For other Arrangement maximum motor frame size and dimensions, please contact factory.

Series 41P

Class	Fan Size	Flanges				Shaft & Bearings			FA Type Wheel WR² (Lbs.-Ft.²)	Maximum Motor Frame Arr. #10	Installation Weights (Lbs. Less Motor)
		Inlet		Outlet							
		Thickness	Holes	Thickness	Holes 100% & 66%	Size	Drive Side	Inlet Side			
II	12	1/8	7/16 X 8	1/4	7/16 X 10	1 11/16	P3U-227	P3U-227	1.6	215T	188
	15	3/16	7/16 X 8	1/4	7/16 X 14	1 11/16	P3U-227	P3U-227	4.7	215T	215
	18	3/16	7/16 X 8	1/4	7/16 X 14	1 15/16	P3U-231	P3U-231	11	254T	309
	22	1/4	7/16 X 8	1/4	7/16 X 18	1 11/16	P3U-227	P3U-227	29	256T	397
	24	1/4	7/16 X 8	1/4	7/16 X 18	1 15/16	P3U-231	P3U-231	44	256T	554
	27	5/16	9/16 X 8	3/8	7/16 X 18	2 3/16	P3U-235	P3U-235	78	286T	728
	30	5/16	9/16 X 8	3/8	7/16 X 18	2 3/16	PB-22435	P3U-235	119	324T	878
	33	5/16	9/16 X 8	3/8	7/16 X 22	2 3/16	P3U-235	P3U-235	160	324T	1013
	36	5/16	9/16 X 8	3/8	7/16 X 22	2 3/16	P3U-235	P3U-235	251	326T	1131





SPECIFICATIONS

System

- Oxidant Output: ≤ 1.5 lbs/day
- Number of Nozzles: 1-3
 - Standard 500 Nozzle
 - 20 CFM
 - 5 to 10 GPH
 - Low Volume Nozzle
 - 1 CFM
 - 1 to 2 GPH
- Treatment Area
 - Up to 18,000 ft³
 - Low Volume Nozzle – up to 1,000 ft³

Power supply

- 220 VAC, 30A, 60 Hz, Single Phase or
- 110 VAC, 30A, 60 Hz

Physical

- Aluminum Powder Coated with TGIC polyester
- Dimensions
 - 41.6" L \times 29.5" W \times 39.4" H
- Unit Weight
 - 150 to 165 lbs avg

Operating environment

- 20°F to 100°F

MAIN FEATURES

- Eliminates H₂S and other odorous compounds
- Reduce or eliminate some forms of Fats, Oils, and Grease (FOG)
- Reduce or eliminate biofilm or bacterial growth in the treatment area
- Reduce the rate of corrosion typically associated with low pH
- Impart a residual oxidant to the defined space to absorb unexpected spikes of odors

DESCRIPTION

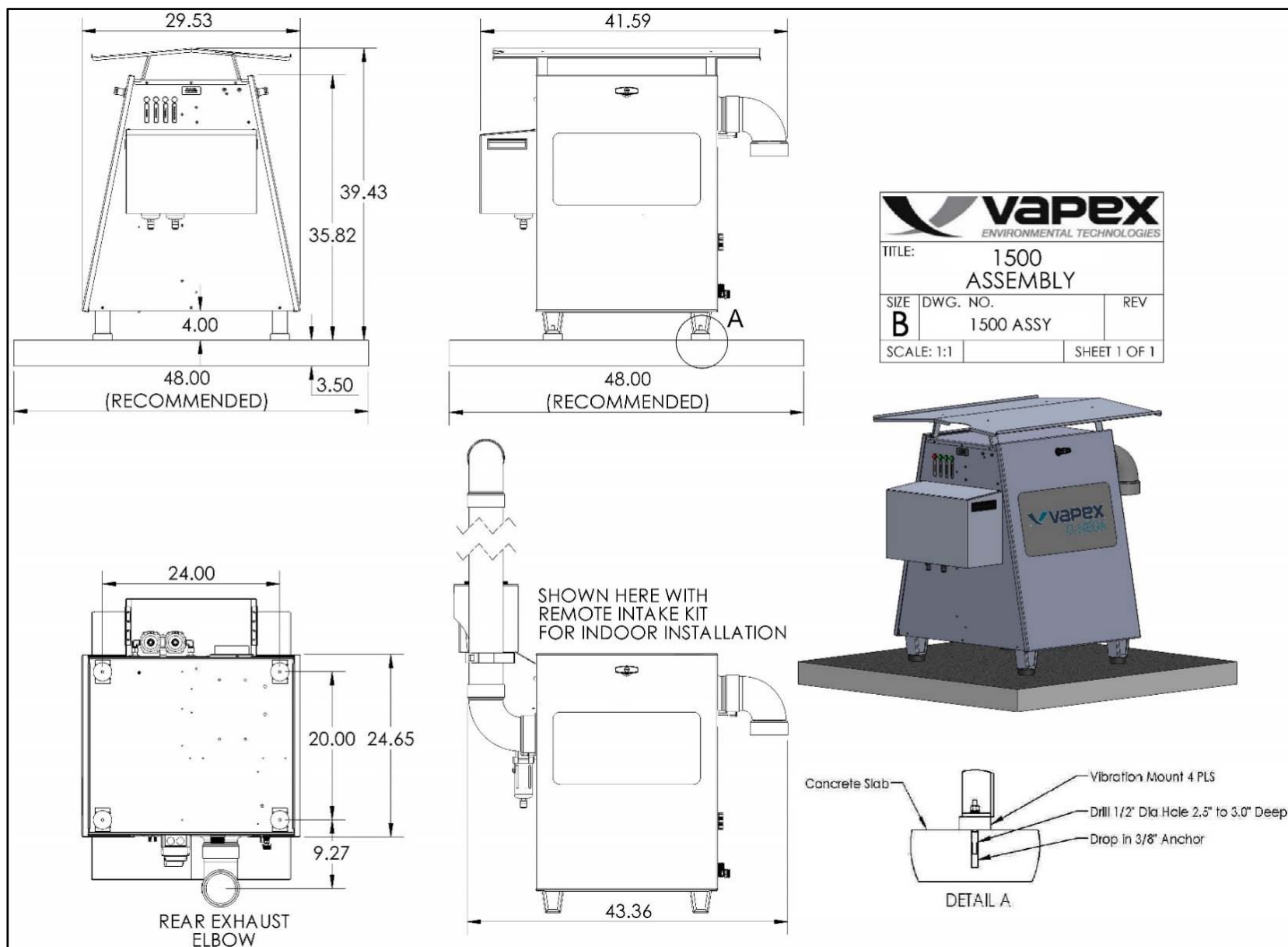
The Vapex 1500 is an odor control system specifically designed to treat H₂S, mercaptans, amines, and other odorous compounds in enclosed spaces. It combines ozone, water, and air using a patented 3-fluid nozzle to atomize the water molecules to create hydroxyl radicals. The odorous air is not extracted instead the odors are treated at the same space where they are generated.

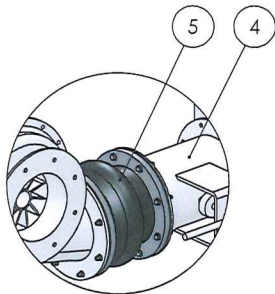
APPLICATIONS

- Lift Stations/Pump Stations
- Wet Wells
- Holding Tanks
- Headworks
- Covered Clarifiers

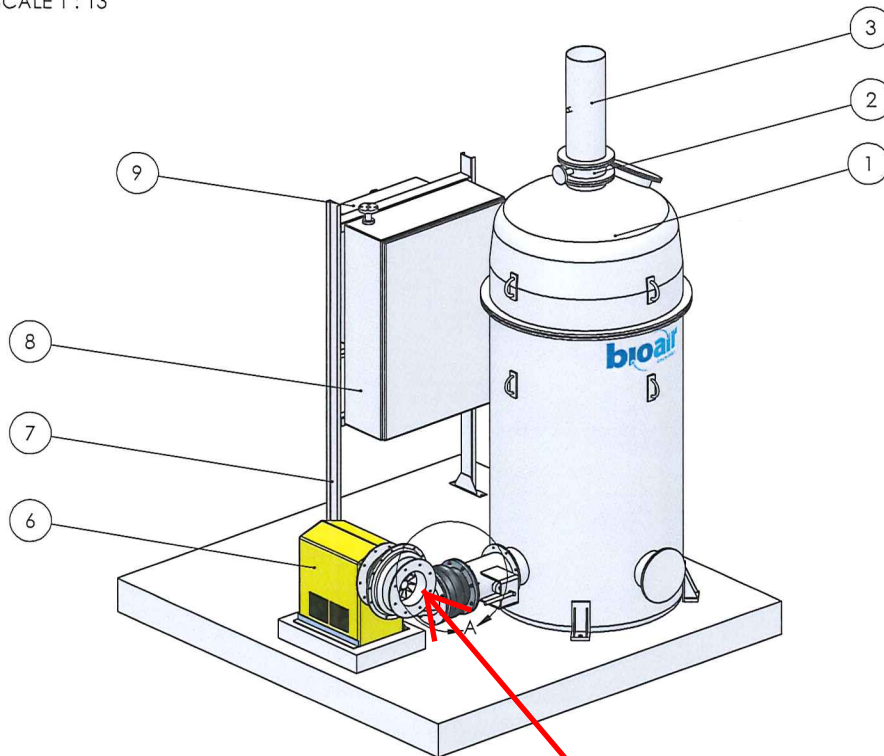
CONTACT INFORMATION

- Contact your local Vapex rep
- Call Vapex – 407-977-7250
- Email Vapex – Info@vapex.com





DETAIL A
SCALE 1 : 13



REV.	DATE	ECN#	REVISION RECORD	DRICK
A	10/24/2011			KZ

ITEM	PART NUMBER	QTY	DESCRIPTION	OPERATING WEIGHT (LBS)
1	RE031000	1	EF31 REACTOR ASSEMBLY WITH Ø2" ANSI 150 DRAIN	1725
2	NA081000	1		
3	ST082400	1	FRP STACK Ø8 PS1569 x 24"	
4	DV081000	1	FRP CONTROL DAMPER VALVE Ø8" PS15-69	10
5	FC080000	1	FLEX CONNECTOR SINGLE ARCH Ø8" PS15-69	10
6	BL200000	1	CAST ALUMINUM BLOWER	150
7	PS000100	1	STAINLESS STEEL PANEL STAND	49
8	WP101000	1	FRP WATER PANEL	65
9	CP000100	1	FRP ELECTRICAL PANEL	60

Content of this drawing (including but not limited to all confidential & copyrighted material, patented or patent pending designs) is proprietary and remains the property of BioAir Solutions, LLC, and subsidiaries. It is sent to you subject to return upon demand and for the sole purpose as expressed in writing in this transmittal. It is not to be copied, reproduced or used in any way, directly or indirectly, detrimental to the interests of BioAir Solutions, LLC.

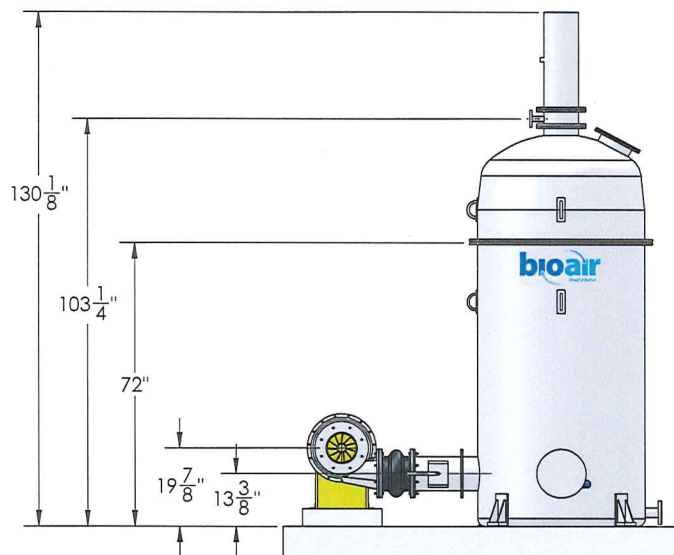
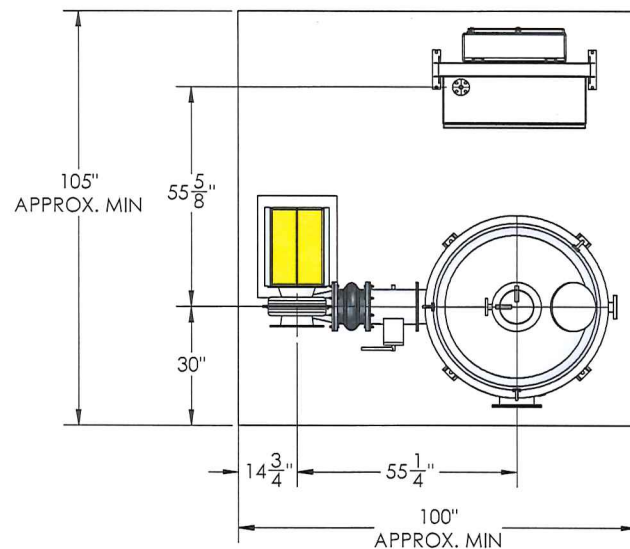
MATERIAL	SEE TABLE	
FINISH		

DRAWN BY	DATE	ENG APPR.	DATE	QC APPR.	DATE
KZ	10/24/2011				

PART NAME
GENERAL ARRANGEMENT ECOFILTER™ 31

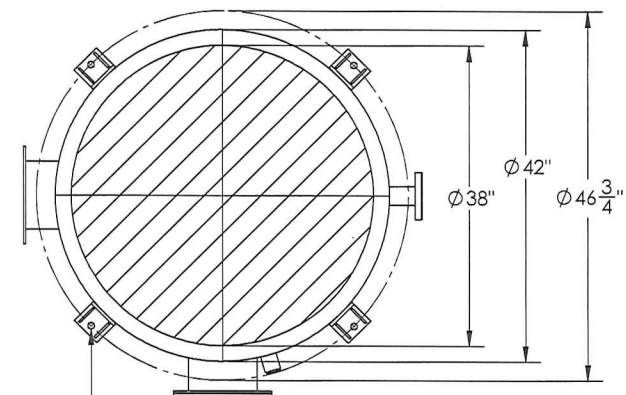
UNLESS OTHERWISE SPECIFIED DIMENSIONS ARE IN INCHES TOLERANCES ARE: ANGULAR: ±1° FRACTIONS: ±1/4 TWO PLACE DECIMAL: ±.060 THREE PLACE DECIMAL: ±.030	SIZE B	DWG. NO. EF31_GA	REV A
SCALE: 1 : 26		SHEET 1 OF 2	

this is the
connection port,
right ?



NOTES:

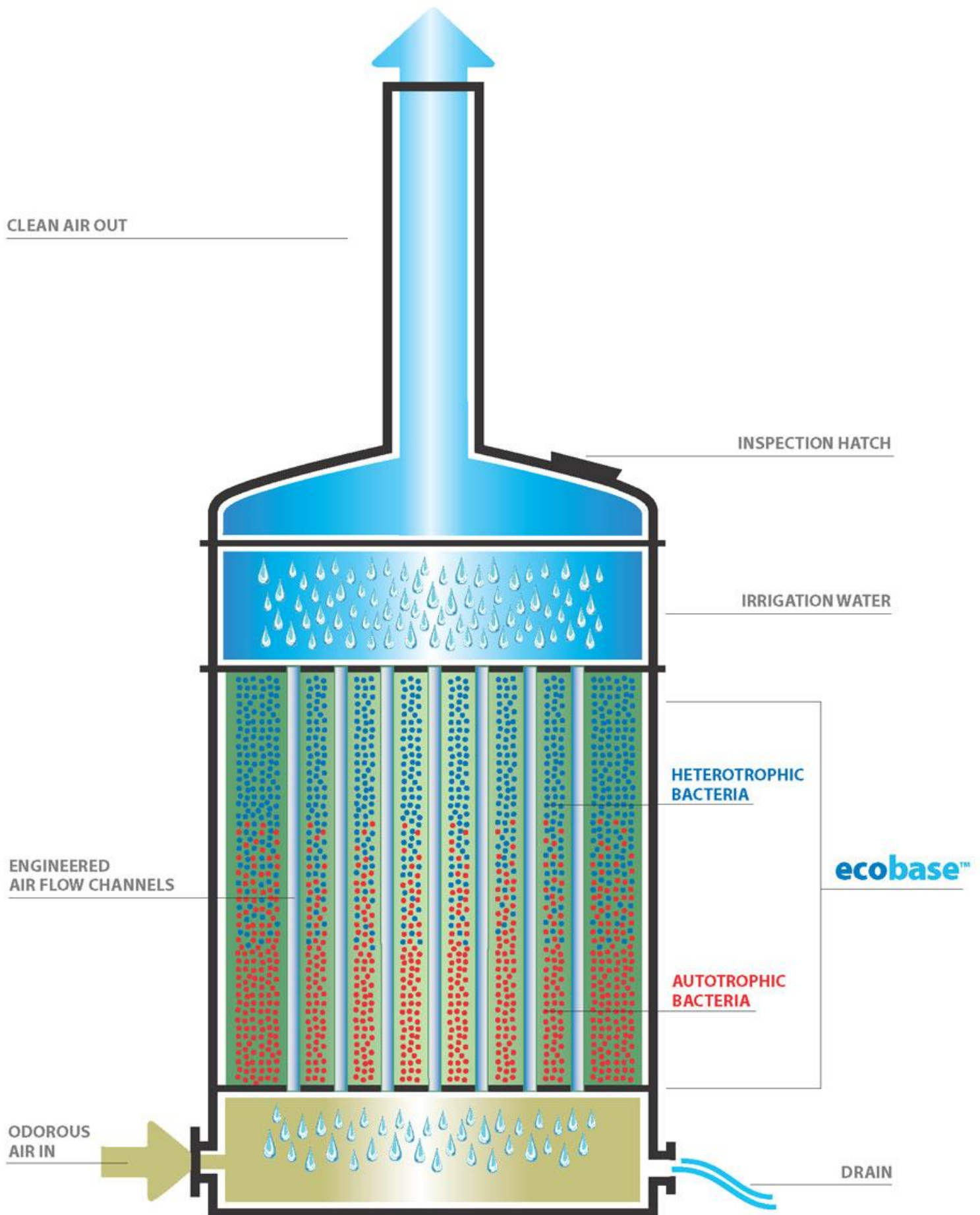
1. FOUNDATION PAD MUST BE FABRICATED UNIFORM IN TEXTURE AND APPEARANCE AND MEET A SURFACE PLANE TOLERANCE OF 1/8" IN 10'.
2. REACTOR TO BE SET ON 30LB FELT PAPER
3. LOAD DISTRIBUTION AREA = 1134 in²
4. SHIPPING WEIGHT = 872 LBS, OPERATING WEIGHT = 1725 LBS



Ø 7/8" MOUNTING HOLE
EQUALLY SPACED ON
46 3/4" B.C.

LOADING DIAGRAM
SCALE: 1 : 15

SIZE	DWG. NO.	REV
B	EF31_GA	A
SCALE: 1 : 30		SHEET 2 OF 2



PUMP **ONE** LIFT STATION



PUMP OUT STATION



Created from more than 25 years of experience.



The OneLift™ by Oldcastle Precast, provides a standard turnkey submersible pump station product with the distinction of having a sole-source point of responsibility. The OneLift Pump Station is designed with an integral valve vault built into the unused top portion of the wet

well, which yields a single-structure product, that solves the problems of differential settlement found with the conventional 2-structure systems. In addition; the single-structure design significantly reduces the product footprint for sites with tight area restrictions, and reduces complexity, size and cost of the excavation. The OneLift has proven to significantly reduce the time required for site installation.

OneLift's standard innovative design allows for quicker project documentation, faster product delivery and repetitive high quality manufacturing. Oldcastle Precast's turnkey obligation means that the OneLift Pump Station will be factory preassembled with all mechanical and electrical components prior to delivery, and that Oldcastle Precast, or our licensed distributors, are responsible for product commissioning, training and complete package warranty.

You can depend on the name and quality of Oldcastle Precast

TURNKEY PACKAGE
station from a
single supplier
and installed in
less than a day.



INNOVATION

Single Structure design

The factory built OneLift pump station offers a monolithically cast valve vault, in what is typically an unused portion of the pump station. This results in a significantly smaller footprint – up to 50% smaller than conventional 2 structure pump stations – allowing it to fit tight sites. The innovative shape and structural design provides ample space for the interior valve vault, while increasing workable system volumes and decreasing the depth of the excavation. The single structure of the OneLift pump station eliminates any potential differential settlement issues, allows for a quick and easy installation by eliminating the typical two-tier excavation of conventional stations, and is proven to be a cost-effective method of providing a high quality pump station solution with rapid and reliable delivery. With the OneLift pump station you get fast turnaround time on submittals; which include standard structural and mechanical components.

Design Features:

- Rounded-corner design – allows for thinner/ lighter walls and prevents solids accumulation
- Near rectangular shape – provides more storage volume in the bottom and more space in the top to integrate the valve chamber
- Single structure design – provides a smaller footprint, simpler excavation, rapid installation, and eliminates potential for differential settlement
- Standard design – allows for stockable components for quicker turnaround and a rapid submittal package

STOCKED
COMPONENTS
of various
heights to
fit specific
jobsite needs.



QUALITY CONSTRUCTION

Factory built to rigid Oldcastle Precast quality standards

With the OneLift pump station you get quick submittal turn-around and high quality repetitive factory outfitting. Assembly of mechanical and electrical components in controlled factory conditions means a consistently higher quality product than site assembled stations. The OneLift pump station is factory preassembled and then partially disassembled for shipping. This prepackaged solution allows for field erection and reassembly in less than four hours. You get standard structural and mechanical components that allow for factory stocking, resulting in quicker station deliveries.

Construction Features:

- Standardized/stockable components that allow for fast production
- Packaging in controlled factory vs. variable field conditions
- Oldcastle quality and local availability
- Quick project turnaround



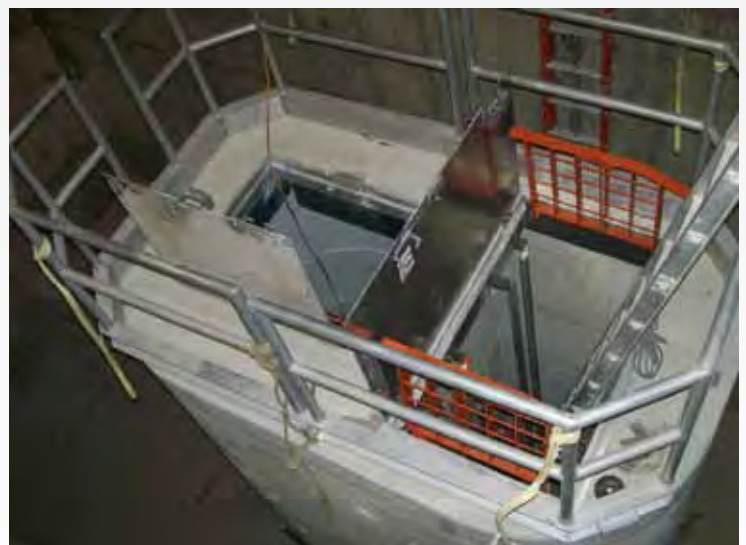
PREASSEMBLED equipment package providing rapid installation and factory built quality.

Technical info:

- Station height 10'-10" to 24'-10"
- Top slab with hatches: 300# pedestrian, or H2O design loading
- Valve vault extension risers (2' & 4')
- Integral valve vault section (4'-8' high)
- Wet well riser sections (2', 3', & 4' high)
- Base section 4'-8' w/ mono fillet & collar
- Factory preassembly of equipment provides: factory assembled quality which further accelerates installation to ½ day



Valve Assembly in Valve Vault, pre-assembled in factory.



Pump Station during factory assembly.

BENEFITS TO ...

the Design Engineer

- Smaller footprint on site
- Accommodates multiple pump manufacturers
- Turn-key package from one supplier
- AutoCAD drawings and specs readily available
- Timely and accurate budget estimates
- Can be considered a "standard off-the-shelf product"

the Owner

- Rapid project cycle (design & construct)
- Smaller footprint on site
- Standardized and repeatable
- Overall cost savings
- Reliability and quality of Oldcastle Precast

the Site Contractor

- Quick delivery timeline and rapid installation process
- Single smaller excavation (possibly shallower)
- Rapid installation limits the dewatering process
- Preassembled-requires only minor reassembly on site
- Safer below grade construction process
- Competitive installed cost
- Greater value and knowr

TYPICAL 1/2-DAY INSTALLATION

As a prepackaged solution, the OneLift pump station arrives at your job site preassembled and ready for installation. The preassembly of the OneLift pump station allows for a simplified construction process reducing installation time to 3-4 hours or just half a day. The single structure of the OneLift pump station is significantly easier to install.



9:30 am

Flat bed trucks arrive with pump station components. Contractor has excavation ready for pump station installation.



10:15 am

Crane off-loads base section into excavation, additional sections are set at a rate of approximately one every thirty minutes.

A single tier excavation is all that is required which improves job site safety as assembly time in the excavation is reduced. Dewatering time and costs are reduced as well. *Case study: RC509 delivered and installed in North Truro, MA.*



11:15 am

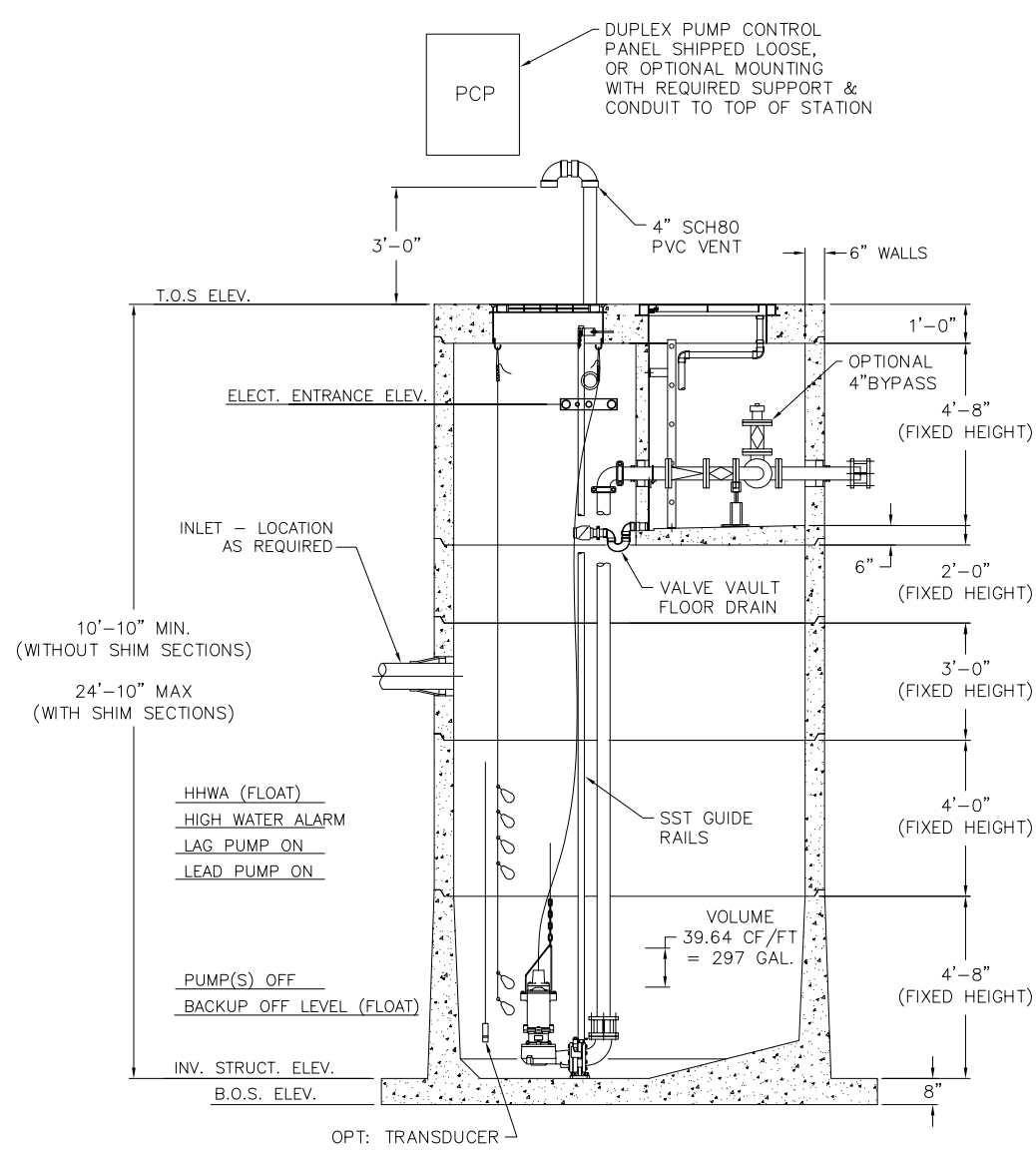
Crane sets long discharge pipes and pump guide rails (shipped loose) into station. Pump station top section with integral vault is set.



12:00 Noon

Minor reassembly of shipped loose items are completed in two hours and backfill begins.

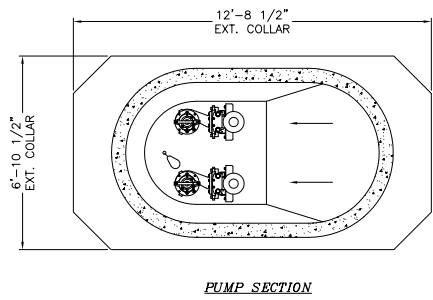
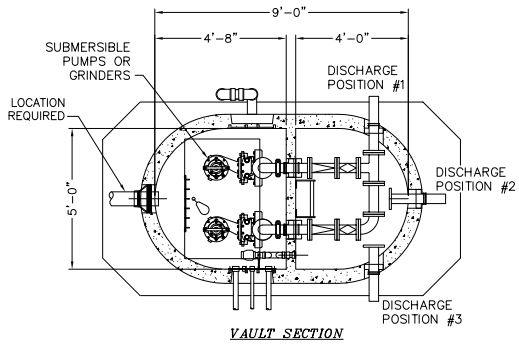
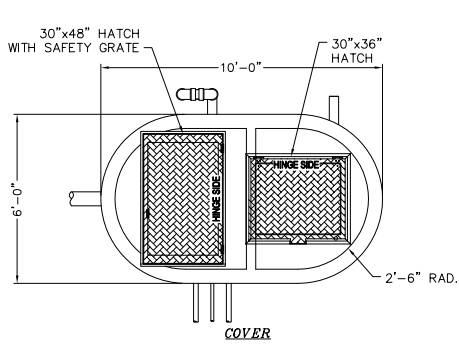
SPECIFICATIONS



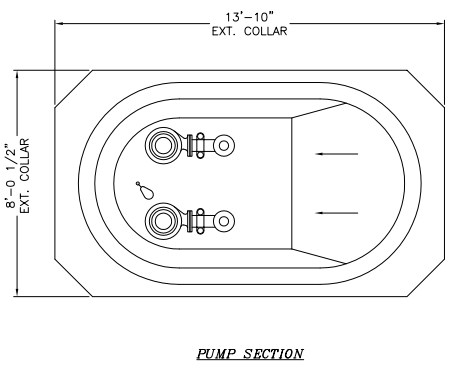
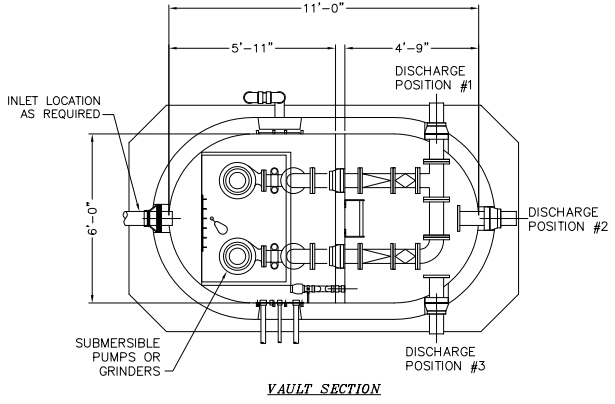
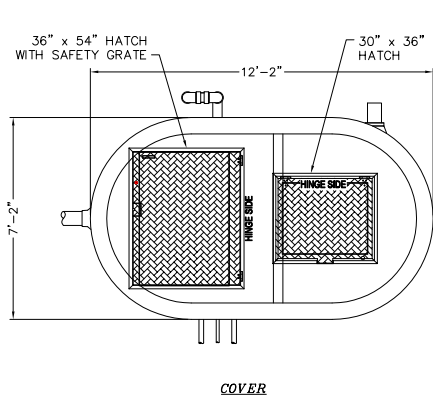
INTERIOR ELEVATION VIEW
RC509 x 10'-10" to 24'-10"

PUMP STATION SIZING CHART

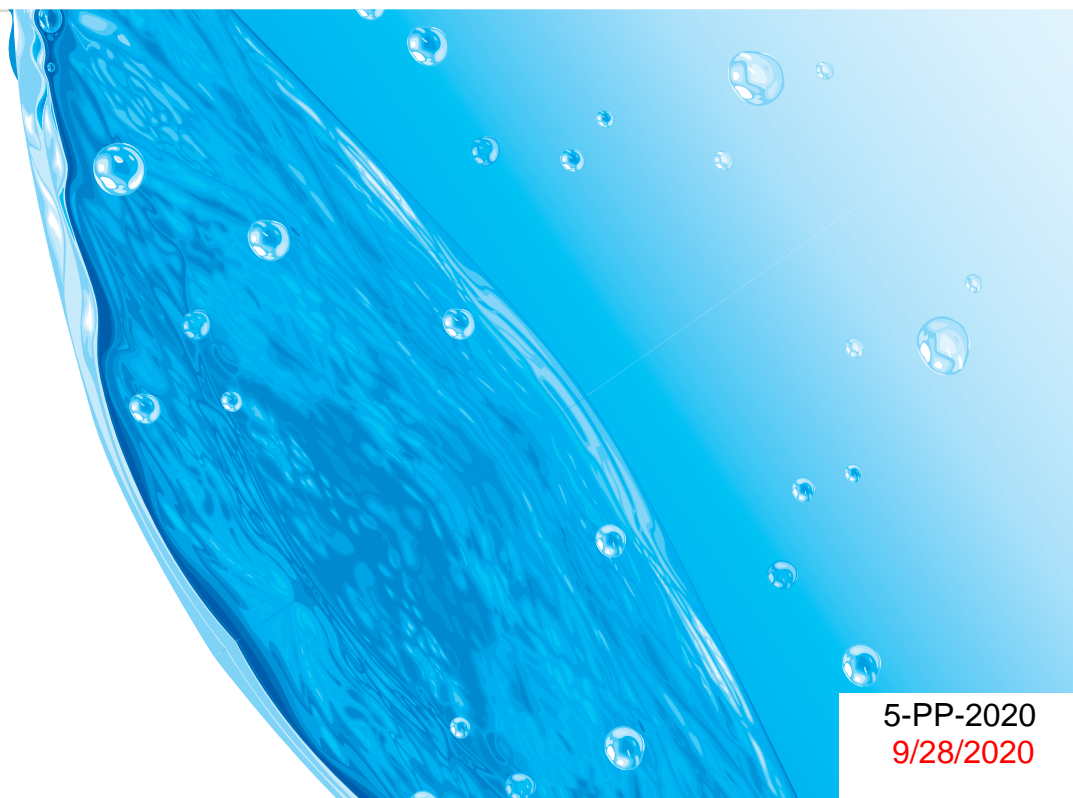
Model Number	RC509	RC611
Interior Width	5'-0"	6'-0"
Interior Length	9'-0"	11'-0"
Min/Max Height (T.O.S. to floor)	10'-10" / 24'-10"	10'-10" / 24'-10"
Volume (gal/vertical foot)	297 gal	436 gal
Standard DI Piping & Discharge Size*	4"	6"
Replaces Conventional Pump Stations	6' diameter & 8' diameter	8' diameter & 10' diameter



RC509 PLAN VIEWS



RC611 PLAN VIEWS





**PUMP ONE
LIFT
STATION**



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5-PP-2020
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