



Drainage Reports

DRAINAGE REPORT FOR KERRY'S CAR CARE SHEA

Plan # _____

Case # 10-UP-2019

Q-S # _____

_____ Accepted

X Correction

GA

10/21/2019

Reviewed By

Date

Scottsdale, Arizona

05 September 2019

PREPARED FOR
Larson Associates Architects, LLC
3807 North 24th Street, Suite 100
Phoenix, Arizona 85016
Attn: James Larson

DEVELOPER
SimonCRE
6900 East 2nd Street
Scottsdale, Arizona 85251
Attn: Peter Krahenbuhl

SITE ADDRESS
11653 East Sahuaro Drive
Scottsdale, Arizona 85259

PREPARED BY

CYPRESS
CIVIL DEVELOPMENT
strength + sustainability

4450 north 12th street, #228
phoenix, arizona 85014
CYPRESS # 19.101



10-UP-2019
09/26/2019

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I. INTRODUCTION

1. PROJECT NAME AND LOCATION

The Project site is located at 11653 East Sahuaro Drive in Scottsdale, Arizona. The project is located in the southeast ¼ of Section 22, Township 3 North, Range 5 East, G&SRM. The Project site occupies approximately 2.52 acres. The Project is currently undeveloped desert and is a portion of the Mountainside Plaza. The Project has street frontage and access to Sahuaro Drive along its northern boundary. To the west and south are existing commercial retail developments within Mountainside Plaza. to the east are single-family homes. Refer to Appendix A for Location Map and Aerial Photo.

The proposed Project consists of the construction of a new single-story automotive service building with required grading and drainage, onsite utility, and parking improvements.

2. PURPOSE

The intent of this Drainage Report is to provide the conceptual drainage scheme for the Project in support of the Preliminary Improvement Plan Submittal.

3. EXISTING STUDIES

As-Built Improvement Plans and a Drainage Report for Mountainside Plaza (previously known as 116th Street & Shea Retail Center), prepared by Evans, Kuhn, and Associates, Inc. in 2002, were obtained and reviewed. The plans show that the Project is Lot 3 of the Mountainside Plaza development. The report shows that the plaza was designed to retain all onsite and offsite runoff in underground retention vaults. Refer to [Appendix B for Mountainside Plaza As-Built Plans & Drainage Report](#).

Need to clarify what is offsite !. No offsite flow from outside the overall development was included in retention volume provided; only the onsite 100-yr, 2-hr retention volume was accounted as required.

4. FEMA FLOOD ZONE

According to the Federal Emergency Management Agency Flood Insurance Rate Map, panel number 04013C1780L dated October 16, 2013, the parcel is located in the shaded Zone X Area, which is an area defined as within the 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood. Refer to Appendix C for FEMA FIRM Map.

The plans included in Appendix B are of poor quality and not readable. Please include a better quality copy, and, if not available, extract the appropriate information to support the statement.

II. EXISTING DRAINAGE INFRASTRUCTURE

1. OFFSITE

Offsite flow is routed through the site via an existing storm system drain that captures offsite flow from a retention basin north of Sahuaro Drive and conveys it to outfall to existing 28" outfall pipes in 116th Street. Runoff from the adjacent Mountainside Fitness building and from the drive aisle running from west to east north of the

The referenced drainage report in Appendix B, as indicated in Section 3 above, talked about exsiting 24" pipes that drain offsite RETENTION basins on the north side of Sahuaro Dr, not Detention basins. If this is the case, and there would be probably some kind of orifice plates at the pipes inlets, not all offsite flow is routed in the pipes; only the corresponding retention requirements. The design storm is typically the 100-yr, 6-hr or 24-hr; offsite flow in excess of the 100-yr, 2-hour need to be addressed. On the other hand if the basins are actually detention basins, then some kind of storage routing would have been generated and need to presented. Please clarify.

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shopping center flows through the drive aisle along the western boundary of the project. This is roughly 11.5 CFS of flow.

Why assumed, the offsite flow needs to be quantified. Please see comment on last page.

STREET CAPACITY CALCULATIONS

The street capacity within Sahuaro Street is 44.94 CFS. It is assumed this is sufficient capacity to convey all offsite flows away from the Project, as it does in the existing condition. Refer to Appendix D for Sahuaro Drive Capacity Calculations.

Please see the comment adjacent to Section 3 on last page.

2. ONSITE

The As-Built Plans and Drainage Report show that the Project site currently drains to a temporary surface retention basin onsite. However, the existing underground storm drain and retention vault system for the overall Mountainside Plaza were designed to convey and retain all onsite flow for the Project site in its future development.

III. PROPOSED DRAINAGE INFRASTRUCTURE

1. CONVEYANCE OF RUNOFF

Per earlier comments, it is not clear that this site was also included in the retention capacity of the underground retention system. This need to be confirmed.

Additionally, the basin is for interim conditions where the surface is not paved. This allow sediment to enter the underground retention system and may create maintenance and capacity issues. It was mentioned that the retained water in the basin would only be 0.8 ft. Can't this volume naturally percolate within 24 hr and remove the interim need of the connection to the underground system?

Offsite runoff from the Mountainside Fitness building and the drive aisle behind it shall be intercepted by a new catch basin at the northwest corner of the Project site. Onsite runoff from the Kerry's car care building and surrounding landscaping shall be conveyed to a new catch basin north of the building. Onsite runoff from the future retail development area shall be conveyed to a new temporary surface retention basin at the southeast, where it shall pond at a depth of 0.80 feet and drain via a single new catch basin. From these three new catch basins, runoff shall be conveyed south via new storm drain and connect directly to the existing underground retention vaults, where runoff shall be discharged via existing drywells.

Runoff from the majority of the new Kerry's parking lot and western drive aisle shall be conveyed via shallow sheet flow and a new valley gutter to an existing catch basin at the southwest corner of the site where it will enter the existing storm drain system and be conveyed to the existing underground retention vaults. Refer to Appendix E for the Proposed Watershed Map and Calculations.

2. STORM WATER RETENTION REQUIREMENTS

In the original Mountainside Plaza development, the existing underground storm drain and retention vault system was designed to convey and retain all onsite flow for the Project site in its future developed condition. Therefore, no new retention is required. The existing underground retention system is designed to drain within 36 hours via a bleed-off pump to existing 28" outfall pipes in 116th Street.

per previous comments, need to be confirmed.

Please confirm that the proposed development is similar to what originally anticipated and no additional retention volume would be generated.

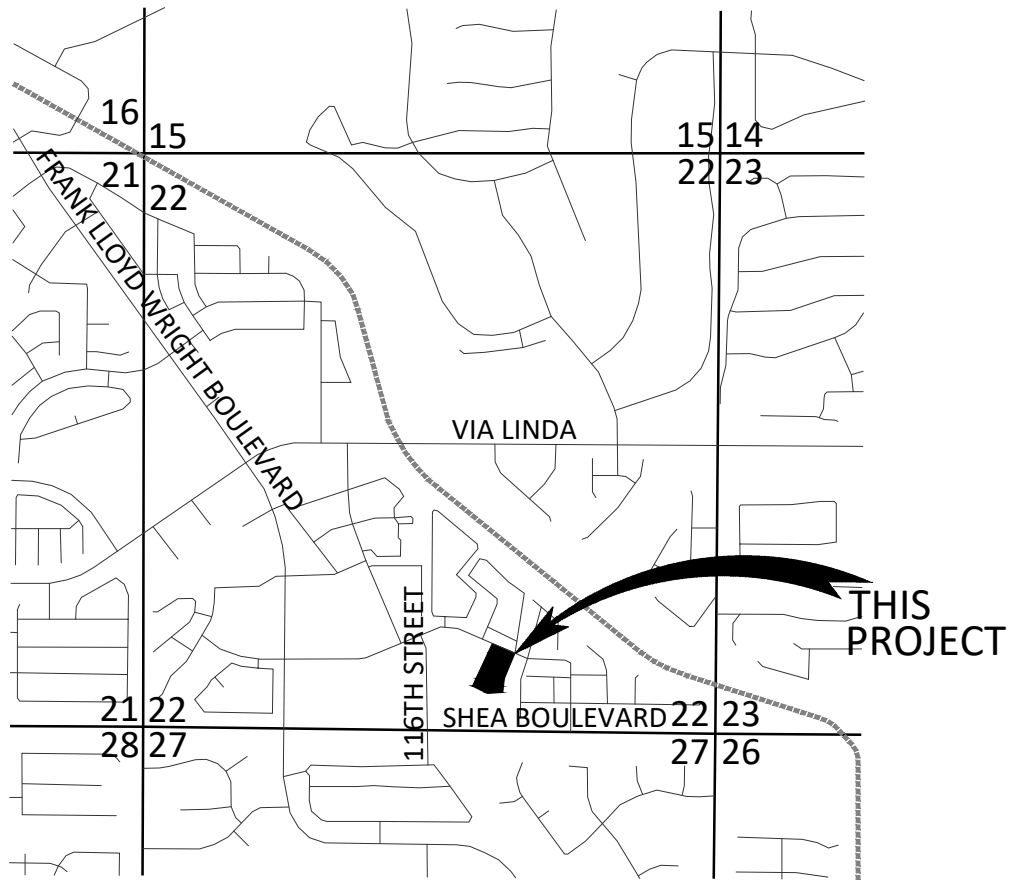
Surface runoff from the project site, being an automotive service, would have additional environmental concerns for water quality entering the underground system then pumped into a stormdrain along 116th St. Mitigation to First Flush requirements need to be addressed.

IV. SUMMARY AND CONCLUSION

This Drainage Report is to accompany the Improvement Plan for the Kerry's Car Care Shea development project. This narrative was written utilizing generally accepted engineering practices and all information herein has been researched through archived documents and all calculations were accomplished through applying the City of Scottsdale Engineering Standards.

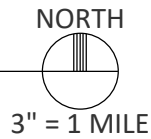
The analysis presented in this narrative evaluates storm water runoff resulting from a statistical evaluation of storm events of particular duration and frequency up to and including a 100-year frequency event. A storm event exceeding the 100-year frequency may cause or create the risk of greater flood impact than is addressed and presented herein. The scope of this assessment does not include evaluation of storm water runoff resulting from storm events exceeding the 100-year frequency. CYPRESS assumes no responsibility for actual flood damage, increased risks of flood damage, or increased construction or development cost resulting from or related to any such events, nor shall CYPRESS be responsible for any changes in, or additions to, regulatory requirements which may result from, or be related to, any such events or changes in hydrologic or hydraulic conditions within the watershed. Refer to Appendix E for Warning and Disclaimer of Liability Form.

APPENDIX A
(Location Map + Aerial Photo)



IN THE SW 1/4 OF THE SE 1/4 OF SECTION 22,
T. 3 N., R. 5 E., G.&S.R.M.,
CITY OF SCOTTSDALE, MARICOPA COUNTY, ARIZONA

LOCATION MAP



AERIAL PHOTO



APPENDIX B
(Mountainside Plaza As-Built Plans & Drainage Report)

NATIVE PLANT PERMIT NUMBER
CASE # 4-100-1000 PERMIT # 70707

EARTHWORK ESTIMATES
CUT 20,193 CY
FILL 10,455 CY
NET (CUT) 10,738 CY

QUANTITIES SHOWN ARE IN PLACE ESTIMATES WITH NO ADJUSTMENTS MADE FOR SOFTENING, SUBSIDENCE OR COMPACTION LOSSES.

ENGINEER'S ESTIMATES
OFF-SITE QUANTITIES

BRIDGE VERTICAL CURVE, SLOTTED & SMOOTH
CONCRETE SIDEWALK
REMOVE & REPLACE 40' PAVEMENT
WATER LINE CONNECTIONS
SANITARY SINKER MAINS & TAPS
CONCRETE VALVE BOXES & AFRAM
CONCRETE DRAINAGE CURB
CONCRETE CURB & GUTTER
PREFABRICATED MANHOLE
DRAINAGE STRUCTURE
8" DUCT IRON-CLAS PIPE
8" DUCT IRON-CLAS PIPE
RANGE EXISTING PAV.

FLOODPLAIN NOTE

THE SUBJECT PARCEL LIES WITHIN FLOOD ZONE "X"
AREAS DETERMINED TO BE CHANGED THE 500-YEAR FLOODPLAIN

FLOOD INSURANCE RATE MAP (FIRM) INFORMATION					
COMMUNITY	PARCEL #	SUFFIX	DATE OF FIRM (ISSUE DATE)	FIRM ZONE	DATE FLOOD CLAY (BY NO. 1000 USE DATES)
04613	1702	C	09/30/95	X	N/A

ENGINEER'S CERTIFICATION

THE LOWEST FLOOD ELEVATION (FEET) AND FLOODPROOFING ELEVATION (FEET) ON THIS PLAN ARE SUFFICIENTLY HIGH TO PROVIDE PROTECTION FROM FLOODING CAUSED BY A ONE-HUNDRED YEAR STORM, AND ARE IN ACCORDANCE WITH CITY OF SCOTTSDALE ZONING CODE, CHAPTER 17-- FLOODING AND FLOODPLAIN ORDINANCE.

NO CONFLICT SIGNATURE BLOCK

Utility	Utility Company	Name of Company Representative	Telephone Number	Date Signed
Water	CITY OF SCOTTSDALE			
Electric	ARIZONA PUBLIC SERVICE	JOHN ROBERTS	602-493-4402	
Gas	CHS	CHRIS LITTON	602-430-0482	
Sanitary Sewer	SOUTHWEST GAS	SCOTT HAMILTON	602-494-8818	
Storm Sewer	CGK	SUZANNE HOLZER	602-322-7249	
Other	CITY OF SCOTTSDALE			

I, Wayne P. Raley, being the person responsible for designing the facilities necessary to serve the requirements, hereby certify that all of the utility companies listed above, have reviewed this project and expressed their approval of this plan. The City of Scottsdale has been notified of this project and the City Engineer has approved this plan. I also certify that all of the facilities shown on this plan are in accordance with the City of Scottsdale Zoning Code, Chapter 17-- FLOODING AND FLOODPLAIN ORDINANCE.

Wayne P. Raley
Signature
3/23/03
Date

CITY OF SCOTTSDALE
REVIEW AND RECOMMENDED APPROVAL BY:

NAME	OFFICIAL	DATE
GRADING	3/23/03	3/23/03
PAVEMENT	3/23/03	3/23/03
WATER & SEWER	3/23/03	3/23/03
APPROVED BY	3-28-03	DATE

IMPROVEMENT PLANS
FOR
MOUNTAINSIDE PLAZA
A SUBDIVISION OF A PORTION OF THE WEST HALF OF THE SOUTHEAST QUARTER OF SECTION 22, TOWNSHIP 3 NORTH, RANGE 5 EAST OF THE GLA AND SALT RIVER BASE AND MERIDIAN, MARICOPA COUNTY, ARIZONA

LOT AREA

- LOT 1 55,402.71 S.F. OR 1.27 ACRES
- LOT 2 150,287.62 S.F. OR 3.44 ACRES
- LOT 3 102,733.21 S.F. OR 2.32 ACRES
- LOT 4 25,402.63 S.F. OR 0.57 ACRES
- LOT 5 34,068.38 S.F. OR 0.76 ACRES
- LOT 6 32,788.91 S.F. OR 0.74 ACRES
- LOT 7 55,136.72 S.F. OR 1.22 ACRES
- LOT 8 35,077.43 S.F. OR 0.79 ACRES
- LOT 9 30,031.04 S.F. OR 0.68 ACRES
- LOT 10 52,204.40 S.F. OR 1.19 ACRES
- LOT 11 37,281.51 S.F. OR 0.85 ACRES
- 10 LOTS TOTAL
- NET LOT SIZE = 0.47 ACRES
- MOORAGE LOT SIZE = 1.15 ACRES

CITY OF SCOTTSDALE GENERAL CONSTRUCTION NOTES FOR PUBLIC WORKS CONSTRUCTION

- ALL CONSTRUCTION IN THE PUBLIC RIGHTS-OF-WAY OR IN ENGINEER'S DESIGNATED PUBLIC-USE RIGHT-OF-WAY SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE CITY OF SCOTTSDALE GENERAL CONSTRUCTION SPECIFICATIONS AND STANDARD DETAILS FOR PUBLIC WORKS CONSTRUCTION AS AMENDED BY THE LATEST VERSION OF THE CITY OF SCOTTSDALE (COC) SUPPLEMENTAL STANDARD SPECIFICATIONS AND STANDARD DETAILS. IF THERE IS A CONFLICT, THE LATEST SHALL CONTROL.
- THE ENGINEER'S DESIGN ON THESE PLANS ARE ONLY APPROVED BY THE CITY OF SCOTTSDALE AND NOT IN DETAIL. IF CONSTRUCTION QUANTITIES ARE GIVEN ON THESE PLANS, THEY ARE NOT VERIFIED BY THE CITY.
- APPROVAL OF PLANS IS VALID FOR SIX (6) MONTHS. IF AN ENGINEER'S PERMIT FOR THE CONSTRUCTION HAS NOT BEEN ISSUED WITHIN SIX MONTHS, THE PLANS SHALL BE RESUBMITTED TO THE CITY FOR RE-APPROVAL.
- A PUBLIC WORKS INSPECTOR SHALL INSPECT ALL WORKS WITHIN THE CITY OF SCOTTSDALE RIGHTS-OF-WAY AND IN ENGINEER'S DESIGNATED PUBLIC-USE RIGHT-OF-WAY PRIOR TO STARTING OF CONSTRUCTION (FOR BRIDGE) (FOR 3-10-03).
- WHenever excavation is to be done, call the "BLUE STAKE CENTER" (602) 203-1100. THIS MARKING GIVE SURFACE EXCAVATION IS TO BE DONE. THE CENTER WILL SEE THAT THE LOCATION OF THE UNDERGROUND UTILITY LINES IS IDENTIFIED FOR THE PROJECT. CALL "COLLECT" IF NECESSARY.
- STRENGTHENMENT PERMITS ARE REQUIRED FOR ALL WORK IN PUBLIC RIGHTS-OF-WAY AND ENGINEER'S DESIGNATED PUBLIC-USE RIGHT-OF-WAY. AN ENGINEER'S PERMIT MUST BE OBTAINED BY THE CITY OF SCOTTSDALE PRIOR TO THE START OF CONSTRUCTION. A FEE FOR THIS PERMIT SHALL BE PAID TO THE CITY OF SCOTTSDALE. COPIES OF ALL PERMITS SHALL BE RETURNED TO THE CITY OF SCOTTSDALE. COPIES OF ALL PERMITS SHALL BE RETURNED TO THE CITY OF SCOTTSDALE. COPIES OF ALL PERMITS SHALL BE RETURNED TO THE CITY OF SCOTTSDALE.
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GENERAL CONSTRUCTION NOTES (FOR PROJECT SPECIFIC)

- EXISTING UNDERGROUND UTILITIES WERE OBTAINED FROM THE LATEST AVAILABLE UTILITY RECORDS AND SHOULD BE CONSIDERED APPROXIMATE IN LOCATION AND DEPTH. EXISTING UTILITIES SHOULD BE IDENTIFIED IN THE FIELD. CALL "BLUE STAKE" A MINIMUM OF TWO (2) WORKING DAYS PRIOR TO EXCAVATION.
- ALL DIMENSIONS SHOWN ARE TO FACE OF CURB, CENTERLINE OF RIGHT-OF-WAY OR CENTERLINE OF STRUCTURE UNLESS NOTED OTHERWISE.
- CONTRACTOR SHALL OBTAIN A COPY OF THE SOIL REPORT FOR THE PROJECT PREPARED BY RICKER-JARVIS-MOORE & ASSOCIATES, ON 1/28/2001, RAN PROJ. NO. 000805.
- IT SHALL BE THE RESPONSIBILITY OF THE OWNER TO VERIFY ALL QUANTITIES INCLUDING EXCAVATION, DRAINAGE, EMBANKMENT, SHIELDING OR SLOTTED CURB CONSTRUCTION SHALL BE DONE BY THE CONTRACTOR AND NOT TO EXCEED THE LIMITS TO THE ELEVATIONS SHOWN ON THE PLANS AND TO BESE THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY REPAIRS TO THE EXISTING UTILITIES. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE ENGINEER PRIOR TO CONSTRUCTION OF ANY MAJOR DISCHARGES.
- ALL STANDING CURBS SHALL BE LEFT UNCHANGED. THE CONTRACTOR SHALL ONLY THE ENGINEER TO REFERENCE AND REPAIR ANY CURBS. POINTS THAT HAVE TO BE DISTURBED. THE CONTRACTOR SHALL BE CHARGED FOR ANY REPAIRS TO ANY CURBS. POINTS THAT HAVE TO BE DISTURBED.
- ANY QUESTIONS RAISED RELATIVE TO THE ACCURACY OF ANY IMPROVEMENT INSTALLATION SHALL NOT BE RAISED SUBSEQUENT TO COMPLETION OF THE WORK UNLESS ALL SURVEY STAKES ARE MAINTAINED INTACT. SHALL SUCH STAKES NOT BE PRESENT AND REPAIRS AS TO THEIR QUALITY. NO CLAIM FOR ADDITIONAL COMPENSATION FOR CORRECTION SHALL BE PRESENTED TO ANY PARTY AND SUCH WORK SHALL BE CORRECTED BY THE CONTRACTOR AT HIS EXPENSE.
- ALL BUILDING ROOFS SHALL BE DRAINED BY INTERNAL UNDERGROUND ROOF DRAIN LEADERS.
- PROVIDE 6% SLOPE AWAY FROM WALK OR CURB FOR 5' FEET ALONG ALL STREETS.

LEGAL DESCRIPTION

PARCEL NO. 1:
A PORTION OF THE WEST HALF OF THE SOUTHEAST QUARTER OF SECTION 22, TOWNSHIP 3 NORTH, RANGE 5 EAST OF THE GLA AND SALT RIVER BASE AND MERIDIAN, MARICOPA COUNTY, ARIZONA, DESCRIBED AS FOLLOWS:

COMMENCING AT THE SOUTH QUARTER CORNER OF SAID SECTION 22; THENCE NORTH 71° 15' 30" E. ALONG THE NORTH-SOUTH 1/4-SECTION LINE OF SAID SECTION 22, A DISTANCE OF 64.94 FEET;

THENCE NORTH 71° 15' 30" E. ALONG THE NORTH-SOUTH 1/4-SECTION LINE OF SAID SECTION 22, A DISTANCE OF 30.00 FEET TO THE POINT OF BEGINNING;

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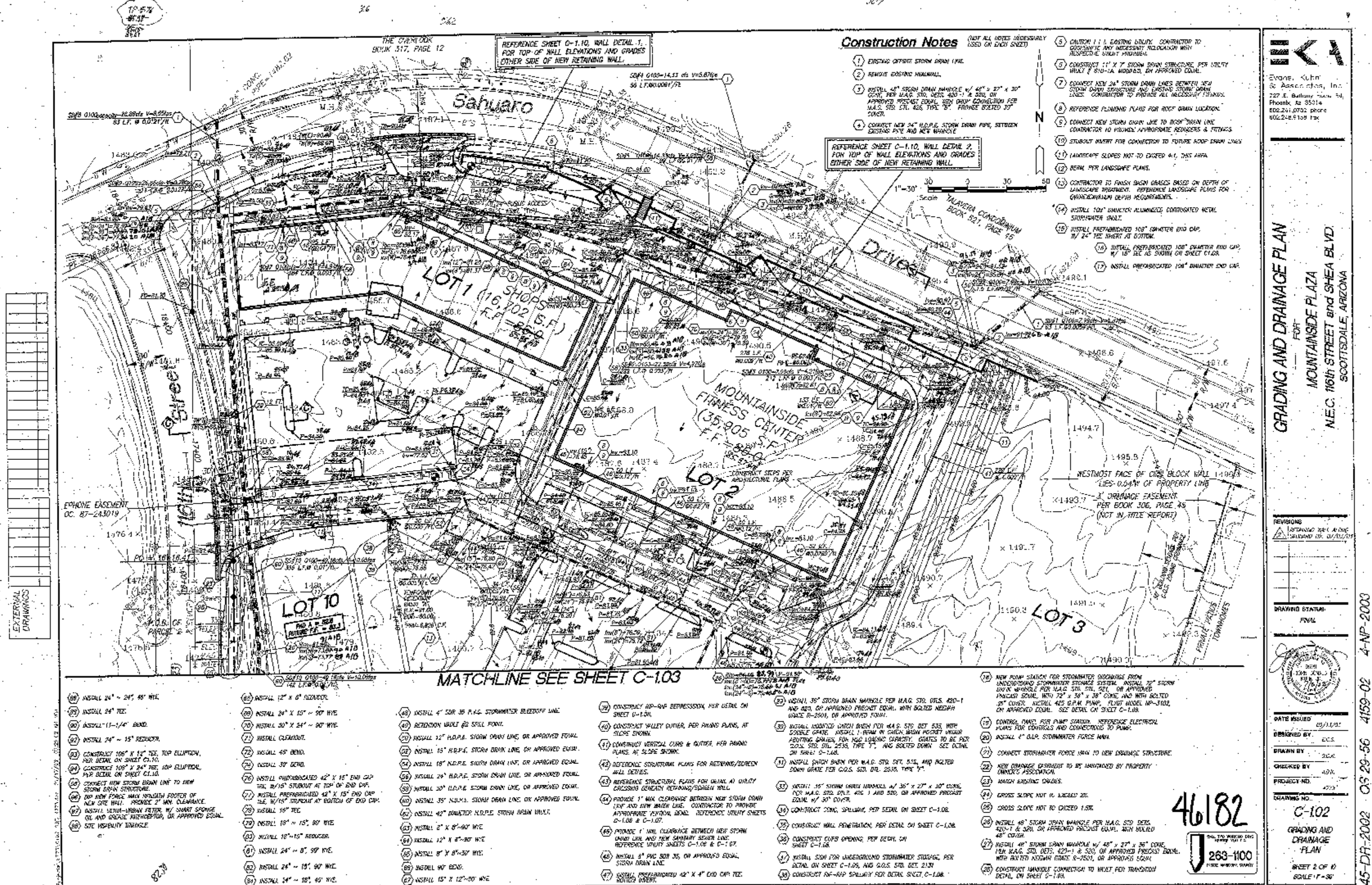
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SHEET INDEX

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- ### Construction Notes
- (NOT ALL NOTES NECESSARILY USED ON EACH SHEET)
- 1. EXISTING OFFSITE STORM DRAIN LINE
 - 2. REMOVE EXISTING MAINWALL
 - 3. INSTALL 48" STORM DRAIN MAINWALL W/ 48" x 27" x 30" CONE PER M.A.G. STD. DETS. 420-1 & 520, ON APPROVED PRECAST EQUAL WITH BOLTED DOWN 30" COVER. PROVIDE BOLTED 30" COVER.
 - 4. CONNECT NEW 24" H.D.P.E. STORM DRAIN PIPE, SETBACK EXISTING PIPE AND NEW MAINWALL
 - 5. CAUTION !!! EXISTING UTILITY. CONTRACTOR TO DISCOVER ANY NECESSARY RELOCATION WITH RESPECTIVE UTILITY PROVIDER.
 - 6. CONSTRUCT 11" x 7" STORM DRAIN STRUCTURE PER UTILITY WHAT IS 810-1A, MODIFIED, OR APPROVED EQUAL.
 - 7. CONNECT NEW 24" STORM DRAIN LINES BETWEEN NEW STORM DRAIN STRUCTURE AND EXISTING STORM DRAIN LINES. CONTRACTOR TO PROVIDE ALL NECESSARY FITTINGS.
 - 8. REFERENCE PLUMBING PLANS FOR ROOF DRAIN LOCATION.
 - 9. CONNECT NEW STORM DRAIN LINE TO ROOF DRAIN LINE. CONTRACTOR TO PROVIDE APPROPRIATE REDUCERS & FITTINGS.
 - 10. STUBOUT INVERT FOR CONNECTION TO FUTURE ROOF DRAIN LINES.
 - 11. LANDSCAPE SLOPES NOT TO EXCEED 4:1, THIS AREA.
 - 12. BERM PER LANDSCAPE PLANS.
 - 13. CONTRACTOR TO FINISH BASH GRASSES BASED ON DEPTH OF LANDSCAPE TREATMENT. REFERENCE LANDSCAPE PLANS FOR GRASS/SEDIMENTATION DEPTH REQUIREMENTS.
 - 14. INSTALL 104" DIAMETER ALLOWANCE, CONTINGENT METAL STORMWATER INLET.
 - 15. INSTALL PREFABRICATED 108" DIAMETER END CAP, W/ 24" TEE INVERT AT BOTTOM.
 - 16. INSTALL PREFABRICATED 108" DIAMETER END CAP, W/ 18" TEE AS SHOWN ON SHEET C-102.
 - 17. INSTALL PREFABRICATED 108" DIAMETER END CAP.

- EXTERNAL DRAWINGS
- LOT 1 (16 SHOPS, 102 S.F.)
- LOT 2 (35,905 S.F.)
- LOT 3
- LOT 10
- MATCHLINE SEE SHEET C-103
- 46182
- 263-1100
- 45-8-2017 1-21-2017
- 10-UP-2019
09/26/2019

Evans, Kuhn & Associates, Inc.
227 E. Balfour Street, Suite 101
Phoenix, AZ 85014
602.241.0732 phone
602.241.0733 fax

GRADING AND DRAINAGE PLAN

FOR

MOUNTAINSIDE PLAZA

N.E.C. 116th STREET and SHEA BLVD.

SCOTTSDALE, ARIZONA

REVISIONS

NO.	DATE	DESCRIPTION
1	05/11/02	ISSUING SET AND
2	05/11/02	REVISIONS

DRAWING STATUS:

FINAL

DATE ISSUED: 05/11/02

DESIGNED BY: ECS

DRAWN BY: ECS

CHECKED BY: ECS

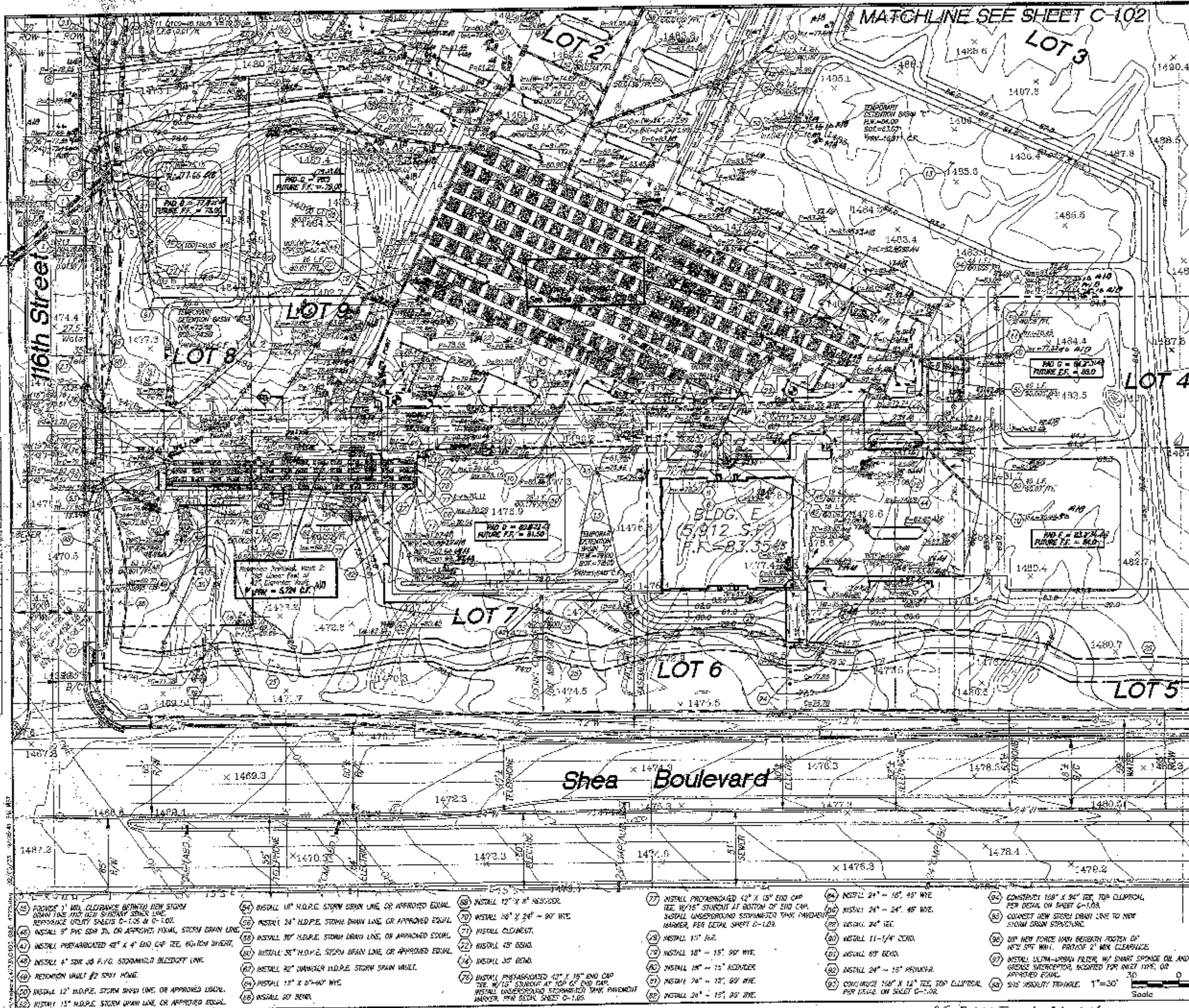
PROJECT NO: 4770

DRAWING NO: C-102

GRADING AND DRAINAGE PLAN

SHEET 2 OF 10

SCALE: 1" = 30'



- ### Construction Notes
- (NOT ALL NOTES NECESSARILY USED ON EACH SHEET)
- EXISTING OFFSITE STORM DRAIN LINE.
 - RETAIN EXISTING HEADWALL.
 - INSTALL 48" STORM DRAIN MANHOLE W/ 48" x 27" x 36" CONE, PER M.A.S. STD. DET. 420-1 & 320, CH. APPROVED PRECAST EQUAL, WITH DRAIN CONNECTION PER M.A.S. STD. DET. 428, TYPE "T". PROVIDE SLOTTED JY COVER.
 - CONNECT NEW 24" H.D.P.E. STORM DRAIN PIPE BETWEEN EXISTING PIPE AND NEW MANHOLE.
 - CONDUCT I.I. EXISTING UTILITY, CONTRACTOR TO COORDINATE ANY NECESSARY RELOCATION WITH NEARBY UTILITY PROVIDER.
 - CONSTRUCT 11" x 7" STORM DRAIN STRUCTURE PER UTILITY W/ 11" x 7" LA, MODIFIED OR APPROVED EQUAL COVER TO BE WEATHERED AND GATED SHUT.
 - CONNECT NEW 24" STORM DRAIN LINES BETWEEN NEW STORM DRAIN STRUCTURE AND EXISTING STORM DRAIN LINES. CONTRACTOR TO PROVIDE ALL NECESSARY FITTINGS.
 - MAINTAIN EXISTING PLANS FOR NEW DRAIN LOCATION.
 - CONNECT NEW STORM DRAIN LINE TO ROOF DRAIN LINE. CONTRACTOR TO PROVIDE APPROPRIATE REDUCERS & FITTINGS.
 - STAKEOUT MARK FOR CONNECTION TO FUTURE ROOF DRAIN LINES.
 - LANDSCAPE SLOPES NOT TO EXCEED 4:3, THIS AREA.
 - DETA. PER LANDSCAPE PLANS.
 - CONTRACTOR TO FINISH DRAIN GRADES BASED ON DEPTH OF LANDSCAPE TREATMENT. REFER TO LANDSCAPE PLANS FOR OVERLAP/UNDERLAP DEPTH REQUIREMENTS.
 - INSTALL 108" DIAMETER UNARMED CORRUGATED METAL STORM DRAIN MANHOLE. SEE DETAILS ON SHEET C-103.
 - INSTALL PRECAST/BOULDER 108" DIAMETER END CAP, W/ 24" I.D. INVERT AT BOTTOM.
 - INSTALL MANHOLE/BOULDER 108" DIAMETER END CAP, W/ 18" TEE AT BOTTOM ON SHEET C-103.
 - INSTALL PRECAST/BOULDER 108" DIAMETER END CAP.
 - NEW PUMP STATION FOR STORMWATER DISCHARGE FROM UNDERGROUND STORMWATER STORAGE SYSTEM. INSTALL 72" STORM DRAIN MANHOLE PER M.A.S. STD. DET. 321, CH. APPROVED PRECAST EQUAL, WITH 72" x 30" x 36" CONE, AND WITH SLOTTED 36" COVER. INSTALL 425 G.P.M. PUMP, FLIGHT MODEL HP-3102 OR APPROVED EQUAL. SEE DETAIL ON SHEET C-103.
 - CONTROL PANEL FOR PUMP STATION. REFERENCE ELECTRICAL PLANS FOR CONTROLS AND CONNECTIONS TO PUMP.
 - INSTALL 4" G.P.I. STORMWATER FORCE MAIN.
 - CONSTRUCT STORMWATER FORCE MAIN TO NEW UPGRADE STRUCTURE.
 - NEW DRAINAGE BASIN.
 - MATCH EXISTING GRADES.
 - GRDES SLOPE NOT TO EXCEED 2%.
 - CROSS SLOPE NOT TO EXCEED 1.5%.
 - INSTALL 48" STORM DRAIN MANHOLE/PIPE W/ 48" x 27" x 36" CONE, PER M.A.S. STD. DET. 420-1 & 320, CH. APPROVED PRECAST EQUAL, WITH SLOTTED 48" COVER FOR HEADWALL IN-BORE, ON APPROVED EQUAL.
 - INSTALL 48" STORM DRAIN MANHOLE W/ 48" x 27" x 36" CONE, PER M.A.S. STD. DET. 420-1 & 320, CH. APPROVED PRECAST EQUAL, WITH SLOTTED 48" GRATE R-2501, OR APPROVED EQUAL.
 - CONSTRUCT MANHOLE CONNECTION TO VALVE PER THAMSON DETAIL, ON SHEET C-103.
 - INSTALL 36" STORM DRAIN MANHOLE PER M.A.S. STD. DET. 420-1 AND 320, CH. APPROVED PRECAST EQUAL, WITH SLOTTED 36" GRATE R-2501, OR APPROVED EQUAL.
 - INSTALL MANHOLE/BOULDER CATCH BASIN PER M.A.S. STD. DET. 335, WITH DOUBLE GRATE. INSTALL 1'-6" x 1'-6" x 1'-6" CATCH BASIN ROCKET. RAINING GRATES, FOR 100% CATCHING CAPACITY. GRATES TO BE PER C.O.S. STD. DET. 353, TYPE "T", AND SLOTTED DOWN. SEE DETAIL ON SHEET C-103.
 - INSTALL CATCH BASIN PER M.A.S. STD. DET. 335, AND BUILD DOWN GRATE C.O.S. STD. DET. 2035, TYPE "T".
 - CONSTRUCT NEW CATCH BASIN TO INLET, PER DETAIL ON SHEET C-103.
 - NOT USED.
 - CONSTRUCT CONC. SALLYWAY, PER DETAIL ON SHEET C-103.
 - CONSTRUCT WALL PENETRATION, PER DETAIL ON SHEET C-103.
 - CONSTRUCT CURB OPENING, PER DETAIL ON SHEET C-103.
 - NOT USED.
 - CONSTRUCT RP-RAP SPRINKLER PER DETAIL ON SHEET C-103.
 - CONSTRUCT RP-RAP DEPRESSION, PER DETAIL ON SHEET C-103.
 - CONSTRUCT VALLEY CUTTER, PER RAVING PLANS, AT SLOPE SHOWN.
 - CONSTRUCT VERTICAL CURB & GUTTER, PER RAVING PLANS, AT SLOPE SHOWN.
 - REFERENCE STRUCTURAL PLANS FOR RETAINING/SCREEN WALL DETAILS.
 - REFERENCE STRUCTURAL PLANS FOR DETAIL AT UTILITY CROSSING BENEATH RETAINING/SCREEN WALL.
 - PROVIDE 1' MIN. CLEARANCE BETWEEN NEW STORM DRAIN LINE AND NEW WATER LINE. CONTRACTOR TO PROVIDE APPROPRIATE VERTICAL SEED. REFERENCE UTILITY SHEETS C-108 & C-107.

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GRADING AND DRAINAGE PLAN

FOR

MOUNTAINSIDE PLAZA

N.E.C. 16th STREET and SHEA BLVD.

SCOTTSDALE, ARIZONA

46183

PROVISIONS

DRAWING STATUS:

FINAL SUBMITTAL

DATE REVISED: 2/12/24

DESIGNED BY: H.P.R./E.C.S.

DRAWN BY: H.A.C.

CHECKED BY: H.P.R.

PROJECT NO.: 1/13

DRAWING NO.: **C-103**

GRADING AND DRAINAGE PLAN

SHEET 3 OF 10

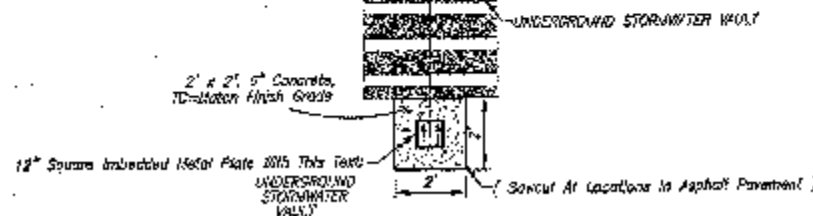
SCALE: 1" = 30'

A5-3014T 1-21-04

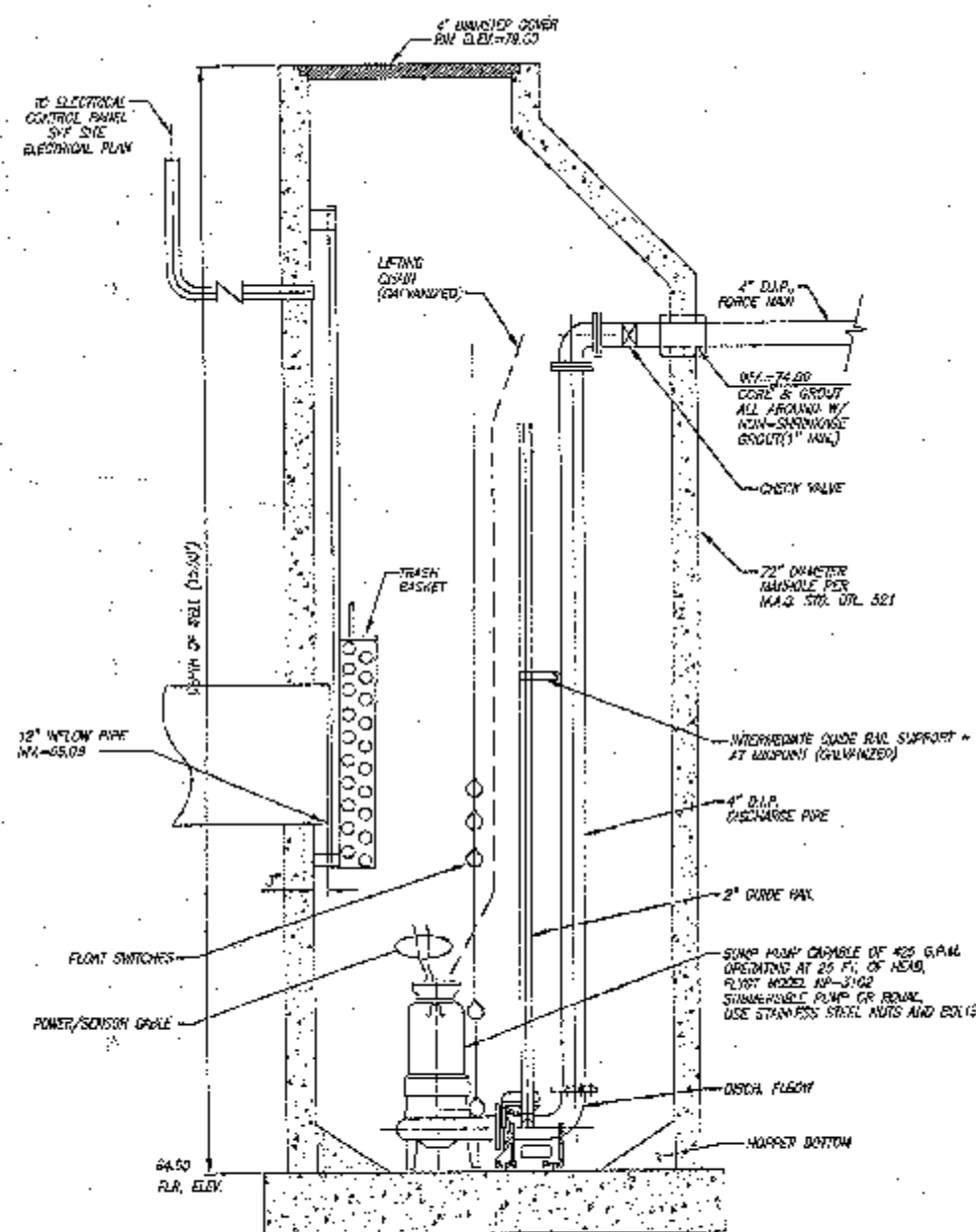
45-DR-2002 03-29-56 4150-02

PUMP CONTROL PANEL REQUIREMENTS

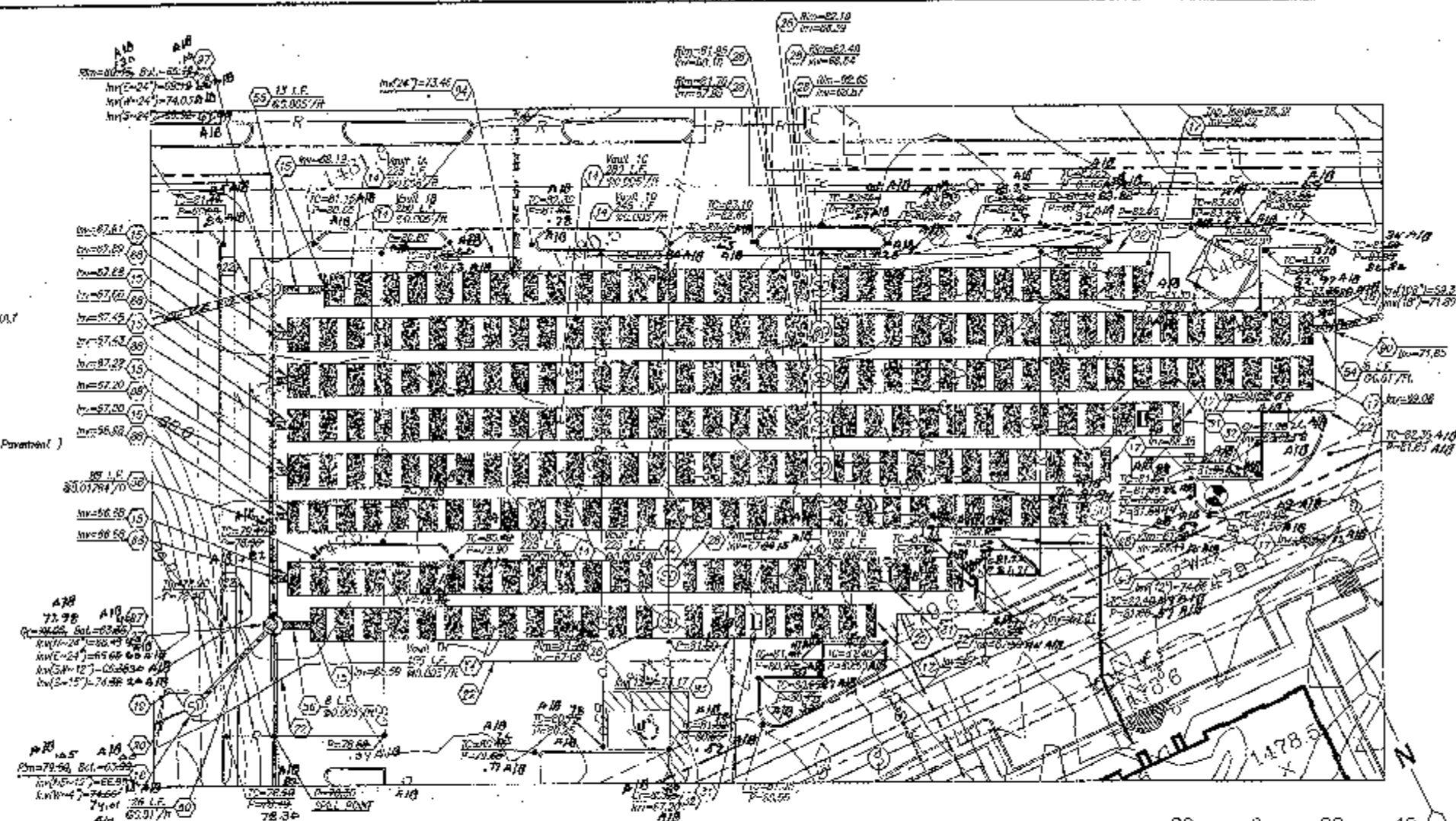
1. OF. BE CONTROLS SIMPLEX PUMP CONTROL PANEL RATED 600, 120/240V WITH THE FOLLOWING FEATURES: NEAR BY GALVANIZED PAINTED ENCLOSURE, BRASS ALUMINUM FRONT DOOR, PUMP CIRCUIT BREAKER, NEAR RATED MINOR STARTER, CONTROL CIRCUIT TRANSFORMER, RUN/STOP, LIGHT, HAND-OFF-AUTO SWITCH, STOP, LEAKAGE ALARM CIRCUIT, RUN/STOP LIGHT (TO DISBURG PUMP IF SEAL LEAKAGE CONDITION OCCURS), HIGH WATER ALARM LIGHT WITH FLASHER, (ONE CLOSING DESIGNED FOR TWO HOSE THE OTHER ONE SHUT FLAT IS MAGE.



**STORMWATER VAULT
PAVEMENT MARKER DETAIL**
NTS



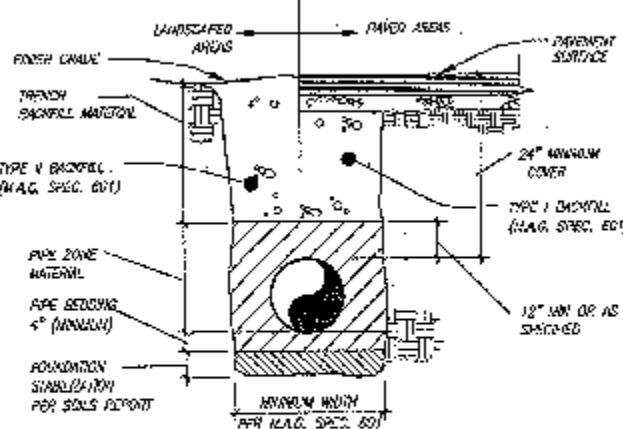
**STORMWATER VAULT SYSTEM #1
DISCHARGE PUMP STATION DETAIL**
NTS



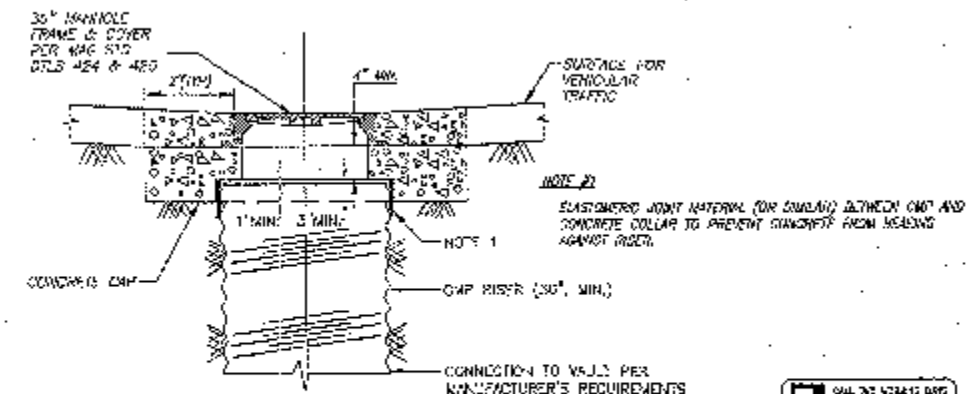
RETENTION VAULT SYSTEM #1 DETAIL
Scale 1\"/>

- * CONTRACTOR NOTE
- 1. 10\"/>

- 1. NEW PUMP STATION FOR STORMWATER DISCHARGE FROM UNDERGROUND STORMWATER SYSTEM. INSTALL 7\"/>



TYPICAL PVC/HDPE STORM DRAIN TRENCH DETAIL
NTS



VAULT ACCESS DETAIL
NTS

46190

EKA
Evans, Kuhn
& Associates, Inc.
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Phoenix, AZ 85014
602.941.0797 ext. 400
602.248.9158 fax

STORMWATER VAULT SYSTEM DETAILS
FOR
MOUNTAINSIDE PLAZA
N.E.C. 16th STREET and SHEA BLVD.
SCOTTSDALE, ARIZONA

REVISIONS

DRAWING STATUS
FINAL SUBMITTAL



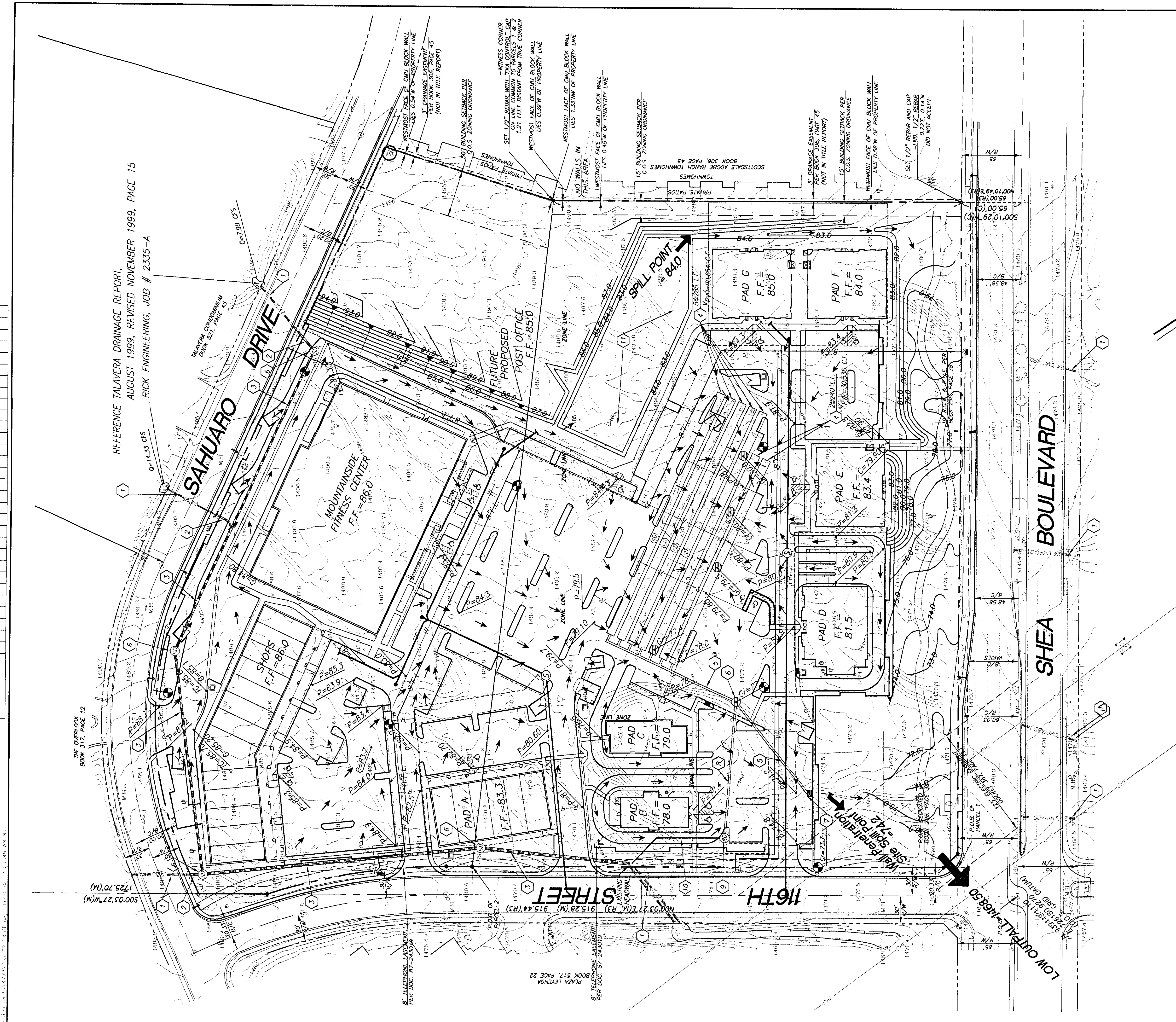
DATE ISSUED 1/2/03
DESIGNED BY M.R./R.S.
DRAWN BY R.V.C.
CHECKED BY M.R.
PROJECT NO. 4723
DRAWING NO.

C-109
STORMWATER
VAULT
SYSTEM
DETAILS
SHEET 9 OF 10
SCALE 1\"/>

45-DR-2002 05 29-56 4159-02 4-NP-2003

10-UP-2019
09/26/2019

EXTERNAL
DRAWINGS

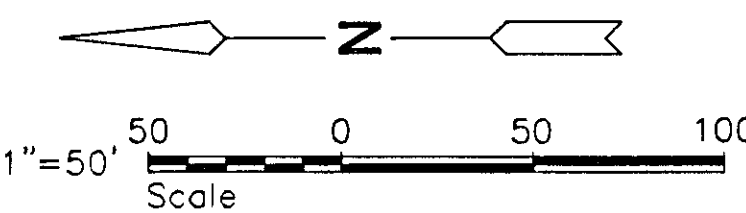


DRAINAGE CALCULATIONS

Retention Required, Undeveloped Northeast Landscape Area Site:
V REQ = $6/12 \times A \times C$
A = 3.1416 x 285 x 5 = 90,654 C.F.
V REQ = $(2.82/12) \times 572,992 \times 0.90$
V REQ = 12,189 C.F.
Retention Provided:
Total, 9' Diameter Retention Vaults @ 285 L.F. = 90,654 C.F.
(4.5)' x 3.1416 x 285 x 5 = 90,654 C.F.
(2) 9' Diameter Retention Vaults @ 240 L.F.
(4.5)' x 3.1416 x 240 x 2 = 30,536 C.F.
V FRY = 12,189 C.F.
Retention Required, Undeveloped Northeast Landscape Area Site:
V REQ = $(2.82/12) \times 90,000 \times 0.50$
V REQ = 1,080 C.F.
Retention Provided, Landscape Basin:
V FRY = 1,080 C.F.

NOTES:

- EXISTING OFFSITE STORM DRAIN LINE.
- CONNECT OFFSITE STORM DRAIN LINE TO NEW STORM DRAIN LINE.
- NEW STORM DRAIN LINE FOR OFFSITE FLOWS.
- NEW 9" DIAMETER STORMWATER RETENTION VAULT.
- NEW STORM DRAIN LINE FOR ONSITE.
- NEW STORM DRAIN MANHOLE WITH GRATE.
- NEW CATCH BASIN.
- NEW STORM DRAIN LINE TO STORMWATER PUMP DISCHARGE.
- NEW STORM DRAIN MANHOLE WITH STORMWATER DISCHARGE PUMP.
- NEW FORCE MAIN.
- 1" DEEP LANDSCAPE RUNOFF DETENTION BASIN.



VICINITY MAP
NO SCALE

PRELIMINARY

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CONCEPTUAL GRADING & DRAINAGE PLAN
FOR
RETAIL CENTER - 116TH ST. & SHEA

DRAWING STATUS
EXHIBIT

DATE ISSUED 05/11/00
DESIGNED BY E.C.S.
DRAWN BY R.A.C./C
CHECKED BY W.P.R.
PROJECT NO. 4773
DRAWING NO. Conceptual Grading and Drainage Plan

Scale: 1" = 50'



Evans, Kuhn &
Associates, Inc.

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DRAINAGE REPORT
For
116TH STREET & SHEA RETAIL CENTER
SCOTTSDALE, ARIZONA

Prepared for:

CITY OF SCOTTSDALE
7447 East Indian School Road
Scottsdale, Arizona 85251

Prepared by:

EVANS, KUHN AND ASSOCIATES, INC.
727 East Bethany Home Road, Suite D-225
Phoenix, Arizona 85014



EKA #4773
December 11, 2001
Revised May 9, 2002

253-PA-2001
6/4/2002

engineers

45-DR-2002

10-UP-2019

09/26/2019

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Appendix A: Concept Grading and Drainage Plan	



Project Description

The existing 13.15-acre site is currently vacant and occupies the northeast corner of 116th Street and Shea Boulevard. Residential properties currently existing on both the north and east, and a commercial warehouse storage facility occupies the property to the west. The proposed development consists of seven individual commercial pads, a pad with retail shops, a Mountainside Fitness Center, and a future proposed Post Office Facility.

Existing Conditions

The existing site slopes to the southwest at an average slope of 3%; losing almost 30' of elevation between the site high and low points. Three existing 24" pipe culverts along the northern boundary empty retention basins from upstream developments onto the site. These off-site flows following existing drainageways through the site, with flows exiting the site via two existing 28" pipe culverts under 116th Street near the southwest corner of the site. The remaining site flows sheet flow or follow shallow drainage swales to ponding locations along the north side of Shea Boulevard. Storm drain crossings under Shea were terminated with the last widening project, so currently there is no outfall for the ponded water in shallow basins.

Off-site Drainage Flows

The two easternmost 24" storm drains under Sahuaro Drive drain retention basins for the Talavera Condominiums which abut the site to the north. From the grading and drainage plans for the development, these 100-year pipe flows are shown to be: 7.99 cfs, and 14.33 cfs. The westernmost 24" storm drain under Sahuaro Drive drain a retention basin for the Overlook Condominiums. Record drawings for this older development were not available, but site observation indicates it drains a retention basin, similar to the other pipes on Sahuaro Drive. With a similar sloping pipe, it has been assumed that the 100-year pipe flow would not be in excess of 15 cfs. The three off-site pipe flows are proposed to be collected in a new underground pipe and directed to the 28" outfall pipes under 116th Street as currently occurs.

On-site Grading and Drainage

The proposed grading concept essentially maintains existing flow patterns. Roof drainage will be primarily by above ground splash blocks to surface drain to new catch basins located throughout

the paved parking areas within the development. ADA grades are maintained within the accessible areas and grades not exceeding 4% used elsewhere on the site. These slopes were not conducive to developing retention basin, therefore an underground storage system was utilized for site runoff for the 100-year, 2-hour storm. A stormwater discharge pump will be used to empty the storage vaults within 36 hours through the 116th Street underground pipe culverts.

The future Post Office Facility in the northeast portion of the site will remain undisturbed, except for the grading/berming necessary to create a temporary retention basin for its runoff. Upon development, flows will be directed into the underground vault system, which has been sized for future flows. Reference Appendix A for the Concept Grading and Drainage Exhibit and retention calculations.

Summary

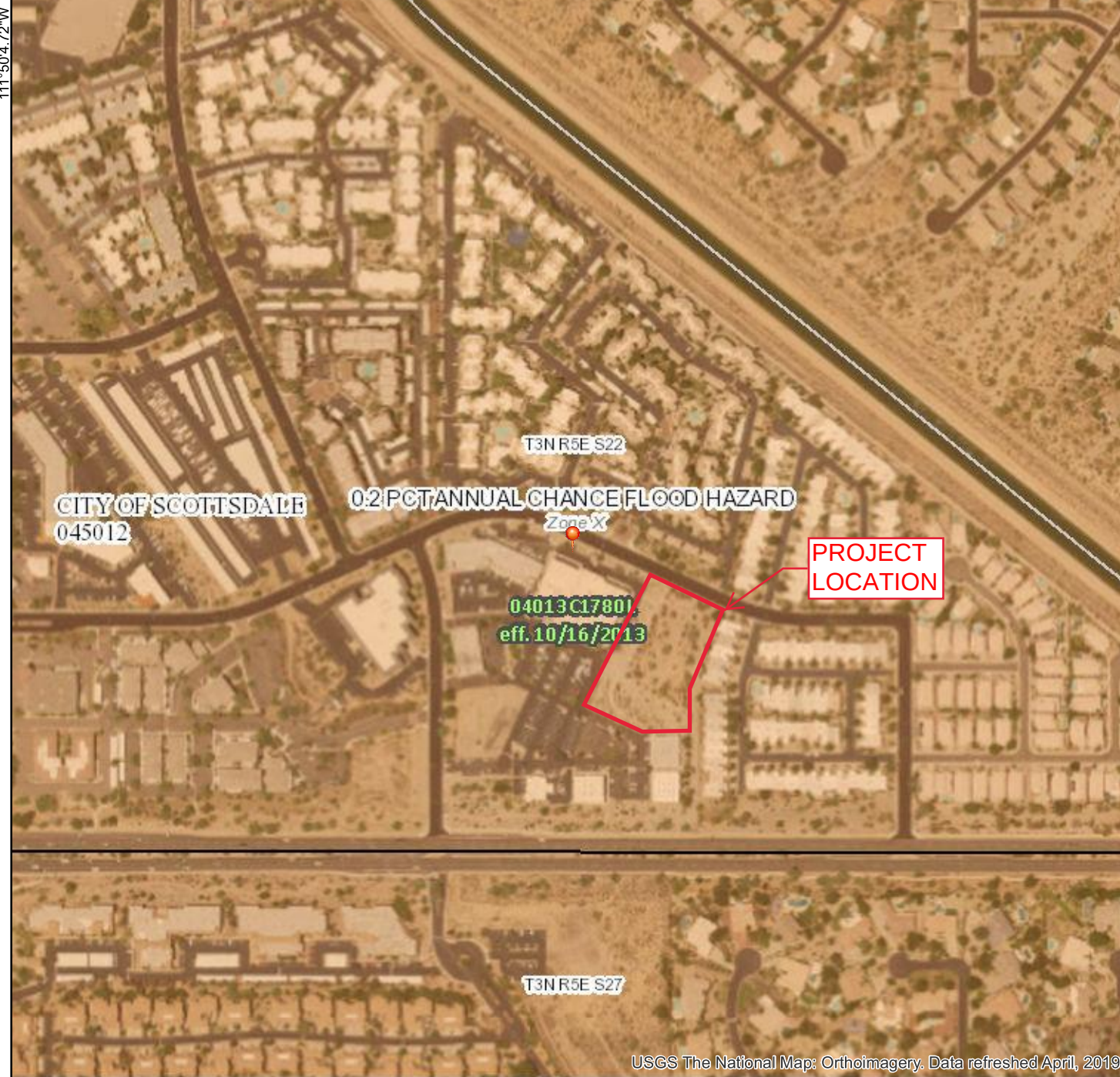
Existing storm water flows will be maintained after site development. Off-site flows intercepted by the property will be rerouted through the site. Underground stormwater storage vaults provide 121,190 cf of storage, and exceed the 100-year, 2-hour retention requirement of 121,188 cf. The building finished floor elevations are all elevated sufficiently above the high top of curb along the abutting streets and well above the low outfall elevation of 1268.5, just northeast of the intersection of 116th Street and Shea Boulevard.

APPENDIX C
(FEMA FIRM Map)

National Flood Hazard Layer FIRMette



33°35'20.51"N



0 250 500 1,000 1,500 2,000 Feet

1:6,000

33°34'50.54"N

USGS The National Map: Orthoimagery. Data refreshed April, 2019.

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
		Area of Undetermined Flood Hazard Zone D
GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance
		17.5 Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
MAP PANELS		Digital Data Available
		No Digital Data Available
		Unmapped



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **8/21/2019 at 11:43:46 AM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

10-UP-2019
09/26/2019

APPENDIX D
(Sahuaro Drive Capacity Calculations)

Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Tuesday, Aug 20 2019

East Sahuaro Drive Capacity

User-defined

Invert Elev (ft) = 1496.59
Slope (%) = 0.78
N-Value = 0.013

Calculations

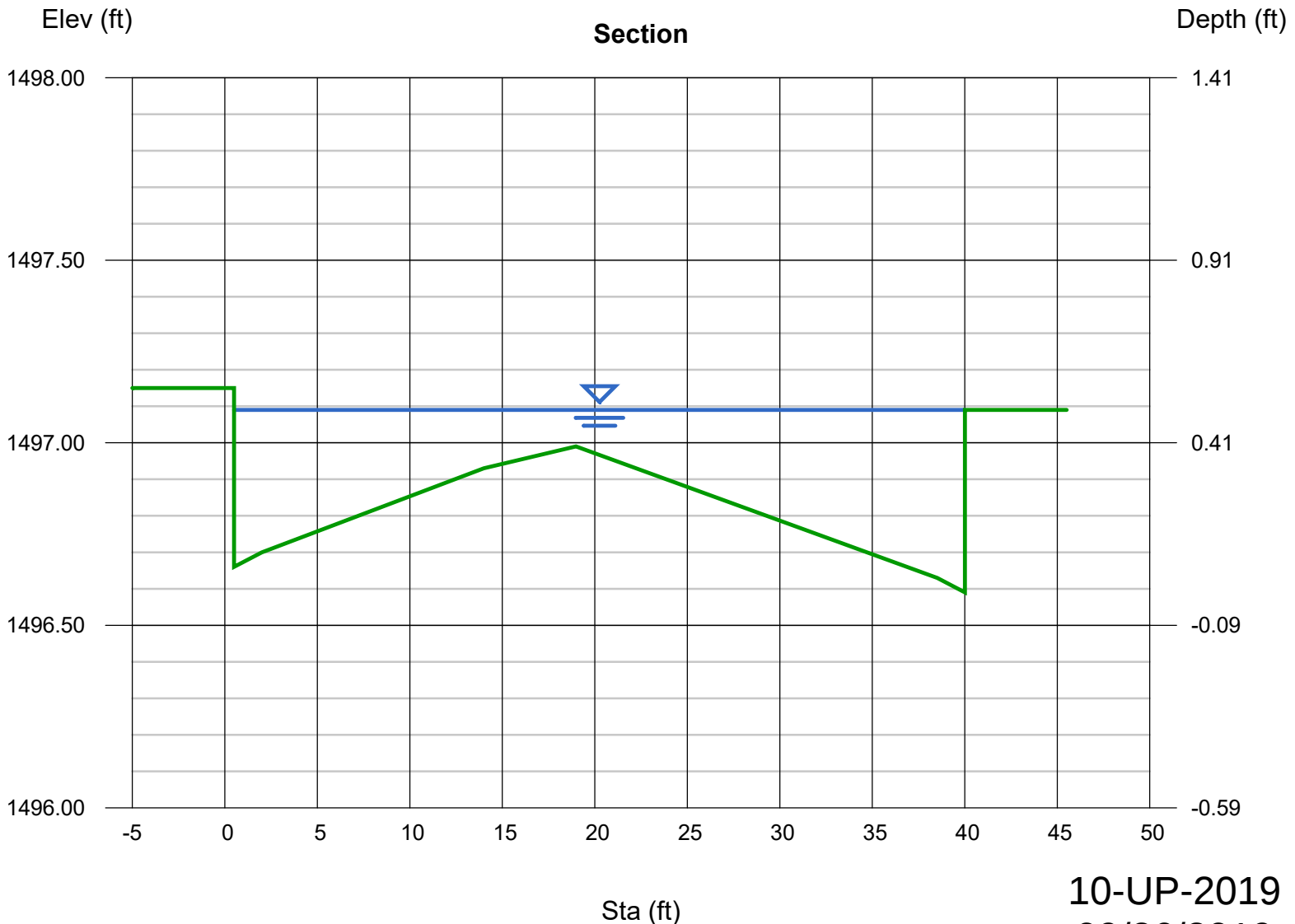
Compute by: Known Depth
Known Depth (ft) = 0.50

Highlighted

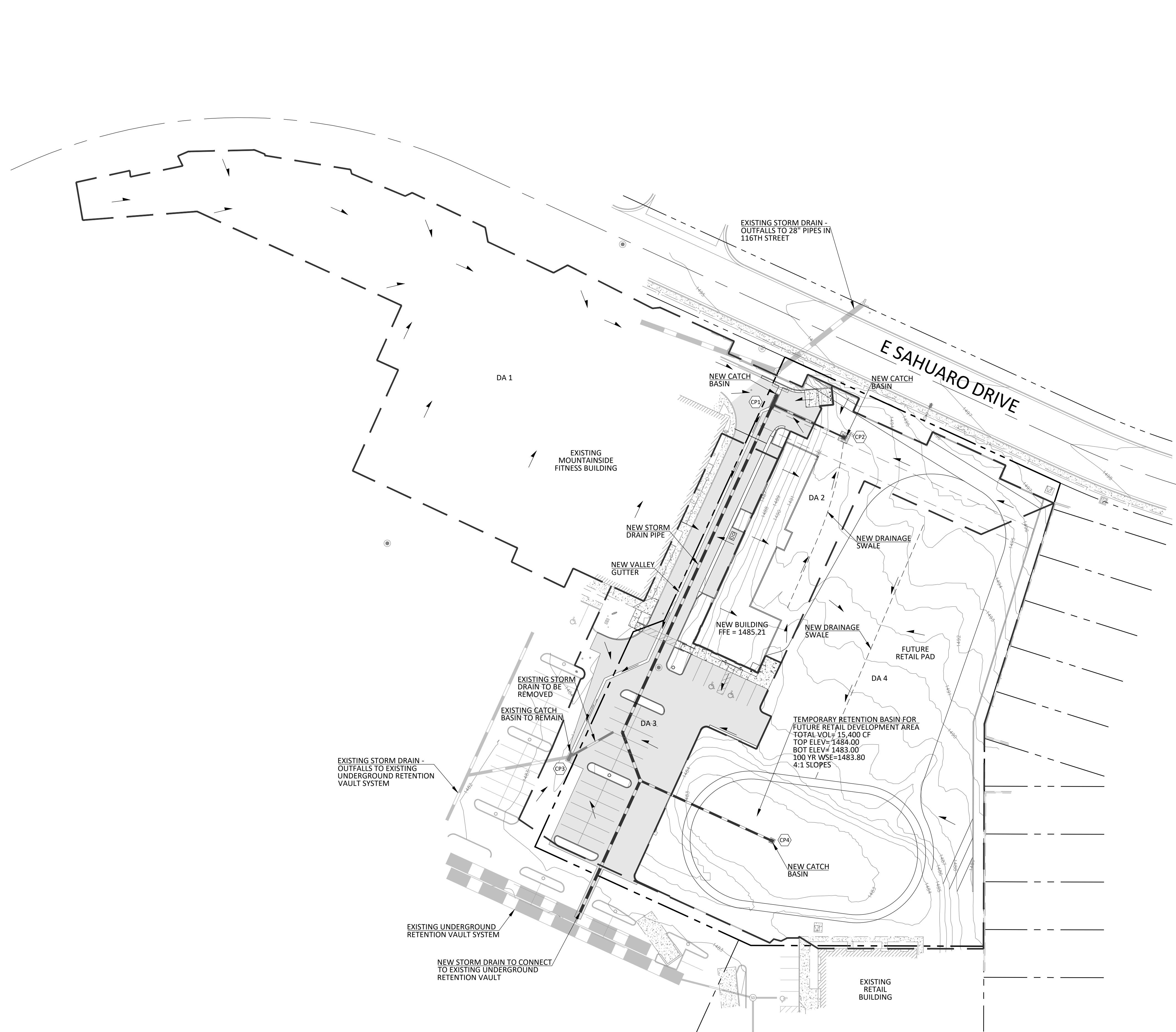
Depth (ft) = 0.50
Q (cfs) = 44.94
Area (sqft) = 10.75
Velocity (ft/s) = 4.18
Wetted Perim (ft) = 40.45
Crit Depth, Yc (ft) = 0.56
Top Width (ft) = 39.51
EGL (ft) = 0.77

(Sta, El, n)-(Sta, El, n)...

(0.00, 1497.15)-(0.50, 1497.15, 0.013)-(0.50, 1496.66, 0.013)-(2.00, 1496.70, 0.013)-(14.00, 1496.93, 0.013)-(18.98, 1496.99, 0.013)-(38.51, 1496.63, 0.013)
-(40.01, 1496.59, 0.013)-(40.01, 1497.09, 0.013)-(40.51, 1497.09, 0.013)



APPENDIX E
(Proposed Conditions Watershed Maps + Calculations)



SCALE 1"=40'

LEGEND

RIGHT-OF-WAY

PROJECT BOUNDARY LINE

ROADWAY CENTERLINE

FLOW ARROW

WATERSHED BOUNDARY

CONCENTRATION POINT

WATERSHED CONCENTRATION POINTS		
	AREA	Q 100
CP1	1.63 AC	11.5 CFS
CP2	0.44 AC	2.8 CFS
CP3	0.64 AC	4.6 CFS
CP4	1.47 AC	10.4 CFS

CYPRESS

CIVIL DEVELOPMENT

strength + sustainability

4450 north 12th street, #228

phoenix, arizona 85014

p: 623.282.2498

e: jphunt@cypresscivil.com

CYPRESS PROJECT NO: 19.101

KERRY'S CAR CARE SHEA
proposed conditions
hydrology map

PROPOSED WATERSHED SUMMARY

PROJECT KERRY'S CAR CARE SHEA

PROPOSED DEVELOPMENT AREA

WATERSHED ID	CONCENTRATION POINT	AREA ROOF + PAVEMENT C=0.95 (SF)	AREA DESERT LANDSCAPING C=0.50 (SF)	FUTURE PAD AREA C=0.90	TOTAL AREA (AC)	WEIGHTED 'C'
DA 1	CP1	62,920	8,000	0	1.63	0.90
DA 2	CP2	7,252	6,128	5,814	0.44	0.79
DA 3	CP3	25,322	2,349	0	0.64	0.91
DA 4	CP4	0	0	64,066	1.47	0.90

RATIONAL METHOD

$$T_c = 11.4L^{0.5}K_b^{0.52}S^{-0.31}i^{-0.38} \times 60$$

Tc= Time of Concentration (min)
 L= Length of longest flow path (miles)
 Kb= Watershed resistance coefficient
 S= Watercourse slope (ft/mi)
 i= rainfall intensity (in/hr)

$$Q = CiA$$

Q = Peak discharge (cfs) 100-YR, 5-MIN 7.82 in/hr
 C = Runoff coefficient 100-YR, 10-MIN 5.96 in/hr
 i = Rainfall intensity (inch/hr) 100-YR, 15-MIN 4.92 in/hr
 A = Drainage area (Acres) 100-YR, 30-MIN 3.33 in/hr

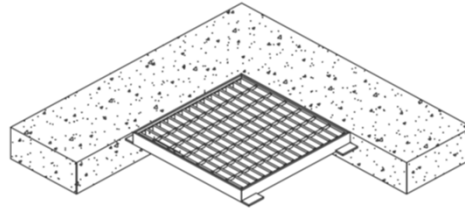
KERRY'S CAR CARE SHEA

WATERSHED ID	CONCENTRATION POINT	L	Kb	S		i	Tc	C	i	A	Q
DA-1	1	0.03	0.04	55	1.0%	7.82	3.1	0.90	7.82	1.63	11.5
DA-2	2	0.03	0.04	55	1.0%	7.82	3.1	0.79	7.82	0.44	2.8
DA-3	3	0.03	0.04	55	1.0%	7.82	3.1	0.91	7.82	0.64	4.6
DA-4	4	0.03	0.04	55	1.0%	7.82	3.1	0.90	7.82	1.47	10.4

INLET CALCULATION

PROJECT KERRY'S CAR CARE SHEA
LOCATION CP1

INLET TYPE MAG - 10'x2'
 LENGTH 10 FT
 WIDTH 2 FT
 OPEN AREA 15.09 SF
 Cw 3.00
 Co 0.67
 CLOGGING FACTOR 50%



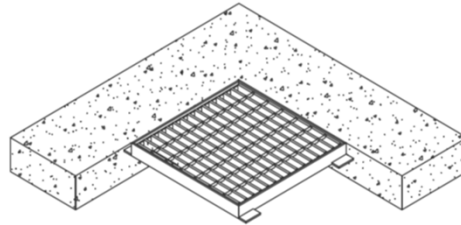
DEPTH (FT)	WEIR (CFS)	ORIFICE (CFS)	CONTROLLING (CFS)
0.00	0.0	0.0	0.0
0.05	0.4	9.1	0.4
0.10	1.1	12.8	1.1
0.15	2.1	15.7	2.1
0.20	3.2	18.1	3.2
0.25	4.5	20.3	4.5
0.30	5.9	22.2	5.9
0.35	7.5	24.0	7.5
0.40	9.1	25.7	9.1
0.45	10.9	27.2	10.9
0.50	12.7	28.7	12.7
0.55	14.7	30.1	14.7
0.60	16.7	31.4	16.7
0.65	18.9	32.7	18.9
0.70	21.1	34.0	21.1
0.75	23.4	35.1	23.4
0.80	25.8	36.3	25.8
0.85	28.2	37.4	28.2

100-YR FLOW 11.5 CFS
 CALCULATED DEPTH 0.50 FT

INLET CALCULATION

PROJECT KERRY'S CAR CARE SHEA
LOCATION CP2

INLET TYPE MAG - 2'x2'
 LENGTH 2 FT
 WIDTH 2 FT
 OPEN AREA 3.02 SF
 Cw 3.00
 Co 0.67
 CLOGGING FACTOR 50%



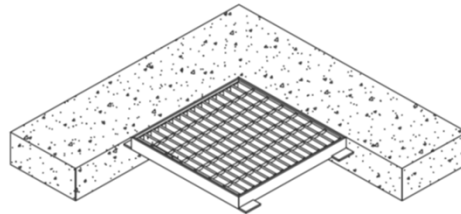
DEPTH (FT)	WEIR (CFS)	ORIFICE (CFS)	CONTROLLING (CFS)	
0.00	0.0	0.0	0.0	0.00
0.05	0.1	1.8	0.1	0.05
0.10	0.4	2.6	0.4	0.10
0.15	0.7	3.1	0.7	0.15
0.20	1.1	3.6	1.1	0.20
0.25	1.5	4.1	1.5	0.25
0.30	2.0	4.4	2.0	0.30
0.35	2.5	4.8	2.5	0.35
0.40	3.0	5.1	3.0	0.40
0.45	3.6	5.4	3.6	0.45
0.50	4.2	5.7	4.2	0.50
0.55	4.9	6.0	4.9	0.55
0.60	5.6	6.3	5.6	0.60
0.65	6.3	6.5	6.3	0.65
0.70	7.0	6.8	6.8	0.70
0.75	7.8	7.0	7.0	0.75
0.80	8.6	7.3	7.3	0.80
0.85	9.4	7.5	7.5	0.85

100-YR FLOW 2.8 CFS
 CALCULATED DEPTH 0.40 FT

INLET CALCULATION

PROJECT KERRY'S CAR CARE SHEA
LOCATION CP3 - EXISTING

INLET TYPE	MAG - 2'x6'	
LENGTH	6	FT
WIDTH	2	FT
OPEN AREA	9.06	SF
Cw	3.00	
Co	0.67	
CLOGGING FACTOR	50%	



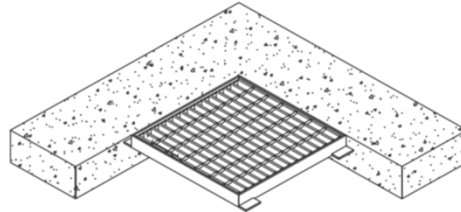
DEPTH (FT)	WEIR (CFS)	ORIFICE (CFS)	CONTROLLING (CFS)	
0.00	0.0	0.0	0.0	0.00
0.05	0.3	5.4	0.3	0.05
0.10	0.8	7.7	0.8	0.10
0.15	1.4	9.4	1.4	0.15
0.20	2.1	10.9	2.1	0.20
0.25	3.0	12.2	3.0	0.25
0.30	3.9	13.3	3.9	0.30
0.35	5.0	14.4	5.0	0.35
0.40	6.1	15.4	6.1	0.40
0.45	7.2	16.3	7.2	0.45
0.50	8.5	17.2	8.5	0.50
0.55	9.8	18.1	9.8	0.55
0.60	11.2	18.9	11.2	0.60
0.65	12.6	19.6	12.6	0.65
0.70	14.1	20.4	14.1	0.70
0.75	15.6	21.1	15.6	0.75
0.80	17.2	21.8	17.2	0.80
0.85	18.8	22.4	18.8	0.85

100-YR FLOW	4.6	CFS
CALCULATED DEPTH	0.35	FT

INLET CALCULATION

PROJECT KERRY'S CAR CARE SHEA
LOCATION CP4

INLET TYPE MAG - 2'x3'
 LENGTH 2 FT
 WIDTH 3 FT
 OPEN AREA 4.53 SF
 Cw 3.00
 Co 0.67
 CLOGGING FACTOR 50%



DEPTH (FT)	WEIR (CFS)	ORIFICE (CFS)	CONTROLLING (CFS)	
0.00	0.0	0.0	0.0	0.00
0.05	0.2	2.7	0.2	0.05
0.10	0.5	3.8	0.5	0.10
0.15	0.9	4.7	0.9	0.15
0.20	1.3	5.4	1.3	0.20
0.25	1.9	6.1	1.9	0.25
0.30	2.5	6.7	2.5	0.30
0.35	3.1	7.2	3.1	0.35
0.40	3.8	7.7	3.8	0.40
0.45	4.5	8.2	4.5	0.45
0.50	5.3	8.6	5.3	0.50
0.55	6.1	9.0	6.1	0.55
0.60	7.0	9.4	7.0	0.60
0.65	7.9	9.8	7.9	0.65
0.70	8.8	10.2	8.8	0.70
0.75	9.7	10.5	9.7	0.75
0.80	10.7	10.9	10.7	0.80
0.85	11.8	11.2	11.2	0.85

100-YR FLOW 10.4 CFS
 CALCULATED DEPTH 0.80 FT



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnini, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps & aeriels](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	2.35 (1.96-2.92)	3.08 (2.57-3.80)	4.15 (3.43-5.12)	4.99 (4.09-6.12)	6.10 (4.92-7.45)	6.95 (5.54-8.44)	7.82 (6.13-9.48)	8.70 (6.71-10.5)	9.88 (7.43-12.0)	10.8 (7.93-13.1)
10-min	1.79 (1.48-2.21)	2.34 (1.95-2.89)	3.16 (2.61-3.89)	3.79 (3.11-4.66)	4.64 (3.75-5.67)	5.29 (4.22-6.42)	5.96 (4.67-7.21)	6.62 (5.11-8.00)	7.52 (5.65-9.10)	8.20 (6.04-9.94)
15-min	1.48 (1.23-1.83)	1.93 (1.61-2.39)	2.62 (2.16-3.22)	3.14 (2.57-3.84)	3.84 (3.10-4.68)	4.37 (3.49-5.31)	4.92 (3.86-5.96)	5.47 (4.22-6.62)	6.21 (4.67-7.52)	6.78 (4.99-8.22)
30-min	0.996 (0.826-1.23)	1.30 (1.09-1.61)	1.76 (1.45-2.17)	2.11 (1.73-2.59)	2.58 (2.08-3.15)	2.94 (2.35-3.57)	3.31 (2.60-4.01)	3.69 (2.84-4.45)	4.18 (3.14-5.06)	4.56 (3.36-5.53)
60-min	0.617 (0.511-0.762)	0.806 (0.672-0.996)	1.09 (0.899-1.34)	1.31 (1.07-1.60)	1.60 (1.29-1.95)	1.82 (1.45-2.21)	2.05 (1.61-2.48)	2.28 (1.76-2.76)	2.59 (1.95-3.13)	2.82 (2.08-3.42)
2-hr	0.360 (0.302-0.434)	0.466 (0.392-0.564)	0.621 (0.520-0.748)	0.740 (0.612-0.889)	0.900 (0.738-1.08)	1.02 (0.827-1.22)	1.15 (0.914-1.37)	1.28 (0.997-1.52)	1.45 (1.10-1.72)	1.58 (1.18-1.89)
3-hr	0.262 (0.220-0.322)	0.336 (0.283-0.414)	0.440 (0.368-0.540)	0.522 (0.433-0.637)	0.637 (0.519-0.771)	0.728 (0.585-0.878)	0.822 (0.650-0.991)	0.921 (0.716-1.11)	1.06 (0.797-1.27)	1.17 (0.859-1.40)
6-hr	0.158 (0.136-0.188)	0.200 (0.172-0.238)	0.255 (0.218-0.302)	0.299 (0.253-0.353)	0.359 (0.300-0.422)	0.406 (0.334-0.475)	0.454 (0.368-0.532)	0.504 (0.402-0.591)	0.572 (0.444-0.669)	0.625 (0.474-0.733)
12-hr	0.089 (0.077-0.104)	0.112 (0.097-0.131)	0.141 (0.122-0.165)	0.164 (0.141-0.191)	0.195 (0.165-0.227)	0.219 (0.184-0.254)	0.244 (0.201-0.282)	0.269 (0.219-0.311)	0.302 (0.240-0.351)	0.328 (0.256-0.383)
24-hr	0.053 (0.047-0.060)	0.067 (0.059-0.077)	0.086 (0.077-0.099)	0.102 (0.090-0.116)	0.124 (0.108-0.141)	0.141 (0.122-0.160)	0.159 (0.137-0.180)	0.177 (0.151-0.201)	0.203 (0.171-0.230)	0.223 (0.186-0.254)
2-day	0.029 (0.025-0.033)	0.037 (0.033-0.042)	0.048 (0.043-0.055)	0.058 (0.051-0.066)	0.070 (0.061-0.080)	0.081 (0.070-0.092)	0.091 (0.079-0.104)	0.103 (0.088-0.117)	0.119 (0.100-0.135)	0.131 (0.109-0.150)
3-day	0.021 (0.018-0.024)	0.026 (0.023-0.030)	0.035 (0.031-0.040)	0.042 (0.037-0.047)	0.051 (0.045-0.058)	0.059 (0.051-0.067)	0.067 (0.058-0.076)	0.076 (0.065-0.086)	0.088 (0.074-0.101)	0.098 (0.082-0.112)
4-day	0.017 (0.015-0.019)	0.021 (0.019-0.024)	0.028 (0.025-0.032)	0.034 (0.030-0.038)	0.042 (0.037-0.047)	0.048 (0.042-0.055)	0.055 (0.048-0.062)	0.063 (0.054-0.071)	0.073 (0.062-0.083)	0.082 (0.068-0.093)
7-day	0.011 (0.009-0.012)	0.014 (0.012-0.016)	0.018 (0.016-0.021)	0.022 (0.019-0.025)	0.027 (0.023-0.031)	0.031 (0.027-0.035)	0.036 (0.031-0.040)	0.040 (0.034-0.046)	0.047 (0.040-0.054)	0.053 (0.044-0.060)
10-day	0.008 (0.007-0.009)	0.010 (0.009-0.012)	0.014 (0.012-0.016)	0.017 (0.015-0.019)	0.021 (0.018-0.023)	0.024 (0.021-0.027)	0.027 (0.023-0.030)	0.031 (0.026-0.035)	0.036 (0.030-0.040)	0.040 (0.033-0.045)
20-day	0.005 (0.004-0.006)	0.006 (0.006-0.007)	0.009 (0.008-0.010)	0.010 (0.009-0.012)	0.012 (0.011-0.014)	0.014 (0.012-0.016)	0.016 (0.014-0.018)	0.018 (0.015-0.020)	0.020 (0.017-0.023)	0.022 (0.018-0.025)
30-day	0.004 (0.003-0.004)	0.005 (0.005-0.006)	0.007 (0.006-0.008)	0.008 (0.007-0.009)	0.010 (0.008-0.011)	0.011 (0.010-0.012)	0.012 (0.011-0.014)	0.014 (0.012-0.015)	0.016 (0.013-0.018)	0.017 (0.014-0.019)
45-day	0.003 (0.003-0.003)	0.004 (0.004-0.005)	0.005 (0.005-0.006)	0.006 (0.006-0.007)	0.008 (0.007-0.009)	0.009 (0.007-0.010)	0.010 (0.008-0.011)	0.011 (0.009-0.012)	0.012 (0.010-0.014)	0.013 (0.011-0.015)
60-day	0.003 (0.002-0.003)	0.003 (0.003-0.004)	0.004 (0.004-0.005)	0.005 (0.005-0.006)	0.006 (0.006-0.007)	0.007 (0.006-0.008)	0.008 (0.007-0.009)	0.009 (0.007-0.010)	0.010 (0.008-0.011)	0.010 (0.009-0.012)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

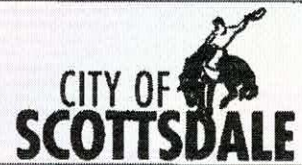
[Back to Top](#)

PF graphical

10-UP-2019
09/26/2019

APPENDIX F
(Section 404 Certification Form)

Section 404 Certification Form



Before the City issues development permits for a project, the developer's Engineer or the property owner must certify that it complies with or is exempt from Section 404 of the Clean Water Act of the United States. Section 404 regulates the discharge of dredged or fill material into a wetland, lake (including dry lakes), river, stream (including intermittent streams, ephemeral washes and arroyos) or other waters of the United States.

Prior to submittal of improvement plans to Project Review, this form must be completed (and submitted with the improvement plans) as evidence of compliance.

Certification of Section 404 Permit Status:

Owner's Name: SimonCRE Phone No.: 480-745-1956
Project Name/Description: Kerry's Car Care - Shea Case No.: _____
Project Location/Address: 11653 East Sabuaro Drive
Scottsdale, Arizona 85259

A registered Engineer or the property owner must check the applicable condition and certify by signing below that:

1. **Section 404 does apply to the project because there will be a discharge of dredged or fill material to waters of the U.S., and:**

☐ A Section 404 Permit has already been obtained for this project.

or

☐ This project qualifies for a "Nationwide Permit," and this project will meet all terms and conditions of the applicable nationwide permit.

2. **Section 404 does not apply to the project because:**

☒ No watercourse waters of the U.S. exist on the property.

☐ No jurisdictional waters of the U.S. exist on property. Attached is a copy of the COE's Jurisdictional Determination.

☐ Watercourses or other waters of the U.S. do exist on the property, but the project will not involve the discharge of dredged or fill material into any of these waters.

I certify that the above statement is true.



Engineer's Signature and Seal, or Owner's Signature

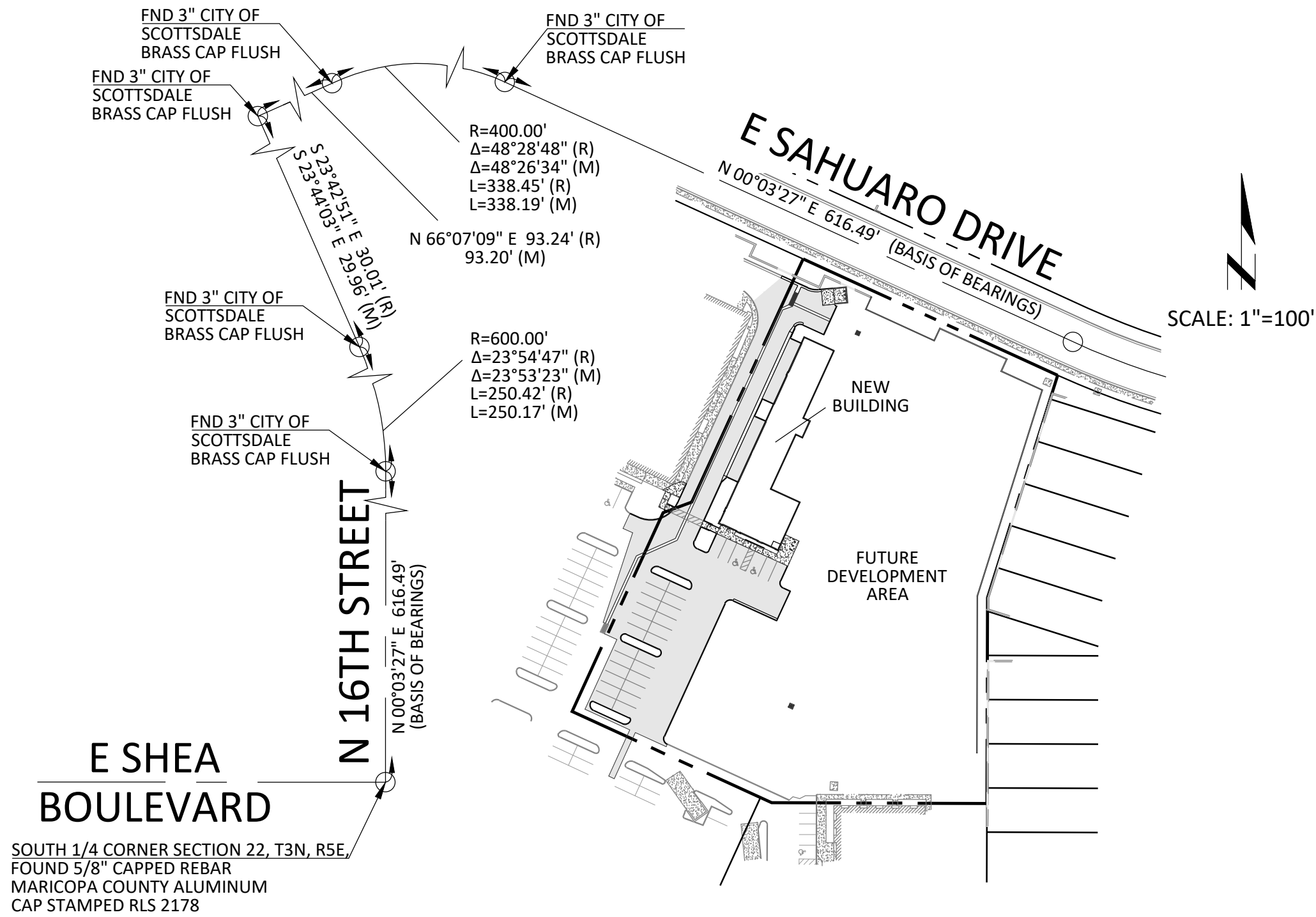
PE, Cypress Civil Development
Title/ Company

Planning and Development Services

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • www.ScottsdaleAZ.gov

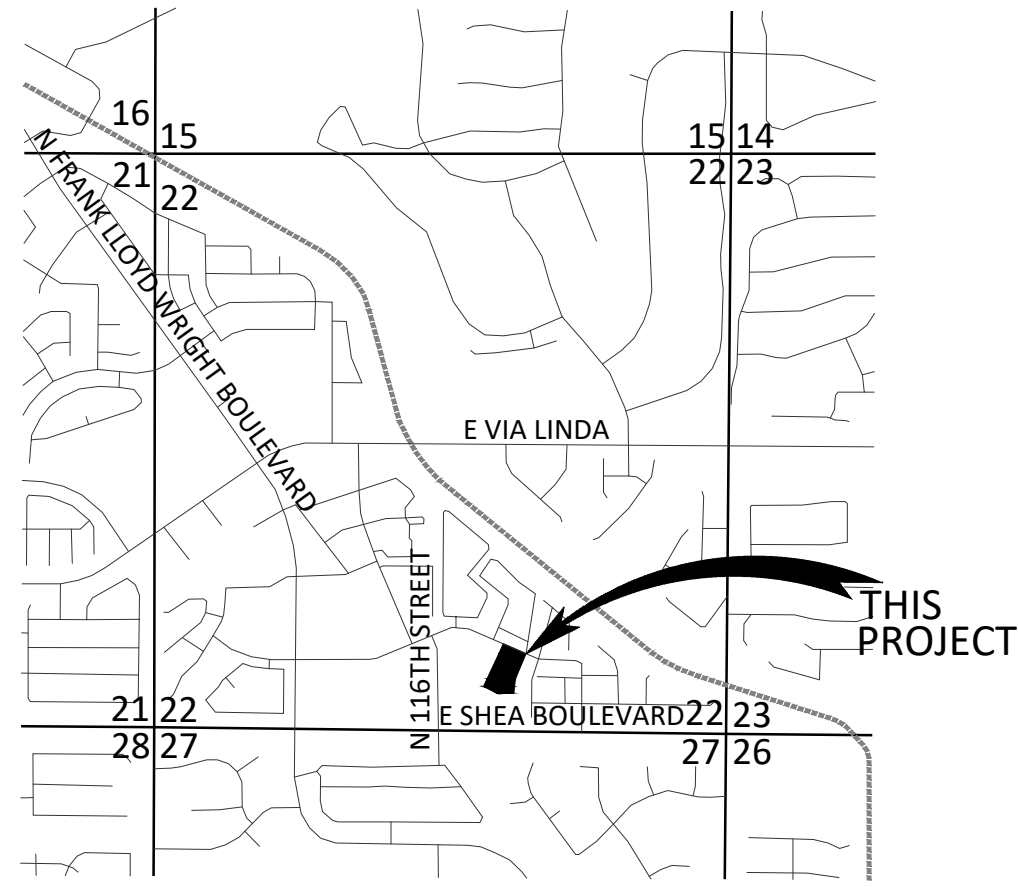
PRELIMINARY IMPROVEMENT PLAN for KERRY'S CAR CARE SHEA

11653 EAST SAHUARO DRIVE SCOTTSDALE, ARIZONA
A PORTION SOUTHEAST QUARTER OF SECTION 22, TOWNSHIP 3 NORTH, RANGE 5 EAST OF THE
GILA AND SALT RIVER MERIDIAN, MARICOPA COUNTY, ARIZONA



PROJECT OVERVIEW

Stormwater Review by:
Ghassan Aouad
Phone: 480-312-7055
e-mail: gaouad@scottsdaleaz.gov
Review Cycle 1 Date 10/21/2019



IN THE SW 1/4 OF THE SE 1/4 OF SECTION 22,
T. 3 N., R. 5 E., G.&S.R.M.,
CITY OF SCOTTSDALE, MARICOPA COUNTY, ARIZONA

LOCATION MAP

3" = 1 MILE

LEGEND

	EXISTING SEWER MANHOLE		PROJECT RIGHT-OF-WAY
	EXISTING SEWER CLEANOUT		EXISTING RIGHT-OF-WAY
	EXISTING WATER STUB		PROJECT/NEW PROPERTY LINE
	EXISTING WATER MANHOLE		EXISTING PROPERTY LINE
	EXISTING WATER VALVE		ROADWAY CENTERLINE
	EXISTING WATER METER		FLOW-LINE
	EXISTING BACKFLOW PREVENTER		EXISTING EASEMENT
	EXISTING FIRE HYDRANT		NEW EASEMENT
	EXISTING FIRE BACKFLOW PREVENTER		EXISTING CONTOUR
	EXISTING GAS METER		NEW CONTOUR
	EXISTING TRANSFORMER		EXISTING CONCRETE
	EXISTING SIGN		EXISTING WALL
	EXISTING SITE LIGHT		NEW ASPHALT
	EXISTING STREET LIGHT		NEW CONCRETE
	EXISTING LIGHT PULLBOX		NEW WALL
	EXISTING TELEPHONE PEDESTAL		EXISTING PAVEMENT EDGE
	EXISTING STORM DRAIN PIPE		EXISTING CURB
	EXISTING STORM DRAIN MANHOLE		EXISTING PAINT STRIPE
	EXISTING UNDERGROUND ELECTRIC		EXISTING RAILING
	EXISTING TELEPHONE LINE		NEW CURB
	EXISTING SEWER LINE		NEW PAINT STRIPE
	EXISTING WATER LINE		NEW RAILING
	EXISTING FIRE SERVICE		NEW STORM DRAIN PIPE
	EXISTING GAS LINE		NEW SEWER LINE
	SPOT ELEV. (EXIST. GRADE)		NEW WATER LINE
	SPOT ELEV. (NEW GRADE)		NEW FIRE SERVICE
	RIGHT-OF-WAY		NEW SEWER CLEANOUT
	BACK OF CURB		NEW WATER VALVE
	SIDEWALK		NEW WATER METER
	RECORDED VALUE		NEW BACKFLOW PREVENTER
	MEASURED VALUE		NEW FIRE HYDRANT
	PAVEMENT (ASPHALT)		NEW FIRE CONNECTION
	CONCRETE		NEW WATER STUB
	GUTTER		NEW REDUCER
	TOP OF CURB		NEW STORM DRAIN MANHOLE
	FINISHED GRADE		NEW SIGN
	TOP OF FOOTING		NEW SITE LIGHT
	TOP OF WALL		SURVEY MONUMENT AS NOTED
	HIGH POINT		
	GRADE BREAK		
	FINISHED FLOOR ELEVATION		
	FINISH GRADE HIGH		
	FINISH GRADE LOW		
	PUBLIC UTILITY EASEMENT		
	MATCH EXISTING		
	RIM		
	INVERT		
	LENGTH		
	LENGTH - FEET		
	SLOPE		

PROJECT INFORMATION

PROJECT DESCRIPTION:
THE PROJECT CONSISTS OF THE CONSTRUCTION OF A NEW SINGLE-STORY
AUTOMOTIVE SERVICE BUILDING WITH ALL REQUIRED GRADING & DRAINAGE,
PARKING, UTILITY AND PAVING REQUIREMENTS

ADDRESS:
11653 EAST SAHUARO DRIVE
SCOTTSDALE, ARIZONA 85259

APN: 217-28-986

ZONING: C-3

SITE AREA: 109,750 SF (2.52 AC)

SURVEY NOTES

- THE SURVEY FOR THIS PROJECT WAS PERFORMED BY:
SUPERIOR SURVEYING SERVICES, INC.
2122 WEST LONE CACTUS DRIVE, SUITE 11
PHOENIX, ARIZONA 85027
PH: 623-869-0223
CONTACT: DAVID S. KLEIN, R.L.S.
- THE BASIS OF BEARINGS FOR THIS PROJECT IS THE MONUMENT LINE OF 116TH
STREET, ALSO BEING THE WEST LINE OF THE SOUTHEAST QUARTER OF SECTION
22, USING A BEARING OF NORTH 00°03'27" EAST PER THE FINAL PLAT FOR
MOUNTAINSIDE PLAZA, BOOK 632 OF MAPS, PAGE 30, RECORDS OF MARICOPA
COUNTY, ARIZONA.
- THE BASIS OF ELEVATION FOR THIS PROJECT IS THE CITY OF SCOTTSDALE
BENCHMARK NO. 8272 BEING A CITY OF SCOTTSDALE BRASS CAP IN HANDHOLE (0.40' DOWN)
MARKING THE INTERSECTION OF SHEA BOULEVARD AND 112TH STREET HAVING AN ELEVATION OF 1426.28' (NAVD88).

BENCHMARK

THE BENCHMARK USED FOR THIS PLAN IS THE CITY OF SCOTTSDALE BENCHMARK NO.
8272 BEING A CITY OF SCOTTSDALE BRASS CAP IN HANDHOLE (0.40' DOWN)
MARKING THE INTERSECTION OF SHEA BOULEVARD AND 112TH STREET HAVING AN
ELEVATION OF 1426.28' (NAVD88).

DRAINAGE STATEMENT

-SITE IS IN A SPECIAL FLOOD HAZARD AREA - NO
-OFFSITE FLOWS AFFECT THIS SITE - NO
-RETENTION PROVIDED IS 100-YR, 2-HR IN AN EXISTING REGIONAL UNDERGROUND
RETENTION VAULT SYSTEM FOR THE MOUNTAINSIDE PLAZA AND IN A NEW
TEMPORARY SURFACE RETENTION BASIN FOR THE FUTURE RETAIL PAD.
-EXTREME STORM OUTFALLS THE SITE AT THE SOUTHWEST CORNER AT THE
ELEVATION OF 1482.38

LEGAL DESCRIPTION

PARCEL NO. 1:

LOT 3, MOUNTAINSIDE PLAZA, ACCORDING TO THE PLAT OF RECORD IN THE OFFICE
OF COUNTY RECORDER OF MARICOPA COUNTY, ARIZONA, RECORDED IN BOOK 632
OF MAPS, PAGE 30.

PARCEL NO. 2:

RECIPROCAL EASEMENTS FOR DRIVEWAY PURPOSES AND REASONABLE PASSAGE OF
MOTOR VEHICLES, ACCESS, INGRESS AND EGRESS, REASONABLE PARKING OF MOTOR
VEHICLES, AS CREATED AND DESCRIBED IN THAT CERTAIN DECLARATION OF
ESTABLISHMENT OF COVENANTS, CONDITIONS AND RESTRICTIONS AND GRANT OF
RECIPROCAL EASEMENTS RECORDED AS 2003-0518720, AND THEREAFTER SECOND
DECLARATION OF COVENANTS, CONDITIONS AND RESTRICTIONS RECORDED MARCH
11, 2004 AS 2004-0251028 OF OFFICIAL RECORDS, AND THEREAFTER THIRD
DECLARATION OF ESTABLISHMENT OF COVENANTS, CONDITIONS AND RESTRICTIONS
RECORDED JANUARY 19, 2006 AS 2006-0081182 OF OFFICIAL RECORDS, AND
THEREAFTER AMENDMENT TO DECLARATION OF ESTABLISHMENT OF COVENANTS,
CONDITIONS AND RESTRICTIONS AND GRANT OF RECIPROCAL EASEMENTS
RECORDED JUNE 28, 2006 AS 2006-0872205 OF OFFICIAL RECORDS.

PROJECT RETENTION

THIS PROJECT LIES WITHIN THE MOUNTAINSIDE PLAZA DEVELOPMENT.

REQUIRED 100-YR 2-HR RETENTION VOLUME
[AC-FT] = C x P ([IN] / 12) x AREA [AC]

$$V = 0.90 \times (2.30/12) \times 109,750 = 18,932 \text{ CF}$$

PROVIDED RETENTION:

TOTAL REQUIRED RETENTION VOLUME IS PROVIDED VIA AN EXISTING REGIONAL
UNDERGROUND RETENTION VAULT SYSTEM SOUTH OF THE PROJECT. THE
ORIGINAL MOUNTAINSIDE PLAZA DEVELOPMENT DESIGNED THIS SYSTEM TO
PROVIDE RETENTION FOR THE ENTIRE PLAZA, INCLUDING THE PROJECT SITE IN
THE FUTURE DEVELOPED CONDITION.

15,400 CF TEMPORARY DETENTION PROVIDED IN A NEW SURFACE RETENTION
BASIN FOR THE FUTURE RETAIL DEVELOPMENT TO THE EAST. TEMPORARY
BASIN BLEEDS OFF TO REGIONAL RETENTION VAULTS VIA A NEW CATCH BASIN
AND STORM DRAIN.

FLOODPLAIN INFORMATION

ACCORDING TO THE FEDERAL EMERGENCY MANAGEMENT AGENCY FLOOD
INSURANCE RATE MAP PANEL NUMBER 04013C1780L, DATED OCTOBER 16, 2013
THE PARCEL IS LOCATED IN THE ZONE X (SHADED) AREA, WHICH IS DEFINED AS
AREAS OF 0.2% ANNUAL CHANCE FLOOD; AREAS OF 1% ANNUAL CHANCE FLOOD
WITH AVERAGE DEPTHS OF LESS THAN 1 FOOT OR WITH DRAINAGE AREAS LESS
THAN 1 SQUARE MILE; AND AREAS PROTECTED BY LEVEES FROM 1% ANNUAL
CHANCE FLOOD.

Please review all notes per
comments on these plans and
the corresponding drainage
report and update as
necessary

FEMA Block and Engineer FEMA Certification
Block should be provided, per
page 56 and 57 of Scottsdale DS&PM

CYPRESS
CIVIL DEVELOPMENT
strength + sustainability

4450 north 12th street, #228
phoenix, arizona 85014
p: 623.282.2498
e: jphunt@cypresscivil.com

CYPRESS PROJECT NO: 19.101



NO.	DATE	REVISION

PRELIMINARY IMPROVEMENT PLAN for
KERRY'S CAR CARE SHEA
11653 EAST SAHUARO DRIVE SCOTTSDALE, ARIZONA

cover

DEVELOPER

SIMONCRE
6900 EAST 2ND STREET
SCOTTSDALE, ARIZONA 85251
PH: 480-745-1956
ATTN: PETER KRAHENBUHL

CIVIL ENGINEER

CYPRESS CIVIL DEVELOPMENT
4450 NORTH 12TH STREET, #228
PHOENIX, ARIZONA 85014
PH: 623-282-2498
ATTN: JEFF HUNT

ARCHITECT

LARSON ASSOCIATES ARCHITECTS, INC.
3807 NORTH 24TH STREET, SUITE 100
PHOENIX, ARIZONA 85016
PH: 602-956-9929
ATTN: JAMES LARSON

UTILITIES

WATER: CITY OF SCOTTSDALE
SEWER: CITY OF SCOTTSDALE
ELECTRIC: ARIZONA PUBLIC SERVICE
GAS: SOUTHWEST GAS
TELEPHONE: CENTURYLINK
CABLE: COX COMMUNICATIONS

SHEET INDEX

- COVER SHEET
- GRADING & DRAINAGE PLAN
- ONSITE UTILITY PLAN

DEVELOPER

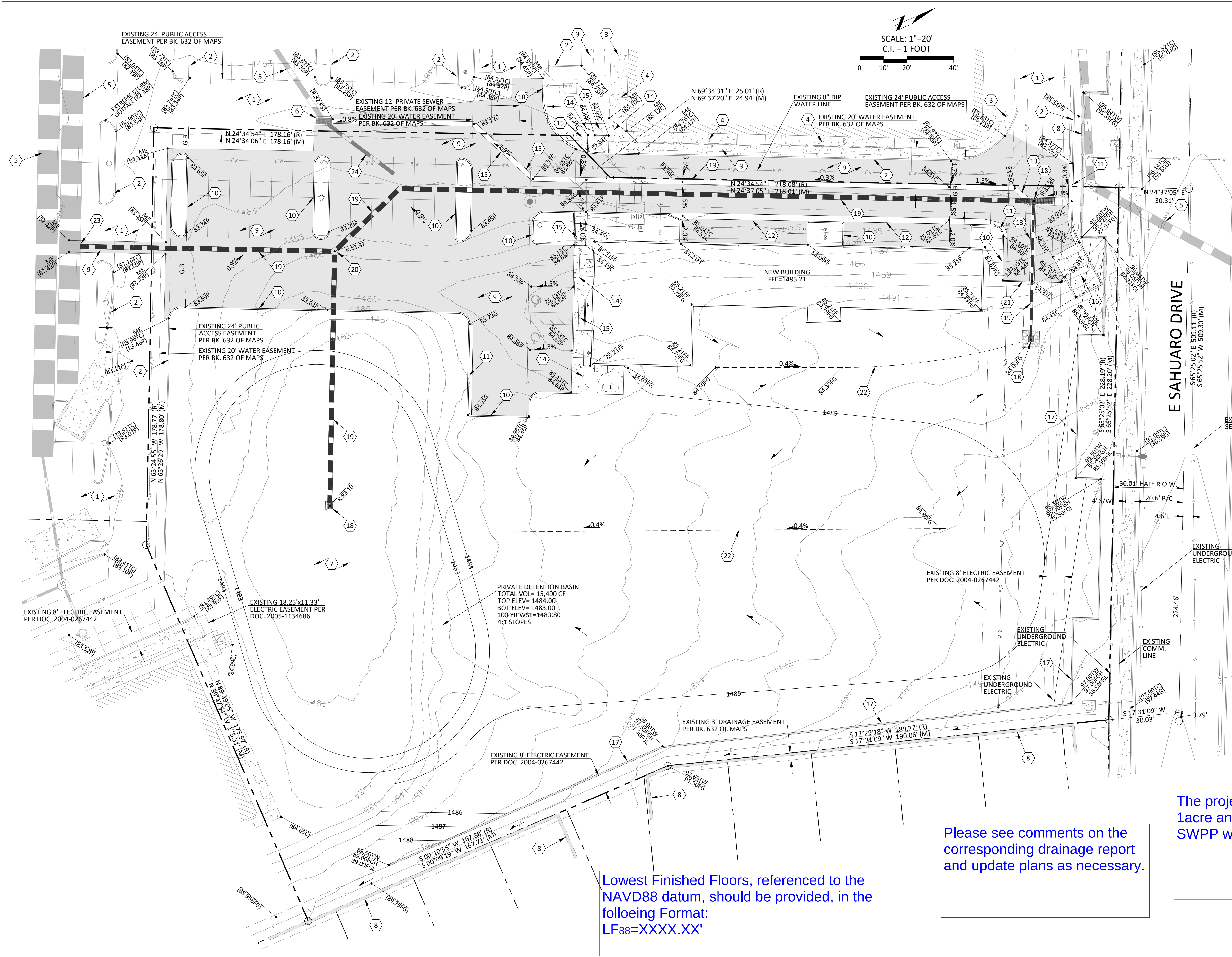
SIMONCRE
6900 EAST 2ND STREET
SCOTTSDALE, ARIZONA 85251
PH: 480-745-1956
ATTN: PETER KRAHENBUHL

SITE ADDRESS

11653 EAST SAHUARO DRIVE
SCOTTSDALE, ARIZONA 85259
APN: 217-28-986

SHEET NUMBER





KEYNOTES

- 1 EXISTING ASPHALT TO REMAIN.
- 2 EXISTING CURB TO REMAIN.
- 3 EXISTING CONCRETE SIDEWALK TO REMAIN.
- 4 EXISTING ACCESSIBLE RAMP TO REMAIN.
- 5 EXISTING STORM DRAIN PIPE/VAULT TO REMAIN.
- 6 EXISTING STORM DRAIN CATCH BASIN TO REMAIN.
- 7 EXISTING SURFACE RETENTION BASIN TO BE MODIFIED.
- 8 EXISTING MASONRY RETAINING WALL TO REMAIN.
- 9 NEW ASPHALT PAVEMENT.
- 10 NEW CONCRETE VERTICAL CURB.
- 11 NEW CONCRETE VERTICAL CURB AND GUTTER.
- 12 NEW CONCRETE ROLL CURB.
- 13 NEW VALLEY GUTTER.
- 14 NEW CONCRETE SIDEWALK.
- 15 NEW ACCESSIBLE ACCESS RAMP.
- 16 NEW TRASH ENCLOSURE.
- 17 NEW MASONRY RETAINING WALL.
- 18 NEW STORM DRAIN INLET.
- 19 NEW STORM DRAIN PIPE.
- 20 NEW STORM DRAIN MANHOLE.
- 21 NEW TEMPORARY EXTRUDED CURB.
- 22 NEW GRADED SWALE.
- 23 CONNECT TO EXISTING RETENTION VAULT.
- 24 EXISTING STORM DRAIN PIPE TO BE REMOVED.

Please see comments on the corresponding drainage report and update plans as necessary.

The project site is more than 1acre and, accordingly, a SWPP would be required.

Lowest Finished Floors, referenced to the NAVD88 datum, should be provided, in the folloeing Format:
LF88=XXXX.XX'



4450 north 12th street, #228
phoenix, arizona 85014
p: 623.282.2498
e: jphunt@cypresscivil.com

CYPRESS PROJECT NO: 19.101



NO.	DATE	REVISION

PRELIMINARY IMPROVEMENT PLAN for
KERRY'S CAR CARE SHEA
11653 EAST SAHUARO DRIVE SCOTTSDALE, ARIZONA
grading and drainage plan

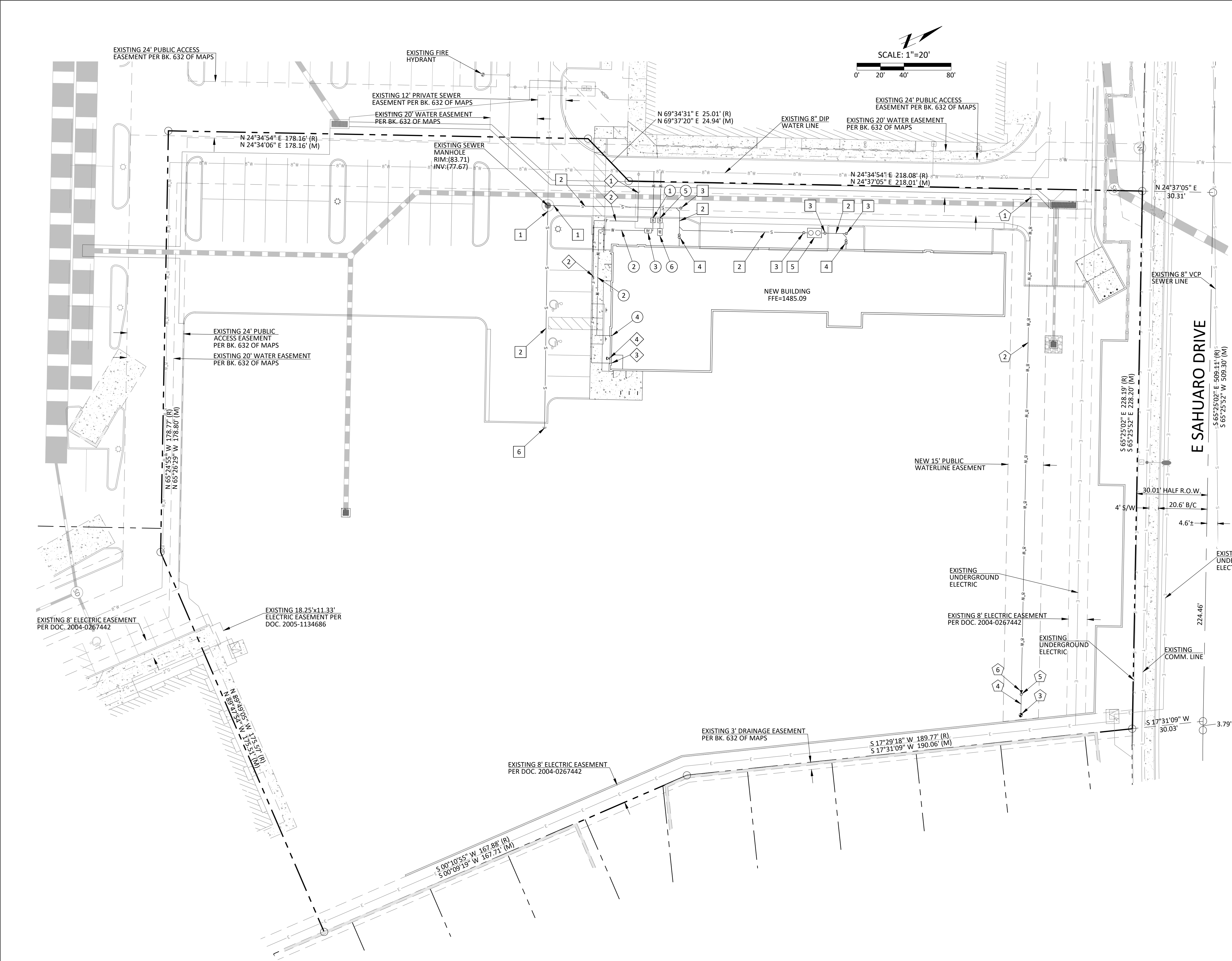
DEVELOPER

SIMONCRE
6900 EAST 2ND STREET
SCOTTSDALE, ARIZONA 85251
PH: 480-745-1956
ATTN: PETER KRAHENBUHL

SITE ADDRESS

11653 EAST SAHUARO DRIVE
SCOTTSDALE, ARIZONA 85259
APN: 217-28-986

SHEET NUMBER



PRIVATE WATER KEYNOTES

- 1 NEW 2" SERVICE AND METER FOR DOMESTIC WATER SERVICE.
- 2 NEW 2" PVC DOMESTIC WATER LINE.
- 3 NEW 2" DOMESTIC BACKFLOW PREVENTER.
- 4 CONNECT TO BUILDING PER PLUMBING PLANS.
- 5 NEW 1" SERVICE AND METER FOR IRRIGATION SERVICE.
- 6 NEW 1" IRRIGATION BACKFLOW PREVENTER.

PRIVATE FIRE LINE KEYNOTES

- 1 CONNECT TO EXISTING 6" WATER STUB FOR FIRE SERVICE.
- 2 NEW 6" DIP FIRE LINE.
- 3 CONNECT TO BUILDING PER PLUMBING PLANS.
- 4 NEW FDC LOCATION.

PRIVATE SEWER KEYNOTES

- 1 CONNECT TO EXISTING SEWER MANHOLE.
- 2 NEW 6" PVC SDR-35 SEWER LINE.
- 3 NEW ONE-WAY SEWER CLEAN OUT.
- 4 NEW TWO-WAY SEWER CLEAN OUT.
- 5 NEW GREASE INTERCEPTOR.
- 6 STUB AND CAP NEW 6" SERVICE.

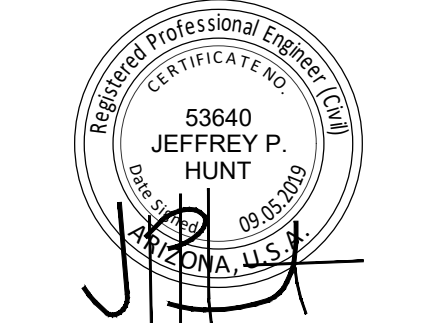
PUBLIC WATER KEYNOTES

- 1 CONNECT TO EXISTING 8" STUB.
- 2 NEW 8" DIP PUBLIC WATER LINE.
- 3 NEW FIRE HYDRANT.
- 4 NEW 6" DIP PUBLIC WATER LINE.
- 5 NEW 6" VALVE.
- 6 NEW 8"x6" REDUCER FITTING.

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phoenix, arizona 85014
p: 623.282.2498
e: jphunt@cypresscivil.com

CYPRESS PROJECT NO: 19.101



NO.	DATE	REVISION

PRELIMINARY IMPROVEMENT PLAN for
KERRY'S CAR CARE SHEA
11653 EAST SAHUARO DRIVE SCOTTSDALE, ARIZONA
onsite utility plan

DEVELOPER

SIMONCRE
6900 EAST 2ND STREET
SCOTTSDALE, ARIZONA 85251
PH: 480-745-1956
ATTN: PETER KRAHENBUHL

SITE ADDRESS

11653 EAST SAHUARO DRIVE
SCOTTSDALE, ARIZONA 85259
APN: 217-28-986

SHEET NUMBER

DRAINAGE REPORT FOR KERRY'S CAR CARE SHEA

Scottsdale, Arizona

05 September 2019

PREPARED FOR
Larson Associates Architects, LLC
3807 North 24th Street, Suite 100
Phoenix, Arizona 85016
Attn: James Larson

DEVELOPER
SimonCRE
6900 East 2nd Street
Scottsdale, Arizona 85251
Attn: Peter Krahenbuhl

SITE ADDRESS
11653 East Sahuaro Drive
Scottsdale, Arizona 85259

PREPARED BY

CYPRESS
CIVIL DEVELOPMENT
strength + sustainability

4450 north 12th street, #228
phoenix, arizona 85014
CYPRESS # 19.101



10-UP-2019
09/26/2019

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B	Mountainside Plaza As-Built Plans & Drainage Report
C	FEMA FIRM Map
D	Sahuaro Drive Street Capacity Calculations
E	Proposed Watershed Map + Calculations
F	Section 404 Certification Form

I. INTRODUCTION

1. PROJECT NAME AND LOCATION

The Project site is located at 11653 East Sahuaro Drive in Scottsdale, Arizona. The project is located in the southeast $\frac{1}{4}$ of Section 22, Township 3 North, Range 5 East, G&SRM. The Project site occupies approximately 2.52 acres. The Project is currently undeveloped desert and is a portion of the Mountainside Plaza. The Project has street frontage and access to Sahuaro Drive along its northern boundary. To the west and south are existing commercial retail developments within Mountainside Plaza. to the east are single-family homes. Refer to Appendix A for Location Map and Aerial Photo.

The proposed Project consists of the construction of a new single-story automotive service building with required grading and drainage, onsite utility, and parking improvements.

2. PURPOSE

The intent of this Drainage Report is to provide the conceptual drainage scheme for the Project in support of the Preliminary Improvement Plan Submittal.

3. EXISTING STUDIES

As-Built Improvement Plans and a Drainage Report for Mountainside Plaza (previously known as 116th Street & Shea Retail Center), prepared by Evans, Kuhn, and Associates, Inc. in 2002, were obtained and reviewed. The plans show that the Project is Lot 3 of the Mountainside Plaza development. The report shows that the plaza was designed to retain all onsite and offsite runoff in underground retention vaults. Refer to Appendix B for Mountainside Plaza As-Built Plans & Drainage Report.

4. FEMA FLOOD ZONE

According to the Federal Emergency Management Agency Flood Insurance Rate Map, panel number 04013C1780L dated October 16, 2013, the parcel is located in the shaded Zone X Area, which is an area defined as within the 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood. Refer to Appendix C for FEMA FIRM Map.

II. EXISTING DRAINAGE INFRASTRUCTURE

1. OFFSITE

Offsite flow is routed through the site via an existing storm system drain that captures offsite flow from a retention basin north of Sahuaro Drive and conveys it to outfall to existing 28" outfall pipes in 116th Street. Runoff from the adjacent Mountainside Fitness building and from the drive aisle running from west to east north of the

shopping center flows through the drive aisle along the western boundary of the project. This is roughly 11.5 CFS of flow.

STREET CAPACITY CALCULATIONS

The street capacity within Sahuaro Street is 44.94 CFS. It is assumed this is sufficient capacity to convey all offsite flows away from the Project, as it does in the existing condition. Refer to Appendix D for Sahuaro Drive Capacity Calculations.

2. ONSITE

The As-Built Plans and Drainage Report show that the Project site currently drains to a temporary surface retention basin onsite. However, the existing underground storm drain and retention vault system for the overall Mountainside Plaza were designed to convey and retain all onsite flow for the Project site in its future development.

III. PROPOSED DRAINAGE INFRASTRUCTURE

1. CONVEYANCE OF RUNOFF

Offsite runoff from the Mountainside Fitness building and the drive aisle behind it shall be intercepted by a new catch basin at the northwest corner of the Project site. Onsite runoff from the Kerry's car care building and surrounding landscaping shall be conveyed to a new catch basin north of the building. Onsite runoff from the future retail development area shall be conveyed to a new temporary surface retention basin at the southeast, where it shall pond at a depth of 0.80 feet and drain via a single new catch basin. From these three new catch basins, runoff shall be conveyed south via new storm drain and connect directly to the existing underground retention vaults, where runoff shall be discharged via existing drywells.

Runoff from the majority of the new Kerry's parking lot and western drive aisle shall be conveyed via shallow sheet flow and a new valley gutter to an existing catch basin at the southwest corner of the site where it will enter the existing storm drain system and be conveyed to the existing underground retention vaults. Refer to Appendix E for the Proposed Watershed Map and Calculations.

2. STORM WATER RETENTION REQUIREMENTS

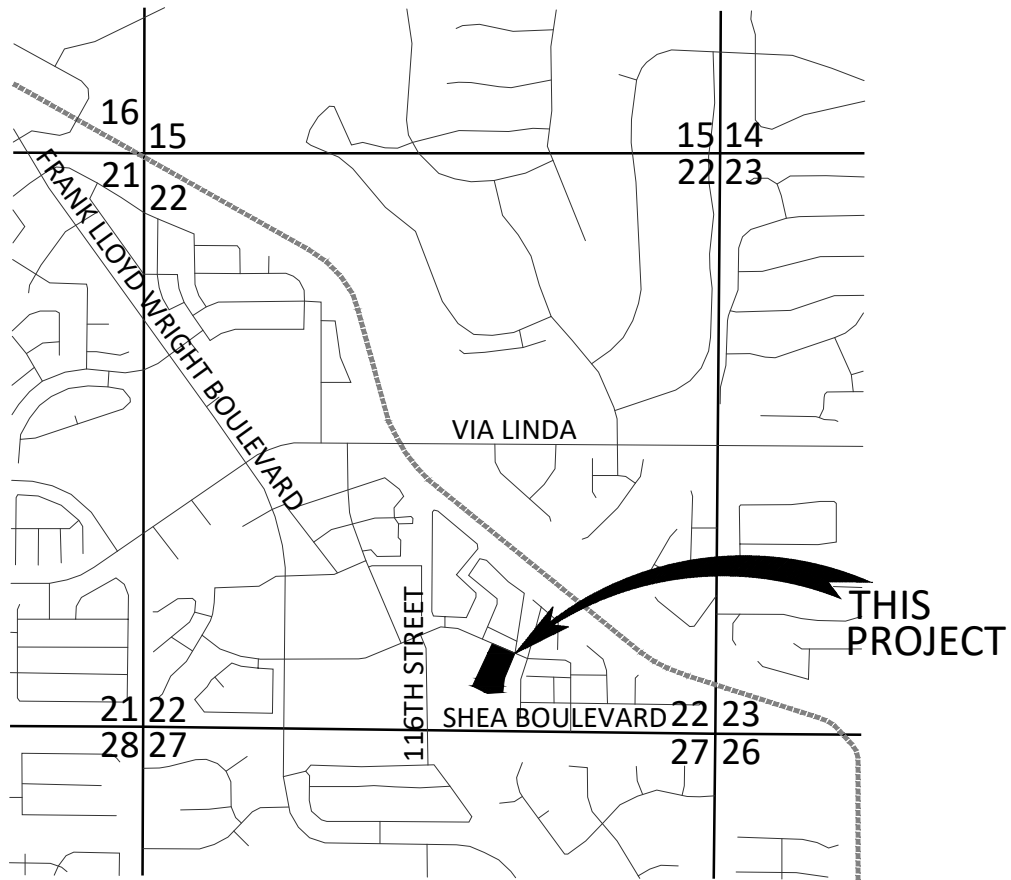
In the original Mountainside Plaza development, the existing underground storm drain and retention vault system was designed to convey and retain all onsite flow for the Project site in its future developed condition. Therefore, no new retention is required. The existing underground retention system is designed to drain within 36 hours via a bleed-off pump to existing 28" outfall pipes in 116th Street.

IV. SUMMARY AND CONCLUSION

This Drainage Report is to accompany the Improvement Plan for the Kerry's Car Care Shea development project. This narrative was written utilizing generally accepted engineering practices and all information herein has been researched through archived documents and all calculations were accomplished through applying the City of Scottsdale Engineering Standards.

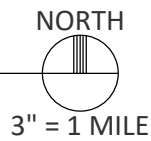
The analysis presented in this narrative evaluates storm water runoff resulting from a statistical evaluation of storm events of particular duration and frequency up to and including a 100-year frequency event. A storm event exceeding the 100-year frequency may cause or create the risk of greater flood impact than is addressed and presented herein. The scope of this assessment does not include evaluation of storm water runoff resulting from storm events exceeding the 100-year frequency. CYPRESS assumes no responsibility for actual flood damage, increased risks of flood damage, or increased construction or development cost resulting from or related to any such events, nor shall CYPRESS be responsible for any changes in, or additions to, regulatory requirements which may result from, or be related to, any such events or changes in hydrologic or hydraulic conditions within the watershed. Refer to Appendix E for Warning and Disclaimer of Liability Form.

APPENDIX A
(Location Map + Aerial Photo)



IN THE SW 1/4 OF THE SE 1/4 OF SECTION 22,
T. 3 N., R. 5 E., G.&S.R.M.,
CITY OF SCOTTSDALE, MARICOPA COUNTY, ARIZONA

LOCATION MAP



AERIAL PHOTO



APPENDIX B
(Mountainside Plaza As-Built Plans & Drainage Report)

NATIVE PLANT PERMIT NUMBER
CASE # 4-100-1000 PERMIT # 70707

EARTHWORK ESTIMATES
CUT 20,193 CY
FILL 10,455 CY
NET (CUT) 10,738 CY

QUANTITIES SHOWN ARE IN PLACE ESTIMATES WITH NO ADJUSTMENTS MADE FOR SOFTENING, SUBSIDENCE OR COMPACTION LOSSES.

ENGINEER'S ESTIMATES
OFF-SITE QUANTITIES

REMOVE VERTICAL CURB, SLOTTED & SMOOTH CONCRETE SIDEWALK
REMOVE & REPLACE 4" ASPHALT
WATER LINE CONNECTIONS
SANITARY SINKER MAINS & TAPS
CONCRETE VALVE BOXES & AFRAM
CONCRETE DRAIN COPS
CONCRETE CURB & GUTTER
PREFABRICATED MANHOLE
DRAINAGE STRUCTURE
8" DUCT IRON-CLAS PIPE
8" DUCT IRON-CLAS PIPE
RANGE EXISTING PAVES

FLOODPLAIN NOTE

THE SUBJECT PARCEL LIES WITHIN FLOOD ZONE "X" HIGHS DETERMINED TO BE CHARGE THE 500-YEAR FLOODPLAIN

FLOOD INSURANCE RATE MAP (FIRM) INFORMATION					
COMMUNITY	PARCEL #	SUFFIX	DATE OF FIRM (ISSUE DATE)	FIRM ZONE	DATE FLOOD CLAY (BY NO. ZONE USE DATES)
04613	1702	C	09/30/95	X	N/A

ENGINEER'S CERTIFICATION

THE LOWEST FLOOD ELEVATION (FEET) AND FLOODPROOFING ELEVATION (FEET) ON THIS PLAN ARE SUFFICIENTLY HIGH TO PROVIDE PROTECTION FROM FLOODING CAUSED BY A ONE-HUNDRED YEAR STORM, AND ARE IN ACCORDANCE WITH CITY OF SCOTTSDALE ZONING CODE CHAPTER 17-- FLOODWAYS AND FLOODPLAIN ORDINANCE.

NO CONFLICT SIGNATURE BLOCK

Utility	Utility Company	Name of Company Representative	Telephone Number	Date Signed
Water	CITY OF SCOTTSDALE			
Electric	ARIZONA PUBLIC SERVICE	JOHN ROBERTS	602-493-4402	
Gas	CHS	CHRIS LITTON	602-430-0482	
Sanitary Sewer	SOUTHWEST GAS	SCOTT HAMILTON	602-494-8918	
Storm Sewer	CGK	SUZANNE HOLZER	602-322-7249	
Other	CITY OF SCOTTSDALE			

Engineer's Certification
I, Wayne P. Raley, being the person responsible for designing the facilities necessary to serve the requirements, hereby certify that all of the utility companies listed above, have reviewed this project and approved it. I also certify that all of the utility companies have been contacted and the necessary permits have been obtained. I also certify that all of the utility companies have been contacted and the necessary permits have been obtained. I also certify that all of the utility companies have been contacted and the necessary permits have been obtained.

CITY OF SCOTTSDALE
REVIEW AND RECOMMENDED APPROVAL BY:

NAME	DATE	TITLE
Wayne P. Raley	3/25/03	Engineer
City Engineer		
City Manager		
City Council		

APPROVED BY: *[Signature]* DATE: 3-25-03

IMPROVEMENT PLANS
FOR
MOUNTAINSIDE PLAZA
A SUBDIVISION OF A PORTION OF THE WEST HALF OF THE SOUTHEAST QUARTER OF SECTION 22, TOWNSHIP 3 NORTH, RANGE 5 EAST OF THE GLA AND SALT RIVER BASE AND MERIDIAN, MARICOPA COUNTY, ARIZONA

LOT AREA

- LOT 1 55,402.71 S.F. OR 1.27 ACRES
- LOT 2 102,287.02 S.F. OR 2.34 ACRES
- LOT 3 102,733.21 S.F. OR 2.35 ACRES
- LOT 4 25,402.65 S.F. OR 0.58 ACRES
- LOT 5 34,006.38 S.F. OR 0.78 ACRES
- LOT 6 32,788.91 S.F. OR 0.74 ACRES
- LOT 7 55,136.72 S.F. OR 1.26 ACRES
- LOT 8 35,007.40 S.F. OR 0.80 ACRES
- LOT 9 30,001.09 S.F. OR 0.69 ACRES
- LOT 10 52,204.00 S.F. OR 1.20 ACRES
- LOT 11 37,281.51 S.F. OR 0.85 ACRES
- 10 LOTS TOTAL
- NET LOT SIZE = 0.47 ACRES
- MOORAGE LOT SIZE = 1.315 ACRES

CITY OF SCOTTSDALE GENERAL CONSTRUCTION NOTES FOR PUBLIC WORKS CONSTRUCTION

- ALL CONSTRUCTION IN THE PUBLIC RIGHTS-OF-WAY OR IN ENGINEER'S DESIGNATED PUBLIC RIGHTS-OF-WAY MUST CONFORM TO THE LATEST MARICOPA SPECIFICATIONS AND STANDARD DETAILS FOR PUBLIC WORKS CONSTRUCTION AS AMENDED BY THE LATEST VERSION OF THE CITY OF SCOTTSDALE (CSC) SUPPLEMENTAL STANDARD SPECIFICATIONS AND STANDARD DETAILS. IF THERE IS A CONFLICT, THE LATEST SHALL GOVERN.
- THE ENGINEER'S DESIGN ON THESE PLANS ARE ONLY APPROVED BY THE CITY OF SCOTTSDALE AND NOT IN DETAIL. IF CONSTRUCTION QUANTITIES ARE GIVEN ON THESE PLANS, THEY ARE NOT VERIFIED BY THE CITY.
- APPROVAL OF PLANS IS VALID FOR SIX (6) MONTHS. IF AN EXTENSION OF TIME FOR THE CONSTRUCTION HAS NOT BEEN ISSUED WITHIN SIX MONTHS, THE PLANS SHALL BE RESUBMITTED TO THE CITY FOR RE-APPROVAL.
- A PUBLIC WORKS INSPECTOR SHALL INSPECT ALL WORKS WITHIN THE CITY OF SCOTTSDALE RIGHTS-OF-WAY AND IN ENGINEER'S DESIGNATED PUBLIC RIGHTS-OF-WAY PRIOR TO STARTING OF CONSTRUCTION (FOR EXISTING) AND PRIOR TO STARTING OF CONSTRUCTION (FOR NEW).
- WHenever excavation is to be done, call the "BLUE STAKE CENTER" (602) 203-1100. THIS MARKING GIVE SURFACE EXCAVATION IS TO BE DONE. THE CENTER WILL SEE THAT THE LOCATION OF THE UNDERGROUND UTILITY LINES IS IDENTIFIED FOR THE PROJECT. CALL "COLLECT" IF NECESSARY.
- STRENGTHENMENT PERMITS ARE REQUIRED FOR ALL WORK IN PUBLIC RIGHTS-OF-WAY AND ENGINEER'S DESIGNATED PUBLIC RIGHTS-OF-WAY. AN ENGINEER'S DESIGN MUST BE SUBMITTED TO THE CITY FOR REVIEW AND APPROVAL PRIOR TO THE START OF CONSTRUCTION. A PERMIT FOR THIS DESIGN MUST BE OBTAINED FROM THE CITY FOR A FEE ESTABLISHED BY THE CITY. THE DESIGN MUST BE AVAILABLE FOR INSPECTION AT ALL TIMES. FAILURE TO PROVIDE THE REQUIRED PERMITS WILL RESULT IN IMMEDIATE WORK STOPPAGE UNTIL THE PROPER PERMIT DOCUMENTATION IS OBTAINED.

GENERAL CONSTRUCTION NOTES (FOR BEST PRACTICES)

- EXISTING UNDERGROUND UTILITIES MUST BE IDENTIFIED FROM THE LATEST AVAILABLE UTILITY RECORDS AND SHOULD BE CONSIDERED APPROXIMATE IN LOCATION AND DEPTH. EXISTING UTILITIES MUST BE IDENTIFIED BY THE CITY. CALL "BLUE STAKE" A MINIMUM OF TWO (2) WORKING DAYS PRIOR TO EXCAVATION.
- ALL DIMENSIONS SHOWN ARE TO FACE OF CURB, CENTERLINE OF RIGHT-OF-WAY OR CENTERLINE OF STRUCTURE UNLESS NOTED OTHERWISE.
- CONTRACTOR SHALL OBTAIN A COPY OF THE SOIL REPORT FOR THE PROJECT PREPARED BY RICKER-JARVIS-MOORE & ASSOCIATES, ON 1/26/2001, RAN PROJ. NO. 000805.
- IT SHALL BE THE RESPONSIBILITY OF THE OWNER TO VERIFY ALL QUANTITIES INCLUDING EXCAVATION, DRAINAGE, EMBANKMENT, SHIELD OR SINK, GROUND CONSTRUCTION, AND ANY OTHER ITEMS. CONTRACTOR AND SOIL TO EXCAVATE OR TO THE ELEVATIONS SHOWN ON THE PLANS AND TO BE MADE BY THE OWNER. THE OWNER SHALL BE RESPONSIBLE FOR THE ACCURACY OF THE INFORMATION PROVIDED. IT SHALL BE THE OWNER'S RESPONSIBILITY TO NOTIFY THE ENGINEER PRIOR TO CONSTRUCTION OF ANY MAJOR DISCREPANCIES.
- ALL STAKING CONTROL SHALL BE LEFT UNDISTURBED. THE CONTRACTOR SHALL ONLY USE THE ENGINEER TO REFERENCE AND RESET ANY CONTROL POINTS THAT HAVE TO BE DISTURBED. THE CONTRACTOR SHALL BE CHARGED FOR ANY RESETTING OF ANY CONTROL POINTS ARE DESTROYED.
- ANY QUESTIONS RAISED RELATIVE TO THE ACCURACY OF ANY IMPROVEMENT INSTALLATION SHALL NOT BE RAISED SUBSEQUENT TO COMPLETION OF THE WORK UNLESS ALL SURVEY STAKES ARE MAINTAINED INTACT. SHOULD SUCH STAKES NOT BE PRESENT AND REPAIRED AS TO THEIR QUALITY, NO CLAIM FOR ADDITIONAL COMPENSATION FOR CORRECTION SHALL BE PRESENTED TO ANY PARTY AND SUCH WORK SHALL BE CORRECTED BY THE CONTRACTOR AT HIS EXPENSE.
- ALL BUILDING ROOFS SHALL BE DRAINED BY INTERNAL UNDERGROUND ROOF DRAIN LEADERS.
- PROVIDE 6% SLOPE AWAY FROM WALK OR CURB FOR 5' FEET ALONG ALL STREETS.

LEGAL DESCRIPTION

PARCEL NO. 1:
A PORTION OF THE WEST HALF OF THE SOUTHEAST QUARTER OF SECTION 22, TOWNSHIP 3 NORTH, RANGE 5 EAST OF THE GLA AND SALT RIVER BASE AND MERIDIAN, MARICOPA COUNTY, ARIZONA, DESCRIBED AS FOLLOWS:

COMMENCING AT THE SOUTH QUARTER CORNER OF SAID SECTION 22; THENCE NORTH 71°15'30"E ALONG THE NORTH-SOUTH 1/4-SECTION LINE OF SAID SECTION 22, A DISTANCE OF 64.94 FEET;

THENCE NORTH 71°15'30"E ALONG A LINE 30.00 FEET OF 1/4-SECTION 22, A DISTANCE OF 30.00 FEET TO THE POINT OF BEGINNING;

THENCE NORTH 71°15'30"E ALONG THE NORTH-SOUTH 1/4-SECTION LINE OF SAID SECTION 22, A DISTANCE OF 71.93 FEET TO A POINT ON THE WEST LINE OF SAID SECTION 22, A DISTANCE OF 64.94 FEET;

THENCE NORTH 71°15'30"E ALONG THE WEST LINE OF SAID SECTION 22, A DISTANCE OF 440.00 FEET TO THE SOUTHWEST CORNER OF SAID PARCEL;

THENCE NORTH 71°15'30"E, A DISTANCE OF 71.93 FEET TO THE POINT OF BEGINNING.

PARCEL NO. 2:
A PORTION OF THE WEST HALF OF THE SOUTHEAST QUARTER OF SECTION 22, TOWNSHIP 3 NORTH, RANGE 5 EAST OF THE GLA AND SALT RIVER BASE AND MERIDIAN, MARICOPA COUNTY, ARIZONA, DESCRIBED AS FOLLOWS:

COMMENCING AT THE SOUTH QUARTER CORNER OF SAID SECTION 22; THENCE NORTH 71°15'30"E ALONG THE NORTH-SOUTH 1/4-SECTION LINE OF SAID SECTION 22, A DISTANCE OF 64.94 FEET;

THENCE NORTH 71°15'30"E, A DISTANCE OF 30.00 FEET TO THE POINT OF BEGINNING;

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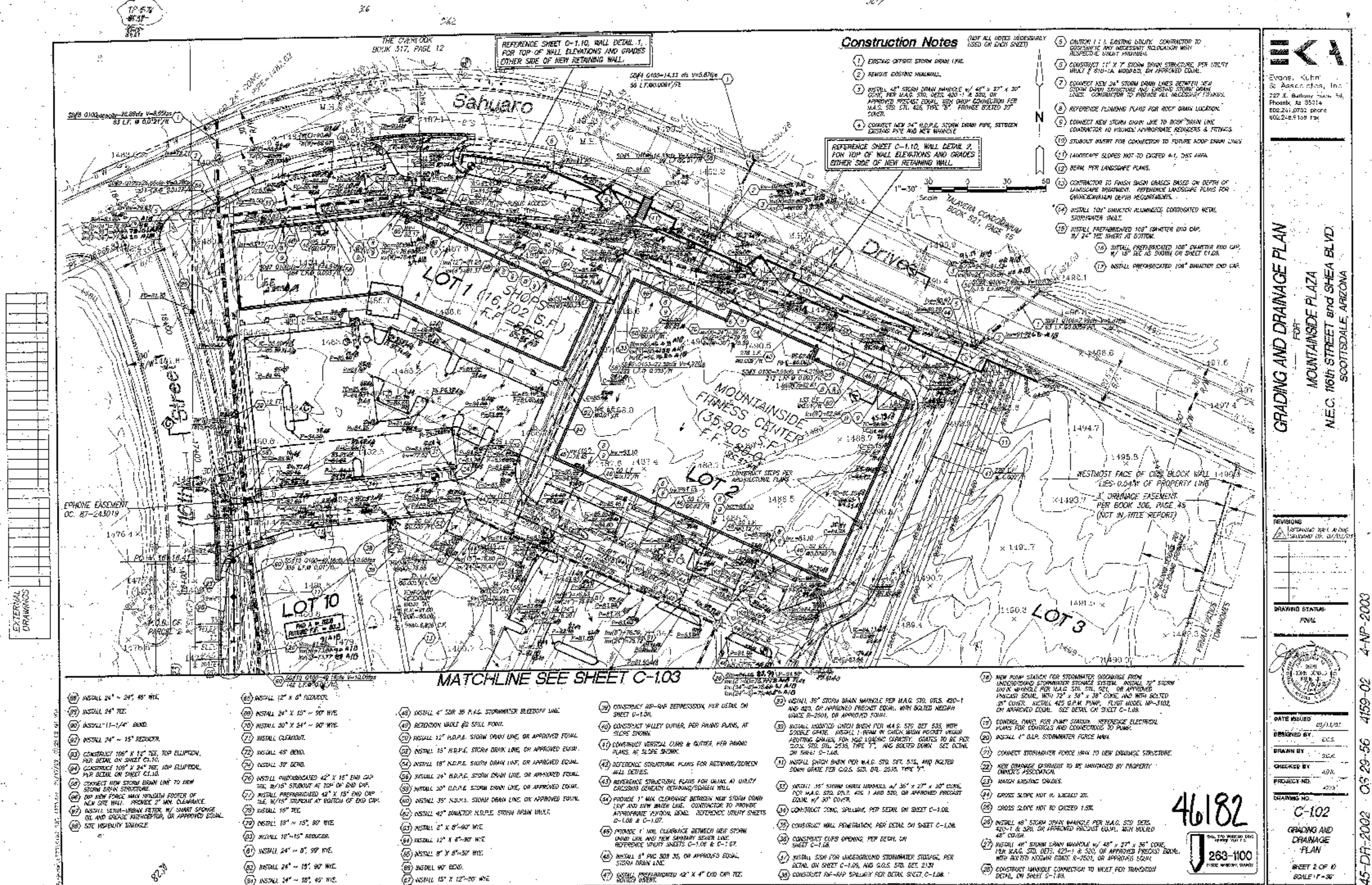
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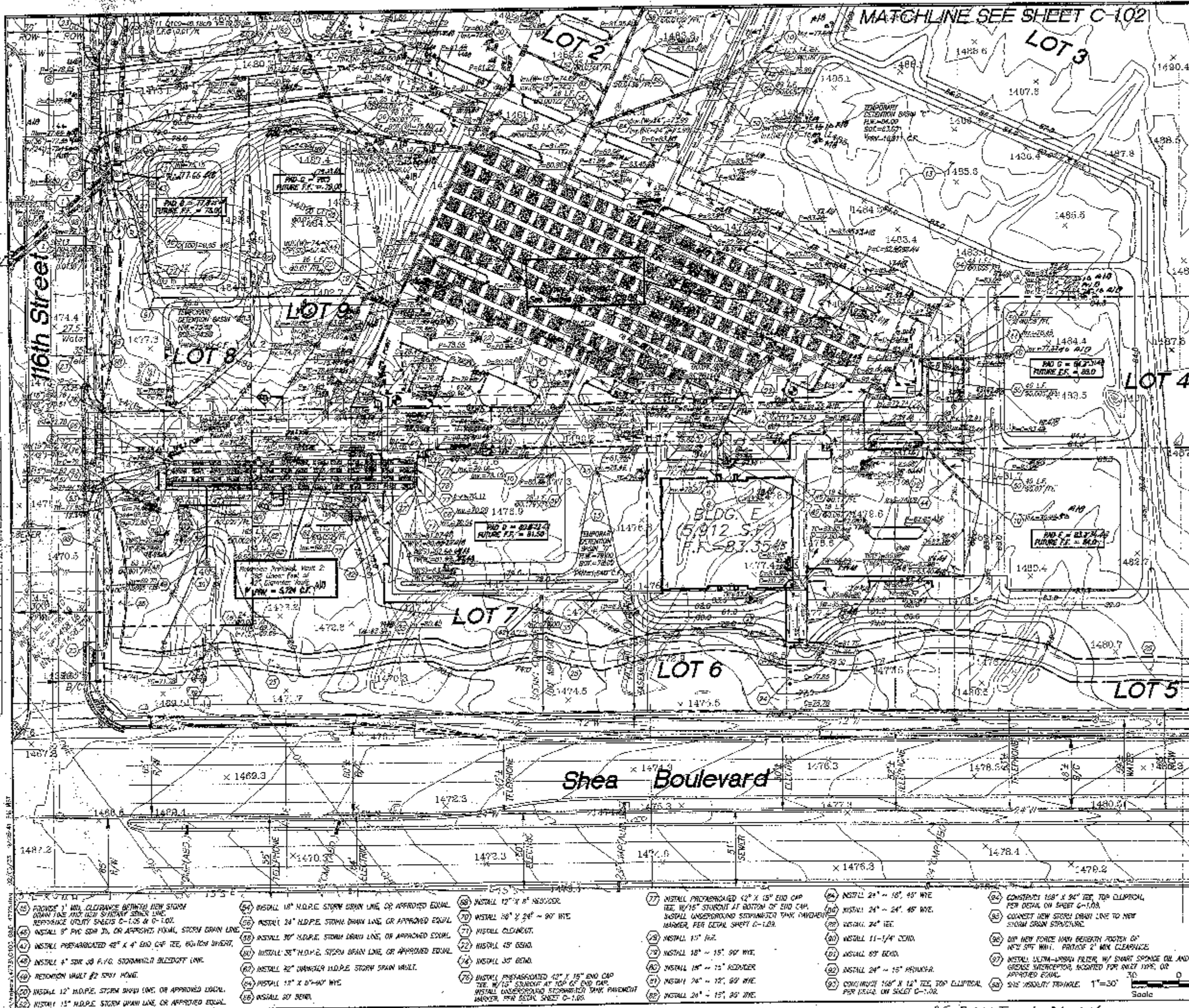
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- 0-5.50 DRAINAGE



- ### Construction Notes
- (NOT ALL NOTES NECESSARILY USED ON EACH SHEET)
- EXISTING OFFSITE STORM DRAIN LINE
 - REMOVE EXISTING MAINWALL
 - INSTALL 48" STORM DRAIN MAINWALL W/ 48" x 27" x 30" CONE PER M.A.G. STD. DETS. 420-1 & 520, ON APPROVED PRECAST EQUAL WITH DRAIN CONNECTION PER M.A.G. STD. DET. 425, TYPE "B", PROVIDE BOLTED JOINT COVER
 - CONNECT NEW 24" H.D.P.E. STORM DRAIN PIPE, SETBACK EXISTING PIPE AND NEW MAINWALL
 - CAUTION !!! EXISTING UTILITY. CONTRACTOR TO DETERMINE ANY NECESSARY RELOCATION WITH RESPECTIVE UTILITY PROVIDER.
 - CONSTRUCT 11" x 7" STORM DRAIN STRUCTURE PER UTILITY WHAT IS 810-1A, MODIFIED, OR APPROVED EQUAL.
 - CONNECT NEW 24" STORM DRAIN LINES BETWEEN NEW STORM DRAIN STRUCTURE AND EXISTING STORM DRAIN LINES. CONTRACTOR TO PROVIDE ALL NECESSARY FITTINGS.
 - REFERENCE PLUMBING PLANS FOR ROOF DRAIN LOCATION.
 - CONNECT NEW STORM DRAIN LINE TO ROOF DRAIN LINE. CONTRACTOR TO PROVIDE APPROPRIATE REDUCERS & FITTINGS.
 - STUBOUT INVERT FOR CONNECTION TO FUTURE ROOF DRAIN LINES.
 - LANDSCAPE SLOPES NOT TO EXCEED 4:1, THIS AREA.
 - BERM PER LANDSCAPE PLANS.
 - CONTRACTOR TO FINISH BASH GRASSES BASED ON DEPTH OF LANDSCAPE TREATMENT. REFERENCE LANDSCAPE PLANS FOR GRASS/SEDIMENTATION DEPTH REQUIREMENTS.
 - INSTALL 104" DIAMETER ALLOWED, CONTINGENT METAL STORMWATER INLET.
 - INSTALL PREFABRICATED 108" DIAMETER END CAP, W/ 24" TEE INVERT AT BOTTOM.
 - INSTALL PREFABRICATED 108" DIAMETER END CAP W/ 18" TEE AS SHOWN ON SHEET C-102.
 - INSTALL PREFABRICATED 108" DIAMETER END CAP.

- EXTERNAL DRAWINGS
- LOT 1 (16 SHOPS, 102 S.F.)
- LOT 2 (35,905 S.F.)
- LOT 3
- LOT 10
- MATCHLINE SEE SHEET C-103
- 46182
- 263-1100
- 45-DR-2002 05-29-06 4159-02 4-1P-2003
- 10-UP-2019 09/26/2019



- Construction Notes** (NOT ALL NOTES NECESSARILY USED ON EACH SHEET)
- EXISTING OFFSITE STORM DRAIN LINE.
 - RETAIN EXISTING HEADWALL.
 - INSTALL 48" STORM DRAIN MANHOLE W/ 48" x 27" x 36" CONE, PER M.A.S. STD. DET. 420-1 & 320, CH. APPROVED PRECAST EQUAL, WITH DRAIN CONNECTION PER M.A.S. STD. DET. 428, TYPE "T". PROVIDE BOLTED JOINT COVER.
 - CONNECT NEW 24" H.D.P.E. STORM DRAIN PIPE BETWEEN EXISTING PIPE AND NEW MANHOLE.
 - CONDUCT 11.1 ENVELOPE UTILITY, CONTRACTOR TO COORDINATE ANY NECESSARY RELOCATION WITH NEARBY UTILITY PROVIDER.
 - CONSTRUCT 11" x 7" STORM DRAIN STRUCTURE PER UTILITY W/ 11" x 7" LA, MODIFIED OR APPROVED EQUAL COVER TO BE WEATHERED AND BOLTED SHUT.
 - CONNECT NEW 24" STORM DRAIN LINES BETWEEN NEW STORM DRAIN STRUCTURE AND EXISTING STORM DRAIN LINES. CONTRACTOR TO PROVIDE ALL NECESSARY FITTINGS.
 - MAINTAIN EXISTING PLANS FOR NEW DRAIN LOCATION.
 - CONNECT NEW STORM DRAIN LINE TO ROOF DRAIN LINE. CONTRACTOR TO PROVIDE APPROPRIATE REDUCERS & FITTINGS.
 - STAKEOUT MARK FOR CONNECTION TO FUTURE ROOF DRAIN LINES.
 - LANDSCAPE SLOPES NOT TO EXCEED 4:3, THIS AREA.
 - DETAILED PER LANDSCAPE PLANS.
 - CONTRACTOR TO FINISH DRAIN GRADES BASED ON DEPTH OF LANDSCAPE TREATMENT. REFER TO LANDSCAPE PLANS FOR OVERLAP/STITCH DEPTH REQUIREMENTS.
 - INSTALL 108" DIAMETER UNARMED CORRUGATED METAL STORM DRAIN MANHOLE. SEE DETAILS ON SHEET C-103.
 - INSTALL PRECAST/BOULDER 108" DIAMETER END CAP, W/ 24" I.D. INVERT AT BOTTOM.
 - INSTALL MANHOLE/BOULDER 108" DIAMETER END CAP, W/ 18" TEE AT BOTTOM ON SHEET C-103.
 - INSTALL PRECAST/BOULDER 108" DIAMETER END CAP.
 - NEW PUMP STATION FOR STORMWATER DISCHARGE FROM UNDERGROUND STORMWATER STORAGE SYSTEM. INSTALL 72" STORM DRAIN MANHOLE PER M.A.S. STD. DET. 321, OR APPROVED PRECAST EQUAL, WITH 72" x 30" x 36" CONE, AND WITH BOLTED 36" COVER. INSTALL 425 G.P.M. PUMP, FLIGHT MODEL HP-3102 OR APPROVED EQUAL. SEE DETAIL ON SHEET C-103.
 - CONTRACTOR PANEL FOR PUMP STATION. REFERENCE ELECTRICAL PLANS FOR CONTROLS AND CONNECTIONS TO PUMP.
 - INSTALL 4" G.P.I. STORMWATER FORCE MAIN.
 - CONSTRUCT STORMWATER FORCE MAIN TO NEW UPGRADE STRUCTURE.
 - NEW DRAINAGE BASIN.
 - MATCH EXISTING GRADES.
 - GRDES SLOPE NOT TO EXCEED 2%.
 - CROSS SLOPE NOT TO EXCEED 1.5%.
 - INSTALL 48" STORM DRAIN MANHOLE PER M.A.S. STD. DET. 420-1 & 320, OR APPROVED PRECAST EQUAL, WITH BOLTED 48" COVER FOR HEADWALL IN-SCOPS, OR APPROVED EQUAL.
 - INSTALL 48" STORM DRAIN MANHOLE W/ 48" x 27" x 36" CONE, PER M.A.S. STD. DET. 420-1 & 320, OR APPROVED PRECAST EQUAL, WITH BOLTED METAL GRATE R-2501, OR APPROVED EQUAL.
 - CONSTRUCT MANHOLE CONNECTION TO VALVE PER THAMSON DETAIL, ON SHEET C-103.
 - INSTALL 36" STORM DRAIN MANHOLE PER M.A.S. STD. DET. 420-1 AND 320, OR APPROVED PRECAST EQUAL, WITH BOLTED METAL GRATE R-2501, OR APPROVED EQUAL.
 - INSTALL MANHOLE CATCH BASIN PER M.A.S. STD. DET. 535, WITH DOUBLE GRATE. INSTALL 1'-6" IN CATCH BASIN ROCKET. RAINING GRATES, FOR 100% CATCHING CAPACITY. GRATES TO BE PER C.O.S. STD. DET. 1523, TYPE "T", AND BOLTED DOWN. SEE DETAIL ON SHEET C-103.
 - INSTALL CATCH BASIN PER M.A.S. STD. DET. 535, AND BUILD DOWN GRATE C.O.S. STD. DET. 2036, TYPE "T".
 - CONSTRUCT NEW CATCH BASIN TO INLET, PER DETAIL ON SHEET C-103.
 - NOT USED.
 - CONSTRUCT CONC. SALLYWAY, PER DETAIL ON SHEET C-103.
 - CONSTRUCT WALL PENETRATION, PER DETAIL ON SHEET C-103.
 - CONSTRUCT CURB OPENING, PER DETAIL ON SHEET C-103.
 - NOT USED.
 - CONSTRUCT RP-RAP SPRINKLER PER DETAIL ON SHEET C-103.
 - CONSTRUCT RP-RAP DEPRESSION, PER DETAIL ON SHEET C-103.
 - CONSTRUCT VALLEY CUTTER, PER RAVING PLANS, AT SLOPE SHOWN.
 - CONSTRUCT VERTICAL CURB & GUTTER, PER RAVING PLANS, AT SLOPE SHOWN.
 - REFERENCE STRUCTURAL PLANS FOR RETAINING/SCREEN WALL DETAILS.
 - REFERENCE STRUCTURAL PLANS FOR DETAIL AT UTILITY CROSSING BENEATH RETAINING/SCREEN WALL.
 - PROVIDE 1' MIN. CLEARANCE BETWEEN NEW STORM DRAIN LINE AND NEW WATER LINE. CONTRACTOR TO PROVIDE APPROPRIATE VERTICAL SEED. REFERENCE UTILITY SHEETS C-108 & C-107.

Evans, Kuhn & Associates, Inc.
227 E. Bellway Home Rd.
Phoenix, AZ 85014
602.241.0782 phone
602.241.0165 fax

GRADING AND DRAINAGE PLAN
FOR
MOUNTAINSIDE PLAZA
N.E.C. 16th STREET and SHEA BLVD.
SCOTTSDALE, ARIZONA

46183

PROVISIONS

DRAWING STATUS:
FINAL SUBMITTAL

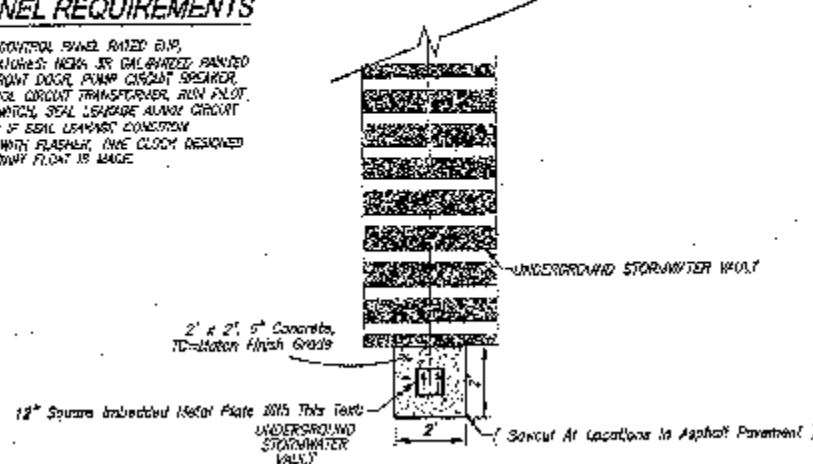
DATE ISSUED: 2/12/24
DESIGNED BY: H.P.R./E.C.S.
DRAWN BY: H.A.C.
CHECKED BY: H.P.R.
PROJECT NO.: 4173
DRAWING NO.: C-103
GRADING AND DRAINAGE PLAN
SHEET 3 OF 10
SCALE: 1" = 30'

45-DR-2002 03-29-56 4150-02

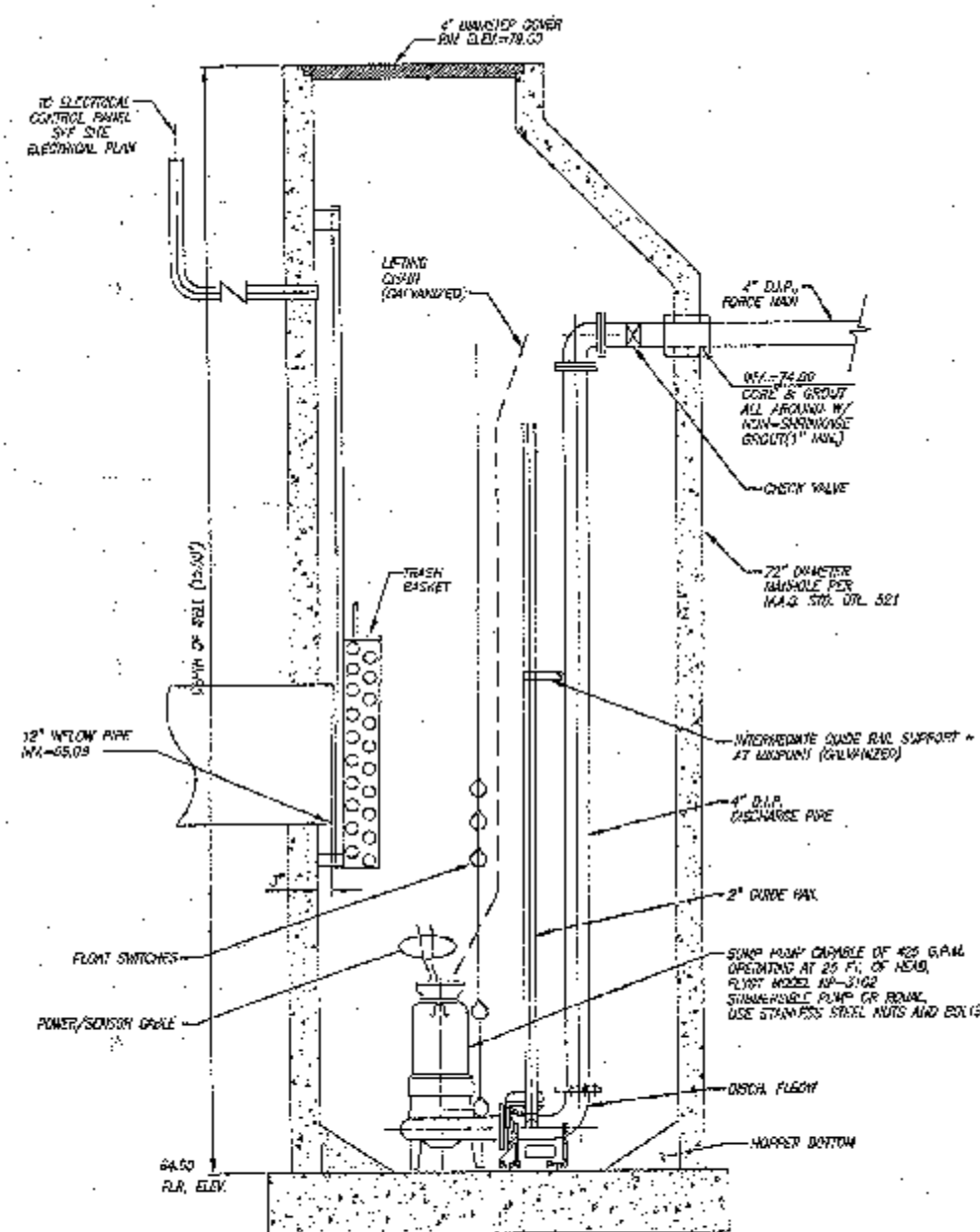
EXTERNAL
DRAWINGS

A5-3014T 1-21-04

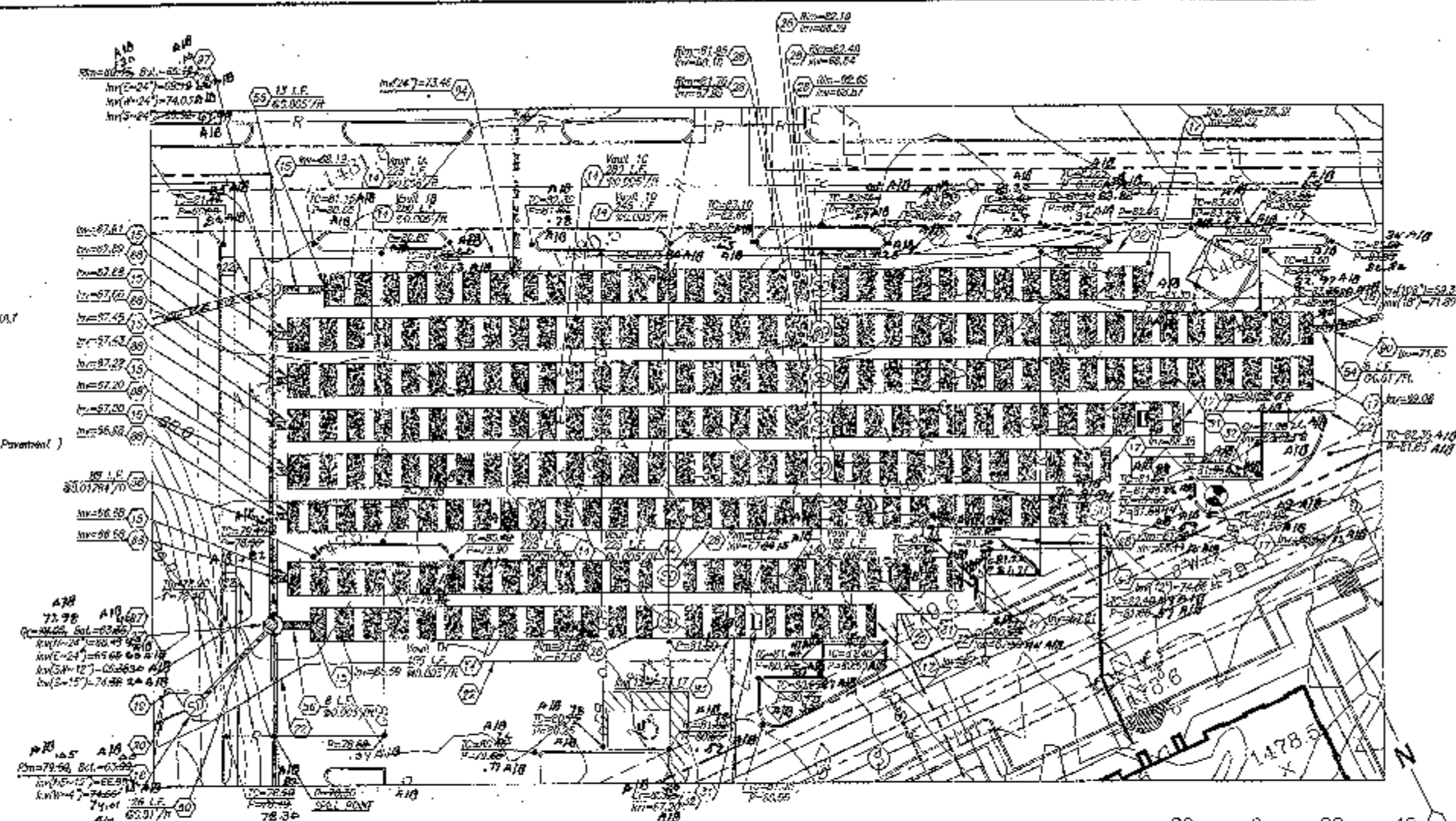
1. THE REMOTE CONTROLS SWIMWAY PUMP CONTROL SWAGE BRASS BUSH,
3/8X3/4X1/2 WITH THE FOLLOWING FEATURES: NEMA 3R GALVANIZED PAINTED
ENCLOSURE, BRASS ALUMINUM WEIGHT DOOR, PUMP CIRCUIT BREAKER,
NEMA 4X12 MOTOR STARTER, CONTROL CIRCUIT TRANSFORMER, RUN PILOT
LIGHT, HAND-OFF-AUTO SELFLOCK SWITCH, SEAL LEAKAGE ALARM CIRCUIT
WITH PILOT LIGHT, LOGS (10 DISHING PUMP) & BELL LEAKAGE CONTROL
DOORS, HIGH WATER ALARM LIGHT WITH FLASHER, (ONE CLOSURE DESIGNED
FOR TWO HOSE) THE DRAIN DRAIN SWIMWAY PUMP IS MADE.



**STORMWATER VAULT
PAVEMENT MARKER DETAIL**



STORMWATER VAULT SYSTEM #1
DISCHARGE PUMP STATION DETAIL



RETENTION VAULT SYSTEM #1 DETAIL

- CONTRACTOR NOTE:**

10" DIAMETER CORRUGATED METAL PIPE (CMP) SHALL MEET THE FOLLOWING REQUIREMENTS:

 - PIPE SHALL BE 10" DIAMETER ALUMINUM CLAD, 20% 74 GA. W/ MIN. 3" CORRUGATIONS.
 - CONTRACTOR SHALL SUBMIT SHOP DRAWINGS WHICH DEMONSTRATE THAT THE PROPOSED PIPE WALL THICKNESS, CORRUPTION OR DEPTH, RESIDUAL WALL THICKNESS, IS ADEQUATE TO SUPPORT H=20 LADING WITH A TWO-FOOT TO FOUR-FOOT HEIGHT OF COVER.
 - WELD FIELD JOINTS IN CORRUGATED METAL PIPE WITH WELDED JOINTS CONNECTING BANCHES OF THE SAME SIZE AND TREATED METAL, AS PER.

14. INSTALL 10" DIAMETER ALUMINUM CORRUGATED METAL ELONGATION JOINT.

15. INSTALL PREPARED 10" DIAMETER END CAP, W/ 24" TEE WHEAT AT BOTTOM, WITH UNDERGROUND STORMWATER DRAIN FLOWMETER MARKER, PER DETAIL THIS SHEET.

16. INSTALL PREPARED 10" DIAMETER END CAP, W/ 18" TEE WHEAT AS SHOWN, WITH UNDERGROUND STORMWATER DRAIN FLOWMETER MARKER, PER DETAIL THIS SHEET.

17. INSTALL POLYPROPYLENE 10" DIAMETER END CAP, WITH UNDERGROUND STORMWATER DRAIN FLOWMETER MARKER, PER DETAIL THIS SHEET.

18. NEW PUMP STATION FOR STORMWATER DISCHARGE FROM UNDERGROUND STORMWATER STORAGE SYSTEM. MINIMUM 72" STORM DRAIN MANHOLE PER M.A.S. STD. DET. 122, OR APPROVED EQUIVALENT EQUAL, WITH 72" x 36" x 36" CONE AND WITH SLOTTED 36" COVER, MINIMUM 426 CUM. SUBMERSIBLE PUMP, PUMP HOIST 10'-31/2', OR APPROVED EQUAL, SEE DETAIL ON THIS SHEET.

19. CONTROL PANEL FOR PUMP STATION. REFERENCE ELECTRICAL PLANS FOR CONNECTIONS AND CONNECTIONS TO PUMP.

20. INSTALL 4" O.D. STORMWATER FORCE MAIN.

22. NEW DRAINAGE ASSEMBLY.

30. INSTALL 18" STORM DRAIN MANHOLE PER M.A.S. STD. DETS. 120-1 & 122, OR APPROVED EQUIVALENT, WITH SLOTTED 48" COVER, PER MESHAN S-5003, OR APPROVED EQUAL.

37. INSTALL 48" STORM DRAIN MANHOLE 6' 48" x 24" x 36" CONE, PER M.A.S. STD. DETS. 122-1 & 122, OR APPROVED EQUIVALENT, WITH SLOTTED 48" COVER, PER MESHAN S-5003, OR APPROVED EQUAL.

38. CONSTRUCT WHEAT ACCESS MANHOLE PER WHEAT ACCESS DETAIL ON THIS SHEET.

39. INSTALL CATCH BASIN PER M.A.S. STD. DET. 122, AND SLOTTED DOWN SPATE C.O.S. STD. DET. 122, TYPE "F".

40. CONSTRUCT CATCH BASIN CONNECTION TO WHEAT FOR TRANSDUCER DETAIL ON THIS SHEET.

41. INSTALL 12" H.O.P.E. STORM DRAIN LINE OR APPROVED EQUAL.

44. INSTALL 18" H.O.P.E. STORM DRAIN LINE, OR APPROVED EQUAL.

46. INSTALL 24" H.O.P.E. STORM DRAIN LINE, OR APPROVED EQUAL.

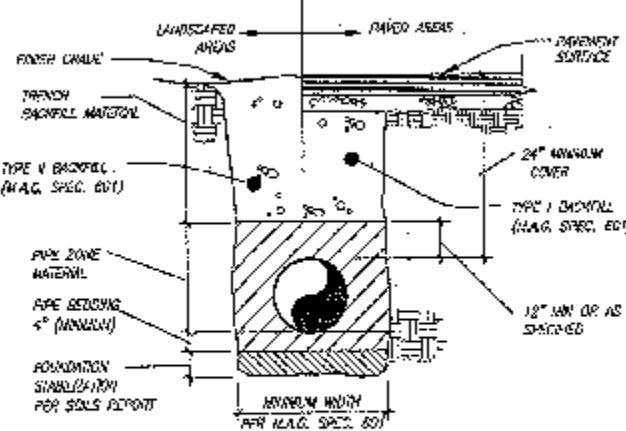
48. INSTALL 18" TEE.

50. INSTALL 24" TEE.

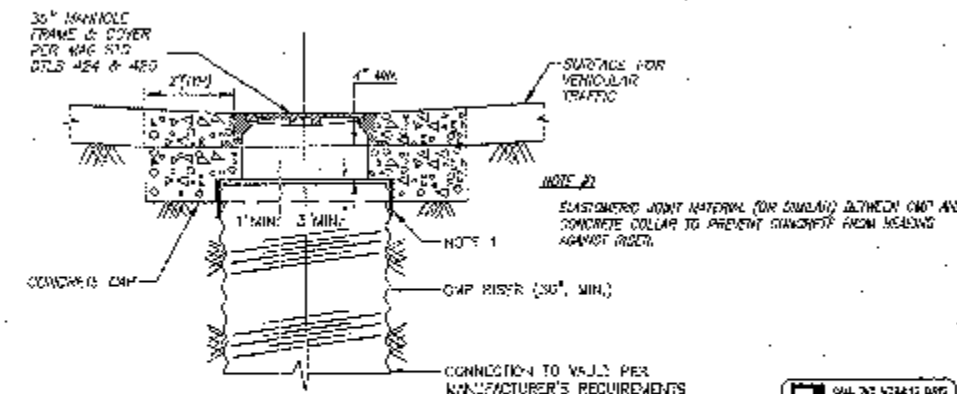
53. CONSTRUCT 108" x 12" TEE, TOP ELUTION, PER DETAIL ON SHEET C1.10.

54. CONSTRUCT 108" x 24" TEE, TOP ELUTION, PER DETAIL ON SHEET C1.10.

57. INSTALL ULTRA-URBAN FILTER, W/ SLOTTED SPACER OR AND DRAINAGE INTRUSION, OR APPROVED EQUAL.



TYPICAL PVC/HDPE STORM DRAIN TRENCH DETAIL

**VAULT ACCESS DETAIL**

46190

Call 767-4722-17 or
800-672-2633

263-1100
FREE HOMEOWNERS COUNCIL

Evans, Kunn
& Associates, Inc.
227 E. Bethany Home Rd.
Phoenix, Az. 85014
EO2-941-5787 drgmk
EO2-248-9159 luj

STORMWATER VAULT SYSTEM DETAILS

FOR
MOUNTAINSIDE PLAZA
N.E.C. 116th STREET and SHEA BLVD.
SCOTTSDALE, ARIZONA

4-WF-2003

4159-02

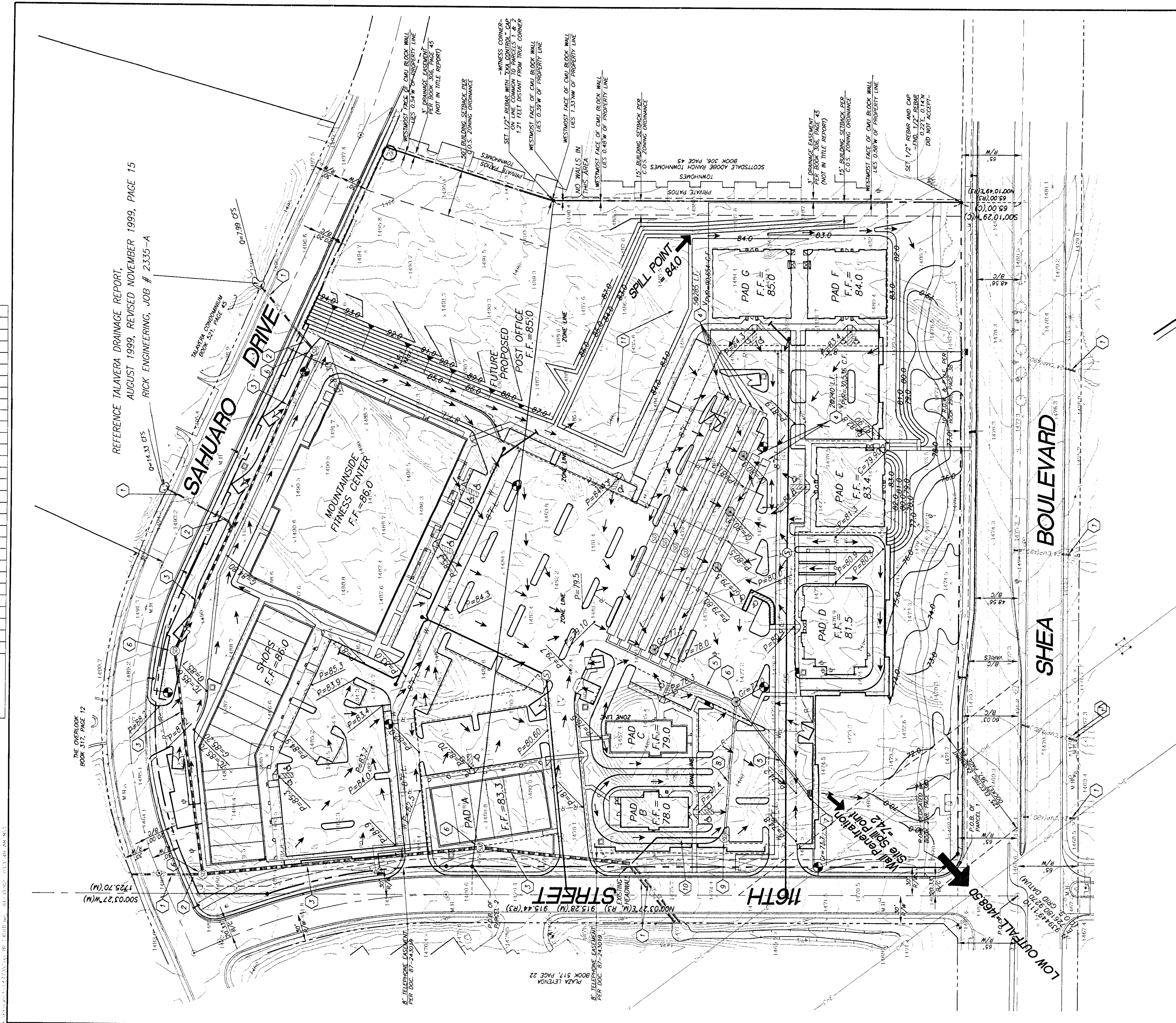
OS 29-58

45-DR-2002

10-UP-2019

09/26/2019

EXTERNAL
DRAWINGS



DRAINAGE CALCULATIONS

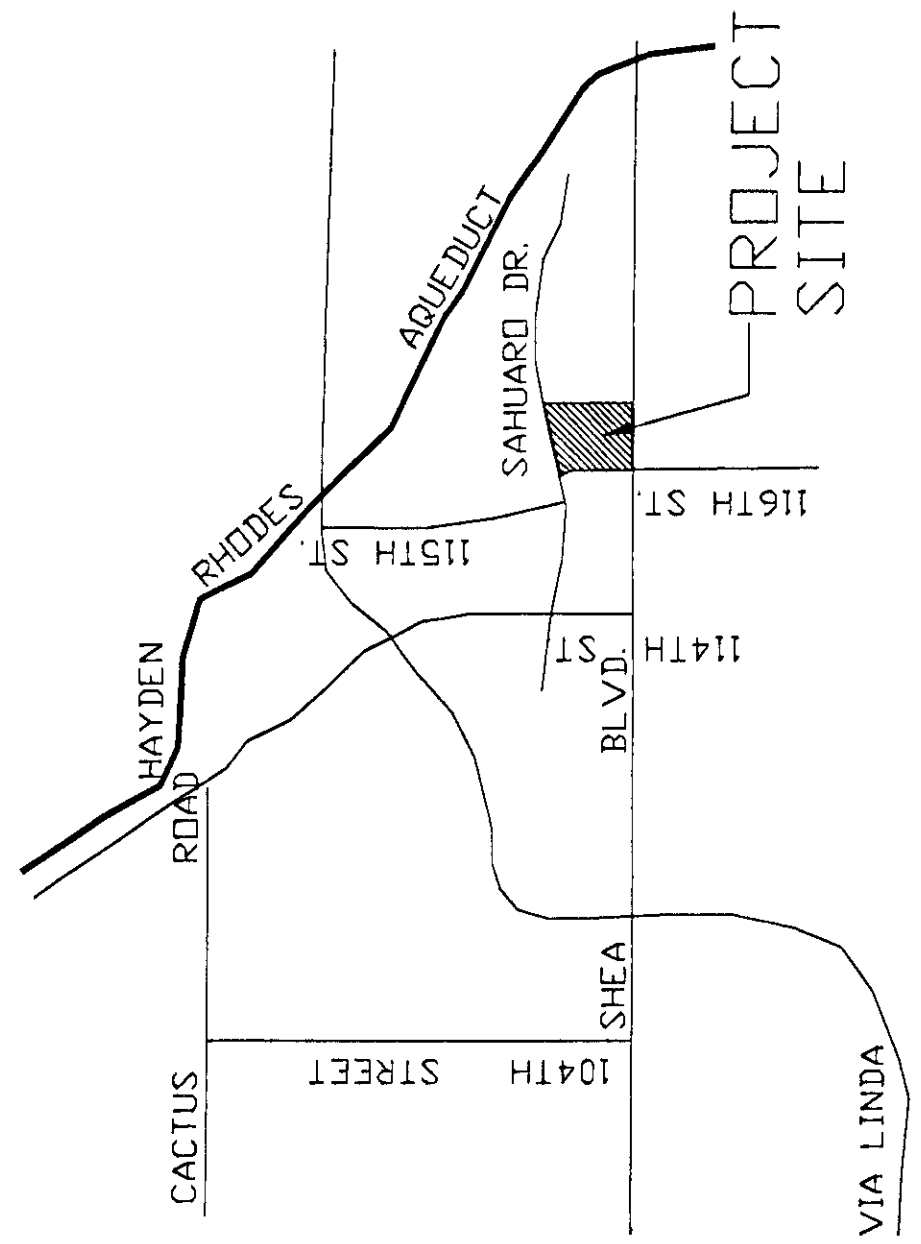
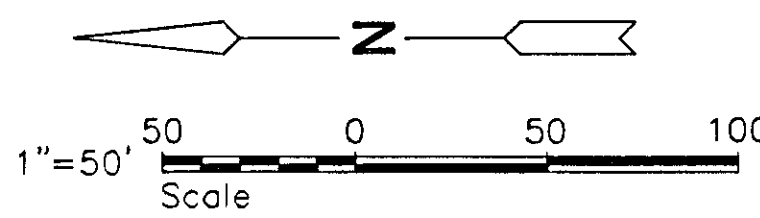
Retention Required, Undeveloped Northeast Landscape Area Site.
V REQ = $(2.82/12) \times 90,000 \times 0.50$
V REQ = 10,500 CF
Retention Provided, Landscape Basin:
V PROV = 10,500 CF

Retention Required, Total Developed Site:
V REQ = $(6/12) \times A \times C$
A = 572,992 S.F.
C = 0.50
V REQ = $(6/12) \times 572,992 \times 0.50$
V REQ = 143,248 CF

Retention Provided:
Total, 9' Diameter Retention Vaults @ 285 L.F.
 $(4.5)' \times 3,1416 \times 285 \times 5 = 90,654 \text{ C.F.}$
(2) 9' Diameter Retention Vaults @ 240 L.F.
 $(4.5)' \times 3,1416 \times 240 \times 2 = 30,536 \text{ C.F.}$
121,190 C.F.
V PROV = 121,190 CF

NOTES:

- EXISTING OFFSITE STORM DRAIN LINE.
- CONNECT OFFSITE STORM DRAIN LINE TO NEW STORM DRAIN LINE.
- NEW STORM DRAIN LINE FOR OFFSITE FLOWS.
- NEW 9" DIAMETER STORMWATER RETENTION VAULT.
- NEW STORM DRAIN LINE FOR ONSITE.
- NEW STORM DRAIN MANHOLE WITH GRATE.
- NEW CATCH BASIN.
- NEW STORM DRAIN LINE TO STORMWATER PUMP DISCHARGE.
- NEW STORM DRAIN MANHOLE WITH STORMWATER DISCHARGE PUMP.
- NEW FORCE MAIN.
- 1" DEEP LANDSCAPE RIMOFF DETENTION BASIN.



VICINITY MAP
NO SCALE

PRELIMINARY

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& Associates,
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Phoenix, AZ 85014
602.241.0782 phone
602.248.9158 fax
EKA@Worldnet.att.net

CONCEPTUAL GRADING & DRAINAGE PLAN FOR RETAIL CENTER - 116TH ST. & SHEA

REVISIONS

DRAWING STATUS

EXHIBIT



DATE ISSUED 05/11

DESIGNED BY E.C.S.

DRAWN BY R.A.C./C

CHECKED BY W.P.R.

PROJECT NO. 4773

DRAWING NO.

Conceptual
Grading
and
Drainage
Plan

Scale:
1" = 50'

45-DR-2002

253-PA-01
6/4/02

10-UP-2019
09/26/2019



Evans, Kuhn &
Associates, Inc.

727 E. Bethany Home Road

Suite D225

Phoenix, AZ 85014

602.241.0782 phone

602.248.9158 fax

DRAINAGE REPORT
For
116TH STREET & SHEA RETAIL CENTER
SCOTTSDALE, ARIZONA

Prepared for:

CITY OF SCOTTSDALE
7447 East Indian School Road
Scottsdale, Arizona 85251

Prepared by:

EVANS, KUHN AND ASSOCIATES, INC.
727 East Bethany Home Road, Suite D-225
Phoenix, Arizona 85014



EKA #4773
December 11, 2001
Revised May 9, 2002

253-PA-2001
6/4/2002

engineers

45-DR-2002

10-UP-2019

09/26/2019

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Off-site Drainage Flows	1
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Summary	2
Appendix A: Concept Grading and Drainage Plan	



Project Description

The existing 13.15-acre site is currently vacant and occupies the northeast corner of 116th Street and Shea Boulevard. Residential properties currently existing on both the north and east, and a commercial warehouse storage facility occupies the property to the west. The proposed development consists of seven individual commercial pads, a pad with retail shops, a Mountainside Fitness Center, and a future proposed Post Office Facility.

Existing Conditions

The existing site slopes to the southwest at an average slope of 3%; losing almost 30' of elevation between the site high and low points. Three existing 24" pipe culverts along the northern boundary empty retention basins from upstream developments onto the site. These off-site flows following existing drainageways through the site, with flows exiting the site via two existing 28" pipe culverts under 116th Street near the southwest corner of the site. The remaining site flows sheet flow or follow shallow drainage swales to ponding locations along the north side of Shea Boulevard. Storm drain crossings under Shea were terminated with the last widening project, so currently there is no outfall for the ponded water in shallow basins.

Off-site Drainage Flows

The two easternmost 24" storm drains under Sahuaro Drive drain retention basins for the Talavera Condominiums which abut the site to the north. From the grading and drainage plans for the development, these 100-year pipe flows are shown to be: 7.99 cfs, and 14.33 cfs. The westernmost 24" storm drain under Sahuaro Drive drain a retention basin for the Overlook Condominiums. Record drawings for this older development were not available, but site observation indicates it drains a retention basin, similar to the other pipes on Sahuaro Drive. With a similar sloping pipe, it has been assumed that the 100-year pipe flow would not be in excess of 15 cfs. The three off-site pipe flows are proposed to be collected in a new underground pipe and directed to the 28" outfall pipes under 116th Street as currently occurs.

On-site Grading and Drainage

The proposed grading concept essentially maintains existing flow patterns. Roof drainage will be primarily by above ground splash blocks to surface drain to new catch basins located throughout

the paved parking areas within the development. ADA grades are maintained within the accessible areas and grades not exceeding 4% used elsewhere on the site. These slopes were not conducive to developing retention basin, therefore an underground storage system was utilized for site runoff for the 100-year, 2-hour storm. A stormwater discharge pump will be used to empty the storage vaults within 36 hours through the 116th Street underground pipe culverts.

The future Post Office Facility in the northeast portion of the site will remain undisturbed, except for the grading/berming necessary to create a temporary retention basin for its runoff. Upon development, flows will be directed into the underground vault system, which has been sized for future flows. Reference Appendix A for the Concept Grading and Drainage Exhibit and retention calculations.

Summary

Existing storm water flows will be maintained after site development. Off-site flows intercepted by the property will be rerouted through the site. Underground stormwater storage vaults provide 121,190 cf of storage, and exceed the 100-year, 2-hour retention requirement of 121,188 cf. The building finished floor elevations are all elevated sufficiently above the high top of curb along the abutting streets and well above the low outfall elevation of 1268.5, just northeast of the intersection of 116th Street and Shea Boulevard.

APPENDIX C
(FEMA FIRM Map)

National Flood Hazard Layer FIRMette



33°35'20.51"N



0 250 500 1,000 1,500 2,000 Feet

1:6,000

33°34'50.54"N

USGS The National Map: Orthoimagery. Data refreshed April, 2019.

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
		Area of Undetermined Flood Hazard Zone D
GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance
		17.5 Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
MAP PANELS		Digital Data Available
		No Digital Data Available
		Unmapped



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **8/21/2019 at 11:43:46 AM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

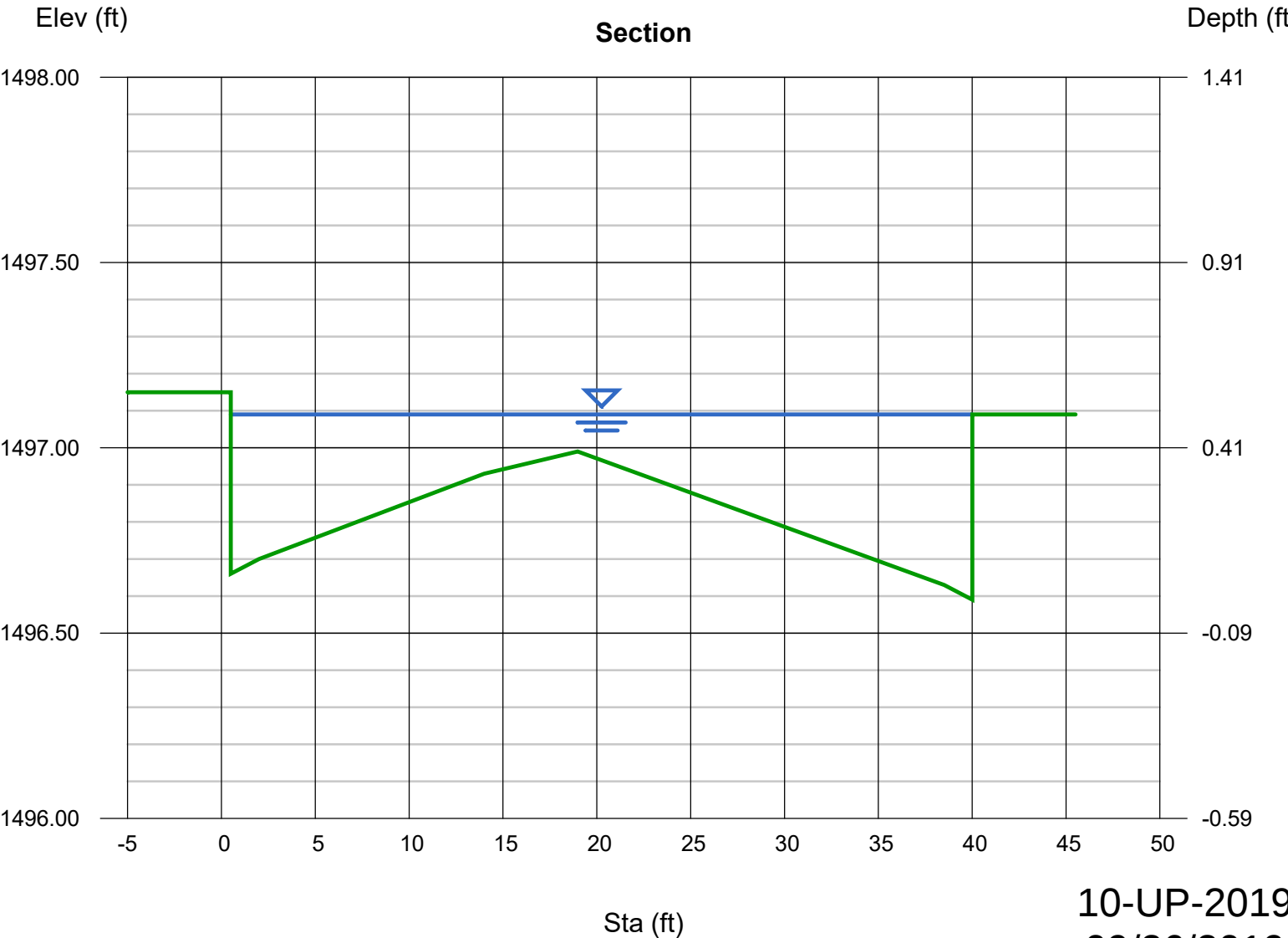
10-UP-2019
09/26/2019

APPENDIX D
(Sahuaro Drive Capacity Calculations)

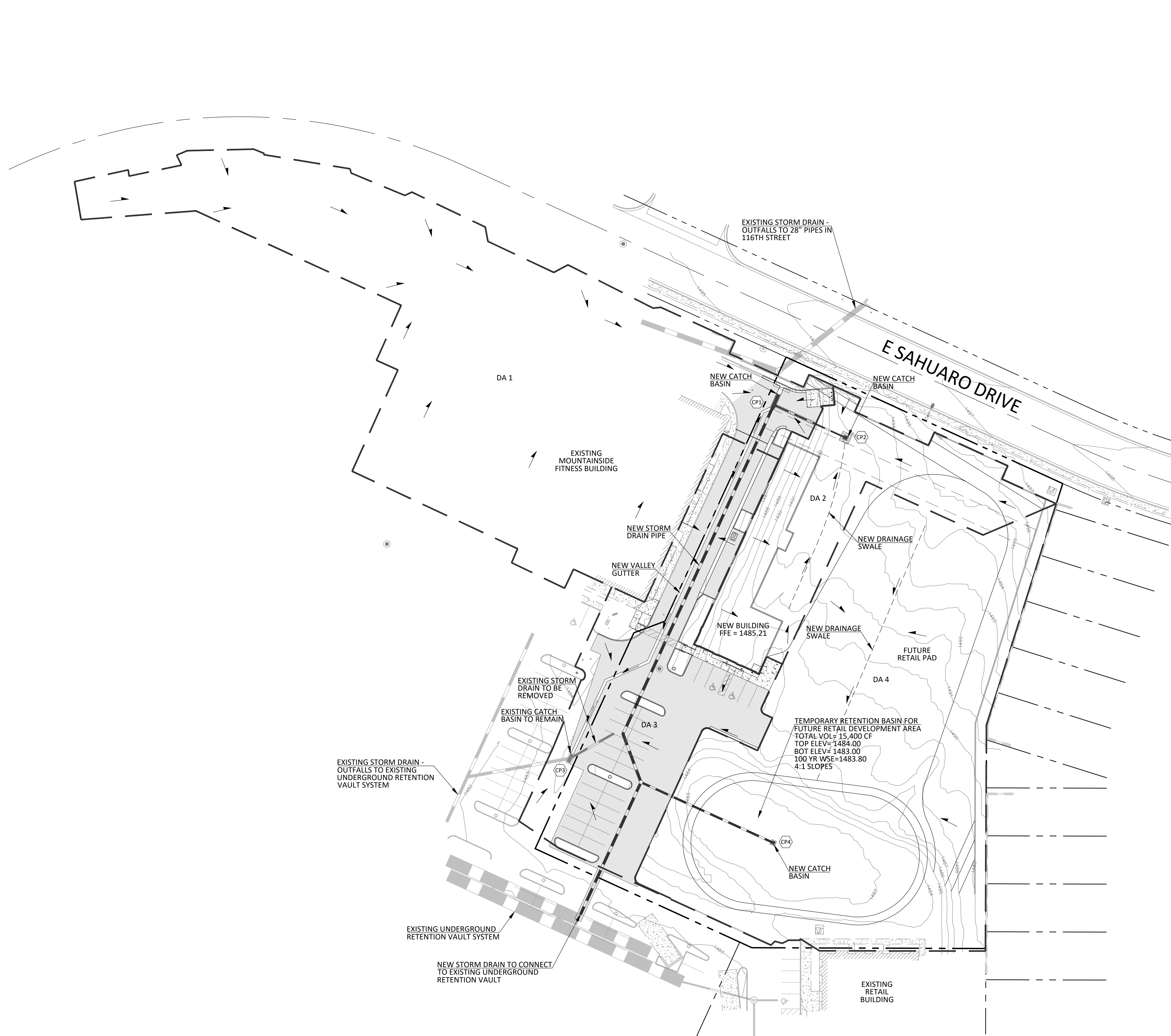
Channel Report

East Sahuaro Drive Capacity

User-defined		Highlighted	
Invert Elev (ft)	= 1496.59	Depth (ft)	= 0.50
Slope (%)	= 0.78	Q (cfs)	= 44.94
N-Value	= 0.013	Area (sqft)	= 10.75
Calculations		Velocity (ft/s)	= 4.18
Compute by:	Known Depth	Wetted Perim (ft)	= 40.45
Known Depth (ft)	= 0.50	Crit Depth, Yc (ft)	= 0.56
		Top Width (ft)	= 39.51
		EGL (ft)	= 0.77
(Sta, El, n)-(Sta, El, n)...			
(0.00, 1497.15)-(0.50, 1497.15, 0.013)-(0.50, 1496.66, 0.013)-(2.00, 1496.70, 0.013)-(14.00, 1496.93, 0.013)-(18.98, 1496.99, 0.013)-(38.51, 1496.63, 0.013)			
-(40.01, 1496.59, 0.013)-(40.01, 1497.09, 0.013)-(40.51, 1497.09, 0.013)			



APPENDIX E
(Proposed Conditions Watershed Maps + Calculations)



SCALE 1"=40'

LEGEND

RIGHT-OF-WAY

PROJECT BOUNDARY LINE

ROADWAY CENTERLINE

FLOW ARROW

WATERSHED BOUNDARY

CONCENTRATION POINT

WATERSHED CONCENTRATION POINTS		
	AREA	Q 100
CP1	1.63 AC	11.5 CFS
CP2	0.44 AC	2.8 CFS
CP3	0.64 AC	4.6 CFS
CP4	1.47 AC	10.4 CFS

CYPRESS

CIVIL DEVELOPMENT

strength + sustainability

4450 north 12th street, #228

phoenix, arizona 85014

p: 623.282.2498

e: jphunt@cypresscivil.com

CYPRESS PROJECT NO: 19.101

KERRY'S CAR CARE SHEA
proposed conditions
hydrology map

PROPOSED WATERSHED SUMMARY

PROJECT KERRY'S CAR CARE SHEA

PROPOSED DEVELOPMENT AREA

WATERSHED ID	CONCENTRATION POINT	AREA ROOF + PAVEMENT C=0.95 (SF)	AREA DESERT LANDSCAPING C=0.50 (SF)	FUTURE PAD AREA C=0.90	TOTAL AREA (AC)	WEIGHTED 'C'
DA 1	CP1	62,920	8,000	0	1.63	0.90
DA 2	CP2	7,252	6,128	5,814	0.44	0.79
DA 3	CP3	25,322	2,349	0	0.64	0.91
DA 4	CP4	0	0	64,066	1.47	0.90

RATIONAL METHOD

$$T_c = 11.4L^{0.5}K_b^{0.52}S^{-0.31}i^{-0.38} \times 60$$

Tc= Time of Concentration (min)
 L= Length of longest flow path (miles)
 Kb= Watershed resistance coefficient
 S= Watercourse slope (ft/mi)
 i= rainfall intensity (in/hr)

$$Q = CiA$$

Q = Peak discharge (cfs) 100-YR, 5-MIN 7.82 in/hr
 C = Runoff coefficient 100-YR, 10-MIN 5.96 in/hr
 i = Rainfall intensity (inch/hr) 100-YR, 15-MIN 4.92 in/hr
 A = Drainage area (Acres) 100-YR, 30-MIN 3.33 in/hr

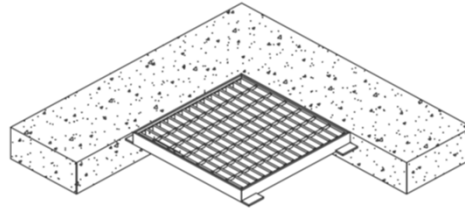
KERRY'S CAR CARE SHEA

WATERSHED ID	CONCENTRATION POINT	L	Kb	S		i	Tc	C	i	A	Q
DA-1	1	0.03	0.04	55	1.0%	7.82	3.1	0.90	7.82	1.63	11.5
DA-2	2	0.03	0.04	55	1.0%	7.82	3.1	0.79	7.82	0.44	2.8
DA-3	3	0.03	0.04	55	1.0%	7.82	3.1	0.91	7.82	0.64	4.6
DA-4	4	0.03	0.04	55	1.0%	7.82	3.1	0.90	7.82	1.47	10.4

INLET CALCULATION

PROJECT KERRY'S CAR CARE SHEA
LOCATION CP1

INLET TYPE	MAG - 10'x2'	
LENGTH	10	FT
WIDTH	2	FT
OPEN AREA	15.09	SF
Cw	3.00	
Co	0.67	
CLOGGING FACTOR	50%	



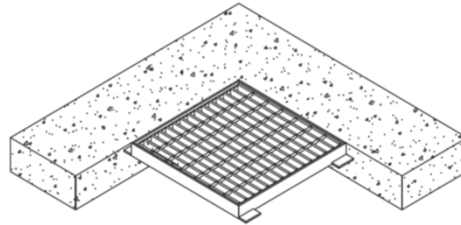
DEPTH (FT)	WEIR (CFS)	ORIFICE (CFS)	CONTROLLING (CFS)
0.00	0.0	0.0	0.0
0.05	0.4	9.1	0.4
0.10	1.1	12.8	1.1
0.15	2.1	15.7	2.1
0.20	3.2	18.1	3.2
0.25	4.5	20.3	4.5
0.30	5.9	22.2	5.9
0.35	7.5	24.0	7.5
0.40	9.1	25.7	9.1
0.45	10.9	27.2	10.9
0.50	12.7	28.7	12.7
0.55	14.7	30.1	14.7
0.60	16.7	31.4	16.7
0.65	18.9	32.7	18.9
0.70	21.1	34.0	21.1
0.75	23.4	35.1	23.4
0.80	25.8	36.3	25.8
0.85	28.2	37.4	28.2

100-YR FLOW	11.5	CFS
CALCULATED DEPTH	0.50	FT

INLET CALCULATION

PROJECT KERRY'S CAR CARE SHEA
LOCATION CP2

INLET TYPE MAG - 2'x2'
 LENGTH 2 FT
 WIDTH 2 FT
 OPEN AREA 3.02 SF
 Cw 3.00
 Co 0.67
 CLOGGING FACTOR 50%



DEPTH (FT)	WEIR (CFS)	ORIFICE (CFS)	CONTROLLING (CFS)	
0.00	0.0	0.0	0.0	0.00
0.05	0.1	1.8	0.1	0.05
0.10	0.4	2.6	0.4	0.10
0.15	0.7	3.1	0.7	0.15
0.20	1.1	3.6	1.1	0.20
0.25	1.5	4.1	1.5	0.25
0.30	2.0	4.4	2.0	0.30
0.35	2.5	4.8	2.5	0.35
0.40	3.0	5.1	3.0	0.40
0.45	3.6	5.4	3.6	0.45
0.50	4.2	5.7	4.2	0.50
0.55	4.9	6.0	4.9	0.55
0.60	5.6	6.3	5.6	0.60
0.65	6.3	6.5	6.3	0.65
0.70	7.0	6.8	6.8	0.70
0.75	7.8	7.0	7.0	0.75
0.80	8.6	7.3	7.3	0.80
0.85	9.4	7.5	7.5	0.85

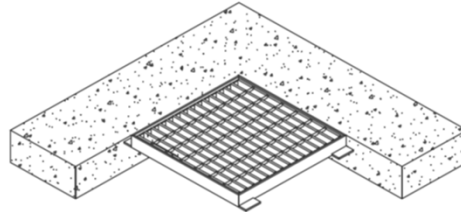
100-YR FLOW 2.8 CFS
 CALCULATED DEPTH 0.40 FT

INLET CALCULATION

PROJECT KERRY'S CAR CARE SHEA

LOCATION CP3 - EXISTING

INLET TYPE	MAG - 2'x6'	
LENGTH	6	FT
WIDTH	2	FT
OPEN AREA	9.06	SF
Cw	3.00	
Co	0.67	
CLOGGING FACTOR	50%	



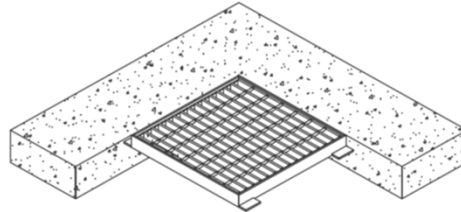
DEPTH (FT)	WEIR (CFS)	ORIFICE (CFS)	CONTROLLING (CFS)	
0.00	0.0	0.0	0.0	0.00
0.05	0.3	5.4	0.3	0.05
0.10	0.8	7.7	0.8	0.10
0.15	1.4	9.4	1.4	0.15
0.20	2.1	10.9	2.1	0.20
0.25	3.0	12.2	3.0	0.25
0.30	3.9	13.3	3.9	0.30
0.35	5.0	14.4	5.0	0.35
0.40	6.1	15.4	6.1	0.40
0.45	7.2	16.3	7.2	0.45
0.50	8.5	17.2	8.5	0.50
0.55	9.8	18.1	9.8	0.55
0.60	11.2	18.9	11.2	0.60
0.65	12.6	19.6	12.6	0.65
0.70	14.1	20.4	14.1	0.70
0.75	15.6	21.1	15.6	0.75
0.80	17.2	21.8	17.2	0.80
0.85	18.8	22.4	18.8	0.85

100-YR FLOW	4.6	CFS
CALCULATED DEPTH	0.35	FT

INLET CALCULATION

PROJECT KERRY'S CAR CARE SHEA
LOCATION CP4

INLET TYPE MAG - 2'x3'
 LENGTH 2 FT
 WIDTH 3 FT
 OPEN AREA 4.53 SF
 Cw 3.00
 Co 0.67
 CLOGGING FACTOR 50%



DEPTH (FT)	WEIR (CFS)	ORIFICE (CFS)	CONTROLLING (CFS)	
0.00	0.0	0.0	0.0	0.00
0.05	0.2	2.7	0.2	0.05
0.10	0.5	3.8	0.5	0.10
0.15	0.9	4.7	0.9	0.15
0.20	1.3	5.4	1.3	0.20
0.25	1.9	6.1	1.9	0.25
0.30	2.5	6.7	2.5	0.30
0.35	3.1	7.2	3.1	0.35
0.40	3.8	7.7	3.8	0.40
0.45	4.5	8.2	4.5	0.45
0.50	5.3	8.6	5.3	0.50
0.55	6.1	9.0	6.1	0.55
0.60	7.0	9.4	7.0	0.60
0.65	7.9	9.8	7.9	0.65
0.70	8.8	10.2	8.8	0.70
0.75	9.7	10.5	9.7	0.75
0.80	10.7	10.9	10.7	0.80
0.85	11.8	11.2	11.2	0.85

100-YR FLOW 10.4 CFS
 CALCULATED DEPTH 0.80 FT



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnini, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps & aeriels](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	2.35 (1.96-2.92)	3.08 (2.57-3.80)	4.15 (3.43-5.12)	4.99 (4.09-6.12)	6.10 (4.92-7.45)	6.95 (5.54-8.44)	7.82 (6.13-9.48)	8.70 (6.71-10.5)	9.88 (7.43-12.0)	10.8 (7.93-13.1)
10-min	1.79 (1.48-2.21)	2.34 (1.95-2.89)	3.16 (2.61-3.89)	3.79 (3.11-4.66)	4.64 (3.75-5.67)	5.29 (4.22-6.42)	5.96 (4.67-7.21)	6.62 (5.11-8.00)	7.52 (5.65-9.10)	8.20 (6.04-9.94)
15-min	1.48 (1.23-1.83)	1.93 (1.61-2.39)	2.62 (2.16-3.22)	3.14 (2.57-3.84)	3.84 (3.10-4.68)	4.37 (3.49-5.31)	4.92 (3.86-5.96)	5.47 (4.22-6.62)	6.21 (4.67-7.52)	6.78 (4.99-8.22)
30-min	0.996 (0.826-1.23)	1.30 (1.09-1.61)	1.76 (1.45-2.17)	2.11 (1.73-2.59)	2.58 (2.08-3.15)	2.94 (2.35-3.57)	3.31 (2.60-4.01)	3.69 (2.84-4.45)	4.18 (3.14-5.06)	4.56 (3.36-5.53)
60-min	0.617 (0.511-0.762)	0.806 (0.672-0.996)	1.09 (0.899-1.34)	1.31 (1.07-1.60)	1.60 (1.29-1.95)	1.82 (1.45-2.21)	2.05 (1.61-2.48)	2.28 (1.76-2.76)	2.59 (1.95-3.13)	2.82 (2.08-3.42)
2-hr	0.360 (0.302-0.434)	0.466 (0.392-0.564)	0.621 (0.520-0.748)	0.740 (0.612-0.889)	0.900 (0.738-1.08)	1.02 (0.827-1.22)	1.15 (0.914-1.37)	1.28 (0.997-1.52)	1.45 (1.10-1.72)	1.58 (1.18-1.89)
3-hr	0.262 (0.220-0.322)	0.336 (0.283-0.414)	0.440 (0.368-0.540)	0.522 (0.433-0.637)	0.637 (0.519-0.771)	0.728 (0.585-0.878)	0.822 (0.650-0.991)	0.921 (0.716-1.11)	1.06 (0.797-1.27)	1.17 (0.859-1.40)
6-hr	0.158 (0.136-0.188)	0.200 (0.172-0.238)	0.255 (0.218-0.302)	0.299 (0.253-0.353)	0.359 (0.300-0.422)	0.406 (0.334-0.475)	0.454 (0.368-0.532)	0.504 (0.402-0.591)	0.572 (0.444-0.669)	0.625 (0.474-0.733)
12-hr	0.089 (0.077-0.104)	0.112 (0.097-0.131)	0.141 (0.122-0.165)	0.164 (0.141-0.191)	0.195 (0.165-0.227)	0.219 (0.184-0.254)	0.244 (0.201-0.282)	0.269 (0.219-0.311)	0.302 (0.240-0.351)	0.328 (0.256-0.383)
24-hr	0.053 (0.047-0.060)	0.067 (0.059-0.077)	0.086 (0.077-0.099)	0.102 (0.090-0.116)	0.124 (0.108-0.141)	0.141 (0.122-0.160)	0.159 (0.137-0.180)	0.177 (0.151-0.201)	0.203 (0.171-0.230)	0.223 (0.186-0.254)
2-day	0.029 (0.025-0.033)	0.037 (0.033-0.042)	0.048 (0.043-0.055)	0.058 (0.051-0.066)	0.070 (0.061-0.080)	0.081 (0.070-0.092)	0.091 (0.079-0.104)	0.103 (0.088-0.117)	0.119 (0.100-0.135)	0.131 (0.109-0.150)
3-day	0.021 (0.018-0.024)	0.026 (0.023-0.030)	0.035 (0.031-0.040)	0.042 (0.037-0.047)	0.051 (0.045-0.058)	0.059 (0.051-0.067)	0.067 (0.058-0.076)	0.076 (0.065-0.086)	0.088 (0.074-0.101)	0.098 (0.082-0.112)
4-day	0.017 (0.015-0.019)	0.021 (0.019-0.024)	0.028 (0.025-0.032)	0.034 (0.030-0.038)	0.042 (0.037-0.047)	0.048 (0.042-0.055)	0.055 (0.048-0.062)	0.063 (0.054-0.071)	0.073 (0.062-0.083)	0.082 (0.068-0.093)
7-day	0.011 (0.009-0.012)	0.014 (0.012-0.016)	0.018 (0.016-0.021)	0.022 (0.019-0.025)	0.027 (0.023-0.031)	0.031 (0.027-0.035)	0.036 (0.031-0.040)	0.040 (0.034-0.046)	0.047 (0.040-0.054)	0.053 (0.044-0.060)
10-day	0.008 (0.007-0.009)	0.010 (0.009-0.012)	0.014 (0.012-0.016)	0.017 (0.015-0.019)	0.021 (0.018-0.023)	0.024 (0.021-0.027)	0.027 (0.023-0.030)	0.031 (0.026-0.035)	0.036 (0.030-0.040)	0.040 (0.033-0.045)
20-day	0.005 (0.004-0.006)	0.006 (0.006-0.007)	0.009 (0.008-0.010)	0.010 (0.009-0.012)	0.012 (0.011-0.014)	0.014 (0.012-0.016)	0.016 (0.014-0.018)	0.018 (0.015-0.020)	0.020 (0.017-0.023)	0.022 (0.018-0.025)
30-day	0.004 (0.003-0.004)	0.005 (0.005-0.006)	0.007 (0.006-0.008)	0.008 (0.007-0.009)	0.010 (0.008-0.011)	0.011 (0.010-0.012)	0.012 (0.011-0.014)	0.014 (0.012-0.015)	0.016 (0.013-0.018)	0.017 (0.014-0.019)
45-day	0.003 (0.003-0.003)	0.004 (0.004-0.005)	0.005 (0.005-0.006)	0.006 (0.006-0.007)	0.008 (0.007-0.009)	0.009 (0.007-0.010)	0.010 (0.008-0.011)	0.011 (0.009-0.012)	0.012 (0.010-0.014)	0.013 (0.011-0.015)
60-day	0.003 (0.002-0.003)	0.003 (0.003-0.004)	0.004 (0.004-0.005)	0.005 (0.005-0.006)	0.006 (0.006-0.007)	0.007 (0.006-0.008)	0.008 (0.007-0.009)	0.009 (0.007-0.010)	0.010 (0.008-0.011)	0.010 (0.009-0.012)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

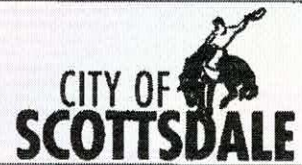
[Back to Top](#)

PF graphical

10-UP-2019
09/26/2019

APPENDIX F
(Section 404 Certification Form)

Section 404 Certification Form



Before the City issues development permits for a project, the developer's Engineer or the property owner must certify that it complies with or is exempt from Section 404 of the Clean Water Act of the United States. Section 404 regulates the discharge of dredged or fill material into a wetland, lake (including dry lakes), river, stream (including intermittent streams, ephemeral washes and arroyos) or other waters of the United States.

Prior to submittal of improvement plans to Project Review, this form must be completed (and submitted with the improvement plans) as evidence of compliance.

Certification of Section 404 Permit Status:

Owner's Name: SimonCRE Phone No.: 480-745-1956
Project Name/Description: Kerry's Car Care - Shea Case No.: _____
Project Location/Address: 11653 East Sabuaro Drive
Scottsdale, Arizona 85259

A registered Engineer or the property owner must check the applicable condition and certify by signing below that:

1. **Section 404 does apply to the project because there will be a discharge of dredged or fill material to waters of the U.S., and:**

☐ A Section 404 Permit has already been obtained for this project.

or

☐ This project qualifies for a "Nationwide Permit," and this project will meet all terms and conditions of the applicable nationwide permit.

2. **Section 404 does not apply to the project because:**

☒ No watercourse waters of the U.S. exist on the property.

☐ No jurisdictional waters of the U.S. exist on property. Attached is a copy of the COE's Jurisdictional Determination.

☐ Watercourses or other waters of the U.S. do exist on the property, but the project will not involve the discharge of dredged or fill material into any of these waters.

I certify that the above statement is true.



Engineer's Signature and Seal, or Owner's Signature

PE, Cypress Civil Development
Title/ Company

Planning and Development Services

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • www.ScottsdaleAZ.gov