PRELIMINARY WASTEWATER MASTER PLAN

PRELIMINARY Basis of Design SCOTTS DALE Report □ ACCEPTED ACCEPTED AS NOTED ☐ REVISE AND RESUBMIT Disclaimer: If accepted; the preliminary approval is granted under submitted for city review and approval (typically during the DR or PP case). The final report shall incorporate further water or sewer design and analysis requirements as defined in the city design standards and policy manual and address those items noted in the preliminary review comments (both separate and included herein). The final report shall be submitted and approved prior to the plan review submission. For questions or clarifications contact the Water Resources Planning and Engineering Department at 480-312-5685. DATE 9/10/2020 BY Idillon **DATE** 8/31/2020

Scottsdale Collective

Entertainment/Canal Districts Scottsdale, AZ 85251

Prepared For:

Nelsen Partners

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Prepared by:

Address comments on resubmittal:

- 1) Minor cleanup and clarification throughout
- 2) Detail and discuss what will be done to with the existing east-west 8" VCP sewer in Camelback.
- 3) Currently herein the City Center On the Waterfront development. This is not acceptable without a proposed alternative scenario to provide an alternate connection. Please provide additional information and description of this to confirm understanding and properly stipulate. Address items detailed in separate 8/25/2020 email correspondence.
- 12-inch main will require existing service connections.

lateral connections.

plans to cut off sewer for proposed

4) Clarify and detail that the new reconfiguration or replacement of

Sustainability Engineering Group

8280 E. Gelding Drive, Suite 101 Scottsdale, AZ 85260 480.588.7226 www.azSEG.com

Project Number: 200409

Date: June 26, 2020 (Zoning)

Describe or note what will be done.

Revision Date: August 7, 2020 (Zoning Resubmittal)

Case No.: 9-ZN-2020; 1-II-2020 Plan Check No.: TBD

Policy and Design Related Issues:

- 3. Sewer BOD, V2, LDillon 8/31/2020, **= comments added 9/10/2020
 - The City Center building extension is dependent on sewer service rerouting to Shoeman and extension of sewer on Shoeman. Regarding offsite properties on Shoeman impacted by proposed City Center layout:
 - STIPULATION: The Collective/ Collection shall relocate any laterals and make any necessary plumbing modifications to the impacted parcels to maintain continuous sewer service.
 - ii. STIPULATION: In the final BOD the Collective/Collection shall provide written concurrence with all impacted property owners, or official reps, confirming who will be responsible for revising respective already approved plans or otherwise responsible for necessary coordination items related to sewer and service lateral modifications.
 - STIPULATION: Provide documentable written confirmation from Marquee Engineer that the extended <u>Shoeman</u> sewer will not conflict with the current Marquee design.
 - iv. STPULATION: Complete design and analysis of Shoeman sewer extension. Sewer extension required will be 190 feet of 8" SDR35. Either vertically realign necessary water lines (currently shown for 20", 8") or at each crossing concrete encase both water and sewer lines within 10ft where vertical clearance is between 1-2ft. Crossings shall only be made in the middle of the water main pipe segment.
 - b. ** Regarding the new sewer design on Camelback:
 - STIPULATION: The design shall allow for reasonable third party oversizing of the new Camelback sewer prior to either final BOD approval or plan approvals.
 Oversizing of the Camelback sewer will be pursued by the City and/or by any potential future development that will need to connect to the new sewer (particularly the NWC of Brown and Shoeman).
 - STIPULATION: The design depth of the new Camelback sewer shall accommodate connection of potential future redevelopment north of <u>Shoeman</u> and west of Brown and north of Indian Plaza between Brown and 75th St.
 - c. ** STIPULATION: Public sewer to be rerouted for Maya Hotel
 - d. ** STIPULATION: The existing sewer on south side of Camelback shall be removed. A new 12" sewer shall be installed from City Center project to 75th Street.
 - e. **STIPULATION: A payment in-lieu of construction payment for the proportional capacity exceedance for the existing Camelback sewer from 75th Street to Miller Rd shall be made to the City. This shall be finalized and documented within the final BOD report. Payment shall be made prior to issuance of construction permits.





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APPENDIX I - Camelback Sewer Monitoring

APPENDIX II - Bentley FlowMaster® Calculations

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APPENDIX IV - Preliminary Utility Exhibits



1. INTRODUCTION

1.1 SUMMARY OF PROPOSED DEVELOPMENT:

The report presents a wastewater infrastructure analysis for redevelopment of three areas within the City of Scottsdale located south and east of the Camelback Road / Scottsdale Road intersection. Development options for the three sites include high density residential, resort hotel, restaurant, fitness/spa and pool facilities. Only the option for each site producing the greatest demand is presented within the body of this report for Zoning review and approval. It is likely that demands developed during final site design will be less than those shown herein.

Previously approved redevelopments (entitled capacity) relative to this report include Safari II, Blue Sky, Waterview and the DC Hotel. Wastewater flow from a future phase of the Caesars development located at the southwest corner of Highland and Scottsdale Road is yet to be entitled.

The intent of this Preliminary Master Plan is to a) discuss the proposed sewer infrastructure required to provide service to the following three redevelopments, and b) along with those entitled developments noted above, address their combined impact on the 15-inch Camelback Road sewer trunk line and existing sewer lines within the Entertainment District.

- A. City Center is a six-story residential development located adjacent to the intersection within the Arizona Canal District and will replace an existing commercial/office building.
- B. The Maya Hotel is a 12-story residential development located in the Entertainment District east of Buckboard Trail between Indian Plaza and Shoeman Lane.
- C. The Mint is a 9-story residential and retail development located south of Camelback Road and west of Civic Center Plaza.

Refer to FIGURE 1 for a Vicinity Map and Figure 2, highlighting the projects noted above.

1.2 LEGAL DESCRIPTIONS AND AREA:

Each of the three Master Planned areas consist of multiple land parcels lying within Section 23, Township 2 North, Range 4 East of the Gila and Salt River Base and Meridian, Maricopa County, Scottsdale, Arizona.

- A. City Center: Parcels 173-41-016B, -017A, -015A and -021A, 173-41-005, and 173-41-004 consisting of 118,880 square feet or 2.729 acres, more or less.
- B. The Maya: Parcels 173-41-260, -182 and -183 consisting of 16,292 square feet or 0.374 acres, more or less.
- C. The Mint: Parcels 173-41-216, -149 thru -153, -119A and 173-40-123 consisting of 56,986 square feet or 1.308 acres, more or less.



2. DESIGN DOCUMENTATION

2.1 DESIGN COMPLIANCE:

Preparation of this report has been done in accordance with requirements of the City of Scottsdale Design Standards & Policies Manual (DS&PM) 2018 ¹, direction from the City of Scottdale Water Resources Department and requirements of Maricopa County Environmental Health Department. Design flow calculations for the proposed sewer infrastructure are all based on the DS+PM recommendations in Section 7-1.403.

3. EXISTING CONDITIONS

3.1 EXISTING ZONING & LAND USE:

Existing zoning across the three parcels includes C-3 DO and P-C-2/P-3 DO with land uses varying from commercial office, retail, restaurant and entertainment. This project proposes rezoning.

3.2 EXISTING TOPOGRAPHY, VEGETATION AND LANDFORM FEATURES:

This area of Scottsdale is relatively flat with minor grades sloping to the Indian Bend Wash approximately one-half mile to the east. All three parcels have been previously developed with buildings and pavement covering most of the properties and minor site landscaping typical of the downtown vicinity.

Refer to **FIGURE 3** for an aerial of the overall project existing conditions.

3.3 EXISTING UTILITIES:

A 15-inch PVC sewer trunk line flows east under the westbound Camelback traffic lanes to a 24-inch trunk line in Miller Road. A 12-inch VCP sewer line parallels the 15-inch pipe, crossing under the Arizona Canal, and connecting with it north of the W Hotel. At that location both lines appear to be on the north side of a 144-inch storm drain and a 16-inch water line. The storm drain transitions to a 9.5' x 11' reinforced box culvert just west of Saddleback Trail. Refer to **FIGURE 4** for COS Q-S Maps 17-45 and 18-45.

Wastewater generated in the Entertainment District is largely directed to an 8-inch collection system that flows south and does not contribute to the 15-inch Camelback Road trunk line. This area discharges to a 24-inch sewer line in Miller Road via a 12-inch line along 6th Avenue, this serves the Mint East. Exceptions to the Entertainment District include a row of lots fronting Camelback Road between Buckboard and Saddlebag Trail that have a separate 8-inch line connecting to the 15-inch line and service lines to a few commercial lots (including the Mint West and DC Hotel) east of 75th Street. The City Center and neighboring developments discharge to the existing 8-inch VCP line running west to east along the site. The line connects to an 8-inch PVC that flows north to south along Brown Avenue. The 8-inch line connects to the existing 8-inch line to Shoeman Lane and turns south along Buckboard to Indian School Road, and ultimately, discharges to Miller Road.



Refer to **FIGURE 4** for City Sewer Q-S Maps 17-45 and 18-45.

4. PROPOSED METHODOLOGY

4.1 FLOW ROUTING:

This report proposes that wastewater generated by two of the proposed sites (City Center and The Mint) along with wastewater generated by the approved offsite developments (Safari Phase II, Blue Sky, Waterview and the DC Hotel) be evaluated for their capacity impacts to the 15-inch line in Camelback Road. Wastewater generated by the proposed Maya site will be taken south into the Entertainment District's collection system and east in a 12-inch line along 6th Avenue to Miller Road where that system connects to a 24-inch line.

4.2 EXISTING PEAK FLOWS:

Camelback Road 15-inch sewer:

This report will utilize an existing 540 gpm peak flow in the 15-inch line just west of Miller Road based on the City's calibrated wastewater model. For reference, two manholes on this line were monitored in August 2018 for the Waterview project and resulted in a peak flow rate of 703 gpm at 74th Street. Those results are included in **APPENDIX I**. Updated monitoring of the 15-inch line is not being proposed with this Master Plan as the effects of the COVID-19 pandemic have significantly reduced commercial flows.

Entertainment District sewer system:

Existing wastewater flow to the 12-inch line in 6th Avenue at Miller Road has been evaluated using the provided peak flow based on the city model for this area. This results in an existing peak flow of 190 gpm. Present sewer monitoring at this location is also not feasible due to several business closures resulting from the COVID-19 pandemic.

4.3 SEWER EVALUATION APPROACH:

Proposed methodology for this master plan will include utilizing the City's calibrated model flows and adding:

- a) the DS+PM based flows for City Center, The Mint and The Maya,
- b) the DS+PM based flows for Safari Phase II, Blue Sky, Water View and the DC Hotel prorated up to 10% relative to the distance from the intersection of Camelback and Miller Roads (see **Appendix III**), and
- c) a 50% credit for the sum of pool backwash flows from the projects shown in paragraphs a) and b) above relative to their discharge points or 100 gpm whichever is less.

08/11/20



5. SANITARY SYSTEM COMPUTATIONS

5.1 SEWER FLOW DEMANDS:

Table 1 shows the DS+PM unit demand criteria.

TABLE 1 - COS DS+PM DESIGN CRITERIA

Land Use	Unit Demand (gpd) Unit		Design Peaking Factor
High Density Residential	140	per unit	4.5
Resort Hotel	380	per room	4.5
Restaurant	1.2	per sq. ft.	6
Office	0.4	per sq. ft.	3
Commercial/Retail	0.5	per sq. ft.	3
Fitness/Spa	0.8	per sq. ft.	3.5
Pool Backwash	144,000	per day	1

Tables 2 through 4 show the site options for City Center, the Mint and The Maya resulting in the greatest flow.

TABLE 2 - SEWER DEMAND CALCULATIONS - Parcel A (City Center)

	Units or Area (sq. ft.)	Unit Demand (gpd)	Avg. Day Demand (gpd)	Avg. Day Demand (gpm)	Peak Hour (gpm)
High Density Residential	146	140	20,440.0	14.2	63.9
Resort Hotel	214	380	81,320.0	56.5	254.1
Restaurant	41,109	1.2	49,330.8	34.3	205.5
Office					
Commercial/Retail					
Fitness/Spa					
			SUBTOTAL	104.9	523.5
Pool Backwash			144,000.0	100.0	100.0
	295,090.8	204.9	623.5		

TABLE 3A: SEWER DEMAND CALCULATIONS - Parcel B (The Mint - West Tower)

	Units or Area (sq. ft.)	Unit Demand (gpd)	Avg. Day Demand (gpd)	Avg. Day Demand (gpm)	Peak Hour (gpm)
High Density Residential					
Resort Hotel					
Restaurant	13,881	1.2	16,657.2	11.6	69.4
Office					
Commercial/Retail					
Fitness/Spa					
	SUBTOTAL	11.6	69.4		
Pool Backwash					
	16,657.2	11.6	69.4		



TABLE 3B: SEWER DEMAND CALCULATIONS - Parcel B (The Mint - East Tower)

		. a. ee. 2 (
	Units or Area (sq. ft.)	Unit Demand (gpd)	Avg. Day Demand (gpd)	Avg. Day Demand (gpm)	Peak Hour (gpm)
High Density Residential					
Resort Hotel	116	380	44,080.0	30.6	137.8
Restaurant	14,135	1.2	16,962.0	11.8	70.7
Office					
Commercial/Retail					
Fitness/Spa					
			SUBTOTAL	42.4	208.4
Pool Backwash			144,000.0	100.0	100.0
TOTAL DEMANDS:			205,042.0	142.4	308.4

TABLE 4: SEWER DEMAND CALCULATIONS - Parcel C (The Maya)

	Units or Area (sq. ft.)	Unit Demand (gpd)	Avg. Day Demand (gpd)	Avg. Day Demand (gpm)	Peak Hour (gpm)
High Density Residential					
Resort Hotel	161	380	61,180.0	42.5	191.2
Restaurant	10,440	1.2	12,528.0	8.7	39.2
Office					
Commercial/Retail					
Fitness/Spa	1,120	0.8	896.0	0.6	2.8
			SUBTOTAL	51.8	233.1
Pool Backwash			144,000.0	100.0	100.0
	то	TAL DEMANDS:	218,604.0	151.8	333.1

The available flow information for the offsite (entitled Safarii II, Blue Sky, Water View and DC Hotel) developments was provided by the City and is summarized in Table 5.

TABLE 5 - OFFSITE ENTITLED DEVELOPMENTS (planned or under construction)								
Development	Peak Sewer Demand (gpm)	Pool Backwash Rate (gpm)	Total Demand (gpm)					
Safari Phase II	70	100	170					
Blue Sky	750	150	900					
Waterview	448	100	548					
DC Hotel	184	0	184					
Total Flow	1452	350	1802					

Allowed adjustments for the pool backwash rates and demands based on relative distance from the Camelback/Miller Road point of analysis (entitled developments only) are described and shown in Section 5.2 below.

5.2 FLOW CALCULATIONS TO THE CAMELBACK AND MILLER ROADS INTERSECTION:

Scottsdale Collective proposes to discharge the proposed City Center, the Mint Hotel, and existing flows south of Camelback Road to a proposed 12-inch sewer line. Future developments, Safari Phase II, Blue Sky, and Waterview will discharge to the existing 15-inch running along Camelback Road.

Referring to **APPENDIX III**, the City has allowed a slight reduction of wastewater demand for Safari II, Blue Sky, and Waterview based on their relative distance from a point of analysis. Additionally, the total pool backwash from those projects along with the pool backwash from the

West of 75th or @ Camelback and Miller?? Section title is to Camelback and Miller. Clarify "LEED" ing and Developing Smart Projects"

Scottsdale Collective project can be reduced based on 50% of the total proposed backwash of the projects shown or 100 gpm, whichever is less.

Per the following tables, the peak flows, without pool backwash, at the intersection of Camelback and Willer Roads with the construction of the 15-inch parallel pipe are:

Table 6 - EXISTING 15" WEST OF 75TH ST OFFSITE FLOW AT CAMELBACK AND MILLER ROAD

Table 0 - Existing 15 WE	31 01 /31113	OITSITE LOW A	CAIVILLEDACK AIVI	7 WILLELY WOAD
Development	Peak Flow (gpm)	Distance Erom Monitored MH (mi)	Prorated Demand Factor (%)	Prorated Demand (gpm)
Safari Phase II	70	0.65	93	65
Blue Sky	750	0.55	94	705
Waterview	448	0.27	97	435
Existing Flow (2)	540	N/A	100	540
Total Flows	1808	4 , , , ,		1745

what is ref point? Add keynote

Table 7 - PROPOSED 12" WEST OF 75TH ST OFFSITE FLOW AT CAMELBACK AND MILLER ROAD							
Development	Peak Flow (gpm)	Distance From Monitored MH (mi)	Prorated Demand Factor (%)	Prorated Demand (gpm)			
City Center	524	N/A	100	524			
The Mint (west)	69	N/A	100	69			
The Mint (east)	208	N/A	100	208			
The W and Existing Flow (2)	25	N/A	100	25			
Total Flows	826.4			826			

Notes: (1) Pool backwash demand reduced to 50% for multiple pools

(2) Existing offsite flow provided by the COS Water Resources Department

*Analysis east of 75th Street not included \(\) OK but need to address as in-lieu payment for proportional share of

Calculated peak flows at the intersection of 75th Street and Camelback Road with pool backwash are the following:

Table 8 - EXISTING 15" WEST OF 75TH ST OFFSITE FLOW AT CAMELBACK AND MILLER ROAD W/POOL BACKWASH								
	Distance From		Prorated					

Development	Peak Flow (gpm)	Distance From Monitored MH (mi)	Pool Allowance (gpm) (1)	Prorated Demand Factor (%)	Prorated Demand (gpm)
Safari Phase II	70	0.65	100	93	158
Blue Sky	750	0.55	100	94	799
Waterview	448	0.27	0	97	435
Existing Flow (2)	540	N/A	0	100	540
Total Flows	1808		200		1932

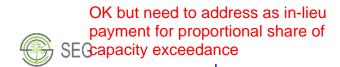


Table 9 <mark>- PROP</mark> C	Table 9 - PROPOSED 12" WEST OF 75TH ST OFFSITE FLOW AT CAMELBACK AND MILLER ROAD W/POOL						
Development	i	Peak Flow (gpm)	Distance From Monitored MH (mi)	Pool Allowance (gpm) (1)	Prorated Demand Factor (%)	Prorated Demand (gpm)	
City Center		524	N/A	100	100	624	
The Mint (west	t)	69	N/A	0	100	69	
The Mint (east	:)	208	N/A	0	100	208	
The W and Existing F	low (2)	25	N/A	0	100	25	
Tota	l Flows	826.4		100		926	

Notes: (1) Pool backwash demand reduced to 50% for multiple pools

(2) Existing offsite flow provided by the COS-Water Resources Department

*Analysis east of 75th Street not included

Wastewater from a future phase of the Caesars Hotel will discharge to a 10-inch line located in Scottsdale Road 10-inch pipe connecting to the existing 12-inch line in Camelback Road.

5.3 FLOW CALCULATIONS TO 6th AVENUE THROUGH THE ENTERTAINMENT DISTRICT:

The Maya proposed to take its' wastewater south through the Entertainment District to an existing 12-inch pipe in 6th Avenue where it will connect to the existing 24-inch trunk line in Miller Road.

Table 10 - PRORATED OFFSITE FLOW AT 6TH AVENUE AND MILLER ROAD				
Development	Peak Flow (gpm)	Distance From Monitored MH (mi)	Prorated Demand Factor (%)	Prorated Demand (gpm)
The Maya	70	0.50	95	67
Existing Flow (2)	190	N/A	100	190
Total Flows	260			257

Та	Table 11 - PRORATED OFFSITE FLOW AT 6TH AVENUE AND MILLER ROAD					
Development	Peak Flow (gpm)	Distance From Monitored MH (mi)	Pool Allowance (gpm) (1)	Prorated Demand Factor (%)	Prorated Demand (gpm)	
The Maya	70	0.50	100	95	162	
Existing Flow (2)	190	N/A	0	100	190	
Total Flows	260		100		352	

Notes: (1) Pool backwash demand reduced to 50% for multiple pools

(2) Existing offsite flow provided by the COS Water Resources Department

5.4 VARIANCE FROM STATED DESIGN FLOWS:

No variance to the DS+PM requirements is being requested with this Master Plan for the proposed three projects. Adjustments to demands shown are per direction of city staff.



SEWER SYSTEM ANALYSIS (OFF-SITE): 5.5

Based on the evaluation described below, Scottsdale Collective proposes to construct a new 12-inch sewer along the south side of Scottsdale Road between Buckboard Trail and 75th Street (see Figure 5). The proposed 12-inch sewer line along Camelback Road will run parallel, and south of the existing 12-inch pipe, located north of City Center. The proposed 12-inch will replace a portion of the existing 8-inch sewer line, north of the proposed The Mint building (west). The proposed 12inch will continue to run east and will cross over the existing storm box at 75th Street and connect to the existing 15-inch sewer line. (A service line to the DC Hotel may be reconnected to the existing 15-inch sewer.

A. Camelback Road 15-inch Sewer

yes, why is this stated?

Per Tables 6 and 8 the peak flow at the intersection of Camelback and 75th Street with the construction of the new 12-inch parallel pipe is:

- a) 1,745 gpm without the allowed pool backwash rate and
- b) 1,932 gpm including the allowed pool backwash rate

Hydraulic calculations provided in APPENDIX II indicate 2,427 gpm is available for the existing 15-inch (S=1.00%) at a pipe d/D = 0.70 and 2,833 gpm available at d/D = 0.80. The full flow capacity of the pipe is 3119 gpm.



B. Camelback Road 12-inch Sewer

Per Table 7 and 9 the peak flow at the intersection of Camelback and 75th Street with the construction of the new 12-inch parallel pipe is: d/D limit=0.65 for 12"

and less. Qmax= a) 826 gpm without the allowed pool backwash rate and 858gpm (still ok)

b) 926 gpm including the allowed pool backwash rate

Hydraulic calculations provided in ARPENDIX II indicate 946 gpm is available for the proposed 12-inch (S=0.50%) at a pipe d/D = 0.70 and 1,105 gpm available at d/D = 0.80. The full flow capacity of the pipe is 1,131 gpm.



Based on the previous hydraulic calculations discussed, the existing 15-inch and proposed 12inch are capable of conveying the calculated flows.

C. Cannelback Road/Existing 1/5-irrch/Reach/East of 7/5TH/ST/

The existing 15" reach east of 75th St to Miller Rd, will be reaching close to capacity. As such, an additional in-lieu payment would be required for the fair proportional share of the capacity exceedance over this reach. Refer to tables 12 and 13 for peak flow calculations the existing 15" reach.

> Including in-lieu for the proportional share of the capacity



Table 12 - EXISTING 15" REA	Table 12 - EXISTING 15" REACH EAST OF 75TH ST OFFSITE FLOW AT CAMELBACK AND MILLER					
Development	Peak Flow (gpm)	Distance From Monitored MH (mi)	Prorated Demand Factor (%)	Prorated Demand (gpm)		
Safari Phase II	70	0.80	92	64		
Blue Sky	750	0.70	93	698		
Waterview	448	0.35	96	430		
DC Hotel	184	0.15	98	180		
City Center	524	N/A	100	524		
The Mint (west)	69	N/A	100	69		
The Mint (east)	208	N/A	100	208		
Existing Flow (2)	540	N/A	100	540		
Total Flows	2793.4			2714		

should be same as Table 5

Table 13 - EXISTING 15" EAST OF 75TH ST OFFSITE FLOW AT CAMELBACK AND MILLER ROAD W/POOL BACKWASH

Development	Peak Flow (gpm)	Distance From Monitored MH (mi)	Pool Allowance (gpm) (1)	Prorated Demand Factor (%)	Prorated Demand (gpm)	should be 150
Safari Phase II	70	0.80	100	92	156	
Blue Sky	750	0.70	(100)	93	791	should be
Waterview	448	0.35		96	430	100
DC Hotel	184	0.15		98	180	
City Center	524	N/A	100	100	624	
The Mint (west)	69	N/A	0	100	69	
The Mint (east)	208	N/A	0	100	208	
Existing Flow (2)	540	N/A	×	100	590	
Total Flows	2793.4		350		3049	

a) 2,714 gpm without the allowed pool backwash rate and

b) 3,049 gpm including the allowed pool backwash rate

2699gpm proposed (exceedance=272gpm)

conditions i.e. not peak w/ pool backwash

Hydraulic calculations provided in APPENDIX II indicate 2,427 gpm is available for the existing 15-inch (S=1.00%) at a pipe d/D = 0.70 and 2,833 gpm available at d/D = 0.80. The full flow 3049gpm proposed capacity of the pipe is 3119 gpm. (exceedance=216gpm)

Based on the calculations provided above, the existing 15-inch reach exceeds the d/D=0.70 and In-lieu shall be based on peak d/d=0.80, as such an in-lieu payment will be required. exceedance under normal peak d/D

D. 6th Avenue 12-inch Sewer

Per Table 7, the peak flow at the intersection of 6th Avenue and Miller Roads is:

- a) 257gpm without the allowed pool backwash rate and
- b) 352 gpm including the allowed pool backwash rate

Hydraulic calculations provided in **APPENDIX II** indicate a 998 gpm available flow at a pipe d/D = 0.65 and 1289 gpm available at d/D = 0.80. The full flow capacity of the pipe is 1418 gpm.



5.6 SEWER SYSTEM ANALYSIS (ON-SITE SERVICES):

Preliminary utility exhibits for the three sites are included as **Appendix IV**. City Center and The Mint will require 10" and 6" service lines, respectively, at a minimum 0.005 ft/ft slope.

City Center will connect directly to the new 12" sewer in Camelback Road fronting that site. A proposed alley abandonment will remove the sewer west of the Upton Condominium parcel, the sewer line east of Upton Condominium is to remain in place along with the dedicated utility easement. The furthest east adjacent parcel, APN 173-41-002, will continue to discharge to the existing 8-inch sewer line. This will require a vacant parcel located at the northeast corner of Scottsdale Road and Shoeman Lane to connect to an existing 8" sewer in Shoeman Lane. That pipe will need to be inspected and surveyed for location and condition. The proposed Upton Condominiums will also connect their future development to the existing 8-inch sewer line along Shoeman Lane.

The west and east Mint buildings will connect directly to the new 12-inch sewer in Camelback Road fronting that site.

The Maya proposes a connection to the south to the existing collection system within the Entertainment District. The Maya will require a 6" service line at a minimum 0.01 ft/ft and with a proposed alley abandonment will need to relocate an existing 8-inch sewer. Refer to **FIGURE 5** for the proposed sewer relocation.

The Maya proposes to abandon an alley east of the site and will need to reroute an existing 8" sewer. Refer to **FIGURES 5 and 6** for the proposed reroute.

All unused existing sewer service lines to these sites will be removed to their respective mains per City requirements.

5.7 MAINTENANCE RESPONSIBILITIES:

The proposed building service connections for City Center and The Mint will be private and maintained by the building owners. The proposed 12-inch sewer and relocated 8-inch sewer will be part of the public system owned and maintained by the City of Scottsdale. The Maya service connection to the new 8-inch sewer pipe will be private. Grease interceptors will be provided for all restaurant facilities and be maintained by the property owners.

6. SUMMARY

change to "may be - required", could be another in-lieu

6.1 SUMMARY OF PROPOSED IMPROVEMENTS:

By Scottsdale Collective constructing a parallel 12-inch line along the south side of Camelback Road 15" between Scottsdale Road and 75th Street, additional capacity will be provided meeting the demands of City Center and The Mint. A future extension of this line by others between 75th Street and Miller Road will be required prior to the construction of either Blue Sky or Caesars Phase II.



The Maya, being approximately 400 feet south of Camelback Road will connect to the available local sewer infrastructure. The proposed alley abandonment will require rerouting a section of sewer along Indian Plaza.

6.2 PROJECT SCHEDULE:

No timeline has presently been documented for the construction of each Parcel. It is anticipated as each Parcel develops, all related sewer service and main lines will be constructed in a single phase.

7. SUPPORTING MAPS

7.1 UTILITY PLANS

Preliminary utility exhibits are provided in APPENDIX IV.

8. REFERENCES

- 1. City of Scottsdale Sewer Quarter Section number 17-45
- 2. City of Scottsdale Design Standards & Policies Manual, 2018 (Chapter 7 Wastewater)
- 3. City of Scottsdale 2012 Water Reuse Master Plan Update, Table 2.4 and Table 2.5



FIGURES

- 1 Vicinity Map
- 2 Local Projects, Approved or Proposed
- 3 Aerial
- 4 City Sewer Q-S Maps 17-45 and 18-45
- 5 New Camelback Road 15-inch Sewer
- 6 Proposed Sewer Reroute (North of The Maya)
- 7 Preliminary Site Utility Plans

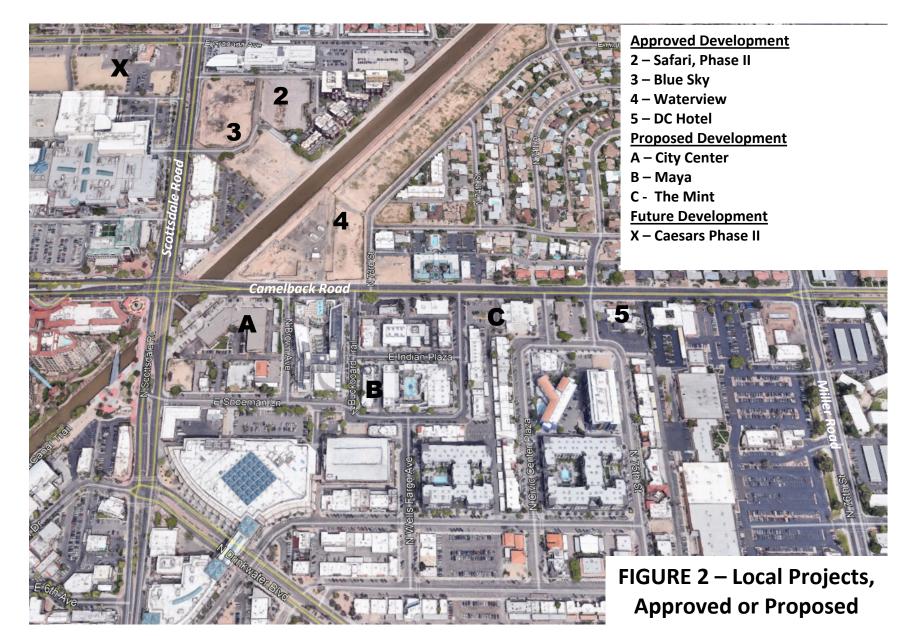


Figure 1- Vicinity Map



City Center The Mint Maya Hotel





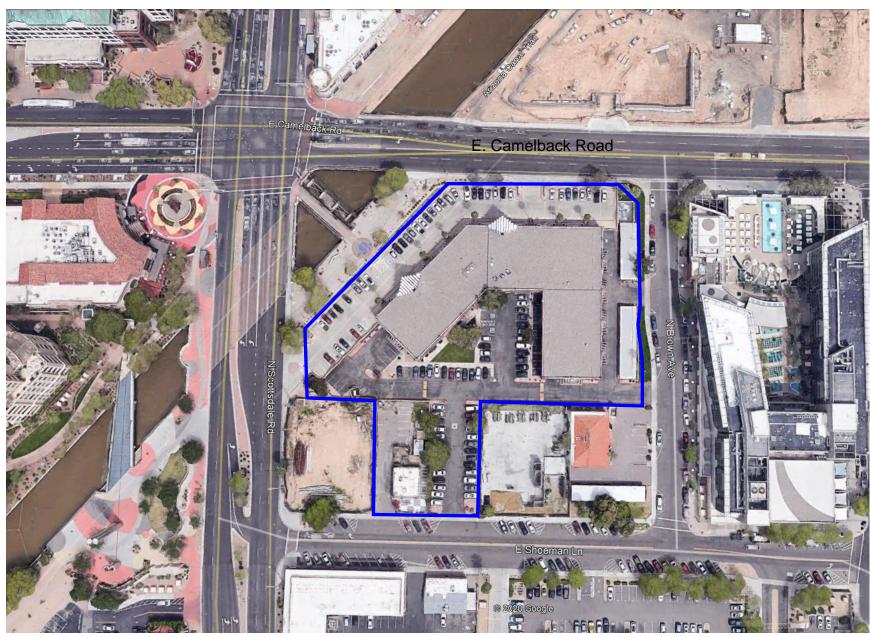




Figure 3A- Aerial Map City Center

SCALE: N.T.S

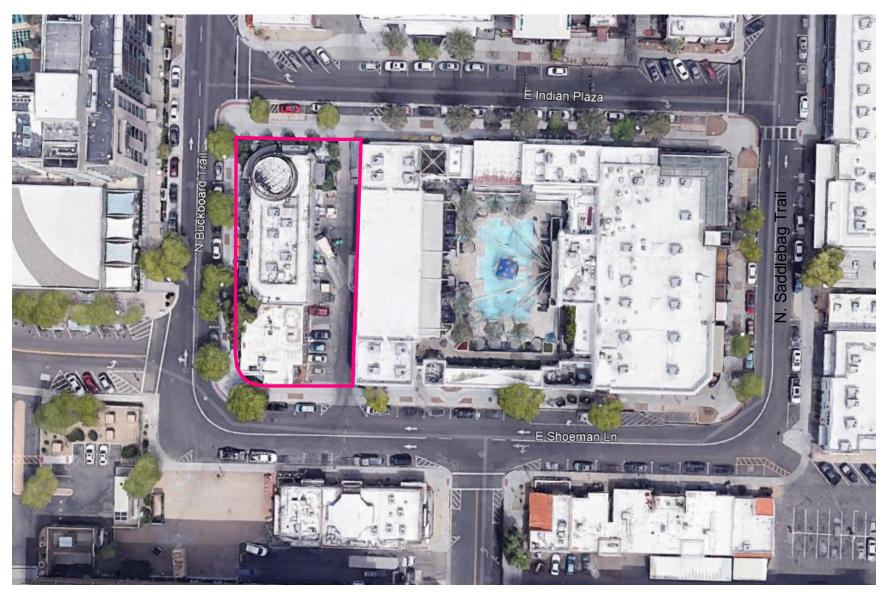


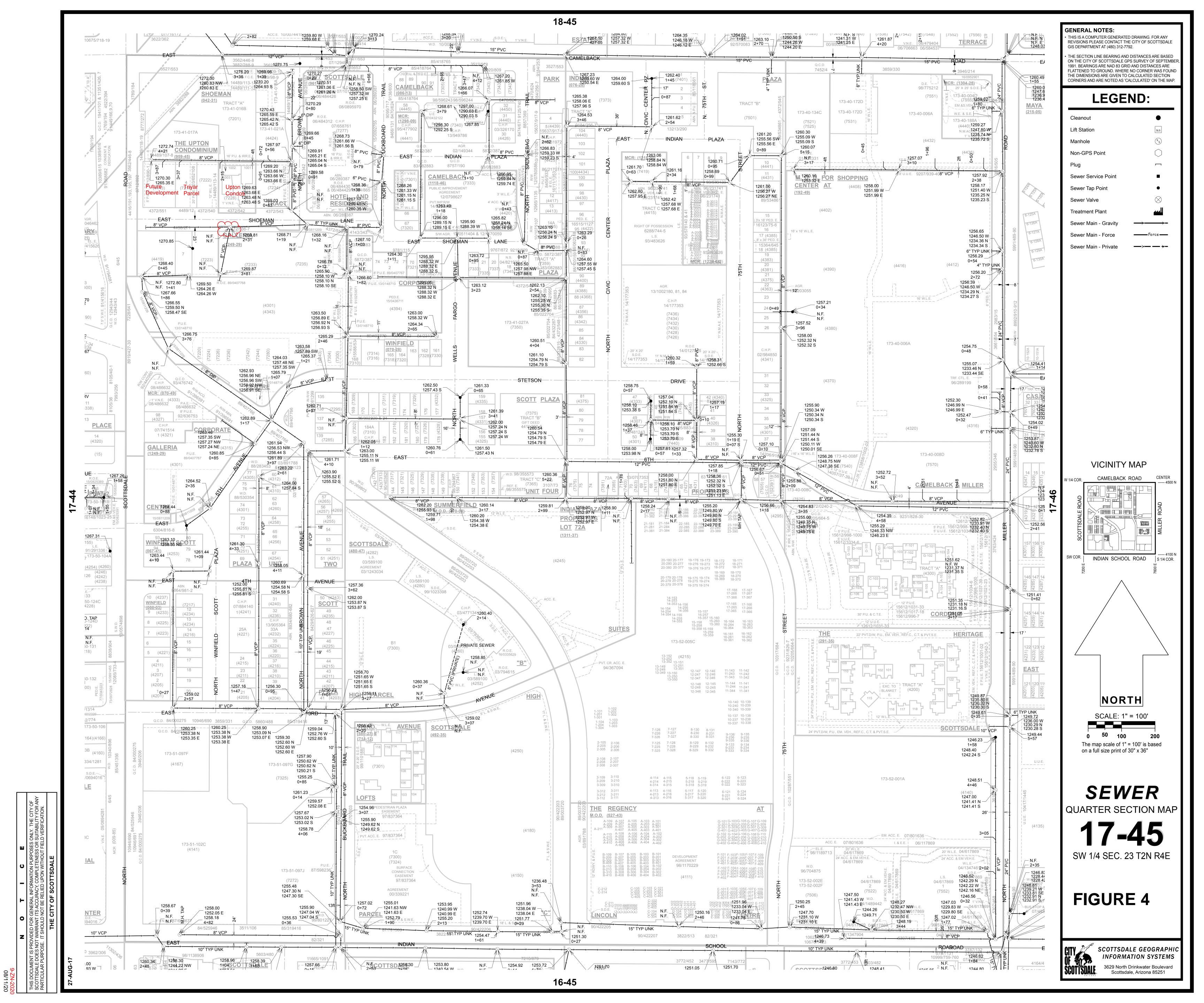


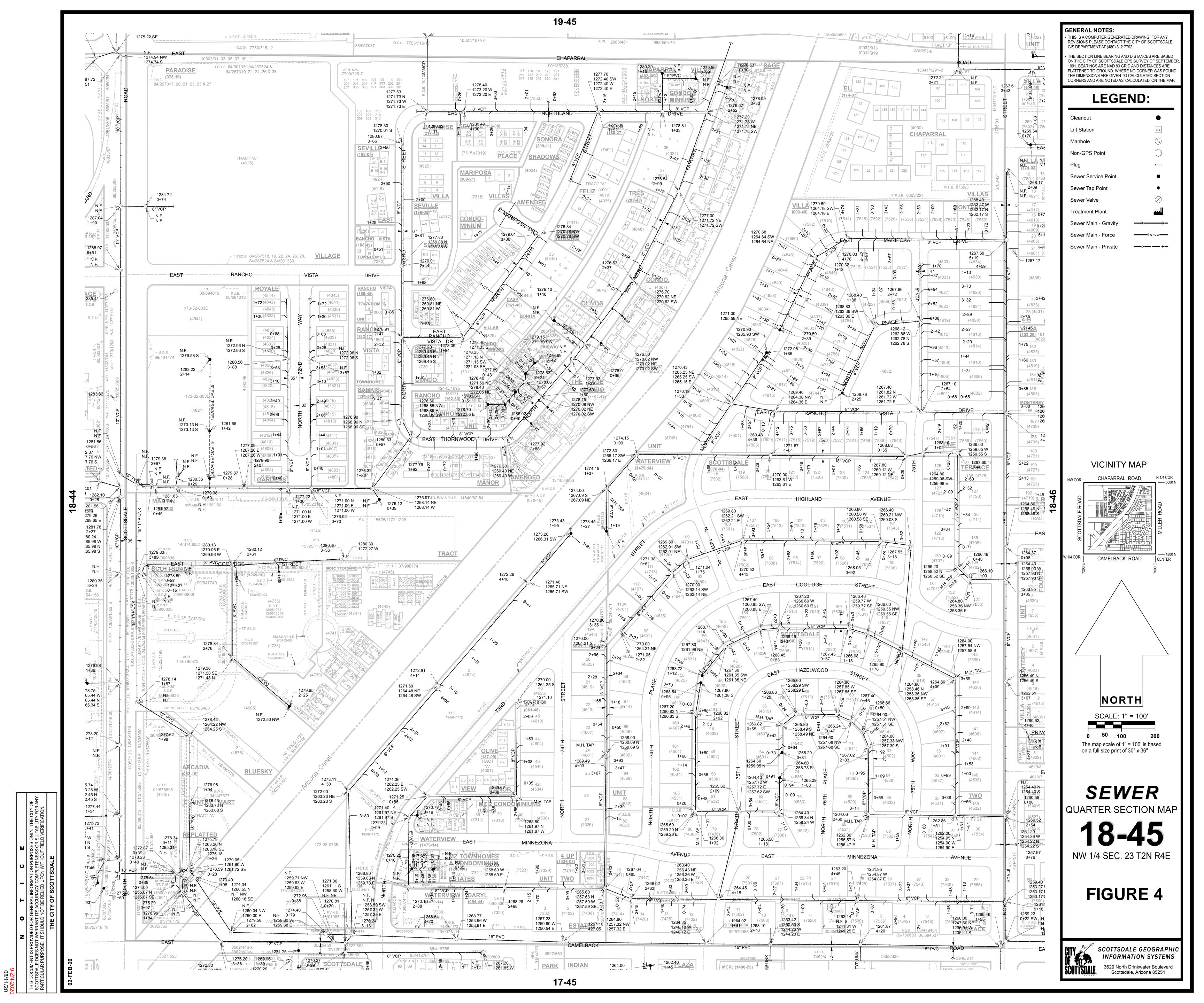
Figure 3B- Aerial Map Maya Hotel

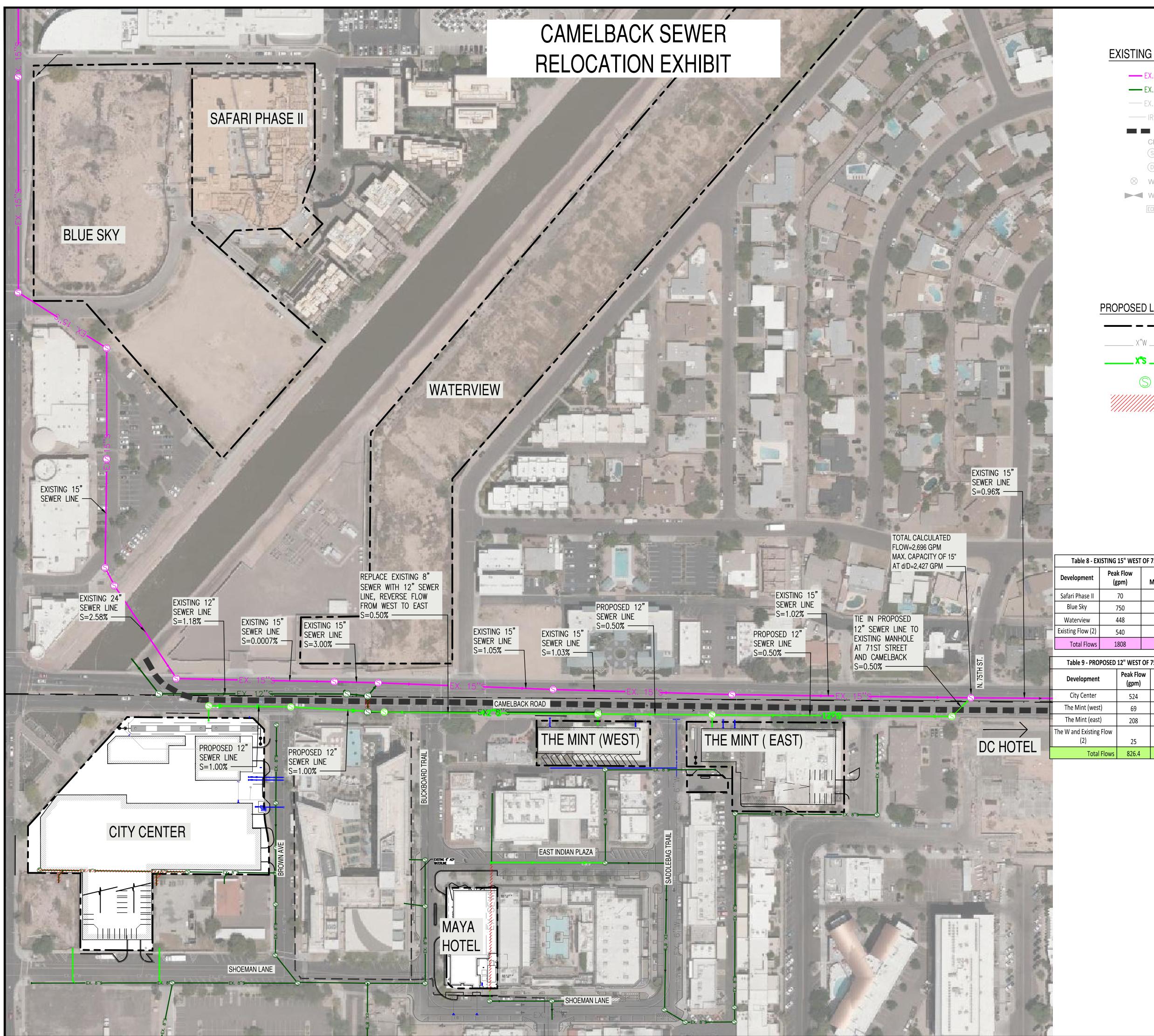




Figure 3C- Aerial Map
The Mint







EXISTING LEGEND

CATCH BASIN

SEWER MANHOLE STORM MANHOLE

WATER METER BOX ELECTRIC CABINET

PROPOSED LEGEND:

PROPERTY LINE WATER LINE SEWER LINE

SEWER MANHOLE SEWER LINE ABANDONMENT

Table 8 - EX	(ISTING 15" WEST	OF 75TH ST OFFSITE FLOV	V AT CAMELBACK AND	MILLER ROAD W/POOL	BACKWASH
Development	Peak Flow (gpm)	Distance From Monitored MH (mi)	Pool Allowance (gpm) (1)	Prorated Demand Factor (%)	Prorated Demand (gpm
Safari Phase II	70	0.65	100	93	158
Blue Sky	750	0.55	100	94	799
Waterview	448	0.27	0	97	435
Existing Flow (2)	540	N/A	0	100	540
Total Flows	1808		200		1932

Table 9 - PROPOSED 12" WEST OF 75TH ST OFFSITE FLOW AT CAMELBACK AND MILLER ROAD W/POOL BACKWASH						
Development	Peak Flow (gpm)	Distance From Monitored MH (mi)	Pool Allowance (gpm) (1)	Prorated Demand Factor (%)	Prorated Demand (gpm)	
City Center	524	N/A	100	100	624	
The Mint (west)	69	N/A	0	100	69	
The Mint (east)	208	N/A	0	100	208	
The W and Existing Flow (2)	25	N/A	0	100	25	
T . 151	006.4		400		000	



CTIVE	
PROJECT SCOTTSDALE COLLECTIVE	LOCATION

SC		
RAWN	 KA	04/28/2020
ESIGNED	 KA	04/28/2020
2	 SC	
A	 AF	

07/24/2020 ISSUED FOR: PRELIMINARY

REVIEW REVISION NO.:

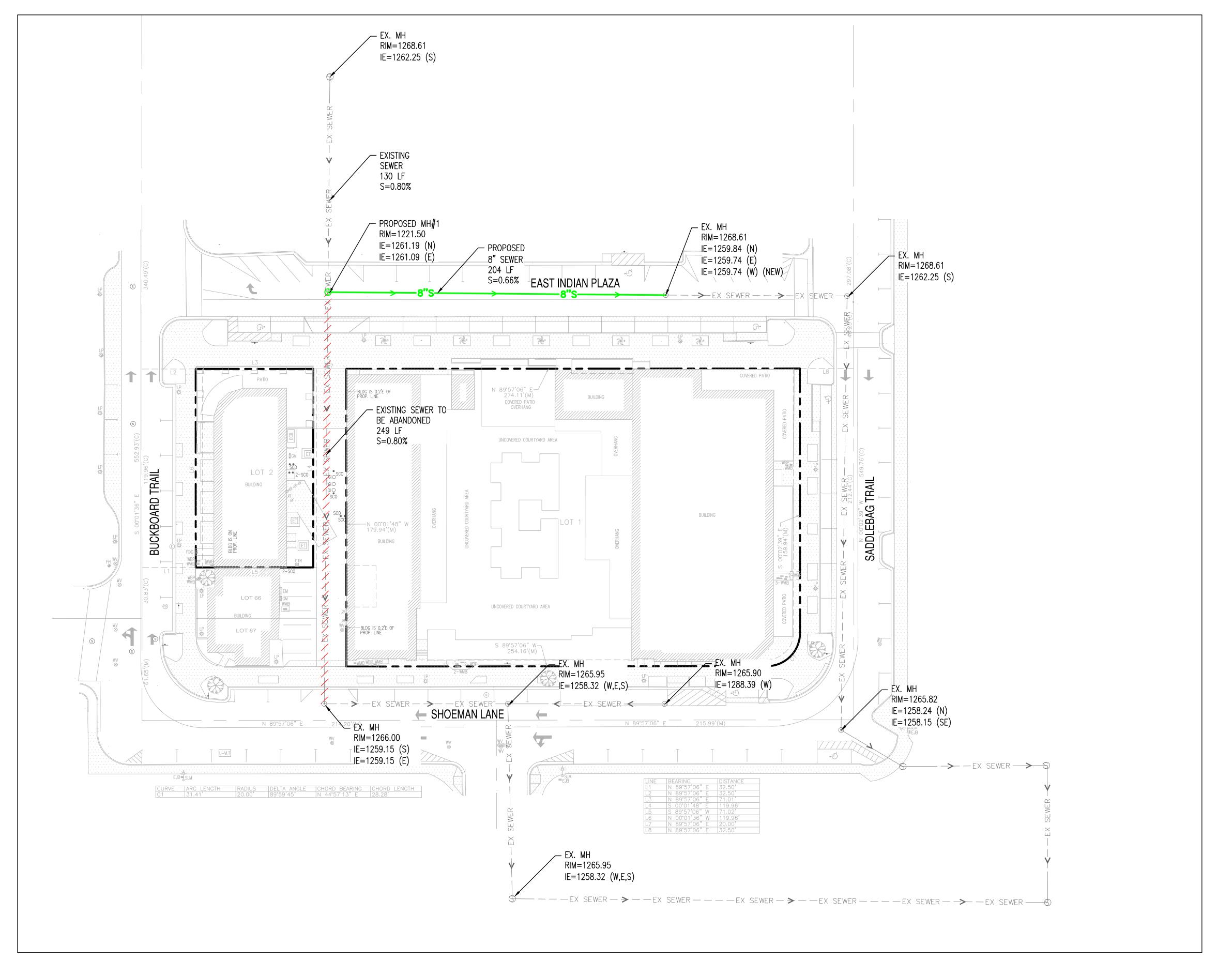
JOB NO.: 200226

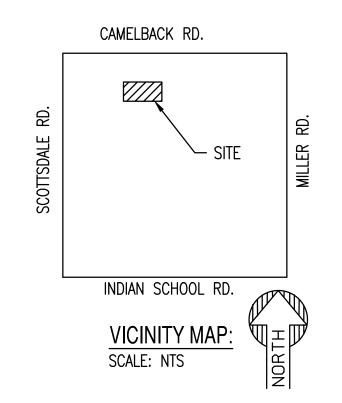
FIGURE 5

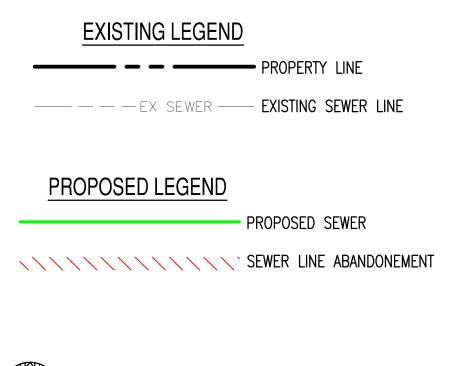
SHEET NO.:

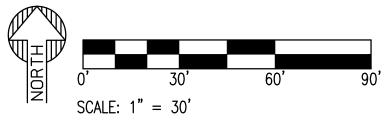
SCALE: 1" = 60'

MAYA HOTEL SEWER LINE RELOCATION EXHIBIT









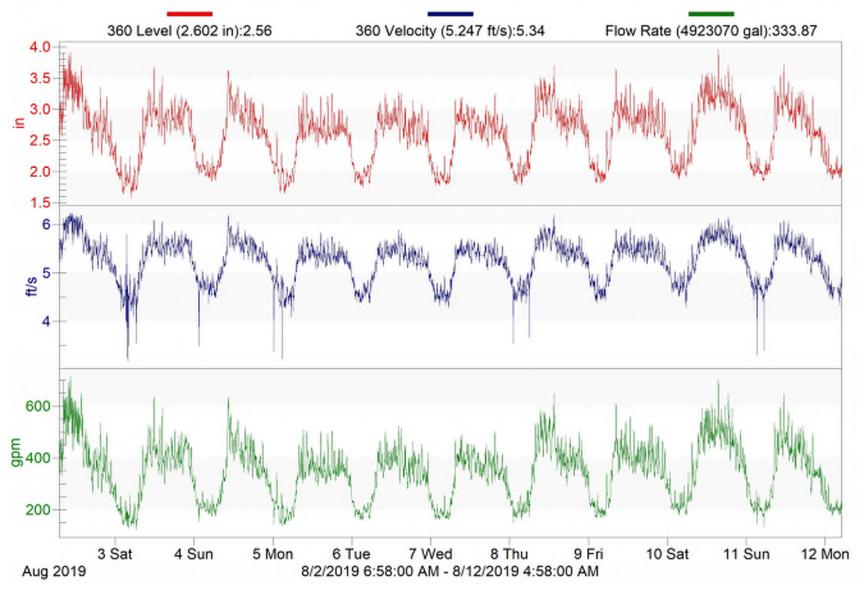


APPENDICIES

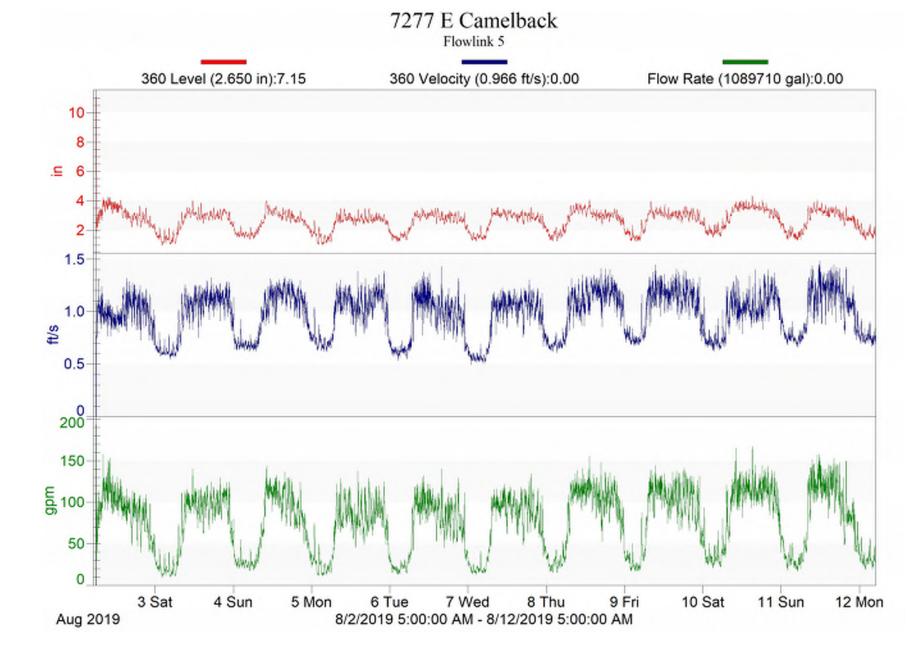
- I. Camelback Sewer Monitoring
- II. Bentley FlowMaster® Calculations
- III. 2020-04-23 Memo from COS Water Resources
- IV. Sewer Surveys

Site Name	7402 E Camelback								
Isco Quantity	Velocity	Min/Max	Velocity	Min/Max	Flow Rate	Min/Max	Flow Rate	Min/Max	Volume
Label	Min Velocity	Min/Max	Peak Velocity	Min/Max	Min Flow Rate	Min/Max	Peak Flow Rate	Min/Max	Total Flow
Units	ft/s	Date/Time	ft/s	Date/Time	gpm	Date/Time	gpm	Date/Time	gal
Resolution	0.1	N/A	0.1	N/A	0.1	N/A	0.1	N/A	0.1
Significant Digits	0	N/A	0	N/A	0	N/A	0	N/A	0
8/3/2019 0:00	3.178	3:54:00 AM	6.097	11:46:00 AM	123.787	6:26:00 AM	635.133	11:48:00 AM	482219
8/4/2019 0:00	3.493	1:30:00 AM	6.201	10:28:00 AM	155.448	1:30:00 AM	634.564	10:28:00 AM	481137
8/5/2019 0:00	3.228	2:52:00 AM	5.834	2:34:00 PM	138.016	2:52:00 AM	509.935	9:12:00 AM	452016
8/6/2019 0:00	4.367	5:30:00 AM	5.891	2:20:00 PM	160.268	3:22:00 AM	523.528	2:20:00 PM	463678
8/7/2019 0:00	4.278	5:24:00 AM	5.729	7:32:00 PM	162.497	1:36:00 AM	503.092	7:34:00 PM	460544
8/8/2019 0:00	3.534	1:06:00 AM	6.209	1:28:00 PM	149.764	1:06:00 AM	647.476	1:30:00 PM	520023
8/9/2019 0:00	4.383	2:36:00 AM	5.926	12:38:00 PM	169.704	3:36:00 AM	610.986	8:20:00 AM	500622
8/10/2019 0:00	4.288	5:38:00 AM	6.154	6:32:00 PM	179.502	3:26:00 AM	703.678	3:28:00 PM	561301
8/11/2019 0:00	3.314	3:18:00 AM	6.085	11:22:00 AM	134.766	5:28:00 AM	646.798	11:22:00 AM	494046
8/12/2019 0:00	4.326	4:20:00 AM	5.181	12:58:00 AM	175.513	2:06:00 AM	300.694	12:56:00 AM	107374

7402 E Camelback Flowlink 5



Site Name	7277 E Camelback								
Isco Quantity	Velocity	Min/Max	Velocity	Min/Max	Flow Rate	Min/Max	Flow Rate	Min/Max	Volume
Label	Min Velocity	Min/Max	Peak Velocity	Min/Max	Min Flow Rate	Min/Max	Peak Flow Rate	Min/Max	Total Flow
Units	ft/s	Date/Time	ft/s	Date/Time	gpm	Date/Time	gpm	Date/Time	gal
Resolution	0.1	N/A	0.1	N/A	0.1	N/A	0.1	N/A	0.1
Significant Digits	0	N/A	0	N/A	0	N/A	0	N/A	0
8/3/2019 0:00	0.543	4:34:00 AM	1.322	5:10:00 PM	9.51	2:12:00 AM	140.096	11:30:00 AM	102108
8/4/2019 0:00	0.618	6:56:00 AM	1.332	3:46:00 PM	17.154	5:04:00 AM	143.818	10:14:00 AM	101127
8/5/2019 0:00	0.616	4:42:00 AM	1.387	10:10:00 PM	10.959	3:18:00 AM	137.644	2:16:00 PM	96922.1
8/6/2019 0:00	0.532	2:34:00 AM	1.429	3:48:00 PM	13.099	2:34:00 AM	140.233	3:48:00 PM	97206.4
8/7/2019 0:00	0.496	12:52:00 AM	1.301	6:48:00 PM	13.007	2:30:00 AM	134.45	7:18:00 PM	100952
8/8/2019 0:00	0.607	3:18:00 AM	1.411	4:48:00 PM	15.14	3:20:00 AM	155.711	1:14:00 PM	121226
8/9/2019 0:00	0.674	3:10:00 AM	1.444	9:24:00 AM	16.633	1:56:00 AM	144.611	9:24:00 AM	120365
8/10/2019 0:00	0.624	3:26:00 AM	1.399	4:00:00 PM	21.07	3:10:00 AM	167.371	3:16:00 PM	126226
8/11/2019 0:00	0.662	5:00:00 AM	1.481	11:56:00 AM	16.827	5:00:00 AM	157.926	11:48:00 AM	116034
8/12/2019 0:00	0.665	3:50:00 AM	0.929	12:38:00 AM	18.552	2:00:00 AM	58.321	12:44:00 AM	15276.3





	15" Sewer at .01 ft/ft, d/D=0.7						
Project Description							
Friction Method	Manning Formula						
Solve For	Discharge						
Input Data							
Roughness Coefficient		0.013					
Channel Slope		0.01000	ft/ft				
Normal Depth		10.50	in				
Diameter		15	in				
Results							
Discharge		2427.3	gal/min				
Flow Area		0.92	ft²				
Wetted Perimeter		2.48	ft				
Hydraulic Radius		4.44	in				
Top Width		1.15	ft				
Critical Depth		0.94	ft				
Percent Full		70.0	%				
Critical Slope		0.00832	ft/ft				
Velocity		5.89	ft/s				
Velocity Head		0.54	ft				
Specific Energy		1.41	ft				
Froude Number		1.16					
Maximum Discharge		3118.69	gal/min				
Discharge Full		2899.2	gal/min				
SlopeFull		0.00701	ft/ft				
Flow Type	SuperCritical						

15" CAMELBACK SEWER W/O POOL BACKWASH

APPENDIX II



	15" Sewer at	.01 ft/ft,	d/D=0.8	
Project Description				
Friction Method	Manning Formula			
Solve For	Discharge			
Input Data				
Roughness Coefficient		0.013		
Channel Slope		0.01000	ft/ft	
Normal Depth		12.00	in	
Diameter		15	in	
Results				
Discharge		2833.9	gal/min	
Flow Area		1.05	ft²	
Wetted Perimeter		2.77	ft	
Hydraulic Radius		4.56	in	
Top Width		1.00	ft	
Critical Depth		1.01	ft	
Percent Full		80.0	%	
Critical Slope		0.00974	ft/ft	
Velocity		6.00	ft/s	
Velocity Head		0.56	ft	
Specific Energy		1.56	ft	
Froude Number		1.03		
Maximum Discharge		3118.69	gal/min	
Discharge Full		2899.2	gal/min	
Slope Full		0.00955	ft/ft	
Flow Type	SuperCritical			

15" CAMELBACK SEWER W/ POOL BACKWASH

APPENDIX II



		(
	12" Sewer at (0.005 ft/ft, d/D=0.70	
Project Description		-	should be 0.65
Friction Method	Manning Formula		-
Solve For	Discharge		_
Input Data			-
Roughness Coefficient	0.013		_
Channel Slope	0.005 ft/ft		
Normal Depth	8.4 in		
Diameter	12.0 in		_
Results			-
Discharge	946.64 gpm		_
Flow Area	0.6 ft ²		
Wetted Perimeter	2.0 ft		
Hydraulic Radius	3.6 in		
Top Width	0.92 ft		
Critical Depth	7.4 in		
Percent Full	70.0 %		
Critical Slope	0.007 ft/ft		
Velocity	3.59 ft/s		
Velocity Head	0.20 ft		
Specific Energy	0.90 ft		
Froude Number	0.791		
Maximum Discharge	1,216.27 gpm		
Discharge Full	1,130.67 gpm		
Slope Full	0.004 ft/ft		
Flow Type	Subcritical		_
GVF Input Data			-
Downstream Depth	0.0 in		_
Length	0.0 ft		
Number Of Steps	0		_
GVF Output Data			-
Upstream Depth	0.0 in		-
Profile Description	N/A		
Profile Headloss	0.00 ft		
Average End Depth Over Rise	0.0 %		
Normal Depth Over Rise	70.0 %		
Downstream Velocity	Infinity ft/s		
Upstream Velocity	Infinity ft/s		
Normal Depth	8.4 in		
Critical Depth	7.4 in		
Channel Slope	0.005 ft/ft		
Critical Slope	0.007 ft/ft		_

12" PROPOSED **CAMELBACK SEWER W/O POOL BACKWASH**

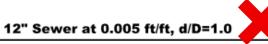


12" Sewer at 0.005 ft/ft, d/D=0.80

		1 01000 1t/1t, u/b-0100
Project Description		
Friction Method	Manning	
	Formula	
Solve For	Discharge	
Input Data		
Roughness Coefficient	0.013	
Channel Slope	0.005 ft/ft	
Normal Depth	9.6 in	
Diameter	12.0 in	
Results		
Discharge	1,105.19 gpm	
Flow Area	0.7 ft ²	
Wetted Perimeter	2.2 ft	
Hydraulic Radius	3.7 in	
Top Width	0.80 ft	
Critical Depth	8.1 in	
Percent Full	80.0 %	
Critical Slope	0.008 ft/ft	
Velocity	3.66 ft/s	
Velocity Head	0.21 ft	
Specific Energy	1.01 ft	
Froude Number	0.702	
Maximum Discharge	1,216.27 gpm	
Discharge Full	1,130.67 gpm	
Slope Full	0.005 ft/ft	
Flow Type	Subcritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description	N/A	
Profile Headloss	0.00 ft	
Average End Depth Over Rise	0.0 %	
Normal Depth Over Rise	70.0 %	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	9.6 in	
Critical Depth	8.1 in	
Channel Slope	0.005 ft/ft	
Critical Slope	0.008 ft/ft	

12" PROPOSED CAMELBACK SEWER W/O POOL BACKWASH





	12 Sewer at	t 0.005 ft/ft, d/D=1.0
Project Description		
Friction Method	Manning	
Solve For	Formula Discharge	
Solve For	Discharge	
Input Data		
Roughness Coefficient	0.013	
Channel Slope	0.005 ft/ft	
Normal Depth	12.0 in	
Diameter	12.0 in	
Results		
Discharge	1,130.67 gpm	
Flow Area	0.8 ft ²	
Wetted Perimeter	3.1 ft	
Hydraulic Radius	3.0 in	
Top Width	0.00 ft	
Critical Depth	8.2 in	
Percent Full Critical Slope	100.0 % 0.008 ft/ft	
Velocity	3.21 ft/s	
Velocity Head	0.16 ft	
Specific Energy	1.16 ft	
Froude Number	(N/A)	
Maximum Discharge	1,216.27 gpm	
Discharge Full	1,130.67 gpm	
Slope Full	0.005 ft/ft	
Flow Type	Subcritical	
GVF Input Data		
Downstream Depth	0.0 in	
Length	0.0 ft	
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.0 in	
Profile Description	N/A	
Profile Headloss	0.00 ft	
Average End Depth Over Rise	0.0 %	
Normal Depth Over Rise	70.0 %	
Downstream Velocity	Infinity ft/s	
Upstream Velocity	Infinity ft/s	
Normal Depth	12.0 in 8.2 in	
Critical Depth Channel Slope	0.005 ft/ft	
•	,	
Critical Slope	0.008 ft/ft	



8" Pipe at 0.003	3 ft/ft;	d/D=0
Project Description		
Friction Method Manning Formula		
Solve For Discharge		
Input Data		
Roughness Coefficient	0.013	
Channel Slope	0.00330	ft/ft
Normal Depth	6.40	in
Diameter	8	in
Results		
Discharge	304.5	gal/min
Flow Area	0.30	ft²
Wetted Perimeter	1.48	ft
Hydraulic Radius	2.43	in
Top Width	0.53	ft
Critical Depth	0.39	ft
Percent Full	80.0	%
Critical Slope	0.00765	ft/ft
Velocity	2.27	ft/s
Velocity Head	0.08	ft
Specific Energy	0.61	ft
Froude Number	0.53	
Maximum Discharge	335.14	gal/min
Discharge Full	311.6	gal/min
Slope Full	0.00315	ft/ft
Flow Type SubCritical		

8" ENTERTAINMENT DISTRICT SEWER W/ POOL BACKWASH

APPENDIX II



	12" Sewer at .0	068 ft/ft;	d/D=0
Project Description			
Friction Method	Manning Formula		
Solve For	Discharge		
Input Data			
Roughness Coefficient		0.013	
Channel Slope		0.00680	ft/ft
Normal Depth		7.80	in
Diameter		12	in
Results			
Discharge		997.4	gal/min
Flow Area		0.54	ft²
Wetted Perimeter		1.88	ft
Hydraulic Radius		3.46	in
Top Width		0.95	ft
Critical Depth		0.64	ft
Percent Full		65.0	%
Critical Slope		0.00719	ft/ft
Velocity		4.11	ft/s
Velocity Head		0.26	ft
Specific Energy		0.91	ft
Froude Number		0.96	
Maximum Discharge		1418.40	gal/min
Discharge Full			gal/min
SlopeFull		0.00389	ft/ft
Flow Type	SubCritical		

12" 6TH AVENUE SEWER W/O POOL BACKWASH



	12"Sewer at .0	0068 ft/ft	; d/D=
Project Description			
Friction Method	Manning Formula		
Solve For	Discharge		
Input Data			
Roughness Coefficient		0.013	
Channel Slope		0.00680	ft/ft
Normal Depth		9.60	in
Diameter		12	in
Results			
Discharge		1288.9	gal/min
Flow Area		0.67	ft²
Wetted Perimeter		2.21	ft
Hydraulic Radius		3.65	in
Top Width		0.80	ft
Critical Depth		0.73	ft
Percent Full		80.0	%
Critical Slope		0.00843	ft/ft
Velocity		4.26	ft/s
Velocity Head		0.28	ft
Specific Energy		1.08	ft
Froude Number		0.82	
Maximum Discharge		1418.40	gal/min
Discharge Full		1318.6	gal/min
SlopeFull		0.00650	ft/ft
Flow Type	SubCritical		

12" 6TH AVENUE SEWER W/ POOL BACKWASH



	8" Sewer at .00	052 ft/ft;	d/D=0.65	
Project Description				
Friction Method	Manning Formula			
Solve For	Discharge			
Input Data				
Roughness Coefficient		0.013		
Channel Slope		0.00520	ft/ft	
Normal Depth		5.20	in	
Diameter		8	in	
Results				
Discharge		295.8	gal/min	
Flow Area		0.24	ft²	
Wetted Perimeter		1.25	ft	
Hydraulic Radius		2.31	in	
Top Width		0.64	ft	
Critical Depth		0.38	ft	
Percent Full		65.0	%	
Critical Slope		0.00757	ft/ft	
Velocity		2.74	ft/s	
Velocity Head		0.12	ft	
Specific Energy		0.55	ft	
Froude Number		0.79		
Maximum Discharge		420.70	gal/min	
Discharge Full		391.1	gal/min	
SlopeFull		0.00298	ft/ft	
Flow Type	SubCritical			

8" MAYA RELOCATED SEWER W/O POOL BACKWASH



1	2" Sewer at 0.	01 ft/ft; (d/D = 0
Project Description			
Friction Method	Manning Formula		
Solve For	Discharge		
Innut Data			
Input Data			
Roughness Coefficient		0.013	
Channel Slope		0.01000	ft/ft
Normal Depth		7.80	in
Diameter		15	in
Results			
Discharge		1548.5	gal/min
Flow Area		0.64	ft²
Wetted Perimeter		2.01	ft
Hydraulic Radius		3.84	in
Top Width		1.25	ft
Critical Depth		0.75	ft
Percent Full		52.0	%
Critical Slope		0.00633	ft/ft
Vel o city		5.35	ft/s
Velocity Head		0.44	ft
Specific Energy		1.09	ft
Froude Number		1.31	
Maximum Discharge		3118.69	gal/min
Discharge Full		2899.2	gal/min
SlopeFull		0.00285	ft/ft
Flow Type	SuperCritical		

12" CAMELBACK ROAD
PARALLEL SEWER W/O POOL
BACKWASH



	12" Sewer at	0.01 ft/ft;	d/D =
Project Description			
Friction Method	Manning Formula		
Solve For	Discharge		
Input Data			
Roughness Coefficient		0.013	
Channel Slope		0.01000	ft/ft
Normal Depth		9.60	in
Diameter		15	in
Results			
Discharge		2144.6	gal/min
Flow Area		0.83	ft²
Wetted Perimeter		2.32	ft
Hydraulic Radius		4.29	in
Top Width		1.20	ft
Critical Depth		0.89	ft
Percent Full		64.0	%
Critical Slope		0.00755	ft/ft
Velocity		5.76	ft/s
Velocity Head		0.52	ft
Specific Energy		1.32	ft
Froude Number		1.22	
Maximum Discharge		3118.69	gal/min
Discharge Full		2899.2	gal/min
Slope Full		0.00547	ft/ft
Flow Type	SuperCritical		

12" CAMELBACK ROAD
PARALLEL SEWER W/ POOL
BACKWASH

 From:
 Dillon, Levi

 To:
 Doug Mann

 Cc:
 Ali Fakih

Subject: RE: Stockdale Collective Master Plan

Date: Thursday, April 23, 2020 7:04:13 PM

Attachments: <u>image003.png</u> <u>image004.png</u>

image004.png image005.png

[EXTERNAL EMAIL]

Doug, here is some additional guidance on sewer analysis. -Levi

Additional guidance on sewer flow capacity analysis:

- 1. If clear outliers on flow monitoring data to be used are present, make the case with Water Resources and obtain approval to use an alternate peak from the data. Sometimes using a percentile of the flow data is a valid method for filtering out outliers.
- 2. 50% of the total pool backwash flows for all developments pending construction or case approval within the sewer basin being analyzed should be applied at the most upstream applicable point(s) in the analysis reach (Safari and Bluesky in this case)
- 3. d/D max can be 0.80 with pool backwash flows incorporated as these are short-term conditions (but accommodations for I&I still need to be made so this is a firm upper limit)
- 4. d/D max without pool backwash flows should adhere to DS&PM (0.65/0.70) unless approved otherwise by Water Resources...some accommodations are sometimes made with d/D max where I&I potential is low i.e. basin area, geometry, and topography; and build-out conditions in the basin are apparent.(likely not applicable in this case)
- 5. Pending development flow peaking factors for approved but pending developments (but not including the subject development) can be reduced in direct linear proportion to their distance from the critical point of hydraulic analysis. A maximum peaking factor reduction of 10% at 5,280ft and further can be applied e.g. a PF of 4.5 would become 4.05 at a mile or more out, or 4.28 at half-a mile out.
 - This is intended to address some attenuation on short-term peak flows originating in pending projects some distance away.
 - This reduction only applies to approved developments that are not yet discharging flows to the collection system but need to be addressed in the current capacity analysis.
 - The distance to the development should be the edge of the parcel at the furthest upstream point.
 - If a reach has equal critical hydraulic conditions the point of beginning measurement for distance is the most downstream point of the reach.

From: Dillon, Levi

Sent: Thursday, April 23, 2020 2:33 PM **To:** Doug Mann <doug@azseg.com> **Cc:** Ali Fakih <ALI@azseg.com>

Subject: RE: Stockdale Collective Master Plan

Please remove Ceasar's flow from Camelback analysis. including Macerich flows was an error in the Atwell (Waterview) report caused in-part by a last-minute decision by Macerich to not include the analysis of Phase 2 in their submittal. The capacity for the hotel is approved but it goes to Goldwater. Thanks, Levi



EXECUTIVE SUMMARY

This report presents details of future developments that are yet to be master planned with respect to wastewater infrastructure. This information is provided for reference only.

This report is intended to present and analyze the necessary water and sewer infrastructure for the hotel portion only. This revised Basis of Design (BOD) report will be for the Caesar's Republic Scottsdale, a hotel development on Lot 2 of Scottsdale Fashion Square Amended (BK 1201, PG 8). The report will address the City comments dated July 18, 2019 & January 15, 2020, and will provide a summary of the sanitary sewer analysis for the proposed hotel.

In the future, the ultimate build-out for Lot 2 will consist of the Caesar's Republic Scottsdale, 2- four story 287,500 square foot office buildings, and 2-one story 10,000 square foot restaurants. At the time of the future build-out, a separate BOD report will be required when an application for a design review on the remaining portion of Lot 2 is presented to the City of Scottsdale.

From: Doug Mann < doug@azseg.com>
Sent: Thursday, April 23, 2020 1:36 PM
To: Dillon, Levi < LDillon@Scottsdaleaz.gov>

Cc: Ali Fakih < <u>ALI@azseg.com</u>>

Subject: RE: Stockdale Collective Master Plan

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So the Master Plan will contain the following tables for your comment. Scottsdale Collective has multiple scenarios for each area so I evaluated a max/min flow in Tables 5 and 6.

My flows for the three proposed sites are per DS+PM. The 5 offsite entitled developments are close to the numbers you provided below. Doug

From: Dillon, Levi < LDillon@Scottsdaleaz.gov>

Sent: Thursday, April 23, 2020 1:05 PM **To:** Doug Mann < doug@azseg.com **Cc:** Ali Fakih < ALI@azseg.com

Subject: RE: Stockdale Collective Master Plan

[EXTERNAL EMAIL]

Doug, please give me a call to discuss further. While we want to be accurate and have assurance that any infrastructure modifications are justified, we also need to make sure that there is a consistent approach in analysis. While using average water meter data and DS&PM peaking factors could be technically valid it is not the standard analysis approach outlined in DS&PM. It would be interesting to see how it compares, but I cannot allow conclusions to be based on this approach at this time.

Given the following flows into Camelback lets discuss how you're thinking the analysis will proceed along with the conclusions for The Collective:

- 1. Existing flow peak 703gpm
- 2. Safari II: 170gpm (includes 100gpm of pool backwash)
- 3. Don and Charlie's Hotel/Restaurant: 184gpm (no pool backwash*)
- 4. Waterview Hotel and Residential: 548gpm (includes 100gpm of pool backwash)
- 5. Bluesky (Phase 1 and 2): 750gpm (no pool backwash*)
- 6. Total flow: 2,355gpm

7. Resultant d/D: 0.69 with 15" sewer @ 1% (at point east/downstream of Don and Charlie's)

*Only 50% of the total basin pool backwash needs to be accounted for with capacity analysis i.e. 200gpm

Thanks,

Levi C. Dillon, P.E. | *Sr. Water Resources Engineer*



"Water Sustainability through Stewardship, Innovation and People"

Contact Info

Direct: (480) 312-5319 Main office: (480) 312-5685 Fax: (480) 312-5615

Mailing/Office Address

Water Resources Administration 9379 E. San Salvador Dr. Scottsdale, AZ. 85258

Sending me an attachment over 5MB? Please use the link below:

https://securemail.scottsdaleaz.gov/dropbox/ldillon@scottsdaleaz.gov

From: Dillon, Levi

Sent: Thursday, April 23, 2020 8:46 AM **To:** Doug Mann < doug@azseg.com > **Cc:** Ali Fakih < ALI@azseg.com >

Subject: RE: Stockdale Collective Master Plan

I'll give you a call around 10am. This data won't be hard to get just want to discuss a few things. Thanks, Levi

From: Doug Mann < doug@azseg.com>
Sent: Thursday, April 23, 2020 8:42 AM
To: Dillon, Levi < LDillon@Scottsdaleaz.gov>

Cc: Ali Fakih < ALI@azseg.com>

Subject: RE: Stockdale Collective Master Plan

↑ External Email: Please use caution if opening links or attachments!

Hi Levi – it's Thursday. I'm at work. We need to your expert direction on this. Please call. Doug

From: Doug Mann

Sent: Friday, April 17, 2020 1:42 PM

To: Dillon, Levi < LDillon@Scottsdaleaz.gov>

Cc: Ali Fakih < ALI@azseg.com>

Subject: Stockdale Collective Master Plan

Hey Levi – as you recall, the Waterview Report concluded that additional flow from Blue Sky would exceed the city's d/D criteria on the 15" Camelback sewer.

So, it is obvious the three projects Stockdale Partners are master planning will not help this situation by using the DS+PM unit demands.

I propose we do a sanity check here and look at the yearly metered demands from Camelview Optima as that project is high-end residential and nearly fully occupied. Stockdale's product appears to have some similar residential component. For resort hotel land use, we can evaluate the metered demands for the W Hotel vs. the DS+PM. . If you can provide me the monthly metered demands for 2019 and pull the two design reports, I can evaluate the data and make a recommendation to go forward.

As it stands now, the Camelback sewer needs to be 18" or a relief sewer provided. Also beware that is a large box culvert under Camelback and any further pipe construction would be difficult, deep and costly. Thanks for any assistance you can provide and have a good weekend.

Douglas L. Mann Senior Project Manager



8280 E. Gelding Dr., Suite 101, Scottsdale, AZ 85260 M 480.265.5349 480.588.7226 doug@azseg.com

doug@azseg.com

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SHEET UPDATED 9/10/20, SEE LAST PAGES OF REPORT

OVERALL UTILITY EXHIBIT & KEY MAP

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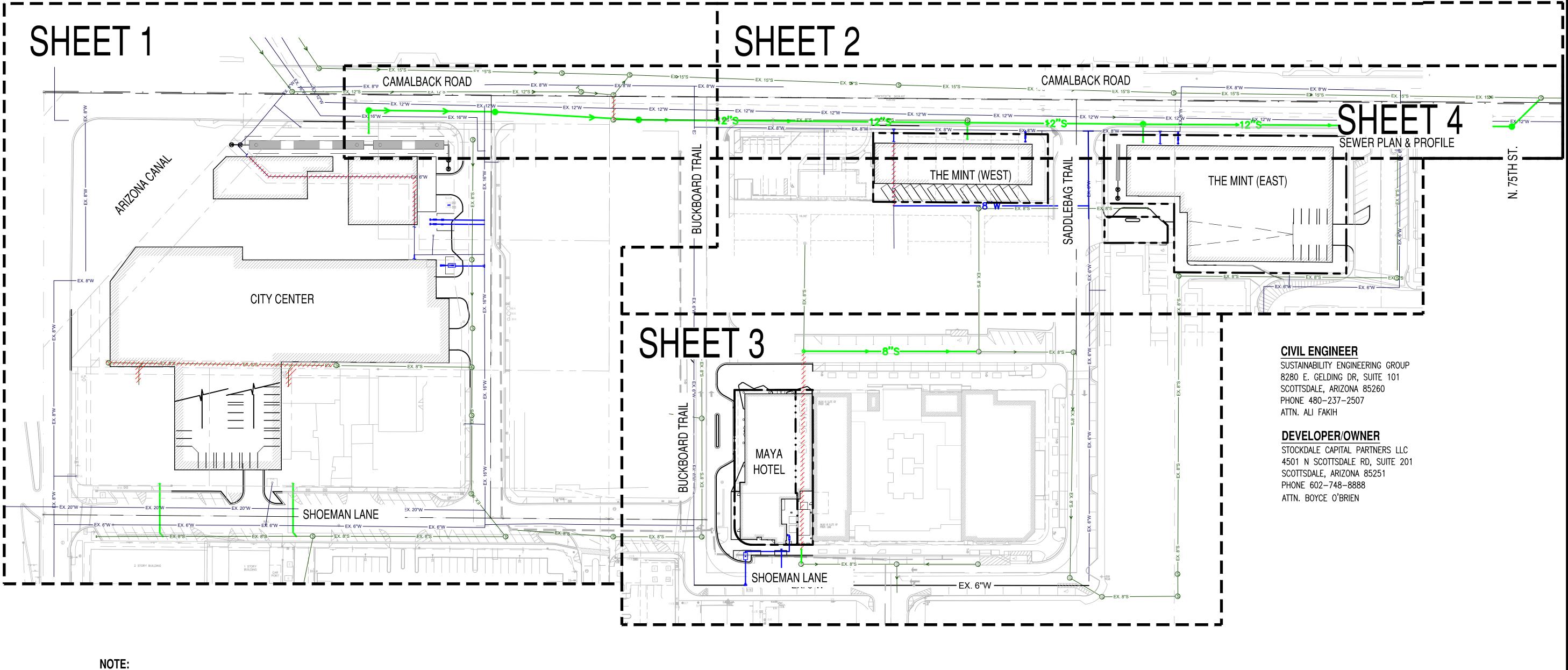
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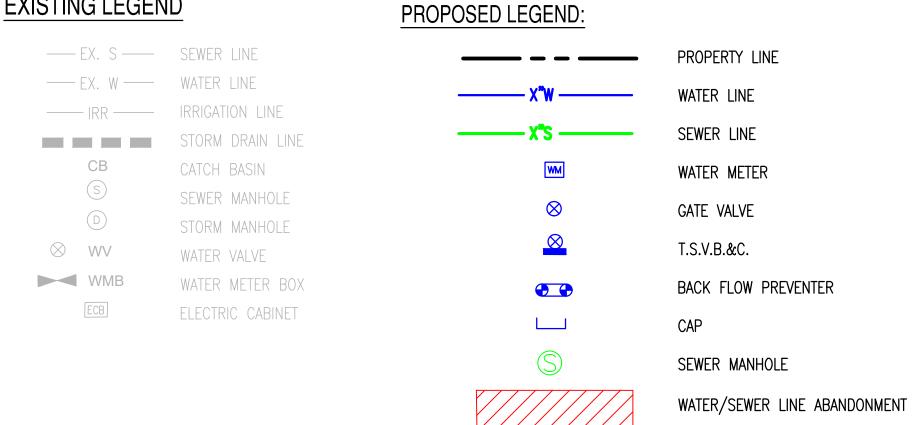
> UTILITY EXHIBIT KEY MAP

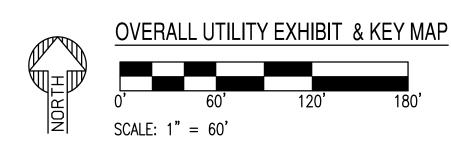
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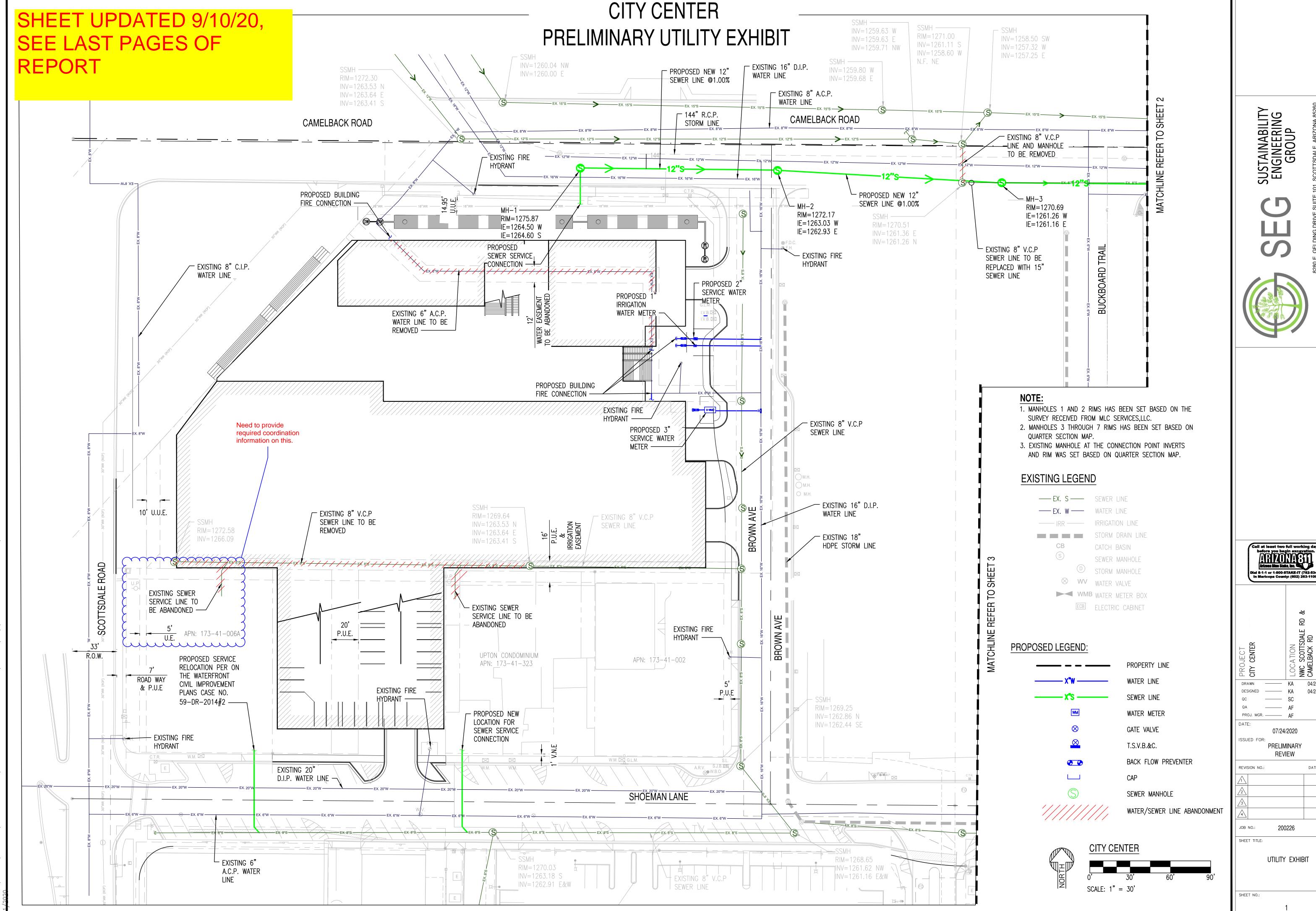


- 1. MANHOLES 1 AND 2 RIMS HAS BEEN SET BASED ON THE SURVEY RECEIVED FROM MLC SERVICES, LLC.
- 2. MANHOLES 3 THROUGH 7 RIMS HAS BEEN SET BASED ON
- QUARTER SECTION MAP.
- 3. EXISTING MANHOLE AT THE CONNECTION POINT INVERTS
- AND RIM WAS SET BASED ON QUARTER SECTION MAP.

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SHEET UPDATED 9/10/20, SEE LAST PAGES OF REPORT

> RIM=1267.20 IE=1259.50 S IE=1259.25 W

IE=1259.15 E →

THE MINT PRELIMINARY UTILITY EXHIBIT

PROPOSED NEW 12"

SEWER LINE @0.5%

- MH-5 RIM=1266.34

INV=1258.50 S

- EXISTING 8" A.C.P.

WATER LINE

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| CAMELBACK RD & CAME

PROJ. MGR. AF

DATE:

08/07/2020

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PRELIMINARY

REVIEW

REVISION NO.:

DATE

1

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/1 /2 /3 /4 JOB NO.: 200226

UTILITY EXHIBI

UTILITY EXHIBIT

SHEET NO.:

RIM=1268.77 INV=1258.13 W / RIM=1267.23 EXISTING 8" V.C.P SEWER LINE EXISTING 15" INV=1253.96 W INV=1258.03 E INV=1250.60 W RIM=1267.2 SEWER LINE TO BE REPLACED WITH 12" INV=1253.91 E INV=1261.36 W SEWER LINE — EXISTING 1 CAMELBACK ROAD SEWER LINE EXISTING 9.5' X PROPOSED NEW 12" EXISTING 6" A.C.P. WATER LINE TO - PROPOSED NEW 15" PROPOSED SEWER PROPOSED 2" 11' RCB STORM - EXISTING FIRE _SEWER LINE @0.50% $\stackrel{\searrow}{}$ EXISTING $\rat{8}$ " A.C.P. SERVICE CONNECTION 💳 SERVICE WATER SEWER LINE @0.50% — 2 BE REPLACED WATER LINE PROPOSED BUILDING WITH 8" LINE - PROPOSED 2" EXISTING 12" A.C.P. EXISTING FIRE FIRE CONNECTION WATER METER EXISTING 12" A.C.P. WATER LINE HYDRANT — APN: 173-41-208 WATER LINE PROPOSED BUILDING REMOVED. RE-ROUTED R.O.W. FIRE CONNECTION THROUGH ALLEY - PROPOSED "SEWER SERVICE CONNECTION PROPOSED 8" 90° BEND. CONNECT TO EXISTING 8" WATER LINE PAVED 16' PUBLIC ALLEY PROPOSED 8"X6" - EXISTING 8" V.C.P PROPOSED RE-ROUTED REDUCER — SEWER LINE 8" WATER SERVICE LINE -/ PROPOSED 6" WATER LINE CONNECT TO - EXISTING 8" EXISTING LINE V.C.P SEWER EXISTING 6" A.C.P. WATER EXISTING FIRE EXISTING 6" A.C.P. WATER LINE MATCHLINE REFER TO SHEET 3 THE MINT SCALE: 1" = 30STORM TOP =1253.23 SEWER BOT =1255.69 -— SSMH RIM=1264.35 INV=1246.18 W UC-2 **EXISTING LEGEND** SEWER TOP =1256.72 NOTE: IE=1255.55 SW PROPOSED LEGEND: IE=1246.50 SW (DROP-MH) WATER BOT =1260.50 — 1. MANHOLES 1 AND 2 RIMS HAS BEEN SET — EX. S — SEWER LINE PROPERTY LINE BASED ON THE SURVEY RECEIVED FROM MLC SERVICES,LLC. — EX. W — WATER LINE WATER LINE 2. MANHOLES 3 THROUGH 7 RIMS HAS BEEN CAMELBACK ROAD ----- IRR ------ IRRIGATION LINE SET BASED ON QUARTER SECTION MAP. SEWER LINE STORM DRAIN LINE 3. EXISTING MANHOLE AT THE CONNECTION POINT INVERTS AND RIM WAS SET BASED CATCH BASIN WATER METER ON QUARTER SECTION MAP. SEWER MANHOLE PROPOSED NEW 12" GATE VALVE RIM = 1264.54SEWER LINE @0.50% STORM MANHOLE IE=1258.88 W T.S.V.B.&C. \otimes WV WATER VALVE IE=1258.78 NE WMB WATER METER BOX BACK FLOW PREVENTER ELECTRIC CABINET SEWER MANHOLE PLAN VIEW WATER/SEWER LINE ABANDONMENT SHEET TITLE: SCALE: 1" = 30'

SHEET UPDATED 9/10/20, SEE LAST PAGES OF **REPORT**

ILINE REFER TO SHEET

MAYA HOTEL PRELIMINARY UTILITY EXHIBIT

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MAYA HOTEL

SCALE: 1" = 30'

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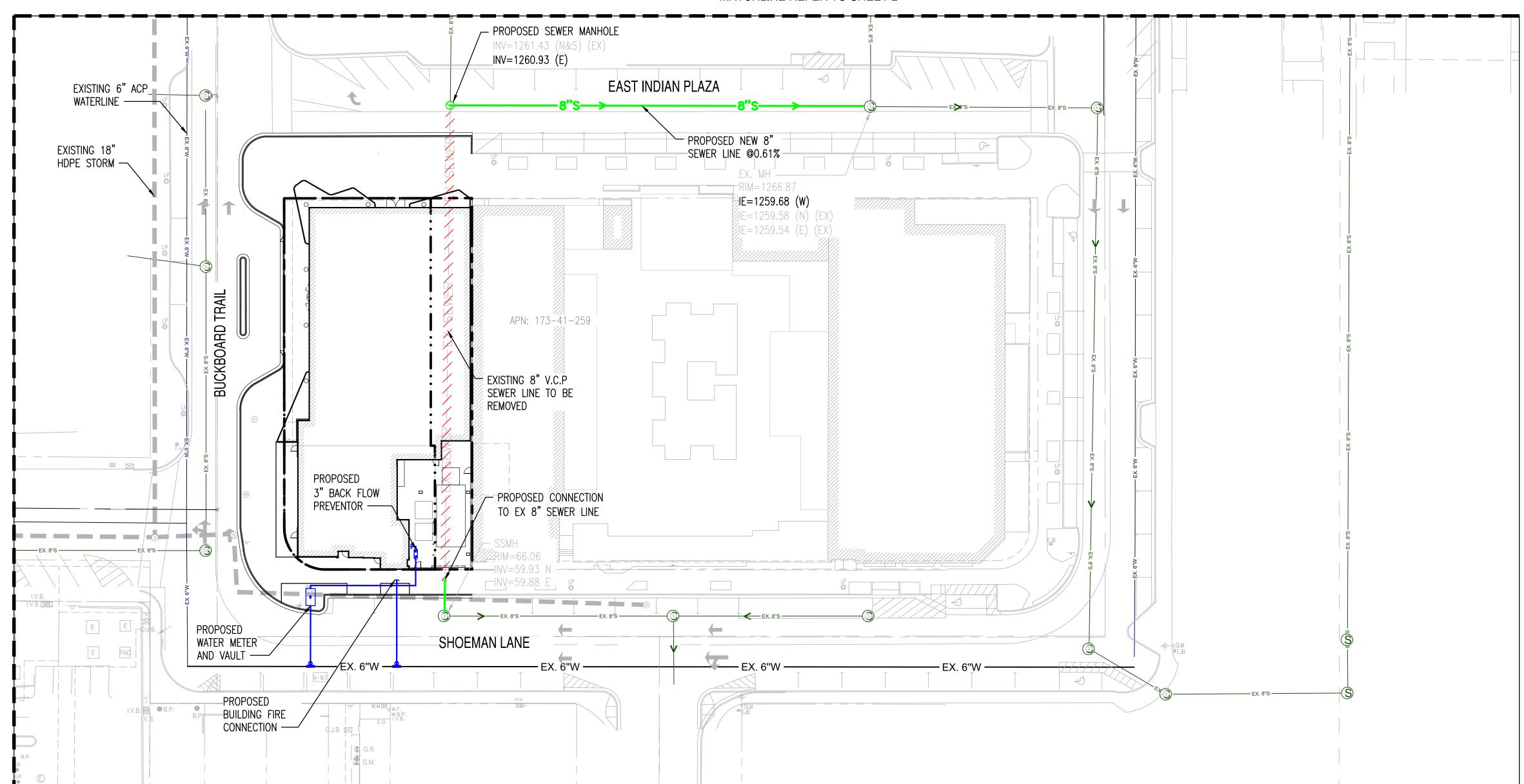
JOB NO.: 200226 SHEET TITLE:

SHEET NO.:

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UTILITY EXHIBIT

MATCHLINE REFER TO SHEET 2



EXISTING LEGEND	

---- EX. S ---- SEWER LINE — EX. W — WATER LINE ----- IRR ----- IRRIGATION LINE STORM DRAIN LINE CB CATCH BASIN SEWER MANHOLE STORM MANHOLE \otimes WV WATER VALVE WATER METER BOX ELECTRIC CABINET

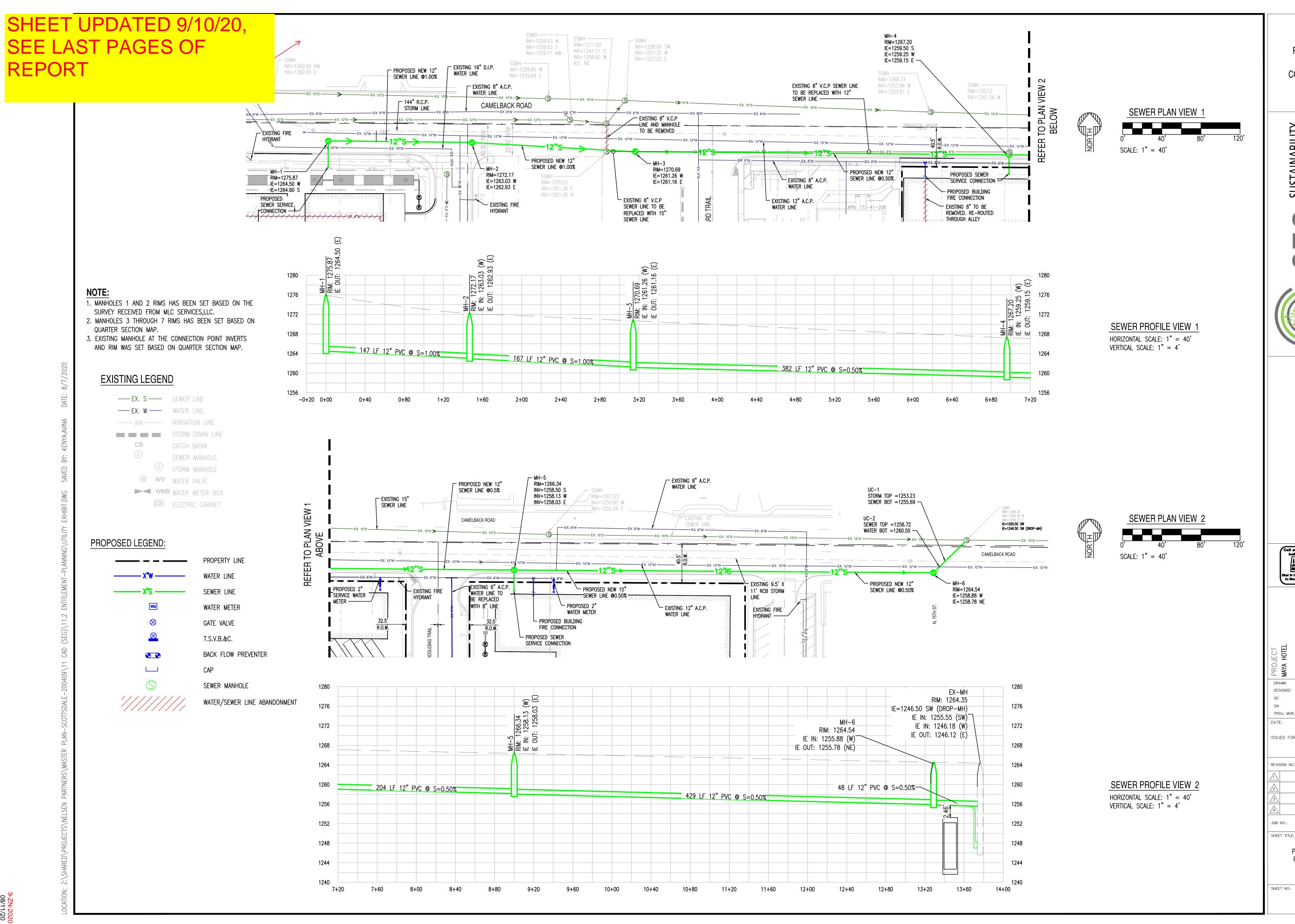
PROPOSED LEGEND:

PROPERTY LINE WATER LINE SEWER LINE WATER METER GATE VALVE T.S.V.B.&C. BACK FLOW PREVENTER **+** CAP SEWER MANHOLE

1. MANHOLES 1 AND 2 RIMS HAS BEEN SET BASED ON THE SURVEY RECEIVED FROM MLC SERVICES,LLC.

2. MANHOLES 3 THROUGH 7 RIMS HAS BEEN SET BASED ON QUARTER SECTION MAP.

3. EXISTING MANHOLE AT THE CONNECTION POINT INVERTS AND RIM WAS SET BASED ON QUARTER SECTION MAP.



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——— КА 04/28/2020 PROJ. MGR. — AF

DATE: PRELIMINARY

ISSUED FOR: REVIEW REVISION NO.: DATE:

SHEET TITLE: PROPOSED SEWER

200226

PLAN & PROFILE

RESOURCES HAS ADDED ADDITIONAL COMMENTS.

OVERALL UTILITY EXHIBIT & KEY MAP

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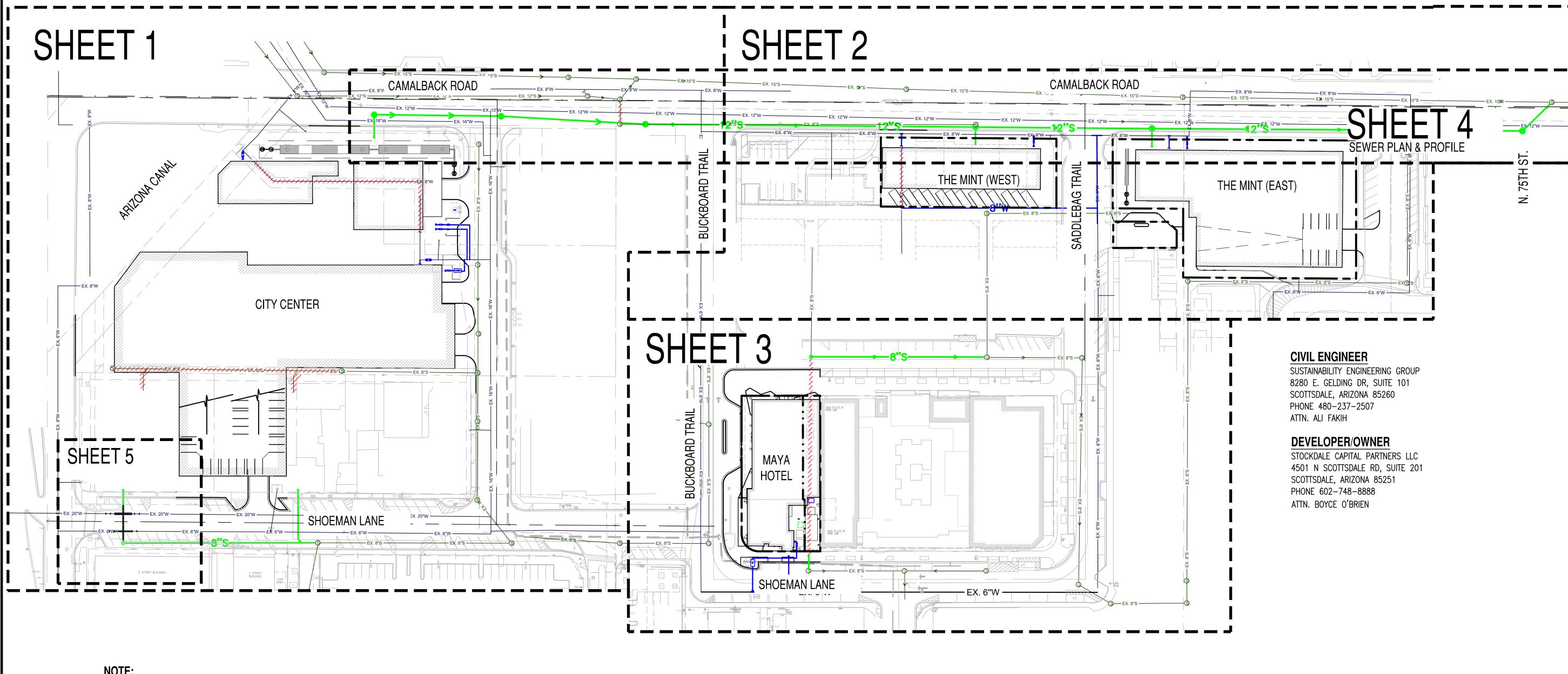
ISSUED FOR: ZONING

REVISION NO.: DATE: JOB NO.: 200226

> UTILITY EXHIBIT KEY MAP

SHEET NO.:

SHEET TITLE:

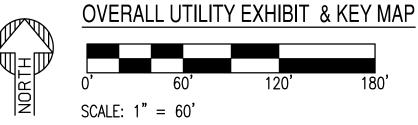


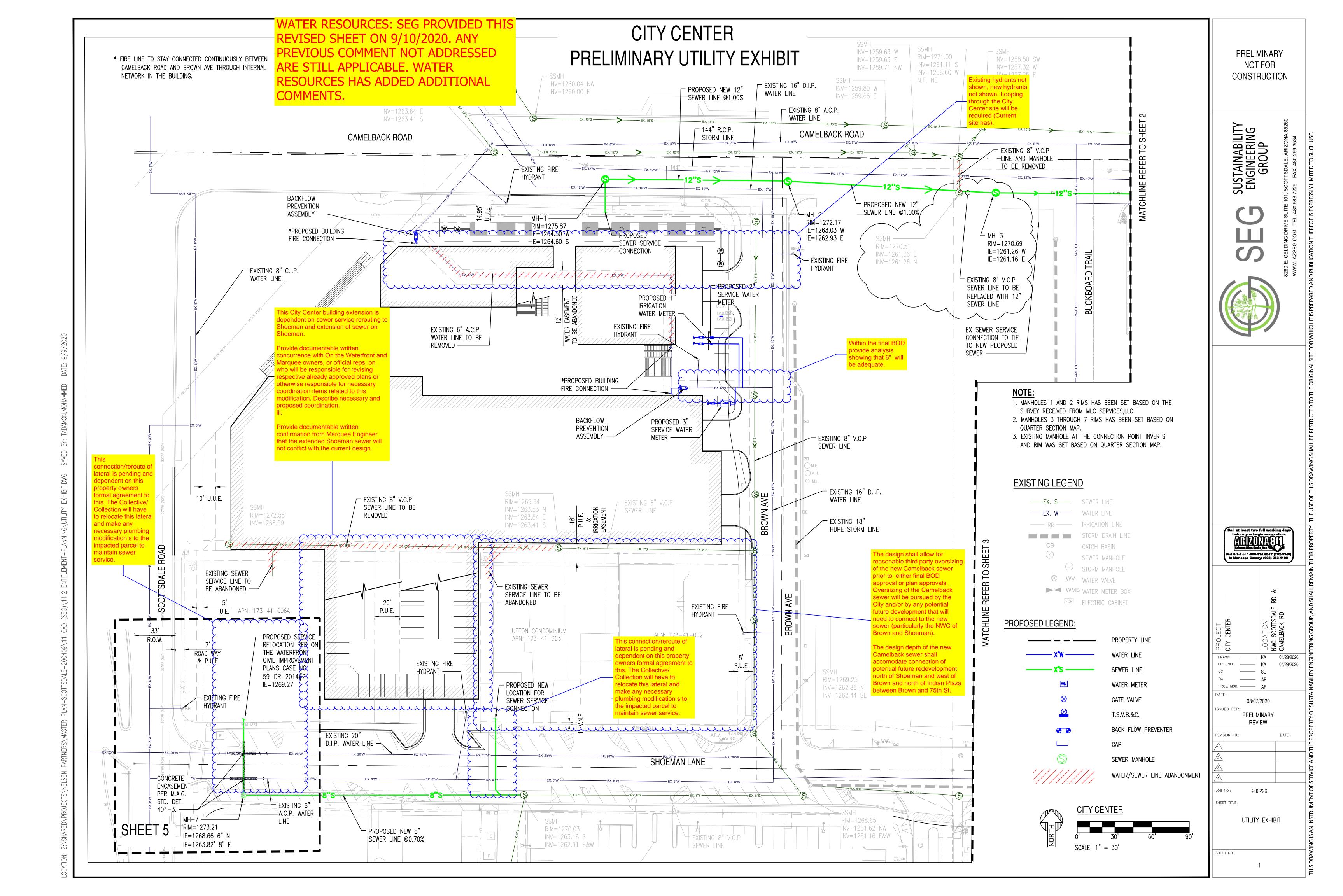
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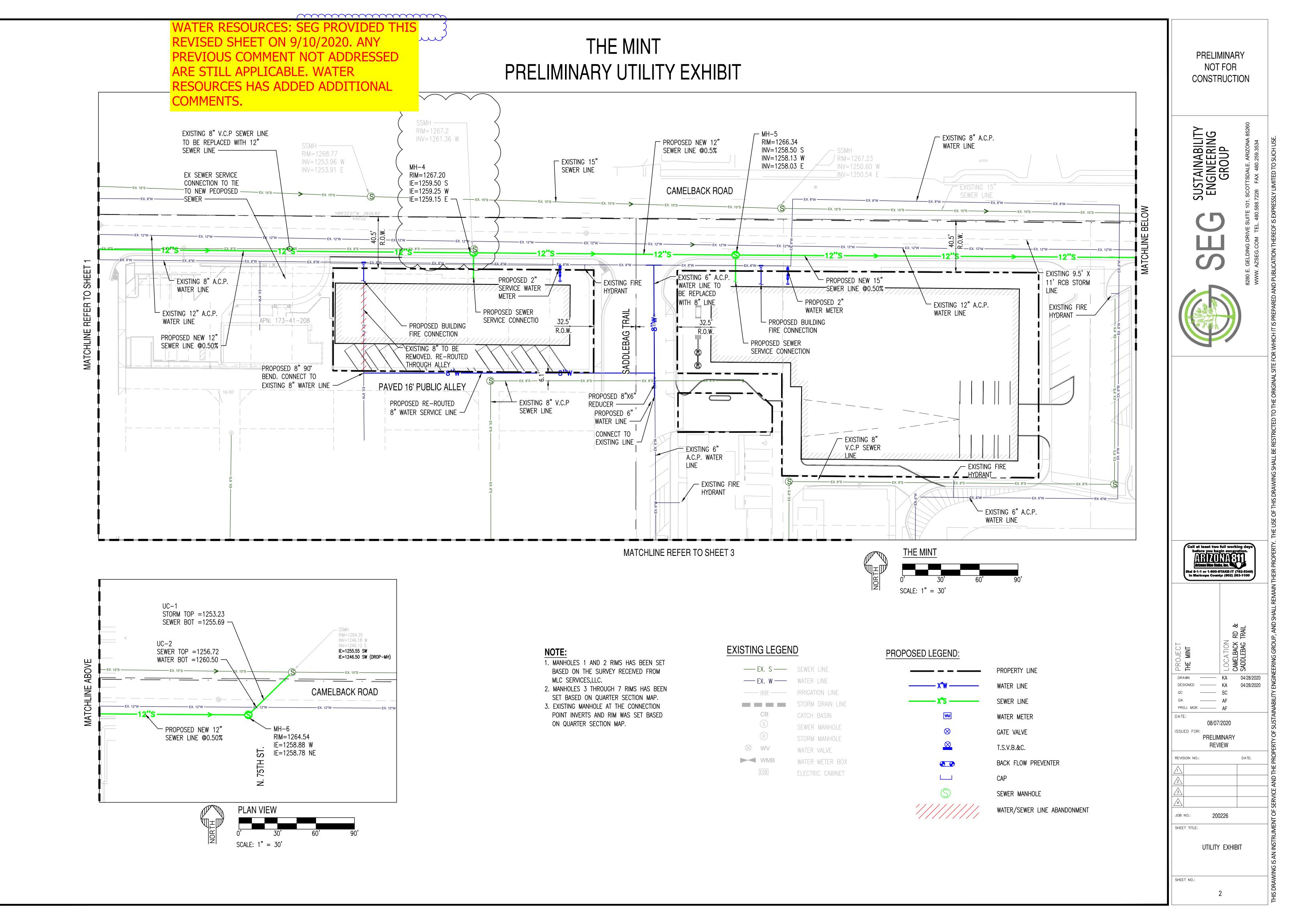
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EXISTING LEGEND

PROPOSED LEGEND: ---- EX. S ---- SEWER LINE PROPERTY LINE ---- EX. W ---- WATER LINE WATER LINE ----- IRR ----- IRRIGATION LINE SEWER LINE STORM DRAIN LINE CATCH BASIN WATER METER SEWER MANHOLE GATE VALVE STORM MANHOLE \otimes WV T.S.V.B.&C. WATER VALVE **₩MB** WATER METER BOX BACK FLOW PREVENTER ELECTRIC CABINET CAP SEWER MANHOLE WATER/SEWER LINE ABANDONMENT



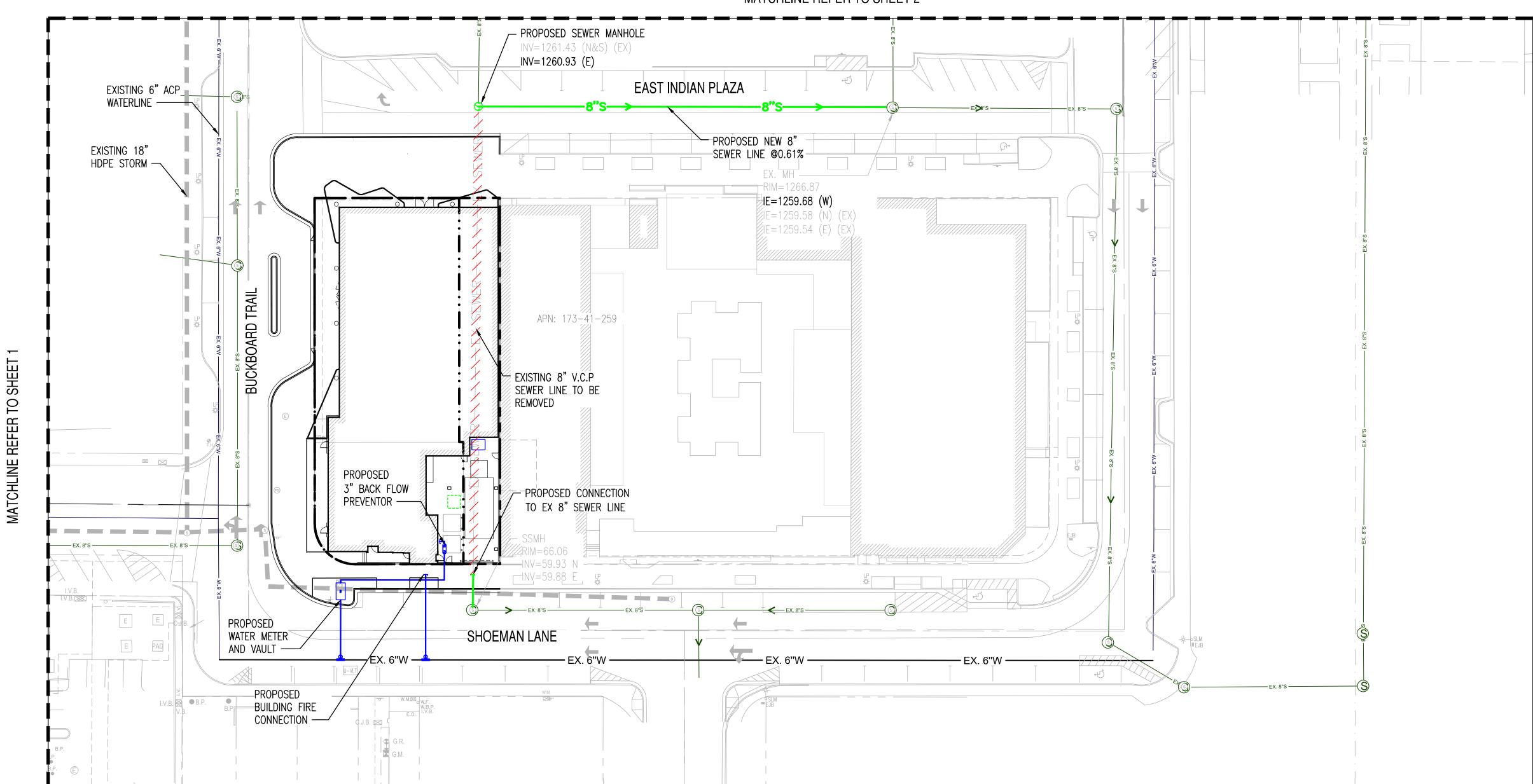




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MAYA HOTEL PRELIMINARY UTILITY EXHIBIT

MATCHLINE REFER TO SHEET 2



EXISTING LEGEND		PROPOSED LEGEND:				
—— EX. S ——	SEWER LINE		PROPERTY LINE			
—— EX. W ——	WATER LINE	x*w	WATER LINE			
—— IRR ——	IRRIGATION LINE	_				
	STORM DRAIN LINE	X*S	SEWER LINE			
СВ	CATCH BASIN	WM	WATER METER			
(S)	SEWER MANHOLE	\otimes	GATE VALVE			
D	STORM MANHOLE	_	GAIL VALVE			
\otimes WV	WATER VALVE		T.S.V.B.&C.			
WMB	WATER METER BOX		BACK FLOW PREVENTER			
ECB	ELECTRIC CABINET		040			

NOTE:

SEWER MANHOLE

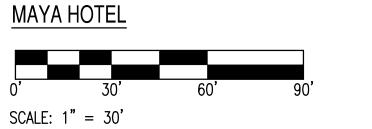
WATER/SEWER LINE ABANDONMENT

1. MANHOLES 1 AND 2 RIMS HAS BEEN SET BASED ON THE SURVEY RECEIVED FROM MLC SERVICES,LLC.

2. MANHOLES 3 THROUGH 7 RIMS HAS BEEN SET BASED ON QUARTER SECTION MAP.

3. EXISTING MANHOLE AT THE CONNECTION POINT INVERTS AND RIM WAS SET BASED ON QUARTER SECTION MAP.





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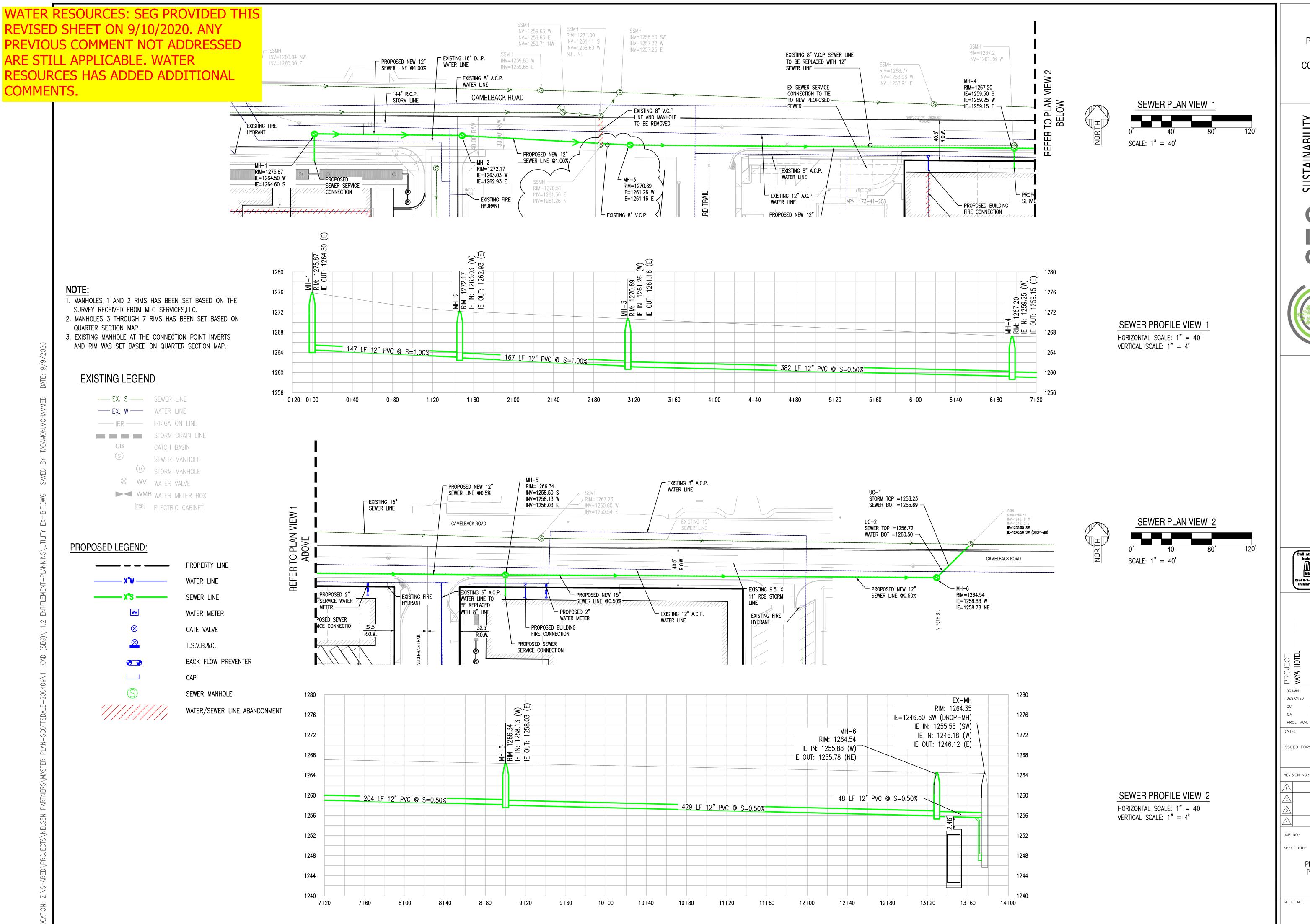
PROJECT Maya Hotel	LOCATION	NWC EAST INDIAN PLAZA & BUCKBOARD TRAIL
DRAWN	 KA	04/28/20
DESIGNED	 KA	04/28/20
QC	 SC	
QA	 AF	
DDO I MOD	۸.	

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DATE:	
	08/07/2020
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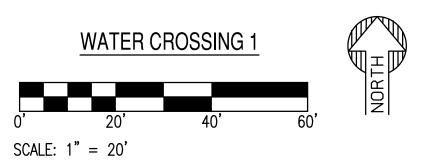
PROPOSED SEWER PLAN & PROFILE

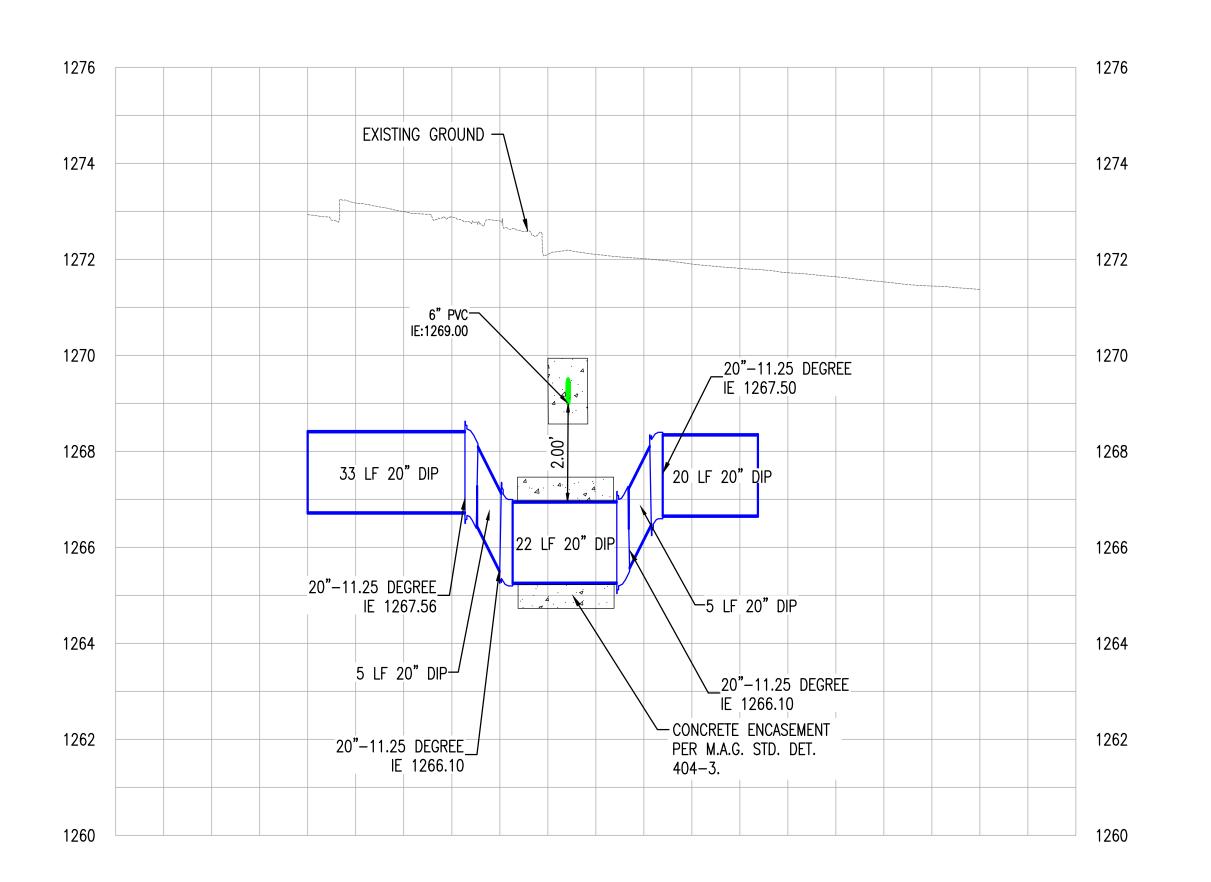
WATER RESOURCES: SEG PROVIDED THIS REVISED SHEET ON 9/10/2020. ANY PREVIOUS COMMENT NOT ADDRESSED ARE STILL APPLICABLE. WATER RESOURCES HAS ADDED ADDITIONAL COMMENTS.

- PROPOSED SERVICE RELOCATION PER ON THE WATERFRONT CIVIL IMPROVEMENT PLANS CASE NO. 59-DR-2014#2 WAY & IE=1269.27 P.U.E R/W & U.E. 10' R/W & U.E. 3' R/W - EXISTING FIRE EXISTING 20" D.I.P. WATER LINE — NOTHING SET EXISTING 6"

__A.C.P. WATER
LINE FD. C.O.S. 3.C.F. SHOEMAN LANE CONCRETE ENCASEMENT PER M.A.G. STD. DET. 404-3. PROPOSED NEW 8" / SEWER LINE @0.70% / /RIM = 1273.21/ IE=1268.66 6" N IE=1263.82' 8" E 2 STORY BUILDING

NOTE:
REQUIRES VERTICAL REALIGNMENT PER MAG STD. DET.
370. PROVIDE 2' MINIMUM CLEAR SEPARATION WITH
CONCRETE ENCASEMENT PER MAG STD. DET. 404-3 &
MAG SPECIFICATION 610.5.





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PROJECT Maya Hotel	LOCATION	NWC EAST INDIAN PLAZA	& BUCKBOARD TRAIL
DRAWN	 KA		04/28/2020
DESIGNED	 KA		04/28/2020
QC	 SC		
QA	 AF		
PRO L MCP	۸۲		

PROJ. MGR. —	—— AF
DATE:	08/07/2020
ISSUED FOR:	

ISSUED FOR: PRELIMINARY REVIEW	
REVISION NO.:	DA
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2			
3			
4			
JOB	NO.:	200226	

WATER CROSSING

SHEET NO.:

SHEET TITLE:

5

WATER CROSSING 1

HORIZONTAL SCALE: 1" = 20'
VERTICAL SCALE: 1" = 2'