

# PRELIMINARY DRAINAGE REPORT

## The Triangle

7120 E. Indian School Road  
Scottsdale, AZ 85251

Prepared For:

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Project Number: 200504

Revision Date: October 16, 2020 (Rezoning)

Plan #	_____
Case #	10-ZN-2020
Q-S #	_____
<input checked="" type="checkbox"/> Accepted	
<input type="checkbox"/> Corrections	
N. Baronas	11/9/2020
Reviewed By	Date

Case No.: 63-PA-2020  
10-ZN-2020

Plan Check No.: TBD

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## 1. INTRODUCTION

This Preliminary Drainage Report represents the storm water analysis for the HoJo Development (The Triangle) proposed in Scottsdale, Arizona. The purpose of this preliminary report is to provide the hydrologic and hydraulic analysis, required by the City of Scottsdale, to support the proposed rezoning for said development. This report includes discussions and calculations defining the storm water management concepts for the collection and conveyance necessary to comply with the drainage requirements of the City of Scottsdale and Maricopa County. Preparation of this report has been done in accordance with the requirements of the City of Scottsdale Design Standards & Policies Manual (DS&PM) 2018 <sup>1</sup>, and the Drainage Design Manuals for Maricopa County, Arizona, Volumes I<sup>2</sup> and Volume II<sup>3</sup>.

## 2. LOCATION AND PROJECT DESCRIPTION

### 2.1 LOCATION:

The subject property consists of land located south of the 3<sup>rd</sup> Avenue and Craftsman Court, the between 3<sup>rd</sup> Avenue and Indian School Road in Scottsdale, AZ:

- A portion of the southeast 1/4 of Section 22, Township 2 North, Range 4 East of the Gila and Salt River Base and Meridian, Maricopa County, Scottsdale, Arizona.
- Parcel ID: Parcel 173-50-034; The Venue, Zoning C-2 (7117 E. 3<sup>rd</sup> Avenue)  
Parcel 173-50-108A; Kimberly at Michael V. Salon, Vape Sky, Teres A Nail Bar, Zoning C-2 (7120 E. Indian School Road, Scottsdale, AZ 85251)  
Parcel 173-50-117B; Howard Johnson Inn, Zoning C-2 (7110 E. Indian School Road, Scottsdale, AZ 85251)

Refer to **FIGURE 1 - Vicinity Map** for the project's location with respect to major cross streets

### 2.2 EXISTING AND PROPOSED DEVELOPMENTS SURROUNDING THE SITE:

- West: The site is bound by an alley with the following across as follows:  
Parcel 173-50-129; Marshall Way Plaza; Zoning is C-2  
Parcel 173-50-114; Pink Plaza; Zoning is C-2
- East: Parcel 173-50-119E, 173-50-094, -100A; commercial shops, Zoning D-/OR-2
- North: Across 3<sup>rd</sup> Avenue: Parcel 173-50-019; Foxy Spray Tans; Zoning is C-2  
Across 3<sup>rd</sup> Avenue: Parcel 173-50-146; Cadre Condominiums; Zoning is C-2
- South: Across Indian School Road are parcels:  
Parcel 130-12-013, -012, -011; Commercial offices; Zoning is C-2.  
Parcel 130-12-007A

### 2.3 EXISTING SITE DESCRIPTION:

The project area includes approximately 144,173 sf. ft. (3.310 acres) of land designated as C-2 per C.O.S. zoning map 5. The site is currently developed and includes three commercial developments with parking lots: Howard Johnson Inn, The Venue and a commercial building comprised of several retail stores. The developments are separated by parking areas.

Per Topographic Survey prepared by AWL Land Surveying, the site slopes from northwest to southeast at approximately 1.00%. Elevation varies from approximately 1266.24 at the northwest corner to approximately 1262.21 at the southeast corner. The site drains to the perimeter streets and alley.

Refer to **FIGURE 2** attached for an aerial of the site.

#### **2.4 PROPOSED SITE DEVELOPMENT:**

Site development includes the demolition of the Howard Johnson Inn, The Venue structures and their designated parking lots as well as the construction of a new hotel, residential building and townhome complex. The development will include one proposed access on the north side to 3<sup>rd</sup> Avenue, two access entrances to the alley on the west, and will maintain the two existing driveway entrances to Indian School Road. An underground parking structure is proposed for the development. Refer to **Appendix III - Preliminary Grading Plan** for site layout.

#### **2.5 FLOOD HAZARD ZONE:**

FIRM Map Number 04013C2235L dated October 16, 2013 indicates the site is designated as Zone "X". As such, it is defined as areas determined to be outside the 0.2% annual chance floodplain and therefore is not in a special flood hazard area.

Refer to **FIGURE 3** for the FIRM.

### **3. EXISTING DRAINAGE CONDITIONS**

#### **3.1 OFF-SITE DRAINAGE PATTERNS**

The topographic survey provides the following information for offsite drainage:

- There is an existing alley to the west approximately 16' wide that slopes from north to south. The alley conveys drainage from the site onto Indian School Road.
- The existing sidewalks to the north and south of the site drain into their adjacent streets, 3<sup>rd</sup> Avenue and Indian School Road, respectively.
- Adjacent roads have curb and gutters conveying flow within the rights-of-way. Flows from Indian School Road are collected in EX. CB-1 and EX. CB-2.
- An existing inlet, EX. CB-3, located east of the EX-3 drainage area concentration point, collects runoff from EX-3 and adjacent parking area. Refer to **Appendix IV** for site photos.
- Based on existing conditions obtained by the topo and site photos, the site sits 1-2' above the pavement grade along the east parcels. Therefore, it was concluded that the existing site is not affected by any offsite flows.
- Refer to Section 5.1 for additional discussion of safety of finish floor elevations.

#### **3.2 ONSITE DRAINAGE**

Based on the topographic information, only drainage areas EX-1 and EX-2 located at the north end of the property drain to 3<sup>rd</sup> Avenue. The runoff along 3<sup>rd</sup> avenue flows east where it is ultimately conveyed onto N. Scottsdale Road. Drainage area EX-3 and EX-4 drain to the neighboring parcel to the east, but ultimately makes their way to Indian School Road. Similarly, drainage area EX-7 drains to the alley adjacent to the west but ultimately discharges to Indian School Road. Drainage areas EX-5 and EX-6 drain to Indian School Road and flows are captured by catch basins along Indian School Road.

Refer to Appendix II for **Existing Conditions Drainage Area Map**.

The Rational Method was utilized to compute the on-site peak discharges. The Rational Method equation is calculated as shown below:

$$Q=C_{wt}IA$$

Where:  $C_{wt}$  = The runoff coefficient relating runoff to rainfall

$I$  = Average rainfall intensity in inches/hour, lasting for  $T_c$

$T_c$  = The time of concentration (using five minutes for the developed areas)

$A$  = The contributing drainage area in acres

Refer to section 4.3 for land characteristics.

Table 1 below is a summary of existing  $Q_{100}$  runoff and required storage volume:

**TABLE 1:**

Existing Runoff Calculations				
Drainage	Area	$C_w$	intensity	Q
Area ID	(acres)	(-)	(in/hr)	(cfs)
<b>Contributing Areas to 3rd Ave</b>				
EX-1	0.20	0.95	7.44	1.41
EX-2	0.67	0.95	7.44	4.74
<b>Totals</b>	<b>0.87</b>	<b>0.95</b>		<b>6.15</b>
<b>Contributing Areas to Indian School Road</b>				
EX-3	0.24	0.95	7.44	1.70
EX-4	0.36	0.95	7.44	2.54
EX-5	0.48	0.94	7.44	3.36
EX-6	1.04	0.87	7.44	6.73
EX-7	0.19	0.95	7.44	1.34
<b>Totals</b>	<b>2.31</b>	<b>0.91</b>		<b>15.67</b>

Overall project area includes **3.18 Acres at  $C_{wt} = 0.92$**  (Existing conditions, to back of sidewalk)

Refer to the **Existing Cwt Exhibit (Exhibit A)** and **Existing Conditions Drainage Area Map (Exhibit C)** in **Appendix II**.

## 4. PROPOSED STORM WATER MANAGEMENT

### 4.1 DESIGN INTENT:

On-site drainage will be directed off-site via overland flow to the historical outlets. This is a re-development of existing commercial land; therefore, the City of Scottsdale specifies that on-site retention shall be provided as described in Section 4.2 below.

The majority of the entire site is proposed impervious with minor increases in runoff compared to existing conditions.

Refer to **Appendix II** for **Proposed Conditions Drainage Area Map**.

### 4.2 DESIGN STORM REQUIREMENTS:

In accordance with City of Scottsdale requirements, stormwater storage for the 100-year 2-hour storm event is required based on maintaining existing retention volume plus the difference between the pre

vs. post development runoff from the 100-year 2-hour storm event if increased or first flush, whichever is greater.

**4.3 LAND CHARACTERISTICS:**

The proposed project site consists mainly of roofs and covered gathering areas, sidewalks and minor landscape areas along the south and north boundaries. Based on the DS&PM, runoff coefficients for the 100-year storm event used are as follows:

- C=0.95 for building or concrete
- C=0.95 for paved surface
- C=0.45 for undisturbed natural desert or desert landscape

**HYDROLOGIC ANALYSIS:** The hydrologic analysis is determined using the procedures in the City of Scottsdale Design Standards & Policies Manual and the Drainage Design Manual for Maricopa County, Arizona, Volume I. The Rational Method was utilized to compute the on-site peak discharges. The Rational Method equation is displayed as shown below:

$$Q=C_{wt}IA$$

Where:  $C_{wt}$  = The runoff coefficient relating runoff to rainfall

$I$  = Average rainfall intensity in inches/hour, lasting for  $T_c$

$T_c$  = The time of concentration (Using Five minutes for the developed areas)

$A$  = The contributing drainage area in acres

Table 2 below is a summary of Proposed  $Q_{100}$  runoff.

**TABLE 2:**

<b>Proposed Runoff Calculations</b>				
Drainage	Area	$C_w$	intensity	Q
Area ID	(acres)	(-)	(in/hr)	(cfs)
<b>Contributing Areas to 3rd Ave</b>				
DA-1	0.37	0.95	7.44	2.62
DA-2	0.47	0.93	7.44	3.25
<b>Totals</b>	<b>0.84</b>	<b>0.94</b>		<b>5.87</b>
<b>Contributing Areas to Indian School Road</b>				
DA-3	1.13	0.95	7.44	7.99
DA-4	0.48	0.95	7.44	3.39
DA-5	0.46	0.91	7.44	3.11
DA-6	0.27	0.95	7.44	1.91
<b>Totals</b>	<b>2.34</b>	<b>0.94</b>		<b>16.40</b>

Overall project area includes **3.18 Acres at  $C_{wt} = 0.94$**  (Proposed conditions, to back of curb) Refer to the **Proposed Cwt Exhibit (Exhibit B)**, **Proposed Conditions Drainage Area Map (Exhibit D)** and Calculations in **Appendix II**.

Table 5 summarizes the calculated onsite peak flows for the 100-yr storm event under proposed and existing conditions.

**TABLE 3:**

<b>Runoff Calculations Summary</b>			
<b>Outfall</b>	<b>Proposed</b>	<b>Existing</b>	<b>Difference</b>
3rd Avenue	5.87	6.15	<b>-0.28</b>
Indian School Road	16.40	15.67	<b>0.73</b>

The increase in flow to Indian School Road is less than 1 cfs and the storm drains have the capacity to accommodate the project flow based on calculations in Appendix II and Section 4.6.

**4.4 STORMWATER RETENTION:**

**PRE VS POST:** Based on topographic survey there is no retention provided on the existing development. Based on the performed calculations above, existing condition and proposed development storage requirements for the 100-yr, 2-hr storm event are calculated as follows:

**TABLE 4:**

<b>Pre vs. Post Required Storage Volume Calculation Summary</b>					
$V = A * (C_{wpost} - C_{wpre}) * D / 12$					
Area	$C_{wpost}$	$C_{wpre}$	Depth	Volume Req.	
(acres)	(-)	(-)	(in)	(acre-ft)	(CF)
3.18	0.94	0.92	2.16	0.011	<b>498.67</b>

**FIRST FLUSH:** First Flush storage required is calculated in accordance with COS– DS&PM. According to the DS&PM, sites less than one (1) acre in size may be waived from the First Flush requirement with approval from staff. The area considered in the first flush calculation is the disturbed area minus any true roof top area. As shown in the Proposed Conditions Roof Area Exhibit (Exhibit E) in Appendix II, the areas considered in the first flush calculation (**0.92 ac**) quantified to be less than 1 acre. As such, the site could be considered exempt from the first flush requirement.

Refer to **Proposed Conditions Roof Area Exhibit (Exhibit E)** in **Appendix II** for areas considered in the first flush calculation.

**4.5 STORMWATER RETENTION WAIVER:**

The proposed development includes an underground parking structure to the limits of the property thereby eliminating potential open or underground retention areas. Runoff to the north (3<sup>RD</sup> Avenue) is reduced by 0.28 cfs. The total increase to the south (Indian School Road) is 0.73 cfs and the street and storm drains have capacity to accommodate the project flows based on projected peak flows in Table 3 and offsite flows presented in section 4.6. First flush treatment is not required. Therefore, a Request for Stormwater Storage Waiver for 499 cf will be applied for based on section 4-1.203 DS&PM. Refer to **Appendix IV** for Stormwater Retention Waiver.

**4.6 OFFSITE STORM SYSTEM ANALYSIS:**

For the purpose of this report, the calculated runoff was used to analyze the effects of the increase in runoff to the existing system based on the existing peak flows presented in the Lower Indian Bend Wash Area Drainage Master Study Hydrology and Hydraulics Report, Contract No.: FCD 2011C019, dated December 2017.



The hydraulic grade line and capacity was analyzed for the existing 48" RGRCP reach along E. Indian School Road. The reach begins at the upstream manhole, MJ8W2STEISRL, located at the Alley and E. Indian School Road intersection and continues east to N Scottsdale Road at manhole MJ23W2STSRL, refer to Appendix V for an excerpt of the Lower Indian Bend Wash ADMP corresponding to the analyzed reach along Indian School Road.

The onsite peak flows from Table 6 were calculated through the use of the Rational Method and indicate that there is a 0.73 cfs increase to Indian School Road. The hydraulic analysis was performed for the existing 36" storm drain (upstream) and the 48" (downstream) to verify the capacity of the existing system in respect to the additional flow. The analysis was performed using Flowmaster to analyze the existing 36" and 48" RGRCP reach based on the existing peak flow of 32.4 cfs and 71.5 cfs at pipes **C6W2STEISRL** and **C1W2STEISRL**, respectively, obtained from the Lower Indian Bend Wash ADMP, and the additional 0.73 cfs obtained from the calculated on-site peak flow. Refer to **Appendix V** for Inlet Summary Table and Pipe Discharge Tables in the Lower Indian Bend Wash ADMP.

36" Pipe C6W2STEISRL: 32.4 cfs (Existing) + 0.73 cfs (Post) = **33.13cfs**

48" Pipe C1W2STEISRL: 71.5 cfs (Existing) + 0.73cfs (Post) = **72.23 cfs**

Hydraulic calculations indicate that 52.94 cfs is available for the existing 36" RGRCP (S=0.63%) at full capacity and 114.01 cfs available for the existing 48" RGRCP (S=0.63%) at full capacity. The existing 36" and 48" RGRCP storm drains are capable of conveying the total post conditions flow increase of 0.73 cfs. Refer to **Appendix II** for pipe capacity calculations.

#### **4.7 ADEQ WATER QUALITY REQUIREMENTS:**

The total disturbed area of this site is approximately 3.18 acres. The Arizona Department of Environmental Quality requires that any site disturbance over an acre is required to submit an NOI. An NOI will be submitted to ADEQ for this site after the first submittal of the construction documents as this site disturbance is over 1 acre.

## **5. FLOOD SAFETY FOR DWELLINGS**

### **5.1 FINISHED FLOOR ELEVATIONS**

This project lies in an "X" Flood Zone. Therefore, the proposed building finished floor elevations will be set a minimum of 12 inches above the 100-year high-water elevation of any adjacent streets and drainage paths and a minimum of 14 inches above the lowest top of curb of the lot. This will ensure that each building will be well above the 100-year water level. All buildings with the exception of Buildings 4 and 5 are set 14 inches above the lowest top of curb but are set more than 12 inches above the 100-year water surface elevation at the ultimate outfall. The site ultimate outfall is located at the southeast corner at an elevation of 1261.15. The ultimate outfall elevation is greater than 14" below the minimum finish floor elevation.

**TABLE 5:**

<b>FFE Summary</b>			
<b>BLDG (ID)</b>	<b>Finish Floor Elevation (ft)</b>	<b>Lowest Top of Curb (ft)</b>	<b>Difference (in)</b>
1	66.00	64.62	16.56
2	66.00	64.42	18.96
3	66.00	62.81	38.28
4	65.00	64.20	9.60
5	65.00	63.98	12.24
6	64.50	62.40	25.20

## 6. CONCLUSIONS

### 6.1 OVERALL PROJECT:

1. The finish floor elevations will be designed a minimum of 12 inches above the 100-year water surface in adjacent streets and drainage paths and a minimum of 14 inches above the lowest top of curb of the lot.
2. A stormwater storage waiver will be requested for the redevelopment of the site.

### 6.2 PROJECT PHASING:

This project will be constructed in a single phase.

## 7. WARNING AND DISCLAIMER OF LIABILITY

RE: following page.

## 8. REFERENCES

1. *Design Standards & Policies Manual, City of Scottsdale – January 2018*
2. *Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology, Flood Control District of Maricopa County, Fourth Edition, December 14, 2018*
3. *Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics, Flood Control District of Maricopa County, December 14, 2018*

# GRADING & DRAINAGE LANGUAGE

## WARNING AND DISCLAIMER OF LIABILITY

The City’s Stormwater and Floodplain Management Ordinance is intended to minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding. The Stormwater and Floodplain Management Ordinance identifies floodplains, floodways, flood fringes and special flood hazard areas. However, a property outside these areas could be inundated by floods. Also, much of the city is a dynamic flood area; floodways, floodplains, flood fringes and special flood hazard areas may shift from one location to another, over time, due to natural processes.

### WARNING AND DISCLAIMER OF LIABILITY

The flood protection provided by the Stormwater and Floodplain Management Ordinance is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by constructed or natural causes. The Stormwater and Floodplain Management Ordinance does not create liability on the part of the city, any officer or employee thereof, or the federal, state or county government for any flood damages that result from reliance on the Ordinance or any administrative decision lawfully made thereunder.

Compliance with the Stormwater and Floodplain Management Ordinance does not ensure complete protection from flooding. Flood-related problems such as natural erosion, streambed meander, or constructed obstructions and diversions may occur and have an adverse effect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations.

I have read and understand the above.

\_\_\_\_\_  
Plan Check #

\_\_\_\_\_  
Owner

\_\_\_\_\_  
Date



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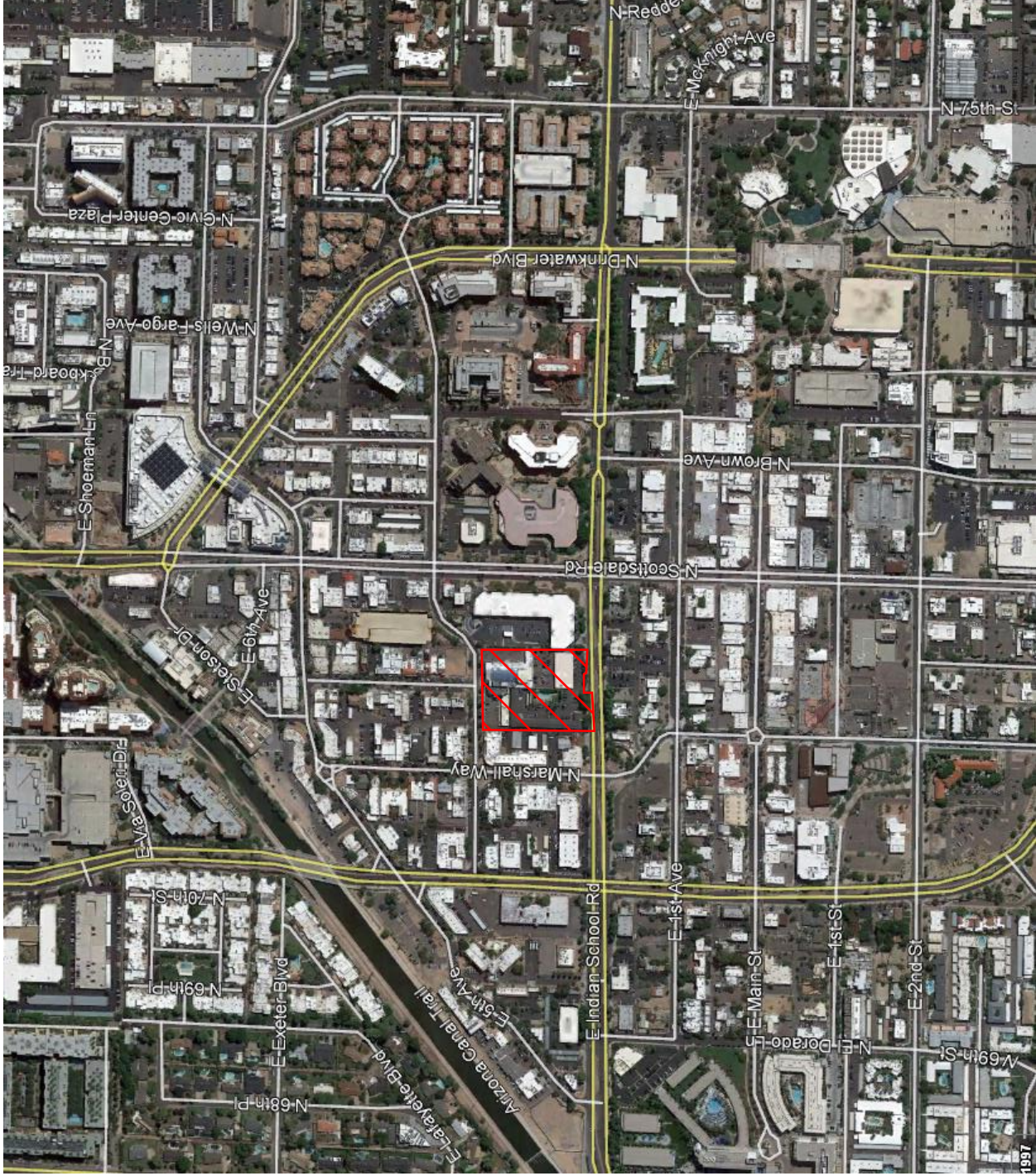


FIGURE 1  
VICINITY MAP

8280 E. Gelding Dr., Suite 101  
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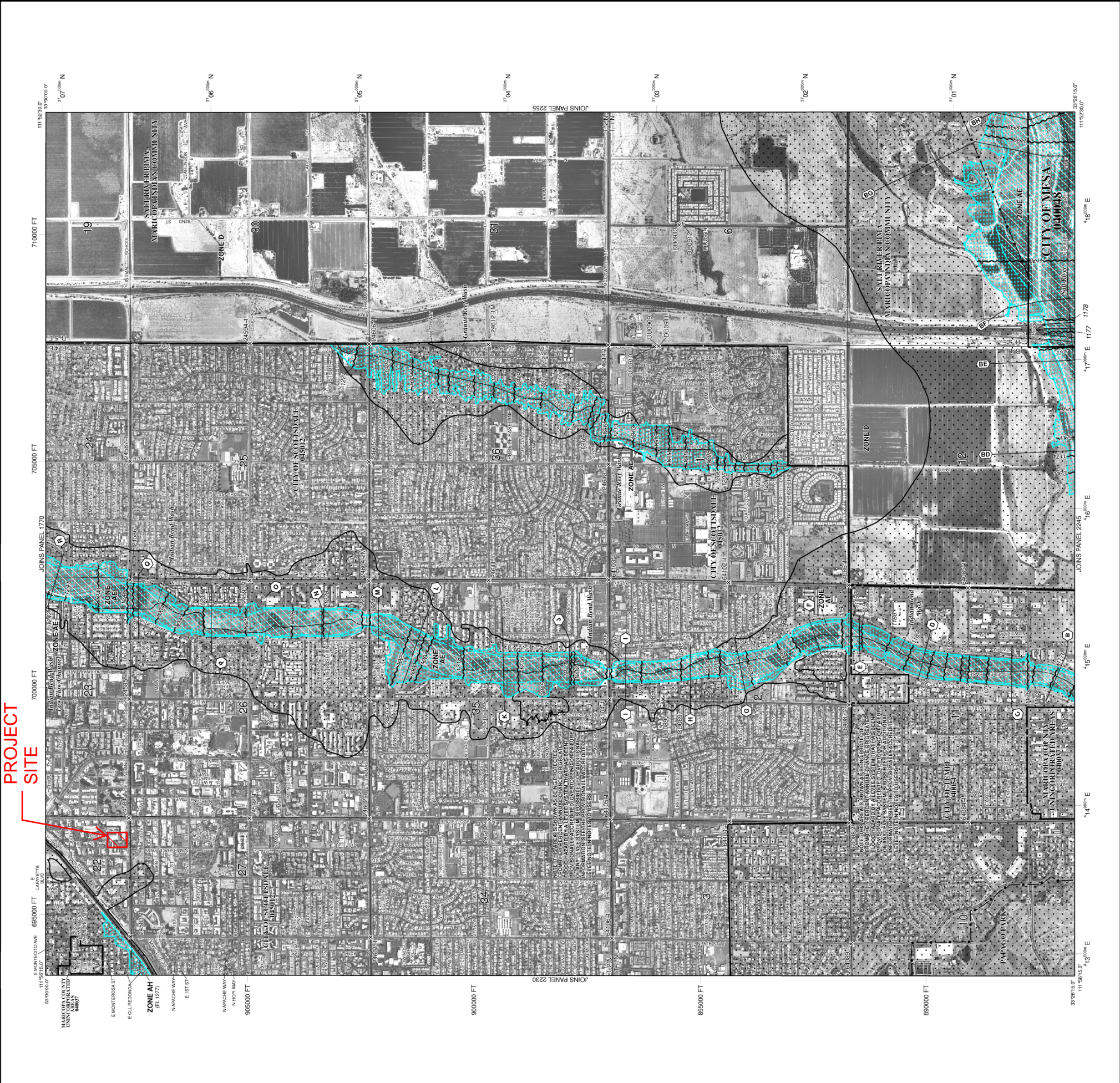
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FIGURE 2  
AERIAL MAP

**PROJECT SITE**

**NOTES TO USERS**



**LEGEND**

**SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD**  
 The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equalled or exceeded in any given year. The Special Flood Hazard Areas (SFHAs) are shown on this map in accordance with the National Flood Insurance Program (NFIP) and the Federal Emergency Management Agency (FEMA) Flood Insurance Study (FIS) and Flood Insurance Rate Study (FIRS).  
**ZONE A**  
 No Base Flood Elevations determined.  
**ZONE AH**  
 Base Flood Elevations determined.  
 Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.  
**ZONE AO**  
 Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of shallow fan flooding, velocities are also determined.  
**ZONE AR**  
 Areas shown are normally located within or adjacent to Special Flood Hazard Areas.  
**ZONE AN**  
 Areas shown are normally located within or adjacent to Special Flood Hazard Areas.  
**ZONE AV**  
 Areas shown are normally located within or adjacent to Special Flood Hazard Areas.  
**ZONE AX**  
 Areas shown are normally located within or adjacent to Special Flood Hazard Areas.  
**ZONE AY**  
 Areas shown are normally located within or adjacent to Special Flood Hazard Areas.  
**ZONE AZ**  
 Areas shown are normally located within or adjacent to Special Flood Hazard Areas.

**OTHER AREAS**  
 Areas determined to be outside the 0.2% annual chance floodplain.  
**ZONE X**  
 Areas in which flood hazards are undetermined, but possible.  
**COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS**  
 CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.  
**OTHERWISE PROTECTED AREAS (OPAs)**  
 OPAs are normally located within or adjacent to Special Flood Hazard Areas.  
**FLOODWAY AREAS IN ZONE AE**  
 This boundary is the extent of a stream plus any adjacent floodplain areas that might be most free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.  
**OTHER FLOOD AREAS**  
**ZONE X**  
 Areas of 0.2% annual chance flood; areas of 1% annual chance flood; areas of 0.2% annual chance flood; areas of 1% annual chance flood; areas of 0.2% annual chance flood; areas of 1% annual chance flood.  
**OTHER AREAS**  
 Areas determined to be outside the 0.2% annual chance floodplain.  
**ZONE X**  
 Areas in which flood hazards are undetermined, but possible.  
**COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS**  
 CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.  
**OTHERWISE PROTECTED AREAS (OPAs)**  
 OPAs are normally located within or adjacent to Special Flood Hazard Areas.  
**FLOODWAY AREAS IN ZONE AE**  
 This boundary is the extent of a stream plus any adjacent floodplain areas that might be most free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.  
**OTHER FLOOD AREAS**  
**ZONE X**  
 Areas of 0.2% annual chance flood; areas of 1% annual chance flood; areas of 0.2% annual chance flood; areas of 1% annual chance flood; areas of 0.2% annual chance flood; areas of 1% annual chance flood.

**BASE MAP INFORMATION**  
 The base map information shown on this map was derived from multiple sources. Aerial imagery was provided in digital format by the Maricopa County Department of Information Systems and is the most current available. The base map information was provided by the Arizona State Land Department (ALRS) and is dated 2007. The coordinate system used for the production of the digital FIRM is State Plane Arizona Central NAD83 HARN, International Feet.  
**PROFILE BASELINE**  
 The profile baseline depicted on this map represents the hydraulic modeling from the channel centerline or appear outside the SFHA. In some cases, they deviate significantly from the channel centerline or appear outside the SFHA.  
**CORPORATE LIMITS**  
 Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.  
**MAP INDEX**  
 Please refer to the separately printed Map Index for an overview map of the community and identify the location of this panel.  
**MAP REPOSITORIES**  
 Refer to Map Repositories list on Map Index.  
**EFFECTIVE DATE OF CONFORMANCE**  
 FLOOD INSURANCE RATE MAP  
 APRIL 15, 1988  
 EFFECTIVE DATE OF THIS PANEL  
 JULY 19, 2010  
 OCTOBER 20, 2010  
 OCTOBER 20, 2013  
 The date of the study for which the flood insurance study report was prepared is indicated by a star symbol. Flood insurance study reports are subject to periodic re-evaluation and update. Preliminary revised letters of map revision, to add base flood elevations, to correct errors, or to update the study, will be posted on the FEMA website.  
**COMMUNITY**  
 The community map number history refers to the community map number for the jurisdiction. Map history is available in this community map. Contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6262.  
**PROVISIONALLY ACCREDITED LEVEE NOTES TO USERS:**  
 Check with your local community to obtain more information, such as the estimated level of protection provided (which is the level of protection for the levee system) shown as providing protection for areas on this panel. To maintain accreditation, the levee owner or community is required to submit the data and documentation necessary to comply with Section 65.10 of the NFIP. Necessary data and documentation include: a) a site and documentation provided to indicate the levee system does not comply with Section 65.10 requirements; FEMA will revise the flood hazard and risk information for this area to reflect de-accreditation of the levee system. To mitigate flood risk in residual risk areas, the community should consider other protective measures. For more information on flood insurance, interested parties should visit the FEMA Website at <http://www.fema.gov/business/nfipindex.shtml>.

**MAP SCALE 1" = 1000'**

0 100 200 300 400 500 600  
 0 100 200 300 400 500 600  
 FEET METERS

**MAP NUMBER**  
 04013CZ235L  
**MAP REVISED**  
 OCTOBER 16, 2013  
**FEDERAL EMERGENCY MANAGEMENT AGENCY**

**NOTES TO USERS**  
 This map is for use in determining flood insurance rates under the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.  
 To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodway boundaries have been determined, users are encouraged to consult the Flood Insurance Study (FIS) and Flood Insurance Rate Study (FIRS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information for purposes of construction and/or floodplain management. For more information on flood insurance, interested parties should visit the FEMA Website at <http://www.fema.gov/business/nfipindex.shtml>.  
**Coastal Base Flood Elevations**  
 Coastal Base Flood Elevations shown on this map apply only to landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations shown in the Summary of Stillwater Elevations tab. These stillwater elevations should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.  
**Boundaries of the Floodway Areas**  
 Boundaries of the Floodway Areas were computed at cross sections and interpolated between cross sections. The floodway areas were computed based on hydraulic considerations and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.  
**Certain areas not in Special Flood Hazard Areas may be protected by flood control structures.**  
 Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.  
**The projection used in the preparation of this map was Arizona, State Plane North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that differences in map features across jurisdiction boundaries may result in slight differences that do not affect the accuracy of this FIRM.**  
**Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that differences in map features across jurisdiction boundaries may result in slight differences that do not affect the accuracy of this FIRM.**  
**Vertical datum conversion values**  
 The web tool allows users to obtain point-specific datum conversion values by clicking on a point in the Flood Insurance Study report. The tool also allows users to obtain existing flood elevations from NGVD 29 to NAVD 88.  
**Obtain current elevation, description, and/or location information for National Geospatial Survey**  
 To obtain current elevation, description, and/or location information for National Geospatial Survey, please contact the National Geospatial Survey at (301) 713-3242, or visit its website at <http://www.ngs.noaa.gov>. To obtain information about Geographic Names Information System (GNIS) data, please visit the National Geospatial Survey website at <http://www.fgdl.maricopa.gov/mgis/apps/geoapp/geoapp.cfm>.  
**Base map information**  
 The base map information shown on this FIRM was derived from multiple sources. Aerial imagery was provided in digital format by the Maricopa County Department of Information Systems and is the most current available. The base map information was provided by the Arizona State Land Department (ALRS) and is dated 2007. The coordinate system used for the production of the digital FIRM is State Plane Arizona Central NAD83 HARN, International Feet.  
**Profile baseline**  
 The profile baseline depicted on this map represents the hydraulic modeling from the channel centerline or appear outside the SFHA. In some cases, they deviate significantly from the channel centerline or appear outside the SFHA.  
**Corporate limits**  
 Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.  
**Map index**  
 Please refer to the separately printed Map Index for an overview map of the community and identify the location of this panel.  
**Map repositories**  
 Refer to Map Repositories list on Map Index.  
**Effective date of conformance**  
 FLOOD INSURANCE RATE MAP  
 APRIL 15, 1988  
**Effective date of this panel**  
 JULY 19, 2010  
 OCTOBER 20, 2010  
 OCTOBER 20, 2013  
 The date of the study for which the flood insurance study report was prepared is indicated by a star symbol. Flood insurance study reports are subject to periodic re-evaluation and update. Preliminary revised letters of map revision, to add base flood elevations, to correct errors, or to update the study, will be posted on the FEMA website.  
**Community**  
 The community map number history refers to the community map number for the jurisdiction. Map history is available in this community map. Contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6262.  
**Provisionally Accredited Levee Notes to Users:**  
 Check with your local community to obtain more information, such as the estimated level of protection provided (which is the level of protection for the levee system) shown as providing protection for areas on this panel. To maintain accreditation, the levee owner or community is required to submit the data and documentation necessary to comply with Section 65.10 of the NFIP. Necessary data and documentation include: a) a site and documentation provided to indicate the levee system does not comply with Section 65.10 requirements; FEMA will revise the flood hazard and risk information for this area to reflect de-accreditation of the levee system. To mitigate flood risk in residual risk areas, the community should consider other protective measures. For more information on flood insurance, interested parties should visit the FEMA Website at <http://www.fema.gov/business/nfipindex.shtml>.

*APPENDIX I*

*Rainfall Data*



**NOAA Atlas 14, Volume 1, Version 5**  
**Location name: Scottsdale, Arizona, USA\***  
**Latitude: 33.4955°, Longitude: -111.9279°**  
**Elevation: 1262.91 ft\*\***



\* source: ESRI Maps  
 \*\* source: USGS

**POINT PRECIPITATION FREQUENCY ESTIMATES**

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF\\_tabular](#) | [PF\\_graphical](#) | [Maps\\_&\\_aerials](#)

**PF tabular**

<b>PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)<sup>1</sup></b>										
<b>Duration</b>	<b>Average recurrence interval (years)</b>									
	<b>1</b>	<b>2</b>	<b>5</b>	<b>10</b>	<b>25</b>	<b>50</b>	<b>100</b>	<b>200</b>	<b>500</b>	<b>1000</b>
<b>5-min</b>	<b>0.183</b> (0.154-0.223)	<b>0.240</b> (0.202-0.292)	<b>0.326</b> (0.273-0.395)	<b>0.392</b> (0.326-0.473)	<b>0.481</b> (0.393-0.578)	<b>0.550</b> (0.444-0.658)	<b>0.620</b> (0.492-0.740)	<b>0.693</b> (0.539-0.825)	<b>0.789</b> (0.598-0.941)	<b>0.862</b> (0.641-1.03)
<b>10-min</b>	<b>0.279</b> (0.234-0.340)	<b>0.365</b> (0.307-0.444)	<b>0.496</b> (0.415-0.602)	<b>0.596</b> (0.496-0.720)	<b>0.733</b> (0.599-0.880)	<b>0.838</b> (0.676-1.00)	<b>0.944</b> (0.748-1.13)	<b>1.05</b> (0.821-1.26)	<b>1.20</b> (0.910-1.43)	<b>1.31</b> (0.976-1.57)
<b>15-min</b>	<b>0.345</b> (0.290-0.421)	<b>0.452</b> (0.381-0.550)	<b>0.614</b> (0.514-0.746)	<b>0.739</b> (0.614-0.893)	<b>0.908</b> (0.742-1.09)	<b>1.04</b> (0.838-1.24)	<b>1.17</b> (0.927-1.40)	<b>1.31</b> (1.02-1.56)	<b>1.49</b> (1.13-1.78)	<b>1.63</b> (1.21-1.95)
<b>30-min</b>	<b>0.465</b> (0.390-0.567)	<b>0.609</b> (0.513-0.741)	<b>0.827</b> (0.692-1.00)	<b>0.995</b> (0.827-1.20)	<b>1.22</b> (0.999-1.47)	<b>1.40</b> (1.13-1.67)	<b>1.58</b> (1.25-1.88)	<b>1.76</b> (1.37-2.10)	<b>2.00</b> (1.52-2.39)	<b>2.19</b> (1.63-2.62)
<b>60-min</b>	<b>0.576</b> (0.483-0.702)	<b>0.754</b> (0.635-0.917)	<b>1.02</b> (0.857-1.24)	<b>1.23</b> (1.02-1.49)	<b>1.51</b> (1.24-1.82)	<b>1.73</b> (1.40-2.07)	<b>1.95</b> (1.55-2.33)	<b>2.18</b> (1.70-2.60)	<b>2.48</b> (1.88-2.96)	<b>2.71</b> (2.02-3.24)
<b>2-hr</b>	<b>0.667</b> (0.569-0.796)	<b>0.864</b> (0.736-1.03)	<b>1.16</b> (0.983-1.38)	<b>1.38</b> (1.16-1.64)	<b>1.69</b> (1.40-1.99)	<b>1.92</b> (1.57-2.26)	<b>2.16</b> (1.74-2.54)	<b>2.41</b> (1.91-2.83)	<b>2.74</b> (2.12-3.22)	<b>2.99</b> (2.26-3.54)
<b>3-hr</b>	<b>0.726</b> (0.615-0.873)	<b>0.931</b> (0.793-1.13)	<b>1.22</b> (1.04-1.47)	<b>1.46</b> (1.22-1.74)	<b>1.78</b> (1.47-2.12)	<b>2.04</b> (1.66-2.42)	<b>2.31</b> (1.85-2.74)	<b>2.59</b> (2.04-3.07)	<b>2.98</b> (2.28-3.53)	<b>3.30</b> (2.46-3.91)
<b>6-hr</b>	<b>0.874</b> (0.757-1.03)	<b>1.11</b> (0.963-1.30)	<b>1.42</b> (1.23-1.67)	<b>1.67</b> (1.43-1.95)	<b>2.01</b> (1.70-2.33)	<b>2.28</b> (1.90-2.63)	<b>2.56</b> (2.10-2.95)	<b>2.84</b> (2.28-3.29)	<b>3.23</b> (2.53-3.75)	<b>3.54</b> (2.71-4.11)
<b>12-hr</b>	<b>0.977</b> (0.855-1.14)	<b>1.24</b> (1.08-1.44)	<b>1.57</b> (1.36-1.81)	<b>1.83</b> (1.58-2.11)	<b>2.17</b> (1.86-2.50)	<b>2.44</b> (2.07-2.81)	<b>2.72</b> (2.27-3.13)	<b>3.00</b> (2.47-3.45)	<b>3.38</b> (2.71-3.91)	<b>3.67</b> (2.89-4.27)
<b>24-hr</b>	<b>1.17</b> (1.04-1.32)	<b>1.48</b> (1.32-1.67)	<b>1.92</b> (1.71-2.17)	<b>2.27</b> (2.02-2.55)	<b>2.75</b> (2.42-3.09)	<b>3.13</b> (2.74-3.51)	<b>3.53</b> (3.07-3.96)	<b>3.94</b> (3.40-4.42)	<b>4.51</b> (3.85-5.06)	<b>4.96</b> (4.19-5.58)
<b>2-day</b>	<b>1.26</b> (1.13-1.42)	<b>1.61</b> (1.44-1.82)	<b>2.12</b> (1.89-2.38)	<b>2.52</b> (2.24-2.83)	<b>3.08</b> (2.72-3.46)	<b>3.53</b> (3.10-3.96)	<b>4.00</b> (3.49-4.50)	<b>4.50</b> (3.89-5.06)	<b>5.19</b> (4.44-5.85)	<b>5.74</b> (4.86-6.49)
<b>3-day</b>	<b>1.33</b> (1.19-1.50)	<b>1.71</b> (1.52-1.92)	<b>2.25</b> (2.00-2.53)	<b>2.68</b> (2.37-3.01)	<b>3.29</b> (2.90-3.69)	<b>3.78</b> (3.31-4.24)	<b>4.30</b> (3.74-4.83)	<b>4.85</b> (4.18-5.45)	<b>5.62</b> (4.79-6.32)	<b>6.24</b> (5.26-7.04)
<b>4-day</b>	<b>1.41</b> (1.25-1.59)	<b>1.80</b> (1.60-2.03)	<b>2.37</b> (2.11-2.67)	<b>2.84</b> (2.51-3.19)	<b>3.50</b> (3.08-3.92)	<b>4.03</b> (3.52-4.51)	<b>4.60</b> (3.99-5.15)	<b>5.20</b> (4.47-5.84)	<b>6.05</b> (5.14-6.79)	<b>6.74</b> (5.67-7.58)
<b>7-day</b>	<b>1.56</b> (1.39-1.76)	<b>1.99</b> (1.78-2.25)	<b>2.63</b> (2.34-2.97)	<b>3.15</b> (2.79-3.55)	<b>3.88</b> (3.42-4.36)	<b>4.47</b> (3.91-5.02)	<b>5.10</b> (4.43-5.73)	<b>5.76</b> (4.96-6.48)	<b>6.70</b> (5.70-7.54)	<b>7.46</b> (6.28-8.41)
<b>10-day</b>	<b>1.70</b> (1.51-1.91)	<b>2.17</b> (1.93-2.44)	<b>2.86</b> (2.54-3.22)	<b>3.42</b> (3.03-3.84)	<b>4.21</b> (3.70-4.71)	<b>4.83</b> (4.23-5.40)	<b>5.50</b> (4.78-6.15)	<b>6.20</b> (5.35-6.94)	<b>7.18</b> (6.12-8.05)	<b>7.97</b> (6.72-8.94)
<b>20-day</b>	<b>2.08</b> (1.86-2.33)	<b>2.68</b> (2.39-3.00)	<b>3.54</b> (3.16-3.95)	<b>4.19</b> (3.73-4.67)	<b>5.07</b> (4.48-5.65)	<b>5.74</b> (5.06-6.40)	<b>6.42</b> (5.64-7.17)	<b>7.12</b> (6.22-7.95)	<b>8.05</b> (6.97-9.02)	<b>8.77</b> (7.53-9.84)
<b>30-day</b>	<b>2.43</b> (2.17-2.73)	<b>3.13</b> (2.79-3.50)	<b>4.13</b> (3.67-4.61)	<b>4.88</b> (4.34-5.44)	<b>5.90</b> (5.21-6.57)	<b>6.68</b> (5.88-7.43)	<b>7.48</b> (6.55-8.32)	<b>8.29</b> (7.23-9.23)	<b>9.39</b> (8.12-10.5)	<b>10.2</b> (8.78-11.4)
<b>45-day</b>	<b>2.82</b> (2.52-3.15)	<b>3.63</b> (3.25-4.06)	<b>4.78</b> (4.28-5.34)	<b>5.64</b> (5.03-6.29)	<b>6.76</b> (6.01-7.54)	<b>7.61</b> (6.74-8.49)	<b>8.46</b> (7.47-9.45)	<b>9.32</b> (8.18-10.4)	<b>10.4</b> (9.11-11.7)	<b>11.3</b> (9.79-12.7)
<b>60-day</b>	<b>3.12</b> (2.80-3.48)	<b>4.03</b> (3.62-4.49)	<b>5.30</b> (4.75-5.90)	<b>6.22</b> (5.56-6.92)	<b>7.42</b> (6.62-8.26)	<b>8.31</b> (7.39-9.25)	<b>9.20</b> (8.15-10.2)	<b>10.1</b> (8.89-11.2)	<b>11.2</b> (9.84-12.5)	<b>12.1</b> (10.5-13.5)

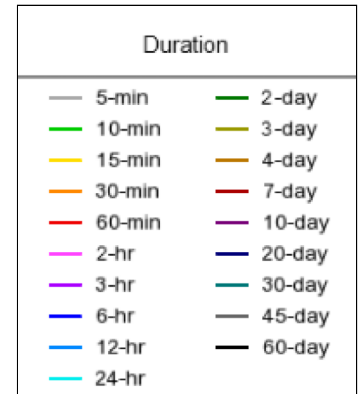
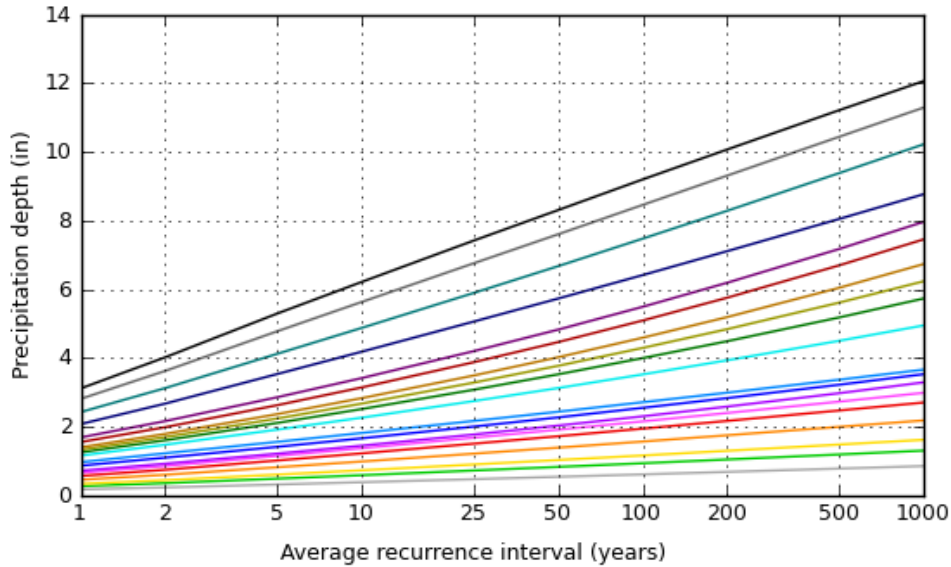
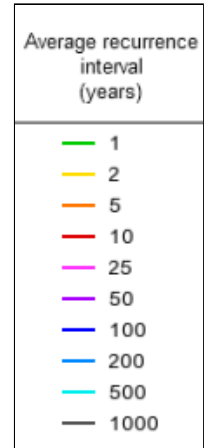
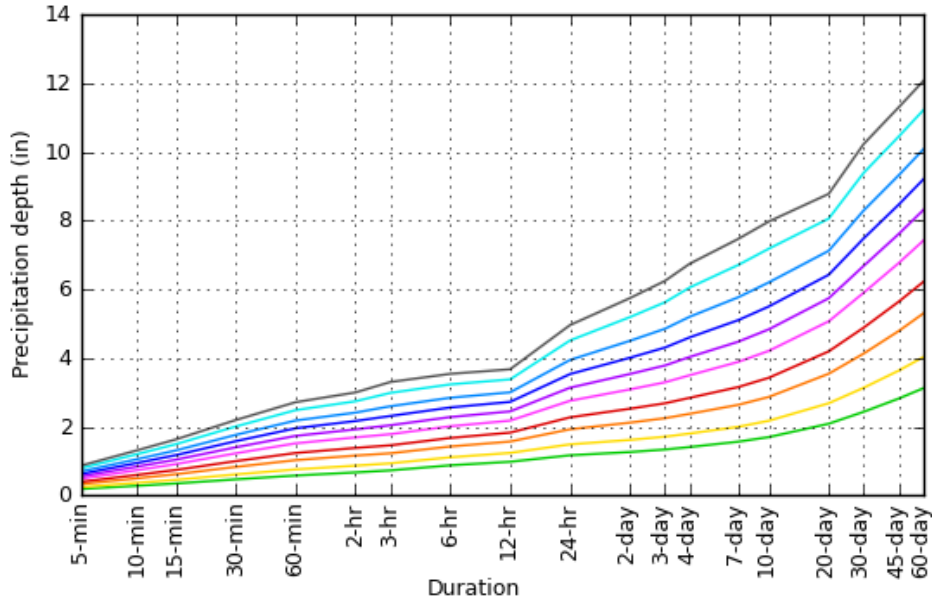
<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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**PF graphical**



PDS-based depth-duration-frequency (DDF) curves  
 Latitude: 33.4955°, Longitude: -111.9279°



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**Maps & aerials**

**Small scale terrain**



**NOAA Atlas 14, Volume 1, Version 5**  
**Location name: Scottsdale, Arizona, USA\***  
**Latitude: 33.4955°, Longitude: -111.9279°**  
**Elevation: 1262.91 ft\*\***



\* source: ESRI Maps  
 \*\* source: USGS

**POINT PRECIPITATION FREQUENCY ESTIMATES**

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF\\_tabular](#) | [PF\\_graphical](#) | [Maps\\_&\\_aerials](#)

**PF tabular**

<b>PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)<sup>1</sup></b>										
<b>Duration</b>	<b>Average recurrence interval (years)</b>									
	<b>1</b>	<b>2</b>	<b>5</b>	<b>10</b>	<b>25</b>	<b>50</b>	<b>100</b>	<b>200</b>	<b>500</b>	<b>1000</b>
<b>5-min</b>	<b>2.20</b> (1.85-2.68)	<b>2.88</b> (2.42-3.50)	<b>3.91</b> (3.28-4.74)	<b>4.70</b> (3.91-5.68)	<b>5.77</b> (4.72-6.94)	<b>6.60</b> (5.33-7.90)	<b>7.44</b> (5.90-8.88)	<b>8.32</b> (6.47-9.90)	<b>9.47</b> (7.18-11.3)	<b>10.3</b> (7.69-12.4)
<b>10-min</b>	<b>1.67</b> (1.40-2.04)	<b>2.19</b> (1.84-2.66)	<b>2.98</b> (2.49-3.61)	<b>3.58</b> (2.98-4.32)	<b>4.40</b> (3.59-5.28)	<b>5.03</b> (4.06-6.01)	<b>5.66</b> (4.49-6.76)	<b>6.32</b> (4.93-7.54)	<b>7.20</b> (5.46-8.59)	<b>7.87</b> (5.86-9.41)
<b>15-min</b>	<b>1.38</b> (1.16-1.68)	<b>1.81</b> (1.52-2.20)	<b>2.46</b> (2.06-2.98)	<b>2.96</b> (2.46-3.57)	<b>3.63</b> (2.97-4.36)	<b>4.15</b> (3.35-4.97)	<b>4.68</b> (3.71-5.59)	<b>5.23</b> (4.07-6.23)	<b>5.95</b> (4.52-7.10)	<b>6.51</b> (4.84-7.78)
<b>30-min</b>	<b>0.930</b> (0.780-1.13)	<b>1.22</b> (1.03-1.48)	<b>1.65</b> (1.38-2.01)	<b>1.99</b> (1.65-2.40)	<b>2.45</b> (2.00-2.94)	<b>2.80</b> (2.26-3.34)	<b>3.15</b> (2.50-3.76)	<b>3.52</b> (2.74-4.19)	<b>4.01</b> (3.04-4.78)	<b>4.38</b> (3.26-5.24)
<b>60-min</b>	<b>0.576</b> (0.483-0.702)	<b>0.754</b> (0.635-0.917)	<b>1.02</b> (0.857-1.24)	<b>1.23</b> (1.02-1.49)	<b>1.51</b> (1.24-1.82)	<b>1.73</b> (1.40-2.07)	<b>1.95</b> (1.55-2.33)	<b>2.18</b> (1.70-2.60)	<b>2.48</b> (1.88-2.96)	<b>2.71</b> (2.02-3.24)
<b>2-hr</b>	<b>0.334</b> (0.284-0.398)	<b>0.432</b> (0.368-0.517)	<b>0.578</b> (0.492-0.688)	<b>0.690</b> (0.580-0.820)	<b>0.843</b> (0.700-0.994)	<b>0.960</b> (0.786-1.13)	<b>1.08</b> (0.872-1.27)	<b>1.20</b> (0.953-1.41)	<b>1.37</b> (1.06-1.61)	<b>1.50</b> (1.13-1.77)
<b>3-hr</b>	<b>0.242</b> (0.205-0.291)	<b>0.310</b> (0.264-0.375)	<b>0.408</b> (0.345-0.490)	<b>0.485</b> (0.407-0.580)	<b>0.593</b> (0.491-0.705)	<b>0.679</b> (0.554-0.805)	<b>0.769</b> (0.616-0.911)	<b>0.863</b> (0.680-1.02)	<b>0.993</b> (0.759-1.18)	<b>1.10</b> (0.818-1.30)
<b>6-hr</b>	<b>0.146</b> (0.126-0.172)	<b>0.185</b> (0.161-0.218)	<b>0.237</b> (0.205-0.278)	<b>0.279</b> (0.239-0.325)	<b>0.336</b> (0.284-0.389)	<b>0.380</b> (0.317-0.440)	<b>0.427</b> (0.350-0.493)	<b>0.474</b> (0.381-0.549)	<b>0.539</b> (0.423-0.625)	<b>0.591</b> (0.452-0.687)
<b>12-hr</b>	<b>0.081</b> (0.071-0.094)	<b>0.103</b> (0.090-0.119)	<b>0.130</b> (0.113-0.150)	<b>0.151</b> (0.131-0.175)	<b>0.181</b> (0.154-0.208)	<b>0.203</b> (0.171-0.233)	<b>0.226</b> (0.188-0.260)	<b>0.249</b> (0.205-0.287)	<b>0.280</b> (0.225-0.324)	<b>0.305</b> (0.240-0.355)
<b>24-hr</b>	<b>0.049</b> (0.043-0.055)	<b>0.062</b> (0.055-0.070)	<b>0.080</b> (0.071-0.090)	<b>0.095</b> (0.084-0.106)	<b>0.115</b> (0.101-0.129)	<b>0.131</b> (0.114-0.146)	<b>0.147</b> (0.128-0.165)	<b>0.164</b> (0.142-0.184)	<b>0.188</b> (0.160-0.211)	<b>0.206</b> (0.174-0.232)
<b>2-day</b>	<b>0.026</b> (0.023-0.030)	<b>0.034</b> (0.030-0.038)	<b>0.044</b> (0.039-0.050)	<b>0.053</b> (0.047-0.059)	<b>0.064</b> (0.057-0.072)	<b>0.074</b> (0.065-0.083)	<b>0.083</b> (0.073-0.094)	<b>0.094</b> (0.081-0.105)	<b>0.108</b> (0.092-0.122)	<b>0.120</b> (0.101-0.135)
<b>3-day</b>	<b>0.019</b> (0.017-0.021)	<b>0.024</b> (0.021-0.027)	<b>0.031</b> (0.028-0.035)	<b>0.037</b> (0.033-0.042)	<b>0.046</b> (0.040-0.051)	<b>0.052</b> (0.046-0.059)	<b>0.060</b> (0.052-0.067)	<b>0.067</b> (0.058-0.076)	<b>0.078</b> (0.067-0.088)	<b>0.087</b> (0.073-0.098)
<b>4-day</b>	<b>0.015</b> (0.013-0.017)	<b>0.019</b> (0.017-0.021)	<b>0.025</b> (0.022-0.028)	<b>0.030</b> (0.026-0.033)	<b>0.036</b> (0.032-0.041)	<b>0.042</b> (0.037-0.047)	<b>0.048</b> (0.042-0.054)	<b>0.054</b> (0.047-0.061)	<b>0.063</b> (0.054-0.071)	<b>0.070</b> (0.059-0.079)
<b>7-day</b>	<b>0.009</b> (0.008-0.010)	<b>0.012</b> (0.011-0.013)	<b>0.016</b> (0.014-0.018)	<b>0.019</b> (0.017-0.021)	<b>0.023</b> (0.020-0.026)	<b>0.027</b> (0.023-0.030)	<b>0.030</b> (0.026-0.034)	<b>0.034</b> (0.030-0.039)	<b>0.040</b> (0.034-0.045)	<b>0.044</b> (0.037-0.050)
<b>10-day</b>	<b>0.007</b> (0.006-0.008)	<b>0.009</b> (0.008-0.010)	<b>0.012</b> (0.011-0.013)	<b>0.014</b> (0.013-0.016)	<b>0.018</b> (0.015-0.020)	<b>0.020</b> (0.018-0.023)	<b>0.023</b> (0.020-0.026)	<b>0.026</b> (0.022-0.029)	<b>0.030</b> (0.025-0.034)	<b>0.033</b> (0.028-0.037)
<b>20-day</b>	<b>0.004</b> (0.004-0.005)	<b>0.006</b> (0.005-0.006)	<b>0.007</b> (0.007-0.008)	<b>0.009</b> (0.008-0.010)	<b>0.011</b> (0.009-0.012)	<b>0.012</b> (0.011-0.013)	<b>0.013</b> (0.012-0.015)	<b>0.015</b> (0.013-0.017)	<b>0.017</b> (0.015-0.019)	<b>0.018</b> (0.016-0.020)
<b>30-day</b>	<b>0.003</b> (0.003-0.004)	<b>0.004</b> (0.004-0.005)	<b>0.006</b> (0.005-0.006)	<b>0.007</b> (0.006-0.008)	<b>0.008</b> (0.007-0.009)	<b>0.009</b> (0.008-0.010)	<b>0.010</b> (0.009-0.012)	<b>0.012</b> (0.010-0.013)	<b>0.013</b> (0.011-0.015)	<b>0.014</b> (0.012-0.016)
<b>45-day</b>	<b>0.003</b> (0.002-0.003)	<b>0.003</b> (0.003-0.004)	<b>0.004</b> (0.004-0.005)	<b>0.005</b> (0.005-0.006)	<b>0.006</b> (0.006-0.007)	<b>0.007</b> (0.006-0.008)	<b>0.008</b> (0.007-0.009)	<b>0.009</b> (0.008-0.010)	<b>0.010</b> (0.008-0.011)	<b>0.010</b> (0.009-0.012)
<b>60-day</b>	<b>0.002</b> (0.002-0.002)	<b>0.003</b> (0.003-0.003)	<b>0.004</b> (0.003-0.004)	<b>0.004</b> (0.004-0.005)	<b>0.005</b> (0.005-0.006)	<b>0.006</b> (0.005-0.006)	<b>0.006</b> (0.006-0.007)	<b>0.007</b> (0.006-0.008)	<b>0.008</b> (0.007-0.009)	<b>0.008</b> (0.007-0.009)

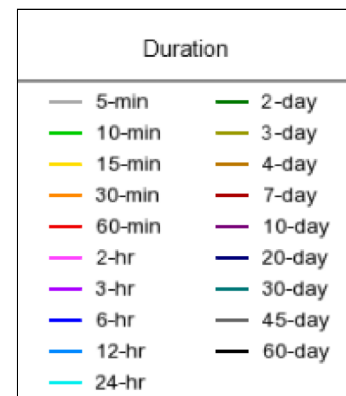
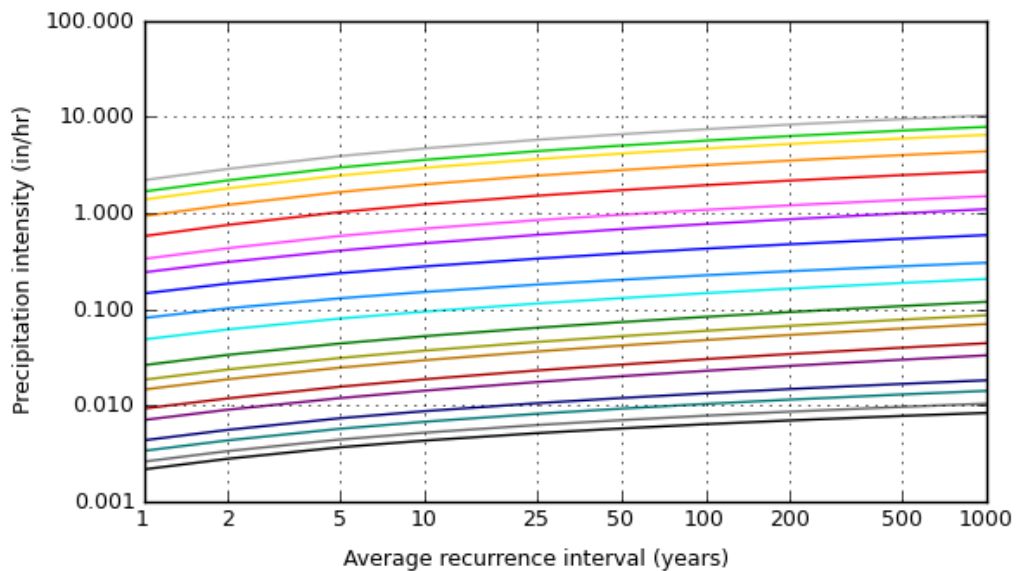
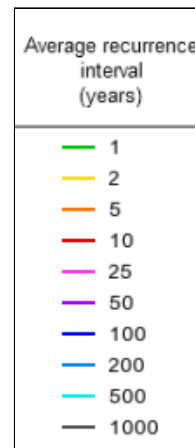
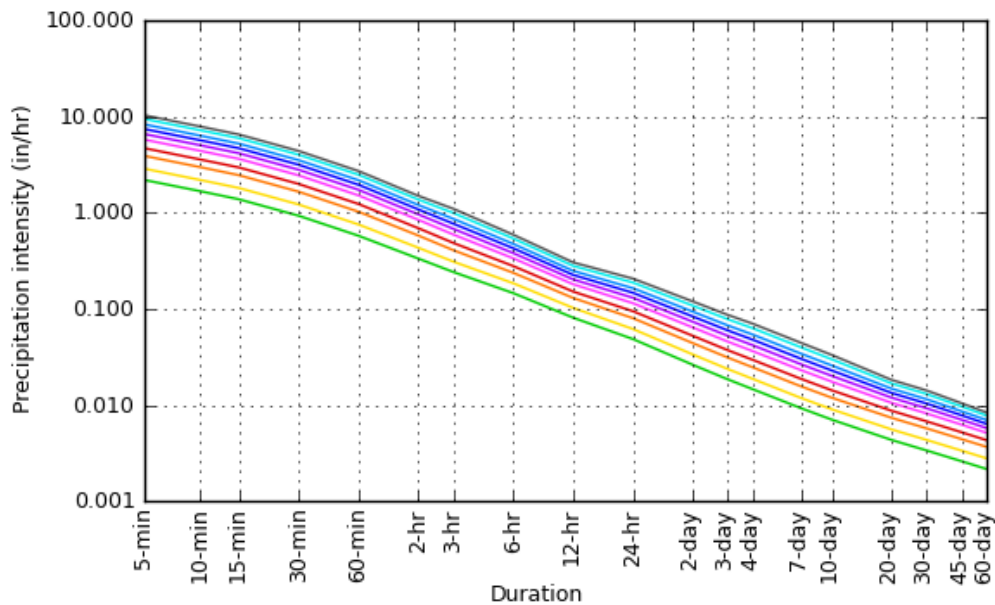
<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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**PF graphical**

PDS-based intensity-duration-frequency (IDF) curves

Latitude: 33.4955°, Longitude: -111.9279°



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**Maps & aerials**

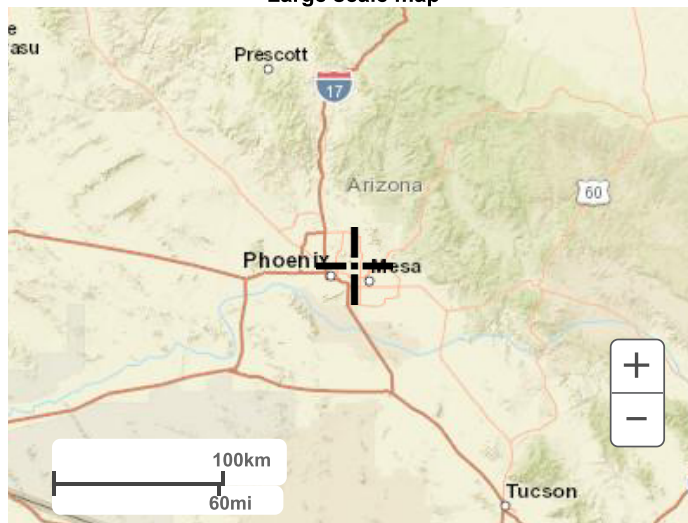
**Small scale terrain**



Large scale terrain



Large scale map



Large scale aerial



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# *APPENDIX II*

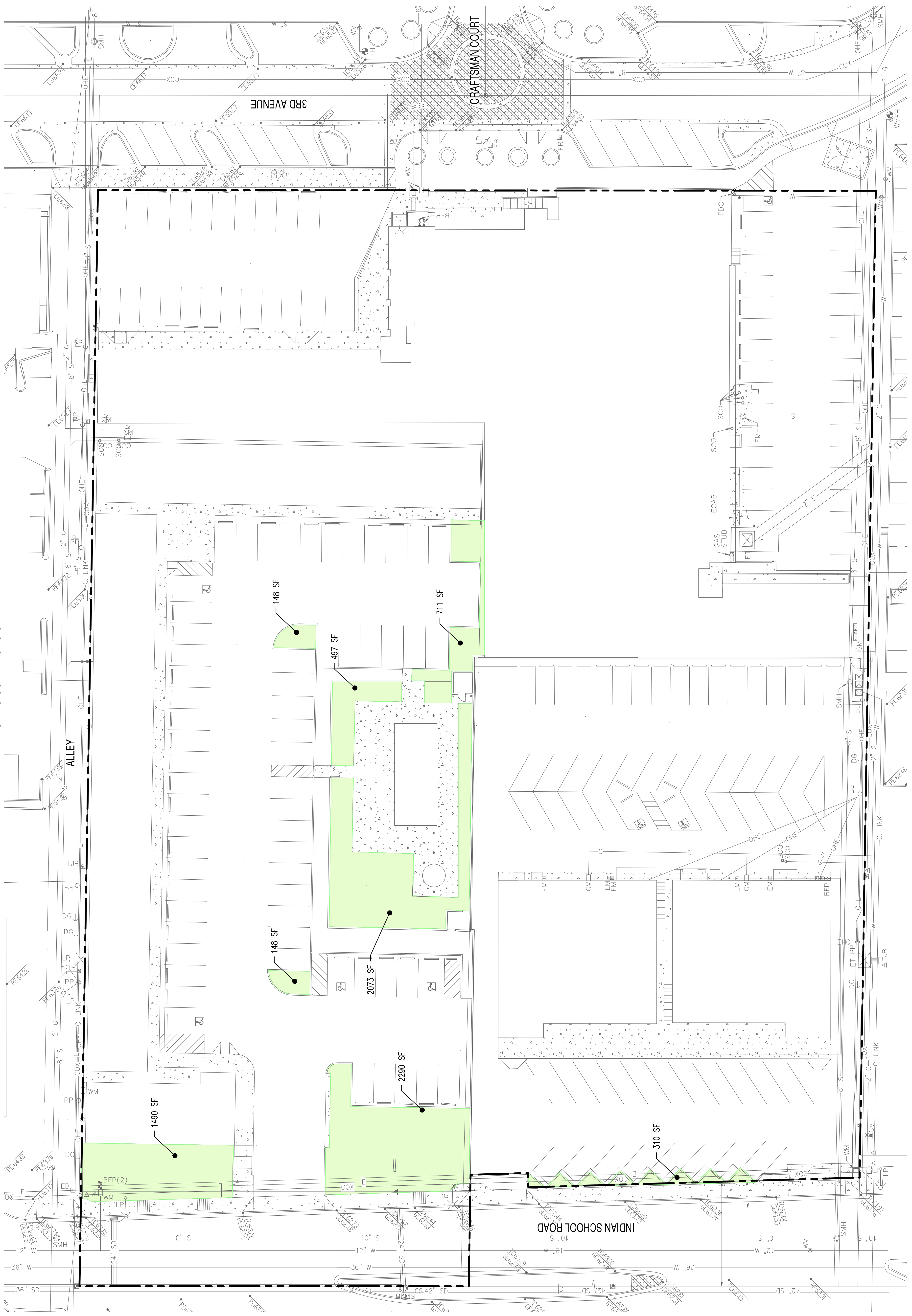
## *Calculations*

Weighted Runoff Coefficient-Calculations (C<sub>w</sub>)

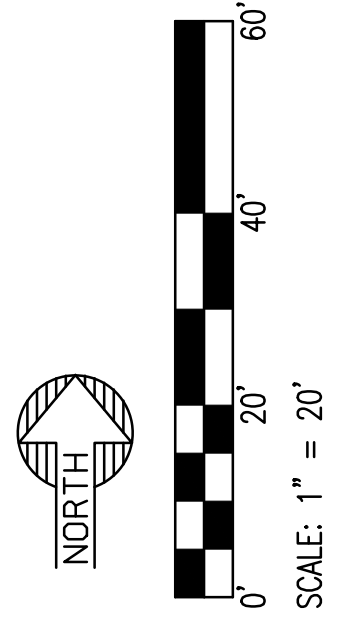
PROPOSED OVERALL SITE C <sub>w</sub>					
	BUILDING or CONCRETE	ASPHALT	DESERT LANDSCAPE	TOTAL AREA	C <sub>w</sub>
C-VALUE	0.95	0.95	0.45		
AREA (ac)	2.46	0.65	0.07	3.18	<b>0.94</b>
DA-1	0.37	0.00	0.00	0.37	0.95
DA-2	0.19	0.26	0.02	0.47	0.93
DA-3	1.07	0.05	0.01	1.13	0.95
DA-4A	0.12	0.00	0.00	0.12	0.95
DA-4B	0.20	0.16	0.00	0.36	0.95
DA-5	0.42	0.00	0.04	0.46	0.91
DA-6	0.09	0.18	0.00	0.27	0.95

EXISTING OVERALL SITE C <sub>w</sub>					
	BUILDING or CONCRETE	ASPHALT	DESERT LANDSCAPE	TOTAL AREA	C <sub>w</sub>
C-VALUE	0.95	0.95	0.45		
AREA (ac)	1.66	1.34	0.18	3.18	<b>0.92</b>
EX-1	0.20	0.00	0.00	0.20	0.95
EX-2	0.67	0.00	0.00	0.67	0.95
EX-3	0.00	0.24	0.00	0.24	0.95
EX-4	0.00	0.36	0.00	0.36	0.95
EX-5A	0.12	0.00	0.00	0.12	0.95
EX-5B	0.35	0.00	0.01	0.36	0.94
EX-6	0.13	0.74	0.17	1.04	0.87
EX-7	0.19	0.00	0.00	0.19	0.95

**THE TRIANGLE**  
3RD AVENUE AND INDIAN SCHOOL ROAD  
EXISTING CONDITIONS CWT EXHIBIT



<span style="display:inline-block; width:15px; height:15px; background-color:lightgreen; border:1px solid black;"></span> ON-SITE PERVIOUS =	0.18 AC	CWT=0.45
<span style="display:inline-block; width:15px; height:15px; background-color:white; border:1px solid black;"></span> ON-SITE IMPERVIOUS =	3.00 AC	CWT=0.95
<b>TOTAL CWT</b>	<b>=</b>	<b>3.18 AC @ CWT=0.92</b>



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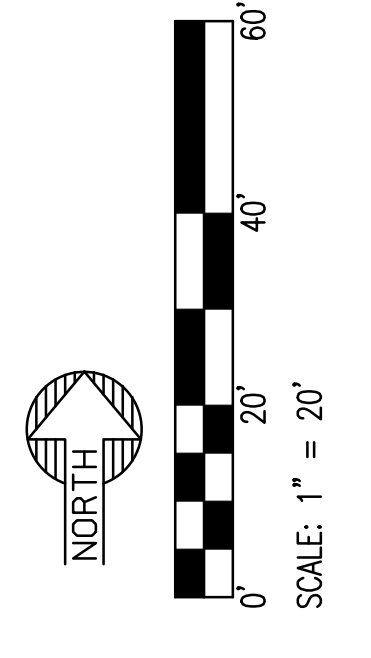
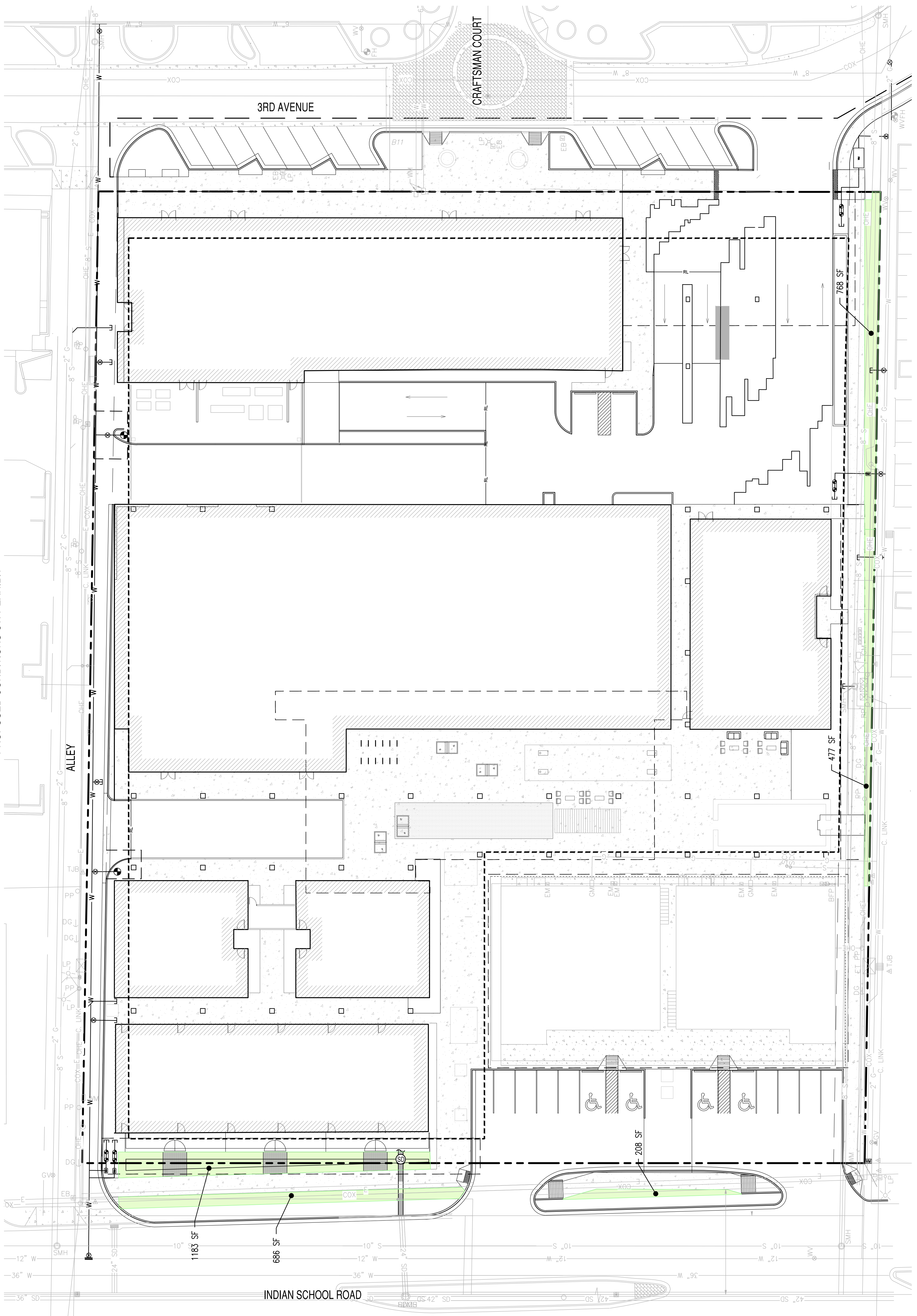


PROJECT: THE TRIANGLE  
LOCATION: 7120 E. INDIAN SCHOOL ROAD, SCOTTSDALE, AZ 85251  
DRAWN: KA 10/16/2020  
DESIGNED: KA 10/16/2020  
CHECKED: SC 10/16/2020  
IN CHARGE: AF 10/16/2020  
DATE: 10/16/2020  
ISSUED FOR:

PERSON NO.:  
DATE:  
JOB NO.: 200504  
SHEET TITLE:

EXISTING CONDITIONS  
DRAINAGE AREA MAP  
SHEET NO.: EXHIBIT A

**THE TRIANGLE**  
3RD AVENUE AND INDIAN SCHOOL ROAD  
PROPOSED CONDITIONS CWT EXHIBIT



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PROJECT: THE TRIANGLE

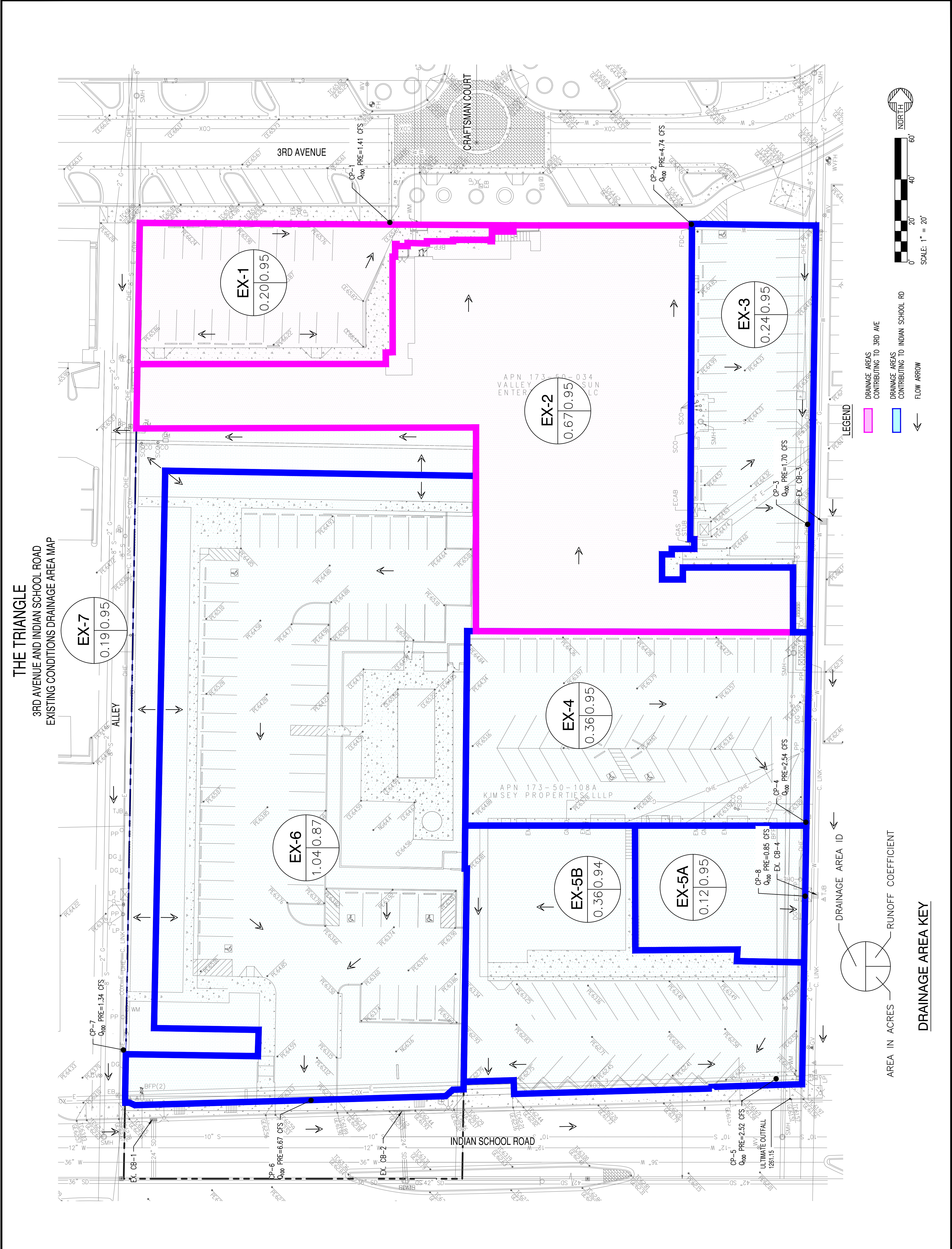
LOCATION: 7120 E. INDIAN SCHOOL ROAD, SCOTTSDALE, AZ 85251

DATE:	10/16/2020
ISSUED FOR:	
PERSON NO.:	
DATE:	

DESIGNED:	KA	10/16/2020
CHECKED:	KA	10/16/2020
IN CHARGE:	SC	10/16/2020
PROJECT MANAGER:	AF	10/16/2020

JOB NO.:	200504
SHEET TITLE:	PROPOSED CONDITIONS CWT MAP
SHEET NO.:	EXHIBIT B





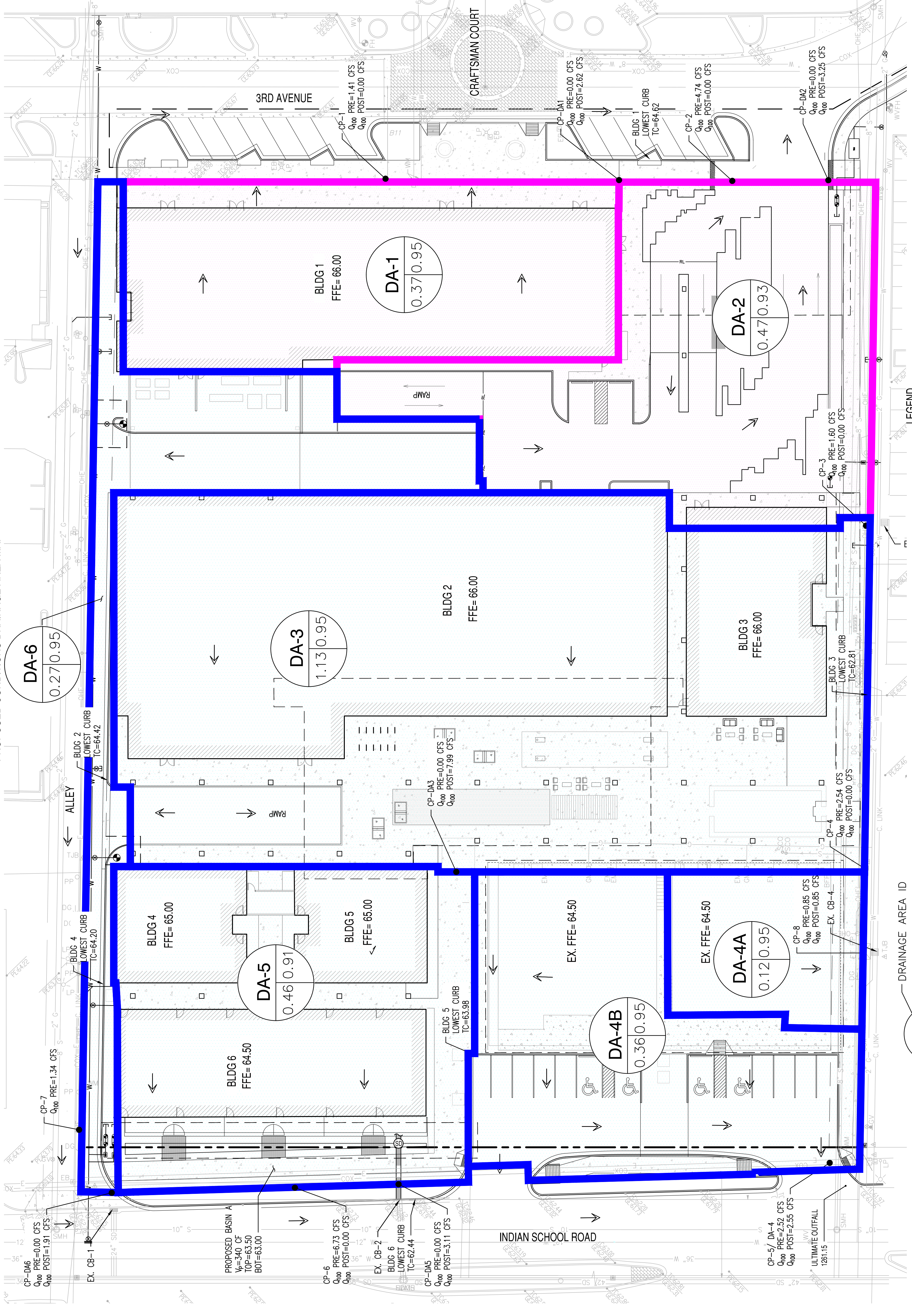
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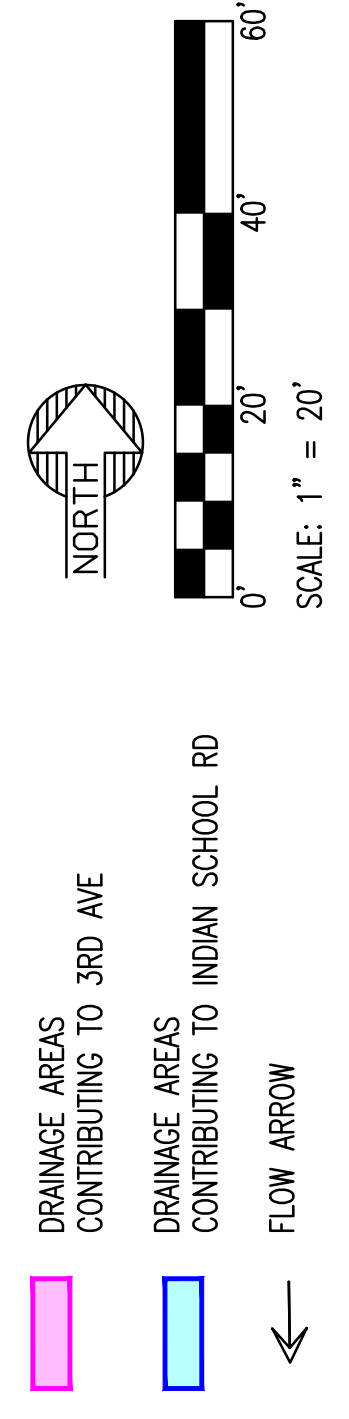


PROJECT: THE TRIANGLE  
LOCATION: 7120 E INDIAN SCHOOL ROAD, SCOTTSDALE, AZ 85251  
DRAWN: KA 10/16/2020  
DESIGNED: KA 10/16/2020  
CHECKED: SC  
CALCULATED: AF  
PROJECT MANAGER: AF  
ISSUED FOR: DATE: 10/16/2020  
PERSON NO.: DATE:  
JOB NO.: 200504  
SHEET TITLE:  
PROPOSED CONDITION DRAINAGE AREA MAP  
SHEET NO.: EXHIBIT D

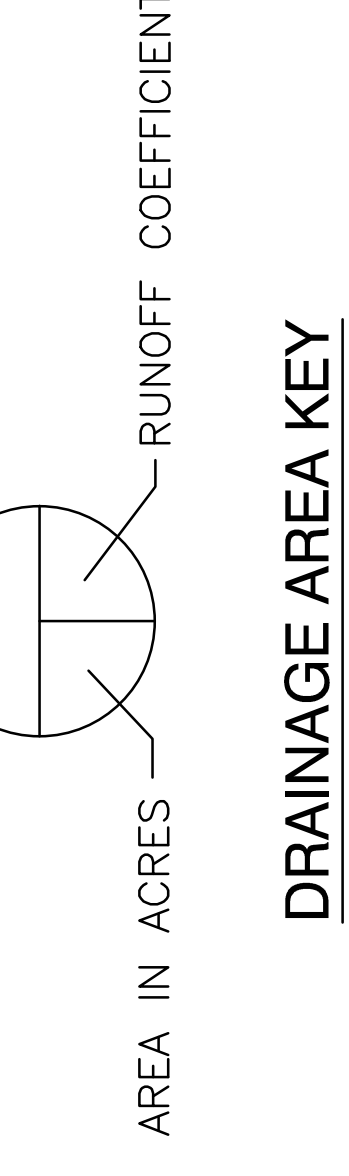
# HOJO DEVELOPMENT 3RD AVENUE AND INDIAN SCHOOL ROAD PROPOSED CONDITIONS DRAINAGE AREA MAP



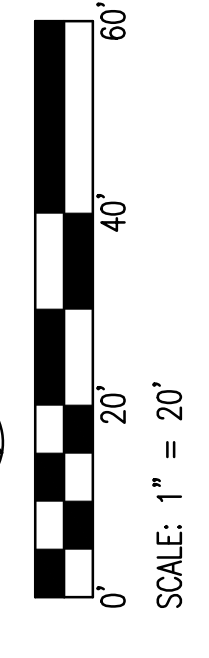
### LEGEND



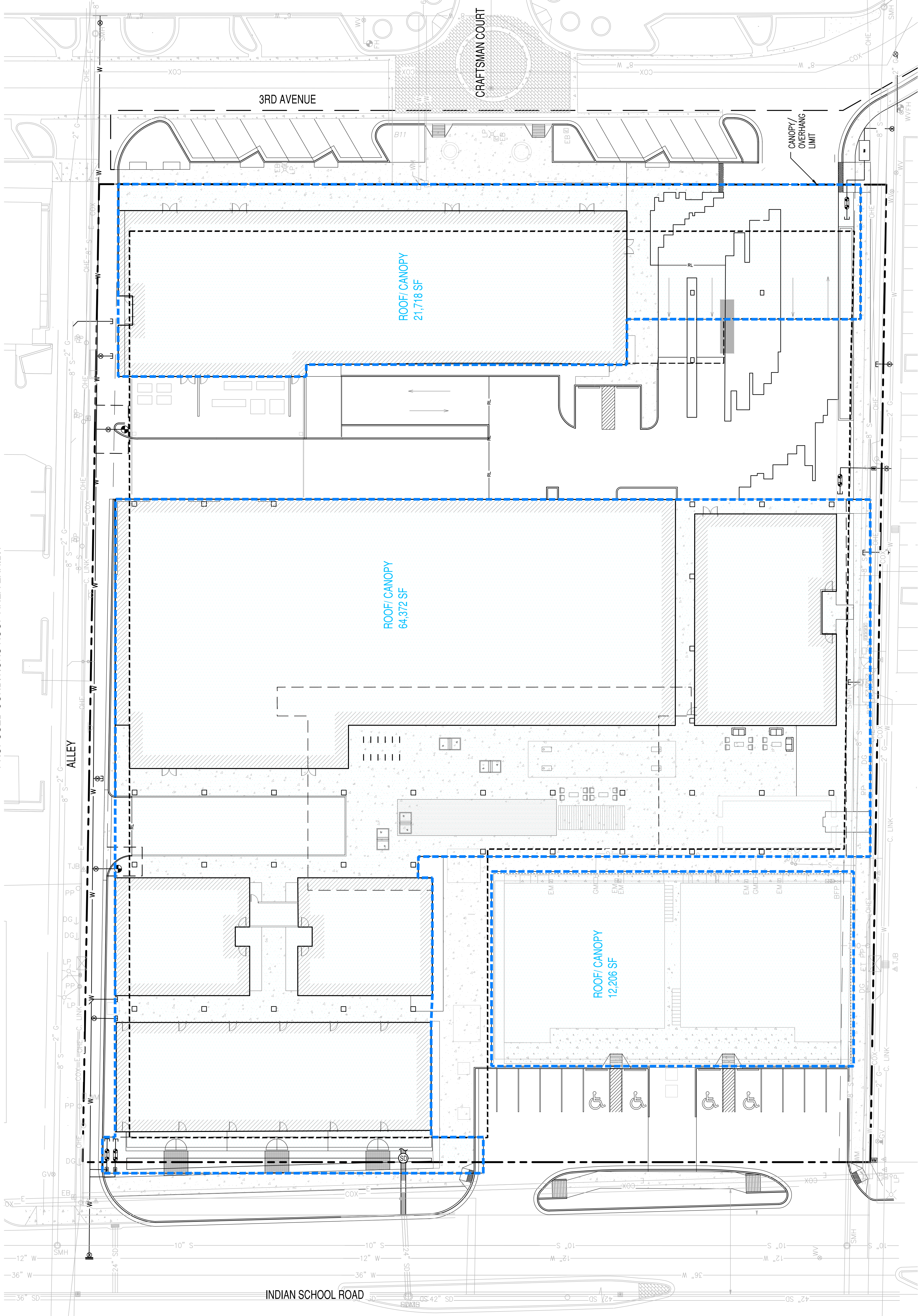
### DRAINAGE AREA KEY





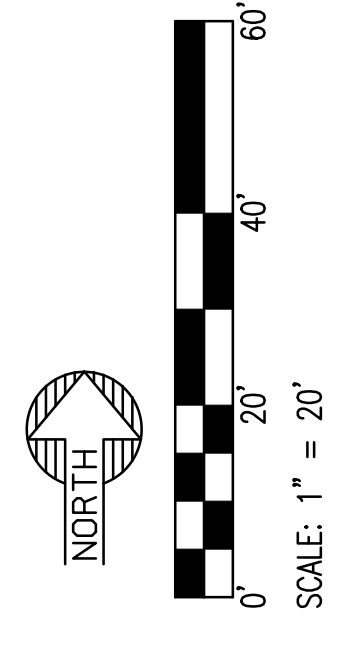
AREA IN ACRES



**THE TRIANGLE**  
3RD AVENUE AND INDIAN SCHOOL ROAD  
PROPOSED CONDITIONS ROOF AREA EXHIBIT



	ON-SITE ROOF/ CANOPY AREA = 98,296 (2.26 AC)
	ON-SITE FIRST FLUSH AREA = 39,921 (0.92 AC)
	TOTAL DISTURBED AREA = 138,117 (3.18 AC)



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PROJECT: THE TRIANGLE

LOCATION: 7120 E. INDIAN SCHOOL ROAD, SCOTTSDALE, AZ 85251

DATE:	10/16/2020
ISSUED FOR:	

DESIGNED:	KA	10/16/2020
CHECKED:	KA	10/16/2020
IN CHARGE:	SC	10/16/2020
PROJECT MANAGER:	AF	10/16/2020

REVISION NO.:	DATE:

JOB NO.: 200504

SHEET TITLE: PROPOSED CONDITIONS ROOF AREA EXHIBIT

SHEET NO.: EXHIBIT E

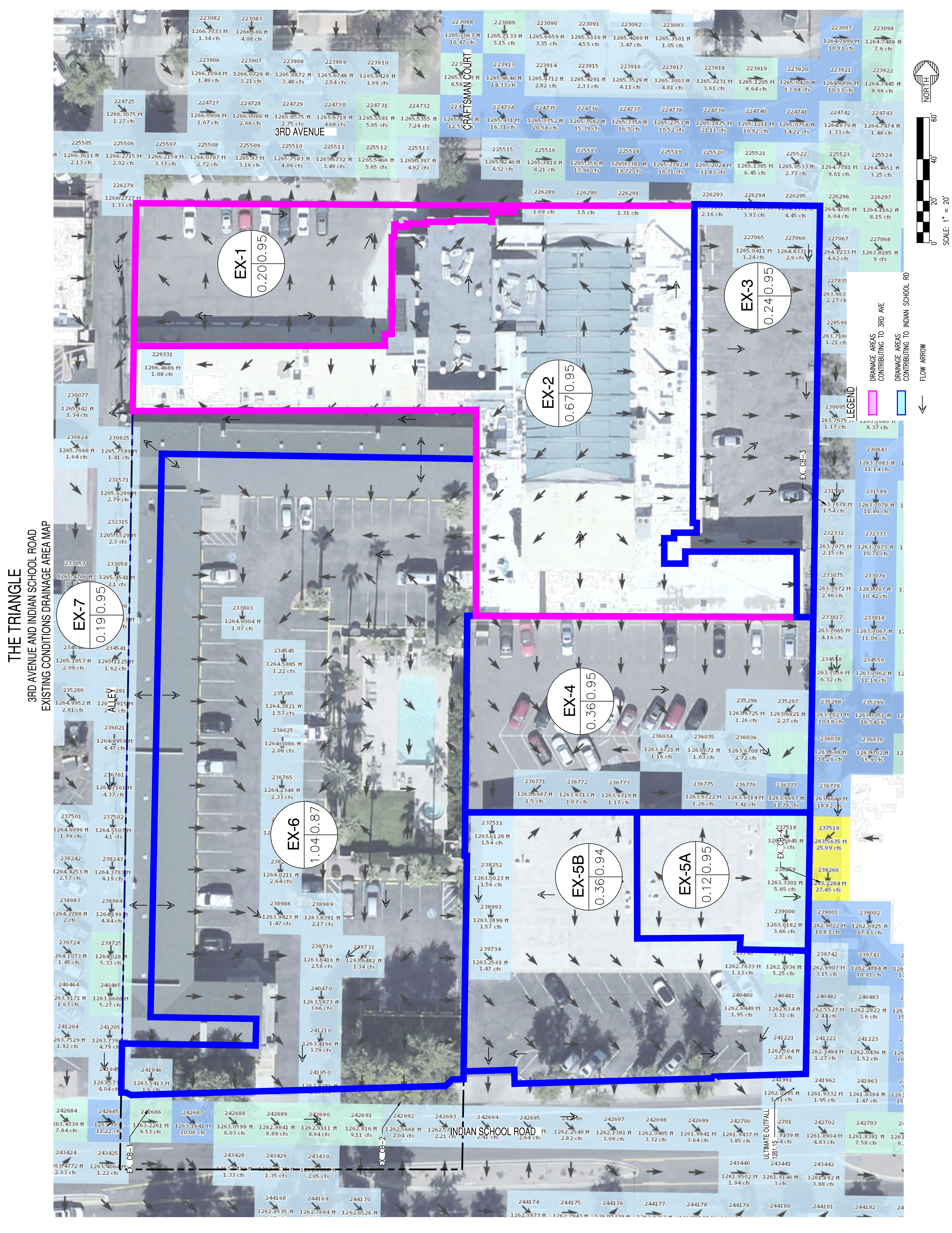
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CONSTRUCTION

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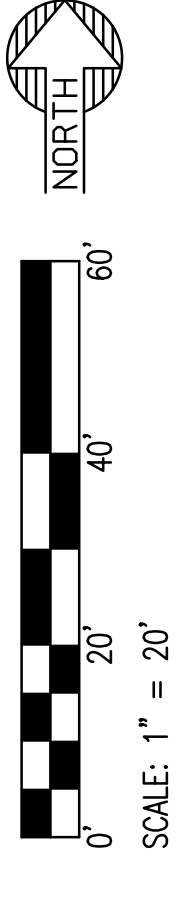
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PROJECT: THE TRIANGLE	DATE: 10/16/2020	ISSUED FOR:	REVISION NO.:
DESIGNED: KA	10/16/2020	LOCATION: 7120 E. INDIAN SCHOOL ROAD, SCOTTSDALE, AZ 85251	DATE:
CHECKED: KA	10/16/2020		
IN CHARGE: SC	10/16/2020		
PROJ. MGR.: AF	10/16/2020		
DATE: 10/16/2020			
JOB NO.: 200504			
SHEET TITLE: FLO-2D MAP			
SHEET NO.: 200504			



THE TRIANGLE  
3RD AVENUE AND INDIAN SCHOOL ROAD  
EXISTING CONDITIONS DRAINAGE AREA MAP



**LEGEND**

█ DRAINAGE AREAS CONTRIBUTING TO 3RD AVE

█ DRAINAGE AREAS CONTRIBUTING TO INDIAN SCHOOL RD

← FLOW ARROW

### 36" at S=0.0063 ft/ft; d/D=1.0

Project Description	
Friction Method	Manning Formula
Solve For	Discharge

---

Input Data	
Roughness Coefficient	0.013
Channel Slope	0.006 ft/ft
Normal Depth	36.0 in
Diameter	36.0 in

---

Results	
Discharge	52.94 cfs
Flow Area	7.1 ft <sup>2</sup>
Wetted Perimeter	9.4 ft
Hydraulic Radius	9.0 in
Top Width	0.00 ft
Critical Depth	28.4 in
Percent Full	100.0 %
Critical Slope	0.007 ft/ft
Velocity	7.49 ft/s
Velocity Head	0.87 ft
Specific Energy	3.87 ft
Froude Number	(N/A)
Maximum Discharge	56.94 cfs
Discharge Full	52.94 cfs
Slope Full	0.006 ft/ft
Flow Type	Undefined

---

GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

---

GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Average End Depth Over Rise	0.0 %
Normal Depth Over Rise	100.0 %
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	36.0 in
Critical Depth	28.4 in
Channel Slope	0.006 ft/ft
Critical Slope	0.007 ft/ft

## 48" at S=0.0063 ft/ft; d/D=1.0

Project Description	
Friction Method	Manning Formula
Solve For	Discharge

---

Input Data	
Roughness Coefficient	0.013
Channel Slope	0.006 ft/ft
Normal Depth	48.0 in
Diameter	48.0 in

---

Results	
Discharge	114.01 cfs
Flow Area	12.6 ft <sup>2</sup>
Wetted Perimeter	12.6 ft
Hydraulic Radius	12.0 in
Top Width	0.00 ft
Critical Depth	38.7 in
Percent Full	100.0 %
Critical Slope	0.007 ft/ft
Velocity	9.07 ft/s
Velocity Head	1.28 ft
Specific Energy	5.28 ft
Froude Number	(N/A)
Maximum Discharge	122.64 cfs
Discharge Full	114.01 cfs
Slope Full	0.006 ft/ft
Flow Type	Undefined

---

GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

---

GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Average End Depth Over Rise	0.0 %
Normal Depth Over Rise	100.0 %
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	48.0 in
Critical Depth	38.7 in
Channel Slope	0.006 ft/ft
Critical Slope	0.007 ft/ft



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## *APPENDIX III*

# *Preliminary Grading and Drainage Plans*

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Scottsdale, AZ 85260

**CIVIL ENGINEER:**  
SEG  
8280 E. GELDING DR. SUITE #101  
SCOTTSDALE, AZ 85260  
480-588-7226  
ATTN: ALI FAKH

**CLIENT:**  
PEG DEVELOPMENT  
180 N. UNIVERSITY AVE  
SUITE 200, PROVO UT 84601  
801-655-1998  
ATTN: MATT KRAMBULE

**ARCHITECT:**  
GENSLER  
2575 E. CAMELBACK RD  
SUITE 175, PHOENIX AZ 85016  
602-253-4900  
ATTN: JOHANNA COLLINS

7120 E. INDIAN SCHOOL ROAD SCOTTSDALE, AZ 85251

**THE TRIANGLE**  
7120 E INDIAN SCHOOL RD,  
SCOTTSDALE, AZ 85251

- CASE PRE-APP NUMBER -  
63-PA-2020

**Gensler**  
2575 E Camelback Road  
Suite 175  
Phoenix, AZ 85016  
United States  
Tel: 602.533.4900  
Fax: 602.533.4949

**SYDNOR**  
4806 N 78TH Place  
Scottsdale, AZ 85251  
United States  
Tel: 480.206.4593

Date	Revised/Initial	Description
10/16/20		Reubmittal

**SEG**  
8280 E. GELDING DRIVE  
Suite 101  
Scottsdale, AZ 85260  
United States  
Tel: 480.588.7226

**NOT FOR CONSTRUCTION**

Project Name  
3RD AVENUE+INDIAN SCHOOL ROAD - SCOTTSDALE, AZ

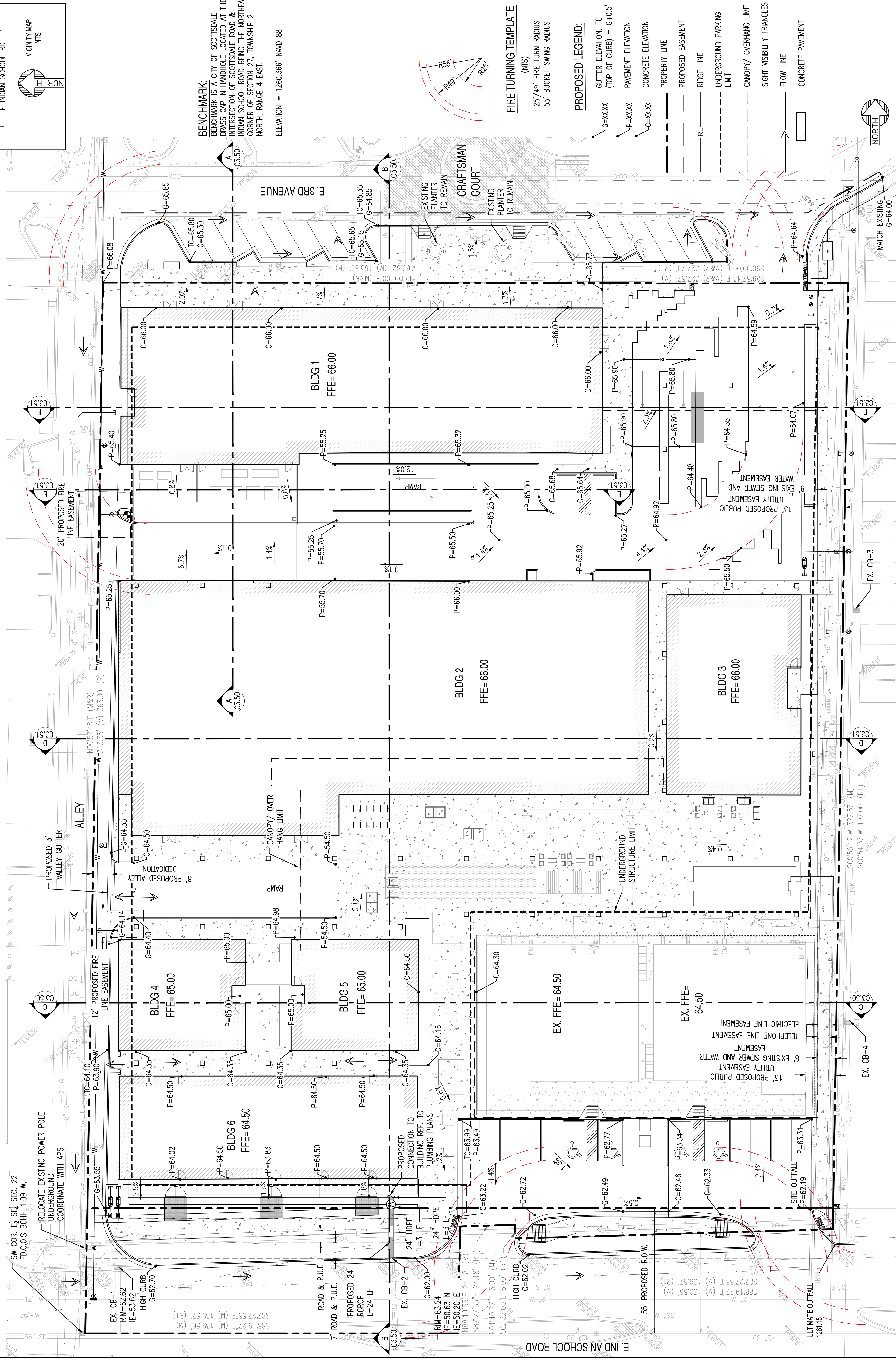
Project Number  
200504 (SEG)

Description  
PRELIMINARY GRADING PLAN

Scale  
As indicated

C3.00

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**BENCHMARK:**  
BENCHMARK IS A CITY OF SCOTTSDALE BRASS CAP IN HANDHOLE LOCATED AT THE INTERSECTION OF SCOTTSDALE ROAD & INDIAN SCHOOL ROAD BEING THE NORTHEAST CORNER OF SECTION 27, TOWNSHIP 2 NORTH, RANGE 4 EAST.  
ELEVATION = 1260.366' NAVD 88

**FIRE TURNING TEMPLATE (NTS)**  
25' / 49' FIRE TURN RADIUS  
55' BUCKET SWING RADIUS

**PROPOSED LEGEND:**  
GUTTER ELEVATION, TC (TOP OF CURB) = G+0.5'  
PAVEMENT ELEVATION  
CONCRETE ELEVATION  
PROPERTY LINE  
PROPOSED EASEMENT  
RIDGE LINE  
UNDERGROUND PARKING LIMIT  
CANOPY / OVERHANG LIMIT  
SIGHT VISIBILITY TRIANGLES  
FLOW LINE  
CONCRETE PAVEMENT

**20' PROPOSED FIRE LINE EASEMENT**

**PROPOSED 3' VALLEY GUTTER**

**8' PROPOSED ALLEY DEDICATION**

**RELOCATE EXISTING POWER POLE UNDERGROUND COORDINATE WITH APS**

**PROPOSED CONNECTION TO BUILDING REF. TO PLUMBING PLANS**

**24" HDPE L=5' LF**

**24" HDPE L=24' LF**

**PROPOSED 24" RGRCP L=24' LF**

**ROAD & P.U.E.**

**55' PROPOSED R.O.W.**

**13' PROPOSED PUBLIC UTILITY EASEMENT AND EXISTING SEWER**

**1'3" PROPOSED PUBLIC UTILITY EASEMENT**

**8' EXISTING SEWER AND WATER UTILITY EASEMENT**

**TELEPHONE LINE EASEMENT**

**ELECTRIC LINE EASEMENT**

**8' EXISTING SEWER AND WATER UTILITY EASEMENT**

**13' PROPOSED PUBLIC UTILITY EASEMENT**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

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**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

**EX. FFE= 64.50**

SW COR. E4 SE4 SEC. 22  
FD.C.O.S BCHH. 1.09 W.

ULTIMATE CUTTAWAY  
1261.15

55' PROPOSED R.O.W.

ROAD & P.U.E.

PROPOSED 24" RGRCP L=24' LF

24" HDPE L=24' LF

24" HDPE L=5' LF

PROPOSED CONNECTION TO BUILDING REF. TO PLUMBING PLANS

RELOCATE EXISTING POWER POLE UNDERGROUND COORDINATE WITH APS

8' PROPOSED ALLEY DEDICATION

PROPOSED 3' VALLEY GUTTER

20' PROPOSED FIRE LINE EASEMENT



**CIVIL ENGINEER:**

SEG  
8280 E. GELDING DR, SUITE #101  
SCOTTSDALE, AZ 85260  
480-588-7226  
ATTN: ALI FAKIH

**DEVELOPER/OWNER:**

PEG/ COMPANIES  
180 N. UNIVERSITY AVE  
SUITE 200, UT 84601  
801-655-1988  
ATTN: MATT KRAMBULE

**ARCHITECT:**

GENSLER  
2575 E. CAMELBACK RD  
SUITE 175, PHOENIX AZ 85016  
602-253-4900  
ATTN: JOHANNA COLLINS

**THE TRIANGLE**

7120 E. INDIAN SCHOOL ROAD SCOTTSDALE, AZ 85251

**THE TRIANGLE**  
7120 E INDIAN SCHOOL RD,  
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**SYDNOR**

4806 N 78TH Place  
Scottsdale, AZ 85251  
United States  
Tel: 480.206.4593

Date	Resubmittal	Description
10/16/20		



8280 E. GELDING DRIVE  
Suite 101  
Scottsdale, AZ 85260  
United States  
Tel: 480.588.7226

Seal / Signature

**NOT FOR  
CONSTRUCTION**

Project Name  
**3RD AVENUE+INDIAN SCHOOL  
ROAD - SCOTTSDALE, AZ**

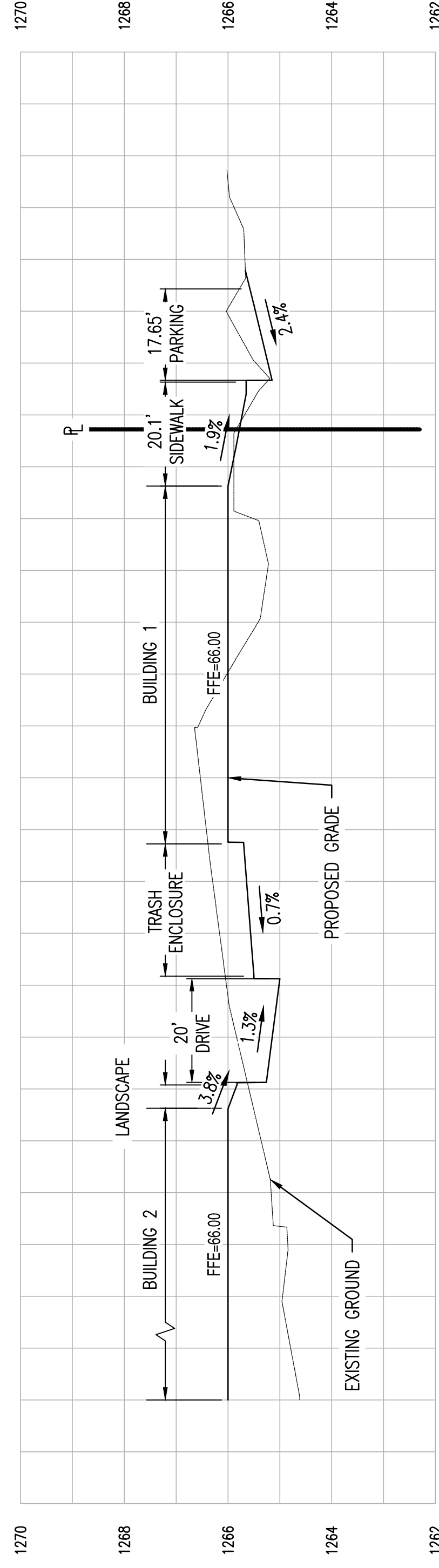
Project Number  
**200504 (SEG)**

Description  
**PRELIMINARY CROSS SECTION**

Scale  
As indicated

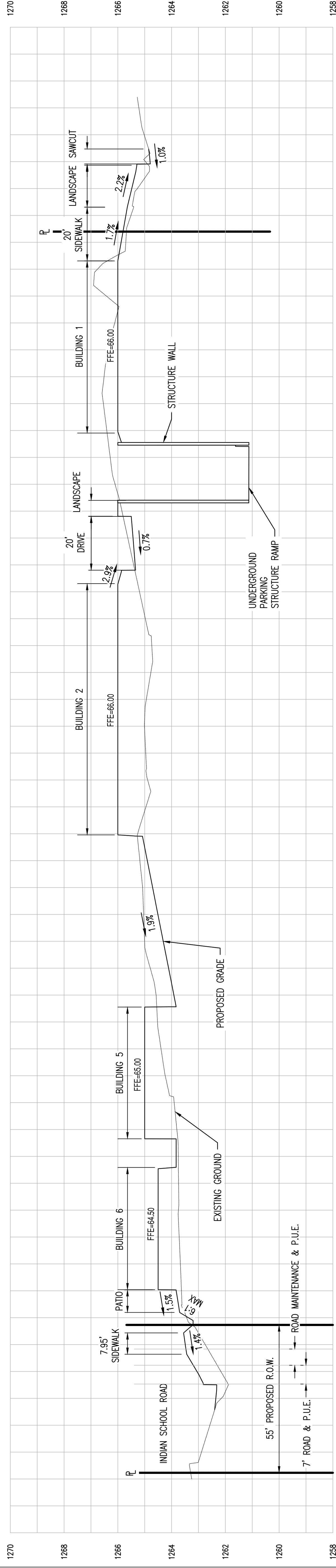
C3.50

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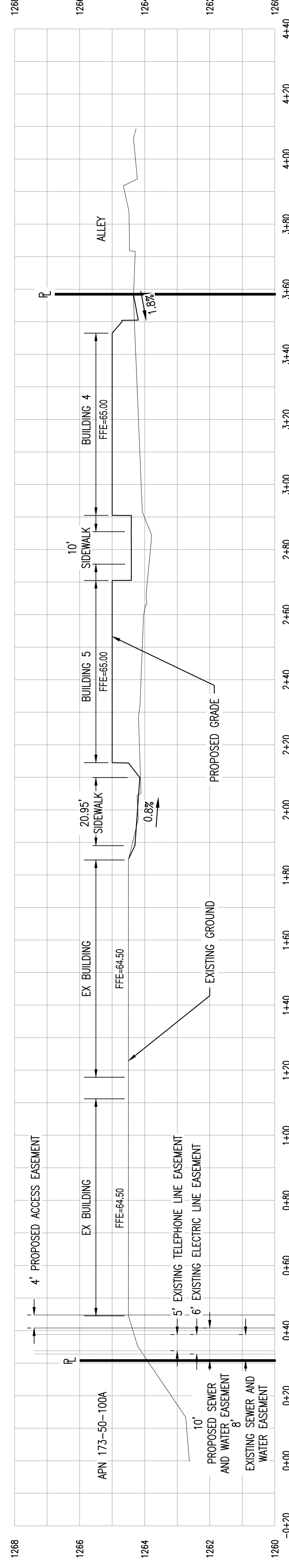
**PROFILE VIEW A-A**

SCALE:  
HORZ: 1"=20'  
VERT: 1"=2'



**PROFILE VIEW B-B**

SCALE:  
HORZ: 1"=20'  
VERT: 1"=2'



**PROFILE VIEW C-C**

SCALE:  
HORZ: 1"=20'  
VERT: 1"=2'

**CIVIL ENGINEER:**  
SEG  
8280 E. GELDING DR, SUITE #101  
SCOTTSDALE, AZ 85260  
480-588-7226  
ATTN: ALI FAKIH

**DEVELOPER/OWNER:**  
PEG/ COMPANIES  
180 N. UNIVERSITY AVE  
SUITE 200, UT 84601  
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Date	Resubmittal	Description
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Seal / Signature

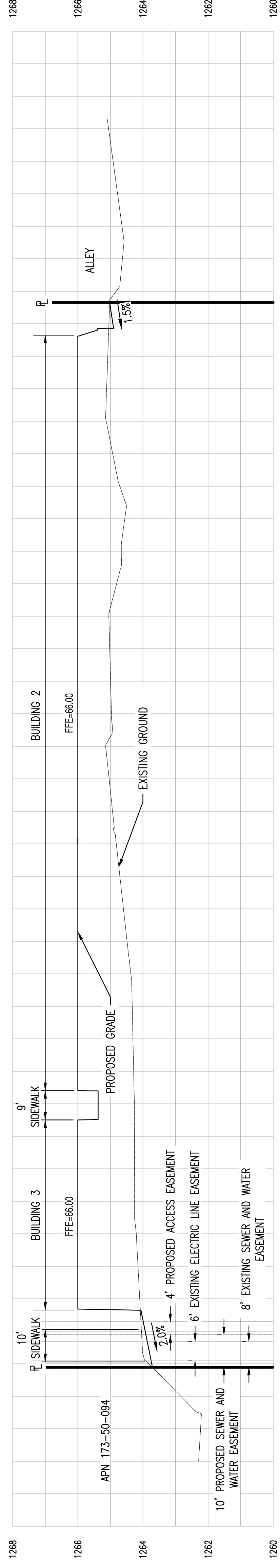
**NOT FOR  
CONSTRUCTION**

Project Name	3RD AVENUE+INDIAN SCHOOL ROAD - SCOTTSDALE, AZ
Project Number	200504 (SEG)
Description	PRELIMINARY CROSS SECTION

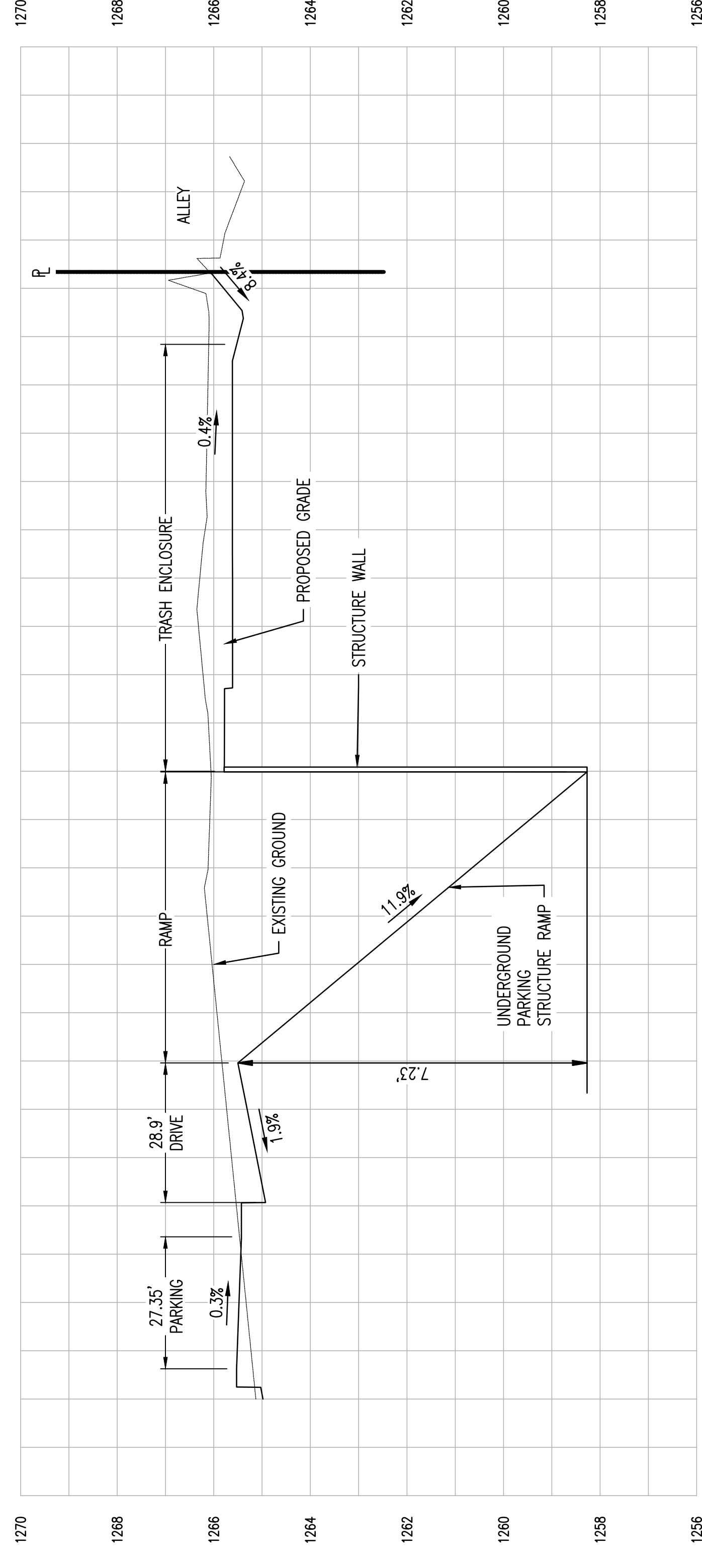
Scale  
As indicated

C3.51

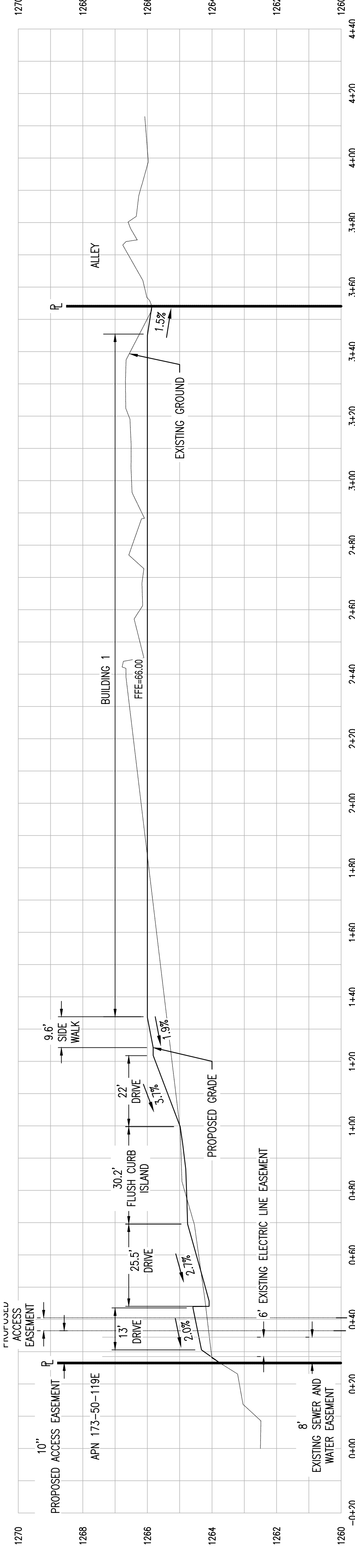
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**PROFILE VIEW D-D**



**PROFILE VIEW E-E**



**PROFILE VIEW F-F**



*“LEED®ing and Developing Smart Projects”*

*APPENDIX IV*  
*Site Visit Photos*

**EAST BOUNDARY SITE PHOTOS 8/27/2020**



NORTHEAST BOUNDARY OF PARCEL 173-50-119E



PARCEL 173-50-119E EXISTING INLET STRUCTURE, EX. CB-3



EAST BOUNDARY BETWEEN PARCELS 173-50-108A AND 173-50-094 LOOKING NORTH



EAST BOUNDARY BETWEEN PARCELS 173-50-108A AND 173-50-094 LOOKING SOUTH



SOUTHEAST BOUNDARY BETWEEN PARCELS  
173-50-108A AND 173-50-100A LOOKING NORTH



SOUTHEAST BOUNDARY BETWEEN THE TRIANGLE  
BUILDING AND PARCEL 173-50-100A LOOKING NORTH



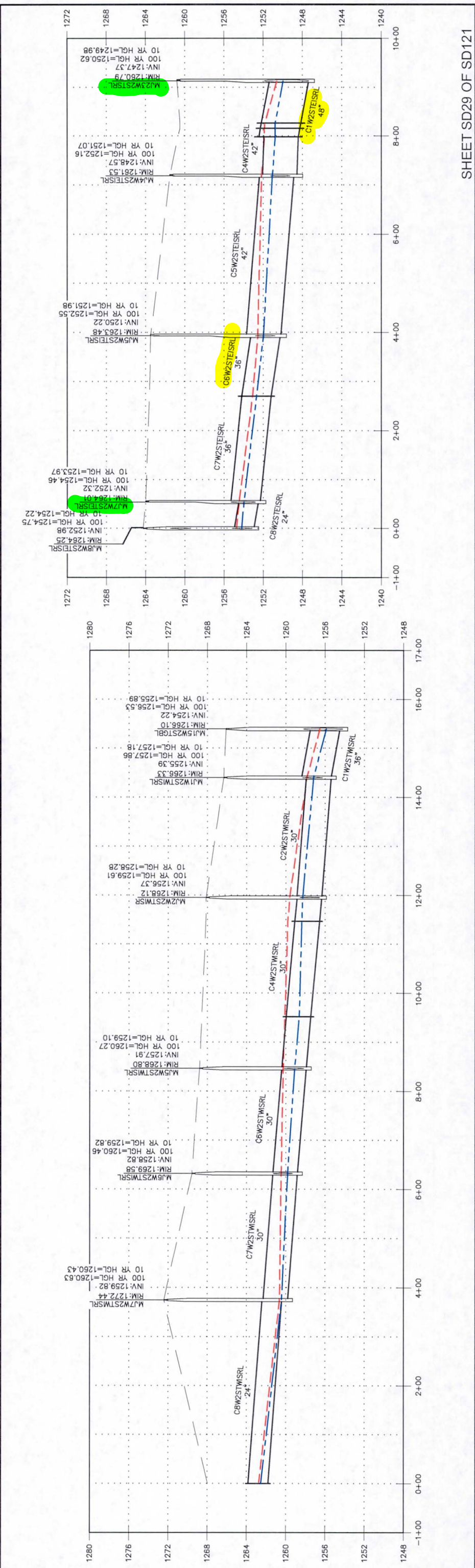
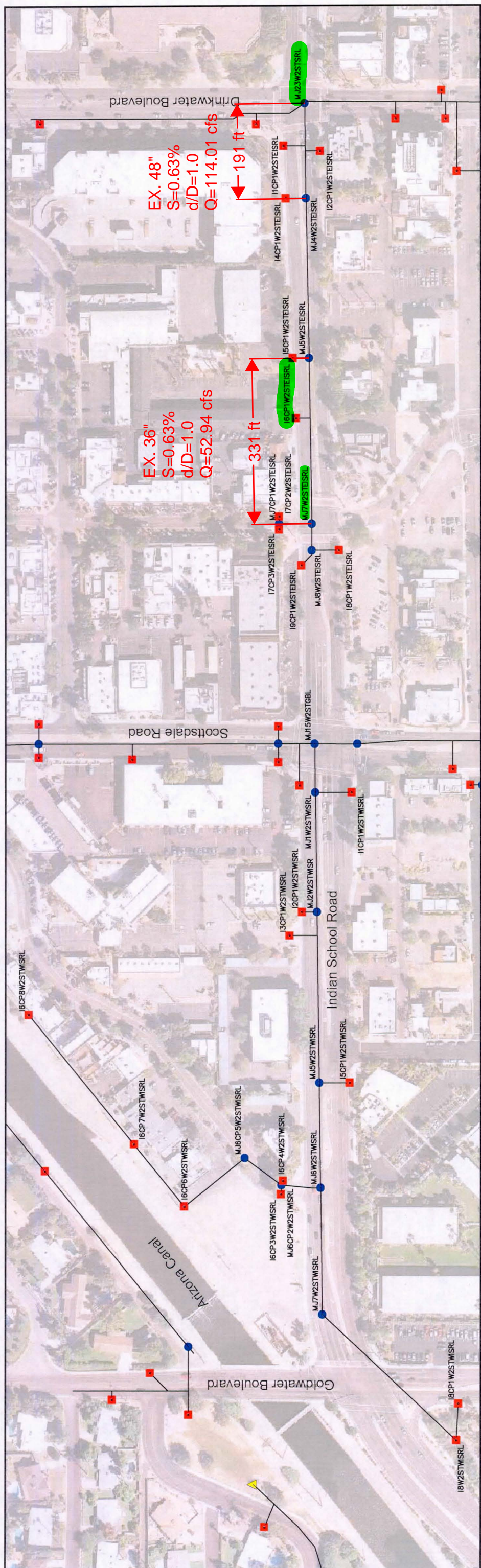
SOUTHWEST CORNER OF PARCEL 173-50-100A



EXISTING INLET, EX. CB-4, IN PARCEL 173-50-100A

## *APPENDIX V*

### *Lower Indian Bend Wash ADMP Excerpt*



**LOWER INDIAN BEND WASH ADMS/P  
STUDY AREA-SOUTH**

SHEET SD29 OF SD121

**Legend (Plan)**

- Inlet & SWMM Identifier
- Manhole & SWMM Identifier
- Outlet & SWMM Identifier
- Storm Drain Pipes

**Legend (Profile)**

- Ground Elevation
- Hydraulic Grade Line (100yr. 6-hr)
- Hydraulic Grade Line (10yr. 6-hr)

**Gavan & Barker**  
engineers, planners, scientists

**TYLIN INTERNATIONAL**  
engineers, planners, scientists

**Flood Control District**

**SWMM Outfall:**  
W2STBOUTFALL  
(West 2nd Street S.D. Outfall)

Prepared	By	Date	Date
Checked	AJA	12/18/2017	12/18/2017
	MITG		

Scale: 1" = 10' Vertical  
1" = 200' Horizontal

**Inlet Summary Table**

FLO-2D/SWMM Model

SWM M Name	Curb High / Soffit High Inflow (cfs)	FLO-2D/SWMM Model					
		100-yr, 24-hr		100-yr, 6-hr		10-yr, 6-hr	
Inlet	Connector Pipe	Inflow (cfs)	Pipe Max (cfs)	Inflow (cfs)	Pipe Max (cfs)	Inflow (cfs)	Pipe Max (cfs)
I2CPW2ST	C32CPW2ST	0.5	0.5	0.5	0.3	0.4	0.4
I3CPW2ST	C32CPW2ST	0.9	0.9	1.0	0.6	0.7	0.7
I2CPW2STSR	C2CPW2STSR	4.9	4.9	5.4	3.4	4.0	4.0
I1CPW2STSR	C1CPW2STSR	1.5	1.4	3.0	0.5	0.6	0.6
I4CPW2STSR	C4CPW2STSR	5.6	5.6	6.4	2.5	3.5	3.5
I6CPW2STSR	C6CPW2STSR	1.5	1.5	1.7	1.0	1.2	1.2
I7CPW2STSR	C7CPW2STSR	7.3	7.3	8.6	4.0	5.1	5.0
I8CPW2STSR	C8CPW2STSR	0.7	0.7	0.8	0.4	0.5	0.5
I9CPW2STSR	C9CPW2STSR	0.9	0.9	1.8	0.6	0.7	0.7
I5CPW2STSR	C5CPW2STSR	6.7	6.7	8.1	3.8	4.7	4.7
I6CPW2STSR	C6CPW2STSR	4.8	4.8	4.7	11.4	12.7	12.7
I2CPW2STSR	C2CPW2STSR	4.7	4.6	5.9	2.7	3.3	3.3
I20CPW2STSR	C20CPW2STSR	2.7	2.7	4.0	1.5	1.8	1.8
I1CPW2STSR	C1CPW2STSR	0.1	0.1	0.1	0.1	0.1	0.1
I17CPW2STSR	C17CPW2STSR	1.1	1.1	1.3	0.6	0.7	0.7
I16CPW2STSR	C16CPW2STSR	1.9	1.9	2.2	0.9	1.1	1.1
I5CPW2STSR	C5CPW2STSR	4.0	4.0	4.2	2.9	3.3	3.3
I4CPW2STSR	C4CPW2STSR	2.0	2.0	3.0	0.3	0.4	0.4
I3CPW2STSR	C3CPW2STSR	3.3	3.2	3.8	1.6	2.0	2.0
I10CPW2STSR	C10CPW2STSR	13.8	13.8	15.0	6.7	9.2	9.2
I10CPW2STSR	C10CPW2STSR	1.2	1.2	3.2	3.1	0.6	0.6
I2CPW2STSR	C2CPW2STSR	1.6	1.5	2.1	2.0	0.9	0.9
I8CPW2STSR	C8CPW2STSR	8.4	8.3	9.9	5.3	6.2	6.2
I8CPW2STSR	C8CPW2STSR	1.1	9.4	14	11.2	0.6	6.8
I20CPW2ST	C20CPW2ST	1.1	1.1	2.0	0.3	0.5	0.5
I2CPW2ST	C2CPW2ST	9.7	9.7	14.5	3.5	5.3	5.3
I20CPW2ST	C20CPW2ST	0.8	0.8	1.6	0.3	0.4	0.4
I1W2STBAL	C1W2STBAL	119	29.0	16.8	34.7	7.3	23.4
I24CPW2ST	C24CPW2ST	1.6	1.6	2.0	2.0	1.2	1.2
I24CPW2ST	C24CPW2ST	1.9	1.9	2.3	2.2	1.4	1.4
I3CPW2ST	C33CPW2ST	1.5	1.5	1.9	0.9	1.1	1.1
I34CPW2ST	C34CPW2ST	1.3	1.2	1.6	0.7	0.9	0.9
I5CPW2ST	C35CPW2ST	2.0	1.8	2.3	1.0	1.2	1.2
I7CPW2ST	C37CPW2ST	8.9	8.9	10.4	5.8	7.0	7.0
I37CPW2ST	C37CPW2ST	3.0	3.0	3.6	1.8	2.2	2.2
I36CPW2ST	C36CPW2ST	8.7	8.7	10.6	4.7	6.1	6.1
I43CPW2ST	C43CPW2ST	11.0	11.0	13.2	13.1	6.7	6.7
I47CPW2ST	C47CPW2ST	5.8	5.8	6.7	3.7	4.5	4.5
I46CPW2ST	C46CPW2ST	1.6	1.6	2.0	0.8	1.1	1.1
I2CPW2ST69STL	C2CPW2ST69STL	0.9	0.9	1.0	0.5	0.6	0.6
I1CPW2ST69STL	C1CPW2ST69STL	3.1	3.1	3.2	1.7	2.0	2.0
I9CPW2STBAL	C9CPW2STBAL	1.8	3.9	3.2	6.2	2.1	4.0
I9CPW2STBAL	C9CPW2STBAL	1.9	2.0	3.1	3.0	2.0	2.1
I7CPW2STBAL	C7CPW2STBAL	2.0	1.9	2.6	2.5	2.1	2.1
I4CPW2STBAL	C4CPW2STBAL	5.5	5.4	5.9	5.4	5.4	5.3

INLET SUMMARY TABLE NOTES:

- The curb high/soffit high inflow discharge were calculated according to the procedures outlined in the District's Hydraulics Manual.
- The inflow discharge is the peak hydrograph discharge taken from the SWMMQIN.OUT file.
- The pipe Max Discharge is the peak hydrograph discharge taken from the 'Link Results' in the SWMM.RTP file.

**Inlet Summary Table**

FLO-2D/SWMM Model

SWM M Name	Curb High / Soffit High Inflow (cfs)	FLO-2D/SWMM Model					
		100-yr, 24-hr		100-yr, 6-hr		10-yr, 6-hr	
Inlet	Connector Pipe	Inflow (cfs)	Pipe Max (cfs)	Inflow (cfs)	Pipe Max (cfs)	Inflow (cfs)	Pipe Max (cfs)
I2CPW2STSR	C2CPW2STSR	7.4	7.9	10.5	10.6	3.1	4.5
I1CPW2STSR	C1CPW2STSR	20.0	10.1	13.1	13.0	3.5	5.6
I26CPW2STSR	C26CPW2STSR	4.8	5.8	7.6	7.5	3.6	4.4
I5CPW2STSR	C5CPW2STSR	13.9	7.2	7.8	7.8	5.1	6.1
I4CPW2STSR	C4CPW2STSR	20.0	14.9	17.0	17.0	10.7	12.2
I6CPW2STSR	C6CPW2STSR	3.4	3.9	4.3	4.3	2.7	3.2
I9CPW2STSR	C9CPW2STSR	17.4	7.1	8.2	8.2	5.0	5.8
I8CPW2STSR	C8CPW2STSR	3.1	2.3	2.7	2.7	1.6	1.9
I7CPW2STSR	C7CPW2STSR	5.2	8.5	8.9	8.8	5.5	6.2
I7CPW2STSR	C7CPW2STSR	13.1	10.8	12.7	12.7	7.4	8.9
I3CPW2STSR	C3CPW2STSR	7.8	7.2	8.4	8.4	4.7	5.5
I3CPW2STSR	C3CPW2STSR	5.4	9.2	9.1	10.6	5.6	7.0
I1CPW2STSR	C1CPW2STSR	4.8	3.9	4.6	4.6	2.5	2.9
I16CPW2STSR	C16CPW2STSR	17.4	3.9	4.4	4.4	2.6	3.1
I8W2STSR	C8W2STSR	5.0	3.6	4.3	4.3	2.2	2.7
I5CPW2STSR	C5CPW2STSR	4.8	4.0	4.3	4.3	3.1	3.4
I8CPW2STSR	C8CPW2STSR	3.1	1.0	1.2	1.2	0.7	0.8
I6CPW2STSR	C6CPW2STSR	5.0	1.3	1.6	1.6	0.7	0.8
I6CPW2STSR	C6CPW2STSR	5.0	2.0	2.4	2.4	1.2	1.5
I6CPW2STSR	C6CPW2STSR	2.6	1.7	2.0	2.0	1.3	1.5
I6CPW2STSR	C6CPW2STSR	2.6	1.7	2.0	2.0	1.3	1.5
I6CPW2STSR	C6CPW2STSR	5.0	0.7	0.9	0.9	0.4	0.5
I5CPW2STSR	C5CPW2STSR	5.1	0.0	0.0	0.0	0.0	0.0
I5CPW2STSR	C5CPW2STSR	7.2	6.1	7.4	7.1	3.5	4.3
I2CPW2STSR	C2CPW2STSR	5.2	1.2	1.6	1.6	0.6	0.8
I2CPW2STSR	C2CPW2STSR	5.2	3.5	4.4	4.4	1.8	2.3
I3CPW2STSR	C3CPW2STSR	17.4	7.3	9.1	9.1	3.6	4.7
I4CPW2STSR	C4CPW2STSR	7.8	7.6	8.1	8.0	5.1	5.9
I4CPW2STSR	C4CPW2STSR	5.2	1.5	1.8	1.8	0.8	1.0
I6CPW2STSR	C6CPW2STSR	3.6	1.8	2.1	2.1	1.3	1.5
I5CPW2STSR	C5CPW2STSR	17.4	6.4	8.0	8.0	3.3	4.4
I7CPW2STSR	C7CPW2STSR	11.3	4.3	5.0	5.0	2.8	3.3
I7CPW2STSR	C7CPW2STSR	5.2	3.8	4.3	4.3	2.7	3.1
I8CPW2STSR	C8CPW2STSR	5.2	0.5	0.6	0.6	0.3	0.4
I9CPW2STSR	C9CPW2STSR	17.4	3.7	3.6	5.5	0.9	1.4
I10CPW2STSR	C10CPW2STSR	17.4	5.8	6.9	6.9	3.4	4.1
I10CPW2STSR	C10CPW2STSR	7.8	4.1	4.7	4.7	2.7	3.2
I11CPW2STSR	C11CPW2STSR	17.4	8.4	10.2	10.1	4.3	5.7
I17CPW2STSR	C17CPW2STSR	17.4	3.8	4.8	4.7	2.2	2.8
I17CPW2STSR	C17CPW2STSR	17.4	3.1	3.8	3.8	1.9	2.4
I18CPW2STSR	C18CPW2STSR	17.4	3.5	4.7	4.7	1.5	2.0
I2CPW2STSR	C2CPW2STSR	4.8	0.2	0.2	0.2	0.1	0.1
I2CPW2STSR	C2CPW2STSR	5.0	0.2	0.3	0.3	0.1	0.2
I2CPW2STSR	C2CPW2STSR	4.8	0.0	0.0	0.3	0.0	0.2
I8CPW2STSR	C8CPW2STSR	7.0	2.5	3.0	3.0	1.5	1.9

SHEET SD30 OF SD121

SWMM Outfall:

W2STBWOUTFALL  
(West 2nd Street S.D. Outfall)

Prepared	By	Date
Checked	AJA	12/18/2017
	MIG	12/18/2017

**LOWER INDIAN BEND WASH ADMS/P  
STUDY AREA-SOUTH**

GAVAN & BARKER  
engineers/planners/scientists





Inlet Summary Table

SWM M Name		FLO-2D/SWMM Model											
Inlet	Connector/Pipe	Curb High / Soffit High Inflow		100-yr, 24-hr		100-yr, 6-hr		10-yr, 24-hr		10-yr, 6-hr			
		(cfs)		Inflow (cfs)	Pipe Max (cfs)	Inflow (cfs)	Pipe Max (cfs)	Inflow (cfs)	Pipe Max (cfs)	Inflow (cfs)	Pipe Max (cfs)		
I18CP2W2STGBL	C19CP2W2STGBL	7.0		4.3	4.3	4.9	4.9	2.8	2.7	3.3	3.3		
I20CP7W2STGBL	C20CP7W2STGBL	5.0		0.7	0.7	0.8	0.8	0.4	0.4	0.5	0.5		
I20CP10W2STGBL	C20CP10W2STGBL	7.0		0.3	0.3	0.3	0.3	0.2	0.2	0.2	0.2		
I20CP9W2STGBL	C20CP9W2STGBL	7.0		5.9	5.9	6.9	6.9	3.8	3.8	4.6	4.6		
I20CP4W2STGBL	C20CP4W2STGBL	7.0		5.2	5.2	5.8	5.8	3.6	3.6	4.2	4.2		
I20CP5W2STGBL	C20CP5W2STGBL	7.0		2.0	2.0	2.5	2.4	1.2	1.2	1.4	1.4		
I20CP2W2STGBL	C20CP2W2STGBL	5.0		1.3	1.3	1.5	1.5	0.7	0.7	0.8	0.8		
I8CP2W2STBAL	C8CP2W2STBAL	3.1		1.0	1.0	1.1	1.1	0.7	0.7	1.0	1.1		
I29CP1W2STSR	C29CP1W2STSR	11.3		11.3	11.2	12.4	12.4	8.6	8.5	9.6	9.6		
I7CP1W2ST	C7CP1W2ST	3.4		1.5	1.5	1.7	1.6	1.1	1.1	1.2	1.2		
I9CP1W2ST	C9CP1W2ST	3.4		2.7	2.7	2.9	2.9	2.1	2.1	2.3	2.3		
I6ACP1W2ST	C6ACP1W2ST	5.2		10.2	10.2	10.8	10.9	8.3	8.3	8.6	8.6		
I5CP1W2STCCL	C5CP1W2STCCL	10.8		0.6	0.6	0.7	0.7	0.4	0.4	0.4	0.4		
I8CP1W2STCCL	C8CP1W2STCCL	2.5		2.5	2.5	2.9	2.9	1.6	1.6	1.9	1.9		
I4CP3W2STCCL	C4CP3W2STCCL	2.5		2.3	2.3	2.8	2.8	1.3	1.3	1.6	1.6		
I4CP2W2STCCL	C4CP2W2STCCL	2.5		0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1		
I1CP1W2STCCL	C1CP1W2STCCL	2.5		0.4	1.1	0.5	1.3	0.2	0.6	0.3	0.8		
I1CP2W2STCCL	C1CP2W2STCCL	2.5		0.7	0.7	0.9	0.9	0.4	0.4	0.5	0.5		
I1CP3W2STCCL	C1CP3W2STCCL	2.5		0.5	2.0	0.5	2.4	0.3	1.2	0.4	1.5		
I1CP4W2STCCL	C1CP4W2STCCL	2.5		1.6	1.6	1.9	1.9	0.9	0.9	1.2	1.2		
I10CP1W2STCCL	C10CP1W2STCCL	2.5		0.9	0.9	1.0	1.0	0.5	0.5	0.6	0.6		
I9CP1W2STCCL	C9CP1W2STCCL	2.5		1.4	1.4	1.7	1.7	0.9	0.9	1.1	1.1		
I6CP1W2STCCL	C6CP1W2STCCL	10.8		0.4	0.4	0.5	0.5	0.2	0.2	0.3	0.3		
I7CP1W2STCCL	C7CP1W2STCCL	2.5		1.6	1.6	2.0	2.0	0.8	0.8	1.1	1.1		
I2CP2W2STBAL	C2CP2W2STBAL	4.4		3.0	3.0	3.3	3.3	2.4	2.3	2.6	2.6		
I2CP1W2STBAL	C2CP1W2STBAL	2.6		1.8	4.8	2.1	5.4	1.1	3.5	1.4	4.0		
I37ACP2W2ST	C37ACP2W2ST	2.6		3.3	3.3	3.7	3.7	2.6	2.6	2.9	2.9		
I10ACP1W2ST	C10ACP1W2ST	3.5		3.0	3.0	3.3	3.3	2.4	2.4	2.6	2.6		
I10ACP2W2ST	C10ACP2W2ST	3.6		1.8	1.8	2.0	2.0	1.1	1.1	1.4	1.3		
I3CP5W2STDBL	C3CP5W2STDBL	17.0		11.1	11.0	11.7	11.7	5.9	5.9	7.3	7.2		
I3CP4W2STDBL	C3CP4W2STDBL	11.0		8.5	32.3	17.9	38.2	1.4	17.4	1.9	21.5		
I3CP6W2STDBL	C3CP6W2STDBL	17.0		14.7	14.2	14.8	14.0	10.3	10.2	12.6	12.6		
I3CP2W2STDBL	C3CP2W2STDBL	17.0		9.2	9.1	10.0	9.7	4.7	4.7	5.9	5.9		
I3CP1W2STDBL	C3CP1W2STDBL	20.4		2.4	19.4	6.1	22.4	1.0	10.0	1.3	12.4		
I3CP3W2STDBL	C3CP3W2STDBL	17.0		8.5	8.5	9.3	9.9	4.5	4.5	5.5	5.5		
I2B1W2ST	C12B1W2ST	4.7		0.3	0.3	0.4	0.4	0.2	0.2	0.2	0.2		
I11B1W2ST	C11B1W2ST	31.4		4.3	4.3	5.0	4.9	2.9	2.9	3.4	3.4		
I11B1W2ST	C11B1W2ST	19.5		2.4	6.7	3.0	7.8	1.4	4.3	1.7	5.1		
I11ACP1W2ST	C11ACP1W2ST	3.6		0.5	0.5	0.6	0.6	0.3	0.3	0.3	0.3		
I2B1W2ST	C12B1W2ST	3.5		0.9	0.9	1.0	1.0	0.6	0.6	0.7	0.7		
I11B1W2ST	C11B1W2ST	7.6		8.8	8.8	10.3	10.1	5.2	5.2	6.3	6.3		
I11B1W2ST	C11B1W2ST	7.8		5.5	5.5	6.8	6.6	2.8	2.8	3.6	3.5		
I19B1W2ST	C19B1W2ST	3.5		2.5	2.5	2.9	2.9	1.6	1.5	1.9	1.9		
I19B1W2ST	C19B1W2ST	3.2		1.9	4.3	2.4	5.3	1.2	2.8	1.4	3.2		
I19ACP1W2ST	C19ACP1W2ST	3.4		1.3	1.3	1.7	1.6	0.8	0.8	1.0	1.0		

INLET SUMMARY TABLE NOTES:

1. The curb high/soffit high inflow discharge were calculated according to the procedures outlined in the District's Hydraulics Manual.
2. The inflow discharge is the peak hydrograph discharge taken from the SWMMQIN.OUT file.
3. The pipe Max Discharge is the peak hydrograph discharge taken from the 'Link Results' in the SWMM.RTP file.

SHEET SD31 OF SD121

SWMM Outfall:		W2ST1BWOITFALL (West 2nd Street S.D. Outfall)	
Prepared	By	AJA	Date 12/18/2017
Checked		MIG	12/18/2017

LOWER INDIAN BEND WASH ADMS/P  
STUDY AREA-SOUTH





*APPENDIX VI*

*Stormwater Storage Waiver*

# Request for Stormwater Storage Waiver



## City of Scottsdale Plan/Case Numbers:

10-ZN-2020 \_\_\_\_\_ - DR - \_\_\_\_\_ - PP - \_\_\_\_\_ PC# \_\_\_\_\_

Requests for stormwater storage waivers are reviewed as part of case submittals for the associated project. This form should be included in the preliminary drainage report with the applicant's portion completed. The preliminary drainage report shall include supporting documentation and analysis as needed to support the requested waiver.

Date \_\_\_\_\_ Project Name \_\_\_\_\_  
Project Location \_\_\_\_\_  
Applicant Contact \_\_\_\_\_ Company Name \_\_\_\_\_  
Phone \_\_\_\_\_ E-mail \_\_\_\_\_  
Address \_\_\_\_\_

### Waiver Criteria

A project must meet at least one of three criteria listed below for the city to consider waiving some or all required stormwater storage. **However, regardless of the criteria, a waiver will only be granted if the applicant can demonstrate that the effect of a waiver will not increase the potential for flooding on any property.** Check the applicable box and provide a signed and sealed engineering report and supporting engineering analysis that demonstrate the project meets the criteria and that the effect of a waiver will not increase the potential for flooding on any property.

If the runoff for the project has been included in a storage facility at another location, the applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility.

It should be noted that reductions in stormwater storage relating to

- 1. The development is adjacent to a conveyance facility that an engineering analysis shows is designed and constructed to handle the additional runoff from the site as a result of development.
- 2. The development is on a parcel less than one-half acre in size.
- 3. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO).

For a full storage waiver, a conflict with ESLO is limited to:

- Property located in the hillside landform as defined in the city Zoning Ordinance
- Property in the upper desert landform that has a land slope steeper than 5% as defined in the city Zoning Ordinance
- Property within the ESL zoning overlay district where the only viable location for a stormwater storage basin requires blasting

This full waiver only applies to those portions of property meeting one of these three requirements.

100-year/2-hour storage is allowed, but not required for redevelopment projects and development within the ESL zoning overlay. Rather, these projects must store enough stormwater to attenuate post-development flows to predevelopment levels, considering the 10- and 100-year storm events (S.R.C. Sections 37-50 and 37-51).

By signing below, I certify that the stated project meets the waiver criteria selected above as demonstrated by the attached documentation.

## Stormwater Management Department

7447 E Indian School Road, Suite 125, Scottsdale, AZ 85251 • Phone: 480-312-2500

# Request for Stormwater Storage Waiver



## City of Scottsdale Plan/Case Numbers:

10-ZN-2020 \_\_\_\_\_ - DR - \_\_\_\_\_ - PP - \_\_\_\_\_ PC# \_\_\_\_\_

### CITY STAFF TO COMPLETE THIS PAGE

Project Name \_\_\_\_\_

#### Check Appropriate Boxes:

Meets waiver criteria (specify):  1  2  3

#### Recommended Conditions of Waiver:

- All storage requirements waived.
- Post-development peak discharge rates do not exceed pre-development conditions.
- Other:

Explain: \_\_\_\_\_  
\_\_\_\_\_

**Waiver approved per above conditions.**

\_\_\_\_\_  
Floodplain Administrator or Designee

\_\_\_\_\_  
Date

## Stormwater Management Department

7447 E Indian School Road, Suite 125, Scottsdale, AZ 85251 ♦ Phone: 480-312-2500

# Request for Stormwater Storage Waiver



10-ZN-2020 \_\_\_\_\_ - DR - \_\_\_\_\_ - PP - \_\_\_\_\_ PC# \_\_\_\_\_

## In-Lieu Fee and In-Kind Contributions

In-lieu fees are only applicable to projects where post-development peak discharge rates exceed pre-development levels, based on the 10- and 100-year storm events. If the city grants a waiver, the developer is required to calculate and contribute an in-lieu fee based on what it would cost the city to provide a storage basin, sized as described below, including costs such as land acquisition, construction, landscaping, design, construction management, and maintenance over a 75-year design life. The fee for this cost is \$3.00 per cubic foot of stormwater storage for a virtual storage basin designed to mitigate the increase in runoff associated with the 100-year/2-hour storm event. The applicant may submit site-specific in-lieu fee calculations subject to the Floodplain Administrator's approval.

The Floodplain Administrator considers in-kind contributions on a case-by-case basis. An in-kind contribution can serve as part of or instead of the calculated in-lieu fee. In-kind contributions must be stormwater-related and must constitute a public benefit. In-lieu fees and in-kind contributions are subject to the approval of the Floodplain Administrator or designee.

Project Name \_\_\_\_\_

The waived stormwater storage volume is calculated using a simplified approach as follows:

### **V = ΔCRA; where**

V = stormwater storage volume required, in cubic feet,

ΔC = increase in weighted average runoff coefficient over disturbed area ( $C_{post} - C_{pre}$ ),

R = 100-year/2-hour precipitation depth, in feet (DSPM, Appendix 4-1D, page 11), and

A = area of disturbed ground, in square feet

Furthermore,

$V_w = V - V_p$ ; where

$V_w$  = volume waived,

V = volume required, and

$V_p$  = volume provided

R = \_\_\_\_\_

ΔC = \_\_\_\_\_

A = \_\_\_\_\_

V = \_\_\_\_\_

$V_p$  = \_\_\_\_\_

$V_w$  = \_\_\_\_\_

An in-lieu fee will be paid, based on the following calculations and supporting documentation:  
In-lieu fee (\$) =  $V_w$  (cu. ft.) x \$3.00 per cubic foot = \_\_\_\_\_

An in-kind contribution will be made, as follows:

\_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

No in-lieu fee is required. Reason:

\_\_\_\_\_  
\_\_\_\_\_

### Approved by:

\_\_\_\_\_  
Floodplain Administrator or Designee

\_\_\_\_\_  
Date

## Stormwater Management Department

7447 E Indian School Road, Suite 125, Scottsdale, AZ 85251 • Phone: 480-312-2500