



CITY OF SCOTTSDALE

WESTWORLD SPORTS FIELDS

PRELIMINARY DRAINAGE REPORT

Plan # _____

Project No.: PG09

Case # 9-UP-2021

Q-S # 35-51

MAY 2021

Approved

Corrections

Richard M. Anderson 07/06/2021
Reviewed By Date

Prepared For:

City of Scottsdale
7447 East Indian School Road
Scottsdale, Arizona 85251

This report and the preliminary g/d plan contained herein will need to be updated to a 75% level of design and analysis in accordance with DSPM as part of the DR submittal for the project.

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Job No. 2101

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1.0 INTRODUCTION

1.1 PROJECT DESCRIPTION/BACKGROUND

The purpose of this drainage study is to provide a basis of design for the drainage infrastructure associated with the new Westworld Sports Fields at the southwest corner of McDowell Mountain Ranch Road and Thompson Peak Parkway. The proposed complex will consist of five lighted multi-use fields, curbed parking lots, a restroom and office building with potable water and sewer connections, sidewalks, offsite street improvements and a raw (CAP Canal) water connection for sports field and landscape irrigation. The improvements are situated on a 40-acre area on the east end of Westworld which is located within the Bureau of Reclamation (Reclamation) floodwater reservoir behind Dike 4 of the CAP Canal dikes. The sports complex will be designed to meet the drainage requirements set forth by the Reclamation for development within their floodwater impoundment area as well as the design requirements outlined in the City of Scottsdale *Design Standards & Policies Manual* (DSPM).

1.2 PROJECT LOCATION

The project is located within the City of Scottsdale on the southwest corner of McDowell Mountain Ranch Road and Thompson Peak Parkway. It is located on the east end of Westworld and bound by Thompson Peak Parkway on the south and McDowell Mountain Ranch on the north and east. Refer to Figure 1 below for a detailed vicinity map.

{Figure 1 will be included with the next submittal of the Drainage Report}

Figure 1: Vicinity Map

2.0 STORMWATER RETENTION ANALYSIS

The site lies within the 100-year flood pool behind Dike 4 of the CAP Canal. Therefore, the subsurface drainage systems for the sports complex will essentially add to the volume of the floodwater storage reservoir and will be used to provide the required stormwater retention. These subsurface systems include the void space within the 4" thick gravel layer that underlies the sand-based sports fields plus the volume associated with the proposed field drains, parking lot storm drain pipes and culverts.

The required stormwater retention was calculated as the combined total from the following:

1. **Undisturbed Desert** – The full 100-year, 2-hour runoff volume was added to the retention requirement for the undisturbed desert areas of the site.
2. **Existing Westworld Parking Areas and Drives** – The pre versus post runoff volume associated with the existing gravel parking lot and driveways was also added to the required retention volume.
3. **ESLO Parcel** - The pre versus post runoff volume was added for the ESLO parcel. This is the parcel of land on the east side of the project site that the City recently purchased from the State Land Department. It lies outside of the Reclamation's jurisdiction and is included within the ESLO area and therefore is only required to store the pre versus post runoff volume.

Refer to Appendix A for the runoff volume calculations and an Exhibit showing the drainage areas associated with the runoff volumes.

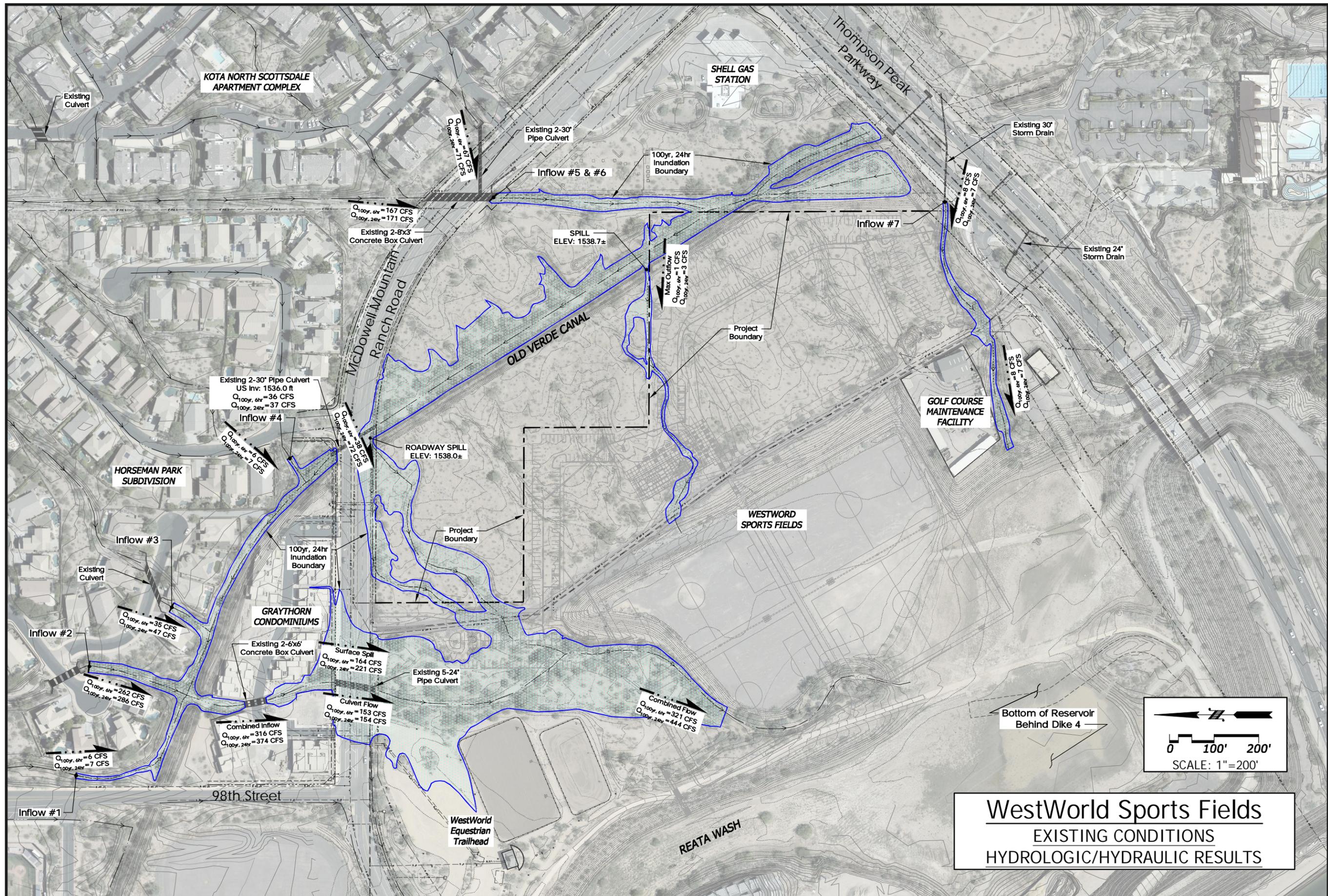
{a more complete description of the stormwater retention analysis will be included with the next submittal of the Drainage Report}

3.0 OFFSITE HYDROLOGIC ANALYSIS

Offsite flows that impact the site from north of McDowell Mountain Ranch Road were determined using the Pinnacle Peak South FLO-2D model. Several adjustments were made to the model to better define the drainage patterns. These adjustments will be fully described in the next submittal of the drainage report. **Refer to Appendix B for the Offsite Watershed Map and the FLO-2D inflow hydrographs. The FLO-2D digital data can be downloaded from the link provided in Appendix F.**

The offsite flows concentrate in the Old Verde Canal upstream of the proposed sports complex. To determine the hydraulic impact of the Old Verde Canal and better define flows that enter site, a two-dimensional HEC-RAS model was developed that covers the sports complex and the Old Verde Canal. **The results of the HEC-RAS analysis are summarized on the following Exhibit. The HEC-RAS digital data can be downloaded from the link provided in Appendix F.**

{a more complete description of the Offsite Hydrologic Analysis will be included in the next submittal of the Drainage Report}



WestWorld Sports Fields
EXISTING CONDITIONS
HYDROLOGIC/HYDRAULIC RESULTS

4.0 DESIGN HYDROLOGIC ANALYSIS

A HEC-1 model was developed to determine design flows for the onsite drainage systems. This model also includes the small, offsite contributing drainage areas that lie downstream of the Old Verde Canal. **Refer to Appendix C for the HEC-1 Schematic and Drainage Area Map which includes a summary of peak discharges calculated with HEC-1. Appendix C also includes printouts of the 100-yr, 6-hr and 100-yr, 24-hr HEC-1 models. The HEC-1 digital data can be downloaded from the link provided in Appendix F.**

{a more complete description of the Design Hydrologic Analysis will be included in the next submittal of the Drainage Report}

5.0 STORM DRAIN DESIGN AND ANALYSIS

New storm drains are proposed for the site which will 1) collect and convey offsite flows from the properties adjacent to the east side of the site that lie downstream of the Old Verde Canal and 2) convey runoff collected in catch basins installed within the new parking lot. Preliminary design calculations have been completed to size the proposed storm drain and catch basins that are shown on the Storm Drian Layout Exhibit in Appendix D. The design calculations will be finalized and documented with the next submittal of the Drainage Report.

Refer to Appendix D for the proposed storm drain layout.

{the design calculations, along with a more complete description of the analysis will be included in the next submittal of the Drainage Report}

6.0 CULVERT DESIGN & WASH HYDRUALIC ANALYSIS

The two primary offsite flows will be routed through the site in existing washes. One that runs along the south side of the site and the other that runs through the northwest corner of the site. New culverts will be provided within the washes to convey flows under the driveways and pathways. Preliminary wash hydraulic analysis and culvert sizing calculations have been completed. The layout and preliminary sizing of the proposed culverts is presented in the Exhibit in Appendix D.

The drainage plan for the site includes routing offsite flow from the Old Verde Canal through the wash that runs along the south side of the site. As can be seen from the Exhibit on page 4, the 100-year flood currently overtops the Old Verde Canal in two locations. To prevent the overtopping, it is proposed to install a culvert that will divert excess flow from the Old Verde Canal into the south wash. Since the adjacent property's stormwater retention basin is hydraulically connected to the Canal, the proposed culvert will run from their retention basin and into the south wash.

The offsite flow that runs through the NW corner of the site currently overtops McDowell Mountain Ranch Road because the existing 5-24" pipes under the roadway do not have enough capacity to convey the 100-yr flow. In accordance with the DSPM, the plan is to design the new street improvements in a manner that will ensure that the maximum flow depth over the roadway will not exceed 6 inches. The calculations of this flow depth will be included in the next submittal of the Drainage Report.

Refer to Appendix D for the proposed pipe culverts.

{the design calculations, along with a more complete description of the analysis will be included in the next submittal of the Drainage Report}

7.0 FEMA FLOOD ZONE / LOWEST FLOOR ELEVATION

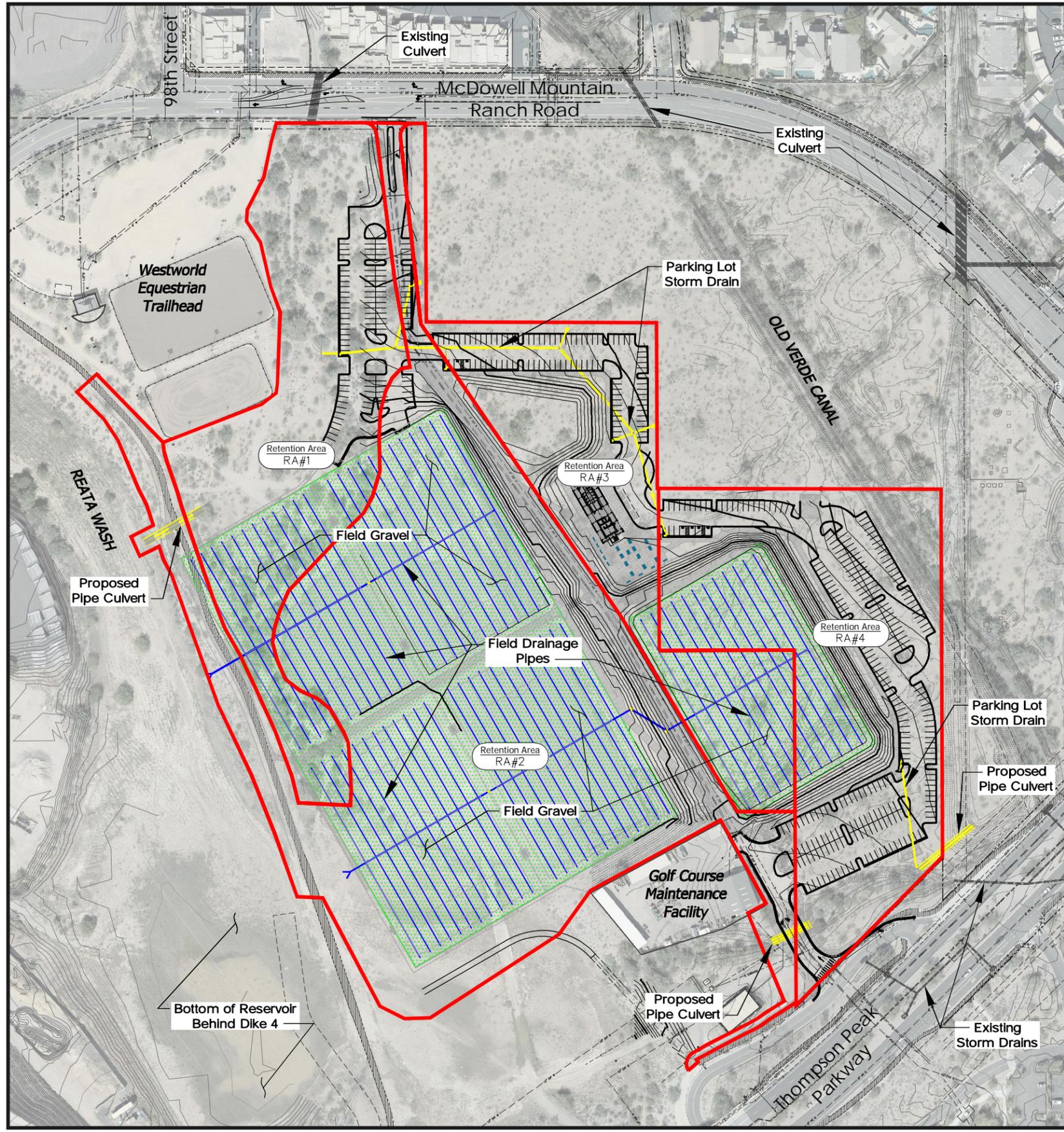
The site is located within FEMA Flood Zone A (FEMA Map No. 04013C1340L, dated Oct. 16, 2013). The Zone A Floodplain does not include a Base Flood Elevation (BFE), but the Reclamation established a 100-year water surface elevation (WSEL) of 1526.00 ft (NGVD29) for the flood pool behind Dike 4. This is a very conservative estimate of the BFE because it includes a 100-year runoff volume of 2320 ac-ft plus a long-term sediment accumulation of 1080 ac-ft. With the level of development at Westworld, it seems very unlikely that 1080 ac-ft of sediment would be allowed to accumulate. If the site did experience such sediment loads, the City would be forced to remove the sediment, or it would cover much of the developed area within Westworld.

Since the site design is based on City of Scottsdale vertical datum (NAVD88), we converted the Reclamation's WSEL to NAVD88 using the National Geodetic Survey's VERTCON program. The conversion obtained from VERTCON is $NGVD29 + 1.75 \text{ ft} = NAVD88$. Therefore, the WSEL for the flood pool behind Dike 4 is 1527.75 ft based on City of Scottsdale's vertical datum.

We propose to set the finished floor of the site's Restroom/Office Building at 1528.75 or higher to be at least one foot above the BFE.

Appendix A: Stormwater Retention Calculations

Retention Design – Drainage Area Map



LEGEND

Retention Area Boundary

100-yr, 2-hr RUNOFF VOLUME SUMMARY TABLE

RETENTION AREA	Contributing Drainage Area (sq/ft)	100-yr, 2-hr Runoff Volume (cu.ft.)
RA#1	222,200	20,910
RB#3	175,800	18,980

PRE. vs. POST RUNOFF VOLUME SUMMARY TABLE

RETENTION AREA	Contributing Drainage Area (sq/ft)	Pre Development Runoff Volume (cu.ft.)	Post Development Runoff Volume (cu.ft.)	Increase in Runoff Volume (cu.ft.)
RA#2	249,030	21,946	29,971	8,025
RB#4	548,770	68,510	45,209	-23,300

NOTES:

- 1) All required retention storage for the Westworld Sports Fields site is provided within the subsurface drainage system of the new sports fields.
- 2) Under existing conditions, the project site consists of undeveloped desert and previously developed gravel parking areas and access roads. The retention requirements are as follows:
 - A. Retention Areas #1 and #3 consists of undeveloped desert. Therefore, the full 100-year, 2-hour runoff was included in the required retention volume.
 - B. Retention Area #2 of the project site has been previously developed and therefore only the increase runoff was included in the required retention volume.
 - C. Retention Area #4 consists of undeveloped desert. However, since it is located within the ESL Ordinance, only the increase in runoff volume was added to the retention requirement.

REQUIRED RUNOFF VOLUME

TOTAL RUNOFF VOLUME = RA#1 + RA#2 + RA#3 + RA#4
 TOTAL RUNOFF VOLUME = 20,910 + 8,025 + 18,980 - 23,300
TOTAL RUNOFF VOLUME = 24,615 cu.ft

PROVIDED STORAGE VOLUME

SUBSURFACE STORAGE VOLUME = Field Drainage Pipes + Field Gravel + Parking Lot SD
 SUBSURFACE STORAGE VOLUME = 3,100 + 57,400 + 4,300
SUBSURFACE STORAGE VOLUME = 64,800 cu.ft



0 100' 200'
 SCALE: 1" = 200'

Submittal :
 G&B No. 2101
 Issue Date: 04/21
 Drawn By: OK
 Checked By: MITG

Sheet Title:
Retention Volume Drainage Area Map

Sheet Number:
1
 1 of 1

100-year, 2-hour Volume Calculation

100-year, 2-hour Runoff Volume Calculations

WestWorld

Multi-Use Sports Fields

Gavan & Barker No. 2101

Project No.: PG09



Retention Area#1: 100-yr 2-hr Runoff Volume

Cover Type	Area (A) (sq.ft)	Runoff Coefficient* (C)	Area x C	Rainfall Depth^ (inches)	Runoff Volume (cu.ft)
Impermeable (Sidewalk, Parking, etc)	35,540	0.95	33,763.0	2.35	6,611.9
Desert Landscaping	117,430	0.45	52,843.5	2.35	10,348.5
Grass Areas (Turf Fields)	67,230	0.30	20,169.0	2.35	3,949.8
Total Contributing Drainage Area:	220,200		Total 100-year, 2-hour Runoff Volume		20,910

Retention Area#3: 100-yr 2-hr Runoff Volume

Cover Type	Area (A) (sq.ft)	Runoff Coefficient* (C)	Area x C	Rainfall Depth^ (inches)	Runoff Volume (cu.ft)
Impermeable (Sidewalk, Parking, etc)	61,850	0.95	58,757.5	2.35	11,506.7
Desert Landscaping	26,510	0.45	11,929.5	2.35	2,336.2
Grass Areas (Turf Fields)	87,440	0.30	26,232.0	2.35	5,137.1
Total Contributing Drainage Area:	175,800		Total 100-year, 2-hour Runoff Volume		18,980

^The 100-year, 2-hour rainfall depth was obtained from Appendix 4-1D of the *City of Scottsdale Drainage Policies and Standards Manual*.

*The runoff coefficients were obtained from Figure 4-1.5 of the *City of Scottsdale Drainage Policies and Standards Manual*.

Pre vs. Post 100-year, 2-hour Runoff Volume Calculation

Retention Area #2: Pre vs Post 100-year, 2-hour Runoff Volume Calculations

WestWorld

Multi-Use Sports Fields

Gavan & Barker No. 2101

Project No.: PG09



Retention Area #2: Pre Development 100-yr 2-hr Runoff Volume

Cover Type	Area (A) (sq.ft)	Runoff Coefficient* (C)	Area x C	Rainfall Depth^ (inches)	Runoff Volume (cu.ft)
Undeveloped Desert	249,030	0.45	112,063.5	2.35	21,945.8
Total Contributing Drainage Area:	249,030			Total Pre Development Runoff Volume	21,946

Retention Area #2: Post Development 100-yr 2-hr Runoff Volume

Cover Type	Area (A) (sq.ft)	Runoff Coefficient* (C)	Area x C	Rainfall Depth^ (inches)	Runoff Volume (cu.ft)
Impermeable (Sidewalk, Parking, etc)	104,200	0.95	98,990.0	2.35	19,385.5
Desert Landscaping	36,930	0.45	16,618.5	2.35	3,254.5
Grass Areas (Turf Fields)	74,130	0.30	22,239.0	2.35	4,355.1
Undeveloped Desert (NAOS)	33,770	0.45	15,196.5	2.35	2,976.0
Total Contributing Drainage Area:	249,030			Total Post Development Runoff Volume	29,971
<u>Total Pre vs. Post Runoff Volume Increase :</u>					<u>8,025</u>

^The 100-year, 2-hour rainfall depth was obtained from Appendix 4-1D of the *City of Scottsdale Drainage Policies and Standards Manual*.

*The runoff coefficients were obtained from Figure 4-1.5 of the *City of Scottsdale Drainage Policies and Standards Manual*.

Retention Area #4: Pre vs Post 100-year, 2-hour Runoff Volume Calculations

WestWorld

Multi-Use Sports Fields

Gavan & Barker No. 2101

Project No.: PG09



Retention Area #4: Pre Development 100-yr 2-hr Runoff Volume

Cover Type	Area (A) (sq.ft)	Runoff Coefficient* (C)	Area x C	Rainfall Depth^ (inches)	Runoff Volume (cu.ft)
Gravel Access Road & Parking Area	278,080	0.82	228,025.6	2.35	44,655.0
Desert Landscaping	270,690	0.45	121,810.5	2.35	23,854.6
Total Contributing Drainage Area:	548,770			Total Pre Development Runoff Volume	68,510

Retention Area #4: Post Development 100-yr 2-hr Runoff Volume

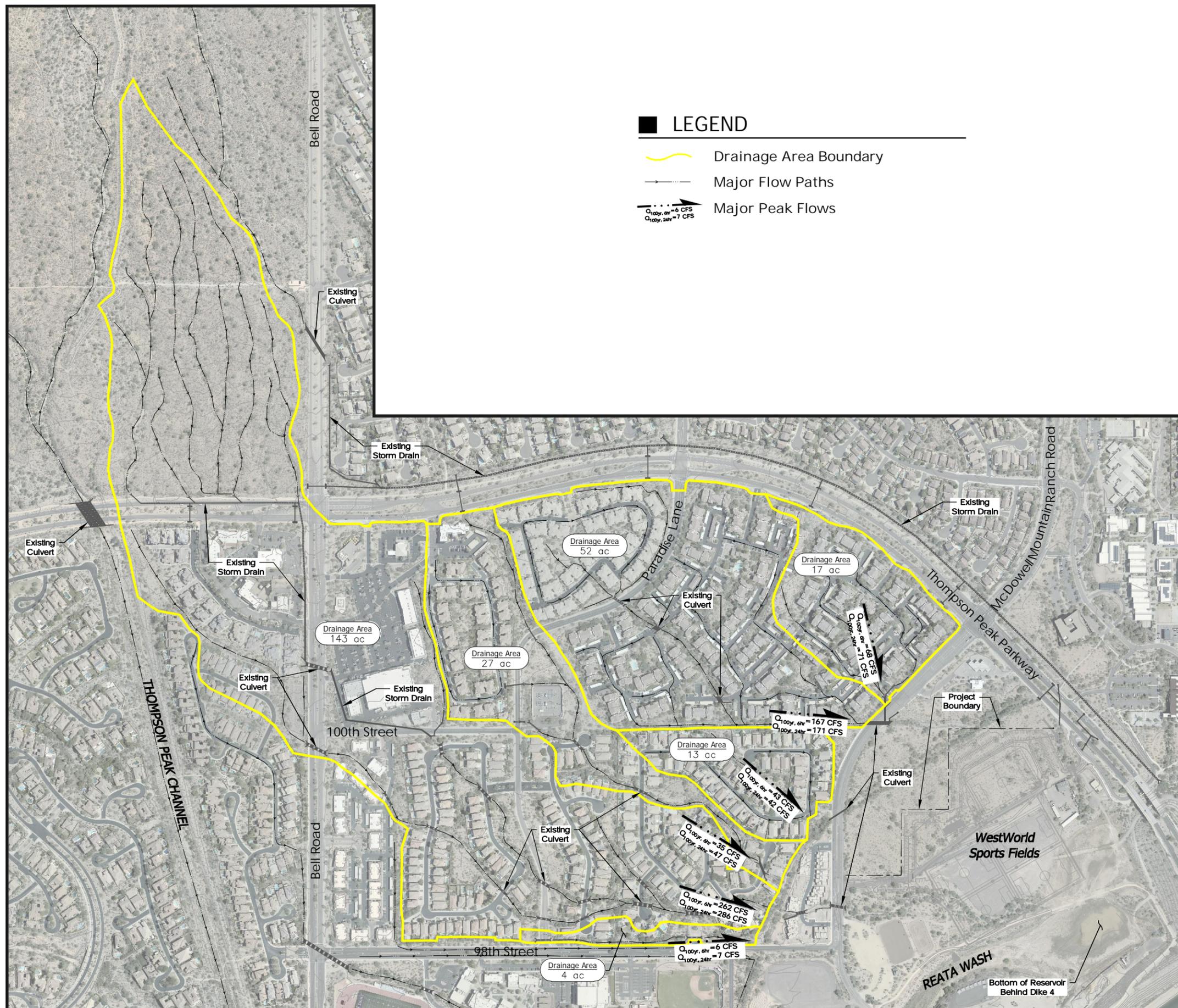
Cover Type	Area (A) (sq.ft)	Runoff Coefficient* (C)	Area x C	Rainfall Depth^ (inches)	Runoff Volume (cu.ft)
Impermeable (Sidewalk, Parking, etc)	76,140	0.95	72,333.0	2.35	14,165.2
Desert Landscaping	111,560	0.45	50,202.0	2.35	9,831.2
Grass Areas (Turf Fields)	361,070	0.30	108,321.0	2.35	21,212.9
Total Contributing Drainage Area:	548,770			Total Post Development Runoff Volume	45,209
				<u>Total Pre vs. Post Runoff Volume Increase :</u>	<u>-23,300</u>

^The 100-year, 2-hour rainfall depth was obtained from Appendix 4-1D of the *City of Scottsdale Drainage Policies and Standards Manual*.

*The runoff coefficients were obtained from Figure 4-1.5 of the *City of Scottsdale Drainage Policies and Standards Manual*.

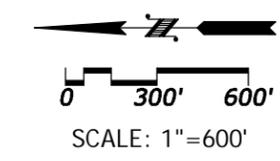
Appendix B: Offsite Hydrologic Analysis

Offsite FLO-2D Model Watershed Map



LEGEND

- Drainage Area Boundary
- Major Flow Paths
- Major Peak Flows
 $Q_{100yr, 6hr} = 6 \text{ CFS}$
 $Q_{100yr, 24hr} = 7 \text{ CFS}$



**WestWorld
Sports Fields**
OFFSITE WATERSHED
FLO-2D EXHIBIT

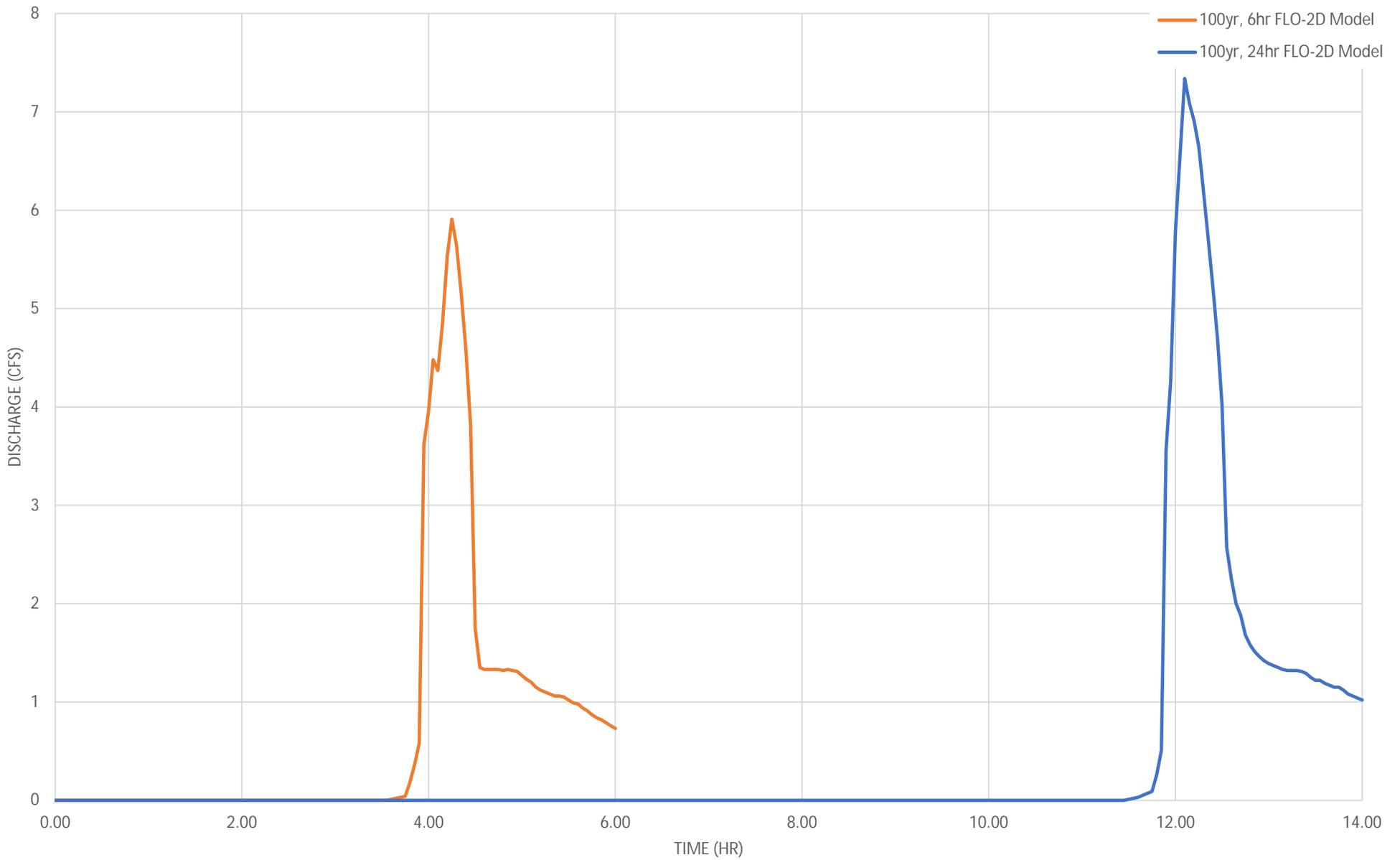
Submittal:	
G&B No.	2101
Issue Date:	04/21
Drawn By:	OK
Checked By:	MITG

Sheet Title:
**Offsite
Watershed
FLO-2D
Exhibit**

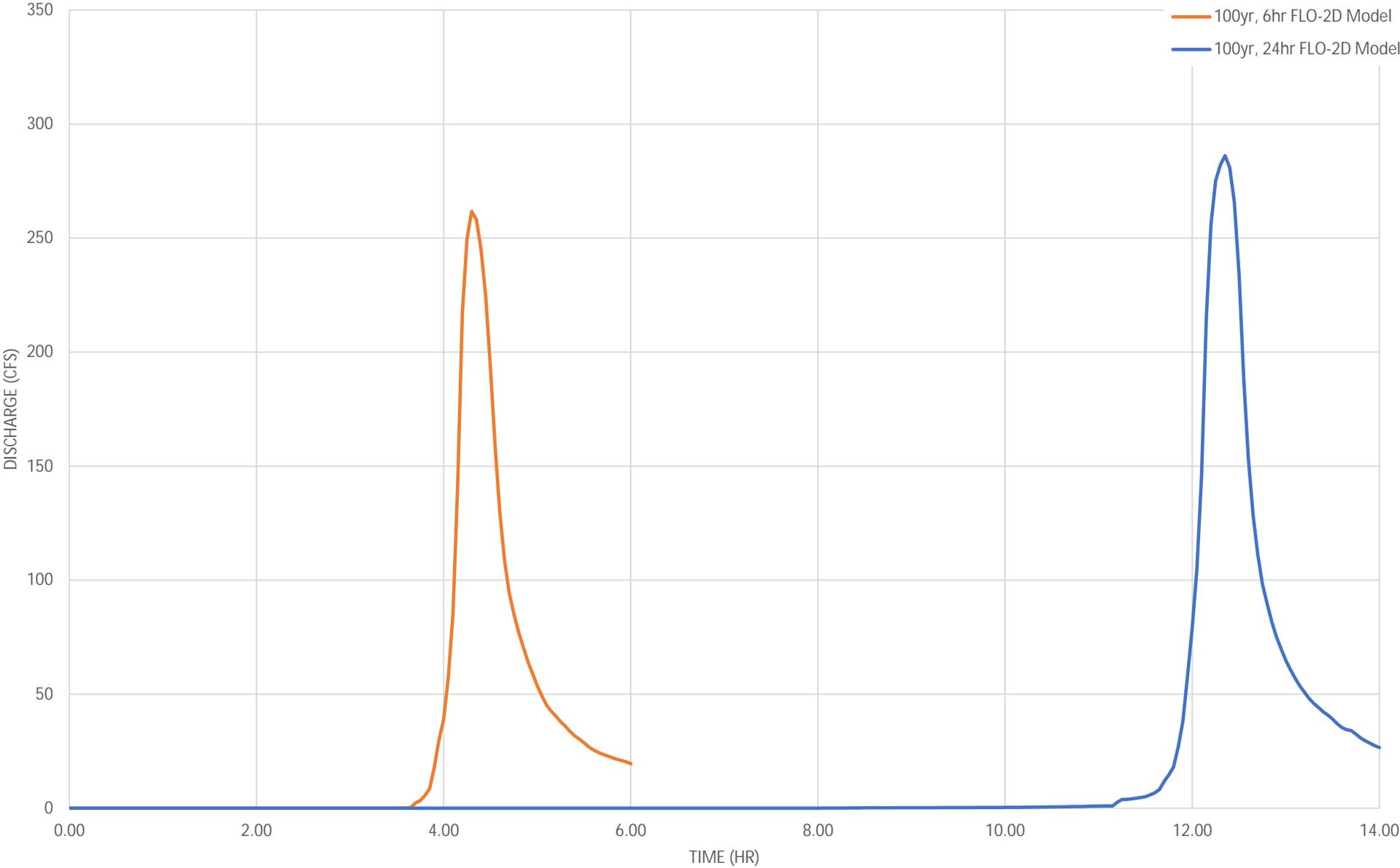
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1
1 of 1

Offsite FLO-2D Model Inflow Hydrographs

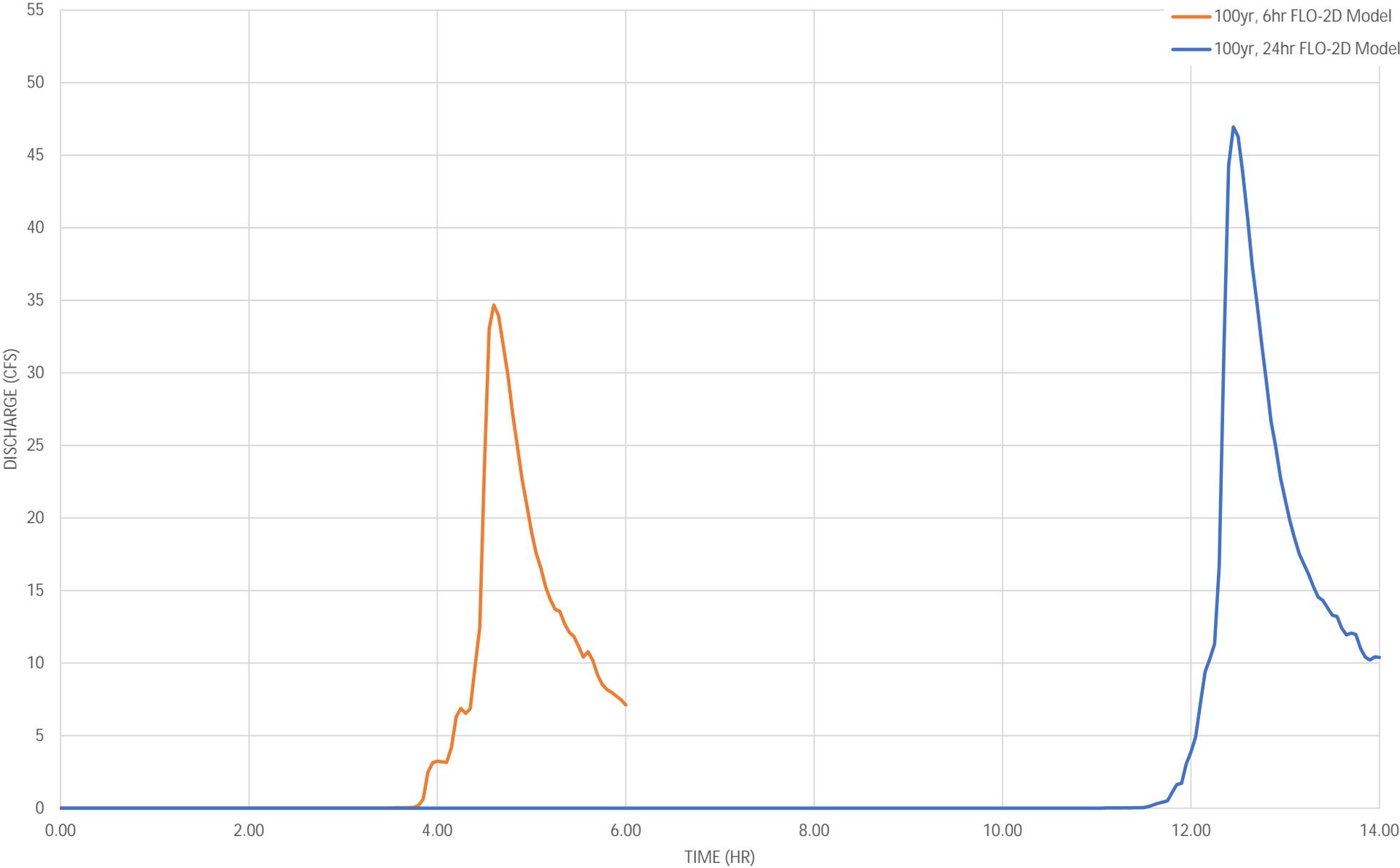
INFLOW #1 HYDROGRAPH (FLO-2D FP XSEC: 248)



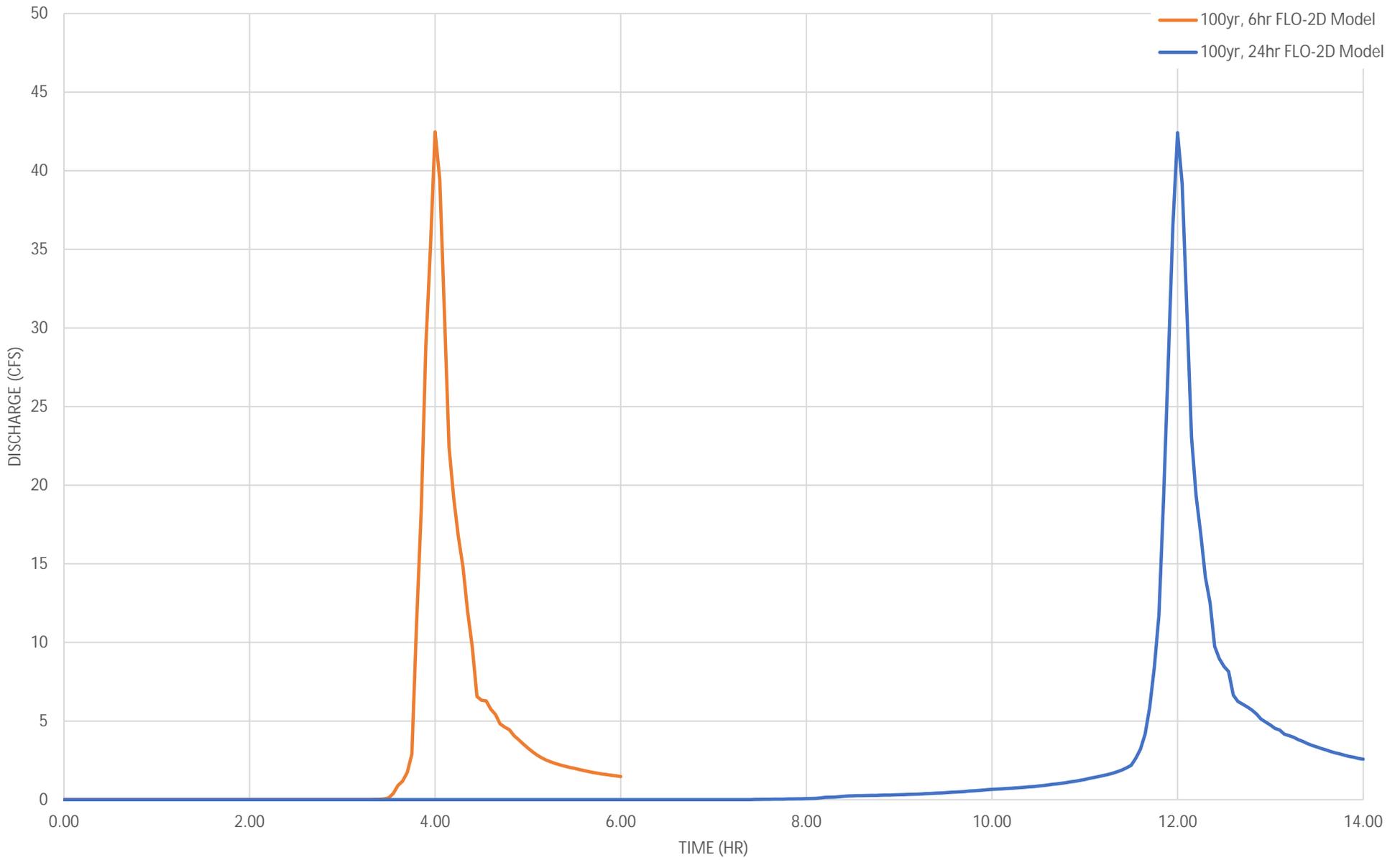
INFLOW #2 HYDROGRAPH (FLO-2D FP XSEC: 245)



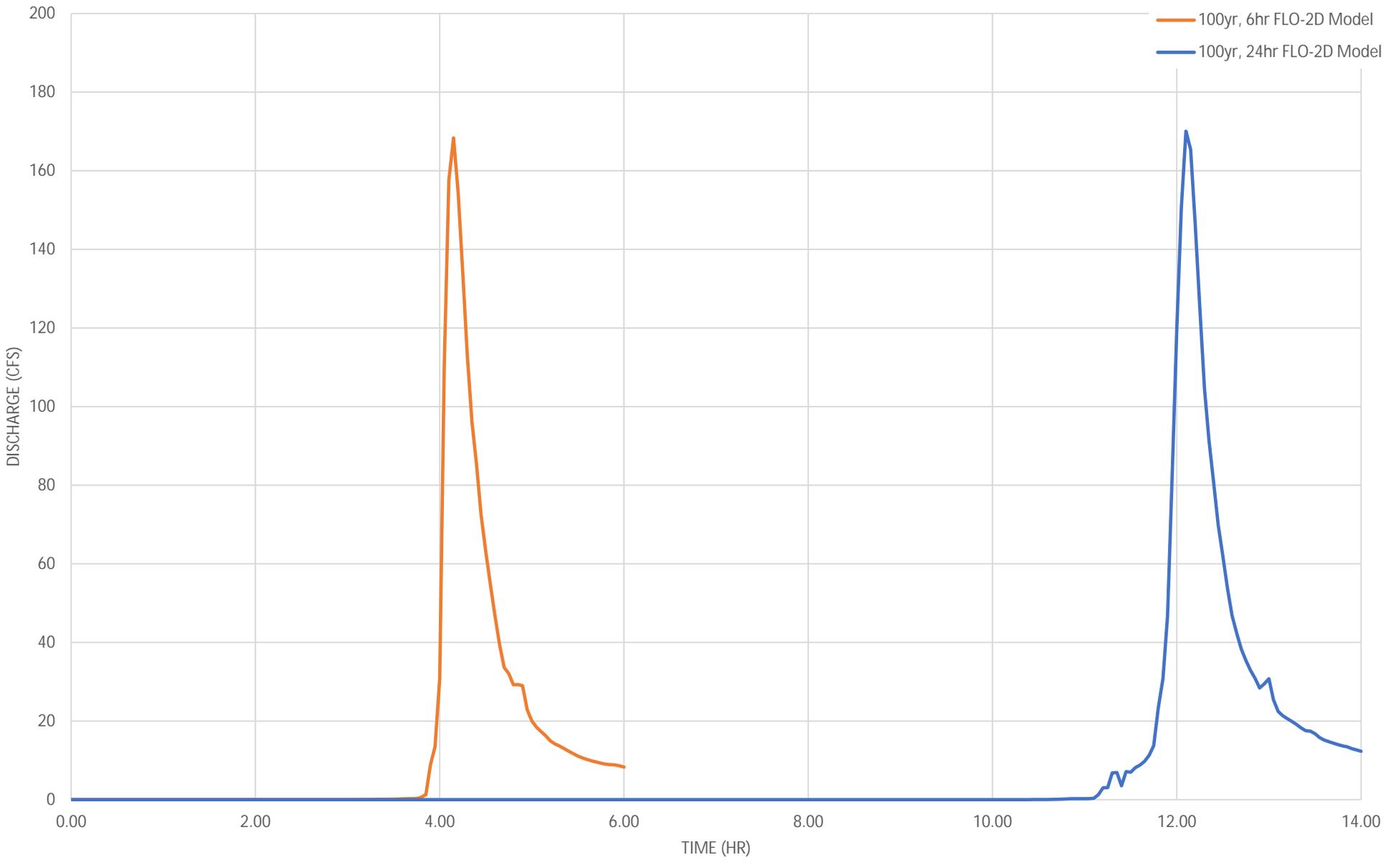
INFLOW #3 HYDROGRAPH (FLO-2D FP XSEC: 250)



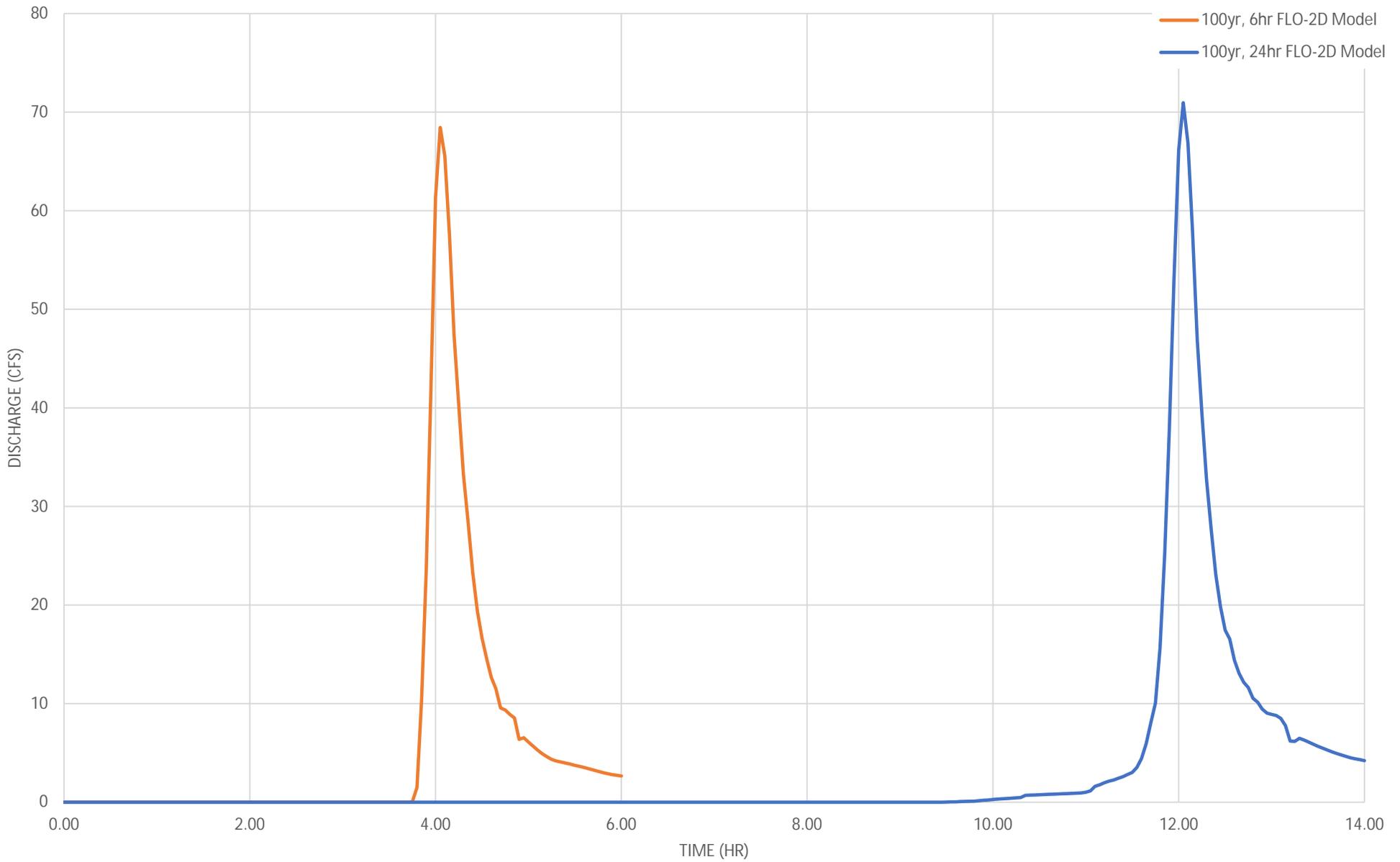
INFLOW #4 HYDROGRAPH (FLO-2D FP XSEC: 240)



INFLOW #5 HYDROGRAPH (FLO-2D FP XSEC: 242)



INFLOW #6 HYDROGRAPH (FLO-2D FP XSEC: 243)



Appendix C: Design Hydrologic Analysis

Design HEC-1 Schematic and Drainage Area Map

LEGEND & HEC-1 SYMBOLOGY

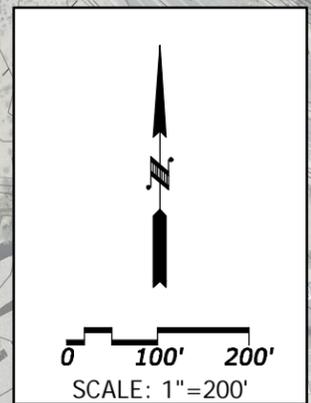
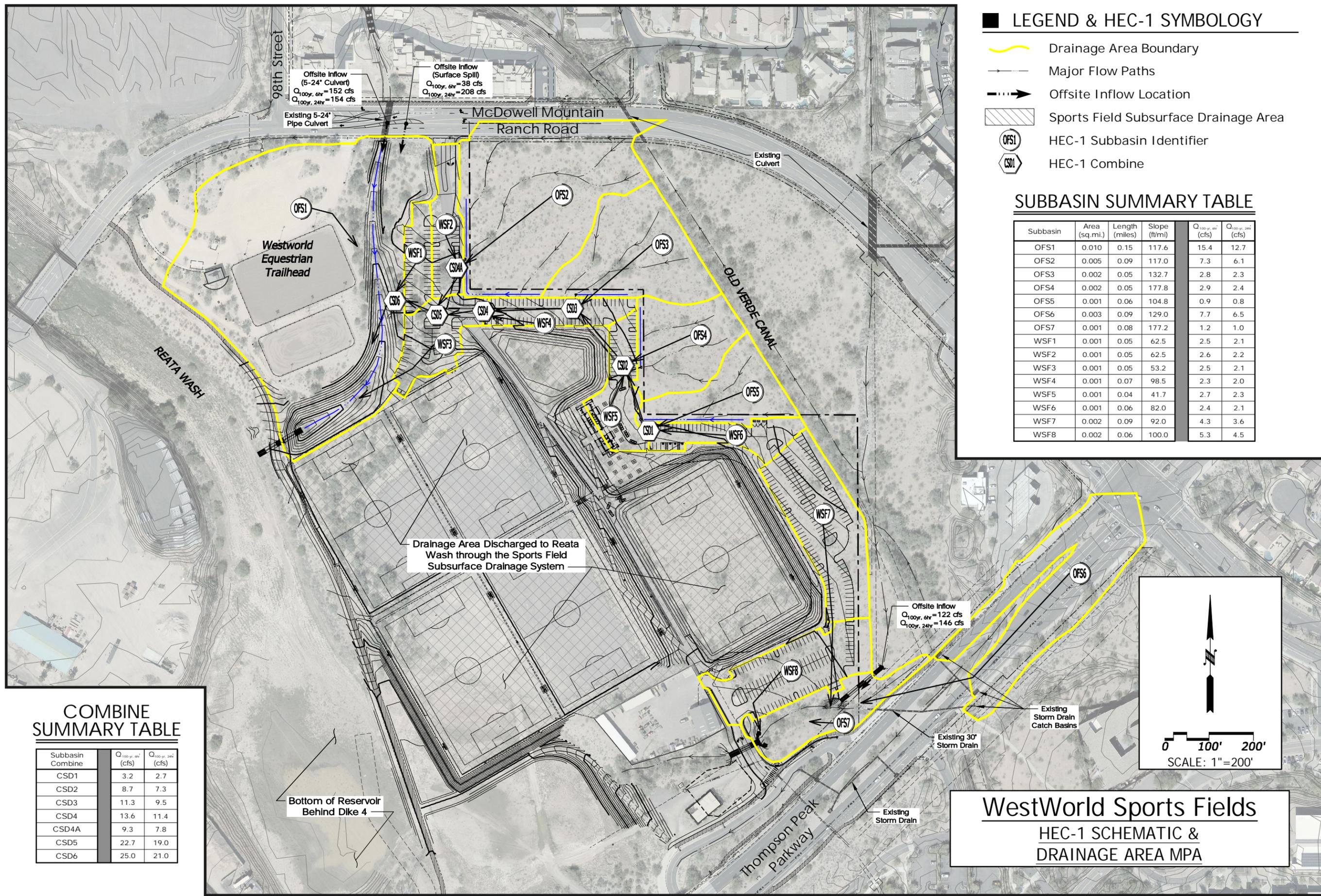
- Drainage Area Boundary
- Major Flow Paths
- Offsite Inflow Location
- Sports Field Subsurface Drainage Area
- HEC-1 Subbasin Identifier
- HEC-1 Combine

SUBBASIN SUMMARY TABLE

Subbasin	Area (sq. mi.)	Length (miles)	Slope (ft/mi)	$Q_{100-yr, 6hr}$ (cfs)	$Q_{100-yr, 24hr}$ (cfs)
OFS1	0.010	0.15	117.6	15.4	12.7
OFS2	0.005	0.09	117.0	7.3	6.1
OFS3	0.002	0.05	132.7	2.8	2.3
OFS4	0.002	0.05	177.8	2.9	2.4
OFS5	0.001	0.06	104.8	0.9	0.8
OFS6	0.003	0.09	129.0	7.7	6.5
OFS7	0.001	0.08	177.2	1.2	1.0
WSF1	0.001	0.05	62.5	2.5	2.1
WSF2	0.001	0.05	62.5	2.6	2.2
WSF3	0.001	0.05	53.2	2.5	2.1
WSF4	0.001	0.07	98.5	2.3	2.0
WSF5	0.001	0.04	41.7	2.7	2.3
WSF6	0.001	0.06	82.0	2.4	2.1
WSF7	0.002	0.09	92.0	4.3	3.6
WSF8	0.002	0.06	100.0	5.3	4.5

COMBINE SUMMARY TABLE

Subbasin Combine	$Q_{100-yr, 6hr}$ (cfs)	$Q_{100-yr, 24hr}$ (cfs)
CSD1	3.2	2.7
CSD2	8.7	7.3
CSD3	11.3	9.5
CSD4	13.6	11.4
CSD4A	9.3	7.8
CSD5	22.7	19.0
CSD6	25.0	21.0



WestWorld Sports Fields
 HEC-1 SCHEMATIC &
 DRAINAGE AREA MPA

Submittal:
 G&B No. 2101
 Issue Date: 04/21
 Drawn By: OK
 Checked By: MITG

Sheet Title:
HEC-1 SCHEMATIC & DRAINAGE AREA MAP

Sheet Number:
1
 1 of 1

100-year, 6-hour HEC-1 Model

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 21MAY21 TIME 09:44:19 *
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*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID City of Scottsdale
2 ID WESTWORLD MUSF - WestWorld Multi-Use Sports Fields
3 ID 100 YEAR
4 ID 6 Hour Storm
5 ID Unit Hydrograph: Clark
6 ID 05/21/2021
*DIAGRAM
7 IT 2 1JAN99 0 360
8 IO 5
9 IN 15
*
10 KK OFS5 BASIN
11 BA 0.001
12 PB 2.755 0.0001
13 PC 0.000 0.008 0.016 0.025 0.033 0.041 0.050 0.058 0.066 0.074

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52	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
53	UA	100									
54	ZW	A=OFS3	B=BASIN	C=FLOW	F=CALC						
	*										
55	KK	CSD3	COMBINE								
56	HC	2									
57	ZW	A=CSD3	B=COMBINE	C=FLOW	F=CALC						
	*										
58	KK	WSF4	BASIN								
59	BA	0.001									
60	LG	0.07	0.34	2.75	0.93	81					
61	UC	0.106	0.188								
62	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
63	UA	100									
64	ZW	A=WSF4	B=BASIN	C=FLOW	F=CALC						
	*										
65	KK	CSD4	COMBINE								
66	HC	2									
67	ZW	A=CSD4	B=COMBINE	C=FLOW	F=CALC						
	*										
68	KK	OFS2	BASIN								
69	BA	0.005									
70	LG	0.32	0.35	2.75	1.06	11					
71	UC	0.189	0.173								
72	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
73	UA	100									
74	ZW	A=OFS2	B=BASIN	C=FLOW	F=CALC						
	*										
75	KK	WSF2	BASIN								
76	BA	0.001									
77	LG	0.07	0.34	2.75	0.93	83					
78	UC	0.103	0.139								
79	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
80	UA	100									
81	ZW	A=WSF2	B=BASIN	C=FLOW	F=CALC						
	*										

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

82	KK	CSD4A	COMBINE								
83	HC	2									
84	ZW	A=CSD4A	B=COMBINE	C=FLOW	F=CALC						
	*										
85	KK	CSD5	COMBINE								
86	HC	2									
87	ZW	A=CSD5	B=COMBINE	C=FLOW	F=CALC						

```

*
88      KK      WSF1  BASIN
89      BA      0.001
90      LG      0.08   0.34   2.75   0.93   76
91      UC      0.105  0.141
92      UA      0     5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
93      UA      100
94      ZW      A=WSF1  B=BASIN  C=FLOW  F=CALC
*

```

```

95      KK      CSD6  COMBINE
96      HC      2
97      ZW      A=CSD6  B=COMBINE  C=FLOW  F=CALC
*

```

```

98      KK      OFS1  BASIN
99      BA      0.010
100     LG      0.16   0.31   2.75   1.01   3
101     UC      0.173  0.160
102     UA      0     3.0   5.0   8.0   12.0   20.0   43.0   75.0   90.0   96.0
103     UA      100
104     ZW      A=OFS1  B=BASIN  C=FLOW  F=CALC
*

```

```

105     KK      WSF3  BASIN
106     BA      0.001
107     LG      0.07   0.34   2.75   0.93   84
108     UC      0.108  0.146
109     UA      0     5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
110     UA      100
111     ZW      A=WSF3  B=BASIN  C=FLOW  F=CALC
*

```

```

112     KK      WSF7  BASIN
113     BA      0.002
114     LG      0.12   0.35   2.75   0.93   71
115     UC      0.135  0.202
116     UA      0     5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
117     UA      100
118     ZW      A=WSF7  B=BASIN  C=FLOW  F=CALC
*

```

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

119     KK      WSF8  BASIN
120     BA      0.002
121     LG      0.10   0.35   2.75   0.93   76
122     UC      0.104  0.109
123     UA      0     5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
124     UA      100
125     ZW      A=WSF8  B=BASIN  C=FLOW  F=CALC

```

```

*
126      KK      OFS6  BASIN
127      BA      0.003
128      LG      0.08   0.34   2.87   0.85   76
129      UC      0.108  0.124
130      UA      0      5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
131      UA      100
132      ZW      A=OFS6  B=BASIN  C=FLOW  F=CALC
*

133      KK      OFS7  BASIN
134      BA      0.001
135      LG      0.35   0.35   3.86   0.51   0
136      UC      0.163  0.335
137      UA      0      3.0   5.0    8.0   12.0   20.0   43.0   75.0   90.0   96.0
138      UA      100
139      ZW      A=OFS7  B=BASIN  C=FLOW  F=CALC
*
140      ZZ

```

1

SCHMATIC DIAGRAM OF STREAM NETWORK

```

INPUT
LINE  (V) ROUTING      (--->) DIVERSION OR PUMP FLOW

NO.   (.) CONNECTOR    (<---) RETURN OF DIVERTED OR PUMPED FLOW

10    OFS5
      .
      .
21    .      WSF6
      .      .
      .      .
28    CSD1.....
      .
      .
31    .      OFS4
      .      .
      .      .
38    .      .      WSF5
      .      .      .
      .      .      .
45    CSD2.....
      .
      .
48    .      OFS3
      .      .
      .      .
55    CSD3.....
      .
      .
58    .      WSF4
      .      .
      .      .

```

```

65      CSD4.....
      .
      .
68      .      OFS2
      .      .
      .      .
75      .      .      WSF2
      .      .      .
      .      .      .
82      .      CSD4A.....
      .      .
      .      .
85      CSD5.....
      .
      .
88      .      WSF1
      .      .
      .      .
95      CSD6.....
      .
      .
98      .      OFS1
      .      .
      .      .
105     .      .      WSF3
      .      .      .
      .      .      .
112     .      .      .      WSF7
      .      .      .      .
      .      .      .      .
119     .      .      .      .      WSF8
      .      .      .      .      .
      .      .      .      .      .
126     .      .      .      .      .      OFS6
      .      .      .      .      .      .
      .      .      .      .      .      .
133     .      .      .      .      .      .      OFS7

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
*
* RUN DATE 21MAY21 TIME 09:44:19 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

WESTWORLD MUSF - WestWorld Multi-Use Sports Fields
100 YEAR
6 Hour Storm
Unit Hydrograph: Clark
05/21/2021

8 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 2 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 360 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 1JAN99 ENDING DATE
 NDTIME 1158 ENDING TIME
 ICENT 19 CENTURY MARK

 COMPUTATION INTERVAL .03 HOURS
 TOTAL TIME BASE 11.97 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

-----DSS---ZOPEN: New File Opened, File: 100YR 6HR DESIGN MODEL.DSS
 Unit: 71; DSS Version: 6-JG

-----DSS---ZWRITE Unit 71; Vers. 1: /OFS5/BASIN/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /OFS5/BASIN/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /WSF6/BASIN/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /WSF6/BASIN/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /CSD1/COMBINE/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /CSD1/COMBINE/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /OFS4/BASIN/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /OFS4/BASIN/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /WSF5/BASIN/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /WSF5/BASIN/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /CSD2/COMBINE/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /CSD2/COMBINE/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /OFS3/BASIN/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /OFS3/BASIN/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /CSD3/COMBINE/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /CSD3/COMBINE/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /WSF4/BASIN/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /WSF4/BASIN/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /CSD4/COMBINE/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /CSD4/COMBINE/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 1: /OFS2/BASIN/FLOW/31DEC1998/2MIN/CALC/

+		CSD3	11.	4.03	1.	0.	0.	.01
	HYDROGRAPH AT							
+		WSF4	2.	4.03	0.	0.	0.	.00
	2 COMBINED AT							
+		CSD4	14.	4.03	1.	1.	1.	.01
	HYDROGRAPH AT							
+		OFS2	7.	4.10	1.	0.	0.	.00
	HYDROGRAPH AT							
+		WSF2	3.	4.03	0.	0.	0.	.00
	2 COMBINED AT							
+		CSD4A	9.	4.07	1.	0.	0.	.01
	2 COMBINED AT							
+		CSD5	23.	4.07	2.	1.	1.	.01
	HYDROGRAPH AT							
+		WSF1	2.	4.03	0.	0.	0.	.00
	2 COMBINED AT							
+		CSD6	25.	4.03	2.	1.	1.	.02
	HYDROGRAPH AT							
+		OFS1	15.	4.10	1.	0.	0.	.01
	HYDROGRAPH AT							
+		WSF3	3.	4.03	0.	0.	0.	.00
	HYDROGRAPH AT							
+		WSF7	4.	4.03	0.	0.	0.	.00
	HYDROGRAPH AT							
+		WSF8	5.	4.00	0.	0.	0.	.00
	HYDROGRAPH AT							
+		OFS6	8.	4.03	1.	0.	0.	.00
	HYDROGRAPH AT							
+		OFS7	1.	4.10	0.	0.	0.	.00

*** NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 100YR 6HR DESIGN MODEL.DSS
 Pointer Utilization: .26
 Number of Records: 44
 File Size: 163.9 Kbytes
 Percent Inactive: .0

100-year, 24-hour HEC-1 Model

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 21MAY21 TIME 10:07:32 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

```

X X XXXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID City of Scottsdale
2 ID WESTWORLD MUSF - WestWorld Multi-Use Sports Fields
3 ID 100 YEAR
4 ID 24 Hour Storm
5 ID Unit Hydrograph: Clark
6 ID 05/21/2021
*DIAGRAM
7 IT 2 1JAN99 0 1220
8 IO 5
9 IN 15
*
10 KK OFS5 BASIN
11 BA 0.001
12 PB 3.842 0.0001
13 PC 0.000 0.002 0.005 0.008 0.011 0.014 0.017 0.020 0.023 0.026

```



```

54      ZW  A=CSD2  B=COMBINE  C=FLOW  F=CALC
      *

55      KK   OFS3   BASIN
56      BA  0.002
57      LG  0.35   0.35   2.75   1.09   0
58      UC  0.154  0.146
59      UA   0     3.0    5.0    8.0    12.0   20.0   43.0   75.0   90.0   96.0
60      UA  100
61      ZW  A=OFS3  B=BASIN  C=FLOW  F=CALC
      *

62      KK   CSD3  COMBINE
63      HC   2
64      ZW  A=CSD3  B=COMBINE  C=FLOW  F=CALC
      *

65      KK   WSF4   BASIN
66      BA  0.001
67      LG  0.07   0.34   2.75   0.93   81
68      UC  0.106  0.188
69      UA   0     5.0    16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
70      UA  100
71      ZW  A=WSF4  B=BASIN  C=FLOW  F=CALC
      *

72      KK   CSD4  COMBINE
73      HC   2
74      ZW  A=CSD4  B=COMBINE  C=FLOW  F=CALC
      *

75      KK   OFS2   BASIN
76      BA  0.005
77      LG  0.32   0.35   2.75   1.06   11
78      UC  0.189  0.173
79      UA   0     3.0    5.0    8.0    12.0   20.0   43.0   75.0   90.0   96.0
80      UA  100
81      ZW  A=OFS2  B=BASIN  C=FLOW  F=CALC
      *

```

1

HEC-1 INPUT

PAGE 3

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

82      KK   WSF2   BASIN
83      BA  0.001
84      LG  0.07   0.34   2.75   0.93   83
85      UC  0.103  0.139
86      UA   0     5.0    16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
87      UA  100
88      ZW  A=WSF2  B=BASIN  C=FLOW  F=CALC
      *

89      KK   CSD4A  COMBINE

```

```

90      HC      2
91      ZW      A=CSD4A  B=COMBINE  C=FLOW  F=CALC
          *

92      KK      CSD5  COMBINE
93      HC      2
94      ZW      A=CSD5  B=COMBINE  C=FLOW  F=CALC
          *

95      KK      WSF1  BASIN
96      BA      0.001
97      LG      0.08   0.34   2.75   0.93   76
98      UC      0.105  0.141
99      UA      0     5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
100     UA      100
101     ZW      A=WSF1  B=BASIN  C=FLOW  F=CALC
          *

102     KK      CSD6  COMBINE
103     HC      2
104     ZW      A=CSD6  B=COMBINE  C=FLOW  F=CALC
          *

105     KK      OFS1  BASIN
106     BA      0.010
107     LG      0.16   0.31   2.75   1.01   3
108     UC      0.173  0.160
109     UA      0     3.0   5.0    8.0    12.0   20.0   43.0   75.0   90.0   96.0
110     UA      100
111     ZW      A=OFS1  B=BASIN  C=FLOW  F=CALC
          *

112     KK      WSF3  BASIN
113     BA      0.001
114     LG      0.07   0.34   2.75   0.93   84
115     UC      0.108  0.146
116     UA      0     5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
117     UA      100
118     ZW      A=WSF3  B=BASIN  C=FLOW  F=CALC
          *

```

1

HEC-1 INPUT

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

119     KK      WSF7  BASIN
120     BA      0.002
121     LG      0.12   0.35   2.75   0.93   71
122     UC      0.135  0.202
123     UA      0     5.0   16.0   30.0   65.0   77.0   84.0   90.0   94.0   97.0
124     UA      100
125     ZW      A=WSF7  B=BASIN  C=FLOW  F=CALC
          *

```

126	KK	WSF8	BASIN								
127	BA	0.002									
128	LG	0.10	0.35	2.75	0.93	76					
129	UC	0.104	0.109								
130	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
131	UA	100									
132	ZW	A=WSF8	B=BASIN	C=FLOW	F=CALC						
	*										
133	KK	OFS6	BASIN								
134	BA	0.003									
135	LG	0.08	0.34	2.87	0.85	76					
136	UC	0.108	0.124								
137	UA	0	5.0	16.0	30.0	65.0	77.0	84.0	90.0	94.0	97.0
138	UA	100									
139	ZW	A=OFS6	B=BASIN	C=FLOW	F=CALC						
	*										
140	KK	OFS7	BASIN								
141	BA	0.001									
142	LG	0.35	0.35	3.86	0.51	0					
143	UC	0.163	0.335								
144	UA	0	3.0	5.0	8.0	12.0	20.0	43.0	75.0	90.0	96.0
145	UA	100									
146	ZW	A=OFS7	B=BASIN	C=FLOW	F=CALC						
	*										
147	ZZ										

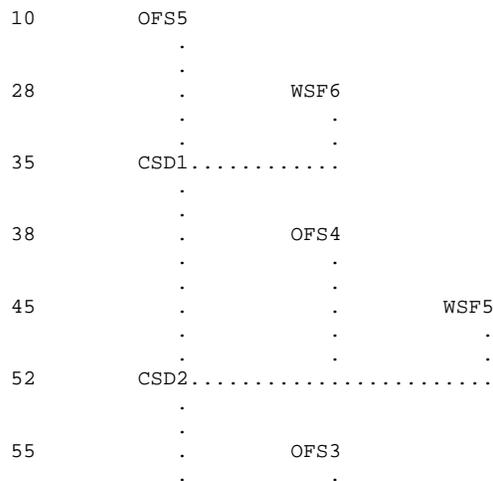
1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
LINE

(V) ROUTING (--->) DIVERSION OR PUMP FLOW

NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW



```

62      .
      CSD3.....
      .
65      .      WSF4
      .      .
      .      .
72      CSD4.....
      .
      .
75      .      OFS2
      .      .
      .      .
82      .      .      WSF2
      .      .      .
      .      .      .
89      .      CSD4A.....
      .      .
      .      .
92      CSD5.....
      .
      .
95      .      WSF1
      .      .
      .      .
102     CSD6.....
      .
      .
105     .      OFS1
      .      .
      .      .
112     .      .      WSF3
      .      .      .
      .      .      .
119     .      .      .      WSF7
      .      .      .      .
      .      .      .      .
126     .      .      .      .      WSF8
      .      .      .      .      .
      .      .      .      .      .
133     .      .      .      .      .      OFS6
      .      .      .      .      .      .
      .      .      .      .      .      .
140     .      .      .      .      .      .      OFS7

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 21MAY21 TIME 10:07:32 *
*

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*

```

City of Scottsdale
WESTWORLD MUSF - WestWorld Multi-Use Sports Fields
100 YEAR
24 Hour Storm
Unit Hydrograph: Clark
05/21/2021

8 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 2 MINUTES IN COMPUTATION INTERVAL
 IDATE 1JAN99 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 1220 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 2JAN99 ENDING DATE
 NDTIME 1638 ENDING TIME
 ICENT 19 CENTURY MARK

 COMPUTATION INTERVAL .03 HOURS
 TOTAL TIME BASE 40.63 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

-----DSS---ZOPEN: Existing File Opened, File: 100YR 24HR DESIGN MODEL.DSS
 Unit: 71; DSS Version: 6-JG

-----DSS---ZWRITE Unit 71; Vers. 2: /OFS5/BASIN/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /OFS5/BASIN/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /OFS5/BASIN/FLOW/02JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /WSF6/BASIN/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /WSF6/BASIN/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /WSF6/BASIN/FLOW/02JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /CSD1/COMBINE/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /CSD1/COMBINE/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /CSD1/COMBINE/FLOW/02JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /OFS4/BASIN/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /OFS4/BASIN/FLOW/01JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /OFS4/BASIN/FLOW/02JAN1999/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /WSF5/BASIN/FLOW/31DEC1998/2MIN/CALC/
-----DSS---ZWRITE Unit 71; Vers. 2: /WSF5/BASIN/FLOW/01JAN1999/2MIN/CALC/

+		CSD6	21.	12.03	2.	1.	0.	.02
	+	HYDROGRAPH AT						
		OFS1	13.	12.10	1.	0.	0.	.01
	+	HYDROGRAPH AT						
		WSF3	2.	12.03	0.	0.	0.	.00
	+	HYDROGRAPH AT						
		WSF7	4.	12.03	0.	0.	0.	.00
	+	HYDROGRAPH AT						
		WSF8	4.	12.00	0.	0.	0.	.00
	+	HYDROGRAPH AT						
		OFS6	7.	12.00	1.	0.	0.	.00
	+	HYDROGRAPH AT						
		OFS7	1.	12.10	0.	0.	0.	.00

*** NORMAL END OF HEC-1 ***

-----DSS---ZCLOSE Unit: 71, File: 100YR 24HR DESIGN MODEL.DSS
 Pointer Utilization: .28
 Number of Records: 66
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Appendix D: Storm Drain and Culvert Design Hydraulic Analysis

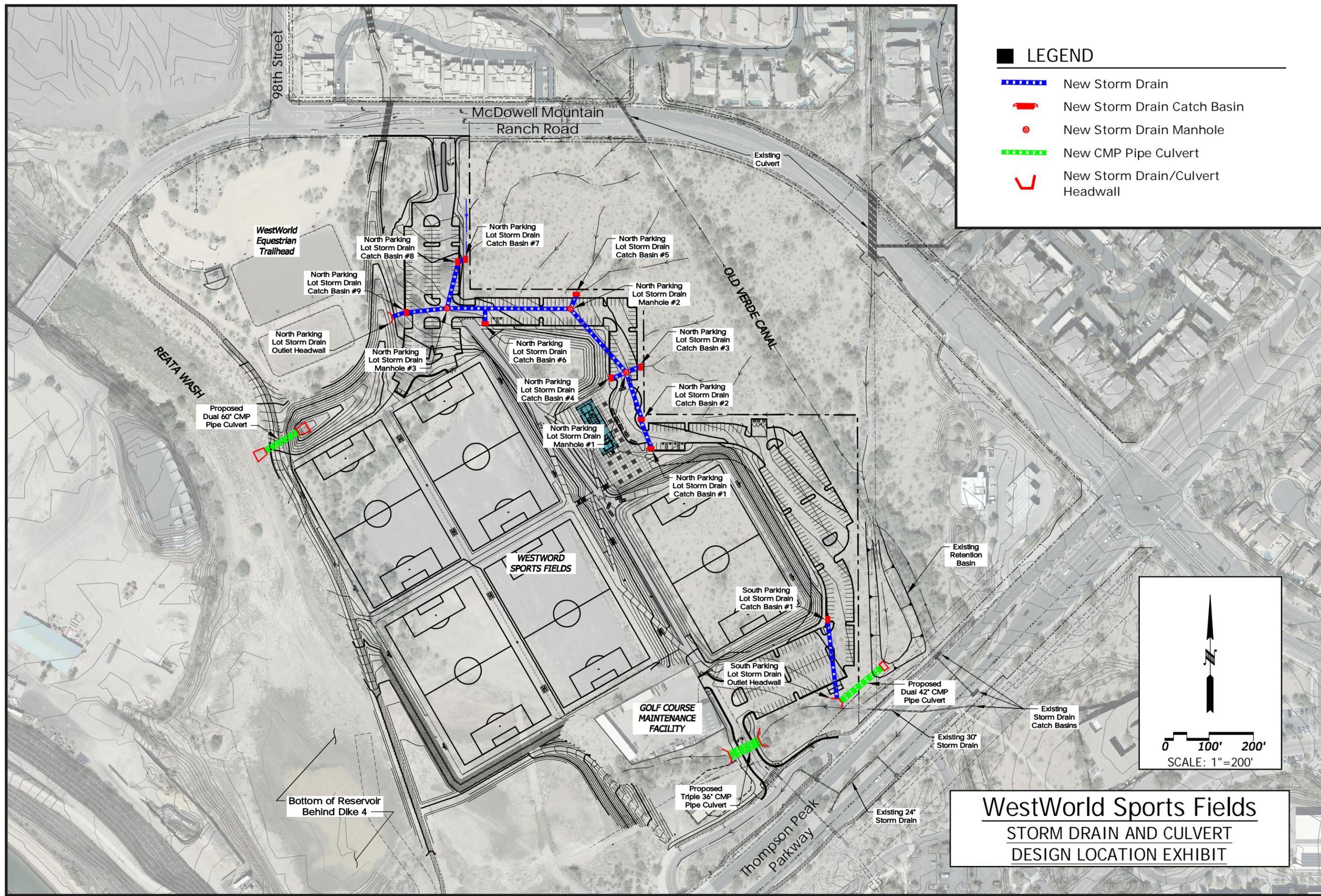
Project :

**WESTWORLD
 SPORTS FIELDS**
 CITY OF SCOTTSDALE
 PROJECT NUMBER: PG09

Submittal :
 G&B No. 2101
 Issue Date: 04/21
 Drawn By: OK
 Checked By: MITG

Sheet Title :
**STORM DRAIN
 & CULVERT
 DESIGN
 LOCATION
 EXHIBIT**

Sheet Number:
1
 1 of 1



LEGEND

- - - - - New Storm Drain
- ▭ New Storm Drain Catch Basin
- New Storm Drain Manhole
- - - - - New CMP Pipe Culvert
- ∩ New Storm Drain/Culvert Headwall

WestWorld Sports Fields
 STORM DRAIN AND CULVERT
 DESIGN LOCATION EXHIBIT

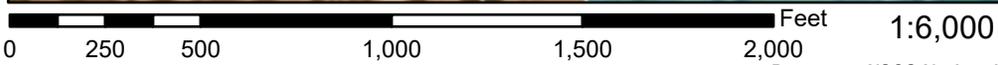
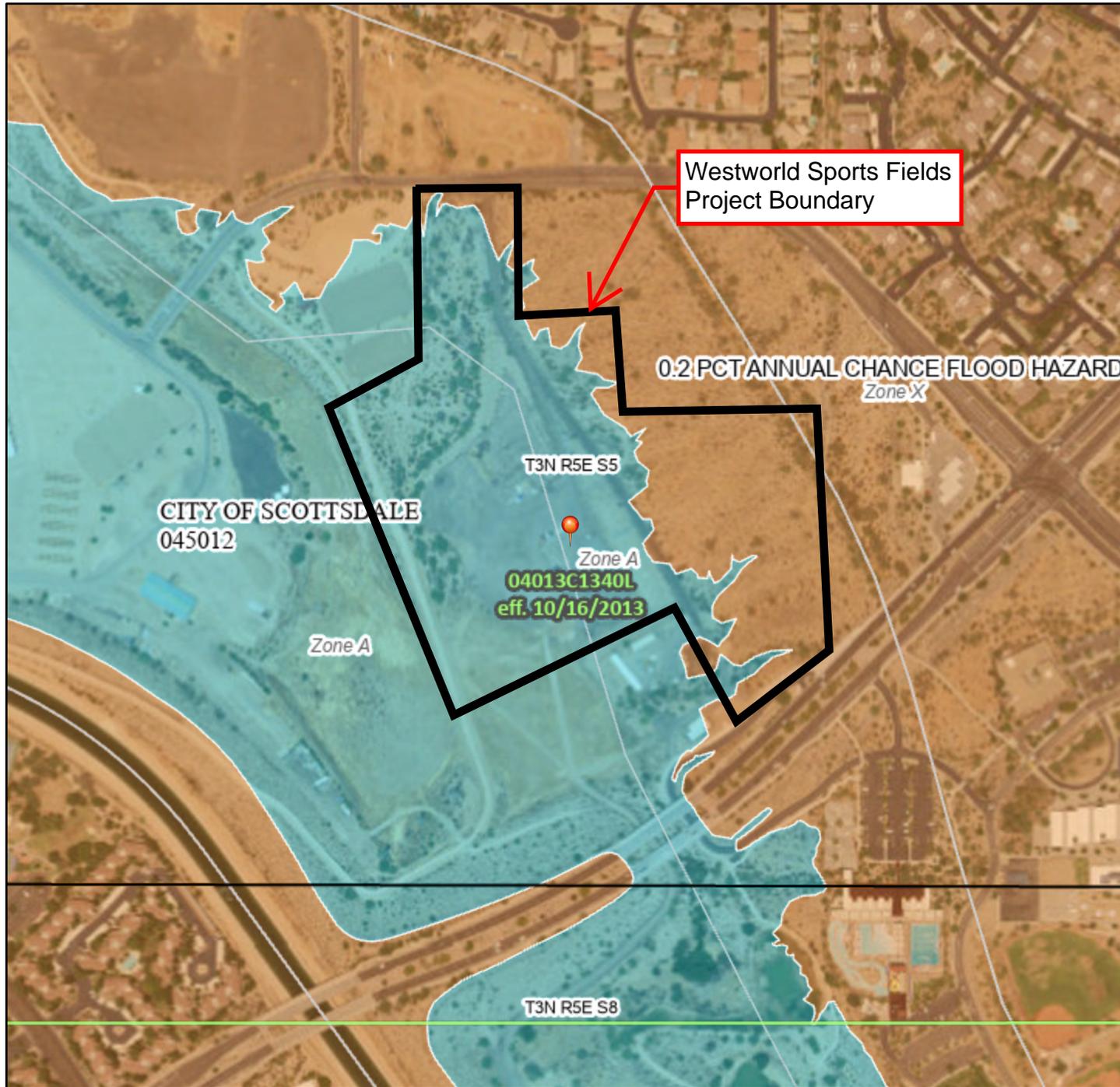
Preliminary sizing calculations of the proposed storm drain and culvert infrastructure has been done. Documentation of the final design calculations will be included in the subsequent Drainage Report Submittal.

Appendix E: FEMA FIRMette

National Flood Hazard Layer FIRMette



111°52'22"W 33°37'58"N



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS	Without Base Flood Elevation (BFE) <i>Zone A, V, A99</i>	With BFE or Depth <i>Zone AE, AO, AH, VE, AR</i>
	Regulatory Floodway	

		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile <i>Zone X</i>
		Future Conditions 1% Annual Chance Flood Hazard <i>Zone X</i>
		Area with Reduced Flood Risk due to Levee. See Notes. <i>Zone X</i>
		Area with Flood Risk due to Levee <i>Zone D</i>

		NO SCREEN Area of Minimal Flood Hazard <i>Zone X</i>
		Effective LOMRs
		Area of Undetermined Flood Hazard <i>Zone D</i>

		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall

		20.2 Cross Sections with 1% Annual Chance Water Surface Elevation
		17.5 Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature

		Digital Data Available
		No Digital Data Available
		Unmapped

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

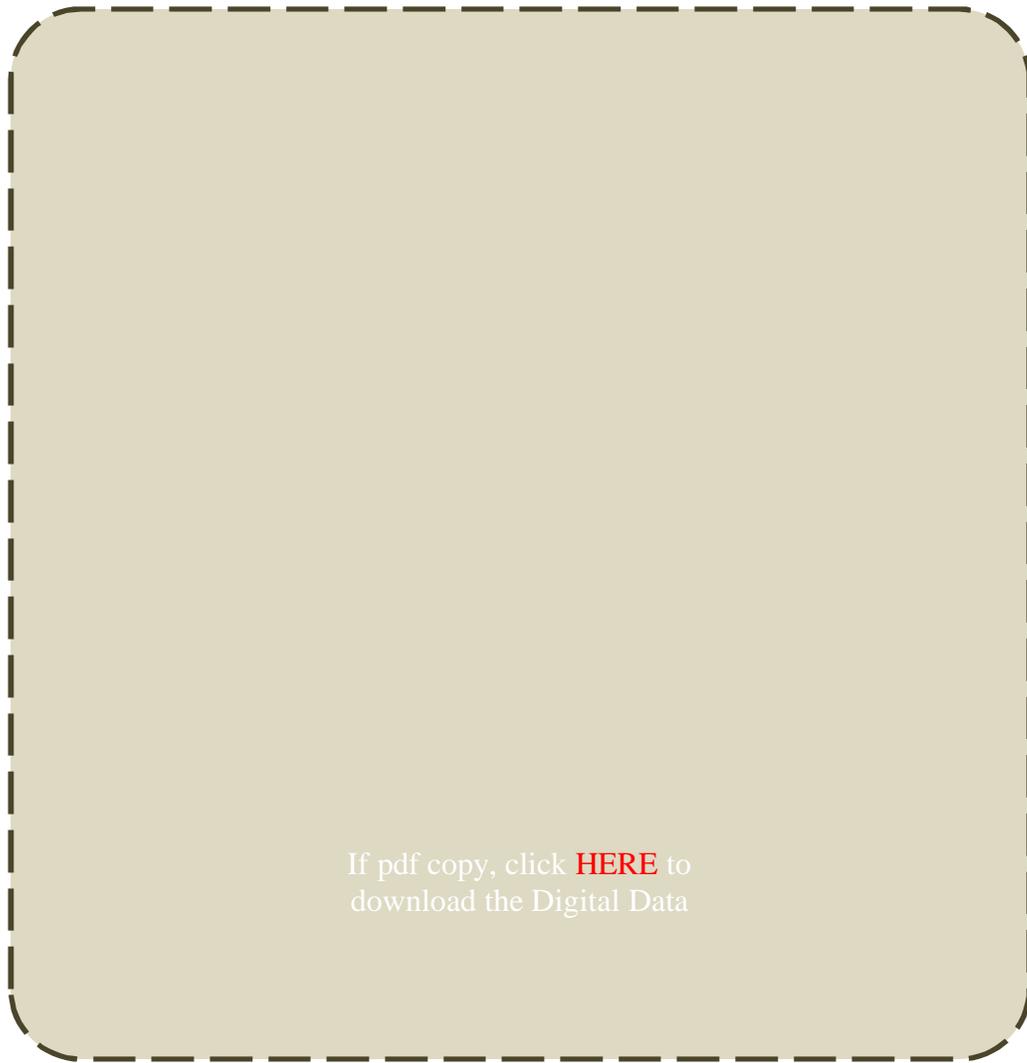
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The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 5/28/2021 at 5:12 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



Appendix F: Digital Data



If pdf copy, click [HERE](#) to
download the Digital Data

[Digital Data CD]