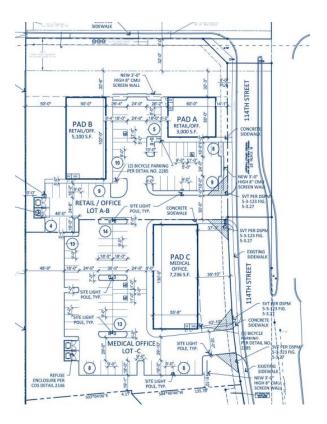


Traffic Impact & Mitigation Analysis



Prepared for:



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ROTESSIONALE NOINE

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Project Number: 20.5093 May 8, 2020



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1. INTRODUCTION AND EXECUTIVE SUMMARY

1.1. PURPOSE OF REPORT AND STUDY OBJECTIVES

Lōkahi, LLC (Lōkahi) was retained by Hawkins Companies to complete a Traffic Impact & Mitigation Analysis for the proposed 114th Street and Shea Boulevard development. The development is located at the southwest corner of 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard in Scottsdale, Arizona.

The objective of this Traffic Impact & Mitigation Analysis is to analyze the traffic related impacts of the proposed development to the adjacent roadway network. See **Figure 1** for the vicinity map.

The proposed site will be comprised of three (3) buildings, totaling approximately 15,336 square feet (SF) of leasable area. Approximately 8,100 square feet is anticipated to be utilized for retail use, while the remaining 7,236 square feet is anticipated for medical or general office use.

1.2. EXECUTIVE SUMMARY

This report presents the analyses and the results of a traffic study prepared for the proposed 114th Street and Shea Boulevard development that will be located on the southwest corner of 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard. The proposed development will be comprised of 8,100 square feet (SF) of retail space and 7,236 square feet of medical/general office space.

This Traffic Impact and Mitigation Analysis includes:

- Level of service analysis of existing conditions for the weekday AM and PM peak hours
- Crash Analysis
- Trip Generation for the existing and proposed development
- Trip Generation for the prior approved development and the proposed development
- Trip Generation comparison for the previously approved development with the proposed development
- Level of service analysis for the <u>no build</u> and <u>build</u> capacity analysis with the buildout of development (Year 2021) weekday AM and PM peak hours

The following are the three (3) intersections included in this study:

- 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1)
- 114th Street and Driveway B/Mirage Crossing Driveway (2)
- 114th Street and Driveway A (3)





Existing Capacity Analysis

The AM and PM peak hour existing conditions capacity analysis were completed for the two (2) existing study intersections. All movements operate at a LOS D or better with the exception of the following intersection:

114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1) – Signalized

- EB left AM and PM peak hours operate at LOS E
- NB left AM and PM peak hours operate at LOS E
- NB shared through-right AM and PM peak hours operate at LOS E
- SB left AM and PM peak hours operate at LOS F, and LOS E, respectively

Trip Generation

Previously Approved Development

In 2007 there was a prior site plan that was approved for the proposed site. The prior site included a 5,250 (SF) bank and a 5,200 (SF) office building.

Trip Generation – Prior Approved Development

| Land Use | ITE | Qty | Unit | Weekday | AM Peak Hour | | | PM Peak Hour | | |
|-------------------------|------|------|----------------|---------|--------------|----|-----|--------------|----|-----|
| Land Ose | Code | | | Total | Total | ln | Out | Total | ln | Out |
| Drive-in Bank | 912 | 5.25 | 1000 SF GFA | 525 | 50 | 29 | 21 | 107 | 54 | 53 |
| General Office Building | 710 | 5.2 | 1000 SF GFA | 60 | 31 | 27 | 4 | 7 | 1 | 6 |
| Total | | | | | 81 | 56 | 25 | 114 | 55 | 59 |

The previously approved development is anticipated to generate 585 weekday trips, with 81 trips occurring during the AM peak hour and 114 trips during the PM peak hour.

Proposed Development

The proposed development will be comprised of 8,100 square feet (SF) of retail space and 7,236 square feet of medical/general office space. For the purposes of this report, the medical/general office building is assumed to be occupied entirely by medical office uses.

Trip Generation – Proposed Development

| Land Use | ITE | Qty | Unit | Weekday | AM Peak Hour | | | PM Peak Hour | | |
|--------------------------------|------|-------|----------------|---------|--------------|----|-----|--------------|----|-----|
| Land OSC | Code | | | Total | Total | ln | Out | Total | ln | Out |
| Shopping Center | 820 | 8.1 | 1000 SF GLA | 306 | 8 | 5 | 3 | 31 | 15 | 16 |
| Medical-Dental Office Building | 720 | 7.236 | 1000 SF GFA | 190 | 22 | 17 | 5 | 27 | 8 | 19 |
| | | | Total | 496 | 30 | 22 | 8 | 58 | 23 | 35 |





The proposed development is anticipated to generate 496 weekday trips, with 30 trips occurring during the AM peak hour and 58 trips during the PM peak hour.

Trip Generation Comparison

The build out of the proposed development requires a minor general plan amendment. A comparison between the trips generated by the prior approved development and the proposed development was calculated.

Trip Generation – Comparison

| Land Use | ITE Qty | Unit | Weekday | y AM Peak Hour | | | | PM Peak Hour | | |
|--------------------------------|---------|---------|----------------|----------------|-------|-----|-----|--------------|----|-----|
| Land OSC | Code | Qty | Qty Offic | Total | Total | ln | Out | Total | In | Out |
| Drive-in Bank | 912 | 5.25 | 1000 SF GFA | 525 | 50 | 29 | 21 | 107 | 54 | 53 |
| General Office Building | 710 | 5.2 | 1000 SF GFA | 60 | 31 | 27 | 4 | 7 | 1 | 6 |
| Previously App | roved D | evelopn | nent - Total | 585 | 81 | 56 | 25 | 114 | 55 | 59 |
| Shopping Center | 820 | 8.1 | 1000 SF GLA | 306 | 8 | 5 | 3 | 31 | 15 | 16 |
| Medical-Dental Office Building | 720 | 7.236 | 1000 SF GFA | 190 | 22 | 17 | 5 | 27 | 8 | 19 |
| Proposed Development - Total | | | | | 30 | 22 | 8 | 58 | 23 | 35 |
| | -89 | -51 | -34 | -17 | -56 | -32 | -24 | | | |

The build out of the proposed development is anticipated to generate 89 fewer weekday trips, 51 fewer trips during the AM peak hour, and 56 fewer trips during the PM peak hour, when compared with the prior approved development.

Year 2021

Year 2021 analyses was completed <u>with</u> and <u>without</u> the build out of the proposed development. An annual growth rate of 1.2% was applied to the existing traffic volumes.

A capacity analysis was completed for both the AM and PM peak hours for year 2021, <u>with</u> and <u>without</u> the build out of the proposed development. All movements are maintained at the existing level of service <u>without</u> the build out of the proposed development.

All movements operate at a LOS D or better or are maintained at the no build level of service with the build out of the proposed development.

Recommendations

The recommendations with the build out of the proposed development include:

• 114th Street and Driveway B (2)
Buildout of full access driveway, approximately 350 feet south of Shea Boulevard



3 5-GP-2020 5/22/2020





• 114th Street and Driveway A (3)
Buildout of right-in/right-out driveway, approximately 215 feet south of Shea Boulevard

As with any new development and potential change in traffic patterns, the following is recommended:

• Monitor and Adjust Signal Timing

Monitor traffic patterns in the area and if necessary, adjust nearby signal timing





2. PROPOSED DEVELOPMENT

The study area is located in the City of Scottsdale, Arizona. The proposed development will be located on the southwest corner of 114th Street and Shea Boulevard. See **Figure 1** for a vicinity map.

The proposed site will be comprised of three (3) buildings, totaling approximately 15,336 square feet (SF) of leasable area. Approximately 8,100 square feet is anticipated to be utilized for retail use, while the remaining 7,236 square feet is anticipated for medical or general office use.

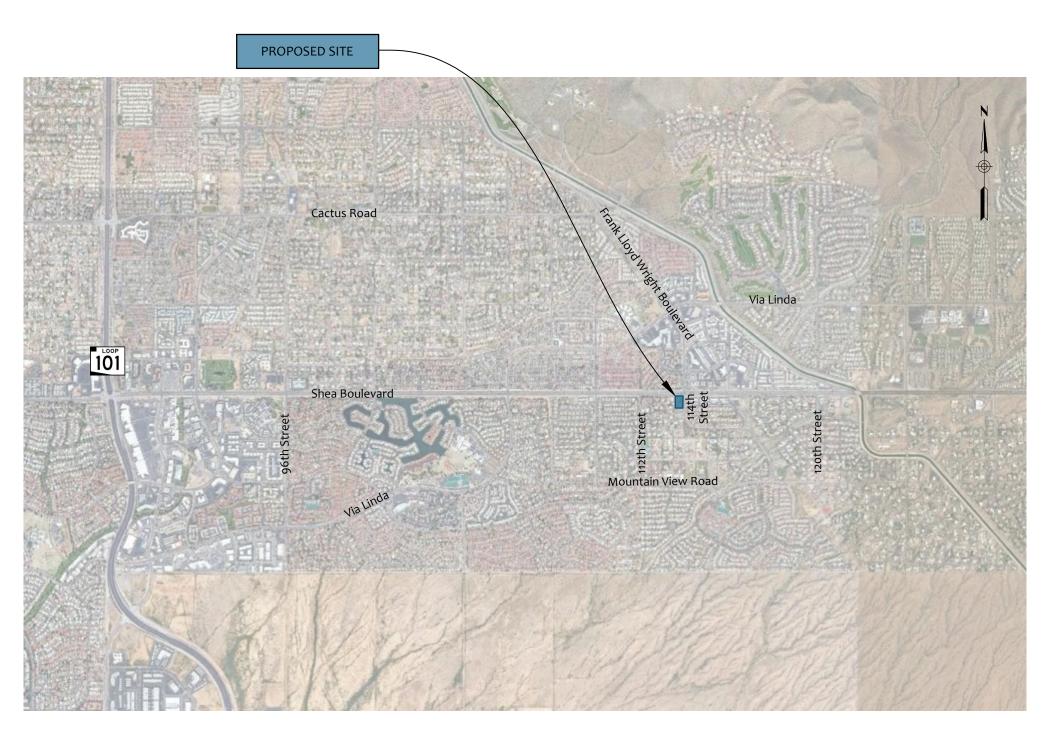
There are two (2) proposed access points to the development along 114th Street evaluated in this study.

114th Street and Driveway B/Mirage Crossing Driveway (2) is located along 114th Street, approximately 350 feet south of Shea Boulevard. This will be a full access driveway, allowing all movements into and out of the site. The buildout of this driveway will form the western leg of the existing 114th Street and Mirage Crossing Driveway intersection.

114th Street and Driveway A (3) is located along 114th Street approximately 215 feet south of Shea Boulevard. This will be a right-in/right-out driveway.

See Figure 2 and Appendix A for the proposed site plan.





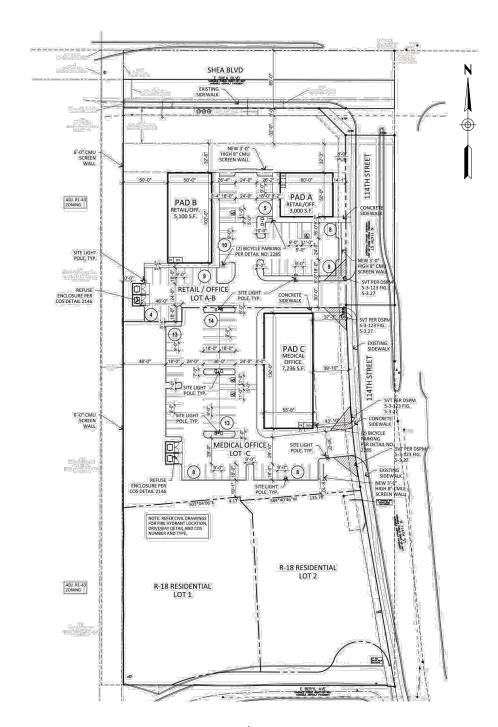


FIGURE 2 | SITE PLAN



3. AREA CONDITIONS

The study area is located in the City of Scottsdale, Arizona. **Sections 3.1** and **3.2** provide detailed descriptions of the study roadway segments and intersections. See **Figure 3** for study area.

3.1. STUDY ROADWAY SEGMENTS

Shea Boulevard, bordering the proposed development to the north, runs east-west and provides three (3) through lanes for each direction of travel, with a raised landscaped median. There is a posted speed limit of 50 miles per hour (mph), within the vicinity area. The City of Scottsdale classifies Shea Boulevard as a major arterial, according to the City of Scottsdale Transportation Master Plan, dated July 5, 2016. The City of Scottsdale's 2018 Average Daily Segment Traffic Volumes map reports an ADT of 39,700 vehicles per day, between Via Linda and 114th Street, and an ADT of 44,000 vehicles per day, between 114th Street/Frank Lloyd Wright Boulevard and 124th Street.

114th Street is generally a north-south roadway, bordering the proposed development to the east. South of Shea Boulevard, 114th Street provides one (1) through lane for northbound travel and approximately 300 feet of two (2) through lanes that taper into one (1) for southbound travel. North of Shea Boulevard is 114th Street becomes Frank Lloyd Wright Boulevard and provides two (2) through lanes in each direction of travel, with a raised landscaped median. Approximately 800 feet south of Shea Boulevard, 114th Street becomes Cochise Drive. There is a posted speed limit of 30 mph south of Shea Boulevard and a posted speed limit of 40 mph north of Shea Boulevard. According to the City of Scottsdale Transportation Master Plan, dated July 5, 2016, 114th Street is classified as a minor collector, while Frank Lloyd Wright Boulevard is classified as a minor arterial. The City of Scottsdale's 2018 Average Daily Segment Traffic Volumes map reports an ADT of 20,500 vehicles per day, north of Shea Boulevard, and an ADT of 2,800 vehicles per day, south of Shea Boulevard.

3.2. STUDY INTERSECTIONS

114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1) currently operates as a signalized intersection. The northbound approach provides two (2) dedicated left turn lanes and one (1) shared through-right turn lane. The southbound approach provides two (2) dedicated left turn lanes, two (2) through lanes, and one (1) dedicated right turn lane. The eastbound approach provides two (2) dedicated left turn lanes, two (2) through lanes, and one (1) shared through-right turn lane. The westbound approach provides one (1) dedicated left turn lane, three (3) through lanes, and one (1) channelized right turn lane.

114th Street and Driveway B/Mirage Crossing Driveway (2) currently operates as a one-way stop-controlled T-intersection, with stop control on the westbound approach. The northbound approach provides one (1) shared through-right turn lane. The southbound approach provides one (1)



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dedicated left turn lane and two (2) through lanes. The westbound approach provides one (1) shared left-right turn lane.





Legend



Intersection



3.3. SURROUNDING AREA LAND USE

The approximate 3.8-acre site is bordered by Shea Boulevard to the north, and 114th Street to the east. Residential developments border the western and southern edges of the proposed development. Additionally, offices and commercial developments are located to the east, across 114th Street, as well as on the northeast corner of 114th Street and Shea Boulevard (1).

3.4. SITE ACCESSIBILITY

Roadway System

The study area is located in the City of Scottsdale, Arizona. Arizona State Route 101 is located approximately three and one-quarter mile west of the proposed development. Scottsdale's street network is generally built as a one-mile grid system. Within the near vicinity of the proposed site there is a well-developed roadway network.

Pedestrian Facilities

There are continuous sidewalks provided adjacent to 114th Street and Shea Boulevard, with the exception of an approximate 900 feet segment along the south side of Shea Boulevard, between 112th Street and 114th Street, where no sidewalk is currently provided. Marked crosswalks are provided at all four (4) legs of the signalized intersection of 114th Street and Shea Boulevard (1).

Bicycle Facilities, Trails and Pathways

Bike lanes are currently provided along 114th Street, south of Shea Boulevard in both directions of travel.

Transit Facilities

Valley Metro Express Route 514 operates along Shea Boulevard within the study area. This route connects Fountain Hills to Downtown Phoenix. According to the Valley Metro System Map, a stop is located at on the northeast corner of the intersection of 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1). This route operates Monday – Friday.

In addition, the City of Scottsdale provides five trolley routes. The Mustang Route (MSTG) circulates around the north Scottsdale area, including along Via Linda and Frank Lloyd Wright Boulevard. A trolley stop is located approximately one-half mile north of the proposed development at the intersection of Via Linda and Frank Lloyd Wright Boulevard. This trolley route operates every 20 minutes between 4:42 am and 11:15 pm Monday – Friday and 5:13 am – 10:45 pm Saturday and Sunday. See **Figure 4.**





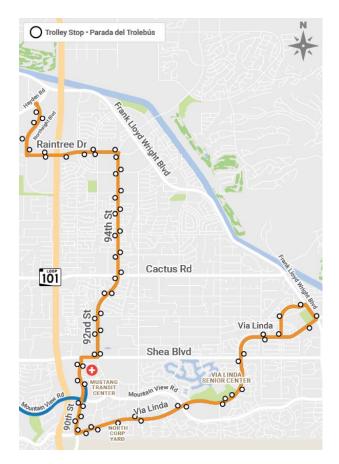


Figure 4 – City of Scottsdale Trolley Mustang Route





3.5. COLLISION HISTORY

The most recent 3-year collision history, from March 2017 to February 2020, was obtained from the City of Scottsdale. See **Appendix B** for collision data. The data included the following intersection and segment:

- 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1)
- Shea Boulevard, between 112th Street and 116th Street

114th Street and Shea Boulevard (1)

During the three-year period, there were a total of seven (7) crashes, of which four (4) were reported as no injury, and three (3) were either not reported or unknown. There was a total of five (5) rear end, one (1) sideswipe same direction, and one (1) single vehicle crashes.

Of the seven (7) reported collisions, four (4) collisions involved vehicles speeding too fast for roadway conditions and colliding with a vehicle slowing or stopping in the roadway, one (1) involved an improper turn of a vehicle, and 2 collisions involved unknown violations.

Shea Boulevard, between 112th Street and 116th Street

During the three-year period, there were a total of four (4) crashes, of which one (1) reported possible injury, one (1) reported no injury, and two (2) were either not reported or unknown. All four (4) were reported as rear end crashes.

Of the four (4) reported collisions, all involved vehicles that were speeding too fast for roadway conditions and colliding with a vehicle slowing or stopping in the roadway.

3.6. COLLISION RATES

The City of Scottsdale's 2018 Traffic Volume and Collision Rate Data report provides collision rate and traffic volume information on major roadway segments and at major intersections within the City. Segment collisions are collisions that occur on a major street more than 100 feet from the major intersections that define the segment, including at minor intersections within the segment. Intersection collisions are collisions that occur at or within 100 feet of a major intersection.

The collision rates and city-wide rankings for the study roadway segments are shown in **Table 1.** The collision rates and city-wide rankings for the study intersections are shown in **Table 2.**





Table 1 – Collision Rates – Study Roadway Segments

| Segment | From | То | Collision Rate | Rank |
|---------------------------------------|------------------------------|------------------------------|----------------|------|
| Shea Boulevard | Via Linda | Frank Lloyd Wright Boulevard | 0.45 | 237 |
| Shea Boulevard | Frank Lloyd Wright Boulevard | 124th Street | 0.45 | 239 |
| 114th/117th Street Mountain View Road | | Shea Boulevard | 0.0 | 307 |
| 2018 City (| 1.53 | | | |

Table 2 – Collision Rates – Study Intersections

| Intersection | Collision Rate | Rank |
|--|----------------|------|
| Frank Lloyd Wright and Shea | 0.78 | 54 |
| 2018 Average Intersection Collision Rate | 0.58 | |





4. EXISTING CONDITIONS

4.1. EXISTING LAND USE

The approximate 3.8-acre site is currently undeveloped land and comprised of four (4) parcels. The existing zoning consists of approximately 1.50-acres of Commercial Office (C-o) and approximately 2.57-acres of Single-Family Residential (R1-18). Commercial Office (C-o) zoning accommodates the development of office and related uses adjacent to commercial areas. Single-Family Residential (R1-18) zoning accommodates the development of individual homes on lot sizes such that a low-density population is maintained.

See **Appendix C** for detailed parcel information.

4.2. EXISTING TRAFFIC COUNTS

A local data collection firm, Field Data Services of Arizona, Inc., was utilized to collect traffic counts. On Wednesday, March 4, 2020 turning movement counts were obtained from 7:00 to 9:00 am and from 4:00 to 6:00 pm at the following intersections:

- 114th Street and Shea Boulevard (1)
- 114th Street and Driveway B/Mirage Crossing Driveway (2)

Additionally, on Wednesday, March 4, 2020, bi-directional tube counts for 24-hours in 15-minute intervals were collected along the following two (2) roadway segments:

- Shea Boulevard, west of 114th Street
- 114th Street, south of Shea Boulevard

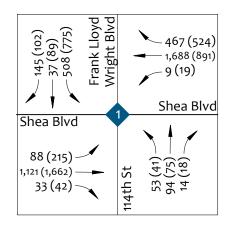
The turning movement counts were analyzed for the highest 1-hour within each time period. These are the peak hours that were analyzed throughout this study.

AM Peak Hour 7:00 am – 8:00 am
 PM Peak Hour 4:30 pm – 5:30 pm

The City of Scottsdale seasonal adjustment factors were used to adjust the traffic counts. The traffic volumes were adjusted based on the month the counts were taken. See **Appendix D** for detailed count data. See **Figure 5** for the existing adjusted AM and PM peak hour weekday traffic volumes.







Legend

AM(PM)

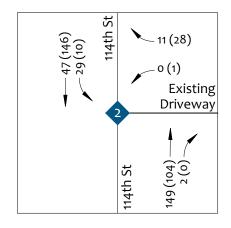
Peak Hour Traffic Volumes



Intersection

<ADT>

Average Daily Traffic





4.3. EXISTING CAPACITY ANALYSIS

The existing conditions capacity analysis was completed for the existing study intersections. The capacity and level of service for the study area intersections were evaluated using the methodology presented in the 6th Edition of the Highway Capacity Manual. Traffic analysis software, Synchro Version 10.3, was used to perform the analyses using the existing Peak Hour Factor (PHF) obtained from the traffic counts. The existing signal timing was provided by the City of Scottsdale. See **Appendix E** for the existing signal timing.

Table 3 is from the 6th Edition of the Highway Capacity Manual Exhibit 20-2, which lists the Level of Service (LOS) thresholds for signalized and stop-controlled intersections.

| Level of Service | Control Delay pe | r Vehicle (s/veh) |
|------------------|--------------------------|----------------------------|
| Level of Service | Signalized Intersections | Unsignalized Intersections |
| Α | ≤ 10 | 0 - 10 |
| В | > 10-20 | > 10–15 |
| С | > 20-35 | > 15-25 |
| D | > 35-55 | > 25-35 |
| E | > 55-80 | > 35-50 |
| F | > 80 | > 50 |

Table 3 - Level of Service Criteria

The results of the capacity analyses reveal the following locations with an existing level of service (LOS) E or F:

114th Street and Shea Boulevard (1) - Signalized

- EB left AM and PM peak hours operate at LOS E
- NB left AM and PM peak hours operate at LOS E
- NB shared through-right AM and PM peak hours operate at LOS E
- SB left AM and PM peak hours operate at LOS F and E, respectively

The existing AM and PM peak hour level of service and delay for unsignalized and signalized intersections are shown in **Table 4**.

See **Figure 6** for the existing AM and PM peak hour capacity analysis. The detailed capacity analysis sheets can be found in **Appendix F.**



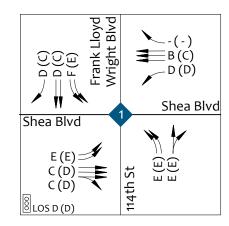


Table 4 – Existing Level of Service and Delay

| | Existing Conditions | | | | | |
|--|---------------------|-------------|--------------|-------|--|--|
| Intersection | AM I | PEAK | PM I | PEAK | | |
| | LOS | DELAY | LOS | DELAY | | |
| 114th Street/Frank Lloyd Wright Boulevard ar | id Shea Bo | oulevard (1 |) - Signaliz | ed | | |
| Oveall Intersection | D | 50.4 | D | 42.1 | | |
| Eastbound Left | Е | 62.0 | Е | 63.2 | | |
| Eastbound Through | C | 26.7 | D | 37.2 | | |
| Eastbound Shared Through-Right | C | 27.9 | D | 41.4 | | |
| Westbound Left | D | 41.3 | D | 51.5 | | |
| Westbound Thorugh | В | 18.9 | C | 26.6 | | |
| Westbound Right | - | - | - | - | | |
| Northbound Left | Е | 56.6 | Е | 56.2 | | |
| Northbound Shared Through-Right | E | 58.0 | Е | 66.5 | | |
| Southbound Left | F | 205.4 | Е | 59.7 | | |
| Southbound Through | D | 42.7 | C | 32.1 | | |
| Southbound Right | D | 47.6 | C | 33.6 | | |
| 114th Street and Mirage Crossing Driveway (2 | 2) - Unsign | alized | | | | |
| Westbound Shared Left-Right | Α | 9.2 | Α | 9.0 | | |
| Southbound Left | А | 7.6 | Α | 7.5 | | |







Legend

AM(PM)

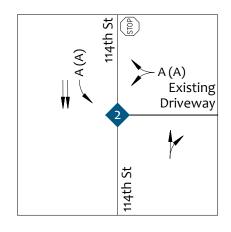
Peak Hour Capacity Analysis



Intersection

>

Lane Configuration





5. PROJECTED TRAFFIC

5.1. TRIP GENERATION

The trip generation for the proposed development was calculated utilizing the Institute of Institute of Transportation Engineers (ITE) publication entitled *Trip Generation*, 10th Edition. The ITE trip generation rates and fitted curve equations are based on studies that measure trip generation characteristics for various types of lane uses. The rates are expressed in terms of trips per unit of lane use type. This publication is the standard for the transportation engineering profession.

Previously Approved Development

In 2007 there was a prior site plan that was approved for the proposed site. See the site plan in **Appendix G.** The prior site included a 5,250 (SF) bank and a 5,200 (SF) office building. The trip generation for the prior approved development was calculated utilizing ITE Land Use 912 – Drive-In Bank and ITE Land Use Code 710 – General Office Building. Trips generated by the Drive-In Bank, with 5,250 (SF) and General Office Building, with 5,200 (SF) are shown in **Table 5** below. Detailed trip generation calculations are provided in **Appendix H.**

Table 5 – Trip Generation – Previously Approved Development

| Land Use | ITE | Qty | Qty Unit | Weekday | AN | 1 Peak H | our | PM Peak Hour | | |
|-------------------------|------|------|----------------|---------|-------|----------|-----|--------------|----|-----|
| Land Ose | Code | | | Total | Total | ln | Out | Total | ln | Out |
| Drive-in Bank | 912 | 5.25 | 1000 SF GFA | 525 | 50 | 29 | 21 | 107 | 54 | 53 |
| General Office Building | 710 | 5.2 | 1000 SF GFA | 60 | 31 | 27 | 4 | 7 | 1 | 6 |
| | | | Total | 585 | 81 | 56 | 25 | 114 | 55 | 59 |

The previously approved development is anticipated to generate 585 weekday trips, with 81 trips occurring during the AM peak hour and 114 trips during the PM peak hour.

Proposed Development

The proposed site will be comprised of three (3) buildings, totaling approximately 15,336 square feet (SF) of leasable area. Approximately 8,100 square feet is anticipated to be utilized for retail use, while the remaining 7,236 square feet is anticipated for medical or general office use.

Therefore, the trip generation for the proposed office development was calculated utilizing ITE Land Use 720 – Medical-Dental Office Building and Land Use 850 – Shopping Center. Trips generated by the three (3) proposed buildings, comprised of 15,336 (SF), are shown in **Table 6** below. Detailed trip generation calculations are provided in **Appendix H**.



20



| Table 6 – Trip G | Generation – Pro | posed Develo | pment |
|------------------|------------------|--------------|-------|
|------------------|------------------|--------------|-------|

| Land Use | ITE Qty | Unit | Weekday | AM Peak Hour | | | PM Peak Hour | | | |
|--------------------------------|---------|-------|----------------|--------------|-------|----|--------------|-------|----|-----|
| Land OSC | Code | Yty | acy Offic | Total | Total | In | Out | Total | ln | Out |
| Shopping Center | 820 | 8.1 | 1000 SF GLA | 306 | 8 | 5 | 3 | 31 | 15 | 16 |
| Medical-Dental Office Building | 720 | 7.236 | 1000 SF GFA | 190 | 22 | 17 | 5 | 27 | 8 | 19 |
| | | | Total | 496 | 30 | 22 | 8 | 58 | 23 | 35 |

The proposed development is anticipated to generate 496 weekday trips, with 30 trips occurring during the AM peak hour and 58 trips during the PM peak hour.

Trip Generation Comparison

The build out of the proposed development requires a minor general plan amendment. A trip generation comparison between the prior approved development and proposed development is shown in **Table 7** below, in order to demonstrate the trip generation difference.

Table 7 - Trip Generation - Comparison

| Land Use | ITE Code | Qty | Unit | Weekday | AM | Peak H | our | PM Peak Hour | | |
|--------------------------------|---|-------|----------------|---------|-------|--------|-----|--------------|-----|-----|
| | | | | Total | Total | ln | Out | Total | ln | Out |
| Drive-in Bank | 912 | 5.25 | 1000 SF GFA | 525 | 50 | 29 | 21 | 107 | 54 | 53 |
| General Office Building | 710 | 5.2 | 1000 SF GFA | 60 | 31 | 27 | 4 | 7 | 1 | 6 |
| Previously App | Previously Approved Development - Total | | | | 81 | 56 | 25 | 114 | 55 | 59 |
| Shopping Center | 820 | 8.1 | 1000 SF GLA | 306 | 8 | 5 | 3 | 31 | 15 | 16 |
| Medical-Dental Office Building | 720 | 7.236 | 1000 SF GFA | 190 | 22 | 17 | 5 | 27 | 8 | 19 |
| Proposed Development - Total | | | | 496 | 30 | 22 | 8 | 58 | 23 | 35 |
| Difference | | | | -89 | -51 | -34 | -17 | -56 | -32 | -24 |

The build out of the proposed development is anticipated to generate 89 fewer weekday trips, 51 fewer trips during the AM peak hour, and 56 fewer trips during the PM peak hour, when compared with the prior approved development.

5.2. TRIP DISTRIBUTION AND ASSIGNMENT

The trip distribution procedure determines the general pattern of travel for vehicles entering and leaving the proposed development. The trip distribution for the proposed development is based on the distribution of the existing traffic. The trip distribution is shown in **Figure 7.**



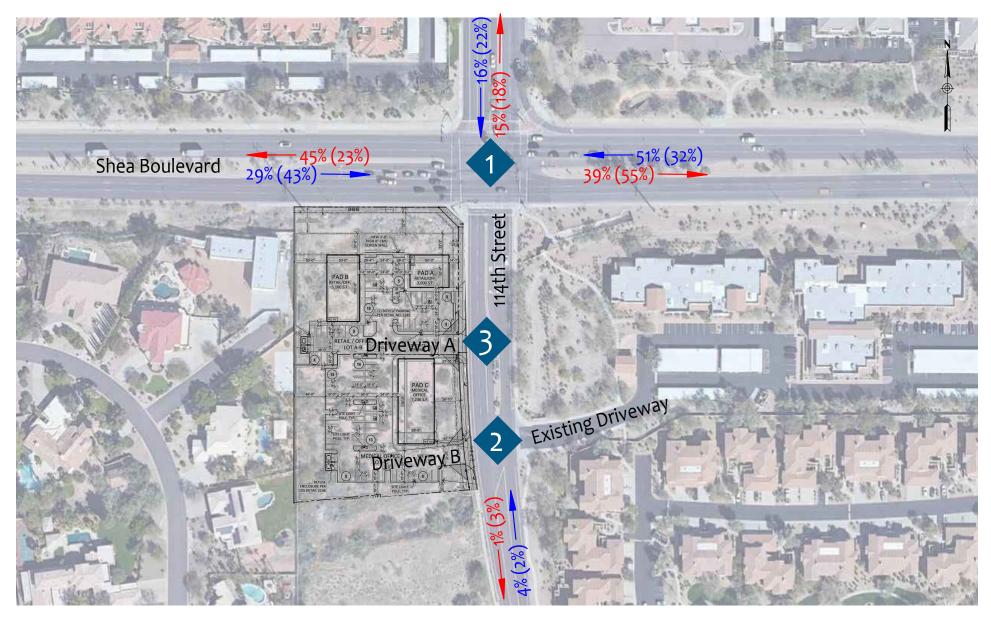
21





The trip assignment was generally based on proximity of the driveways, permitted turn movements, as well as ease and probability of use. The site generated traffic volumes are shown in **Figure 8**.





Legend

AM(PM) Inbound Trip Distribution Percentages

AM(PM) Outbound Trip Distribution Percentages





AM(PM)

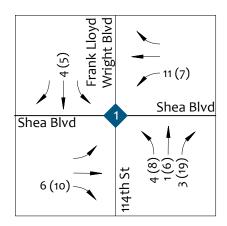
Peak Hour Traffic Volumes

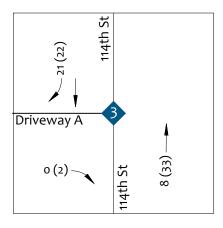


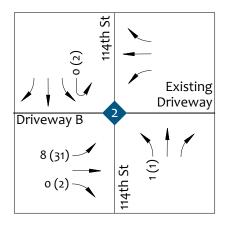
Intersection

<ADT>

Average Daily Traffic









6. FUTURE CONDITIONS (YEAR 2021 - OPENING YEAR)

The proposed 114th Street and Shea development is anticipated to be constructed and ready to open in the year 2021. This section analyzes the effects the proposed development will have on the surrounding roadway network during the opening year of 2021.

6.1. YEAR 2021 BACKGROUND TRAFFIC VOLUMES

According to the 2019 Maricopa Associations of Governments (MAG) socioeconomic projections in the City of Scottsdale within the study area, it is estimated that in the year 2018 the population will be approximately 20,900. MAG estimates that the 2020 population of the surrounding area to be 21,400. This results in an approximate annual growth rate of 1.19%.

A 1.2% annual growth rate was utilized. See **Appendix I** for the MAG socioeconomic projections.

See **Figure 9** for the year 2021 background traffic volumes, which includes a 1.2% annual growth applied to the existing traffic volumes.

6.2. YEAR 2021 BUILD TRAFFIC VOLUMES

When the site traffic (**Figure 8**) is added to the year 2021 background traffic (**Figure 9**), the result is the 2021 <u>build</u> traffic volumes. This represents the traffic volumes <u>with</u> the build out of the proposed development. The year 2021 <u>build</u> traffic volumes are shown in **Figure 10**.

6.3. YEAR 2021 NO BUILD CAPACITY ANALYSIS

The capacity and level of service for the study area intersections were evaluated for the year 2021 <u>no build</u> scenario. The corresponding traffic volumes are shown in **Figure 9**. The signal timing splits were optimized and adjusted for the future traffic volumes. PHF was assumed to be 0.92.

The detailed capacity analysis sheets can be found in **Appendix J.**

The 2021 <u>no build</u> AM and PM peak hour level of service and delay for unsignalized and signalized intersections are shown in **Table 8**.

The results of the 2021 <u>no build</u> capacity analysis level of service is shown in **Figure 11.** All movements operate at a LOS D or better or at the same level of service as the existing conditions.





6.4. YEAR 2021 BUILD CAPACITY ANALYSIS

The capacity and level of service for the study area intersections were evaluated for the year 2021 <u>build</u> traffic volumes. See **Figure 10**. The signal timing splits were optimized and adjusted for the future traffic volumes, and a PHF of 0.92 was used.

The capacity and level of service for the study area intersections were evaluated for the year 2021 build traffic volumes.

The results of the 2021 <u>build</u> capacity analysis level of service is shown in **Figure 12**. All movements operate at a LOS D or better or at the same level of service as the 2021 <u>no build</u> capacity analysis. The HCM 6th Edition does not give any level of service results for the u-turns that occur at the southbound left turn lane at the intersection of 114th Street and Mirage Crossing Driveway/Driveway B (2). The result for that movement were taken from the HCM 2000 analysis results.

The 2021 <u>build</u> AM and PM peak hour level of service and delay for unsignalized and signalized intersections are shown in **Table 8**.

See **Figure 12** year 2021 AM and PM peak hour capacity analysis. The detailed capacity analysis sheets can be found in **Appendix K**.



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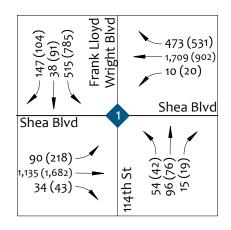
Table 8 – 2021 No Build and Build Level of Service and Delay

| | 2021 | Backgrou | ınd Condi | tions | 2021 Build Conditions | | | | | |
|--|-------------|----------|-----------|-------|-----------------------|-------|-----|-------|--|--|
| Intersection | AM I | PEAK | PM I | PEAK | AM | PEAK | PM | PEAK | | |
| | LOS | DELAY | LOS | DELAY | LOS | DELAY | LOS | DELAY | | |
| 114th Street/Frank Lloyd Wright Boulevard an | | | | | | | | | | |
| Oveall Intersection | D | 53.7 | D | 45.0 | D | 53.7 | D | 48.0 | | |
| Eastbound Left | Е | 62.9 | Е | 64.9 | Е | 62.9 | Е | 64.9 | | |
| Eastbound Through | C | 27.0 | D | 40.8 | C | 27.1 | D | 43.6 | | |
| Eastbound Right | C | 28.3 | D | 46.4 | C | 28.4 | D | 50.2 | | |
| Westbound Left | D | 41.6 | D | 53.6 | D | 42.3 | D | 54.1 | | |
| Westbound Thorugh | В | 19.7 | C | 29.1 | C | 20.0 | C | 30.0 | | |
| Westbound Right | 1 | - | 1 | - | 1 | - | 1 | - | | |
| Northbound Left | Е | 56.8 | E | 56.3 | E | 56.9 | E | 56.6 | | |
| Northbound Right | Е | 57.8 | E | 70.0 | E | 57.7 | E | 73.0 | | |
| Southbound Left | F | 225.7 | Е | 62.2 | F | 225.7 | Е | 68.5 | | |
| Southbound Through | D | 42.5 | C | 30.7 | D | 42.2 | C | 29.9 | | |
| Southbound Right | D | 47.6 | C | 32.2 | D | 47.2 | C | 31.4 | | |
| 114th Street and Mirage Crossing Driveway (2 | 2) - Unsign | alized | | | | | | | | |
| Eastbound Shared Left-Through-Right | 1 | - | 1 | - | В | 10.1 | В | 10.3 | | |
| Westbound Shared Left-Right | Α | 9.2 | Α | 9.0 | 1 | 1 | 1 | - | | |
| Westbound Shared Left-Through-Right | - | - | - | - | Α | 9.3 | Α | 9.1 | | |
| Northbound Left | - | - | - | - | Α | 7.3 | Α | 7.5 | | |
| Southbound Left* | А | 7.6 | Α | 7.5 | А | 7.6 | А | 7.5 | | |
| 114th Street and Driveway A (3) - Unsignalized | d | | | | | | | | | |
| Eastbound Right | - | - | - | - | Α | 0.0 | Α | 8.8 | | |

^{*}Results from HCM 2000, Not HCM 6th Edition







Legend

AM(PM)

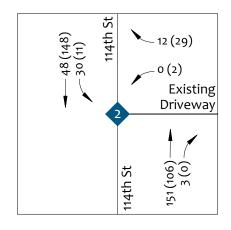
Peak Hour Traffic Volumes



Intersection

<ADT>

Average Daily Traffic







AM(PM)

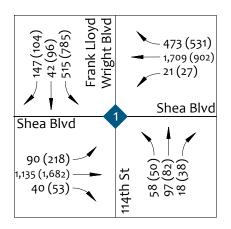
Peak Hour Traffic Volumes

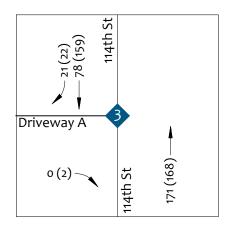


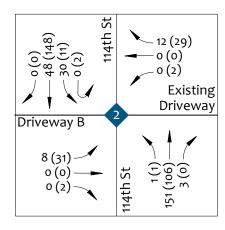
Intersection

<ADT>

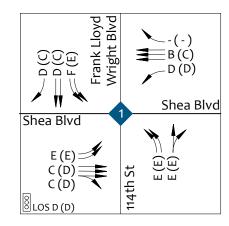
Average Daily Traffic











Legend

AM(PM)

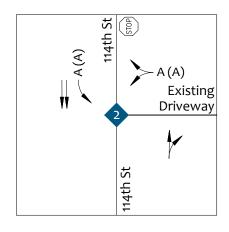
Peak Hour Capacity Analysis



Intersection

>

Lane Configuration







AM(PM)

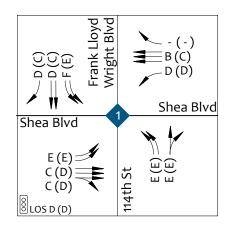
Peak Hour Capacity Analysis

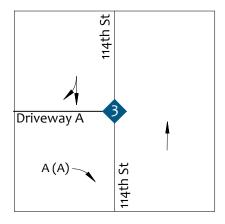


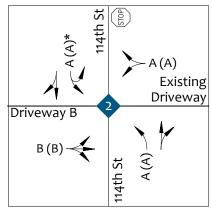
Intersection



Lane Configuration







*LOS from HCM 2000 Results



7. RECOMMENDATIONS & CONCLUSIONS

The proposed 114th Street and Shea Boulevard development will be located on the southwest corner of 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard, in Scottsdale, Arizona. The proposed site will be comprised of three (3) buildings, totaling approximately 15,336 square feet (SF) of leasable area. Approximately 8,100 square feet is anticipated to be utilized for retail use, while the remaining 7,236 square feet is anticipated for medical or general office use.

Existing Capacity Analysis

The AM and PM peak hour existing conditions capacity analysis were completed for the two (2) existing study intersections. All intersections currently operate with movements at a level of service D or better with the exception of the following intersection:

114th Street and Shea Boulevard (1) – Signalized

- EB left AM and PM peak hours operate at LOS E
- NB left AM and PM peak hours operate at LOS E
- NB shared through-right AM and PM peak hours operate at LOS E
- SB left AM and PM peak hours operate at LOS F, and LOS E, respectively

Trip Generation

Previously Approved Development

In 2007 there was a prior site plan that was approved for the proposed site. The prior site included a 5,250 (SF) bank and a 5,200 (SF) office building.

Trip Generation - Prior Approved Development

| Land Use | ITE | Qty | Unit | Weekday | AN | l Peak H | our | PM Peak Hour | | | |
|-------------------------|------|------|----------------|---------|-------|----------|-----|--------------|----|-----|--|
| | Code | | | Total | Total | ln | Out | Total | ln | Out | |
| Drive-in Bank | 912 | 5.25 | 1000 SF GFA | 525 | 50 | 29 | 21 | 107 | 54 | 53 | |
| General Office Building | 710 | 5.2 | 1000 SF GFA | 60 | 31 | 27 | 4 | 7 | 1 | 6 | |
| Total | | | | | 81 | 56 | 25 | 114 | 55 | 59 | |

The previously approved development is anticipated to generate 585 weekday trips, with 81 trips occurring during the AM peak hour and 114 trips during the PM peak hour.

Proposed Development

The proposed development will be comprised of 8,100 square feet (SF) of retail space and 7,236 square feet of medical/general office space. For the purposes of this report, the medical/general office building is assumed to be occupied entirely by medical office uses.





Trip Generation – Proposed Development

| Land Use | ITE | Qty | Unit | Weekday | AM Peak Hour | | | PM Peak Hour | | |
|--------------------------------|------|-------|----------------|---------|--------------|----|-----|--------------|----|-----|
| | Code | Ϋ́ιy | | Total | Total | ln | Out | Total | ln | Out |
| Shopping Center | 820 | 8.1 | 1000 SF | 306 | 8 | 5 | 3 | 31 | 15 | 16 |
| 2 | | | GLA | J | | , | , | ٠. | ., | |
| Medical-Dental Office Building | 720 | 7.236 | 1000 SF GFA | 190 | 22 | 17 | 5 | 27 | 8 | 19 |
| | | | Total | 496 | 30 | 22 | 8 | 58 | 23 | 35 |

The proposed development is anticipated to generate 496 weekday trips, with 30 trips occurring during the AM peak hour and 58 trips during the PM peak hour.

Trip Generation Comparison

The build out of the proposed development requires a minor general plan amendment. A comparison between the trips generated by the prior approved development and the proposed development was calculated.

Trip Generation – Comparison

| Land Use | ITE Code Qty | Otv | Unit | Weekday | AM Peak Hour | | | PM Peak Hour | | |
|--------------------------------|---|-------|----------------|---------|--------------|-----|-------|--------------|-----|-----|
| | | Offic | Total | Total | ln | Out | Total | ln | Out | |
| Drive-in Bank | 912 | 5.25 | 1000 SF GFA | 525 | 50 | 29 | 21 | 107 | 54 | 53 |
| General Office Building | 710 | 5.2 | 1000 SF GFA | 60 | 31 | 27 | 4 | 7 | 1 | 6 |
| Previously App | Previously Approved Development - Total | | | | 81 | 56 | 25 | 114 | 55 | 59 |
| Shopping Center | 820 | 8.1 | 1000 SF GLA | 306 | 8 | 5 | 3 | 31 | 15 | 16 |
| Medical-Dental Office Building | 720 | 7.236 | 1000 SF GFA | 190 | 22 | 17 | 5 | 27 | 8 | 19 |
| Proposed Development - Total | | | | 496 | 30 | 22 | 8 | 58 | 23 | 35 |
| Difference | | | | -89 | -51 | -34 | -17 | -56 | -32 | -24 |

The build out of the proposed development is anticipated to generate 89 fewer weekday trips, 51 fewer trips during the AM peak hour, and 56 fewer trips during the PM peak hour, when compared with the prior approved development.

Year 2021

Year 2021 analyses was completed <u>with</u> and <u>without</u> the build out of the proposed development. An annual growth rate of 1.2% was applied to the existing traffic volumes.







A capacity analysis was completed for both the AM and PM peak hours for year 2021, <u>with</u> and <u>without</u> the build out of the proposed development. All movements are maintained at the existing level of service <u>without</u> the build out of the proposed development.

All movements operate at a LOS D or better or are maintained at the no build level of service with the build out of the proposed development.

Recommendations

In summary and as included in the discussion and analyses throughout this report, the following are the recommended improvements:

- 114th Street and Driveway B (2)
 Buildout of full access driveway, approximately 350 feet south of Shea Boulevard
- 114th Street and Driveway A (3)
 Buildout of right-in/right-out driveway, approximately 215 feet south of Shea Boulevard

As with any new development and potential change in traffic patterns, the following is recommended:

Monitor and Adjust Signal Timing
 Monitor traffic patterns in the area and if necessary, adjust nearby signal timing

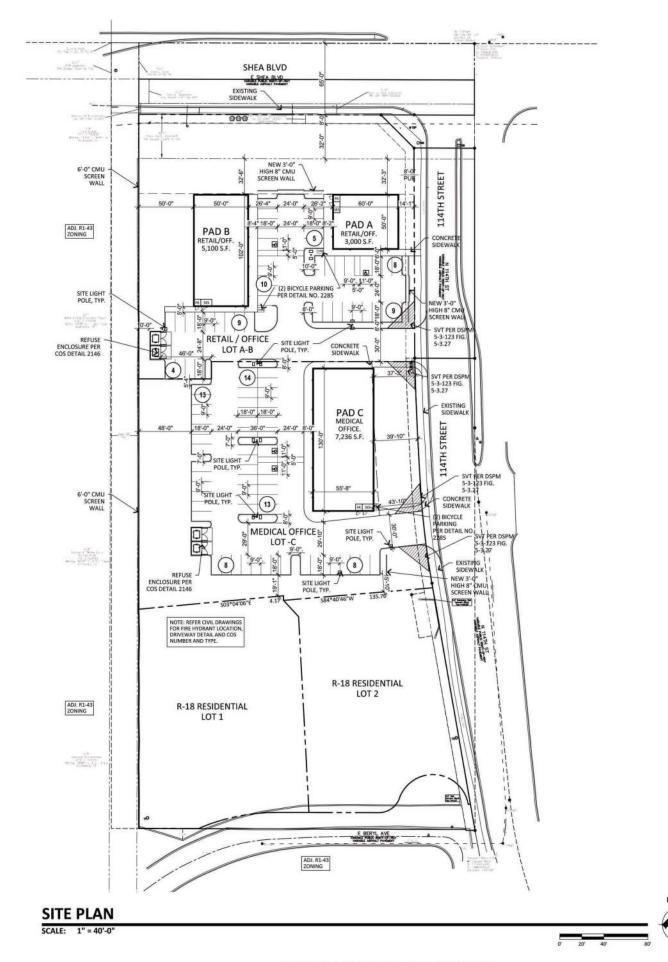




Appendix A – Proposed Site Plan







SWC OF 114TH STREET AND SHEA BLVD SCOTTSDALE ARIZONA 85259 DATE: 05-01-2020 (PRELIMINARY)













PROJECT DIRECTORY

ARCHITECT: RKAA ARCHITECTS, INC. 2233 EAST THOMAS ROAD PHOENIX, ARIZONA 85016 CONTACT: NEIL FEASER PHONE: (602) 955-3900 FAX: (602) 955-0496 E-MAIL: nfeaser@rkaa.com

SITE DATA

PARCEL NUMBER: 217-33-034G, 034F, 034M & 034K **EXISTING ZONING:** PROPOSED ZONING: C1 (PAD A & B), SR (PAD C) AND R-18 PROPOSED SITE AREA: 3.8077 ACRES (165,864.90 S.F.) MAX.BUILDING HEIGHT:

C-1 ZONING: 22'-0" MAX. 18'-0" MAX. PROPOSED BUILDING HEIGHT:

PROPOSED USE: RETAIL / MED. OFFICE / OFFICE

BUILDING AREA:

PAD A: RETAIL / OFFICE/MEDICAL 3,000 S.F. PAD B: RETAIL / OFFICE/MEDICAL 5,100 S.F. PAD C: MEDICAL OFFICE 7,236 S.F.

15,336 S.F. TOTAL BUILDING AREA:

PARKING REQUIRED:

12 SPACES PAD A: RETAIL-OFFICE / 250 S.F. 21 SPACES PAD B: RETAIL-OFFICE / 250 S.F. 29 SPACES PAD C: MEDICAL OFFICE / 250 S.F.

PARKING PROVIDED:

22 SPACES PAD A: RETAIL-OFFICE / 250 S.F. 23 SPACES PAD B: RETAIL-OFFICE / 250 S.F. 56 SPACES PAD C: MEDICAL OFFICE / 250 S.F.

62 SPACES TOTAL PARKING REQUIRED: 101 SPACES TOTAL PARKING PROVIDED:

05 SPACES BICYCLE SPACES REQUIRED: 06 SPACES BICYCLE SPACES PROVIDED:

PROPOSED LOT A-B AREA: 1.3032 ACRES (56,770.44 S.F.) PROPOSED LOT -C AREA: 1.2438 ACRES (54,182.40 S.F.) PROPOSED LOT 1 AREA: 0.6795 ACRES (29,599.48 S.F.) PROPOSED LOT 2 AREA: 0.5811 ACRES (25,312.58 S.F.)



VICINITY MAP SCALE: N.T.S.

THIS SITE PLAN HAS BEEN PREPARED WITHOUT THE AID OF A SURVEY. ALL PROPERTY BOUNDARIES ARE ROUGH APPROXIMATIONS. THIS DRAWING IS TO BE USED FOR CONCEPTUAL PURPOSES ONLY AND IT IS NOT TO BE THE BASIS FOR ANY LEGALLY

114TH AND SHEA RETAIL



Appendix B – Crash Data



B 5-GP-2020 5/22/2020

CITY OF SCOTTSDALE

COLLISION SUMMARY

| REPORT# | DATE YYMMDD | TIME HHMM | NORTH / SOUTH ST. | TYPE | EAST WEST ST. | TYPE | | DIST FROM | INJ. SE #1 # | | PHYS. COND. #1 #2 | | | ACTIC #1 # | | | MANNER OF COLLISION | COMMENTS |
|---------|----------------|--------------|-------------------|------|---------------|------|----|--------------|-----------------|---|----------------------|----|---|---------------|---|-------|---------------------|--------------------------|
| 1718446 | 170820 | 1103 | 114 | ST | SHEA | BL | AT | | 1 | 1 | 0 0 | 2 | 1 | 1 3 | 3 | EB EB | 4 | |
| 1725363 | 171115 | 2101 | 114 | ST | SHEA | BL | AT | | 99 | 1 | 99 0 | 99 | 1 | 1 3 | 3 | EB EB | 4 | MULTI VEH 3, HIT AND RUN |
| 1726933 | 171205 | 1823 | 114 | ST | SHEA | BL | AT | | 1 | 1 | 0 0 | 2 | 1 | 2 2 | 2 | WB WB | 4 | |
| 1803981 | 180219 | 1824 | 114 | ST | SHEA | BL | AT | | 1 | | 0 | 2 | | 4 | | WB | 1 | |
| 1816869 | 180801 | 1538 | 114 | ST | SHEA | BL | AT | | 99 | 1 | 99 0 | 99 | 1 | 99 2 | 2 | EB EB | 4 | HIT AND RUN |
| 1826669 | 181211 | 0635 | 114 | ST | SHEA | BL | N | 20 | 1 | 1 | 0 0 | 7 | 1 | 5 3 | 3 | SB SB | 6 | |
| 1922306 | 191025 | 1810 | 114 | ST | SHEA | BL | AT | | | | 0 0 | 2 | 1 | 1 3 | 3 | NB NB | 4 | |

KEY

INJURY SEVERITY:

1=NO INJURY, 2=POSSIBLE INJURY, 3=NON-INCAPACITATING INJURY, 4=INCAPACITATING INJURY, 5=FATAL INJURY, 99=NOT REPORTED / UNKNOWN

PHYSICAL CONDITION:

0=NO APPARENT INFLUENCE, 1=ILLNESS, 2=PHYSICAL IMPAIRMENT, 3=FELL ASLEEP / FATIGUED 4=ALCOHOL, 5=DRUGS, 6=MEDICATIONS, A=NO TEST GIVEN, B=TEST GIVEN, C=TEST REFUSED, D=TESTING UNKNOWN, 97=OTHER, 99=UNKNOWN

VIOLATION:

1=NO IMPROPER ACTION, 2=SPEED TOO FAST FOR CONDITIONS, 3=EXCEEDED LAWFUL SPEED 4=FOLLOWED TOO CLOSELY. 5=RAN STOP SIGN, 6=DISREGAREDED TRAFFIC SIGNAL7=MADE IMPROPER TURN, 8=DROVE/RODE IN OPPOSING TRAFFIC LANE, 9=KNOWINGLY OPERATED WITH FAULTY / MISSING EQUIPMENT, 10=REQUIRED MOTORCYCLE SAFETY EQUIPMENT NOT USED, 11=PASSED IN NO PASSING ZONE, 12=UNSAFE LANE CHANGE, 13=FAILED TO KEEP IN PROPER LANE, 14=DISREGARDED PAVEMENT MARKINGS, 15=OTHER UNSAFE PASSING, 16=INATTENTION/DISTRACTION, 17=DID NOT USE CROSSWALK, 18=WALKED ON WRONG SIDE OF ROAD, 19=ELECTRONIC COMMUNICATIONS DEVICE, 20=FAILED TO YIELD RIGHT OF WAY (added August 2014), 97=OTHER, 99 UNKNOWN

ACTION:

1=GOING STRAIGHT AHEAD, 2=SLOWING IN TRAFFICWAY, 3=STOPPED IN TRAFFICWAY, 4=MAKING LEFT TURN, 5=MAKING RIGHT TURN, 6=MAKING U-TURN, 7=OVERTAKING/PASSING, 8=CHANGING LANES, 9=NEGOTIATING A CURVE, 10=BACKING, 11=AVOIDING VEH/OBJ/PED/CYCLIST/ANIMAL, 12=ENTERING PARKING POSITION, 13=LEAVING PARKING POSITION, 14=PROPERLY PARKED, 15=IMPROPERLY PARKED, 16=DRIVERLESS MOVING VEHICLE, 17=CROSING ROAD, 18=WALKING WITH TRAFFIC, 19=WALKING AGAINST TRAFFIC, 20=STANDING, 21=LYING, 22=GETTING ON OR OFF VEHICLE, 23=WORKING ON/PUSHING VEHICLE, 24=WORKING ON ROAD, 97=OTHER, 99=UKNOWN

MANNER OF COLLISION:

1=SINGLE VEHICLE, 2=ANGLE (front to side, other than left turn), 3=LEFT TURN, 4=REAR END (front to rear), 5=HEAD-ON (front to front, other than left turn), 6=SIDESWIPE (same direction), 7=SIDESWIPE (opposite direction), 8=REAR-TO-SIDE, 9=REAR TO REAR, 97=OTHER, 99=UNKNOWN

TOTAL

7

CITY OF SCOTTSDALE

COLLISION SUMMARY

| REPORT # | DATE YYMMDD | TIME HHMM | NORTH / SOUTH ST. | TYPE | EAST WEST ST. | TYPE | | DIST FROM | | | 'S. COND. #2 | VIOL #1 | | ACTI #1 | | TRAV. DIR. #1 #2 | MANNER OF COLLISION | COMMENTS |
|----------|----------------|--------------|-------------------|------|---------------|------|---|--------------|-----|---|-----------------|------------|---|------------|---|---------------------|---------------------|----------|
| 2002222 | 200130 | 1709 | 112 | ST | SHEA | BL | Е | 240 | | 0 | 0 | 2 | 1 | 1 | 2 | EB EB | 4 | |
| 1804668 | 180301 | 1223 | 114 | ST | SHEA | BL | W | 104 | 1 1 | 0 | 0 | 2 | 1 | 1 | 3 | WB WB | 4 | |
| 1804672 | 180301 | 1300 | 114 | ST | SHEA | BL | W | 200 | 2 1 | 0 | 0 | 2 | 1 | 1 | 2 | WB WB | 4 | |
| 1901735 | 190124 | 0803 | 114 | ST | SHEA | BL | E | 300 | | 0 | 0 | 2 | 1 | 8 | 3 | WB WB | 4 | |

KEY

INJURY SEVERITY:

1=NO INJURY, 2=POSSIBLE INJURY, 3=NON-INCAPACITATING INJURY, 4=INCAPACITATING INJURY, 5=FATAL INJURY, 99=NOT REPORTED / UNKNOWN

PHYSICAL CONDITION:

0=NO APPARENT INFLUENCE, 1=ILLNESS, 2=PHYSICAL IMPAIRMENT, 3=FELL ASLEEP / FATIGUED 4=ALCOHOL, 5=DRUGS, 6=MEDICATIONS, A=NO TEST GIVEN, B=TEST GIVEN, C=TEST REFUSED, D=TESTING UNKNOWN, 97=OTHER, 99=UNKNOWN

VIOLATION:

1=NO IMPROPER ACTION, 2=SPEED TOO FAST FOR CONDITIONS, 3=EXCEEDED LAWFUL SPEED 4=FOLLOWED TOO CLOSELY. 5=RAN STOP SIGN, 6=DISREGAREDED TRAFFIC SIGNAL7=MADE IMPROPER TURN, 8=DROVE/RODE IN OPPOSING TRAFFIC LANE, 9=KNOWINGLY OPERATED WITH FAULTY / MISSING EQUIPMENT, 10=REQUIRED MOTORCYCLE SAFETY EQUIPMENT NOT USED, 11=PASSED IN NO PASSING ZONE, 12=UNSAFE LANE CHANGE, 13=FAILED TO KEEP IN PROPER LANE, 14=DISREGARDED PAVEMENT MARKINGS, 15=OTHER UNSAFE PASSING, 16=INATTENTION/DISTRACTION, 17=DID NOT USE CROSSWALK, 18=WALKED ON WRONG SIDE OF ROAD, 19=ELECTRONIC COMMUNICATIONS DEVICE, 20=FAILED TO YIELD RIGHT OF WAY (added August 2014), 97=OTHER, 99 UNKNOWN

ACTION:

1=GOING STRAIGHT AHEAD, 2=SLOWING IN TRAFFICWAY, 3=STOPPED IN TRAFFICWAY, 4=MAKING LEFT TURN, 5=MAKING RIGHT TURN, 6=MAKING U-TURN, 7=OVERTAKING/PASSING, 8=CHANGING LANES, 9=NEGOTIATING A CURVE, 10=BACKING, 11=AVOIDING VEH/OBJ/PED/CYCLIST/ANIMAL, 12=ENTERING PARKING POSITION, 13=LEAVING PARKING POSITION, 14=PROPERLY PARKED, 15=IMPROPERLY PARKED, 16=DRIVERLESS MOVING VEHICLE, 17=CROSING ROAD, 18=WALKING WITH TRAFFIC, 19=WALKING AGAINST TRAFFIC, 20=STANDING, 21=LYING, 22=GETTING ON OR OFF VEHICLE, 23=WORKING ON/PUSHING VEHICLE, 24=WORKING ON ROAD, 97=OTHER, 99=UKNOWN

MANNER OF COLLISION:

1=SINGLE VEHICLE, 2=ANGLE (front to side, other than left turn), 3=LEFT TURN, 4=REAR END (front to rear), 5=HEAD-ON (front to front, other than left turn), 6=SIDESWIPE (same direction), 7=SIDESWIPE (opposite direction), 8=REAR-TO-SIDE, 9=REAR TO REAR, 97=OTHER, 99=UNKNOWN

TOTAL

1



Appendix C – Parcel Information



5-GP-2020 5/22/2020

217-33-034G Land Parcel

This is a land parcel located at <u>11355 E SHEA BLVD SCOTTSDALE 85259</u>, and the current owner is KCI 114 INVESTMENTS LLC. Its current year full cash value is \$477,600.

Property Information

11355 E SHEA BLVD SCOTTSDALE 85259

MCR#

Description: W2 OF FOL DESC PROP N 286F OF E2 E2 NW4 NW4 EX W 25F & EX N 65F

Lat/Long 33.58197000 | -111.83579100

Lot Size 33,715 sq ft.

Zoning C-O

Lot#

High School District SCOTTSDALE UNIFIED #48

Elementary School District SCOTTSDALE UNIFIED SCHOOL DISTRICT

Local Jurisdiction SCOTTSDALE

S/T/R 27 3N 5E Market Area/Neighborhood 05/019

Subdivision (0 Parcels)

Owner Information

KCI 114 INVESTMENTS LLC

Mailing Address 7339 E EVANS RD SUITE 201, SCOTTSDALE, AZ 85260

Deed Number <u>140568992</u> Last Deed Date <u>08/27/2014</u>

Sale Date n/a
Sale Price n/a

Valuation Information

We provide valuation information for the past 5 years. For mobile display, we only show 1 year of valuation information. Should you need more data, please look at our <u>data sales</u>.

The Valuation Information displayed below may not reflect the taxable value used on the tax bill due to any special valuation relief program.

CLICK HERE TO PAY YOUR TAXES OR VIEW YOUR TAX BILL

| Tax Year | 2021 | 2020 | 2019 | 2018 | 2017 |
|------------------------------|--|--|--|--|--|
| Full Cash Value | \$477,600 | \$438,100 | \$417,300 | \$422,600 | \$202,400 |
| Limited Property Value | \$122,276 | \$116,453 | \$110,908 | \$105,627 | \$100,597 |
| Legal Class | 2.R | 2.R | 2.R | 2.R | 2.R |
| Description | AG / VACANT LAND / NON- PROFIT R/P |
| Assessment Ratio | 15% | 15% | 15% | 15% | 15% |
| Assessed LPV | \$18,341 | \$17,468 | \$16,636 | \$15,844 | \$15,090 |
| Property Use Code | 0022 | 0022 | 0022 | 0022 | 0012 |
| PU Description | Vacant Commercial Land | Vacant Commercial Land | Vacant Commercial Land | Vacant Commercial Land | Vacant Residential Land |
| Tax Area Code | 481400 | 481400 | 481400 | 481400 | 481400 |
| Valuation Source | Notice | Notice | Notice | Notice | Notice |

217-33-034F Land Parcel

This is a land parcel located at 11355 E SHEA BLVD SCOTTSDALE 85259. and the current owner is KCI 114 INVESTMENTS LLC. Its current year full cash value is \$480,100.

Property Information

11355 E SHEA BLVD SCOTTSDALE 85259

MCR#

E2 OF FOL N 286F OF E2 E2 NW4 NW4 EX W 25F & EX N 65F & ALSO EX RD Description:

PER 88-469357

33.58197000 | -111.83579100 Lat/Long

Lot Size 31,494 sq ft.

Zoning C-O

Lot#

SCOTTSDALE UNIFIED #48 High School District

Elementary School

SCOTTSDALE UNIFIED SCHOOL DISTRICT District

Local Jurisdiction SCOTTSDALE

S/T/R 27 3N 5E

Market

05/019

Area/Neighborhood

Subdivision (0 Parcels)

Owner Information

KCI 114 INVESTMENTS LLC

Mailing Address 7339 E EVANS RD SUITE 201, SCOTTSDALE, AZ 85260

Deed Number 140568992 Last Deed Date 08/27/2014

Sale Date n/a Sale Price n/a

Valuation Information

We provide valuation information for the past 5 years. For mobile display, we only show 1 year of valuation information. Should you need more data, please look at our <u>data sales</u>.

The Valuation Information displayed below may not reflect the taxable value used on the tax bill due to any special valuation relief program.

CLICK HERE TO PAY YOUR TAXES OR VIEW YOUR TAX BILL

| Tax Year | 2021 | 2020 | 2019 | 2018 | 2017 |
|------------------------------|--|--|--|--|--|
| Full Cash Value | \$480,100 | \$455,300 | \$433,600 | \$432,900 | \$194,700 |
| Limited Property Value | \$114,819 | \$109,351 | \$104,144 | \$99,185 | \$94,462 |
| Legal Class | 2.R | 2.R | 2.R | 2.R | 2.R |
| Description | AG / VACANT LAND / NON- PROFIT R/P |
| Assessment Ratio | 15% | 15% | 15% | 15% | 15% |
| Assessed LPV | \$17,223 | \$16,403 | \$15,622 | \$14,878 | \$14,169 |
| Property Use Code | 0022 | 0022 | 0022 | 0022 | 0012 |
| PU Description | Vacant Commercial Land | Vacant Commercial Land | Vacant Commercial Land | Vacant Commercial Land | Vacant Residential Land |
| Tax Area Code | 481400 | 481400 | 481400 | 481400 | 481400 |
| Valuation Source | Notice | Notice | Notice | Notice | Notice |

217-33-034M Land Parcel

This is a land parcel located at 11355 E SHEA BLVD SCOTTSDALE 85259. and the current owner is KCI 114 INVESTMENTS LLC. Its current year full cash value is \$443,000.

Property Information

11355 E SHEA BLVD SCOTTSDALE 85259

MCR#

S 428.10F OF N 714.10F OF E2 E2 NW4 NW4 SEC 27 EX W 25F & EX S 203F OF W

240F OF S 428.10F OF N 714.10F E2 E2 NW4 NW4 SEC 27 & ALSO EX TH PT LY E OF

LI DAF COM NE COR NW4 NW4 SD SEC TH S 715 TH W 8.12F TO POB SD LI TH N Description: 9D 51M W 179.54F TH W 12F TH N 9D 51M W 40F TH N 80D 8M E 12F TH N 9D

517.05F TO BEG TAN CUR CONC TO NE TH NLY ALG CUR 99.17F TO BEG COMP

CUR CONC TO NE TH NLY ALG CUR 107.56F TO PT & POE SD LI

33.58197000 | -111.83579100 Lat/Long

Lot Size 68,484 sq ft.

Zoning R1-18

Lot#

High School

SCOTTSDALE UNIFIED #48 District

Elementary School

District

SCOTTSDALE UNIFIED SCHOOL DISTRICT

Local Jurisdiction **SCOTTSDALE**

S/T/R 27 3N 5E

Market

Area/Neighborhood 05/019

Subdivision (0

Parcels)

Owner Information

KCI 114 INVESTMENTS LLC

Mailing Address 7339 E EVANS RD SUITE 201, SCOTTSDALE, AZ 85260

Deed Number <u>140568992</u> Last Deed Date <u>08/27/2014</u>

Sale Date n/a
Sale Price n/a

Valuation Information

We provide valuation information for the past 5 years. For mobile display, we only show 1 year of valuation information. Should you need more data, please look at our <u>data sales</u>.

The Valuation Information displayed below may not reflect the taxable value used on the tax bill due to any special valuation relief program. CLICK HERE TO PAY YOUR TAXES OR VIEW YOUR TAX BILL

| Tax Year | 2021 | 2020 | 2019 | 2018 | 2017 |
|------------------------------|--|--|--|--|--|
| Full Cash Value | \$443,000 | \$381,100 | \$352,200 | \$382,400 | \$395,900 |
| Limited Property Value | \$273,964 | \$260,918 | \$248,493 | \$236,660 | \$225,390 |
| Legal Class | 2.R | 2.R | 2.R | 2.R | 2.R |
| Description | AG / VACANT LAND / NON- PROFIT R/P |
| Assessment Ratio | 15% | 15% | 15% | 15% | 15% |
| Assessed LPV | \$41,095 | \$39,138 | \$37,274 | \$35,499 | \$33,809 |
| Property Use Code | 0012 | 0012 | 0012 | 0012 | 0012 |
| PU Description | | Vacant Residential Land | Vacant Residential Land | Vacant Residential Land | Vacant Residential Land |
| Tax Area Code | 481400 | 481400 | 481400 | 481400 | 481400 |
| Valuation Source | Notice | Notice | Notice | Notice | Notice |
| | | | | | |

217-33-034K Land Parcel

This is a land parcel located at 11355 E SHEA BLVD SCOTTSDALE 85259. and the current owner is KCI 114 INVESTMENTS LLC. Its current year full cash value is \$370,000.

Property Information

11355 E SHEA BLVD SCOTTSDALE 85259

MCR#

S 203F OF W 240F OF S 428.10F OF N 714.10F OF E2 E2 NW4 NW4 SEC 27 EX W Description:

25F TH/OF

33.58197000 | -111.83579100 Lat/Long

Lot Size 43,647 sq ft. R1-18

Zoning

Lot#

High School District SCOTTSDALE UNIFIED #48

05/019

Elementary School

District

SCOTTSDALE UNIFIED SCHOOL DISTRICT

Local Jurisdiction SCOTTSDALE

S/T/R 27 3N 5E

Market

Area/Neighborhood

Subdivision (0 Parcels)

Owner Information

KCI 114 INVESTMENTS LLC

Mailing Address 7339 E EVANS RD SUITE 201, SCOTTSDALE, AZ 85260

Deed Number 140568992 Last Deed Date 08/27/2014

Sale Date n/a Sale Price n/a

Valuation Information

We provide valuation information for the past 5 years. For mobile display, we only show 1 year of valuation information. Should you need more data, please look at our <u>data sales</u>.

The Valuation Information displayed below may not reflect the taxable value used on the tax bill due to any special valuation relief program.

CLICK HERE TO PAY YOUR TAXES OR VIEW YOUR TAX BILL

| Tax Year | 2021 | 2020 | 2019 | 2018 | 2017 |
|------------------------------|--|--|--|--|--|
| Full Cash Value | \$370,000 | \$311,800 | \$283,300 | \$294,500 | \$305,600 |
| Limited Property Value | \$209,377 | \$199,407 | \$189,911 | \$180,868 | \$172,255 |
| Legal Class | s 2.R | 2.R | 2.R | 2.R | 2.R |
| _ | AG / VACANT LAND / NON- PROFIT R/P |
| Assessment Ratio | t 15% | 15% | 15% | 15% | 15% |
| Assessed LPV | \$31,407 | \$29,911 | \$28,487 | \$27,130 | \$25,838 |
| Property Use Code | 0014 | 0014 | 0014 | 0014 | 0014 |
| PU Description | | Vacant Residential Land | Vacant Residential Land | Vacant Residential Land | Vacant Residential Land |
| Tax Area Code | 481400 | 481400 | 481400 | 481400 | 481400 |
| Valuation Source | Notice | Notice | Notice | Notice | Notice |



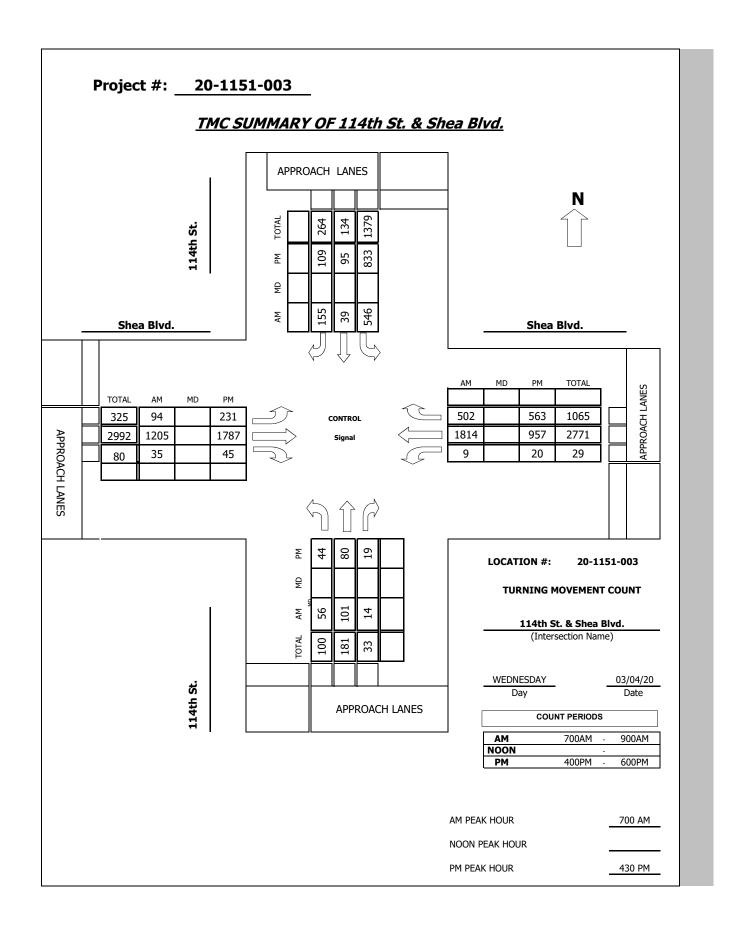
Appendix D – Traffic Count Data



5-GP-2020 5/22/2020

Intersection Turning Movement Prepared by:





Intersection Turning Movement Prepared by:





N-S STREET: 114th St. DATE: 03/04/20 LOCATION: Scottsdale

E-W STREET: Shea Blvd. DAY: WEDNESDAY PROJECT# 20-1151-003

| | NC | RTHBO | UND | SO | UTHBO | UND | E | ASTBOU | IND | W | 'ESTBOI | JND | |
|--|---|--|--------------------------------------|---|---|--|--|--|--|--------------------------------------|--|--|--|
| LANES: | NL 2 | NT 1 | NR 0 | SL 2 | ST 2 | SR 1 | EL 2 | ET 3 | ER 0 | WL 1 | WT 3 | WR 1 | TOTAL |
| 6:00 AM 6:15 AM 6:30 AM 6:45 AM 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 9:00 AM 9:15 AM 9:30 AM 9:45 AM 10:00 AM 10:15 AM 10:30 AM 10:45 AM | 12 17 10 17 10 13 20 9 | 28 29 22 22 23 16 32 26 | 1 5 0 8 1 2 1 4 | 155 137 138 116 103 101 114 79 | 6 5 12 16 13 9 11 12 | 34 32 43 46 41 40 42 43 | 22 24 16 32 28 47 33 37 | 342 310 313 240 290 254 231 244 | 11 7 7 10 15 7 7 10 | 2 2 5 0 0 1 4 0 | 431 417 524 442 374 353 376 294 | 143 142 125 92 146 146 140 95 | 1187 1127 1215 1041 1044 989 1011 853 |

| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
|------------|-------|-------|------|-------|------|-------|------|-------|------|------|-------|-------|-------|
| Volumes | 108 | 198 | 22 | 943 | 84 | 321 | 239 | 2224 | 74 | 14 | 3211 | 1029 | 8467 |
| Approach % | 32.93 | 60.37 | 6.71 | 69.96 | 6.23 | 23.81 | 9.42 | 87.66 | 2.92 | 0.33 | 75.48 | 24.19 | |
| App/Depart | 328 | / | 1466 | 1348 | / | 172 | 2537 | / | 3189 | 4254 | / | 3640 | |

AM Peak Hr Begins at: 700 AM

PEAK

Volumes 56 101 14 546 39 155 94 1205 35 9 1814 502 4570 Approach % 32.75 59.06 8.19 73.78 5.27 20.95 7.05 90.33 2.62 0.39 78.02 21.59

PEAK HR.

FACTOR: 0.838 0.949 0.889 0.889 0.940

CONTROL: Signal

COMMENT 1:

GPS: 33.582462, -111.835079

Intersection Turning Movement



N-S STREET: 114th St. DATE: 03/04/20 LOCATION: Scottsdale

0

E-W STREET: Shea Blvd. DAY: WEDNESDAY PROJECT# 20-1151-003

| | NO | RTHBOU | JND | SOL | JTHBOU | JND | E/ | ASTBOU | ND | WI | ESTBOL | IND | |
|---|--------------------|----------------------|------------------|--------------------------|----------------------|----------------------|----------------------|--------------------------|---------------------|------------------|--------------------------|--------------------------|------------------------------|
| LANES: | NL 2 | NT 1 | NR 0 | SL 2 | ST 2 | SR 1 | EL 2 | ET 3 | ER 0 | WL 1 | WT 3 | WR 1 | TOTAL |
| 1:00 PM 1:15 PM 1:30 PM 1:45 PM 2:00 PM 2:15 PM 2:30 PM 2:45 PM 3:00 PM 3:15 PM 3:30 PM 3:45 PM 4:00 PM 4:15 PM 4:30 PM | 11 9 13 6 | 26 18 17 18 | 0 4 6 3 | 162 223 202 232 | 23 37 22 29 | 39 29 25 28 | 50 53 51 57 | 403 409 463 409 | 16 14 18 8 | 3 1 7 6 | 287 237 244 219 | 132 136 164 133 | 1152 1170 1232 1148 |
| 5:00 PM 5:15 PM | 15 10 | 20 25 | 6 4 | 187 212 | 17 27 | 26 30 | 55 68 | 440 475 | 12 7 | 4 | 268 226 | 144 122 | 1194 1209 |
| 5:30 PM 5:45 PM 6:00 PM 6:15 PM 6:30 PM 6:45 PM | 9 12 | 18 21 | 3 3 | 165 150 | 18 24 | 33 40 | 65 47 | 380 308 | 8 11 | 4 5 | 255 197 | 131 91 | 1089 909 |
| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| Volumes Approach % | 85 30.69 | 163 58.84 | 29 10.47 | 1533 77.42 | 197 9.95 | 250 12.63 | 446 11.65 | 3287 85.89 | 94 2.46 | 33 1.09 | 1933 64.03 | 1053 34.88 | 9103 |
| App/Depart | 277 | <u>/</u> | 1662 | 1980 | / | 324 | 3827 | / | 4849 | 3019 | / | 2268 | |
| | ık Hr Beç | gins at: | 430 | PΜ | | | | | | | | | |
| PEAK | | | | | | | | | | | | 1 | 1 |

PEAK HR.

FACTOR: 0.872 0.897 0.938 0.925 0.971

CONTROL: Signal COMMENT 1: 0

GPS: 33.582462, -111.835079

5-GP-2020 5/22/2020



Pedestrian & Bicycle Study

N-S STREET: 114th St.

Date: 03/04/20

City: Scottsdale

Day: WEDNESDAY

Project #: 20-1151-003

| | | PEDES | TRIANS | |
|---------|-------|-------|--------|-------|
| | N-LEG | S-LEG | E-LEG | W-LEG |
| 7:00 AM | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 |
| 7:45 AM | 2 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 0 | 0 | 0 |
| 8:45 AM | 1 | 0 | 2 | 0 |
| TOTAL | 3 | 0 | 2 | 0 |

| | | BICY | CLES | |
|---------|-------|-------|-------|-------|
| | N-LEG | S-LEG | E-LEG | W-LEG |
| 7:00 AM | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 0 | 0 | 0 |

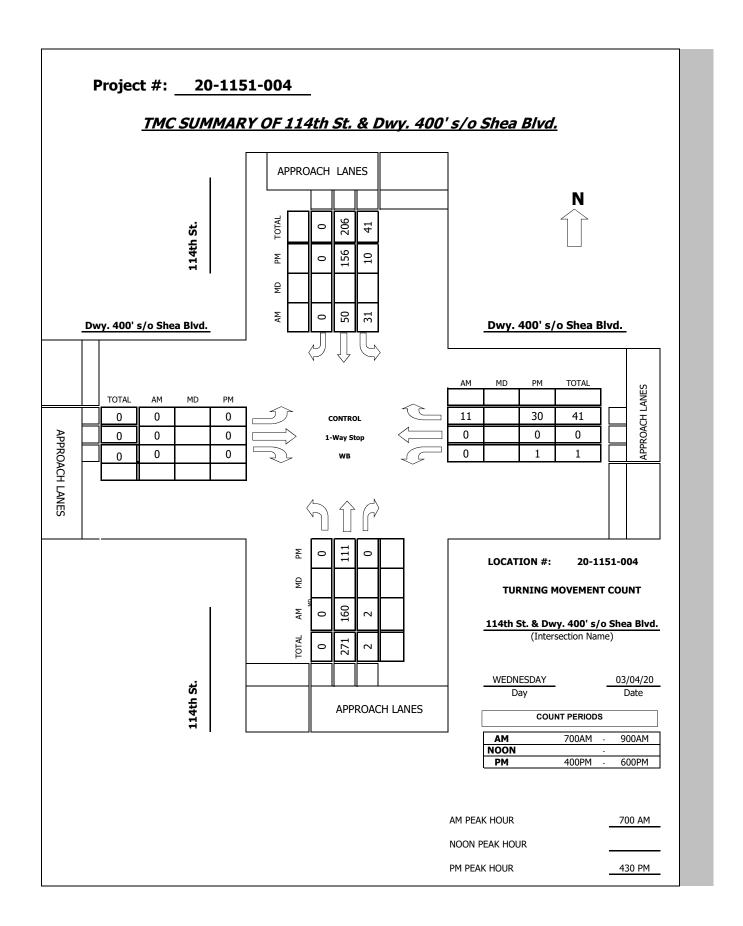
| | | PEDES | TRIANS | |
|---------|-------|-------|--------|-------|
| | N-LEG | S-LEG | E-LEG | W-LEG |
| 4:00 PM | 0 | 0 | 0 | 0 |
| 4:15 PM | 0 | 0 | 0 | 0 |
| 4:30 PM | 0 | 0 | 0 | 0 |
| 4:45 PM | 0 | 0 | 0 | 0 |
| 5:00 PM | 1 | 0 | 0 | 0 |
| 5:15 PM | 1 | 0 | 0 | 0 |
| 5:30 PM | 0 | 0 | 0 | 0 |
| 5:45 PM | 0 | 0 | 0 | 0 |
| TOTAL | 2 | 0 | 0 | 0 |

| | | BICYCLES | | | | | | | | | | |
|---------|-------|-------------------------|---|---|--|--|--|--|--|--|--|--|
| | N-LEG | N-LEG S-LEG E-LEG W-LEG | | | | | | | | | | |
| 4:00 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 4:15 PM | 0 | 0 | 0 | 1 | | | | | | | | |
| 4:30 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 4:45 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 5:00 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 5:15 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 5:30 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 5:45 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| TOTAL | 0 | 0 | 0 | 1 | | | | | | | | |

| | North Leg | |
|----------|-----------|----------|
| West Leg | | East Leg |
| | South Leg | |

Intersection Turning Movement Prepared by:





Intersection Turning Movement Prepared by:





N-S STREET: 114th St. DATE: 03/04/20 LOCATION: Scottsdale

E-W STREET: Dwy. 400' s/o Shea Blvd. DAY: WEDNESDAY PROJECT# 20-1151-004

| | NC | NORTHBOUND | | SC | UTHBO | UND | E | ASTBOL | IND | W | 'ESTBOL | JND | |
|--|-------------|----------------------|------------------|-------------------|--------------------|------------------|------------------|------------------|------------------|------------------|-------------|------------------|----------------------|
| LANES: | NL 0 | NT 1 | NR 0 | SL 1 | ST 2 | SR 0 | EL 0 | ET 0 | ER 0 | WL 0 | WT 1 | WR 0 | TOTAL |
| 6:00 AM 6:15 AM 6:30 AM | | | | | | | | | | | | | |
| 6:45 AM 7:00 AM 7:15 AM 7:30 AM 7:45 AM | 0 0 0 | 42 45 30 43 | 1 0 0 1 | 12 4 9 6 | 8 9 14 19 | 0 0 0 0 | 0 0 0 0 | 0 0 0 0 | 0 0 0 0 | 0 0 0 0 | 0 0 0 | 1 4 3 3 | 64 62 56 72 |
| 8:00 AM 8:15 AM 8:30 AM | 0 0 0 | 33 26 47 | 0 0 0 | 8 3 9 | 20 15 14 | 0 0 0 | 0 0 0 | 0 0 0 | 0 0 0 | 0 0 0 | 0 0 0 | 2 6 5 | 50 75 |
| 8:45 AM 9:00 AM 9:15 AM 9:30 AM 9:45 AM | 0 | 37 | 0 | 6 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 61 |
| 10:00 AM 10:15 AM 10:30 AM 10:45 AM 11:00 AM | | | | | | | | | | | | | |
| 11:15 AM 11:30 AM 11:45 AM | | | | | | | | | | | | | |

| TOTAL | NL | ΝI | NR | SL | SI | SR | EL | El | ER | WL | WI | WR | IOIAL |
|------------|------|-------|------|-------|-------|------|------|------|------|------|------|--------|-------|
| Volumes | 0 | 303 | 2 | 57 | 114 | 0 | 0 | 0 | 0 | 0 | 0 | 27 | 503 |
| Approach % | 0.00 | 99.34 | 0.66 | 33.33 | 66.67 | 0.00 | #### | #### | #### | 0.00 | 0.00 | 100.00 | |
| App/Depart | 305 | / | 330 | 171 | / | 114 | 0 | / | 59 | 27 | / | 0 | |

AM Peak Hr Begins at: 700 AM

PEAK

Volumes 0 160 2 31 50 0 0 0 0 0 0 11 254 Approach % 0.00 98.77 1.23 38.27 61.73 0.00 #### #### #### 0.00 0.00 100.00

PEAK HR.

FACTOR: 0.900 0.810 0.000 0.688 0.882

CONTROL: 1-Way Stop (WB)

COMMENT 1:

GPS: 33.581309, -111.835054

Intersection Turning Movement



N-S STREET: 114th St. DATE: 03/04/20 LOCATION: Scottsdale

0

E-W STREET: Dwy. 400' s/o Shea Blvd. DAY: WEDNESDAY PROJECT# 20-1151-004

| | NC | RTHBOU | JND | SO | UTHBOL | JND | E | ASTBOL | JND | WI | ESTBOL | JND | |
|------------|---------|----------|---------|---------|---------|---------|---------|---------|---------|---------|---------|---------|-------|
| LANES: | NL 0 | NT 1 | NR 0 | SL 1 | ST 2 | SR 0 | EL 0 | ET 0 | ER 0 | WL 0 | WT 1 | WR 0 | TOTAL |
| 1:00 PM | | | | | | | | | | | | | |
| 1:15 PM | | | | | | | | | | | | | |
| 1:30 PM | | | | | | | | | | | | | |
| 1:45 PM | | | | | | | | | | | | | |
| 2:00 PM | | | | | | | | | | | | | |
| 2:15 PM | | | | | | | | | | | | | |
| 2:30 PM | | | | | | | | | | | | | |
| 2:45 PM | | | | | | | | | | | | | |
| 3:00 PM | | | | | | | | | | | | | |
| 3:15 PM | | | | | | | | | | | | | |
| 3:30 PM | | | | | | | | | | | | | |
| 3:45 PM | | | | | | | | | | | | | |
| 4:00 PM | 0 | 25 | 0 | 9 | 35 | 0 | 0 | 0 | 0 | 2 | 0 | 10 | 81 |
| 4:15 PM | 0 | 24 | 0 | 2 | 49 | 0 | 0 | 0 | 0 | 2 | 0 | 8 | |
| 4:30 PM | 0 | 28 | 0 | 6 | 43 | 0 | 0 | 0 | 0 | 1 | 0 | 7 | 85 |
| 4:45 PM | 0 | 20 | 0 | 2 | 42 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 73 |
| 5:00 PM | 0 | 30 | 0 | 2 | 36 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 77 |
| 5:15 PM | 0 | 33 | 0 | 0 | 35 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 73 |
| 5:30 PM | 0 | 24 | 0 | 1 | 30 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 62 |
| 5:45 PM | 0 | 26 | 0 | 4 | 35 | 0 | 0 | 0 | 0 | 1 | 0 | 11 | 77 |
| 6:00 PM | | | | | | | | | | | | | |
| 6:15 PM | | | | | | | | | | | | | |
| 6:30 PM | | | | | | | | | | | | | |
| 6:45 PM | | | | | | | | | | | | | |
| TOTAL | NL | NT | NR | SL | ST | SR | EL | ET | ER | WL | WT | WR | TOTAL |
| Volumes | 0 | 210 | 0 | 26 | 305 | 0 | 0 | 0 | 0 | 6 | 0 | 66 | 613 |
| Approach % | 0.00 | 100.00 | 0.00 | 7.85 | 92.15 | 0.00 | #### | #### | #### | 8.33 | 0.00 | 91.67 | - |
| App/Depart | 210 | / | 276 | 331 | / | 311 | 0 | / | 26 | 72 | / | 0 | |
| | k Hr Be | gins at: | 430 | | , | | | · · | | | • | | |

PEAK

111 0 10 156 0 0 Approach % 0.00 100.00 0.00 6.02 93.98 0.00 #### #### #### 3.23 0.00 96.77

PEAK HR.

0.841 0.847 0.000 0.861 0.906 FACTOR:

CONTROL: 1-Way Stop (WB)

COMMENT 1: 0

GPS: 33.581309, -111.835054



Pedestrian & Bicycle Study

 N-S STREET:
 114th St.
 Date:
 03/04/20
 City:
 Scottsdale

 E-W STREET:
 Dwy. 400' s/o Shea Blvd.
 Day:
 WEDNESDAY
 Project #:
 20-1151-004

| | | PEDES | TRIANS | |
|---------|-------|-------|--------|-------|
| | N-LEG | S-LEG | E-LEG | W-LEG |
| 7:00 AM | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 0 | 2 | 0 |
| TOTAL | 0 | 0 | 2 | 0 |

| | | BICY | CLES | | | | | | | | |
|---------|-------|-------------------|------|---|--|--|--|--|--|--|--|
| | N-LEG | N-LEG S-LEG W-LEG | | | | | | | | | |
| 7:00 AM | 0 | 0 | 0 | 0 | | | | | | | |
| 7:15 AM | 0 | 0 | 0 | 0 | | | | | | | |
| 7:30 AM | 0 | 0 | 0 | 0 | | | | | | | |
| 7:45 AM | 0 | 0 | 0 | 0 | | | | | | | |
| 8:00 AM | 0 | 0 | 0 | 0 | | | | | | | |
| 8:15 AM | 0 | 0 | 0 | 0 | | | | | | | |
| 8:30 AM | 0 | 0 | 0 | 0 | | | | | | | |
| 8:45 AM | 0 | 0 | 0 | 0 | | | | | | | |
| TOTAL | 0 | 0 | 0 | 0 | | | | | | | |

| | | PEDES | TRIANS | |
|---------|-------|-------|--------|-------|
| | N-LEG | S-LEG | E-LEG | W-LEG |
| 4:00 PM | 0 | 0 | 0 | 0 |
| 4:15 PM | 0 | 0 | 0 | 0 |
| 4:30 PM | 0 | 0 | 0 | 0 |
| 4:45 PM | 0 | 0 | 0 | 0 |
| 5:00 PM | 0 | 0 | 0 | 0 |
| 5:15 PM | 0 | 0 | 0 | 0 |
| 5:30 PM | 0 | 0 | 0 | 0 |
| 5:45 PM | 0 | 0 | 0 | 0 |
| TOTAL | 0 | 0 | 0 | 0 |

| | | BICY | CLES | | | | | | | | | |
|---------|-------|-------------------|------|---|--|--|--|--|--|--|--|--|
| | N-LEG | N-LEG S-LEG W-LEG | | | | | | | | | | |
| 4:00 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 4:15 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 4:30 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 4:45 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 5:00 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 5:15 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 5:30 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| 5:45 PM | 0 | 0 | 0 | 0 | | | | | | | | |
| TOTAL | 0 | 0 | 0 | 0 | | | | | | | | |

| | North Leg | |
|----------|-----------|----------|
| West Leg | | East Leg |
| | | |
| | Cauth Lan | |
| | South Leg | |
| | | |

Prepared by: Field Data Services of Arizona/Veracity Traffic Group (520) 316-6745

Volumes for: Wednesday, March 4, 2020 City: Scottsdale Project #: 20-1151-001

| ı | l ocation: | Choa | Blvd | wort o | f 11/1+h C | Ή. |
|---|------------|------|-------|--------|------------------------|-----|
| ı | LOCALION: | Snea | BIVU. | west o | 1 11 4 th 5 | ıL. |

| AM Period NB | SB | EB | | WB | | | PM Period | NB | SB | EB | | WB | | |
|--------------|----|-----|------|-----|------|-------|-----------|----|----|-----|-------|-----|------|-------|
| 00:00 | | 28 | | 13 | | | 12:00 | | | 302 | | 313 | | |
| 00:15 | | 23 | | 11 | | | 12:15 | | | 368 | | 312 | | |
| 00:30 | | 10 | | 14 | | | 12:30 | | | 340 | | 302 | | |
| 00:45 | | 7 | 68 | 5 | 43 | 111 | 12:45 | | | 331 | 1341 | 336 | 1263 | 2604 |
| 01:00 | | 16 | | 15 | | | 13:00 | | | 314 | | 299 | | |
| 01:15 | | 6 | | 18 | | | 13:15 | | | 358 | | 284 | | |
| 01:30 | | 12 | | 9 | | | 13:30 | | | 355 | | 330 | | |
| 01:45 | | 12 | 46 | 7 | 49 | 95 | 13:45 | | | 338 | 1365 | 328 | 1241 | 2606 |
| 02:00 | | 6 | | 13 | | | 14:00 | | | 331 | | 293 | | |
| 02:15 | | 6 | | 8 | | | 14:15 | | | 355 | | 355 | | |
| 02:30 | | 7 | | 5 | | | 14:30 | | | 366 | | 289 | | |
| 02:45 | | 6 | 25 | 2 | 28 | 53 | 14:45 | | | 414 | 1466 | 319 | 1256 | 2722 |
| 03:00 | | 6 | | 8 | | | 15:00 | | | 444 | | 344 | | |
| 03:15 | | 4 | | 4 | | | 15:15 | | | 446 | | 327 | | |
| 03:30 | | 9 | | 7 | | | 15:30 | | | 469 | | 332 | | |
| 03:45 | | 4 | 23 | 12 | 31 | 54 | 15:45 | | | 429 | 1788 | 270 | 1273 | 3061 |
| 04:00 | | 16 | | 19 | | | 16:00 | | | 469 | | 337 | | |
| 04:15 | | 20 | | 21 | | | 16:15 | | | 476 | | 275 | | |
| 04:30 | | 20 | | 35 | | | 16:30 | | | 532 | | 282 | | |
| 04:45 | | 32 | 88 | 40 | 115 | 203 | 16:45 | | | 474 | 1951 | 253 | 1147 | 3098 |
| 05:00 | | 26 | | 50 | | | 17:00 | | | 507 | | 309 | | |
| 05:15 | | 57 | | 80 | | | 17:15 | | | 550 | | 266 | | |
| 05:30 | | 72 | | 97 | | | 17:30 | | | 453 | | 297 | | |
| 05:45 | | 88 | 243 | 112 | 339 | 582 | 17:45 | | | 366 | 1876 | 249 | 1121 | 2997 |
| 06:00 | | 95 | | 140 | | | 18:00 | | | 374 | | 272 | | |
| 06:15 | | 169 | | 193 | | | 18:15 | | | 335 | | 210 | | |
| 06:30 | | 252 | | 274 | | | 18:30 | | | 282 | | 187 | | |
| 06:45 | | 294 | 810 | 292 | 899 | 1709 | 18:45 | | | 280 | 1271 | 162 | 831 | 2102 |
| 07:00 | | 375 | | 477 | | | 19:00 | | | 258 | | 153 | | |
| 07:15 | | 341 | | 466 | | | 19:15 | | | 245 | | 128 | | |
| 07:30 | | 336 | | 577 | | | 19:30 | | | 223 | | 117 | | |
| 07:45 | | 282 | 1334 | 505 | 2025 | 3359 | 19:45 | | | 180 | 906 | 82 | 480 | 1386 |
| 08:00 | | 333 | | 425 | | | 20:00 | | | 195 | | 106 | | |
| 08:15 | | 308 | | 406 | | | 20:15 | | | 185 | | 104 | | |
| 08:30 | | 271 | | 438 | | | 20:30 | | | 178 | | 79 | | |
| 08:45 | | 291 | 1203 | 346 | 1615 | 2818 | 20:45 | | | 135 | 693 | 77 | 366 | 1059 |
| 09:00 | | 307 | | 323 | | | 21:00 | | | 139 | | 109 | | |
| 09:15 | | 268 | | 332 | | | 21:15 | | | 125 | | 67 | | |
| 09:30 | | 291 | | 330 | | | 21:30 | | | 101 | | 68 | | |
| 09:45 | | 268 | 1134 | 327 | 1312 | 2446 | 21:45 | | | 89 | 454 | 60 | 304 | 758 |
| 10:00 | | 256 | | 315 | | | 22:00 | | | 90 | | 46 | | |
| 10:15 | | 309 | | 321 | | | 22:15 | | | 79 | | 49 | | |
| 10:30 | | 284 | | 343 | | | 22:30 | | | 59 | | 44 | | |
| 10:45 | | 293 | 1142 | 301 | 1280 | 2422 | 22:45 | | | 44 | 272 | 34 | 173 | 445 |
| 11:00 | | 264 | | 315 | | | 23:00 | | | 42 | | 35 | | |
| 11:15 | | 317 | | 369 | | | 23:15 | | | 37 | | 22 | | |
| 11:30 | | 276 | | 297 | | | 23:30 | | | 33 | | 19 | | |
| 11:45 | | 295 | 1152 | 328 | 1309 | 2461 | 23:45 | | | 21 | 133 | 13 | 89 | 222 |
| Total Vol. | | | 7268 | | 9045 | 16313 | | | | | 13516 | | 9544 | 23060 |
| | | | | | | | | | | | | | | |

| GPS Coordinates: | 33.582456, -111.836613 | | . <u>-</u> | NB | SB | Daily Totals EB | | Combined | |
|------------------|------------------------|--------------------|------------|----|----|--------------------|-------|----------|----------|
| | | | | | | 20784 | 18589 | 39373 | |
| | AM | | | | | PM | | | |
| Split % | 44.6% | 55.4% 41.4% | | | | 58.6% | 41.4% | 58.6% | |
| Peak Hour | 06:45 | 07:00 07:00 | | | | 16:30 | 14:45 | 16:30 | |
| Volume | 1346 | 2025 3359 | | | | 2063 | 1322 | 3173 | |
| P.H.F. | 0.90 | 0.88 0.92 | | | | 0 | 5-GP | -2020 | 1 |
| | | | | | | | J-G1 | -202(| J |

5/22/2020

Prepared by: Field Data Services of Arizona/Veracity Traffic Group (520) 316-6745

Volumes for: Wednesday, March 4, 2020 City: Scottsdale Project #: 20-1151-002

Location: 114th St. south of Shea Blvd.

| AM Period | NB | | SB | | EB | WB | | PM Period | NB | | SB | | EB | WB | |
|-------------|-------------|-------|-----|------------|-----------|----|-------|-----------|----|-------|----|-------|---------------------------|----------------|----------|
| 00:00 | 2 | | 1 | | | | | 12:00 | 43 | | 22 | | | | |
| 00:15 | 1 | | 0 | | | | | 12:15 | 30 | | 43 | | | | |
| 00:30 | 2 | | 2 | | | | | 12:30 | 41 | | 47 | | | | |
| 00:45 | 1 | 6 | 2 | 5 | | | 11 | 12:45 | 26 | 140 | 40 | 152 | | | 292 |
| 01:00 | 0 | | 1 | | | | | 13:00 | 32 | | 32 | | | | |
| 01:15 | 0 | | 0 | | | | | 13:15 | 29 | | 36 | | | | |
| 01:30 | 0 | | 0 | | | | | 13:30 | 41 | | 32 | | | | |
| 01:45 | 0 | 0 | 1 | 2 | | | 2 | 13:45 | 25 | 127 | 31 | 131 | | | 258 |
| 02:00 | 0 | | 0 | | | | | 14:00 | 40 | | 42 | | | | |
| 02:15 | 0 | | 0 | | | | | 14:15 | 40 | | 35 | | | | |
| 02:30 | 0 | | 0 | | | | | 14:30 | 31 | | 36 | | | | |
| 02:45 | 0 | 0 | 0 | 0 | | | | 14:45 | 42 | 153 | 35 | 148 | | | 301 |
| 03:00 | 0 | | 0 | | | | | 15:00 | 32 | | 39 | | | | |
| 03:15 | 0 | | 0 | | | | | 15:15 | 42 | | 39 | | | | |
| 03:30 | 0 | | 0 | | | | | 15:30 | 38 | | 37 | | | | |
| 03:45 | 2 | 2 | 0 | 0 | | | 2 | 15:45 | 22 | 134 | 32 | 147 | | | 281 |
| 04:00 | 0 | | 0 | | | | | 16:00 | 37 | | 42 | | | | |
| 04:15 | 3 | | 0 | | | | | 16:15 | 31 | | 52 | | | | |
| 04:30 | 2 | | 0 | | | | | 16:30 | 36 | | 47 | | | | |
| 04:45 | 2 | 7 | 1 | 1 | | | 8 | 16:45 | 27 | 131 | 43 | 184 | | | 315 |
| 05:00 | 3 | | 2 | | | | | 17:00 | 41 | | 33 | | | | |
| 05:15 | 0 | | 6 | | | | | 17:15 | 39 | | 37 | | | | |
| 05:30 | 10 | | 4 | | | | | 17:30 | 30 | | 30 | | | | |
| 05:45 | 2 | 15 | 2 | 14 | | | 29 | 17:45 | 36 | 146 | 40 | 140 | | | 286 |
| 06:00 | 10 | | 5 | | | | | 18:00 | 32 | | 26 | | | | |
| 06:15 | 16 | | 6 | | | | | 18:15 | 16 | | 29 | | | | |
| 06:30 | 18 | | 4 | | | | | 18:30 | 9 | | 22 | | | | |
| 06:45 | 17 | 61 | 11 | 26 | | | 87 | 18:45 | 13 | 70 | 32 | 109 | | | 179 |
| 07:00 | 41 | | 19 | | | | | 19:00 | 13 | | 23 | | | | |
| 07:15 | 51 | | 14 | | | | | 19:15 | 11 | | 29 | | | | |
| 07:30 | 32 | | 24 | | | | | 19:30 | 13 | | 19 | | | | |
| 07:45 | 47 | 171 | 26 | 83 | | | 254 | 19:45 | 7 | 44 | 19 | 90 | | | 134 |
| 08:00 | 34 | | 28 | | | | | 20:00 | 7 | | 8 | | | | |
| 08:15 | 31 | | 17 | | | | | 20:15 | 6 | | 20 | | | | |
| 08:30 | 53 | | 22 | | | | | 20:30 | 9 | | 23 | | | | |
| 08:45 | 39 | 157 | 22 | 89 | | | 246 | 20:45 | 5 | 27 | 12 | 63 | | | 90 |
| 09:00 | 29 | | 23 | | | | | 21:00 | 5 | | 6 | | | | |
| 09:15 | 35 | | 32 | | | | | 21:15 | 17 | | 14 | | | | |
| 09:30 | 44 | | 28 | | | | | 21:30 | 5 | | 7 | | | | |
| 09:45 | 24 | 132 | 19 | 102 | | | 234 | 21:45 | 3 | 30 | 5 | 32 | | | 62 |
| 10:00 | 40 | | 38 | | | | | 22:00 | 0 | | 3 | | | | |
| 10:15 | 41 | | 26 | | | | | 22:15 | 4 | | 2 | | | | |
| 10:30 | 50 | | 31 | | | | | 22:30 | 16 | | 3 | | | | |
| 10:45 | 46 | 177 | 18 | 113 | | | 290 | 22:45 | 7 | 27 | 3 | 11 | | | 38 |
| 11:00 | 45 | | 30 | | | | | 23:00 | 2 | | 1 | | | | |
| 11:15 | 32 | | 36 | | | | | 23:15 | 2 | | 1 | | | | |
| 11:30 | 36 | | 35 | | | | | 23:30 | 1 | | 2 | | | | |
| 11:45 | 37 | 150 | 29 | 130 | | | 280 | 23:45 | 0 | 5 | 0 | 4 | | | 9 |
| Total Vol. | | 878 | | 565 | | | 1443 | | | 1034 | | 1211 | | | 2245 |
| | n a t = - : | | 22 | | 11 025040 | | | | | 100 1 | • | 1 | Daily Tate | . | |
| GPS Coordii | ıates: | | 33. | 581488, -1 | 11.835049 | | | | | NB | | SB | Daily Tota l EB | s WB | Combined |
| | | | | | | | | | | 1912 | | 1776 | | ,,,, | 3688 |
| | | | | | ΔМ | | | | | 1912 | | 1//0 | DM | | 3000 |
| Split % | _ | 60.8% | | 39.2% | AM | | 39.1% | | | 46.1% | 5 | 3.9% | PM | | 60.9% |
| | | | | | | | | | | | | | | | |
| Peak Hour | | 10:15 | | 11:45 | | | 11:45 | | | 14:45 | | 16:00 | | | 16:00 |
| Volume | | 182 | | 141 | | | 292 | | | 154 | | 184 | | | 315 |
| P.H.F. | | 0.91 | | 0.75 | | | 0.83 | | | 0.92 | | 0.88 | | 5-GF | P-202 |
| | | | | | | | | | | | | | | | 2/2020 |
| | | | | | | | | | | | | | | J/ Z/ | ./ とひとし |



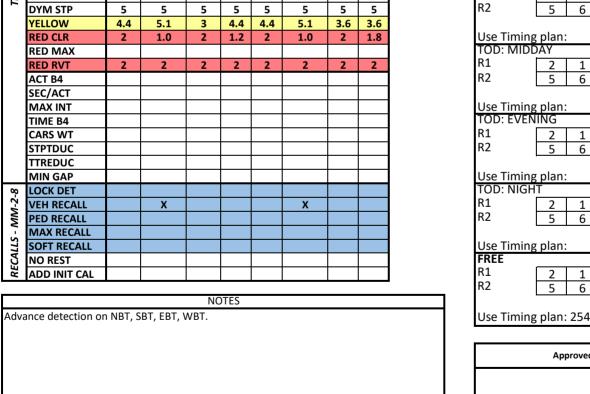
Appendix E – Existing Signal Timing



5-GP-2020 5/22/2020

| SHEA & FLW/11 | System # 139 | | |
|-------------------|--------------|--------------------------------|---------------|
| BASIC TIMING PLAN | Section # | I.P. Address MM1-5-1 | Date Designed |
| | | 172.27.11.39 | 3/12/2020 |

| | r | | 1 | | | 1 | 1 | | 1 |
|----------------------|--------------|------|-------|-----|-----|-----|-------|------|-----|
| | Phase | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
| | Movement | WBL | EBT | NBL | SBT | EBL | WBT | SBL | NBT |
| | NOTES | PROT | COORD | L-P | | L-P | COORD | PROT | |
| | MIN GRN | 5 | 10 | 5 | 7 | 5 | 10 | 5 | 7 |
| | BK MGRN | | | | | | | | |
| | CS MGRN | | | | | | | | |
| | DLY GRN | | | | | | | | |
| | WALK | | 4 | | 4 | | 4 | | 4 |
| | WALK2 | | | | | | | | |
| | WLK MAX | | | | | | | | |
| | PED CLR/FDW | | 21 | | 31 | | 22 | | 28 |
| | PD CLR2 | | | | | | | | |
| 7 | PC MAX | | | | | | | | |
| 1-2 | PED CO | | | | | | | | |
| TIMING PLAN - MM-2-1 | VEH EXT | 2 | | 2 | | 2 | | 2 | |
| > | VH EXT2 | | | | | | | | |
| Ε | MAX 1 | 20 | 55 | 20 | 50 | 20 | 55 | 30 | 50 |
| 3.0 | MAX 2 | 30 | 65 | 30 | 60 | 30 | 65 | 40 | 60 |
| Ĭ | MAX 3 | | | | | | | | |
| Įξ | DYM MAX | 30 | 65 | 30 | 60 | 30 | 65 | 40 | 60 |
| - | DYM STP | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 |
| | YELLOW | 4.4 | 5.1 | 3 | 4.4 | 4.4 | 5.1 | 3.6 | 3.6 |
| | RED CLR | 2 | 1.0 | 2 | 1.2 | 2 | 1.0 | 2 | 1.8 |
| | RED MAX | | | | | | | | |
| | RED RVT | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| | ACT B4 | | | | | | | | |
| | SEC/ACT | | | | | | | | |
| | MAX INT | | | | | | | | |
| | TIME B4 | | | | | | | | |
| | CARS WT | | | | | | | | |
| | STPTDUC | | | | | | | | |
| | TTREDUC | | | | | | | | |
| | MIN GAP | | | | | | | | |
| 8- | LOCK DET | | | | | | | | |
| 1-2 | VEH RECALL | | Х | | | | Х | | |
| \$ | PED RECALL | | | | | | | | |
| S- | MAX RECALL | | | | | | | | |
| 177 | SOFT RECALL | | | | | | | | |
| RECALLS - MM-2-8 | NO REST | | | | | | | | |
| R | ADD INIT CAL | | | | | | | | |



| 4TH - | | | | | - | | | | | |
|-------------|---------|------------|------|-------|---|----------|-----------|----------|--|--|
| FLW/114TH - | SB | Ţ | 4 ↓↓ | 7 | | SHEA - W | | | | |
| | | | | | | Ţ | | | | |
| | 5 | t t | | Ŵ | | 1 1 | 6 | | | |
| | 2 | → → | Р | HASIN | G | Ĺ | 1 | | | |
| | | | | 44 | † | | ÷ | | | |
| SI | HEA - I | ЕВ | | 3 | 8 | | FLW/114TH | NB NB | | |
| | | | | | | | FLW/ | | | |

| PHASING SEQUENCES | | | | | | | | | | | |
|----------------------|-------|---|--------|---|--|--|--|--|--|--|--|
| TOD: MOR | NING | | | | | | | | | | |
| R1 | 5 | 1 | 3 | 4 | | | | | | | |
| R2 | 5 | 6 | 8 | 7 | | | | | | | |
| | | В | | В | | | | | | | |
| Use Timing | plan: | | | | | | | | | | |
| TOD: MIDE | PΑΥ | | | | | | | | | | |
| R1 | 2 | 1 | 3 8 | 4 | | | | | | | |
| R2 | 5 | 6 | 8 | 7 | | | | | | | |
| | | В | | В | | | | | | | |
| Use Timing | plan: | | | | | | | | | | |
| TOD: EVEN | ING | | | | | | | | | | |
| R1 | 2 | 1 | 3 | 4 | | | | | | | |
| R2 | 5 | 6 | 8 | 7 | | | | | | | |
| | | В | | В | | | | | | | |
| Use Timing | | | | | | | | | | | |
| TOD: NIGH | T | | | | | | | | | | |
| R1 | 2 | 1 | 3 | 4 | | | | | | | |
| R2 | 5 | 6 | 8 | 7 | | | | | | | |
| | | В | | В | | | | | | | |
| Use Timing | plan: | | | | | | | | | | |
| FREE | | | | | | | | | | | |
| R1 | 2 | 1 | 3 | 4 | | | | | | | |
| R2 | 5 | 6 | 8 | 7 | | | | | | | |
| | | В | | В | | | | | | | |
| Use Timing plan: 254 | | | | | | | | | | | |
| | - | | | | | | | | | | |

| Approved By | | | | | | | | | |
|----------------|--|--|--|--|--|--|--|--|--|
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| Effective Date | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |

| | SHEA | Sy | System # 139 | | | | | | | | | |
|------------------------------------|---------|-----|---------------|------|------|------|----------|------|------|---------------------|------------|--|
| | 2000011 | | 20 | | | Se | ection | # | | Date Upda | ated | |
| ' | COORDII | NAI | JR | | | | 0 | | | 3/12/20 | 20 | |
| | PHASE | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | |
| | FDW | | 21 | | 31 | | 22 | | 28 | | | |
| | YELLOW | 4.4 | 5.1 | 3 | 4.4 | 4.4 | 5.1 | 3.6 | 3.6 | | | |
| | ALL RED | 2 | 1 | 2 | 1.2 | 2 | 1 | 2 | 1.8 | | | |
| | WALK | | 21 | | 31 | | 22 | | 28 | | | |
| | R1 | 2 | → | 1 | Ţ | 3 | Ţ | 4 | 1 | COORD PATTERN | OFFSET | |
| PLAN 1 | R2 | 5 | 1 | 6 | Ţ | 8 | † | 7 | 1 | Balanced | 44 | |
| AM PLAN | | | RIN | G 1 | | | RIN | IG 2 | | <u> </u> | | |
| OPERATIVE | PHASE | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | |
| TIMES | SPLIT | 12 | 58 | 11 | 39 | 12 | 58 | 20 | 30 | Target Cyc | | |
| 6:00 | COORD | | Х | | | | Х | | | 12 | | |
| | RECALLS | | V | | | | V | | | Actual Cyc | | |
| | GREEN | 5.6 | 51.9 | 6.0 | 33.4 | 5.6 | 51.9 | 14.4 | 24.6 | 12 | 20 | |
| | R1 | 2 | \rightarrow | 1 | Ţ | 3 | ← | 4 | 1 | COORD PATTERN | OFFSET | |
| PLAN 2 MIDDAY PLAN OPERATIVE | R2 | 5 | Ţ | 6 | 1 | 8 | 1 | 7 | → | Balanced | 103 | |
| | | | RIN | G 1 | | | RIN | G 2 | | | | |
| | PHASE | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | • | | |
| TIMES | SPLIT | 12 | 65 | 18 | 25 | 18 | 59 | 25 | 18 | Target Cycle Length | | |
| 9:00 | COORD | | Χ | | | | Χ | | | 120 | | |
| | RECALLS | | V | | | | V | | | Actual Cyc | cle Length | |
| | GREEN | 5.6 | 58.9 | 13.0 | 19.4 | 11.6 | 52.9 | 19.4 | 12.6 | 12 | 20 | |
| | R1 | 2 | → | 1 | Ţ | 3 | Ţ | 4 | 1 | COORD PATTERN | OFFSET | |
| PLAN 3 | R2 | 5 | 1 | 6 | 1 | 8 | 1 | 7 | 1 | Balanced | 4 | |
| PM PLAN | | | RIN | G 1 | | | RIN | IG 2 | | | | |
| OPERATIVE | PHASE | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | |
| TIMES | SPLIT | 12 | 55 | 16 | 37 | 18 | 49 | 37 | 16 | Target Cyc | | |
| 15:00 | COORD | | Х | | | | Х | | | 12 | | |
| | RECALLS | | V | | | | V | | | Actual Cyc | | |
| | GREEN | 5.6 | 48.9 | 11.0 | 31.4 | 11.6 | 42.9 | 31.4 | 10.6 | 12 | 20 | |
| | R1 | 2 | → | 1 | Ţ | 3 | — | 4 | 1 | COORD PATTERN | OFFSET | |
| PLAN 4 | R2 | 5 | 1 | 6 | 1 | 8 | 1 | 7 | → | Balanced | 84 | |
| MIDNIGHT | | | RIN | G 1 | | | RIN | IG 2 | | | | |
| PLAN OPERATIVE | PHASE | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | |
| TIMES | SPLIT | 12 | 43 | 12 | 23 | 14 | 41 | 21 | 14 | Target Cyc | cle Length | |
| 22:00 | COORD | | Х | | | | Х | | | 90 | | |
| | RECALLS | | V | | | | V | | | Actual Cycle Length | | |
| | GREEN | 5.6 | 36.9 | 7.0 | 17.4 | 7.6 | 34.9 | 15.4 | 8.6 | 9 | 0 | |



Appendix F – Existing Capacity Analysis



5-GP-2020 5/22/2020

| | • | → | \rightarrow | • | ← | • | 4 | † | / | > | ļ | 4 |
|------------------------------|----------|-------------|---------------|-------|----------|-------|----------|----------|------|-------------|----------|----------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ሻሻ | ↑ ↑₽ | | ሻ | ^ | 7 | ሻሻ | ₽ | | ሻሻ | ^ | 7 |
| Traffic Volume (veh/h) | 88 | 1121 | 33 | 9 | 1688 | 467 | 53 | 94 | 14 | 508 | 37 | 145 |
| Future Volume (veh/h) | 88 | 1121 | 33 | 9 | 1688 | 467 | 53 | 94 | 14 | 508 | 37 | 145 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | 40-0 | No | 10-0 | 10=0 | No | 10-0 | 10-0 | No | 10=0 | 10-0 | No | 10-0 |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 94 | 1193 | 35 | 10 | 1796 | 0 | 56 | 100 | 15 | 540 | 39 | 154 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 144 | 2205 | 65 | 307 | 2864 | 0.00 | 144 | 126 | 19 | 415 | 572 | 255 |
| Arrive On Green | 0.04 | 0.43 | 0.43 | 0.17 | 0.56 | 0.00 | 0.04 | 0.08 | 0.08 | 0.12 | 0.16 | 0.16 |
| Sat Flow, veh/h | 3456 | 5098 | 150 | 1781 | 5106 | 1585 | 3456 | 1589 | 238 | 3456 | 3554 | 1585 |
| Grp Volume(v), veh/h | 94 | 797 | 431 | 10 | 1796 | 0 | 56 | 0 | 115 | 540 | 39 | 154 |
| Grp Sat Flow(s),veh/h/ln | 1728 | 1702 | 1843 | 1781 | 1702 | 1585 | 1728 | 0 | 1827 | 1728 | 1777 | 1585 |
| Q Serve(g_s), s | 3.2 | 20.8 | 20.8 | 0.6 | 28.6 | 0.0 | 1.9 | 0.0 | 7.4 | 14.4 | 1.1 | 10.8 |
| Cycle Q Clear(g_c), s | 3.2 | 20.8 | 20.8 | 0.6 | 28.6 | 0.0 | 1.9 | 0.0 | 7.4 | 14.4 | 1.1 | 10.8 |
| Prop In Lane | 1.00 | | 0.08 | 1.00 | 2221 | 1.00 | 1.00 | | 0.13 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 144 | 1472 | 797 | 307 | 2864 | | 144 | 0 | 145 | 415 | 572 | 255 |
| V/C Ratio(X) | 0.65 | 0.54 | 0.54 | 0.03 | 0.63 | | 0.39 | 0.00 | 0.79 | 1.30 | 0.07 | 0.60 |
| Avail Cap(c_a), veh/h | 161 | 1472 | 797 | 307 | 2864 | 4.00 | 173 | 0 | 375 | 415 | 989 | 441 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 56.7 | 25.2 | 25.2 | 41.3 | 17.9 | 0.0 | 56.0 | 0.0 | 54.3 | 52.8 | 42.7 | 46.8 |
| Incr Delay (d2), s/veh | 5.3 | 1.4 | 2.6 | 0.0 | 1.1 | 0.0 | 0.6 | 0.0 | 3.7 | 152.6 | 0.0 | 0.9 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ln | 1.5 | 8.2 | 9.1 | 0.2 | 10.3 | 0.0 | 8.0 | 0.0 | 3.6 | 14.9 | 0.5 | 4.3 |
| Unsig. Movement Delay, s/veh | | 06.7 | 27.0 | 44.2 | 10.0 | 0.0 | EC C | 0.0 | E0.0 | 205.4 | 40.7 | 47 C |
| LnGrp Delay(d),s/veh | 62.0 | 26.7 | 27.9 | 41.3 | 18.9 | 0.0 | 56.6 | 0.0 | 58.0 | | 42.7 | 47.6 |
| LnGrp LOS | <u>E</u> | C 4200 | С | D | B | Δ. | <u>E</u> | A 474 | E | F | D | <u>D</u> |
| Approach Vol, veh/h | | 1322 | | | 1806 | А | | 171 | | | 733 | |
| Approach Delay, s/veh | | 29.6 | | | 19.0 | | | 57.5 | | | 163.6 | |
| Approach LOS | | С | | | В | | | Е | | | Г | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), s | 27.1 | 58.0 | 10.0 | 24.9 | 11.4 | 73.7 | 20.0 | 14.9 | | | | |
| Change Period (Y+Rc), s | 6.4 | 6.1 | 5.0 | * 5.6 | 6.4 | * 6.4 | * 5.6 | 5.4 | | | | |
| Max Green Setting (Gmax), s | 5.6 | 51.9 | 6.0 | * 33 | 5.6 | * 52 | * 14 | 24.6 | | | | |
| Max Q Clear Time (g_c+I1), s | 2.6 | 22.8 | 3.9 | 12.8 | 5.2 | 30.6 | 16.4 | 9.4 | | | | |
| Green Ext Time (p_c), s | 0.0 | 1.0 | 0.0 | 0.0 | 0.0 | 2.3 | 0.0 | 0.1 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 50.4 | | | | | | | | | |
| HCM 6th LOS | | | D | | | | | | | | | |

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - Existing AM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

| | • | → | 4 | 4 | ۶ | 4. | > | † | |
|---|-------------|-----------|---------|-------|------------|-------|-------------|----------|--|
| Phase Number | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | |
| Movement | WBL | EBT | NBL | SBT | EBL | WBT | SBL | NBT | |
| Lead/Lag | Lag | Lead | Lead | Lag | Lead | Lag | Lag | Lead | |
| Lead-Lag Optimize | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Recall Mode | None | C-Max | None | None | None | C-Max | None | None | |
| Maximum Split (s) | 12 | 58 | 11 | 39 | 12 | 58 | 20 | 30 | |
| Maximum Split (%) | 10.0% | 48.3% | 9.2% | 32.5% | 10.0% | 48.3% | 16.7% | 25.0% | |
| Minimum Split (s) | 11.4 | 31.1 | 10 | 40.6 | 11.4 | 32.1 | 10.6 | 37.4 | |
| Yellow Time (s) | 4.4 | 5.1 | 3 | 4.4 | 4.4 | 5.1 | 3.6 | 3.6 | |
| All-Red Time (s) | 2 | 1 | 2 | 1.2 | 2 | 1 | 2 | 1.8 | |
| Minimum Initial (s) | 5 | 10 | 5 | 7 | 5 | 10 | 5 | 7 | |
| Vehicle Extension (s) | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | |
| Minimum Gap (s) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | |
| Time Before Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Time To Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Walk Time (s) | | 4 | | 4 | | 4 | | 4 | |
| Flash Dont Walk (s) | | 21 | | 31 | | 22 | | 28 | |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | Yes | Yes | |
| Inhibit Max | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Start Time (s) | 90 | 32 | 102 | 113 | 32 | 44 | 12 | 102 | |
| End Time (s) | 102 | 90 | 113 | 32 | 44 | 102 | 32 | 12 | |
| Yield/Force Off (s) | 95.6 | 83.9 | 108 | 26.4 | 37.6 | 95.9 | 26.4 | 6.6 | |
| Yield/Force Off 170(s) | 95.6 | 62.9 | 108 | 115.4 | 37.6 | 73.9 | 26.4 | 98.6 | |
| Local Start Time (s) | 46 | 108 | 58 | 69 | 108 | 0 | 88 | 58 | |
| Local Yield (s) | 51.6 | 39.9 | 64 | 102.4 | 113.6 | 51.9 | 102.4 | 82.6 | |
| Local Yield 170(s) | 51.6 | 18.9 | 64 | 71.4 | 113.6 | 29.9 | 102.4 | 54.6 | |
| Intersection Summary | | | | | | | | | |
| Cycle Length | | | 120 | | | | | | |
| Control Type | Actu | ated-Coor | dinated | | | | | | |
| Natural Cycle | | | 125 | | | | | | |
| Offset: 44 (37%), Referenced to phase 2:EBT and 6:WBT, Start of Green | | | | | | | | | |
| Splits and Phases: 1: 114 | th Street 8 | Shea Bo | ulevard | | | | | | |
| →Ø2 (R) • | | | | | √ ø | , | \ ø3 | ₩ Ø4 | |

†_{Ø8}

| Intersection Int Delay, s/veh | | | | | | |
|---|--|--------------------|-----------------------------|---|----------------------------|-------------------------|
| iiit Delay, S/Vell | 1.4 | | | | | |
| | | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ¥ | | ₽ | | 7 | ^ |
| Traffic Vol, veh/h | 0 | 11 | 149 | 2 | 29 | 47 |
| Future Vol, veh/h | 0 | 11 | 149 | 2 | 29 | 47 |
| Conflicting Peds, #/hr | . 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 100 | - |
| Veh in Median Storag | je,# 0 | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 88 | 88 | 88 | 88 | 88 | 88 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mymt Flow | 0 | 13 | 169 | 2 | 33 | 53 |
| | | - 10 | .00 | _ | - 00 | - 00 |
| | | | | | | |
| Major/Minor | Minor1 | | /lajor1 | | Major2 | |
| Conflicting Flow All | 263 | 170 | 0 | 0 | 171 | 0 |
| Stage 1 | 170 | - | - | - | - | - |
| Stage 2 | 93 | - | - | - | - | - |
| Critical Hdwy | 6.63 | 6.23 | - | - | 4.13 | - |
| Critical Hdwy Stg 1 | 5.43 | - | _ | - | _ | - |
| Critical Hdwy Stg 2 | 5.83 | _ | - | - | - | - |
| Follow-up Hdwy | 3.519 | 3.319 | _ | - | 2.219 | - |
| Pot Cap-1 Maneuver | 715 | 873 | _ | _ | 1405 | _ |
| Stage 1 | 859 | - | _ | _ | - | _ |
| Stage 2 | 921 | _ | _ | _ | _ | _ |
| Olage Z | 021 | | | | | _ |
| Platoon blocked % | | | _ | _ | | _ |
| Platoon blocked, % | | 272 | - | - | 1/105 | - |
| Mov Cap-1 Maneuver | f 699 | 873 | - | - | 1405 | - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver | 699 719 | - | - - - | | 1405 | |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 | 699 719 859 | - | - - - | - - - | - | - - - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver | 699 719 | - | - | - | | - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 | 699 719 859 | - | - - - | - - - | - | - - - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 | 699 719 859 | - | - - - | - - - | - | - - - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach | 7 699 719 859 900 WB | - | - - - | - - - | - - - SB | - - - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s | 699 719 859 900 WB | - | - - - - NB | - - - | - - - | - - - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach | 7 699 719 859 900 WB | - | - - - - NB | - - - | - - - SB | - - - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS | 7 699 719 859 900 WB 9.2 A | | - - - - NB 0 | - | SB 2.9 | - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvi | 7 699 719 859 900 WB 9.2 A | - | - - - - NB 0 | - - - - VBLn1 | - - - SB 2.9 | - - - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvi Capacity (veh/h) | 7 699 719 859 900 WB 9.2 A | | - - - - NB 0 | - - - - - - - - 873 | SB 2.9 SBL 1405 | - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvi Capacity (veh/h) HCM Lane V/C Ratio | 7 699 7 719 859 900 WB S 9.2 A | - - - NBT | - - - - NB 0 | - - - - - - - - - - - - - - - - - - - | SB 2.9 SBL 1405 0.023 | - - - - SBT |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvi Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s | 7 699 7 719 859 900 WB S 9.2 A | - - - NBT | - - - - NB 0 | - - - - - - 873 0.014 9.2 | SB 2.9 SBL 1405 0.023 7.6 | - - - - SBT |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvi Capacity (veh/h) HCM Lane V/C Ratio | 7 699 719 859 900 WB S 9.2 A | - - - NBT | - - - - NB 0 | - - - - - - - - - - - - - - - - - - - | SB 2.9 SBL 1405 0.023 | - - - - SBT |

| | • | → | • | • | ← | • | 4 | † | ~ | > | ţ | 1 |
|------------------------------|------|-------------|------|------------|----------|-------|-------|----------|------|-------------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ሻሻ | ↑ ↑₽ | | ሻ | ^ | 7 | ሻሻ | ₽ | | ሻሻ | ^ | 7 |
| Traffic Volume (veh/h) | 215 | 1662 | 42 | 19 | 891 | 524 | 41 | 75 | 18 | 775 | 89 | 102 |
| Future Volume (veh/h) | 215 | 1662 | 42 | 19 | 891 | 524 | 41 | 75 | 18 | 775 | 89 | 102 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | 4070 | No | 4070 | 4070 | No | 4070 | 4070 | No | 4070 | 4070 | No | 4070 |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 222 | 1713 | 43 | 20 | 919 | 0 | 42 | 77 | 19 | 799 | 92 | 105 |
| Peak Hour Factor | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 | 0.97 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 278 | 2088 | 52 | 143 | 2067 | 0.00 | 144 | 100 | 25 | 855 | 988 | 441 |
| Arrive On Green | 0.08 | 0.41 | 0.41 | 0.08 | 0.40 | 0.00 | 0.04 | 0.07 | 0.07 | 0.25 | 0.28 | 0.28 |
| Sat Flow, veh/h | 3456 | 5123 | 129 | 1781 | 5106 | 1585 | 3456 | 1449 | 357 | 3456 | 3554 | 1585 |
| Grp Volume(v), veh/h | 222 | 1138 | 618 | 20 | 919 | 0 | 42 | 0 | 96 | 799 | 92 | 105 |
| Grp Sat Flow(s),veh/h/ln | 1728 | 1702 | 1847 | 1781 | 1702 | 1585 | 1728 | 0 | 1806 | 1728 | 1777 | 1585 |
| Q Serve(g_s), s | 7.6 | 35.7 | 35.7 | 1.3 | 15.7 | 0.0 | 1.4 | 0.0 | 6.3 | 27.2 | 2.3 | 6.1 |
| Cycle Q Clear(g_c), s | 7.6 | 35.7 | 35.7 | 1.3 | 15.7 | 0.0 | 1.4 | 0.0 | 6.3 | 27.2 | 2.3 | 6.1 |
| Prop In Lane | 1.00 | | 0.07 | 1.00 | | 1.00 | 1.00 | | 0.20 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 278 | 1387 | 753 | 143 | 2067 | | 144 | 0 | 125 | 855 | 988 | 441 |
| V/C Ratio(X) | 0.80 | 0.82 | 0.82 | 0.14 | 0.44 | | 0.29 | 0.00 | 0.77 | 0.93 | 0.09 | 0.24 |
| Avail Cap(c_a), veh/h | 334 | 1387 | 753 | 143 | 2067 | 4.00 | 317 | 0 | 160 | 904 | 988 | 441 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 54.2 | 31.6 | 31.7 | 51.3 | 25.9 | 0.0 | 55.8 | 0.0 | 54.9 | 44.2 | 32.1 | 33.5 |
| Incr Delay (d2), s/veh | 9.0 | 5.5 | 9.8 | 0.2 | 0.7 | 0.0 | 0.4 | 0.0 | 11.6 | 15.5 | 0.0 | 0.1 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ln | 3.5 | 14.7 | 16.9 | 0.5 | 6.1 | 0.0 | 0.6 | 0.0 | 3.3 | 13.1 | 1.0 | 2.4 |
| Unsig. Movement Delay, s/veh | | 27.0 | 44.4 | 545 | 00.0 | 0.0 | FC 0 | 0.0 | CC F | F0 7 | 20.4 | 22.0 |
| LnGrp Delay(d),s/veh | 63.2 | 37.2 | 41.4 | 51.5 | 26.6 | 0.0 | 56.2 | 0.0 | 66.5 | 59.7 | 32.1 | 33.6 |
| LnGrp LOS | E | D | D | D | С | | E | A | E | E | С | С |
| Approach Vol, veh/h | | 1978 | | | 939 | А | | 138 | | | 996 | |
| Approach Delay, s/veh | | 41.4 | | | 27.1 | | | 63.4 | | | 54.4 | |
| Approach LOS | | D | | | С | | | Е | | | D | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), s | 16.0 | 55.0 | 10.0 | 39.0 | 16.1 | 55.0 | 35.3 | 13.7 | | | | |
| Change Period (Y+Rc), s | 6.4 | 6.1 | 5.0 | * 5.6 | 6.4 | * 6.4 | * 5.6 | 5.4 | | | | |
| Max Green Setting (Gmax), s | 5.6 | 48.9 | 11.0 | * 31 | 11.6 | * 43 | * 31 | 10.6 | | | | |
| Max Q Clear Time (g_c+I1), s | 3.3 | 37.7 | 3.4 | 8.1 | 9.6 | 17.7 | 29.2 | 8.3 | | | | |
| Green Ext Time (p_c), s | 0.0 | 1.5 | 0.0 | 0.1 | 0.1 | 1.0 | 0.5 | 0.0 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 42.1 | | | | | | | | | |
| HCM 6th LOS | | | D | | | | | | | | | |

Notes

User approved pedestrian interval to be less than phase max green.

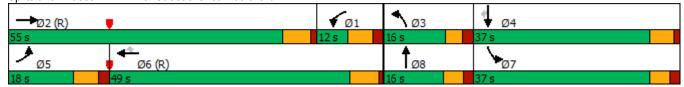
* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - Existing PM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

| | • | → | 4 | 4 | ۶ | 40 | - | † | |
|-------------------------------|-------------|----------|----------|-------------|-------|-------|-------|----------|--|
| Phase Number | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | |
| Movement | WBL | EBT | NBL | SBT | EBL | WBT | SBL | NBT | |
| Lead/Lag | Lag | Lead | Lead | Lag | Lead | Lag | Lag | Lead | |
| Lead-Lag Optimize | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Recall Mode | None | C-Max | None | None | None | C-Max | None | None | |
| Maximum Split (s) | 12 | 55 | 16 | 37 | 18 | 49 | 37 | 16 | |
| Maximum Split (%) | 10.0% | 45.8% | 13.3% | 30.8% | 15.0% | 40.8% | 30.8% | 13.3% | |
| Minimum Split (s) | 11.4 | 31.1 | 10 | 40.6 | 11.4 | 32.1 | 10.6 | 37.4 | |
| Yellow Time (s) | 4.4 | 5.1 | 3 | 4.4 | 4.4 | 5.1 | 3.6 | 3.6 | |
| All-Red Time (s) | 2 | 1 | 2 | 1.2 | 2 | 1 | 2 | 1.8 | |
| Minimum Initial (s) | 5 | 10 | 5 | 7 | 5 | 10 | 5 | 7 | |
| Vehicle Extension (s) | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | |
| Minimum Gap (s) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | |
| Time Before Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Time To Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Walk Time (s) | | 4 | | 4 | | 4 | | 4 | |
| Flash Dont Walk (s) | | 21 | | 31 | | 22 | | 28 | |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | Yes | Yes | |
| Inhibit Max | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Start Time (s) | 41 | 106 | 53 | 69 | 106 | 4 | 69 | 53 | |
| End Time (s) | 53 | 41 | 69 | 106 | 4 | 53 | 106 | 69 | |
| Yield/Force Off (s) | 46.6 | 34.9 | 64 | 100.4 | 117.6 | 46.9 | 100.4 | 63.6 | |
| Yield/Force Off 170(s) | 46.6 | 13.9 | 64 | 69.4 | 117.6 | 24.9 | 100.4 | 35.6 | |
| Local Start Time (s) | 37 | 102 | 49 | 65 | 102 | 0 | 65 | 49 | |
| Local Yield (s) | 42.6 | 30.9 | 60 | 96.4 | 113.6 | 42.9 | 96.4 | 59.6 | |
| Local Yield 170(s) | 42.6 | 9.9 | 60 | 65.4 | 113.6 | 20.9 | 96.4 | 31.6 | |
| Intersection Summary | | | | | | | | | |
| Cycle Length | | | 120 | | | | | | |
| Control Type | Actu | ated-Coo | rdinated | | | | | | |
| Natural Cycle | | | 145 | | | | | | |
| Offset: 4 (3%), Referenced to | o phase 2 | EBT and | 6:WBT, S | Start of Gr | een | | | | |
| Splits and Phases: 1: 114 | th Street 8 | Shoa Bo | ulovard | | | | | | |



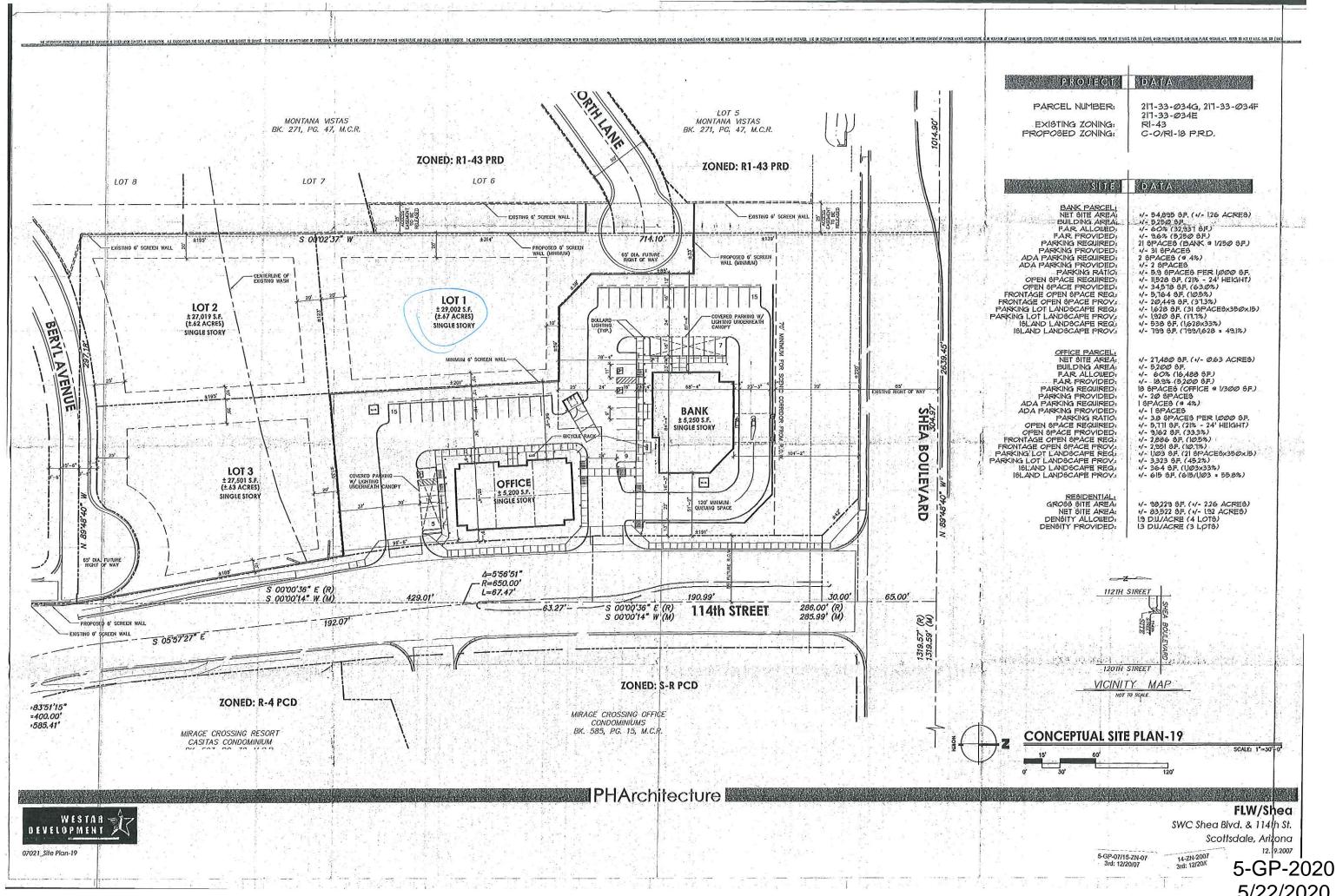
| Intersection | | | | | | |
|------------------------|--------|-------|---------|-------|--------|----------|
| Int Delay, s/veh | 1.2 | | | | | |
| | | 14/55 | NET | NEE | 051 | 0.5. |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ¥ | | ₽ | | - ነ | ^ |
| Traffic Vol, veh/h | 1 | 28 | 104 | 0 | 10 | 146 |
| Future Vol, veh/h | 1 | 28 | 104 | 0 | 10 | 146 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 100 | - |
| Veh in Median Storage | e, # 0 | - | 0 | _ | _ | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 91 | 91 | 91 | 91 | 91 | 91 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 31 | 114 | 0 | 11 | 160 |
| IVIVIIILIIOW | Į. | 31 | 117 | U | - 11 | 100 |
| | | | | | | |
| Major/Minor | Minor1 | N | //ajor1 | | Major2 | |
| Conflicting Flow All | 216 | 114 | 0 | 0 | 114 | 0 |
| Stage 1 | 114 | - | - | - | - | - |
| Stage 2 | 102 | - | _ | _ | _ | _ |
| Critical Hdwy | 6.63 | 6.23 | _ | _ | 4.13 | _ |
| Critical Hdwy Stg 1 | 5.43 | - | _ | _ | - | _ |
| Critical Hdwy Stg 2 | 5.83 | _ | _ | | _ | _ |
| Follow-up Hdwy | 3.519 | 3.319 | _ | _ | 2.219 | _ |
| | 762 | 938 | | | 1474 | |
| Pot Cap-1 Maneuver | | | - | - | 14/4 | - |
| Stage 1 | 910 | - | - | - | - | - |
| Stage 2 | 911 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuver | 757 | 938 | - | - | 1474 | - |
| Mov Cap-2 Maneuver | 760 | - | - | - | - | - |
| Stage 1 | 910 | - | - | - | - | - |
| Stage 2 | 905 | - | - | - | - | - |
| Ŭ | | | | | | |
| | M/D | | ND | | 0.0 | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | 9 | | 0 | | 0.5 | |
| HCM LOS | Α | | | | | |
| | | | | | | |
| Minor Lano/Major Myn | ot . | NBT | NDDV | VBLn1 | SBL | SBT |
| Minor Lane/Major Mvn | IL | INDI | INDIX | | | |
| Capacity (veh/h) | | - | - | | 1474 | - |
| HCM Lane V/C Ratio | | - | - | 0.034 | | - |
| HCM Control Delay (s | | - | - | 9 | 7.5 | - |
| HCM Lane LOS | | - | - | Α | Α | - |
| HCM 95th %tile Q(veh |) | - | - | 0.1 | 0 | - |
| | | | | | | |



Appendix G – Prior Approved Site Plan



G 5-GP-2020 5/22/2020



5/22/2020



Appendix H – Trip Generation









Trip Generation Calculations (10th Edition)

| Drive-in Bank | | | | | | | | | | | | | | | | | | | | | | |
|--|-----------------------------------|---------------------------------|---|--|---------------------------|-------------------|--|---|----------------------------|---|--|-------------------|-----------------------|--------------------------------|-----------------|-----------------------|--------------------|------------------------|---------------------------|-----------------------|--------------------|-----|
| Land Use | ITE | Otv | Unit | Weekday | 1 | | AM Peak I | lour | | PM Peak H | our | | , | Weekday | / | AN | 1 Peak H | our | PΛ | 1 Peak F | our | |
| Land Ose | Code | Qty | Unit | Rate | % In | % Out | Rate | % In | % Out | Rate | % In | % Out | Total | ln | Out | Total | In | Out | Total | In | Out | |
| Drive-in Bank | 912 | 5.25 | 1000 SF GFA | 100.03 | 50% | 50% | 9.50 | 58% | 42% | 20.45 | 50% | 50% | 525 | 263 | 262 | 50 | 29 | 21 | 107 | 54 | 53 |] . |
| Drive-in Bank | 912 | 5.25 | 1000 SF GFA | 32.67 | 50% | 50% | 0.89 | 58% | 42% | 3.04 | 50% | 50% | 172 | 86 | 86 | 5 | 3 | 2 | 16 | 8 | 8 | |
| Drive-in Bank | 912 | 5.25 | 1000 SF GFA | 408.42 | 50% | 50% | 29.47 | 58% | 42% | 109.91 | 50% | 50% | 2,144 | 1,072 | 1,072 | 155 | 90 | 65 | 577 | 289 | 288 | ı |
| Land Use | ITE | 04 | Unit | Weekday | , | | AM Peak I | lour | | PM Peak H | our | | , | Weekday | / | A٨ | 1 Peak H | our | PΛ | A Peak F | our | |
| Land Ose | Code | Qty | Unit | Equation | % In | % Out | Equation | % In | % Out | Equation | % In | % Out | Total | In | Out | Total | In | Out | Total | In | Out | |
| Drive-in Bank | 912 | 5.25 | 1000 SF GFA | T=82.87(X)+117.10 | 50% | 50% | N/A | N/A | N/A | N/A | N/A | N/A | 552 | 276 | 276 | N/A | N/A | N/A | N/A | N/A | N/A | |
| | | | • | | | | | | | • | | | | | | | | | • | | • | _ |
| | | ndard De | | 61.61 | | | 5.85 | | | 15.01 | | | | | | | | | | | | |
| Drive-in Bank | | mber of | | 21 | | | 46 | | | 115 | | | | | | | | | | | | |
| DITVE III DUIN | | Average | Size | 7 | | | 5 | | | 4 | | | | | | | | | | | | |
| | | R ² | | 0.66 | | | N/A | | | N/A | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | |
| General Office Building | | | | | | | | | | | | | | | | | | | | | | _ |
| | | | 1 | | | | | | | | | | | | | | | | | | | |
| Land Use | ITE | Otv | Unit | Weekday | _ | _ | AM Peak I | | | PM Peak H | | | | Weekday | | | 1 Peak H | | | A Peak H | | |
| Land Use | ITE Code | Qty | Unit | Weekday Rate | _ | % Out | AM Peak I Rate | lour % In | % Out | | our % In | % Out | | Weekday In | / Out | AM Total | 1 Peak H In | our Out | PA Total | A Peak H | our Out | |
| Land Use General Office Building | | Qty 5.2 | 1000 SF GFA | | _ | % Out 50% | | | % Out 14% | | | % Out 84% | | | | | | | | | | |
| | Code | | 1000 SF | Rate | % In | | Rate | % In | | Rate | % In | | Total | ln | Out | Total | ln | Out | Total | | Out | - |
| General Office Building | 710 | 5.2 | 1000 SF GFA 1000 SF | Rate 9-74 | % In 50% | 50% | Rate 1.16 | % In 86% | 14% | Rate 1.15 | % In 16% | 84% | Total 51 | ln | Out 25 | Total 6 | In 5 | Out 1 | Total 6 | In 1 | Out 5 | |
| General Office Building General Office Building General Office Building | 710 710 | 5.2 5.2 5.2 | 1000 SF GFA 1000 SF GFA 1000 SF GFA | 9.74 2.71 | % In 50% 50% | 50% 50% | 1.16 0.37 | % In 86% 86% 86% | 14% | 1.15 0.47 | % In 16% 16% | 84% 84% | 51 14 143 | In 26 7 | Out 25 7 71 | Total 6 2 22 | 5 2 | Out 1 0 3 | Total 6 2 17 | 1 0 | Out 5 2 14 | |
| General Office Building General Office Building | 710 710 710 | 5.2 | 1000 SF GFA 1000 SF GFA 1000 SF | 9.74 2.71 27.56 | % In 50% 50% 50% | 50% 50% | 1.16 0.37 4.23 | % In 86% 86% 86% | 14% | Rate 1.15 0.47 3.23 | % In 16% 16% | 84% 84% | Total 51 14 143 | In 26 7 72 | Out 25 7 71 | Total 6 2 22 | 5 2 19 | Out 1 0 3 | Total 6 2 17 | 1 0 3 | Out 5 2 14 | |
| General Office Building General Office Building General Office Building | 710 710 710 1TE | 5.2 5.2 5.2 | 1000 SF GFA 1000 SF GFA 1000 SF GFA | 9.74 2.71 27.56 Weekday | % In 50% 50% 50% | 50% 50% 50% | Rate 1.16 0.37 4.23 AM Peak H | % In 86% 86% 86% | 14% 14% 14% % Out | Rate 1.15 0.47 3.23 PM Peak H | % In 16% 16% 16% our % In | 84% 84% 84% | Total 51 14 143 | In 26 7 72 Weekday | Out 25 7 71 | Total 6 2 22 AM | 1n 5 2 19 1 Peak H | Out 1 0 3 our | Total 6 2 17 PM | In 1 0 3 N Peak F | Out 5 2 14 our | , |
| General Office Building General Office Building General Office Building Land Use | 710 710 TE Code 710 | 5.2 5.2 5.2 Qty 5.2 | 1000 SF GFA 1000 SF GFA 1000 SF GFA Unit 1000 SF | Rate 9.74 2.71 27.56 Weekday Equation Ln(T)=0.97Ln(X)+2.50 | % In 50% 50% 50% | 50% 50% 50% | Rate 1.16 0.37 4.23 AM Peak F Equation T=.94(X)+26.49 | % In 86% 86% 86% Iour % In | 14% 14% 14% % Out | Rate 1.15 0.47 3.23 PM Peak H Equation Ln(T)=0.95Ln(X)+0.36 | % In 16% 16% 16% our % In | 84% 84% 84% | Total 51 14 143 Total | In 26 7 72 Weekday | Out 25 7 71 Out | Total 6 2 22 AN Total | 1n 5 2 19 1 Peak H | Out 1 0 3 our Out | Total 6 2 17 PM Total | In 1 0 3 N Peak F | Out 5 2 14 our Out | , |
| General Office Building General Office Building General Office Building Land Use | Code | 5.2 5.2 5.2 Qty 5.2 | 1000 SF GFA 1000 SF GFA 1000 SF GFA Unit 1000 SF GFA | Rate 9.74 2.71 27.56 Weekday Equation Ln(T)=0.97Ln(X)+2.50 | % In 50% 50% 50% | 50% 50% 50% | Rate 1.16 0.37 4.23 AM Peak I Equation T=.94(X)+26.49 0.47 | % In 86% 86% 86% Iour % In | 14% 14% 14% % Out | Rate 1.15 0.47 3.23 PM Peak H Equation | % In 16% 16% 16% our % In | 84% 84% 84% | Total 51 14 143 Total | In 26 7 72 Weekday | Out 25 7 71 Out | Total 6 2 22 AN Total | 1n 5 2 19 1 Peak H | Out 1 0 3 our Out | Total 6 2 17 PM Total | In 1 0 3 N Peak F | Out 5 2 14 our Out | , |
| General Office Building General Office Building General Office Building Land Use | 710 710 710 1TE Code 710 Sta Nui | 5.2 5.2 5.2 Qty 5.2 | 1000 SF GFA 1000 SF GFA 1000 SF GFA Unit 1000 SF GFA eviation Studies | Rate 9.74 2.71 27.56 Weekday Equation Ln(T)=0.97Ln(X)+2.50 | % In 50% 50% 50% | 50% 50% 50% | Rate 1.16 0.37 4.23 AM Peak F Equation T=.94(X)+26.49 | % In 86% 86% 86% Iour % In | 14% 14% 14% % Out | Rate 1.15 0.47 3.23 PM Peak H Equation Ln(T)=0.95Ln(X)+0.36 | % In 16% 16% 16% our % In | 84% 84% 84% | Total 51 14 143 Total | In 26 7 72 Weekday | Out 25 7 71 Out | Total 6 2 22 AN Total | 1n 5 2 19 1 Peak H | Out 1 0 3 our Out | Total 6 2 17 PM Total | In 1 0 3 N Peak F | Out 5 2 14 our Out | , |



Trip Generation Calculations (10th Edition)

| o Medical-Dental Office Building | | | | | | | | | | | | | | | | | | | | | | 1 |
|----------------------------------|-------------|---------------------------|----------------|----------------------|------|-------|----------------------|------|-------|------------------------|------|-------|-------|---------------|----------|-------------|----------------|------------|-------------|----------|------------|----------|
| | ITE | | | Weekday | | | AM Peak Ho | our | | PM Peak Ho | our | | | Weekday | , | ΑΛ | 1 Peak H | our | PΛ | 1 Peak H | our | |
| Land Use | Code | Qty | Unit | Rate | | % Out | Rate | % In | % Out | Rate | | % Out | Total | In | Out | Total | In | Out | Total | In | Out | |
| Medical-Dental Office Building | 720 | 7.2 | 1000 SF GFA | 34.80 | 50% | 50% | 2.78 | 78% | 22% | 3.46 | 28% | 72% | 252 | 126 | 126 | 20 | 16 | 4 | 25 | 7 | 18 | Average |
| Medical-Dental Office Building | 720 | 7.2 | 1000 SF GFA | 9.14 | 50% | 50% | 0.85 | 78% | 22% | 0.25 | 28% | 72% | 66 | 33 | 33 | 6 | 5 | 1 | 2 | 1 | 1 | Minimun |
| Medical-Dental Office Building | 720 | 7.2 | 1000 SF GFA | 100.75 | 50% | 50% | 14.30 | 78% | 22% | 8.86 | 28% | 72% | 729 | 365 | 364 | 103 | 80 | 23 | 64 | 18 | 46 | Maximur |
| Land Use | ITE Code | Qty | Unit | Weekday Equation | | % Out | AM Peak Ho | _ | % Out | PM Peak Ho Equation | | % Out | Total | Weekday In | / Out | AN Total | l Peak H In | our Out | PA Total | A Peak H | our Out | |
| Medical-Dental Office Building | 720 | 7.2 | 1000 SF GFA | T=38.42(X)-87.62 | 50% | 50% | Ln(T)=0.89Ln(X)+1.31 | 78% | 22% | T=3.39(X)+2.02 | 28% | 72% | 190 | 95 | 95 | 22 | 17 | 5 | 27 | 8 | 19 | Equation |
| | | | | | | | | | | | | | | | | | | | | | | |
| | | ndard De | | 9.79 | | | 1.28 | | | 1.58 | | | | | | | | | | | | |
| Medical-Dental Office Building | | mber of | | 28 | | | 44 | | | 65 | | | | | | | | | | | | |
| | | Average | Size | 24 | | | 32 | | | 28 | | | | | | | | | | | | |
| | | R ² | | 0.95 | | | 0.80 | | | 0.73 | | | | | | | | | | | | |
| o Shopping Center | | | | | | | | | | | | | | | | | | | | | | 1 |
| | ITE | 0. | | Weekday | | | AM Peak Ho | our | | PM Peak Ho | our | | | Weekda | , | A٨ | 1 Peak H | our | P۸ | 1 Peak H | our | |
| Land Use | Code | Qty | Unit | Rate | % In | % Out | Rate | % In | % Out | Rate | % In | % Out | Total | ln | Out | Total | ln | Out | Total | In | Out | |
| Shopping Center | 820 | 8.1 | 1000 SF GLA | 37-75 | 50% | 50% | 0.94 | 62% | 38% | 3.81 | 48% | 52% | 306 | 153 | 153 | 8 | 5 | 3 | 31 | 15 | 16 | Average |
| Shopping Center | 820 | 8.1 | 1000 SF GLA | 7.42 | 50% | 50% | 0.18 | 62% | 38% | 0.74 | 48% | 52% | 60 | 30 | 30 | 1 | 1 | 0 | 6 | 3 | 3 | Minimun |
| Shopping Center | 820 | 8.1 | 1000 SF GLA | 207.98 | 50% | 50% | 23.74 | 62% | 38% | 18.69 | 48% | 52% | 1,685 | 843 | 842 | 192 | 119 | 73 | 151 | 72 | 79 | Maximur |
| Land Use | ITE | Qty | Unit | Weekday | | | AM Peak Ho | | | PM Peak Ho | | | | Weekday | | | l Peak H | | | 1 Peak H | | |
| 20.10 030 | Code | 90) | | Equation | % In | % Out | Equation | % In | % Out | Equation | % In | % Out | Total | In | Out | Total | In | Out | Total | In | Out | |
| Shopping Center | 820 | 8.1 | 1000 SF GLA | Ln(T)=0.68Ln(X)+5.57 | 50% | 50% | T=0.50(X)+151.78 | 62% | 38% | Ln(T)=0.74Ln(X)+2.89 | 48% | 52% | 1,088 | 544 | 544 | 156 | 97 | 59 | 85 | 41 | 44 | Equation |
| | C | | | | | | - 0- | | | | | | | | | | | _ | | | | ł |
| | | ndard De | | 16.41 | | | 0.87 | | | 2.04 | | | | | | | | | | | | |
| Shopping Center | | Mber of a | | 147 | | | 84 | | | 261 | | | | | | | | | | | | |
| | | Average R ² | 3126 | 453 0.76 | | | 351 | | | 327 0.82 | | | | | | | | | | | | |
| | | | | | | | 0.50 | | | | | | | | | | | | | | | |



Appendix I – MAG Socioeconomic Projections



Socioeconomic Projections

Population and Employment

by Municipal Planning Area, Jurisdiction, and Regional Analysis Zone

June 2019





302 North 1st Avenue, Suite 300 Phoenix, Arizona 85003 (602) 254-6300

Maricopa Association of Governments

Table 1: Total Population by Municipal Planning Area July 1, 2018 and Projections July 1, 2020 to July 1, 2055

| | | | Total Po | pulation | | |
|--|-----------|-----------|-----------|-----------|-----------|-----------|
| Municipal Planning Area | 2018 | 2020 | 2030 | 2040 | 2050 | 2055 |
| Apache Junction | 59,000 | 60,800 | 70,000 | 92,000 | 117,100 | 132,600 |
| Avondale | 84,200 | 86,700 | 101,800 | 111,900 | 119,000 | 122,100 |
| Buckeye | 89,000 | 97,700 | 186,600 | 305,400 | 409,900 | 459,300 |
| Carefree | 3,700 | 3,800 | 4,100 | 4,200 | 4,200 | 4,300 |
| Cave Creek | 5,900 | 6,000 | 6,500 | 7,000 | 7,200 | 7,300 |
| Chandler | 270,300 | 279,500 | 309,100 | 321,100 | 329,000 | 332,400 |
| El Mirage | 34,300 | 35,100 | 36,500 | 36,900 | 37,200 | 37,200 |
| Florence | 79,400 | 85,500 | 120,300 | 160,500 | 209,900 | 231,400 |
| Fort McDowell Yavapai Native Nation | 1,000 | 1,100 | 1,100 | 1,100 | 1,100 | 1,100 |
| Fountain Hills | 24,000 | 24,700 | 26,200 | 26,600 | 26,900 | 27,000 |
| Gila Bend | 2,500 | 2,700 | 3,700 | 3,700 | 3,900 | 4,200 |
| Gila River Indian Native Nation | 12,000 | 12,200 | 12,300 | 12,300 | 12,300 | 12,300 |
| Gilbert | 256,500 | 265,900 | 293,500 | 308,800 | 318,100 | 321,400 |
| Glendale | 272,200 | 279,100 | 306,400 | 323,400 | 333,200 | 338,800 |
| Goodyear | 87,300 | 92,100 | 140,300 | 192,200 | 228,600 | 247,900 |
| Guadalupe | 6,300 | 6,400 | 6,700 | 6,800 | 6,800 | 6,800 |
| Litchfield Park | 13,300 | 14,000 | 15,400 | 15,700 | 16,100 | 16,400 |
| Maricopa | 59,800 | 67,000 | 90,800 | 106,400 | 121,600 | 128,900 |
| Mesa | 533,400 | 552,800 | 607,500 | 649,400 | 680,000 | 690,300 |
| Paradise Valley | 14,000 | 14,100 | 14,700 | 15,100 | 15,200 | 15,300 |
| Peoria | 188,500 | 196,600 | 232,400 | 273,700 | 312,600 | 329,900 |
| Phoenix | 1,653,500 | 1,697,700 | 1,881,900 | 2,019,300 | 2,117,400 | 2,155,300 |
| Queen Creek | 58,700 | 65,000 | 90,900 | 109,000 | 120,900 | 128,500 |
| Salt River Pima-Maricopa Native Nation | 6,800 | 6,100 | 5,700 | 5,800 | 5,800 | 5,800 |
| Scottsdale | 245,500 | 253,800 | 281,900 | 299,400 | 311,400 | 316,700 |
| Surprise | 144,000 | 150,300 | 216,700 | 307,500 | 383,300 | 417,200 |
| Tempe | 185,300 | 190,000 | 217,100 | 247,000 | 272,400 | 282,200 |
| Tolleson | 7,000 | 7,100 | 8,600 | 10,300 | 11,400 | 11,800 |
| Unicorporated Pinal County | 66,800 | 68,600 | 79,100 | 93,700 | 110,800 | 122,700 |
| Unincorporated Maricopa County | 97,900 | 101,200 | 110,500 | 116,800 | 137,000 | 152,600 |
| Wickenburg | 8,200 | 8,500 | 9,400 | 9,500 | 9,800 | 10,000 |
| Youngtown | 6,600 | 6,800 | 7,300 | 7,700 | 7,800 | 7,800 |

Notes: Numbers rounded to the nearest 100. These projections include both the Maricopa County and Pinal County portions for Apache Junction, Queen Creek, and the Gila River Indian Community. Peoria and Wickenburg include only the Maricopa County portion.

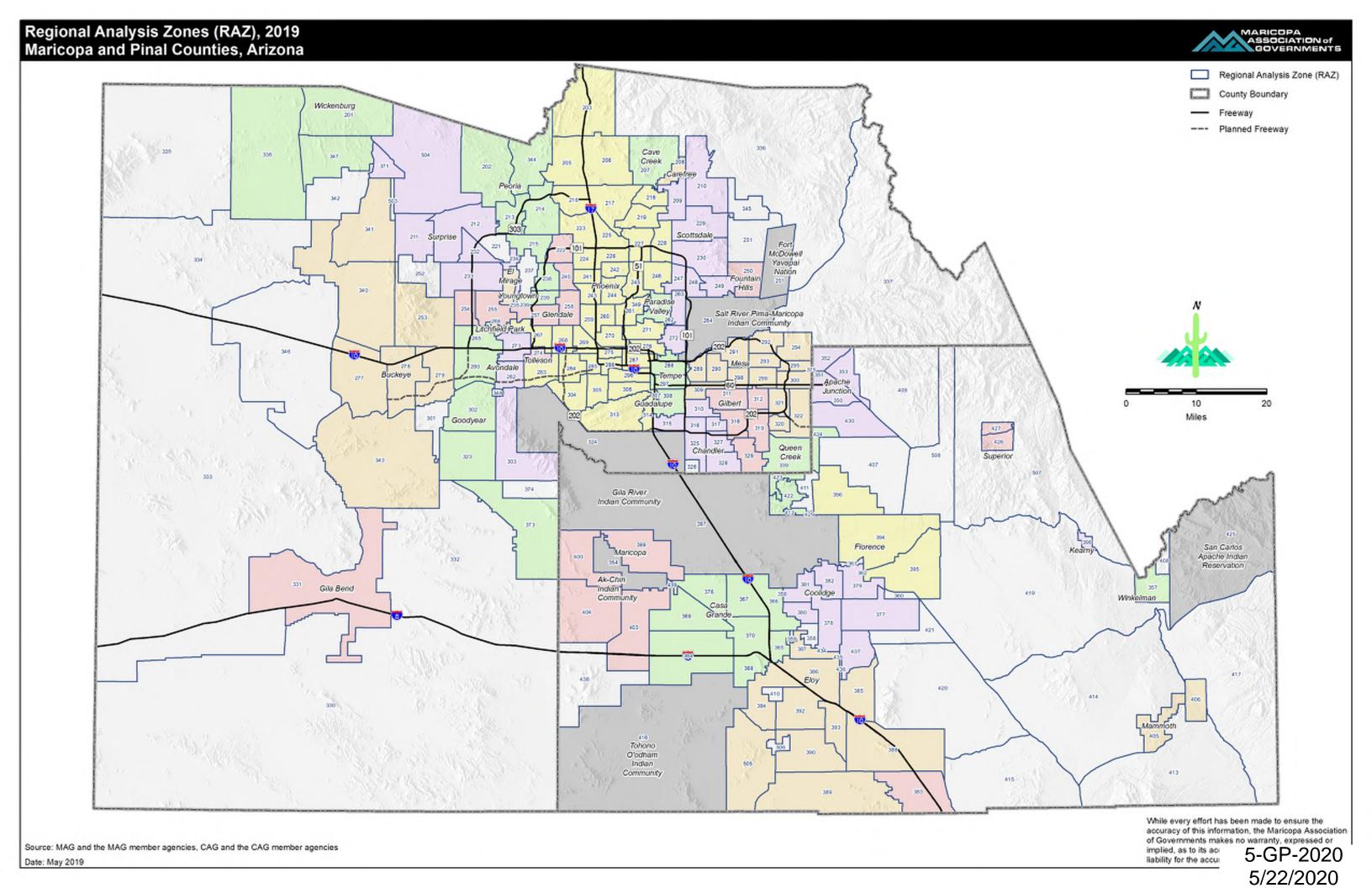
Source: Maricopa Association of Governments (MAG) Socioeconomic Projections of Population and Employment by Municipal Planning Area (MPA) and Regional Analysis Zone (RAZ), June 2019

Maricopa Association of Governments Table 2: Total Employment by Municipal Planning Area July 1, 2018 and Projections July 1, 2020 to July 1, 2055

| | | | Total Em | ployment | | |
|--|---------|---------|-----------|-----------|-----------|-----------|
| Municipal Planning Area | 2018 | 2020 | 2030 | 2040 | 2050 | 2055 |
| Apache Junction | 7,800 | 8,800 | 13,100 | 17,800 | 26,400 | 30,500 |
| Avondale | 22,400 | 23,200 | 30,400 | 36,200 | 42,800 | 45,400 |
| Buckeye | 21,600 | 26,900 | 42,900 | 64,500 | 98,000 | 128,900 |
| Carefree | 1,600 | 1,600 | 2,100 | 2,400 | 2,500 | 2,600 |
| Cave Creek | 2,200 | 2,400 | 2,700 | 2,900 | 3,000 | 3,200 |
| Chandler | 145,500 | 154,700 | 182,300 | 202,100 | 215,200 | 222,000 |
| El Mirage | 5,000 | 5,100 | 6,500 | 7,200 | 8,000 | 8,900 |
| Florence | 11,000 | 12,100 | 17,000 | 26,400 | 40,900 | 51,100 |
| Fort McDowell Yavapai Native Nation | 2,200 | 2,400 | 2,400 | 2,500 | 2,600 | 2,600 |
| Fountain Hills | 7,100 | 7,700 | 9,100 | 9,800 | 10,200 | 10,300 |
| Gila Bend | 900 | 900 | 1,200 | 1,300 | 1,500 | 1,700 |
| Gila River Indian Native Nation | 10,500 | 10,700 | 11,500 | 13,100 | 14,800 | 15,500 |
| Gilbert | 92,800 | 98,600 | 120,200 | 135,900 | 146,600 | 152,200 |
| Glendale | 103,800 | 111,400 | 134,000 | 153,100 | 168,900 | 175,900 |
| Goodyear | 35,900 | 37,200 | 50,600 | 69,000 | 92,600 | 102,500 |
| Guadalupe | 1,300 | 1,300 | 1,500 | 1,600 | 1,600 | 1,600 |
| Litchfield Park | 3,800 | 4,400 | 5,200 | 5,900 | 6,400 | 6,700 |
| Maricopa | 6,200 | 7,100 | 11,400 | 18,200 | 28,200 | 33,500 |
| Mesa | 197,200 | 205,900 | 249,000 | 296,000 | 333,700 | 351,000 |
| Paradise Valley | 6,300 | 6,300 | 6,800 | 7,100 | 7,500 | 7,700 |
| Peoria | 58,200 | 62,400 | 73,100 | 84,800 | 91,900 | 96,300 |
| Phoenix | 897,700 | 937,600 | 1,084,000 | 1,189,200 | 1,264,900 | 1,298,900 |
| Queen Creek | 15,500 | 16,400 | 19,900 | 24,000 | 28,900 | 31,100 |
| Salt River Pima-Maricopa Native Nation | 21,200 | 22,900 | 28,200 | 33,900 | 35,900 | 36,400 |
| Scottsdale | 197,200 | 207,400 | 235,500 | 252,000 | 261,700 | 267,000 |
| Surprise | 33,600 | 36,400 | 59,500 | 86,400 | 113,400 | 130,500 |
| Tempe | 190,000 | 200,500 | 231,200 | 257,700 | 280,000 | 290,900 |
| Tolleson | 17,700 | 18,300 | 21,200 | 23,900 | 26,000 | 26,700 |
| Unicorporated Pinal County | 3,500 | 3,900 | 6,000 | 8,900 | 13,500 | 17,800 |
| Unincorporated Maricopa County | 28,600 | 31,500 | 35,500 | 41,100 | 51,200 | 58,400 |
| Wickenburg | 4,400 | 4,600 | 5,200 | 5,600 | 6,000 | 6,200 |
| Youngtown | 1,500 | 1,800 | 2,200 | 2,700 | 2,800 | 3,100 |

Notes: Numbers rounded to the nearest 100. These projections include both the Maricopa County and Pinal County portions for Apache Junction, Queen Creek, and the Gila River Indian Community. Peoria and Wickenburg include only the Maricopa County portion.

Source: Maricopa Association of Governments (MAG) Socioeconomic Projections of Population and Employment by Municipal Planning Area (MPA) and Regional Analysis Zone (RAZ), June 2019



Maricopa Association of Governments Table 4: Population by Regional Analysis Zone (RAZ) by MPA July 1, 2018 and Projections July 1, 2020 to July 1, 2055

| | | | | Total Pop | ulation | | |
|---------|----------------------------|-----------|-----------|-----------|-----------|-----------|-----------|
| RAZ | County | 2018 | 2020 | 2030 | 2040 | 2050 | 2055 |
| | Total | 1,653,469 | 1,697,722 | 1,881,876 | 2,019,269 | 2,117,427 | 2,155,333 |
| Queen | Creek MPA | | | | | | |
| 339 | Maricopa County | 49,781 | 53,579 | 72,670 | 82,172 | 87,155 | 89,586 |
| 422 | Pinal County | 13 | 13 | 300 | 437 | 564 | 638 |
| 423 | Pinal County | 1,286 | 1,410 | 3,714 | 6,136 | 7,457 | 8,686 |
| 424 | Pinal County | 7,642 | 10,003 | 14,200 | 20,287 | 25,759 | 29,586 |
| | Total | 58,722 | 65,005 | 90,884 | 109,032 | 120,935 | 128,496 |
| Salt Ri | ver Pima-Maricopa Native N | ation MPA | | | | | |
| 264 | Maricopa County | 6,798 | 6,073 | 5,708 | 5,820 | 5,820 | 5,820 |
| | Total | 6,798 | 6,073 | 5,708 | 5,820 | 5,820 | 5,820 |
| Scotts | dale MPA | | | | | | |
| 209 | Maricopa County | 12,188 | 12,605 | 13,961 | 14,512 | 14,984 | 15,255 |
| 210 | Maricopa County | 6,013 | 6,591 | 10,463 | 12,339 | 13,491 | 13,961 |
| 229 | Maricopa County | 20,542 | 21,269 | 25,221 | 27,864 | 29,698 | 30,229 |
| 230 | Maricopa County | 32,232 | 33,028 | 38,882 | 43,580 | 46,789 | 48,510 |
| 247 | Maricopa County | 13,549 | 13,858 | 15,420 | 16,342 | 16,871 | 17,019 |
| 248 | Maricopa County | 36,178 | 37,227 | 38,468 | 38,807 | 39,048 | 39,143 |
| 249 | Maricopa County | 20,903 | 21,410 | 22,543 | 22,768 | 22,839 | 22,848 |
| 263 | Maricopa County | 34,908 | 35,814 | 37,002 | 37,252 | 37,584 | 37,773 |
| 272 | Maricopa County | 68,987 | 71,970 | 79,910 | 85,942 | 90,054 | 91,927 |
| | Total | 245,500 | 253,772 | 281,870 | 299,406 | 311,358 | 316,665 |
| Surpri | se MPA | | | | | | |
| 211 | Maricopa County | 863 | 884 | 4,471 | 23,112 | 36,704 | 40,737 |
| 212 | Maricopa County | 10,265 | 11,365 | 37,615 | 69,296 | 85,862 | 93,806 |
| 232 | Maricopa County | 29,296 | 30,200 | 34,506 | 37,144 | 37,927 | 38,313 |
| 233 | Maricopa County | 87,834 | 91,276 | 111,822 | 119,384 | 123,777 | 126,523 |
| 234 | Maricopa County | 8,969 | 9,467 | 10,460 | 10,878 | 11,335 | 11,488 |
| 371 | Maricopa County | 342 | 344 | 434 | 734 | 2,584 | 4,316 |
| 504 | Maricopa County | 6,460 | 6,718 | 17,425 | 46,912 | 85,127 | 102,004 |
| | Total | 144,029 | 150,254 | 216,733 | 307,460 | 383,316 | 417,187 |
| Tempe | e MPA | | | | | | |
| 288 | Maricopa County | 73,442 | 76,444 | 100,651 | 129,202 | 150,094 | 157,410 |
| 297 | Maricopa County | 53,146 | 54,092 | 56,336 | 57,432 | 61,780 | 64,273 |
| 308 | Maricopa County | 58,756 | 59,473 | 60,120 | 60,348 | 60,476 | 60,559 |
| | Total | 185,344 | 190,009 | 217,107 | 246,982 | 272,350 | 282,242 |

Notes: Numbers rounded to the nearest 100. These projections include both the Maricopa County and Pinal County portions for Apache Junction, Queen Creek, and the Gila River Indian Community. Peoria and Wickenburg include only the Maricopa County portion.

Source: Maricopa Association of Governments (MAG) Socioeconomic Projections of Population and Employment by Municipal Planning Area (MPA) and Regional Analysis Zone (RAZ), May 2019

Maricopa Association of Governments Table 5: Employment by Regional Analysis Zone (RAZ) by MPA July 1, 2018 and Projections July 1, 2020 to July 1, 2055

| | | | | Total Emp | loyment | | |
|----------|----------------------------|-----------|---------|-----------|-----------|-----------|-----------|
| RAZ | County | 2018 | 2020 | 2030 | 2040 | 2050 | 2055 |
| | Total | 897,713 | 937,622 | 1,083,980 | 1,189,209 | 1,264,941 | 1,298,903 |
| Queen | Creek MPA | | | | | | |
| 339 | Maricopa County | 13,933 | 14,696 | 16,482 | 18,825 | 20,733 | 21,151 |
| 422 | Pinal County | 9 | 8 | 18 | 22 | 31 | 39 |
| 423 | Pinal County | 89 | 109 | 351 | 620 | 1,068 | 1,639 |
| 424 | Pinal County | 1,435 | 1,576 | 3,073 | 4,571 | 7,020 | 8,309 |
| | Total | 15,466 | 16,389 | 19,924 | 24,038 | 28,852 | 31,138 |
| Salt Riv | ver Pima-Maricopa Native N | ation MPA | | | | | |
| 264 | Maricopa County | 21,160 | 22,869 | 28,215 | 33,871 | 35,903 | 36,442 |
| | Total | 21,160 | 22,869 | 28,215 | 33,871 | 35,903 | 36,442 |
| Scotts | dale MPA | | | | | | |
| 209 | Maricopa County | 4,488 | 4,659 | 4,851 | 5,174 | 5,161 | 5,344 |
| 210 | Maricopa County | 2,386 | 3,018 | 2,759 | 3,091 | 3,139 | 3,191 |
| 229 | Maricopa County | 9,604 | 10,005 | 11,231 | 11,962 | 12,193 | 12,896 |
| 230 | Maricopa County | 23,272 | 24,919 | 32,112 | 36,968 | 40,834 | 42,136 |
| 247 | Maricopa County | 44,254 | 47,089 | 52,652 | 54,822 | 55,679 | 56,105 |
| 248 | Maricopa County | 29,603 | 30,901 | 33,285 | 34,001 | 34,234 | 34,548 |
| 249 | Maricopa County | 7,409 | 7,692 | 8,179 | 8,684 | 8,906 | 9,045 |
| 263 | Maricopa County | 26,351 | 26,961 | 28,903 | 30,245 | 30,919 | 31,381 |
| 272 | Maricopa County | 49,833 | 52,185 | 61,540 | 67,039 | 70,676 | 72,330 |
| | Total | 197,200 | 207,429 | 235,512 | 251,986 | 261,741 | 266,976 |
| Surpris | se MPA | | | | | | |
| 211 | Maricopa County | 60 | 53 | 1,560 | 3,172 | 4,766 | 7,017 |
| 212 | Maricopa County | 2,008 | 2,338 | 5,821 | 9,965 | 13,362 | 15,709 |
| 232 | Maricopa County | 8,349 | 9,228 | 11,297 | 12,187 | 12,875 | 13,116 |
| 233 | Maricopa County | 19,943 | 21,079 | 32,661 | 44,032 | 52,007 | 57,402 |
| 234 | Maricopa County | 2,588 | 2,711 | 3,354 | 3,922 | 4,239 | 4,386 |
| 371 | Maricopa County | 18 | 20 | 327 | 423 | 2,381 | 2,937 |
| 504 | Maricopa County | 677 | 1,020 | 4,460 | 12,695 | 23,763 | 29,886 |
| | Total | 33,643 | 36,449 | 59,480 | 86,396 | 113,393 | 130,453 |
| Tempe | | | | | | | |
| 288 | Maricopa County | 88,927 | 94,229 | 111,010 | 128,894 | 144,714 | 152,703 |
| 297 | Maricopa County | 44,730 | 47,069 | 53,149 | 57,125 | 60,725 | 62,552 |
| 308 | Maricopa County | 56,380 | 59,208 | 67,052 | 71,701 | 74,542 | 75,596 |
| | Total | 190,037 | 200,506 | 231,211 | 257,720 | 279,981 | 290,851 |

Notes: Numbers rounded to the nearest 100. These projections include both the Maricopa County and Pinal County portions for Apache Junction, Queen Creek, and the Gila River Indian Community. Peoria and Wickenburg include only the Maricopa County portion.

Source: Maricopa Association of Governments (MAG) Socioeconomic Projections of Population and Employment by Municipal Planning Area (MPA) and Regional Analysis Zone (RAZ), May 2019



Appendix J – Year 2021 No Build Capacity Analysis



5-GP-2020 5/22/2020

| | • | → | • | • | ← | • | • | † | / | > | ļ | 4 |
|------------------------------|----------|-------------|------|-------|----------|-------|-------|----------|------|-------------|----------|----------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ሻሻ | ↑ ↑₽ | | ሻ | ^ | 7 | ሻሻ | ₽ | | ሻሻ | ^ | 7 |
| Traffic Volume (veh/h) | 90 | 1135 | 34 | 10 | 1709 | 473 | 54 | 96 | 15 | 515 | 38 | 147 |
| Future Volume (veh/h) | 90 | 1135 | 34 | 10 | 1709 | 473 | 54 | 96 | 15 | 515 | 38 | 147 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | 40-0 | No | 10-0 | 10=0 | No | 10-0 | 40-0 | No | 10=0 | 40-0 | No | 10-0 |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 98 | 1234 | 37 | 11 | 1858 | 0 | 59 | 104 | 16 | 560 | 41 | 160 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 148 | 2203 | 66 | 302 | 2843 | 0.00 | 144 | 130 | 20 | 415 | 582 | 259 |
| Arrive On Green | 0.04 | 0.43 | 0.43 | 0.17 | 0.56 | 0.00 | 0.04 | 0.08 | 0.08 | 0.12 | 0.16 | 0.16 |
| Sat Flow, veh/h | 3456 | 5094 | 153 | 1781 | 5106 | 1585 | 3456 | 1583 | 244 | 3456 | 3554 | 1585 |
| Grp Volume(v), veh/h | 98 | 825 | 446 | 11 | 1858 | 0 | 59 | 0 | 120 | 560 | 41 | 160 |
| Grp Sat Flow(s),veh/h/ln | 1728 | 1702 | 1843 | 1781 | 1702 | 1585 | 1728 | 0 | 1827 | 1728 | 1777 | 1585 |
| Q Serve(g_s), s | 3.4 | 21.8 | 21.8 | 0.6 | 30.4 | 0.0 | 2.0 | 0.0 | 7.7 | 14.4 | 1.2 | 11.3 |
| Cycle Q Clear(g_c), s | 3.4 | 21.8 | 21.8 | 0.6 | 30.4 | 0.0 | 2.0 | 0.0 | 7.7 | 14.4 | 1.2 | 11.3 |
| Prop In Lane | 1.00 | | 0.08 | 1.00 | | 1.00 | 1.00 | | 0.13 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 148 | 1472 | 797 | 302 | 2843 | | 144 | 0 | 150 | 415 | 582 | 259 |
| V/C Ratio(X) | 0.66 | 0.56 | 0.56 | 0.04 | 0.65 | | 0.41 | 0.00 | 0.80 | 1.35 | 0.07 | 0.62 |
| Avail Cap(c_a), veh/h | 161 | 1472 | 797 | 302 | 2843 | 4.00 | 173 | 0 | 374 | 415 | 989 | 441 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 56.6 | 25.5 | 25.5 | 41.6 | 18.5 | 0.0 | 56.1 | 0.0 | 54.1 | 52.8 | 42.5 | 46.7 |
| Incr Delay (d2), s/veh | 6.3 | 1.5 | 2.8 | 0.0 | 1.2 | 0.0 | 0.7 | 0.0 | 3.7 | 172.9 | 0.0 | 0.9 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ln | 1.5 | 8.5 | 9.5 | 0.3 | 11.0 | 0.0 | 0.9 | 0.0 | 3.7 | 16.1 | 0.5 | 4.5 |
| Unsig. Movement Delay, s/veh | | 07.0 | 00.0 | 44.0 | 40.7 | 0.0 | FC 0 | 0.0 | F7 0 | 005.7 | 40 F | 47 C |
| LnGrp Delay(d),s/veh | 62.9 | 27.0 | 28.3 | 41.6 | 19.7 | 0.0 | 56.8 | 0.0 | 57.8 | 225.7 | 42.5 | 47.6 |
| LnGrp LOS | <u>E</u> | C | С | D | B | | E | A | E | F | D | <u>D</u> |
| Approach Vol, veh/h | | 1369 | | | 1869 | А | | 179 | | | 761 | |
| Approach Delay, s/veh | | 30.0 | | | 19.8 | | | 57.5 | | | 178.4 | |
| Approach LOS | | С | | | В | | | Е | | | F | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), s | 26.8 | 58.0 | 10.0 | 25.2 | 11.6 | 73.2 | 20.0 | 15.2 | | | | |
| Change Period (Y+Rc), s | 6.4 | 6.1 | 5.0 | * 5.6 | 6.4 | * 6.4 | * 5.6 | 5.4 | | | | |
| Max Green Setting (Gmax), s | 5.6 | 51.9 | 6.0 | * 33 | 5.6 | * 52 | * 14 | 24.6 | | | | |
| Max Q Clear Time (g_c+I1), s | 2.6 | 23.8 | 4.0 | 13.3 | 5.4 | 32.4 | 16.4 | 9.7 | | | | |
| Green Ext Time (p_c), s | 0.0 | 1.1 | 0.0 | 0.0 | 0.0 | 2.4 | 0.0 | 0.1 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 53.7 | | | | | | | | | |
| HCM 6th LOS | | | D | | | | | | | | | |

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - 2021 No Build AM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

| | • | → | 4 | 4 | ၨ | 44 | > | † | |
|-----------------------------|--------------|-----------|---------|------------|------------|-------|-------------|----------|--|
| Phase Number | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | |
| Movement | WBL | EBT | NBL | SBT | EBL | WBT | SBL | NBT | |
| Lead/Lag | Lag | Lead | Lead | Lag | Lead | Lag | Lag | Lead | |
| Lead-Lag Optimize | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Recall Mode | None | C-Max | None | None | None | C-Max | None | None | |
| Maximum Split (s) | 12 | 58 | 11 | 39 | 12 | 58 | 20 | 30 | |
| Maximum Split (%) | 10.0% | 48.3% | 9.2% | 32.5% | 10.0% | 48.3% | 16.7% | 25.0% | |
| Minimum Split (s) | 11.4 | 31.1 | 10 | 40.6 | 11.4 | 32.1 | 10.6 | 37.4 | |
| Yellow Time (s) | 4.4 | 5.1 | 3 | 4.4 | 4.4 | 5.1 | 3.6 | 3.6 | |
| All-Red Time (s) | 2 | 1 | 2 | 1.2 | 2 | 1 | 2 | 1.8 | |
| Minimum Initial (s) | 5 | 10 | 5 | 7 | 5 | 10 | 5 | 7 | |
| Vehicle Extension (s) | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | |
| Minimum Gap (s) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | |
| Time Before Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Time To Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Walk Time (s) | | 4 | | 4 | | 4 | | 4 | |
| Flash Dont Walk (s) | | 21 | | 31 | | 22 | | 28 | |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | Yes | Yes | |
| Inhibit Max | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Start Time (s) | 90 | 32 | 102 | 113 | 32 | 44 | 12 | 102 | |
| End Time (s) | 102 | 90 | 113 | 32 | 44 | 102 | 32 | 12 | |
| Yield/Force Off (s) | 95.6 | 83.9 | 108 | 26.4 | 37.6 | 95.9 | 26.4 | 6.6 | |
| Yield/Force Off 170(s) | 95.6 | 62.9 | 108 | 115.4 | 37.6 | 73.9 | 26.4 | 98.6 | |
| Local Start Time (s) | 46 | 108 | 58 | 69 | 108 | 0 | 88 | 58 | |
| Local Yield (s) | 51.6 | 39.9 | 64 | 102.4 | 113.6 | 51.9 | 102.4 | 82.6 | |
| Local Yield 170(s) | 51.6 | 18.9 | 64 | 71.4 | 113.6 | 29.9 | 102.4 | 54.6 | |
| Intersection Summary | | | | | | | | | |
| Cycle Length | | | 120 | | | | | | |
| Control Type | Actu | ated-Coor | | | | | | | |
| Natural Cycle | | | 125 | | | | | | |
| Offset: 44 (37%), Reference | ed to phase | 2:EBT ar | d 6:WBT | , Start of | Green | | | | |
| Splits and Phases: 1: 114 | 4th Street 8 | Shea Bo | ulevard | | | | | | |
| ' <u>.</u> | +u1 OUCCL 0 | onea DU | uievaiu | | | 14 | | el . | |
| →Ø2 (R) | | | | | √ ø | | Ø3 | ▼ Ø4 | |
| 58 s | | | | | 12 s | 11: | S | 39 s | |

†_{Ø8}

| Intersection | | | | | | |
|------------------------|--------|-------|----------|-------|--------|----------|
| Int Delay, s/veh | 1.4 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ¥ | 11511 | 1 | TIDIT | ሻ | ^ |
| Traffic Vol, veh/h | 0 | 12 | 151 | 3 | 30 | 48 |
| Future Vol, veh/h | 0 | 12 | 151 | 3 | 30 | 48 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| • | | | | | Free | |
| Sign Control | Stop | Stop | Free | Free | | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 100 | - |
| Veh in Median Storage | | - | 0 | - | - | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 13 | 164 | 3 | 33 | 52 |
| | | | | _ | | |
| | | | | | | |
| Major/Minor | Minor1 | N | Major1 | | Major2 | |
| Conflicting Flow All | 258 | 166 | 0 | 0 | 167 | 0 |
| Stage 1 | 166 | - | - | - | - | - |
| Stage 2 | 92 | - | - | - | - | - |
| Critical Hdwy | 6.63 | 6.23 | _ | _ | 4.13 | _ |
| Critical Hdwy Stg 1 | 5.43 | - | _ | _ | - | _ |
| Critical Hdwy Stg 2 | 5.83 | _ | | | _ | _ |
| | 3.519 | | _ | _ | 2.219 | |
| Follow-up Hdwy | | | - | | | - |
| Pot Cap-1 Maneuver | 720 | 878 | - | - | 1410 | - |
| Stage 1 | 863 | - | - | - | - | - |
| Stage 2 | 922 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuver | 703 | 878 | - | - | 1410 | - |
| Mov Cap-2 Maneuver | 722 | - | - | - | - | - |
| Stage 1 | 863 | - | - | _ | - | - |
| Stage 2 | 901 | _ | _ | _ | _ | _ |
| Olugo Z | 301 | | | | | |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | 9.2 | | 0 | | 2.9 | |
| HCM LOS | Α | | _ | | | |
| | , , | | | | | |
| | | | | | | |
| Minor Lane/Major Mvr | nt | NBT | NBRV | VBLn1 | SBL | SBT |
| Capacity (veh/h) | | - | - | 878 | 1410 | - |
| HCM Lane V/C Ratio | | - | _ | 0.015 | | _ |
| HCM Control Delay (s |) | _ | _ | 9.2 | 7.6 | _ |
| HCM Lane LOS | | _ | _ | A | Α | _ |
| HCM 95th %tile Q(veh | 1) | | _ | 0 | 0.1 | _ |
| HOW SOUT MILE W(VEI | 1) | _ | - | U | 0.1 | _ |

| | ٠ | → | • | • | ← | • | 4 | † | ~ | / | ļ | 4 |
|------------------------------|------|-------------|------|-------|----------|-------|-------|----------|------|----------|----------|----------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ሻሻ | ↑ ↑₽ | | ሻ | ተተተ | 7 | ሻሻ | ₽ | | ሻሻ | ^ | 7 |
| Traffic Volume (veh/h) | 218 | 1682 | 43 | 20 | 902 | 531 | 42 | 76 | 19 | 785 | 91 | 104 |
| Future Volume (veh/h) | 218 | 1682 | 43 | 20 | 902 | 531 | 42 | 76 | 19 | 785 | 91 | 104 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 237 | 1828 | 47 | 22 | 980 | 0 | 46 | 83 | 21 | 853 | 99 | 113 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 292 | 2086 | 54 | 113 | 1959 | | 144 | 106 | 27 | 898 | 1049 | 468 |
| Arrive On Green | 0.08 | 0.41 | 0.41 | 0.06 | 0.38 | 0.00 | 0.04 | 0.07 | 0.07 | 0.26 | 0.30 | 0.30 |
| Sat Flow, veh/h | 3456 | 5119 | 132 | 1781 | 5106 | 1585 | 3456 | 1440 | 364 | 3456 | 3554 | 1585 |
| Grp Volume(v), veh/h | 237 | 1215 | 660 | 22 | 980 | 0 | 46 | 0 | 104 | 853 | 99 | 113 |
| Grp Sat Flow(s),veh/h/ln | 1728 | 1702 | 1847 | 1781 | 1702 | 1585 | 1728 | 0 | 1805 | 1728 | 1777 | 1585 |
| Q Serve(g_s), s | 8.1 | 39.5 | 39.5 | 1.4 | 17.6 | 0.0 | 1.6 | 0.0 | 6.8 | 29.1 | 2.4 | 6.5 |
| Cycle Q Clear(g_c), s | 8.1 | 39.5 | 39.5 | 1.4 | 17.6 | 0.0 | 1.6 | 0.0 | 6.8 | 29.1 | 2.4 | 6.5 |
| Prop In Lane | 1.00 | | 0.07 | 1.00 | | 1.00 | 1.00 | | 0.20 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 292 | 1387 | 753 | 113 | 1959 | | 144 | 0 | 133 | 898 | 1049 | 468 |
| V/C Ratio(X) | 0.81 | 0.88 | 0.88 | 0.20 | 0.50 | | 0.32 | 0.00 | 0.78 | 0.95 | 0.09 | 0.24 |
| Avail Cap(c_a), veh/h | 334 | 1387 | 753 | 113 | 1959 | | 317 | 0 | 159 | 904 | 1049 | 468 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 54.0 | 32.8 | 32.8 | 53.3 | 28.2 | 0.0 | 55.8 | 0.0 | 54.7 | 43.6 | 30.7 | 32.1 |
| Incr Delay (d2), s/veh | 10.9 | 8.0 | 13.7 | 0.3 | 0.9 | 0.0 | 0.5 | 0.0 | 15.4 | 18.6 | 0.0 | 0.1 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ln | 3.8 | 16.6 | 19.2 | 0.6 | 6.9 | 0.0 | 0.7 | 0.0 | 3.7 | 14.4 | 1.0 | 2.5 |
| Unsig. Movement Delay, s/veh | | 40.0 | 10.1 | 50.0 | 00.4 | 0.0 | 50.0 | 0.0 | 70.0 | 00.0 | 00.7 | 00.0 |
| LnGrp Delay(d),s/veh | 64.9 | 40.8 | 46.4 | 53.6 | 29.1 | 0.0 | 56.3 | 0.0 | 70.0 | 62.2 | 30.7 | 32.2 |
| LnGrp LOS | E | D | D | D | С | | E | Α | E | E | С | <u>C</u> |
| Approach Vol, veh/h | | 2112 | | | 1002 | Α | | 150 | | | 1065 | |
| Approach Delay, s/veh | | 45.2 | | | 29.7 | | | 65.8 | | | 56.1 | |
| Approach LOS | | D | | | С | | | Е | | | Е | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), s | 14.0 | 55.0 | 10.0 | 41.0 | 16.6 | 52.4 | 36.8 | 14.2 | | | | |
| Change Period (Y+Rc), s | 6.4 | 6.1 | 5.0 | * 5.6 | 6.4 | * 6.4 | * 5.6 | 5.4 | | | | |
| Max Green Setting (Gmax), s | 5.6 | 48.9 | 11.0 | * 31 | 11.6 | * 43 | * 31 | 10.6 | | | | |
| Max Q Clear Time (g_c+I1), s | 3.4 | 41.5 | 3.6 | 8.5 | 10.1 | 19.6 | 31.1 | 8.8 | | | | |
| Green Ext Time (p_c), s | 0.0 | 1.5 | 0.0 | 0.1 | 0.1 | 1.1 | 0.1 | 0.0 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 45.0 | | | | | | | | | |
| HCM 6th LOS | | | D | | | | | | | | | |

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

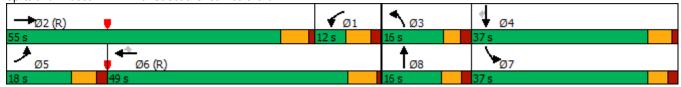
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - 2021 No Build PM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

| | • | → | 4 | 4 | ۶ | 44 | / | † | |
|-------------------------------|---------|-----------|----------|-------------|-------|-------|----------|-------|--|
| Phase Number | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | |
| Movement | WBL | EBT | NBL | SBT | EBL | WBT | SBL | NBT | |
| Lead/Lag | Lag | Lead | Lead | Lag | Lead | Lag | Lag | Lead | |
| Lead-Lag Optimize | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Recall Mode | None | C-Max | None | None | None | C-Max | None | None | |
| Maximum Split (s) | 12 | 55 | 16 | 37 | 18 | 49 | 37 | 16 | |
| Maximum Split (%) | 10.0% | 45.8% | 13.3% | 30.8% | 15.0% | 40.8% | 30.8% | 13.3% | |
| Minimum Split (s) | 11.4 | 31.1 | 10 | 40.6 | 11.4 | 32.1 | 10.6 | 37.4 | |
| Yellow Time (s) | 4.4 | 5.1 | 3 | 4.4 | 4.4 | 5.1 | 3.6 | 3.6 | |
| All-Red Time (s) | 2 | 1 | 2 | 1.2 | 2 | 1 | 2 | 1.8 | |
| Minimum Initial (s) | 5 | 10 | 5 | 7 | 5 | 10 | 5 | 7 | |
| Vehicle Extension (s) | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | |
| Minimum Gap (s) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | |
| Time Before Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Time To Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Walk Time (s) | | 4 | | 4 | | 4 | | 4 | |
| Flash Dont Walk (s) | | 21 | | 31 | | 22 | | 28 | |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | Yes | Yes | |
| Inhibit Max | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Start Time (s) | 41 | 106 | 53 | 69 | 106 | 4 | 69 | 53 | |
| End Time (s) | 53 | 41 | 69 | 106 | 4 | 53 | 106 | 69 | |
| Yield/Force Off (s) | 46.6 | 34.9 | 64 | 100.4 | 117.6 | 46.9 | 100.4 | 63.6 | |
| Yield/Force Off 170(s) | 46.6 | 13.9 | 64 | 69.4 | 117.6 | 24.9 | 100.4 | 35.6 | |
| Local Start Time (s) | 37 | 102 | 49 | 65 | 102 | 0 | 65 | 49 | |
| Local Yield (s) | 42.6 | 30.9 | 60 | 96.4 | 113.6 | 42.9 | 96.4 | 59.6 | |
| Local Yield 170(s) | 42.6 | 9.9 | 60 | 65.4 | 113.6 | 20.9 | 96.4 | 31.6 | |
| Intersection Summary | | | | | | | | | |
| Cycle Length | | | 120 | | | | | | |
| Control Type | Actu | ated-Cooi | rdinated | | | | | | |
| Natural Cycle | | | 145 | | | | | | |
| Offset: 4 (3%), Referenced to | phase 2 | EBT and | 6:WBT, S | Start of Gr | een | | | | |
| | | | | | | | | | |

Splits and Phases: 1: 114th Street & Shea Boulevard



| Intersection | | | | | | |
|------------------------|--------|-------|----------|-------|--------|----------|
| Int Delay, s/veh | 1.2 | | | | | |
| | | 14/55 | NET | NET | 051 | 0.5. |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ¥ | | ₽ | | - ነ | ^ |
| Traffic Vol, veh/h | 2 | 29 | 106 | 0 | 11 | 148 |
| Future Vol, veh/h | 2 | 29 | 106 | 0 | 11 | 148 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 100 | - |
| Veh in Median Storage | e, # 0 | - | 0 | _ | _ | 0 |
| Grade, % | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 32 | 115 | 0 | 12 | 161 |
| IVIVIIILIIOW | 2 | 52 | 110 | U | 12 | 101 |
| | | | | | | |
| Major/Minor | Minor1 | N | Major1 | | Major2 | |
| Conflicting Flow All | 220 | 115 | 0 | 0 | 115 | 0 |
| Stage 1 | 115 | - | - | - | - | - |
| Stage 2 | 105 | - | - | - | _ | _ |
| Critical Hdwy | 6.63 | 6.23 | _ | _ | 4.13 | _ |
| Critical Hdwy Stg 1 | 5.43 | - | _ | _ | - | _ |
| Critical Hdwy Stg 2 | 5.83 | _ | _ | _ | _ | _ |
| Follow-up Hdwy | 3.519 | 3.319 | _ | _ | 2.219 | _ |
| Pot Cap-1 Maneuver | 758 | 937 | | | 1473 | |
| | | | - | - | 1473 | - |
| Stage 1 | 909 | - | - | - | - | - |
| Stage 2 | 908 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuver | 752 | 937 | - | - | 1473 | - |
| Mov Cap-2 Maneuver | 756 | - | - | - | - | - |
| Stage 1 | 909 | - | - | - | - | - |
| Stage 2 | 901 | - | - | - | - | - |
| | | | | | | |
| A I. | \A/D | | ND | | 0.0 | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | 9 | | 0 | | 0.5 | |
| HCM LOS | Α | | | | | |
| | | | | | | |
| Minor Lane/Major Mvn | nt . | NBT | NDDV | VBLn1 | SBL | SBT |
| | π | INDI | אאטוו | | | |
| Capacity (veh/h) | | - | - | | 1473 | - |
| HCM Lane V/C Ratio | | - | - | 0.037 | | - |
| HCM Control Delay (s) | | - | - | 9 | 7.5 | - |
| HCM Lane LOS | | - | - | Α | Α | - |
| HCM 95th %tile Q(veh |) | - | - | 0.1 | 0 | - |
| | | | | | | |



Appendix K – Year 2021 Build Capacity Analysis





| | • | → | • | • | ← | • | 4 | † | / | / | ļ | 4 |
|------------------------------|------|-------------|------|-------|----------|-------|-------------|----------|------|----------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ሻሻ | ↑ ↑₽ | | ሻ | ተተተ | 7 | ሻሻ | ₽ | | ሻሻ | ^ | 7 |
| Traffic Volume (veh/h) | 90 | 1135 | 40 | 21 | 1709 | 473 | 58 | 97 | 18 | 515 | 42 | 147 |
| Future Volume (veh/h) | 90 | 1135 | 40 | 21 | 1709 | 473 | 58 | 97 | 18 | 515 | 42 | 147 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 98 | 1234 | 43 | 23 | 1858 | 0 | 63 | 105 | 20 | 560 | 46 | 160 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 148 | 2191 | 76 | 297 | 2827 | | 144 | 130 | 25 | 415 | 592 | 264 |
| Arrive On Green | 0.04 | 0.43 | 0.43 | 0.17 | 0.55 | 0.00 | 0.04 | 0.09 | 0.09 | 0.12 | 0.17 | 0.17 |
| Sat Flow, veh/h | 3456 | 5066 | 177 | 1781 | 5106 | 1585 | 3456 | 1527 | 291 | 3456 | 3554 | 1585 |
| Grp Volume(v), veh/h | 98 | 829 | 448 | 23 | 1858 | 0 | 63 | 0 | 125 | 560 | 46 | 160 |
| Grp Sat Flow(s),veh/h/ln | 1728 | 1702 | 1839 | 1781 | 1702 | 1585 | 1728 | 0 | 1818 | 1728 | 1777 | 1585 |
| Q Serve(g_s), s | 3.4 | 21.9 | 21.9 | 1.3 | 30.6 | 0.0 | 2.1 | 0.0 | 8.1 | 14.4 | 1.3 | 11.2 |
| Cycle Q Clear(g_c), s | 3.4 | 21.9 | 21.9 | 1.3 | 30.6 | 0.0 | 2.1 | 0.0 | 8.1 | 14.4 | 1.3 | 11.2 |
| Prop In Lane | 1.00 | | 0.10 | 1.00 | | 1.00 | 1.00 | | 0.16 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 148 | 1472 | 795 | 297 | 2827 | | 144 | 0 | 155 | 415 | 592 | 264 |
| V/C Ratio(X) | 0.66 | 0.56 | 0.56 | 0.08 | 0.66 | | 0.44 | 0.00 | 0.81 | 1.35 | 0.08 | 0.61 |
| Avail Cap(c_a), veh/h | 161 | 1472 | 795 | 297 | 2827 | | 173 | 0 | 373 | 415 | 989 | 441 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 56.6 | 25.5 | 25.5 | 42.2 | 18.8 | 0.0 | 56.1 | 0.0 | 53.9 | 52.8 | 42.2 | 46.3 |
| Incr Delay (d2), s/veh | 6.3 | 1.6 | 2.9 | 0.0 | 1.2 | 0.0 | 0.8 | 0.0 | 3.8 | 172.9 | 0.0 | 0.8 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ln | 1.5 | 8.6 | 9.6 | 0.6 | 11.1 | 0.0 | 0.9 | 0.0 | 3.9 | 16.1 | 0.6 | 4.5 |
| Unsig. Movement Delay, s/veh | | 07.4 | 00.4 | 40.0 | 00.0 | 0.0 | 50.0 | 0.0 | | 005.7 | 10.0 | 47.0 |
| LnGrp Delay(d),s/veh | 62.9 | 27.1 | 28.4 | 42.3 | 20.0 | 0.0 | 56.9 | 0.0 | 57.7 | 225.7 | 42.2 | 47.2 |
| LnGrp LOS | E | С | С | D | В | | E | A | E | F | D | D |
| Approach Vol, veh/h | | 1375 | | | 1881 | Α | | 188 | | | 766 | |
| Approach Delay, s/veh | | 30.1 | | | 20.3 | | | 57.4 | | | 177.4 | |
| Approach LOS | | С | | | С | | | E | | | F | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), s | 26.4 | 58.0 | 10.0 | 25.6 | 11.6 | 72.8 | 20.0 | 15.6 | | | | |
| Change Period (Y+Rc), s | 6.4 | 6.1 | 5.0 | * 5.6 | 6.4 | * 6.4 | * 5.6 | 5.4 | | | | |
| Max Green Setting (Gmax), s | 5.6 | 51.9 | 6.0 | * 33 | 5.6 | * 52 | * 14 | 24.6 | | | | |
| Max Q Clear Time (g_c+I1), s | 3.3 | 23.9 | 4.1 | 13.2 | 5.4 | 32.6 | 16.4 | 10.1 | | | | |
| Green Ext Time (p_c), s | 0.0 | 1.1 | 0.0 | 0.0 | 0.0 | 2.4 | 0.0 | 0.1 | | | | |
| Intersection Summary | | | | | | | | | | | | _ |
| HCM 6th Ctrl Delay | | | 53.7 | | | | | | | | | |
| HCM 6th LOS | | | D | | | | | | | | | |

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - 2021 Build AM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

| | • | → | 4 | 4 | ۶ | 40 | > | † | |
|-----------------------------|--------------|-----------|----------|------------|-------|-------|-------------|----------|--|
| Phase Number | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | |
| Movement | WBL | EBT | NBL | SBT | EBL | WBT | SBL | NBT | |
| Lead/Lag | Lag | Lead | Lead | Lag | Lead | Lag | Lag | Lead | |
| Lead-Lag Optimize | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Recall Mode | None | C-Max | None | None | None | C-Max | None | None | |
| Maximum Split (s) | 12 | 58 | 11 | 39 | 12 | 58 | 20 | 30 | |
| Maximum Split (%) | 10.0% | 48.3% | 9.2% | 32.5% | 10.0% | 48.3% | 16.7% | 25.0% | |
| Minimum Split (s) | 11.4 | 31.1 | 10 | 40.6 | 11.4 | 32.1 | 10.6 | 37.4 | |
| Yellow Time (s) | 4.4 | 5.1 | 3 | 4.4 | 4.4 | 5.1 | 3.6 | 3.6 | |
| All-Red Time (s) | 2 | 1 | 2 | 1.2 | 2 | 1 | 2 | 1.8 | |
| Minimum Initial (s) | 5 | 10 | 5 | 7 | 5 | 10 | 5 | 7 | |
| Vehicle Extension (s) | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | |
| Minimum Gap (s) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | |
| Time Before Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Time To Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Walk Time (s) | | 4 | | 4 | | 4 | | 4 | |
| Flash Dont Walk (s) | | 21 | | 31 | | 22 | | 28 | |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | Yes | Yes | |
| Inhibit Max | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Start Time (s) | 90 | 32 | 102 | 113 | 32 | 44 | 12 | 102 | |
| End Time (s) | 102 | 90 | 113 | 32 | 44 | 102 | 32 | 12 | |
| Yield/Force Off (s) | 95.6 | 83.9 | 108 | 26.4 | 37.6 | 95.9 | 26.4 | 6.6 | |
| Yield/Force Off 170(s) | 95.6 | 62.9 | 108 | 115.4 | 37.6 | 73.9 | 26.4 | 98.6 | |
| Local Start Time (s) | 46 | 108 | 58 | 69 | 108 | 0 | 88 | 58 | |
| Local Yield (s) | 51.6 | 39.9 | 64 | 102.4 | 113.6 | 51.9 | 102.4 | 82.6 | |
| Local Yield 170(s) | 51.6 | 18.9 | 64 | 71.4 | 113.6 | 29.9 | 102.4 | 54.6 | |
| Intersection Summary | | | | | | | | | |
| Cycle Length | | | 120 | | | | | | |
| Control Type | Actu | ated-Coor | dinated | | | | | | |
| Natural Cycle | | | 125 | | | | | | |
| Offset: 44 (37%), Reference | ed to phase | 2:EBT ar | nd 6:WBT | , Start of | Green | | | | |
| Splits and Phases: 1: 11 | 4th Street 8 | Shea Bo | ulevard | | | | | | |
| | | | | | | 4 | | 4 04 | |
| 732 (D) | | | | | (a) | 1 I T | 03 | 0.4 | |

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| | • | → | • | • | ← | • | 4 | † | / | > | ↓ | 1 |
|---------------------------------|------|----------|-------|------|----------|------------|------|----------|----------|-------------|------------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | ň | ĵ» | | ă | ∱ ∱ | |
| Traffic Volume (veh/h) | 8 | 0 | 0 | 0 | 0 | 12 | 1 | 151 | 3 | 30 | 48 | 0 |
| Future Volume (Veh/h) | 8 | 0 | 0 | 0 | 0 | 12 | 1 | 151 | 3 | 30 | 48 | 0 |
| Sign Control | | Stop | | | Stop | | | Free | | | Free | |
| Grade | | 0% | | | 0% | | | 0% | | | 0% | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 9 | 0 | 0 | 0 | 0 | 13 | 1 | 164 | 3 | 33 | 52 | 0 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | TWLTL | | | None | |
| Median storage veh) | | | | | | | | 2 | | | | |
| Upstream signal (ft) | | | | | | | | | | | 407 | |
| pX, platoon unblocked | | | | | | | | | | | | |
| vC, conflicting volume | 297 | 287 | 26 | 260 | 286 | 166 | 52 | | | 167 | | |
| vC1, stage 1 conf vol | 118 | 118 | | 168 | 168 | | | | | | | |
| vC2, stage 2 conf vol | 179 | 169 | | 92 | 118 | | | | | | | |
| vCu, unblocked vol | 297 | 287 | 26 | 260 | 286 | 166 | 52 | | | 167 | | |
| tC, single (s) | 7.5 | 6.5 | 6.9 | 7.5 | 6.5 | 6.9 | 4.1 | | | 4.1 | | |
| tC, 2 stage (s) | 6.5 | 5.5 | | 6.5 | 5.5 | | | | | | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 2.2 | | |
| p0 queue free % | 99 | 100 | 100 | 100 | 100 | 98 | 100 | | | 98 | | |
| cM capacity (veh/h) | 714 | 682 | 1044 | 763 | 697 | 850 | 1552 | | | 1408 | | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | SB 3 | | | | | |
| Volume Total | 9 | 13 | 1 | 167 | 33 | 35 | 17 | | | | | |
| Volume Left | 9 | 0 | 1 | 0 | 33 | 0 | 0 | | | | | |
| Volume Right | 0 | 13 | 0 | 3 | 0 | 0 | 0 | | | | | |
| cSH | 714 | 850 | 1552 | 1700 | 1408 | 1700 | 1700 | | | | | |
| Volume to Capacity | 0.01 | 0.02 | 0.00 | 0.10 | 0.02 | 0.02 | 0.01 | | | | | |
| Queue Length 95th (ft) | 1 | 1 | 0 | 0 | 2 | 0 | 0 | | | | | |
| Control Delay (s) | 10.1 | 9.3 | 7.3 | 0.0 | 7.6 | 0.0 | 0.0 | | | | | |
| Lane LOS | В | А | Α | | Α | | | | | | | |
| Approach Delay (s) | 10.1 | 9.3 | 0.0 | | 3.0 | | | | | | | |
| Approach LOS | В | Α | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 1.7 | | | | | | | | | |
| Intersection Capacity Utilizati | on | | 28.1% | IC | U Level | of Service | | | Α | | | |
| Analysis Period (min) | | | 15 | , , | | | | | | | | |

| | ۶ | → | • | • | — | • | 1 | † | ~ | / | + | ✓ |
|------------------------------|------|-------------|------|-------|----------|-------|-------|----------|------|----------|----------|----------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ሻሻ | ↑ ↑₽ | | ሻ | ተተተ | 7 | ሻሻ | ₽ | | ሻሻ | ^ | 7 |
| Traffic Volume (veh/h) | 218 | 1682 | 53 | 27 | 902 | 531 | 50 | 82 | 38 | 785 | 96 | 104 |
| Future Volume (veh/h) | 218 | 1682 | 53 | 27 | 902 | 531 | 50 | 82 | 38 | 785 | 96 | 104 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 237 | 1828 | 58 | 29 | 980 | 0 | 54 | 89 | 41 | 853 | 104 | 113 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 292 | 2029 | 64 | 111 | 1913 | | 144 | 108 | 50 | 875 | 1081 | 482 |
| Arrive On Green | 0.08 | 0.40 | 0.40 | 0.06 | 0.37 | 0.00 | 0.04 | 0.09 | 0.09 | 0.25 | 0.30 | 0.30 |
| Sat Flow, veh/h | 3456 | 5084 | 161 | 1781 | 5106 | 1585 | 3456 | 1212 | 558 | 3456 | 3554 | 1585 |
| Grp Volume(v), veh/h | 237 | 1223 | 663 | 29 | 980 | 0 | 54 | 0 | 130 | 853 | 104 | 113 |
| Grp Sat Flow(s),veh/h/ln | 1728 | 1702 | 1841 | 1781 | 1702 | 1585 | 1728 | 0 | 1770 | 1728 | 1777 | 1585 |
| Q Serve(g_s), s | 8.1 | 40.4 | 40.5 | 1.9 | 17.8 | 0.0 | 1.8 | 0.0 | 8.7 | 29.4 | 2.5 | 6.4 |
| Cycle Q Clear(g_c), s | 8.1 | 40.4 | 40.5 | 1.9 | 17.8 | 0.0 | 1.8 | 0.0 | 8.7 | 29.4 | 2.5 | 6.4 |
| Prop In Lane | 1.00 | | 0.09 | 1.00 | | 1.00 | 1.00 | | 0.32 | 1.00 | | 1.00 |
| Lane Grp Cap(c), veh/h | 292 | 1359 | 735 | 111 | 1913 | | 144 | 0 | 158 | 875 | 1081 | 482 |
| V/C Ratio(X) | 0.81 | 0.90 | 0.90 | 0.26 | 0.51 | | 0.38 | 0.00 | 0.82 | 0.97 | 0.10 | 0.23 |
| Avail Cap(c_a), veh/h | 334 | 1359 | 735 | 111 | 1913 | | 346 | 0 | 186 | 875 | 1081 | 482 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 54.0 | 33.8 | 33.8 | 53.6 | 29.0 | 0.0 | 56.0 | 0.0 | 53.7 | 44.4 | 29.9 | 31.3 |
| Incr Delay (d2), s/veh | 10.9 | 9.8 | 16.4 | 0.5 | 1.0 | 0.0 | 0.6 | 0.0 | 19.3 | 24.1 | 0.0 | 0.1 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ln | 3.8 | 17.4 | 20.1 | 0.8 | 7.1 | 0.0 | 0.8 | 0.0 | 4.7 | 15.1 | 1.1 | 2.5 |
| Unsig. Movement Delay, s/veh | | 10.0 | | | | | | | | | | 21.1 |
| LnGrp Delay(d),s/veh | 64.9 | 43.6 | 50.2 | 54.1 | 30.0 | 0.0 | 56.6 | 0.0 | 73.0 | 68.5 | 29.9 | 31.4 |
| LnGrp LOS | E | D | D | D | С | | E | A | E | E | С | <u>C</u> |
| Approach Vol, veh/h | | 2123 | | | 1009 | Α | | 184 | | | 1070 | |
| Approach Delay, s/veh | | 48.1 | | | 30.7 | | | 68.2 | | | 60.8 | |
| Approach LOS | | D | | | С | | | Е | | | Е | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), s | 13.9 | 54.0 | 10.0 | 42.1 | 16.6 | 51.4 | 36.0 | 16.1 | | | | |
| Change Period (Y+Rc), s | 6.4 | 6.1 | 5.0 | * 5.6 | 6.4 | * 6.4 | * 5.6 | 5.4 | | | | |
| Max Green Setting (Gmax), s | 5.6 | 47.9 | 12.0 | * 31 | 11.6 | * 42 | * 30 | 12.6 | | | | |
| Max Q Clear Time (g_c+I1), s | 3.9 | 42.5 | 3.8 | 8.4 | 10.1 | 19.8 | 31.4 | 10.7 | | | | |
| Green Ext Time (p_c), s | 0.0 | 1.4 | 0.0 | 0.1 | 0.1 | 1.1 | 0.0 | 0.0 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 48.0 | | | | | | | | | |
| HCM 6th LOS | | | D | | | | | | | | | |

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - 2021 Build PM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

| | • | → | 4 | 4 | ۶ | 44 | > | † | |
|----------------------------|--------------|----------|----------|------------|-------|-------------|-------------|----------|----|
| Phase Number | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | |
| Movement | WBL | EBT | NBL | SBT | EBL | WBT | SBL | NBT | |
| Lead/Lag | Lag | Lead | Lead | Lag | Lead | Lag | Lag | Lead | |
| Lead-Lag Optimize | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Recall Mode | None | C-Max | None | None | None | C-Max | None | None | |
| Maximum Split (s) | 12 | 54 | 17 | 37 | 18 | 48 | 36 | 18 | |
| Maximum Split (%) | 10.0% | 45.0% | 14.2% | 30.8% | 15.0% | 40.0% | 30.0% | 15.0% | |
| Minimum Split (s) | 11.4 | 31.1 | 10 | 40.6 | 11.4 | 32.1 | 10.6 | 37.4 | |
| Yellow Time (s) | 4.4 | 5.1 | 3 | 4.4 | 4.4 | 5.1 | 3.6 | 3.6 | |
| All-Red Time (s) | 2 | 1 | 2 | 1.2 | 2 | 1 | 2 | 1.8 | |
| Minimum Initial (s) | 5 | 10 | 5 | 7 | 5 | 10 | 5 | 7 | |
| Vehicle Extension (s) | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | 2 | 0.2 | |
| Minimum Gap (s) | 3 | 3 | 3 | 3 | 3 | 3 | 3 | 3 | |
| Time Before Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Time To Reduce (s) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Walk Time (s) | | 4 | | 4 | | 4 | | 4 | |
| Flash Dont Walk (s) | | 21 | | 31 | | 22 | | 28 | |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | Yes | Yes | |
| Inhibit Max | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | |
| Start Time (s) | 40 | 106 | 52 | 69 | 106 | 4 | 70 | 52 | |
| End Time (s) | 52 | 40 | 69 | 106 | 4 | 52 | 106 | 70 | |
| Yield/Force Off (s) | 45.6 | 33.9 | 64 | 100.4 | 117.6 | 45.9 | 100.4 | 64.6 | |
| Yield/Force Off 170(s) | 45.6 | 12.9 | 64 | 69.4 | 117.6 | 23.9 | 100.4 | 36.6 | |
| Local Start Time (s) | 36 | 102 | 48 | 65 | 102 | 0 | 66 | 48 | |
| Local Yield (s) | 41.6 | 29.9 | 60 | 96.4 | 113.6 | 41.9 | 96.4 | 60.6 | |
| Local Yield 170(s) | 41.6 | 8.9 | 60 | 65.4 | 113.6 | 19.9 | 96.4 | 32.6 | |
| Intersection Summary | | | | | | | | | |
| Cycle Length | | | 120 | | | | | | |
| Control Type | Actu | ated-Coo | | | | | | | |
| Natural Cycle | | | 145 | | | | | | |
| Offset: 4 (3%), Referenced | to phase 2: | EBT and | 6:WBT, S | Start of G | reen | | | | |
| Splits and Phases: 1: 114 | 4th Street & | Shea Bo | ulevard | | | | | | |
| →Ø2 (R) | | | | | Ø1 | ↑ Ø3 | 2 | 4 0 | 14 |
| - wz (K) | | | | , | ושי | 1 200 | , | ¥ 10 | т. |

Ø6 (R)

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|---------------------------------|-----------|----------|-------|------|-----------|------------|------|----------|-------------|------|-------------|-------------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBU | SBL | SBT |
| Lane Configurations | | 4 | | | 4 | | 7 | 1> | | | ă | ↑ 1> |
| Traffic Volume (veh/h) | 31 | 0 | 2 | 2 | 0 | 29 | 1 | 106 | 0 | 2 | 11 | 148 |
| Future Volume (Veh/h) | 31 | 0 | 2 | 2 | 0 | 29 | 1 | 106 | 0 | 2 | 11 | 148 |
| Sign Control | | Stop | | | Stop | | | Free | | | | Free |
| Grade | | 0% | | | 0% | | | 0% | | | | 0% |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 34 | 0 | 2 | 2 | 0 | 32 | 1 | 115 | 0 | 0 | 12 | 161 |
| Pedestrians | | | | | | | | | | | | |
| Lane Width (ft) | | | | | | | | | | | | |
| Walking Speed (ft/s) | | | | | | | | | | | | |
| Percent Blockage | | | | | | | | | | | | |
| Right turn flare (veh) | | | | | | | | | | | | |
| Median type | | | | | | | | TWLTL | | | | None |
| Median storage veh) | | | | | | | | 2 | | | | |
| Upstream signal (ft) | | | | | | | | | | | | 407 |
| pX, platoon unblocked | | | | | | | | | | 0.00 | | |
| vC, conflicting volume | 334 | 302 | 80 | 224 | 302 | 115 | 161 | | | 0 | 115 | |
| vC1, stage 1 conf vol | 185 | 185 | | 117 | 117 | | | | | | | |
| vC2, stage 2 conf vol | 149 | 117 | | 106 | 185 | | | | | | | |
| vCu, unblocked vol | 334 | 302 | 80 | 224 | 302 | 115 | 161 | | | 0 | 115 | |
| tC, single (s) | 7.5 | 6.5 | 6.9 | 7.5 | 6.5 | 6.9 | 4.1 | | | 0.0 | 4.1 | |
| tC, 2 stage (s) | 6.5 | 5.5 | 0.0 | 6.5 | 5.5 | 0.0 | | | | 0.0 | | |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 | | | 0.0 | 2.2 | |
| p0 queue free % | 95 | 100 | 100 | 100 | 100 | 97 | 100 | | | 0.0 | 99 | |
| cM capacity (veh/h) | 703 | 690 | 963 | 800 | 692 | 916 | 1416 | | | 0 | 1472 | |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | NB 2 | SB 1 | SB 2 | SB 3 | | | | | |
| Volume Total | 36 | 34 | 1 | 115 | 12 | 107 | 54 | | | | | |
| Volume Left | 34 | 2 | 1 | 0 | 12 | 0 | 0 | | | | | |
| | 2 | 32 | 0 | 0 | 0 | 0 | 0 | | | | | |
| Volume Right cSH | 714 | 908 | 1416 | 1700 | 1472 | 1700 | 1700 | | | | | |
| | | | | | | 0.06 | | | | | | |
| Volume to Capacity | 0.05 | 0.04 | 0.00 | 0.07 | 0.01 | | 0.03 | | | | | |
| Queue Length 95th (ft) | | 3 | 7.5 | 0 | | 0 | 0 | | | | | |
| Control Delay (s) | 10.3 | 9.1 | 7.5 | 0.0 | 7.5 | 0.0 | 0.0 | | | | | |
| Lane LOS | B | A | A | | A | | | | | | | |
| Approach Delay (s) Approach LOS | 10.3 B | 9.1 A | 0.1 | | 0.5 | | | | | | | |
| •• | Ь | | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Average Delay | | | 2.2 | | | | | | | | | |
| Intersection Capacity Utiliza | ation | | 22.6% | IC | U Level o | of Service | | | Α | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |



| Movement | SBR |
|------------------------|------|
| | SDR |
| Lare Configurations | 0 |
| Traffic Volume (veh/h) | 0 |
| Future Volume (Veh/h) | 0 |
| Sign Control | |
| Grade | |
| Peak Hour Factor | 0.92 |
| Hourly flow rate (vph) | 0 |
| Pedestrians | |
| Lane Width (ft) | |
| Walking Speed (ft/s) | |
| Percent Blockage | |
| Right turn flare (veh) | |
| Median type | |
| Median storage veh) | |
| Upstream signal (ft) | |
| pX, platoon unblocked | |
| vC, conflicting volume | |
| vC1, stage 1 conf vol | |
| vC2, stage 2 conf vol | |
| vCu, unblocked vol | |
| tC, single (s) | |
| tC, 2 stage (s) | |
| tF (s) | |
| p0 queue free % | |
| cM capacity (veh/h) | |
| | |
| Direction, Lane # | |

| Intersection | | | | | | |
|------------------------|--------|---------|--------|----------|------------|------|
| Int Delay, s/veh | 0.1 | | | | | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| | EDL | | INDL | | | SDR |
| Lane Configurations | ٥ | | ٥ | 160 | ↑ } | 22 |
| Traffic Vol, veh/h | 0 | 2 | 0 | 168 | 159 | |
| Future Vol, veh/h | 0 | 2 | 0 | 168 | 159 | 22 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | _ 0 | _ 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | 0 | - | - | - | - |
| Veh in Median Storage | | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 2 | 0 | 183 | 173 | 24 |
| | | | | | | |
| | | | | | | |
| | Minor2 | | Major1 | | //ajor2 | |
| Conflicting Flow All | - | 99 | - | 0 | - | 0 |
| Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |
| Critical Hdwy | - | 6.93 | - | - | - | - |
| Critical Hdwy Stg 1 | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | _ | - | _ | - | _ | - |
| Follow-up Hdwy | _ | 3.319 | _ | - | _ | - |
| Pot Cap-1 Maneuver | 0 | 938 | 0 | _ | _ | - |
| Stage 1 | 0 | - | 0 | _ | _ | _ |
| Stage 2 | 0 | _ | 0 | | _ | _ |
| Platoon blocked, % | U | - | U | | | _ |
| | | റാറ | | - | - | |
| Mov Cap-1 Maneuver | - | 938 | - | - | - | - |
| Mov Cap-2 Maneuver | - | - | - | - | - | - |
| Stage 1 | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - |
| | | | | | | |
| Approach | EB | | NB | | SB | |
| HCM Control Delay, s | 8.8 | | 0 | | 0 | |
| HCM LOS | | | U | | U | |
| HCIVI LUS | Α | | | | | |
| | | | | | | |
| Minor Lane/Major Mvn | nt | NBT I | EBLn1 | SBT | SBR | |
| Capacity (veh/h) | | _ | 938 | _ | _ | |
| HCM Lane V/C Ratio | | _ | 0.002 | <u>-</u> | _ | |
| HCM Control Delay (s) | 1 | _ | 8.8 | _ | _ | |
| HCM Lane LOS | | | | | | |
| | | - | A | - | - | |
| HCM 95th %tile Q(veh |) | - | 0 | - | - | |