

### PRELIMINARY DRAINAGE REPORT

FOR

### NORTH SCOTTSDALE AIRPARK 16115 NORTH 81<sup>ST</sup> STREET SCOTTSDALE, ARIZONA

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April 30, 2021 Bowman Project No. 050941



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#### 1.0 INTRODUCTION

This project is a 0.81-acre site plan involving the construction of an airplane hangar and is located near the southeast corner of Frank Lloyd Wright Boulevard and Greenway Haydon Loop in the City of Scottsdale, Arizona. As shown in appendix A, the project site is within a portion of Section 1, Township 3 North, Range 4 East of the Gila and Salt River Meridian, Maricopa County, Arizona.

The site will be designed in accordance with the City of Scottsdale and Design Standards and Policies Manual (Reference 1) and the Drainage Policies and Standards for Maricopa County, Arizona (Reference 2).

The purpose of this report is the following:

- Identify the development's offsite flows.
- Establish onsite drainage criteria for the development
- Show compliance with the City of Scottsdale Design Standards (Reference 1).

#### 2.0 FLOODPLAIN DESIGNATION

The site lies within the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) map number 04013C1320L, with a current effective date of October 16, 2013. The site is located within Zone X. Flood Zone X is defined as:

Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance of flood.

#### 3.0 OFFSITE DRAINAGE MANAGEMENT

#### 3.1 GENERAL CONCEPT

The site lies within the Scottsdale Road Corridor Drainage Master Plan. The drainage master plan shows that runoff north of the site is intercepted by the CAP canal. The project accepts a small amount of flow from the taxiway directly north of the northeast corner of the site via sheet flow. A proposed onsite catch basin will capture this flow and retain it onsite.

#### 4.0 ONSITE DRAINAGE MANAGEMENT

#### 4.1 **GENERAL CONCEPT**

Currently the site is undeveloped desert and slopes southeasterly. Once developed, onsite storm water runoff will be retained using underground retention and a surface basin. Catch basins and storm drains will be used to capture and convey the storm water into the retention system.

#### 4.2 **DESIGN PEAK FLOWS**

Onsite design peak flows were estimated using the Rational Method in accordance with the City of Scottsdale criteria. See appendix A for the corresponding drainage map illustrating the grading and drainage areas for the Project. The runoff



coefficient used in the calculations is 0.95, as the site is comprised of desert landscape, rooftop and concrete. The peak flow calculations are provided in Appendix B.

#### 4.3 STREET INLET AND PIPE STRUCTURES

The onsite runoff will be conveyed into the underground retention system and surface basin via storm drain, and MAG 537 catch basins. These inlet and pipe structures are designed to convey the 10-year peak flow assuming that each type of inlet is 50% clogged. Inlet sizing calculations are provided in Appendix C. A StormCAD model was created to obtain hydraulic grade lines and ensure that the pipes are properly sized. The StormCAD model results are provided in Appendix D.

#### 4.4 **RETENTION REQUIREMENT**

The retention requirement for the site is the volume of runoff produced by the 100year 2-hour storm. According to the NOAA Atlas 14 database, the 100-year 2-hour storm rainfall depth is 2.26 inches. A 10-foot diameter corrugated metal pipe and a surface basin are proposed to accommodate the storm water runoff volume. The surface basin will be one foot deep and will maintain 4:1 side slopes. Retention volume calculations are included in Appendix E.

#### 4.5 STORM WATER DISSIPATION

The City of Scottsdale requires retained storm water to be drained from the basin within a 36-hour time period. In order to conform to this design requirement, a drywell will be used. A drywell percolation rate of 0.1 cfs will be assumed until a percolation test is completed. Dissipation calculations are located in Appendix E.

#### 5.0 ULTIMATE OUTFALL

An outfall will be provided for each onsite drainage area to allow flow in excess of the design storm or in back to back events to leave the drainage area without inundating finished floors of structures within that area. The ultimate outfall for the project site is located near at the proposed driveway at an elevation of 1509.53. From here, excess runoff will flow south down 81<sup>st</sup> Street.

Finished floors will be set a minimum of 14 inches above the highest applicable ultimate outfall of the lot, drainage area, or project.

#### 6.0 CONCLUSION

The proposed development shall be designed in compliance with the City of Scottsdale design standards and other applicable drainage standards set forth by the Flood Control District of Maricopa County. This report has established:

- Onsite storm water from storms up to the 100-year 2-hour event will be retained in in a surface basin and underground retention system.
- Offsite flows will be properly routed through the site and retained.



- Catch basins and storm drains will be sized for the 10-year storm event.
- The retention will drain via drywell within 36-hours.
- Finished floors will be set a minimum of 14 inches above the highest applicable ultimate outfall of the lot, drainage area, or project.

#### 7.0 **REFERENCES**

- 1. City of Scottsdale, 2018. <u>Design Standards and Policies Manual</u>. City of Scottsdale, Arizona.
- Flood Control District of Maricopa County, August 22, 2018. <u>Drainage Policies</u> and Standards for Maricopa County, Arizona. Flood Control District of Maricopa County, Phoenix, Arizona.



## **APPENDIX A**

**Exhibits** 





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# National Flood Hazard Layer FIRMette



#### Legend



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020



### **APPENDIX B**

### **Rational Calculations**

#### Peak Inlet Flow Calculations Using The Rational Method

		R	Rainfall Depth-Duration-Frequency (D-D-F), (inch)				
		Desired	ired Time				
Project:	Scottsdale Airpark	5 min	10 min	15 min	30 min	60 min	
Proj #:	050941	10-Yr	0.41	0.62	0.77	1.03	1.28
Date:	4/30/21	100-Yr	0.64	0.97	1.21	1.63	2.01
By:	HT	Ra	ainfall Intensity-Duration-Frequency (I-D-F), (in/hr)				
		10-Yr	4.88	3.72	3.07	2.06	1.28
		100-Yr	<b>100-Yr</b> 7.63 5.81 4.84 3.26 2.01				

AF for Cw per Cw <sub>100-Yr</sub>							
Freq.	Typical	Applic.					
2-Yr	1.00	1.00					
5-Yr	1.00	1.00					
10-Yr	1.00	1.00					
25-Yr	1.10	1.00					
50-Yr	1.20	1.00					
100-Yr	1.25	1.00					
	AF for Cw Freq. 2-Yr 5-Yr 10-Yr 25-Yr 50-Yr 100-Yr	AF for Cw per Cw100.           Freq.         Typical           2-Yr         1.00           5-Yr         1.00           10-Yr         1.00           25-Yr         1.10           50-Yr         1.20           100-Yr         1.25					

AF=Frequency Adjustment Factor

Drainage A	Drainage Area ID:							100-Yr		
					Cw is adjust	ed as a functi	on of the 10	0-year value	per the table	above
Concent.	Contributing	Total					Q			Q
Point	Sub-basins	Area	2-10 year	Тс	Cw	i	10-Yr	Cw	i	100-Yr
		(ac)	Cw	(min)	AF=1.00	(in/hr)	(cfs)	AF=1.00	(in/hr)	(cfs)
1	A1	0.121	0.95	5.0	0.95	4.88	0.6	0.95	7.63	0.9
2	A2	0.305	0.95	5.0	0.95	4.88	1.4	0.95	7.63	2.2
3	A3	0.120	0.95	5.0	0.95	4.88	0.6	0.95	7.63	0.9
4	A4	0.222	0.95	5.0	0.95	4.88	1.0	0.95	7.63	1.6

.

Notes:



## **APPENDIX C**

## **Inlet Sizing Calculations**

### CATCH BASIN DESIGN FOR 10-YR EVENT



Project:	North Scottsdale Airpark
Project #:	050941
Prepared by:	HT

DATE: 4/30/2021

INTENT: Verify adequacy of catch basin/scupper size per following equations, capacities are as noted below. Evaluation compares capacity at maximum ponding depth to the peak flow. Calculations are as outlined in the Drainage Design Manual for Maricopa County unless otherwise specified.

#### Curb open catch basin:

-							
For length of curb opening: $Q_{CO} = C_w (L + 1.8W) d^{1.5} (F_{CL})$							
Where:	C <sub>w</sub> =	2.3					
	W =	1 / 2 ft	MAG 533 W = 2. ft				
	(Scupper)	1.42 IL	Curb Opening W = 1.42 ft				
	F <sub>CL</sub> =	0.80	Clogging factor for curb-open catch basin/scuppers				

#### Grated catch basin:

For the grated portion of the catch basin:  $Q_{GR}$  =  $C_w Pd^{1.5}(F_{CL})$ 

Where:

 $F_{CL} = 0.50$   $C_{W} = 3.0$ MAG 537 Single P = 7.67 ft
MAG 537 Double \*P = 10.75 ft
\*Perimeter calculated at edge of concrete collar

Concentration Point	Inlet ID	Flow Q <sub>10</sub>	Catch Basin/Scupper Type and Length (ft)	Water Height (d)	Max. Flow into Catch Basin (CFS)
A1	CB A1	0.6	MAG 537 Single	0.20	1.0
A2	CB A2	1.4	MAG 537 Single	0.25	1.4
A3	CB A3	0.6	MAG 537 Single	0.14	0.6

### **Flow-By Catch Basin Calculation**

#### **Project Data**

Project Title: Inlet A4 Designer: HT Project Date: Tuesday, February 23, 2021 Project Units: U.S. Customary Units

#### Curb and Gutter Analysis: Curb and Gutter Analysis

#### **Gutter Input Parameters**

Longitudinal Slope of Road: 0.0150 ft/ft Cross-Slope of Pavement: 0.0200 ft/ft Uniform Gutter Geometry Manning's n: 0.0150 Gutter Width: 0.0010 ft Design Flow: 1.0000 cfs

#### **Gutter Result Parameters**

Width of Spread: 6.5208 ft Gutter Depression: 0.0000 in Area of Flow: 0.4252 ft<sup>2</sup> Eo (Gutter Flow to Total Flow): 0.0004

#### Inlet Input Parameters

Inlet Location: Inlet on Grade Inlet Type: Grate Grate Type: P - 1-1/8 Grate Width: 4.4200 ft Grate Length: 2.4100 ft Local Depression: 1.0000 in

#### Inlet Result Parameters

Intercepted Flow: 1.0000 cfs Bypass Flow: 0.0000 cfs Approach Velocity: 2.3518 ft/s Splash-over Velocity: 7.1221 ft/s Efficiency: 1.1614



## APPENDIX D

## **StormCAD Model Results**

### FlexTable: Catch Basin Table

ID	Label	Elevation (Rim) (ft)	Elevation (Invert) (ft)	Flow (Additional Subsurface) (cfs)	Hydraulic Grade Line (In) (ft)
43	CB-1	1,509.00	1,507.00	0.60	1,507.43
48	CB-3	1,507.91	1,504.12	0.60	1,504.55
36	CB-4	1,507.64	1,505.64	1.00	1,506.20

#### Active Scenario: 10-year

050941.stsw 4/30/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

### FlexTable: Conduit Table

ID	Label	Start Node	Stop Node	Length (Scaled) (ft)	Slope (Calculated) (ft/ft)	Diameter (in)	Manning's n	Flow (cfs)	Velocity (ft/s)
41	CO-3	CB-1	0-1	60.1	0.051	15.0	0.013	0.60	5.86
50	CO-4	CB-3	0-2	15.8	0.013	15.0	0.013	0.60	3.58
52	CO-5	CB-4	O-3	139.1	0.012	15.0	0.013	1.00	4.12

#### Active Scenario: 10-year

050941.stsw 4/30/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

### Profile Report Engineering Profile - CB-1 to O-1 (050941.stsw) Active Scenario: 10-year



Station (ft)

050941.stsw 4/30/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666



### Station (ft)

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 StormCAD [10.03.02.04] Page 1 of 1

050941.stsw 4/29/2021

### Profile Report Engineering Profile - CB-4 to O-3 (050941.stsw) Active Scenario: 10-year



Station (ft)

050941.stsw 3/12/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666



## APPENDIX E

## **Retention and Dissipation Calculations**

### **RETENTION WORKSHEET FOR 100-YR, 2-HR EVENT**

Project: 050941 Scottsdale Airpark

Description: Preliminary

Prepared by:  $\mathsf{H}\mathsf{T}$ 

DATE: 4/30/2021

Basin A Summary					
Extra P	<b>Required Drywells</b>				
305 cf	0.0070 Ac-ft	1.0			

#### Volume Required:

 $Vr = D/12 \times A \times C$ 

		A = Area in Square Feet C = Coefficient of Runoff			
Sub Area Type	Area (sf)	Runoff Coefficient	Retention Required (cf)	Retention Required (ac-ft)	
Concrete/Roof	20,601	0.95	3,686 cf	0.08 Ac-ft	
Desert L/S	14,600	0.70	1,925 cf	0.04 Ac-ft	
Totals			5,611 cf	0.13 Ac-ft	
	То	tal Retention Required	5,611 cf	0.13 Ac-ft	

#### **Retention Basin Volume Provided:**

Contour Elevation	Depth Increment	Area (sf)	Volume (cf)	Cumm. Volume (cf)
1507.00	0.00	447	0	0
1508.00	1.00	860	653	653
TOTAL DEPTH:	0.00		Volume Provided:	653

Estimated High Water: 1,508.00

D = 100yr 2hr Rainfall, 2.26

#### **Underground Retention Volume Provided:**

Pipe Diameter (ft)	Pipe Length (ft)	Volume (cf)	
10.00	67.00	5,262	
		Volume Provided: Total Volume Provided:	5,262 cf 5,915 cf

#### **Dissipation Calculations**

Retention	Derated Surface	Basin		Volume to be
Required	Percolation Rate	Bottom Area	Surface Percolation	Drained Through
(cf)	(cf/hr/sf)	(sf)	in 36hrs (cf)	Drywells (cf)
5,611	0.00	0	0	5,611

#### **Dissipation Calculations for Drywells**

Volume to be drain Through Drywells (cf)	Drywell Percolation Rate (cf/36hr)	Number of Drywells Required
5,611	12,960	1.0



## APPENDIX F

## Warning and Disclaimer Liability

### GRADING & DRAINAGE GRADING & DRAINAGE LANGUAGE

#### WARNING AND DISCLAIMER OF LIABILITY

The City's Stormwater and Floodplain Management Ordinance is intended to minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding. The Stormwater and Floodplain Management Ordinance identifies floodplains, floodways, flood fringes and special flood hazard areas. However, a property outside these areas could be inundated by floods. Also, much of the city is a dynamic flood area; floodways, floodplains, flood fringes and special flood hazard areas may shift from one location to another, over time, due to natural processes.

WARNING AND DISCLAIMER OF LIABILITY

The flood protection provided by the Stormwater and Floodplain Management Ordinance is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by constructed or natural causes. The Stormwater and Floodplain Management Ordinance does not create liability on the part of the city, any officer or employee thereof, or the federal, state or county government for any flood damages that result from reliance on the Ordinance or any administrative decision lawfully made thereunder.

Compliance with the Stormwater and Floodplain Management Ordinance does not ensure complete protection from flooding. Flood-related problems such as natural erosion, streambed meander, or constructed obstructions and diversions may occur and have an adverse effect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations.

I have read and understand the above.

Plan Check #

Owner

Date