Preliminary Basis of Design

Drainage

FOR

Scottsdale Boutique Hotel

APN# 130-21-014D

7321 E Osborn Dr Scottsdale, AZ 85251 Submittal - *April 3,* 2020

Prepared for: Gerald Kesler GKI Architects 1823 E. Desert Lane Phoenix, AZ 85251

Prepared by: Mike Jackson, P.E.



1600 N Desert Drive, Suite 230

Tempe, AZ 85281

Phone: (480) 951-0517 Fax: (480) 951-2353



April 3, 2020

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1600 N. Desert Drive, Suite 230, Tempe, AZ 85281 \$480.951.0517 \$Fax: 480.951.2353 \$imegcom.com

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INTRODUCTION

The purpose of this report is to indicate the methods used and to provide discussion of the proposed stormwater drainage for a new 4 story boutique hotel near the intersection of Osborn Drive and N Scottsdale Rd, serving the City of Scottsdale. The boutique hotel will be a 68 room limited service hotel as well as 57 parking spaces with an additional loading/shuttle parking space. The proposed project is approximately 0.726 acres. The proposed improvements include construction of a parking lot, open space areas, a new building, driveways, pedestrian access ramps, sidewalks, utilities, and stormwater drainage structures. This report will indicate how the storm water generated on the subject property is addressed and how it complies with the City of Scottsdale drainage requirements.

EXISTING CONDITIONS

The project site is located southeast of the intersection of Osborn Road and N Scottsdale Rd at the end of a cul-de sac, E. Osborn Drive. is currently an existing medical facility on site. The site is bounded by existing commercial on all hotel to the south, restaurant to the east, and medical/office to the north and east. The total property area is approximately 0.76 acres and the existing development consists of an existing medical facility and supporting site improvements. Refer to Vicinity Map (Figure 1) below for a general graphical representation of the Site location.



Figure 1

1600 N. Desert Drive, Suite 230, Tempe, AZ 85281 \$480.951.0517 \$Fax: 480.951.2353 \$imegcorp.com

EXISITNG DRAINAGE

The Site is currently completely developed and is currently a dialyses center. The site generally slopes toward an existing drywell in the eastern row of the parking lot. The overall area generally drains to the southeast. Based on field observation and available topographic data some shallow ponding currently occurs in the existing open spaces but appears to eventually discharge to the pavement and to the drywell. The site ultimately overflows to the northeast into the cul-de-sac of Osborn Drive.

FEMA FLOOD ZONE / FIRM MAP

This is to certify that the above subject property lies within unshaded zone 'x' as designated on the FIRM flood insurance rate map, map number 04013C2235L, dated October 16, 2013. Unshaded zone 'x' is designated as being areas outside of the 0.2% annual chance flood.

PROPOSED DRAINAGE PLAN

To mitigate the drainage flows we propose to retain the first flush volume within an underground storage system on-site.

An rational method analysis was performed for the prevs post development and the first flush storage volumes for this project. Due to the site being completely developed and lacking any apparent existing retention volume, the first flush volume was greater than the prevs post volume. The required storage volume is proposed to be stored in an 8ft dia. underground CMP pipe, referred to in this report as UG Pipe. The entire site will discharge via catch basins to the underground Pipe, the underground retention consists of 28 LF, 8ft dia. CMP pipe with a provided retention volume of 1,407 CF. Discharges to this storage pipe will be routed in 18" HDPE storm drain pipe and bleed off to a drywell which will drain in approximately 4 hours.

Building Finished Floor elevation shall be set a minimum of one foot above low top of curb of adjacent site outfall.

DATA ANALYSIS METHODS

The computations included in this report are based on the procedure described in the City of Scottsdale Design Standards and Policies Manual and the design standards and methodologies developed by the Flood Control District of Maricopa County. The first flush 2 storm event was used to calculate the retention volume required across the site.

| Condition | Area (sf) | Area (ac) | Cw * | Precipitation** (in) | Retention Volume Required (cf) | Pre vs. Post Volume Analysis |
|-----------|--------------|--------------|------|-------------------------|---|---------------------------------|
| Pre | 31,612 | 0.73 | 0.89 | 2.16 | 5,064 | Δ (cf) pre vs. post |
| Post | 31,612 | 0.73 | 0.93 | 2.16 | 5,292 | 228 |
| FF | 31,612 | 0.73 | 1 | 0.5 | 1,317 | FF > Pre vs. Post |

Table 1-Retention Volume Requirements

*Weighed C for drainage areas are based on the Commercial and Industrial Areas from Figure 4-1.4 of the DSPM, where C =1.0 was assumed for the FF analysis. **100-yr, 2 hr precipitation = 2.16" Per NOAA14

Table 2- Retention Volume Provided

| Retention | Pipe Dia [ft] | Pipe area [ft2] | Length [ft] | Volume [ft3] |
|-----------|---------------|-----------------|-------------|--------------|
| UG pipe | 8 | 50.27 | 28 | 1,407 |

Table 3- Drawdown Time

| Retention Basin ID | Volume Pro [ft3] | perc rate[cfs] | # of drywells | drawdown time [hrs] |
|-----------------------|------------------|----------------|---------------|------------------------|
| UG pipe | 1,407 | 0.1 | 1 | 4 |

WARNING AND DISCLAIMER OF LIABILITY

Refer to Appendix 4 for a copy of the Warning & Disclaimer of Liability form.

CONCLUSION

This project has been designed to conform to the City of Scottsdale storm drainage design requirements. Proposed drainage improvements will include an underground retention system sized to retain the onsite volume for the first flush. Proposed storm drain inlets and storm drain pipes will be adequately sized to convey the expected peak flows to the underground storage system which will drain the retained volume within 36 hours via drywell. Excess flows generated onsite will overflow to E Osborn Drive. No

adverse impacts to the offsite downstream properties are anticipated as a result of the proposed improvements.

REFERENCES

City of Scottsdale, Design Standards & Policies Manual, January 2010.

Flood Control District of Maricopa County. Drainage Design Manual for Maricopa County, Arizona, Volume 1. August 15, 2013.

Flood Control District of Maricopa County. Drainage Design Manual for Maricopa County, Arizona, Volume 2. August 15, 2013.

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Scottsdale Boutique 7321 E Osborn Dr

Appendix 1: Figures

PRELIMINARY GRADING AND DRAINAGE PLAN

LEGAL DESCRIPTION

THAT PORTION OF LOTS 1 AND 2, OF SCOTTSDALE OSBORN PLAZA AMENDED, ACCORDING TO BOOK 179 OF MAPS, PAGE 45; THEREAFTER AFFIDAVIT OF CORRECTION RECORDED IN DOCKET 11372, PAGE 500 AND IN RECORDING NO. 2004- 1449076, RECORDS OF MARICOPA COUNTY, ARIZONA, DESCRIBED AS FOLLOWS:

BEGINNING AT THE NORTHEAST CORNER OF SAID LOT 2:

THENCE SOUTH 89 DEGREES 58 MINUTES 35 SECONDS WEST, A DISTANCE OF 240.10 FEET TO THE SOUTHEAST CORNER OF THE PROPERTY DESCRIBED IN INSTRUMENT RECORDED IN DOCKET 11594, PAGE 834. RECORDS OF MARICOPA COUNTY. ARIZONA:

THENCE NORTH, A DISTANCE OF 142.11 FEET ALONG THE EAST LINE OF THE LAST REFERENCED PROPERTY TO THE SOUTHWEST CORNER OF THE PROPERTY DESCRIBED IN DEED RECORDED IN DOCKET 12434, PAGE 816, RECORDS OF MARICOPA COUNTY, ARIZONA;

THENCE NORTH 89 DEGREES 58 MINUTES 35 SECONDS EST, A DISTANCE OF 158.15 FEET ALONG THE SOUTH LINE OF THE LAST REFERENCED PROPERTY TO A POINT ON A CIRCULAR CURVE CONCAVE TO THE NORTHEAST AND WHOSE RADIUS POINT BEARS NORTH 81 DEGREES 01 MINUTE 38 SECONDS EAST, A DISTANCE OF 45:00 FEET;

THENCE SOUTHEASTERLY ALONG THE ARC OF SAID LAST MENTIONED CURVE THROUGH A CENTRAL ANGLE OF 127 DEGREES 14 MINUTES 15 SECONDS, A DISTANCE OF 99.93 FEET TO A POINT ON A CIRCULAR CURVE CONCAVE TO THE SOUTHEAST AND WHOSE RADIUS POINT BEARS SOUTH 46 DEGREES 12 MINUTES 37 SECONDS EAST, A DISTANCE OF 20.00 FEET;

THENCE NORTHEASTERLY ALONG THE ARC OF THE LAST MENTIONED CURVE THROUGH A CENTRAL ANGLE OF 18 DEGREES 15 MINUTES 25 SECONDS, A DISTANCE OF 6.37 FEET TO THE POINT OF BEGINNING.

LEGEND

| | PROPERTY BOUNDARY | S | SLOPE |
|----------|-------------------------------|-------------|-------------------------|
| | SAWCUT LINE/LIMITS OF GRADING | FT | FOOT |
| | CENTER LINE | TC | TOP OF CURB ELEVATION |
| 1581 — — | EXISTING MINOR CONTOUR | GB | GRADE BREAK |
| 1580 | EXISTING MAJOR CONTOUR | SW | SIDEWALK |
| | | Р | PAVEMENT |
| 1581 ——— | PROPOSED MINOR CONTOUR | С | CONCRETE |
| 1580 | PROPOSED MAJOR CONTOUR | | GRADE BREAK |
| W | PROPOSED WATER LINE SERVICE | 1.0% | FLOW ARROW |
| | CONCEPTUAL GAS LINE | • P=1425.00 | PROPOSED SPOT ELEVATION |
| 6"S | PROPOSED SANITARY SEWER LINE | (P=1424.25) | EXISTING SPOT ELEVATION |

STORMWATER MAINTENANCE PLAN

DURING CONSTRUCTION IT IS THE OWNER'S AND CONTRACTOR'S RESPONSIBILITY TO PERFORM THE MAINTENANCE IN REGARDS TO THE STORM WATER SYSTEM AND STRUCTURE.

FOLLOWING CONSTRUCTION IT WILL BE THE OWNER'S RESPONSIBILITY TO CONTINUE MAINTENANCE. THE OWNERS OF THE PROPERTY SHALL BE ADVISED TO CLEAN OUT THE SUMPS OF ALL CATCH BASINS AND UNDERGROUND CHAMBERS SYSTEM AND ROUTINELY CHECK RESTRICTED OUTLETS FOR OBSTRUCTIONS AT A MINIMUM.

DRAINAGE STATEMENT

THE PROPOSED DEVELOPMENT IS IN COMPLIANCE WITH THE CITY OF SCOTTSDALE CRITERIA AND OTHER REQUIRED DRAINAGE DESIGN REQUIREMENTS. NO ADVERSE DRAINAGE IMPACTS ARE EXPECTED TO EITHER DOWNSTREAM EXISTING PROPERTIES OR DRAINAGE WAYS FROM THE SITE.

BASED ON THE PROPOSED GRADING THE SITE HAS BEEN ANALYZED AS ONE DRAINAGE AREA. THE 100-YEAR, 2-HOUR RUNOFF VOLUME GENERATED ONSITE HAS BEEN DETERMINED USING A WEIGHTED C FOR THE PRE AND POST DEVELOPMENT CONDITIONS. THE DIFFERENCE IN THE PRE VS POST DEVELOPMENT VOLUMES WAS DETERMINED TO BE SMALLER THAN THE VOLUME GENERATED BY THE FIRST FLUSH (FF). IT IS PROPOSED THE LARGER OF THE TWO, THE FIRST FLUSH VOLUME BE RETAINED ONSITE IN AN 8' DIA CMP UNDERGROUND RETENTION PIPE. THE UNDERGROUND SYSTEM IS PROPOSED TO DRAIN THE RETAINED VOLUME WITHIN 36 HOURS VIA DRYWELL.

UTILITY NOTES

ALL NEW WATER AND SEWER CONSTRUCTION, MATERIALS, AND APPURTENANCES SHALL BE PER DSPM, CHAPTERS 6 & 7 AND THE CITY OF SCOTTSDALE STANDARD SERIES DETAIL 2300 AND 2400

| RETENTION VOLUMES | | | | | | | | | | |
|---|---------------|---------------------------------|------|---------------|-------|----------------|--|--|--|--|
| Condition | Area (ft²) | Pre vs. Post Volume Analysis | | | | | | | | |
| Pre | 31,612 | 0.73 | 0.89 | 2.16 | 5,064 | Δ (cf) pre vs. | | | | |
| Post | 31,612 | 0.73 | 0.93 | 93 2.16 5,292 | | - post 228 | | | | |
| FF 31,612 0.73 1 0.5 1,317 FF > Pre vs. F | | | | | | | | | | |
| *Weighed C for drainage areas are based on the Commercial and Industrial Areas from Figure 4-1.4 of the DSPM, where C=1.0 was assumed for the FF analysis. **100-yr. 2 hr precipitation=2.16" Per NOAA14. | | | | | | | | | | |

7321 E OSBORN DRIVE

A PORTION OF THE NORTHWEST QUARTER OF THE SOUTHEAST QUARTER OF SECTION 26, TOWNSHIP 2 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER BASE AND MERIDIAN, MARICOPA COUNTY, ARIZONA



RETENTION REQUIRED

RETENTION REQUIRED = 1,317 CF

RETENTION PROVIDED

UNDERGROUND RETENTION PIPE 8' PIPE X 28 LF RETENTION PIPE VOL. PROVIDED = 1,407 CF

TOTAL VOL. PROVIDED = 1,407 CF

DRYWELL DISCHARGE

ASSUMED 0.1 CFS (TO BE CONFIRMED BY CONTRACTOR) DRAWDOWN: 1,407 CF/ 0.1 CFS = 4 HRS USE ONE DRYWELL 4 HRS < MAX 36-HR STATE REQUIREMENT



| SHEE | T INDEX |
|--------------|--|
| AWING NUMBER | SHEET TITLE |
| C1 | COVER SHEET |
| C2 | PRELIMINARY GRADING & DRAINAGE PLAN |

ARCHITECT

GKI ARCHITECTS 1823 E. DESERT LANE PHOENIX, AZ 85251 CONTACT: GERALD KESLER

CIVIL ENGINEER

IMEG CORP 1600 N. DESERT DRIVE, SUITE 230 TEMPE, AZ 85281 PHONE: 480-378-3925 CONTACT: MIKE JACKSON

SITE DATA

A.P.N: 130-21-014D AREA : 31,612 SF OR 0.726 AC. ADDRESS: 7321 E OSBORN DRIVE SCOTTSDALE, ARIZONA 85251

BASIS OF BEARING

THE BASIS OF BEARING USED FOR THIS SURVEY IS THE WEST LINE OF THE NORTHWEST QUARTER OF SECTION 26, TOWNSHIP 2 NORTH, RANGE 4 EAST, AS SHOWN ON THE FINAL PLAT AMENDED OF "SCOTTSDALE OSBORN PLAZA" RECORDED IN BOOK 179 PAGE 45, MARICOPA COUNTY RECORDS.

SAID BEARING = NORTH 00° 00' 00" EAST

SITE BENCHMARK

BRASS CAP IN HANDHOLE LOCATED AT THE CENTERLINE INTERSECTION OF OSBORN ROAD & SCOTTSDALE ROAD.

SAID ELEVATION = 1246.82 (N.A.V.D.88)

FLOOD PLAIN CERTIFICATION

THIS IS TO CERTIFY THAT THE SUBJECT PROPERTY LIES WITHIN ZONE 'X' AS DESIGNATED ON THE FIRM FLOOD INSURANCE RATE MAP, MAP NUMBER 04013C2235L, DATED OCTOBER 16, 2013.

| COMMUNITY NUMBER | PANEL # PANEL DATE | SUFFIX | DATE OF FIRM (INDEX DATE) | FIRM ZONE | BASE FLOOD ELEV (IN AO ZONE, USE DEPTH) |
|---------------------|--------------------------|--------|------------------------------|--------------|---|
| 045012 | 2235 10/16/13 | L | 12.04.2015 | Х | N/A |



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SCALE: 1'' = 30'



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| 1" = 20' Call at least two full working days before you begin excavation. | Dra Ch Da | eck te: | (ed B 03/3 | 1 y: 31/2 2 | N 202 | 1A. 20 | J |

SCALE: 1" = 20'

National Flood Hazard Layer FIRMette

Legend

0 250

500

1,000

1,500

Feet 1:6,000

2,000

33°28'57.54"N

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unmapped and unmodernized areas can

regulatory purposes.

Appendix 2: NOAA 14 Precipitation Values

NOAA Atlas 14, Volume 1, Version 5 Location name: Scottsdale, Arizona, USA* Latitude: 33.4866°, Longitude: -111.9249° Elevation: 1242.64 ft** * source: ESRI Maps ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

| PDS | PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹ | | | | | | | | | | | | |
|----------|--|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|------------------------------|--|--|--|
| Duration | | | | Averag | ge recurrenc | e interval (y | /ears) | | | | | | |
| Duration | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 500 | 1000 | | | |
| 5-min | 0.183 (0.153-0.222) | 0.239 (0.202-0.290) | 0.324 (0.272-0.394) | 0.390 (0.325-0.471) | 0.480 (0.393-0.576) | 0.549 (0.444-0.656) | 0.619 (0.491-0.738) | 0.691 (0.539-0.822) | 0.787 (0.598-0.938) | 0.860 (0.641-1.03) | | | |
| 10-min | 0.278 (0.233-0.338) | 0.363 (0.307-0.442) | 0.494 (0.414-0.599) | 0.594 (0.495-0.717) | 0.730 (0.598-0.876) | 0.835 (0.675-0.998) | 0.941 (0.747-1.12) | 1.05 (0.820-1.25) | 1.20 (0.909-1.43) | 1.31 (0.975-1.56) | | | |
| 15-min | 0.344 (0.289-0.419) | 0.450 (0.380-0.548) | 0.612 (0.513-0.742) | 0.737 (0.613-0.889) | 0.905 (0.741-1.09) | 1.03 (0.837-1.24) | 1.17 (0.926-1.39) | 1.30 (1.02-1.55) | 1.48 (1.13-1.77) | 1.62 (1.21-1.94) | | | |
| 30-min | 0.464 (0.389-0.564) | 0.607 (0.512-0.738) | 0.825 (0.691-1.00) | 0.992 (0.826-1.20) | 1.22 (0.998-1.46) | 1.39 (1.13-1.67) | 1.57 (1.25-1.87) | 1.75 (1.37-2.09) | 2.00 (1.52-2.38) | 2.19 (1.63-2.61) | | | |
| 60-min | 0.574 (0.481-0.698) | 0.751 (0.633-0.913) | 1.02 (0.855-1.24) | 1.23 (1.02-1.48) | 1.51 (1.24-1.81) | 1.73 (1.39-2.06) | 1.95 (1.54-2.32) | 2.17 (1.69-2.59) | 2.47 (1.88-2.95) | 2.71 (2.01-3.23) | | | |
| 2-hr | 0.664 (0.567-0.792) | 0.861 (0.735-1.03) | 1.15 (0.981-1.37) | 1.38 (1.16-1.63) | 1.68 (1.40-1.98) | 1.92 (1.57-2.25) | 2.16 (1.74-2.53) | 2.40 (1.90-2.82) | 2.73 (2.11-3.21) | 2.99 (2.26-3.53) | | | |
| 3-hr | 0.722 (0.613-0.867) | 0.926 (0.790-1.12) | 1.22 (1.03-1.46) | 1.45 (1.22-1.73) | 1.77 (1.47-2.10) | 2.03 (1.66-2.40) | 2.30 (1.85-2.72) | 2.58 (2.04-3.05) | 2.97 (2.27-3.51) | 3.29 (2.45-3.90) | | | |
| 6-hr | 0.869 (0.754-1.02) | 1.10 (0.959-1.30) | 1.41 (1.23-1.66) | 1.66 (1.43-1.94) | 2.00 (1.70-2.32) | 2.27 (1.89-2.62) | 2.55 (2.09-2.94) | 2.83 (2.28-3.27) | 3.22 (2.53-3.73) | 3.52 (2.70-4.10) | | | |
| 12-hr | 0.972 (0.851-1.13) | 1.23 (1.08-1.43) | 1.56 (1.36-1.80) | 1.82 (1.58-2.10) | 2.17 (1.86-2.49) | 2.43 (2.06-2.79) | 2.71 (2.26-3.11) | 2.99 (2.46-3.44) | 3.36 (2.70-3.89) | 3.66 (2.88-4.26) | | | |
| 24-hr | 1.16 (1.04-1.31) | 1.48 (1.32-1.67) | 1.92 (1.71-2.15) | 2.26 (2.01-2.54) | 2.74 (2.42-3.07) | 3.12 (2.74-3.49) | 3.52 (3.06-3.93) | 3.93 (3.39-4.39) | 4.49 (3.84-5.03) | 4.94 (4.18-5.54) | | | |
| 2-day | 1.26 (1.12-1.42) | 1.61 (1.44-1.81) | 2.11 (1.88-2.37) | 2.51 (2.23-2.82) | 3.07 (2.72-3.44) | 3.52 (3.09-3.94) | 3.99 (3.48-4.48) | 4.48 (3.88-5.03) | 5.17 (4.43-5.81) | 5.72 (4.85-6.45) | | | |
| 3-day | 1.33 (1.19-1.50) | 1.70 (1.52-1.91) | 2.24 (1.99-2.51) | 2.67 (2.37-2.99) | 3.28 (2.90-3.67) | 3.77 (3.30-4.21) | 4.28 (3.73-4.80) | 4.83 (4.17-5.41) | 5.60 (4.78-6.28) | 6.21 (5.25-6.99) | | | |
| 4-day | 1.40 (1.25-1.58) | 1.79 (1.60-2.02) | 2.36 (2.10-2.65) | 2.83 (2.51-3.17) | 3.48 (3.07-3.90) | 4.01 (3.52-4.49) | 4.58 (3.98-5.12) | 5.18 (4.47-5.80) | 6.02 (5.13-6.74) | 6.71 (5.66-7.53) | | | |
| 7-day | 1.55 (1.38-1.75) | 1.98 (1.77-2.24) | 2.62 (2.33-2.94) | 3.13 (2.78-3.52) | 3.86 (3.41-4.33) | 4.44 (3.90-4.98) | 5.07 (4.41-5.68) | 5.73 (4.95-6.43) | 6.66 (5.68-7.48) | 7.42 (6.25-8.34) | | | |
| 10-day | 1.69 (1.51-1.90) | 2.16 (1.93-2.43) | 2.85 (2.54-3.20) | 3.41 (3.02-3.81) | 4.18 (3.69-4.67) | 4.81 (4.22-5.37) | 5.47 (4.76-6.11) | 6.16 (5.33-6.89) | 7.14 (6.10-7.99) | 7.92 (6.70-8.88) | | | |
| 20-day | 2.07 (1.86-2.32) | 2.67 (2.39-2.98) | 3.52 (3.15-3.93) | 4.17 (3.71-4.64) | 5.04 (4.47-5.61) | 5.71 (5.05-6.35) | 6.39 (5.62-7.12) | 7.08 (6.20-7.89) | 8.01 (6.95-8.95) | 8.72 (7.51-9.76) | | | |
| 30-day | 2.42 (2.16-2.71) | 3.12 (2.79-3.48) | 4.11 (3.66-4.57) | 4.86 (4.33-5.40) | 5.87 (5.20-6.52) | 6.64 (5.86-7.38) | 7.44 (6.54-8.26) | 8.25 (7.21-9.16) | 9.34 (8.10-10.4) | 10.2 (8.75-11.3) | | | |
| 45-day | 2.81 (2.52-3.13) | 3.62 (3.25-4.03) | 4.76 (4.27-5.31) | 5.61 (5.02-6.25) | 6.73 (6.00-7.49) | 7.57 (6.73-8.43) | 8.42 (7.45-9.38) | 9.27 (8.16-10.3) | 10.4 (9.08-11.6) | 11.2 (9.76-12.6) | | | |
| 60-day | 3.11 (2.80-3.46) | 4.01 (3.61-4.46) | 5.28 (4.74-5.86) | 6.20 (5.55-6.88) | 7.39 (6.61-8.21) | 8.28 (7.37-9.19) | 9.17 (8.13-10.2) | 10.0 (8.87-11.2) | 11.2 (9.81-12.5) | 12.0 (10.5-13.4) | | | |

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

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Maps & aerials

Large scale terrain

Large scale map

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US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

Disclaimer

Appendix 3: Warning and Disclaimer of Liability