3 engineering

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SOUTHDALE

Preliminary Drainage Report

3 engineering Job #: 1872

June 28, 2021

COS #: 5-ZN-2020

3 engineering
planning civil engineering surveying

SOUTHDALE

7000 E. MCDOWELL ROAD, SCOTTSDALE, AZ 85257

PRELIMINARY DRAINAGE REPORT

Prepared for:

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4700 S. McClintock Drive #160
Scottsdale, Arizona 85257
Contact: Mark Mitchell
Phone: (480) 223-8239

Daniel G. Mann, P.E.

June 28, 2021

Submittal to:
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7447 E. Indian School Road, Suite 105
Scottsdale, AZ 85251

Prepared by:
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Scottsdale, Arizona 85251
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Job Number 1872

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Appendix

1. Introduction

The purpose of this report is to present the existing and proposed drainage plan for the project site, Southdale. It is our opinion the proposed grading and drainage concept is in accordance with the City of Scottsdale drainage requirements.

The project site, Southdale, is located in the southeast quarter of Section 34, Township 2 North, Range 4 East of the Gila and Salt River Meridian, Maricopa County, Arizona within the City of Scottsdale. The project is located at 7000 E. McDowell Road, Scottsdale, AZ 85257. The site is bounded on the north by an apartment complex, on the east by a commercial development, on the south by McDowell Road, and on the west by 70th Street. See Appendix A for a vicinity map and Appendix G for offsite aerial photographs.

The existing zoning is C-3. The land is currently used as a commercial development. The proposed zoning is PUD. The City of Scottsdale 2001 General Plan shows the site as a Mixed-Use Neighborhood. The proposed site is a 267-unit apartment complex with office and retail space. The site currently lies within the "Zone X" floodplain designation.

2. Site Description

Existing

The project site currently consists of a fully developed commercial development with landscaping, drive aisles, parking, and utilities. The existing topography of the site generally slopes from northwest to southeast at approximately 1.0 percent (1.0%). The site currently does not retain stormwater and discharges flows to McDowell Road at the site outfall located at the southeast corner of the site. The site does not show any signs of containing waters of the US (404 washes). See Appendix F for an aerial photograph of the site.

Federal Emergency Management Agency (FEMA) Designation

According to FEMA Flood Insurance Rate Map (FIRM) # 04013C2235L, dated October 16, 2013, the site is located within the "Zone X" floodplain designation.

"Zone X" is described as follows:

"Areas determined to be outside the 0.2% annual chance floodplain."

Refer to the updated Flood Insurance Rate Map information in Appendix B.

Proposed

The proposed project includes constructing a 267-unit multi-family apartment complex with office and retail space. The site also includes drive aisles, surface parking, a parking structure, and landscape. See the proposed improvements in the Onsite Drainage Area Map in Appendix H.

3. Drainage Design - Offsite

The site is not considered to be affected by offsite flows. The Maricopa County Flood Control District Flo-2d maps from the Lower Indian bend Wash Study show offsite flows entering the site from the west. See Appendix K for Flo-2d maps. The physical conditions of the site differ from the assumptions in the Flo-2d analysis. The site is blocked from offsite flows and is discussed below.

Flows are generated within an offsite drainage area west of the site that travel east toward 70th Street through Coronado Road, Almeria Road and an alley south of Almeria Road. See Watershed Delineation and Topography Map in Appendix D. The peak discharge for the

drainage area was determined to be 34 cfs for the 100-year storm event using the FCDMC DDMSW HEC-1 Model, which is consistent with the flows shown in the Flo-2d map. The flow is conveyed to the west half of 70th Street which has a capacity of 21.53 cfs (cross section A). Therefore, 12.47 cfs breaks over to the east side of 70th Street. With a flow of 12.47 cfs, the water surface elevation in the east side of 70th street is at an elevation of 42.35 at the location of a 20-ft wide driveway to the apartment complex north of the site (cross section B). The back of the driveway has an elevation of 42.17. The breakover was modeled as a weir with a width of 20-ft and a depth of 0.18-ft, resulting in a flow of 4.58 cfs that enters the apartment complex (weir section B). The remaining 7.89 cfs travels south in the east half of 70th street, adjacent to the site, which has a capacity of 11.34 cfs (cross section C). There is a block wall/solid building walls that extend the entire property line between the project site and the apartment complex. There is an opening at a foot-gate along the wall, however it is 1.3-ft above the adjacent catch basin in the apartment complex drive aisle. Therefore, no offsite flows affect the site. There is a drywell in the alley behind the existing apartment complex to the northwest that dissipates any storm water in this area.

Refer to Appendix D for a Watershed Delineation and Topography map and Appendix E for Offsite Drainage Calculations including HEC-1 Results and Flowmaster Calculations. Cross section locations are shown in the Onsite Drainage Area Map, Appendix H.

Hydraulic Parameters

Bentley Flowmaster V8i was used to calculate the street capacity for sections in 70th Street for 100-year flows. The Flood Control District of Maricopa County Drainage Design Management System for Windows (DDMSW) was used to determine the flow for 100-year storm event . See results in Appendix E.

4. ***Drainage Design - Onsite***

The City of Scottsdale Design Standards and Policies Manual and the Drainage Design Manual for Maricopa County, Volume 1 was followed in designing on-site drainage facilities for the site.

Refer to the Preliminary Grading and Drainage Plan in Appendix L, the Onsite Drainage Map in Appendix H and Inlet Area Exhibit in Appendix I for the following discussion. The proposed site is required to provide retention for the pre- vs. post-project runoff for the 100-year, 2-hour storm or the 0.5" First Flush Storm (with weighted runoff coefficient), whichever is greater. The c value for the proposed site is less than the existing site, so the pre- vs. post- runoff method results in no required volume. Therefore, the First Flush Storm is used. There is no retention provided for the "true rooftop" areas with no amenities, see Appendix H for areas. See Appendix J for Onsite Drainage Calculations.

The required retention volume for Drainage Area A is 2,706 c.f. There is 2,764 c.f. provided in 220-L.F. of 48" storm drain pipe. Flows generated within this drainage area are conveyed via roof drains and surface flow to five catch basins that connect directly into the underground retention pipe. In a storm event greater than the 0.5" first flush the retention pipe will fill and overflow through the southernmost catch basin to McDowell Road. The system is modeled in Bentley StormCAD using a tail water elevation of 6" above the highpoint breakover at the driveway. A proposed dry-well will provide a dry up time of 36 hours with an infiltration rate of 0.02 cfs. Additionally, the two courtyard areas have secondary overflow catch basins elevated 0.25-ft above the primary catch basin. The overflow catch basins will not receive flow until the

underground retention is full or the primary catch basins are clogged. The overflow catch basin in the north courtyard, area A-4, is conveyed to a bubble-up outlet in the fire lane west of the building. The bubble-up structure is connected to the underground retention with a bleed off pipe and does not require an additional drywell. The overflow catch basin for the south courtyard, area A-5, outlets to the right of way in McDowell Road.

The required retention volume for Drainage Area B is 479 c.f. There is 528 c.f. provided in 42-L.F. of 48" storm drain pipe. Flows generated within this drainage area are conveyed via surface flow to two catch basins that connect directly into the underground retention pipe. In a storm event greater than the 0.5" first flush the retention pipe will fill and overflow through the north catch basin to 70th Street. The system is modeled in Bentley StormCAD using a tail water elevation of 0.13-ft above the highpoint breakover at the driveway. A proposed dry-well will provide a dry up time of 36 hours with an infiltration rate of 0.01 cfs.

The proposed finish floor of the new building is 1243.00, which is 12" above the highest adjacent catch basin inlet elevation of 1242.00. Therefore, the finish floor meets the requirement of 12" above the adjacent high-water elevation per the Maricopa County Drainage Standards. Additionally, the proposed finish floor elevations are greater than 14" above the site outfall of 1237.85, exceeding the Maricopa County Drainage Standard requirement.

Hydraulic Parameters

For onsite peak flows, the Rational Method will be used as follows:

$$Q=CiA$$

where: C = Composite runoff coefficient = weighted by ground cover

i = Intensity corresponding to T_c

T_c = Time of concentration (10 minute minimum used)

A = Area in acres

The 100-year runoff coefficient for this development to be used is 0.95 for impervious areas and 0.45 for pervious areas per the Maricopa County Drainage Policies and Standards. The rainfall is based upon the NOAA Atlas 14, Volume 1, Version 5, dated 2011, 90% confidence interval, mean partial duration time series data.

Determination of Catch Basin capacity operating as a weir by using the following formula: $Q=C_wxPxd^{1.5}$ x (1-CF)

*w*here: C_w = Weir coefficient (3.0)
Q = Discharge Capacity (cfs)
P = Inlet Perimeter
d = Flow depth
CF = Clogging Factor. 40% clogging (or 0.40) used

Please refer to APPENDIX J for catch basin computations.

The on-site storm drain pipes are designed to accommodate flows resulting from the 100-year storm event. To calculate the capacity of the storm drain pipes, StormCAD V8i by Bentley Systems, Inc. was used, see results in Appendix J. The 100-year flow calculated for a given drainage area was assigned to the corresponding catch basin. Inlet areas are shown in the Inlet Area Exhibit in Appendix I. The tailwater for each outlet of the retention pipes were set above the adjacent driveway breakover elevations.

5. Conclusions

The following is a summary of the Southdale Preliminary Drainage Report.

- The site currently lies within the "Zone X" floodplain designation.
- Retention is provided for the 0.5" first flush storm for the site.
- No offsite flows affect the site.
- The finish floors are safe from the 100-year storm event.

6. References

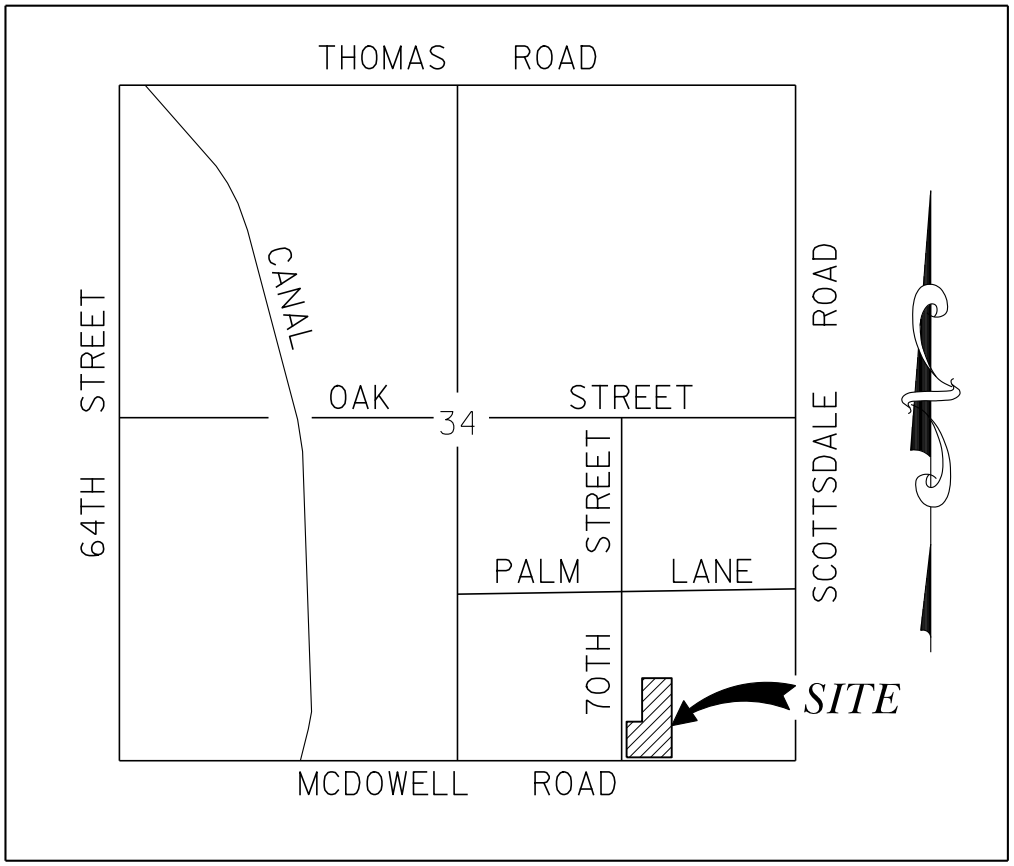
1. City of Scottsdale, *Design Standards and Policies Manual,* 2018.

2. Maricopa County, Drainage Policies and Standards, August 2018.

3. Maricopa County Flood Control District Flo-2d maps from the Lower Indian bend Wash Study

APPENDIX A

Vicinity Map



VICINITY MAP

N.T.S.

APPENDIX B

FEMA FIRM Map

MAP SCALE 1" = 1000'

500 0 1000 2000 FEET

METE

PANEL 2235L

NFIP

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

MARICOPA COUNTY, ARIZONA

AND INCORPORATED AREAS

PANEL 2235 OF 4425

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
MARICOPA COUNTY	040037	2235	L
MESA, CITY OF	040046	2235	L
SCOTTSDALE, CITY OF	045012	2235	L
TEMPE, CITY OF	040054	2235	L

Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER
04013C2235L

MAP REVISED
OCTOBER 16, 2013

Federal Emergency Management Agency

APPENDIX C

Warning and Disclaimer of Liability

GRADING & DRAINAGE LANGUAGE

WARNING AND DISCLAIMER OF LIABILITY

The City's Stormwater and Floodplain Management Ordinance is intended to minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding.

The Stormwater and Floodplain Management Ordinance identifies floodplains, floodways, flood fringes and special flood hazard areas. However, a property outside these areas could be inundated by floods. Also, much of the city is a dynamic flood area; floodways, floodplains, flood fringes and special flood hazard areas may shift from one location to another, over time, due to natural processes.

WARNING AND DISCLAIMER OF LIABILITY

The flood protection provided by the Stormwater and Floodplain Management Ordinance is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by constructed or natural causes. The Stormwater and Floodplain Management Ordinance does not create liability on the part of the city, any officer or employee thereof, or the federal, state or county government for any flood damages that result from reliance on the Ordinance or any administrative decision lawfully made thereunder.

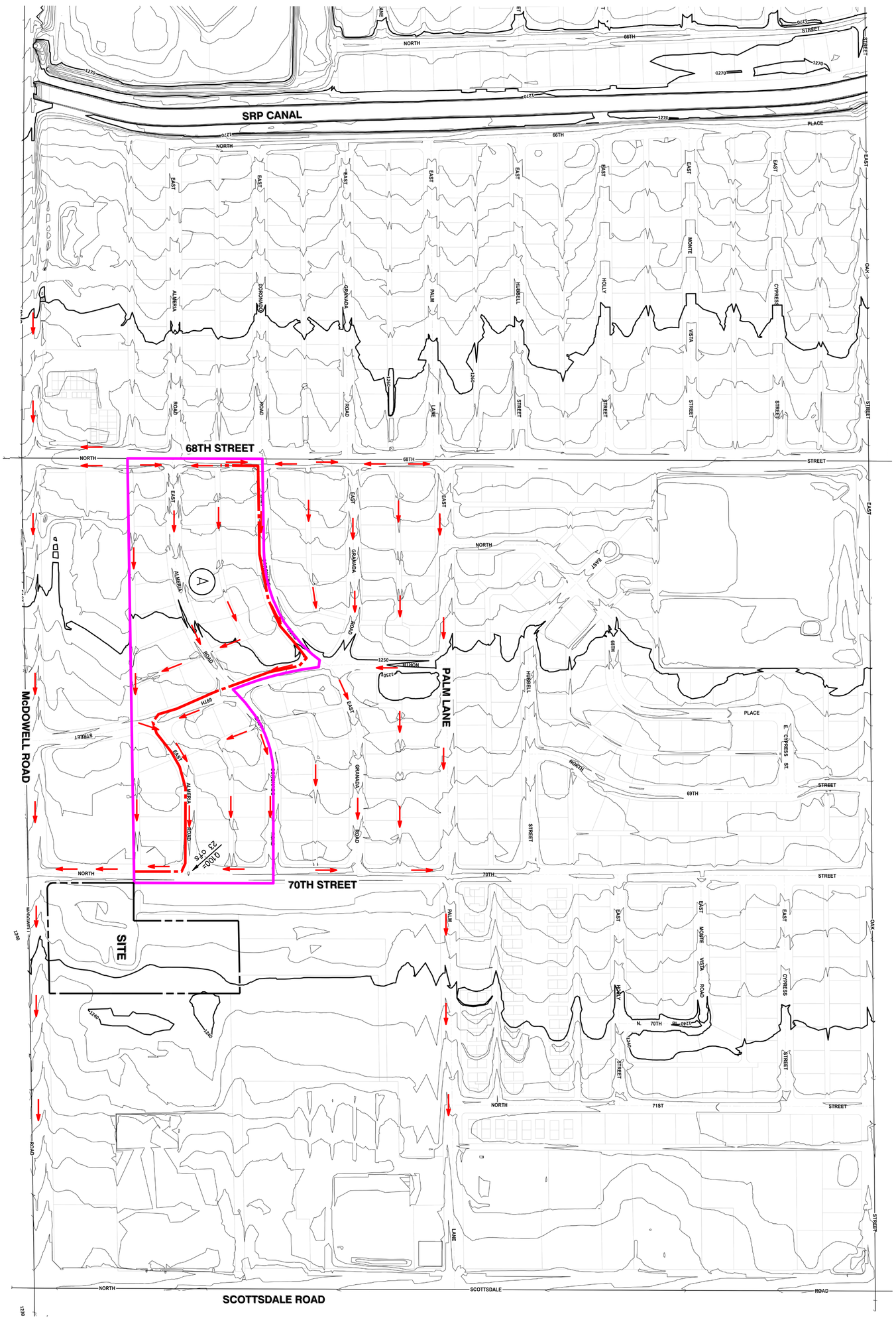
Compliance with the Stormwater and Floodplain Management Ordinance does not ensure complete protection from flooding. Flood-related problems such as natural erosion, streambed meander, or constructed obstructions and diversions may occur and have an adverse effect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations.

I have read and understand the above.

Plan Check # _____ Owner _____ Date _____

APPENDIX D

Watershed Delineation and Topography Map



SCALE: 1" = 150'

0' 75' 150' 300'

SHEET NO.	WS01 1 of 1
PROJECT NO.	1872
DATE	06/25/20

3 ENGINEERING, LLC
8777 E. ZURAS ROAD
SCOTTSDALE, ARIZONA 85251
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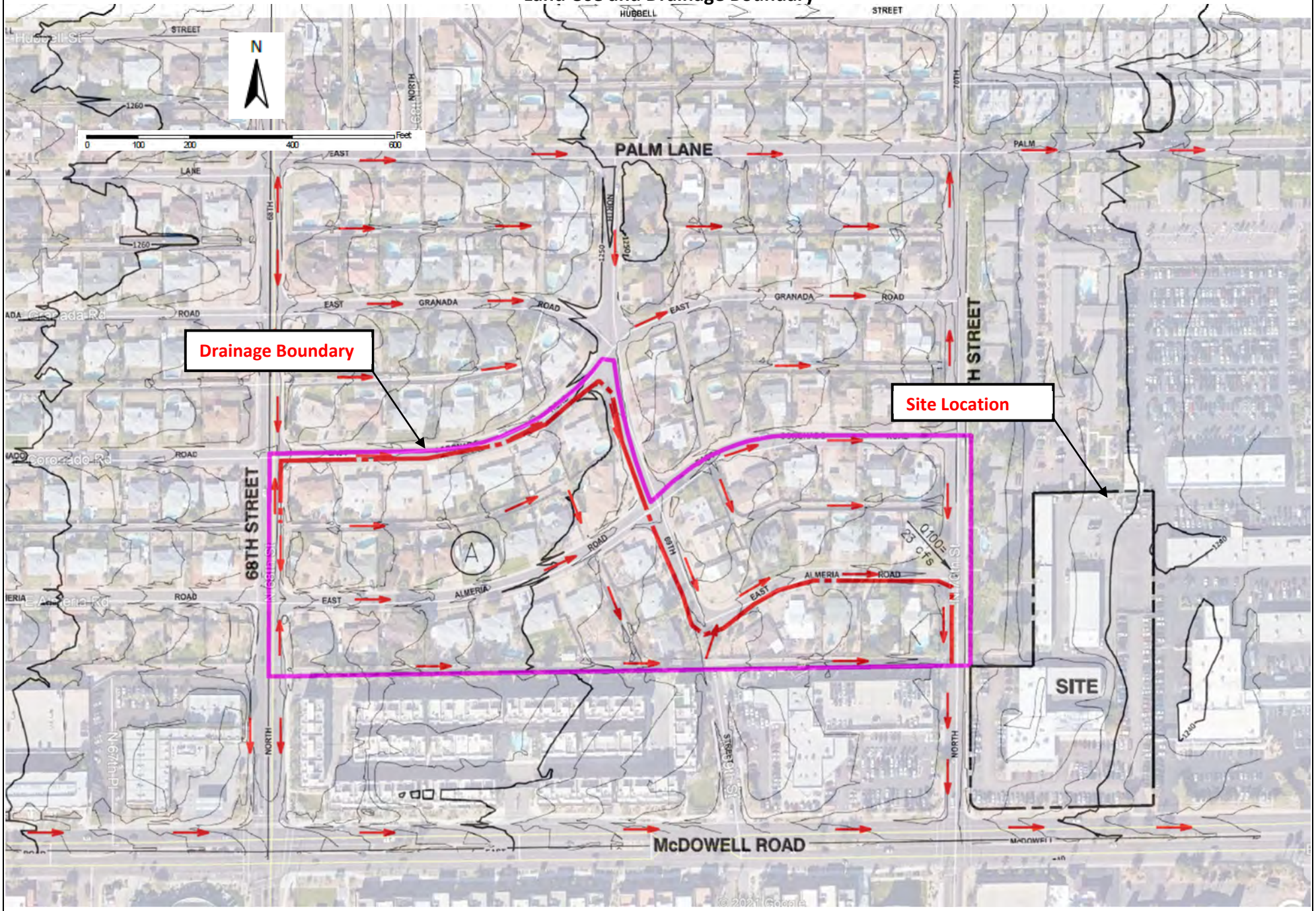
SOUTHDALE

7000 E. MCDOWELL ROAD, SCOTTSDALE, AZ 85257

WATERSHED DELINEATION AND TOPOGRAPHY MAP

EXHIBIT: Watershed Map

Land Use and Drainage Boundary



APPENDIX E

Offsite Drainage Calculations

EXHIBIT: SOILS MAP

Source: Web Soil Survey

111° 55' 6" W

111° 55' 48" W

N

Map Scale: 1:2,240 if printed on A landscape (11" x 8.5") sheet.

Meters: 0 30 60 120 180

Feet: 0 100 200 400 600

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 12N WGS84

Tables — Hydrologic Soil Group — Summary By Map Unit

Summary by Map Unit — Eastern Maricopa and Northern Pinal Counties Area, Arizona (AZ655)

Summary by Map Unit — Eastern Maricopa and Northern Pinal Counties Area, Arizona (AZ655)

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
LaA	Laveen loam, 0 to 1 percent slopes	B	1.9	13.9%
RiA	Rillito gravelly loam, 0 to 1 percent slopes	B	11.6	86.1%
Totals for Area of Interest			**13.4**	**100.0%**

Map Scale: 1:2,240 if printed on A landscape (11" x 8.5") sheet.

N

Meters: 0 30 60 120 180

Feet: 0 100 200 400 600

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 12N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features

- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Eastern Maricopa and Northern Pinal Counties Area, Arizona
Survey Area Data: Version 12, Sep 16, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Apr 1, 2018—Jun 1, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
LaA	Laveen loam, 0 to 1 percent slopes	1.9	14.2%
RiA	Rillito gravelly loam, 0 to 1 percent slopes	11.7	85.8%
Totals for Area of Interest		**13.7**	**100.0%**

SOUTHDALE

7000 E. MCDOWELL ROAD, SCOTTSDALE, AZ 85257

IMPERVIOUS AREA EXHIBIT

LEGEND

DRAINAGE BOUNDARY

IMPERVIOUS AREA

DRAINAGE AREA = 594,860 SF

IMPERVIOUS AREA = 295,882 SF

IMPERVIOUS AREA PERCENTAGE = 49.74% (50% USED IN HEC-1)

SCALE: 1"= 100'

0' 50' 100' 200'

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DATE: 06/28/21

PROJECT NO.
1872

EXHIBIT: DDMSW HEC-1

Flood Control District of Maricopa County
Drainage Design Management System
SOILS
Project Reference: 70TH&MCDOWEL

Page 1

Area ID	Book Number	Map Unit	Soil ID	Area (sq mi)	Area (%)	XKSAT	Rock Percent (%)	Effective Rock (%)	Comments
Major Basin ID: 01									
A	655	LaA	655422120	0.003	13.80	0.250	-	5	
	655	RiA	655543720	0.018	86.20	0.400	-	100	

Flood Control District of Maricopa County
Drainage Design Management System
LAND USE
Project Reference: 70TH&MCDOWEL

Page 1

Sub Basin	Land Use Code	Area (sq mi)	Area (%)	Initial Loss (IA)	Percent Impervious (RTIMP)	Vegetation Cover (%)	DTHETA	Kb	Description
Major Basin ID: 01									
A	160	13.4000	100.0	0.25	50 *	50.0	NORMAL	0.033	
		13.4000	**100.0**						

Flood Control District of Maricopa County
Drainage Design Management System
SUB BASINS
Project Reference: 70TH&MCDOWEL

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Area ID	Sub Basin Parameters: Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	Rainfall Losses: IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)		Return Period Parameters: 2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin ID: 01																		
A	0.021	0.25	48.0	48.0	URBAN	0.033	0.25	0.25	4.08	0.540	50	**Tc (Hrs)**	0.379	0.327	0.300	0.272*	0.257*	0.243 *
												Vel (f/s)	0.97	1.12	1.22	1.35	1.43	1.51
												R (Hrs)	0.376	0.320	0.290	0.261	0.244	0.229

Flood Control District of Maricopa County
Drainage Design Management System
HEC-1 FLOW SUMMARY
Project Reference: 70TH&MCDOWEL

Page 1 6/28/2021

ID	Type	Area (sq mi)	Discharge cfs: 2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Major Basin 01								
A	Hydrograph	0.02		12	17	23	28	34

EXHIBIT: HEC-1 Output 100Yr

```
              SCHEMATIC DIAGRAM OF STREAM NETWORK
INPUT
 LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

  NO.      (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

   19           A

 (***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1****************************************                    ****************************************
*                                      *                    *                                      *
*   FLOOD HYDROGRAPH PACKAGE  (HEC-1)  *                    *    U.S. ARMY CORPS OF ENGINEERS      *
*               JUN   1998             *                    *    HYDROLOGIC ENGINEERING CENTER     *
*            VERSION 4.1               *                    *          609 SECOND STREET           *
*                                      *                    *       DAVIS, CALIFORNIA 95616        *
*  RUN DATE   28JUN21  TIME  07:45:00  *                    *          (916) 756-1104              *
*                                      *                    *                                      *
****************************************                    ****************************************



                            Flood Control District of Maricopa County
                            70TH&MCDOWEL - 3 Engineers
                            100 YEAR
                            6 Hour  Storm
                            Unit Hydrograph: Clark
                            Storm: Multiple
                            06/28/2021

   9 IO          OUTPUT CONTROL VARIABLES
                       IPRNT          5  PRINT CONTROL
                       IPLOT          0  PLOT CONTROL
                       QSCAL         0.  HYDROGRAPH PLOT SCALE

     IT          HYDROGRAPH TIME DATA
                        NMIN          5  MINUTES IN COMPUTATION INTERVAL
                       IDATE     1JAN99  STARTING DATE
                       ITIME       0000  STARTING TIME
                          NQ       2000  NUMBER OF HYDROGRAPH ORDINATES
                       NDDATE    7JAN99  ENDING DATE
                       NDTIME       2235  ENDING TIME
                       ICENT          19  CENTURY MARK

                 COMPUTATION INTERVAL     .08 HOURS
                      TOTAL TIME BASE  166.58 HOURS

          ENGLISH UNITS
               DRAINAGE AREA         SQUARE MILES
               PRECIPITATION DEPTH   INCHES
               LENGTH, ELEVATION     FEET
               FLOW                  CUBIC FEET PER SECOND
               STORAGE VOLUME        ACRE-FEET
               SURFACE AREA          ACRES
               TEMPERATURE           DEGREES FAHRENHEIT

  11 JD          INDEX STORM NO. 1
                       STRM       2.52  PRECIPITATION DEPTH
                       TRDA        .00  TRANSPOSITION DRAINAGE AREA

  12 PI            PRECIPITATION PATTERN
                     .00      .00      .00      .00      .00      .00      .00      .00      .00      .00
                     .00      .00      .00      .00      .00      .00      .00      .00      .00      .00
                     .00      .00      .00      .00      .00      .00      .00      .00      .00      .00
                     .00      .00      .00      .01      .01      .01      .01      .01      .01      .03
                     .03      .03      .05      .05      .05      .15      .15      .15      .03      .03
                     .03      .01      .01      .01      .01      .01      .01      .00      .00      .00
                     .00      .00      .00      .00      .00      .00      .00      .00      .00      .00
                     .00      .00
```

EXHIBIT: HEC-1 Output 100Yr

```
   15 JD          INDEX STORM NO. 2
                        STRM        2.50  PRECIPITATION DEPTH
                        TRDA         .50  TRANSPOSITION DRAINAGE AREA

   16 PI            PRECIPITATION PATTERN
                       .00       .00       .00       .00       .00       .00       .00       .00       .00       .00
                       .00       .00       .00       .00       .00       .00       .00       .00       .00       .00
                       .00       .00       .00       .00       .00       .00       .00       .00       .00       .00
                       .00       .00       .00       .01       .01       .01       .01       .01       .01       .03
                       .03       .03       .05       .05       .05       .15       .15       .15       .03       .03
                       .03       .01       .01       .01       .01       .01       .01       .00       .00       .00
                       .00       .00       .00       .00       .00       .00       .00       .00       .00       .00
                       .00       .00
1
                                                    RUNOFF SUMMARY
                                                FLOW IN CUBIC FEET PER SECOND
                                              TIME IN HOURS,  AREA IN SQUARE MILES

                                          PEAK    TIME OF     AVERAGE FLOW FOR MAXIMUM PERIOD      BASIN     MAXIMUM     TIME OF
         OPERATION       STATION          FLOW     PEAK                                           AREA      STAGE     MAX STAGE
+                                                                 6-HOUR     24-HOUR     72-HOUR

         HYDROGRAPH AT
+                                A          34.     4.08           4.          1.          0.        .02


 *** NORMAL END OF HEC-1 ***
```

Cross Section for 70th Street West Section A

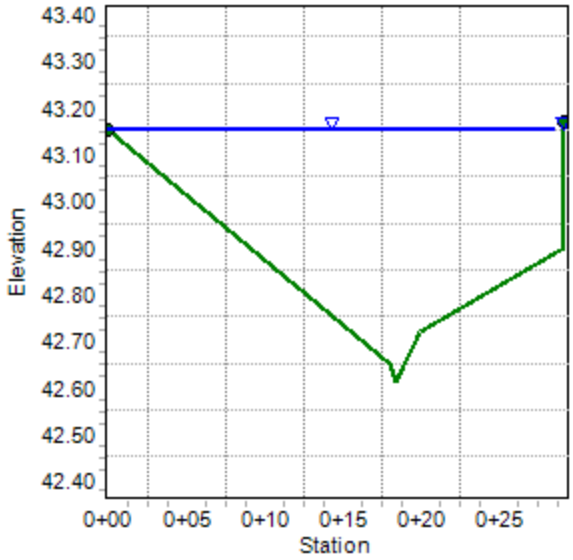
Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.25	%
Normal Depth	0.54	ft
Discharge	21.53	ft³/s

Cross Section Image



Worksheet for 70th Street West Section A

Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.25	%
Normal Depth	0.54	ft

Section Definitions

Station (ft)	Elevation (ft)
0+29	43.17
0+29	42.90
0+20	42.72
0+18	42.61
0+18	42.65
0+00	43.15

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+29, 43.17)	(0+00, 43.15)	0.013

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Discharge	21.53	ft³/s
Elevation Range	42.61 to 43.17 ft	
Flow Area	8.55	ft²
Wetted Perimeter	29.26	ft
Hydraulic Radius	0.29	ft
Top Width	29.00	ft
Normal Depth	0.54	ft

Worksheet for 70th Street West Section A

Results

Critical Depth		0.50	ft
Critical Slope		0.00386	ft/ft
Velocity		2.52	ft/s
Velocity Head		0.10	ft
Specific Energy		0.64	ft
Froude Number		0.82	
Flow Type	Subcritical		

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.54	ft
Critical Depth	0.50	ft
Channel Slope	0.25	%
Critical Slope	0.00386	ft/ft

Cross Section for 70th Street East Section B

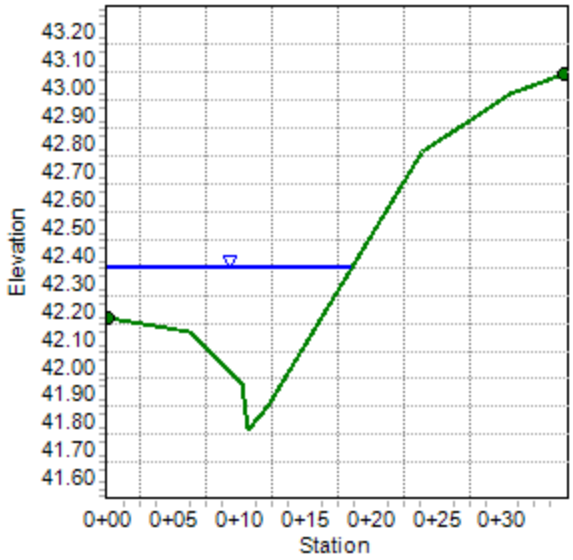
Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.25	%
Normal Depth	0.58	ft
Discharge	12.47	ft³/s

Cross Section Image



Worksheet for 70th Street East Section B

Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.25	%
Discharge	12.47	ft³/s

Section Definitions

Station (ft)	Elevation (ft)
0+00	42.17
0+06	42.12
0+10	41.93
0+11	41.77
0+12	41.85
0+24	42.76
0+31	42.97
0+35	43.04

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 42.17)	(0+35, 43.04)	0.013

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Normal Depth	0.58	ft
Elevation Range	41.77 to 43.04 ft	
Flow Area	5.17	ft²
Wetted Perimeter	18.81	ft
Hydraulic Radius	0.27	ft

Worksheet for 70th Street East Section B

Results

Top Width		18.58	ft
Normal Depth		0.58	ft
Critical Depth		0.54	ft
Critical Slope		0.00399	ft/ft
Velocity		2.41	ft/s
Velocity Head		0.09	ft
Specific Energy		0.67	ft
Froude Number		0.81	
Flow Type	Subcritical		

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.58	ft
Critical Depth	0.54	ft
Channel Slope	0.25	%
Critical Slope	0.00399	ft/ft

Cross Section for Section B Weir

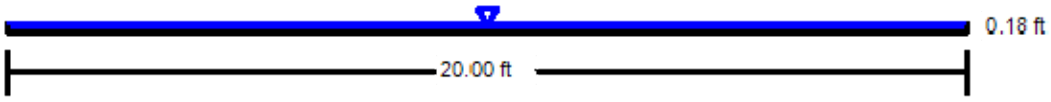
Project Description

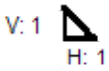
Solve For	Discharge

Input Data

Discharge	4.58	ft³/s
Headwater Elevation	0.18	ft
Crest Elevation	0.00	ft
Tailwater Elevation	0.00	ft
Weir Coefficient	3.00	US
Crest Length	20.00	ft
Number Of Contractions	0	

Cross Section Image





Worksheet for Section B Weir

Project Description

Solve For	Discharge

Input Data

Headwater Elevation		0.18	ft
Crest Elevation		0.00	ft
Tailwater Elevation		0.00	ft
Weir Coefficient		3.00	US
Crest Length		20.00	ft
Number Of Contractions	0		

Results

Discharge	4.58	ft^3/s
Headwater Height Above Crest	0.18	ft
Tailwater Height Above Crest	0.00	ft
Flow Area	3.60	ft^2
Velocity	1.27	ft/s
Wetted Perimeter	20.36	ft
Top Width	20.00	ft

Cross Section for 70th Street Section C

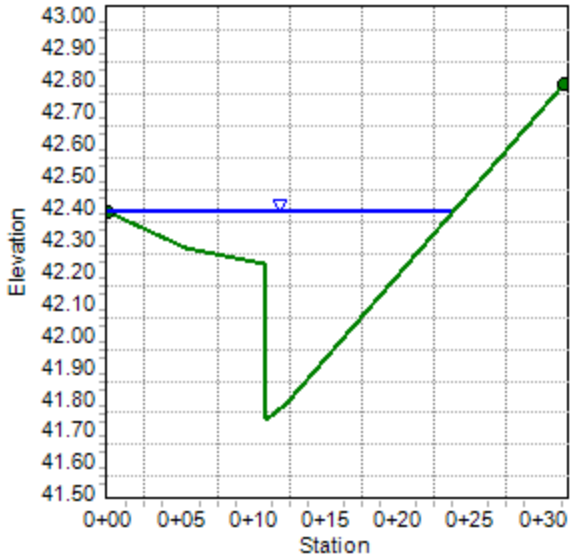
Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.25	%
Normal Depth	0.65	ft
Discharge	11.34	ft³/s

Cross Section Image



Worksheet for 70th Street Section C

Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.25	%
Normal Depth	0.65	ft

Section Definitions

Station (ft)	Elevation (ft)
0+00	42.38
0+05	42.27
0+11	42.22
0+11	41.73
0+12	41.78
0+31	42.78

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 42.38)	(0+31, 42.78)	0.013

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Discharge	11.34	ft³/s
Elevation Range	41.73 to 42.78 ft	
Flow Area	5.40	ft²
Wetted Perimeter	24.27	ft
Hydraulic Radius	0.22	ft
Top Width	23.76	ft
Normal Depth	0.65	ft

Worksheet for 70th Street Section C

Results

Critical Depth		0.60	ft
Critical Slope		0.00427	ft/ft
Velocity		2.10	ft/s
Velocity Head		0.07	ft
Specific Energy		0.72	ft
Froude Number		0.78	
Flow Type	Subcritical		

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.65	ft
Critical Depth	0.60	ft
Channel Slope	0.25	%
Critical Slope	0.00427	ft/ft

APPENDIX F

Aerial Site Photographs

1"=80'

SOUTHDALE
7000 E. MCDOWELL ROAD, SCOTTSDALE, AZ 85257

AERIAL SITE PHOTOGRAPHS

3engineering
planning civil engineering surveying

3 ENGINEERING, LLC
6370 E. THOMAS ROAD, SUITE # 200- PHOENIX, ARIZONA 85251
PHONE: (602) 334-4387 - FAX: (602) 490-3230
WWW.3ENGINEERING.COM

DATE:	PROJECT NO.
04/09/20	1872

APPENDIX G

Offsite Aerial Photographs

1"=600'

SOUTHDALE

7000 E. MCDOWELL ROAD, SCOTTSDALE, AZ 85257

AERIAL SITE PHOTOGRAPHS

3engineering
planning civil engineering surveying

3 ENGINEERING, LLC
6370 E. THOMAS ROAD, SUITE # 200- PHOENIX, ARIZONA 85251
PHONE: (602) 334-4387 - FAX: (602) 490-3230
WWW.3ENGINEERING.COM

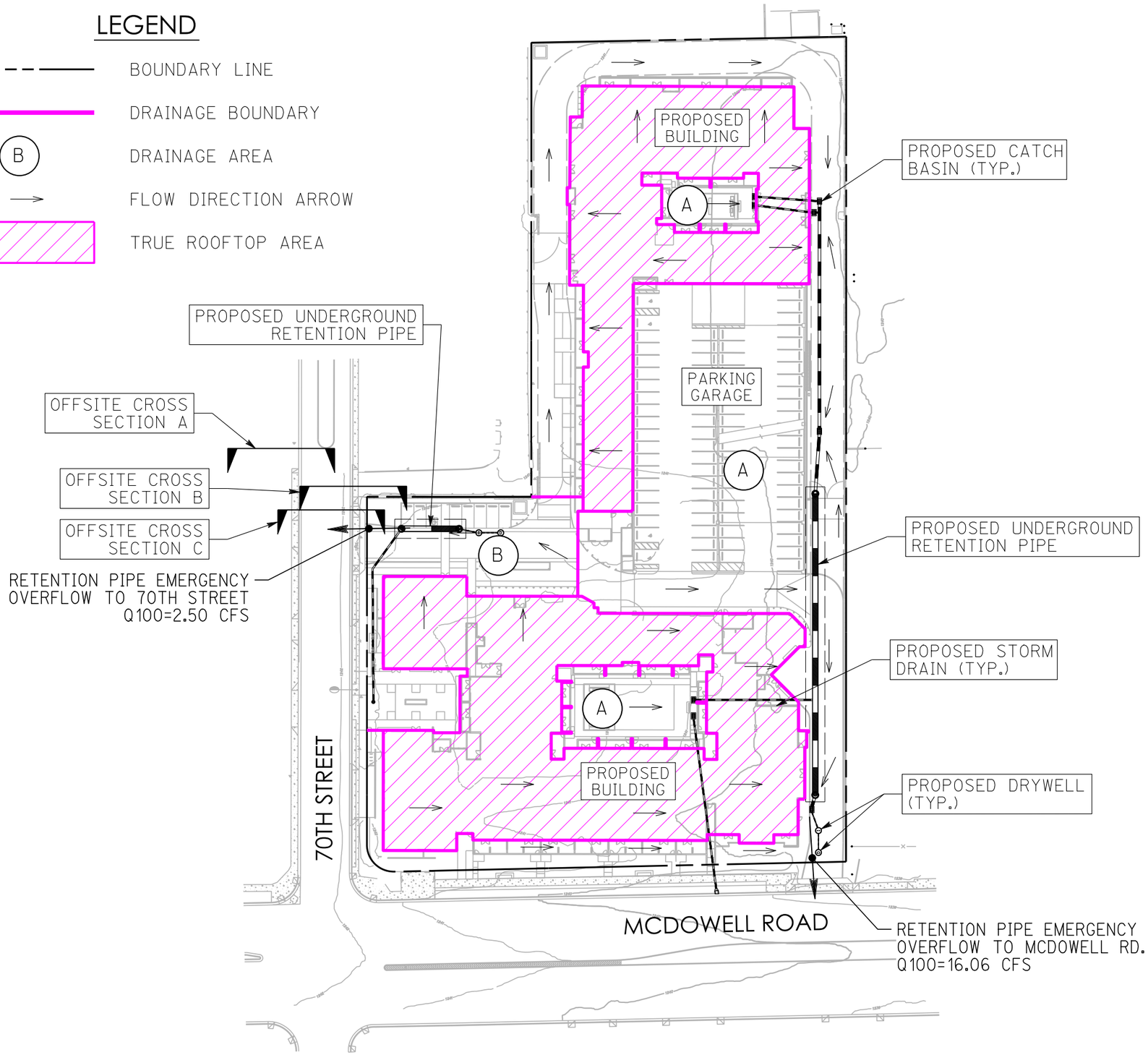
DATE:	PROJECT NO.
04/09/20	1872

APPENDIX H

Onsite Drainage Area Map

LEGEND

- BOUNDARY LINE
- DRAINAGE BOUNDARY
- (B) DRAINAGE AREA
- FLOW DIRECTION ARROW
- TRUE ROOFTOP AREA



SCALE: 1"= 80'

0' 40' 80' 160'

SOUTHDALE

7000 E. MCDOWELL ROAD, SCOTTSDALE, AZ 85257

DRAINAGE AREA MAP

3engineering
civil engineering
planning
surveying

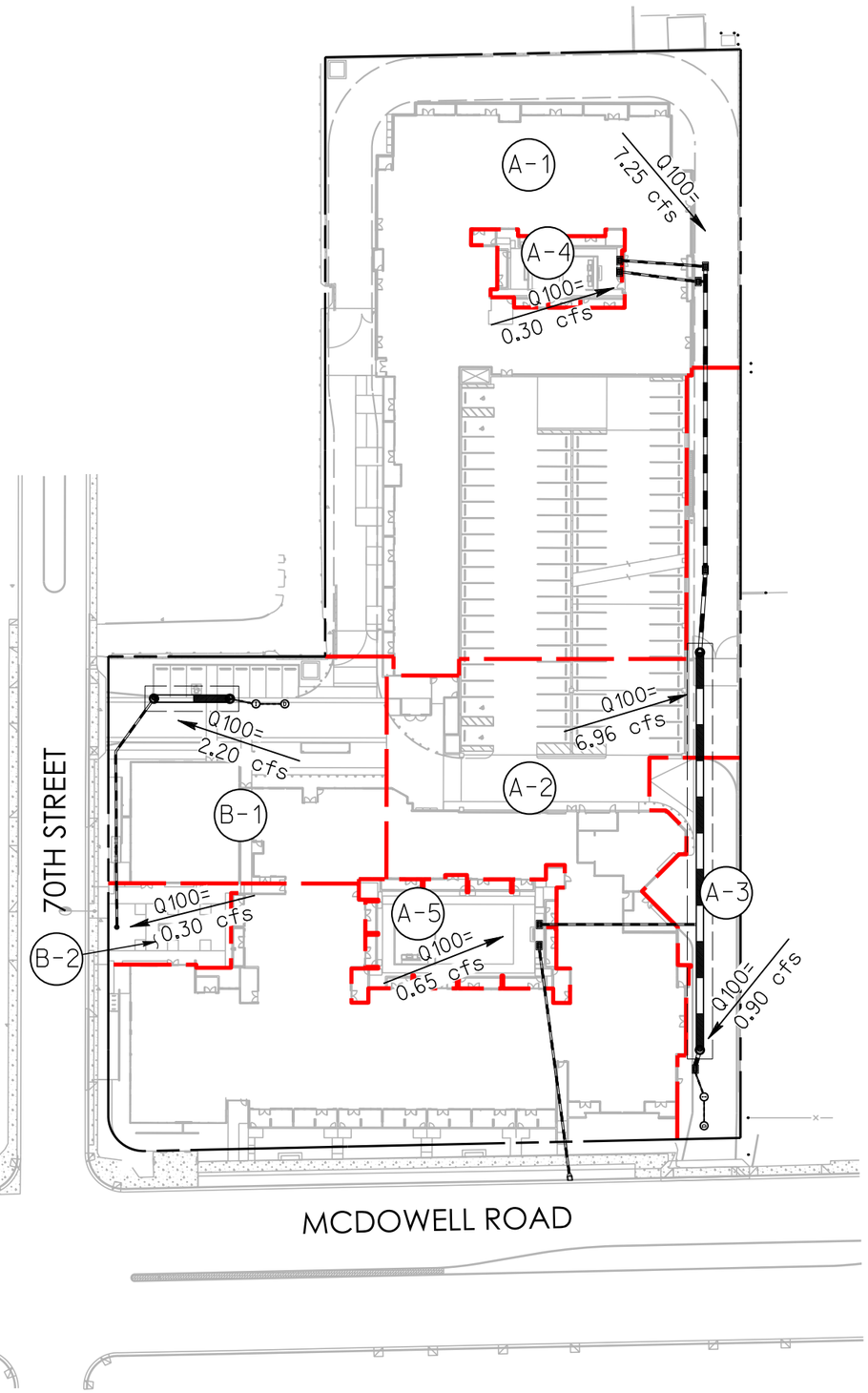
3 ENGINEERING, LLC
6370 E. THOMAS ROAD, SUITE 200 - SCOTTSDALE, ARIZONA 85251
PHONE: (602) 334-4787 - FAX: (602) 496-5280
WWW.3ENGINEERING.COM

DATE: 04/23/21

PROJECT NO. 1872

APPENDIX I

Inlet Area Exhibit



LEGEND

- BOUNDARY LINE
- DRAINAGE BOUNDARY
- DRAINAGE SUB-BOUNDARY
- (B-1) DRAINAGE INLET SUB-AREA

SCALE: 1"= 80'

0' 40' 80' 160'

SOUTHDALE

7000 E. MCDOWELL ROAD, SCOTTSDALE, AZ 85257

INLET AREA EXHIBIT

3engineering
planning civil engineering surveying

3 ENGINEERING, LLC
6370 E. THOMAS ROAD, SUITE # 200 - SCOTTSDALE, ARIZONA 85251
PHONE: (602) 314-3483 - FAX: (602) 490-0230
WWW.3ENGINEERING.COM

DATE: 04/23/21

PROJECT NO.
1872

APPENDIX J

Onsite Drainage Calculations

Post-Development Rational Method Calculations

Sub-Area	Area	C_{10}	C_{100}	Tc	i_{10}	i_{100}	Q_{10}	Q_{100}
	(acre)	(weighted)	(weighted)	(min)	(in/hr)	(in/hr)	(cfs)	(cfs)
A-1	1.60	0.76	0.81	10	3.53	5.6	4.29	7.25
A-2	1.41	0.83	0.88	10	3.53	5.6	4.14	6.96
A-3	0.18	0.84	0.89	10	3.53	5.6	0.53	0.90
A-4	0.07	0.76	0.81	10	3.53	5.6	0.18	0.30
A-5	0.16	0.70	0.75	10	3.53	5.6	0.39	0.65
B-1	0.44	0.84	0.89	10	3.53	5.6	1.31	2.20
B-2	0.07	0.69	0.74	10	3.53	5.6	0.17	0.30

On-Site Retention for the First Flush Storm

Sub-Area	Area	C_{100}	P		Total Vol. Req.	Surface Vol. Prov.	U.G. Retention Provided	Total Vol. Prov.
	(acre)		(in)		(cf)	(cf)	(cf)	(cf)
A	1.97	0.76	0.50		2,706	-	2,764	2,764
B	0.32	0.82	0.50		479	-	528	528

PRE V. POST RETENTION CALCULATIONS

C = 0.95 (IMPERVIOUS), C = 0.45 (PERVIOUS)
P = 2.14 IN
AREA OF LOT = 171,290 SF

PRE (EXISTING)

AREA OF IMPERVIOUS = 161,857 SF
AREA OF PERVIOUS = 9,433 SF

$$\text{WEIGHTED C} = \left(\frac{(161{,}857 \times 0.95) + (9{,}433 \times 0.45)}{171{,}290}\right)$$

WEIGHTED C = 0.92

POST (PROPOSED)

AREA OF IMPERVIOUS = 134,825 SF
AREA OF PERVIOUS = 36,465 SF

$$\text{WEIGHTED C} = \left(\frac{(134{,}825 \times 0.95) + (36{,}465 \times 0.45)}{171{,}290}\right)$$

WEIGHTED C = 0.84

$$(0.84-0.92)\left(\frac{2.14}{12}\right)(171{,}290) = \boxed{0 \text{ CF REQUIRED}}$$

Grated Inlet Capacity - Weir Condition

Inlet Type	Inlet	Q_{100}	Inlet Capacity w/ 40% Clogging	d	Cw	P
		(cfs)	(cfs)	(ft)		(ft)
M.A.G. type "F"	A-1	7.25	7.53	0.50	3	11.83
M.A.G. type "F"	A-2	6.96	7.53	0.50	3	11.83
M.A.G. type "F"	A-3	0.90	7.53	0.50	3	11.83
M.A.G. type "F"	A-4	0.30	2.66	0.25	3	11.83
M.A.G. type "F"	A-5	0.65	2.66	0.25	3	11.83
30" Grated Inlet	B-1	2.20	5.00	0.50	3	7.85
24" Grated Inlet	B-2	0.30	0.86	0.18	3	6.28

Q=Cw*P*d^1.5

Cw= 3.0 weir coefficient
Q = discharge capacity
P = inlet perimeter
d = flow depth

Weighted C Coefficient Calculations

	Overall Area (s.f.)	Building & Parking Area (s.f.)	Landscape Area (s.f.)	Overall Area (Ac.)	C_{10}	C_{100}
A	85633	52,841	32,793	1.97	0.71	0.76
B	14091	10,312	3,779	0.32	0.77	0.82
A-1	69664	50,135	19,529	1.60	0.76	0.81
A-2	61567	52,908	8,658	1.41	0.83	0.88
A-3	7881	6,866	1,015	0.18	0.84	0.89
A-4	2912	2,122	790	0.07	0.76	0.81
A-5	6806	4,059	2,748	0.16	0.70	0.75
B-1	19342	16,883	2,459	0.44	0.84	0.89
B-2	3117	1,797	1,320	0.07	0.69	0.74

C=((Building & Hardscape Area x 0.95) + (Landscape Area x 0.45)) / Overall Area

C_{100} Building & Hardscape = 0.95
C_{100} Landscape = 0.45

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Drywell Percolation Rates

Sub-Area	Total Basin Vol.	Dry-Up Time	# of Drywells Provided	Drywell Perc Rate Required
	(cf)	(hr)		(cf/s)
A	2,764	36.00	1	0.021
B	528	36.00	1	0.004

NOAA Atlas 14, Volume 1, Version 5
Location name: Scottsdale, Arizona, USA*
Latitude: 33.4662°, Longitude: -111.93°
Elevation: 1237.91 ft**
* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.180 (0.152-0.218)	0.235 (0.199-0.286)	0.321 (0.269-0.387)	0.386 (0.322-0.464)	0.475 (0.390-0.567)	0.544 (0.440-0.647)	0.613 (0.487-0.727)	0.684 (0.535-0.812)	0.780 (0.594-0.926)	0.853 (0.637-1.02)
10-min	0.274 (0.231-0.332)	0.359 (0.303-0.435)	0.488 (0.410-0.589)	0.588 (0.491-0.705)	0.722 (0.593-0.864)	0.827 (0.670-0.984)	0.933 (0.742-1.11)	1.04 (0.815-1.24)	1.19 (0.904-1.41)	1.30 (0.969-1.55)
15-min	0.340 (0.286-0.412)	0.444 (0.376-0.538)	0.606 (0.508-0.730)	0.729 (0.608-0.874)	0.895 (0.735-1.07)	1.02 (0.831-1.22)	1.16 (0.920-1.37)	1.29 (1.01-1.53)	1.47 (1.12-1.75)	1.61 (1.20-1.92)
30-min	0.457 (0.385-0.555)	0.598 (0.506-0.725)	0.815 (0.684-0.983)	0.982 (0.819-1.18)	1.21 (0.990-1.44)	1.38 (1.12-1.64)	1.56 (1.24-1.85)	1.74 (1.36-2.06)	1.98 (1.51-2.35)	2.17 (1.62-2.58)
60-min	0.566 (0.477-0.686)	0.740 (0.626-0.897)	1.01 (0.847-1.22)	1.22 (1.01-1.46)	1.49 (1.23-1.78)	1.71 (1.39-2.03)	1.93 (1.53-2.29)	2.15 (1.68-2.55)	2.45 (1.87-2.91)	2.68 (2.00-3.19)
2-hr	0.657 (0.563-0.781)	0.851 (0.728-1.01)	1.14 (0.974-1.35)	1.36 (1.15-1.61)	1.67 (1.39-1.95)	1.90 (1.56-2.22)	2.14 (1.73-2.50)	2.38 (1.89-2.78)	2.71 (2.10-3.17)	2.96 (2.25-3.49)
3-hr	0.712 (0.606-0.850)	0.912 (0.781-1.10)	1.20 (1.02-1.44)	1.43 (1.21-1.70)	1.75 (1.46-2.07)	2.00 (1.65-2.36)	2.27 (1.83-2.68)	2.55 (2.02-3.00)	2.94 (2.26-3.46)	3.25 (2.43-3.84)
6-hr	0.857 (0.745-1.00)	1.09 (0.949-1.27)	1.40 (1.21-1.63)	1.64 (1.42-1.91)	1.98 (1.68-2.29)	2.24 (1.88-2.58)	2.52 (2.07-2.90)	2.80 (2.26-3.23)	3.18 (2.51-3.68)	3.49 (2.68-4.05)
12-hr	0.961 (0.843-1.11)	1.22 (1.07-1.41)	1.54 (1.35-1.78)	1.80 (1.56-2.07)	2.15 (1.84-2.47)	2.41 (2.05-2.77)	2.69 (2.25-3.08)	2.96 (2.44-3.41)	3.34 (2.68-3.85)	3.63 (2.86-4.22)
24-hr	1.16 (1.04-1.30)	1.47 (1.32-1.65)	1.90 (1.70-2.13)	2.25 (2.00-2.51)	2.72 (2.41-3.04)	3.10 (2.73-3.45)	3.49 (3.05-3.89)	3.90 (3.38-4.34)	4.46 (3.82-4.97)	4.90 (4.16-5.47)
2-day	1.25 (1.12-1.40)	1.60 (1.44-1.79)	2.10 (1.88-2.35)	2.50 (2.23-2.79)	3.06 (2.71-3.41)	3.50 (3.08-3.91)	3.97 (3.48-4.44)	4.45 (3.87-4.98)	5.14 (4.41-5.76)	5.68 (4.83-6.40)
3-day	1.32 (1.19-1.48)	1.69 (1.52-1.90)	2.22 (1.99-2.49)	2.65 (2.37-2.96)	3.26 (2.89-3.63)	3.74 (3.30-4.17)	4.26 (3.73-4.75)	4.80 (4.17-5.36)	5.56 (4.77-6.21)	6.17 (5.24-6.92)
4-day	1.39 (1.25-1.56)	1.78 (1.60-2.00)	2.35 (2.10-2.62)	2.81 (2.50-3.13)	3.46 (3.07-3.85)	3.98 (3.51-4.43)	4.55 (3.98-5.06)	5.14 (4.46-5.73)	5.98 (5.12-6.67)	6.66 (5.65-7.45)
7-day	1.54 (1.38-1.73)	1.97 (1.77-2.20)	2.60 (2.32-2.90)	3.10 (2.77-3.47)	3.82 (3.39-4.27)	4.40 (3.88-4.91)	5.02 (4.39-5.60)	5.67 (4.92-6.33)	6.60 (5.65-7.36)	7.34 (6.22-8.21)
10-day	1.68 (1.50-1.87)	2.14 (1.92-2.40)	2.83 (2.53-3.15)	3.38 (3.01-3.76)	4.14 (3.68-4.61)	4.76 (4.20-5.30)	5.42 (4.75-6.03)	6.11 (5.31-6.80)	7.07 (6.07-7.88)	7.85 (6.67-8.76)
20-day	2.06 (1.85-2.29)	2.65 (2.38-2.94)	3.49 (3.13-3.88)	4.13 (3.70-4.58)	4.99 (4.45-5.54)	5.65 (5.02-6.27)	6.33 (5.59-7.03)	7.01 (6.16-7.79)	7.93 (6.91-8.84)	8.64 (7.47-9.64)
30-day	2.40 (2.15-2.67)	3.09 (2.78-3.44)	4.07 (3.65-4.51)	4.82 (4.31-5.33)	5.82 (5.18-6.44)	6.59 (5.84-7.28)	7.37 (6.51-8.15)	8.17 (7.18-9.04)	9.25 (8.07-10.2)	10.1 (8.71-11.2)
45-day	2.79 (2.51-3.10)	3.59 (3.24-3.99)	4.73 (4.26-5.25)	5.57 (5.00-6.18)	6.68 (5.98-7.41)	7.52 (6.70-8.33)	8.36 (7.42-9.27)	9.20 (8.13-10.2)	10.3 (9.04-11.5)	11.1 (9.72-12.4)
60-day	3.09 (2.79-3.42)	3.99 (3.60-4.42)	5.25 (4.73-5.81)	6.16 (5.54-6.81)	7.35 (6.59-8.13)	8.23 (7.35-9.11)	9.11 (8.11-10.1)	9.97 (8.84-11.0)	11.1 (9.78-12.3)	11.9 (10.5-13.3)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

Back to Top

NOAA Atlas 14, Volume 1, Version 5
Location name: Scottsdale, Arizona, USA*
Latitude: 33.4662°, Longitude: -111.93°
Elevation: 1237.91 ft**
* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

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NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	**2.16** (1.82-2.62)	**2.82** (2.39-3.43)	**3.85** (3.23-4.64)	**4.63** (3.86-5.57)	**5.70** (4.68-6.80)	**6.53** (5.28-7.76)	**7.36** (5.84-8.72)	**8.21** (6.42-9.74)	**9.36** (7.13-11.1)	**10.2** (7.64-12.2)
10-min	**1.64** (1.39-1.99)	**2.15** (1.82-2.61)	**2.93** (2.46-3.53)	**3.53** (2.95-4.23)	**4.33** (3.56-5.18)	**4.96** (4.02-5.90)	**5.60** (4.45-6.64)	**6.25** (4.89-7.41)	**7.12** (5.42-8.46)	**7.79** (5.81-9.28)
15-min	**1.36** (1.14-1.65)	**1.78** (1.50-2.15)	**2.42** (2.03-2.92)	**2.92** (2.43-3.50)	**3.58** (2.94-4.28)	**4.10** (3.32-4.88)	**4.63** (3.68-5.49)	**5.16** (4.04-6.12)	**5.88** (4.48-6.99)	**6.44** (4.80-7.66)
30-min	**0.914** (0.770-1.11)	**1.20** (1.01-1.45)	**1.63** (1.37-1.97)	**1.96** (1.64-2.36)	**2.41** (1.98-2.88)	**2.76** (2.24-3.29)	**3.11** (2.48-3.70)	**3.48** (2.72-4.12)	**3.96** (3.02-4.71)	**4.34** (3.24-5.16)
60-min	**0.566** (0.477-0.686)	**0.740** (0.626-0.897)	**1.01** (0.847-1.22)	**1.22** (1.01-1.46)	**1.49** (1.23-1.78)	**1.71** (1.39-2.03)	**1.93** (1.53-2.29)	**2.15** (1.68-2.55)	**2.45** (1.87-2.91)	**2.68** (2.00-3.19)
2-hr	**0.328** (0.282-0.390)	**0.426** (0.364-0.506)	**0.571** (0.487-0.676)	**0.682** (0.575-0.804)	**0.832** (0.694-0.977)	**0.949** (0.780-1.11)	**1.07** (0.866-1.25)	**1.19** (0.947-1.39)	**1.35** (1.05-1.59)	**1.48** (1.12-1.75)
3-hr	**0.237** (0.202-0.283)	**0.304** (0.260-0.365)	**0.400** (0.340-0.478)	**0.476** (0.402-0.566)	**0.582** (0.485-0.689)	**0.667** (0.548-0.787)	**0.757** (0.610-0.892)	**0.849** (0.673-0.999)	**0.978** (0.751-1.15)	**1.08** (0.811-1.28)
6-hr	**0.143** (0.124-0.168)	**0.181** (0.158-0.213)	**0.233** (0.203-0.272)	**0.274** (0.236-0.319)	**0.330** (0.281-0.382)	**0.374** (0.313-0.431)	**0.420** (0.346-0.484)	**0.467** (0.377-0.539)	**0.532** (0.419-0.615)	**0.582** (0.448-0.676)
12-hr	**0.080** (0.070-0.092)	**0.101** (0.089-0.117)	**0.128** (0.112-0.148)	**0.150** (0.130-0.172)	**0.178** (0.153-0.205)	**0.200** (0.170-0.229)	**0.223** (0.186-0.256)	**0.246** (0.203-0.283)	**0.277** (0.223-0.320)	**0.301** (0.237-0.350)
24-hr	**0.048** (0.043-0.054)	**0.061** (0.055-0.069)	**0.079** (0.071-0.089)	**0.094** (0.083-0.105)	**0.113** (0.100-0.127)	**0.129** (0.114-0.144)	**0.145** (0.127-0.162)	**0.162** (0.141-0.181)	**0.186** (0.159-0.207)	**0.204** (0.173-0.228)
2-day	**0.026** (0.023-0.029)	**0.033** (0.030-0.037)	**0.044** (0.039-0.049)	**0.052** (0.046-0.058)	**0.064** (0.056-0.071)	**0.073** (0.064-0.081)	**0.083** (0.072-0.092)	**0.093** (0.081-0.104)	**0.107** (0.092-0.120)	**0.118** (0.101-0.133)
3-day	**0.018** (0.016-0.021)	**0.023** (0.021-0.026)	**0.031** (0.028-0.035)	**0.037** (0.033-0.041)	**0.045** (0.040-0.050)	**0.052** (0.046-0.058)	**0.059** (0.052-0.066)	**0.067** (0.058-0.074)	**0.077** (0.066-0.086)	**0.086** (0.073-0.096)
4-day	**0.015** (0.013-0.016)	**0.019** (0.017-0.021)	**0.024** (0.022-0.027)	**0.029** (0.026-0.033)	**0.036** (0.032-0.040)	**0.041** (0.037-0.046)	**0.047** (0.041-0.053)	**0.054** (0.046-0.060)	**0.062** (0.053-0.069)	**0.069** (0.059-0.078)
7-day	**0.009** (0.008-0.010)	**0.012** (0.011-0.013)	**0.015** (0.014-0.017)	**0.018** (0.016-0.021)	**0.023** (0.020-0.025)	**0.026** (0.023-0.029)	**0.030** (0.026-0.033)	**0.034** (0.029-0.038)	**0.039** (0.034-0.044)	**0.044** (0.037-0.049)
10-day	**0.007** (0.006-0.008)	**0.009** (0.008-0.010)	**0.012** (0.011-0.013)	**0.014** (0.013-0.016)	**0.017** (0.015-0.019)	**0.020** (0.018-0.022)	**0.023** (0.020-0.025)	**0.025** (0.022-0.028)	**0.029** (0.025-0.033)	**0.033** (0.028-0.036)
20-day	**0.004** (0.004-0.005)	**0.006** (0.005-0.006)	**0.007** (0.007-0.008)	**0.009** (0.008-0.010)	**0.010** (0.009-0.012)	**0.012** (0.010-0.013)	**0.013** (0.012-0.015)	**0.015** (0.013-0.016)	**0.017** (0.014-0.018)	**0.018** (0.016-0.020)
30-day	**0.003** (0.003-0.004)	**0.004** (0.004-0.005)	**0.006** (0.005-0.006)	**0.007** (0.006-0.007)	**0.008** (0.007-0.009)	**0.009** (0.008-0.010)	**0.010** (0.009-0.011)	**0.011** (0.010-0.013)	**0.013** (0.011-0.014)	**0.014** (0.012-0.016)
45-day	**0.003** (0.002-0.003)	**0.003** (0.003-0.004)	**0.004** (0.004-0.005)	**0.005** (0.005-0.006)	**0.006** (0.006-0.007)	**0.007** (0.006-0.008)	**0.008** (0.007-0.009)	**0.009** (0.008-0.009)	**0.010** (0.008-0.011)	**0.010** (0.009-0.012)
60-day	**0.002** (0.002-0.002)	**0.003** (0.002-0.003)	**0.004** (0.003-0.004)	**0.004** (0.004-0.005)	**0.005** (0.005-0.006)	**0.006** (0.005-0.006)	**0.006** (0.006-0.007)	**0.007** (0.006-0.008)	**0.008** (0.007-0.009)	**0.008** (0.007-0.009)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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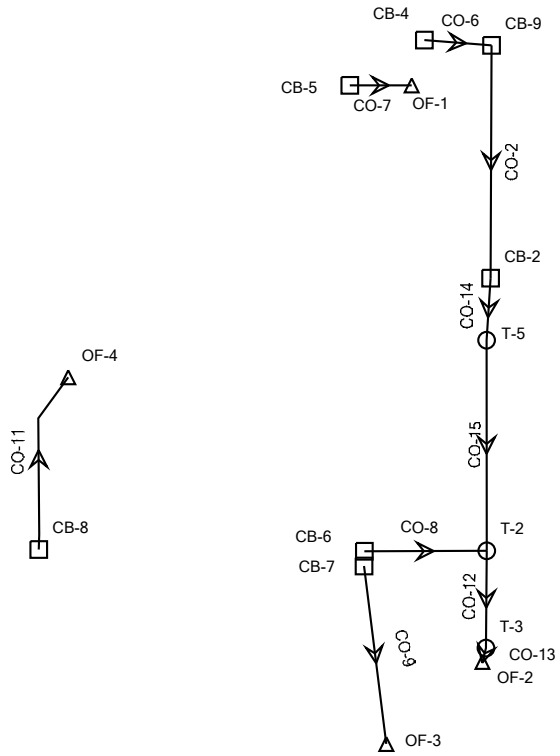
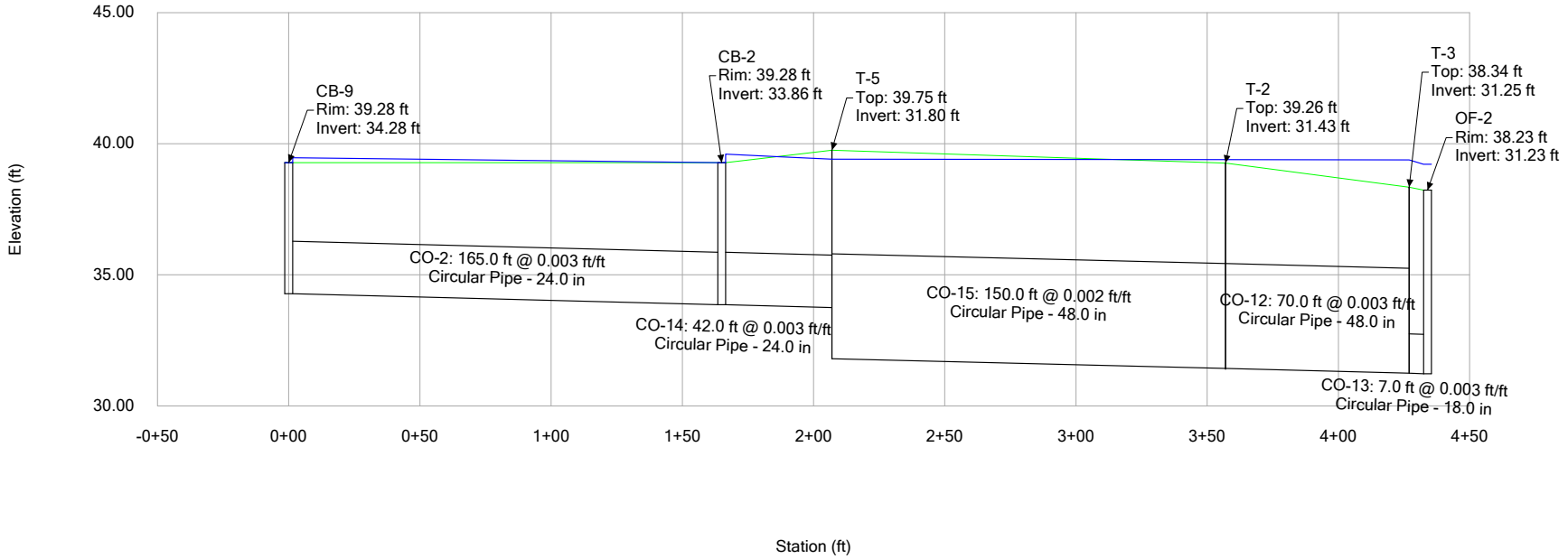
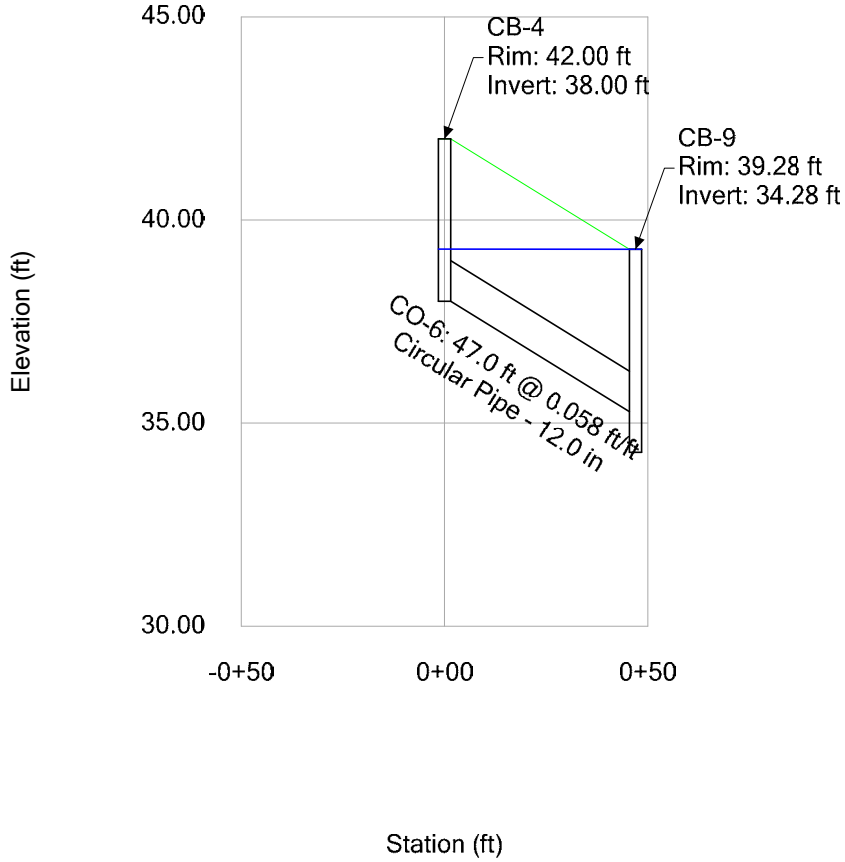
Scenario: Base



Profile Report
Engineering Profile - East Retention Pipe (1872.stc)



Profile Report
Engineering Profile - North Courtyard Inlet (1872.stc)



Profile Report
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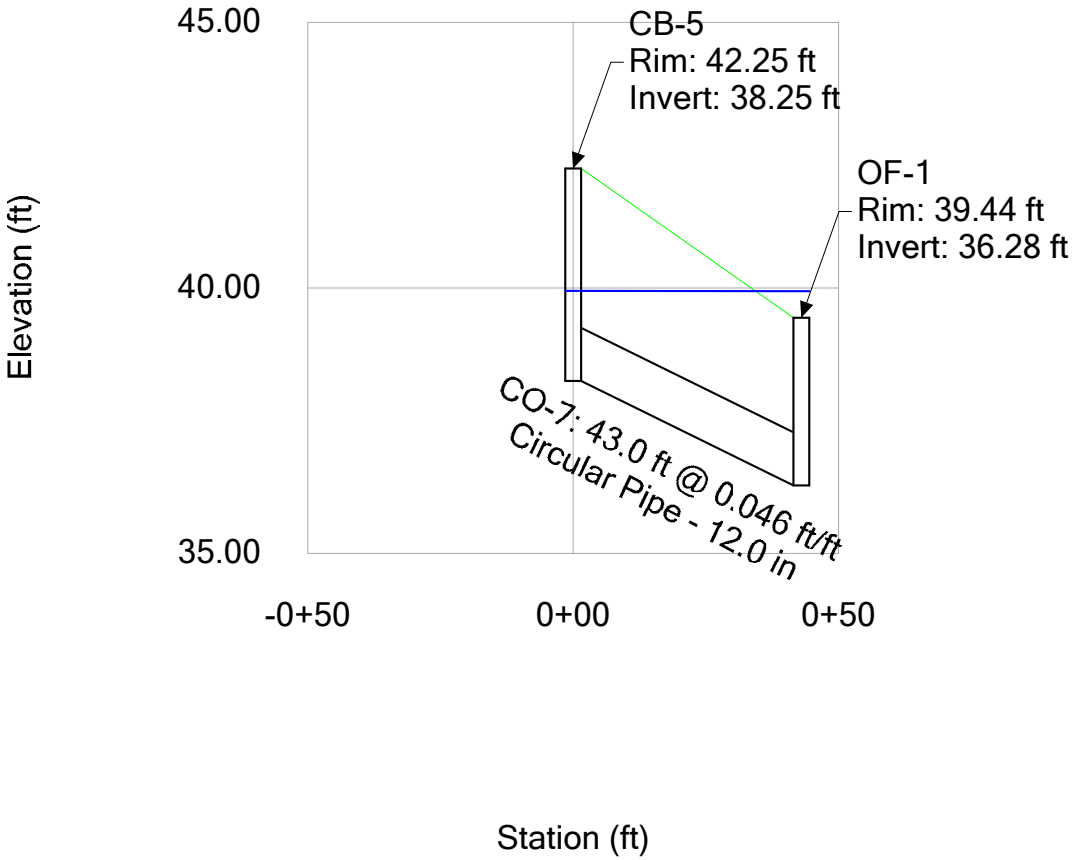
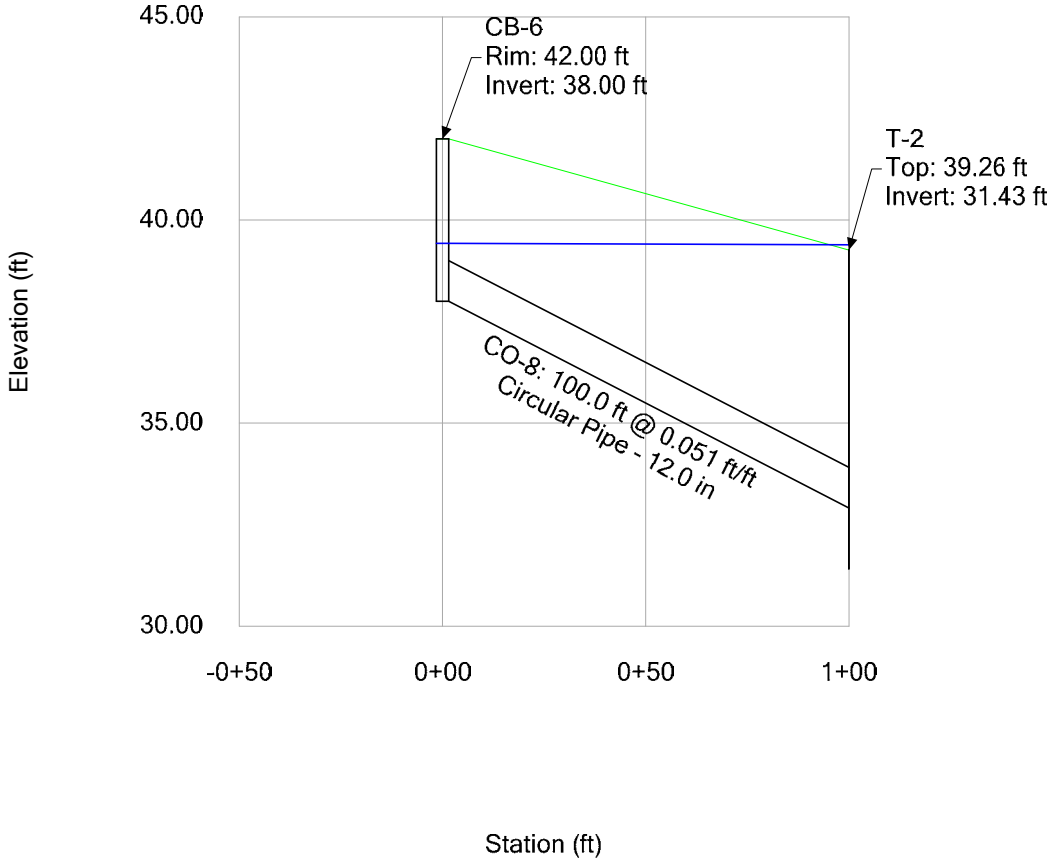


Profile Report
Engineering Profile - South Courtyard Inlet (1872.stc)



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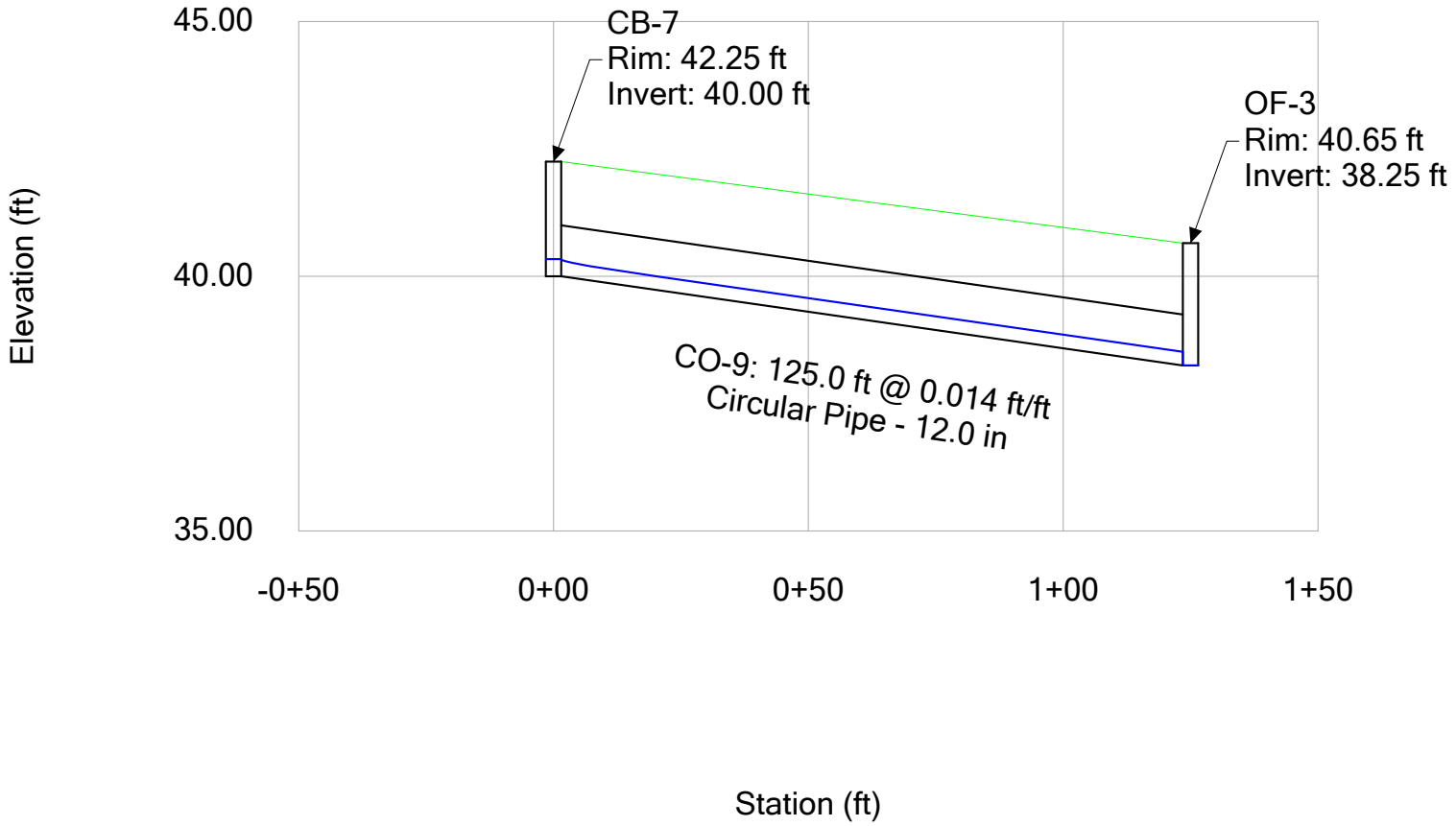
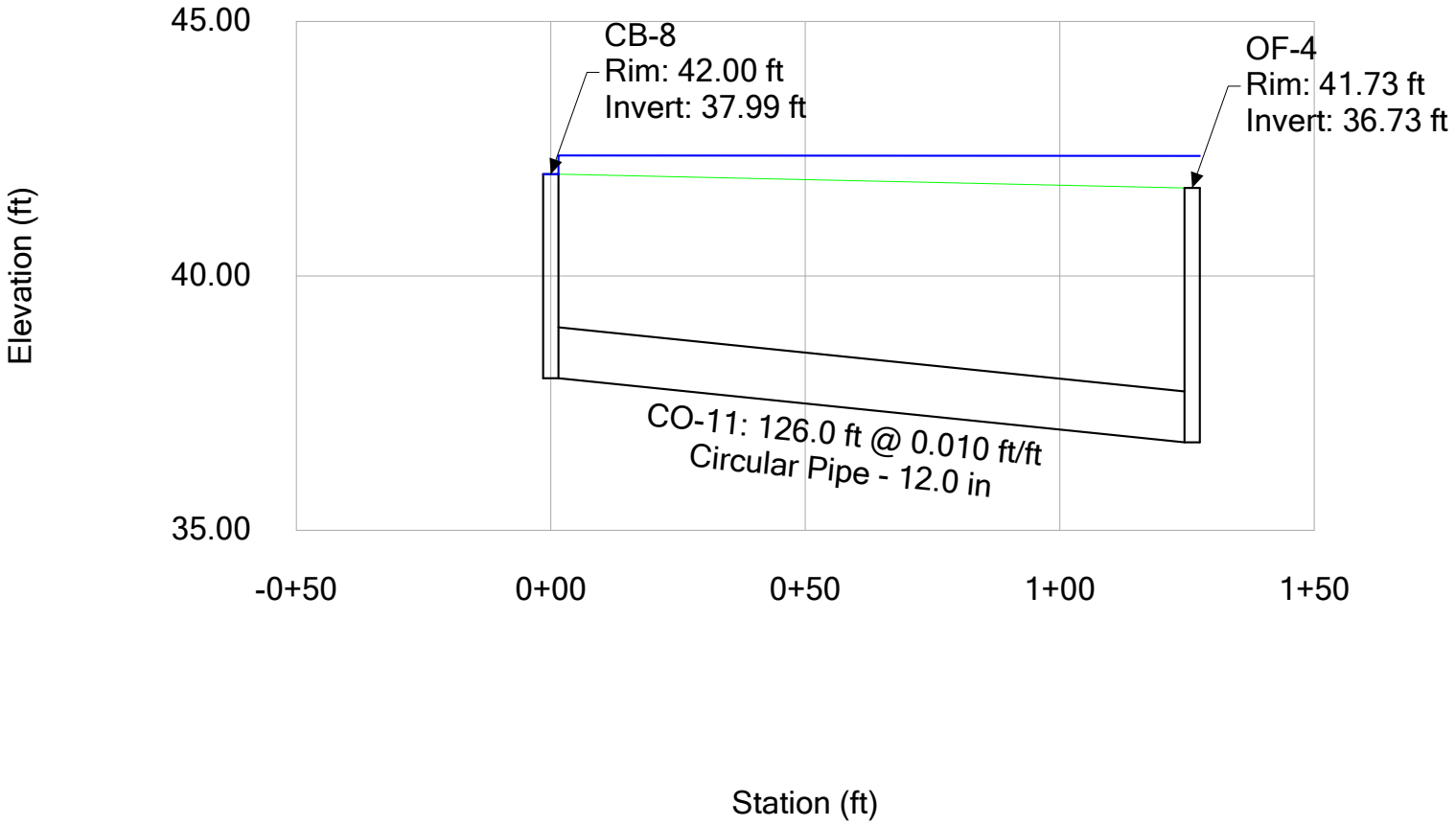
Engineering Profile - South Courtyard Overflow Pipe (1872.stc)



Profile Report

Engineering Profile - West Retention Pipe (1872.stc)



Conduit FlexTable: Combined Pipe/Node Report (1872.stc)

Label	Start Node	Stop Node	Length (Unified) (ft)	Total Flow (ft^3/s)	Rise (Unified) (in)	Capacity (Full Flow) (ft^3/s)	Velocity (Average) (ft/s)	Invert (Upstream) (ft)	Invert (Downstream) (ft)	Slope (ft/ft)	Manning's n	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
CO-9	CB-7	OF-3	125.0	0.65	12.0	4.22	3.89	40.00	38.25	0.014	0.013	40.34	38.52
CO-11	CB-8	OF-4	126.0	0.30	12.0	3.56	0.38	37.99	36.73	0.010	0.013	42.37	42.36
CO-7	CB-5	OF-1	43.0	0.37	12.0	7.63	0.47	38.25	36.28	0.046	0.013	39.94	39.94
CO-8	CB-6	T-2	100.0	0.65	12.0	8.04	0.83	38.00	32.91	0.051	0.013	39.43	39.39
CO-6	CB-4	CB-9	47.0	0.30	12.0	8.57	0.38	38.00	35.28	0.058	0.013	39.28	39.28
CO-2	CB-9	CB-2	165.0	7.55	24.0	11.41	2.40	34.28	33.86	0.003	0.013	39.46	39.28
CO-14	CB-2	T-5	42.0	15.41	24.0	11.58	4.91	33.86	33.75	0.003	0.013	39.60	39.41
CO-15	T-5	T-2	150.0	15.41	48.0	71.58	1.23	31.80	31.43	0.002	0.013	39.41	39.39
CO-12	T-2	T-3	70.0	16.06	48.0	72.84	1.28	31.43	31.25	0.003	0.013	39.39	39.38
CO-13	T-3	OF-2	7.0	16.06	18.0	5.61	9.09	31.25	31.23	0.003	0.013	39.38	39.22

FlexTable: Catch Basin Table (1872.stc)

Label	Station (Calculated) (ft)	Elevation (Ground) (ft)	Set Rim to Ground Elevation?	Elevation (Rim) (ft)	Elevation (Invert) (ft)	Flow (Additional) (ft³/s)	Carryover Additional Flow (ft³/s)	Flow (Known) (ft³/s)	Inlet Type	Maximum Inflow (ft³/s)	Capture Efficiency (%)
CB-2	2+69	39.28	True	39.28	33.86	7.86	0.00	0.00	Percent Capture	0.00	100.0
CB-4	4+81	42.00	True	42.00	38.00	0.30	0.00	0.00	Percent Capture	0.00	100.0
CB-5	0+43	42.25	True	42.25	38.25	0.37	0.00	0.00	Percent Capture	0.00	100.0
CB-6	1+77	42.00	True	42.00	38.00	0.65	0.00	0.00	Percent Capture	0.00	100.0
CB-7	1+25	42.25	True	42.25	40.00	0.65	0.00	0.00	Percent Capture	0.00	100.0
CB-8	1+26	42.00	True	42.00	37.99	0.30	0.00	0.00	Percent Capture	0.00	100.0
CB-9	4+34	39.28	True	39.28	34.28	7.25	0.00	0.00	Percent Capture	0.00	100.0

Inlet Location	Desired Sump Depth (ft)	Depth (In) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	System Fixed Flow (ft³/s)
On Grade	0.00	5.42	5.42	39.28	39.28	15.41
On Grade	0.00	1.28	1.28	39.28	39.28	0.30
On Grade	0.00	1.69	1.69	39.94	39.94	0.37
On Grade	0.00	1.43	1.43	39.43	39.43	0.65
On Grade	0.00	0.34	0.34	40.34	40.34	0.65
On Grade	0.00	4.01	4.01	42.00	42.00	0.30
On Grade	0.00	5.00	5.00	39.28	39.28	7.55

FlexTable: Outfall Table (1872.stc)

Label	Station (ft)	Elevation (Ground) (ft)	Set Rim to Ground Elevation	Elevation (Invert) (ft)	Boundary Condition Type	Elevation (Tailwater) (ft)	Flow (Outfall) (ft^3/s)
OF-1	0+00	39.44	True	36.28	User Defined Tailwater	39.94	0.37
OF-2	0+00	38.23	True	31.23	User Defined Tailwater	39.22	16.06
OF-3	0+00	40.65	True	38.25	Free Outfall	0.00	0.65
OF-4	0+00	41.73	True	36.73	User Defined Tailwater	42.36	0.30

FlexTable: Transition Table (1872.stc)

Label	Elevation (Ground) (ft)	Elevation (Top) (ft)	Elevation (Invert) (ft)	Transition Length (ft)	Headloss Method	Hydraulic Grade Line (Out) (ft)	System Fixed Flow (ft^3/s)
T-2	39.26	39.26	31.43	0.0	Absolute	39.39	16.06
T-3	38.34	38.34	31.25	0.0	Absolute	39.38	16.06
T-5	39.75	39.75	31.80	0.0	Absolute	39.41	15.41

APPENDIX K

Flo-2d Maps

679_LIBW - South 100YR6HR

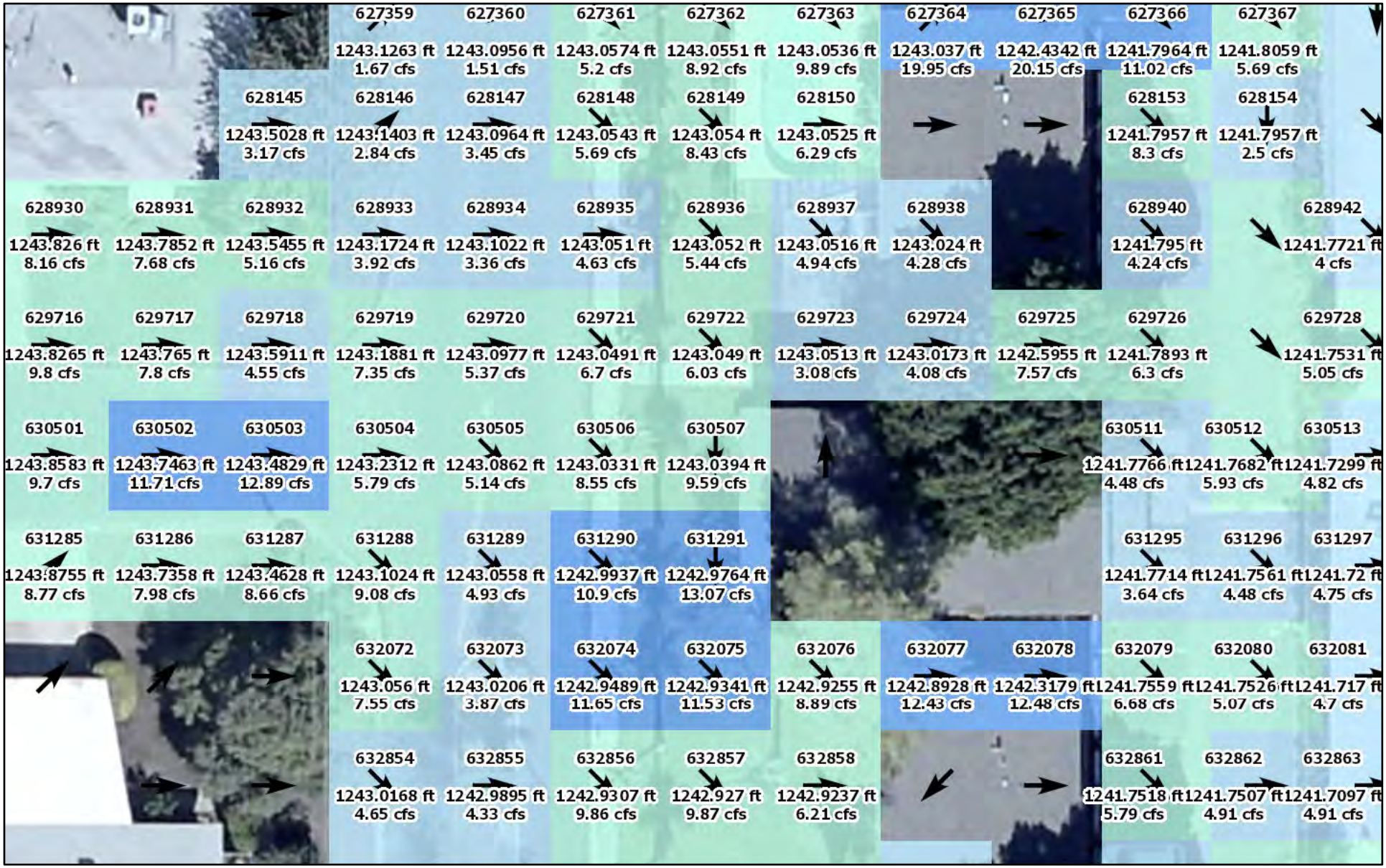
March 24, 2020

1:300

0 12.5 25 50 ft

0 4.25 8.5 17 m

679_LIBW - South 100YR6HR



March 24, 2020

1:300

0 12.5 25 50 ft

0 4.25 8.5 17 m

679_LIBW - South 100YR6HR

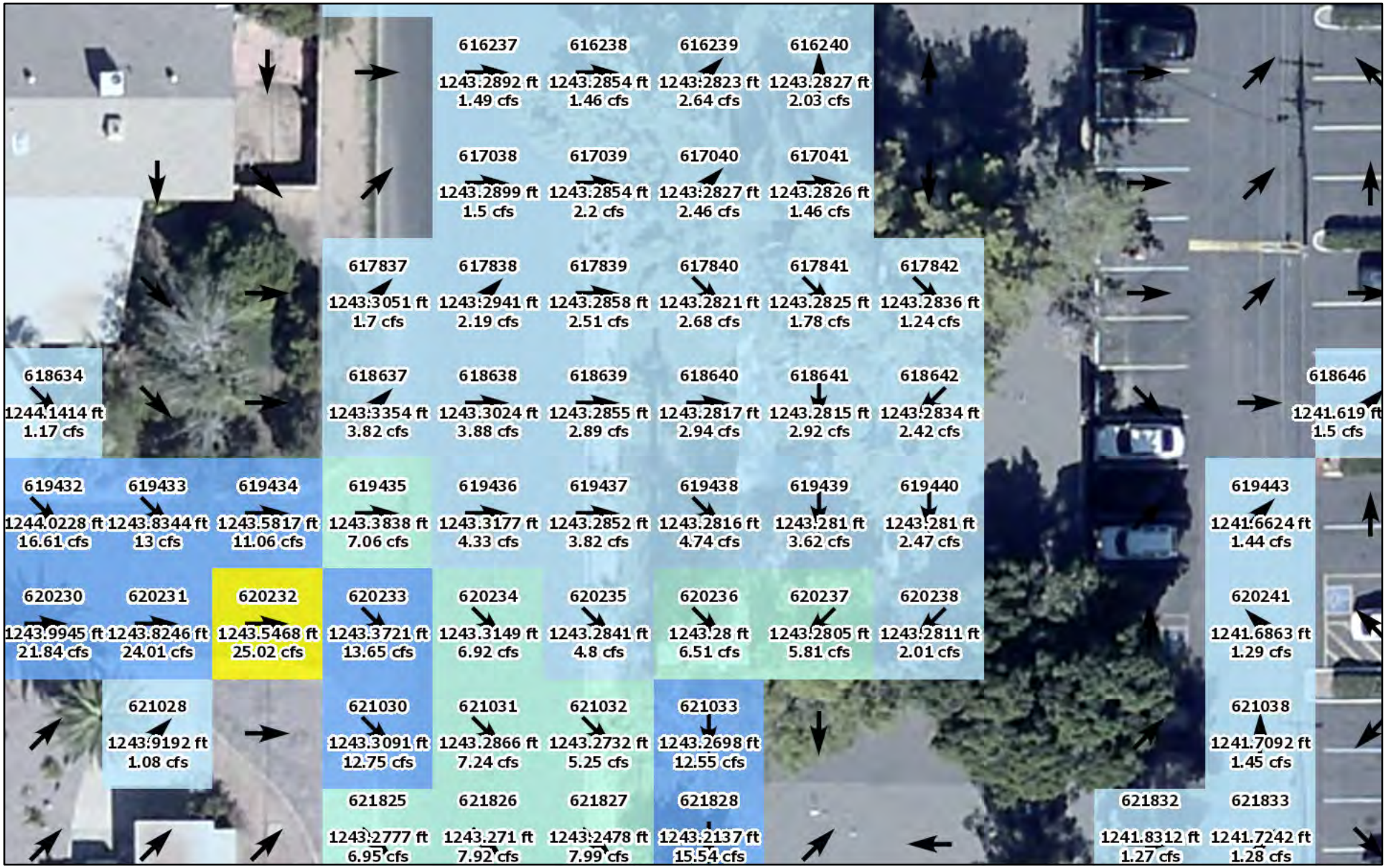
March 24, 2020

1:300

0 12.5 25 50 ft

0 4.25 8.5 17 m

679_LIBW - South 100YR6HR



March 24, 2020

1:300

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0 4.25 8.5 17 m

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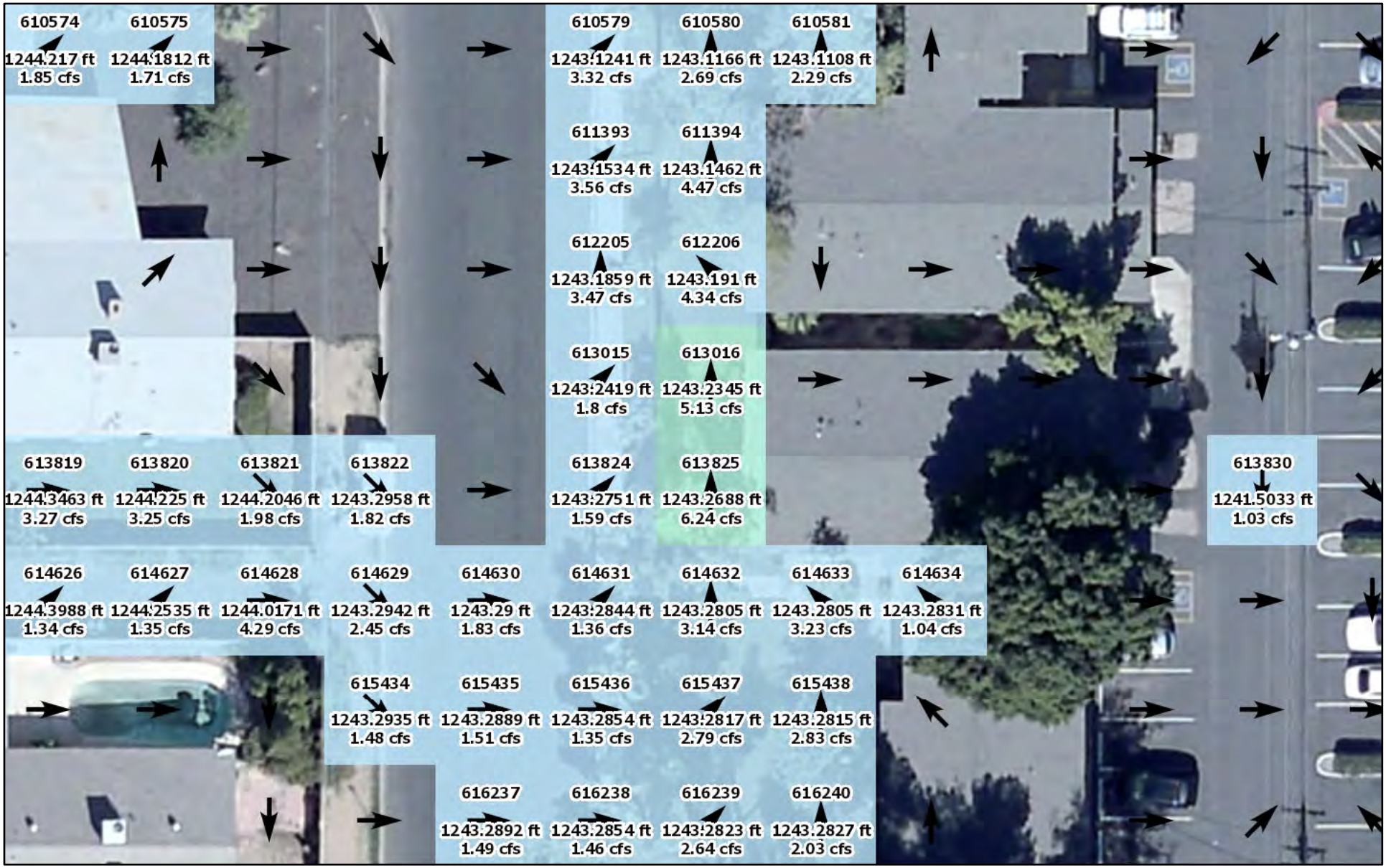


March 24, 2020

1:300

0 12.5 25 50 ft

0 4.25 8.5 17 m

APPENDIX L

Preliminary Grading and Drainage Plans

PRELIMINARY GRADING & DRAINAGE PLAN FOR

SOUTHDALE

7000 E. MCDOWELL ROAD, SCOTTSDALE, ARIZONA 85257

LOCATED IN A PORTION OF THE SOUTHEAST QUARTER OF SECTION 34, TOWNSHIP 2 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER MERIDIAN, MARICOPA COUNTY, ARIZONA

LEGAL DESCRIPTION:

THAT PORTION OF THE SOUTHEAST QUARTER OF THE SOUTHEAST QUARTER OF SECTION 34, TOWNSHIP 2 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER BASE AND MERIDIAN, MARICOPA COUNTY, ARIZONA, MORE PARTICULARLY DESCRIBED AS FOLLOWS:

BEGINNING AT A POINT ON THE NORTH LINE OF THE SOUTH HALF OF SAID SOUTHEAST QUARTER OF THE SOUTHEAST QUARTER, A DISTANCE OF 380.00 FEET EAST OF THE WEST LINE OF SAID SOUTHEAST QUARTER OF THE SOUTHEAST QUARTER;

THENCE WEST ALONG SAID NORTH LINE TO A POINT ON THE EAST LINE OF A NORTH-SOUTH ALLEY AS SHOWN ON THE PLAT OF WONDERLAND, ACCORDING TO BOOK 100 OF MAPS, PAGE 19, RECORDS OF MARICOPA COUNTY, ARIZONA;

THENCE SOUTH ALONG THE EAST LINE OF AFORESAID ALLEY TO AN ANGLE POINT THEREON;

THENCE WEST ALONG THE SOUTH LINE OF AN EAST-WEST ALLEY AS SHOWN ON AFORESAID PLAT OF WONDERLAND, TO A POINT ON THE EAST LINE OF 70TH STREET AS SHOWN ON AFORESAID PLAT;

THENCE SOUTH ALONG SAID EAST LINE, A DISTANCE OF 253.88 FEET TO THE BEGINNING OF A CURVE TO THE LEFT HAVING A CENTRAL ANGLE OF 91 DEGREES 14 MINUTES 49 SECONDS AND A TANGENT OF 20.00 FEET;

THENCE SOUTHEASTERLY ALONG SAID CURVE TO THE LEFT, AN ARC DISTANCE OF 31.17 FEET;

THENCE SOUTH PARALLEL WITH THE WEST LINE OF THE SOUTHEAST QUARTER OF THE SOUTHEAST QUARTER OF SAID SECTION 34, A DISTANCE OF 65.00 FEET TO A POINT ON THE SOUTH LINE OF THE SOUTHEAST QUARTER OF THE SOUTHEAST QUARTER OF SAID SECTION 34, FROM WHICH THE SOUTHWEST CORNER THEREOF BEARS WEST, A DISTANCE OF 52.42 FEET;
THENCE EAST ALONG THE SOUTH LINE OF THE SOUTHEAST QUARTER OF THE SOUTHEAST QUARTER OF SAID SECTION 34, TO A POINT 380.00 FEET EAST OF THE SOUTHWEST CORNER OF SAID SOUTHEAST QUARTER OF THE SOUTHEAST QUARTER;

THENCE NORTH ALONG A LINE PARALLEL TO AND 380.00 FEET EAST OF THE WEST LINE OF SAID SOUTHEAST QUARTER OF THE SOUTHEAST QUARTER TO THE POINT OF BEGINNING;

EXCEPT THE SOUTH 65.00 FEET THEREOF.

GENERAL NOTES FOR PUBLIC WORKS CONSTRUCTION:

1. ALL CONSTRUCTION IN THE PUBLIC RIGHTS-OF-WAY OR IN EASEMENTS GRANTED FOR PUBLIC USE MUST CONFORM TO THE LATEST MARICOPA ASSOCIATION OF GOVERNMENTS (MAG) UNIFORM STANDARD SPECIFICATIONS AND UNIFORM STANDARD DETAILS FOR PUBLIC WORKS CONSTRUCTION AS AMENDED BY THE LATEST VERSION OF THE CITY OF SCOTTSDALE STANDARD SPECIFICATIONS AND SUPPLEMENTAL STANDARD DETAILS. IF THERE IS A CONFLICT, THE CITY'S SUPPLEMENTAL STANDARD DETAILS WILL GOVERN.
2. THE CITY ONLY APPROVES THE SCOPE, NOT THE DETAIL OF ENGINEERING DESIGNS; THEREFORE IF CONSTRUCTION QUANTITIES ARE SHOWN ON THESE PLANS, THEY ARE NOT VERIFIED BY THE CITY.
3. THE APPROVAL OF PLANS IS VALID FOR SIX (6) MONTHS. IF ASSOCIATED PERMIT HAS NOT BEEN ISSUED WITHIN THIS TIME FRAME, THE PLANS MUST BE RESUBMITTED TO THE CITY FOR RE-APPROVAL.
4. A CITY INSPECTOR WILL INSPECT ALL WORKS WITHIN THE CITY OF SCOTTSDALE. NOTIFY INSPECTION SERVICES 72 HOURS BEFORE BEGINNING WORK.
5. WHENEVER EXCAVATION IS NECESSARY, CALL THE BLUE STAKE CENTER, 811, TWO WORKING DAYS BEFORE EXCAVATION BEGINS.
6. PERMISSION TO WORK IN THE RIGHT-OF-WAY (PWR) PERMITS ARE REQUIRED FOR ALL WORKS WITHIN THE RIGHTS-OF-WAY AND EASEMENTS GRANTED FOR PUBLIC PURPOSES. COPIES OF ALL PERMITS MUST BE RETAINED ON-SITE AND BE AVAILABLE FOR INSPECTION AT ALL TIMES. FAILURE TO PRODUCE THE REQUIRED PERMITS WILL RESULT IN IMMEDIATE SUSPENSION OF ALL WORK UNTIL THE PROPER PERMIT DOCUMENTATION IS OBTAINED.

WEIGHTED C CALCULATIONS

DRAINAGE AREA A

AREA OF IMPERVIOUS = 52,841 SF
AREA OF PERVIOUS = 32,793 SF

WEIGHTED C = $\left(\frac{(52{,}841 \text{ X } 0.95) + (32{,}793 \text{ X } 0.45)}{85{,}633}\right)$

WEIGHTED C = 0.76

DRAINAGE AREA B

AREA OF IMPERVIOUS = 10,312 SF
AREA OF PERVIOUS = 3,779 SF

WEIGHTED C = $\left(\frac{(10{,}312 \text{ X } 0.95) + (3{,}779 \text{ X } 0.45)}{14{,}091}\right)$

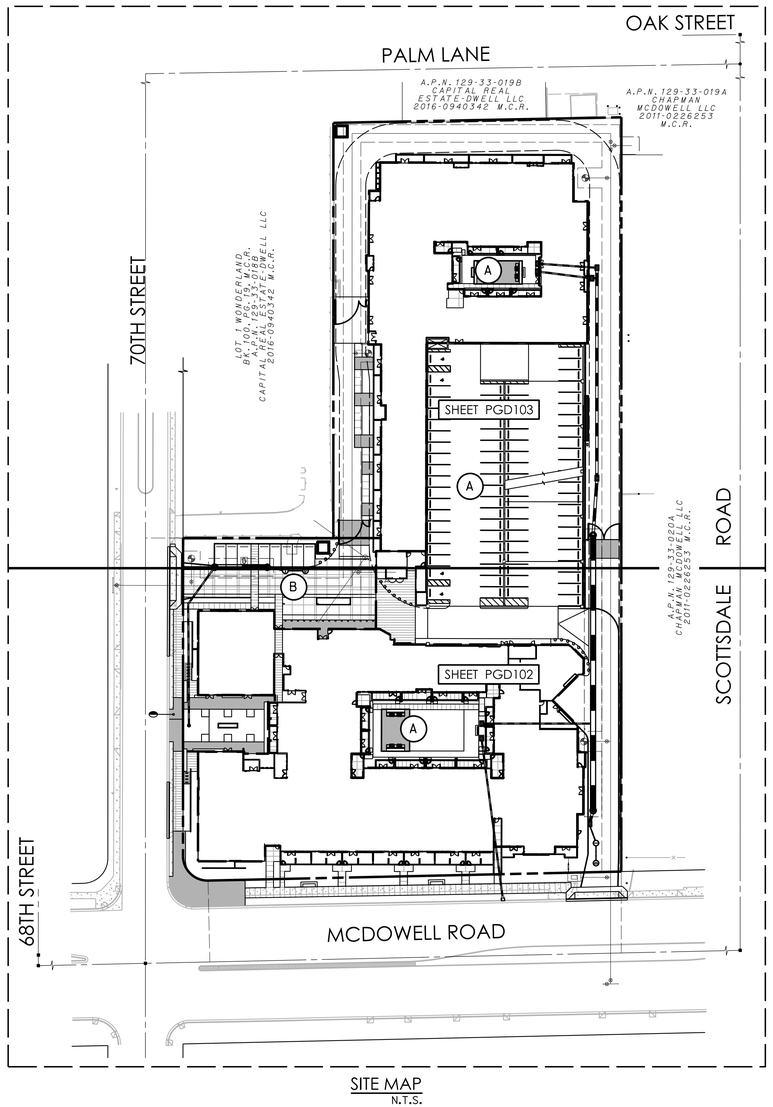
WEIGHTED C = 0.82

UTILITY	UTILITY COMPANY	NAME OF COMPANY REPRESENTATIVE	TELEPHONE NUMBER	DATE SIGNED
ELECTRIC	APS			
TELEPHONE	CENTURY LINK			
NATURAL GAS	SOUTHWEST GAS			
CABLE TV	COX COMMUNICATIONS			
OTHER	A.T.&T.			
OTHER				

ENGINEER'S CERTIFICATION

I DANIEL G. MANN, AS THE ENGINEER OF RECORD FOR THIS DEVELOPMENT, HEREBY CERTIFY THAT ALL UTILITY COMPANIES LISTED ABOVE HAVE BEEN PROVIDED FINAL IMPROVEMENT PLANS FOR REVIEW, AND THAT ALL CONFLICTS IDENTIFIED BY THE UTILITIES HAVE BEEN RESOLVED. IN ADDITION 'NO CONFLICT' FORMS HAVE BEEN OBTAINED FROM EACH UTILITY COMPANY AND ARE INCLUDED IN THIS SUBMITTAL.

SIGNATURE _____ DATE _____



SITE MAP
N.T.S.

INDEX OF SHEETS	
SHEET NO.	DESCRIPTION
PGD101	COVER SHEET - PRELIMINARY GRADING & DRAINAGE PLAN
PGD102	PRELIMINARY GRADING & DRAINAGE PLAN
PGD103	PRELIMINARY GRADING & DRAINAGE PLAN

FLOOD INSURANCE RATE MAP (FIRM) INFORMATION:

COMMUNITY NUMBER	PANEL NUMBER	PANEL DATE	SUFFIX	FIRM DATE	FIRM ZONE	BASE FLOOD ELEVATION
045012	2235	OCTOBER 16, 2013	L	OCTOBER 16, 2013	X	N/A

ENGINEER'S CERTIFICATION:

THE LOWEST FINISH FLOOR ELEVATION(S) AND/OR FLOOD PROOFING ELEVATION(S) ON THIS PLAN ARE SUFFICIENTLY HIGH TO PROVIDE PROTECTION FROM FLOODING CAUSED BY A 100-YEAR STORM, AND ARE IN ACCORDANCE WITH SCOTTSDALE REVISED CODE, CHAPTER 37 - FLOODPLAIN AND STORMWATER REGULATION.

RETENTION CALCULATIONS

PRE V. POST

C = 0.95 (IMPERVIOUS), C = 0.45 (PERVIOUS)
P = 2.14 IN
AREA OF LOT = 171,290 SF

PRE (EXISTING)

AREA OF IMPERVIOUS = 161,857 SF
AREA OF PERVIOUS = 9,433 SF

WEIGHTED C = $\left(\frac{(161{,}857 \text{ X } 0.95) + (9{,}433 \text{ X } 0.45)}{171{,}290}\right)$

WEIGHTED C = 0.92

POST (PROPOSED)

AREA OF IMPERVIOUS = 134,825 SF
AREA OF PERVIOUS = 36,465 SF

WEIGHTED C = $\left(\frac{(134{,}825 \text{ X } 0.95) + (36{,}465 \text{ X } 0.45)}{171{,}290}\right)$

WEIGHTED C = 0.84

(0.84-0.92) $\left(\frac{2.14}{12}\right)$ (171,290) = 0 CF REQUIRED

FIRST FLUSH STORM EVENT IS GREATER THAN PRE V POST

FIRST FLUSH STORM EVENT

DRAINAGE AREA A

C = 0.76
P = 0.5 IN
AREA = 85,633 SF

(0.76) $\left(\frac{0.5}{12}\right)$ (85,633) = 2,706 CF REQUIRED

2,764 CF PROVIDED

DRAINAGE AREA B

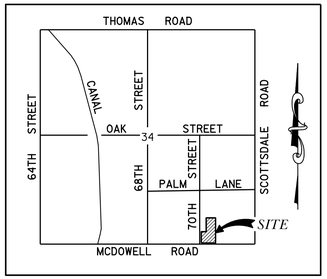
C = 0.82
P = 0.5 IN
AREA = 14,091 SF

(0.82) $\left(\frac{0.5}{12}\right)$ (14,091) = 479 CF REQUIRED

528 CF PROVIDED

LEGEND

———	INDICATES PROPERTY / BOUNDARY LINE
1560	INDICATES EXISTING CONTOUR ELEVATION
58	INDICATES PROPOSED CONTOUR ELEVATION
TC: 59.13 G: 58.64	INDICATES EXISTING TOP OF CURB ELEVATION / INDICATES EXISTING GUTTER ELEVATION
+ 62.52	INDICATES EXISTING GROUND ELEVATION
+ P: 60.11	INDICATES EXISTING PAVEMENT ELEVATION
+ C: 56.69	INDICATES EXISTING CONCRETE ELEVATION
15.9	INDICATES PROPOSED GROUND ELEVATION
1%	INDICATES DIRECTION OF FLOW & SLOPE
GB	INDICATES GRADE BREAK
P=	INDICATES PROPOSED PAVEMENT ELEVATION
C=	INDICATES PROPOSED TOP OF CONC. ELEVATION
G=	INDICATES PROPOSED GUTTER ELEVATION
LF_{88} =	INDICATES LOWEST FINISH FLOOR ELEVATION
W	INDICATES PROPOSED WATERLINE
S	INDICATES PROPOSED SEWERLINE
	INDICATES PROPOSED METER
	INDICATES PROPOSED SEWER CLEANOUT
	INDICATES PROPOSED CATCH BASIN
	INDICATES PROPOSED STORM DRAIN PIPE
	INDICATES PROPOSED STORM DRAIN MANHOLE
	INDICATES PROPOSED FIRE HYDRANT
FH	INDICATES EXISTING FIRE HYDRANT
24" SD	INDICATES EXISTING STORM DRAIN PIPE
8" S	INDICATES EXISTING SEWER LINE & SIZE
12"W (ACP) WV	INDICATES EXISTING WATER LINE,VALVE & SIZE
E	INDICATES EXISTING BURIED ELECTRIC CONDUIT
G	INDICATES EXISTING GAS LINE
OHE	INDICATES EXISTING OVERHEAD ELECTRIC
PP	INDICATES EXISTING POWER POLE
LP	INDICATES EXISTING LIGHT POLE
ET	INDICATES EXISTING ELECTRIC TRANSFORMER
EB	INDICATES EXISTING ELECTRIC BOX
WM	INDICATES EXISTING WATER METER
BWV	INDICATES EXISTING BACKFLOW PREVENTER VALVE



VICINITY MAP
N.T.S.

PROJECT SCOPE:

THE SCOPE OF THIS PROJECT IS A NEW RESIDENTIAL APARTMENT COMPLEX WITH 267 UNITS ALSO WITH PARKING GARAGE, UTILITY IMPROVEMENTS AND LANDSCAPE.

CLIENT:

HAWKINS COMPANIES LLC
4700 S. MCCLINTOCK DR. #160
TEMPE, ARIZONA 85282

CONTACT: MARK MITCHELL
PHONE: (480) 223-8239
EMAIL: MMITCHELL@HCOLLC.COM

ENGINEER:

3 ENGINEERING
6370 E. THOMAS ROAD, SUITE #200
SCOTTSDALE, ARIZONA 85251

CONTACT: DANIEL G. MANN, P.E.
PHONE: (602) 334-4387
EMAIL: DAN@3ENGINEERING.COM

PARCEL ADDRESS:

7000 E. MCDOWELL ROAD,
SCOTTSDALE, ARIZONA 85257

ASSESSORS PARCEL NUMBER:

129-33-001S

LOT AREA:

GROSS AREA:	4.690 ACRES
NET AREA:	3.932 ACRES
DISTURBED AREA:	3.932 ACRES

BENCHMARK:

MARICOPA COUNTY DEPARTMENT OF TRANSPORTATION UNIQUE ID: 12052, BEING A 3" CITY OF SCOTTSDALE BRASS CAP IN HANDHOLE, LOCATED AT THE INTERSECTION OF SCOTTSDALE ROAD & MCDOWELL ROAD

ELEVATION = 1230.474 (NAVD'88)
CITY OF SCOTTSDALE DATUM

I HEREBY CERTIFY THAT ALL ELEVATIONS REPRESENTED ON THIS PLAN ARE BASED ON THE ELEVATION DATUM FOR THE CITY OF SCOTTSDALE BENCHMARK PROVIDED ABOVE.

ENGINEER _____ 6/28/2021 DATE

BASIS OF BEARING

THE BASIS OF BEARING IS THE MONUMENT LINE OF SCOTTSDALE ROAD, ALSO BEING THE WEST LINE OF THE SOUTHEAST CORNER, SECTION 34, USING A BEARING OF NORTH 00 DEGREES 00 MINUTES 00 SECONDS EAST, AS PER THE RECORD OF SURVEY IN BOOK 1250 OF MAPS, PAGE 15, RECORDS OF MARICOPA COUNTY, ARIZONA.

ENGINEER'S STATEMENT:

THE ENGINEER OF RECORD ON THESE PLANS HAS RECEIVED A COPY OF THE APPROVED STIPULATIONS FOR THIS PROJECT AND HAS DESIGNED THESE PLANS IN CONFORMANCE WITH THE APPROVED STIPULATIONS

AS-BUILT CERTIFICATION:

I HEREBY CERTIFY THAT THE RECORD DRAWING MEASUREMENTS AS SHOWN HEREON WERE MADE UNDER MY SUPERVISION OR AS NOTED AND ARE CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

REGISTERED ENGINEER/LAND SURVEYOR _____ DATE _____

REGISTRATION NUMBER _____

CIVIL APPROVAL			
REVIEW & RECOMMENDED APRROVAL BY:			
PAVING		TRAFFIC	
G & D		PLANNING	
W & S		FIRE	
RET. WALLS			

ENGINEERING COORDINATION MGR. (OR DESIGNEE) _____ DATE _____

Contact Arizona 811 at least two full working days before you begin excavation
Arizona811.com
Call 811 or click Arizona811.com

SNOISIA3S

SOUTHDALE
7000 E. MCDOWELL ROAD, SCOTTSDALE, AZ 85257
PRELIMINARY GRADING & DRAINAGE PLAN

Registered Professional Engineer (Civil) CERTIFICATE NO. 46857 DANIEL G. MANN 4/30/21 RENEWAL DATE ARIZONA, U.S.A.

3engineering civil engineering surveying planning

ARCHITECT: PINLIE Y.
DESIGNER: D. MANN
DRAFTER: D. MANN
ENGINEER: D. MANN

3 ENGINEERING, LLC
6370 E. THOMAS ROAD
SUITE # 200
SCOTTSDALE, ARIZONA 85251
PHONE: (602) 334-4387
FAX: (602) 490-3230
WWW.3ENGINEERING.COM

DATE: 06/28/21

PROJECT NO.
1872

SHEET NO.
PGD101
1 of 3

Q.S. #13-44

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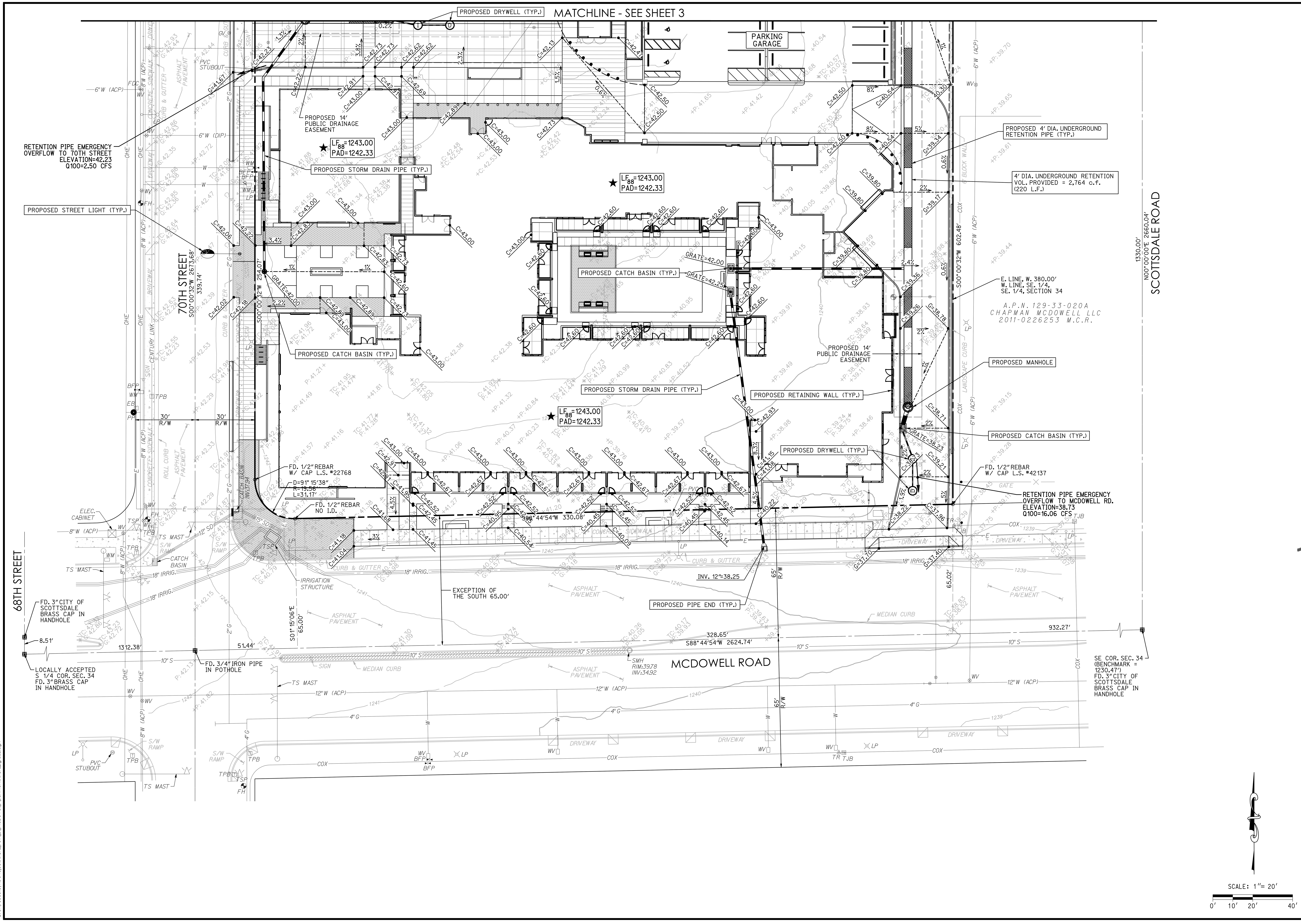

MATCHLINE - SEE SHEET 3
PROPOSED DRYWELL (TYP.)
PARKING GARAGE
PROPOSED 14' PUBLIC DRAINAGE EASEMENT
RETENTION PIPE EMERGENCY OVERFLOW TO 70TH STREET ELEVATION=42.23 Q100=2.50 CFS
PROPOSED STORM DRAIN PIPE (TYP.)
PROPOSED STREET LIGHT (TYP.)
PROPOSED CATCH BASIN (TYP.)
PROPOSED 4' DIA. UNDERGROUND RETENTION PIPE (TYP.)
4' DIA. UNDERGROUND RETENTION VOL. PROVIDED = 2,764 c.f. (220 L.F.)
E. LINE, W. 380.00' W. LINE, SE. 1/4, SE. 1/4, SECTION 34
A.P.N. 129-33-020A CHAPMAN MCDOWELL LLC 2011-0226253 M.C.R.
PROPOSED MANHOLE
PROPOSED RETAINING WALL (TYP.)
PROPOSED DRYWELL (TYP.)
FD. 1/2" REBAR W/ CAP L.S. #42137
RETENTION PIPE EMERGENCY OVERFLOW TO MCDOWELL RD. ELEVATION=38.73 Q100=16.06 CFS
70TH STREET
SCOTTSDALE ROAD
68TH STREET
MCDOWELL ROAD
FD. 1/2" REBAR W/ CAP L.S. #22768
FD. 1/2" REBAR NO I.D.
IRRIGATION STRUCTURE
EXCEPTION OF THE SOUTH 65.00'
INV. 12"=38.25
PROPOSED PIPE END (TYP.)
FD. 3" CITY OF SCOTTSDALE BRASS CAP IN HANDHOLE
LOCALLY ACCEPTED S 1/4 COR. SEC. 34 FD. 3" BRASS CAP IN HANDHOLE
FD. 3/4" IRON PIPE IN POTHOLE
SE COR. SEC. 34 (BENCHMARK = 1230.47') FD. 3" CITY OF SCOTTSDALE BRASS CAP IN HANDHOLE
S88°44'54"W 2624.74'
SCALE: 1"= 20'

Contact Arizona 811 at least two full working days before you begin excavation
Call 811 or click Arizona811.com

REVISIONS

SOUTHDALE

7000 E. MCDOWELL ROAD, SCOTTSDALE, AZ 85257

PRELIMINARY GRADING & DRAINAGE PLAN

Registered Professional Engineer (CIVIL) CERTIFICATE NO. 46857 DANIEL G. MANN Date Signed 4/26/21 ARIZONA, U.S.A.

3engineering
planning civil engineering surveying

ENGINEER: D. MANN | DESIGNER: D. MANN | CAD TECH: PHILLIE V.

3 ENGINEERING, LLC
6370 E. THOMAS ROAD SUITE # 200
SCOTTSDALE, ARIZONA 85251
PHONE: (602) 334-4387
FAX: (602) 490-3230
WWW.3ENGINEERING.COM

DATE: 06/28/21

PROJECT NO. 1872

SHEET NO. PGD102 2 of 3

Q.S.#13-44

OAK STREET

E 1/4 COR. SEC. 34
FD. 3" CITY OF SCOTTSDALE
BRASS CAP IN HANDHOLE

1330.03'

FD. 3" CITY OF SCOTTSDALE
BRASS CAP IN FLUSH

PALM LANE
S 89°03'15" W 2624.03'
1312.02'

LOT 1 WONDERLAND
BK. 100, PG. 19, M.C.R.
A.P.N. 129-33-018B
CAPITAL REAL ESTATE-DWELL LLC
2016-0940342 M.C.R.

A.P.N. 129-33-019B
CAPITAL REAL ESTATE-DWELL LLC
2016-0940342 M.C.R.

A.P.N. 129-33-019A
CHAPMAN MCDOWELL LLC
2011-0226253 M.C.R.

SMH
RIM: 41.36
INV 8"N: 34.98
INV 8"S: 34.99

N. LINE, S. 1/2,
SE. 1/4, SE. 1/4,
SECTION 34

N88°54'05"E 230.06'

EXISTING BUILDING

FD. NAIL IN
WASHER L.S. #42137

PROPOSED RETAINING WALL (TYP.)

LF88=1243.00
PAD=1242.33

PROPOSED RETAINING WALL (TYP.)

PROPOSED STORM DRAIN PIPE (TYP.)

E. LINE, W. 380.00'
W. LINE, SE. 1/4,
SE. 1/4, SECTION 34

PROPOSED CATCH BASIN (TYP.)

PROPOSED CATCH BASIN (TYP.)

PROPOSED CATCH BASIN (TYP.)

PROPOSED RETAINING WALL (TYP.)

PARKING GARAGE

PROPOSED STORM DRAIN PIPE (TYP.)

PARKING GARAGE

PROPOSED CATCH BASIN (TYP.)

PROPOSED MANHOLE

PROPOSED 4' DIA. UNDERGROUND RETENTION PIPE (TYP.)

PROPOSED 14' PUBLIC DRAINAGE EASEMENT

70TH STREET
S00°00'32"W 2673.68'
997.27'

FD. 1/2" REBAR
NO I.D.

S00°00'32"W 955.77'

S00°00'32"W 331.66'

S00°00'32"W 602.48'

N00°00'00"E 2660.04'
1330.00'

SCOTTSDALE ROAD

LOT 1 WONDERLAND
BK. 100, PG. 19, M.C.R.
A.P.N. 129-33-018B
CAPITAL REAL ESTATE-DWELL LLC
2016-0940342 M.C.R.

A.P.N. 129-33-020A
CHAPMAN MCDOWELL LLC
2011-0226253 M.C.R.

SMH
RIM: 40.74
INV 8"N: 36.07
INV 8"S: 36.17

FD. 1/2" REBAR
W/ CAP
L.S. #21081

SMH
RIM: 40.88
INV 8"N: 36.39
INV 8"W: 36.46

5' ELECTRIC
EASEMENT
PER REC. NO.
2000-0291367

FD. 1/2" REBAR
W/ CAP
L.S. #42137

LF88=1243.00
PAD=1242.33

SMH
RIM: 43.86
8"E: 37.29
8"W: 37.32

0.100=21.53 cfs

PROPOSED
NON-MOTORIZED
PUBLIC ACCESS
EASEMENT

0.100=4.58 cfs

0.100=7.89 cfs

N89°59'28"W 119.98'

PROPOSED MANHOLE W/ CATCH BASIN

PROPOSED 4' DIA. UNDERGROUND
RETENTION PIPE (TYP.)

PROPOSED 14'
PUBLIC DRAINAGE
EASEMENT

4' DIA. UNDERGROUND RETENTION
VOL. PROVIDED = 528 c.f.
(42 L.F.)

PROPOSED 14'
PUBLIC DRAINAGE
EASEMENT

MATCHLINE - SEE SHEET 2

SCALE: 1"= 20'
0' 10' 20' 40'

Contact Arizona 811 at least two full working days before you begin excavation
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SNOISIA3d

SOUTHDALE
7000 E. MCDOWELL ROAD, SCOTTSDALE, AZ 85257

PRELIMINARY GRADING & DRAINAGE PLAN

Registered Professional Engineer
CERTIFICATE NO.
46857
DANIEL G. MANN
4/26/21
DATE SIGNED
ARIZONA, U.S.A.

3engineering
civil engineering surveying
planning

CAD TECH: PAULIE S.
DESIGNER: D. MANN
ENGINEER: D. MANN

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DATE: 06/28/21

PROJECT NO.
1872

SHEET NO.
PGD103
3 of 3

Q.S.#13-44

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