CAVASSON

LOCATED NEAR THE NORTHEAST CORNER OF NORTH MILLER ROAD AND EAST CAVASSON BOULEVARD

PRELIMINARY DRAINAGE REPORT- VI AT CAVASSON

Basis of Design

September 16, 2021

Project No.: 18114-701

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1. INTRODUCTION

1.1 Project Scope

This report presents the results of a *Preliminary Drainage Study* conducted by Hubbard Engineering at the request of Orcutt | Winslow ("client"), for Vi at Cavasson development ("site"). The purpose of this report is to provide a hydrologic evaluation for the site as required by the City of Scottsdale per Ordinances 4346 and 4347. This report addresses off-site and on-site conditions. Drainage calculations and methodologies conform to the City of Scottsdale requirements and standards and to Hubbard Engineering's submitted *Master Drainage Report* for the Cavasson development.

This report is focused on providing practical design information, evaluation, and calculations for statistical flood events up to and including the 100-year frequency flood. The procedures used herein are derived from and performed with currently accepted engineering methodologies and practices. Additionally, the criteria for this evaluation are designed to conform to currently applicable ordinances, regulations and policies affected by the appropriate jurisdictional regulatory authorities for the site.

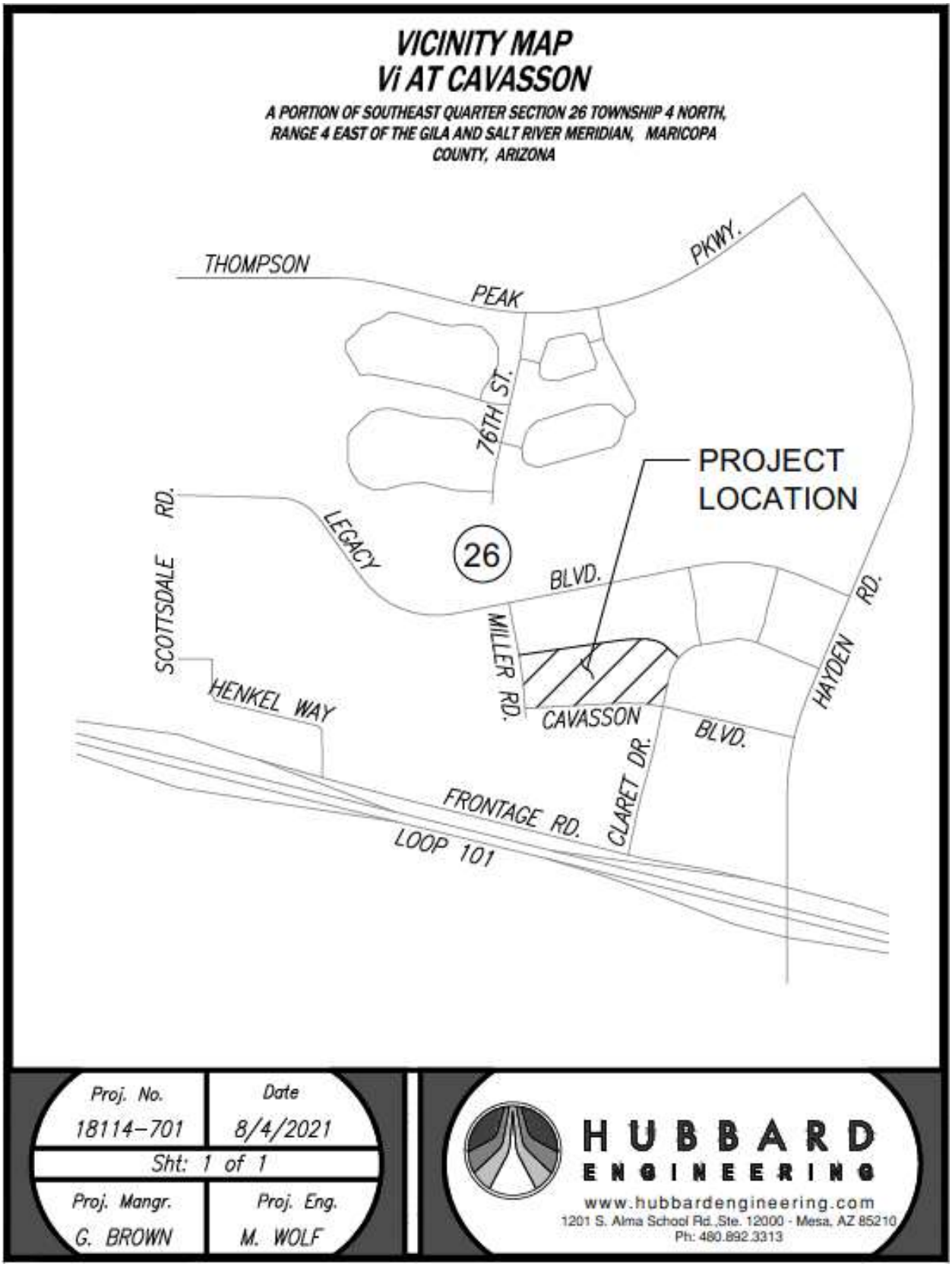
The analysis presented herein focuses on developing design estimates of storm water runoff resulting from a statistical evaluation of storm events of a particular duration and frequency, up to and including a 100-year frequency event. A storm event exceeding the 100-year frequency event may cause or create the risk of greater flood impact than is addressed and presented herein. The scope of this assessment does not include evaluation of storm water runoff resulting from storm events exceeding the 100-year frequency event. Hubbard assumes no responsibility for actual flood damage, increased risks of flood damage, or increased construction or development costs resulting from or related to any such events. Nor shall Hubbard be responsible for any changes in, or additions to, regulatory requirements which may result from, or be related to, any such events or changes in hydrologic or hydraulic conditions within the watershed.

1.2 Site Description

The project site is located in the southeast quarter of Section 26, Township 4N, Range 4E of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. The site is currently undeveloped, and prior to Nationwide Realty Investor's acquisition, was held in trust by the Arizona State Land Department (ASLD) as a portion of the overall Crossroads East development, which encompasses approximately 883 gross acres. The Vi at Cavasson development is located in the Northwest corner of the overall Crossroads East development, near the Cavasson Boulevard and Miller Road intersection. The land naturally falls from northeast to southwest.

The project site is bounded by undeveloped desert to the north, Claret Road to the east, North Miller Road to the west, and Cavasson Boulevard to the south. The site location is shown in **Figure 1.1 – Vicinity Map**.

Figure 1.1 – Vicinity Map



1.3 Project Type

The Cavasson development is being developed by Nationwide Reality Investors as a master planned mixed use development with office, retail, hotels, and multifamily residential parcels with public and private roadways that run adjacent and through the development. Vi at Cavasson will include construction of a new senior living facility on an 11.3-acre site with multiple buildings. Improvements will also include surrounding access drives, parking, and extending existing private utility stubs to service the building.

The analyses of pre-development and post-development peak discharges were addressed in Hubbard's *Master Drainage Report Phase 3 update* and will be referenced and further discussed as it applies to the Grayhawk Residences at Cavasson.

1.4 Special Conditions- 404 Washes

Located throughout the project site are waters of the United States 404 washes. The handling of the 404 washes has been coordinated with engineering consultants and the City of Scottsdale. The Section 404 Certification form has been completed by consultants. As discussed in the *Master Drainage Report*, Hubbard has met with the Army Corps of Engineer (USACE) and worked with consultants and the Corps to complete a mitigation plan. Approval of the 404 permit was granted on April 2, 2019. That permit number is SPL-2018-00704. ADEQ has issued the 401 permit. This permit has been attached for reference in **Appendix G.**

1.5 Regulatory Criteria

The criterion used in the drainage design and analysis of the site was established using the guidelines as described in the following:

- City of Scottsdale, *Design Standards & Policies Manual*, January 18, 2018.
- City of Scottsdale, *Ordinance No. 4346,* June 17, 2018.
- *Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology*.
- *Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics*.
- Hubbard Engineering*, Master Drainage Report for Cavasson, Phase 3 Update,* July 19, 2021.
- Hubbard Engineering*, Master Drainage Report for Cavasson,* December 23, 2018.

2. EXISTING DRAINAGE CONDITIONS

2.1 Existing Off-Site Conditions Characteristics

Development in the surrounding area has increased in the last decade and many of the surrounding properties which sit in the Reata Pass basin, part of the Pinnacle Peak South Area, have installed infrastructure to route the offsite flows through their developments. As-built plans for the developments show that channels and culverts have been constructed to divert and route off-site flows. Historical runoff for the areas on and surrounding the project site flow south towards collections points (i.e. existing culverts) where the off-site storm runoff is ultimately conveyed to the TPC golf course just north of the Central Arizona Project canal. Existing culverts located at the edges of Hayden Road are currently utilized to route off-site runoff. Off-site routing for the developed surrounding areas has been verified with their corresponding as-builts, including Hayden Road Improvement Plans, 76th Street & Infrastructure Design, Center Drive (now Legacy Boulevard) Improvements, and One Scottsdale Civic Center Improvement Plans.

Hubbard Engineering completed a *Master Drainage Report* which included an extensive analysis of the off-site flow conditions affecting the entire Crossroads East area, including Cavasson and Hayden Road, for the existing site and proposed development. Results from the study indicate that the storm event affecting the Vi at Cavasson is a 100-year, 6-hour storm event. Since the submittal of the *Master Drainage Report,* Hubbard has met with the City of Scottsdale and the developers south of the Cavasson development. It was determined in the meeting that the HEC-1 models be adjusted to reflect a time interval of 15 minutes rather than 3 minutes, as was originally modeled in TY Lin's report. The flows affecting the site will be discussed in section 3.1. Details on Hubbard's HEC-1 methodology and analyses, in addition to the modeling results, can be referenced in the approved *Cavasson Master Drainage Report Phase 3 Update* dated April 30, 2021.

The Vi at Cavasson site specifically is being impacted by flows from two sets of dual barrel elliptical culverts crossing Legacy Boulevard in the existing condition. These culverts are conveying flow form basins SB01-B and SB01-C north of Legacy Boulevard. This flow then runs through the currently undeveloped parcel 1C-1 before reaching the Vi at Cavasson site along with the runoff from parcel 1C-1. Additionally, the site is currently being impacted by the offsite flow from Cavasson Boulevard which is being discharged to a newly constructed sediment basin along the south property line through two existing culverts.

2.2 On-site Drainage

As the Cavasson project site was undeveloped prior to Phase 1, there were no on-site drainage structures. The project site was included in two previous studies, Bob Ward's *Core North/ Core South Drainage Study* and TY Lin's *Pinnacle Peak South Area Drainage Master Study.* The TY Lin report analyzed the Crossroads East area and included FLO2d models combined with HEC-1 analyses on localized basins. The TY Lin report provided a basis on which Hubbard completed its hydrologic studies for the Cavasson project development.

The project site naturally falls from north to south at approximately 1.2% and approximately 0.7% east to west. The site outfall for Vi at Cavasson occurs at the southern boundary of the project along the northeast corner of the intersection of Miller Road and Cavasson Boulevard at an elevation of 1622'.

2.2.1 HEC-1 Analysis

The TY-Lin *Pinnacle Peak South Area Drainage Master Study* report provides an analysis of the entire Pinnacle Peak South Area, which covers an approximate area of 40 square miles. According to the report, the project site is located within the Dobson Wash Watershed, which includes inflow from the southwestern flow split of the Reata Pass Wash. TY Lin's existing and proposed exhibits and results can be found in **Appendices A** and **B**, respectively. Hubbard replicated TY-Lin's model in application to the approximate nine square miles including and surrounding the project site, shown in **Exhibit 1,** and included the inflow hydrographs from TY Lin's Master Drainage study to account for run-off affecting the project site from the 76th Street Channel and the Powerline for both the 100-year, 6-hour and 100-year, 24-hour storm events. Data collected for the model includes the precipitation, soil, and land use for the existing site conditions in application to the delineated sub-basins. All data Hubbard obtained and used in the model was compared to the existing TY Lin model for accuracy and adjusted accordingly based on current site conditions at maximum densities per approved zoning cases. Precipitation data was obtained from the NOAA Atlas 14 precipitation database. The map index used in DDMSW is 64, cells 687-689. Existing soil data was obtained from the United States Department of Agriculture's Natural Resources Conservation Service. Additionally, land use data was determined based on current zoning of the project site. Data for the HEC-1 analysis are in **Appendix C**. The results of the ultimate condition at full build out (after the powerline channel is constructed) are included in **Appendix D**.

Hubbard has met with the City of Scottsdale and the developers south of the Cavasson development in order to coordinate the analyses for the Crossroads East development. It was determined in the meeting that the HEC-1 models be adjusted to reflect a time interval of 15 minutes rather than 3 minutes, as was originally modeled in TY Lin's report. Thus, the inflow hydrographs provided by TY Lin were convoluted to accurately represent this change in time interval resulting in more accurate time to peak values. In addition, it was also decided in the meeting that Hubbard's analysis for the existing, proposed, and ultimate conditions will be used by developers south of Hayden Road. Hubbard's sub-basin delineation, shown in **Exhibit 1**, deviates from TY Lin's analysis in that Hubbard added collection points at existing culverts along Legacy Boulevard and Hayden road to model the peak flows coming through each infrastructure for utilization in routing design. Additionally, detailed analysis of the existing topography and as-builts along Hayden road and Legacy Boulevard revealed that the basins were not segmented by Hayden Road or Legacy Boulevard, in the TY Lin Report. These larger basins were subdivided in the Hubbard analysis and a similar naming convention was used to compare baseline flows.

Hubbard's analysis split the original TY Lin sub-basins 7 and 8 into east and west components to more accurately model the conveyance of the flows on either side of Hayden Road. An additional basin, SB09N was also added per the city's request as this was not included in the TY Lin analysis. In Hubbard's routing, flows from SB07E and SB08E are routed east, following the topography. Since the TY-Lin model does not delineate a SB-08W, the TY-Lin model shows the flow from SB08 flowing from the southwest corner of the basin east across Hayden Road. This analysis is not correct considering that the eastern elevation is approximately 20 feet higher than the western elevation (CP-08 in TY-Lin's exhibit). This routing also neglects the flow being conveyed across the Loop 101 from SB07W and SB08W. Hubbard therefore routed the flow from SB07W and SB08W to the existing ADOT culvert along Frontage road and across the Loop 101,

thus affecting the developments south of the Loop 101, a condition not represented in the TY-Lin report.

In TY-Lin's analysis, the 100-year, 24-hour storm event controlled for the entire area, with the 100-year, 6-hour storm event controlling the localized flows. TY-Lin recommended the creation of the Powerline Channel to intercept and route the 100-year, 24-hour flows to Basin 53R. Since it was determined in meetings with the City of Scottsdale that the channel will be built during construction of the Cavasson development, Hubbard's existing model includes this channel and results for the 100-year, 24-hour, 100-year, 6-hour, and 10-year, 6-hour storm events are shown in **Appendix D.** The Existing 100-year, 6-hour flows prior to the offsite channel being constructed were 196 cfs entering the site and an Existing condition 100-Year, 6-hour discharge exiting the site at the Loop 101 of 740 cfs. With the addition of the channel and routing to Basin 53R, a significant amount of flow is being diverted from the affected study area to Basin 53R. The addition of the channel results in higher peak flows for the Crossroads East development from the 100-year, 6-hour storm event rather than the 100-year, 24-hour storm.

In addition to the existing model post-channel and proposed model, Hubbard completed an ultimate condition model considering the surrounding lots will be developed. In the model, this was represented by increasing the imperviousness of SB01-B, SB01-C, and SB07W to reflect maximum assumed densities per the zoning stipulations pertinent to the Cavasson development and Cross Roads East approved zoning cases.

2.3 Flood Zone Information

The Maricopa County, Arizona and Incorporated Areas Flood Insurance Rate Map (F.I.R.M.) number 04013C1320L, Panel number 1320 of 4425, dated October 16, 2013 indicates that Vi at Cavasson falls within Zone AO (Depth 1 foot) on the west side of the project with the remainder classified as Zone X.

Zone AO is defined as:

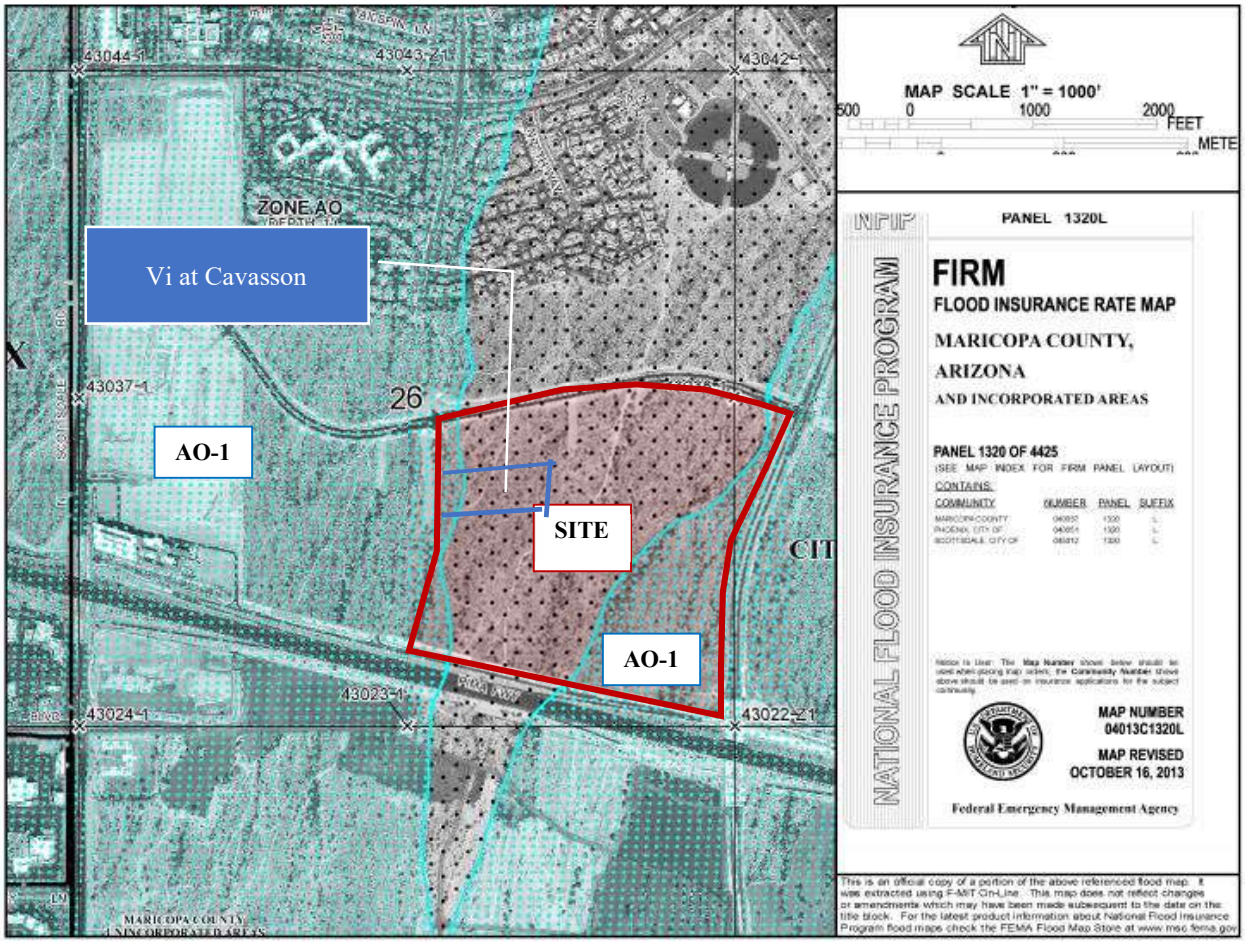
"*Areas subject to inundation by 1-percent-annual-chance shallow flooding (usually sheet flow on sloping terrain) where average depths are between one and three feet. Average flood depths derived from detailed hydraulic analyses are shown in this zone. Mandatory flood insurance purchase requirements and floodplain management standards apply.*"

Zone X is defined as:

"Areas of 0.2% annual chance flood; areas of 1.0% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood."

Refer to FEMA Firmette in **Figure 2.1.**

Figure 2.1 – FIRM 04013C1320L



3. PROPOSED DRAINAGE PLAN

3.1 Off-Site Flows

Off-site flows for the site were analyzed and addressed in Hubbard's *Master Drainage Report Phase 3 Update.* Running along the west side of the site is the Offsite Storm Drain Network (Plan Check #1838-21) designed by Hubbard Engineering, construction expected to begin by November of 2021, which routes the existing offsite flow from north of Legacy boulevard along the outside perimeter of the site and across Cavasson Boulevard. This allows the offsite flows to maintain their existing flow patterns without impacting Vi at Cavasson. In the event that the Offsite Storm Drain System infrastructure fails, the offsite flow from north of Legacy would be contained within an existing swale to the north of Legacy Boulevard before entering an existing 10'x3' curved box culvert that discharges the flow to the south of Legacy Boulevard and west of Miller Road. This flow would then continue along the west side of Miller Road before joining its historic flow pattern just south of Cavasson Boulevard. It should be noted that due to sedimentation within the existing culverts crossing Legacy Boulevard, this overflow route is already being used in this manner for some of the flow. The Offsite Storm Drain System was discussed in further detail within Hubbard's *Master Drainage Report Phase 3 Update*.

The offsite flow from the adjacent Claret Drive will be conveyed to the site through an existing catch basin that will be tied directly into the onsite storm drain system. This offsite flow was calculated as part of the *Phase 3 Roads Drainage Report* written by Hubbard Engineering. Offsite flows from the adjacent half street of Miller Road will not impact the site because Miller Road was designed to convey all of its flow to two curb openings south of Cavasson Boulevard. Vi at Cavasson will still however retain the appropriate volume for the 100-year 6-hour storm event for the adjacent half street to remove those flows from the system. The offsite flow from the adjacent Cavasson Boulevard will be conveyed to the site through two existing catch basins that will be tied directly into the onsite storm drain system. This offsite flow was calculated as part of the *Final Drainage Report-Cavasson Boulevard* written by Hubbard Engineering. A summary of the offsite flows impacting the site has been provided in **Table 1** below. Post-Development condition includes the development of the Vi at Cavasson Site, the Phase 3 roads, and the Offsite Storm Drain Only. The Ultimate Build Out condition additionally includes the development of Parcel 1C-1

Table 1: Offsite Flow Summary Table

Contributing Area	Pre-Development Q100 (cfs)	Post-Development Q100 (cfs)	Ultimate Build Out Q100 (cfs)
SB01-B	19	0	0
SB01-C	19	0	0
Claret Drive	0	2.5	2.5
Cavasson Boulevard	6.25	6.25	6.25
Parcel 1C-1	45.01	45.01	0

3.2 On-Site Hydraulics

The storm event affecting the site was determined to be a 100-year, 6-hour storm, as discussed in Hubbard's *Master Drainage Report Phase 3 Update.* The Vi at Cavasson will include the installation of several multifamily apartment buildings throughout the site as well as a parking lot and private drives servicing all of the buildings. Additionally, the Offsite Storm Drain System runs along the west boundary of the site within an existing drainage easement.

See **Exhibit 2** for the drainage map. The onsite storm drain system for the Vi at Cavasson consists of two systems running along the north and south drives of the site. The north system ties into the end of one of the underground retention tanks with a 36-inch diameter HDPE pipe running along the north drive. This pipe will then become a 24-inch diameter HDPE pipe after about 150 feet and continues along the north drive. The north system will also service the enclosed courtyard for the main building. It was determined that an 18-inch pipe is required in order to properly discharge the storm water. A second 18-inch pipe will run in parallel to act as an emergency outlet in case the other pipe has an issue such as breaking or clogging. Along this main run, there are several inlets that are teed off of the main run to drain front courtyards of buildings and accept the flow from side parking lots for the buildings. The flows from adjacent building roof drains will sheet flow to inlets located along the north storm drain which will then convey the flows to the underground retention tanks.

The south storm drain system ties into the end of a different underground retention tank with a 36-inch diameter HDPE pipe that runs along the southern drive. The 36-inch diameter pipe becomes a 24-inch diameter HDPE pipe at a manhole structure after about 185 feet. At this same manhole structure. The south storm drain system then continues along the drive for another 200 feet where the system that drains the main courtyard located on the eastern side of the site. The branch in the east of the courtyard ties in to an existing headwall, which will be replaced with a manhole, that is connected to the Claret Drive catch basin to accept its flow. Similar to the north system, the south system has multiple inlets that are teed off of the main system to capture flow from different parking areas. Additionally, flows from adjacent building roof drains will sheet flow to inlets located along the south storm drain to be conveyed to the underground retention tanks.

There are five underground retention tanks that are all equalized together on the near the southern drive entrance that the south storm drain systems drain to. There are two equalized storm drain tanks in the northeast that receive the flows from the north storm drain system. In greater storm events and back-to-back storm events an emergency overflow has been provided for the retention tanks. A 12-inch diameter HDPE overflow pipe has been designed to connect into the riser of one of the underground retention tanks for the south and north system that will allow excess volume to overflow. This pipe runs down the west side of the property to connect to the Offsite Storm Drain System as an emergency overflow only. The Offsite Storm Drain System was designed to accept up to 20 cfs of emergency overflow from the site. For design purposes, the tailwater for the north and south storm drain systems were set at the top of their respective equalized tanks and the tailwater for the emergency overflow storm drain system was set at the HGL of the Offsite Storm Drain System at the connection point. Head loss for the pipes was calculated using Manning's Equation and head loss though the structures was calculated using the equation $k(\frac{v^2}{2g})$ where k is the junction loss coefficient, v is the velocity exiting the junction and g is the acceleration of gravity. See **Appendix E** for hydrologic calculations and **Appendix F** for hydraulic calculations.

3.3 Storm Water Storage

The project development is required to provide retention for the first flush storm event for onsite and the 100 year 6-hour storm event for the adjacent half streets. The required first flush volume determined for each drainage area site will be designed per City of Scottsdale *Drainage Policies and Standards for Maricopa County, Arizona* (Reference 1). The first flush retention required for the site will be 18,753 cubic feet. The required retention for the adjacent half streets for the site is 21,433 cubic feet. This results in a total required retention of 40,186 which will be retained in 533 lf of 10' CMP underground retention tanks. These tanks provide a total of 41,862 cubic feet of retention which meets the requirement. The required and provided retention calculations can be found in **Appendix F**.

3.3.1 Time to Drain

The site will be required to dewater the required storm water retention through natural percolation and/or drywells. The City of Scottsdale requires all retention facilities be completely drained within a 36-hour time frame. For design purposes it was assumed that each drywell will have a percolation rate of 0.1 cfs. During construction, the first drywell constructed will be tested to verify this percolation rate. In the event that the tested percolation rate plus a clogging factor exceed 0.1 cfs, the number of drywells may be reduced while ensuring all retention facilities fully drain within 36 hours. Using this discharge rate, it was determined that 5 separate drywells be provided to drain the total volume of the underground retention tanks below the emergency outfall pipes located in the risers. This will allows the retention tanks to fully dewater in a total time of 22.3 hours. See **Appendix F** for drywell calculations.

4. SUMMARY AND CONCLUSION

- The site is located in Section 26 of Township 4N, Range 4E of the Gila and Salt River Base and Meridian, Maricopa County, Arizona.
- The site is partially located in the Flood Plain Zone AO-1 on the west side and in Flood Plain Zone X on the remainder. No building are in the Zone AO-I
- The storm event affecting the site was modeled as a 100-yr, 6-hour event in Hubbard's *Master Drainage Report* as it was more conservative.
- The sites onsite storm drain system is directly accepting the offsite flows from Claret Drive and Cavasson Boulevard.
- The site will be required to provide retention for the first flush storm event for onsite and provide retention for the 100 year 6 hour storm event for the adjacent half street roads.
- 533 lf of 10' CMP retention will provide 41,862 cf of retention, for the required retention of 40,186 cf.

5. REFERENCES

1) City of Scottsdale. *Drainage Policies & Standards for Maricopa County, Arizona.* January 18, 2018.

2) Flood Control District of Maricopa County. *Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology*. November 2003.

3) Flood Control District of Maricopa County. *Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics*. November 2003.

4) Flood Control District of Maricopa County. *Drainage Design Manual for Maricopa County, Arizona, Volume III, Erosion.* July 2018.

5) City of Scottsdale. *Ordinance No. 4346.* June 17, 2018.

6) TY Lin, *Pinnacle Peak South Area Drainage Master Study,* 2014.

7) City of Scottsdale, *Ordinance No. 4346,* June 17, 2018.

8) Hubbard Engineering*, Master Drainage Report for Cavasson, Phase 3 Update,* July 19, 2021.

9) Hubbard Engineering*, Master Drainage Report for Cavasson,* December 23, 2018.

10) Hubbard Engineering*, Final Drainage Report-Cavasson Boulevard*, August 6, 2019.

11) Hubbard Engineering*, Phase 3 Roads Drainage Report,* May 26, 2021.

6. LIMITATIONS

This report is focused on providing practical design information, evaluation, and calculations for statistical flood events up to and including the 100-year frequency flood. The procedures used herein are derived from, and performed with, currently accepted engineering methodologies and practices. Additionally, the criteria for this evaluation is designed to conform to currently applicable ordinances, regulations and policies effected by the appropriate jurisdictional regulatory authorities for the site.

The analysis presented herein focuses on developing design estimates of storm water runoff resulting from a statistical evaluation of storm events of particular duration and frequency up to and including a 100-year frequency event. A storm event exceeding the 100-year frequency event may cause or create the risk of greater flood impact than is addressed and presented herein. However, the scope of this assessment does not include evaluation of storm water runoff resulting from storm events exceeding the 100-year frequency event. Hubbard Engineering assumes no responsibility for actual flood damage, increased risks of flood damage, or increased construction or development cost resulting from or related to any such events.

Nor shall Hubbard Engineering be responsible for any changes in, or additions to, regulatory requirements which may result from, or be related to, any such events or changes in hydrologic or hydraulic conditions within the watershed.

In performing the services contained herein, Hubbard Engineering has received or will receive information prepared or compiled by others. Hubbard Engineering, as engineering professionals, are not required to verify the information, but may rely on the information unless actual knowledge concerning the validity of the information is known or is obvious to the professional. Therefore, Hubbard Engineering is entitled to rely upon the accuracy and completeness of this information without independent evaluation or verification.

Appendix A
TY Lin HEC-1 Existing
Vi at Cavasson

WATERSHED MAP

0 200 400 800 Feet

TY LIN INTERNATIONAL
engineers| planners| scientists

Legend

- Flow Path
- Sub-basins
- Proposed ROW

SB01
SB02
SB03
SB04
SB05
SB06
SB07
SB08
SB09
SB10
SB11

N 76TH ST
E LEGACY BL
N HAYDEN RD
N 84TH WY
N GRAYHAWK DR
N 84TH ST
E HUALAPAI DR
N 89TH PL
N 88TH WY
N 88TH ST
N 90TH PL
N 90TH WY
N SCOTTSDALE RD
L101 EB
L101 WB
E UNION HILLS DR
N PERIMETER DR
N 81ST WY
N 82ND ST
E ST JOHN RD
E PRINCESS BL
E PRINCESS DR
N 77TH PL

Drainage Sub-Basin ID	Area (sq. mi.)	Longest Flow Path (mi.)	Upstream Ground Elev. (ft)	Downstream Ground Elev. (ft)	Slope (ft/mi)
SB01	0.112	0.641	1678	1637	64.0
SB02	0.157	0.728	1651	1605	63.2
SB03	0.048	0.352	1600	1594	17.1
SB04	0.144	0.695	1592	1546	66.2
SB05	0.130	0.699	1650	1613	52.9
SB06	0.136	0.575	1584	1557	47.0
SB07	0.074	0.601	1602	1567	58.3
SB08	0.224	0.321	1683	1653	93.5
SB09	0.052	0.450	1654	1618	80.0
SB10	0.040	0.271	1603	1590	48.1
SB11	0.071	0.457	1606	1583	50.4

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Appendix B
TY Lin HEC-1 Proposed
Vi at Cavasson

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1****************************************
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 *                                      *                                                *
  *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)  *                                                *    U.S. ARMY CORPS OF ENGINEERS
  *
 *               JUN   1998             *                                                *    HYDROLOGIC ENGINEERING CENTER
  *
 *            VERSION 4.1               *                                                *          609 SECOND STREET
  *
 *                                      *                                                *       DAVIS, CALIFORNIA 95616
  *
 *  RUN DATE   12SEP14  TIME  11:45:27  *                                                *           (916) 756-1104
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XXXXXXX  XXXX     X        XXXXX   X
X     X  X        X                X
X     X  X        X     X          X
X     X  XXXXXXX   XXXXX          XXX
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


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          LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

            1           ID        CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
            2           ID        RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
            3           ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
            4           ID       100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
            5           ID           PIMA ROAD FLOWS)
            6           ID
            7           ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
            8           ID       MODELERS: RK, MW
            9           ID
           10           IT       3       0       0    1000
           11           IN       3
           12           IO       5
                        *DIAGRAM
                        *
           13           JD   3.849  0.0001
           14           PC   0.000   0.002   0.005   0.008   0.011   0.014   0.017   0.020   0.023   0.026
           15           PC   0.029   0.032   0.035   0.038   0.041   0.044   0.048   0.052   0.056   0.060
           16           PC   0.064   0.068   0.072   0.076   0.080   0.085   0.090   0.095   0.100   0.105
           17           PC   0.110   0.115   0.120   0.126   0.133   0.140   0.147   0.155   0.163   0.172
           18           PC   0.181   0.191   0.203   0.218   0.236   0.257   0.283   0.387   0.663   0.707
           19           PC   0.735   0.758   0.776   0.791   0.804   0.815   0.825   0.834   0.842   0.849
           20           PC   0.856   0.863   0.869   0.875   0.881   0.887   0.893   0.898   0.903   0.908
           21           PC   0.913   0.918   0.922   0.926   0.930   0.934   0.938   0.942   0.946   0.950
           22           PC   0.953   0.956   0.959   0.962   0.965   0.968   0.971   0.974   0.977   0.980
           23           PC   0.983   0.986   0.989   0.992   0.995   0.998   1.000
           24           JD   3.657    10.0
           25           JD   3.533    20.0
                        *
           26           KK  76THST
           27           KM        76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
           28           KM        100-YR, 6-HR FLO-2D MODEL (XS 98)
           29           BA    0.24
           30           QI       0       0       0       0       0       0       0       0       0
           31           QI       0       0       0       0       0       0       0       0       0
           32           QI       0       0       0       0       0       0       0       0       0
           33           QI       0       0       0       0       0       0       0       0       0
           34           QI       0       0       0       0       0       0       0       0       0
           35           QI       0       0       0       0       0       0       0       0       0
           36           QI       0       0       0       0       0       0       0       0       0
           37           QI       0       0       0       0       0       0       0       0       0
           38           QI       0       0       0       0       0       0       0       0       0
           39           QI       0       0       0       0       0       0       0       0       0
           40           QI       0       0    0.01       0       0       0       0       0       0
           41           QI       0       0       0       0       0       0       0       0       0
           42           QI       0       0       0       0       0       0       0       0       0
           43           QI       0       0       0       0       0       0       0       0       0
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     44          QI       0       0       0       0    0.01    0.01       0       0       0       0
     45          QI       0       0       0       0       0       0       0       0       0       0
     46          QI       0       0       0       0       0       0       0       0       0       0
     47          QI    0.01       0       0    0.01       0       0       0       0    0.01    0.01
     48          QI       0    0.01    0.01    0.02    0.02    0.02    0.02    0.02    0.03    0.03
     49          QI    0.03    0.04    0.04    0.05    0.05    0.05    0.06    0.06    0.07    0.08
     50          QI    0.09    0.09     0.1     0.1    0.11    0.11    0.12    0.12    0.13    0.14
     51          QI    0.14    0.15    0.16    0.18    0.19    0.21    0.23    0.25    0.27     0.3
1                                        HEC-1 INPUT                                          PAGE  2

   LINE          ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

     52          QI    0.34    0.36     0.4    0.44    0.47    0.51    0.56     0.6    0.66    0.71
     53          QI    0.76    0.95    1.61    1.73    1.95     3.7    4.26    6.53    8.52   10.41
     54          QI   17.69   31.39   35.75   41.23   44.49   47.79   51.13   56.33   62.34   66.18
     55          QI   68.57   67.36   65.05   61.38   58.37   54.61   52.38   50.75    48.4   44.37
     56          QI   42.75    40.4   38.91   37.61   36.75   35.61   34.75   34.25   33.52   32.65
     57          QI   32.34   31.59   31.31   30.73   30.02   29.25   28.51   28.42   27.67   26.83
     58          QI   26.13   25.37   24.53   23.95   23.37   22.83   22.17   21.48   20.81   20.94
     59          QI   19.41   19.05   18.62    18.3   18.17   18.11   17.52    17.1    16.7    16.5
     60          QI   16.15   15.74   15.37   15.07   14.74   14.54   14.51   14.24   14.07   13.65
     61          QI   13.75   13.24   12.99   13.09   12.94   12.53   12.53   12.38   12.22   12.18
     62          QI      12   11.59   11.89   11.53   11.85   11.44   11.16   11.22   11.21   10.82
     63          QI   10.52   10.43   10.88   10.42    10.1    9.81    9.72    9.66    9.78    9.55
     64          QI    9.34    9.36    9.33    9.23     9.1    9.09    9.48     9.3    9.02    9.07
     65          QI    9.11     9.1    8.93    8.81    8.73    8.71    8.57     8.5     8.5    8.43
     66          QI    8.37    8.31    8.26    8.17    8.14    8.05    7.98    7.97    7.83    7.62
     67          QI    7.67    7.67    7.57    7.66    7.74    7.75    7.91    7.88    7.62    7.55
     68          QI    7.47     7.3    7.22    7.25     7.2    7.08    7.02    7.01    6.99    6.96
     69          QI    6.97    6.92    6.89    6.85    6.81    6.77    6.75     6.7    6.67    6.62
     70          QI    6.59    6.56    6.51    6.48    6.47    6.44    6.44    6.47    6.43    6.37
     71          QI    6.32    6.27    6.23    6.22    6.16    6.12    6.08    6.05    6.02    5.98
     72          QI    5.95    5.95    5.95    5.93     5.9    5.87    5.84    5.76    5.72    5.64
     73          QI     5.7    5.76    5.62    5.65    5.74    5.84    5.95     4.5    4.58    5.32
     74          QI    5.41    5.43    4.49    5.32    5.36    5.44    5.48     5.6    5.67    4.68
     75          QI    4.56    4.55    4.53    4.82    4.84    4.81    4.81    4.83    4.82     4.6
     76          QI    4.75    4.82    4.66    4.85    4.81    4.78    4.77    4.75    4.73    4.72
     77          QI    4.69    4.64    4.62    4.61    4.63    4.67    4.71    4.72    4.71    4.66
     78          QI    4.61    4.53    4.43    4.39    4.37    4.36    4.33    4.33    4.28    4.26
     79          QI     4.2    4.17    4.13    4.07    4.03       4    3.96    3.92    3.86     3.8
     80          QI    3.75    3.71    3.65     3.6    3.56    3.51    3.45    3.42    3.39    2.43
     81          QI    2.49    2.39    2.38    3.55    3.52    3.43    2.35     2.4    2.46    3.65
     82          QI    3.61     3.3    2.37     2.4    2.46    3.65    3.59    3.59    3.61    3.67
     83          QI    2.48    2.53    2.53    2.47    2.41    2.36    2.32    2.28    2.25    2.22
     84          QI    2.19    2.16    2.14    2.12    2.09    2.07    2.04    2.02       2    1.97
     85          QI    1.94    1.91    1.88    1.85     1.8    1.72    1.63    1.56    1.58    1.65
     86          QI    1.71    1.72    1.67    1.57    1.49    1.36    1.49    1.56    1.58    1.57
     87          QI    1.48    1.35    1.21    1.15    1.13    1.13    1.13    1.13    1.13    1.13
     88          QI    1.13    1.13    1.12    1.14    1.17     1.2    1.23    1.22    1.18    1.09
     89          QI     1.1    1.11    1.13    1.18    1.26    1.06     1.1    1.08    1.06    1.05
     90          QI    1.06    1.05    1.04    1.02    1.01       1    0.98    0.97    0.96    0.95
     91          QI    0.94    0.93    0.92    0.91     0.9    0.89    0.88    0.87    0.86    0.86
     92          QI    0.85    0.84    0.84    0.83    0.82    0.81     0.8     0.8    0.79    0.78
     93          QI    0.78    0.77    0.76    0.76    0.75    0.74    0.73    0.73    0.72    0.72
     94          QI    0.72    0.71    0.71    0.71     0.7     0.7    0.69    0.69    0.68    0.67
     95          QI    0.67    0.66    0.66    0.65    0.65    0.64    0.64    0.63    0.63    0.62
     96          QI    0.62    0.61    0.61     0.6     0.6    0.59    0.59    0.59    0.58    0.58
     97          QI    0.57    0.57    0.56    0.56    0.56    0.55    0.55    0.54    0.54    0.53
     98          QI    0.53    0.53    0.52    0.52    0.52    0.51    0.51     0.5     0.5     0.5
     99          QI    0.49    0.49    0.48    0.48    0.48    0.47    0.47    0.47    0.46    0.46
    100          QI    0.45    0.45    0.45    0.44    0.44    0.44    0.43    0.43    0.43    0.42
    101          QI    0.42    0.42    0.41    0.41    0.41     0.4     0.4     0.4     0.4    0.39
    102          QI    0.39    0.39
                 *
1                                        HEC-1 INPUT                                          PAGE  3

   LINE          ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10


    103          KK    SB01   BASIN
    104          BA   0.112
    105          LG    0.25    0.25    4.10    0.55      45
    106          UC   0.318   0.253
    107          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
    108          UA     100
                 *


    109          KK    CP-1 COMBINE
    110          KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
    111          HC       2
                 *


    112          KK    R1-2   ROUTE
    113          KM        MILLER RD CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
    114          RK    2104  0.0015   0.03             TRAP      66       4
                 *


    115          KK    SB02   BASIN
    116          BA   0.157
    117          LG    0.15    0.25    4.15    0.58      56
    118          UC   0.328   0.240
    119          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
    120          UA     100
                 *
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 121          KK    CP-2 COMBINE
 122          KM        SR 101L FREEWAY AND 76TH ST (MILLER RD CHANNEL)
 123          HC       2
              *

 124          KK    R2-3   ROUTE
 125          KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
 126          RK    1260  0.0015   0.03             TRAP      92       4
              *

 127          KK    SB03   BASIN
 128          BA   0.048
 129          LG    0.15    0.25    4.50    0.47      55
 130          UC   0.364   0.294
 131          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 132          UA     100
              *

 133          KK    CP-3 COMBINE
 134          KM        MAYO BLVD AND 76TH ST (MILLER RD CHANNEL)
 135          HC       2
              *
1                                        HEC-1 INPUT                                          PAGE  4

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 136          KK    R3-5   ROUTE
 137          KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
 138          RK    2396  0.0015   0.03             TRAP      98       4
              *

 139          KK    SB04   BASIN
 140          BA   0.144
 141          LG    0.14    0.25    4.60    0.44      61
 142          UC   0.305   0.199
 143          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 144          UA     100
              *

 145          KK    R4-5   ROUTE
 146          KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 147          RK    2005  0.0013   0.03             TRAP      39       4
              *

 148          KK    SB05   BASIN
 149          BA   0.126
 150          LG    0.22    0.25    4.50    0.44      48
 151          UC   0.327   0.226
 152          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 153          UA     100
              *

 154          KK    CP-5 COMBINE
 155          KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 156          HC       3
              *

 157          KK    R5-6   ROUTE
 158          KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 159          RK    1550  0.0015   0.03             TRAP     188       4
              *

 160          KK    SB06   BASIN
 161          BA   0.136
 162          LG    0.16    0.25    4.55    0.45      53
 163          UC   0.321   0.246
 164          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 165          UA     100
              *

 166          KK    CP-6 COMBINE
 167          KM        PRINCESS BLVD AND SCOTTSDALE RD (PRINCESS BLVD CHANNEL)
 168          HC       2
              *
1                                        HEC-1 INPUT                                          PAGE  5

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 169          KK    SB07   BASIN
 170          BA   0.074
 171          LG    0.21    0.25    4.00    0.58      49
 172          UC   0.206   0.113
 173          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 174          UA     100
              *

 175          KK    R7-8   ROUTE
 176          KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 177          RK    2778  0.0014   0.03             TRAP      46       4
              *
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178          KK    SB08   BASIN
179          BA   0.224
180          LG    0.14    0.25    4.10    0.59      59
181          UC   0.338   0.195
182          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
183          UA     100
             *

184          KK    CP-8 COMBINE
185          KM        SR 101L FREEWAY AND HAYDEN ROAD (HAYDEN ROAD NORTH CHANNEL)
186          HC       2
             *

187          KK    R8-9   ROUTE
188          KM        HAYDEN ROAD NORTH CHANNEL FROM HAYDEN ROAD TO BASIN 53R
189          RK    1250  0.0013   0.03             TRAP      67       4
             *

190          KK    SB09   BASIN
191          BA   0.052
192          LG    0.15    0.25    4.00    0.61      55
193          UC   0.254   0.230
194          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
195          UA     100
             *

196          KK    CP-9 COMBINE
197          KM        SR 101L FREEWAY AND BASIN 53R (HAYDEN ROAD NORTH CHANNEL)
198          HC       2
             *

199          KK   PWRCH
200          KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
201          KM        FROM PINNACLE PEAK SOUTH ADMS 100-YR, 24-HR FLO-2D MODEL
202          BA     7.0
203          QI       0       0       0       0       0       0       0       0       0       0
204          QI       0       0       0       0       0       0       0       0       0       0
205          QI       0       0       0       0       0       0       0       0       0       0
206          QI       0       0       0       0       0       0       0       0       0       0
207          QI       0       0       0       0       0       0       0       0       0       0
208          QI       0       0       0       0       0       0       0       0       0       0
```



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LINE         ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

209          QI       0       0       0       0       0       0       0       0       0       0
210          QI       0       0       0       0       0       0       0       0       0       0
211          QI       0       0       0       0       0       0       0       0       0       0
212          QI       0       0       0       0       0       0       0       0       0       0
213          QI       0       0       0       0       0       0       0       0       0       0
214          QI       0       0       0       0       0       0       0       0       0       0
215          QI       0       0       0       0       0       0       0       0       0       0
216          QI       0       0       0       0       0       0       0       0       0       0
217          QI       0       0       0       0       0       0       0       0       0       0
218          QI       0       0       0       0       0       0       0       0       0       0
219          QI       0       0       0       0       0       0       0       0       0       0
220          QI       0       0       0       0       0       0       0       0       0       0
221          QI       0       0       0       0       0       0       0       0       0       0
222          QI       0       0       0       0       0       0       0       0       0       0
223          QI       0       0       0       0       0       0       0       0       0       0
224          QI       0       0       0       0       0       0       0       0       0       0
225          QI       0       0       1       1       1       1       1       1       1       1
226          QI       1       2       2       2       3       3       5       9      18      28
227          QI      43      50      65      87     194     495     872     967    1051    1095
```

```
1****************************************
****************************************
 *                                      *                                                    *
  *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)  *                                                    *    U.S. ARMY CORPS OF ENGINEERS
  *
 *               JUN   1998             *                                                    *    HYDROLOGIC ENGINEERING CENTER
  *
 *            VERSION 4.1               *                                                    *          609 SECOND STREET
  *
 *                                      *                                                    *       DAVIS, CALIFORNIA 95616
  *
 *  RUN DATE   12SEP14  TIME  11:47:04  *                                                    *           (916) 756-1104
  *
 *                                      *                                                    *
  *
 ****************************************
****************************************
```

```
X     X  XXXXXXX   XXXXX           X
X     X  X        X     X         XX
X     X  X        X                X
XXXXXXX  XXXX     X        XXXXX   X
X     X  X        X                X
X     X  X        X     X          X
X     X  XXXXXXX   XXXXX          XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

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          LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

            1           ID        CROSS ROADS EAST DRAINAGE ALTERNATIVES
            2           ID        RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
            3           ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
            4           ID       100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
            5           ID           PIMA ROAD FLOWS)
            6           ID        PHASE 2 BASIN - SIDESLOPES STEEPENED FROM ~5:1 TO 3:1
            7           ID
            8           ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
            9           ID       MODELERS: RK, MW
           10           ID
           11           IT       3       0       0    1000
           12           IO       3
                        *DIAGRAM
                        *
           13           JD   2.755  0.0001
           14           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
           15           PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
           16           PC   0.962   0.972   0.983   0.991   1.000
           17           JD   2.738  0.5000
           18           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
           19           PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
           20           PC   0.962   0.972   0.983   0.991   1.000
           21           JD   2.686     2.8
           22           PC   0.000   0.009   0.016   0.025   0.034   0.042   0.051   0.059   0.068   0.077
           23           PC   0.088   0.101   0.121   0.164   0.253   0.451   0.694   0.836   0.900   0.938
           24           PC   0.950   0.963   0.975   0.988   1.000
           25           JD   2.540    16.0
           26           PC   0.000   0.015   0.020   0.030   0.048   0.063   0.076   0.090   0.105   0.119
           27           PC   0.135   0.152   0.175   0.222   0.304   0.472   0.670   0.796   0.868   0.912
           28           PC   0.946   0.960   0.973   0.987   1.000
           29           JD   2.232    90.0
           30           PC   0.000   0.021   0.035   0.051   0.071   0.087   0.105   0.125   0.143   0.160
           31           PC   0.179   0.201   0.232   0.281   0.364   0.500   0.658   0.773   0.841   0.888
           32           PC   0.927   0.945   0.964   0.982   1.000
                        *

           33           KK  76THST
           34           KM        76TH STREET CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
           35           KM        100-YR, 6-HR FLO-2D MODEL (XS 98)
           36           BA    0.24
           37           QI       0       0       0       0       0       0       0       0       0
           38           QI       0       0       0       0       0       0       0       0       0
           39           QI       0       0       0       0       0       0       0       0       0
           40           QI       0       0       0       0       0       0       0       0       0
           41           QI       0       0       0       0       0       0       0       0       0
           42           QI       0       0    0.01       0       0       0       0       0       0
           43           QI    0.01       0       0    0.01    0.01    0.01    0.02    0.03    0.05    0.07
```

```
   44          QI    0.09    0.16    0.27    0.42    0.63     0.9    2.34    3.89    7.48   10.26
   45          QI    12.7   14.07   17.67   27.43   38.25   42.26   42.56   42.35   43.77   48.39
   46          QI   49.92   51.19   50.98   50.52   49.38    48.6   44.64   42.62   39.72   36.96
   47          QI   34.49   32.83   31.02    29.6   28.15   26.72   25.66   24.48   23.53   22.56
   48          QI   21.55   20.83   19.46   20.34   18.32   17.51   16.55   16.22   15.59   15.15
   49          QI   14.81   14.23   13.72   13.18   13.21   12.84   12.36   12.34   12.16      12
   50          QI   11.63   11.31   11.37   11.23   11.13   10.56   10.63   10.27    9.98    9.86
   51          QI    9.58    9.02    9.34    8.67    8.51    8.32    8.08    7.75    7.43    7.37
1                                        HEC-1 INPUT                                        PAGE  2

  LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

   52          QI    7.29    7.19    7.08       7    6.89    6.73    6.55     6.4    6.23    6.02
   53          QI    5.83    5.66    5.06    4.57    4.75    4.76    4.65    4.55    4.44    4.32
   54          QI    4.23    4.15    4.05    3.95    3.87    3.81    3.68    3.54    3.48    3.42
   55          QI     3.4    2.42    2.38     3.4    3.61    3.55    3.49    2.89    2.36    2.36
   56          QI    2.36    2.37    2.44    3.67    2.44    2.51    2.49    2.47    2.42    2.37
   57          QI    2.32    2.29    2.26    2.23    2.19    2.16    2.13     2.1    2.07    2.05
   58          QI    2.02    1.99    1.96    1.91    1.86    1.83    1.78    1.69    1.58    1.54
   59          QI    1.58    1.65    1.69    1.67    1.59     1.5    1.36    1.41    1.51    1.54
   60          QI    1.52    1.35    1.43    1.37    1.25    1.21     1.2     1.2    1.23    1.28
   61          QI    1.31    1.34    1.35    1.35    1.34    1.34    1.33    1.33    1.26    1.09
   62          QI    1.09    1.11    1.16    1.22    1.06    1.07    1.15    1.06    1.05    1.05
   63          QI    1.04    1.02    1.01       1    0.98    0.97    0.96    0.95    0.94    0.92
   64          QI    0.91     0.9    0.89    0.88    0.87    0.86    0.85    0.85    0.84    0.84
   65          QI    0.83    0.82    0.81     0.8    0.79    0.79    0.78    0.77    0.77    0.76
   66          QI    0.76    0.75    0.74    0.74    0.73    0.72    0.71    0.71     0.7    0.69
   67          QI    0.69    0.68    0.68    0.67    0.67    0.67    0.66    0.66    0.65    0.65
   68          QI    0.65    0.64    0.64    0.63    0.63    0.62    0.62    0.62    0.61    0.61
   69          QI     0.6     0.6    0.59    0.59    0.58    0.58    0.57    0.57    0.56    0.56
   70          QI    0.55    0.55    0.54    0.54    0.54    0.53    0.53    0.52    0.52    0.52
   71          QI    0.51    0.51     0.5     0.5     0.5    0.49    0.49    0.48    0.48    0.48
   72          QI    0.47    0.47    0.46    0.46    0.46    0.45    0.45    0.45    0.44    0.44
   73          QI    0.44
               *

   74          KK    SB01   BASIN
   75          BA   0.112
   76          LG    0.25    0.25    4.10    0.55      45
   77          UC   0.317   0.252
   78          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
   79          UA     100
               *

   80          KK    CP-1 COMBINE
   81          KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
   82          HC       2
               *

   83          KK    R1-2   ROUTE
   84          KM        MILLER RD CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
   85          RK    2104  0.0015   0.03             TRAP      66       4
               *

   86          KK    SB02   BASIN
   87          BA   0.157
   88          LG    0.15    0.25    4.15    0.58      56
   89          UC   0.324   0.237
   90          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
   91          UA     100
               *
1                                        HEC-1 INPUT                                        PAGE  3

  LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


   92          KK    CP-2 COMBINE
   93          KM        SR 101L FREEWAY AND 76TH ST (MILLER RD CHANNEL)
   94          HC       2
               *

   95          KK    R2-3   ROUTE
   96          KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
   97          RK    1260  0.0015   0.03             TRAP      92       4
               *

   98          KK    SB03   BASIN
   99          BA   0.048
  100          LG    0.15    0.25    4.50    0.47      55
  101          UC   0.360   0.290
  102          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
  103          UA     100
               *

  104          KK    CP-3 COMBINE
  105          KM        MAYO BLVD AND 76TH ST (MILLER RD CHANNEL)
  106          HC       2
               *

  107          KK    R3-5   ROUTE
  108          KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
  109          RK    2396  0.0015   0.03             TRAP      98       4
               *
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 110          KK    SB04   BASIN
 111          BA   0.144
 112          LG    0.14    0.25    4.60    0.44      61
 113          UC   0.301   0.196
 114          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 115          UA     100
              *

 116          KK    R4-5   ROUTE
 117          KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 118          RK    2005  0.0013   0.03             TRAP      39       4
              *

 119          KK    SB05   BASIN
 120          BA   0.126
 121          LG    0.22    0.25    4.50    0.44      48
 122          UC   0.326   0.224
 123          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 124          UA     100
              *
1                                        HEC-1 INPUT                                          PAGE  4

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 125          KK    CP-5 COMBINE
 126          KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 127          HC       3
              *

 128          KK    R5-6   ROUTE
 129          KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 130          RK    1550  0.0015   0.03             TRAP     188       4
              *

 131          KK    SB06   BASIN
 132          BA   0.136
 133          LG    0.16    0.25    4.55    0.45      53
 134          UC   0.318   0.243
 135          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 136          UA     100
              *

 137          KK    CP-6 COMBINE
 138          KM        PRINCESS BLVD AND SCOTTSDALE RD (PRINCESS BLVD CHANNEL)
 139          HC       2
              *

 140          KK    SB07   BASIN
 141          BA   0.074
 142          LG    0.21    0.25    4.00    0.58      49
 143          UC   0.204   0.113
 144          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 145          UA     100
              *

 146          KK    R7-8   ROUTE
 147          KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 148          RK    2778  0.0014   0.03             TRAP      46       4
              *

 149          KK    SB08   BASIN
 150          BA   0.224
 151          LG    0.14    0.25    4.10    0.59      59
 152          UC   0.333   0.193
 153          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 154          UA     100
              *

 155          KK    CP-8 COMBINE
 156          KM        SR 101L FREEWAY AND HAYDEN ROAD (HAYDEN ROAD NORTH CHANNEL)
 157          HC       2
              *
1                                        HEC-1 INPUT                                          PAGE  5

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 158          KK    R8-9   ROUTE
 159          KM        HAYDEN ROAD NORTH CHANNEL FROM HAYDEN ROAD TO BASIN 53R
 160          RK    1250  0.0013   0.03             TRAP      67       4
              *

 161          KK    SB09   BASIN
 162          BA   0.052
 163          LG    0.15    0.25    4.00    0.61      55
 164          UC   0.251   0.227
 165          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 166          UA     100
              *

 167          KK    CP-9 COMBINE
 168          KM        SR 101L FREEWAY AND BASIN 53R (HAYDEN ROAD NORTH CHANNEL)
 169          HC       2
```

```
             *
 170         KK  PIMACH
 171         KM        PIMA ROAD CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
 172         KM        100-YR, 6-HR FLO-2D MODEL (XS 183)
 173         BA    12.3
 174         QI       0       0       0       0       0       0       0       0       0       0
 175         QI       0       0       0       0       0       0       0       0       0       0
 176         QI       0       0       0       0       0       0       0       0       0       0
 177         QI       0       0       0       0       0       0       0       0       0       0
 178         QI       0       0       0       0       0       0       0       0       0       0
 179         QI       0       0       0       0       0       0       0       0       0       0
 180         QI       0       0       0       0       0       0    0.01    0.01    0.01    0.01
 181         QI    0.02    0.06    0.12    0.19    0.29    0.43    1.62    3.73    6.28   13.12
 182         QI   19.17   18.99   20.23   23.44    37.1    65.6  118.28  536.07  740.49  784.96
 183         QI  795.73  792.08  785.39  784.16  826.86  835.79  839.66  856.83  890.85 1074.56
 184         QI 1522.8 1924.46 2612.09 2761.62 3125.86 3450.94 3565.35 3786.18 3953.42 4005.23
 185         QI 4017.2 3956.42 3828.66 3704.96 3584.74 3457.47 3327.91 3209.52  3068.2 2902.83
 186         QI 2698.1 2519.92 2353.91 2218.47 2091.25 1970.95 1868.74 1749.96 1656.67 1564.23
 187         QI 1473.2 1392.42 1326.33 1253.44 1190.18 1121.44 1068.33  1012.4  961.04  907.15
 188         QI    859  817.16  777.05  741.05  708.67  676.64  650.15     620  595.68  569.46
 189         QI  552.9  528.08  508.87  488.92  471.15  456.27  440.43  424.35  410.65  396.26
 190         QI 384.05   374.5  363.39  351.31  341.86  332.93  325.13  317.66  308.91  300.01
 191         QI 291.96  282.64   276.3  270.44  264.03  257.52  251.57  244.49  238.62  232.96
 192         QI 227.38  223.46  217.91  210.73  207.24  202.22  197.15  192.34  189.04  184.57
 193         QI 178.93   173.7  170.95   166.9  162.57  158.97  155.56  151.97  149.46  146.92
 194         QI  143.8  141.62  138.05  136.38   130.4  128.32  123.98  120.84   117.3  114.09
 195         QI 113.28  110.56  108.01  104.98  103.24  100.82   98.69   97.77    95.8   93.56
 196         QI   92.8   91.19   88.35   86.44   84.49   82.18    80.7   78.31   76.07   74.85
 197         QI  73.81   73.17   71.94   70.66   69.66   69.54   67.38   67.61   65.17   62.74
 198         QI  61.41   60.29   61.36   59.06   56.97   56.32   55.01   53.88   52.91   52.23
 199         QI  52.11   50.68   50.15    49.6   48.36    47.6   46.82    46.4   45.54   44.69
 200         QI  43.67   43.34   42.77    41.9   41.04   40.86   40.39   39.72   38.49   38.26
 201         QI  37.01   36.83   36.64   35.35   35.64   34.94   35.17    33.9    33.7   33.71
 202         QI  33.78   32.55   32.35   31.97   31.61   31.38   31.21   30.84   30.41      30
 203         QI  29.61   29.27   28.72   28.17   27.62   27.26   26.95   26.26   25.93   25.66
 204         QI   25.7   25.62    25.7   25.39   24.94   24.57   24.28   23.92   23.55   22.97
 205         QI   22.7   22.79   22.23   21.94   21.87    21.6    21.3   21.18   21.05   20.75
1                                          HEC-1 INPUT                                        PAGE  6

LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 206         QI  20.64    20.4   20.19   19.93    19.9   19.01   18.72    18.7   18.37   18.27
 207         QI   18.1   18.05    17.3   17.39   17.27   17.09   16.86   16.81   16.73    16.6
 208         QI  16.55   16.33   16.05   15.82   15.63   15.43   15.38   15.42   15.17   14.98
 209         QI  14.82   14.78   14.76   14.57   14.24   14.01   13.85   13.94   13.77   13.77
 210         QI   12.6
             *
 211         KK  PIMAIN
 212         KM    BYPASS ALONG SE SIDE OF BASIN 53R TO ADOT CULVERT NEAR UNION HILLS DR
 213         DT  PIMABY
 214         DI       0    1000   10000
 215         DQ       0    1000    1000
             *
 216         KK   PWRCH
 217         KM        POWERLINE CHANNEL HYDROGRAPH FROM FLO-2D
 218         KM        100-YR, 6-HR FLO-2D MODEL (XS 107)
 219         BA     7.0
 220         QI       0       0       0       0       0       0       0       0       0       0
 221         QI       0       0       0       0       0       0       0       0       0       0
 222         QI       0       0       0       0       0       0       0       0       0       0
 223         QI       0       0       0       0       0       0       0       0       0       0
 224         QI       0       0       0       0       0       0       0       0       0       0
 225         QI       0       0       0       0       0       0       0       0       0       0
 226         QI       0       0       0       0       0       0       0       0       0       0
 227         QI       0       0    0.03    0.05    0.08    0.11    0.58    1.13    1.62    2.35
 228         QI   17.74   24.61   32.43    41.1      46   50.91  131.64  355.54   399.7  422.85
 229         QI  433.53  433.73  422.44  409.12  397.71  391.08  388.91   381.3  371.02  361.84
 230         QI  359.13  372.83  390.54  400.38  398.98  391.64  390.01  419.65  505.31  577.51
 231         QI  616.52   654.6  689.73   713.7  727.81  737.85  739.81  733.87  725.12  714.86
 232         QI  699.64  690.12  677.33  666.62  655.55  645.41  637.05  633.81  630.71  627.88
 233         QI  621.52  616.56  611.87  606.82  603.07  599.48  598.68  597.46  596.17  595.28
 234         QI  594.12  590.26  586.44  582.19  576.08  570.84  564.08  555.83  546.74  537.07
 235         QI  525.86  516.15  505.83  495.17  484.12  472.45  462.13  451.31  441.83  429.35
 236         QI  419.09  410.42  401.94   394.3  384.43  375.56  366.51  358.68  350.24   342.9
 237         QI     335  326.95  320.66  313.52  306.21  299.42  294.88  279.13  274.41  272.37
 238         QI   270.9  258.49  256.83  252.87  239.19  235.68  236.09  226.97  221.44  220.04
 239         QI  211.13  213.73  201.19  203.88   194.6  196.81  183.61  188.47  178.11  181.39
 240         QI  172.83  170.18  168.28  163.33  169.45  155.83  153.79  154.42  148.45  145.47
 241         QI  146.04  140.33  138.63  140.19  133.49  130.23  135.24  122.64  121.61  121.05
 242         QI   120.9  117.47  115.13  113.28   116.6  110.32  107.03  105.71  109.02  101.22
 243         QI   99.25   97.86   99.48  103.28   92.95    90.9   89.54   87.83   91.59   84.19
 244         QI   82.93    81.6   80.75   79.42   82.41   77.68   75.61   74.44   74.51   72.29
 245         QI    79.3   70.93   70.15   69.48   68.44   67.58   66.72   72.25   65.21   64.67
 246         QI    63.5   62.83    62.6   62.06   61.31   64.86   60.25   59.05   58.48   55.87
 247         QI   56.01   55.72    54.8   54.06   53.38   51.64   51.71   50.75   49.83    49.2
 248         QI   48.75   46.82   45.85   45.55    47.8   43.99   43.77   43.28   42.64   42.31
 249         QI   42.04   41.65   41.41   40.69    40.3   40.09   39.93   39.53   42.18   38.64
 250         QI   37.99   37.44   37.13   36.96    36.8   36.19   35.56   35.28   34.71    33.9
 251         QI   33.42   33.14   32.73   39.25   32.39   31.56   30.95   30.66    30.3      30
 252         QI   29.22   28.72   29.47   28.39   28.32   28.13   28.24   28.22   27.67   27.34
 253         QI   27.15   27.03   26.51   25.94   25.77   31.57   25.38   25.06   25.04   24.52
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 254          QI   24.31   24.43   24.27   23.35   23.93    23.1   22.66   22.71   22.57   22.79
 255          QI   22.09   21.91   21.94   21.84   21.71   21.66   21.87   21.85   21.36   21.06
 256          QI   20.95
              *
1                                        HEC-1 INPUT                                        PAGE  7

         LINE         ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10


 257          KK  BINFLO
 258          KM         TOTAL INFLOW INTO BASIN 53R.
 259          HC       3
              *

 260          KK   BASIN
 261          KM    BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
 262          KM    ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
 263          KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
 264          KM        THE OUTLET PIPES ARE INLET CONTROLLED.
 265          RS       1    STOR       0
 266          SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
 267          SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
 268          SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
 269          SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
 270          SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
 271          SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
              *

 272          KK  BSNRT1
 273          KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
 274          KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
 275          KM        EXCESS CAPACITY.
 276          RK     550  0.0052   0.024            CIRC     7.0
              *

 277          KK  BSNRT2
 278          KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BASIN 53R OUTFAL
 279          RK    1200  0.0077   0.013            CIRC     7.0
              *

 280          KK  PIMABY
 281          KM    RETRIEVE PIMA BYPASS CHANNEL HYDROGRAPH
 282          DR  PIMABY
              *

 283          KK  PMB-RT
 284          KM        UNION HILLS DR CHANNEL FROM SR 101L FREEWAY TO 82ND ST
 285          RK    3157  0.0082   0.013       0    TRAP      24       4
              *

 286          KK    SB10   BASIN
 287          BA   0.040
 288          LG    0.15    0.25    4.25    0.55      55
 289          UC   0.230   0.159
 290          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 291          UA     100
              *
1                                        HEC-1 INPUT                                        PAGE  8

         LINE         ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10


 292          KK   CP-10 COMBINE
 293          KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
 294          HC       3
              *

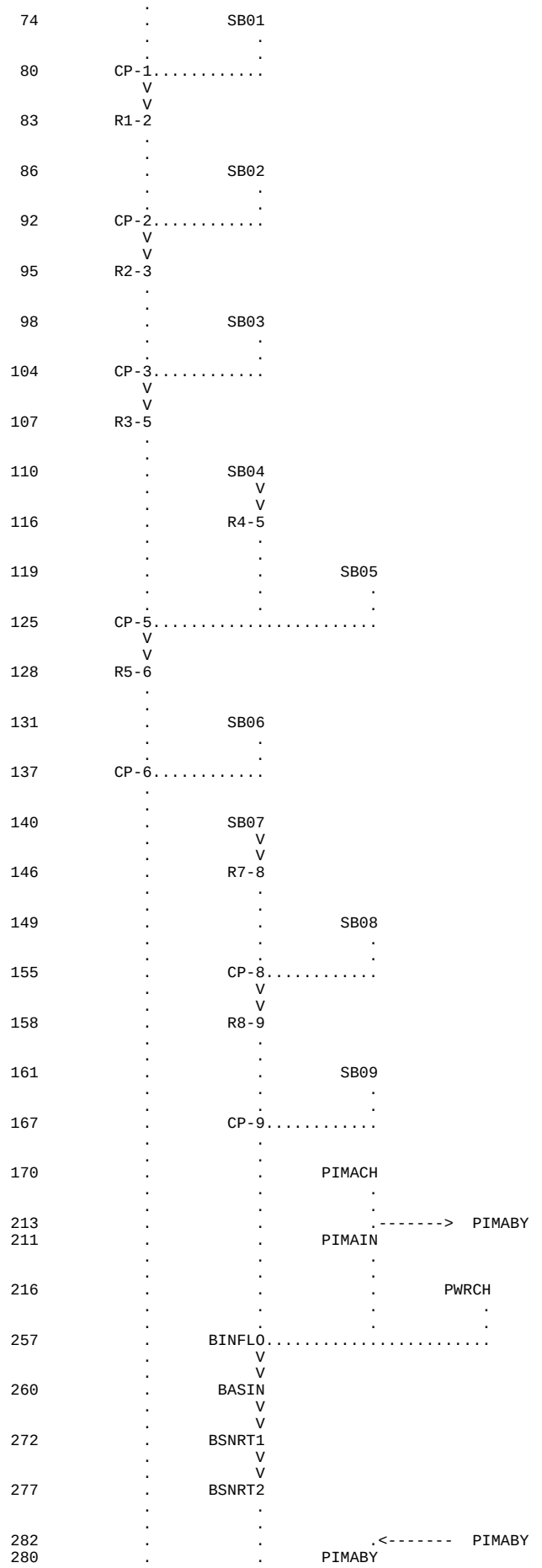
 295          KK  R10-11   ROUTE
 296          KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
 297          RK    1277  0.0014   0.03             TRAP     220       4
              *

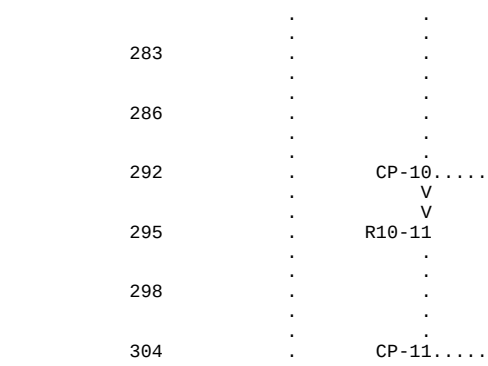
 298          KK    SB11   BASIN
 299          BA   0.071
 300          LG    0.15    0.25    4.15    0.58      55
 301          UC   0.292   0.229
 302          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 303          UA     100
              *

 304          KK   CP-11 COMBINE
 305          KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
 306          HC       2
              *
 307          ZZ
1
              SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

    NO.     (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

     33     76THST
              .
```


74 SB01
80 CP-1
83 R1-2
86 SB02
92 CP-2
95 R2-3
98 SB03
104 CP-3
107 R3-5
110 SB04
116 R4-5
119 SB05
125 CP-5
128 R5-6
131 SB06
137 CP-6
140 SB07
146 R7-8
149 SB08
155 CP-8
158 R8-9
161 SB09
167 CP-9
170 PIMACH
213 PIMABY
211 PIMAIN
216 PWRCH
257 BINFLO
260 BASIN
272 BSNRT1
277 BSNRT2
282 PIMABY
280 PIMABY



(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

1**********

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* * *

* FLOOD HYDROGRAPH PACKAGE (HEC-1) * * U.S. ARMY CORPS OF ENGINEERS

* JUN 1998 * * HYDROLOGIC ENGINEERING CENTER

* VERSION 4.1 * * 609 SECOND STREET

* * * DAVIS, CALIFORNIA 95616

* RUN DATE 12SEP14 TIME 11:47:04 * * (916) 756-1104

* * *

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****

CROSS ROADS EAST DRAINAGE ALTERNATIVES
RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
PIMA ROAD FLOWS)
PHASE 2 BASIN - SIDESLOPES STEEPENED FROM ~5:1 TO 3:1

PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
MODELERS: RK, MW

12 IO OUTPUT CONTROL VARIABLES
IPRNT 3 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
NMIN 3 MINUTES IN COMPUTATION INTERVAL
IDATE 1 0 STARTING DATE
ITIME 0000 STARTING TIME
NQ 1000 NUMBER OF HYDROGRAPH ORDINATES
NDDATE 3 0 ENDING DATE
NDTIME 0157 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.05 HOURS
TOTAL TIME BASE 49.95 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

13 JD INDEX STORM NO. 1
STRM 2.76 PRECIPITATION DEPTH
TRDA 0.00 TRANSPOSITION DRAINAGE AREA

14 PI PRECIPITATION PATTERN
0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01
0.01 0.02 0.02 0.08 0.16 0.46 0.08 0.02 0.02 0.01
0.01 0.01 0.01 0.01

17 JD INDEX STORM NO. 2
STRM 2.74 PRECIPITATION DEPTH
TRDA 0.50 TRANSPOSITION DRAINAGE AREA

18 PI PRECIPITATION PATTERN

```
                    0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                    0.01      0.02      0.02      0.08      0.16      0.46      0.08      0.02      0.02      0.01
                    0.01      0.01      0.01      0.01

   21 JD          INDEX STORM NO. 3
                        STRM        2.69  PRECIPITATION DEPTH
                        TRDA        2.80  TRANSPOSITION DRAINAGE AREA

   22 PI            PRECIPITATION PATTERN
                    0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                    0.01      0.02      0.04      0.09      0.20      0.24      0.14      0.06      0.04      0.01
                    0.01      0.01      0.01      0.01

   25 JD          INDEX STORM NO. 4
                        STRM        2.54  PRECIPITATION DEPTH
                        TRDA       16.00  TRANSPOSITION DRAINAGE AREA

   26 PI            PRECIPITATION PATTERN
                    0.01      0.00      0.01      0.02      0.02      0.01      0.01      0.01      0.01      0.02
                    0.02      0.02      0.05      0.08      0.17      0.20      0.13      0.07      0.04      0.03
                    0.01      0.01      0.01      0.01

   29 JD          INDEX STORM NO. 5
                        STRM        2.23  PRECIPITATION DEPTH
                        TRDA       90.00  TRANSPOSITION DRAINAGE AREA

   30 PI            PRECIPITATION PATTERN
                    0.02      0.01      0.02      0.02      0.02      0.02      0.02      0.02      0.02      0.02
                    0.02      0.03      0.05      0.08      0.14      0.16      0.12      0.07      0.05      0.04
                    0.02      0.02      0.02      0.02


 *** *** ***
```

	(INCHES)	0.582	0.664	0.728	0.728
	(AC-FT)	7.	9.	9.	9.

CUMULATIVE AREA = 0.24 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION 76THST
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 51.	4.55	15.	4.	2.	2.
	(INCHES)	0.582	0.664	0.728	0.728
	(AC-FT)	7.	9.	9.	9.

CUMULATIVE AREA = 0.24 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION 76THST
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 51.	4.55	15.	4.	2.	2.
	(INCHES)	0.582	0.664	0.728	0.728
	(AC-FT)	7.	9.	9.	9.

CUMULATIVE AREA = 0.24 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT 76THST

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 51.	4.55	15.	4.	2.	2.
	(INCHES)	0.582	0.664	0.728	0.728
	(AC-FT)	7.	9.	9.	9.

CUMULATIVE AREA = 0.24 SQ MI

*** *** *** *

15.	58.	129.	172.	167.	153.	134.	112.	91.	75.
61.	50.	41.	34.	28.	23.	19.	15.	12.	10.
8.	7.	6.	5.	4.	3.	3.	2.	2.	1.

*** *** *** *** ***

HYDROGRAPH AT STATION SB01
TRANSPOSITION AREA 0.0 SQ MI

TOTAL RAINFALL = 2.76, TOTAL LOSS = 0.59, TOTAL EXCESS = 2.17

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 326.	0.95	26.	7.	3.	3.
		(INCHES) 2.159	2.159	2.159	2.159
		(AC-FT) 13.	13.	13.	13.

CUMULATIVE AREA = 0.11 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB01
TRANSPOSITION AREA 0.5 SQ MI

TOTAL RAINFALL = 2.74, TOTAL LOSS = 0.59, TOTAL EXCESS = 2.15

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 323.	0.95	26.	6.	3.	3.
		(INCHES) 2.143	2.143	2.143	2.143
		(AC-FT) 13.	13.	13.	13.

CUMULATIVE AREA = 0.11 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB01
TRANSPOSITION AREA 2.8 SQ MI

TOTAL RAINFALL = 2.69, TOTAL LOSS = 0.63, TOTAL EXCESS = 2.05

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 287.	1.00	25.	6.	3.	3.
		(INCHES) 2.044	2.044	2.044	2.044
		(AC-FT) 12.	12.	12.	12.

CUMULATIVE AREA = 0.11 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB01
TRANSPOSITION AREA 16.0 SQ MI

TOTAL RAINFALL = 2.54, TOTAL LOSS = 0.68, TOTAL EXCESS = 1.86

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 241.	1.00	22.	6.	3.	3.
		(INCHES) 1.850	1.850	1.850	1.850
		(AC-FT) 11.	11.	11.	11.

CUMULATIVE AREA = 0.11 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB01
TRANSPOSITION AREA 90.0 SQ MI

TOTAL RAINFALL = 2.23, TOTAL LOSS = 0.72, TOTAL EXCESS = 1.52

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 182.	1.00	18.	5.	2.	2.
		(INCHES) 1.510	1.510	1.510	1.510
		(AC-FT) 9.	9.	9.	9.

CUMULATIVE AREA = 0.11 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT SB01

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 324.	0.95	26.	6.	3.	3.
		(INCHES) 2.146	2.146	2.146	2.146
		(AC-FT) 13.	13.	13.	13.

CUMULATIVE AREA = 0.11 SQ MI

*** *** *** *

CUMULATIVE AREA = 0.35 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-1
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 182.	1.00	29.	9.	4.	4.
		(INCHES) 0.776	0.927	0.977	0.977
		(AC-FT) 15.	17.	18.	18.

CUMULATIVE AREA = 0.35 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT CP-1

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 323.	0.95	37.	11.	5.	5.
		(INCHES) 0.982	1.128	1.178	1.178
		(AC-FT) 18.	21.	22.	22.

CUMULATIVE AREA = 0.35 SQ MI

*** *** *** *

CUMULATIVE AREA = 0.35 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.17	1.56	3.00	701.33	321.94	63.70	1.18	3.88

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2212E+02 EXCESS=0.0000E+00 OUTFLOW=0.2223E+02 BASIN STORAGE=0.5936E-01 PERCENT ERROR=-0.8

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.17	1.56	3.00		314.95	66.00	1.18

*** *** *** *** ***

HYDROGRAPH AT STATION R1-2
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
315.	1.10	37.	11.	5.	5.
		(INCHES) 0.988	1.134	1.184	1.184
		(AC-FT) 19.	21.	22.	22.

CUMULATIVE AREA = 0.35 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.17	1.56	3.00	701.33	281.88	67.46	1.15	3.72

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2153E+02 EXCESS=0.0000E+00 OUTFLOW=0.2160E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=-0.6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.17	1.56	3.00		280.31	66.00	1.15

*** *** *** *** ***

HYDROGRAPH AT STATION R1-2
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
280.	1.10	36.	10.	5.	5.
		(INCHES) 0.955	1.100	1.149	1.149
		(AC-FT) 18.	21.	22.	22.

CUMULATIVE AREA = 0.35 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.17	1.56	3.00	701.33	239.43	66.99	1.09	3.49

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2036E+02 EXCESS=0.0000E+00 OUTFLOW=0.2044E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=-0.7

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.17	1.56	3.00		234.05	69.00	1.09

*** *** *** *** ***

HYDROGRAPH AT STATION R1-2
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 234.	1.15	34.	10.	5.	5.
		(INCHES) 0.887	1.038	1.087	1.087
		(AC-FT) 17.	19.	20.	20.

CUMULATIVE AREA = 0.35 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.17	1.56	2.91	526.00	179.26	70.19	0.98	3.16

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1834E+02 EXCESS=0.0000E+00 OUTFLOW=0.1842E+02 BASIN STORAGE=0.6416E-01 PERCENT ERROR= -0.8

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.17	1.56	3.00	178.38	69.00	0.98

*** *** *** *** ***

HYDROGRAPH AT STATION R1-2
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 178.	1.15	30.	9.	4.	4.
		(INCHES) 0.780	0.932	0.981	0.981
		(AC-FT) 15.	17.	18.	18.

CUMULATIVE AREA = 0.35 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT R1-2

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 315.	1.10	37.	11.	5.	5.
		(INCHES) 0.989	1.134	1.184	1.184
		(AC-FT) 19.	21.	22.	22.

CUMULATIVE AREA = 0.35 SQ MI

*** *** *** *

90 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES
0.0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
100.0

***

UNIT HYDROGRAPH PARAMETERS
CLARK TC= 0.32 HR, R= 0.24 HR
SNYDER TP= 0.19 HR, CP= 0.49

UNIT HYDROGRAPH
28 END-OF-PERIOD ORDINATES

21.	81.	185.	250.	242.	220.	192.	160.	129.	104.
85.	68.	55.	45.	36.	29.	24.	19.	16.	13.
10.	8.	7.	5.	4.	4.	3.	2.		

*** *** *** *** ***

HYDROGRAPH AT STATION SB02
TRANSPOSITION AREA 0.0 SQ MI

TOTAL RAINFALL = 2.76, TOTAL LOSS = 0.45, TOTAL EXCESS = 2.30

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 492.	0.95	39.	10.	5.	5.
		(INCHES) 2.292	2.292	2.292	2.292
		(AC-FT) 19.	19.	19.	19.

CUMULATIVE AREA = 0.16 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB02
TRANSPOSITION AREA 0.5 SQ MI

TOTAL RAINFALL = 2.74, TOTAL LOSS = 0.45, TOTAL EXCESS = 2.29

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 488.	0.95	38.	10.	5.	5.
		(INCHES) 2.276	2.276	2.276	2.276
		(AC-FT) 19.	19.	19.	19.

CUMULATIVE AREA = 0.16 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB02
TRANSPOSITION AREA 2.8 SQ MI

TOTAL RAINFALL = 2.69, TOTAL LOSS = 0.49, TOTAL EXCESS = 2.19

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 433.	1.00	37.	9.	4.	4.
		(INCHES) 2.183	2.183	2.183	2.183
		(AC-FT) 18.	18.	18.	18.

CUMULATIVE AREA = 0.16 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB02
TRANSPOSITION AREA 16.0 SQ MI

TOTAL RAINFALL = 2.54, TOTAL LOSS = 0.54, TOTAL EXCESS = 2.00

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 365.	1.00	34.	8.	4.	4.
		(INCHES) 1.994	1.994	1.994	1.994
		(AC-FT) 17.	17.	17.	17.

CUMULATIVE AREA = 0.16 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB02
TRANSPOSITION AREA 90.0 SQ MI

TOTAL RAINFALL = 2.23, TOTAL LOSS = 0.56, TOTAL EXCESS = 1.67

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 278.	1.00	28.	7.	3.	3.
		(INCHES) 1.660	1.660	1.660	1.660
		(AC-FT) 14.	14.	14.	14.

CUMULATIVE AREA = 0.16 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT SB02

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 489.	0.95	38.	10.	5.	5.
		(INCHES) 2.278	2.278	2.278	2.278
		(AC-FT) 19.	19.	19.	19.

CUMULATIVE AREA = 0.16 SQ MI

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HYDROGRAPH AT STATION CP-2
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 556.	1.10	66.	18.	9.	9.
		(INCHES) 1.207	1.332	1.367	1.367
		(AC-FT) 33.	36.	37.	37.

CUMULATIVE AREA = 0.51 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-2
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 422.	1.10	56.	16.	8.	8.
		(INCHES) 1.028	1.156	1.190	1.190
		(AC-FT) 28.	31.	32.	32.

CUMULATIVE AREA = 0.51 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT CP-2

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 731.	1.05	75.	20.	10.	10.
		(INCHES) 1.362	1.485	1.520	1.520
		(AC-FT) 37.	40.	41.	41.

CUMULATIVE AREA = 0.51 SQ MI

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HYDROGRAPH AT STATION R2-3
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 726.	1.10	75.	20.	10.	10.
		(INCHES) 1.373	1.495	1.529	1.529
		(AC-FT) 37.	41.	42.	42.

CUMULATIVE AREA = 0.51 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.13	1.58	1.61	420.00	719.73	65.75	1.52	4.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4128E+02 EXCESS=0.0000E+00 OUTFLOW=0.4128E+02 BASIN STORAGE=0.4197E-01 PERCENT ERROR= -0.1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.13	1.58	3.00		719.25	66.00	1.52

*** *** *** *** ***

HYDROGRAPH AT STATION R2-3
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 719.	1.10	75.	20.	10.	10.
		(INCHES) 1.364	1.486	1.521	1.521
		(AC-FT) 37.	40.	41.	41.

CUMULATIVE AREA = 0.51 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.13	1.58	1.65	420.00	650.44	66.16	1.47	4.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3985E+02 EXCESS=0.0000E+00 OUTFLOW=0.3986E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= -0.1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.13	1.58	3.00		647.31	66.00	1.47

*** *** *** *** ***

HYDROGRAPH AT STATION R2-3
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 647.	1.10	72.	20.	10.	10.
		(INCHES) 1.313	1.435	1.469	1.469
		(AC-FT) 36.	39.	40.	40.

CUMULATIVE AREA = 0.51 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.13	1.58	1.61	420.00	553.64	68.36	1.37	4.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3711E+02 EXCESS=0.0000E+00 OUTFLOW=0.3710E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= -0.1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.13	1.58	3.00		550.82	69.00	1.37

*** *** *** *** ***

HYDROGRAPH AT STATION R2-3
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 551.	1.15	66.	18.	9.	9.
		(INCHES) 1.206	1.332	1.366	1.366
		(AC-FT) 33.	36.	37.	37.

CUMULATIVE AREA = 0.51 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.13	1.58	1.77	420.00	421.93	69.46	1.19	3.95

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3232E+02 EXCESS=0.0000E+00 OUTFLOW=0.3234E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= -0.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.13	1.58	3.00		418.11	69.00	1.19

*** *** *** *** ***

HYDROGRAPH AT STATION R2-3
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 418.	1.15	56.	16.	8.	8.
		(INCHES) 1.029	1.157	1.192	1.192
		(AC-FT) 28.	31.	32.	32.

CUMULATIVE AREA = 0.51 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT R2-3

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 719.	1.10	75.	20.	10.	10.
		(INCHES) 1.363	1.486	1.520	1.520
		(AC-FT) 37.	40.	41.	41.

CUMULATIVE AREA = 0.51 SQ MI

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  100 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.15  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.50  WETTING FRONT SUCTION
                        XKSAT        0.47  HYDRAULIC CONDUCTIVITY
                        RTIMP       55.00  PERCENT IMPERVIOUS AREA

  101 UC          CLARK UNITGRAPH
                           TC        0.36  TIME OF CONCENTRATION
                            R        0.29  STORAGE COEFFICIENT

  102 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                      ***

                                                UNIT HYDROGRAPH PARAMETERS
                                             CLARK   TC=  0.36 HR,       R=  0.29 HR
                                            SNYDER   TP=  0.21 HR,      CP=  0.43

                                                   UNIT HYDROGRAPH
                                            34 END-OF-PERIOD ORDINATES
                      5.       17.       43.       62.       64.       61.       56.       49.       42.       35.
                     30.       25.       21.       18.       15.       12.       10.        9.        7.        6.
                      5.        4.        4.        3.        3.        2.        2.        2.        1.        1.
                      1.        1.        1.        1.

       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB03
                          TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.44, TOTAL EXCESS =    2.31

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                           (CFS)
+      130.      1.00                   12.          3.          1.          1.
                          (INCHES)     2.302       2.302       2.302       2.302
                           (AC-FT)       6.          6.          6.          6.

                           CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB03
                          TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.44, TOTAL EXCESS =    2.30

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                           (CFS)
+      129.      1.00                   12.          3.          1.          1.
                          (INCHES)     2.286       2.286       2.286       2.286
                           (AC-FT)       6.          6.          6.          6.

                           CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB03
                          TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.48, TOTAL EXCESS =    2.21

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                           (CFS)
+      117.      1.00                   11.          3.          1.          1.
                          (INCHES)     2.198       2.198       2.198       2.198
                           (AC-FT)       6.          6.          6.          6.

                           CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB03
                          TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.52, TOTAL EXCESS =    2.02

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                           (CFS)
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+      99.      1.05                    10.          3.          1.          1.
                          (INCHES)     2.014       2.014       2.014       2.014
                           (AC-FT)       5.          5.          5.          5.

                           CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB03
                        TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.54, TOTAL EXCESS =    1.69

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+      77.      1.05                    9.          2.          1.          1.
                          (INCHES)     1.679       1.679       1.679       1.679
                           (AC-FT)       4.          4.          4.          4.

                           CUMULATIVE AREA =    0.05 SQ MI

       ***             ***             ***             ***             ***


                           INTERPOLATED HYDROGRAPH AT     SB03

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     129.      1.00                   12.          3.          1.          1.
                          (INCHES)     2.290       2.290       2.290       2.290
                           (AC-FT)       6.          6.          6.          6.

                           CUMULATIVE AREA =    0.05 SQ MI



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TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 755.	1.10	83.	22.	11.	11.
		(INCHES) 1.385	1.500	1.532	1.532
		(AC-FT) 41.	45.	46.	46.

CUMULATIVE AREA = 0.56 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-3
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 638.	1.15	76.	21.	10.	10.
		(INCHES) 1.271	1.390	1.422	1.422
		(AC-FT) 38.	41.	42.	42.

CUMULATIVE AREA = 0.56 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-3
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 487.	1.15	65.	18.	9.	9.
		(INCHES) 1.080	1.202	1.234	1.234
		(AC-FT) 32.	36.	37.	37.

CUMULATIVE AREA = 0.56 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT CP-3

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 828.	1.10	86.	23.	11.	11.
		(INCHES) 1.435	1.552	1.583	1.583
		(AC-FT) 43.	46.	47.	47.

CUMULATIVE AREA = 0.56 SQ MI

*** *** *** *

MAIN	0.12	1.58	2.78	798.67	817.36	73.14	1.60	5.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4741E+02 EXCESS=0.0000E+00 OUTFLOW=0.4763E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=-0.6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.12	1.58	3.00		816.94	72.00	1.60

*** *** *** *** ***

HYDROGRAPH AT STATION R3-5
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 817.	1.20	87.	24.	12.	12.
		(INCHES) 1.450	1.571	1.602	1.602
		(AC-FT) 43.	47.	48.	48.

CUMULATIVE AREA = 0.56 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.12	1.58	2.79	798.67	810.60	70.45	1.59	4.99

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4713E+02 EXCESS=0.0000E+00 OUTFLOW=0.4736E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=-0.7

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.12	1.58	3.00		810.54	72.00	1.59

*** *** *** *** ***

HYDROGRAPH AT STATION R3-5
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 811.	1.20	86.	23.	11.	11.
		(INCHES) 1.440	1.562	1.593	1.593
		(AC-FT) 43.	46.	47.	47.

CUMULATIVE AREA = 0.56 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.12	1.58	2.90	798.67	742.52	73.90	1.54	4.81

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4551E+02 EXCESS=0.0000E+00 OUTFLOW=0.4571E+02 BASIN STORAGE=0.8226E-01 PERCENT ERROR=-0.6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.12	1.58	3.00		740.16	72.00	1.54

*** *** *** *** ***

HYDROGRAPH AT STATION R3-5
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 740.	1.20	83.	23.	11.	11.
		(INCHES) 1.385	1.506	1.537	1.537

(AC-FT) 41. 45. 46. 46.

CUMULATIVE AREA = 0.56 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.12	1.58	3.00	798.67	634.55	73.70	1.43	4.52

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4224E+02 EXCESS=0.0000E+00 OUTFLOW=0.4240E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR= -0.6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.12	1.58	3.00		626.49	75.00	1.43

*** *** *** *** ***

HYDROGRAPH AT STATION R3-5
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
626.	1.25	76.	21.	10.	10.
		(INCHES) 1.271	1.396	1.427	1.427
		(AC-FT) 38.	41.	42.	42.

CUMULATIVE AREA = 0.56 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.12	1.58	3.00	798.67	478.66	74.64	1.24	4.10

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3665E+02 EXCESS=0.0000E+00 OUTFLOW=0.3688E+02 BASIN STORAGE=0.8233E-01 PERCENT ERROR= -0.8

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.12	1.58	3.00		478.05	75.00	1.24

*** *** *** *** ***

HYDROGRAPH AT STATION R3-5
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
478.	1.25	65.	18.	9.	9.
		(INCHES) 1.082	1.210	1.242	1.242
		(AC-FT) 32.	36.	37.	37.

CUMULATIVE AREA = 0.56 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT R3-5

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
806.	1.20	86.	23.	11.	11.
		(INCHES) 1.437	1.558	1.589	1.589
		(AC-FT) 43.	46.	47.	47.

CUMULATIVE AREA = 0.56 SQ MI

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            *            *
  110 KK    *      SB04  *        BASIN
            *            *
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               SUBBASIN RUNOFF DATA

  111 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.14  SUBBASIN AREA

  112 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.14  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.60  WETTING FRONT SUCTION
                        XKSAT        0.44  HYDRAULIC CONDUCTIVITY
                        RTIMP       61.00  PERCENT IMPERVIOUS AREA

  113 UC          CLARK UNITGRAPH
                           TC        0.30  TIME OF CONCENTRATION
                            R        0.20  STORAGE COEFFICIENT

  114 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                        ***

                                                UNIT HYDROGRAPH PARAMETERS
                                          CLARK   TC=  0.30 HR,       R=  0.20 HR
                                         SNYDER   TP=  0.18 HR,      CP=  0.54

                                                    UNIT HYDROGRAPH
                                                24 END-OF-PERIOD ORDINATES
                    26.      107.      218.      267.      245.      215.      177.      137.      106.       82.
                    63.       49.       38.       29.       23.       18.       14.       11.        8.        6.
                     5.        4.        3.        2.

       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB04
                          TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.38, TOTAL EXCESS =    2.38

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     528.      0.95                37.          9.          4.          4.
                         (INCHES)     2.369       2.369       2.369       2.369
                          (AC-FT)       18.         18.         18.         18.

                           CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB04
                          TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.37, TOTAL EXCESS =    2.36

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     524.      0.95                36.          9.          4.          4.
                         (INCHES)     2.353       2.353       2.353       2.353
                          (AC-FT)       18.         18.         18.         18.

                           CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB04
                          TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.40, TOTAL EXCESS =    2.28

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     452.      0.95                35.          9.          4.          4.
                         (INCHES)     2.272       2.272       2.272       2.272
                          (AC-FT)       17.         17.         17.         17.

                           CUMULATIVE AREA =    0.14 SQ MI
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       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB04
                        TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.44, TOTAL EXCESS =    2.10

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     382.      1.00                   32.          8.          4.          4.
                         (INCHES)     2.095       2.095       2.095       2.095
                          (AC-FT)       16.         16.         16.         16.

                          CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB04
                        TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.46, TOTAL EXCESS =    1.77

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     293.      1.00                   27.          7.          3.          3.
                         (INCHES)     1.763       1.763       1.763       1.763
                          (AC-FT)       14.         14.         14.         14.

                          CUMULATIVE AREA =    0.14 SQ MI

       ***             ***             ***             ***             ***


                          INTERPOLATED HYDROGRAPH AT     SB04

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     525.      0.95                   36.          9.          4.          4.
                         (INCHES)     2.356       2.356       2.356       2.356
                          (AC-FT)       18.         18.         18.         18.

                          CUMULATIVE AREA =    0.14 SQ MI



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INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		513.38	63.00	2.40

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 513.	1.05	37.	9.	4.	4.
		(INCHES) 2.401	2.402	2.402	2.402
		(AC-FT) 18.	18.	18.	18.

CUMULATIVE AREA = 0.14 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.23	1.51	2.42	668.33	516.56	62.40	2.38	4.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1807E+02 EXCESS=0.0000E+00 OUTFLOW=0.1825E+02 BASIN STORAGE=0.4744E-04 PERCENT ERROR= -1.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		510.06	63.00	2.39

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 510.	1.05	37.	9.	4.	4.
		(INCHES) 2.386	2.386	2.386	2.386
		(AC-FT) 18.	18.	18.	18.

CUMULATIVE AREA = 0.14 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.23	1.51	2.55	668.33	451.64	62.77	2.29	4.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1745E+02 EXCESS=0.0000E+00 OUTFLOW=0.1762E+02 BASIN STORAGE=0.4583E-04 PERCENT ERROR= -1.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		451.16	63.00	2.30

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 451.	1.05	36.	9.	4.	4.
		(INCHES) 2.301	2.302	2.302	2.302
		(AC-FT) 18.	18.	18.	18.

CUMULATIVE AREA = 0.14 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.23	1.51	2.73	668.33	379.50	64.90	2.12	4.31

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1609E+02 EXCESS=0.0000E+00 OUTFLOW=0.1629E+02 BASIN STORAGE=0.4132E-04 PERCENT ERROR=-1.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		374.76	63.00	2.12

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
		(CFS)			
375.	1.05	33.	8.	4.	4.
		(INCHES) 2.115	2.116	2.116	2.116
		(AC-FT) 16.	16.	16.	16.

CUMULATIVE AREA = 0.14 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.23	1.51	2.87	668.33	291.80	65.24	1.78	3.94

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1354E+02 EXCESS=0.0000E+00 OUTFLOW=0.1370E+02 BASIN STORAGE=0.4337E-04 PERCENT ERROR=-1.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		288.81	66.00	1.78

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
		(CFS)			
289.	1.10	28.	7.	3.	3.
		(INCHES) 1.782	1.782	1.782	1.782
		(AC-FT) 14.	14.	14.	14.

CUMULATIVE AREA = 0.14 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT R4-5

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
		(CFS)			
511.	1.05	37.	9.	4.	4.
		(INCHES) 2.388	2.388	2.388	2.388
		(AC-FT) 18.	18.	18.	18.

CUMULATIVE AREA = 0.14 SQ MI

*** *** *** *

SUBBASIN RUNOFF DATA

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  120 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.13  SUBBASIN AREA

  121 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.22  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.50  WETTING FRONT SUCTION
                        XKSAT        0.44  HYDRAULIC CONDUCTIVITY
                        RTIMP       48.00  PERCENT IMPERVIOUS AREA

  122 UC          CLARK UNITGRAPH
                           TC        0.33  TIME OF CONCENTRATION
                            R        0.22  STORAGE COEFFICIENT

  123 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0
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UNIT HYDROGRAPH PARAMETERS
CLARK TC= 0.33 HR, R= 0.22 HR
SNYDER TP= 0.19 HR, CP= 0.50

UNIT HYDROGRAPH
27 END-OF-PERIOD ORDINATES

18.	67.	154.	208.	200.	181.	156.	129.	103.	82.
66.	53.	42.	34.	27.	21.	17.	14.	11.	9.
7.	6.	4.	4.	3.	2.	2.			

*** *** *** *** ***

HYDROGRAPH AT STATION SB05
TRANSPOSITION AREA 0.0 SQ MI

TOTAL RAINFALL = 2.76, TOTAL LOSS = 0.52, TOTAL EXCESS = 2.23

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 402.	0.95	30.	8.	4.	4.
		(INCHES) 2.223	2.223	2.223	2.223
		(AC-FT) 15.	15.	15.	15.

CUMULATIVE AREA = 0.13 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB05
TRANSPOSITION AREA 0.5 SQ MI

TOTAL RAINFALL = 2.74, TOTAL LOSS = 0.52, TOTAL EXCESS = 2.22

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 399.	0.95	30.	7.	4.	4.
		(INCHES) 2.208	2.208	2.208	2.208
		(AC-FT) 15.	15.	15.	15.

CUMULATIVE AREA = 0.13 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB05
TRANSPOSITION AREA 2.8 SQ MI

TOTAL RAINFALL = 2.69, TOTAL LOSS = 0.56, TOTAL EXCESS = 2.13

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 352.	1.00	29.	7.	3.	3.
		(INCHES) 2.118	2.118	2.118	2.118
		(AC-FT) 14.	14.	14.	14.

CUMULATIVE AREA = 0.13 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB05
TRANSPOSITION AREA 16.0 SQ MI

TOTAL RAINFALL = 2.54, TOTAL LOSS = 0.60, TOTAL EXCESS = 1.94

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 296.	1.00	26.	7.	3.	3.
		(INCHES) 1.930	1.930	1.930	1.930
		(AC-FT) 13.	13.	13.	13.

CUMULATIVE AREA = 0.13 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB05
TRANSPOSITION AREA 90.0 SQ MI

TOTAL RAINFALL = 2.23, TOTAL LOSS = 0.63, TOTAL EXCESS = 1.60

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 225.	1.00	22.	5.	3.	3.
		(INCHES) 1.593	1.593	1.593	1.593
		(AC-FT) 11.	11.	11.	11.

CUMULATIVE AREA = 0.13 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT SB05

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 399.	0.95	30.	7.	4.	4.
		(INCHES) 2.210	2.210	2.210	2.210
		(AC-FT) 15.	15.	15.	15.

CUMULATIVE AREA = 0.13 SQ MI

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       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-5
                        TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1335.      1.15                146.         39.         19.          19.
                        (INCHES)     1.638       1.737       1.759        1.759
                         (AC-FT)       72.         77.         78.          78.

                          CUMULATIVE AREA =    0.83 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-5
                        TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1121.      1.15                133.         36.         17.          17.
                        (INCHES)     1.499       1.602       1.623        1.623
                         (AC-FT)       66.         71.         72.          72.

                          CUMULATIVE AREA =    0.83 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-5
                        TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     844.      1.20                112.         30.         15.          15.
                        (INCHES)     1.262       1.368       1.389        1.389
                         (AC-FT)       56.         60.         61.          61.

                          CUMULATIVE AREA =    0.83 SQ MI

       ***             ***             ***             ***             ***


                          INTERPOLATED HYDROGRAPH AT     CP-5

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1410.      1.15                150.         40.         19.          19.
                        (INCHES)     1.684       1.784       1.805        1.805
                         (AC-FT)       74.         79.         80.          80.

                          CUMULATIVE AREA =    0.83 SQ MI



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ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.07	1.62	1.71	516.67	1430.15	73.26	1.84	5.13

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8096E+02 EXCESS=0.0000E+00 OUTFLOW=0.8101E+02 BASIN STORAGE=0.7278E-01 PERCENT ERROR=-0.1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.07	1.62	3.00		1428.13	72.00	1.84	

*** *** *** *** ***

HYDROGRAPH AT STATION R5-6
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
1428.	1.20	153.	40.	20.	20.
		(INCHES) 1.715	1.815	1.836	1.836
		(AC-FT) 76.	80.	81.	81.

CUMULATIVE AREA = 0.83 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.07	1.62	1.72	516.67	1417.75	71.63	1.82	5.12

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8047E+02 EXCESS=0.0000E+00 OUTFLOW=0.8049E+02 BASIN STORAGE=0.7282E-01 PERCENT ERROR=-0.1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.07	1.62	3.00		1417.68	72.00	1.82	

*** *** *** *** ***

HYDROGRAPH AT STATION R5-6
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
1418.	1.20	151.	40.	19.	19.
		(INCHES) 1.703	1.803	1.824	1.824
		(AC-FT) 75.	80.	80.	80.

CUMULATIVE AREA = 0.83 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.07	1.62	1.83	516.67	1320.58	72.30	1.76	4.97

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7757E+02 EXCESS=0.0000E+00 OUTFLOW=0.7762E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR=-0.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.07	1.62	3.00		1310.29	72.00	1.76	

*** *** *** *** ***

HYDROGRAPH AT STATION R5-6
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR

+	(CFS)	(HR)				
			(CFS)			
+	1310.	1.20	146.	39.	19.	19.
		(INCHES)	1.639	1.739	1.760	1.760
		(AC-FT)	72.	77.	78.	78.

CUMULATIVE AREA = 0.83 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.07	1.62	1.92	516.67	1120.07	73.97	1.62	4.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7160E+02 EXCESS=0.0000E+00 OUTFLOW=0.7164E+02 BASIN STORAGE=0.7274E-01 PERCENT ERROR= -0.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.07	1.62	3.00	1117.54	75.00	1.62

*** *** *** *** ***

HYDROGRAPH AT STATION R5-6
TRANSPOSITION AREA 16.0 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	1118.	1.25	133.	36.	17.	17.
		(INCHES)	1.500	1.603	1.624	1.624
		(AC-FT)	66.	71.	72.	72.

CUMULATIVE AREA = 0.83 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.07	1.62	2.16	516.67	837.38	77.35	1.39	4.17

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6127E+02 EXCESS=0.0000E+00 OUTFLOW=0.6143E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR= -0.4

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.07	1.62	3.00	834.70	75.00	1.39

*** *** *** *** ***

HYDROGRAPH AT STATION R5-6
TRANSPOSITION AREA 90.0 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	835.	1.25	112.	30.	15.	15.
		(INCHES)	1.264	1.371	1.392	1.392
		(AC-FT)	56.	60.	61.	61.

CUMULATIVE AREA = 0.83 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT R5-6

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	1386.	1.20	150.	40.	19.	19.
		(INCHES)	1.684	1.785	1.806	1.806
		(AC-FT)	74.	79.	80.	80.

CUMULATIVE AREA = 0.83 SQ MI

*** *** *** *

(AC-FT) 16. 16. 16. 16.

CUMULATIVE AREA = 0.14 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB06
TRANSPOSITION AREA 16.0 SQ MI

TOTAL RAINFALL = 2.54, TOTAL LOSS = 0.53, TOTAL EXCESS = 2.01

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 316.	1.00	29.	7.	4.	4.
		(INCHES) 1.997	1.997	1.997	1.997
		(AC-FT) 14.	14.	14.	14.

CUMULATIVE AREA = 0.14 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB06
TRANSPOSITION AREA 90.0 SQ MI

TOTAL RAINFALL = 2.23, TOTAL LOSS = 0.56, TOTAL EXCESS = 1.67

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 241.	1.00	24.	6.	3.	3.
		(INCHES) 1.661	1.661	1.661	1.661
		(AC-FT) 12.	12.	12.	12.

CUMULATIVE AREA = 0.14 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT SB06

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 420.	0.95	33.	8.	4.	4.
		(INCHES) 2.272	2.272	2.272	2.272
		(AC-FT) 16.	16.	16.	16.

CUMULATIVE AREA = 0.14 SQ MI

*** *** *** *

TRANSPOSITION AREA 0.5 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	1650.	1.20	183.	48.	23.	23.
			(INCHES) 1.769	1.869	1.887	1.887
			(AC-FT) 91.	96.	97.	97.

CUMULATIVE AREA = 0.96 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-6
TRANSPOSITION AREA 2.8 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	1542.	1.20	176.	47.	23.	23.
			(INCHES) 1.702	1.801	1.820	1.820
			(AC-FT) 87.	93.	93.	93.

CUMULATIVE AREA = 0.96 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-6
TRANSPOSITION AREA 16.0 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	1308.	1.20	161.	43.	21.	21.
			(INCHES) 1.555	1.659	1.677	1.677
			(AC-FT) 80.	85.	86.	86.

CUMULATIVE AREA = 0.96 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-6
TRANSPOSITION AREA 90.0 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	982.	1.25	135.	37.	18.	18.
			(INCHES) 1.305	1.412	1.430	1.430
			(AC-FT) 67.	73.	73.	73.

CUMULATIVE AREA = 0.96 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT CP-6

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	1609.	1.20	181.	48.	23.	23.
			(INCHES) 1.743	1.843	1.862	1.862
			(AC-FT) 90.	95.	96.	96.

CUMULATIVE AREA = 0.96 SQ MI

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  142 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.21  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.00  WETTING FRONT SUCTION
                        XKSAT        0.58  HYDRAULIC CONDUCTIVITY
                        RTIMP       49.00  PERCENT IMPERVIOUS AREA

  143 UC          CLARK UNITGRAPH
                           TC        0.20  TIME OF CONCENTRATION
                            R        0.11  STORAGE COEFFICIENT

  144 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                      ***

                                                  UNIT HYDROGRAPH PARAMETERS
                                             CLARK   TC=  0.20 HR,       R=  0.11 HR
                                            SNYDER   TP=  0.13 HR,      CP=  0.62

                                                      UNIT HYDROGRAPH
                                                14 END-OF-PERIOD ORDINATES
                    39.      156.      219.      180.      130.       84.       53.       34.       22.       14.
                     9.        6.        4.        2.

       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB07
                          TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.54, TOTAL EXCESS =    2.22

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+      386.      0.90                   18.          4.          2.          2.
                         (INCHES)     2.209       2.209       2.209       2.209
                          (AC-FT)        9.          9.          9.          9.

                           CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB07
                          TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.54, TOTAL EXCESS =    2.20

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+      384.      0.90                   17.          4.          2.          2.
                         (INCHES)     2.193       2.193       2.193       2.193
                          (AC-FT)        9.          9.          9.          9.

                           CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB07
                          TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.58, TOTAL EXCESS =    2.10

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+      308.      0.90                   17.          4.          2.          2.
                         (INCHES)     2.095       2.095       2.095       2.095
                          (AC-FT)        8.          8.          8.          8.

                           CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB07
                          TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.63, TOTAL EXCESS =    1.91

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+      249.      0.90                   15.          4.          2.          2.
                         (INCHES)     1.901       1.901       1.901       1.901
                          (AC-FT)        8.          8.          8.          8.
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CUMULATIVE AREA = 0.07 SQ MI

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HYDROGRAPH AT STATION SB07
TRANSPOSITION AREA 90.0 SQ MI

TOTAL RAINFALL = 2.23, TOTAL LOSS = 0.66, TOTAL EXCESS = 1.57

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 181.	0.90	12.	3.	1.	1.
		(INCHES) 1.563	1.563	1.563	1.563
		(AC-FT) 6.	6.	6.	6.

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT SB07

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 384.	0.90	17.	4.	2.	2.
		(INCHES) 2.197	2.197	2.197	2.197
		(AC-FT) 9.	9.	9.	9.

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *

(INCHES)	2.311	2.314	2.314	2.314
(AC-FT)	9.	9.	9.	9.

CUMULATIVE AREA = 0.07 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.21	1.53	2.80	694.50	373.06	62.02	2.31	4.31

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8655E+01 EXCESS=0.0000E+00 OUTFLOW=0.9098E+01 BASIN STORAGE=0.1282E-03 PERCENT ERROR=
-5.1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.21	1.53	3.00		359.23	63.00	2.30

*** *** *** *** ***

HYDROGRAPH AT STATION R7-8
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
+ 359.	1.05	18.	5.	2.	2.
	(INCHES)	2.295	2.298	2.298	2.298
	(AC-FT)	9.	9.	9.	9.

CUMULATIVE AREA = 0.07 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.21	1.53	3.00	694.50	300.60	62.65	2.18	4.00

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8267E+01 EXCESS=0.0000E+00 OUTFLOW=0.8591E+01 BASIN STORAGE=0.1511E-03 PERCENT ERROR=
-3.9

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.21	1.53	3.00		299.77	63.00	2.18

*** *** *** *** ***

HYDROGRAPH AT STATION R7-8
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
+ 300.	1.05	17.	4.	2.	2.
	(INCHES)	2.176	2.178	2.179	2.179
	(AC-FT)	9.	9.	9.	9.

CUMULATIVE AREA = 0.07 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.21	1.53	3.00	694.50	243.19	65.55	1.99	3.72

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7502E+01 EXCESS=0.0000E+00 OUTFLOW=0.7866E+01 BASIN STORAGE=0.1118E-03 PERCENT ERROR=
-4.8

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.21	1.53	3.00		238.95	66.00	2.00

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      ***            ***            ***            ***            ***

                        HYDROGRAPH AT STATION     R7-8
                      TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     239.      1.10                16.          4.          2.           2.
                            (INCHES)     1.995       1.997       1.997        1.997
                             (AC-FT)        8.          8.          8.           8.

                             CUMULATIVE AREA =    0.07 SQ MI

                                      COMPUTED KINEMATIC PARAMETERS
                                           VARIABLE TIME STEP
                                       (DT SHOWN IS A MINIMUM)

                        ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                        PEAK                CELERITY
                                                                    (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                       MAIN          0.21      1.53       2.82    555.60    178.83     67.24      1.61      3.33


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6168E+01  EXCESS=0.0000E+00 OUTFLOW=0.6362E+01 BASIN STORAGE=0.1260E-03 PERCENT ERROR=
-3.1


                                          INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                       MAIN          0.21      1.53       3.00              176.29     66.00      1.61

      ***            ***            ***            ***            ***

                        HYDROGRAPH AT STATION     R7-8
                      TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     176.      1.10                13.          3.          2.           2.
                            (INCHES)     1.609       1.611       1.611        1.611
                             (AC-FT)        6.          6.          6.           6.

                             CUMULATIVE AREA =    0.07 SQ MI

      ***            ***            ***            ***            ***


                         INTERPOLATED HYDROGRAPH AT     R7-8

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     360.      1.05                18.          5.          2.           2.
                            (INCHES)     2.299       2.301       2.301        2.301
                             (AC-FT)        9.          9.          9.           9.

                             CUMULATIVE AREA =    0.07 SQ MI



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R 0.19 STORAGE COEFFICIENT

153 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES
0.0 5.0 16.0 30.0 65.0 77.0 84.0 90.0 94.0 97.0
100.0

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UNIT HYDROGRAPH PARAMETERS
CLARK TC= 0.33 HR, R= 0.19 HR
SNYDER TP= 0.19 HR, CP= 0.55

UNIT HYDROGRAPH
24 END-OF-PERIOD ORDINATES
35. 127. 298. 409. 385. 339. 288. 232. 179. 138.
106. 82. 63. 49. 37. 29. 22. 17. 13. 10.
8. 6. 5. 4.

*** *** *** *** ***

HYDROGRAPH AT STATION SB08
TRANSPOSITION AREA 0.0 SQ MI

TOTAL RAINFALL = 2.76, TOTAL LOSS = 0.42, TOTAL EXCESS = 2.34

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 796.	0.95	56.	14.	7.	7.
		(INCHES) 2.325	2.325	2.325	2.325
		(AC-FT) 28.	28.	28.	28.

CUMULATIVE AREA = 0.22 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB08
TRANSPOSITION AREA 0.5 SQ MI

TOTAL RAINFALL = 2.74, TOTAL LOSS = 0.42, TOTAL EXCESS = 2.32

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 791.	0.95	56.	14.	7.	7.
		(INCHES) 2.309	2.309	2.309	2.309
		(AC-FT) 28.	28.	28.	28.

CUMULATIVE AREA = 0.22 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB08
TRANSPOSITION AREA 2.8 SQ MI

TOTAL RAINFALL = 2.69, TOTAL LOSS = 0.46, TOTAL EXCESS = 2.23

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 686.	1.00	53.	13.	6.	6.
		(INCHES) 2.218	2.218	2.218	2.218
		(AC-FT) 26.	26.	26.	26.

CUMULATIVE AREA = 0.22 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB08
TRANSPOSITION AREA 16.0 SQ MI

TOTAL RAINFALL = 2.54, TOTAL LOSS = 0.50, TOTAL EXCESS = 2.04

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 577.	1.00	49.	12.	6.	6.
		(INCHES) 2.032	2.032	2.032	2.032
		(AC-FT) 24.	24.	24.	24.

CUMULATIVE AREA = 0.22 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB08
TRANSPOSITION AREA 90.0 SQ MI

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     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.53, TOTAL EXCESS =    1.71

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     439.      1.00                41.         10.          5.           5.
                         (INCHES)    1.699       1.699       1.699        1.699
                          (AC-FT)      20.         20.         20.          20.

                          CUMULATIVE AREA =    0.22 SQ MI

       ***             ***             ***             ***             ***


                          INTERPOLATED HYDROGRAPH AT     SB08

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     792.      0.95                56.         14.          7.           7.
                         (INCHES)    2.311       2.311       2.311        2.311
                          (AC-FT)      28.         28.         28.          28.

                          CUMULATIVE AREA =    0.22 SQ MI



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                         HYDROGRAPH AT STATION     CP-8
                       TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     783.      1.05                65.         16.          8.           8.
                        (INCHES)     2.023       2.023       2.023        2.023
                         (AC-FT)      32.         32.         32.          32.

                          CUMULATIVE AREA =    0.30 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-8
                       TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     561.      1.10                54.         13.          6.           6.
                        (INCHES)     1.676       1.677       1.677        1.677
                         (AC-FT)      27.         27.         27.          27.

                          CUMULATIVE AREA =    0.30 SQ MI

       ***             ***             ***             ***             ***


                          INTERPOLATED HYDROGRAPH AT     CP-8

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1035.      1.05                74.         18.          9.           9.
                        (INCHES)     2.307       2.308       2.308        2.308
                         (AC-FT)      37.         37.         37.          37.

                          CUMULATIVE AREA =    0.30 SQ MI



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MAIN	0.15	1.56	3.00	1013.13	66.00	2.33

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HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1013.	1.10	75.	19.	9.	9.
		(INCHES) 2.329	2.330	2.330	2.330
		(AC-FT) 37.	37.	37.	37.

CUMULATIVE AREA = 0.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.15	1.56	1.33	416.67	1027.12	65.64	2.31	5.60

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3666E+02 EXCESS=0.0000E+00 OUTFLOW=0.3678E+02 BASIN STORAGE=0.2024E-03 PERCENT ERROR=
-0.3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.15	1.56	3.00	1010.29	66.00	2.32

*** *** *** *** ***

HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1010.	1.10	74.	19.	9.	9.
		(INCHES) 2.314	2.315	2.316	2.316
		(AC-FT) 37.	37.	37.	37.

CUMULATIVE AREA = 0.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.15	1.56	1.45	416.67	913.69	66.38	2.21	5.40

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3510E+02 EXCESS=0.0000E+00 OUTFLOW=0.3519E+02 BASIN STORAGE=0.1735E-03 PERCENT ERROR=
-0.3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.15	1.56	3.00	911.34	66.00	2.21

*** *** *** *** ***

HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 911.	1.10	71.	18.	9.	9.
		(INCHES) 2.212	2.214	2.214	2.214
		(AC-FT) 35.	35.	35.	35.

CUMULATIVE AREA = 0.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)

MAIN	0.15	1.56	1.48	416.67	772.14	65.64	2.03	5.07

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3216E+02 EXCESS=0.0000E+00 OUTFLOW=0.3228E+02 BASIN STORAGE=0.1954E-03 PERCENT ERROR=-0.4

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.15	1.56	3.00		769.23	66.00	2.03

*** *** *** *** ***

HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 769.	1.10	65.	16.	8.	8.
		(INCHES) 2.031	2.033	2.033	2.033
		(AC-FT) 32.	32.	32.	32.

CUMULATIVE AREA = 0.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.15	1.56	1.59	416.67	554.15	67.90	1.68	4.50

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2666E+02 EXCESS=0.0000E+00 OUTFLOW=0.2676E+02 BASIN STORAGE=0.1898E-03 PERCENT ERROR=-0.4

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.15	1.56	3.00		554.05	69.00	1.68

*** *** *** *** ***

HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 554.	1.15	54.	13.	6.	6.
		(INCHES) 1.682	1.684	1.684	1.684
		(AC-FT) 27.	27.	27.	27.

CUMULATIVE AREA = 0.30 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT R8-9

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1010.	1.10	74.	19.	9.	9.
		(INCHES) 2.315	2.316	2.316	2.316
		(AC-FT) 37.	37.	37.	37.

CUMULATIVE AREA = 0.30 SQ MI

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  162 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.05  SUBBASIN AREA

  163 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.15  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.00  WETTING FRONT SUCTION
                        XKSAT        0.61  HYDRAULIC CONDUCTIVITY
                        RTIMP       55.00  PERCENT IMPERVIOUS AREA

  164 UC          CLARK UNITGRAPH
                          TC        0.25  TIME OF CONCENTRATION
                           R        0.23  STORAGE COEFFICIENT

  165 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                       0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                     100.0

                                                      ***

                                                 UNIT HYDROGRAPH PARAMETERS
                                             CLARK   TC=  0.25 HR,       R=  0.23 HR
                                            SNYDER   TP=  0.15 HR,      CP=  0.42

                                                    UNIT HYDROGRAPH
                                                27 END-OF-PERIOD ORDINATES
                    11.       51.       86.       89.       82.       70.       56.       45.       36.       29.
                    23.       19.       15.       12.       10.        8.        6.        5.        4.        3.
                     3.        2.        2.        1.        1.        1.        1.

       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB09
                          TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.47, TOTAL EXCESS =    2.29

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+      174.      0.95                   13.          3.          2.          2.
                         (INCHES)     2.280       2.280       2.280       2.280
                          (AC-FT)       6.          6.          6.          6.

                           CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB09
                          TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.27

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+      173.      0.95                   13.          3.          2.          2.
                         (INCHES)     2.264       2.264       2.264       2.264
                          (AC-FT)       6.          6.          6.          6.

                           CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB09
                          TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.51, TOTAL EXCESS =    2.18

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+      153.      0.95                   12.          3.          1.          1.
                         (INCHES)     2.168       2.168       2.168       2.168
                          (AC-FT)       6.          6.          6.          6.

                           CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB09
                          TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.55, TOTAL EXCESS =    1.99

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
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                          (CFS)
+     127.      0.95                  11.          3.          1.          1.
                       (INCHES)     1.978       1.978       1.978       1.978
                        (AC-FT)       5.          5.          5.          5.

                        CUMULATIVE AREA =    0.05 SQ MI


       ***            ***            ***            ***            ***

                        HYDROGRAPH AT STATION     SB09
                      TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.58, TOTAL EXCESS =    1.65

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+      96.      0.95                  9.          2.          1.          1.
                       (INCHES)     1.642       1.642       1.642       1.642
                        (AC-FT)       5.          5.          5.          5.

                        CUMULATIVE AREA =    0.05 SQ MI

       ***            ***            ***            ***            ***


                        INTERPOLATED HYDROGRAPH AT     SB09

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     173.      0.95                  13.          3.          2.          2.
                       (INCHES)     2.269       2.269       2.269       2.269
                        (AC-FT)       6.          6.          6.          6.

                        CUMULATIVE AREA =    0.05 SQ MI



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HYDROGRAPH AT STATION CP-9
TRANSPOSITION AREA 2.8 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	1023.	1.10	83.	21.	10.	10.
			(INCHES) 2.206	2.207	2.207	2.207
			(AC-FT) 41.	41.	41.	41.

CUMULATIVE AREA = 0.35 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-9
TRANSPOSITION AREA 16.0 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	869.	1.10	76.	19.	9.	9.
			(INCHES) 2.023	2.024	2.025	2.025
			(AC-FT) 38.	38.	38.	38.

CUMULATIVE AREA = 0.35 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-9
TRANSPOSITION AREA 90.0 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	622.	1.15	63.	16.	8.	8.
			(INCHES) 1.676	1.678	1.678	1.678
			(AC-FT) 31.	31.	31.	31.

CUMULATIVE AREA = 0.35 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT CP-9

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
			(CFS)			
+	1124.	1.10	87.	22.	10.	10.
			(INCHES) 2.307	2.309	2.309	2.309
			(AC-FT) 43.	43.	43.	43.

CUMULATIVE AREA = 0.35 SQ MI

*** *** *** *

	(AC-FT)	541.	580.	603.	603.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMACH
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 4017.	5.50	1092.	292.	146.	146.
		(INCHES) 0.825	0.884	0.919	0.919
		(AC-FT) 541.	580.	603.	603.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMACH
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 4017.	5.50	1092.	292.	146.	146.
		(INCHES) 0.825	0.884	0.919	0.919
		(AC-FT) 541.	580.	603.	603.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMACH
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 4017.	5.50	1092.	292.	146.	146.
		(INCHES) 0.825	0.884	0.919	0.919
		(AC-FT) 541.	580.	603.	603.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMACH
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 4017.	5.50	1092.	292.	146.	146.
		(INCHES) 0.825	0.884	0.919	0.919
		(AC-FT) 541.	580.	603.	603.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT PIMACH

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 4017.	5.50	1092.	292.	146.	146.
		(INCHES) 0.825	0.884	0.919	0.919
		(AC-FT) 541.	580.	603.	603.

CUMULATIVE AREA = 12.30 SQ MI

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BYPASS ALONG SE SIDE OF BASIN 53R TO ADOT CULVERT NEAR UNION HILLS DR

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      DT          DIVERSION
                       ISTAD      PIMABY  DIVERSION HYDROGRAPH IDENTIFICATION

      DI             INFLOW        0.00   1000.00  10000.00

      DQ      DIVERTED FLOW        0.00   1000.00   1000.00

                                                                        ***

       ***            ***            ***            ***            ***

                          DIVERSION HYDROGRAPH   PIMABY
                         TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                     (CFS)
+    1000.      4.95                605.        171.         88.          88.
                     (INCHES)     0.458       0.516       0.551        0.551
                      (AC-FT)      300.        338.        361.         361.

                       CUMULATIVE AREA =   12.30 SQ MI


       ***            ***            ***            ***            ***

                          HYDROGRAPH AT STATION   PIMAIN
                         TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                     (CFS)
+    3017.      5.50                486.        122.         58.          58.
                     (INCHES)     0.368       0.368       0.368        0.368
                      (AC-FT)      241.        241.        241.         241.

                       CUMULATIVE AREA =   12.30 SQ MI


       ***            ***            ***            ***            ***

                          DIVERSION HYDROGRAPH   PIMABY
                         TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                     (CFS)
+    1000.      4.95                605.        171.         88.          88.
                     (INCHES)     0.458       0.516       0.551        0.551
                      (AC-FT)      300.        338.        361.         361.

                       CUMULATIVE AREA =   12.30 SQ MI


       ***            ***            ***            ***            ***

                          HYDROGRAPH AT STATION   PIMAIN
                         TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                     (CFS)
+    3017.      5.50                486.        122.         58.          58.
                     (INCHES)     0.368       0.368       0.368        0.368
                      (AC-FT)      241.        241.        241.         241.

                       CUMULATIVE AREA =   12.30 SQ MI


       ***            ***            ***            ***            ***

                          DIVERSION HYDROGRAPH   PIMABY
                         TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                     (CFS)
+    1000.      4.95                605.        171.         88.          88.
                     (INCHES)     0.458       0.516       0.551        0.551
                      (AC-FT)      300.        338.        361.         361.

                       CUMULATIVE AREA =   12.30 SQ MI


       ***            ***            ***            ***            ***
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HYDROGRAPH AT STATION PIMAIN
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 3017.	5.50	486.	122.	58.	58.
		(INCHES) 0.368	0.368	0.368	0.368
		(AC-FT) 241.	241.	241.	241.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

DIVERSION HYDROGRAPH PIMABY
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	4.95	605.	171.	88.	88.
		(INCHES) 0.458	0.516	0.551	0.551
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMAIN
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 3017.	5.50	486.	122.	58.	58.
		(INCHES) 0.368	0.368	0.368	0.368
		(AC-FT) 241.	241.	241.	241.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

DIVERSION HYDROGRAPH PIMABY
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	4.95	605.	171.	88.	88.
		(INCHES) 0.458	0.516	0.551	0.551
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMAIN
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 3017.	5.50	486.	122.	58.	58.
		(INCHES) 0.368	0.368	0.368	0.368
		(AC-FT) 241.	241.	241.	241.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

INTERPOLATED DIVERSION HYDROGRAPH AT PIMABY

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	4.95	605.	171.	88.	88.
		(INCHES) 0.458	0.516	0.551	0.551
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT PIMAIN

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
(CFS)	(HR)				
		(CFS)			
3017.	5.50	486.	122.	58.	58.
		(INCHES) 0.368	0.368	0.368	0.368
		(AC-FT) 241.	241.	241.	241.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *

		(CFS)				
+	740.	5.80	432.	136.	75.	75.
		(INCHES)	0.574	0.724	0.826	0.826
		(AC-FT)	214.	270.	308.	308.

CUMULATIVE AREA = 7.00 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PWRCH
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
	(CFS)				
+ 740.	5.80	432.	136.	75.	75.
	(INCHES)	0.574	0.724	0.826	0.826
	(AC-FT)	214.	270.	308.	308.

CUMULATIVE AREA = 7.00 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT PWRCH

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
	(CFS)				
+ 740.	5.80	432.	136.	75.	75.
	(INCHES)	0.574	0.724	0.826	0.826
	(AC-FT)	214.	270.	308.	308.

CUMULATIVE AREA = 7.00 SQ MI

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  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                  (CFS)
+    3634.      5.50                919.        275.        143.         143.
                (INCHES)           0.435       0.521       0.564        0.564
                 (AC-FT)           456.        546.        591.         591.

                 CUMULATIVE AREA =   19.65 SQ MI


       ***            ***             ***             ***             ***

                   HYDROGRAPH AT STATION   BINFLO
                  TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                  (CFS)
+    3634.      5.50                919.        274.        142.         142.
                (INCHES)           0.435       0.518       0.560        0.560
                 (AC-FT)           456.        543.        587.         587.

                 CUMULATIVE AREA =   19.65 SQ MI


       ***            ***             ***             ***             ***

                   HYDROGRAPH AT STATION   BINFLO
                  TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                  (CFS)
+    3634.      5.50                919.        270.        141.         141.
                (INCHES)           0.435       0.512       0.554        0.554
                 (AC-FT)           456.        536.        581.         581.

                 CUMULATIVE AREA =   19.65 SQ MI

       ***            ***             ***             ***             ***


                   INTERPOLATED HYDROGRAPH AT   BINFLO

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                  (CFS)
+    3634.      5.50                919.        273.        142.         142.
                (INCHES)           0.435       0.517       0.560        0.560
                 (AC-FT)           456.        542.        587.         587.

                 CUMULATIVE AREA =   19.65 SQ MI



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  270 SQ          DISCHARGE          0.       95.      153.      210.      244.      265.      284.      303.      320.
337.
                                   353.      369.      384.      398.      412.      426.      439.      452.      465.
477.

                                                                                                        ***

       ***            ***             ***             ***             ***

                          HYDROGRAPH AT STATION    BASIN
                        TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     363.      8.30                  355.        250.        141.         141.
                          (INCHES)    0.168       0.474       0.556        0.556
                          (AC-FT)     176.        496.        582.         582.

 PEAK STORAGE   TIME                          MAXIMUM AVERAGE STORAGE
                                      6-HR       24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      351.      8.30                  332.        189.        101.         101.

  PEAK STAGE    TIME                          MAXIMUM AVERAGE STAGE
                                      6-HR       24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.68      8.30               1606.11     1601.55     1598.29      1598.29

                           CUMULATIVE AREA =   19.65 SQ MI


       ***            ***             ***             ***             ***

                          HYDROGRAPH AT STATION    BASIN
                        TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     363.      8.30                  355.        250.        141.         141.
                          (INCHES)    0.168       0.474       0.555        0.555
                          (AC-FT)     176.        496.        582.         582.

 PEAK STORAGE   TIME                          MAXIMUM AVERAGE STORAGE
                                      6-HR       24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      351.      8.30                  332.        189.        100.         100.

  PEAK STAGE    TIME                          MAXIMUM AVERAGE STAGE
                                      6-HR       24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.67      8.30               1606.11     1601.55     1598.28      1598.28

                           CUMULATIVE AREA =   19.65 SQ MI


       ***            ***             ***             ***             ***

                          HYDROGRAPH AT STATION    BASIN
                        TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     363.      8.30                  354.        250.        141.         141.
                          (INCHES)    0.168       0.473       0.553        0.553
                          (AC-FT)     176.        495.        580.         580.

 PEAK STORAGE   TIME                          MAXIMUM AVERAGE STORAGE
                                      6-HR       24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      350.      8.30                  331.        188.        100.         100.

  PEAK STAGE    TIME                          MAXIMUM AVERAGE STAGE
                                      6-HR       24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.65      8.30               1606.08     1601.53     1598.27      1598.27

                           CUMULATIVE AREA =   19.65 SQ MI


       ***            ***             ***             ***             ***

                          HYDROGRAPH AT STATION    BASIN
                        TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
```

+							
+	362.	8.30		353.	249.	140.	140.
			(INCHES)	0.167	0.471	0.550	0.550
			(AC-FT)	175.	494.	577.	577.

PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	49.95-HR
+ (AC-FT)	(HR)				
349.	8.35	330.	187.	99.	99.

PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	49.95-HR
+ (FEET)	(HR)				
1606.61	8.30	1606.04	1601.50	1598.24	1598.24

CUMULATIVE AREA = 19.65 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION BASIN
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 361.	8.35	352.	247.	138.	138.
		(INCHES) 0.167	0.468	0.544	0.544
		(AC-FT) 175.	491.	570.	570.

PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	49.95-HR
+ (AC-FT)	(HR)				
346.	8.35	327.	185.	98.	98.

PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	49.95-HR
+ (FEET)	(HR)				
1606.52	8.35	1605.96	1601.44	1598.18	1598.18

CUMULATIVE AREA = 19.65 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT BASIN

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 362.	8.30	353.	249.	140.	140.
		(INCHES) 0.167	0.471	0.550	0.550
		(AC-FT) 175.	493.	576.	576.

CUMULATIVE AREA = 19.65 SQ MI

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*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

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COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.29	183.33	363.48	498.70	0.56	10.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03 EXCESS=0.0000E+00 OUTFLOW=0.5822E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR=0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00	363.48	501.00	0.56

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 0.0 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
		(CFS)				
+	363.	8.35	355.	250.	141.	141.
		(INCHES)	0.168	0.474	0.556	0.556
		(AC-FT)	176.	496.	582.	582.

CUMULATIVE AREA = 19.65 SQ MI

*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

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*** FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION USED - ITERATION= 1

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.29	183.33	363.42	498.84	0.56	10.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03 EXCESS=0.0000E+00 OUTFLOW=0.5819E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00	363.42	501.00	0.56

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 0.5 SQ MI

	PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	363.	8.35	(CFS) 355.	250.	141.	141.
			(INCHES) 0.168	0.474	0.555	0.555
			(AC-FT) 176.	496.	582.	582.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.45	183.33	363.06	498.87	0.55	10.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5801E+03 EXCESS=0.0000E+00 OUTFLOW=0.5800E+03 BASIN STORAGE=0.3288E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00	363.06	501.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
363.	8.35	354. (CFS)	250.	141.	141.
		0.168 (INCHES)	0.473	0.553	0.553
		176. (AC-FT)	495.	580.	580.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.38	183.33	362.38	500.88	0.55	10.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5767E+03 EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3287E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00	362.38	501.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
362.	8.35	353. (CFS)	249.	140.	140.
		0.167 (INCHES)	0.471	0.550	0.550
		175. (AC-FT)	494.	577.	577.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.34	183.33	361.10	501.33	0.54	10.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03 EXCESS=0.0000E+00 OUTFLOW=0.5702E+03 BASIN STORAGE=0.3284E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00	361.10	501.00	0.54

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 361.	8.35	352.	248.	138.	138.
		(INCHES) 0.167	0.468	0.544	0.544
		(AC-FT) 175.	491.	570.	570.

CUMULATIVE AREA = 19.65 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT BSNRT1

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 362.	8.35	353.	249.	140.	140.
		(INCHES) 0.167	0.471	0.549	0.549
		(AC-FT) 175.	494.	576.	576.

CUMULATIVE AREA = 19.65 SQ MI

*** *** *** *

						PEAK		CELERITY
		(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	7.51	1.25	0.44	400.00	363.42	501.35	0.56	20.39

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03 EXCESS=0.0000E+00 OUTFLOW=0.5818E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	7.51	1.25	3.00		363.42	501.00	0.56

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT2
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)	(CFS)			
+ 363.	8.35	355.	250.	141.	141.
		(INCHES) 0.168	0.474	0.555	0.555
		(AC-FT) 176.	496.	582.	582.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	7.51	1.25	0.36	400.00	363.06	501.42	0.55	20.38

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5800E+03 EXCESS=0.0000E+00 OUTFLOW=0.5799E+03 BASIN STORAGE=0.3755E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	7.51	1.25	3.00		363.06	501.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT2
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)	(CFS)			
+ 363.	8.35	354.	250.	140.	140.
		(INCHES) 0.168	0.473	0.553	0.553
		(AC-FT) 176.	495.	580.	580.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	7.51	1.25	0.48	400.00	362.38	501.44	0.55	20.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5766E+03 EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3753E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	7.51	1.25	3.00		362.38	501.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT2
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)	(CFS)			

+							
+	362.	8.35		353.	249.	140.	140.
			(INCHES)	0.167	0.471	0.550	0.550
			(AC-FT)	175.	494.	577.	577.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	7.51	1.25	0.38	400.00	361.10	501.56	0.54	20.36

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03 EXCESS=0.0000E+00 OUTFLOW=0.5701E+03 BASIN STORAGE=0.3750E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

	MAIN	7.51	1.25	3.00	361.09	501.00	0.54

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT2
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
361.	8.35	352.	248.	138.	138.
		(INCHES) 0.167	0.468	0.544	0.544
		(AC-FT) 175.	491.	570.	570.

CUMULATIVE AREA = 19.65 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT BSNRT2

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
362.	8.35	353.	249.	139.	139.
		(INCHES) 0.167	0.471	0.549	0.549
		(AC-FT) 175.	494.	576.	576.

CUMULATIVE AREA = 19.65 SQ MI

*** *** *** *

HYDROGRAPH AT STATION PIMABY
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	4.95	605.	171.	88.	88.
		(INCHES) 0.286	0.323	0.345	0.345
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMABY
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	4.95	605.	171.	88.	88.
		(INCHES) 0.286	0.323	0.345	0.345
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMABY
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	4.95	605.	171.	88.	88.
		(INCHES) 0.286	0.323	0.345	0.345
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMABY
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	4.95	605.	171.	88.	88.
		(INCHES) 0.286	0.323	0.345	0.345
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT PIMABY

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	4.95	605.	171.	88.	88.
		(INCHES) 0.458	0.516	0.551	0.551
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *

L	3157.	CHANNEL LENGTH
S	0.0082	SLOPE
N	0.013	CHANNEL ROUGHNESS COEFFICIENT
CA	0.00	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	24.00	BOTTOM WIDTH OR DIAMETER
Z	4.00	SIDE SLOPE
NDXMIN	2	MINIMUM NUMBER OF DX INTERVALS

***

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	1.87	1.47	0.99	1052.33	1000.00	299.31	0.55	20.34

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	1.87	1.47	3.00	1000.00	300.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION PMB-RT
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
		(CFS)			
1000.	5.00	605.	171.	87.	87.
		(INCHES) 0.457	0.516	0.551	0.551
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	1.87	1.47	0.99	1052.33	1000.00	299.31	0.55	20.34

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	1.87	1.47	3.00	1000.00	300.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION PMB-RT
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
		(CFS)			
1000.	5.00	605.	171.	87.	87.
		(INCHES) 0.457	0.516	0.551	0.551
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	1.87	1.47	0.99	1052.33	1000.00	299.31	0.55	20.34

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	1.87	1.47	3.00	1000.00	300.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION PMB-RT
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	5.00	605.	171.	87.	87.
		(INCHES) 0.457	0.516	0.551	0.551
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	1.87	1.47	0.99	1052.33	1000.00	299.31	0.55	20.34

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	1.87	1.47	3.00	1000.00	300.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION PMB-RT
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	5.00	605.	171.	87.	87.
		(INCHES) 0.457	0.516	0.551	0.551
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	1.87	1.47	0.99	1052.33	1000.00	299.31	0.55	20.34

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	1.87	1.47	3.00	1000.00	300.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION PMB-RT
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	5.00	605.	171.	87.	87.
		(INCHES) 0.457	0.516	0.551	0.551
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT PMB-RT

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 1000.	5.00	605.	171.	87.	87.
		(INCHES) 0.457	0.516	0.551	0.551
		(AC-FT) 300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *

HYDROGRAPH AT STATION SB10
TRANSPOSITION AREA 2.8 SQ MI

TOTAL RAINFALL = 2.69, TOTAL LOSS = 0.50, TOTAL EXCESS = 2.19

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 142.	0.95	9.	2.	1.	1.
		(INCHES) 2.179	2.179	2.179	2.179
		(AC-FT) 5.	5.	5.	5.

CUMULATIVE AREA = 0.04 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB10
TRANSPOSITION AREA 16.0 SQ MI

TOTAL RAINFALL = 2.54, TOTAL LOSS = 0.54, TOTAL EXCESS = 2.00

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 118.	0.95	9.	2.	1.	1.
		(INCHES) 1.990	1.990	1.990	1.990
		(AC-FT) 4.	4.	4.	4.

CUMULATIVE AREA = 0.04 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB10
TRANSPOSITION AREA 90.0 SQ MI

TOTAL RAINFALL = 2.23, TOTAL LOSS = 0.57, TOTAL EXCESS = 1.66

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 88.	0.95	7.	2.	1.	1.
		(INCHES) 1.655	1.655	1.655	1.655
		(AC-FT) 4.	4.	4.	4.

CUMULATIVE AREA = 0.04 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT SB10

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 169.	0.90	10.	2.	1.	1.
		(INCHES) 2.276	2.276	2.276	2.276
		(AC-FT) 5.	5.	5.	5.

CUMULATIVE AREA = 0.04 SQ MI

*** *** *** *

+ (CFS)	(HR)					
		(CFS)				
+ 1355.	6.85		909.	420.	230.	230.
		(INCHES)	0.429	0.794	0.903	0.903
		(AC-FT)	451.	834.	948.	948.

CUMULATIVE AREA = 19.69 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-10
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1355.	6.85		909.	420.	230.	230.
		(INCHES)	0.429	0.794	0.903	0.903
		(AC-FT)	451.	834.	948.	948.

CUMULATIVE AREA = 19.69 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-10
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1355.	6.85		909.	420.	229.	229.
		(INCHES)	0.429	0.793	0.901	0.901
		(AC-FT)	451.	833.	946.	946.

CUMULATIVE AREA = 19.69 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-10
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1354.	6.85		907.	419.	228.	228.
		(INCHES)	0.429	0.791	0.897	0.897
		(AC-FT)	450.	831.	942.	942.

CUMULATIVE AREA = 19.69 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-10
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1353.	6.85		905.	417.	226.	226.
		(INCHES)	0.427	0.788	0.890	0.890
		(AC-FT)	449.	828.	935.	935.

CUMULATIVE AREA = 19.69 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT CP-10

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1354.	6.85		907.	419.	228.	228.
		(INCHES)	0.428	0.791	0.896	0.896
		(AC-FT)	450.	831.	941.	941.

CUMULATIVE AREA = 19.69 SQ MI

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              **************
              *            *
  295 KK      *    R10-11  *        ROUTE
              *            *
              **************
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UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD

HYDROGRAPH ROUTING DATA

297 RK KINEMATIC WAVE STREAM ROUTING

L	1277.	CHANNEL LENGTH
S	0.0014	SLOPE
N	0.030	CHANNEL ROUGHNESS COEFFICIENT
CA	0.00	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	220.00	BOTTOM WIDTH OR DIAMETER
Z	4.00	SIDE SLOPE
NDXMIN	2	MINIMUM NUMBER OF DX INTERVALS

***

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.06	1.62	1.62	425.67	1355.03	412.64	0.90	4.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9482E+03 EXCESS=0.0000E+00 OUTFLOW=0.9474E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.06	1.62	3.00		1354.77	411.00	0.90

*** *** *** *** ***

HYDROGRAPH AT STATION R10-11
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
	(CFS)				
+ 1355.	6.85	909.	420.	229.	229.
	(INCHES)	0.429	0.794	0.902	0.902
	(AC-FT)	451.	834.	947.	947.

CUMULATIVE AREA = 19.69 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.06	1.62	1.56	425.67	1354.98	412.72	0.90	4.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9479E+03 EXCESS=0.0000E+00 OUTFLOW=0.9473E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.06	1.62	3.00		1354.72	411.00	0.90

*** *** *** *** ***

HYDROGRAPH AT STATION R10-11
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
	(CFS)				
+ 1355.	6.85	909.	420.	229.	229.
	(INCHES)	0.429	0.794	0.902	0.902
	(AC-FT)	451.	834.	947.	947.

CUMULATIVE AREA = 19.69 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.06	1.62	1.57	425.67	1354.58	412.38	0.90	4.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9458E+03 EXCESS=0.0000E+00 OUTFLOW=0.9450E+03 BASIN STORAGE=0.9164E+00 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.06	1.62	3.00		1354.33	411.00	0.90	

*** *** *** *** ***

HYDROGRAPH AT STATION R10-11
TRANSPOSITION AREA 2.8 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
		(CFS)				
+	1354.	6.85	908.	420.	229.	229.
		(INCHES)	0.429	0.793	0.900	0.900
		(AC-FT)	450.	833.	945.	945.

CUMULATIVE AREA = 19.69 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.06	1.62	1.67	425.67	1353.83	412.54	0.90	4.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9420E+03 EXCESS=0.0000E+00 OUTFLOW=0.9412E+03 BASIN STORAGE=0.9165E+00 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.06	1.62	3.00		1353.59	411.00	0.90	

*** *** *** *** ***

HYDROGRAPH AT STATION R10-11
TRANSPOSITION AREA 16.0 SQ MI

	PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)				
		(CFS)				
+	1354.	6.85	907.	419.	228.	228.
		(INCHES)	0.428	0.791	0.896	0.896
		(AC-FT)	450.	831.	941.	941.

CUMULATIVE AREA = 19.69 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.06	1.62	1.66	425.67	1352.40	412.19	0.89	4.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9349E+03 EXCESS=0.0000E+00 OUTFLOW=0.9341E+03 BASIN STORAGE=0.9161E+00 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.06	1.62	3.00		1352.18	411.00	0.89	

*** *** *** *** ***

HYDROGRAPH AT STATION R10-11
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR

```
+   (CFS)       (HR)
                  (CFS)
+    1352.      6.85                904.        417.        226.         226.
                (INCHES)     0.427       0.788       0.889        0.889
                 (AC-FT)      448.        828.        934.         934.

                 CUMULATIVE AREA =   19.69 SQ MI

       ***            ***            ***            ***            ***


                 INTERPOLATED HYDROGRAPH AT   R10-11

  PEAK FLOW     TIME                      MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                  (CFS)
+    1353.      6.85                906.        419.        228.         228.
                (INCHES)     0.428       0.791       0.895        0.895
                 (AC-FT)      449.        831.        940.         940.

                 CUMULATIVE AREA =   19.69 SQ MI


 *** *** ***
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PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 231.	0.95	17.	4.	2.	2.
		(INCHES) 2.267	2.267	2.267	2.267
		(AC-FT) 9.	9.	9.	9.

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB11
TRANSPOSITION AREA 2.8 SQ MI

TOTAL RAINFALL = 2.69, TOTAL LOSS = 0.50, TOTAL EXCESS = 2.18

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 201.	1.00	17.	4.	2.	2.
		(INCHES) 2.172	2.172	2.172	2.172
		(AC-FT) 8.	8.	8.	8.

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB11
TRANSPOSITION AREA 16.0 SQ MI

TOTAL RAINFALL = 2.54, TOTAL LOSS = 0.55, TOTAL EXCESS = 1.99

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 169.	1.00	15.	4.	2.	2.
		(INCHES) 1.982	1.982	1.982	1.982
		(AC-FT) 8.	8.	8.	8.

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SB11
TRANSPOSITION AREA 90.0 SQ MI

TOTAL RAINFALL = 2.23, TOTAL LOSS = 0.58, TOTAL EXCESS = 1.65

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 129.	1.00	13.	3.	2.	2.
		(INCHES) 1.647	1.647	1.647	1.647
		(AC-FT) 6.	6.	6.	6.

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT SB11

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)				
		(CFS)			
+ 231.	0.95	17.	4.	2.	2.
		(INCHES) 2.270	2.270	2.270	2.270
		(AC-FT) 9.	9.	9.	9.

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *

306 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

***

*** *** *** *** ***

HYDROGRAPH AT STATION CP-11
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
1355.	6.85	909.	420.	232.	232.
		(INCHES) 0.428	0.791	0.907	0.907
		(AC-FT) 451.	834.	956.	956.

CUMULATIVE AREA = 19.76 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-11
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
1355.	6.85	909.	420.	232.	232.
		(INCHES) 0.428	0.791	0.907	0.907
		(AC-FT) 451.	834.	956.	956.

CUMULATIVE AREA = 19.76 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-11
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
1354.	6.85	908.	420.	231.	231.
		(INCHES) 0.427	0.790	0.904	0.904
		(AC-FT) 450.	833.	953.	953.

CUMULATIVE AREA = 19.76 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-11
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
1354.	6.85	907.	419.	230.	230.
		(INCHES) 0.427	0.789	0.900	0.900
		(AC-FT) 450.	831.	949.	949.

CUMULATIVE AREA = 19.76 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION CP-11
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR
1352.	6.85	904.	417.	228.	228.
		(INCHES) 0.425	0.785	0.892	0.892
		(AC-FT) 448.	828.	940.	940.

CUMULATIVE AREA = 19.76 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT CP-11

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CFS)	24-HR	72-HR	49.95-HR

+	1353.	6.85		906.	419.	230.	230.
			(INCHES)	0.426	0.788	0.899	0.899
			(AC-FT)	449.	831.	948.	948.

CUMULATIVE AREA = 19.76 SQ MI

1

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+	HYDROGRAPH AT	76THST	51.	4.55	15.	4.	2.	0.24		
+	HYDROGRAPH AT	SB01	324.	0.95	26.	6.	3.	0.11		
+	2 COMBINED AT	CP-1	323.	0.95	37.	11.	5.	0.35		
+	ROUTED TO	R1-2	315.	1.10	37.	11.	5.	0.35		
+	HYDROGRAPH AT	SB02	489.	0.95	38.	10.	5.	0.16		
+	2 COMBINED AT	CP-2	731.	1.05	75.	20.	10.	0.51		
+	ROUTED TO	R2-3	719.	1.10	75.	20.	10.	0.51		
+	HYDROGRAPH AT	SB03	129.	1.00	12.	3.	1.	0.05		
+	2 COMBINED AT	CP-3	828.	1.10	86.	23.	11.	0.56		
+	ROUTED TO	R3-5	806.	1.20	86.	23.	11.	0.56		
+	HYDROGRAPH AT	SB04	525.	0.95	36.	9.	4.	0.14		
+	ROUTED TO	R4-5	511.	1.05	37.	9.	4.	0.14		
+	HYDROGRAPH AT	SB05	399.	0.95	30.	7.	4.	0.13		
+	3 COMBINED AT	CP-5	1410.	1.15	150.	40.	19.	0.83		
+	ROUTED TO	R5-6	1386.	1.20	150.	40.	19.	0.83		
+	HYDROGRAPH AT	SB06	420.	0.95	33.	8.	4.	0.14		
+	2 COMBINED AT	CP-6	1609.	1.20	181.	48.	23.	0.96		
+	HYDROGRAPH AT	SB07	384.	0.90	17.	4.	2.	0.07		
+	ROUTED TO	R7-8	360.	1.05	18.	5.	2.	0.07		
+	HYDROGRAPH AT	SB08	792.	0.95	56.	14.	7.	0.22		
+	2 COMBINED AT	CP-8	1035.	1.05	74.	18.	9.	0.30		
+	ROUTED TO	R8-9	1010.	1.10	74.	19.	9.	0.30		
+	HYDROGRAPH AT	SB09	173.	0.95	13.	3.	2.	0.05		
+	2 COMBINED AT	CP-9	1124.	1.10	87.	22.	10.	0.35		
+	HYDROGRAPH AT	PIMACH	4017.	5.50	1092.	292.	146.	12.30		
+	DIVERSION TO	PIMABY	1000.	4.95	605.	171.	88.	12.30		
	HYDROGRAPH AT									

+	PIMAIN	3017.	5.50	486.	122.	58.	12.30
	HYDROGRAPH AT						
+	PWRCH	740.	5.80	432.	136.	75.	7.00
	3 COMBINED AT						
+	BINFLO	3634.	5.50	919.	273.	142.	19.65
	ROUTED TO						
+	BASIN	362.	8.30	353.	249.	140.	19.65
	ROUTED TO						
+	BSNRT1	362.	8.35	353.	249.	140.	19.65
	ROUTED TO						
+	BSNRT2	362.	8.35	353.	249.	139.	19.65
	HYDROGRAPH AT						
+	PIMABY	1000.	4.95	605.	171.	88.	12.30
	ROUTED TO						
+	PMB-RT	1000.	5.00	605.	171.	87.	12.30
	HYDROGRAPH AT						
+	SB10	169.	0.90	10.	2.	1.	0.04
	3 COMBINED AT						
+	CP-10	1354.	6.85	907.	419.	228.	19.69
	ROUTED TO						
+	R10-11	1353.	6.85	906.	419.	228.	19.69
	HYDROGRAPH AT						
+	SB11	231.	0.95	17.	4.	2.	0.07
	2 COMBINED AT						
+	CP-11	1353.	6.85	906.	419.	230.	19.76

1

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING

(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

						INTERPOLATED TO COMPUTATION INTERVAL			
ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1 STORM AREA (SQ MI) =			0.00						
R1-2	MANE	3.00	324.42	63.63	1.19	3.00	317.42	66.00	1.19

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2221E+02 EXCESS=0.0000E+00 OUTFLOW=0.2234E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=
-0.8

FOR STORM = 2 STORM AREA (SQ MI) =			0.50						
R1-2	MANE	3.00	321.94	63.70	1.18	3.00	314.95	66.00	1.18

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2212E+02 EXCESS=0.0000E+00 OUTFLOW=0.2223E+02 BASIN STORAGE=0.5936E-01 PERCENT ERROR=
-0.8

FOR STORM = 3 STORM AREA (SQ MI) =			2.80						
R1-2	MANE	3.00	281.88	67.46	1.15	3.00	280.31	66.00	1.15

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2153E+02 EXCESS=0.0000E+00 OUTFLOW=0.2160E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=

-0.6

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R1-2	MANE	3.00	239.43	66.99	1.09	3.00	234.05	69.00	1.09

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2036E+02 EXCESS=0.0000E+00 OUTFLOW=0.2044E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=
-0.7

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R1-2	MANE	2.91	179.26	70.19	0.98	3.00	178.38	69.00	0.98

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1834E+02 EXCESS=0.0000E+00 OUTFLOW=0.1842E+02 BASIN STORAGE=0.6416E-01 PERCENT ERROR=
-0.8

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R2-3	MANE	1.61	726.00	65.69	1.53	3.00	725.57	66.00	1.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4152E+02 EXCESS=0.0000E+00 OUTFLOW=0.4152E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.1

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R2-3	MANE	1.61	719.73	65.75	1.52	3.00	719.25	66.00	1.52

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4128E+02 EXCESS=0.0000E+00 OUTFLOW=0.4128E+02 BASIN STORAGE=0.4197E-01 PERCENT ERROR=
-0.1

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R2-3	MANE	1.65	650.44	66.16	1.47	3.00	647.31	66.00	1.47

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3985E+02 EXCESS=0.0000E+00 OUTFLOW=0.3986E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.1

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R2-3	MANE	1.61	553.64	68.36	1.37	3.00	550.82	69.00	1.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3711E+02 EXCESS=0.0000E+00 OUTFLOW=0.3710E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.1

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R2-3	MANE	1.77	421.93	69.46	1.19	3.00	418.11	69.00	1.19

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3232E+02 EXCESS=0.0000E+00 OUTFLOW=0.3234E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.2

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R3-5	MANE	2.78	817.36	73.14	1.60	3.00	816.94	72.00	1.60

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4741E+02 EXCESS=0.0000E+00 OUTFLOW=0.4763E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.6

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R3-5	MANE	2.79	810.60	70.45	1.59	3.00	810.54	72.00	1.59

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4713E+02 EXCESS=0.0000E+00 OUTFLOW=0.4736E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.7

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R3-5	MANE	2.90	742.52	73.90	1.54	3.00	740.16	72.00	1.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4551E+02 EXCESS=0.0000E+00 OUTFLOW=0.4571E+02 BASIN STORAGE=0.8226E-01 PERCENT ERROR=
-0.6

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R3-5	MANE	3.00	634.55	73.70	1.43	3.00	626.49	75.00	1.43

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4224E+02 EXCESS=0.0000E+00 OUTFLOW=0.4240E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.6

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R3-5	MANE	3.00	478.66	74.64	1.24	3.00	478.05	75.00	1.24

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3665E+02 EXCESS=0.0000E+00 OUTFLOW=0.3688E+02 BASIN STORAGE=0.8233E-01 PERCENT ERROR=
-0.8

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R4-5	MANE	2.41	520.58	62.34	2.39	3.00	513.38	63.00	2.40

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1819E+02 EXCESS=0.0000E+00 OUTFLOW=0.1838E+02 BASIN STORAGE=0.3179E-04 PERCENT ERROR=
-1.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R4-5 MANE 2.42 516.56 62.40 2.38 3.00 510.06 63.00 2.39

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1807E+02 EXCESS=0.0000E+00 OUTFLOW=0.1825E+02 BASIN STORAGE=0.4744E-04 PERCENT ERROR=
-1.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R4-5 MANE 2.55 451.64 62.77 2.29 3.00 451.16 63.00 2.30

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1745E+02 EXCESS=0.0000E+00 OUTFLOW=0.1762E+02 BASIN STORAGE=0.4583E-04 PERCENT ERROR=
-1.0

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R4-5 MANE 2.73 379.50 64.90 2.12 3.00 374.76 63.00 2.12

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1609E+02 EXCESS=0.0000E+00 OUTFLOW=0.1629E+02 BASIN STORAGE=0.4132E-04 PERCENT ERROR=
-1.2

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R4-5 MANE 2.87 291.80 65.24 1.78 3.00 288.81 66.00 1.78

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1354E+02 EXCESS=0.0000E+00 OUTFLOW=0.1370E+02 BASIN STORAGE=0.4337E-04 PERCENT ERROR=
-1.2

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R5-6 MANE 1.71 1430.15 73.26 1.84 3.00 1428.13 72.00 1.84

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8096E+02 EXCESS=0.0000E+00 OUTFLOW=0.8101E+02 BASIN STORAGE=0.7278E-01 PERCENT ERROR=
-0.1

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R5-6 MANE 1.72 1417.75 71.63 1.82 3.00 1417.68 72.00 1.82

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8047E+02 EXCESS=0.0000E+00 OUTFLOW=0.8049E+02 BASIN STORAGE=0.7282E-01 PERCENT ERROR=
-0.1

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R5-6 MANE 1.83 1320.58 72.30 1.76 3.00 1310.29 72.00 1.76

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7757E+02 EXCESS=0.0000E+00 OUTFLOW=0.7762E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR=
-0.2

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R5-6	MANE	1.92	1120.07	73.97	1.62	3.00	1117.54	75.00	1.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7160E+02 EXCESS=0.0000E+00 OUTFLOW=0.7164E+02 BASIN STORAGE=0.7274E-01 PERCENT ERROR=-0.2

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R5-6	MANE	2.16	837.38	77.35	1.39	3.00	834.70	75.00	1.39

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6127E+02 EXCESS=0.0000E+00 OUTFLOW=0.6143E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR=-0.4

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R7-8	MANE	2.79	374.54	61.97	2.32	3.00	360.59	63.00	2.31

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8717E+01 EXCESS=0.0000E+00 OUTFLOW=0.9160E+01 BASIN STORAGE=0.1297E-03 PERCENT ERROR=-5.1

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R7-8	MANE	2.80	373.06	62.02	2.31	3.00	359.23	63.00	2.30

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8655E+01 EXCESS=0.0000E+00 OUTFLOW=0.9098E+01 BASIN STORAGE=0.1282E-03 PERCENT ERROR=-5.1

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R7-8	MANE	3.00	300.60	62.65	2.18	3.00	299.77	63.00	2.18

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8267E+01 EXCESS=0.0000E+00 OUTFLOW=0.8591E+01 BASIN STORAGE=0.1511E-03 PERCENT ERROR=-3.9

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R7-8	MANE	3.00	243.19	65.55	1.99	3.00	238.95	66.00	2.00

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7502E+01 EXCESS=0.0000E+00 OUTFLOW=0.7866E+01 BASIN STORAGE=0.1118E-03 PERCENT ERROR=-4.8

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R7-8	MANE	2.82	178.83	67.24	1.61	3.00	176.29	66.00	1.61

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6168E+01 EXCESS=0.0000E+00 OUTFLOW=0.6362E+01 BASIN STORAGE=0.1260E-03 PERCENT ERROR=
-3.1

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R8-9	MANE	1.32	1031.83	65.57	2.33	3.00	1013.13	66.00	2.33

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3691E+02 EXCESS=0.0000E+00 OUTFLOW=0.3702E+02 BASIN STORAGE=0.1702E-03 PERCENT ERROR=
-0.3

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R8-9	MANE	1.33	1027.12	65.64	2.31	3.00	1010.29	66.00	2.32

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3666E+02 EXCESS=0.0000E+00 OUTFLOW=0.3678E+02 BASIN STORAGE=0.2024E-03 PERCENT ERROR=
-0.3

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R8-9	MANE	1.45	913.69	66.38	2.21	3.00	911.34	66.00	2.21

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3510E+02 EXCESS=0.0000E+00 OUTFLOW=0.3519E+02 BASIN STORAGE=0.1735E-03 PERCENT ERROR=
-0.3

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R8-9	MANE	1.48	772.14	65.64	2.03	3.00	769.23	66.00	2.03

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3216E+02 EXCESS=0.0000E+00 OUTFLOW=0.3228E+02 BASIN STORAGE=0.1954E-03 PERCENT ERROR=
-0.4

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R8-9	MANE	1.59	554.15	67.90	1.68	3.00	554.05	69.00	1.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2666E+02 EXCESS=0.0000E+00 OUTFLOW=0.2676E+02 BASIN STORAGE=0.1898E-03 PERCENT ERROR=
-0.4

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

BSNRT1	MANE	0.29	363.48	498.70	0.56	3.00	363.48	501.00	0.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03 EXCESS=0.0000E+00 OUTFLOW=0.5822E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR=
0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

BSNRT1	MANE	0.29	363.42	498.84	0.56	3.00	363.42	501.00	0.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03 EXCESS=0.0000E+00 OUTFLOW=0.5819E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR=
0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

BSNRT1	MANE	0.45	363.06	498.87	0.55	3.00	363.06	501.00	0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5801E+03 EXCESS=0.0000E+00 OUTFLOW=0.5800E+03 BASIN STORAGE=0.3288E-01 PERCENT ERROR=
0.0

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

BSNRT1	MANE	0.38	362.38	500.88	0.55	3.00	362.38	501.00	0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5767E+03 EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3287E-01 PERCENT ERROR=
0.0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

BSNRT1	MANE	0.34	361.10	501.33	0.54	3.00	361.10	501.00	0.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03 EXCESS=0.0000E+00 OUTFLOW=0.5702E+03 BASIN STORAGE=0.3284E-01 PERCENT ERROR=
0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

BSNRT2	MANE	0.44	363.48	501.11	0.56	3.00	363.48	501.00	0.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03 EXCESS=0.0000E+00 OUTFLOW=0.5821E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR=
0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

BSNRT2	MANE	0.44	363.42	501.35	0.56	3.00	363.42	501.00	0.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03 EXCESS=0.0000E+00 OUTFLOW=0.5818E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR=
0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

BSNRT2	MANE	0.36	363.06	501.42	0.55	3.00	363.06	501.00	0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5800E+03 EXCESS=0.0000E+00 OUTFLOW=0.5799E+03 BASIN STORAGE=0.3755E-01 PERCENT ERROR=
0.0

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

BSNRT2	MANE	0.48	362.38	501.44	0.55	3.00	362.38	501.00	0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5766E+03 EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3753E-01 PERCENT ERROR=0.0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

BSNRT2	MANE	0.38	361.10	501.56	0.54	3.00	361.09	501.00	0.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03 EXCESS=0.0000E+00 OUTFLOW=0.5701E+03 BASIN STORAGE=0.3750E-01 PERCENT ERROR=0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

PMB-RT	MANE	0.99	1000.00	299.31	0.55	3.00	1000.00	300.00	0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

PMB-RT	MANE	0.99	1000.00	299.31	0.55	3.00	1000.00	300.00	0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

PMB-RT	MANE	0.99	1000.00	299.31	0.55	3.00	1000.00	300.00	0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=0.0

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

PMB-RT	MANE	0.99	1000.00	299.31	0.55	3.00	1000.00	300.00	0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=0.0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

PMB-RT	MANE	0.99	1000.00	299.31	0.55	3.00	1000.00	300.00	0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=

0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R10-11	MANE	1.62	1355.03	412.64	0.90	3.00	1354.77	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9482E+03 EXCESS=0.0000E+00 OUTFLOW=0.9474E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR=
0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R10-11	MANE	1.56	1354.98	412.72	0.90	3.00	1354.72	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9479E+03 EXCESS=0.0000E+00 OUTFLOW=0.9473E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR=
0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R10-11	MANE	1.57	1354.58	412.38	0.90	3.00	1354.33	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9458E+03 EXCESS=0.0000E+00 OUTFLOW=0.9450E+03 BASIN STORAGE=0.9164E+00 PERCENT ERROR=
0.0

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R10-11	MANE	1.67	1353.83	412.54	0.90	3.00	1353.59	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9420E+03 EXCESS=0.0000E+00 OUTFLOW=0.9412E+03 BASIN STORAGE=0.9165E+00 PERCENT ERROR=
0.0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R10-11	MANE	1.66	1352.40	412.19	0.89	3.00	1352.18	411.00	0.89

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9349E+03 EXCESS=0.0000E+00 OUTFLOW=0.9341E+03 BASIN STORAGE=0.9161E+00 PERCENT ERROR=
0.0

*** NORMAL END OF HEC-1 ***

Appendix C
Cavasson HEC-1 Data
Vi at Cavasson

ID	Method	Duration	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
DEFAULT	NOAA14	5 MIN	0.258	0.349	0.418	0.511	0.583	0.656
	NOAA14	10 MIN	0.393	0.530	0.636	0.778	0.887	0.998
	NOAA14	15 MIN	0.487	0.658	0.789	0.965	1.099	1.237
	NOAA14	30 MIN	0.656	0.886	1.063	1.299	1.480	1.666
	NOAA14	1 HOUR	0.812	1.096	1.315	1.608	1.832	2.062
	NOAA14	2 HOUR	0.939	1.251	1.490	1.814	2.060	2.315
	NOAA14	3 HOUR	1.024	1.339	1.588	1.936	2.212	2.498
	NOAA14	6 HOUR	1.215	1.549	1.817	2.182	2.465	2.760
	NOAA14	12 HOUR	1.374	1.734	2.016	2.399	2.690	2.993
	NOAA14	24 HOUR	1.622	2.097	2.476	3.005	3.425	3.865

Soil Map—Aguila-Carefree Area, Arizona, Parts of Maricopa and Pinal Counties

Map Scale: 1:17,700 if printed on B landscape (17" x 11") sheet.

N

Meters: 0, 250, 500, 1000, 1500

Feet: 0, 500, 1000, 2000, 3000

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 12N WGS84

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Aguila-Carefree Area, Arizona, Parts of Maricopa and Pinal Counties
Survey Area Data: Version 11, Sep 11, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Oct 31, 2014—Dec 7, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
55	Gilman loams	768.3	30.2%
90	Momoli gravelly sandy loam, 1 to 5 percent slopes	1,778.8	69.8%
Totals for Area of Interest		**2,547.2**	**100.0%**

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 8/14/2020

The following spreadsheets were used to prepare input data for HEC-1 Green and Ampt Method described in The Drainage Design Manual for Maricopa County

Spreadsheet equations and assumptions were as follows:

SHEET 1: *Summary* — Summary of HEC-1 Input.

SHEET 2: *Basin Geometry* — Drainage areas, watercourse length, slope, adjusted slope, land type, and K_b.

Slopes were adjusted according to Figure 5.4.

Land type and K_b were determined using Table 5.1.

SHEET 3: *Green-Ampt Parameters* — Soil characteristics, XKSAT, PSIF, DTHETA, and RTEMP.

Soil type and percentages determined from Flood Control soils map.

XKSAT determined using Appendix A. If applicable a logrithmic weighted value was determined for multiple soils within a basin according to the following equation:

$$\overline{XSAT} = ALOG \sum \left[\%\,SOIL\,TYPE(IN\,DECIMAL) \cdot LOG(XSAT) \right]$$

Where: XSAT determined from Appendix A

PSIF and DTHETA determined using Figure 4.3.

RTEMP is a weighted value based on soil data. The following equation was used to determine the weighted value:

$$RTEMP = \sum \%\,SOIL\,TYPE \cdot \left[\%ROCK\,OUTCROP(IN\,DECIMAL) \right]$$

These values may be used for predeveloped conditions.

SHEET 4: *Land Use Parameters* — Land use categories, IA, and RTEMP. IA and RTEMP are weighted based on percentage of land use. These values may be used for post developement conditions.

SHEET 5: Retention Required — This is the Proposed Condition Retention that will be required for each SubBasin. It is calculated as the FIRST FLUSH per the Development Agreement (Construction of Offsite Powerline Channel)
The Formula used is from COS: Vol = Cw*(P/12)*Area
In HEC-1 this will be input as a Divert and not brought back
The Divert will be labeled: DIVXXX, where XXX is the Basin ID

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

	SEE SHEET _ FOR DETERMINATION OF THE FOLLOWING INPUT								
Sheet # →	2	2	4	2	4	3	3	3	4
BASIN NUMBER	BASIN AREA ($Miles^2$)	WATERCOURSE LENGTH (Miles)	Kb WEIGHTED	SLOPE ADJUSTED (Ft/Mile)	IA WEIGHTED	(in)	PSIF (in)	$\overline{XKSAT}$ (in/hr)	GREATER OF RTEMP SOIL OR WEIGHTED
SB01-C	0.0270	0.3286	0.032	96	0.25	0.35	4.3	0.5778	45%
SB01-B	0.0240	0.2750	0.033	115	0.25	0.35	4.3	0.5778	45%
SB01-A	0.0610	0.2750	0.030	121	0.25	0.35	4.3	0.5778	45%
SB02NE	0.0335	0.3873	0.032	72	0.25	0.35	4.3	0.5778	45%
SB02NW	0.0193	0.1919	0.033	86	0.25	0.35	4.3	0.5778	45%
SB02W	0.0571	0.3483	0.111	88	0.35	0.35	4.3	0.5111	0%
SB02E	0.0440	0.3557	0.031	82	0.10	0.35	4.3	0.6222	80%
SB8NW	0.0147	0.1445	0.034	83	0.10	0.35	4.3	0.6222	80%
SB8NE	0.0150	0.1405	0.034	85	0.10	0.35	4.3	0.6222	80%
SB8SW	0.0129	0.1383	0.034	116	0.10	0.35	4.3	0.6222	80%
SB8SE	0.0131	0.1580	0.034	104	0.10	0.35	4.3	0.6222	80%
SB08W	0.0589	0.3428	0.030	64	0.10	0.35	4.3	0.6222	80%
SB07W	0.0468	0.3138	0.031	99	0.25	0.35	4.3	0.5778	45%

SUM= 0.4272

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

						BASIN ELEVATION			SLOPE		
BASIN	BASIN AREA			WATER COURSE LENGTH			Low	Δ Elevation			ADJUSTED
NUMBER	(Feet^2)	(ACRES)	(Miles^2)	(Feet)	(Miles)	(Feet)	(Feet)	(Feet)	(Ft/Ft)	(Ft/Mile)	(Ft/Mile)
SB01-C	752,914	17.28	0.0270	1735	0.329	1681.0	1649.3	31.7	0.0183	96.47	96.5
SB01-B	668,111	15.34	0.0240	1452	0.275	1668.5	1637.0	31.5	0.0217	114.55	114.5
SB01-A	1,700,557	39.04	0.0610	1452	0.275	1659.3	1626.0	33.3	0.0229	121.09	121.1
SB02NE	934,725	21.46	0.0335	2045	0.387	1652.0	1624.0	28.0	0.0137	72.29	72.3
SB02NW	537,015	12.33	0.0193	1013	0.192	1640.5	1624.0	16.5	0.0163	86.00	86.0
SB02W	1,592,151	36.55	0.0571	1839	0.348	1634.5	1604.0	30.5	0.0166	87.57	87.6
SB02E	1,227,661	28.18	0.0440	1878	0.356	1633.2	1604.0	29.2	0.0155	82.10	82.1
SB8NW	408,919	9.39	0.0147	763	0.145	1654.0	1642.0	12.0	0.0157	83.04	83.0
SB8NE	417,750	9.59	0.0150	742	0.141	1654.0	1642.0	12.0	0.0162	85.39	85.4
SB8SW	358,807	8.24	0.0129	730	0.138	1642.0	1626.0	16.0	0.0219	115.73	115.7
SB8SE	365,480	8.39	0.0131	834	0.158	1642.5	1626.0	16.5	0.0198	104.46	104.5
SB08W	1,641,602	37.69	0.0589	1810	0.343	1634.0	1612.0	22.0	0.0122	64.18	64.2
SB07W	1,303,601	29.93	0.0468	1657	0.314	1685.0	1654.0	31.0	0.0187	98.78	98.8
SUM=	11,909,293	273	0.4272								

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA (Miles^2)	SOIL TYPE	% OF SOIL TYPE	ROCK OUTCROP % OF SOIL IMPERVIOUS	XKSAT (in/hr)	XKSAT ADJUSTED FOR 60% VEGATATION*	$\overline{XKSAT}$ (in/hr)	PSIF (in)	DTHETA (in)	RTEMP SOIL IN SUBBASIN
SB01-C	0.0270	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB01-B	0.0240	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB01-A	0.0610	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB02NE	0.0335	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB02NW	0.0193	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB02W	0.0571	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02E	0.0440	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8NW	0.0147	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8NE	0.0150	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8SW	0.0129	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8SE	0.0131	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB08W	0.0589	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB07W	0.0468	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%

*60% vegetation was used for C2 land uses, MFR land uses used 50% and NDR used 35% vegetation

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA ACRES	LAND USE	% OF BASIN	SUB AREA ACRES	LAND TYPE	K_b	K_b WEIGHTED	IA FOR LAND USE	IA WEIGHTED	RTEMP FOR SOIL IN BASIN	RTEMP FOR LAND USE	RTEMP WEIGHTED	RTEMP MAX
SB01-C	17.28	MFR	100%	17.28	A	0.0323	0.0323	0.25	0.25	0%	45%	45%	45%
SB01-B	15.34	MFR	100%	15.34	A	0.0326	0.0326	0.25	0.25	0%	45%	45%	45%
SB01-A	39.04	MFR	100%	39.04	A	0.0301	0.0301	0.25	0.25	0%	45%	45%	45%
SB02NE	21.46	MFR	100%	21.46	A	0.0317	0.0317	0.25	0.25	0%	45%	45%	45%
SB02NW	12.33	MFR	100%	12.33	A	0.0332	0.0332	0.25	0.25	0%	45%	45%	45%
SB02W	36.55	NDR	100%	36.55	C	0.1109	0.1109	0.35	0.35	0%	0%	0%	0%
SB02E	28.18	C2	100%	28.18	A	0.0309	0.0309	0.1	0.10	0%	80%	80%	80%
SB8NW	9.39	C2	100%	9.39	A	0.0339	0.0339	0.1	0.10	0%	80%	80%	80%
SB8NE	9.59	C2	100%	9.59	A	0.0339	0.0339	0.1	0.10	0%	80%	80%	80%
SB8SW	8.24	C2	100%	8.24	A	0.0343	0.0343	0.1	0.10	0%	80%	80%	80%
SB8SE	8.39	C2	100%	8.39	A	0.0342	0.0342	0.1	0.10	0%	80%	80%	80%
SB08W	37.69	C2	100%	37.69	A	0.0301	0.0301	0.1	0.10	0%	80%	80%	80%
SB07W	29.93	MFR	100%	29.93	A	0.0308	0.0308	0.25	0.25	0%	45%	45%	45%
SUM=	273			273									

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET

					Volume Required	
BASIN	BASIN AREA			Weighted C		
NUMBER	(Feet^2)	(ACRES)	(Miles^2)	Proposed Cond.	[acre-ft]	[ft^3]
SB01-C	752,914	17.28	0.0270	0.85	0.612	26,665.7
SB01-B	668,111	15.34	0.0240	0.85	0.543	23,662.3
SB01-A	1,700,557	39.04	0.0610	0.85	1.383	60,228.1
SB02NE	934,725	21.46	0.0335	0.85	0.760	33,104.9
SB02NW	537,015	12.33	0.0193	0.85	0.437	19,019.3
SB02W	1,592,151	36.55	0.0571	0.85	1.295	56,388.7
SB02E	1,227,661	28.18	0.0440	0.85	0.998	43,479.6
SB8NW	408,919	9.39	0.0147	0.85	0.332	14,482.5
SB8NE	417,750	9.59	0.0150	0.85	0.340	14,795.3
SB8SW	358,807	8.24	0.0129	0.85	0.292	12,707.8
SB8SE	365,480	8.39	0.0131	0.85	0.297	12,944.1
SB08W	1,641,602	37.69	0.0589	0.85	1.335	58,140.1
SB07W	1,303,601	29.93	0.0468	0.85	1.060	46,169.2
SUM=	11,909,293	273	0.43		9.683	421,787.5

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 8/14/2020

The following spreadsheets were used to prepare input data for HEC-1 Green and Ampt Method described in The Drainage Design Manual for Maricopa County

Spreadsheet equations and assumptions were as follows:

SHEET 1: *Summary* — Summary of HEC-1 Input.

SHEET 2: *Basin Geometry* — Drainage areas, watercourse length, slope, adjusted slope, land type, and K_b.

Slopes were adjusted according to Figure 5.4.

Land type and K_b were determined using Table 5.1.

SHEET 3: *Green-Ampt Parameters* — Soil characteristics, XKSAT, PSIF, DTHETA, and RTEMP.

Soil type and percentages determined from Flood Control soils map.

XKSAT determined using Appendix A. If applicable a logrithmic weighted value was determined for multiple soils within a basin according to the following equation:

$$\overline{XSAT} = ALOG\sum\left[\%\,SOIL\,TYPE(IN\,DECIMAL)\cdot LOG(XSAT)\right]$$

Where: XSAT determined from Appendix A

PSIF and DTHETA determined using Figure 4.3.

RTEMP is a weighted value based on soil data. The following equation was used to determine the weighted value:

$$RTEMP = \sum \%\,SOIL\,TYPE\cdot\left[\%\,ROCK\,OUTCROP(IN\,DECIMAL)\right]$$

These values may be used for predeveloped conditions.

SHEET 4: *Land Use Parameters* — Land use categories, IA, and RTEMP. IA and RTEMP are weighted based on percentage of land use. These values may be used for post developement conditions.

SHEET 5: Retention Required — This is the Proposed Condition Retention that will be required for each SubBasin. It is calculated as the FIRST FLUSH per the Development Agreement (Construction of Offsite Powerline Channel)
The Formula used is from COS: Vol = Cw*(P/12)*Area
In HEC-1 this will be input as a Divert and not brought back
The Divert will be labeled: DIVXXX, where XXX is the Basin ID

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

	SEE SHEET _ FOR DETERMINATION OF THE FOLLOWING INPUT								
Sheet # →	2	2	4	2	4	3	3	3	4
BASIN NUMBER	BASIN AREA (Miles^2)	WATERCOURSE LENGTH (Miles)	Kb WEIGHTED	SLOPE ADJUSTED (Ft/Mile)	IA WEIGHTED	DTHETA (in)	PSIF (in)	$\overline{XKSAT}$ (in/hr)	GREATER OF RTEMP SOIL OR WEIGHTED
SB01-C	0.0270	0.3286	0.119	96	0.35	0.35	4.3	0.5111	0%
SB01-B	0.0240	0.2750	0.120	115	0.35	0.35	4.3	0.5111	0%
SB01-A	0.0610	0.2750	0.110	121	0.35	0.35	4.3	0.5111	0%
SB02NE	0.0335	0.3873	0.117	72	0.35	0.35	4.3	0.5111	0%
SB02NW	0.0193	0.1919	0.123	86	0.35	0.35	4.3	0.5111	0%
SB02W	0.0571	0.3483	0.111	88	0.35	0.35	4.3	0.5111	0%
SB02E	0.0440	0.3557	0.114	82	0.35	0.35	4.3	0.5111	0%
SB8NW	0.0147	0.1445	0.126	83	0.35	0.35	4.3	0.5111	0%
SB8NE	0.0150	0.1405	0.125	85	0.35	0.35	4.3	0.5111	0%
SB8SW	0.0129	0.1383	0.127	116	0.35	0.35	4.3	0.5111	0%
SB8SE	0.0131	0.1580	0.127	104	0.35	0.35	4.3	0.5111	0%
SB08W	0.0589	0.3428	0.030	64	0.10	0.35	4.3	0.5111	80%
SB07W	0.0468	0.3138	0.113	99	0.35	0.35	4.3	0.5111	0%

SUM= 0.4272

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

						BASIN ELEVATION			SLOPE		
BASIN	BASIN AREA			WATER COURSE LENGTH		High	Low	Δ Elevation			ADJUSTED
NUMBER	(Feet^2)	(ACRES)	(Miles^2)	(Feet)	(Miles)	(Feet)	(Feet)	(Feet)	(Ft/Ft)	(Ft/Mile)	(Ft/Mile)
SB01-C	752,914	17.28	0.0270	1735	0.329	1681.0	1649.3	31.7	0.0183	96.47	96.5
SB01-B	668,111	15.34	0.0240	1452	0.275	1668.5	1637.0	31.5	0.0217	114.55	114.5
SB01-A	1,700,557	39.04	0.0610	1452	0.275	1659.3	1626.0	33.3	0.0229	121.09	121.1
SB02NE	934,725	21.46	0.0335	2045	0.387	1652.0	1624.0	28.0	0.0137	72.29	72.3
SB02NW	537,015	12.33	0.0193	1013	0.192	1640.5	1624.0	16.5	0.0163	86.00	86.0
SB02W	1,592,151	36.55	0.0571	1839	0.348	1634.5	1604.0	30.5	0.0166	87.57	87.6
SB02E	1,227,661	28.18	0.0440	1878	0.356	1633.2	1604.0	29.2	0.0155	82.10	82.1
SB8NW	408,919	9.39	0.0147	763	0.145	1654.0	1642.0	12.0	0.0157	83.04	83.0
SB8NE	417,750	9.59	0.0150	742	0.141	1654.0	1642.0	12.0	0.0162	85.39	85.4
SB8SW	358,807	8.24	0.0129	730	0.138	1642.0	1626.0	16.0	0.0219	115.73	115.7
SB8SE	365,480	8.39	0.0131	834	0.158	1642.5	1626.0	16.5	0.0198	104.46	104.5
SB08W	1,641,602	37.69	0.0589	1810	0.343	1634.0	1612.0	22.0	0.0122	64.18	64.2
SB07W	1,303,601	29.93	0.0468	1657	0.314	1685.0	1654.0	31.0	0.0187	98.78	98.8
SUM=	11,909,293	273	0.4272								

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA (Miles^2)	SOIL TYPE	% OF SOIL TYPE	ROCK OUTCROP % OF SOIL IMPERVIOUS	XKSAT (in/hr)	XKSAT ADJUSTED FOR 35% VEGATATION	$\overline{XKSAT}$ (in/hr)	PSIF (in)	DTHETA (in)	RTEMP SOIL IN SUBBASIN
SB01-C	0.0270	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB01-B	0.0240	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB01-A	0.0610	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02NE	0.0335	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02NW	0.0193	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02W	0.0571	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02E	0.0440	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8NW	0.0147	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8NE	0.0150	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8SW	0.0129	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8SE	0.0131	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB08W	0.0589	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB07W	0.0468	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%

Check: 0.4272

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA ACRES	LAND USE	% OF BASIN	SUB AREA ACRES	LAND TYPE	K_b	K_b WEIGHTED	IA FOR LAND USE	IA WEIGHTED	RTEMP FOR SOIL IN BASIN	RTEMP FOR LAND USE	RTEMP WEIGHTED	RTEMP MAX
SB01-C	17.28	NDR	100%	17.28	C	0.1191	0.1191	0.35	0.35	0%	0%	0%	0%
SB01-B	15.34	NDR	100%	15.34	C	0.1204	0.1204	0.35	0.35	0%	0%	0%	0%
SB01-A	39.04	NDR	100%	39.04	C	0.1102	0.1102	0.35	0.35	0%	0%	0%	0%
SB02NE	21.46	NDR	100%	21.46	C	0.1167	0.1167	0.35	0.35	0%	0%	0%	0%
SB02NW	12.33	NDR	100%	12.33	C	0.1227	0.1227	0.35	0.35	0%	0%	0%	0%
SB02W	36.55	NDR	100%	36.55	C	0.1109	0.1109	0.35	0.35	0%	0%	0%	0%
SB02E	28.18	NDR	100%	28.18	C	0.1138	0.1138	0.35	0.35	0%	0%	0%	0%
SB8NW	9.39	NDR	100%	9.39	C	0.1257	0.1257	0.35	0.35	0%	0%	0%	0%
SB8NE	9.59	NDR	100%	9.59	C	0.1255	0.1255	0.35	0.35	0%	0%	0%	0%
SB8SW	8.24	NDR	100%	8.24	C	0.1271	0.1271	0.35	0.35	0%	0%	0%	0%
SB8SE	8.39	NDR	100%	8.39	C	0.1269	0.1269	0.35	0.35	0%	0%	0%	0%
SB08W	37.69	C2	100%	37.69	A	0.0301	0.0301	0.1	0.10	0%	80%	80%	80%
SB07W	29.93	NDR	100%	29.93	C	0.1131	0.1131	0.35	0.35	0%	0%	0%	0%
SUM=	273			273									

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

					Volume Required		
BASIN	BASIN AREA			Weighted C			
NUMBER	(Feet^2)	(ACRES)	(Miles^2)	Proposed Cond.	[acre-ft]	[ft^3]	eet)
SB01-C	752,914	17.28	0.0270	0.85	0.612	26,665.7	
SB01-B	668,111	15.34	0.0240	0.85	0.543	23,662.3	
SB01-A	1,700,557	39.04	0.0610	0.85	1.383	60,228.1	
SB02NE	934,725	21.46	0.0335	0.85	0.760	33,104.9	
SB02NW	537,015	12.33	0.0193	0.85	0.437	19,019.3	
SB02W	1,592,151	36.55	0.0571	0.85	1.295	56,388.7	
SB02E	1,227,661	28.18	0.0440	0.85	0.998	43,479.6	
SB8NW	408,919	9.39	0.0147	0.85	0.332	14,482.5	
SB8NE	417,750	9.59	0.0150	0.85	0.340	14,795.3	
SB8SW	358,807	8.24	0.0129	0.85	0.292	12,707.8	
SB8SE	365,480	8.39	0.0131	0.85	0.297	12,944.1	
SB08W	1,641,602	37.69	0.0589	0.85	1.335	58,140.1	
SB07W	1,303,601	29.93	0.0468	0.85	1.060	46,169.2	
SUM=	11,909,293	273	0.43		9.683	421,787.5	

Appendix D
Cavasson HEC-1 Ultimate
Vi at Cavasson

10yr-6hr-Ultimate

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1****************************************
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 *                                      *                                                    *
*
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)  *                                                    *    U.S. ARMY CORPS OF ENGINEERS
*
 *               JUN   1998             *                                                    *    HYDROLOGIC ENGINEERING CENTER
*
 *            VERSION 4.1               *                                                    *          609 SECOND STREET
*
 *                                      *                                                    *       DAVIS, CALIFORNIA 95616
*
 *  RUN DATE   30APR21  TIME  13:35:28  *                                                    *           (916) 756-1104
*
 *                                      *                                                    *
*
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X     X  XXXXXXX   XXXXX           X
X     X  X        X     X         XX
X     X  X        X                X
XXXXXXX  XXXX     X        XXXXX   X
X     X  X        X                X
X     X  X        X     X          X
X     X  XXXXXXX   XXXXX          XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


```
          LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

            1           ID        CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
            2           ID        POWERLINE CORRIDOR
            3           ID        RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
            4           ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
            5           ID        10-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
            6           ID           PIMA ROAD FLOWS.
            7           ID
            8           ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
            9           ID        MODELERS: RK, MW
           10           ID
           11           ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
           12           ID        MODELERS: MSW, ES
           13           ID        REVISIONS NOTED WITH HE
           14           ID           ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
           15           ID           THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
           16           ID           UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.
           17           ID
           18           IT       3       0       0    1000
           19           IN      15
           20           IO       5
                        *DIAGRAM
                        *
           21           JD   1.817  0.0001
           22           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
           23           PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
           24           PC   0.962   0.972   0.983   0.991   1.000
           25           JD   1.806  0.5000
           26           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
           27           PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
           28           PC   0.962   0.972   0.983   0.991   1.000
           29           JD   1.772     2.8
           30           PC   0.000   0.009   0.016   0.025   0.034   0.042   0.051   0.059   0.068   0.077
           31           PC   0.088   0.101   0.121   0.164   0.253   0.451   0.694   0.836   0.900   0.938
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          32          PC   0.950   0.963   0.975   0.988   1.000
                      *

          33          KK  76THST
          34          KM        76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
          35          KM        10-YR, 6-HR, DERIVED FROM 100-YR DATA HYDROGRAPHS
          36          BA    0.24
          37          QI       0       0       0       0       0       0       0       0       0       0
          38          QI       0       0    0.01    0.01    0.06    0.58    8.13   27.04   31.94    31.1
          39          QI   22.07   17.10   13.79   11.20    9.48    8.22    7.44    6.76    6.13    5.32
          40          QI    4.66    4.31    3.73    3.05    2.71    2.44    2.18    2.27    1.51    1.61
          41          QI    1.48    1.38    1.29    1.17    1.01    0.96    0.97    0.77    0.84    0.86
          42          QI    0.70    0.68    0.67    0.62    0.58    0.55    0.53    0.51    0.49    0.46
          43          QI    0.44    0.43    0.42    0.40    0.38    0.37    0.35    0.34    0.33    0.31
          44          QI    0.30    0.29    0.28
                      *
1                                        HEC-1 INPUT                                                PAGE  2

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


          45          KK   SB01A   BASIN
          46          BA   0.061
          47          PB   2.721
          48          PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.075
          49          PC   0.087   0.099   0.119   0.150   0.234   0.413   0.766   0.875   0.916   0.944
          50          PC   0.956   0.968   0.979   0.990   1.000
          51          LG    0.25    0.25    4.03    0.56      45
          52          UC   0.439   0.231
          53          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
          54          UA     100
                      *

          55          KK   SB01A  DIVERT
          56          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01A
          57          KM        FIRST FLUSH VOL = 1.383 AC-FT
          58          DT  RET01A   1.383
          59          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
          60          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                      *

          61          KK    CP-1 COMBINE
          62          KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
          63          HC       2   0.301
                      *

          64          KK  R1A-2W   ROUTE
          65          KM        ROUTE CP-1 TO CP-2W
          66          RK    1114  0.0200   0.018            TRAP   2.000    0.00
                      *

          67          KK  SB02NW   BASIN
          68          KM        BASIN SB02E WAS SPLIT INTO SUB BASINS NORTH OF CAVASSON
          69          KM        AND SOUTH OF CAVASSON
          70          BA  0.0193
          71          LG    0.25    0.25    4.03    0.56      45
          72          UC   0.211   0.167
          73          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
          74          UA     100
                      *

          75          KK  SB02NW  DIVERT
          76          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NW
          77          KM        FIRST FLUSH VOL = 0.437 AC-FT
          78          DT  RET2NW   0.437
          79          DI     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
          80          DI    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
          81          DQ     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
          82          DQ    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
                      *
1                                        HEC-1 INPUT                                                PAGE  3

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


          83          KK  CP-02W COMBINE
          84          KM        COMBINE ROUTE FROM 1A-2W WITH NEW SUB BASIN SB02NW
          85          HC       2   0.305
                      *
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 86          KK   R2W-2   ROUTE
 87          RK    1380  0.0140   0.018            TRAP   2.000    0.00
 88
             *

 89          KK   SB01C   BASIN
 90          BA   0.027
 91          LG    0.25    0.25    4.03    0.56      45
 92          UC   0.264   0.272
 93          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 94          UA     100
             *

 95          KK   SB01C  DIVERT
 96          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01C
 97          KM        FIRST FLUSH VOL = 0.612 AC-FT
 98          DT  RET01C   0.612
 99          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
100          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *

101          KK  R1C-1B   ROUTE
102          RK     542   0.005   0.013            CIRC   5.000
             *

103          KK   SB01B   BASIN
104          BA   0.024
105          LG    0.25    0.25    4.03    0.56      45
106          UC   0.234   0.224
107          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
108          UA     100
             *

109          KK   SB01B  DIVERT
110          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01B
111          KM        FIRST FLUSH VOL = 0.543 AC-FT
112          DT  RET01B   0.543
113          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
114          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *

115          KK  CP-01B COMBINE
116          KM        COMBINE ROUTE FROM 1C-1B WITH NEW SUB BASIN SB01B
117          HC       2   0.051
             *
```



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LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

118          KK  R1B-2N   ROUTE
119          RK    2144   0.008   0.013            CIRC   5.000
             *

120          KK  SB02NE   BASIN
121          BA  0.0335
122          LG    0.25    0.25    4.03    0.56      45
123          UC   0.314   0.336
124          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
125          UA     100
             *

126          KK  SB02NE  DIVERT
127          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NE
128          KM        FIRST FLUSH VOL = 0.760 AC-FT
129          DT  RET2NE   0.760
130          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
131          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *

132          KK  CP-02N COMBINE
133          KM        COMBINE ROUTE 1B-2N (BASINS SB01-B AND SB01-C) WITH SB02NE
134          HC       2   0.085
             *

135          KK   R2N-2   ROUTE
136          RK    1380   0.014   0.018            TRAP   2.000    0.00
             *

137          KK   SB02E   BASIN
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```
 138          BA   0.040
 139          LG    0.10    0.25    4.03    0.61      80
 140          UC   0.254   0.212
 141          UA      0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 142          UA    100
              *

 143          KK   SB02E  DIVERT
 144          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E
 145          KM        FIRST FLUSH VOL = 0.998 AC-FT
 146          DT   RET2E   0.998
 147          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 148          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 149          KK   SB02W   BASIN
 150          BA  0.0571
 151          LG    0.35    0.35    4.33    0.51       0
 152          UC   0.686   0.535
 153          UA      0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 154          UA    100
              *
1                                        HEC-1 INPUT                                          PAGE  5

            LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 155          KK   CP-02 COMBINE
 156          HC       4  0.1816
              *

 157          KK    R2-3   ROUTE
 158          KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
 159          RK    1260  0.0015   0.030            TRAP      92       4
              *

 160          KK    SB03   BASIN
 161          BA   0.048
 162          LG    0.15    0.25    4.50    0.47      55
 163          UC   0.364   0.294
 164          UA      0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 165          UA    100
              *

 166          KK   CP-03 COMBINE
 167          HC       2
              *

 168          KK    R3-5   ROUTE
 169          KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
 170          RK    2396  0.0015   0.03             TRAP      98       4
              *

 171          KK    SB05   BASIN
 172          BA   0.126
 173          LG    0.22    0.25    4.50    0.44      48
 174          UC   0.327   0.226
 175          UA      0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 176          UA    100
              *

 177          KK   CP-5A COMBINE
 178          HC       2
              *

 179          KK   SB07W   BASIN
 180          BA  0.0468
 181          LG    0.25    0.25    4.03    0.56      45
 182          UC   0.250   0.178
 183          UA      0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 184          UA    100
              *

 185          KK   SB07W  DIVERT
 186          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB07W
 187          KM        FIRST FLUSH VOL = 1.060 AC-FT
 188          DT  RET07W   1.060
 189          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 190          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *
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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

  191     KK  R7W-8W   ROUTE
  192     KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
  193     KM        HE MODIFIED ROUTE TO LOOP 101 W OF HAYDEN
  194     RK    3754  0.0013   0.03            TRAP      46       4
          *

  195     KK   SB8NW   BASIN
  196     KM        BASIN JUST SOUTH OF LEGACY - FUTURE MOB SITE
  197     BA  0.0147
  198     LG   0.10    0.25    4.03    0.61      80
  199     UC   0.171   0.131
  200     UA      0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  201     UA    100
          *

  202     KK   SB8NW  DIVERT
  203     KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NW
  204     KM        FIRST FLUSH VOL = 0.332 AC-FT
  205     DT  RET8NW   0.332
  206     DI    0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  207     DQ    0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
          *

  208     KK  R8NW8N   ROUTE
  209     KM        ROUTE NEW SUB BASIN SB8NW TO OUTLET AT CAVASSON BLVD
  210     KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
  211     RK     643  0.0113   0.018            TRAP      46       4
          *

  212     KK   SB8NE   BASIN
  213     KM        SITE JUST SOUTH OF LEGACY AND WEST OF HAYDEN - FUTURE RETAIL
  214     BA   0.015
  215     LG   0.10    0.25    4.03    0.61      80
  216     UC   0.164   0.113
  217     UA      0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  218     UA    100
          *

  219     KK   SB8NE  DIVERT
  220     KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NE
  221     KM        FIRST FLUSH VOL = 0.340 AC-FT
  222     DT  RET8NE   0.340
  223     DI    0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  224     DQ    0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
          *

  225     KK  R8NE8N   ROUTE
  226     KM        ROUTE NEW SUB BASIN SB8NE TO OUTLET AT CAVASSON BLVD
  227     KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
  228     RK     622   0.011   0.018            TRAP      46       4
          *
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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

  229     KK   SB8SW   BASIN
  230     KM        SITE JUST NORTH OF CAVASSON BLVD EAST OF CLARET DR - FUTURE MF
  231     BA  0.0129
  232     LG   0.10    0.25    4.03    0.61      80
  233     UC   0.149   0.110
  234     UA      0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  235     UA    100
          *

  236     KK   SB8SW  DIVERT
  237     KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SW
  238     KM        FIRST FLUSH VOL = 0.292 AC-FT
  239     DT  RET8SW   0.292
  240     DI    0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  241     DQ    0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
          *

  242     KK   SB8SE   BASIN
  243     KM        SITE JUST NORTH OF CAVASSON AND WEST OF HAYDEN - FUTURE HOTEL OR RETAIL
```

```
244          BA  0.0131
245          LG   0.10    0.25    4.03    0.61      80
246          UC   0.165   0.137
247          UA      0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
248          UA    100
             *

249          KK   SB8SE  DIVERT
250          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SE
251          KM        FIRST FLUSH VOL = 0.297 AC-FT
252          DT  RET8SE   0.297
253          DI    0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
254          DQ    0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *

255          KK  CP-08N COMBINE
256          KM        CP AT INLET ON NORTH SIDE OF CAVASSON - ENTERS EXIST STORM DRAIN
257          HC       4  0.0557
             *

258          KK  R8N-8W   ROUTE
259          KM        ROUTE NEW SUB BASINS SB8NE SB8NW SB8SW SB8SE TO CP-08W
260          KM        ROUTE IN STORM DRAIN
261          RK    1592   0.015   0.013            CIRC       3
             *

262          KK   SB08W   BASIN
263          BA  0.0589
264          LG   0.10    0.25    4.03    0.61      80
265          UC   0.262   0.177
266          UA      0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
267          UA    100
             *
1                                        HEC-1 INPUT                                          PAGE  8

        LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

268          KK   SB08W  DIVERT
269          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB08W
270          KM        FIRST FLUSH VOL REQ = 1.335 AC-FT, PROVIDED = 1.415
271          KM        THIS IS EXISTING AND WAS BUILT IN PHASE 1
272          DT   RET8W   1.415
273          DI    0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
274          DQ    0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *

275          KK  CP-08W COMBINE
276          HC       3  0.1614
             *
             *

277          KK   R8W-4   ROUTE
278          RK    3137  0.0130   0.030            TRAP   2.000    0.00
             *

279          KK    SB04   BASIN
280          BA   0.144
281          LG    0.14    0.25    4.60    0.44      61
282          UC   0.305   0.199
283          UA      0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
284          UA    100
             *

285          KK    CP-4 COMBINE
286          HC       2
             *

287          KK    R4-5   ROUTE
288          KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
289          RK    1992  0.0013   0.03             TRAP      39       4
             *

290          KK   CP-05 COMBINE
291          KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
292          KM        COMBINE CP-05A AND R4-5
293          HC       2
             *

294          KK    R5-6   ROUTE
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 295          KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 296          RK    1694  0.0015   0.03             TRAP     188       4
              *

 297          KK    SB06   BASIN
 298          BA   0.136
 299          LG    0.16    0.25    4.55    0.45      53
 300          UC   0.321   0.246
 301          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 302          UA     100
              *
1                                       HEC-1 INPUT                                          PAGE  9

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 303          KK   CP-06 COMBINE
 304          HC       2
              *

 305          KK   SB07E   BASIN
 306          BA   0.026
 307          LG    0.10    0.25    4.03    0.74       0
 308          UC   0.574   0.527
 309          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 310          UA     100
              *

 311          KK  R7E-8E   ROUTE
 312          RK    3662  0.0130   0.030            TRAP   2.000    0.00
              *

 313          KK   SB08E   BASIN
 314          BA   0.078
 315          LG    0.10    0.25    4.08    0.72       0
 316          UC   0.798   0.652
 317          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 318          UA     100
              *

 319          KK   SB09N   BASIN
 320          BA   0.028
 321          LG    0.10    0.25    4.79    0.49       0
 322          UC   0.740   1.111
 323          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 324          UA     100
              *

 325          KK  R9NR8E   ROUTE
 326          RK    2586  0.0013   0.020            TRAP   2.000    0.00
              *

 327          KK  CP-08E COMBINE
 328          HC       3
              *

 329          KK  R8E-R9   ROUTE
 330          RK     501  0.0013   0.020            TRAP   2.000    0.00
              *

 331          KK    SB09   BASIN
 332          BA   0.086
 333          LG    0.10    0.25    4.79    0.49       0
 334          UC   0.683   0.518
 335          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 336          UA     100
              *
1                                       HEC-1 INPUT                                          PAGE 10

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 337          KK   CP-09 COMBINE
 338          HC       2
              *

 339          KK   PWRCH
 340          KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
 341          KM        10-YR, 6-HR, DERIVED FROM 100-YR DATA HYDROGRAPHS
 342          BA     7.0
```
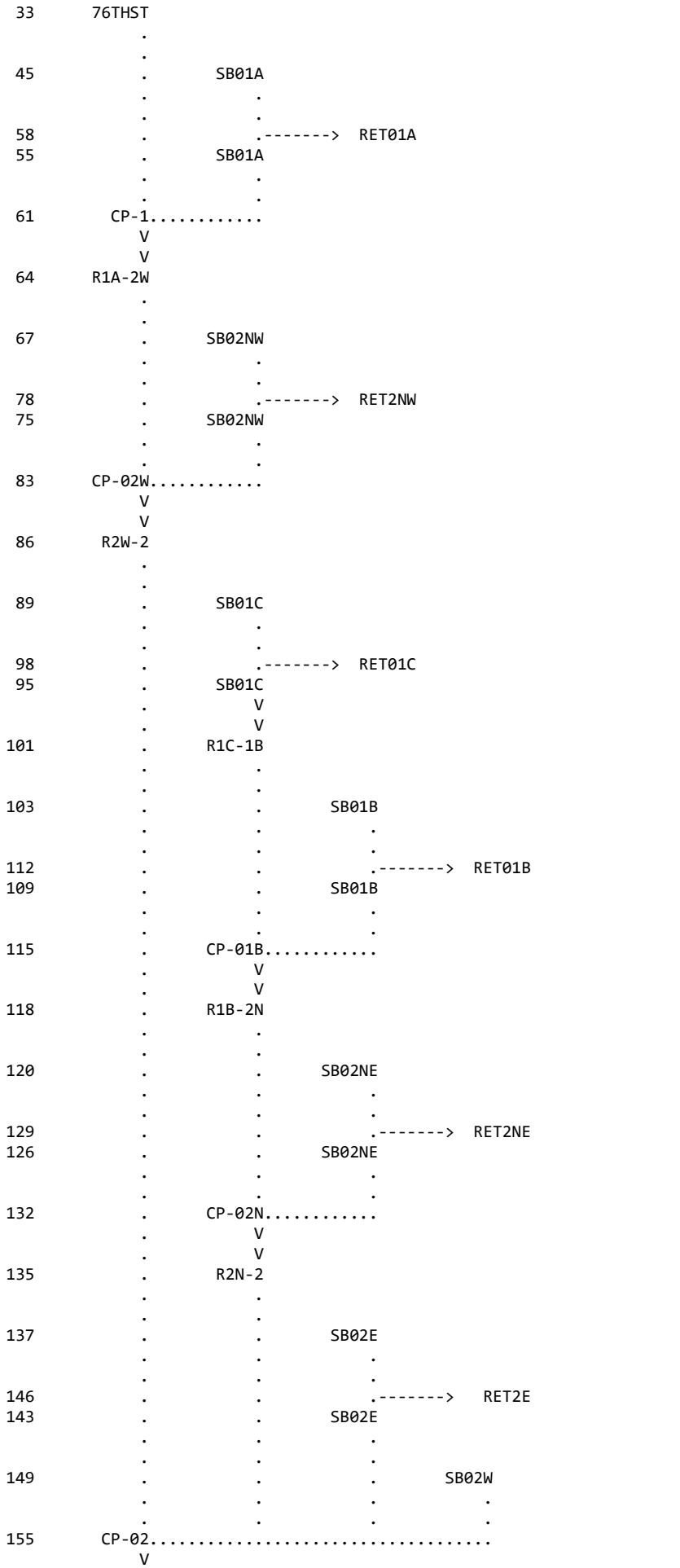
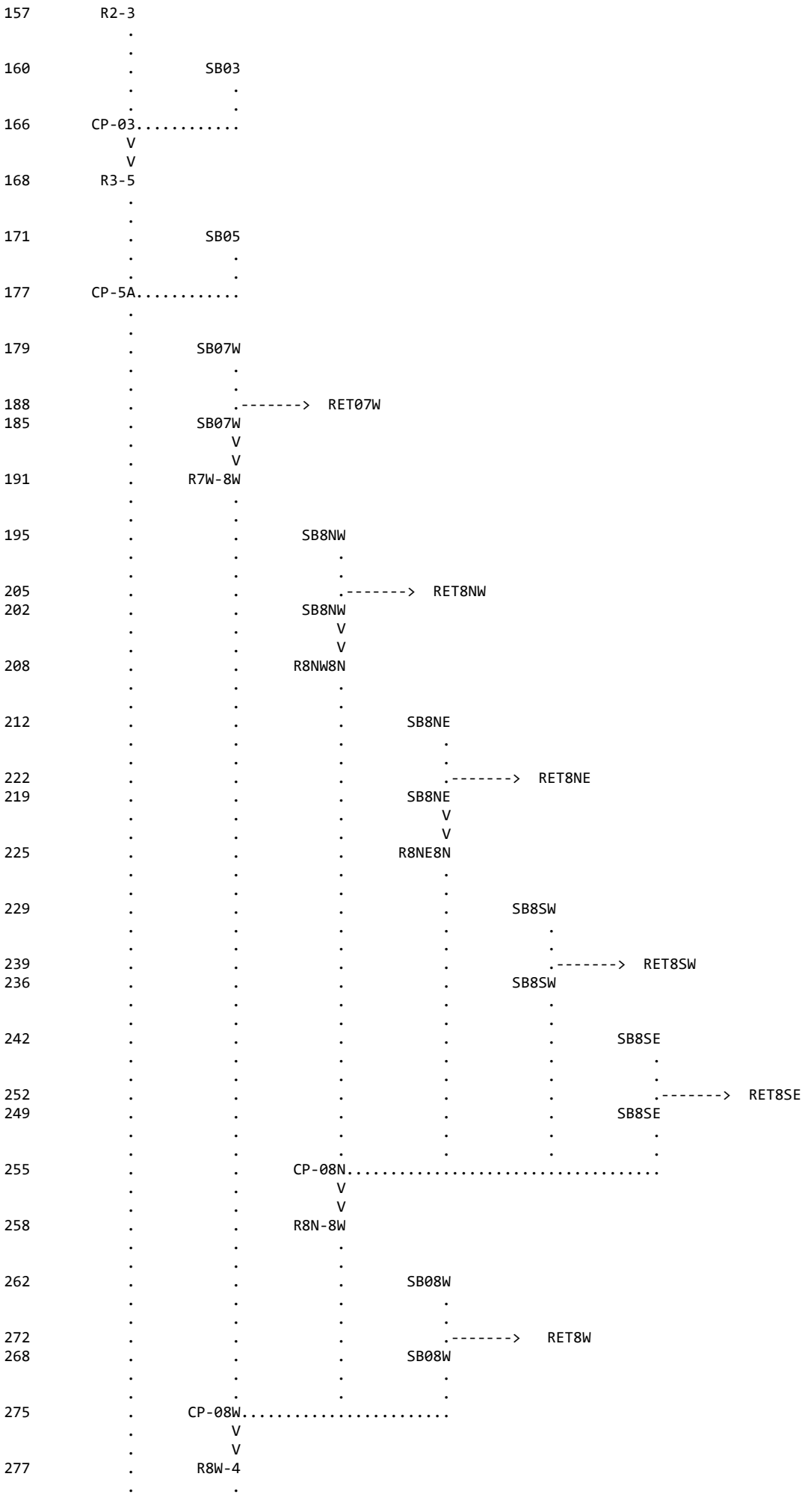
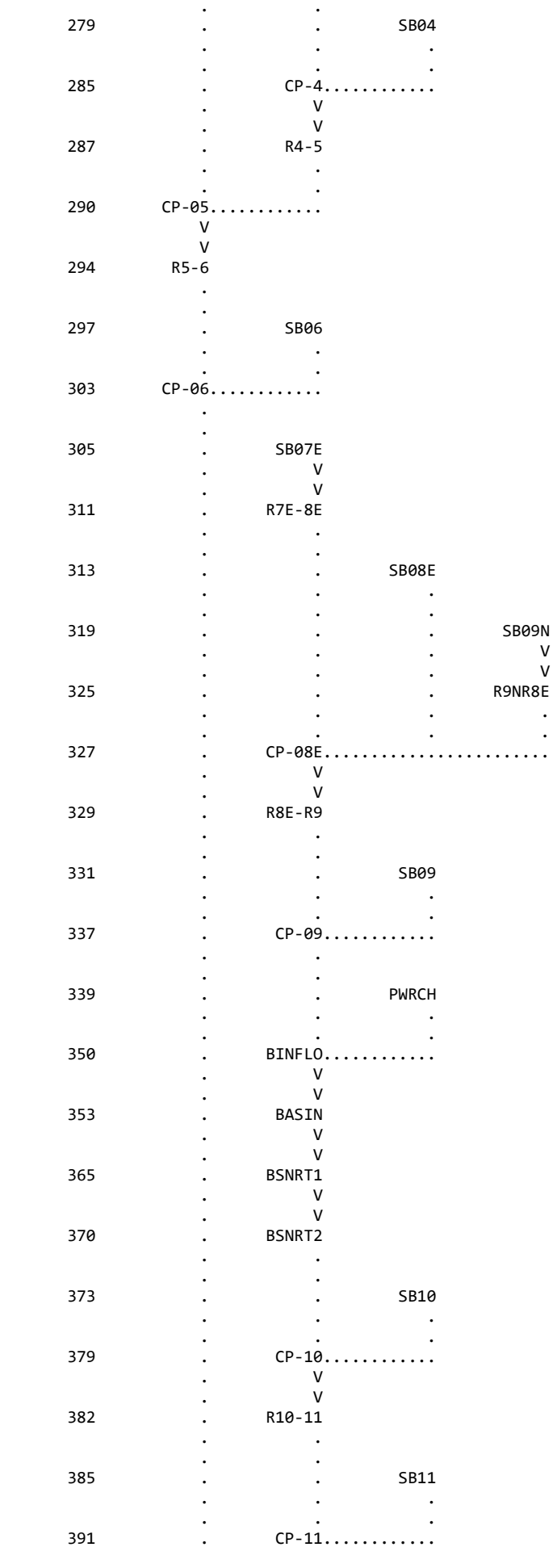

```
          343          QI       0       0       0       0       0       0       0       0       0       0
          344          QI       0       0       0       0       0    0.07   11.70   33.57  285.55  257.89
          345          QI  236.82  258.26  406.55  486.55  461.36  425.60  409.84  395.31  391.77  376.42
          346          QI  346.76  376.76  276.32  247.65  220.91  197.44  178.64  155.41  139.22  129.78
          347          QI  113.97  102.76   96.30   85.88    79.7   72.75   65.45   59.94   54.69   51.22
          348          QI    52.3   44.56   41.87   42.77   36.93   34.05   32.15   29.01   27.72   27.03
          349          QI   25.05   23.86   22.04   20.81   19.27   18.55   17.90   20.82   16.03   15.23
                       *

          350          KK  BINFLO
          351          KM         TOTAL INFLOW INTO BASIN 53R.
          352          HC       2
                       *

          353          KK   BASIN
          354          KM     BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
          355          KM     ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
          356          KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
          357          KM        THE OUTLET PIPES ARE INLET CONTROLLED.
          358          RS       1    STOR       0
          359          SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
          360          SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
          361          SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
          362          SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
          363          SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
          364          SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
                       *

          365          KK  BSNRT1
          366          KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
          367          KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
          368          KM        EXCESS CAPACITY.
          369          RK     550  0.0052   0.024            CIRC     7.0
                       *

          370          KK  BSNRT2
          371          KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BAS 53R OUTFALL)
          372          RK    1200  0.0077   0.013            CIRC     7.0
                       *

          373          KK    SB10   BASIN
          374          BA   0.040
          375          LG    0.15    0.25    4.25    0.55      55
          376          UC   0.233   0.161
          377          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          378          UA     100
                       *
1                                          HEC-1 INPUT                                                PAGE 11

         LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

          379          KK   CP-10 COMBINE
          380          KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
          381          HC       2
                       *

          382          KK  R10-11   ROUTE
          383          KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
          384          RK    1277  0.0014   0.03             TRAP     220       4
                       *

          385          KK    SB11   BASIN
          386          BA   0.071
          387          LG    0.15    0.25    4.15    0.58      55
          388          UC   0.296   0.232
          389          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          390          UA     100
                       *

          391          KK   CP-11 COMBINE
          392          KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
          393          HC       2
                       *
          394          ZZ
1
               SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW
```


NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW
33 76THST
45 SB01A
58 RET01A
55 SB01A
61 CP-1
64 R1A-2W
67 SB02NW
78 RET2NW
75 SB02NW
83 CP-02W
86 R2W-2
89 SB01C
98 RET01C
95 SB01C
101 R1C-1B
103 SB01B
112 RET01B
109 SB01B
115 CP-01B
118 R1B-2N
120 SB02NE
129 RET2NE
126 SB02NE
132 CP-02N
135 R2N-2
137 SB02E
146 RET2E
143 SB02E
149 SB02W
155 CP-02


157 R2-3
160 SB03
166 CP-03
168 R3-5
171 SB05
177 CP-5A
179 SB07W
188 RET07W
185 SB07W
191 R7W-8W
195 SB8NW
205 RET8NW
202 SB8NW
208 R8NW8N
212 SB8NE
222 RET8NE
219 SB8NE
225 R8NE8N
229 SB8SW
239 RET8SW
236 SB8SW
242 SB8SE
252 RET8SE
249 SB8SE
255 CP-08N
258 R8N-8W
262 SB08W
272 RET8W
268 SB08W
275 CP-08W
277 R8W-4



(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

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1****************************************
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 *                                      *                                                    *
*
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)  *                                                    *    U.S. ARMY CORPS OF ENGINEERS
*
 *               JUN   1998             *                                                    *    HYDROLOGIC ENGINEERING CENTER
*
 *            VERSION 4.1               *                                                    *          609 SECOND STREET
*
 *                                      *                                                    *       DAVIS, CALIFORNIA 95616
*
 *  RUN DATE   30APR21  TIME  13:35:28  *                                                    *           (916) 756-1104
*
 *                                      *                                                    *
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CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
POWERLINE CORRIDOR
RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
10-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
PIMA ROAD FLOWS.

PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
MODELERS: RK, MW

REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
MODELERS: MSW, ES
REVISIONS NOTED WITH HE
ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.

20 IO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA

NMIN	3	MINUTES IN COMPUTATION INTERVAL
IDATE	1 0	STARTING DATE
ITIME	0000	STARTING TIME
NQ	1000	NUMBER OF HYDROGRAPH ORDINATES
NDDATE	3 0	ENDING DATE
NDTIME	0157	ENDING TIME
ICENT	19	CENTURY MARK

COMPUTATION INTERVAL 0.05 HOURS
TOTAL TIME BASE 49.95 HOURS

ENGLISH UNITS

DRAINAGE AREA	SQUARE MILES
PRECIPITATION DEPTH	INCHES
LENGTH, ELEVATION	FEET
FLOW	CUBIC FEET PER SECOND
STORAGE VOLUME	ACRE-FEET
SURFACE AREA	ACRES
TEMPERATURE	DEGREES FAHRENHEIT

21 JD INDEX STORM NO. 1

STRM	1.82	PRECIPITATION DEPTH
TRDA	0.00	TRANSPOSITION DRAINAGE AREA

22 PI PRECIPITATION PATTERN

0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.02	0.02	0.02	0.02	0.02
0.03	0.03	0.03	0.03	0.03	0.09	0.09	0.09	0.09	0.09

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                0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

25 JD          INDEX STORM NO. 2
                  STRM        1.81  PRECIPITATION DEPTH
                  TRDA        0.50  TRANSPOSITION DRAINAGE AREA

26 PI            PRECIPITATION PATTERN
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
                0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

29 JD          INDEX STORM NO. 3
                  STRM        1.77  PRECIPITATION DEPTH
                  TRDA        2.80  TRANSPOSITION DRAINAGE AREA

30 PI            PRECIPITATION PATTERN
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                0.04      0.04      0.04      0.04      0.04      0.05      0.05      0.05      0.05      0.05
                0.03      0.03      0.03      0.03      0.03      0.01      0.01      0.01      0.01      0.01
                0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

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*** HEC-1 ERROR 1 *** INVALID CARD IDENTIFICATION CODE OR CARD OUT OF SEQUENCE
     CARD NO.   88

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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
HYDROGRAPH AT									
+	76THST	32.	4.50	10.	3.	1.	0.24		
HYDROGRAPH AT									
+	SB01A	59.	4.25	7.	2.	1.	0.06		
DIVERSION TO									
+	RET01A	48.	4.15	3.	1.	0.	0.06		
HYDROGRAPH AT									
+	SB01A	59.	4.25	4.	1.	0.	0.06		
2 COMBINED AT									
+	CP-1	86.	4.25	14.	4.	2.	0.30		
ROUTED TO									
+	R1A-2W	86.	4.30	14.	4.	2.	0.30		
HYDROGRAPH AT									
+	SB02NW	23.	4.10	2.	1.	0.	0.02		
DIVERSION TO									
+	RET2NW	18.	4.00	1.	0.	0.	0.02		
HYDROGRAPH AT									
+	SB02NW	23.	4.10	1.	0.	0.	0.02		
2 COMBINED AT									
+	CP-02W	97.	4.30	15.	4.	2.	0.31		
ROUTED TO									
+	R2W-2	97.	4.30	15.	4.	2.	0.31		
HYDROGRAPH AT									
+	SB01C	26.	4.15	3.	1.	0.	0.03		
DIVERSION TO									
+	RET01C	24.	4.10	1.	0.	0.	0.03		
HYDROGRAPH AT									
+	SB01C	26.	4.15	2.	0.	0.	0.03		
ROUTED TO									
+	R1C-1B	25.	4.20	2.	0.	0.	0.03		
HYDROGRAPH AT									
+	SB01B	25.	4.15	3.	1.	0.	0.02		
DIVERSION TO									
+	RET01B	22.	4.05	1.	0.	0.	0.02		
HYDROGRAPH AT									
+	SB01B	25.	4.15	2.	0.	0.	0.02		
2 COMBINED AT									
+	CP-01B	48.	4.20	3.	1.	0.	0.05		
ROUTED TO									

+	R1B-2N	47.	4.20	3.	1.	0.	0.05
HYDROGRAPH AT							
+	SB02NE	29.	4.20	4.	1.	0.	0.03
DIVERSION TO							
+	RET2NE	27.	4.15	2.	0.	0.	0.03
HYDROGRAPH AT							
+	SB02NE	27.	4.25	2.	1.	0.	0.03
2 COMBINED AT							
+	CP-02N	73.	4.25	6.	1.	1.	0.09
ROUTED TO							
+	R2N-2	72.	4.25	6.	1.	1.	0.09
HYDROGRAPH AT							
+	SB02E	55.	4.15	7.	2.	1.	0.04
DIVERSION TO							
+	RET2E	28.	3.95	2.	1.	0.	0.04
HYDROGRAPH AT							
+	SB02E	55.	4.15	5.	1.	1.	0.04
HYDROGRAPH AT							
+	SB02W	16.	4.50	2.	1.	0.	0.06
4 COMBINED AT							
+	CP-02	210.	4.25	28.	7.	4.	0.18
ROUTED TO							
+	R2-3	209.	4.35	28.	7.	4.	0.18
HYDROGRAPH AT							
+	SB03	47.	4.15	6.	2.	1.	0.05
2 COMBINED AT							
+	CP-03	241.	4.35	33.	9.	4.	0.23
ROUTED TO							
+	R3-5	234.	4.50	33.	9.	4.	0.23
HYDROGRAPH AT							
+	SB05	134.	4.10	15.	4.	2.	0.13
2 COMBINED AT							
+	CP-5A	277.	4.50	47.	13.	6.	0.36
HYDROGRAPH AT							
+	SB07W	54.	4.15	5.	1.	1.	0.05
DIVERSION TO							
+	RET07W	41.	4.05	2.	1.	0.	0.05
HYDROGRAPH AT							
+	SB07W	54.	4.15	3.	1.	0.	0.05
ROUTED TO							
+	R7W-8W	51.	4.65	5.	1.	1.	0.05
HYDROGRAPH AT							
+	SB8NW	24.	4.05	2.	1.	0.	0.01
DIVERSION TO							
+	RET8NW	8.	3.80	1.	0.	0.	0.01
HYDROGRAPH AT							
+	SB8NW	24.	4.05	2.	0.	0.	0.01
ROUTED TO							
+	R8NW8N	24.	4.10	2.	0.	0.	0.01
HYDROGRAPH AT							
+	SB8NE	25.	4.05	2.	1.	0.	0.01
DIVERSION TO							
+	RET8NE	8.	3.80	1.	0.	0.	0.01

+	HYDROGRAPH AT	SB8NE	25.	4.05	2.	0.	0.	0.01
+	ROUTED TO	R8NE8N	25.	4.10	2.	0.	0.	0.01
+	HYDROGRAPH AT	SB8SW	22.	4.05	2.	1.	0.	0.01
+	DIVERSION TO	RET8SW	7.	3.75	1.	0.	0.	0.01
+	HYDROGRAPH AT	SB8SW	22.	4.05	2.	0.	0.	0.01
+	HYDROGRAPH AT	SB8SE	21.	4.05	2.	1.	0.	0.01
+	DIVERSION TO	RET8SE	7.	3.80	1.	0.	0.	0.01
+	HYDROGRAPH AT	SB8SE	21.	4.05	2.	0.	0.	0.01
+	4 COMBINED AT	CP-08N	90.	4.05	7.	2.	1.	0.06
+	ROUTED TO	R8N-8W	89.	4.10	7.	2.	1.	0.06
+	HYDROGRAPH AT	SB08W	85.	4.15	10.	2.	1.	0.06
+	DIVERSION TO	RET8W	36.	3.90	3.	1.	0.	0.06
+	HYDROGRAPH AT	SB08W	85.	4.15	7.	2.	1.	0.06
+	3 COMBINED AT	CP-08W	172.	4.10	18.	5.	2.	0.16
+	ROUTED TO	R8W-4	168.	4.15	18.	5.	2.	0.16
+	HYDROGRAPH AT	SB04	178.	4.10	20.	5.	2.	0.14
+	2 COMBINED AT	CP-4	346.	4.10	38.	10.	5.	0.31
+	ROUTED TO	R4-5	338.	4.20	38.	10.	5.	0.31
+	2 COMBINED AT	CP-05	432.	4.20	82.	22.	11.	0.66
+	ROUTED TO	R5-6	427.	4.30	82.	22.	11.	0.66
+	HYDROGRAPH AT	SB06	145.	4.10	17.	4.	2.	0.14
+	2 COMBINED AT	CP-06	499.	4.25	96.	25.	12.	0.80
+	HYDROGRAPH AT	SB07E	9.	4.40	1.	0.	0.	0.03
+	ROUTED TO	R7E-8E	8.	4.50	1.	0.	0.	0.03
+	HYDROGRAPH AT	SB08E	21.	4.55	3.	1.	0.	0.08
+	HYDROGRAPH AT	SB09N	6.	4.55	2.	0.	0.	0.03
+	ROUTED TO	R9NR8E	6.	4.65	2.	0.	0.	0.03

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          3 COMBINED AT
+                         CP-08E        34.    4.60           6.          2.          1.       0.13

          ROUTED TO
+                         R8E-R9        34.    4.60           6.          2.          1.       0.13

          HYDROGRAPH AT
+                           SB09        34.    4.45           5.          1.          1.       0.09

          2 COMBINED AT
+                           CP-09       66.    4.55          11.          3.          1.       0.22

          HYDROGRAPH AT
+                           PWRCH      487.    5.75         285.         91.         50.       7.00

          2 COMBINED AT
+                           BINFLO     484.    5.75         283.         90.         50.       7.22

          ROUTED TO
+                           BASIN      171.    9.10         156.         85.         48.       7.22

          ROUTED TO
+                           BSNRT1     171.    9.10         156.         85.         48.       7.22

          ROUTED TO
+                           BSNRT2     171.    9.10         156.         85.         48.       7.22

          HYDROGRAPH AT
+                           SB10        52.    4.05           5.          1.          1.       0.04

          2 COMBINED AT
+                           CP-10      171.    9.10         156.         85.         49.       7.26

          ROUTED TO
+                           R10-11     171.    9.20         156.         85.         48.       7.26

          HYDROGRAPH AT
+                           SB11        77.    4.10           9.          2.          1.       0.07

          2 COMBINED AT
+                           CP-11      171.    9.20         156.         85.         49.       7.33
1
                                      SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
                                          (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)
                                                                                    INTERPOLATED TO
                                                                                  COMPUTATION INTERVAL
            ISTAQ    ELEMENT      DT       PEAK    TIME TO     VOLUME      DT       PEAK    TIME TO     VOLUME
                                                      PEAK                                    PEAK

                                  (MIN)     (CFS)      (MIN)     (IN)     (MIN)     (CFS)     (MIN)       (IN)

           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R1A-2W  MANE         0.28     86.74    255.73      0.51      3.00     86.50    258.00        0.51


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8110E+01 EXCESS=0.0000E+00 OUTFLOW=0.8108E+01 BASIN STORAGE=0.2428E-02 PERCENT ERROR=   0.0


           FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R1A-2W  MANE         0.22     86.23    255.60      0.50      3.00     85.99    258.00        0.50


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8082E+01 EXCESS=0.0000E+00 OUTFLOW=0.8081E+01 BASIN STORAGE=0.2428E-02 PERCENT ERROR=   0.0


           FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R1A-2W  MANE         0.23     56.86    261.41      0.46      3.00     56.60    261.00        0.46


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7389E+01 EXCESS=0.0000E+00 OUTFLOW=0.7388E+01 BASIN STORAGE=0.2428E-02 PERCENT ERROR=   0.0


           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R2W-2  MANE         0.41     97.65    258.39      0.54      3.00     97.55    258.00        0.54


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8773E+01 EXCESS=0.0000E+00 OUTFLOW=0.8771E+01 BASIN STORAGE=0.3356E-02 PERCENT ERROR=   0.0
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          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R2W-2  MANE         0.41     96.96    258.16      0.54      3.00     96.86    258.00        0.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8738E+01 EXCESS=0.0000E+00 OUTFLOW=0.8735E+01 BASIN STORAGE=0.3354E-02 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R2W-2  MANE         0.36     63.48    261.95      0.48      3.00     62.70    264.00        0.48

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7822E+01 EXCESS=0.0000E+00 OUTFLOW=0.7820E+01 BASIN STORAGE=0.3350E-02 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1C-1B  MANE         0.47     26.07    249.94      0.64      3.00     25.45    252.00        0.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9235E+00 EXCESS=0.0000E+00 OUTFLOW=0.9234E+00 BASIN STORAGE=0.1159E-13 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1C-1B  MANE         0.40     25.48    250.16      0.63      3.00     25.16    252.00        0.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9114E+00 EXCESS=0.0000E+00 OUTFLOW=0.9112E+00 BASIN STORAGE=0.1161E-13 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1C-1B  MANE         0.40     12.02    255.91      0.42      3.00     11.53    258.00        0.42

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6045E+00 EXCESS=0.0000E+00 OUTFLOW=0.6050E+00 BASIN STORAGE=0.1171E-13 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1B-2N  MANE         1.00     48.05    253.91      0.64      3.00     47.04    252.00        0.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1748E+01 EXCESS=0.0000E+00 OUTFLOW=0.1752E+01 BASIN STORAGE=0.3293E-11 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1B-2N  MANE         0.96     47.49    253.51      0.63      3.00     46.32    252.00        0.63

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1727E+01 EXCESS=0.0000E+00 OUTFLOW=0.1726E+01 BASIN STORAGE=0.3440E-11 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1B-2N  MANE         1.20     21.35    259.25      0.42      3.00     21.16    258.00        0.42

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1144E+01 EXCESS=0.0000E+00 OUTFLOW=0.1146E+01 BASIN STORAGE=0.3398E-11 PERCENT ERROR=  -0.2

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2N-2  MANE         0.38     73.38    255.29      0.64      3.00     73.19    255.00        0.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2890E+01 EXCESS=0.0000E+00 OUTFLOW=0.2895E+01 BASIN STORAGE=0.5835E-06 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R2N-2  MANE         0.42     72.58    255.46      0.63      3.00     72.21    255.00        0.63

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2852E+01 EXCESS=0.0000E+00 OUTFLOW=0.2855E+01 BASIN STORAGE=0.4437E-06 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R2N-2  MANE         0.51     34.33    261.97      0.42      3.00     34.12    261.00        0.42

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1894E+01 EXCESS=0.0000E+00 OUTFLOW=0.1895E+01 BASIN STORAGE=0.4997E-06 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R2-3  MANE         2.42    211.40    259.77      1.55      3.00    210.49    261.00        1.55
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1506E+02 EXCESS=0.0000E+00 OUTFLOW=0.1503E+02 BASIN STORAGE=0.3148E-01 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R2-3	MANE	2.38	209.22	260.63	1.54	3.00	208.79	261.00	1.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1494E+02 EXCESS=0.0000E+00 OUTFLOW=0.1492E+02 BASIN STORAGE=0.3148E-01 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R2-3	MANE	2.90	122.62	266.89	1.22	3.00	122.54	267.00	1.22

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1184E+02 EXCESS=0.0000E+00 OUTFLOW=0.1186E+02 BASIN STORAGE=0.3148E-01 PERCENT ERROR= -0.4

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R3-5	MANE	3.00	238.72	269.68	1.48	3.00	238.21	270.00	1.48

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1817E+02 EXCESS=0.0000E+00 OUTFLOW=0.1812E+02 BASIN STORAGE=0.6963E-01 PERCENT ERROR= -0.1

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R3-5	MANE	3.00	233.59	269.44	1.47	3.00	233.56	270.00	1.47

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1804E+02 EXCESS=0.0000E+00 OUTFLOW=0.1798E+02 BASIN STORAGE=0.6661E-01 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R3-5	MANE	3.00	140.70	278.84	1.18	3.00	140.60	279.00	1.18

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1453E+02 EXCESS=0.0000E+00 OUTFLOW=0.1444E+02 BASIN STORAGE=0.7428E-01 PERCENT ERROR= 0.1

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R7W-8W	MANE	1.68	52.79	276.79	0.96	3.00	51.01	279.00	0.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1600E+01 EXCESS=0.0000E+00 OUTFLOW=0.2399E+01 BASIN STORAGE=0.2981E-03 PERCENT ERROR= -50.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R7W-8W	MANE	1.69	52.24	276.99	0.95	3.00	50.66	279.00	0.95

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1579E+01 EXCESS=0.0000E+00 OUTFLOW=0.2373E+01 BASIN STORAGE=0.2851E-03 PERCENT ERROR= -50.4

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R7W-8W	MANE	0.57	23.36	290.76	0.62	3.00	23.27	291.00	0.63

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1047E+01 EXCESS=0.0000E+00 OUTFLOW=0.1550E+01 BASIN STORAGE=0.2509E-03 PERCENT ERROR= -48.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R8NW8N	MANE	0.90	23.67	245.12	1.12	3.00	23.61	246.00	1.12

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8762E+00 EXCESS=0.0000E+00 OUTFLOW=0.8785E+00 BASIN STORAGE=0.1427E-06 PERCENT ERROR= -0.3

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R8NW8N	MANE	0.89	23.51	245.06	1.11	3.00	23.45	246.00	1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8684E+00 EXCESS=0.0000E+00 OUTFLOW=0.8709E+00 BASIN STORAGE=0.1495E-06 PERCENT ERROR= -0.3

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R8NW8N	MANE	0.99	12.76	245.64	1.01	3.00	12.75	246.00	1.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7895E+00 EXCESS=0.0000E+00 OUTFLOW=0.7947E+00 BASIN STORAGE=0.1506E-06 PERCENT ERROR= -0.7

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          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NE8N  MANE         0.90     25.10    244.37      1.12      3.00     24.86    246.00        1.12

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8929E+00 EXCESS=0.0000E+00 OUTFLOW=0.8983E+00 BASIN STORAGE=0.1026E-06 PERCENT ERROR=  -0.6

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8NE8N  MANE         0.87     24.95    244.34      1.11      3.00     24.69    246.00        1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8849E+00 EXCESS=0.0000E+00 OUTFLOW=0.8866E+00 BASIN STORAGE=0.1149E-06 PERCENT ERROR=  -0.2

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8NE8N  MANE         1.02     13.29    245.08      1.01      3.00     13.20    246.00        1.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8044E+00 EXCESS=0.0000E+00 OUTFLOW=0.8070E+00 BASIN STORAGE=0.1185E-06 PERCENT ERROR=  -0.3

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8N-8W  MANE         0.50     90.26    244.05      1.12      3.00     89.79    246.00        1.12

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3323E+01 EXCESS=0.0000E+00 OUTFLOW=0.3326E+01 BASIN STORAGE=0.1332E-07 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8N-8W  MANE         0.50     89.65    244.04      1.11      3.00     89.18    246.00        1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3296E+01 EXCESS=0.0000E+00 OUTFLOW=0.3298E+01 BASIN STORAGE=0.1340E-07 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8N-8W  MANE         0.71     48.48    244.61      1.01      3.00     48.33    246.00        1.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2995E+01 EXCESS=0.0000E+00 OUTFLOW=0.2995E+01 BASIN STORAGE=0.1327E-07 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
               R8W-4  MANE         0.74    171.86    247.74      1.06      3.00    169.25    249.00        1.06

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9153E+01 EXCESS=0.0000E+00 OUTFLOW=0.9156E+01 BASIN STORAGE=0.1251E-03 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
               R8W-4  MANE         0.75    170.53    247.11      1.05      3.00    168.12    249.00        1.05

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9058E+01 EXCESS=0.0000E+00 OUTFLOW=0.9060E+01 BASIN STORAGE=0.1245E-03 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
               R8W-4  MANE         0.85     96.14    247.40      0.89      3.00     95.60    249.00        0.89

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7653E+01 EXCESS=0.0000E+00 OUTFLOW=0.7655E+01 BASIN STORAGE=0.1199E-03 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
                R4-5  MANE         2.78    341.92    253.01      1.19      3.00    340.48    252.00        1.19

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1923E+02 EXCESS=0.0000E+00 OUTFLOW=0.1934E+02 BASIN STORAGE=0.3703E-03 PERCENT ERROR=  -0.6

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
                R4-5  MANE         2.78    338.84    253.21      1.18      3.00    337.96    252.00        1.18

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1906E+02 EXCESS=0.0000E+00 OUTFLOW=0.1916E+02 BASIN STORAGE=0.4184E-03 PERCENT ERROR=  -0.5

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
                R4-5  MANE         3.00    193.48    252.47      1.01      3.00    192.50    252.00        1.01
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1647E+02 EXCESS=0.0000E+00 OUTFLOW=0.1651E+02 BASIN STORAGE=0.3235E-03 PERCENT ERROR= -0.2

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R5-6	MANE	2.88	465.68	257.00	1.28	3.00	461.39	258.00	1.28

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4510E+02 EXCESS=0.0000E+00 OUTFLOW=0.4501E+02 BASIN STORAGE=0.5641E-01 PERCENT ERROR= 0.1

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R5-6	MANE	2.88	462.64	257.18	1.27	3.00	458.66	258.00	1.27

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4472E+02 EXCESS=0.0000E+00 OUTFLOW=0.4464E+02 BASIN STORAGE=0.5641E-01 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R5-6	MANE	3.00	269.39	286.19	1.06	3.00	264.39	285.00	1.06

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3723E+02 EXCESS=0.0000E+00 OUTFLOW=0.3723E+02 BASIN STORAGE=0.5640E-01 PERCENT ERROR= -0.1

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R7E-8E	MANE	2.48	8.47	267.86	0.41	3.00	8.45	270.00	0.41

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5640E+00 EXCESS=0.0000E+00 OUTFLOW=0.5642E+00 BASIN STORAGE=0.1424E-04 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R7E-8E	MANE	2.49	8.36	268.11	0.40	3.00	8.32	270.00	0.40

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5555E+00 EXCESS=0.0000E+00 OUTFLOW=0.5565E+00 BASIN STORAGE=0.1227E-04 PERCENT ERROR= -0.2

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R7E-8E	MANE	3.00	0.79	281.23	0.04	3.00	0.79	282.00	0.04

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5136E-01 EXCESS=0.0000E+00 OUTFLOW=0.5564E-01 BASIN STORAGE=0.1218E-04 PERCENT ERROR= -8.4

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R9NR8E	MANE	3.00	6.31	281.03	0.51	3.00	6.30	279.00	0.51

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7576E+00 EXCESS=0.0000E+00 OUTFLOW=0.7564E+00 BASIN STORAGE=0.1663E-04 PERCENT ERROR= 0.2

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R9NR8E	MANE	3.00	6.24	281.18	0.50	3.00	6.22	279.00	0.50

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7487E+00 EXCESS=0.0000E+00 OUTFLOW=0.7475E+00 BASIN STORAGE=0.1891E-04 PERCENT ERROR= 0.2

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R9NR8E	MANE	3.00	1.69	287.45	0.14	3.00	1.68	288.00	0.14

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2051E+00 EXCESS=0.0000E+00 OUTFLOW=0.2049E+00 BASIN STORAGE=0.1591E-04 PERCENT ERROR= 0.1

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R8E-R9	MANE	0.31	34.65	276.05	0.43	3.00	34.65	276.00	0.43

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3043E+01 EXCESS=0.0000E+00 OUTFLOW=0.3043E+01 BASIN STORAGE=0.7386E-05 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R8E-R9	MANE	0.31	34.16	276.08	0.43	3.00	34.16	276.00	0.43

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3001E+01 EXCESS=0.0000E+00 OUTFLOW=0.3002E+01 BASIN STORAGE=0.7554E-05 PERCENT ERROR= 0.0

```
          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8E-R9  MANE         0.73      4.44    286.10      0.06      3.00      4.42    285.00        0.06


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4419E+00 EXCESS=0.0000E+00 OUTFLOW=0.4419E+00 BASIN STORAGE=0.7013E-05 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT1  MANE         0.43    176.85    541.04      0.53      3.00    176.85    543.00        0.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2055E+03 EXCESS=0.0000E+00 OUTFLOW=0.2055E+03 BASIN STORAGE=0.2436E-01 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            BSNRT1  MANE         0.43    176.79    541.14      0.53      3.00    176.79    543.00        0.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2054E+03 EXCESS=0.0000E+00 OUTFLOW=0.2054E+03 BASIN STORAGE=0.2436E-01 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            BSNRT1  MANE         0.47    172.81    544.06      0.52      3.00    172.80    543.00        0.52


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2012E+03 EXCESS=0.0000E+00 OUTFLOW=0.2012E+03 BASIN STORAGE=0.2435E-01 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE         0.44    176.85    543.47      0.53      3.00    176.85    543.00        0.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2055E+03 EXCESS=0.0000E+00 OUTFLOW=0.2054E+03 BASIN STORAGE=0.2781E-01 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            BSNRT2  MANE         0.44    176.78    543.55      0.53      3.00    176.78    543.00        0.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2054E+03 EXCESS=0.0000E+00 OUTFLOW=0.2054E+03 BASIN STORAGE=0.2781E-01 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            BSNRT2  MANE         0.48    172.80    544.14      0.52      3.00    172.80    546.00        0.52


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2012E+03 EXCESS=0.0000E+00 OUTFLOW=0.2011E+03 BASIN STORAGE=0.2781E-01 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R10-11  MANE         3.46    176.84    548.85      0.54      3.00    176.84    549.00        0.54


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2080E+03 EXCESS=0.0000E+00 OUTFLOW=0.2075E+03 BASIN STORAGE=0.5529E+00 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R10-11  MANE         3.45    176.78    548.95      0.54      3.00    176.78    549.00        0.54


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2079E+03 EXCESS=0.0000E+00 OUTFLOW=0.2074E+03 BASIN STORAGE=0.5529E+00 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R10-11  MANE         3.43    172.80    553.00      0.52      3.00    172.80    552.00        0.52


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2033E+03 EXCESS=0.0000E+00 OUTFLOW=0.2028E+03 BASIN STORAGE=0.5528E+00 PERCENT ERROR=   0.0




***    1 ERROR(S) DETECTED BY HEC-1 ***
```

100yr-6hr-Ultimate

```
1****************************************
****************************************
 *                                      *                                                       *
*
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)  *                                                       *    U.S. ARMY CORPS OF ENGINEERS
*
 *               JUN   1998             *                                                       *    HYDROLOGIC ENGINEERING CENTER
*
 *            VERSION 4.1               *                                                       *          609 SECOND STREET
*
 *                                      *                                                       *       DAVIS, CALIFORNIA 95616
*
 *  RUN DATE   26APR21  TIME  08:56:05  *                                                       *           (916) 756-1104
*
 *                                      *                                                       *
*
 ****************************************
****************************************
```

```
                                        X     X  XXXXXXX   XXXXX           X
                                        X     X  X        X     X         XX
                                        X     X  X        X                X
                                        XXXXXXX  XXXX     X        XXXXX   X
                                        X     X  X        X                X
                                        X     X  X        X     X          X
                                        X     X  XXXXXXX   XXXXX          XXX
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


```
          LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

            1            ID        CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
            2            ID        POWERLINE CORRIDOR
            3            ID        RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
            4            ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
            5            ID        100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
            6            ID           PIMA ROAD FLOWS.
            7            ID
            8            ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
            9            ID        MODELERS: RK, MW
           10            ID
           11            ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
           12            ID        MODELERS: MSW, ES
           13            ID        REVISIONS NOTED WITH HE
           14            ID           ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
           15            ID           THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
           16            ID           UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.
           17            ID
           18            ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
           19            ID        MODELERS: MSW
           20            ID        XKSAT ADJUSTMENT
           21            ID
           22            ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
           23            ID        MODELERS: MSW, TSW
           24            ID        REVISIONS NOTED WITH HE (Hubbard Engineering)
           25            ID           NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
           26            ID           CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
           27            ID           ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
           28            ID           FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED
           29            ID
           30            IT       3       0       0    1000
           31            IN      15
           32            IO       5
                         *DIAGRAM
```

```
                *
      33        JD   2.755  0.0001
      34        PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
      35        PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
      36        PC   0.962   0.972   0.983   0.991   1.000
      37        JD   2.738  0.5000
      38        PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
      39        PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
      40        PC   0.962   0.972   0.983   0.991   1.000
      41        JD   2.686     2.8
      42        PC   0.000   0.009   0.016   0.025   0.034   0.042   0.051   0.059   0.068   0.077
      43        PC   0.088   0.101   0.121   0.164   0.253   0.451   0.694   0.836   0.900   0.938
      44        PC   0.950   0.963   0.975   0.988   1.000
      45        JD   2.540    16.0
      46        PC   0.000   0.015   0.020   0.030   0.048   0.063   0.076   0.090   0.105   0.119
      47        PC   0.135   0.152   0.175   0.222   0.304   0.472   0.670   0.796   0.868   0.912
      48        PC   0.946   0.960   0.973   0.987   1.000
      49        JD   2.232    90.0
      50        PC   0.000   0.021   0.035   0.051   0.071   0.087   0.105   0.125   0.143   0.160
      51        PC   0.179   0.201   0.232   0.281   0.364   0.500   0.658   0.773   0.841   0.888
      52        PC   0.927   0.945   0.964   0.982   1.000
      53        JD   3.657    10.0
1                                        HEC-1 INPUT                                          PAGE  2

     LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

      54        JD   3.533    20.0
                *

      55        KK  76THST
      56        KM       76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
      57        KM       100-YR, 6-HR FLO-2D MODEL (XS 98)
      58        BA    0.24
      59        QI       0       0       0       0       0       0       0       0       0       0
      60        QI       0       0    0.01    0.01    0.09     0.9    12.7   42.26   49.92    48.6
      61        QI   34.49   26.72   21.55   17.51   14.81   12.84   11.63   10.56    9.58    8.32
      62        QI    7.29    6.73    5.83    4.76    4.23    3.81     3.4    3.55    2.36    2.51
      63        QI    2.32    2.16    2.02    1.83    1.58     1.5    1.52    1.21    1.31    1.34
      64        QI    1.09    1.07    1.04    0.97    0.91    0.86    0.83    0.79    0.76    0.72
      65        QI    0.69    0.67    0.65    0.62     0.6    0.58    0.55    0.53    0.51    0.49
      66        QI    0.47    0.45    0.44
                *

      67        KK   SB01A   BASIN
      68        BA   0.061
      69        PB   2.721
      70        PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.075
      71        PC   0.087   0.099   0.119   0.150   0.234   0.413   0.766   0.875   0.916   0.944
      72        PC   0.956   0.968   0.979   0.990   1.000
      73        LG    0.25    0.25    4.03    0.56      45
      74        UC   0.439   0.231
      75        UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
      76        UA     100
                *

      77        KK   SB01A  DIVERT
      78        KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01A
      79        KM        FIRST FLUSH VOL = 1.383 AC-FT
      80        DT  RET01A   1.383
      81        DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
      82        DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                *

      83        KK    CP-1 COMBINE
      84        KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
      85        HC       2   0.301
                *

      86        KK  R1A-2W   ROUTE
      87        KM        ROUTE CP-1 TO CP-2W
      88        RK    1114  0.0200   0.018            TRAP   2.000    0.00
                *

      89        KK  SB02NW   BASIN
      90        KM        BASIN SB02E WAS SPLIT INTO SUB BASINS NORTH OF CAVASSON
      91        KM        AND SOUTH OF CAVASSON
      92        BA  0.0193
      93        LG    0.25    0.25    4.03    0.56      45
      94        UC   0.172   0.133
      95        UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
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  96          UA     100
              *
1                                     HEC-1 INPUT                                     PAGE  3

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  97          KK  SB02NW  DIVERT
  98          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NW
  99          KM        FIRST FLUSH VOL = 0.437 AC-FT
 100          DT  RET2NW   0.437
 101          DI     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
 102          DI    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
 103          DQ     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
 104          DQ    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
              *

 105          KK  CP-02W COMBINE
 106          KM        COMBINE ROUTE FROM 1A-2W WITH NEW SUB BASIN SB02NW
 107          HC       2   0.305
              *

 108          KK   R2W-2   ROUTE
 109          RK    1380  0.0140   0.018            TRAP   2.000    0.00
              *

 110          KK   SB01C   BASIN
 111          BA   0.027
 112          LG    0.25    0.25    4.03    0.56      45
 113          UC   0.215   0.216
 114          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 115          UA     100
              *

 116          KK   SB01C  DIVERT
 117          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01C
 118          KM        FIRST FLUSH VOL = 0.612 AC-FT
 119          DT  RET01C   0.612
 120          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 121          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 122          KK  R1C-1B   ROUTE
 123          KM        KO     2       2
 124          RK     542   0.005   0.013            CIRC   5.000
              *

 125          KK   SB01B   BASIN
 126          BA   0.024
 127          LG    0.25    0.25    4.03    0.56      45
 128          UC   0.191   0.178
 129          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 130          UA     100
              *

 131          KK   SB01B  DIVERT
 132          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01B
 133          KM        FIRST FLUSH VOL = 0.543 AC-FT
 134          DT  RET01B   0.543
 135          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 136          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *
1                                     HEC-1 INPUT                                     PAGE  4

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 137          KK  CP-01B COMBINE
 138          KM        COMBINE ROUTE FROM 1C-1B WITH NEW SUB BASIN SB01B
 139          HC       2   0.051
              *

 140          KK  R1B-2N   ROUTE
 141          KM     KO     2       2
 142          RK    2144   0.008   0.013            CIRC   5.000
              *

 143          KK  SB02NE   BASIN
 144          BA  0.0335
 145          LG    0.25    0.25    4.03    0.56      45
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146          UC   0.255   0.267
147          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
148          UA     100
             *

149          KK  SB02NE  DIVERT
150          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NE
151          KM        FIRST FLUSH VOL = 0.760 AC-FT
152          DT  RET2NE   0.760
153          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
154          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *

155          KK  CP-02N COMBINE
156          KM        COMBINE ROUTE 1B-2N (BASINS SB01-B AND SB01-C) WITH SB02NE
157          HC       2   0.085
             *

158          KK   R2N-2   ROUTE
159          RK    1380   0.014   0.018            TRAP   2.000    0.00
             *

160          KK   SB02E   BASIN
161          BA   0.040
162          LG    0.10    0.25    4.03    0.61      80
163          UC   0.214   0.175
164          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
165          UA     100
             *

166          KK   SB02E  DIVERT
167          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E
168          KM        FIRST FLUSH VOL = 0.998 AC-FT
169          DT   RET2E   0.998
170          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
171          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *
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LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

172          KK   SB02W   BASIN
173          BA  0.0571
174          LG    0.35    0.35    4.33    0.51       0
175          UC   0.686   0.535
176          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
177          UA     100
             *

178          KK   CP-02 COMBINE
179          HC       4  0.1816
             *

180          KK    R2-3   ROUTE
181          KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
182          RK    1260  0.0015   0.030            TRAP      92       4
             *

183          KK    SB03   BASIN
184          BA   0.048
185          LG    0.15    0.25    4.50    0.47      55
186          UC   0.364   0.294
187          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
188          UA     100
             *

189          KK   CP-03 COMBINE
190          HC       2
             *

191          KK    R3-5   ROUTE
192          KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
193          RK    2396  0.0015   0.03             TRAP      98       4
             *

194          KK    SB05   BASIN
195          BA   0.126
196          LG    0.22    0.25    4.50    0.44      48
197          UC   0.327   0.226
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 198          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 199          UA     100
              *

 200          KK   CP-5A COMBINE
 201          HC       2
              *

 202          KK   SB07W   BASIN
 203          BA  0.0468
 204          LG    0.25    0.25    4.03    0.56      45
 205          UC   0.203   0.141
 206          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 207          UA     100
              *
1                                     HEC-1 INPUT                                          PAGE  6

 LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 208          KK   SB07W  DIVERT
 209          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB07W
 210          KM        FIRST FLUSH VOL = 1.060 AC-FT
 211          DT  RET07W   1.060
 212          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 213          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 214          KK  R7W-8W   ROUTE
 215          KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 216          KM        HE MODIFIED ROUTE TO LOOP 101 W OF HAYDEN
 217          RK    3754  0.0013   0.03             TRAP      46       4
              *

 218          KK   SB8NW   BASIN
 219          KM        BASIN JUST SOUTH OF LEGACY - FUTURE MOB SITE
 220          BA  0.0147
 221          LG    0.10    0.25    4.03    0.61      80
 222          UC   0.144   0.108
 223          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 224          UA     100
              *

 225          KK   SB8NW  DIVERT
 226          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NW
 227          KM        FIRST FLUSH VOL = 0.332 AC-FT
 228          DT  RET8NW   0.332
 229          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 230          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 231          KK  R8NW8N   ROUTE
 232          KM        ROUTE NEW SUB BASIN SB8NW TO OUTLET AT CAVASSON BLVD
 233          KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 234          RK     643  0.0113   0.018            TRAP      46       4
              *

 235          KK   SB8NE   BASIN
 236          KM        SITE JUST SOUTH OF LEGACY AND WEST OF HAYDEN - FUTURE RETAIL
 237          BA   0.015
 238          LG    0.10    0.25    4.03    0.61      80
 239          UC   0.138   0.094
 240          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 241          UA     100
              *

 242          KK   SB8NE  DIVERT
 243          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NE
 244          KM        FIRST FLUSH VOL = 0.340 AC-FT
 245          DT  RET8NE   0.340
 246          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 247          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *
1                                     HEC-1 INPUT                                          PAGE  7

 LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 248          KK  R8NE8N   ROUTE
 249          KM        ROUTE NEW SUB BASIN SB8NE TO OUTLET AT CAVASSON BLVD
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250          KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
251          RK     622   0.011   0.018            TRAP      46       4
             *

252          KK   SB8SW   BASIN
253          KM        SITE JUST NORTH OF CAVASSON BLVD EAST OF CLARET DR - FUTURE MF
254          BA  0.0129
255          LG    0.10    0.25    4.03    0.61      80
256          UC   0.126   0.091
257          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
258          UA     100
             *

259          KK   SB8SW  DIVERT
260          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SW
261          KM        FIRST FLUSH VOL = 0.292 AC-FT
262          DT  RET8SW   0.292
263          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
264          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *

265          KK   SB8SE   BASIN
266          KM        SITE JUST NORTH OF CAVASSON AND WEST OF HAYDEN - FUTURE HOTEL OR RETAIL
267          BA  0.0131
268          LG    0.10    0.25    4.03    0.61      80
269          UC   0.139   0.113
270          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
271          UA     100
             *

272          KK   SB8SE  DIVERT
273          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SE
274          KM        FIRST FLUSH VOL = 0.297 AC-FT
275          DT  RET8SE   0.297
276          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
277          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *

278          KK  CP-08N COMBINE
279          KM        CP AT INLET ON NORTH SIDE OF CAVASSON - ENTERS EXIST STORM DRAIN
280          HC       4  0.0557
             *

281          KK  R8N-8W   ROUTE
282          KM        ROUTE NEW SUB BASINS SB8NE SB8NW SB8SW SB8SE TO CP-08W
283          KM        ROUTE IN STORM DRAIN
284          RK    1592   0.015   0.013            CIRC       3
             *
1                                      HEC-1 INPUT                                        PAGE  8

          LINE      ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

285          KK   SB08W   BASIN
286          BA  0.0589
287          LG    0.10    0.25    4.03    0.61      80
288          UC   0.221   0.146
289          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
290          UA     100
             *

291          KK   SB08W  DIVERT
292          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB08W
293          KM        FIRST FLUSH VOL REQ = 1.335 AC-FT, PROVIDED = 1.415
294          KM        THIS IS EXISTING AND WAS BUILT IN PHASE 1
295          DT   RET8W   1.415
296          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
297          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *

298          KK  CP-08W COMBINE
299          HC       3  0.1614
             *
             *

300          KK   R8W-4   ROUTE
301          RK    3137  0.0130   0.030            TRAP   2.000    0.00
             *

302          KK    SB04   BASIN
```

```
 303          BA   0.144
 304          LG    0.14    0.25    4.60    0.44      61
 305          UC   0.305   0.199
 306          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 307          UA     100
              *

 308          KK    CP-4 COMBINE
 309          HC       2
              *

 310          KK    R4-5   ROUTE
 311          KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 312          RK    1992  0.0013   0.03             TRAP      39       4
              *

 313          KK   CP-05 COMBINE
 314          KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 315          KM        COMBINE CP-05A AND R4-5
 316          HC       2
              *
1                                        HEC-1 INPUT                                          PAGE  9

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 317          KK    R5-6   ROUTE
 318          KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 319          RK    1694  0.0015   0.03             TRAP     188       4
              *

 320          KK    SB06   BASIN
 321          BA   0.136
 322          LG    0.16    0.25    4.55    0.45      53
 323          UC   0.321   0.246
 324          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 325          UA     100
              *

 326          KK   CP-06 COMBINE
 327          HC       2
              *

 328          KK   SB07E   BASIN
 329          BA   0.026
 330          LG    0.10    0.25    4.03    0.74       0
 331          UC   0.574   0.527
 332          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 333          UA     100
              *

 334          KK  R7E-8E   ROUTE
 335          RK    3662  0.0130   0.030            TRAP   2.000    0.00
              *

 336          KK   SB08E   BASIN
 337          BA   0.078
 338          LG    0.10    0.25    4.08    0.72       0
 339          UC   0.798   0.652
 340          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 341          UA     100
              *

 342          KK   SB09N   BASIN
 343          BA   0.028
 344          LG    0.10    0.25    4.79    0.49       0
 345          UC   0.740   1.111
 346          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 347          UA     100
              *

 348          KK  R9N-R9   ROUTE
 349          RK    2586  0.0013   0.020            TRAP   2.000    0.00
              *
1                                        HEC-1 INPUT                                          PAGE 10

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 350          KK  CP-08E COMBINE
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```
 351          HC       3
              *

 352          KK  R8E-R9   ROUTE
 353          RK     501  0.0013   0.020            TRAP   2.000    0.00
              *

 354          KK    SB09   BASIN
 355          BA   0.086
 356          LG    0.10    0.25    4.79    0.49       0
 357          UC   0.683   0.518
 358          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 359          UA     100
              *

 360          KK   CP-09 COMBINE
 361          HC       2
              *

 362          KK   PWRCH
 363          KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
 364          KM        100-YR, 6-HR FLO-2D MODEL (XS 107)
 365          BA     7.0
 366          QI       0       0       0       0       0       0       0       0       0       0
 367          QI       0       0       0       0       0    0.11   17.74   50.91  433.53  391.08
 368          QI  359.13  391.64  616.52  737.85  699.64  645.41  621.52  599.48  594.12  570.84
 369          QI  525.86  472.45  419.04  375.56     335  299.45   270.9  235.68  211.13  196.81
 370          QI  172.83  155.83  146.04  130.32   120.9  110.32   99.25    90.9   82.93   77.68
 371          QI    79.3   67.58    63.5   64.86   56.01   51.64   48.75   43.99   42.04   40.99
 372          QI   37.99   36.19   33.42   31.56   29.22   28.13   27.15   31.57   24.31    23.1
 373          QI   22.09   21.66   20.95
              *

 374          KK  BINFLO
 375          KM         TOTAL INFLOW INTO BASIN 53R.
 376          HC       2
              *

 377          KK   BASIN
 378          KM     BASIN 53R STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
 379          KM     ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
 380          KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
 381          KM        THE OUTLET PIPES ARE INLET CONTROLLED.
 382          RS       1    STOR       0
 383          SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
 384          SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
 385          SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
 386          SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
 387          SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
 388          SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
              *
1                                        HEC-1 INPUT                                        PAGE 11

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 389          KK  BSNRT1
 390          KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
 391          KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
 392          KM        EXCESS CAPACITY.
 393          RK     550  0.0052   0.024            CIRC     7.0
              *

 394          KK  BSNRT2
 395          KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BAS 53R OUTFALL)
 396          RK    1200  0.0077   0.013            CIRC     7.0
              *

 397          KK    SB10   BASIN
 398          BA   0.040
 399          LG    0.15    0.25    4.25    0.55      55
 400          UC   0.233   0.161
 401          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 402          UA     100
              *

 403          KK   CP-10 COMBINE
 404          KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
 405          HC       2   0.229
              *
```

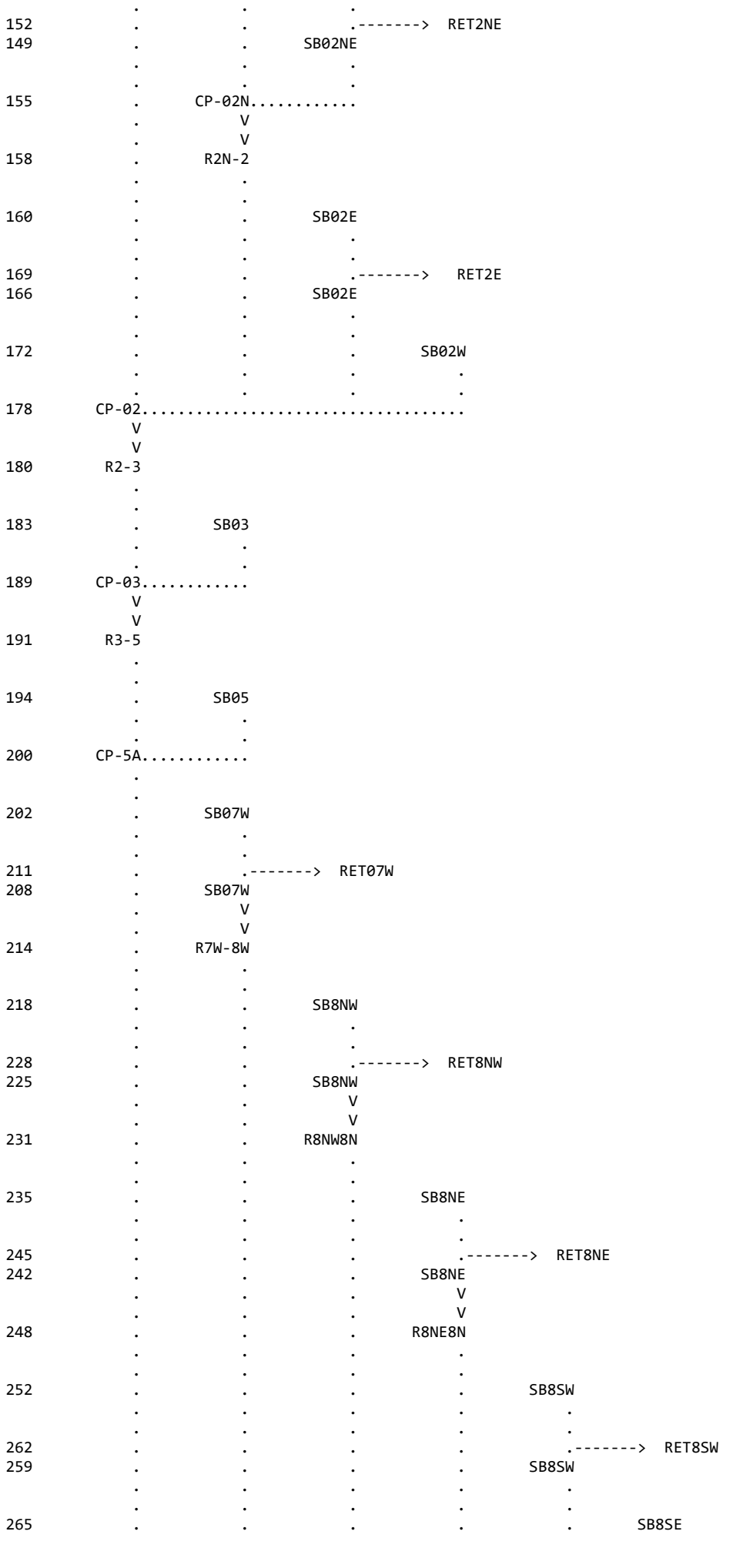
```
          406           KK  R10-11   ROUTE
          407           KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
          408           RK    1277  0.0014   0.03             TRAP     220       4
                        *

          409           KK    SB11   BASIN
          410           BA   0.071
          411           LG    0.15    0.25    4.15    0.58      55
          412           UC   0.296   0.232
          413           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          414           UA     100
                        *

          415           KK   CP-11 COMBINE
          416           KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
          417           HC       2     0.3
                        *
          418           ZZ
1
               SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

    NO.     (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

     55     76THST
                 .
                 .
     67          .       SB01A
                 .           .
                 .           .
     80          .           .-------->  RET01A
     77          .       SB01A
                 .           .
                 .           .
     83        CP-1.............
                 V
                 V
     86     R1A-2W
                 .
                 .
     89          .      SB02NW
                 .           .
                 .           .
    100          .           .-------->  RET2NW
     97          .      SB02NW
                 .           .
                 .           .
    105     CP-02W.............
                 V
                 V
    108      R2W-2
                 .
                 .
    110          .       SB01C
                 .           .
                 .           .
    119          .           .-------->  RET01C
    116          .       SB01C
                 .           V
                 .           V
    122          .      R1C-1B
                 .           .
                 .           .
    125          .           .       SB01B
                 .           .           .
                 .           .           .
    134          .           .           .-------->  RET01B
    131          .           .       SB01B
                 .           .           .
                 .           .           .
    137          .      CP-01B.............
                 .           V
                 .           V
    140          .      R1B-2N
                 .           .
                 .           .
    143          .           .      SB02NE
                 .           .           .
```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248
252
262
259
265
RET2NE
SB02NE
CP-02N
R2N-2
SB02E
RET2E
SB02E
SB02W
CP-02
R2-3
SB03
CP-03
R3-5
SB05
CP-5A
SB07W
RET07W
SB07W
R7W-8W
SB8NW
RET8NW
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R8NW8N
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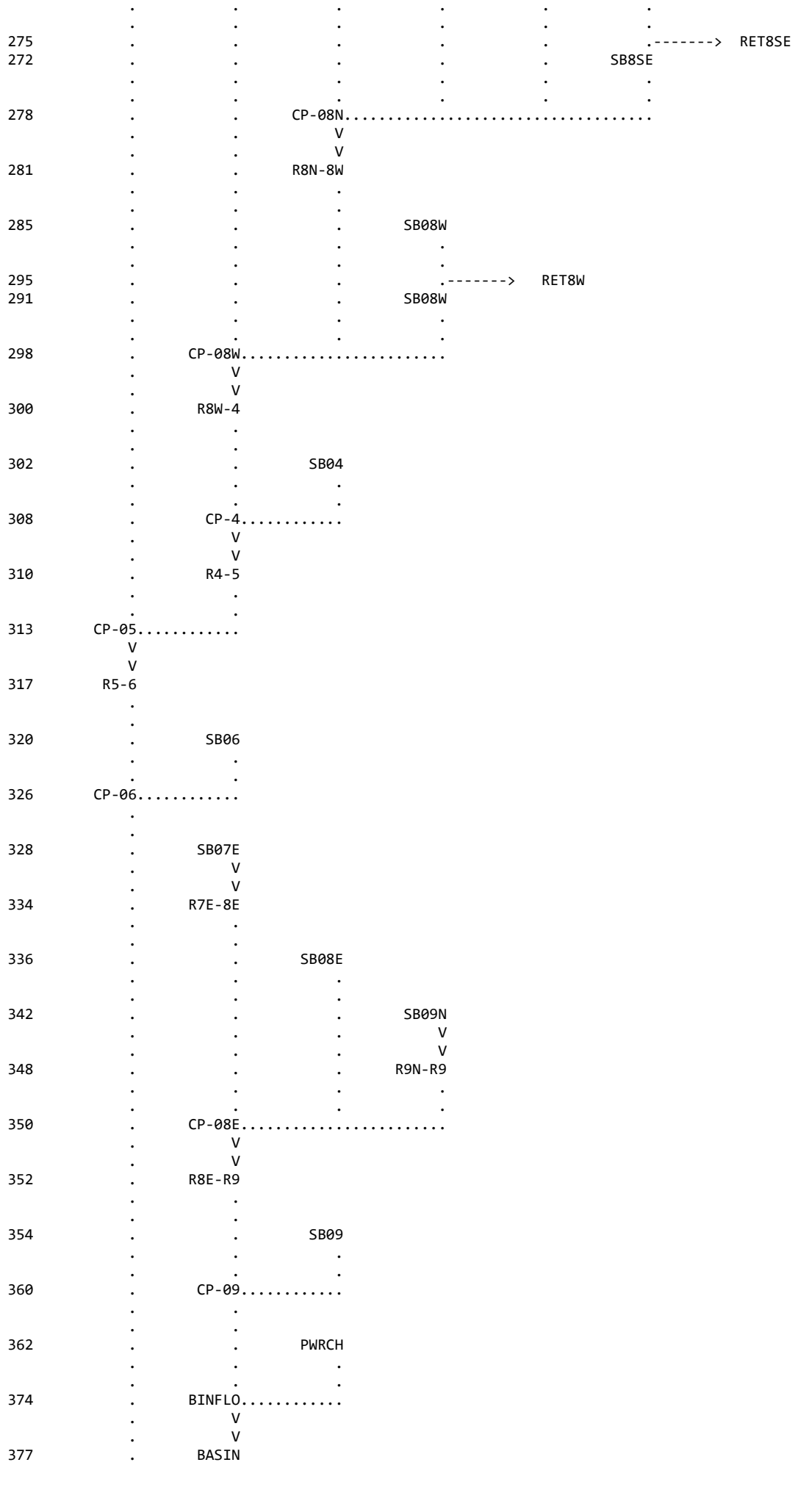
R8NE8N
SB8SW
RET8SW
SB8SW
SB8SE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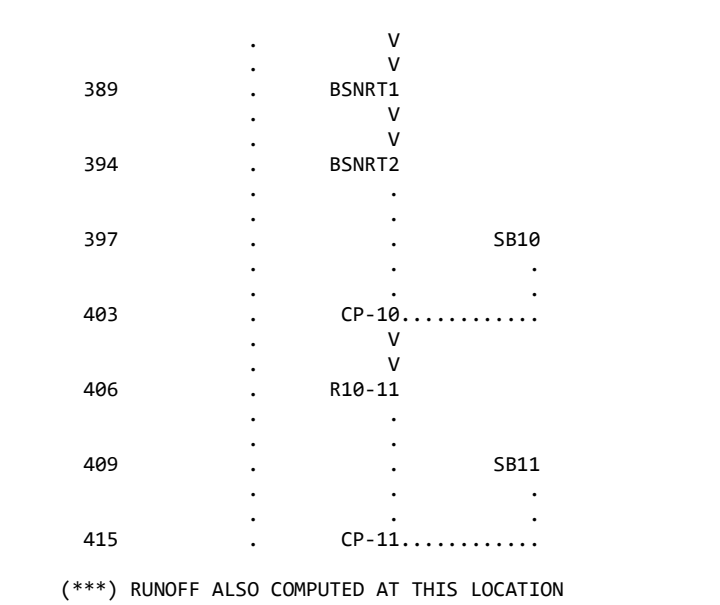
352
354
360
362
374
377
RET8SE
SB8SE
CP-08N
R8N-8W
SB08W
RET8W
SB08W
CP-08W
R8W-4
SB04
CP-4
R4-5
CP-05
R5-6
SB06
CP-06
SB07E
R7E-8E
SB08E
SB09N
R9N-R9
CP-08E
R8E-R9
SB09
CP-09
PWRCH
BINFLO
BASIN



(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

1**********

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* * * *

* FLOOD HYDROGRAPH PACKAGE (HEC-1) * * U.S. ARMY CORPS OF ENGINEERS

* JUN 1998 * * HYDROLOGIC ENGINEERING CENTER

* VERSION 4.1 * * 609 SECOND STREET

* * * DAVIS, CALIFORNIA 95616

* RUN DATE 26APR21 TIME 08:56:05 * * (916) 756-1104

* * * *

****

****

CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
POWERLINE CORRIDOR
RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
PIMA ROAD FLOWS.

PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
MODELERS: RK, MW

REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
MODELERS: MSW, ES
REVISIONS NOTED WITH HE
ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.

REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
MODELERS: MSW
XKSAT ADJUSTMENT

REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
MODELERS: MSW, TSW
REVISIONS NOTED WITH HE (Hubbard Engineering)
NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED

32 IO OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE

```
   IT          HYDROGRAPH TIME DATA
                     NMIN           3  MINUTES IN COMPUTATION INTERVAL
                    IDATE      1    0  STARTING DATE
                    ITIME        0000  STARTING TIME
                       NQ        1000  NUMBER OF HYDROGRAPH ORDINATES
                   NDDATE      3    0  ENDING DATE
                   NDTIME        0157  ENDING TIME
                   ICENT           19  CENTURY MARK

                 COMPUTATION INTERVAL    0.05 HOURS
                      TOTAL TIME BASE   49.95 HOURS

          ENGLISH UNITS
               DRAINAGE AREA         SQUARE MILES
               PRECIPITATION DEPTH   INCHES
               LENGTH, ELEVATION     FEET
               FLOW                  CUBIC FEET PER SECOND
               STORAGE VOLUME        ACRE-FEET
               SURFACE AREA          ACRES
               TEMPERATURE           DEGREES FAHRENHEIT

33 JD          INDEX STORM NO. 1
                     STRM        2.76  PRECIPITATION DEPTH
                     TRDA        0.00  TRANSPOSITION DRAINAGE AREA

34 PI            PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                   0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
                   0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

37 JD          INDEX STORM NO. 2
                     STRM        2.74  PRECIPITATION DEPTH
                     TRDA        0.50  TRANSPOSITION DRAINAGE AREA

38 PI            PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                   0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
                   0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

41 JD          INDEX STORM NO. 3
                     STRM        2.69  PRECIPITATION DEPTH
                     TRDA        2.80  TRANSPOSITION DRAINAGE AREA

42 PI            PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                   0.04      0.04      0.04      0.04      0.04      0.05      0.05      0.05      0.05      0.05
                   0.03      0.03      0.03      0.03      0.03      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

45 JD          INDEX STORM NO. 4
                     STRM        2.54  PRECIPITATION DEPTH
                     TRDA       16.00  TRANSPOSITION DRAINAGE AREA
```

```
46 PI            PRECIPITATION PATTERN
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                    0.03      0.03      0.03      0.03      0.03      0.04      0.04      0.04      0.04      0.04
                    0.03      0.03      0.03      0.03      0.03      0.01      0.01      0.01      0.01      0.01
                    0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

49 JD          INDEX STORM NO. 5
                      STRM        2.23  PRECIPITATION DEPTH
                      TRDA       90.00  TRANSPOSITION DRAINAGE AREA

50 PI            PRECIPITATION PATTERN
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                    0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
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                    0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

53 JD          INDEX STORM NO. 6
                      STRM        3.66  PRECIPITATION DEPTH
                      TRDA       10.00  TRANSPOSITION DRAINAGE AREA

 0 PI            PRECIPITATION PATTERN
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
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                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                    0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                    0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03
                    0.02      0.02      0.02      0.02      0.02      0.01      0.01      0.01      0.01      0.01
                    0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

54 JD          INDEX STORM NO. 7
                      STRM        3.53  PRECIPITATION DEPTH
                      TRDA       20.00  TRANSPOSITION DRAINAGE AREA

 0 PI            PRECIPITATION PATTERN
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	76THST	50.	4.50	15.	4.	2.	0.24		
HYDROGRAPH AT	SB01A	103.	4.25	12.	3.	1.	0.06		
DIVERSION TO	RET01A	44.	4.05	3.	1.	0.	0.06		
HYDROGRAPH AT	SB01A	103.	4.25	9.	2.	1.	0.06		
2 COMBINED AT	CP-1	145.	4.25	24.	7.	3.	0.30		
ROUTED TO	R1A-2W	144.	4.30	24.	7.	3.	0.30		
HYDROGRAPH AT	SB02NW	43.	4.05	4.	1.	0.	0.02		
DIVERSION TO	RET2NW	14.	3.85	1.	0.	0.	0.02		

	HYDROGRAPH AT						
+	SB02NW	43.	4.05	3.	1.	0.	0.02
	2 COMBINED AT						
+	CP-02W	164.	4.25	27.	7.	4.	0.31
	ROUTED TO						
+	R2W-2	163.	4.25	27.	7.	4.	0.31
	HYDROGRAPH AT						
+	SB01C	51.	4.10	5.	1.	1.	0.03
	DIVERSION TO						
+	RET01C	20.	3.90	1.	0.	0.	0.03
	HYDROGRAPH AT						
+	SB01C	51.	4.10	4.	1.	0.	0.03
	ROUTED TO						
+	R1C-1B	50.	4.10	4.	1.	0.	0.03
	HYDROGRAPH AT						
+	SB01B	50.	4.10	5.	1.	1.	0.02
	DIVERSION TO						
+	RET01B	17.	3.90	1.	0.	0.	0.02
	HYDROGRAPH AT						
+	SB01B	50.	4.10	3.	1.	0.	0.02
	2 COMBINED AT						
+	CP-01B	100.	4.10	7.	2.	1.	0.05
	ROUTED TO						
+	R1B-2N	97.	4.15	7.	2.	1.	0.05
	HYDROGRAPH AT						
+	SB02NE	57.	4.15	6.	2.	1.	0.03
	DIVERSION TO						
+	RET2NE	27.	3.95	2.	0.	0.	0.03
	HYDROGRAPH AT						
+	SB02NE	57.	4.15	5.	1.	1.	0.03
	2 COMBINED AT						
+	CP-02N	154.	4.15	12.	3.	1.	0.09
	ROUTED TO						
+	R2N-2	154.	4.15	12.	3.	1.	0.09
	HYDROGRAPH AT						
+	SB02E	93.	4.10	10.	3.	1.	0.04
	DIVERSION TO						
+	RET2E	22.	3.70	2.	1.	0.	0.04
	HYDROGRAPH AT						
+	SB02E	93.	4.10	8.	2.	1.	0.04
	HYDROGRAPH AT						
+	SB02W	39.	4.50	6.	1.	1.	0.06
	4 COMBINED AT						
+	CP-02	395.	4.15	53.	14.	7.	0.18
	ROUTED TO						
+	R2-3	388.	4.25	53.	14.	7.	0.18
	HYDROGRAPH AT						
+	SB03	78.	4.15	10.	3.	1.	0.05
	2 COMBINED AT						
+	CP-03	458.	4.20	62.	16.	8.	0.23
	ROUTED TO						
+	R3-5	454.	4.35	62.	16.	8.	0.23
	HYDROGRAPH AT						
+	SB05	229.	4.10	25.	6.	3.	0.13

2 COMBINED AT							
+	CP-5A	581.	4.30	85.	23.	11.	0.36
HYDROGRAPH AT							
+	SB07W	104.	4.10	9.	2.	1.	0.05
DIVERSION TO							
+	RET07W	29.	3.85	2.	1.	0.	0.05
HYDROGRAPH AT							
+	SB07W	104.	4.10	7.	2.	1.	0.05
ROUTED TO							
+	R7W-8W	104.	4.45	8.	2.	1.	0.05
HYDROGRAPH AT							
+	SB8NW	40.	4.05	4.	1.	0.	0.01
DIVERSION TO							
+	RET8NW	6.	3.55	1.	0.	0.	0.01
HYDROGRAPH AT							
+	SB8NW	40.	4.05	3.	1.	0.	0.01
ROUTED TO							
+	R8NW8N	39.	4.05	3.	1.	0.	0.01
HYDROGRAPH AT							
+	SB8NE	42.	4.05	4.	1.	0.	0.01
DIVERSION TO							
+	RET8NE	6.	3.55	1.	0.	0.	0.01
HYDROGRAPH AT							
+	SB8NE	42.	4.05	3.	1.	0.	0.01
ROUTED TO							
+	R8NE8N	42.	4.05	3.	1.	0.	0.01
HYDROGRAPH AT							
+	SB8SW	37.	4.05	3.	1.	0.	0.01
DIVERSION TO							
+	RET8SW	5.	3.55	1.	0.	0.	0.01
HYDROGRAPH AT							
+	SB8SW	37.	4.05	3.	1.	0.	0.01
HYDROGRAPH AT							
+	SB8SE	36.	4.05	3.	1.	0.	0.01
DIVERSION TO							
+	RET8SE	5.	3.55	1.	0.	0.	0.01
HYDROGRAPH AT							
+	SB8SE	36.	4.05	3.	1.	0.	0.01
4 COMBINED AT							
+	CP-08N	153.	4.05	12.	3.	1.	0.06
ROUTED TO							
+	R8N-8W	150.	4.05	12.	3.	1.	0.06
HYDROGRAPH AT							
+	SB08W	145.	4.10	15.	4.	2.	0.06
DIVERSION TO							
+	RET8W	27.	3.65	3.	1.	0.	0.06
HYDROGRAPH AT							
+	SB08W	145.	4.10	12.	3.	1.	0.06
3 COMBINED AT							
+	CP-08W	287.	4.10	32.	8.	4.	0.16
ROUTED TO							
+	R8W-4	287.	4.10	32.	8.	4.	0.16
HYDROGRAPH AT							

+	SB04	292.	4.10	32.	8.	4.	0.14
	2 COMBINED AT						
+	CP-4	578.	4.10	64.	16.	8.	0.31
	ROUTED TO						
+	R4-5	567.	4.15	64.	16.	8.	0.31
	2 COMBINED AT						
+	CP-05	951.	4.30	146.	38.	19.	0.66
	ROUTED TO						
+	R5-6	926.	4.35	146.	38.	18.	0.66
	HYDROGRAPH AT						
+	SB06	243.	4.10	29.	7.	3.	0.14
	2 COMBINED AT						
+	CP-06	1020.	4.35	171.	45.	22.	0.80
	HYDROGRAPH AT						
+	SB07E	19.	4.40	3.	1.	0.	0.03
	ROUTED TO						
+	R7E-8E	19.	4.45	3.	1.	0.	0.03
	HYDROGRAPH AT						
+	SB08E	46.	4.55	8.	2.	1.	0.08
	HYDROGRAPH AT						
+	SB09N	13.	4.55	3.	1.	0.	0.03
	ROUTED TO						
+	R9N-R9	13.	4.65	3.	1.	0.	0.03
	3 COMBINED AT						
+	CP-08E	76.	4.55	13.	3.	2.	0.13
	ROUTED TO						
+	R8E-R9	76.	4.55	13.	3.	2.	0.13
	HYDROGRAPH AT						
+	SB09	71.	4.45	10.	2.	1.	0.09
	2 COMBINED AT						
+	CP-09	144.	4.50	23.	6.	3.	0.22
	HYDROGRAPH AT						
+	PWRCH	738.	5.75	429.	135.	74.	7.00
	2 COMBINED AT						
+	BINFLO	749.	5.75	439.	138.	76.	7.22
	ROUTED TO						
+	BASIN	241.	9.20	228.	130.	73.	7.22
	ROUTED TO						
+	BSNRT1	241.	9.25	228.	130.	73.	7.22
	ROUTED TO						
+	BSNRT2	241.	9.25	228.	130.	73.	7.22
	HYDROGRAPH AT						
+	SB10	87.	4.05	8.	2.	1.	0.04
	2 COMBINED AT						
+	CP-10	244.	9.20	232.	133.	76.	0.23
	ROUTED TO						
+	R10-11	244.	9.30	232.	133.	76.	0.23
	HYDROGRAPH AT						
+	SB11	130.	4.10	15.	4.	2.	0.07
	2 COMBINED AT						
+	CP-11	244.	9.30	232.	136.	77.	0.30

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SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	COMPUTATION INTERVAL DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1 STORM AREA (SQ MI) = 0.00									
R1A-2W	MANE	0.31	145.99	255.52	0.86	3.00	145.05	258.00	0.86

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1385E+02 EXCESS=0.0000E+00 OUTFLOW=0.1385E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50									
R1A-2W	MANE	0.31	145.24	255.66	0.86	3.00	144.23	258.00	0.86

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1380E+02 EXCESS=0.0000E+00 OUTFLOW=0.1380E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80									
R1A-2W	MANE	0.30	102.74	258.17	0.82	3.00	102.70	258.00	0.82

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1311E+02 EXCESS=0.0000E+00 OUTFLOW=0.1311E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR= 0.0

FOR STORM = 4 STORM AREA (SQ MI) = 16.00									
R1A-2W	MANE	0.30	86.26	258.55	0.77	3.00	86.14	258.00	0.77

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1240E+02 EXCESS=0.0000E+00 OUTFLOW=0.1240E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR= 0.0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00									
R1A-2W	MANE	0.35	70.99	270.31	0.71	3.00	70.97	270.00	0.71

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1143E+02 EXCESS=0.0000E+00 OUTFLOW=0.1143E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR= 0.0

FOR STORM = 6 STORM AREA (SQ MI) = 10.00									
R1A-2W	MANE	0.22	101.82	261.19	0.92	3.00	101.82	261.00	0.92

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1483E+02 EXCESS=0.0000E+00 OUTFLOW=0.1483E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR= 0.0

FOR STORM = 7 STORM AREA (SQ MI) = 20.00									
R1A-2W	MANE	0.27	99.04	261.08	0.90	3.00	99.04	261.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1451E+02 EXCESS=0.0000E+00 OUTFLOW=0.1451E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00									
R2W-2	MANE	0.36	164.36	255.38	0.94	3.00	163.85	255.00	0.94

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1526E+02 EXCESS=0.0000E+00 OUTFLOW=0.1526E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50									
R2W-2	MANE	0.27	163.57	255.50	0.93	3.00	162.94	255.00	0.93

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1520E+02 EXCESS=0.0000E+00 OUTFLOW=0.1520E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80									
R2W-2	MANE	0.35	117.20	258.14	0.88	3.00	117.19	258.00	0.88

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1428E+02 EXCESS=0.0000E+00 OUTFLOW=0.1428E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR= 0.0

FOR STORM = 4 STORM AREA (SQ MI) = 16.00									
R2W-2	MANE	0.38	96.83	258.51	0.82	3.00	96.72	258.00	0.82

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1335E+02 EXCESS=0.0000E+00 OUTFLOW=0.1335E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR= 0.0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R2W-2	MANE	0.37	75.72	261.93	0.74	3.00	75.66	261.00	0.74	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1207E+02 EXCESS=0.0000E+00 OUTFLOW=0.1206E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR= 0.0

FOR STORM = 6 STORM AREA (SQ MI) = 10.00

R2W-2	MANE	0.24	117.87	258.45	1.02	3.00	117.68	258.00	1.02	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1655E+02 EXCESS=0.0000E+00 OUTFLOW=0.1655E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR= 0.0

FOR STORM = 7 STORM AREA (SQ MI) = 20.00

R2W-2	MANE	0.24	114.22	258.50	0.99	3.00	114.02	258.00	0.99	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1613E+02 EXCESS=0.0000E+00 OUTFLOW=0.1613E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R1C-1B	MANE	0.30	51.03	246.76	1.36	3.00	50.29	246.00	1.37	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1961E+01 EXCESS=0.0000E+00 OUTFLOW=0.1962E+01 BASIN STORAGE=0.1121E-13 PERCENT ERROR= -0.1

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R1C-1B	MANE	0.40	50.59	246.72	1.35	3.00	49.89	246.00	1.35	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1941E+01 EXCESS=0.0000E+00 OUTFLOW=0.1942E+01 BASIN STORAGE=0.1138E-13 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R1C-1B	MANE	0.42	27.84	246.95	1.13	3.00	27.64	249.00	1.14	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1632E+01 EXCESS=0.0000E+00 OUTFLOW=0.1631E+01 BASIN STORAGE=0.1119E-13 PERCENT ERROR= 0.1

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R1C-1B	MANE	0.40	19.93	246.90	0.91	3.00	19.81	249.00	0.92	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1321E+01 EXCESS=0.0000E+00 OUTFLOW=0.1317E+01 BASIN STORAGE=0.1123E-13 PERCENT ERROR= 0.3

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R1C-1B	MANE	0.50	11.56	247.25	0.62	3.00	11.55	249.00	0.62	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8898E+00 EXCESS=0.0000E+00 OUTFLOW=0.8904E+00 BASIN STORAGE=0.1117E-13 PERCENT ERROR= -0.1

FOR STORM = 6 STORM AREA (SQ MI) = 10.00

R1C-1B	MANE	0.35	26.68	246.72	1.66	3.00	26.64	249.00	1.66	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2396E+01 EXCESS=0.0000E+00 OUTFLOW=0.2395E+01 BASIN STORAGE=0.1140E-13 PERCENT ERROR= 0.0

FOR STORM = 7 STORM AREA (SQ MI) = 20.00

R1C-1B	MANE	0.32	25.38	246.62	1.57	3.00	25.34	249.00	1.57	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2256E+01 EXCESS=0.0000E+00 OUTFLOW=0.2254E+01 BASIN STORAGE=0.1122E-13 PERCENT ERROR= 0.1

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R1B-2N	MANE	0.92	99.26	247.53	1.36	3.00	98.01	249.00	1.36	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3711E+01 EXCESS=0.0000E+00 OUTFLOW=0.3711E+01 BASIN STORAGE=0.3220E-11 PERCENT ERROR= 0.0

```
          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1B-2N  MANE         0.83     98.83    247.91      1.35      3.00     97.26    249.00        1.35

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3673E+01 EXCESS=0.0000E+00 OUTFLOW=0.3684E+01 BASIN STORAGE=0.3207E-11 PERCENT ERROR=  -0.3

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1B-2N  MANE         1.03     53.55    247.69      1.14      3.00     53.37    249.00        1.13

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3088E+01 EXCESS=0.0000E+00 OUTFLOW=0.3089E+01 BASIN STORAGE=0.3137E-11 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R1B-2N  MANE         1.09     38.34    248.42      0.92      3.00     38.21    249.00        0.92

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2500E+01 EXCESS=0.0000E+00 OUTFLOW=0.2500E+01 BASIN STORAGE=0.3264E-11 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R1B-2N  MANE         1.19     22.19    248.72      0.62      3.00     22.17    249.00        0.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1684E+01 EXCESS=0.0000E+00 OUTFLOW=0.1686E+01 BASIN STORAGE=0.3255E-11 PERCENT ERROR=  -0.1

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R1B-2N  MANE         0.90     51.28    247.95      1.67      3.00     51.12    249.00        1.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4529E+01 EXCESS=0.0000E+00 OUTFLOW=0.4530E+01 BASIN STORAGE=0.3257E-11 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R1B-2N  MANE         1.05     48.69    248.70      1.57      3.00     48.64    249.00        1.57

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4264E+01 EXCESS=0.0000E+00 OUTFLOW=0.4266E+01 BASIN STORAGE=0.3118E-11 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
               R2N-2  MANE         0.31    154.68    249.16      1.36      3.00    154.46    249.00        1.36

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6138E+01 EXCESS=0.0000E+00 OUTFLOW=0.6147E+01 BASIN STORAGE=0.5105E-06 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
               R2N-2  MANE         0.31    153.62    249.29      1.34      3.00    153.29    249.00        1.35

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6080E+01 EXCESS=0.0000E+00 OUTFLOW=0.6088E+01 BASIN STORAGE=0.5465E-06 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
               R2N-2  MANE         0.42     85.54    249.46      1.13      3.00     85.38    249.00        1.13

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5110E+01 EXCESS=0.0000E+00 OUTFLOW=0.5111E+01 BASIN STORAGE=0.4442E-06 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
               R2N-2  MANE         0.42     61.35    249.48      0.91      3.00     61.19    249.00        0.91

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4137E+01 EXCESS=0.0000E+00 OUTFLOW=0.4138E+01 BASIN STORAGE=0.5190E-06 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
               R2N-2  MANE         0.47     35.73    249.69      0.62      3.00     35.56    249.00        0.61

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2788E+01 EXCESS=0.0000E+00 OUTFLOW=0.2789E+01 BASIN STORAGE=0.5140E-06 PERCENT ERROR=  -0.1

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
               R2N-2  MANE         0.30     82.46    249.44      1.65      3.00     82.26    249.00        1.65
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7500E+01 EXCESS=0.0000E+00 OUTFLOW=0.7502E+01 BASIN STORAGE=0.4816E-06 PERCENT ERROR= 0.0

FOR STORM = 7 STORM AREA (SQ MI) = 20.00

R2N-2	MANE	0.39	78.41	249.52	1.56	3.00	78.24	249.00	1.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7064E+01 EXCESS=0.0000E+00 OUTFLOW=0.7065E+01 BASIN STORAGE=0.4315E-06 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R2-3	MANE	1.87	394.92	253.51	2.92	3.00	390.70	255.00	2.92

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2831E+02 EXCESS=0.0000E+00 OUTFLOW=0.2825E+02 BASIN STORAGE=0.4180E-01 PERCENT ERROR= 0.1

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R2-3	MANE	1.87	391.85	253.62	2.90	3.00	387.99	255.00	2.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2813E+02 EXCESS=0.0000E+00 OUTFLOW=0.2806E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= 0.1

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R2-3	MANE	2.18	249.49	258.70	2.55	3.00	249.46	258.00	2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2474E+02 EXCESS=0.0000E+00 OUTFLOW=0.2468E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= 0.1

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R2-3	MANE	2.42	191.22	260.01	2.23	3.00	190.55	261.00	2.23

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2169E+02 EXCESS=0.0000E+00 OUTFLOW=0.2163E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= 0.1

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R2-3	MANE	2.74	131.58	263.67	1.83	3.00	131.54	261.00	1.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1774E+02 EXCESS=0.0000E+00 OUTFLOW=0.1770E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= 0.0

FOR STORM = 6 STORM AREA (SQ MI) = 10.00

R2-3	MANE	2.27	254.73	261.04	3.27	3.00	254.71	261.00	3.27

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3175E+02 EXCESS=0.0000E+00 OUTFLOW=0.3170E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= 0.0

FOR STORM = 7 STORM AREA (SQ MI) = 20.00

R2-3	MANE	2.30	243.97	260.84	3.14	3.00	243.92	261.00	3.14

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3044E+02 EXCESS=0.0000E+00 OUTFLOW=0.3040E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R3-5	MANE	3.00	458.43	261.68	2.73	3.00	458.12	261.00	2.73

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3337E+02 EXCESS=0.0000E+00 OUTFLOW=0.3338E+02 BASIN STORAGE=0.7696E-01 PERCENT ERROR= -0.2

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R3-5	MANE	3.00	455.77	258.51	2.71	3.00	454.00	261.00	2.71

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3315E+02 EXCESS=0.0000E+00 OUTFLOW=0.3316E+02 BASIN STORAGE=0.7696E-01 PERCENT ERROR= -0.3

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R3-5	MANE	3.00	294.25	265.05	2.39	3.00	293.16	267.00	2.39

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2933E+02 EXCESS=0.0000E+00 OUTFLOW=0.2923E+02 BASIN STORAGE=0.8862E-01 PERCENT ERROR= 0.0

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          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
              R3-5  MANE         3.00    223.97    267.76      2.10      3.00    223.17    267.00        2.10

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2574E+02 EXCESS=0.0000E+00 OUTFLOW=0.2567E+02 BASIN STORAGE=0.9261E-01 PERCENT ERROR=  -0.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
              R3-5  MANE         3.00    152.87    273.07      1.71      3.00    152.80    273.00        1.71

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2102E+02 EXCESS=0.0000E+00 OUTFLOW=0.2089E+02 BASIN STORAGE=0.9561E-01 PERCENT ERROR=   0.2

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
              R3-5  MANE         2.96    299.06    267.64      3.09      3.00    298.54    270.00        3.09

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3795E+02 EXCESS=0.0000E+00 OUTFLOW=0.3782E+02 BASIN STORAGE=0.9250E-01 PERCENT ERROR=   0.1

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
              R3-5  MANE         2.96    286.26    268.14      2.96      3.00    285.63    270.00        2.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3638E+02 EXCESS=0.0000E+00 OUTFLOW=0.3628E+02 BASIN STORAGE=0.8862E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7W-8W  MANE         2.73    102.63    267.82      1.79      3.00    102.19    267.00        1.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3402E+01 EXCESS=0.0000E+00 OUTFLOW=0.4461E+01 BASIN STORAGE=0.3428E-03 PERCENT ERROR= -31.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R7W-8W  MANE         2.59    104.73    266.79      1.55      3.00    104.06    267.00        1.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3367E+01 EXCESS=0.0000E+00 OUTFLOW=0.3871E+01 BASIN STORAGE=0.3311E-03 PERCENT ERROR= -15.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R7W-8W  MANE         2.99     52.62    270.98      1.33      3.00     52.50    273.00        1.33

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2832E+01 EXCESS=0.0000E+00 OUTFLOW=0.3326E+01 BASIN STORAGE=0.3101E-03 PERCENT ERROR= -17.5

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R7W-8W  MANE         3.00     37.32    275.61      1.13      3.00     37.24    276.00        1.13

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2292E+01 EXCESS=0.0000E+00 OUTFLOW=0.2822E+01 BASIN STORAGE=0.3108E-03 PERCENT ERROR= -23.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R7W-8W  MANE         3.00     21.45    284.63      0.78      3.00     21.39    282.00        0.78

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1544E+01 EXCESS=0.0000E+00 OUTFLOW=0.1942E+01 BASIN STORAGE=0.3316E-03 PERCENT ERROR= -25.8

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R7W-8W  MANE         3.00     49.46    273.94      1.67      3.00     49.38    273.00        1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4156E+01 EXCESS=0.0000E+00 OUTFLOW=0.4179E+01 BASIN STORAGE=0.3376E-03 PERCENT ERROR=  -0.6

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R7W-8W  MANE         3.00     47.01    271.91      1.58      3.00     46.97    273.00        1.58

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3912E+01 EXCESS=0.0000E+00 OUTFLOW=0.3956E+01 BASIN STORAGE=0.3474E-03 PERCENT ERROR=  -1.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NW8N  MANE         0.75     40.02    244.34      1.98      3.00     39.44    243.00        1.98
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1550E+01 EXCESS=0.0000E+00 OUTFLOW=0.1552E+01 BASIN STORAGE=0.1215E-06 PERCENT ERROR= -0.1

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R8NW8N	MANE	0.77	39.68	244.02	1.96	3.00	39.17	243.00	1.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1538E+01 EXCESS=0.0000E+00 OUTFLOW=0.1540E+01 BASIN STORAGE=0.1352E-06 PERCENT ERROR= -0.2

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R8NW8N	MANE	0.97	21.41	244.47	1.86	3.00	21.33	243.00	1.86

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1449E+01 EXCESS=0.0000E+00 OUTFLOW=0.1455E+01 BASIN STORAGE=0.1154E-06 PERCENT ERROR= -0.4

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R8NW8N	MANE	1.05	16.17	244.69	1.68	3.00	16.11	243.00	1.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1313E+01 EXCESS=0.0000E+00 OUTFLOW=0.1317E+01 BASIN STORAGE=0.1153E-06 PERCENT ERROR= -0.2

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R8NW8N	MANE	1.03	10.82	245.03	1.37	3.00	10.72	243.00	1.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1074E+01 EXCESS=0.0000E+00 OUTFLOW=0.1077E+01 BASIN STORAGE=0.1398E-06 PERCENT ERROR= -0.3

FOR STORM = 6 STORM AREA (SQ MI) = 10.00

R8NW8N	MANE	0.91	19.33	244.37	2.65	3.00	19.25	243.00	2.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2076E+01 EXCESS=0.0000E+00 OUTFLOW=0.2081E+01 BASIN STORAGE=0.1314E-06 PERCENT ERROR= -0.2

FOR STORM = 7 STORM AREA (SQ MI) = 20.00

R8NW8N	MANE	0.88	18.61	244.50	2.54	3.00	18.51	243.00	2.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1987E+01 EXCESS=0.0000E+00 OUTFLOW=0.1989E+01 BASIN STORAGE=0.1316E-06 PERCENT ERROR= -0.1

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R8NE8N	MANE	0.69	42.06	244.33	1.98	3.00	41.68	243.00	1.98

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1579E+01 EXCESS=0.0000E+00 OUTFLOW=0.1583E+01 BASIN STORAGE=0.1124E-06 PERCENT ERROR= -0.2

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R8NE8N	MANE	0.68	41.82	244.25	1.96	3.00	41.41	243.00	1.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1567E+01 EXCESS=0.0000E+00 OUTFLOW=0.1569E+01 BASIN STORAGE=0.1055E-06 PERCENT ERROR= -0.1

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R8NE8N	MANE	0.92	22.13	243.92	1.85	3.00	22.08	243.00	1.86

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1476E+01 EXCESS=0.0000E+00 OUTFLOW=0.1484E+01 BASIN STORAGE=0.1034E-06 PERCENT ERROR= -0.5

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R8NE8N	MANE	0.97	16.69	243.96	1.68	3.00	16.65	243.00	1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1338E+01 EXCESS=0.0000E+00 OUTFLOW=0.1340E+01 BASIN STORAGE=0.1343E-06 PERCENT ERROR= -0.2

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R8NE8N	MANE	1.12	11.10	244.46	1.37	3.00	11.06	243.00	1.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1094E+01 EXCESS=0.0000E+00 OUTFLOW=0.1095E+01 BASIN STORAGE=0.1098E-06 PERCENT ERROR= -0.1

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          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R8NE8N  MANE         0.90     19.88    244.18      2.65      3.00     19.84    243.00        2.65


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2116E+01 EXCESS=0.0000E+00 OUTFLOW=0.2117E+01 BASIN STORAGE=0.9623E-07 PERCENT ERROR=  -0.1


          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R8NE8N  MANE         0.86     19.14    244.71      2.54      3.00     19.09    243.00        2.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2025E+01 EXCESS=0.0000E+00 OUTFLOW=0.2028E+01 BASIN STORAGE=0.1302E-06 PERCENT ERROR=  -0.2


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8N-8W  MANE         0.52    152.44    243.65      1.98      3.00    150.95    243.00        1.98


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5871E+01 EXCESS=0.0000E+00 OUTFLOW=0.5874E+01 BASIN STORAGE=0.1353E-07 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8N-8W  MANE         0.51    151.61    243.84      1.96      3.00    149.94    243.00        1.96


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+01 EXCESS=0.0000E+00 OUTFLOW=0.5823E+01 BASIN STORAGE=0.1343E-07 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8N-8W  MANE         0.53     81.55    243.95      1.85      3.00     81.15    243.00        1.85


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5500E+01 EXCESS=0.0000E+00 OUTFLOW=0.5501E+01 BASIN STORAGE=0.1336E-07 PERCENT ERROR=   0.0


          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R8N-8W  MANE         0.65     61.43    243.75      1.68      3.00     61.23    243.00        1.68


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4978E+01 EXCESS=0.0000E+00 OUTFLOW=0.4978E+01 BASIN STORAGE=0.1320E-07 PERCENT ERROR=   0.0


          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R8N-8W  MANE         0.74     40.89    244.25      1.37      3.00     40.71    243.00        1.37


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4070E+01 EXCESS=0.0000E+00 OUTFLOW=0.4071E+01 BASIN STORAGE=0.1350E-07 PERCENT ERROR=   0.0


          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R8N-8W  MANE         0.57     73.39    244.05      2.65      3.00     73.13    243.00        2.65


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7867E+01 EXCESS=0.0000E+00 OUTFLOW=0.7867E+01 BASIN STORAGE=0.1321E-07 PERCENT ERROR=   0.0


          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R8N-8W  MANE         0.64     70.52    244.05      2.53      3.00     70.34    243.00        2.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7526E+01 EXCESS=0.0000E+00 OUTFLOW=0.7527E+01 BASIN STORAGE=0.1341E-07 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R8W-4  MANE         0.61    288.74    246.26      1.91      3.00    288.70    246.00        1.91


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1646E+02 EXCESS=0.0000E+00 OUTFLOW=0.1645E+02 BASIN STORAGE=0.1371E-03 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R8W-4  MANE         0.53    286.94    246.33      1.83      3.00    286.84    246.00        1.83


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1577E+02 EXCESS=0.0000E+00 OUTFLOW=0.1578E+02 BASIN STORAGE=0.1364E-03 PERCENT ERROR=  -0.1


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
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            R8W-4  MANE         0.74    161.70    245.00      1.69      3.00    161.56    246.00        1.69

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1454E+02 EXCESS=0.0000E+00 OUTFLOW=0.1454E+02 BASIN STORAGE=0.1408E-03 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R8W-4  MANE         0.78    122.55    244.62      1.51      3.00    122.49    246.00        1.51

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1298E+02 EXCESS=0.0000E+00 OUTFLOW=0.1297E+02 BASIN STORAGE=0.1356E-03 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R8W-4  MANE         0.97     82.08    246.65      1.19      3.00     82.01    246.00        1.19

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1023E+02 EXCESS=0.0000E+00 OUTFLOW=0.1023E+02 BASIN STORAGE=0.1378E-03 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R8W-4  MANE         0.72    147.07    246.77      2.36      3.00    147.06    246.00        2.36

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2027E+02 EXCESS=0.0000E+00 OUTFLOW=0.2028E+02 BASIN STORAGE=0.1355E-03 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R8W-4  MANE         0.85    141.44    246.46      2.25      3.00    141.43    246.00        2.25

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1935E+02 EXCESS=0.0000E+00 OUTFLOW=0.1936E+02 BASIN STORAGE=0.1431E-03 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R4-5  MANE         2.25    581.22    250.37      2.01      3.00    571.59    249.00        2.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3268E+02 EXCESS=0.0000E+00 OUTFLOW=0.3273E+02 BASIN STORAGE=0.4091E-03 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R4-5  MANE         2.30    574.86    250.76      1.96      3.00    566.30    249.00        1.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3188E+02 EXCESS=0.0000E+00 OUTFLOW=0.3195E+02 BASIN STORAGE=0.4397E-03 PERCENT ERROR=  -0.2

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R4-5  MANE         2.78    331.68    249.80      1.81      3.00    330.74    252.00        1.81

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2948E+02 EXCESS=0.0000E+00 OUTFLOW=0.2950E+02 BASIN STORAGE=0.3881E-03 PERCENT ERROR=  -0.1

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
             R4-5  MANE         3.00    250.94    251.05      1.62      3.00    249.92    252.00        1.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2640E+02 EXCESS=0.0000E+00 OUTFLOW=0.2643E+02 BASIN STORAGE=0.3813E-03 PERCENT ERROR=  -0.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
             R4-5  MANE         3.00    164.62    254.83      1.31      3.00    164.54    252.00        1.31

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2125E+02 EXCESS=0.0000E+00 OUTFLOW=0.2129E+02 BASIN STORAGE=0.3682E-03 PERCENT ERROR=  -0.2

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
             R4-5  MANE         2.85    309.77    251.59      2.49      3.00    309.09    252.00        2.49

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4041E+02 EXCESS=0.0000E+00 OUTFLOW=0.4051E+02 BASIN STORAGE=0.3879E-03 PERCENT ERROR=  -0.2

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
             R4-5  MANE         2.86    295.96    250.39      2.38      3.00    295.45    252.00        2.38
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3868E+02 EXCESS=0.0000E+00 OUTFLOW=0.3875E+02 BASIN STORAGE=0.3882E-03 PERCENT ERROR= -0.2

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R5-6	MANE	2.20	1010.21	262.68	2.23	3.00	1000.34	261.00	2.23

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7880E+02 EXCESS=0.0000E+00 OUTFLOW=0.7874E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R5-6	MANE	2.21	994.60	262.88	2.20	3.00	987.77	261.00	2.20

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7768E+02 EXCESS=0.0000E+00 OUTFLOW=0.7757E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R5-6	MANE	2.48	665.14	266.57	1.99	3.00	662.19	267.00	1.99

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7008E+02 EXCESS=0.0000E+00 OUTFLOW=0.7004E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR= 0.0

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R5-6	MANE	2.81	502.74	267.30	1.76	3.00	499.70	267.00	1.76

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6201E+02 EXCESS=0.0000E+00 OUTFLOW=0.6198E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR= -0.1

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R5-6	MANE	3.36	325.91	273.89	1.42	3.00	322.56	276.00	1.42

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5001E+02 EXCESS=0.0000E+00 OUTFLOW=0.4998E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR= -0.1

FOR STORM = 6 STORM AREA (SQ MI) = 10.00

R5-6	MANE	2.61	680.31	263.26	2.65	3.00	679.88	264.00	2.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9360E+02 EXCESS=0.0000E+00 OUTFLOW=0.9357E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR= 0.0

FOR STORM = 7 STORM AREA (SQ MI) = 20.00

R5-6	MANE	2.52	649.46	264.71	2.54	3.00	649.25	264.00	2.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8961E+02 EXCESS=0.0000E+00 OUTFLOW=0.8960E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR= -0.1

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R7E-8E	MANE	1.76	19.03	266.67	0.91	3.00	19.02	267.00	0.91

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1268E+01 EXCESS=0.0000E+00 OUTFLOW=0.1268E+01 BASIN STORAGE=0.1101E-04 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R7E-8E	MANE	1.77	18.85	266.81	0.90	3.00	18.84	267.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1254E+01 EXCESS=0.0000E+00 OUTFLOW=0.1254E+01 BASIN STORAGE=0.1092E-04 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R7E-8E	MANE	2.41	8.57	268.50	0.49	3.00	8.54	270.00	0.49

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6750E+00 EXCESS=0.0000E+00 OUTFLOW=0.6756E+00 BASIN STORAGE=0.1319E-04 PERCENT ERROR= -0.1

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R7E-8E	MANE	3.00	3.99	269.13	0.21	3.00	3.99	270.00	0.21

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2917E+00 EXCESS=0.0000E+00 OUTFLOW=0.2928E+00 BASIN STORAGE=0.1256E-04 PERCENT ERROR= -0.4

```
          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R7E-8E  MANE         3.00      0.26    292.33      0.01      3.00      0.25    294.00        0.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1646E-01 EXCESS=0.0000E+00 OUTFLOW=0.2054E-01 BASIN STORAGE=0.1096E-04 PERCENT ERROR= -24.8

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R7E-8E  MANE         2.36      9.61    272.07      0.60      3.00      9.60    273.00        0.60

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8313E+00 EXCESS=0.0000E+00 OUTFLOW=0.8321E+00 BASIN STORAGE=0.1454E-04 PERCENT ERROR=  -0.1

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R7E-8E  MANE         2.43      8.70    271.22      0.54      3.00      8.69    273.00        0.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7492E+00 EXCESS=0.0000E+00 OUTFLOW=0.7500E+00 BASIN STORAGE=0.1049E-04 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R9N-R9  MANE         2.26     13.31    278.00      1.08      3.00     13.26    279.00        1.08

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1613E+01 EXCESS=0.0000E+00 OUTFLOW=0.1611E+01 BASIN STORAGE=0.2018E-04 PERCENT ERROR=   0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R9N-R9  MANE         2.27     13.16    278.20      1.07      3.00     13.12    279.00        1.07

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1595E+01 EXCESS=0.0000E+00 OUTFLOW=0.1593E+01 BASIN STORAGE=0.1743E-04 PERCENT ERROR=   0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R9N-R9  MANE         2.76      8.13    284.79      0.74      3.00      8.13    285.00        0.74

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1103E+01 EXCESS=0.0000E+00 OUTFLOW=0.1102E+01 BASIN STORAGE=0.2044E-04 PERCENT ERROR=   0.1

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R9N-R9  MANE         3.00      5.05    284.63      0.46      3.00      5.05    285.00        0.46

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6843E+00 EXCESS=0.0000E+00 OUTFLOW=0.6838E+00 BASIN STORAGE=0.2050E-04 PERCENT ERROR=   0.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R9N-R9  MANE         3.00      1.59    293.56      0.14      3.00      1.59    294.00        0.14

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2079E+00 EXCESS=0.0000E+00 OUTFLOW=0.2079E+00 BASIN STORAGE=0.1894E-04 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R9N-R9  MANE         2.65      9.46    288.90      0.92      3.00      9.46    288.00        0.92

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1380E+01 EXCESS=0.0000E+00 OUTFLOW=0.1379E+01 BASIN STORAGE=0.1805E-04 PERCENT ERROR=   0.1

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R9N-R9  MANE         2.68      8.74    288.39      0.84      3.00      8.73    288.00        0.84

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1261E+01 EXCESS=0.0000E+00 OUTFLOW=0.1260E+01 BASIN STORAGE=0.1932E-04 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8E-R9  MANE         0.32     77.02    273.53      0.96      3.00     76.82    273.00        0.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6728E+01 EXCESS=0.0000E+00 OUTFLOW=0.6728E+01 BASIN STORAGE=0.7838E-05 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8E-R9  MANE         0.32     76.18    273.66      0.95      3.00     75.99    273.00        0.95
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6652E+01 EXCESS=0.0000E+00 OUTFLOW=0.6653E+01 BASIN STORAGE=0.7629E-05 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R8E-R9	MANE	0.45	38.17	276.54	0.55	3.00	38.13	276.00	0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3882E+01 EXCESS=0.0000E+00 OUTFLOW=0.3882E+01 BASIN STORAGE=0.7462E-05 PERCENT ERROR= 0.0

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R8E-R9	MANE	0.52	19.07	276.61	0.27	3.00	19.04	276.00	0.27

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1925E+01 EXCESS=0.0000E+00 OUTFLOW=0.1925E+01 BASIN STORAGE=0.7540E-05 PERCENT ERROR= 0.0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R8E-R9	MANE	0.94	2.52	293.35	0.04	3.00	2.51	294.00	0.04

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3008E+00 EXCESS=0.0000E+00 OUTFLOW=0.3008E+00 BASIN STORAGE=0.6726E-05 PERCENT ERROR= 0.0

FOR STORM = 6 STORM AREA (SQ MI) = 10.00

R8E-R9	MANE	0.36	44.12	279.67	0.68	3.00	44.08	279.00	0.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4782E+01 EXCESS=0.0000E+00 OUTFLOW=0.4782E+01 BASIN STORAGE=0.7472E-05 PERCENT ERROR= 0.0

FOR STORM = 7 STORM AREA (SQ MI) = 20.00

R8E-R9	MANE	0.43	40.11	279.56	0.62	3.00	40.07	279.00	0.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4334E+01 EXCESS=0.0000E+00 OUTFLOW=0.4334E+01 BASIN STORAGE=0.7666E-05 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

BSNRT1	MANE	0.39	244.41	552.63	0.80	3.00	244.41	552.00	0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3084E+03 EXCESS=0.0000E+00 OUTFLOW=0.3084E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

BSNRT1	MANE	0.39	244.36	552.54	0.80	3.00	244.36	552.00	0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3083E+03 EXCESS=0.0000E+00 OUTFLOW=0.3082E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

BSNRT1	MANE	0.41	241.71	552.95	0.79	3.00	241.71	552.00	0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3040E+03 EXCESS=0.0000E+00 OUTFLOW=0.3039E+03 BASIN STORAGE=0.3152E-01 PERCENT ERROR= 0.0

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

BSNRT1	MANE	0.33	239.65	552.95	0.78	3.00	239.64	555.00	0.78

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3008E+03 EXCESS=0.0000E+00 OUTFLOW=0.3007E+03 BASIN STORAGE=0.3152E-01 PERCENT ERROR= 0.0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

BSNRT1	MANE	0.44	237.69	555.51	0.77	3.00	237.69	555.00	0.77

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2977E+03 EXCESS=0.0000E+00 OUTFLOW=0.2976E+03 BASIN STORAGE=0.3151E-01 PERCENT ERROR= 0.0

FOR STORM = 6 STORM AREA (SQ MI) = 10.00

BSNRT1	MANE	0.46	242.84	552.79	0.79	3.00	242.84	552.00	0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3057E+03 EXCESS=0.0000E+00 OUTFLOW=0.3057E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR= 0.0

```
          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            BSNRT1  MANE         0.37    242.32    552.68      0.79      3.00    242.32    552.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3049E+03 EXCESS=0.0000E+00 OUTFLOW=0.3049E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE         0.50    244.41    552.74      0.80      3.00    244.41    552.00        0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3084E+03 EXCESS=0.0000E+00 OUTFLOW=0.3083E+03 BASIN STORAGE=0.3601E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            BSNRT2  MANE         0.43    244.36    552.89      0.80      3.00    244.36    552.00        0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3082E+03 EXCESS=0.0000E+00 OUTFLOW=0.3082E+03 BASIN STORAGE=0.3601E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            BSNRT2  MANE         0.49    241.71    553.29      0.79      3.00    241.71    555.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3040E+03 EXCESS=0.0000E+00 OUTFLOW=0.3039E+03 BASIN STORAGE=0.3600E-01 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            BSNRT2  MANE         0.36    239.64    555.76      0.78      3.00    239.64    555.00        0.78

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3007E+03 EXCESS=0.0000E+00 OUTFLOW=0.3007E+03 BASIN STORAGE=0.3599E-01 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            BSNRT2  MANE         0.37    237.69    555.79      0.77      3.00    237.69    555.00        0.77

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2976E+03 EXCESS=0.0000E+00 OUTFLOW=0.2976E+03 BASIN STORAGE=0.3598E-01 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            BSNRT2  MANE         0.50    242.84    553.18      0.79      3.00    242.83    552.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3057E+03 EXCESS=0.0000E+00 OUTFLOW=0.3057E+03 BASIN STORAGE=0.3600E-01 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            BSNRT2  MANE         0.36    242.32    552.92      0.79      3.00    242.31    555.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3049E+03 EXCESS=0.0000E+00 OUTFLOW=0.3048E+03 BASIN STORAGE=0.3600E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R10-11  MANE         3.04    244.41    557.55     25.54      3.00    244.41    558.00       25.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3125E+03 EXCESS=0.0000E+00 OUTFLOW=0.3119E+03 BASIN STORAGE=0.6746E+00 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R10-11  MANE         3.07    244.36    558.85     25.53      3.00    244.35    558.00       25.52

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3124E+03 EXCESS=0.0000E+00 OUTFLOW=0.3117E+03 BASIN STORAGE=0.6746E+00 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R10-11  MANE         3.00    241.71    559.89     25.15      3.00    241.70    561.00       25.13

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3077E+03 EXCESS=0.0000E+00 OUTFLOW=0.3071E+03 BASIN STORAGE=0.6744E+00 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R10-11  MANE         3.00    239.64    561.16     24.84      3.00    239.64    561.00       24.84
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3040E+03 EXCESS=0.0000E+00 OUTFLOW=0.3034E+03 BASIN STORAGE=0.6743E+00 PERCENT ERROR= 0.0

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R10-11	MANE	3.07	237.68	560.85	24.53	3.00	237.68	561.00	24.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3003E+03 EXCESS=0.0000E+00 OUTFLOW=0.2996E+03 BASIN STORAGE=0.6743E+00 PERCENT ERROR= 0.0

FOR STORM = 6 STORM AREA (SQ MI) = 10.00

R10-11	MANE	3.07	242.83	560.81	25.40	3.00	242.83	558.00	25.38

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3108E+03 EXCESS=0.0000E+00 OUTFLOW=0.3102E+03 BASIN STORAGE=0.6744E+00 PERCENT ERROR= 0.0

FOR STORM = 7 STORM AREA (SQ MI) = 20.00

R10-11	MANE	3.00	242.31	558.78	25.30	3.00	242.31	561.00	25.30

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3097E+03 EXCESS=0.0000E+00 OUTFLOW=0.3090E+03 BASIN STORAGE=0.6745E+00 PERCENT ERROR= 0.0

*** NORMAL END OF HEC-1 ***

100yr-24hr-Ultimate

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1****************************************
****************************************
 *                                      *                                                       *
*
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)  *                                                       *    U.S. ARMY CORPS OF ENGINEERS
*
 *               JUN   1998             *                                                       *    HYDROLOGIC ENGINEERING CENTER
*
 *            VERSION 4.1               *                                                       *          609 SECOND STREET
*
 *                                      *                                                       *       DAVIS, CALIFORNIA 95616
*
 *  RUN DATE   26APR21  TIME  09:31:12  *                                                       *           (916) 756-1104
*
 *                                      *                                                       *
*
 ****************************************
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X     X  XXXXXXX   XXXXX           X
X     X  X        X     X         XX
X     X  X        X                X
XXXXXXX  XXXX     X        XXXXX   X
X     X  X        X                X
X     X  X        X     X          X
X     X  XXXXXXX   XXXXX          XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


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          LINE      ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

            1       ID        CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
            2       ID        POWERLINE CORRIDOR
            3       ID        RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
            4       ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
            5       ID        100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
            6       ID           PIMA ROAD FLOWS.
            7       ID
            8       ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
            9       ID        MODELERS: RK, MW
           10       ID
           11       ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 08/07/18
           12       ID        MODELERS: MSW, ES
           13       ID        REVISIONS NOTED WITH HE
           14       ID           NEW PROP CONDITION MODEL,WITH POWERLINE AND BASIN 53R
           15       ID           THIS IS THE ULTIMATE CONDITION MODEL
           16       ID
           17       ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
           18       ID        MODELERS: MSW
           19       ID        XKSAT ADJUSTMENT
           20       ID
           21       ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
           22       ID        MODELERS: MSW, TSW
           23       ID        REVISIONS NOTED WITH HE (Hubbard Engineering)
           24       ID           NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
           25       ID           CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
           26       ID           ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
           27       ID           FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED
           28       ID
           29       IT       3       0       0    1000
           30       IN      15
           31       IO       5
                    *DIAGRAM
                    *
```

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 32          JD   3.849  0.0001
 33          PC   0.000   0.002   0.005   0.008   0.011   0.014   0.017   0.020   0.023   0.026
 34          PC   0.029   0.032   0.035   0.038   0.041   0.044   0.048   0.052   0.056   0.060
 35          PC   0.064   0.068   0.072   0.076   0.080   0.085   0.090   0.095   0.100   0.105
 36          PC   0.110   0.115   0.120   0.126   0.133   0.140   0.147   0.155   0.163   0.172
 37          PC   0.181   0.191   0.203   0.218   0.236   0.257   0.283   0.387   0.663   0.707
 38          PC   0.735   0.758   0.776   0.791   0.804   0.815   0.825   0.834   0.842   0.849
 39          PC   0.856   0.863   0.869   0.875   0.881   0.887   0.893   0.898   0.903   0.908
 40          PC   0.913   0.918   0.922   0.926   0.930   0.934   0.938   0.942   0.946   0.950
 41          PC   0.953   0.956   0.959   0.962   0.965   0.968   0.971   0.974   0.977   0.980
 42          PC   0.983   0.986   0.989   0.992   0.995   0.998   1.000
 43          JD   3.657    10.0
 44          JD   3.533    20.0
             *

 45          KK  76THST
 46          KM        76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
 47          KM        100-YR, 24-HR FLO-2D MODEL (XS 98)
 48          BA    0.24
 49          QI    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00
 50          QI    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00
 51          QI    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.01
1                                       HEC-1 INPUT                                          PAGE  2

LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 52          QI    0.00    0.00    0.00    0.00    0.01    0.00    0.00    0.02    0.03    0.05
 53          QI    0.09    0.11    0.14    0.21    0.34    0.51    0.76    3.70   17.69   47.79
 54          QI   68.57   54.61   42.75   35.61   32.34   29.25   26.13   22.83   19.41   18.11
 55          QI   16.15   14.54   13.75   12.53   12.00   11.44   10.52    9.81    9.34    9.09
 56          QI    9.11    8.71    8.37    8.05    7.67    7.75    7.47    7.08    6.97    6.77
 57          QI    6.59    6.44    6.32    6.12    5.95    5.87    5.70    5.84    5.41    5.44
 58          QI    4.56    4.81    4.75    4.78    4.69    4.67    4.61    4.36    4.20    4.00
 59          QI    3.75    3.51    2.49    3.43    3.61    3.65    2.48    2.36    2.19    2.07
 60          QI    1.94    1.72    1.71    1.36    1.48    1.13    1.13    1.20    1.10    1.06
 61          QI    1.06    1.00    0.94    0.89    0.85    0.81    0.78    0.74    0.72    0.70
 62          QI    0.67    0.64    0.62    0.59    0.57    0.55    0.53    0.51    0.49    0.47
 63          QI    0.45    0.44    0.42    0.40    0.39
             *

 64          KK   SB01A   BASIN
 65          BA   0.061
 66          PB   2.721
 67          PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.075
 68          PC   0.087   0.099   0.119   0.150   0.234   0.413   0.766   0.875   0.916   0.944
 69          PC   0.956   0.968   0.979   0.990   1.000
 70          LG    0.25    0.25    4.03    0.56      45
 71          UC   0.439   0.231
 72          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 73          UA     100
             *

 74          KK   SB01A  DIVERT
 75          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01A
 76          KM        FIRST FLUSH VOL = 1.383 AC-FT
 77          DT  RET01A   1.383
 78          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 79          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
             *

 80          KK    CP-1 COMBINE
 81          KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
 82          HC       2   0.301
             *

 83          KK  R1A-2W   ROUTE
 84          KM        ROUTE CP-1 TO CP-2W
 85          RK    1114  0.0200   0.018            TRAP   2.000    0.00
             *

 86          KK  SB02NW   BASIN
 87          KM        BASIN SB02E WAS SPLIT INTO SUB BASINS NORTH OF CAVASSON
 88          KM        AND SOUTH OF CAVASSON
 89          BA  0.0193
 90          LG   0.25    0.25    4.03    0.56      45
 91          UC   0.276   0.224
 92          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 93          UA     100
             *
1                                       HEC-1 INPUT                                          PAGE  3
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LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

  94          KK  SB02NW  DIVERT
  95          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NW
  96          KM        FIRST FLUSH VOL = 0.437 AC-FT
  97          DT  RET2NW   0.437
  98          DI     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
  99          DI    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
 100          DQ     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
 101          DQ    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
              *

 102          KK  CP-02W COMBINE
 103          KM        COMBINE ROUTE FROM 1A-2W WITH NEW SUB BASIN SB02NW
 104          HC       2   0.305
              *

 105          KK   R2W-2   ROUTE
 106          RK    1380  0.0140   0.018            TRAP   2.000    0.00
              *

 107          KK   SB01C   BASIN
 108          BA   0.027
 109          LG    0.25    0.25    4.03    0.56      45
 110          UC   0.345   0.366
 111          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 112          UA     100
              *

 113          KK   SB01C  DIVERT
 114          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01C
 115          KM        FIRST FLUSH VOL = 0.612 AC-FT
 116          DT  RET01C   0.612
 117          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 118          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 119          KK  R1C-1B   ROUTE
 120          RK     542   0.005   0.013            CIRC   5.000
              *

 121          KK   SB01B   BASIN
 122          BA   0.024
 123          LG    0.25    0.25    4.03    0.56      45
 124          UC   0.304   0.298
 125          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 126          UA     100
              *

 127          KK   SB01B  DIVERT
 128          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01B
 129          KM        FIRST FLUSH VOL = 0.543 AC-FT
 130          DT  RET01B   0.543
 131          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 132          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *
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LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 133          KK  CP-01B COMBINE
 134          KM        COMBINE ROUTE 1C-1B WITH SB01B
 135          HC       2   0.051
              *

 136          KK  R1B-2N   ROUTE
 137          RK    1268   0.015   0.025            TRAP   80.00    0.00
              *

 138          KK  SB02NE   BASIN
 139          BA  0.0335
 140          LG    0.25    0.25    4.03    0.56      45
 141          UC   0.406   0.448
 142          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 143          UA     100
              *
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 144          KK  SB02NE  DIVERT
 145          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NE
 146          KM        FIRST FLUSH VOL = 0.760 AC-FT
 147          DT  RET2NE   0.760
 148          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 149          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 150          KK  CP-02N COMBINE
 151          KM        COMBINE ROUTE 1B-2N (BASINS SB01-B AND SB01-C) WITH SB02NE
 152          HC       2   0.085
              *

 153          KK   R2N-2   ROUTE
 154          RK    1380   0.014   0.018            TRAP   2.000    0.00
              *

 155          KK   SB02E   BASIN
 156          BA   0.040
 157          LG    0.10    0.25    4.03    0.61      80
 158          UC   0.369   0.321
 159          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 160          UA     100
              *

 161          KK   SB02E  DIVERT
 162          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E
 163          KM        FIRST FLUSH VOL = 0.998 AC-FT
 164          DT   RET2E   0.998
 165          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 166          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *
1                                      HEC-1 INPUT                                          PAGE  5

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 167          KK   SB02W   BASIN
 168          BA  0.0571
 169          LG    0.35    0.35    4.33    0.51       0
 170          UC   0.686   0.535
 171          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 172          UA     100
              *

 173          KK   CP-02 COMBINE
 174          HC       4  0.1816
              *

 175          KK    R2-3   ROUTE
 176          KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
 177          RK    1260  0.0015   0.030            TRAP      92       4
              *

 178          KK    SB03   BASIN
 179          BA   0.048
 180          LG    0.15    0.25    4.50    0.47      55
 181          UC   0.364   0.294
 182          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 183          UA     100
              *

 184          KK   CP-03 COMBINE
 185          HC       2
              *

 186          KK    R3-5   ROUTE
 187          KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
 188          RK    2396  0.0015   0.03             TRAP      98       4
              *

 189          KK    SB05   BASIN
 190          BA   0.126
 191          LG    0.22    0.25    4.50    0.44      48
 192          UC   0.327   0.226
 193          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 194          UA     100
              *

 195          KK   CP-5A COMBINE
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```
 196          HC       2
              *

 197          KK   SB07W   BASIN
 198          BA  0.0468
 199          LG    0.25    0.25    4.03    0.56      45
 200          UC   0.323   0.237
 201          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 202          UA     100
              *
1                                      HEC-1 INPUT                                          PAGE  6

        LINE  ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 203          KK   SB07W  DIVERT
 204          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB07W
 205          KM        FIRST FLUSH VOL = 1.060 AC-FT
 206          DT  RET07W   1.060
 207          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 208          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 209          KK  R7W-8W   ROUTE
 210          KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 211          KM        HE MODIFIED ROUTE TO LOOP 101 W OF HAYDEN
 212          RK    3754  0.0013   0.03             TRAP      46       4
              *

 213          KK   SB8NW   BASIN
 214          KM        BASIN JUST SOUTH OF LEGACY - FUTURE MOB SITE
 215          BA  0.0147
 216          LG    0.10    0.25    4.03    0.61      80
 217          UC   0.252   0.200
 218          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 219          UA     100
              *

 220          KK   SB8NW  DIVERT
 221          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NW
 222          KM        FIRST FLUSH VOL = 0.332 AC-FT
 223          DT  RET8NW   0.332
 224          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 225          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 226          KK  R8NW8N   ROUTE
 227          KM        ROUTE NEW SUB BASIN SB8NW TO OUTLET AT CAVASSON BLVD
 228          KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 229          RK     643  0.0113   0.018            TRAP      46       4
              *

 230          KK   SB8NE   BASIN
 231          KM        SITE JUST SOUTH OF LEGACY AND WEST OF HAYDEN - FUTURE RETAIL
 232          BA   0.015
 233          LG    0.10    0.25    4.03    0.61      80
 234          UC   0.239   0.172
 235          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 236          UA     100
              *

 237          KK   SB8NE  DIVERT
 238          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NE
 239          KM        FIRST FLUSH VOL = 0.340 AC-FT
 240          DT  RET8NE   0.340
 241          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 242          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *
1                                      HEC-1 INPUT                                          PAGE  7

        LINE  ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 243          KK  R8NE8N   ROUTE
 244          KM        ROUTE NEW SUB BASIN SB8NE TO OUTLET AT CAVASSON BLVD
 245          KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 246          RK     622   0.011   0.018            TRAP      46       4
              *

 247          KK   SB8SW   BASIN
```

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          KM        SITE JUST NORTH OF CAVASSON BLVD EAST OF CLARET DR - FUTURE MF
          BA  0.0129
          LG    0.10    0.25    4.03    0.61      80
          UC   0.219   0.169
          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
          UA     100
          *

          KK   SB8SW  DIVERT
          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SW
          KM        FIRST FLUSH VOL = 0.292 AC-FT
          DT  RET8SW   0.292
          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
          *

          KK   SB8SE   BASIN
          KM        SITE JUST NORTH OF CAVASSON AND WEST OF HAYDEN - FUTURE HOTEL OR RETAIL
          BA  0.0131
          LG    0.10    0.25    4.03    0.61      80
          UC   0.242   0.210
          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
          UA     100
          *

          KK   SB8SE  DIVERT
          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SE
          KM        FIRST FLUSH VOL = 0.297 AC-FT
          DT  RET8SE   0.297
          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
          *

          KK  CP-08N COMBINE
          KM        CP AT INLET ON NORTH SIDE OF CAVASSON - ENTERS EXIST STORM DRAIN
          HC       4  0.0557
          *

          KK  R8N-8W   ROUTE
          KM        ROUTE NEW SUB BASINS SB8NE SB8NW SB8SW SB8SE TO CP-08W
          KM        ROUTE IN STORM DRAIN
          RK    1592   0.015   0.013            CIRC       3
          *
1                                      HEC-1 INPUT                                          PAGE  8

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

          KK   SB08W   BASIN
          BA  0.0589
          LG    0.10    0.25    4.03    0.61      80
          UC   0.225   0.150
          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
          UA     100
          *

          KK   SB08W  DIVERT
          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB08W
          KM        FIRST FLUSH VOL REQ = 1.335 AC-FT, PROVIDED = 1.415
          KM        THIS IS EXISTING AND WAS BUILT IN PHASE 1
          DT   RET8W   1.415
          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
          *

          KK  CP-08W COMBINE
          HC       3  0.1614
          *
          *

          KK   R8W-4   ROUTE
          RK    3137  0.0130   0.030            TRAP   2.000    0.00
          *

          KK    SB04   BASIN
          BA   0.144
          LG    0.14    0.25    4.60    0.44      61
          UC   0.305   0.199
          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          UA     100
```

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                *
 303            KK    CP-4 COMBINE
 304            HC       2
                *
 305            KK    R4-5   ROUTE
 306            KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 307            RK    1992  0.0013   0.03             TRAP      39       4
                *
 308            KK   CP-05 COMBINE
 309            KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 310            KM        COMBINE CP-05A AND R4-5
 311            HC       2
                *
1                                       HEC-1 INPUT                                          PAGE  9

           LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 312            KK    R5-6   ROUTE
 313            KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 314            RK    1694  0.0015   0.03             TRAP     188       4
                *
 315            KK    SB06   BASIN
 316            BA   0.136
 317            LG    0.16    0.25    4.55    0.45      53
 318            UC   0.321   0.246
 319            UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 320            UA     100
                *
 321            KK   CP-06 COMBINE
 322            HC       2
                *
 323            KK   SB07E   BASIN
 324            BA   0.026
 325            LG    0.10    0.25    4.03    0.74       0
 326            UC   0.574   0.527
 327            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 328            UA     100
                *
 329            KK  R7E-8E   ROUTE
 330            RK    3662  0.0130   0.030            TRAP   2.000    0.00
                *
 331            KK   SB08E   BASIN
 332            BA   0.078
 333            LG    0.10    0.25    4.08    0.72       0
 334            UC   0.798   0.652
 335            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 336            UA     100
                *
 337            KK   SB09N   BASIN
 338            BA   0.028
 339            LG    0.10    0.25    4.79    0.49       0
 340            UC   0.740   1.111
 341            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 342            UA     100
                *
 343            KK  R9N-R9   ROUTE
 344            RK    2586  0.0013   0.020            TRAP   2.000    0.00
                *
1                                       HEC-1 INPUT                                          PAGE 10

           LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 345            KK  CP-08E COMBINE
 346            HC       3
                *
 347            KK  R8E-R9   ROUTE
 348            RK     501  0.0013   0.020            TRAP   2.000    0.00
```

```
                *
 349            KK    SB09   BASIN
 350            BA   0.086
 351            LG    0.10    0.25    4.79    0.49       0
 352            UC   0.683   0.518
 353            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 354            UA     100
                *

 355            KK   CP-09 COMBINE
 356            HC       2
                *

 357            KK   PWRCH
 358            KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
 359            KM        100-YR, 24-HR FLO-2D MODEL (XS 107)
 360            KM        FROM PINNACLE PEAK SOUTH ADMS 100-YR, 24-HR FLO-2D MODEL
 361            BA     7.0
 362            QI       0       0       0       0       0       0       0       0       0       0
 363            QI       0       0       0       0       0       0       0       0       0       0
 364            QI       0       0       0       0       0       0       0       0       0       0
 365            QI       0       0       0       0       0       0       0       0       0       0
 366            QI       0       0       0       0       0       1       1       3      43     495
 367            QI    1099    1198    2055    3747    3658    3175    2822    2378    2084    1898
 368            QI    1699    1499    1331    1179    1047     940     851     773     705     645
 369            QI     597     559     521     488     460     435     410     387     370     356
 370            QI     339     328     317     303     293     281     270     257     258     248
 371            QI     240     237     223     220     214     209     205     201     192     188
 372            QI     180     179     164     158     149     142     137     126     120     117
 373            QI     107     102      97      99      86      83      82      74      70      66
 374            QI      62      59      57      55      52      50      50      48      42      41
 375            QI      40      37      35      40      36      32      31      30      31      30
 376            QI      31      30      24      23      23
                *

 377            KK  BINFLO
 378            KM         TOTAL INFLOW INTO BASIN 53R.
 379            HC       2
                *

 380            KK   BASIN
 381            KM     BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
 382            KM     ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
 383            KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
 384            KM        THE OUTLET PIPES ARE INLET CONTROLLED.
 385            RS       1    STOR       0
 386            SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
1                                      HEC-1 INPUT                                          PAGE 11

              LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 387            SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
 388            SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
 389            SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
 390            SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
 391            SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
                *

 392            KK  BSNRT1
 393            KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
 394            KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
 395            KM        EXCESS CAPACITY.
 396            RK     550  0.0052   0.024            CIRC     7.0
                *

 397            KK  BSNRT2
 398            KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BAS 53R OUTFALL)
 399            RK    1200  0.0077   0.013            CIRC     7.0
                *

 400            KK    SB10   BASIN
 401            BA   0.040
 402            LG    0.15    0.25    4.25    0.55      55
 403            UC   0.233   0.161
 404            UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 405            UA     100
                *

 406            KK   CP-10 COMBINE
```

```
           407           KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
           408           HC       2
                         *

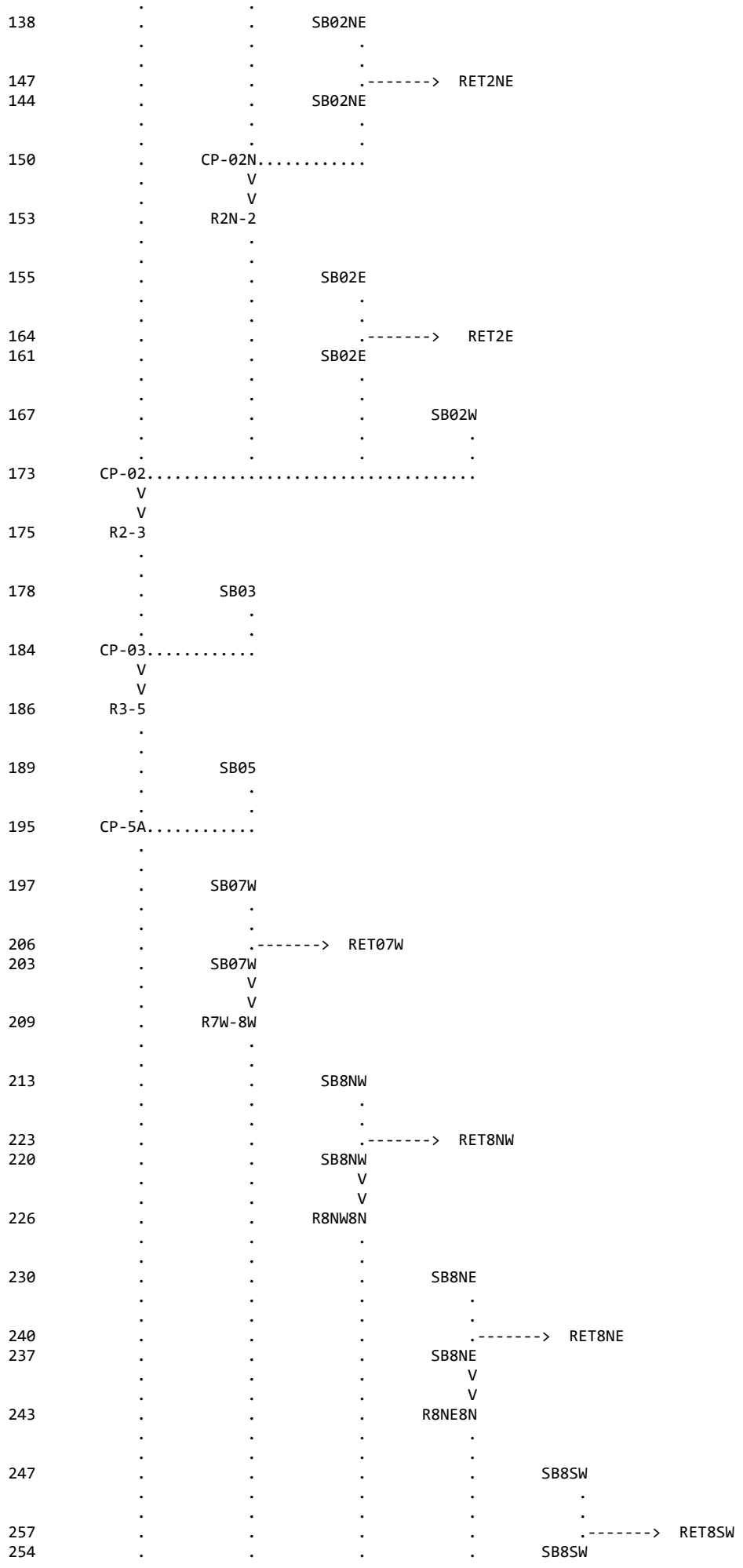
           409           KK  R10-11   ROUTE
           410           KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
           411           RK    1277  0.0014   0.03             TRAP     220       4
                         *

           412           KK    SB11   BASIN
           413           BA   0.071
           414           LG    0.15    0.25    4.15    0.58      55
           415           UC   0.296   0.232
           416           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
           417           UA     100
                         *

           418           KK   CP-11 COMBINE
           419           KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
           420           HC       2
                         *
           421           ZZ
1
                SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

   NO.      (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

    45      76THST
                 .
                 .
    64           .       SB01A
                 .           .
                 .           .
    77           .           .-------->  RET01A
    74           .       SB01A
                 .           .
                 .           .
    80         CP-1.............
                 V
                 V
    83      R1A-2W
                 .
                 .
    86           .      SB02NW
                 .           .
                 .           .
    97           .           .-------->  RET2NW
    94           .      SB02NW
                 .           .
                 .           .
   102      CP-02W.............
                 V
                 V
   105       R2W-2
                 .
                 .
   107           .       SB01C
                 .           .
                 .           .
   116           .           .-------->  RET01C
   113           .       SB01C
                 .           V
                 .           V
   119           .      R1C-1B
                 .           .
                 .           .
   121           .           .       SB01B
                 .           .           .
                 .           .           .
   130           .           .           .-------->  RET01B
   127           .           .       SB01B
                 .           .           .
                 .           .           .
   133           .      CP-01B.............
                 .           V
                 .           V
   136           .      R1B-2N
                 .           .
```


SB02NE
RET2NE
SB02NE
CP-02N
R2N-2
SB02E
RET2E
SB02E
SB02W
CP-02
R2-3
SB03
CP-03
R3-5
SB05
CP-5A
SB07W
RET07W
SB07W
R7W-8W
SB8NW
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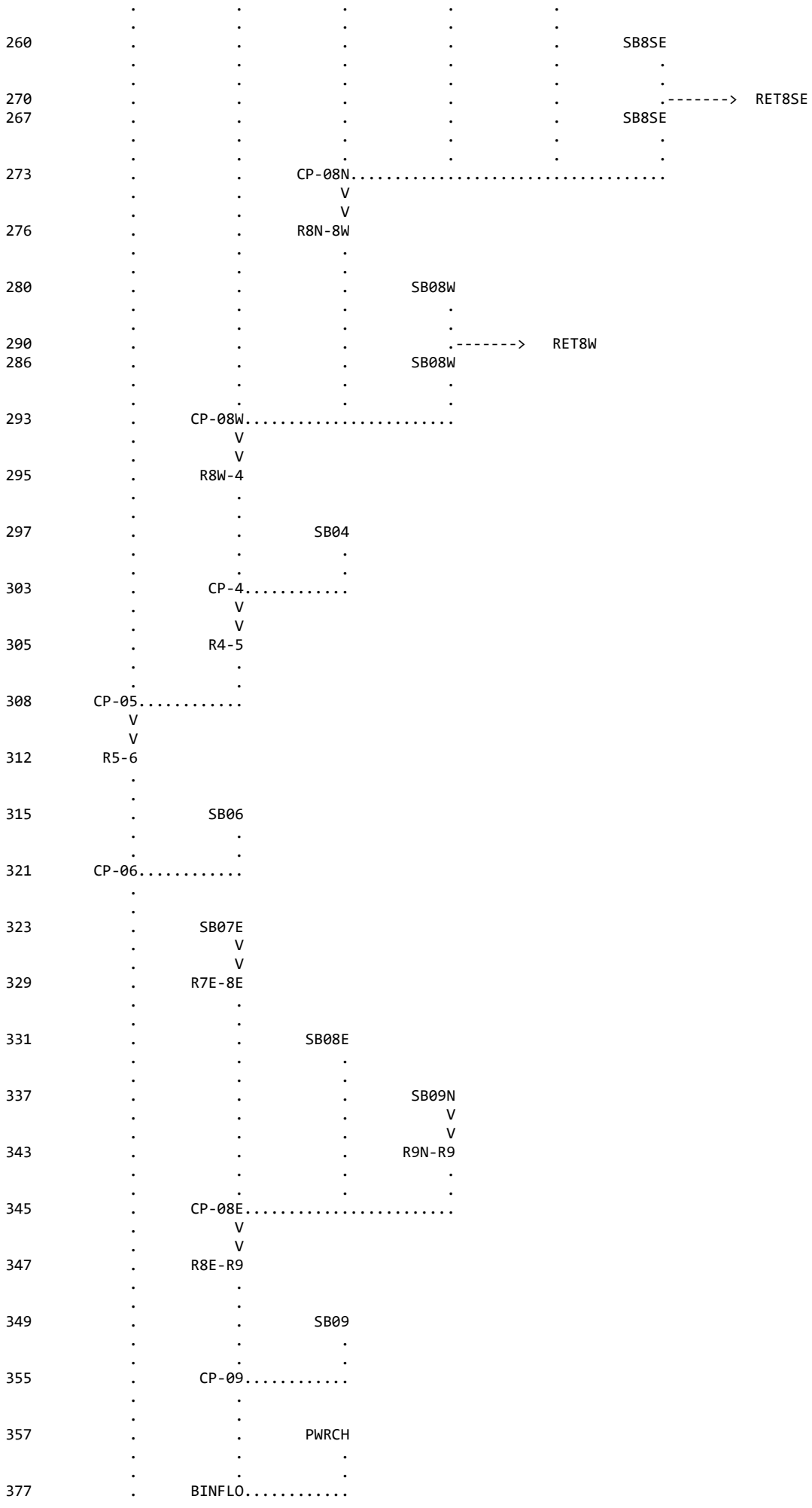
SB8NW
R8NW8N
SB8NE
RET8NE
SB8NE
R8NE8N
SB8SW
RET8SW
SB8SW


260
SB8SE
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267
SB8SE
RET8SE
273
CP-08N
276
R8N-8W
280
SB08W
290
RET8W
286
SB08W
293
CP-08W
295
R8W-4
297
SB04
303
CP-4
305
R4-5
308
CP-05
312
R5-6
315
SB06
321
CP-06
323
SB07E
329
R7E-8E
331
SB08E
337
SB09N
343
R9N-R9
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CP-08E
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R8E-R9
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SB09
355
CP-09
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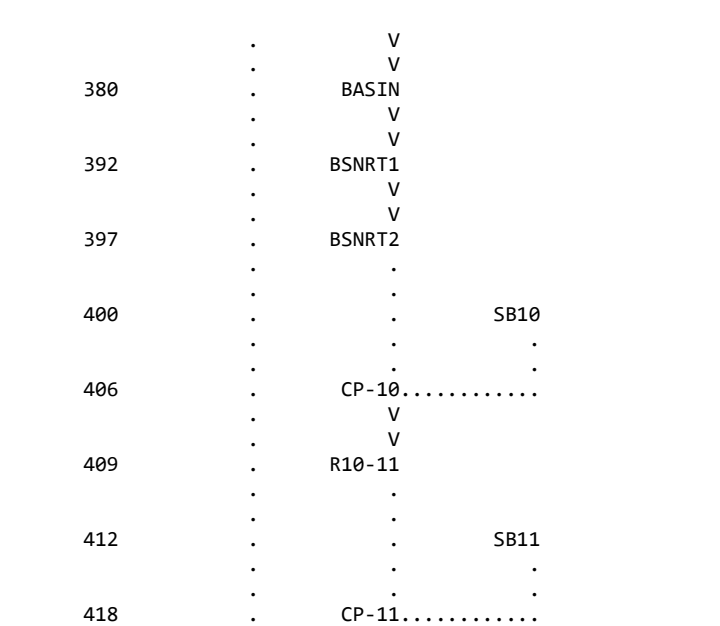
PWRCH
377
BINFLO



(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

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FLOOD HYDROGRAPH PACKAGE (HEC-1)

JUN 1998

VERSION 4.1

U.S. ARMY CORPS OF ENGINEERS

HYDROLOGIC ENGINEERING CENTER

609 SECOND STREET

DAVIS, CALIFORNIA 95616

(916) 756-1104

RUN DATE 26APR21 TIME 09:31:12

CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
POWERLINE CORRIDOR
RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
PIMA ROAD FLOWS.

PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
MODELERS: RK, MW

REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 08/07/18
MODELERS: MSW, ES
REVISIONS NOTED WITH HE
NEW PROP CONDITION MODEL,WITH POWERLINE AND BASIN 53R
THIS IS THE ULTIMATE CONDITION MODEL

REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
MODELERS: MSW
XKSAT ADJUSTMENT

REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
MODELERS: MSW, TSW
REVISIONS NOTED WITH HE (Hubbard Engineering)
NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED

31 IO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL

```
                IPLOT           0  PLOT CONTROL
                QSCAL          0.  HYDROGRAPH PLOT SCALE

  IT          HYDROGRAPH TIME DATA
                NMIN            3  MINUTES IN COMPUTATION INTERVAL
                IDATE      1    0  STARTING DATE
                ITIME         0000  STARTING TIME
                   NQ         1000  NUMBER OF HYDROGRAPH ORDINATES
                NDDATE      3    0  ENDING DATE
                NDTIME        0157  ENDING TIME
                ICENT           19  CENTURY MARK

             COMPUTATION INTERVAL    0.05 HOURS
                  TOTAL TIME BASE   49.95 HOURS

       ENGLISH UNITS
            DRAINAGE AREA         SQUARE MILES
            PRECIPITATION DEPTH   INCHES
            LENGTH, ELEVATION     FEET
            FLOW                  CUBIC FEET PER SECOND
            STORAGE VOLUME        ACRE-FEET
            SURFACE AREA          ACRES
            TEMPERATURE           DEGREES FAHRENHEIT

32 JD       INDEX STORM NO. 1
                STRM        3.85  PRECIPITATION DEPTH
                TRDA        0.00  TRANSPOSITION DRAINAGE AREA

33 PI         PRECIPITATION PATTERN
              0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00
```

TRDA 10.00 TRANSPOSITION DRAINAGE AREA

0 PI PRECIPITATION PATTERN

0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

0.02	0.02	0.02	0.02	0.02	0.06	0.06	0.06	0.06	0.06
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
HYDROGRAPH AT	76THST	69.	12.50	23.	8.	4.	0.24		
HYDROGRAPH AT	SB01A	86.	12.25	11.	4.	2.	0.06		
DIVERSION TO	RET01A	6.	11.55	2.	1.	0.	0.06		
HYDROGRAPH AT	SB01A	86.	12.25	10.	3.	1.	0.06		
2 COMBINED AT	CP-1	135.	12.30	33.	11.	5.	0.30		
ROUTED TO	R1A-2W	134.	12.30	33.	11.	5.	0.30		
HYDROGRAPH AT	SB02NW	29.	12.15	3.	1.	1.	0.02		
DIVERSION TO	RET2NW	2.	11.45	1.	0.	0.	0.02		
HYDROGRAPH AT	SB02NW	29.	12.15	3.	1.	0.	0.02		
2 COMBINED AT	CP-02W	156.	12.25	36.	12.	6.	0.31		
ROUTED TO	R2W-2	155.	12.25	36.	12.	6.	0.31		
HYDROGRAPH AT	SB01C	32.	12.20	5.	2.	1.	0.03		
DIVERSION TO	RET01C	3.	11.60	1.	0.	0.	0.03		

	HYDROGRAPH AT						
+	SB01C	32.	12.20	5.	1.	1.	0.03
	ROUTED TO						
+	R1C-1B	31.	12.25	5.	1.	1.	0.03
	HYDROGRAPH AT						
+	SB01B	32.	12.20	4.	1.	1.	0.02
	DIVERSION TO						
+	RET01B	2.	11.50	1.	0.	0.	0.02
	HYDROGRAPH AT						
+	SB01B	32.	12.20	4.	1.	1.	0.02
	2 COMBINED AT						
+	CP-01B	63.	12.20	9.	2.	1.	0.05
	ROUTED TO						
+	R1B-2N	62.	12.25	9.	2.	1.	0.05
	HYDROGRAPH AT						
+	SB02NE	35.	12.25	6.	2.	1.	0.03
	DIVERSION TO						
+	RET2NE	3.	11.70	1.	0.	0.	0.03
	HYDROGRAPH AT						
+	SB02NE	35.	12.25	6.	2.	1.	0.03
	2 COMBINED AT						
+	CP-02N	96.	12.25	14.	4.	2.	0.09
	ROUTED TO						
+	R2N-2	95.	12.25	14.	4.	2.	0.09
	HYDROGRAPH AT						
+	SB02E	57.	12.25	10.	3.	2.	0.04
	DIVERSION TO						
+	RET2E	2.	9.65	1.	1.	0.	0.04
	HYDROGRAPH AT						
+	SB02E	57.	12.25	10.	3.	1.	0.04
	HYDROGRAPH AT						
+	SB02W	34.	12.45	5.	1.	1.	0.06
	4 COMBINED AT						
+	CP-02	324.	12.30	64.	20.	10.	0.18
	ROUTED TO						
+	R2-3	324.	12.35	64.	20.	10.	0.18
	HYDROGRAPH AT						
+	SB03	65.	12.15	10.	3.	2.	0.05
	2 COMBINED AT						
+	CP-03	367.	12.35	73.	23.	11.	0.23
	ROUTED TO						
+	R3-5	365.	12.45	72.	23.	11.	0.23
	HYDROGRAPH AT						
+	SB05	190.	12.10	24.	8.	4.	0.13
	2 COMBINED AT						
+	CP-5A	437.	12.45	94.	30.	15.	0.36
	HYDROGRAPH AT						
+	SB07W	68.	12.20	8.	3.	1.	0.05
	DIVERSION TO						
+	RET07W	4.	11.50	2.	1.	0.	0.05
	HYDROGRAPH AT						
+	SB07W	68.	12.20	8.	2.	1.	0.05
	ROUTED TO						
+	R7W-8W	66.	12.60	8.	2.	1.	0.05

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          HYDROGRAPH AT
+                          SB8NW        27.   12.15           4.          1.          1.       0.01

          DIVERSION TO
+                         RET8NW         1.    9.00           1.          0.          0.       0.01

          HYDROGRAPH AT
+                          SB8NW        27.   12.15           4.          1.          1.       0.01

          ROUTED TO
+                         R8NW8N        26.   12.15           4.          1.          1.       0.01

          HYDROGRAPH AT
+                          SB8NE        29.   12.10           4.          1.          1.       0.01

          DIVERSION TO
+                         RET8NE         1.    8.95           1.          0.          0.       0.01

          HYDROGRAPH AT
+                          SB8NE        29.   12.10           4.          1.          1.       0.01

          ROUTED TO
+                         R8NE8N        28.   12.15           4.          1.          1.       0.01

          HYDROGRAPH AT
+                          SB8SW        25.   12.10           3.          1.          1.       0.01

          DIVERSION TO
+                         RET8SW         1.    8.95           0.          0.          0.       0.01

          HYDROGRAPH AT
+                          SB8SW        25.   12.10           3.          1.          0.       0.01

          HYDROGRAPH AT
+                          SB8SE        23.   12.15           3.          1.          1.       0.01

          DIVERSION TO
+                         RET8SE         1.    9.00           0.          0.          0.       0.01

          HYDROGRAPH AT
+                          SB8SE        23.   12.15           3.          1.          0.       0.01

          4 COMBINED AT
+                         CP-08N       102.   12.15          14.          4.          2.       0.06

          ROUTED TO
+                         R8N-8W       101.   12.15          14.          4.          2.       0.06

          HYDROGRAPH AT
+                          SB08W       118.   12.10          14.          5.          2.       0.06

          DIVERSION TO
+                          RET8W         3.    9.20           2.          1.          0.       0.06

          HYDROGRAPH AT
+                          SB08W       118.   12.10          14.          4.          2.       0.06

          3 COMBINED AT
+                         CP-08W       214.   12.10          36.         11.          5.       0.16

          ROUTED TO
+                          R8W-4       213.   12.15          36.         11.          5.       0.16

          HYDROGRAPH AT
+                           SB04       241.   12.10          31.         10.          5.       0.14

          2 COMBINED AT
+                            CP-4      446.   12.10          66.         21.         10.       0.31

          ROUTED TO
+                            R4-5      442.   12.20          66.         21.         10.       0.31

          2 COMBINED AT
+                           CP-05      723.   12.35         158.         50.         25.       0.66

          ROUTED TO
+                            R5-6      717.   12.40         158.         50.         25.       0.66

          HYDROGRAPH AT
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+                         SB06       202.   12.10          27.          9.          4.       0.14

          2 COMBINED AT
+                         CP-06      817.   12.35         184.         59.         29.       0.80

          HYDROGRAPH AT
+                         SB07E       16.   12.40           2.          1.          0.       0.03

          ROUTED TO
+                        R7E-8E       16.   12.45           2.          1.          0.       0.03

          HYDROGRAPH AT
+                         SB08E       39.   12.55           6.          2.          1.       0.08

          HYDROGRAPH AT
+                         SB09N       11.   12.55           3.          1.          0.       0.03

          ROUTED TO
+                        R9N-R9       11.   12.65           3.          1.          0.       0.03

          3 COMBINED AT
+                        CP-08E       63.   12.55          11.          3.          1.       0.13

          ROUTED TO
+                        R8E-R9       63.   12.55          11.          3.          1.       0.13

          HYDROGRAPH AT
+                          SB09        60.   12.45           9.          2.          1.       0.09

          2 COMBINED AT
+                         CP-09      120.   12.50          20.          5.          2.       0.22

          HYDROGRAPH AT
+                         PWRCH     3747.   13.25        1540.        502.        248.       7.00

          2 COMBINED AT
+                        BINFLO     3788.   13.25        1558.        507.        250.       7.22

          ROUTED TO
+                         BASIN      462.   18.45         458.        399.        241.       7.22

          ROUTED TO
+                        BSNRT1      462.   18.50         458.        399.        241.       7.22

          ROUTED TO
+                        BSNRT2      462.   18.50         458.        399.        241.       7.22

          HYDROGRAPH AT
+                          SB10        72.   12.05           8.          3.          1.       0.04

          2 COMBINED AT
+                         CP-10      463.   18.50         459.        400.        242.       7.26

          ROUTED TO
+                        R10-11      463.   18.55         459.        400.        241.       7.26

          HYDROGRAPH AT
+                          SB11       107.   12.10          14.          5.          2.       0.07

          2 COMBINED AT
+                         CP-11      465.   18.55         460.        401.        244.       7.33
1
                                    SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
                                        (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)
                                                                                 INTERPOLATED TO
                                                                                 COMPUTATION INTERVAL
    ISTAQ    ELEMENT      DT       PEAK    TIME TO     VOLUME      DT       PEAK    TIME TO     VOLUME
                                             PEAK                                    PEAK

                          (MIN)     (CFS)      (MIN)     (IN)     (MIN)     (CFS)     (MIN)       (IN)

   FOR STORM = 1  STORM AREA (SQ MI) =       0.00
     R1A-2W  MANE         0.27    138.22    738.13      1.40      3.00    138.18    738.00        1.40

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2249E+02 EXCESS=0.0000E+00 OUTFLOW=0.2249E+02 BASIN STORAGE=0.2963E-02 PERCENT ERROR=   0.0


   FOR STORM = 2  STORM AREA (SQ MI) =      10.00
     R1A-2W  MANE         0.23    132.78    738.11      1.37      3.00    132.73    738.00        1.37
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2206E+02 EXCESS=0.0000E+00 OUTFLOW=0.2206E+02 BASIN STORAGE=0.2963E-02 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 20.00

R1A-2W	MANE	0.23	129.25	738.12	1.36	3.00	129.19	738.00	1.36

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2178E+02 EXCESS=0.0000E+00 OUTFLOW=0.2178E+02 BASIN STORAGE=0.2963E-02 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R2W-2	MANE	0.20	160.62	735.42	1.50	3.00	159.58	735.00	1.50

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2435E+02 EXCESS=0.0000E+00 OUTFLOW=0.2435E+02 BASIN STORAGE=0.4089E-02 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00

R2W-2	MANE	0.28	153.29	735.70	1.46	3.00	152.40	738.00	1.46

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2379E+02 EXCESS=0.0000E+00 OUTFLOW=0.2378E+02 BASIN STORAGE=0.4089E-02 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 20.00

R2W-2	MANE	0.22	148.68	735.55	1.44	3.00	147.99	738.00	1.44

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2342E+02 EXCESS=0.0000E+00 OUTFLOW=0.2342E+02 BASIN STORAGE=0.4090E-02 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R1C-1B	MANE	0.43	32.64	732.90	1.80	3.00	32.39	735.00	1.81

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2599E+01 EXCESS=0.0000E+00 OUTFLOW=0.2599E+01 BASIN STORAGE=0.9756E-13 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00

R1C-1B	MANE	0.39	30.53	732.80	1.67	3.00	30.31	735.00	1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2408E+01 EXCESS=0.0000E+00 OUTFLOW=0.2408E+01 BASIN STORAGE=0.9547E-13 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 20.00

R1C-1B	MANE	0.44	29.15	732.86	1.59	3.00	28.96	735.00	1.59

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2284E+01 EXCESS=0.0000E+00 OUTFLOW=0.2285E+01 BASIN STORAGE=0.9767E-13 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R1B-2N	MANE	1.55	64.38	735.51	1.81	3.00	64.19	735.00	1.81

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4913E+01 EXCESS=0.0000E+00 OUTFLOW=0.4913E+01 BASIN STORAGE=0.5632E-04 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00

R1B-2N	MANE	1.51	60.32	734.68	1.67	3.00	60.22	735.00	1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4553E+01 EXCESS=0.0000E+00 OUTFLOW=0.4552E+01 BASIN STORAGE=0.5975E-04 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 20.00

R1B-2N	MANE	1.46	57.82	735.22	1.59	3.00	57.66	735.00	1.59

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4319E+01 EXCESS=0.0000E+00 OUTFLOW=0.4320E+01 BASIN STORAGE=0.5659E-04 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R2N-2	MANE	0.26	99.78	735.68	1.80	3.00	99.10	735.00	1.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8138E+01 EXCESS=0.0000E+00 OUTFLOW=0.8139E+01 BASIN STORAGE=0.1854E-04 PERCENT ERROR= 0.0

```
          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
             R2N-2  MANE         0.41     93.34    735.52      1.66      3.00     92.74    735.00        1.66


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7542E+01 EXCESS=0.0000E+00 OUTFLOW=0.7543E+01 BASIN STORAGE=0.1946E-04 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             R2N-2  MANE         0.30     89.45    735.69      1.58      3.00     88.67    735.00        1.58


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7158E+01 EXCESS=0.0000E+00 OUTFLOW=0.7159E+01 BASIN STORAGE=0.1834E-04 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R2-3  MANE         2.04    336.24    739.95      4.22      3.00    335.93    741.00        4.22


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4096E+02 EXCESS=0.0000E+00 OUTFLOW=0.4090E+02 BASIN STORAGE=0.3882E-01 PERCENT ERROR=   0.1


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R2-3  MANE         2.10    317.06    739.89      4.04      3.00    316.94    741.00        4.03


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3916E+02 EXCESS=0.0000E+00 OUTFLOW=0.3908E+02 BASIN STORAGE=0.3878E-01 PERCENT ERROR=   0.1


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R2-3  MANE         2.15    304.76    739.74      3.92      3.00    304.74    741.00        3.92


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3802E+02 EXCESS=0.0000E+00 OUTFLOW=0.3794E+02 BASIN STORAGE=0.3635E-01 PERCENT ERROR=   0.1


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R3-5  MANE         2.78    380.48    747.40      3.86      3.00    378.99    747.00        3.86


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4743E+02 EXCESS=0.0000E+00 OUTFLOW=0.4725E+02 BASIN STORAGE=0.8213E-01 PERCENT ERROR=   0.2


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R3-5  MANE         2.76    359.46    749.07      3.68      3.00    358.16    747.00        3.68


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4525E+02 EXCESS=0.0000E+00 OUTFLOW=0.4507E+02 BASIN STORAGE=0.8573E-01 PERCENT ERROR=   0.2


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R3-5  MANE         2.78    345.68    749.01      3.57      3.00    343.26    747.00        3.57


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4389E+02 EXCESS=0.0000E+00 OUTFLOW=0.4376E+02 BASIN STORAGE=0.8213E-01 PERCENT ERROR=   0.1


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7W-8W  MANE         3.00     68.93    754.57      1.91      3.00     68.50    756.00        1.91


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4511E+01 EXCESS=0.0000E+00 OUTFLOW=0.4779E+01 BASIN STORAGE=0.9374E-03 PERCENT ERROR=  -6.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R7W-8W  MANE         3.00     64.32    754.67      1.83      3.00     64.21    756.00        1.83


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4179E+01 EXCESS=0.0000E+00 OUTFLOW=0.4572E+01 BASIN STORAGE=0.8368E-03 PERCENT ERROR=  -9.4


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R7W-8W  MANE         3.00     63.27    756.75      1.63      3.00     59.66    759.00        1.63


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3965E+01 EXCESS=0.0000E+00 OUTFLOW=0.4073E+01 BASIN STORAGE=0.8362E-03 PERCENT ERROR=  -2.8


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
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           R8NW8N  MANE         0.83     27.24    730.38      2.83      3.00     27.08    729.00        2.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2215E+01 EXCESS=0.0000E+00 OUTFLOW=0.2217E+01 BASIN STORAGE=0.3350E-06 PERCENT ERROR=  -0.1


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
           R8NW8N  MANE         0.90     25.74    730.14      2.66      3.00     25.61    729.00        2.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2082E+01 EXCESS=0.0000E+00 OUTFLOW=0.2084E+01 BASIN STORAGE=0.2981E-06 PERCENT ERROR=  -0.1


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
           R8NW8N  MANE         0.87     24.80    730.23      2.55      3.00     24.66    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1996E+01 EXCESS=0.0000E+00 OUTFLOW=0.1998E+01 BASIN STORAGE=0.2840E-06 PERCENT ERROR=  -0.1


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
           R8NE8N  MANE         0.80     29.24    728.00      2.83      3.00     29.14    729.00        2.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2259E+01 EXCESS=0.0000E+00 OUTFLOW=0.2263E+01 BASIN STORAGE=0.3312E-06 PERCENT ERROR=  -0.1


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
           R8NE8N  MANE         0.87     27.62    728.44      2.66      3.00     27.56    729.00        2.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2124E+01 EXCESS=0.0000E+00 OUTFLOW=0.2127E+01 BASIN STORAGE=0.3201E-06 PERCENT ERROR=  -0.2


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
           R8NE8N  MANE         0.87     26.65    727.97      2.55      3.00     26.55    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2036E+01 EXCESS=0.0000E+00 OUTFLOW=0.2038E+01 BASIN STORAGE=0.3381E-06 PERCENT ERROR=  -0.1


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
           R8N-8W  MANE         0.63    104.30    729.26      2.83      3.00    104.19    729.00        2.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8393E+01 EXCESS=0.0000E+00 OUTFLOW=0.8394E+01 BASIN STORAGE=0.4795E-07 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
           R8N-8W  MANE         0.63     98.64    729.78      2.66      3.00     98.54    729.00        2.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7889E+01 EXCESS=0.0000E+00 OUTFLOW=0.7890E+01 BASIN STORAGE=0.4709E-07 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
           R8N-8W  MANE         0.51     95.24    729.83      2.55      3.00     94.91    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7563E+01 EXCESS=0.0000E+00 OUTFLOW=0.7564E+01 BASIN STORAGE=0.4620E-07 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R8W-4  MANE         0.71    221.46    727.41      2.55      3.00    220.48    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2196E+02 EXCESS=0.0000E+00 OUTFLOW=0.2197E+02 BASIN STORAGE=0.3358E-03 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
             R8W-4  MANE         0.64    209.50    727.30      2.41      3.00    208.59    729.00        2.41

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2072E+02 EXCESS=0.0000E+00 OUTFLOW=0.2072E+02 BASIN STORAGE=0.3435E-03 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             R8W-4  MANE         0.73    201.38    727.98      2.27      3.00    200.85    729.00        2.27
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1955E+02 EXCESS=0.0000E+00 OUTFLOW=0.1955E+02 BASIN STORAGE=0.3555E-03 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R4-5	MANE	2.47	458.39	732.52	2.64	3.00	457.71	732.00	2.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4297E+02 EXCESS=0.0000E+00 OUTFLOW=0.4302E+02 BASIN STORAGE=0.1194E-02 PERCENT ERROR= -0.1

FOR STORM = 2 STORM AREA (SQ MI) = 10.00

R4-5	MANE	2.57	435.01	731.91	2.49	3.00	434.59	732.00	2.50

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4059E+02 EXCESS=0.0000E+00 OUTFLOW=0.4063E+02 BASIN STORAGE=0.1021E-02 PERCENT ERROR= -0.1

FOR STORM = 3 STORM AREA (SQ MI) = 20.00

R4-5	MANE	2.59	416.96	732.36	2.38	3.00	415.70	732.00	2.38

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3868E+02 EXCESS=0.0000E+00 OUTFLOW=0.3873E+02 BASIN STORAGE=0.1074E-02 PERCENT ERROR= -0.1

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R5-6	MANE	2.34	763.02	745.86	3.01	3.00	762.26	744.00	3.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1062E+03 EXCESS=0.0000E+00 OUTFLOW=0.1061E+03 BASIN STORAGE=0.6943E-01 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00

R5-6	MANE	2.51	707.12	745.07	2.85	3.00	703.42	744.00	2.85

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1007E+03 EXCESS=0.0000E+00 OUTFLOW=0.1006E+03 BASIN STORAGE=0.6945E-01 PERCENT ERROR= 0.1

FOR STORM = 3 STORM AREA (SQ MI) = 20.00

R5-6	MANE	2.52	674.69	746.57	2.74	3.00	673.28	747.00	2.74

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9685E+02 EXCESS=0.0000E+00 OUTFLOW=0.9676E+02 BASIN STORAGE=0.6943E-01 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R7E-8E	MANE	1.93	16.43	748.32	0.80	3.00	16.39	747.00	0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1113E+01 EXCESS=0.0000E+00 OUTFLOW=0.1112E+01 BASIN STORAGE=0.1951E-04 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) = 10.00

R7E-8E	MANE	1.94	14.90	748.28	0.72	3.00	14.82	747.00	0.72

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9964E+00 EXCESS=0.0000E+00 OUTFLOW=0.9968E+00 BASIN STORAGE=0.1628E-04 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 20.00

R7E-8E	MANE	2.13	13.91	748.07	0.67	3.00	13.81	747.00	0.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9248E+00 EXCESS=0.0000E+00 OUTFLOW=0.9254E+00 BASIN STORAGE=0.1626E-04 PERCENT ERROR= -0.1

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R9N-R9	MANE	2.43	11.87	758.35	0.97	3.00	11.83	759.00	0.97

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1453E+01 EXCESS=0.0000E+00 OUTFLOW=0.1451E+01 BASIN STORAGE=0.2431E-04 PERCENT ERROR= 0.1

FOR STORM = 2 STORM AREA (SQ MI) = 10.00

R9N-R9	MANE	2.57	10.88	757.62	0.89	3.00	10.84	759.00	0.89

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1330E+01 EXCESS=0.0000E+00 OUTFLOW=0.1328E+01 BASIN STORAGE=0.2146E-04 PERCENT ERROR= 0.1

```
          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R9N-R9  MANE         2.61     10.25    758.60      0.84      3.00     10.23    759.00        0.84

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1249E+01 EXCESS=0.0000E+00 OUTFLOW=0.1248E+01 BASIN STORAGE=0.2338E-04 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8E-R9  MANE         0.31     67.27    753.42      0.85      3.00     67.12    753.00        0.85

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5955E+01 EXCESS=0.0000E+00 OUTFLOW=0.5955E+01 BASIN STORAGE=0.1051E-04 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R8E-R9  MANE         0.38     60.94    753.41      0.76      3.00     60.79    753.00        0.76

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5368E+01 EXCESS=0.0000E+00 OUTFLOW=0.5368E+01 BASIN STORAGE=0.1067E-04 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R8E-R9  MANE         0.36     56.87    753.51      0.71      3.00     56.68    753.00        0.71

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4995E+01 EXCESS=0.0000E+00 OUTFLOW=0.4995E+01 BASIN STORAGE=0.1021E-04 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT1  MANE         0.38    462.76   1109.87      2.58      3.00    462.76   1110.00        2.58

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9944E+03 EXCESS=0.0000E+00 OUTFLOW=0.9943E+03 BASIN STORAGE=0.9346E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            BSNRT1  MANE         0.38    462.49   1110.02      2.58      3.00    462.49   1110.00        2.58

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9936E+03 EXCESS=0.0000E+00 OUTFLOW=0.9935E+03 BASIN STORAGE=0.9328E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            BSNRT1  MANE         0.38    462.32   1110.01      2.58      3.00    462.32   1110.00        2.58

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9930E+03 EXCESS=0.0000E+00 OUTFLOW=0.9929E+03 BASIN STORAGE=0.9316E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE         0.38    462.76   1110.48      2.58      3.00    462.76   1110.00        2.58

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9943E+03 EXCESS=0.0000E+00 OUTFLOW=0.9941E+03 BASIN STORAGE=0.1069E+00 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            BSNRT2  MANE         0.38    462.49   1110.30      2.58      3.00    462.49   1110.00        2.58

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9935E+03 EXCESS=0.0000E+00 OUTFLOW=0.9933E+03 BASIN STORAGE=0.1067E+00 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            BSNRT2  MANE         0.38    462.32   1110.49      2.58      3.00    462.32   1110.00        2.58

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9929E+03 EXCESS=0.0000E+00 OUTFLOW=0.9927E+03 BASIN STORAGE=0.1065E+00 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R10-11  MANE         2.34    463.63   1114.77      2.58      3.00    463.63   1113.00        2.58

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9994E+03 EXCESS=0.0000E+00 OUTFLOW=0.9982E+03 BASIN STORAGE=0.1212E+01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R10-11  MANE         2.39    463.32   1113.19      2.58      3.00    463.32   1113.00        2.58
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9983E+03 EXCESS=0.0000E+00 OUTFLOW=0.9972E+03 BASIN STORAGE=0.1068E+01 PERCENT ERROR=   0.0

FOR STORM = 3 STORM AREA (SQ MI) =   20.00

R10-11	MANE	2.35	463.12	1115.43	2.57	3.00	463.12	1113.00	2.57	

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9977E+03 EXCESS=0.0000E+00 OUTFLOW=0.9964E+03 BASIN STORAGE=0.1562E+01 PERCENT ERROR=   0.0

*** NORMAL END OF HEC-1 ***

Appendix E
Hydrology Calculations
Vi at Cavasson

HYDROLOGIC CALCULATION SUMMARY SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Project Name: The Vi at Cavasson
Project No.: 18114-701

Prepared by: MJA
Revised by:

Date: 09/01/21
Date:

Complete calculations for each concentration point are presented in the attached hydrologic calculation sheets.

Sub-Basin & Concentration Point ID	C Runoff Coefficient Frequency			I Intensity Frequency			A Area	Q Peak Discharge Frequency		
	10-year	50-year	100-year	10-year	50-year	100-year	[acres]	10-year	50-year	100-year
				[in/hr]				[cfs]		
DA-N1	0.90	0.90	0.95	3.52	5.13	5.78	0.45	1.43	2.08	2.47
DA-N2	0.90	0.90	0.95	3.66	5.13	5.78	0.39	1.27	1.79	2.12
DA-N3	0.90	0.90	0.95	3.66	5.13	5.78	0.24	0.78	1.09	1.30
DA-N4	0.90	0.90	0.95	3.66	5.13	5.78	0.20	0.66	0.92	1.10
DA-N5	0.90	0.90	0.95	3.66	5.13	5.78	0.19	0.64	0.90	1.07
DA-N6	0.90	0.90	0.95	3.66	5.13	5.78	0.14	0.45	0.63	0.74
DA-N7	0.90	0.90	0.95	3.66	5.13	5.78	0.17	0.56	0.78	0.93
DA-N8	0.90	0.90	0.95	3.66	5.13	5.78	0.18	0.59	0.83	0.98
DA-N9	0.90	0.90	0.95	3.66	5.13	5.78	0.18	0.58	0.81	0.97
DA-N10	0.90	0.90	0.95	3.66	5.13	5.78	0.70	2.30	3.22	3.83
DA-N11	0.90	0.90	0.95	3.66	5.13	5.78	0.19	0.64	0.90	1.07
DA-N12	0.90	0.90	0.95	3.66	5.13	5.78	0.39	1.27	1.78	2.12
DA-S1	0.90	0.90	0.95	3.66	5.13	5.78	0.46	1.51	2.12	2.52
DA-S2	0.90	0.90	0.95	3.66	5.13	5.78	0.17	0.56	0.78	0.93
DA-S3	0.90	0.90	0.95	3.66	5.13	5.78	0.37	1.23	1.73	2.06
DA-S4	0.90	0.90	0.95	3.66	5.13	5.78	0.22	0.74	1.04	1.23
DA-S5	0.90	0.90	0.95	3.66	5.13	5.78	0.23	0.77	1.07	1.28
DA-S6	0.90	0.90	0.95	3.66	5.13	5.78	0.33	1.10	1.54	1.84
DA-S7	0.85	0.90	0.95	3.66	5.13	5.78	0.23	0.73	1.08	1.29
DA-S8	0.85	0.90	0.95	3.66	5.13	5.78	0.27	0.85	1.26	1.50
DA-S9	0.86	0.90	0.95	3.66	5.13	5.78	0.41	1.27	1.87	2.23
DA-S10	0.85	0.90	0.95	3.66	5.13	5.78	0.18	0.55	0.82	0.97
DA-S11	0.84	0.90	0.95	3.66	5.13	5.78	0.31	0.97	1.45	1.73
DA-S12	0.83	0.90	0.95	3.66	5.13	5.78	0.23	0.71	1.08	1.28
DA-S13	0.83	0.90	0.95	3.66	5.13	5.78	0.24	0.72	1.10	1.31
DA-R1	0.82	0.90	0.95	3.66	5.13	5.78	0.12	0.37	0.57	0.67
DA-R2	0.81	0.90	0.95	3.66	5.13	5.78	0.11	0.33	0.52	0.62
DA-R3	0.81	0.90	0.95	3.66	5.13	5.78	0.11	0.33	0.51	0.61
DA-R4	0.80	0.90	0.95	3.66	5.13	5.78	0.16	0.46	0.73	0.87
DA-R5	0.79	0.90	0.95	3.66	5.13	5.78	0.08	0.24	0.38	0.45
DA-R6	0.78	0.90	0.95	3.66	5.13	5.78	0.08	0.24	0.38	0.45
DA-R7	0.78	0.90	0.95	3.66	5.13	5.78	0.16	0.45	0.73	0.87
DA-R8	0.77	0.90	0.95	3.66	5.13	5.78	0.11	0.32	0.52	0.62
DA-R9	0.76	0.90	0.95	3.66	5.13	5.78	0.12	0.35	0.57	0.68
DA-R10	0.76	0.90	0.95	3.66	5.13	5.78	0.12	0.34	0.57	0.68
DA-R11	0.75	0.90	0.95	3.66	5.13	5.78	0.16	0.45	0.75	0.90
DA-R12	0.74	0.90	0.95	3.66	5.13	5.78	0.13	0.37	0.62	0.74
DA-R13	0.73	0.90	0.95	3.66	5.13	5.78	0.17	0.45	0.77	0.92
DA-R14	0.73	0.90	0.95	3.66	5.13	5.78	0.17	0.44	0.77	0.92
DA-R15	0.72	0.90	0.95	3.66	5.13	5.78	0.09	0.24	0.42	0.50
DA-R16	0.71	0.90	0.95	3.66	5.13	5.78	0.09	0.24	0.42	0.50
DA-R17	0.71	0.90	0.95	3.66	5.13	5.78	0.12	0.30	0.53	0.63
DA-R18	0.70	0.90	0.95	3.66	5.13	5.78	0.10	0.26	0.46	0.55
DA-R19	0.69	0.90	0.95	3.66	5.13	5.78	0.10	0.25	0.46	0.55
DA-R20	0.68	0.90	0.95	3.66	5.13	5.78	0.13	0.33	0.61	0.73
DA-R21	0.68	0.90	0.95	3.66	5.13	5.78	0.16	0.41	0.76	0.90
DA-R22	0.67	0.90	0.95	3.66	5.13	5.78	0.12	0.29	0.55	0.66
DA-R23	0.66	0.90	0.95	3.66	5.13	5.78	0.14	0.35	0.66	0.79
DA-R24	0.66	0.90	0.95	3.66	5.13	5.78	0.12	0.29	0.55	0.66
DA-R25	0.65	0.90	0.95	3.66	5.13	5.78	0.13	0.30	0.58	0.69
DA-R26	0.64	0.90	0.95	3.66	5.13	5.78	0.13	0.29	0.58	0.69
DA-R27	0.63	0.90	0.95	3.66	5.13	5.78	0.07	0.16	0.31	0.37
DA-R28	0.63	0.90	0.95	3.66	5.13	5.78	0.13	0.30	0.60	0.72
DA-R29	0.62	0.90	0.95	3.66	5.13	5.78	0.13	0.30	0.60	0.72
DA-R30	0.61	0.90	0.95	3.66	5.13	5.78	0.13	0.29	0.60	0.72

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N1

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	141.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	28.35	[ft]
Drainage Area (A)=	0.45	[acres]	Elevation $_{Difference}$:	2.65	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01879	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(1)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042166	10.0	3.66
50	-0.00625	0.04	0.042166	10.0	5.1
100	-0.00625	0.04	0.042166	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 1.43 **[cfs]**
Q_{50} = 2.08 **[cfs]**
Q_{100} = 2.47 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N2

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	116.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	27.60	[ft]
Drainage Area (A)=	0.39	[acres]	Elevation $_{Difference}$:	3.40	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02931	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042577	10.0	3.66
50	-0.00625	0.04	0.042577	10.0	5.1
100	-0.00625	0.04	0.042577	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 1.27 **[cfs]**
Q_{50} = 1.79 **[cfs]**
Q_{100} = 2.12 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N3

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	134.50	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	28.30	[ft]
Drainage Area (A)=	0.24	[acres]	Elevation $_{Difference}$:	2.70	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02007	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043908	10.0	3.66
50	-0.00625	0.04	0.043908	10.0	5.1
100	-0.00625	0.04	0.043908	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.78 **[cfs]**
Q_{50} = 1.09 **[cfs]**
Q_{100} = 1.30 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N4

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	126.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.00	[ft]
Drainage Area (A)=	0.20	[acres]	Elevation $_{Difference}$:	2.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01587	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044375	10.0	3.66
50	-0.00625	0.04	0.044375	10.0	5.1
100	-0.00625	0.04	0.044375	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.66 **[cfs]**
Q_{50} = 0.92 **[cfs]**
Q_{100} = 1.10 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N5

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	133.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.00	[ft]
Drainage Area (A)=	0.19	[acres]	Elevation $_{Difference}$:	2.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01504	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044446	10.0	3.66
50	-0.00625	0.04	0.044446	10.0	5.1
100	-0.00625	0.04	0.044446	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.64 **[cfs]**
Q_{50} = 0.90 **[cfs]**
Q_{100} = 1.07 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N6

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	69.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.00	[ft]
Drainage Area (A)=	0.14	[acres]	Elevation $_{Difference}$:	2.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02899	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045428	10.0	3.66
50	-0.00625	0.04	0.045428	10.0	5.1
100	-0.00625	0.04	0.045428	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.45 **[cfs]**
Q_{50} = 0.63 **[cfs]**
Q_{100} = 0.74 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N7

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	86.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.00	[ft]
Drainage Area (A)=	0.17	[acres]	Elevation $_{Difference}$:	2.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02326	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044828	10.0	3.66
50	-0.00625	0.04	0.044828	10.0	5.1
100	-0.00625	0.04	0.044828	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.56 **[cfs]**
Q_{50} = 0.78 **[cfs]**
Q_{100} = 0.93 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N8

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	85.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.00	[ft]
Drainage Area (A)=	0.18	[acres]	Elevation $_{Difference}$:	2.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02353	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(1)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044668	10.0	3.66
50	-0.00625	0.04	0.044668	10.0	5.1
100	-0.00625	0.04	0.044668	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.59 **[cfs]**
Q_{50} = 0.83 **[cfs]**
Q_{100} = 0.98 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N9

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	83.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.00	[ft]
Drainage Area (A)=	0.18	[acres]	Elevation $_{Difference}$:	2.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02410	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.04471	10.0	3.66
50	-0.00625	0.04	0.04471	10.0	5.1
100	-0.00625	0.04	0.04471	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.58 **[cfs]**
Q_{50} = 0.81 **[cfs]**
Q_{100} = 0.97 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N10

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	327.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	35.50	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.30	[ft]
Drainage Area (A)=	0.70	[acres]	Elevation $_{Difference}$:	6.20	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01896	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.040975	10.0	3.66
50	-0.00625	0.04	0.040975	10.0	5.1
100	-0.00625	0.04	0.040975	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 2.30 **[cfs]**
Q_{50} = 3.22 **[cfs]**
Q_{100} = 3.83 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N11

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	102.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	30.00	[ft]
Drainage Area (A)=	0.19	[acres]	Elevation $_{Difference}$:	1.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00980	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044447	10.0	3.66
50	-0.00625	0.04	0.044447	10.0	5.1
100	-0.00625	0.04	0.044447	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.64 **[cfs]**
Q_{50} = 0.90 **[cfs]**
Q_{100} = 1.07 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-N12

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	122.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.80	[ft]
Drainage Area (A)=	0.39	[acres]	Elevation $_{Difference}$:	1.20	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00984	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042582	10.0	3.66
50	-0.00625	0.04	0.042582	10.0	5.1
100	-0.00625	0.04	0.042582	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 1.27 **[cfs]**
Q_{50} = 1.78 **[cfs]**
Q_{100} = 2.12 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S1

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	111.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	23.50	[ft]
Drainage Area (A)=	0.46	[acres]	Elevation $_{Difference}$:	7.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.06757	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042113	10.0	3.66
50	-0.00625	0.04	0.042113	10.0	5.1
100	-0.00625	0.04	0.042113	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 1.51 **[cfs]**
Q_{50} = 2.12 **[cfs]**
Q_{100} = 2.52 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S2

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	73.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	30.00	[ft]
Drainage Area (A)=	0.17	[acres]	Elevation $_{Difference}$:	1.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01370	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044831	10.0	3.66
50	-0.00625	0.04	0.044831	10.0	5.1
100	-0.00625	0.04	0.044831	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.56 **[cfs]**
Q_{50} = 0.78 **[cfs]**
Q_{100} = 0.93 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S3

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	159.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	24.20	[ft]
Drainage Area (A)=	0.37	[acres]	Elevation $_{Difference}$:	6.80	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.04277	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(1)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042666	10.0	3.66
50	-0.00625	0.04	0.042666	10.0	5.1
100	-0.00625	0.04	0.042666	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 1.23 **[cfs]**
Q_{50} = 1.73 **[cfs]**
Q_{100} = 2.06 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S4

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	128.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	27.90	[ft]
Drainage Area (A)=	0.22	[acres]	Elevation $_{Difference}$:	3.10	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02422	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044052	10.0	3.66
50	-0.00625	0.04	0.044052	10.0	5.1
100	-0.00625	0.04	0.044052	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.74 **[cfs]**
Q_{50} = 1.04 **[cfs]**
Q_{100} = 1.23 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S5

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	123.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	27.90	[ft]
Drainage Area (A)=	0.23	[acres]	Elevation $_{Difference}$:	3.10	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02520	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043957	10.0	3.66
50	-0.00625	0.04	0.043957	10.0	5.1
100	-0.00625	0.04	0.043957	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.77 **[cfs]**
Q_{50} = 1.07 **[cfs]**
Q_{100} = 1.28 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S6

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	160.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	31.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	22.50	[ft]
Drainage Area (A)=	0.33	[acres]	Elevation $_{Difference}$:	8.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.05313	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042973	10.0	3.66
50	-0.00625	0.04	0.042973	10.0	5.1
100	-0.00625	0.04	0.042973	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.90
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 1.10 **[cfs]**
Q_{50} = 1.54 **[cfs]**
Q_{100} = 1.84 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S7

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	71.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	34.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.75	[ft]
Drainage Area (A)=	0.23	[acres]	Elevation $_{Difference}$:	4.25	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.05986	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043938	10.0	3.66
50	-0.00625	0.04	0.043938	10.0	5.1
100	-0.00625	0.04	0.043938	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.85
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.73 **[cfs]**
Q_{50} = 1.08 **[cfs]**
Q_{100} = 1.29 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S8

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	76.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	30.60	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.50	[ft]
Drainage Area (A)=	0.27	[acres]	Elevation $_{Difference}$:	1.10	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01447	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043523	10.0	3.66
50	-0.00625	0.04	0.043523	10.0	5.1
100	-0.00625	0.04	0.043523	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.85
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.85 **[cfs]**
Q_{50} = 1.26 **[cfs]**
Q_{100} = 1.50 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S9

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	104.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	33.50	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	33.00	[ft]
Drainage Area (A)=	0.41	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00481	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.04245	10.0	3.66
50	-0.00625	0.04	0.04245	10.0	5.1
100	-0.00625	0.04	0.04245	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.86
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 1.27 **[cfs]**
Q_{50} = 1.87 **[cfs]**
Q_{100} = 2.23 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S10

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	76.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	30.60	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.50	[ft]
Drainage Area (A)=	0.18	[acres]	Elevation $_{Difference}$:	1.10	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01447	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(1)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044696	10.0	3.66
50	-0.00625	0.04	0.044696	10.0	5.1
100	-0.00625	0.04	0.044696	10.0	5.78

Peak Discharge (Q):
$Q = C_*i_*A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.85
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.55 **[cfs]**
Q_{50} = 0.82 **[cfs]**
Q_{100} = 0.97 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S11

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	98.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	30.60	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.50	[ft]
Drainage Area (A)=	0.31	[acres]	Elevation $_{Difference}$:	1.10	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01122	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043139	10.0	3.66
50	-0.00625	0.04	0.043139	10.0	5.1
100	-0.00625	0.04	0.043139	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.84
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.97 **[cfs]**
Q_{50} = 1.45 **[cfs]**
Q_{100} = 1.73 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S12

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	81.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	30.60	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	29.50	[ft]
Drainage Area (A)=	0.23	[acres]	Elevation $_{Difference}$:	1.10	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01358	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043945	10.0	3.66
50	-0.00625	0.04	0.043945	10.0	5.1
100	-0.00625	0.04	0.043945	10.0	5.78

Peak Discharge (Q):
$Q = C_*i_*A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.83
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.71 **[cfs]**
Q_{50} = 1.08 **[cfs]**
Q_{100} = 1.28 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-S13

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	66.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	35.50	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	33.50	[ft]
Drainage Area (A)=	0.24	[acres]	Elevation $_{Difference}$:	2.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.03030	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043888	10.0	3.66
50	-0.00625	0.04	0.043888	10.0	5.1
100	-0.00625	0.04	0.043888	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.83
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.72 **[cfs]**
Q_{50} = 1.10 **[cfs]**
Q_{100} = 1.31 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R1

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.12	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045701	10.0	3.66
50	-0.00625	0.04	0.045701	10.0	5.1
100	-0.00625	0.04	0.045701	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.82
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.37 **[cfs]**
Q_{50} = 0.57 **[cfs]**
Q_{100} = 0.67 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R2

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.11	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045931	10.0	3.66
50	-0.00625	0.04	0.045931	10.0	5.1
100	-0.00625	0.04	0.045931	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.81
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.33 **[cfs]**
Q_{50} = 0.52 **[cfs]**
Q_{100} = 0.62 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R3

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.11	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.04597	10.0	3.66
50	-0.00625	0.04	0.04597	10.0	5.1
100	-0.00625	0.04	0.04597	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.81
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.33 **[cfs]**
Q_{50} = 0.51 **[cfs]**
Q_{100} = 0.61 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R4

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.16	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045015	10.0	3.66
50	-0.00625	0.04	0.045015	10.0	5.1
100	-0.00625	0.04	0.045015	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.80
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.46 **[cfs]**
Q_{50} = 0.73 **[cfs]**
Q_{100} = 0.87 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R5

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.08	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.046767	10.0	3.66
50	-0.00625	0.04	0.046767	10.0	5.1
100	-0.00625	0.04	0.046767	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.79
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.24 **[cfs]**
Q_{50} = 0.38 **[cfs]**
Q_{100} = 0.45 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R6

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.08	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.046767	10.0	3.66
50	-0.00625	0.04	0.046767	10.0	5.1
100	-0.00625	0.04	0.046767	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.78
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.24 **[cfs]**
Q_{50} = 0.38 **[cfs]**
Q_{100} = 0.45 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R7

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.16	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045001	10.0	3.66
50	-0.00625	0.04	0.045001	10.0	5.1
100	-0.00625	0.04	0.045001	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.78
C_{50}[1] = 0.9
C_{100}[1] = 0.95

Q_{10} = 0.45 **[cfs]**
Q_{50} = 0.73 **[cfs]**
Q_{100} = 0.87 **[cfs]**

Reference:
1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R8

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.11	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045921	10.0	3.66
50	-0.00625	0.04	0.045921	10.0	5.1
100	-0.00625	0.04	0.045921	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.77
C_{50}[1] = 0.9
C_{100}[1] = 0.95

Q_{10} = 0.32 **[cfs]**
Q_{50} = 0.52 **[cfs]**
Q_{100} = 0.62 **[cfs]**

Reference:

1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R9

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.12	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.04567	10.0	3.66
50	-0.00625	0.04	0.04567	10.0	5.1
100	-0.00625	0.04	0.04567	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.76
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.35 **[cfs]**
Q_{50} = 0.57 **[cfs]**
Q_{100} = 0.68 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R10

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.12	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045659	10.0	3.66
50	-0.00625	0.04	0.045659	10.0	5.1
100	-0.00625	0.04	0.045659	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.76
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.34 **[cfs]**
Q_{50} = 0.57 **[cfs]**
Q_{100} = 0.68 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R11

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.16	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044919	10.0	3.66
50	-0.00625	0.04	0.044919	10.0	5.1
100	-0.00625	0.04	0.044919	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.75
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.45 **[cfs]**
Q_{50} = 0.75 **[cfs]**
Q_{100} = 0.90 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R12

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.13	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045441	10.0	3.66
50	-0.00625	0.04	0.045441	10.0	5.1
100	-0.00625	0.04	0.045441	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.74
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.37 **[cfs]**
Q_{50} = 0.62 **[cfs]**
Q_{100} = 0.74 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R13

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.17	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044862	10.0	3.66
50	-0.00625	0.04	0.044862	10.0	5.1
100	-0.00625	0.04	0.044862	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.73
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.45 **[cfs]**
Q_{50} = 0.77 **[cfs]**
Q_{100} = 0.92 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R14

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.17	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044859	10.0	3.66
50	-0.00625	0.04	0.044859	10.0	5.1
100	-0.00625	0.04	0.044859	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.73
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.44 **[cfs]**
Q_{50} = 0.77 **[cfs]**
Q_{100} = 0.92 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R15

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.09	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.046525	10.0	3.66
50	-0.00625	0.04	0.046525	10.0	5.1
100	-0.00625	0.04	0.046525	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.72
$C_{50}^{(1)}$ = 0.9
$C_{100}^{(1)}$ = 0.95

Q_{10} = 0.24 **[cfs]**
Q_{50} = 0.42 **[cfs]**
Q_{100} = 0.50 **[cfs]**

Reference:

1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R16

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.09	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.046525	10.0	3.66
50	-0.00625	0.04	0.046525	10.0	5.1
100	-0.00625	0.04	0.046525	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.71
C_{50}[1] = 0.9
C_{100}[1] = 0.95

Q_{10} = 0.24 **[cfs]**
Q_{50} = 0.42 **[cfs]**
Q_{100} = 0.50 **[cfs]**

Reference:

1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R17

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.12	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.04587	10.0	3.66
50	-0.00625	0.04	0.04587	10.0	5.1
100	-0.00625	0.04	0.04587	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.71
$C_{50}^{(1)}$ = 0.9
$C_{100}^{(1)}$ = 0.95

Q_{10} = 0.30 **[cfs]**
Q_{50} = 0.53 **[cfs]**
Q_{100} = 0.63 **[cfs]**

Reference:
1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R18

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.10	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.046245	10.0	3.66
50	-0.00625	0.04	0.046245	10.0	5.1
100	-0.00625	0.04	0.046245	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.70
C_{50}[1] = 0.9
C_{100}[1] = 0.95

Q_{10} = 0.26 **[cfs]**
Q_{50} = 0.46 **[cfs]**
Q_{100} = 0.55 **[cfs]**

Reference:

1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R19

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.10	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.046245	10.0	3.66
50	-0.00625	0.04	0.046245	10.0	5.1
100	-0.00625	0.04	0.046245	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.69
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.25 **[cfs]**
Q_{50} = 0.46 **[cfs]**
Q_{100} = 0.55 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R20

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.13	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045488	10.0	3.66
50	-0.00625	0.04	0.045488	10.0	5.1
100	-0.00625	0.04	0.045488	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.68
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.33 **[cfs]**
Q_{50} = 0.61 **[cfs]**
Q_{100} = 0.73 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R21

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.16	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044904	10.0	3.66
50	-0.00625	0.04	0.044904	10.0	5.1
100	-0.00625	0.04	0.044904	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.68
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.41 **[cfs]**
Q_{50} = 0.76 **[cfs]**
Q_{100} = 0.90 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R22

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.12	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.04575	10.0	3.66
50	-0.00625	0.04	0.04575	10.0	5.1
100	-0.00625	0.04	0.04575	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.67
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.29 **[cfs]**
Q_{50} = 0.55 **[cfs]**
Q_{100} = 0.66 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R23

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.14	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, $(Tc)^{(2)}$:
$Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38$

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045276	10.0	3.66
50	-0.00625	0.04	0.045276	10.0	5.1
100	-0.00625	0.04	0.045276	10.0	5.78

Peak Discharge (Q):
$Q = C_* i_* A$ [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	$C_{10}^{(1)}$ =	0.66
	$C_{50}^{(1)}$ =	0.90
	$C_{100}^{(1)}$ =	0.95

Q_{10} = 0.35 **[cfs]**
Q_{50} = 0.66 **[cfs]**
Q_{100} = 0.79 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R24

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.12	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045752	10.0	3.66
50	-0.00625	0.04	0.045752	10.0	5.1
100	-0.00625	0.04	0.045752	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.66
$C_{50}^{(1)}$ = 0.9
$C_{100}^{(1)}$ = 0.95

Q_{10} = 0.29 **[cfs]**
Q_{50} = 0.55 **[cfs]**
Q_{100} = 0.66 **[cfs]**

Reference:

1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R25

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.13	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.04563	10.0	3.66
50	-0.00625	0.04	0.04563	10.0	5.1
100	-0.00625	0.04	0.04563	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.65
$C_{50}^{(1)}$ = 0.9
$C_{100}^{(1)}$ = 0.95

Q_{10} = 0.30 **[cfs]**
Q_{50} = 0.58 **[cfs]**
Q_{100} = 0.69 **[cfs]**

Reference:
1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R26

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.13	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045631	10.0	3.66
50	-0.00625	0.04	0.045631	10.0	5.1
100	-0.00625	0.04	0.045631	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.64
$C_{50}^{(1)}$ = 0.9
$C_{100}^{(1)}$ = 0.95

Q_{10} = 0.29 **[cfs]**
Q_{50} = 0.58 **[cfs]**
Q_{100} = 0.69 **[cfs]**

Reference:

1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R27

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.07	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.047335	10.0	3.66
50	-0.00625	0.04	0.047335	10.0	5.1
100	-0.00625	0.04	0.047335	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]	
i	[in/hr]	
C_2	Runoff Coefficient	
	C_{10}[1] =	0.63
	C_{50}[1] =	0.9
	C_{100}[1] =	0.95

Q_{10} = 0.16 **[cfs]**
Q_{50} = 0.31 **[cfs]**
Q_{100} = 0.37 **[cfs]**

Reference:
1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R28

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.13	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045525	10.0	3.66
50	-0.00625	0.04	0.045525	10.0	5.1
100	-0.00625	0.04	0.045525	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.63
$C_{50}^{(1)}$ = 0.9
$C_{100}^{(1)}$ = 0.95

Q_{10} = 0.30 **[cfs]**
Q_{50} = 0.60 **[cfs]**
Q_{100} = 0.72 **[cfs]**

Reference:
1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R29

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.13	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045525	10.0	3.66
50	-0.00625	0.04	0.045525	10.0	5.1
100	-0.00625	0.04	0.045525	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.62
$C_{50}^{(1)}$ = 0.9
$C_{100}^{(1)}$ = 0.95

Q_{10} = 0.30 **[cfs]**
Q_{50} = 0.60 **[cfs]**
Q_{100} = 0.72 **[cfs]**

Reference:
1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-701

Concentration Point ID: DA-R30

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	84.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	0.50	[ft]
Drainage Area (A)=	0.13	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00595	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38+Time to Retention

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.00625
b= 0.04
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045525	10.0	3.66
50	-0.00625	0.04	0.045525	10.0	5.1
100	-0.00625	0.04	0.045525	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.61
$C_{50}^{(1)}$ = 0.9
$C_{100}^{(1)}$ = 0.95

Q_{10} = 0.29 **[cfs]**
Q_{50} = 0.60 **[cfs]**
Q_{100} = 0.72 **[cfs]**

Reference:

1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology
2. Town of Gilbert Public Works And Engineering Standards, September 21,2015

Appendix F
Hydraulic Calculations
Vi at Cavasson

Hubbard Engineering
Project No. 18114-701

Project Name: Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-601 | **Revised By:** | **Date:**

VOLUME PROVIDED

$$V_{U.P.} = \pi * r^2 * L$$

$V_{U.P.}$	*Volume of underground pipe provided in cubic feet*	
π	*pi*	
r	*Underground pipe radius, ft*	*5*
L	*Length of underground pipe in feet*	

UNDERGROUND RETENTION/DETENTION COMBINED

Retention Method	*Length*	*Volume Provided*
(ID)	*(ft)*	*(cf)*
UG-TANK 1	88	6,912
UG-TANK 2	88	6,912
UG-TANK 3	117	9,189
UG-TANK 4	60	4,712
UG-TANK 5	60	4,712
UG-TANK 6	60	4,712
UG-TANK 7	60	4,712
Total	533	41,862

HYDRAULIC CALCULATION SHEET
Retention Calculations
Hubbard Engineering
Project No. 18114-701

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised By:** | **Date:**

Purpose: Evaluate the required and provided retention volumes in order to assess conformance to project criteria.

Methodology: Calculate the volume of stormwater required to be retained using City of Scottsdale criteria. Calculate the estimated volume of stormwater retained using retention basin geometry.

Criteria: Retain the calculated stormwater run-off for the first flush condition and 100-YEAR 6-HOUR duration storm event.

References: 1. City of Scottsdale *Design Standards and Policies Manual*

Calculations: First Flush Volume Required = $C_{Composite}$ *D/12* A [ft^3] (Reference 1)

D = 0.5	[in]	(First Flush)	(Reference 1)
C = 0.90	(Paved Parking)		(Reference 1)
P = 2.75	[in]	(100 YR 6 HR)	

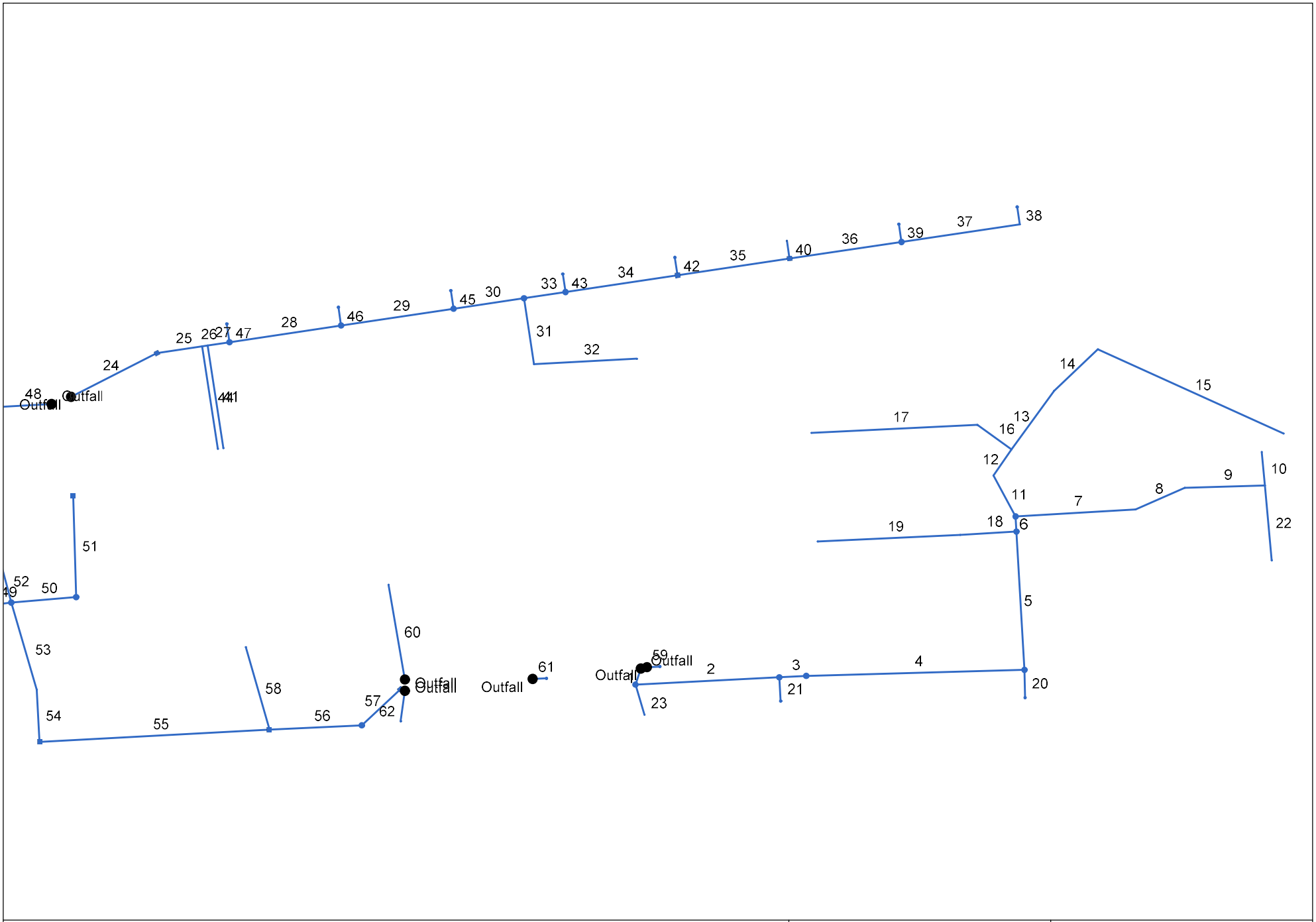
Composite C= (C1*A1 + C2*A2…+….)/(A1+A2+….)

Volume Required = Composite C*P/12*A

Results:

Identifiers	CALCULATE RETENTION VOLUME REQUIRED		Volume Required		
Contributory Area ID	Area [acres]	C	[acre-ft]	[ft^3]	[yd^3]
1C-2 Miller	0.45	0.95	0.10	4,300	159
1C-2 Cavasson	1.51	0.95	0.33	14,351	532
1-C2 P3	0.29	0.95	0.06	2,782	103
North	5.44	0.95	0.22	9,380	347
South	5.44	0.95	0.22	9,373	347
Total Area:	**13.14**	**TOTAL VOLUME REQUIRED:**	**0.92**	**40,186.26**	**1,488.38**

Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



Project File: 18114-701 SD.stm	Number of lines: 62	Date: 8/30/2021

MyReport

Line No.	Line ID	Vel Ave (ft/s)	Line Length (ft)	Flow Rate (cfs)	HGL Up (ft)	HGL Dn (ft)	Gnd/Rim El Up (ft)	Gnd/Rim El Dn (ft)	Invert Up (ft)	Invert Dn (ft)	EGL Dn (ft)	EGL Up (ft)	Line Size (in)	Line Slope (%)
1	SPipe1	3.39	15.040	23.94	22.02	22.00	27.90	28.20	12.06	12.00	22.18	22.20	36	0.40
2	SPipe2	2.80	127.248	19.78	22.31	22.20	22.70	27.90	12.57	12.06	22.32	22.43	36	0.40
3	SPipe4	2.54	23.790	17.94	22.45	22.43	22.80	22.70	12.66	12.57	22.53	22.55	36	0.38
4	SPipe5	5.71	192.829	17.94	23.68	22.46	30.00	22.80	13.44	12.66	22.97	24.18	24	0.40
5	SPipe6	5.30	122.769	16.65	24.85	24.18	30.00	30.00	13.93	13.44	24.62	25.29	24	0.40
6	SPipe7	7.68	13.502	13.57	25.51	25.29	31.00	30.00	13.98	13.93	26.20	26.43	18	0.37
7	R28	4.44	106.132	6.00	27.43 j	26.43	33.00	31.00	26.48	25.16	26.84	27.83	18	1.24
8	R29	4.69	47.452	5.28	27.83 j	27.43	33.00	33.00	26.95	26.48	27.79	28.20	18	0.99
9	R30	4.41	70.812	4.56	28.48 j	27.83	33.00	33.00	27.66	26.95	28.17	28.81	18	1.00
10	S9	3.63	29.800	2.23	30.13	29.98	33.00	33.00	29.40	29.25	30.18	30.34	12	0.50
11	S8	5.19	41.132	7.57	26.63 j	26.43	30.25	31.00	25.57	25.16	26.92	27.13	18	1.00
12	Pipe - (109) (1)	7.73	28.056	6.07	27.49	26.67	31.84	30.25	25.96	25.67	27.60	28.41	12	1.03
13	R27	3.90	64.073	3.06	29.35	28.88	30.80	31.84	26.60	25.96	29.11	29.59	12	1.00
14	R25	3.43	53.617	2.69	29.78	29.47	34.00	30.80	27.23	26.70	29.65	29.96	12	0.99
15	S13 R26	2.55	180.153	2.00	30.60	30.03	33.50	34.00	29.14	27.33	30.13	30.70	12	1.00
16	S11	3.83	37.084	3.01	29.14	28.88	29.50	31.84	25.42	25.05	29.11	29.37	12	1.00
17	S12	1.63	146.664	1.28	29.56	29.37	29.50	29.50	26.99	25.52	29.42	29.61	12	1.00
18	R24	3.92	49.687	3.08	25.66	25.29	29.50	30.00	15.43	13.93	25.53	25.90	12	3.02
19	S10 R22 R23	3.08	125.983	2.42	26.36	25.78	29.50	29.50	19.21	15.43	25.93	26.51	12	3.00
20	S7	1.64	24.691	1.29	24.22	24.18	29.75	30.00	16.24	15.50	24.23	24.26	12	3.00
21	Pipe - (115)	2.34	21.103	1.84	22.49	22.43	26.73	22.70	13.20	12.57	22.52	22.57	12	2.99
22	CLARET	2.49	66.596	1.61	29.48 j	28.48	32.52	33.00	29.00	27.66	28.65	29.65	18	2.01
23	CAVASSON2	2.35	27.897	4.16	22.24	22.20	27.01	27.90	13.26	12.06	22.28	22.33	18	4.30

Project File: 18114-701 SD.stm | Number of lines: 62 | Date: 8/30/2021

NOTES: ** Critical depth

MyReport

Line No.	Line ID	Vel Ave (ft/s)	Line Length (ft)	Flow Rate (cfs)	HGL Up (ft)	HGL Dn (ft)	Gnd/Rim El Up (ft)	Gnd/Rim El Dn (ft)	Invert Up (ft)	Invert Dn (ft)	EGL Dn (ft)	EGL Up (ft)	Line Size (in)	Line Slope (%)
24	NPipe1	3.42	84.987	24.09	24.10	24.00	28.85	29.00	21.17	20.74	24.18	24.28	36	0.51
25	NPipe2	4.89	40.666	24.09	22.95	24.17	28.85	28.85	21.37	21.17	24.80	23.58	36	0.49
26	NPipe3	6.38	4.885	24.08	22.98	22.95	28.85	28.85	21.40	21.37	23.58	23.61	36	0.61
27	NPipe4	6.16	19.444	19.34	23.50	23.40	29.20	28.85	21.50	21.40	23.99	24.09	24	0.51
28	NPipe5	5.74	99.635	18.04	24.72	24.09	29.20	29.20	22.00	21.50	24.60	25.24	24	0.50
29	NPipe6	5.39	100.429	16.94	25.80	25.24	29.20	29.20	22.50	22.00	25.69	26.25	24	0.50
30	NPipe7	4.58	62.879	14.38	26.51	26.25	29.65	29.20	22.81	22.50	26.58	26.83	24	0.49
31	N12 Connect	1.39	58.976	4.38	26.85	26.83	29.80	29.65	24.00	22.82	26.86	26.88	24	2.00
32	N12 R9-11	1.97	91.078	4.38	26.90	26.90	29.80	29.80	25.82	24.00	26.93	27.00	24	2.00
33	NPipe8	3.18	37.121	10.00	26.90	26.83	29.20	29.65	23.00	22.81	26.99	27.06	24	0.51
34	NPipe9	2.95	99.988	9.26	27.23	27.06	29.20	29.20	23.50	23.00	27.20	27.36	24	0.50
35	NPipe10	2.43	100.012	7.62	27.48	27.36	29.20	29.20	24.00	23.50	27.46	27.57	24	0.50
36	NPipe11	1.82	99.988	5.72	27.63	27.57	29.20	29.20	24.50	24.00	27.62	27.68	24	0.50
37	NPipe12	1.22	105.631	3.83	27.72	27.68	29.30	29.20	25.02	24.50	27.71	27.74	24	0.49
38	N10	4.88	15.546	3.83	27.93	27.75	29.00	29.30	25.19	25.03	28.12	28.30	12	1.03
39	N9 R14	2.41	15.952	1.89	27.73	27.68	29.00	29.20	24.66	24.50	27.77	27.82	12	1.00
40	N8 R13	2.42	16.036	1.90	27.62	27.57	29.00	29.20	24.16	24.00	27.66	27.71	12	1.00
41	N11 R1-6	6.08	91.927	4.74	26.18	24.08	30.00	28.85	25.34	23.50	24.42	26.52	18	2.00
42	N7 R12	2.09	16.105	1.64	27.40	27.36	29.00	29.20	23.46	23.50	27.43	27.47	12	-0.25
43	N6	0.94	16.197	0.74	27.07	27.06	29.00	29.20	23.16	23.00	27.08	27.08	12	0.99
44	Pipe - (160)	0.46	91.915	0.01	24.45 j	22.95	30.00	28.85	24.41	22.57	22.96	24.46	18	2.00
45	N5 R7 R8	3.26	16.283	2.56	26.34	26.25	29.00	29.20	22.46	22.50	26.42	26.50	12	-0.25
46	N4	1.40	16.388	1.10	25.25	25.24	29.00	29.20	22.23	22.00	25.27	25.28	12	1.40

Project File: 18114-701 SD.stm | Number of lines: 62 | Date: 8/30/2021

NOTES: ** Critical depth

MyReport

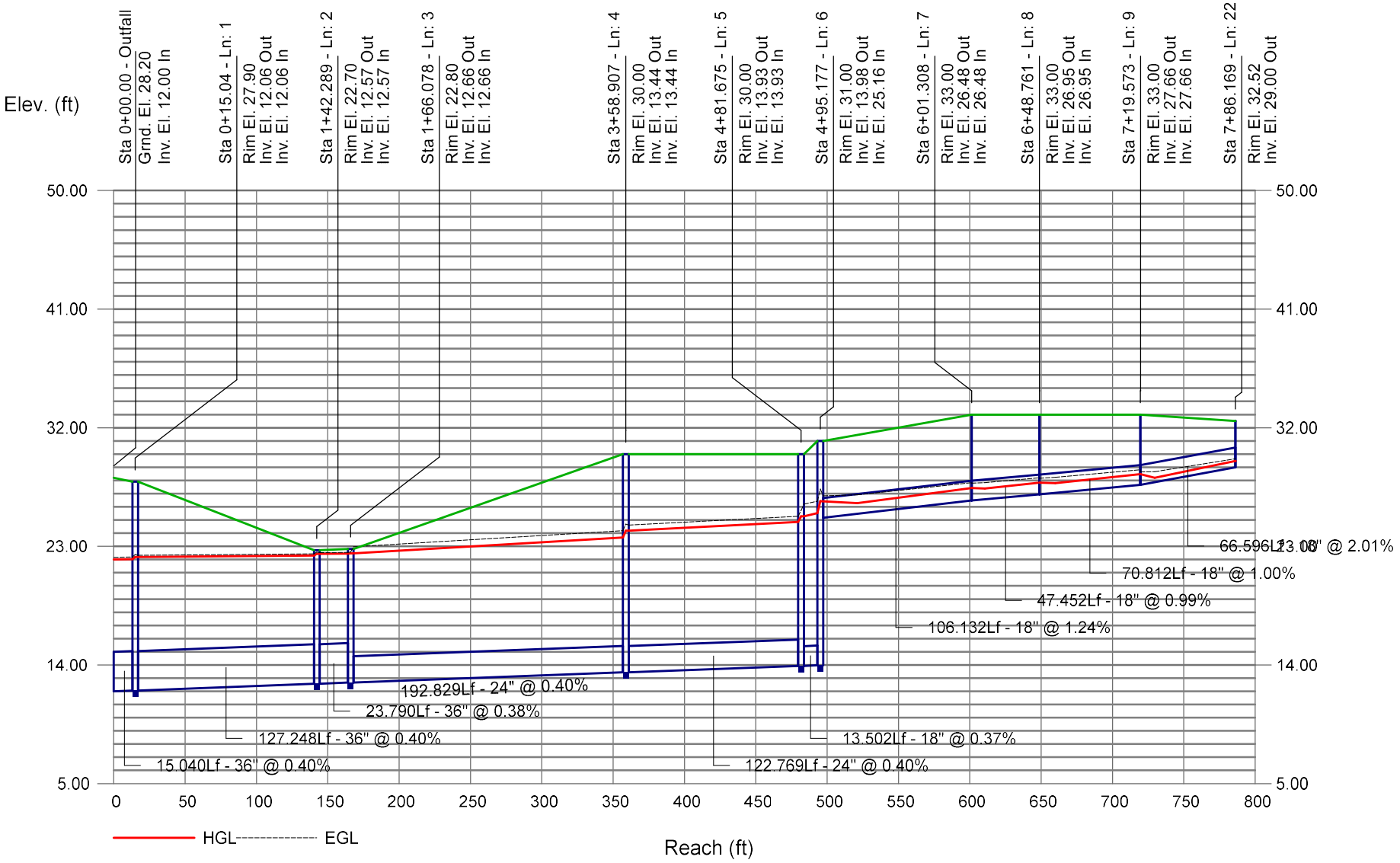
Line No.	Line ID	Vel Ave (ft/s)	Line Length (ft)	Flow Rate (cfs)	HGL Up (ft)	HGL Dn (ft)	Gnd/Rim El Up (ft)	Gnd/Rim El Dn (ft)	Invert Up (ft)	Invert Dn (ft)	EGL Dn (ft)	EGL Up (ft)	Line Size (in)	Line Slope (%)
47	N3	1.66	16.410	1.30	24.11	24.09	29.00	29.20	21.73	21.50	24.13	24.15	12	1.40
48	N2	2.80	43.822	2.12	24.13	24.00	27.38	28.50	23.25	22.27	24.11	24.26	12	2.24
49	EOUT1	6.86	13.530	18.51	16.74	16.55	29.50	25.75	15.08	15.00	17.33	17.43	24	0.59
50	R15 R16	6.65	57.501	4.40	23.36	22.03	29.80	29.50	22.48	21.31	22.59	23.92	12	2.03
51	EOutN	6.23	89.615	3.40	25.52	23.36	29.70	29.80	24.25	22.48	23.96	26.12	10	1.98
52	N1	4.67	39.121	2.47	20.66	20.20	28.35	29.50	19.99	19.59	20.50	20.96	12	1.02
53	EOutS1	3.71	80.436	11.64	17.64	17.43	29.75	29.50	15.48	15.08	17.64	17.85	24	0.50
54	EOutS2	3.71	46.218	11.64	17.87	17.75	21.75	29.75	15.71	15.48	17.96	18.08	24	0.50
55	CAVASSON1	3.78	203.389	11.64	18.57	18.08	24.00	21.75	16.73	15.71	18.29	18.80	24	0.50
56	S1	7.54	81.598	5.92	23.29	21.03	24.25	24.00	21.25	20.03	21.91	24.17	12	1.50
57	EOutS3	6.23	46.731	3.40	25.02	23.90	25.20	24.25	22.25	21.25	24.50	25.63	10	2.14
58	S2	4.80	76.105	1.56	22.78	20.33	30.00	24.00	22.31	20.03	20.50	22.95	18	3.00
59	S5	1.63	11.453	1.28	22.01	22.00	27.90	28.20	18.41	18.29	22.04	22.06	12	1.05
60	R18-R20	3.12	85.287	1.83	23.29 j	22.00	26.87	25.40	22.71	20.00	22.08	23.52	12	3.18
61	S4	1.57	12.239	1.23	22.01	22.00	27.90	27.60	16.54	16.43	22.04	22.05	12	0.90
62	S3	2.62	27.210	2.06	22.09	22.00	21.03	25.20	13.76	11.61	22.11	22.20	12	7.90

Project File: 18114-701 SD.stm | Number of lines: 62 | Date: 8/30/2021

NOTES: ** Critical depth

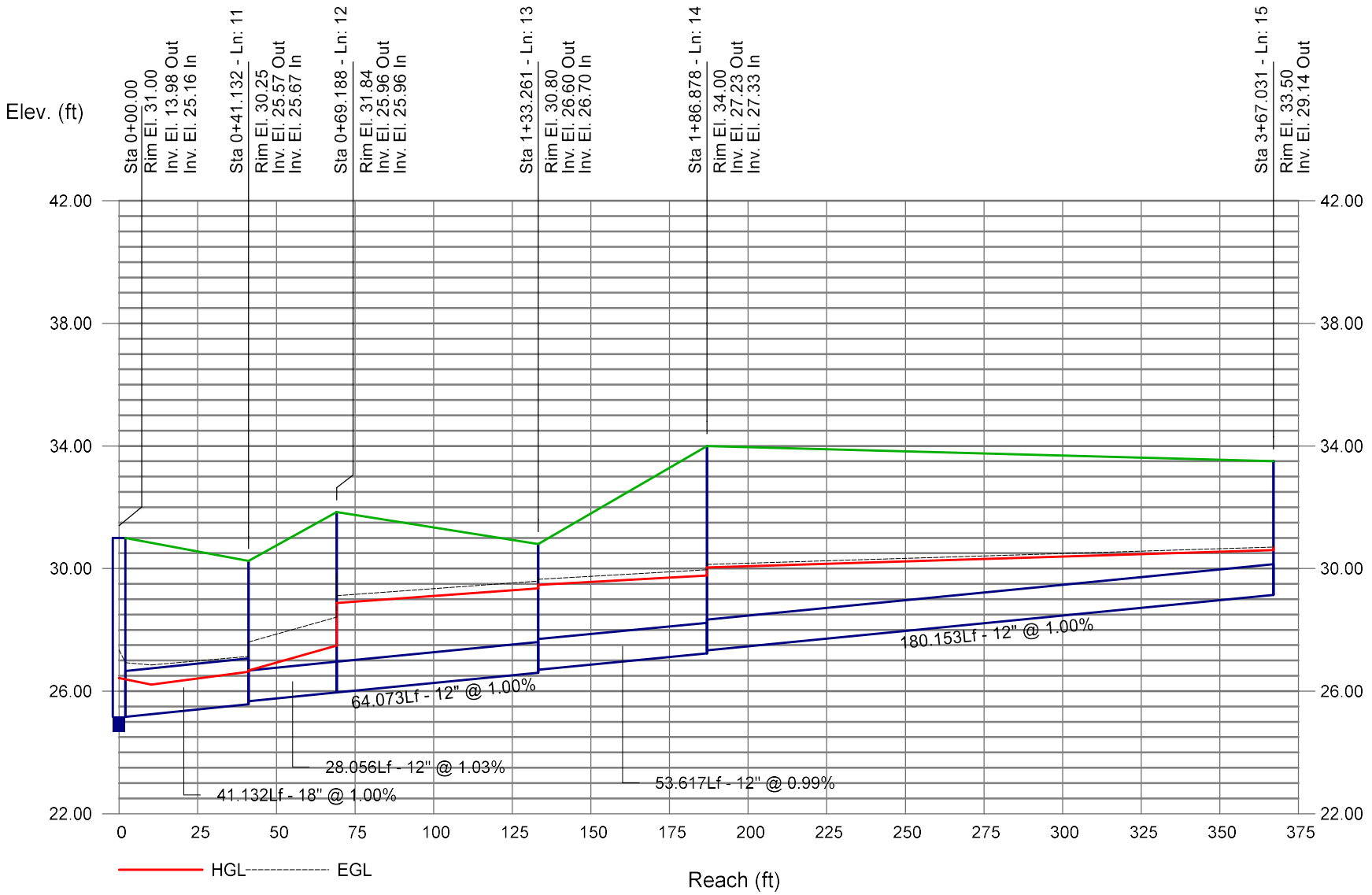
Storm Sewer Profile

Proj. file: 18114-701 SD.stm



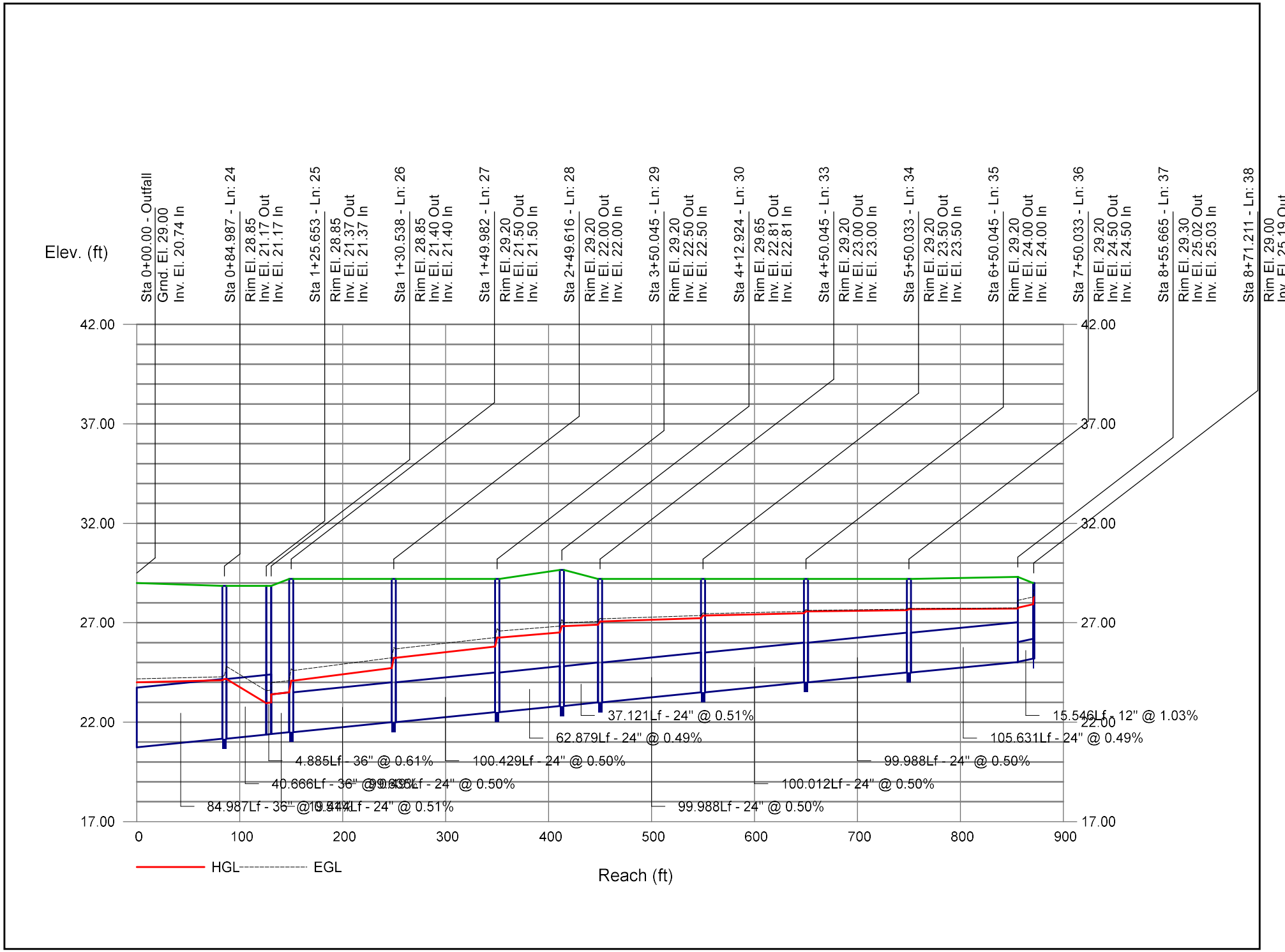
Storm Sewer Profile

Proj. file: 18114-701 SD.stm



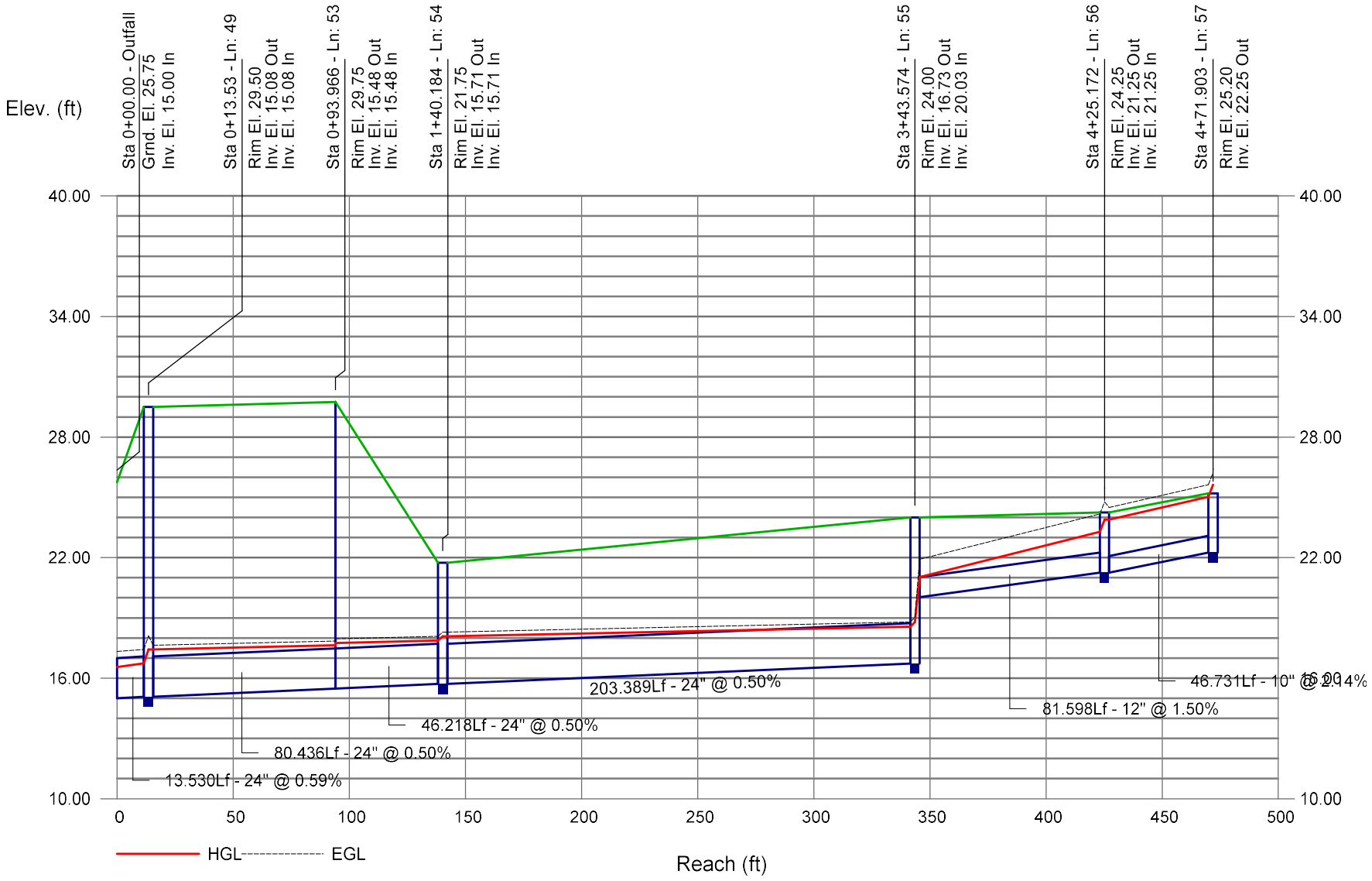
Storm Sewer Profile

Proj. file: 18114-701 SD.stm



Storm Sewer Profile

Proj. file: 18114-701 SD.stm



HYDRAULIC CALCULATION SHEET
Retention Basin Drain Time Calculations
Hubbard Engineering
Project No. 18114-701

Project Name: The Vi at Cavasson | **Prepared by:** MJA | **Date:** 09/01/21
Project No.: 18114-701 | **Revised by:** | **Date:**

Purpose: Calculate the number of drywells required to facilitate drainage of the required volume within 36 hours.

Methodology: Calculate the number of drywells necessary to drain the retention basin and undeground storage tanks within 36 hours, assuming each drywell is not to exceed a drain volume of 43,560 cubic feet in 36 hours.

Criteria:

1. Drywell drainage capacity must discharge the retention volume provided within 36 hours.
2. Percolation rates have been assumed to be 0.1 cfs and will be field verified during construction

References: 1. Design Standards & Policies Manual, City of Scottsdale, January 2018

Calculations:

Number of Drywells Required = V_{DW} / (Drywell Infiltration Rate * 3600 * 36 hours)

Drywell Infiltration Rate = 0.1 [cfs] (Reference 1)

Results:

Identifiers	Infiltration Drainage Capacity Calculations					V_{DW}	Number of Drywells Required
Retention Basin ID	Volume[Ref. 1] Required [ft^3]	Bottom[Ref 1] Area [ft^2]	Percolation Rate[Ref. 2] Test Location*	Percolation Rate [$ft^3/hr/ft^2$]^2	Infiltration Drain Capacity [ft^3]	[ft^3]	
North	13,679.9	0.0	--	0.000	0.0	13,679.9	2
South	26,506.4	0.0	--	0.000	0.0	26,506.4	3
					Total Number of Drywells =		**5**

Appendix G
404 Permit
Vi at Cavasson

March 27, 2019

U.S. Army Corps of Engineers
Los Angeles District – Regulatory Division
915 Wilshire Blvd.
Los Angeles, CA 90017

Subject: Statement of Sale of 0.89 Credits for the Cavasson Mixed-Use Development project, Corps permit number SPL-2018-00704, from the Arizona Game and Fish Department In-Lieu Fee Program to Nationwide Reality Investors Ltd.

The Arizona Game and Fish Department has an agreement with the U.S. Army Corps of Engineers – Los Angeles District to operate an In-Lieu-Fee Program. This letter confirms the sale of 0.89 credits of Advance Credits for establishment/enhancement of wetlands and Mesquite Bosque within the Arlington Wildlife Area; an Arizona Game and Fish Commission owned property. These credits are being used as compensatory mitigation for 0.89 acres of impact calculated at a 1:1 ratio for impacts to Waters of The U.S. associated with the Cavasson Mixed-Use Development Project. By selling credits to the above permittee, AGFD is the party responsible for fulfilling the mitigation aspect of Special Condition of the Permit listed above.

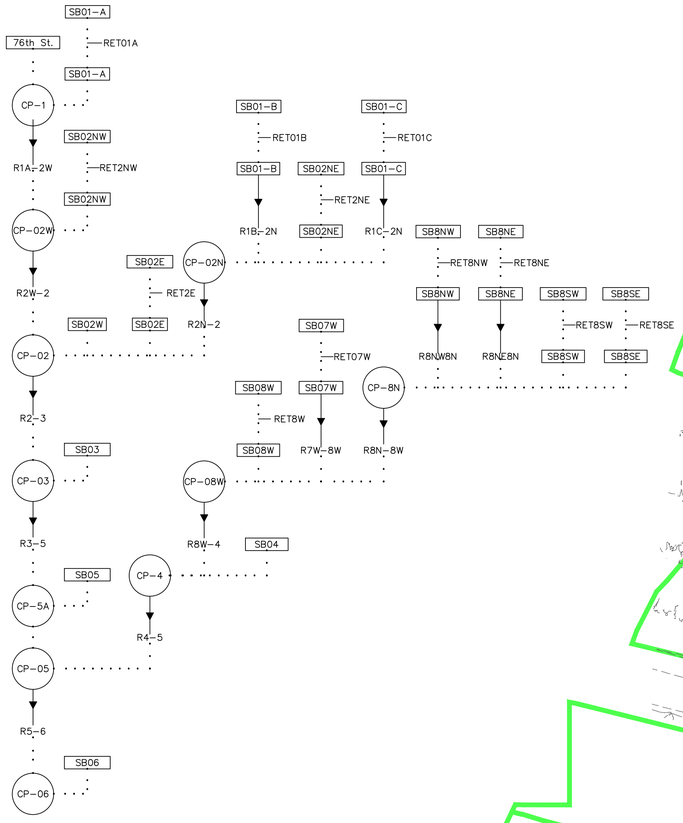
Signed,

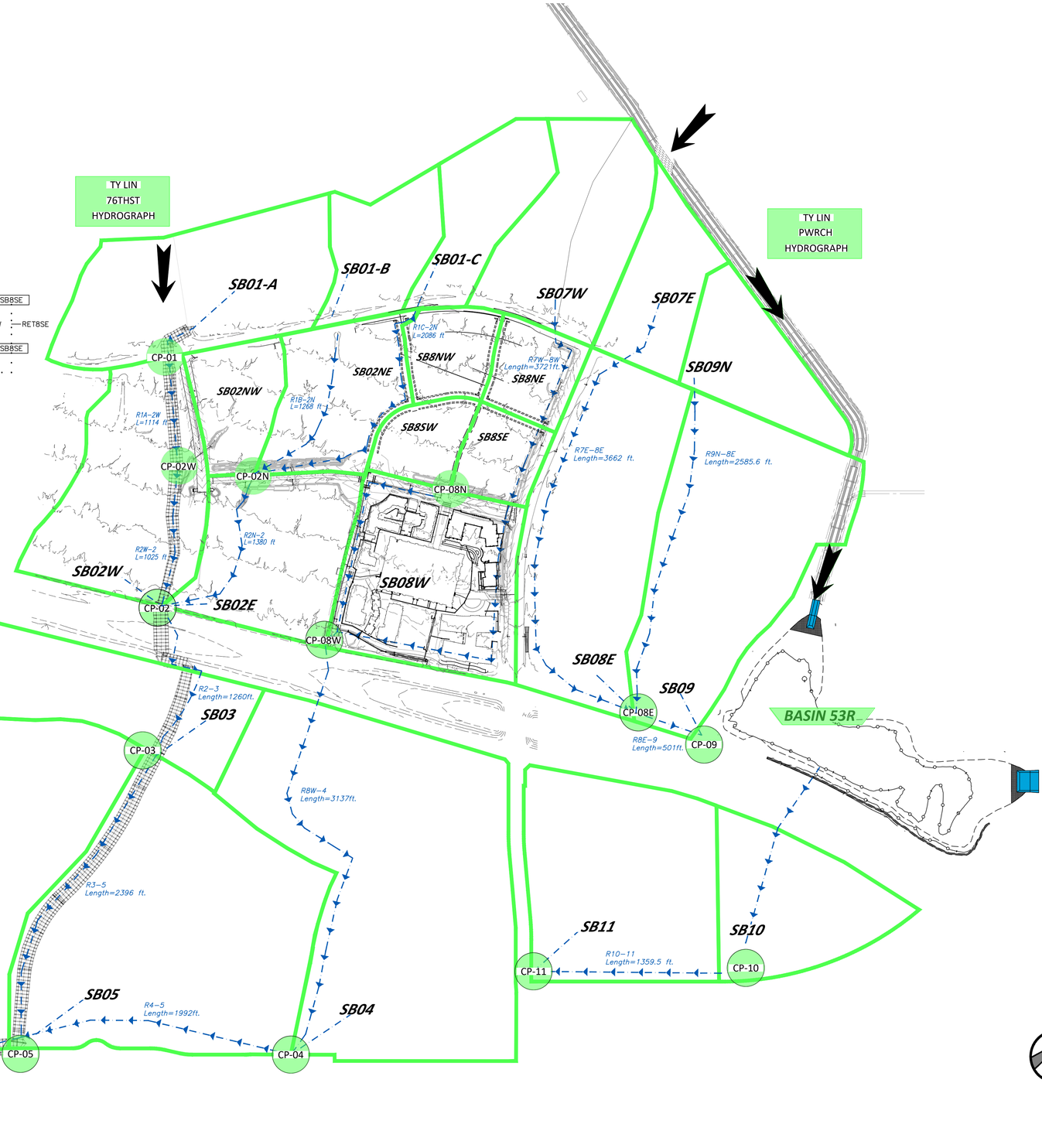
Shawn F. Lowery
Statewide ILF Restoration Program Manager
Wildlife Contracts Branch
Arizona Game and Fish Department

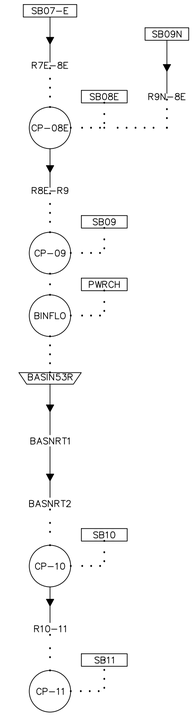
GOVERNOR: DOUGLAS A. DUCEY **COMMISSIONERS:** CHAIRMAN, JAMES R. AMMONS, YUMA | JAMES S. ZIELER, ST. JOHNS | ERIC S. SPARKS, TUSCON
KURT R. DAVIS, PHOENIX | EDWARD "PAT" MADDEN, FLAGSTAFF **DIRECTOR:** TY E. GRAY **DEPUTY DIRECTOR:** TOM P. FINLEY

Exhibits
Vi at Cavasson

HEC -1 ULTIMATE CONDITION







N

400 0 400 800
scale feet

HEC-1 ULTIMATE CONDITIONS

ASLD CORE NORTH

Project No.	Date	
18114-101	rev. 07/23/20	EX-04

HUBBARD ENGINEERING
www.hubbardengineering.com
1201 S. Alma School Rd.,Ste. 12000 - Mesa, AZ 85210
Ph: 480.892.3313

P:\2018\18114\Design-Reports_MP\18114-101\Drainage\Exhibits\DWG Working\HEC1-CrossRoadsEast_Future_MA.dwg Aug 03, 2020 - 6:17pm mwolf

UG-TANK 1
UG-TANK 2
N1
N2
N3
N4
N5
N6
N7
N8
N9
N10
N11
N12
R1
R2
R3
R4
R5
R6
R7
R8
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R24
R25
R26
R27
R28
R29
R30
S1
S2
S3
S4
S5
S6
S7
S8
S9
S10
S11
S12
S13
FFE=31.00
FFE=31.00
FFE=31.00
FFE=31.00
FFE=31.00
LEVEL 1 FFE=31.00
GARAGE FF=19.50 (-11' 6")
LEVEL 1
FFE=35.50
GARAGE
FF=24.00
FFE=33.50
LEVEL 1 FFE=31.00
GARAGE FF=19.50 (-11' 6")
LEVEL 1
FFE=34.00
GARAGE
FF=22.50
ROOF DRAIN (TYP)
UG-TANK 5
UG-TANK 4
UG-TANK 7
UG-TANK 6
UG-TANK 3
SAN
W
NVS
N
40 0 40 80
scale feet

HUBBARD ENGINEERING
1201 S. Alma School Rd.
Suite 12000
Mesa, AZ 85210
Ph: 480.892.3313
www.hubbardengineering.com

EMERGENCY OVERFLOW EXHIBIT WITH OFFSITE DRAINAGE

V1 AT CAVASSON

Date	Project Eng.
09/16/2021	MSW
Project No.	Project Mgr.
18114-701	GMB

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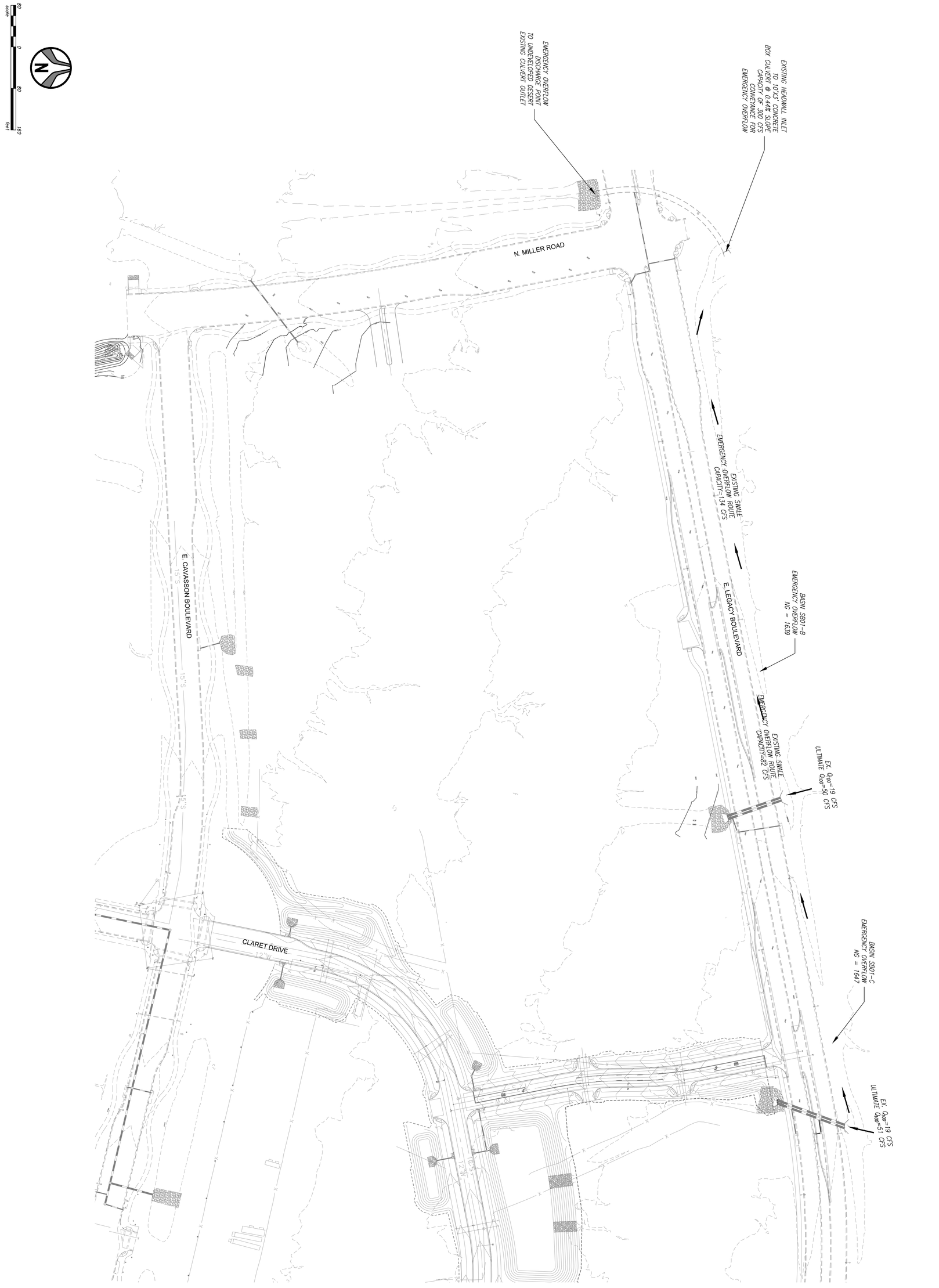
811
Call before you dig.
1.800.STAKE.IT
602.263.1100

EX. 2

P:\2018\18114\Design-Reports\18114-701\Drainage\Preliminary\Sub 1\Exhibits\Exhibit 2 Drainage Map.dwg Sep 07, 2021 - 9:01am mashbrook

P:\2018\18114\Design-Reports\18114-701\Drainage\Preliminary\Sub 1\Exhibits\Exhibit 3 and 4 Emergency Overflow and Existing Flows Exhibits.dwg Sep 07, 2021 - 9:02am mashbrook



EX. 3

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Project No.	Date
18114-701	09/16/2021
Project Mgr.	Project Eng.
GMB	MSW

EXISTING OFFSITE FLOW EXHIBIT WITH EXISTING OVERFLOW

VI AT CAVASSON

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EXISTING HEADWALL INLET TO 10'X3' CONCRETE BOX CULVERT @ 0.44% SLOPE CAPACITY OF 300 CFS CONVEYANCE FOR EMERGENCY OVERFLOW

EMERGENCY OVERFLOW DISCHARGE POINT TO UNDEVELOPED DESERT EXISTING CULVERT OUTLET

N. MILLER ROAD

EMERGENCY OVERFLOW IN THE EVENT OF BACK TO BACK STORM EVENTS PRIOR TO TANKS SURCHARGING OUT OF RIMS FOR PARCEL 1C-1

PROPOSED OFFSITE STORM DRAIN SYSTEM TO CAPTURE FLOWS FROM NORTH OF LEGACY BOULEVARD AND ROUTE AROUND THE SITE. CONSTRUCTION IS EXPECTED TO BEGIN BY NOVEMBER 2021 CAPACITY = 200 CFS

EMERGENCY OVERFLOW IN THE EVENT OF BACK TO BACK STORM EVENTS PRIOR TO TANKS SURCHARGING OUT OF RIMS INV=1622.60

EMERGENCY OVERFLOW IN THE EVENT THAT THE STORM DRAIN IS CLOGGED AND EMERGENCY SURFACE OVERFLOW FG=1622.0

EMERGENCY SURFACE OVERFLOW THROUGH EXISTING CULVERT RIM=23.80

EXISTING SWALE EMERGENCY OVERFLOW ROUTE CAPACITY=134 CFS

E. LEGACY BOULEVARD

BASIN SB01-B EMERGENCY OVERFLOW NG = 1639

EXISTING SWALE EMERGENCY OVERFLOW ROUTE CAPACITY=82 CFS

EX. Q_{100}=19 CFS ULTIMATE Q_{100}=50 CFS

BASIN SB01-C EMERGENCY OVERFLOW NG = 1647

EX. Q_{100}=19 CFS ULTIMATE Q_{100}=51 CFS

CLARET DRIVE

E. CAVASSON BOULEVARD

15"S

LIMIT OF CONSTRUCTION

N

80 0 80 160
scale feet

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EMERGENCY OVERFLOW EXHIBIT WITH OFFSITE DRAINAGE

VI AT CAVASSON

Project No. 18114-701	Date 09/16/2021
Project Mgr. CMB	Project Eng. MSW

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EX. 4

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CATCH BASIN EXHIBIT

V/ AT CAVASSON

A PORTION OF THE NORTHWEST QUARTER OF SECTION 26
TOWNSHIP 4 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER
MERIDIAN, MARICOPA COUNTY, ARIZONA

Date	Project Eng.
09/16/2021	M. WOLF
Project No.	Project Mgr.
18114-701	G. BROWN

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EX 06

P:\2018\18114\Design--Reports\18114-701\Drainage\Preliminary\Sub 1\Exhibits\Exhibit 5 Grate Exhibit.dwg Sep 07, 2021 - 9:06am mashbrook