DRAINAGE REPORT FOR WESTERN TECH OFFICE

		Scottsdale, Arizona
Plan #		April 11 th 2022
Case #		April 14 , 2022
Q-S #		
X Accepted		
Corrections		Capital Project Management
		7447 East Indian School Road, #205
M.R.	07/05/2022	Scottsuale, Arizona 65251
Reviewed By	Date	SITE ADDRESS
		1395 North Hayden Road
		Scottsdale, Arizona 85257

PREPARED BY



4450 north 12th street, #228 phoenix, arizona 85014 CYPRESS # 21.148



TABLE OF CONTENTS

I. INTRODUCTION	
1. PROJECT NAME AND LOCATION	1
2. PURPOSE	1
3. EXISTING STUDIES & REGIONAL DRAINAGE PLAN	1
4. FEMA FLOOD ZONE	1
II. EXISTING DRAINAGE INFRASTRUCTURE	
1. OFFSITE	1
2. ONSITE	2
III. PROPOSED DRAINAGE INFRASTRUCTURE	
1. CONVEYANCE OF RUNOFF	2
2. STORM WATER DETENTION	2
IV.SUMMARY AND CONCLUSION	
1. CONCLUSION	2

APPENDICES

- A Location Map + Aerial Photo
- B FEMA FIRM Map
- C Proposed Watershed Map + Calculations
- D Retention Calculations

I. INTRODUCTION

1. PROJECT NAME AND LOCATION

The Project is known as 'Western Tech Office' and is located in Scottsdale, Arizona at 1395 North Hayden Road, within the Scottsdale Commerce Center industrial park at the southeast corner of Hayden Road and McDowell Road.

The Project site occupies approximately 2.84 acres and is currently developed as an office building with associated parking and drainage infrastructure. The Project has street frontage and access to Commerce Court, a private roadway, along the northern boundary. To the east, west, and south, are developed commercial properties. Refer to Appendix A for Location Map and Aerial Photo. The project lies within an area that historically has been irrigated farmland but has been commercially developed since the early 1980s. The area has slopes consistently and gently to the northwest with average slopes in the range of 0.5% and 1.0%.

The Project consists of the construction of an expansion to the existing office building at the northeast corner of the Project with some minor parking lot improvements.

2. PURPOSE

The intent of this Drainage Report is to provide the drainage scheme for the Project in support the Improvement Plan Submittal.

3. EXISTING STUDIES & REGIONAL DRAINAGE PLAN

The LOOP ROAD GRADING AND PAVING plan for SCOTTSDALE COMMERCE CENTRE dated March 1984, prepared by Ellis Murphy Consulting Engineers was reviewed in preparation of this Drainage Report. The Commerce Court Private Roadway was designed to discharge at the southwest corner of the loop and each project was responsible for onsite retention.

4. FEMA FLOOD ZONE

According to the Federal Emergency Management Agency Flood Insurance Rate Map Panel Number 04013C2235M, dated September 9th, 2020, the Parcel is located in the Zone X area, which is defined as areas outside of the 0.2% annual chance flood. Refer to Appendix B for FEMA FIRM Map.

II. EXISTING DRAINAGE INFRASTRUCTURE

1. OFFSITE

According to a recent topographic survey and field investigations the Project is not impacted by offsite runoff. Runoff generated Commerce Court Private Roadway was designed to discharge at the southwest corner of the private roadway loop via a depressed curb and sidewalk.

2. ONSITE

There currently exists onsite stormdrain and an existing retention basin at the northwest corner of the Project. Total retention volume stored in the existing drainage basin is equal to 9,550 CF.

III. PROPOSED DRAINAGE INFRASTRUCTURE

1. CONVEYANCE OF RUNOFF

In the developed condition, the disturbed portion of the site site is divided into a series of drainage areas (DA) based on proposed grading. The existing basin area consists of DA1. The north half of the new building expansion consists of DA2. The revised parking area at the northeast corner of the Project consists of DA3. The south half of the new building expansion and parking lot revisions immediately south and southeast of the new expansion consists of DA4. DA2, DA3, and DA4 all convey runoff to new drainage basins and ultimately discharges to the new onsite retention chambers. The retention basin in DA1 is modified to accommodate the new building expansion and new stormdrain infrastructure is to be installed to balance the new stormdrain chambers with the revised above ground retention basin. Refer to Appendix C for the Proposed Watershed Map + Calculations.

2. STORM WATER RETENTION

The Project shall match the existing retention volume provided as well as capture additional volume based on increases in the runoff coefficient due to the new building expansion replacing existing landscape and basin area. The site is designed with a network of storm drain pipes that outfalls to a surface retention basin located west of the proposed building expansion and additional stormdrain to balance retention volume between the above ground basin and underground retention chambers. The existing drywell in the above ground basin is being modified to allow for continued use and the underground retention chambers are to be planned with an underdrain and drywell, to be installed only if adequate perc rates cannot be achieved at the base of the new chamber system. When the retention chambers and basin are full, they will outfall to the private roadway loop at the southwest corner, as they did in the existing condition. The finished floor has been set more than 15" above the outfall. Thus, no offsite runoff will impact the Project. Refer to Appendix D for Retention Calculations.

IV. SUMMARY AND CONCLUSION

This Drainage Report is to accompany the Improvement Plan submittal for the Western

Tech Office development project. The Project is designed to convey the 100-year storm event. This Report was written utilizing generally accepted engineering practices and all information herein has been researched through archived documents and all calculations were accomplished through applying the City of Scottsdale Stormwater Policies and Standards Manual and the Hydrology/Hydraulic Manual published by the Maricopa County Flood Control District. APPENDIX A (Location Map + Aerial Photo)



AERIAL PHOTO



APPENDIX B (FEMA FIRM Map)

National Flood Hazard Layer FIRMette

111°54'44"W 33°28'1"N



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

APPENDIX C (Proposed Watershed Map + Calculations)



LEGEND

(CP1)
1200

PROJECT/NEW PROPERTY LINE

EXISTING RIGHT-OF-WAY

- ROADWAY CENTERLINE
- FLOW ARROW WATERSHED BOUNDARY
- CONCENTRATION POINT
- EXISTING STORM DRAIN PIPE
- EXISTING CONTOUR





WESTERN TECH OFFICE EXISTING CONDITIONS HYDROLOGY MAP

JOB NO.: 21.148
SHEET NUMBER



LEGEND

(CP1)

1200

— – – – — EXISTING RIGHT-OF-WAY PROJECT/NEW PROPERTY LINE ROADWAY CENTERLINE FLOW ARROW WATERSHED BOUNDARY CONCENTRATION POINT EXISTING STORM DRAIN PIPE NEW STORM DRAIN PIPE EXISTING CONTOUR ---- FLOW-LINE

CONC	WATERSHI	ed Points
ID	AREA	Q 100
CP1	0.18 AC	0.7 CFS
CP2	0.14 AC	1.0 CFS
CP3	0.14 AC	1.0 CFS
CP4	0.34 AC	2.3 CFS





AP Σ WESTERN TECH OFFICE PROPOSED CONDITIONS HYDROLOGY I

JOB NO.:	21.14
SHEET NUMBER	



SITE ENGINEERING

WATERSHED SUMMARY

PROJECT WESTERN TECH OFFICE

EXISTING

		AREA ROOF +	AREA LANDSCAPING		
WATERSHED	CONCENTRATION	C= 0.95	C= 0.50	TOTAL AREA	WEIGHTED
ID	POINT	(SF)	(SF)	(AC)	'C'
DA1	1	21,065	13,360	0.79	0.78

PROPOSED

		AREA ROOF +	AREA LANDSCAPING		
WATERSHED	CONCENTRATION	C= 0.95	C= 0.50	TOTAL AREA	WEIGHTED
ID	POINT	(SF)	(SF)	(AC)	'C'
DA1	1	400	7,500	0.18	0.52
DA2	2	5,975	0	0.14	0.95
DA3	3	4,975	980	0.14	0.88
DA4	4	13,145	1,450	0.34	0.91

CONTRIBUTING AREA WEIGHTED C: 0.82 F

Precipitation Frequency Data Server



NOAA Atlas 14, Volume 1, Version 5 Location name: Scottsdale, Arizona, USA* Latitude: 33.4628°, Longitude: -111.9071° Elevation: 1212.06 ft** * source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) ¹									
Duration				Avera	ge recurren	ce interval (years)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	2.18 (1.84-2.65)	2.86 (2.41-3.47)	3.88 (3.25-4.70)	4.67 (3.89-5.63)	5.74 (4.70-6.88)	6.58 (5.32-7.84)	7.42 (5.89-8.82)	8.28 (6.47-9.84)	9.44 (7.18-11.2)	10.3 (7.69-12.3)
10-min	1.66 (1.40-2.02)	2.17 (1.84-2.64)	2.95 (2.48-3.58)	3.55 (2.96-4.28)	4.37 (3.58-5.24)	5.00 (4.04-5.97)	5.64 (4.48-6.71)	6.30 (4.92-7.49)	7.18 (5.46-8.54)	7.85 (5.86-9.36)
15-min	1.37	1.79	2.44	2.94	3.61	4.13	4.66	5.21	5.93	6.49
	(1.15-1.67)	(1.52-2.18)	(2.05-2.96)	(2.45-3.54)	(2.96-4.33)	(3.34-4.93)	(3.70-5.55)	(4.07-6.19)	(4.51-7.06)	(4.84-7.74)
30-min	0.924	1.21	1.64	1.98	2.43	2.78	3.14	3.51	4.00	4.37
	(0.776-1.12)	(1.02-1.47)	(1.38-1.99)	(1.65-2.38)	(1.99-2.91)	(2.25-3.32)	(2.49-3.73)	(2.74-4.17)	(3.04-4.75)	(3.26-5.21)
60-min	0.572	0.747	1.02	1.22	1.50	1.72	1.94	2.17	2.47	2.71
	(0.480-0.695)	(0.632-0.909)	(0.853-1.23)	(1.02-1.47)	(1.23-1.80)	(1.39-2.06)	(1.54-2.31)	(1.69-2.58)	(1.88-2.94)	(2.02-3.22)
2-hr	0.332	0.430	0.576	0.687	0.839	0.956	1.08	1.20	1.36	1.49
	(0.283-0.395)	(0.367-0.513)	(0.490-0.684)	(0.578-0.814)	(0.697-0.988)	(0.784-1.12)	(0.869-1.26)	(0.950-1.41)	(1.05-1.60)	(1.13-1.76)
3-hr	0.240 (0.204-0.288)	0.307 (0.263-0.371)	0.405 (0.343-0.485)	0.481 (0.405-0.574)	0.589 (0.488-0.699)	0.675 (0.551-0.798)	0.765 (0.613-0.903)	0.858 (0.677-1.01)	0.987 (0.755-1.17)	1.09 (0.815-1.29)
6-hr	0.144	0.183	0.235	0.277	0.333	0.377	0.424	0.471	0.536	0.587
	(0.125-0.170)	(0.160-0.215)	(0.204-0.276)	(0.238-0.322)	(0.282-0.386)	(0.315-0.436)	(0.348-0.489)	(0.379-0.545)	(0.421-0.621)	(0.450-0.683)
12-hr	0.080	0.102	0.129	0.150	0.179	0.201	0.224	0.247	0.278	0.303
	(0.071-0.093)	(0.089-0.118)	(0.113-0.149)	(0.130-0.173)	(0.154-0.206)	(0.171-0.231)	(0.187-0.257)	(0.204-0.284)	(0.224-0.321)	(0.239-0.352)
24-hr	0.048	0.061	0.079	0.093	0.113	0.129	0.145	0.162	0.185	0.203
	(0.043-0.054)	(0.055-0.068)	(0.071-0.088)	(0.084-0.104)	(0.101-0.126)	(0.114-0.143)	(0.127-0.161)	(0.141-0.180)	(0.159-0.206)	(0.173-0.227)
2-day	0.026	0.033	0.044	0.052	0.063	0.073	0.082	0.092	0.107	0.118
	(0.023-0.029)	(0.030-0.037)	(0.039-0.049)	(0.046-0.058)	(0.056-0.071)	(0.064-0.081)	(0.072-0.092)	(0.081-0.103)	(0.092-0.119)	(0.101-0.133)
3-day	0.018	0.023	0.031	0.037	0.045	0.052	0.059	0.066	0.077	0.085
	(0.016-0.020)	(0.021-0.026)	(0.028-0.034)	(0.033-0.041)	(0.040-0.050)	(0.046-0.058)	(0.052-0.066)	(0.058-0.074)	(0.066-0.086)	(0.073-0.096)
4-day	0.014	0.019	0.024	0.029	0.036	0.041	0.047	0.053	0.062	0.069
	(0.013-0.016)	(0.017-0.021)	(0.022-0.027)	(0.026-0.032)	(0.032-0.040)	(0.037-0.046)	(0.041-0.052)	(0.046-0.059)	(0.053-0.069)	(0.059-0.077)
7-day	0.009 (0.008-0.010)	0.012 (0.011-0.013)	0.015 (0.014-0.017)	0.018 (0.016-0.021)	0.023 (0.020-0.025)	0.026 (0.023-0.029)	0.030 (0.026-0.033)	0.034 (0.029-0.037)	0.039 (0.034-0.044)	0.044 (0.037-0.049)
10-day	0.007	0.009	0.012	0.014	0.017	0.020	0.023	0.025	0.029	0.033
	(0.006-0.008)	(0.008-0.010)	(0.011-0.013)	(0.013-0.016)	(0.015-0.019)	(0.018-0.022)	(0.020-0.025)	(0.022-0.028)	(0.025-0.033)	(0.028-0.036)
20-day	0.004 (0.004-0.005)	0.005 (0.005-0.006)	0.007 (0.007-0.008)	0.009 (0.008-0.010)	0.010 (0.009-0.011)	0.012 (0.010-0.013)	0.013 (0.012-0.015)	0.015 (0.013-0.016)	0.016 (0.014-0.018)	0.018 (0.016-0.020)
30-day	0.003 (0.003-0.004)	0.004 (0.004-0.005)	0.006 (0.005-0.006)	0.007 (0.006-0.007)	0.008 (0.007-0.009)	0.009 (0.008-0.010)	0.010 (0.009-0.011)	0.011 (0.010-0.012)	0.013 (0.011-0.014)	0.014 (0.012-0.015)
45-day	0.003 (0.002-0.003)	0.003 (0.003-0.004)	0.004 (0.004-0.005)	0.005 (0.005-0.006)	0.006 (0.006-0.007)	0.007 (0.006-0.008)	0.008 (0.007-0.009)	0.008 (0.008-0.009)	0.010 (0.008-0.011)	0.010 (0.009-0.011)
60-day	0.002	0.003	0.004	0.004	0.005	0.006	0.006	0.007	0.008	0.008

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

Back to Top

PF graphical



PROPOSED RATIONAL METHOD

PROJECT WESTERN TECH OFFICE

$$T_C = 11.4L^{0.5}K_b^{.52}S^{-0.31}i^{-0.38} \times 60$$

Tc= Time of Concentration (min)

L= Length of longest flow path (miles)

Kb= Watershed resistance coefficient

- S= Watercourse slope (ft/mi)
- i= rainfall intensity (in/hr)

Q = CiA

Q = Peak discharge (cfs)	100-YR, 5-MIN	7.42	in/hı
C = Runoff coefficient	100-YR, 10-MIN	5.64	in/hı
i = Rainfall intensity (inch/hr)	100-YR, 15-MIN	4.66	in/hı
A = Drainage area (Acres)	100-YR, 30-MIN	3.17	in/hı

WATERSHED ID	CONCENTRATION POINT	L	Kb		s	i	Тс	с	i	А	Q
DA 1	1	0.01	0.04	15	0.3%	7.42	2.5	0.52	7.42	0.18	0.7
DA 2	2	0.01	0.04	20	0.4%	7.42	2.3	0.95	7.42	0.14	1.0
DA 3	3	0.02	0.04	105	2.0%	7.42	1.9	0.88	7.42	0.14	1.0
DA 4	4	0.02	0.04	55	1.0%	7.42	2.4	0.91	7.42	0.34	2.3



INLET CALCULATION

PROJECT WESTERN TECH OFFICE LOCATION CP2



INLET TYPE Nyloplast - 2'x3' LENGTH 2 FΤ WIDTH 3 FT WEIR LENGTH* 5.7 FT **OPEN AREA*** 5.2 SF

3.0

50%

Cw Со 0.67

CLOGGING FACTOR

Weir Equation:				
$Q_i = C_w P d^{1.5}$				

SURVEY

SITE CONSULTING

SITE ENGINEERING

where:	Cw	=	Weir coefficient
	P	=	Perimeter of the grate, disregarding bars and side against curb, ft
	d	=	Depth of flow at curb, ft
Orifice Equ	uatio	ı:	
		0.5	

$$Q_i = C_o A_g (2gd)^0$$

= Orifice coefficient A_g

= Clear opening area of the grate, sq ft

Depth of flow at curb, ft d =

Gravity, 32.2 ft/sec² g =

DEPTH	WEIR	ORIFICE	CON	TROLLING
(FT)	(CFS)	(CFS)		(CFS)
0.00	0.0	0.0		0.0
0.05	0.1	3.1		0.1
0.10	0.3	4.4		0.3
0.15	0.5	5.4		0.5
0.20	0.8	6.2		0.8
0.25	1.1	6.9		1.1
0.30	1.4	7.6		1.4
0.35	1.8	8.2		1.8
0.40	2.2	8.8		2.2
0.45	2.6	9.3		2.6
0.50	3.0	9.8		3.0
0.55	3.5	10.3		3.5
0.60	4.0	10.8		4.0
0.65	4.5	11.2		4.5
0.70	5.0	11.6		5.0
0.75	5.6	12.0		5.6
0.80	6.1	12.4		6.1
0.85	6.7	12.8		6.7
1	00-YR FLOW	1.0	CFS	
CALCUL	ATED DEPTH	0.25	FT	

NOTE: * Open area and weir length are reduced by width of grate bars and transverse rods.



INLET CALCULATION

PROJECT WESTERN TECH OFFICE **LOCATION** CP3



INLET TYPE	Nyloplast - 2	2'x3'
LENGTH	2	FT
WIDTH	3	FT
WEIR LENGTH*	7.9	FT
OPEN AREA*	5.2	SF
Cw	3.0	
Со	0.67	
CLOGGING FACTOR	25%	

Weir Equation:

SURVEY

SITE CONSULTING

SITE ENGINEERING

$Q_i = C_w P d^{1.5}$			
where:	Cw	=	Weir coefficient
	P	=	Perimeter of the grate, disregarding bars and side against curb, ft
	d	-	Depth of flow at curb, ft
Orifice Eq	uatioı	ı:	
$Q_i = C_o A_g$	(2gd)	0.5	

where:

 C_o = Orifice coefficient

 A_g = Clear opening area of the grate, sq ft

d = Depth of flow at curb, ft

g = Gravity, 32.2 ft/sec²

DEPTH	WEIR	ORIFICE	CON	ITROLLING
(FT)	(CFS)	(CFS)		(CFS)
0.00	0.0	0.0		0.0
0.05	0.2	4.7		0.2
0.10	0.6	6.6		0.6
0.15	1.0	8.1		1.0
0.20	1.6	9.3		1.6
0.25	2.2	10.4		2.2
0.30	2.9	11.4		2.9
0.35	3.7	12.3		3.7
0.40	4.5	13.2		4.5
0.45	5.4	14.0		5.4
0.50	6.3	14.7		6.3
0.55	7.3	15.4		7.3
0.60	8.3	16.1		8.3
0.65	9.3	16.8		9.3
0.70	10.4	17.4		10.4
0.75	11.6	18.0		11.6
0.80	12.7	18.6		12.7
0.85	14.0	19.2		14.0
1		1.0		
		1.U 0.15	сгэ ст	
CALCUL		0.12	r I	

NOTE: * Open area and weir length are reduced by width of grate bars and transverse rods.



INLET CALCULATION

PROJECT WESTERN TECH OFFICE **LOCATION** CP4



INLET TYPE	Nyloplast - 2	2'x3'
LENGTH	2	FT
WIDTH	3	FT
WEIR LENGTH*	7.9	FT
OPEN AREA*	5.2	SF
Cw	3.0	
Со	0.67	
CLOGGING FACTOR	25%	

Weir Equation:

where:

SURVEY

SITE CONSULTING

SITE ENGINEERING

$Q_i = C_w P d^{1.5}$				
where:	Cw	=	Weir coefficient	
	P	=	Perimeter of the grate, disregarding bars and side against curb, ft	
	d	=	Depth of flow at curb, ft	
Orifice Equ	uatio	ı:		
$Q_i = C_o A_g$	(2gd)	0.5		

Co	=	Orifice coefficient
A_g	=	Clear opening area of the grate, sq ft
d	=	Depth of flow at curb. ft

d = Depth of flow at curb, ft g = Gravity, 32.2 ft/sec²

DEPTH	WEIR	ORIFICE	CON	ITROLLING
(FT)	(CFS)	(CFS)		(CFS)
0.00	0.0	0.0		0.0
0.05	0.2	4.7		0.2
0.10	0.6	6.6		0.6
0.15	1.0	8.1		1.0
0.20	1.6	9.3		1.6
0.25	2.2	10.4		2.2
0.30	2.9	11.4		2.9
0.35	3.7	12.3		3.7
0.40	4.5	13.2		4.5
0.45	5.4	14.0		5.4
0.50	6.3	14.7		6.3
0.55	7.3	15.4		7.3
0.60	8.3	16.1		8.3
0.65	9.3	16.8		9.3
0.70	10.4	17.4		10.4
0.75	11.6	18.0		11.6
0.80	12.7	18.6		12.7
0.85	14.0	19.2		14.0
1	00-YR FLOW	2.3	CFS	
CALCUL	ATED DEPTH	0.30	FT	

NOTE: * Open area and weir length are reduced by width of grate bars and transverse rods.

APPENDIX D (Retention Calculations) Precipitation Frequency Data Server

NOAA Atlas 14, Volume 1, Version 5 Location name: Scottsdale, Arizona, USA* Latitude: 33.4628°, Longitude: -111.9071° Elevation: 1212.06 ft** * source: ESRI Maps



* source: ESRI Maps ** source: USGS POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PD	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹									es) ¹
Duration		Average recurrence interval (years)								
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.182 (0.153-0.221)	0.238 (0.201-0.289)	0.323 (0.271-0.392)	0.389 (0.324-0.469)	0.478 (0.392-0.573)	0.548 (0.443-0.653)	0.618 (0.491-0.735)	0.690 (0.539-0.820)	0.787 (0.598-0.935)	0.860 (0.641-1.02)
10-min	0.276 (0.233-0.337)	0.362 (0.306-0.440)	0.492 (0.413-0.596)	0.592 (0.494-0.714)	0.728 (0.597-0.873)	0.833 (0.674-0.995)	0.940 (0.746-1.12)	1.05 (0.820-1.25)	1.20 (0.910-1.42)	1.31 (0.976-1.56)
15-min	0.343 (0.288-0.417)	0.448 (0.379-0.545)	0.610 (0.512-0.739)	0.734 (0.612-0.884)	0.902 (0.740-1.08)	1.03 (0.836-1.23)	1.17 (0.925-1.39)	1.30 (1.02-1.55)	1.48 (1.13-1.77)	1.62 (1.21-1.93)
30-min	0.462 (0.388-0.562)	0.604 (0.510-0.734)	0.822 (0.689-0.995)	0.989 (0.824-1.19)	1.22 (0.996-1.46)	1.39 (1.13-1.66)	1.57 (1.25-1.87)	1.75 (1.37-2.08)	2.00 (1.52-2.38)	2.19 (1.63-2.61)
60-min	0.572 (0.480-0.695)	0.747 (0.632-0.909)	1.02 (0.853-1.23)	1.22 (1.02-1.47)	1.50 (1.23-1.80)	1.72 (1.39-2.06)	1.94 (1.54-2.31)	2.17 (1.69-2.58)	2.47 (1.88-2.94)	2.71 (2.02-3.22)
2-hr	0.663 (0.566-0.790)	0.859 (0.734-1.03)	1.15 (0.979-1.37)	1.37 (1.16-1.63)	1.68 (1.39-1.98)	1.91 (1.57-2.25)	2.15 (1.74-2.53)	2.40 (1.90-2.81)	2.73 (2.11-3.21)	2.99 (2.26-3.53)
3-hr	0.720 (0.612-0.864)	0.923 (0.789-1.11)	1.22 (1.03-1.46)	1.45 (1.22-1.73)	1.77 (1.47-2.10)	2.03 (1.66-2.40)	2.30 (1.84-2.71)	2.58 (2.03-3.04)	2.96 (2.27-3.50)	3.28 (2.45-3.89)
6-hr	0.865 (0.751-1.02)	1.10 (0.956-1.29)	1.41 (1.22-1.65)	1.66 (1.42-1.93)	1.99 (1.69-2.31)	2.26 (1.89-2.61)	2.54 (2.08-2.93)	2.82 (2.27-3.26)	3.21 (2.52-3.72)	3.51 (2.69-4.09)
12-hr	0.969 (0.850-1.12)	1.23 (1.07-1.42)	1.55 (1.36-1.79)	1.81 (1.57-2.08)	2.16 (1.85-2.48)	2.43 (2.06-2.78)	2.70 (2.26-3.10)	2.98 (2.45-3.42)	3.35 (2.69-3.87)	3.65 (2.88-4.24)
24-hr	1.15 (1.04-1.29)	1.46 (1.32-1.64)	1.90 (1.70-2.12)	2.24 (2.00-2.50)	2.72 (2.41-3.02)	3.09 (2.73-3.43)	3.48 (3.05-3.86)	3.88 (3.38-4.31)	4.44 (3.82-4.94)	4.88 (4.16-5.44)
2-day	1.25 (1.12-1.40)	1.60 (1.44-1.79)	2.09 (1.88-2.34)	2.49 (2.23-2.78)	3.05 (2.71-3.40)	3.49 (3.08-3.89)	3.96 (3.47-4.42)	4.44 (3.87-4.96)	5.12 (4.41-5.73)	5.66 (4.83-6.37)
3-day	1.32 (1.19-1.47)	1.69 (1.52-1.89)	2.22 (1.99-2.47)	2.65 (2.36-2.95)	3.25 (2.89-3.61)	3.73 (3.29-4.15)	4.24 (3.72-4.72)	4.78 (4.16-5.33)	5.54 (4.76-6.18)	6.15 (5.23-6.88)
4-day	1.39 (1.25-1.55)	1.78 (1.60-1.99)	2.34 (2.10-2.61)	2.80 (2.50-3.11)	3.45 (3.06-3.83)	3.97 (3.51-4.41)	4.53 (3.97-5.03)	5.12 (4.45-5.69)	5.95 (5.11-6.63)	6.63 (5.63-7.40)
7-day	1.53 (1.38-1.71)	1.96 (1.77-2.19)	2.59 (2.32-2.89)	3.09 (2.77-3.45)	3.81 (3.39-4.24)	4.39 (3.88-4.87)	5.00 (4.39-5.56)	5.65 (4.92-6.29)	6.57 (5.64-7.32)	7.31 (6.21-8.16)
10-day	1.67 (1.50-1.86)	2.14 (1.92-2.38)	2.82 (2.53-3.13)	3.37 (3.01-3.74)	4.13 (3.68-4.58)	4.75 (4.20-5.26)	5.40 (4.75-5.99)	6.09 (5.31-6.76)	7.05 (6.07-7.83)	7.82 (6.67-8.70)
20-day	2.05 (1.85-2.28)	2.64 (2.38-2.93)	3.48 (3.13-3.86)	4.12 (3.70-4.57)	4.98 (4.45-5.52)	5.64 (5.02-6.25)	6.31 (5.59-7.00)	6.99 (6.16-7.76)	7.91 (6.90-8.80)	8.61 (7.46-9.60)
30-day	2.39 (2.16-2.66)	3.08 (2.78-3.42)	4.06 (3.65-4.49)	4.80 (4.31-5.31)	5.80 (5.19-6.41)	6.56 (5.85-7.24)	7.35 (6.52-8.11)	8.14 (7.18-9.00)	9.22 (8.07-10.2)	10.0 (8.71-11.1)
45-day	2.78 (2.51-3.08)	3.58 (3.24-3.97)	4.72 (4.26-5.23)	5.56 (5.01-6.15)	6.67 (5.98-7.37)	7.50 (6.71-8.30)	8.34 (7.43-9.23)	9.18 (8.13-10.2)	10.3 (9.04-11.4)	11.1 (9.71-12.4)
60-day	3.08 (2.79-3.41)	3.98 (3.60-4.40)	5.23 (4.73-5.78)	6.14 (5.53-6.78)	7.33 (6.59-8.08)	8.20 (7.35-9.05)	9.08 (8.10-10.0)	9.94 (8.83-11.0)	11.1 (9.77-12.3)	11.9 (10.4-13.2)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

Back to Top

PF graphical



PROJECT RETENTION REQUIREMENTS

REQUIRED RETENTION (100 – *YEAR*, 2 – *HOUR PRE VS POST*)

EXISTING RETENTION VOLUME = 9,550 CF

PRE VS POST (DISTURBED AREA) Cw (PRE) = (0.50 x 13,360 SF) + (0.95 x 21,065 SF) / 34,425 SF Cw (PRE) = 0.78

Cw (POST) = (0.50 x 9,930 SF) + (0.95 x 24,495 SF) / 34,425 SFCw (POST) = 0.82

V = Cw x (d/12) x A V (PRE) = 0.78 x (2.15/12) x 34,425 = 4,811 CFV (POST) = 0.82 x (2.15/12) x 34,425 = 5,058 CF

PRE VS POST VOLUME = 5,058 - 4,811 = 247 CF

TOTAL REQUIRED VOLUME = 9,550 + 247 = 9,797 CF

PROVIDED: SURFACE BASIN = 5,600 CF STORMTECH CHAMBERS = 4,470 CF -24 MC-3500 CHAMBERS @ 175.0 CF/CHAMBER = 4,200 CF -6 END CAPS @ 45.1 CF PER END CAP = 270.6 CF (WITH 9" BASE STONE)TOTAL = 10,070 CF



SITE ENGINEERING

BASIN VOLUME CALCULATIONS

EXISTING BASIN RETENTION

		CUMULATIVE
ELEVATION	AREA (SF)	VOLUME (CF)
1215.6	528	
1216	1468	399.2
1217	3,057	2262.5
1218	4,927	3992.0
1218.5	6,667	2898.5

VOLUME PROVIDED:

9552.2

PROPOSED BASIN RETENTION

		CUMULATIVE
ELEVATION	AREA (SF)	VOLUME (CF)
1215.5	919	
1216.5	1,502	1210.5
1217.5	2,187	1844.5
1218.5	2,971	2579.0
	5624.0	

VOLUME PROVIDED:

5634.0