



114<sup>th</sup> & Shea Retail Preliminary Drainage Report 3 engineering Job #: 1831 April 21, 2022





### 114<sup>™</sup> & SHEA RETAIL

### SWC 114<sup>TH</sup> STREET & SHEA BLVD, SCOTTSDALE, AZ 85259

### PRELIMINARY DRAINAGE REPORT

Prepared for:

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April 21, 2022

### Submittal to:

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Job Number 1831



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#### 1. Introduction

The purpose of this report is to present the existing and proposed drainage plan for the project site, 114<sup>th</sup> & Shea Retail. It is our opinion the proposed grading and drainage concept is in accordance with the City of Scottsdale drainage requirements.

The project site, 114<sup>th</sup> & Shea Retail, is located in the northwest guarter of Section 27, Township 3 North, Range 5 East of the Gila and Salt River Meridian, Maricopa County, Arizona within the City of Scottsdale. The project is located at the southwest corner of 114<sup>th</sup> Street and Shea Boulevard, Scottsdale, AZ 85259. The site is bounded on the north by Shea Boulevard, on the east by 114th Street, on the south by Beryl Avenue, and on the west by single family residential homes. See Appendix A for a vicinity map and Appendix G for offsite aerial photographs.

The existing zoning is C-O and R1-18. The site is currently undeveloped. The proposed zoning is C1, SR, and R1-18. The City of Scottsdale 2001 General Plan shows the site land use is Office and Rural Neighborhood. Additionally, the site is located within the Shea Corridor. The proposed site consists of two portions that are separated by an existing wash that runs west through the site. The northern portion is a mixed-use office, medical office, and retail center with three pads. Access is provided by 114<sup>th</sup> Street. The southern portion consists of one residential lot with access provided by Beryl Avenue. The residential lot will be developed at a future date. See Appendix H for an onsite map. The site currently lies within the "Zone X-shaded" floodplain designation.

#### 2. <u>Site Description</u>

#### Existing

The project site currently consists of undeveloped land. The existing topography of the site generally slopes from northeast to southwest at approximately 1.5 percent (1.5%). There is a wash that runs east to west through the site. The site currently does not retain stormwater and discharges to the west property line within the wash. The site does not show any signs of containing waters of the US (404 washes). See Appendix F for an aerial photograph of the site.

#### Federal Emergency Management Agency (FEMA) Designation

According to FEMA Flood Insurance Rate Map (FIRM) # 04013C1780L, dated October 16, 2013, the site is located within the "Zone X – shaded" floodplain designation.

"Zone X – Shaded" is described as follows:

"Areas of 0.2% annual chance of flood; areas of 1% annual chance of flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood."

Refer to the updated Flood Insurance Rate Map information in Appendix B.

#### Proposed

The proposed project includes constructing a retail, office and medical office center with three buildings. The site also includes drive aisles, surface parking, landscape, and retention. Additionally, the site includes a residential lot with final grading being done when a house is developed on the lot. See the proposed improvements in the Onsite Drainage Area Map in Appendix H.

#### 3. Drainage Design - Offsite

The site is affected by offsite flows. There is an existing 3 - 36" R.G.R.C.P Culvert under 114th Street, that outlets directly to the wash at the east property line. As-built information for the culvert is shown in the Paving plans for Mirage Crossing, located in Appendix K.



The plans show a 100-year flow of 152 cfs being conveyed through the culvert. The existing wash was analyzed at two cross-sections within the site. Cross section locations are shown in the Onsite Drainage Area Map, Appendix H. The flow was found to be contained within the top banks of the wash at both upstream and downstream locations, with respective water surface elevations of 1437.62 and 1435.92. The proposed buildings have a lowest finish floor of 1441.00, which exceeds the requirement to be 12" above the water surface elevation. See Appendix E for Flowmaster Results.

There are no other offsite flows that affect the site. Shea Boulevard and 114<sup>th</sup> Street are both fully developed with curb & gutter. Flows in Shea Boulevard travel west to a curb opening catch basin west of the site. Flows in 114<sup>th</sup> Street travel south to a curb opening catch basin south of the site. There is an existing block wall along the south and west property lines that protects the site from offsite flows.

Refer to Appendix D for a Watershed Delineation and Topography map.

#### Hydraulic Parameters

Bentley Flowmaster V8i was used to calculate the water surface elevations in the existing wash through the site. See results in Appendix E.

#### 4. Drainage Design - Onsite

The City of Scottsdale Design Standards and Policies Manual and the Drainage Design Manual for Maricopa County, Volume 1 was followed in designing on-site drainage facilities for the site.

Refer to the Preliminary Grading and Drainage Plan in Appendix L, the Onsite Drainage Map in Appendix H and Inlet Area Exhibit in Appendix I for the following discussion. The proposed commercial site provides retention for the 100-year, 2-hour storm event, with a weighted runoff coefficient. See Appendix J for Onsite Drainage Calculations.

The required retention volume for Drainage Area A is 5,780 c.f. There is 6,234 c.f. provided in a proposed surface retention basin. Flows generated within this drainage area are conveyed via surface flow and curb openings to the retention basin. The basin outlets to retention basin B via a bleed off pipe with a 2" orifice plate will provide a dry up time within 36 hours.

The required retention volume for Drainage Area B is 8,031 c.f. There is 10,110 c.f. provided in a proposed surface retention basin. Flows generated within this drainage area are conveyed via surface flow and curb openings to the retention basin. The basin outlets to the existing wash via a bleed off pipe with a 2" orifice plate will provide a dry up time within 36 hours including the volumes from the other two basins.

The required retention volume for Drainage Area C is 1,758 c.f. There is 2,050 c.f. provided in a proposed surface retention basin. Flows generated within this drainage area are conveyed via surface flow to the retention basin. The basin outlets to a swale leading to retention basin B via a bleed off pipe with a 2" orifice plate will provide a dry up time within 36 hours.

The required retention volume for Drainage Area D (The residential lot) is 6,251 c.f. This volume will be required to be retained on the lot. This volume will be non-waivable and will be required with the house plans. The residential lot will not be graded at this time.



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The lowest finish floor of the proposed buildings is 1441.00, which is 12" above the highest proposed retention basin high water of 1439.50. Therefore, the finish floor meets the requirement of 12" above the adjacent high-water elevation per the Maricopa County Drainage Standards. Additionally, the proposed finish floor elevations are greater than 14" above the site outfall of 1432.80, exceeding the Maricopa County Drainage Standard requirement.

#### Hydraulic Parameters

For onsite peak flows, the Rational Method will be used as follows:

O=CiA

where:

C = Composite runoff coefficient = weighted by ground cover

i = Intensity corresponding to T<sub>c</sub>

 $T_c$  = Time of concentration (10 minute minimum used)

A = Area in acres

The 100-year runoff coefficient for this development to be used is 0.95 for impervious areas and 0.45 for pervious areas per the Maricopa County Drainage Policies and Standards. The rainfall is based upon the NOAA Atlas 14, Volume 1, Version 5, dated 2011, 90% confidence interval, mean partial duration time series data.

Bentley Flowmaster V8i was used to calculate the bleed off rates of the orifice openings for the retention basins. The dry up time was calculated by dividing the volume of the retention basin by the bleed off rate to ensure a time less than 36 hours. See results in Appendix J.

Determination of Curb Opening capacity operating as a weir by using the following formula:

 $Q = C_w x L x d^{1.5} \rightarrow L = (Q / (C_w x d^{1.5}) x 1.20)$ 

where:

 $C_W$  = Weir coefficient (3.0) L = Length of Openingd = Depth at the lip of the weir CF = Clogging Factor = 0.20 -> 1.20xL (20% clogging)

#### 5. <u>Conclusions</u>

The following is a summary of the 114<sup>th</sup> & Shea Retail Preliminary Drainage Report.

- The site currently lies within the "Zone X Shaded" floodplain designation. •
- Retention is provided for the 100-year, 2-hour storm event for the commercial site. •
- Retention will be provided for the residential lot with the house plans.
- The site is affected by offsite flows.
- The finish floors are safe from the 100-year storm event. •

#### 6. <u>References</u>

- 1. City of Scottsdale, Design Standards and Policies Manual, 2018.
- 2. Maricopa County, Drainage Policies and Standards, August 2018.
- 3. Mirage Crossing Paving Plans, C.O.S. As-built no. 41625

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# APPENDIX A

# Vicinity Map



# VICINITY MAP

N.T.S.

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# APPENDIX B

# FEMA FIRM Map



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# APPENDIX C

### Warning and Disclaimer of Liability

### GRADING & DRAINAGE GRADING & DRAINAGE LANGUAGE

#### WARNING AND DISCLAIMER OF LIABILITY

The City's Stormwater and Floodplain Management Ordinance is intended to minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding. The Stormwater and Floodplain Management Ordinance identifies floodplains, floodways, flood fringes and special flood hazard areas. However, a property outside these areas could be inundated by floods. Also, much of the city is a dynamic flood area; floodways, floodplains, flood fringes and special flood hazard areas may shift from one location to another, over time, due to natural processes.

WARNING AND DISCLAIMER OF LIABILITY

The flood protection provided by the Stormwater and Floodplain Management Ordinance is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by constructed or natural causes. The Stormwater and Floodplain Management Ordinance does not create liability on the part of the city, any officer or employee thereof, or the federal, state or county government for any flood damages that result from reliance on the Ordinance or any administrative decision lawfully made thereunder.

Compliance with the Stormwater and Floodplain Management Ordinance does not ensure complete protection from flooding. Flood-related problems such as natural erosion, streambed meander, or constructed obstructions and diversions may occur and have an adverse effect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations. I have read and understand the above.

> MAAAA Owner

Plan Check #

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# APPENDIX D

# Watershed Delineation and Topography Map



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# APPENDIX E

# Offsite Drainage Calculations

### **Cross Section for Wash Section A**

Project Description		
Friction Method Solve For	Manning Formula Normal Depth	
Input Data		
Channel Slope	1.25000	%
Normal Depth	2.41	ft
Discharge	152.00	ft³/s

#### **Cross Section Image**



### Worksheet for Wash Section A

Project Description			
Friction Method Solve For	Manning Formula Normal Depth		
Input Data			
Channel Slope	1.25	5000	%
Discharge	153	2.00	ft³/s

Section Definitions

Station (ft)	Elevation (ft)
0+00	) 38.50
0+21	38.20
0+24	38.00
0+30	) 37.00
0+37	36.00
0+41	35.21
0+45	36.00
0+50	) 37.00
0+52	2 37.40
0+69	38.00
0+77	38.20
0+86	38.00
0+88	37.95
0+90	38.00
0+94	4 39.00
0+98	3 40.00

**Roughness Segment Definitions** 

Start Station	1	Ending Station	Roughness Coefficient
Otari Otation			Roughness Oberneient
(0+00, 3	8.50)	(0+98, 40.00)	0.035
Options			
Current Roughness Weighted Method	Pavlovskii's Methoo	d	
Open Channel Weighting Method	Pavlovskii's Methoo	d	

Bentley Systems, Inc. Haestad Methods SoBdtittle@EttarMaster V8i (SELECTseries 1) [08.11.01.03]4/21/2020 1:32:59 PM27 Siemons Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666Page 1 of 2

### Worksheet for Wash Section A

#### Options

Closed Channel Weighting Method Pavlovskii's Method

Regulte			
Neoullo			
Normal Depth		2.41	ft
Elevation Range	35.21 to 40.00 ft		
Flow Area		32.12	ft²
Wetted Perimeter		32.27	ft
Hydraulic Radius		1.00	ft
Top Width		31.86	ft
Normal Depth		2.41	ft
Critical Depth		2.19	ft
Critical Slope		0.01784	ft/ft
Velocity		4.73	ft/s
Velocity Head		0.35	ft
Specific Energy		2.76	ft
Froude Number		0.83	
Flow Type	Subcritical		
GVF Input Data			
Downstream Depth		0.00	ft
Length		0.00	ft
Number Of Steps		0	
GVF Output Data			
Upstream Depth		0.00	ft
Profile Description			
Profile Headloss		0.00	ft
Downstream Velocity		Infinity	ft/s
Upstream Velocity		Infinity	ft/s
Normal Depth		2.41	ft
Critical Depth		2.19	ft
Channel Slope		1.25000	%
Critical Slope		0.01784	ft/ft

### **Cross Section for Wash Section B**

Project Description		
Friction Method Solve For	Manning Formula Normal Depth	
Input Data		
Channel Slope	1.39000	%
Normal Depth	2.44	ft
Discharge	152.00	ft³/s

#### **Cross Section Image**



### Worksheet for Wash Section B

Project Description		
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Channel Slope	1.39000	%
Discharge	152.00	ft³/s

Section Definitions

Station (ft)	Elevation (ft)
0+	00 36.20
0+	25 36.20
0+	28 36.00
0+	34 35.00
0+	40 34.00
0+	42 33.52
0+	44 34.00
0+	47 35.00
0+	52 36.00
0+	55 36.30
0+	75 36.50

#### **Roughness Segment Definitions**

Start Station	Ending St	tation	Roughness Coefficient	
(0+00, 3	5.20)	(0+75, 36.50)	0.0	)35
Options				
Current Roughness Weighted Method Open Channel Weighting Method	Pavlovskii's Method Pavlovskii's Method			
Closed Channel Weighting Method	Pavlovskii's Method			
Results				
Normal Depth		2.44 ft		
Elevation Range	33.52 to 36.50 ft			

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<u> </u>	Norksheet for Wash S	ection B
Results		
Flow Area	27.89	ft²
Wetted Perimeter	24.56	ft
Hydraulic Radius	1.14	ft
Top Width	24.05	ft
Normal Depth	2.44	ft
Critical Depth	2.34	ft
Critical Slope	0.01773	ft/ft
Velocity	5.45	ft/s
Velocity Head	0.46	ft
Specific Energy	2.90	ft
Froude Number	0.89	
Flow Type	Subcritical	
GVF Input Data		
Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	2.44	ft
Critical Depth	2.34	ft
Channel Slope	1.39000	%
Critical Slope	0.01773	ft/ft

### **Cross Section for Proposed Wash**

Project Description		
Friction Method Solve For	Manning Formula Normal Depth	
Input Data		
Roughness Coefficient	0.035	
Channel Slope	0.70	%
Normal Depth	1.97	ft
Left Side Slope	4.00	ft/ft (H:V)
Right Side Slope	4.00	ft/ft (H:V)
Bottom Width	10.00	ft
Discharge	152.00	ft³/s

**Cross Section Image** 



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	Worksheet for Propos	ed Wash
Project Description		
Friction Method	Manning Formula	
Solve For	Normal Depth	
Input Data		
Roughness Coefficient	0.035	
Channel Slope	0.70	%
Left Side Slope	4.00	ft/ft (H:V)
Right Side Slope	4.00	ft/ft (H:V)
Bottom Width	10.00	ft
Discharge	152.00	ft³/s
Results		
Normal Depth	1.97	ft
Flow Area	35.18	ft²
Wetted Perimeter	26.23	ft
Hydraulic Radius	1.34	ft
Top Width	25.75	ft
Critical Depth	1.56	ft
Critical Slope	0.01755	ft/ft
Velocity	4.32	ft/s
Velocity Head	0.29	ft
Specific Energy	2.26	ft
Froude Number	0.65	
Flow Type	Subcritical	
GVF Input Data		
Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	
GVF Output Data		
Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	1.97	ft
Critical Depth	1.56	ft
Channel Slope	0.70	%

Bentley Systems, Inc. Haestad Methods SoBatitite CEnterMaster V8i (SELECTseries 1) [08.11.01.03]

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### Worksheet for Proposed Wash

#### GVF Output Data

Critical Slope

0.01755 ft/ft

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# APPENDIX F

# Aerial Site Photographs



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# APPENDIX G

# Offsite Aerial Photographs



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# APPENDIX H

### Onsite Drainage Area Map



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# APPENDIX I

# Inlet Area Exhibit



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### APPENDIX J

## **Onsite Drainage Calculations**

#### Post-Development Rational Method Calculations

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Sub-Area	Area	<b>C</b> 10	<b>C</b> 100	Тс	<b>İ</b> 10	<b>İ</b> 100	<b>Q</b> 10	Q100
	(acre)	(weighted)	(weighted)	(min)	(in/hr)	(in/hr)	(cfs)	(cfs)
A-1	0.29	0.90	0.95	10	3.79	5.96	1.01	1.67
B-1	0.27	0.90	0.95	10	3.79	5.96	0.93	1.54
B-2	0.20	0.90	0.95	10	3.79	5.96	0.70	1.16
B-3	0.15	0.90	0.95	10	3.79	5.96	0.50	0.83
B-4	0.15	0.90	0.95	10	3.79	5.96	0.50	0.82
B-5	0.07	0.90	0.95	10	3.79	5.96	0.23	0.38

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#### On-Site Retention for the 100-year, 2-hour Storm

						U.G.	
				Total	Surface	Retention	Total
Sub-Area	Area	<b>C</b> 100	P100yr, 2hr	Vol. Req.	Vol. Prov.	Provided	Vol. Prov.
	(acre)		(in)	(cf)	(cf)	(cf)	(cf)
A	1.02	0.68	2.30	5,780	6,234	-	6,234
В	1.24	0.78	2.30	8,031	10,110	-	10,110
С	0.28	0.74	2.30	1,758	2,050	-	2,050
D	1.17	0.64	2.30	6,251	*	-	*

\* Volume is required to be provided onsite. To be shown on final grading plan for lot.

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		Building &				
	Overall	Parking Area	Landscape	Overall		
	Area (s.f.)	(s.f.)	Area (s.f.)	Area (Ac.)	<b>C</b> 10	<b>C</b> 100
A	44556	20,210	24,346	1.02	0.63	0.68
В	54059	35,146	18,913	1.24	0.73	0.78
С	12339	7,237	5,102	0.28	0.69	0.74

C=((Building & Hardscape Area x 0.95) + (Landscape Area x 0.45)) / Overall Area

C<sub>100</sub> Building & Hardscape = 0.95

C<sub>100</sub> Landscape = 0.45

114th Shea Retail 11/3/2021

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### **Curb Opening - Weir Condition**

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			Inlet Capacity			
			W/ 20%			
Inlet Type	Inlet	<b>Q</b> 100	Clogging	d	Cw	L
		(cfs)	(cfs)	(ft)		(ft)
2.5' Curb Opening	A-1	1.67	1.70	0.42	3	2.5
2.0' Curb Opening	B-1	1.54	1.77	0.50	3	2
2.0' Curb Opening	B-2	1.16	1.77	0.50	3	2
2.0' Curb Opening	B-3	0.83	1.77	0.50	3	2
2.0' Curb Opening	B-4	0.82	1.77	0.50	3	2
2.0' Curb Opening	B-5	0.38	1.77	0.50	3	2

Q=Cw\*L\*d^1.5

Cw= 3.0 weir coefficient

L=(Q/(Cw\*d^1.5))\*1.20

Q = discharge capacityL = curb opening length

d = flow depth

CF = clogging factor = 20% (1.20xL)



NOAA Atlas 14, Volume 1, Version 5 Location name: Scottsdale, Arizona, USA\* Latitude: 33.5816°, Longitude: -111.8357° Elevation: 1440.71 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

#### **PF** tabular

PDS	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) <sup>1</sup>									
Duration				Averaç	ge recurrend	ce interval (y	/ears)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.196</b> (0.163-0.243)	<b>0.257</b> (0.214-0.317)	<b>0.346</b> (0.286-0.427)	<b>0.416</b> (0.341-0.510)	<b>0.508</b> (0.410-0.621)	<b>0.579</b> (0.462-0.703)	<b>0.652</b> (0.511-0.790)	<b>0.725</b> (0.559-0.877)	<b>0.823</b> (0.619-0.996)	<b>0.898</b> (0.661-1.09)
10-min	<b>0.299</b> (0.247-0.369)	<b>0.390</b> (0.325-0.482)	<b>0.527</b> (0.435-0.649)	<b>0.632</b> (0.518-0.776)	<b>0.773</b> (0.625-0.945)	<b>0.882</b> (0.703-1.07)	<b>0.993</b> (0.778-1.20)	<b>1.10</b> (0.851-1.33)	<b>1.25</b> (0.942-1.52)	<b>1.37</b> (1.01-1.66)
15-min	<b>0.370</b> (0.307-0.457)	<b>0.483</b> (0.403-0.598)	<b>0.654</b> (0.539-0.805)	<b>0.784</b> (0.643-0.961)	<b>0.959</b> (0.774-1.17)	<b>1.09</b> (0.872-1.33)	<b>1.23</b> (0.964-1.49)	<b>1.37</b> (1.06-1.65)	<b>1.55</b> (1.17-1.88)	<b>1.70</b> (1.25-2.05)
30-min	<b>0.498</b> (0.413-0.615)	<b>0.652</b> (0.543-0.805)	<b>0.880</b> (0.726-1.08)	<b>1.06</b> (0.865-1.30)	<b>1.29</b> (1.04-1.58)	<b>1.47</b> (1.17-1.79)	<b>1.66</b> (1.30-2.01)	<b>1.84</b> (1.42-2.23)	<b>2.09</b> (1.57-2.53)	<b>2.28</b> (1.68-2.77)
60-min	<b>0.617</b> (0.511-0.762)	<b>0.806</b> (0.672-0.996)	<b>1.09</b> (0.899-1.34)	<b>1.31</b> (1.07-1.60)	<b>1.60</b> (1.29-1.95)	<b>1.82</b> (1.45-2.21)	<b>2.05</b> (1.61-2.48)	<b>2.28</b> (1.76-2.76)	<b>2.59</b> (1.95-3.13)	<b>2.82</b> (2.08-3.42)
2-hr	<b>0.721</b> (0.605-0.868)	<b>0.933</b> (0.785-1.13)	<b>1.24</b> (1.04-1.50)	<b>1.48</b> (1.23-1.78)	<b>1.80</b> (1.48-2.15)	<b>2.05</b> (1.65-2.44)	<b>2.30</b> (1.83-2.74)	<b>2.55</b> (1.99-3.04)	<b>2.89</b> (2.21-3.44)	<b>3.16</b> (2.36-3.78)
3-hr	<b>0.788</b> (0.661-0.967)	<b>1.01</b> (0.850-1.24)	<b>1.32</b> (1.11-1.62)	<b>1.57</b> (1.30-1.91)	<b>1.91</b> (1.56-2.32)	<b>2.19</b> (1.76-2.64)	<b>2.47</b> (1.95-2.98)	<b>2.77</b> (2.15-3.33)	<b>3.17</b> (2.39-3.81)	<b>3.50</b> (2.58-4.21)
6-hr	<b>0.947</b> (0.813-1.13)	<b>1.20</b> (1.03-1.42)	<b>1.53</b> (1.31-1.81)	<b>1.79</b> (1.52-2.11)	<b>2.15</b> (1.80-2.53)	<b>2.43</b> (2.00-2.84)	<b>2.72</b> (2.21-3.18)	<b>3.02</b> (2.40-3.54)	<b>3.42</b> (2.66-4.01)	<b>3.74</b> (2.84-4.39)
12-hr	<b>1.07</b> (0.925-1.25)	<b>1.35</b> (1.17-1.58)	<b>1.70</b> (1.47-1.98)	<b>1.98</b> (1.69-2.30)	<b>2.35</b> (1.99-2.73)	<b>2.64</b> (2.21-3.06)	<b>2.94</b> (2.42-3.40)	<b>3.24</b> (2.64-3.75)	<b>3.64</b> (2.89-4.23)	<b>3.95</b> (3.08-4.62)
24-hr	<b>1.26</b> (1.12-1.44)	<b>1.60</b> (1.43-1.84)	<b>2.08</b> (1.84-2.37)	<b>2.45</b> (2.16-2.79)	<b>2.97</b> (2.60-3.38)	<b>3.38</b> (2.94-3.84)	<b>3.81</b> (3.28-4.33)	<b>4.25</b> (3.63-4.83)	<b>4.86</b> (4.10-5.53)	<b>5.35</b> (4.45-6.10)
2-day	<b>1.38</b> (1.22-1.58)	<b>1.77</b> (1.56-2.02)	<b>2.32</b> (2.05-2.65)	<b>2.76</b> (2.43-3.15)	<b>3.38</b> (2.95-3.84)	<b>3.87</b> (3.35-4.40)	<b>4.39</b> (3.77-5.00)	<b>4.93</b> (4.21-5.62)	<b>5.69</b> (4.78-6.50)	<b>6.30</b> (5.23-7.22)
3-day	<b>1.49</b> (1.32-1.69)	<b>1.91</b> (1.69-2.17)	<b>2.52</b> (2.22-2.86)	<b>3.01</b> (2.64-3.41)	<b>3.70</b> (3.23-4.19)	<b>4.26</b> (3.70-4.82)	<b>4.85</b> (4.18-5.50)	<b>5.48</b> (4.68-6.22)	<b>6.37</b> (5.36-7.24)	<b>7.08</b> (5.90-8.08)
4-day	<b>1.59</b> (1.41-1.81)	<b>2.04</b> (1.81-2.32)	<b>2.71</b> (2.39-3.07)	<b>3.25</b> (2.86-3.67)	<b>4.02</b> (3.52-4.54)	<b>4.64</b> (4.04-5.24)	<b>5.31</b> (4.59-6.00)	<b>6.03</b> (5.15-6.82)	<b>7.04</b> (5.93-7.97)	<b>7.86</b> (6.56-8.94)
7-day	<b>1.79</b> (1.58-2.04)	<b>2.29</b> (2.02-2.61)	<b>3.05</b> (2.68-3.47)	<b>3.66</b> (3.20-4.16)	<b>4.53</b> (3.94-5.14)	<b>5.24</b> (4.53-5.94)	<b>6.00</b> (5.15-6.80)	<b>6.80</b> (5.79-7.74)	<b>7.95</b> (6.67-9.05)	<b>8.89</b> (7.37-10.2)
10-day	<b>1.95</b> (1.73-2.22)	<b>2.51</b> (2.22-2.85)	<b>3.33</b> (2.93-3.77)	<b>3.99</b> (3.50-4.51)	<b>4.92</b> (4.30-5.56)	<b>5.67</b> (4.92-6.40)	<b>6.48</b> (5.58-7.32)	<b>7.33</b> (6.26-8.29)	<b>8.53</b> (7.19-9.66)	<b>9.50</b> (7.92-10.8)
20-day	<b>2.42</b> (2.14-2.74)	<b>3.12</b> (2.76-3.53)	<b>4.13</b> (3.65-4.67)	<b>4.90</b> (4.31-5.53)	<b>5.94</b> (5.21-6.71)	<b>6.75</b> (5.89-7.62)	<b>7.58</b> (6.58-8.57)	<b>8.42</b> (7.27-9.54)	<b>9.57</b> (8.19-10.9)	<b>10.5</b> (8.87-11.9)
30-day	<b>2.84</b> (2.51-3.21)	<b>3.66</b> (3.25-4.14)	<b>4.85</b> (4.28-5.46)	<b>5.75</b> (5.07-6.47)	<b>6.97</b> (6.12-7.84)	<b>7.91</b> (6.91-8.89)	<b>8.89</b> (7.72-9.98)	<b>9.88</b> (8.53-11.1)	<b>11.2</b> (9.61-12.7)	<b>12.3</b> (10.4-13.9)
45-day	<b>3.35</b> (2.97-3.78)	<b>4.32</b> (3.84-4.88)	<b>5.72</b> (5.07-6.45)	<b>6.76</b> (5.97-7.62)	<b>8.15</b> (7.17-9.19)	<b>9.21</b> (8.06-10.4)	<b>10.3</b> (8.96-11.6)	<b>11.4</b> (9.85-12.9)	<b>12.8</b> (11.0-14.6)	<b>14.0</b> (11.9-15.9)
60-day	<b>3.74</b> (3.33-4.21)	<b>4.85</b> (4.31-5.46)	<b>6.40</b> (5.68-7.20)	<b>7.54</b> (6.67-8.49)	<b>9.03</b> (7.97-10.2)	<b>10.2</b> (8.91-11.4)	<b>11.3</b> (9.86-12.7)	<b>12.4</b> (10.8-14.0)	<b>13.9</b> (12.0-15.7)	<b>15.0</b> (12.9-17.1)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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NOAA Atlas 14, Volume 1, Version 5 Location name: Scottsdale, Arizona, USA\* Latitude: 33.5816°, Longitude: -111.8357° Elevation: 1440.71 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

#### **PF** tabular

PDS-b	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) <sup>1</sup>									
Duration		Average recurrence interval (years)								
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>2.35</b> (1.96-2.92)	<b>3.08</b> (2.57-3.80)	<b>4.15</b> (3.43-5.12)	<b>4.99</b> (4.09-6.12)	<b>6.10</b> (4.92-7.45)	<b>6.95</b> (5.54-8.44)	<b>7.82</b> (6.13-9.48)	<b>8.70</b> (6.71-10.5)	<b>9.88</b> (7.43-12.0)	<b>10.8</b> (7.93-13.1)
10-min	<b>1.79</b> (1.48-2.21)	<b>2.34</b> (1.95-2.89)	<b>3.16</b> (2.61-3.89)	<b>3.79</b> (3.11-4.66)	<b>4.64</b> (3.75-5.67)	<b>5.29</b> (4.22-6.42)	<b>5.96</b> (4.67-7.21)	<b>6.62</b> (5.11-8.00)	<b>7.52</b> (5.65-9.10)	<b>8.20</b> (6.04-9.94)
15-min	<b>1.48</b>	<b>1.93</b>	<b>2.62</b>	<b>3.14</b>	<b>3.84</b>	<b>4.37</b>	<b>4.92</b>	<b>5.47</b>	<b>6.21</b>	<b>6.78</b>
	(1.23-1.83)	(1.61-2.39)	(2.16-3.22)	(2.57-3.84)	(3.10-4.68)	(3.49-5.31)	(3.86-5.96)	(4.22-6.62)	(4.67-7.52)	(4.99-8.22)
30-min	<b>0.996</b> (0.826-1.23)	<b>1.30</b> (1.09-1.61)	<b>1.76</b> (1.45-2.17)	<b>2.11</b> (1.73-2.59)	<b>2.58</b> (2.08-3.15)	<b>2.94</b> (2.35-3.57)	<b>3.31</b> (2.60-4.01)	<b>3.69</b> (2.84-4.45)	<b>4.18</b> (3.14-5.06)	<b>4.56</b> (3.36-5.53)
60-min	<b>0.617</b>	<b>0.806</b>	<b>1.09</b>	<b>1.31</b>	<b>1.60</b>	<b>1.82</b>	<b>2.05</b>	<b>2.28</b>	<b>2.59</b>	<b>2.82</b>
	(0.511-0.762)	(0.672-0.996)	(0.899-1.34)	(1.07-1.60)	(1.29-1.95)	(1.45-2.21)	(1.61-2.48)	(1.76-2.76)	(1.95-3.13)	(2.08-3.42)
2-hr	<b>0.360</b>	<b>0.466</b>	<b>0.621</b>	<b>0.740</b>	<b>0.900</b>	<b>1.02</b>	<b>1.15</b>	<b>1.28</b>	<b>1.45</b>	<b>1.58</b>
	(0.302-0.434)	(0.392-0.564)	(0.520-0.748)	(0.612-0.889)	(0.738-1.08)	(0.827-1.22)	(0.914-1.37)	(0.997-1.52)	(1.10-1.72)	(1.18-1.89)
3-hr	<b>0.262</b>	<b>0.336</b>	<b>0.440</b>	<b>0.522</b>	<b>0.637</b>	<b>0.728</b>	<b>0.822</b>	<b>0.921</b>	<b>1.06</b>	<b>1.17</b>
	(0.220-0.322)	(0.283-0.414)	(0.368-0.540)	(0.433-0.637)	(0.519-0.771)	(0.585-0.878)	(0.650-0.991)	(0.716-1.11)	(0.797-1.27)	(0.859-1.40)
6-hr	<b>0.158</b>	<b>0.200</b>	<b>0.255</b>	<b>0.299</b>	<b>0.359</b>	<b>0.406</b>	<b>0.454</b>	<b>0.504</b>	<b>0.572</b>	<b>0.625</b>
	(0.136-0.188)	(0.172-0.238)	(0.218-0.302)	(0.253-0.353)	(0.300-0.422)	(0.334-0.475)	(0.368-0.532)	(0.402-0.591)	(0.444-0.669)	(0.474-0.733)
12-hr	<b>0.089</b>	<b>0.112</b>	<b>0.141</b>	<b>0.164</b>	<b>0.195</b>	<b>0.219</b>	<b>0.244</b>	<b>0.269</b>	<b>0.302</b>	<b>0.328</b>
	(0.077-0.104)	(0.097-0.131)	(0.122-0.165)	(0.141-0.191)	(0.165-0.227)	(0.184-0.254)	(0.201-0.282)	(0.219-0.311)	(0.240-0.351)	(0.256-0.383)
24-hr	<b>0.053</b>	<b>0.067</b>	<b>0.086</b>	<b>0.102</b>	<b>0.124</b>	<b>0.141</b>	<b>0.159</b>	<b>0.177</b>	<b>0.203</b>	<b>0.223</b>
	(0.047-0.060)	(0.059-0.077)	(0.077-0.099)	(0.090-0.116)	(0.108-0.141)	(0.122-0.160)	(0.137-0.180)	(0.151-0.201)	(0.171-0.230)	(0.186-0.254)
2-day	<b>0.029</b>	<b>0.037</b>	<b>0.048</b>	<b>0.058</b>	<b>0.070</b>	<b>0.081</b>	<b>0.091</b>	<b>0.103</b>	<b>0.119</b>	<b>0.131</b>
	(0.025-0.033)	(0.033-0.042)	(0.043-0.055)	(0.051-0.066)	(0.061-0.080)	(0.070-0.092)	(0.079-0.104)	(0.088-0.117)	(0.100-0.135)	(0.109-0.150)
3-day	<b>0.021</b>	<b>0.026</b>	<b>0.035</b>	<b>0.042</b>	<b>0.051</b>	<b>0.059</b>	<b>0.067</b>	<b>0.076</b>	<b>0.088</b>	<b>0.098</b>
	(0.018-0.024)	(0.023-0.030)	(0.031-0.040)	(0.037-0.047)	(0.045-0.058)	(0.051-0.067)	(0.058-0.076)	(0.065-0.086)	(0.074-0.101)	(0.082-0.112)
4-day	<b>0.017</b>	<b>0.021</b>	<b>0.028</b>	<b>0.034</b>	<b>0.042</b>	<b>0.048</b>	<b>0.055</b>	<b>0.063</b>	<b>0.073</b>	<b>0.082</b>
	(0.015-0.019)	(0.019-0.024)	(0.025-0.032)	(0.030-0.038)	(0.037-0.047)	(0.042-0.055)	(0.048-0.062)	(0.054-0.071)	(0.062-0.083)	(0.068-0.093)
7-day	<b>0.011</b>	<b>0.014</b>	<b>0.018</b>	<b>0.022</b>	<b>0.027</b>	<b>0.031</b>	<b>0.036</b>	<b>0.040</b>	<b>0.047</b>	<b>0.053</b>
	(0.009-0.012)	(0.012-0.016)	(0.016-0.021)	(0.019-0.025)	(0.023-0.031)	(0.027-0.035)	(0.031-0.040)	(0.034-0.046)	(0.040-0.054)	(0.044-0.060)
10-day	<b>0.008</b>	<b>0.010</b>	<b>0.014</b>	<b>0.017</b>	<b>0.021</b>	<b>0.024</b>	<b>0.027</b>	<b>0.031</b>	<b>0.036</b>	<b>0.040</b>
	(0.007-0.009)	(0.009-0.012)	(0.012-0.016)	(0.015-0.019)	(0.018-0.023)	(0.021-0.027)	(0.023-0.030)	(0.026-0.035)	(0.030-0.040)	(0.033-0.045)
20-day	<b>0.005</b>	<b>0.006</b>	<b>0.009</b>	<b>0.010</b>	<b>0.012</b>	<b>0.014</b>	<b>0.016</b>	<b>0.018</b>	<b>0.020</b>	<b>0.022</b>
	(0.004-0.006)	(0.006-0.007)	(0.008-0.010)	(0.009-0.012)	(0.011-0.014)	(0.012-0.016)	(0.014-0.018)	(0.015-0.020)	(0.017-0.023)	(0.018-0.025)
30-day	<b>0.004</b>	<b>0.005</b>	<b>0.007</b>	<b>0.008</b>	<b>0.010</b>	<b>0.011</b>	<b>0.012</b>	<b>0.014</b>	<b>0.016</b>	<b>0.017</b>
	(0.003-0.004)	(0.005-0.006)	(0.006-0.008)	(0.007-0.009)	(0.008-0.011)	(0.010-0.012)	(0.011-0.014)	(0.012-0.015)	(0.013-0.018)	(0.014-0.019)
45-day	<b>0.003</b>	<b>0.004</b>	<b>0.005</b>	<b>0.006</b>	<b>0.008</b>	<b>0.009</b>	<b>0.010</b>	<b>0.011</b>	<b>0.012</b>	<b>0.013</b>
	(0.003-0.003)	(0.004-0.005)	(0.005-0.006)	(0.006-0.007)	(0.007-0.009)	(0.007-0.010)	(0.008-0.011)	(0.009-0.012)	(0.010-0.014)	(0.011-0.015)
60-day	<b>0.003</b>	<b>0.003</b>	<b>0.004</b>	<b>0.005</b>	<b>0.006</b>	<b>0.007</b>	<b>0.008</b>	<b>0.009</b>	<b>0.010</b>	<b>0.010</b>
	(0.002-0.003)	(0.003-0.004)	(0.004-0.005)	(0.005-0.006)	(0.006-0.007)	(0.006-0.008)	(0.007-0.009)	(0.007-0.010)	(0.008-0.011)	(0.009-0.012)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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### Worksheet for 2" Orifice Basin A

Project Description		
Solve For	Discharge	
Input Data		
Headwater Elevation	1436.50	ft
Centroid Elevation	1433.58	ft
Tailwater Elevation	1435.00	ft
Discharge Coefficient	3.00	
Diameter	2.00	in
Results		
Discharge	0.64	ft³/s
Headwater Height Above Centroid	2.92	ft
Tailwater Height Above Centroid	1.42	ft
Flow Area	0.02	ft²
Velocity	29.47	ft/s

### Worksheet for 2" Orifice Basin B

Project Description		
Solve For	Discharge	
Input Data		
Headwater Elevation	1435.00	ft
Centroid Elevation	1433.08	ft
Tailwater Elevation	1432.80	ft
Discharge Coefficient	3.00	
Diameter	2.00	in
Results		
Discharge	0.73	ft³/s
Headwater Height Above Centroid	1.92	ft
Tailwater Height Above Centroid	-0.28	ft
Flow Area	0.02	ft²
Velocity	33.35	ft/s

### Worksheet for 2" Orifice Basin C

Project Description		
Solve For	Discharge	
Input Data		
Headwater Elevation	1439.50	ft
Centroid Elevation	1438.08	ft
Tailwater Elevation	1437.00	ft
Discharge Coefficient	3.00	
Diameter	2.00	in
Results		
Discharge	0.63	ft³/s
Headwater Height Above Centroid	1.42	ft
Tailwater Height Above Centroid	-1.08	ft
Flow Area	0.02	ft²
Velocity	28.68	ft/s

# 3 engineering surveying

#### **Basin Percolation Rates**

	Total	Rate of	Dry-Up
Sub-Area	Basin Vol.	Bleedoff	Time
	(cf)	(cfs)	(hr)
А	6,234	0.64	2.71
A+B+C	18,394	0.73	7.00
С	2,050	0.63	0.90

<u>3®engineering</u> civil engineering planning survevina

### APPENDIX K

# Mirage Crossing As-Builts



COMHUNITY NUMBER

045012

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HOCEDT HEY EN UM

1795

SEPT 30, 1995

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DATE OF FIRM

(Index Cote)

SFPT 30, 1995

FIRM 70%

MAR. FLOCO ELEVATION

(In AU Zone, Use Depth)

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surveying planning civil engineering

# APPENDIX L

### Preliminary Grading and Drainage Plans

<u>G</u> 1.	ENERAL NOTES FOR F	UBLIC WORKS C	ONSTRUCTIO	<u>DN:</u>	Р	RELIM
-	GRANTED FOR PUBLIC USE MU ASSOCIATION OF GOVERNMENT AND UNIFORM STANDARD DET AMENDED BY THE LATEST VE	JST CONFORM TO THE L S (MAG) UNIFORM STANE AILS FOR PUBLIC WORKS RSION OF THE CITY OF	ATEST MARICOPA ARD SPECIFICATIO CONSTRUCTION A SCOTTSDALE	DNS AS		11
	THERE IS A CONFLICT, THE C GOVERN.	ND SUPPLEMENTAL STAN ITY'S SUPPLEMENTAL ST	ANDARD DETAILS. IF	WILL	SWC	••• 2114T
2.	THE CITY ONLY APPROVES THE CITY ONLY APPROVES THE DESIGNS; THEREFORE IF CONS PLANS, THEY ARE NOT VERIFI	HE SCOPE, NOT THE DET TRUCTION QUANTITIES A ED BY THE CITY.	AIL OF ENGINEERIN RE SHOWN ON THE	NG ESE	LOCATED IN	A PORTIC
3.	THE APPROVAL OF PLANS IS PERMIT HAS NOT BEEN ISSUE BE RESUBMITTED TO THE CIT	VALID FOR SIX (6) MON <sup>.</sup> D WITHIN THIS TIME FR Y FOR RE-APPROVAL.	THS.IF ASSOCIATE AME,THE PLANS N	D NUST	<u> </u>	OF T 
4.	A CITY INSPECTOR WILL INSP SCOTTSDALE.NOTIFY INSPECT WORK.	ECT ALL WORKS WITHIN ION SERVICES 72 HOURS	THE CITY OF 5 BEFORE BEGINNI	NG		
5.	WHENEVER EXCAVATION IS NE TWO WORKING DAYS BEFORE I	CESSARY, CALL THE BLL EXCAVATION BEGINS.	IE STAKE CENTER,	811,		•
6.	PERMISSION TO WORK IN THE FOR ALL WORKS WITHIN THE PUBLIC PURPOSES. COPIES OF AND BE AVAILABLE FOR INSP THE REQUIRED PERMITS WILL	RIGHT-OF-WAY (PWR) PE RIGHTS-OF-WAY AND EA ALL PERMITS MUST BE ECTION AT ALL TIMES.I RESULT IN IMMEDIATE	RMITS ARE REQUI SEMENTS GRANTED RETAINED ON-SIT FAILURE TO PRODU SUSPENSION OF A	RED ) FOR TE JCE LL		<u>-</u>
	WEIGHTED C CA		OBTAINED.			
		- 20 210 SE				
	AREA OF IMPERVIOUS AREA OF PERVIOUS = / (20,2	- 20,210 SF 24,346 SF 210 X 0.95) + (24,346	X 0.45)			
	WEIGHTED C = $($	44,556	)			
	DRAINAGE AREA B AREA OF IMPERVIOUS	= 35,146 SF				
	AREA OF PERVIOUS = $(\frac{35}{35},$	18,913 SF 146 X 0.95) + (18,913	X 0.45)			
	WEIGHTED C = 0.78	54,059	)		Ì	APN: 21733088
	DRAINAGE AREA C AREA OF IMPERVIOUS	= 7,237 SF				
	WEIGHTED C = $\left(\frac{(7,2)}{(7,2)}\right)$	237 X 0.95) + (5,102 12,339	X 0.45)			
	WEIGHTED C = 0.74		/			
	RETENTION CAL	CULATIONS				
	100-YEAR 2-HOUR STO DRAINAGE AREA A	DRM EVENT				
	C = 0.68 P = 2.30 IN AREA = 44,556 SF					APN: 21733089
	$(0.68) \left(\frac{2.30}{12}\right) (44)$	1,556) = 5,780 CF REC	UIRED		İ	
	$\frac{\text{DRAINAGE AREA B}}{\text{C} = 0.78}$	6,234 CF PR	DVIDED			
	P = 2.30  IN AREA = 54,059 SF (0.70) (2.30) (5)					
	$(0.78)\left(\frac{12}{12}\right)$	1,059) = 8,031 CF REQ 10,110 CF PR				
	$\frac{\text{DRAINAGE AREA C}}{\text{C} = 0.74}$ $\text{P} = 2.30 \text{ IN}$					
	AREA = 12,339 SF (0.74) $\left(\frac{2.30}{42}\right)$ (12)	2,339) = 1,758 CF REQI	JIRED			APN: 21733090
	DRAINAGE AREA D	2,050 CF PR	OVIDED			
	C = 0.64 (R1-18 ZON) P = 2.30 IN	NG)				
	$(0.64) \left(\frac{2.30}{12}\right) (50)$	),962) = 6,251 CF REQ	UIRED			
	*NOTE: 6,251 CUBIC FE	ET OF RETENTION IS R	EQUIRED		L	
	THIS VOLUME WILL BE	OR LOT 1. Required to be reta: The Lot. F non-waivable and w	INED ON			
		NAME OF COMPANY	TELEPHONE	DATE		SH
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I LLEPHONE NATURAL GAS	CENTURY LINK SOUTHWEST GAS					PC
OTHER OTHER	A.T.&T.					FLOOD IN
ENGINEER'S CI	ERTIFICATION	CORD FOR THIS DEVELOR	MENT, HEREBY CE	RTIFY THAT	COMMUNITY NUMBER	PANEL NUMBER
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IN THIS SUBMI	IIAL.				ENGINEER	CINISH FLOOP FLO
SIGNATURE		D	ATE		TO PROVIDE P REVISED CODE	ROTECTION FROM CHAPTER 37 - F

# AINARY GRADING & DRAINAGE PLAN FOR 4TH & SHEA BLVD., SCOTTSDALE, AZ 85259 ION OF THE NORTHWEST QUARTER OF SECTION 27, TOWNSHIP 3 NORTH, RANGE 5 EAST

THE GILA AND SALT RIVER MERIDIAN, MARICOPA COUNTY, ARIZONA



	INDEX OF SHEETS
Sheet no.	DESCRIPTION
PGRD01	COVER SHEET - PRELIMINARY GRADING & DRAINAGE PLAN
PGRD02	PRELIMINARY GRADING & DRAINAGE PLAN
PGRD03	PRELIMINARY GRADING & DRAINAGE PLAN

# NSURANCE RATE MAP (FIRM) INFORMATION:

BER	PANEL DATE	SUFFIX	FIRM DATE	FIRM ZONE	BASE FLOOD ELEVATION
	OCTOBER 16,2013	L	OCTOBER 16,2013	х	NZA

### CATION:

ELEVATION(S) AND/OR FLOOD PROOFING ELEVATION(S) ON THIS PLAN ARE SUFFICIENTLY HIGH OM FLOODING CAUSED BY A 100-YEAR STORM, AND ARE IN ACCORDANCE WITH SCOTTSDALE - FLOODPLAIN AND STORMWATER REGULATION.

### LEGEND

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