

## FINAL Basis of Design Report

☐ APPROVED

☒ APPROVED AS NOTED

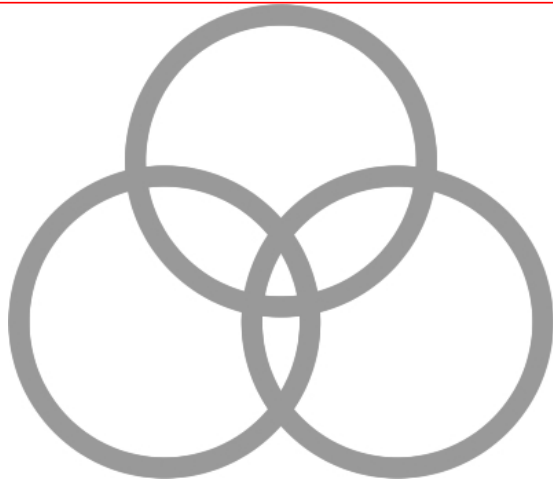
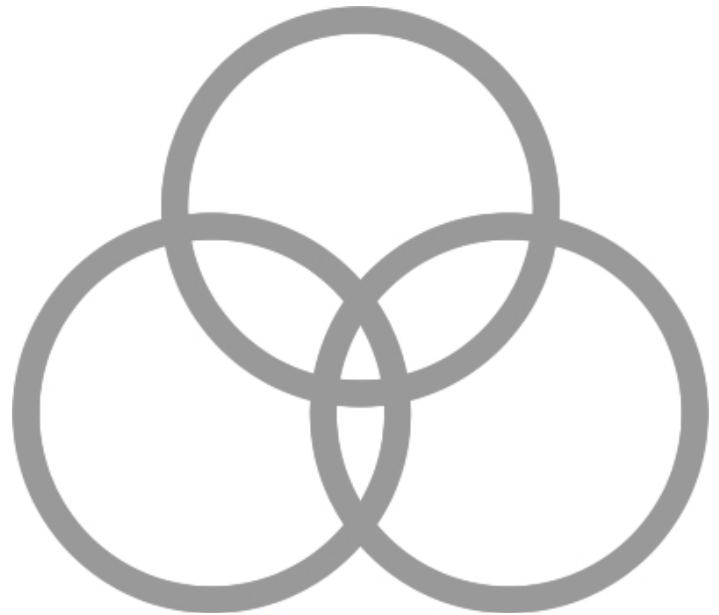
☐ REVISE AND RESUBMIT



Disclaimer: If approved; the approval is granted under the condition that the final construction documents submitted for city review will match the information herein. Any subsequent changes in the water or sewer design that materially impact design criteria or standards will require re-analysis, re-submittal, and approval of a revised basis of design report prior to the plan review submission.; this approval is not a guarantee of construction document acceptance. For questions or clarifications contact the Water Resources Planning and Engineering Department at 480-312-5685.

BY rsacks

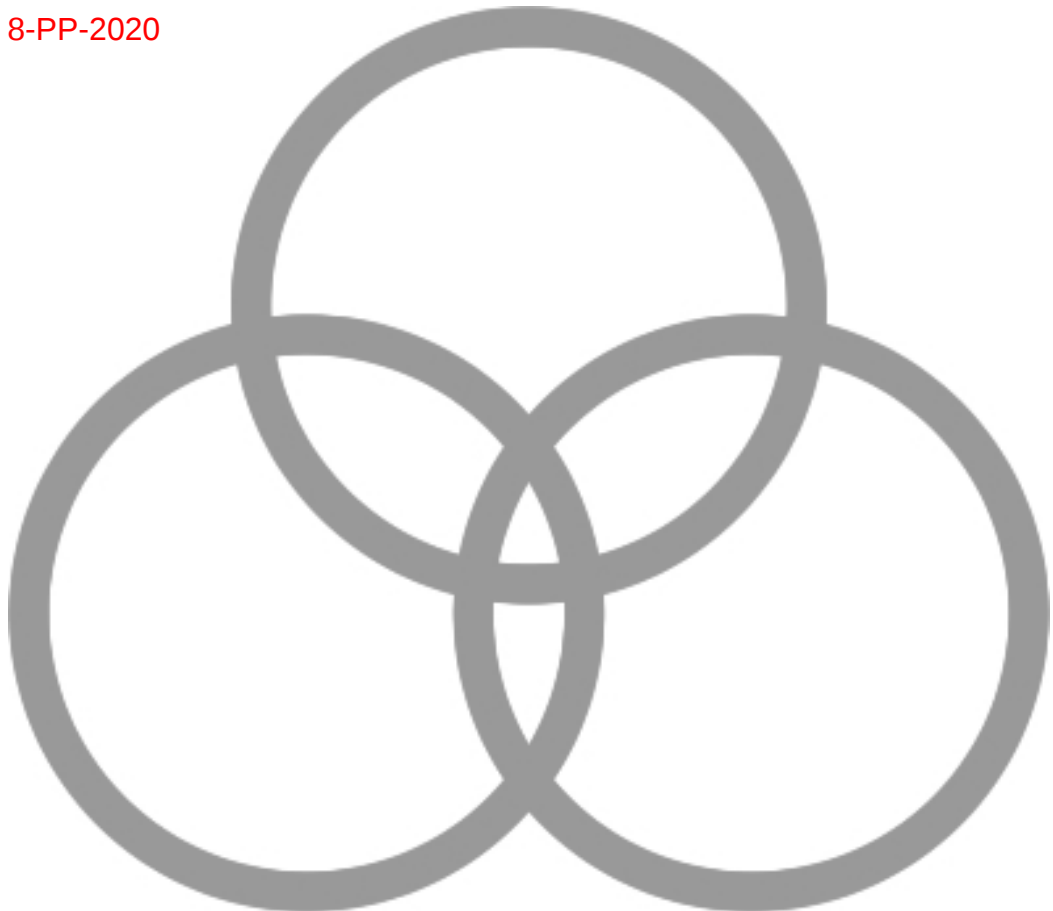
DATE 1/26/2022



## 114<sup>th</sup> & Shea Retail Final Wastewater Basis of Design Report

3 engineering Job #: 1831  
November 3, 2021

See comments on 8-PP-2020



# **114<sup>TH</sup> & SHEA RETAIL**

## **FINAL WASTEWATER BASIS OF DESIGN REPORT**

***Prepared for:***

Hawkins Companies LLC  
4700 S. McClintock Drive #160  
Scottsdale, Arizona 85257  
Contact: Mark Mitchell  
Phone: (480) 223-8239



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Daniel G. Mann, P.E.

November 3, 2021

***Submittal to:***

City of Scottsdale  
7447 E. Indian School Road, Suite 105  
Scottsdale, AZ 85251

***Prepared by:***

3 engineering, LLC  
6370 E. Thomas Road, Suite #200  
Scottsdale, Arizona 85251  
Contact: Dan G. Mann, P.E.

***Job Number 1831***

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## **1. Introduction**

The project site, 114<sup>th</sup> & Shea Retail, is located in the northwest quarter of Section 27, Township 3 North, Range 5 East of the Gila and Salt River Meridian, Maricopa County, Arizona within the City of Scottsdale. The project is located at the southwest corner of 114<sup>th</sup> Street and Shea Boulevard, Scottsdale, AZ 85259. The site is bounded on the north by Shea Boulevard, on the east by 114<sup>th</sup> Street, on the south by Beryl Avenue, and on the west by single family residential homes. See Appendix A for a site map.

The existing zoning is C-O and R1-18. The site is currently undeveloped. The proposed zoning is C1, SR, and R1-18. The City of Scottsdale 2001 General Plan shows the site land use is Office and Rural Neighborhood. Additionally, the site is located within the Shea Corridor. The proposed site consists of two portions that are separated by an existing wash that runs west through the site. The northern portion is a mixed-use office, medical office, and retail center with three pads. Access is provided by 114<sup>th</sup> Street. The southern portion consists of two residential lots with access provided by Beryl Avenue. The residential lots will be developed at a future date. The proposed mixed-use site will connect to existing City of Scottsdale sewer facilities within 114<sup>th</sup> Street.

## **2. Existing Conditions**

The existing zoning is C-O and R1-18. The site is currently undeveloped. See Appendix A for a site map. There is an existing 8" V.C.P. public sewer main that adjacent to the site in 114<sup>th</sup> Street. This 8" line flows south to a manhole in 114<sup>th</sup> Street that is aligned with Beryl Avenue. There is an existing 8" V.C.P. public sewer main in Beryl Avenue that flows west beyond the site. See Sewer System Exhibit in Appendix C and Sewer Plans in Appendix D for existing sewer line layout.

## **3. Proposed Conditions**

The proposed project consists a mixed-use office and retail center and two residential lots. There are three proposed buildings in the mixed-use center. Building A is 3,000 s.f. of retail/office space, building B is 5,100 s.f. of retail/office space, and building C is 7,236 s.f. of medical office space. The three buildings are serviced by 6" P.V.C. service lines that connect to a proposed 8" P.V.C. public sewer line that connects to an existing manhole in 114<sup>th</sup> Avenue, east of the site. There are no proposed improvements to the two residential lots at this time but the 4" sewer stubs will be installed. See Sewer System Exhibit in Appendix C and Sewer Plans in Appendix D for proposed sewer line layout. The proposed sewer line is to be private and is to be maintained by the owner of the property.

## **4. Design Documentation/ Computations/ Hydraulic Modeling**

The purpose of this basis of design report is to verify that the proposed private sewer system is able to accommodate demands generated by the project, 114<sup>th</sup> & Shea Retail. FlowMaster V8i by Bentley Systems was used to model and analyze the proposed sewer system for compliance with the City of Scottsdale design requirements. Demands were calculated using Section 7-1.403B of the City of Scottsdale Design Standards and Policies Manual dated 2018. It is our opinion that this report is in accordance with the 2018 City of Scottsdale Design Standards and Policies Manual.

The following demand criteria were used in determining the system demands for the proposed site.

1. Building A – 3,000 s.f. proposed Retail space
2. Building B – 5,100 s.f. proposed Retail space
3. Building C – 7,236 s.f. proposed Office space
4. Residential lots not included in analysis as they are just service taps.
5. 0.5 gpd/s.f. (Per DSPM, Commercial/Retail)
6. 0.4 gpd/s.f. (Per DSPM, Office)
7. Peaking factor = 3 (Per DSPM, Commercial/Retail, Office)

| TABLE 1: ON-SITE SEWER DEMANDS |              |
|--------------------------------|--------------|
| Avg. daily demand              | 6,944.4 gpd  |
| Design Flow Rate               | 20,833.2 gpd |

Retail (Buildings A & B)

Average daily demand:  $8,100 \text{ s.f.} \times 0.5 \text{ gpd per s.f.} = 4,050 \text{ gpd}$   
 $= 4,050 \text{ gpd}/1440 \text{ mpd} = 2.81 \text{ gpm}$

Peak flow rate =  $3.0 \times 4,050 \text{ gpd} = 12,150 \text{ gpd}$   
 $= 12,150 \text{ gpd}/1440 \text{ mpd} = 8.44 \text{ gpm}$

Office (Building C)

Average daily demand:  $7,236 \text{ s.f.} \times 0.4 \text{ gpd per s.f.} = 2,894.4 \text{ gpd}$   
 $= 2,894.4 \text{ gpd}/1440 \text{ mpd} = 2.01 \text{ gpm}$

Peak flow rate =  $3.0 \times 2,894.4 \text{ gpd} = 8,683.2 \text{ gpd}$   
 $= 8,683.2 \text{ gpd}/1440 \text{ mpd} = 6.03 \text{ gpm}$

Total

Average daily demand = 6,944.4 gpd  
 Peak flow rate = 20,833.2 gpd

FlowMaster V8i by Bentley Systems was used to model and analyze the proposed sewer system for compliance with the C.O.S. design requirements. Based on the results from the sewer system analysis, the proposed 8" sewer system has a full flow capacity of 552,232.73 gpd and a maximum d/D of 0.133 at the design flow rate of 20,833.2 gpd. The analysis was performed using a minimum slope of 0.50%. The maximum allowable d/D for lines less than 12" is 0.65 per the C.O.S. design requirements. Refer to the FlowMaster data in Appendix B for loading of the sewer line with the peak flow. See Sewer System Exhibit in Appendix C and Sewer Plans in Appendix D for proposed sewer line layout.

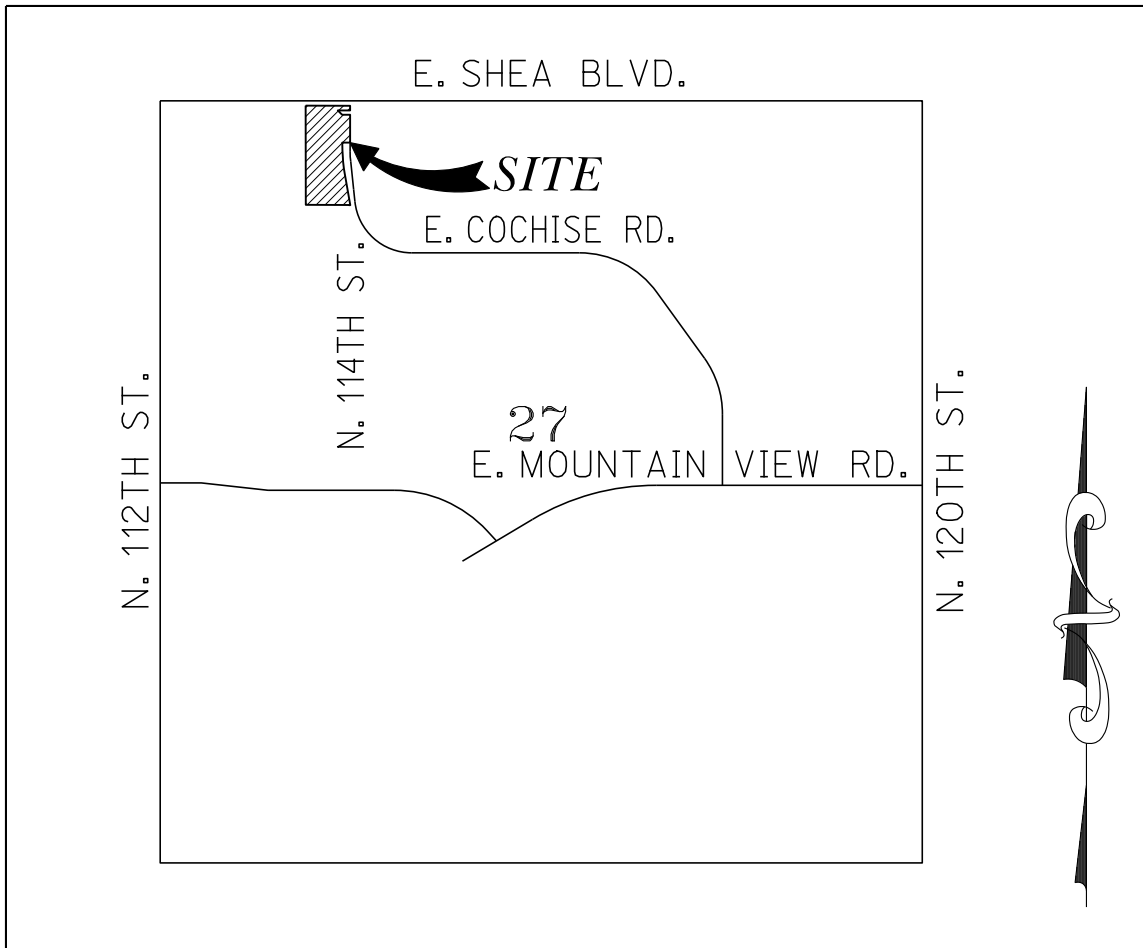
## 5. Summary

The Peak Flow for the proposed site is 20,833.2 gpd. The proposed sewer system has a maximum d/D of 0.133 at peak flow. The proposed sewer facilities are in accordance with the City of Scottsdale Design Standards.



*APPENDIX A*

*Vicinity Map*



## VICINITY MAP

N.T.S.



*APPENDIX B*  
*FlowMaster Data*

Cross Section for Proposed 6" Sewer Peak Flow

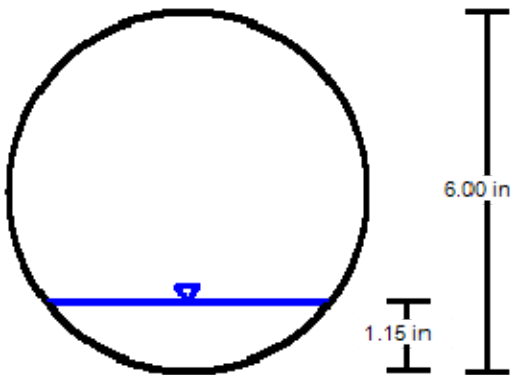
Project Description

Friction Method                      Manning Formula  
Solve For                                Normal Depth

Input Data

|                       |          |         |
|-----------------------|----------|---------|
| Roughness Coefficient | 0.013    |         |
| Channel Slope         | 0.50000  | %       |
| Normal Depth          | 1.15     | in      |
| Diameter              | 6.00     | in      |
| Discharge             | 20833.20 | gal/day |

Cross Section Image



V: 1  
H: 1

## Worksheet for Proposed 6" Sewer Peak Flow

### Project Description

|                 |                 |
|-----------------|-----------------|
| Friction Method | Manning Formula |
| Solve For       | Normal Depth    |

### Input Data

|                       |          |         |
|-----------------------|----------|---------|
| Roughness Coefficient | 0.013    |         |
| Channel Slope         | 0.50000  | %       |
| Diameter              | 6.00     | in      |
| Discharge             | 20833.20 | gal/day |

### Results

|                   |             |                    |
|-------------------|-------------|--------------------|
| Normal Depth      | 1.15        | in                 |
| Flow Area         | 0.03        | ft <sup>2</sup>    |
| Wetted Perimeter  | 0.45        | ft                 |
| Hydraulic Radius  | 0.70        | in                 |
| Top Width         | 0.39        | ft                 |
| Critical Depth    | 0.09        | ft                 |
| Percent Full      | 19.2        | %                  |
| Critical Slope    | 0.00740     | ft/ft              |
| Velocity          | 1.22        | ft/s               |
| Velocity Head     | 0.02        | ft                 |
| Specific Energy   | 0.12        | ft                 |
| Froude Number     | 0.83        |                    |
| Maximum Discharge | 0.43        | ft <sup>3</sup> /s |
| Discharge Full    | 0.40        | ft <sup>3</sup> /s |
| Slope Full        | 0.00003     | ft/ft              |
| Flow Type         | SubCritical |                    |

### GVF Input Data

|                  |      |    |
|------------------|------|----|
| Downstream Depth | 0.00 | in |
| Length           | 0.00 | ft |
| Number Of Steps  | 0    |    |

### GVF Output Data

|                             |          |      |
|-----------------------------|----------|------|
| Upstream Depth              | 0.00     | in   |
| Profile Description         |          |      |
| Profile Headloss            | 0.00     | ft   |
| Average End Depth Over Rise | 0.00     | %    |
| Normal Depth Over Rise      | 19.23    | %    |
| Downstream Velocity         | Infinity | ft/s |

---

## Worksheet for Proposed 6" Sewer Peak Flow

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### GVF Output Data

|                   |          |       |
|-------------------|----------|-------|
| Upstream Velocity | Infinity | ft/s  |
| Normal Depth      | 1.15     | in    |
| Critical Depth    | 0.09     | ft    |
| Channel Slope     | 0.50000  | %     |
| Critical Slope    | 0.00740  | ft/ft |

Cross Section for Proposed 6" Sewer Full Flow

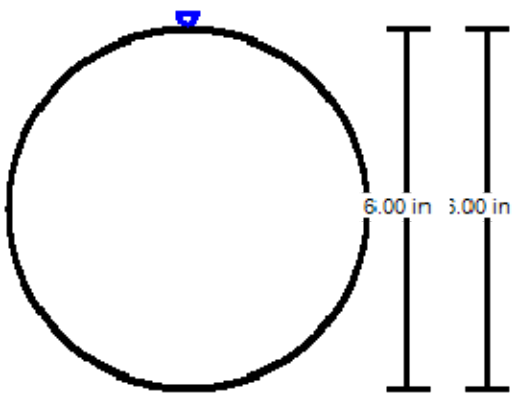
Project Description

|                 |                    |
|-----------------|--------------------|
| Friction Method | Manning Formula    |
| Solve For       | Full Flow Capacity |

Input Data

|                       |                   |
|-----------------------|-------------------|
| Roughness Coefficient | 0.013             |
| Channel Slope         | 0.50000 %         |
| Normal Depth          | 6.00 in           |
| Diameter              | 6.00 in           |
| Discharge             | 256420.17 gal/day |

Cross Section Image



V: 1  
H: 1

## Worksheet for Proposed 6" Sewer Full Flow

### Project Description

|                 |                    |
|-----------------|--------------------|
| Friction Method | Manning Formula    |
| Solve For       | Full Flow Capacity |

### Input Data

|                       |           |         |
|-----------------------|-----------|---------|
| Roughness Coefficient | 0.013     |         |
| Channel Slope         | 0.50000   | %       |
| Normal Depth          | 6.00      | in      |
| Diameter              | 6.00      | in      |
| Discharge             | 256420.17 | gal/day |

### Results

|                   |             |                    |
|-------------------|-------------|--------------------|
| Discharge         | 256420.17   | gal/day            |
| Normal Depth      | 6.00        | in                 |
| Flow Area         | 0.20        | ft <sup>2</sup>    |
| Wetted Perimeter  | 1.57        | ft                 |
| Hydraulic Radius  | 1.50        | in                 |
| Top Width         | 0.00        | ft                 |
| Critical Depth    | 0.32        | ft                 |
| Percent Full      | 100.0       | %                  |
| Critical Slope    | 0.00911     | ft/ft              |
| Velocity          | 2.02        | ft/s               |
| Velocity Head     | 0.06        | ft                 |
| Specific Energy   | 0.56        | ft                 |
| Froude Number     | 0.00        |                    |
| Maximum Discharge | 0.43        | ft <sup>3</sup> /s |
| Discharge Full    | 0.40        | ft <sup>3</sup> /s |
| Slope Full        | 0.00500     | ft/ft              |
| Flow Type         | SubCritical |                    |

### GVF Input Data

|                  |      |    |
|------------------|------|----|
| Downstream Depth | 0.00 | in |
| Length           | 0.00 | ft |
| Number Of Steps  | 0    |    |

### GVF Output Data

|                             |      |    |
|-----------------------------|------|----|
| Upstream Depth              | 0.00 | in |
| Profile Description         |      |    |
| Profile Headloss            | 0.00 | ft |
| Average End Depth Over Rise | 0.00 | %  |

---

## Worksheet for Proposed 6" Sewer Full Flow

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### GVF Output Data

|                        |          |       |
|------------------------|----------|-------|
| Normal Depth Over Rise | 100.00   | %     |
| Downstream Velocity    | Infinity | ft/s  |
| Upstream Velocity      | Infinity | ft/s  |
| Normal Depth           | 6.00     | in    |
| Critical Depth         | 0.32     | ft    |
| Channel Slope          | 0.50000  | %     |
| Critical Slope         | 0.00911  | ft/ft |

Cross Section for Proposed 8" Sewer Peak Flow

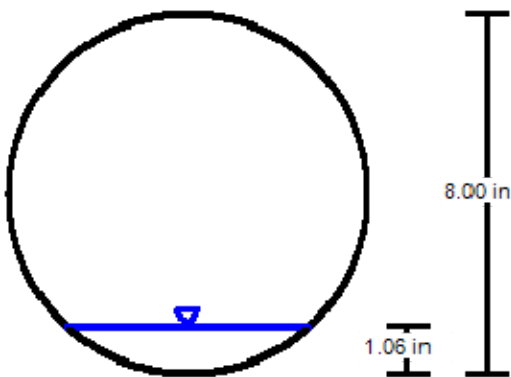
Project Description

Friction Method                      Manning Formula  
Solve For                                Normal Depth

Input Data

|                       |          |         |
|-----------------------|----------|---------|
| Roughness Coefficient | 0.013    |         |
| Channel Slope         | 0.50000  | %       |
| Normal Depth          | 1.06     | in      |
| Diameter              | 8.00     | in      |
| Discharge             | 20833.20 | gal/day |

Cross Section Image



V: 1  
H: 1

## Worksheet for Proposed 8" Sewer Peak Flow

### Project Description

|                 |                 |
|-----------------|-----------------|
| Friction Method | Manning Formula |
| Solve For       | Normal Depth    |

### Input Data

|                       |          |         |
|-----------------------|----------|---------|
| Roughness Coefficient | 0.013    |         |
| Channel Slope         | 0.50000  | %       |
| Diameter              | 8.00     | in      |
| Discharge             | 20833.20 | gal/day |

### Results

|                   |             |                    |
|-------------------|-------------|--------------------|
| Normal Depth      | 1.06        | in                 |
| Flow Area         | 0.03        | ft <sup>2</sup>    |
| Wetted Perimeter  | 0.50        | ft                 |
| Hydraulic Radius  | 0.66        | in                 |
| Top Width         | 0.45        | ft                 |
| Critical Depth    | 0.08        | ft                 |
| Percent Full      | 13.3        | %                  |
| Critical Slope    | 0.00722     | ft/ft              |
| Velocity          | 1.17        | ft/s               |
| Velocity Head     | 0.02        | ft                 |
| Specific Energy   | 0.11        | ft                 |
| Froude Number     | 0.84        |                    |
| Maximum Discharge | 0.92        | ft <sup>3</sup> /s |
| Discharge Full    | 0.85        | ft <sup>3</sup> /s |
| Slope Full        | 0.00001     | ft/ft              |
| Flow Type         | SubCritical |                    |

### GVF Input Data

|                  |      |    |
|------------------|------|----|
| Downstream Depth | 0.00 | in |
| Length           | 0.00 | ft |
| Number Of Steps  | 0    |    |

### GVF Output Data

|                             |          |      |
|-----------------------------|----------|------|
| Upstream Depth              | 0.00     | in   |
| Profile Description         |          |      |
| Profile Headloss            | 0.00     | ft   |
| Average End Depth Over Rise | 0.00     | %    |
| Normal Depth Over Rise      | 13.27    | %    |
| Downstream Velocity         | Infinity | ft/s |

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## Worksheet for Proposed 8" Sewer Peak Flow

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### GVF Output Data

|                   |          |       |
|-------------------|----------|-------|
| Upstream Velocity | Infinity | ft/s  |
| Normal Depth      | 1.06     | in    |
| Critical Depth    | 0.08     | ft    |
| Channel Slope     | 0.50000  | %     |
| Critical Slope    | 0.00722  | ft/ft |

Cross Section for Proposed 8" Sewer Full Flow

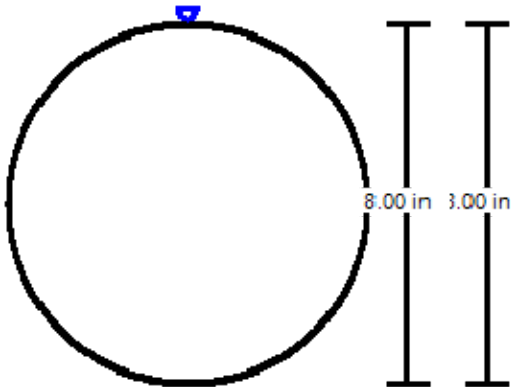
Project Description

|                 |                    |
|-----------------|--------------------|
| Friction Method | Manning Formula    |
| Solve For       | Full Flow Capacity |

Input Data

|                       |                   |
|-----------------------|-------------------|
| Roughness Coefficient | 0.013             |
| Channel Slope         | 0.50000 %         |
| Normal Depth          | 8.00 in           |
| Diameter              | 8.00 in           |
| Discharge             | 552232.73 gal/day |

Cross Section Image



V: 1  
H: 1

## Worksheet for Proposed 8" Sewer Full Flow

### Project Description

|                 |                    |
|-----------------|--------------------|
| Friction Method | Manning Formula    |
| Solve For       | Full Flow Capacity |

### Input Data

|                       |           |         |
|-----------------------|-----------|---------|
| Roughness Coefficient | 0.013     |         |
| Channel Slope         | 0.50000   | %       |
| Normal Depth          | 8.00      | in      |
| Diameter              | 8.00      | in      |
| Discharge             | 552232.73 | gal/day |

### Results

|                   |             |                    |
|-------------------|-------------|--------------------|
| Discharge         | 552232.73   | gal/day            |
| Normal Depth      | 8.00        | in                 |
| Flow Area         | 0.35        | ft <sup>2</sup>    |
| Wetted Perimeter  | 2.09        | ft                 |
| Hydraulic Radius  | 2.00        | in                 |
| Top Width         | 0.00        | ft                 |
| Critical Depth    | 0.44        | ft                 |
| Percent Full      | 100.0       | %                  |
| Critical Slope    | 0.00848     | ft/ft              |
| Velocity          | 2.45        | ft/s               |
| Velocity Head     | 0.09        | ft                 |
| Specific Energy   | 0.76        | ft                 |
| Froude Number     | 0.00        |                    |
| Maximum Discharge | 0.92        | ft <sup>3</sup> /s |
| Discharge Full    | 0.85        | ft <sup>3</sup> /s |
| Slope Full        | 0.00500     | ft/ft              |
| Flow Type         | SubCritical |                    |

### GVF Input Data

|                  |      |    |
|------------------|------|----|
| Downstream Depth | 0.00 | in |
| Length           | 0.00 | ft |
| Number Of Steps  | 0    |    |

### GVF Output Data

|                             |      |    |
|-----------------------------|------|----|
| Upstream Depth              | 0.00 | in |
| Profile Description         |      |    |
| Profile Headloss            | 0.00 | ft |
| Average End Depth Over Rise | 0.00 | %  |

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## Worksheet for Proposed 8" Sewer Full Flow

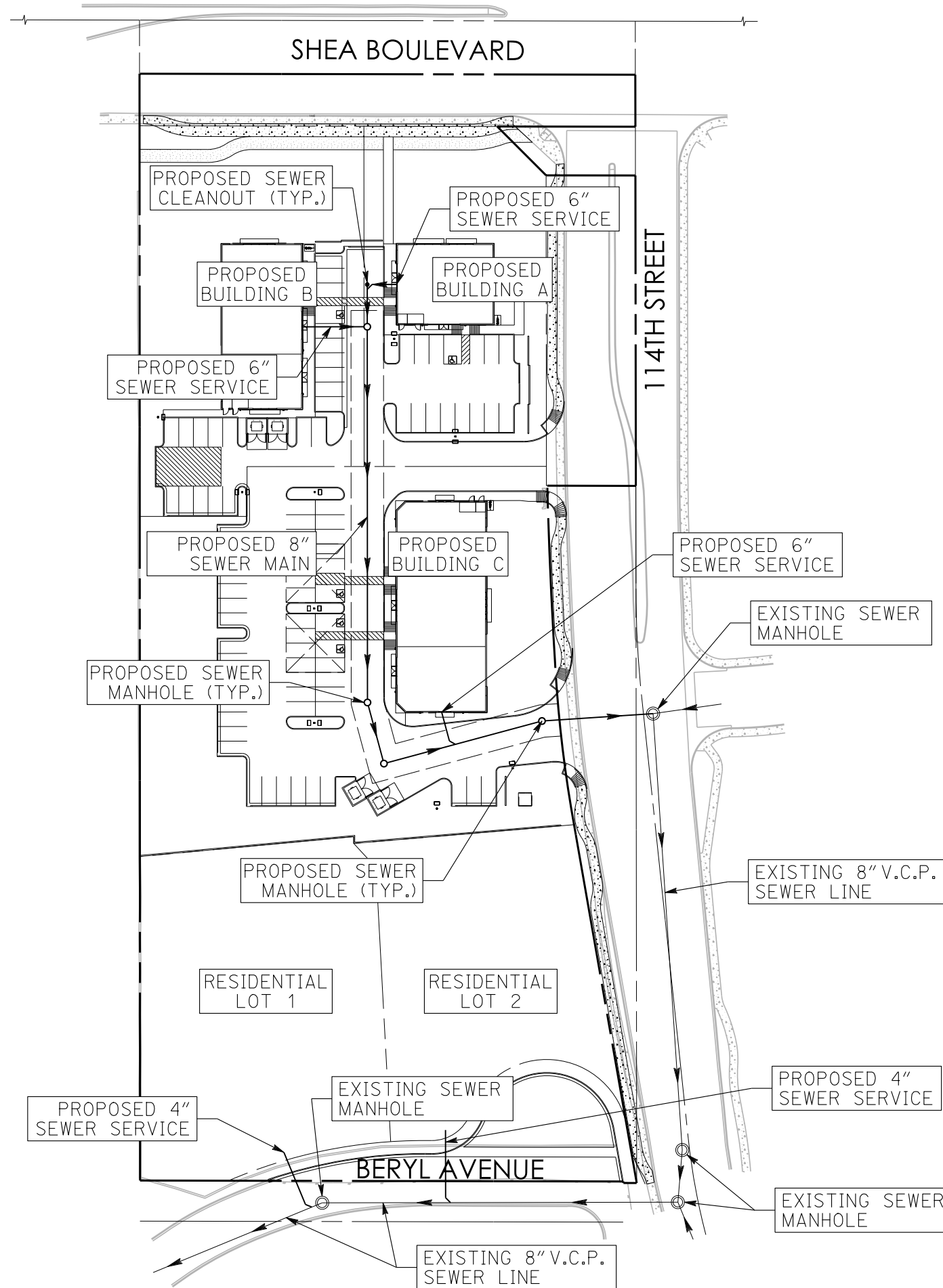
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### GVF Output Data

|                        |          |       |
|------------------------|----------|-------|
| Normal Depth Over Rise | 100.00   | %     |
| Downstream Velocity    | Infinity | ft/s  |
| Upstream Velocity      | Infinity | ft/s  |
| Normal Depth           | 8.00     | in    |
| Critical Depth         | 0.44     | ft    |
| Channel Slope          | 0.50000  | %     |
| Critical Slope         | 0.00848  | ft/ft |

## *APPENDIX C*

### *Sewer System Exhibit*



1"=80'

# 114TH & SHEA RETAIL

SWC 114TH ST. & SHEA BLVD., SCOTTSDALE, AZ 85259

**300engineering**  
civil engineering  
surveying  
planning

300 ENGINEERING, LLC  
6370 E. THOMAS ROAD, SUITE # 200 - SCOTTSDALE, ARIZONA 85251  
PHONE: (602) 334-4387 - FAX: (602) 490-3230  
WWW.300ENGINEERING.COM

DATE: 11/03/21  
PROJECT NO.  
**1831**

SEWER SYSTEM EXHIBIT

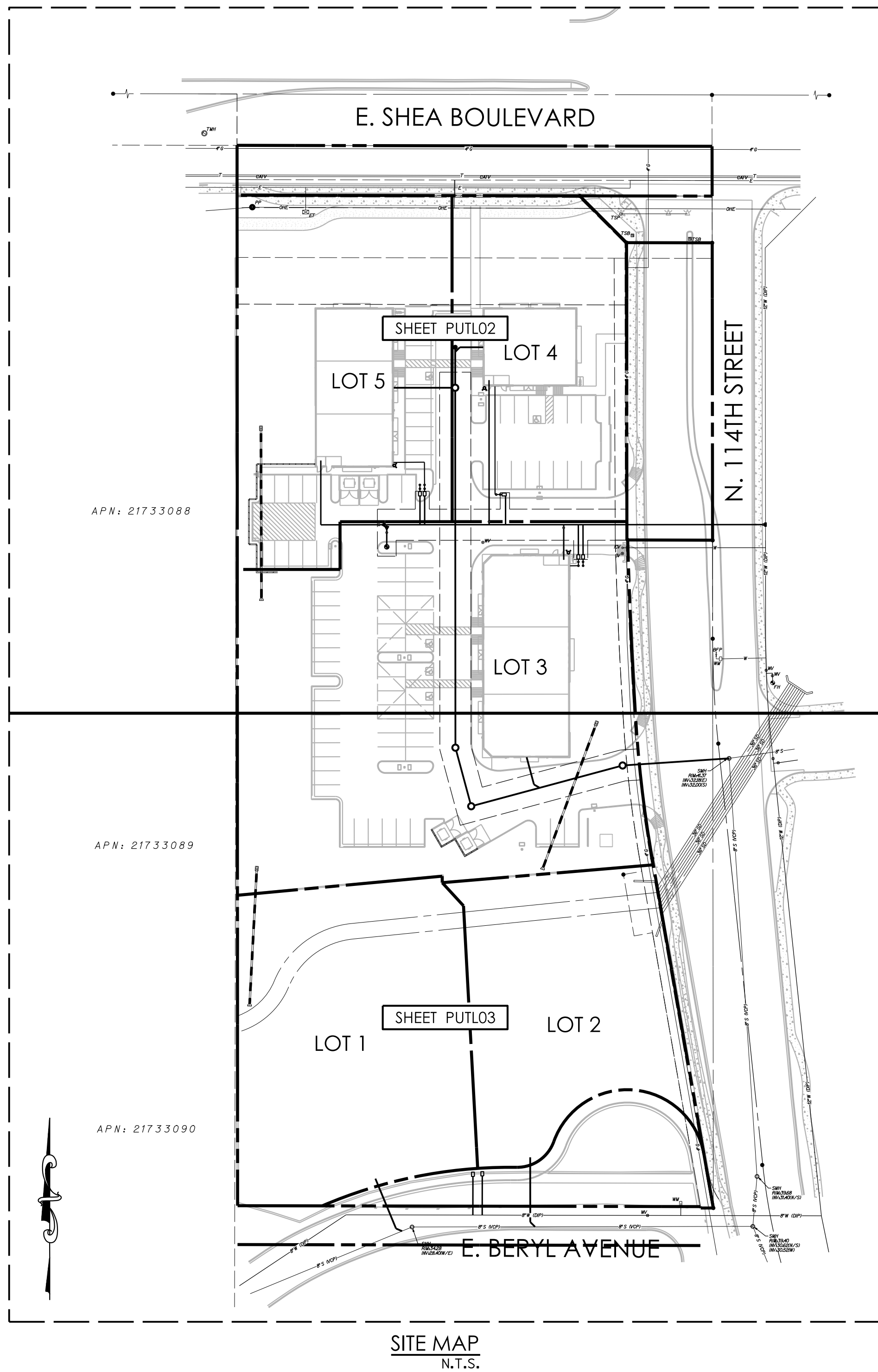
## *APPENDIX D*

### *Preliminary Sewer Plans*

LOCATED IN A PORTION OF THE NORTHWEST QUARTER OF SECTION 27, TOWNSHIP 3 NORTH, RANGE 5 EAST  
OF THE GILA AND SALT RIVER MERIDIAN, MARICOPA COUNTY, ARIZONA

1. ALL CONSTRUCTION IN THE PUBLIC RIGHTS-OF-WAY OR IN EASEMENTS GRANTED FOR PUBLIC USE MUST CONFORM TO THE LATEST MARICOPA ASSOCIATION OF GOVERNMENTS (MAG) UNIFORM STANDARD SPECIFICATIONS AND UNIFORM STANDARD DETAILS FOR PUBLIC WORKS CONSTRUCTION AS AMENDED BY THE LATEST VERSION OF THE CITY OF SCOTTSDALE STANDARD SPECIFICATIONS AND SUPPLEMENTAL STANDARD DETAILS. IF THERE IS A CONFLICT, THE CITY'S SUPPLEMENTAL STANDARD DETAILS WILL GOVERN.

6. PERMISSION TO WORK IN THE RIGHT-OF-WAY (PWR) PERMITS ARE REQUIRED FOR ALL WORKS WITHIN THE RIGHTS-OF-WAY AND EASEMENTS GRANTED FOR PUBLIC PURPOSES. COPIES OF ALL PERMITS MUST BE RETAINED ON-SITE AND BE AVAILABLE FOR INSPECTION AT ALL TIMES. FAILURE TO PRODUCE THE REQUIRED PERMITS WILL RESULT IN IMMEDIATE SUSPENSION OF ALL WORK UNTIL THE PROPER PERMIT DOCUMENTATION IS OBTAINED.



| INDEX OF SHEETS |                                        |
|-----------------|----------------------------------------|
| SHEET NO.       | DESCRIPTION                            |
| PUTL01          | COVER SHEET - PRELIMINARY UTILITY PLAN |
| PUTL02          | PRELIMINARY UTILITY PLAN               |
| PUTL03          | PRELIMINARY UTILITY PLAN               |

### FLOOD INSURANCE RATE MAP (FIRM) INFORMATION:

| COMMUNITY NUMBER | PANEL NUMBER | PANEL DATE       | SUFFIX | FIRM DATE        | FIRM ZONE | BASE FLOOD ELEVATION |
|------------------|--------------|------------------|--------|------------------|-----------|----------------------|
| 045012           | 1780         | OCTOBER 16, 2013 | L      | OCTOBER 16, 2013 | X         | N/A                  |

## ENGINEER'S CERTIFICATION:

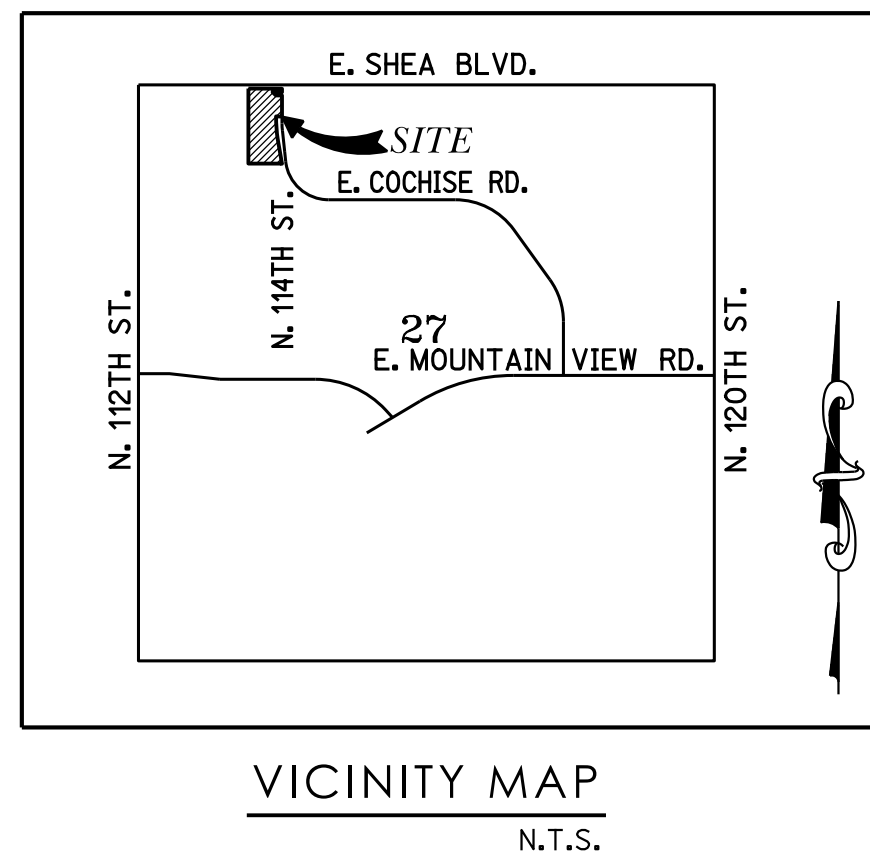
THE LOWEST FINISH FLOOR ELEVATION(S) AND/OR FLOOD PROOFING ELEVATION(S) ON THIS PLAN ARE SUFFICIENTLY HIGH TO PROVIDE PROTECTION FROM FLOODING CAUSED BY A 100-YEAR STORM, AND ARE IN ACCORDANCE WITH SCOTTSDALE REVISED CODE, CHAPTER 37 - FLOODPLAIN AND STORMWATER REGULATION.

| UTILITY     | UTILITY COMPANY    | NAME OF COMPANY REPRESENTATIVE | TELEPHONE NUMBER | DATE SIGNED |
|-------------|--------------------|--------------------------------|------------------|-------------|
| ELECTRIC    | APS                |                                |                  |             |
| TELEPHONE   | CENTURY LINK       |                                |                  |             |
| NATURAL GAS | SOUTHWEST GAS      |                                |                  |             |
| CABLE TV    | COX COMMUNICATIONS |                                |                  |             |
| OTHER       | A.T.&T.            |                                |                  |             |
| OTHER       |                    |                                |                  |             |

ENGINEER'S CERTIFICATION

I **DANIEL G. MANN**, AS THE ENGINEER OF RECORD FOR THIS DEVELOPMENT, HEREBY CERTIFY THAT ALL UTILITY COMPANIES LISTED ABOVE HAVE BEEN PROVIDED FINAL IMPROVEMENT PLANS FOR REVIEW, AND THAT ALL CONFLICTS IDENTIFIED BY THE UTILITIES HAVE BEEN RESOLVED. IN ADDITION NO CONFLICT FORMS HAVE BEEN OBTAINED FROM EACH UTILITY COMPANY AND ARE INCLUDED IN THIS SUBMITTAL.

SIGNATURE \_\_\_\_\_ DATE \_\_\_\_\_



PROJECT SCOPE:

THE SCOPE OF THIS PROJECT IS A NEW RETAIL DEVELOPEMENT, UTILITY IMPROVEMENTS AND LANDSCAPE.

CLIENT:

HAWKINS COMPANIES LLC  
4700 S. MCCLINTOCK DR. #160  
TEMPE, ARIZONA 85282

## ENGINEER:

3 ENGINEERING  
6370 E. THOMAS ROAD, SUITE #200  
SCOTTSDALE, ARIZONA 85251

CONTACT: MARK MITCHELL  
PHONE: (480) 223-8239  
EMAIL: MMITCHELL@HCOLLC.COM

CONTACT: DANIEL G. MANN, P.E.  
PHONE: (602) 334-4387  
EMAIL: DAN@3ENGINEERING.COM

ASSESSORS PARCEL NUMBERS:

PORTIONS OF APN 217-33-034G, 034F,  
034M, 034K, & 034N.

BENCHMARK:

3" BRASS DISK, LOCATED AT THE INTERSECTION OF SHEA  
BOULEVARD & 114TH STREET.

ELEVATION = 1444.23 (NAVD'88)  
CITY OF SCOTTSDALE DATUM

HEREBY CERTIFY THAT ALL ELEVATIONS REPRESENTED  
ON THIS PLAN ARE BASED ON THE ELEVATION DATUM FOR  
THE CITY OF SCOTTSDALE BENCHMARK PROVIDED ABOVE.




ENGINEER DATE

## BASIS OF BEARING

THE BASIS OF BEARING IS THE CENTERLINE OF EAST SHEA BOULEVARD, ALSO BEING THE NORTHERN SECTION LINE OF SECTION 27, TOWNSHIP 3 NORTH, RANGE 5 EAST, WHICH BEARS N89°53'00"W PER GPS COORDINATE OBSERVATIONS ARIZONA STATE PLANE, CENTRAL ZONE NAD83.

## ENGINEER'S STATEMENT:

THE ENGINEER OF RECORD ON THESE PLANS HAS RECEIVED A COPY OF THE APPROVED STIPULATIONS FOR THIS PROJECT AND HAS DESIGNED THESE PLANS IN CONFORMANCE WITH THE APPROVED STIPULATIONS

AS-BUILT CERTIFICATION:

I HEREBY CERTIFY THAT THE RECORD DRAWING MEASUREMENTS AS SHOWN HEREON WERE MADE UNDER MY SUPERVISION OR AS NOTED AND ARE CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

REGISTERED ENGINEER/LAND SURVEYOR

DATE \_\_\_\_\_

REGISTRATION NUMBER

**CIVIL APPROVAL**

## REVIEW &amp; RECOMMENDED APPROVAL BY:

|                                                                                                                                                                   |  |          |  |
|-------------------------------------------------------------------------------------------------------------------------------------------------------------------|--|----------|--|
| PAVING                                                                                                                                                            |  | TRAFFIC  |  |
| G & D                                                                                                                                                             |  | PLANNING |  |
| W & S                                                                                                                                                             |  | FIRE     |  |
| RET. WALLS                                                                                                                                                        |  |          |  |
| <div style="display: flex; justify-content: space-between; align-items: flex-end;"> <div>ENGINEERING COORDINATION MGR. (OR DESIGNEE)</div> <div>DATE</div> </div> |  |          |  |

C:\V\SecurSync\Projects\1831\_114th\_Shea\1831putl02.dgn

| MEASURED LINE TABLE |             |          |
|---------------------|-------------|----------|
| LINE                | BEARING     | DISTANCE |
| L1                  | N89°52'10"W | 8.12'    |

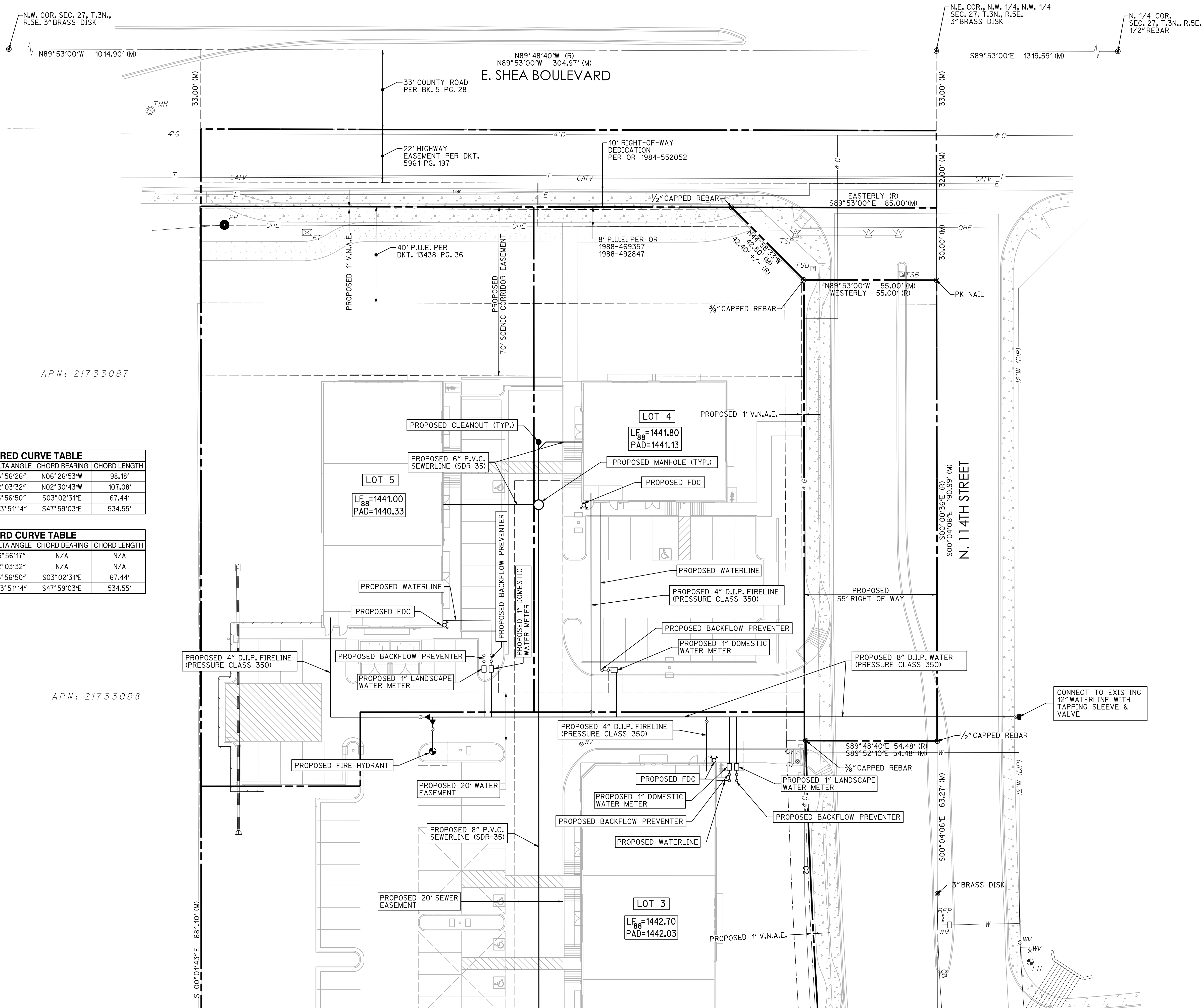
| RECORD LINE TABLE |             |          |
|-------------------|-------------|----------|
| LINE              | BEARING     | DISTANCE |
| L1                | N89°48'40"W | 8.12'    |

| MEASURED CURVE TABLE |            |          |             |               |              |
|----------------------|------------|----------|-------------|---------------|--------------|
| CURVE                | ARC LENGTH | RADIUS   | DELTA ANGLE | CHORD BEARING | CHORD LENGTH |
| C1                   | 98.24'     | 811.00'  | 6°56'26"    | N06°26'53"W   | 98.18'       |
| C2                   | 107.09'    | 2979.97' | 2°03'32"    | N02°30'43"W   | 107.08'      |
| C3                   | 67.47'     | 650.00'  | 5°56'50"    | S03°02'31"E   | 67.44'       |
| C4                   | 585.41'    | 400.00'  | 83°51'14"   | S47°59'03"E   | 534.55'      |

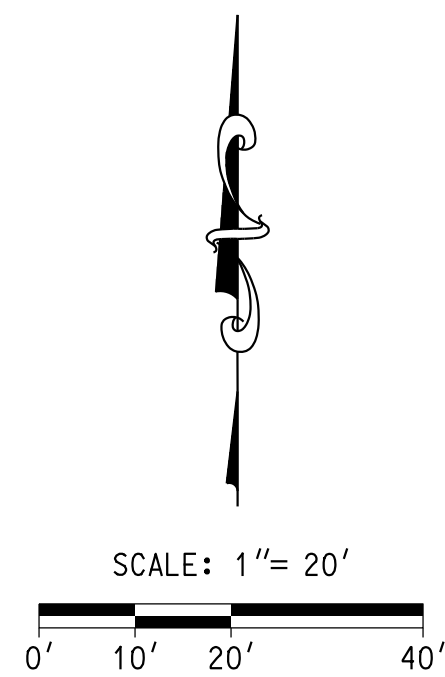
| RECORD CURVE TABLE |            |          |             |               |              |
|--------------------|------------|----------|-------------|---------------|--------------|
| CURVE              | ARC LENGTH | RADIUS   | DELTA ANGLE | CHORD BEARING | CHORD LENGTH |
| C1                 | 98.21'     | 811.00'  | 6°56'17"    | N/A           | N/A          |
| C2                 | 107.13'    | 2981.15' | 2°03'32"    | N/A           | N/A          |
| C3                 | 67.47'     | 650.00'  | 5°56'50"    | S03°02'31"E   | 67.44'       |
| C4                 | 585.41'    | 400.00'  | 83°51'14"   | S47°59'03"E   | 534.55'      |

APN: 21733087

APN: 21733088



MATCHLINE - SEE SHEET 3



Contact Arizona 911 at least two full working days before you begin excavation.  
Call 911 or click Arizona911.com

114TH & SHEA RETAIL  
SWC 114TH ST. & SHEA BLVD., SCOTTSDALE, AZ 85259  
PRELIMINARY UTILITY PLAN



300engineering  
planning  
civil engineering  
SURVEYING  
COPYRIGHT 2021 300ENGINEERING, LLC  
DESIGNED: D. MANN  
CHECKED: D. MANN  
DRAWN: D. MANN  
DATE: 11/03/21

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PHONE: (602) 334-4387  
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PROJECT NO.  
1831  
SHEET NO.  
PUTL02  
2 of 3

Q.S.#28-55

Registered Professional Engineer (CMT)  
 CERTIFICATE NO.  
 46857  
 DANIEL G. MANN  
 11/3/21  
 Date Signed  
 ARIZONA, U.S.A.

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