

PRELIMINARY DRAINAGE REPORT FOR FAIRMONT SCOTTSDALE PRINCESS PRIVADO WELCOME BUILDING AND PARKING MODIFICATIONS

October 12, 2022 WP# 215319



EXPIRES 06-30-25

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EXPIRES 06-30-25

1.0 INTRODUCTION

1.1 General Background

The Fairmont Scottsdale Princess Privado Welcome Building and Parking Modifications (Site) is a proposed welcome building and parking lot development on an approximate 6-acre parcel in the City of Scottsdale (APN#215-08-003C). The proposed development will consist of one (1) building and a new parking lot expansion. The project will include parking, hardscape, landscape, and utility improvements to support the development. The Site is located approximately 1,300 feet southeast of Scottsdale Road and Princess Boulevard, at the end of Cottage Terrace within Section 35, Township 4 North, Range 4 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. Refer to Exhibit 1 – *Vicinity Map* for project location. The existing property, currently zoned C-2, is primarily developed with buildings, parking lots, pools, sidewalks, and a variety of landscaping (desert and grass).

This Drainage Report has been prepared in accordance with Wood, Patel & Associates, Inc.'s (WOODPATEL's) understanding of the City of Scottsdale technical drainage requirements (Ref. 1) and the *Drainage Design Manuals for Maricopa County Hydrology and Hydraulics (2018),* as applicable to the Site.

1.2 FEMA Regulated Flood Zones

The Federal Emergency Management Agency (FEMA) publishes Flood Insurance Rate Map (FIRM) information for communities that adhere to FEMA regulations. The FEMA FIRM panel for this Site is 04013C1320L, effective date October 16, 2013, and indicates the Site falls within "Zone AO" shaded (Refer to Exhibit 2 – *FEMA FIRM*).

"Zone AO" shaded is defined by FEMA as follows:

"Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined".

It is the understanding of WOODPATEL, based on past experience and interpretations of the City of Scottsdale floodplain ordinance that development of land within FEMA Zone "AO" is acceptable as long as, in general, the lowest finish floor elevation is above or properly protected from the anticipated 100year water surface elevations. No problems are anticipated with developing the parcel in accordance with the City's floodplain ordinance. No underground structures or parking are proposed.

2.0 HYDROLOGY ANALYSIS

2.1 Offsite Hydrology

The proposed Site does not receive any offsite flows, only modifications to pre-existing flows from the Fairmont Scottsdale community. However, the community does receive offsite flows from a watershed north of the Site. See Appendix A - *Drainage Report for Fairmont Scottsdale Hotel Expansion by Wood, Patel & Associates, Inc., dated May 1, 2015,* which provides a history of the current offsite drainage and retention.

2.2 Onsite Hydrology

As a part of the proposed expansion, the existing storm drain and TPC Golf Course basin is planned to handle the increase in onsite flows from proposed improvements. All runoff from the proposed building and south of the building will sheet flow directly into the TPC Golf Course. Runoff from the proposed parking lot is designed to direct stormwater away from the proposed building and drain overland to existing catch basins and storm drain which ultimately outflows to the TPC Golf Course to the south. The flow into the existing storm drain system has increased by 0.3 cfs with the total increase in flow to the TPC Golf Course calculated to be 1.3 cfs for the 100-year condition. See Appendix G - *Hydrologic and Hydraulic Calculations* for the existing versus proposed drainage calculations.

Onsite peak flow estimates for the proposed development were generated using the Rational Method, as outlined in the *Drainage Design Manual for Maricopa County, Arizona: Volume I – Hydrology (*Ref 2). NOAA Atlas 14 precipitation data was obtained and utilized to develop Intensity-Duration-Frequency (I-D-F) curves for the Site. Rational Method peak flows were computed at concentration points within the Site at key design locations. Runoff coefficients were estimated to reflect post-development land use conditions for the 2-year, 10-year, and 100-year events (Refer to Appendix G – *Hydrologic and Hydraulic Calculations*).

Appendix A - Drainage Report Fairmont Scottsdale Hotel Expansion by Wood, Patel & Associates, Inc., dated May 1, 2015, provides a history of the current onsite drainage and retention. Based on the information above, the proposed Site improvements mimic current drainage patterns and areas of retention for onsite with very minimal alteration.

2.3 Establishing Lowest Finish Floor (LF88 Elevations)

The Grading and Drainage Plan has been designed to comply with the City of Scottsdale floodplain ordinance for a Zone "AO" floodplain. It is our understanding, unless other flood proof measures are presented and approved, the proposed Lowest habitable Finished Floor (LFF) elevation must be designed a minimum of 1 foot above the anticipated 100-year flood elevation. Scottsdale currently requires the lowest finished floor elevation of 1 foot above the flood depth, which results in a finished floor elevation of 2 feet above the Highest Adjacent natural Grade (HAG) to the proposed building which would be the regulatory flood elevation. Due to the Site being disturbed after the Zone "AO" Special Flood Hazard was established, the current condition of the Site cannot determine the HAG. Due to this change the HAG must be established using topographical information showing the pre disturbed condition of the Site.

According to Curry's Corner 7.5-minute Topographic Survey Map by USGS from 1964 with a contour interval of 10 feet, the approximate highest natural grade of this Site prior to development must be changed from the NAVD29 datum to the NAVD88 datum. This change consists of an elevation increase of 1.749 feet determined using surveyed elevations of a nearby monument on both vertical datums.

Using Auto CAD Civil 3D, the quad map was aligned to the site using common monument lines (section lines) contained within the quad map and previously surveyed by WOODPATEL. The 10-foot interval contours were digitized, adjusting to NAVD88 and applied to a TIN surface model. The surface model was supplemented with break lines at estimated ridge and flowline locations. The surface was used to display interpolated 1-foot contours for the pre-disturbed condition, the proposed building limits were overlaid on the contour map and the HAG was determined for the proposed building. The proposed building lowest finished floor elevation is a minimum of 2 feet above the HAG. Refer to Appendix E - *Regional Contour Map / Opinion of Existing Highest Natural Grade Elevation* and Appendix F - *Curry's Corner Quadrangle Map.* Overlaying the building over the adjusted digitized lowest finish floor elevation map was determined to be 1541.00 making it 1.2-feet above the regulatory flood elevation of 1539.80.

3.0 HYDRAULIC ANALYSIS

The Site was designed to utilize the existing storm drain system in Cottage Terrace Drive and to maintain the existing site outfall locations at the southwest corner of the property. The existing storm drain system was designed to capture runoff through catch basins at low points throughout the Site and convey it to the TPC Golf Course. With the minimal increase in runoff from the proposed improvements, the storm drain system will capture the 10-year storm event. As previously designed, runoff from the 100-year storm event was expected to exceed capacity of the storm drain system and the excess will be conveyed overland (south on Cottage Terrace Drive) to the outfall at the southwest corner of the Site and to the TPC golf course. The additional flow for the 100-year event will continue to exceed capacity and overland flow south to the Site outfall. See Appendix A – Drainage Report for Fairmont Scottsdale Hotel Expansion by Wood, Patel & Associates, Inc., dated May 1, 2015, for the previously designed system. The increase in runoff is not significant enough to affect the existing drainage conditions and therefore no additional stormwater infrastructure is required.

4.0 MAINTENANCE

Ongoing maintenance of the designed or recommended drainage systems is required to preserve the design integrity and purpose of the drainage system. Failure to provide maintenance can prevent the drainage system from performing to its intended design purpose and can result in reduced performance. Maintenance within the public right-of-way is the responsibility of the governing municipality. However, it is the responsibility of landowners (such as private developers or property owners' associations) for facilities on private property. Prior to ultimate condition build-out upstream of drainage structures, additional maintenance may be required due to an increase in sedimentation build-up. A regular maintenance program is required to have drainage systems perform to the level of protection or service, as presented in this report and the projects' plans and specifications.

5.0 CONCLUSIONS

Based on our analysis of the Site, the following conclusions can be made:

- 1. This Drainage Report has been prepared in accordance with WOODPATEL's understanding of the City of Scottsdale technical drainage requirements and the *Drainage Design Manuals for Maricopa County Hydrology and Hydraulics (2018)*, as applicable to the Site.
- 2. The Site is within a FEMA designated 100-year floodplain (Zone "AO-Depth 1 foot") in both pre- and postdevelopment conditions.
- 3. The Site is protected from offsite flows from the north by improvements, as outlined in the approved stormwater storage waiver.
- 4. No stormwater retention has been provided for this project, per the approved stormwater storage waiver.
- 5. The onsite 100-year storm event is to be conveyed south, by existing storm drain and overland flow, to the existing TPC Golf Course.
- 6. The 100-year high water elevation is 1537.80 in the adjacent TPC golf course, which is the same estimated highest natural ground elevation. This is 3.2 feet below the proposed Welcome Center lowest finish floor elevation of 1541.00. It is our understanding this is in compliance with the City of Scottsdale floodplain ordinance, which requires the lowest finished floor to be a minimum of 2 feet above (1 foot for AO and 1 foot of freeboard) the natural highest grade.
- Ongoing maintenance is required for the existing drainage systems to assure design performance. Maintenance is the responsibility of the private parties involved.

6.0 REFERENCES

- 1. Design Standards & Policies Manual, City of Scottsdale, 2018
- Drainage Report for Fairmont Scottsdale Hotel Expansion by Wood, Patel & Associates, Inc., dated May 1, 2015
- 3. Drainage Report for Fairmont Scottsdale Western Theme Town by Wood, Patel & Associates, Inc., dated May 15, 2015
- 4. Drainage Memo for Fairmont Scottsdale Sunset Beach Pool by Wood, Patel & Associated, Inc., dated September 11, 2015
- 5. Curry's Corner Quadrangle, 7.5 Minute Series Topographic Map, USGS, 1964.

APPENDIX A – DRAINAGE REPORT FOR FAIRMONT SCOTTSDALE HOTEL EXPANSION BY WOOD, PATEL & ASSOCIATES, INC., DATED MAY 1, 2015

DRAINAGE REPORT FOR FAIRMONT SCOTTSDALE HOTEL EXPANSION

May 1, 2015 WP# 154302 C.O.S. No.: 1217-PA-2014

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APPENDICES

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- Appendix B Stormwater Storage Waiver / Proposed Drainage Improvements Exhibit
- Appendix C City of Scottsdale Forms
- Appendix D Offsite Watershed Exhibits
 - Regional Contour Map / Opinion of Existing Highest Natural Grade Elevation
 - Aerial Map
 - Plate 1 Exhibit (From Core North/Core South Drainage Study)
 - Table 1 Spreadsheet (From Core North/Core South Drainage Study)

EXHIBITS

- Exhibit 1 Vicinity Map
- Exhibit 2 FEMA Map
- Exhibit 3 Maravilla East Property Line Road Drainage Map
- Exhibit 4 Existing Drainage Map
- Exhibit 5A Proposed Drainage Map 1
- Exhibit 5B Proposed Drainage Map 2

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WOOD/PATEL MISSION: CLIENT SERVICE

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1.0 INTRODUCTION

The proposed Fairmont Scottsdale Hotel Expansion (Fairmont) is a new 102-room (144-bed) hotel building with associated parking. The proposed development is located at the existing Fairmont Scottsdale Hotel, located east of Scottsdale Road and south of Princess Drive. The project includes one (1) new building and a new parking lot expansion (refer to Exhibit 1 – *Vicinity Map*). The parking lot expansion is located along Cottage Terrace, on approximately 1.22 acres (ac), and is west of the existing parking lot for the Ballroom. The building addition is located on approximately 2.19 ac, and is north of the tennis cottages on the east side of Cottage Terrace. More specifically, the sites are located in the southwest quarter of Section 35, Township 4 North, Range 4 East of the Gila and Salt River Meridian (refer to Exhibit 1). The existing property, currently zoned C-2, is primarily developed with buildings, parking lots, tennis courts, sidewalks, and a variety of landscaping (desert and grass). There is some undeveloped area at the northeast corner of the property that is currently dirt and decomposed granite. Current zoning is expected to stay the same for the proposed improvements.

The purpose of this report is to obtain City of Scottsdale Approval for the proposed Fairmont Scottsdale Hotel Expansion, with respect to the City of Scottsdale's drainage criteria.

Analysis and modeling for this study was performed in accordance with the requirements of the *Design Standards and Policies Manual, Chapter 4: Grading and Drainage* (DS&PM), City of Scottsdale, 2010 (Ref. 1). The methods of analysis, sources of data and assumptions, and the results of the analysis are discussed in detail in the following sections of this report.

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2.0 EXISTING DRAINAGE CONDITIONS AND CHARACTERISTICS

2.1 FEMA Floodplain

The Federal Emergency Management Agency (FEMA) has published a 100-year floodplain, per Flood Insurance Rate Map (FIRM) Panel 1320 of 4425, Map Number 04013C1320L, dated October 16, 2013. The site is within a flood zone labeled "AO-Depth 1 Foot, Velocity 3 FPS".

Zone "AO" is defined by FEMA as follows:

"Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined."

As displayed on the FEMA FIRM panel, the floodplain delineation includes a large portion of land, which includes this project site (refer to Exhibit 2 – *FEMA Map*).

It is the understanding of Wood, Patel & Associates, Inc. (Wood/Patel), based on past experience and interpretations of the City of Scottsdale's floodplain ordinance that development of land within a FEMA Zone "AO" is acceptable as long as, in general, the lowest floor elevation is above or properly protected from the anticipated 100-year water surface elevations. No problems are anticipated with developing the parcel in accordance with the City's floodplain ordinance. No underground structures or parking structures are proposed.

2.2 Offsite Drainage Conditions

A large watershed occurs north of the Fairmont Scottsdale Resort. The watershed has been previously studied and peak flows estimated for existing and proposed conditions, and results were published in the Core North/Core South Drainage Study (Ref. 5). Refer to Appendix D - Offsite Watershed Exhibits for the Regional Contour Map, Aerial Map, and Plate 1 and Table 1 from the Core North/Core South study and report (Ref. 5). The Fairmont Scottsdale Resort property is protected from this offsite flow by several past improvements. Originally, when Princess Boulevard was constructed, offsite flows were diverted to the west by a channel on the north site of Princess Boulevard, just east of waters are routed to an existing drainage culvert under Princess Boulevard, just east of

Scottsdale Road. These flood waters are routed south along the east side of Scottsdale Road to the Tournament Players Club (TPC) Golf Course. As the Fairmont Scottsdale Resort continued to develop over the years, the City of Scottsdale requested additional improvements to protect the property from offsite flows from the north. Additional improvements were contingent with the Request for Stormwater Storage Waiver for the Fairmont Scottsdale Resort property in 2008 as development continued (refer to Exhibit B – Stormwater Storage Waiver / Proposed Drainage Improvements Exhibit). The additional improvements included adding a flood/screen wall along the south side of Princess Boulevard, providing a high point on Cottage Terrace just south of Princess Drive, providing a high point just south of the traffic circle on Princess Drive, providing a channel and floodwall along the west side of the Maravilla site, and removing and replacing the existing culverts on Princess Drive with a new bridge structure. Currently, all of these improvements have been constructed with past projects (Fairmont Ballroom Addition and the Maravilla Senior Living Community) with the exception of replacing the culverts on Princess Drive with a bridge structure. It is our understanding the Fairmont ownership is working with the City of Scottsdale on an agreement to replace these culverts.

The Hayden/Rhodes (Granite Reef) Aqueduct was constructed as part of the (CAP) by the U.S. Bureau of Reclamation. This existing aqueduct is located along the south edge of the TPC golf course and extends east-west the length of CAP canal. The aqueduct is at a significantly higher elevation than the golf course and acts as a dike which creates stormwater ponding in the existing golf course. The 100-year high water level is at an elevation of 1536.00 NGVD 29 Datum (Ref. 6). The conversion to the NAVD 88 Datum is 1.8 feet, so the 100-year high water level elevation is approximately 1537.80. The proposed finish floor for the new hotel building will be at elevation 1550.41, or 12.61 feet above the 100-year high water elevation.

When the Maravilla Senior Living Community was constructed, Cottage Terrace Drive (a private drive) was relocated to the west side of the Fairmont Scottsdale Resort property to accommodate vehicle access to the Resort. A storm drain system was installed with the driveway improvements to benefit both properties and assumed developed conditions for both sites. Based on new topography obtained for the Fairmont Scottsdale Hotel addition, we have re-analyzed this storm drain system due to an increase in tributary area

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from the Fairmont site. Existing drainage areas E2 and E3 have been adjusted accordingly from their original boundaries, and the "B" drainage areas added from the Ballroom. The "M" drainage areas on Maravilla have been revised to more accurately represent the contributing areas to the existing storm drain system in Cottage Terrace. Refer to Exhibit 3 - Maravilla East Property Line Road Drainage Map, and Exhibit 4 for a comparison. In addition, existing drainage area E2 has an existing 1-foot deep sediment basin which acts as a retention basin. Therefore, we have not included this in the flows for the existing storm drain system since the volume provided exceeds the volume required for the 100-year, 2-hour storm (refer to Appendix A – Hydrologic and Hydraulic Calculations).

Runoff from the existing Princess MXD Mercer Institute (MXD) site drains to the west to the existing flood control channel located between Scottsdale Road and the Maravilla project (Exhibit 4).

2.3 Onsite Drainage Conditions (Pre-Development)

The existing sites slope generally from the northeast to the southwest with an average slope of approximately two percent (2%). Stormwater is captured into an existing storm drain system via existing catch basins and curb inlets located within Cottage Terrace Drive (private drive), which ultimately drains into the existing TPC Golf Course at the Fairmont Scottsdale Princess Resort, located immediately north of the Hayden-Rhodes Aqueduct. Currently, the site consists of parking lots, tennis courts, sidewalks, and a variety of desert landscaping (desert and grass). There is some undeveloped area at the northeast corner of the property that is currently dirt and decomposed granite (refer to Exhibit 4).

When the existing storm drain system was first analyzed as part of the Maravilla East Property Line Road, all the contributing areas were assigned a weighted C-value of 0.90 for the 100-year storm event to account for future buildout in those areas, which accounted for approximately 20.5 ac. Since that time, Maravilla, the Ballroom, and the Maravilla East Property Line Road have all been constructed, and a detailed topographic survey of the area for this project has been completed. In re-analyzing the data from those completed projects and the current survey information, the existing tributary drainage area boundaries were adjusted (refer to Section 2.2 of this report) and weighted C-values were calculated. An adjusted C-value of 0.77 was calculated for the combined existing storm

tributary area, which accounts for approximately 22.0 ac. Even though the overall tributary area went up 1.5 ac, the overall runoff coefficient went down. The result is the existing storm drain system still has adequate capacity for the 10-year storm event, with the 100-year storm flowing overland within Cottage Terrace, as originally designed. Refer to the *Drainage Report for Maravilla East Property Line Road*, dated May 12, 2008, revised October 15, 2008 (Ref. 4), and Appendix A for specific tributary area and C-value information.

3.0 PROPOSED DRAINAGE PLAN

3.1 Onsite Drainage Conditions (Post-Development)

This site was designed to utilize the existing storm drain system in Cottage Terrace Drive and to maintain the existing site outfall locations at the southwest corner of the property. During a 10-year storm event, the proposed storm drain system is designed to capture the runoff through a series of roof drains and catch basins at low points throughout the project site and convey it to the existing storm drain system located within Cottage Terrace. During a 100-year storm event, the runoff is expected to exceed the capacity of the proposed storm drain system and the excess will be conveyed overland (south on Cottage Terrace Drive) to the outfall at the southwest corner of the site and to the TPC golf course. The proposed grading is designed to allow for no more than 1.8 feet of ponding before the runoff overtops the local high points and continue towards the outfall location. The existing onsite private storm drain has also been extended to the outfall location of the new parking lot expansion where a new catch basin has been added. A catch basin has also been added to the existing sediment basin adjacent to the ultimate outfall along Hacienda Way. This portion of new storm drain has also been designed to accept runoff from a future Western Theme Town that will be located in this area (refer to Exhibits 5A and 5B – Proposed Drainage Maps 1 & 2 for location). Cottage Terrace Drive has been designed to handle the 100-year flow, per the approved Drainage Report for Maravilla East Property Line Road, dated May 12, 2008, revised October 15, 2008 (Ref. 4).

The tributary areas contributing to the existing storm drain system within Cottage Terrace is approximately 22.0 ac. The combined weighted C-value has been calculated for all the tributary areas contributing to the existing storm drain system, including the proposed improvements. The combined weighted C-value has been calculated to be 0.78. Refer to Appendix A for specific tributary area and C-value information, and Exhibits 5A and 5B for tributary area delineation. For a summary of the pre- versus post-weighted C-Values, see the table below:

Drainage Condition	Tributary Area (ac)	Weighted C-Value
Maravilla	20.5	0.90
Pre-Development (including Maravilla)	22.0	0.77
Post-Development (including Maravilla)	22.0	0.78
Pre-Fairmont	8.3	0.59
Post-Fairmont	8.3	0.61

Tributary areas, flow directions, catch basins, storm drains, 100-year overland flow paths, and the ultimate site outfall location are labeled on Exhibit 5A and 5B. The downstream location of the existing storm drain system that the proposed system is to tie into can be seen on Exhibit 4. Storm drain sizing calculations are provided in Appendix A.

3.2 Lowest Habitable Finished Floor Elevations

The Grading and Drainage Plan has been designed to comply with the City of Scottsdale's floodplain ordinance for a Zone "AO" floodplain. It is our understanding, unless other flood proof measures are presented and approved, the proposed lowest habitable finished floor (LFF) elevation must be designed a minimum of 1 foot above the anticipated 100-year flood elevation. As previously mentioned, the proposed finish floor for the new hotel building will be at elevation 1550.41, or 12.61 feet above the 100-year high water elevation in the TPC Golf Course. In addition, the lowest finish floor elevation is designed to be a minimum of 1 foot above the adjacent finish grade. Refer to Exhibits 5A and 5B for graphical representation.

The Site is located within a Zone "AO" floodplain, defined as having a flood depth of 1 foot. Therefore, the anticipated 100-year flood elevation is 1 foot above "natural" grade, and proposed LFF elevation must have 1 foot additional freeboard or be a minimum of 2 feet above natural grade. Due to the disturbed condition of the Site from previous development, the natural grade has been modified. According to Curry's Corner 7.5 minute Topographic Survey Map by USGS from 1964 with a contour interval of 10-feet, the approximate highest natural grade of this site prior to development is 1545.00, which is 5.41 feet lower than the proposed lowest finish floor of 1550.41. It is our understanding this is in compliance with the City's floodplain ordinance. Refer to the *Regional Contour Map* within Appendix D, which shows the highest natural grade elevation relative to the proposed building location.

3.3 Stormwater Retention

A Request for Stormwater Storage Waiver was submitted and approved by the City of Scottsdale on October 23, 2008 (refer to Appendix B). Therefore, onsite stormwater retention is not required.

3.4 Operation and Maintenance

The property owner shall be solely responsible for the operation and maintenance of the stormwater drainage system.

4.0 SPECIAL CONDITIONS

4.1 Special Conditions

Currently, there are no washes with 100-year flows greater than 50 cfs that traverse the project site. Also, there are no designated Section 404 washes within the site; therefore, no Section 404 permit is required.

5.0 DATA ANALYSIS

5.1 Hydrologic Analysis

The drainage improvements are to be developed consistent with Chapter 4 of the City of Scottsdale *Design Standards and Policies Manual*, 2010. The Rational Method has been used to quantify peak discharge values for onsite concentration points for the proposed build out scenario during the 10-year and 100-year storm events. Weighted "C" runoff coefficients were referenced from Chapter 4 of the City of Scottsdale *Design Standards and Policies Manual*, 2010. Refer to Appendix A – *Hydrologic and Hydraulic Calculations* for rational calculation printouts, and Exhibit 5 – *Proposed Drainage Map 1* for drainage basin tributary areas and concentration point locations.

5.2 Hydraulic Analysis

The onsite storm drain system has been designed to accommodate the 10-year storm event. Bentley StormCAD Version 8i was utilized to analyze the existing and proposed storm sewer system. StormCAD printouts and storm drain profiles can be located in Appendix A.

6.0 CONCLUSIONS

The project site is believed to be capable of development as presented with regard to the City of Scottsdale's onsite and offsite drainage requirements. The project's drainage highlights are as follows:

- The site is protected from offsite flows from the north by improvements, as outlined in the approved stormwater storage waiver.
- The proposed drainage infrastructure has been designed in accordance with the City of Scottsdale *Design Standards & Policies Manual*, 2010. The Rational Method was used to estimate peak discharges for all drainage areas. The flow capacities of the proposed storm system have been designed to accept the 10-year storm event.
- No stormwater retention has been provided for this project, per the approved stormwater storage waiver from the City of Scottsdale.
- The onsite 100-year storm event is to be conveyed south, by storm drain and overland flow, to the existing TPC Golf Course.
- The 100-year high water elevation is 1537.80 in the adjacent TPC golf course, which is 12.61 feet below the proposed new building lowest finish floor elevation of 1550.41.
- The project site is in a FEMA designated 100-year floodplain (Zone "AO-Depth 1 foot") in both pre- and post-development conditions.
- The lowest finish floor elevation of 1550.41 is approximately 5.41 feet higher than the highest natural ground elevation of 1545.00. It is our understanding this is in compliance with the City's floodplain ordinance, which requires the lowest finished floor to be a minimum of 2 feet above (1 foot for AO and 1 foot of freeboard) the natural highest grade.



7.0 REFERENCES

- 1. Design Standards and Policies Manual, Chapter 4: Grading and Drainage, City of Scottsdale, January 2010.
- 2. Onsite Drainage Report for Fairmont Scottsdale Ballroom Addition, Wood, Patel & Associates, Inc., July 14, 2011.
- 3. Offsite Drainage Report for Fairmont Scottsdale Ballroom Addition, Wood, Patel & Associates, Inc., July 14, 2011.
- 4. *Drainage Report for Maravilla East Property Line Road*, Wood, Patel & Associates, Inc., dated May 12, 2008, revised October 15, 2008.
- 5. *Drainage Study Core North/Core South Scottsdale, AZ*, Robert Ward, P.E., Consulting Engineering, September 25, 2001. Prepared for Arizona State Land Department.
- Sedimentation Report Reach 11 Dikes Hayden/Rhodes (Granite Reef) Aqueduct, Earth Science Division, Surface Water Branch, Sedimentation Office of Denver, Colorado, July 1990.
- Maravilla Scottsdale Senior Living Community, Wood, Patel & Associates, Inc., December 18, 2008, Revised February 13, 2009.
- 8. Curry's Corner Quadrangle, 7.5 Minute Series Topographic Map, USGS, 1964.

APPENDIX A

HYDROLOGIC AND HYDRAULIC CALCULATIONS

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Site I-D-F Curve

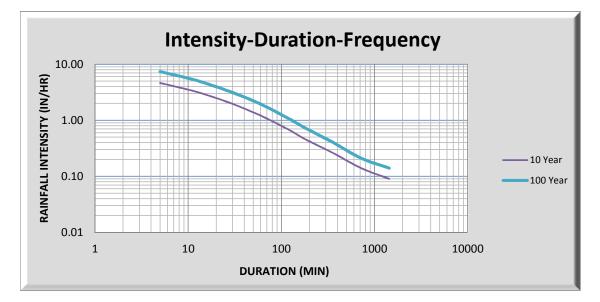
Description:	Rational Method IDF Curve
Location:	Fairmont Scottsdale Hotel Expansion
	City of Scottsdale, Arizona

NOAA ATLAS 14 RAINFALL DEPTHS, INCHES

Duration		Average	e Recurrence Ir	nterval (years)		
Duration	2	5	10	25	50	100
5-min	0.257	0.346	0.415	0.507	0.578	0.651
10-min	0.391	0.526	0.631	0.772	0.880	0.990
15-min	0.484	0.652	0.782	0.957	1.090	1.230
30-min	0.651	0.879	1.050	1.290	1.470	1.650
60-min	0.806	1.090	1.300	1.600	1.820	2.050
2-hr	0.931	1.240	1.480	1.800	2.040	2.290
3-hr	1.020	1.330	1.580	1.920	2.200	2.480
6-hr	1.210	1.540	1.810	2.170	2.450	2.750
12-hr	1.360	1.720	2.000	2.380	2.670	2.970
24-hr	1.610	2.070	2.450	2.970	3.380	3.810

RAINFALL INTENSITY, INCHES/HOUR

Duration	Frequency, years									
minutes	2	5	10	25	50	100				
5	3.08	4.15	4.98	6.08	6.94	7.81				
10	2.35	3.16	3.79	4.63	5.28	5.94				
15	1.94	2.61	3.13	3.83	4.36	4.92				
30	1.30	1.76	2.10	2.58	2.94	3.30				
60	0.81	1.09	1.30	1.60	1.82	2.05				
120	0.47	0.62	0.74	0.90	1.02	1.15				
180	0.34	0.44	0.53	0.64	0.73	0.83				
360	0.20	0.26	0.30	0.36	0.41	0.46				
720	0.11	0.14	0.17	0.20	0.22	0.25				
1440	0.07	0.09	0.10	0.12	0.14	0.16				





NOAA Atlas 14, Volume 1, Version 5 Location name: Scottsdale, Arizona, US* Latitude: 33.6488°, Longitude: -111.9060° Elevation: 1583 ft* * source: Google Maps



Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹												
Duration				Avera	ge recurrend	e interval (y	ears)					
Duration	1	2	5	10	25	50	100	200	500	1000		
5-min	0.196	0.257	0.346	0.415	0.507	0.578	0.651	0.723	0.821	0.896		
	(0.163–0.241)	(0.215-0.315)	(0.287-0.422)	(0.342-0.505)	(0.412-0.615)	(0.463-0.696)	(0.512-0.782)	(0.560-0.867)	(0.620-0.987)	(0.663–1.08)		
10-min	0.299	0.391	0.526	0.631	0.772 0.880		0.990	1.10	1.25	1.36		
	(0.248-0.366)	(0.326-0.479)	(0.436-0.643)	(0.520-0.768)	(0.626-0.936) (0.705-1.06)		(0.780-1.19)	(0.852–1.32)	(0.943-1.50)	(1.01–1.64)		
15-min	0.371	0.484	0.652	0.782	0.957	1.09	1.23	1.36	1.55	1.69		
	(0.308-0.454)	(0.405-0.593)	(0.540-0.797)	(0.645-0.953)	(0.777-1.16)	(0.874–1.31)	(0.966-1.48)	(1.06-1.64)	(1.17–1.86)	(1.25–2.03)		
30-min	0.499	0.651	0.879	1.05	1.29	1.47	1.65	1.84	2.09	2.28		
	(0.414-0.611)	(0.545-0.799)	(0.728-1.07)	(0.868-1.28)	(1.05–1.56)	(1.18–1.77)	(1.30-1.99)	(1.42–2.20)	(1.57-2.51)	(1.68–2.74)		
60-min	0.617	0.806	1.09	1.30	1.60	1.82	2.05	2.27	2.58	2.82		
	(0.513-0.756)	(0.674-0.989)	(0.901–1.33)	(1.08–1.59)	(1.29–1.93)	(1.46-2.19)	(1.61–2.46)	(1.76-2.73)	(1.95–3.10)	(2.08–3.39)		
2-hr	0.720	0.931	1.24	1.48	1.80	2.04	2.29	2.55	2.89	3.15		
	(0.606-0.862)	(0.786-1.12)	(1.04–1.48)	(1.22–1.76)	(1.48-2.13)	(1.65–2.41)	(1.83–2.70)	(1.99-3.00)	(2.21-3.40)	(2.36–3.73)		
3-hr	0.795 (0.670-0.973)	1.02 (0.861-1.25)	1.33 (1.12–1.63)	1.58 (1.31–1.92)	1.92 (1.58–2.32)	2.20 (1.78–2.64)	2.48 (1.97-2.98)	2.78 (2.17-3.33)	3.19 (2.42-3.82)	3.52 (2.61–4.22)		
6-hr	0.958	1.21	1.54	1.81	2.17	2.45	2.75	3.05	3.45	3.77		
	(0.823-1.14)	(1.04–1.44)	(1.32–1.82)	(1.53–2.13)	(1.81–2.54)	(2.02–2.86)	(2.23-3.20)	(2.43-3.56)	(2.68-4.03)	(2.86-4.41)		
12-hr	1.08	1.36	1.72	2.00	2.38	2.67	2.97	3.27	3.67	3.98		
	(0.933-1.27)	(1.17-1.60)	(1.48–2.01)	(1.71–2.33)	(2.01–2.77)	(2.22–3.10)	(2.44-3.44)	(2.65–3.79)	(2.90-4.28)	(3.09-4.67)		
24-hr	1.27	1.61	2.07	2.45	2.97	3.38	3.81	4.26	4.89	5.39		
	(1.11-1.47)	(1.41–1.86)	(1.81–2.41)	(2.13–2.83)	(2.56-3.43)	(2.89–3.89)	(3.22-4.40)	(3.56–4.92)	(4.01–5.65)	(4.36-6.26)		
2-day	1.37	1.75	2.29	2.72	3.32	3.80	4.30	4.83	5.56	6.15		
	(1.19–1.59)	(1.52–2.02)	(1.98–2.64)	(2.35–3.13)	(2.84-3.82)	(3.22–4.37)	(3.62–4.96)	(4.02–5.59)	(4.55-6.46)	(4.95-7.17)		
3-day	1.47	1.88	2.48	2.96	3.65	4.19	4.78	5.40	6.27	6.97		
	(1.29–1.69)	(1.65–2.16)	(2.17–2.84)	(2.58-3.39)	(3.15-4.17)	(3.60–4.79)	(4.06-5.48)	(4.54-6.21)	(5.19-7.23)	(5.70-8.08)		
4-day	1.58	2.02	2.67	3.21	3.97	4.59	5.26	5.96	6.97	7.80		
	(1.39–1.80)	(1.78–2.30)	(2.35-3.04)	(2.81–3.65)	(3.46-4.51)	(3.97–5.22)	(4.51–5.99)	(5.06-6.82)	(5.83-8.00)	(6.44-8.99)		
7-day	1.78	2.28	3.03	3.64	4.51	5.22	5.97	6.79	7.94	8.89		
	(1.56–2.05)	(2.00-2.61)	(2.65–3.46)	(3.17–4.15)	(3.91–5.14)	(4.49–5.95)	(5.10-6.83)	(5.73-7.80)	(6.61–9.15)	(7.30–10.3)		
10-day	1.94 (1.70-2.22)	2.48 (2.18–2.84)	3.29 (2.89–3.75)	3.94 (3.45–4.49)	4.87 (4.23–5.53)	5.62 (4.85-6.39)	6.42 (5.49-7.32)	7.27 (6.16-8.31)	8.48 (7.08-9.73)	9.46 (7.79–10.9)		
20-day	2.40 (2.12–2.74)	3.10 (2.73-3.52)	4.10 (3.61–4.65)	4.86 (4.26–5.51)	5.90 (5.15-6.69)	6.71 (5.83-7.61)	7.54 (6.51-8.58)	8.39 (7.19-9.58)	9.55 (8.10-11.0)	10.5 (8.78–12.1)		
30-day	2.82 (2.48-3.21)	3.64 (3.21-4.13)	4.81 (4.23–5.45)	5.70 (5.00-6.45)	6.91 (6.03-7.82)	7.85 (6.82–8.88)	8.82 (7.62-9.98)	9.80 (8.41-11.1)	11.1 (9.47–12.7)	12.2 (10.3-14.0)		
45-day	3.30 (2.92–3.74)	4.26 (3.77-4.82)	5.63 (4.97–6.36)	6.65 (5.86-7.52)	8.02 (7.03–9.06)	9.06 (7.90–10.2)	10.1 (8.78–11.5)	11.2 (9.65–12.7)	12.6 (10.8–14.5)	13.8 (11.6–15.8)		
60-day	3.67 (3.26-4.14)	4.75 (4.21–5.35)	6.26 (5.54–7.05)	7.37 (6.50-8.30)	8.82 (7.76-9.94)	9.92 (8.68–11.2)	11.0 (9.59–12.5)	12.1 (10.5–13.7)	13.6 (11.7–15.5)	14.7 (12.5–16.8)		

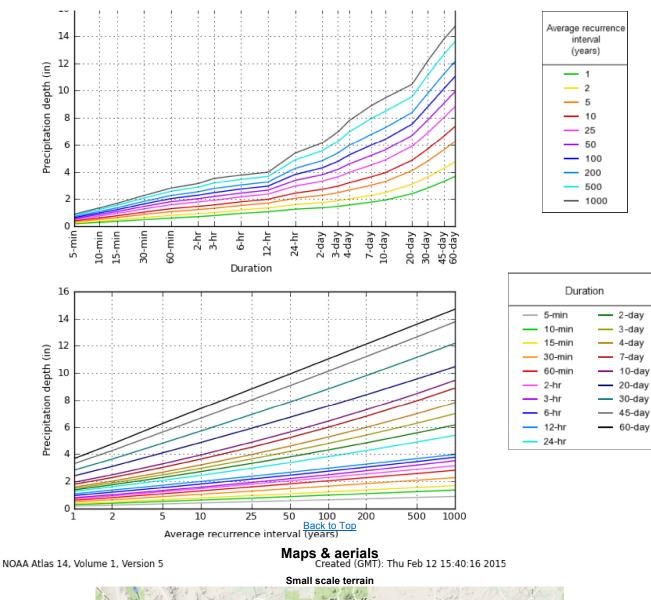
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PF graphical









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US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service Office of Hydrologic Development 1325 East West Highway Silver Spring, MD 20910

http://hdsc.nws.noaa.gov/hdsc/pfds_printpage.html?lat=33.6488&lon=-111.9060&dat... 2/12/2015

WOOD/PATEL

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Retention Volume Required

Description: Location: Reference:	Calculation of Required Retention Volume Using the Rational Method Fairmont Scottsdale Hotel Expansion City of Scottsdale, Arizona City of Scottsdale Design Standards and Policies Manual, 2010
Known Values:	Design storm: 100-yr, 2-hr Rainfall, D: 2.29
Calc. Values:	$Vrequired = C \times \frac{D}{12} \times A$
	Where: V = Retention Volume Required C = Runoff Coefficient D = Depth of Rainfall A = Area of Watershed Contributing

Retention Basin	Contributing Sub-Basins	Area (ac)	Land Use	100-Year Runoff Coefficient	100-Year, 2- Hour Volume (cu.ft)	100-Year, 2- Hour Volume (ac.ft)
Pre-Condition						
E2	E2	0.44	100% Desert	0.45	1,646	0.04
Post-Condition						
E2	E2	0.40	100% Desert	0.45	1,496	0.03

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Retention Volume Provided

Description: Location: Reference: Calculation of Proposed Retention Volume Provided Fairmont Scottsdale Hotel Expansion City of Scottsdale Design Standards and Policies Manual, 2010

Basin ID	Bottom Contour	Top Contour	Bottom	Top	Volume	Total Volume
	Area	Area	Elevation	Elevation	Provided	Provided
	(sq.ft.)	(sq.ft.)	(ft)	(ft)	(cu. ft.)	(acre-ft.)
E2	2,320	5,240	1556.0	1557.0	3,780	0.09

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Rational Method Summary

Description: Rational Method Inputs and Results **Location:** Fairmont Scottsdale Hotel Expansion

City of Scottsdale, Arizona

Existing Runoff

Exioting Ital	-																
Drainage ID	Longest Watercourse (ft)	Longest Watercourse "L" (mi)	Drainage Area (s.f.)	Drainage Area "A" (acres)	Watershed Resistance Coefficient "K _b "	Top Elev. (ft)	Bottom Elev. (ft)	Basin Slope "S" (ft/mi)	Land Use (1)	Post Q100 "Tc" (min)	100 YR Intensity "i" (in/hr)	100YR Runoff Coefficient "C"	Post Q100 (cfs)	Post Q10 "Tc" (min)	10 YR Intensity "i" (in/hr)	10YR Runoff Coefficient "C"	Post Q10 (cfs)
B1	544.0	0.103	138121	3.17	0.0731	60.0	54.0	58	100% Desert Landscaping	7.7	6.80	0.45	9.7	9.5	3.91	0.37	4.6
B2	209.0	0.040	38479	0.88	0.0403	59.5	56.3	81	100% Paved	2.9	8.60	0.95	7.2	3.5	5.34	0.90	4.2
B3	209.0	0.040	40583	0.93	0.0402	60.0	57.2	71	100% Paved	3.0	8.56	0.95	7.6	3.6	5.31	0.90	4.4
B4	218.0	0.041	41039	0.94	0.0402	58.0	54.7	80	100% Paved	3.0	8.56	0.95	7.6	3.5	5.34	0.90	4.5
B5	204.0	0.039	39679	0.91	0.0403	59.0	55.5	91	100% Paved	2.8	8.63	0.95	7.5	3.3	5.38	0.90	4.4
B6	397.0	0.075	86513	1.99	0.0381	57.4	53.0	59	71% Paved 29% Desert	4.4	8.03	0.80	12.8	5.3	4.91	0.74	7.2
B7	227.0	0.043	34517	0.79	0.0406	59.0	54.9	95	100% Commercial	2.9	8.60	0.86	5.8	3.5	5.34	0.80	3.4
B8	777.0	0.147	111872	2.57	0.0374	58.5	54.9	24	75% Paved 16% Grass 9% Desert	8.8	6.39	0.80	13.1	10.9	3.58	0.74	6.8
B9	120.0	0.023	9010	0.21	0.0442	75.5	75.2	13	100% Roof	4.2	8.11	0.95	1.6	5.0	4.98	0.90	0.9
B10	213.0	0.040	25528	0.59	0.0414	75.5	74.9	15	100% Roof	5.2	7.74	0.95	4.3	6.3	4.67	0.90	2.5
B11	207.0	0.039	15190	0.35	0.0428	75.5	74.9	15	100% Roof	5.2	7.74	0.95	2.6	6.3	4.67	0.90	1.5
B12	110.0	0.021	11907	0.27	0.0436	75.5	75.2	14	100% Roof	3.9	8.22	0.95	2.1	4.6	5.08	0.90	1.2
E3	186.4	0.035	6320	0.15	0.0913	60.2	53.5	190	100% Desert Landscaping	3.2	8.48	0.45	0.6	3.9	5.24	0.37	0.3
E4	572.6	0.108	115992	2.66	0.0373	53.0	47.1	54	59% Paved 41% Desert	5.5	7.62	0.74	15.0	6.6	4.60	0.68	8.3
E5	815.3	0.154	110212	2.53	0.0375	52.3	44.6	50	57% Paved 24% Desert 19% Grass	6.9	7.10	0.71	12.8	8.4	4.17	0.64	6.8
E6	881.3	0.167	114537	2.63	0.0374	51.9	41.0	65	100% Commercial	6.6	7.21	0.86	16.3	8.0	4.27	0.80	9.0
M1	533.3	0.101	133357	3.06	0.0370	43.1	41.1	20	100% Commercial	7.4	6.91	0.86	18.2	9.1	4.00	0.80	9.8
M2	212.3	0.040	17531	0.40	0.0855	34.4	31.2	80	100% Desert Landscaping	4.4	8.03	0.45	1.4	5.4	4.88	0.37	0.7

C_{Pre} (Overall) = 0.77

C_{Pre} (Fairmont) = 0.59

Notes:

1. The "B" areas represent the areas contributing to the storm drain line in Cottage Terrace from the Ballroom Addition project.

2. The "E" areas represent the areas contributing to the storm drain line in Cottage Terrace from the Maravilla East Property Line Road project.

3. The "M" areas represent the areas contributing to the storm drain line in Cottage Terrace from the Maravilla Senior Living project.

Active Scenario: 10 yr - Existing Catch Basin FlexTable: CatchBasin Table

Label	Elevation (Rim) (ft)	Elevation (Invert) (ft)	Flow (Additional) (cfs)	Flow (Total Out) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Velocity (Out) (ft/s)	Headloss (ft)
EX CB-B1	56.00	44.73	4.60	4.60	50.03	50.00	1.46	0.03
EX CB-B2	56.30	52.80	4.20	4.20	53.90	53.59	4.48	0.31
EX CB-B3	57.20	53.70	4.40	4.40	54.83	54.50	4.56	0.32
EX CB-B4	54.70	51.20	4.50	4.50	52.70	52.59	2.63	0.11
EX CB-B5	55.50	52.00	4.40	4.40	53.90	53.80	2.49	0.10
EX CB-B6A	53.00	49.50	4.32	4.32	51.47	51.38	2.44	0.09
EX CB-B6B	60.95	51.00	1.44	1.44	55.04	54.78	4.13	0.26
EX CB-B6C	60.84	50.70	1.44	2.88	54.38	53.32	8.25	1.06
EX CB-B7	54.90	51.40	3.40	3.40	53.42	53.42	1.92	0.00
EX CB-B8	54.90	51.40	6.80	6.80	53.51	53.43	2.16	0.07
EX CB-B9	57.00	51.38	0.90	0.90	53.15	52.82	4.58	0.33
EX CB-B10	59.70	52.30	2.50	2.50	62.22	59.70	12.73	2.52
EX CB-B11	60.20	51.68	1.50	1.50	59.47	58.56	7.64	0.91
EX CB-B12A	60.80	51.00	0.60	0.60	51.51	51.36	3.07	0.15
EX CB-B12B	60.81	51.00	0.60	0.60	51.51	51.39	2.86	0.13
EX CB-E3	53.52	49.52	0.30	0.30	49.82	49.75	2.26	0.08
EX CB-E4	47.03	38.21	8.30	8.30	45.97	45.63	4.70	0.34
EX CB-E6A	42.78	37.06	0.90	0.90	41.79	41.79	0.51	0.00
EX CB-E6B	41.10	36.37	3.60	3.60	41.00	40.93	2.04	0.06
EX CB-E6C	38.00	35.95	0.90	0.90	38.00	38.00	0.51	0.00
EX CB-E6D	37.00	32.97	3.60	3.60	35.37	35.36	0.60	0.01
EX CB-M1	39.20	35.83	9.80	9.80	39.68	39.20	5.55	0.48
EX CB-M2	39.70	32.70	0.70	80.50	35.36	34.48	7.56	0.89

Active Scenario: 10 yr - Existing

FlexTable: Conduit Table

Label	Start Node	Stop Node	Manning's n	Diameter (in)	Flow (cfs)	Length (Unified) (ft)	Slope (Calculated) (ft/ft)	Capacity (Full Flow) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Velocity (ft/s)
EX CO-15	EX MH-1	EX MH-6	0.012	18.0	8.70	77.7	0.021	16.32	52.28	51.82	4.92
EX CO-25	EX MH-2	EX MH-4	0.012	18.0	8.80	76.6	0.016	14.42	53.49	53.03	4.98
EX CO-35	EX MH-3	EX MH-4	0.012	24.0	10.20	135.2	0.007	19.77	53.26	53.03	3.25
EX CO-45	EX MH-4	EX MH-5	0.012	30.0	20.50	154.0	0.007	35.99	52.81	52.49	4.18
EX CO-55	EX MH-5	EX MH-6	0.012	30.0	23.00	158.0	0.007	36.05	52.25	51.82	4.69
EX CO-65	EX MH-6	EX MH-7	0.012	36.0	31.70	125.3	0.007	58.46	51.57	51.33	4.48
EX CO-75	EX MH-7	EX MH-8	0.012	36.0	32.60	67.0	0.007	58.56	51.33	51.20	4.61
EX CO-85	EX MH-8	EX MH-9	0.012	36.0	36.92	97.3	0.007	58.61	50.90	50.65	5.22
EX CO-95	EX MH-9	EX MH-10	0.012	36.0	41.00	73.7	0.014	84.14	50.23	49.99	5.80
EX CO-105	EX MH-10	EX MH-11	0.012	36.0	45.60	89.4	0.014	84.41	49.47	49.12	6.45
EX CO-115	EX MH-11	EX MH-12	0.012	36.0	45.90	101.8	0.014	85.63	48.59	48.18	6.49
EX CO-125	EX MH-12	EX MH-13	0.012	36.0	45.90	142.3	0.019	100.07	47.59	47.02	6.49
EX CO-135	EX MH-13	EX MH-14	0.012	36.0	45.90	16.4	0.002	35.67	46.82	46.75	6.49
EX CO-145	EX MH-14	EX MH-15	0.012	36.0	45.90	239.4	0.003	39.63	46.56	45.59	6.49
EX CO-155	EX MH-15	EX MH-16	0.012	36.0	54.20	208.3	0.003	39.73	44.95	43.78	7.67
EX CO-165	EX MH-16	EX MH-17	0.012	36.0	61.00	271.6	0.003	39.46	43.72	41.79	8.63
EX CO-175	EX MH-17	EX MH-18	0.012	42.0	61.90	127.7	0.003	59.45	41.34	40.92	6.43
EX CO-185	EX MH-18	EX MH-19	0.012	42.0	65.50	155.0	0.003	59.37	40.20	39.64	6.81
EX CO-195	EX MH-19	EX CB-M2	0.012	42.0	76.20	97.9	0.032	193.90	38.53	34.42	18.94
EX CO-205	EX CB-M2	0-1	0.013		80.50	162.0	0.009	195.32	34.48	32.56	10.22
EX CO-215	EX MH-20	EX MH-21	0.012	8.0	1.20	38.8	0.035	2.45	51.28	50.95	3.44
EX CO-225	EX MH-21	EX MH-9	0.012	18.0	4.08	187.4	0.016	14.28	50.89	50.65	2.31
EX CO-235	EX CB-E3	EX MH-11	0.013	12.0	0.30	219.9	0.028	5.99	49.75	49.12	3.97
EX CO-B1	EX CB-B1	EX MH-10	0.012	24.0	4.60	21.7	0.005	16.62	50.00	49.99	1.46
EX CO-B2	EX CB-B2	EX MH-1	0.012	18.0	4.20	176.9	0.020	15.94	53.59	52.54	7.61
EX CO-B3	EX CB-B3	EX MH-2	0.012	18.0	4.40	164.0	0.016	14.30	54.50	53.76	7.12
EX CO-B4	EX CB-B4	EX MH-1	0.012	18.0	4.50	37.1	0.050	25.54	52.59	52.54	10.89
EX CO-B5	EX CB-B5	EX MH-2	0.012	18.0	4.40	29.0	0.031	19.92	53.80	53.76	2.49
EX CO-B6A	EX CB-B6A	EX MH-8	0.012	18.0	4.32	124.1	0.019	15.76	51.38	51.20	2.44

4302 StormCAD - Existing.stsw 3/3/2015

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Bentley StormCAD V8i (SELECTseries 3) [08.11.03.84] Page 1 of 2

Active Scenario: 10 yr - Existing

FlexTable: Conduit Table

Label	Start Node	Stop Node	Manning's n	Diameter (in)	Flow (cfs)	Length (Unified) (ft)	Slope (Calculated) (ft/ft)	Capacity (Full Flow) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Velocity (ft/s)
EX CO-B6B	EX CB-B6B	EX CB-B6C	0.012	8.0	1.44	31.3	0.010	1.25	54.78	54.38	4.13
EX CO-B6C	EX CB-B6C	EX MH-21	0.012	8.0	2.88	49.0	0.041	2.66	53.32	50.95	8.25
EX CO-B7	EX CB-B7	EX MH-3	0.012	18.0	3.40	22.4	0.020	15.94	53.42	53.40	1.92
EX CO-B8	EX CB-B8	EX MH-3	0.012	24.0	6.80	48.6	0.009	23.32	53.43	53.40	2.16
EX CO-B9	EX CB-B9	EX MH-7	0.012	6.0	0.90	68.0	0.047	1.32	52.82	51.33	4.58
EX CO-B10	EX CB-B10	EX MH-5	0.012	6.0	2.50	101.4	0.024	0.94	69.64	52.49	12.73
EX CO-B11	EX CB-B11	EX MH-4	0.012	6.0	1.50	90.8	0.019	0.83	58.56	53.03	7.64
EX CO-B12A	EX CB-B12A	EX MH-20	0.012	8.0	0.60	5.5	0.175	5.48	51.36	51.41	10.31
EX CO-B12B	EX CB-B12B	EX MH-20	0.012	8.0	0.60	27.7	0.035	2.44	51.39	51.41	5.79
EX CO-E4	EX CB-E4	EX MH-15	0.012	18.0	8.30	6.3	0.021	16.36	45.63	45.59	4.70
EX CO-E6A	EX CB-E6A	EX MH-17	0.012	18.0	0.90	22.9	0.018	15.40	41.79	41.79	0.51
EX CO-E6B	EX CB-E6B	EX MH-18	0.012	18.0	3.60	10.6	0.010	11.57	40.93	40.92	2.04
EX CO-E6C	EX CB-E6C	EX MH-19	0.012	18.0	0.90	16.3	0.009	10.90	39.64	39.64	0.51
EX CO-E6D	EX CB-E6D	EX CB-M2	0.012	36.0	3.60	32.3	0.008	66.08	35.36	35.36	5.00
EX CO-M1	EX CB-M1	EX MH-19	0.012	18.0	9.80	15.4	0.002	5.01	39.76	39.64	5.55

4302 StormCAD - Existing.stsw 3/3/2015

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Active Scenario: 10 yr - Existing

Label	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Elevation (Invert) (ft)	Diameter (in)	Flow (Total Out) (cfs)	Headloss Coefficient (Standard)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Local Fixed Flow (cfs)
EX MH-1	55.40	55.40	49.33	48.0	8.70	0.700	52.54	52.28	0.00
EX MH-2	55.70	55.70	51.11	48.0	8.80	0.700	53.76	53.49	0.00
EX MH-3	56.00	56.00	50.86	48.0	10.20	0.800	53.40	53.26	0.00
EX MH-4	56.15	56.15	49.88	48.0	20.50	0.800	53.03	52.81	0.00
EX MH-5	56.00	56.00	48.87	48.0	23.00	0.700	52.49	52.25	0.00
EX MH-6	54.82	54.82	47.73	48.0	31.70	0.800	51.82	51.57	0.00
EX MH-7	54.78	54.78	46.81	48.0	32.60		51.33	51.33	0.00
EX MH-8	54.72	54.72	46.37	48.0	36.92	0.700	51.20	50.90	0.00
EX MH-9	52.60	52.60	45.63	48.0	41.00	0.800	50.65	50.23	0.00
EX MH-10	53.30	53.30	44.53	48.0	45.60	0.800	49.99	49.47	0.00
EX MH-11	52.60	52.60	43.21	48.0	45.90	0.800	49.12	48.59	0.00
EX MH-12	53.00	53.00	41.68	48.0	45.90	0.900	48.18	47.59	0.00
EX MH-13	50.50	50.50	38.84	48.0	45.90	0.300	47.02	46.82	0.00
EX MH-14	50.00	50.00	38.80	48.0	45.90	0.300	46.75	46.56	0.00
EX MH-15	47.70	47.70	38.08	48.0	54.20	0.700	45.59	44.95	0.00
EX MH-16	45.20	45.20	37.45	48.0	61.00	0.050	43.78	43.72	6.80
EX MH-17	42.80	42.80	36.64	48.0	61.90	0.700	41.79	41.34	0.00
EX MH-18	42.20	42.20	36.26	48.0	65.50	1.000	40.92	40.20	0.00
EX MH-19	41.50	41.50	35.80	48.0	76.20	0.800	39.64	38.53	0.00
EX MH-20	60.00	60.00	50.04	48.0	1.20	0.700	51.41	51.28	0.00
EX MH-21	52.30	52.30	48.68	48.0	4.08	0.800	50.95	50.89	0.00

FlexTable: Manhole Table

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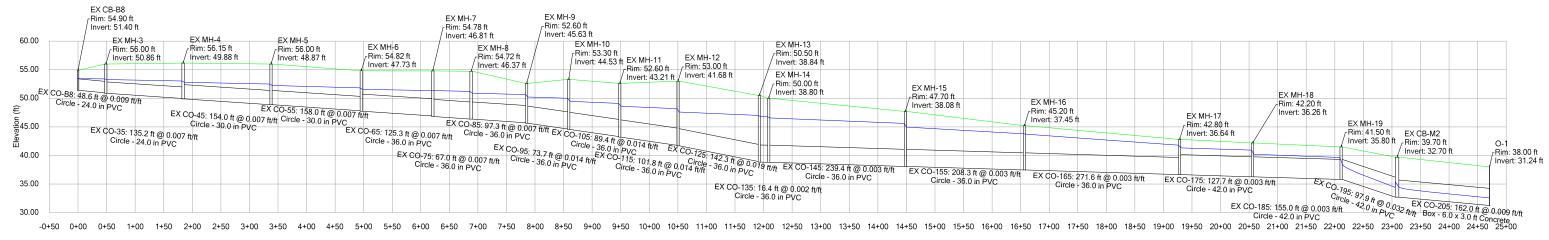
Active Scenario: 10 yr - Existing FlexTable: Outfall Table

Label	Elevation (Ground) (ft)	Set Rim to Ground Elevation	Elevation (Invert) (ft)	Flow (Total Out) (cfs)	Boundary Condition Type
0-1	38.00	True	31.24	80.50	Free Outfall

4302 StormCAD - Existing.stsw 3/3/2015

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Active Scenario: 10 yr - Existing Profile Report Engineering Profile - EX CB-B8 TO O-1 (4302 StormCAD - Existing.stsw)



Station (ft)

Bentley StormCAD V8i (SELECTseries 3) [08.11.03.84] Page 1 of 1

WOOD/PATEL

Rational Method Summary

Description:Rational Method Inputs and ResultsLocation:Fairmont Scottsdale Hotel ExpansionCity of Scottsdale, Arizona

Proposed Runoff

Drainage ID	Longest Watercours e (ft)	Longest Watercourse "L" (mi)	Drainage Area (s.f.)	Drainage Area "A" (acres)	Watershed Resistance Coefficient "K _b "	Top Elev. (ft)	Bottom Elev. (ft)	Basin Slope "S" (ft/mi)	Land Use (1)	Post Q100 "Tc" (min)	100 YR Intensity "i" (in/hr)	100YR Runoff Coefficient "C"	Post Q100 (cfs)	Post Q10 "Tc" (min)	10 YR Intensity "i" (in/hr)	10YR Runoff Coefficien t	Post Q10 (cfs)
Existing																	
B1	570.8	0.108	98439	2.26	0.0378	58.5	54.0	42	93% Desert 7% Paved	6.0	7.44	0.49	8.2	7.3	4.43	0.41	4.1
B2	209.0	0.040	38479	0.88	0.0403	59.5	56.3	81	100% Paved	2.9	8.60	0.95	7.2	3.5	5.34	0.90	4.2
B3	209.0	0.040	40583	0.93	0.0402	60.0	57.2	71	100% Paved	3.0	8.56	0.95	7.6	3.6	5.31	0.90	4.4
B4	218.0	0.041	41039	0.94	0.0402	58.0	54.7	80	100% Paved	3.0	8.56	0.95	7.6	3.5	5.34	0.90	4.5
B5	204.0	0.039	39679	0.91	0.0403	59.0	55.5	91	100% Paved	2.8	8.63	0.95	7.5	3.3	5.38	0.90	4.4
B6	397.0	0.075	86513	1.99	0.0381	57.4	53.0	59	71% Paved 29% Desert	4.4	8.03	0.80	12.8	5.3	4.91	0.74	7.2
B7	227.0	0.043	34517	0.79	0.0406	59.0	54.9	95	100% Commercial	2.9	8.60	0.86	5.8	3.5	5.34	0.80	3.4
B8	777.0	0.147	111872	2.57	0.0374	58.5	54.9	24	75% Paved 16% Grass 9% Desert	8.8	6.39	0.80	13.1	10.9	3.58	0.74	6.8
B9	120.0	0.023	9010	0.21	0.0442	75.5	75.2	13	100% Roof	4.2	8.11	0.95	1.6	5.0	4.98	0.90	0.9
B10	213.0	0.040	25528	0.59	0.0414	75.5	74.9	15	100% Roof	5.2	7.74	0.95	4.3	6.3	4.67	0.90	2.5
B11	207.0	0.039	15190	0.35	0.0428	75.5	74.9	15	100% Roof	5.2	7.74	0.95	2.6	6.3	4.67	0.90	1.5
B12	110.0	0.021	11907	0.27	0.0436	75.5	75.2	14	100% Roof	3.9	8.22	0.95	2.1	4.6	5.08	0.90	1.2
E3	186.4	0.035	6320	0.15	0.0913	60.2	53.5	190	100% Desert	3.2	8.48	0.45	0.6	3.9	5.24	0.37	0.3
E4	572.6	0.108	115992	2.66	0.0373	53.0	47.1	54	59% Paved 41% Desert	5.5	7.62	0.74	15.0	6.6	4.60	0.68	8.3
E5A	362.1	0.069	23934	0.55	0.0416	52.3	51.5	12	25% Grass 37% Desert 38% Paved	7.7	6.80	0.60	2.2	9.5	3.91	0.53	1.1
E5B	461.3	0.087	18390	0.42	0.0424	47.8	43.6	48	63% Paved 37% Desert	5.4	7.62	0.76	2.4	6.6	4.60	0.70	1.4
E6	362.1	0.069	96144	2.21	0.0378	49.2	41.0	120	100% Commercial	3.3	8.45	0.86	16.1	4.0	5.22	0.80	9.2
M1	533.3	0.101	133357	3.06	0.0370	43.1	41.1	20	100% Commercial	7.4	6.91	0.86	18.2	9.1	4.00	0.80	9.8
M2	212.3	0.040	17531	0.40	0.0855	34.4	31.2	80	100% Desert	4.4	8.03	0.45	1.4	5.3	4.91	0.37	0.7

WOOD/PATEL

Rational Method Summary

Description: Rational Method Inputs and Results Location: Fairmont Scottsdale Hotel Expansion City of Scottsdale, Arizona

Proposed Runoff

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Drainage ID	Longest Watercours e (ft)	Longest Watercourse "L" <u>(mi)</u>	Drainage Area (s.f.)	Drainage Area "A" (acres)	Watershed Resistance Coefficient "K _b "	Top Elev. (ft)	Bottom Elev. (ft)	Basin Slope "S" (ft/mi)	Land Use (1)	Post Q100 "Tc" (min)	100 YR Intensity "i" (in/hr)	100YR Runoff Coefficient "C"	Post Q100 (cfs)	Post Q10 "Tc" (min)	10 YR Intensity "i" (in/hr)	10YR Runoff Coefficien t	Post Q10 (cfs)
Proposed																	
N1	214.3	0.041	23664	0.54	0.0417	59.0	55.0	99	100% Paved	2.8	8.63	0.95	4.4	3.4	5.36	0.90	2.6
N2	177.1	0.034	17431	0.40	0.0425	56.9	54.0	86	100% Paved	2.7	8.67	0.95	3.3	3.2	5.41	0.90	1.9
S1	213.4	0.040	10161	0.23	0.0440	51.3	47.1	104	100% Desert	2.8	8.63	0.45	0.9	3.4	5.36	0.37	0.5
S2	142.7	0.027	14219	0.33	0.0430	50.3	47.5	104	90% Grass 6% Desert 4% Paved	2.3	8.82	0.34	1.0	2.7	5.53	0.24	0.4
S3	223.3	0.042	9702	0.22	0.0441	61.3	49.0	291	53% Paved 47% Grass	2.1	8.89	0.64	1.3	2.5	5.58	0.57	0.7
S4	95.2	0.018	4391	0.10	0.0463	75.0	74.1	50	100% Roof	2.4	8.78	0.95	0.8	2.9	5.48	0.90	0.5
S5	94.2	0.018	4494	0.10	0.0463	75.0	74.1	50	100% Roof	2.4	8.78	0.95	0.8	2.9	5.48	0.90	0.5
S6	98.0	0.019	4535	0.10	0.0463	75.0	74.0	54	100% Roof	2.4	8.78	0.95	0.8	2.9	5.48	0.90	0.5
S7	98.1	0.019	4481	0.10	0.0463	75.0	74.0	54	100% Roof	2.4	8.78	0.95	0.8	2.9	5.48	0.90	0.5
S8	95.4	0.018	4336	0.10	0.0463	75.0	74.1	50	100% Roof	2.4	8.78	0.95	0.8	2.9	5.48	0.90	0.5
S9	70.1	0.013	4212	0.10	0.0463	49.9	44.0	444	90% Desert 10% Paved	1.0	9.31	0.50	0.5	1.2	5.88	0.42	0.2
S10	63.7	0.012	2915	0.07	0.0472	75.0	74.4	50	100% Roof	2.0	8.93	0.95	0.6	2.4	5.60	0.90	0.4
S11	73.0	0.014	3903	0.09	0.0465	75.0	74.3	51	100% Roof	2.1	8.89	0.95	0.8	2.5	5.58	0.90	0.5
S12	151.2	0.029	19168	0.44	0.0422	51.6	47.5	143	49% Grass 35% Desert 16% Paved	2.1	8.89	0.45	1.8	2.5	5.58	0.37	0.9

 C_{post} (Overall) = 0.78 C_{post} (Fairmont) = 0.61

Notes:

The "B" areas represent the areas contributing to the storm drain line in Cottage Terrace from the Ballroom Addition project.
 The "E" areas represent the areas contributing to the storm drain line in Cottage Terrace from the Maravilla East Property Line Road project.

3. The "M" areas represent the areas contributing to the storm drain line in Cottage Terrace from the Maravilla Senior Living project.

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Curb Opening Summary

Description: Design Calculations for Curb Openings

Reference: FCDMC Drainage Design Manual for Maricopa County, Arizona, Hydraulics (2013)

Location: Fairmont Scottsdale Hotel Expansion

Known Values and Equations:

$$Q^{(1)} = CLH^{3/2}$$

Where:

Q = Capacity (cfs)

C = Coefficient = 3.0 for Curb Opening, Use: 3.0

L = Length of Opening (ft)

H = Max Headwater Depth of Flow (ft)

Curb Opening Summary

Drainage Area	Opening Type	Length, L (ft)	Coefficient	Depth, H (ft)	Capacity Q (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)
N1	Curb Opening	4	3.0	0.5	4.2	2.6	4.4
N2	Curb Opening	4	3.0	0.5	4.2	1.9	3.3

Notes

1) Per FCDMC Drainage Design Manual, Volume II, Hydraulics, (2013) equation 8.19

2) When the curb opening for drainage area N1 is exceeded, the excess will flow to the curb opening in drainage area N2.

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Inlet Capacity Summary

Description:Summary of Inlet Sizing CalculationsLocation:Fairmont Scottsdale Hotel Expansion

Inlet ID	Contributing Drainage Area ID	Q10 (cfs)	Available Head (ft)	Inlet Type	Inlet Capacity (cfs)	10-Year Actual Ponding Depth (ft)
CB-S1	S1 & E5A	1.6	0.5	MAG 535	9.02	0.17
CB-S2	S2	0.4	0.5	MAG 535	9.02	0.07
CB-S3	S3	0.7	0.5	MAG 535	9.02	0.06
CB-S9	S9	0.2	0.5	MAG 535	9.02	0.05
CB-S12	S12	0.9	0.5	MAG 535	9.02	0.09
CB-N12	N1 & N2	4.5	1.0	MAG 535	25.50	0.31
CB-B1	1/3 B1	1.4	1.5	MAG 535	35.69	0.15

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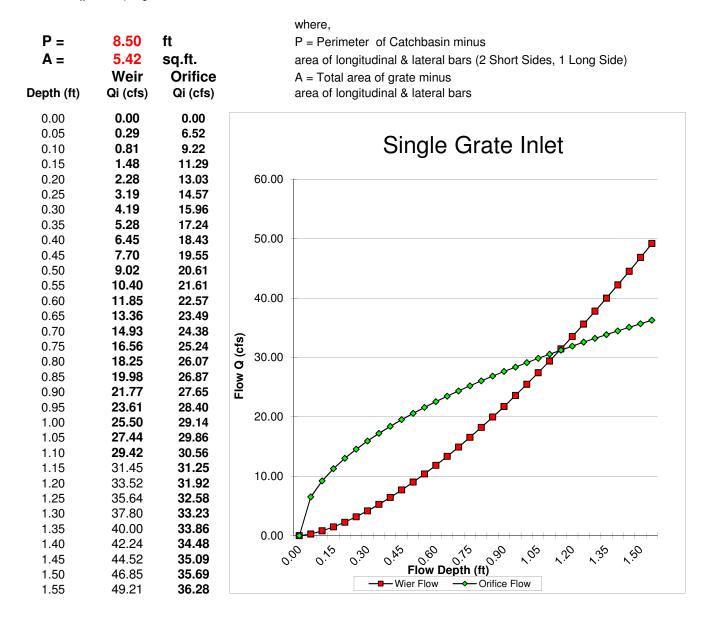
Inlet Capacity - Sump Locations

Description:	Calculation of Inlet Capacity for Single MAG 535 Catch Basin (w/o Curb)
Location:	Fairmont Scottsdale Hotel Expansion
Reference:	Drainage Design Manual for Maricopa County, Vol. II, Hydraulics, pg. 3-27

Weir EQ. $Q_i = C_w Pd^{1.5}$

Orifice EQ. $Q_i = C_o A(2gd)^{0.5}$

Where: $C_w = 3.0$, $C_o = 0.67$



Active Scenario: 10 yr - Proposed Catch Basin FlexTable: CatchBasin Table

Label	Elevation (Rim) (ft)	Elevation (Invert) (ft)	Flow (Additional) (cfs)	Flow (Total Out) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Velocity (Out) (ft/s)	Headloss (ft)
CB-B1	51.00	44.93	1.36	8.60	51.12	51.00	2.74	0.12
CB-N12	52.50	48.04	4.50	4.50	51.32	51.29	1.43	0.03
CB-S1	46.00	38.60	1.60	6.10	46.06	46.00	1.94	0.06
CB-S2	47.50	42.66	0.40	4.00	46.13	46.11	1.27	0.03
CB-S3	49.00	44.00	0.70	0.70	46.14	46.13	0.40	0.00
CB-S4	50.41	46.40	0.50	0.50	46.93	46.76	3.30	0.17
CB-S5	50.41	46.40	0.50	0.50	48.48	47.97	5.73	0.51
CB-S6	50.41	46.40	0.50	0.50	46.93	46.76	3.30	0.17
CB-S7	50.41	44.56	0.50	0.50	46.71	46.61	2.55	0.10
CB-S8	50.41	46.40	0.50	0.50	49.68	49.17	5.73	0.51
CB-S9	44.00	39.00	0.20	1.10	44.01	44.00	0.62	0.01
CB-S10A	48.90	43.64	0.20	0.20	44.17	44.09	2.29	0.08
CB-S10B	50.41	43.60	0.20	0.20	44.31	44.23	2.29	0.08
CB-S11A	50.41	45.00	0.25	0.25	45.44	45.28	3.20	0.16
CB-S11B	50.41	44.54	0.25	0.25	44.98	44.82	3.20	0.16
CB-S12	47.50	44.00	0.90	1.90	46.24	46.22	1.08	0.02
EX CB-B1	56.00	44.73	0.00	8.60	53.44	53.32	2.74	0.12
EX CB-E4	47.30	38.21	8.30	8.30	47.64	47.30	4.70	0.34
EX CB-E6A	42.78	37.06	0.92	0.92	42.14	42.13	0.52	0.00
EX CB-E6B	41.10	36.37	3.68	3.68	41.17	41.10	2.08	0.07
EX CB-E6C	38.00	35.95	0.92	0.92	38.00	38.00	0.52	0.00
EX CB-E6D	37.00	32.97	3.68	3.68	35.50	35.49	0.58	0.01
EX CB-M1	39.20	35.83	9.80	9.80	39.68	39.20	5.55	0.48
EX CB-M2	39.70	32.70	0.70	86.50	35.49	34.56	7.74	0.93

Active Scenario: 10 yr - Proposed

FlexTable: Conduit Table

Label	Start Node	Stop Node	Manning's n	Diameter (in)	Flow (cfs)	Length (Unified) (ft)	Slope (Calculated) (ft/ft)	Capacity (Full Flow) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Velocity (ft/s)
CO-15	CB-S1	MH-1	0.012	24.0	6.10	9.4	0.011	25.26	47.55	47.54	1.94
CO-25	MH-2	CB-S1	0.012	24.0	4.50	223.4	0.016	30.81	46.08	46.00	1.43
CO-35	CB-S4	MH-2	0.012	6.0	0.50	21.9	0.157	2.41	46.76	46.10	9.67
CO-45	CB-S2	MH-2	0.012	24.0	4.00	33.8	0.016	30.68	46.11	46.10	1.27
CO-55	CB-S5	MH-10	0.012	4.0	0.50	30.7	0.119	0.71	47.97	46.16	5.73
CO-65	CB-S3	CB-S2	0.012	18.0	0.70	75.3	0.018	15.18	46.13	46.13	0.40
CO-75	MH-10	CB-S2	0.012	18.0	2.90	5.7	0.016	14.26	46.14	46.13	1.64
CO-77	MH-3	MH-10	0.012	18.0	2.40	63.6	0.015	14.06	46.19	46.16	1.36
CO-85	CB-S6	MH-3	0.012	6.0	0.50	28.2	0.074	1.66	46.76	46.21	7.39
CO-95	CB-S12	MH-3	0.012	18.0	1.90	17.7	0.016	14.30	46.22	46.21	1.08
CO-105	MH-4	CB-S12	0.012	8.0	1.00	23.8	0.034	2.42	46.37	46.24	2.86
CO-115	CB-S7	MH-4	0.012	6.0	0.50	20.2	-0.021	0.88	46.61	46.48	2.55
CO-125	CB-S8	MH-4	0.012	4.0	0.50	45.9	0.035	0.38	49.17	46.48	5.73
CO-135	CB-S11A	MH-5	0.012	4.0	0.25	2.5	0.020	0.29	45.28	45.20	3.74
CO-145	MH-5	MH-6	0.010	12.0	0.25	16.5	0.028	7.73	44.83	44.53	4.51
CO-155	CB-S11B	MH-6	0.012	4.0	0.25	2.5	0.020	0.29	44.82	44.74	3.74
CO-165	MH-6	MH-7	0.012	12.0	0.50	32.0	0.028	6.48	44.45	44.06	4.88
CO-175	CB-S10A	MH-7	0.012	4.0	0.20	2.5	0.020	0.29	44.09	44.06	2.29
CO-185	MH-7	MH-8	0.012	12.0	0.70	44.6	0.028	6.49	44.05	44.04	5.40
CO-195	CB-S10B	MH-8	0.012	4.0	0.20	19.8	0.064	0.52	44.23	44.04	2.29
CO-205	MH-8	CB-S9	0.012	12.0	0.90	47.1	0.064	9.74	44.03	44.00	1.15
CO-215	CB-S9	MH-9	0.012	18.0	1.10	14.9	0.080	32.18	44.32	44.32	0.62
CO-220	MH-10	EX CB-B1	0.010	24.0	8.60	7.2	0.006	21.96	53.45	53.44	2.74
CO-225	CB-B1	MH-10	0.012	24.0	8.60	31.9	0.005	17.36	53.52	53.48	2.74
CO-230	MH-11	CB-B1	0.012	24.0	7.24	163.4	0.007	20.47	51.14	51.00	2.30
CO-235	MH-12	MH-11	0.010	24.0	5.87	90.0	0.007	24.40	51.24	51.20	1.87
CO-240	CB-N12	MH-12	0.012	24.0	4.50	22.7	0.015	30.41	51.29	51.29	1.43
EX CO-105	EX MH-10	EX MH-11	0.012	36.0	49.60	89.4	0.014	84.41	52.99	52.56	7.02
EX CO-115	EX MH-11	EX MH-12	0.012	36.0	49.90	101.8	0.014	85.63	51.95	51.46	7.06

4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

Active Scenario: 10 yr - Proposed

FlexTable: Conduit Table

Label	Start Node	Stop Node	Manning's n	Diameter (in)	Flow (cfs)	Length (Unified) (ft)	Slope (Calculated) (ft/ft)	Capacity (Full Flow) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Velocity (ft/s)
EX CO-125	EX MH-12	EX MH-13	0.012	36.0	49.90	142.3	0.019	100.08	50.76	50.08	7.06
EX CO-135	EX MH-13	EX MH-14	0.012	36.0	49.90	16.2	0.002	35.87	49.85	49.77	7.06
EX CO-145	EX MH-14	EX MH-15	0.012	36.0	49.90	239.4	0.003	39.63	49.54	48.40	7.06
EX CO-155A	EX MH-15	MH-1	0.012	36.0	58.20	25.0	0.003	40.84	47.70	47.54	8.23
EX CO-155B	MH-1	EX MH-16	0.012	36.0	64.30	183.3	0.003	39.58	46.65	45.20	9.10
EX CO-165A	EX MH-16	MH-9	0.012	36.0	65.70	129.0	0.003	39.72	45.39	44.32	9.29
EX CO-165B	MH-9	EX MH-17	0.012	36.0	66.80	142.6	0.003	39.22	43.35	42.13	9.45
EX CO-175	EX MH-17	EX MH-18	0.012	42.0	67.72	127.3	0.003	59.55	41.59	41.10	7.04
EX CO-185	EX MH-18	EX MH-19	0.012	42.0	71.40	155.2	0.003	59.33	40.50	39.84	7.42
EX CO-195	EX MH-19	EX CB-M2	0.012	42.0	82.12	96.5	0.032	195.30	38.63	34.50	19.42
EX CO-205	EX CB-M2	O-1	0.013		86.50	163.2	0.009	194.59	34.56	32.64	10.42
EX CO-B1	EX CB-B1	EX MH-10	0.012	24.0	8.60	19.6	0.005	17.49	53.32	53.30	2.74
EX CO-E4	EX CB-E4	EX MH-15	0.012	18.0	8.30	5.8	0.023	17.10	48.43	48.40	4.70
EX CO-E6A	EX CB-E6A	EX MH-17	0.012	18.0	0.92	22.6	0.019	15.52	42.13	42.13	0.52
EX CO-E6B	EX CB-E6B	EX MH-18	0.012	18.0	3.68	10.7	0.010	11.55	41.11	41.10	2.08
EX CO-E6C	EX CB-E6C	EX MH-19	0.012	18.0	0.92	16.7	0.009	10.77	39.84	39.84	0.52
EX CO-E6D	EX CB-E6D	EX CB-M2	0.012	36.0	3.68	30.1	0.009	68.42	35.49	35.49	5.15
EX CO-M1	EX CB-M1	EX MH-19	0.012	18.0	9.80	16.0	0.002	4.93	39.96	39.84	5.55

4302 StormCAD - Proposed.stsw 4/30/2015

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

Active Scenario: 10 yr - Proposed

Label Elevation Elevation Elevation Diameter Flow Headloss Hydraulic Hydraulic Local (Ground) (Rim) (in) (Total Coefficient Grade Line Grade Line Fixed (Invert) (ft) (ft) (ft) Out) (Standard) (In) (Out) Flow (cfs) (ft) (ft) (cfs) **EX MH-10** 53.30 53.30 44.53 49.60 0.800 53.60 52.99 41.00 48.0 52.60 52.60 43.21 49.90 0.800 52.56 0.30 EX MH-11 48.0 51.95 EX MH-12 53.00 53.00 41.68 49.90 0.900 51.46 50.76 0.00 48.0 50.50 50.50 0.00 **EX MH-13** 38.84 36.0 49.90 0.300 50.08 49.85 EX MH-14 50.00 50.00 38.80 48.0 49.90 0.300 49.77 49.54 0.00 **EX MH-15** 48.40 48.40 38.08 58.20 0.800 48.55 47.70 0.00 36.0 45.20 37.45 65.70 **EX MH-16** 45.20 48.0 0.050 45.27 45.20 1.40 **EX MH-17** 42.80 42.80 36.64 41.59 0.00 48.0 67.72 0.700 42.13 **EX MH-18** 42.20 42.20 36.26 48.0 71.40 0.700 41.10 40.50 0.00 EX MH-19 41.50 41.50 35.80 48.0 82.12 0.800 39.84 38.63 0.00 MH-1 47.54 47.54 38.00 48.0 64.30 0.700 47.55 46.65 0.00 MH-2 48.50 48.50 42.13 36.0 4.50 0.700 46.10 46.08 0.00 MH-3 47.75 47.75 43.72 36.0 2.40 0.700 46.21 46.19 0.00 MH-4 47.75 47.75 44.81 48.0 1.00 0.800 46.48 46.37 0.00 MH-5 0.25 44.83 0.00 49.91 49.91 44.62 36.0 0.900 44.89 MH-6 0.50 0.00 49.91 49.91 44.16 36.0 0.700 44.53 44.45 MH-7 43.26 0.70 0.00 48.38 48.38 36.0 0.700 44.06 44.05 MH-8 47.80 47.80 42.00 36.0 0.90 0.700 44.04 44.03 0.00 MH-9 45.00 45.00 37.06 48.0 66.80 0.700 44.32 43.35 0.00 MH-10 47.52 47.52 42.75 2.90 46.14 0.00 36.0 0.700 46.16 MH-10 53.65 53.65 44.77 0.00 36.0 8.60 0.300 53.48 53.45 MH-11 53.50 53.50 46.07 48.0 7.24 0.700 51.20 51.14 1.37 MH-12 55.60 55.60 46.69 48.0 5.87 0.900 51.29 51.24 1.37

FlexTable: Manhole Table

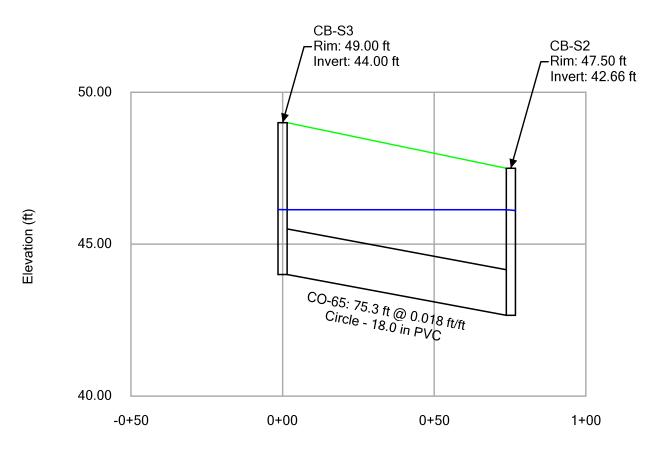
4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

Active Scenario: 10 yr - Proposed FlexTable: Outfall Table

Label	Elevation (Ground) (ft)	Set Rim to Ground Elevation	Elevation (Invert) (ft)	Flow (Total Out) (cfs)	Boundary Condition Type
0-1	38.00	True	31.24	86.50	Free Outfall

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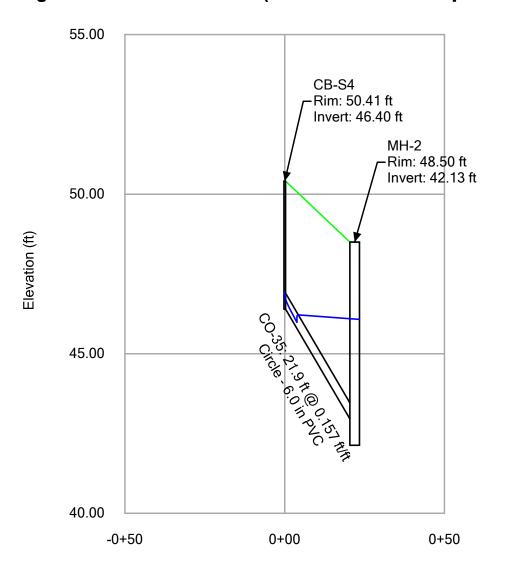
Active Scenario: 10 yr - Proposed Profile Report Engineering Profile - CB-S3 TO CB-S2 (4302 StormCAD -Proposed.stsw)



Station (ft)

4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

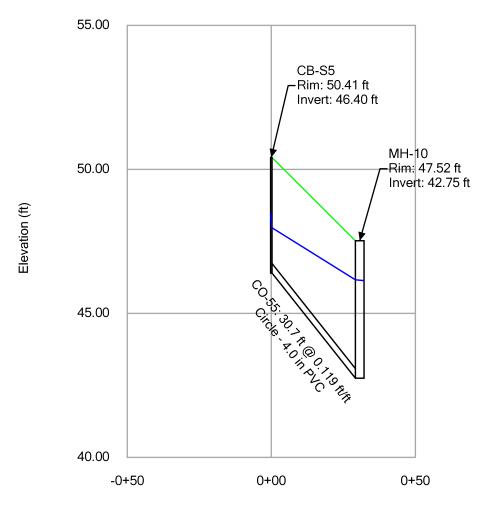
Active Scenario: 10 yr - Proposed Profile Report Engineering Profile - CB-S4 TO MH-2 (4302 StormCAD - Proposed.stsw)



Station (ft)

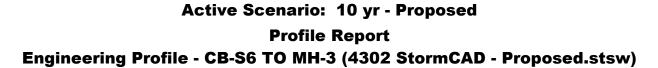
4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

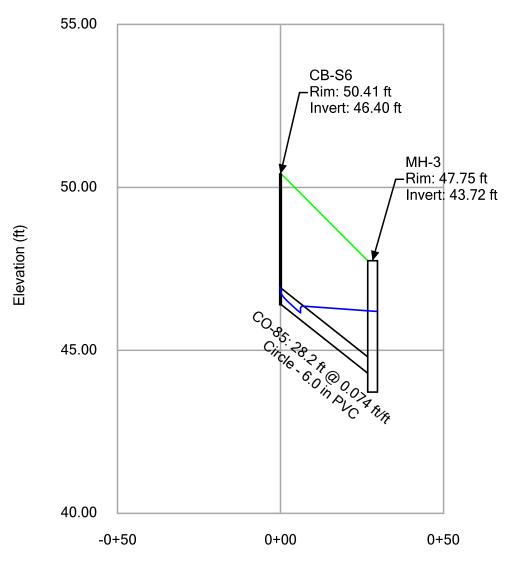




Station (ft)

4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666



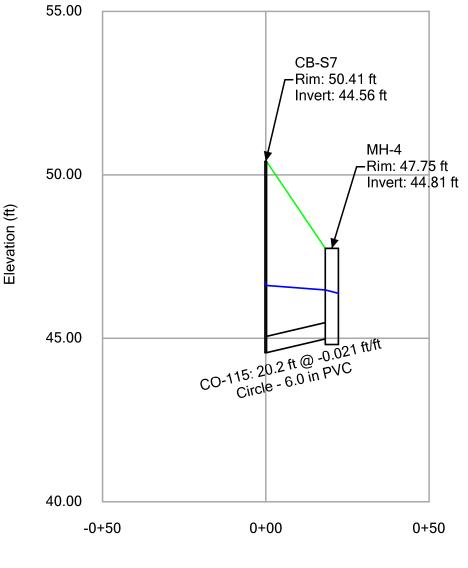


Station (ft)

4302 StormCAD - Proposed.stsw 4/30/2015

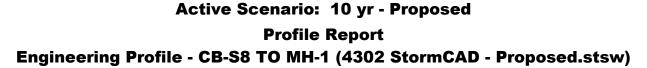
Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

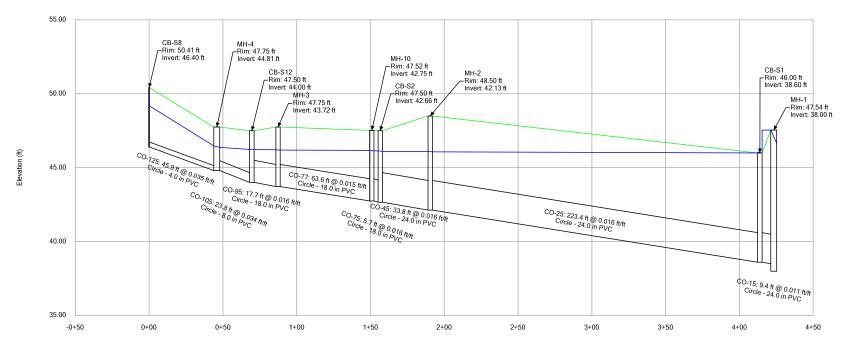
Active Scenario: 10 yr - Proposed Profile Report Engineering Profile - CB-S7 TO MH-4 (4302 StormCAD - Proposed.stsw)



Station (ft)

4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

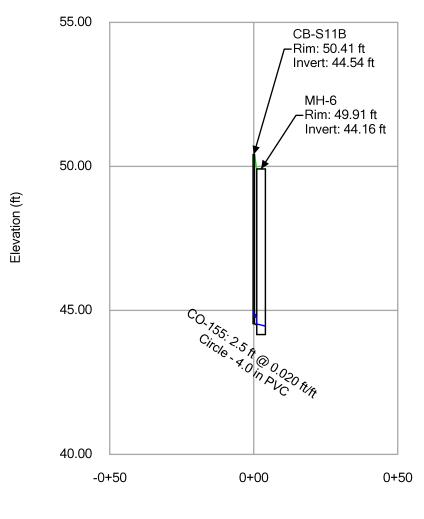




Station (ft)

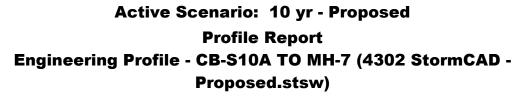
4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

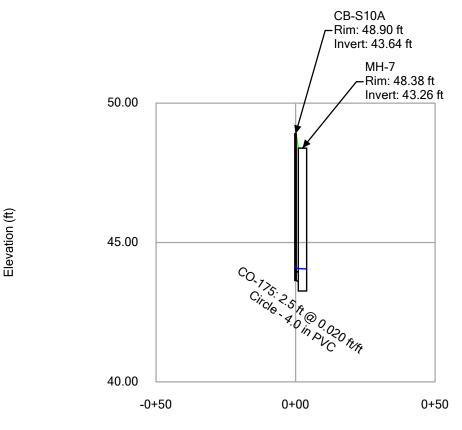




Station (ft)

4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

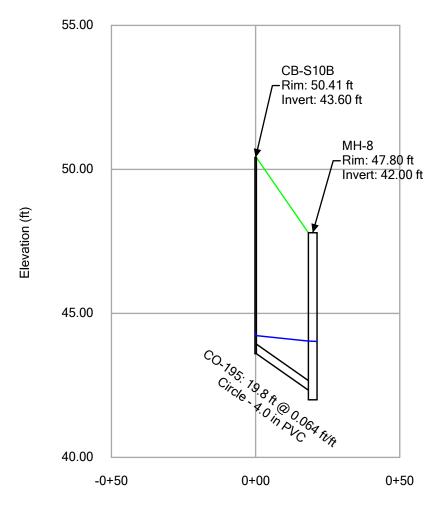




Station (ft)

4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

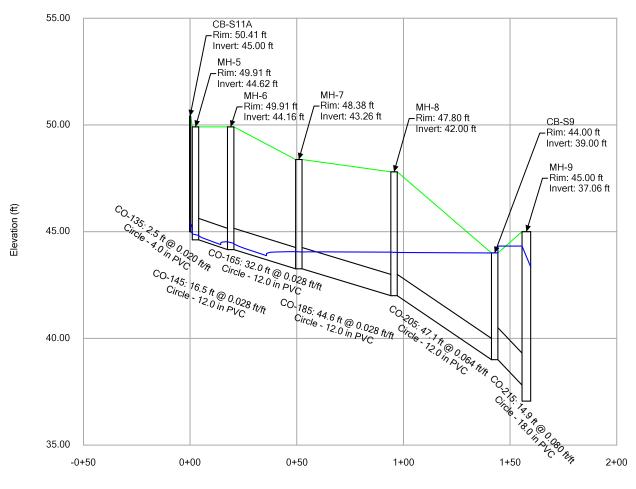




Station (ft)

4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

Active Scenario: 10 yr - Proposed Profile Report Engineering Profile - CB-S11A TO MH-9 (4302 StormCAD -Proposed.stsw)

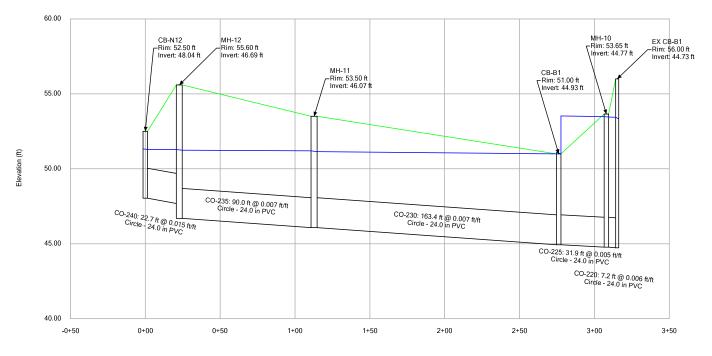


Station (ft)

4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

Active Scenario: 10 yr - Proposed

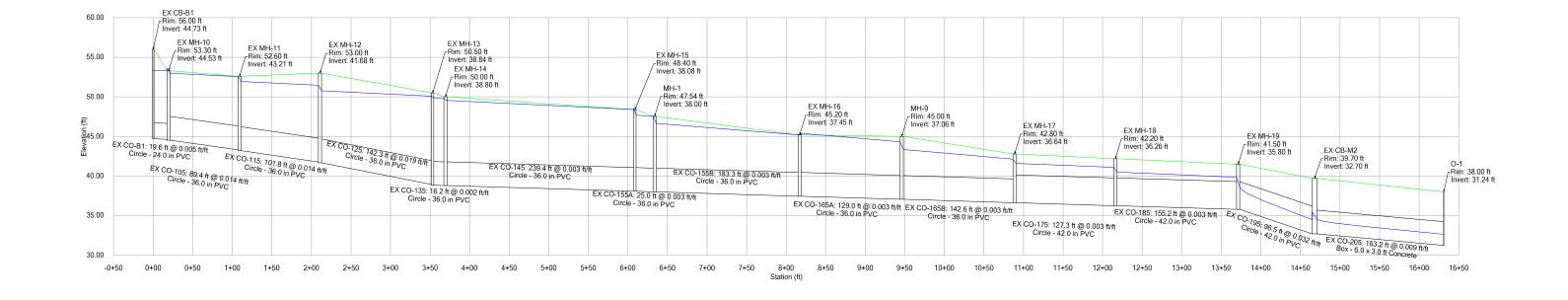
Profile Report Engineering Profile - CB-N12 TO EX CB-B1 (4302 StormCAD - Proposed.stsw)



Station (ft)

4302 StormCAD - Proposed.stsw 4/30/2015 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

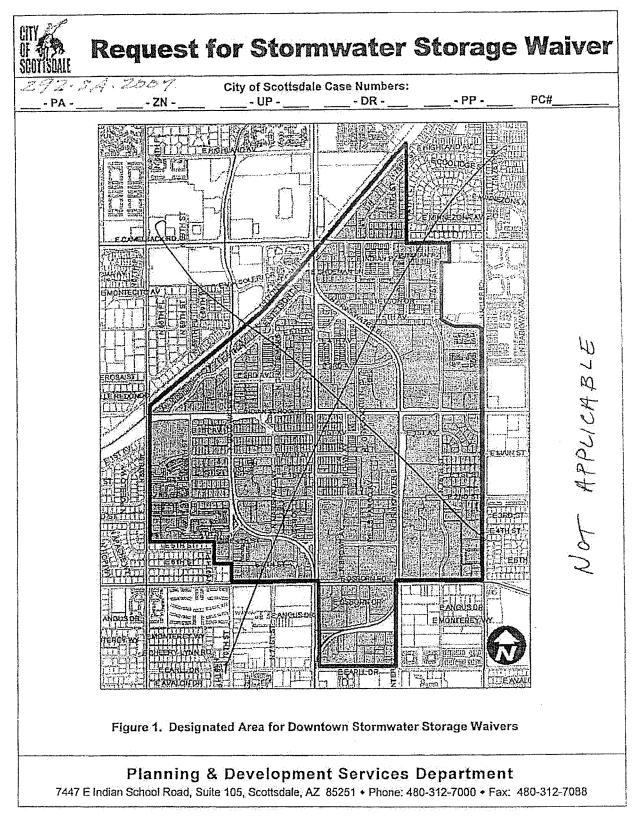
Active Scenario: 10 yr - Proposed Profile Report Engineering Profile - EX CB-B1 TO O-1 (4302 StormCAD - Proposed.stsw)

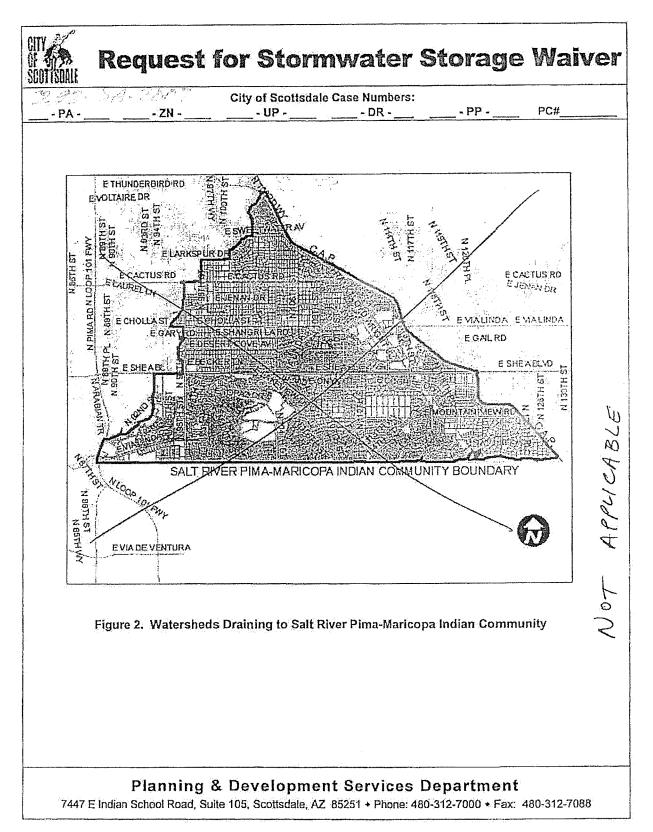


APPENDIX B

STORMWATER STORAGE WAIVER / PROPOSED DRAINAGE IMPROVEMENTS EXHIBIT

92.5A - 2007	City of Scottsdale	Case Numbers: - DR -	- PP -	PC#6332-0	2-
PA ZN he applicant/developer must complete ubmitting improvement plans. Deni- eview Board.	and submit this form to the	city for processing and of	tain approval of w	aiver request before	
ate <u>7/14/08</u> Project N	ame Fairmont Scottsdale Princ	xess Resort			
roject Location 7575 East Princess Drive	e Scottsdale, AZ 85255	Sompany Name Wood Pe	lat & Associates		
pplicant Contact John Bulka hone 480-834-3300	Fax 480-834-3320	E-mail jbulka@wood	alel.com		
ddress 1855 N. Stapley Mesa, AZ 85203	Fax to barroad	C-Inan <u>3</u>			
					-
 <u>/aiver Criteria</u> A waiver is an intentional relinquishme consider waiving some or all required supporting engineering analyses that coordinate for flooding on any property. 1. The runoff for the project must demonstrate that the from the subject property 	stormwater storage. Check lemonstrate the project mee has been included in a s e stormwater storage fa	the applicable box and p is the criteria and that th storage facility at ano cility was specifically	ovide a signed en e effect of a waive ther location. T designed to acc	n will not increase the he applicant	n der der Andrea ander and
designed conveyance fac	sility.				
2. The development is adjaced and constructed to handle subject property or to any	e the additional runoff wi / other property.	thout increasing the p	ootential for floo	d damage to the	
3. The development is on a demonstrates there is no	parcel less than one-ha significant increase in p	If acre in size in an ar otential for flood dam	ea where the e age due to its d	ngineering analysis evelopment.	
4. Stormwater storage required Ordinance (ESLO). The to the subject property or	applicant must demonst	rate there is no increa	ased potential for	or flood damage	
allowable footprint pe		sements, selbacks, a	nd NAOS preve	ent building	
	building storage basin. cllity requires wash mod	lification.			
 Instances where the 	Zoning Administrator ca	nnot allow a modifica	tion to ESL requ	uirements.	
5. The project is located will Council Resolution #623 for flood damage to any creates additional potent mitigation methods to pro-	8 (see map). The applic property. Even if the pro ial for increased flood da	ant must demonstrat ject is located in the	e there is no inc Downtown area	reased potential	
 6. The project is located with Community (SRPMIC) (see the SRPMIC, and attenue 	ee map). The project m	ust provide the pre-d	t River Pima-M evelopment pea	aricopa Indian ak discharge flow to	
By signing below, I certify that the attached documentation.	a / Dela	Λ	1 above as demo 7-16-0		and a second
Developer or Engineer circle one		Di	2660		





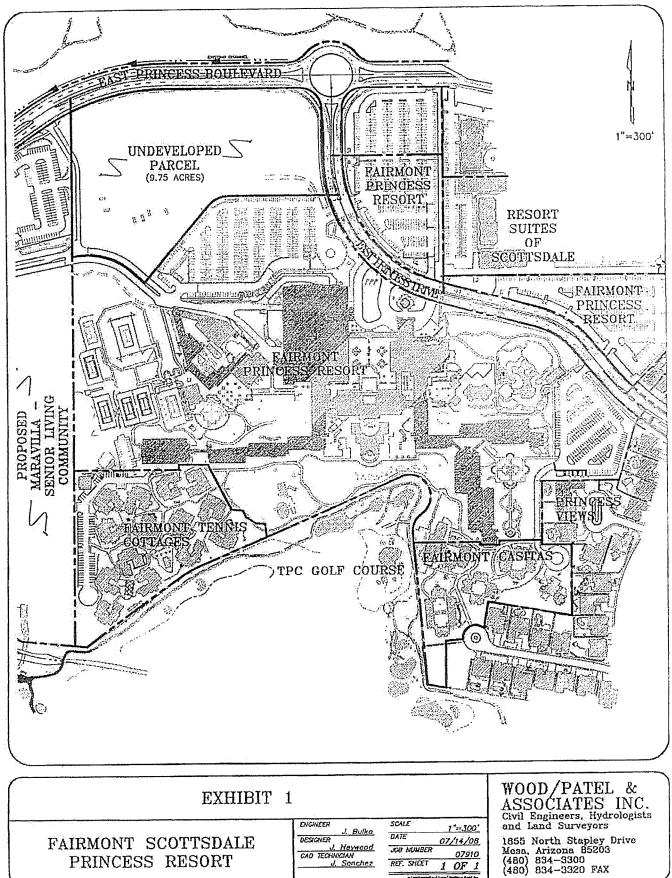
Revision Dolo: 18-Jul-07

	Request	for	Storm	nwater	Storage	Waiver
292 - P/	с. <i>S.A. 200</i> 7 гх	City o	of Scottsdale UP	Case Numbers	PP	PC#
				PLETE THIS PA		
Project	Name FAIRMONT					
Check /	Appropriate Boxes:					
	Meets waiver criteria (sp	ecify): 🗆 :	1 📮 2	3 4		
Ū	Recommend approve wa	liver.				
	Recommend <u>deny</u> waive None of waiver criteria Downstream condition Other: Explain:	i met. Is prohibit				
	Return waiver request: Insufficient data provid Other: Explain:					
	ommended Conditions of All storage requirements Pre development conditi Other: Jain: <u>In Kind Improv</u>	waived. ons must l			<u>lee .</u>	
	Waiver <u>approved</u> per a Waiver <u>denied</u> .	bove con	ditions.			
لسا	C. AShleys	(mr)			10/23/08	
	Floodplain Administrator or Des				Date	
7	Planning 447 E Indian School Road, S	& Dev Suite 105, S	elopmer Scottsdale, AZ	nt Services 85251 • Phone:	• Department 480-312-7000 • Fax:	480-312-7088

Request for Stormwater Storage Waiver

CITY Request	for Stor	mwater	Storage	Waiver
292. SA. 2007		lale Case Numbers: _ DR	- PP -	PC#
PA ZN	UP	- <u>- DR -</u>	• FF •	
	In-Lieu Fee and In	n-Kind Contribution	S	
If the city grants a waiver, the de it would cost the city to provide t construction, landscaping, desig For FY 2007/2008, this cost is \$ annually, but the city reserves th	the waived storage in, construction ma 3.22 per cubic foot	volume, including co nagement, and main of stormwater stored	sts such as land ac- tenance over a 75-y I. This unit cost will	ear design life. be updated
The Floodplain Administrator co contribution can serve as part of designee must approve In-lieu fe	f or instead of the c	alculated In-lieu fee.	by-case basis. An I The Floodplain Adi	n-kind ministrator or
Project Name Fairmont	Scottsdale	Princess Re	sort	
The waived stormwater storage	volume is calculate	ed as follows:		
V =CRA; where V =stormwater storage volume C =weighted average runoff coe R =100-year/2-hour precipitation A = area of disturbed ground, in	efficient over disturl n depth, in feet (2.8	bed area,	et, for all regions of	Scottsdale), and
Furthermore, $V_{yy} = V - V_p$; where $V_w =$ volume waived, V = volume required, and $V_p =$ volume provided	C = A = V = V _p = V _w =	0.9 424,753 89,826 0 		
□ An In-Lieu Fee will be paid, In-lieu fee (\$) = V _w (cu. ft.) x	based on the follov \$3.22 per cubic fo	ving calculations and ot = $259,290$	supporting docume	ntation:
An In-Kind Contribution will See attachment. with approved plans	Priperss Prive	s: Bridge Reconstruction	non, in geological	<u> </u>
No In-Lieu Fee is required.	Reason:		an an 11-11-11-11-11-11-11-11-11-11-11-11-11-	
Approved by: C. Jalu	iglanh		10/23/	9B
Floodplain Administrator or Designee	×.	<u>`</u>	Date	
Planning 7447 E Indian School Road, S		nent Services , AZ 85251 • Phone: 4		480-312-7088

Request for Stormwater Storage Waiver



1 OF 1

October 23, 2008 WP# 072910 Sheet 1 of 2

Attachment to Stormwater Storage Waiver Request for Fairmont Scottsdale Princess Resort & Regional Flood Control

The Fairmont Scottsdale Princess Resort (Site) is a 60 acre resort located near the southwest corner of Princess Boulevard and Princess Drive. The Site is bounded by the Princess Blvd, to the north, the Maravilla Scottsdale Senior Living Community to the east, the TPC Golf Course to the south and existing residential developments to the west (see Exhibit 1, attached). The existing Fairmont Scottsdale Princess Resort consists of multiple hotel buildings, a ballroom, spa, tennis cottages, tennis courts, and parking. A majority of the site is developed and portions are being updated and renovated. At the north end of the site there is a 9.75 acre portion of the property that has yet to be developed, and other portions are scheduled for upgrades.

It is Wood/Patel's understanding that the ownership of the Fairmont Scottsdale Princess Resort, Strategic Hotels and Resorts, has agreed to fund regional flood control improvements to the public road/channel crossing at Princess Blvd and Scottsdale Road, in return for the City approving this waiver and it being applicable to the entire site. The improvements consist of removing the existing concrete box culvert crossing and replacing it with a bridge structure. The cost of a new bridge structure is estimated at \$1,053,000.

City of Scottsdale In-Lieu Fees: V(req) Volume required = CRA = (0.90) x (0.235 feet) x (9.75 acres) = 89,826 cu-ft. C (Runoff Coefficient) = 0.90 R (100-year/2-hour precipitation depth) = 0.235 feet Site area = 9.75 acres

City of Scottsdale In-Lieu Fees = $V(req) \times 3.22 = (89,826 \text{ cu-ft}) \times 3.22 = 289,240$

Summary: Public Drainage Improvements = \$1,053,000 (*) City of Scottsdale in Lieu Fee = \$289,240

(*) See Sheet 2 of 2 Engineering Preliminary Opinion of Probable Cost

Attachment to Stormwater Storage Waiver Request

October 23, 2008 WP# 072910 Sheet 2 of 2

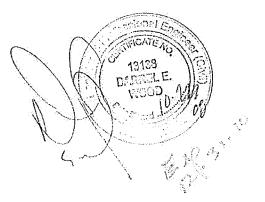
Attachment to Stormwater Storage Waiver Request for Fairmont Scottsdale Princess Resort & Regional Flood Control

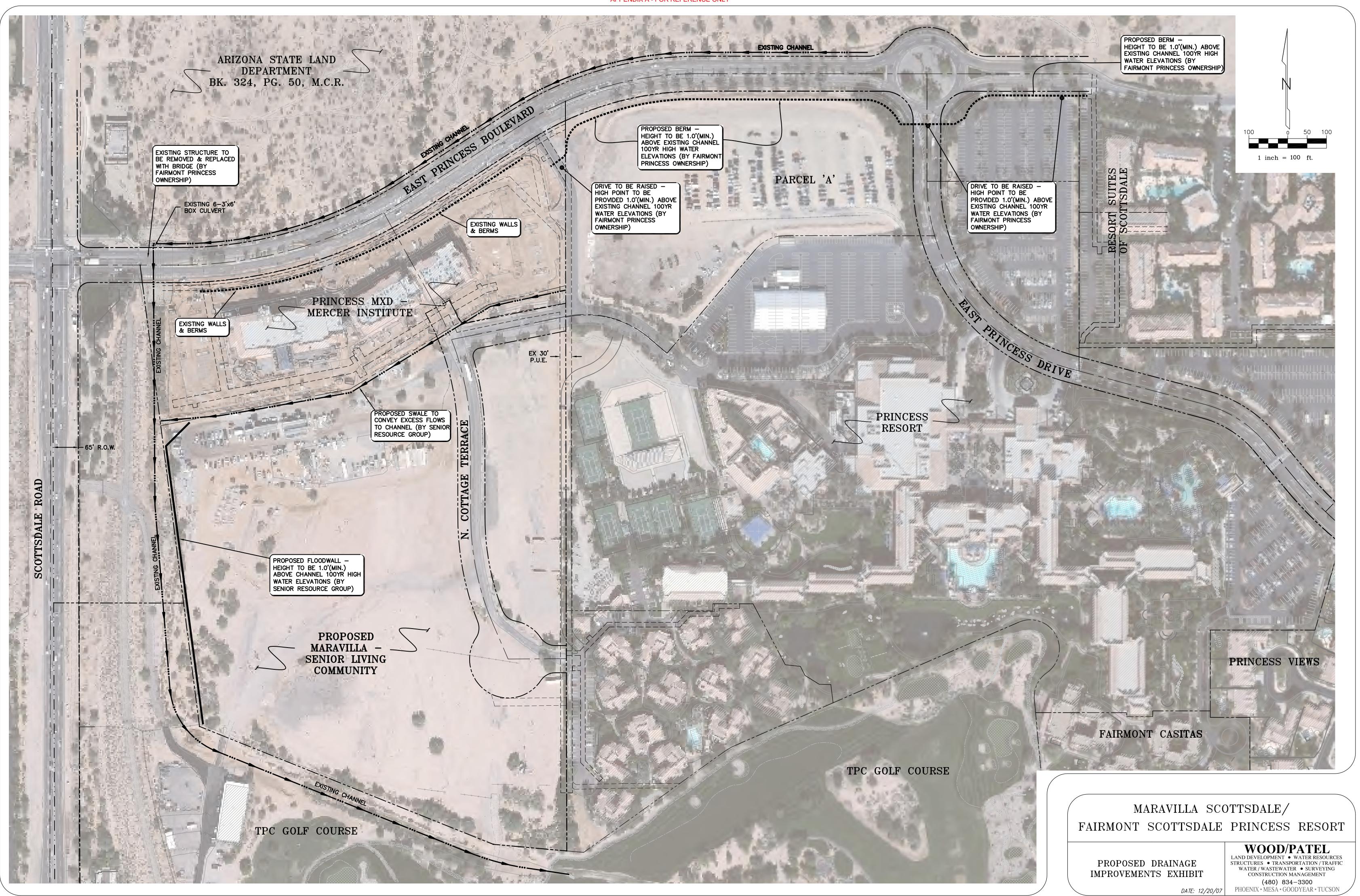
Engineering Preliminary Opinion of Probable Cost (*)

Prepared Bridge Structure at Princess Drive, just east of Scottsdale Road serving unnamed wash.

Estimated Bridge Surface = 8,100 square feet x \$130/s.f. \$1,053,000

(*) Offered without the benefit of construction documents and specifications.





N: \2007\073091\dwg\Exhibits\3091EX01-AERIAL.dwg

APPENDIX C

CITY OF SCOTTSDALE FORMS



Appendix 4-1C WARNING & DISCLAIMER OF LIABILITY

The Drainage and Floodplain Regulations and Ordinances of the City of Scottsdale are intended to "minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding caused by the surface runoff of rainfall" (Scottsdale Revised Code §37-16).

As defined in S.R.C. §37-17, a flood plain or "Special flood hazard area means an area having flood and/or flood related erosion hazards as shown on a FHBM or FIRM as zone A, AO, A1-30, AE, A99, AH, or E, and those areas identified as such by the floodplain administrator, delineated in accordance with subsection 37-18(b) and adopted by the floodplain board." It is possible that a property could be inundated by greater frequency flood events or by a flood greater in magnitude than a 100-year flood. Additionally, much of the Scottsdale area is a dynamic flood area; that is, the floodplains may shift from one location to another, over time, due to natural processes.

WARNING AND DISCLAIMER OF LIABILITY PURSUANT TO S.R.C §37-22

"The degree of flood protection provided by the requirements in this article is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by manmade or natural causes. This article (Chapter 37, Article II) shall not create liability on the part of the city, any officer or employee thereof, or the federal government for any flood damages that result from reliance on this article or any administrative decision lawfully made thereunder."

Compliance with Drainage and Floodplain Regulations and Ordinances does not insure complete protection from flooding. The Floodplain Regulations and Ordinances meet established local and federal standards for floodplain management, but neither this review nor the Regulations and Ordinances take into account such flood related problems as natural erosion, streambed meander or man-made obstructions and diversions, all of which may have an adverse affect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations.

I have read and understand the above. If I am an agent for an owner I have made the owner aware of and explained this disclaimer.

216/2015 Date Owner or Agent

Plan Check No.



Section 404 Certification

Before the City issues development permits for a project, the developer's Engineer or the property owner must certify that it complies with, or is exempt from, Section 404 of the Clean Water Act of the United States. Section 404, administered by the U.S. Army Corps of Engineers (COE), regulates the discharge of dredged or fill material into a wetland, lake, (including dry lakes), river, stream (including intermittent streams, ephemeral washes, and arroyos), or other waters of the United States.

Prior to submittal of improvement plans to Project Review the form below must be completed (and submitted with the improvement plans) as evidence of compliance

Certification of	Section 4	04 Permit Status	5
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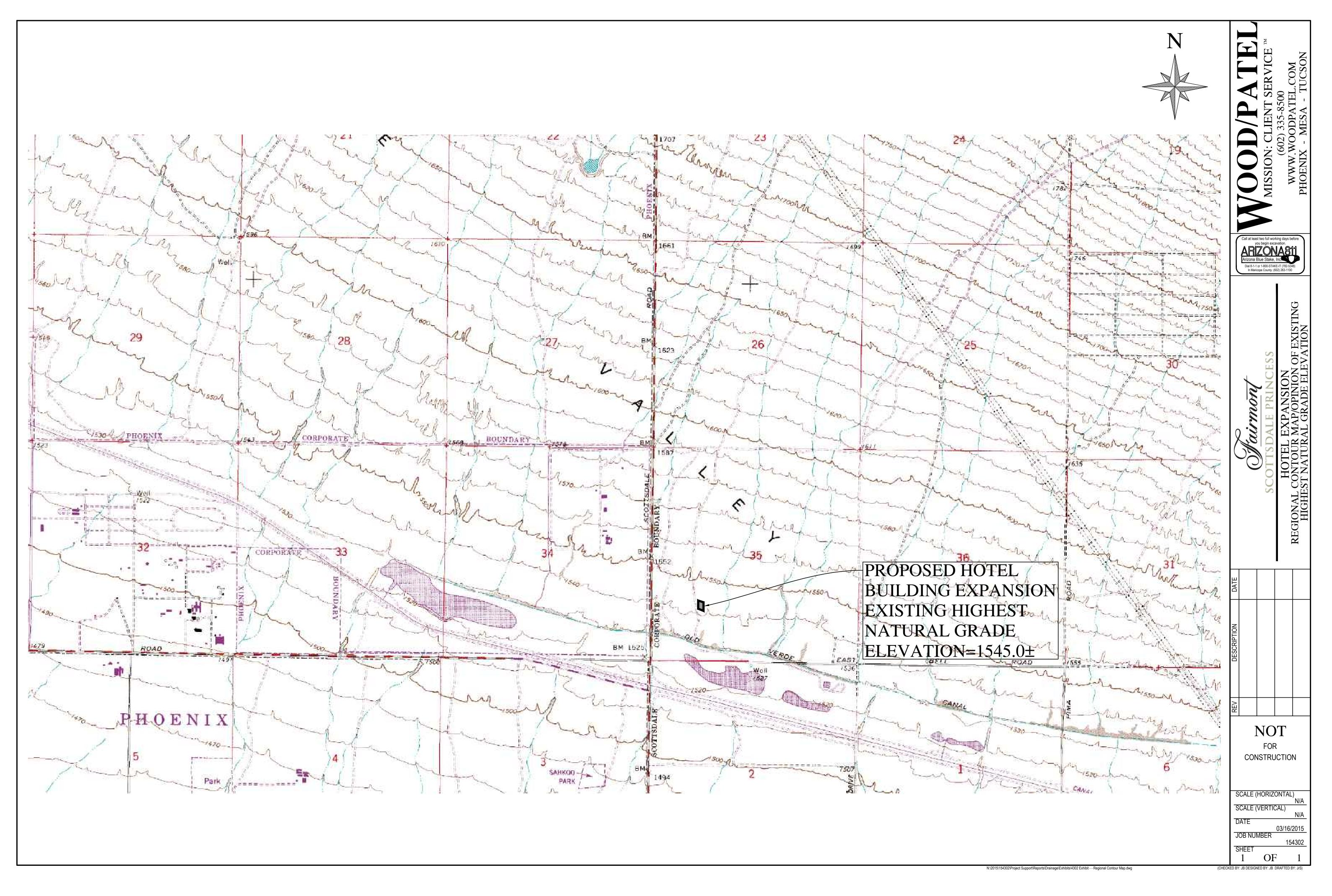
Own	or's N	ame: S	Strategic Hotels and	Resorts	Phone No.	312-658-6016
			Fairmont Scottso			
Proje	ect inar	ne/Description: _	7575	E Princess Blvd	1	
Proje	ect Loc	ation/Address: _	1515	E. Thiless bive.	, 0001130410, 712. 0	
	gister w that		he property Owner m	nust check the applic	cable condition and	certify by signing
1.		ion 404 <u>does</u> ap rs of the U.S., a	ply to the project bec nd:	cause there will be a	discharge of dredge	ed or fill material to
		A Section 404 F	Permit has already bee	n obtained for this pro	oject.	
		-or-				
		This project qua the applicable r	alifies for a "Nationwide nationwide permit.	e Permit," and this pro	ject will meet all term	s and conditions of
2. S	Sectior	a 404 <u>does not</u> a	pply to the project be	ecause:		
	\boxtimes	No watercourse	es or other waters of th	e U.S. exist on the pro	operty.	
		No jurisdictiona Jurisdictional D	al waters of the U.S. ex petermination.	ist on the property. A	ttached is a copy of th	ne COE's
		Watercourses of dr	or other waters of the L edged or fill material in	J.S. do exist on the pr to any of these waters	operty, but the projec s.	t will not involve the
Eng	ineer's	MBn Signature and S out Mana pany No of	Seal, or Owner's Signat ger R, Patcl	TOMAUS CALLER CALLER CALLER	Date	6-15
		Plan	ning & Devel	opment Serv	ices Departn	nent

7447 E Indian School Road, Suite 100, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7088

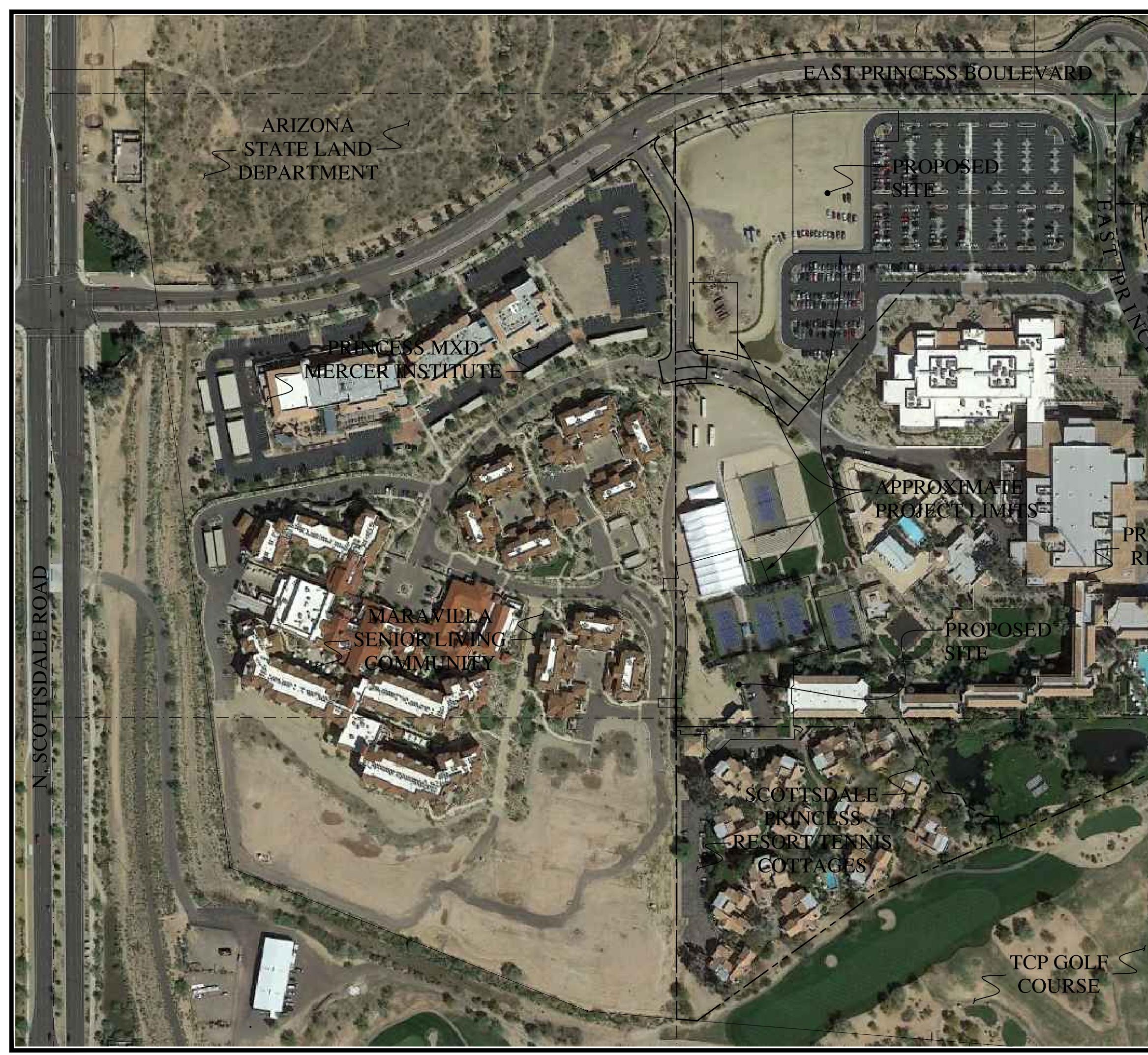
APPENDIX D

OFFSITE WATERSHED EXHIBITS

Regional Contour Map / Opinion of Existing Highest Natural Grade Elevation



Aerial Map



FAIRMONT SCOTTSDALE HOTEL EXPANSION

2015 AERIAL PHOTOGRAPH

WOOD/PATEL 2220 S. Country Club Dr. Mesa, AZ 85210 (480) 834-3300 www.woodpatel.com PHOENIX • MESA • TUCSON

N

JOB# 154302

Plate 1 Exhibit

(from Core North/Core South Drainage Study)

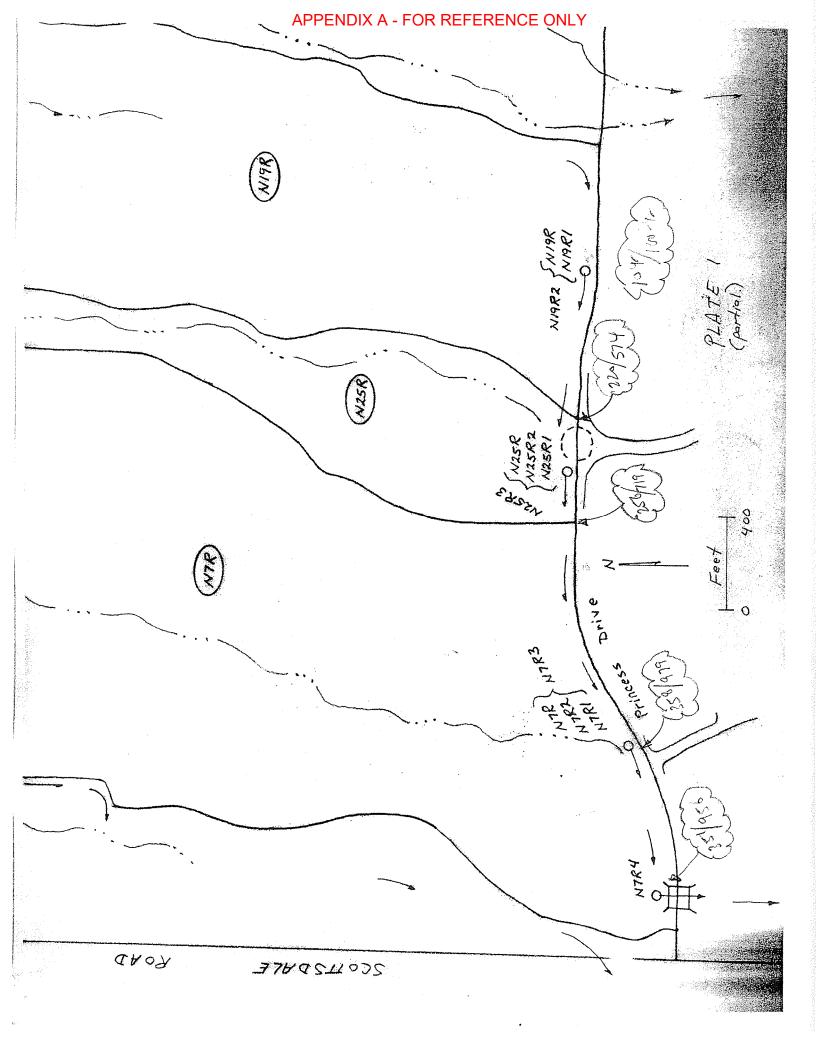


Table 1 Spreadsheet(from Core North/Core South Drainage Study)

**************************************		I	Table 1 Peak Discharge Sum	ımary			
			Princess Drive Cha ont Scottsdale Prin Scottsdale, Arizo	cess Resort			
			l	Peak Disc By Storm Return Inter	harge (cfs) val (6-Hour Duratio	n)	
Location	HEC-1 CP	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
750-Ft East of Roundabout	N19R2	60	146	220	342	444	574
At Roundabout	N25R3	62	167	256	417	549	719
700-Ft Upstream of Culvert	N7R3	90	228	358	583	787	979
At Princess Drive Culvert	N7R4	89	227	351	580	779	956
HEC-1 File:		PRN2	PRN5	PRN10	PRN25	PRN50	PRN100
Note: See Plate 1 f	or HEC-1 concentrat	tion point locations.				5/31/2007	

4. 4

EXHIBIT 1

VICINITY MAP

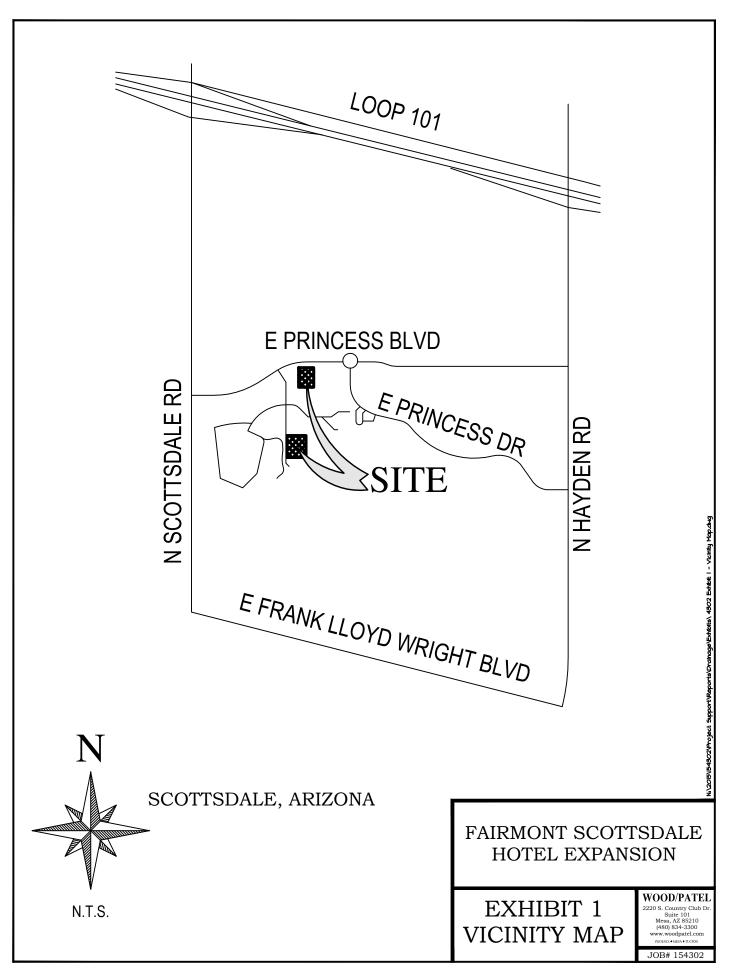


EXHIBIT 2

FEMA MAP

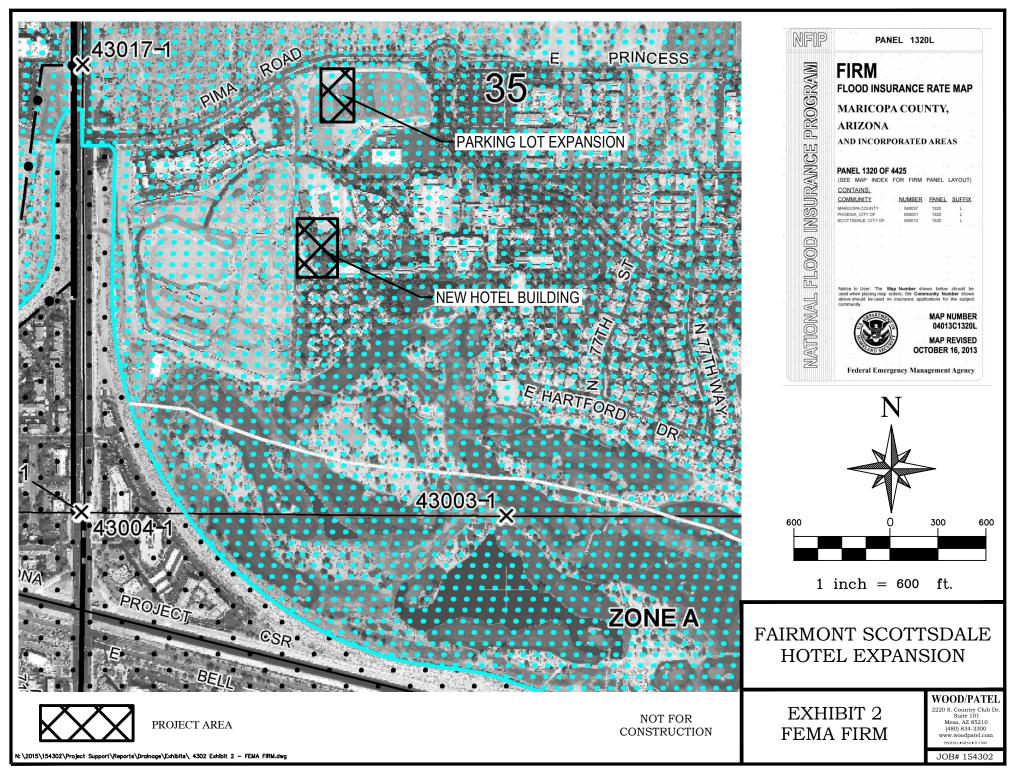


EXHIBIT 3

MARAVILLA EAST PROPERTY LINE ROAD DRAINAGE MAP

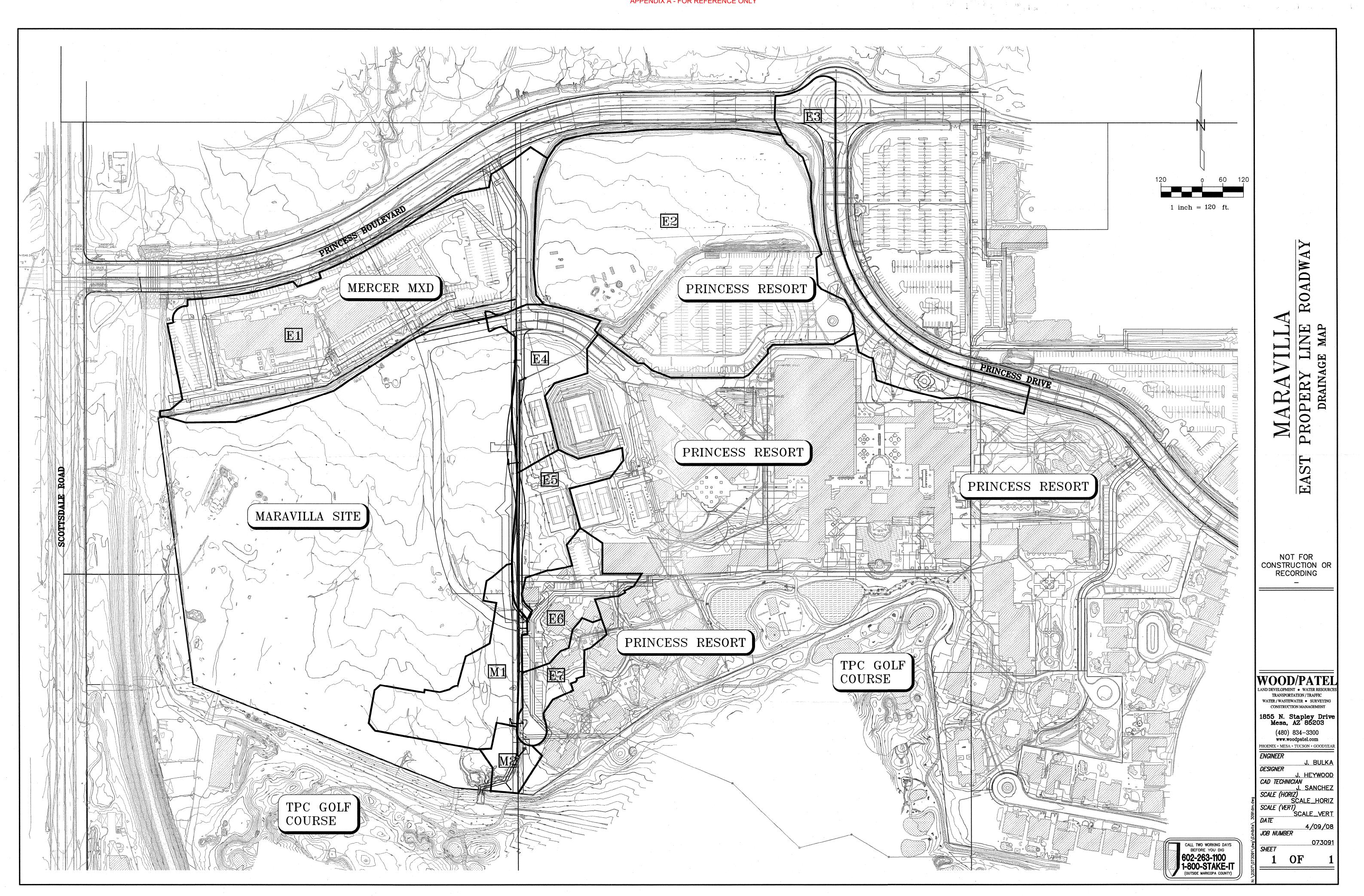




EXHIBIT 4

EXISTING DRAINAGE MAP

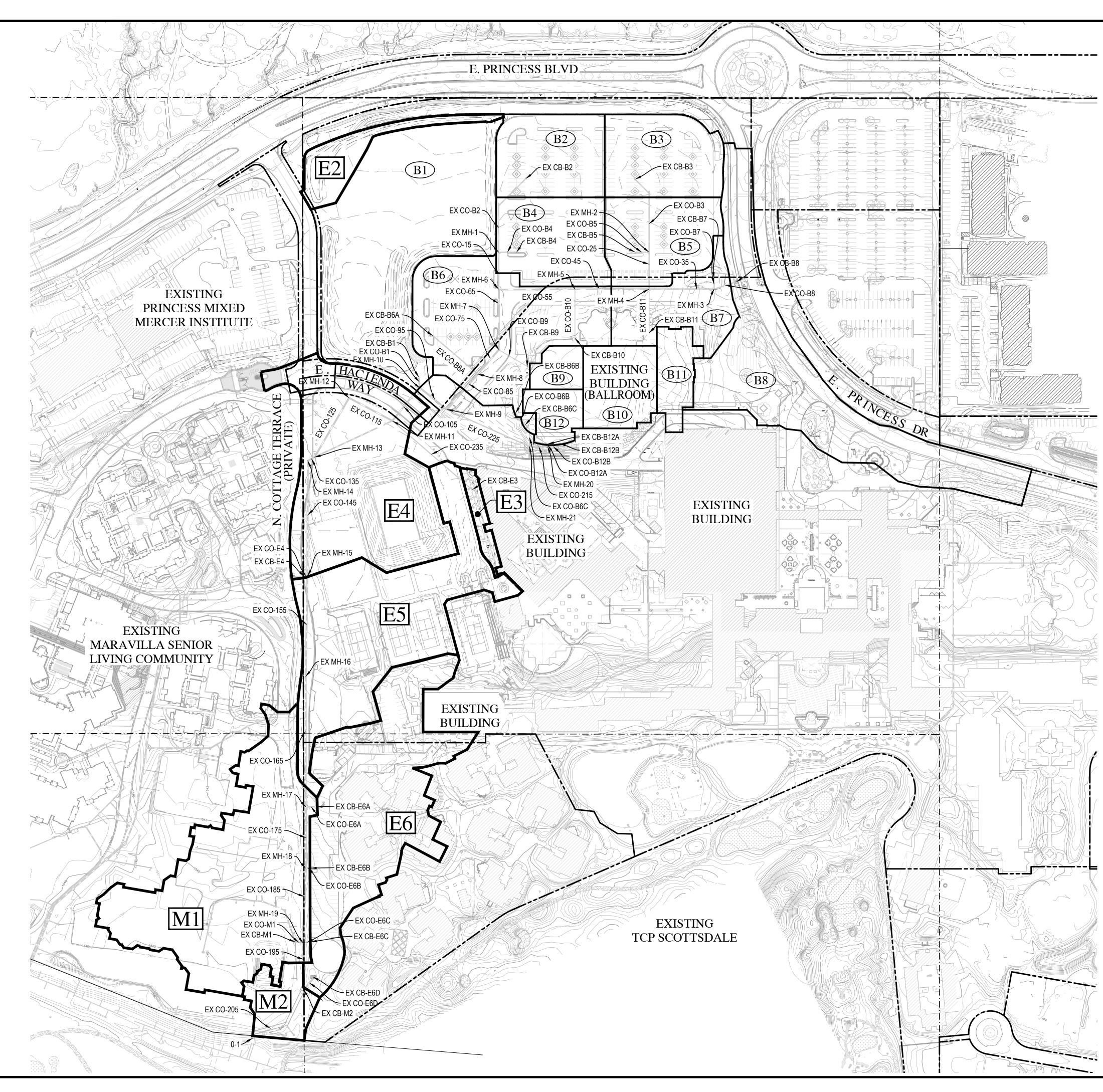


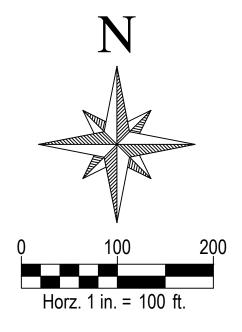


EXHIBIT 4 EXISTING DRAINAGE MAP

WOOD/PATEL 2220 S. Country Club Dr. Mesa, AZ 85210 (480) 834-3300 www.woodpatel.com

PHOENIX • MESA • TUCSON

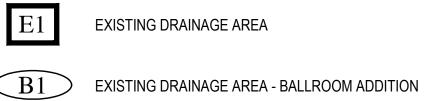
JOB# 154302



LEGEND

E1

BASIN BOUNDARY EXISTING DRAINAGE AREA



----- PROPERTY BOUNDARY FLOW ARROW EXISTING STORM DRAIN



EXISTING DRAINAGE AREA - MARAVILLA

FAIRMONT SCOTTSDALE

HOTEL EXPANSION

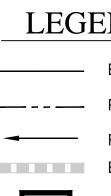
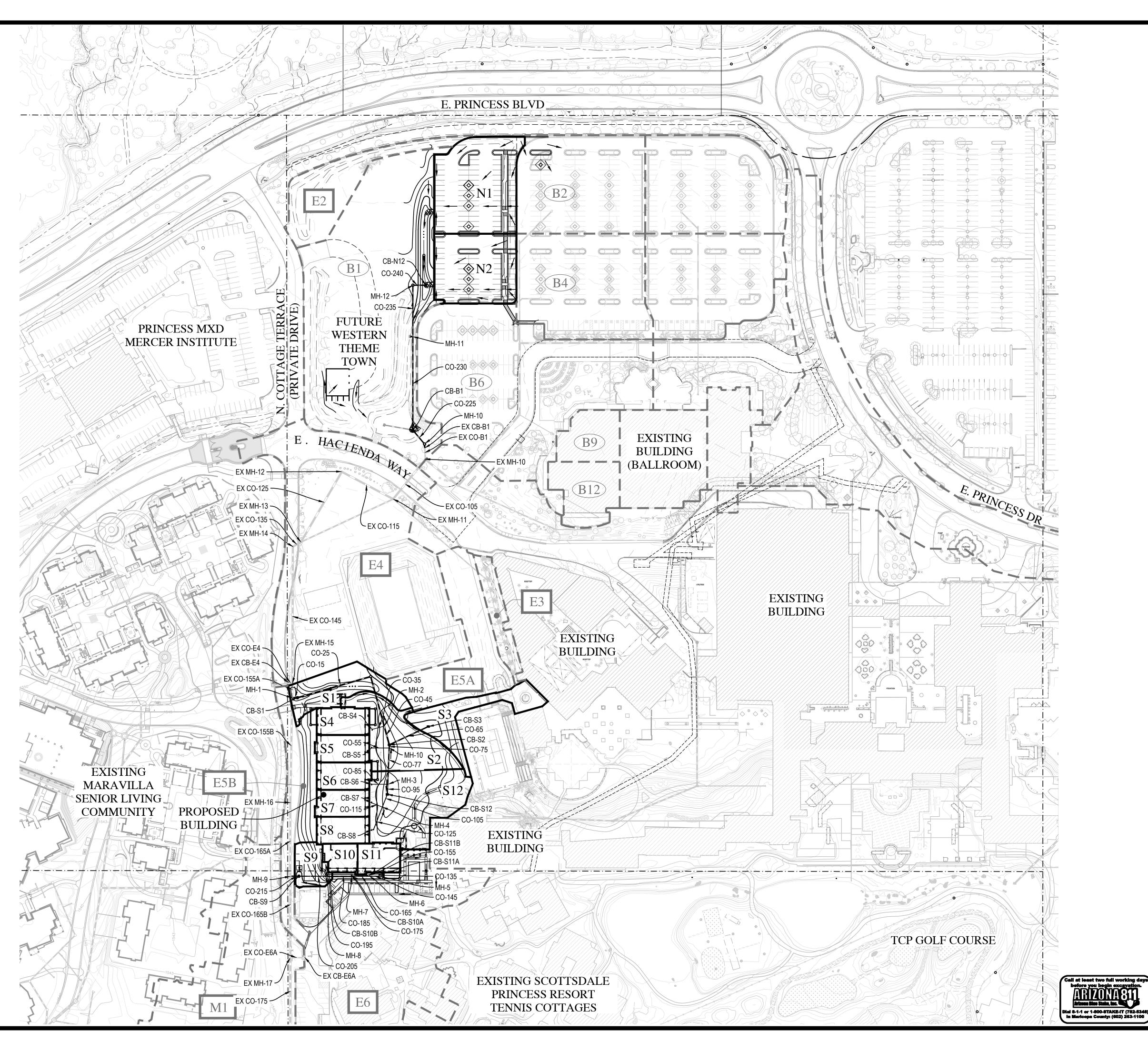
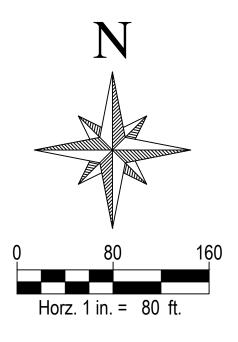


EXHIBIT 5A

PROPOSED DRAINAGE MAP 1





LEGEND

 \frown

	PROPOSED DRAINAGE AREA
	EXISTING DRAINAGE AREA
	PROPERTY BOUNDARY
	FLOW ARROW
	PROPOSED STORM DRAIN
	EXISTING STORM DRAIN
	EXISTING INTERMEDIATE CONTOUR
	EXISTING INDEX CONTOUR
\sim	PROPOSED INTERMEDIATE CONTOUR
\sim	PROPOSED INDEX CONTOUR
N1	PROPOSED DRAINAGE AREA - NORTH
S 1	PROPOSED DRAINAGE AREA - SOUTH
E1	EXISTING DRAINAGE AREA
Bl	EXISTING DRAINAGE AREA - BALLROOM ADDITION
M 1	EXISTING DRAINAGE AREA - MARAVILLA
ULTIMATE OUTFALL	ULTIMATE OUTFALL

FAIRMONT SCOTTSDALE HOTEL EXPANSION

EXHIBIT 5A PROPOSED DRAINAGE MAP-1

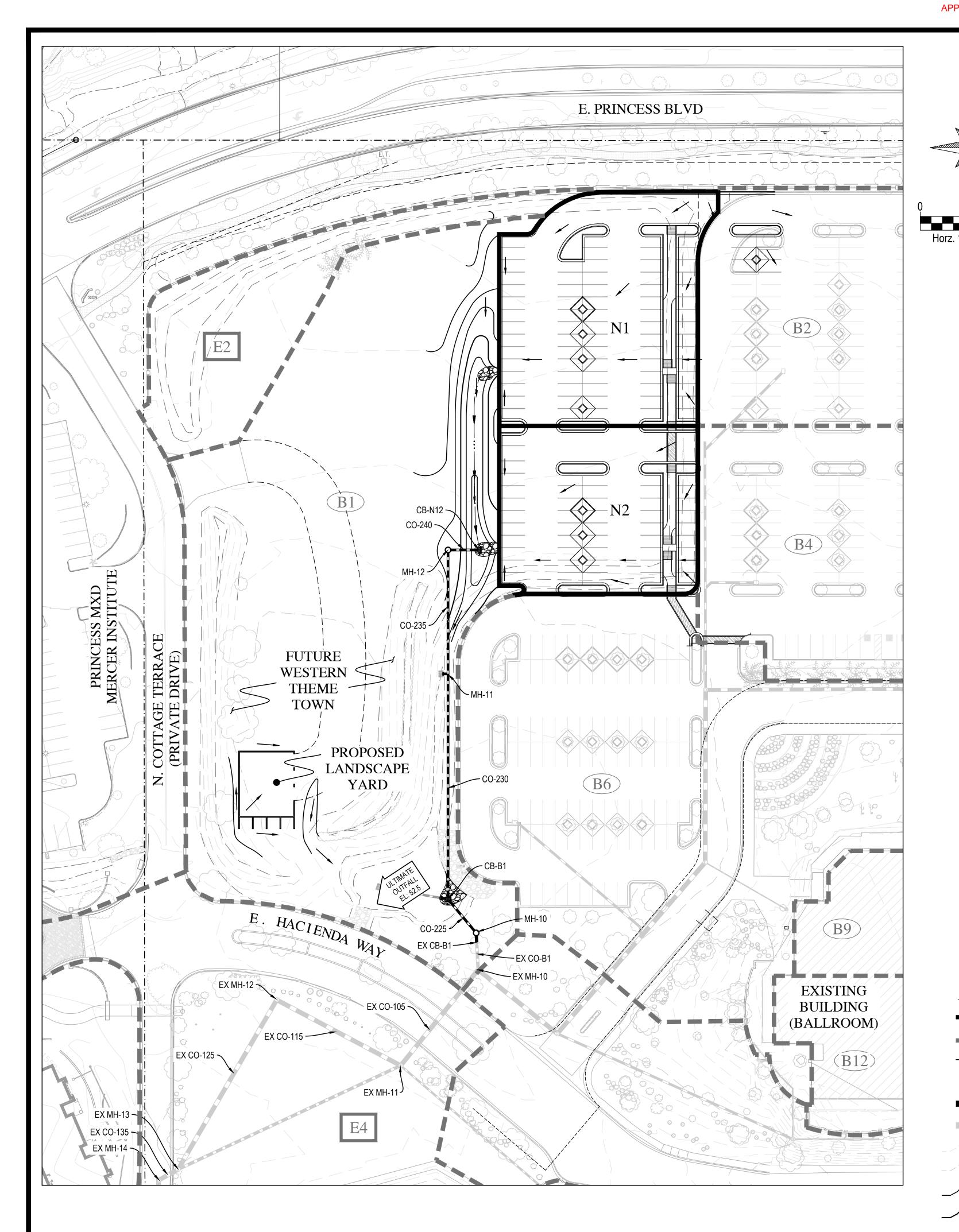
WOOD/PATEL

2220 S. Country Club Dr Mesa, AZ 85210 (480) 834-3300 www.woodpatel.com PHOENIX • MESA • TUCSON

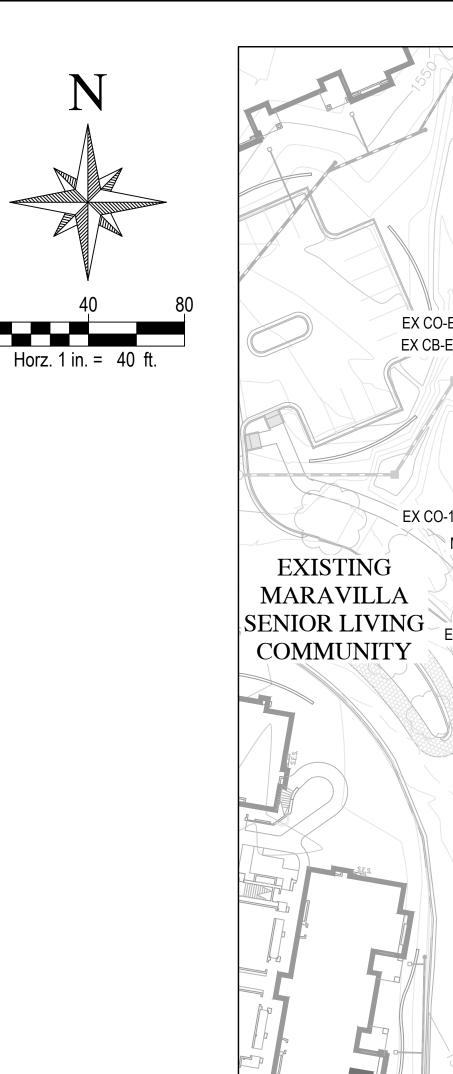
JOB# 154302

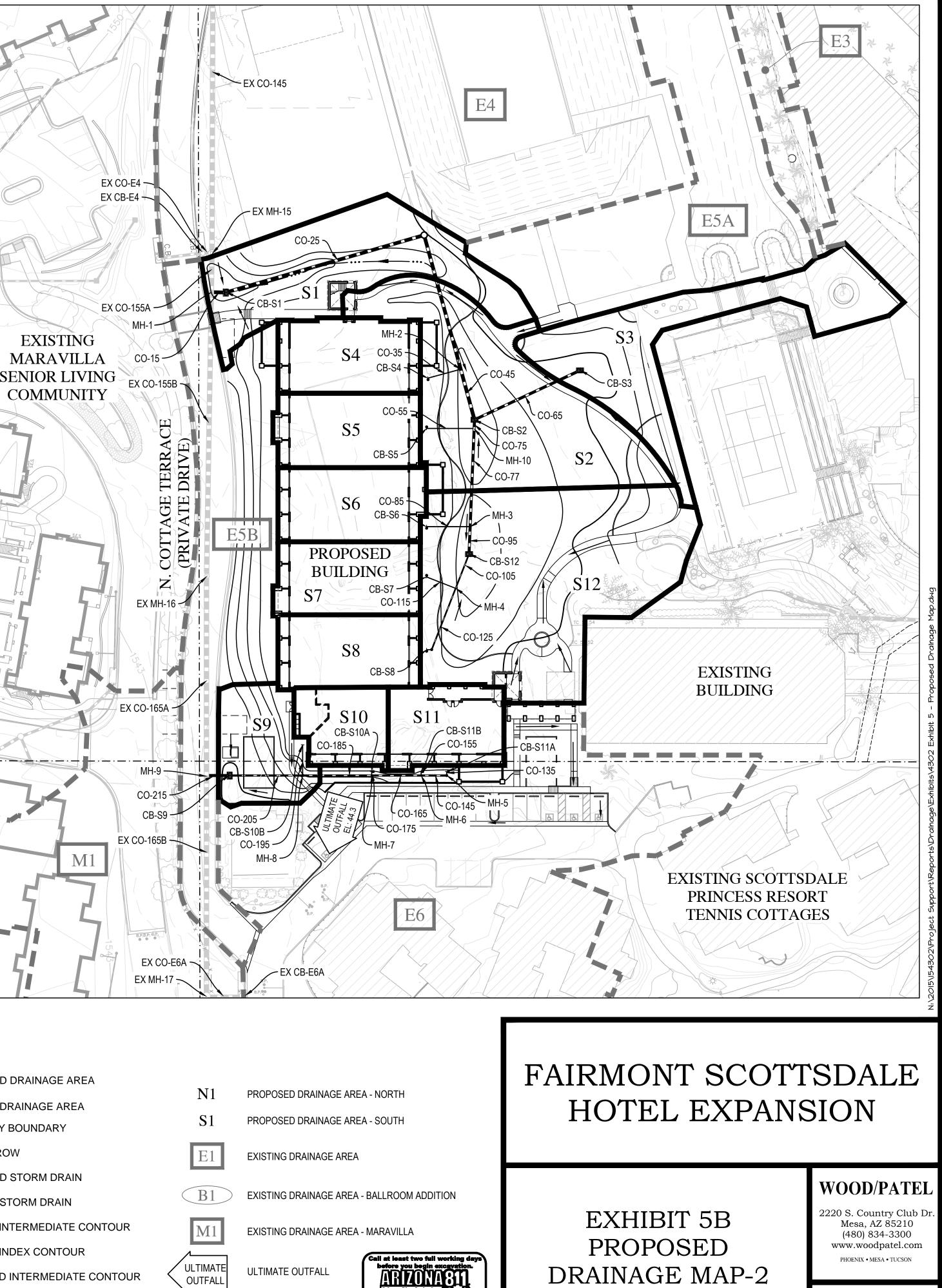
EXHIBIT 5B

PROPOSED DRAINAGE MAP 2









LEGEND

	PROPOSED DRAINAGE AREA	NT1	
	EXISTING DRAINAGE AREA	N1	PROPOSED DR/
	PROPERTY BOUNDARY	S 1	PROPOSED DR
◄	FLOW ARROW	E1	EXISTING DRAII
	PROPOSED STORM DRAIN		
	EXISTING STORM DRAIN	(B1)	EXISTING DRAI
	EXISTING INTERMEDIATE CONTOUR	M 1	EXISTING DRAII
	EXISTING INDEX CONTOUR		
\sim	PROPOSED INTERMEDIATE CONTOUR		ULTIMATE OUTI
\sim	PROPOSED INDEX CONTOUR	\	

before you begin excavation ARIZONA81 Arizees Bies State, Inc. Dial 8-1-1 or 1-800-STAK in Maricopa County: (602) 263-1

JOB# 154302

APPENDIX B – DRAINAGE REPORT FOR FAIRMONT SCOTTSDALE WESTERN THEME TOWN BY WOOD, PATEL & ASSOCIATES, INC., DATED MAY 15, 2015

DRAINAGE REPORT FOR FAIRMONT SCOTTSDALE WESTERN THEME TOWN

May 15, 2015 WP# 154302.10 C.O.S. No.: 321-PA-2015

Prepared for:

Strategic Hotels and Resorts Mr. George Stowers 200 West Madison Suite 1700 Chicago, Illinois 60606 *Phone: (312) 658-6016*

Submitted to:

City of Scottsdale 9388 East San Salvador Drive Scottsdale, Arizona 85258 Phone: (480) 312-5636

Prepared by:

Wood, Patel & Associates, Inc. 2220 South Country Club Drive Suite 101 Mesa, Arizona 85210 Phone: (480) 834-3300 Website: www.woodpatel.com





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	2.2	Offsite Drainage Conditions	2
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3.0	PROF	POSED DRAINAGE PLAN	5
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	5.2	Hydraulic Analysis	8
6.0	CONC	CLUSIONS	9
7.0	REFE	RENCES 1	.0

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- Appendix A Hydrologic and Hydraulic Calculations
- Appendix B Stormwater Storage Waiver / Proposed Drainage Improvements Exhibit
- Appendix C City of Scottsdale Forms
- Appendix D Offsite Watershed Exhibits
 - Regional Contour Map / Opinion of Existing Highest Natural Grade Elevation
 - Aerial Map
 - Plate 1 Exhibit (From Core North/Core South Drainage Study)
 - Table 1 Spreadsheet (From Core North/Core South Drainage Study)

EXHIBITS

- Exhibit 1 Vicinity Map
- Exhibit 2 FEMA Map
- Exhibit 3 Existing Drainage Map
- Exhibit 4 Proposed Drainage Map

km X:\Y-Drive\WP\Reports\Commercial\154302.10 Fairmont Scottsdale Western Theme Town Drainage Report.docx



Drainage Report Fairmont Scottsdale Western Theme Town WP# 154302.10

/OOD/PATEL

i

1.0 INTRODUCTION

The proposed Fairmont Scottsdale Western Theme Town (Town) is a western-themed, outdoor entertainment theater with associated pantry and restroom facilities. The proposed development is located adjacent to the existing parking lot for the existing Fairmont Ballroom, located east of Scottsdale Road and south of Princess Drive. The project includes three (3) new buildings (refer to Exhibit 1 - Vicinity Map). The proposed Town is located along Cottage Terrace, on approximately 2.26 acres (ac), and is west of the existing parking lot for the Ballroom. More specifically, the site is located in the southwest quarter of Section 35, Township 4 North, Range 4 East of the Gila and Salt River Meridian (refer to Exhibit 1). The existing property, currently zoned C-2, is primarily undeveloped with a few bushes and trees. Current zoning is expected to stay the same for the proposed improvements.

The purpose of this report is to obtain City of Scottsdale approval for the proposed Fairmont Scottsdale Western Theme Town, with respect to the City of Scottsdale's drainage criteria.

Analysis and modeling for this study was performed in accordance with the requirements of the *Design Standards and Policies Manual, Chapter 4: Grading and Drainage* (DS&PM), City of Scottsdale, 2010 (Ref. 1). The methods of analysis, sources of data and assumptions, and the results of the analysis are discussed in detail in the following sections of this report.

2.0 EXISTING DRAINAGE CONDITIONS AND CHARACTERISTICS

2.1 FEMA Floodplain

The Federal Emergency Management Agency (FEMA) has published a 100-year floodplain, per Flood Insurance Rate Map (FIRM) Panel 1320 of 4425, Map Number 04013C1320L, dated October 16, 2013. The site is within a flood zone labeled "AO-Depth 1 Foot, Velocity 3 FPS".

Zone "AO" is defined by FEMA as follows:

"Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined."

As displayed on the FEMA FIRM panel, the floodplain delineation includes a large portion of land, which includes this project site (refer to Exhibit 2 – *FEMA Map*).

It is the understanding of Wood, Patel & Associates, Inc. (Wood/Patel), based on past experience and interpretations of the City of Scottsdale's floodplain ordinance that development of land within a FEMA Zone "AO" is acceptable as long as, in general, the lowest floor elevation is above or properly protected from the anticipated 100-year water surface elevations. No problems are anticipated with developing the parcel in accordance with the City's floodplain ordinance. No underground structures or parking structures are proposed.

2.2 Offsite Drainage Conditions

A large watershed occurs north of the Fairmont Scottsdale Resort. The watershed has been previously studied and peak flows estimated for existing and proposed conditions, and results were published in the Core North/Core South Drainage Study (Ref. 5). Refer to Appendix D - Offsite Watershed Exhibits for the Regional Contour Map, Aerial Map, and Plate 1 and Table 1 from the Core North/Core South study and report (Ref. 5). The Fairmont Scottsdale Resort property is protected from this offsite flow by several past improvements. Originally, when Princess Boulevard was constructed, offsite flows were diverted to the west by a channel on the north site of Princess Boulevard. The flood waters are routed to an existing drainage culvert under Princess Boulevard, just east of

Scottsdale Road. These flood waters are routed south along the east side of Scottsdale Road to the Tournament Players Club (TPC) Golf Course. As the Fairmont Scottsdale Resort continued to develop over the years, the City of Scottsdale requested additional improvements to protect the property from offsite flows from the north. Additional improvements were contingent with the Request for Stormwater Storage Waiver for the Fairmont Scottsdale Resort property in 2008 as development continued (refer to Exhibit B – Stormwater Storage Waiver / Proposed Drainage Improvements Exhibit). The additional improvements included adding a flood/screen wall along the south side of Princess Boulevard, providing a high point on Cottage Terrace just south of Princess Drive, providing a high point just south of the traffic circle on Princess Drive, providing a channel and floodwall along the west side of the Maravilla site, and removing and replacing the existing culverts on Princess Drive with a new bridge structure. Currently, all of these improvements have been constructed with past projects (Fairmont Ballroom Addition and the Maravilla Senior Living Community) with the exception of replacing the culverts on Princess Drive with a bridge structure. It is our understanding the Fairmont ownership is working with the City of Scottsdale on an agreement to replace these culverts.

The Hayden/Rhodes (Granite Reef) Aqueduct was constructed as part of the (CAP) by the U.S. Bureau of Reclamation. This existing aqueduct is located along the southern edge of the TPC golf course, and extends east-west the length of the CAP canal. The aqueduct is at a significantly higher elevation than the golf course and acts as a dike which creates stormwater ponding in the existing golf course. The 100-year high water level is at an elevation of 1536.00 NGVD 29 Datum (Ref. 6). The conversion to the NAVD 88 Datum is 1.8 feet, so the 100-year high water level elevation is approximately 1537.80. The proposed finish floors for the new Town buildings will be elevation 1557.00 for the pantry/restroom and restroom buildings, and elevation 1559.30 for the theater building, which are19.20 and 21.50 feet above the 100-year high water elevation, respectively.

When the Maravilla Senior Living Community was constructed, Cottage Terrace Drive (a private drive) was relocated to the west side of the Fairmont Scottsdale Resort property to accommodate vehicle access to the Resort. A storm drain system was installed with the driveway improvements to benefit both properties. An analysis of the storm drain system

was done as a part of the Fairmont Scottsdale Hotel Expansion project for the existing and proposed conditions, and results were published in the *Drainage Report for the Fairmont Scottsdale Hotel Expansion* (Ref. 9).

Runoff from the existing Princess MXD Mercer Institute (MXD) site drains to the west to the existing flood control channel located between Scottsdale Road and the Maravilla project (Ref. 9).

2.3 Onsite Drainage Conditions (Pre-Development)

The existing site slopes generally from the northeast to the southwest with an average slope of approximately two percent (2%). Stormwater is captured into an existing storm drain system via existing catch basins and curb inlets located within Cottage Terrace Drive (private drive), which ultimately drains into the existing TPC Golf Course at the Fairmont Scottsdale Princess Resort, located immediately north of the Hayden-Rhodes Aqueduct. Currently, the site consists of a variety of desert landscaping (bushes and trees). There is some undeveloped area at the northeast corner of the property that is currently dirt and decomposed granite (refer to Exhibit 3). A C-value of 0.37 was used for the existing conditions of the site for desert landscaping (Ref. 9).

3.0 PROPOSED DRAINAGE PLAN

3.1 Onsite Drainage Conditions (Post-Development)

This site was designed to utilize the existing storm drain system in Cottage Terrace Drive and to maintain the existing site outfall locations at the southwest corner of the property. During a 10-year storm event, the proposed storm drain system is designed to capture the runoff through a series of catch basins at low points throughout the project site and convey it to the existing storm drain system located within Cottage Terrace. During a 100-year storm event, the runoff is expected to exceed the capacity of the proposed storm drain system and the excess will be conveyed overland (south on Cottage Terrace Drive) to the outfall at the TPC golf course. The proposed grading is designed to allow for approximately 2.6 feet of ponding for the area to the north before the runoff overtops the local high points and continue towards the outfall location. This depth is a temporary condition, and will likely reduce once the area is developed. Cottage Terrace Drive has been designed to handle the 100-year flow, per the approved Drainage Report for Maravilla East Property Line Road, dated May 12, 2008, revised October 15, 2008 (Ref. 4). A weighted C-value of 0.41 was used for the proposed condition of the site for desert landscaping and the buildings of the Town (Ref. 9). Tributary areas, flow directions, catch basins, storm drains, 100-year overland flow paths, and the ultimate site outfall location are labeled on Exhibit 4.

3.2 Lowest Habitable Finished Floor Elevations

The Grading and Drainage Plan has been designed to comply with the City of Scottsdale's floodplain ordinance for a Zone "AO" floodplain. It is our understanding, unless other flood proof measures are presented and approved, the proposed lowest habitable finished floor (LFF) elevation must be designed a minimum of 1 foot above the anticipated 100-year flood elevation. As previously mentioned, the proposed finish floors for the new Town buildings will be at elevations 1557.00 and 1559.30, or 19.20 and 21.50 feet, respectively, above the 100-year high water elevation in the TPC Golf Course. In addition, the lowest finish floor elevation is designed to be a minimum of 1 foot above the adjacent finish grade. Refer to Exhibit 4 for a graphical representation.

The Site is located within a Zone "AO" floodplain, defined as having a flood depth of 1 foot. Therefore, the anticipated 100-year flood elevation is 1 foot above "natural" grade, and proposed LFF elevation must have 1 foot additional freeboard or be a minimum of 2 feet above natural grade. Due to the disturbed condition of the Site from previous

development, the natural grade has been modified. According to Curry's Corner 7.5 minute Topographic Survey Map by USGS from 1964 with a contour interval of 10-feet, the approximate highest natural grade of this site prior to development is 1555.00, which is 2.00 and 4.30 feet lower than the proposed lowest finish floors of 1557.00 and 1559.30, respectively. It is our understanding this is in compliance with the City's floodplain ordinance. Refer to the *Regional Contour Map* within Appendix D, which shows the highest natural grade elevation relative to the proposed building location.

3.3 Stormwater Retention

A Request for Stormwater Storage Waiver was submitted and approved by the City of Scottsdale on October 23, 2008 (refer to Appendix B). Therefore, onsite stormwater retention is not required.

3.4 Operation and Maintenance

The property owner shall be solely responsible for the operation and maintenance of the stormwater drainage system.

4.0 SPECIAL CONDITIONS

4.1 Special Conditions

Currently, there are no washes with 100-year flows greater than 50 cfs that traverse the project site. Also, there are no designated Section 404 washes within the site; therefore, no Section 404 permit is required.

5.0 DATA ANALYSIS

5.1 Hydrologic Analysis

The drainage improvements are to be developed consistent with Chapter 4 of the City of Scottsdale *Design Standards and Policies Manual*, 2010. The Rational Method has been used to quantify peak discharge values for onsite concentration points for the proposed build out scenario during the 10-year and 100-year storm events. Weighted "C" runoff coefficients were referenced from Chapter 4 of the City of Scottsdale *Design Standards and Policies Manual*, 2010. Refer to Appendix A – *Hydrologic and Hydraulic Calculations* for inlet capacity information, Exhibit 4 – *Proposed Drainage Map* for drainage basin tributary area and concentration point locations, and the *Drainage Report for Fairmont Scottsdale Hotel Expansion* for Rational Method calculations and rainfall data.

5.2 Hydraulic Analysis

The onsite storm drain system has been designed to accommodate the 10-year storm event. Bentley StormCAD Version 8i was utilized to analyze the existing and proposed storm sewer system with printouts and storm drain profiles as part of the *Drainage Report for the Fairmont Scottsdale Hotel Expansion* (Ref. 9).

6.0 CONCLUSIONS

The project site is believed to be capable of development as presented with regard to the City of Scottsdale's onsite and offsite drainage requirements. The project's drainage highlights are as follows:

- The site is protected from offsite flows from the north by improvements, as outlined in the approved stormwater storage waiver.
- The proposed drainage infrastructure has been designed in accordance with the City of Scottsdale *Design Standards & Policies Manual*, 2010. The Rational Method was used to estimate peak discharges for all drainage areas. The flow capacities of the proposed storm system have been designed to accept the 10-year storm event.
- No stormwater retention has been provided for this project, per the approved stormwater storage waiver from the City of Scottsdale.
- The onsite 100-year storm event is to be conveyed south, by storm drain and overland flow, to the existing TPC Golf Course, per the *Drainage Report for Fairmont Scottsdale Hotel Expansion*.
- The 100-year high water elevation is 1537.80 in the adjacent TPC golf course, which is 19.20 and 21.50 feet below the proposed new buildings lowest finish floor elevations of 1557.00 and 1559.30, respectively.
- The project site is in a FEMA designated 100-year floodplain (Zone "AO-Depth 1 foot") in both pre- and post-development conditions.
- The lowest finish floor elevations of 1557.00 and 1559.30 are approximately 2.00 and 4.30 feet higher than the highest natural ground elevation of 1555.00. It is our understanding this is in compliance with the City's floodplain ordinance, which requires the lowest finished floor to be a minimum of 2 feet above (1 foot for AO and 1 foot of freeboard) the natural highest grade.

7.0 **REFERENCES**

- 1. Design Standards and Policies Manual, Chapter 4: Grading and Drainage, City of Scottsdale, January 2010.
- 2. Onsite Drainage Report for Fairmont Scottsdale Ballroom Addition, Wood, Patel & Associates, Inc., July 14, 2011.
- 3. Offsite Drainage Report for Fairmont Scottsdale Ballroom Addition, Wood, Patel & Associates, Inc., July 14, 2011.
- 4. *Drainage Report for Maravilla East Property Line Road*, Wood, Patel & Associates, Inc., dated May 12, 2008, revised October 15, 2008.
- 5. *Drainage Study Core North/Core South Scottsdale, AZ*, Robert Ward, P.E., Consulting Engineering, September 25, 2001. Prepared for Arizona State Land Department.
- Sedimentation Report Reach 11 Dikes Hayden/Rhodes (Granite Reef) Aqueduct, Earth Science Division, Surface Water Branch, Sedimentation Office of Denver, Colorado, July 1990.
- Maravilla Scottsdale Senior Living Community, Wood, Patel & Associates, Inc., December 18, 2008, Revised February 13, 2009.
- 8. Curry's Corner Quadrangle, 7.5 Minute Series Topographic Map, USGS, 1964.
- Drainage Report for Fairmont Scottsdale Hotel Expansion, Wood, Patel & Associates, Inc., May 1, 2015.

APPENDIX A

HYDROLOGIC AND HYDRAULIC CALCULATIONS

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Inlet Capacity Summary

Description:Summary of Inlet Sizing CalculationsLocation:Fairmont Scottsdale Western Town

Inlet ID	Contributing Drainage Area ID*	Q10 (cfs)*	Available Head (ft)	Inlet Type	Inlet Capacity (cfs)	10-Year Actual Ponding Depth (ft)
MAG	1/2 B1	2.1	1.0	MAG 535	25.50	0.17
Neenah	1/2 B1	2.1	1.0	Neenah R-2557	9.60	0.15

*Contributing drainage area ID and flow information taken from the *Drainage Report for the Fairmont Scottsdale Hotel Expansion*, Wood, Patel and Associates, dated May 1, 2015.

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

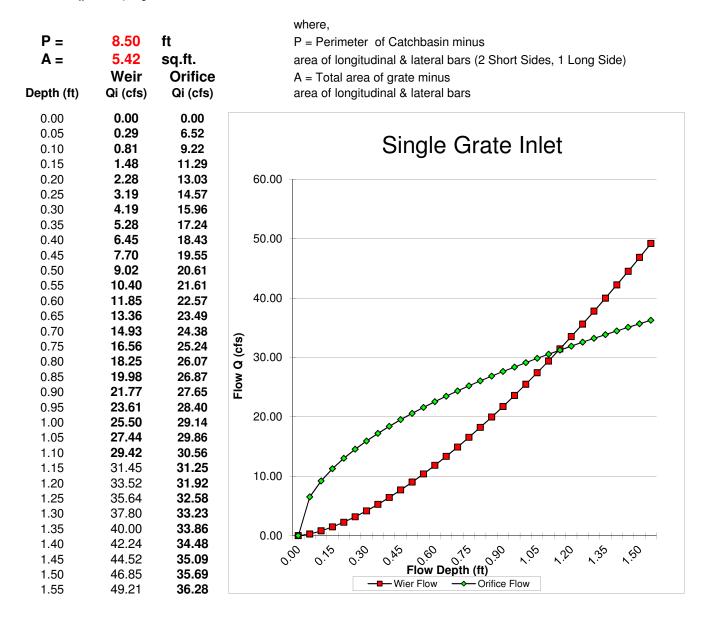
Inlet Capacity - Sump Locations

Description:	Calculation of Inlet Capacity for Single MAG 535 Catch Basin (w/o Curb)
Location:	Fairmont Scottsdale Western Town
Reference:	Drainage Design Manual for Maricopa County, Vol. II, Hydraulics, pg. 3-27

Weir EQ. $Q_i = C_w Pd^{1.5}$

Orifice EQ. $Q_i = C_0 A(2gd)^{0.5}$

Where: $C_w = 3.0$, $C_o = 0.67$





ENGINEERING TOOLS & CALCULATORS LITERATURE & VIDEOS SALES STAFF DISTRIBUTION YARDS NEENAH PRODUCTS

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ENGINEERING TOOLS

Modified Manning Calculators

Weir and Orifice Calculator

- Weir Flow
- Orifice Flow

Curb Opening Hydraulics Calculator

R-4999 Vane Trench Grate Hydraulics

Neenah Grate Information

Engineering Literature & Videos

WEIR & ORIFICE CALCULATOR

The Weir and Orifice Calculator is used to determine the inlet capacity in sag (ponding) conditions by use of the Weir and Orifice equations. Knowing this information will allow you to select the proper grate type and size for your specific job or project.

HOME // MUNICIPAL // ENGINEERING TOOLS & CALCULATORS // WEIR AND ORIFICE CALCULATOR

Orifice Flow Calculations

• Q = Capacity in CFS

• h = Head in feet Orifice Information

Orifice Flow Equation: $Q = 0.6A \sqrt{2gh}$

• g = 32.2 (feet per sec/sec)

• A = Free open area of grate in sq. ft.

Weir Flow Calculations

- P = Feet perimeter
- h = Head in feet
- Weir Information

Instructions:

2. Enter head value

1. Select a catalog number (will automatically fill in Open Area and Perimeter) or enter your own values

The results will determine automatically if your situation falls into a Weir, Transitional or Orifice flow. Additionally, Neenah grates which fall within the parameters chosen will appear below the calculator

Catalog Number and Grate Type:

R-2557:G	~	
Feet perimeter (P):	Head in feet (h):	Free open area in sq. ft. (A):
8.4	1	2.0
	Calculate	
Weir capacity in cfs:	Transitional flow in cfs:	Orifice capacity in cfs:
		9.6
Catalog Number		Grate Type
R-1792-GG		
		G
R-1879-A6G		G A or C
R-1879-A6G R-2557		

- Weir Equation: Q = 3.3P(h)^{1.5}
 - Q = Capacity in CFS

R-2560-F2

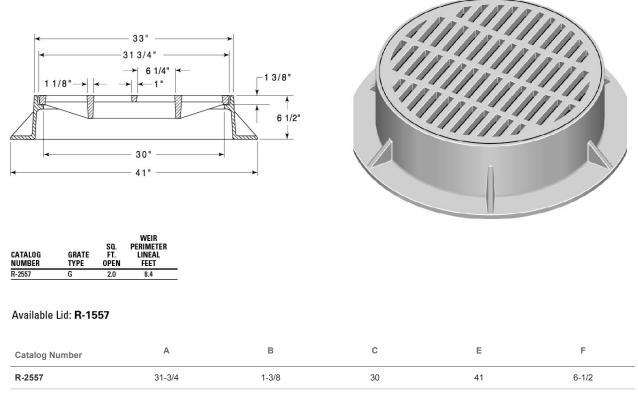
- 3. Click "calculate"



R-2557 - Inlet Frames & Grates

R-2557

Inlet Frame, Grate Heavy Duty



All dimensions are listed in inches unless otherwise noted.

To order Neenah Foundry products, please call 800.558.5075.

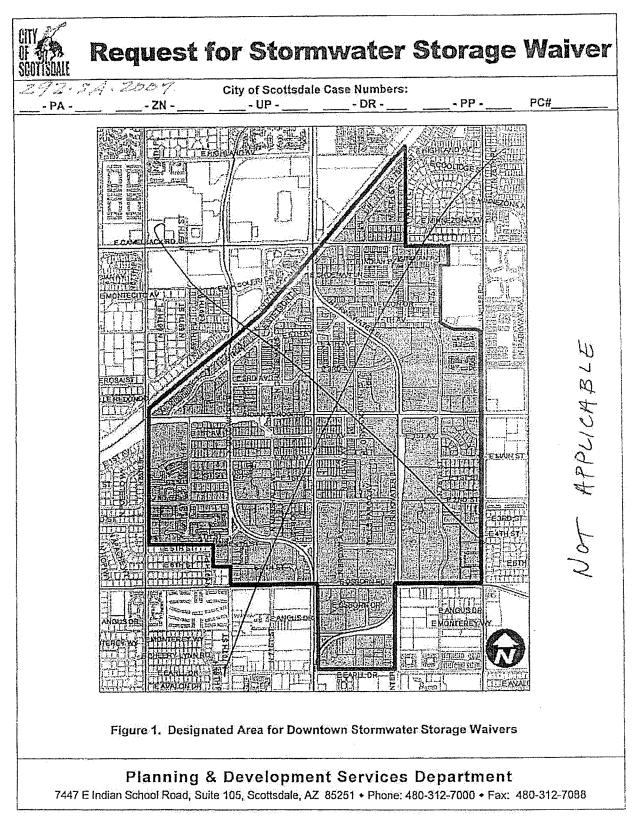
© 2015 Neenah Foundry, all rights reserved.

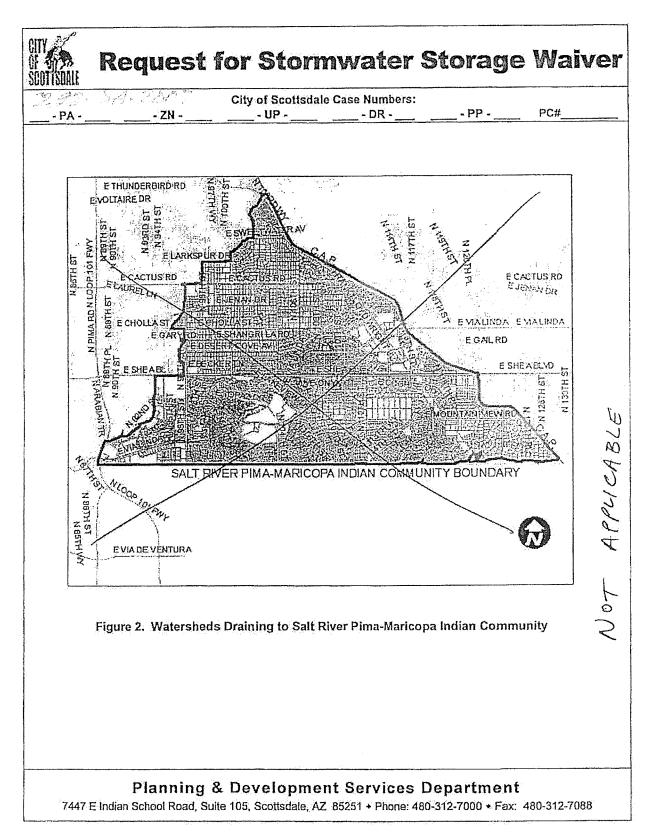
APPENDIX B

STORMWATER STORAGE WAIVER / PROPOSED DRAINAGE IMPROVEMENTS EXHIBIT

TY DISSIALE	Request	for Storn	nwater (Storag	e Waiver
292.9 - PA -	5A - 2007 - ZN -	City of Scottsdal	e Case Numbers:	- PP -	PC#6332-6
The applican	l/developer must complete mprovement plans. Deni	and submit this form to the al of the waiver may require	city for processing and	obtain approval of it a revised site pla	waiver request before n to the Development
Date _7/14/08	Project N	ame Fairmont Scottsdale Prir	ncess Resort		
	tion 7575 East Princess Driv	e Scottsdele, AZ 85255	Company Name Wood	Ontol & Appoplates	
pplicant C	ontact John Bulka	Fax 480-834-3320	E-mail jbulka@wo	ndoalal com	
hone 480-8	5 N. Stapley Mesa, AZ 85203	Fax 400-004-0020	E-mail Journa Giro		
Idress 185	N. Supley Mesa, AZ 00200	and the second			
consider was supporting potential for 1. TI m fro	an intentional refinquishme living some or all required to engineering analyses that of flooding on any property. The runoff for the project ust demonstrate that the	nt of a claim or right. A pro- slormwater storage. Check lemonstrate the project me has been included in a e stormwater storage fa and that the runoff will	the applicable box and ets the criteria and that storage facility at ar acility was specifical	provide a signed e the effect of a waiv nother location. y designed to ac	ngineering report and ar will not increase the The applicant commodate runoff
2. Th ar su	e development is adja d constructed to handl bject property or to any	cent to a watercourse o e the additional runoff w / other property.	vithout increasing the	e potential for flo	od damage to the
□ 3. TI de	e development is on a monstrates there is no	parcel less than one-ha significant increase in J	alf acre in size in an potential for flood da	area where the mage due to its	engineering analysis development.
0	rdinance (ESLO). The the subject property or	irements conflict with re applicant must demons to any other property.	strate there is no incl Such conflicts with	reased potential ESLO may inclu	for flood damage de:
6 6 6	allowable footprint pe Topography prevents Creating a storage fa	nts for storage basin, ea er zoning. building storage basin cllity requires wash mo Zoning Administrator ca	dification.		
5. T C fc	ne project is located wil ouncil Resolution #623 r flood damage to any	hin the Downtown Fee 8 (see map). The appli property. Even if the pr ial for increased flood d	Reduction Area as a cant must demonstroject is located in th	tescribed and ap ate there is no in e Downtown are	proved by City creased potential a, if the project
С	ommunity (SRPMIC) (s	thin a watershed that dr ee map). The project r ate flows over and abov	nust provide the pre	 development per 	Maricopa Indian eak discharge flow to
attached	below, I certify that the locumentation. MmBulk oper or Engineer (circle one	stated project meets the	Λ	red above as dem 7-16-0 Date	
$-\theta$	Planning E Indian School Road,	& Developme			

FRINCESS





Revision Dolo: 18-Jul-07

CITY 27 OF SPOTIST	Reque	st for	Storm	nwater	Storage	Waiven
297 P	2+ <i>SA</i> . 2007 A−ZN-	City	of Scottsdale - UP	Case Numbers:	PP	PC#
				PLETE THIS PA		
Project	Name <u>FAIRMON</u>	7 Juin	BOALS PY	INCESS IZES	orzí	
Check	Appropriate Boxes:					
	Meets waiver criteria	(specify):]1 📮2	□3 □4	□5 □6	
Į	Recommend approve	a waiver.				
	Recommend <u>deny</u> w None of waiver cri Downstream cond Other: Explain:	teria met. itions prohibi				
	Return waiver reque Insufficient data p Other: Explain:	rovided.				
	ommended Conditions All storage requirem Pre development con Other: Dain: <u>In kmd m</u>	ents waived. Iditions must			ĺ <u>ee</u>	
	Waiver <u>approved</u> p Waiver <u>denied</u> .	er above co	nditions.			
	C. Ashle	up lauch		i	10/23/08	
	Floodplain Administrator o	r Designee			Dale	
		ng & De	velopmer Scottsdale, AZ	nt Services 85251 • Phone:	Department 480-312-7000 • Fax:	480-312-7088

Request for Stormwater Storage Walver

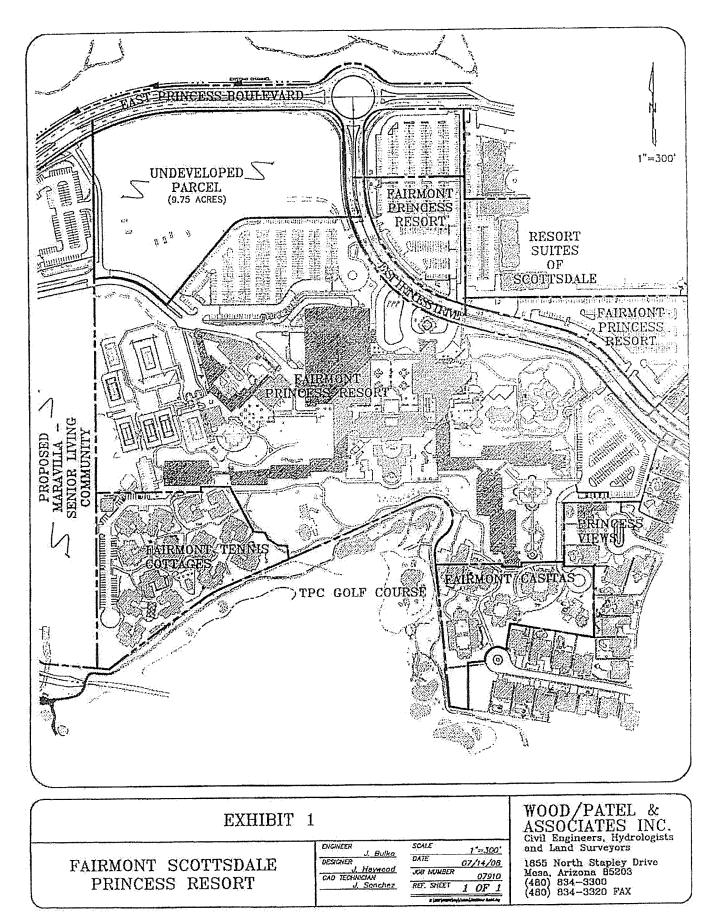
Page 4 of 5

Revision Date: 18-Jul-07

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	for Stor	mwater	Storage	Waiver
292. SA. 2007		dale Case Numbers:	~~~	PC#
PA ZN	UP	DR	PP	PC#
	In-Lieu Fee and In	n-Kind Contribution	S	
If the city grants a waiver, the d it would cost the city to provide construction, landscaping, desig For FY 2007/2008, this cost is \$ annually, but the city reserves the	the waived storage gn, construction ma 53.22 per cubic foot	volume, including co nagement, and main of stormwater stored	sts such as land ac tenance over a 75- I. This unit cost wil	equisition, year design life. I be updated
The Floodplain Administrator co contribution can serve as part o designee must approve in-lieu f	f or instead of the c	alculated In-lieu fee.	by-case basis. An The Floodplain Ad	In-kind Iministrator or
Project Name Fairmont	Scottsdale	Princess Re	sort	
The waived stormwater storage	volume is calculate	ed as follows:		
V =CRA; where V =stormwater storage volume C =weighted average runolf co R =100-year/2-hour precipitatio A =area of disturbed ground, in	efficient over distur in depth, in feet (2.8	bed area,	et, for all regions o	f Scottsdale), and
Furthermore,		×		
$V_w = V - V_p$; where $V_w = volume waived$, V = volume required, and $V_p = volume provided$	C = A = $V_{p} = $ $V_{w} = $	0,9 424,753 89,826 		
An In-Lieu Fee will be paid, In-lieu fee (\$) = V _w (cu. ft.) 5	based on the follow \$3.22 per cubic fo	wing calculations and ot = $269,290$	supporting docum	entation:
An In-Kind Contribution will <u>See attachment</u> . with approved plan	Papiess Prive	s: <u>Bridek</u> Reconstru	nim, in quaryolin	<u> </u>
No In-Lieu Fee is required.	Reason:	an a sa an		
Approved by: C. And Floodplain Administrator or Designee	eglarh		ル 23 Date	08
r complete internation of coolditor				
Planning 7447 E Indian School Road, 3		AZ 85251 • Phone: 4	· · · · · · · · · · · · · · · · · · ·	

Request for Stormwater Storage Waiver



October 23, 2008 WP# 072910 Sheet 1 of 2

Attachment to Stormwater Storage Waiver Request for Fairmont Scottsdale Princess Resort & Regional Flood Control

The Fairmont Scottsdale Princess Resort (Site) is a 60 acre resort located near the southwest corner of Princess Boulevard and Princess Drive. The Site is bounded by the Princess Blvd, to the north, the Maravilla Scottsdale Senior Living Community to the east, the TPC Golf Course to the south and existing residential developments to the west (see Exhibit 1, attached). The existing Fairmont Scottsdale Princess Resort consists of multiple hotel buildings, a ballroom, spa, tennis cottages, tennis courts, and parking. A majority of the site is developed and portions are being updated and renovated. At the north end of the site there is a 9.75 acre portion of the property that has yet to be developed, and other portions are scheduled for upgrades.

It is Wood/Patel's understanding that the ownership of the Fairmont Scottsdale Princess Resort, Strategic Hotels and Resorts, has agreed to fund regional flood control improvements to the public road/channel crossing at Princess Blvd and Scottsdale Road, in return for the City approving this waiver and it being applicable to the entire site. The improvements consist of removing the existing concrete box culvert crossing and replacing it with a bridge structure. The cost of a new bridge structure is estimated at \$1,053,000.

City of Scottsdale In-Lieu Fees: V(req) Volume required = CRA = (0.90) x (0.235 feet) x (9.75 acres) = 89,826 cu-ft. C (Runoff Coefficient) = 0.90 R (100-year/2-hour precipitation depth) = 0.235 feet Site area = 9.75 acres

City of Scottsdale In-Lieu Fees = $V(req) \times 3.22 = (89,826 \text{ cu-ft}) \times 3.22 = 289,240$

Summary: Public Drainage Improvements = \$1,053,000 (*) City of Scottsdale in Lieu Fee = \$289,240

(*) See Sheet 2 of 2 Engineering Preliminary Opinion of Probable Cost

Attachment to Stormwater Storage Waiver Request

October 23, 2008 WP# 072910 Sheet 2 of 2

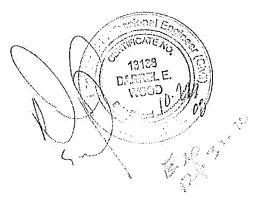
Attachment to Stormwater Storage Waiver Request for Fairmont Scottsdale Princess Resort & Regional Flood Control

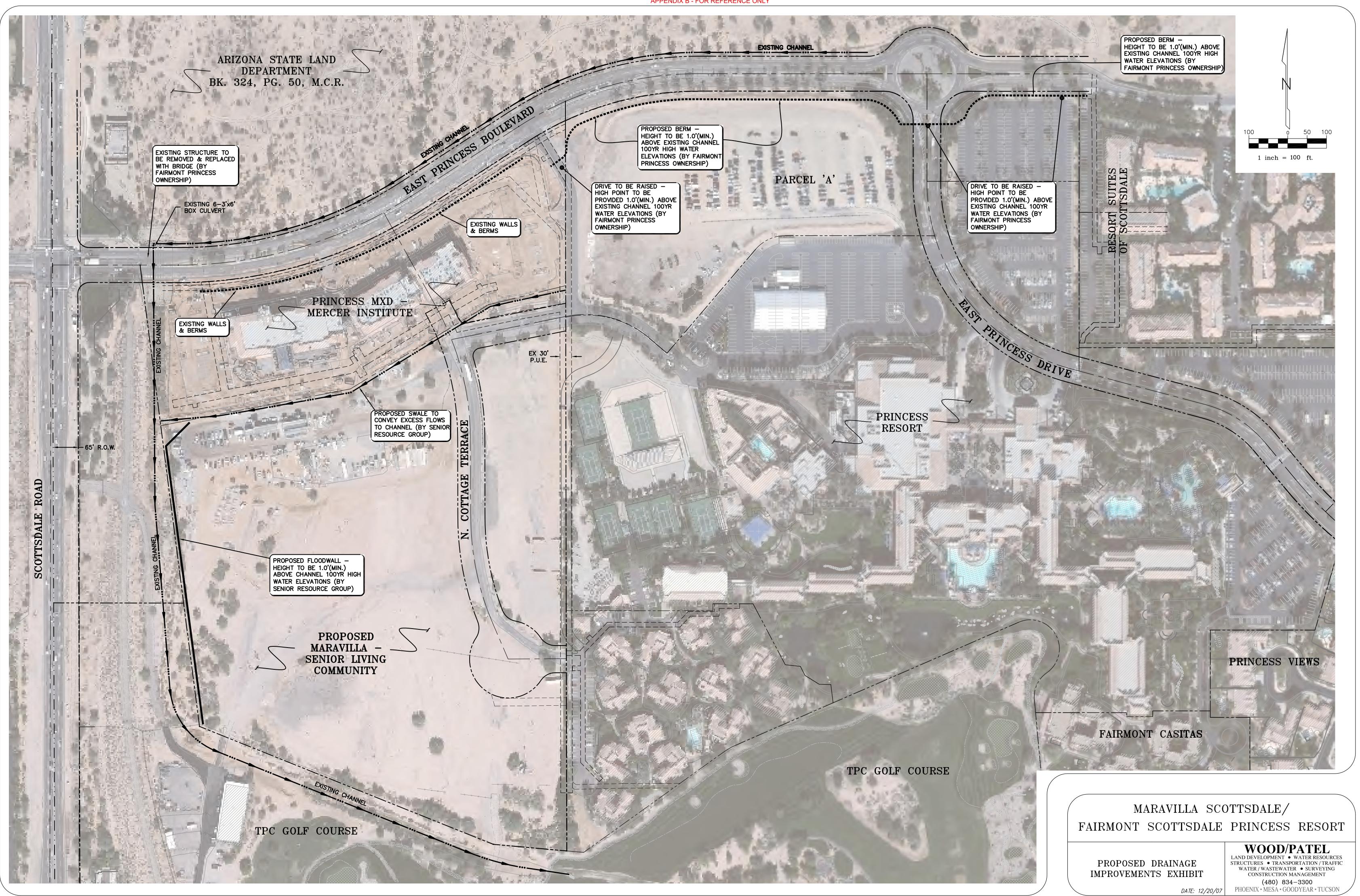
Engineering Preliminary Opinion of Probable Cost (*)

Prepared Bridge Structure at Princess Drive, just east of Scottsdale Road serving unnamed wash.

Estimated Bridge Surface = 8,100 square feet x \$130/s.f. \$1,053,000

(*) Offered without the benefit of construction documents and specifications.





N: \2007\073091\dwg\Exhibits\3091EX01-AERIAL.dwg

APPENDIX C

CITY OF SCOTTSDALE FORMS



Appendix 4-1C WARNING & DISCLAIMER OF LIABILITY

The Drainage and Floodplain Regulations and Ordinances of the City of Scottsdale are intended to "minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding caused by the surface runoff of rainfall" (Scottsdale Revised Code §37-16).

As defined in S.R.C. §37-17, a flood plain or "*Special flood hazard* area means an area having flood and/or flood related erosion hazards as shown on a FHBM or FIRM as zone A, AO, A1-30, AE, A99, AH, or E, and those areas identified as such by the floodplain administrator, delineated in accordance with subsection 37-18(b) and adopted by the floodplain board." It is possible that a property could be inundated by greater frequency flood events or by a flood greater in magnitude than a 100-year flood. Additionally, much of the Scottsdale area is a dynamic flood area; that is, the floodplains may shift from one location to another, over time, due to natural processes.

WARNING AND DISCLAIMER OF LIABILITY PURSUANT TO S.R.C §37-22

"The degree of flood protection provided by the requirements in this article is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by manmade or natural causes. This article (Chapter 37, Article II) shall not create liability on the part of the city, any officer or employee thereof, or the federal government for any flood damages that result from reliance on this article or any administrative decision lawfully made thereunder."

Compliance with Drainage and Floodplain Regulations and Ordinances does not insure complete protection from flooding. The Floodplain Regulations and Ordinances meet established local and federal standards for floodplain management, but neither this review nor the Regulations and Ordinances take into account such flood related problems as natural erosion, streambed meander or man-made obstructions and diversions, all of which may have an adverse affect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations.

I have read and understand the above. If I am an agent for an owner I have made the owner aware of and explained this disclaimer.

216/2015 Date Owner or Agent

Plan Check No.



Section 404 Certification

Before the City issues development permits for a project, the developer's Engineer or the property owner must certify that it complies with, or is exempt from, Section 404 of the Clean Water Act of the United States. Section 404, administered by the U.S. Army Corps of Engineers (COE), regulates the discharge of dredged or fill material into a wetland, lake, (including dry lakes), river, stream (including intermittent streams, ephemeral washes, and arroyos), or other waters of the United States.

Prior to submittal of improvement plans to Project Review the form below must be completed (and submitted with the improvement plans) as evidence of compliance

Certification of	Section 404 Perm	nit Status
-------------------------	------------------	------------

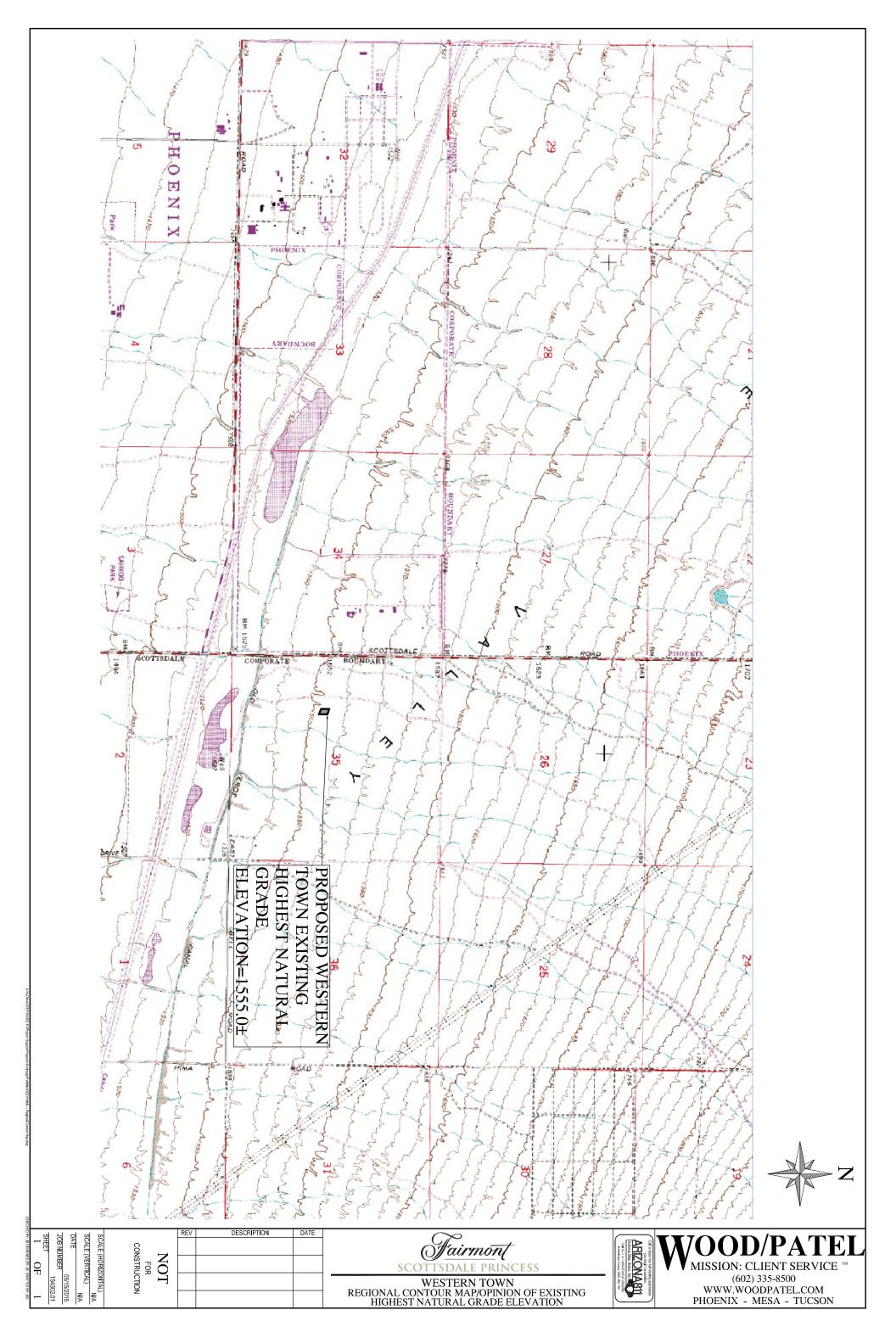
0	an'a Ni		Strategic Hotels and	Resorts	Phone No	312-658-6016
			Fairmont Scottsdale Hotel Exp.			
Proje	ect Nar	ne/Description:	Faimoni Scousu		1	
Proje	ect Loc	ation/Address: _	7575	E. Princess Blvd.	., Scottsdale, Az. 8	5255
	gister w that		the property Owner m	ust check the appli	cable condition and	certify by signing
1.		ion 404 <u>does</u> a _l rs of the U.S., a	pply to the project beca and:	ause there will be a	discharge of dredge	ed or fill material to
		A Section 404	Permit has already beer	n obtained for this pro	oject.	
		-or-				
			alifies for a "Nationwide nationwide permit.	Permit," and this pro	oject will meet all term	s and conditions of
2. S	Sectior	404 <u>does not</u> a	apply to the project be	cause:		
	\mathbf{X}	No watercours	es or other waters of the	e U.S. exist on the pr	operty.	
		No jurisdiction Jurisdictional [al waters of the U.S. exis	st on the property. A	ttached is a copy of th	ne COE's
		Watercourses discharge of d	or other waters of the U redged or fill material int	S. do exist on the properties of the second se	roperty, but the projec s.	t will not involve the
I ce	rtify th	at the above st	atement is true.	Destinational Contraction		1 1 -
	A	In Bu		33359 ·		6-15
Eng	1 1	1 0	Seal, or Owner's Signatu	BULKA S	Date	
Title	e Comp	pany Wood	K O I A	PINAU S	sulfa-	
		Plar	nning & Develo		ices Departn	nent

7447 E Indian School Road, Suite 100, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7088

APPENDIX D

OFFSITE WATERSHED EXHIBITS

Regional Contour Map / Opinion of Existing Highest Natural Grade Elevation



Aerial Map



Plate 1 Exhibit

(From Core North/Core South Drainage Study)

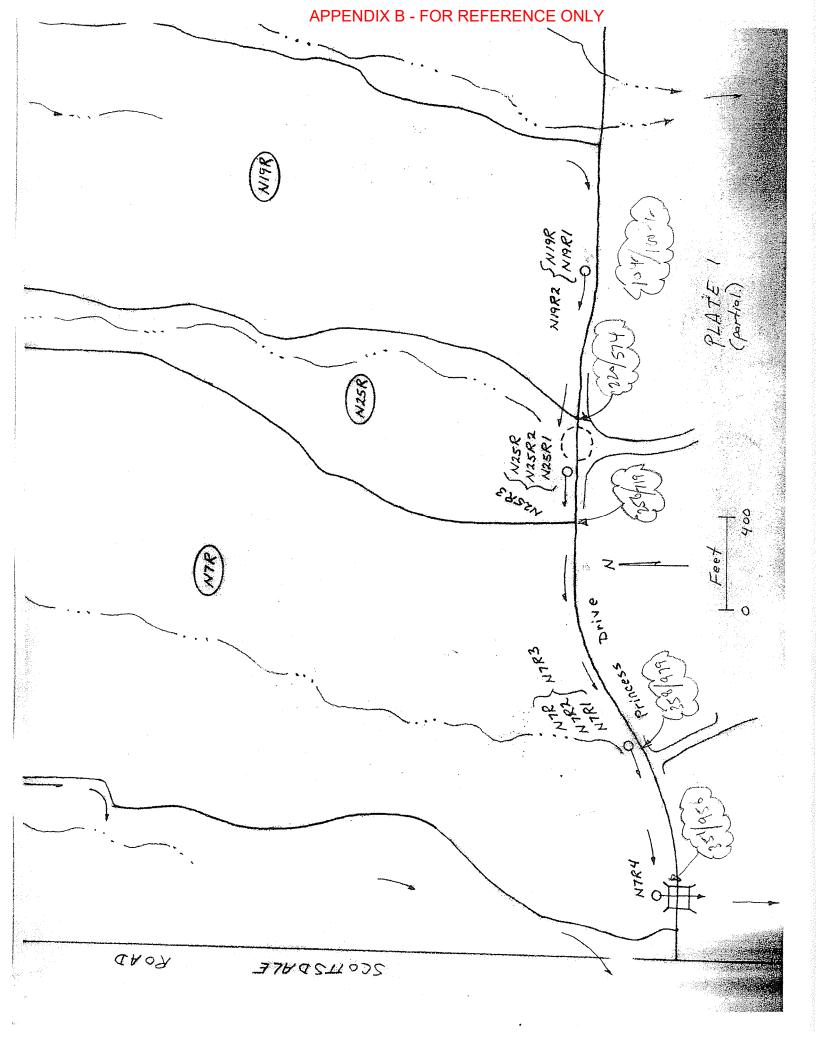


Table 1 Spreadsheet(From Core North/Core South Drainage Study)

Table 1 Peak Discharge Summary Princess Drive Channel Fairmont Scottsdale Princess Resort Scottsdale, Arizona											
		Peak Discharge (cfs) By Storm Return Interval (6-Hour Duration)									
Location	HEC-1 CP	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr				
750-Ft East of Roundabout	N19R2	60	146	220	342	444	574				
At Roundabout	N25R3 N7R3	62	167	256	417	549	719				
700-Ft Upstream of Culvert		90	228	358	583	787	979				
At Princess Drive Culvert	N7R4	89	227	351	580	779	956				
HEC-1 File: PRN2 PRN5 PRN10 PRN25 PRN50 PRN100											
Note: See Plate 1 for HEC-1 concentration point locations. 5/31/2007											

4

EXHIBIT 1

VICINITY MAP

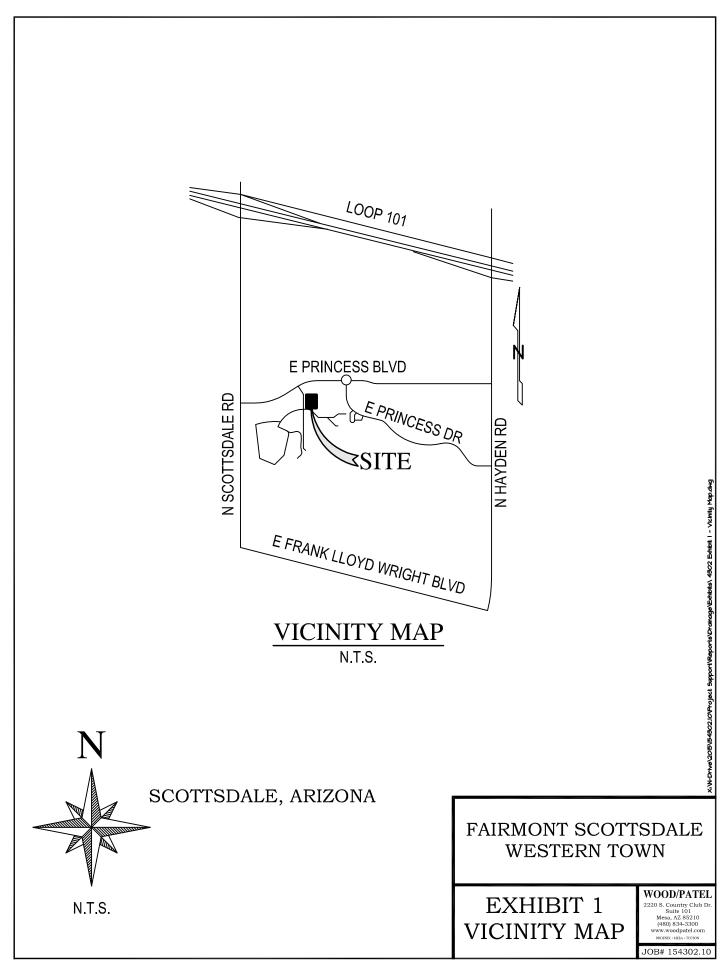


EXHIBIT 2

FEMA MAP

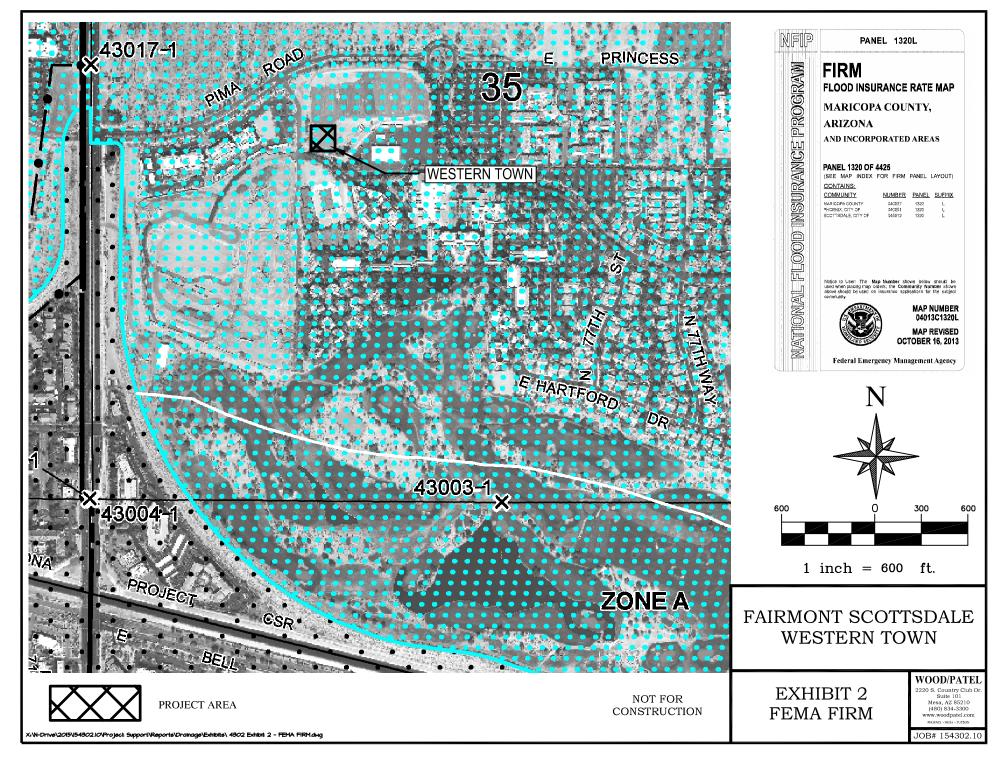


EXHIBIT 3

EXISTING DRAINAGE MAP

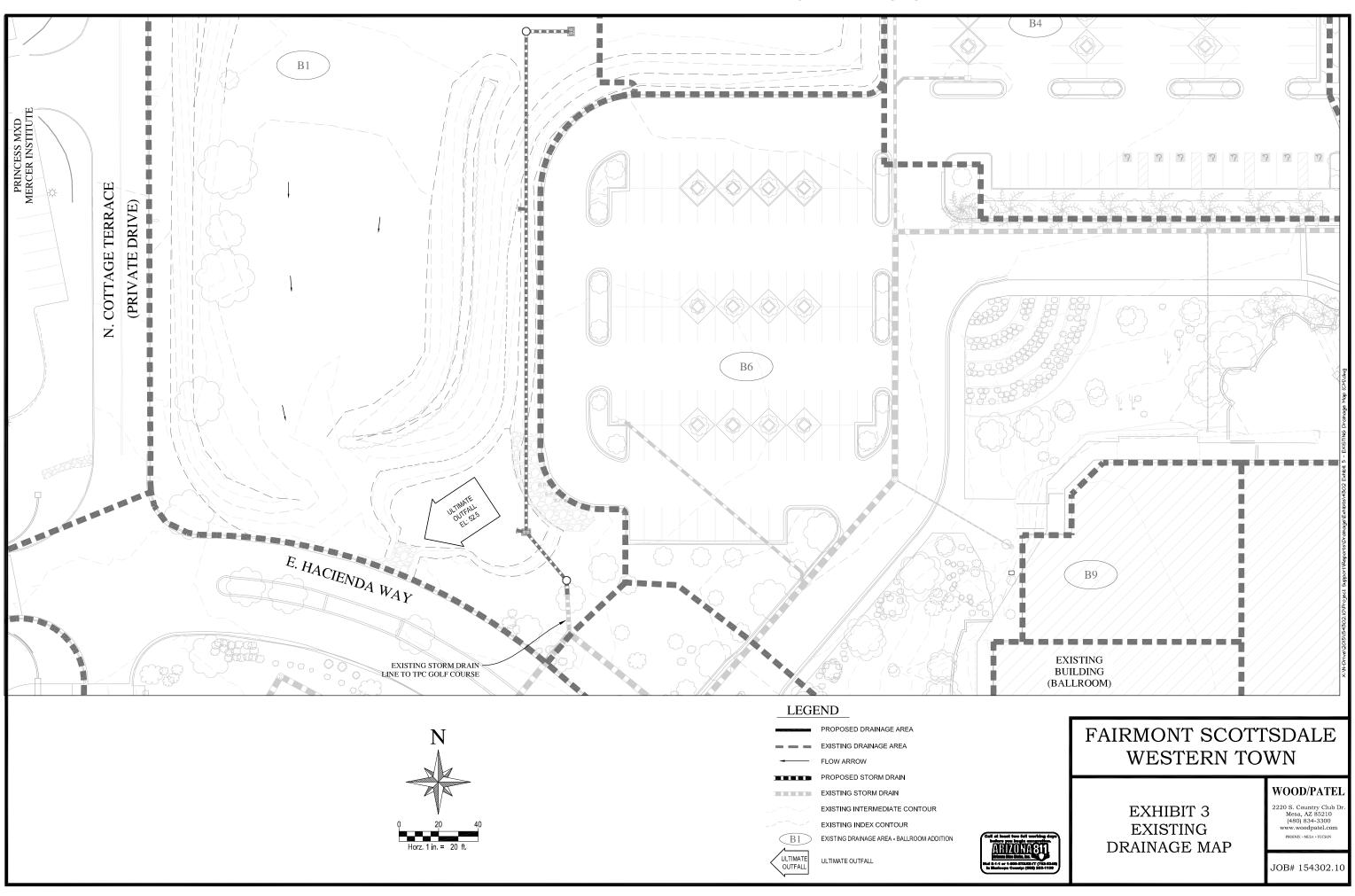
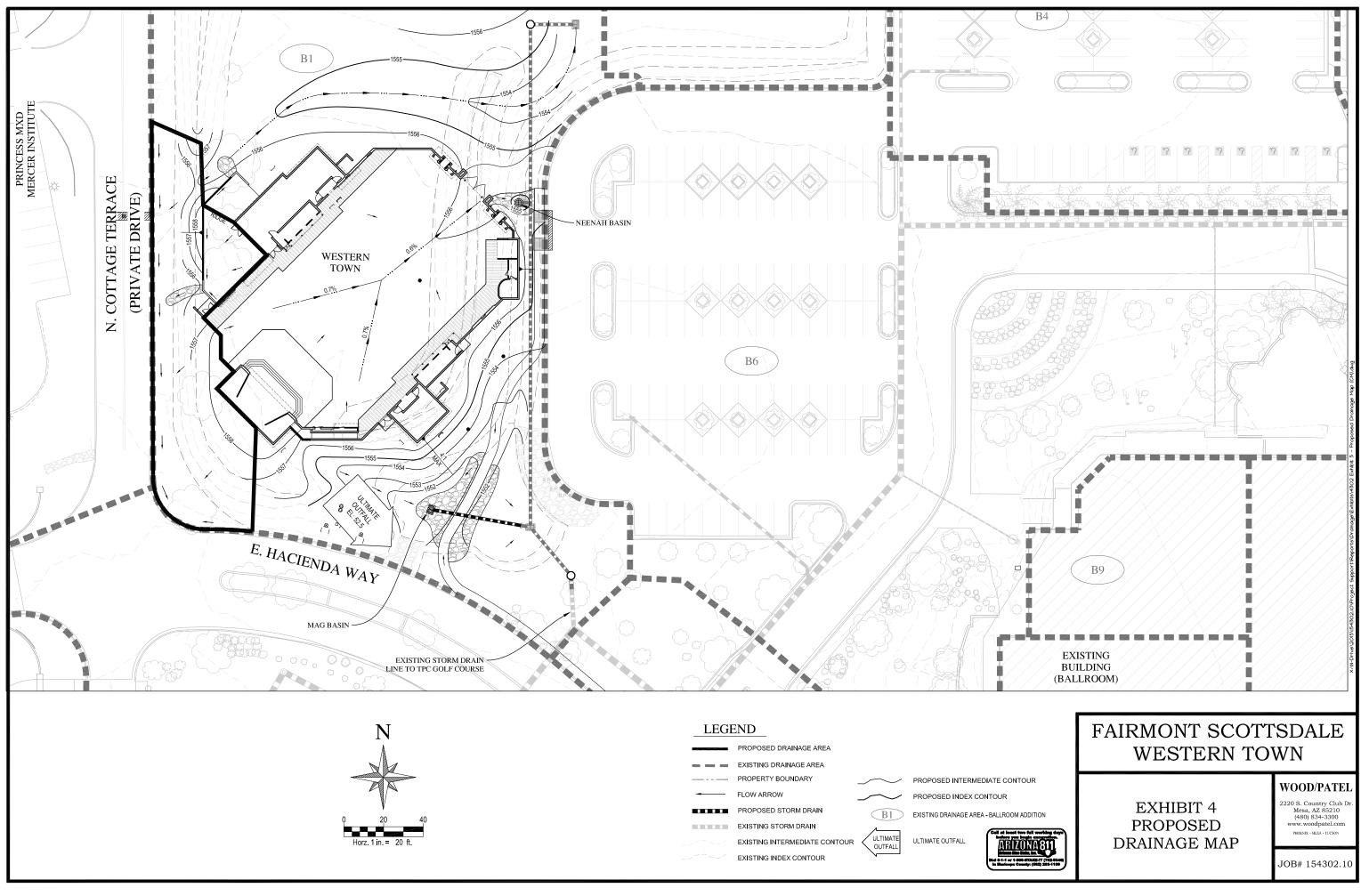


EXHIBIT 4

PROPOSED DRAINAGE MAP



APPENDIX C – DRAINAGE MEMO FOR FAIRMONT SCOTTSDALE SUNSET BEACH POOL BY WOOD, PATEL & ASSOCIATES, INC., DATED SEPTEMBER 11, 2015

DRAINAGE MEMO

FOR

FAIRMONT SCOTTSDALE

SUNSET BEACH POOL

September 11, 2015 WP# 154302.30

Prepared For:

Strategic Hotels and Resorts

Mr. George Stowers 200 West Madison Suite 1700 Chicago, Illinois 60606 *Phone: (312) 658-6016*

Submitted To:

City of Scottsdale 9388 East San Salvador Drive Scottsdale, Arizona 85258 *Phone: (480) 312-5636*

Prepared By:

Wood, Patel & Associates, Inc. 2220 South Country Club Drive Suite 101 Mesa, Arizona 85210 Phone: (480) 834-3300 Website: <u>www.woodpatel.com</u>





CIVIL ENGINEERS • HYDROLOGISTS • LAND SURVEYORS • CONSTRUCTION MANAGERS

Darrel E. Wood, P.E., R.LS. Ashok C. Patel, P.E., R.L.S., CFM Michael T. Young, P.E. James S. Campbell, P.E. Thomas R. Gettings, R.L.S. Darin L. Moore, P.E. Jeffrey R. Minch, P.E., CFM Robert D. Gofonia, P.E., R.L.S.

September 11, 2015

City of Scottsdale 9388 East San Salvador Drive Scottsdale, AZ 85258

(480) 312- 5636 dmann@scottsdaleaz.gov

Re: Fairmont Scottsdale Sunset Beach Pool Drainage Memo WP# 154302.30

To Whom It May Concern:

The proposed Fairmont Scottsdale Sunset Beach Pool (Pool) project is a 7,000 S.F. pool project with a sand/beach area, splash pad, hardscape and landscaping. The proposed development is located east of Scottsdale Road and south of Princess Drive along Cottage Terrace. More specifically, the site is located in the southwest quarter of Section 35, Township 4 North, Range 4 East of the Gila and Salt River Meridian. Refer to Exhibit 1 – *Vicinity Map* at the back of this report for the project location. The existing property, currently zoned C-2, includes a hotel, parking lot, multiple tennis courts, hardscape and desert landscaping.

This project occurs in the courtyard of the previously-approved Fairmont Scottsdale Hotel Expansion (Plan Check #1708-15-1). This memo will act as a supplement to the drainage report for that project. The items addressed herein are drainage-related items that have changed to accommodate this project. Refer to the *Final Drainage Report for the Fairmont Scottsdale Hotel Expansion*, dated May 1, 2015, for the drainage specifics related to that project.

Storm water flows from the Pool will ultimately discharge into an existing 42-inch storm drain line within Cottage Terrace through a proposed storm drain system. The existing 42-inch storm drain line discharges into the Tournament Players Club (TPC) Golf Course, and has been sized to convey the 10-year storm. The 100-year storm flows overland within Cottage Terrace, as designed to the TPC.

The design criteria used to estimate storm water flows and evaluate system hydraulics are based on Wood, Patel & Associates, Inc.'s (Wood/Patel) understanding of the requirements listed in the *City of Scottsdale Design Standards and Policies Manual, Chapter 4: Grading and Drainage, 2010.*

Existing drainage areas S1 through S3, S12, and E5A, from the Fairmont Scottsdale Hotel Expansion, have been further delineated to include S13 through S20 to accommodate the additional inlets required to accommodate this project. The Pool has been omitted from this study, as the 10-year storm is contained within the pool design. The 100-year storm is anticipated to flow overland to Cottage Terrace. Refer to the *Rational Method Summary* and Exhibit 2 - Proposed Drainage Map at the back of this report for more information.

Wood, Patel & Associates, Inc.	2220 South Country Club Drive, Suite 101	• Mesa, Arizona 85210 • (480) 834-3300 • Fax (602) 335-8580
		A.

September 11, 2015

City of Scottsdale **Fairmont Scottsdale Sunset Beach Pool** Drainage Memo WP# 154302.30

The proposed catch basins have been sized to convey the 100-year, 2-hour storm event with virtually no ponding. Refer to the *Inlet Capacity Summary* and the associated catch basin information at the back of this report for more information.

Thank you for your review of this Drainage Memo provided for the Fairmont Scottsdale Sunset Beach Pool. Please feel free to contact me if you have any questions.

Sincerely,

Wood, Patel & Associates, Inc.



John M. Bulka, P.E. Project Manager

JMB/km

X:\Y-Drive\WP\Reports\Commercial\154302.30 Fairmont Scottsdale Sunset Beach Pool Drainage Memo.docx

HYDRAULIC AND HYDROLOGIC CALCULATIONS

WOOD/PATEL

Rational Method Summary

 Description:
 Rational Method Inputs and Results

 Location:
 Fairmont Scottsdale Sunset Pool

 City of Scottsdale, Arizona

Drainage ID	Longest Watercours e (ft)	Longest Watercourse "L" (mi)	Drainage Area (s.f.)	Drainage Area "A" (acres)	Watershed Resistance Coefficient "K _b "	Top Elev. (ft)	Bottom Elev. (ft)	Basin Slope "S" (ft/mi)	Land Use (1)	Post Q100 "Tc" (min)	100 YR Intensity "i" (in/hr)	100YR Runoff Coefficient "C"	Post Q100 (cfs)	Post Q10 "Tc" (min)	10 YR Intensity "i" (in/hr)	10YR Runoff Coefficient "C"	Post Q10 (cfs)
Existing																	
E5A	277.4	0.053	12884	0.30	0.0433	52.3	51.5	15	25% Grass 37% Desert 38% Paved	6.4	6.80	0.60	1.2	7.9	3.91	0.53	0.6
Proposed																	
S1	323.2	0.061	14586	0.33	0.0430	57.0	46.0	180	90% Desert 10% Paved	2.9	8.63	0.50	1.4	3.5	5.36	0.42	0.7
S2	47.5	0.009	2354	0.05	0.0481	50.0	48.0	222	66% Desert 34% Paved	1.1	8.82	0.62	0.3	1.3	5.53	0.54	0.1
S3	92.2	0.017	6522	0.15	0.0451	52.5	50.6	109	90% Desert 10% Paved	1.8	8.89	0.50	0.7	2.2	5.58	0.42	0.4
S12	97.5	0.018	9636	0.22	0.0441	50.4	48.0	130	49% Grass 35% Desert 16% Paved	1.7	8.89	0.45	0.9	2.1	5.58	0.37	0.5
S13	43.2	0.008	2299	0.05	0.0481	50.0	48.0	244	15% Grass 50% Desert 35% Paved	1.0	8.89	0.60	0.3	1.2	5.58	0.51	0.1
S14	54.7	0.010	3874	0.09	0.0465	52.5	51.5	97	100% Paved	1.5	8.89	0.95	0.8	1.7	5.58	0.90	0.5
S15	53.0	0.010	2685	0.06	0.0476	52.5	51.5	100	100% Paved	1.5	8.89	0.95	0.5	1.8	5.58	0.90	0.3
S16	70.7	0.013	4194	0.10	0.0463	52.5	48.2	321	50% Grass 15% Desert 35% Paved	1.1	8.89	0.53	0.5	1.4	5.58	0.44	0.2
S17	111.5	0.021	6800	0.16	0.0450	52.5	51.5	47	100% Sand	2.6	8.89	0.25	0.4	3.1	5.58	0.15	0.1
S18	83.3	0.016	5982	0.14	0.0453	52.5	51.3	76	100% Paved	2.0	8.89	0.95	1.2	2.4	5.58	0.90	0.7
S19	128.1	0.024	5448	0.13	0.0455	61.2	59.8	58	100% Paved	2.6	8.89	0.95	1.1	3.1	5.58	0.90	0.7
S20	103.1	0.020	10101	0.23	0.0440	61.2	50.2	563	50% Desert 50% Paved	1.2	8.89	0.70	1.4	1.4	5.58	0.61	0.8

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

Inlet Capacity Summary

Description:Summary of Inlet Sizing CalculationsLocation:Fairmont Scottsdale Sunset Pool

Inlet ID	Contributing Drainage Area ID	Q10 (cfs)	Available Head (ft)	Inlet Type	Inlet Capacity (cfs)	10-Year Actual Ponding Depth (ft)
CB-S1	S1	0.7	0.5	MAG 535	9.02	0.09
CB-S2	S2	0.1	1.1	MAG 535	29.42	0.02
CB-S3	S3	0.4	0.7	15" Area Drain	1.70	0.11
CB-S12	S12	0.5	1.1	MAG 535	29.42	0.07
CB-S13	S13	0.1	1.1	MAG 535	29.42	0.02
CB-S14	S14	0.5	0.5	15" Area Drain	1.45	0.30
CB-S15	S15	0.3	0.5	15" Area Drain	1.45	0.08
CB-S16	S16	0.2	1.0	15" Area Drain	2.05	0.08
CB-S17	S17	0.1	1.0	15" Area Drain	2.05	0.04
CB-S19	S19	0.7	0.5	15" Area Drain	1.45	0.16
CB-S20	S20	0.8	1.0	15" Area Drain	2.05	0.17

WOOD/PATEL

CIVIL ENGINEERS * HYDROLOGISTS * LAND SURVEYORS * CONSTRUCTION MANAGERS

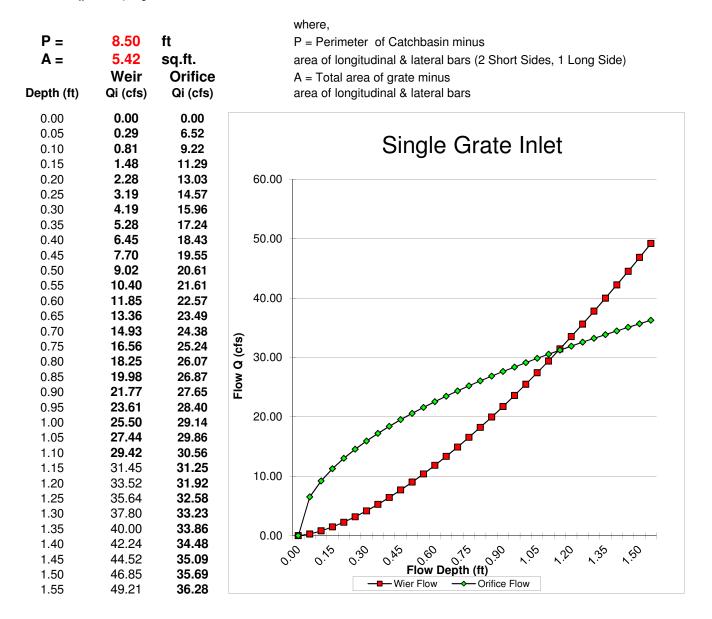
Inlet Capacity - Sump Locations

Description:	Calculation of Inlet Capacity for Single MAG 535 Catch Basin (w/o Curb)
Location:	Fairmont Scottsdale Sunset Pool
Reference:	Drainage Design Manual for Maricopa County, Vol. II, Hydraulics, pg. 3-27

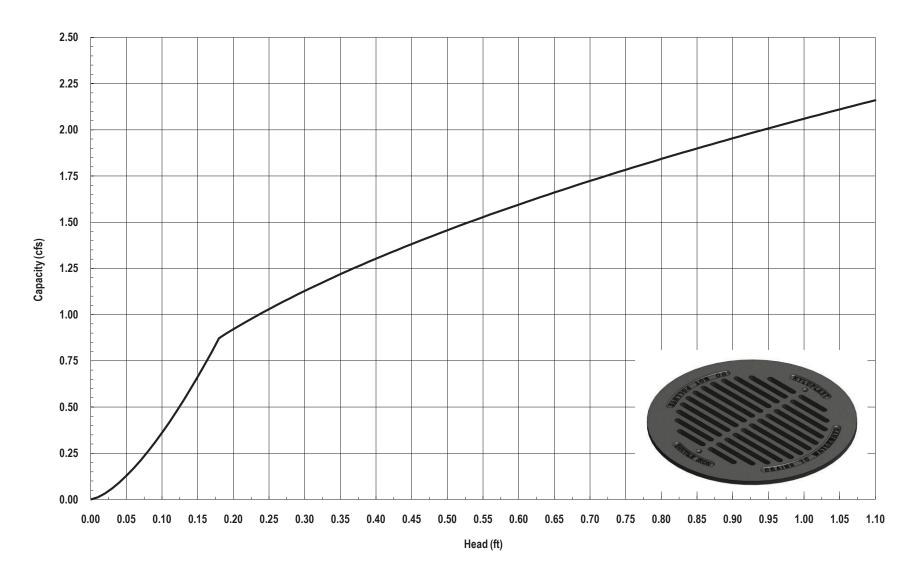
Weir EQ. $Q_i = C_w Pd^{1.5}$

Orifice EQ. $Q_i = C_o A(2gd)^{0.5}$

Where: $C_w = 3.0$, $C_o = 0.67$



Nyloplast 15" Drop In Grate Inlet Capacity Chart





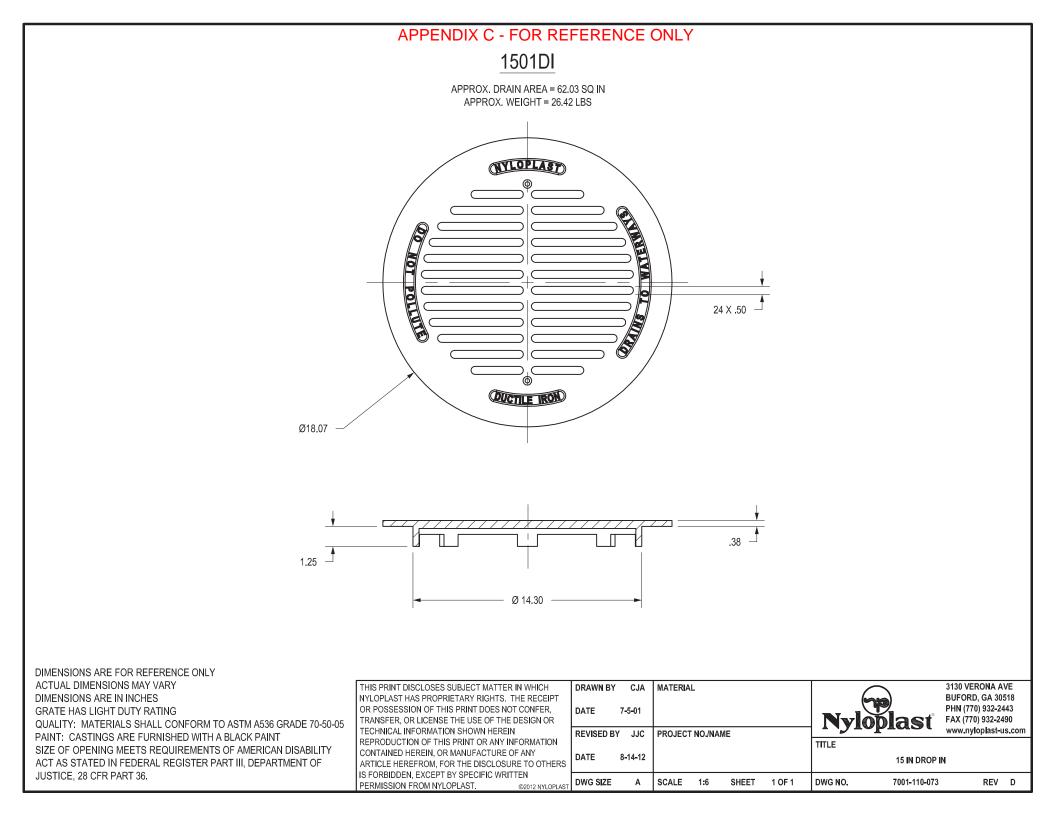


EXHIBIT 1

VICINITY MAP

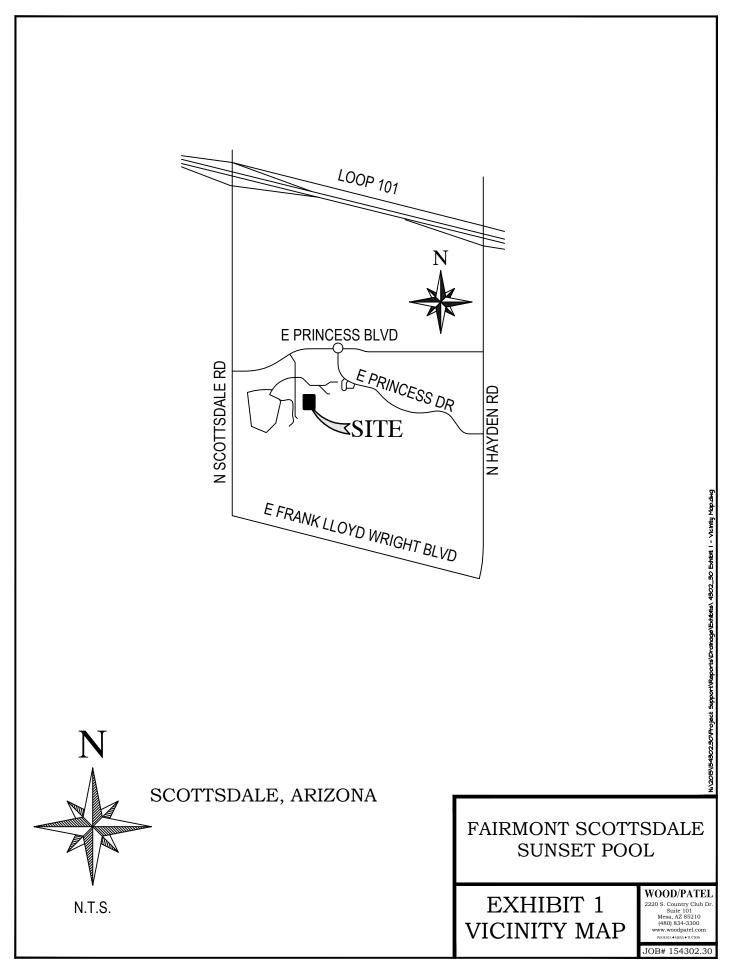
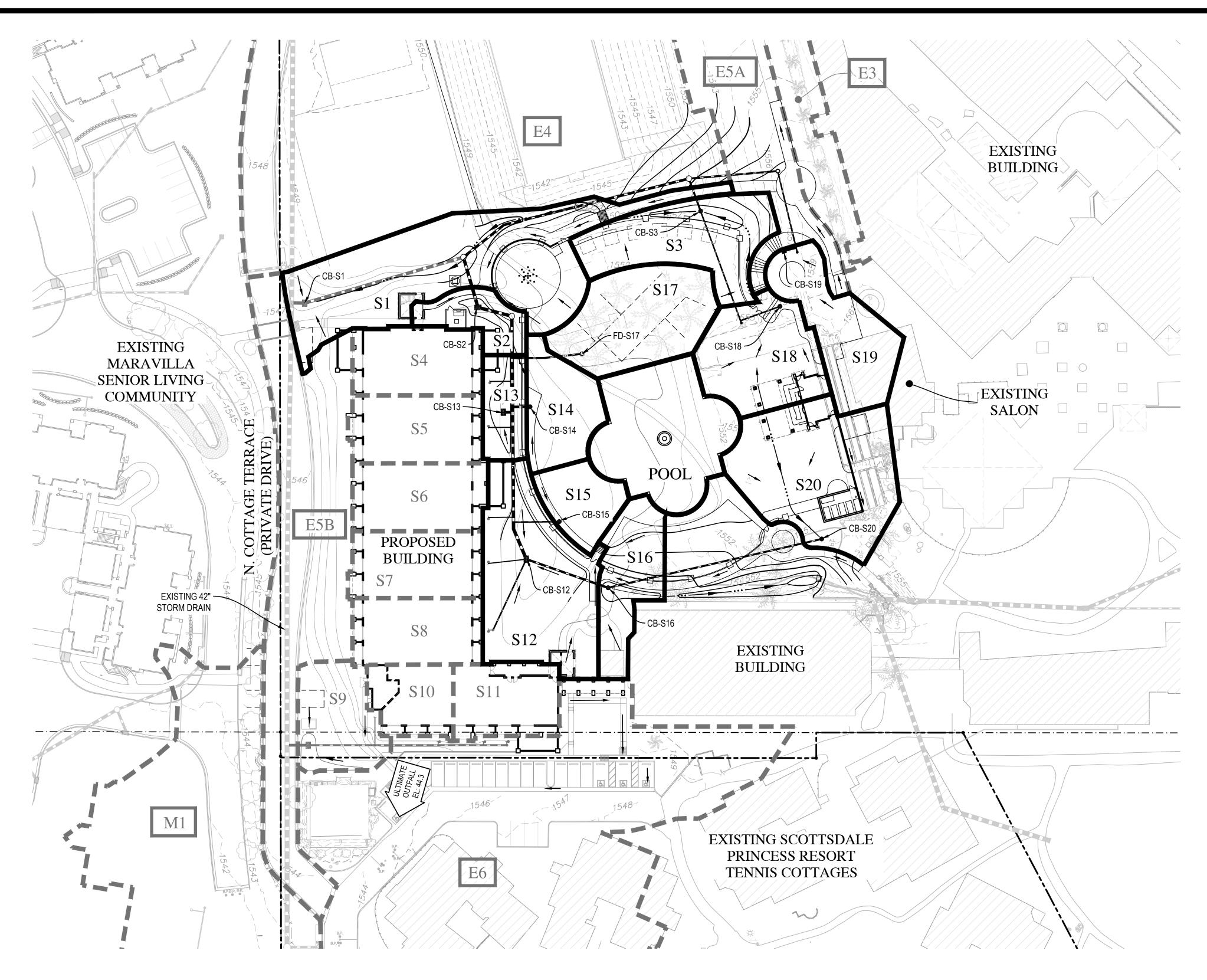


EXHIBIT 2

PROPOSED DRAINAGE MAP



LEGEND

	PROPOSED DRAINAGE AREA
	EXISTING DRAINAGE AREA
	PROPERTY BOUNDARY
	FLOW ARROW
	PROPOSED STORM DRAIN
	EXISTING STORM DRAIN
	EXISTING INTERMEDIATE CONTOUR
/^_/	EXISTING INDEX CONTOUR
	PROPOSED INTERMEDIATE CONTOUR
\sim	PROPOSED INDEX CONTOUR

S 1	PROPOSED DRAINAG
E1	EXISTING DRAINAGE
M 1	EXISTING DRAINAGE
ULTIMATE OUTFALL	ULTIMATE OUTFALL



EXISTING DRAINAGE AREA - MARAVILLA

EXISTING DRAINAGE AREA

PROPOSED DRAINAGE AREA

EXHIBIT 2 PROPOSED DRAINAGE MAP

FAIRMONT SCOTTSDALE

SUNSET POOL

N

Horz. 1 in. = 40 ft.

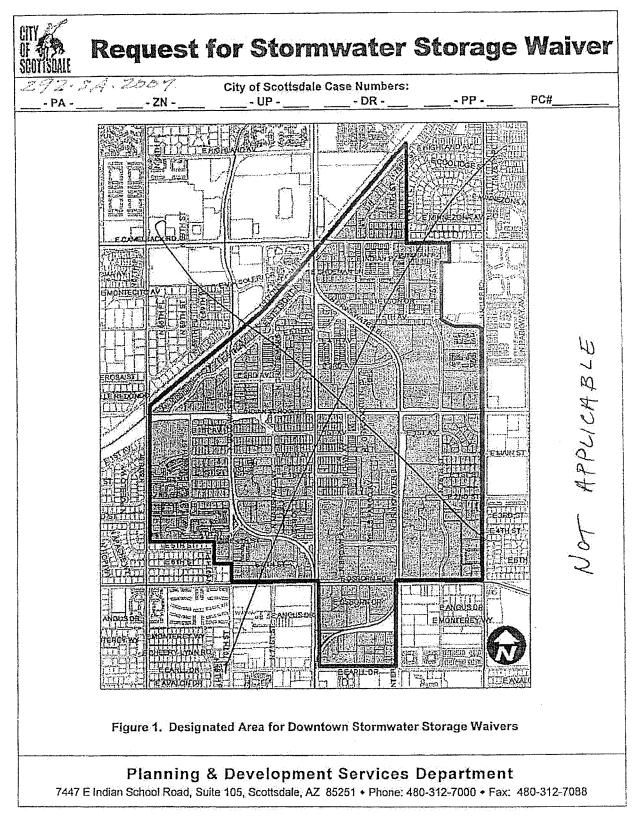
WOOD/PATEL 2220 S. Country Club Dr. Mesa, AZ 85210 (480) 834-3300 www.woodpatel.com

PHOENIX • MESA • TUCSON

JOB# 154302.30

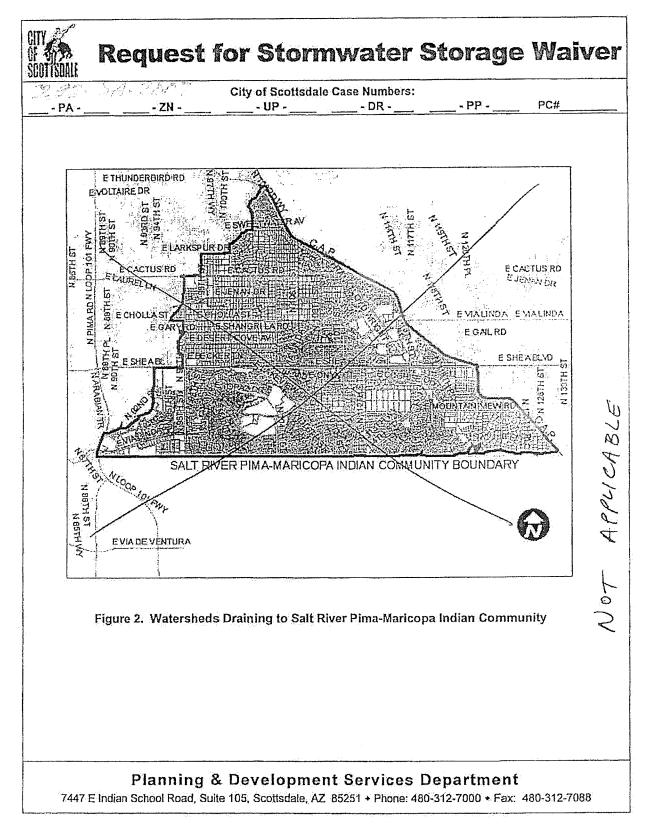
APPENDIX D – STORM STORAGE WAIVER / PROPOSED DRAINAGE IMPROVEMENTS EXHIBIT

Phone 480-834-3300 Fax 480-634-3320 E-mail jbulka@woodpatel.com deferss 1855 N. Stapley Mess, AZ 85203 Fax 480-634-3320 E-mail jbulka@woodpatel.com Valver Criteria A waiver is an intentional relinquishment of a claim or right. A project must meet at least one of six criteria listed below for the city to consider waiving some or all required stormwater storage. Check the applicable box and provide a signed engineering report and supporting engineering analyses that demonstrate the project meets the criteria and that the effect of a waiver will not increase the potential for flooding on any property. 1. The runoff for the project has been included in a storage facility at another location. The applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility. 2. The development is adjacent to a watercourse or channel that an engineering analysis shows is designed and constructed to handle the additional runoff without increasing the potential for flood damage to the subject property or to any other property. 3. The development is on a parcel less than one-half acre in size in an area where the engineering analysis demonstrates there is no significant increase in potential for flood damage due to its development. 4. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO). The applicant must demonstrate there is no increased potential for flood damage to the subject property or to any other property. Stormwater storage requirements for storage basin, easements, setbacks, and NAOS prevent building allowable footprint per zoning.
roject Location <u>7575 East Princess Drive Scottsdele, AZ 85255</u> , pplicant Contact John Bulka Company Name Wood Patel & Associates pplicant Contact John Bulka Company Name Wood Patel & Associates hong <u>480-834-3300</u> , Fax <u>480-834-3320</u> , E-mail Joulka@woodpatel.com ddress <u>1855 N. Stepley Mess</u> , AZ 85203 <i>Talver Criteria</i> A waiver is an intentional relinquishment of a claim or right. A project must meet at least one of stx criteria listed below for the city to consider waiving some or all required stormwater storage. Check the applicable box and provide a signed engineering report and supporting engineering analyses that demonstrate the project meets the criteria <i>and</i> that the effect of a waiver will not increase the potential for flooding on any property. 1. The runoff for the project has been included in a storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location. The applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff without increasing the potential for flood damage to the subject property or to any other property. 2. The development is adjacent to a watercourse or channel that an engineering analysis shows is designed and constructed to handle the additional runoff without increasing the potential for flood damage to the subject property or to any other property. 3. The development is on a parcel less than one-half acre in size in an area where the engineering analysis demonstrates there is no significant increase in potential for flood damage due to its development. 4. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO). The applicant must demonstrate there is no increased potential for flood damage to the subject property or to any other properfy. Such conflicts with ESLO may include: • Total land requirements for storage basin, easements, setback
upplicant Contact_John Bulka Company Name_Wood Patel & Associates ubnon 480-834-3300 Fax_480-834-3320 E-mail_bulka@woodpatel.com vidress 1855 N. Stepley Mesa, AZ 85203 E-mail_bulka@woodpatel.com Waiver Criteria A waiver is an intentional relinquishment of a claim or right. A project must meet at least one of six criteria listed below for the city to consider waiving some or all required stormwater storage. Check the applicable box and provide a signed engineering report and supporting analyses that demonstrate the project meets the criteria and that the effect of a waiver will not increase the potential for flooding on any property. 1 The runoff for the project has been included in a storage facility at another location. The applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility. 2. The development is adjacent to a watercourse or channel that an engineering analysis shows is designed and constructed to handle the additional runoff without increasing the potential for flood damage to the subject property or to any other property. 3. The development is on a parcel less than one-half acre in size in an area where the engineering analysis demonstrates there is no significant increase in potential for flood damage due to its development. 4. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO). The applicant must demonstrate there is no increased
Hone 480-834-3300 Fax 480-834-3320 E-mail jbulka@woodpatet.com Walver Criteria A waiver is an intentional relinquishment of a claim or right. A project must meet at least one of six criteria listed below for the city to consider waiving some or all required stormwater storage. Check the applicable box and provide a signed engineering report and supporting engineering analyses that demonstrate the project meets the criteria and that the effect of a waiver will not increase the potential for flooding on any property. 1. The runoff for the project has been included in a storage facility at another location. The applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility. 2. The development is adjacent to a watercourse or channel that an engineering analysis shows is designed constructed to handle the additional runoff without increasing the potential for flood damage to the subject property or to any other property. 3. The development is on a parcel less than one-half acre in size in an area where the engineering analysis demonstrates there is no significant increase in potential for flood damage due to its development. 4. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO). The applicant must demonstrate there is no increased potential for flood damage to the subject property or to any other property. Such conflicts with ESLO may include: Total land requirements for storage basin, easements, setbacks, and NAOS prevent building allowable footprint per zoning.
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 Topography prevents building storage basin. Creating a storage facility requires wash modification. Instances where the Zoning Administrator cannot allow a modification to ESL requirements.
5. The project is located within the Downtown Fee Reduction Area as described and approved by City Council Resolution #6238 (see map). The applicant must demonstrate there is no increased potential for flood damage to any property. Even if the project is located in the Downtown area, if the project creates additional potential for increased flood damage, the developer must provide alternative mitigation methods to prevent the damage.
6. The project is located within a watershed that drains directly to the Salt River Pima-Maricopa Indian Community (SRPMIC) (see map). The project must provide the pre-development peak discharge flow to the SRPMIC, and attenuate flows over and above pre-development.
By signing below, I certify that the stated project meets the waiver criteria selected above as demonstrated by the attached documentation. $\frac{1}{Developer or Engineer (circle one)} = \frac{1}{Date}$
Planning & Development Services Department



Request for Stormwater Storage Waiver

Revision Date: 18-Jul-07



Revision Data: 18-Jui-07

CITY OF SCOTISDA	Request	for Stor	mwater	Storage	Waiver
292 PA	- <i>S.A. 200</i> 7 ZN	City of Scotts: UP	tale Case Numbers: DR	PP	PC#
			OMPLETE THIS PA		
Project I	Name <u>FAIRMENT</u>	· · · · ·			
Check A	ppropriate Boxes:				
	Meets waiver criteria (spe	ecify): 🗆 1 🛛 🖵 2	3 4	□5 □6	
Ø	Recommend approve wa	iver.			
	Recommend <u>deny</u> waive None of waiver criteria Downstream condition Other: Explain:	i met. s prohibit waiver of			
	Return waiver request: Insufficient data provid Other: Explain:				
	mmended Conditions of M All storage requirements Pre development condition Other: ain: <u>In kmd Improv</u>	waived. ons must be mainta		lee	
	Waiver <u>approved</u> per al	bove conditions.			
	Waiver <u>denied</u> .	(m. O.		10172/05	
	Floodplain Administrator or Des			Date	
7,	Planning 147 E Indian School Road, S	& Developm Suite 105, Scottsdale	nent Services AZ 85251 • Phone:	Department 480-312-7000 • Fax:	480-312-7088

Request for Stormwater Storage Walver

Page 4 of 5

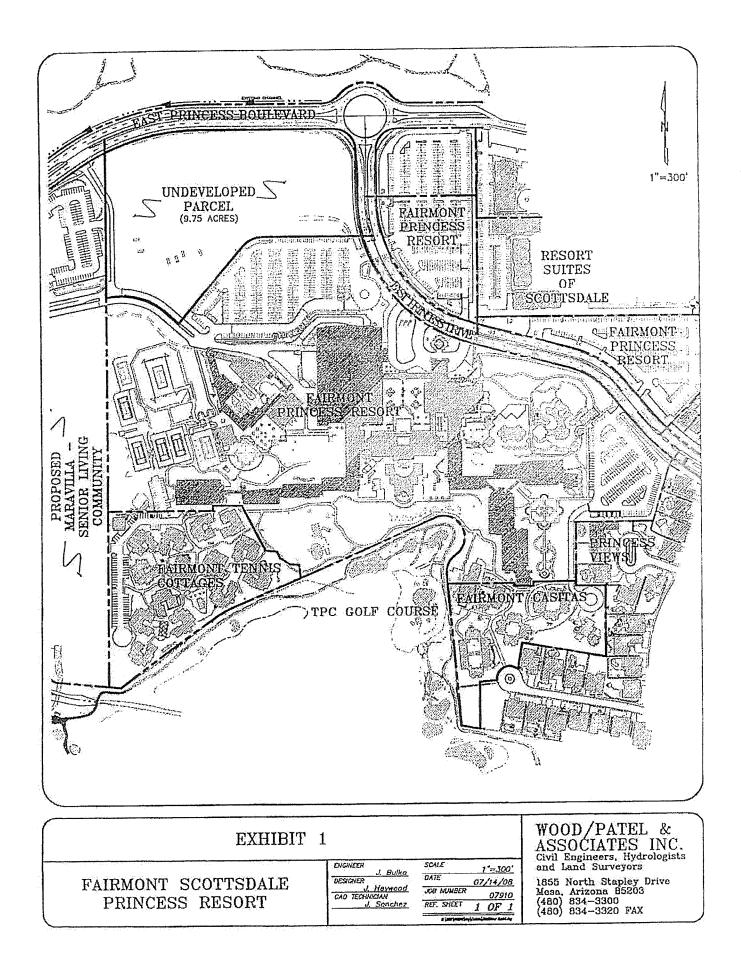
Revision Date: 18-Jul-07

CITY OF 55 55 SCOTISBALL	Request	for §	Storm	water	Storage	Waiver						
292.J	A · 2007 ZN	City of	f Scottsdale C UP -	ase Numbers: - DR -	- PP -	PC#						
= F.M.*												
	In-Lieu Fee and In-Kind Contributions If the city grants a waiver, the developer is required to calculate and contribute an In-Lieu F ee based on what											
it would c constructi For FY 20 annually,	ost the city to provide to on, landscaping, desig 107/2008, this cost is \$ but the city reserves th	the waived in, constru- 3.22 per ci he right to r	storage volun ction manager ubic foot of sto revise the unit	ne, including co ment, and main prowater stored cost at any tim	ests such as land ac tenance over a 75-y d. This unit cost will e at its sole discretion	ear design life. be updated on.						
contributio	Iplain Administrator co on can serve as part o must approve in-lieu f	f or instead	I of the calcula	ated In-lieu fee.	by-case basis. An li The Floodplain Adi	n-kind ninistrator or						
Project N	ame Fairmont	Scott	solute Pr	incess Re	sort							
The waive	ed stormwater storage	volume is	calculated as	follows:								
C =weigh R =100-y	where water storage volume ted average runolf con ear/2-hour precipitatio of disturbed ground, in	efficient ov n depth, in	er disturbed a feet (2,82 inc	rea, hes, or 0.235 fc	eet, for all regions of	Scottsdale), and						
V = volun			$C = 0, C$ $A = 424$ $V = 89, C$ $V_{\mu} = 0$ $V_{w} = 80, C$	1 ,753 826 1,826								
An In In-liet	Lieu Fee will be paid, ı fee (\$) = V _w (cu. ft.) >	based on 1 \$3.22 per	the following c cubic foot = _	alculations and	supporting docume	ntation:						
1_50	An In-Kind Contribution will be made, as follows: See attachment. Princess Drive. Bridge Reconstruction, in geocondence with approved plans.											
□ No In	No In-Lieu Fee is required. Reason:											
Approve	C. AMAR	iglarh			10/23/	2B						
Floodplain	Administrator or Designee	ът.			Dale							
7447	Planning E Indian School Road, S				Department 480-312-7000 • Fax:	480-312-7088						

.

Request for Stormwater Storage Waiver

Page 5 of 5



October 23, 2008 WP# 072910 Sheet 1 of 2

Attachment to Stormwater Storage Waiver Request for Fairmont Scottsdale Princess Resort & Regional Flood Control

The Fairmont Scottsdale Princess Resort (Site) is a 60 acre resort located near the southwest corner of Princess Boulevard and Princess Drive. The Site is bounded by the Princess Blvd, to the north, the Maravilla Scottsdale Senior Living Community to the east, the TPC Golf Course to the south and existing residential developments to the west (see Exhibit 1, attached). The existing Fairmont Scottsdale Princess Resort consists of multiple hotel buildings, a ballroom, spa, tennis cottages, tennis courts, and parking. A majority of the site is developed and portions are being updated and renovated. At the north end of the site there is a 9.75 acre portion of the property that has yet to be developed, and other portions are scheduled for upgrades.

It is Wood/Patel's understanding that the ownership of the Fairmont Scottsdale Princess Resort, Strategic Hotels and Resorts, has agreed to fund regional flood control improvements to the public road/channel crossing at Princess Blvd and Scottsdale Road, in return for the City approving this waiver and it being applicable to the entire site. The improvements consist of removing the existing concrete box culvert crossing and replacing it with a bridge structure. The cost of a new bridge structure is estimated at \$1,053,000.

City of Scottsdale In-Lieu Fees: V(req) Volume required = CRA = (0.90) x (0.235 feet) x (9.75 acres) = 89,826 cu-ft. C (Runoff Coefficient) = 0.90 R (100-year/2-hour precipitation depth) = 0.235 feet Site area = 9.75 acres

City of Scottsdale In-Lieu Fees = $V(req) \times 3.22 = (89,826 \text{ cu-ft}) \times 3.22 = 289,240$

Summary: Public Drainage Improvements = \$1,053,000 (*) City of Scottsdale in Lieu Fee = \$289,240

(*) See Sheet 2 of 2 Engineering Preliminary Opinion of Probable Cost

Attachment to Stormwater Storage Waiver Request

October 23, 2008 WP# 072910 Sheet 2 of 2

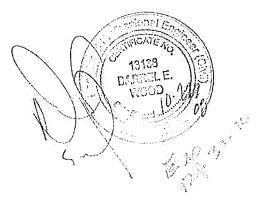
Attachment to Stormwater Storage Waiver Request for Fairmont Scottsdale Princess Resort & Regional Flood Control

Engineering Preliminary Opinion of Probable Cost (*)

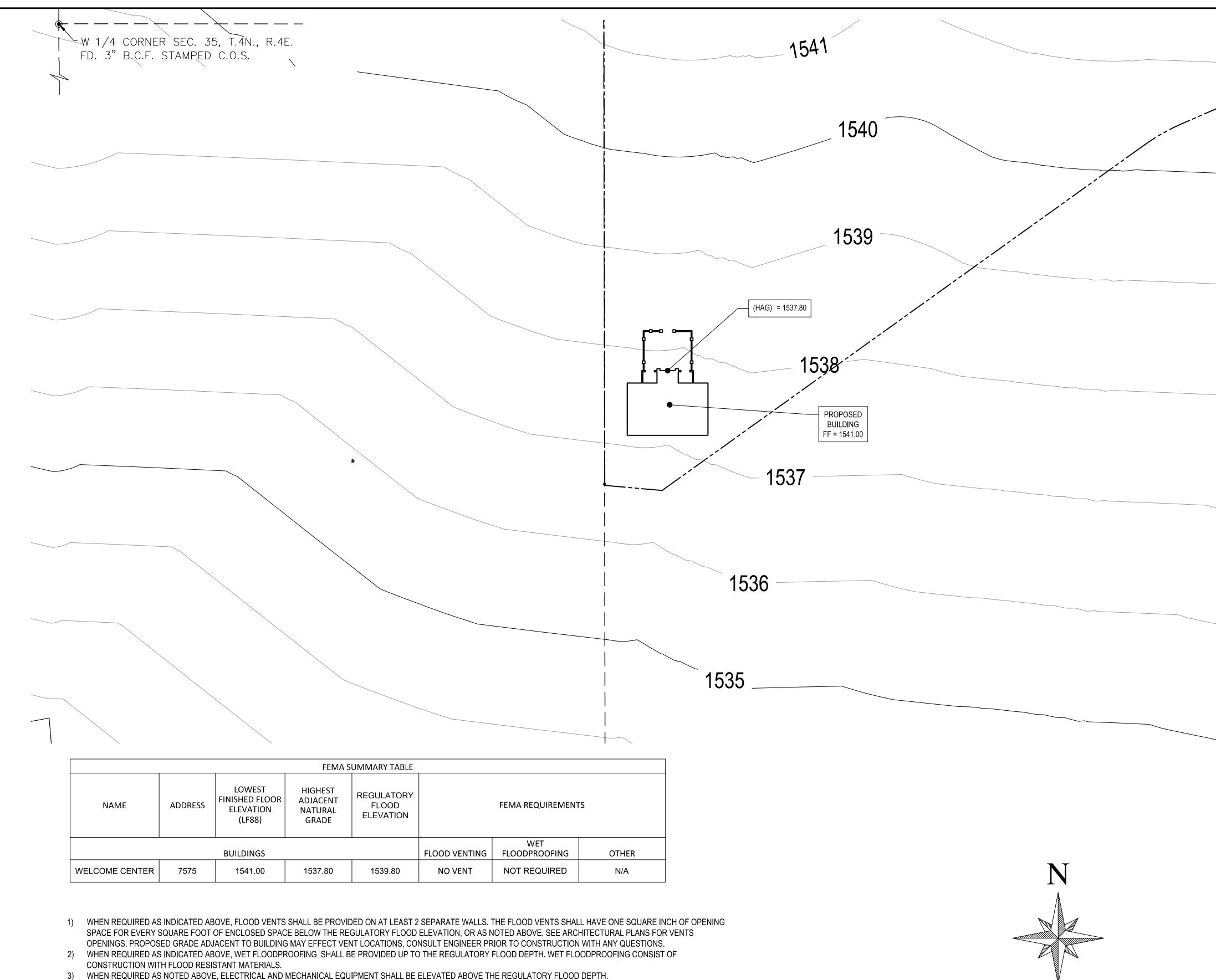
Prepared Bridge Structure at Princess Drive, just east of Scottsdale Road serving unnamed wash.

Estimated Bridge Surface = 8,100 square fect x \$130/s.f. \$1,053,000

(*) Offered without the benefit of construction documents and specifications.

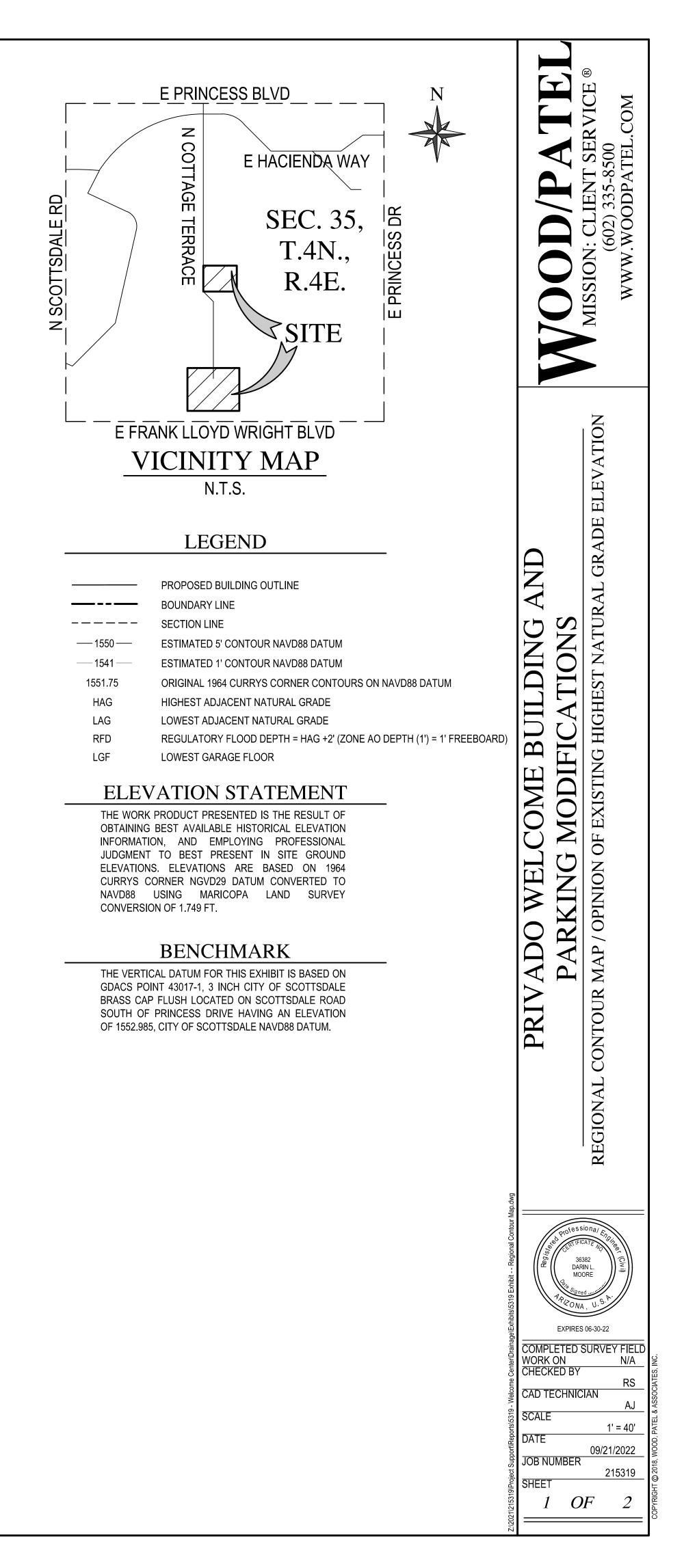


APPENDIX E – REGIONAL CONTOUR MAP / OPINION OF EXISTING HIGHEST NATURAL GRADE ELEVATION

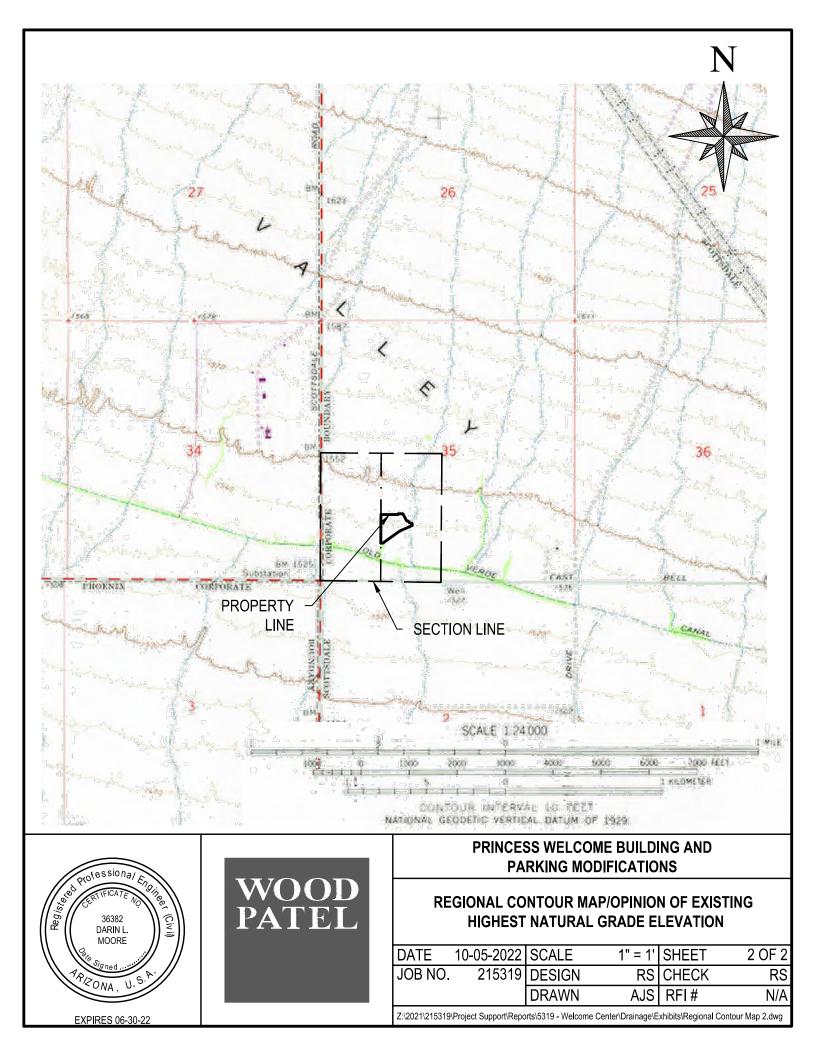


FEMA DEFINES DRY FLOODPROOFING AS A COMBINATION OF MEASURES THAT RESULT IN A STRUCTURE, INCLUDING THE ATTENDANT UTILITIES AND EQUIPMENT, BEING WATERTIGHT WITH ALL ELEMENTS SUBSTANTIALLY IMPERMEABLE TO THE ENTRANCE OF FLOODWATER AND WITH STRUCTURAL COMPONENTS HAVING THE CAPACITY TO RESIST FLOOD LOADS.

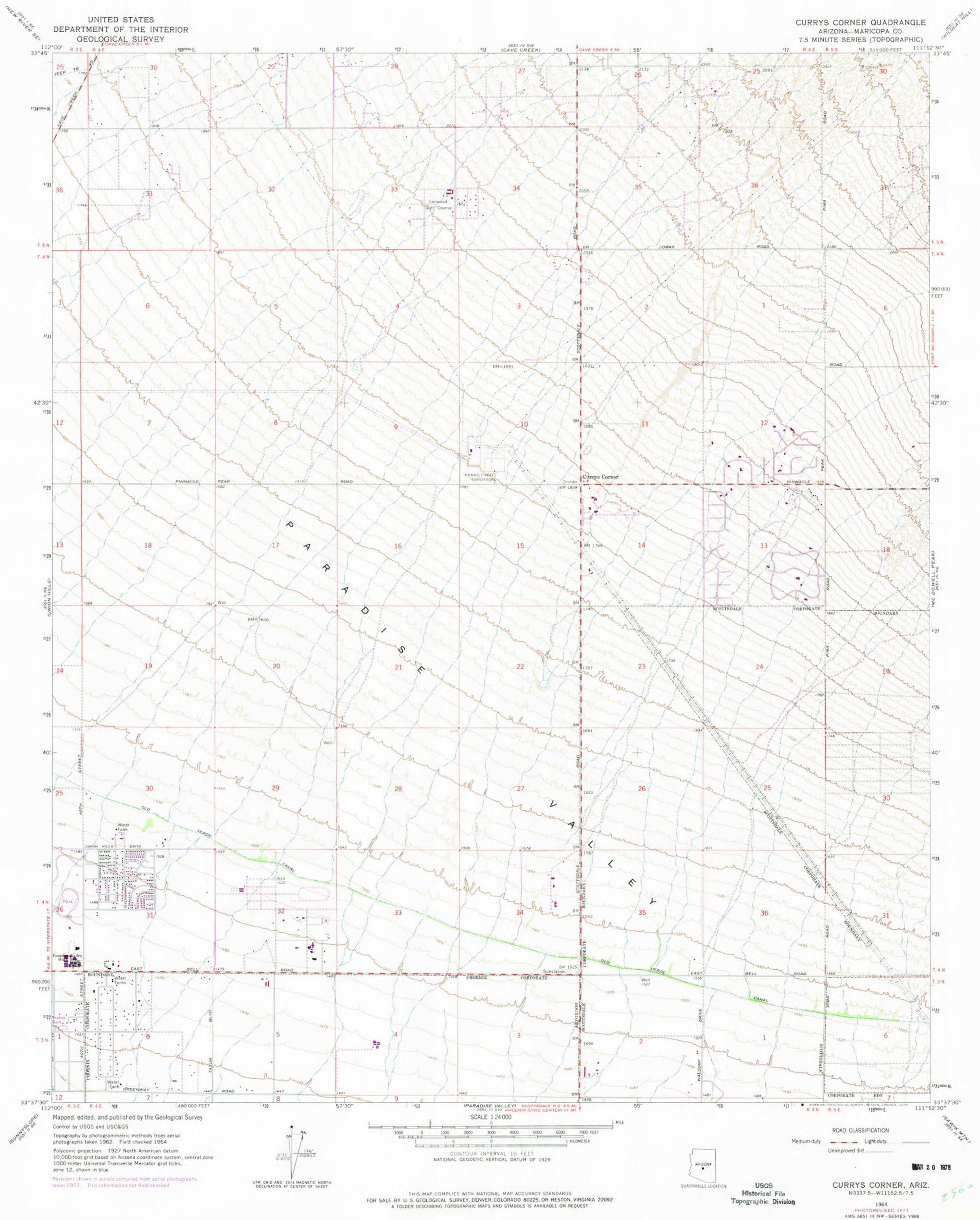
Horz 1 in = 40 ft



PROPOSED BUILDING M1 WILL BE A STRUCTURALLY INDEPENDENT NON-RESIDENTIAL STRUCTURE. 4)



APPENDIX F – CURRY'S CORNER QUADRANGLE MAP



APPENDIX G – HYDROLOGIC AND HYDRAULIC CALCULATIONS

IDF DATA FROM FCDMC NOAA – ATLAS 14 PRECIPITATION DATA



SITE I-D-F CURVE

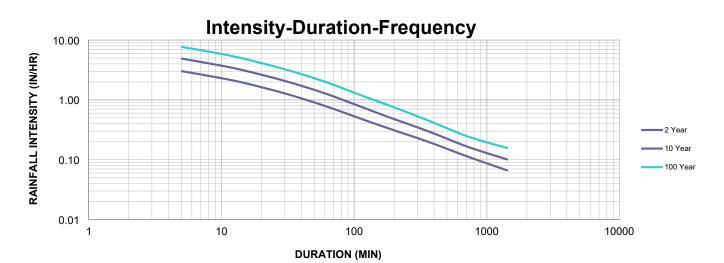
Project	Fairmont Scottsdale Princess Privado Welcome Center and Parking Expansion
Location	Scottsdale AZ
Project Number	215319
Project Engineer	Darin Moore, PE

RAINFALL DEPTHS, INCHES

Duration	Average Rec	Average Reccurence Interval (years)										
Duration	2	5	10	25	50	100						
5-min	0.253	0.341	0.409	0.501	0.571	0.643						
10-min	0.385	0.520	0.623	0.762	0.869	0.978						
15-min	0.478	0.644	0.772	0.945	1.080	1.210						
30-min	0.643	0.867	1.040	1.270	1.450	1.630						
60-min	0.796	1.070	1.290	1.580	1.800	2.020						
2-hr	0.921	1.230	1.460	1.780	2.020	2.270						
3-hr	1.010	1.320	1.560	1.910	2.180	2.460						
6-hr	1.200	1.530	1.800	2.150	2.440	2.730						
12-hr	1.350	1.700	1.970	2.350	2.630	2.930						
24-hr	1.580	2.040	2.410	2.920	3.320	3.740						

RAINFALL INTENSITY, INCHES/HOUR

Duration	Frequency, y	Frequency, years										
minutes	2	5	10	25	50	100						
5	3.04	4.09	4.91	6.01	6.85	7.72						
10	2.31	3.12	3.74	4.57	5.21	5.87						
15	1.91	2.58	3.09	3.78	4.32	4.84						
30	1.29	1.73	2.08	2.54	2.90	3.26						
60	0.80	1.07	1.29	1.58	1.80	2.02						
120	0.46	0.62	0.73	0.89	1.01	1.14						
180	0.34	0.44	0.52	0.64	0.73	0.82						
360	0.20	0.26	0.30	0.36	0.41	0.46						
720	0.11	0.14	0.16	0.20	0.22	0.24						
1440	0.07	0.09	0.10	0.12	0.14	0.16						



5319-Drainage Workbook.xls

TABLE 1 – EXISTING WEIGHTED C VALUES



COMPOSITE WEIGHTED "C" FACTOR CALCULATIONS 100 YEAR

ProjectFairmont Scottsdale Princess Privado Welcome Center and Parking ExpansionLocationScottsdale AZProject Number215319Project EngineerDarin Moore, PE

Existing C Factor

Drainage Subbasin ID	Area	Desert		Paved	Paved Roof Grass Commercial		ed Roof Grass Commercial		Grass			100 YR Runoff Coefficient
(Description/ID)	(Acres)	%	"C"	%	"C"	%	"C"	%	"C"	%	"C"	"C"
S9	0.10	90	0.45	10	0.95	0	0.95	0	0.30	0	0.86	0.50
M2	0.40	100	0.45	0	0.95	0	0.95	0	0.30	0	0.86	0.45
E6	2.21	0	0.45	0	0.95	0	0.95	0	0.30	100	0.86	0.86

 TABLE 2 – PROPOSED WEIGHTED C VALUES



COMPOSITE WEIGHTED "C" FACTOR CALCULATIONS 100 YEAR

ProjectFairmont Scottsdale Princess Privado Welcome Center and Parking ExpansionLocationScottsdale AZProject Number215319Project EngineerDarin Moore, PE

Proposed C Factor

Drainage Subbasin ID	Area	Area Desert		Paved		Roof		Grass		Commercial		100 YR Runoff Coefficient	
(Description/ID)	(Acres)	%	"C"	%	"C"	%	"C"	%	"C"	%	"C"	"C"	
S9	0.04	100	0.45	0	0.95	0	0.95	0	0.30	0	0.86	0.45	
M2	0.36	100	0.45	0	0.95	0	0.95	0	0.30	0	0.86	0.45	
E6	2.31	0	0.45	0	0.95	0	0.95	0	0.30	100	0.86	0.86	
E7	0.19	0	0.45	0	0.95	0	0.95	0	0.30	100	0.86	0.86	

TABLE 3 – EXISTING RATIONAL METHOD



Fairmont Scottsdale Princess Privado Welcome Center and Parking Expansion Project Location Scottsdale AZ 215319 Project Number **Project Engineer** Darin Moore, PE

EXISTING ON-SITE WATERSHEDS						100 YEAR				10 YEAR							
	Longest Watercourse 'L'	Longest Watercourse 'L'	Drainage Area 'A'	Drainage Area 'A'	'K₀' Type¹	Watershed Resistance Coefficient	Top Elevation	Bottom Elevation	Basin Slone 'S'		100 YEAR Intensity 'i'		Q100 Flow	Calculated Q10 'Tc' (See Note 2)	10 YEAR Intensity 'i'	Bunoff	Q10 Flow
	(ft)	(mi)	(sf)	(Acres)		'K _{b'}			(ft/mi)	(min)	(in/hr)	'C'	(cfs)	(min)	(in/hr)	'C'	(cfs)
S9	70	0.013	4,212	0.10	А	0.0463	49.9	44.0	444.4	1.0	9.31	0.50	0.5	1.2	5.88	0.41	0.2
M2	212	0.040	17,531	0.40	В	0.0854	34.4	31.2	79.7	4.4	8.03	0.86	2.8	5.4	4.91	0.37	0.7
E6	362	0.069	96,144	2.21	А	0.0379	49.2	41.0	119.6	3.3	8.45	0.45	8.4	4.0	5.22	0.80	9.2

Notes

1. Per Drainage Design Manual for Maricopa County, Vol. I, Hydrology (2013), Table 3.1: Equation for Estimating Kb in the Tc Equation

2. Minimum Tc is 5 minutes.

RATIONAL METHOD SUMMARY 100 YEAR, 10 YEAR

TABLE 4 – PROPOSED RATIONAL METHOD



STORMCEPTOR RATIONAL METHOD SUMMARY 100 YEAR, 10 YEAR

ProjectFairmont Scottsdale Princess Privado Welcome Center and Parking ExpansionLocationScottsdale AZProject Number215319Project EngineerDarin Moore, PE

PROPOSED ON-SITE WATERSHEDS 1						100 YEAR				10 YEAR							
	Longest Watercourse 'L'	Watorcoureo		Drainage Area 'A'	'K _b '	Watershed Resistance Coefficient	Top Elevation	Bottom Elevation	Basin		100 YEAR Intensity 'i'		Q100 Flow		10 YEAR Intensity 'i'	Bunoff	Q10 Flow
	(ft)	(mi)	(sf)	(Acres)		'K _{b'}			(ft/mi)	(min)	(in/hr)	'C'	(cfs)	(min)	(in/hr)	'C'	(cfs)
S9	53	0.010	1,896	0.04	А	0.0485	49.9	44.0	587.8	0.8	9.31	0.45	0.2	1.0	5.60	0.45	0.1
M2	171	0.032	15,864	0.36	В	0.0860	34.4	31.2	98.8	3.8	8.03	0.86	2.5	4.5	4.98	0.86	1.6
E6	554	0.105	100,754	2.31	А	0.0377	49.2	41.0	78.2	4.6	8.45	0.45	8.8	5.7	4.81	0.45	5.0
E7	136	0.026	8,111	0.19	А	0.0446	51.2	1539.8	38.0	3.1	8.78	0.86	1.4	3.8	4.98	0.86	0.8

Notes

1. Per Drainage Design Manual for Maricopa County, Vol. I, Hydrology (2013), Table 3.1: Equation for Estimating Kb in the Tc Equation

APPENDIX H – PRIVADO WELCOME BUILDING AND PARKING MODIFICATIONS – IMPROVEMENT PLANS

ENGINEER'S NOTES

MARICOPA ASSOCIATION OF GOVERNMENTS (M.A.G.) UNIFORM STANDARD SPECIFICATIONS AND DETAILS FOR PUBLIC WORKS CONSTRUCTION (LATEST EDITION INCLUDING LATEST REVISION AND CURRENT SUPPLEMENTALS THEREOF PER THE LOCAL TOWN OR CITY) ARE INCORPORATED INTO THIS PLAN IN THEIF ENTIRETY.

ALL WORK REQUIRED TO COMPLETE THE CONSTRUCTION COVERED BY THIS PLAN SHALL BE IN ACCORDANCE WITH THE M.A.G. STANDARD SPECIFICATIONS AND DETAILS AND CURRENT SUPPLEMENTS THEREOF PER THE LOCAL CITY OR TOWN UNLESS SPECIFIED OTHERWISE IN THESE PLANS OR ELSEWHERE IN THE CONTRACT DOCUMENTS. CONTRACTORS SHALL FAMILIARIZE THEMSELVES WITH ALL REQUIRED STANDARD SPECIFICATIONS, DETAILS AND SUPPLEMENTS PRIOR TO BIDDING THE WORK FOR THE CONSTRUCTION COVERED BY THIS PLAN.

- THE CONTRACTOR IS RESPONSIBLE FOR ALL METHODS, SEQUENCING, AND SAFETY CONCERNS ASSOCIATED WITH THIS PROJECT DURING CONSTRUCTION, UNLESS SPECIFICALLY ADDRESSED OTHERWISE IN THIS PLAN OR ELSEWHERE IN THE CONTRACT.
- THE CONTRACTOR IS TO COMPLY WITH ALL LOCAL, STATE, AND FEDERAL LAWS AND REGULATIONS APPLICABLE TO THE CONSTRUCTION COVERED BY THIS PLAN.
- THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND COMPLYING WITH ALL PERMITS REQUIRED TO COMPLETE ALL WORK COVERED BY THIS PLAN.
- THE QUANTITIES AND SITE CONDITIONS DEPICTED IN THESE PLANS ARE FOR GENERAL INFORMATIONAL PURPOSES ONLY AND MIGHT NOT REFLECT ACTUAL QUANTITIES AND SITE CONDITIONS. CONTRACTORS SHALL SATISFY THEMSELVES AS TO ACTUAL QUANTITIES AND SITE CONDITIONS PRIOR TO BIDDING THE WORK FOR THE CONSTRUCTION COVERED BY THIS PLAN.
- A REASONABLE EFFORT HAS BEEN MADE TO SHOW THE LOCATIONS OF EXISTING UNDERGROUND FACILITIES AND UTILITIES IN THE CONSTRUCTION AREA. THE CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE TO UTILITIES AND/OR FACILITIES CAUSED DURING THEIR CONSTRUCTION OPERATIONS. THE CONTRACTOR SHALL CALL 48 HOURS IN ADVANCE FOR BLUE STAKE (1-800-STAKE-IT) PRIOR TO ANY EXCAVATION.
- THE CONTRACTOR IS RESPONSIBLE FOR ALL COORDINATION OF CONSTRUCTION AFFECTING UTILITIES AND THE COORDINATION OF ANY NECESSARY UTILITY RELOCATION WORK
- ALL PAVING, GRADING, EXCAVATION, TRENCHING, PIPE BEDDING, CUT FILL AND BACKFILL SHALL COMPLY WITH THE RECOMMENDATIONS SET FORTH IN THE SOILS (GEOTECHNICAL) REPORT FOR THIS PROJECT IN ADDITION TO THE REFERENCED REQUIRED SPECIFICATIONS AND DETAILS. THE CONTRACTOR SHALL BE AWARE THAT CERTAIN UTILITIES REQUIRE PROPER ATTENTION AND CAREFUL PLANNING DURING SITE CONSTRUCTION. PLEASE NOTE THAT UTILITIES ON THESE PLANS MAY NOT EXHIBIT THE FULL PROTECTIVE COVER REQUIRED DURING THE SUBGRADE PREPARATION PHASE OF THE CONSTRUCTION. IN SUCH INSTANCES, THE CONTRACTOR SHALL PROVIDE ADDITIONAL PROTECTION (SUCH AS RAMPING) OF INCREASED PIPE STRENGTH TO PROVIDE THE NECESSARY PROTECTION REQUIRED TO PREVENT DAMAGE DURING THE CONSTRUCTION OF THIS PROJECT. THE CONTRACTOR SHALL HOLD THE ENGINEER HARMLESS IN ALL CASES FOR DAMAGES TO UTILITIES WHERE INADEQUATE PROTECTIVE MEASURES OCCUR.
- 10. THE CONTRACTOR IS TO VERIFY THE LOCATION AND THE ELEVATIONS OF ALL EXISTING UTILITIES AT POINTS OF TIE-IN PRIOR TO COMMENCING ANY NEW CONSTRUCTION. SHOULD ANY LOCATION OR ELEVATION DIFFER FROM THAT SHOWN ON THESE PLANS. THE CONTRACTOR SHALL CONTACT THE OWNER'S AGENT
- CONTRACTOR TO VERIFY AND COORDINATE ALL DIMENSIONS AND SITE LAYOUT WITH ARCHITECT'S FINAL SITE PLAN AND FINAL BUILDING DIMENSIONS BEFORE STARTING WORK. REPORT DISCREPANCIES TO OWNER'S AGENT.
- 12. COORDINATION BETWEEN ALL PARTIES IS ESSENTIAL PART OF CONTRACT.
- 13. CONTRACTOR IS RESPONSIBLE FOR PROJECT AND SITE CONDITIONS, AND TO WORK WITH WEATHER CONDITIONS AS THE PROJECT SITE MAY BE LOCATED IN A FLOOD PRONE AREA AND SUBJECT TO FLOODING AND ITS HAZARDS
- 14. THE CONTRACTOR IS TO VERIFY THE LOCATION, ELEVATION, CONDITION, AND PAVEMENT CROSS-SLOPE OF ALL EXISTING SURFACES AT POINTS OF TIE-IN AND MATCHING, PRIOR TO COMMENCEMENT OF GRADING, PAVING, CURB AND GUTTER, OR OTHER SURFACE CONSTRUCTION. SHOULD EXISTING LOCATIONS, ELEVATIONS, CONDITION, OR PAVEMENT CROSS-SLOPE DIFFER FROM THAT SHOWN ON THESE PLANS, RESULTING IN THE DESIGN INTENT REFLECTED ON THESE PLANS NOT ABLE TO BE CONSTRUCTED, THE CONTRACTOR SHALL NOTIFY THE OWNER'S AGENT IMMEDIATELY FOR DIRECTION ON HOW TO PROCEED PRIOR TO COMMENCEMENT OF CONSTRUCTION. THE CONTRACTOR ACCEPTS RESPONSIBILITY FOR ALL COSTS ASSOCIATED WITH CORRECTIVE ACTION IF THESE PROCEDURES ARE NOT FOLLOWED.
- CONTRACTOR IS RESPONSIBLE TO COORDINATE UTILITY CROSSINGS AT CULVERT CROSSINGS BEFORE STARTING WORK ON CULVERT, COORDINATE WITH OWNER REPRESENTATIVE. VERIFY UTILITY LINES AND/OR CONDUITS ARE IN PLACE BEFORE STARTING CULVERT WORK.
- 16. CONSTRUCT RETENTION BASIN AS SHOWN. CONTRACTOR TO SCARIFY BOTTOM OF BASIN TWO FEET DEEP AND NOT ALLOW COMPACTION OVER 80%.
- 17. THIS PROJECT REQUIRES A REGULAR ONGOING MAINTENANCE PROGRAM FOR THE DESIGNED DRAINAGE SYSTEM(S) TO PRESERVE THE DESIGN INTEGRITY AND THE ABILITY TO PERFORM ITS OPERATIONAL INTENT. FAILURE TO PROVIDE MAINTENANCE WILL JEOPARDIZE THE DRAINAGE SYSTEM(S)' PERFORMANCE AND MAY LEAD TO IT'S INABILITY TO PERFORM PROPERLY AND/OR CAUSE DAMAGE ELSEWHERE IN THE PROJECT.
- SEWER LINES DESIGNED IN PROFILE AND PUBLIC WATER LINES ARE REQUIRED TO BE ASBUILT AND THE INSTALLATION AND TESTING WITNESSED BY A PROFESSIONAL ENGINEER IN ACCORDANCE WITH ARIZONA ADMINISTRATIVE CODES R18-9-E301 "4.01 GENERAL PERMIT: SEWAGE COLLECTIONS SYSTEMS" AND R18-5-507 AND 508 "APPROVAL OF CONSTRUCTION" AND "RECORD DRAWINGS", RESPECTIVELY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY OWNER 72 HOURS IN ADVANCE WHEN THOSE SYSTEMS ARE READY TO BE WITNESSED.
- THE WORK PRODUCT PRESENTED IS BELIEVED TO BE COMPLIANT WITH THE INTENT OF THE CURRENT AMERICANS DISABILITIES ACT (ADA) REQUIREMENTS AS INTERPRETED BY THE REVIEWING AGENCY(S). IF CONSTRUCTION OF THE PROJECT IS DELAYED, THIS WORK PRODUCT SHOULD BE UPDATED TO ACCOUNT FOR ANY RELEVANT ADA UPDATES BEFORE CONSTRUCTION BEGINS.
- 20. LOWEST FLOOR (LF) REFERS TO EITHER FLOOR/SLAB ELEVATION OR TOP OF BASEMENT SLAB. LF ELEVATIONS ON THE GRADING AND DRAINAGE PLANS FOR RESIDENTIAL UNITS REFLECT SLAB ON GRADE CONDITIONS AND CANNOT BE LOWERED WITHOUT AGENCY APPROVAL IN LOCATIONS WHERE 'SPECIAL FLOOD HAZARD AREAS' EXIST. IN NON-FLOOD HAZARD LOCATIONS, TO ENSURE THAT ADEQUATE RESIDENTIAL LOT DRAINAGE CAN BE ACHIEVED, A PROFESSIONAL ENGINEER SHOULD BE CONSULTED IF THE LF FOR THE SLAB IS PROPOSED TO BE LOWERED. OR IF A BASEMENT IS TO BE CONSTRUCTED.

EARTHWORK QUANTITIES (ESTIMATED)

RAW CUT:

RAW FILL:

QUANTITIES ARE ESTIMATED IN PLACE, NO PRECOMPACTION, SHRINK OR SWELL IS ASSUMED.

CITY OF SCOTTSDALE NOTES PLEASE REFER TO SHEET C2 FOR CITY OF SCOTTSDALE NOTES.

QUANTITIES

PLEASE REFER TO SHEET C2 FOR ESTIMATED QUANTITIES FOR WORK IN PUBLIC **RIGHTS-OF-WAY AND EASEMENTS.**

LEGEND

PLEASE REFER TO SHEET C2 FOR LEGEND AND LIST OF ABBREVIATIONS.

UTILITY NOTES

- THESE PLANS HAVE BEEN SUBMITTED TO THE FOLLOWING UTILITY COMPANIES FOR APPROVAL WITHIN THEIR AREA OF INTEREST. THE SIZE AND LOCATIONS, AS SHOWN, OF THE GAS, TELEPHONE AND POWER LINES, AND CONNECTIONS AGREE WITH THE FURNISHED INFORMATION CONTAINED IN THE UTILITY COMPANY'S RECORDS. WHERE THE WORK TO BE DONE CONFLICTS WITH ANY OF THESE UTILITIES, THE CONFLICTS SHALL BE RESOLVED AS SPECIFIED IN THE SPECIAL PROVISIONS AND/OR AS OTHERWISE NOTED ON THESE PLANS. CONFLICTS ARISING DURING THE COURSE OF CONSTRUCTION FROM UNFORESEEN CIRCUMSTANCES SHALL BE REPORTED TO THE INTERESTED UTILITY COMPANY AND BE RESOLVED BY THEM AND THE DESIGN ENGINEER
- THE CITY WILL NOT PARTICIPATE IN THE COST OF CONSTRUCTION OR UTILITY RELOCATION.

FEMA FIRM NOTE (ZONE AO)

ACCORDING TO FEMA FLOOD INSURANCE RATE MAPPING, THE SUBJECT PROPERTY IS LOCATED IN 'SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD' "ZONE AO". ZONE AO IS DESCRIBED AS: "FLOOD DEPTHS OF 1 TO 3 FEET (USUALLY SHEET FLOW ON SLOPING TERRAIN); AVERAGE DEPTHS DETERMINED. FOR AREAS OF ALLUVIAL FAN FLOODING, VELOCITIES ALSO DETERMINED."

FLOOD INSURANCE RATE MAP (FIRM) INFORMATION

COMMUNITY NUMBER	PANEL NUMBER	SUFFIX	DATE OF FIRM	FIRM ZONE	BASE FLOOD ELEVATION (IN AO ZONE, USE DEPTH)
04013C	1320	L	07/20/2021	AO	1

ENGINEER'S CERTIFICATION

ENGINEER'S CERTIFICATION: THE LOWEST FLOOR ELEVATION(S) AND/OR FLOOD PROOFING ELEVATION(S) ON THIS PLAN ARE SUFFICIENTLY HIGH TO PROVIDE PROTECTION FROM FLOODING CAUSED BY A ONE-HUNDRED YEAR STORM, AND ARE IN ACCORDANCE WITH CITY OF SCOTTSDALE REVISED CODE, CHAPTER 37-FLOODPLAIN AND STORMWATER REGULATIONS.



AS-BUILT CERTIFICATION I HEREBY CERTIFY THAT THE "RECORD DRAWING" MEASUREMENTS AS SHOWN HEREON WERE MADE UNDER MY SUPERVISION OR AS NOTED AND ARE CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

REGISTERED ENGINEER/ LAND SURVEYOR

SEAL

PRIVADO WELCOME BUILDING AND PARKING MODIFICATIONS **IMPROVEMENT PLAN**

SCOTTSDALE, ARIZONA

A PORTION OF SECTION 35, TOWNSHIP 4 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER MERIDIAN, MARICOPA COUNTY, ARIZONA

213 CY 397 CY

10/12/2022

DATE

DATE

PARCEL DESCRIPTION

PARCEL NO, 2: (TENNIS COTTAGES PARCEL) THAT PORT OF THE SOUTHWEST ONE-QUARTER OF SECTION 35, TOWNSHIP 4 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER BASE AND MERIDIAN, MARICOPA COUNTY. ARIZONA, DESCRIBED AS FOLLOWS COMMENCING AT THE SOUTH ONE-QUARTER CORNER OF SAID SECTION 35;

THENCE NORTH 00 DEGREES 08 MINUTES 41 SECONDS EAST, ALONG THE NORTH-SOUTH MED-SECTION LINE OF SAID SECTION 35, A DISTANCE OF 1486.58 FEET; THENCE NORTH 89 DEGREES 49 MINUTES 06 SECONDS WEST, 840.00 FEET OF THE POINT OF BEGINNING:

THENCE SOUTH 27 DEGREES 44 MINUTES 13 SECONDS EAST, 177.75 FEET THENCE SOUTH 83 DEGREES 46 MINUTES 19 SECONDS EAST, 39.13 FEET THENCE SOUTH 26 DEGREES 47 MINUTES 27 SECONDS EAST, 26.35 FEET THENCE SOUTH 58 DEGREES 07 MINUTES 53 SECONDS EAST, 43.04 FEET THENCE SOUTH 88 DEGREES 18 MINUTES 25 SECONDS EAST, 29.86 FEET THENCE SOUTH 07 DEGREES 07 MINUTES 02 SECONDS EAST, 47.49 FEET THENCE SOUTH 66 DEGREES 38 MINUTES 00 SECONDS WEST, 275.66 FEET TO THE BEGINNING OF A CURVE CONCAVE TO THE SOUTHEAST HAVING A RADIUS OF 150.00

FEET; THENCE SOUTH WESTERLY ALONG SAID CURVE THOUGH A CENTRAL ANGLE OF 12 DEGREES 08 MINUTES 15 SECONDS, AN ARCH DISTANCE OF 31.78 FEET; THENCE SOUTH 54 DEGREES 29 MINUTES 46 SECONDS WEST, 446.31 FEET; THENCE NORTH 84 DEGREES 49 MINUTES 13 SECONDS WEST, 43.57 FEET; THENCE NORTH 00 DEGREES 01 MINUTES 45 SECONDS EAST, 619.54 FEET; THENCE SOUTH 89 DEGREES 49 MINUTES 06 SECONDS EAST, 377.78; THENCE NORTH 00 DEGREES 01 MINUTES 45 SECONDS EAST, 18.00 FEET THENCE SOUTH 89 DEGREES 49 MINUTES 06 SECONDS EAST, 103.52 FEET TO THE

POINT OF BEGINNING. EXCEPT ONE-HALF OF ALL OIL AND MINERAL RIGHTS AS RESERVED IN INSTRUMENT RECORDED IN BOOK 124, PAGE 39, RECORDS OF MARICOPA COUNTY, ARIZONA; AND

ALSO EXCEPT ALL OIL GAS, OTHER HYDROCARBON SUBSTANCES, HELIUM OR OTHER SUBSTANCES OF A GASEOUS NATURE, COAL, METALS, MINERALS, FOSSILS, FERTILIZER

OF EVERY NAME AND DESCRIPTION; AND ALSO EXCEPT ALL URANIUM, THORIUM OR ANY OTHER MATERIAL WHICH IS OR MAY BE DETERMINED TO BE PECULIARLY ESSENTIAL TO THE PRODUCTION OF FISSIONABLE MATERIALS WHETHER OR NOT OF COMMERCIAL VALUE, AS SET FORTH IN SECTION 37-231. ARIZONA REVISED STATUTES.

SOILS REPORT NOTE

A SOILS GEOTECHNICAL REPORT HAS BEEN PREPARED FOR THIS PROJECT TITLED FAIRMONT CASITAS ADDITION BY ALPHA GEOTECHNICAL & MATERIALS, INC, DATED DECEMBER 15, 2021. REPORT NO. 21-G-12692.

NO CONFLICT SIGNATURE BLOCK UTILITY UTILITY NAME OF COMPANY REPRESENTATIVE TELEPHONE NUMBER DATE CONTACTED DATE SIGNED ELECTRIC ARIZONA PUBLIC SERVICE HAILEY PARKS 602-493-4401 08/22/2022 TELEPHONE LUMEN JEANETTE DEBOARD 480-221-7810 08/22/2022 NATURAL GAS SOUTHWEST GAS ANDY SAKS 480-730-3857 08/22/2022 CABLE TV COX COMMUNICATIONS JACOB HORSMAN - 08/22/2022 OTHER MCI RICHARD YOUNG 602-615-8995 08/22/2022 OTHER MCI RICHARD YOUNG 602-615-8995 08/22/2022 ENGINEER'S CERTIFICATION I I I								
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CABLE TVCOX COMMUNICATIONSJACOB HORSMAN-08/22/2022OTHERMCIRICHARD YOUNG602-615-899508/22/2022ENGINEER'S CERTIFICATION	TELEPHONE	LUMEN	JEANETTE DEBOARD	480-221-7810	08/22/2022			
OTHER MCI RICHARD YOUNG 602-615-8995 08/22/2022 ENGINEER'S CERTIFICATION Image: Certification in the second s	NATURAL GAS	SOUTHWEST GAS	ANDY SAKS	480-730-3857	08/22/2022			
ENGINEER'S CERTIFICATION	CABLE TV	COX COMMUNICATIONS	JACOB HORSMAN	-	08/22/2022			
	OTHER	MCI	RICHARD YOUNG	602-615-8995	08/22/2022			
I DARIN L MOORE P.F. AS THE ENGINEER OF RECORD FOR THIS DEVELOPMENT. HEREBY	ENGINEER'S CERTIFICATION							

FBY CERTIFY THAT ALL UTILITY COMPANIES LISTED ABOVE HAVE BEEN PROVIDED FINAL IMPROVEMENT PLANS FOR REVIEW, AND THAT ALL CONFLICTS IDENTIFIED BY THE UTILITIES HAVE BEEN RESOLVED. IN ADDITION, "NO CONFLICT" FORMS HAVE BEEN OBTAINED FROM EACH UTILITY COMPANY AND ARE INCLUDED IN THIS SUBMITTAL.

DARW L. MARE

SIGNATURE

10/12/2022

DATE

SHEET INDEX C1 COVER SHEET

01	OUVER C
C2	NOTES &
C3	INDEX M/
C4-C5	DEMOLIT
C6-C7	GRADING
C8	WATER &
C9-C10	SIGNING
C11	DETAILS

BENCHMARK

CITY OF SCOTTSDALE BRASS CAP FLUSH 450'± NORTH OF PRINCESS DRIVE ON SCOTTSDALE ROAD, BEING THE WEST QUARTER CORNER OF SECTION 35, TOWNSHIP 4 NORTH, RANGE 4 EAST, CITY OF SCOTTSDALE DATUM, NAVD88 DATUM ELEVATION=1553.22'.

MAINTENANCE (BMM) CRITERIA.

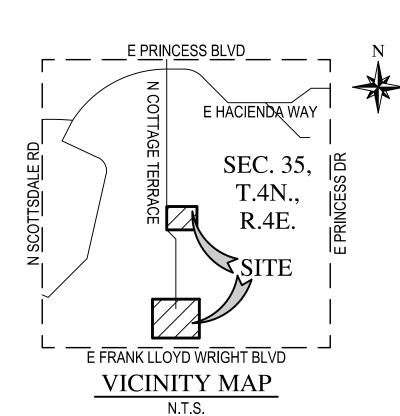
PUBLIC UTILITIES

WATER SEWER ELECTRIC **TELEPHONE** NATURAL GAS CABLE TV

QUANTITIES 1AP TION PLAN G & DRAINAGE PLAN & SEWER PLAN & STRIPING PLAN **DETAILS & SECTIONS**

I HEREBY CERTIFY THAT ALL ELEVATIONS REPRESENTED ON THIS PLAN ARE BASED ON NAVD 1988, MCDOT, AND MEET THE FEMA BENCHMARK

CITY OF SCOTTSDALE CITY OF SCOTTSDALE APS LUMEN SOUTHWEST GAS COX COMMUNICATIONS



OWNER / DEVELOPER

STRATEGIC HOTELS & RESORTS 150 NORTH RIVERSIDE PLAZA, SUITE 4270 CHICAGO, IL 60606 CONTACT: TIMOTHY TAYLOR PHONE: (312) 658-6038

ENGINEER

WOOD, PATEL & ASSOCIATES, INC. 2051 WEST NORTHERN AVENUE, SUITE 100 PHOENIX, ARIZONA 85021 CONTACT: DARIN MOORE, P.E. PHONE: (602) 335-8500 FAX: (602) 335-8580

ARCHITEC

KOLLIN ALTOMARE ARCHITECTS 4265 E. CONANT STREET, SUITE 101 LONG BEACH, CA 90808 CONTACT: PAUL ALTOMARE PHONE: (562) 597-8760

PROJECT SITE DATA

ASSESSOR PARCEL NUMBER(S): 215-08-003C PROJECT SITE ADDRESS: 7575 E PRINCESS BLVD SCOTTSDALE, ARIZONA 85255 PROJECT SITE AREA(S): NET AREA = 6.09 AC DISTURBED AREA = 0.68± AC ZONING: C2

CITY OF SCOTTSDALE CIVIL APPROVAL

REVIEW & RECOMMENDED APPROVAL BY: PAVING **GRADING &** DRAINAGE WATER & SEWER RETAINING WALLS

ENGINEERING DEPARTMENT MANAGER

SIGNS & MARKINGS PLANNING FIRE SIGNALS & STREET

DATE



WOOD

CHECKED BY: DM DESIGNED BY: RS DRAFTED BY: JR

CITY OF SCOTTSDALE NOTES

- GENERAL CONSTRUCTION NOTES FOR CAPITAL PROJECTS
- 1. ALL IMPROVEMENT CONSTRUCTION SHALL COMPLY WITH THE 2020 MARICOPA COUNTY ASSOCIATION OF GOVERNMENTS STANDARD SPECIFICATIONS AND DETAILS FOR PUBLIC WORKS CONSTRUCTION AS AMENDED BY THE LATEST VERSION OF THE CITY OF SCOTTSDALE SUPPLEMENTAL STANDARD SPECIFICATIONS AND DETAILS AND CITY OF SCOTTSDALE'S DESIGN STANDARDS & POLICIES MANUAL (DS&PM). IF THERE IS A CONFLICT. THE LATTER SHALL APPLY, ALL FACILITIES CONSTRUCTION SHALL COMPLY WITH THE LATEST BUILDING CODES AS AMENDED AND ADOPTED BY THE CITY OF SCOTTSDALE.
- 2. THE ENGINEERING DESIGNS ON THESE PLANS ARE APPROVED BY THE CITY IN SCOPE AND NOT IN DETAIL. IF CONSTRUCTION QUANTITIES ARE SHOWN ON THESE PLANS, THEY ARE NOT VERIFIED BY THE CITY.
- BASED ON THE INFORMATION SUBMITTED ON THE PLANS AND ASSOCIATED DOCUMENTS, THE CITY HAS REVIEWED AND FOUND THEM TO BE IN ACCORDANCE WITH THE SCOTTSDALE REVISED CODE AND ARE ACCEPTABLE FOR PERMIT ISSUANCE. THIS ACCEPTANCE BY THE CITY DOES NOT AUTHORIZE VIOLATIONS OF ANY APPLICABLE CODE, ORDINANCE OR STANDARD AS ADOPTED BY THE SCOTTSDALE REVISED CODE.
- APPROVAL OF THE PLANS BY THE CITY IS VALID FOR SIX MONTHS. IF A PERMIT FOR THE CONSTRUCTION HAS NOT BEEN ISSUED WITHIN SIX MONTHS OF REVIEW, THE PLANS SHALL BE RESUBMITTED TO THE CITY FOR REAPPROVAL.
- 5. ANY DEVIATION FROM THE APPROVED PLANS SHALL BE REVIEWED AND APPROVED BY THE CITY PRIOR TO THAT CHANGE BEING INCORPORATED INTO THE PROJECT.
- A CITY CAPITAL PROJECTS INSPECTOR WILL INSPECT ALL WORK WITHIN THE CITY RIGHTS-OF-WAY, EASEMENTS AND FACILITIES.
- 7. ANY SPECIAL INSPECTION REQUIRED SHALL BE IN ADDITION TO ANY ROUTINE INSPECTION BY THE CITY.
- 8. CITY ENCROACHMENT AND BUILDING PERMITS ARE REQUIRED FOR WORK IN PUBLIC RIGHTS-OF-WAY, EASEMENTS GRANTED FOR PUBLIC PURPOSES AND FACILITIES. PERMITS WILL BE ISSUED BY THE CITY THROUGH THE CITY'S ONE STOP SHOP. COPIES OF ALL PERMITS SHALL BE RETAINED ON-SITE AND SHALL BE AVAILABLE FOR INSPECTION AT ALL TIMES. FAILURE TO PRODUCE THE REQUIRED PERMITS WILL RESULT IN IMMEDIATE WORK STOPPAGE UNTIL THE PROPER PERMIT DOCUMENTATION IS OBTAINED.
- 9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS FOR SALVAGING PROTECTED NATIVE PLANTS PRIOR TO THE START OF CONSTRUCTION.
- 10. CONTRACTOR SHALL CONTACT AZ 811 TWO FULL WORKING DAYS PRIOR TO BEGINNING EXCAVATION.
- 11. ALL EXCAVATION AND GRADING WHICH IS NOT IN PUBLIC RIGHTS-OF-WAY OR IN EASEMENTS GRANTED FOR PUBLIC PURPOSES MUST CONFORM TO SECTION 1803 AND APPENDIX J OF THE LATEST INTERNATIONAL CODE COUNCIL AS ADOPTED AND AMENDED BY THE CITY OF SCOTTSDALE. A PERMIT FOR THIS GRADING MUST BE SECURED FROM THE CITY.
- 12. THRUST RESTRAINT, WHERE REQUIRED, ON ALL CITY WATER LINES SHALL BE PROVIDED USING MEGALUG MECHANICAL JOINT RESTRAINTS OR CITY-APPROVED EQUAL
- 13. ANY ASPHALT MIX DESIGN USED ON CITY PROJECTS SHALL HAVE BEEN APPROVED FOR THAT USE PER SECTION 5-10 OF THE CITY'S DS&PM AND APPEAR ON THE "APPROVED LIST OF ASPHALT MIXES" AS DISTRIBUTED BY THE EAST VALLEY ASPHALT COMMITTEE (EVAC).
- 14. THE CONTRACTOR SHALL BE RESPONSIBLE TO REMOVE AND REPLACE, AT NO ADDITIONAL COST TO THE CITY, ANY AND ALL PAVEMENT, SIDEWALK, CURB AND GUTTER, DRAINAGE STRUCTURES, ETC. OUTSIDE THE PAY LIMIT THAT ARE DAMAGED DUE TO THEIR ACTIVITIES ON THE PROJECT. THIS INCLUDES, BUT IS NOT LIMITED TO, THE REMOVAL AND REPLACEMENT OF NEWLY CRACKED ROADWAY INFRASTRUCTURE, THE REMOVAL AND REPLACEMENT OF EXISTING CRACKED ROADWAY INFRASTRUCTURE WHERE THE CRACKS HAVE BEEN ENLARGED DUE TO THE CONTRACTOR'S OPERATIONS, THE REMOVAL AND REPLACEMENT OF DEFORMED ROADWAY INFRASTRUCTURE. ALL SAWCUTS USED FOR THE REMOVAL OF THESE ITEMS SHALL BE PERPENDICULAR AND PARALLEL TO THE CENTERLINE CONTROLLING THAT ITEM. OR AT THE DIRECTION OF THE CITY'S CAPITAL PROJECTS INSPECTOR.
- 15. ALL CAPITAL IMPROVEMENT PROJECTS SHALL MEET THE PROCEDURES AND STANDARDS FOR THE USE OF TEMPORARY/SECURITY FENCING AROUND THE PERIMETER OF CONSTRUCTION SITES, AS DEFINED IN THE CITY'S ZONING ORDINANCE, ARTICLE VII, SECTION 7.700.

GENERAL NOTES FOR PUBLIC WORKS CONSTRUCTION

- 1. ALL CONSTRUCTION IN THE PUBLIC RIGHTS-OF-WAY OR IN EASEMENTS GRANTED FOR PUBLIC USE MUST CONFORM TO THE LATEST MAG UNIFORM STANDARD SPECIFICATIONS AND UNIFORM STANDARD DETAILS FOR PUBLIC WORKS CONSTRUCTION AS AMENDED BY THE LATEST VERSION OF THE CITY OF SCOTTSDALE SUPPLEMENTAL STANDARD SPECIFICATIONS AND SUPPLEMENTAL STANDARD DETAILS. IF THERE IS A CONFLICT, THE CITY'S SUPPLEMENTAL STANDARD DETAILS WILL GOVERN.
- 2. THE CITY ONLY APPROVES THE SCOPE, NOT THE DETAIL, OF ENGINEERING DESIGNS: THEREFORE, IF CONSTRUCTION QUANTITIES ARE SHOWN ON THESE PLANS, THEY ARE NOT VERIFIED BY THE CITY.
- 3. THE APPROVAL OF PLANS IS VALID FOR SIX (6) MONTHS. IF A RIGHT-OF-WAY PERMIT FOR THE CONSTRUCTION HAS NOT BEEN ISSUED WITHIN THIS TIME FRAME, THE PLANS MUST BE RESUBMITTED TO THE CITY FOR REAPPROVAL.
- 4. A CITY INSPECTOR WILL INSPECT ALL WORKS WITHIN THE CITY OF SCOTTSDALE. NOTIFY INSPECTION SERVICES 72 HOURS BEFORE BEGINNING WORK.
- 5. WHENEVER EXCAVATION IS NECESSARY, CALL THE BLUE STAKE CENTER, 811, TWO WORKING DAYS **BEFORE EXCAVATION BEGINS.**
- 6. PERMISSION TO WORK IN THE RIGHT-OF-WAY (PWR) PERMITS ARE REQUIRED FOR ALL WORKS WITHIN THE RIGHTS-OF-WAY AND EASEMENTS GRANTED FOR PUBLIC PURPOSES. COPIES OF ALL PERMITS MUST BE RETAINED ON-SITE AND BE AVAILABLE FOR INSPECTION AT ALL TIMES. FAILURE TO PRODUCE THE REQUIRED PERMITS WILL RESULT IN IMMEDIATE SUSPENSION OF ALL WORK UNTIL THE PROPER PERMIT DOCUMENTATION IS OBTAINED.

FIRE NOTE:

1. ALL PRIVATE STREETS AND DRIVES SHALL CONFORM TO THE FIRE DEPARTMENT GUIDELINES FOR EMERGENCY VEHICLE ACCESS.

SEWER NOTE:

- 1. THE ONSITE SEWER SYSTEM CONSTRUCTED BY THIS PLAN SET IS A PRIVATE SYSTEM AND WILL NOT BE MAINTAINED BY THE CITY OF SCOTTSDALE.
- 2. MAINTENANCE OF THE ONSITE SEWER SYSTEM IS THE RESPONSIBILITY OF THE OWNER.

WATER NOTE:

1. THE WATER SYSTEM SHOWN HEREIN HAS BEEN DESIGNED TO ADEQUATELY SUPPLY WATER IN SUFFICIENT QUANTITY AND PRESSURE TO MEET LOCAL FIRE REQUIREMENTS.

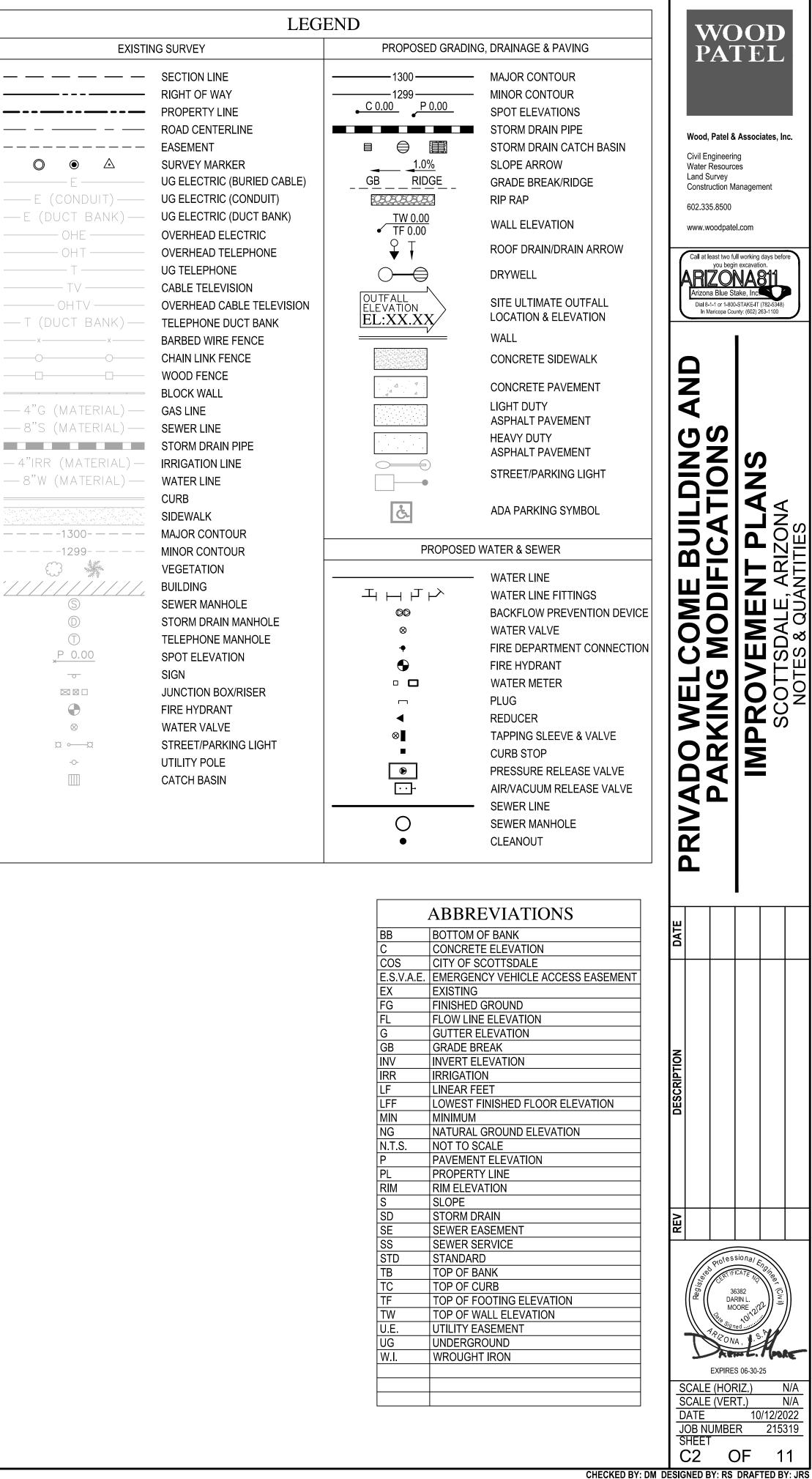
PAVING QUANTITIES (ESTIMATED)

3" A.C. OVER 7" A.B.C.	1593.37	SY
3" A.C. OVER 11" A.B.C.	5829.1	SY
6" VERTICAL CURB & GUTTER	260.4	LF
6" P.C.C OVER 4" A.B.C. OVERCOMPACTED	486.0	SY
6" SINGLE CURB	128.8	LF
CONCRETE SIDEWALK	3,771.84	SF
CONCRETE APRON	709.10	SF
VALLEY GUTTER	140.15	LF
SIDEWALK RAMP	1	EA
SAWCUT, REMOVE & REPLACE EXISTING PAVEMENT	7776.03	SY
PRIVATE WATER QUANTITIES (EST	IMATED)	
2" DOMESTIC WATER SERVICE & METER	1	EA
2" REDUCED PRESSURE PRINCIPLE BACKFLOW PREVENTION	2	EA
6" REDUCED PRESSURE PRINCIPLE BACKFLOW PREVENTION	1	EA
FIRE HYDRANT	1	EA
6" WATER VALVE	1	EA
45° BEND	4	EA
2" SCH 80 PVC WATER LINE	196.30	LF

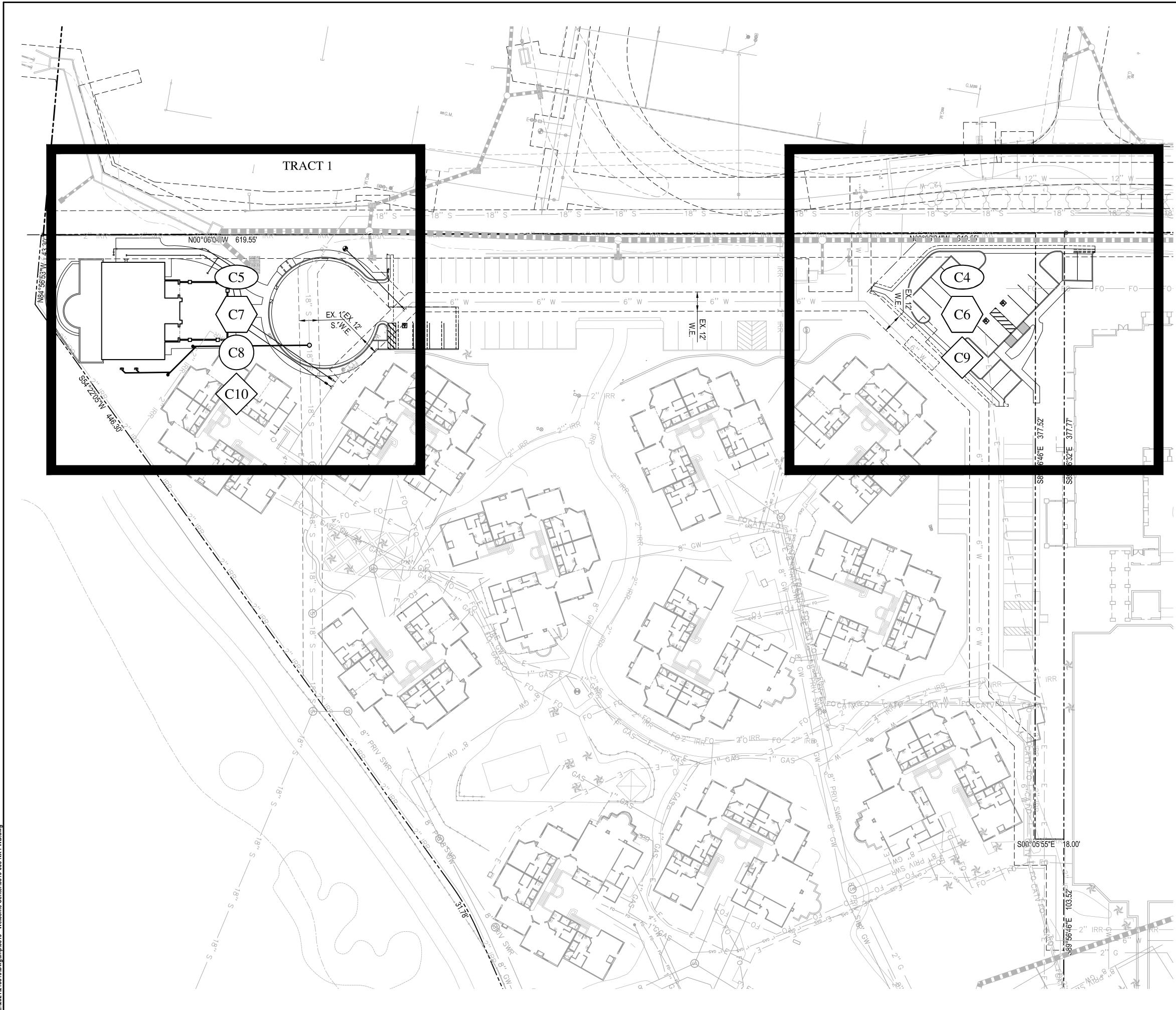
6" POLYWRAPPED D.I.P. PRESSURE CLASS 350 WATER LINE	223.0	LF
CONNECT TO EXISTING WATERLINE	3	EA
PRIVATE SEWER QUANTITIES (ESTIMATE	ED)	
CONNECT TO EXISTING SEWER LINE	1	EA
6" PVC SDR 35 SANITARY SEWER PIPE	127.8	LF
SANITARY SEWER CLEAN OUT	2	ΕA
WYE OR WYE WITH 45° BEND	2	ΕA

30" SANITARY SEWER MANHOLE

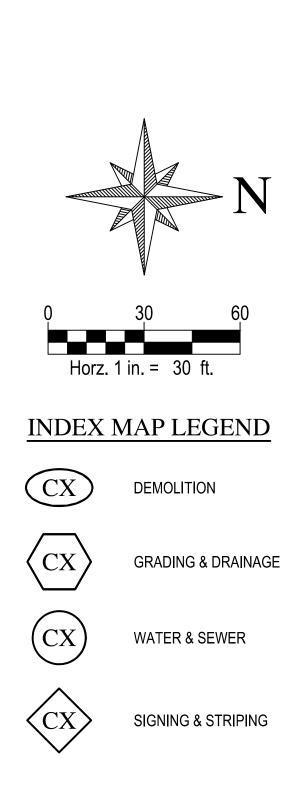
1 EA



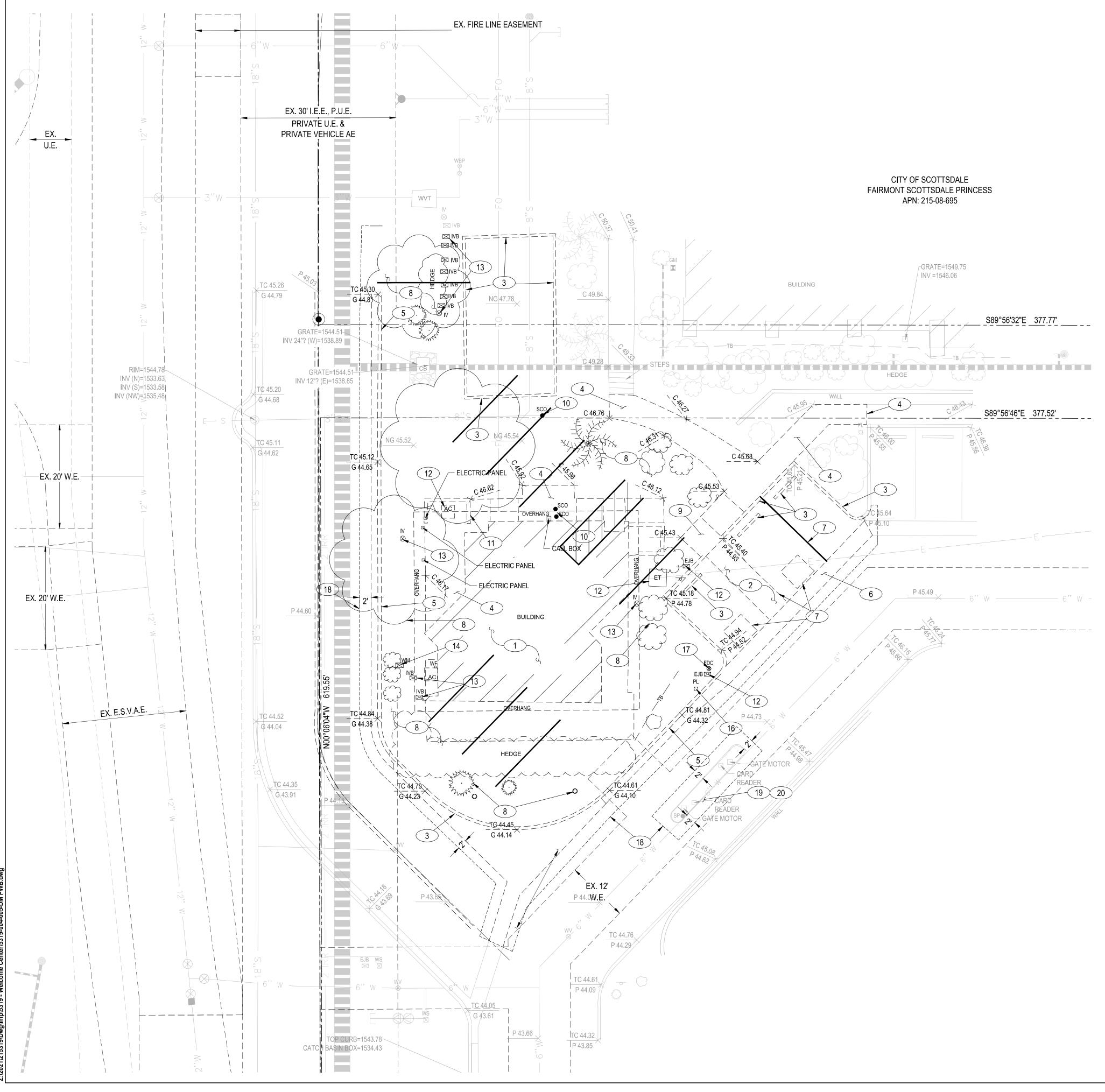
BB	BOTTOM OF BANK
С	CONCRETE ELEVATION
COS	CITY OF SCOTTSDALE
E.S.V.A.E.	EMERGENCY VEHICLE ACCESS EASEMENT
EX	EXISTING
FG	FINISHED GROUND
FL	FLOW LINE ELEVATION
G	GUTTER ELEVATION
GB	GRADE BREAK
INV	INVERT ELEVATION
IRR	IRRIGATION
LF	LINEAR FEET
LFF	LOWEST FINISHED FLOOR ELEVATION
MIN	MINIMUM
NG	NATURAL GROUND ELEVATION
N.T.S.	NOT TO SCALE
Ρ	PAVEMENT ELEVATION
PL	PROPERTY LINE
RIM	RIM ELEVATION
S	SLOPE
SD	STORM DRAIN
SE	SEWER EASEMENT
SS	SEWER SERVICE
STD	STANDARD
ТВ	TOP OF BANK
TC	TOP OF CURB
TF	TOP OF FOOTING ELEVATION
TW	TOP OF WALL ELEVATION
U.E.	UTILITY EASEMENT
UG	UNDERGROUND
W.I.	WROUGHT IRON



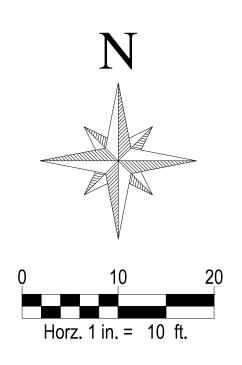






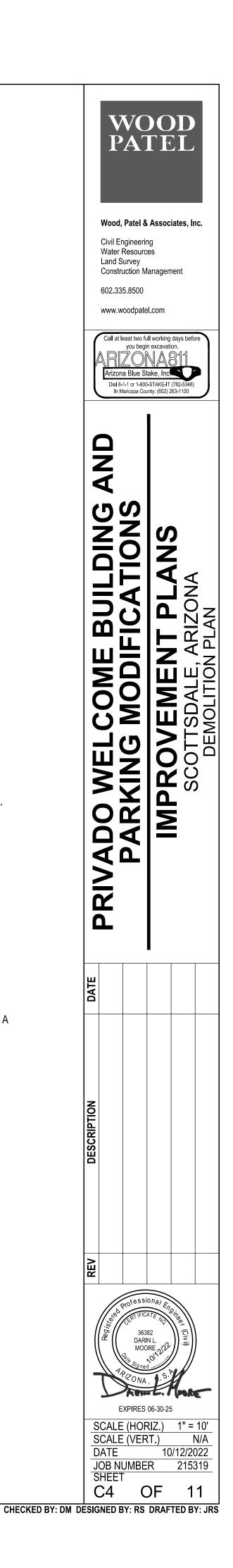


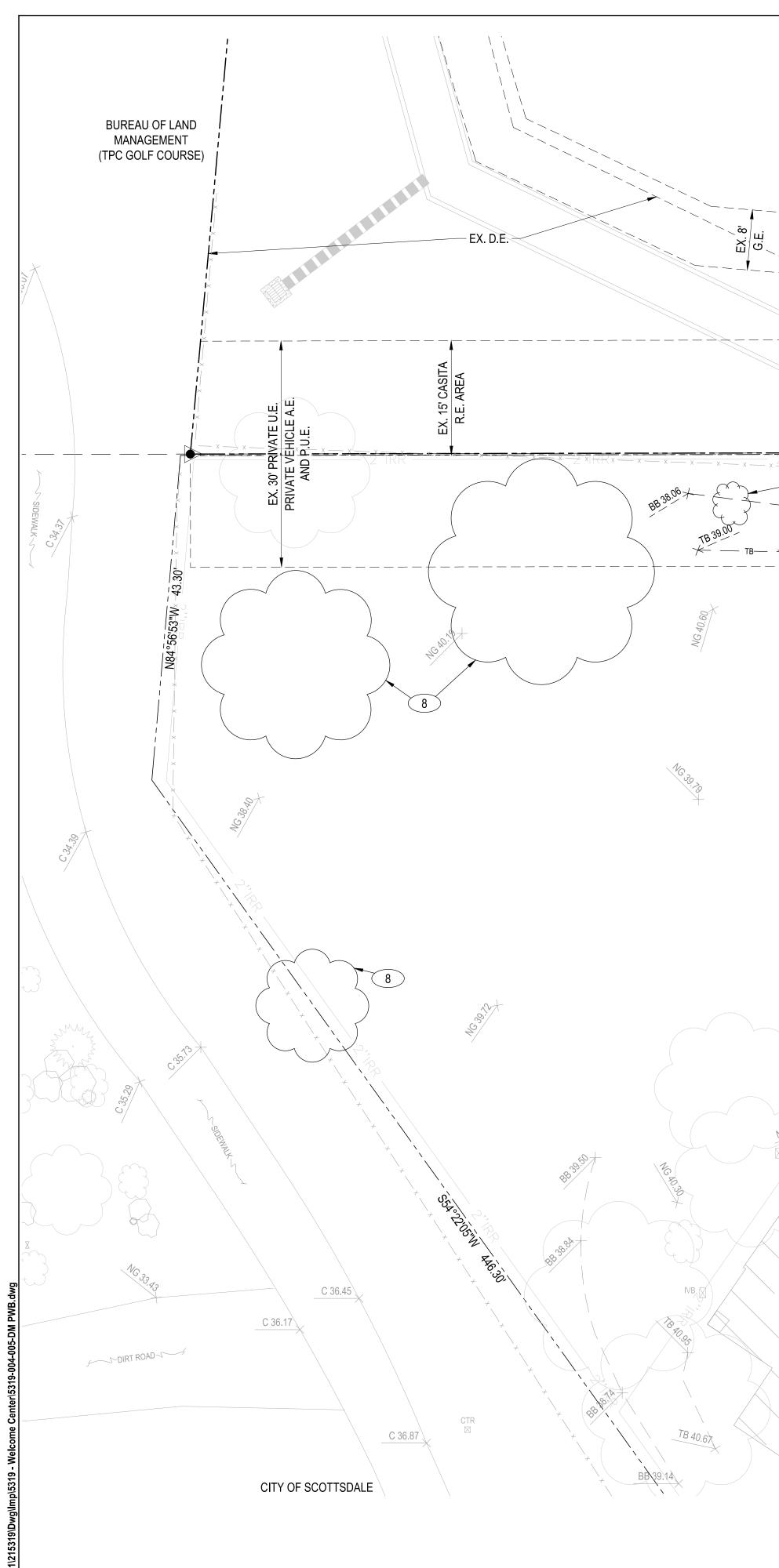
.215319\Dwg\Imp\5319 - Welcome Center\5319-004-005-DM PWB



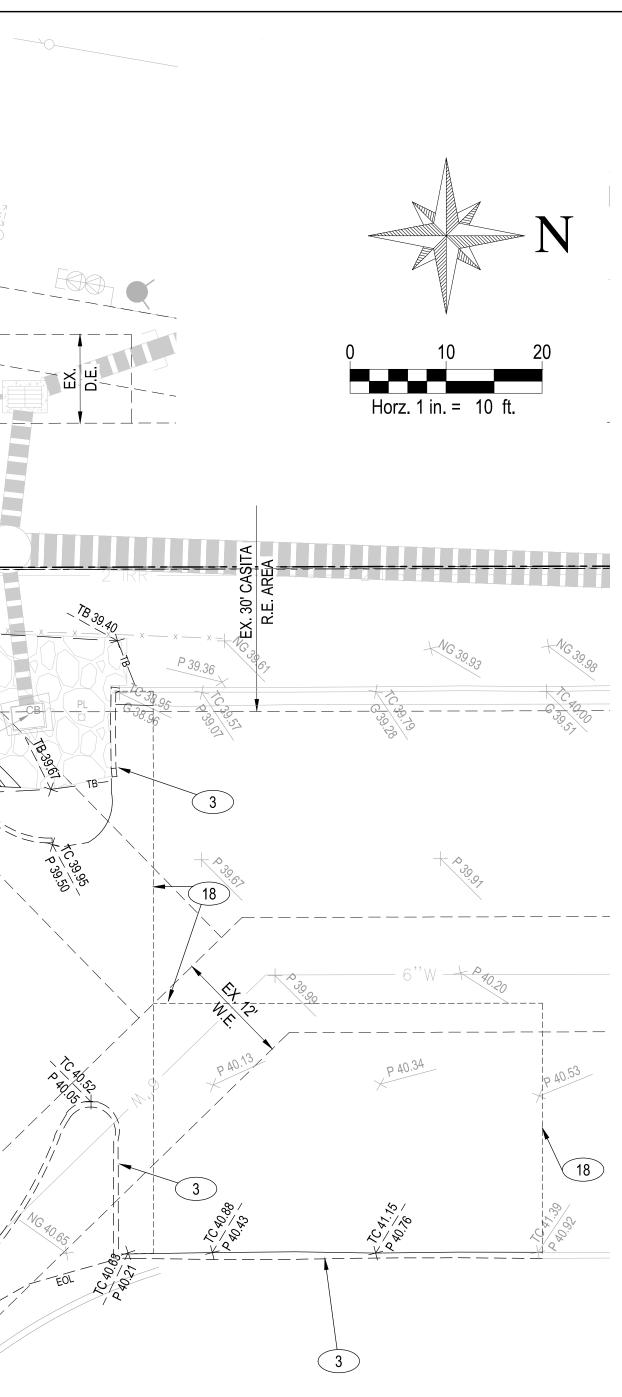
DEMOLITION NOTES

- 1 REMOVE EXISTING BUILDING.
- 2 REMOVE EXISTING ASPHALT PAVEMENT.
- 3 REMOVE EXISTING CURB.
- 4 REMOVE EXISTING SIDEWALK.
- 5 REMOVE EXISTING CURB & GUTTER.
- 6 REMOVE EXISTING VALLEY GUTTER.
- 7 OBLITERATE EXISTING STRIPING TO THE LIMITS SHOWN PER ADOT STD. SPECIFICATIONS FOR ROAD AND BRIDGE. PAINTING OVER EXISTING STRIPING IS NOT ACCEPTABLE.
- 8 REMOVE EXISTING LANDSCAPE.
- 9 REMOVE EXISTING SIDEWALK RAMP.
- 10 EXISTING SEWER SERVICE TO BE CAPPED & CLEANOUT TO BE REMOVED. CONTRACTOR TO COORDINATE WITH CITY OF SCOTTSDALE PRIOR TO REMOVALS.
- (1) REMOVE EXISTING FENCE.
- 12 EXISTING ELECTRICAL LINE AND EQUIPMENT TO BE REMOVED. CONTRACTOR TO COORDINATE WITH APS PRIOR TO REMOVALS.
- (13) EXISTING IRRIGATION LINE AND EQUIPMENT TO BE RELOCATED. SEE LANDSCAPE PLAN FOR DETAILS.
- 14 EXISTING WATER LINE TO BE CAPPED AND EQUIPMENT TO BE REMOVED. CONTRACTOR TO COORDINATE WITH CITY OF SCOTTSDALE PRIOR TO REMOVALS.
- (16) EXISTING LIGHT POLE TO BE SALVAGED FOR RELOCATION.
- (17) EXISTING FDC TO REMAIN AND PROTECTED IN-PLACE.
- (18) SAWCUT AND REMOVE EXISTING PAVEMENT TO THE LIMITS
- SHOWN PER M.A.G. SPEC. 336. (19) EXISTING GATE AND CARD READER TO BE REMOVED. CONTRACTOR TO COORDINATE WITH OWNER
- 20 CONTRACTOR TO VERIFY DEPTH TO EXISTING 6" WATER LINE IS A MINIMUM OF 3'. NOTIFY ENGINEER OF ANY DISCREPANCY.





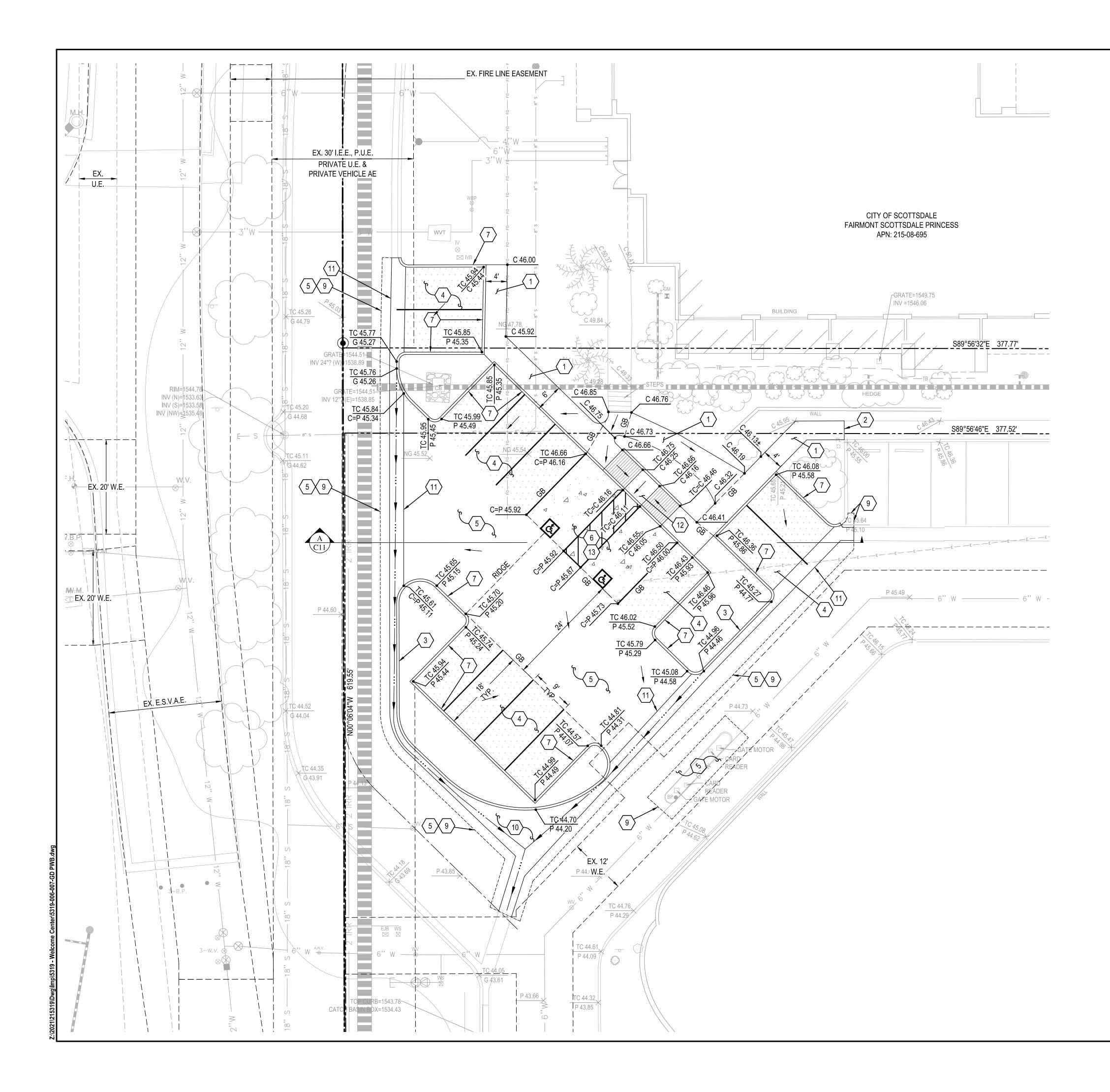
TRACT 1 N00°06'04"W 619.55' <u>م</u>ز کر 3 CAI INV 12" OR 15" (E)=1531.40 INV (NW)=1531.29 INV (SW)=1531.39 15 EX. 12' S.E. (8)ĺ 13 (3) BUILDING FMT SCOTTSDALE OWNER LLC

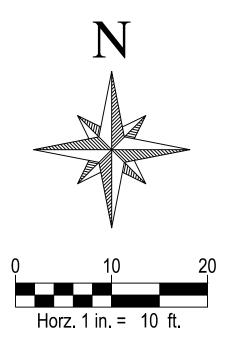




- 2 REMOVE EXISTING ASPHALT PAVEMENT.
- 3 REMOVE EXISTING CURB.
- 8 REMOVE EXISTING LANDSCAPE.
- (13) EXISTING IRRIGATION LINE AND EQUIPMENT TO BE RELOCATED. SEE LANDSCAPE PLAN FOR DETAILS.
- (15) REMOVE EXISTING RIP-RAP.
- (18) SAWCUT AND REMOVE EXISTING PAVEMENT TO THE LIMITS SHOWN PER M.A.G. SPEC. 336.
- (21) EXISTING WATER SERVICE AND BACKFLOW PREVENTER TO BE RELOCATED. SEE SHEET C8 FOR MORE INFORMATION.



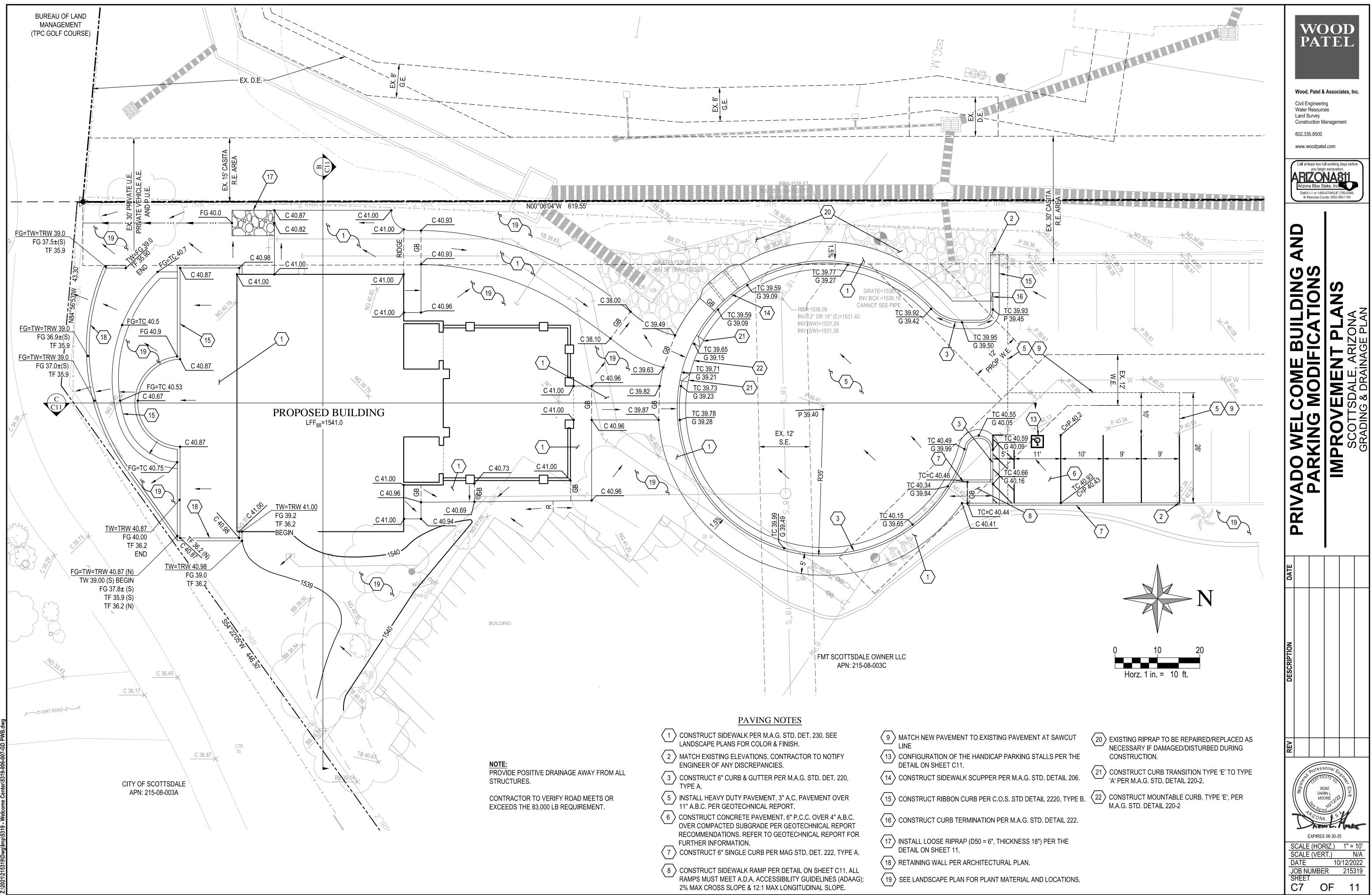


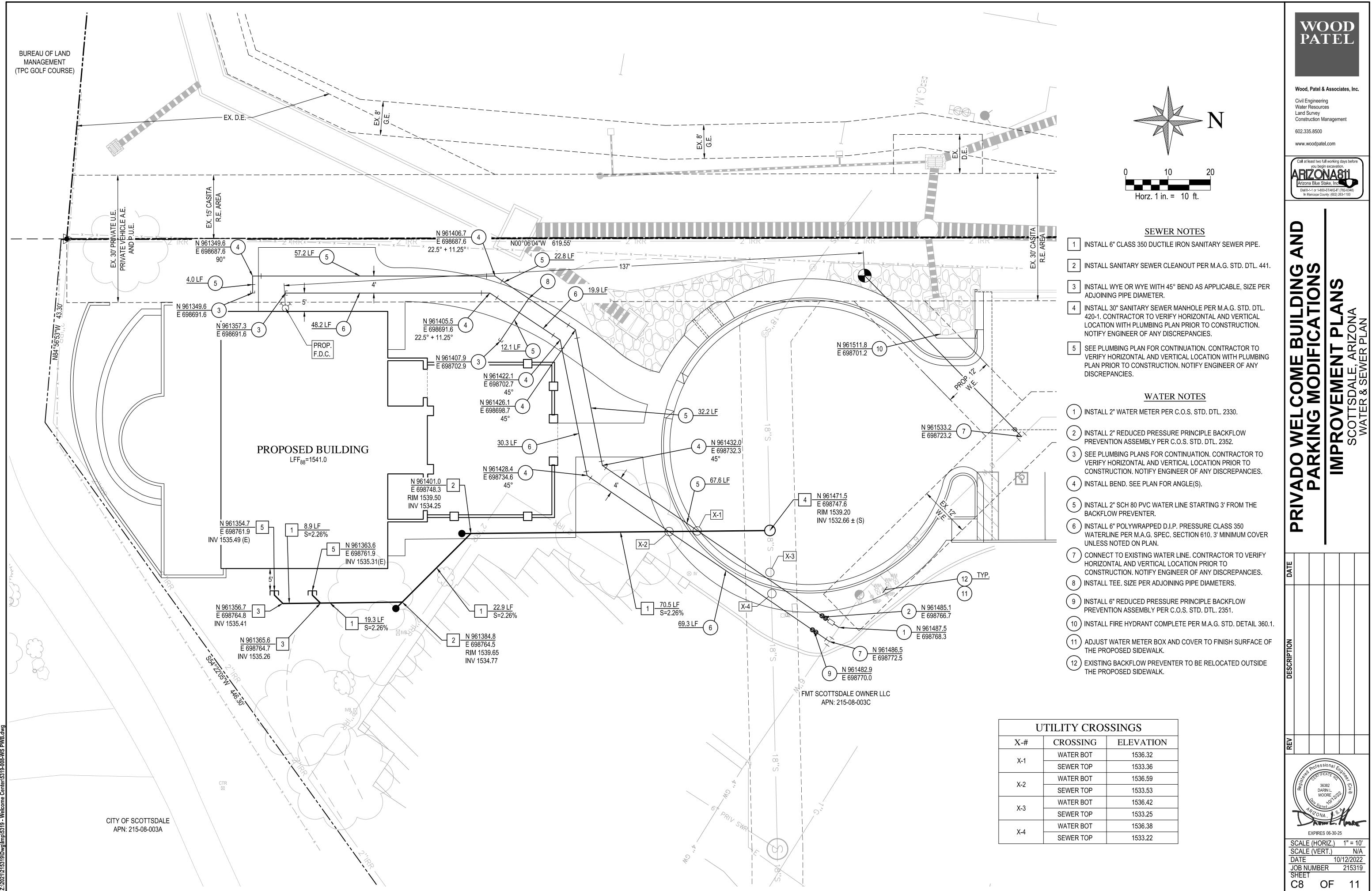


PAVING NOTES

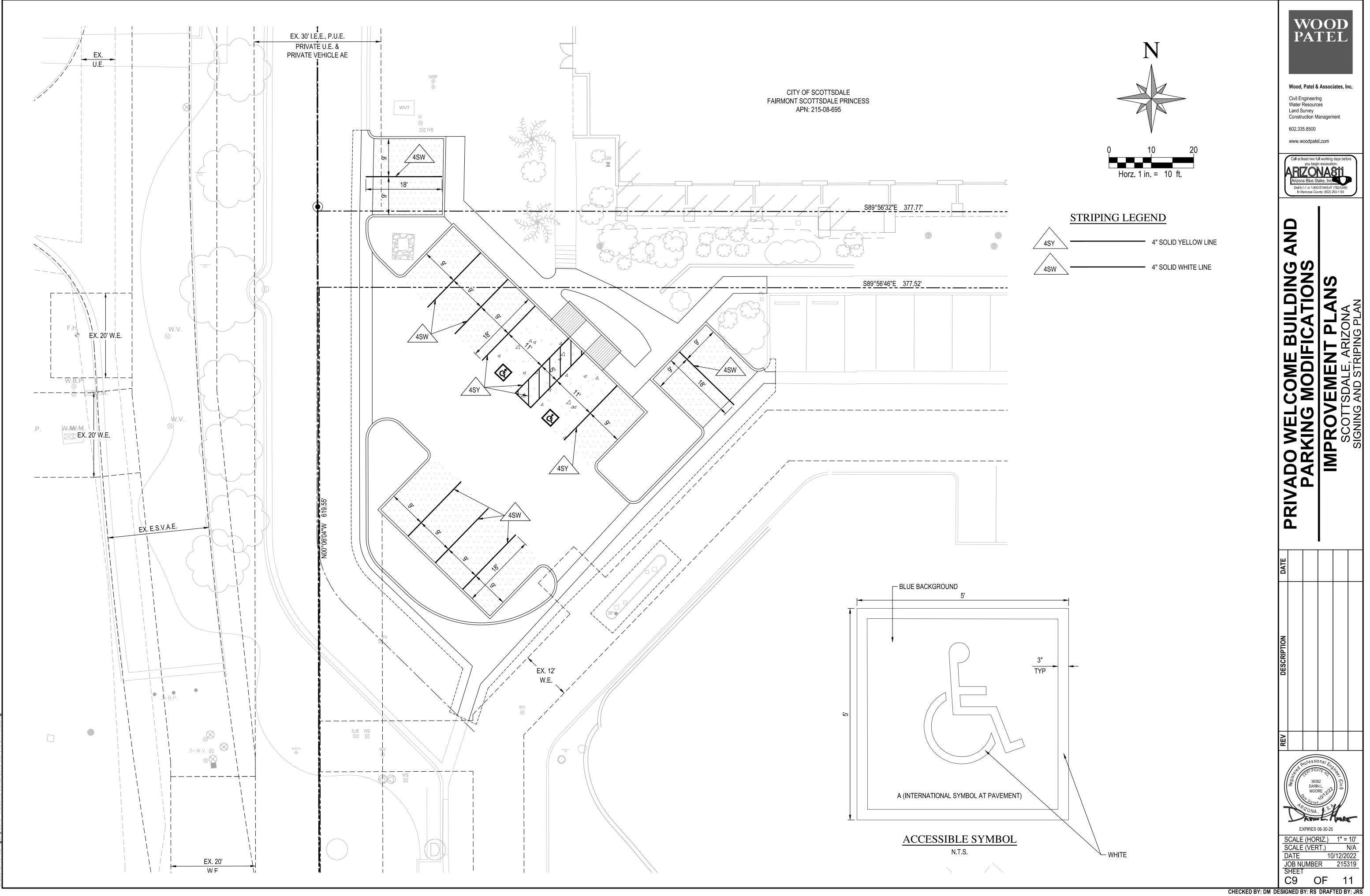
- 1 CONSTRUCT SIDEWALK PER M.A.G. STD. DET. 230. SEE LANDSCAPE PLANS FOR COLOR & FINISH.
- 2 MATCH EXISTING ELEVATIONS. CONTRACTOR TO NOTIFY ENGINEER OF ANY DISCREPANCIES.
- (3) CONSTRUCT 6" CURB & GUTTER PER M.A.G. STD. DET. 220, TYPE A.
- 4 INSTALL LIGHT DUTY PAVEMENT, 3" A.C. PAVEMENT OVER 7" A.B.C. PER GEOTECHNICAL REPORT.
- 5 INSTALL HEAVY DUTY PAVEMENT, 3" A.C. PAVEMENT OVER 11" A.B.C. PER GEOTECHNICAL REPORT.
- $\langle 6 \rangle$ CONSTRUCT CONCRETE PAVEMENT. 6" P.C.C. OVER 4" A.B.C. OVER COMPACTED SUBGRADE PER GEOTECHNICAL REPORT RECOMMENDATIONS. REFER TO GEOTECHNICAL REPORT FOR FURTHER INFORMATION.
- $\langle 7 \rangle$ CONSTRUCT 6" SINGLE CURB PER MAG STD. DET. 222, TYPE A.
- 9 MATCH NEW PAVEMENT TO EXISTING PAVEMENT AT SAWCUT LINE
- $\langle 10 \rangle$ CONSTRUCT CONCRETE APRON PER M.A.G. STD. DET. 240.
- (11) CONSTRUCT VALLEY GUTTER PER M.A.G. STD. DET. 240 MODIFIED TO A WIDTH OF 3'.
- (12) CONSTRUCT SIDEWALK RAMP PER C.O.S. STD. DETAIL 2231 AND THE DETAIL ON SHEET C11.
- (13) CONFIGURATION OF THE HANDICAP PARKING STALLS PER THE DETAIL ON SHEET C11.



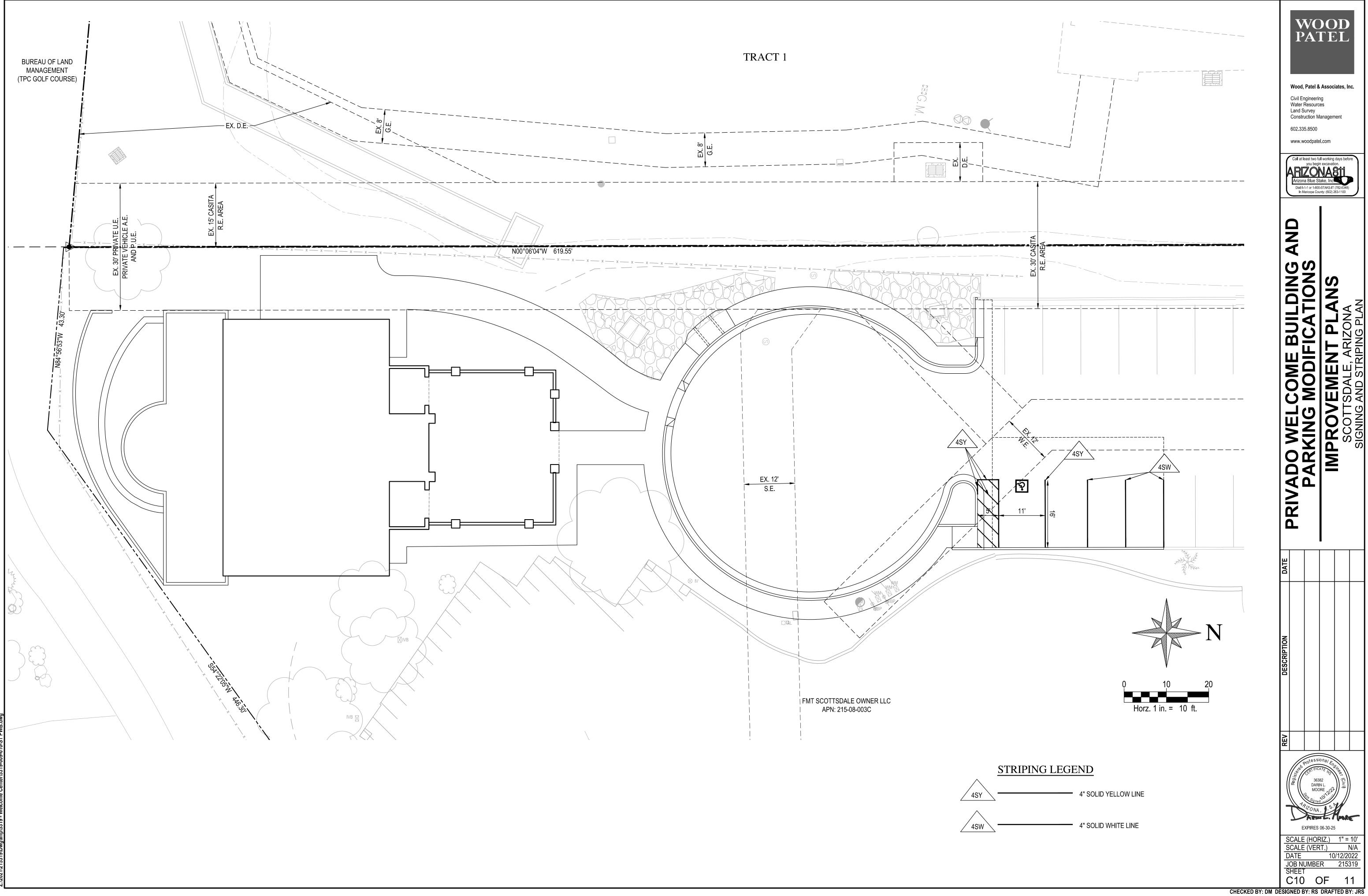


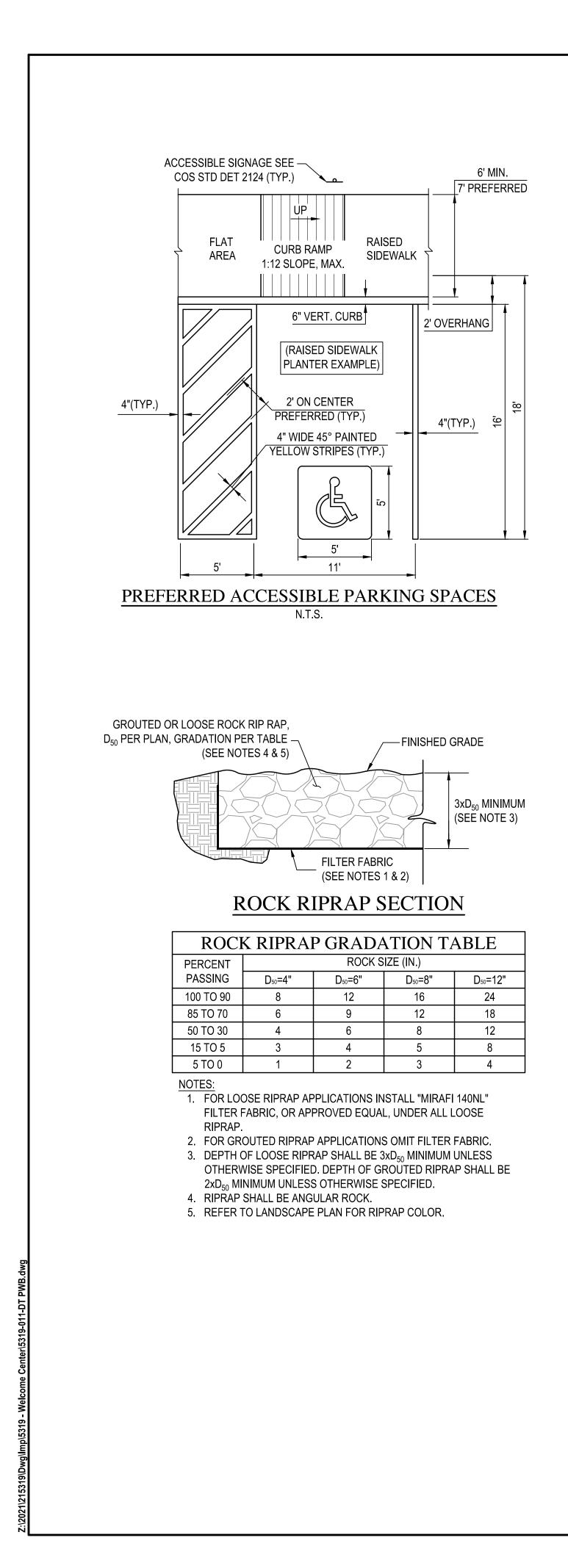


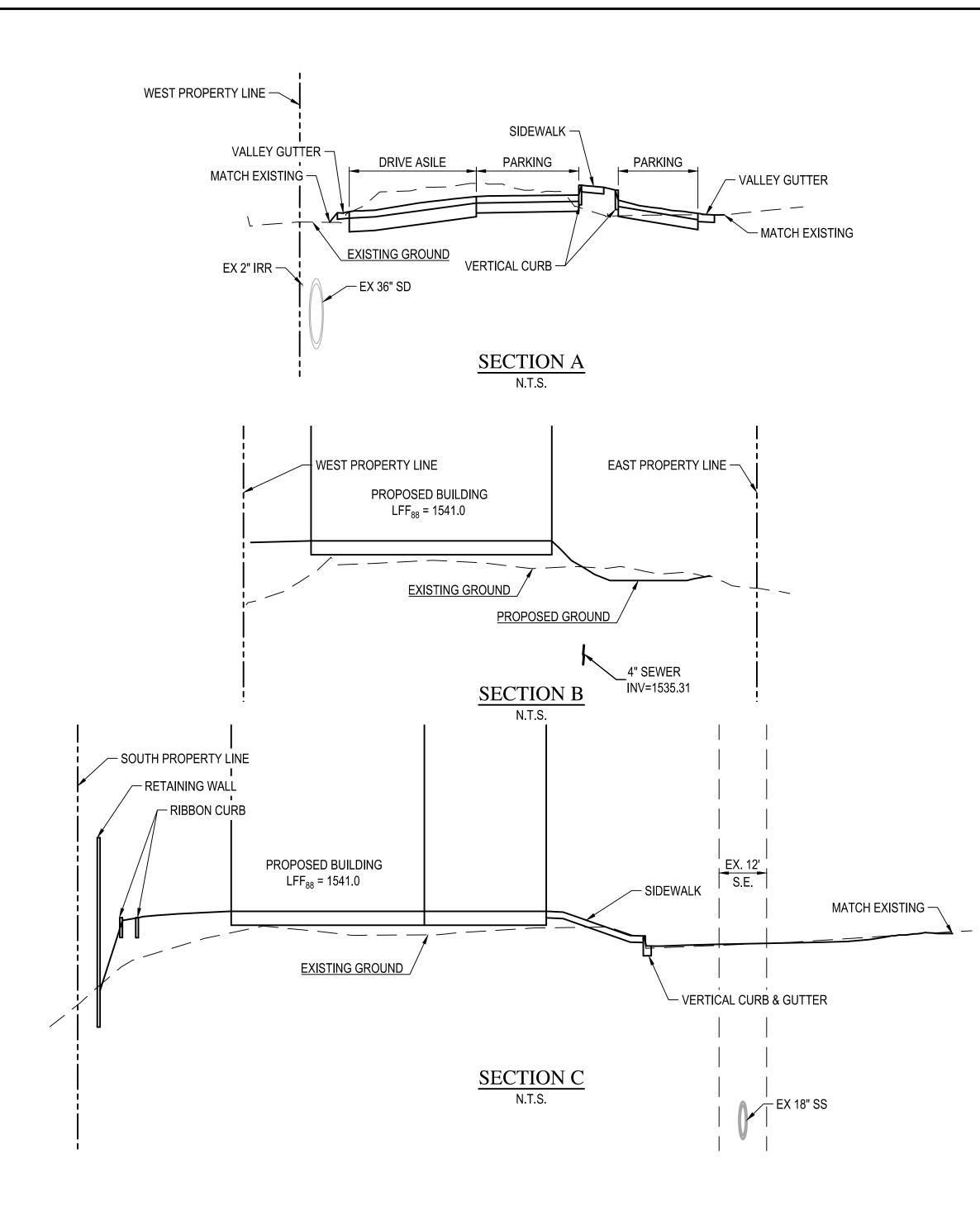
	UTILITY CROSSINGS				
	X-#	CROSSING	ELEVATION		
	X-1	WATER BOT	1536.32		
		SEWER TOP	1533.36		
	X-2	WATER BOT	1536.59		
		SEWER TOP	1533.53		
	X-3	WATER BOT	1536.42		
		SEWER TOP	1533.25		
	X-4	WATER BOT	1536.38		
		SEWER TOP	1533.22		











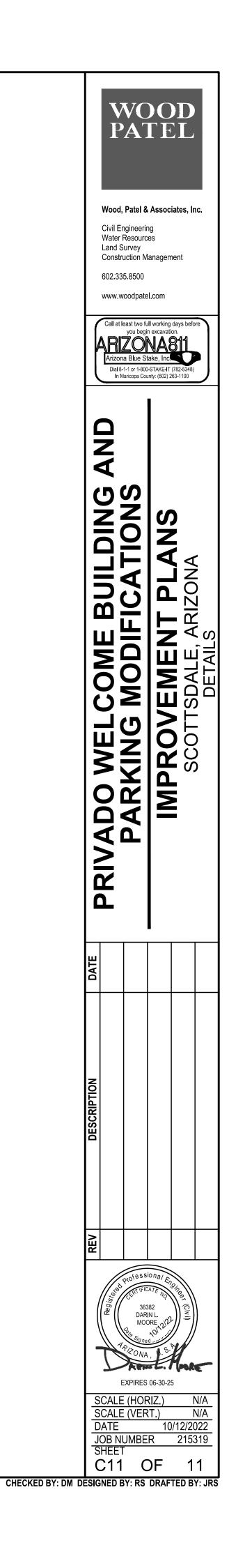


EXHIBIT 1 – VICINITY MAP

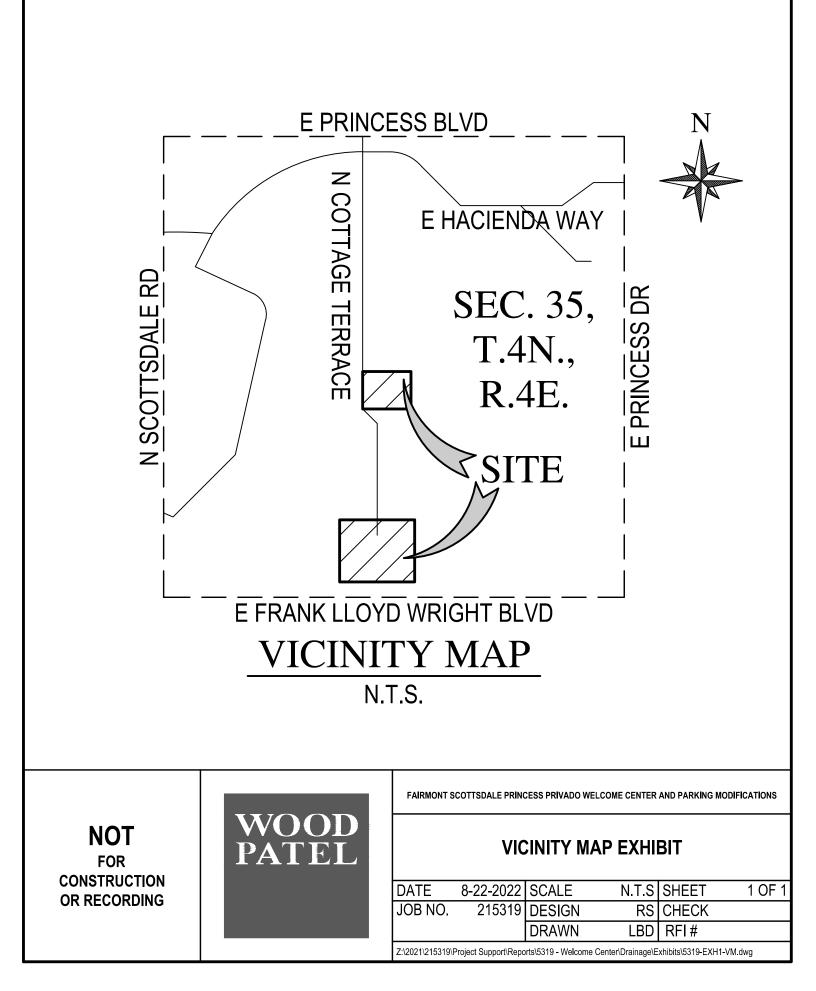
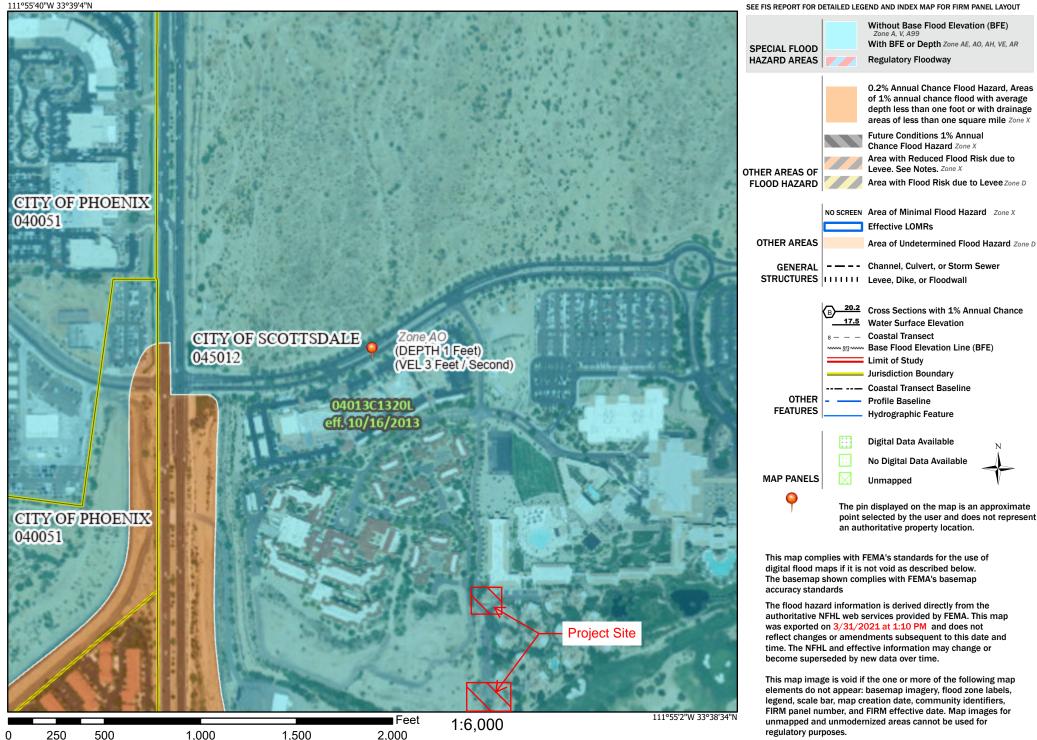


EXHIBIT 2 – FEMA FIRM

National Flood Hazard Layer FIRMette



Legend

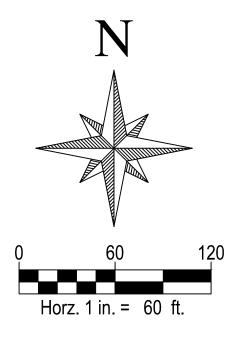


Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

EXHIBIT 3 – EXISTING DRAINAGE MAP



LEGEND		
	EXISTING DRAINAGE AREA	
1	EXISTING DRAINAGE AREA	



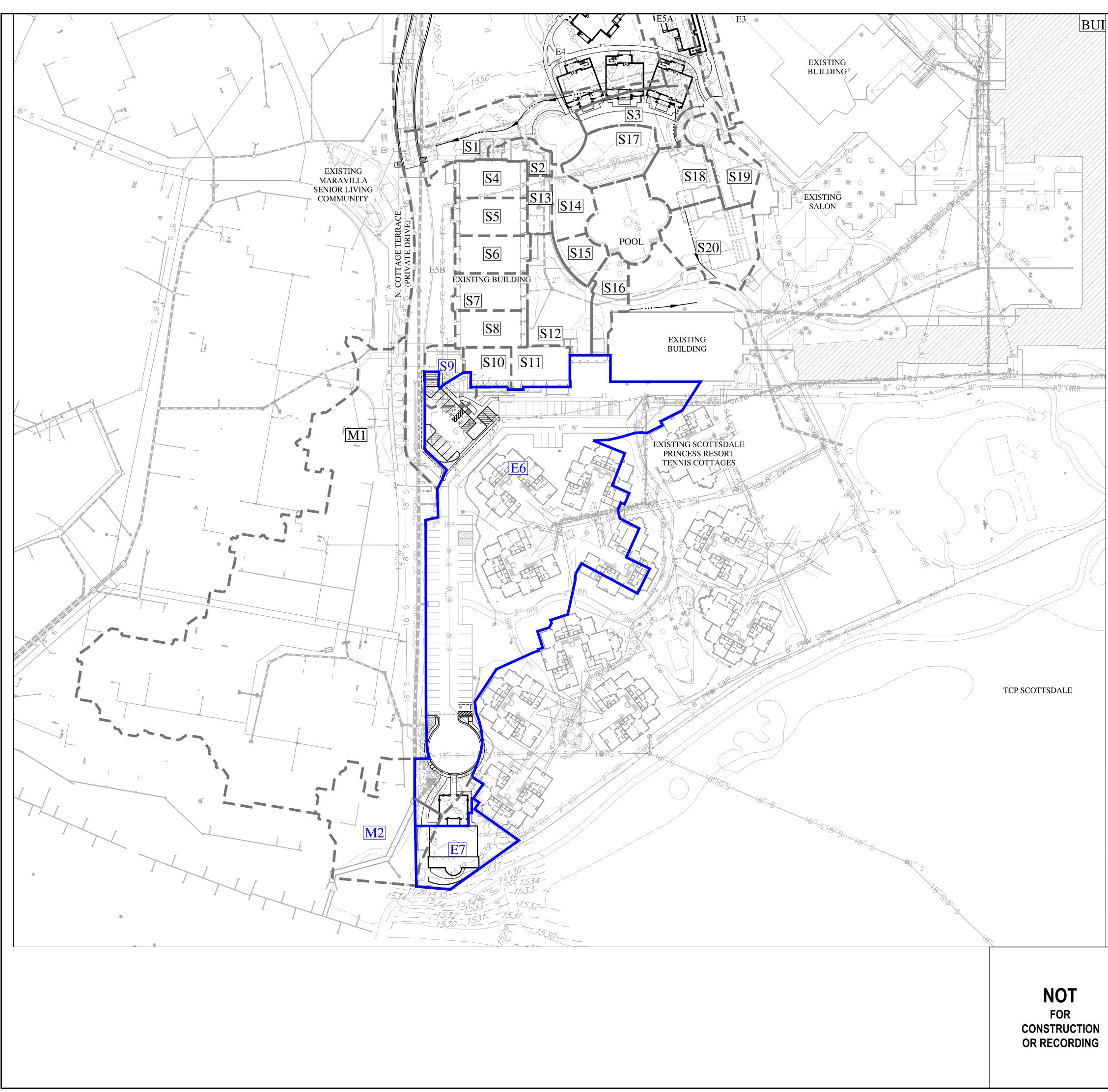
WOOD PATEL

FAIRMONT SCOTTSDALE PRINCESS PRIVADO WELCOME CENTER AND PARKING MODIFICATIONS

EXISTING DRAINAGE MAP

DATE	10/12/2022	SCALE	1" = 60'	SHEET	1 OF 1
JOB NO.	215305	DESIGN	DCN	DRAWN	MHS
Z:\2021\215319\Project Support\Reports\5319 - Welcome Center\Drainage\Exhibits\5319-EXH2- Existing Drainage Area.dwg					

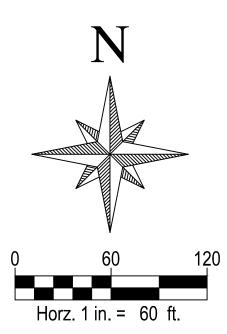
EXHIBIT 4 – PROPOSED DRAINAGE MAP



LEGEND

_	
1	
1	

PROPOSED DRAINAGE AREA EXISTING DRAINAGE AREA UPDATED DRAINAGE AREA EXISTING DRAINAGE AREA



WOOD PATEL

FAIRMONT SCOTTSDALE PRINCESS PRIVADO WELCOME CENTER AND PARKING MODIFICATIONS

PROPOSED DRAINAGE MAP

DATE	10/12/2022	SCALE	1" = 60'	SHEET	1 OF 1
JOB NO.	215305	DESIGN	DCN	DRAWN	MHS
Z:\2021\215319\Project Support\Reports\5319 - Welcome Center\Drainage\Exhibits\5319-EXH2-Drainage Area (not comp					(not completed).dwg