



Shea Boulevard

Traffic Impact & Mitigation Analysis



Prepared for:



Hawkins Companies 855 W. Broad St., Suite 300 Boise, ID 83702



Lōkahi, LLC 4657 E. Cotton Gin Loop, Suite 102 Phoenix, AZ 85004

A Professional Engine

51806 SHELLY A.

SORENSEN

, u.s

Date Signed:

Regi

Project Number: 20.5093 May 8, 2020



TABLE OF CONTENTS:

1.	Int	troduction and Executive Summary1
	1.1.	Purpose of Report and Study Objectives1
	1.2.	Executive Summary1
2.	Pr	oposed Development
3.	Ar	ea Conditions
-	3.1.	Study Roadway Segments8
	3.2.	Study Intersections
	3.3.	Surrounding Area Land Use
	3.4.	Site Accessibility
	3.5.	Collision History
	3.6.	Collision Rates
4.	Ex	isting Conditions15
	4.1.	Existing Land Use15
	4.2.	Existing Traffic Counts15
	4.3.	Existing Capacity Analysis17
5.	Pr	ojected Traffic
	5.1.	Trip Generation
	5.2.	Trip Distribution and Assignment21
6.	Fu	ture Conditions (Year 2021 – Opening Year)
	6.1.	Year 2021 Background Traffic Volumes
	6.2.	Year 2021 Build Traffic Volumes
	6.3.	Year 2021 No Build Capacity Analysis25
 1.1. Purpose of Report and Study Objectives 1.2. Executive Summary. 2. Proposed Development 3. Area Conditions 3.1. Study Roadway Segments		
7.	Re	commendations & Conclusions





FIGURES:

Figure 1 – Vicinity Map	6
Figure 2 – Site Plan	7
Figure 3 – Study Area	
Figure 4 – City of Scottsdale Trolley Mustang Route	12
Figure 5 – Existing Traffic Volumes	
Figure 6 – Existing Capacity Analysis	
Figure 7 – Trip Distribution	
Figure 8 – Site Traffic Volumes	
Figure 9 – Year 2021 Background Traffic Volumes	
Figure 10 – Year 2021 Build Traffic Volumes	
Figure 11 – Year 2021 Background Capacity Analysis	
Figure 12 – Year 2021 Build Capacity Analysis	

TABLES:

Гable 1 – Collision Rates – Study Roadway Segments 1	4
۲able 2 – Collision Rates – Study Intersections ۱	4
Гаble 3 – Level of Service Criteria1	17
Гable 4 – Existing Level of Service and Delay 1	8
able 5 – Trip Generation – Previously Approved Development 2	0
Fable 6 – Trip Generation – Proposed Development	21
Fable 7 – Trip Generation – Comparison	21
able 8 – 2021 No Build and Build Level of Service and Delay 2	27

APPENDICES:

Appendix A – Proposed Site Plan	А
Appendix B – Crash Data	В
Appendix C – Parcel Information	.С
Appendix D – Traffic Count Data	D
Appendix E – Existing Signal Timing	.E
Appendix F – Existing Capacity Analysis	.F
Appendix G – Prior Approved Site Plan	G
Appendix H – Trip Generation	Н
Appendix I – MAG Socioeconomic Projections	
Appendix J — Year 2021 No Build Capacity Analysis	J.
Appendix K – Year 2021 Build Capacity Analysis	К





1. INTRODUCTION AND EXECUTIVE SUMMARY

1.1. PURPOSE OF REPORT AND STUDY OBJECTIVES

Lōkahi, LLC (Lōkahi) was retained by Hawkins Companies to complete a Traffic Impact & Mitigation Analysis for the proposed 114th Street and Shea Boulevard development. The development is located at the southwest corner of 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard in Scottsdale, Arizona.

The objective of this Traffic Impact & Mitigation Analysis is to analyze the traffic related impacts of the proposed development to the adjacent roadway network. See **Figure 1** for the vicinity map.

The proposed site will be comprised of three (3) buildings, totaling approximately 15,336 square feet (SF) of leasable area. Approximately 8,100 square feet is anticipated to be utilized for retail use, while the remaining 7,236 square feet is anticipated for medical or general office use.

1.2. EXECUTIVE SUMMARY

This report presents the analyses and the results of a traffic study prepared for the proposed 114th Street and Shea Boulevard development that will be located on the southwest corner of 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard. The proposed development will be comprised of 8,100 square feet (SF) of retail space and 7,236 square feet of medical/general office space.

This Traffic Impact and Mitigation Analysis includes:

- Level of service analysis of existing conditions for the weekday AM and PM peak hours
- Crash Analysis
- Trip Generation for the existing and proposed development
- Trip Generation for the prior approved development and the proposed development
- Trip Generation comparison for the previously approved development with the proposed development
- Level of service analysis for the <u>no build</u> and <u>build</u> capacity analysis with the buildout of development (Year 2021) weekday AM and PM peak hours

The following are the three (3) intersections included in this study:

- 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1)
- 114th Street and Driveway B/Mirage Crossing Driveway (2)
- 114th Street and Driveway A (3)





Existing Capacity Analysis

The AM and PM peak hour existing conditions capacity analysis were completed for the two (2) existing study intersections. All movements operate at a LOS D or better with the exception of the following intersection:

114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1) – Signalized

- EB left AM and PM peak hours operate at LOS E
- NB left AM and PM peak hours operate at LOS E
- NB shared through-right AM and PM peak hours operate at LOS E
- SB left AM and PM peak hours operate at LOS F, and LOS E, respectively

Trip Generation

Previously Approved Development

In 2007 there was a prior site plan that was approved for the proposed site. The prior site included a 5,250 (SF) bank and a 5,200 (SF) office building.

Trip Generation – Prior Approved Development 🖌

1 single family lot ignored. 2 other single family lots in common not shown

Land Lico		ITE	Unit	Weekday	AM Peak Hour			PM Peak Hour		
	Code	Quy	onit	Total	Total	In	Out	Total	ln	Out
Drive-in Bank	912	5.25	1000 SF GFA	525	50	29	21	107	54	53
General Office Building	710	5.2	1000 SF GFA	60	31	27	4	7	1	6
	585	81	56	25	114	55	59			

The previously approved development is anticipated to generate 585 weekday trips, with 81 trips occurring during the AM peak hour and 114 trips during the PM peak hour.

Proposed Development

The proposed development will be comprised of 8,100 square feet (SF) of retail space and 7,236 square feet of medical/general office space. For the purposes of this report, the medical/general office building is assumed to be occupied entirely by medical office uses.

Uses weighted average rates instead of fitted curve as expected for ADT and PM. Possibly expect more trips. Overall, not a large difference.

Trip Generation – Proposed Development

Land Lico	\	ITE	Otu	Unit	Weekday	AM	Peak H	our	PM	Peak H	our
Land Ose		Code	Quy		Total	Total	In	Out	Total	In	Out
Shopping Center		820	8.1	1000 SF GLA	306	8	5	3	31	15	16
Medical-Dental Office Building		720	7.236	1000 SF GFA	190	22	17	5	27	8	19
Total						30	22	8	58	23	35





8-ZN-2020

5/22/2020

The proposed development is anticipated to generate 496 weekday trips, with 30 trips occurring during the AM peak hour and 58 trips during the PM peak hour.

Trip Generation Comparison

The build out of the proposed development requires a minor general plan amendment. A comparison between the trips generated by the prior approved development and the proposed development was calculated.

Land Lico		Othe	ty Unit	Weekday	AM	Peak H	our	PM Peak Hour		
Land Use	Code	Total		Total	In	Out	Total	In	Out	
Drive-in Bank	912	5.25	1000 SF GFA	525	50	29	21	107	54	53
General Office Building	710	5.2	1000 SF GFA	60	31	27	4	7	1	6
Previously App	roved D	evelopn	nent - Total	585	81	56	25	114	55	59
Shopping Center	820	8.1	1000 SF GLA	306	8	5	3	31	15	16
Medical-Dental Office Building	720	7.236	1000 SF GFA	190	22	17	5	27	8	19
Proposed Development - Total					30	22	8	58	23	35
Difference					-51	-34	-17	-56	-32	-24

Trip Generation – Comparison

The build out of the proposed development is anticipated to generate 89 fewer weekday trips, 51 fewer trips during the AM peak hour, and 56 fewer trips during the PM peak hour, when compared with the prior approved development.

Year 2021

Year 2021 analyses was completed <u>with</u> and <u>without</u> the build out of the proposed development. An annual growth rate of 1.2% was applied to the existing traffic volumes.

A capacity analysis was completed for both the AM and PM peak hours for year 2021, <u>with</u> and <u>without</u> the build out of the proposed development. All movements are maintained at the existing level of service <u>without</u> the build out of the proposed development.

All movements operate at a LOS D or better or are maintained at the no build level of service with the build out of the proposed development.

Pacommondations Carry over stipulations from prior case

Recommendations

The recommendations with the build out of the proposed development include:

• 114th Street and Driveway B (2)

Buildout of full access driveway, approximately 350 feet south of Shea Boulevard





• 114th Street and Driveway A (3)

Buildout of right-in/right-out driveway, approximately 215 feet south of Shea Boulevard

As with any new development and potential change in traffic patterns, the following is recommended:

• Monitor and Adjust Signal Timing Monitor traffic patterns in the area and if necessary, adjust nearby signal timing





2. PROPOSED DEVELOPMENT

The study area is located in the City of Scottsdale, Arizona. The proposed development will be located on the southwest corner of 114th Street and Shea Boulevard. See **Figure 1** for a vicinity map.

The proposed site will be comprised of three (3) buildings, totaling approximately 15,336 square feet (SF) of leasable area. Approximately 8,100 square feet is anticipated to be utilized for retail use, while the remaining 7,236 square feet is anticipated for medical or general office use.

There are two (2) proposed access points to the development along 114th Street evaluated in this study.

114th Street and Driveway B/Mirage Crossing Driveway (2) is located along 114th Street, approximately 350 feet south of Shea Boulevard. This will be a full access driveway, allowing all movements into and out of the site. The buildout of this driveway will form the western leg of the existing 114th Street and Mirage Crossing Driveway intersection.

114th Street and Driveway A (3) is located along 114th Street approximately 215 feet south of Shea Boulevard. This will be a right-in/right-out driveway.

See Figure 2 and Appendix A for the proposed site plan.



PROPOSED SITE



FIGURE 1 | VICINITY MAP









3. AREA CONDITIONS

The study area is located in the City of Scottsdale, Arizona. **Sections 3.1** and **3.2** provide detailed descriptions of the study roadway segments and intersections. See **Figure 3** for study area.

3.1. STUDY ROADWAY SEGMENTS

Shea Boulevard, bordering the proposed development to the north, runs east-west and provides three (3) through lanes for each direction of travel, with a raised landscaped median. There is a posted speed limit of 50 miles per hour (mph), within the vicinity area. The City of Scottsdale classifies Shea Boulevard as a major arterial, according to the *City of Scottsdale Transportation Master Plan*, dated July 5, 2016. The City of Scottsdale's *2018 Average Daily Segment Traffic Volumes* map reports an ADT of 39,700 vehicles per day, between Via Linda and 114th Street, and an ADT of 44,000 vehicles per day, between 114th Street/Frank Lloyd Wright Boulevard and 124th Street.

114th Street is generally a north-south roadway, bordering the proposed development to the east. South of Shea Boulevard, 114th Street provides one (1) through lane for northbound travel and approximately 300 feet of two (2) through lanes that taper into one (1) for southbound travel. North of Shea Boulevard is 114th Street becomes Frank Lloyd Wright Boulevard and provides two (2) through lanes in each direction of travel, with a raised landscaped median. Approximately 800 feet south of Shea Boulevard, 114th Street becomes Cochise Drive. There is a posted speed limit of 30 mph south of Shea Boulevard and a posted speed limit of 40 mph north of Shea Boulevard. According to the *City of Scottsdale Transportation Master Plan*, dated July 5, 2016, 114th Street is classified as a minor collector, while Frank Lloyd Wright Boulevard is classified as a minor arterial. The City of Scottsdale's 2018 Average Daily Segment Traffic Volumes map reports an ADT of 20,500 vehicles per day, north of Shea Boulevard, and an ADT of 2,800 vehicles per day, south of Shea Boulevard.

3.2. STUDY INTERSECTIONS

114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1) currently operates as a signalized intersection. The northbound approach provides two (2) dedicated left turn lanes and one (1) shared through-right turn lane. The southbound approach provides two (2) dedicated left turn lanes, two (2) through lanes, and one (1) dedicated right turn lane. The eastbound approach provides two (2) dedicated left turn lanes, two (2) through lanes, and one (1) dedicated right turn lane. The eastbound approach provides two (2) dedicated left turn lanes, two (2) through lanes, and one (1) dedicated left turn lane, through-right turn lane. The westbound approach provides one (1) dedicated left turn lane, three (3) through lanes, and one (1) channelized right turn lane.

114th Street and Driveway B/Mirage Crossing Driveway (2) currently operates as a one-way stopcontrolled T-intersection, with stop control on the westbound approach. The northbound approach provides one (1) shared through-right turn lane. The southbound approach provides one (1)





dedicated left turn lane and two (2) through lanes. The westbound approach provides one (1) shared left-right turn lane.







Intersection

FIGURE 3 |STUDY AREA





3.3. SURROUNDING AREA LAND USE

The approximate 3.8-acre site is bordered by Shea Boulevard to the north, and 114th Street to the east. Residential developments border the western and southern edges of the proposed development. Additionally, offices and commercial developments are located to the east, across 114th Street, as well as on the northeast corner of 114th Street and Shea Boulevard (1).

3.4. SITE ACCESSIBILITY

Roadway System

The study area is located in the City of Scottsdale, Arizona. Arizona State Route 101 is located approximately three and one-quarter mile west of the proposed development. Scottsdale's street network is generally built as a one-mile grid system. Within the near vicinity of the proposed site there is a well-developed roadway network.

Pedestrian Facilities

There are continuous sidewalks provided adjacent to 114th Street and Shea Boulevard, with the exception of an approximate 900 feet segment along the south side of Shea Boulevard, between 112th Street and 114th Street, where no sidewalk is currently provided. Marked crosswalks are provided at all four (4) legs of the signalized intersection of 114th Street and Shea Boulevard (1).

Bicycle Facilities, Trails and Pathways

Bike lanes are currently provided along 114th Street, south of Shea Boulevard in both directions of travel. An unpaved trail exists on the south side of Shea Boulevard, east of 114th Street. West of

Transit Facilities

An unpaved trail exists on the south side of Shea Boulevard, east of 114th Street. West o 114th Street, a shared use path and an unpaved path are planned. The parcels are stipulated to dedicate ROW and construct the paths.

Valley Metro Express Route 514 operates along Shea Boulevard within the study area. This route connects Fountain Hills to Downtown Phoenix. According to the Valley Metro System Map, a stop is located at on the northeast corner of the intersection of 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1). This route operates Monday – Friday.

In addition, the City of Scottsdale provides five trolley routes. The Mustang Route (MSTG) circulates around the north Scottsdale area, including along Via Linda and Frank Lloyd Wright Boulevard. A trolley stop is located approximately one-half mile north of the proposed development at the intersection of Via Linda and Frank Lloyd Wright Boulevard. This trolley route operates every 20 minutes between 4:42 am and 11:15 pm Monday – Friday and 5:13 am – 10:45 pm Saturday and Sunday. See **Figure 4**.





Hawkins Companies Traffic Impact & Mitigation Analysis



Figure 4 – City of Scottsdale Trolley Mustang Route





3.5. COLLISION HISTORY

The most recent 3-year collision history, from March 2017 to February 2020, was obtained from the City of Scottsdale. See **Appendix B** for collision data. The data included the following intersection and segment:

- 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1)
- Shea Boulevard, between 112th Street and 116th Street

114th Street and Shea Boulevard (1)

During the three-year period, there were a total of seven (7) crashes, of which four (4) were reported as no injury, and three (3) were either not reported or unknown. There was a total of five (5) rear end, one (1) sideswipe same direction, and one (1) single vehicle crashes.

Of the seven (7) reported collisions, four (4) collisions involved vehicles speeding too fast for roadway conditions and colliding with a vehicle slowing or stopping in the roadway, one (1) involved an improper turn of a vehicle, and 2 collisions involved unknown violations.

Shea Boulevard, between 112th Street and 116th Street

During the three-year period, there were a total of four (4) crashes, of which one (1) reported possible injury, one (1) reported no injury, and two (2) were either not reported or unknown. All four (4) were reported as rear end crashes.

Of the four (4) reported collisions, all involved vehicles that were speeding too fast for roadway conditions and colliding with a vehicle slowing or stopping in the roadway.

3.6. COLLISION RATES

The City of Scottsdale's 2018 Traffic Volume and Collision Rate Data report provides collision rate and traffic volume information on major roadway segments and at major intersections within the City. Segment collisions are collisions that occur on a major street more than 100 feet from the major intersections that define the segment, including at minor intersections within the segment. Intersection collisions are collisions that occur at or within 100 feet of a major intersection.

The collision rates and city-wide rankings for the study roadway segments are shown in **Table 1**. The collision rates and city-wide rankings for the study intersections are shown in **Table 2**.





Table 1 – Collision Rates – Study Roadway Segments

Segment	From	То	Collision Rate	Rank				
Shea Boulevard	Via Linda	Frank Lloyd Wright Boulevard	0.45	237				
Shea Boulevard	Frank Lloyd Wright Boulevard	124th Street	0.45	239				
114th/117th Street	Mountain View Road	Shea Boulevard	0.0	307				
2018 City c	2018 City of Scottsdale Average Segment Collision Rate							

Table 2 – Collision Rates – Study Intersections

Intersection	Collision Rate	Rank
Frank Lloyd Wright and Shea	0.78	54
2018 Average Intersection Collision Rate	0.58	





4. EXISTING CONDITIONS

4.1. EXISTING LAND USE

The approximate 3.8-acre site is currently undeveloped land and comprised of four (4) parcels. The existing zoning consists of approximately 1.50-acres of Commercial Office (C-0) and approximately 2.57-acres of Single-Family Residential (R1-18). Commercial Office (C-0) zoning accommodates the development of office and related uses adjacent to commercial areas. Single-Family Residential (R1-18) zoning accommodates the development of individual homes on lot sizes such that a low-density population is maintained.

See **Appendix C** for detailed parcel information.

4.2. EXISTING TRAFFIC COUNTS

A local data collection firm, Field Data Services of Arizona, Inc., was utilized to collect traffic counts. On Wednesday, March 4, 2020 turning movement counts were obtained from 7:00 to 9:00 am and from 4:00 to 6:00 pm at the following intersections:

- 114th Street and Shea Boulevard (1)
- 114th Street and Driveway B/Mirage Crossing Driveway (2)

Additionally, on Wednesday, March 4, 2020, bi-directional tube counts for 24-hours in 15-minute intervals were collected along the following two (2) roadway segments:

- Shea Boulevard, west of 114th Street
- 114th Street, south of Shea Boulevard

The turning movement counts were analyzed for the highest 1-hour within each time period. These are the peak hours that were analyzed throughout this study.

- AM Peak Hour 7:00 am 8:00 am
- PM Peak Hour 4:30 pm 5:30 pm

The City of Scottsdale seasonal adjustment factors were used to adjust the traffic counts. The traffic volumes were adjusted based on the month the counts were taken. See **Appendix D** for detailed count data. See **Figure 5** for the existing adjusted AM and PM peak hour weekday traffic volumes.









Intersection

Peak Hour Traffic Volumes

<ADT> Average Daily Traffic





FIGURE 5 | EXISTING TRAFFIC VOLUMES



4.3. EXISTING CAPACITY ANALYSIS

The existing conditions capacity analysis was completed for the existing study intersections. The capacity and level of service for the study area intersections were evaluated using the methodology presented in the 6th Edition of the Highway Capacity Manual. Traffic analysis software, Synchro Version 10.3, was used to perform the analyses using the existing Peak Hour Factor (PHF) obtained from the traffic counts. The existing signal timing was provided by the City of Scottsdale. See **Appendix E** for the existing signal timing.

Table 3 is from the 6th Edition of the Highway Capacity Manual Exhibit 20-2, which lists the Level of Service (LOS) thresholds for signalized and stop-controlled intersections.

Loval of Samica	Control Delay per Vehicle (s/veh)								
Level of Service	Signalized Intersections	Unsignalized Intersections							
А	≤ 10	0 - 10							
В	> 10-20	> 10–15							
С	> 20-35	> 15-25							
D	> 35-55	> 25-35							
E	> 55-80	> 35-50							
F	> 80	> 50							

Table 3 – Level of Service Criteria

The results of the capacity analyses reveal the following locations with an existing level of service (LOS) E or F:

114th Street and Shea Boulevard (1) – Signalized

- EB left AM and PM peak hours operate at LOS E
- NB left AM and PM peak hours operate at LOS E
- NB shared through-right AM and PM peak hours operate at LOS E
- SB left AM and PM peak hours operate at LOS F and E, respectively

The existing AM and PM peak hour level of service and delay for unsignalized and signalized intersections are shown in **Table 4**.

See **Figure 6** for the existing AM and PM peak hour capacity analysis. The detailed capacity analysis sheets can be found in **Appendix F.**





	Existing Conditions						
Intersection	AM F	PEAK	PM PEAK				
	LOS	DELAY	LOS	DELAY			
114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1) - Signalized							
Oveall Intersection	D	50.4	D	42.1			
Eastbound Left	E	62.0	E	63.2			
Eastbound Through	C	26.7	D	37.2			
Eastbound Shared Through-Right	C	27.9	D	41.4			
Westbound Left	D	41.3	D	51.5			
Westbound Thorugh	В	18.9	С	26.6			
Westbound Right	-	-	-	-			
Northbound Left	E	56.6	E	56.2			
Northbound Shared Through-Right	E	58.0	E	66.5			
Southbound Left	F	205.4	E	59.7			
Southbound Through	D	42.7	С	32.1			
Southbound Right	D	47.6	С	33.6			
114th Street and Mirage Crossing Driveway (2	2) - Unsign	alized					
Westbound Shared Left-Right	А	9.2	А	9.0			
Southbound Left	А	7.6	А	7.5			

Table 4 – Existing Level of Service and Delay





AM(PM)

Intersection

Lane Configuration

Peak Hour Capacity Analysis





FIGURE 6 | EXISTING CAPACITY ANALYSIS





5. PROJECTED TRAFFIC

5.1. TRIP GENERATION

The trip generation for the proposed development was calculated utilizing the Institute of Institute of Transportation Engineers (ITE) publication entitled *Trip Generation*, 10th Edition. The ITE trip generation rates and fitted curve equations are based on studies that measure trip generation characteristics for various types of lane uses. The rates are expressed in terms of trips per unit of lane use type. This publication is the standard for the transportation engineering profession.

Previously Approved Development

In 2007 there was a prior site plan that was approved for the proposed site. See the site plan in **Appendix G**. The prior site included a 5,250 (SF) bank and a 5,200 (SF) office building. The trip generation for the prior approved development was calculated utilizing ITE Land Use 912 – Drive-In Bank and ITE Land Use Code 710 – General Office Building. Trips generated by the Drive-In Bank, with 5,250 (SF) and General Office Building, with 5,200 (SF) are shown in **Table 5** below. Detailed trip generation calculations are provided in **Appendix H**.

Land Use		Qty	Unit	Weekday	AM Peak Hour			PM Peak Hour		
				Total	Total	ln	Out	Total	In	Out
Drive-in Bank	912	5.25	1000 SF GFA	525	50	29	21	107	54	53
General Office Building	710	5.2	1000 SF GFA	60	31	27	4	7	1	6
	585	81	56	25	114	55	59			

Table 5 – Trip Generation – Previously Approved Development

The previously approved development is anticipated to generate 585 weekday trips, with 81 trips occurring during the AM peak hour and 114 trips during the PM peak hour.

Proposed Development

The proposed site will be comprised of three (3) buildings, totaling approximately 15,336 square feet (SF) of leasable area. Approximately 8,100 square feet is anticipated to be utilized for retail use, while the remaining 7,236 square feet is anticipated for medical or general office use.

Therefore, the trip generation for the proposed office development was calculated utilizing ITE Land Use 720 – Medical-Dental Office Building and Land Use 850 – Shopping Center. Trips generated by the three (3) proposed buildings, comprised of 15,336 (SF), are shown in **Table 6** below. Detailed trip generation calculations are provided in **Appendix H**.





Table 6 – Trip Generation – Proposed Development

Land Use	ITE	Unit	Weekday	AM Peak Hour			PM Peak Hour			
	Code	QUY	Unit	Total	Total	In	Out	Total	ln	Out
Shopping Center	820	8.1	1000 SF GLA	306	8	5	3	31	15	16
Medical-Dental Office Building	720	7.236	1000 SF GFA	190	22	17	5	27	8	19
			Total	496	30	22	8	58	23	35

The proposed development is anticipated to generate 496 weekday trips, with 30 trips occurring during the AM peak hour and 58 trips during the PM peak hour.

Trip Generation Comparison

The build out of the proposed development requires a minor general plan amendment. A trip generation comparison between the prior approved development and proposed development is shown in **Table 7** below, in order to demonstrate the trip generation difference.

Table 7 – Trip Generation – Comparison

Land Use	ITE	Qty	Unit	Weekday	AM	l Peak Hour		PM Peak Hour		our
	Code			Total	Total	In	Out	Total	In	Out
Drive-in Bank	912	5.25	1000 SF	525	50	29	21	107	54	53
	-		GFA			-				
General Office Building	710	5.2	1000 SF GFA	60	31	27	4	7	1	6
Previously Approved Development - Total					81	56	25	114	55	59
Shopping Center	820	8.1	1000 SF GLA	306	8	5	3	31	15	16
Medical-Dental Office Building	720	7.236	1000 SF GFA	190	22	17	5	27	8	19
Proposed Development - Total				496	30	22	8	58	23	35
Difference				-89	-51	-34	-17	-56	-32	-24

The build out of the proposed development is anticipated to generate 89 fewer weekday trips, 51 fewer trips during the AM peak hour, and 56 fewer trips during the PM peak hour, when compared with the prior approved development.

5.2. TRIP DISTRIBUTION AND ASSIGNMENT

The trip distribution procedure determines the general pattern of travel for vehicles entering and leaving the proposed development. The trip distribution for the proposed development is based on the distribution of the existing traffic. The trip distribution is shown in **Figure 7**.





The trip assignment was generally based on proximity of the driveways, permitted turn movements, as well as ease and probability of use. The site generated traffic volumes are shown in **Figure 8**.





AM(PM) Inbound Trip Distribution Percentages

AM(PM) Outbound Trip Distribution Percentages

FIGURE 7 | TRIP DISTRIBUTION



AM(PM)

Intersection

Peak Hour Traffic Volumes

<ADT> Average Daily Traffic







FIGURE 8 | SITE TRAFFIC VOLUMES





6. FUTURE CONDITIONS (YEAR 2021 – OPENING YEAR)

The proposed 114th Street and Shea development is anticipated to be constructed and ready to open in the year 2021. This section analyzes the effects the proposed development will have on the surrounding roadway network during the opening year of 2021.

6.1. YEAR 2021 BACKGROUND TRAFFIC VOLUMES

According to the 2019 Maricopa Associations of Governments (MAG) socioeconomic projections in the City of Scottsdale within the study area, it is estimated that in the year 2018 the population will be approximately 20,900. MAG estimates that the 2020 population of the surrounding area to be 21,400. This results in an approximate annual growth rate of 1.19%.

A 1.2% annual growth rate was utilized. See **Appendix I** for the MAG socioeconomic projections.

See **Figure 9** for the year 2021 background traffic volumes, which includes a 1.2% annual growth applied to the existing traffic volumes.

6.2. YEAR 2021 BUILD TRAFFIC VOLUMES

When the site traffic (**Figure 8**) is added to the year 2021 background traffic (**Figure 9**), the result is the 2021 <u>build</u> traffic volumes. This represents the traffic volumes <u>with</u> the build out of the proposed development. The year 2021 <u>build</u> traffic volumes are shown in **Figure 10**.

6.3. YEAR 2021 NO BUILD CAPACITY ANALYSIS

The capacity and level of service for the study area intersections were evaluated for the year 2021 <u>no build</u> scenario. The corresponding traffic volumes are shown in **Figure 9**. The signal timing splits were optimized and adjusted for the future traffic volumes. PHF was assumed to be 0.92.

The detailed capacity analysis sheets can be found in **Appendix J**.

The 2021 <u>no build</u> AM and PM peak hour level of service and delay for unsignalized and signalized intersections are shown in **Table 8**.

The results of the 2021 <u>no build</u> capacity analysis level of service is shown in **Figure 11.** All movements operate at a LOS D or better or at the same level of service as the existing conditions.





6.4. YEAR 2021 BUILD CAPACITY ANALYSIS

The capacity and level of service for the study area intersections were evaluated for the year 2021 <u>build</u> traffic volumes. See **Figure 10**. The signal timing splits were optimized and adjusted for the future traffic volumes, and a PHF of 0.92 was used.

The capacity and level of service for the study area intersections were evaluated for the year 2021 <u>build</u> traffic volumes.

The results of the 2021 <u>build</u> capacity analysis level of service is shown in **Figure 12**. All movements operate at a LOS D or better or at the same level of service as the 2021 <u>no build</u> capacity analysis. The *HCM* 6th *Edition* does not give any level of service results for the u-turns that occur at the southbound left turn lane at the intersection of 114th Street and Mirage Crossing Driveway/Driveway B (2). The result for that movement were taken from the *HCM* 2000 analysis results.

The 2021 <u>build</u> AM and PM peak hour level of service and delay for unsignalized and signalized intersections are shown in **Table 8**.

See **Figure 12** year 2021 AM and PM peak hour capacity analysis. The detailed capacity analysis sheets can be found in **Appendix K**.





Table 8 – 2021 No Build and Build Level of Service and Delay

	2021	Backgrou	ind Condi	tions	2021 Build Conditions				
Intersection	AM I	PEAK	PM I	PEAK	AM	PEAK	PM PEAK		
	LOS	DELAY	LOS	DELAY	LOS	DELAY	LOS	DELAY	
114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard (1) - Signalized									
Oveall Intersection	D	53.7	D	45.0	D	53.7	D	48.0	
Eastbound Left	Е	62.9	E	64.9	E	62.9	Е	64.9	
Eastbound Through	С	27.0	D	40.8	С	27.1	D	43.6	
Eastbound Right	С	28.3	D	46.4	С	28.4	D	50.2	
Westbound Left	D	41.6	D	53.6	D	42.3	D	54.1	
Westbound Thorugh	В	19.7	С	29.1	C	20.0	С	30.0	
Westbound Right	-	-	-	-	-	-	-	-	
Northbound Left	E	56.8	E	56.3	E	56.9	E	56.6	
Northbound Right	E	57.8	E	70.0	Е	57.7	E	73.0	
Southbound Left	F	225.7	Е	62.2	F	225.7	Е	68.5	
Southbound Through	D	42.5	С	30.7	D	42.2	С	29.9	
Southbound Right	D	47.6	С	32.2	D	47.2	С	31.4	
114th Street and Mirage Crossing Driveway (2) - Unsign	alized							
Eastbound Shared Left-Through-Right	-	-	-	-	В	10.1	В	10.3	
Westbound Shared Left-Right	А	9.2	А	9.0	-	-	-	-	
Westbound Shared Left-Through-Right	-	-	-	-	А	9.3	А	9.1	
Northbound Left	-	-	-	-	А	7.3	А	7.5	
Southbound Left*	А	7.6	А	7.5	А	7.6	А	7.5	
114th Street and Driveway A (3) - Unsignalized	d								
Eastbound Right	-	-	-	-	А	0.0	А	8.8	

*Results from HCM 2000, Not HCM 6th Edition





AM(PM)

Intersection

Peak Hour Traffic Volumes

<ADT> Average Daily Traffic





FIGURE 9 | YEAR 2021 BACKGROUND TRAFFIC VOLU



AM(PM)

Intersection

Peak Hour Traffic Volumes

<ADT> Average Daily Traffic







FIGURE 10 | YEAR 2021 BUILD TRAFFIC VOLUME!



÷

AM(PM) Peak Hour Capacity Analysis
Intersection

Lane Configuration





FIGURE 11 | YEAR 2021 BACKGROUND CAPACITY ANAL'





Intersection

Lane Configuration







*LOS from HCM 2000 Results

8-ZN-2020

5/22/2020

FIGURE 12 | YEAR 2021 BUILD CAPACITY ANAL'



7. RECOMMENDATIONS & CONCLUSIONS

The proposed 114th Street and Shea Boulevard development will be located on the southwest corner of 114th Street/Frank Lloyd Wright Boulevard and Shea Boulevard, in Scottsdale, Arizona. The proposed site will be comprised of three (3) buildings, totaling approximately 15,336 square feet (SF) of leasable area. Approximately 8,100 square feet is anticipated to be utilized for retail use, while the remaining 7,236 square feet is anticipated for medical or general office use.

Existing Capacity Analysis

The AM and PM peak hour existing conditions capacity analysis were completed for the two (2) existing study intersections. All intersections currently operate with movements at a level of service D or better with the exception of the following intersection:

114th Street and Shea Boulevard (1) – Signalized

- EB left AM and PM peak hours operate at LOS E
- NB left AM and PM peak hours operate at LOS E
- NB shared through-right AM and PM peak hours operate at LOS E
- SB left AM and PM peak hours operate at LOS F, and LOS E, respectively

Trip Generation

Previously Approved Development

In 2007 there was a prior site plan that was approved for the proposed site. The prior site included a 5,250 (SF) bank and a 5,200 (SF) office building.

Land Use	ITE	Otv	Unit	Weekday	AM Peak Hour			PM Peak Hour		
	Code	Quy		Total	Total	ln	Out	Total	ln	Out
Drive-in Bank	912	5.25	1000 SF GFA	525	50	29	21	107	54	53
General Office Building	710	5.2	1000 SF GFA	60	31	27	4	7	1	6
			Total	585	81	56	25	114	55	50

Trip Generation – Prior Approved Development

The previously approved development is anticipated to generate 585 weekday trips, with 81 trips occurring during the AM peak hour and 114 trips during the PM peak hour.

Proposed Development

The proposed development will be comprised of 8,100 square feet (SF) of retail space and 7,236 square feet of medical/general office space. For the purposes of this report, the medical/general office building is assumed to be occupied entirely by medical office uses.




Trip Generation – Proposed Development

Lond Liza		Othi	Unit	Weekday	AM	AM Peak Hour			PM Peak Hour		
Land Use	Code	QU	Onic	Total	Total	In	Out	Total	In	Out	
Shopping Center	820	8.1	1000 SF GLA	306	8	5	3	31	15	16	
Medical-Dental Office Building	720	7.236	1000 SF GFA	190	22	17	5	27	8	19	
			Total	496	30	22	8	58	23	35	

The proposed development is anticipated to generate 496 weekday trips, with 30 trips occurring during the AM peak hour and 58 trips during the PM peak hour.

Trip Generation Comparison

The build out of the proposed development requires a minor general plan amendment. A comparison between the trips generated by the prior approved development and the proposed development was calculated.

Trip Generation – Comparison

LandLico		Otv	Unit	Weekday	AM Peak Hour			PM Peak Hour		
Land Use	Code	Quy	Unit	Total	Total	In	Out	Total	ln	Out
Drive-in Bank	912	5.25	1000 SF GFA	525	50	29	21	107	54	53
General Office Building	710	5.2	1000 SF GFA	60	31	27	4	7	1	6
Previously App	roved D	evelopr	nent - Total	585	81	56	25	114	55	59
Shopping Center	820	8.1	1000 SF GLA	306	8	5	3	31	15	16
Medical-Dental Office Building		7.236	1000 SF GFA	190	22	17	5	27	8	19
Proposed Development - Total					30	22	8	58	23	35
Difference					-51	-34	-17	-56	-32	-24

The build out of the proposed development is anticipated to generate 89 fewer weekday trips, 51 fewer trips during the AM peak hour, and 56 fewer trips during the PM peak hour, when compared with the prior approved development.

Year 2021

Year 2021 analyses was completed <u>with</u> and <u>without</u> the build out of the proposed development. An annual growth rate of 1.2% was applied to the existing traffic volumes.





A capacity analysis was completed for both the AM and PM peak hours for year 2021, <u>with</u> and <u>without</u> the build out of the proposed development. All movements are maintained at the existing level of service <u>without</u> the build out of the proposed development.

All movements operate at a LOS D or better or are maintained at the no build level of service with the build out of the proposed development.

Recommendations

In summary and as included in the discussion and analyses throughout this report, the following are the recommended improvements:

- 114th Street and Driveway B (2) Buildout of full access driveway, approximately 350 feet south of Shea Boulevard
- **114th Street and Driveway A (3)** Buildout of right-in/right-out driveway, approximately 215 feet south of Shea Boulevard

As with any new development and potential change in traffic patterns, the following is recommended:

• Monitor and Adjust Signal Timing Monitor traffic patterns in the area and if necessary, adjust nearby signal timing





Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix A – Proposed Site Plan







THIS SITE PLAN HAS BEEN PREPARED WITHOUT THE AID OF A SURVEY. ALL PROPERTY BOUNDARIES ARE ROUGH APPROXIMATIONS. THIS DRAWING IS TO BE USED FOR CONCEPTUAL PURPOSES ONLY AND IT IS NOT TO BE THE BASIS FOR ANY LEGALLY NDING DOCUMENTATION.

RKAA# 20047.50

SP-1

PROJECT DIRECTORY

ARCHITECT: RKAA ARCHITECTS, INC. 2233 EAST THOMAS ROAD PHOENIX, ARIZONA 85016 CONTACT: NEIL FEASER PHONE: (602) 955-3900 FAX: (602) 955-0496 E-MAIL: nfeaser@rkaa.com

SITE DATA

PARCEL NUMBER: 217-33-034G, 034F, 034M & 034K **EXISTING ZONING:** C-0 AND R1-18 PROPOSED ZONING: C1 (PAD A & B), SR (PAD C) AND R-18 **PROPOSED SITE AREA:** 3.8077 ACRES (165,864.90 S.F.) MAX.BUILDING HEIGHT: C-1 ZONING: 22'-0" MAX. SR ZONING 18'-0" MAX. PROPOSED BUILDING HEIGHT: 18'-0" MAX. PROPOSED USE: RETAIL / MED. OFFICE / OFFICE BUILDING AREA: PAD A: RETAIL / OFFICE/MEDICAL 3,000 S.F. PAD B: RETAIL / OFFICE/MEDICAL 5,100 S.F. PAD C: MEDICAL OFFICE 7,236 S.F. 15,336 S.F. TOTAL BUILDING AREA: PARKING REQUIRED: 12 SPACES PAD A: RETAIL-OFFICE / 250 S.F. 21 SPACES PAD B: RETAIL-OFFICE / 250 S.F. 29 SPACES PAD C: MEDICAL OFFICE / 250 S.F. PARKING PROVIDED: 22 SPACES PAD A: RETAIL-OFFICE / 250 S.F. 23 SPACES PAD B: RETAIL-OFFICE / 250 S.F. 56 SPACES PAD C: MEDICAL OFFICE / 250 S.F. 62 SPACES TOTAL PARKING REQUIRED: **101 SPACES** TOTAL PARKING PROVIDED: 05 SPACES BICYCLE SPACES REQUIRED: **06 SPACES** BICYCLE SPACES PROVIDED: PROPOSED LOT A-B AREA: 1.3032 ACRES (56,770.44 S.F.) PROPOSED LOT -C AREA: 1.2438 ACRES (54,182.40 S.F.) PROPOSED LOT 1 AREA: 0.6795 ACRES (29,599.48 S.F.) PROPOSED LOT 2 AREA: 0.5811 ACRES (25,312.58 S.F.) Blue Adobe Santa Fa Grile 0 Q ARCO E SAHUARO DR ...0 0



VICINITY MAP

SCALE: N.T.S.





Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix B – Crash Data





CITY OF SCOTTSDALE

COLLISION SUMMARY

REPORT #	DATE YYMMDD	ТІМЕ ННММ	NORTH / SOUTH ST.	TYPE	EAST WEST ST.	TYPE	dir From	DIST FROM	INJ. : #1	SEV. #2	PHY #1	S. COND. #2	VIO #1	LATION #2	AC ⁻ #1	TION #2	TRA #1	V. DIR. #2	MANNER OF COLLISION	COMMENTS
1718446	170820	1103	114	ST	SHEA	BL	AT		1	1	0	0	2	1	1	3	EB	EB	4	
1725363	171115	2101	114	ST	SHEA	BL	AT		99	1	99	0	99	1	1	3	EB	EB	4	MULTI VEH 3, HIT AND RUN
1726933	171205	1823	114	ST	SHEA	BL	AT		1	1	0	0	2	1	2	2	WB	WB	4	
1803981	180219	1824	114	ST	SHEA	BL	AT		1		0		2		4		WB		1	
1816869	180801	1538	114	ST	SHEA	BL	AT		99	1	99	0	99	1	99	2	EB	EB	4	HIT AND RUN
1826669	181211	0635	114	ST	SHEA	BL	Ν	20	1	1	0	0	7	1	5	3	SB	SB	6	
1922306	191025	1810	114	ST	SHEA	BL	AT				0	0	2	1	1	3	NB	NB	4	

KEY

INJURY SEVERITY:

1=NO INJURY, 2=POSSIBLE INJURY, 3=NON-INCAPACITATING INJURY, 4=INCAPACITATING INJURY, 5=FATAL INJURY, 99=NOT REPORTED / UNKNOWN

PHYSICAL CONDITION:

0=NO APPARENT INFLUENCE, 1=ILLNESS, 2=PHYSICAL IMPAIRMENT, 3=FELL ASLEEP / FATIGUED 4=ALCOHOL, 5=DRUGS, 6=MEDICATIONS, A=NO TEST GIVEN, B=TEST GIVEN, C=TEST REFUSED, D=TESTING UNKNOWN, 97=OTHER, 99=UNKNOWN

VIOLATION:

1=NO IMPROPER ACTION, 2=SPEED TOO FAST FOR CONDITIONS, 3=EXCEEDED LAWFUL SPEED 4=FOLLOWED TOO CLOSELY. 5=RAN STOP SIGN, 6=DISREGAREDED TRAFFIC SIGNAL7=MADE IMPROPER TURN, 8=DROVE/RODE IN OPPOSING TRAFFIC LANE, 9=KNOWINGLY OPERATED WITH FAULTY / MISSING EQUIPMENT, 10=REQUIRED MOTORCYCLE SAFETY EQUIPMENT NOT USED, 11=PASSED IN NO PASSING ZONE, 12=UNSAFE LANE CHANGE, 13=FAILED TO KEEP IN PROPER LANE, 14=DISREGARDED PAVEMENT MARKINGS, 15=OTHER UNSAFE PASSING, 16=INATTENTION/DISTRACTION, 17=DID NOT USE CROSSWALK, 18=WALKED ON WRONG SIDE OF ROAD, 19=ELECTRONIC COMMUNICATIONS DEVICE, 20=FAILED TO YIELD RIGHT OF WAY (added August 2014), 97=OTHER, 99 UNKNOWN

ACTION:

1=GOING STRAIGHT AHEAD, 2=SLOWING IN TRAFFICWAY, 3=STOPPED IN TRAFFICWAY, 4=MAKING LEFT TURN, 5=MAKING RIGHT TURN, 6=MAKING U-TURN, 7=OVERTAKING/PASSING, 8=CHANGING LANES, 9=NEGOTIATING A CURVE, 10=BACKING, 11=AVOIDING VEH/OBJ/PED/CYCLIST/ANIMAL, 12=ENTERING PARKING POSITION, 13=LEAVING PARKING POSITION, 14=PROPERLY PARKED, 15=IMPROPERLY PARKED, 16=DRIVERLESS MOVING VEHICLE, 17=CROSING ROAD, 18=WALKING WITH TRAFFIC, 19=WALKING AGAINST TRAFFIC, 20=STANDING, 21=LYING, 22=GETTING ON OR OFF VEHICLE, 23=WORKING ON/PUSHING VEHICLE, 24=WORKING ON ROAD, 97=OTHER, 99=UKNOWN

MANNER OF COLLISION:

1=SINGLE VEHICLE, 2=ANGLE (front to side, other than left turn), 3=LEFT TURN, 4=REAR END (front to rear), 5=HEAD-ON (front to front, other than left turn), 6=SIDESWIPE (same direction), 7=SIDESWIPE (opposite direction), 8=REAR-TO-SIDE, 9=REAR TO REAR, 97=OTHER, 99=UNKNOWN

TOTAL 7



CITY OF SCOTTSDALE

COLLISION SUMMARY

REPORT #	DATE YYMMDD	ТІМЕ ННММ	NORTH / SOUTH ST.	TYPE	EAST WEST ST.	TYPE	dir From	DIST FROM	INJ. SEV. #1 #2	PH #1	IYS. COND. #2	VIOLA #1 #2	TION 2	ACTION #1 #2	TRAV. DIR. #1 #2	MANNER OF	COMMENTS
2002222	200130	1709	112	ST	SHEA	BL	Е	240			0 0	2	I	1 2	EB EB	4	
1804668	180301	1223	114	ST	SHEA	BL	W	104	1 1		0 0	2 1	1	1 3	WB WB	4	
1804672	180301	1300	114	ST	SHEA	BL	W	200	2 1		0 0	2 1	1	1 2	WB WB	4	
1901735	190124	0803	114	ST	SHEA	BL	Е	300			0 0	2 1	1	8 3	WB WB	4	

KEY

INJURY SEVERITY:

1=NO INJURY, 2=POSSIBLE INJURY, 3=NON-INCAPACITATING INJURY, 4=INCAPACITATING INJURY, 5=FATAL INJURY, 99=NOT REPORTED / UNKNOWN

PHYSICAL CONDITION:

0=NO APPARENT INFLUENCE, 1=ILLNESS, 2=PHYSICAL IMPAIRMENT, 3=FELL ASLEEP / FATIGUED 4=ALCOHOL, 5=DRUGS, 6=MEDICATIONS, A=NO TEST GIVEN, B=TEST GIVEN, C=TEST REFUSED, D=TESTING UNKNOWN, 97=OTHER, 99=UNKNOWN

VIOLATION:

1=NO IMPROPER ACTION, 2=SPEED TOO FAST FOR CONDITIONS, 3=EXCEEDED LAWFUL SPEED 4=FOLLOWED TOO CLOSELY. 5=RAN STOP SIGN, 6=DISREGAREDED TRAFFIC SIGNAL7=MADE IMPROPER TURN, 8=DROVE/RODE IN OPPOSING TRAFFIC LANE, 9=KNOWINGLY OPERATED WITH FAULTY / MISSING EQUIPMENT, 10=REQUIRED MOTORCYCLE SAFETY EQUIPMENT NOT USED, 11=PASSED IN NO PASSING ZONE, 12=UNSAFE LANE CHANGE, 13=FAILED TO KEEP IN PROPER LANE, 14=DISREGARDED PAVEMENT MARKINGS, 15=OTHER UNSAFE PASSING, 16=INATTENTION/DISTRACTION, 17=DID NOT USE CROSSWALK, 18=WALKED ON WRONG SIDE OF ROAD, 19=ELECTRONIC COMMUNICATIONS DEVICE, 20=FAILED TO YIELD RIGHT OF WAY (added August 2014), 97=OTHER, 99 UNKNOWN

ACTION:

1=GOING STRAIGHT AHEAD, 2=SLOWING IN TRAFFICWAY, 3=STOPPED IN TRAFFICWAY, 4=MAKING LEFT TURN, 5=MAKING RIGHT TURN, 6=MAKING U-TURN, 7=OVERTAKING/PASSING, 8=CHANGING LANES, 9=NEGOTIATING A CURVE, 10=BACKING, 11=AVOIDING VEH/OBJ/PED/CYCLIST/ANIMAL, 12=ENTERING PARKING POSITION, 13=LEAVING PARKING POSITION, 14=PROPERLY PARKED, 15=IMPROPERLY PARKED, 16=DRIVERLESS MOVING VEHICLE, 17=CROSING ROAD, 18=WALKING WITH TRAFFIC, 19=WALKING AGAINST TRAFFIC, 20=STANDING, 21=LYING, 22=GETTING ON OR OFF VEHICLE, 23=WORKING ON/PUSHING VEHICLE, 24=WORKING ON ROAD, 97=OTHER, 99=UKNOWN

MANNER OF COLLISION:

1=SINGLE VEHICLE, 2=ANGLE (front to side, other than left turn), 3=LEFT TURN, 4=REAR END (front to rear), 5=HEAD-ON (front to front, other than left turn), 6=SIDESWIPE (same direction), 7=SIDESWIPE (opposite direction), 8=REAR-TO-SIDE, 9=REAR TO REAR, 97=OTHER, 99=UNKNOWN

TOTAL 4



Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix C – Parcel Information





217-33-034G Land Parcel

This is a land parcel located at <u>11355 E SHEA BLVD SCOTTSDALE 85259</u>. and the current owner is KCI 114 INVESTMENTS LLC. Its current year full cash value is \$477,600.

Property Information

11355 E SHEA BLVD SCOTTSDALE 85259

MCR #	
Description:	W2 OF FOL DESC PROP N 286F OF E2 E2 NW4 NW4 EX W 25F & EX N 65F
Lat/Long	<u>33.58197000 -111.83579100</u>
Lot Size	33,715 sq ft.
Zoning	C-0
Lot #	
High School District	SCOTTSDALE UNIFIED #48
Elementary School District	SCOTTSDALE UNIFIED SCHOOL DISTRICT
Local Jurisdiction	SCOTTSDALE
S/T/R	27 3N 5E
Market Area/Neighborhood	05/019
Subdivision (0 Parcels)	

Owner Information

KCI 114 INVESTMENTS LLC

Mailing Address7339 E EVANS RD SUITE 201, SCOTTSDALE, AZ 85260Deed Number140568992Last Deed Date08/27/2014Sale Daten/aSale Pricen/a

Valuation Information

We provide valuation information for the past 5 years. For mobile display, we only show 1 year of valuation information. Should you need more data, please look at our <u>data sales</u>.

The Valuation Information displayed below may not reflect the taxable value used on the tax bill due to any special valuation relief program. <u>CLICK HERE TO PAY YOUR TAXES OR VIEW YOUR TAX BILL</u>

Tax Year	2021	2020	2019	2018	2017
Full Cash Value	\$477,600	\$438,100	\$417,300	\$422,600	\$202,400
Limited	\$122.276	\$116 453	\$110.009	\$105 627	£100 507
Value	\$122,270	\$110,455	\$110,908	\$103,027	\$100,397
Legal Class	2.R	2.R	2.R	2.R	2.R
Description	AG / VACANT LAND / NON- PROFIT R/P				
Assessment Ratio	15%	15%	15%	15%	15%
Assessed LPV	\$18,341	\$17,468	\$16,636	\$15,844	\$15,090
Property Use Code	0022	0022	0022	0022	0012
PU	Vacant	Vacant Commercial	Vacant Commercial	Vacant Commercial	Vacant Residential
Description	Commercial Land	Land	Land	Land	Land
Tax Area Code	481400	481400	481400	481400	481400
Valuation Source	Notice	Notice	Notice	Notice	Notice

217-33-034F Land Parcel

This is a land parcel located at <u>11355 E SHEA BLVD SCOTTSDALE 85259</u>. and the current owner is KCI 114 INVESTMENTS LLC. Its current year full cash value is \$480,100.

Property Information

11355 E SHEA BLVD SCOTTSDALE 85259

MCR

Description:	E2 OF FOL N 286F OF E2 E2 NW4 NW4 EX W 25F & EX N 65F & ALSO EX RD PER 88-469357
Lat/Long	<u>33.58197000 -111.83579100</u>
Lot Size	31,494 sq ft.
Zoning	C-0
Lot #	
High School District	SCOTTSDALE UNIFIED #48
Elementary School District	SCOTTSDALE UNIFIED SCHOOL DISTRICT
Local Jurisdiction	SCOTTSDALE
S/T/R	27 3N 5E
Market Area/Neighborhood	05/019
Subdivision (0 Parcels)	

Owner Information

KCI 114 INVESTMENTS LLC

Mailing Address7339 E EVANS RD SUITE 201, SCOTTSDALE, AZ 85260Deed Number140568992Last Deed Date08/27/2014Sale Daten/aSale Pricen/a

Valuation Information

We provide valuation information for the past 5 years. For mobile display, we only show 1 year of valuation information. Should you need more data, please look at our <u>data sales</u>.

The Valuation Information displayed below may not reflect the taxable value used on the tax bill due to any special valuation relief program. <u>CLICK HERE TO PAY YOUR TAXES OR VIEW YOUR TAX BILL</u>

Tax Year	2021	2020	2019	2018	2017
Full Cash Value	\$480,100	\$455,300	\$433,600	\$432,900	\$194,700
Limited Property Value	\$114,819	\$109,351	\$104,144	\$99,185	\$94,462
Legal Class	2.R	2.R	2.R	2.R	2.R
Description	AG / VACANT LAND / NON- PROFIT R/P				
Assessment Ratio	15%	15%	15%	15%	15%
Assessed LPV	\$17,223	\$16,403	\$15,622	\$14,878	\$14,169
Property Use Code	0022	0022	0022	0022	0012
PU	Vacant	Vacant Commercial	Vacant Commercial	Vacant Commercial	Vacant Residential
Description	Commercial Land	Land	Land	Land	Land
Tax Area Code	481400	481400	481400	481400	481400
Valuation Source	Notice	Notice	Notice	Notice	Notice

217-33-034M Land Parcel

This is a land parcel located at <u>11355 E SHEA BLVD SCOTTSDALE 85259</u>. and the current owner is KCI 114 INVESTMENTS LLC. Its current year full cash value is \$443,000.

Property Information

11355 E SHEA BLVD SCOTTSDALE 85259

MCR

Subdivision (0 Parcels)	
Market Area/Neighborhood	05/019
S/T/R	27 3N 5E
Local Jurisdiction	SCOTTSDALE
Elementary School District	SCOTTSDALE UNIFIED SCHOOL DISTRICT
High School District	SCOTTSDALE UNIFIED #48
Lot #	
Zoning	R1-18
Lot Size	68,484 sq ft.
Lat/Long	<u>33.58197000 -111.83579100</u>
Description:	S 428.10F OF N 714.10F OF E2 E2 NW4 NW4 SEC 27 EX W 25F & EX S 203F OF W 240F OF S 428.10F OF N 714.10F E2 E2 NW4 NW4 SEC 27 & ALSO EX TH PT LY E OF LI DAF COM NE COR NW4 NW4 SD SEC TH S 715 TH W 8.12F TO POB SD LI TH N 9D 51M W 179.54F TH W 12F TH N 9D 51M W 40F TH N 80D 8M E 12F TH N 9D 517.05F TO BEG TAN CUR CONC TO NE TH NLY ALG CUR 99.17F TO BEG COMP CUR CONC TO NE TH NLY ALG CUR 107.56F TO PT & POE SD LI
	S 428.10F OF N 714.10F OF E2 E2 NW4 NW4 SEC 27 EX W 25F & EX S 203F OF W 240E OF S 428 10F OF N 714 10F E2 E2 NW4 NW4 SEC 27 & ALSO EX TH PT LY E C

Owner Information

KCI 114 INVESTMENTS LLC

Mailing Address7339 E EVANS RD SUITE 201, SCOTTSDALE, AZ 85260Deed Number140568992Last Deed Date08/27/2014Sale Daten/aSale Pricen/a

Valuation Information

We provide valuation information for the past 5 years. For mobile display, we only show 1 year of valuation information. Should you need more data, please look at our <u>data sales</u>.

The Valuation Information displayed below may not reflect the taxable value used on the tax bill due to any special valuation relief program. CLICK HERE TO PAY YOUR TAXES OR VIEW YOUR TAX BILL

Tax Year	2021	2020	2019	2018	2017
Full Cash Value	\$443,000	\$381,100	\$352,200	\$382,400	\$395,900
Limited Property Value	\$273,964	\$260,918	\$248,493	\$236,660	\$225,390
Legal Class	2.R	2.R	2.R	2.R	2.R
Description	AG / VACANT LAND / NON- PROFIT R/P				
Assessment Ratio	15%	15%	15%	15%	15%
Assessed LPV	\$41,095	\$39,138	\$37,274	\$35,499	\$33,809
Property Use Code	0012	0012	0012	0012	0012
PU Description	Vacant Residential Land				
Tax Area Code	481400	481400	481400	481400	481400
Valuation Source	Notice	Notice	Notice	Notice	Notice

217-33-034K Land Parcel

This is a land parcel located at <u>11355 E SHEA BLVD SCOTTSDALE 85259</u>. and the current owner is KCI 114 INVESTMENTS LLC. Its current year full cash value is \$370,000.

Property Information

11355 E SHEA BLVD SCOTTSDALE 85259

MCR

Description:	S 203F OF W 240F OF S 428.10F OF N 714.10F OF E2 E2 NW4 NW4 SEC 27 EX W 25F TH/OF
Lat/Long	<u>33.58197000 -111.83579100</u>
Lot Size	43,647 sq ft.
Zoning	R1-18
Lot #	
High School District	SCOTTSDALE UNIFIED #48
Elementary School District	SCOTTSDALE UNIFIED SCHOOL DISTRICT
Local Jurisdiction	SCOTTSDALE
S/T/R	27 3N 5E
Market Area/Neighborhood	05/019
Subdivision (0 Parcels)	

Owner Information

KCI 114 INVESTMENTS LLC

Mailing Address7339 E EVANS RD SUITE 201, SCOTTSDALE, AZ 85260Deed Number140568992Last Deed Date08/27/2014Sale Daten/aSale Pricen/a

Valuation Information

We provide valuation information for the past 5 years. For mobile display, we only show 1 year of valuation information. Should you need more data, please look at our <u>data sales</u>.

The Valuation Information displayed below may not reflect the taxable value used on the tax bill due to any special valuation relief program. <u>CLICK HERE TO PAY YOUR TAXES OR VIEW YOUR TAX BILL</u>

Tax Year	2021	2020	2019	2018	2017
Full Cash Value	\$370,000	\$311,800	\$283,300	\$294,500	\$305,600
Limited Property Value	\$209,377	\$199,407	\$189,911	\$180,868	\$172,255
Legal Class	2.R	2.R	2.R	2.R	2.R
Description	AG / VACANT LAND / NON- PROFIT R/P				
Assessment Ratio	15%	15%	15%	15%	15%
Assessed LPV	\$31,407	\$29,911	\$28,487	\$27,130	\$25,838
Property Use Code	0014	0014	0014	0014	0014
PU Description	Vacant Residential Land				
Tax Area Code	481400	481400	481400	481400	481400
Valuation Source	Notice	Notice	Notice	Notice	Notice



Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix D – Traffic Count Data





Intersection Turning Movement Prepared by: Field Data Services of Arizona, Inc. 520.316.6745



			In	terse	ction T Prep	urnin ared l	g Mov by:	emen	t				
FIELD	Д ат	a Sei	RVICI	ES OF		ZONA 20.31	, I NC 6.674	5	e vera	city	traf	ficgr	oup
N-S STREET:	114th S	it.			DATE:	03/04/2	20		LOCA	TION:	Scottsd	ale	
F-W STRFFT	Shea Bl	vd			DAY	WEDNE	SDAY		PRO1	FCT#	20-1151	-003	
	Shea Bi	DAT: WEDNESDAT PROJECT# 20-1151-003											
	NORTHBOUND SOUTHBOUND EASTBOUND WESTBOUND												
LANES:	NL 2	NT 1	NR 0	SL 2	ST 2	SR 1	EL 2	ET 3	ER 0	WL 1	WT 3	WR 1	TOTAL
6:00 AM 6:15 AM 6:30 AM 6:45 AM 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:45 AM 9:00 AM 9:15 AM 9:30 AM 9:30 AM 10:15 AM 10:00 AM 10:15 AM 10:30 AM 10:45 AM 11:45 AM	12 17 10 17 10 13 20 9	28 29 22 23 16 32 26	1 5 0 8 1 2 1 4	155 137 138 116 103 101 114 79	6 5 12 16 13 9 11 12	34 32 43 46 41 40 42 43	22 24 16 32 28 47 33 37	342 310 313 240 290 254 231 244	11 7 10 15 7 7 10	2 2 5 0 1 4 0	431 417 524 442 374 353 376 294	143 142 125 92 146 146 140 95	1187 1127 1215 1041 1044 989 1011 853
TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes Approach %	108 32.93	198 60.37	22 6.71	943 69.96	84 6.23	321 23.81	239 9.42	2224 87.66	74 2.92	14 0.33	3211 75.48	1029 24.19	8467
App/Depart	328	/	1466	1348	/	172	2537	/	3189	4254	/	3640	
AM Pea	ak Hr Beg	gins at:	700	AM									
PEAK Volumes Approach %	56 32.75	101 59.06	14 8.19	546 73.78	39 5.27	155 20.95	94 7.05	1205 90.33	35 2.62	9 0.39	1814 78.02	502 21.59	4570
PEAK HR. FACTOR:	I	0.838	l		0.949	I		0.889	I		0.889	I	0.940
CONTROL: COMMENT 1: GPS:	Signal 33.5824	162, -11 3	1.83507	79									

Intersection Turning Movement

FIEL	ο D ατ	'A Se	RVIC	ES O	F A R	IZON 520.31	a, In 16.674	c. V	P vera	city	traf	ficgr	oup	
N-S STREET:	114th S	t.			DATE:	03/04/2	0		LOCATION: Scottsdale					
E-W STREET:	Shea Bl	C vd.)		DAY:	WEDNE	SDAY		PROJ	ECT#	20-115 1	-003		
	NO	SO	UTHBOU	JND	EA	ASTBOU	ND	W	ESTBOU	ND				
LANES:	NL 2	NT 1	NR 0	SL 2	ST 2	SR 1	EL 2	ET 3	ER 0	WL 1	WT 3	WR 1	TOTAL	
1:00 PM 1:15 PM 1:30 PM 1:45 PM 2:00 PM 2:15 PM 2:30 PM 2:45 PM 3:00 PM 3:15 PM 3:30 PM 3:45 PM 4:00 PM 4:15 PM 4:30 PM 5:15 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM 6:00 PM 6:15 PM 6:30 PM	11 9 13 6 15 10 9 12	26 18 17 18 20 25 18 21	0 4 6 3 6 4 3 3	162 223 202 232 187 212 165 150	23 37 22 29 17 27 18 24	39 29 25 28 26 30 33 40	50 53 51 57 55 68 65 47	403 409 463 409 440 475 380 308	16 14 18 8 12 7 8 11	3 1 7 6 4 3 4 5	287 237 244 219 268 226 255 197	132 136 164 133 144 122 131 91	1152 1170 1232 1148 1194 1209 1089 909	
TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL	
Volumes Approach %	85 30.69	163 58.84	29 10.47	1533 77.42	197 9.95	250 12.63	446 11.65	3287 85.89	94 2.46	33 1.09	1933 64.03	1053 34.88	9103	
App/Depart	277	/	1662	1980	/	324	3827	/	4849	3019	/	2268		
PM Pea	ak Hr Beg	gins at:	430	PM										
PEAK Volumes Approach %	44 30.77	80 55.94	19 13.29	833 80.33	95 9.16	109 10.51	231 11.20	1787 86.62	45 2.18	20 1.30	957 62.14	563 36.56	4783	
PEAK HR. FACTOR:		0.872			0.897	I		0.938	I		0.925	I	0.971	
CONTROL: COMMENT 1: GPS:	Signal 0 33.5824	62, -11	1.83507	79								0 7		



Vveracitytrafficgroup

Pedestrian & Bicycle Study

N-S STREET: 114th St. **E-W STREET:** Shea Blvd. Date: 03/04/20 Day: WEDNESDAY City: Scottsdale Project #: 20-1151-003

		PEDES	TRIANS	
	N-LEG	S-LEG	E-LEG	W-LEG
7:00 AM	0	0	0	0
7:15 AM	0	0	0	0
7:30 AM	0	0	0	0
7:45 AM	2	0	0	0
8:00 AM	0	0	0	0
8:15 AM	0	0	0	0
8:30 AM	0	0	0	0
8:45 AM	1	0	2	0
TOTAL	3	0	2	0

		PEDES	TRIANS	
	N-LEG	S-LEG	E-LEG	W-LEG
4:00 PM	0	0	0	0
4:15 PM	0	0	0	0
4:30 PM	0	0	0	0
4:45 PM	0	0	0	0
5:00 PM	1	0	0	0
5:15 PM	1	0	0	0
5:30 PM	0	0	0	0
5:45 PM	0	0	0	0
TOTAL	2	0	0	0

		BICY	CLES	
	N-LEG	S-LEG	E-LEG	W-LEG
7:00 AM	0	0	0	0
7:15 AM	0	0	0	0
7:30 AM	0	0	0	0
7:45 AM	0	0	0	0
8:00 AM	0	0	0	0
8:15 AM	0	0	0	0
8:30 AM	0	0	0	0
8:45 AM	0	0	0	0
TOTAL	0	0	0	0

		BICY	CLES	
	N-LEG	S-LEG	E-LEG	W-LEG
4:00 PM	0	0	0	0
4:15 PM	0	0	0	1
4:30 PM	0	0	0	0
4:45 PM	0	0	0	0
5:00 PM	0	0	0	0
5:15 PM	0	0	0	0
5:30 PM	0	0	0	0
5:45 PM	0	0	0	0
TOTAL	0	0	0	1

North Leg

East Leg

West Leg

South Leg

Intersection Turning Movement Prepared by: Field Data Services of Arizona, Inc. 520.316.6745



Intersection Turning Movement Prepared by:



Intersection Turning Movement





veracitytrafficgroup

Pedestrian & Bicycle Study

N-S STREET: 114th St. E-W STREET: Dwy. 400' s/o Shea Blvd. Date: 03/04/20 Day: WEDNESDAY

City: Scottsdale Project #: 20-1151-004

		PEDES	TRIANS	
	N-LEG	S-LEG	E-LEG	W-LEG
7:00 AM	0	0	0	0
7:15 AM	0	0	0	0
7:30 AM	0	0	0	0
7:45 AM	0	0	0	0
8:00 AM	0	0	0	0
8:15 AM	0	0	0	0
8:30 AM	0	0	0	0
8:45 AM	0	0	2	0
TOTAL	0	0	2	0

		PEDES	TRIANS	
	N-LEG	S-LEG	E-LEG	W-LEG
4:00 PM	0	0	0	0
4:15 PM	0	0	0	0
4:30 PM	0	0	0	0
4:45 PM	0	0	0	0
5:00 PM	0	0	0	0
5:15 PM	0	0	0	0
5:30 PM	0	0	0	0
5:45 PM	0	0	0	0
TOTAL	0	0	0	0

		BICY	CLES	
	N-LEG	S-LEG	E-LEG	W-LEG
7:00 AM	0	0	0	0
7:15 AM	0	0	0	0
7:30 AM	0	0	0	0
7:45 AM	0	0	0	0
8:00 AM	0	0	0	0
8:15 AM	0	0	0	0
8:30 AM	0	0	0	0
8:45 AM	0	0	0	0
TOTAL	0	0	0	0

		BICY	CLES	
	N-LEG	S-LEG	E-LEG	W-LEG
4:00 PM	0	0	0	0
4:15 PM	0	0	0	0
4:30 PM	0	0	0	0
4:45 PM	0	0	0	0
5:00 PM	0	0	0	0
5:15 PM	0	0	0	0
5:30 PM	0	0	0	0
5:45 PM	0	0	0	0
TOTAL	0	0	0	0

North Leg

East Leg

West Leg

South Leg

Prepared by: Field Data Services of Arizona/Veracity Traffic Group (520) 316-6745

Volumes for: Wednesday, March 4, 2020

City: Scottsdale

Project #: 20-1151-001

AM Period NB	SB	<u>EB</u>	WB			PM Period	NB	SB		EB		WB		
00:00		28	13			12:00				302		313		
00:15		23	11			12:15				368		312		
00:45		10 7 68	14	43	111	12:30				340	1341	302	1263	2604
00.43		7 00	15	Ъ	111	12.45				214	1741	200	1205	2004
01:00		10 6	10			13:00				314		299 201		
01:15		0 12	10			13:15				320		204		
01:50		12 46	7	49	95	13:45				338	1365	328	1241	2606
02:00		6	13			14:00				331	1000	203		2000
02:00		6	8			14.00				355		355		
02:30		7	5			14:30				366		289		
02:45		6 25	2	28	53	14:45				414	1466	319	1256	2722
03:00		6	8			15:00				444		344		
03:15		4	4			15:15				446		327		
03:30		9	7			15:30				469		332		
03:45		4 23	12	31	54	15:45				429	1788	270	1273	3061
04:00		16	19			16:00				469		337		
04:15		20	21			16:15				476		275		
04:30		20	35			16:30				532		282		
04:45		32 88	40	115	203	16:45				474	1951	253	1147	3098
05:00		26	50			17:00				507		309		
05:15		57	80			17:15				550		266		
05:30		72	97			17:30				453		297		
05:45		88 243	112	339	582	17:45				366	1876	249	1121	2997
06:00		95	140			18:00				374		272		
06:15	1	.69	193			18:15				335		210		
06:30	2	252	274			18:30				282		187		
06:45	2	94 810	292	899	1709	18:45				280	1271	162	831	2102
07:00	3	875	477			19:00				258		153		
07:15	3	841	466			19:15				245		128		
07:30	3	36	577			19:30				223		117		
07:45	2	82 1334	505	2025	3359	19:45				180	906	82	480	1386
08:00	3	33	425			20:00				195		106		
08:15	3	808	406			20:15				185		104		
08:30	2	271	438			20:30				178		79		
08:45	2	91 1203	346	1615	2818	20:45				135	693	77	366	1059
09:00	3	807	323			21:00				139		109		
09:15	2	268	332			21:15				125		67		
09:30	2	91	330			21:30				101		68		
09:45	2	1134	327	1312	2446	21:45				89	454	60	304	758
10:00	2	256	315			22:00				90		46		
10:15	3	809	321			22:15				79		49		
10:30	2	284	343	1200	2 4 2 2	22:30				59		44	470	
10:45	2	93 1142	301	1280	2422	22:45				44	272	34	1/3	445
11:00	2	264	315			23:00				42		35		
11:15		31/ 	369			23:15				37		22		
11:30	2	2/6	297	1200	2461	23:30				33 21	122	19	80	222
11.45	2	.95 1152	520	1309	2401	23.43				21	155	15	09	222
Total Vol.		7268		9045	16313						13516		9544	23060
GPS Coordinates:	33.582456, -1	11.836613								Da	ily Total	5		
								NB	SB		EB		WB	Combined
											20784		18589	39373
		AM				_					PM			
Split %		44.6%	D	55.4%	41.4%						58.6%		41.4%	58.6%
Peak Hour		06:45		07:00	07:00						16:30		14:45	16:30
Mala		1346		2025	3359						2063		1322	3173
volume														
P.H.F.		0.90		0.88	0.92						0	Q	_7NI	_ງ∩ງ

Prepared by: Field Data Services of Arizona/Veracity Traffic Group (520) 316-6745

Volumes for: Wednesday, March 4, 2020

City: Scottsdale

Project #: 20-1151-002

Location:	114t	h St. so	outh	of Shea	Blvd.										
AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
00:00	2		1					12:00	43		22				
00:15	1		0					12:15	30		43				
00:30	2		2					12:30	41		47				
00:45	1	6	2	5			11	12:45	26	140	40	152			292
01:00	0		1					13:00	32		32				
01:15	0		0					13:15	29		36				
01:30	0	•	0	-			-	13:30	41	107	32				250
01:45	0	0	1	2			2	13:45	25	127	31	131			258
02:00	0		0					14:00	40		42				
02:15	0		0					14:15	40		35				
02:30	0	٥	0	0				14:30	31 47	153	30	148			301
02.45	0	0	0	0				15.00	27	155	20	140			501
03:00	0		0					15:00	32 42		39				
03.13	0		0					15.15	38		35				
03:45	2	2	0	0			2	15:45	22	134	32	147			281
04.00	0		0	0			_	16:00	37	10 .	42	,			
04:15	3		0					16:15	31		52				
04:30	2		0					16:30	36		47				
04:45	2	7	1	1			8	16:45	27	131	43	184			315
05:00	3		2					17:00	41		33				
05:15	0		6					17:15	39		37				
05:30	10		4					17:30	30		30				
05:45	2	15	2	14			29	17:45	36	146	40	140			286
06:00	10		5					18:00	32		26				
06:15	16		6					18:15	16		29				
06:30	18		4					18:30	9		22				
06:45	17	61	11	26			87	18:45	13	70	32	109			179
07:00	41		19					19:00	13		23				
07:15	51		14					19:15	11		29				
07:30	32	474	24	02			254	19:30	13		19	00			124
07:45	4/	1/1	26	83			254	19:45	/	44	19	90			134
08:00	34		28					20:00			8				
08:15	51		1/					20:15	0		20				
08:30	20	157	22	89			246	20:30	9	27	25 12	63			90
00.00	20	157	22	05			210	20.45		27	6	05			50
09:00	29		25					21:00	5 17		0 14				
09.15	44		28					21.15	5		7				
09:45	24	132	19	102			234	21:45	3	30	, 5	32			62
10.00	40	-	38	-				22.00	0		3	-			
10:15	41		26					22:15	4		2				
10:30	50		31					22:30	16		3				
10:45	46	177	18	113			290	22:45	7	27	3	11			38
11:00	45		30					23:00	2		1				
11:15	32		36					23:15	2		1				
11:30	36		35					23:30	1		2				
11:45	37	150	29	130			280	23:45	0	5	0	4			9
Total Vol.		878		565			1443			1034		1211			2245
GPS Coordi	nates	:	33	.5814881	1.835049								Daily Totals		
2. C 000101		-	00							NB		SB	EB	WB	Combined
										1912		1776			3688
					AM								PM		
Split %		60.8%		39.2%			<u>39.1%</u>			46.1%	5	53. <u>9</u> %			60.9%
Peak Hour		10:15		11:45			11:45			14:45		16:00			16:00
Volume		182		141			202			154		184			315
P.H.F.		0.91		0.75			0.83			0.92		0.88		Q 7N	1 2020
														0-21	N-2020
														5/22	/2020



Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix E – Existing Signal Timing





			S	SHE	8 A	FL	W/11	4TH			System # 139
	BA	SIC	TIMIN	G PL	.AN			Sect	ion #	I.P. Address MM1-5-1	Date Designed
										172.27.11.39	3/12/2020
TIMING PLAN - MM-2-1	Phase Movement NOTES MIN GRN BK MGRN CS MGRN DLY GRN WALK WALK2 WLK MAX PED CLR/FDW PD CLR2 PC MAX PED CO VEH EXT VH EXT2 MAX 1 MAX 2 MAX 3 DYM MAX DYM STP YELLOW RED CLR RED MAX RED RVT ACT B4 SEC/ACT MAX INT TIME B4 CARS WT STPTDUC TTREDUC	1 WBL PROT 5 	2 EBT COORD 10 4 21 21 55 65 5 5 5 5 5 5 1 0 2 2	3 NBL L-P 5 	4 SBT 7 4 31 31 50 60 50 60 5 4.4 1.2 2	5 EBL J-P 5 	6 WBT COORD 10 4 22 22 22 55 65 55 65 55 55 55 55 1.0 2 2	7 SBL PROT 5 	8 NBT 7 4 28 28 50 60 50 60 5 3.6 1.8 2	$\frac{1}{1}$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
LLS - MM-2-8	MIN GAP LOCK DET VEH RECALL PED RECALL MAX RECALL SOFT RECALL		X				X			Use Timing TOD: NIGH R1 R2 Use Timing	g plan: T 2 1 3 4 5 6 8 7 B B g plan:

NOTES

Advance detection on NBT, SBT, EBT, WBT.

R2

2 5

Use Timing plan: 254

3 8

1

6

Approved By

Effective Date

В

4 7

В

	SHEA	&	FLW	//11	4Tł	1			Sy	stem #	139	
			סר			Se	ection	#		Date Upd	ated	
			JK				0		3/12/2020			
	PHASE	1	2	3	4	5	6	7	8			
	FDW		21		31		22		28			
	YELLOW	4.4	5.1	3	4.4	4.4	5.1	3.6	3.6			
	ALL RED	2	1	2	1.2	2	1	2	1.8			
	WALK		21		31		22		28			
	R1	2	\rightarrow	1	L	3	t	4	↓	COORD PATTERN	OFFSET	
ΡΙΔΝ 1	R2	5	Ĵ	6	↓	8	1	7	$ \rightarrow $	Balanced	44	
AM PLAN			RIN	G 1	1		RIN	G 2				
OPERATIVE	PHASE	1	2	3	4	5	6	7	8			
TIMES	SPLIT	12	58	11	39	12	58	20	30	Target Cy	cle Length	
6:00	COORD		Х				Х			12	20	
	RECALLS		V				V			Actual Cy	cle Length	
	GREEN	5.6	51.9	6.0	33.4	5.6	51.9	14.4	24.6	12	20	
	R1	2	\rightarrow	1	L	3	←	4	Ţ	COORD PATTERN	OFFSET	
	R2	5	Ť	6	+	8	1	7	L)	Balanced	103	
PLAN Z MIDDAV PLAN		<u> </u>	RIN	G 1		0	RIN	, G 2				
	PHASE	1	2	3	4	5	6	7	8			
TIMES	SPLIT	12	65	18	25	18	59	25	18	Target Cv	cle Length	
9:00	COORD		Х				X			12	20	
	RECALLS		V				V			Actual Cyc	cle Length	
	GREEN	5.6	58.9	13.0	19.4	11.6	52.9	19.4	12.6	12	20	
	R1	2	+	1	Г	3	Ţ	Δ	Ι	COORD	OFFSET	
	R2	5	1	6	• •	8	' ↑	7	• ↓	PATTERN	4	
			 RIN	G 1		0	RIN	, G 2				
OPERATIVE	PHASE	1	2	3	4	5	6	7	8			
TIMES	SPLIT	12	55	16	37	18	49	37	16	Target Cy	cle Length	
15:00	COORD		Х				Х			12	20	
	RECALLS		V				V			Actual Cyc	cle Length	
	GREEN	5.6	48.9	11.0	31.4	11.6	42.9	31.4	10.6	12	20	
	R1	2	\rightarrow	1	L	3	←	4	Ţ	COORD PATTERN	OFFSET	
PLAN 4	R2	5	Ĵ	6	↓	8	1	7	$ \rightarrow $	Balanced	84	
MIDNIGHT			RIN	G 1			RIN	IG 2				
PLAN	PHASE	1	2	3	4	5	6	7	8	þ.		
	SPLIT	12	43	12	23	14	41	21	14	Target Cy	cle Length	
111VIES	COORD		Х				Х			9	0	
22.00	RECALLS		V				V			Actual Cyc	cle Length	
	GREEN	5.6	36.9	7.0	17.4	7.6	34.9	15.4	8.6	9	0	



Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix F – Existing Capacity Analysis





1: 114th Street & Shea Boulevard

03/30/2020	
------------	--

	≯	-	$\mathbf{\hat{z}}$	4	+	•	•	Ť	۲	1	Ŧ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	ተተኈ		۲.	^	1	ኘኘ	eî 🗧		ሻሻ	† †	1
Traffic Volume (veh/h)	88	1121	33	9	1688	467	53	94	14	508	37	145
Future Volume (veh/h)	88	1121	33	9	1688	467	53	94	14	508	37	145
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	94	1193	35	10	1796	0	56	100	15	540	39	154
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	144	2205	65	307	2864		144	126	19	415	572	255
Arrive On Green	0.04	0.43	0.43	0.17	0.56	0.00	0.04	0.08	0.08	0.12	0.16	0.16
Sat Flow, veh/h	3456	5098	150	1781	5106	1585	3456	1589	238	3456	3554	1585
Grp Volume(v), veh/h	94	797	431	10	1796	0	56	0	115	540	39	154
Grp Sat Flow(s),veh/h/ln	1728	1702	1843	1781	1702	1585	1728	0	1827	1728	1777	1585
Q Serve(g_s), s	3.2	20.8	20.8	0.6	28.6	0.0	1.9	0.0	7.4	14.4	1.1	10.8
Cycle Q Clear(g_c), s	3.2	20.8	20.8	0.6	28.6	0.0	1.9	0.0	7.4	14.4	1.1	10.8
Prop In Lane	1.00		0.08	1.00		1.00	1.00		0.13	1.00		1.00
Lane Grp Cap(c), veh/h	144	1472	797	307	2864		144	0	145	415	572	255
V/C Ratio(X)	0.65	0.54	0.54	0.03	0.63		0.39	0.00	0.79	1.30	0.07	0.60
Avail Cap(c_a), veh/h	161	1472	797	307	2864		173	0	375	415	989	441
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	56.7	25.2	25.2	41.3	17.9	0.0	56.0	0.0	54.3	52.8	42.7	46.8
Incr Delay (d2), s/veh	5.3	1.4	2.6	0.0	1.1	0.0	0.6	0.0	3.7	152.6	0.0	0.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/In	1.5	8.2	9.1	0.2	10.3	0.0	0.8	0.0	3.6	14.9	0.5	4.3
Unsig. Movement Delay, s/veh		~~ -	07.0	44.0	(0.0	• •				005 4	40 7	17.0
LnGrp Delay(d),s/veh	62.0	26.7	27.9	41.3	18.9	0.0	56.6	0.0	58.0	205.4	42.7	47.6
LnGrp LOS	E	C	С	D	В		E	<u>A</u>	E	F	D	D
Approach Vol, veh/h		1322			1806	A		171			733	
Approach Delay, s/veh		29.6			19.0			57.5			163.6	
Approach LOS		С			В			E			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	27.1	58.0	10.0	24.9	11.4	73.7	20.0	14.9				
Change Period (Y+Rc), s	6.4	6.1	5.0	* 5.6	6.4	* 6.4	* 5.6	5.4				
Max Green Setting (Gmax), s	5.6	51.9	6.0	* 33	5.6	* 52	* 14	24.6				
Max Q Clear Time (g_c+l1), s	2.6	22.8	3.9	12.8	5.2	30.6	16.4	9.4				
Green Ext Time (p_c), s	0.0	1.0	0.0	0.0	0.0	2.3	0.0	0.1				
Intersection Summary												
HCM 6th Ctrl Delay			50.4									
HCM 6th LOS			D									

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - Existing AM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

1: 114th Street & Shea Boulevard

	4	-	•	4	۶	4	1	t	
Phase Number	1	2	3	4	5	6	7	8	5
Movement	WBL	EBT	NBL	SBT	EBL	WBT	SBL	NBT	•
Lead/Lag	Lag	Lead	Lead	Lag	Lead	Lag	Lag	Lead	
Lead-Lag Optimize	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	;
Recall Mode	None	C-Max	None	None	None	C-Max	None	None	;
Maximum Split (s)	12	58	11	39	12	58	20	30	1
Maximum Split (%)	10.0%	48.3%	9.2%	32.5%	10.0%	48.3%	16.7%	25.0%)
Minimum Split (s)	11.4	31.1	10	40.6	11.4	32.1	10.6	37.4	,
Yellow Time (s)	4.4	5.1	3	4.4	4.4	5.1	3.6	3.6	i
All-Red Time (s)	2	1	2	1.2	2	1	2	1.8	,
Minimum Initial (s)	5	10	5	7	5	10	5	7	ſ
Vehicle Extension (s)	2	0.2	2	0.2	2	0.2	2	0.2	
Minimum Gap (s)	3	3	3	3	3	3	3	3	•
Time Before Reduce (s)	0	0	0	0	0	0	0	0)
Time To Reduce (s)	0	0	0	0	0	0	0	0	1
Walk Time (s)		4		4		4		4	,
Flash Dont Walk (s)		21		31		22		28	;
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	Yes	Yes	;
Inhibit Max	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	;
Start Time (s)	90	32	102	113	32	44	12	102	
End Time (s)	102	90	113	32	44	102	32	12	
Yield/Force Off (s)	95.6	83.9	108	26.4	37.6	95.9	26.4	6.6	i
Yield/Force Off 170(s)	95.6	62.9	108	115.4	37.6	73.9	26.4	98.6	;
Local Start Time (s)	46	108	58	69	108	0	88	58	,
Local Yield (s)	51.6	39.9	64	102.4	113.6	51.9	102.4	82.6	į
Local Yield 170(s)	51.6	18.9	64	71.4	113.6	29.9	102.4	54.6	i

Intersection Summary

Cycle Length 120 Control Type Actuated-Coordinated 125

Natural Cycle

Offset: 44 (37%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Splits and Phases: 1: 114th Street & Shea Boulevard

→Ø2 (R)	1	Ø1	Ø 3	∲ Ø4		
58 s		12 s	11 s	39 s		
∕ ø₅	 Ø6 (R)		¶ø8		Ø7	
12 s	58 s		30 s		20 s	

8-ZN-2020

5/22/2020

Intersection						
Int Delay, s/veh	1.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		- î >		<u>۲</u>	- 11
Traffic Vol, veh/h	0	11	149	2	29	47
Future Vol, veh/h	0	11	149	2	29	47
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage,	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	13	169	2	33	53

Major/Minor	Minor1	Ν	lajor1	Maj	or2		
Conflicting Flow All	263	170	0	0	171	0	
Stage 1	170	-	-	-	-	-	
Stage 2	93	-	-	-	-	-	
Critical Hdwy	6.63	6.23	-	- 4	.13	-	
Critical Hdwy Stg 1	5.43	-	-	-	-	-	
Critical Hdwy Stg 2	5.83	-	-	-	-	-	
Follow-up Hdwy	3.519	3.319	-	- 2.	219	-	
Pot Cap-1 Maneuver	715	873	-	- 1	405	-	
Stage 1	859	-	-	-	-	-	
Stage 2	921	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	699	873	-	- 1	405	-	
Mov Cap-2 Maneuver	719	-	-	-	-	-	
Stage 1	859	-	-	-	-	-	
Stage 2	900	-	-	-	-	-	
Approach	WB		NB		SB		
HCM Control Delay, s	9.2		0		2.9		
HCM LOS	А						

Minor Lane/Major Mvmt	NBT	NBRWB	Ln1	SBL	SBT	
Capacity (veh/h)	-	-	873	1405	-	
HCM Lane V/C Ratio	-	- 0.	014	0.023	-	
HCM Control Delay (s)	-	-	9.2	7.6	-	
HCM Lane LOS	-	-	Α	А	-	
HCM 95th %tile Q(veh)	-	-	0	0.1	-	

114th Street and Shea Boulevard - Existing AM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th TWSC

1: 114th Street & Shea Boulevard

	۶	-	\mathbf{F}	∢	←	•	1	Ť	1	1	Ļ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ኘኘ	^		7	^	1	ኘኘ	eî 🕺		ኘኘ	^	1
Traffic Volume (veh/h)	215	1662	42	19	891	524	41	75	18	775	89	102
Future Volume (veh/h)	215	1662	42	19	891	524	41	75	18	775	89	102
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	222	1713	43	20	919	0	42	77	19	799	92	105
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	278	2088	52	143	2067		144	100	25	855	988	441
Arrive On Green	0.08	0.41	0.41	0.08	0.40	0.00	0.04	0.07	0.07	0.25	0.28	0.28
Sat Flow, veh/h	3456	5123	129	1781	5106	1585	3456	1449	357	3456	3554	1585
Grp Volume(v), veh/h	222	1138	618	20	919	0	42	0	96	799	92	105
Grp Sat Flow(s),veh/h/ln	1728	1702	1847	1781	1702	1585	1728	0	1806	1728	1777	1585
Q Serve(g_s), s	7.6	35.7	35.7	1.3	15.7	0.0	1.4	0.0	6.3	27.2	2.3	6.1
Cycle Q Clear(g_c), s	7.6	35.7	35.7	1.3	15.7	0.0	1.4	0.0	6.3	27.2	2.3	6.1
Prop In Lane	1.00		0.07	1.00		1.00	1.00		0.20	1.00		1.00
Lane Grp Cap(c), veh/h	278	1387	753	143	2067		144	0	125	855	988	441
V/C Ratio(X)	0.80	0.82	0.82	0.14	0.44		0.29	0.00	0.77	0.93	0.09	0.24
Avail Cap(c_a), veh/h	334	1387	753	143	2067		317	0	160	904	988	441
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.2	31.6	31.7	51.3	25.9	0.0	55.8	0.0	54.9	44.2	32.1	33.5
Incr Delay (d2), s/veh	9.0	5.5	9.8	0.2	0.7	0.0	0.4	0.0	11.6	15.5	0.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/in	3.5	14.7	16.9	0.5	6.1	0.0	0.6	0.0	3.3	13.1	1.0	2.4
Unsig. Movement Delay, s/veh	<u> </u>	07.0		F 4 F	00.0	0.0	50.0	0.0	00 F	F0 7	20.4	00.0
LnGrp Delay(d),s/ven	63.2	37.2	41.4	51.5	26.6	0.0	56.2	0.0	66.5	59.7	32.1	33.6
	E	U	D	D		•	E	A	E	E		<u> </u>
Approach Vol, veh/h		1978			939	A		138			996	
Approach Delay, s/ven		41.4			27.1			63.4			54.4	
Approach LOS		D			C			E			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	16.0	55.0	10.0	39.0	16.1	55.0	35.3	13.7				
Change Period (Y+Rc), s	6.4	6.1	5.0	* 5.6	6.4	* 6.4	* 5.6	5.4				
Max Green Setting (Gmax), s	5.6	48.9	11.0	* 31	11.6	* 43	* 31	10.6				
Max Q Clear Time (g_c+I1), s	3.3	37.7	3.4	8.1	9.6	17.7	29.2	8.3				
Green Ext Time (p_c), s	0.0	1.5	0.0	0.1	0.1	1.0	0.5	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			42.1									
HCM 6th LOS			D									

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - Existing PM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

1: 114th Street & Shea Boulevard

	-	-	1	4	≯	4	1	1	
Phase Number	1	2	3	4	5	6	7	8	
Movement	WBL	EBT	NBL	SBT	EBL	WBT	SBL	NBT	
Lead/Lag	Lag	Lead	Lead	Lag	Lead	Lag	Lag	Lead	
Lead-Lag Optimize	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Max	None	None	None	C-Max	None	None	
Maximum Split (s)	12	55	16	37	18	49	37	16	
Maximum Split (%)	10.0%	45.8%	13.3%	30.8%	15.0%	40.8%	30.8%	13.3%	
Minimum Split (s)	11.4	31.1	10	40.6	11.4	32.1	10.6	37.4	
Yellow Time (s)	4.4	5.1	3	4.4	4.4	5.1	3.6	3.6	
All-Red Time (s)	2	1	2	1.2	2	1	2	1.8	
Minimum Initial (s)	5	10	5	7	5	10	5	7	
Vehicle Extension (s)	2	0.2	2	0.2	2	0.2	2	0.2	
Minimum Gap (s)	3	3	3	3	3	3	3	3	
Time Before Reduce (s)	0	0	0	0	0	0	0	0	
Time To Reduce (s)	0	0	0	0	0	0	0	0	
Walk Time (s)		4		4		4		4	
Flash Dont Walk (s)		21		31		22		28	
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	Yes	Yes	
Inhibit Max	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Start Time (s)	41	106	53	69	106	4	69	53	
End Time (s)	53	41	69	106	4	53	106	69	
Yield/Force Off (s)	46.6	34.9	64	100.4	117.6	46.9	100.4	63.6	
Yield/Force Off 170(s)	46.6	13.9	64	69.4	117.6	24.9	100.4	35.6	
Local Start Time (s)	37	102	49	65	102	0	65	49	
Local Yield (s)	42.6	30.9	60	96.4	113.6	42.9	96.4	59.6	
Local Yield 170(s)	42.6	9.9	60	65.4	113.6	20.9	96.4	31.6	
Intersection Summary									
Cycle Length			120						
Control Type	Actu	ated-Coor	dinated						
Natural Cycle			145						
Offset: 4 (3%), Referenced to	phase 2	EBT and	6:WBT, S	Start of Gr	een				

Splits and Phases: 1: 114th Street & Shea Boulevard

→1Ø2 (R)		Ø 1	↑ ø3		
55 s		12 s	16 s	37 s	
	 Ø6 (R)		¶ø8	Ø7	
18 s	49 s		16 s	37 s	

8-ZN-2020

5/22/2020
Intersection						
Int Delay, s/veh	1.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		4		- ሽ	- 11
Traffic Vol, veh/h	1	28	104	0	10	146
Future Vol, veh/h	1	28	104	0	10	146
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage,	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	31	114	0	11	160

Major/Minor	Minor1	N	1ajor1	Majo	r2	
Conflicting Flow All	216	114	0	0 1	14 0	
Stage 1	114	-	-	-		
Stage 2	102	-	-	-		
Critical Hdwy	6.63	6.23	-	- 4.	13 -	
Critical Hdwy Stg 1	5.43	-	-	-		
Critical Hdwy Stg 2	5.83	-	-	-		
Follow-up Hdwy	3.519	3.319	-	- 2.2	19 -	
Pot Cap-1 Maneuver	762	938	-	- 14	74 -	
Stage 1	910	-	-	-		
Stage 2	911	-	-	-		
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	757	938	-	- 14	74 -	
Mov Cap-2 Maneuver	760	-	-	-		
Stage 1	910	-	-	-		
Stage 2	905	-	-	-		
Approach	WB		NB	ç	SB	

Approach	WB	NB	SB	
HCM Control Delay, s	9	0	0.5	
HCM LOS	А			

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBL	SBT	
Capacity (veh/h)	-	-	930	1474	-	
HCM Lane V/C Ratio	-	-	0.034	0.007	-	
HCM Control Delay (s)	-	-	9	7.5	-	
HCM Lane LOS	-	-	Α	Α	-	
HCM 95th %tile Q(veh)	-	-	0.1	0	-	

114th Street and Shea Boulevard - Existing PM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th TWSC

8-ZN-2020 5/22/2020



Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix G – Prior Approved Site Plan









Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix H – Trip Generation



H 8-ZN-2020 5/22/2020



Existing Development: Bank and Office

Completed: KS 3/16/2020 Checked: SAS 3/24/2020

Trip Generation Calculations (10th Edition)

912 Drive-in Bank																					/	
Landling	ITE	0.00	11	Weekday			AM Peak H	our		PM Peak Ho	ur			Weekday	/	AN	1 Peak H	our	PN	I Peak H	bur	
Land Use	Code	Qty	Unit	Rate	% In	% Out	Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
Drive-in Bank	912	5.25	1000 SF GFA	100.03	50%	50%	9.50	58%	42%	20.45	50%	50%	525	263	262	50	29	21	107	54	53	Average
Drive-in Bank	912	5.25	1000 SF GFA	32.67	50%	50%	0.89	58%	42%	3.04	50%	50%	172	86	86	5	3	2	16	8	8	Minimum
Drive-in Bank	912	5.25	1000 SF GFA	408.42	50%	50%	29.47	58%	42%	109.91	50%	50%	2,144	1,072	1,072	155	90	65	577	289	288	Maximum
Land Lise	ITE	Otv	Unit	Weekday	/		AM Peak H	our		PM Peak Ho	ur			Weekday	/	AN	1 Peak H	our	PN	I Peak H	bur	
Land Use	Code	QU	onic	Equation	%In	% Out	Equation	% In	% Out	Equation	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
Drive-in Bank	912	5.25	1000 SF GFA	T=82.87(X)+117.10	50%	50%	N/A	N/A	N/A	N/A	N/A	N/A	552	276	276	N/A	N/A	N/A	N/A	N/A	N/A	Equation
	Sta	ndard D	eviation	61.61			5.85			15.01												
Drive-in Bank	Nu	imber of	Studies	21			46			115												
brive in ballik		Average Size		7			5			4												
		R ²		0.66			N/A			N/A												
710 General Office Building																						
Land Lice	ITE	Oth	Unit	Weekday	/		AM Peak H	our		PM Peak Ho	ur			Weekday	/	AN	1 Peak H	our	PN	I Peak H	our	
Land Use	Code	Qty	Unit	Rate	% In	% Out	Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	ln	Out	Total	In	Out	
General Office Building	710	5.2	1000 SF GFA	9.74	50%	50%	1.16	86%	14%	1.15	16%	84%	51	26	25	6	5	1	6	1	5	Average
General Office Building	710	5.2	1000 SF GFA	2.71	50%	50%	0.37	86%	14%	0.47	16%	84%	14	7	7	2	2	0	2	0	2	Minimum
General Office Building	710	5.2	1000 SF GFA	27.56	50%	50%	4.23	86%	14%	3.23	16%	84%	143	72	71	22	19	3	17	3	14	Maximum
Land Lise	ITE	Otv	Unit	Weekday	/		AM Peak H	our		PM Peak Ho	ur			Weekday	/	AN	1 Peak H	our	PN	I Peak H	bur	
Land Use	Code	QU	onic	Equation	%In	% Out	Equation	% In	% Out	Equation	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
General Office Building	710	5.2	1000 SF GFA	Ln(T)=0.97Ln(X)+2.50	50%	50%	T=.94(X)+26.49	86%	14%	Ln(T)=0.95Ln(X)+0.36	16%	84%	60	30	30	31	27	4	7	1	6	Equation

	Standard Deviation	5.15	0.47	0.42	
Concerned Officer Duilding	Number of Studies	66	35	32	
General Office Building	Average Size	171	117	114	
	R ²	0.83	0.85	0.88	

(okahi

Completed: KS 3/16/2020 Checked: SAS 3/24/2020

Trip Generation Calculations (10th Edition)

720 Medical-Dental Office Building																						
Land Lice	ITE	Otu	Unit	Weekday			AM Peak Ho	our		PM Peak Ho	our			Weekday	/	AN	1 Peak H	our	PN	1 Peak H	our	
Laild Ose	Code	Qty	Unit	Rate	% In	% Out	Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out	Total	ln	Out	
Medical-Dental Office Building	720	7.2	1000 SF GFA	34.80	50%	50%	2.78	78%	22%	3.46	28%	72%	252	126	126	20	16	4	25	7	18	Average
Medical-Dental Office Building	720	7.2	1000 SF GFA	9.14	50%	50%	0.85	78%	22%	0.25	28%	72%	66	33	33	6	5	1	2	1	1	Minimum
Medical-Dental Office Building	720	7.2	1000 SF GFA	100.75	50%	50%	14.30	78%	22%	8.86	28%	72%	729	365	364	103	80	23	64	18	46	Maximum
Land Lice	ITE	Otu	Unit	Weekday			AM Peak Ho	our		PM Peak Ho	bur			Weekday	/	AN	1 Peak H	our	PN	1 Peak H	our	
Land Use	Code	QU	Onic	Equation	% In	% Out	Equation	% In	% Out	Equation	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
Medical-Dental Office Building	720	7.2	1000 SF GFA	T=38.42(X)-87.62	50%	50%	Ln(T)=0.89Ln(X)+1.31	78%	22%	T=3.39(X)+2.02	28%	72%	190	95	95	22	17	5	27	8	19	Equation
	Sta	ndard D	eviation	9.79			1.28			1.58												
Madical Dantal Office Building	Nu	mber of	Studies	28			44			65												
Medical-Dental Office Building		Average	Size	24			32			28												
		R ²		0.95			0.80			0.73												
820 Shopping Center																						
Land Lice	ITE			Weekdav			AM Peak Ho	our		PM Peak Ho	our			Weekday	/	AN	1 Peak H	our	PN	1 Peak H	our	
		()+>/	l linit																			
Land Use	Code	Qty	Unit	Rate	% In	% Out	Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
Shopping Center	Code 820	Qty 8.1	1000 SF GLA	Rate 37-75	% In 50%	% Out 50%	Rate 0.94	% In 62%	38%	Rate 3.81	% In 48%	% Out 52%	Total 306	In 153	Out 153	Total 8	In 5	Out 3	Total 31	In 15	Out 16	Average
Shopping Center	Code 820 820	Qty 8.1 8.1	1000 SF GLA 1000 SF GLA	Rate 37.75 7.42	% In 50% 50%	<mark>% Out</mark> 50%	Rate 0.94 0.18	<mark>% In</mark> 62% 62%	38%	Rate 3.81 0.74	% In 48% 48%	% Out 52% 52%	Total 306 60	In 153 30	Out 153 30	Total 8 1	In 5 1	Out 3 0	Total 31 6	In 15 3	Out 16 3	Average Minimum
Shopping Center Shopping Center Shopping Center	Code 820 820 820	Qty 8.1 8.1 8.1	Unit 1000 SF GLA 1000 SF GLA 1000 SF GLA	Rate 37.75 7.42 207.98	% In 50% 50%	 % Out 50% 50% 	Rate 0.94 0.18 23.74	% In 62% 62% 62%	38% 38% 38%	Rate 3.81 0.74 18.69	% In 48% 48% 48%	% Out 52% 52% 52%	Total 306 60 1,685	In 153 30 843	Out 153 30 842	Total 8 1 192	In 5 1 119	Out 3 0 73	Total 31 6 151	In 15 3 72	Out 16 3 79	Average Minimum Maximum
Shopping Center Shopping Center Shopping Center	Code 820 820 820 ITE	Qty 8.1 8.1 8.1	1000 SF GLA 1000 SF GLA 1000 SF GLA	Rate 37-75 7-42 207.98 Weekday	% In 50% 50%	<pre>% Out 50% 50%</pre>	Rate 0.94 0.18 23.74 AM Peak Ho	% In 62% 62% 62% 62% 0ur	38% 38% 38%	Rate 3.81 0.74 18.69 PM Peak Hc	% In 48% 48% 48%	% Out 52% 52% 52%	Total 306 60 1,685	In 153 30 843 Weekday	Out 153 30 842	Total 8 1 192 AN	In 5 1 119 1 Peak H	Out 3 0 73 our	Total 31 6 151 PM	In 15 3 72 1 Peak H	Out 16 3 79 Dur	Average Minimum Maximum
Shopping Center Shopping Center Land Use	Code 820 820 820 ITE Code	Qty 8.1 8.1 8.1 Qty	1000 SF GLA 1000 SF GLA 1000 SF GLA Unit	Rate 37.75 7.42 207.98 Weekday Equation	<pre>% In 50% 50% 50% % In</pre>	 % Out 50% 50% 50% % Out 	Rate 0.94 0.18 23.74 AM Peak Ho Equation	% In 62% 62% 62% 0ur % In	X Out 38% 38% 38% X Out	Rate 3.81 0.74 18.69 PM Peak Ho Equation	% In 48% 48% 48% 48% ur % In	% Out 52% 52% 52% % Out	Total 306 60 1,685 Total	In 153 30 843 Weekday In	Out 153 30 842 Out	Total 8 1 192 AW Total	In 5 1 119 I Peak H In	Out 3 0 73 Our Out	Total 31 6 151 PM Total	In 15 3 72 Peak He In	Out 16 3 79 Our Out	Average Minimum Maximum
Shopping Center Shopping Center Land Use Shopping Center	Code 820 820 820 ITE Code 820	Qty 8.1 8.1 8.1 8.1 8.1 8.1 8.1	Unit 1000 SF GLA Unit 1000 SF GLA	Rate 37-75 7.42 207.98 Weekday Equation Ln(T)=0.68Ln(X)+5.57	<pre>% In 50% 50% 50% % In 50%</pre>	 % Out 50% 50% % Out 50% 	Rate 0.94 0.18 23.74 AM Peak Ho Equation T=0.50(X)+151.78	% In 62% 62% 62% 0ur % In 62%	% Out 38% 38% 38% % Out 38%	Rate 3.81 0.74 18.69 PM Peak Hc Equation Ln(T)=0.74Ln(X)+2.89	% In 48% 48% 48% our % In 48%	% Out 52% 52% 52% 52% % Out 52%	Total 306 60 1,685 Total 1,088	In 153 30 843 Weekday In 544	Out 153 30 842 7 Out 544	Total 8 1 192 AN Total 156	In 5 1 119 1 Peak H In 97	Out 3 0 73 our Out 59	Total 31 6 151 PM Total 85	In 15 3 72 Peak H In 41	Out 16 3 79 Our Out 44	Average Minimum Maximum Equation
Shopping Center Shopping Center Land Use Shopping Center	Code 820 820 820 ITE Code 820	Qty 8.1 8.1 Qty 8.1 Qty	Unit 1000 SF GLA 1000 SF GLA Unit 1000 SF GLA	Rate 37.75 7.42 207.98 Weekday Equation Ln(T)=0.68Ln(X)+5.57	<pre>% In 50% 50% % In 50%</pre>	 % Out 50% 50% % Out 50% 	Rate 0.94 0.18 23.74 AM Peak Ho Equation T=0.50(X)+151.78	% In 62% 62% 62% 0ur % In 62%	% Out 38% 38% 38% % Out 38%	Rate 3.81 0.74 18.69 PM Peak Ho Equation Ln(T)=0.74Ln(X)+2.89	% In 48% 48% 48% 0ur % In 48%	% Out 52% 52% 52% % Out 52%	Total 306 60 1,685 Total 1,088	In 153 30 843 Weekday In 544	Out 153 30 842 / Out 544	Total 8 1 192 AM Total 156	In 5 1 119 Peak H In 97	Out 3 0 73 Our Out 59	Total 31 6 151 PN Total 85 1000000000000000000000000000000000000	In 15 3 72 1 Peak H In 41	Out 16 3 79 Our Out 44	Average Minimum Maximum Equation
Shopping Center Shopping Center Land Use Shopping Center	Code 820 820 820 ITE Code 820 Sta	Qty 8.1 8.1 Qty 8.1 Qty	Unit 1000 SF GLA 1000 SF GLA Unit 1000 SF GLA eviation	Rate 37.75 7.42 207.98 Weekday Equation Ln(T)=0.68Ln(X)+5.57 16.41	<pre>% In 50% 50% 50% % In 50%</pre>	% Out 50% 50% 50% 50% 50% 50%	Rate 0.94 0.18 23.74 AM Peak Ho Equation T=0.50(X)+151.78 0.87	% In 62% 62% 62% 0ur % In 62%	% Out 38% 38% 38% % Out 38%	Rate 3.81 0.74 18.69 PM Peak Ho Equation Ln(T)=0.74Ln(X)+2.89 2.04	% In 48% 48% 48% 0ur % In 48%	% Out 52% 52% 52% % Out 52%	Total 306 60 1,685 Total 1,088	In 153 30 843 Weekday In 544	Out 153 30 842 Out 544	Total 8 1 192 AW Total 156 1	In 5 1 119 Peak H In 97	Out 3 0 73 our Out 59	Total 31 6 151 PN Total 85 1000000000000000000000000000000000000	In 15 3 72 1 Peak H In 41	Out 16 3 79 Our Out 44	Average Minimum Maximum Equation
Shopping Center Shopping Center Land Use Shopping Center Shopping Center Cand Use	Code 820 820 820 ITE Code 820 Sta Nu	Qty 8.1 8.1 Qty 8.1 Qty 8.1 Other 0 0 0 8.1	Unit 1000 SF GLA 1000 SF GLA Unit 1000 SF GLA Unit 1000 SF GLA eviation Studies	Rate 37-75 7.42 207.98 Weekday Equation Ln(T)=0.68Ln(X)+5.57 16.41 147	<pre>% In 50% 50% % In 50%</pre>	<pre>% Out 50% 50% % Out 50%</pre>	Rate 0.94 0.18 23.74 AM Peak Ho Equation T=0.50(X)+151.78 0.87 84	% In 62% 62% 62% 62% 0ur % In 62%	% Out 38% 38% 38% 38% % Out 38%	Rate 3.81 0.74 18.69 PM Peak Ho Equation Ln(T)=0.74Ln(X)+2.89 2.04 261	% In 48% 48% 48% our % In 48%	 % Out 52% 52% % Out 52% 	Total 306 60 1,685 Total 1,088	In 153 30 843 Weekday In 544	Out 153 30 842 7 Out 544	Total 8 1 192 AW Total 156 1	In 5 1 119 1 Peak H In 97	Out 3 0 73 our Out 59	Total 31 6 151 PM Total 85 1000000000000000000000000000000000000	In 15 3 72 1 Peak H In 41	Out 16 3 79 Our Out 44	Average Minimum Maximum Equation
Shopping Center Shopping Center Land Use Shopping Center Shopping Center Shopping Center	Code 820 820 820 ITE Code 820 Sta Nu	Qty 8.1 8.1 Qty 8.1 ndard D mber of Average	Unit 1000 SF GLA 1000 SF GLA Unit 1000 SF GLA Eviation Studies Size	Rate 37.75 7.42 207.98 Weekday Equation Ln(T)=0.68Ln(X)+5.57 16.41 147 453	<pre>% In 50% 50% % In 50%</pre>	 % Out 50% 50% % Out 50% 	Rate 0.94 0.18 23.74 AM Peak Ho Equation T=0.50(X)+151.78 0.87 84 351	% In 62% 62% 0ur % In 62%	% Out 38% 38% 38% 38% 38%	Rate 3.81 0.74 18.69 PM Peak Ho Equation Ln(T)=0.74Ln(X)+2.89 2.04 261 327	<pre>% In 48% 48% 48% 000 % In 48%</pre>	 % Out 52% 52% % Out 52% 	Total 306 60 1,685 Total 1,088	In 153 30 843 Weekday In 544	Out 153 30 842 Out 544	Total 8 1 192 AW Total 156 1	In 5 1 119 1 Peak H In 97	Out 3 0 73 our Out 59	Total 31 6 151 PN Total 85	In 15 3 72 1 Peak H In 41	Out 16 3 79 Our Out 44	Average Minimum Maximum Equation





Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix I – MAG Socioeconomic Projections



8-ZN-2020 5/22/2020

Socioeconomic Projections

Population and Employment

by Municipal Planning Area, Jurisdiction, and Regional Analysis Zone

June 2019



302 North 1st Avenue, Suite 300 Phoenix, Arizona 85003 (602) 254-6300

8-ZN-2020

5/22/2020

Maricopa Association of Governments Table 1: Total Population by Municipal Planning Area July 1, 2018 and Projections July 1, 2020 to July 1, 2055

	Total Population											
Municipal Planning Area	2018	2020	2030	2040	2050	2055						
Apache Junction	59,000	60,800	70,000	92,000	117,100	132,600						
Avondale	84,200	86,700	101,800	111,900	119,000	122,100						
Buckeye	89,000	97,700	186,600	305,400	409,900	459,300						
Carefree	3,700	3,800	4,100	4,200	4,200	4,300						
Cave Creek	5,900	6,000	6,500	7,000	7,200	7,300						
Chandler	270,300	279,500	309,100	321,100	329,000	332,400						
El Mirage	34,300	35,100	36,500	36,900	37,200	37,200						
Florence	79,400	85,500	120,300	160,500	209,900	231,400						
Fort McDowell Yavapai Native Nation	1,000	1,100	1,100	1,100	1,100	1,100						
Fountain Hills	24,000	24,700	26,200	26,600	26,900	27,000						
Gila Bend	2,500	2,700	3,700	3,700	3,900	4,200						
Gila River Indian Native Nation	12,000	12,200	12,300	12,300	12,300	12,300						
Gilbert	256,500	265,900	293,500	308,800	318,100	321,400						
Glendale	272,200	279,100	306,400	323,400	333,200	338,800						
Goodyear	87,300	92,100	140,300	192,200	228,600	247,900						
Guadalupe	6,300	6,400	6,700	6,800	6,800	6,800						
Litchfield Park	13,300	14,000	15,400	15,700	16,100	16,400						
Maricopa	59,800	67,000	90,800	106,400	121,600	128,900						
Mesa	533,400	552,800	607,500	649,400	680,000	690,300						
Paradise Valley	14,000	14,100	14,700	15,100	15,200	15,300						
Peoria	188,500	196,600	232,400	273,700	312,600	329,900						
Phoenix	1,653,500	1,697,700	1,881,900	2,019,300	2,117,400	2,155,300						
Queen Creek	58,700	65,000	90,900	109,000	120,900	128,500						
Salt River Pima-Maricopa Native Nation	6,800	6,100	5,700	5,800	5,800	5,800						
Scottsdale	245,500	253,800	281,900	299,400	311,400	316,700						
Surprise	144,000	150,300	216,700	307,500	383,300	417,200						
Тетре	185,300	190,000	217,100	247,000	272,400	282,200						
Tolleson	7,000	7,100	8,600	10,300	11,400	11,800						
Unicorporated Pinal County	66,800	68,600	79,100	93,700	110,800	122,700						
Unincorporated Maricopa County	97,900	101,200	110,500	116,800	137,000	152,600						
Wickenburg	8,200	8,500	9,400	9,500	9,800	10,000						
Youngtown	6,600	6,800	7,300	7,700	7,800	7,800						

Notes: Numbers rounded to the nearest 100. These projections include both the Maricopa County and Pinal County portions for Apache Junction, Queen Creek, and the Gila River Indian Community. Peoria and Wickenburg include only the Maricopa County portion.

Source: Maricopa Association of Governments (MAG) Socioeconomic Projections of Population and Employment by Municipal Planning Area (MPA) and Regional Analysis Zone (RAZ), June 2019

For explanation of variables and complete notation on this series, please refer to the Notes and Caveats in Appendix A.

Maricopa Association of Governments Table 2: Total Employment by Municipal Planning Area July 1, 2018 and Projections July 1, 2020 to July 1, 2055

	Total Employment											
Municipal Planning Area	2018	2020	2030	2040	2050	2055						
Apache Junction	7,800	8,800	13,100	17,800	26,400	30,500						
Avondale	22,400	23,200	30,400	36,200	42,800	45,400						
Buckeye	21,600	26,900	42,900	64,500	98,000	128,900						
Carefree	1,600	1,600	2,100	2,400	2,500	2,600						
Cave Creek	2,200	2,400	2,700	2,900	3,000	3,200						
Chandler	145,500	154,700	182,300	202,100	215,200	222,000						
El Mirage	5,000	5,100	6,500	7,200	8,000	8,900						
Florence	11,000	12,100	17,000	26,400	40,900	51,100						
Fort McDowell Yavapai Native Nation	2,200	2,400	2,400	2,500	2,600	2,600						
Fountain Hills	7,100	7,700	9,100	9,800	10,200	10,300						
Gila Bend	900	900	1,200	1,300	1,500	1,700						
Gila River Indian Native Nation	10,500	10,700	11,500	13,100	14,800	15,500						
Gilbert	92,800	98,600	120,200	135,900	146,600	152,200						
Glendale	103,800	111,400	134,000	153,100	168,900	175,900						
Goodyear	35,900	37,200	50,600	69,000	92,600	102,500						
Guadalupe	1,300	1,300	1,500	1,600	1,600	1,600						
Litchfield Park	3,800	4,400	5,200	5,900	6,400	6,700						
Maricopa	6,200	7,100	11,400	18,200	28,200	33,500						
Mesa	197,200	205,900	249,000	296,000	333,700	351,000						
Paradise Valley	6,300	6,300	6,800	7,100	7,500	7,700						
Peoria	58,200	62,400	73,100	84,800	91,900	96,300						
Phoenix	897,700	937,600	1,084,000	1,189,200	1,264,900	1,298,900						
Queen Creek	15,500	16,400	19,900	24,000	28,900	31,100						
Salt River Pima-Maricopa Native Nation	21,200	22,900	28,200	33,900	35,900	36,400						
Scottsdale	197,200	207,400	235,500	252,000	261,700	267,000						
Surprise	33,600	36,400	59,500	86,400	113,400	130,500						
Тетре	190,000	200,500	231,200	257,700	280,000	290,900						
Tolleson	17,700	18,300	21,200	23,900	26,000	26,700						
Unicorporated Pinal County	3,500	3,900	6,000	8,900	13,500	17,800						
Unincorporated Maricopa County	28,600	31,500	35,500	41,100	51,200	58,400						
Wickenburg	4,400	4,600	5,200	5,600	6,000	6,200						
Youngtown	1,500	1,800	2,200	2,700	2,800	3,100						

Notes: Numbers rounded to the nearest 100. These projections include both the Maricopa County and Pinal County portions for Apache Junction, Queen Creek, and the Gila River Indian Community. Peoria and Wickenburg include only the Maricopa County portion.

Source: Maricopa Association of Governments (MAG) Socioeconomic Projections of Population and Employment by Municipal Planning Area (MPA) and Regional Analysis Zone (RAZ), June 2019

For explanation of variables and complete notation on this series, please refer to the Notes and Caveats in Appendix A.

Regional Analysis Zones (RAZ), 2019 Maricopa and Pinal Counties, Arizona





While every effort has been made to ensure the accuracy of this information, the Maricopa Association of Governments makes no warranty, expressed or implied, as to its ac liability for the accur 8-ZN-2020

5/22/2020

liability for the accur

Maricopa Association of Governments Table 4: Population by Regional Analysis Zone (RAZ) by MPA July 1, 2018 and Projections July 1, 2020 to July 1, 2055

				Total Pop	ulation		
RAZ	County	2018	2020	2030	2040	2050	2055
	Total	1,653,469	1,697,722	1,881,876	2,019,269	2,117,427	2,155,333
Queen	Creek MPA						
339	Maricopa County	49,781	53,579	72,670	82,172	87,155	89,586
422	Pinal County	13	13	300	437	564	638
423	Pinal County	1,286	1,410	3,714	6,136	7,457	8,686
424	Pinal County	7,642	10,003	14,200	20,287	25,759	29,586
	Total	58,722	65,005	90,884	109,032	120,935	128,496
Salt Riv	er Pima-Maricopa Native N	lation MPA					
264	Maricopa County	6,798	6,073	5,708	5,820	5,820	5,820
	Total	6,798	6,073	5,708	5,820	5,820	5,820
Scottsd	ale MPA						
209	Maricopa County	12,188	12,605	13,961	14,512	14,984	15,255
210	Maricopa County	6,013	6,591	10,463	12,339	13,491	13,961
229	Maricopa County	20,542	21,269	25,221	27,864	29,698	30,229
230	Maricopa County	32,232	33,028	38,882	43,580	46,789	48,510
247	Maricopa County	13,549	13,858	15,420	16,342	16,871	17,019
248	Maricopa County	36,178	37,227	38,468	38,807	39,048	39,143
249	Maricopa County	20,903	21,410	22,543	22,768	22,839	22,848
263	Maricopa County	34,908	35,814	37,002	37,252	37,584	37,773
272	Maricopa County	68,987	71,970	79,910	85,942	90,054	91,927
	Total	245,500	253,772	281,870	299,406	311,358	316,665
Surpris	e MPA						
211	Maricopa County	863	884	4,471	23,112	36,704	40,737
212	Maricopa County	10,265	11,365	37,615	69,296	85,862	93,806
232	Maricopa County	29,296	30,200	34,506	37,144	37,927	38,313
233	Maricopa County	87,834	91,276	111,822	119,384	123,777	126,523
234	Maricopa County	8,969	9,467	10,460	10,878	11,335	11,488
371	Maricopa County	342	344	434	734	2,584	4,316
504	Maricopa County	6,460	6,/18	17,425	46,912	85,127	102,004
	Total	144,029	150,254	216,733	307,460	383,316	417,187
Tempe	МРА						
288	Maricopa County	73,442	76,444	100,651	129,202	150,094	157,410
297	Maricopa County	53,146	54,092	56,336	57,432	61,780	64,273
308	Maricopa County	58,756	59,473	60,120	60,348	60,476	60,559
	Total	185,344	190,009	217,107	246,982	272,350	282,242

Notes: Numbers rounded to the nearest 100. These projections include both the Maricopa County and Pinal County portions for Apache Junction, Queen Creek, and the Gila River Indian Community. Peoria and Wickenburg include only the Maricopa County portion. Source: Maricopa Association of Governments (MAG) Socioeconomic Projections of Population and Employment by Municipal Planning Area (MPA) and Regional Analysis Zone (RAZ), May 2019

For explanation of variables and complete notation on this series, please refer to the Notes and Caveats in Appendix A.



Maricopa Association of Governments Table 5: Employment by Regional Analysis Zone (RAZ) by MPA July 1, 2018 and Projections July 1, 2020 to July 1, 2055

				Total Emp	loyment		
RAZ	County	2018	2020	2030	2040	2050	2055
	Total	897,713	937,622	1,083,980	1,189,209	1,264,941	1,298,903
Queen	Creek MPA						
339	Maricopa County	13,933	14,696	16,482	18,825	20,733	21,151
422	Pinal County	9	8	18	22	31	39
423	Pinal County	89	109	351	620	1,068	1,639
424	Pinal County	1,435	1,576	3,073	4,571	7,020	8,309
	Total	15,466	16,389	19,924	24,038	28,852	31,138
Salt Riv	ver Pima-Maricopa Native N	ation MPA					
264	Maricopa County	21,160	22,869	28,215	33,871	35,903	36,442
	Total	21,160	22,869	28,215	33,871	35,903	36,442
Scottsd	lale MPA						
209	Maricopa County	4,488	4,659	4,851	5,174	5,161	5,344
210	Maricopa County	2,386	3,018	2,759	3,091	3,139	3,191
229	Maricopa County	9,604	10,005	11,231	11,962	12,193	12,896
230	Maricopa County	23,272	24,919	32,112	36,968	40,834	42,136
247	Maricopa County	44,254	47,089	52,652	54,822	55,679	56,105
248	Maricopa County	29,603	30,901	33,285	34,001	34,234	34,548
249	Maricopa County	7,409	7,692	8,179	8,684	8,906	9,045
263	Maricopa County	26,351	26,961	28,903	30,245	30,919	31,381
272	Maricopa County	49,833	52,185	61,540	67,039	70,676	72,330
	Total	197,200	207,429	235,512	251,986	261,741	266,976
Surpris	e MPA						
211	Maricopa County	60	53	1,560	3,172	4,766	7,017
212	Maricopa County	2,008	2,338	5,821	9,965	13,362	15,709
232	Maricopa County	8,349	9,228	11,297	12,187	12,875	13,116
233	Maricopa County	19,943	21,079	32,661	44,032	52,007	57,402
234	Maricopa County	2,588	2,711	3,354	3,922	4,239	4,386
371	Maricopa County	18	20	327	423	2,381	2,937
504	Maricopa County	677	1,020	4,460	12,695	23,763	29,886
	Total	33,643	36,449	59,480	86,396	113,393	130,453
Tempe	МРА						
288	Maricopa County	88,927	94,229	111,010	128,894	144,714	152,703
297	Maricopa County	44,730	47,069	53,149	57,125	60,725	62,552
308	Maricopa County	56,380	59,208	67,052	71,701	74,542	75,596
	Total	190,037	200,506	231,211	257,720	279,981	290,851

Notes: Numbers rounded to the nearest 100. These projections include both the Maricopa County and Pinal County portions for Apache Junction, Queen Creek, and the Gila River Indian Community. Peoria and Wickenburg include only the Maricopa County portion. Source: Maricopa Association of Governments (MAG) Socioeconomic Projections of Population and Employment by Municipal Planning Area (MPA) and Regional Analysis Zone (RAZ), May 2019

For explanation of variables and complete notation on this series, please refer to the Notes and Caveats in Appendix A.



Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix J – Year 2021 No Build Capacity Analysis



J 8-ZN-2020 5/22/2020

03/30/2020	
------------	--

	≯	-	$\mathbf{\hat{v}}$	4	+	•	٠	Ť	۲	1	Ļ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ኘኘ	4 4 12		7	^	1	ኘኘ	eî 🗧		ሻሻ	^	1
Traffic Volume (veh/h)	90	1135	34	10	1709	473	54	96	15	515	38	147
Future Volume (veh/h)	90	1135	34	10	1709	473	54	96	15	515	38	147
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	98	1234	37	11	1858	0	59	104	16	560	41	160
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	148	2203	66	302	2843		144	130	20	415	582	259
Arrive On Green	0.04	0.43	0.43	0.17	0.56	0.00	0.04	0.08	0.08	0.12	0.16	0.16
Sat Flow, veh/h	3456	5094	153	1781	5106	1585	3456	1583	244	3456	3554	1585
Grp Volume(v), veh/h	98	825	446	11	1858	0	59	0	120	560	41	160
Grp Sat Flow(s),veh/h/ln	1728	1702	1843	1781	1702	1585	1728	0	1827	1728	1777	1585
Q Serve(g_s), s	3.4	21.8	21.8	0.6	30.4	0.0	2.0	0.0	7.7	14.4	1.2	11.3
Cycle Q Clear(g_c), s	3.4	21.8	21.8	0.6	30.4	0.0	2.0	0.0	7.7	14.4	1.2	11.3
Prop In Lane	1.00		0.08	1.00		1.00	1.00		0.13	1.00		1.00
Lane Grp Cap(c), veh/h	148	1472	797	302	2843		144	0	150	415	582	259
V/C Ratio(X)	0.66	0.56	0.56	0.04	0.65		0.41	0.00	0.80	1.35	0.07	0.62
Avail Cap(c_a), veh/h	161	1472	797	302	2843		173	0	374	415	989	441
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	56.6	25.5	25.5	41.6	18.5	0.0	56.1	0.0	54.1	52.8	42.5	46.7
Incr Delay (d2), s/veh	6.3	1.5	2.8	0.0	1.2	0.0	0.7	0.0	3.7	172.9	0.0	0.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.5	8.5	9.5	0.3	11.0	0.0	0.9	0.0	3.7	16.1	0.5	4.5
Unsig. Movement Delay, s/veh					(a =			• •	•		10 -	
LnGrp Delay(d),s/veh	62.9	27.0	28.3	41.6	19.7	0.0	56.8	0.0	57.8	225.7	42.5	47.6
LnGrp LOS	E	С	C	D	В		E	A	E	F	D	<u> </u>
Approach Vol, veh/h		1369			1869	А		179			761	
Approach Delay, s/veh		30.0			19.8			57.5			178.4	
Approach LOS		С			В			E			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	26.8	58.0	10.0	25.2	11.6	73.2	20.0	15.2				
Change Period (Y+Rc), s	6.4	6.1	5.0	* 5.6	6.4	* 6.4	* 5.6	5.4				
Max Green Setting (Gmax), s	5.6	51.9	6.0	* 33	5.6	* 52	* 14	24.6				
Max Q Clear Time (g_c+l1), s	2.6	23.8	4.0	13.3	5.4	32.4	16.4	9.7				
Green Ext Time (p_c), s	0.0	1.1	0.0	0.0	0.0	2.4	0.0	0.1				
Intersection Summary												
HCM 6th Ctrl Delay			53.7									
HCM 6th LOS			D									

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - 2021 No Build AM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

	4	-	•	4	۶	4	1	t	
Phase Number	1	2	3	4	5	6	7	8	5
Movement	WBL	EBT	NBL	SBT	EBL	WBT	SBL	NBT	•
Lead/Lag	Lag	Lead	Lead	Lag	Lead	Lag	Lag	Lead	
Lead-Lag Optimize	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	;
Recall Mode	None	C-Max	None	None	None	C-Max	None	None	;
Maximum Split (s)	12	58	11	39	12	58	20	30	1
Maximum Split (%)	10.0%	48.3%	9.2%	32.5%	10.0%	48.3%	16.7%	25.0%)
Minimum Split (s)	11.4	31.1	10	40.6	11.4	32.1	10.6	37.4	,
Yellow Time (s)	4.4	5.1	3	4.4	4.4	5.1	3.6	3.6	i
All-Red Time (s)	2	1	2	1.2	2	1	2	1.8	,
Minimum Initial (s)	5	10	5	7	5	10	5	7	ſ
Vehicle Extension (s)	2	0.2	2	0.2	2	0.2	2	0.2	
Minimum Gap (s)	3	3	3	3	3	3	3	3	•
Time Before Reduce (s)	0	0	0	0	0	0	0	0)
Time To Reduce (s)	0	0	0	0	0	0	0	0	1
Walk Time (s)		4		4		4		4	,
Flash Dont Walk (s)		21		31		22		28	;
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	Yes	Yes	;
Inhibit Max	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	;
Start Time (s)	90	32	102	113	32	44	12	102	
End Time (s)	102	90	113	32	44	102	32	12	
Yield/Force Off (s)	95.6	83.9	108	26.4	37.6	95.9	26.4	6.6	i
Yield/Force Off 170(s)	95.6	62.9	108	115.4	37.6	73.9	26.4	98.6	;
Local Start Time (s)	46	108	58	69	108	0	88	58	,
Local Yield (s)	51.6	39.9	64	102.4	113.6	51.9	102.4	82.6	į
Local Yield 170(s)	51.6	18.9	64	71.4	113.6	29.9	102.4	54.6	i

Intersection Summary

Cycle Length 120 Control Type Actuated-Coordinated 125

Natural Cycle

Offset: 44 (37%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Splits and Phases: 1: 114th Street & Shea Boulevard

•1Ø2 (R)	,	Ø1	▲ Ø3	∲ Ø4		
58 s		12 s	11 s	39 s		
∕ ø₅	 Ø6 (R)		[†] ø8		Ø7	
12 s	58 s		30 s		20 s	

Synchro 10 Report Timing Report, Sorted By Phase



Intersection						
Int Delay, s/veh	1.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	- Y		4			- 11
Traffic Vol, veh/h	0	12	151	3	30	48
Future Vol, veh/h	0	12	151	3	30	48
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	13	164	3	33	52

Major/Minor	Minor1	N	1ajor1	Ν	lajor2		
Conflicting Flow All	258	166	0	0	167	0	
Stage 1	166	-	-	-	-	-	
Stage 2	92	-	-	-	-	-	
Critical Hdwy	6.63	6.23	-	-	4.13	-	
Critical Hdwy Stg 1	5.43	-	-	-	-	-	
Critical Hdwy Stg 2	5.83	-	-	-	-	-	
Follow-up Hdwy	3.519	3.319	-	-	2.219	-	
Pot Cap-1 Maneuver	720	878	-	-	1410	-	
Stage 1	863	-	-	-	-	-	
Stage 2	922	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	703	878	-	-	1410	-	
Mov Cap-2 Maneuver	722	-	-	-	-	-	
Stage 1	863	-	-	-	-	-	
Stage 2	901	-	-	-	-	-	
Approach	WB		NB		SB		
HCM Control Delay, s	9.2		0		2.9		
HCM LOS	A						

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBL	SBT	
Capacity (veh/h)	-	-	878	1410	-	
HCM Lane V/C Ratio	-	-	0.015	0.023	-	
HCM Control Delay (s)	-	-	9.2	7.6	-	
HCM Lane LOS	-	-	Α	А	-	
HCM 95th %tile Q(veh)	-	-	0	0.1	-	

114th Street and Shea Boulevard - 2021 No Build AM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th TWSC

8-ZN-2020 5/22/2020

03/30/2020	
------------	--

	۶	-	$\mathbf{\hat{z}}$	4	-	•	٠	Ť	۲	5	Ļ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	<u>ተተ</u> ኑ		٦	^	1	ሻሻ	eî 🗧		ሻሻ	^	1
Traffic Volume (veh/h)	218	1682	43	20	902	531	42	76	19	785	91	104
Future Volume (veh/h)	218	1682	43	20	902	531	42	76	19	785	91	104
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	237	1828	47	22	980	0	46	83	21	853	99	113
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	292	2086	54	113	1959		144	106	27	898	1049	468
Arrive On Green	0.08	0.41	0.41	0.06	0.38	0.00	0.04	0.07	0.07	0.26	0.30	0.30
Sat Flow, veh/h	3456	5119	132	1781	5106	1585	3456	1440	364	3456	3554	1585
Grp Volume(v), veh/h	237	1215	660	22	980	0	46	0	104	853	99	113
Grp Sat Flow(s),veh/h/ln	1728	1702	1847	1781	1702	1585	1728	0	1805	1728	1777	1585
Q Serve(g_s), s	8.1	39.5	39.5	1.4	17.6	0.0	1.6	0.0	6.8	29.1	2.4	6.5
Cycle Q Clear(g_c), s	8.1	39.5	39.5	1.4	17.6	0.0	1.6	0.0	6.8	29.1	2.4	6.5
Prop In Lane	1.00		0.07	1.00		1.00	1.00		0.20	1.00		1.00
Lane Grp Cap(c), veh/h	292	1387	753	113	1959		144	0	133	898	1049	468
V/C Ratio(X)	0.81	0.88	0.88	0.20	0.50		0.32	0.00	0.78	0.95	0.09	0.24
Avail Cap(c_a), veh/h	334	1387	753	113	1959		317	0	159	904	1049	468
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.0	32.8	32.8	53.3	28.2	0.0	55.8	0.0	54.7	43.6	30.7	32.1
Incr Delay (d2), s/veh	10.9	8.0	13.7	0.3	0.9	0.0	0.5	0.0	15.4	18.6	0.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/In	3.8	16.6	19.2	0.6	6.9	0.0	0.7	0.0	3.7	14.4	1.0	2.5
Unsig. Movement Delay, s/veh					•• (
LnGrp Delay(d),s/veh	64.9	40.8	46.4	53.6	29.1	0.0	56.3	0.0	70.0	62.2	30.7	32.2
LnGrp LOS	E	D	D	D	C		E	A	E	E	С	<u> </u>
Approach Vol, veh/h		2112			1002	A		150			1065	
Approach Delay, s/veh		45.2			29.7			65.8			56.1	
Approach LOS		D			С			E			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.0	55.0	10.0	41.0	16.6	52.4	36.8	14.2				
Change Period (Y+Rc), s	6.4	6.1	5.0	* 5.6	6.4	* 6.4	* 5.6	5.4				
Max Green Setting (Gmax), s	5.6	48.9	11.0	* 31	11.6	* 43	* 31	10.6				
Max Q Clear Time (g_c+l1), s	3.4	41.5	3.6	8.5	10.1	19.6	31.1	8.8				
Green Ext Time (p_c), s	0.0	1.5	0.0	0.1	0.1	1.1	0.1	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			45.0									
HCM 6th LOS			D									

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - 2021 No Build PM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

	4	→	1	4	٦	4	1	1	
Phase Number	1	2	3	4	5	6	7	8	
Movement	WBL	EBT	NBL	SBT	EBL	WBT	SBL	NBT	
Lead/Lag	Lag	Lead	Lead	Lag	Lead	Lag	Lag	Lead	
Lead-Lag Optimize	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Max	None	None	None	C-Max	None	None	
Maximum Split (s)	12	55	16	37	18	49	37	16	
Maximum Split (%)	10.0%	45.8%	13.3%	30.8%	15.0%	40.8%	30.8%	13.3%	
Minimum Split (s)	11.4	31.1	10	40.6	11.4	32.1	10.6	37.4	
Yellow Time (s)	4.4	5.1	3	4.4	4.4	5.1	3.6	3.6	
All-Red Time (s)	2	1	2	1.2	2	1	2	1.8	
Minimum Initial (s)	5	10	5	7	5	10	5	7	
Vehicle Extension (s)	2	0.2	2	0.2	2	0.2	2	0.2	
Minimum Gap (s)	3	3	3	3	3	3	3	3	
Time Before Reduce (s)	0	0	0	0	0	0	0	0	
Time To Reduce (s)	0	0	0	0	0	0	0	0	
Walk Time (s)		4		4		4		4	
Flash Dont Walk (s)		21		31		22		28	
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	Yes	Yes	
Inhibit Max	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Start Time (s)	41	106	53	69	106	4	69	53	
End Time (s)	53	41	69	106	4	53	106	69	
Yield/Force Off (s)	46.6	34.9	64	100.4	117.6	46.9	100.4	63.6	
Yield/Force Off 170(s)	46.6	13.9	64	69.4	117.6	24.9	100.4	35.6	
Local Start Time (s)	37	102	49	65	102	0	65	49	
Local Yield (s)	42.6	30.9	60	96.4	113.6	42.9	96.4	59.6	
Local Yield 170(s)	42.6	9.9	60	65.4	113.6	20.9	96.4	31.6	
Intersection Summary									

Cycle Length

Control Type Actuated-Coordinated

Natural Cycle 145 Offset: 4 (3%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Splits and Phases: 1: 114th Street & Shea Boulevard



120

Synchro 10 Report Timing Report, Sorted By Phase

8-ZN-2020

5/22/2020

Intersection						
Int Delay, s/veh	1.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		4		- ሽ	- 11
Traffic Vol, veh/h	2	29	106	0	11	148
Future Vol, veh/h	2	29	106	0	11	148
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	2	32	115	0	12	161

Major/Minor	Minor1	Ν	1ajor1	Ν	lajor2							
Conflicting Flow All	220	115	0	0	115	0						
Stage 1	115	-	-	-	-	-						
Stage 2	105	-	-	-	-	-						
Critical Hdwy	6.63	6.23	-	-	4.13	-						
Critical Hdwy Stg 1	5.43	-	-	-	-	-						
Critical Hdwy Stg 2	5.83	-	-	-	-	-						
Follow-up Hdwy	3.519	3.319	-	-	2.219	-						
Pot Cap-1 Maneuver	758	937	-	-	1473	-						
Stage 1	909	-	-	-	-	-						
Stage 2	908	-	-	-	-	-						
Platoon blocked, %			-	-		-						
Mov Cap-1 Maneuver	752	937	-	-	1473	-						
Mov Cap-2 Maneuver	756	-	-	-	-	-						
Stage 1	909	-	-	-	-	-						
Stage 2	901	-	-	-	-	-						
Approach	\\/D		ND		CD							

Approach	WB	NB	SB	
HCM Control Delay, s	9	0	0.5	
HCM LOS	А			

Minor Lane/Major Mvmt	NBT	NBRW	BLn1	SBL	SBT
Capacity (veh/h)	-	-	923	1473	-
HCM Lane V/C Ratio	-	-	0.037	0.008	-
HCM Control Delay (s)	-	-	9	7.5	-
HCM Lane LOS	-	-	А	А	-
HCM 95th %tile Q(veh)	-	-	0.1	0	-

114th Street and Shea Boulevard - 2021 No Build PM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th TWSC

8-ZN-2020 5/22/2020



Hawkins Companies Traffic Impact & Mitigation Analysis

Appendix K – Year 2021 Build Capacity Analysis





05/07/2020	
------------	--

	≯	-	\mathbf{r}	4	←	•	1	Ť	۲	1	Ļ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	<u> ተተ</u> ኈ		۲	^	1	ሻሻ	4Î		ካካ	^	7
Traffic Volume (veh/h)	90	1135	40	21	1709	473	58	97	18	515	42	147
Future Volume (veh/h)	90	1135	40	21	1709	473	58	97	18	515	42	147
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	98	1234	43	23	1858	0	63	105	20	560	46	160
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	148	2191	76	297	2827		144	130	25	415	592	264
Arrive On Green	0.04	0.43	0.43	0.17	0.55	0.00	0.04	0.09	0.09	0.12	0.17	0.17
Sat Flow, veh/h	3456	5066	177	1781	5106	1585	3456	1527	291	3456	3554	1585
Grp Volume(v), veh/h	98	829	448	23	1858	0	63	0	125	560	46	160
Grp Sat Flow(s),veh/h/ln	1728	1702	1839	1781	1702	1585	1728	0	1818	1728	1777	1585
Q Serve(g_s), s	3.4	21.9	21.9	1.3	30.6	0.0	2.1	0.0	8.1	14.4	1.3	11.2
Cycle Q Clear(g_c), s	3.4	21.9	21.9	1.3	30.6	0.0	2.1	0.0	8.1	14.4	1.3	11.2
Prop In Lane	1.00		0.10	1.00		1.00	1.00		0.16	1.00		1.00
Lane Grp Cap(c), veh/h	148	1472	795	297	2827		144	0	155	415	592	264
V/C Ratio(X)	0.66	0.56	0.56	0.08	0.66		0.44	0.00	0.81	1.35	0.08	0.61
Avail Cap(c_a), veh/h	161	1472	795	297	2827		173	0	373	415	989	441
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	56.6	25.5	25.5	42.2	18.8	0.0	56.1	0.0	53.9	52.8	42.2	46.3
Incr Delay (d2), s/veh	6.3	1.6	2.9	0.0	1.2	0.0	0.8	0.0	3.8	172.9	0.0	0.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.5	8.6	9.6	0.6	11.1	0.0	0.9	0.0	3.9	16.1	0.6	4.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	62.9	27.1	28.4	42.3	20.0	0.0	56.9	0.0	57.7	225.7	42.2	47.2
LnGrp LOS	E	С	С	D	В		E	А	E	F	D	D
Approach Vol, veh/h		1375			1881	А		188			766	
Approach Delay, s/veh		30.1			20.3			57.4			177.4	
Approach LOS		С			С			E			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	26.4	58.0	10.0	25.6	11.6	72.8	20.0	15.6				
Change Period (Y+Rc), s	6.4	6.1	5.0	* 5.6	6.4	* 6.4	* 5.6	5.4				
Max Green Setting (Gmax), s	5.6	51.9	6.0	* 33	5.6	* 52	* 14	24.6				
Max Q Clear Time (g_c+l1), s	3.3	23.9	4.1	13.2	5.4	32.6	16.4	10.1				
Green Ext Time (p_c), s	0.0	1.1	0.0	0.0	0.0	2.4	0.0	0.1				
Intersection Summary												
HCM 6th Ctrl Delay			53.7									
HCM 6th LOS			D									

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - 2021 Build AM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

	4	-	•	4	۶	+	1	Ť	
Phase Number	1	2	3	4	5	6	7	8	
Movement	WBL	EBT	NBL	SBT	EBL	WBT	SBL	NBT	
Lead/Lag	Lag	Lead	Lead	Lag	Lead	Lag	Lag	Lead	
Lead-Lag Optimize	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Recall Mode	None	C-Max	None	None	None	C-Max	None	None	
Maximum Split (s)	12	58	11	39	12	58	20	30	
Maximum Split (%)	10.0%	48.3%	9.2%	32.5%	10.0%	48.3%	16.7%	25.0%	
Minimum Split (s)	11.4	31.1	10	40.6	11.4	32.1	10.6	37.4	
Yellow Time (s)	4.4	5.1	3	4.4	4.4	5.1	3.6	3.6	
All-Red Time (s)	2	1	2	1.2	2	1	2	1.8	
Minimum Initial (s)	5	10	5	7	5	10	5	7	
Vehicle Extension (s)	2	0.2	2	0.2	2	0.2	2	0.2	
Minimum Gap (s)	3	3	3	3	3	3	3	3	
Time Before Reduce (s)	0	0	0	0	0	0	0	0	
Time To Reduce (s)	0	0	0	0	0	0	0	0	
Walk Time (s)		4		4		4		4	
Flash Dont Walk (s)		21		31		22		28	
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	Yes	Yes	
Inhibit Max	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Start Time (s)	90	32	102	113	32	44	12	102	
End Time (s)	102	90	113	32	44	102	32	12	
Yield/Force Off (s)	95.6	83.9	108	26.4	37.6	95.9	26.4	6.6	
Yield/Force Off 170(s)	95.6	62.9	108	115.4	37.6	73.9	26.4	98.6	
Local Start Time (s)	46	108	58	69	108	0	88	58	
Local Yield (s)	51.6	39.9	64	102.4	113.6	51.9	102.4	82.6	
Local Yield 170(s)	51.6	18.9	64	71.4	113.6	29.9	102.4	54.6	

Intersection Summary

Cycle Length 120 Control Type Actuated-Coordinated 125

Natural Cycle

Offset: 44 (37%), Referenced to phase 2:EBT and 6:WBT, Start of Green

Splits and Phases: 1: 114th Street & Shea Boulevard

→Ø2 (R)	1	Ø1	Ø 3	♦ Ø4				
58 s		12 s	11 s	39 s				
∕ ø₅	 Ø6 (R)		¶ø8		Ø7			
12 s	58 s		30 s		20 s			

Synchro 10 Report Timing Report, Sorted By Phase



2: 114	th Street 8	& Driveway	B/Mirage	Crossing	Driveway	/
			·		1	

05/07/2020

	≯	-	\rightarrow	1	-	•	٠	Ť	1	1	↓	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		5	ţ,		Ä	† 12	
Traffic Volume (veh/h)	8	0	0	0	0	12	1	151	3	30	48	0
Future Volume (Veh/h)	8	0	0	0	0	12	1	151	3	30	48	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	0	0	0	0	13	1	164	3	33	52	0
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL			None	
Median storage veh)								2				
Upstream signal (ft)											407	
pX, platoon unblocked												
vC, conflicting volume	297	287	26	260	286	166	52			167		
vC1, stage 1 conf vol	118	118		168	168							
vC2, stage 2 conf vol	179	169		92	118							
vCu, unblocked vol	297	287	26	260	286	166	52			167		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)	6.5	5.5		6.5	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	99	100	100	100	100	98	100			98		
cM capacity (veh/h)	714	682	1044	763	697	850	1552			1408		
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2	SB 3					
Volume Total	9	13	1	167	33	35	17					
Volume Left	9	0	1	0	33	0	0					
Volume Right	0	13	0	3	0	0	0					
cSH	714	850	1552	1700	1408	1700	1700					
Volume to Capacity	0.01	0.02	0.00	0.10	0.02	0.02	0.01					
Queue Length 95th (ft)	1	1	0	0	2	0	0					
Control Delay (s)	10.1	9.3	7.3	0.0	7.6	0.0	0.0					
Lane LOS	В	А	А		А							
Approach Delay (s)	10.1	9.3	0.0		3.0							
Approach LOS	В	А										
Intersection Summary												
Average Delay			1.7									
Intersection Capacity Utilization	ation		28.1%	IC	CU Level of	of Service			А			
Analysis Period (min)			15									

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1		↑	_ ≜ î≽	
Traffic Vol, veh/h	0	0	0	171	78	21
Future Vol, veh/h	0	0	0	171	78	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	0	186	85	23

Major/Minor	Minor ₂	Ν	/lajor1	Ν	/lajor2	
Conflicting Flow All	-	54	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.93	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.319	-	-	-	-
Pot Cap-1 Maneuver	0	1002	0	-	-	-
Stage 1	0	-	0	-	-	-
Stage 2	0	-	0	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	-	1002	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Annroach	FR		NR		SB	
HCM Control Dology					00	
HCM LOS	0		0		U	
	A					
Minor Lane/Major Mvr	nt	NBT E	BLn1	SBT	SBR	
Capacity (veh/h)		-	-	-	-	
HCM Lane V/C Ratio		-	_	_	_	

HCM Lane V/C Ratio	-	-	-	-		
HCM Control Delay (s)	-	0	-	-		
HCM Lane LOS	-	А	-	-		
HCM 95th %tile Q(veh)	-	-	-	-		

8-ZN-2020

5/22/2020

05/07/2020

	۶	-	$\mathbf{\hat{z}}$	4	+	•	٠	Ť	۲	5	Ļ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	<u>ተተኑ</u>		۲.	^	1	ሻሻ	eî 🗧		ሻሻ	^	1
Traffic Volume (veh/h)	218	1682	53	27	902	531	50	82	38	785	96	104
Future Volume (veh/h)	218	1682	53	27	902	531	50	82	38	785	96	104
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	237	1828	58	29	980	0	54	89	41	853	104	113
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	292	2029	64	111	1913		144	108	50	875	1081	482
Arrive On Green	0.08	0.40	0.40	0.06	0.37	0.00	0.04	0.09	0.09	0.25	0.30	0.30
Sat Flow, veh/h	3456	5084	161	1781	5106	1585	3456	1212	558	3456	3554	1585
Grp Volume(v), veh/h	237	1223	663	29	980	0	54	0	130	853	104	113
Grp Sat Flow(s),veh/h/ln	1728	1702	1841	1781	1702	1585	1728	0	1770	1728	1777	1585
Q Serve(g_s), s	8.1	40.4	40.5	1.9	17.8	0.0	1.8	0.0	8.7	29.4	2.5	6.4
Cycle Q Clear(g_c), s	8.1	40.4	40.5	1.9	17.8	0.0	1.8	0.0	8.7	29.4	2.5	6.4
Prop In Lane	1.00		0.09	1.00		1.00	1.00		0.32	1.00		1.00
Lane Grp Cap(c), veh/h	292	1359	735	111	1913		144	0	158	875	1081	482
V/C Ratio(X)	0.81	0.90	0.90	0.26	0.51		0.38	0.00	0.82	0.97	0.10	0.23
Avail Cap(c_a), veh/h	334	1359	735	111	1913		346	0	186	875	1081	482
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.0	33.8	33.8	53.6	29.0	0.0	56.0	0.0	53.7	44.4	29.9	31.3
Incr Delay (d2), s/veh	10.9	9.8	16.4	0.5	1.0	0.0	0.6	0.0	19.3	24.1	0.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/In	3.8	17.4	20.1	0.8	7.1	0.0	0.8	0.0	4.7	15.1	1.1	2.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	64.9	43.6	50.2	54.1	30.0	0.0	56.6	0.0	73.0	68.5	29.9	31.4
LnGrp LOS	E	D	D	D	C		E	A	E	E	C	<u> </u>
Approach Vol, veh/h		2123			1009	А		184			1070	
Approach Delay, s/veh		48.1			30.7			68.2			60.8	
Approach LOS		D			С			E			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	13.9	54.0	10.0	42.1	16.6	51.4	36.0	16.1				
Change Period (Y+Rc), s	6.4	6.1	5.0	* 5.6	6.4	* 6.4	* 5.6	5.4				
Max Green Setting (Gmax), s	5.6	47.9	12.0	* 31	11.6	* 42	* 30	12.6				
Max Q Clear Time (g_c+l1), s	3.9	42.5	3.8	8.4	10.1	19.8	31.4	10.7				
Green Ext Time (p_c), s	0.0	1.4	0.0	0.1	0.1	1.1	0.0	0.0				
Intersection Summary												
HCM 6th Ctrl Delay			48.0									
HCM 6th LOS			D									

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

114th Street and Shea Boulevard - 2021 Build PM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th Signalized Intersection Summary

	4	-	1	4	٦		1	Ť	
Phase Number	1	2	3	4	5	6	7	8	
Movement	WBL	EBT	NBL	SBT	EBL	WBT	SBL	NBT	
Lead/Lag	Lag	Lead	Lead	Lag	Lead	Lag	Lag	Lead	
Lead-Lag Optimize	Yes								
Recall Mode	None	C-Max	None	None	None	C-Max	None	None	
Maximum Split (s)	12	54	17	37	18	48	36	18	
Maximum Split (%)	10.0%	45.0%	14.2%	30.8%	15.0%	40.0%	30.0%	15.0%	
Minimum Split (s)	11.4	31.1	10	40.6	11.4	32.1	10.6	37.4	
Yellow Time (s)	4.4	5.1	3	4.4	4.4	5.1	3.6	3.6	
All-Red Time (s)	2	1	2	1.2	2	1	2	1.8	
Minimum Initial (s)	5	10	5	7	5	10	5	7	
Vehicle Extension (s)	2	0.2	2	0.2	2	0.2	2	0.2	
Minimum Gap (s)	3	3	3	3	3	3	3	3	
Time Before Reduce (s)	0	0	0	0	0	0	0	0	
Time To Reduce (s)	0	0	0	0	0	0	0	0	
Walk Time (s)		4		4		4		4	
Flash Dont Walk (s)		21		31		22		28	
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	Yes	Yes	
Inhibit Max	Yes								
Start Time (s)	40	106	52	69	106	4	70	52	
End Time (s)	52	40	69	106	4	52	106	70	
Yield/Force Off (s)	45.6	33.9	64	100.4	117.6	45.9	100.4	64.6	
Yield/Force Off 170(s)	45.6	12.9	64	69.4	117.6	23.9	100.4	36.6	
Local Start Time (s)	36	102	48	65	102	0	66	48	
Local Yield (s)	41.6	29.9	60	96.4	113.6	41.9	96.4	60.6	
Local Yield 170(s)	41.6	8.9	60	65.4	113.6	19.9	96.4	32.6	
Intersection Summary									

intersection Summar

Cycle Length	120
Control Type	Actuated-Coordinated

Natural Cycle

Offset: 4 (3%), Referenced to phase 2:EBT and 6:WBT, Start of Green

120

145

Splits and Phases: 1: 114th Street & Shea Boulevard



Synchro 10 Report Timing Report, Sorted By Phase



2: 114th Street & Driveway B/Mirage Crossing Driveway

05/07/2020

	≯	-	\rightarrow	1	-	•	٩.	1	1	L#	1	ŧ
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT
Lane Configurations		\$			\$		ኘ	eî 👘			a a a a a a a a a a a a a a a a a a a	
Traffic Volume (veh/h)	31	0	2	2	0	29	1	106	0	2	11	148
Future Volume (Veh/h)	31	0	2	2	0	29	1	106	0	2	11	148
Sign Control		Stop			Stop			Free				Free
Grade		0%			0%			0%				0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	34	0	2	2	0	32	1	115	0	0	12	161
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								TWLTL				None
Median storage veh)								2				
Upstream signal (ft)												407
pX, platoon unblocked										0.00		
vC, conflicting volume	334	302	80	224	302	115	161			0	115	
vC1, stage 1 conf vol	185	185		117	117							
vC2, stage 2 conf vol	149	117		106	185							
vCu, unblocked vol	334	302	80	224	302	115	161			0	115	
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			0.0	4.1	
tC, 2 stage (s)	6.5	5.5		6.5	5.5							
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			0.0	2.2	
p0 queue free %	95	100	100	100	100	97	100			0	99	
cM capacity (veh/h)	703	690	963	800	692	916	1416			0	1472	
Direction, Lane #	EB 1	WB 1	NB 1	NB 2	SB 1	SB 2	SB 3					
Volume Total	36	34	1	115	12	107	54					
Volume Left	34	2	1	0	12	0	0					
Volume Right	2	32	0	0	0	0	0					
cSH	714	908	1416	1700	1472	1700	1700					
Volume to Capacity	0.05	0.04	0.00	0.07	0.01	0.06	0.03					
Queue Length 95th (ft)	4	3	0	0	1	0	0					
Control Delay (s)	10.3	9.1	7.5	0.0	7.5	0.0	0.0					
Lane LOS	В	А	А		А							
Approach Delay (s)	10.3	9.1	0.1		0.5							
Approach LOS	В	А										
Intersection Summary												
Average Delay			2.2									
Intersection Capacity Utiliza	ation		22.6%	IC	CU Level	of Service			А			
Analysis Period (min)			15									

	-
L .	-
Movement	SBR
Lareconfigurations	
Traffic Volume (veh/h)	0
Future Volume (Veh/h)	0
Sign Control	
Grade	
Peak Hour Factor	0.92
Hourly flow rate (vph)	0
Pedestrians	
Lane Width (ft)	
Walking Speed (ft/s)	
Percent Blockage	
Right turn flare (veh)	
Median type	
Median storage veh)	
Upstream signal (ft)	
pX, platoon unblocked	
vC, conflicting volume	
vC1, stage 1 conf vol	
vC2, stage 2 conf vol	
vCu, unblocked vol	
tC, single (s)	
tC, 2 stage (s)	
tF (s)	
p0 queue free %	
cM capacity (veh/h)	
Direction, Lane #	



Intersection						
Int Delay, s/veh	0.1					
	EDI			NDT	ODT	000
Movement	EBL	EBK	NBL	NRI	SBT	SBR
Lane Configurations		1		↑	_ ≜î ≽	
Traffic Vol, veh/h	0	2	0	168	159	22
Future Vol, veh/h	0	2	0	168	159	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	2	0	183	173	24

Major/Minor	Minor2	Ν	/lajor1	Ν	lajor2		
Conflicting Flow All	-	99	-	0	-	0	
Stage 1	-	-	-	-	-	-	
Stage 2	-	-	-	-	-	-	
Critical Hdwy	-	6.93	-	-	-	-	
Critical Hdwy Stg 1	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	
Follow-up Hdwy	-	3.319	-	-	-	-	
Pot Cap-1 Maneuver	0	938	0	-	-	-	
Stage 1	0	-	0	-	-	-	
Stage 2	0	-	0	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	-	938	-	-	-	-	
Mov Cap-2 Maneuver	-	-	-	-	-	-	
Stage 1	-	-	-	-	-	-	
Stage 2	-	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay s	8.8		0		0		
HCMLOS	A		v		Ū		
Minor Lane/Major Mvn	nt	NBT E	BLn1	SBT	SBR		
Capacity (veh/h)		-	938	-	-		
HCM Lane V/C Ratio		-	0.002	-	-		

HCM Lane V/C Ratio	- 0.002	-	-	
HCM Control Delay (s)	- 8.8	-	-	
HCM Lane LOS	- A	-	-	
HCM 95th %tile Q(veh)	- 0	-	-	

114th Street and Shea Boulevard - 2021 Build PM Peak Hour Lokahi, LLC

Synchro 10 Report HCM 6th TWSC

8-ZN-2020 5/22/2020