

PRELIMINARY DRAINAGE REPORT

Senior Living Scottsdale + Osborn

3380 N Scottsdale Road, Scottsdale AZ 85251

Prepared For:



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1. INTRODUCTION

This Preliminary Drainage Report represents the storm water analysis for a multi-family residential development proposed in Scottsdale, Arizona. The purpose of this report is to provide the hydrologic and hydraulic analysis, required by the City of Scottsdale, to support the proposed site plan for said development. This report includes discussions and calculations defining the storm water management concepts for the collection and conveyance necessary to comply with the drainage requirements of the City of Scottsdale and Maricopa County. Preparation of this report has been done in accordance with the requirements of the City of Scottsdale Design Standards & Policies Manual (DS&PM) 2018¹, and the Drainage Design Manuals for Maricopa County, Arizona, Volumes I² and Volume II³.

2. LOCATION AND PROJECT DESCRIPTION

2.1 LOCATION:

The subject property consists of land located in a portion of the Southeast Quarter of Section 27, Township 2 North, Range 4 East of the Gila and Salt Meridian, Maricopa County Arizona:

- Parcel ID: Parcel 130-16-114; Zoning is D/DMU-2 (Downtown Multiple Use)
- Address: 3380 N. Scottsdale Road, Scottsdale 85251

Refer to **FIGURE 1 - Vicinity Map** for the project's location with respect to major cross streets

2.2 EXISTING AND PROPOSED DEVELOPMENTS SURROUNDING THE SITE:

- South:
 - The site is bound to the south by the following properties:
 - Parcel 130-16-115; The Carter (apartment complex); Zoning is C-3
- North: Across Osborn Road
 - Parcel 130-13-316; Ten Wife Lofts (apartments/condominiums); Zoning is D/OC-2.
 - Parcel 130-13-046 and 130-13-044A; Walgreens Drug Store; Zoning is C-3.
- West: Across 71st Street
 - Parcel 130-16-087; First Baptist Church of Scottsdale; Zoning is R-5
- East: Across Scottsdale Road
 - Parcel 130-21-001U; Gasoline Service Station and a convenience market; Zoning is C-3.

2.3 EXISTING SITE DESCRIPTION:

The project area includes approximately 2.57 acres of land and is designated with zoning D/DMU-2. The site is currently developed as an Olive Garden restaurant with associated parking lot, driveways, and landscape areas.

Per Topographic Survey prepared by AW Land Surveying, LLC., the site slopes from north to south at approximately less than 1%. The site drains south into existing catch basins along the property line.

Refer to **FIGURE 2** attached for an aerial of the site.

2.4 PROPOSED SITE DEVELOPMENT:

Site development includes the demolition of existing structures and designated parking lots for the construction of a new high density multifamily project. The development will include two access points proposed at Osborn Road and 71st Street.

Refer to **Appendix III** – Grading and Drainage Plan for site layout.

2.5 FLOOD HAZARD ZONE:

FIRM Map Number 04013C2235M dated September 18, 2020 indicates the site is designated as Zone "X". As such, it is defined as areas determined to be outside the 0.2% annual chance floodplain and therefore is not in a special flood hazard area.

Refer to **FIGURE 3** for the FIRM.

3. EXISTING DRAINAGE CONDITIONS

3.1 OFF-SITE DRAINAGE PATTERNS

The topographic survey provides the following information for offsite drainage:

- *North:* Half of the runoff from Osborn Road flows towards the site, where it is conveyed through curb and gutter into an existing catch basin, EX-CB-6, located northeast of the site at the SWC of Osborn Road and Scottsdale Road. No offsite flows from the north affect the site.
- *East:* Half of the runoff from Scottsdale Road flows towards the site, where it is conveyed through curb and gutter and flows southerly away from the site. No offsite flows from the east affect the site.
- *West:* Half of the runoff from N. 71st is also conveyed via existing curb and gutter. Approximately half of this runoff is collected by an existing catch basin, EX-CB-7, located near the southwestern corner of the property. The other half of the runoff flows north into Osborn Road. No offsite flows from the west affect the site.
- *South:* Runoff from the apartment complex south of the site is captured by existing catch basins inside the apartment complex's property. No offsite flows from the south affect the site.

3.2 ON-SITE DRAINAGE

Based on the topographic information, the historical outfalls are as follows:

- Flows from drainage areas EX-A1 through EX-A4 are collected by existing catch basins located at the south along the property line and retained in existing underground basin (Ex-Basin 1). Flows from drainage area EX-5 is collected by existing catch basin EX-CB-5 and retained by existing underground retention basin (Ex-Basin 2). According to Plan C1.4 “Stormdrain Plan” by Hilgart Wilson, Ex-Basin 1 and Ex-Basin 2 provide a volume of 4,516 cf and 4,123 cf respectively. Refer to Appendix IV for Plan C1.4 “Stormdrain Plan” by Hilgart Wilson.
- Drainage area EX-B1 is self-retaining.
- Runoff from drainage area EX-C1 flows into Osborn Road, where it is ultimately collected by EX-CB-6.
- All flows draining into existing on-site catch basins are stored in two underground storage tanks located south of the property (Ex-Basin 1 and Ex-Basin 2).

Refer to Appendix II for **Existing Conditions Drainage Area Map**.

Table 1 below is a summary of existing conditions runoff calculations:

TABLE 1:

EXISTING SITE DISCHARGES									
	TOTAL AREA	Cwt	Intensity 10 yr 5-min	Q 10	Intensity 100 yr 5-min	Q 100	Control Point	Total flows Q10	Total flows Q100
	(ac)	(-)	(in/hr)	(cfs)	(in/hr)	(cfs)	CP#	(cfs)	(cfs)
	1.34		4.68	-	7.43	-	-	-	-
EX-A1	0.32	0.87	4.68	1.30	7.43	2.06	EX-CB-1	11.07	17.58
EX-A2	0.33	0.90	4.68	1.40	7.43	2.22	EX-CB-2		
EX-A3	0.36	0.88	4.68	1.50	7.43	2.38	EX-CB-3		
EX-A4	0.33	0.89	4.68	1.37	7.43	2.17	EX-CB-4		
EX-A5	0.93	0.87	4.68	3.77	7.43	5.99	EX-CB-5		
EX-B1	0.04	0.45	4.68	0.09	7.43	0.14	EX-CB-1		
EX-C1	0.26	0.61	4.68	0.75	7.43	1.19	Osborn Road		
OFF-2	0.11	0.83	4.68	0.43	7.43	0.68			
OFF-1	0.06	0.73	4.68	0.20	7.43	0.32			
OFF-3	0.08	0.73	4.68	0.27	7.43	0.43	Scottsdale Road		

Overall project area includes **2.57 Acres at C_{wt} = 0.84** (Existing conditions)

Refer to the **Existing Cwt Exhibit and Existing Conditions Drainage Area Map** in **Appendix II**.

4. PROPOSED STORM WATER MANAGEMENT

4.1 DESIGN INTENT:

Given that the site has been previously developed, on-site retention shall be calculated per City of Scottsdale DSPM 4-1.201. In order to preserve existing drainage patterns, most of the on-site drainage will discharge to the historical outlets, a portion of the site run-off will also be stored in basins to fulfill stormwater retention and first flush requirements. The majority of the building's roof run-off will be directed to existing catch basins south of the site (EX-CB-1 to EX-CB-5) as in existing conditions. A portion of the roof will be drained to a proposed underground storage tank located at the south boundary of the property, which will store the minimum pre-vs-post associated volume of the site. Landscape and sidewalk areas around the project building will discharge minor flows to the right of way. Flows will be conveyed via roof drains, storm pipes and overland flow.

4.2 DESIGN STORM REQUIREMENTS:

In accordance with City of Scottsdale requirements for lots that are already developed, stormwater storage for the 100-year 2-hour storm event is required based on maintaining existing retention volume plus the difference between the pre vs. post development runoff from the 100-year 2-hour storm event if increased or first flush, whichever is greater.

4.3 LAND CHARACTERISTICS:

The proposed project site consists of a multi-family residential building with a restaurant and landscape areas along the perimeter of the structure. Based on the DS&PM, runoff coefficients for the 100-year storm event used are as follows:

- C=0.95 for building or concrete
- C=0.95 for paved surface
- C=0.45 for undisturbed natural desert or desert landscape

HYDROLOGIC ANALYSIS: The hydrologic analysis is determined using the procedures in the City of Scottsdale Design Standards & Policies Manual and the Drainage Design Manual for Maricopa County, Arizona, Volume I.

Table 2 below is a summary of proposed conditions runoff calculations:

TABLE 2:

PROPOSED SITE DISCHARGES									
	TOTAL AREA	Cwt	Intensity 10 yr 5-min	Q 10	Intensity 100 yr 5-min	Q 100	Control Point	Total flows Q10	Total flows Q100
	(ac)	(-)	(in/hr)	(cfs)	(in/hr)	(cfs)	CP#	(cfs)	(cfs)
	0.92		4.68	-	7.43	-	-	-	-
DA-A1	0.17	0.95	4.68	0.73	7.43	1.17	11.99	19.03	
DA-A2	0.26	0.95	4.68	1.17	7.43	1.86			
DA-A3	0.18	0.95	4.68	0.81	7.43	1.29			
DA-A4	0.30	0.95	4.68	1.36	7.43	2.15			
DA-A5	0.28	0.95	4.68	1.24	7.43	1.96			
DA-A6	0.54	0.95	4.68	2.41	7.43	3.83			
DA-A7	0.26	0.95	4.68	1.14	7.43	1.81			
DA-A8	0.08	0.95	4.68	0.37	7.43	0.59			
DA-E1	0.21	0.95	4.68	0.95	7.43	1.51			
DA-E2	0.03	0.95	4.68	0.16	7.43	0.25			
DA-B1	0.06	0.56	4.68	0.17	7.43	0.27			
OFF-1	0.06	0.80	4.68	0.22	7.43	0.36			
DA-C1	0.18	0.72	4.68	0.59	7.43	0.94			
OFF-2	0.11	0.70	4.68	0.36	7.43	0.58			
DA-D1	0.01	0.73	4.68	0.03	7.43	0.04			
OFF-3	0.08	0.73	4.68	0.27	7.43	0.43			

Overall project area includes **2.57 Acres at C_{wt} = 0.92** (Proposed conditions)

Refer to the **Proposed Cwt Exhibit and Proposed Conditions Drainage Area Map** in **Appendix II**.

4.4 STORMWATER RETENTION:

100-YR, 2-HR STORM: Per City of Scottsdale DSPM 4-1.201, development storage requirements for the 100-yr, 2-hr storm event are calculated as follows:

$$V_r = \Delta C \left(\frac{R}{12} \right) A$$

where:

V_r = Required storage (cf)

R = Precipitation amount =2.16 in per NOAA Atlas 14 Precipitation Frequency Estimates

A = Total area of site (sf)

$\Delta C = C_{post} - C_{pre}$

$$V_r = (0.92 - 0.84) \left(\frac{2.16}{12} \right) (112,097) = 1,615 \text{ cf}$$

Since the difference of the weighted coefficients is positive, stormwater flows in the project area will increase, generating additional flow contributions to existing drainage patterns. Therefore, stormwater retention is required for the development following the pre vs. post analysis.

FIRST FLUSH: First Flush storage required is calculated in accordance with City of Scottsdale DSPM 4-1.201. Only the areas where runoff could be affected by vehicular contact are considered in the first flush calculation. The roof drainage is considered to be free of heavy traffic pollutants, therefore, on-site driveway areas and sidewalks will be considered for the calculation. As shown in the Proposed Conditions Cwt Exhibit, first flush area is calculated as the total project area (112,097 sf) minus roof area (89,482 sf) and landscape areas (5821 sf), equating to 16,794 sf.

$$FF_r = C \left(\frac{P}{12} \right) A$$

where:

FF_r = First Flush required storage volume (cf)

P = Precipitation amount =0.5 in per C.O.S. DSPM

A = Area of site excluding roofs and landscape (sf)

C = The weighted average runoff coefficient =0.95

$$FF_r = (0.95) \left(\frac{0.5}{12} \right) 16,794 = 665 \text{ cf}$$

The above assessment indicates that the required First Flush storage is 665 cf.

Retention shall be provided for the greater of Pre vs Post and First Flush volumes, therefore on-site retention will be designed to store the Pre vs. Post volume. (1,615 cf). The provided storage volume will also fulfill the First Flush requirement. The proposed on-site retention will store flows from drainage areas E-1 and E-2 to (1,856 CF) to comply with Pre vs Post and First Flush volumes. Refer to **Appendix II** for calculations.

Provided storage of *Basin 1*:

Basin 1 will consist of a 6' diameter corrugated metal pipe and will have a length of 70 lf.

$V_p = \pi * \text{Pipe radius}^2 * \text{Pipe length}$

$$V_p = (\pi * 3^2) * (70) = 1,979 \text{ cf}$$

The proposed basin has enough capacity to store the required additional volume generated from the Pre vs. Post analysis.

4.6 STORMWATER DISCHARGE

Bleed-off for the underground retention basin (Basin 1) will be provided by an 8" pipe connected to existing catch basin EX-CB-3, which conveys flow to the public storm drain system. A 1.2" orifice plate will be placed on CB-1 to reduce bleed off rate. The bleed-off rate can be calculated by using the following formula:

$$Q = C_d A \sqrt{2gh}$$

where:

Q= Orifice flow (cfs)

C_d = Orifice coefficient = 0.65

A = Area of orifice (sf) = 0.01227 for a 1.5" orifice

g = Acceleration from gravity (fps) = 32.2

h = Head (ft)

$$Q = (0.65) * (0.01227) \sqrt{2(32.2)(0.08)} = 0.01811 \text{ cfs}$$

The proposed 8" bleed-off pipe at a slope of 1% has a full capacity of 1.21 cfs. Since the calculated orifice flow is smaller than the capacity of the pipe, the orifice flow rate will govern. At this rate, the detention volume will be drained in $(V_p / 0.1811) / 3600 = 30.36$ hours

Pre vs post discharges

Proposed conditions will ultimately reduce site flow contributions to the existing public storm drain system. Even though the overall run-off coefficient of the site will be increased by 0.8, provided on-site retention will enable run-off reductions to the public drains.

Table 3 below summarizes the project discharges per outfall for the 10-year and 100-year storm events, providing the differences between existing and proposed peak flows for each case. The overall site contributions to the existing public drain system are also shown for the drainage areas that will ultimately discharge to it either through catch basins or the proposed manhole connection.

TABLE 3:

Outfall							Ultimate flows to public storm drain system			
	Q10 (cfs)			Q100 (cfs)			Q10 (cfs)		Q100 (cfs)	
	Existing	Proposed	Δ	Existing	Proposed	Δ	Existing	Proposed	Existing	Proposed
N. 71 St	0.29	0.39	0.10	0.46	0.63	0.17	11.08	10.87	17.58	17.30
Osborn Road	1.18	0.95	-0.23	1.87	1.52	-0.35				
Scottsdale Road	0.27	0.30	0.03	0.43	0.47	0.04				
EX-CB-1	1.30	1.90	0.60	2.06	3.03	0.97				
EX-CB-2	1.40	2.17	0.77	2.22	3.44	1.22				
EX-CB-3	1.5	0.00	-1.50	2.38	0.02	-2.36				
EX-CB-4	1.37	0.37	-1.00	2.17	0.59	-1.58				
EX-CB-5	3.77	4.79	1.02	5.99	7.6	1.61				

During the 100-year storm event, discharges to the overall public storm drain system will be decreased by 0.28 cfs. Discharge to N. 71 St will be increased by 0.17 cfs while peak flows to Osborn Road will be decreased by 0.35 cfs. Discharge to Scottsdale Road is increased by 0.04 cfs. Discharge to EX-CB-1 will be increased by 0.97. Discharge to EX-CB-2 will be increased by 1.22. Discharge to EX-CB-3 and EX-CB-4 will be decreased by 2.36 cfs and 1.58 cfs respectively due to the proposed on-site retention. Discharge to EX-CB-5 will be increased by 1.61 cfs. Street and inlet capacities will be verified at later stages of the project with the newly modified peak flows to check hydraulics of existing gutter sections in adjacent streets.

Refer to **Existing Conditions Drainage Area Map** and **Proposed Conditions Drainage Area Map** in **Appendix II**.

4.7 ADEQ WATER QUALITY REQUIREMENTS

The total disturbed area of this site is approximately 2.57 acres. The Arizona Department of Environmental Quality requires that any site disturbance over an acre is required to submit an NOI. A NOI will be submitted to ADEQ for this site after the first submittal of the construction documents as this site disturbance is over 1 acre.

5. FLOOD SAFETY FOR DWELLINGS

5.1 FINISHED FLOOR ELEVATIONS

This project lies in an "X" Flood Zone. Therefore, the proposed building finished floor elevation will be set a minimum of 14 inches above the lot ultimate outfall, located at the southeast corner of the site at an elevation of 1246.88'.

6. CONCLUSIONS

6.1 OVERALL PROJECT:

1. The finish floor elevations will be designed a minimum of 14 inches above the low top of curb of the lot.
2. Most of the historical outfalls will be affected at proposed conditions and overall discharge to the public storm drain system will be reduced.

3. On-site storage facilities will be provided to account for the Pre vs. Post volume and First Flush volumes.

6.2 PROJECT PHASING:

This project will be constructed in a single phase.

7. WARNING AND DISCLAIMER OF LIABILITY

RE: following page.

8. REFERENCES

1. *Design Standards & Policies Manual, City of Scottsdale – January 2018*
2. *Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology, Flood Control District of Maricopa County, Fourth Edition, December 14, 2018*
3. *Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics, Flood Control District of Maricopa County, December 14, 2018*



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FIGURES



FIGURE 1. VICINITY MAP

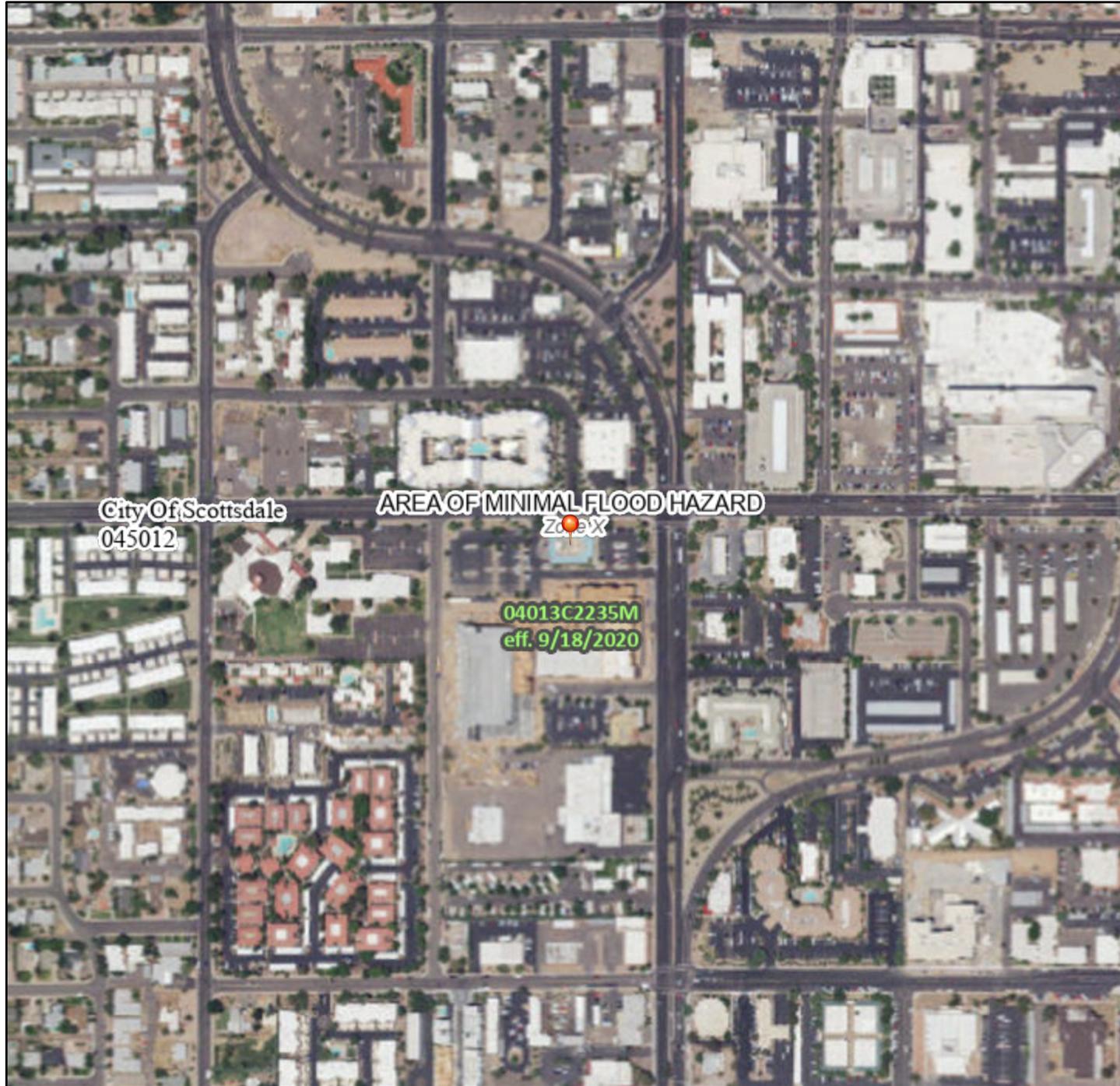


FIGURE 2. AERIAL

National Flood Hazard Layer FIRMMette



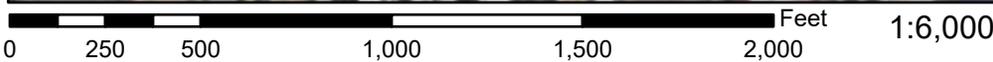
111°55'56"W 33°29'29"N



City Of Scottsdale
045012

AREA OF MINIMAL FLOOD HAZARD
Zone X

04013C2235M
eff. 9/18/2020



111°55'19"W 33°28'59"N

Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
GENERAL STRUCTURES		Area of Undetermined Flood Hazard Zone D
		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance Water Surface Elevation
		17.5 Cross Sections with 1% Annual Chance Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
MAP PANELS		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature
		Digital Data Available
		No Digital Data Available
		Unmapped

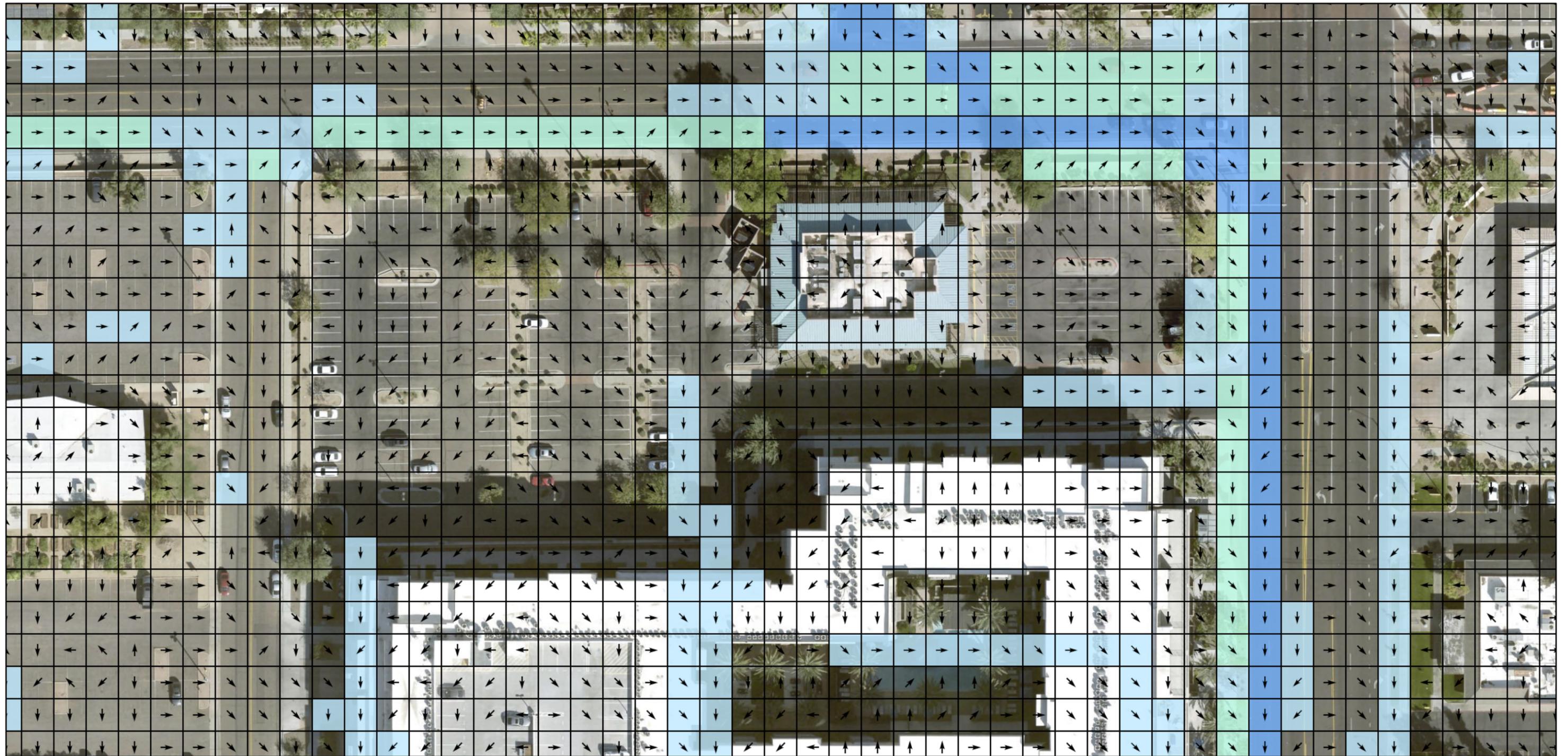
The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **10/11/2021 at 11:50 AM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

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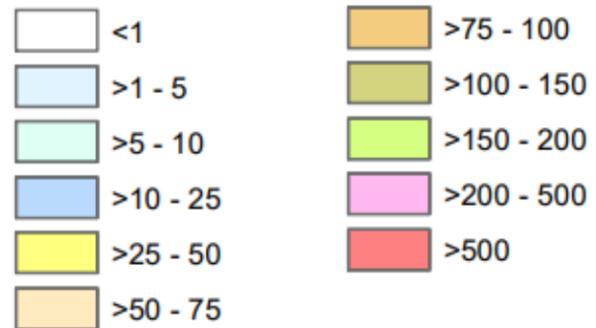
679_LIBW - South 100YR6HR



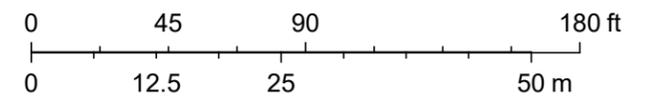
October 15, 2021

Peak Discharge

(cfs)



1:720



FLO-2D MAP

APPENDIX I

RAINFALL DATA



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

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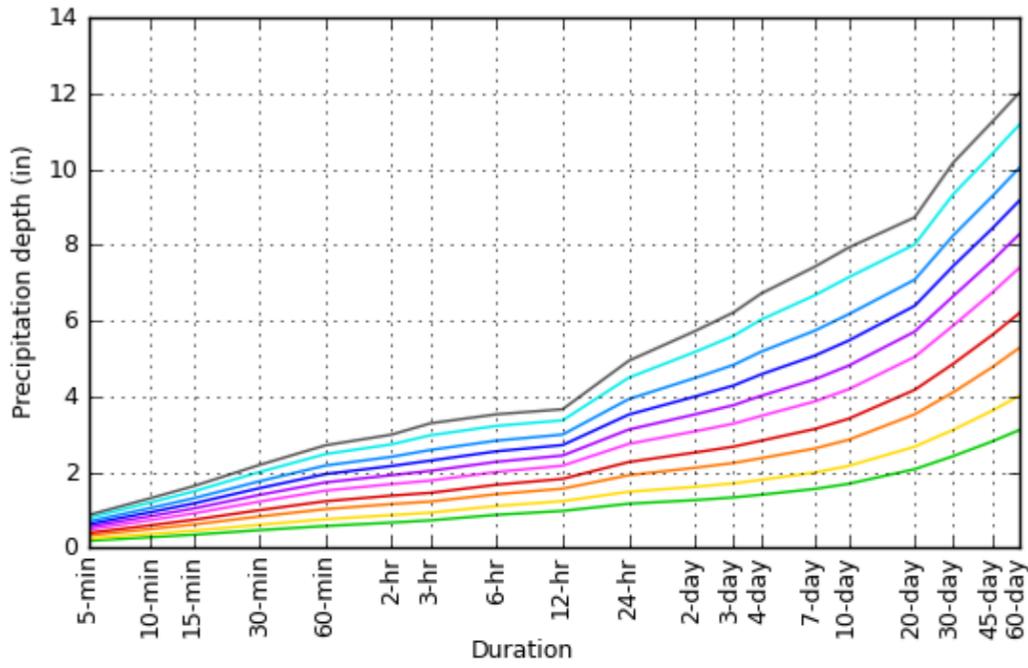
PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.183 (0.153-0.222)	0.239 (0.202-0.290)	0.324 (0.272-0.394)	0.390 (0.325-0.471)	0.480 (0.393-0.576)	0.549 (0.444-0.656)	0.619 (0.491-0.738)	0.691 (0.539-0.822)	0.787 (0.598-0.938)	0.860 (0.641-1.03)
10-min	0.278 (0.233-0.338)	0.363 (0.307-0.442)	0.494 (0.414-0.599)	0.594 (0.495-0.717)	0.730 (0.598-0.876)	0.835 (0.675-0.998)	0.941 (0.747-1.12)	1.05 (0.820-1.25)	1.20 (0.909-1.43)	1.31 (0.975-1.56)
15-min	0.344 (0.289-0.419)	0.450 (0.380-0.548)	0.612 (0.513-0.742)	0.737 (0.613-0.889)	0.905 (0.741-1.09)	1.03 (0.837-1.24)	1.17 (0.926-1.39)	1.30 (1.02-1.55)	1.48 (1.13-1.77)	1.62 (1.21-1.94)
30-min	0.464 (0.389-0.564)	0.607 (0.512-0.738)	0.825 (0.691-1.00)	0.992 (0.826-1.20)	1.22 (0.998-1.46)	1.39 (1.13-1.67)	1.57 (1.25-1.87)	1.75 (1.37-2.09)	2.00 (1.52-2.38)	2.19 (1.63-2.61)
60-min	0.574 (0.481-0.698)	0.751 (0.633-0.913)	1.02 (0.855-1.24)	1.23 (1.02-1.48)	1.51 (1.24-1.81)	1.73 (1.39-2.06)	1.95 (1.54-2.32)	2.17 (1.69-2.59)	2.47 (1.88-2.95)	2.71 (2.01-3.23)
2-hr	0.664 (0.567-0.792)	0.861 (0.735-1.03)	1.15 (0.981-1.37)	1.38 (1.16-1.63)	1.68 (1.40-1.98)	1.92 (1.57-2.25)	2.16 (1.74-2.53)	2.40 (1.90-2.82)	2.73 (2.11-3.21)	2.99 (2.26-3.53)
3-hr	0.722 (0.613-0.867)	0.926 (0.790-1.12)	1.22 (1.03-1.46)	1.45 (1.22-1.73)	1.77 (1.47-2.10)	2.03 (1.66-2.40)	2.30 (1.85-2.72)	2.58 (2.04-3.05)	2.97 (2.27-3.51)	3.29 (2.45-3.90)
6-hr	0.869 (0.754-1.02)	1.10 (0.959-1.30)	1.41 (1.23-1.66)	1.66 (1.43-1.94)	2.00 (1.70-2.32)	2.27 (1.89-2.62)	2.55 (2.09-2.94)	2.83 (2.28-3.27)	3.22 (2.53-3.73)	3.52 (2.70-4.10)
12-hr	0.972 (0.851-1.13)	1.23 (1.08-1.43)	1.56 (1.36-1.80)	1.82 (1.58-2.10)	2.17 (1.86-2.49)	2.43 (2.06-2.79)	2.71 (2.26-3.11)	2.99 (2.46-3.44)	3.36 (2.70-3.89)	3.66 (2.88-4.26)
24-hr	1.16 (1.04-1.31)	1.48 (1.32-1.67)	1.92 (1.71-2.15)	2.26 (2.01-2.54)	2.74 (2.42-3.07)	3.12 (2.74-3.49)	3.52 (3.06-3.93)	3.93 (3.39-4.39)	4.49 (3.84-5.03)	4.94 (4.18-5.54)
2-day	1.26 (1.12-1.42)	1.61 (1.44-1.81)	2.11 (1.88-2.37)	2.51 (2.23-2.82)	3.07 (2.72-3.44)	3.52 (3.09-3.94)	3.99 (3.48-4.48)	4.48 (3.88-5.03)	5.17 (4.43-5.81)	5.72 (4.85-6.45)
3-day	1.33 (1.19-1.50)	1.70 (1.52-1.91)	2.24 (1.99-2.51)	2.67 (2.37-2.99)	3.28 (2.90-3.67)	3.77 (3.30-4.21)	4.28 (3.73-4.80)	4.83 (4.17-5.41)	5.60 (4.78-6.28)	6.21 (5.25-6.99)
4-day	1.40 (1.25-1.58)	1.79 (1.60-2.02)	2.36 (2.10-2.65)	2.83 (2.51-3.17)	3.48 (3.07-3.90)	4.01 (3.52-4.49)	4.58 (3.98-5.12)	5.18 (4.47-5.80)	6.02 (5.13-6.74)	6.71 (5.66-7.53)
7-day	1.55 (1.38-1.75)	1.98 (1.77-2.24)	2.62 (2.33-2.94)	3.13 (2.78-3.52)	3.86 (3.41-4.33)	4.44 (3.90-4.98)	5.07 (4.41-5.68)	5.73 (4.95-6.43)	6.66 (5.68-7.48)	7.42 (6.25-8.34)
10-day	1.69 (1.51-1.90)	2.16 (1.93-2.43)	2.85 (2.54-3.20)	3.41 (3.02-3.81)	4.18 (3.69-4.67)	4.81 (4.22-5.37)	5.47 (4.76-6.11)	6.16 (5.33-6.89)	7.14 (6.10-7.99)	7.92 (6.70-8.88)
20-day	2.07 (1.86-2.32)	2.67 (2.39-2.98)	3.52 (3.15-3.93)	4.17 (3.71-4.64)	5.04 (4.47-5.61)	5.71 (5.05-6.35)	6.39 (5.62-7.12)	7.08 (6.20-7.89)	8.01 (6.95-8.95)	8.72 (7.51-9.76)
30-day	2.42 (2.16-2.71)	3.12 (2.79-3.48)	4.11 (3.66-4.57)	4.86 (4.33-5.40)	5.87 (5.20-6.52)	6.64 (5.86-7.38)	7.44 (6.54-8.26)	8.25 (7.21-9.16)	9.34 (8.10-10.4)	10.2 (8.75-11.3)
45-day	2.81 (2.52-3.13)	3.62 (3.25-4.03)	4.76 (4.27-5.31)	5.61 (5.02-6.25)	6.73 (6.00-7.49)	7.57 (6.73-8.43)	8.42 (7.45-9.38)	9.27 (8.16-10.3)	10.4 (9.08-11.6)	11.2 (9.76-12.6)
60-day	3.11 (2.80-3.46)	4.01 (3.61-4.46)	5.28 (4.74-5.86)	6.20 (5.55-6.88)	7.39 (6.61-8.21)	8.28 (7.37-9.19)	9.17 (8.13-10.2)	10.0 (8.87-11.2)	11.2 (9.81-12.5)	12.0 (10.5-13.4)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

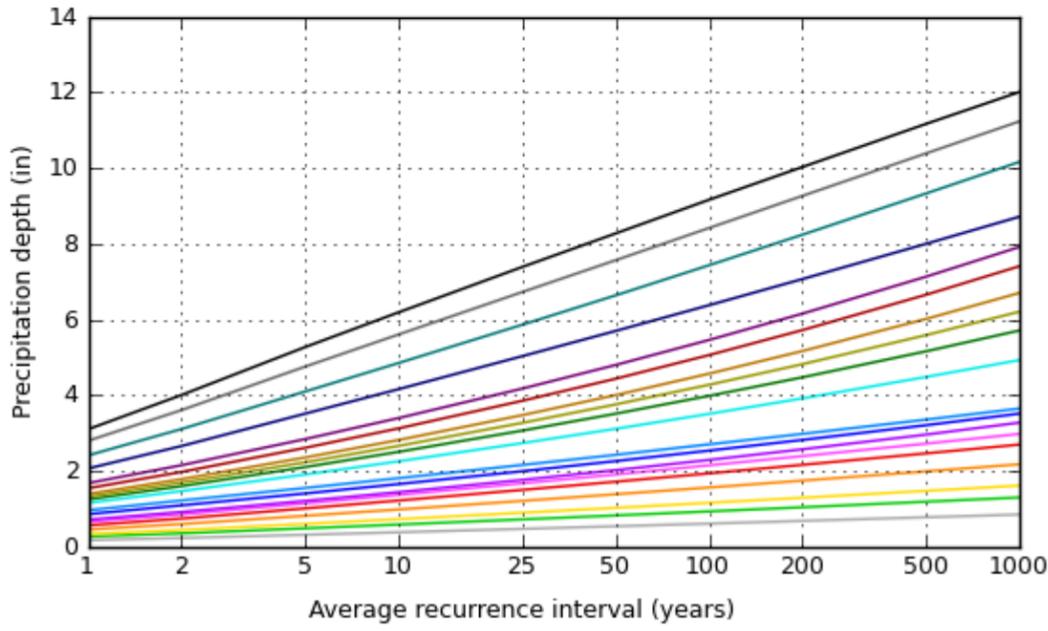
[Back to Top](#)

PF graphical

PDS-based depth-duration-frequency (DDF) curves
 Latitude: 33.4873°, Longitude: -111.9271°



Average recurrence interval (years)
1
2
5
10
25
50
100
200
500
1000

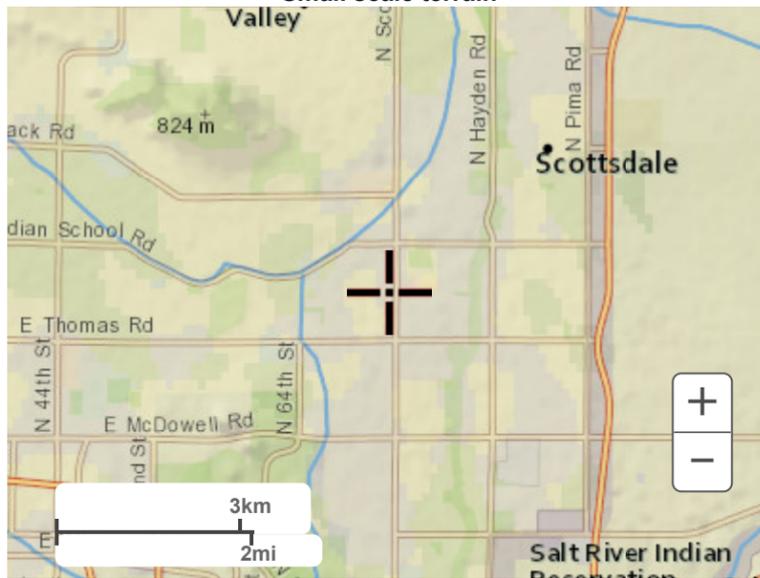


Duration
5-min
10-min
15-min
30-min
60-min
2-hr
3-hr
6-hr
12-hr
24-hr
2-day
3-day
4-day
7-day
10-day
20-day
30-day
45-day
60-day

[Back to Top](#)

Maps & aerials

Small scale terrain



Large scale terrain





POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps & aerials](#)

PF tabular

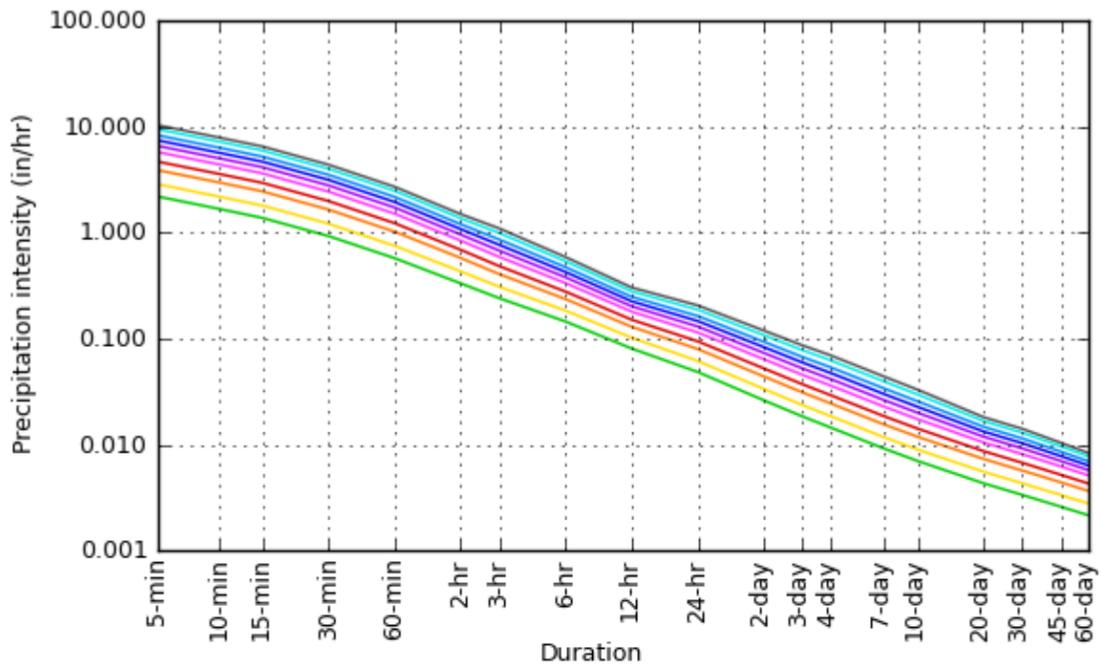
PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	2.20 (1.84-2.66)	2.87 (2.42-3.48)	3.89 (3.26-4.73)	4.68 (3.90-5.65)	5.76 (4.72-6.91)	6.59 (5.33-7.87)	7.43 (5.89-8.86)	8.29 (6.47-9.86)	9.44 (7.18-11.3)	10.3 (7.69-12.3)
10-min	1.67 (1.40-2.03)	2.18 (1.84-2.65)	2.96 (2.48-3.59)	3.56 (2.97-4.30)	4.38 (3.59-5.26)	5.01 (4.05-5.99)	5.65 (4.48-6.73)	6.31 (4.92-7.51)	7.18 (5.45-8.57)	7.85 (5.85-9.38)
15-min	1.38 (1.16-1.68)	1.80 (1.52-2.19)	2.45 (2.05-2.97)	2.95 (2.45-3.56)	3.62 (2.96-4.34)	4.14 (3.35-4.95)	4.67 (3.70-5.56)	5.21 (4.07-6.20)	5.94 (4.51-7.08)	6.49 (4.84-7.76)
30-min	0.928 (0.778-1.13)	1.21 (1.02-1.48)	1.65 (1.38-2.00)	1.98 (1.65-2.39)	2.44 (2.00-2.93)	2.79 (2.25-3.33)	3.14 (2.49-3.75)	3.51 (2.74-4.18)	4.00 (3.04-4.77)	4.37 (3.26-5.22)
60-min	0.574 (0.481-0.698)	0.751 (0.633-0.913)	1.02 (0.855-1.24)	1.23 (1.02-1.48)	1.51 (1.24-1.81)	1.73 (1.39-2.06)	1.95 (1.54-2.32)	2.17 (1.69-2.59)	2.47 (1.88-2.95)	2.71 (2.01-3.23)
2-hr	0.332 (0.284-0.396)	0.430 (0.368-0.514)	0.576 (0.490-0.685)	0.688 (0.578-0.816)	0.840 (0.698-0.990)	0.958 (0.785-1.13)	1.08 (0.870-1.27)	1.20 (0.952-1.41)	1.37 (1.06-1.60)	1.49 (1.13-1.76)
3-hr	0.240 (0.204-0.289)	0.308 (0.263-0.372)	0.406 (0.344-0.487)	0.483 (0.405-0.576)	0.590 (0.489-0.701)	0.676 (0.552-0.800)	0.766 (0.615-0.906)	0.859 (0.678-1.01)	0.989 (0.757-1.17)	1.09 (0.816-1.30)
6-hr	0.145 (0.126-0.171)	0.184 (0.160-0.216)	0.236 (0.205-0.277)	0.278 (0.238-0.324)	0.334 (0.283-0.387)	0.379 (0.316-0.437)	0.425 (0.349-0.491)	0.472 (0.380-0.547)	0.537 (0.422-0.623)	0.588 (0.451-0.684)
12-hr	0.081 (0.071-0.094)	0.102 (0.089-0.119)	0.129 (0.113-0.150)	0.151 (0.131-0.174)	0.180 (0.154-0.207)	0.202 (0.171-0.232)	0.225 (0.188-0.258)	0.248 (0.204-0.285)	0.279 (0.224-0.323)	0.304 (0.239-0.353)
24-hr	0.048 (0.043-0.055)	0.062 (0.055-0.069)	0.080 (0.071-0.090)	0.094 (0.084-0.106)	0.114 (0.101-0.128)	0.130 (0.114-0.145)	0.147 (0.128-0.164)	0.164 (0.141-0.183)	0.187 (0.160-0.209)	0.206 (0.174-0.231)
2-day	0.026 (0.023-0.029)	0.033 (0.030-0.038)	0.044 (0.039-0.049)	0.052 (0.047-0.059)	0.064 (0.057-0.072)	0.073 (0.064-0.082)	0.083 (0.073-0.093)	0.093 (0.081-0.105)	0.108 (0.092-0.121)	0.119 (0.101-0.134)
3-day	0.018 (0.016-0.021)	0.024 (0.021-0.027)	0.031 (0.028-0.035)	0.037 (0.033-0.042)	0.046 (0.040-0.051)	0.052 (0.046-0.059)	0.060 (0.052-0.067)	0.067 (0.058-0.075)	0.078 (0.066-0.087)	0.086 (0.073-0.097)
4-day	0.015 (0.013-0.016)	0.019 (0.017-0.021)	0.025 (0.022-0.028)	0.029 (0.026-0.033)	0.036 (0.032-0.041)	0.042 (0.037-0.047)	0.048 (0.041-0.053)	0.054 (0.047-0.060)	0.063 (0.053-0.070)	0.070 (0.059-0.078)
7-day	0.009 (0.008-0.010)	0.012 (0.011-0.013)	0.016 (0.014-0.018)	0.019 (0.017-0.021)	0.023 (0.020-0.026)	0.026 (0.023-0.030)	0.030 (0.026-0.034)	0.034 (0.029-0.038)	0.040 (0.034-0.045)	0.044 (0.037-0.050)
10-day	0.007 (0.006-0.008)	0.009 (0.008-0.010)	0.012 (0.011-0.013)	0.014 (0.013-0.016)	0.017 (0.015-0.019)	0.020 (0.018-0.022)	0.023 (0.020-0.025)	0.026 (0.022-0.029)	0.030 (0.025-0.033)	0.033 (0.028-0.037)
20-day	0.004 (0.004-0.005)	0.006 (0.005-0.006)	0.007 (0.007-0.008)	0.009 (0.008-0.010)	0.010 (0.009-0.012)	0.012 (0.011-0.013)	0.013 (0.012-0.015)	0.015 (0.013-0.016)	0.017 (0.014-0.019)	0.018 (0.016-0.020)
30-day	0.003 (0.003-0.004)	0.004 (0.004-0.005)	0.006 (0.005-0.006)	0.007 (0.006-0.007)	0.008 (0.007-0.009)	0.009 (0.008-0.010)	0.010 (0.009-0.011)	0.011 (0.010-0.013)	0.013 (0.011-0.014)	0.014 (0.012-0.016)
45-day	0.003 (0.002-0.003)	0.003 (0.003-0.004)	0.004 (0.004-0.005)	0.005 (0.005-0.006)	0.006 (0.006-0.007)	0.007 (0.006-0.008)	0.008 (0.007-0.009)	0.009 (0.008-0.010)	0.010 (0.008-0.011)	0.010 (0.009-0.012)
60-day	0.002 (0.002-0.002)	0.003 (0.003-0.003)	0.004 (0.003-0.004)	0.004 (0.004-0.005)	0.005 (0.005-0.006)	0.006 (0.005-0.006)	0.006 (0.006-0.007)	0.007 (0.006-0.008)	0.008 (0.007-0.009)	0.008 (0.007-0.009)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

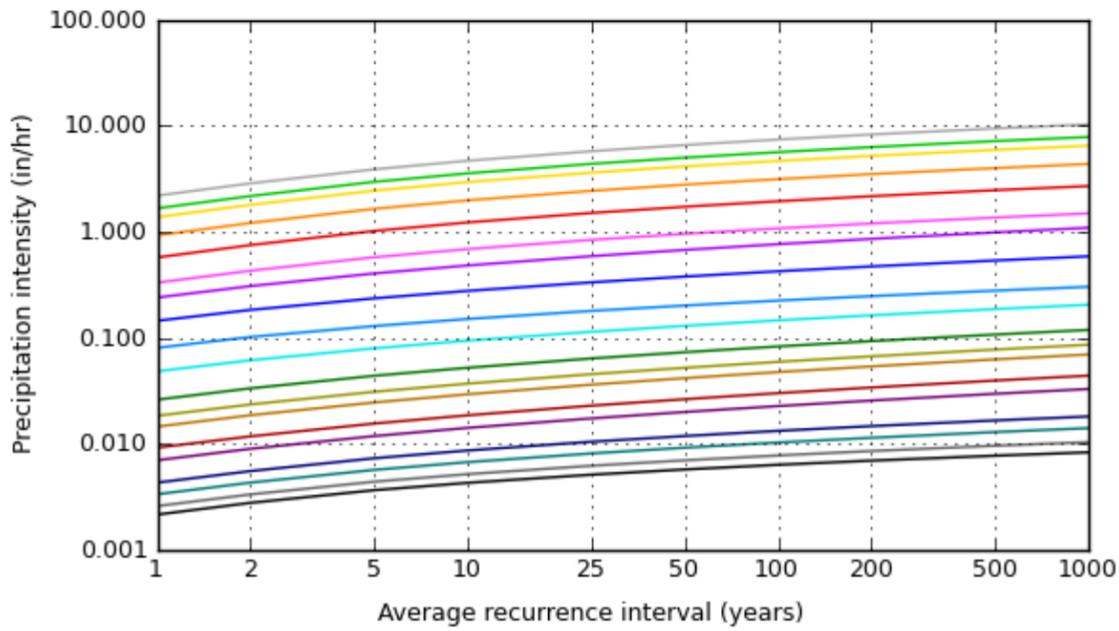
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PF graphical

PDS-based intensity-duration-frequency (IDF) curves
 Latitude: 33.4873°, Longitude: -111.9271°



Average recurrence interval (years)
1
2
5
10
25
50
100
200
500
1000

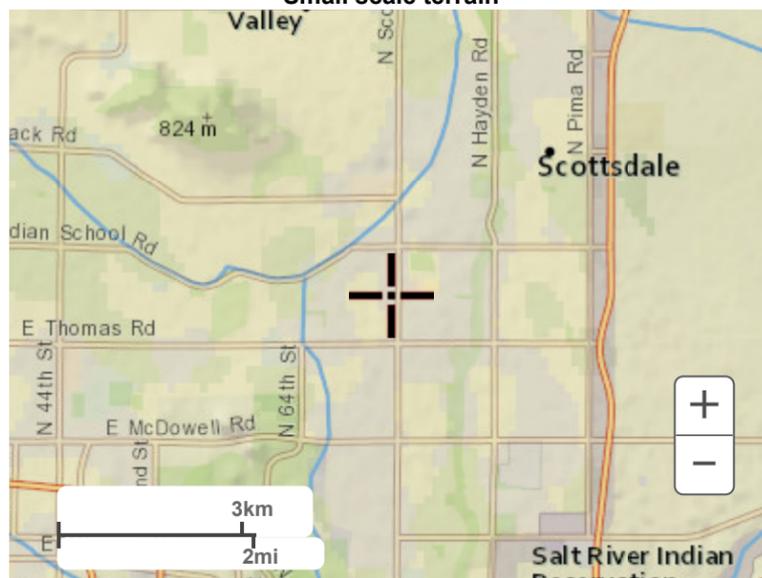


Duration
5-min
10-min
15-min
30-min
60-min
2-hr
3-hr
6-hr
12-hr
24-hr
2-day
3-day
4-day
7-day
10-day
20-day
30-day
45-day
60-day

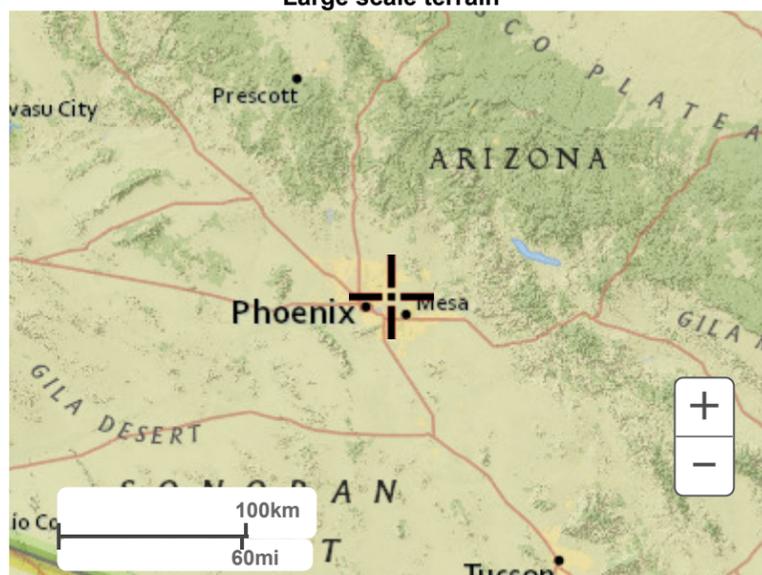
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Maps & aerials

Small scale terrain



Large scale terrain





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APPENDIX II

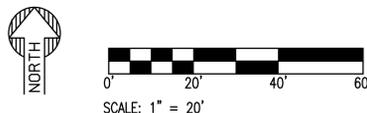
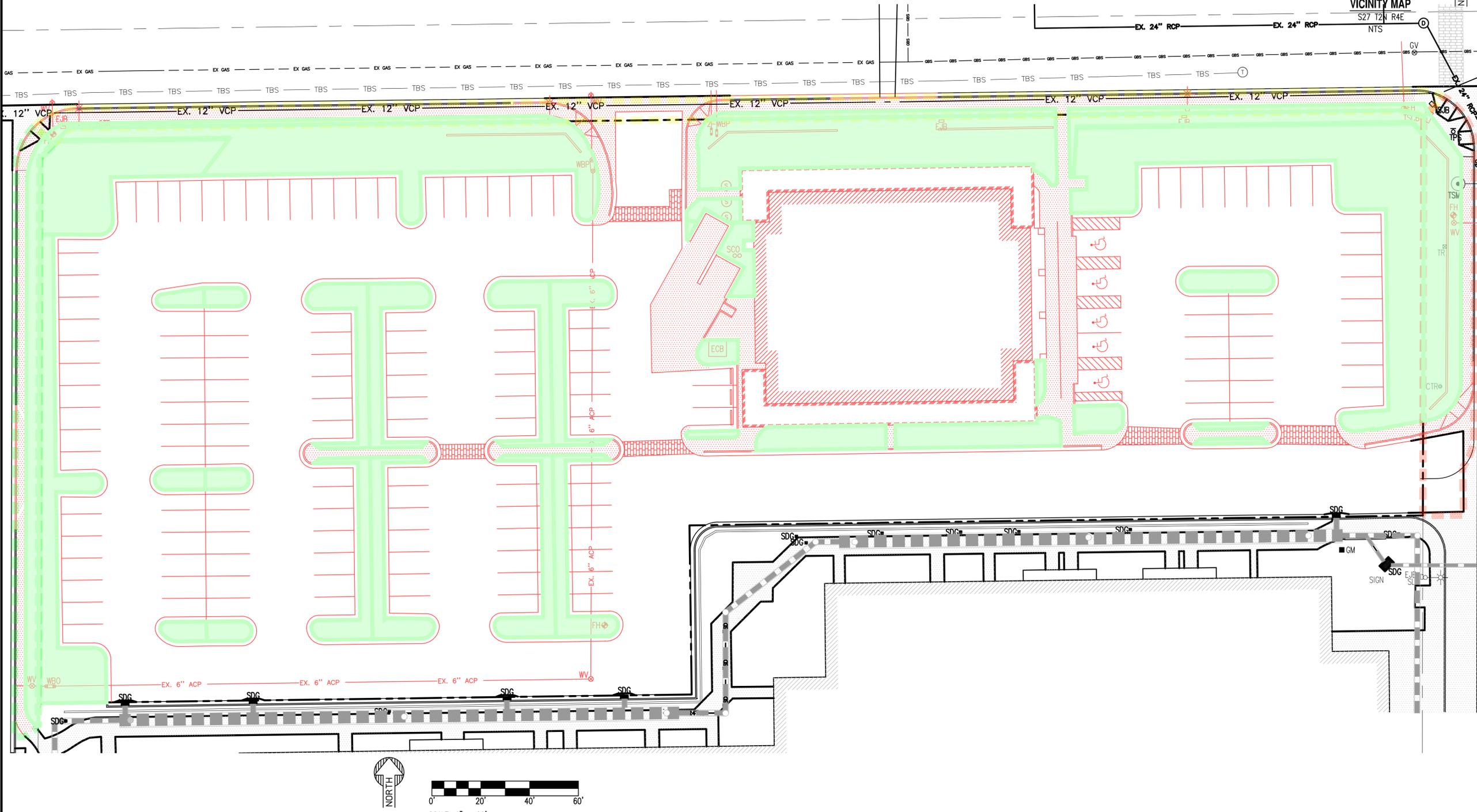
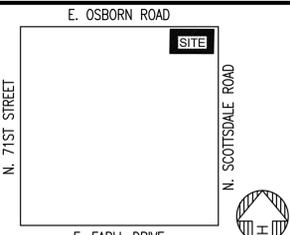
CALCULATIONS

*8280 E. Gelding Dr., Suite 101
Scottsdale, AZ 85260*

OLIVE GARDEN

EXISTING CONDITIONS C_{WT}

3380 N. SCOTTSDALE ROAD, SCOTTSDALE, AZ



	BUILDING/PAVED SURFACE =	88,389 SF (2.03 AC)	● CWT=0.95
	NATURAL DESERT =	23,708 SF (0.54 AC)	● CWT=0.45
TOTAL ON-SITE CWT =		112,097 SF (2.57 AC)	● CWT=0.84

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ENGINEERING
GROUP



8280 E. GELDING DRIVE SUITE 101, SCOTTSDALE, ARIZONA 85260
WWW.AZSEG.COM TEL. 480.987.7226 FAX. 480.259.3534



PROJECT	SENIOR LIVING	OSBORN
LOCATION	3380 N. SCOTTSDALE ROAD, SCOTTSDALE, AZ	
DRAWN	JC	11/17/2021
DESIGNED	JC	11/17/2021
QC	SC	10/25/2021
FINAL QC		
PROJ. MGR.	AF	11/17/2021
DATE:	11/17/2021	
ISSUED FOR:	DRB	

REVISION NO.:	DATE:
JOB NO.:	210126
SHEET TITLE:	X-C _{WT}

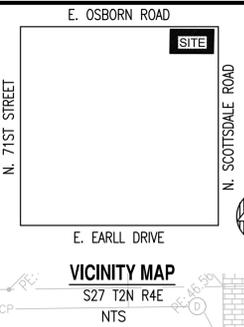
LOCATION: Z:\SHARED\PROJECTS\EMPIRE\OLIVE GARDEN - SCOTTSDALE - 210126\11 CAD (SEG)\11.3 ENTITLEMENT-PLANNING\210126-X-CWT.DWG SAVED BY: JUAN CHACON DATE: 11/17/2021

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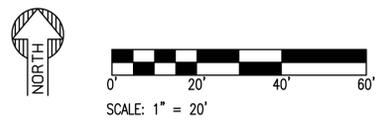
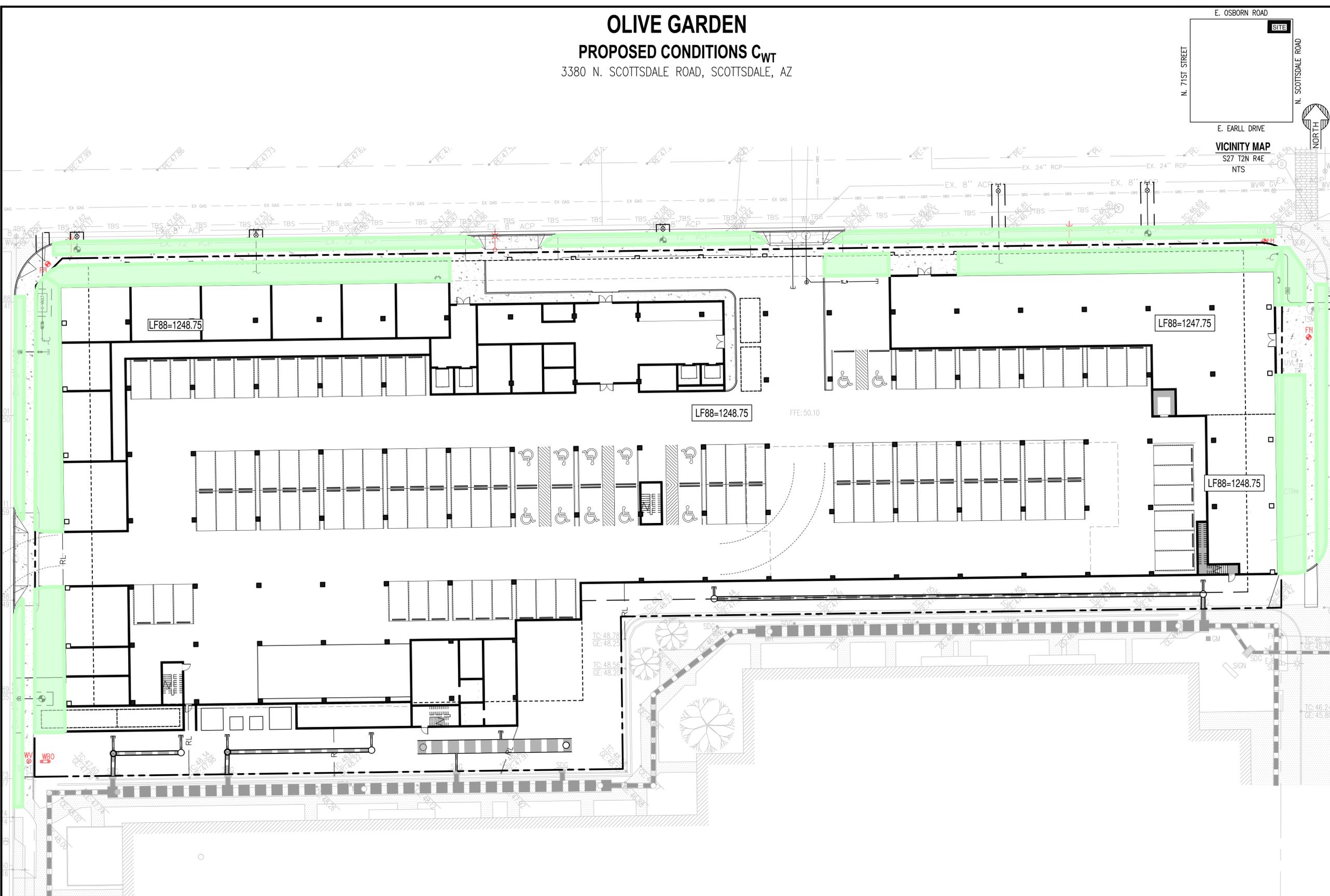
OLIVE GARDEN

PROPOSED CONDITIONS C_{WT}

3380 N. SCOTTSDALE ROAD, SCOTTSDALE, AZ



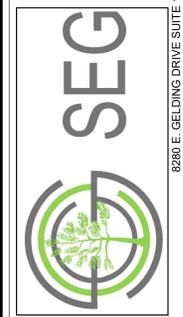
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 DRAWN BY: JUAN CHACON DATE: 11/17/2021



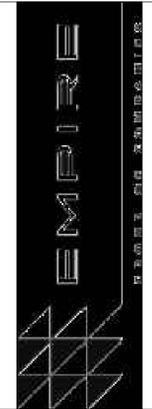
	BUILDING/PAVED SURFACE =	106276 SF (2.44 AC)	● CWT=0.95
	NATURAL DESERT =	5,821 SF (0.13 AC)	● CWT=0.45
	TOTAL ON-SITE CWT =	112,097 SF (2.57 AC)	● CWT=0.92

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CONSTRUCTION

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 WWW.AZSEG.COM TEL: 480.988.7226 FAX: 480.259.3534



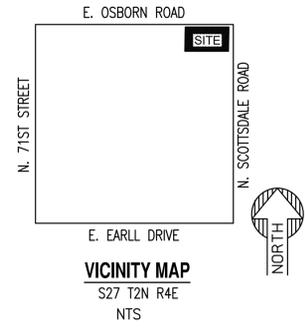
PROJECT SENIOR LIVING + OSBORN		LOCATION 3380 N. SCOTTSDALE ROAD, SCOTTSDALE, AZ	
DRAWN	JC	11/17/2021	
DESIGNED	JC	11/17/2021	
QC	SC	10/25/2021	
FINAL QC			
PROJ. MGR.	AF	11/17/2021	
DATE:	11/17/2021		
ISSUED FOR:	DRB		
REVISION NO.:		DATE:	
JOB NO.:	210126		
SHEET TITLE:	P-C_{WT}		
PAGE NO.:	1 OF 1		
SHEET NO.:	P-C _{WT}		

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OLIVE GARDEN

EXISTING CONDITIONS DRAINAGE AREA MAP

3380 N. SCOTTSDALE ROAD, SCOTTSDALE, AZ



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8280 E. GELDING DRIVE SUITE 101, SCOTTSDALE, ARIZONA 85260
WWW.AZSEG.COM TEL. 480.986.7226 FAX. 480.259.3534



PROJECT: SENIOR LIVING + OSBORN
LOCATION: 3380 N. SCOTTSDALE ROAD, SCOTTSDALE, AZ

DRAWN: JC 11/17/2021
DESIGNED: JC 11/17/2021
QC: JC
FINAL QC: JC
PROJ. MGR.: AF 11/17/2021
DATE: 11/17/2021

ISSUED FOR: DRB

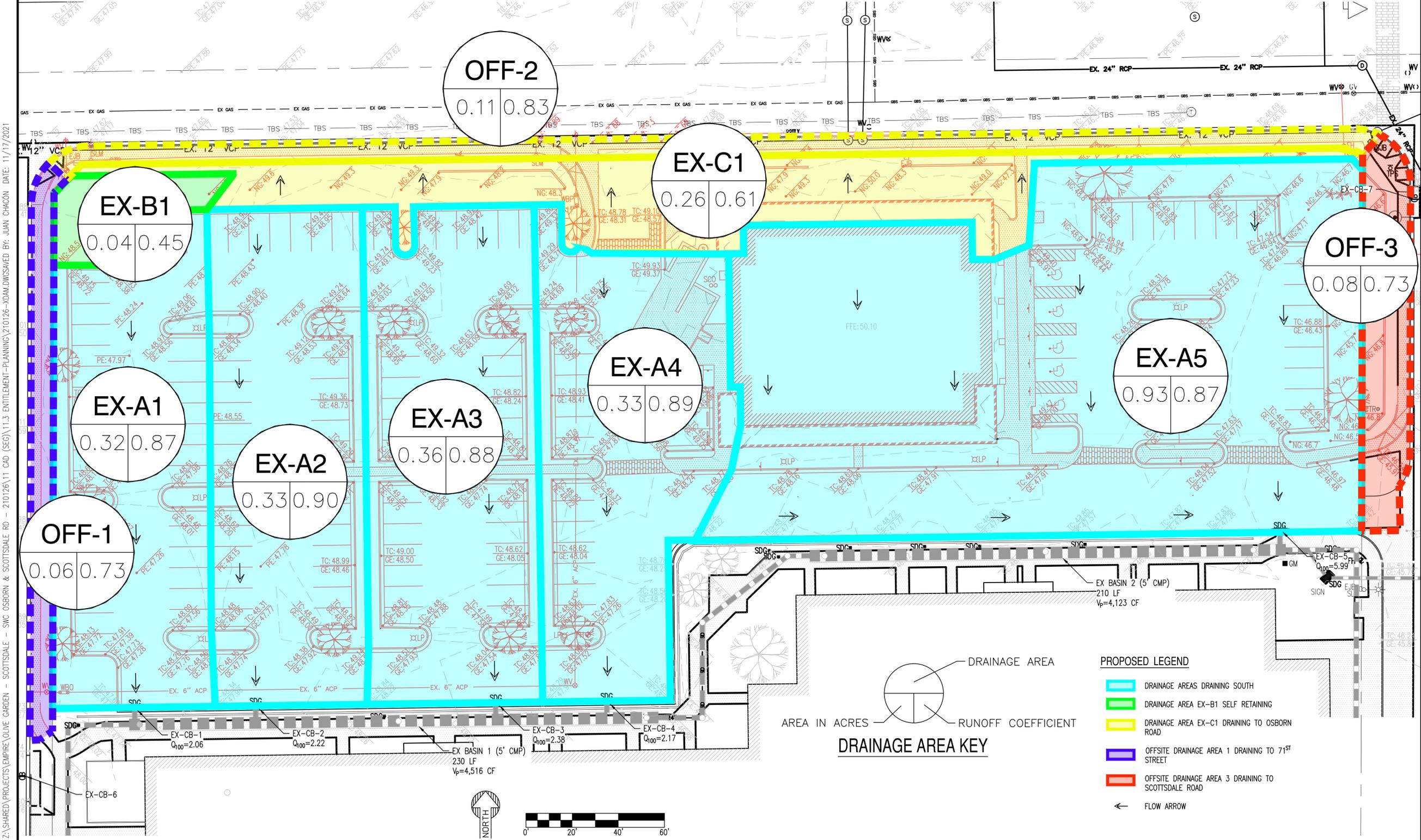
REVISION NO.: DATE:

JOB NO.: 210126

SHEET TITLE: X-DAM

PAGE NO.: 1 OF 1

SHEET NO.: X-DAM



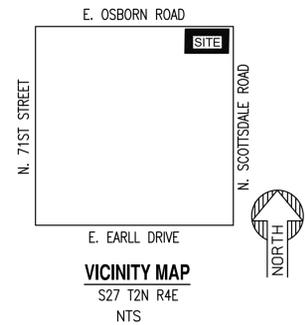
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OLIVE GARDEN

PROPOSED CONDITIONS DRAINAGE AREA MAP

3380 N. SCOTTSDALE ROAD, SCOTTSDALE, AZ



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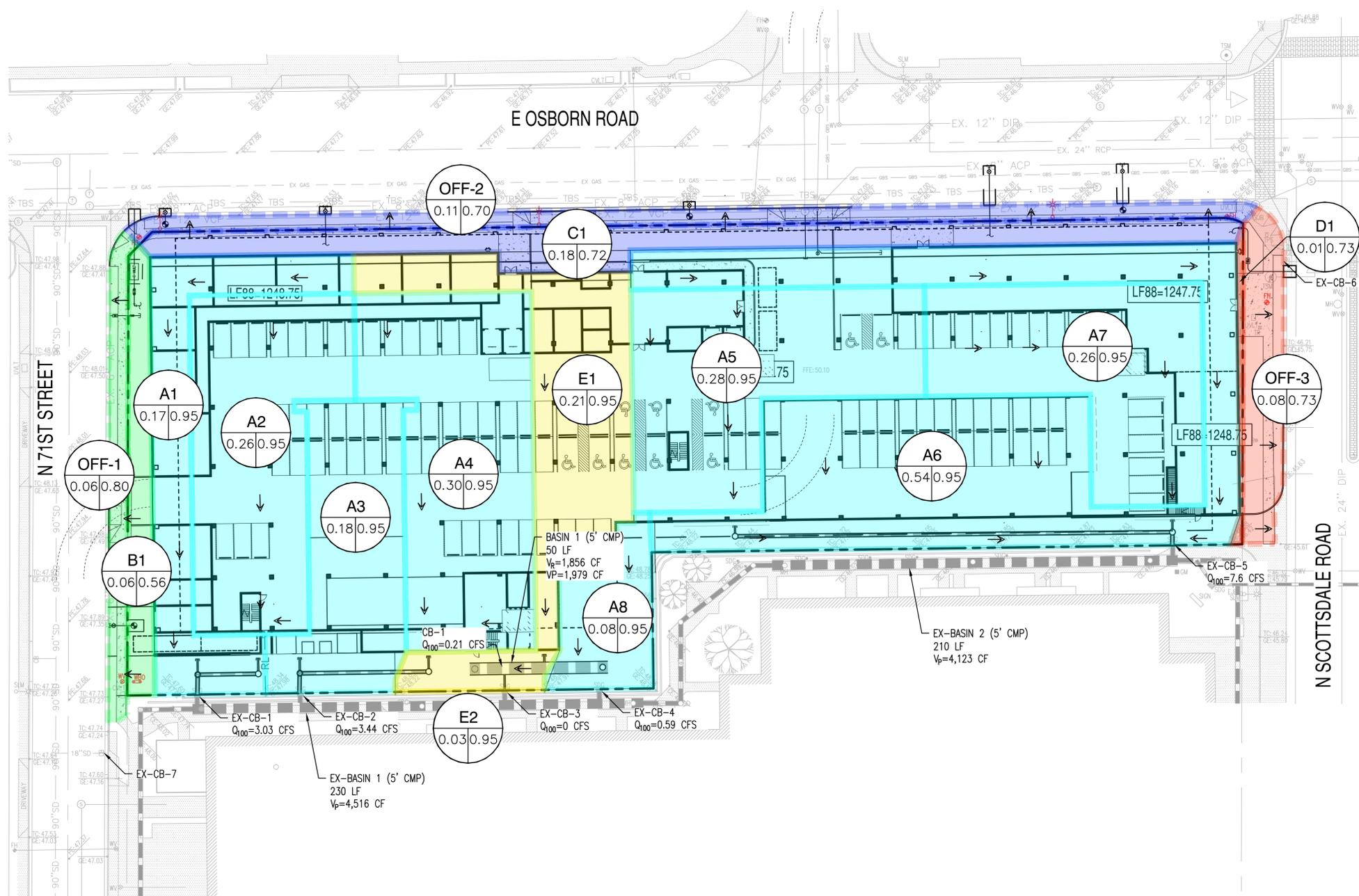
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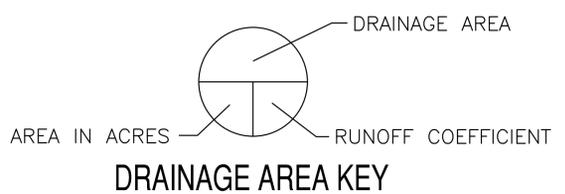
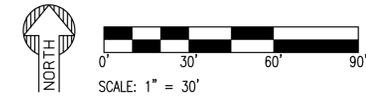
JOB NO.: 210126
SHEET TITLE:

P-DAM

PAGE NO.: 1 OF 1
SHEET NO.: **P-DAM**



- PROPOSED LEGEND**
- █ DRAINAGE AREAS DRAINING TO EXISTING CATCH BASINS
 - █ DRAINAGE AREAS DRAINING TO 71ST STREET
 - █ DRAINAGE AREAS DRAINING TO OSBORN ROAD
 - █ DRAINAGE AREAS DRAINING TO SCOTTSDALE ROAD
 - █ DRAINAGE AREAS DRAINING TO UNDERGROUND RETENTION BASIN A
 - ← FLOW ARROW



LOCATION: Z:\SHARED\PROJECTS\EMPIRE\OLIVE GARDEN - SCOTTSDALE - SWC OSBORN & SCOTTSDALE RD - 210126\11 CAD (SEG)\11.3 ENTITLEMENT-PLANNING\210126-PDAM.DWG
DRAWN BY: JUAN CHACON DATE: 11/17/2021

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EXISTING OVERALL SITE C_w

	Pavement	DESERT LANDSCAPE	TOTAL AREA	Cwt
C-VALUE	0.95	0.45		
AREA (ac)	2.03	0.54	2.57	0.84
EX-A1	0.27	0.05	0.32	0.87
EX-A2	0.30	0.03	0.33	0.90
EX-A3	0.31	0.05	0.36	0.88
EX-A4	0.29	0.04	0.33	0.89
EX-A5	0.78	0.15	0.93	0.87
EX-B1	0.00	0.04	0.04	0.45
EX-C1	0.09	0.18	0.26	0.61

EXISTING OFFSITE SITE C_w

	Pavement	DESERT LANDSCAPE	TOTAL AREA	Cwt
C-VALUE	0.95	0.50		
AREA (ac)	0.11	0.06	0.17	0.79
OFF-1	0.03	0.03	0.06	0.73
OFF-2	0.08	0.03	0.11	0.83
OFF-3	0.04	0.04	0.08	0.73

PROPOSED OVERALL SITE C_w

	Building/ Pavement	DESERT LANDSCAPE	TOTAL AREA	Cwt
C-VALUE	0.95	0.45		
AREA (ac)	2.44	0.13	2.57	0.92
DA-A1	0.17	0.00	0.17	0.95
DA-A2	0.26	0.00	0.26	0.95
DA-A3	0.18	0.00	0.18	0.95
DA-A4	0.30	0.00	0.30	0.95
DA-A5	0.28	0.00	0.28	0.95
DA-A6	0.54	0.00	0.54	0.95
DA-A7	0.26	0.00	0.26	0.95
DA-A8	0.08	0.00	0.08	0.95
DA-B1	0.01	0.05	0.06	0.56
DA-C1	0.10	0.08	0.18	0.72
DA-D1	0.00	0.004	0.01	0.73
DA-E1	0.21	0.00	0.21	0.95
DA-E2	0.03	0.00	0.03	0.95

PROPOSED OFFSITE SITE C_w

	Pavement	DESERT LANDSCAPE	TOTAL AREA	Cwt
C-VALUE	0.95	0.50		
AREA (ac)	0.13	0.12	0.25	0.73
OFF-1	0.04	0.02	0.06	0.80
OFF-2	0.05	0.06	0.11	0.70
OFF-3	0.04	0.04	0.08	0.73

Required Storage Volume Calculations

P=100-yr, 5min = 7.90					Vr=1*(P/12)*Cw*A
P=100-yr, 24-hr= 2.16in.					
Drainage	Area	C	Depth	Volume Req.	Volume Req.
Area ID	(acres)	(-)	(in)	(acre-ft)	(CF)
ON-SITE RETENTION - BASIN A - UG Storage + Open Retention					
DA-E1	0.21	0.95	2.16	0.037	1,595.86
DA-E2	0.03	0.95	2.16	0.006	260.12
Basin A Total	0.25	0.82		0.04	1,855.98



“LEED®ing and Developing Smart Projects”

APPENDIX III

PRELIMINARY GRADING PLAN

*8280 E. Gelding Dr., Suite 101
Scottsdale, AZ 85260*

CIVIL ENGINEER
 SUSTAINABILITY ENGINEERING GROUP
 8280 E. GELDING DR., SUITE 101
 SCOTTSDALE, ARIZONA 85260
 PHONE: 480-588-7226
 ATTN: ALI FAKIH
 EMAIL: ALI@AZSEG.COM

DEVELOPER:
 EMPIRE GROUP
 6617 N SCOTTSDALE RD, SUITE 101
 SCOTTSDALE, ARIZONA 85250

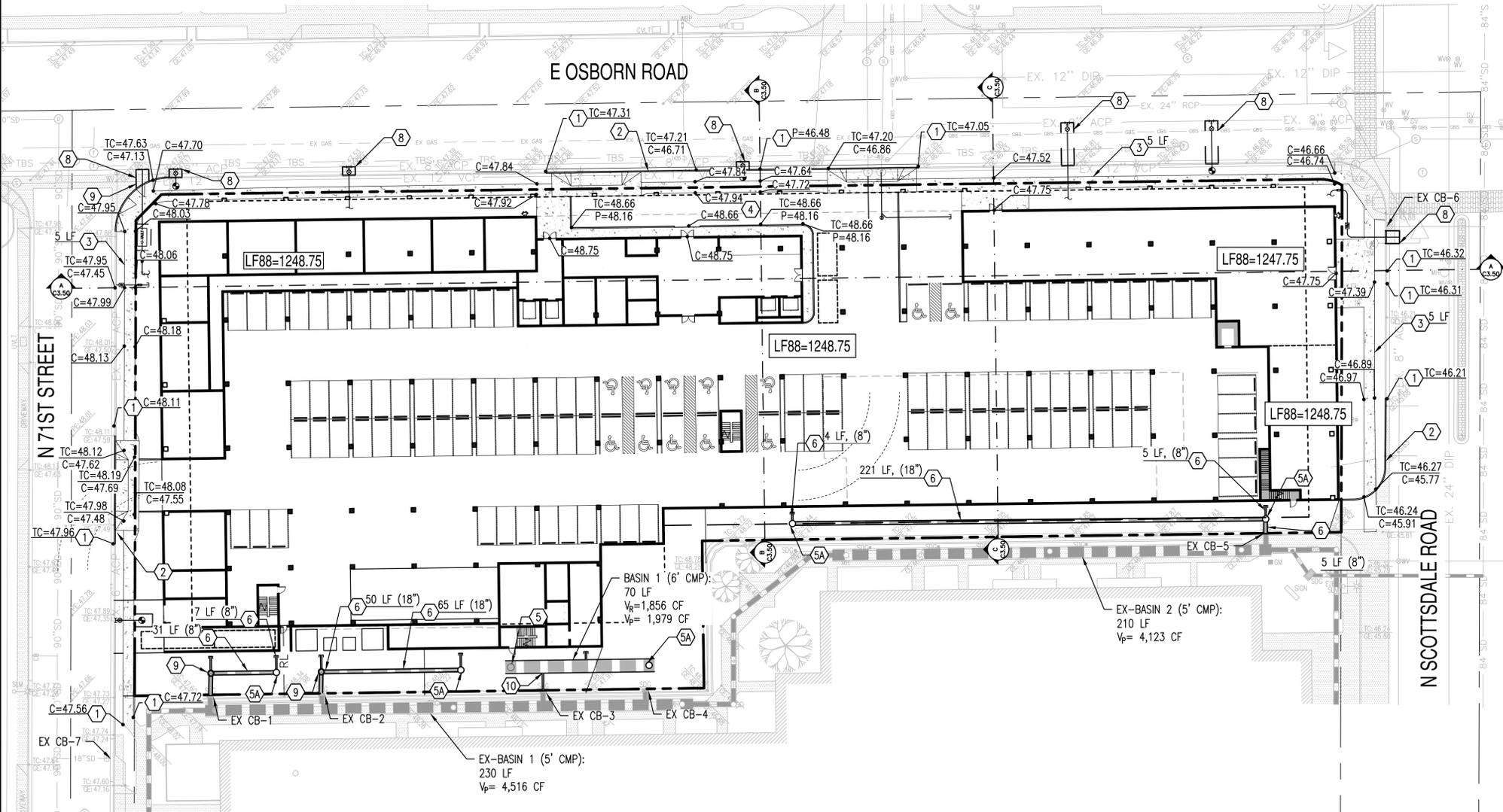
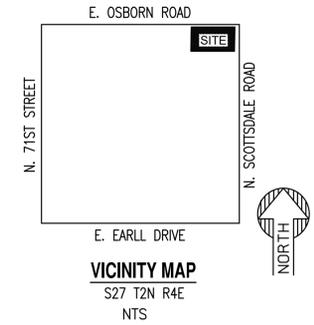
SURVEYOR
 AW LAND SURVEYING, LLC
 P.O. BOX 2170
 CHANDLER, ARIZONA 85244
 PHONE: 480-244-7630
 ATTN: DANIEL ARMUO

SENIOR LIVING SCOTTSDALE + OSBORN

PRELIMINARY GRADING PLAN

3380 N. SCOTTSDALE ROAD, SCOTTSDALE AZ 85250
 A PORTION OF THE EAST HALF OF THE SOUTHEAST QUARTER OF SECTION 27, TOWNSHIP 2 NORTH, RANGE 4 EAST
 OF THE GILA AND SALT RIVER MERIDIAN, MARICOPA COUNTY, ARIZONA.

APN: 130-16-114

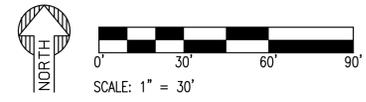


PRELIMINARY GRADING NOTES

- 1 MATCH EXISTING GRADE.
- 2 6" VERTICAL CURB AND GUTTER.
- 3 CONSTRUCT CONCRETE SIDEWALK. WIDTH PER PLAN.
- 4 LIGHT DUTY PAVEMENT.
- 5 INSTALL 30" CMP RISER WITH STANDARD H-20 TRAFFIC GRATED LID.
- 5A INSTALL 30" NYLOPLAST DRAIN BASIN WITH SOLID LID.
- 6 INSTALL HDPE DOUBLE WALL PIPE. LENGTH, SIZE AND SLOPE PER PLAN.
- 7 INSTALL UNDERGROUND STORMWATER STORAGE SYSTEM.
- 8 SAWCUT EXISTING PAVEMENT TO PROVIDE STRAIGHT VERTICAL EDGES, FREE FROM IRREGULARITIES. MIN 2' FROM GUTTER.
- 9 FURNISH AND INSTALL BUBBLER BOX.
- 10 INSTALL 1.5" ORIFICE CORED IN CAP AT INSERTION OF THE 8" BLEED OFF PIPE.

C.O.S. GENERAL NOTES FOR PUBLIC WORKS CONSTRUCTION

1. ALL CONSTRUCTION IN THE PUBLIC RIGHTS-OF-WAY OR IN EASEMENTS GRANTED FOR PUBLIC USE MUST CONFORM TO THE LATEST MAG UNIFORM STANDARD SPECIFICATIONS AND UNIFORM STANDARD DETAILS FOR PUBLIC WORKS CONSTRUCTION AS AMENDED BY THE LATEST VERSION OF THE CITY OF SCOTTSDALE SUPPLEMENTAL STANDARD SPECIFICATIONS AND SUPPLEMENTAL STANDARD DETAILS. IF THERE IS A CONFLICT, THE CITY'S SUPPLEMENTAL STANDARD DETAILS WILL GOVERN.
2. THE CITY ONLY APPROVES THE SCOPE, NOT THE DETAIL, OF ENGINEERING DESIGNS; THEREFORE, IF CONSTRUCTION QUANTITIES ARE SHOWN ON THESE PLANS, THEY ARE NOT VERIFIED BY THE CITY.
3. THE APPROVAL OF PLANS IS VALID FOR SIX (6) MONTHS. IF A RIGHT-OF-WAY PERMIT FOR THE CONSTRUCTION HAS NOT BEEN ISSUED WITHIN SIX MONTHS, THE PLANS MUST BE RESUBMITTED TO THE CITY FOR REAPPROVAL.
4. A PUBLIC WORKS INSPECTOR WILL INSPECT ALL WORKS WITHIN THE CITY RIGHTS-OF-WAY AND IN EASEMENTS. NOTIFY INSPECTION SERVICES 24 HOURS PRIOR TO BEGINNING CONSTRUCTION BY CALLING 480-312-5750.
5. WHENEVER EXCAVATION IS NECESSARY, CALL THE BLUE STAKE CENTER, 811, TWO WORKING DAYS BEFORE EXCAVATION BEGINS. THE CENTER WILL SEE THAT THE LOCATION OF THE UNDERGROUND UTILITY LINES IS IDENTIFIED FOR THE PROJECT.
6. RIGHT-OF-WAY PERMITS ARE REQUIRED FOR ALL WORK IN PUBLIC RIGHTS-OF-WAY AND EASEMENTS GRANTED FOR PUBLIC PURPOSES. A RIGHT-OF-WAY PERMIT WILL BE ISSUED BY THE CITY ONLY AFTER THE REGISTRANT HAS PAID A BASE FEE PLUS A FEE FOR INSPECTION SERVICES. COPIES OF ALL PERMITS MUST BE RETAINED ON-SITE AND BE AVAILABLE FOR INSPECTION AT ALL TIMES. FAILURE TO PRODUCE THE REQUIRED PERMITS WILL RESULT IN IMMEDIATE SUSPENSION OF ALL WORK UNTIL THE PROPER PERMIT DOCUMENTATION IS OBTAINED.
7. ALL EXCAVATION AND GRADING THAT IS NOT IN THE PUBLIC RIGHTS-OF-WAY OR NOT IN EASEMENTS GRANTED FOR PUBLIC USE MUST CONFORM TO APPENDIX J, GRADING, OF THE LATEST EDITION OF THE INTERNATIONAL BUILDING CODE. A PERMIT FOR THIS GRADING MUST BE SECURED FROM THE CITY FOR A FEE ESTABLISHED BY THE CITY.



EXISTING LEGEND:

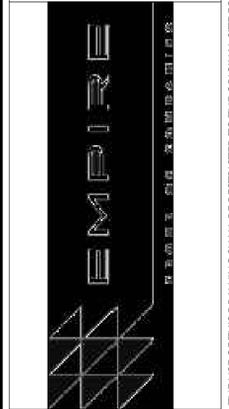
--- XXXX ---	EX. MAJOR CONTOURS	EX. S	SEWER LINE	---	STORM DRAIN LINE	+	SIGN
---	EX. MINOR CONTOURS	(S)	SEWER MANHOLE	CB	STORM CATCH BASIN	+	STREET LIGHT
TC: XX.XX	EX. SPOT ELEVATION	EX. W	WATER LINE	(O)	STORM MANHOLE	+	TREE
CE: XX.XX	EASEMENT LINE AS NOTED	ww	WATER VALVE	GAS	GAS LINE	+	ROAD CENTERLINE
		+	FIRE HYDRANT	X X	FENCE		

PROPOSED GRADING LEGEND:

G=XX.XX	GUTTER ELEVATION, TC = G+0.5'	---	PROPERTY LINE	---	SETBACK	+	END SECTION	+	GATE VALVE	+	HEAVY DUTY PAVEMENT
P=XX.XX	PAVEMENT ELEVATION TC = P+0.5'	---	RIGHT OF WAY	---	FLOW ARROW	(SD)	STORM MANHOLE	+	FIRE HYDRANT	+	LIGHT DUTY PAVEMENT
C=XX.XX	CONCRETE ELEVATION	---	CURB AND GUTTER	+	CATCH BASIN	(O)	NYLOPLAST BASIN	+	SEWER MANHOLE	+	
		---	RL	---	STORM PIPE	+	WATER METER	+	CONCRETE PAVEMENT		

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 SHEET TITLE: PRELIMINARY GRADING PLAN

PAGE NO.: 1 OF 3
 SHEET NO.: C3.10

LOCATION: Z:\SHARED\PROJECTS\EMPIRE\VOLUME GARDEN - SCOTTSDALE - SWC OSBORN & SCOTTSDALE RD - 210126\11 CAD (SEG)\11.3 ENTITLEMENT-PLANNING\210126-C3.00.DWG
 SAVER BY: JUAN CHACON DATE: 11/17/2021

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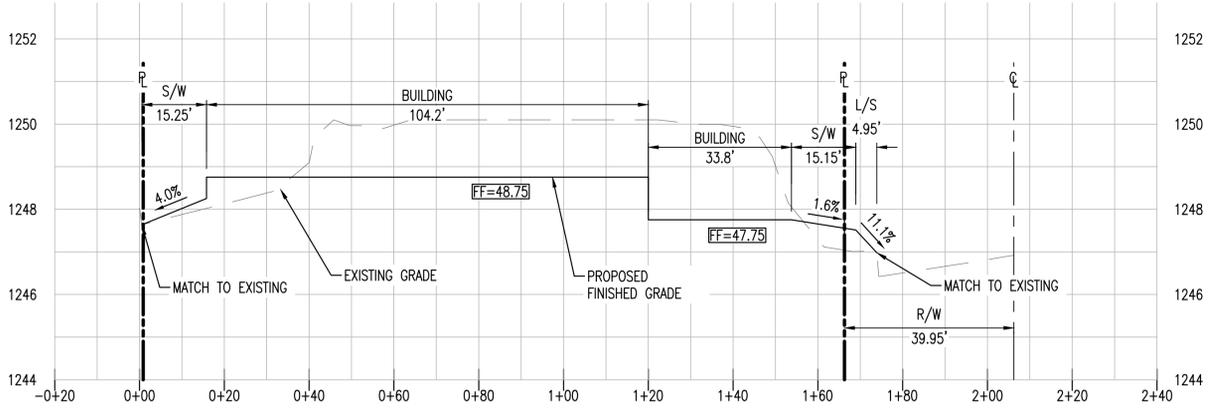
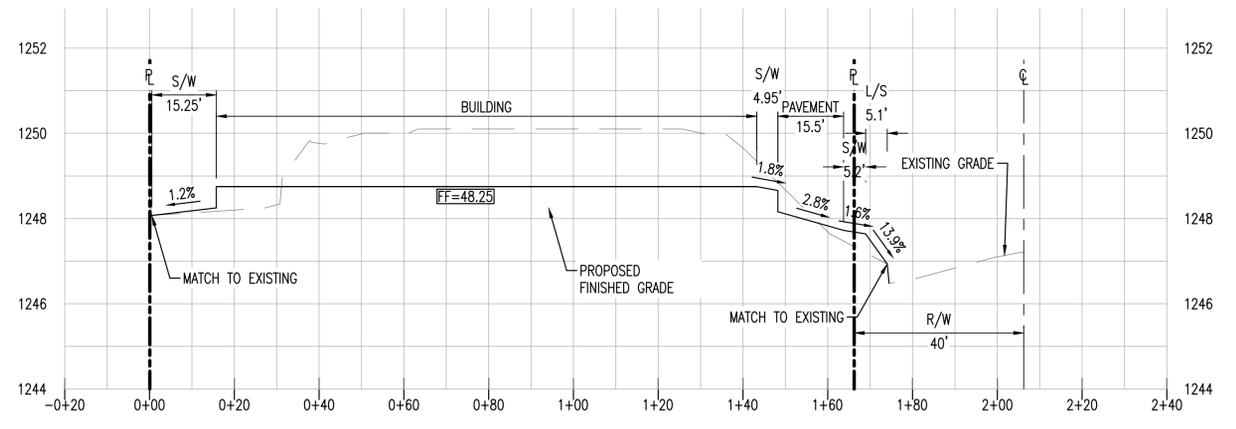
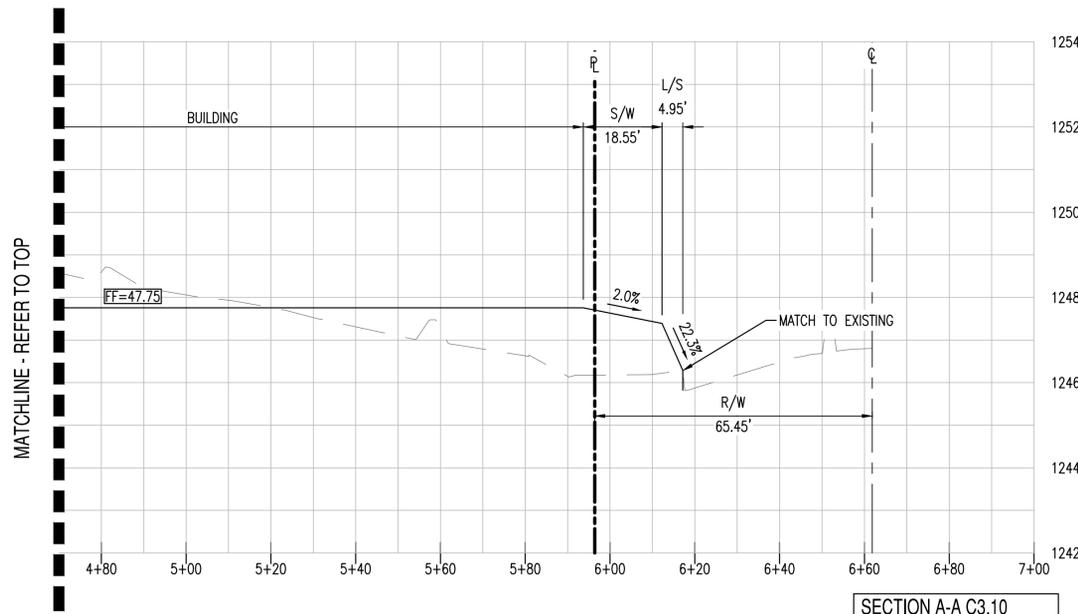
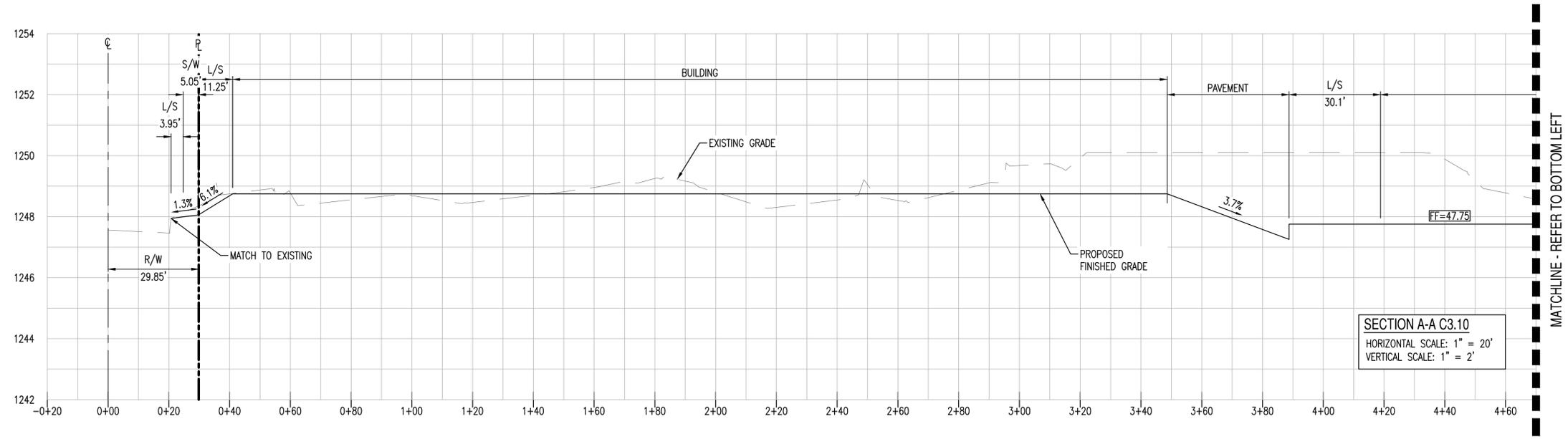
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 CROSS SECTIONS**

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SECTION C-C C3.10
 HORIZONTAL SCALE: 1" = 20'
 VERTICAL SCALE: 1" = 2'

APPENDIX IV

Plan C1.4 “Stormdrain Plan” – Hilgart Wilson

