GOLD DUST APARTMENTS

Preliminary Drainage Report

Prepared For: ESG Architecture & Design

April 17, 2023

DIBBLE

1122028

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Preliminary Drainage Report

10050 N Scottsdale Road, Paradise Valley, AZ

1122028

Prepared For: ESG Architecture & Design
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April 17, 2023

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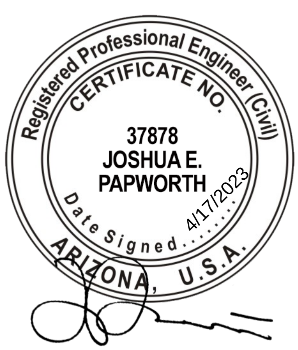

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1. INTRODUCTION

This report presents a preliminary drainage plan for the City of Scottsdale as a part of the Gold Dust Apartments project. The Gold Dust Apartments site is approximately 4.8 acres and fully developed. There is an existing wash on the west limit of the site. The project proposes a new mixed-use building and associated utility and hardscape improvements. This report presents the drainage analysis and results for the project.

The project is in the northeast quadrant of Township 3 North, Range 4 East, Section 27 and has an Assessor's Parcel Number (APN) of 175-56-002H. See **Figure 1** for a location map.

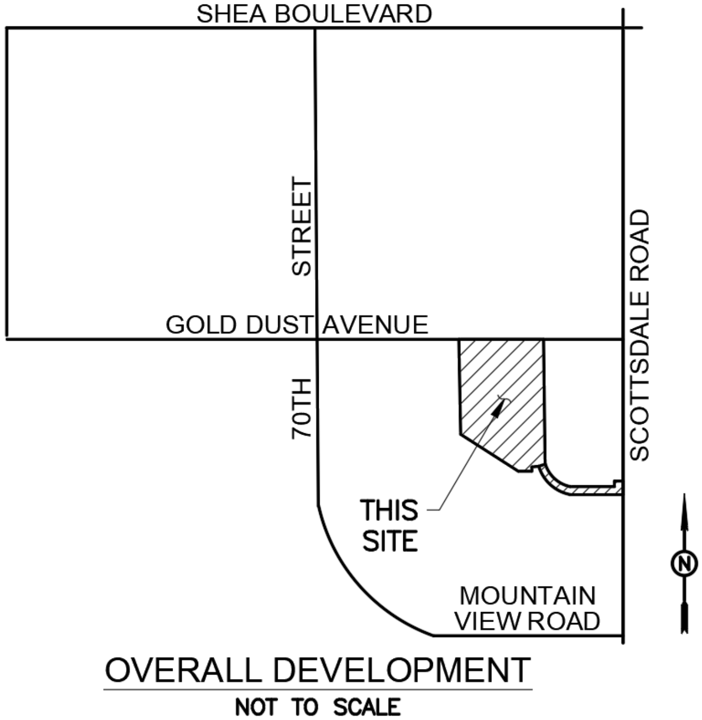

Figure 1 – VICINITY MAP

1.1 PURPOSE

The drainage analysis and design documented herein supports the zoning submittal for the ultimate buildout of Gold Dust Apartments. The design will provide drainage infrastructure to meet the City of Scottsdale and the Flood Control District of Maricopa County requirements for stormwater protection and floodplain management.

1.2 STUDY AREA

The Gold Dust Apartments project is bounded by a wash to the west of the site, existing infrastructure to the east of the site, Gold Dust Avenue to the north of the site, and residential apartment buildings to the south. On-site improvements planned for this 4.8-acre site include extensions of an existing building, hardscape improvements, and associated utility and drainage improvements, which will be discussed within this report.

2. EXISTING CONDITIONS

This site is in a FEMA Zone 'X', as defined as "Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depth less than 1 foot or areas less than 1 square mile; and areas protected by levees from 1% annual chance flood." This hazard designation is considered minimal, and flood insurance is not federally mandated. The FEMA Flood Insurance Rate (FIRM) panel for this area is provided in **Appendix B**.

2.1 ON-SITE

Currently this site is a 4.8-acre parcel that is fully developed and uses most of its land use on an existing building and parking lot. The site is generally flat with an existing wash on the west side of the site. This wash runs the entire west side of the parcel as it forces water travel from north to south. The existing surface forces water runoff to run from the northeast to the southwest of the site There is an existing 24-inch storm drain on the southwest side of the building. This storm drain exports water straight into the existing wash that is located on the west end of the site. There is a 12-inch storm drain on the west side of the building. This storm drain also exports water straight into the existing wash that is located on the west side of the site. The existing building had three different elevations within the building as stairs led up to different sections of the building. The finished floor elevations are 1342.48 in the north end of the building. 1341.68 in the middle section of the building and 1340.78 at the southernmost end of the building. The ultimate site outfall is located at the southwestern edge of the site in the existing wash at an elevation of 1335.16.

2.2 OFF-SITE

An unnamed channel exists along the western property boundary of the site. The channel extends approximately 1000 feet downstream of the site where it joins a channel at Mountain View Road, ultimately emptying to Indian Bend Wash. The channel was designed as part of previous adjacent development and consists of a partially natural bed and banks with areas of riprap erosion control and grass and desert landscaped bank lining. As-built plans date the original channel construction as 1984, and historical aerial photography supports that modifications resulting in its current configuration took place as late as 1996. A field investigation of the wash was conducted by Dibble engineers on July 12, 2022. The wash showed few signs of instability with intermittent areas of local scour. Local scour appeared to be limited to culvert outlets and at the toes of bends on the west bank. Selected field photographs of the channel can be found in **Appendix E**.

3. DESIGN CRITERIA

The project is designed consistent with standards set forth in the City of Scottsdale *(DS&PM)* and the Flood Control District of Maricopa County (FDCMC) Drainage Design Manual for Maricopa Count Volume I Hydrology (2018) and Volume II Hydraulics (2018) with exceptions noted herein.

Examples of specific design criteria for various design elements are provided in the following sections.

3.1 ON-SITE RAINFALL DEPTH

Rainfall depth was determined to be 0.5 inches based on the first flush retention requirement for the project.

Explain why Pre-v-Post is not used.

3.2 ON-SITE RAINFALL INTENSITY

Rainfall intensities of 4.73 inches and 7.48 inches were used to analyze the capacity of the proposed system based on the 10 year, 5-min storm and the 100-year, 5-min storm respectively. Rainfall precipitation values were obtained directly from the NOAA Atlas 14 Precipitation Data Frequency Server. NOAA Atlas 14 rainfall intensities may be found in **Appendix C.**

3.3 ON-SITE RUNOFF COEFFICIENT

The existing site may be categorized as being fully developed with parking lot, hardscape, and structures. The runoff coefficient for paved streets, parking lots, roofs and driveways shall be 0.95, as defined by the city. For areas using desert landscaping a runoff coefficient of 0.45, as defined by the city.

4. STUDY APPROACH

4.1 ON-SITE HYDROLOGY & HYDRAULICS

Stormwater will be collected for storm drain conveyance via combination of roof drains and surface inline storm drain inlets. Drainage areas are shown on the Preliminary Grading and Drainage Plan in **Appendix A**. Surface inlets are provided to capture runoff generated from the landscape/hardscape areas. The minimum grated area opening is 24 inches. Roof drain connections are planned to convey stormwater runoff to the storm drain pipe and to the underground retention tanks, then ultimately to the existing wash. The peak flow for each inlet was calculated with **Equation 1** below.

Equation 1 – RATIONAL METHOD

$$Q = CiA$$

Q = peak discharge (cubic feet per second)
C = runoff coefficient
I = average rainfall intensity (inch/hour) for duration Tc (5 minutes minimum)
A = drainage area (acres)

The pre vs post volumes were calculated for the project site, and the first flush volume was greater. The proposed onsite StormCapture retention tanks are concrete and are designed to store the first flush as required by the DS&PM. A Maxwell Plus dual chamber drywell is planned to dissipate basin storm water within 36 hours. The required first flush retention volume was determined by using **Equation 2**, and these calculations can be reviewed in **Appendix D.**

Equation 2 – REQUIRED FIRST FLUSH RETENTION VOLUME

$$V_R = 1\left(\frac{P}{12}\right)A,$$

V_R = retention volume required (cubic feet)
P = 0.5 (inch), per City of Scottsdale
A = area of project, (square feet)

Storm drain outlet capacity was calculated using **Equation 3** below, the overflow pipe to the wash will have a backwater valve to prevent water from the wash from entering the retention chamber. See **Appendix D** for provided retention volume calculations.

Equation 3 – STORM DRAIN OUTLET CAPACITY

$$\boldsymbol{Q = \left(\frac{1.49}{n}\right) * AR^{0.67}S^{0.5}}$$

Q = flow rate (cubic feet per second)
A = flow area (square feet)
R = hydraulic radius (feet)
S = slope (feet/feet)
n = manning's number

Bleed off time was calculated using **Equation 4** below. See **Appendix D** for provided retention volume calculations.

Equation 4 – BLEED OFF TIME

$$t = \left(\frac{V_R}{Q}\right) / \frac{1\ hour}{3600\ s}$$

t = bleed off time (hours)
A = retention volume provided (cubic feet)
Q = flow rate (cubic feet per second) from outlet pipe or drywell (0.1 CFS [Minimum rate per MCFCD])

4.2 OFF-SITE HYDROLOGY

The unnamed wash at the western property boundary is within the *East Shea Area Drainage Master Study/Plan* (ADMS/P) project area. A previous study, the *Lower Indian Bend Wash Area Drainage Master Study* (ADMS) included the area of the project site as well; however, the area was considered a buffer area for flow exchanged between low-resolution modeling areas and focused modeling areas. Therefore, use of the results in the buffer were advised to be used with caution, and Dibble has decided to make use of the more recent East Shea ADMS/P modeling. As of the writing of this report, all components of the East Shea ADMS modeling are complete, the models have completed multiple rounds of review by the Flood Control District of Maricopa County (FCDMC), and the hydrology is in a final review stage. For this project, Dibble has been provided the ADMS's 2-dimensional hydrology models, reviewed the models for appropriateness in the context of the Gold Dust Apartments project, and adopted them for use. All modeling for use in establishing peak discharges at the Gold Dust Apartments site is being submitted under this report's seal.

The area was modeling using the Professional Version of FLO-2D (FLO-2D PRO) (FLO-2D Software, Inc., 2018), Build No. 19.07.21, having an executable date of March 20, 2020. Dibble created a 100-year, 6-hour version of the model to ensure the greater of the 24-hour and 6-hour storm results are used for analysis and design of the unnamed wash on the site. The project is within Subdomain 1 of the East Shea ADMS/P model area and the 6-hour storm duration was found to produce the highest 100-year peak discharges at the site. **Figure 2** provides maximum depth results and peak discharges at the project site. **Exhibit F-1** in **Appendix F** provides results for all of Subdomain 1. Additional supporting documentation of the FLO-2D modeling parameters is provided with the electronic data for this report.

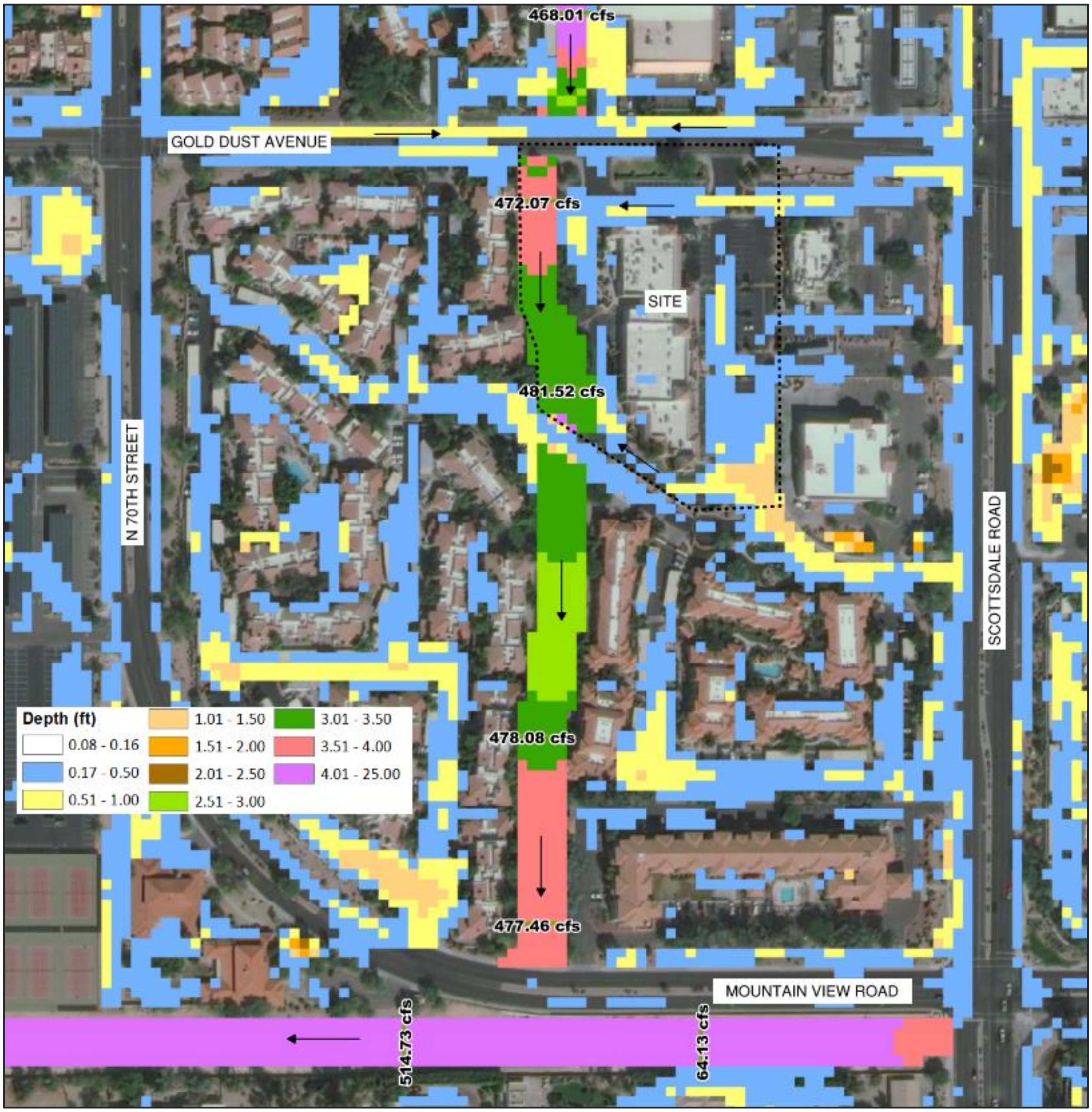

Figure 2 – OFFSITE HYDROLOGY RESULTS

4.3 OFF-SITE HYDRAULICS

Detailed, one-dimensional hydraulic modeling of the unnamed channel at the site was performed using GeoHEC-RAS v.3.1.0 software. Hydraulic modeling provided the existing condition and post-project water surface elevations at the site boundary and basis of scour and lateral migration potential computations.

Hydraulic modeling made use of field survey of the wash obtained in October of 2021. Field survey was supplemented for reaches upstream and downstream of the project site with Quality Level 1 LiDAR data, sampled to a ½ meter digital elevation model, obtained from the United States Geological Survey. Elevations provided herein are on NAVD 88 vertical datum.

Post-project hydraulic modeling included modification of cross sections for the proposed bank grading and Manning's roughness values were modified to represent the proposed riprap lining.

Table 1 provides Manning's n values selected for hydraulic modeling.

Table 1 – MANNING'S N VALUES

Ground Feature	Manning's N Value
Shotcrete Lining	0.019
Wash Bed	0.035-0.040
Wash Bank – Grass Landscaping	0.035
Wash Bank - Vegetated	0.04-0.05
Riprap Lining (D50=4")	0.040

Hydraulic modeling results, including water surface profiles, can be found in **Appendix G**. Water surface elevations at selected locations are provided on the Preliminary Grading and Drainage Plan.

4.4 OFF-SITE SCOUR & LATERAL MIGRATION POTENTIAL

The potential for lateral erosion at the east bank of the unnamed channel was performed in a spreadsheet and followed methodology provided in Arizona Department of Water Resources (ADWR) *State Standard Attachment 5-96 Watercourse Sediment Balance* (SSA 5-96). Bed and bank sediment samples were analyzed to provide gradations. Level II analyses were performed for both the bed and bank gradation data at each HEC-RAS cross section fronting the site. The more conservative of the results of both were used as the basis for erodibility. These calculations can be found in **Appendix H**. A summary of the erodibility estimate results are provided in **Table 2** below. Level II analysis guidance states that if either the allowable velocity or tractive stress analysis approach support a non-erosive result, the channel may be considered non-erosive. These results support that an approximately 150-foot length of the existing channel, beginning at the downstream end of an existing concrete access ramp and extending south, is potentially erosive and has lateral migration potential during the peak of the 100-year storm event.

Table 2 – UNNAMED CHANNEL MIGRATION POTENTIAL SUMMARY

HEC-RAS CROSS SECTION	SS 5-96 LEVEL II APPROACH	
	ALLOWABLE VELOCITY	TRACTIVE STRESS ANALYSIS
1572	Erosive	Not Erosive
1496	Erosive	Erosive
1422	Erosive	Erosive
1319	Erosive	Erosive
1223	Erosive	Not Erosive
1193	Not Erosive	Not Erosive

Scour calculations were performed using post-project HEC-RAS modeling results and wash bed sample gradation. The analysis included the summation of several scour components. The following equation was used to estimate the total scour.

Equation 5 – TOTAL SCOUR

$$Z_t = FS * (Z_{long\text{-}term} + Z_{general} + Z_{bend} + Z_{bedform} + Z_{low\ flow} + Z_{local})$$

Where Zt is the total design scour and FS is the factor of safety. A FS of 1.3 has been used for this condition.

Long-term scour, $Z_{long\text{-}term}$, is an estimation of the ultimate degradation of movable channel beds as they seek an equilibrium sediment transport condition. The reach is protected from long-term scour by the concrete box culvert at its downstream end. This hard point cuts off progression of long-term degradation. Also, the existing bed slope is nearly 0%. For these reasons, a value of zero was used for the long-term scour component.

General scour, $Z_{general}$, is the component of scour that represents the mobile portion of the bed-material of the channel bottom during the peak flow event. General scour was computed using the Blanch Equation, which is well suited to channels in which there is little or no sediment supply from the upstream reach. Upstream of Gold Dust Avenue, the wash is completely channelized with a concrete lining.

Bend scour, Z_{bend}, occurs on the outside of bends in the wash channel. The Blanch Equation for general scour accounts for a moderate bend. Therefore, this value was set to zero.

Bedform scour, $Z_{bedform}$, also known as dune and anti-dune scour is the component of scour caused by movement of dune shaped bed forms along the bottom of the channel and is primarily confined to sand bed washes. The bedform scour component was estimated by calculating both dune and anti-dune scour. The actual type of bedform present is a function of the flow regime. Since the flow regime may change with the fluctuating discharges of the flood hydrograph, both bedforms could be present in the project reach during a flood event. The scour component is limited to a maximum of ½ the dune height or ½ the flow depth.

Low-flow thalweg scour, Z_{lft}, occurs if a small wash forms to convey minor flows within the main channel of a wash cross-section. The intent of the low-flow thalweg scour component is to address a feature that sometimes forms within large washes (with bottom widths greater than the natural bank full width) with a high width to depth ratio and with mobile bed sediments. Based on field observation, a low-flow depression already exists in the wash bed, and computed scour depth was applied below this elevation. Therefore, a value of zero was used for this component.

Local scour, Z_{local}, results from an obstruction and abrupt change in the direction of flow. It occurs at bridge piers, abutments, embankments, and other structures obstructing flow. There are no local scour elements in this reach. Therefore, the value for this item was set to zero.

Scour calculations are provided in **Appendix H**, and the total scour result is discussed in **Section 5.4**.

5. PROPOSED DRAINAGE CONDITIONS

The Preliminary Grading and Drainage Plans are provided within **Appendix A**.

5.1 ON-SITE

The Gold Dust Apartments project includes a new building and associated site improvements, Existing storm drains that lead to the wash will be removed. A combination of storm drains, roof drains, and surface flow will capture the runoff and outfall into underground concrete tanks. There will also be 1 drywell implemented to meet the requirement of bleed off in 36 hours.

A total of five (5) StormCapture concrete tanks will be used. These tanks are sized at 7x15x14. The volume required from these retention tanks were determined using the first flush precipitation depth of 0.5 inches. Any water in excess of the first flush will flow into the wash via pipes from the retention tank or surface runoff. The east drainage area was calculated to be 91,008 square feet and produces approximately 14.84 cfs and is planned to flow into the east storm drain. The west drainage area was calculated to be 65,116 square feet and is planned to drain into the retention tanks from a drain system internal to the garage. The total site drainage area is in the table below and summarizes the drainage calculations for the total site. Within the retention tanks, a system with a single storm drain is planned to allow extra water above the first flush (0.5-inch depth) to flow into the wash. The first flush outflows will be treated with a drywell within the StormCapture concrete tanks. The water will be discharged back into the ground. See **Appendix I** for storm drain profiles based on the 100-year storm, the overflow pipe for bypass beyond first flush has not been modeled, but is sized to match the highest flow in the on-site conveyance.

How many drywells will be required? Reference the drywell calculations here.

Refer to **Table 3** for a summary of the drainage calculations.

Table 3 - PROPOSED DRAINAGE VOLUME SUMMARY

DRAINAGE AREA	DRAINAGE AREA [SF]	VOLUME REQUIRED [CF]	VOLUME PROVIDED [CF]	BLEED OFF TIME [HR]
A	156,124	6,180	7,350.00	17.2

The proposed site has a volume required less than that of the existing site due to the increase of desert landscaping areas. See **Appendix D** for volume calculations.

5.2 MAINTENANCE PLAN

Regular maintenance is recommended annually. Deposited silt and sediments may need to be removed from the retention tanks annually. It is recommended that the removal of sediment is to be done when levels fill up to 10 percent of the effective settling capacity.

5.3 FINSIHED FLOORS

The site drainage design will provide protection of the proposed building on the site from the 100-year storm event. The lowest habitable finish floor is 1344.50, which is approximately 3.5 feet above the ultimate overland outfall of the site, and 4.44 feet above the 100-year water surface elevation (1340.06) at the north end of the building on the high end of the site. The wash will be armored to protect from lateral migration, and there is no indication of water reaching the underground parking, the walls will be waterproofed, but not floodproofed based on their distance from the wash.

5.4 OFF-SITE

Hydraulic modeling of the unnamed channel on the western site boundary supports that the 100-year design storm event is contained within the existing channel banks. Water surface elevations at selected locations are provided on the Grading and Drainage Plan.

An assessment of the erodibility of the wash and potential for lateral migration supports that a 150-foot length of the existing channel, beginning at the downstream end of an existing concrete access ramp and extending south, is potentially erosive and has lateral migration potential during the peak of the 100-year storm event.

To eliminate the potential for lateral migration, riprap erosion protection has been designed for the the at-risk portion of the east bank. The riprap lining follows the existing bank geometry with some smoothing of existing irregularities in the bank slope. The proposed condition maximum velocity is 5 feet per second, and the calculated total scour depth of the bed, Z_t, is 2.3'. A riprap D_{50} value of 4 inches and an apron depth of 12 inches have been provided for bank protection. A riprap sizing calculation is provided in **Appendix H**. The bank protection extends below grade (a toe-down) to below the calculated total scour depth. The total scour depth was applied to the lowest, adjacent bed elevation. Because the proposed bank toe elevation is slightly above the existing bed elevation at some locations, a constant toe-down depth below the toe was calculated to provide a value deep enough for all locations. This resulted in the toe-down dimension of 3.2' shown on the Grading and Drainage Plan. The remainder of the channel was found to be non-erodible, and, therefore, no erosion setback or lining is necessary. No other modifications to the existing channel are expected.

6. CONCLUSIONS

The construction of the Gold Dust Apartments facility drainage is designed to retain the first flush runoff as part of the proposed site improvements. Flows are routed via roof drains, surface drainage and storm drain pipes to underground retention tanks and the existing wash. The drainage features associated with this project will have adequate capacity to retain the first flush and to convey the 100-year storm. The ultimate outfall is located on the southwest corner of the site over the box culvert at an elevation of 1340.56 or at the bottom of the culvert in smaller storms at 1335.16. The unnamed wash at the western site boundary has the capacity to convey the 100-year peak discharge without overtopping its banks and riprap bank protection of erodible areas will eliminate the potential for lateral migration into the developed areas of the site.

7. REFERENCES

City of Scottsdale, *Design Standards and Policies Manual*, 2018.

Flood Control District of Maricopa County, *Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology,* 2018.

Arizona Department of Water Resources, *State Standard Attachment SSA 5-96 for Watercourse System Sediment Balance*, 1996

Appendix A – PRELIMINARY GRADING AND DRAINAGE PLAN

Gold Dust Ave & Scottsdale Rd

Scottsdale, AZ

HIGHSTREET RESIDENTIAL

DIBBLE

eSG ARCHITECTURE & DESIGN

500 Washington Avenue South, Suite 1080
Minneapolis, MN 55415
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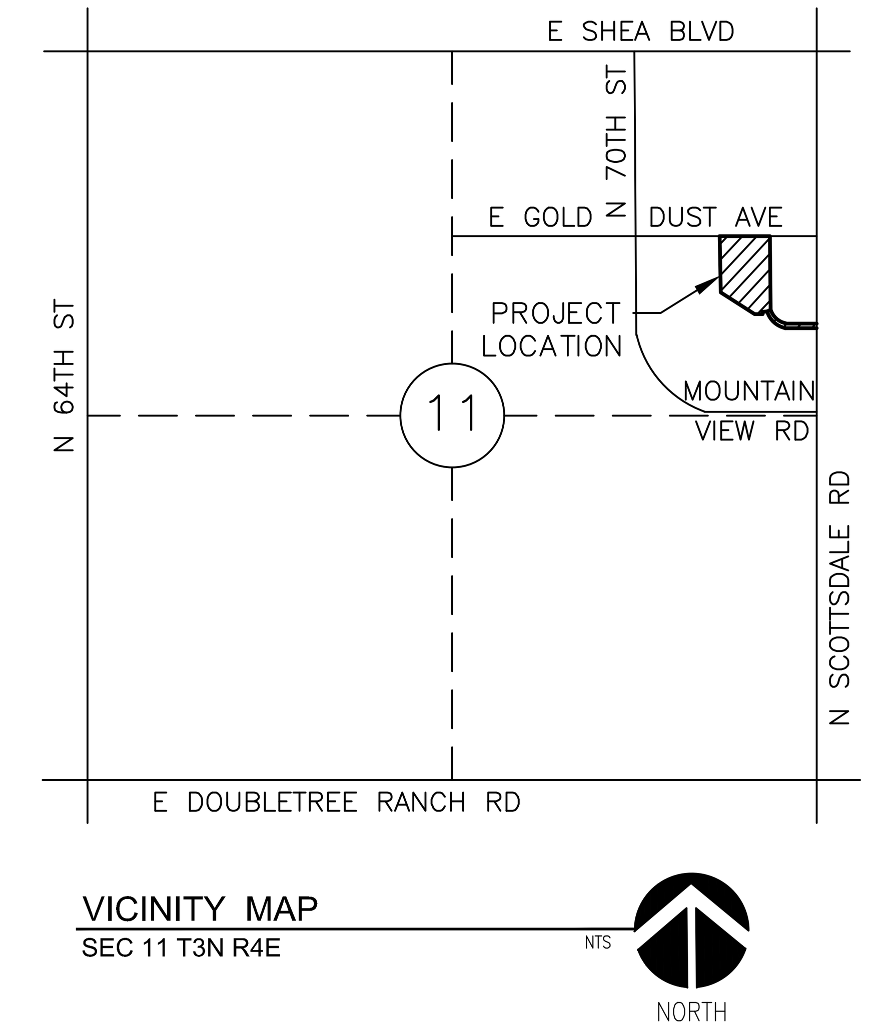
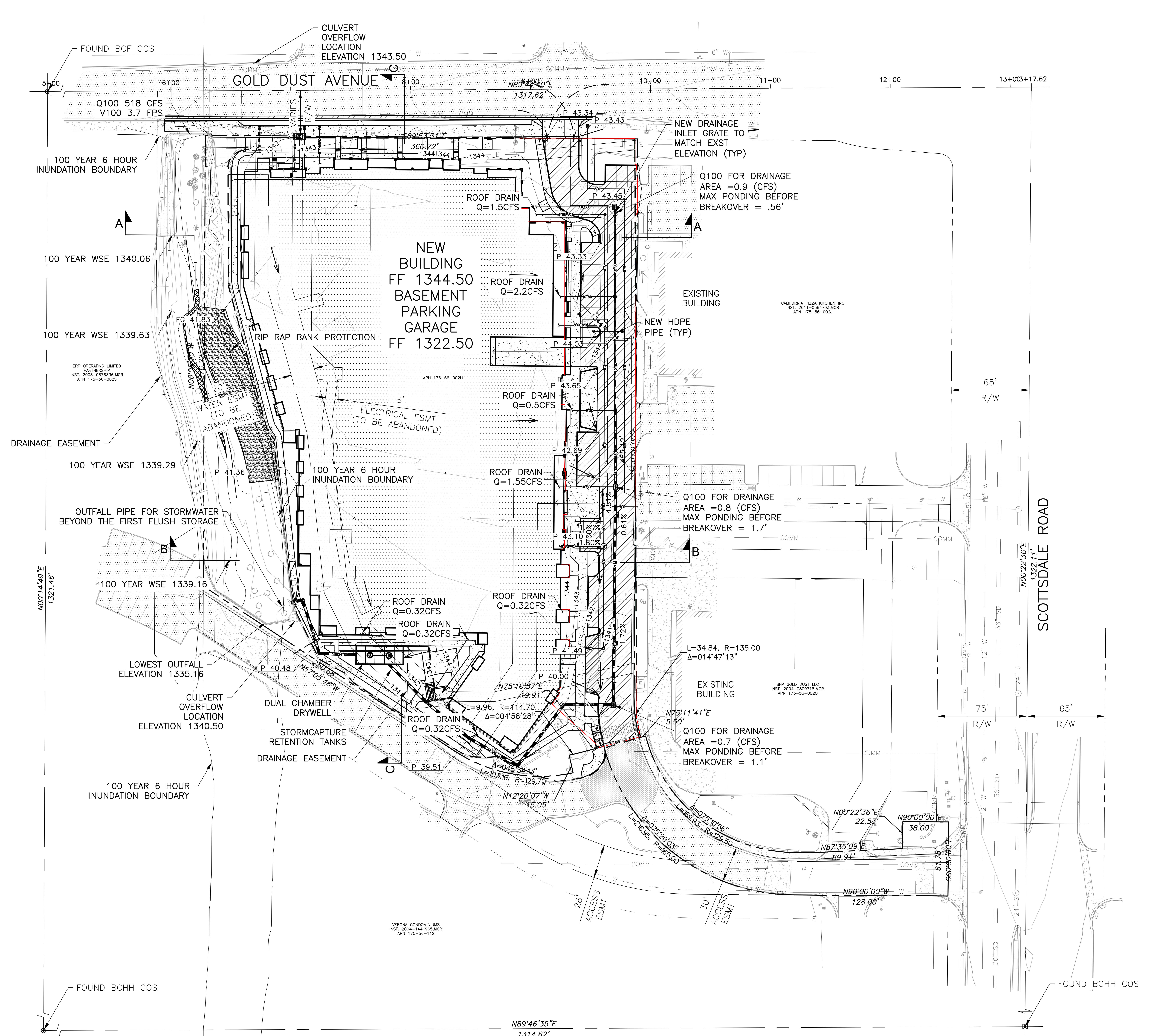


VICINITY MAP

SEC 11 T3N R4E



RETENTION CALCULATIONS

DRAINAGE AREA

VOLUME REQUIRED:
V=C*A*(P/12)
V=(0.95)*(156,124 SF)*(0.5 IN/12)
V=6,180 CF

VOLUME PROVIDED:
FIVE 14 FT TALL STORM CAPTURE RETENTION TANKS @ VOLUME PER TANK CF/TANK = 1,470 CF

TOTAL VOLUME (5) TANKS = 7,350 CF

1 DRYWELLS TO DRAIN IN 17.2 HRS

NOTE:
1. ALL CURB RAMPS ON THE SUBJECT PARCEL, AND WITHIN THE PUBLIC ROW AND EASEMENTS, TO BE INVESTIGATED FOR ADA COMPLIANCE AND UPDATED ACCORDINGLY.

LEGEND

← PROPOSED FLOW DIRECTION

— — DRAINAGE BOUNDARY

0' 20' 40' 80'
SCALE: 1"=40'

Contact Arizona 811 at least two full working days before you begin excavation. ARIZONA811 Call 811 or click Arizona811.com

PRELIMINARY GRADING & DRAINAGE PLAN

C1.1

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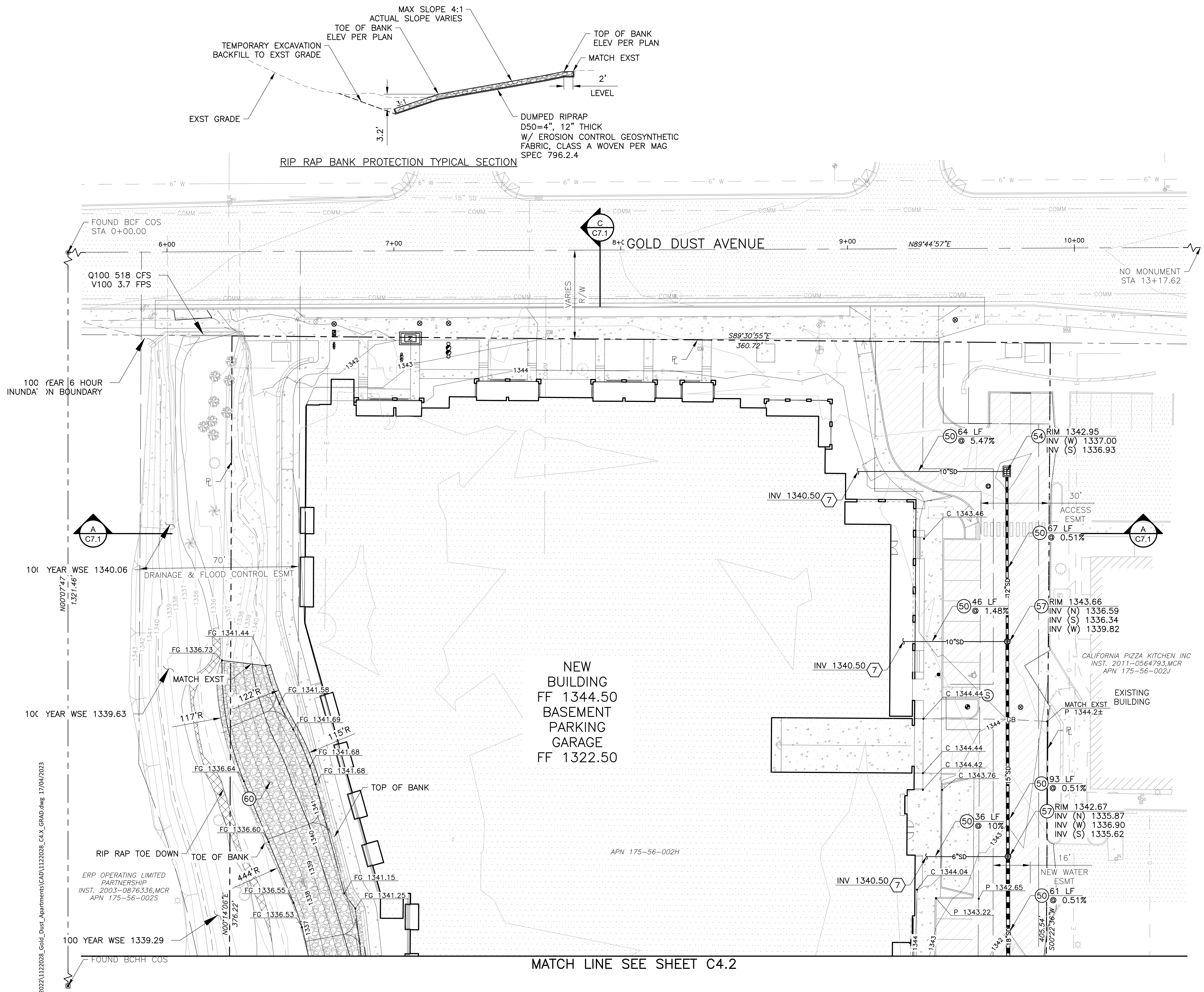

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CONSTRUCTION NOTES

(50) STORM DRAIN, HDPE (WATER TIGHT)
SIZE PER PLAN
ADS N-12 WT OR APPROVED EQUAL

(54) DRAIN BASIN W/2'X3' MAG GRATE
(H-20 TRAFFIC), DET 4, SHEET C6.1
ADS NYLOPLAST OR APPROVED EQUAL

(57) DRAIN BASIN W/GRATED LID (H-20 TRAFFIC)
DET 4, SHEET C6.1
ADS NYLOPLAST OR APPROVED EQUAL

(60) RIP RAP BANK PROTECTION
PER TYPICAL SECTION THIS SHEET

REFERENCE NOTES

(7) FOR CONTINUATION
REFER TO PLUMBING PLANS

NOTES:

1. ONLY STORM DRAIN UTILITIES SHOWN FOR CLARITY.
2. ADD 1000' TO ALL SPOT GRADES & INVERT ELEVATIONS.
3. ALL TOP OF CURBS ARE 6" ABOVE PAVEMENT ELEVATION UNLESS OTHERWISE NOTED.
4. ALL UTILITY SURFACE FEATURES SHALL BE ADJUSTED TO FINISH GRADE INCLUDING BUT NOT LIMITED TO UTILITY BOXES, VALVES, METER BOXES, VAULTS, MANHOLES, BACKFLOWS, GRATES, ETC.
5. ALL NEW SIDEWALK, PAVING AND OTHER HARDSCAPE FEATURES SHALL MATCH IN WITH EXISTING ADJACENT GRADES.
6. ALL SIDEWALK & ADA ROUTES SHALL BE CONSTRUCTED WITH 5% MAX SLOPE IN THE DIRECTION OF TRAVEL AND 2% MAX CROSS SLOPE.

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Gold Dust Ave & Scottsdale Rd

Scottsdale, AZ

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DRB
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04/17/2023

ORIGINAL ISSUE:

REVISIONS

No.	Description	Date

221564
PROJECT NUMBER

KR
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SM
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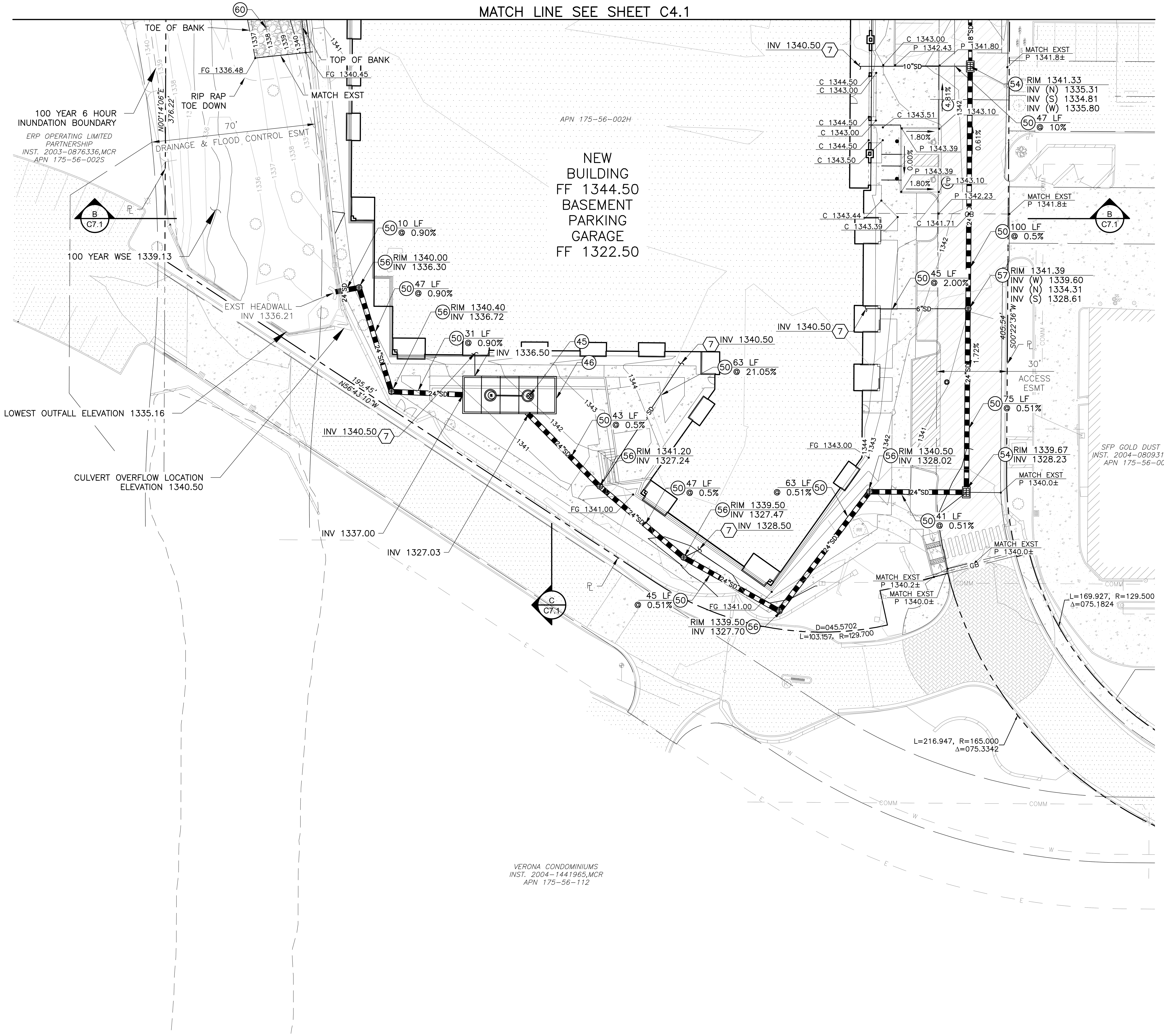
KEY PLAN

Gold Dust Ave &
Scottsdale Rd

GRADING & DRAINAGE PLAN

C4.1



CONSTRUCTION NOTES

- (45) DUAL CHAMBER DRYWELL MAXWELL PLUS OR APPROVED EQUAL DET 2, SHEET C6.2
- (46) STORMCAPTURE CONCRETE TANKS DET 1, SHEET C6.2
- (50) STORM DRAIN, HDPE (WATER TIGHT) SIZE PER PLAN ADS N-12 WT OR APPROVED EQUAL
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- (56) DRAIN BASIN W/GRATED LID (PEDESTRIAN) DET 5, SHEET C6.1 ADS NYLOPLAST OR APPROVED EQUAL
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SM — CHECKED BY

KEY PLAN

Gold Dust Ave & Scottsdale Rd

GRADING & DRAINAGE PLAN

C4.2

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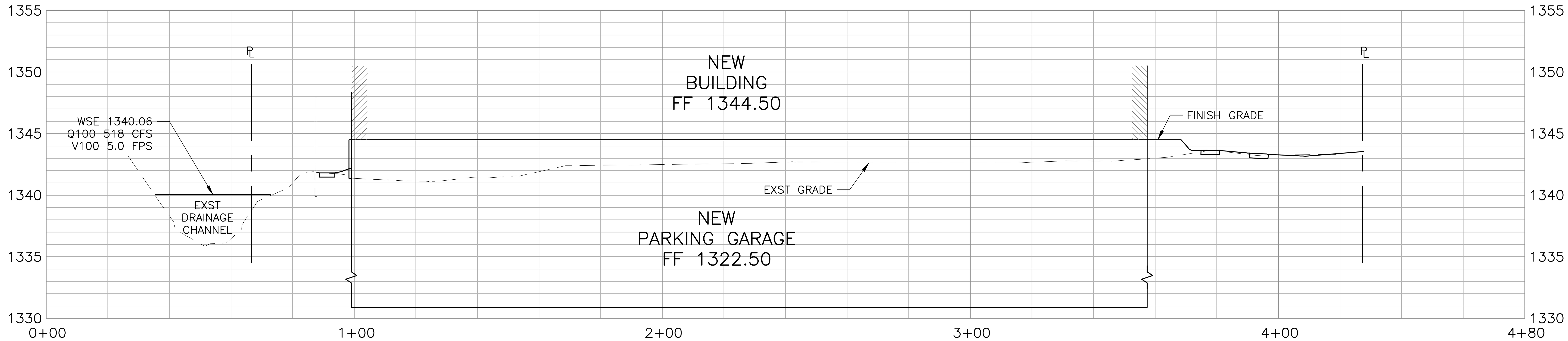
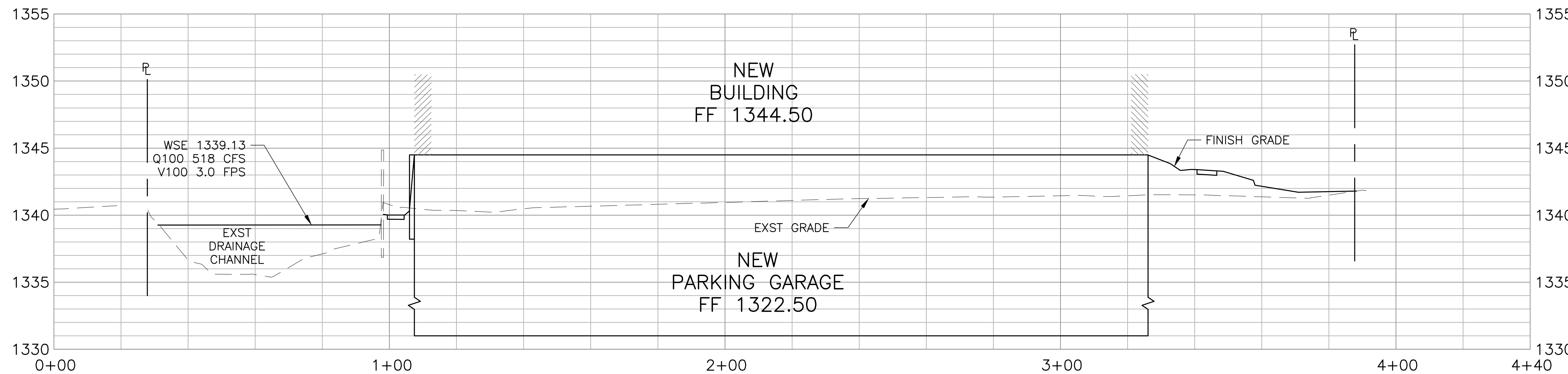
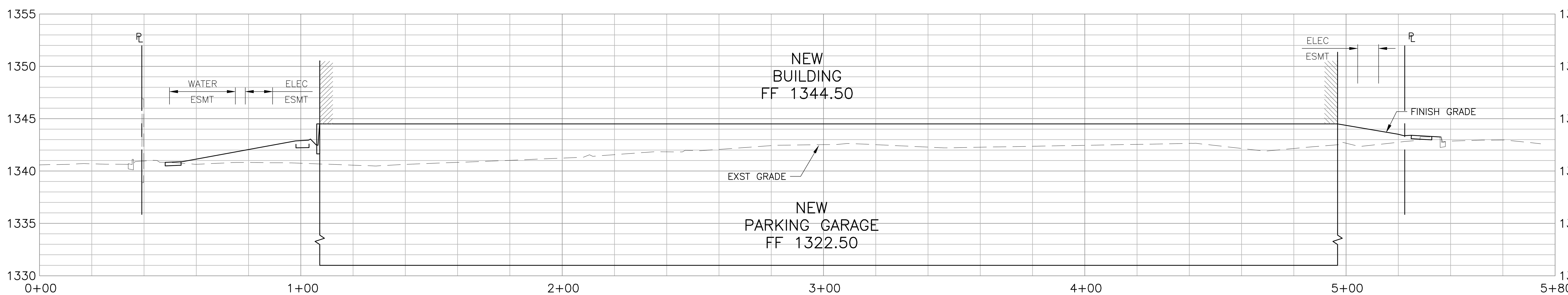
p 612.339.5508 | f 612.339.5382
www.esgarch.com



SECTION A-A



SECTION B-B



SECTION C-C

G:\2022\1122028_Gold_Dust_Apartments\CAD\1122028_C7_X_XSEC.dwg 17/04/2023

DRB
RE-SUBMITTAL
04/17/2023

ORIGINAL ISSUE:

REVISIONS

No.	Description	Date

221564
PROJECT NUMBER

KR — DRAWN BY
SM — CHECKED BY

KEY PLAN

Gold Dust Ave &
Scottsdale Rd

CROSS SECTIONS

C7.1

Appendix B – FEMA FIRMETTE MAP

National Flood Hazard Layer FIRMette

FEMA

111°55'58"W 33°34'54"N

0 250 500 1,000 1,500 2,000 Feet

1:6,000

111°55'20"W 33°34'24"N

Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) *Zone A, V, A99*
		With BFE or Depth *Zone AE, AO, AH, VE, AR*
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
		Future Conditions 1% Annual Chance Flood Hazard *Zone X*
		Area with Reduced Flood Risk due to Levee. See Notes. *Zone X*
		Area with Flood Risk due to Levee *Zone D*
OTHER AREAS	NO SCREEN	Area of Minimal Flood Hazard *Zone X*
		Effective LOMRs
		Area of Undetermined Flood Hazard *Zone D*
GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES	B 20.2 17.5	Cross Sections with 1% Annual Chance Water Surface Elevation
	8	Coastal Transect
	513	Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature
MAP PANELS		Digital Data Available
		No Digital Data Available
		Unmapped

N

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 3/17/2022 at 1:15 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Arizona State Plane Central zone (FIPSZONE 0202). The **horizontal datum** was NAD 83 HARN, GRS1980 spheroid. Differences in datum, spheroid, projection or State Plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD 88). These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. Map users wishing to obtain flood elevations referenced to the National Geodetic Vertical Datum of 1929 (NGVD 29) may use the following Maricopa County website application: http://www.fcd.maricopa.gov/Maps/gismaps/apps/gdacs/application/index.cfm

This web tool allows users to obtain point-specific datum conversion values by zooming in and hovering over a VERTCON checkbox on the layers menu on the left side of the screen. The VERTCON grid referenced in this web application was also used to convert existing flood elevations from NGVD 29 to NAVD 88.

To obtain current elevation, description, and/or location information for National Geodetic Survey bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit its website at http://www.ngs.noaa.gov. To obtain information about Geodetic Densification and Cadastral Survey bench marks produced by the Maricopa County Department of Transportation, please visit the Flood Control District of Maricopa County website at:
http://www.fcd.maricopa.gov/Maps/gismaps/apps/gdacs/application/index.cfm

Base map information shown on this FIRM was derived from multiple sources. Aerial imagery was provided in digital format by the Maricopa County Department of Public Works, Flood Control District. The imagery is dated October 2009 to November 2009. Additional National Agricultural Imagery Program (NAIP) imagery was provided by the Arizona State Land Department (ALRIS) and is dated 2007. The coordinate system used for the production of the digital FIRM is State Plane Arizona Central NAD83 HARN, International Feet.

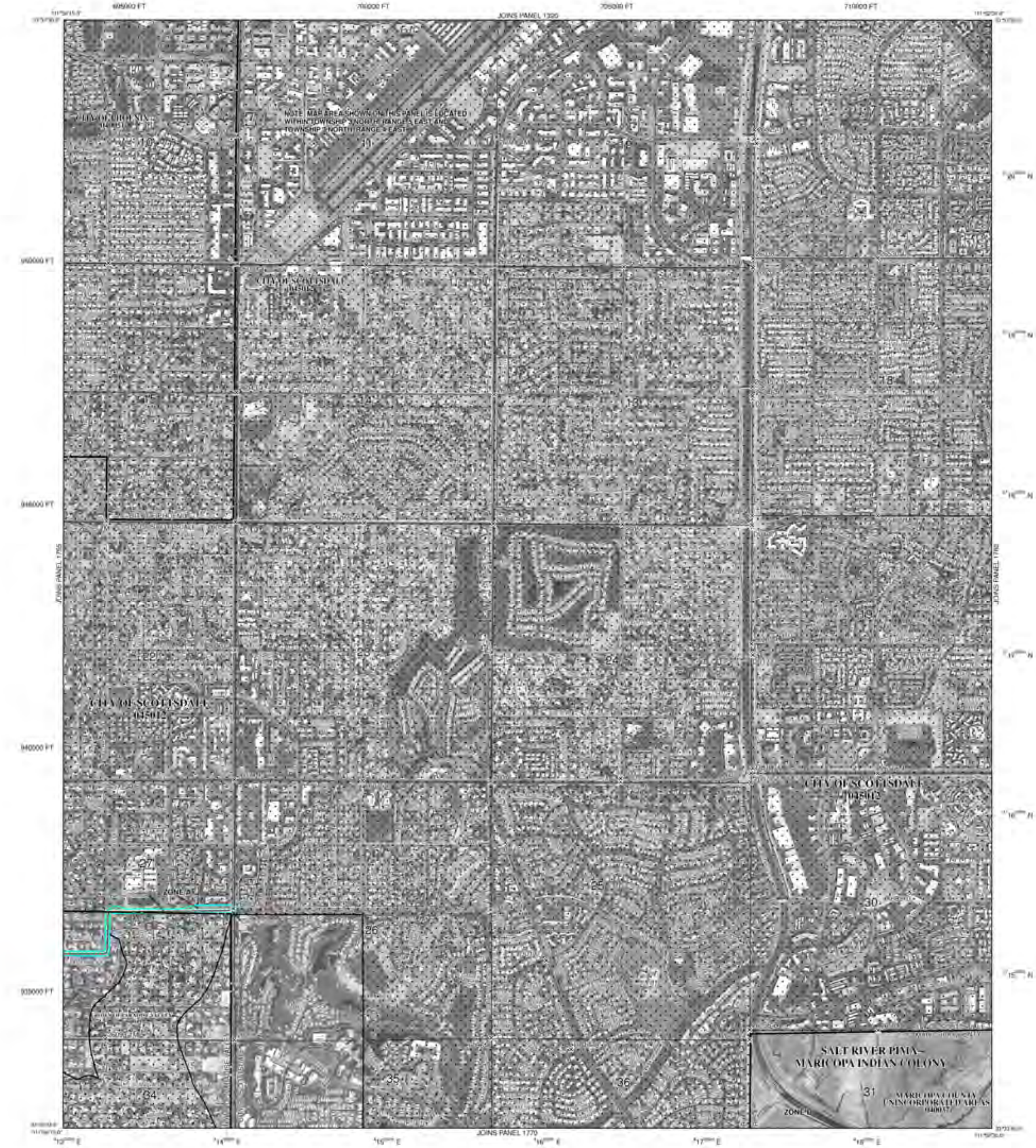
The **profile base line** depicted on this map represents the hydraulic modeling baselines that match flood profiles in the FIS report. As a result of improved topographic data, the profile base line, in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

For information on available products associated with this FIRM, visit the **FEMA Map Service Center (MSC)** website at http://msc.fema.gov. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, or digital versions of this map. Many of these products can be ordered or obtained directly from the MSC website.

If you have **questions about this map**, how to order products, or the National Flood Insurance Program in general, please call the **FEMA Map Information eXchange (FMIX)** at **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at http://www.fema.gov.



LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A	No Base Flood Elevations determined.
ZONE AE	Base Flood Elevations determined.
ZONE AH	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO	Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X	Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X	Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D	Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- 1% annual chance floodplain boundary
- 0.2% annual chance floodplain boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
- ~~~ 513 ~~~ Base Flood Elevation line and value; elevation in feet*
- Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

- (A)—(A) Cross section line
- (23)—(23) Transect line
- Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
- 1000-meter Universal Transverse Mercator grid ticks, zone 12
- 5000-foot grid ticks: Arizona State Plane coordinate system, central zone (FIPSZONE 0202), Transverse Mercator
- Bench mark (see explanation in Notes to Users section of this FIRM panel)
- River Mile

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 1000'

500 0 1000 2000 FEET

300 0 300 600 METERS

NFIP

PANEL 1760L

NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP
MARICOPA COUNTY, ARIZONA
AND INCORPORATED AREAS

PANEL 1760 OF 4425
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
[illegible]	[illegible]	[illegible]	L
[illegible]	[illegible]	[illegible]	L
[illegible]	[illegible]	[illegible]	L
[illegible]	[illegible]	[illegible]	L

MAP NUMBER
04013C1760L

MAP REVISED
OCTOBER 16, 2013

Federal Emergency Management Agency

Appendix C – NOAA ATLAS PRECIPITATION DATA

NOAA Atlas 14, Volume 1, Version 5
Location name: Paradise Valley, Arizona, USA*
Latitude: 33.5777°, Longitude: -111.9277°
Elevation: 1341.56 ft**
* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

DEPTH

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.186 (0.155-0.227)	0.243 (0.203-0.297)	0.328 (0.272-0.400)	0.394 (0.325-0.479)	0.483 (0.392-0.585)	0.551 (0.443-0.664)	0.623 (0.491-0.748)	0.694 (0.537-0.832)	0.790 (0.596-0.949)	0.863 (0.638-1.04)
10-min	0.283 (0.235-0.346)	0.369 (0.309-0.452)	0.499 (0.414-0.609)	0.600 (0.495-0.729)	0.736 (0.597-0.891)	0.840 (0.674-1.01)	0.948 (0.747-1.14)	1.06 (0.818-1.27)	1.20 (0.908-1.44)	1.31 (0.972-1.58)
15-min	0.351 (0.292-0.429)	0.458 (0.383-0.560)	0.619 (0.514-0.755)	0.743 (0.614-0.904)	0.912 (0.741-1.11)	1.04 (0.835-1.25)	1.18 (0.926-1.41)	1.31 (1.01-1.57)	1.49 (1.13-1.79)	1.63 (1.21-1.96)
30-min	0.473 (0.393-0.578)	0.617 (0.516-0.755)	0.834 (0.692-1.02)	1.00 (0.827-1.22)	1.23 (0.997-1.49)	1.40 (1.13-1.69)	1.58 (1.25-1.90)	1.76 (1.37-2.11)	2.01 (1.52-2.41)	2.19 (1.62-2.64)
60-min	0.585 (0.486-0.715)	0.763 (0.639-0.934)	1.03 (0.856-1.26)	1.24 (1.02-1.51)	1.52 (1.23-1.84)	1.74 (1.39-2.09)	1.96 (1.54-2.35)	2.18 (1.69-2.62)	2.49 (1.88-2.98)	2.71 (2.01-3.26)
2-hr	0.684 (0.577-0.817)	0.884 (0.749-1.06)	1.18 (0.992-1.41)	1.41 (1.17-1.67)	1.72 (1.42-2.03)	1.95 (1.59-2.30)	2.19 (1.75-2.58)	2.44 (1.92-2.87)	2.77 (2.13-3.26)	3.03 (2.27-3.58)
3-hr	0.762 (0.643-0.930)	0.977 (0.827-1.20)	1.28 (1.08-1.56)	1.52 (1.26-1.84)	1.85 (1.52-2.23)	2.12 (1.72-2.53)	2.39 (1.91-2.86)	2.68 (2.10-3.20)	3.08 (2.34-3.68)	3.40 (2.52-4.07)
6-hr	0.918 (0.789-1.09)	1.16 (0.997-1.38)	1.48 (1.27-1.75)	1.74 (1.47-2.04)	2.09 (1.75-2.44)	2.36 (1.95-2.75)	2.65 (2.15-3.08)	2.94 (2.34-3.43)	3.34 (2.59-3.89)	3.65 (2.77-4.26)
12-hr	1.02 (0.878-1.19)	1.28 (1.11-1.51)	1.62 (1.39-1.90)	1.88 (1.61-2.20)	2.24 (1.90-2.61)	2.51 (2.10-2.92)	2.80 (2.30-3.25)	3.08 (2.51-3.58)	3.46 (2.75-4.04)	3.76 (2.93-4.42)
24-hr	1.19 (1.03-1.39)	1.51 (1.31-1.77)	1.95 (1.69-2.29)	2.30 (1.99-2.69)	2.78 (2.39-3.25)	3.16 (2.70-3.69)	3.56 (3.01-4.16)	3.97 (3.33-4.64)	4.54 (3.75-5.30)	4.99 (4.07-5.84)
2-day	1.28 (1.11-1.49)	1.63 (1.42-1.90)	2.13 (1.85-2.49)	2.54 (2.19-2.95)	3.09 (2.65-3.59)	3.54 (3.01-4.10)	4.00 (3.38-4.65)	4.48 (3.76-5.22)	5.16 (4.26-6.01)	5.70 (4.65-6.65)
3-day	1.36 (1.19-1.58)	1.74 (1.52-2.02)	2.29 (1.99-2.65)	2.73 (2.37-3.16)	3.35 (2.88-3.87)	3.84 (3.29-4.44)	4.37 (3.71-5.05)	4.92 (4.14-5.70)	5.70 (4.73-6.59)	6.32 (5.20-7.34)
4-day	1.45 (1.27-1.67)	1.85 (1.62-2.14)	2.45 (2.13-2.82)	2.92 (2.54-3.37)	3.60 (3.11-4.15)	4.15 (3.56-4.78)	4.74 (4.04-5.45)	5.36 (4.53-6.18)	6.23 (5.20-7.18)	6.95 (5.74-8.03)
7-day	1.64 (1.42-1.90)	2.09 (1.82-2.42)	2.77 (2.40-3.20)	3.31 (2.86-3.83)	4.08 (3.51-4.72)	4.70 (4.02-5.43)	5.36 (4.55-6.19)	6.07 (5.10-7.02)	7.06 (5.86-8.16)	7.86 (6.46-9.10)
10-day	1.76 (1.54-2.03)	2.26 (1.97-2.60)	2.98 (2.59-3.43)	3.56 (3.08-4.09)	4.37 (3.77-5.02)	5.02 (4.31-5.76)	5.72 (4.87-6.56)	6.44 (5.45-7.41)	7.47 (6.23-8.58)	8.29 (6.85-9.54)
20-day	2.17 (1.91-2.50)	2.80 (2.45-3.21)	3.70 (3.24-4.24)	4.38 (3.82-5.01)	5.30 (4.60-6.05)	6.00 (5.19-6.85)	6.71 (5.78-7.69)	7.44 (6.37-8.53)	8.42 (7.15-9.68)	9.18 (7.74-10.6)
30-day	2.55 (2.22-2.92)	3.28 (2.87-3.76)	4.32 (3.77-4.94)	5.12 (4.46-5.84)	6.18 (5.36-7.06)	7.00 (6.05-7.98)	7.84 (6.74-8.94)	8.70 (7.44-9.89)	9.85 (8.36-11.2)	10.7 (9.04-12.3)
45-day	2.93 (2.58-3.34)	3.78 (3.32-4.31)	4.97 (4.37-5.66)	5.86 (5.14-6.67)	7.03 (6.14-8.00)	7.91 (6.88-9.00)	8.80 (7.61-10.0)	9.69 (8.35-11.0)	10.9 (9.29-12.4)	11.7 (9.98-13.4)
60-day	3.22 (2.85-3.66)	4.16 (3.67-4.72)	5.47 (4.83-6.20)	6.43 (5.65-7.28)	7.67 (6.73-8.68)	8.59 (7.50-9.72)	9.51 (8.27-10.8)	10.4 (9.02-11.8)	11.6 (9.98-13.2)	12.5 (10.7-14.2)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

Back to Top

PF graphical

NOAA Atlas 14, Volume 1, Version 5
Location name: Paradise Valley, Arizona, USA*
Latitude: 33.5777°, Longitude: -111.9277°
Elevation: 1341.56 ft**
* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

INTENSITY

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)[1]

Duration	Average recurrence interval (years)									
	1	**2**	**5**	**10**	**25**	**50**	**100**	**200**	**500**	**1000**
5-min	**2.23** (1.86-2.72)	**2.92** (2.44-3.56)	**3.94** (3.26-4.80)	**4.73** (3.90-5.75)	**5.80** (4.70-7.02)	**6.61** (5.32-7.97)	**7.48** (5.89-8.98)	**8.33** (6.44-9.98)	**9.48** (7.15-11.4)	**10.4** (7.66-12.5)
10-min	**1.70** (1.41-2.08)	**2.21** (1.85-2.71)	**2.99** (2.48-3.65)	**3.60** (2.97-4.37)	**4.42** (3.58-5.35)	**5.04** (4.04-6.07)	**5.69** (4.48-6.83)	**6.34** (4.91-7.60)	**7.22** (5.45-8.66)	**7.88** (5.83-9.47)
15-min	**1.40** (1.17-1.72)	**1.83** (1.53-2.24)	**2.48** (2.06-3.02)	**2.97** (2.46-3.62)	**3.65** (2.96-4.42)	**4.16** (3.34-5.01)	**4.70** (3.70-5.64)	**5.24** (4.06-6.28)	**5.96** (4.50-7.16)	**6.51** (4.82-7.83)
30-min	**0.946** (0.786-1.16)	**1.23** (1.03-1.51)	**1.67** (1.38-2.03)	**2.00** (1.65-2.44)	**2.46** (1.99-2.98)	**2.80** (2.25-3.37)	**3.17** (2.49-3.80)	**3.53** (2.73-4.23)	**4.02** (3.03-4.82)	**4.39** (3.24-5.27)
60-min	**0.585** (0.486-0.715)	**0.763** (0.639-0.934)	**1.03** (0.856-1.26)	**1.24** (1.02-1.51)	**1.52** (1.23-1.84)	**1.74** (1.39-2.09)	**1.96** (1.54-2.35)	**2.18** (1.69-2.62)	**2.49** (1.88-2.98)	**2.71** (2.01-3.26)
2-hr	**0.342** (0.288-0.408)	**0.442** (0.374-0.530)	**0.590** (0.496-0.702)	**0.702** (0.586-0.836)	**0.858** (0.708-1.01)	**0.974** (0.793-1.15)	**1.10** (0.876-1.29)	**1.22** (0.958-1.43)	**1.39** (1.06-1.63)	**1.51** (1.14-1.79)
3-hr	**0.254** (0.214-0.310)	**0.325** (0.275-0.399)	**0.425** (0.358-0.518)	**0.505** (0.421-0.611)	**0.616** (0.506-0.741)	**0.705** (0.571-0.843)	**0.797** (0.634-0.953)	**0.894** (0.700-1.07)	**1.03** (0.779-1.23)	**1.13** (0.840-1.36)
6-hr	**0.153** (0.132-0.182)	**0.194** (0.166-0.230)	**0.247** (0.212-0.292)	**0.291** (0.246-0.341)	**0.349** (0.292-0.408)	**0.395** (0.325-0.460)	**0.442** (0.359-0.514)	**0.491** (0.391-0.572)	**0.557** (0.433-0.649)	**0.610** (0.462-0.711)
12-hr	**0.084** (0.073-0.099)	**0.106** (0.092-0.125)	**0.134** (0.116-0.157)	**0.156** (0.134-0.183)	**0.186** (0.157-0.217)	**0.209** (0.174-0.242)	**0.232** (0.191-0.269)	**0.256** (0.208-0.297)	**0.288** (0.228-0.335)	**0.312** (0.243-0.367)
24-hr	**0.050** (0.043-0.058)	**0.063** (0.055-0.074)	**0.081** (0.070-0.095)	**0.096** (0.083-0.112)	**0.116** (0.100-0.136)	**0.132** (0.112-0.154)	**0.148** (0.125-0.173)	**0.165** (0.139-0.193)	**0.189** (0.156-0.221)	**0.208** (0.170-0.243)
2-day	**0.027** (0.023-0.031)	**0.034** (0.030-0.040)	**0.044** (0.039-0.052)	**0.053** (0.046-0.061)	**0.064** (0.055-0.075)	**0.074** (0.063-0.085)	**0.083** (0.070-0.097)	**0.093** (0.078-0.109)	**0.107** (0.089-0.125)	**0.119** (0.097-0.139)
3-day	**0.019** (0.017-0.022)	**0.024** (0.021-0.028)	**0.032** (0.028-0.037)	**0.038** (0.033-0.044)	**0.047** (0.040-0.054)	**0.053** (0.046-0.062)	**0.061** (0.052-0.070)	**0.068** (0.058-0.079)	**0.079** (0.066-0.092)	**0.088** (0.072-0.102)
4-day	**0.015** (0.013-0.017)	**0.019** (0.017-0.022)	**0.025** (0.022-0.029)	**0.030** (0.026-0.035)	**0.038** (0.032-0.043)	**0.043** (0.037-0.050)	**0.049** (0.042-0.057)	**0.056** (0.047-0.064)	**0.065** (0.054-0.075)	**0.072** (0.060-0.084)
7-day	**0.010** (0.008-0.011)	**0.012** (0.011-0.014)	**0.016** (0.014-0.019)	**0.020** (0.017-0.023)	**0.024** (0.021-0.028)	**0.028** (0.024-0.032)	**0.032** (0.027-0.037)	**0.036** (0.030-0.042)	**0.042** (0.035-0.049)	**0.047** (0.038-0.054)
10-day	**0.007** (0.006-0.008)	**0.009** (0.008-0.011)	**0.012** (0.011-0.014)	**0.015** (0.013-0.017)	**0.018** (0.016-0.021)	**0.021** (0.018-0.024)	**0.024** (0.020-0.027)	**0.027** (0.023-0.031)	**0.031** (0.026-0.036)	**0.035** (0.029-0.040)
20-day	**0.005** (0.004-0.005)	**0.006** (0.005-0.007)	**0.008** (0.007-0.009)	**0.009** (0.008-0.010)	**0.011** (0.010-0.013)	**0.012** (0.011-0.014)	**0.014** (0.012-0.016)	**0.016** (0.013-0.018)	**0.018** (0.015-0.020)	**0.019** (0.016-0.022)
30-day	**0.004** (0.003-0.004)	**0.005** (0.004-0.005)	**0.006** (0.005-0.007)	**0.007** (0.006-0.008)	**0.009** (0.007-0.010)	**0.010** (0.008-0.011)	**0.011** (0.009-0.012)	**0.012** (0.010-0.014)	**0.014** (0.012-0.016)	**0.015** (0.013-0.017)
45-day	**0.003** (0.002-0.003)	**0.003** (0.003-0.004)	**0.005** (0.004-0.005)	**0.005** (0.005-0.006)	**0.007** (0.006-0.007)	**0.007** (0.006-0.008)	**0.008** (0.007-0.009)	**0.009** (0.008-0.010)	**0.010** (0.009-0.011)	**0.011** (0.009-0.012)
60-day	**0.002** (0.002-0.003)	**0.003** (0.003-0.003)	**0.004** (0.003-0.004)	**0.004** (0.004-0.005)	**0.005** (0.005-0.006)	**0.006** (0.005-0.007)	**0.007** (0.006-0.007)	**0.007** (0.006-0.008)	**0.008** (0.007-0.009)	**0.009** (0.007-0.010)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.
Please refer to NOAA Atlas 14 document for more information.

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PF graphical

Appendix D – DETAILED DRAINAGE CALCULATIONS

DIBBLE

Gold Dust Apartments

DIBBLE PROJECT NO. 1122028

ON-SITE DRAINAGE CALCULATIONS

DES: KR

DATE: 2023-0417

HYDROLOGY CALCULATIONS

DRAINAGE AREA	TOTAL AREA [SF]	WEIGHTED COEFFICIENT	*RAINFALL DEPTH [IN]	VOLUME REQUIRED [CF]
A	156,124	1.00	0.50	6,505
TOTAL	**156,124**			**6,505**

UNDERGROUND RETENTION

REQUIRED VOLUME [CF]	MODULE HEIGHT [FT]	MODULES REQUIRED	MODULES PROVIDED	TOTAL VOLUME [CF]	NUMBER OF DRYWELLS	DRAIN TIME [HR]
6,505.2	14.0	4.4	5.0	7,350		
TOTAL				**7,350**	**1**	**18.1**
**Assumed Drywell Percolation Rate [CFS]:				0.10		

$$No.\ of\ Drywells\ Required = \frac{Volume\ Required\ [CF]}{Percolation\ Rate\ [CFS]} * \frac{1\ hour}{3600\ seconds} * \frac{1}{Allowable\ Drain\ Time\ [hrs]}$$

STORMCAPTURE SPECS

MIN HEIGHT [FT]	MAX HEIGHT [FT]	MODULE WIDTH [FT]	MODULE LENGTH [FT]	MAX MODULE VOLUME [CF]
2.0	14.0	7.0	15.0	1,470

DIBBLE

Gold Dust Apartments

DIBBLE PROJECT NO. 1122028

PRE VS POST DEVELOPEMNT VOLUME CALCULATIONS

DES: KR

DATE: 2023-0417

HYDROLOGY CALCULATIONS

DRAINAGE AREA	TOTAL AREA [SF]	GRAVELED SURFACES [SF] 0.45	PAVEMENT & BUILDING [SF] 0.95	WEIGHTED COEFFICIENT	*RAINFALL DEPTH [IN]	VOLUME REQUIRED [CF]
PRE CONSTRUCTION 100-YEAR, 2-HOUR						
A	156,124	14,845	141,279	0.90	2.19	25,713
					TOTAL	**25,713**
POST CONSTRUCTION 100-YEAR, 2-HOUR						
A	156,124	24,148	131,976	0.87	2.19	24,865
					TOTAL	**24,865**
					PRE VS POST	-849

Appendix E – UNNAMED CHANNEL FIELD PHOTOGRAPHS

Photo 1: Looking North, upstream of Unnamed Wash. The channels material is shotcrete. Trash and other debris can be seen in the main channel.

Photo 2: Looking South, downstream of Unnamed Wash. Three box culverts are used to convey runoff under Gold Dust Avenue. Note the debris and sediment in each of the culverts.

Photo 3: Looking North, upstream of Unnamed Wash. Outlet side of the culverts passing under Gold Dust Avenue.

Photo 4: Looking South, downstream of Unnamed Wash. The channel banks are lined with grass and thick vegetation. The main flow channel has large rock and little vegetation growth.

Photo 5: Looking South, downstream of Unnamed Wash. Four box culverts convey runoff under a private driveway. Note the sediment and debris collected in the culverts.

Photo 6: Looking South, downstream of Unnamed Wash. Outlet side of the four box culverts.

Photo 7: Looking South, downstream of Unnamed Wash. The banks are lined with gravel and grass. The main flow channel is gravel and has some vegetation obstructions.

Photo 8: Looking North, upstream of Unnamed Wash. The vegetation gets denser. The other conditions remain the same.

Photo 9: Looking South, downstream of Unnamed Wash. Three box culverts convey runoff under Mountain View Road.

Photo 10: Looking North, upstream of Unnamed Wash. The outlet side of the three box culverts passing under Mountain View Road.

Photo 11: Looking West, downstream of outlet wash of Unnamed Wash.

Appendix F – OFF-SITE HYDROLOGY RESULTS (LARGE FORMAT)

Domain Boundary	0.08 - 0.16	1.01 - 1.50
Site Boundary	0.17 - 0.50	1.51 - 2.00
Depth (ft)	0.51 - 1.00	2.01 - 2.50
		2.51 - 3.00
		3.01 - 3.50
		3.51 - 4.00
		4.01 - 25.00

Exhibit F-1

100-yr, 6-hr Storm, FLO-2D Maximum Depth

State the study this information is from.

0 500 1,000 2,000 3,000 4,000
Feet

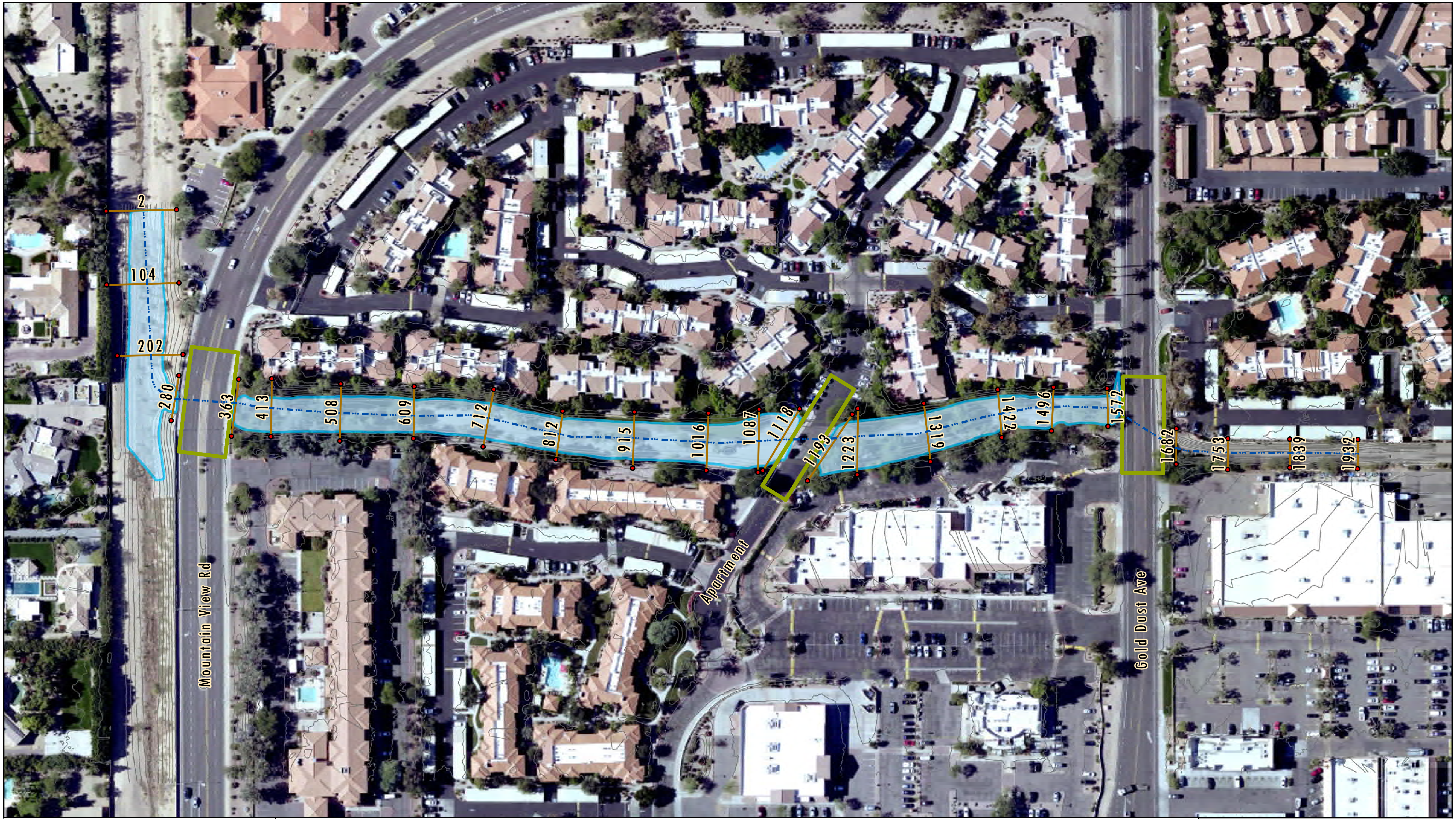
DIBBLE

Exhibit F-1:100-yr, 6-hr Storm, FLO-2D Peak Discharge

Path: G:\2022\1122028_Gold_Dust_Apartments\GIS\Exhibits\100yr 6hr Storm FLO2D Max Depth.mxd

Date Saved: 9/1/2022 6:49:04 PM

Appendix G – UNNAMED CHANNEL HYDRAULIC COMPUTATIONS



Legend

- BANK STATION
- RIVER REACH
- ROADWAY CROSSING
- CROSS SECTION W/ RIVER STATION
- 100 YR INUNDATION

GOLD DUST APARTMENTS - UNNAMED CHANNEL

Exhibit G-1

Existing Condition 100 Year Inundation

0 75 150 300
Feet

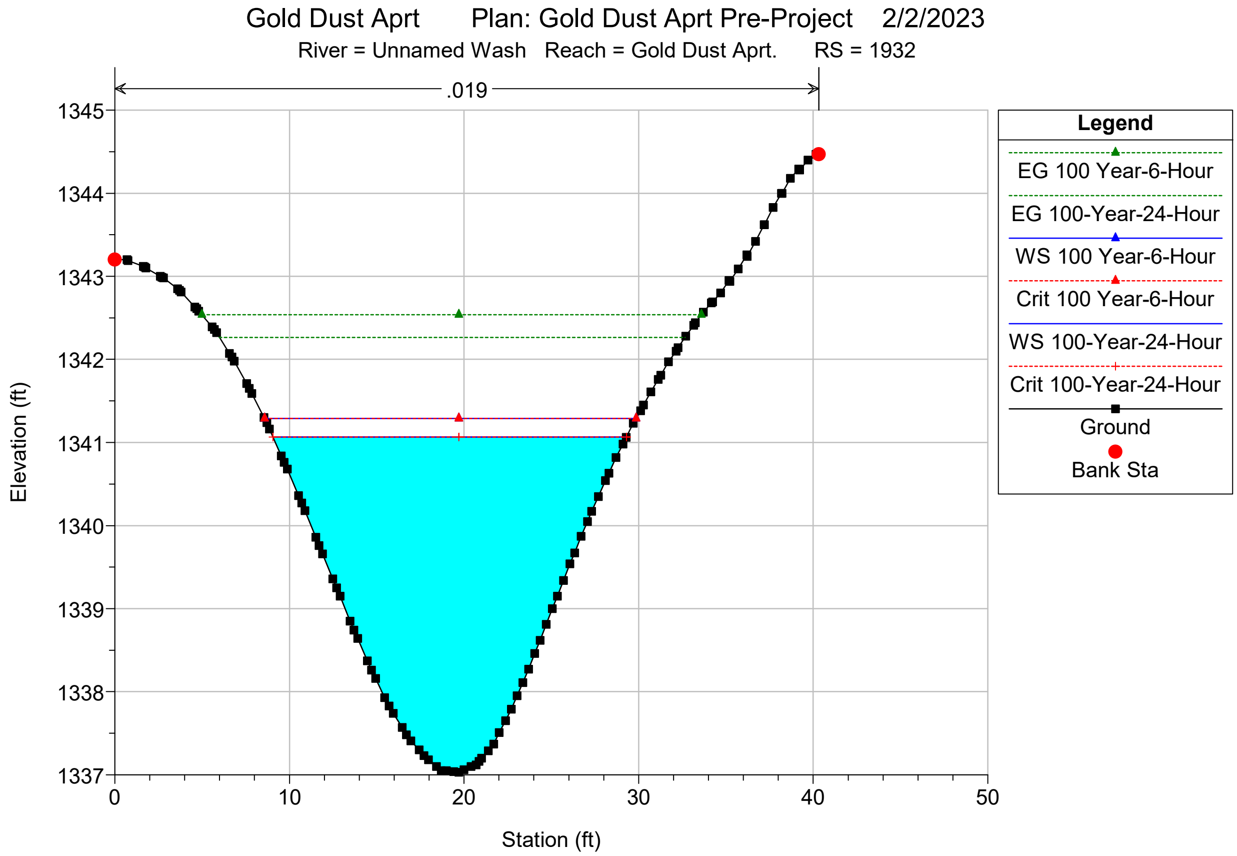
DIBBLE

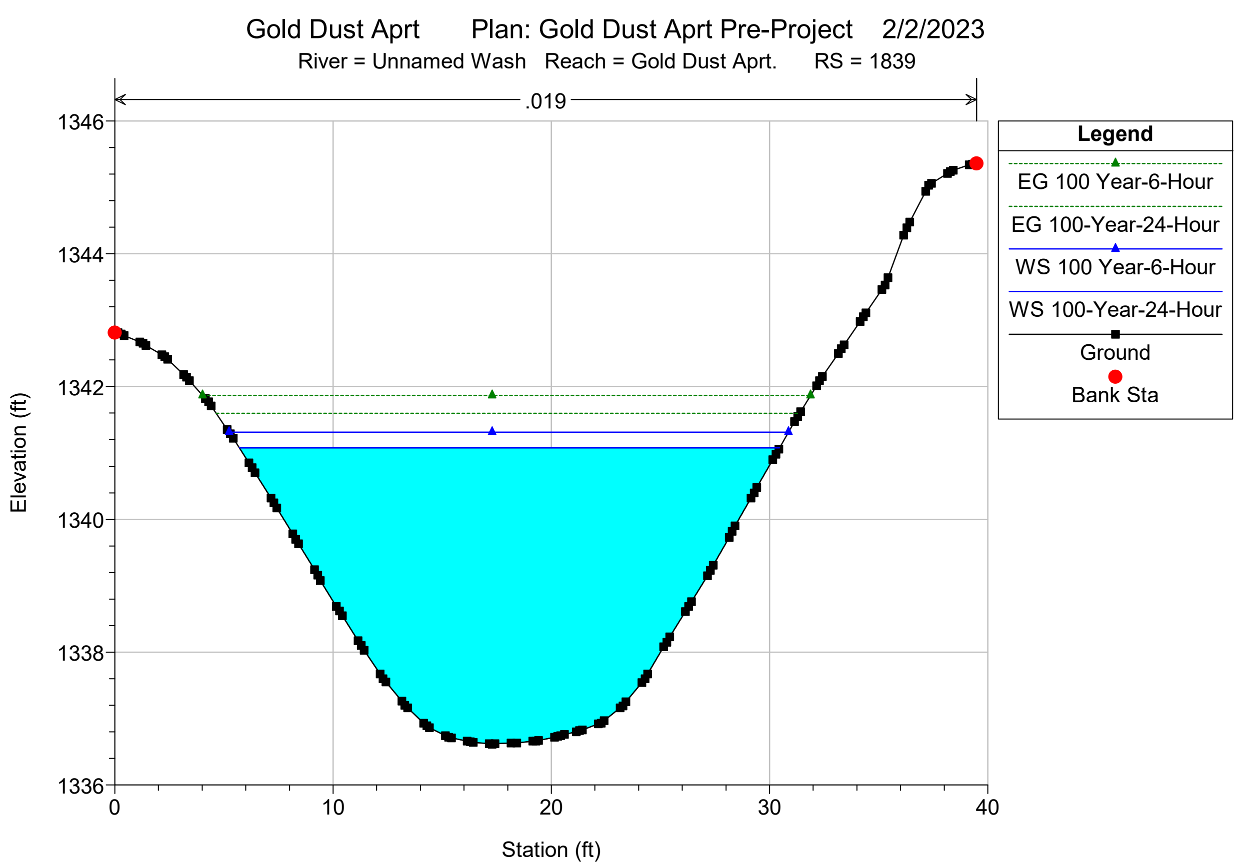
Exhibit G-1: Existing Condition 100 Year Inundation

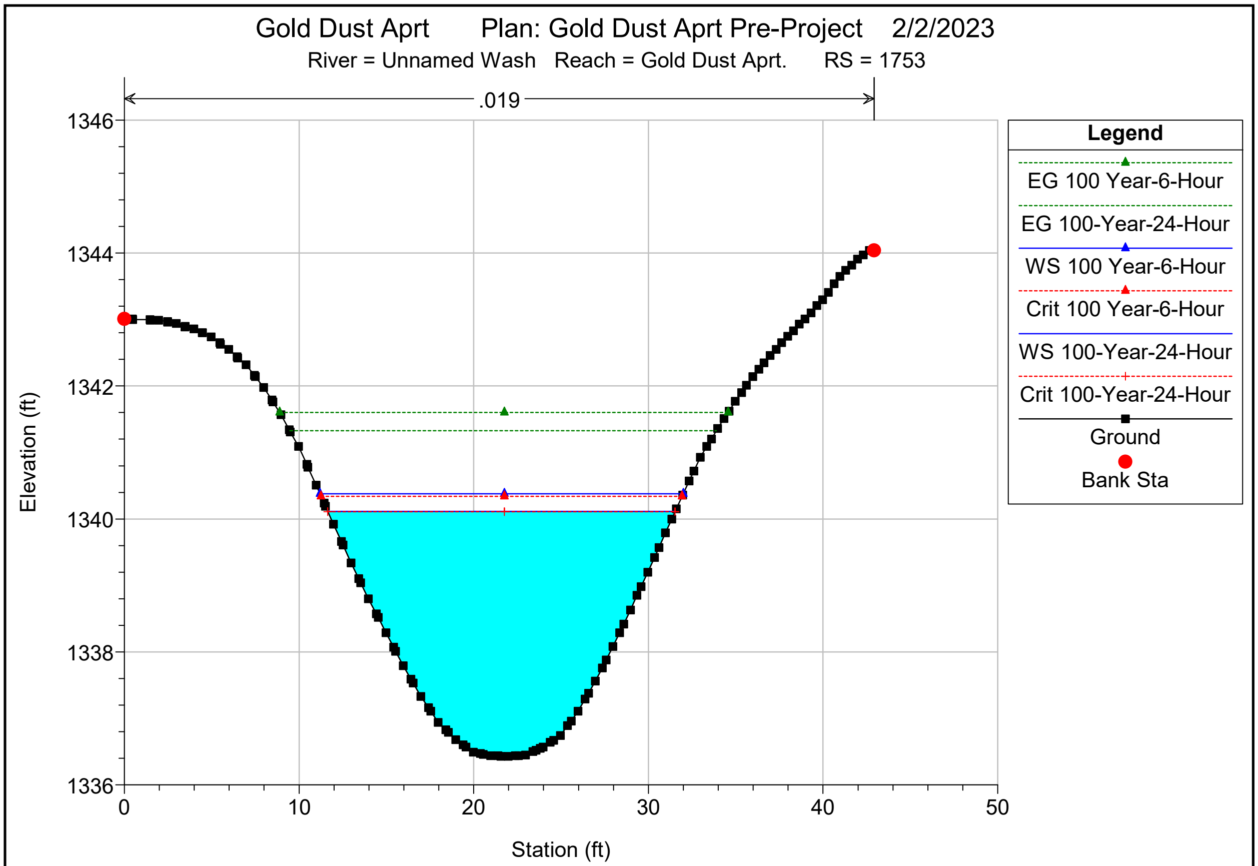
Path: G:\2022\1122028_Gold_Dust_Apartments\GIS\Exhibits\HEC-RAS.mxd

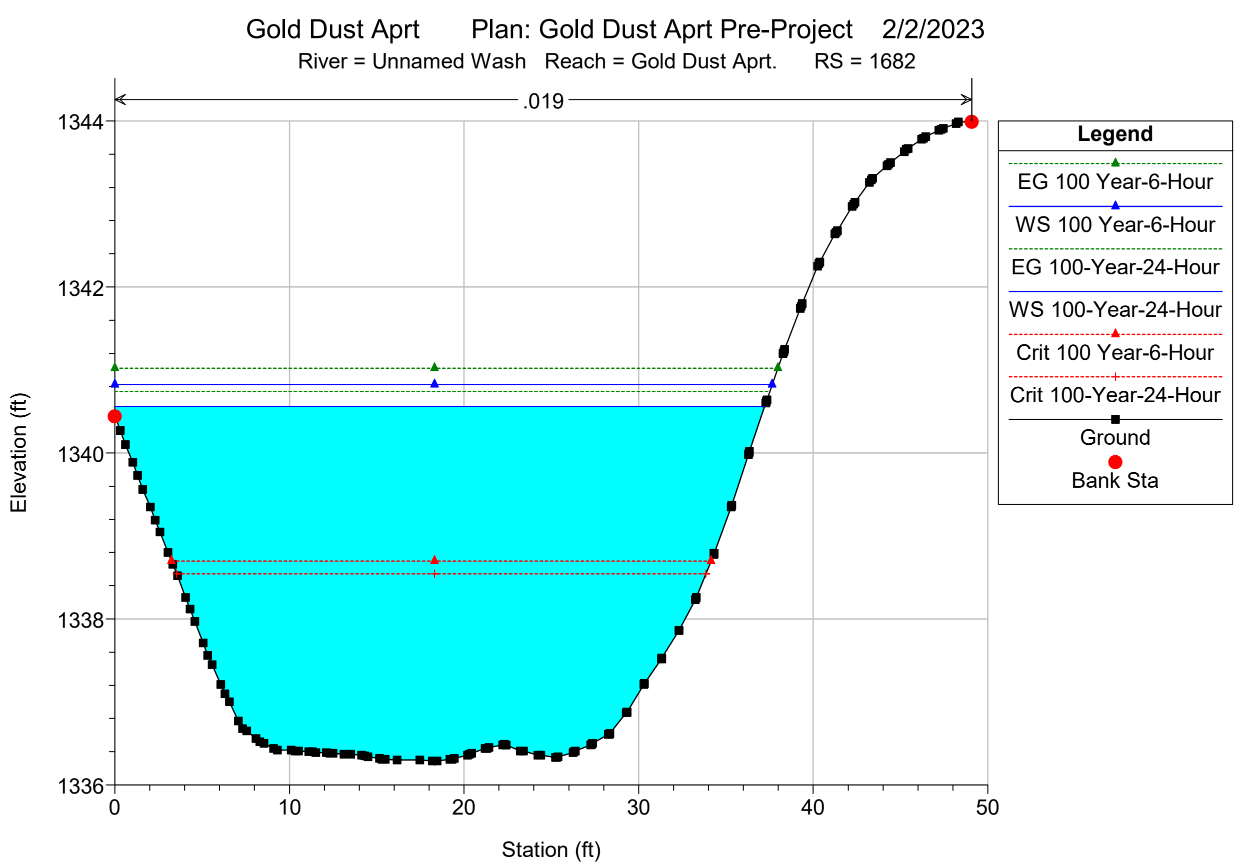
Date Saved: 2/2/2023 1:10:38 PM

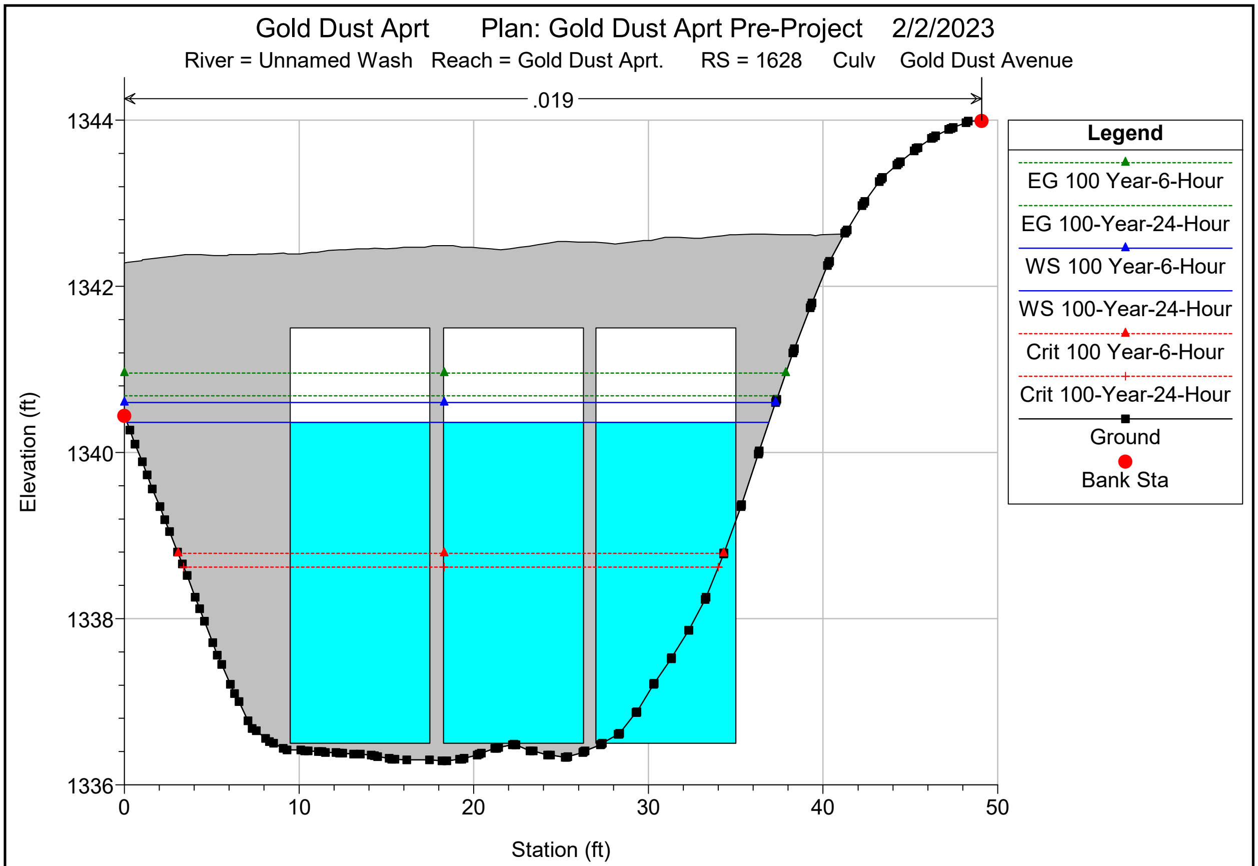
The n-value of 0.019 is not mentioned in the report. Where did it come from? Also, all bank stations are at the limits of the cross sections, please revise in final plans. (typ for all HEC-RAS)

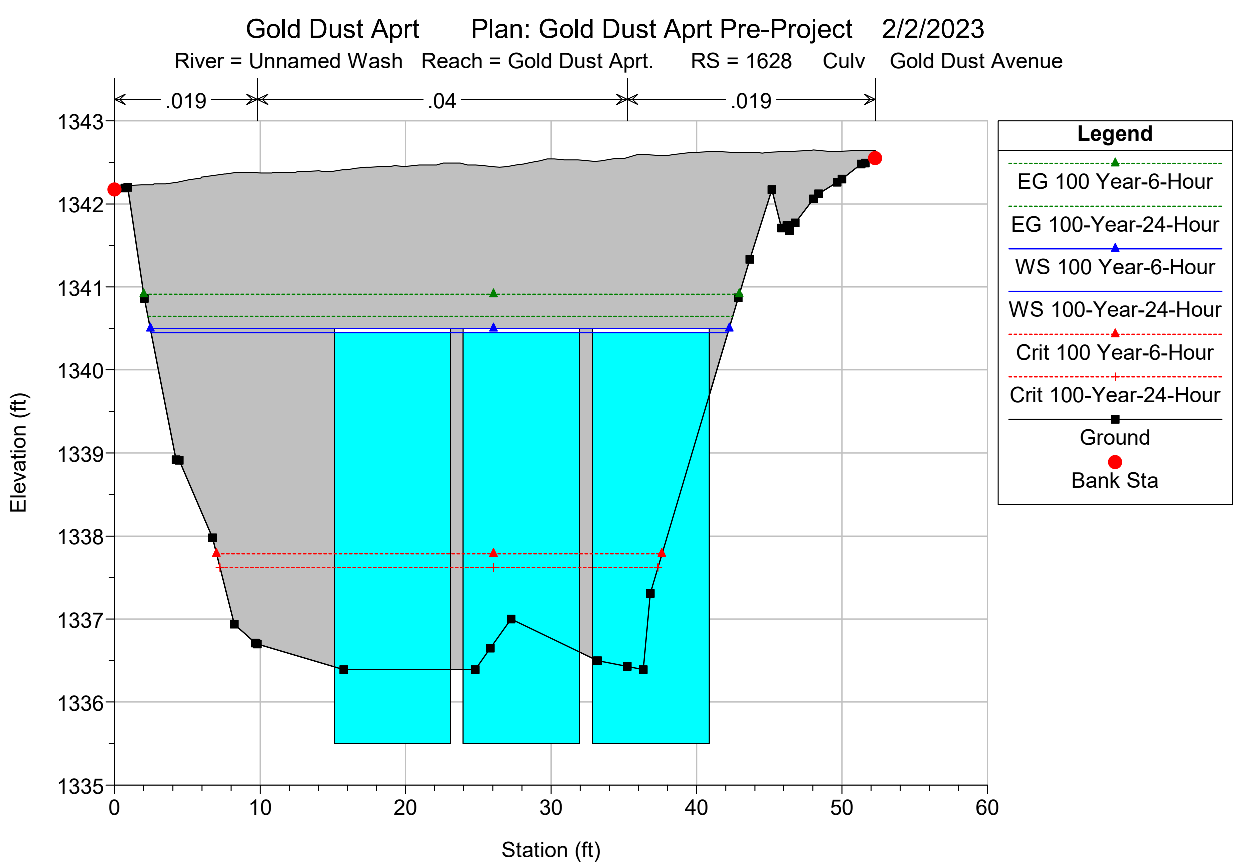


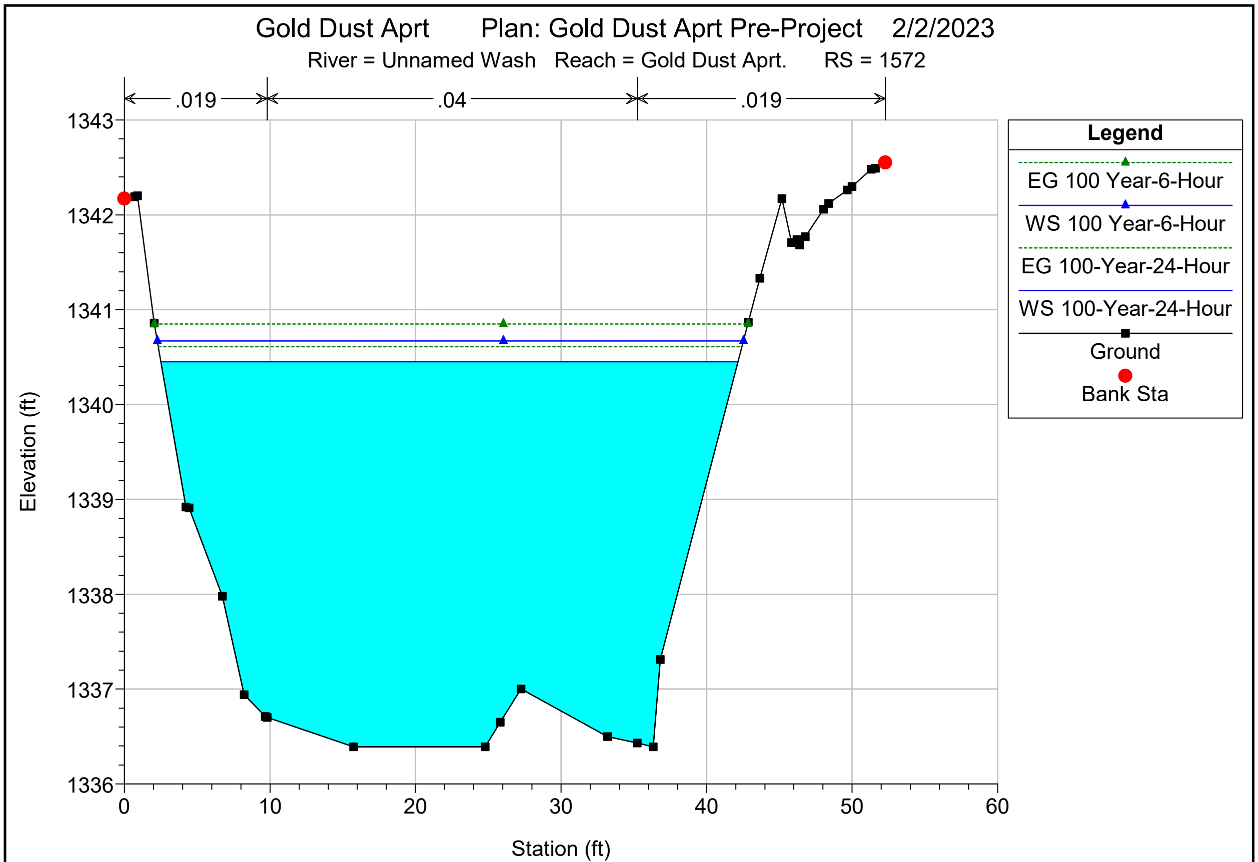


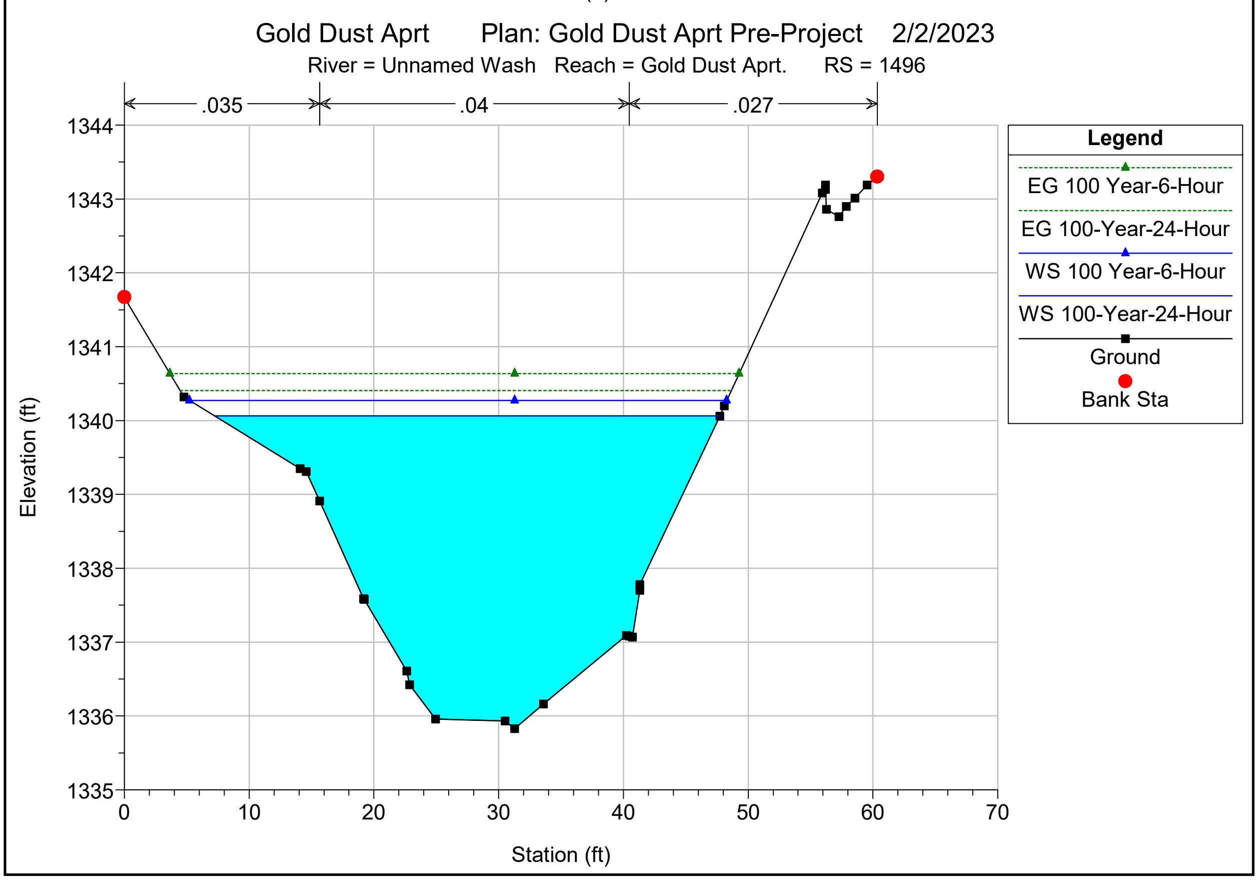

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1753
.019
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
Crit 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1346
1344
1342
1340
1338
1336
0
10
20
30
40
50
Station (ft)

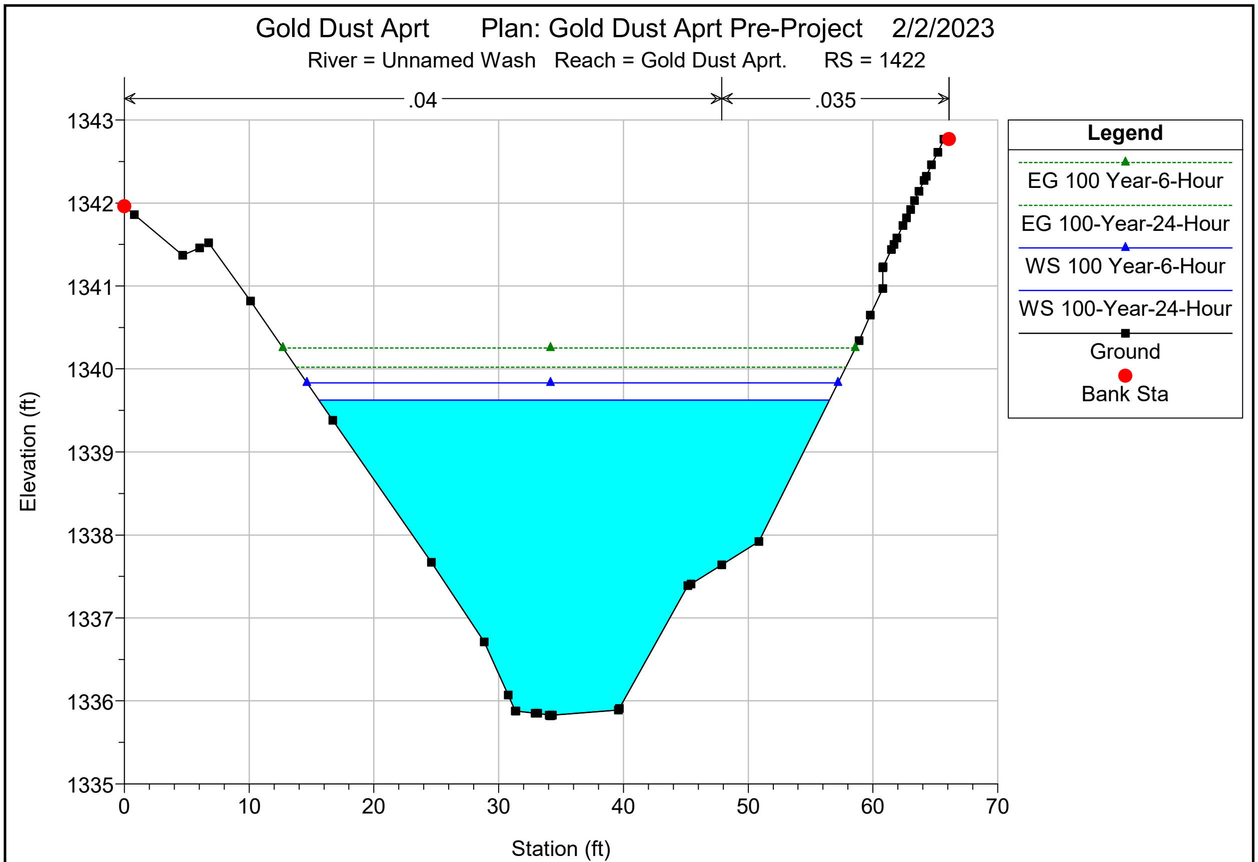

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1682
.019
Legend
EG 100 Year-6-Hour
WS 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1344
1342
1340
1338
1336
0
10
20
30
40
50
Station (ft)

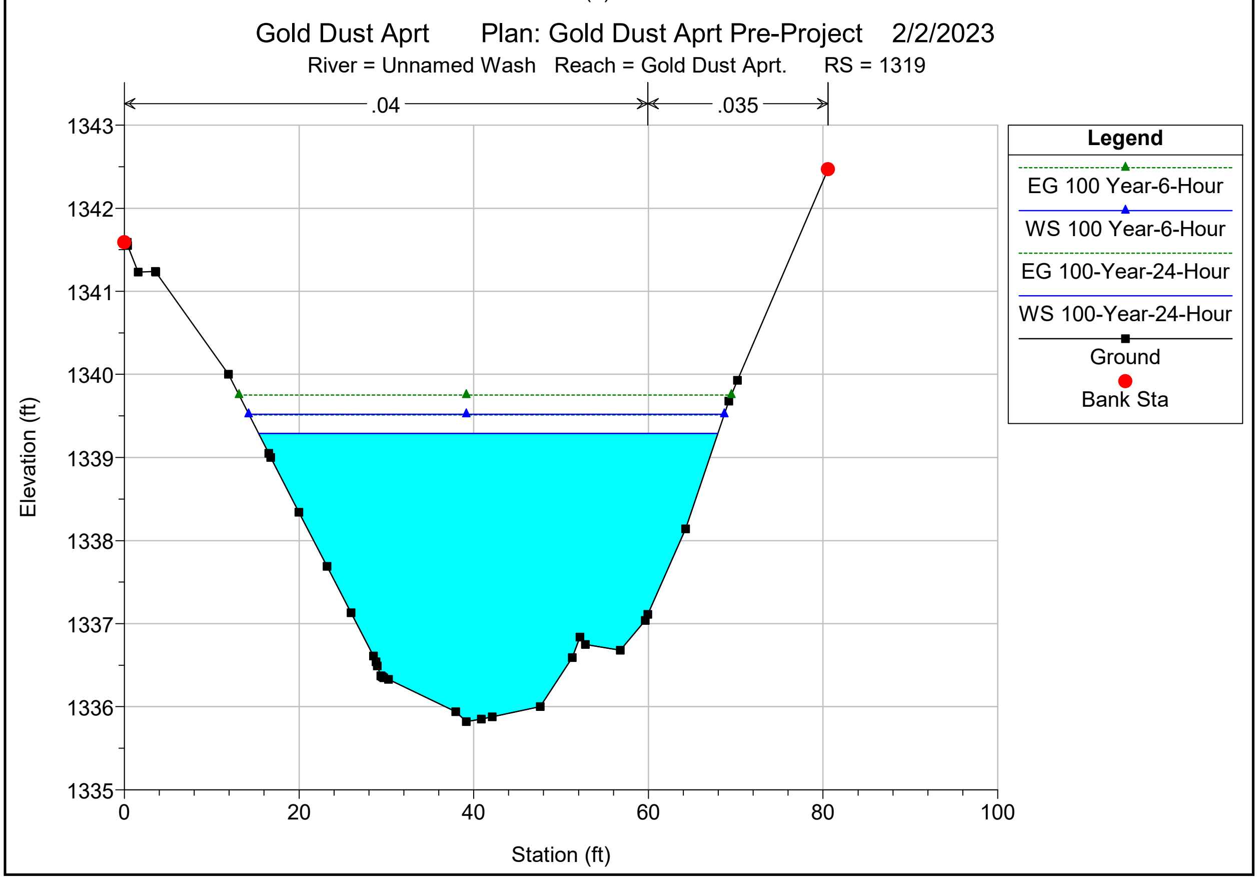

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1628 Culv Gold Dust Avenue
.019
Elevation (ft)
Station (ft)
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta

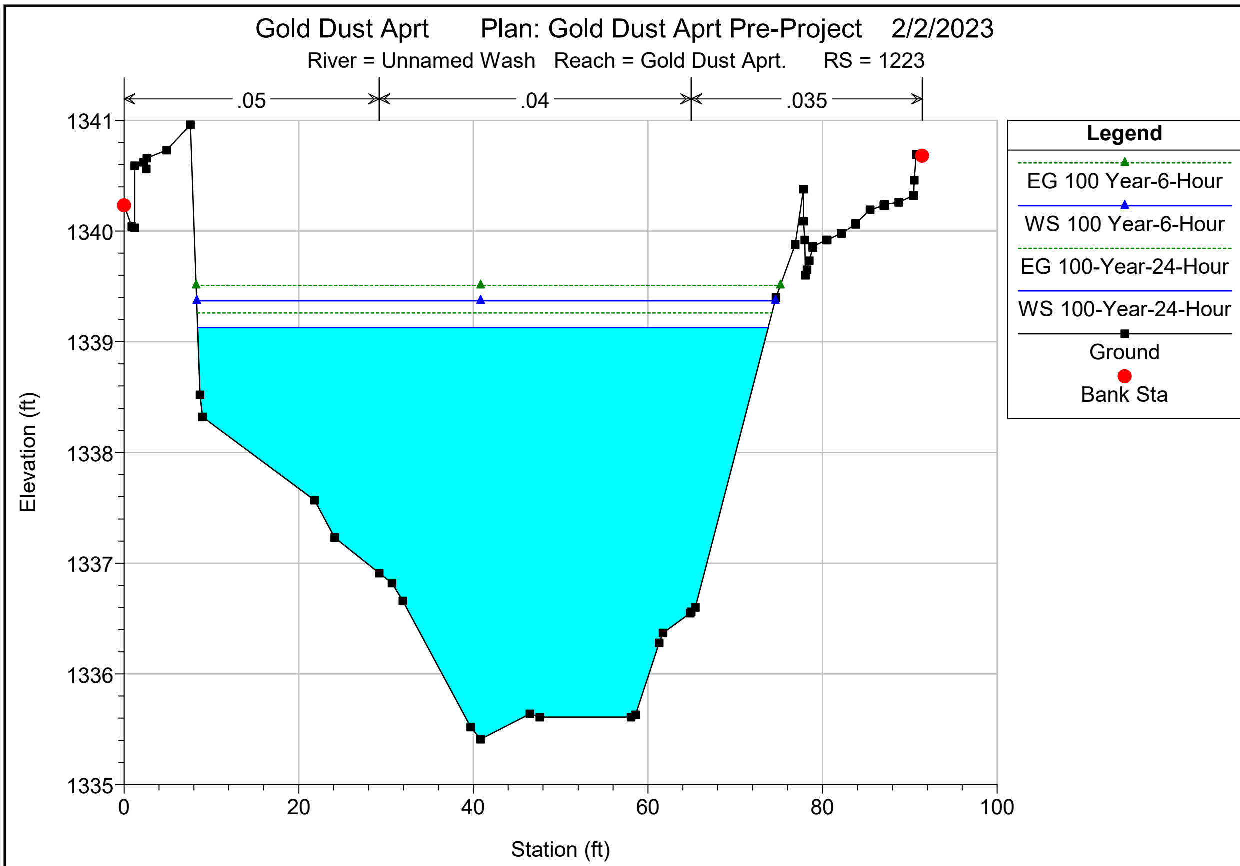

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1628 Culv Gold Dust Avenue
.019
.04
.019
Elevation (ft)
Station (ft)
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1572
.019
.04
.019
Elevation (ft)
Station (ft)
Legend
EG 100 Year-6-Hour
WS 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100-Year-24-Hour
Ground
Bank Sta

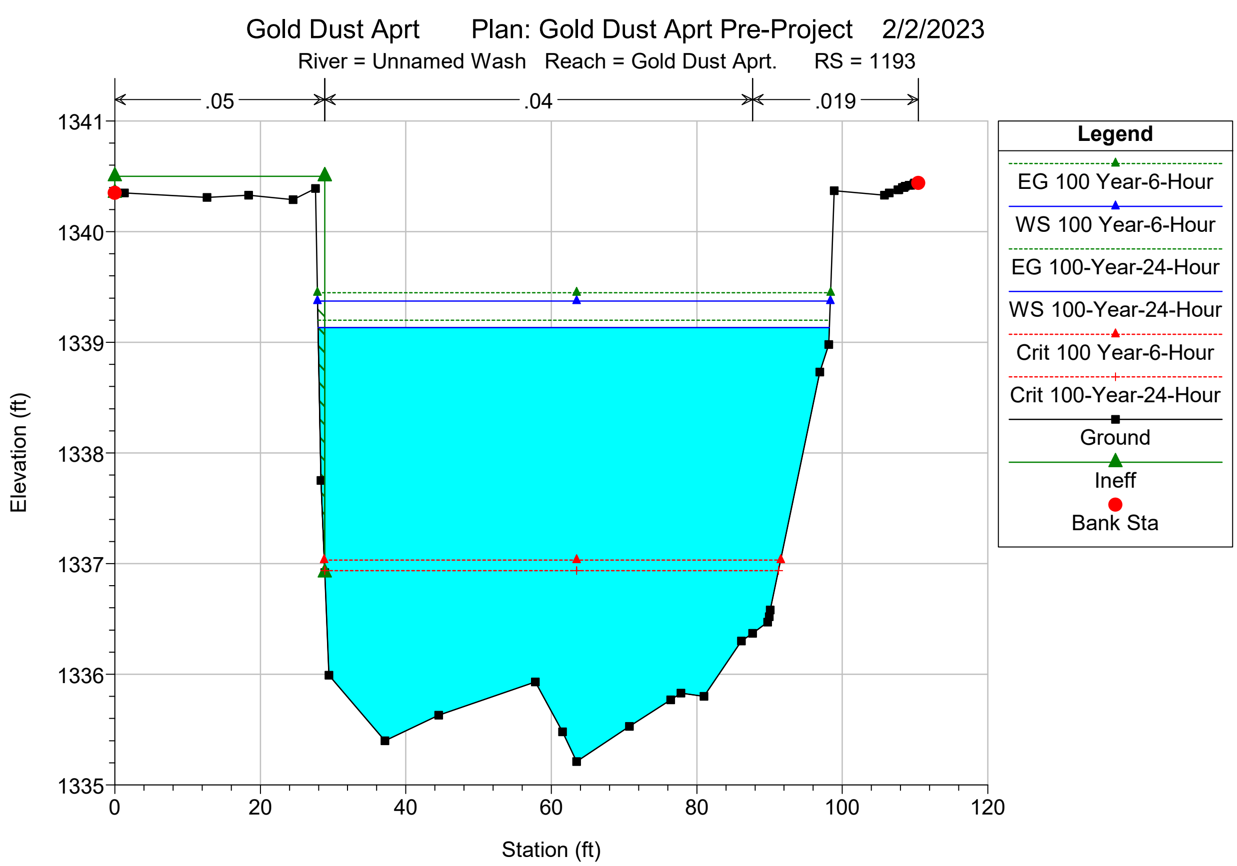

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1496
.035
.04
.027
Elevation (ft)
Station (ft)
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta


Gold Dust Aprt   Plan: Gold Dust Aprt Pre-Project   2/2/2023
River = Unnamed Wash   Reach = Gold Dust Aprt.   RS = 1422
.04
.035
Elevation (ft)
Station (ft)
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta

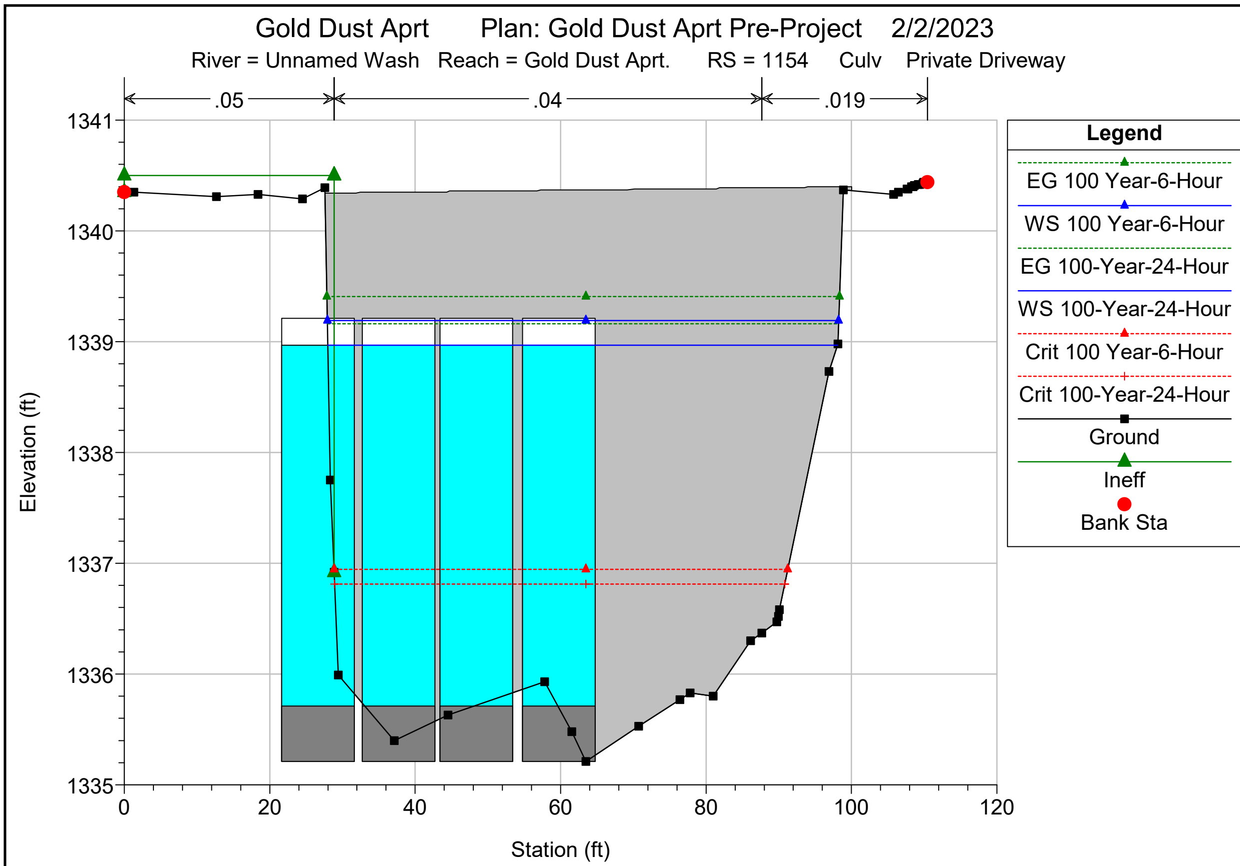

Gold Dust Aprt   Plan: Gold Dust Aprt Pre-Project   2/2/2023
River = Unnamed Wash   Reach = Gold Dust Aprt.   RS = 1319
.04
.035
Elevation (ft)
Station (ft)
Legend
EG 100 Year-6-Hour
WS 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100-Year-24-Hour
Ground
Bank Sta



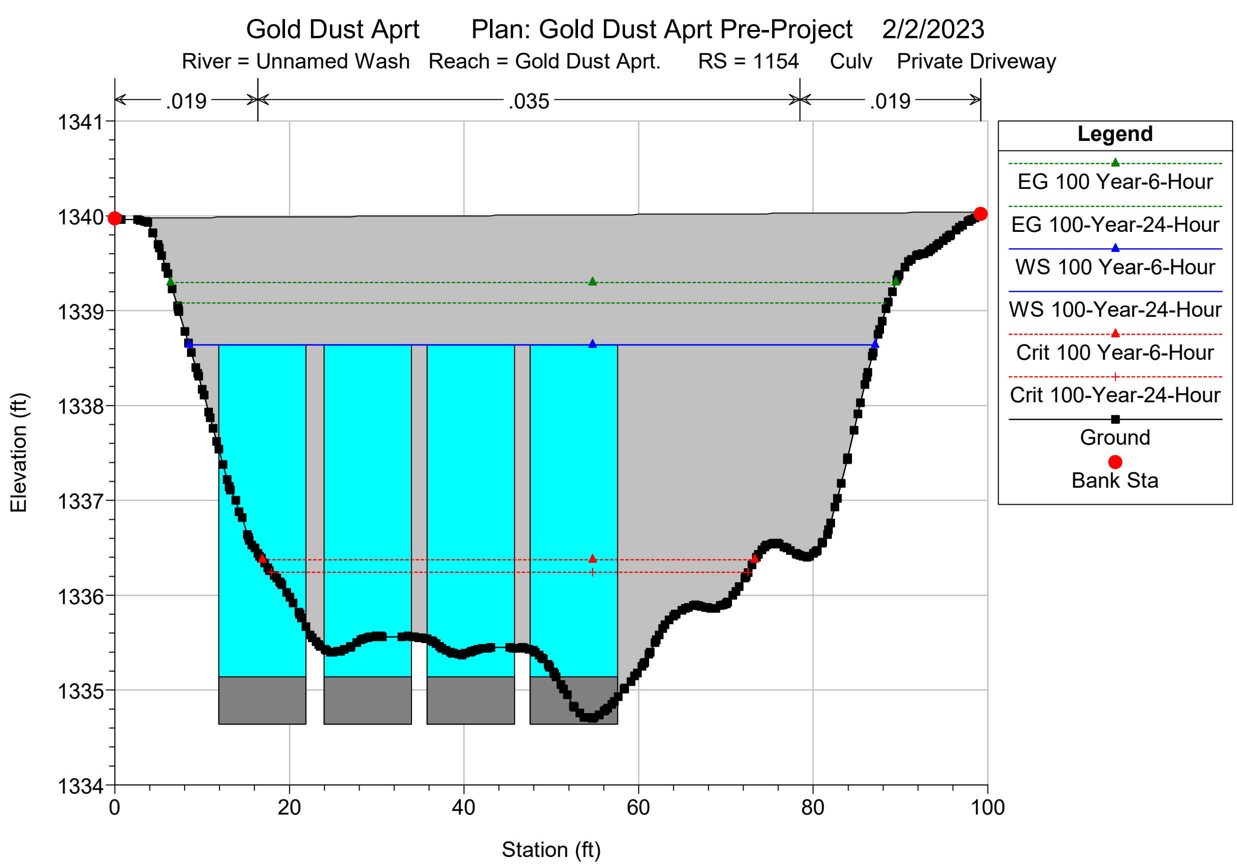
Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1223
.05
.04
.035
Elevation (ft)
Station (ft)
Legend
EG 100 Year-6-Hour
WS 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100-Year-24-Hour
Ground
Bank Sta



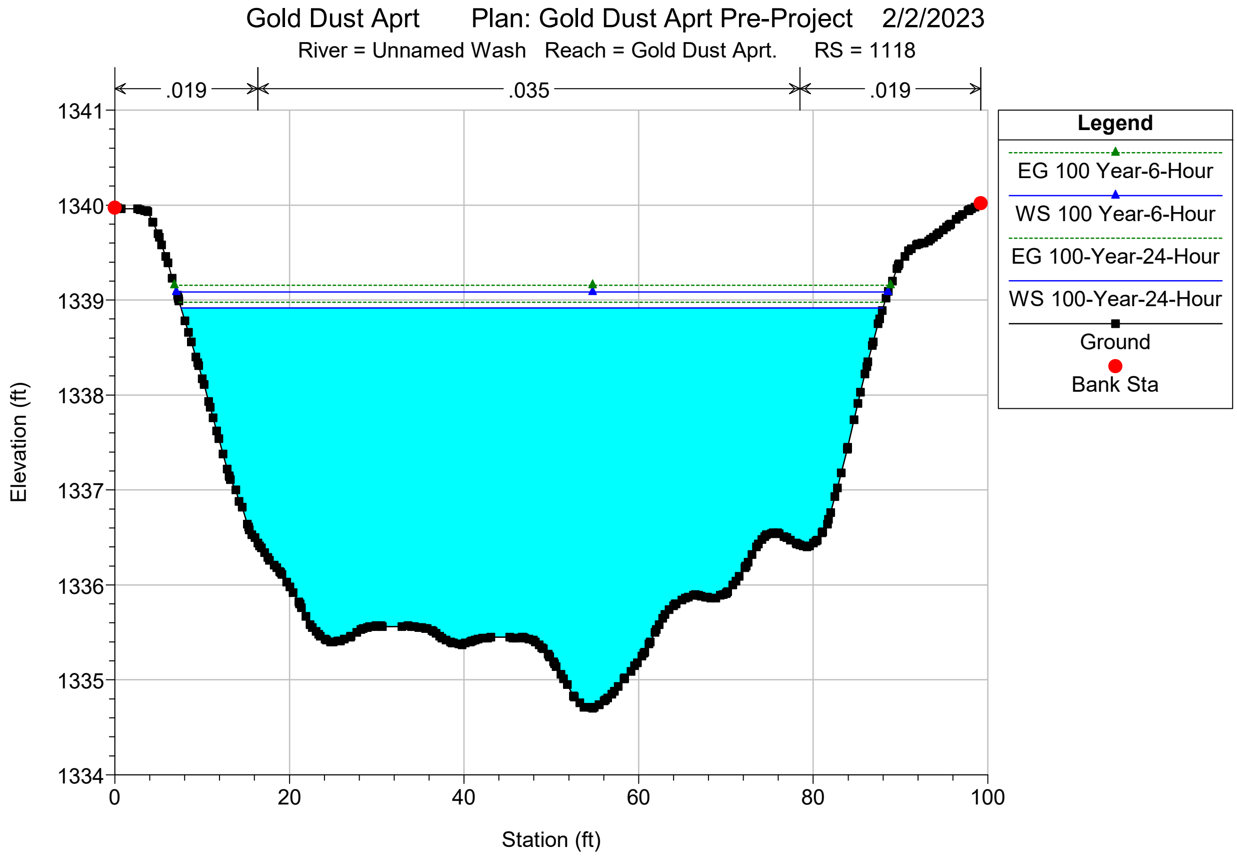
Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1193
.05
.04
.019
Elevation (ft)
Station (ft)
Legend
EG 100 Year-6-Hour
WS 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Ineff
Bank Sta



Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1154 Culv Private Driveway
.05
.04
.019
Elevation (ft)
1335
1336
1337
1338
1339
1340
1341
0
20
40
60
80
100
120
Station (ft)
Legend
EG 100 Year-6-Hour
WS 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Ineff
Bank Sta



Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1154 Culv Private Driveway
.019
.035
.019
Elevation (ft)
1334
1335
1336
1337
1338
1339
1340
1341
0
20
40
60
80
100
Station (ft)
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta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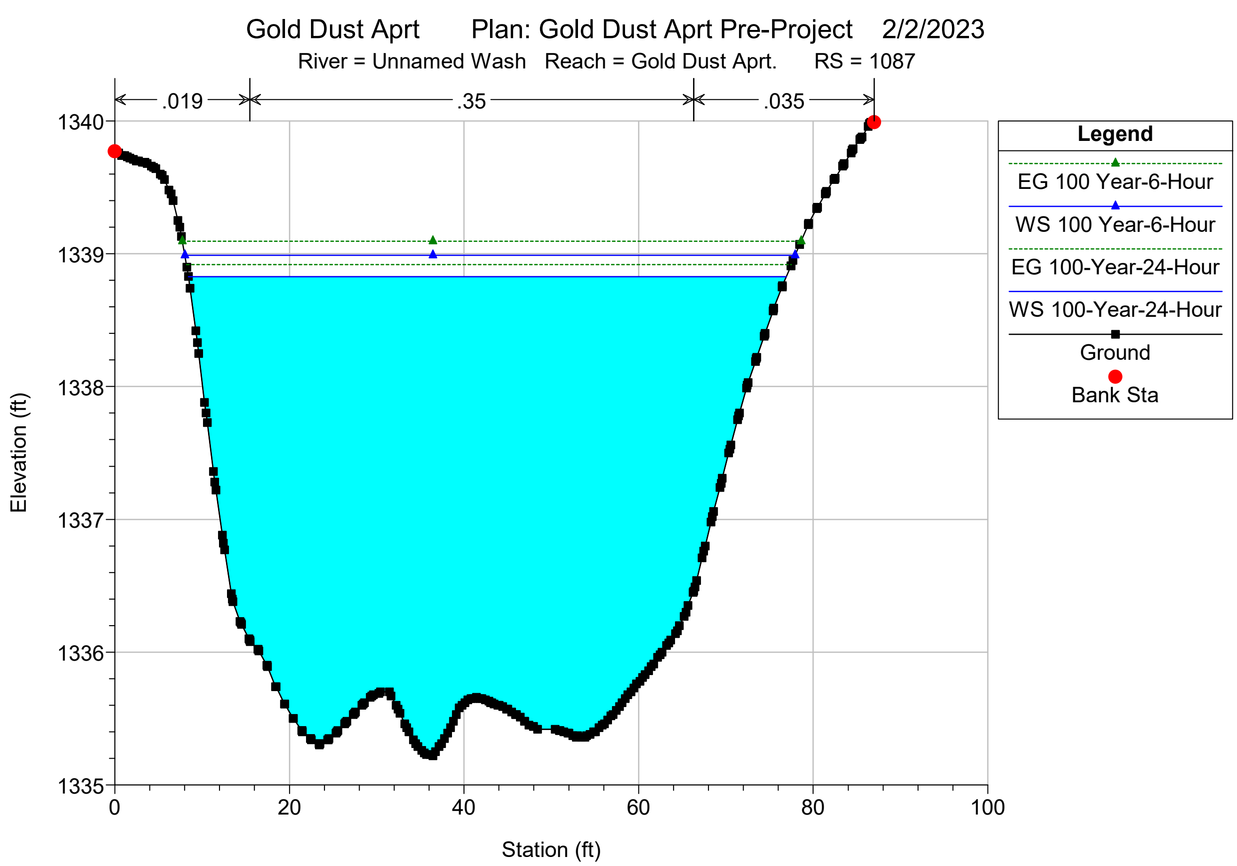
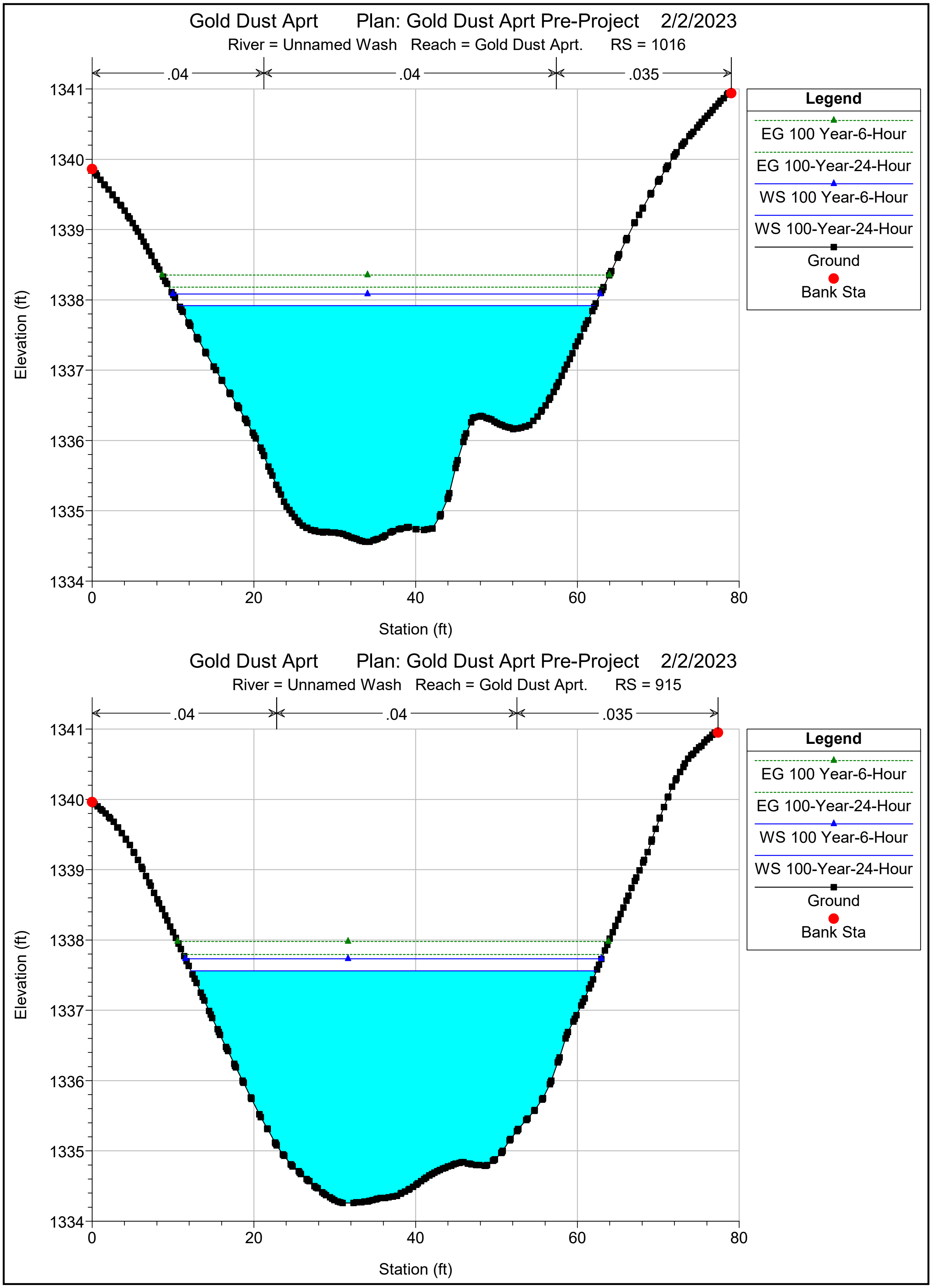
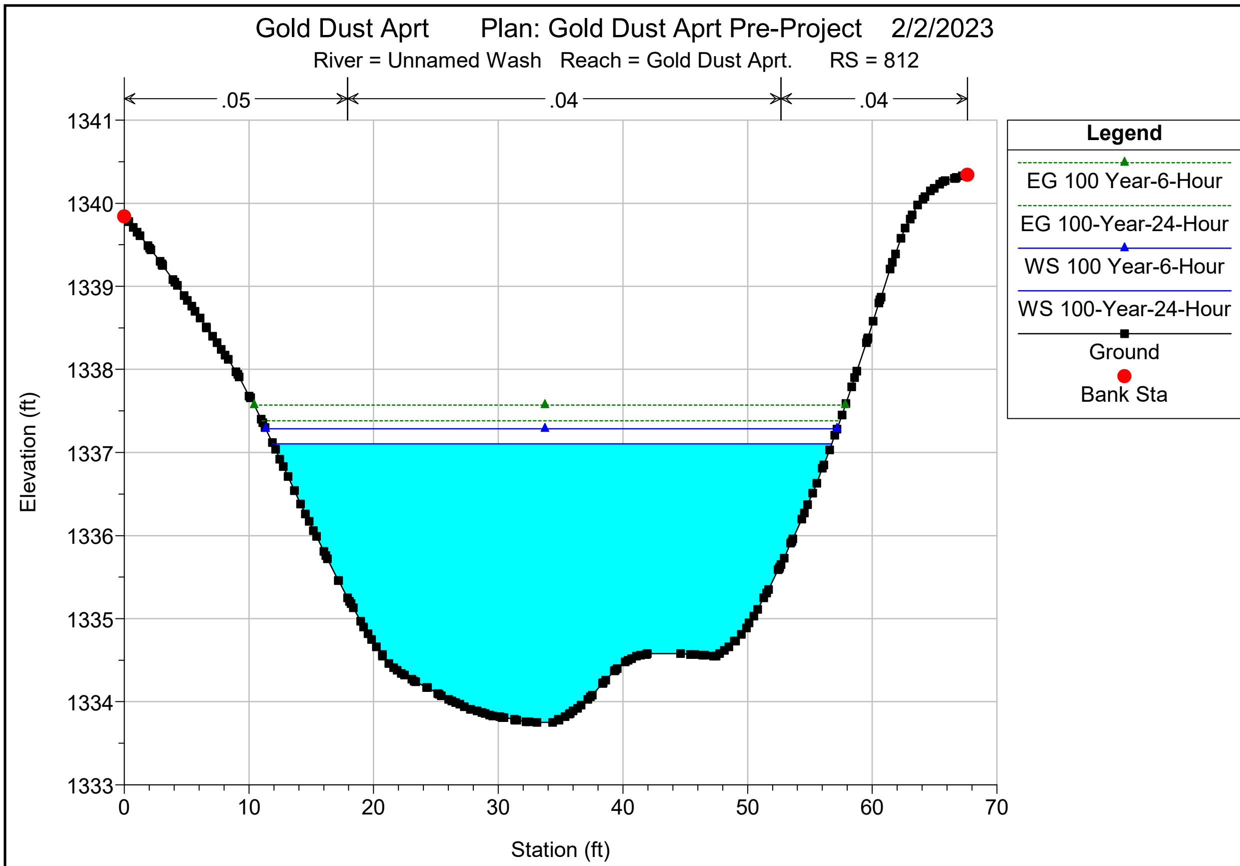
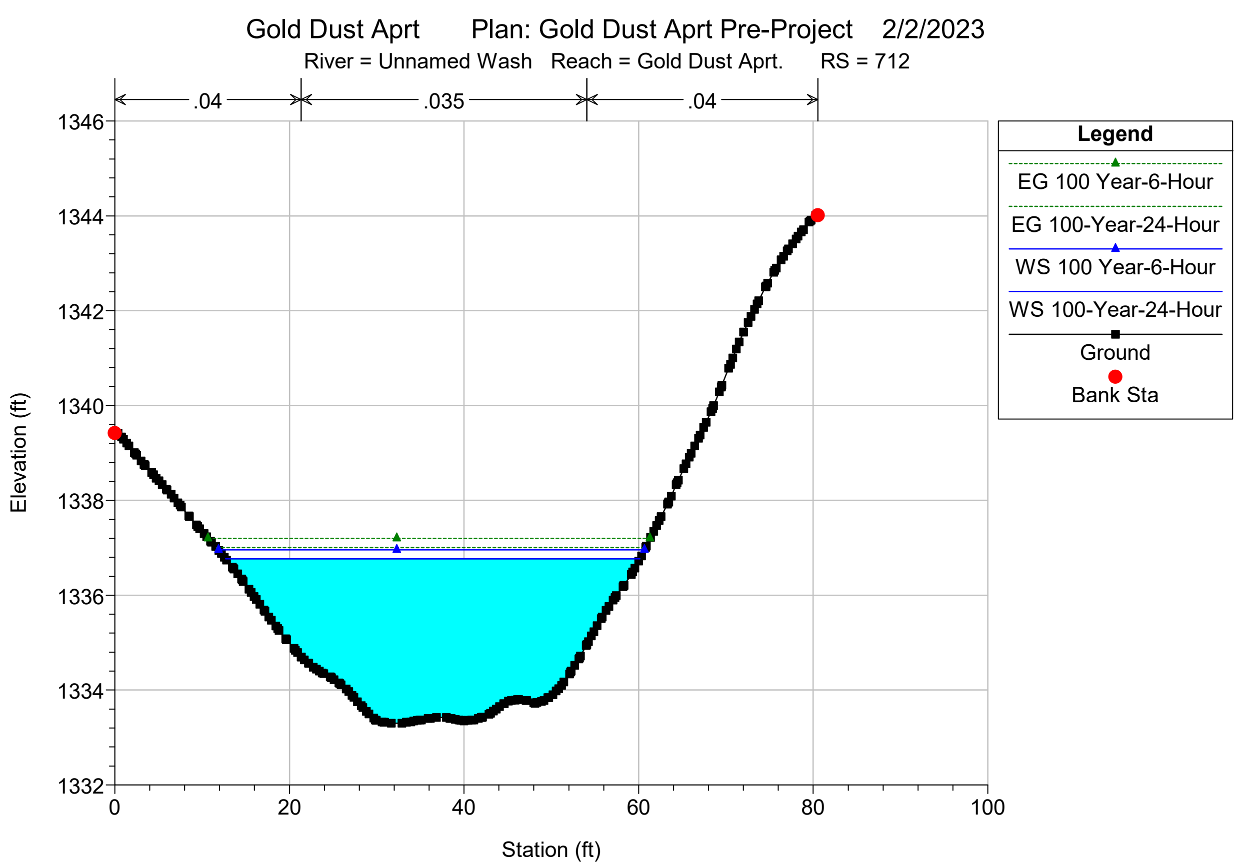
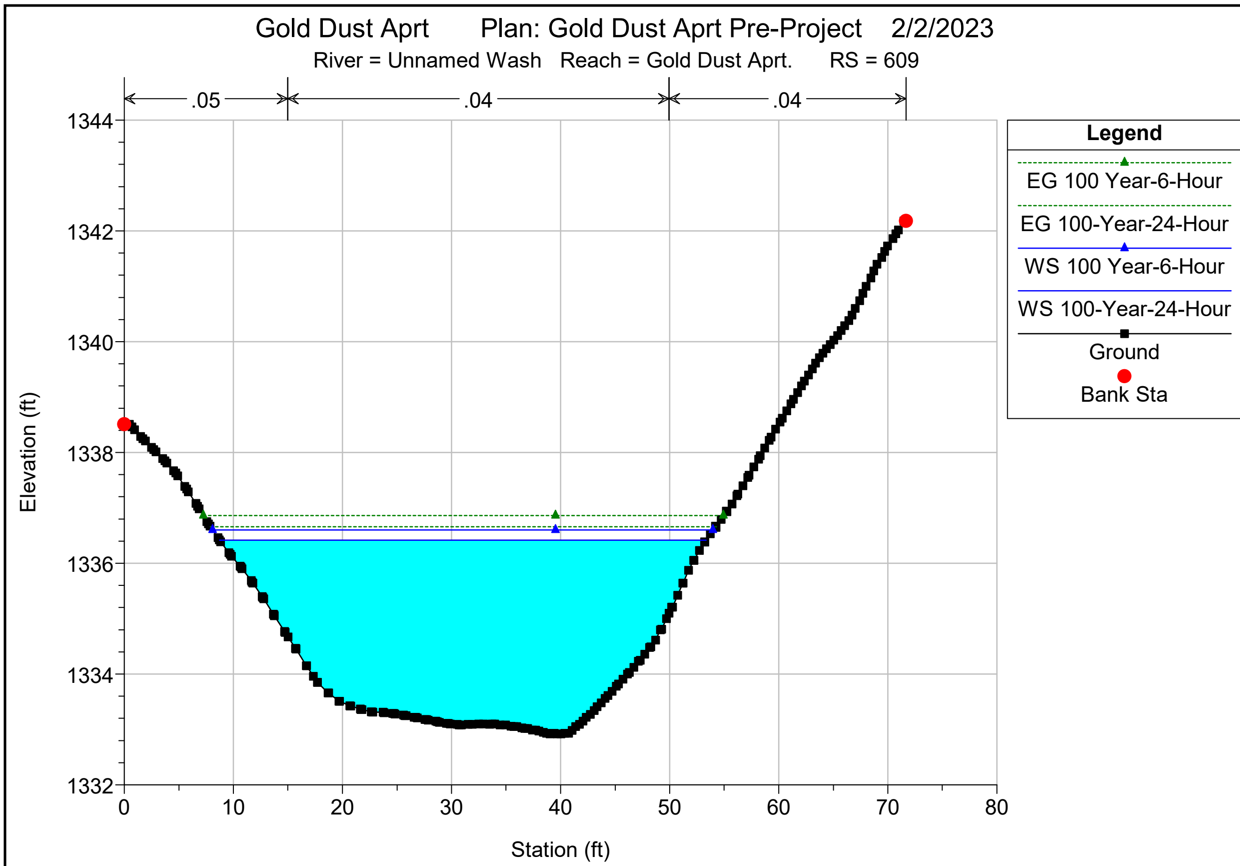
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1118
.019
.035
.019
Elevation (ft)
1341
1340
1339
1338
1337
1336
1335
1334
0
20
40
60
80
100
Station (ft)
Legend
EG 100 Year-6-Hour
WS 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100-Year-24-Hour
Ground
Bank Sta

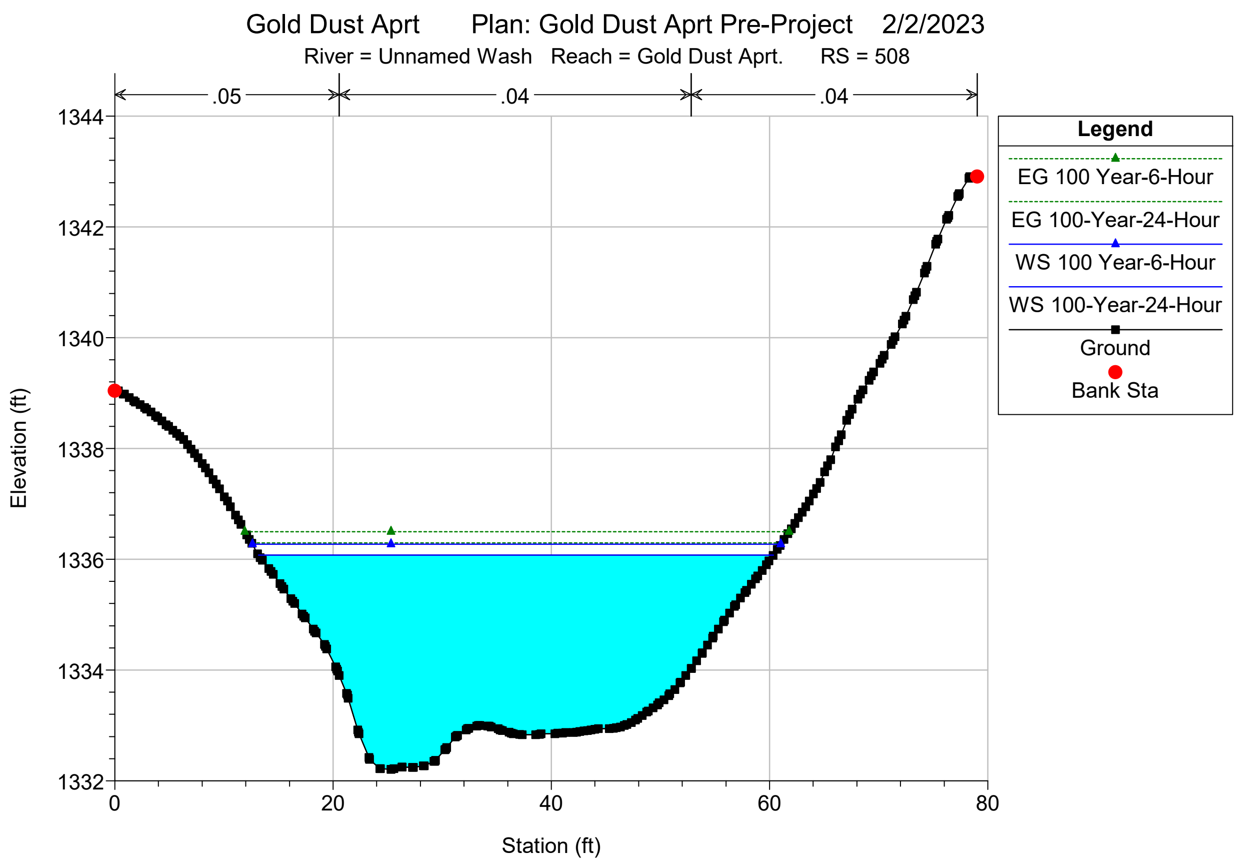

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1087
.019
.35
.035
Elevation (ft)
1340
1339
1338
1337
1336
1335
0
20
40
60
80
100
Station (ft)
Legend
EG 100 Year-6-Hour
WS 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100-Year-24-Hour
Ground
Bank Sta

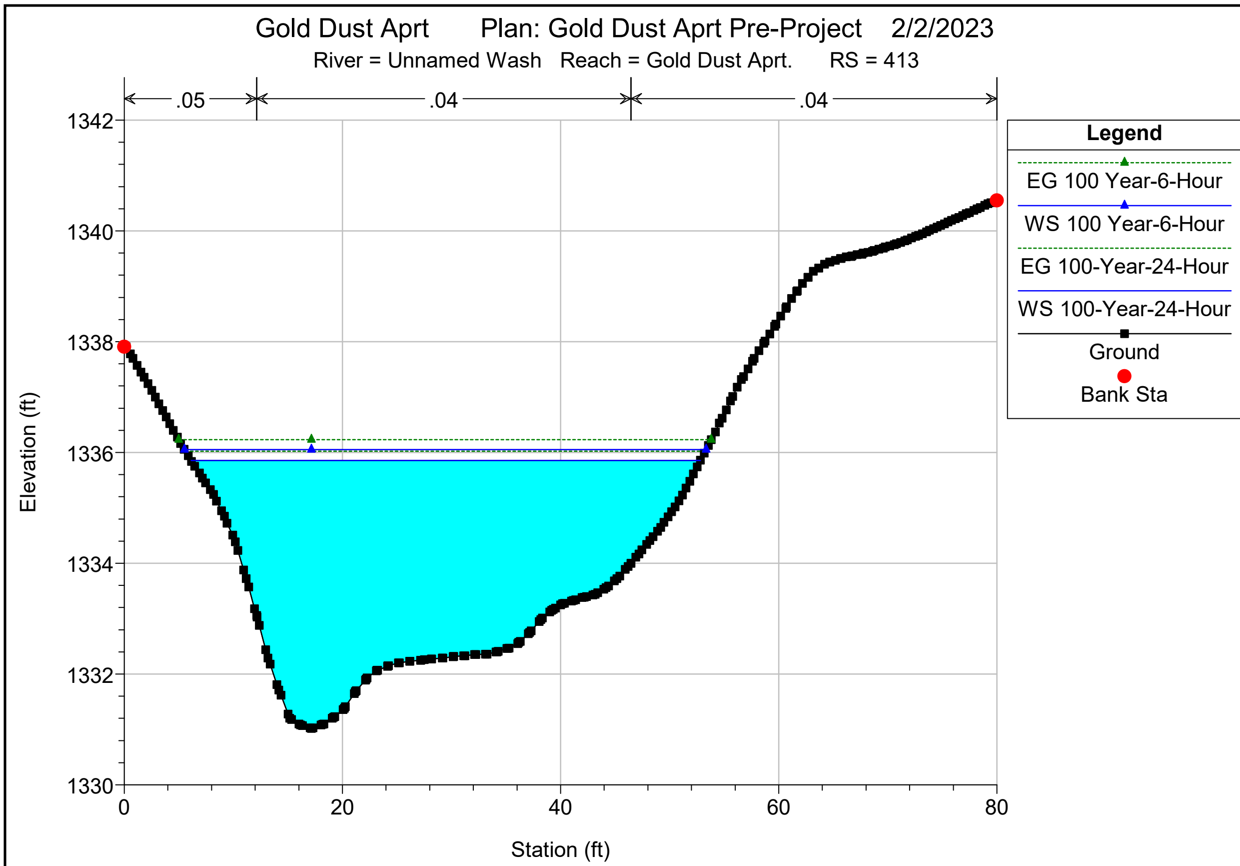

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 1016
.04
.04
.035
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1341
1340
1339
1338
1337
1336
1335
1334
0
20
40
60
80
Station (ft)
Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 915
.04
.04
.035
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1341
1340
1339
1338
1337
1336
1335
1334
0
20
40
60
80
Station (ft)


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 812
.05
.04
.04
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
Station (ft)
1341
1340
1339
1338
1337
1336
1335
1334
1333
0
10
20
30
40
50
60
70

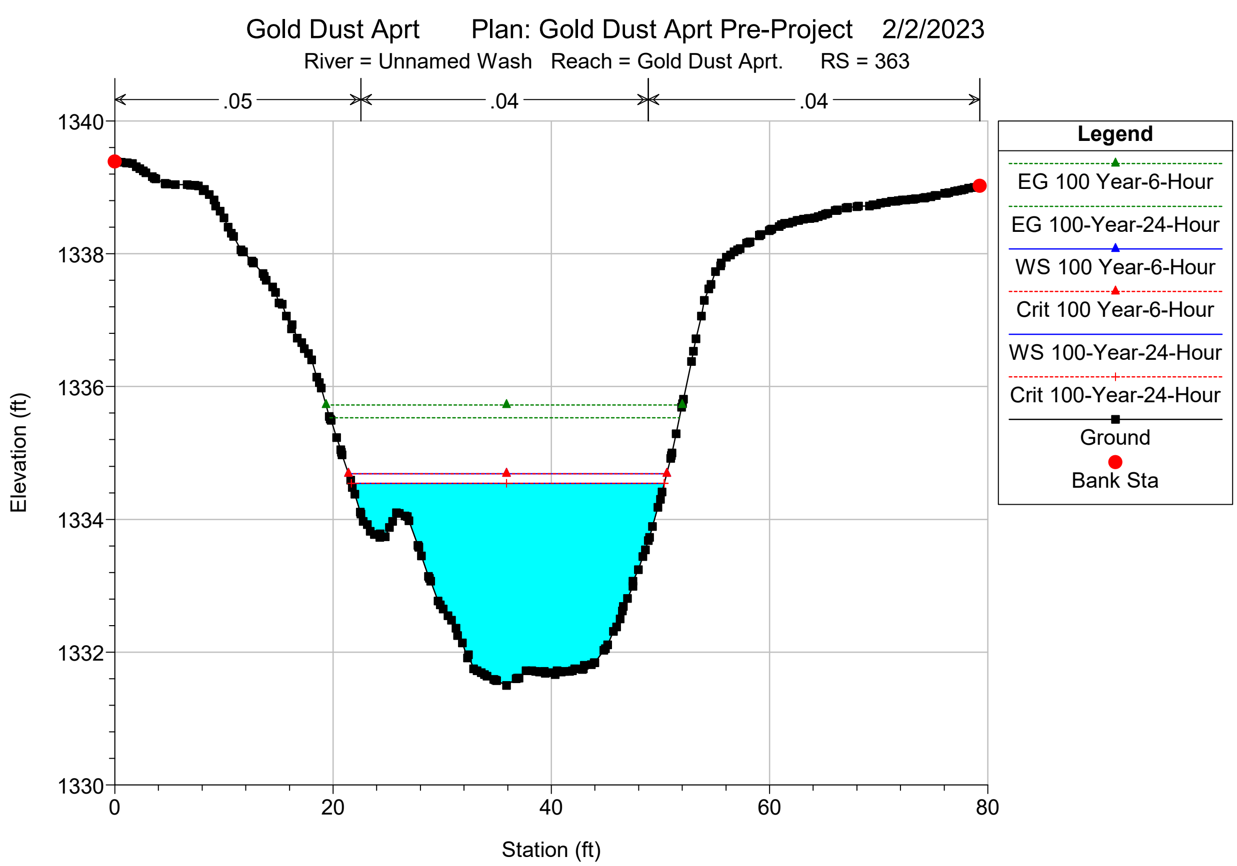

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 712
.04
.035
.04
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
Station (ft)
1346
1344
1342
1340
1338
1336
1334
1332
0
20
40
60
80
100


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 609
.05
.04
.04
Elevation (ft)
1344
1342
1340
1338
1336
1334
1332
0
10
20
30
40
50
60
70
80
Station (ft)
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta

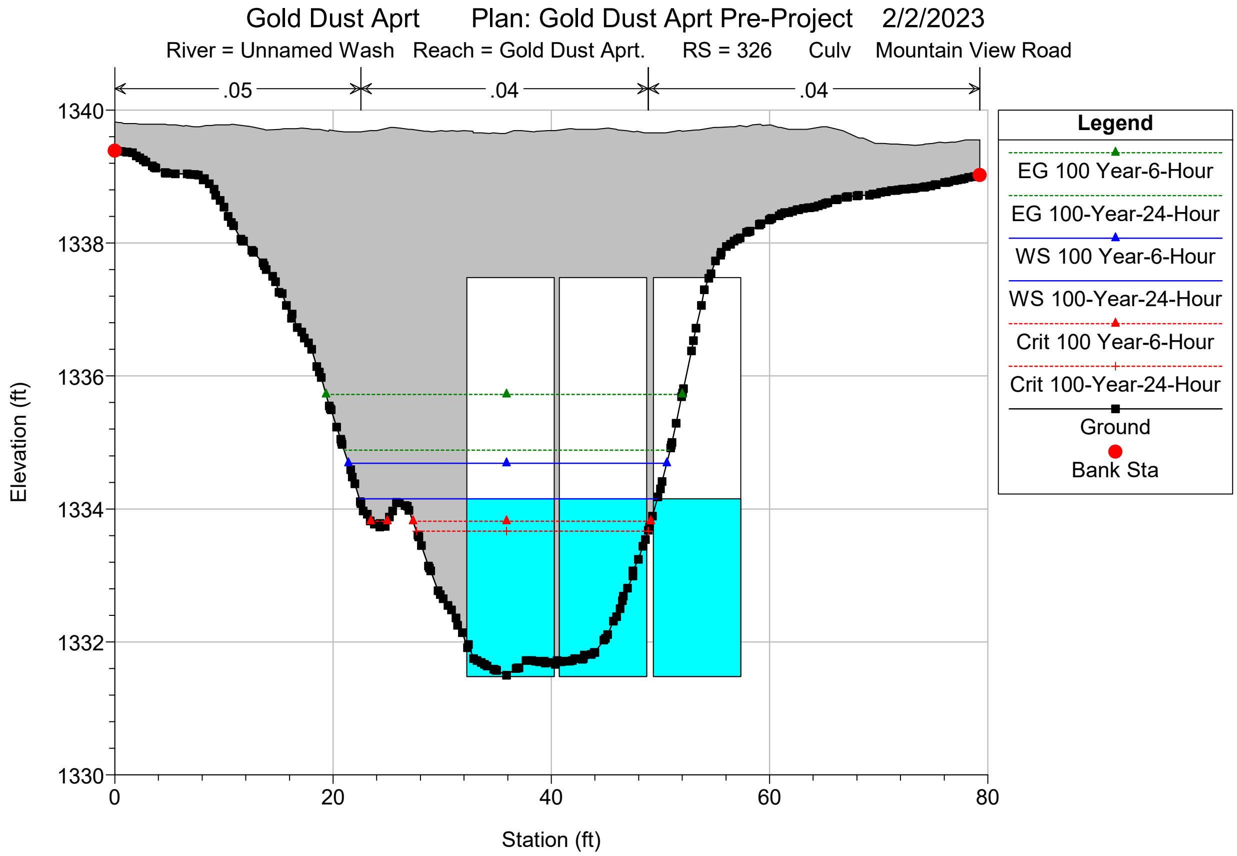

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 508
.05
.04
.04
Elevation (ft)
1344
1342
1340
1338
1336
1334
1332
0
20
40
60
80
Station (ft)
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta

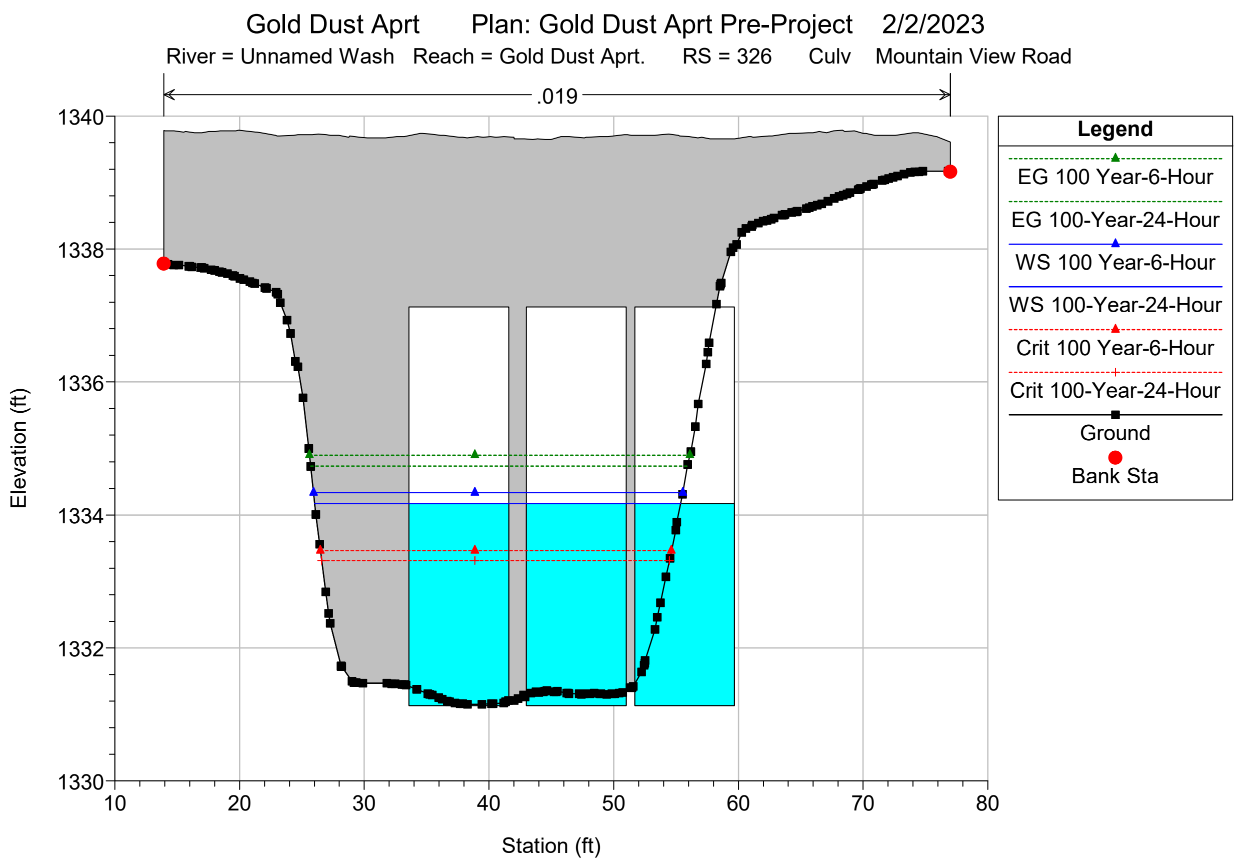


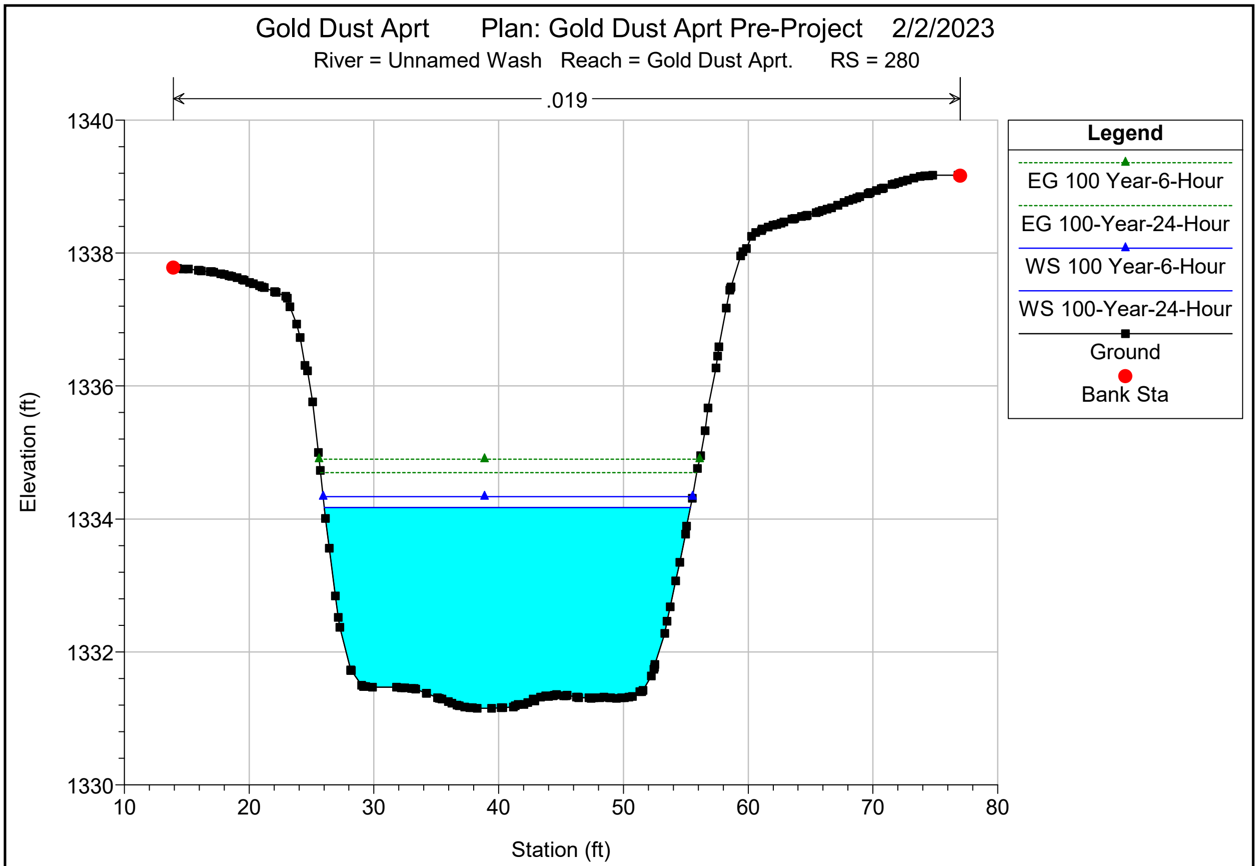
Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 413
.05
.04
.04
Legend
EG 100 Year-6-Hour
WS 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1330
1332
1334
1336
1338
1340
1342
0
20
40
60
80
Station (ft)

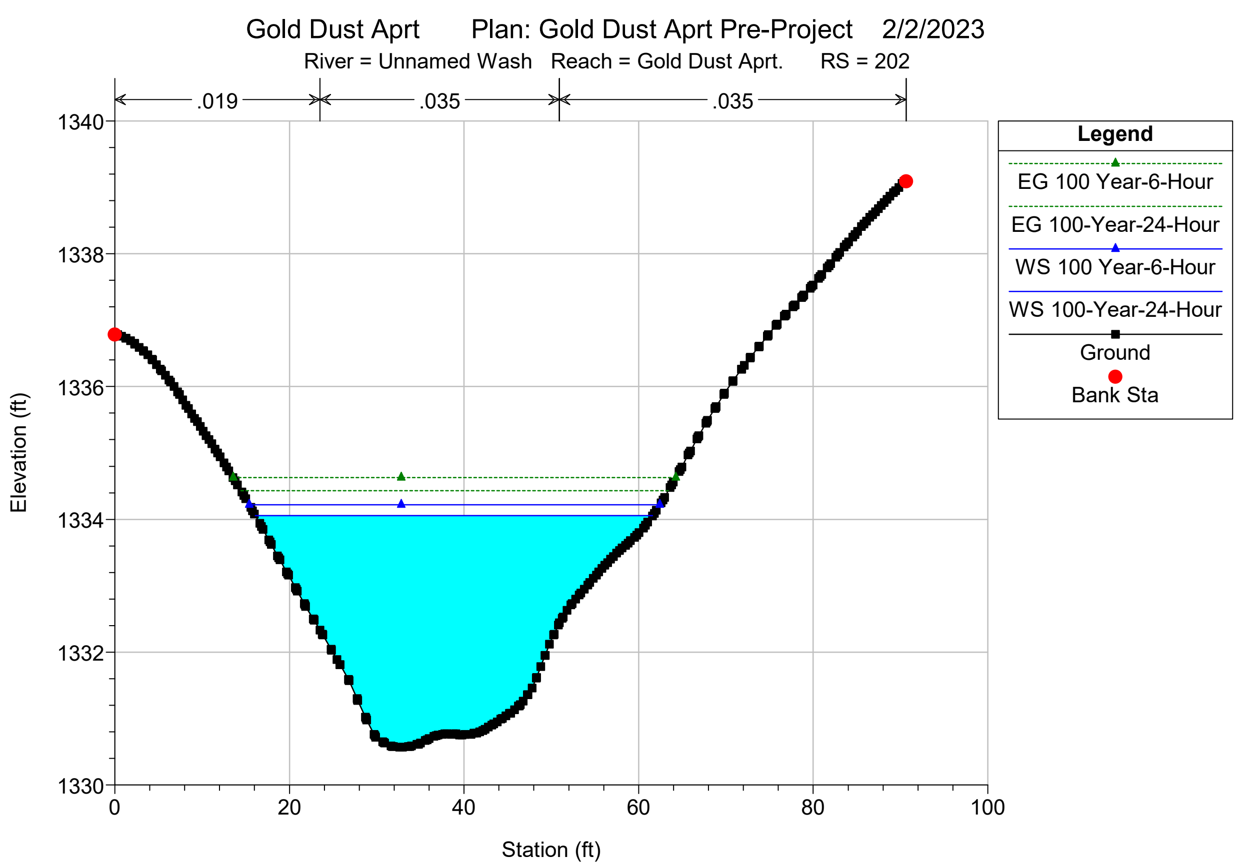


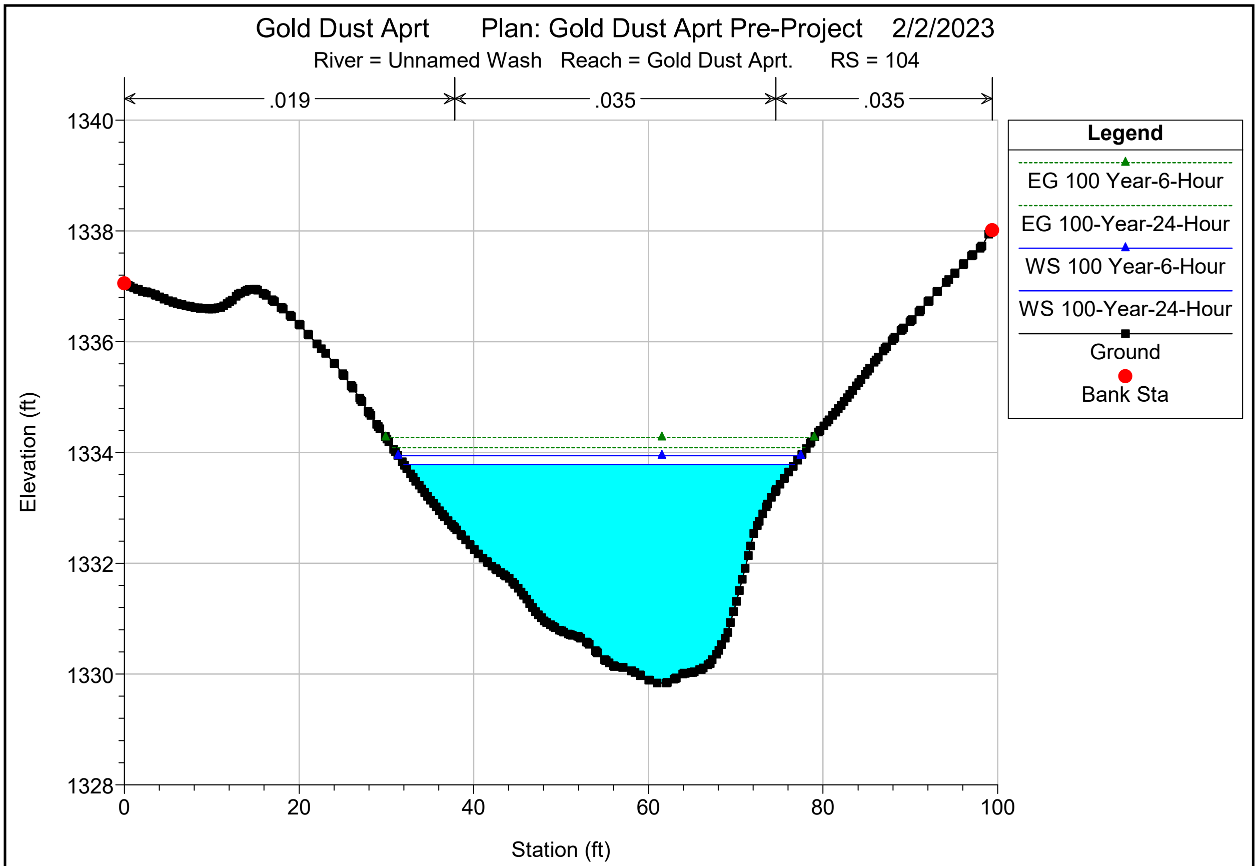
Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 363
.05
.04
.04
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
Crit 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1330
1332
1334
1336
1338
1340
0
20
40
60
80
Station (ft)

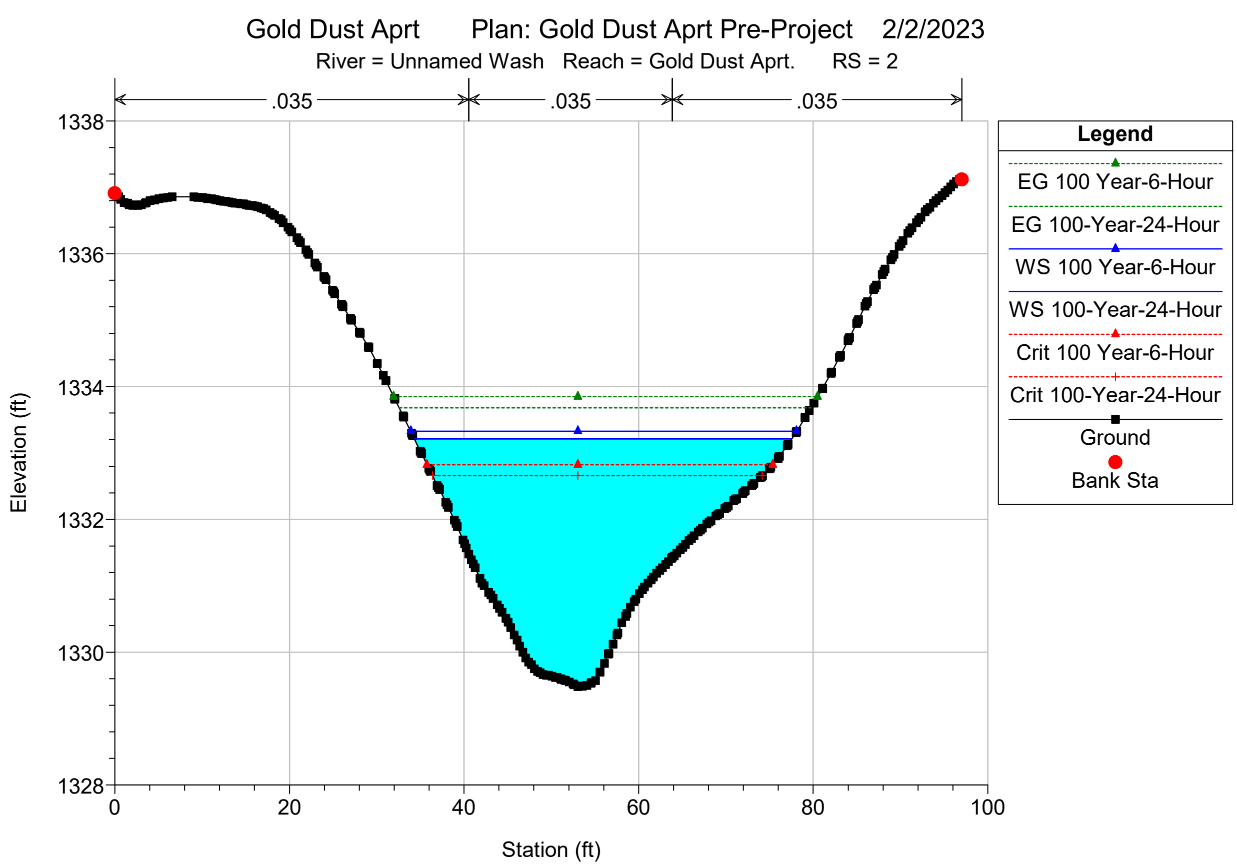

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 326 Culv Mountain View Road
.05
.04
.04
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1340
1338
1336
1334
1332
1330
0
20
40
60
80
Station (ft)

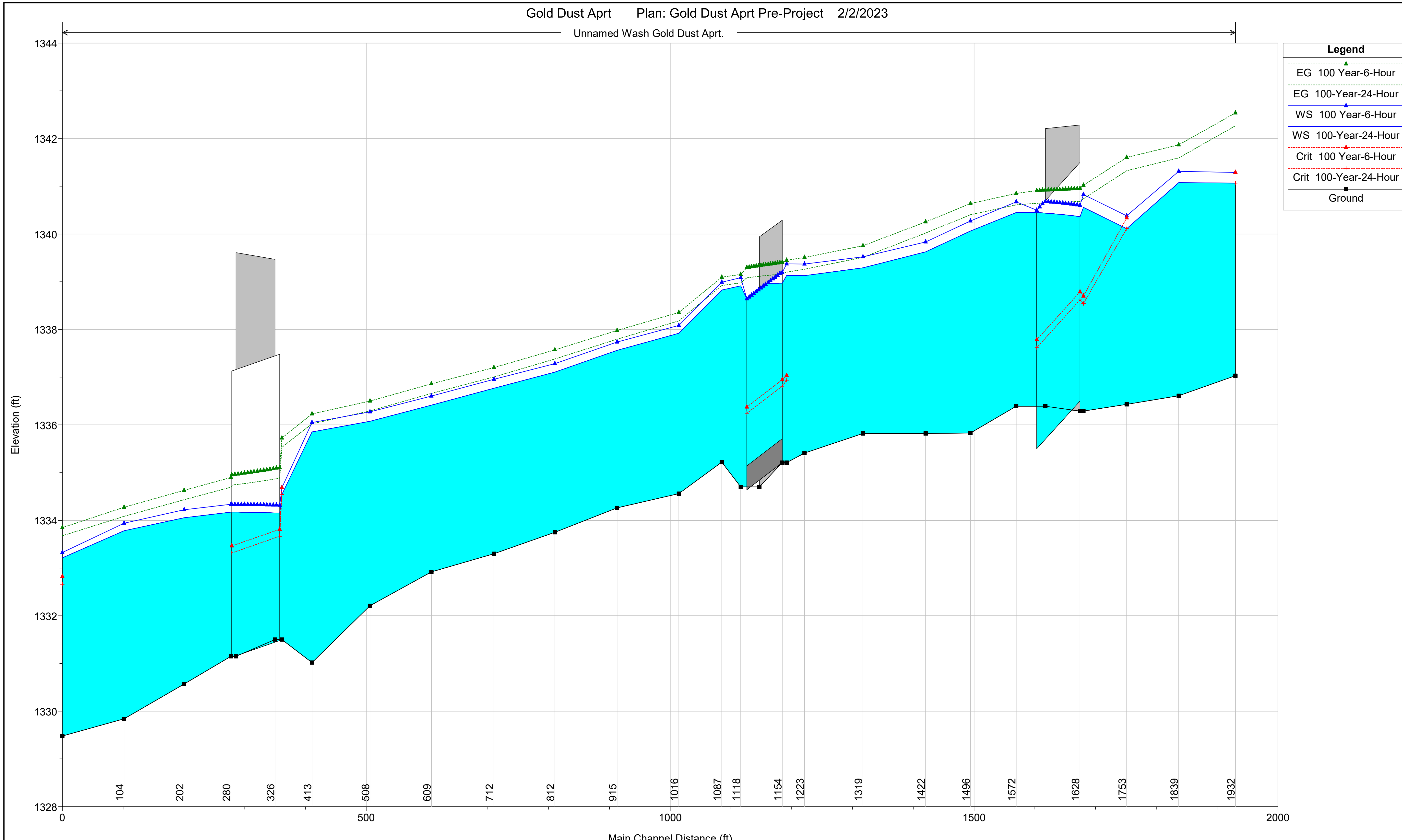

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 326 Culv Mountain View Road
.019
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1340
1338
1336
1334
1332
1330
10
20
30
40
50
60
70
80
Station (ft)


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 280
.019
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1340
1338
1336
1334
1332
1330
10
20
30
40
50
60
70
80
Station (ft)


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 202
.019
.035
.035
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1340
1338
1336
1334
1332
1330
0
20
40
60
80
100
Station (ft)

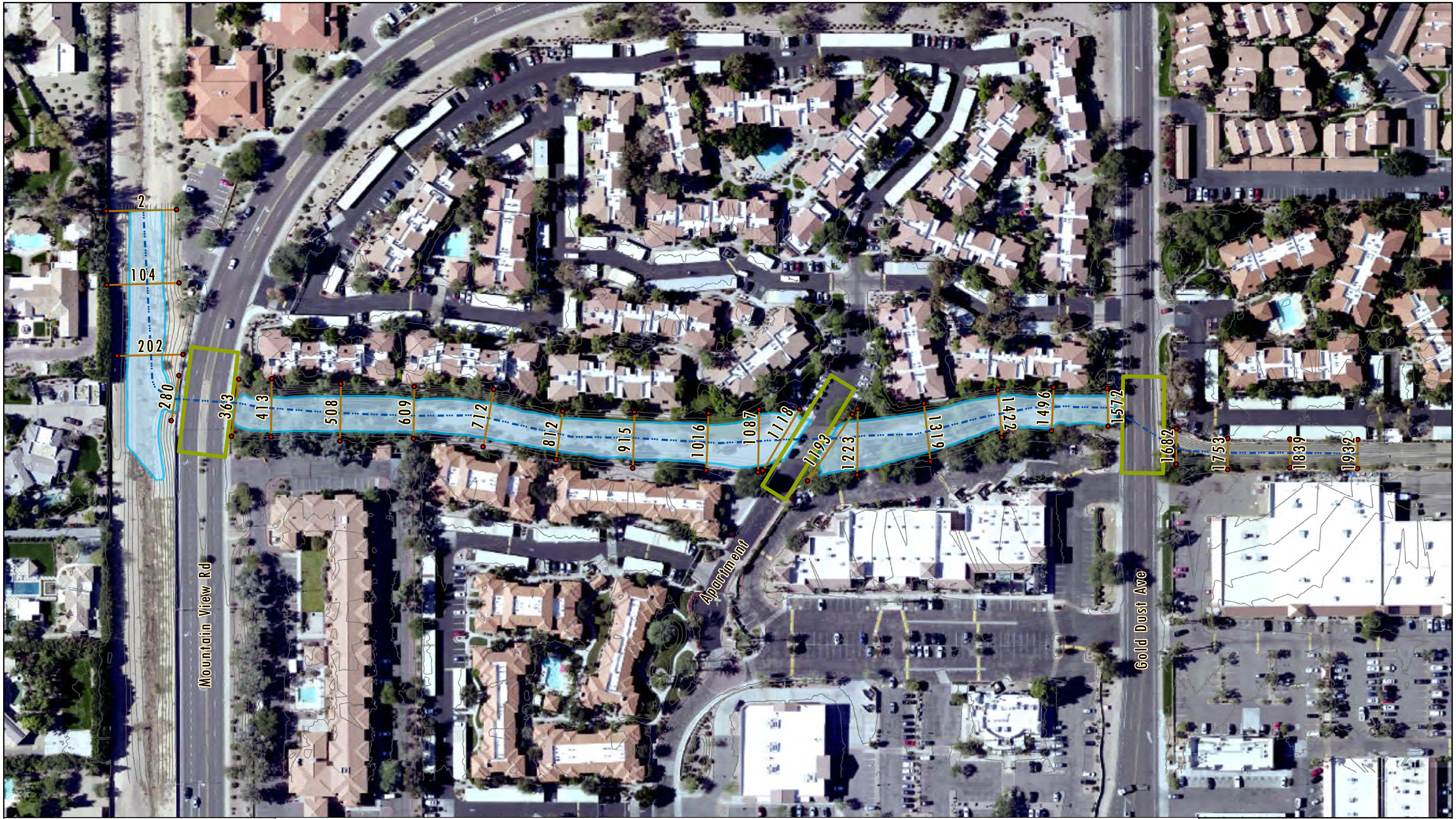

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 104
.019
.035
.035
Elevation (ft)
Station (ft)
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
River = Unnamed Wash Reach = Gold Dust Aprt. RS = 2
.035
.035
.035
Elevation (ft)
Station (ft)
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 2/2/2023
Unnamed Wash Gold Dust Aprt.
Legend
EG 100 Year-6-Hour
EG 100-Year-24-Hour
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Elevation (ft)
Main Channel Distance (ft)
1328
1330
1332
1334
1336
1338
1340
1342
1344
0
500
1000
1500
2000
104
202
280
326
413
508
609
712
812
915
1016
1087
1118
1154
1223
1319
1422
1496
1572
1628
1753
1839
1932

HEC-RAS Plan: Gold Dust Aprt Pre-Project River: Unnamed Wash Reach: Gold Dust Aprt.

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Gold Dust Aprt.	1932	100-Year-24-Hour	420.00	1337.03	1341.07	1341.07	1342.27	0.004486	8.79	47.79	20.25	1.01
Gold Dust Aprt.	1932	100 Year-6-Hour	470.00	1337.03	1341.29	1341.29	1342.54	0.004414	8.96	52.43	21.30	1.01
Gold Dust Aprt.	1839	100-Year-24-Hour	420.00	1336.61	1341.08		1341.60	0.001441	5.78	72.72	24.75	0.59
Gold Dust Aprt.	1839	100 Year-6-Hour	470.00	1336.61	1341.31		1341.87	0.001461	5.98	78.61	25.63	0.60
Gold Dust Aprt.	1753	100-Year-24-Hour	420.00	1336.43	1340.11	1340.11	1341.33	0.004446	8.83	47.54	19.89	1.01
Gold Dust Aprt.	1753	100 Year-6-Hour	470.00	1336.43	1340.38	1340.34	1341.60	0.004138	8.87	52.98	20.81	0.98
Gold Dust Aprt.	1682	100-Year-24-Hour	420.00	1336.29	1340.56	1338.54	1340.74	0.000414	3.41	123.08	37.21	0.33
Gold Dust Aprt.	1682	100 Year-6-Hour	470.00	1336.29	1340.83	1338.70	1341.02	0.000410	3.53	133.11	37.66	0.33
Gold Dust Aprt.	1628		Culvert									
Gold Dust Aprt.	1572	100-Year-24-Hour	420.00	1336.39	1340.45		1340.61	0.001112	3.22	130.24	39.64	0.31
Gold Dust Aprt.	1572	100 Year-6-Hour	470.00	1336.39	1340.67		1340.85	0.001130	3.38	139.13	40.27	0.32
Gold Dust Aprt.	1496	100-Year-24-Hour	460.00	1335.83	1340.06		1340.41	0.004318	4.69	98.09	40.52	0.53
Gold Dust Aprt.	1496	100 Year-6-Hour	518.00	1335.83	1340.27		1340.64	0.004417	4.85	106.82	43.06	0.54
Gold Dust Aprt.	1422	100-Year-24-Hour	460.00	1335.82	1339.63		1340.02	0.006192	5.04	91.20	40.93	0.60
Gold Dust Aprt.	1422	100 Year-6-Hour	518.00	1335.82	1339.83		1340.25	0.006099	5.19	99.90	42.57	0.60
Gold Dust Aprt.	1319	100-Year-24-Hour	460.00	1335.82	1339.29		1339.51	0.003315	3.78	121.72	52.60	0.44
Gold Dust Aprt.	1319	100 Year-6-Hour	518.00	1335.82	1339.52		1339.75	0.003190	3.87	134.02	54.45	0.43
Gold Dust Aprt.	1223	100-Year-24-Hour	460.00	1335.41	1339.13		1339.26	0.001740	2.93	157.03	65.35	0.33
Gold Dust Aprt.	1223	100 Year-6-Hour	518.00	1335.41	1339.37		1339.51	0.001666	3.00	172.95	66.26	0.33
Gold Dust Aprt.	1193	100-Year-24-Hour	460.00	1335.21	1339.13	1336.94	1339.20	0.000605	2.10	218.78	70.27	0.21
Gold Dust Aprt.	1193	100 Year-6-Hour	518.00	1335.21	1339.37	1337.03	1339.45	0.000598	2.20	235.62	70.48	0.21
Gold Dust Aprt.	1154		Culvert									
Gold Dust Aprt.	1118	100-Year-24-Hour	460.00	1334.70	1338.92		1338.98	0.000436	1.98	232.89	80.40	0.20
Gold Dust Aprt.	1118	100 Year-6-Hour	518.00	1334.70	1339.09		1339.15	0.000460	2.10	246.73	81.52	0.21
Gold Dust Aprt.	1087	100-Year-24-Hour	460.00	1335.22	1338.83		1338.92	0.059473	2.45	187.61	68.45	0.26
Gold Dust Aprt.	1087	100 Year-6-Hour	518.00	1335.22	1338.99		1339.09	0.062155	2.60	198.88	69.91	0.27
Gold Dust Aprt.	1016	100-Year-24-Hour	440.00	1334.56	1337.92		1338.18	0.003942	4.10	107.43	51.38	0.50
Gold Dust Aprt.	1016	100 Year-6-Hour	485.00	1334.56	1338.08		1338.35	0.003834	4.18	115.95	52.85	0.50
Gold Dust Aprt.	915	100-Year-24-Hour	440.00	1334.26	1337.56		1337.79	0.003515	3.86	113.86	50.15	0.45
Gold Dust Aprt.	915	100 Year-6-Hour	485.00	1334.26	1337.73		1337.98	0.003448	3.96	122.63	51.46	0.45
Gold Dust Aprt.	812	100-Year-24-Hour	440.00	1333.75	1337.11		1337.38	0.004566	4.21	104.58	44.83	0.49
Gold Dust Aprt.	812	100 Year-6-Hour	485.00	1333.75	1337.28		1337.57	0.004487	4.31	112.64	45.83	0.48
Gold Dust Aprt.	712	100-Year-24-Hour	440.00	1333.30	1336.77		1337.00	0.002967	3.89	112.97	47.48	0.45
Gold Dust Aprt.	712	100 Year-6-Hour	485.00	1333.30	1336.96		1337.20	0.002917	3.98	121.93	48.81	0.44
Gold Dust Aprt.	609	100-Year-24-Hour	440.00	1332.92	1336.41		1336.66	0.003776	3.98	110.68	44.60	0.44
Gold Dust Aprt.	609	100 Year-6-Hour	485.00	1332.92	1336.60		1336.86	0.003739	4.07	119.24	45.89	0.44
Gold Dust Aprt.	508	100-Year-24-Hour	440.00	1332.21	1336.08		1336.29	0.003331	3.73	117.99	47.18	0.42
Gold Dust Aprt.	508	100 Year-6-Hour	485.00	1332.21	1336.27		1336.50	0.003263	3.81	127.37	48.44	0.41
Gold Dust Aprt.	413	100-Year-24-Hour	440.00	1331.02	1335.85		1336.02	0.002207	3.31	133.11	46.67	0.35
Gold Dust Aprt.	413	100 Year-6-Hour	485.00	1331.02	1336.05		1336.23	0.002224	3.41	142.40	47.82	0.35
Gold Dust Aprt.	363	100-Year-24-Hour	440.00	1331.50	1334.54	1334.54	1335.53	0.020865	7.97	55.23	28.69	1.01
Gold Dust Aprt.	363	100 Year-6-Hour	485.00	1331.50	1334.69	1334.69	1335.72	0.020577	8.18	59.32	29.18	1.01
Gold Dust Aprt.	326		Culvert									
Gold Dust Aprt.	280	100-Year-24-Hour	440.00	1331.15	1334.17		1334.70	0.001704	5.80	75.90	29.35	0.64
Gold Dust Aprt.	280	100 Year-6-Hour	485.00	1331.15	1334.34		1334.90	0.001718	6.01	80.70	29.62	0.64
Gold Dust Aprt.	202	100-Year-24-Hour	470.00	1330.57	1334.05		1334.43	0.003746	4.94	95.12	45.49	0.60
Gold Dust Aprt.	202	100 Year-6-Hour	526.00	1330.57	1334.22		1334.63	0.003769	5.12	102.82	47.04	0.61
Gold Dust Aprt.	104	100-Year-24-Hour	470.00	1329.84	1333.78		1334.08	0.002946	4.42	106.32	44.71	0.51
Gold Dust Aprt.	104	100 Year-6-Hour	526.00	1329.84	1333.94		1334.27	0.003020	4.63	113.58	46.12	0.52
Gold Dust Aprt.	2	100-Year-24-Hour	470.00	1329.48	1333.21	1332.66	1333.68	0.005215	5.47	85.96	43.24	0.68
Gold Dust Aprt.	2	100 Year-6-Hour	526.00	1329.48	1333.33	1332.82	1333.85	0.005621	5.79	90.86	44.19	0.71



Legend

- BANK STATION
- RIVER REACH
- ROADWAY CROSSING
- CROSS SECTION W/ RIVER STATION
- 100 YR INUNDATION

GOLD DUST APARTMENTS - UNNAMED CHANNEL

Exhibit G-2

Post Project Condition 100 Year Inundation

0 75 150 300

Feet

DIBBLE

Exhibit G-1: Post Project Condition 100 Year Inundation

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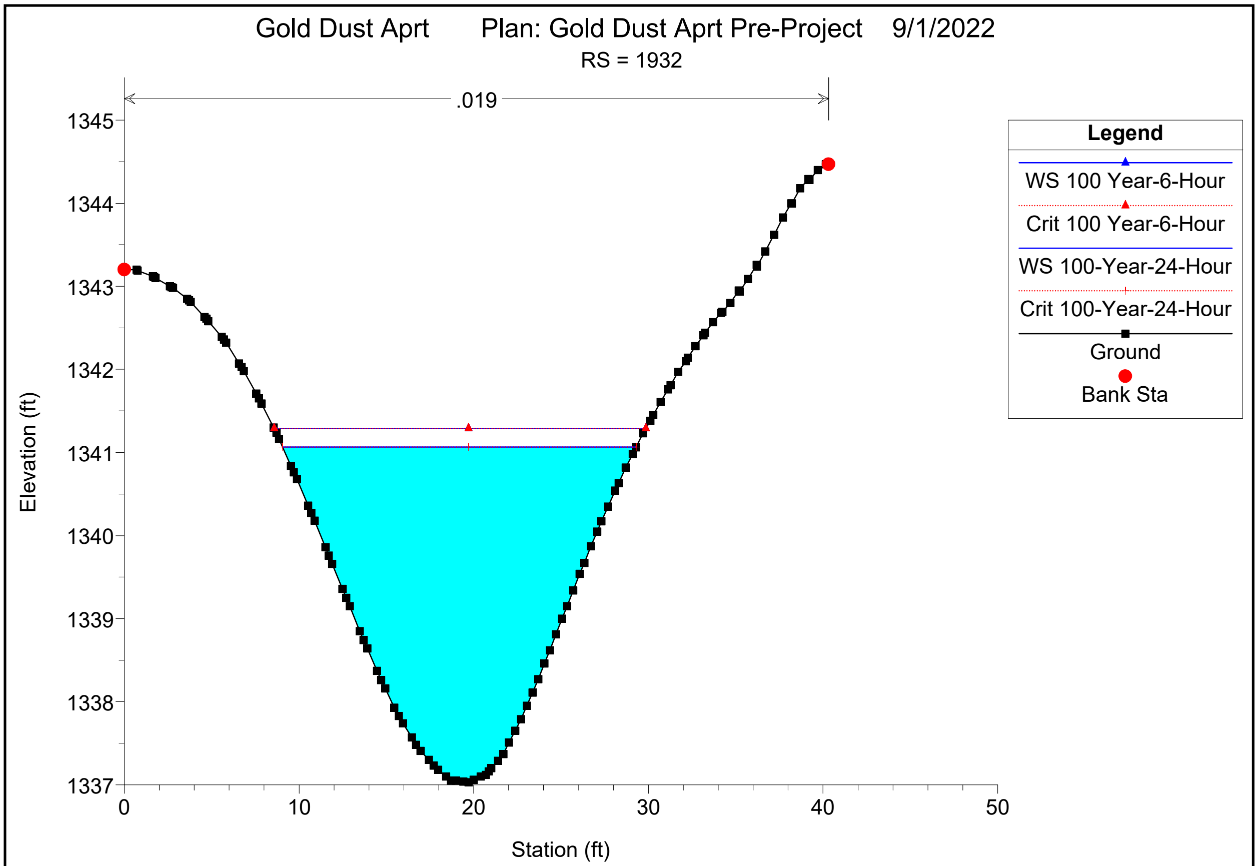
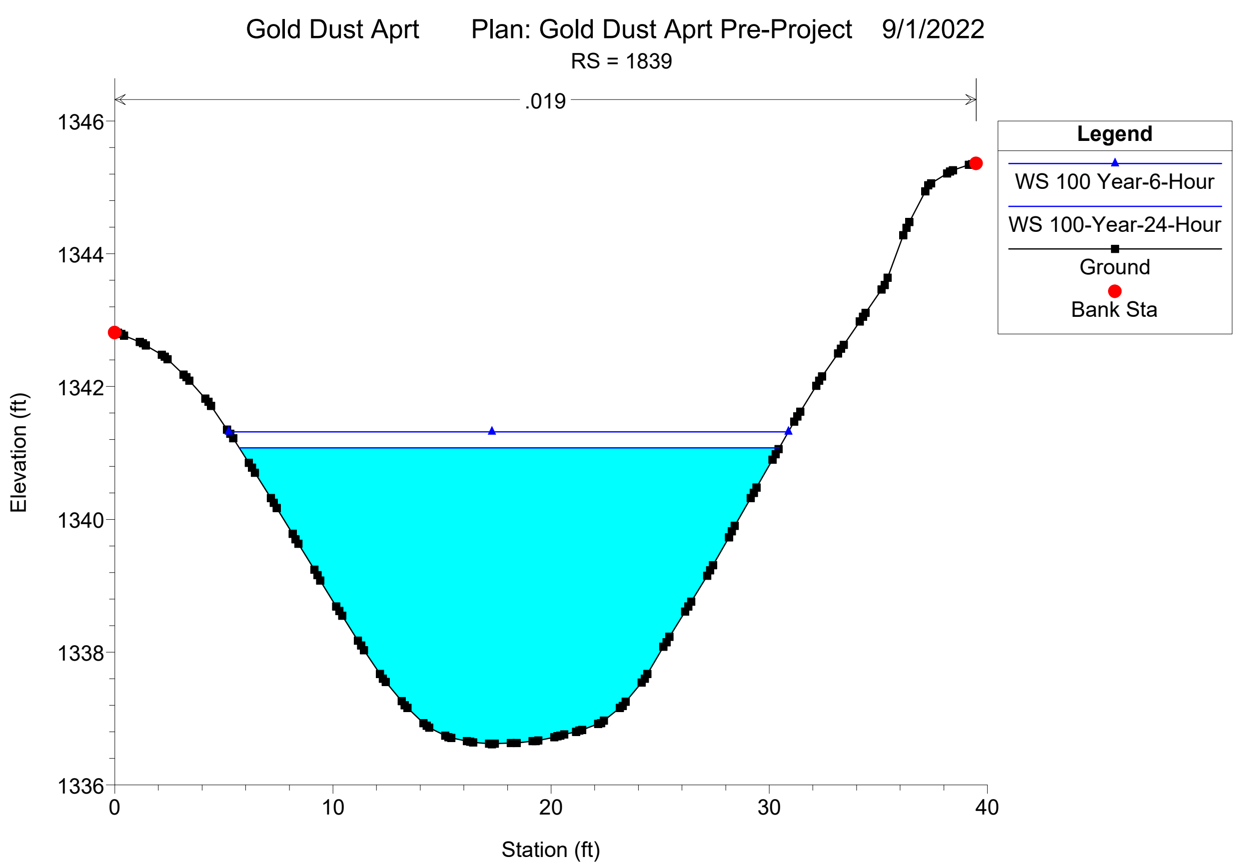
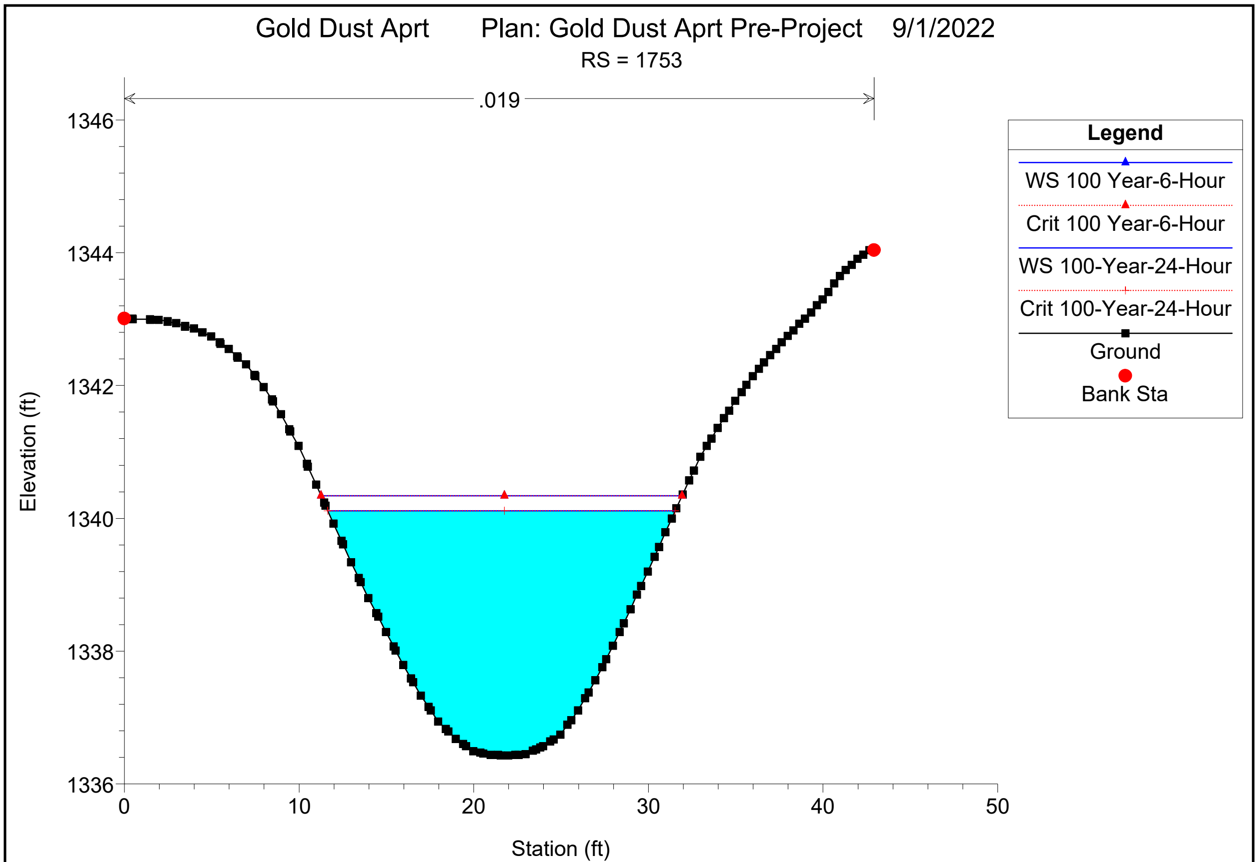
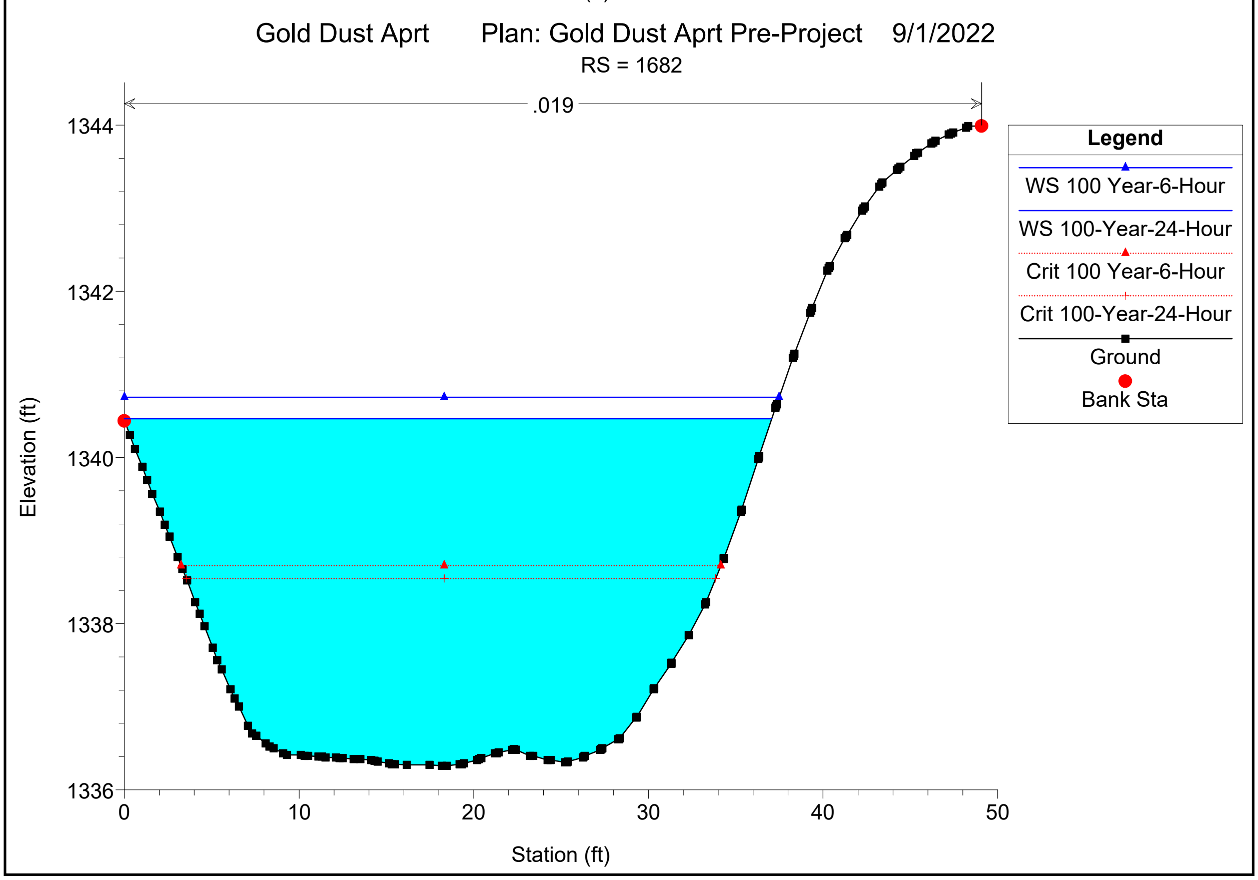
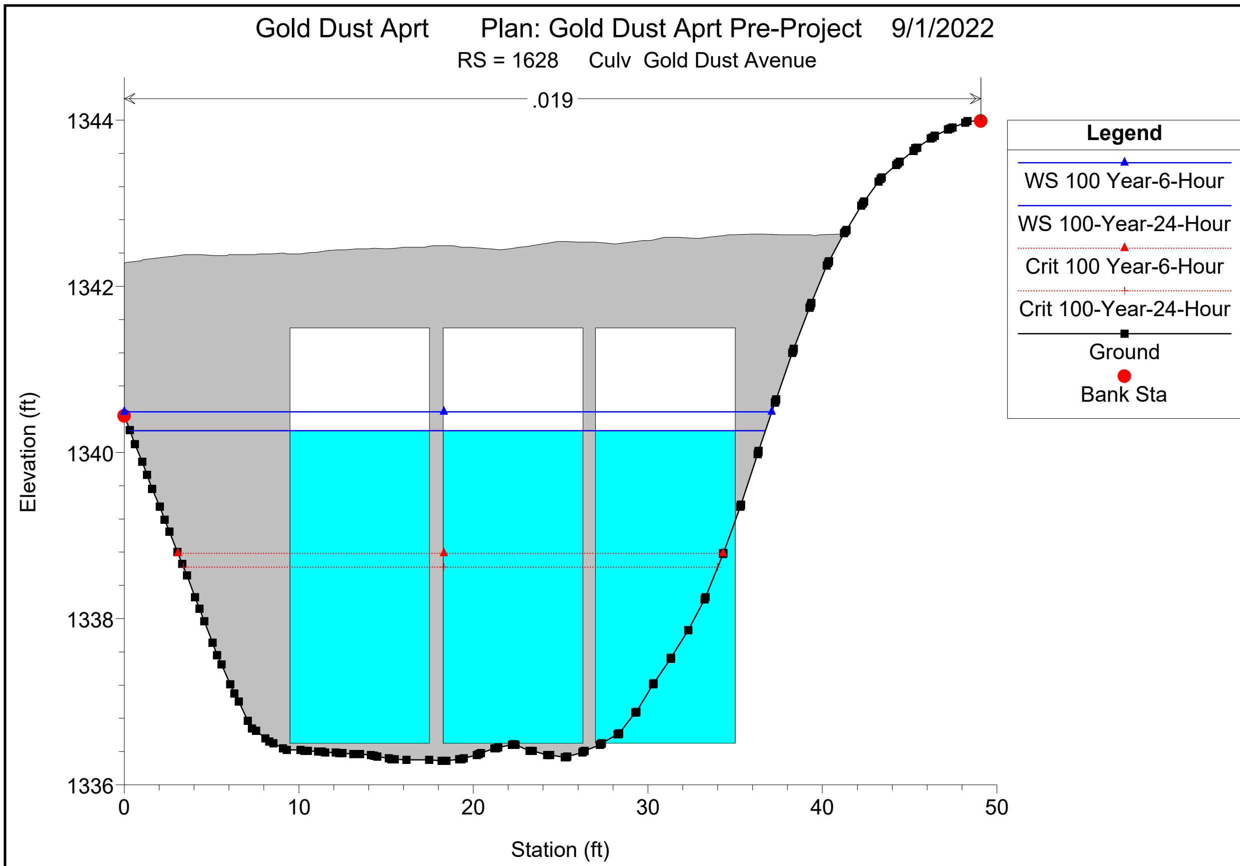
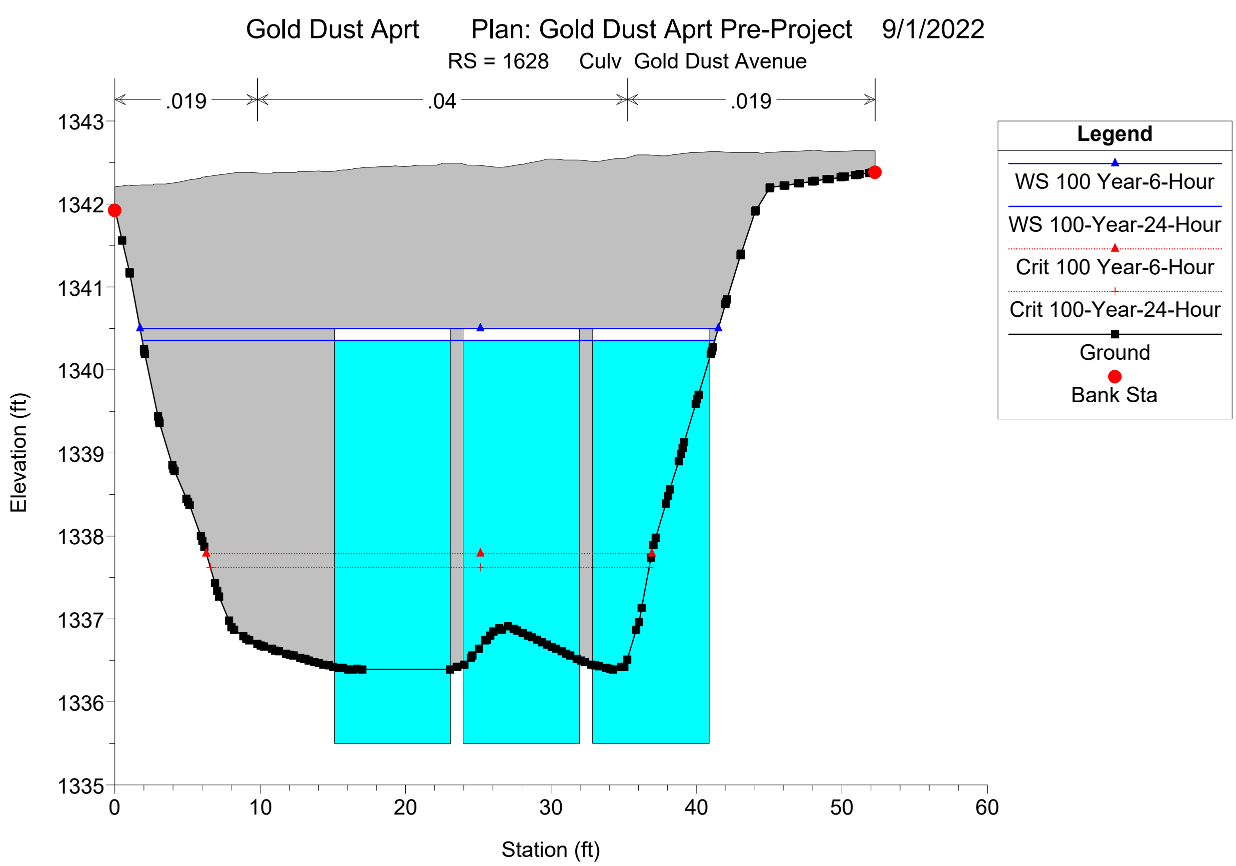

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1932
.019
Legend
WS 100 Year-6-Hour
Crit 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1345
1344
1343
1342
1341
1340
1339
1338
1337
0
10
20
30
40
50
Station (ft)

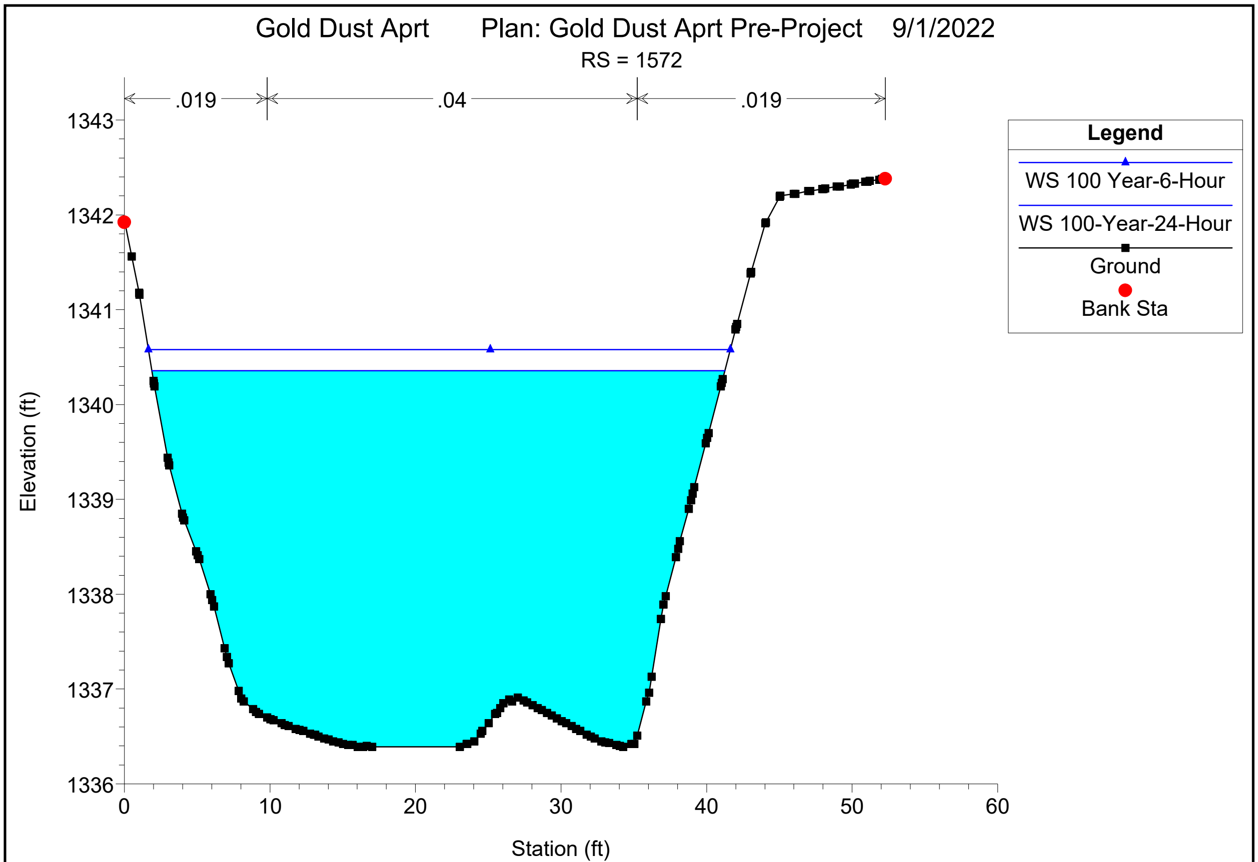

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1839
.019
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1346
1344
1342
1340
1338
1336
0
10
20
30
40
Station (ft)

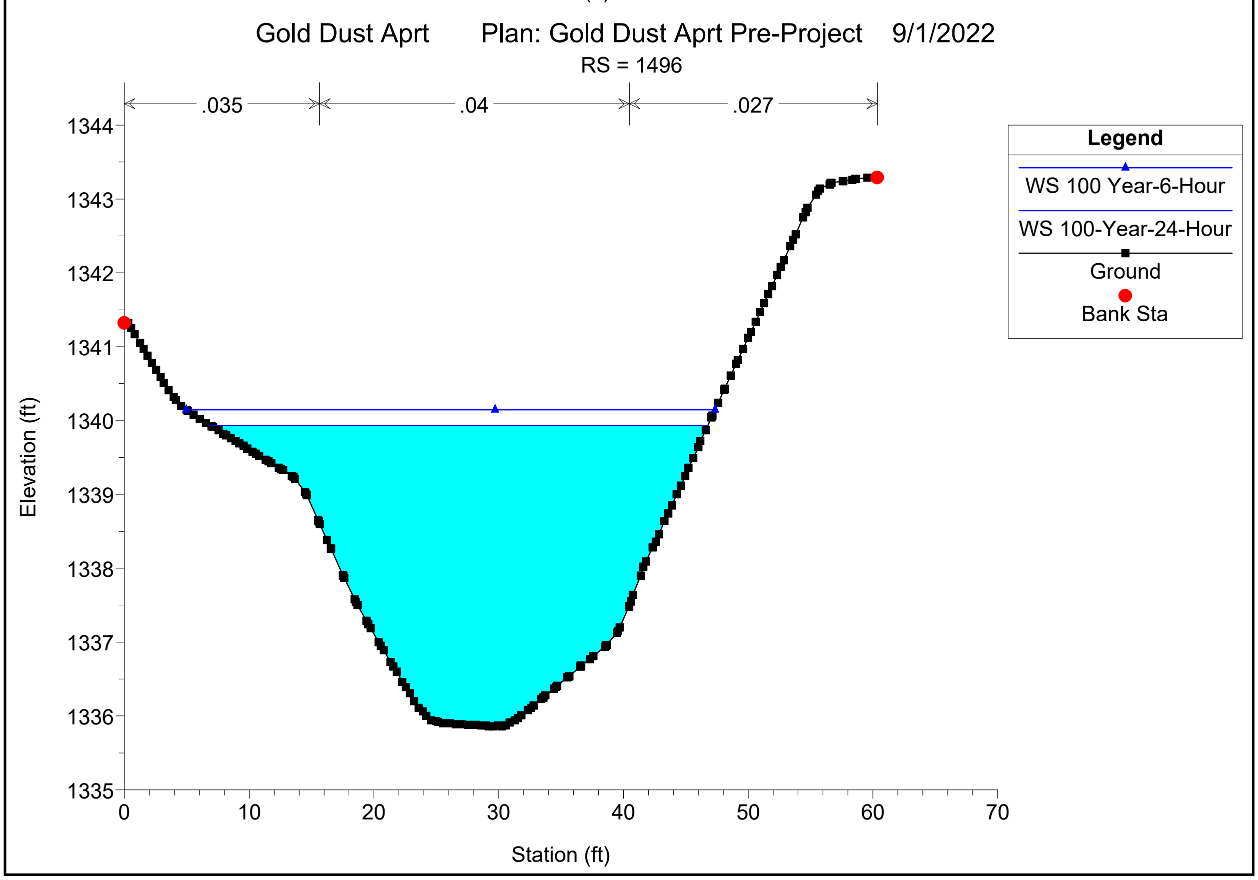

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1753
.019
Elevation (ft)
1346
1344
1342
1340
1338
1336
0
10
20
30
40
50
Station (ft)
Legend
WS 100 Year-6-Hour
Crit 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta

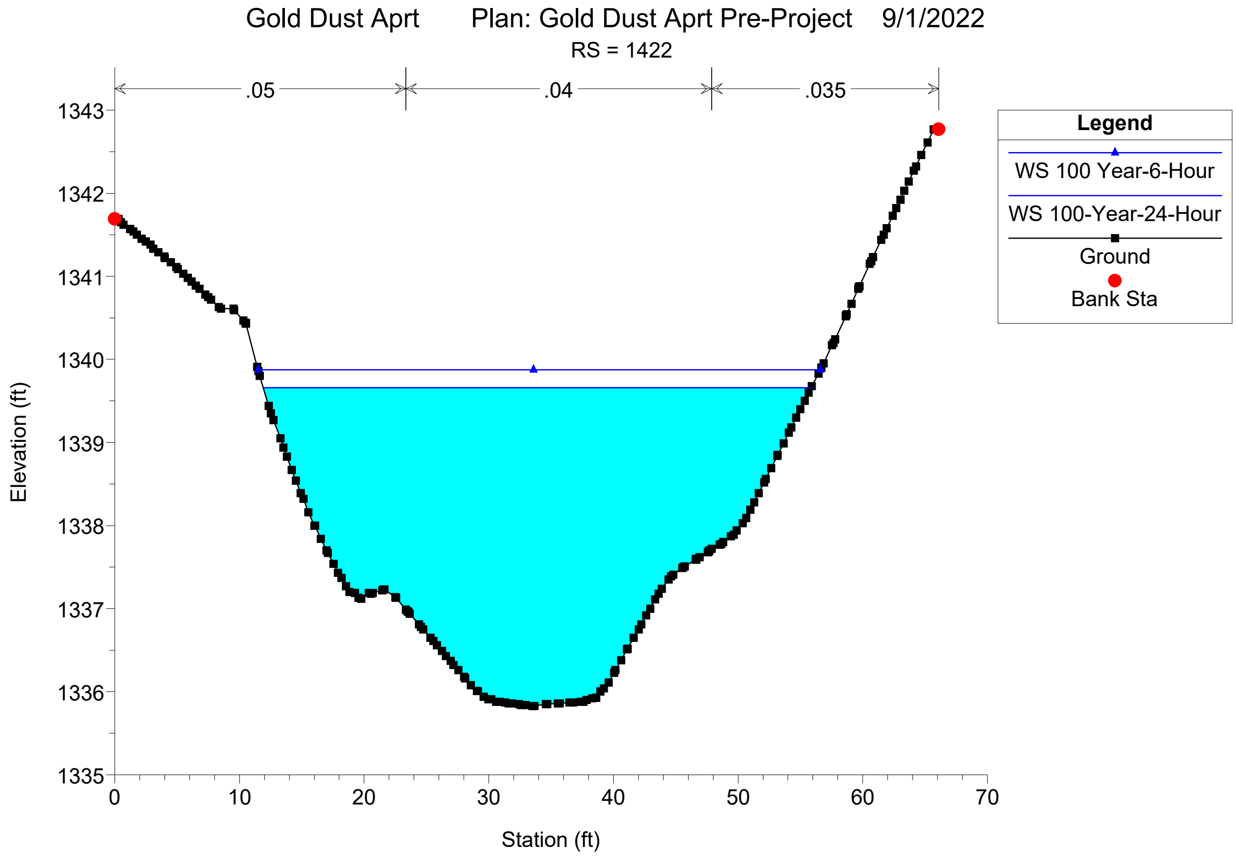

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1682
.019
Elevation (ft)
1344
1342
1340
1338
1336
0
10
20
30
40
50
Station (ft)
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta

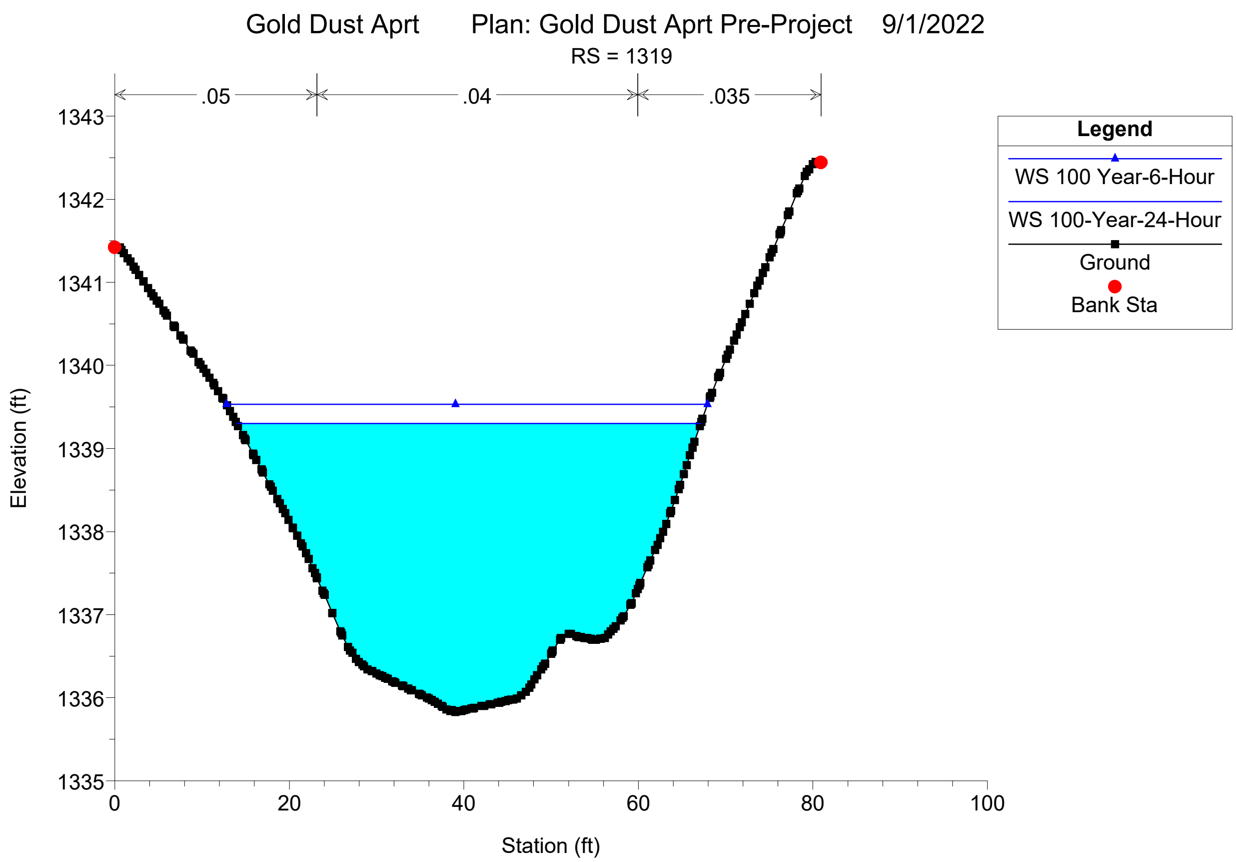

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1628 Culv Gold Dust Avenue
.019
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1344
1342
1340
1338
1336
0
10
20
30
40
50
Station (ft)

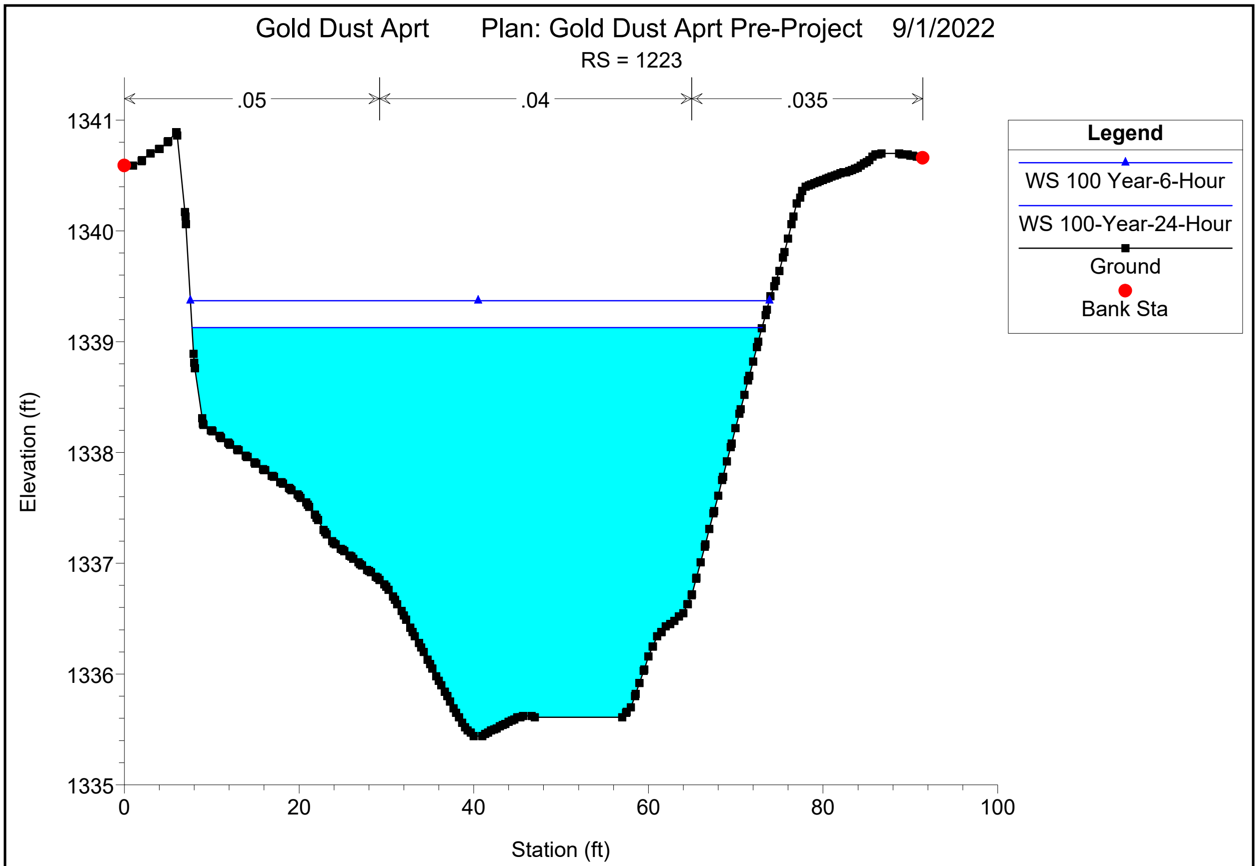

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1628 Culv Gold Dust Avenue
.019
.04
.019
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1343
1342
1341
1340
1339
1338
1337
1336
1335
0
10
20
30
40
50
60
Station (ft)

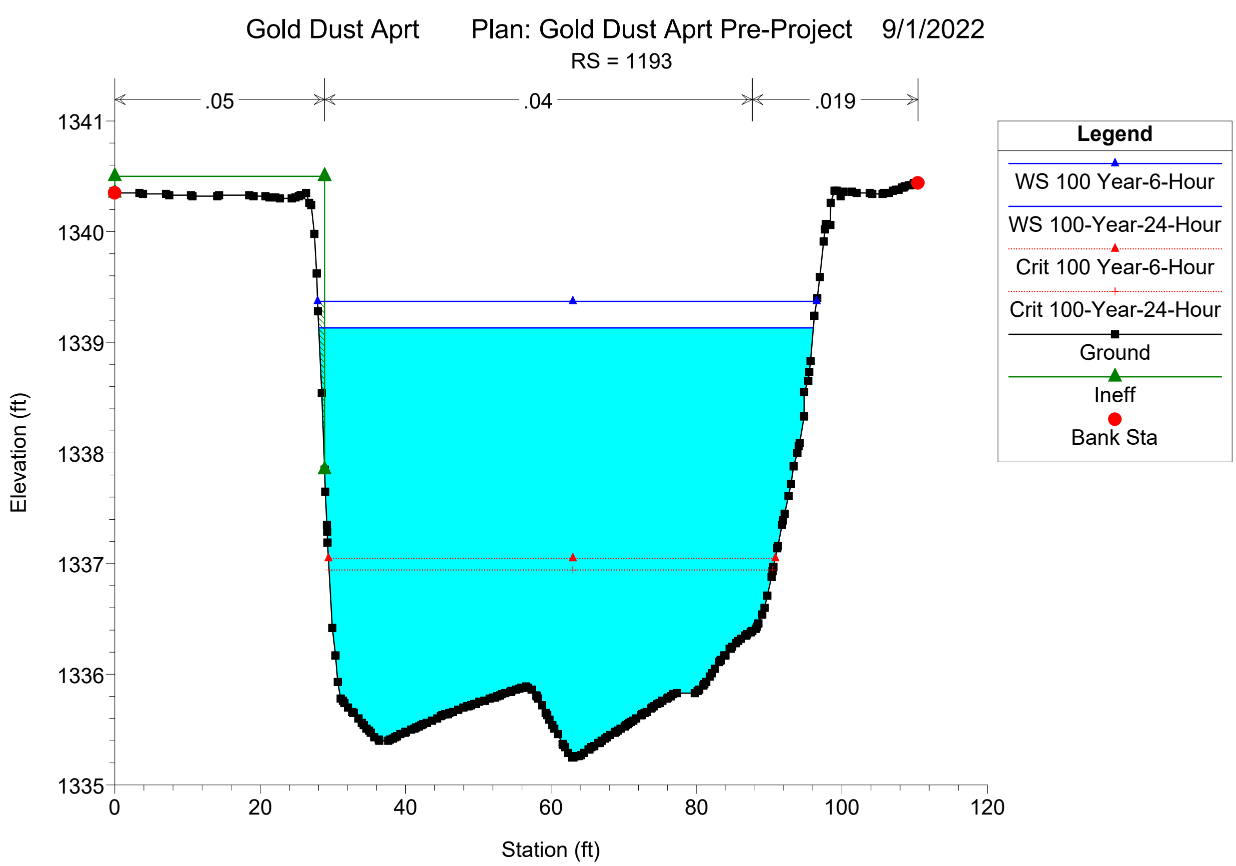

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1572
.019
.04
.019
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1336
1337
1338
1339
1340
1341
1342
1343
Station (ft)
0
10
20
30
40
50
60

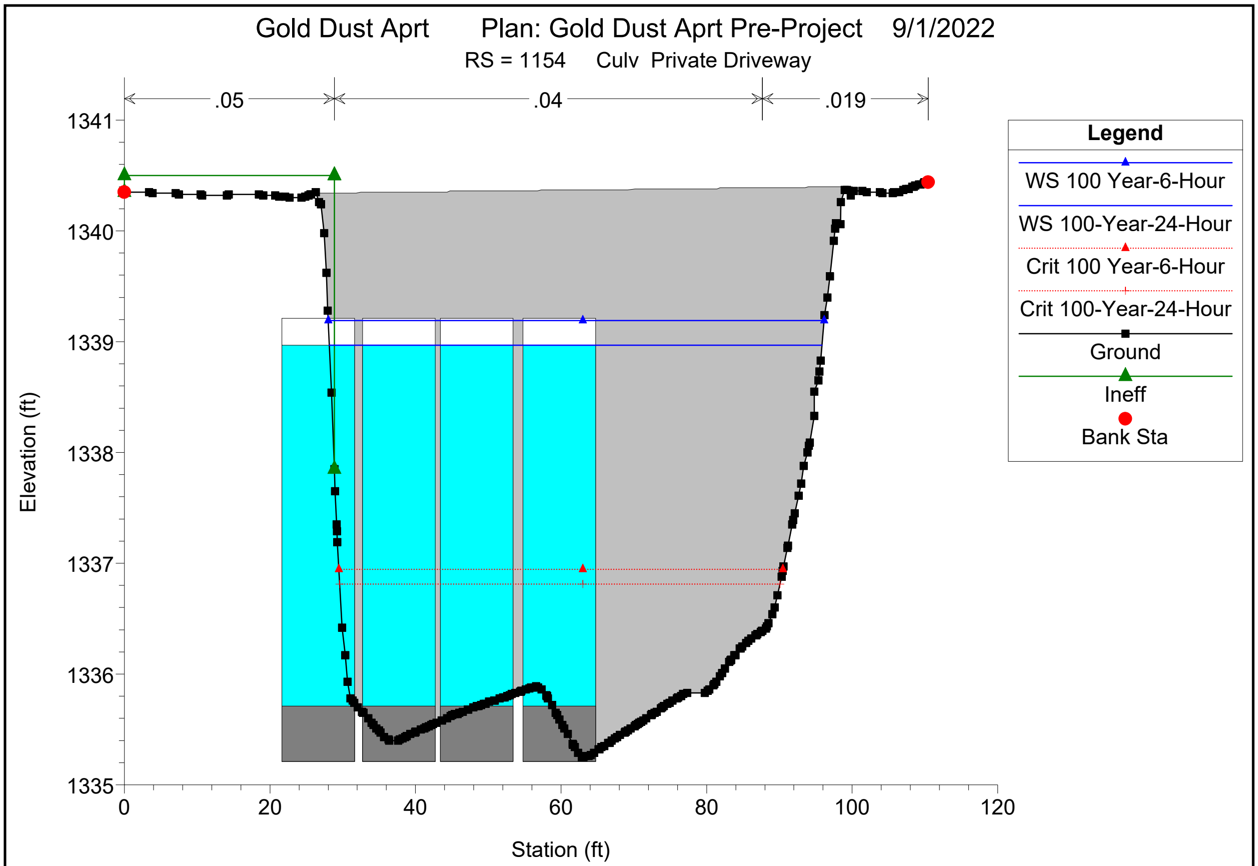

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1496
.035
.04
.027
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1335
1336
1337
1338
1339
1340
1341
1342
1343
1344
Station (ft)
0
10
20
30
40
50
60
70

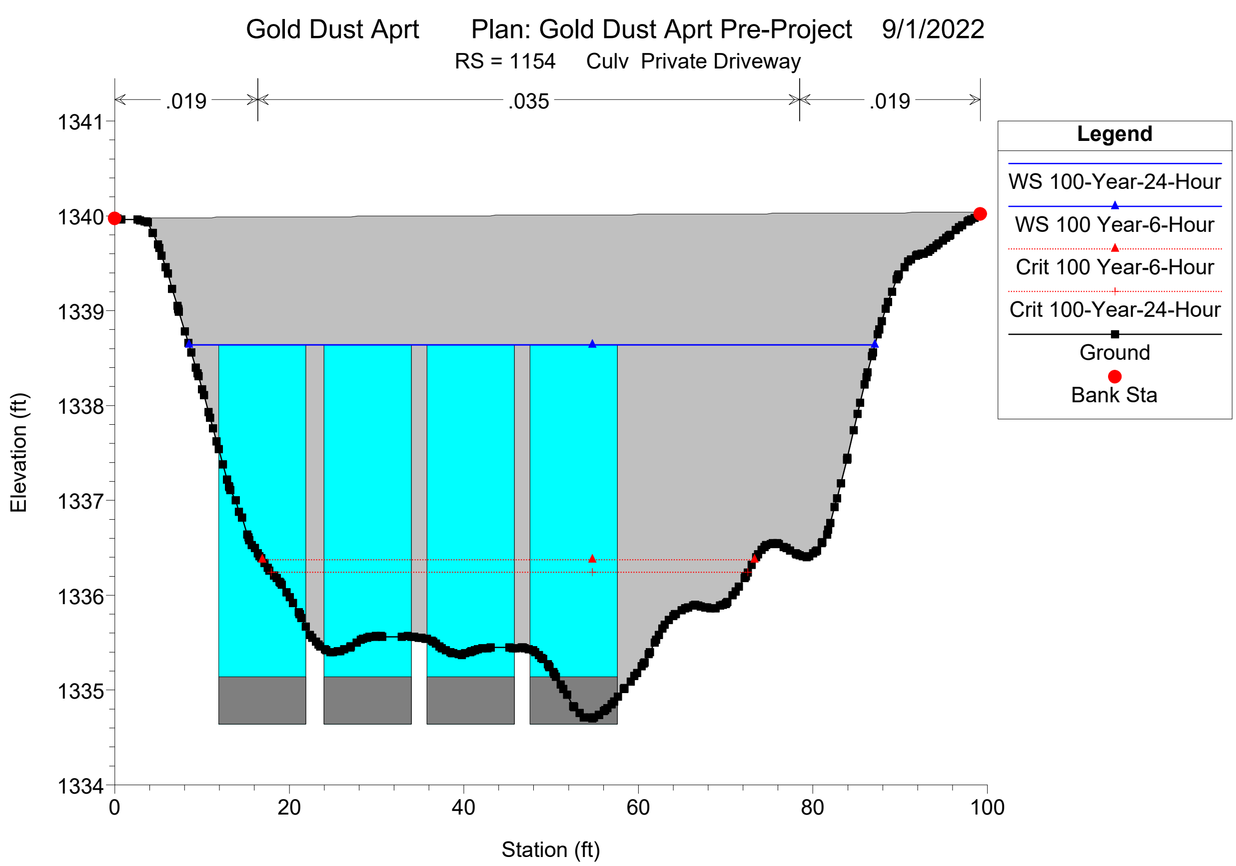

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1422
.05
.04
.035
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1343
1342
1341
1340
1339
1338
1337
1336
1335
0
10
20
30
40
50
60
70
Station (ft)

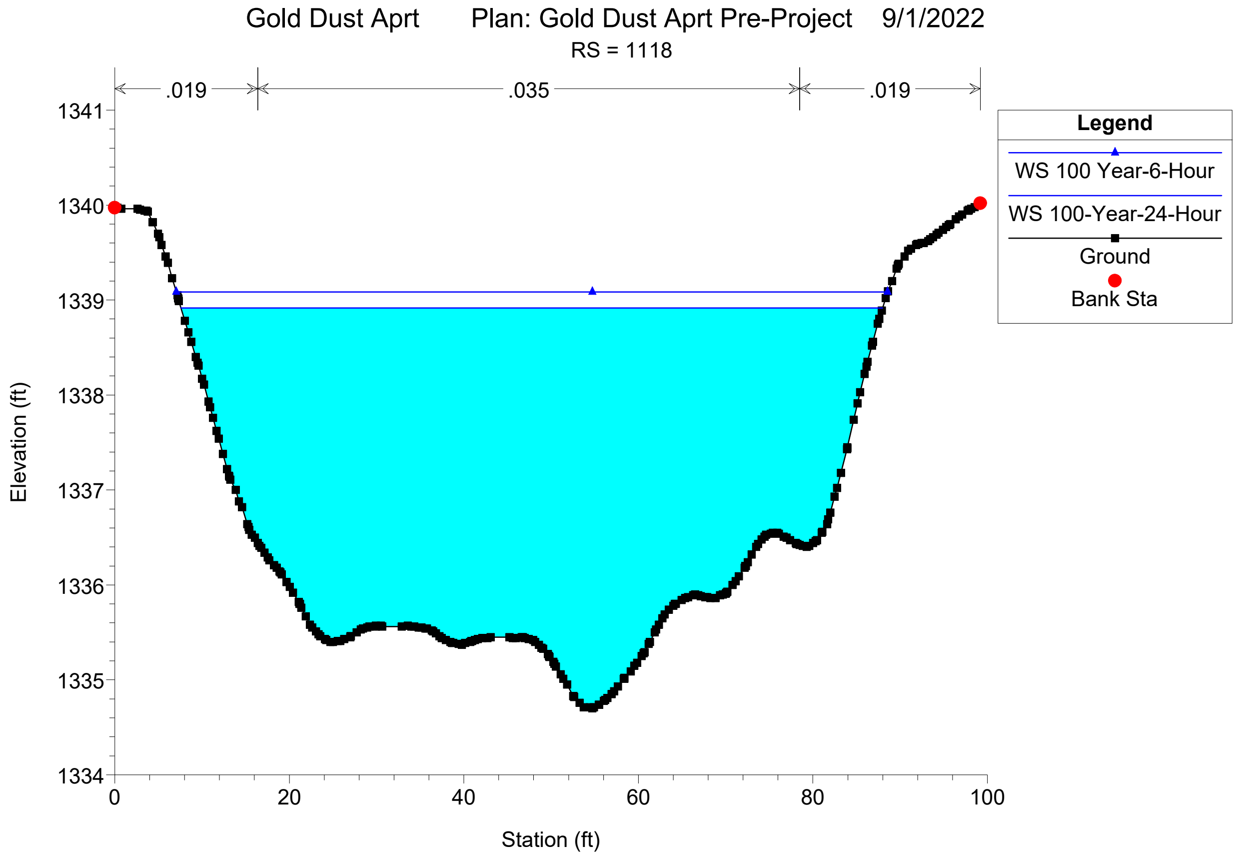

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1319
.05
.04
.035
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1343
1342
1341
1340
1339
1338
1337
1336
1335
0
20
40
60
80
100
Station (ft)

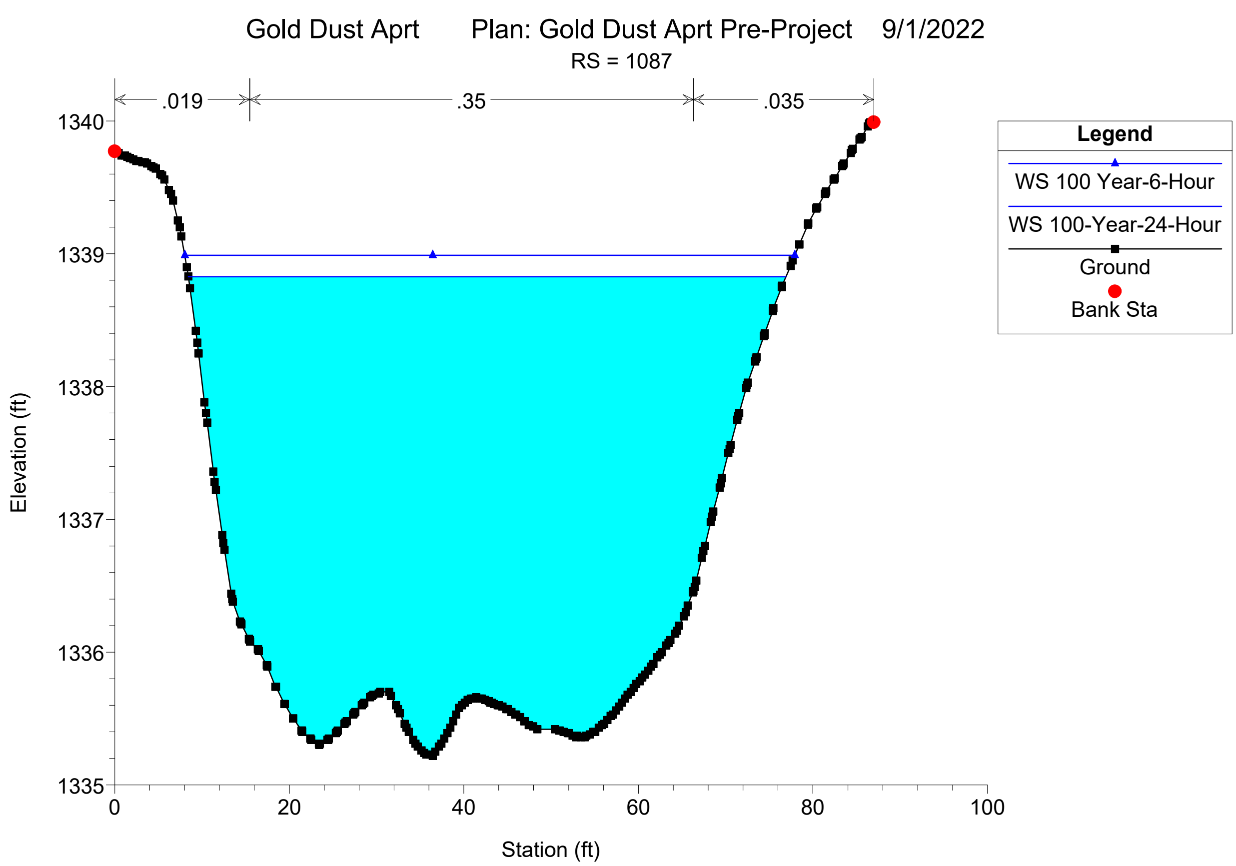

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1223
.05
.04
.035
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1341
1340
1339
1338
1337
1336
1335
0
20
40
60
80
100
Station (ft)

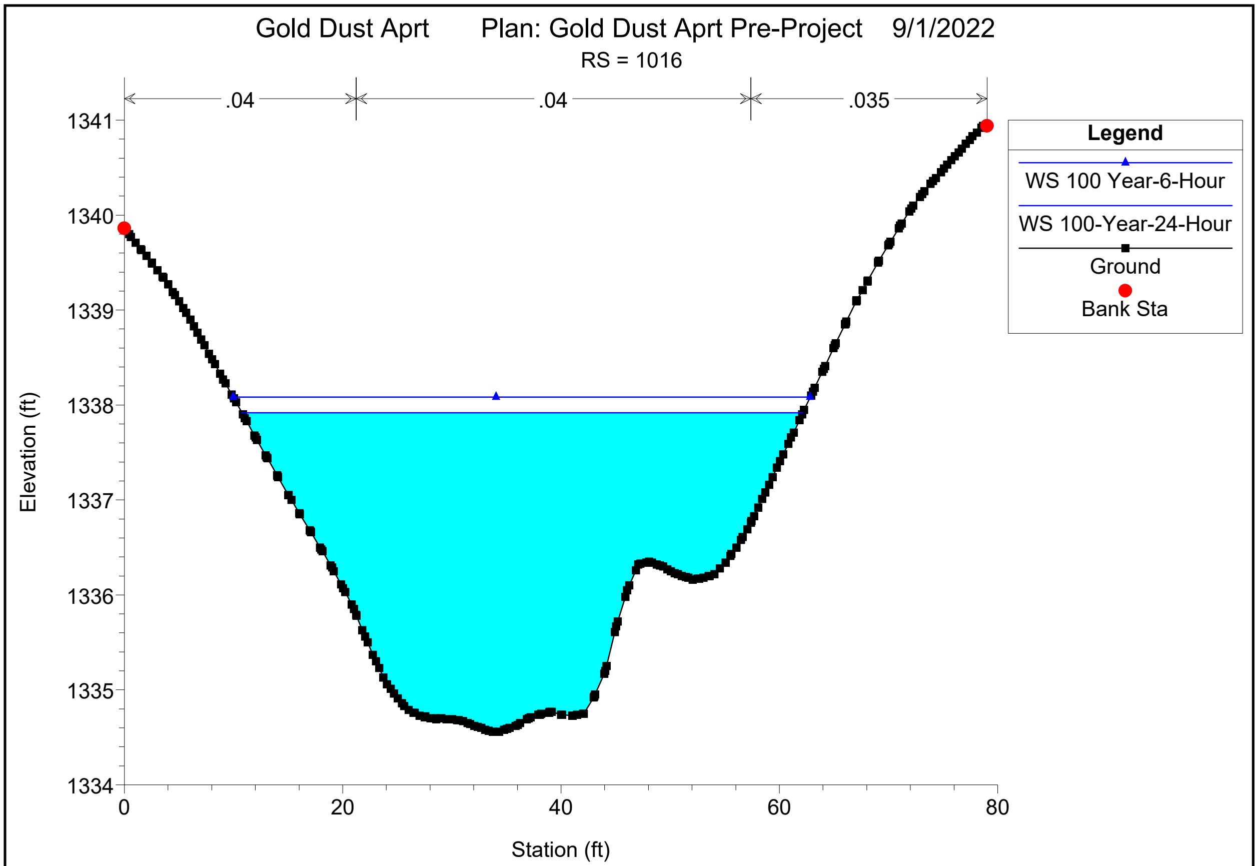

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1193
.05
.04
.019
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Ineff
Bank Sta
Elevation (ft)
1341
1340
1339
1338
1337
1336
1335
0
20
40
60
80
100
120
Station (ft)


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1154 Culv Private Driveway
.05
.04
.019
Elevation (ft)
1341
1340
1339
1338
1337
1336
1335
0
20
40
60
80
100
120
Station (ft)
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Ineff
Bank Sta

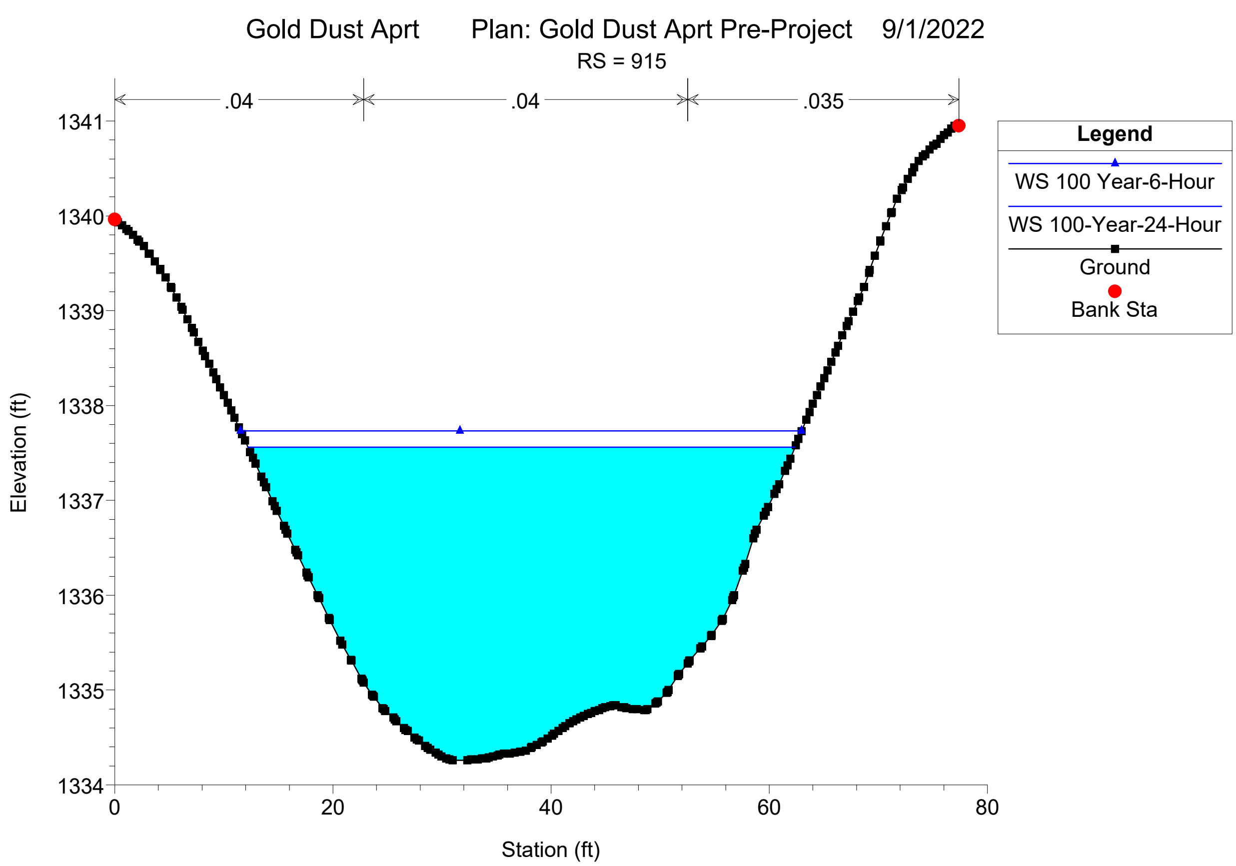

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1154 Culv Private Driveway
.019
.035
.019
Elevation (ft)
1341
1340
1339
1338
1337
1336
1335
1334
0
20
40
60
80
100
Station (ft)
Legend
WS 100-Year-24-Hour
WS 100 Year-6-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1118
.019
.035
.019
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
Station (ft)
1334
1335
1336
1337
1338
1339
1340
1341
0
20
40
60
80
100

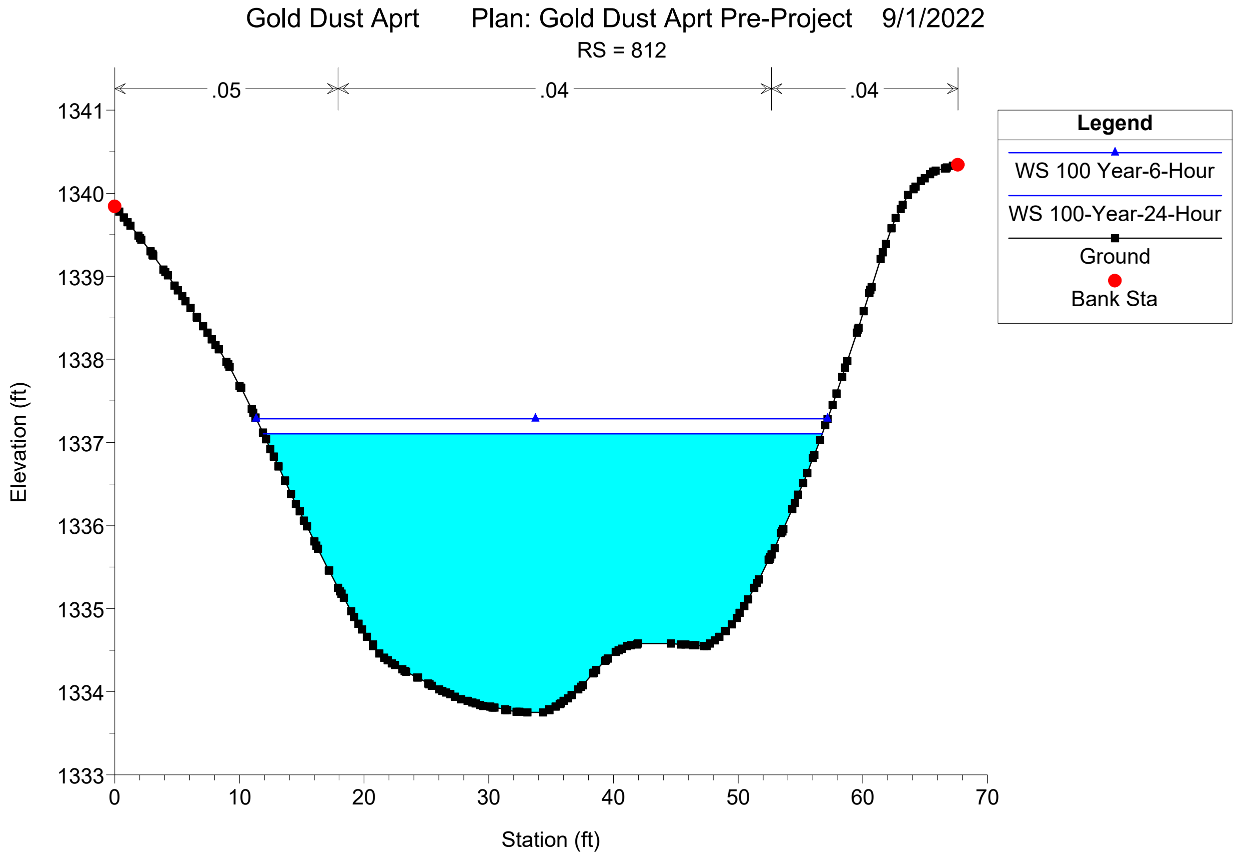

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1087
.019
.35
.035
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
Station (ft)
1335
1336
1337
1338
1339
1340
0
20
40
60
80
100

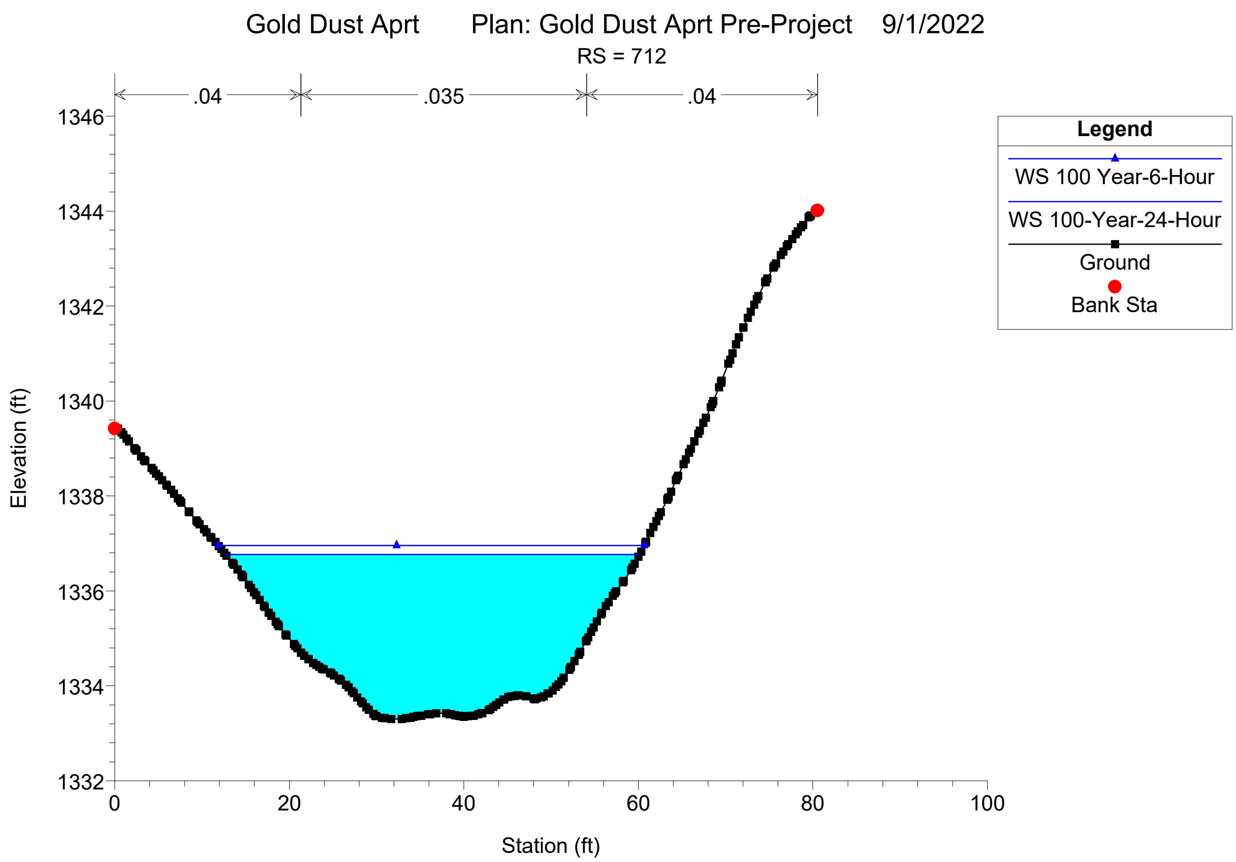


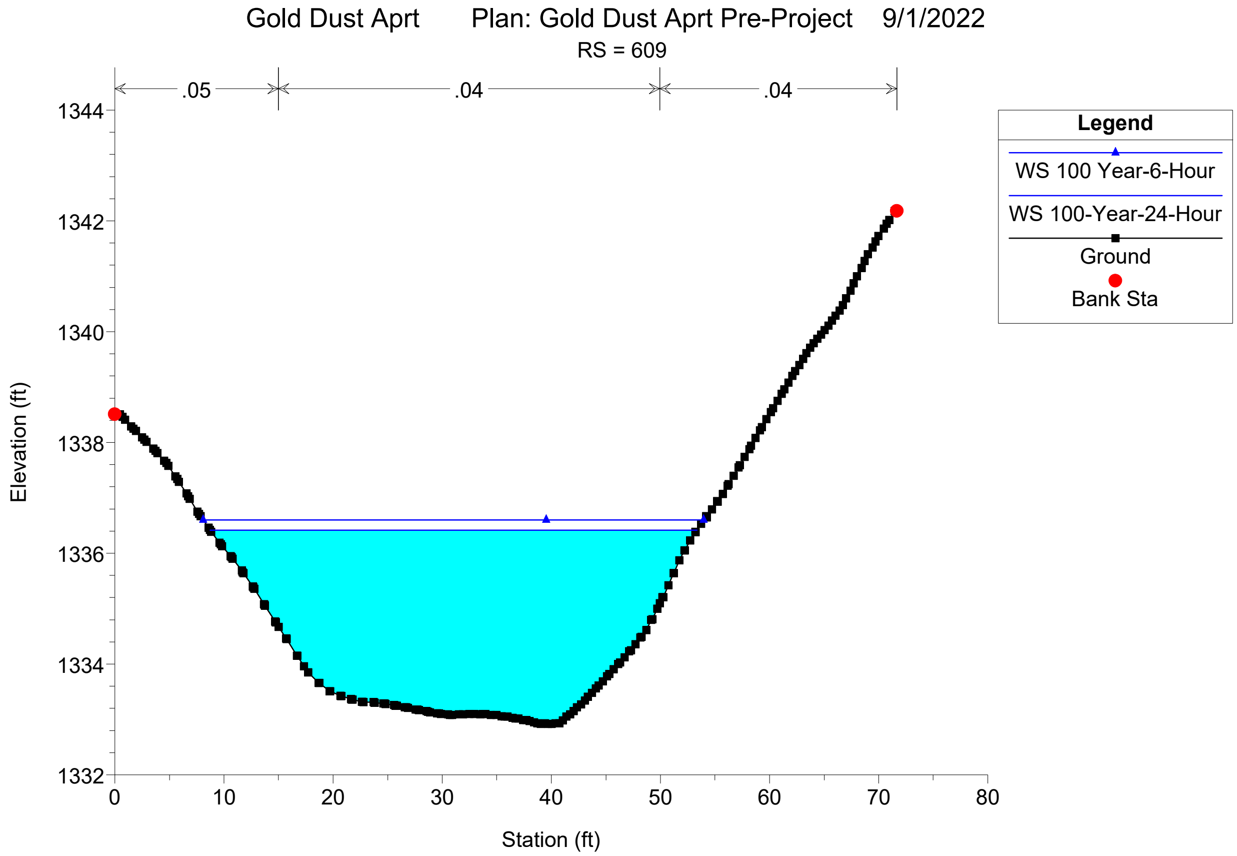
Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 1016
.04
.04
.035
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1341
1340
1339
1338
1337
1336
1335
1334
0
20
40
60
80
Station (ft)

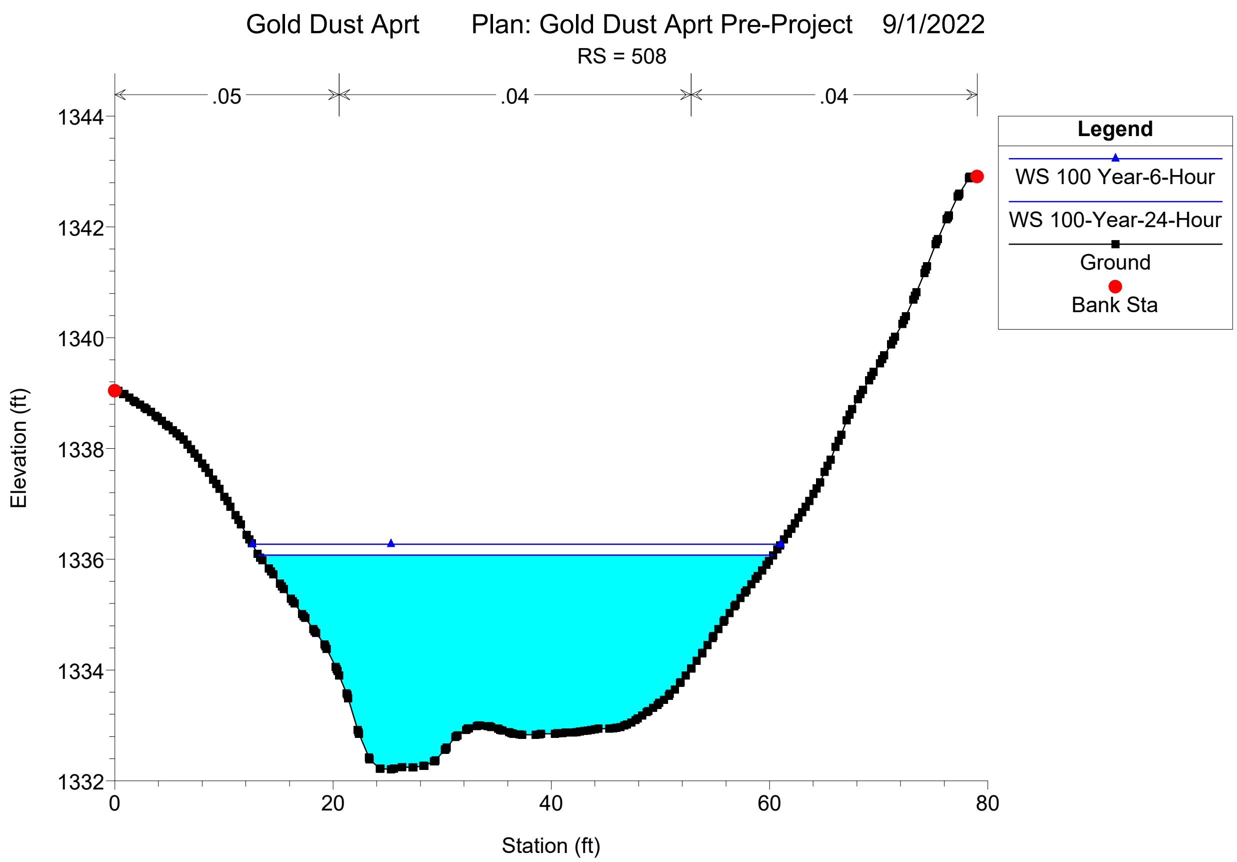


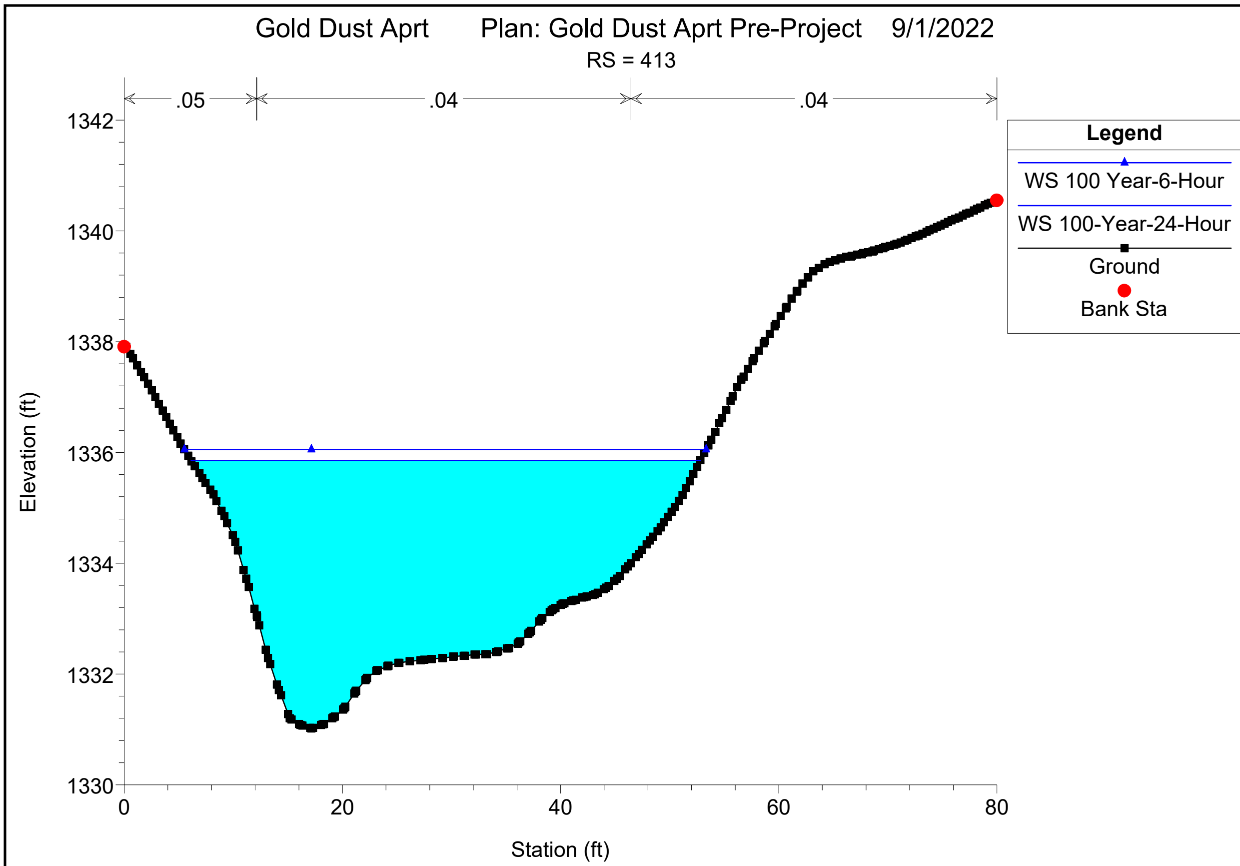
Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 915
.04
.04
.035
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1341
1340
1339
1338
1337
1336
1335
1334
0
20
40
60
80
Station (ft)

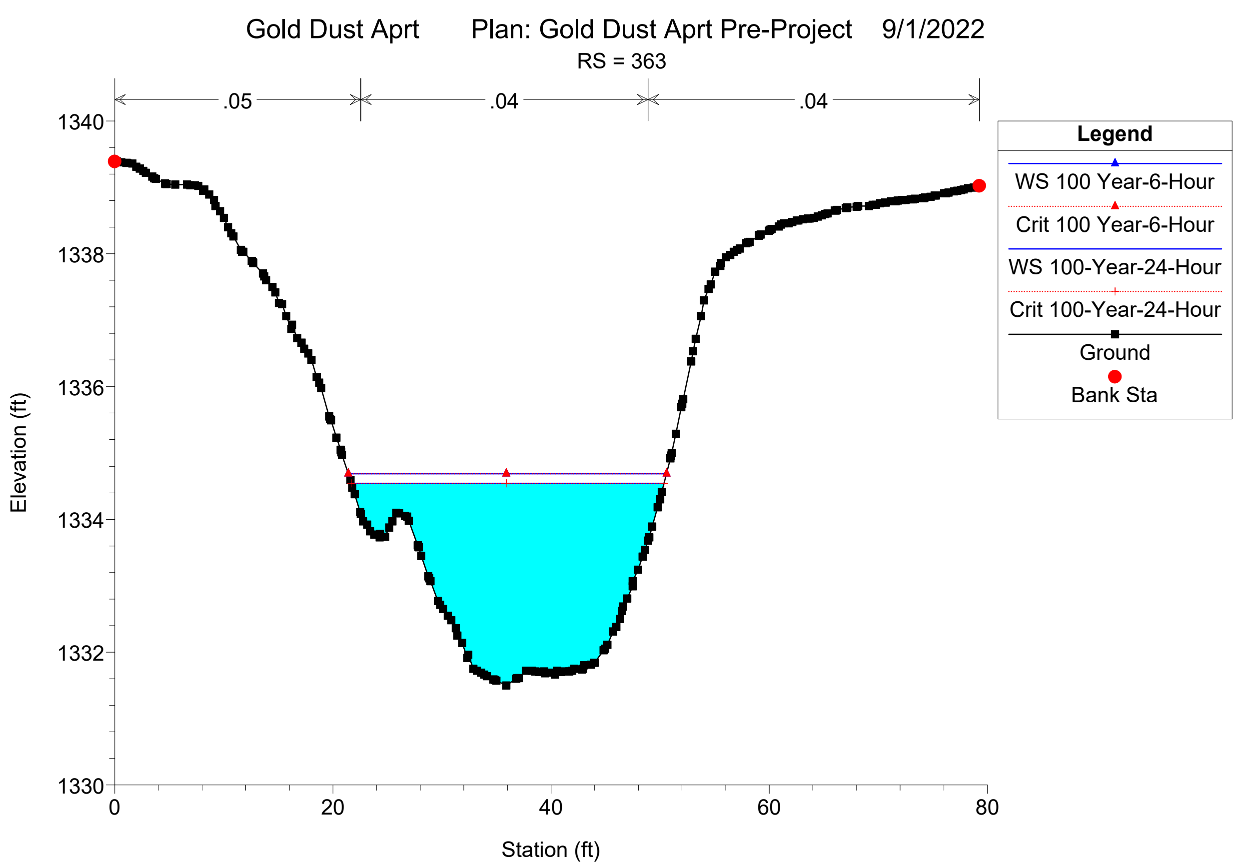

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 812
.05
.04
.04
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1341
1340
1339
1338
1337
1336
1335
1334
1333
0
10
20
30
40
50
60
70
Station (ft)

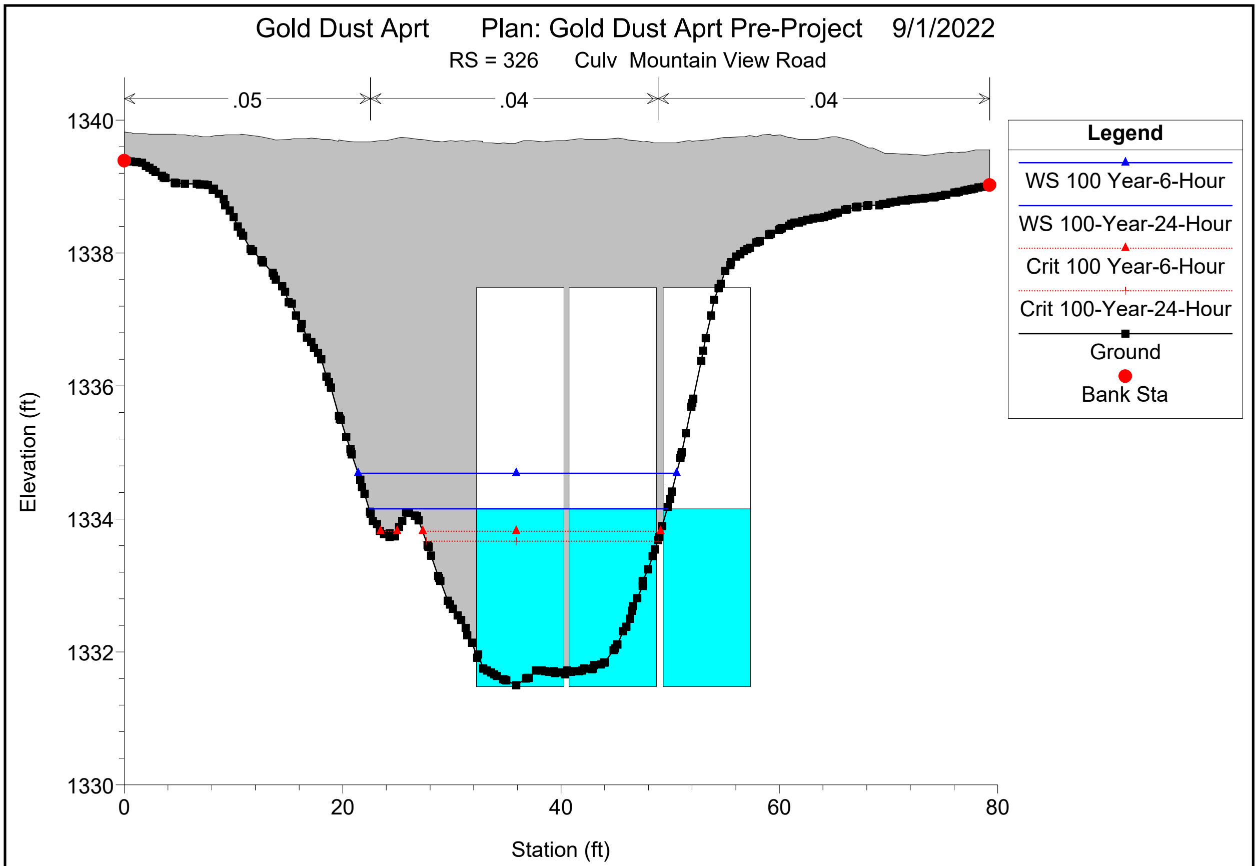

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 712
.04
.035
.04
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1346
1344
1342
1340
1338
1336
1334
1332
0
20
40
60
80
100
Station (ft)

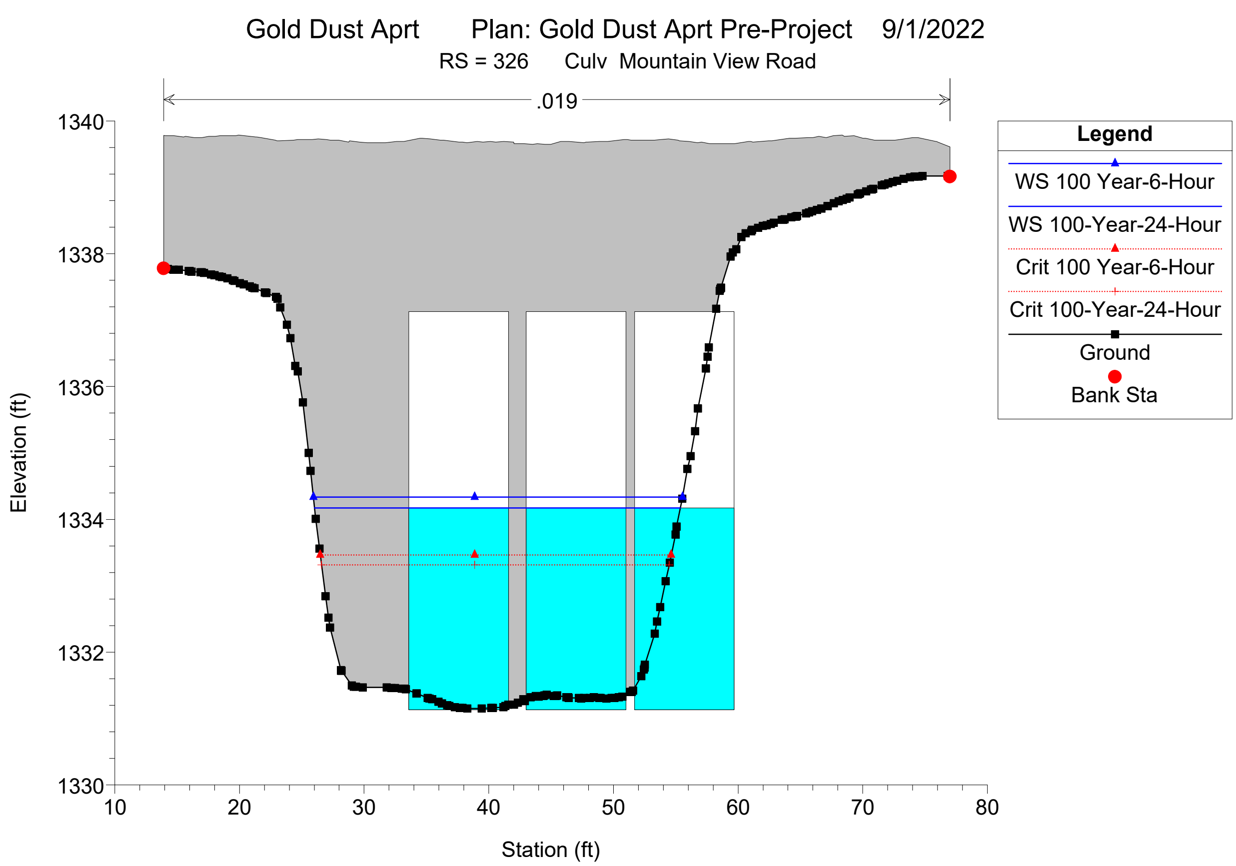

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 609
.05
.04
.04
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1344
1342
1340
1338
1336
1334
1332
0
10
20
30
40
50
60
70
80
Station (ft)

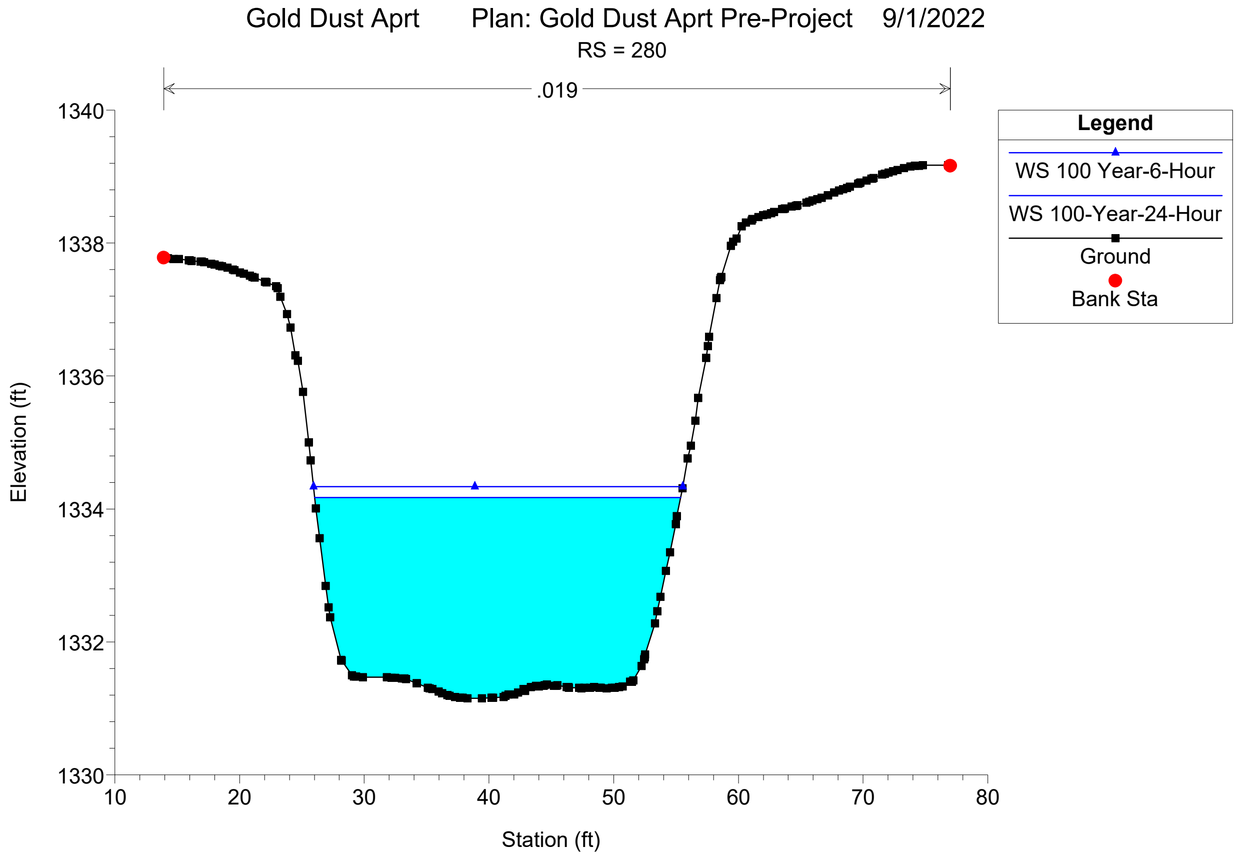

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 508
.05
.04
.04
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1344
1342
1340
1338
1336
1334
1332
0
20
40
60
80
Station (ft)


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 413
.05
.04
.04
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1330
1332
1334
1336
1338
1340
1342
0
20
40
60
80
Station (ft)

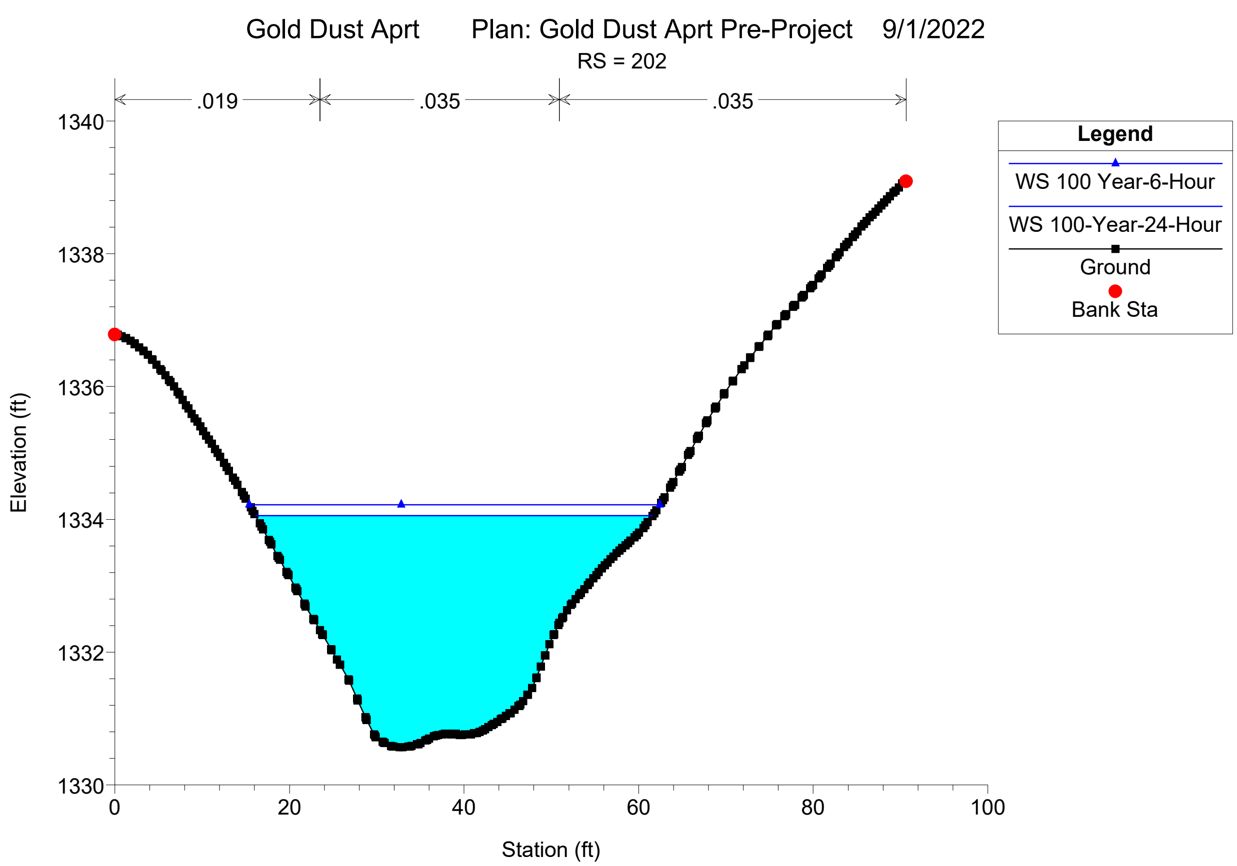

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 363
.05
.04
.04
Legend
WS 100 Year-6-Hour
Crit 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1330
1332
1334
1336
1338
1340
0
20
40
60
80
Station (ft)


Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 326 Culv Mountain View Road
.05
.04
.04
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1340
1338
1336
1334
1332
1330
0
20
40
60
80
Station (ft)

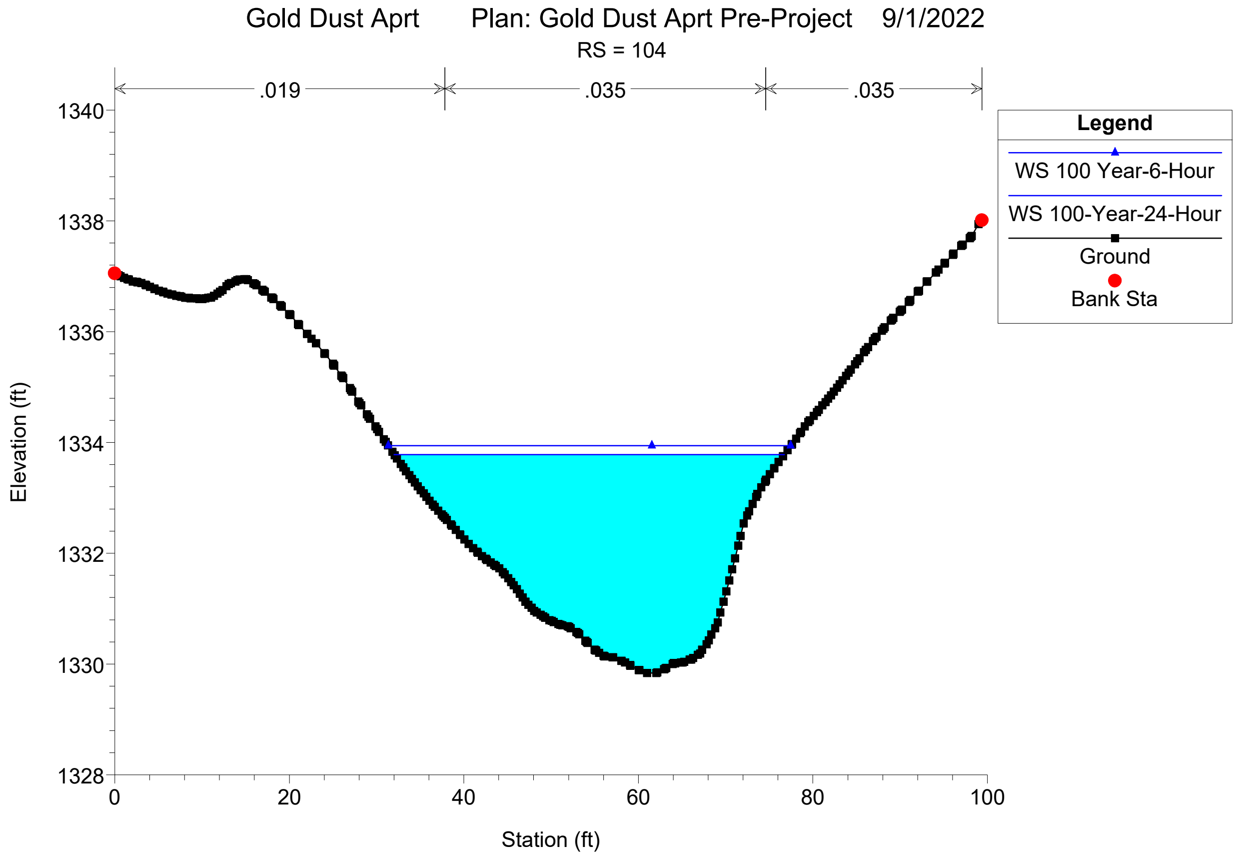

Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 326 Culv Mountain View Road
.019
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1340
1338
1336
1334
1332
1330
10
20
30
40
50
60
70
80
Station (ft)



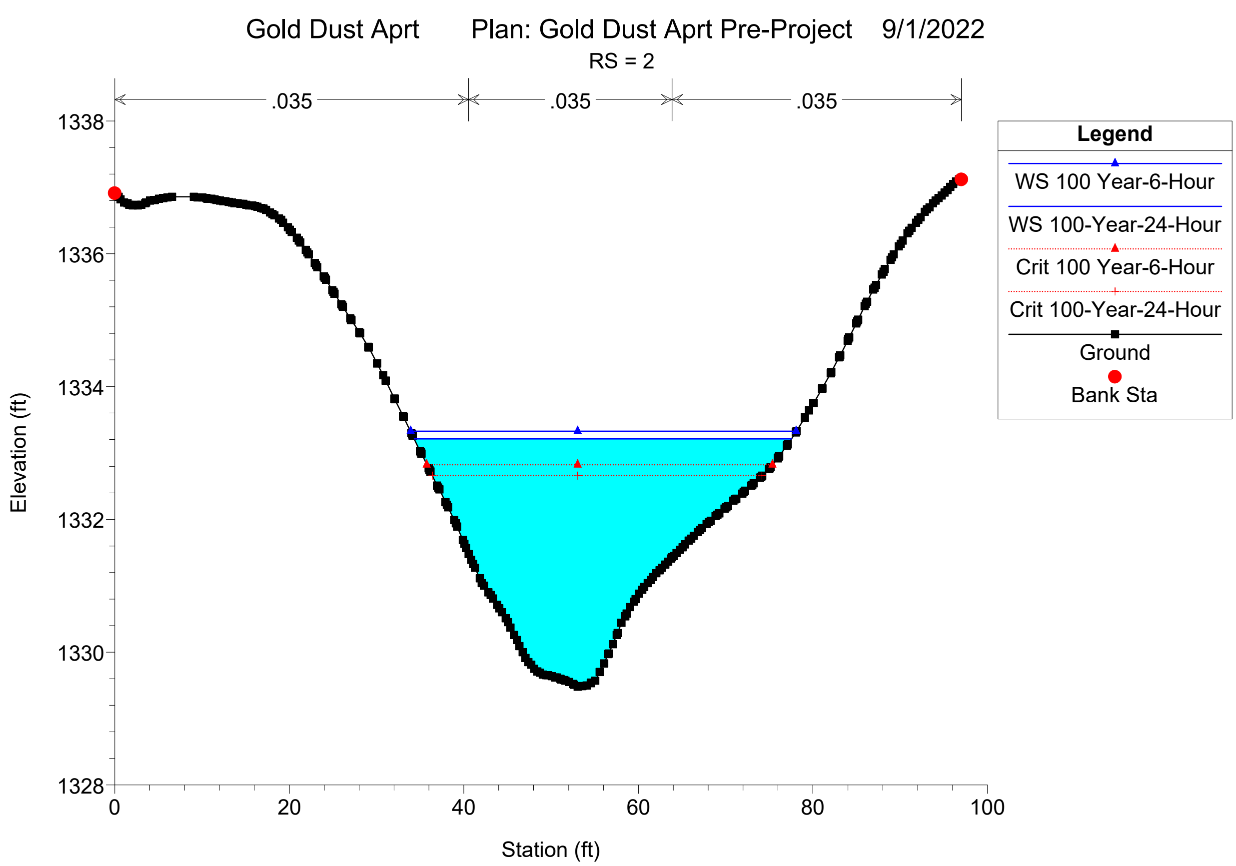
Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 280
.019
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
Station (ft)



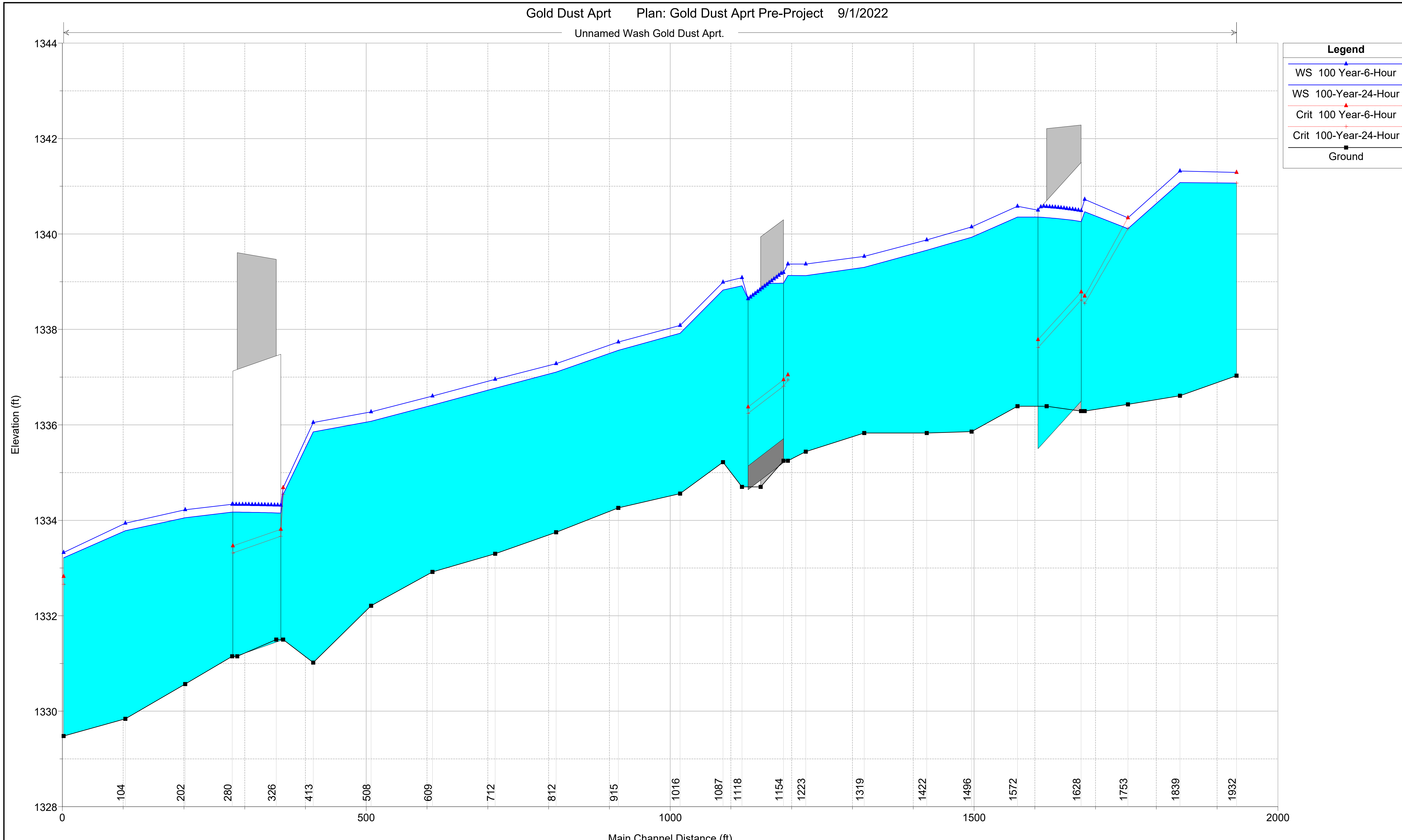
Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 202
.019
.035
.035
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
Station (ft)



Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 104
.019
.035
.035
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1328
1330
1332
1334
1336
1338
1340
0
20
40
60
80
100
Station (ft)



Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
RS = 2
.035
.035
.035
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Bank Sta
Elevation (ft)
1328
1330
1332
1334
1336
1338
0
20
40
60
80
100
Station (ft)



Gold Dust Aprt Plan: Gold Dust Aprt Pre-Project 9/1/2022
Unnamed Wash Gold Dust Aprt.
Legend
WS 100 Year-6-Hour
WS 100-Year-24-Hour
Crit 100 Year-6-Hour
Crit 100-Year-24-Hour
Ground
Elevation (ft)
Main Channel Distance (ft)
1328
1330
1332
1334
1336
1338
1340
1342
1344
0
500
1000
1500
2000
104
202
280
326
413
508
609
712
812
915
1016
1087
1118
1154
1223
1319
1422
1496
1572
1628
1753
1839
1932

HEC-RAS Plan: Gold Dust Aprt Pre-Project River: Unnamed Wash Reach: Gold Dust Aprt.

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Gold Dust Aprt.	1932	100-Year-24-Hour	420.00	1337.03	1341.07	1341.07	1342.27	0.004485	8.79	47.80	20.25	1.01
Gold Dust Aprt.	1932	100 Year-6-Hour	470.00	1337.03	1341.29	1341.29	1342.54	0.004416	8.97	52.42	21.29	1.01
Gold Dust Aprt.	1839	100-Year-24-Hour	420.00	1336.61	1341.08		1341.60	0.001441	5.78	72.73	24.75	0.59
Gold Dust Aprt.	1839	100 Year-6-Hour	470.00	1336.61	1341.32		1341.87	0.001450	5.96	78.84	25.66	0.60
Gold Dust Aprt.	1753	100-Year-24-Hour	420.00	1336.43	1340.11	1340.11	1341.33	0.004452	8.84	47.52	19.89	1.01
Gold Dust Aprt.	1753	100 Year-6-Hour	470.00	1336.43	1340.34	1340.34	1341.60	0.004325	9.02	52.13	20.67	1.00
Gold Dust Aprt.	1682	100-Year-24-Hour	420.00	1336.29	1340.47	1338.54	1340.66	0.000451	3.51	119.65	37.06	0.34
Gold Dust Aprt.	1682	100 Year-6-Hour	470.00	1336.29	1340.73	1338.70	1340.93	0.000447	3.63	129.33	37.49	0.34
Gold Dust Aprt.	1628		Culvert									
Gold Dust Aprt.	1572	100-Year-24-Hour	420.00	1336.39	1340.36		1340.53	0.001221	3.33	126.25	39.36	0.33
Gold Dust Aprt.	1572	100 Year-6-Hour	470.00	1336.39	1340.58		1340.77	0.001232	3.48	135.15	39.98	0.33
Gold Dust Aprt.	1496	100-Year-24-Hour	460.00	1335.86	1339.93		1340.30	0.004799	4.86	94.61	39.88	0.56
Gold Dust Aprt.	1496	100 Year-6-Hour	518.00	1335.86	1340.15		1340.54	0.004845	5.01	103.41	42.38	0.57
Gold Dust Aprt.	1422	100-Year-24-Hour	460.00	1335.83	1339.66		1339.94	0.004353	4.22	108.98	43.95	0.47
Gold Dust Aprt.	1422	100 Year-6-Hour	518.00	1335.83	1339.88		1340.17	0.004312	4.37	118.61	45.11	0.47
Gold Dust Aprt.	1319	100-Year-24-Hour	460.00	1335.83	1339.30		1339.52	0.003524	3.72	123.51	53.19	0.43
Gold Dust Aprt.	1319	100 Year-6-Hour	518.00	1335.83	1339.53		1339.76	0.003419	3.81	136.03	55.14	0.43
Gold Dust Aprt.	1223	100-Year-24-Hour	460.00	1335.44	1339.13		1339.26	0.001741	2.93	157.05	65.27	0.33
Gold Dust Aprt.	1223	100 Year-6-Hour	518.00	1335.44	1339.37		1339.51	0.001667	2.99	172.99	66.29	0.33
Gold Dust Aprt.	1193	100-Year-24-Hour	460.00	1335.25	1339.13	1336.94	1339.20	0.000639	2.14	214.93	68.02	0.21
Gold Dust Aprt.	1193	100 Year-6-Hour	518.00	1335.25	1339.37	1337.05	1339.45	0.000636	2.24	231.26	68.63	0.21
Gold Dust Aprt.	1154		Culvert									
Gold Dust Aprt.	1118	100-Year-24-Hour	460.00	1334.70	1338.92		1338.98	0.000436	1.98	232.89	80.40	0.20
Gold Dust Aprt.	1118	100 Year-6-Hour	518.00	1334.70	1339.09		1339.15	0.000460	2.10	246.73	81.52	0.21
Gold Dust Aprt.	1087	100-Year-24-Hour	460.00	1335.22	1338.83		1338.92	0.059473	2.45	187.61	68.45	0.26
Gold Dust Aprt.	1087	100 Year-6-Hour	518.00	1335.22	1338.99		1339.09	0.062155	2.60	198.88	69.91	0.27
Gold Dust Aprt.	1016	100-Year-24-Hour	440.00	1334.56	1337.92		1338.18	0.003942	4.10	107.43	51.38	0.50
Gold Dust Aprt.	1016	100 Year-6-Hour	485.00	1334.56	1338.08		1338.35	0.003834	4.18	115.95	52.85	0.50
Gold Dust Aprt.	915	100-Year-24-Hour	440.00	1334.26	1337.56		1337.79	0.003515	3.86	113.86	50.15	0.45
Gold Dust Aprt.	915	100 Year-6-Hour	485.00	1334.26	1337.73		1337.98	0.003448	3.96	122.63	51.46	0.45
Gold Dust Aprt.	812	100-Year-24-Hour	440.00	1333.75	1337.11		1337.38	0.004566	4.21	104.58	44.83	0.49
Gold Dust Aprt.	812	100 Year-6-Hour	485.00	1333.75	1337.28		1337.57	0.004487	4.31	112.64	45.83	0.48
Gold Dust Aprt.	712	100-Year-24-Hour	440.00	1333.30	1336.77		1337.00	0.002967	3.89	112.97	47.48	0.45
Gold Dust Aprt.	712	100 Year-6-Hour	485.00	1333.30	1336.96		1337.20	0.002917	3.98	121.93	48.81	0.44
Gold Dust Aprt.	609	100-Year-24-Hour	440.00	1332.92	1336.41		1336.66	0.003776	3.98	110.68	44.60	0.44
Gold Dust Aprt.	609	100 Year-6-Hour	485.00	1332.92	1336.60		1336.86	0.003740	4.07	119.22	45.88	0.44
Gold Dust Aprt.	508	100-Year-24-Hour	440.00	1332.21	1336.08		1336.29	0.003331	3.73	117.99	47.18	0.42
Gold Dust Aprt.	508	100 Year-6-Hour	485.00	1332.21	1336.27		1336.50	0.003264	3.81	127.36	48.43	0.41
Gold Dust Aprt.	413	100-Year-24-Hour	440.00	1331.02	1335.85		1336.02	0.002207	3.31	133.11	46.67	0.35
Gold Dust Aprt.	413	100 Year-6-Hour	485.00	1331.02	1336.05		1336.23	0.002225	3.41	142.39	47.82	0.35
Gold Dust Aprt.	363	100-Year-24-Hour	440.00	1331.50	1334.54	1334.54	1335.53	0.020865	7.97	55.23	28.69	1.01
Gold Dust Aprt.	363	100 Year-6-Hour	485.00	1331.50	1334.69	1334.69	1335.72	0.020559	8.17	59.34	29.18	1.01
Gold Dust Aprt.	326		Culvert									
Gold Dust Aprt.	280	100-Year-24-Hour	440.00	1331.15	1334.17		1334.70	0.001704	5.80	75.90	29.35	0.64
Gold Dust Aprt.	280	100 Year-6-Hour	485.00	1331.15	1334.34		1334.90	0.001718	6.01	80.70	29.62	0.64
Gold Dust Aprt.	202	100-Year-24-Hour	470.00	1330.57	1334.05		1334.43	0.003746	4.94	95.12	45.49	0.60
Gold Dust Aprt.	202	100 Year-6-Hour	526.00	1330.57	1334.22		1334.63	0.003769	5.12	102.82	47.04	0.61
Gold Dust Aprt.	104	100-Year-24-Hour	470.00	1329.84	1333.78		1334.08	0.002946	4.42	106.32	44.71	0.51
Gold Dust Aprt.	104	100 Year-6-Hour	526.00	1329.84	1333.94		1334.27	0.003020	4.63	113.58	46.12	0.52
Gold Dust Aprt.	2	100-Year-24-Hour	470.00	1329.48	1333.21	1332.66	1333.68	0.005215	5.47	85.96	43.24	0.68
Gold Dust Aprt.	2	100 Year-6-Hour	526.00	1329.48	1333.33	1332.82	1333.85	0.005621	5.79	90.86	44.19	0.71

Appendix H – SCOUR AND LATERAL MIGRATION POTENTIAL CALCULATIONS

Channel Riprap Sizing

FCDMC Drainage Desing Manual for Maricopa County - Hydraulics 2018

6.6.3 Riprap Lined Channels

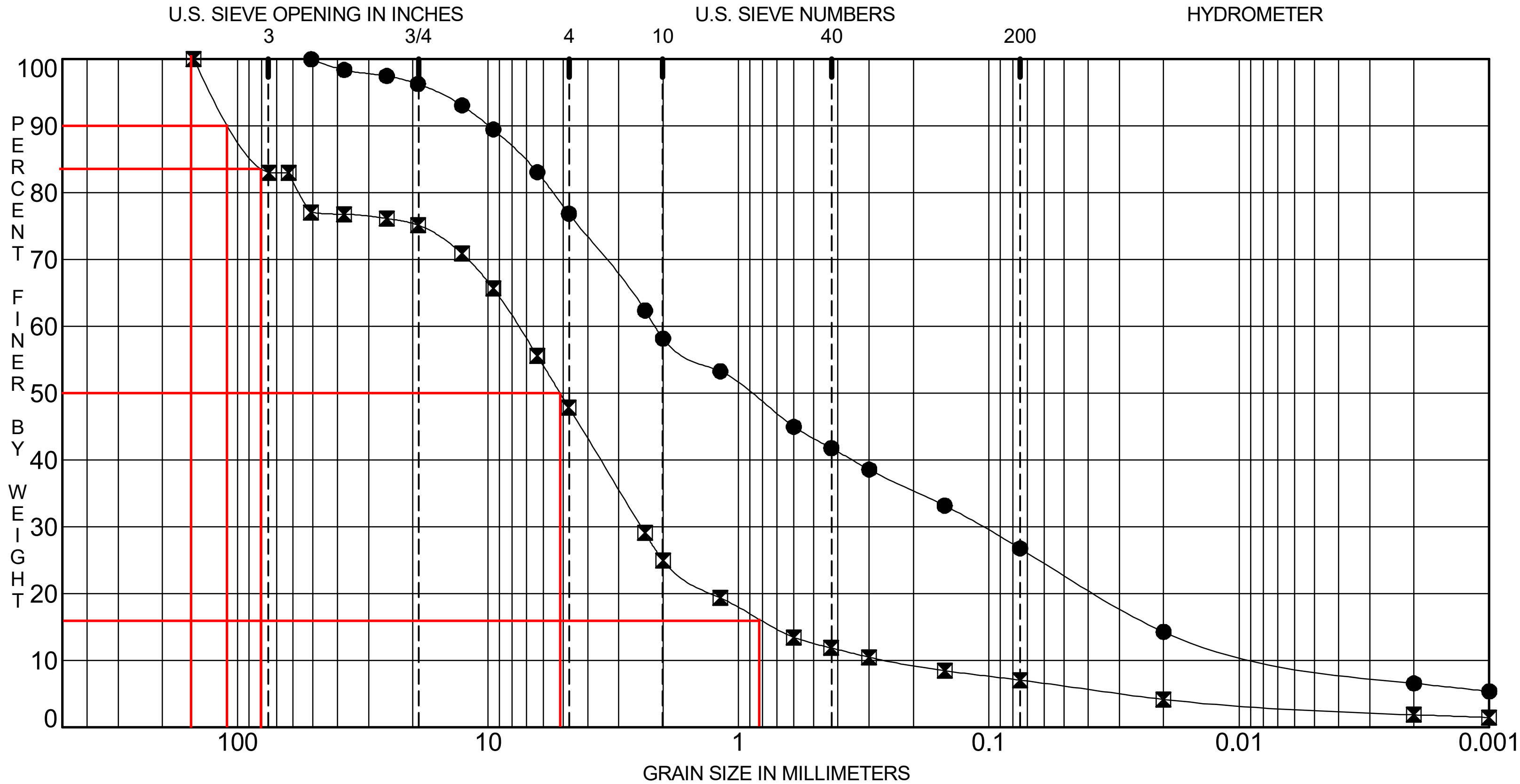
Channel Bank, Loose Angular Riprap Sizing (d50)

Input Parameters:

Vm, Maximum Channel Velocity	5.0	ft/s	(Max Vel from HEC-RAS Flow Dist Analysis is Entered Here)
$\gamma_{s,}$ Spec Weight Stone	156		
φ, Bank Angle (from Horizontal)	18.4	Degrees	
C, Turbulence Coefficent	1.2	(Use 1.2 for low turbulence, 0.86 for high turbulence)	
Minimum D_{50}	**0.19**	ft	Use 4" Rock

Calculated By: **JEP** Date: **1/30/2023**

GRADATION CURVES



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

	Specimen Identification		ASTM Classification		LL	PL	PI	%Gravel	%Sand	%Fines
BANK ●	TP-1	0.0	CLAYEY SAND with GRAVEL	SC	40	20	20	23.1	50.1	26.8
BED ⊠	TP-2	0.0	POORLY GRADED SAND with CLAY and GRAVEL	SP-SC	40	22	19	35.1	40.8	7.1

Sheet 1 of 1

Gold Dust MFR - Wash Sampling
10050 North Scottsdale Road
Scottsdale, Arizona
Project No. 212296SB

SPEEDIE AND ASSOCIATES

GEOTECH GRADATION CURVES 212296SB.GPJ GENGEO.GDT 9/1/22

Gold Dust Apartments - Unnamed Wash Scour Summary

Dibble

Channel Scour

HEC-RAS Cross Section	General Scour[1] (ft)	Bend Scour[2] (ft)	Bedform Scour[3] (ft)	Long Term Scour[4] (ft)	Low Flow Thalweg[5] (ft)	Local Scour[6] (ft)	Safety Factor[7]	Total Scour Depth[8] (ft)
1496	1.7	0.0	0.1	0.0	0.0	0.0	1.3	2.3
1422	1.7	0.0	0.1	0.0	0.0	0.0	1.3	2.3
1319	1.7	0.0	0.1	0.0	0.0	0.0	1.3	2.3
1223	1.7	0.0	0.1	0.0	0.0	0.0	1.3	2.3

[1]General scour calcuated using Blench Equation, see attached.

[2]A moderate bend is included in the General Scour computation using Blench Equation

[3]Bedform Scour per USBR Dune and Anti-Dune Scour Height Calculations, see attached.

[4]Longterm Scour is set to zero due to immediate downstream control.

[5]Low Flow Scour is appropriate for constructed channels; scour depth is applied to the lowest point in each natural wash cross section, so Low Flow Scour has

[6]Local Control Scour depth - there are no local scour sources in the subject reach.

[7]A safety factor equal to 1.3 per Flood Control District of Maricopa County Drainage Design Manual - Hydraulics.

[8]Total Scour Depth is equal to the sum of General, Bend, Bedform, Long Term, Low Flow, and Local multiplied by the Safety Factor. Depth is applied to lowets

Soils Data:

Sediment Gradation	
D (mm)	% Finer
170.00	100
110.00	90
80.00	84
5.00	50
0.80	16

Calculated By: JEP

Date: 1.30.2023

General Scour Calculations - USBR

* Methodology from US Bureau of Reclamation, "Computing Degradation and Local Scour", 1984 *

Blench Equation:

$$d_s = Z \frac{q_f^{2/3}}{F_{bo}^{1/3}}$$

Lacey Equation:

$$d_s = 0.47Z\left(\frac{Q}{1.76D_m^{1/2}}\right)^{1/3}$$

Competent Velocity Equation:

$$d_s = d_m\left(\frac{V_m}{V_c} - 1\right)$$

Where:

ds =	Scour Depth Below Streambed (ft)
Z =	Regime Modifier (See Table Below)
	* Moderate bend assumed *
qf =	Unit Discharge (Q/Top Width)
Fbo =	Blench's "zero bed factor" (ft/s2)
Q =	Design Discharge (cfs)
Dm =	Mean Grain Size of Bed Material (mm)
Vm =	Mean Channel Velocity (ft/s)
Vc =	Channel Competent Velocity (ft/s)
dm =	Mean Depth (ft)

Input Parameters

Dm	5.00	mm
[4]Longterm Scour is set to ze	518	cfs
Z	0.6	
[6]Local Control Scour depth	2.70	ft/s2
Top Width	69	ft
Vc	-	ft/s
Vm	-	ft/s
dm	-	ft

Scour Depths (from Channel Bottom)

Blench:	1.66	ft	1
Lacey:	N/A	ft	2
Competent Vel:	N/A	ft	3

1,2,3 | 1 | Blench Method selected over Lacey and Competent Velocity methods.

Regime Modifier (Z)

	Equation Name		
Equation Types A and B	Neill	Lacey	Blench
Straight Reach	0.5	0.25	0.6
Moderate bend	0.6	0.5	0.6
Severe Bend	0.7	0.75	0.6
Right angle bends		1	1.25
Vertical rock bank or wall		1.25	
Equation Types C and D			
Nose of piers	1		0.5-1.0
Nose of guide banks	0.4-0.7	1.5-1.75	1.0-1.75
Small dam or control across river		1.5	0.75-1.25

Dune and Anti-Dune Scour Height Calculations

* Methodology from US Bureau of Reclamation, "Computing Degradation and Local Scour", 1984 *

Dune Height Equation

dh= $0.066Yh^{1.21}$

Anti-Dune Height Equation

dh= $0.027V^2$

Where:

dh= Dune Height, in feet

Yh= Hydraulic depth of flow, in feet

V= Average Channel Velocity

Fr= Froude Number

Scour Component Equation

if Anti-Dune is controlling (Fr≥0.7):

ds= 0.5(Anti-Dune Height)

if Dune is controlling (Fr≤0.7):

ds= 0.5(Dune Height)

River Station	Yh ft	V ft/s	Fr	Controlling:	Dune Height ft	Anti-Dune Height ft	Scour Component ft
1496	2.440	5.01	0.57	Dune	0.1942	0.6777	0.1
1422	2.630	4.37	0.47	Dune	0.2127	0.5156	0.1
1319	2.470	3.81	0.43	Dune	0.1971	0.3919	0.1
1223	2.610	2.99	0.33	Dune	0.2107	0.2414	0.1

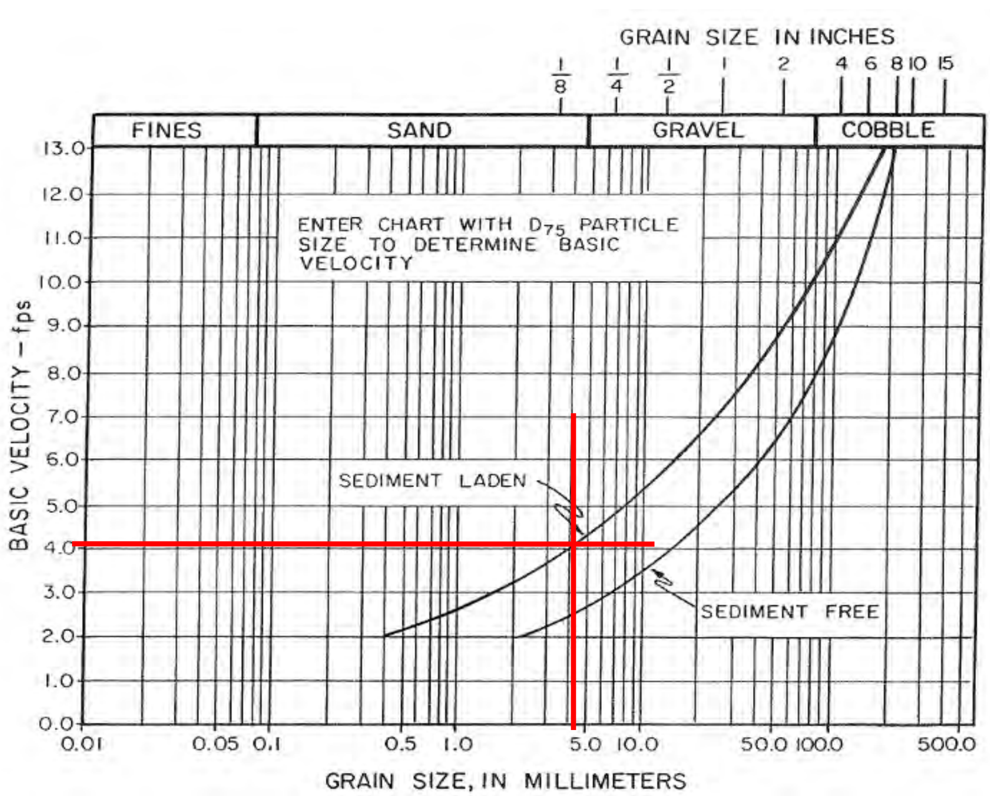
Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

Cross Section 1193 - General Information

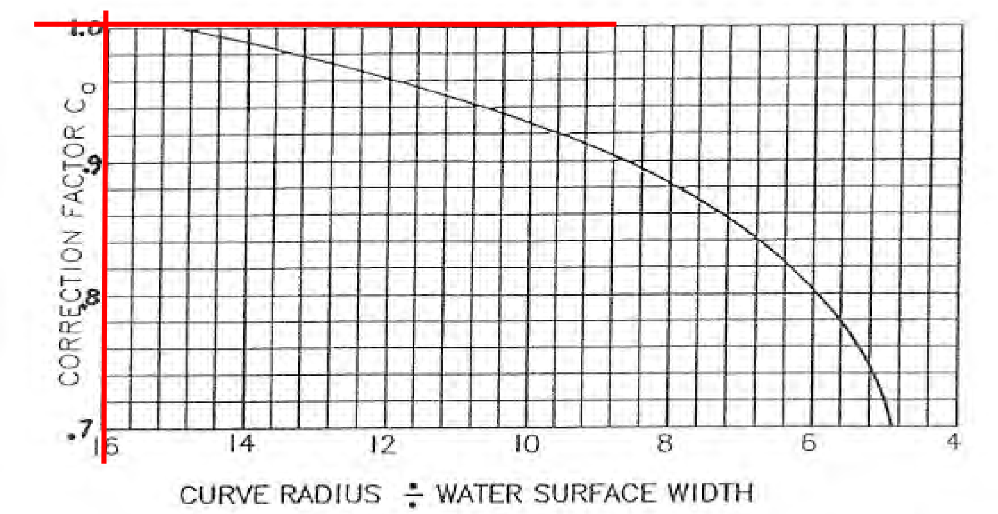
Bottom Width (b)	60 feet		
Side Slope (ft)	22.9 Horizontal	4.3 Vertical	
Channel Slope (S_e)	0.0004 feet/foot		
Radius of Curvature (r)	0 feet		
Water Surface Width	68.63 feet		
Average Manning's n	0.042		
Flow Depth (Y)	4.12 feet		
Flow Velocity (V)	2.24 feet/second		
D_{75}	4.4 mm	0.17 inches	0.014 feet
D_{65}	3.7 mm	0.15 inches	0.012 feet
D_{50}	0.9 mm	0.04 inches	0.003 feet

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 4.2 feet/sec

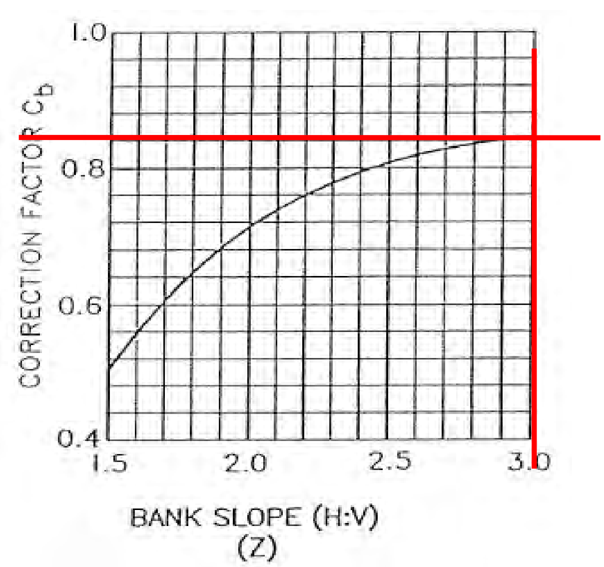


Correction Factor C_a For Channel Alignment 1



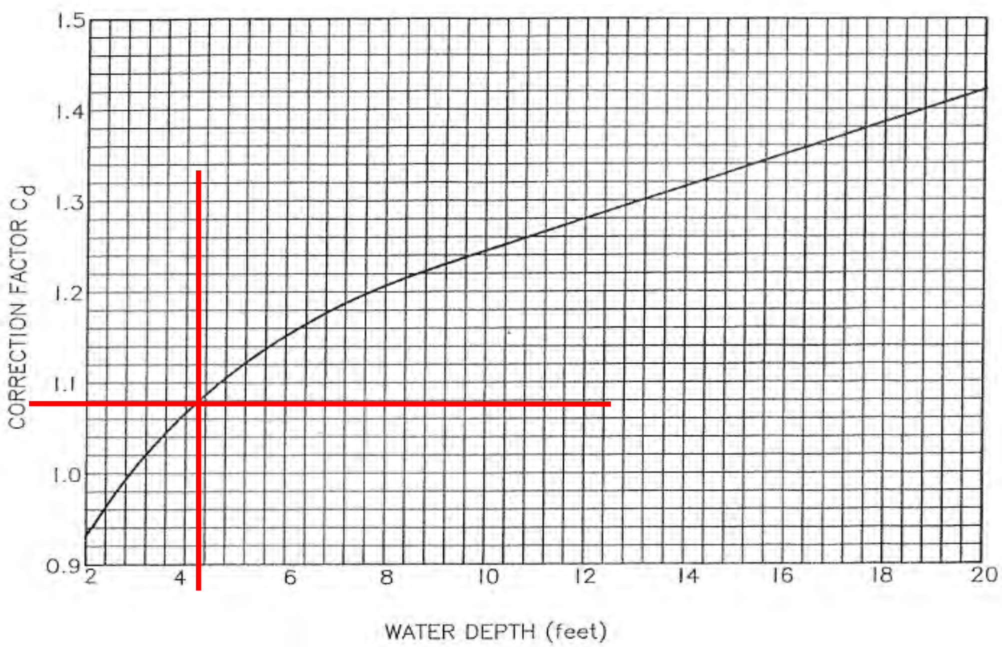
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.84



Horizontal/Vertical (Z) 5.33

Correction Factor C_d For Depth of Flow 1.08



Flow Depth (Y) 4.12 feet

Maximum Allowable Velocity 3.81 feet/second

$$V_a = V_b \; x \; C_a \; x \; C_b \; x \; C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	3.81	feet/second
Flow Velocity	2.24	feet/second

Since the computed flow velocity is less than the maximum allowable velocity, erosion is not expected to occur.

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75}	4.4 mm	conversion	0.17 inches

Since D75 is less or equal to 0.25 inches, case 2 of reference tractive stress method is used.

Assuming a water temperature of 60°F

Kinematic Velocity (ν)	0.0000121	ft^2 / sec
Density (ρ)	1.94	slugs/ft^3
Gravity	32.17	ft/sec^2

Finding Values Needed for Graphs

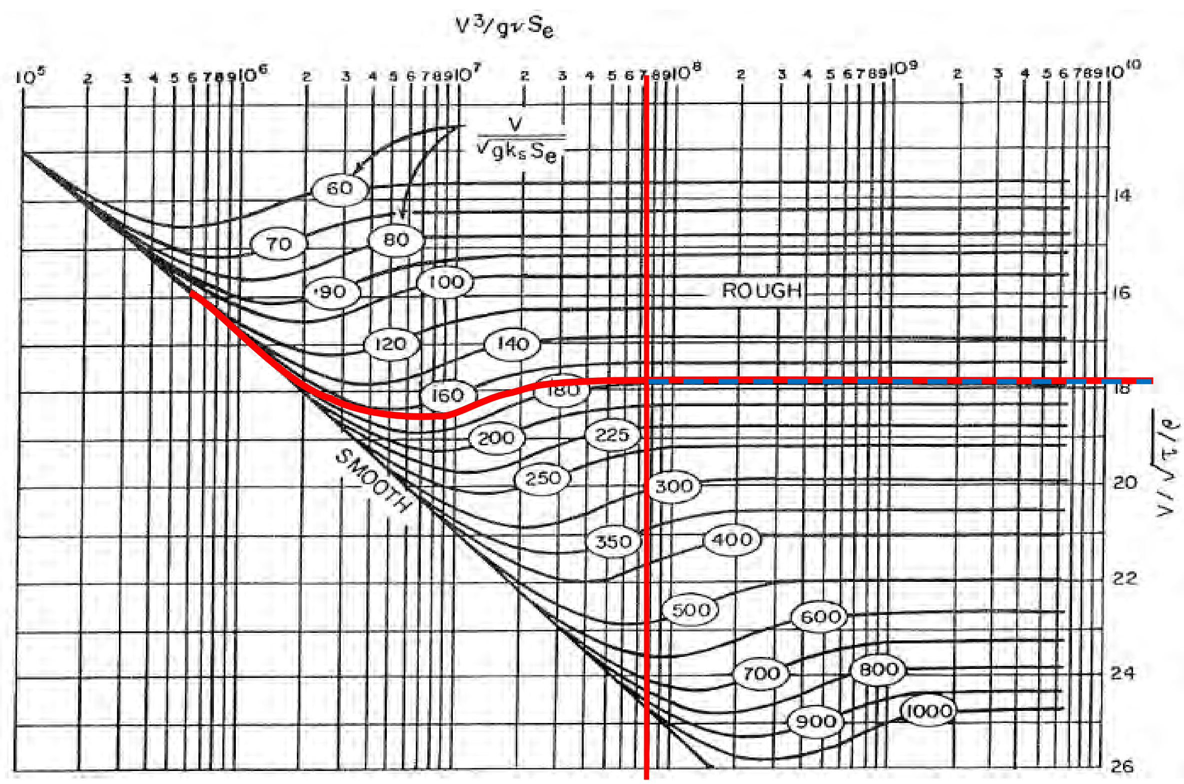
$$V^3/(gvS_e)$$

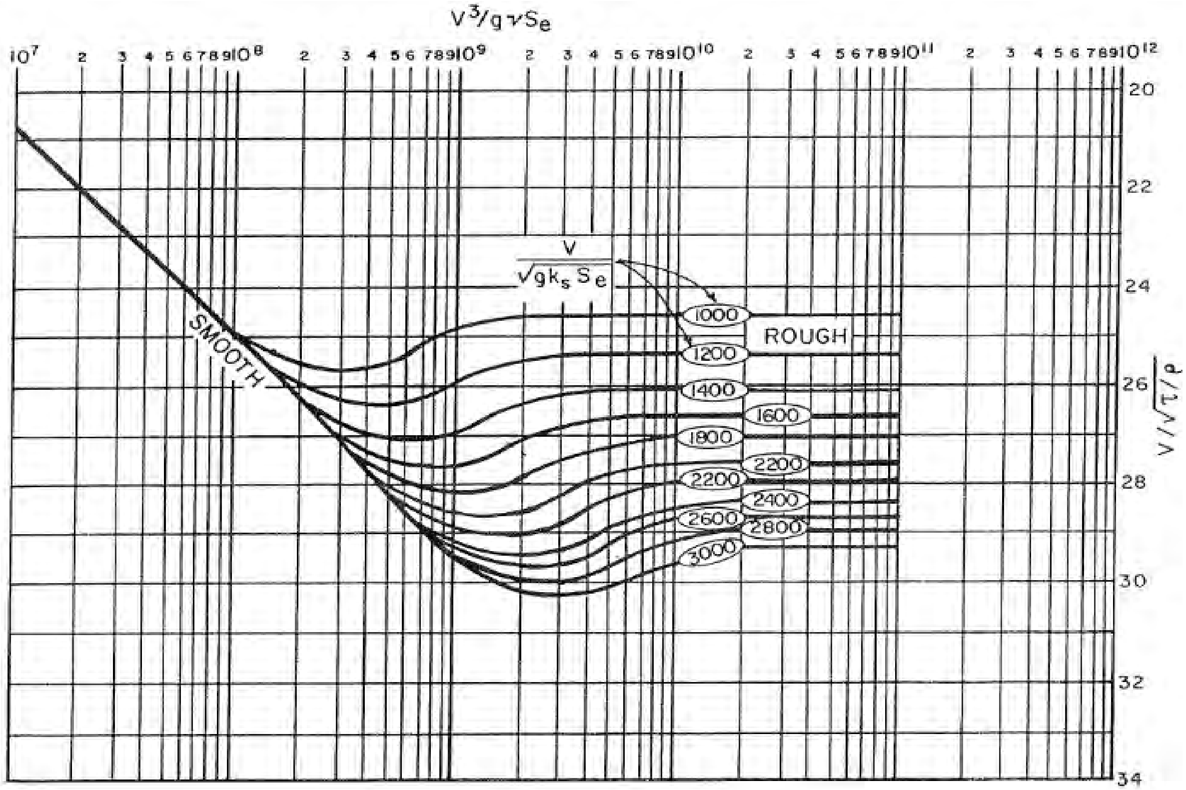
Value 1 7.22E+07

$$V/[(gD_{65}S_e)^{0.5}]$$

Value 2 179.2

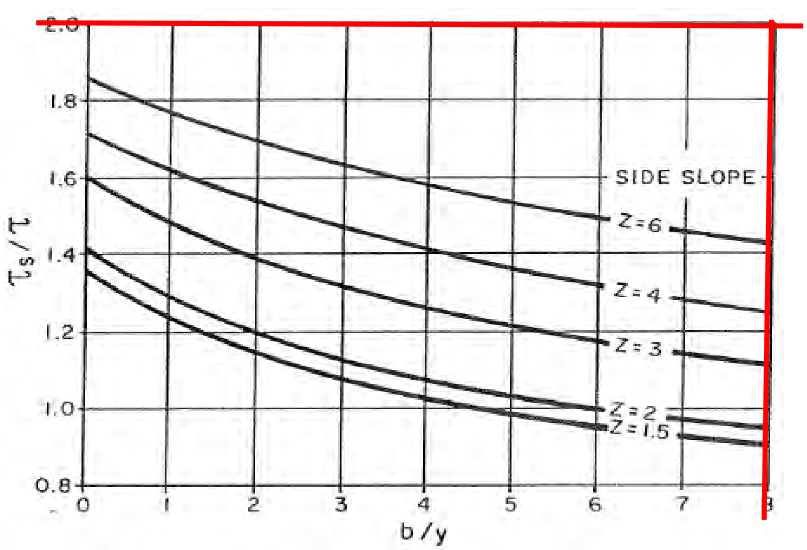
Graphic Solution of Reference Tractive Stress





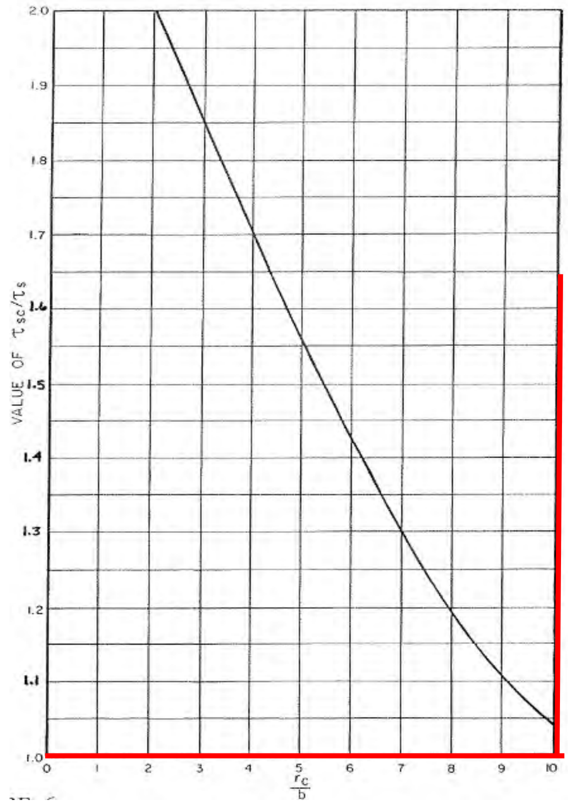
$V/\sqrt{\tau/\rho}$	From Graph Above	17.8
Solving the above equation for τ	0.03 lb/ft^2	

Applied Maximum Tractive Stress, τ_s on sides of straight trapezoidal channels



Bottom Width/Flow depth	14.56
Horizontal/Vertical (Z)	5.33
From Graph	2
Solving for τ_s	0.06 lb/ft^2

Actual Maximum Tractive Stress, τ_s on sides of trapezoidal channels within a curved reach

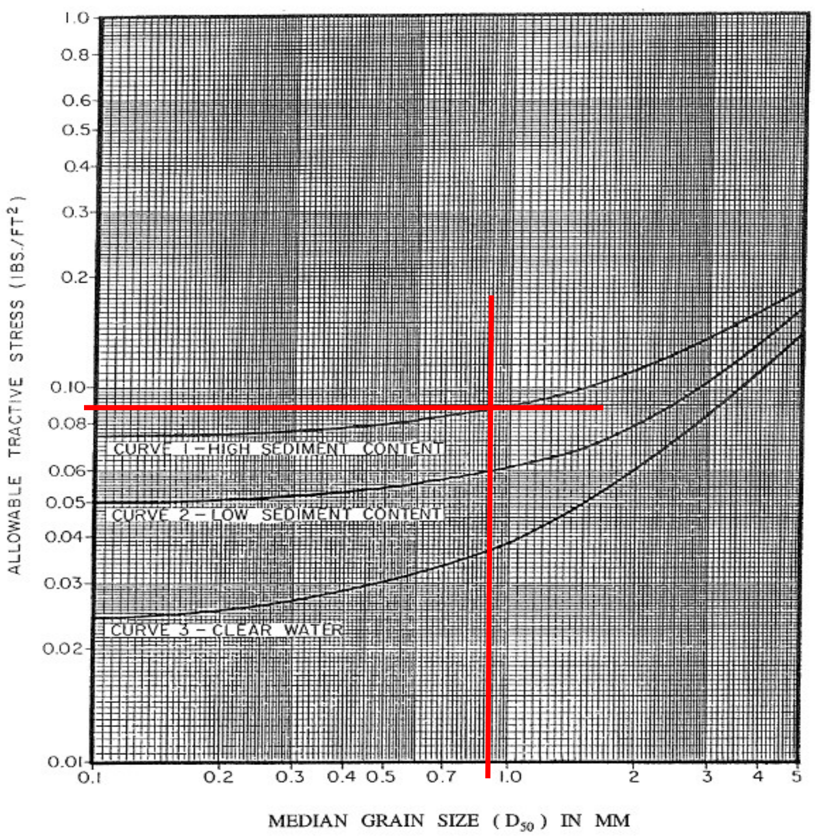


Radius of Curvature/Bottom Width N/A

From Graph Above 1.00

Solving for τ_s 0.06 lb/ft^2

Maximum Allowable Tractive Stress For Non-Cohesive Soils, D_{75} < 0.25"



MEDIAN GRAIN SIZE (D_{50}) IN MM

D_{50}	0.9 mm	0.04 inches	0.003 feet

Allowable Tractive Stress, from graph above 0.092 lb/ft^2

Calculated Tractive Stress, τ 0.03 lb/ft^2

Since the allowable tractive stress is more than the calculated tractive stress, the channel is not erosive.

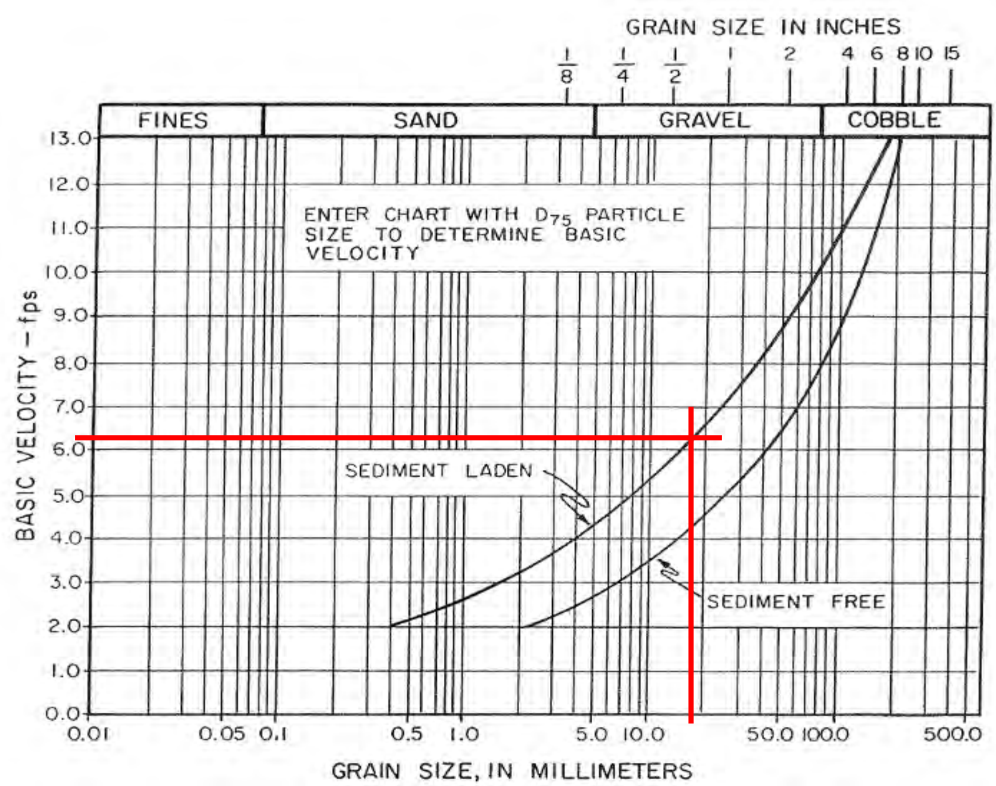
Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

Cross Section 1193 - General Information

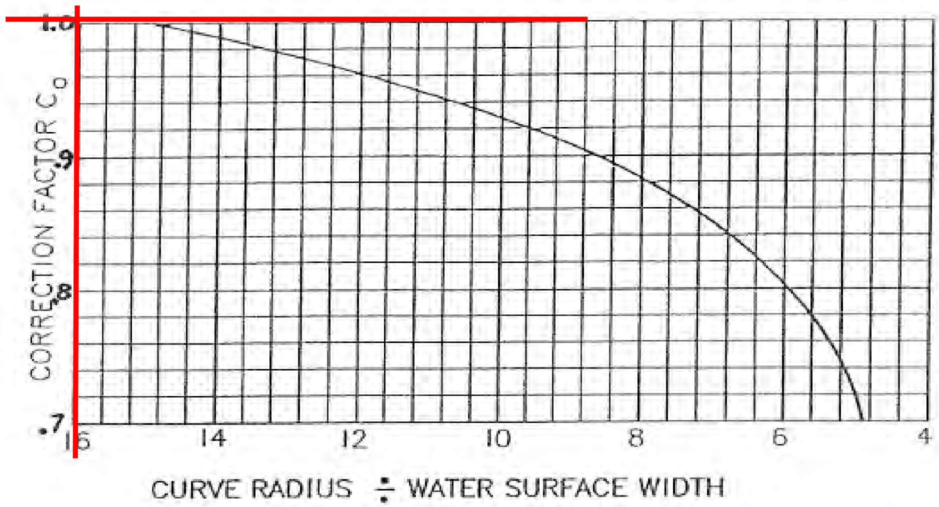
Bottom Width (b)	60 feet		
Side Slope (ft)	22.9 Horizontal	4.3 Vertical	
Channel Slope (S_e)	0.0004 feet/foot		
Radius of Curvature (r)	0 feet		
Water Surface Width	68.63 feet		
Average Manning's n	0.042		
Flow Depth (Y)	4.12 feet		
Flow Velocity (V)	2.24 feet/second		
D_{75}	19 mm	0.75 inches	0.062 feet
D_{65}	9.1 mm	0.36 inches	0.030 feet
D_{50}	5.2 mm	0.20 inches	0.017 feet

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 6.3 feet/sec

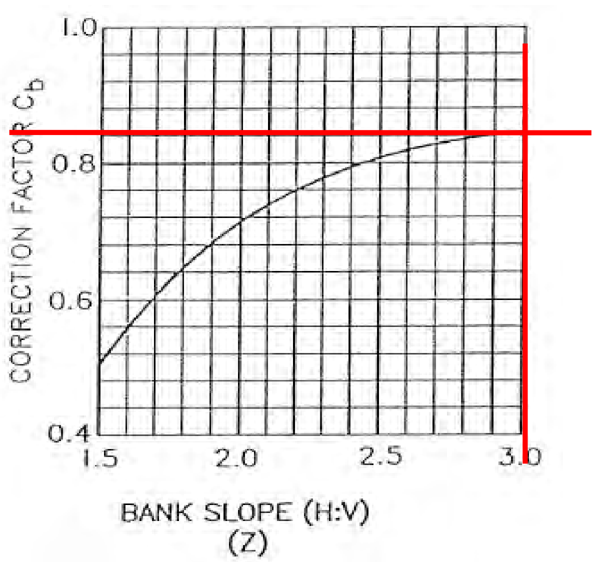


Correction Factor C_a For Channel Alignment 1



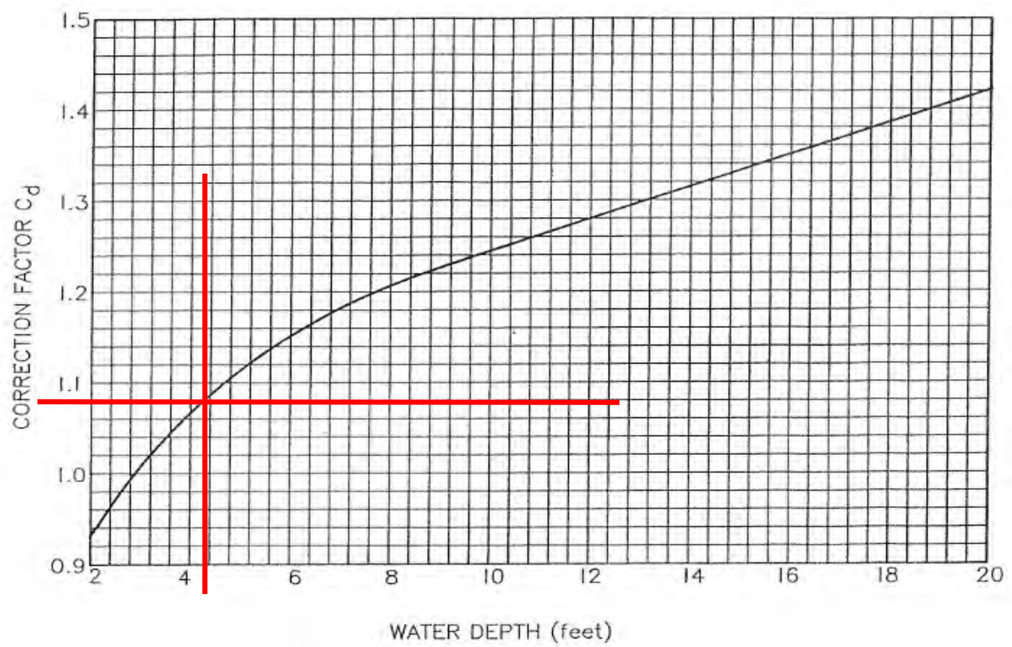
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.84



Horizontal/Vertical (Z) 5.33

Correction Factor C_d For Depth of Flow 1.08



Flow Depth (Y) 4.12 feet

Maximum Allowable Velocity 5.72 feet/second

$$V_a = V_b \ x \ C_a \ x \ C_b \ x \ C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	5.72	feet/second
Flow Velocity	2.24	feet/second

Since the computed flow velocity is less than the maximum allowable velocity, erosion is not expected to occur.

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75} 19 mm conversion 0.75 inches

Since the D75 is more than to 0.25 inches and less than 5.0 inches, case 1 of the reference tractive method is used.

Assuming a water temperature of 60°F

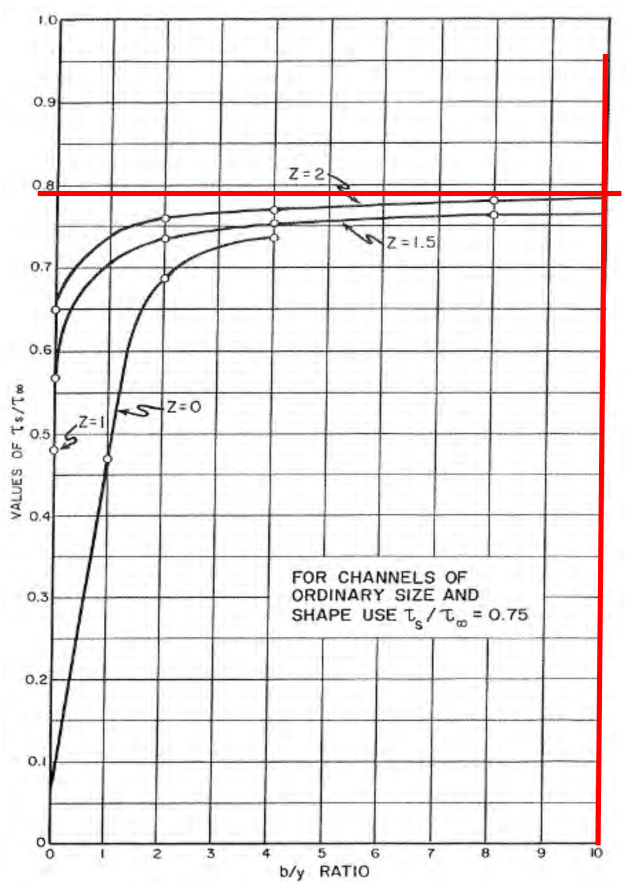
Kinematic Velocity (ν)	0.0000121	ft^2 / sec
Density (ρ)	1.94	slugs/ft^3
Gravity	32.17	ft/sec^2
Unit Weight of Water (γ)	62.4	lbs/ft^3

Tractive Stress for Soils in an infinitely Wide Channel (τ∞)

$$\gamma_w Y \left[\frac{D_{75}^{\frac{1}{6}}}{39n}\right]^2 S_e$$

0.015 lbs/ft^2

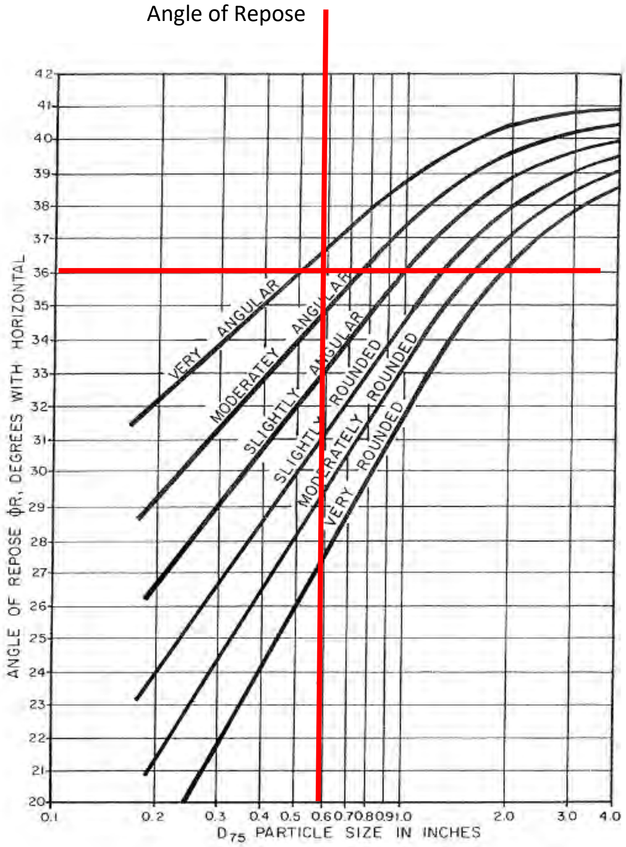
Actual Maximum Tractive Stress, τ_s on Sides of Straight Trapezoial Channels



b/y Ratio	14.56	ft/ft
Z (H/V)	5.33	ft/ft
τs/τ∞	0.79	
τs	0.012	lbs/ft^2

Allowable Tractive Stress (τ_{ls})

Angle of Repose



Moderately Angular

D_{75}	0.75	inches
From Chart (ϕR)	36	Degrees

Solving for Allowable Tractive Stress

$$0.4\left[\frac{Z^2 - Cot^2 \emptyset R}{1+Z^2}\right]^{\frac{1}{2}} D_{75}$$

0.279 lbs/ft^2

Allowable Tractive Stress, from calculation above 0.279 lb/ft^2

Calculated Tractive Stress, τ 0.012 lb/ft^2

Since the allowable tractive stress is more than the calculated tractive stress, the channel is not erosive.

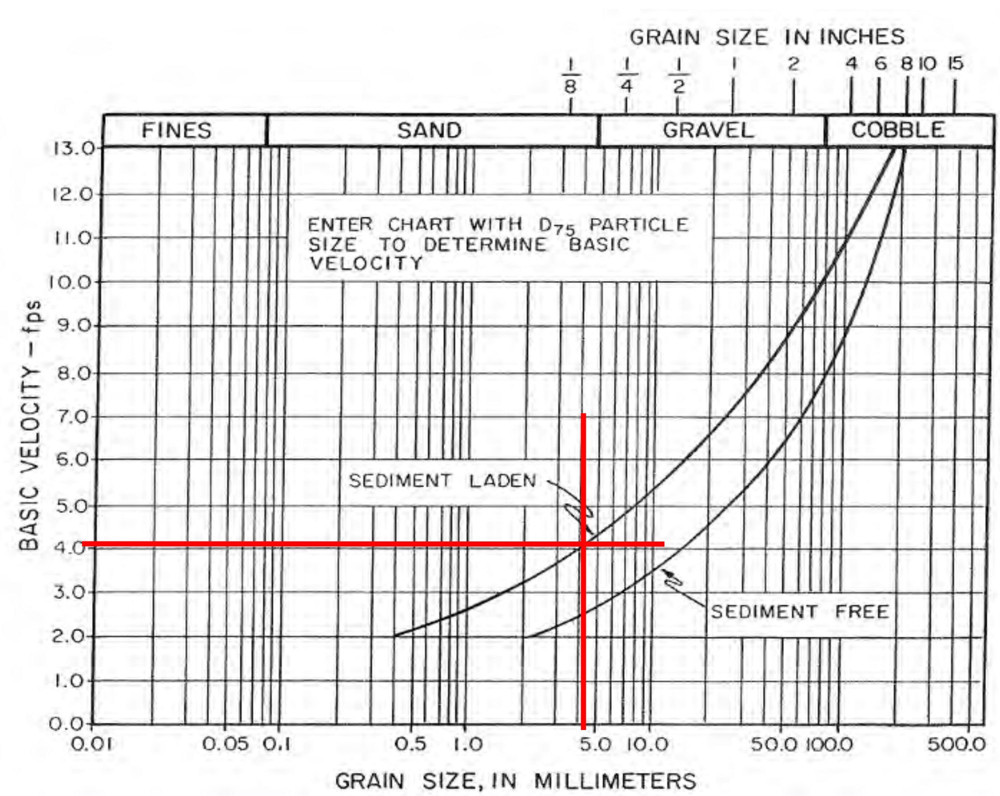
Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

Cross Section 1223 - General Information

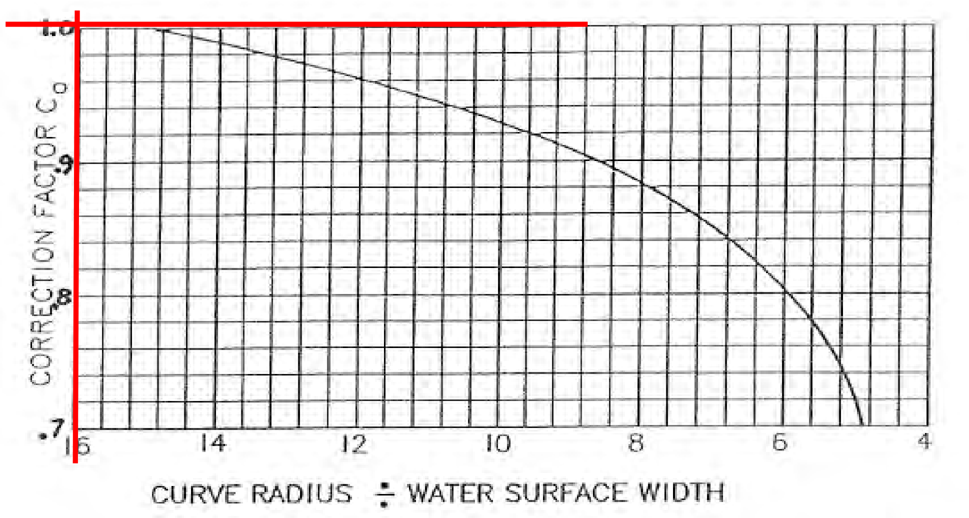
Bottom Width (b)	35.9	feet			
Side Slope (ft)	30	Horizontal	3.7	Vertical	
Channel Slope (S_e)	0.001	feet/foot			
Radius of Curvature (r)	0	feet			
Water Surface Width	66.29	feet			
Average Manning's n	0.042				
Flow Depth (Y)	3.93	feet			
Flow Velocity (V)	2.99	feet/second			
D_{75}	4.4	mm	0.17	inches	0.014 feet
D_{65}	3.7	mm	0.15	inches	0.012 feet
D_{50}	0.9	mm	0.04	inches	0.003 feet

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 4.2 feet/sec

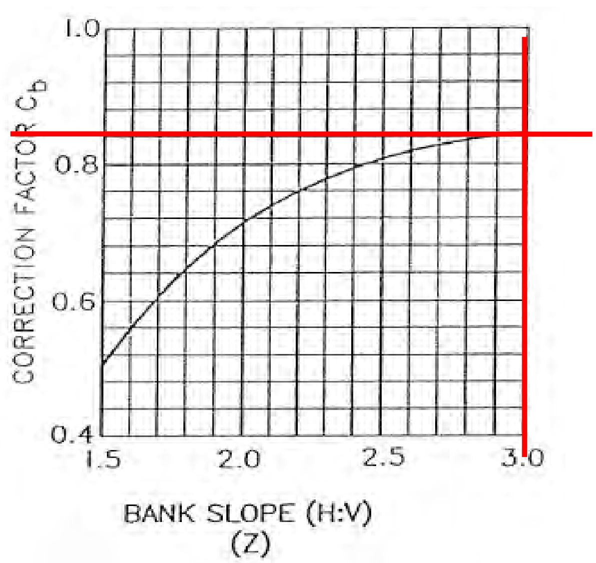


Correction Factor C_a For Channel Alignment 1



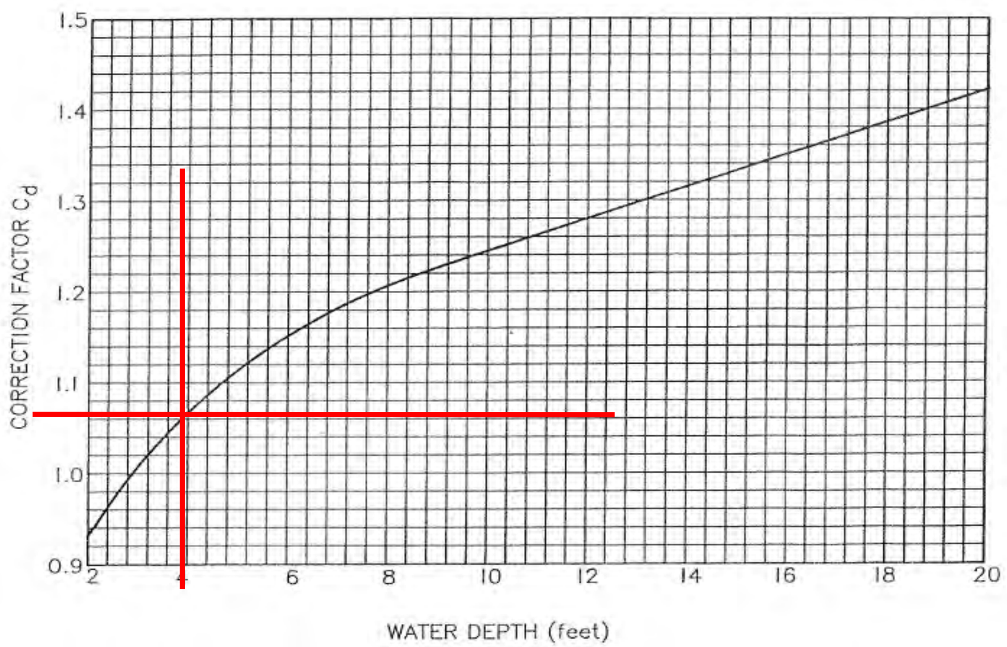
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.84



Horizontal/Vertical (Z) 8.11

Correction Factor C_d For Depth of Flow 1.06



Flow Depth (Y) 3.93 feet

Maximum Allowable Velocity 3.74 feet/second

$$V_a = V_b \ x \ C_a \ x \ C_b \ x \ C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	3.74	feet/second
Flow Velocity	2.99	feet/second

Since the computed flow velocity is less than the maximum allowable velocity, erosion is not expected to occur.

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75}	4.4 mm	conversion	0.17 inches

Since D75 is less or equal to 0.25 inches, case 2 of reference tractive stress method is used.

Assuming a water temperature of 60°F

Kinematic Velocity (ν)	0.0000121	ft^2 / sec
Density (ρ)	1.94	slugs/ft^3
Gravity	32.17	ft/sec^2

Finding Values Needed for Graphs

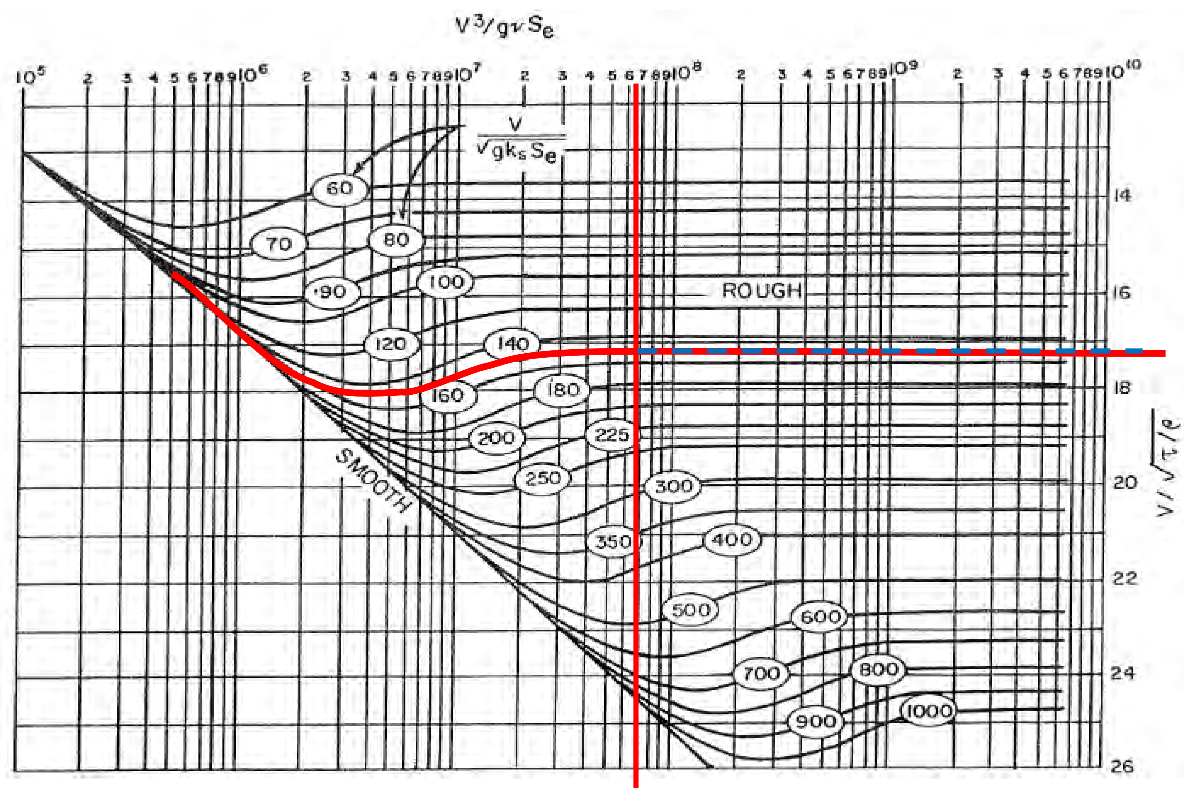
$$V^3/(gvS_e)$$

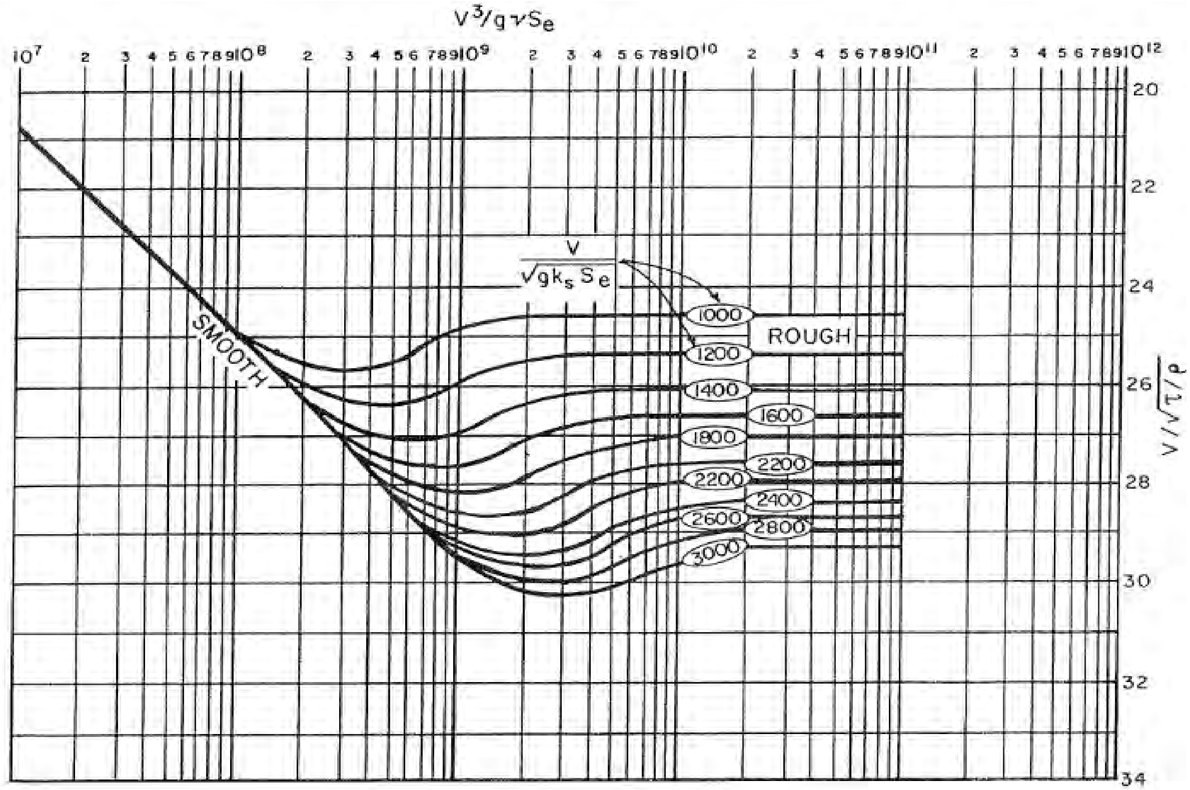
Value 1 6.87E+07

$$V/[(gD_{65}S_e)^{0.5}]$$

Value 2 151.3

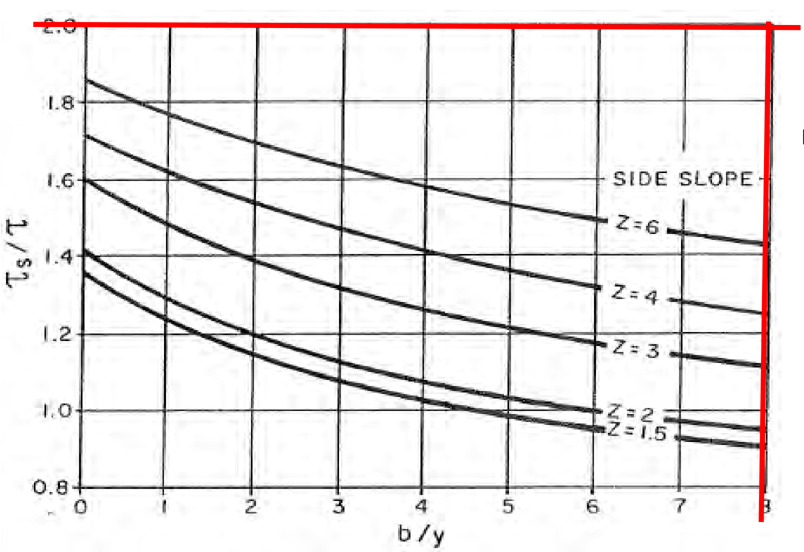
Graphic Solution of Reference Tractive Stress





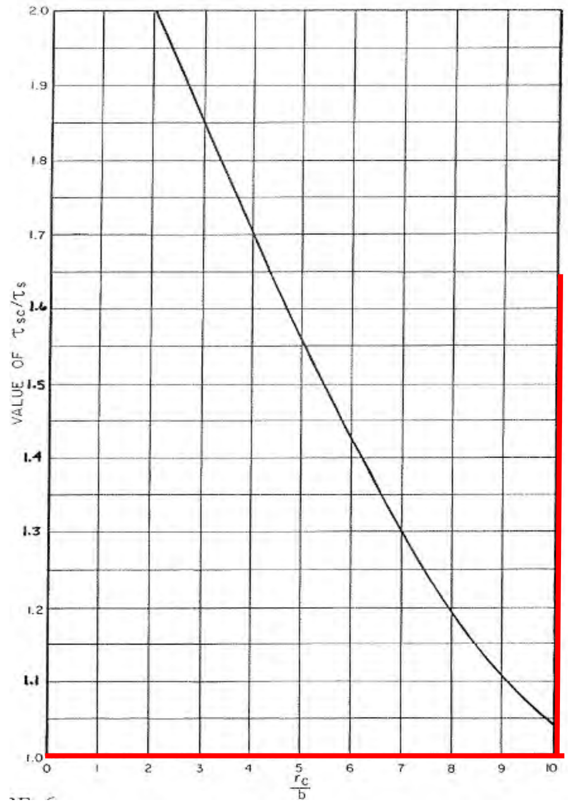
$V/\sqrt{\tau/\rho}$	From Graph Above	17.3
Solving the above equation for τ	0.06	lb/ft^2

Applied Maximum Tractive Stress, τ_s on sides of straight trapezoidal channels



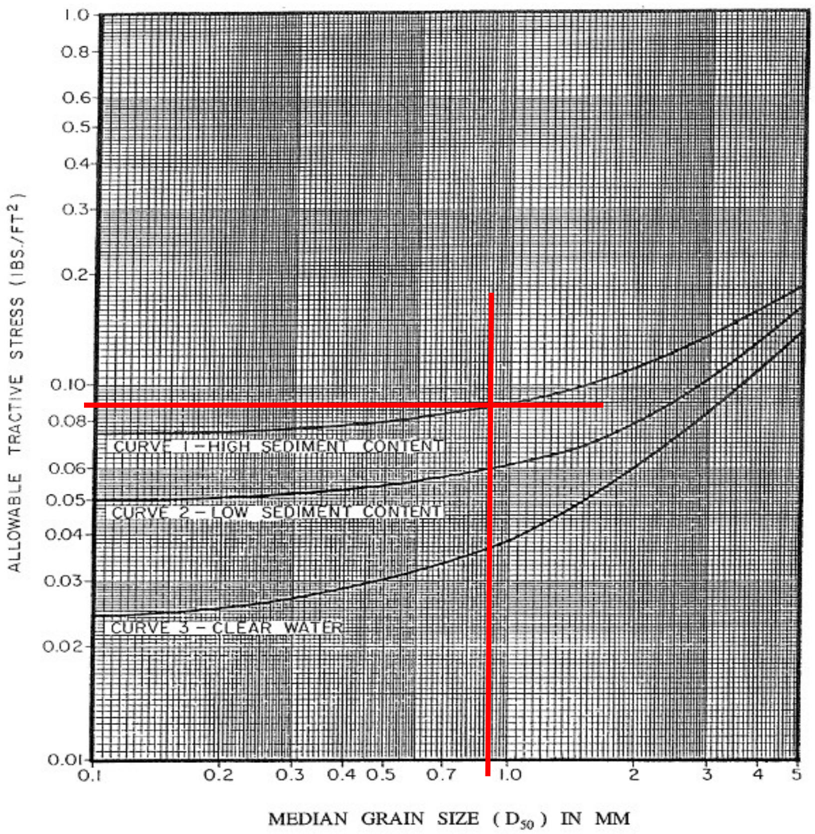
Bottom Width/Flow depth	9.13	
Horizontal/Vertical (Z)	8.11	
From Graph	2	
Solving for τ_s	0.12	lb/ft^2

Actual Maximum Tractive Stress, τ_s on sides of trapezoidal channels within a curved reach



Radius of Curvature/Bottom Width	N/A
From Graph Above	1.00
Solving for τ_s	0.12 lb/ft^2

Maximum Allowable Tractive Stress For Non-Cohesive Soils, D_{75} < 0.25"



MEDIAN GRAIN SIZE (D_{50}) IN MM

D_{50}	0.9 mm	0.04 inches	0.003 feet

Allowable Tractive Stress, from graph above 0.092 lb/ft^2

Calculated Tractive Stress, τ 0.06 lb/ft^2

Since the allowable tractive stress is more than the calculated tractive stress, the channel is not erosive.

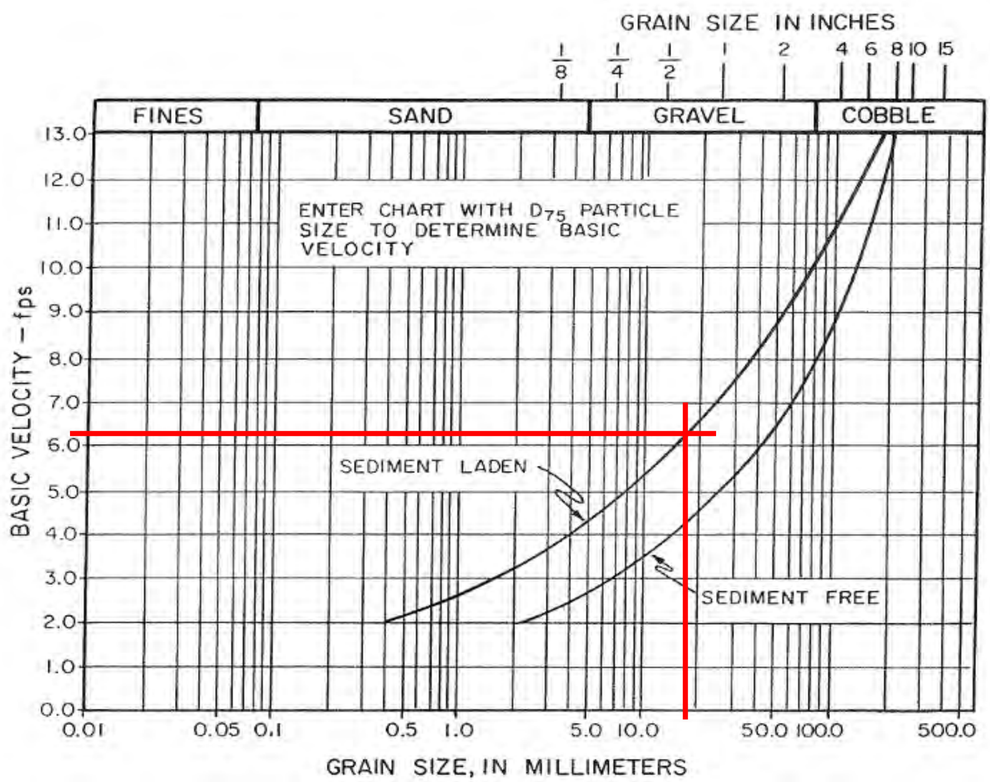
Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

Cross Section 1223 - General Information

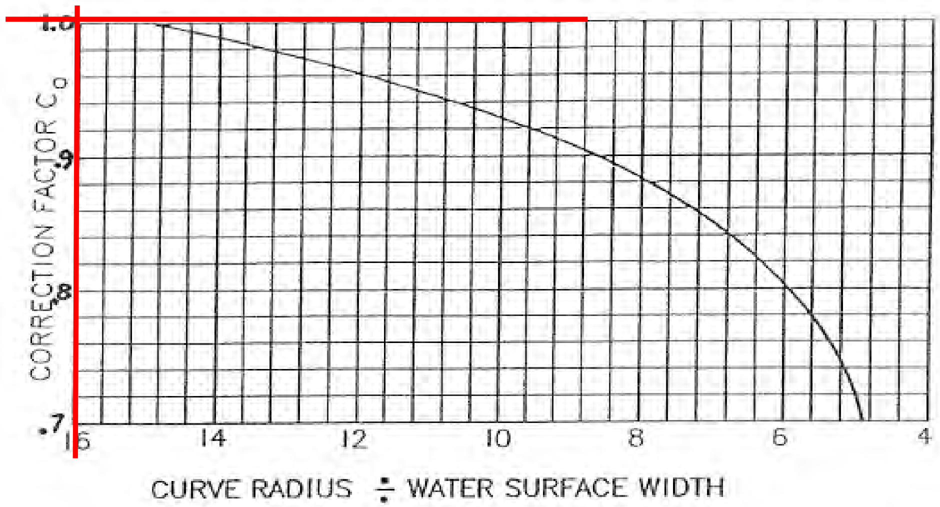
Bottom Width (b)	35.9 feet		
Side Slope (ft)	30 Horizontal	3.7 Vertical	
Channel Slope (S_e)	0.001 feet/foot		
Radius of Curvature (r)	0 feet		
Water Surface Width	66.29 feet		
Average Manning's n	0.042		
Flow Depth (Y)	3.93 feet		
Flow Velocity (V)	2.99 feet/second		
D_{75}	19 mm	0.75 inches	0.062 feet
D_{65}	9.1 mm	0.36 inches	0.030 feet
D_{50}	5.2 mm	0.20 inches	0.017 feet

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 6.3 feet/sec

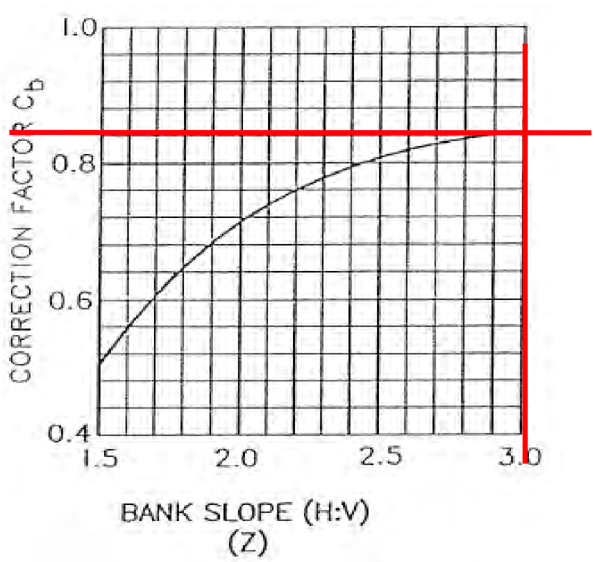


Correction Factor C_a For Channel Alignment 1



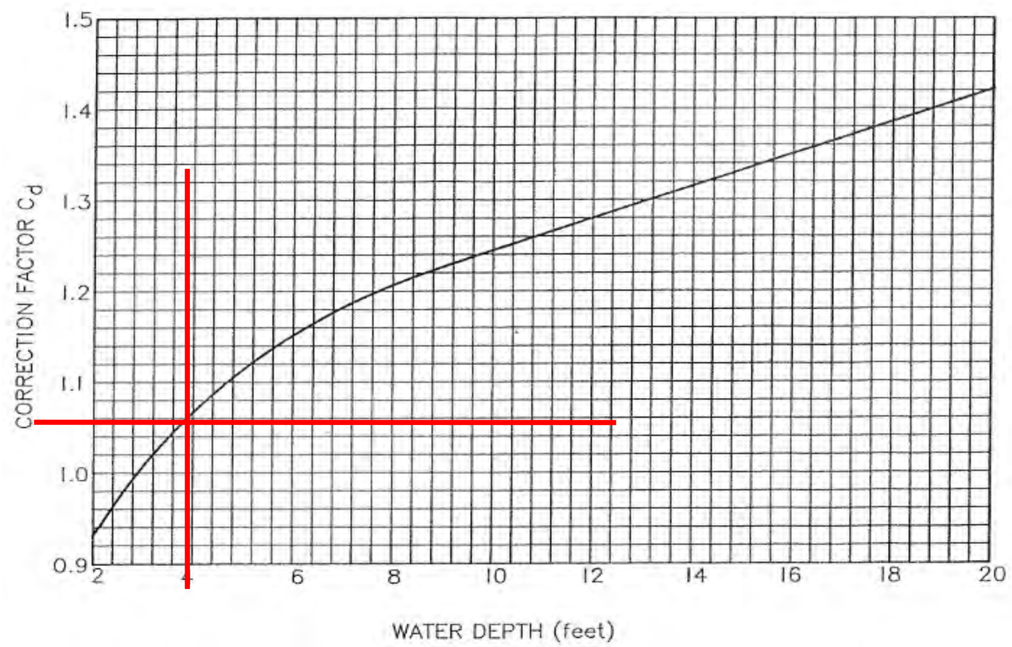
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.84



Horizontal/Vertical (Z) 8.11

Correction Factor C_d For Depth of Flow 1.06



Flow Depth (Y) 3.93 feet

Maximum Allowable Velocity 5.61 feet/second

$$V_a = V_b \; x \; C_a \; x \; C_b \; x \; C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	5.61	feet/second
Flow Velocity	2.99	feet/second

Since the computed flow velocity is less than the maximum allowable velocity, erosion is not expected to occur.

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75}	19 mm	conversion	0.75 inches

Since the D75 is more than to 0.25 inches and less than 5.0 inches, case 1 of the reference tractive method is used.

Assuming a water temperature of 60°F

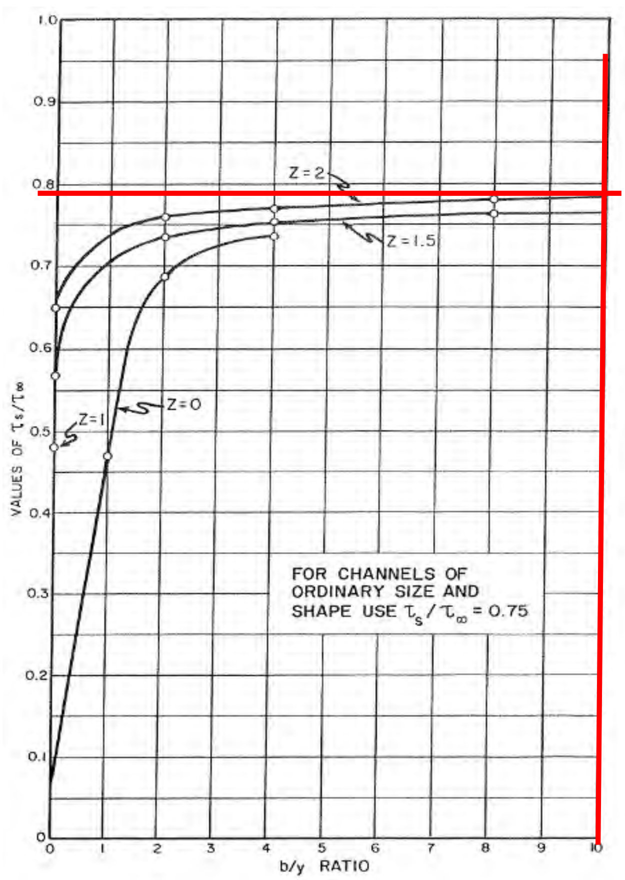
Kinematic Velocity (ν)	0.0000121	ft^2 / sec
Density (ρ)	1.94	slugs/ft^3
Gravity	32.17	ft/sec^2
Unit Weight of Water (γ)	62.4	lbs/ft^3

Tractive Stress for Soils in an infinitely Wide Channel (τ∞)

$$\gamma_w Y \left[\frac{D_{75}^{\frac{1}{6}}}{39n}\right]^2 S_e$$

0.036 lbs/ft^2

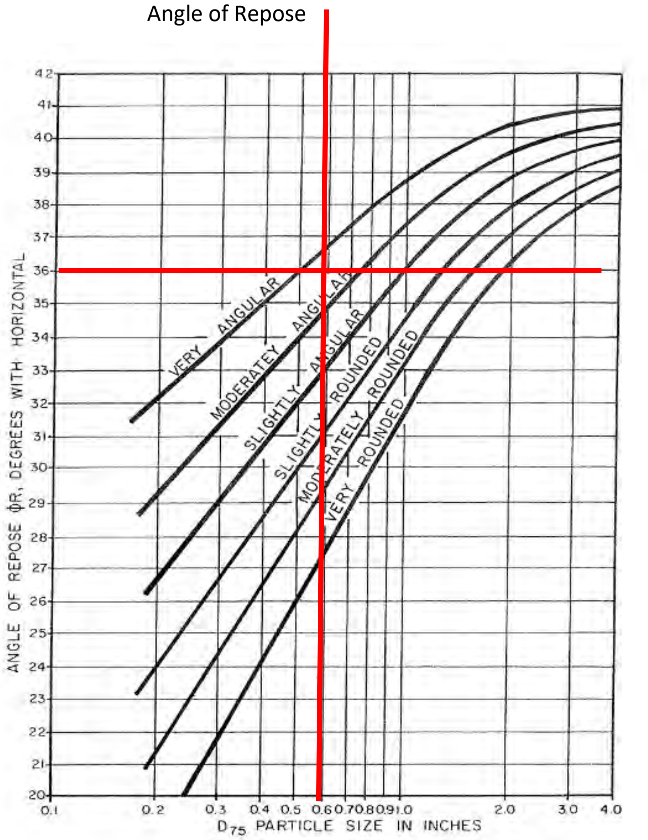
Actual Maximum Tractive Stress, τ_s on Sides of Straight Trapezoial Channels



b/y Ratio	9.13	ft/ft
Z (H/V)	8.11	ft/ft
τs/τ∞	0.79	
τs	0.029	lbs/ft^2

Allowable Tractive Stress (τ_{ls})

Angle of Repose



Moderately Angular

D_{75} 0.75 inches

From Chart (ϕR) 36 Degrees

Solving for Allowable Tractive Stress

$$0.4\left[\frac{Z^2 - Cot^2 \emptyset R}{1 + Z^2}\right]^{\frac{1}{2}} D_{75}$$

0.290 lbs/ft^2

Allowable Tractive Stress, from calculation above 0.290 lb/ft^2

Calculated Tractive Stress, τ 0.029 lb/ft^2

Since the allowable tractive stress is more than the calculated tractive stress, the channel is not erosive.

Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

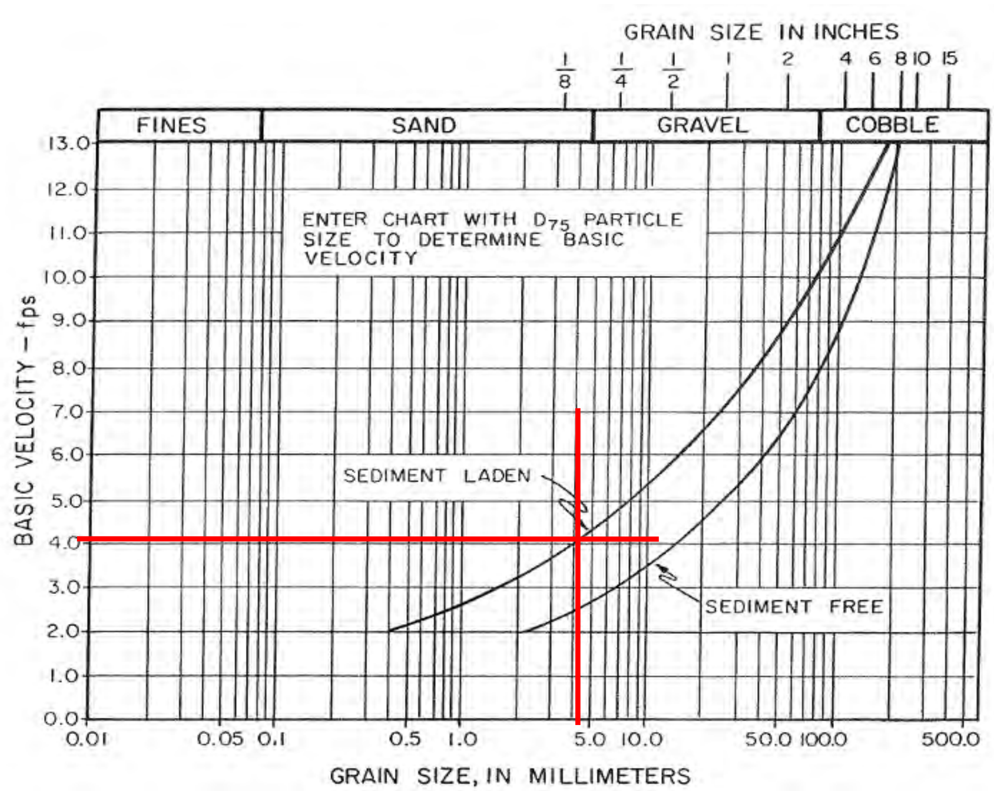
Cross Section 1319 - General Information

Bottom Width (b)	37	feet		
Side Slope (ft)	23.5	Horizontal	4.25	Vertical
Channel Slope (S_e)	0.003	feet/foot		
Radius of Curvature (r)	0	feet		
Water Surface Width	55.14	feet		
Average Manning's n	0.042			
Flow Depth (Y)	3.70	feet		
Flow Velocity (V)	3.81	feet/second		

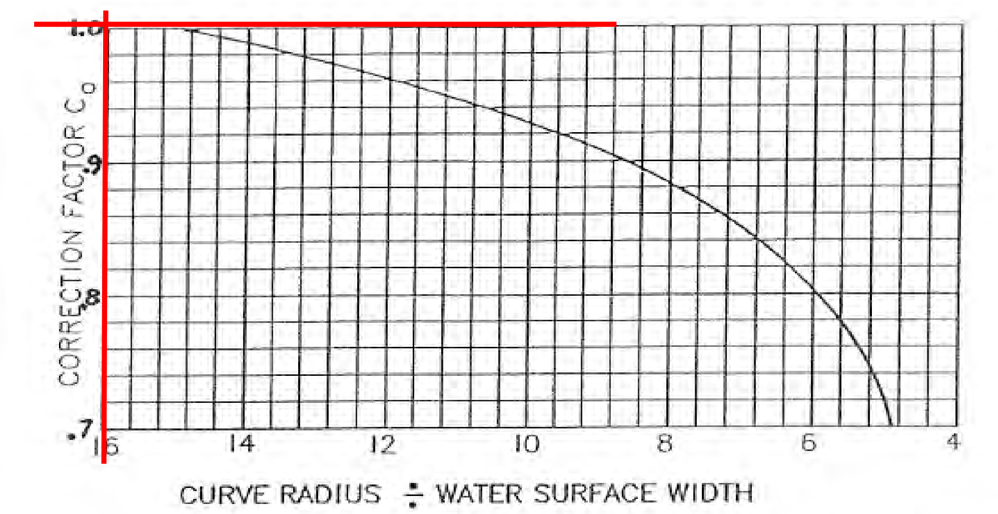
D_{75}	4.4	mm	0.17	inches	0.014	feet
D_{65}	3.7	mm	0.15	inches	0.012	feet
D_{50}	0.9	mm	0.04	inches	0.003	feet

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 4.2 feet/sec

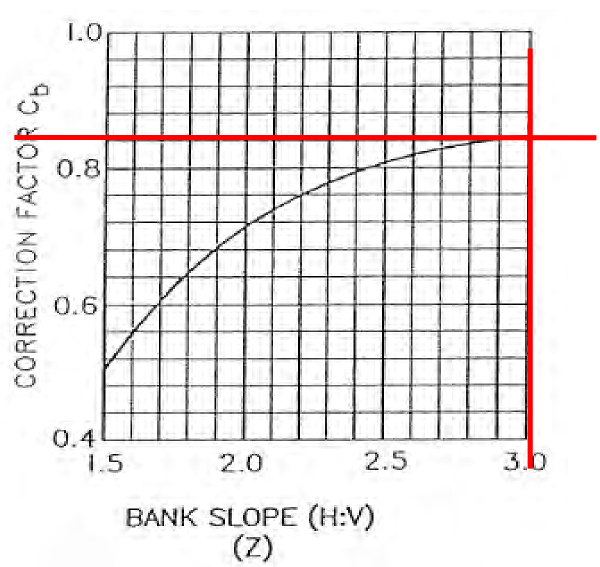


Correction Factor C_a For Channel Alignment 1



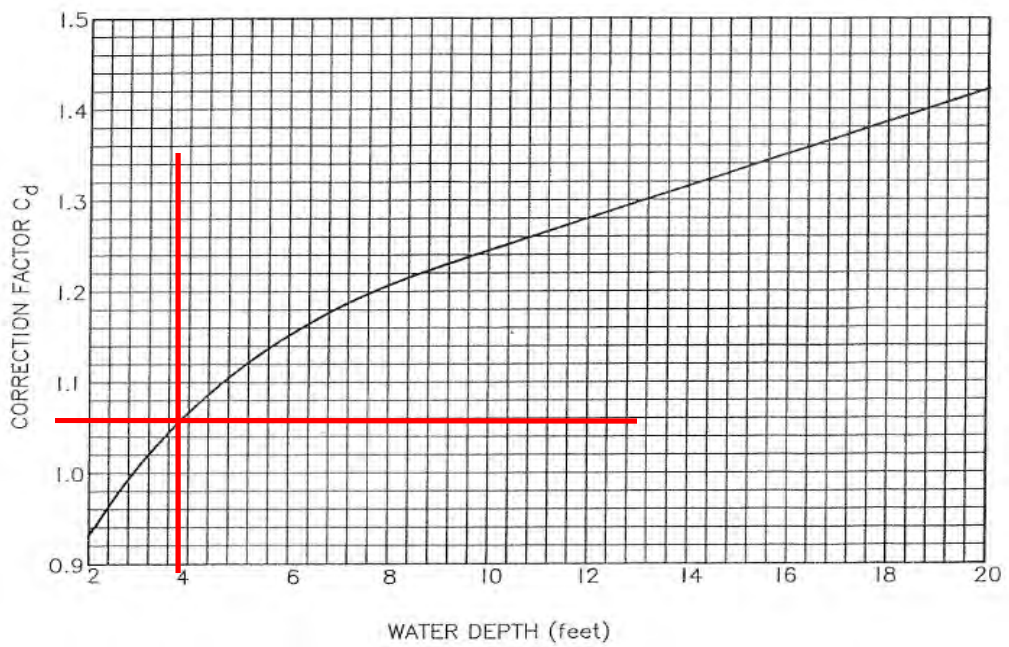
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.84



Horizontal/Vertical (Z) 5.53

Correction Factor C_d For Depth of Flow 1.06



Flow Depth (Y) 3.7 feet

Maximum Allowable Velocity 3.74 feet/second

$$V_a = V_b \ x \ C_a \ x \ C_b \ x \ C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	3.74	feet/second
Flow Velocity	3.81	feet/second

Since the computed flow velocity exceeds the maximum allowable velocity, erosion may be expected to occur

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75}	4.4	mm	conversion	0.17	inches

Since D75 is less or equal to 0.25 inches, case 2 of reference tractive stress method is used.

Assuming a water temperature of 60°F

Kinematic Velocity (ν)	0.0000121	ft^2 / sec
Density (ρ)	1.94	slugs/ft^3
Gravity	32.17	ft/sec^2

Finding Values Needed for Graphs

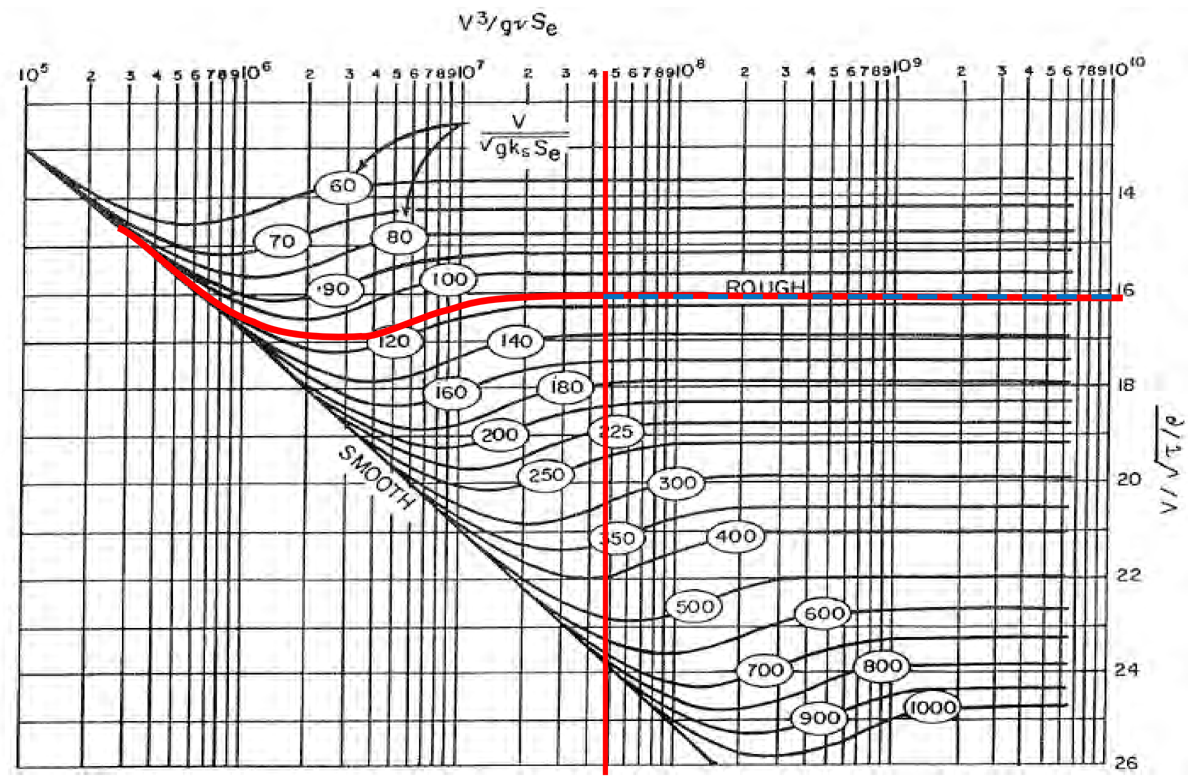
$$V^3/(gvS_e)$$

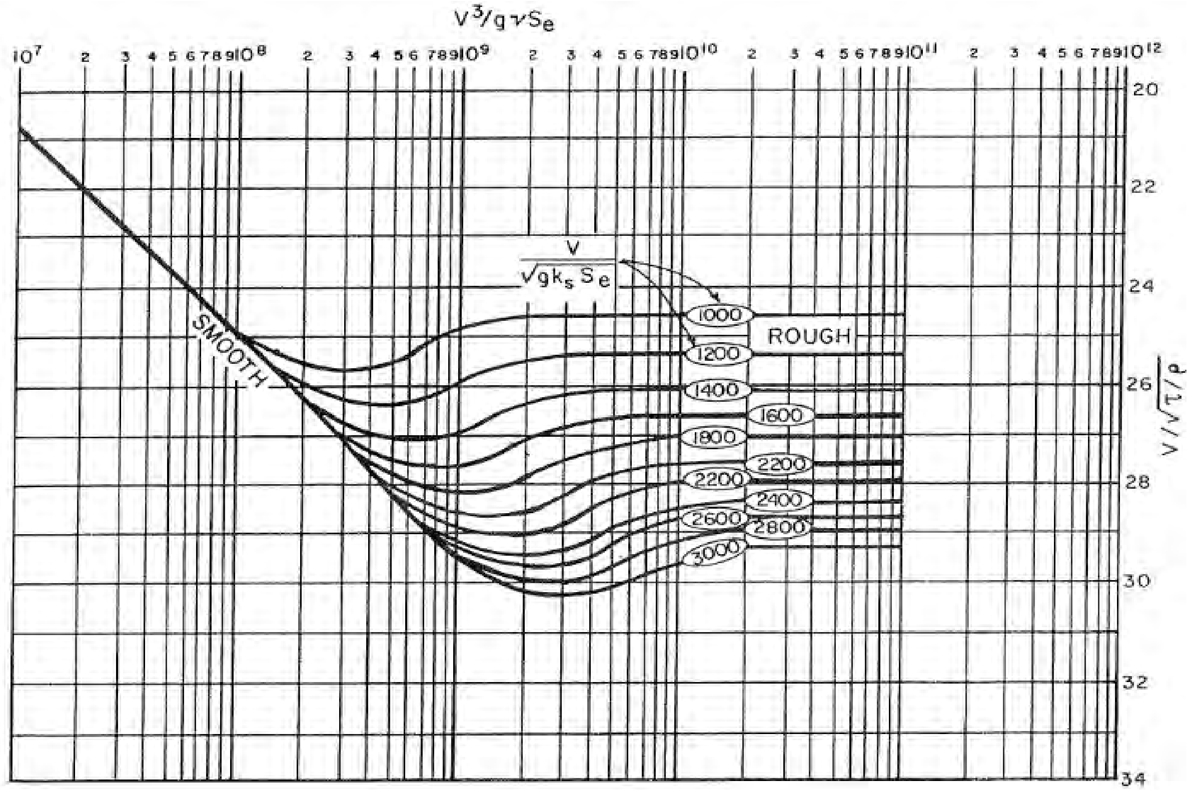
Value 1 4.74E+07

$$V/[(gD_{65}S_e)^{0.5}]$$

Value 2 111.3

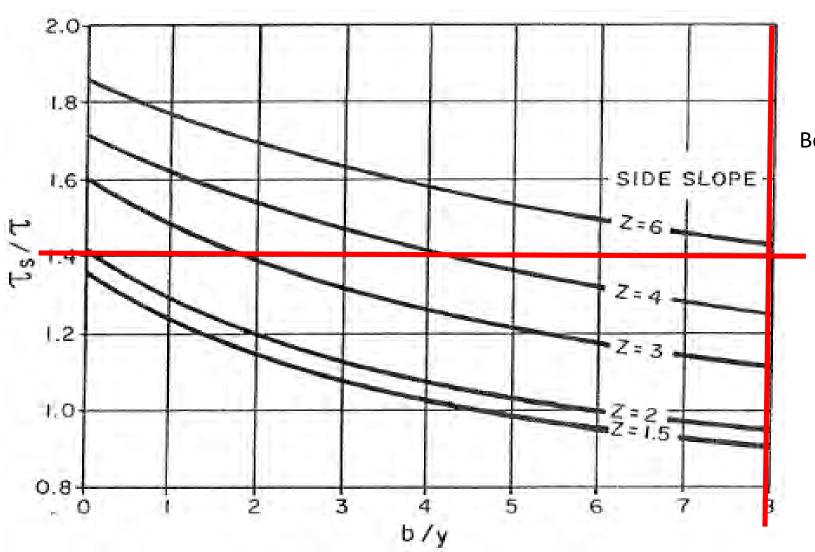
Graphic Solution of Reference Tractive Stress





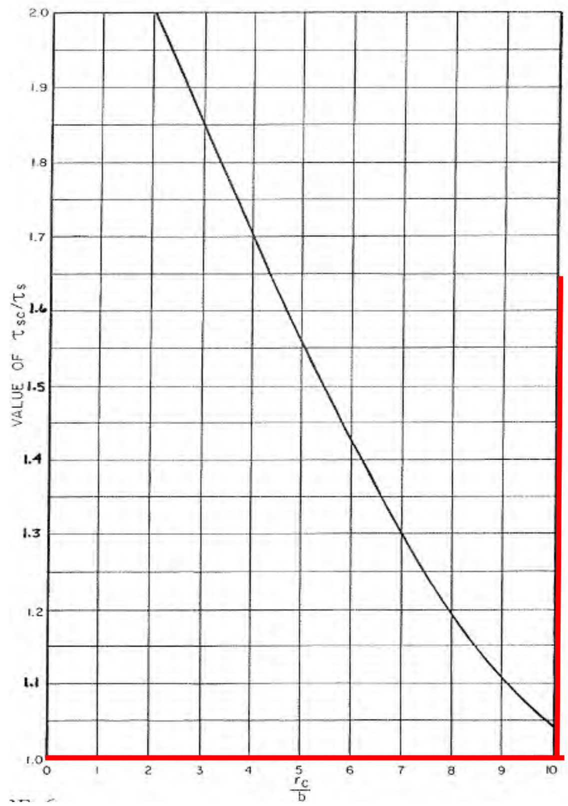
$V/\sqrt{\tau/\rho}$	From Graph Above	16.1
Solving the above equation for τ		0.11 lb/ft^2

Applied Maximum Tractive Stress, τ_s on sides of straight trapezoidal channels



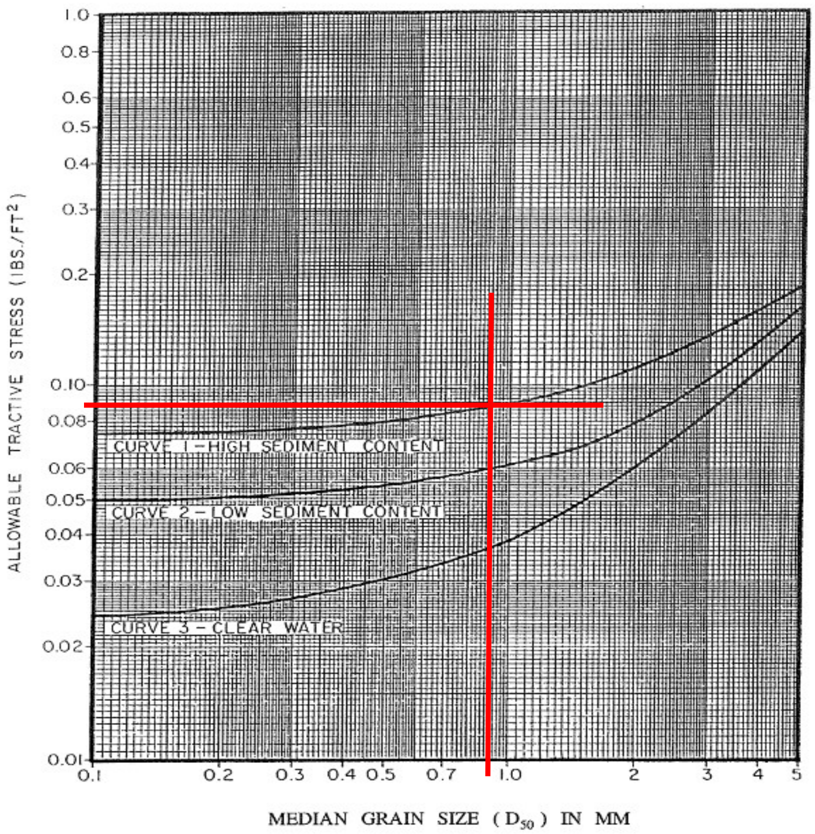
Bottom Width/Flow depth	10.00
Horizontal/Vertical (Z)	5.53
From Graph	1.4
Solving for τ_s	0.15 lb/ft^2

Actual Maximum Tractive Stress, τ_s on sides of trapezoidal channels within a curved reach



Radius of Curvature/Bottom Width	N/A
From Graph Above	1.00
Solving for τ_s	0.15 lb/ft^2

Maximum Allowable Tractive Stress For Non-Cohesive Soils, D_{75} < 0.25"



MEDIAN GRAIN SIZE (D_{50}) IN MM

D_{50}	0.9 mm	0.04 inches	0.003 feet

Allowable Tractive Stress, from graph above 0.092 lb/ft^2

Calculated Tractive Stress, τ 0.109 lb/ft^2

Since the allowable tractive stress is less than the calculated tractive stress, the channel is erosive.

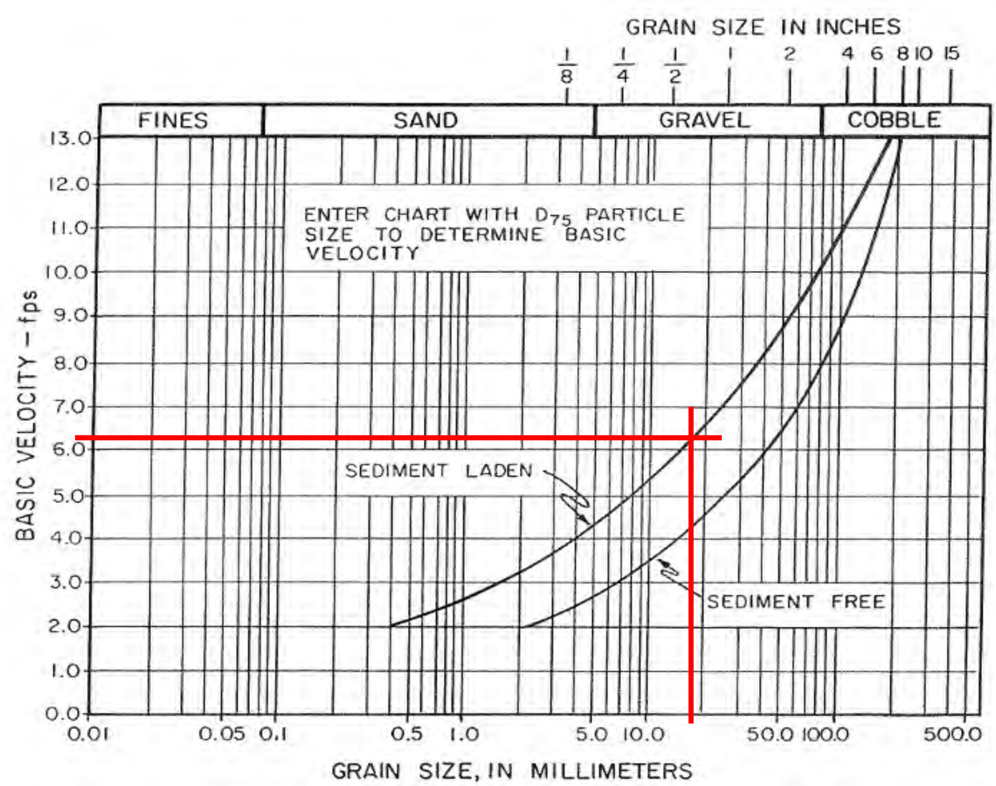
Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

Cross Section 1319 - General Information

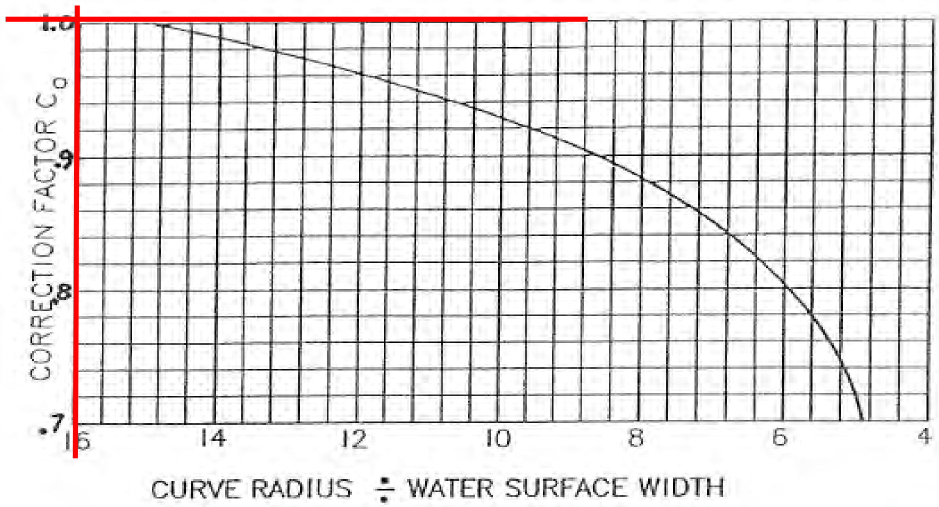
Bottom Width (b)	37	feet				
Side Slope (ft)	23.5	Horizontal	4.25	Vertical		
Channel Slope (S_e)	0.003	feet/foot				
Radius of Curvature (r)	0	feet				
Water Surface Width	55.14	feet				
Average Manning's n	0.042					
Flow Depth (Y)	3.70	feet				
Flow Velocity (V)	3.81	feet/second				
D_{75}	19	mm	0.75	inches	0.062	feet
D_{65}	9.1	mm	0.36	inches	0.030	feet
D_{50}	5.2	mm	0.20	inches	0.017	feet

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 6.3 feet/sec

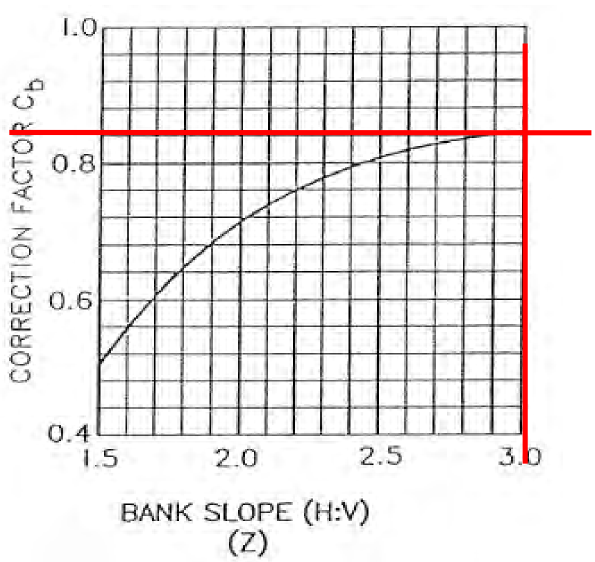


Correction Factor C_a For Channel Alignment 1



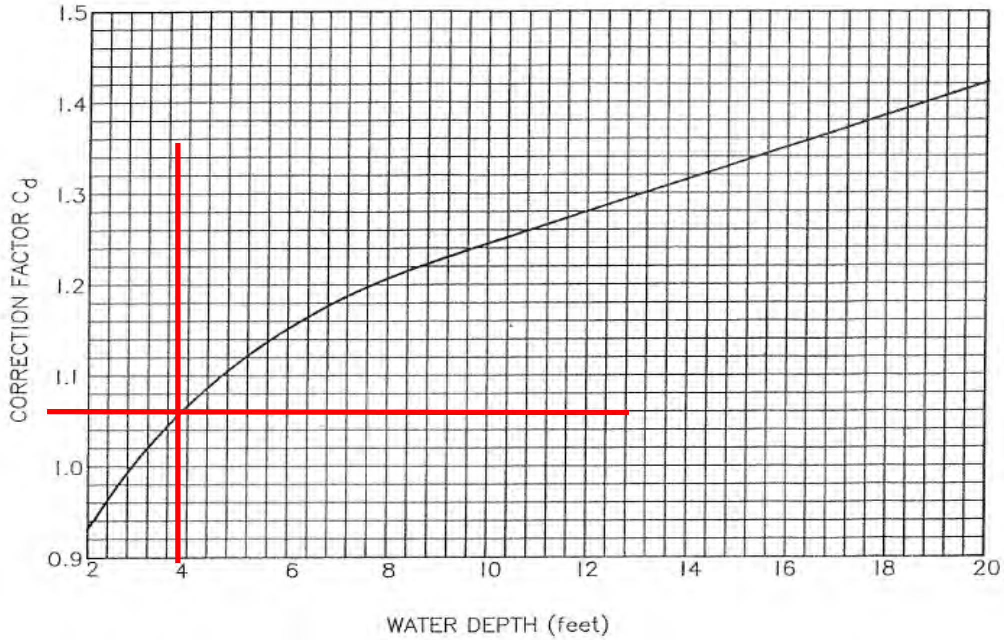
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.84



Horizontal/Vertical (Z) 5.53

Correction Factor C_d For Depth of Flow 1.06



Flow Depth (Y) 3.7 feet

Maximum Allowable Velocity 5.61 feet/second

$$V_a = V_b \; x \; C_a \; x \; C_b \; x \; C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	5.61	feet/second
Flow Velocity	3.81	feet/second

Since the computed flow velocity is less than the maximum allowable velocity, erosion is not expected to occur.

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75}	19 mm	conversion	0.75 inches

Since the D75 is more than to 0.25 inches and less than 5.0 inches, case 1 of the reference tractive method is used.

Assuming a water temperature of 60°F

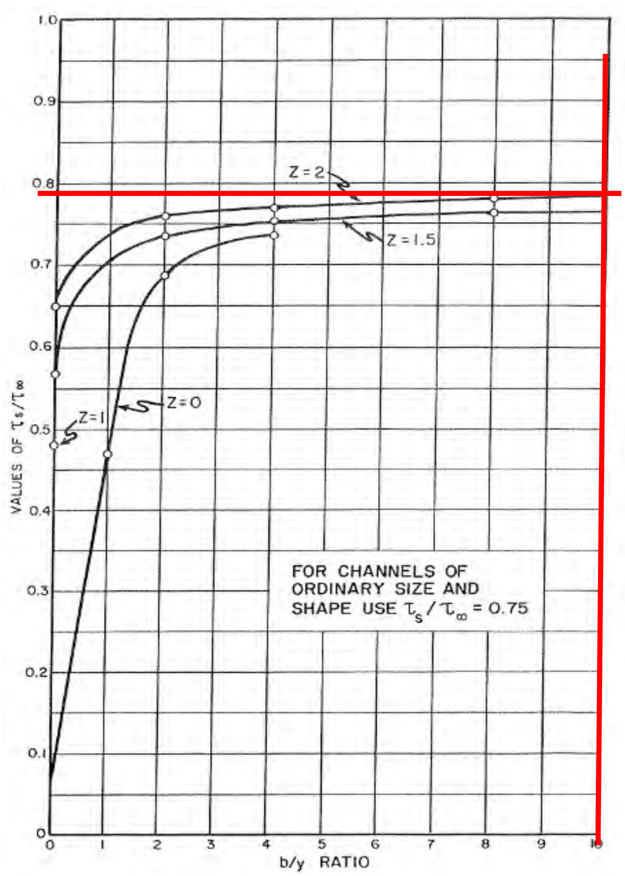
Kinematic Velocity (ν)	0.0000121 ft^2 / sec
Density (ρ)	1.94 slugs/ft^3
Gravity	32.17 ft/sec^2
Unit Weight of Water (γ)	62.4 lbs/ft^3

Tractive Stress for Soils in an infinitely Wide Channel (τ∞)

$$\gamma_w Y \left[\frac{D_{75}^{\frac{1}{6}}}{39n}\right]^2 S_e$$

0.102 lbs/ft^2

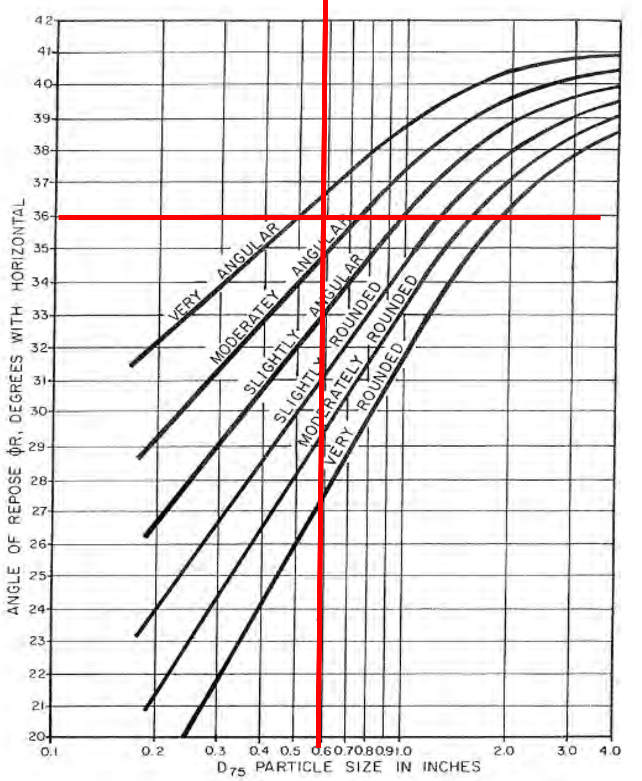
Actual Maximum Tractive Stress, τ_s on Sides of Straight Trapezoial Channels



b/y Ratio	10.00 ft/ft
Z (H/V)	5.53 ft/ft
τs/τ∞	0.79
τs	0.081 lbs/ft^2

Allowable Tractive Stress (τ_{ls})

Angle of Repose



Moderately Angular

D_{75}	0.75	inches
From Chart (ϕR)	36	Degrees

Solving for Allowable Tractive Stress

$$0.4\left[\frac{Z^2 - Cot^2 \emptyset R}{1+Z^2}\right]^{\frac{1}{2}} D_{75}$$

0.280 lbs/ft^2

Allowable Tractive Stress, from calculation above	0.280	lb/ft^2
Calculated Tractive Stress, τ	0.081	lb/ft^2

Since the allowable tractive stress is more than the calculated tractive stress, the channel is not erosive.

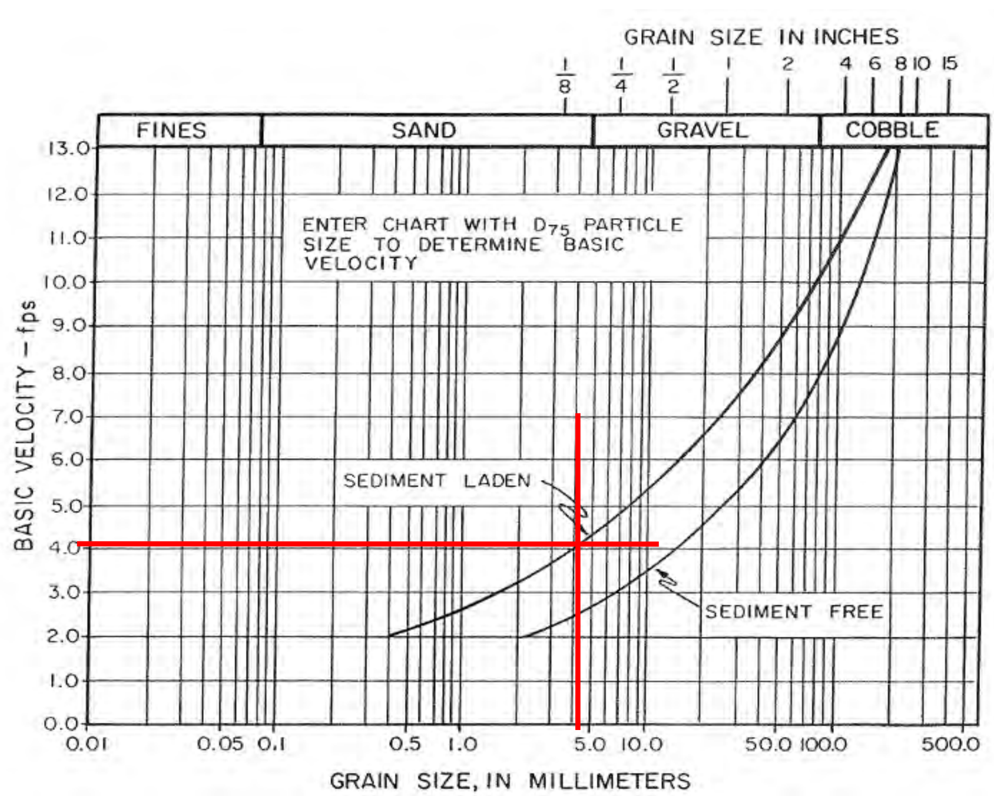
Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

Cross Section 1422 - General Information

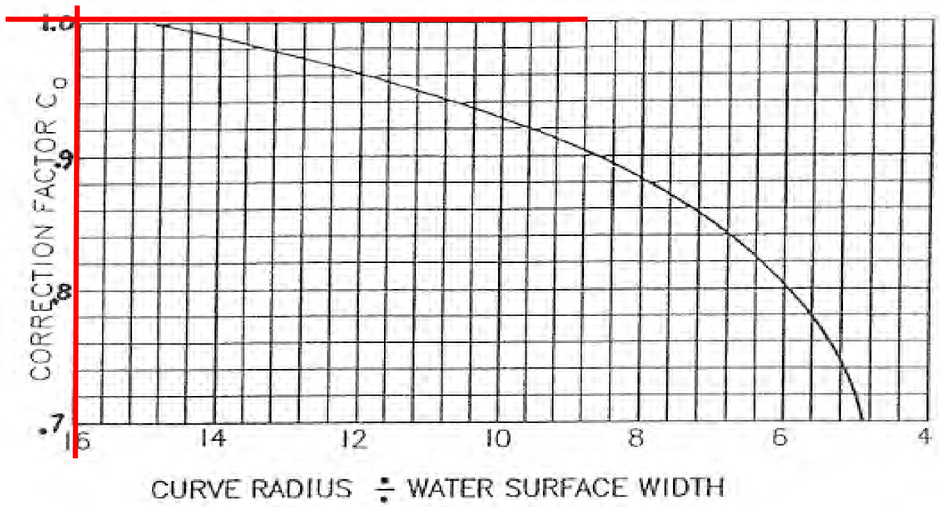
Bottom Width (b)	25	feet			
Side Slope (ft)	23	Horizontal	4.7	Vertical	
Channel Slope (S_e)	0.004	feet/foot			
Radius of Curvature (r)	0	feet			
Water Surface Width	45.11	feet			
Average Manning's n	0.042				
Flow Depth (Y)	4.05	feet			
Flow Velocity (V)	4.37	feet/second			
D_{75}	4.4	mm	0.17 inches	0.014	feet
D_{65}	3.7	mm	0.15 inches	0.012	feet
D_{50}	0.9	mm	0.04 inches	0.003	feet

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 4.2 feet/sec

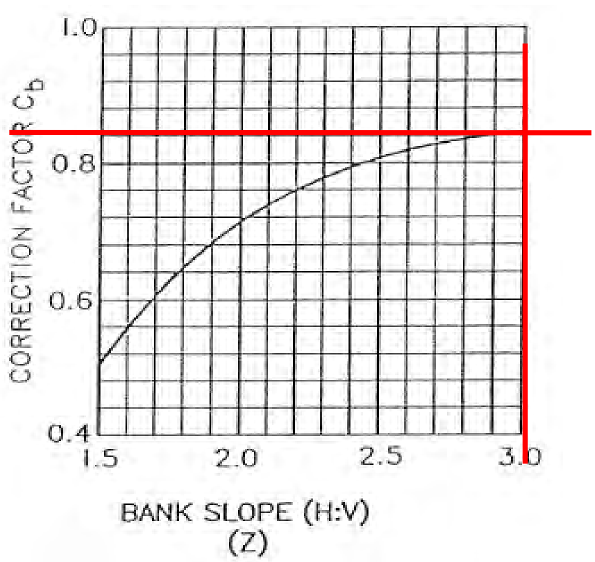


Correction Factor C_a For Channel Alignment 1



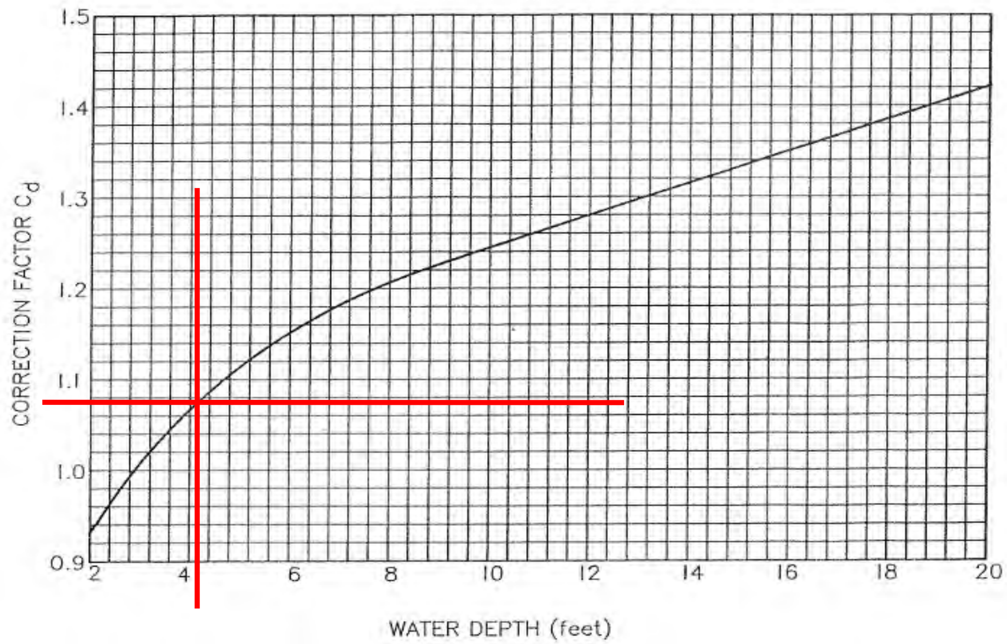
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.84



Horizontal/Vertical (Z) 4.89

Correction Factor C_d For Depth of Flow 1.08



Flow Depth (Y) 4.05 feet

Maximum Allowable Velocity 3.81 feet/second

$$V_a = V_b \; x \; C_a \; x \; C_b \; x \; C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	3.81	feet/second
Flow Velocity	4.37	feet/second

Since the computed flow velocity exceeds the maximum allowable velocity, erosion may be expected to occur

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75}	4.4 mm	conversion	0.17 inches

Since D75 is less or equal to 0.25 inches, case 2 of reference tractive stress method is used.

Assuming a water temperature of 60°F

Kinematic Velocity (ν)	0.0000121 ft^2 / sec
Density (ρ)	1.94 slugs/ft^3
Gravity	32.17 ft/sec^2

Finding Values Needed for Graphs

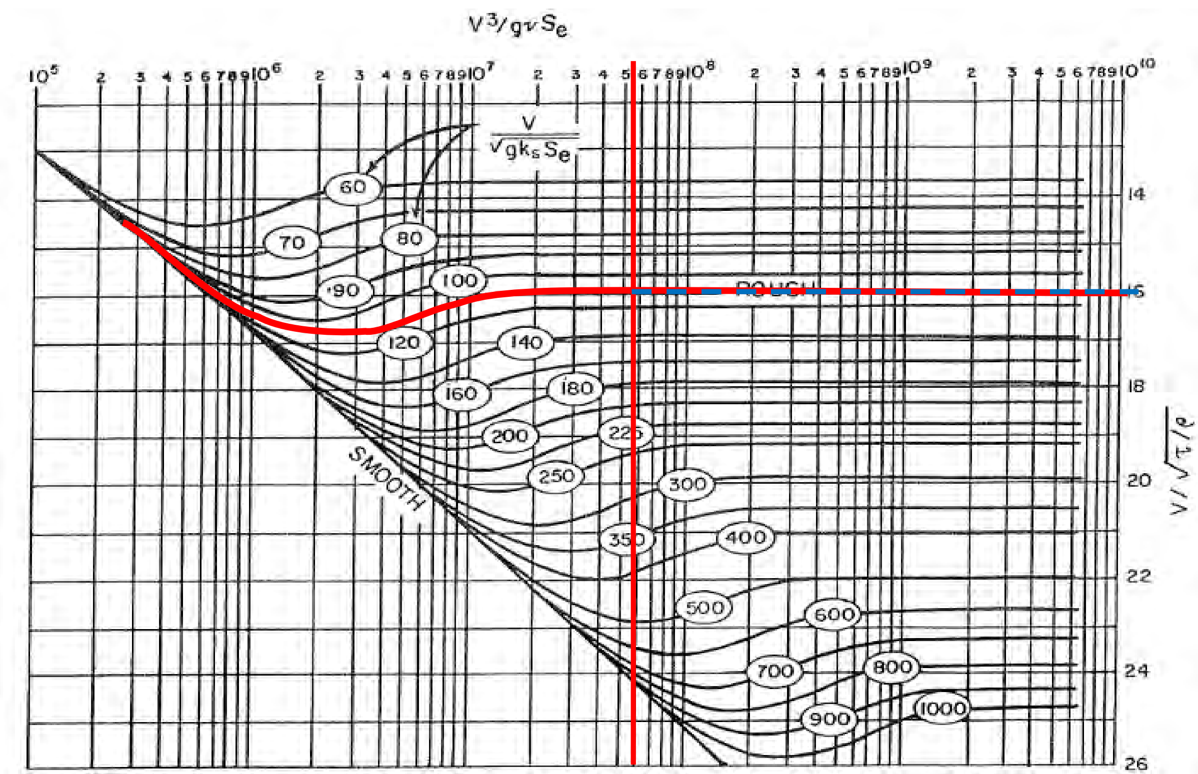
$$V^3/(gvS_e)$$

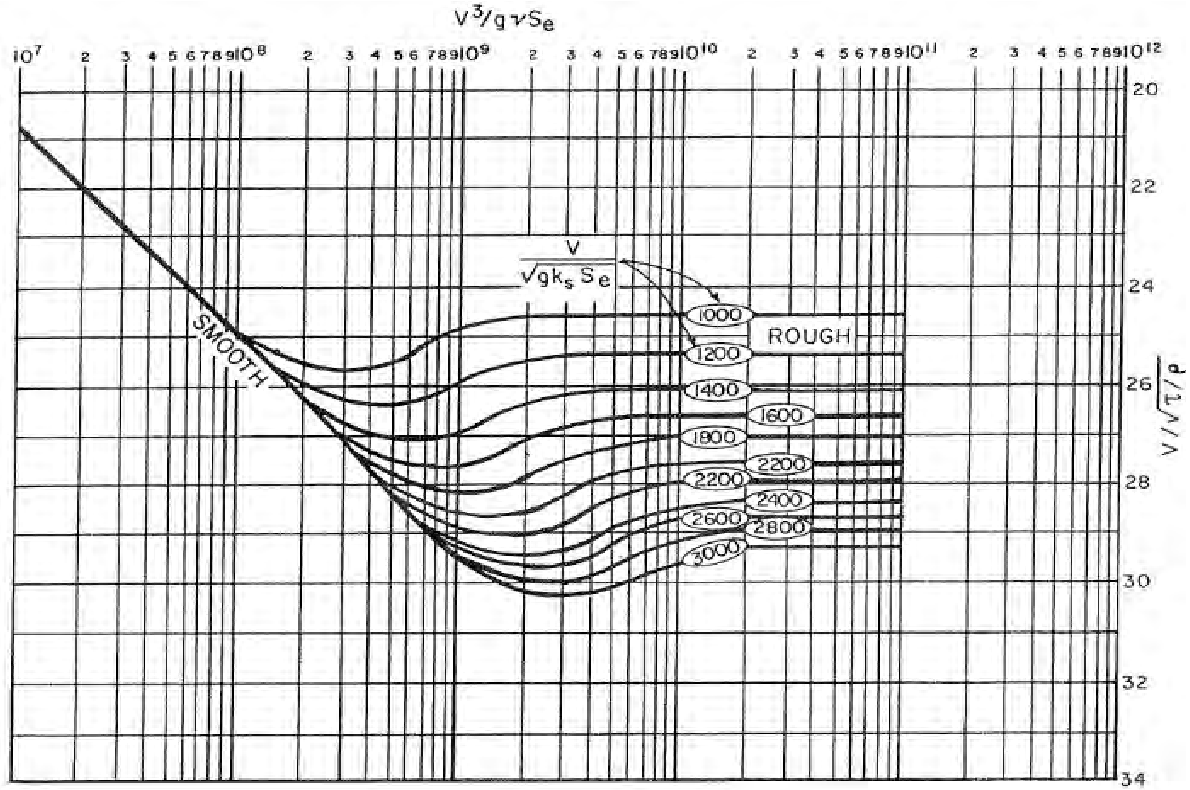
Value 1 5.36E+07

$$V/[(gD_{65}S_e)^{0.5}]$$

Value 2 110.6

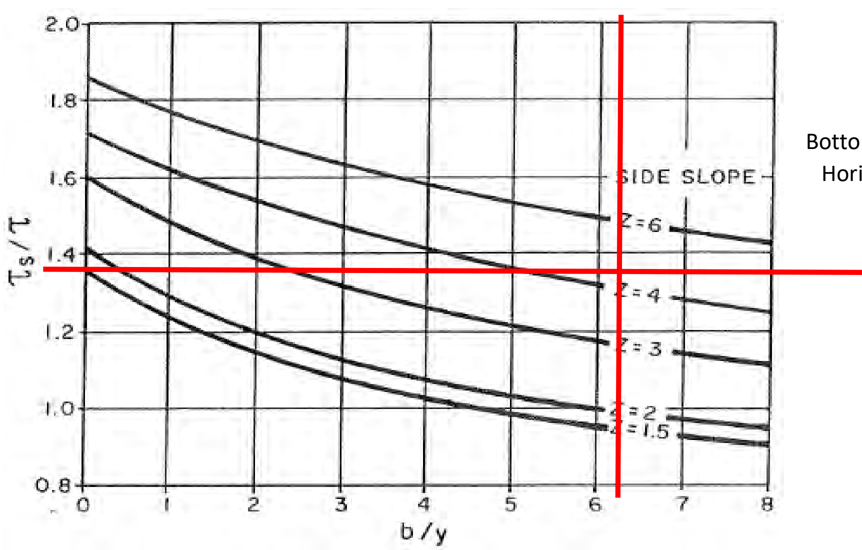
Graphic Solution of Reference Tractive Stress





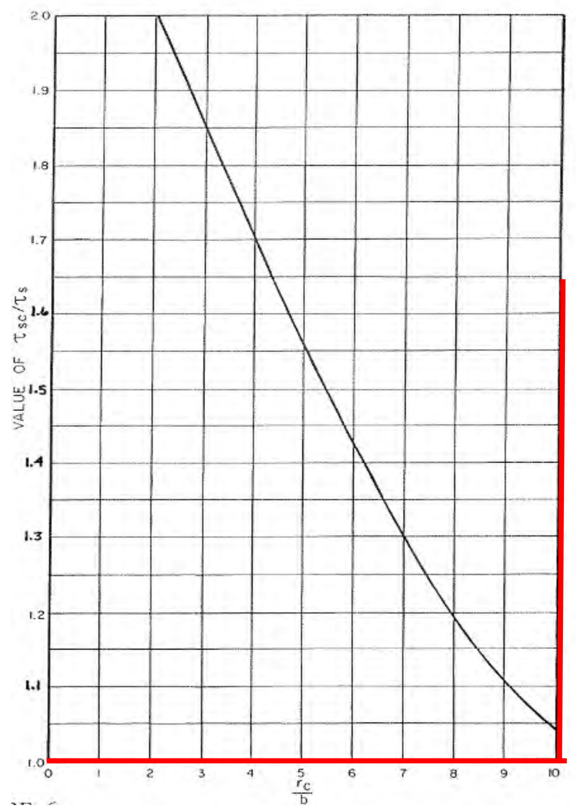
$V/\sqrt{\tau/\rho}$	From Graph Above	16
	Solving the above equation for τ	0.14 lb/ft²

Applied Maximum Tractive Stress, τ_s on sides of straight trapezoidal channels



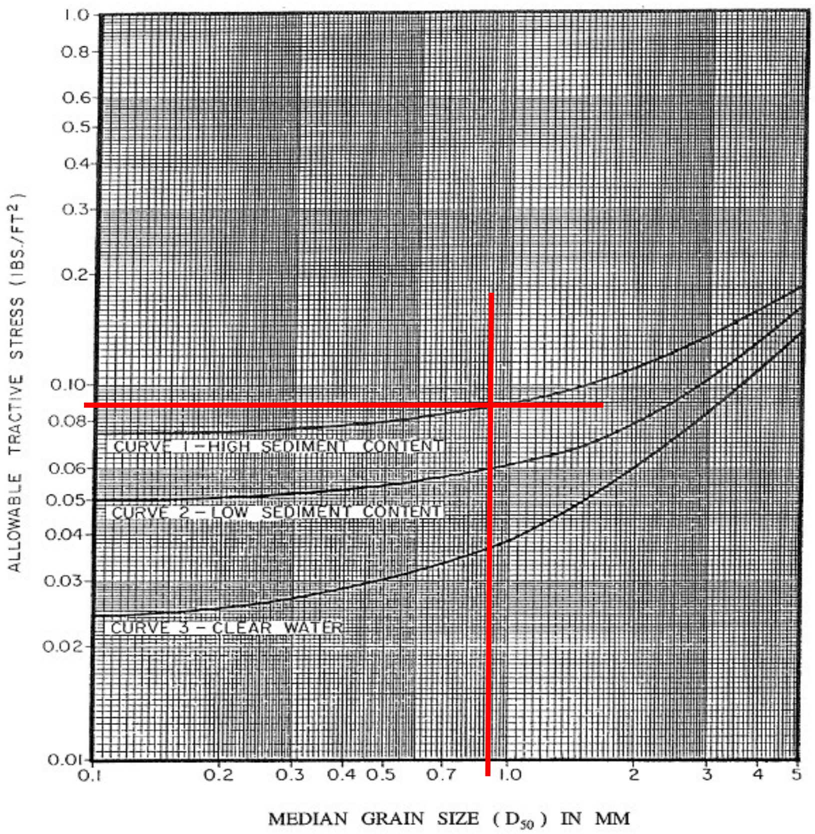
Bottom Width/Flow depth	6.17
Horizontal/Vertical (Z)	4.89
From Graph	1.38
Solving for τ_s	0.20 lb/ft²

Actual Maximum Tractive Stress, τ_s on sides of trapezoidal channels within a curved reach



Radius of Curvature/Bottom Width	N/A
From Graph Above	1.00
Solving for τ_s	0.20 lb/ft^2

Maximum Allowable Tractive Stress For Non-Cohesive Soils, D_{75} < 0.25"



MEDIAN GRAIN SIZE (D_{50}) IN MM

D_{50}	0.9 mm	0.04 inches	0.003 feet

Allowable Tractive Stress, from graph above 0.092 lb/ft^2

Calculated Tractive Stress, τ 0.14 lb/ft^2

Since the allowable tractive stress is less than the calculated tractive stress, the channel is erosive.

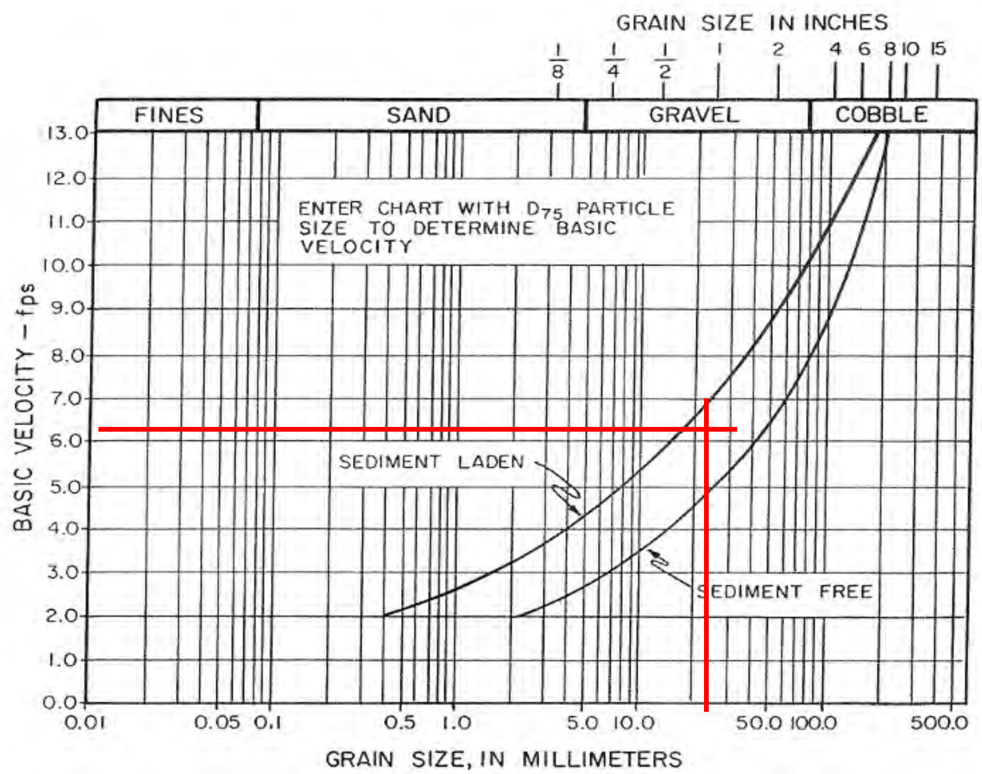
Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

Cross Section 1422 - General Information

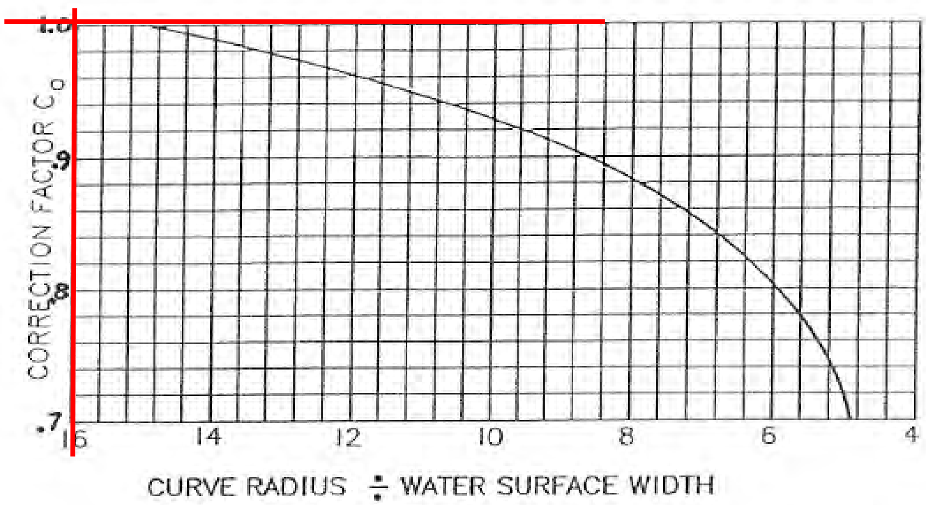
Bottom Width (b)	25 feet			
Side Slope (ft)	23 Horizontal	4.7 Vertical		
Channel Slope (S_e)	0.004 feet/foot			
Radius of Curvature (r)	0 feet			
Water Surface Width	45.11 feet			
Average Manning's n	0.042			
Flow Depth (Y)	4.05 feet			
Flow Velocity (V)	4.37 feet/second			
D_{75}	19 mm	0.75 inches	0.062 feet	
D_{65}	9.1 mm	0.36 inches	0.030 feet	
D_{50}	5.2 mm	0.20 inches	0.017 feet	

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 6.3 feet/sec

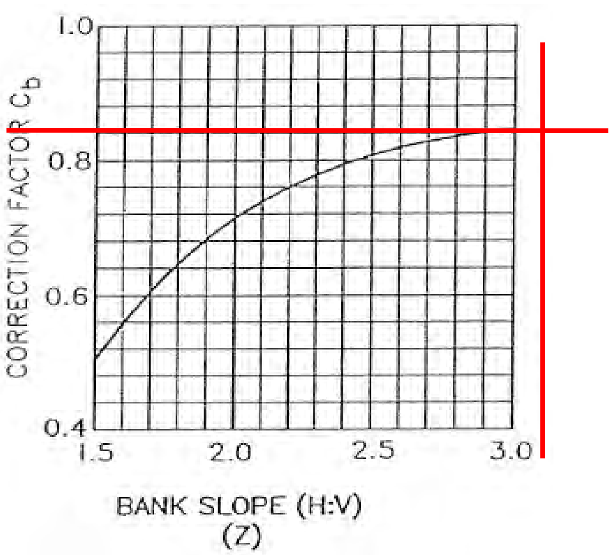


Correction Factor C_a For Channel Alignment 1



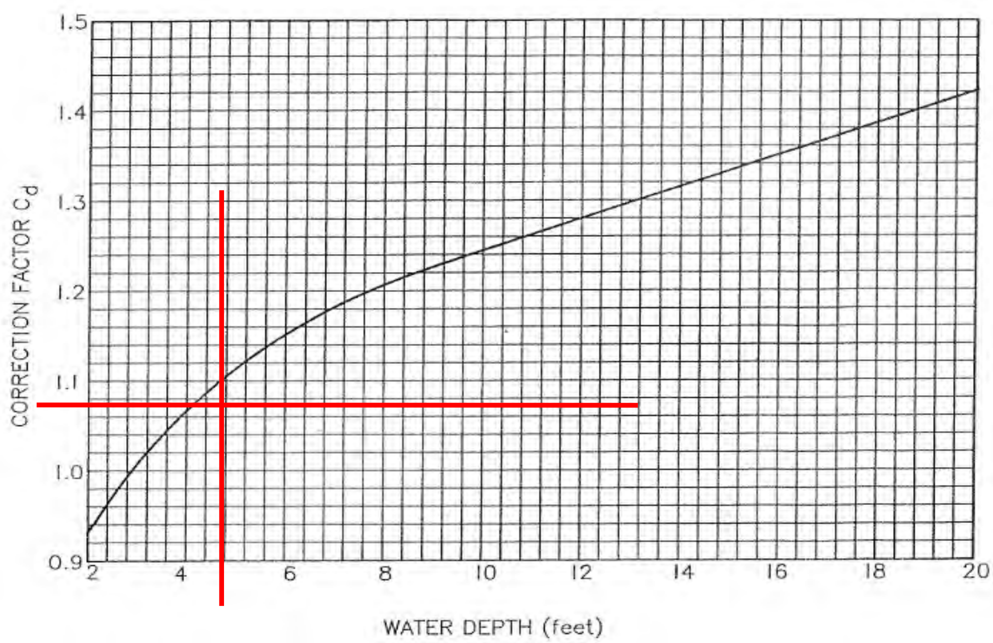
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.84



Horizontal/Vertical (Z) 4.89

Correction Factor C_d For Depth of Flow 1.08



Flow Depth (Y) 4.05 feet

Maximum Allowable Velocity 5.72 feet/second

$$V_a = V_b \ x \ C_a \ x \ C_b \ x \ C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	5.72	feet/second
Flow Velocity	4.37	feet/second

Since the computed flow velocity is less than the maximum allowable velocity, erosion is not expected to occur.

Tractive Stress Analysis (Assuming Sediment Laden Flow)

D_{75} 19 mm conversion 0.75 inches

Since the D75 is more than to 0.25 inches and less than 5.0 inches, case 1 of the reference tractive method is used.

Assuming a water temperature of 60°F

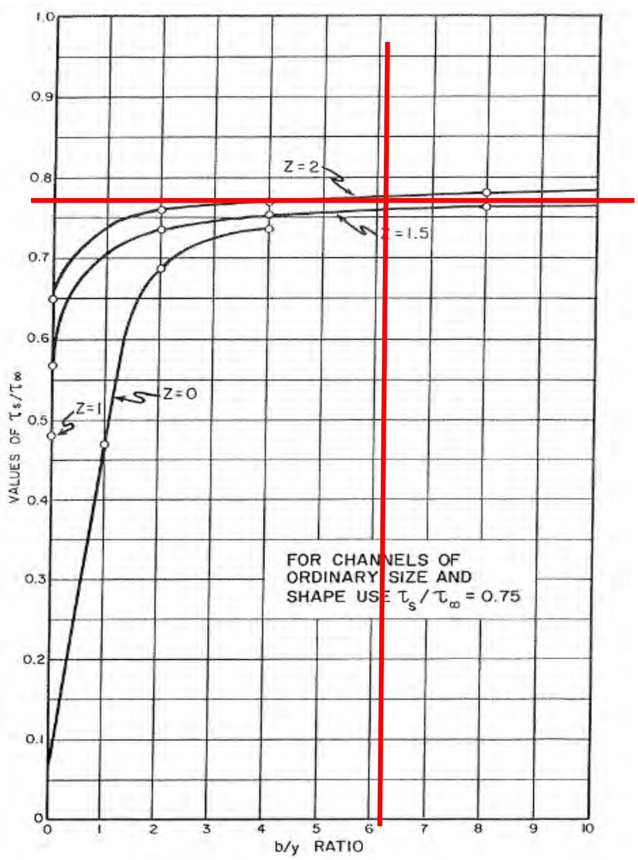
Kinematic Velocity (ν)	0.0000121	ft^2 / sec
Density (ρ)	1.94	$slugs/ft^3$
Gravity	32.17	ft/sec^2
Unit Weight of Water (γ)	62.4	lbs/ft^3

Tractive Stress for Soils in an infinitely Wide Channel (τ∞)

$$\gamma_w Y \left[\frac{D_{75}^{\frac{1}{6}}}{39n}\right]^2 S_e$$

0.149 lbs/ft^2

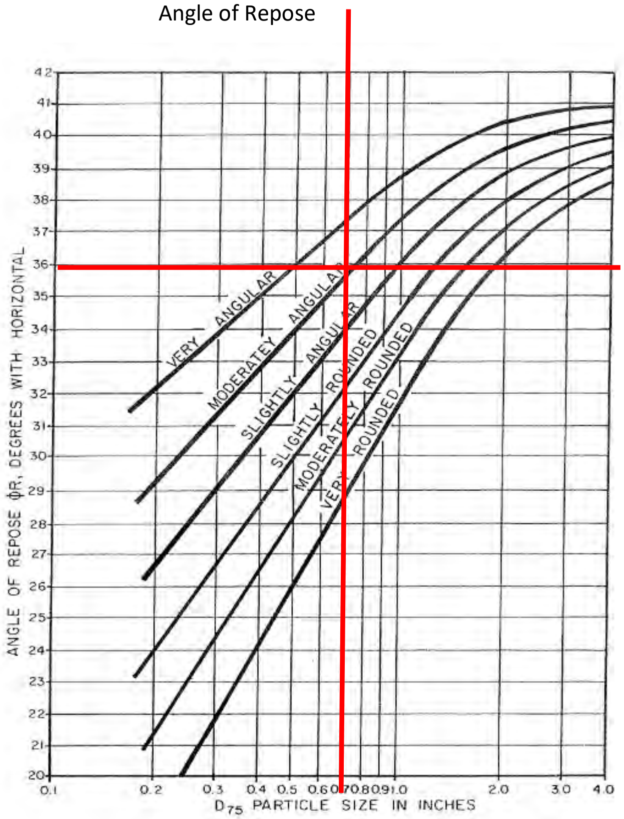
Actual Maximum Tractive Stress, τ_s on Sides of Straight Trapezoial Channels



b/y Ratio	6.17	ft/ft
Z (H/V)	4.89	ft/ft
τs/τ∞	0.77	
τs	0.115	lbs/ft^2

Allowable Tractive Stress (τ_{ls})

Angle of Repose



Moderately Angular

D_{75} 0.75 inches

From Chart (ϕR) 36 Degrees

Solving for Allowable Tractive Stress

$$0.4\left[\frac{Z^2 - Cot^2 \emptyset R}{1+Z^2}\right]^{\frac{1}{2}} D_{75}$$

0.275 lbs/ft^2

Allowable Tractive Stress, from calculation above 0.275 lb/ft^2

Calculated Tractive Stress, τ 0.12 lb/ft^2

Since the allowable tractive stress is more than the calculated tractive stress, the channel is not erosive.

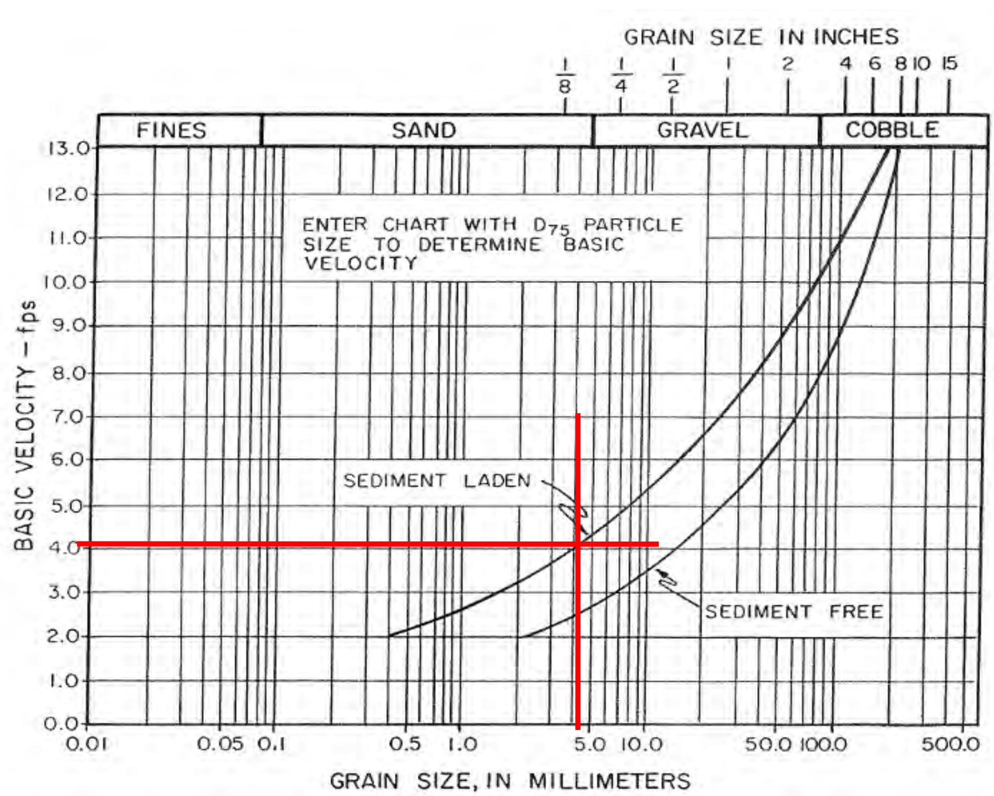
Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

Cross Section 1496 - General Information

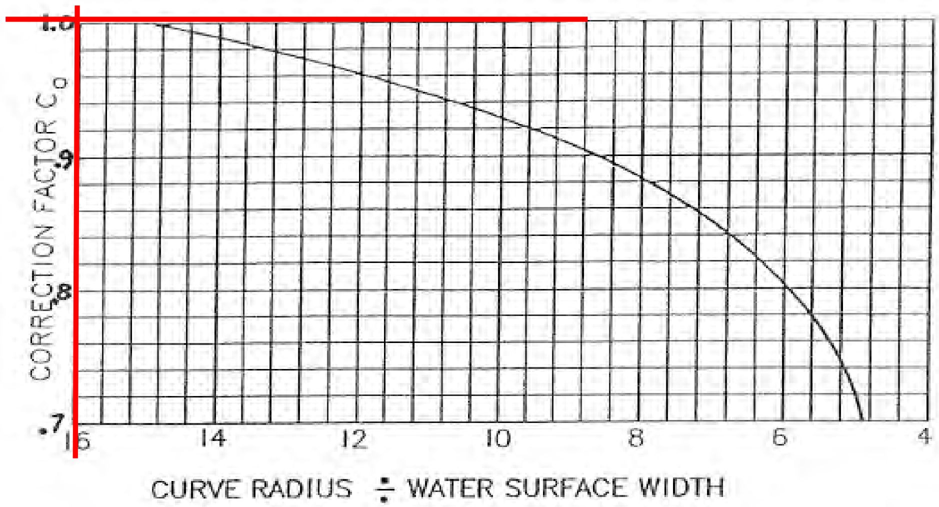
Bottom Width (b)	24.9 feet		
Side Slope (ft)	15.5 Horizontal	2.5 Vertical	
Channel Slope (S_e)	0.004 feet/foot		
Radius of Curvature (r)	0 feet		
Water Surface Width	42.38 feet		
Average Manning's n	0.034		
Flow Depth (Y)	4.29 feet		
Flow Velocity (V)	5.01 feet/second		
D_{75}	4.4 mm	0.17 inches	0.014 feet
D_{65}	3.7 mm	0.15 inches	0.012 feet
D_{50}	0.9 mm	0.04 inches	0.003 feet

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 4.2 feet/sec

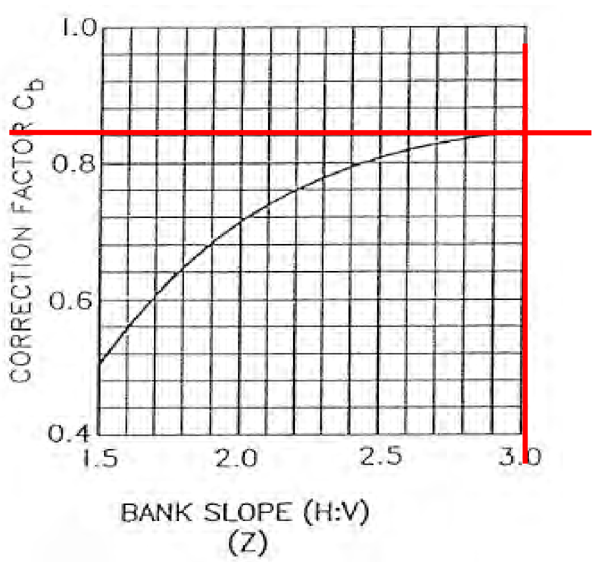


Correction Factor C_a For Channel Alignment 1



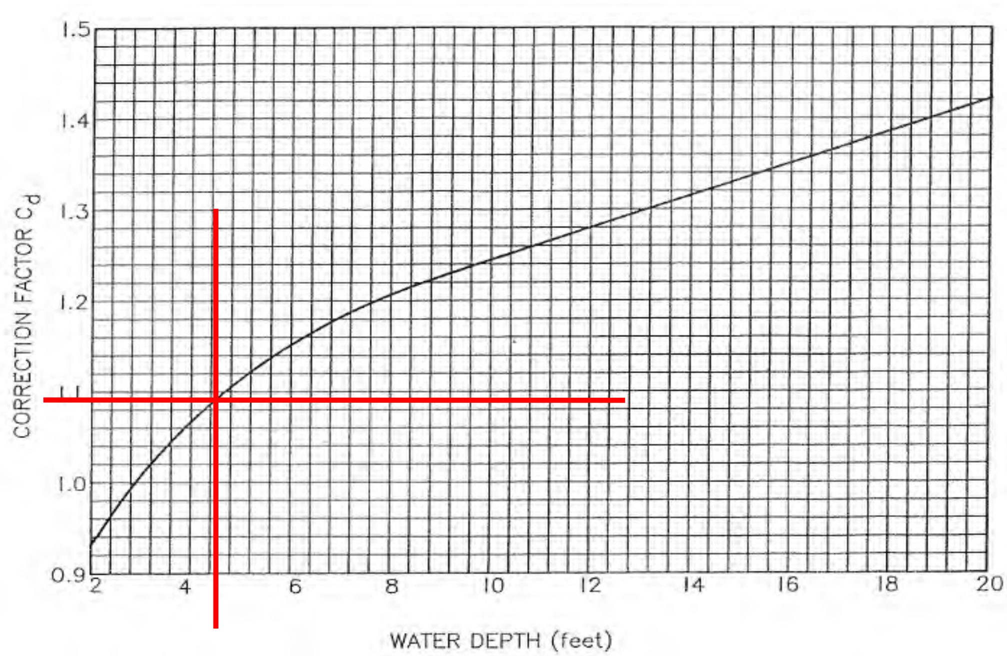
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.84



Horizontal/Vertical (Z) 6.20

Correction Factor C_d For Depth of Flow 1.09



Flow Depth (Y) 4.29 feet

Maximum Allowable Velocity 3.85 feet/second

$$V_a = V_b \ x \ C_a \ x \ C_b \ x \ C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	3.85	feet/second
Flow Velocity	5.01	feet/second

Since the computed flow velocity exceeds the maximum allowable velocity, erosion may be expected to occur

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75}	4.4 mm	conversion	0.17 inches

Since D75 is less or equal to 0.25 inches, case 2 of reference tractive stress method is used.

Assuming a water temperature of 60°F

Kinematic Velocity (ν)	0.0000121	ft^2 / sec
Density (ρ)	1.94	slugs/ft^3
Gravity	32.17	ft/sec^2

Finding Values Needed for Graphs

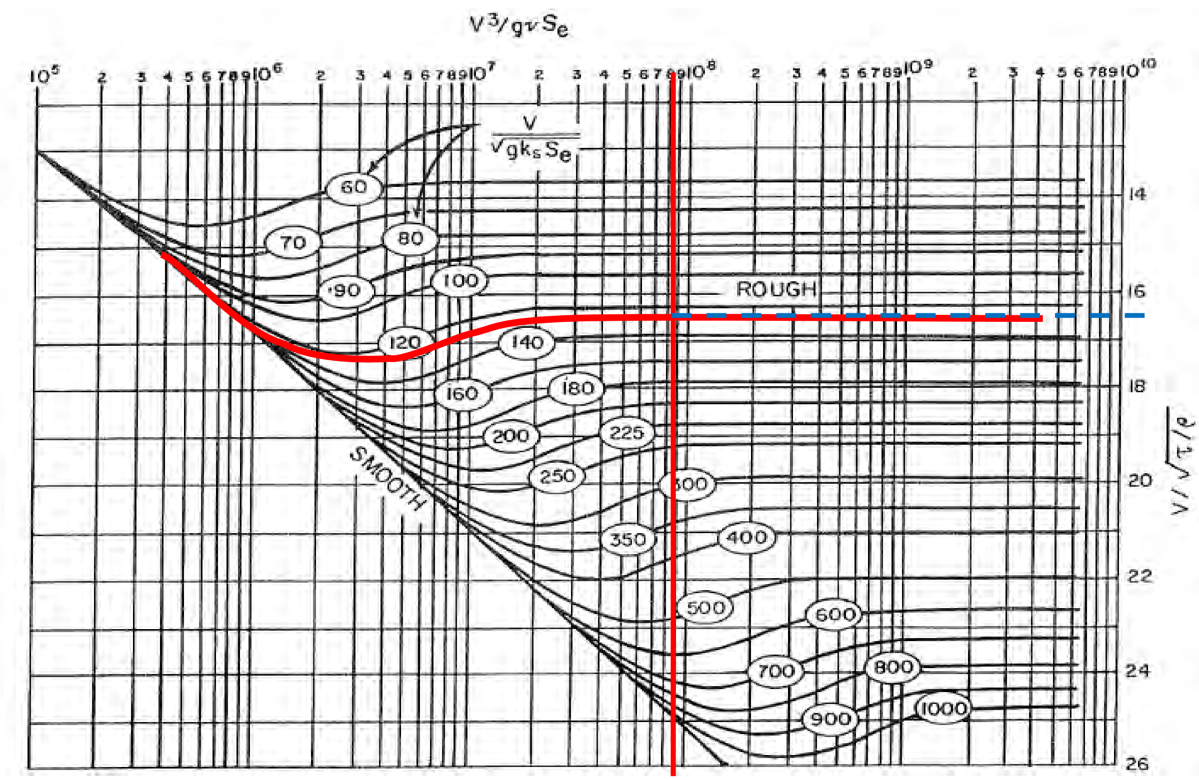
$$V^3/(gvS_e)$$

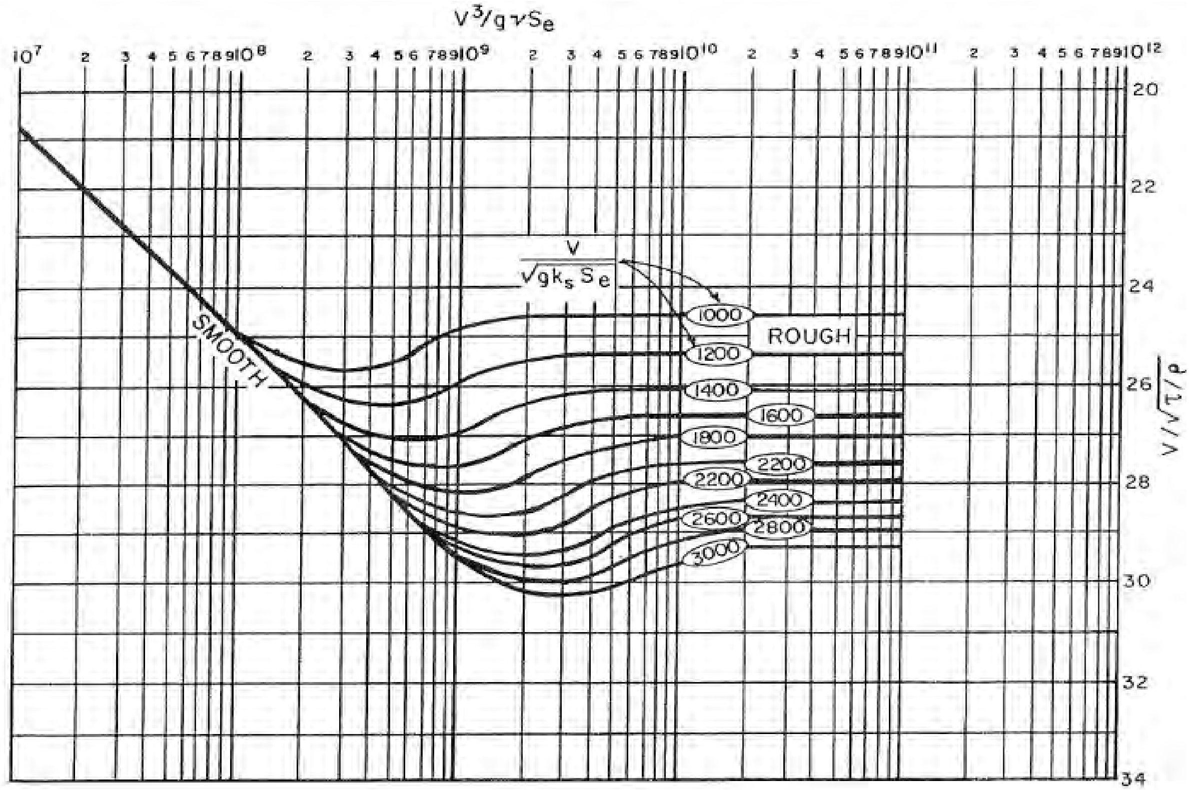
Value 1 8.08E+07

$$V/[(gD_{65}S_e)^{0.5}]$$

Value 2 126.8

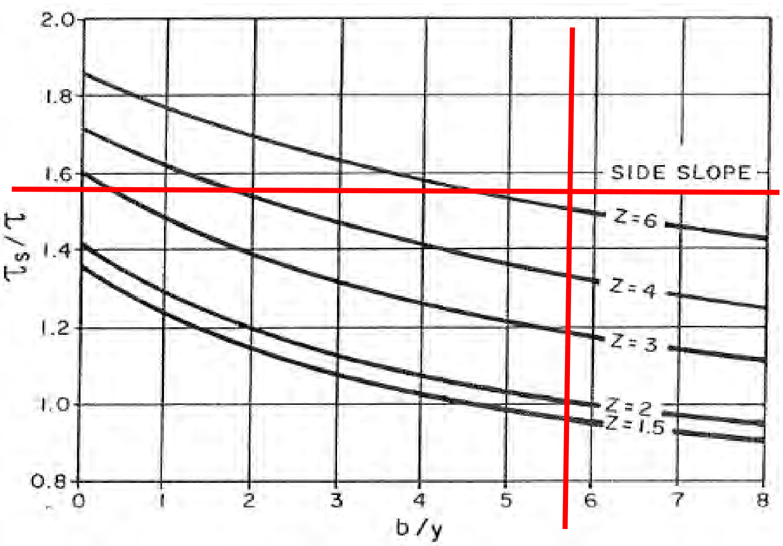
Graphic Solution of Reference Tractive Stress





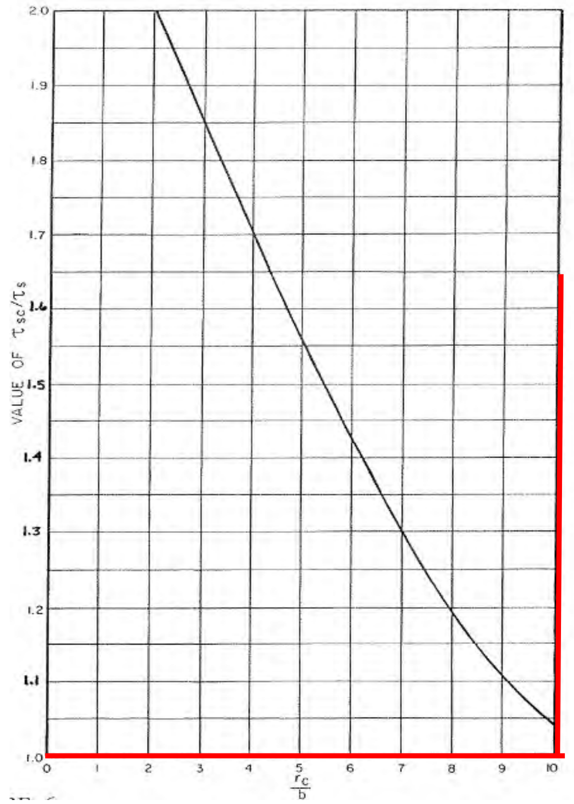
$V/\sqrt{\tau/\rho}$	From Graph Above	16.6
Solving the above equation for τ		0.18 lb/ft^2

Applied Maximum Tractive Stress, τ_s on sides of straight trapezoidal channels



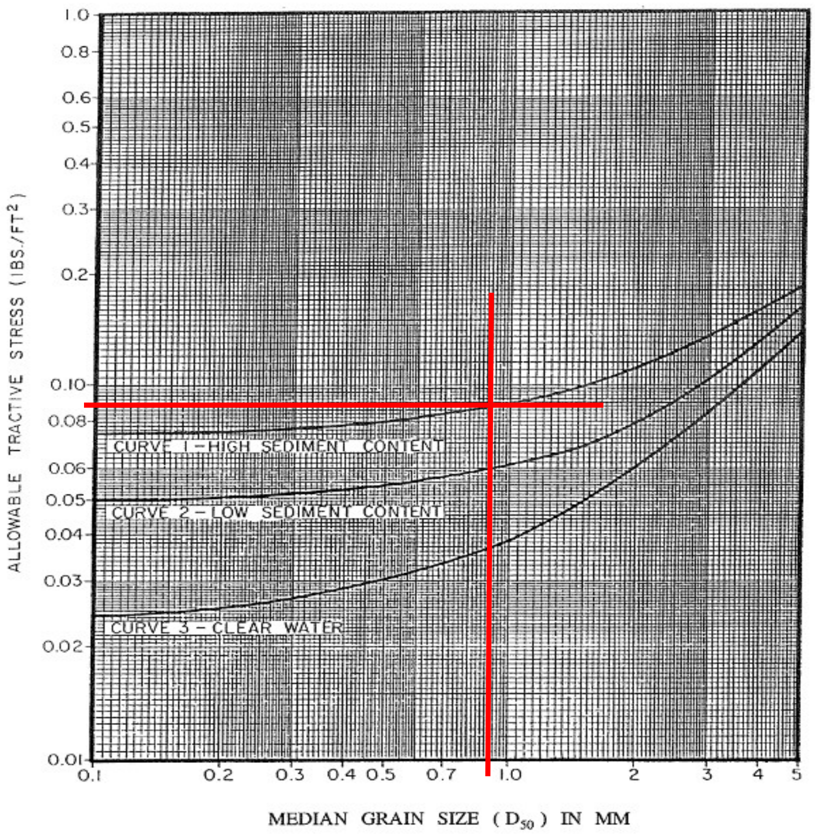
Bottom Width/Flow depth	5.80
Horizontal/Vertical (Z)	6.20
From Graph	1.57
Solving for τ_s	0.28 lb/ft^2

Actual Maximum Tractive Stress, τ_s on sides of trapezoidal channels within a curved reach



Radius of Curvature/Bottom Width	N/A
From Graph Above	1.00
Solving for τ_s	0.28 lb/ft^2

Maximum Allowable Tractive Stress For Non-Cohesive Soils, D_{75} < 0.25"



MEDIAN GRAIN SIZE (D_{50}) IN MM

D_{50}	0.9 mm	0.04 inches	0.003 feet

Allowable Tractive Stress, from graph above 0.092 lb/ft^2

Calculated Tractive Stress, τ 0.18 lb/ft^2

Since the allowable tractive stress is less than the calculated tractive stress, the channel is erosive.

Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

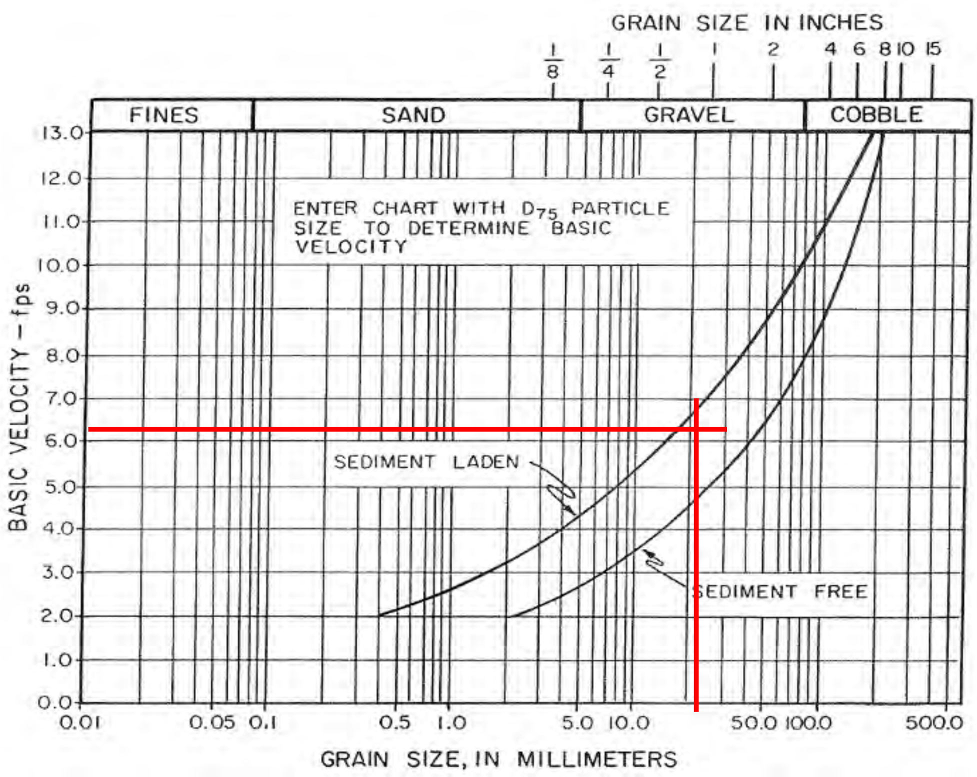
Cross Section 1496 - General Information

Bottom Width (b)	24.9 feet	
Side Slope (ft)	15.5 Horizontal	2.5 Vertical
Channel Slope (S_e)	0.004 feet/foot	
Radius of Curvature (r)	0 feet	
Water Surface Width	42.38 feet	
Average Manning's n	0.034	
Flow Depth (Y)	4.29 feet	
Flow Velocity (V)	5.01 feet/second	

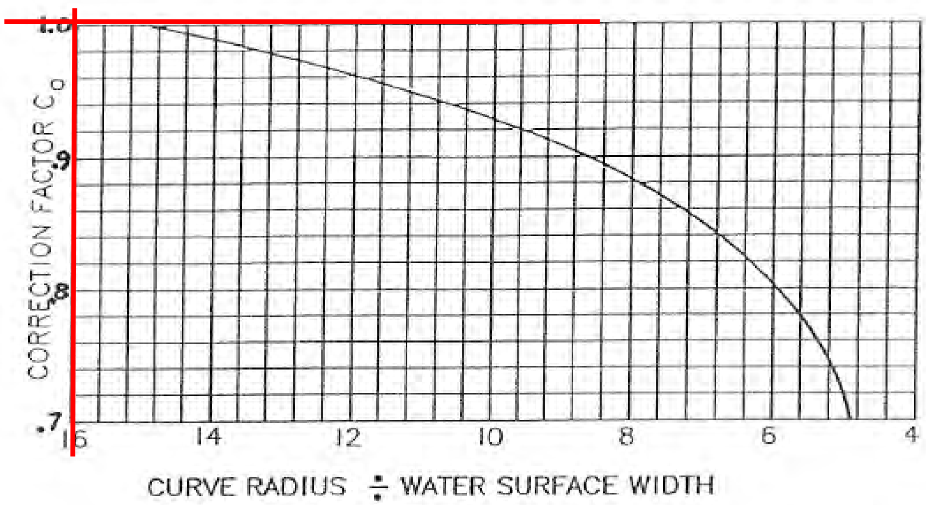
D_{75}	19 mm	0.75 inches	0.062 feet
D_{65}	9.1 mm	0.36 inches	0.030 feet
D_{50}	5.2 mm	0.20 inches	0.017 feet

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 6.3 feet/sec

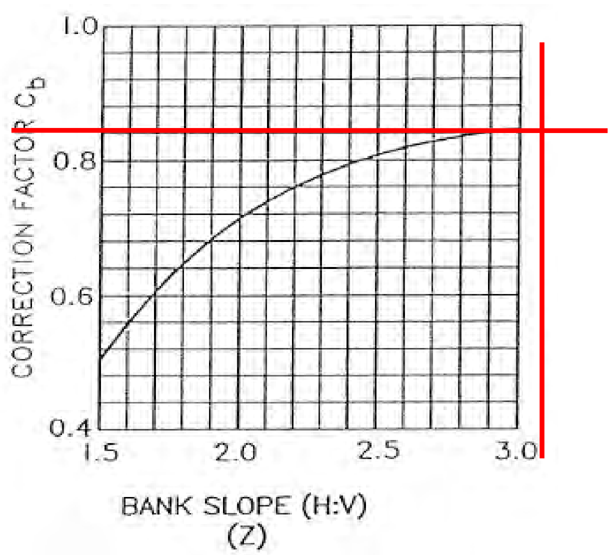


Correction Factor C_a For Channel Alignment 1



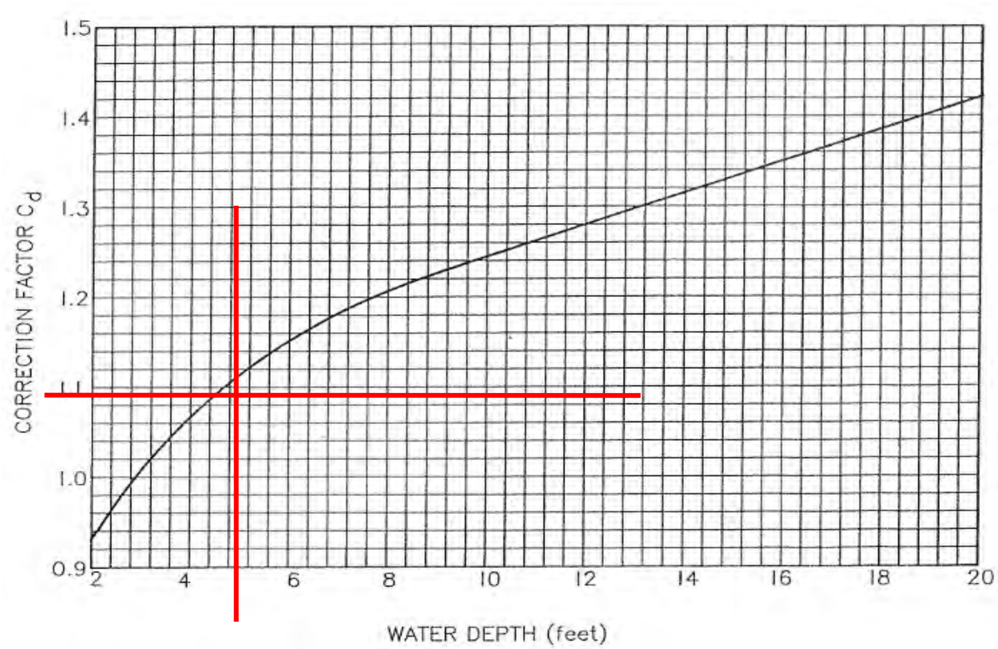
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.84



Horizontal/Vertical (Z) 6.20

Correction Factor C_d For Depth of Flow 1.09



Flow Depth (Y) 4.29 feet

Maximum Allowable Velocity 5.77 feet/second

$$V_a = V_b \ x \ C_a \ x \ C_b \ x \ C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	5.77	feet/second
Flow Velocity	5.01	feet/second

Since the computed flow velocity is less than the maximum allowable velocity, erosion is not expected to occur.

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75}	19 mm	conversion	0.75 inches

Since the D75 is more than to 0.25 inches and less than 5.0 inches, case 1 of the reference tractive method is used.

Assuming a water temperature of 60°F

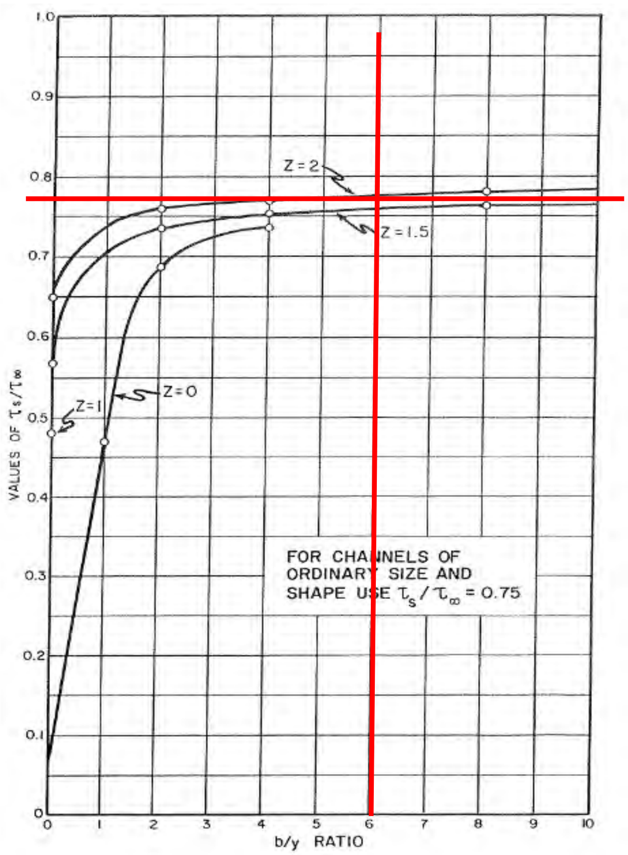
Kinematic Velocity (ν)	0.0000121 ft^2 / sec
Density (ρ)	1.94 slugs/ft^3
Gravity	32.17 ft/sec^2
Unit Weight of Water (γ)	62.4 lbs/ft^3

Tractive Stress for Soils in an infinitely Wide Channel (τ∞)

$$\gamma_w Y \left[\frac{D_{75}^{\frac{1}{6}}}{39n}\right]^2 S_e$$

0.241 lbs/ft^2

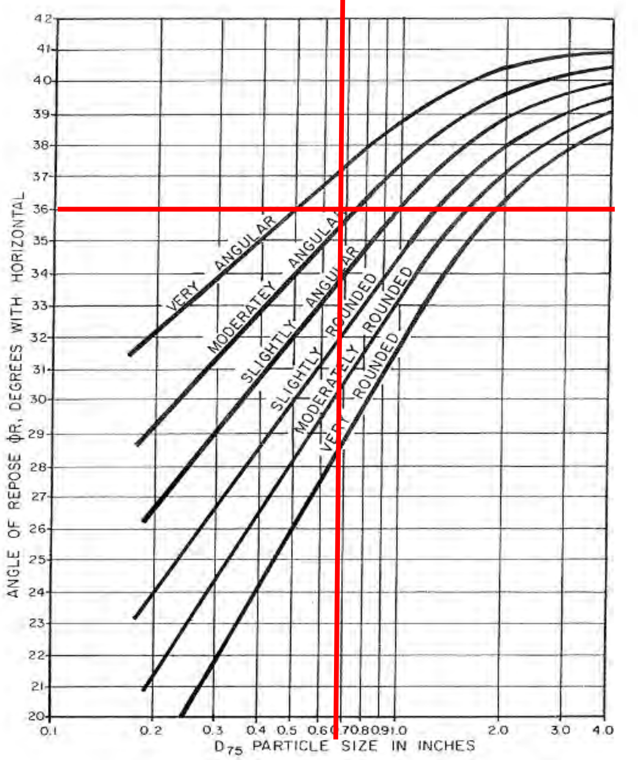
Actual Maximum Tractive Stress, τ_s on Sides of Straight Trapezoial Channels



b/y Ratio	5.80 ft/ft
Z (H/V)	6.20 ft/ft
τs/τ∞	0.77
τs	0.186 lbs/ft^2

Allowable Tractive Stress (τ_{ls})

Angle of Repose



Moderately Angular

D_{75} 0.75 inches

From Chart (ϕR) 36 Degrees

Solving for Allowable Tractive Stress

$$0.4\left[\frac{Z^2 - Cot^2 \emptyset R}{1 + Z^2}\right]^{\frac{1}{2}} D_{75}$$

0.284 lbs/ft^2

Allowable Tractive Stress, from calculation above 0.284 lb/ft^2

Calculated Tractive Stress, τ 0.19 lb/ft^2

Since the allowable tractive stress is more than the calculated tractive stress, the channel is not erosive.

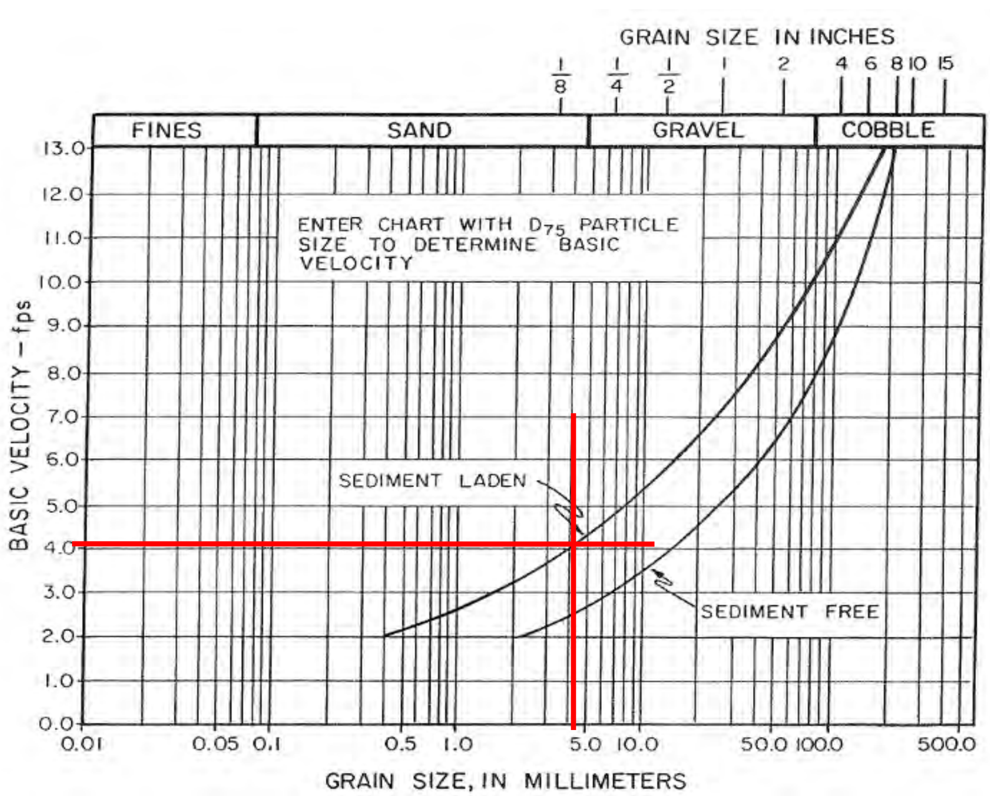
Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

Cross Section 1572 - General Information

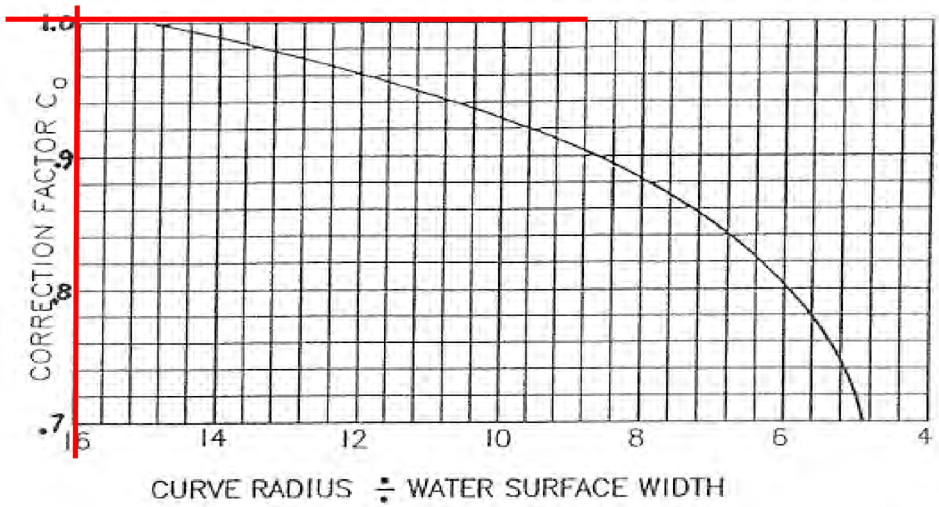
Bottom Width (b)	25.47	feet			
Side Slope (ft)	9.7	Horizontal	5.3	Vertical	
Channel Slope (S_e)	0.0012	feet/foot			
Radius of Curvature (r)	0	feet			
Water Surface Width	39.98	feet			
Average Manning's n	0.026				
Flow Depth (Y)	4.19	feet			
Flow Velocity (V)	3.48	feet/second			
D_{75}	4.4	mm	0.17 inches	0.014 feet	
D_{65}	3.7	mm	0.15 inches	0.012 feet	
D_{50}	0.9	mm	0.04 inches	0.003 feet	

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 4.2 feet/sec

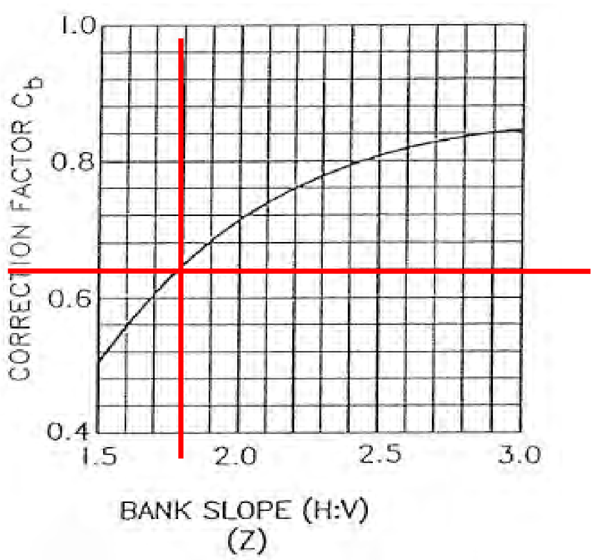


Correction Factor C_a For Channel Alignment 1



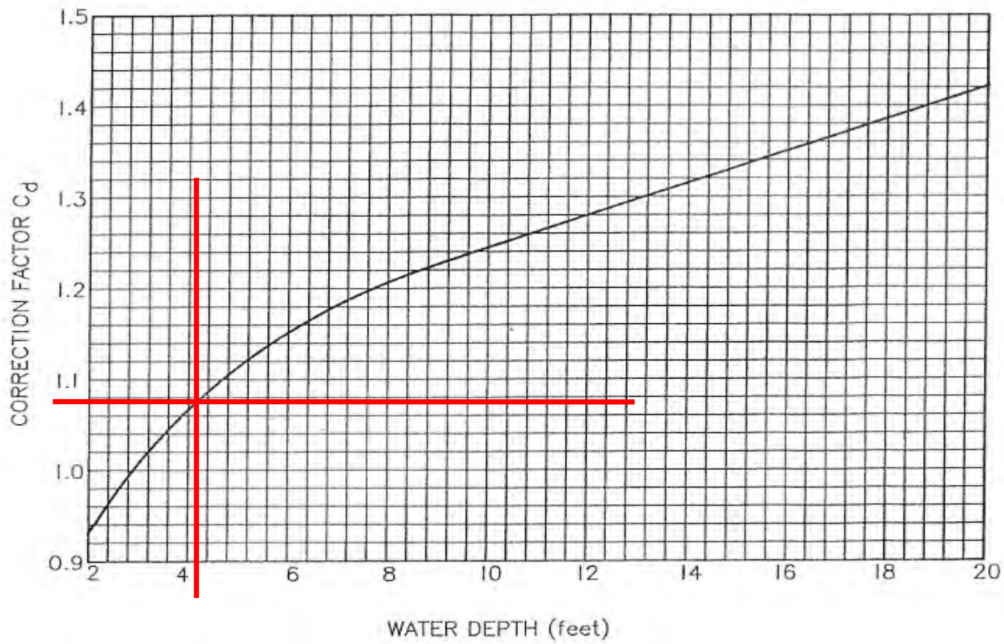
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.62



Horizontal/Vertical (Z) 1.83

Correction Factor C_d For Depth of Flow 1.08



Flow Depth (Y) 4.19 feet

Maximum Allowable Velocity 2.81 feet/second

$$V_a = V_b \; x \; C_a \; x \; C_b \; x \; C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	2.81	feet/second
Flow Velocity	3.48	feet/second

Since the computed flow velocity exceeds the maximum allowable velocity, erosion may be expected to occur

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75} 4.4 mm conversion 0.17 inches

Since D75 is less or equal to 0.25 inches, case 2 of reference tractive stress method is used.

Assuming a water temperature of 60°F

Kinematic Velocity (ν)	0.0000121	ft^2 / sec
Density (ρ)	1.94	$slugs/ft^3$
Gravity	32.17	ft/sec^2

Finding Values Needed for Graphs

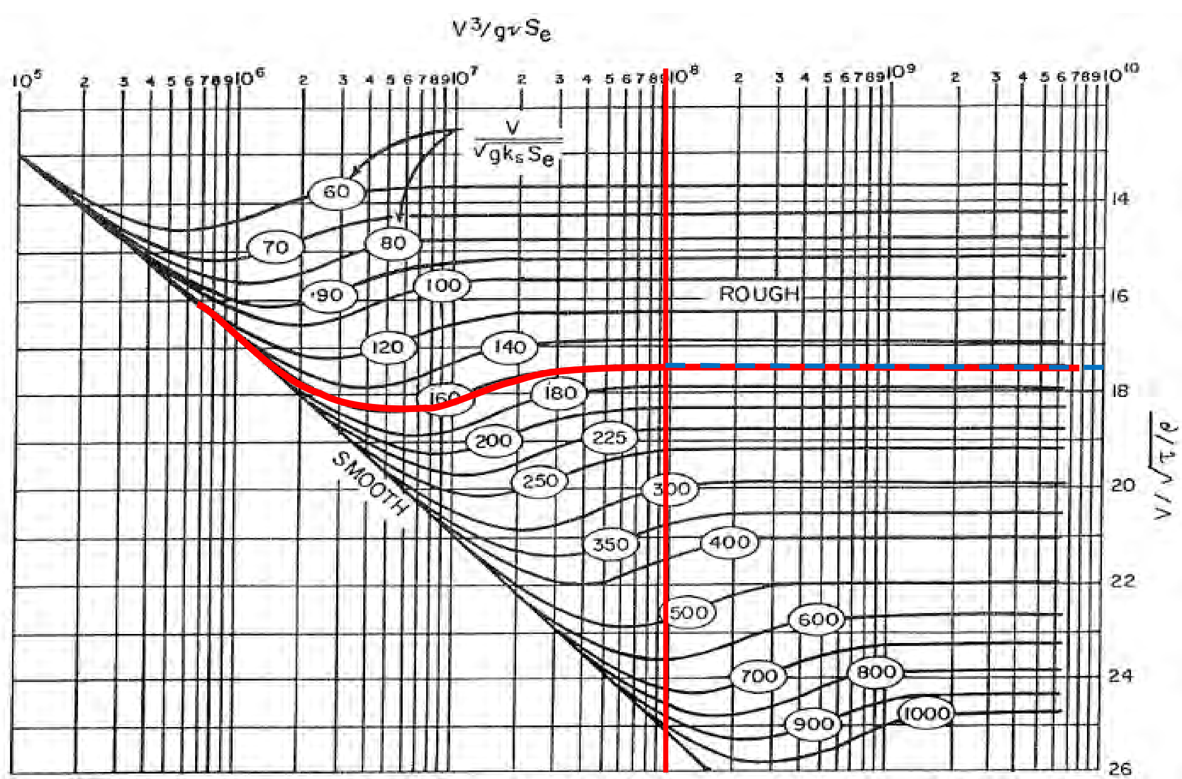
$$V^3/(gvS_e)$$

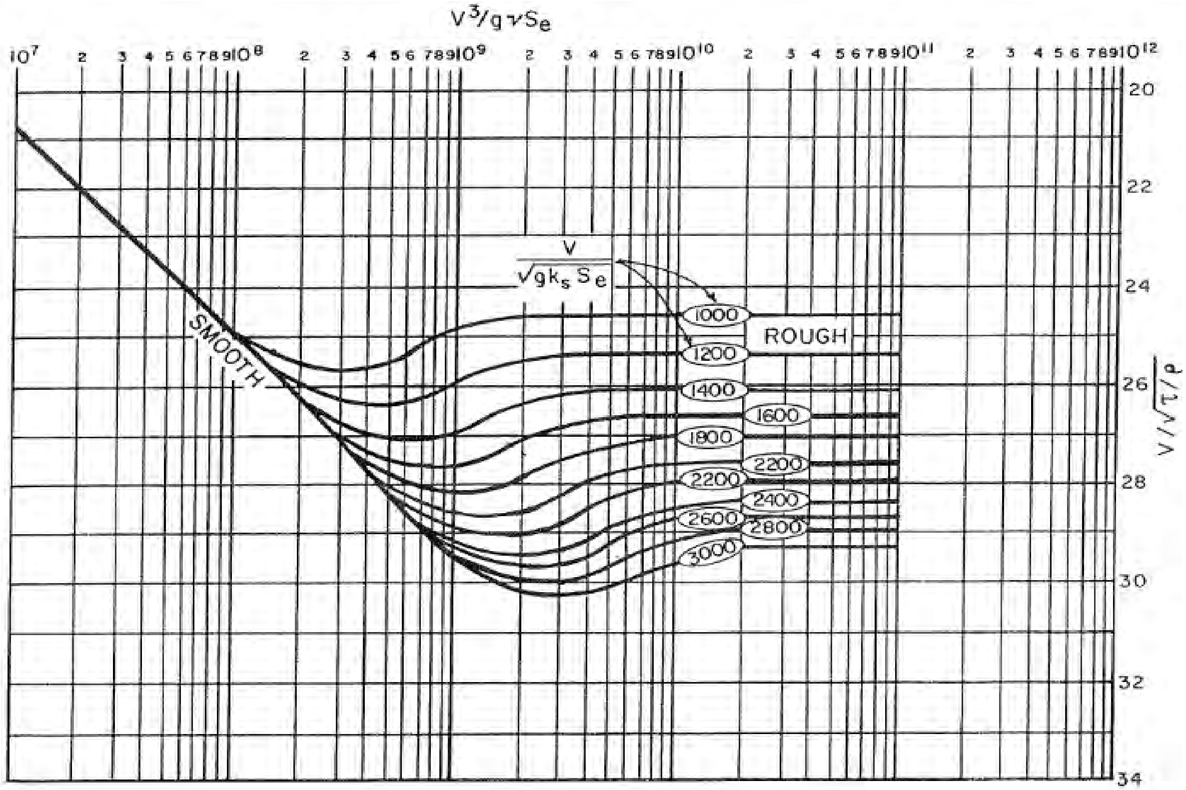
Value 1 9.02E+07

$$V/[(gD_{65}S_e)^{0.5}]$$

Value 2 160.8

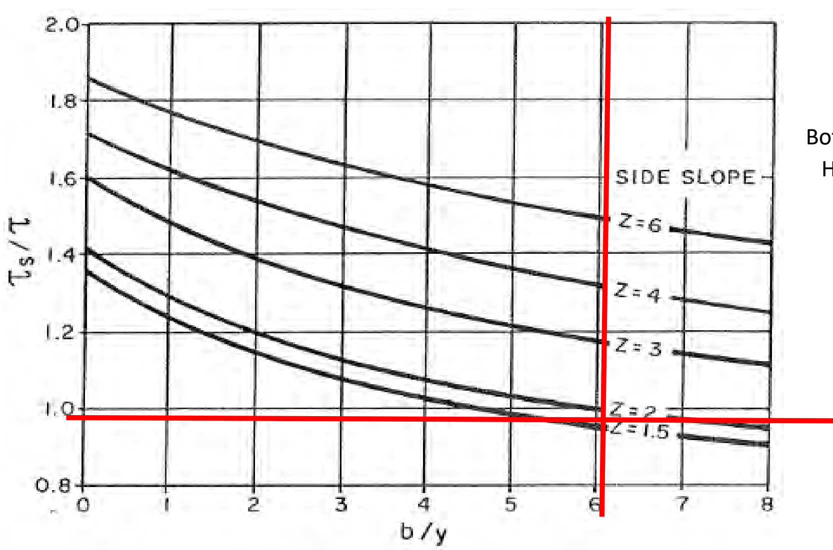
Graphic Solution of Reference Tractive Stress





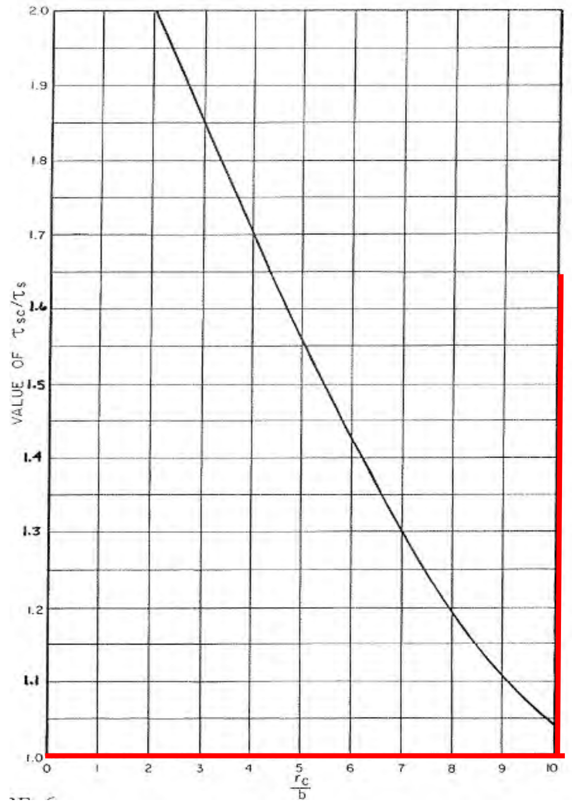
$V/\sqrt{\tau/\rho}$	From Graph Above	17.6
Solving the above equation for τ	0.08 lb/ft^2	

Applied Maximum Tractive Stress, τ_s on sides of straight trapezoidal channels



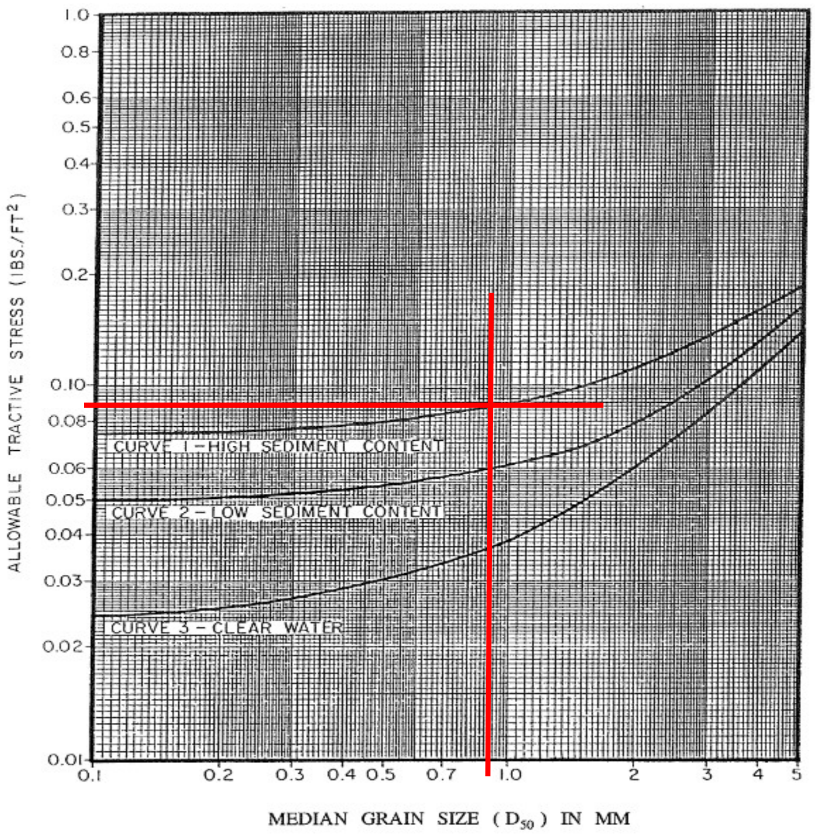
Bottom Width/Flow depth	6.08
Horizontal/Vertical (Z)	1.83
From Graph	0.98
Solving for τ_s	0.07 lb/ft^2

Actual Maximum Tractive Stress, τ_s on sides of trapezoidal channels within a curved reach



Radius of Curvature/Bottom Width	N/A
From Graph Above	1.00
Solving for τ_s	0.07 lb/ft^2

Maximum Allowable Tractive Stress For Non-Cohesive Soils, D_{75} < 0.25"



D_{50}	0.9 mm	0.04 inches	0.003 feet

Allowable Tractive Stress, from graph above 0.092 lb/ft^2

Calculated Tractive Stress, τ 0.08 lb/ft^2

Since the allowable tractive stress is more than the calculated tractive stress, the channel is not erosive.

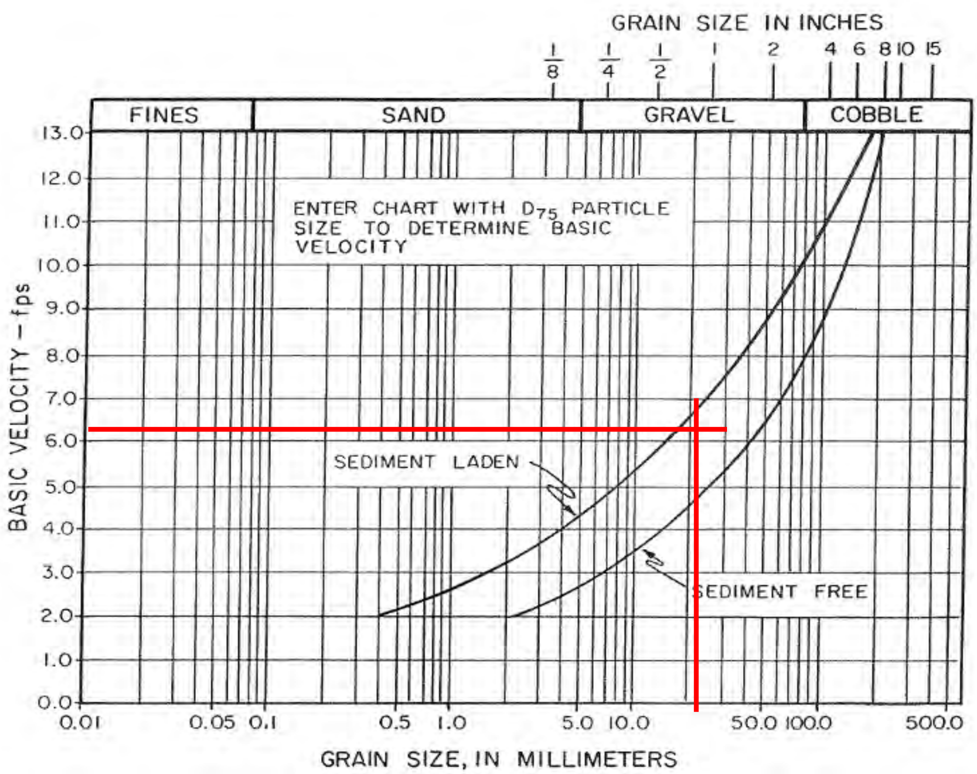
Lateral Migration Setback Allowance for Riverine Floodplains
Level 2 Analysis

Cross Section 1572 - General Information

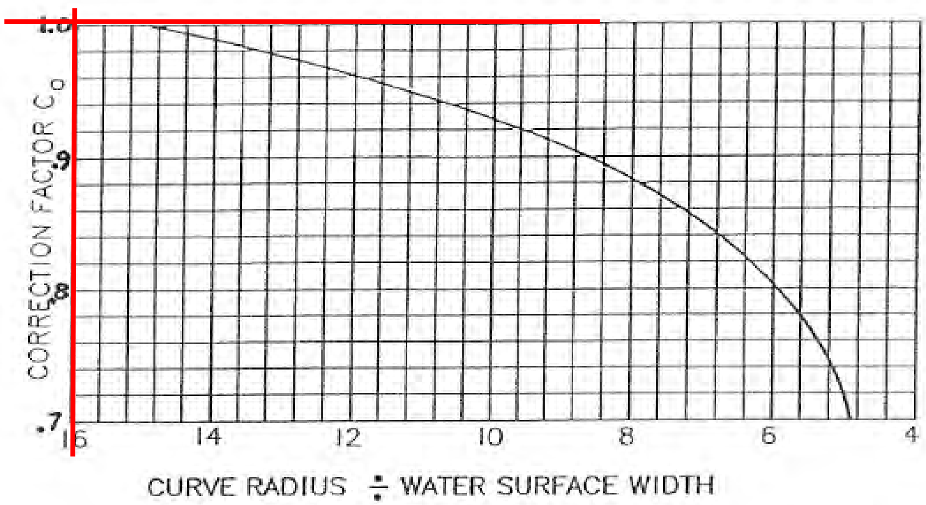
Bottom Width (b)	25.47 feet				
Side Slope (ft)	9.7 Horizontal	5.3 Vertical			
Channel Slope (S_e)	0.0012 feet/foot				
Radius of Curvature (r)	0 feet				
Water Surface Width	39.98 feet				
Average Manning's n	0.026				
Flow Depth (Y)	4.19 feet				
Flow Velocity (V)	3.48 feet/second				
D_{75}	19 mm	0.75 inches	0.062 feet		
D_{65}	9.1 mm	0.36 inches	0.030 feet		
D_{50}	5.2 mm	0.20 inches	0.017 feet		

Allowable Velocity Approach
(Assuming Sediment Laden Flow)

Base Allowable Velocity for Earth Channels V_b 6.3 feet/sec

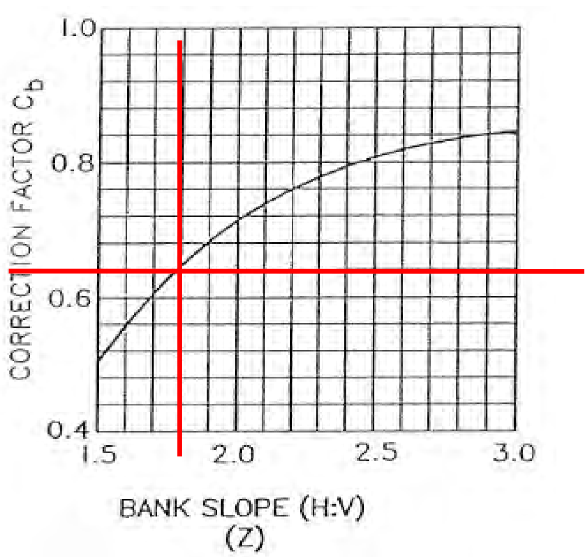


Correction Factor C_a For Channel Alignment 1



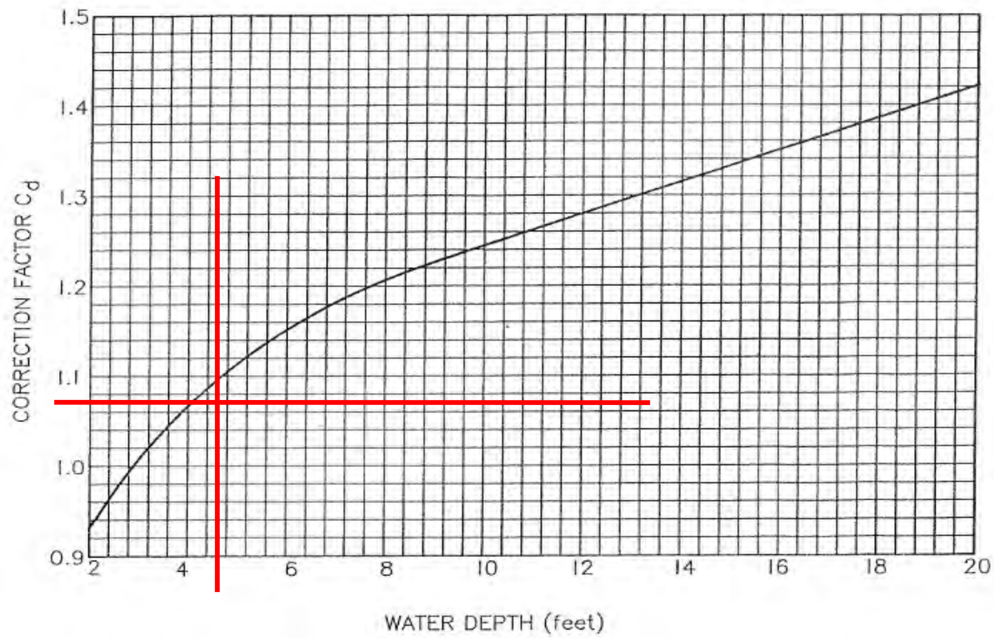
Curve Radius / Water Surface Width 0.0

Correction Factor C_b For Bank Slope 0.62



Horizontal/Vertical (Z) 1.83

Correction Factor C_d For Depth of Flow 1.07



Flow Depth (Y) 4.19 feet

Maximum Allowable Velocity 4.18 feet/second

$$V_a = V_b \ x \ C_a \ x \ C_b \ x \ C_d$$

Comparison of Flow Velocity of the Channel to the Maximum Allowable Velocity

Maximum Allowable Velocity	4.18	feet/second
Flow Velocity	3.48	feet/second

Since the computed flow velocity is less than the maximum allowable velocity, erosion is not expected to occur.

Tractive Stress Analysis
(Assuming Sediment Laden Flow)

D_{75}	19 mm	conversion	0.75 inches

Since the D75 is more than to 0.25 inches and less than 5.0 inches, case 1 of the reference tractive method is used.

Assuming a water temperature of 60°F

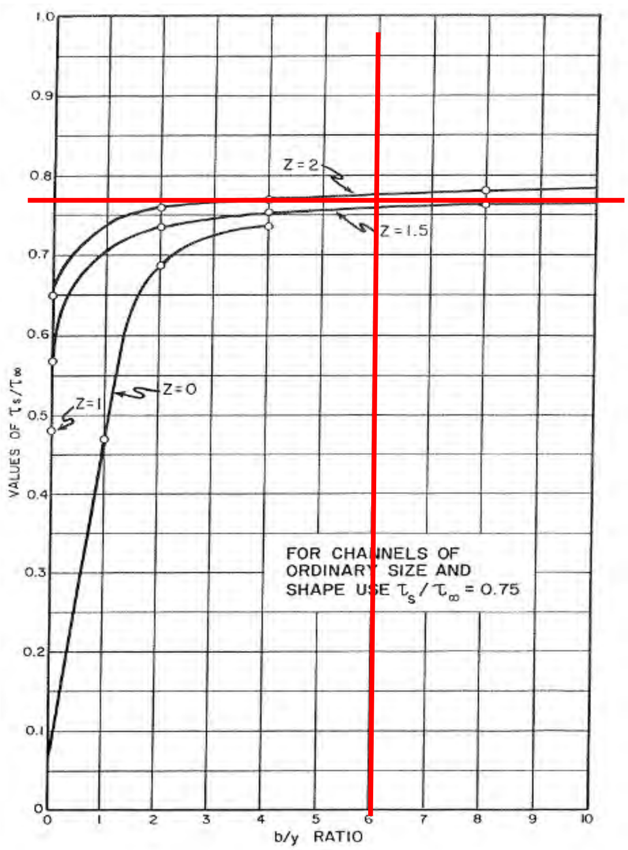
Kinematic Velocity (ν)	0.0000121 ft^2 / sec
Density (ρ)	1.94 slugs/ft^3
Gravity	32.17 ft/sec^2
Unit Weight of Water (γ)	62.4 lbs/ft^3

Tractive Stress for Soils in an infinitely Wide Channel (τ∞)

$$\gamma_w Y \left[\frac{D_{75}^{\frac{1}{6}}}{39n}\right]^2 S_e$$

0.121 lbs/ft^2

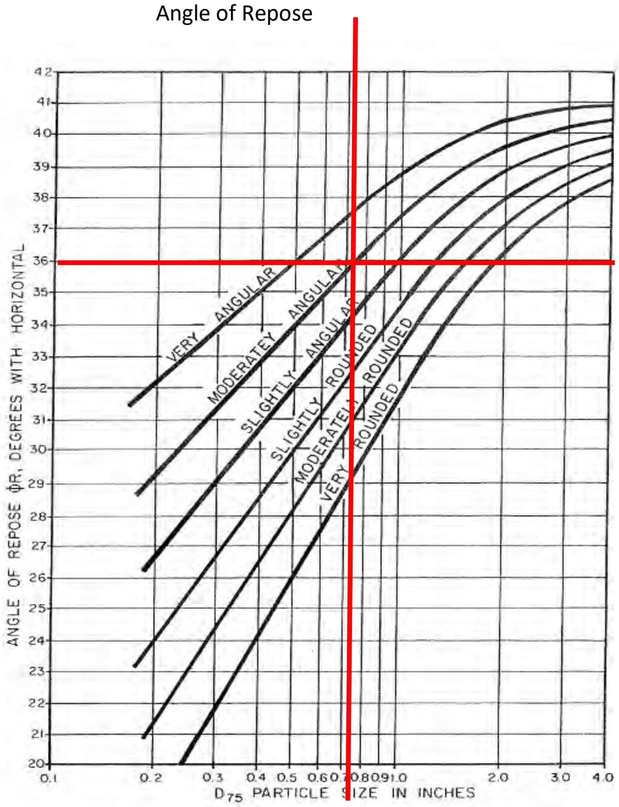
Actual Maximum Tractive Stress, τ_s on Sides of Straight Trapezoial Channels



b/y Ratio	6.08 ft/ft
Z (H/V)	1.83 ft/ft
τs/τ∞	0.77
τs	0.093 lbs/ft^2

Allowable Tractive Stress (τ_{ls})

Angle of Repose



Moderately Angular

D_{75} 0.75 inches

From Chart (ϕR) 36 Degrees

Solving for Allowable Tractive Stress

$$0.4\left[\frac{Z^2 - Cot^2 \emptyset R}{1 + Z^2}\right]^{\frac{1}{2}} D_{75}$$

0.176 lbs/ft^2

Allowable Tractive Stress, from calculation above 0.176 lb/ft^2

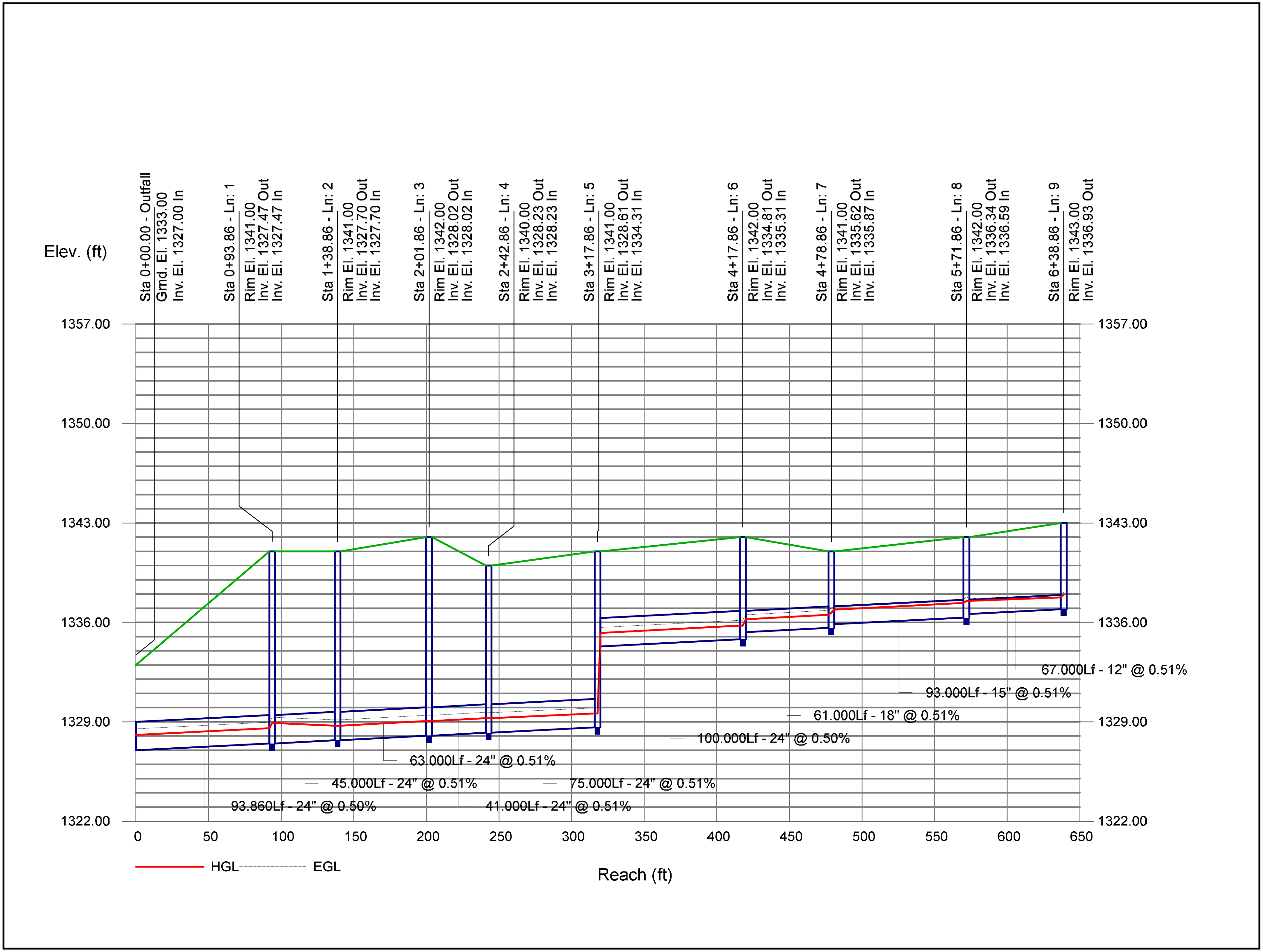
Calculated Tractive Stress, τ 0.09 lb/ft^2

Since the allowable tractive stress is more than the calculated tractive stress, the channel is not erosive.

Appendix I – STORM DRAIN PROFILES

Storm Sewer Profile

Proj. file: Gold Dust Storm Drain.stm



Storm Sewer Inventory Report

Line No.	Alignment				Flow Data				Physical Data								Line ID
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
1	End	93.860	37.070	DrGrt	0.64	0.00	0.00	0.0	1327.00	0.50	1327.47	24	Cir	0.013	0.84	1341.00	
2	1	45.000	-30.321	DrGrt	0.01	0.00	0.00	0.0	1327.47	0.51	1327.70	24	Cir	0.013	1.48	1341.00	
3	2	63.000	-80.804	DrGrt	0.01	0.00	0.00	0.0	1327.70	0.51	1328.02	24	Cir	0.013	1.45	1342.00	
4	3	41.000	74.055	DrGrt	0.70	0.00	0.00	0.0	1328.02	0.51	1328.23	24	Cir	0.013	1.50	1340.00	
5	4	75.000	-90.000	DrGrt	0.32	0.00	0.00	0.0	1328.23	0.51	1328.61	24	Cir	0.013	0.50	1341.00	
6	5	100.000	-0.541	DrGrt	2.25	0.00	0.00	0.0	1334.31	0.50	1334.81	24	Cir	0.013	0.50	1342.00	
7	6	61.000	1.199	DrGrt	0.50	0.00	0.00	0.0	1335.31	0.51	1335.62	18	Cir	0.013	0.50	1341.00	
8	7	93.000	-0.049	DrGrt	2.20	0.00	0.00	0.0	1335.87	0.51	1336.34	15	Cir	0.013	0.50	1342.00	
9	8	67.000	0.259	DrGrt	2.40	0.00	0.00	0.0	1336.59	0.51	1336.93	12	Cir	0.013	1.00	1343.00	

Project File: Gold Dust Storm Drain.stm	Number of lines: 9	Date: 2/2/2023

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns Line No.	Junction Type
1		9.03	24	Cir	93.860	1327.00	1327.47	0.501	1328.08	1328.55	0.36	1328.91	End	DropGrate
2		8.39	24	Cir	45.000	1327.47	1327.70	0.511	1328.91	1328.73	n/a	1328.73	1	DropGrate
3		8.38	24	Cir	63.000	1327.70	1328.02	0.508	1328.73	1329.05	n/a	1329.05	2	DropGrate
4		8.37	24	Cir	41.000	1328.02	1328.23	0.512	1329.05	1329.26	n/a	1329.26	3	DropGrate
5		7.67	24	Cir	75.000	1328.23	1328.61	0.507	1329.26	1329.59	n/a	1329.59	4	DropGrate
6		7.35	24	Cir	100.000	1334.31	1334.81	0.500	1335.26	1335.77	n/a	1335.77	5	DropGrate
7		5.10	18	Cir	61.000	1335.31	1335.62	0.508	1336.22	1336.53	0.16	1336.69	6	DropGrate
8		4.60	15	Cir	93.000	1335.87	1336.34	0.505	1336.90	1337.37	0.14	1337.51	7	DropGrate
9		2.40	12	Cir	67.000	1336.59	1336.93	0.508	1337.51	1337.76	0.18	1337.95	8	DropGrate

Project File: Gold Dust Storm Drain.stm	Number of lines: 9	Run Date: 2/2/2023
NOTES: Known Qs only		

Storm Sewer Tabulation

Station		Len	Drng Area		Rnoff coeff	Area x C		Tc		Rain (I)	Total flow	Cap full	Vel	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr	Total		Incr	Total	Inlet	Syst					Size	Slope	Dn	Up	Dn	Up	Dn	Up	
		(ft)	(ac)	(ac)	(C)			(min)	(min)	(in/hr)	(cfs)	(cfs)	(ft/s)	(in)	(%)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
1	End	93.860	0.00	0.00	0.00	0.00	0.00	0.0	3.3	0.0	9.03	16.00	5.23	24	0.50	1327.00	1327.47	1328.08	1328.55	0.00	1341.00	
2	1	45.000	0.00	0.00	0.00	0.00	0.00	0.0	3.0	0.0	8.39	16.17	4.31	24	0.51	1327.47	1327.70	1328.91	1328.73	1341.00	1341.00	
3	2	63.000	0.00	0.00	0.00	0.00	0.00	0.0	2.6	0.0	8.38	16.12	5.13	24	0.51	1327.70	1328.02	1328.73	1329.05	1341.00	1342.00	
4	3	41.000	0.00	0.00	0.00	0.00	0.00	0.0	2.4	0.0	8.37	16.19	5.13	24	0.51	1328.02	1328.23	1329.05	1329.26	1342.00	1340.00	
5	4	75.000	0.00	0.00	0.00	0.00	0.00	0.0	1.8	0.0	7.67	16.10	4.84	24	0.51	1328.23	1328.61	1329.26	1329.59	1340.00	1341.00	
6	5	100.000	0.00	0.00	0.00	0.00	0.00	0.0	1.1	0.0	7.35	15.99	4.95	24	0.50	1334.31	1334.81	1335.26	1335.77	1341.00	1342.00	
7	6	61.000	0.00	0.00	0.00	0.00	0.00	0.0	0.8	0.0	5.10	7.49	4.55	18	0.51	1335.31	1335.62	1336.22	1336.53	1342.00	1341.00	
8	7	93.000	0.00	0.00	0.00	0.00	0.00	0.0	0.4	0.0	4.60	4.59	4.26	15	0.51	1335.87	1336.34	1336.90	1337.37	1341.00	1342.00	
9	8	67.000	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	2.40	2.54	3.31	12	0.51	1336.59	1336.93	1337.51	1337.76	1342.00	1343.00	

Project File: Gold Dust Storm Drain.stm

Number of lines: 9

Run Date: 2/2/2023

NOTES:Known Qs only ; c = cir e = ellip b = box

MyReport

Line No.	Area Dn (sqft)	Area Up (sqft)	Byp Ln No	Coeff C1 (C)	Coeff C2 (C)	Coeff C3 (C)	Capac Full (cfs)	Crit Depth (ft)	Cross Sl, Sw (ft/ft)	Cross Sl, Sx (ft/ft)	Curb Len (ft)	Defl Ang (Deg)	Depth Dn (ft)	Depth Up (ft)	DnStm Ln No	Drng Area (ac)	Easting X (ft)	EGL Dn (ft)	EGL Up (ft)	Energy Loss (ft)
1	1.71	1.72	Sag	0.20	0.50	0.90	16.00	1.07	0.050	0.020	....	37.070	1.08	1.08**	Outfall	0.00	264.55	1328.50	1328.97	0.465
2	1.63	1.63	1	0.20	0.50	0.90	16.17	1.03	0.050	0.020	....	-30.321	1.44	1.03**	1	0.00	309.23	1329.32	1329.14	0.000
3	1.63	1.63	2	0.20	0.50	0.90	16.12	1.03	0.050	0.020	....	-80.804	1.03	1.03**	2	0.00	326.54	1329.14	1329.46	0.000
4	1.63	1.63	3	0.20	0.50	0.90	16.19	1.03	0.050	0.020	....	74.055	1.03	1.03**	3	0.00	367.54	1329.46	1329.67	0.000
5	1.54	1.54	4	0.20	0.50	0.90	16.10	0.98	0.050	0.020	....	-90.000	1.03	0.98**	4	0.00	367.54	1329.65	1329.98	0.000
6	1.48	1.50	5	0.20	0.50	0.90	15.99	0.96	0.050	0.020	....	-0.541	0.95	0.96**	5	0.00	366.60	1335.64	1336.15	0.000
7	1.12	1.12	6	0.20	0.50	0.90	7.49	0.87	0.050	0.020	....	1.199	0.91	0.91	6	0.00	367.30	1336.54	1336.85	0.309
8	1.08	1.08	7	0.20	0.50	0.90	4.59	0.87	0.050	0.020	....	-0.049	1.03	1.03	7	0.00	368.29	1337.18	1337.65	0.469
9	0.76	0.70	8	0.20	0.50	0.90	2.54	0.66	0.050	0.020	....	0.259	0.92	0.83	8	0.00	369.30	1337.67	1337.95	0.280

Project File: Gold Dust Storm Drain.stm | Number of lines: 9 | Date: 2/2/2023

NOTES: ** Critical depth

MyReport

Flow Rate (cfs)	Sf Ave (ft/ft)	Sf Dn (ft/ft)	Grate Area (sqft)	Grate Len (ft)	Grate Width (ft)	Gnd/Rim El Dn (ft)	Gnd/Rim El Up (ft)	Gutter Depth (ft)	Gutter Slope (ft/ft)	Gutter Spread (ft)	Gutter Width (ft)	HGL Dn (ft)	HGL Up (ft)	HGL Jnct (ft)	HGL Jmp Dn (ft)	HGL Jmp Up (ft)	Incr CxA	Incr Q (cfs)	Inlet Depth (ft)	Inlet Eff (%)
9.03	0.496	0.493	2.00	2.00	2.00	0.00	1341.00	0.09	Sag	10.92	2.00	1328.08	1328.55	1328.91	....	....	0.00	0.64	0.09	100
8.39	0.000	0.000	2.00	2.00	2.00	1341.00	1341.00	0.01	Sag	2.56	2.00	1328.91	1328.73	1328.73	....	....	0.00	0.01	0.01	100
8.38	0.000	0.000	2.00	2.00	2.00	1341.00	1342.00	0.01	Sag	2.56	2.00	1328.73	1329.05	1329.05	....	....	0.00	0.01	0.01	100
8.37	0.000	0.000	2.00	2.00	2.00	1342.00	1340.00	0.09	Sag	11.46	2.00	1329.05	1329.26	1329.26	....	....	0.00	0.70	0.09	100
7.67	0.000	0.000	2.00	2.00	2.00	1340.00	1341.00	0.06	Sag	7.61	2.00	1329.26	1329.59	1329.59	....	....	0.00	0.32	0.06	100
7.35	0.000	0.000	2.00	2.00	2.00	1341.00	1342.00	0.21	Sag	22.62	2.00	1335.26	1335.77	1335.77	....	....	0.00	2.25	0.21	100
5.10	0.507	0.507	2.00	2.00	2.00	1342.00	1341.00	0.08	Sag	9.56	2.00	1336.22	1336.53	1336.69	....	....	0.00	0.50	0.08	100
4.60	0.505	0.505	2.00	2.00	2.00	1341.00	1342.00	0.20	Sag	22.31	2.00	1336.90	1337.37	1337.51	....	....	0.00	2.20	0.20	100
2.40	0.418	0.394	2.00	2.00	2.00	1342.00	1343.00	0.22	Sag	23.53	2.00	1337.51	1337.76	1337.95	....	....	0.00	2.40	0.22	100

Project File: Gold Dust Storm Drain.stm | Number of lines: 9 | Date: 2/2/2023

NOTES: ** Critical depth

MyReport

Line Length (ft)	Line Size (in)	Line Slope (%)	Line Type	Local Depr (in)	n-val Gutter	n-val Pipe	Minor Loss (ft)	Northing Y (ft)	Pipe Travel (min)	Q Byp (cfs)	Q Capt (cfs)	Q Carry (cfs)	Line Rise (in)	Runoff Coeff (C)	Line Span (in)	Area A1 (ac)	Area A2 (ac)	Area A3 (ac)	Tc (min)	Throat Ht (in)	Total Area (ac)	Total CxA
93.860	24	0.50	Cir	....	....	0.013	0.36	82.24	0.54	0.00	0.64	0.00	24	0.00	24	0.00	0.00	0.00	3.3	....	0.00	0.00
45.000	24	0.51	Cir	....	....	0.013	n/a	76.95	0.28	0.00	0.01	0.00	24	0.00	24	0.00	0.00	0.00	3.0	....	0.00	0.00
63.000	24	0.51	Cir	....	....	0.013	n/a	137.52	0.39	0.00	0.01	0.00	24	0.00	24	0.00	0.00	0.00	2.6	....	0.00	0.00
41.000	24	0.51	Cir	....	....	0.013	n/a	137.52	0.26	0.00	0.70	0.00	24	0.00	24	0.00	0.00	0.00	2.4	....	0.00	0.00
75.000	24	0.51	Cir	....	....	0.013	n/a	212.52	0.51	0.00	0.32	0.00	24	0.00	24	0.00	0.00	0.00	1.8	....	0.00	0.00
100.000	24	0.50	Cir	....	....	0.013	n/a	312.52	0.71	0.00	2.25	0.00	24	0.00	24	0.00	0.00	0.00	1.1	....	0.00	0.00
61.000	18	0.51	Cir	....	....	0.013	0.16	373.52	0.35	0.00	0.50	0.00	18	0.00	18	0.00	0.00	0.00	0.8	....	0.00	0.00
93.000	15	0.51	Cir	....	....	0.013	0.14	466.51	0.41	0.00	2.20	0.00	15	0.00	15	0.00	0.00	0.00	0.4	....	0.00	0.00
67.000	12	0.51	Cir	....	....	0.013	0.18	533.50	0.37	0.00	2.40	0.00	12	0.00	12	0.00	0.00	0.00	0.0	....	0.00	0.00

Project File: Gold Dust Storm Drain.stm | Number of lines: 9 | Date: 2/2/2023

NOTES: ** Critical depth

Hydraulic Grade Line Computations

Line	Size	Q	Downstream								Len	Upstream								Check		JL coeff	Minor loss
			Invert elev	HGL elev	Depth	Area	Vel	Vel head	EGL elev	Sf		Invert elev	HGL elev	Depth	Area	Vel	Vel head	EGL elev	Sf	Ave Sf	Enrgy loss		
	(in)	(cfs)	(ft)	(ft)	(ft)	(sqft)	(ft/s)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)	(sqft)	(ft/s)	(ft)	(ft)	(%)	(%)	(ft)	(K)	(ft)
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)	(21)	(22)	(23)	(24)
1	24	9.03	1327.00	1328.08	1.08	1.71	5.22	0.42	1328.50	0.493	93.860	1327.47	1328.55	1.08**	1.72	5.24	0.43	1328.97	0.498	0.496	0.465	0.84	0.36
2	24	8.39	1327.47	1328.91	1.44	1.63	3.48	0.41	1329.32	0.000	45.000	1327.70	1328.73	1.03**	1.63	5.14	0.41	1329.14	0.000	0.000	n/a	1.48	n/a
3	24	8.38	1327.70	1328.73	1.03	1.63	5.13	0.41	1329.14	0.000	63.000	1328.02	1329.05	1.03**	1.63	5.13	0.41	1329.46	0.000	0.000	n/a	1.45	n/a
4	24	8.37	1328.02	1329.05	1.03	1.63	5.13	0.41	1329.46	0.000	41.000	1328.23	1329.26	1.03**	1.63	5.13	0.41	1329.67	0.000	0.000	n/a	1.50	n/a
5	24	7.67	1328.23	1329.26	1.03	1.54	4.70	0.39	1329.65	0.000	75.000	1328.61	1329.59	0.98**	1.54	4.98	0.39	1329.98	0.000	0.000	n/a	0.50	n/a
6	24	7.35	1334.31	1335.26	0.95*	1.48	4.98	0.38	1335.64	0.000	100.000	1334.81	1335.77	0.96**	1.50	4.92	0.38	1336.15	0.000	0.000	n/a	0.50	n/a
7	18	5.10	1335.31	1336.22	0.91*	1.12	4.55	0.32	1336.54	0.507	61.000	1335.62	1336.53	0.91	1.12	4.55	0.32	1336.85	0.507	0.507	0.309	0.50	0.16
8	15	4.60	1335.87	1336.90	1.03*	1.08	4.26	0.28	1337.18	0.505	93.000	1336.34	1337.37	1.03	1.08	4.26	0.28	1337.65	0.504	0.505	0.469	0.50	0.14
9	12	2.40	1336.59	1337.51	0.92	0.76	3.18	0.16	1337.67	0.394	67.000	1336.93	1337.76	0.83	0.70	3.44	0.18	1337.95	0.442	0.418	0.280	1.00	0.18

Project File: Gold Dust Storm Drain.stm | Number of lines: 9 | Run Date: 2/2/2023

Notes: * Normal depth assumed; ** Critical depth. ; c = cir e = ellip b = box

Hydraflow HGL Computation Procedure

General Procedure:

Hydraflow computes the HGL using the Bernoulli energy equation. Manning's equation is used to determine energy losses due to pipe friction. In a standard step, iterative procedure, Hydraflow assumes upstream HGLs until the energy equation balances. If the energy equation cannot balance, supercritical flow exists and critical depth is temporarily assumed at the upstream end. A supercritical flow Profile is then computed using the same procedure in a downstream direction using momentum principles.

Col. 1 The line number being computed. Calculations begin at Line 1 and proceed upstream.

Col. 2 The line size. In the case of non-circular pipes, the line rise is printed above the span.

Col. 3 Total flow rate in the line.

Col. 4 The elevation of the downstream invert.

Col. 5 Elevation of the hydraulic grade line at the downstream end. This is computed as the upstream HGL + Minor loss of this line's downstream line.

Col. 6 The downstream depth of flow inside the pipe (HGL - Invert elevation) but not greater than the line size.

Col. 7 Cross-sectional area of the flow at the downstream end.

Col. 8 The velocity of the flow at the downstream end, (Col. 3 / Col. 7).

Col. 9 Velocity head (Velocity squared / 2g).

Col. 10 The elevation of the energy grade line at the downstream end, HGL + Velocity head, (Col. 5 + Col. 9).

Col. 11 The friction slope at the downstream end (the S or Slope term in Manning's equation).

Col. 12 The line length.

Col. 13 The elevation of the upstream invert.

Col. 14 Elevation of the hydraulic grade line at the upstream end.

Col. 15 The upstream depth of flow inside the pipe (HGL - Invert elevation) but not greater than the line size.

Col. 16 Cross-sectional area of the flow at the upstream end.

Col. 17 The velocity of the flow at the upstream end, (Col. 3 / Col. 16).

Col. 18 Velocity head (Velocity squared / 2g).

Col. 19 The elevation of the energy grade line at the upstream end, HGL + Velocity head, (Col. 14 + Col. 18) .

Col. 20 The friction slope at the upstream end (the S or Slope term in Manning's equation).

Col. 21 The average of the downstream and upstream friction slopes.

Col. 22 Energy loss. Average Sf/100 x Line Length (Col. 21/100 x Col. 12). Equals (EGL upstream - EGL downstream) +/- tolerance.

Col. 23 The junction loss coefficient (K).

Col. 24 Minor loss. (Col. 23 x Col. 18). Is added to upstream HGL and used as the starting HGL for the next upstream line(s).