FINAL SEWER BASIS OF DESIGN REPORT FOR NATURAL RESULTS SURGERY CENTER & MEDICAL SPA

FINAL Basis of Design Report

□ APPROVED

☑ APPROVED AS NOTED

☐ REVISE AND RESUBMIT



Disclaimer: If approved; the approval is granted under the condition that the final construction documents submitted for city review will match the information herein. Any subsequent changes in the water or sewer design that materially impact design criteria or standards will require re-analysis, re-submittal, and approval of a revised basis of design report prior to the plan review submission.; this approval is not a guarantee of construction document acceptance. For questions or clarifications contact the Water Resources Planning and Engineering Department at 480-312-5685.

BY apritchard

DATE 2/8/2023

Scottsdale, Arizona

January 12th, 2023

PREPARED FOR

Kohan Architecture 490 Post Street, #1135 San Francisco, California 94102

DEVELOPER

Natural Results Plastic Surgery 7943 East Thompson Peak Parkway, #101 Scottsdale, Arizona 85255

SITE ADDRESS

20001 North Scottsdale Road Scottsdale, Arizona 85255

61-DR-2014 #4

CYPRESS CYPRESS

4450 north 12th street, #228 phoenix, arizona 85014 CYPRESS # 21.143



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INTRODUCTION: PROJECT DESCRIPTION AND LOCATION

The Project is known as 'Natural Results Surgery Center & Medical Spa' and is located at 20001 North Scottsdale Road in Scottsdale, Arizona. The project is also known as Lot 4, or Pad B, of One Scottsdale (herein referred to as the "Master Development"). A Master Water and Wastewater Study Basis of Design Report dated January 3rd, 2020, by Civil & Environmental Consultants, Inc. (herein referred to as the "Master Report"). Additionally, a Water Basis of Design Report for QuickTrip Store No. 0480 was prepared by Kimley Horn and Associates in May 2022, which references and expands on the Master Report. Refer to Appendix A for Water BOD Report for QuickTrip Store No. 0480, which also contains the Master Report.

The proposed project consists of the development of a two-story medical office building to with the required paved parking, utility and drainage improvements, and amenity landscaping.

The utility provider for sewer facilities is the City of Scottsdale.

EXISTING CONDITIONS

Per available utility maps and as-built records, existing sewer mains are located in Scottsdale Road to the west of the project, within the existing Tract B supporting residential development to the east of the Master Development, and another main through the center of the Master Development. A 15" VCP sewer main is tapped from a manhole within Scottsdale Road to the west of the project and supplies sewer service to existing residential development east of the Project via an existing 10" PVC sewer main. Refer to Appendix B for City of Scottsdale Sewer Quarter Section Map.

The Master Development will be constructing new public sewer infrastructure to support the Project via a new 6" sewer service defined as SL#11 within the Master Report. The service ultimately connects to the existing 10" PVC sewer main in the center of the Master Development, continuing out to the 15" VCP sewer main in Scottsdale Road.

PROPOSED CONDITIONS

There is a pad site built by the Master Development that will be developed with a new 2-story medical office and spa building. The building will have a footprint of 17,000 square feet. Conservatively, the total building use will be split evenly between office use and spa use. The new building will connect to the existing 6" service lateral installed by the Master Development with a minimum slope of 1.0%. This is anticipated to provide adequate sizing to supply building. Refer to Appendix C for Preliminary Utility Plan.

Calculations for estimated average day and peak demands for the development are as follows:

0.40 0.50	/2.000	216,000		(CFS)
0.50		210,000	150	0.33
0.50	4,500	13,500	9	0.02
0.50	4,800	14,400	10	0.02
S 380 PER ROOM	46,740	140,220	97	0.22
1.20	12,840	38,520	27	0.06
0.50	4,500	13,500	9	0.02
0.40	96,000	288,000	200	0.45
00	241,380	724,140	503	1.12
	0.50 0.40	0.50 4,500 0.40 96,000 241,380	0.50 4,500 13,500 0.40 96,000 288,000 00 241,380 724,140	0.50 4,500 13,500 9 0.40 96,000 288,000 200

REQUIRED COMPU

Only a portion of the public sewer line will be built with Phase 1 During the overall construction an

EXISTING SEWER DEMAND:

Average Day Demand (Commercial/Retail): 0.5 GPD/SF x 15,774 SF = 7,887 GPD

Peak Demand: 3 x 7,887 = 23,661 GPD

PROPOSED SEWER DEMAND:

Average Day Demand [PER CITY OF SCOTTSDALE DSPM 2018 - CH. 7]

(**Spa**): 0.8 GPD/SF x 10,560 SF = 8,448 GPD (**Office**): 0.4 GPD/SF x 6,440 SF = 2,576 GPD

Total: 11,024 GPD / 7.66 GPM

Peak Demand:

(Spa): 8,448 x 3.5 = 29,568 GPD (Office): 2,576 x 3.0 = 7,728 GPD Peak Total: 37,296 GPD / 25.9 GPM

The proposed demand is in line with the proposed demand within the Master Report. Thus, it is expected that the new sewer infrastructure will be sufficient to serve the new building, as it will size up the existing 4" service to a 6" service. Minimum design velocity is greater than 2.5 ft/s, refer to Sewer Flow Calculation in Appendix D.

CONCLUSION

CYPRESS respectfully submits this report as the Final Wastewater Design Report for the proposed Natural Results Surgery Center & Medical Spa Development. The proposed wastewater system shall be designed in accordance with ADEQ, International Building Code, and the City of Scottsdale standards.

Appendix A Wastewater Basis of Design Report for QuickTrip Store No. 0480



Wastewater Basis of Design Report

QuikTrip Store No. 0480

NEC of N. Scottsdale Road and E. Legacy Boulevard

Prepared for:

QuikTrip Corporation 4705 South 129th East Ave Tulsa, OK 74134



Prepared by:

Kimley-Horn & Associates, Inc. 7740 North 16th Street, Suite 300 Phoenix, AZ 85020





QuikTrip Store No. 0480

WASTEWATER BASIS OF DESIGN REPORT

MAY 2022

Prepared By:





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Appendix A – Vicinity Map

Appendix B - Preliminary Utility Plan

Appendix C – Water and Wastewater Study Basis of Design Report by CEC Consultants, Inc.



1.0 INTRODUCTION

Kimley-Horn and Associates, Inc. has prepared this wastewater basis of design report for the proposed QuikTrip Convenience Store (#0480) development at the northeast corner of Scottsdale Road and Legacy Boulevard in Scottsdale, Arizona. This report intends to demonstrate that the proposed project conforms to the City of Scottsdale design requirements.

The QuikTrip Development encompasses approximately 2.0 net acres and contains a 5,312 square feet of retail space. The project lies within a portion of the Northwest Quarter of Section 26, Township 4 North, Range 4 East of the Gila and Salt River Base and Meridian in Maricopa County, Arizona. The proposed development is a portion of the One Scottsdale Phase 1 Master Development (herein referred to as the "Master Development"). More specifically, the site is bounded on the west by Scottsdale Road, on the south by Legacy Boulevard, and on the north and east by the remaining portion of the Master Development. See **Appendix A** for the Vicinity Map.



2.0 WASTEWATER COLLECTION SYSTEM

2.1 INTENT AND SCOPE

The intent of this section is to evaluate the proposed sewer infrastructure and wastewater design flows for the development. As a result of this analysis, it will be determined if the sewer infrastructure is capable supporting the proposed development in accordance with the City of Scottsdale Design Standards & Policies Manual (**Reference 1**) and the One Scottsdale Phase 1 Master Plan (**Reference 2**).

2.2 SEWER INFRASTRUCTURE

There is an existing 15-inch VCP Sewer main located in Scottsdale Road along the west side of the project and an existing 12-inch PVC sewer main located in Legacy Road along the south side of the project. Additionally, there is an existing 10-inch PVC sewer main in 73rd Street along the east side of the Master Development. Lastly, there is an existing 10-inch sewer main that runs east-west and bisects the Master Development and discharges to the existing sewer main in Scottsdale Road.

The Master Development will be constructing a new 8-inch PVC private sewer main that connects to the existing 10-inch public sewer main that runs east-west through the master development. The proposed 8-inch sewer main will be stubbed at the north property line of the proposed QuikTrip parcel.

The QuikTrip development will connect to the 8-inch PVC sewer main stub and extend the sewer main through the site. The 8-inch sewer main extension will be stubbed at the east property line to serve future developments. A new 6-inch PVC sewer service will connect to the 8-inch PVC sewer main to serve the QuikTrip building.

Refer to **Appendix B** for the Preliminary Utility Plan.



2.3 WASTEWATER DESIGN FLOWS

Note: The Quik Trip only occupies about half of the "Future Office B" parcel, reserving the remaining 280,032 gpd for the remainder of the parcel.

The following calculations are based on information provided in Section 7-1.403 of R

Table 1: Wastewater Design Flow

Commercial

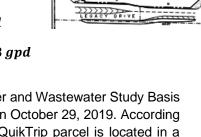
Building	Building Area (SF)	Demand ² (GPD/SF)	Average Daily Flow ¹ (GPD)	Peak Flow ² (GPD)	Peak Flow ² (GPM)
QuikTrip Store #0480	5.312	0.5	2,656	7.968	5.53

Notes:

- 1. For commercial/retail land use, average day sewer demand is 0.5 gpd/sf. (Fig 7-1.2 of Reference 1)
- 2. Peaking factor for commercial/retail is 3.

Wastewater calculations can be found below. Using the City of Scottsdale factors:

- Average Daily Demand $(ADD) = 0.5 \left(\frac{gpd}{sf}\right) * 5.312 sf = 2.656 gpd$
- Max Day Demand = ADD * Peak Factor = 2,656 gpd * 3.0 = 7,968 gpd



A wastewater model of the Master Development was performed in the Water and Wastewater Study Basis of Design Report for One Scottsdale Performed by CEC Consultants, Inc. on October 29, 2019. According to the Site Plan located in Appendix A of the One Scottsdale report, the QuikTrip parcel is located in a portion of a future pad anticipated to be a 240,000 sf type V-B office building. According to the Wastewater Demand table on page nine (9) of the One Scottsdale BOD report, the peak day demand for Future Office B is 288,000 gpd. This previously anticipated flow is greater than the proposed anticipated flow of 7,968 gpd for the QuikTrip site. See **Appendix C** for the Master Development Wastewater BOD Report.

3.0 CONCLUSION

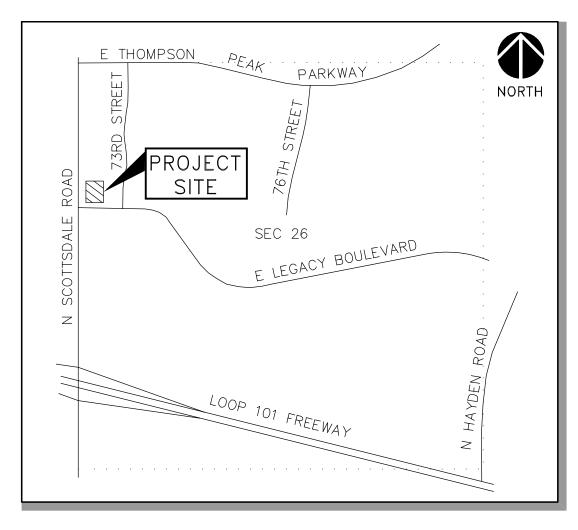
This development proposes to connect to the One Scottsdale Master Development sewer main. The projected flow according to the One Scottsdale Master Development report is greater than the proposed flow for the QuikTrip parcel. Therefore, the master development wastewater infrastructure is sufficient to service the site.

4.0 REFERENCES

- 1. City of Scottsdale, Design Standards and Policies Manual. 2018.
- 2. One Scottsdale Phase 1 Master Plan, March 2021.

Kimley»Horn

Appendix A – Vicinity Map

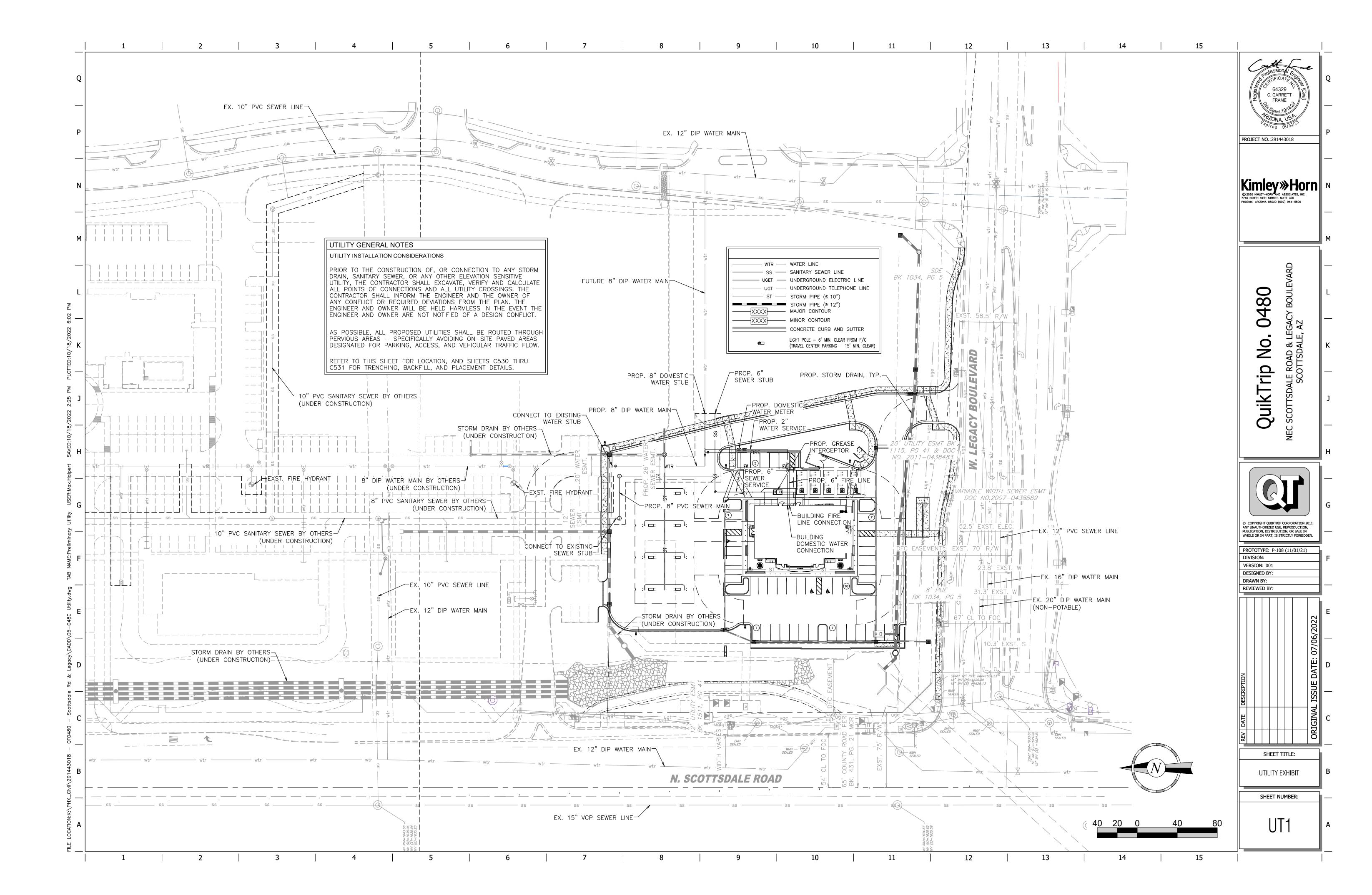


SITE LOCATION MAP SCOTTSDALE, AZ N.T.S.



Kimley»Horn

Appendix B – Preliminary Utility Plan



Kimley»Horn

Appendix C – Master Development Water and Wastewater Study Basis of Design Report

Water and Wastewater Study Basis of Design Report

For

One Scottsdale - Phase 1

Southeast Corner of Scottsdale Road and Thompson Peak Parkway Scottsdale, Arizona

Prepared for

DMB

7600 E Doubletree Ranch Road, Suite 300 Scottsdale, AZ 85258



January 3, 2020 CEC PN # 180-168 61-DR-2015#2



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5/10/2022

1.0 INTRODUCTION

The One Scottsdale project is a proposed 21.73 net acre commercial/retail project located southeast of

the intersection of the Scottsdale Road and Thompson Peak Parkway in Scottsdale, Arizona. The site

is further described as a portion of the NW1/4 of Section 26, Township 4 North, Range 4 East of the

Gila and Salt River Base and Meridian, Maricopa County, Arizona. Refer to the Vicinity Map on the

following page.

The site will consist of a hotel, Class A office buildings, underground and above ground parking

structures, and retail pads. Phase 1 of the project will incorporate the hotel site with parking lot and

drives. Refer to the Site Plan located in Appendix A at the back of this report. This report specifically

addresses Phase 1 of the project. Future phases will have their own water and wastewater basis of

design reports as the DR submittals come in for those particular phases.

This Basis of Design report will document existing and proposed water and wastewater utility

conditions for the 21.73-acre commercial parcel for the overall build-out condition and also the Phase

1 condition. Refer to the Concept Utility Plans located in Appendix B & C at the back of this report.

This site is located east of Scottsdale Road south of Thompson Peak Parkway, west of 73rd Drive, and

north of Legacy Boulevard. Water and sewer utilities exist within the adjacent street and parcels, and

are further described in the following section.

The purpose of this report is to satisfy the City of Scottsdale Design Review Board requirement

regarding the basis of water and wastewater design for the proposed One Scottsdale project and to

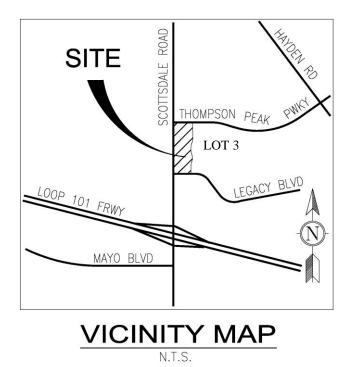
document water and sewer calculations for review and approval by the Maricopa County

Environmental Services Department.

-1-

One Scottsdale Water and Sewer BOD Report January 2020

Civil & Environmental Consultants, Inc.



-2-

development, and a 12" PVC sewer line in Legacy Boulevard just south of the development. Also, there is an existing 10" PVC sewer line that through the proposed development between Scottsdale Road and 73rd Drive.

Proposed Improvements and Demands – Overall (21.73 acres)

The sewer needs for the One Scottsdale project will be served by extending a 15-inch and a 12-inch PVC public sewer line through the site from Scottsdale Road, connecting to the existing 10-inch PVC sewer line in 73rd Drive. The proposed 15-inch PVC public sewer line will connect to an existing manhole in Scottsdale Road. Once fully constructed, the existing 10-inch PVC sewer line will be removed. The proposed 15-inch and 12-inch PVC sewer lines will run at a slope of 0.0024 ft/ft due to site constraints at the connection points. Onsite 8-inch and 6-inch PVC private plumbing lines will then service each of the proposed buildings as they are developed and per Uniform Plumbing Code slopes. Private plumbing slopes onsite are run at approximately between 1.0% and 5.0% slope to service the most remote buildings on the site. Refer to the Concept Utility Plan for proposed sewer line locations, stub location, slopes and inverts. Refer to the Sewer Exhibit located in the Appendix D at the back of this report for proposed sewer line designations in reference to calculations. Hydraulic calculations for each sewer line segment has been provided in Appendix D at the back of this report.

Calculations for estimated average day and peak demands for the development are as follows:

PROPOSED WASTEWATER DEMAND CA	LCULATIONS									
BUILDING/ PARCEL	SQUARE FOOTAGE	UNIT DEMAND (PER SQUARE FOOT)	TOTAL AVG. DAY DEMAND (GPD)	TOTAL PEAK DAY DEMAND (GPD)	TOTAL PEAK DAY DEMAND (GPM)	TOTAL PEAK DAY DEMAND (CFS)				
FUTURE OFFICE A	180,000	0.40	72,000	216,000	150	0.33				
FUTURE PAD A	9,000	0.50	4,500	13,500	9	0.02				
SHOPS A	9,600	0.50	4,800	14,400	10	0.02				
HOTEL	123 ROOMS	380 PER ROOM	46,740	140,220	97	0.22				
SHOPS B	10,700	1.20	12,840	38,520	27	0.06				
FUTURE PAD B	9,000	0.50	4,500	13,500	9	0.02				
FUTURE OFFICE B	240,000	0.40	96,000	288,000	200	0.45				
TOTAL	458,300.00		241,380	724,140	503	1.12				
DEMANDS ARE BASED ON PROPOSED BUILDING SQUARE FOOTAGE, USE, AND CITY OF SCOTTSDALE DESIGN STANDARDS & POLICIES MANUAL FIGURE 7.1-2										

Only a portion of the public sewer line will be built with Phase 1. During the overall construction an additional connection to 73rd Avenue will be constructed and the existing 10-inch PVC sewer will be abandoned. During the future phased construction, specifically Pad D, an additional private sewer

building connection will be constructed from the existing sewer in Legacy Boulevard. These two connections will be single building feeds and will be 6-inch PVC lines with a slope of 0.01 ft/ft.

Based on the One Scottsdale (Stacked 40s) Master On-Site Wastewater Plan, dated February 10, 2012, by Wood, Patel & Associates an upstream sewer flow of 0.69 cfs enters the proposed system at the connection point in 73rd Avenue. A copy of the excerpts from the Stacked 40s Master On-Site Wastewater Plan has been provided in Appendix D at the back of this report. This flow was incorporated into this design. The existing One Scottsdale master plan outfall defined as "F1" had an estimated offsite wastewater flow of 207,479 gallons per day. The TDI development increased the estimated offsite wastewater flow to 446,145 gallons per day. The TDI development accompanied by this project estimates a total wastewater flow of 1.81 cfs at the connection to the existing 15-inch PVC sewer in Scottsdale Road (this project contributing flow of 1.12 cfs + TDI offsite flow of 0.69 cfs).

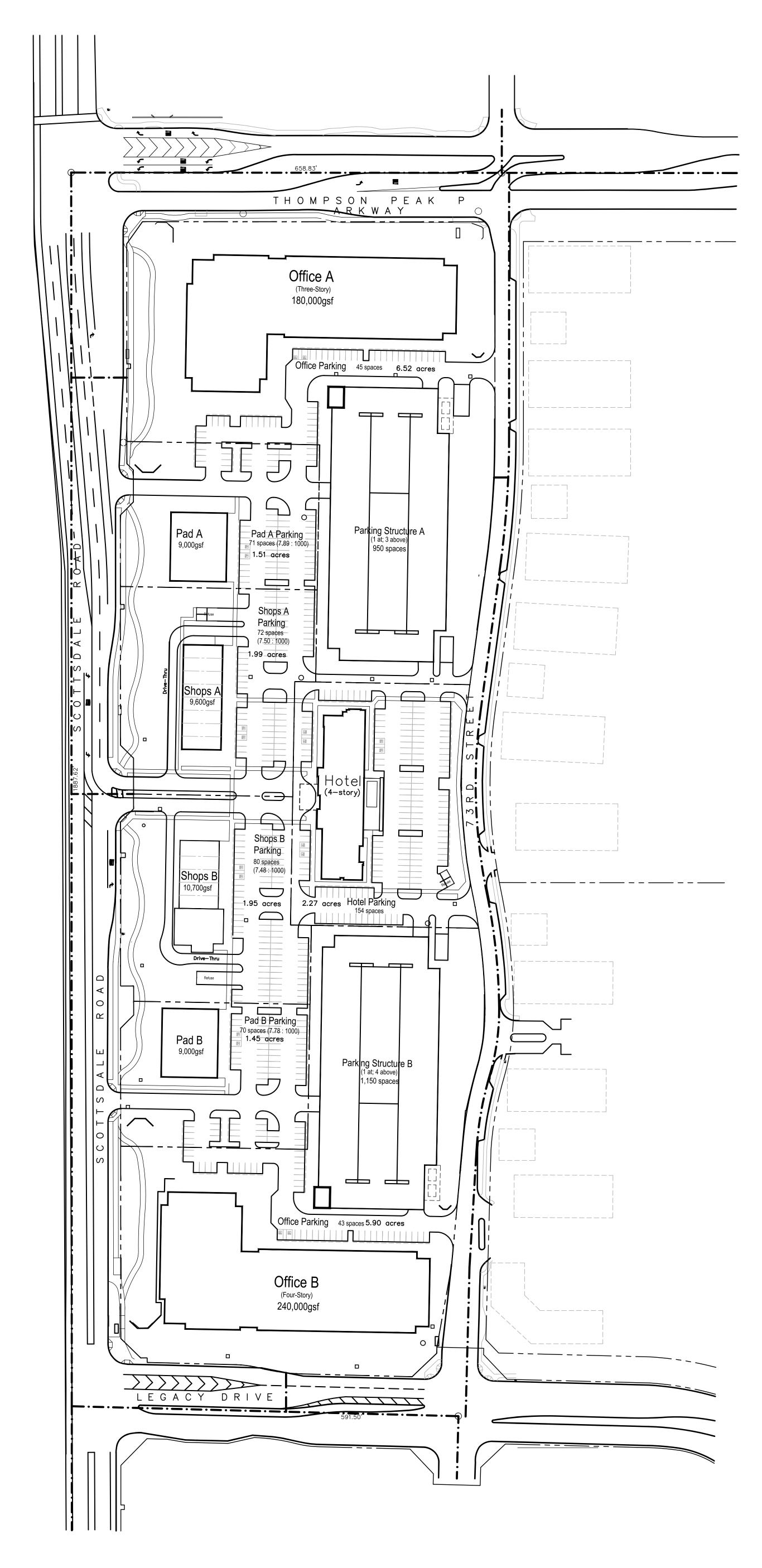
5.0 CONCLUSIONS

The One Scottsdale project will provide service to the approximate 21.73-arce development (7 total buildings) by constructing a public looped 8" waterline system and one public 12" sewer line extended into the site to service proposed and future buildings. The water line will provide adequate protection for domestic, landscape and fire flow requirements.

The 15-inch and 12-inch PVC public sewer line slopes are proposed at 0.24% from the existing 15-inch downstream sewer and the existing 10-inch upstream sewer. A 15-inch PVC public sewer downstream connection will be made to an existing manhole located in Scottsdale Road.

APPENDIX A

Site Plan



Master Site Plan

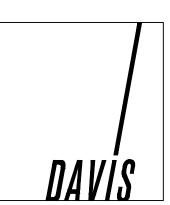
Total Office Area: 420,000gsf (401,000rsf)

Total Retail Area: 38,300gsf
Total Area: 538,300gsf

Hotel: 66,000gsf (123 keys)

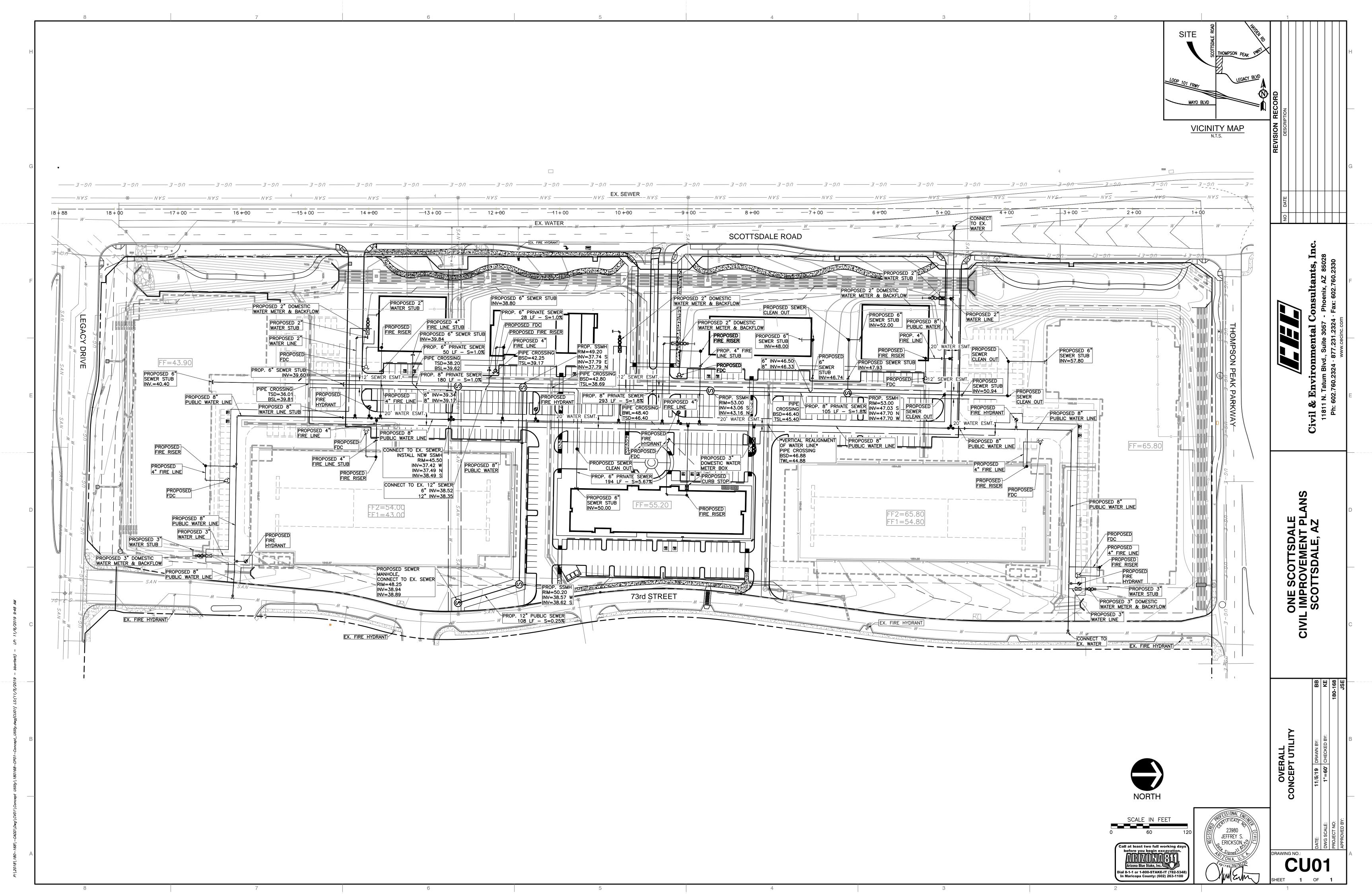
0' 80' 160' 240' 320' NORTH 14164 5-30-19





APPENDIX B

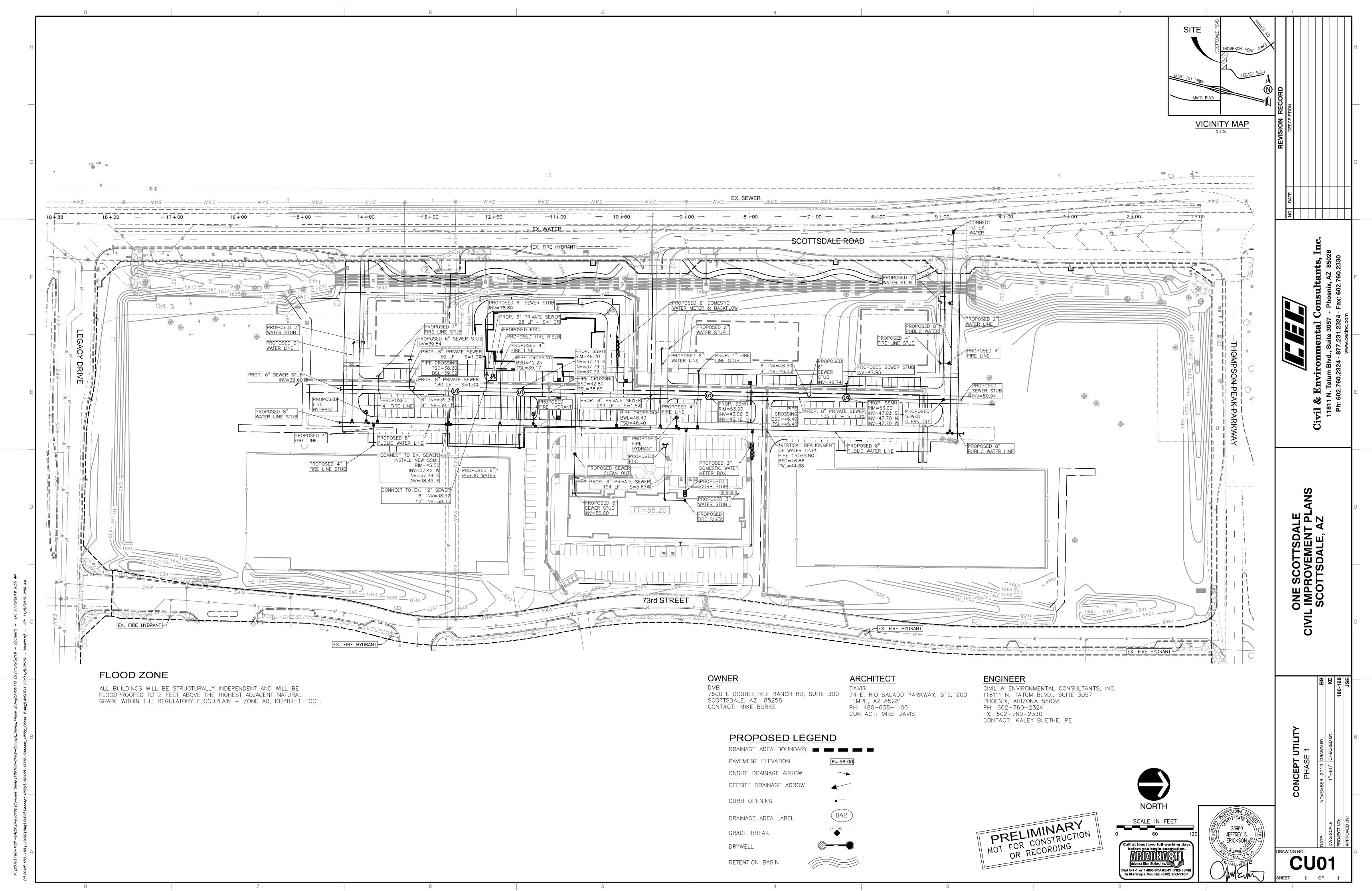
Overall Concept Utility Plan



1-UP-2022 5/10/2022

APPENDIX C

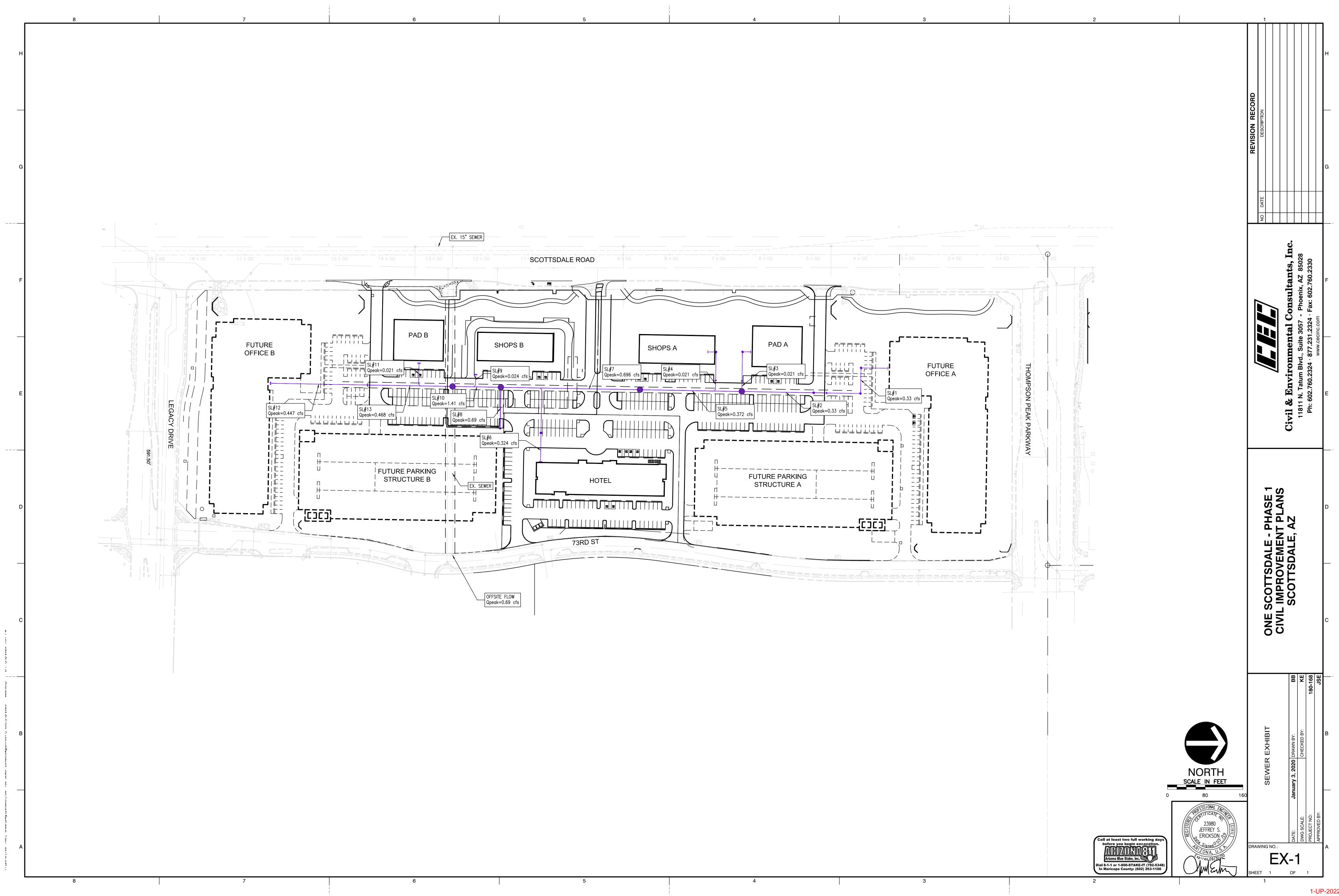
Phase 1 Concept Utility Plan



1-UP-2022 5/10/2022

APPENDIX D

Sewer Exhibit & Calculations



1-UP-2022 5/10/2022

Pipe ID	Q (cfs)	Peaking Factor	Q Peak (cfs)	Dia. (in)	Slope (ft/ft)	n	Depth (ft)	d/D	Vel. (fps)	Fr. #	Dcrit(ft)	Capacity	Max Q
SL#1	0.11	3	0.33	6	0.038	0.013	0.107	0.214	3.568	2.294	0.164	1.17	0.33
SL#2	0.11		0.33	8	0.018	0.013	0.117	0.1755	2.648	1.635	0.151	1.76	0.33
SL#3	0.007	3	0.021	6	0.01	0.013	0.039	0.078	0.981	1.061	0.04	0.6	0.021
SL#4	0.007	3	0.021	6	0.01	0.013	0.039	0.078	0.981	1.061	0.04	0.6	0.021
SL#5	0.124		0.372	8	0.018	0.013	0.125	0.1875	2.743	1.642	0.16	1.76	0.372
SL#6	0.072	4.5	0.324	6	0.0567	0.013	0.079	0.158	3.626	2.737	0.132	1.43	0.324
SL#7	0.196		0.696	8	0.018	0.013	0.156	0.234	3.137	1.665	0.203	1.76	0.696
SL#8	0.23	3	0.69	12	0.0025	0.013	0.243	0.243	1.562	0.664	0.197	1.91	0.69
SL#9	0.008	3	0.024	6	0.01	0.013	0.042	0.084	1.022	1.07	0.043	0.6	0.024
SL#10	0.434		1.41	12	0.0074	0.013	0.254	0.254	2.76	1.145	0.272	3.29	1.41
SL#11	0.007	3	0.021	6	0.01	0.013	0.039	0.078	0.981	1.061	0.04	0.6	0.021
SL#12	0.149	3	0.447	8	0.0038	0.013	0.202	0.303	1.665	0.769	0.176	0.81	0.447
SL#13	0.156	3	0.468	8	0.0038	0.013	0.207	0.3105	1.687	0.769	0.181	0.81	0.468

Hydraulic Analysis Report

Project Data

Project Title:

Designer:

Project Date: Monday, October 14, 2019

Project Units: U.S. Customary Units

Notes:

Channel Analysis: SL#1

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.5000 ft

Longitudinal Slope: 0.0380 ft/ft

Manning's n: 0.0130

Flow: 0.1100 cfs

Result Parameters

Depth: 0.1071 ft

Area of Flow: 0.0308 ft^2 Wetted Perimeter: 0.4811 ft Hydraulic Radius: 0.0641 ft Average Velocity: 3.5683 ft/s

Top Width: 0.4102 ft

Froude Number: 2.2939
Critical Depth: 0.1641 ft
Critical Velocity: 1.9619 ft/s
Critical Slope: 0.0071 ft/ft
Critical Top Width: 0.47 ft

Calculated Max Shear Stress: 0.2539 lb/ft^2 Calculated Avg Shear Stress: 0.1519 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.6700 ft

Longitudinal Slope: 0.0180 ft/ft

Manning's n: 0.0130

Flow: 0.1100 cfs

Result Parameters

Depth: 0.1174 ft

Area of Flow: 0.0415 ft^2 Wetted Perimeter: 0.5788 ft Hydraulic Radius: 0.0718 ft Average Velocity: 2.6484 ft/s

Top Width: 0.5095 ft

Froude Number: 1.6346
Critical Depth: 0.1510 ft
Critical Velocity: 1.8486 ft/s
Critical Slope: 0.0065 ft/ft
Critical Top Width: 0.56 ft

Calculated Max Shear Stress: 0.1319 lb/ft^2 Calculated Avg Shear Stress: 0.0806 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.5000 ft

Longitudinal Slope: 0.0100 ft/ft

Manning's n: 0.0130

Flow: 0.0070 cfs

Result Parameters

Depth: 0.0392 ft

Area of Flow: 0.0071 ft^2 Wetted Perimeter: 0.2837 ft Hydraulic Radius: 0.0251 ft Average Velocity: 0.9813 ft/s

Top Width: 0.2687 ft

Froude Number: 1.0613
Critical Depth: 0.0403 ft
Critical Velocity: 0.9393 ft/s
Critical Slope: 0.0088 ft/ft
Critical Top Width: 0.27 ft

Calculated Max Shear Stress: 0.0244 lb/ft^2 Calculated Avg Shear Stress: 0.0157 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.5000 ft

Longitudinal Slope: 0.0100 ft/ft

Manning's n: 0.0130

Flow: 0.0074 cfs

Result Parameters

Depth: 0.0403 ft

Area of Flow: 0.0074 ft^2 Wetted Perimeter: 0.2877 ft Hydraulic Radius: 0.0258 ft Average Velocity: 0.9988 ft/s

Top Width: 0.2721 ft

Froude Number: 1.0652 Critical Depth: 0.0416 ft Critical Velocity: 0.9529 ft/s Critical Slope: 0.0087 ft/ft Critical Top Width: 0.28 ft

Calculated Max Shear Stress: 0.0251 lb/ft^2 Calculated Avg Shear Stress: 0.0161 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.6700 ft

Longitudinal Slope: 0.0180 ft/ft

Manning's n: 0.0130

Flow: 0.1240 cfs

Result Parameters

Depth: 0.1245 ft

Area of Flow: 0.0452 ft^2 Wetted Perimeter: 0.5973 ft Hydraulic Radius: 0.0757 ft Average Velocity: 2.7434 ft/s

Top Width: 0.5213 ft

Froude Number: 1.6419
Critical Depth: 0.1605 ft
Critical Velocity: 1.9114 ft/s
Critical Slope: 0.0065 ft/ft
Critical Top Width: 0.57 ft

Calculated Max Shear Stress: 0.1399 lb/ft^2 Calculated Avg Shear Stress: 0.0850 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.5000 ft

Longitudinal Slope: 0.0567 ft/ft

Manning's n: 0.0130

Flow: 0.0720 cfs

Result Parameters

Depth: 0.0788 ft

Area of Flow: 0.0199 ft^2 Wetted Perimeter: 0.4084 ft Hydraulic Radius: 0.0486 ft Average Velocity: 3.6258 ft/s

Top Width: 0.3645 ft

Froude Number: 2.7374
Critical Depth: 0.1318 ft
Critical Velocity: 1.7403 ft/s
Critical Slope: 0.0071 ft/ft
Critical Top Width: 0.44 ft

Calculated Max Shear Stress: 0.2790 lb/ft^2 Calculated Avg Shear Stress: 0.1720 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.6700 ft

Longitudinal Slope: 0.0180 ft/ft

Manning's n: 0.0130

Flow: 0.1960 cfs

Result Parameters

Depth: 0.1563 ft

Area of Flow: 0.0625 ft^2 Wetted Perimeter: 0.6754 ft Hydraulic Radius: 0.0925 ft Average Velocity: 3.1371 ft/s

Top Width: 0.5667 ft

Froude Number: 1.6650 Critical Depth: 0.2032 ft Critical Velocity: 2.1709 ft/s Critical Slope: 0.0064 ft/ft Critical Top Width: 0.62 ft

Calculated Max Shear Stress: 0.1755 lb/ft^2 Calculated Avg Shear Stress: 0.1039 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 1.0000 ft

Longitudinal Slope: 0.0025 ft/ft

Manning's n: 0.0130

Flow: 0.2300 cfs

Result Parameters

Depth: 0.2427 ft

Area of Flow: 0.1472 ft^2 Wetted Perimeter: 1.0302 ft Hydraulic Radius: 0.1429 ft Average Velocity: 1.5622 ft/s

Top Width: 0.8574 ft

Froude Number: 0.6644
Critical Depth: 0.1968 ft
Critical Velocity: 2.1052 ft/s
Critical Slope: 0.0058 ft/ft
Critical Top Width: 0.80 ft

Calculated Max Shear Stress: 0.0379 lb/ft^2 Calculated Avg Shear Stress: 0.0223 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.5000 ft

Longitudinal Slope: 0.0100 ft/ft

Manning's n: 0.0130

Flow: 0.0080 cfs

Result Parameters

Depth: 0.0417 ft

Area of Flow: 0.0078 ft^2 Wetted Perimeter: 0.2930 ft Hydraulic Radius: 0.0267 ft Average Velocity: 1.0218 ft/s

Top Width: 0.2765 ft

Froude Number: 1.0702 Critical Depth: 0.0432 ft Critical Velocity: 0.9722 ft/s Critical Slope: 0.0087 ft/ft Critical Top Width: 0.28 ft

Calculated Max Shear Stress: 0.0260 lb/ft^2 Calculated Avg Shear Stress: 0.0167 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 1.0000 ft

Longitudinal Slope: 0.0074 ft/ft

Manning's n: 0.0130

Flow: 0.4340 cfs

Result Parameters

Depth: 0.2542 ft

Area of Flow: 0.1572 ft^2 Wetted Perimeter: 1.0569 ft Hydraulic Radius: 0.1487 ft Average Velocity: 2.7605 ft/s

Top Width: 0.8709 ft

Froude Number: 1.1449
Critical Depth: 0.2725 ft
Critical Velocity: 2.5046 ft/s
Critical Slope: 0.0056 ft/ft
Critical Top Width: 0.89 ft

Calculated Max Shear Stress: 0.1174 lb/ft^2 Calculated Avg Shear Stress: 0.0687 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.5000 ft

Longitudinal Slope: 0.0100 ft/ft

Manning's n: 0.0130

Flow: 0.0070 cfs

Result Parameters

Depth: 0.0392 ft

Area of Flow: 0.0071 ft^2 Wetted Perimeter: 0.2837 ft Hydraulic Radius: 0.0251 ft Average Velocity: 0.9813 ft/s

Top Width: 0.2687 ft

Froude Number: 1.0613
Critical Depth: 0.0403 ft
Critical Velocity: 0.9393 ft/s
Critical Slope: 0.0088 ft/ft
Critical Top Width: 0.27 ft

Calculated Max Shear Stress: 0.0244 lb/ft^2 Calculated Avg Shear Stress: 0.0157 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.6700 ft

Longitudinal Slope: 0.0038 ft/ft

Manning's n: 0.0130

Flow: 0.1490 cfs

Result Parameters

Depth: 0.2018 ft

Area of Flow: 0.0895 ft^2 Wetted Perimeter: 0.7785 ft Hydraulic Radius: 0.1149 ft Average Velocity: 1.6654 ft/s

Top Width: 0.6148 ft

Froude Number: 0.7693 Critical Depth: 0.1763 ft Critical Velocity: 2.0109 ft/s Critical Slope: 0.0064 ft/ft Critical Top Width: 0.59 ft

Calculated Max Shear Stress: 0.0479 lb/ft^2 Calculated Avg Shear Stress: 0.0272 lb/ft^2

Notes:

Input Parameters

Channel Type: Circular Pipe Diameter: 0.6700 ft

Longitudinal Slope: 0.0038 ft/ft

Manning's n: 0.0130

Flow: 0.1560 cfs

Result Parameters

Depth: 0.2067 ft

Area of Flow: 0.0925 ft^2 Wetted Perimeter: 0.7891 ft Hydraulic Radius: 0.1172 ft Average Velocity: 1.6871 ft/s

Top Width: 0.6189 ft

Froude Number: 0.7692 Critical Depth: 0.1806 ft Critical Velocity: 2.0361 ft/s Critical Slope: 0.0064 ft/ft Critical Top Width: 0.59 ft

Calculated Max Shear Stress: 0.0490 lb/ft^2 Calculated Avg Shear Stress: 0.0278 lb/ft^2

WOOD/PATEL

PIPE CAPACITY CALCULATIONS

Project:

Master On-Site Wastewater Plan for One Scottsdale (Stacked 40s)

Proj. Number:

021584.04

Location:

Scottsdale, Arizona

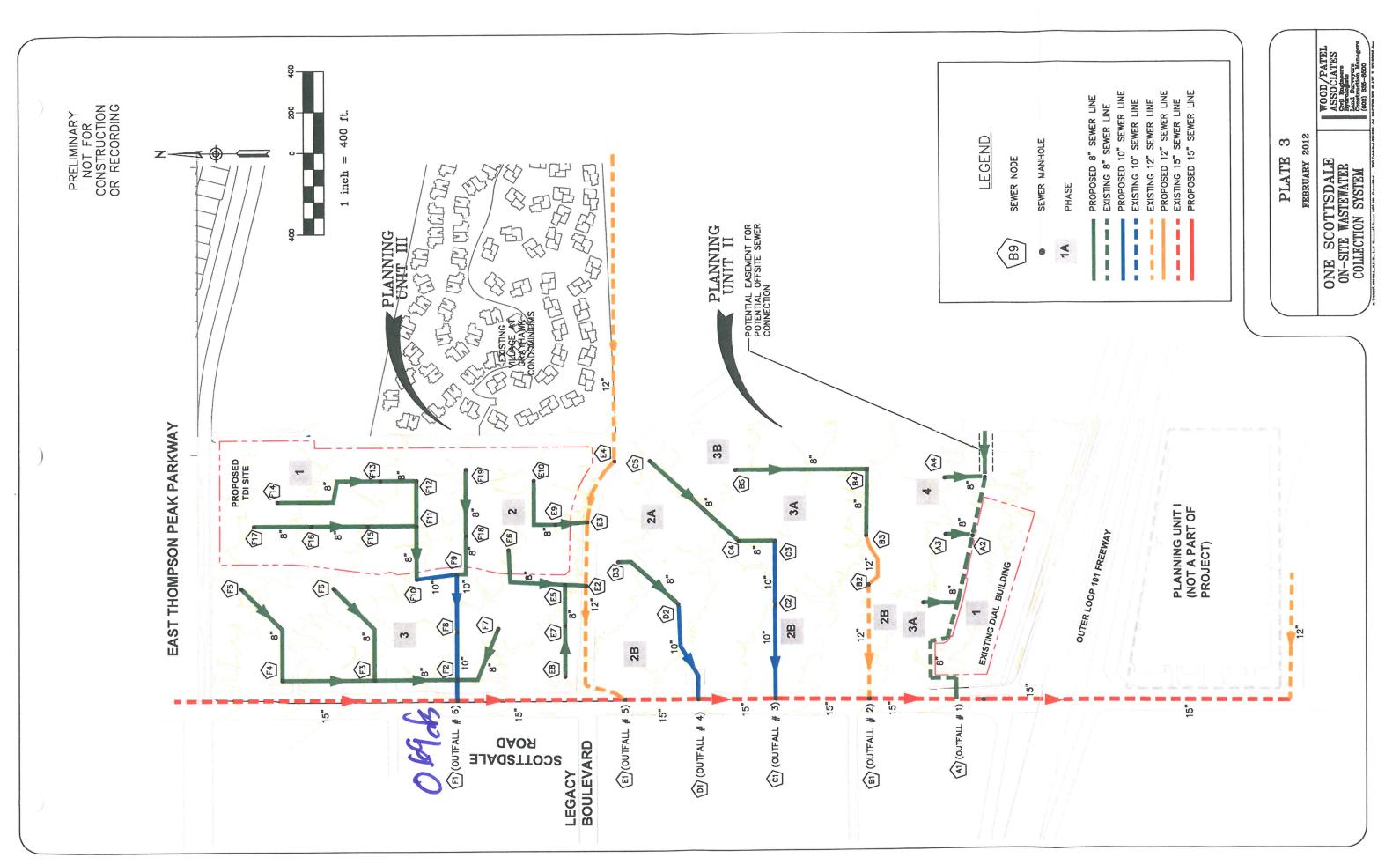
Proj. Engineer:

SAA

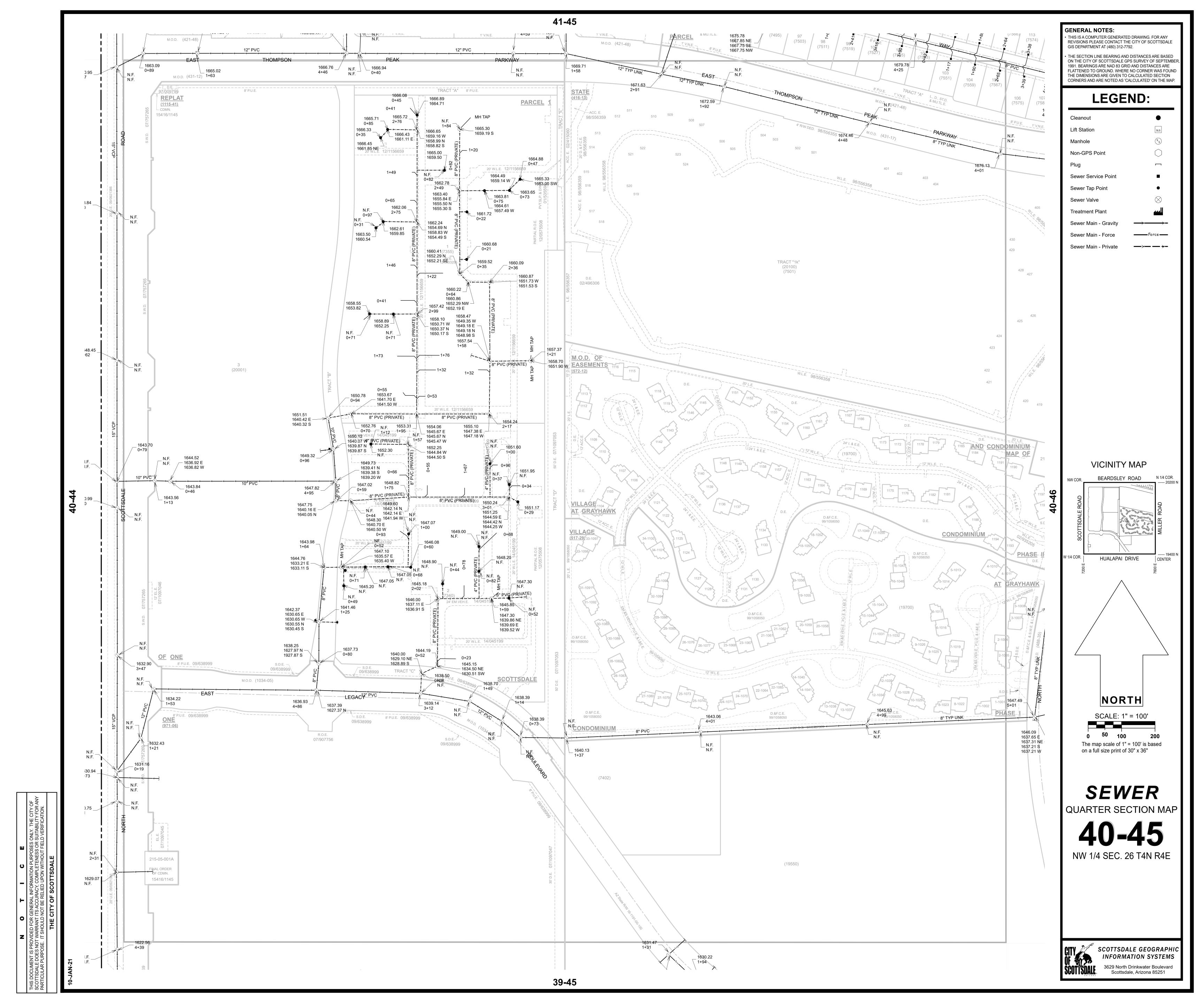
Date:	February 10, 20	12

UPSTREAM NODE	DOWNSTREAM NODE	MAX DAY FLOW (GPD)	PIPE DIA. (IN.)	PIPE SLOPE (FT / FT)	d/D RATIO	FULL FLOW VELOCITY (FPS)	PIPE CAPACITY	SURPLUS	PERCEN
A4	A2	59,239	8	0.00550	21.6%	2.6	(GPD)	(GPD)	CAPACIT
A3	A2	2,500	8	0.00550	4.8%		580,390	521,152	10.2%
A2	A1 (OUTFALL #1)	177.599	8	0.00550		2.6	580,390	577,890	0.4%
	MINOSTINEE	111,000		0.00330	38.0%	2.6	580,390	402,792	30.6%
B5	B4	70,447	8	0.00550	23.6%	2.6	580,390	509,943	12.1%
B4	B3	167,199	8	0.00550	36.8%	2.6	580,390	413,192	28.8%
B3	B2	696,129	12	0.00300	53.0%	2.5	1,263,964	567,835	55.1%
B2	B1 (OUTFALL #2)	846,895	12	0.00300	59.9%	2.5	1,263,964	417.069	67.0%
05									011078
C5	C4	98,037	8	0.00550	27.8%	2.6	580,390	482,353	16.9%
C4 C3	C3	124.451	8	0.00550	31.5%	26	580,390	455,939	21.4%
	C2	275,553	10	0.00430	37.3%	2.6	930,533	654,980	29.6%
C2	C1 (OUTFALL #3)	402,421	10	0.00430	46.0%	2,6	930,533	528,112	43.2%
D3	D2	135,780	8	0.00550	32.9%	2.6	con www		
D2	D1 (OUTFALL #4)	566,463	10	0.00430	56.4%	2.6	580,390	444.610	23.4%
				5.55465	30.478	2.6	930,533	364,071	60.9%
E4	E3	881,109	12	0.00310	60.8%	2.5	1,284,858	403,748	68.6%
E10	E9	50,109	8	0.00550	19.9%	2.6	580,390	530,281	8.6%
E9	E3	144,219	8	0.00550	34.0%	2.6	580,390	436,172	24.8%
E3	E2	1,025,837	12	0.00360	64.1%	2.7	1,384,603	358,766	74.1%
E6	C5	62,109	8	0.00550	22.1%	2.6	580,390	518,281	10.7%
E8	E7	60,509	8	0.00550	21.8%	2.6	580,390	519,881	10.4%
E7	E5	74,585	8	0.00550	24.2%	2.6	580,390	505,806	12.9%
E5	E2	138,659	8	0.00550	33.3%	2.6	580,390	441,731	23.9%
E2	E1 (OUTFALL #5)	1,165,005	12	0.00310	74.6%	2.5	1,284,858	119,852	90.7%
F14	F13	43,189		0.00550					
F13	F12	103,699	8	0.00550	18.5%	2.6	580,390	537,201	7.4%
F12	F11	120,208	8	0.00550	28.7%	2.6	580,390	476,692	17.9%
F17	F16	54.109	8	0.00550	30.9%	2.6	580,390	460,182	20.7%
F16	F15	108,219	8	0.00550	20.7%	2.6	580,390	526,281	9.3%
F15	F11	162,328	8	0.00550	29.3%	2.6	580,390	472,172	18.6%
F11	F10	299.045	8	0.00550	36.2%	2.6	580,390	418,062	28.0%
F10	F9	315,554	10	0.00550	50.9%	2.6	580,390	281,345	51,5%
F19	F18	43,709	10 B	0.00430	40.1%	2.6	930,533	614,979	33.9%
F18	F9	85,819	8	0.00550	18.6%	2.6	580,390	536,681	7.5%
F9	F8	401,882	10	0.00430	26.0% 46.0%	2.6	580,390	494,572	14.8%
F8	F2	409,174	10	0.00430	46.4%	2.6	930,533	528,652	43.2%
F5	F4	45,182	8	0.00550	18.9%	2.6	930,533 580,390	521,359	44.0%
F4	F3	45,691	8	0.00550	19.0%	2.6		535,208	7.8%
F6	F3	45,182	8	0.00550	18.9%	2.6	580,390 580,390	534,699	7.9%
F3	F2	91,383	8	0.00550	26.8%	2.6	580,390	535,208	7.8%
F7	F2	22,846	8	0.00550	13.6%	2.6	580,390	489,007 557,544	15.7%
F2	F1 (OUTFALL #6)	446,145	10	0.00430	48.8%	2.6	930,533	484,389	3.9% 47.9%





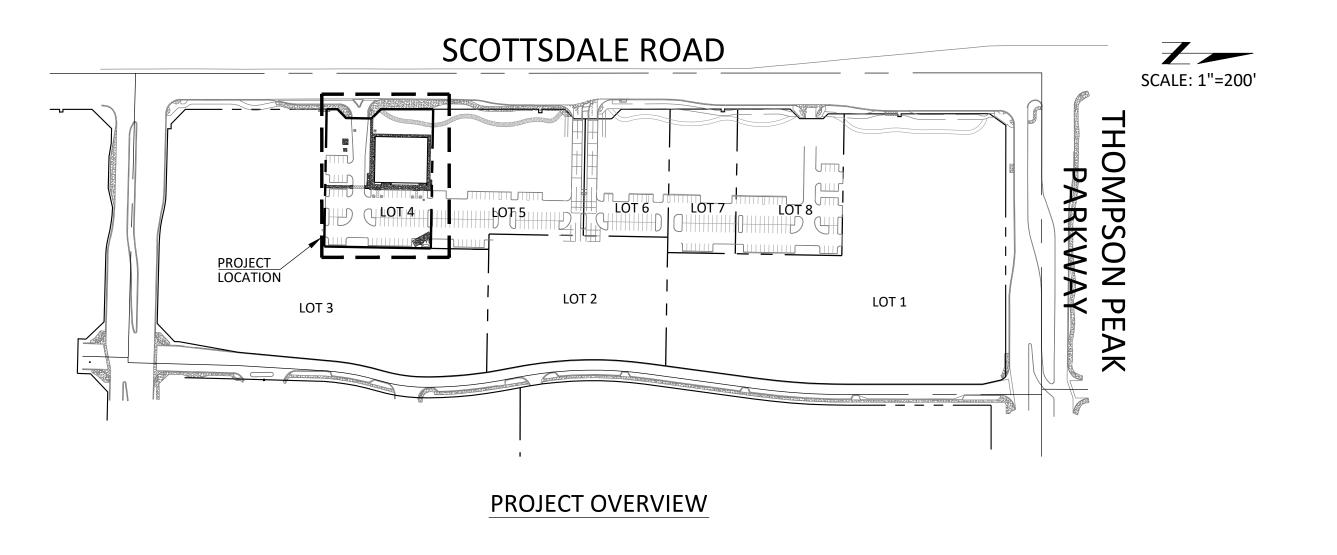
Appendix B City of Scottsdale Sewer Quarter Section Map

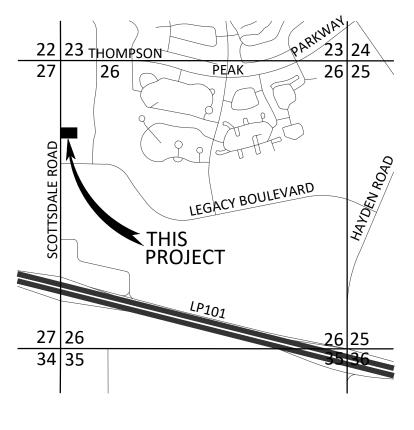


Appendix C Preliminary Utility Plan

PRELIMINARY IMPROVEMENT PLAN for NATURAL RESULT SURGERY CENTER & MEDICAL SPA

SEC THOMPSON PEAK PARKWAY AND SCOTTSDALE ROAD PHOENIX, ARIZONA A PORTION NORTHEAST QUARTER OF SECTION 19, TOWNSHIP 20 NORTH, RANGE 21 WEST OF THE GILA AND SALT RIVER MERIDIAN, MARICOPA COUNTY, ARIZONA

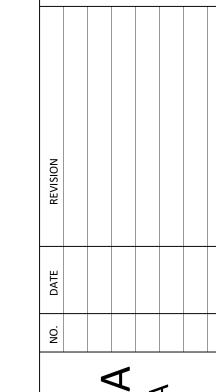




T. 4 N., R. 4 E., G.&S.R.M., CITY OF SCOTTSDALE, MARICOPA COUNTY, ARIZONA

3" = 1 MILE

LOCATION MAP



PROJECT INFORMATION

PROJECT DESCRIPTION: THE PROJECT CONSISTS OF THE CONSTRUCTION OF A TWO-STORY MEDICAL BUILDING ON AN EXISTING PAD SITE WITHIN THE ONE SCOTTSDALE MASTER DEVELOPMENT.

ADDRESS: 20001 NORTH SCOTTSDALE ROAD SCOTTSDALE, ARIZONA 85255

215-05-010 (OVERALL DEVELOPMENT PARCEL)

ZONING: PRC, PCD

PAD B AREA: 63,280 SF (1.45 AC) DISTURBED AREA: 20,550 SF (0.47 AC)

SURVEY NOTES

- L. THE SURVEY FOR THIS PROJECT WAS PERFORMED BY: SURVEY INNOVATION GROUP, INC. 7301 EAST EVANS ROAD SCOTTSDALE, ARIZONA 85260 PH: 623-869-0223 CONTACT: JASON SEGNERI, R.L.S.
- THE BASIS OF BEARINGS FOR THIS PROJECT IS THE NORTH LINE OF THE NORTHWEST QUARTER OF SEC 26 AS SHOWN IN BOOK 1115, PAGE 41 MARICOPA COUNTY RECORDS. SAID LINE BEARING S 89°57'38" E.
- . THE BASIS OF ELEVATION FOR THIS PROJECT IS THE MARICOPA COUNTY 4-INCH MC ENG DEPT BRASS CAP IN HAND HOLE AT THE INTERSECTION OF SCOTTSDALE ROAD AND THOMPSON PEAK PARKWAY, CITY OF SCOTTSDALE GPS POINT #8660, AN ELEVATION OF 1662.725' (NAVD 88).

BENCHMARK

THE BENCHMARK USED FOR THIS PLAN IS THE MARICOPA COUNTY 4-INCH MC ENG DEPT BRASS CAP IN HAND HOLE AT THE INTERSECTION OF SCOTTSDALE ROAD AND THOMPSON PEAK PARKWAY, CITY OF SCOTTSDALE GPS POINT #8660, AN ELEVATION OF 1662.725' (NAVD 88).

LEGAL DESCRIPTION

LOT 4 OF THE ONE SCOTTSDALE FINAL PLAT. BEING REPLAT OF LOT 3. OF THE REPLAT PARCEL 1 OF ONE SCOTTSDALE. ACCORDING TO THE PLAT OF RECORD IN THE OFFICE OF THE COUNTY RECORDER OF MARICOPA COUNTY, ARIZONA, RECORDED IN BOOK

DRAINAGE STATEMENT

-SITE IS IN A SPECIAL FLOOD HAZARD AREA - NO -OFFSITE FLOWS AFFECT THIS SITE - NO -PER THE MASTER DRAINAGE REPORT (FINAL DRAINAGE REPORT FOR ONE SCOTTSDALE, CEC, JANUARY 21, 2021), THE ONE SCOTTSDALE DEVELOPMENT IS ONLY REQUIRED TO PROVIDE FIRST FLUSH STORMWATER TREATMENT. THIS TREATMENT IS PROVIDED FOR THE DEVELOPMENT VIA VORTEX STORMWATER QUALITY STRUCTURES BY THE OVERALL ONE SCOTTSDALE DEVELOPMENT AND THIS PROJECT IS NOT REQUIRED TO PROVIDE ANY ADDITIONAL STORMWATER INFRASTRUCTURE. -EXTREME STORM OUTFALLS THE SITE AT THE SOUTH DRIVEWAY AT THE ELEVATION OF 1641.65.

FLOODPLAIN INFORMATION

ACCORDING TO THE FEDERAL EMERGENCY MANAGEMENT AGENCY FLOOD INSURANCE RATE MAP PANEL NUMBER 04013C1760, DATED NOVEMBER 04, 2015 THE PARCEL IS LOCATED IN THE ZONE X AREA, WHICH IS DEFINED AS AREAS OUTSIDE THE 0.2% ANNUAL CHANCE FLOOD.

		LEGEND	
	PROJECT RIGHT-OF-WAY	(C)	NEW SEWER CLEANOUT
	EXISTING RIGHT-OF-WAY	W	NEW WATER METER
	PROJECT PROPERTY LINE	BF	NEW BACKFLOW PREVENTER
	EXISTING PROPERTY LINE	cn3	NEW FIRE CONNECTION
	ROADWAY CENTERLINE		SURVEY MONUMENT AS NOTED
	EXISTING EASEMENT	• (99.99P)	SPOT ELEV. (EXIST. GRADE)
2321	EXISTING CONTOUR	• [99.99P] • 99.99P	SPOT ELEV. (PER SEPARATE PLAN) SPOT ELEV. (NEW GRADE)
2321	NEW CONTOUR	•	RIGHT-OF-WAY
4	EXISTING CONCRETE	B/C R	BACK OF CURB RADIUS
	NEW ASPHALT	P	PAVEMENT (ASPHALT)
4	NEW CONCRETE	С	CONCRETE
	NEW WALL	G TC	GUTTER TOP OF CURB
	EXISTING CURB	FG	FINISHED GRADE
	EXISTING PAINT STRIPE	GB FFE	GRADE BREAK FINISHED FLOOR ELEVATION
	NEW CURB	116	TINISHED FLOOR ELEVATION
	NEW COND		

NEW PAINT STRIPE

EXISTING SEWER LINE

EXISTING WATER LINE

EXISTING SEWER MANHOLE

EXISTING SEWER CLEANOUT

EXISTING SEWER STUB

EXISTING WATER STUB

EXISTING WATER VALVE

EXISTING FIRE HYDRANT

EXISTING AIR RELEASE VALVE

EXISTING STORM DRAIN PIPE

→ NEW RAILING

——s— NEW SEWER LINE

──₩── NEW WATER LINE

—— F — NEW FIRE SERVICE

OWNER/DEVELOPER

NATURAL RESULTS PLASTIC SURGERY 7930 EAST THOMPSON PEAK PARKWAY, #101 SCOTTSDALE, ARIZONA 85255 PH: 480-418-5300 ATTN: DR. CARLOS MATA

CIVIL ENGINEER

CYPRESS CIVIL DEVELOPMENT 4450 NORTH 12TH STREET, #228 PHOENIX, ARIZONA 85014 PH: 623-282-2498 ATTN: JEFF HUNT, PE

ARCHITECT

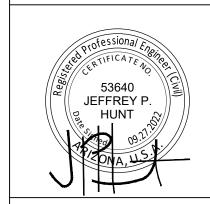
KOHAN ARCHITECTURE 490 POST STREET, #1135 SAN FRANCISCO, CALIFORNIA 94102 PH: 415-348-0008 ATTN: MARZI EMAMI GHOREISHI, AIA

UTILITIES

CITY OF SCOTTSDALE SEWER: CITY OF SCOTTSDALE **ELECTRIC:** APS GAS: **SOUTHWEST GAS** TELEPHONE: CENTURYLINK CABLE: COX COMMUNICATIONS

SHEET INDEX

 COVER SHEET 2. GRADING & DRAINAGE PLAN UTILITY PLAN



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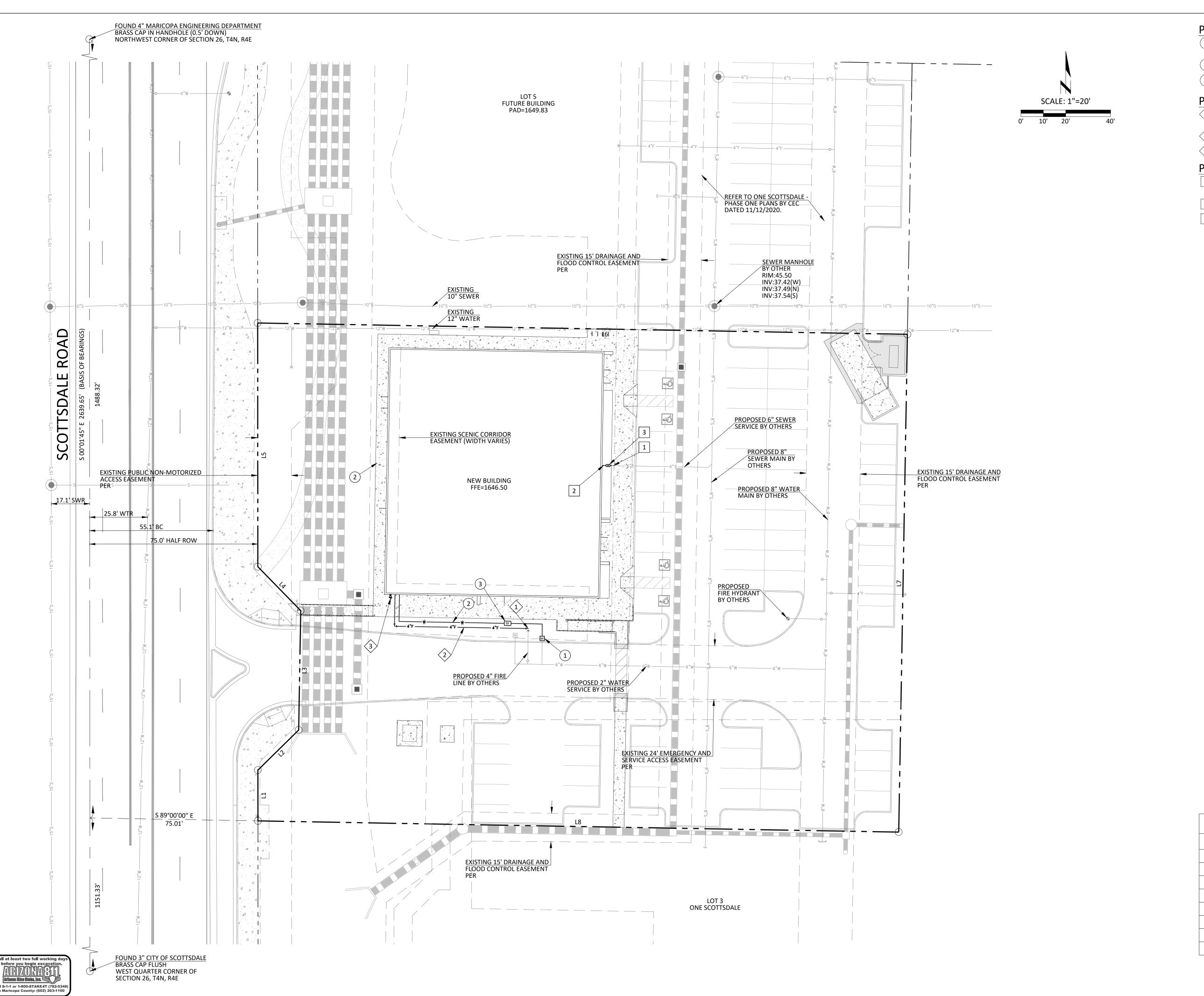
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DRAWN:		PT
DESIGNE	D:	DS
CHECKED	:	JF
DATE:	09	9-27-2022
JOB NO:		20.143
SHEET N	UMBER	

Call at least two full working da



PRIVATE WATER KEYNOTES

- 1 PROPOSED 2" SERVICE BY OTHERS. INSTALL NEW 2" METER FOR DOMESTIC WATER SERVICE.
- 2 NEW 2" DOMESTIC WATER LINE.
- 3 NEW 2" DOMESTIC BACKFLOW PREVENTER.

PRIVATE FIRE LINE KEYNOTES

- PROPOSED 4" FIRE STUB BY OTHERS. CONNECT TO FOR FIRE SERVICE.
- NEW 4" DIP FIRE LINE.
- 3 NEW BUILDING MOUNTED FDC.

PRIVATE SEWER KEYNOTES

1 PROPOSED 6" SEWER SERVICE STUB BY OTHERS. CONNECT TO FOR SEWER SERVICE.

LINE TABLE

LENGTH

22.65'

25.49'

53.21'

27.42'

108.68'

289.98'

222.00'

286.21'

BEARINGS

N 00°01'45" W

N 45°28'06" E

N 01°00'00" E

N 44°18'03" W

N 00°01'45" W

S 89°02'10" E

S 00°56'37" W

N 89°02'10" W

- 2 NEW 6" PVC SDR-35 SEWER LINE. 2% MINIMUM SLOPE.
- 3 NEW TWO-WAY SEWER CLEAN OUT.

JEFFREY P.

09-27-2022

3 of 3

20.143

DRAWN:

DESIGNED:

CHECKED:

DATE:

JOB NO:

SHEET NUMBER

Appendix D Sewer Flow Calculation



SITE CONSULTING SURVEY SITE ENGINEERING

Sewer Design Report Calculations

ONE SCOTTSDALE MEDICAL SPA

Sewage Flow Per Day (From Table 1, A.A.C. Title 18, Chapter 9)

Total Flow (GPD)	11,024
Upstream Population	
Dry Peaking Factor	
Wet Weather Peaking Factor	1
Dry Peak Flow (GPD)	37,296
Wet Peak Flow (GPD)	37,296

$Q = \frac{1}{n} A R^{2/3} S^{1/2}$	2 =	$\frac{1.49}{n}AR^{2/3}S^{1/2}$
-------------------------------------	-----	---------------------------------

Where:

Q = flow in cfs

n = Manning's Roughness Coefficient

A = Cross sectional area of flow

R = hydraulic radius

S = pipe slope

SYSTEM MINIMUM SLOPE

n	=	0.013	
Pipe diameter (in)	=	6	
Pipe Slope (ft/ft)	=	0.011	

Full Flow*

Depth of flow (in) = 4.50

$$\phi$$
 (radian) = 4.19
Area (in2) = 22.75
Wetted Perimeter (in) = 12.57
Hydraulic Radius (in) = 1.81
Velocity (ft/sec) = 3.41
 d/D ratio = 0.75
Pipe Capacity (GPD) 347,745

Design Flow

Depth of flow (in) =	1.27
φ (radian) =	1.91
Area (in2) =	4.35
Wetted Perimeter (in) =	5.73
Hydraulic Radius (in) =	0.76
Velocity (ft/sec) =	1.91
d/D ratio =	0.21
-	

^{*}Full Flow refers to d/D of 0.75 per AAC R18-9-E301.4.01.D.2.e