

# TRAFFIC STUDY, 3<sup>RD</sup> SUBMITTAL

North

August 7, 2023

Mr. Eric Sepanek Gold Standard Property, LLC 9943 East Bell Road Scottsdale, AZ 85260



RE: TRAFFIC STUDY  $-3^{RD}$  SUBMITTAL FOR PROPOSED DEVELOPMENT ON LOT 1B ON HAYDEN ROAD AT 84<sup>TH</sup> STREET - SCOTTSDALE (CASE #1-DR-2023/KEY CODE 6L395)

Dear Mr. Sepanek:

Thank you for retaining CivTech to provide this traffic study for Gold Standard Property's proposed project located on the east side of Hayden Road just north of 84<sup>th</sup> Street in Scottsdale, Arizona (the "Project"). The project vicinity is shown in **Figure 1**.

The proposed Project will develop a single Maricopa County Assessor parcel "Lot 1B" (APN 215-52-107) of approximately 45,712 square feet (sf) with a 12,867-sf building consisting of a cigar bar, a retail area, a gallery, a recording studio, and general office space. A site plan has been provided as



FIGURE 1 - VICINITY MAP

**CivTech Inc.** • 10605 North Hayden Road • Suite 140 • Scottsdale, AZ 85260 Phone: 480.659.4250 • Fax: 480.659.0566

**Attachment A**, which also shows the site layout and orientation of the building on the site. This statement will compare the trips expected from the previously-proposed mixed-use development, which was studied by CivTech in 2019, and the newly proposed development.

This version of the traffic study represents a 3<sup>rd</sup> submittal of an original version and a 2<sup>nd</sup> Submittal sealed by CivTech on December 22, 2022 and June 21, 2023, respectively. In the original version, CivTech provided a Traffic Signal Warrant Analysis for the intersection of 84<sup>th</sup> Street and Hayden Road; it remains herein as it was presented then. CivTech understood that this warrant analysis was stipulated for either or both of remaining two parcels for the Core Center, which include the subject development on Lot 1B and a separate proposed development located to the immediate southwest of the proposed project on Lot 1A on Maricopa County Assessor parcel APN 215-52-106. While the Lot 1A project is not a part of this project, it was a part of the previously-approved development at this location, and is included in the trip generation and comparison for consistency, albeit separately. The reader should be aware that CivTech prepared a traffic study for the Lot 1A project that was sealed for submission on February 3, 2023. CivTech now understands that the developer of Lot 1A has assumed responsibility for the traffic signal at the intersection of 84th Street and Hayden Road. Therefore, CivTech revised the original version of this study for the purpose of acknowledging that new revelation and has prepared this 3<sup>rd</sup> Submittal to respond to three additional City comments, two of which are related, in areas that will be identified in the text.

The requirements for Transportation Impact Studies are detailed in Section 5-1 of the City of Scottsdale's *Design Standards & Policies Manual, 2018.* Based on the level of trip generation expected from the proposed development, a Category 1 Transportation Impact and Mitigation Analysis (TIMA) is warranted.

### **EXISTING CONDITIONS**

#### LAND USE

The project site is currently vacant land previously used as an auto dealership that has since been razed. The site has one (1) right-in-right-out driveway just north of 84<sup>th</sup> Street that will be shared by the proposed development and another project being planned for the vacant site ("Lot 1A") to the (south)west.

#### ROADWAY NETWORK

**Hayden Road** is a north-south four-lane arterial street with two (2) lanes of travel in each direction and a raised median within the vicinity of the Project site. Hayden Road begins to the north at Frank Lloyd Wright Boulevard, becomes McClintock Drive at McKellips Road where it enters the City of Tempe to the south, and ends in the City of Chandler at State Route 202L (SR-202L), an Arizona Department of Transportation (ADOT) facility. There are curb, gutter, and sidewalk facilities on both sides of Hayden Road and a posted speed limit of 45 miles per hour (MPH).

**84<sup>th</sup> Street** is a north-south two-lane collector street with one (1) lane of travel in each direction. This short segment of 84<sup>th</sup> Street begins to the north, approximately ¼ mile north of Hayden Road



and terminates at Hayden Road. There are curb, gutter, and sidewalk facilities on both sides of 84<sup>th</sup> Street and no posted speed limit.

Please note that **Hayden Road** and **84<sup>th</sup> Street** are both roadways that are generally oriented north-south through the City of Scottsdale. In the vicinity of the project, Hayden Road is oriented northeast-southwest around Scottsdale Airpark, while 84<sup>th</sup> Street is oriented northwest-southeast across Hayden Road.

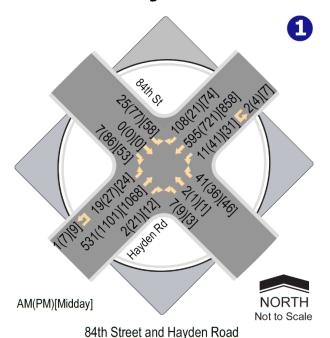
The intersection of **Hayden Road** and **84<sup>th</sup> Street/SW Access** is two-way stop controlled with stop control on the northwestbound (main Core Center driveway/SW Lot 1B Access) and southeastbound (84<sup>th</sup> Street) approaches. The southeast approach is the private main access into the existing Core Scottsdale apartments that will also serve Lots 1B and 1A under the revised, cohesive site plan shown in **Attachment A**. The northeastbound Hayden Road approach consists of one (1) exclusive left turn lane, two (2) through lanes, and one (1) right-turn lane. The southwestbound Hayden Road approach consists of one (1) exclusive left turn lane, one (1) through lane, and one (1) shared through/right-turn lane. The northwestbound SW Access approach consists of one (1) exclusive left turn lane and one (1) shared through/right-turn lane. The southeastbound 84<sup>th</sup> Street approach consists of one (1) shared left-turn/through/right-turn lane. There are curb, gutter, and sidewalk facilities on all corners and no crosswalks.

#### ADJACENT VOLUMES

CivTech engaged All Traffic Data Services to record 24-hour turning movement volumes at the intersection of Hayden Road and 84<sup>th</sup> Street; these counts were conducted on Tuesday, November 15, 2022. The existing 2022 volumes (as recorded) are shown in **Figure 2**. Traffic count data sheets are included as **Attachment B.** 

In addition, due to ongoing construction of a new roundabout at the intersection of Hayden Road and Raintree Drive affecting through traffic volumes on Hayden Road, the City directed CivTech to utilize historic traffic counts for the through volumes on Hayden Road, adjusted to 2022. CivTech previously engaged Field Data Services of Arizona, Inc. to record 24-hour turning movement volumes at intersection; these counts were conducted on Thursday, April 25, 2019.

FIGURE 2 - Existing Peak Hour Volumes



CivTech reviewed and compared the two sets of counts and discovered only minor differences between the through volumes on Hayden Road recorded in 2019 and those recorded recently. These are summarized in **Table 1**, which compares the AM and PM peak hour volumes during the current



TABLE 1 - COMPARISON OF PEAK HOUR THROUGH VOLUMES ON HAYDEN ROAD, 2019 AND 2022

Year	AM Peak Hour Starts		Westbound AM Volume			Westbound PM Volume
2019	7:45	597	594	4:15	1,125	686
2022	<i>7:45*</i>	<i>504</i>	609	<i>4:15*</i>	1,121	650
2022	8:00	531	595	4:00	1,101	721
2022 Adjusted		634	630		1,194	728

<sup>\* 2022 7:45</sup> AM and 4:15 PM peak hours provided for direct comparison of corresponding hours in 2019 and 2022.

peak hours (8:00 AM-9:00 AM and 4:00 PM-5:00 PM) to the peak hour volumes recorded from 7:45 AM-8:45 AM and from 4:15 to 5:15 PM in 2019 and then also to the volumes recently recorded for the corresponding time periods. In both cases, there are few substantive differences, suggesting to CivTech that volumes have returned approximately to pre-Covid levels and that the construction has not deterred many commuters from this route and that these commuters find it the most convenient route to and from their places of employment, despite the construction. The most substantive difference is during the AM peak hour, where 93 more eastbound vehicles were recorded in 2019 during the corresponding hour and 66 more vehicles were recorded during the two peak hours. Comparing the PM peak hours, 24 more eastbound vehicles were recorded in 2019 than in 2022, although there were 35 more westbound vehicles recorded in 2022 than 2019. The through movements on Hayden Road from 2019 were projected to 2022 levels by applying a 2% annual growth rate for three years; these can be seen in the last row of the table. These resulting 2022 through volumes on Hayden Road then replaced the existing 2022 traffic volumes. The adjusted 2022 AM and PM peak hour volumes can be seen in **Figure 3**.

### **CRASH HISTORY**

The most recent three (3) years-worth of historical crash data readily available from ADOT's crash history database for the intersection of Hayden Road and 84<sup>th</sup> Street was reviewed as part of this analysis. In total, there were 19 reported incidents at this intersection over the considered three-year period. A summary of what was revealed by this data is presented in **Table 2** at the top of the next page. Crash analysis worksheets are included as **Attachment C.** 

Of all the incidents reviewed, 6 resulted in 10 non-fatal injuries. The highest percentage of incidents were left turn collisions. No crashes of types "susceptible to correction" by the installation of a traffic control signal (typically

FIGURE 3 – Adjusted 2022 Peak Hour Volumes

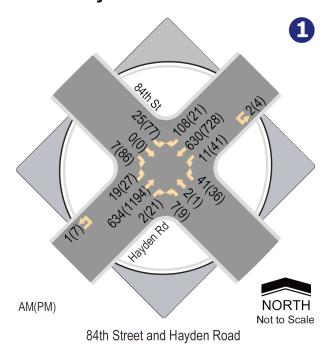




Table 2 - Crash Data Summary for 2019 thru 2021

			ear (	of ence		Incide Severi			I	nciden	t Type		
General Location	3-Year Total	2021	2020	2019	Non-Injury	Injury (# Injuries)	Fatality	Single Vehicle	Angle		Rear End	Head On	Side Swipe
Hayden Rd and 84 <sup>th</sup> St	19	7	5	7	13	6(10)	0	1	3	7	2	0	5

considered angle and left turn crashes) were reported in numbers (5 incidents in a 12-month period) as to allow a traffic signal to be considered at the intersection based on crash experience.

### **FUTURE BACKGROUND TRAFFIC**

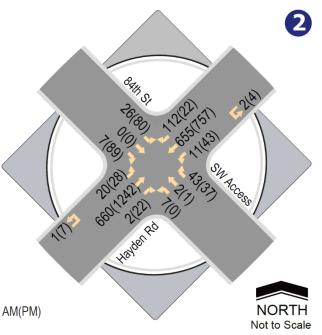
In order to estimate background traffic volumes, a growth rate was needed to estimate growth in ambient traffic, or traffic in the surrounding area not including new trips estimated to be generated by the site. CivTech applied the same growth rate of 2% per year used in prior studies (an annual growth factor of 1.02), which was calculated using City of Scottsdale's Average Daily Traffic Volumes on Northsight Boulevard southeast of 87th Street. CivTech applied a factor of 1.04 (= 1.02²) to each of the adjusted volumes in **Figure 3** to project the opening year—now expected to be 2024—background traffic volumes presented in **Figure 4**.

#### PROPOSED DEVELOPMENT

#### LAND USE AND LOCATION

The Project consists of the following land uses and approximate floor areas (ksf or 1,000 sf): bar

FIGURE 4 – 2024 Background Volumes



84th Street/SW Access and Hayden Road

and retail (1.312 ksf), gallery (3.161 ksf), recording studio (1.245 ksf), and office building (6.780 ksf). The site is a single parcel of approximately 1.05 acres, located on the east side of Hayden Road just north of 84<sup>th</sup> Street.

#### SITE ACCESS

Two City comments required a connection from Lot 1A and the main Core Center access to Lot 1B and a "cohesive site plan" such that both Lots 1B and 1A would have a direct access to the existing main access (the "Southwest Access" for Lot 1B and the expected traffic signal there) and to the secondary "Northeast Access" for Lot 1B, an existing right-in/right-out driveway on Hayden Road.



### TRIP GENERATION AND COMPARISON

The potential trip generation for the project was estimated using the latest ( $11^{th}$ ) edition of the Institute of Transportation Engineers (ITE) *Trip Generation Manual* (TripGen11) and the  $3^{rd}$  edition of ITE's *Trip Generation Handbook*. TripGen11 contains data collected by various transportation professionals for a wide range of different land uses. The data are summarized in the report and average rates and equations have been established that correlate the relationship between an independent variable that describes the development size and generated trips for each categorized land use. The report provides information for daily and peak hour trips. TripGen11 provides trip generation data for offices and bars. No information is available for a recording studio, which is expected to be an amenity for the offices (as a place to record podcasts) and, thus, will be combined with the offices for the purpose of generating trips. The retail shop is intended to be an amenity for cigar smokers, where they can purchase more cigars; thus, the retail and bar floor areas are combined. The data publish for a museum is based on a single observation and provides no daily rate; therefore, CivTech applied to the PM peak hour trips generated by the museum the same ratio of daily to PM peak hour trips as that of a library (72.05  $\div$  8.16, or 8.83, which CivTech rounded to 9).

**Table 3** is a comparison of the weekday daily and peak hour trip generation potentials of previously approved land use to those of the currently proposed land use.

As summarized in **Table 3**, the previously approved multi-use development plan had the potential to generate up to 4,406 weekday daily trips, with 183 trips generated during the AM peak hour and 426 trips generated during the PM peak hour. The currently proposed mixed-use office and bar development on Lot 1B is estimated to generate up to 246 weekday daily trips, with 20 trips generated during the AM peak hour and 70 trips generated during the PM peak hour. The proposed mixed-use office and bar development has the potential to generate 4,166 *fewer* weekday daily trips, with 163

TABLE 3 - WEEKDAY TRIP GENERATION COMPARISON

		<u>-</u>			Trips	Genera	ted		
	ITE		Daily	AM	l Peak H	lour	PN	1 Peak I	lour
Land Use	Code	Size Units	Total	In	Out	Total	In	Out	Total
Previously Approved	(2019)	Developmen	nt Plan	<del>-</del>					
General Office	710	124 ksf	1,308	123	20	143	22	118	140
Shopping Center	820	35 ksf	1,322	20	13	33	64	69	133
Quality Restaurant	931	35 ksf	2,934	22	4	26	183	90	273
		Total Trips	5,564	165	37	202	269	277	546
		Reductions	(1,158)	(15)	(4)	(19)	(70)	(50)	(110)
Total Ex	ternal	Vehicle Trips	4,406	150	33	183	199	227	426
Currently Proposed (2	2022) C	CORE Center	Lot 1B De	evelopm	ent Pla	n			
Bar	975	1.681 ksf	120	1	0	1	13	6	19
General Office	710	8.025 ksf	130	17	2	19	3	17	20
Museum	580	3.161 ksf	6	1	0	1	0	1	1
		Total Trips	256	19	2	21	16	24	40
Inter	nal Capt	ure Reduction	6	0	0	0	1	0	1
Alte	rnate M	ode Reduction	10	1	0	1	1	1	2
Total Ex	ternal	Vehicle Trips	240	18	2	20	14	23	<i>37</i>
Trip Generation Inc	rease/	(Reduction)	(4,166)	(132)	(31)	(163)	(185)	(204)	(389)



*fewer* trips generated during the AM peak hour and 389 *fewer* trips generated during the PM peak hour than that of the previously approved development plan from 2019.

As mentioned previously, there is a separate proposed development located at "Lot 1A" (APN 215-52-106), immediately adjacent to the Project site, to the southwest. The trip generation from this development was included in the original approved overall development and is included in the comparison for consistency. **Table 4** is a comparison of the weekday daily and peak hour trip generation potentials of the previously approved land use to those of the two currently proposed developments.

**Trips Generated** ITE **Daily PM Peak Hour AM Peak Hour** Code Size Units Total **Land Use** In Out Out **Total** Previously Approved (2019) Development Plan (from Table 3) Total External Vehicle Trips 4,406 *150 33 183* 199 227 426 Currently Proposed (2022) CORE Center Lot 1B Development Plan (from Table 3) Total External Vehicle Trips 240 18 2 20 23 *37* Currently Proposed (2022) CORE Center Lot 1A Development Plan 710 25.692 General Office 52 45 54 **KSF** 356 46 79 79 Strip Retail Plaza 822 27.066 **KSF** 1,372 38 26 64 158 310 140 Rooms 1,094 35 28 63 39 37 Hotel 76 2,822 119 60 179 127 288 Total Trips 161 Internal Capture Reduction (3)(2)(16)(16)(32)(192)(5)Alternate Mode Reduction (112)(5)(3)(5)(11)(8)(6)Total External Vehicle Trips 2,518 *111* 55 166 *106* 139 245 Total Proposed 2022 Trips ( 2,758 129 *57* 186 120 162 282 (21)Trip Generation Increase/(Reduction) (1,648) 24 3 (79) (65) (144)

TABLE 4 - WEEKDAY TRIP GENERATION COMPARISON

As summarized in **Table 4**, the *two* currently proposed mixed-use developments (on Lots 1B *and* 1A) are estimated to generate a total of up to 2,758 weekday daily trips, with 120 trips generated during the AM peak hour and 282 trips generated during the PM peak hour. The proposed two mixed-use developments have the potential to generate 1,648 *fewer* weekday daily trips, with 3 *additional* trips generated during the AM peak hour and 144 *fewer* trips generated during the PM peak hour than that of the previously approved development plan from 2019.

### TRIP DISTRIBUTION AND ASSIGNMENT

In prior versions of this study, a single site access (now designated the NE Access for Lot 1B) would have allowed only right-in/right-out movements only. Under the revised, cohesive site plan of **Attachment A**, Lot 1B patrons will have access to the main Core Center access (the SW Access for use by Lot 1B patrons) and the future traffic signal there. Patrons of Lot 1B facilities arriving from the north and departing to the south will now be able to use the signalized SW Access and not need to make U-turns anywhere on Hayden Road. The resulting trip distribution percentages for the study area are shown in **Table 5**.



### **SITE TRAFFIC**

By applying the percentages in **Table 5** to the proposed entering and exiting trips in **Table 4**, the AM and PM peak hour and weekday site volumes into and out from the two site driveways can be estimated. The results are illustrated in **Figure 6**. Please note that, since the site plan is now more cohesive and Lot 1B patrons will have direct access to the southwest/main acccess, there is no longer the need for existing patrons to

TABLE 5 - SITE TRIP DISTRIBUTION

Direction (To/From)	Percentage
North on Hayden Road (to Northsight Boulevard and beyond)	70%
West on Frank Lloyd Wright Boulevard (west of Northsight Boulevard)	20%
East on Frank Lloyd Wright Boulevard (east of Hayden Road)	35%
South on Northsight Boulevard (south of Hayden Road)	15%
South on Hayden Road (after U-turn at Roundabout)	30%
South on Hayden Road (southwest of 83 <sup>rd</sup> Place)	20%
South on 83 <sup>rd</sup> Place (southeast of Hayden Road)	10%
Total	100%

NE Access & Hayden Road

depart to the north and make U-turns at the Hayden/Northsight roundabout to travel to the south as had been shown in prior versions. Adding the site trips in **Figure 6** to the 2024 background trips in **Figure 5** yields the total peak hour volumes, which are illustrated in **Figure 6**.

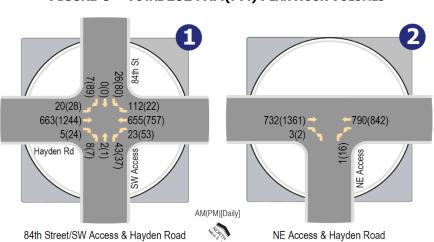
0(0)[0] 0(0)[0] 0(0)[0] 0(0)[0] 0(0)[0] 0(0)[0] 12(10)[84] 3(2)[18] 12(10)[84] 3(2)[18] 12(10)[84] 3(2)[18] 12(10)[84] 3(2)[18] 12(10)[84] 12(1

FIGURE 5 – SITE GENERATED VOLUMES



84th Street/SW Access & Hayden Road

AM(PM)[Daily]





### TRAFFIC SIGNAL WARRANT ANALYSIS

As noted, the City reviewed the 2<sup>nd</sup> (June 2023) version of this statement and made three comments, the first and third of which are addressed above. This section has been revised in response to the second comment. Since a new cohesive site plan has been developed, a review of **Attachment A** now confirms that, whereas previously there was to be no convenient access from Lot 1B to the existing main/SW Access and the potential future traffic signal the intersection of **84<sup>th</sup> Street and Hayden Road/SW Access**, there will now be a direct link from Lot 1B. Since there was an existing zoning stipulation from the City that tasked the developer of either Lot 1B or Lot 1A to conduct a traffic signal warrant analysis for the intersection, the signal warrant analysis below was provided in prior versions. Subsequently, the City reviewer has acknowledged something CivTech noted in its 2<sup>nd</sup> version of this study: that the developer of "Lot 1A property has taken responsibility for the construction of the traffic signal." This, however, does not relieve the developer of Lot 1B from providing this analysis. In this analysis, CivTech utilized projected existing, background, and total traffic volumes for the intersection. These volumes include existing volumes grown to account for regional growth added to the volumes produced by the proposed development. The signal warrant analysis worksheets can be found in **Attachment D**.

The traffic signal warrant analysis was performed in accordance with standard traffic signal warranting criteria found in the *Manual on Uniform Traffic Control Devices, 2009 Edition* (MUTCD). The MUTCD describes eight conditions under which a traffic signal might be warranted, designated Warrants 1 through 8, and indicates that, "The investigation of the need for a traffic control signal shall include an analysis of the applicable factors contained in the [eight] traffic signal warrants and other factors related to existing operation and safety at the study location" while cautioning that, "The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal." The MUTCD suggests that traffic control signals should not be installed unless one or more of the signal warrants are met. However, the satisfaction of a warrant or warrants is not in itself justification for a signal. Every situation is unique and warrant guidelines must be supplemented by the effects of specific site conditions and the application of good engineering judgment. Installation of a traffic signal should improve the overall safety and/or operation of an intersection and should be considered only when deemed necessary by careful traffic analysis and after less restrictive solutions have been attempted. It was this criterion to which the anticipated approach traffic volumes at the one (1) study intersection were compared to determine whether or not a traffic signal is currently warranted.

#### Warrant 1: Eight-Hour Vehicular Volume

The Eight-Hour Vehicular Volume Warrant is intended for locations where either of the following two conditions, or a combination of both, exist for each of any 8 hours of an average day and is, thus, the principal reason to consider the installation of a traffic signal: a large volume of intersecting traffic or traffic volumes so heavy on the major street that entering vehicles suffer extensive delay or conflict.

#### Condition A: Minimum Vehicular Volume

Condition A, the Minimum Vehicular Volume, is intended for application at locations where a large volume of intersecting traffic is the principal reason to consider installing a traffic control signal. The



need for a traffic control signal shall be considered if the vehicles per hour given in both of the 100 percent columns of Condition A in **Table 4C-1** of the MUTCD (reproduced below) occur on the major street and the higher-volume minor-street approaches, respectively, to the intersection for each of any 8 hours of an average day.

### Condition B: Interruption of Continuous Traffic

Condition B, the Interruption of Continuous Traffic, is intended for application at locations where the traffic volume on a major street is so heavy that traffic on a minor intersecting street suffers excessive delay or conflict in entering or crossing the major street. The need for a traffic control signal shall be considered if the vehicles per hour given in both of the 100 percent columns of Condition B in **Table 4C-1** of the MUTCD occur on the major-street and the higher-volume minor-street approaches, respectively, to the intersection for each of any 8 hours of an average day.

Table 4C-1. Warrant 1, Eight-Hour Vehicular Volume
Condition A—Minimum Vehicular Volume

						<del></del>			
Number of lar traffic on ea	nes for moving ch approach		s per hou al of both			Vehicle minor-stre	s per hour et approacl		
Major Street	Minor Street	100%ª	80% <sup>b</sup>	70% <sup>c</sup>	56% <sup>d</sup>	100%ª	80% <sup>b</sup>	70% <sup>c</sup>	56% <sup>d</sup>
1	1	500	400	350	280	150	120	105	84
2 or more	1	600	480	420	336	150	120	105	84
2 or more	2 or more	600	480	420	336	200	160	140	112
1	2 or more	500	400	350	280	200	160	140	112

**Condition B—Interruption of Continuous Traffic** 

Number of lar traffic on ea	nes for moving ch approach			r on majo approach			es per hour et approacl		
Major Street	Minor Street	100%ª	80%b	70% <sup>c</sup>	56% <sup>d</sup>	100%ª	80% <sup>b</sup>	70% <sup>c</sup>	56% <sup>d</sup>
1	1	750 600 525 420 75		60	53	42			
2 or more	1	900	720	630	504	75	60	53	42
2 or more	2 or more	900	720	630	504	100	80	70	56
1	2 or more	750	600	525	420	100	80	70	56

<sup>&</sup>lt;sup>a</sup> Basic minimum hourly volume.

#### Combination of Conditions: A and B

The combination of Conditions A and B is intended for application at locations where Condition A is not satisfied and Condition B is not satisfied and should be applied only after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems. The need for a traffic control signal shall be considered if the vehicles per hour given in both of the 80 percent columns of Conditions A and Condition B in **Table 4C-1** of the MUTCD occur on the major-street and the higher-volume minor-street approaches, respectively, to the intersection for each of any 8 hours of an average day.



<sup>&</sup>lt;sup>b</sup>Used for combination of Conditions A and B after adequate trial of other remedial measures.

<sup>&</sup>lt;sup>c</sup> May be used when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000.

<sup>&</sup>lt;sup>d</sup> May be used for combination of Conditions A and B after adequate trial of other remedial measures when the majorstreet speed exceeds 40 mph or in an isolated community with a population of less than 10,000.

#### Warrant 2: Four-Hour Vehicular Volume

The Four-Hour Vehicular Volume signal warrant conditions are intended to be applied where the volume of intersecting traffic is the principal reason to consider installing a traffic control signal. The need for a traffic control signal shall be considered if an engineering study finds that, for each of any 4 hours of an average day, the plotted points representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the higher-volume minor street approach (one direction only) all fall above the applicable curve in **Figure 4C-2** (this and all other referenced figures are attached) for the existing combination of approach lanes.

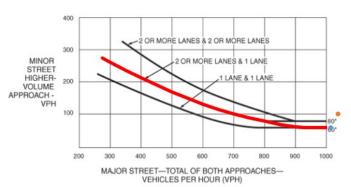
#### Warrant 2: Four-Hour Vehicular Volume

The Four-Hour Vehicular Volume signal warrant conditions are intended to be applied where the volume of intersecting traffic is the principal reason to consider installing a traffic control signal. The need for a traffic control signal shall be considered if an engineering study finds that, for each of any

4 hours of an average day, the plotted points representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the higher-volume minor street approach (one direction only) all fall above the applicable curve in **Figure 4C-2** (this and all other referenced figures are attached) for the existing combination of approach lanes.

Since the posted speed limit on Hayden Road exceeds 40 mph, **Figure 4C-2** was used.

Figure 4C-2. Warrant 2, Four-Hour Vehicular Volume (70% Factor)
(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



"Note: 80 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 60 vph applies as the lower threshold volume for a minor-street approach with one lane.

### Warrant 3: Peak-Hour Vehicular Volume

The Peak Hour signal warrant is intended for use at a location where traffic conditions are such that for a minimum of 1 hour of an average day, the minor-street traffic suffers undue delay when entering or crossing the major street. It shall be applied only in unusual cases, such as office complexes, manufacturing plants, industrial complexes, or high-occupancy vehicle facilities that attract or discharge large numbers of vehicles over a short time.

The need for a traffic control signal shall be considered if an engineering study finds that the criteria in either of the following two categories are met:

A. If all three of the following conditions exist for the same 1 hour (any four consecutive 15-minute periods) of an average day:



- 1. The total stopped time delay experienced by the traffic on one minor-street approach (one direction only) controlled by a STOP sign equals or exceeds: 4 vehicle-hours for a one-lane approach; or 5 vehicle-hours for a two-lane approach, and
- 2. The volume on the same minor-street approach (one direction only) equals or exceeds 100 vehicles per hour for one moving lane of traffic or 150 vehicles per hour for two moving lanes, and
- 3. The total entering volume serviced during the hour equals or exceeds 650 vehicles per hour for intersections with three approaches or 800 vehicles per hour for intersections with four or more approaches.
- B. The plotted point representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the higher-volume minor-street approach (one direction only) for 1 hour (any four consecutive 15-minute periods) of an average day falls above the applicable curve in Figure 4C-3 for the existing combination of approach lanes.

If the posted speed limit on the major street exceeds 40 mph, Figure 4C-4 may be used in place of Figure 4C-3 to satisfy the criteria in the second category of the Standard. Since the posted speed limit on Hayden Road is 45 mph, Figure 4C-4 was used for this analysis.

Please note that\_the values for Warrants 2 and 3 shown on **Attachment D** were automatically calculated using formulae, not determined by the plotting method described in the MUTCD. These formulae approximate the curves in the MUTCD; they were developed by others and found in the Indiana Supplement to the year 2000 MUTCD.

**Table 5** summarizes the volume-based warrant analysis results for the existing conditions.

TABLE 5 - TRAFFIC SIGNAL ANALYSIS SUMMARY WARRANTS 1 - 3

		Warrar				
Intersection	Condition A	Condition B	Combination of A & B	Overall	Warrant 2	Warrant 3
Existing Scenario		-				
84 <sup>th</sup> Street and Hayden Road	No	Yes	No	Yes	Yes	Yes
Opening Year 2023 - No Bui	ld Scenario					
84 <sup>th</sup> Street and Hayden Road	No	Yes	No	Yes	Yes	Yes
Opening Year 2023 - Build S	Scenario	•	•			
84 <sup>th</sup> Street and Hayden Road	No	Yes	No	Yes	Yes	Yes

The results of the volume-based traffic signal warrants analysis indicate that traffic volumes on 84<sup>th</sup> Street and Hayden Road already exceed the criteria for Warrants 1, 2 and 3 under existing conditions, that is, without the project proposed for either Lot 1B or Lot 1A. Worksheets used for the signal warrant analysis are included in **Attachment D**. A traffic signal is warranted and, as indicated in a



review comment on a previous version of this study, the City understands that the developer of Lot 1A property has taken responsibility for the construction of the traffic signal.

### **CONCLUSIONS**

The following can be concluded from the above analysis:

- The proposed multi-use development on Lot 1B is estimated to generate 246 weekday daily trips, with 20 trips generated during the AM peak hour and 70 trips generated during the PM peak hour. This is 4,166 *fewer* vehicular trips on a weekday—almost 95 percent *fewer* trips than could have been generated by previously approved multi-use development—with 163 and 389 *fewer* trips generated during the AM and PM peak hours, respectively.
- The two currently proposed developments (on Lots 1B and 1A) are estimated to generate 2,758 weekday daily trips, with 120 trips generated during the AM peak hour and 282 trips generated during the PM peak hour. This is 1,648 *fewer* vehicular trips on a weekday—33 percent *fewer* trips than could be generated by previously approved multi-use development—with 3 additional trips generated during the AM peak hour and 144 *fewer* trips generated during the PM peak hour.
- The results of the volume-based traffic signal warrant analysis indicate that traffic volumes of 84<sup>th</sup> Street and Hayden Road already exceed the criteria for Warrants 1, 2 and 3 without the project, that is, without the project proposed for either Lot 1B or Lot 1A. A traffic signal is warranted and, as indicated in a review comment on a previous version of this study, the City understands that the developer of Lot 1A property has taken responsibility for the construction of the traffic signal.

Thank you for allowing CivTech to provide engineering services for this project. Should there be any questions please contact me at (480) 659-4250.

Sincerely,

CivTech

Joseph Spadafino, P.E., PTOE, PTP

Senior Project Manager/Traffic Engineer

Attachments (4)

A. Site Plan

B. Existing Traffic Counts

C. Crash Analysis Worksheets

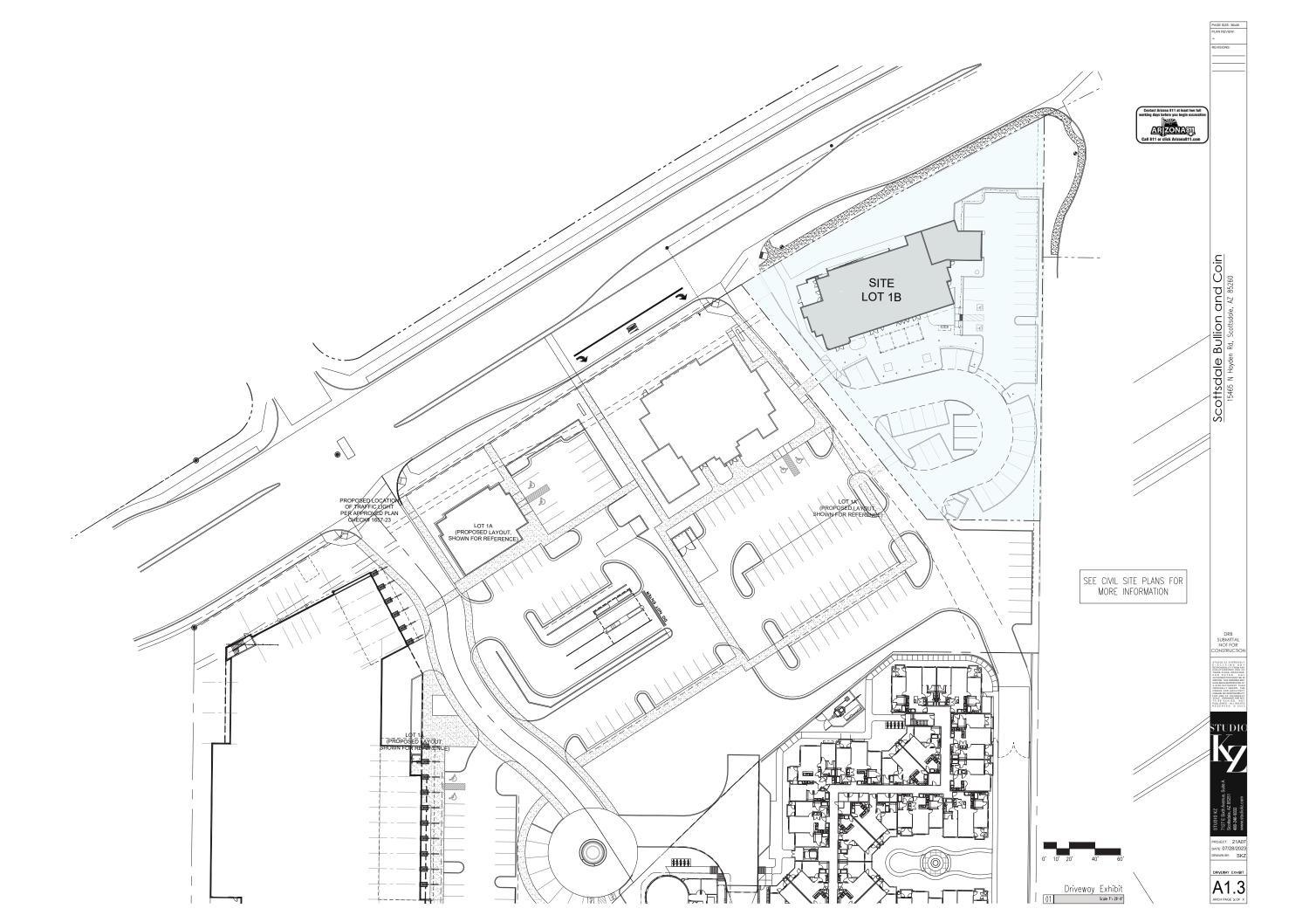
D. Signal Warrant Analysis



# **ATTACHMENT A**

SITE PLAN





# **ATTACHMENT B**

**EXISTING TRAFFIC COUNTS** 





Location: 1 84TH ST & HAYDEN RD AM

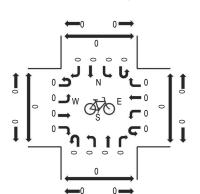
Date: Tuesday, November 15, 2022 Peak Hour: 11:45 AM - 12:45 PM Peak 15-Minutes: 12:00 PM - 12:15 PM

#### **Peak Hour - Motorized Vehicles** (1,123)111 0.70 99 (1,169) 58 HAYDEN RD (9,538)(10, 193)923 970 0.98 0.94 1,113 1,179

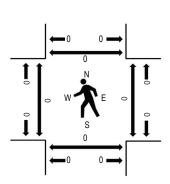
(12,964)

HAYDEN RD

(606)



Peak Hour - Bicycles



Peak Hour - Pedestrians

Note: Total study counts contained in parentheses.

0.77

50

### **Traffic Counts - Motorized Vehicles**

43

(12,317)

84TH ST

(568)

	ranic counts				,,,,,						0.4=				0.45								
		ŀ	HAYDE			H	IAYDE				84TH				84TF				D III	_			
	Interval Start Time	U-Turn	Eastb Left		Right	U-Turn	Westb		iaht	U-Turn	Northb Left		Diaht	U-Turn	South! Left	Thru	Right		Rolling Hour			Crossin South N	•
_	12:00 AM	0-14111	0	2	Night 0	2	1	0	1911t 1	0-14111	0	0	rigiit 1	0-14111	1	0	Night 0	8	31	0	0	0	0
	12:15 AM	0	1	1	0	0	1	0	0	0	0	0	0	0	0	0	0	3	34		0	0	0
	12:30 AM	0	0	2	1	0	2	1	1	0	0	0	2	0	5	0	0	14	38		0	0	0
	12:45 AM	0	0	3	0	0	0	1	0	0	0	0	2	0	0	0	0	6	30		0	0	0
	1:00 AM	0	0	0	2	0	2	2	2	0	0	0	1	0	1	0	1	11	33		0	0	0
	1:15 AM	0	0	1	0	0	2	2	1	0	0	0	0	0	1	0	0	7	26		0	0	0
	1:30 AM	0	0	2	0	1	0	0	1	0	0	0	0	0	0	0	2	6	22		0	0	0
	1:45 AM	0	0	0	0	0	0	8	0	0	0	0	1	0	0	0	0	9	20		0	0	0
	2:00 AM	0	0	1	0	0	0	0	1	0	0	0	0	0	2	0	0	4	14		0	0	0
	2:15 AM	0	0	0	0	1	1	0	0	0	0	0	1	0	0	0	0	3	15		0	0	0
	2:30 AM	0	0	1	0	0	1	0	2	0	0	0	0	0	0	0	0	4	16		0	0	0
	2:45 AM	0	0	0	0	1	0	1	0	0	0	0	0	0	1	0	0	3	16		0	0	0
	3:00 AM	0	0	2	0	0	0	1	1	0	0	0	0	0	1	0	0	5	26		0	0	0
	3:15 AM	0	0	2	0	0	0	1	1	0	0	0	0	0	0	0	0	4	32		0	0	0
	3:30 AM	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0	4	41	0	0	0	0
	3:45 AM	0	0	2	0	0	0	6	4	0	0	0	1	0	0	0	0	13	51	0	0	0	0
	4:00 AM	0	0	1	0	2	2	1	2	0	2	0	1	0	0	0	0	11	66		0	0	0
	4:15 AM	0	0	0	0	2	0	4	4	0	1	0	2	0	0	0	0	13	98		0	0	0
	4:30 AM	0	0	2	0	0	0	6	2	0	0	0	3	0	1	0	0	14	115		0	0	0
	4:45 AM	0	1	5	0	1	0	15	6	0	0	0	0	0	0	0	0	28	157		0	0	0
	5:00 AM	0	1	11	0	0	3	18	7	0	0	0	2	0	1	0	0	43	237		0	0	0
	5:15 AM	0	1	5	0	0	0	15	4	0	3	0	2	0	0	0	0	30	309		0	0	0
	5:30 AM	0	6	6	0	0	0	24	14	0	1	0	2	0	3	0	0	56	408		0	0	1
	5:45 AM	0	7	7	0	0	0	49	36	0	3	0	4	0	2	0	0	108	489		0	0	0
	6:00 AM	0	2	28	2	0	1	47	25	0	1	0	4	0	4	0	1	115	574		0	0	1
	6:15 AM	0	5	33	1	2	1	63	18	0	0	0	4	0	2	0	0	129	683		0	0	0
	6:30 AM	0	1	53	2	0	4	54	10	0	0	0	11	0	1	0	1	137	785		0	0	0
	6:45 AM	0	5	59	0	0	2	94	23	0	0	0	8	0	1	0	1	193	904		0	0	0
	7:00 AM	0	7	68	0	0	2	93	28	0	2	0	18	0	4	0	2	224	1,049	0	0	0	0
	7:15 AM	0	5	91	1	1	2	102	15	0	2	0	7	0	3	0	2	231	1,153	0	0	1	0
	7:30 AM	0	4	89	1	1	4	118	12	0	3	0	18	0	5	0	1	256	1,241	0	0	1	0
	7:45 AM	1	9	116	1	0	1	155	29	0	2	0	13	0	4	0	7	338	1,345		0	1	1
Ē	8:00 AM	1	3	132	0	1	4	135	31	0	2	2	9	0	5	0	3	328	1,351	0	0	0	1
3	8:15 AM	0	5	126	0	0	3	147	19	0	0	0	11	0	8	0	0	319	1,381	0	0	0	0
1	8:30 AM	0	4	130	0	0	1	172	33	0	1	0	9	0	7	0	3	360	1,443	0	0	0	0
	8:45 AM	0	7	143	2	1	3	141	25	0	4	0	12	0	5	0	1	344	1,476	0	0	0	0

		-		-	-	-																
Count Total	80		11,743	176	58	389	8,901	845	0	78	5	523	1	640	3		24,239		4	0	13	15
11:45 PM	0	0	2	0	0	0	ı	0	0	0	0	2	0	2	0	0	7		0	0	0	(
11:15 PM 11:30 PM	0	0	6 3	3	0	1	0	0 1	0	0	0	0 2	0	0 1	0	0	10 10		0	0	0	(
11:00 PM	0	0	4	2	2	1	0	1	0	0	0	1	0	2	0	0	13	40	0	0	0	(
10:45 PM	0	0	13	0	0	2	2	1	0	0	0	0	0	0	0	0	18	51	0	0	0	(
10:30 PM	0	0	14	1	0	0	3	1	0	0	0	0	0	2	0	1	22	63	0	0	0	(
10:15 PM	0	0	10	4	0	3	1	0	0	0	0	2	0	2	0	0	22	75	0	0	0	(
10:00 PM	2	0	16	1	0	3	2	1	0	0	0	0	0	1	0	1	27	89	0	0	0	(
9:45 PM	0	0	19	2	0	0	0	0	0	0	0	1	0	0	0	1	23	94	0	0	0	(
9:30 PM	0	0	25	1	0	5	3	2	0	0	0	0	0	2	0	0	38	110	0	0	0	(
9:15 PM	0	1	23	1	0	1	4	0	0	0	0	2	0	2	0	1	35	123	0	0	0	(
9:00 PM	0	3	29	1	1	4	12	1	0	1	0	4	0	5	0	5	66	162	0	0	0	(
8:45 PM	0	0	56	1	2	3	12	1	0	0	0	2	0	2	0	0	79	218	0	0	2	(
8:30 PM	0	1	76 70	4 1	1	5	14	3	0	0	0	2	0	4	0	1	102	35 <i>1</i> 282	0	0	0	(
8:00 PM 8:15 PM	1	1	87 76	4	0	7 6	19 18	2 1	0	1	0	7 1	0	8 4	0	1	138 110	429 357	0	0	0	(
7:45 PM	0	1	92 97	3	0	3	28	1	0	0	0	2	0	2	0	1	133	483	0	0	0	(
7:30 PM	0	0	116	1	0	5	28	2	0	0	0	6	0	6	0	5	169	550	0	0	0	(
7:15 PM	1	5	106	3	1	7	39	4	0	0	0	7	0	7	0	4	184	624	0	0	0	(
7:00 PM	0	0	147	4	0	10	46	1	0	2	0	4	1	4	1	1	221	707	0	0	0	(
6:45 PM	2	4	129	2	1	6	70	2	0	3	0	2	0	4	0	2	227	801	0	0	0	(
6:30 PM	1	0	174	11	1	7	69	2	0	0	0	5	0	5	0	5	280	912	0	0	0	(
6:15 PM	1	3	212	4	1	14	77	6	0	1	0	8	0	3	0	3	333	1,061	0	0	0	(
6:00 PM	1	2	201	6	0	8	98	2	0	1	0	5	0	11	0	6	341	1,181	1	0	1	(
5:45 PM	0	10	264	2	1	9	95	0	0	3	0	3	0	22	0	13	422	1,376	0	0	0	,
5:30 PM	5	5	261	2	0	7	98	5	0	0	0	7	0	15	0	5	410	1,506	0	0	0	(
5:15 PM	0	11	280	2	0	15	129	2	0	1	0	11	0	8	0	6	465	1,638	0	0	0	(
5:00 PM	3	4	286	6	0	10	136	2	0	2	0	13	0	14	0	7	483	1,780	0	0	0	
4:30 PM 4:45 PM	2	2	279 284	6 8	1	11 7	169 152	7 7	0	4 1	1	10 8	0	27 14	0	34 8	553 493	1,994 1,851	1	0	0	2
4:15 PM	2	17	272	3	2	16	193	4	0	2	0	10	0	18 27	0	26 34	565	2,094	0	0	0	
4:00 PM	3	4	266	4	1	7	207	3	0	2	0	8	0	18	0	18	541 565	2,152	0	0	0	
3:45 PM	3	8	253	4	1	9	198	3	0	0	0	7	0	16	0	15	517	2,176	0	0	0	
3:30 PM	0	1	305	8	0	9	193	8	0	0	0	3	0	23	0	16	566	2,197	0	0	1	
3:15 PM	2	10	254	0	0	4	203	18	0	1	0	12	0	16	0	12	532	2,156	0	0	1	
3:00 PM	2	5	266	2	0	6	203	17	0	1	0	7	0	15	1	11	536	2,151	0	0	1	
2:45 PM	1	6	271	1	1	12	191	8	0	2	0	10	0	17	0	9	529	2,163	0	0	0	
2:30 PM	5	4	269	1	0	7	198	11	0	1	0	11	0	12	0	11	530	2,127	0	0	0	
2:15 PM	2	5	233	1	0	4	224	10	0	0	0	6	0	8	0	12	505	2,100	0	0	0	(
2:00 PM	3	2	299	3	0	3	207	10	0	0	0	10	0	11	0	13	561	2,125	0	0	0	(
1:45 PM	5	3	237	4	1	8	212	19	0	2	0	6	0	16	0	12	525	2,121	0	0	0	,
1:30 PM	2	7	263	3	1	6	218	14	0	1	0	13	0	12	0	12	552	2,143	0	0	0	
1:00 PM 1:15 PM	1	8	204	2	1	o 5	238	16	0	0	0	7	0	11	0	11	547	2,192	0	0	0	
12:45 PM 1:00 PM	0	9	259 284	2 4	3	3 8	204 206	15 22	0	1	0	10 4	0	11 15	0	6 14	523 568	2,190 2,192	0	0	0	
12:30 PM	5	6	252	2	1	12	212	22	0	1	0	6	0	18	0	13	550	2,188	0	0	0	(
12:15 PM	0	11	278	2	1	9	215	19	0	0	0	11	0	7	0	11	564	2,205	0	0	0	(
12:00 PM	2	5	280	4	4	6	210	13	0	2	0	14	0	12	0	16	568	2,205	0	0	0	
11:45 AM	2	2	258	4	1	4	221	20	0	0	1	15	0	21	0	13	562	2,244	0	0	0	-
11:30 AM	3	3	259	1	3	4	169	23	0	1	1	7	0	13	1	15	503	2,197	1	0	0	
11:15 AM	4	9	283	1	2	3	168	9	0	1	0	5	0	6	0	15	506	2,139	0	0	1	
11:00 AM	3	9	274	0	1	2	211	10	0	0	0	6	0	19	0	6	541	2,112	0	0	0	
10:45 AM	5	10	217	7	2	9	224	22	0	0	0	8	0	8	0	10	522	2,072	0	0	0	
10:30 AM	0	5	237	3	2	4	219	17	0	2	0	12	0	10	0	7	518	2,087	0	0	2	
10:15 AM	1	6	234	0	0	4	209	15	0	1	0	7	0	11	0	12	500	2,081	0	0	0	
10:00 AM	1	5	171	2	0	3	204	17	0	2	0	8	0	17	0	0	430	1,970	0	0	0	
9:45 AM	0	7	161	2	0	2	221	14	0	2	0	11	0	6	0	5	431	1,879	0	0	0	
9:15 AM 9:30 AM	0	6	159 156	3	2	3 8	171 188	12 16	0	2	0	8 5	0	9 6	0	6 6	381 393	1,635 1,754	0	0	1	
0.45 484	0	0	450	2	0	2	160	22	0	2	0	10	0	11	0	10	358	1,563	0	0	0	

# **ATTACHMENT C**

**CRASH ANALYSIS WORKSHEETS** 



# Hayden and 84th St

# **SUMMARY BY YEAR**

SEVERITY / INCIDENTS		<u>2019</u>	<u>2020</u>	<u>2021</u>	<u>Totals</u>	Checks
Fatal Injury Inciden	nts					
Non-fatal Injury Inciden	nts	2	2	2	6	
PDO Inciden	nts _	5	3	5	13	
TOTAL	LS	7	5	7	19	
Pedestrian Inciden	nts					
Pedestrians Involve	ed					
Bicycle Inciden	nts					
Bicyclists Involve	ed					
SEVERITY / INVOLVMENT						
Fatal Injurio	es					
Non-Fatal Injurio	es	3	2	5	10	
PDO Vehicle	es	12	5	11	28	
Pedestrians Fatally Injure	ed					
Pedestrians Non-Fatally Injure						
Bicyclists Fatally Injure						
Bicyclists Non-Fatally Injure						
COLLISION MANNER						
SINGLE_VEHICLE	1		1	II.	1	
ANGLE_VEITIGEE	2	1	2		3	
LEFT TURN	3	3		4	7	
REAR END	4	1	1	1	2	
HEAD_ON	5	'	•		_	
SIDESWIPE SAME DIRECTION	6	2	1	2	5	
IDESWIPE_OPPOSITE_DIRECTION	7	_	•	<b>-</b>	Ŭ	
REAR_TO_SIDE	8					
REAR_TO_REAR	9					
— — —	97			1	1	
	99			1	·	
TOTAL	LS	7	5	7	19	

CRASH STATISTICS	<u></u>	nvolveme	<u>ent</u>									<u>Hayden and 84th St</u>
	t S		t S	_								
2010 2021	den		r L - S	ა ი								
2019-2021	ici	n l s	to	Non- ctori.								
	H	o t	Mo	NC Ot O				01 -	77 -	G. J. W.		Palaitional Warful Tufoumation
<u>Incidents</u>	# 19	40 Veh	# 48	# W			First Harmful Event	<u>Code</u>	No.	<u>Code No</u> <u>Month</u>	•	Additional Useful Information
<u>incluents</u> Fatal	0	0 Ppl	0		ircumst	ances	OVERTURN_ROLLOVER	1	0		2	<u>Vehicle Action Codes</u>
Injury	6	10 Ppl	0		& Run?	2	FIRE EXPLOSION	2	0	February 2	1	1 GOING_STRAIGHT_AHEAD
	13	28 Veh		Intersection Re	lated?	10	- IMMERSION	3	0	-	3	2 SLOWING_IN_TRAFFICWAY
<u>Peds/Bikes Summary</u>		σ <u>In</u>	<u>juries</u>				JACKKNIFE	4	0	April 4	1	3 STOPPED_IN_TRAFFICWAY
	ents	ono.					CARGO_EQUIPMENT_LOSS_SHIFT	5	0	May 5	1	4 MAKING_LEFT_TURN
	cide	ers ata	on- ata		<u>Code</u>	No.	FELL_JUMPED_FROM_VEHICLE	6	0		0	5 MAKING_RIGHT_TURN
D 1 4 4	, In	Д Б	NO.	<u>JunctionRelation</u>	0	7	THROWN_OR_FALLING_OBJECT	7	0	July 7	1	6 MAKING_U_TURN
Pedestrian: Bicycle:	0	0 0	0	NOT_JUNCTION_RELATED	0	7 9	OTHER_NON_COLLISION EQUIPMENT_FAILURE_TIRES_BRAKES	8 9	0	3	2	7 OVERTAKING_PASSING
вісусте.	U	0 0		INTERSECTION_NON_INTERCHANGE SECTION_RELATED_NON_INTERCHANGE	2	1	SEPARATION_OF_UNITS	10	0	October 10	1	8 CHANGING_LANES 9 NEGOTIATING_A_CURVE
Cod	de	No.		RANCE_EXIT_RAMP_NON_INTERCHANGE	3	0	RAN_OFF_ROAD_RIGHT	11	0	November 11	1	10 BACKING
<u>LightCondition</u>	<u></u>	<u></u>		RAILWAY_GRADE_CROSSING	4	0	RAN_OFF_ROAD_LEFT	12	0		2	11 Avoiding_Vehicle_Object_Pedestrian
DAYLIGHT	1	16		CROSSOVER_RELATED	5	0	CROSS_MEDIAN	13	0		19	12 ENTERING PARKING POSITION
DAWN	2	0		FRONTAGE_ROAD_NON_INTERCHANGE	6	0	CROSS CENTERLINE	14	0			13 LEAVING_PARKING_POSITION
DUSK	3	1		DRIVEWAY	7	1	DOWNHILL_RUNAWAY	15	0	(Unit) SurfaceCondition		14 PROPERLY_PARKED
DARK_LIGHTED	4	1		ALLEY_ACCESS_RELATED	8	0	MOTOR_VEHICLE_IN_TRANSPORT	16	16	DRY 1		15 IMPROPERLY_PARKED
DARK_NOT_LIGHTED	5	1		UNKNOWN_NON_INTERCHANGE	9	0	PEDESTRIAN	17	0		0	16 DRIVERLESS_MOVING_VEHICLE
DARK_UNKNOWN_LIGHTING	6	0		THRU_ROADWAY	10	0	PEDALCYCLE	18	0		0	17 CROSSING_ROAD
UNKNOWN ! Check Total	99	19	TI	INTERSECTION_INTERCHANGE NTERSECTION_RELATED_INTERCHANGE	11 12	0	RAILWAY_VEHICLE_TRAIN_ENGINE LIGHT_RAILWAY_RAILCAR_VEHICLE	19 20	0		0	18 WALKING_WITH_TRAFFIC 19 WALKING_AGAINST_TRAFFIC
check fotal		10		ENTRANCE_EXIT_RAMP_INTERCHANGE	13	0	ANIMAL_WILD_NON_GAME	21	0	<del>-</del>	0	20 STANDING
Weather				FRONTAGE_ROAD_INTERCHANGE	14	0	ANIMAL_WILD_GAME	22	0		0	21 LYING
CLEAR	1	19		OTHER_PART_OF_INTERCHANGE	15	0	ANIMAL_PET	23	0	MUD_DIRT_GRAVEL 8	0	22 GETTING_ON_OR_OFF_VEHICLE
CLOUDY	2	0		<not defined=""></not>	16	0	ANIMAL_LIVESTOCK	24	0		0	23 WORKING_ON_OR_PUSHING_VEHICLE
SLEET_HAIL_FREEZING_RAIN_OR_DRIZZLE	3	0		UNKNOWN_INTERCHANGE	17	0	PARKED_MOTOR_VEHICLE	25	2		0	24 WORKING_ON_ROAD
RAIN	4	0		UNKNOWN_JUNCTION	18	0	WORK_ZONE_MAINTENANCE_EQUIPMENT	26	0		0	97 OTHER
SNOW SEVERE_CROSSWINDS	5 6	0		UNKNOWN OTHER NON INTERCHANGE	99 109	1 0	STRUCK_BY_FALLING_SHIFTING_CARGO_OR_OBJECT	27 28	0	Total	40	99 UNKNOWN
BLOWING_SAND_SOIL_DIRT	7	0		Check Total	100	19	OTHER_NON_FIXED_OBJECT IMPACT ATTENUATOR CRASH CUSHION	29	0			Body Styles
FOG_SMOG_SMOKE	8	0					BRIDGE_OVERHEAD_STRUCTURE	30	0			-1 NOT_REPORTED
BLOWING SNOW	9	0		<u>CollisionManner</u>			BRIDGE RAIL	31	0			1 \Passenger Vehicles, including RVs
OTHER !	97	0		SINGLE_VEHICLE	1	1	CULVERT	32	0			53 /
UNKNOWN	99	0		ANGLE (front to side)(other than left turn)	2	3	CURB	33	0			54 \TRUCKS
Check Total		19		LEFT_TURN	3	7	DITCH	34	0			88 /
m . CC' . T m				REAR_END	4	2	EMBANKMENT	35	0			89 \MOBILEHOME (NOT RVS)
<u>TrafficWayType</u> ONE_WAY_TRAFFICWAY	1	0		HEAD_ON SIDESWIPE_SAME_DIRECTION	5 6	0 5	GUARDRAIL_FACE GUARDRAIL END	36 37	0			92 / 93 \TRAILERS
TWO_WAY_NOT_DIVIDED	2	2		SIDESWIPE OPPOSITE DIRECTION	7	0	CONCRETE_TRAFFIC_BARRIER	38	0			120 /
DED_WITH_CONTINUOUS_LEFT_TURN_LANE	3	0		REAR_TO_SIDE	8	0	CABLE_TRAFFIC_BARRIER	39	0			121 \MOTORCYCLES
_UNPROTECTED_PAINTED_4_FEET_MEDIAN	4	6		REAR_TO_REAR	9	0	OTHER_TRAFFIC_BARRIER	40	0			128 /
IDED_POSITIVE_MEDIAN_BARRIER	5	9		OTHER	97	1	TREE_BUSH_STUMP_STANDING	41	0			254 UNKNOWN
UNKNOWN	99	2		UNKNOWN	99	0	TRAFFIC_SIGN_SUPPORT	42	0			255 NOT REPORTED
Check Total		19		Check Total		19	TRAFFIC_SIGNAL_SUPPORT	43	0			
				m 151 11 15 15 15 15 15 15 15 15 15 15 15			UTILITY_POLE_LIGHT_SUPPORT	44	0			
<u>Weekday</u>	1	2		TravelDirection 1 NORTH 2 SOUTH		N S	OTHER_POST_POLE_OR_SUPPORT	45 46	1			
Sunday Monday	2	2		2 SOUTH 3 EAST		> E	FENCE MAILBOX	46 47	0			
<del>-</del>	3	3		4 WEST		N .	BUILDING	48	0			
Wednesday	4	0		5 NORTH		NW	OTHER_FIXED_OBJECT	49	0			
Thursday	5	5		6 NORTH	EAST :	NE	UNKNOWN	99	0			
Friday	6	4		7 SOUTH	VEST	SW	Not Reported	255	0			
Saturday	7	3		8 SOUTH		SE	Check Total		19			
Check Total		19		99 UNKNO	٧N	99						

# Hayden and 84th St

<	LOCATION>			UNITS	> <	PERSON	> <	SEVERITY> < GENERAL>
	OFF- D DIS- INTERSECT		- SRFCND ALGMT GRAD			TL TYP INJR VLTN PHSC		
INCIDNT ON STREET	MP SET R TANCE STREET	NCIC NCIC YYMMDD HH:MM W TAI	U1 U2 1 2 1	2 U1 U2 U1 U2				TLMOTNONTTLMOTNON R CN CN REL CWY CD C
3512552 07 HAYDEN	P 0 84th St	725 725 190217 12:43 1 2	1 1 1 1 1	0 0 0 44 44 E	E S 4 1 3		0 255 1	2 0 0 0 0 0 N 1 1 1 4 16 2
						4 2 0 0		
3517982 07 HAYDEN	P 0 84th St	725 725 190319 17:35 3 2	1 1 1 1 1	0 0 0 44 44 E	E S 4 1 2	0 1 1 99 99 7 1 0	0 255 1	0 N 3 1 1 5 16 3
3518214 07 HAYDEN	M 25 84th St	725 725 190315 13:33 6 2	1 1 1 1 1	0 0 0 47 44 S	S S 4 1 3	0 1 4 1 1 99 0 0	0 255 1	0 N 1 1 2 4 16 6
						1 1 99 0		
3576532 07 HAYDEN	P 0 84th St	725 725 190808 15:47 5 2	1 1 1 1 1	0 99 0 30 50 N	N N 1 3 3	0 1 1 1 1 4 1 0	0 255 1	0 N 1 1 1 4 16 4
						4 1 0 0		
3576534 07 HAYDEN	P 150 84th St	725 725 190805 13:19 2 2	1 1 1 1 1	0 0 0 50 47 S	S S 1 1 2	0 1 1 1 1 1 1 0	0 255 1	0 N 1 1 0 5 16 6
3596887 07 HAYDEN	P 0 84th St	725 725 191004 15:55 6 2	1 1 1 1 1	0 0 0 17 17 S	SE S 4 1 3	0 1 1 1 1 20 1 0	0 255 1	1 0 0 0 0 0 N 1 1 1 5 16 3
						4 2 0 0		
3626275 07 HAYDEN	P 0 84th St	725 725 191223 10:23 2 4	1 1 1 1 1	0 0 0 50 30 N	W S 4 1 5	0 1 1 1 1 20 1 0	0 255 1	0 0 N 1 1 1 5 16 3
			1 1 1 1 1	0 0 0 47 44 S	S S 1 1	1 4 1 1 1 0 0	0 255	
						1 1 1 0		
3634000 07 84TH	P 0 Hayden Rd	725 725 200112 11:59 1 2	1 1 1 1 1	0 0 0 44 42 S	SE W 4 1 3	0 1 1 1 1 20 1 0	0 255 1	0 0 N 1 1 0 4 16 2
						4 1 0 0		
3679948 07 HAYDEN	P 0 84th St	725 725 200404 01:32 7 1	1 1 2	99 254 W	v 1 1	0 1 1 2 0	1	0 0 Y 4 1 0 99 46 1
3681900 07 84TH	P 0 Hayden Rd	725 725 200813 16:54 5 2	1 1 1 1 1	0 0 0 47 47 E	E E 4 1 2	0 1 1 2 1 20 1 0	0 255 1	1 0 0 0 0 0 N 1 1 0 5 16 4
3681921 07 HAYDEN	M 279 84th St	725 725 200815 10:25 7 2	1 1 1 1 1	0 0 0 12 12 N	N N 1 1 2	0 1 1 1 1 13 1 0	0 255 1	0 0 N 1 1 0 5 16 6
3684977 07 84TH	P 0 Hayden Rd	725 725 200903 12:02 5 2	1 1 1 1 1	0 0 0 44 41 N	NE SW 4 1 2	0 1 1 1 3 20 1 0	0 255 1	1 0 0 0 0 0 N 1 1 1 5 16 2
3737582 07 84TH	P 1000 Hayden Rd	725 725 210121 14:32 5 3	1 1 1 1 1	0 97 0 83 44 S	S S 16 14 3	0 1 1 1 1 97 1 0	0 255 1	0 0 N 1 1 99 2 25 6
			1 1 1	97 30 S	5 14	1 1 1 0		
3748193 07 HAYDEN	P 0 84th St	725 725 210305 13:45 6 2	1 1 1 1 1	0 99 0 44 50 N	NES 4 1 2	0 1 1 1 1 7 1 0	0 255 1	0 0 N 1 1 1 4 16 3
3782464 07 HAYDEN	P 0 84th St	725 725 210513 13:35 5 2	1 1 1 1 1	0 0 0 44 41 S	S W 4 1 2	0 1 1 3 3 20 1 0	0 255 1	2 0 0 0 0 0 N 1 1 1 5 16 3
3800704 07 HAYDEN	P 118 84th St	725 725 210709 10:48 6 2	1 1 1 1 1	0 0 0 44 83 W	N S 5 4 2	0 1 1 1 1 1 1 0	0 255 1	0 0 N 1 1 7 99 16 6
3835191 07 84TH	M 275 Hayden Rd	725 725 210928 17:50 3 2	1 1 1 1 1	0 99 0 254 44 9	99 W 99 14 2	0 1 1 99 1 97 1 0	0 255 1	0 0 Y 1 1 0 2 25 9°
3854625 07 84TH	P 0 Hayden Rd	725 725 211102 17:41 3 2	1 1 1 1 1	0 0 0 44 50 N	W SE 4 1 3	0 1 1 2 3 20 1 0	0 255 1	3 0 0 0 0 0 N 5 1 1 4 16 3
	-					4 3 0 0		
3858481 07 HAYDEN	P 590 84th St	725 725 211218 12:59 7 2	1 1 1 1 1	0 0 0 31 50 s	S S 4 1 3	0 1 1 1 1 20 1 0	0 255 1	0 0 N 1 1 0 5 16 3
						4 1 0 0		

# **ATTACHMENT D**

**SIGNAL WARRANT ANALYSIS** 



## Summary

# **Volume-Based Traffic Signal Warrants Analysis Summary**

Warrant		Hour(s) of the Day	Hours Required to Meet Warrant	Hours Met	Is Warrant Met?
	Condition A: Minimum Vehicular Volume	Any Eight Hours	8	3	No
Warrant 1. Eight-Hour	Condition B: Interruption of Continuous Traffic	Any Eight Hours	8	10	Yes
Vehicular Volume	Combination of Condition A & Condition B	Any Eight Hours	8	7	No
	(at least 1 of the 3	Overall conditions re	quired to mee	et warrant)	Yes
	rant 2. ehicular Volume	Any Four Hours	4	9	Yes
	rant 3. k Hour	Any One/Peak Hour	1	8	Yes



**Hayden Airpark Development** 

Signal Warrant Analysis

Existing 2022 Counts - 84th Street and Hayden Road

 Major Street: Hayden Road
 Speed Limit:
 45
 Lanes:\*
 2

 Minor Street: 84th Street
 Speed Limit:
 25
 Lanes:\*
 1

 Locale: City of Scottsdale
 \*Number of Approach Lanes of Moving Traffic:

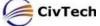
MUTCD Warrants 1-3

Major Street vph - total of both approaches 26 10 56 214 535 956 1,269 1,462 1,855 1,966 2,066 2,056 1,992 1,995 1,943 1,650 1,117 654 30 Minor Street volume - higher-volume approach (vph) 9 17 28 50 59 75 109 103 93 125 163 90 39 5 Direction of higher-volume minor approach SB SB SB SB NB NB NB NB NB SB Beginning of hour 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 18:00 19:00 20:00 21:00 22:00 23:00

Critical speed of major street traffic above 40 mph
In built-up area of isolated community less than 10,000 population

Urban x

arrant 1, Eight-Hou	r Vehicu	lar Volu	me																										
Condition A	Minimum	Vehicula	ar Volum	ne	Criteria	Hour																							
Lanes (M/m):	<u>1/1</u>	2+/1	2+/2+	1/2+		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	2
Minimum Reqmts	500	600	600	500																									
(100% <sup>a</sup> )	150	150	200	200																									
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+	2/1																								
Minimum Reqmts	350	420	420	350	420	No	No	No	No	No	No	Yes	No	No	No	N													
(70%°)	105	105	140	140	105	No	No	No	No	No	No	No	No	No	No	No	Yes	No	No	No	Yes	Yes	No	No	No	No	No	No	١
Warrant met?	No					No	No	No	No	No	No	No	No	No	No	No	Yes	No	No	No	Yes	Yes	No	No	No	No	No	No	N
Condition B	Interruption	on of Co	nt. Traffi	ic .	Criteria	Hour																							
Lanes (M/m):	<u>1/1</u>	2+/1	2+/2+	1/2+		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	:
Minimum Reqmts	750	900	900	750																									
(100% <sup>a</sup> )	75	75	100	100																									
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+	2/1																								
Minimum Reqmts	525	630	630	525	630	No	No	No	No	No	No	No	Yes	No	No	No													
(70%°)	53	53	70	70	53	No	No	No	No	No	No	No	Yes	No	Yes	No	No	No	No	No									
Warrant met?	Yes					No	No	No	No	No	No	No	Yes	No	Yes	No	No	No	No	No									
Combination of	of Conditi	ons A &	В		Criteria	Hour																							
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	
Condition A	400	480	480	400																									
(80% <sup>b</sup> )	120	120	160	160																									
Condition B	600	720	720	600																									
(80% <sup>b</sup> )	60	60	80	80																									
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+	2/1																								
Condition A	280	336	336	280	336	No	No	No	No	No	No	Yes	No	No															
(56% <sup>d</sup> )	84	84	112	112	84	No	No	No	No	No	No	No	No	No	No	No	Yes	No	No	No	No	No							
Condition B	420	504	504	420	504	No	No	No	No	No	No	Yes	No	No	No														
(56% <sup>d</sup> )	42	42	56	56	42	No	No	No	No	No	No	No	Yes	No	No	No	No	No											
Warrant met?	No					No	No	No	No	No	No	No	No	No	No	No	Yes	No	No	No	No	No							
rrant 2, Four Hour	Vehicul	ar Volur	ne		Criteria	Hour																							П
Lanes (M/m):		2+/1	2+/2+	1/2+	2/1	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	
	See to th	•																											
	See to th	e right			Use	No	No	No	No	No	No	No	Yes	No	No	Yes	No	No	No	No	No								
Warrant met?						No	No	No	No	No	No	No	Yes	No	No	Yes	No	No	No	No	No								
errant 3, Peak Hour					Criteria	Hour						_																	
Lanes (M/m):			2+/2+	<u>1/2+</u>	<u>2/1</u>	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	
	See to th				11	NI-	NI-	NI-	NI-	NI-	NI-	NI-	NI-	NI-	NI-	V	V	V	V	V	V	V	V	NI-	NI-	NI-	NI-	NI-	
	See to th	e rignt			Use	No	No	No	No	No	No	No	No	No	No	Yes	No	No	No	No	No								
Warrant met?	Yes					No	No	No	No	No	No	No	No	No	No	Yes	No	No	No	No	No								



Summary

# **Volume-Based Traffic Signal Warrants Analysis Summary**

		<u> </u>		•	
Warrant		Hour(s) of the Day	Hours Required to Meet Warrant	Hours Met	ls Warrant Met?
	Condition A: Minimum Vehicular Volume	Any Eight Hours	8	4	No
Warrant 1. Eight-Hour	Condition B: Interruption of Continuous Traffic	Any Eight Hours	8	10	Yes
Vehicular Volume	Combination of Condition A & Condition B	Any Eight Hours	8	7	No
	(at least 1 of the 3	Overall conditions re	quired to mee	et warrant)	Yes
	rant 2. ehicular Volume	Any Four Hours	4	10	Yes
	rant 3. k Hour	Any One/Peak Hour	1	8	Yes



**Hayden Airpark Development** 

Signal Warrant Analysis

2024 Background Volumes - 84th Street and Hayden Road

 Major Street: Hayden Road
 Speed Limit:
 45
 Lanes:\*
 2

 Minor Street: 84th Street
 Speed Limit:
 25
 Lanes:\*
 1

 Locale: City of Scottsdale
 \*Number of Approach Lanes of Moving Traffic:

**MUTCD Warrants 1-3** 

Major Street vph - total of both approaches 27 10 25 58 223 557 995 1,320 1,521 1,929 2,045 2,149 2,138 2,071 2,075 2,021 1,716 1,162 680 31 Minor Street volume - higher-volume approach (vph) 9 29 61 78 113 98 107 97 130 170 94 5 Direction of higher-volume minor approach SB SB SB SB NB NB NB NB NB SB Beginning of hour 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 18:00 19:00 20:00 21:00 22:00 23:00

Critical speed of major street traffic above 40 mph
In built-up area of isolated community less than 10,000 population

Urban x

arrant 1, Eight-Hou	r Vehicu	lar Volu	me																										
Condition A	Minimum	Vehicula	ar Volum	ne	Criteria	Hour																							
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	2
Minimum Reqmts	500	600	600	500																									
(100% <sup>a</sup> )	150	150	200	200																									
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+	2/1																								
Minimum Reqmts	350	420	420	350	420	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	١
(70%°)	105	105	140	140	105	No	No	Yes	No	Yes	No	Yes	Yes	No	No	No	No	No	No	١									
Warrant met?	No					No	No	Yes	No	Yes	No	Yes	Yes	No	No	No	No	No	No	N									
Condition B	Interruptio	on of Co	nt. Traffi	ic .	Criteria	Hour																							
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	
Minimum Reqmts	750	900	900	750																									
(100% <sup>a</sup> )	75	75	100	100																									
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+	2/1																								
Minimum Reqmts	525	630	630	525	630	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No							
(70%°)	53	53	70	70	53	No	Yes	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No							
Warrant met?	Yes					No	Yes	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No							
Combination of	of Conditi	ons A &	В		Criteria	Hour																							Т
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	
Condition A	400	480	480	400																									
(80% <sup>b</sup> )	120	120	160	160																									
Condition B	600	720	720	600																									
(80% <sup>b</sup> )	60	60	80	80																									
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+	2/1																								
Condition A	280	336	336	280	336	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	
(56% <sup>d</sup> )	84	84	112	112	84	No	No	Yes	No	No	No	No	No																
Condition B	420	504	504	420	504	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	
(56% <sup>d</sup> )	42	42	56	56	42	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No							
Warrant met?	No					No	No	Yes	No	No	No	No	No																
arrant 2, Four Hour	Vehicul	ar Volun	ne		Criteria	Hour																							
Lanes (M/m):		2+/1	2+/2+	1/2+	2/1	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	
	See to the	-																											
	See to the	e right			Use	No	Yes	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No							
Warrant met?						No	Yes	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No							
errant 3, Peak Hour		0.1:	0 (0		Criteria	Hour	_			_		_											4.0					00	
Lanes (M/m):			2+/2+	<u>1/2+</u>	<u>2/1</u>	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	
	See to the				Haa	No	Vos	Voo	Voc	Voo	Voo	Voo	Voc	Vos	No	No	No	No	No										
	See to the <b>Yes</b>	e rignt			Use	No No	Yes	No No	No No	No No	No No	No No																	
Warrant met?	res					No	Yes	No	No	No	No	No																	



Summary

# **Volume-Based Traffic Signal Warrants Analysis Summary**

Warrant		Hour(s) of the Day	Hours Required to Meet Warrant	Hours Met	Is Warrant Met?
	Condition A: Minimum Vehicular Volume	Any Eight Hours	8	4	No
Warrant 1. Eight-Hour	Condition B: Interruption of Continuous Traffic	Any Eight Hours	8	11	Yes
Vehicular Volume	Combination of Condition A & Condition B	Any Eight Hours	8	7	No
	(at least 1 of the 3	Overall conditions re	quired to mee	et warrant)	Yes
	rant 2. ehicular Volume	Any Four Hours	4	10	Yes
	rant 3. k Hour	Any One/Peak Hour	1	8	Yes



**Hayden Airpark Development** 

Signal Warrant Analysis

2024 Total Volumes - 84th Street and Hayden Road

 Major Street: Hayden Road
 Speed Limit:
 45
 Lanes:\*
 2

 Minor Street: 84th Street
 Speed Limit:
 25
 Lanes:\*
 1

 Locale: City of Scottsdale
 \*Number of Approach Lanes of Moving Traffic:

MUTCD Warrants 1-3

Major Street vph - total of both approaches 27 10 25 226 563 1,006 1,334 1,537 1,950 2,065 2,170 2,160 2,093 2,095 2,040 1,731 1,172 686 32 Minor Street volume - higher-volume approach (vph) 11 20 33 77 61 78 113 98 107 97 130 170 94 6 Direction of higher-volume minor approach SB SB SB SB NB NB NB NB NB SB NB Beginning of hour 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 18:00 19:00 20:00 21:00 22:00 23:00

Critical speed of major street traffic above 40 mph
In built-up area of isolated community less than 10,000 population
Urban x

Warrant 1, Eight-Hou	r Vehicu	lar Volu	ıme																										
Condition A					Criteria	Hour																							
Lanes (M/m):	<u>1/1</u>	2+/1	2+/2+	1/2+		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Minimum Reqmts	500 150	600 150	600 200	500 200																									
(100% <sup>a</sup> )					0/4																								
Lanes ( <b>M</b> / <b>m</b> ): Minimum Regmts	<u>1/1</u> 350	2+/1 420	2+/2+ 420	1/2+ 350	<u>2/1</u> 420	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No
(70%°)	105	105	140	140	105	No	No	No	No	No	No	No	No	No	No	No	Yes	No	Yes	No	Yes	Yes	No	No	No	No	No	No	No
Warrant met?	No	100	140	1-10	.00	No	No	No	No	No	No	No	No	No	No	No	Yes	No	Yes	No	Yes	Yes	No	No	No	No	No	No	No
Condition B		on of Co	nt Troff	·	Criteria	Hour										-110	100		100								-110	-110	
Lanes (M/m):	1/1 <u>1/1</u>	2+/1	2+/2+	1/2+	Criteria	noui 1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Minimum Regmts	750	900	900	750		•	-	Ŭ	_	Ü	Ü	•	Ü	Ü	10	• • •		10		10	10		10	10	20			20	
(100% <sup>a</sup> )	75	75	100	100																									
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+	2/1																								
Minimum Reqmts	525	630	630	525	630	No	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No
(70%°)	53	53	70	70	53	No	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No	No
Warrant met?	Yes					No	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No	No
Combination	of Condit	ions A &	В		Criteria	Hour																							
Lanes (M/m):	<u>1/1</u>	2+/1	2+/2+	1/2+		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Condition A	400	480	480	400																									
(80% <sup>b</sup> )	120	120	160	160																									
Condition B	600	720 60	720 80	600 80																									
(80% <sup>b</sup> )	60				0/4																								
Lanes (M/m): Condition A	<u>1/1</u> 280	2+/1 336	2+/2+ 336	<u>1/2+</u> 280	<u>2/1</u> 336	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No
(56% <sup>d</sup> )	84	84	112	112	84	No	No	No	No	No	No	No	No	No	No	No	Yes	No	No	No	No	No	No						
Condition B	420	504	504	420	504	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No
(56% <sup>d</sup> )	42	42	56	56	42	No	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No	No
Warrant met?	No					No	No	No	No	No	No	No	No	No	No	No	Yes	No	No	No	No	No	No						
Warrant 2, Four Hour	Vehicul	ar Volui	me		Criteria	Hour																							
Lanes (M/m):	1/1	2+/1	2+/2+	1/2+	2/1	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
	See to th	•																											
	See to th	e right			Use	No	No	No	No	No	No	No	Yes	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No	No
Warrant met?	Yes					No	No	No	No	No	No	No	Yes	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No	No
Warrant 3, Peak Hour		21/4	2.72	1/0:	Criteria	Hour	2	2	4	_	6	7	0	0	10	11	10	12	11	15	16	17	10	10	20	24	22	22	24
Lanes (M/m):	<u>1/1</u> See to th	2+/1	2+/2+	<u>1/2+</u>	<u>2/1</u>	1	2	3	4	5	6	1	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
	See to th	-			Use	No	No	No	No	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No	No
Warrant met?	Yes	<b>J</b>				No	No	No	No	No	No	No	No	No	No	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	No	No	No	No	No	No

