

Plan #
Case # 3-DR-2024
Q-S#
X Accepted
Corrections
N.Baronas 9/10/2024
Reviewed By Date

APPROVAL DRAINAGE STIPULATION:

Subject case Civil Improvement plans and Drainage Report shall be revised to reflect Federal Emergency Management Agency (FEMA) Flood Zone X with the final construction document submittal. Current design reflects FEMA Special Flood Hazard Zone AO regulatory development criteria.

PRELIMINARY DRAINAGE REPORT FOR ASM SCOTTSDALE

August 30, 2024 WP# 235526

Prepared by Robert G. Saunders, EIT



EXPIRES 06-30-25

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1.0 INTRODUCTION

1.1 General Background

The ASM Scottsdale (Site) is a proposed industrial and office development. This Site is on a parcel with an approximate area of 24.9 acres in the City of Scottsdale (APN#125-07-209K). The project will include multiple buildings with hardscape, landscape, parking, and utility improvements to support the development. The Site is located directly south of the Loop 101 freeway and approximately 1,300-feet to the east of Scottsdale Road along Mayo Boulevard. The Site is split between Sections 26 and 35, Township 4 North, Range 4 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. Refer to Exhibit 1 – *Vicinity Map* for the project location. The existing property, currently zoned PCD, is mostly undeveloped desert with a portion that is asphalt pavement.

This preliminary Drainage Report has been prepared in accordance with Wood, Patel & Associates, Inc.'s (WOODPATEL's) understanding of the City of Scottsdale technical drainage requirements (Ref. 1) and the *Drainage Design Manuals for Maricopa County Hydrology and Hydraulics (2018)*, as applicable to the Site.

1.2 FEMA Regulated Flood Zones

The Federal Emergency Management Agency (FEMA) publishes Flood Insurance Rate Map (FIRM) information for communities that adhere to FEMA regulations. The FEMA FIRM panel for this Site is 04013C1320L, effective date October 16, 2013, and indicates the Site falls within "Zone AO" shaded (Refer to Exhibit 2 – FEMA FIRM).

"Zone AO" shaded is defined by FEMA as follows:

"Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined".

It is the understanding of WOODPATEL, based on past experience and interpretations of the City of Scottsdale floodplain ordinance that development of land within FEMA Zone "AO" is acceptable as long as, in general, the lowest finish floor elevation is above or properly protected from the anticipated 100-year water surface elevations. This Site will be designed in accordance with the City floodplain ordinance to meet Federal and State regulations.

2.0 HYDROLOGY ANALYSIS

2.1 Offsite Hydrology

The proposed Site does not receive offsite flows. The existing topography slopes from north to south. Drainage improvements have been proposed by Kimley Horn & Associates, Inc. (Kimley Horn) in their Master Drainage Report Crossroads East Planning Unit VI and Portion of Planning Unit III (Ref. 2) to divert overland sheet flow from north of the Site and north of the Loop 101 Freeway to underground storm drain pipes along the north and east side of the Site. These storm drain pipes will intercept and convey offsite stormwater flows around the Site where they will discharge into a detention basin at the

southeast corner of Mayo Boulevard and Miller Road. With these proposed improvements by Kimley Horn, it is anticipated that offsite flows will not affect the Site.

2.2 Onsite Hydrology

Existing surface drainage for the Site flows overland to the south into several proposed temporary surface retention basins proposed by Kimley Horn (Ref. 2). These retention basins will be removed with the proposed grading and drainage.

Runoff from the proposed buildings is designed to direct stormwater away from the proposed buildings overland to proposed retention basins or catch basins connected to a StormPrism 14-foot underground storage system. Refer to Appendix D – *StormPrism 14-foot General Information* for general details. Each StormPrism retention system will be connected to a Contech CDS system to treat stormwater prior to entering drywells, which will evacuate the system within 36-hours. (Refer to Appendix F – *Contech CDS 3020-6-C*) Refer to Appendix C - *Hydrologic and Hydraulic Calculations* for proposed drainage calculations.

Areas A15, A18, and A19 are proposed to sheet flow into proposed retention basins. Retention Basin F will contain the flows from areas A15 and A18. Retention Basin H will contain the flows from area A19. The retention basins will percolate at a rate to drain them within the 36-hours.

Onsite peak flow estimates for the proposed development were generated using the Rational Method, as outlined in the *Drainage Design Manual for Maricopa County, Arizona: Volume I – Hydrology (*Ref 3). NOAA Atlas 14 precipitation data was obtained and utilized to develop Intensity-Duration-Frequency (I-D-F) curves for the Site. Rational Method peak flows were computed at concentration points within the Site at key design locations. Runoff coefficients were estimated to reflect post-development land use conditions for the 10-year and 100-year events. (Refer to Appendix C – *Hydrologic and Hydraulic Calculations*)

2.3 Establishing Lowest Finish Floor (LF88 Elevations)

The Grading and Drainage Plan has been designed to comply with the City of Scottsdale floodplain ordinance for a Zone "AO" floodplain. It is our understanding, unless other floodproof measures are presented and approved, the proposed Lowest habitable Finished Floor (LFF) elevation must be designed a minimum of 1-foot above the anticipated 100-year flood elevation. Scottsdale currently requires the lowest finished floor elevation of 1-foot above the flood depth, which results in a finished floor elevation of 2-feet above the Highest Adjacent natural Grade (HAG) for the proposed building which would be the regulatory flood elevation. Due to the Site being disturbed after the Zone "AO" Special Flood Hazard was established, the current condition of the Site cannot determine the HAG. Due to this change the HAG must be established using topographical information showing the pre-disturbed condition of the Site.

WOODPATEL ASM Scottsdale According to Curry's Corner 7.5-minute Topographic Survey Map by USGS from 1964 with a contour interval of 10-feet, the approximate highest natural grade of this Site prior to development must be changed from the NAVD29 datum to the NAVD88 datum. This change consists of an elevation increase of 1.749-feet determined using surveyed elevations of a nearby monument on both vertical datums.

Using AutoCAD Civil 3D, the quad map was aligned to the Site using common monument lines (section lines) contained within the quad map and previously surveyed by WOODPATEL. The 10-foot interval contours were digitized, adjusted to NAVD88 and applied to a TIN surface model. The surface model was supplemented with break lines at estimated ridge and flowline locations. The surface was used to display interpolated 1-foot contours for the pre-disturbed condition, the proposed building limits were overlaid on the contour map and the HAG was determined for the proposed building. The proposed building lowest finished floor elevation is a minimum of 2-feet above the HAG. (Refer to Appendix A - Regional Contour Map / Highest Natural Grade Elevation Calculation and Appendix B - Curry's Corner Quadrangle Map) Overlaying the building over the adjusted digitized Curry's Corner Map the lowest finish floor elevation can be determined. Refer to the table below for the relive HAG, Lowest Adjacent Grade (LAG), Regulatory Flood Elevation, and LFF.

NAME	HAG	LAG	REGULATORY FLOOD ELEVATION	LOWEST FINISHED FLOOR ELEVATION
OFFICE	1602.07	1592.26	1604.07	1604.50
LAB	1599.53	1592.92	1601.53	1602.50
CUP	1601.58	1598.93	1603.58	1603.58
PARKING STRUCTURE	1599.76	1592.16	1601.76	1599.00
CHEMICAL	1601.54	1598.76	1604.54	1604.75
APS YARD	1602.19	1598.33	1604.19	1604.50

The regulatory flood elevation for each structure was determined by adding 2-feet to each HAG, and an additional foot for the chemical storage building.

3.0 HYDRAULIC ANALYSIS

The proposed Site is designed to convey stormwater to proposed roof drains and catch basins at low points throughout the Site. The catch basins and roof drains will connect to underground storage sized to retain the 100-year, 2-hour storm event. Refer to Appendix D - *StormPrism 14-Foot General Information* for details of the underground storage system. Stormwater within the underground storage will be treated by a Contech CDS3020-6-C system before flowing to the proposed drywells. (Refer to Appendix F – *Contech CDS 3020-6-C*) The underground storage will be drained within 36 -hours by drywells designed at a percolation rate of 0.1 cfs. The Site will require fifteen (15) drywells to fully evacuate the underground system. (Refer to Appendix C- *Hydrologic and Hydraulic Calculations*)

Retention basins will retain the runoff from defined areas and will percolate at a rate of 1.75 inches per hour (in/hr). This value was taken from a recent percolation test in the area. A new percolation test will be done to verify the percolation rate.

4.0 MAINTENANCE

Ongoing maintenance of the designed or recommended drainage systems is required to preserve the design integrity and purpose of the drainage system. Failure to provide maintenance can prevent the drainage system from performing to its intended design purpose and can result in reduced performance. Maintenance within the public right-of-way is the responsibility of the governing municipality. However, it is the responsibility of landowners (such as private developers or property owners' associations) for facilities on private property. Prior to ultimate condition build-out upstream of drainage structures, additional maintenance may be required due to an increase in sedimentation build-up. A regular maintenance program is required to have drainage systems perform to the level of protection or service, as presented in this Report and the project plans and specifications.

5.0 CONCLUSIONS

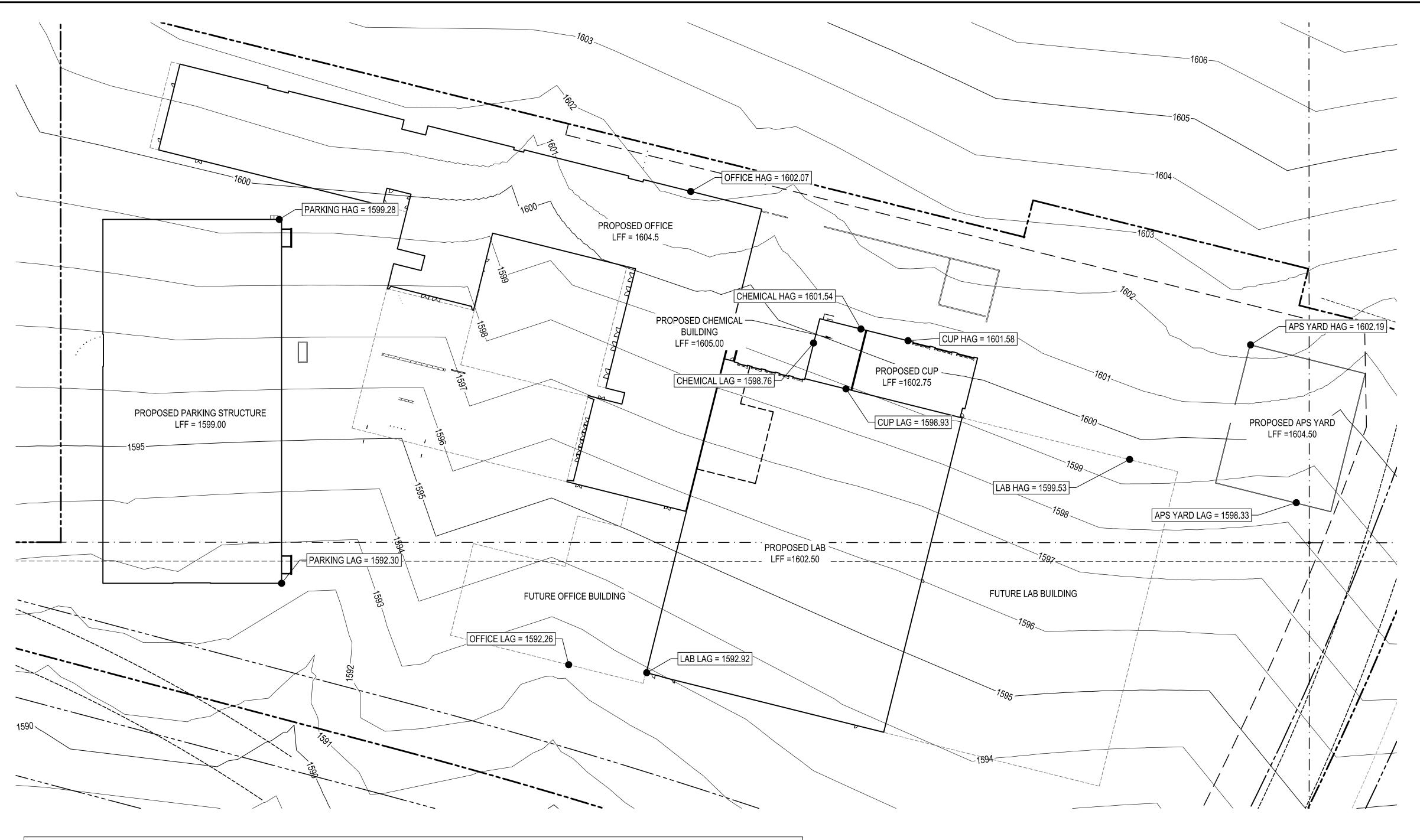
Based on our analysis of the Site, the following conclusions can be made:

- 1. This preliminary Drainage Report has been prepared in accordance with WOODPATEL's understanding of the City of Scottsdale technical drainage requirements and the *Drainage Design Manuals for Maricopa County Hydrology and Hydraulics (2018)*, as applicable to the Site.
- 2. The Site is within a FEMA designated 100-year floodplain (Zone "AO-Depth 1 foot") in both pre- and post-development conditions.
- 3. The Lowest Finish Floor elevation of all the proposed buildings will be a minimum of 2-feet above (1 foot for AO and 1 foot of freeboard) the highest natural grade in accordance with FEMA and City of Scottsdale regulations. An additional foot has been added to the Chemical Building.
- 4. The Site will be protected from offsite flows from the north and east by improvements currently designed for construction by Kimley Horn (Ref. 2) specifically to protect this property.
- 5. Underground storage has been provided for the Site for the 100-year, 2-hour storm event.
- 6. Drywells will be utilized to drain the underground storage within 36 hours after a storm event.
- 7. Retention basins will drain within 36 hours after a storm event.
- 8. Ongoing maintenance is required for the existing drainage systems to maintain design performance. Maintenance is the responsibility of the private parties involved.

6.0 REFERENCES

- 1. Design Standards & Policies Manual, City of Scottsdale, 2018.
- 2. Master Drainage Report Crossroads East Planning Unit VI and Portion of Planning Unit III, Case 37-SA-2023, Scottsdale Arizona, by Kimley-Horn & Associates, Inc., dated August 2023.
- Drainage Design Manual for Maricopa County, Arizona: Volume I Hydrology.
- 4. Curry's Corner Quadrangle, 7.5 Minute Series Topographic Map, USGS, 1964.

APPENDIX A -	- REGIONAL CON CALCULATION	TOUR MAP / HIC	GHEST NATURA	AL GRADE ELEVAT	TON



	FEMA SUMMARY TABLE									
NAME	STRUCTURE CATEGORY	ADDRESS	LOWEST FINISHED FLOOR ELEVATION (LF88)	HIGHEST ADJACENT NATURAL GRADE	LOWEST ADJACENT NATURAL GRADE	REGULATORY FLOOD ELEVATION	FEMA REQUIREMENTS			
	BUILDINGS							WET FLOODPROOFING	OTHER	
FUTURE AND PROPOSED OFFICE	IV		1604.50	1602.07	1592.26	1604.07	NO VENT	NOT REQUIRED	N/A	
FUTURE AND PROPOSED LAB	IV		1602.50	1599.53	1592.92	1601.53	NO VENT	NOT REQUIRED	N/A	
CUP	IV		1603.58	1601.58	1598.93	1603.58	NO VENT	NOT REQUIRED	N/A	
PARKING STRUCTURE	IV		1599.00	1599.76	1592.16	1601.76	NO VENT	NOT REQUIRED	N/A	
CHEMICAL	IV		1604.75	1601.54	1598.76	1604.54	NO VENT	REQUIRED	N/A	
APS YARD	IV		1604.50	1602.19	1598.33	1604.19	NO VENT	NOT REQUIRED	N/A	

- 1) WHEN REQUIRED AS INDICATED ABOVE, FLOOD VENTS SHALL BE PROVIDED ON AT LEAST 2 SEPARATE WALLS. THE FLOOD VENTS SHALL HAVE ONE SQUARE INCH OF OPENING SPACE FOR EVERY SQUARE FOOT OF ENCLOSED SPACE BELOW THE REGULATORY FLOOD ELEVATION, OR AS NOTED ABOVE. SEE ARCHITECTURAL PLANS FOR VENTS OPENINGS. PROPOSED GRADE ADJACENT TO BUILDING MAY EFFECT VENT LOCATIONS, CONSULT ENGINEER PRIOR TO CONSTRUCTION WITH ANY QUESTIONS.
- 2) WHEN REQUIRED AS INDICATED ABOVE, WET FLOODPROOFING SHALL BE PROVIDED UP TO THE REGULATORY FLOOD DEPTH. WET FLOODPROOFING CONSIST OF CONSTRUCTION WITH ANY QUESTIONS.
- 3) WHEN REQUIRED AS NOTED ABOVE, ELECTRICAL AND MECHANICAL EQUIPMENT SHALL BE ELEVATED ABOVE THE REGULATORY FLOOD DEPTH.
- 4) PROPOSED BUILDING M1 WILL BE A STRUCTURALLY INDEPENDENT NON-RESIDENTIAL STRUCTURE.
- 5) FEMA DEFINES DRY FLOODPROOFING AS A COMBINATION OF MEASURES THAT RESULT IN A STRUCTURE, INCLUDING THE ATTENDANT UTILITIES AND EQUIPMENT, BEING WATERTIGHT WITH ALL ELEMENTS SUBSTANTIALLY IMPERMEABLE TO THE ENTRANCE OF FLOODWATER AND WITH STRUCTURAL COMPONENTS HAVING THE CAPACITY TO RESIST FLOOD LOADS.

LEGEND

PROPOSED BUILDING OUTLINE

BOUNDARY LINE

--- SECTION LINE

— 1550 — ESTIMATED 5' CONTOUR NAVD88 DATUM

ESTIMATED 1' CONTOUR NAVD88 DATUM

75— ORIGINAL 1964 CURRYS CORNER CONTOURS ON NAVD88 DATUM

— HAG HIGHEST ADJACENT NATURAL GRADE

LAG LOWEST ADJACENT NATURAL GRADE

RFD REGULATORY FLOOD DEPTH = HAG +2' (ZONE AO DEPTH (1') = 1' FREEBOARD)

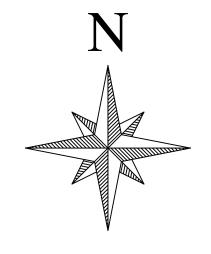
LGF LOWEST GARAGE FLOOR

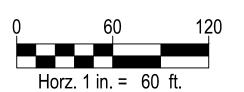
ELEVATION STATEMENT

THE WORK PRODUCT PRESENTED IS THE RESULT OF OBTAINING BEST AVAILABLE HISTORICAL ELEVATION INFORMATION, AND EMPLOYING PROFESSIONAL JUDGMENT TO BEST PRESENT IN SITE GROUND ELEVATIONS. ELEVATIONS ARE BASED ON 1964 CURRYS CORNER NGVD29 DATUM CONVERTED TO NAVD88 USING MARICOPA LAND SURVEY CONVERSION OF 1.749 FT.

BENCHMARK

THE VERTICAL DATUM FOR THIS EXHIBIT IS BASED ON GDACS POINT 43017-1, 3 INCH CITY OF SCOTTSDALE BRASS CAP FLUSH LOCATED ON SCOTTSDALE ROAD SOUTH OF PRINCESS DRIVE HAVING AN ELEVATION OF 1552.985, CITY OF SCOTTSDALE NAVD88 DATUM.





NOT
FOR
CONSTRUCTION
OR RECORDING



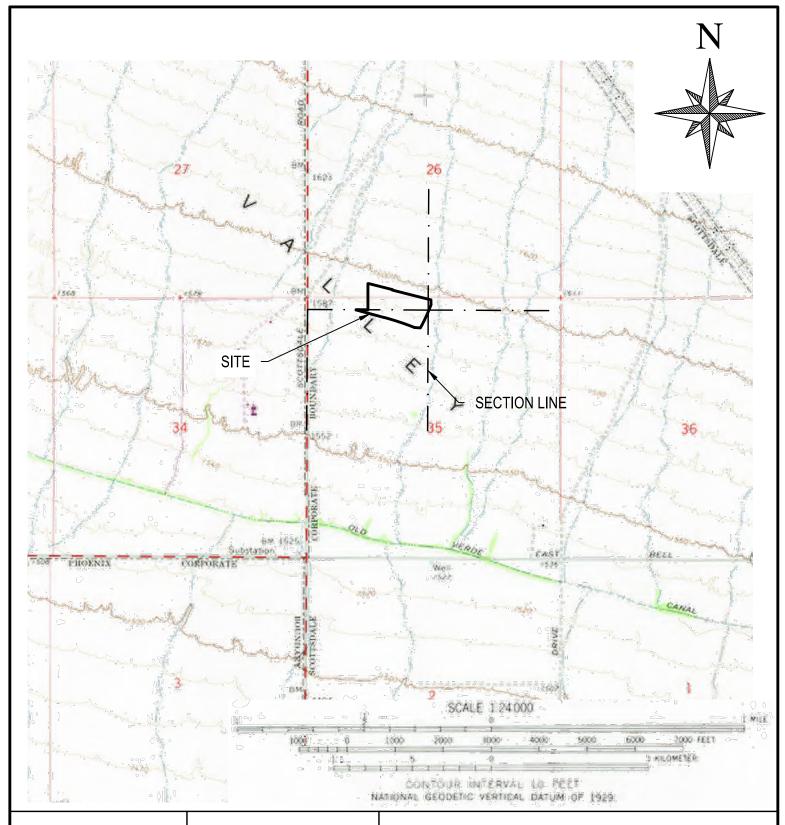
ASM SCOTTSDALE

LOWEST FINISHED FLOOR ELEVATIONS
HIGHEST ADJACENT GRADE

 DATE
 08/30/2024
 SCALE
 1" = 60'
 SHEET
 01 OF 02

 JOB NO
 235526
 DESIGN
 AJS
 DRAWN
 AJS

 $\label{lem:condition} Z:\2023\235526\Project\Support\Reports\Drainage\Exhibits\5526-HAG.dwg$



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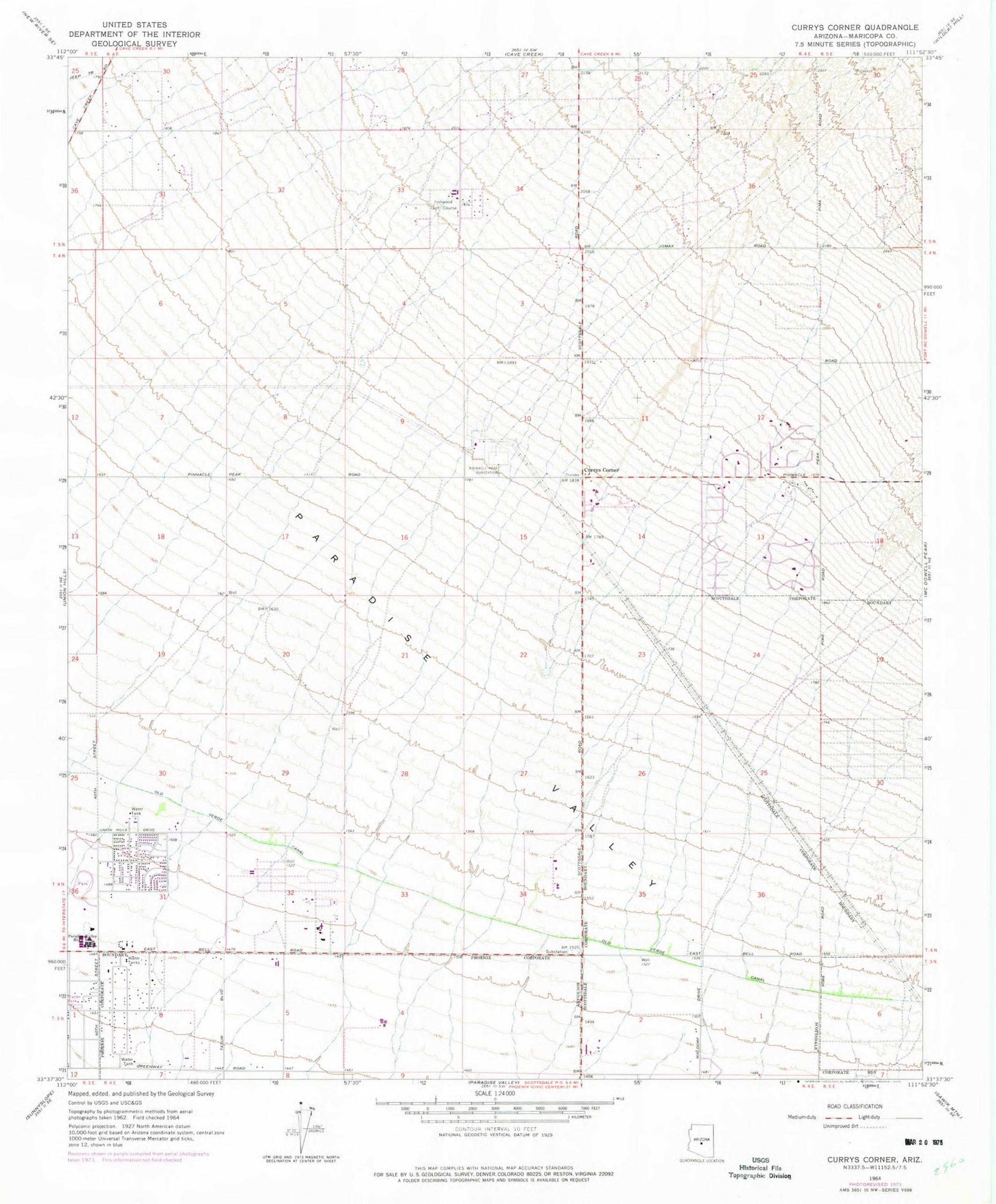


ASM SCOTTSDALE

REGIONAL CONTOUR MAP/OPINION OF EXISTING HIGHEST NATURAL GRADE ELEVATION

DATE	08/30/2024	SCALE	1" = 1'	SHEET	2 OF 2			
JOB NO.	235526	DESIGN	RS	CHECK	RS			
		DRAWN	AJS	RFI#	N/A			
Z.\2023\235526\Project Support\Reports\Drainage\Exhibits\5526 Exhibit - Regional Contour Map 2.dwg								





APPENDIX C – HYDROLOGIC AND HYDRAULIC CALCULATIONS	

IDF DATA FROM FCDMC NOAA – ATLAS 14 PRECIPITATION DATA	





Project ASM Scottsdale
Location Scottsdale AZ
Project Number 235526

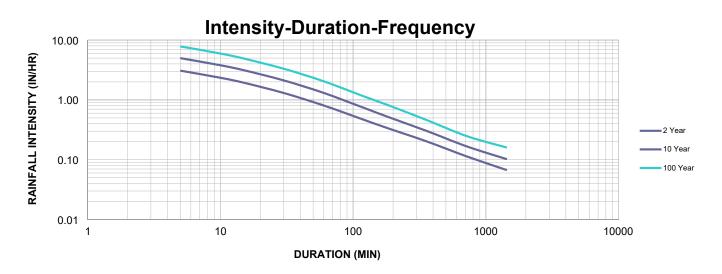
Project Engineer Andrew Sanchez, E.I.T.

RAINFALL DEPTHS, INCHES

Duration	Average Reccurence Interval (years)							
Duration	2	5	10	25	50	100		
5-min	0.257	0.347	0.416	0.509	0.580	0.653		
10-min	0.391	0.528	0.633	0.774	0.883	0.994		
15-min	0.485	0.654	0.785	0.960	1.090	1.230		
30-min	0.653	0.881	1.060	1.290	1.470	1.660		
60-min	0.808	0.109	1.310	1.600	1.820	2.050		
2-hr	0.934	1.240	1.480	1.800	2.050	2.300		
3-hr	1.020	1.330	1.580	1.930	2.200	2.490		
6-hr	1.210	1.540	1.810	2.170	2.460	2.750		
12-hr	1.360	1.720	2.000	2.380	2.670	2.970		
24-hr	1.610	2.080	2.450	2.980	3.390	3.830		

RAINFALL INTENSITY, INCHES/HOUR

Duration	Frequency, years							
minutes	2	5	10	25	50	100		
5	3.08	4.16	4.99	6.11	6.96	7.84		
10	2.35	3.17	3.80	4.64	5.30	5.96		
15	1.94	2.62	3.14	3.84	4.36	4.92		
30	1.31	1.76	2.12	2.58	2.94	3.32		
60	0.81	0.11	1.31	1.60	1.82	2.05		
120	0.47	0.62	0.74	0.90	1.03	1.15		
180	0.34	0.44	0.53	0.64	0.73	0.83		
360	0.20	0.26	0.30	0.36	0.41	0.46		
720	0.11	0.14	0.17	0.20	0.22	0.25		
1440	0.07	0.09	0.10	0.12	0.14	0.16		







PROPOSED COMPOSITE WEIGHTED "C" FACTOR CALCULATIONS

2-YEAR & 10-YEAR

ProjectASM ScottsdaleLocationScottsdale AZ

Project Number 235526

Project Engineer Andrew Sanchez, E.I.T.

Drainage Subbasin ID	Area	Commercial & Industrial Areas		2-YR & 10-YR Runoff Coefficient
(Description/ID)	(Acres)	%	"C"	"C"
A1	0.81	100	0.80	0.80
A2	0.26	100	0.80	0.80
A3	0.41	100	0.80	0.80
A4	0.11	100	0.80	0.80
A5	0.74	100	0.80	0.80
A6	1.47	100	0.80	0.80
A7	0.83	100	0.80	0.80
A8	2.48	100	0.80	0.80
A9	0.66	100	0.80	0.80
A10	0.68	100	0.80	0.80
A11	0.49	100	0.80	0.80
A12	0.89	100	0.80	0.80
A13	1.58	100	0.80	0.80
A14	0.76	100	0.80	0.80
A15	0.01	100	0.80	0.80
A16	0.94	100	0.80	0.80
A17	0.66	100	0.80	0.80
A18	0.50	100	0.80	0.80
A19	0.33	100	0.80	0.80
R1A	0.55	100	0.80	0.80
R1B	0.30	100	0.80	0.80
R1C	0.55	100	0.80	0.80
R1D	0.46	100	0.80	0.80
R2	2.05	100	0.80	0.80
R3	1.67	100	0.80	0.80





PROPOSED COMPOSITE WEIGHTED "C" FACTOR CALCULATIONS

100-YEAR

ProjectASM ScottsdaleLocationScottsdale AZ

Project Number 235526

Project Engineer Andrew Sanchez, E.I.T.

Drainage Subbasin ID	Area	Commercial & Industrial Areas		100 YR Runoff Coefficient
(Description/ID)	(Acres)	%	"C"	"C"
A1	0.81	100	0.86	0.86
A2	0.26	100	0.86	0.86
A3	0.41	100	0.86	0.86
A4	0.11	100	0.86	0.86
A5	0.74	100	0.86	0.86
A6	1.47	100	0.86	0.86
A7	0.83	100	0.86	0.86
A8	2.48	100	0.86	0.86
A9	0.66	100	0.86	0.86
A10	0.68	100	0.86	0.86
A11	0.49	100	0.86	0.86
A12	0.89	100	0.86	0.86
A13	1.58	100	0.86	0.86
A14	0.76	100	0.86	0.86
A15	0.01	100	0.86	0.86
A16	0.94	100	0.86	0.86
A17	0.66	100	0.86	0.86
A18	0.50	100	0.86	0.86
A19	0.33	100	0.86	0.86
R1A	0.55	100	0.86	0.86
R1B	0.30	100	0.86	0.86
R1C	0.55	100	0.86	0.86
R1D	0.46	100	0.86	0.86
R2	2.05	100	0.86	0.86
R3	1.67	100	0.86	0.86





PROPOSED RATIONAL METHOD SUMMARY

10-YEAR & 100-YEAR

Project ASM Scottsdale
Location Scottsdale AZ
Project Number 235526

Project Engineer Andrew Sanchez, E.I.T.

PROPOSED ON-SITE	WATERSHEDS	}								100 YEAR				10 YEAR			
Drainage Subbasin ID	Longest Watercourse 'L'	Longest Watercourse 'L'	Drainage Area 'A'	Drainage Area 'A'	'K _b ' Type ¹	Watershed Resistance Coefficient 'K _{b'}	Top Elevation	Bottom Elevation	Basin Slope 'S'	Calculated Q100 'Tc' (See Note 2)	100 YEAR Intensity 'i'	100 YR Runoff Coefficient 'C'	Q100 Flow	Calculated Q10 'Tc' (See Note 2)	10 YEAR Intensity 'i'	10 YR Runoff Coefficient 'C'	Q10 Flow
	(ft)	(mi)	(sf)	(Acres)					(ft/mi)	(min)	(in/hr)		(cfs)	(min)	(in/hr)		(cfs)
	401	0.076	35,239	0.81	Α	0.0406	1604.4			5.0			5.5		4.92		3.2
A2	161	0.030	11,298	0.26	Α	0.0437	1601.5			5.0			1.7		4.99		1.0
A3	174	0.033	17,877	0.41	A	0.0424	1604.5	1599.3		5.0			2.8		4.99	0.80	1.6
A4	117	0.022	4,722	0.11	Α	0.0460	1604.5	1598.2		5.0	7.84		0.7		4.99		0.4
A5	164	0.031	32,445	0.74	Α	0.0408	1602.5	1599.0	114.5	5.0	7.84	0.86	5.0	5.0	4.99	0.80	3.0
A6	313	0.059	64,156	1.47	Α	0.0389	1603.6	1600.0	61.5	5.0	7.84	0.86	9.9	5.0	4.99	0.80	5.9
A7	242	0.046	35,978	0.83	Α	0.0405	1605.0	1596.7	180.8	5.0	7.84	0.86	5.6	5.0	4.99	0.80	3.3
A8	424	0.080	108,071	2.48	Α	0.0375	1603.6	1595.0	107.1	5.0	7.84	0.86	16.7	5.0	4.99	0.80	9.9
A9	343	0.065	28,582	0.66	Α	0.0411	1602.0	1591.0	169.1	5.0	7.84	0.86	4.4	5.0	4.99	0.80	2.6
A10	198	0.038	29,447	0.68	Α	0.0411	1601.5	1590.0	305.2	5.0	7.84	0.86	4.6	5.0	4.99	0.80	2.7
A11	185	0.035	21,163	0.49	Α	0.0420	1604.5	1590.5	399.4	5.0	7.84	0.86	3.3	5.0	4.99	0.80	1.9
A12	259	0.049	38,863	0.89	Α	0.0403	1604.5	1594.2	210.3	5.0	7.84	0.86	6.0	5.0	4.99	0.80	3.6
A13	349	0.066	68,696	1.58	Α	0.0388	1604.5	1591.1	203.2	5.0	7.84	0.86	10.6	5.0	4.99	0.80	6.3
A14	372	0.070	33,150	0.76	Α	0.0407	1602.3	1595.0	103.7	5.0	7.84	0.86	5.1	5.0	4.99	0.80	3.0
A15	21	0.004	628	0.01	Α	0.0515	1604.5	1604.2	88.0	5.0	7.84	0.86	0.1	5.0	4.99	0.80	0.1
A16	294	0.056	41,157	0.94	Α	0.0402	1603.3	1593.2	180.6	5.0	7.84	0.86	6.4	5.0	4.99	0.80	3.8
A17	129	0.024	28,577	0.66	Α	0.0411	1604.5	1601.5	122.5	5.0	7.84	0.86	4.4	5.0	4.99	0.80	2.6
A18	159	0.030	21,865	0.50	Α	0.0419	1604.3	1603.0	43.2	5.0	7.84	0.86	3.4	5.0	4.99	0.80	2.0
A19	27	0.005	14,305	0.33	Α	0.0430	1604.5	1604.3	41.0	5.0	7.84	0.86	2.2	5.0	4.99	0.80	1.3
R1A		0.000	24,106	0.55	Α	0.0416	0.0	0.0	0.5	5.0	7.84	0.86	3.7	5.0	4.99	0.80	2.2
R1B		0.000	13,083	0.30	Α	0.0433	0.0	0.0	0.5	5.0	7.84	0.86	2.0	5.0	4.99	0.80	1.2
R1C		0.000	23,791	0.55	Α	0.0416	0.0			5.0	7.84	0.86	3.7	5.0	4.99		2.2
R1D		0.000	20,105	0.46	Α	0.0421	0.0			5.0	7.84	0.86	3.1		4.99		1.8
R2		0.000	89,147	2.05	Α		0.0			5.0			13.8		4.99		8.2
R3		0.000	72,745	1.67	Α		0.0			5.0			11.3		4.99		6.7

Total 879,196 20.18 136.1 80.5

Notes

- 1. Per Drainage Design Manual for Maricopa County, Vol. I, Hydrology (2013), Table 3.1: Equation for Estimating Kb in the Tc Equation
- 2. Minimum Tc is 5 minutes.





RETENTION REQUIRED 100-YEAR, 2-HOUR VOLUME

Project ASM Scottsdale Location Scottsdale AZ Project Number 235526

Project Engineer Andrew Sanchez, E.I.T.

Rainfall Depth "P" = 2.30 inches

Retention Basin ID	Drainage Subbasin ID	Drainage Area "A"	100 YR Runoff Coefficient "C"	A*C	Retention Required	Retention Required
		(Acres)			(CF)	(AF)
A	A1	0.81	0.86	30,306	5,809	0.13
A	A2	0.26	0.86	9,716	1,862	0.04
В	A3	0.41	0.86	15,375	2,947	0.07
В	A4	0.11	0.86	4,061	778	0.02
В	A5	0.74	0.86	27,903	5,348	0.12
В	A6	1.47	0.86	55,174	10,575	0.24
С	A7	0.83	0.86	30,941	5,930	0.14
С	A8	2.48	0.86	92,941	17,814	0.41
С	A9	0.66	0.86	24,580	4,711	0.11
С	A10	0.68	0.86	25,325	4,854	0.11
D	A11	0.49	0.86	18,200	3,488	0.08
D	A12	0.89	0.86	33,422	6,406	0.15
D	A13	1.58	0.86	59,079	11,323	0.26
E	A14	0.76	0.86	28,509	5,464	0.13
F	A15	0.01	0.86	540	104	0.00
G	A16	0.94	0.86	35,395	6,784	0.16
G	A17	0.66	0.86	24,576	4,710	0.11
F	A18	0.50	0.86	18,804	3,604	0.08
H	A19	0.33	0.86	12,302	2,358	0.05
A	R1A	0.55	0.86	20,731	3,974	0.09
A	R1B	0.30	0.86	11,251	2,156	0.05
В	R1C	0.55	0.86	20,460	3,922	0.09
G	R1D	0.46	0.86	17,290	3,314	0.08
D	R2	2.05	0.86	76,666	14,694	0.34
E	R3	1.67	0.86	62,561	11,991	0.28

TOTAL 20.18 144,921 3.33

Calculated Values





RETENTION PROVIDED 100-YEAR, 2-HOUR VOLUME

Project ASM Scottsdale Location Scottsdale AZ Project Number 235526

Project Engineer Andrew Sanchez, E.I.T.

Retention Basin ID	Drainage Subbasin ID	Bottom Basin Area	Median Basin Area	Top Basin Area	Basin Depth	Underground Retention Stuctures	Volume for Each Structure	Total Volume Provided	Volume Required 100-yr, 2-hr Retention	Volume Required 100-yr, 2-hr Retention
		(sq. ft)	(sq. ft)	(sq. ft)	(ft)	5151	(CF)	(CF)	(CF)	(AF)
Α	A1, A2, R1A, R1B					8	1,736	13,889	13,801	0.32
В	A3, A4, A5, A6, R1C					14	1,736	24,305	23,570	0.54
С	A7, A8, A9, A10					20	1,736	34,722	33,309	0.76
D	A11, A12, A13, R2					21	1,736	36,458	35,912	0.82
E	A14, R3					11	1,736	19,097	17,455	0.40
F	A15, A18	4,517	5,321	6,180	1.5			8,046	3,708	0.09
G	A16, A17, R1D					9	1,736	15,625	14,809	0.34
Н	A19	3,127	3,661	4,240	1.5			5,590	2,358	0.05

Total 83 157,733 144,921 3.33





DRYWELL BASIN DRAIN TIME

ProjectASM ScottsdaleLocationScottsdale AZ

Project Number 235526

Project Engineer Andrew Sanchez, E.I.T.

Retention Basin ID	Volume Required	Volume Required	Drywell Percolation Rate	Number of Drywells	Drair	ı Time
	(AF)	(CF)	(CFS)		(Minutes)	(Hours)
Α	0.32	13,801	0.10	2	1,150	19.17
В	0.54	23,570	0.10	2	1,964	32.74
С	0.76	33,309	0.10	4	1,388	23.13
D	0.82	35,912	0.10	3	1,995	33.25
E	0.40	17,455	0.10	2	1,455	24.24
G	0.34	14,809	0.10	2	1,234	20.57

Total 15





SURFACE BASIN DRAIN TIME

ProjectASM ScottsdaleLocationScottsdale AZ

Project Number 235526

Project Engineer Andrew Sanchez, E.I.T.

Retention Basin ID	Volume Volume Required (V) Required		Surface Percolation Rate* (Pd)	Bottom of Basin (Ap)	Drain Time** (Td)		
	(AF)	(cf)	(in/hr)	(ac)	(Minutes)	(Hours)	
F	0.09	3,708	1.75	0.104	338	5.63	
Н	0.05	2,358	1.75	0.072	313	5.21	

Notes:

*Percolation rate used from a recent test performed in the local area. A new test will be performed onsite to verify these findings, however, based on the results shown the retention basins will drain within 36-hours.

$$T_d = \frac{V}{A_p \frac{P_d}{12}}$$

^{**}Basin Drain Time equation from Maricopa County Drainage Policies and Standards equation 6.7





INLET CAPACITY AT SUMP LOCATIONS MAG 535 TYPE 'F' SINGLE CATCH BASIN

Project ASM Scottsdale Location Scottsdale AZ Project Number 235526

Project Engineer Andrew Sanchez, E.I.T.

Weir Equation $Q_i = C_w P d^{1.5} (C_f)$ Orifice Equation $Q_i = C_o A (2gd)^{0.5} (C_f)$ Where Cw = 3, Co = 0.67, and Cf = Clogging Factor = 0.5

> P = 9.42 ft A = 5.52 sf

P = Perimeter of catch basin minus area of longitudinal & lateral bars
A = Total area of grate minus area of longitudinal & lateral bars

	Weir	Orifice		
Depth (ft)	Qi (cfs)	Qi (cfs)		
0.00	0.00	0.00		
0.05	0.16	3.32	Cinalo Croto Inlot	
0.10	0.45	4.69	Single Grate Inlet	
0.15	0.82	5.75	00.00	
0.20	1.26	6.64	60.00	
0.25	1.77	7.42	, and the second se	
0.30	2.32	8.13		
0.35	2.93	8.78	50.00	
0.40	3.57	9.39	50.00	
0.45	4.27	9.95	_	
0.50	5.00	10.49]	
0.55	5.76	11.01	40.00	
0.60	6.57	11.49	J	
0.65	7.40	11.96		
0.67	7.75	12.15	(\$\frac{\text{80}}{\text{S}}\)	
0.70	8.28	12.42	o 30.00 ←	
0.75	9.18	12.85	8	
0.80	10.11	13.27	ti	
0.85	11.07	13.68		
0.90	12.06	14.08	20.00	
0.95	13.08	14.46		
1.00	14.13	14.84		
1.05	15.20	15.21		
1.10	16.30	15.56	10.00	
1.15	17.43	15.91	Secretary Page 1	
1.20	18.57	16.26	Service Commence	
1.25	19.75	16.59		
1.30	20.94	16.92	0.00	
1.35	22.16	17.24	000,000,000,000,000,000,000,000,000,000,000,000,000,000,000	
1.40	23.41	17.56	0, 0, 0, 0, 0, 0, 0, v,	
1.45	24.67	17.87	—■— Weir Flow ——— Orifice Flow	
1.50	25.96	18.17		





INLET CAPACITY AT SUMP LOCATIONSMAG 535 TYPE 'F' DOUBLE CATCH BASIN

Project ASM Scottsdale Location Scottsdale AZ Project Number 235526

Project Engineer Andrew Sanchez, E.I.T.

Weir Equation $Q_i = C_w P d^{1.5} (C_f)$ Orifice Equation $Q_i = C_o A (2gd)^{0.5} (C_f)$ Where Cw = 3, Co = 0.67, and Cf = Clogging Factor = 0.5

> P = 18.84 ft A = 11.04 sf

P = Perimeter of catch basin minus area of longitudinal & lateral bars
A = Total area of grate minus area of longitudinal & lateral bars

Donath (ft)	Weir	Orifice			
Depth (ft) 0.00	Qi (cfs) 0.00	Qi (cfs) 0.00			
0.05	0.32	6.64			
0.10	0.89	9.39			Single Grate Inlet
0.15	1.64	11.49			
0.20	2.53	13.27		120.00	
0.25	3.53	14.84			<u>_</u>
0.30	4.64	16.26			<u>,</u>
0.35	5.85	17.56			_
0.40	7.15	18.77		100.00	
					_
0.45	8.53 9.99	19.91 20.99	┑		,
			_		_ *
0.55	11.53	22.01		80.00	
0.60 0.65	13.13 14.81	22.99 23.93			y ^r
			- ·		_
0.67	15.50	24.29	35		
0.70	16.55	24.83	Ø	60.00	
0.75	18.36	25.70	Flow Q (cfs)		<i>y</i> *
0.80	20.22	26.55	□		, , , , , , , , , , , , , , , , , , ,
0.85	22.15	27.36			
0.90	24.13	28.16		40.00	
0.95	26.17	28.93			
1.00	28.26	29.68			
1.05	30.41	30.41			and the second s
1.10	32.60	31.13		20.00	- Advantage
1.15	34.85	31.83			and the second
1.20	37.15	32.51			Service and the service of the servi
1.25	39.49	33.18			
1.30	41.89	33.84		0.00	*** *********************************
1.35	44.33	34.48		2	00,000,000,000,000,000,000,000,000,000,000,000,000,000,000,000
1.40	46.81	35.12		00	50,6030,6060,661616161616
1.45	49.34	35.74			—■— Weir Flow —— Orifice Flow
1.50	51.92	36.35			





CATCH BASIN INTERCEPTION CAPACITY AND BY-PASS FLOWS

ProjectASM ScottsdaleLocationScottsdale AZ

Project Number 235526

Project Engineer Andrew Sanchez, E.I.T.

Catch Basin ID	Catch Basin Type	On-Grade or Sump	Post Q10 Flow (cfs)	Post Q100 Flow (cfs)	Intercept Capacity (cfs)
CB-A1	MAG 535	Sump	3.2	5.5	10.5
CB-A2	MAG 535	Sump	1.0	1.7	10.5
CB-A3	MAG 535	Sump	1.6	2.8	10.5
CB-A4	Trench Drain	Sump	0.4	0.7	10.5
CB-A5	MAG 535	Sump	3.0	5.0	10.5
CB-A6	MAG 535	Sump	5.9	9.9	10.5
CB-A7	MAG 535	Sump	3.3	5.6	10.5
CB-A8	(2x)MAG 535	Sump	9.9	16.7	21.0
CB-A9	MAG 535	Sump	2.6	4.4	10.5
CB-A10	MAG 535	Sump	2.7	4.6	10.5
CB-A11	MAG 535	Sump	1.9	3.3	10.5
CB-A12	MAG 535	Sump	3.6	6.0	10.5
CB-A13	MAG 535	Sump	6.3	10.6	10.5
CB-A14	MAG 535	Sump	3.0	5.1	10.5
CB-A16	MAG 535	Sump	3.8	6.4	10.5
CB-A17	MAG 535	Sump	2.6	4.4	10.5



DESIGN NOTES:

MATERIALS

Materials used in this work shall conform to ACI (American Concrete Institute)

CONCRETE

For SPEO units and walls:

28 Day Compressive 6,000 psi (Minimum) Lifting Strength 2000 psi (Minimum)

REINFORCING STEEL

Reinforcing Bars shall conform to: ASTM A615, Grade 60 Reinforcing Mesh shall conform to: ASTM A1064, Grade 70

Reinforcing Cover: 1.5" minimum

- 1. Design per ACI 350, ASTM C857 and C857 HS-20 live load.
- 2. Bar cover = $1 \frac{1}{2}$ " u.n.o.

DESIGN PARAMETERS

- 3. Earth cover = 2'-10' max
- 4. Equivalent fluid pressure = 60 pcf
- 5. Unit weight of soil = 120 pcf
- 6. fr @ 28 days = 6.000 psi
- 7. Reinforcement = bar per ASTM A615, Grade 60
- 8. Provide replacement-reinforcing equal to half amount of interrupted bars at each side of opening.
- 9. Equivalent mesh can be used in lieu of grade 60 rebar.
- 10. Assumed water table below structure. No surcharge from adjacent buildings or structures. min soil bearing pressure 2000 psf

FABRICATION

Fabrication shall meet the requirements of the following:

Fabrication Tolerances

- 1. Width: +1/4", -1/4"
- 2. Length: +1/4", -1/4"
- 3. Height: +1/4", -1/4"
- 4. Deviation from square: 1/4"
- 5. Member Thickness: +1/4", -1/4"

SHIPPING AND HANDLING

The Precast Supplier and Contractor shall verify that the method of lifting does not overstress the precast concrete pieces in any way.

INSTALLATION SPECIFICATIONS

Storm Prism shall be installed in accordance with ASTM C-891-11 Standard Practice for Installation of Underground Precast Concrete Utility Structures.

The bearing pressure of the underlying soil must be verified by the geotechnical engineer prior to placing aggregate sub base.

The leveling bed material shall comply with APWA 216-2.4.

The leveling bed shall be graded to a tolerance of 1/4" of the plan elevation.

The leveling bed shall extend beyond the Storm Prism perimeter by at least 24".

The Storm Prism modules shall be placed on line and grade per the site plans. Space between modules shall be 1/2" plus / minus 1/2"

Storm Prism modules are designed to be soil tight. All exterior joints between adjacent modules shall be sealed using a preformed, cold applied, self-adhering, polymer wrap conforming to: ASTM E-1745, C-877, C-990 Specifications, and AASHTO M198 Type B.

Wrap shall be applied per manufacture's recommendations.

BACKFILL

Fill placed around the Storm Prism Modules shall be placed evenly around the entire system in 8" lifts, per APWA 300-3.5.2.

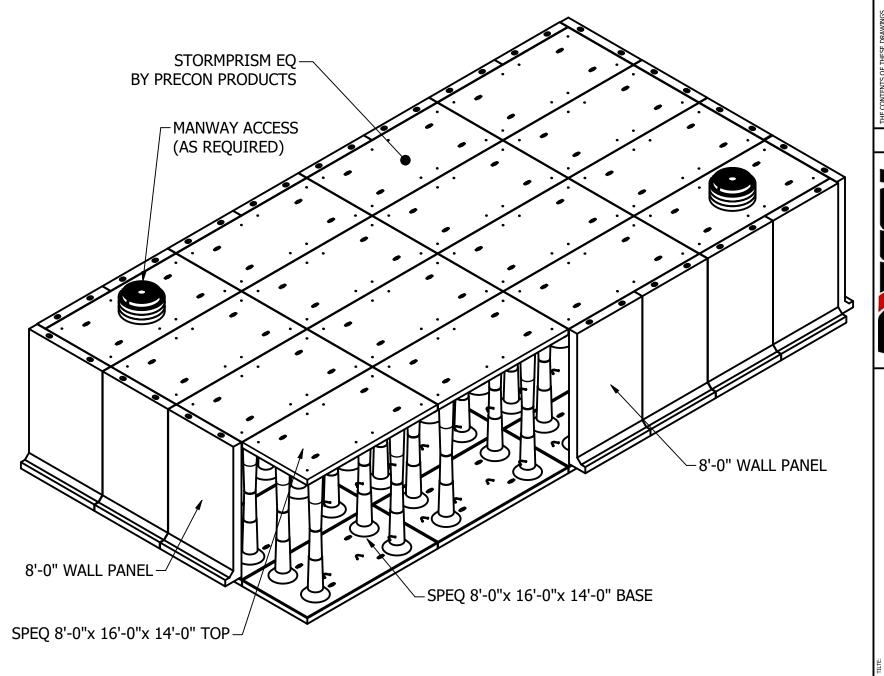
Backfill shall be mechanically compacted (no jetting).

Special care should be taken when compacting adjacent to or near the tape seal not to disrupt the seal. Fill placement over top of Storm Prism shall be done in accordance with ASTM C1675. Fill material not to exceed 120 PCF. At no time shall machinery or vehicles exceeding HS-20 travel over the Storm Prism.



DETENTION

	SHEET INFORMATION		П	
SHEET No.	DESCRIPTION			
1	COVER SHEET / GENERAL NOTES		ı	
2	STORMPRISM 14'-0" DETENTION UNIT DETAIL		ı	
3	STORMPRISM 14'-0" DETENTION BASE		ı	
4	STORMPRISM 14'-0" DETENTION TOP		ı	
5	STORMPRISM 15'-4" WALL DETAIL		2	
6	UNIT MANWAY OPENING AND LADDER INFORMATION	OTTOTO	KIPTION	
7	DETENTION JOINT SEALING INFORMATION	IS:	H H	
8	ELEVATION INFORMATION	VISIO	.UAI	
		R	ž	_



DO NOT SCALE



STORMPRISM 14'-0" DETENTION SYSTEM GENERAL INFORMATION

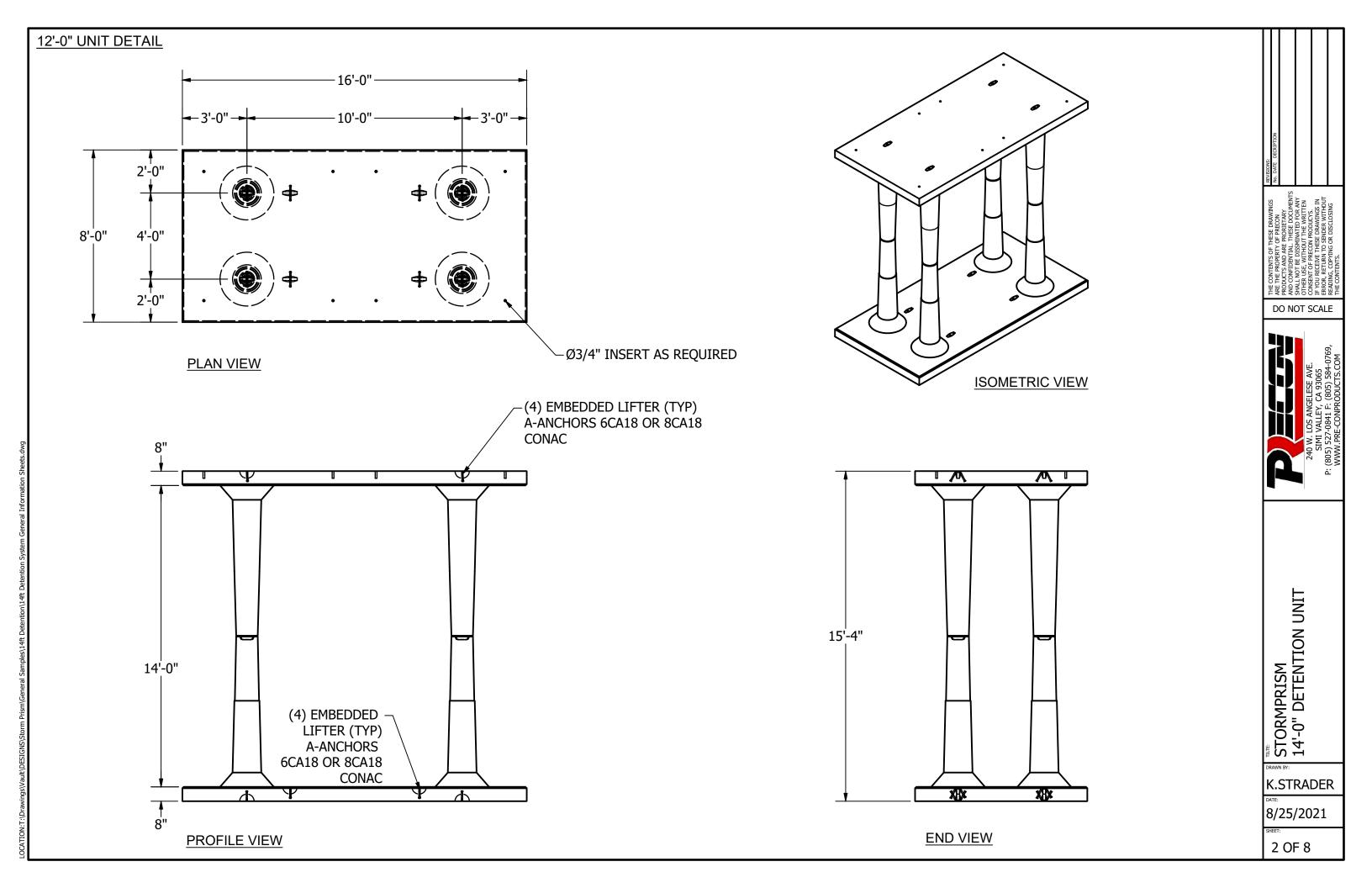
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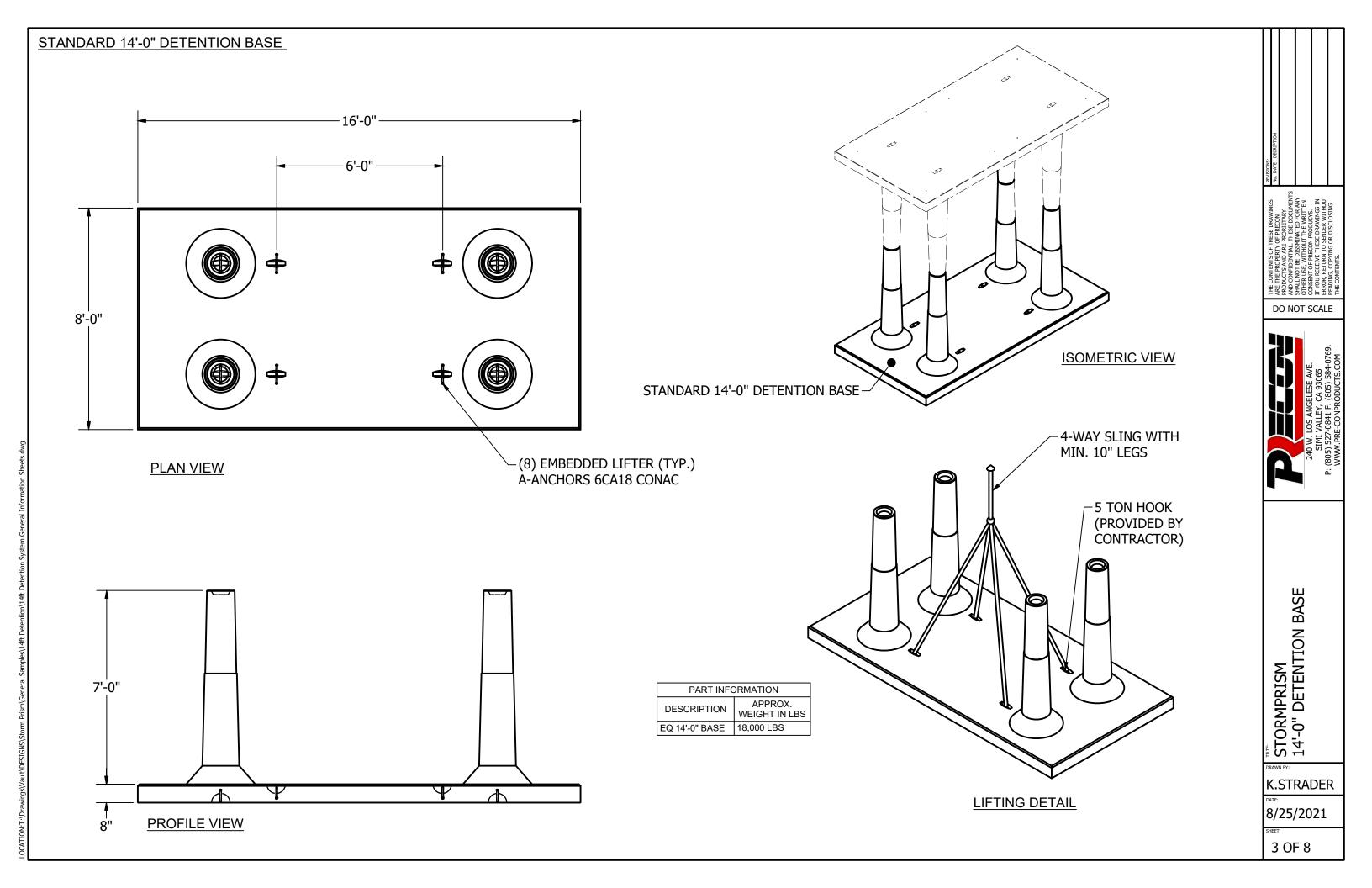
NOTE:

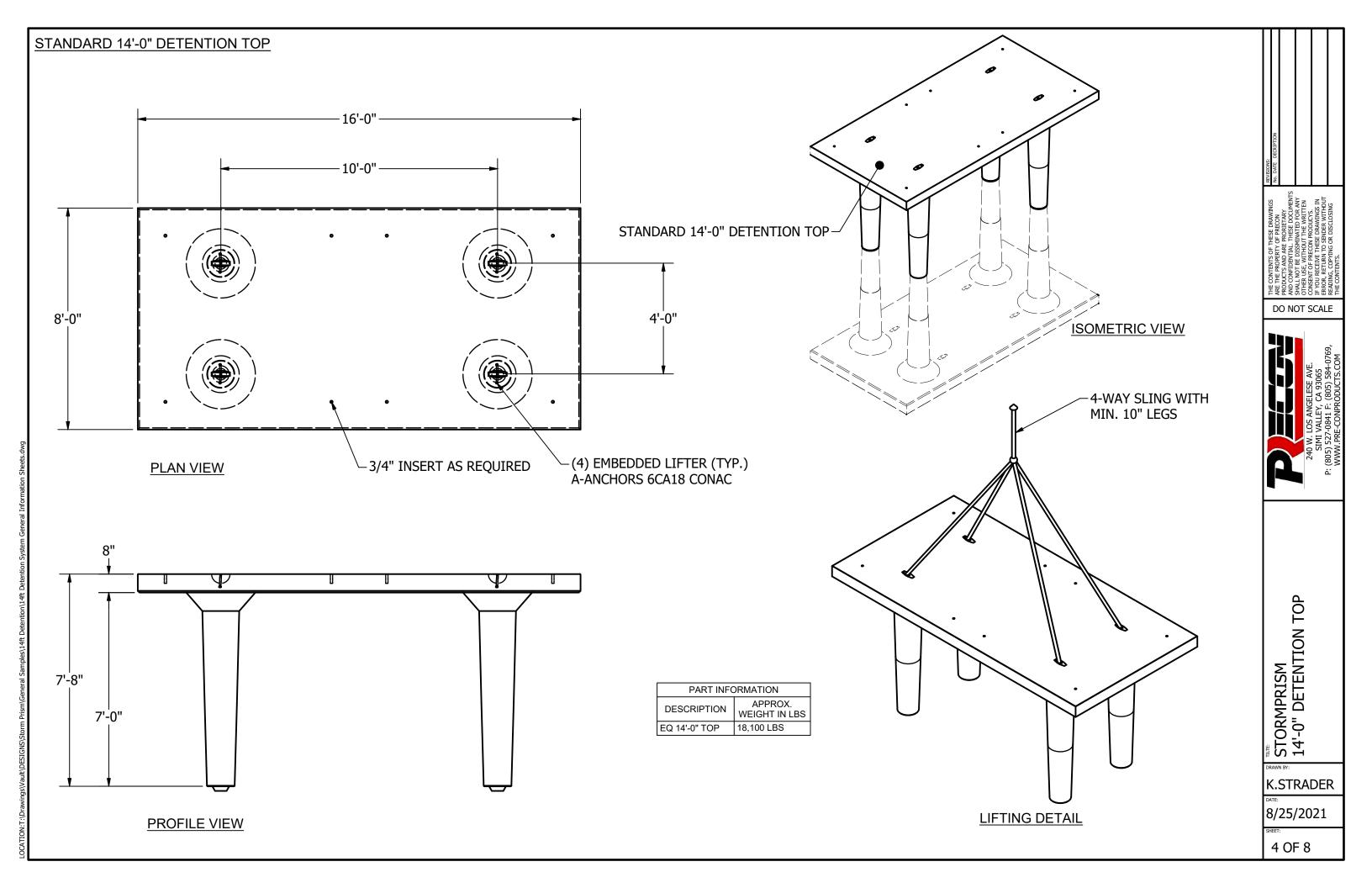
SITE SPECIFIC DRAWINGS.

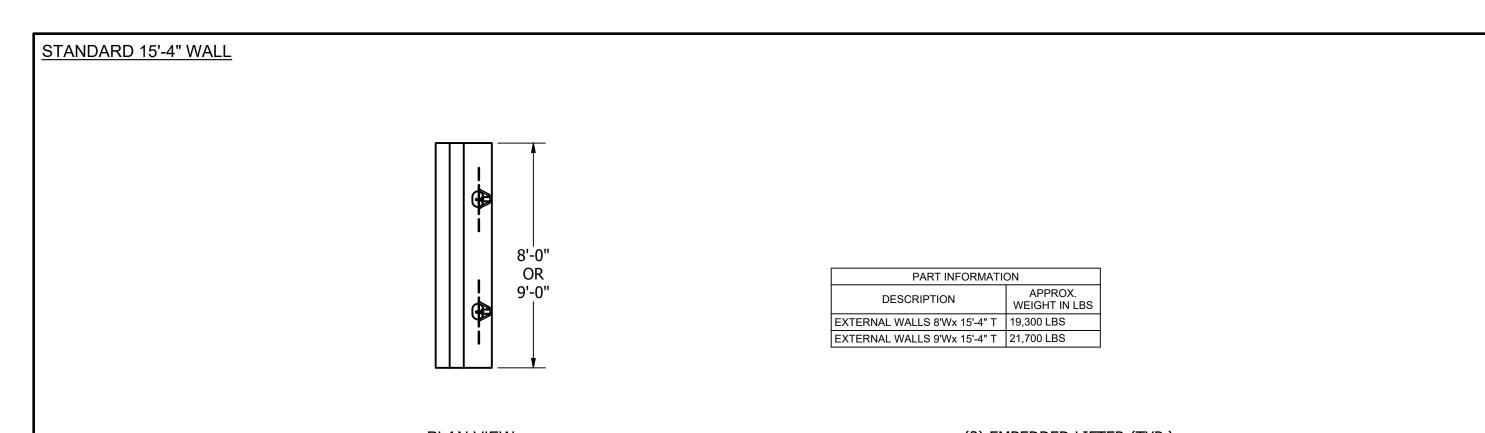
8/25/2021

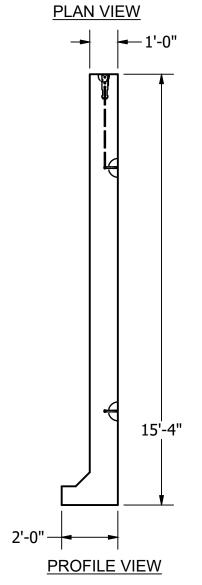
1 OF 8

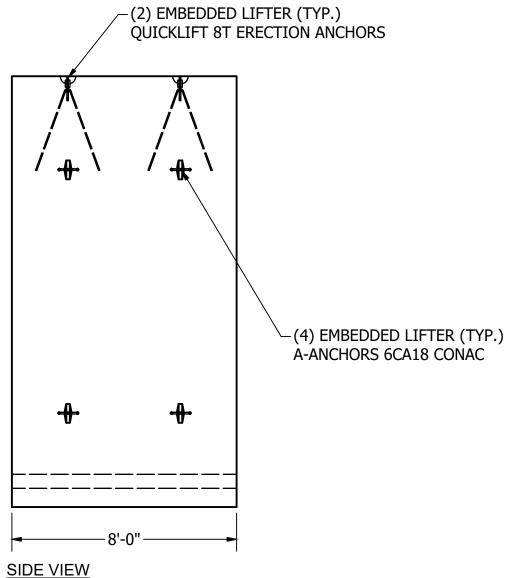












240 W. LOS ANGELESE AVE. SIMI VALLEY, CA 93065 P: (805) 527-0841 F: (805) 584-0769,

DO NOT SCALE

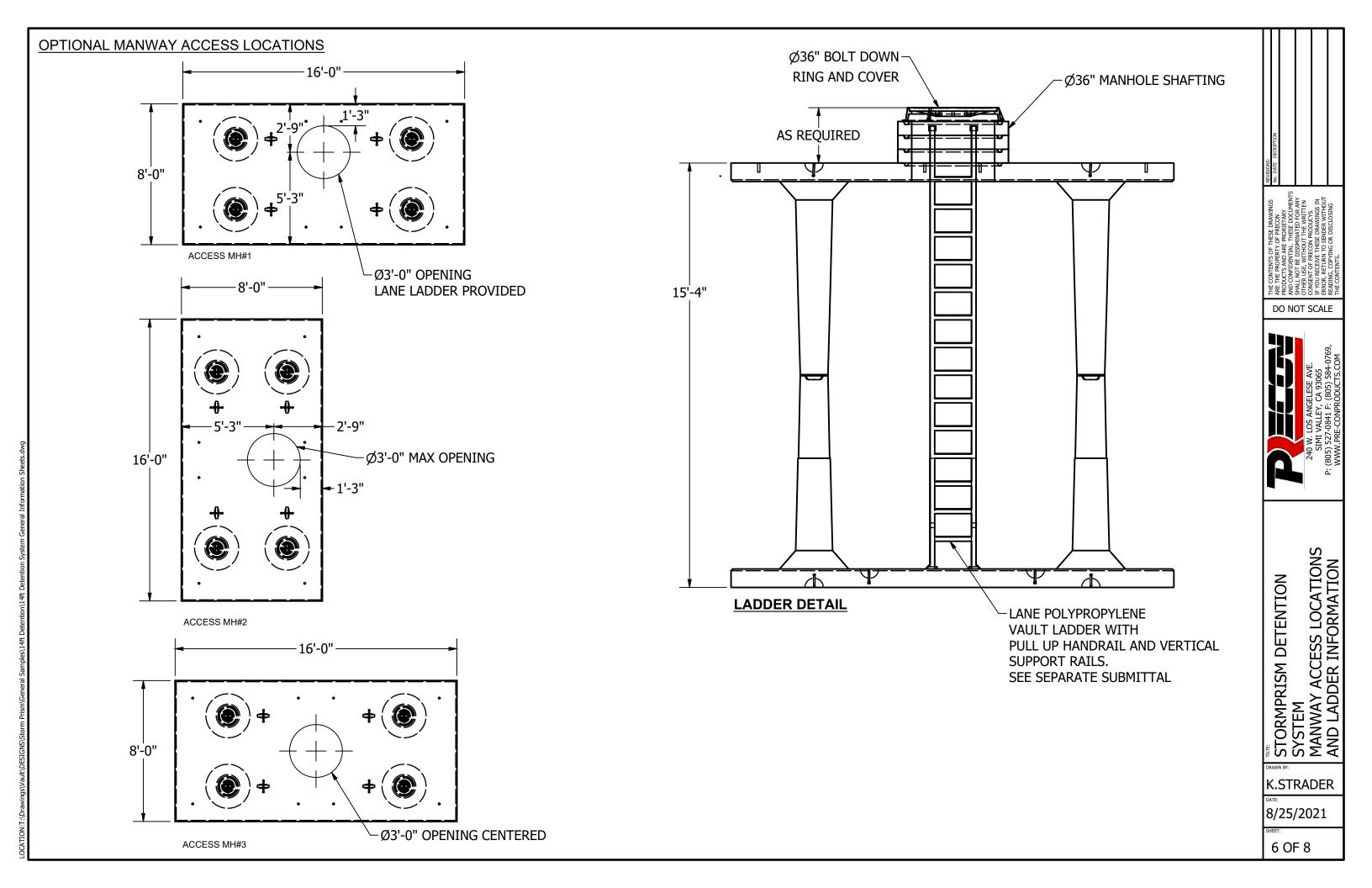
STORMPRISM DETENTION SYSTEM STANDARD 15'-4" WALL DETAIL

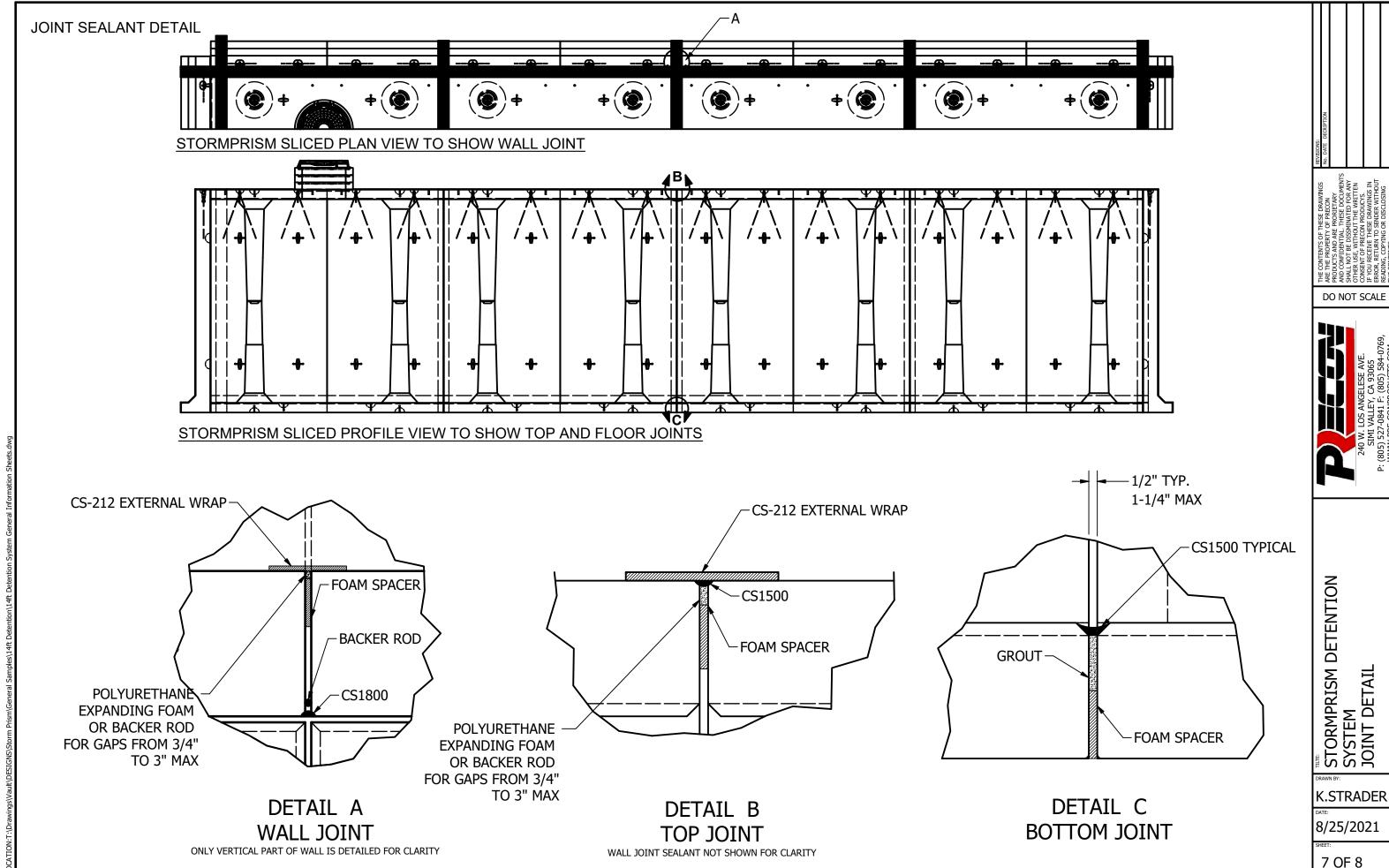
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K.STRADER

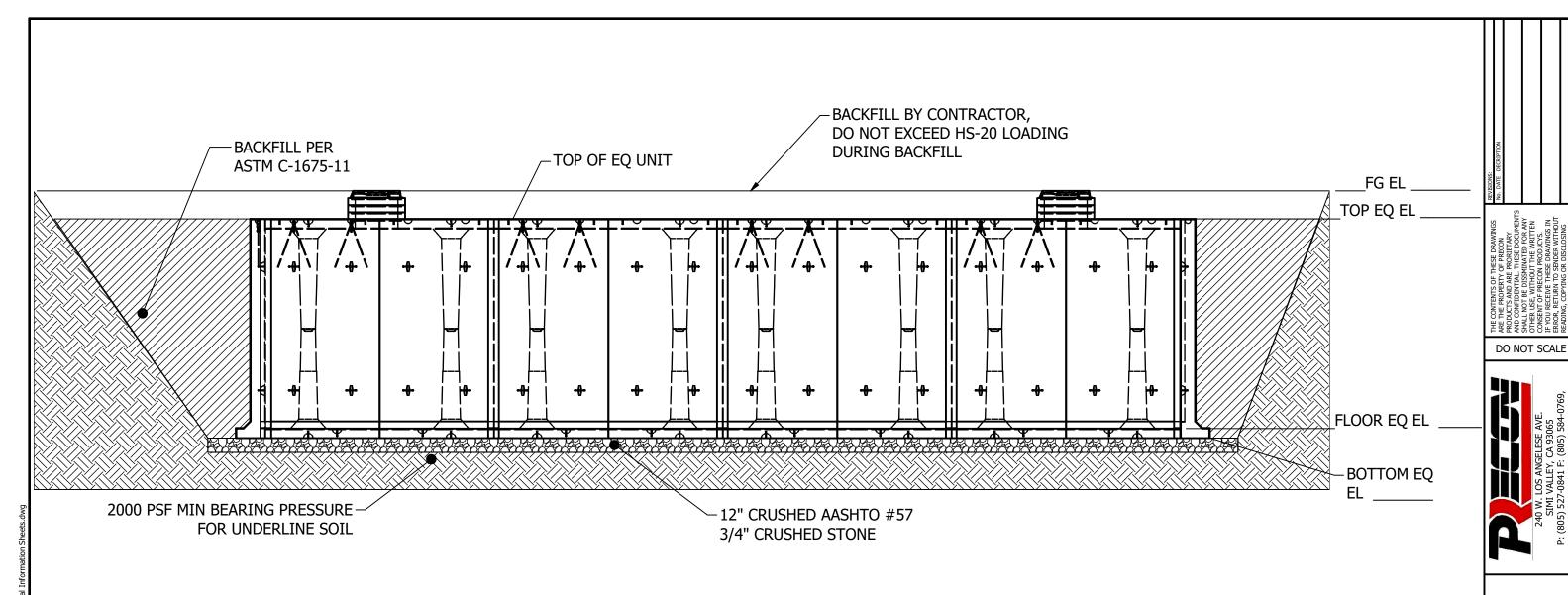
8/25/2021

5 OF 8





8/25/2021



INSTALLATION SPECIFICATIONS

Storm Prism shall be installed in accordance with ASTM C-891-11 Standard Practice for Installation of Underground Precast Concrete Utility Structures.

The bearing pressure of the underlying soil must be verified by the geotechnical engineer prior to placing aggregate sub base.

The leveling bed material shall comply with APWA 216-2.4.

The leveling bed shall be graded to a tolerance of 1/4" of the plan elevation.

The leveling bed shall extend beyond the Storm Prism perimeter by at least 24".

The Storm Prism modules shall be placed on line and grade per the site plans. Space between modules shall be 1/2" plus / minus 1/2"

Storm Prism modules are designed to be soil tight. All exterior joints between adjacent modules shall be sealed using a preformed, cold applied, self-adhering, polymer wrap conforming to: ASTM E-1745, C-877,

C-990 Specifications, and AASHTO M198 Type B.

Wrap shall be applied per manufacture's recommendations.

BACKFILL

Fill placed around the Storm Prism Modules shall be placed evenly around the entire system in 8" lifts,

per APWA 300-3.5.2.

Backfill shall be mechanically compacted (no jetting).

Special care should be taken when compacting adjacent to or near the tape seal not to disrupt the seal.

Fill placement over top of Storm Prism shall be done in accordance with ASTM C1675. Fill material not

to exceed 120 PCF. At no time shall machinery or vehicles exceeding HS-20 travel over the Storm Prism.

STORMPRISM DETENTION SYSTEM BACKFILL AND ELEVATION ELEVATION

K.STRADER

8/25/2021

8 OF 8

APPENDIX E – ASM SCOTTSDALE IMPROVEMENT PLAN BY WOOD, PATEL & ASSOCIATES, INC., DATED AUGUST 30, 2024

ENGINEER'S NOTES

- MARICOPA ASSOCIATION OF GOVERNMENTS (M.A.G.) UNIFORM STANDARD SPECIFICATIONS AND DETAILS FOR PUBLIC WORKS CONSTRUCTION (LATEST EDITION INCLUDING LATEST REVISION AND CURRENT SUPPLEMENTALS THEREOF PER THE LOCAL TOWN OR CITY) ARE INCORPORATED INTO THIS PLAN IN THEIR
- ALL WORK REQUIRED TO COMPLETE THE CONSTRUCTION COVERED BY THIS PLAN SHALL BE IN ACCORDANCE WITH THE M.A.G. STANDARD SPECIFICATIONS AND DETAILS AND CURRENT SUPPLEMENTS THEREOF PER THE LOCAL CITY OR TOWN UNLESS SPECIFIED OTHERWISE IN THESE PLANS OR ELSEWHERE IN THE CONTRACT DOCUMENTS. CONTRACTORS SHALL FAMILIARIZE THEMSELVES WITH ALL REQUIRED STANDARD SPECIFICATIONS, DETAILS AND SUPPLEMENTS PRIOR TO BIDDING THE WORK FOR THE CONSTRUCTION COVERED BY THIS PLAN.
- THE CONTRACTOR IS RESPONSIBLE FOR ALL METHODS, SEQUENCING, AND SAFETY CONCERNS ASSOCIATED WITH THIS PROJECT DURING CONSTRUCTION, UNLESS SPECIFICALLY ADDRESSED OTHERWISE IN THIS PLAN OR ELSEWHERE IN THE
- THE CONTRACTOR IS TO COMPLY WITH ALL LOCAL, STATE, AND FEDERAL LAWS AND REGULATIONS APPLICABLE TO THE CONSTRUCTION COVERED BY THIS PLAN.
- THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND COMPLYING WITH ALL PERMITS REQUIRED TO COMPLETE ALL WORK COVERED BY THIS PLAN.
- THE QUANTITIES AND SITE CONDITIONS DEPICTED IN THESE PLANS ARE FOR GENERAL INFORMATIONAL PURPOSES ONLY AND MIGHT NOT REFLECT ACTUAL QUANTITIES AND SITE CONDITIONS. CONTRACTORS SHALL SATISFY THEMSELVES AS TO ACTUAL QUANTITIES AND SITE CONDITIONS PRIOR TO BIDDING THE WORK FOR THE CONSTRUCTION COVERED BY THIS PLAN.
- A REASONABLE EFFORT HAS BEEN MADE TO SHOW THE LOCATIONS OF EXISTING UNDERGROUND FACILITIES AND UTILITIES IN THE CONSTRUCTION AREA. THE CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE TO UTILITIES AND/OR FACILITIES CAUSED DURING THEIR CONSTRUCTION OPERATIONS. THE CONTRACTOR SHALL CALL 48 HOURS IN ADVANCE FOR BLUE STAKE (1-800-STAKE-IT) PRIOR TO ANY
- THE CONTRACTOR IS RESPONSIBLE FOR ALL COORDINATION OF CONSTRUCTION AFFECTING UTILITIES AND THE COORDINATION OF ANY NECESSARY UTILITY RELOCATION WORK
- ALL PAVING, GRADING, EXCAVATION, TRENCHING, PIPE BEDDING, CUT FILL AND BACKFILL SHALL COMPLY WITH THE RECOMMENDATIONS SET FORTH IN THE SOILS (GEOTECHNICAL) REPORT FOR THIS PROJECT IN ADDITION TO THE REFERENCED REQUIRED SPECIFICATIONS AND DETAILS. THE CONTRACTOR SHALL BE AWARE THAT CERTAIN UTILITIES REQUIRE PROPER ATTENTION AND CAREFUL PLANNING DURING SITE CONSTRUCTION. PLEASE NOTE THAT UTILITIES ON THESE PLANS MAY NOT EXHIBIT THE FULL PROTECTIVE COVER REQUIRED DURING THE SUBGRADE PREPARATION PHASE OF THE CONSTRUCTION. IN SUCH INSTANCES, THE CONTRACTOR SHALL PROVIDE ADDITIONAL PROTECTION (SUCH AS RAMPING) OF INCREASED PIPE STRENGTH TO PROVIDE THE NECESSARY PROTECTION REQUIRED TO PREVENT DAMAGE DURING THE CONSTRUCTION OF THIS PROJECT. THE CONTRACTOR SHALL HOLD THE ENGINEER HARMLESS IN ALL CASES FOR DAMAGES TO UTILITIES WHERE INADEQUATE PROTECTIVE MEASURES OCCUR.
- THE CONTRACTOR IS TO VERIFY THE LOCATION AND THE ELEVATIONS OF ALL EXISTING UTILITIES AT POINTS OF TIE-IN PRIOR TO COMMENCING ANY NEW CONSTRUCTION. SHOULD ANY LOCATION OR ELEVATION DIFFER FROM THAT SHOWN ON THESE PLANS. THE CONTRACTOR SHALL CONTACT THE OWNER'S
- CONTRACTOR TO VERIFY AND COORDINATE ALL DIMENSIONS AND SITE LAYOUT WITH ARCHITECT'S FINAL SITE PLAN AND FINAL BUILDING DIMENSIONS BEFORE STARTING WORK, REPORT DISCREPANCIES TO OWNER'S AGENT.
- 12. COORDINATION BETWEEN ALL PARTIES IS ESSENTIAL PART OF CONTRACT.
- 13. CONTRACTOR IS RESPONSIBLE FOR PROJECT AND SITE CONDITIONS. AND TO WORK WITH WEATHER CONDITIONS AS THE PROJECT SITE MAY BE LOCATED IN A FLOOD PRONE AREA AND SUBJECT TO FLOODING AND ITS HAZARDS
- 14. THE CONTRACTOR IS TO VERIFY THE LOCATION, ELEVATION, CONDITION, AND PAVEMENT CROSS-SLOPE OF ALL EXISTING SURFACES AT POINTS OF TIE-IN AND MATCHING, PRIOR TO COMMENCEMENT OF GRADING, PAVING, CURB AND GUTTER, OR OTHER SURFACE CONSTRUCTION. SHOULD EXISTING LOCATIONS, ELEVATIONS, CONDITION, OR PAVEMENT CROSS-SLOPE DIFFER FROM THAT SHOWN ON THESE PLANS, RESULTING IN THE DESIGN INTENT REFLECTED ON THESE PLANS NOT ABLE TO BE CONSTRUCTED, THE CONTRACTOR SHALL NOTIFY THE OWNER'S AGENT IMMEDIATELY FOR DIRECTION ON HOW TO PROCEED PRIOR TO COMMENCEMENT OF CONSTRUCTION. THE CONTRACTOR ACCEPTS RESPONSIBILITY FOR ALL COSTS ASSOCIATED WITH CORRECTIVE ACTION IF THESE PROCEDURES ARE NOT FOLLOWED.
- CONTRACTOR IS RESPONSIBLE TO COORDINATE UTILITY CROSSINGS AT CULVERT CROSSINGS BEFORE STARTING WORK ON CULVERT, COORDINATE WITH OWNER REPRESENTATIVE. VERIFY UTILITY LINES AND/OR CONDUITS ARE IN PLACE BEFORE STARTING CULVERT WORK.
- 16. CONSTRUCT RETENTION BASIN AS SHOWN. CONTRACTOR TO SCARIFY BOTTOM OF BASIN TWO FEET DEEP AND NOT ALLOW COMPACTION OVER 80%.
- 17. THIS PROJECT REQUIRES A REGULAR ONGOING MAINTENANCE PROGRAM FOR THE DESIGNED DRAINAGE SYSTEM(S) TO PRESERVE THE DESIGN INTEGRITY AND THE ABILITY TO PERFORM ITS OPERATIONAL INTENT. FAILURE TO PROVIDE MAINTENANCE WILL JEOPARDIZE THE DRAINAGE SYSTEM(S)' PERFORMANCE AND MAY LEAD TO IT'S INABILITY TO PERFORM PROPERLY AND/OR CAUSE DAMAGE ELSEWHERE IN THE PROJECT.
- 18. SEWER LINES DESIGNED IN PROFILE AND PUBLIC WATER LINES ARE REQUIRED TO BE ASBUILT AND THE INSTALLATION AND TESTING WITNESSED BY A PROFESSIONAL ENGINEER IN ACCORDANCE WITH ARIZONA ADMINISTRATIVE CODES R18-9-E301 "4.01 GENERAL PERMIT: SEWAGE COLLECTIONS SYSTEMS" AND R18-5-507 AND 508 "APPROVAL OF CONSTRUCTION" AND "RECORD DRAWINGS", RESPECTIVELY, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY OWNER 72 HOURS IN ADVANCE WHEN THOSE SYSTEMS ARE READY TO BE WITNESSED.
- THE WORK PRODUCT PRESENTED IS BELIEVED TO BE COMPLIANT WITH THE INTENT OF THE CURRENT AMERICANS DISABILITIES ACT (ADA) REQUIREMENTS AS INTERPRETED BY THE REVIEWING AGENCY(S). IF CONSTRUCTION OF THE PROJECT IS DELAYED, THIS WORK PRODUCT SHOULD BE UPDATED TO ACCOUNT FOR ANY RELEVANT ADA UPDATES BEFORE CONSTRUCTION BEGINS.
- 20. LOWEST FLOOR (LF) REFERS TO EITHER FLOOR/SLAB ELEVATION OR TOP OF BASEMENT SLAB. LF ELEVATIONS ON THE GRADING AND DRAINAGE PLANS FOR RESIDENTIAL UNITS REFLECT SLAB ON GRADE CONDITIONS AND CANNOT BE LOWERED WITHOUT AGENCY APPROVAL IN LOCATIONS WHERE 'SPECIAL FLOOD HAZARD AREAS' EXIST. IN NON-FLOOD HAZARD LOCATIONS, TO ENSURE THAT ADEQUATE RESIDENTIAL LOT DRAINAGE CAN BE ACHIEVED, A PROFESSIONAL ENGINEER SHOULD BE CONSULTED IF THE LF FOR THE SLAB IS PROPOSED TO BE LOWERED, OR IF A BASEMENT IS TO BE CONSTRUCTED.

ASM SCOTTSDALE

PRELIMINARY IMPROVEMENT PLAN SCOTTSDALE, ARIZONA

A PORTION OF SECTION 35, TOWNSHIP 4 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER MERIDIAN, MARICOPA COUNTY, ARIZONA

EARTHWORK QUANTITIES (ESTIMATED)

RAW CUT: CY CY QUANTITIES ARE ESTIMATED IN PLACE. NO PRECOMPACTION, SHRINK OR SWELL IS

CITY OF SCOTTSDALE NOTES

PLEASE REFER TO SHEET C2 FOR CITY OF SCOTTSDALE NOTES.

QUANTITIES

PLEASE REFER TO SHEET C2 FOR ESTIMATED QUANTITIES FOR WORK IN PUBLIC RIGHTS-OF-WAY AND EASEMENTS.

LEGEND

ASSUMED.

PLEASE REFER TO SHEET C2 FOR LEGEND AND LIST OF ABBREVIATIONS.

UTILITY NOTES

- THESE PLANS HAVE BEEN SUBMITTED TO THE FOLLOWING UTILITY COMPANIES FOR APPROVAL WITHIN THEIR AREA OF INTEREST. THE SIZE AND LOCATIONS, AS SHOWN, OF THE GAS, TELEPHONE AND POWER LINES, AND CONNECTIONS AGREE WITH THE FURNISHED INFORMATION CONTAINED IN THE UTILITY COMPANY'S RECORDS. WHERE THE WORK TO BE DONE CONFLICTS WITH ANY OF THESE UTILITIES, THE CONFLICTS SHALL BE RESOLVED AS SPECIFIED IN THE SPECIAL PROVISIONS AND/OR AS OTHERWISE NOTED ON THESE PLANS. CONFLICTS ARISING DURING THE COURSE OF CONSTRUCTION FROM UNFORESEEN CIRCUMSTANCES SHALL BE REPORTED TO THE INTERESTED UTILITY COMPANY AND BE RESOLVED BY THEM AND THE DESIGN ENGINEER.
- THE CITY WILL NOT PARTICIPATE IN THE COST OF CONSTRUCTION OR UTILITY RELOCATION.

FEMA FIRM NOTE (ZONE AO)

ACCORDING TO FEMA FLOOD INSURANCE RATE MAPPING, THE SUBJECT PROPERTY IS LOCATED IN 'SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD' "ZONE AO". ZONE AO IS DESCRIBED AS: "FLOOD DEPTHS OF 1 TO 3 FEET (USUALLY SHEET FLOW ON SLOPING TERRAIN); AVERAGE DEPTHS DETERMINED. FOR AREAS OF ALLUVIAL FAN FLOODING, VELOCITIES ALSO DETERMINED."

FLOOD INSURANCE RATE MAP (FIRM) INFORMATION

COMMUNITY NUMBER	PANEL NUMBER	SUFFIX	DATE OF FIRM	FIRM ZONE	BASE FLOOD ELEVATION (IN AO ZONE, USE DEPTH)
04013C	1320	L	07/20/2021	AO	1

08/30/2024

DATE

ENGINEER'S CERTIFICATION

ENGINEER'S CERTIFICATION: THE LOWEST FLOOR ELEVATION(S) AND/OR FLOOD PROOFING ELEVATION(S) ON THIS PLAN ARE SUFFICIENTLY HIGH TO PROVIDE PROTECTION FROM FLOODING CAUSED BY A ONE-HUNDRED YEAR STORM, AND ARE IN ACCORDANCE WITH CITY OF SCOTTSDALE REVISED CODE, CHAPTER 37-FLOODPLAIN AND STORMWATER REGULATIONS.

DATE **ENGINEER SIGNATURE**

AS-BUILT CERTIFICATION

I HEREBY CERTIFY THAT THE "RECORD DRAWING" MEASUREMENTS AS SHOWN HEREON WERE MADE UNDER MY SUPERVISION OR AS NOTED AND ARE CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

REGISTERED ENGINEER/ LAND SURVEYOR

SEAL

FINISH FLOOR ELEVATION CALCULATION

NO CONFLICT SIGNATURE BLOCK

REPRESENTATIVE

HAILEY PARKS

JEANETTE DEBOARD

ANDY SAKS

JACOB HORSMAN

RICHARD YOUNG

I DARIN L. MOORE, P.E. , AS THE ENGINEER OF RECORD FOR THIS DEVELOPMENT, HEREBY

IMPROVEMENT PLANS FOR REVIEW, AND THAT ALL CONFLICTS IDENTIFIED BY THE UTILITIES

HAVE BEEN RESOLVED. IN ADDITION, "NO CONFLICT" FORMS HAVE BEEN OBTAINED FROM

CERTIFY THAT ALL UTILITY COMPANIES LISTED ABOVE HAVE BEEN PROVIDED FINAL

NAME OF COMPANY | TELEPHONE | DATE |

NUMBER

602-493-4401

480-221-7810

480-730-3857

602-615-8995

DATE

HAG - LAB - 1599.14 - OFFICE - 1602.07 - PARKING STRUCTURE - 1599.76 - CUP - 1600.56

- CHEMICAL STORAGE - 1601.88

LAG - LAB - 1592.92 - OFFICE - 1598.01 - PARKING STRUCTURE - 1592.16

- CUP - 1599.06 - CHEMICAL STORAGE - 1599.10

LFF - LAB - 1602.50 - OFFICE - 1604.50

- PARKING STRUCTURE - 1599.00 - CUP - 1602.75 - CHEMICAL STORAGE - 1605.00

RFD - LAB - 1601.14 - OFFICE - 1604.07

UTILITY

ELECTRIC

TELEPHONE

NATURAL GAS

SIGNATURE

ENGINEER'S CERTIFICATION

CABLE TV

OTHER

- PARKING STRUCTURE - 1601.76 - CUP - 1603.58

UTILITY

COMPANY

ARIZONA PUBLIC SERVICE

LUMEN

SOUTHWEST GAS

COX COMMUNICATIONS

MCI

N/A

EACH UTILITY COMPANY AND ARE INCLUDED IN THIS SUBMITTAL.

- CHEMICAL STORAGE - 1604.75 ALL ELECTROMECHANICAL EQUIPMENT SHALL BE ELEVATED TO RFD ELEVATION

SHEET INDEX

C1 - COVER SHEET C2 - DETAILS, NOTES & QUANTITIES C3 - INDEX MAP C4 - C5 - DEMOLITION PLAN C6-C14 - GRADING & DRAINAGE PLAN C15-C23 - STORM DRAIN PLAN C24-C34 - WATER PLAN & PROFILE C35-C37 - SEWER PLAN & PROFILE C38-C40 - DETAILS C41 - C43 - SECTIONS

PARCEL DESCRIPTION

PLEASE REFER TO SHEET C2 FOR PARCEL DESCRIPTION.

BASIS OF BEARING

THE NORTH LINE OF THE NORTHEAST QUARTER OF SECTION 35, TOWNSHIP 4 NORTH, RANGE 4 EAST G.&S.R.B.&M. SAID LINE BEARS S89°58'25"E.

BENCHMARK

CITY OF SCOTTSDALE BRASS CAP FLUSH 450'± NORTH OF PRINCESS DRIVE ON SCOTTSDALE ROAD, BEING THE WEST QUARTER CORNER OF SECTION 35, TOWNSHIP 4 NORTH, RANGE 4 EAST. CITY OF SCOTTSDALE DATUM, NAVD88 DATUM **ELEVATION=1553.22'.**

I HEREBY CERTIFY THAT ALL ELEVATIONS REPRESENTED ON THIS PLAN ARE BASED ON NAVD 1988, MCDOT, AND MEET THE FEMA BENCHMARK MAINTENANCE (BMM) CRITERIA.

PUBLIC UTILITIES

CITY OF SCOTTSDALE SEWER CITY OF SCOTTSDALE **ELECTRIC TELEPHONE NATURAL GAS** SOUTHWEST GAS CABLE TV COX COMMUNICATIONS

SOILS REPORT NOTE

A SOILS GEOTECHNICAL REPORT HAS BEEN PREPARED FOR THIS PROJECT TITLED. GEOTECHNICAL EVALUATION MAYO BOULEVARD AND MILLER ROAD DEVELOPMENT BY NINYO & MOORE DATED NOVEMBER 30, 2023. PROJECT NO. 607802001.

STIPULATION CONFORMANCE STATEMENT

"THE ENGINEER OF RECORD ON THESE PLANS HAS RECEIVED A COPY OF THE APPROVED STIPULATIONS FOR THIS PROJECT AND HAS DESIGNED THESE PLANS IN CONFORMANCE WITH THE APPROVED STIPULATIONS."

ENGINEERING DEPARTMENT MANAGER

N/A **ENGINEER SIGNATURE**

DATE

CONTACTED SIGNED

GENSLER

2575 EAST CAMELBACK ROAD, SUITE 175

PROJECT SITE DATA

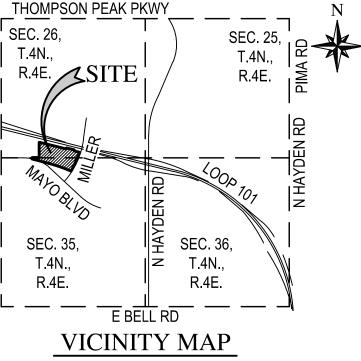
CITY OF SCOTTSDALE CIVIL APPROVAL

REVIEW & RECOMMENDED APPROVAL BY: SIGNS & PAVING MARKINGS **GRADING & PLANNING** DRAINAGE WATER & **FIRE** SEWER RETAINING SIGNALS & WALLS STREET

DATE

EXPIRES 06-30-25 SCALE (HORIZ.) SCALE (VERT.) N/A 08/30/2024

OF 43 CHECKED BY: DM DESIGNED BY: RS DRAFTED BY: JRS



OWNER / DEVELOPER ASM AMERICA INC.

3440 EAST UNIVERSITY DRIVE PHOENIX, ARIZONA 85034 **CONTACT: PAUL CROTHERS** PHONE: (602) 470-2600

ENGINEER

WOOD, PATEL & ASSOCIATES, INC. 2051 WEST NORTHERN AVENUE, SUITE 100 PHOENIX, ARIZONA 85021 CONTACT: DARIN MOORE, P.E. PHONE: (602) 335-8500 FAX: (602) 335-8580

ARCHITEC

PHOENIX, ARIZONA 85016 CONTACT: DAVID HEALY PHONE: (602) 523-4902

ASSESSOR PARCEL NUMBER(S): 215-07-209K PROJECT SITE ADDRESS: SCOTTSDALE, ARIZONA 85255 PROJECT SITE AREA(S): NET AREA = 23.9 AC DISTURBED AREA = 20.3± AC

WOOD

PATEL

Wood, Patel & Associates, In

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ARIZONA811 Arizona Blue Stake, Inc.

Dial 8-1-1 or 1-800-STAKE-IT (782-5344 In Maricopa County: (602) 263-1100

7

IMPROVEMENT SDALE, ARIZONA

LIMINARY SCOTTS

2

ALE

OTTSD

SM

4

Land Survey

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JOB NUMBER 235526 SHEET

CITY OF SCOTTSDALE NOTES

GENERAL CONSTRUCTION NOTES FOR CAPITAL PROJECTS

- 1. ALL IMPROVEMENT CONSTRUCTION SHALL COMPLY WITH THE 2020 MARICOPA COUNTY ASSOCIATION OF GOVERNMENTS STANDARD SPECIFICATIONS AND DETAILS FOR PUBLIC WORKS CONSTRUCTION AS AMENDED BY THE LATEST VERSION OF THE CITY OF SCOTTSDALE SUPPLEMENTAL STANDARD SPECIFICATIONS AND DETAILS AND CITY OF SCOTTSDALE'S DESIGN STANDARDS & POLICIES MANUAL (DS&PM). IF THERE IS A CONFLICT, THE LATTER SHALL APPLY. ALL FACILITIES CONSTRUCTION SHALL COMPLY WITH THE LATEST BUILDING CODES AS AMENDED AND ADOPTED BY THE CITY OF SCOTTSDALE.
- THE ENGINEERING DESIGNS ON THESE PLANS ARE APPROVED BY THE CITY IN SCOPE AND NOT IN DETAIL. IF CONSTRUCTION QUANTITIES ARE SHOWN ON THESE PLANS, THEY ARE NOT VERIFIED BY
- BASED ON THE INFORMATION SUBMITTED ON THE PLANS AND ASSOCIATED DOCUMENTS, THE CITY HAS REVIEWED AND FOUND THEM TO BE IN ACCORDANCE WITH THE SCOTTSDALE REVISED CODE AND ARE ACCEPTABLE FOR PERMIT ISSUANCE. THIS ACCEPTANCE BY THE CITY DOES NOT AUTHORIZE VIOLATIONS OF ANY APPLICABLE CODE, ORDINANCE OR STANDARD AS ADOPTED BY THE SCOTTSDALE REVISED CODE.
- APPROVAL OF THE PLANS BY THE CITY IS VALID FOR SIX MONTHS. IF A PERMIT FOR THE CONSTRUCTION HAS NOT BEEN ISSUED WITHIN SIX MONTHS OF REVIEW, THE PLANS SHALL BE RESUBMITTED TO THE CITY FOR REAPPROVAL.
- ANY DEVIATION FROM THE APPROVED PLANS SHALL BE REVIEWED AND APPROVED BY THE CITY PRIOR TO THAT CHANGE BEING INCORPORATED INTO THE PROJECT.
- A CITY CAPITAL PROJECTS INSPECTOR WILL INSPECT ALL WORK WITHIN THE CITY RIGHTS-OF-WAY EASEMENTS AND FACILITIES.
- 7. ANY SPECIAL INSPECTION REQUIRED SHALL BE IN ADDITION TO ANY ROUTINE INSPECTION BY THE
- CITY ENCROACHMENT AND BUILDING PERMITS ARE REQUIRED FOR WORK IN PUBLIC RIGHTS-OF-WAY, EASEMENTS GRANTED FOR PUBLIC PURPOSES AND FACILITIES. PERMITS WILL BE ISSUED BY THE CITY THROUGH THE CITY'S ONE STOP SHOP. COPIES OF ALL PERMITS SHALL BE RETAINED ON-SITE AND SHALL BE AVAILABLE FOR INSPECTION AT ALL TIMES. FAILURE TO PRODUCE THE REQUIRED PERMITS WILL RESULT IN IMMEDIATE WORK STOPPAGE UNTIL THE PROPER PERMIT DOCUMENTATION IS OBTAINED.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS FOR SALVAGING PROTECTED NATIVE PLANTS PRIOR TO THE START OF CONSTRUCTION.
- 10. CONTRACTOR SHALL CONTACT AZ 811 TWO FULL WORKING DAYS PRIOR TO BEGINNING EXCAVATION.
- 11. ALL EXCAVATION AND GRADING WHICH IS NOT IN PUBLIC RIGHTS-OF-WAY OR IN EASEMENTS GRANTED FOR PUBLIC PURPOSES MUST CONFORM TO SECTION 1803 AND APPENDIX J OF THE LATEST INTERNATIONAL CODE COUNCIL AS ADOPTED AND AMENDED BY THE CITY OF SCOTTSDALE A PERMIT FOR THIS GRADING MUST BE SECURED FROM THE CITY.
- THRUST RESTRAINT, WHERE REQUIRED, ON ALL CITY WATER LINES SHALL BE PROVIDED USING MEGALUG MECHANICAL JOINT RESTRAINTS OR CITY-APPROVED EQUAL
- 13. ANY ASPHALT MIX DESIGN USED ON CITY PROJECTS SHALL HAVE BEEN APPROVED FOR THAT USE PER SECTION 5-10 OF THE CITY'S DS&PM AND APPEAR ON THE "APPROVED LIST OF ASPHALT MIXES AS DISTRIBUTED BY THE EAST VALLEY ASPHALT COMMITTEE (EVAC).
- THE CONTRACTOR SHALL BE RESPONSIBLE TO REMOVE AND REPLACE, AT NO ADDITIONAL COST T THE CITY, ANY AND ALL PAVEMENT, SIDEWALK, CURB AND GUTTER, DRAINAGE STRUCTURES, ETC OUTSIDE THE PAY LIMIT THAT ARE DAMAGED DUE TO THEIR ACTIVITIES ON THE PROJECT. THIS INCLUDES. BUT IS NOT LIMITED TO. THE REMOVAL AND REPLACEMENT OF NEWLY CRACKED ROADWAY INFRASTRUCTURE, THE REMOVAL AND REPLACEMENT OF EXISTING CRACKED ROADWAY INFRASTRUCTURE WHERE THE CRACKS HAVE BEEN ENLARGED DUE TO THE CONTRACTOR'S OPERATIONS, THE REMOVAL AND REPLACEMENT OF DEFORMED ROADWAY INFRASTRUCTURE. ALL SAWCUTS USED FOR THE REMOVAL OF THESE ITEMS SHALL BE PERPENDICULAR AND PARALLEL TO THE CENTERLINE CONTROLLING THAT ITEM, OR AT THE DIRECTION OF THE CITY'S CAPITAL PROJECTS INSPECTOR.
- 15. ALL CAPITAL IMPROVEMENT PROJECTS SHALL MEET THE PROCEDURES AND STANDARDS FOR THE USE OF TEMPORARY/SECURITY FENCING AROUND THE PERIMETER OF CONSTRUCTION SITES. AS DEFINED IN THE CITY'S ZONING ORDINANCE, ARTICLE VII, SECTION 7.700.

GENERAL NOTES FOR PUBLIC WORKS CONSTRUCTION

- 1. ALL CONSTRUCTION IN THE PUBLIC RIGHTS-OF-WAY OR IN EASEMENTS GRANTED FOR PUBLIC USE MUST CONFORM TO THE LATEST MAG UNIFORM STANDARD SPECIFICATIONS AND UNIFORM STANDARD DETAILS FOR PUBLIC WORKS CONSTRUCTION AS AMENDED BY THE LATEST VERSION OF THE CITY OF SCOTTSDALE SUPPLEMENTAL STANDARD SPECIFICATIONS AND SUPPLEMENTAL STANDARD DETAILS, IF THERE IS A CONFLICT. THE CITY'S SUPPLEMENTAL STANDARD DETAILS WILL GOVERN.
- THE CITY ONLY APPROVES THE SCOPE, NOT THE DETAIL, OF ENGINEERING DESIGNS; THEREFORE, IF CONSTRUCTION QUANTITIES ARE SHOWN ON THESE PLANS, THEY ARE NOT VERIFIED BY THE CITY.
- 3. THE APPROVAL OF PLANS IS VALID FOR SIX (6) MONTHS. IF A RIGHT-OF-WAY PERMIT FOR THE CONSTRUCTION HAS NOT BEEN ISSUED WITHIN THIS TIME FRAME, THE PLANS MUST BE RESUBMITTED TO THE CITY FOR REAPPROVAL.
- 4. A CITY INSPECTOR WILL INSPECT ALL WORKS WITHIN THE CITY OF SCOTTSDALE. NOTIFY INSPECTION SERVICES 72 HOURS BEFORE BEGINNING WORK.
- 5. WHENEVER EXCAVATION IS NECESSARY, CALL THE BLUE STAKE CENTER, 811, TWO WORKING DAYS BEFORE EXCAVATION BEGINS.
- PERMISSION TO WORK IN THE RIGHT-OF-WAY (PWR) PERMITS ARE REQUIRED FOR ALL WORKS WITHIN THE RIGHTS-OF-WAY AND EASEMENTS GRANTED FOR PUBLIC PURPOSES. COPIES OF ALL PERMITS MUST BE RETAINED ON-SITE AND BE AVAILABLE FOR INSPECTION AT ALL TIMES. FAILURE TO PRODUCE THE REQUIRED PERMITS WILL RESULT IN IMMEDIATE SUSPENSION OF ALL WORK UNTIL THE PROPER PERMIT DOCUMENTATION IS OBTAINED.

FIRE NOTE:

1. ALL PRIVATE STREETS AND DRIVES SHALL CONFORM TO THE FIRE DEPARTMENT GUIDELINES FOR EMERGENCY VEHICLE ACCESS.

SEWER NOTE:

- 1. THE ONSITE SEWER SYSTEM CONSTRUCTED BY THIS PLAN SET IS A PRIVATE SYSTEM AND WILL NOT BE MAINTAINED BY THE CITY OF SCOTTSDALE.
- 2. MAINTENANCE OF THE ONSITE SEWER SYSTEM IS THE RESPONSIBILITY OF THE OWNER.

WATER NOTE:

1. THE WATER SYSTEM SHOWN HEREIN HAS BEEN DESIGNED TO ADEQUATELY SUPPLY WATER IN SUFFICIENT QUANTITY AND PRESSURE TO MEET LOCAL FIRE REQUIREMENTS.

PARCEL DESCRIPTION

LOCATED IN THE SOUTH HALF OF SECTION 26 AND THE NORTH HALF OF SECTION 35, TOWNSHIP 4 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER BASE AND MERIDIAN, MARICOPA COUNTY, ARIZONA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCING AT THE NORTHEAST CORNER OF SAID SECTION 35, MARKED BY A FOUND 3 INCH MARICOPA COUNTY BRASS CAP FLUSH;

- THENCE ALONG THE NORTH LINE OF THE NORTHEAST QUARTER OF SAID SECTION 35, NORTH 89 DEGREES 58 MINUTES 25 SECONDS WEST, A DISTANCE OF 2640.93 FEET TO THE NORTH QUARTER CORNER OF SAID SECTION 35, MARKED WITH A BUREAU OF LAND MANAGEMENT BRASS CAP STAMPED "T.4N., R.4E., 1/4, S26, S 35,
- THENCE NORTH 89 DEGREES 58 MINUTES 33 SECONDS WEST, ALONG THE NORTH LINE OF THE NORTHWEST QUARTER OF SAID SECTION 35, A DISTANCE OF 1320.42 FEET TO THE POINT OF BEGINNING: THENCE NORTH 00 DEGREES 01 MINUTE 48 SECONDS WEST, A DISTANCE OF 573.93
- FEET TO THE SOUTH RIGHT-OF-WAY OF LOOP 101; THENCE ALONG SAID RIGHT-OF-WAY SOUTH 79 DEGREES 07 MINUTES 59 SECONDS EAST, A DISTANCE OF 42.49 FEET;
- THENCE SOUTH 76 DEGREES 02 MINUTES 04 SECONDS EAST, A DISTANCE OF 1,007.00 FEET;
- THENCE NORTH 13 DEGREES 57 MINUTES 56 SECONDS EAST, A DISTANCE OF 40.00
- THENCE SOUTH 76 DEGREES 02 MINUTES 04 SECONDS EAST, A DISTANCE OF 300.00 FEET;
- THENCE SOUTH 13 DEGREES 57 MINUTES 56 SECONDS WEST, A DISTANCE OF 40.00
- THENCE SOUTH 76 DEGREES 02 MINUTES 04 SECONDS EAST, A DISTANCE OF 209.66 FEET TO THE TO THE POINT OF A NON-TANGENT CURVE TO THE RIGHT, OF WHICH THE RADIUS POINT LIES NORTH 72 DEGREES 01 MINUTE 31 SECONDS WEST, A RADIAL DISTANCE OF 3,730.00 FEET;
- THENCE DEPARTING SAID RIGHT-OF-WAY, SOUTHWESTERLY ALONG THE ARC OF SAID CURVE, THOUGH A CENTRAL ANGLE OF 11 DEGREES 58 MINUTES 10 SECONDS A DISTANCE OF 779.23 FEET TO THE POINT OF A NON-TANGENT CURVE TO THE LEFT OF WHICH THE RADIUS POINT LIES SOUTH 23 DEGREES 22 MINUTES 39 SECONDS WEST, A RADIAL DISTANCE OF 6,000.00 FEET;
- THENCE WESTERLY ALONG THE ARC OF SAID CURVE, THOUGH A CENTRAL ANGLE OF 08 DEGREES 32 MINUTES 27 SECONDS, A DISTANCE OF 894.39 FEET ALONG THE CENTERLINE OF POSSIBLE PROPOSED MAYO BOULEVARD;
- THENCE ALONG SAID NON-TANGENT LINE, NORTH 75 DEGREES 09 MINUTES 57 SECONDS WEST, A DISTANCE OF 645.72 FEET;
- THENCE DEPARTING SAID CENTERLINE NORTH 14 DEGREES 50 MINUTES 12 SECONDS EAST, A DISTANCE OF 55.03 FEET TO THE NORTH LINE OF THE NORTHWEST QUARTER OF SAID SECTION 35;

THENCE SOUTH 89 DEGREES 58 MINUTES 33 SECONDS EAST, A DISTANCE OF 257.05 FEET TO THE POINT OF BEGINNING.

EXPECTING AND PURSUANT TO THE PROVISIONS OF ARIZONA REVISED STATUTES § 37-231, OF THE FOLLOWING SUBSTANCES NOT HERETOFORE RETAINED AND RESERVED BY A PREDECESSOR IN TITLE TO THE STATE OF ARIZONA, ALL OIL, GAS, OTHER HYDROCARBON SUBSTANCES. HELIUM OR OTHER SUBSTANCES OF GASEOUS NATURE, GEOTHERMAL RESOURCES, COAL, METALS, MINERALS, FOSSILS, FERTILIZERS OF EVERY NAME AND DESCRIPTION, TOGETHER WITH ALL URANIUM, THORIUM OR ANY OTHER MATERIAL WHICH IS OR MAY BE DETERMINED BY THE LAWS OF THE UNTIED STATES, OR OF THIS STATE OR DECISIONS OF COURT. TO BE PECULIARLY ESSENTIAL TO THE PRODUCTION OF FISSIONABLE MATERIALS, WHETHER OR NOT OF COMMERCIAL VALUE, AS SET FORTH IN PATENT RECORDED AUGUST 10, 2022 IN RECORDINGS NO. 202206326 39, RECORDS OF MARICOPA

COUNTY, ARIZONA.

4" A.C. OVER 6" A.B.C.	SY
6" VERTICAL CURB & GUTTER	LF
6" SINGLE CURB	LF
CONCRETE SIDEWALK	SF
CONCRETE APRON	SF
SAWCUT, REMOVE & REPLACE EXISTING PAVEMENT	SY
SEWER	
5' DIAMETER SEWER MANHOLE	EA
SEWER CLEANOUT	EA
8" PVC SDR35 SEWER LINE	LF
6" PVC SDR35 SEWER LINE	LF
4" PVC SDR35 SEWER LINE	LF
DROP SEWER CONNECTION	LF
WATER	
GATE VALVE	E
FIRE HYDRANT COMPLETE	L
8" DOUBLE CHECK VALVE BACKFLOW	L
2" WATER SERVICE AND METER BOX	E.
12" POLYWRAPPED DIP CLASS 350	L
6" POLYWRAPPED DIP CLASS 350	L
8" POLYWRAPPED DIP CLASS 350	E.
2" PVC SCH 40	E
10" DOUBLE CHECK VALVE BACKFLOW	E
1.5" WATER SERVICE AND METER BOX	E
4" POLYWRAPPED DIP CLASS 350	L
2" REDUCED PRESSURE PRINCIPLE BACKFLOW	L

QUANTITIES ARE ESTIMATES ONLY. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL QUANITIES FOR BIDDING PURPOSES.

PAVING

C. OVER 6" A.B.C.	SY
ERTICAL CURB & GUTTER	LF
INGLE CURB	LF
NCRETE SIDEWALK	SF
NCRETE APRON	SF
VCUT, REMOVE & REPLACE EXISTING PAVEMENT	SY
SEWER	
AMETER SEWER MANHOLE	EA
VER CLEANOUT	EA
VC SDR35 SEWER LINE	LF
VC SDR35 SEWER LINE	LF
VC SDR35 SEWER LINE	LF
OP SEWER CONNECTION	LF
WATER	
E VALVE	EΑ
HYDRANT COMPLETE	LF
OUBLE CHECK VALVE BACKFLOW	LF
/ATER SERVICE AND METER BOX	ΕA
POLYWRAPPED DIP CLASS 350	LF
OLYWRAPPED DIP CLASS 350	LF
OLYWRAPPED DIP CLASS 350	ΕA
VC SCH 40	ΕA
DOUBLE CHECK VALVE BACKFLOW	ΕA
WATER SERVICE AND METER BOX	ΕA
OLYWRAPPED DIP CLASS 350	LF
EDUCED PRESSURE PRINCIPLE BACKFLOW	LF

ESTIMATED QUANTITIES NOTE

P 0.00

0

 $\boxtimes \boxtimes \Box$

SPOT ELEVATION

FIRE HYDRANT

UTILITY POLE

CATCH BASIN

JUNCTION BOX/RISER

STREET/PARKING LIGHT

EXISTING SURVEY	PROPOSED GRADING, DRAINAGE & PAVING		
— — — — SECTION LINE — — — — — RIGHT OF WAY — — — — PROPERTY LINE — — — — ROAD CENTERLINE	1300 ———————————————————————————————————	MAJOR CONTOUR MINOR CONTOUR SPOT ELEVATIONS STORM DRAIN PIPE	
O O A SURVEY MARKER UG ELECTRIC (BURIED CABLE)	1.0% GB RIDGE	STORM DRAIN CATCH BASIN SLOPE ARROW GRADE BREAK/RIDGE	
—— E (CONDUIT) —— UG ELECTRIC (CONDUIT) —— E (DUCT BANK) —— UG ELECTRIC (DUCT BANK) ————————————————————————————————————	TW 0.00 TF 0.00	RIP RAP WALL ELEVATION	
OHE OVERHEAD ELECTRIC OVERHEAD TELEPHONE UG TELEPHONE	↓ ↓ ↓	ROOF DRAIN/DRAIN ARROW DRYWELL	
CABLE TELEVISION OHTV OVERHEAD CABLE TELEVISION T (DUCT BANK) TELEPHONE DUCT BANK	OUTFALL ELEVATION EL:XX.XX	SITE ULTIMATE OUTFALL LOCATION & ELEVATION	
BARBED WIRE FENCE CHAIN LINK FENCE WOOD FENCE		WALL CONCRETE SIDEWALK	
BLOCK WALL — 4"G (MATERIAL) — GAS LINE		CONCRETE PAVEMENT LIGHT DUTY ASPHALT PAVEMENT	
— 8"S (MATERIAL) — SEWER LINE STORM DRAIN PIPE — 4"IRR (MATERIAL) — IRRIGATION LINE		HEAVY DUTY ASPHALT PAVEMENT	
— 8"W (MATERIAL) — WATER LINE — CURB SIDEWALK	Ġ.	STREET/PARKING LIGHT ADA PARKING SYMBOL	
— — — — −1300− — — — MAJOR CONTOUR — — — — −1299− — — — MINOR CONTOUR	PROPOSED WATER & SEWER		
VEGETATION BUILDING S SEWER MANHOLE STORM DRAIN MANHOLE TELEPHONE MANHOLE	—————————————————————————————————————	WATER LINE WATER LINE FITTINGS BACKFLOW PREVENTION DEVICE WATER VALVE	
	•	FIRE DEPARTMENT CONNECTION	

LEGEND

Dial 8-1-1 or 1-800-STAKE-IT (7: In Maricopa County: (602) 263-1100 MENT 0 **IMPR**(-IMINARY SCOTTS 4

FIRE DEPARTMENT CONNECTION

TAPPING SLEEVE & VALVE

PRESSURE RELEASE VALVE

SEWER MANHOLE

CLEANOUT

AIR/VACUUM RELEASE VALVE

FIRE HYDRANT

WATER METER

PLUG

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REDUCER

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PATEI

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Water Resources

Construction Management

Call at least two full working days before you begin excavation.

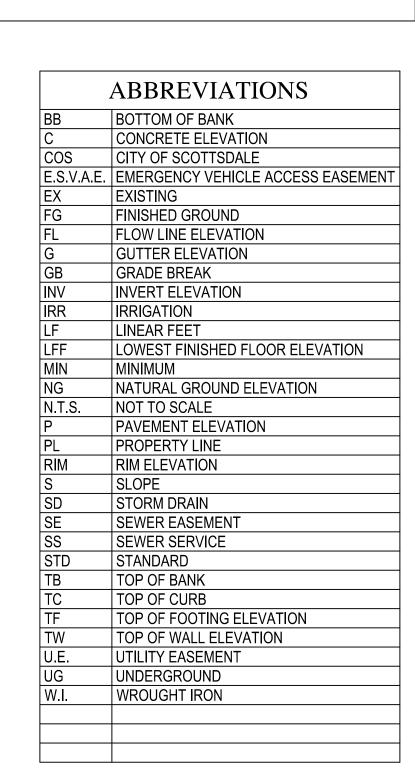
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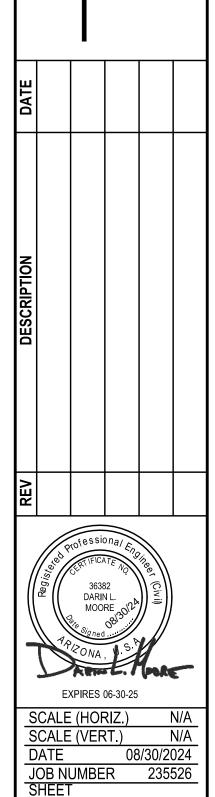
Arizona Blue Stake, Inc.

Land Survey

602.335.8500

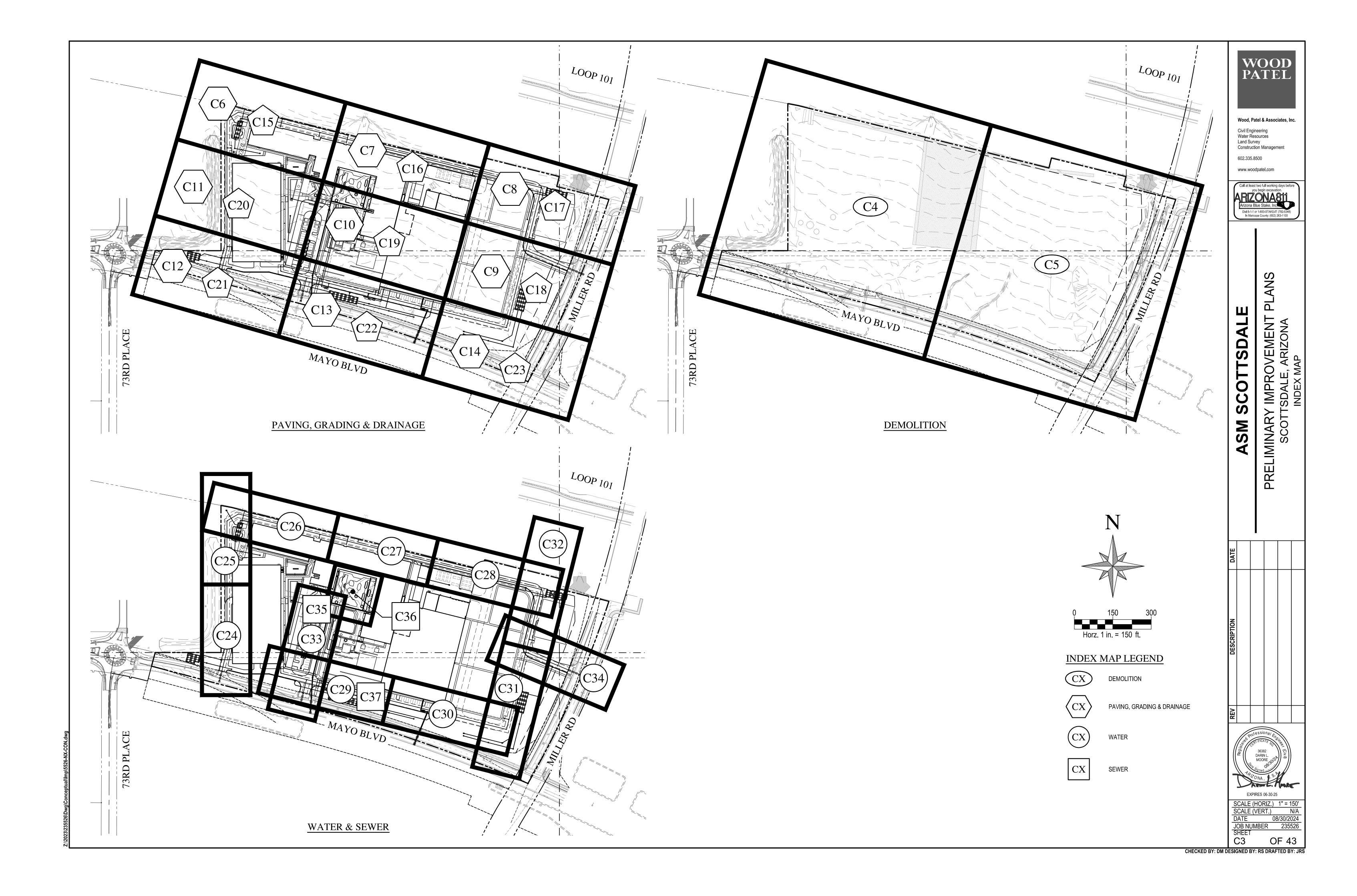
www.woodpatel.com

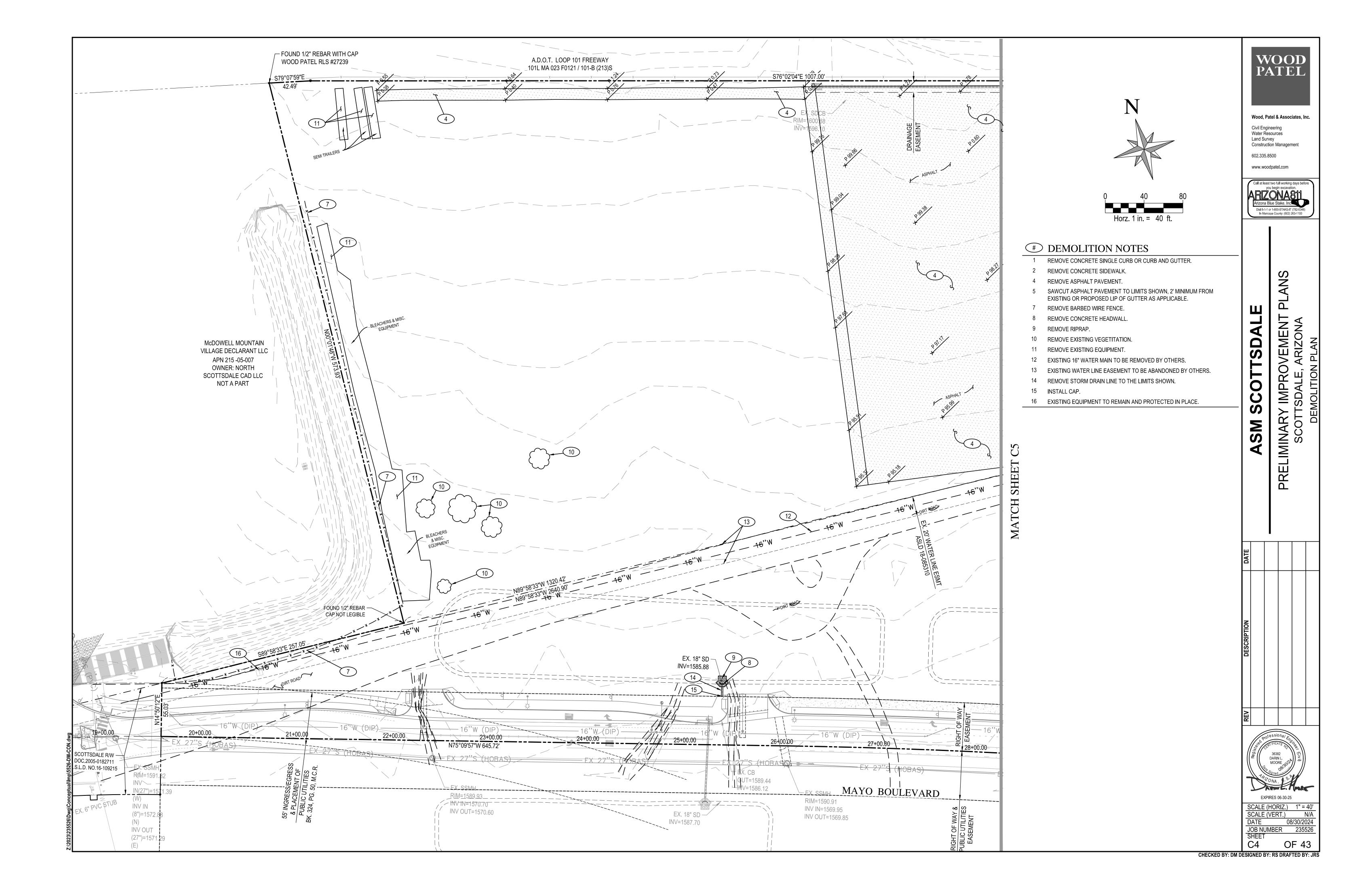


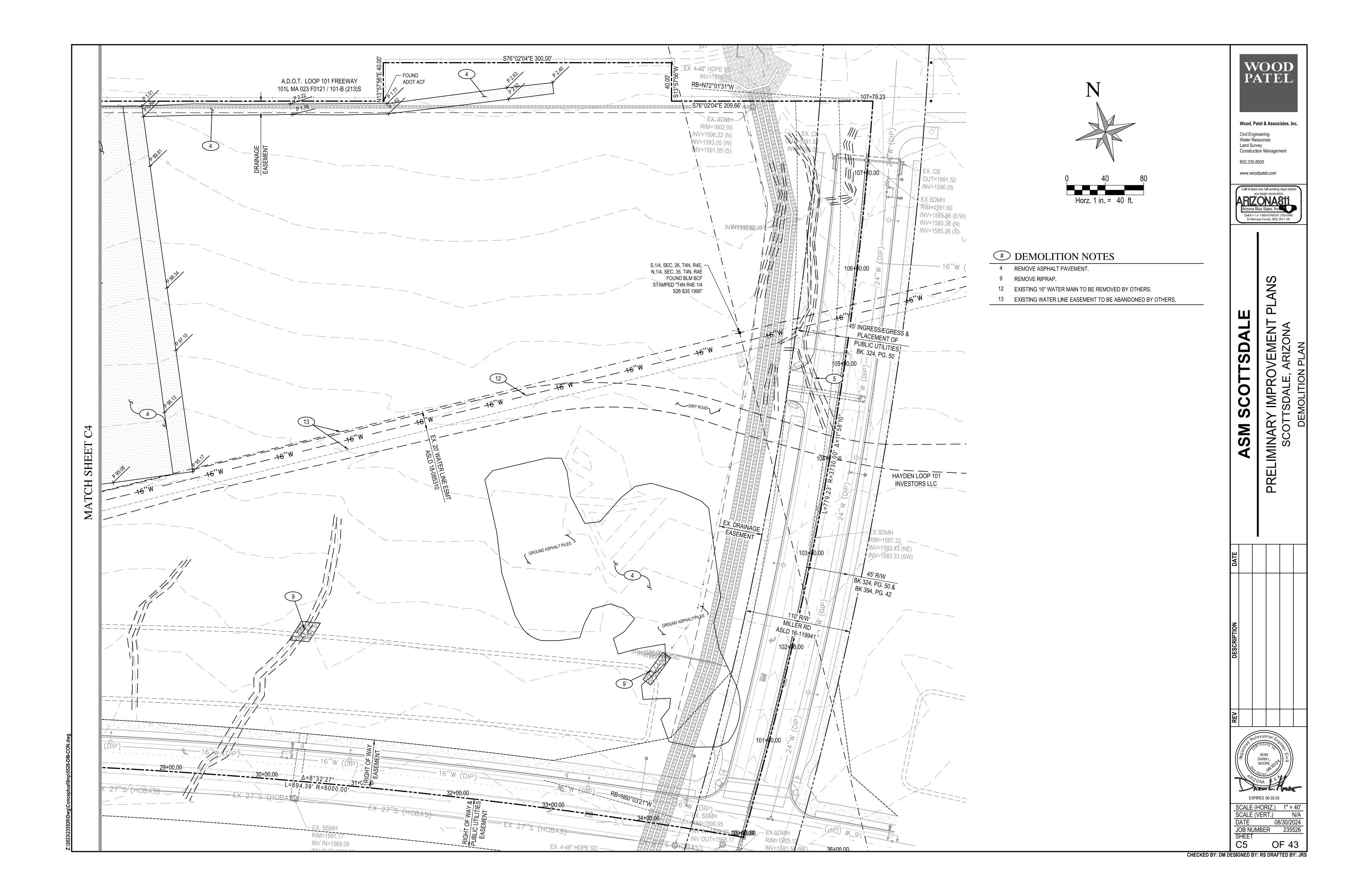


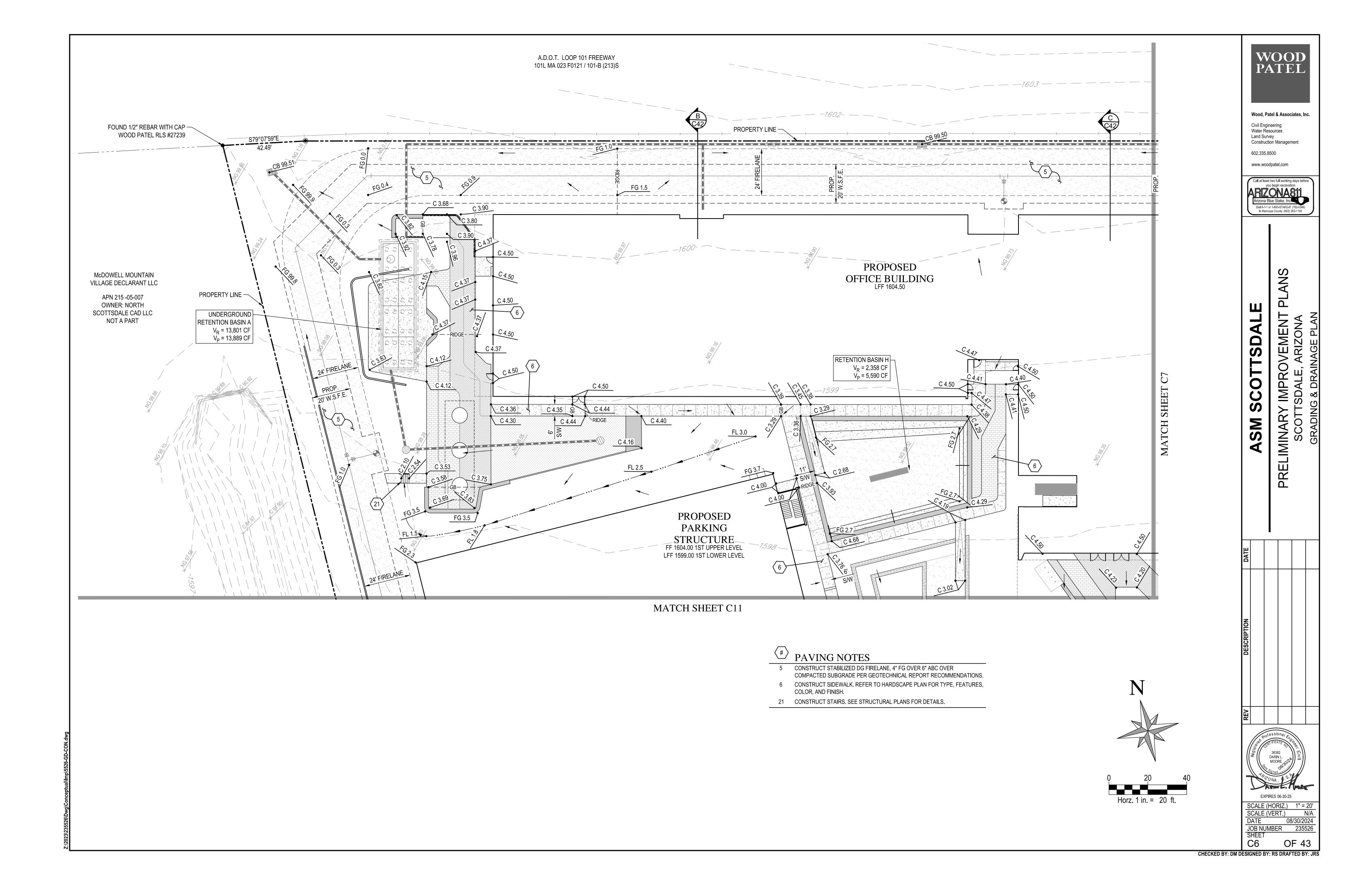
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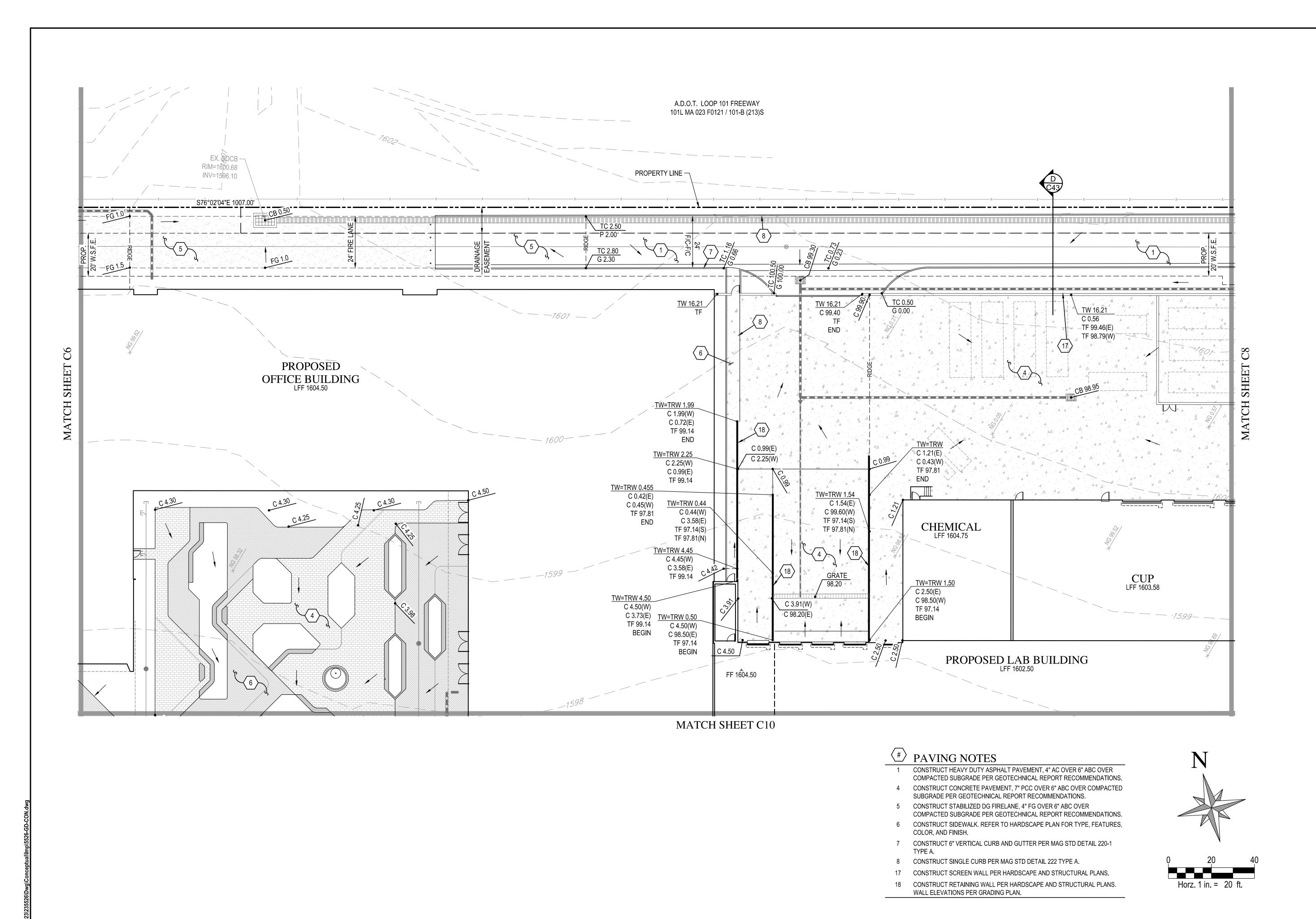
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EXPIRES 06-30-25

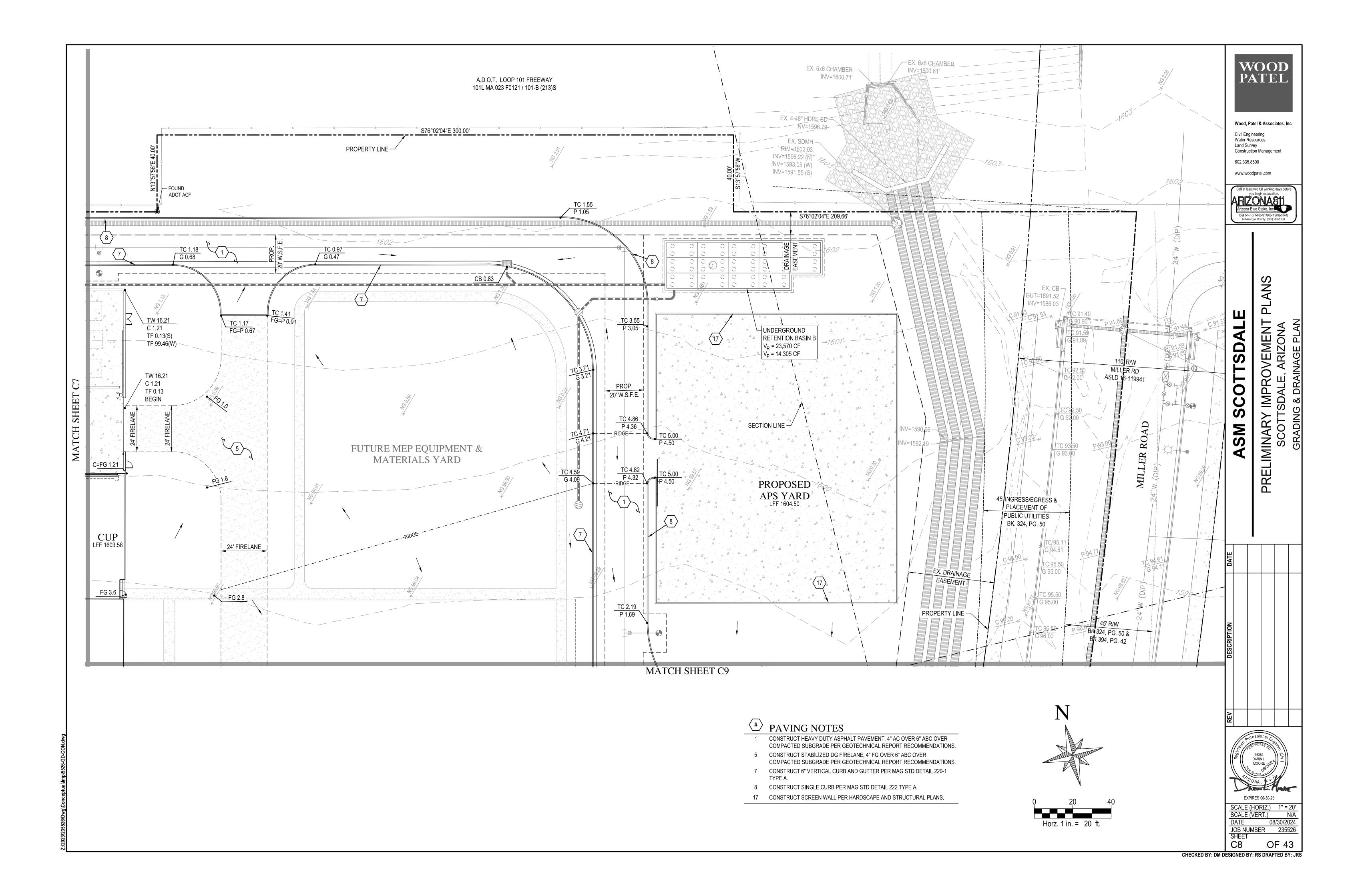
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 1" = 20'

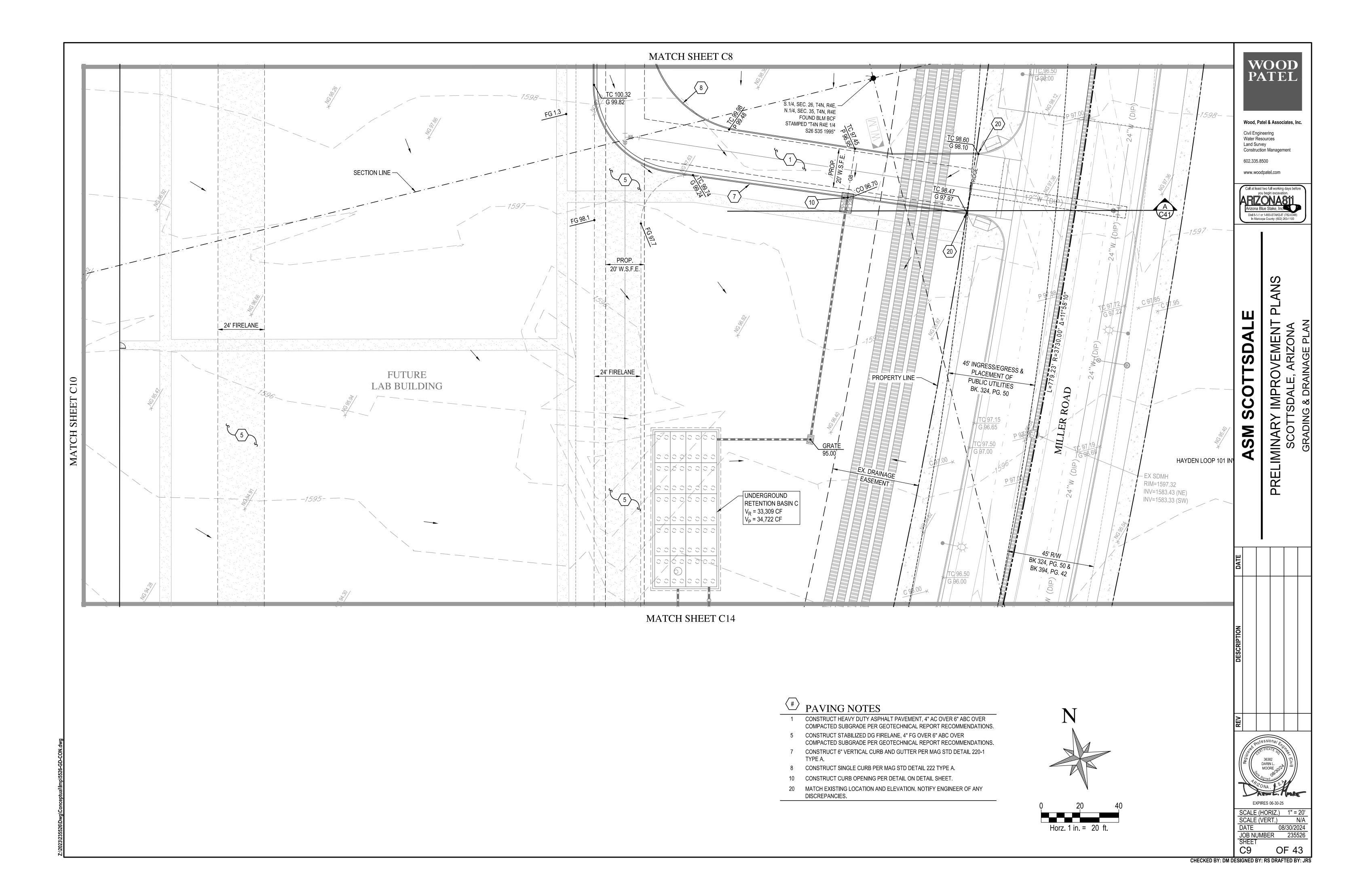
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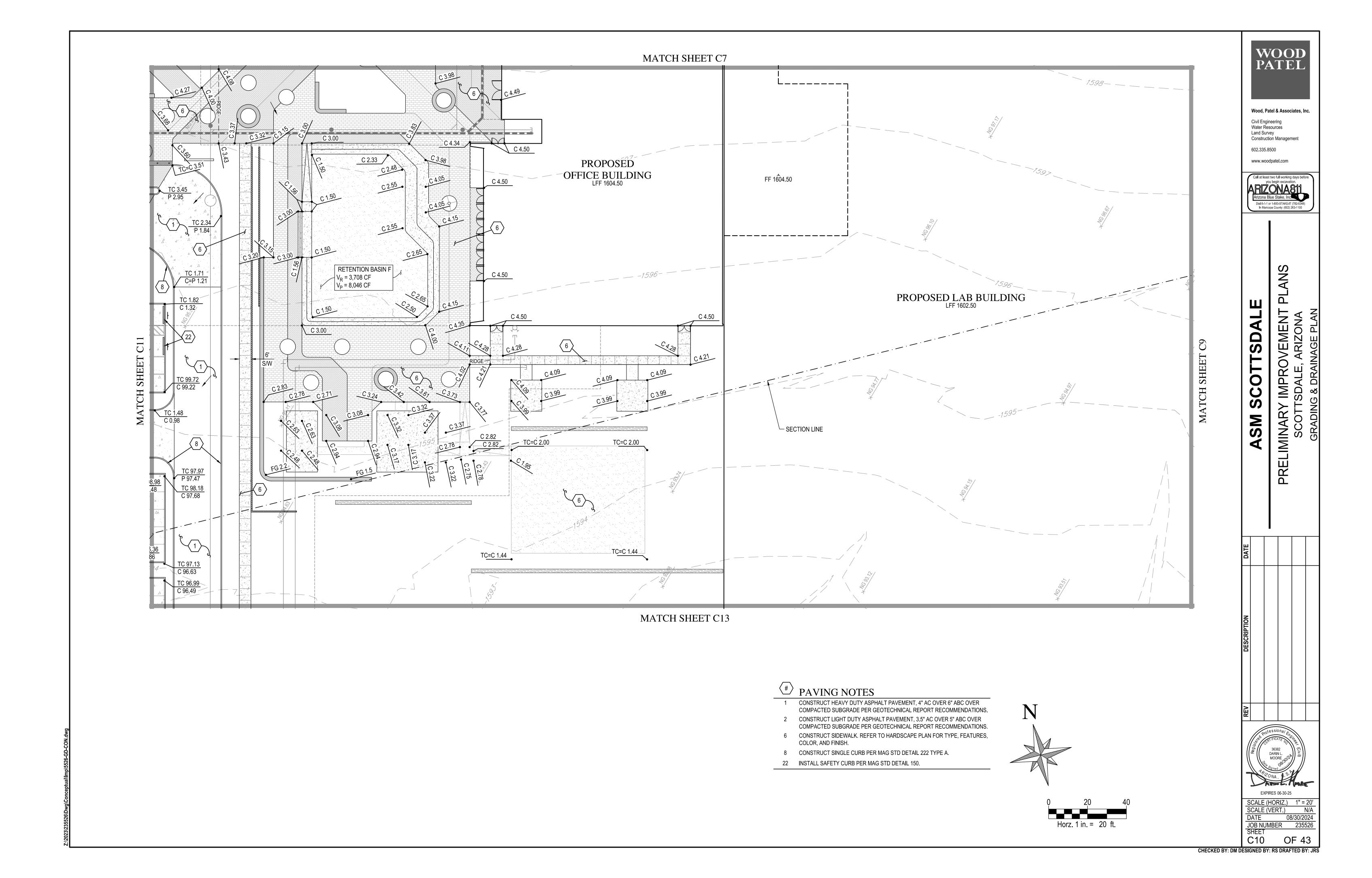
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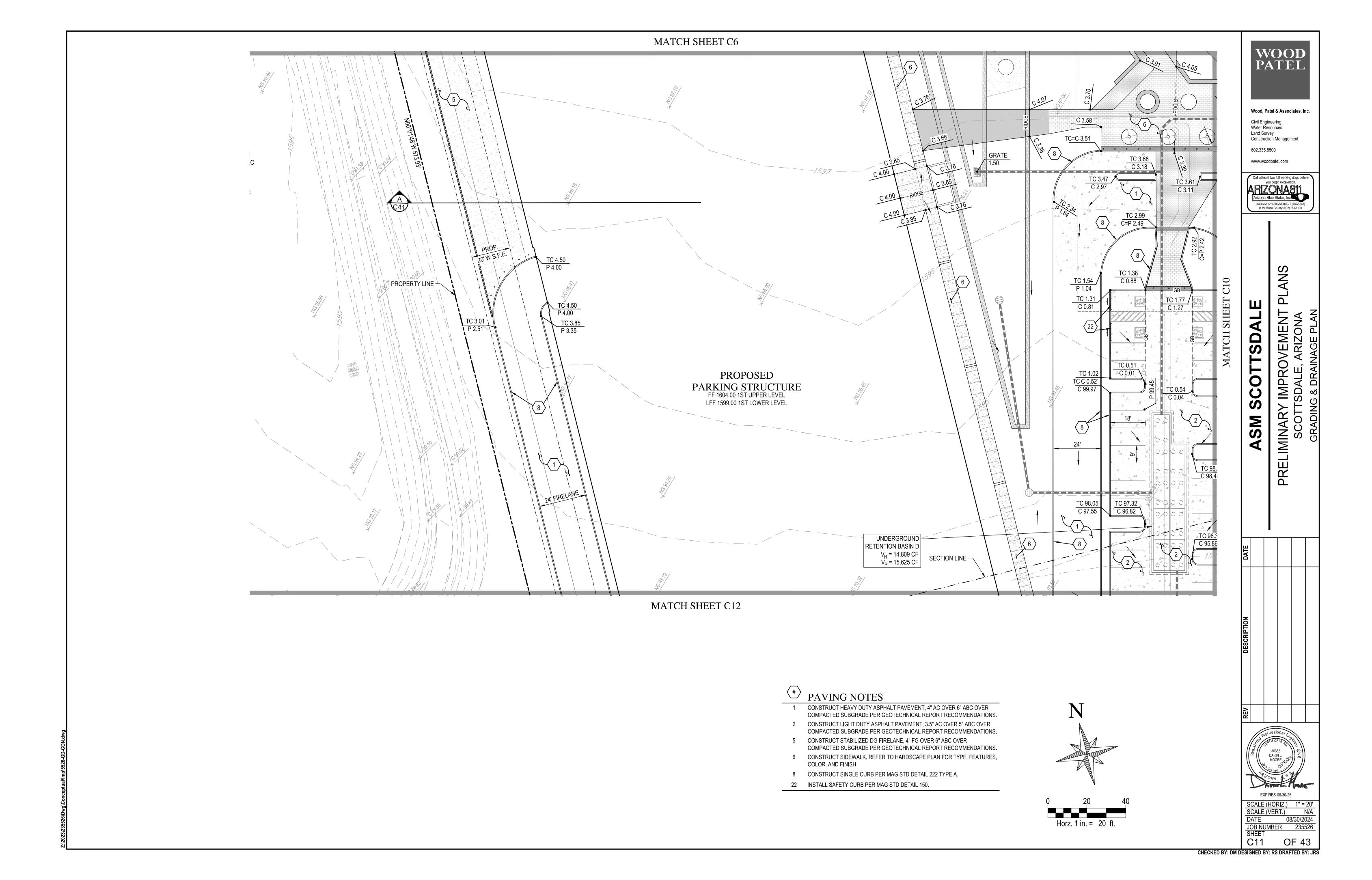
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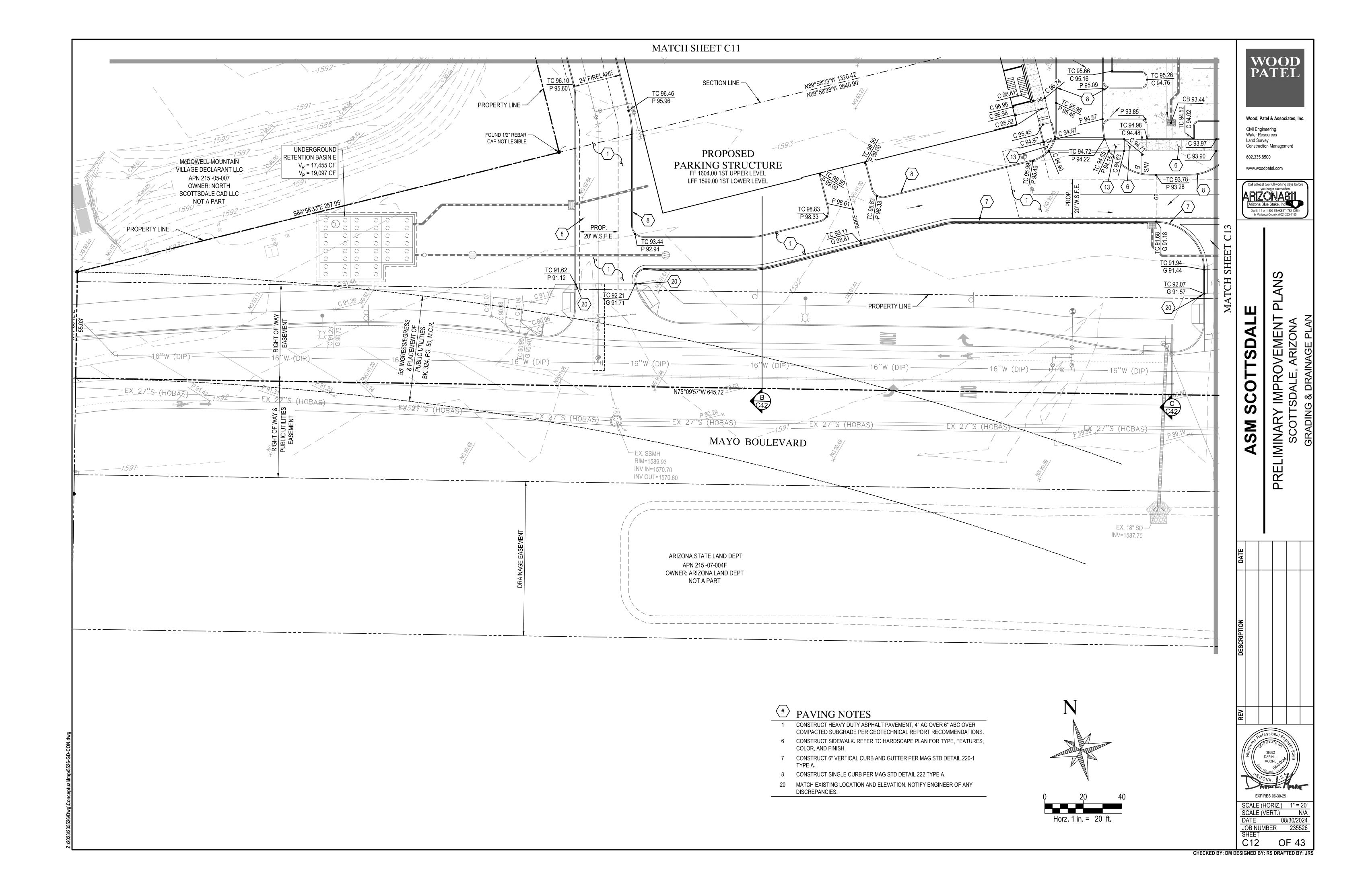
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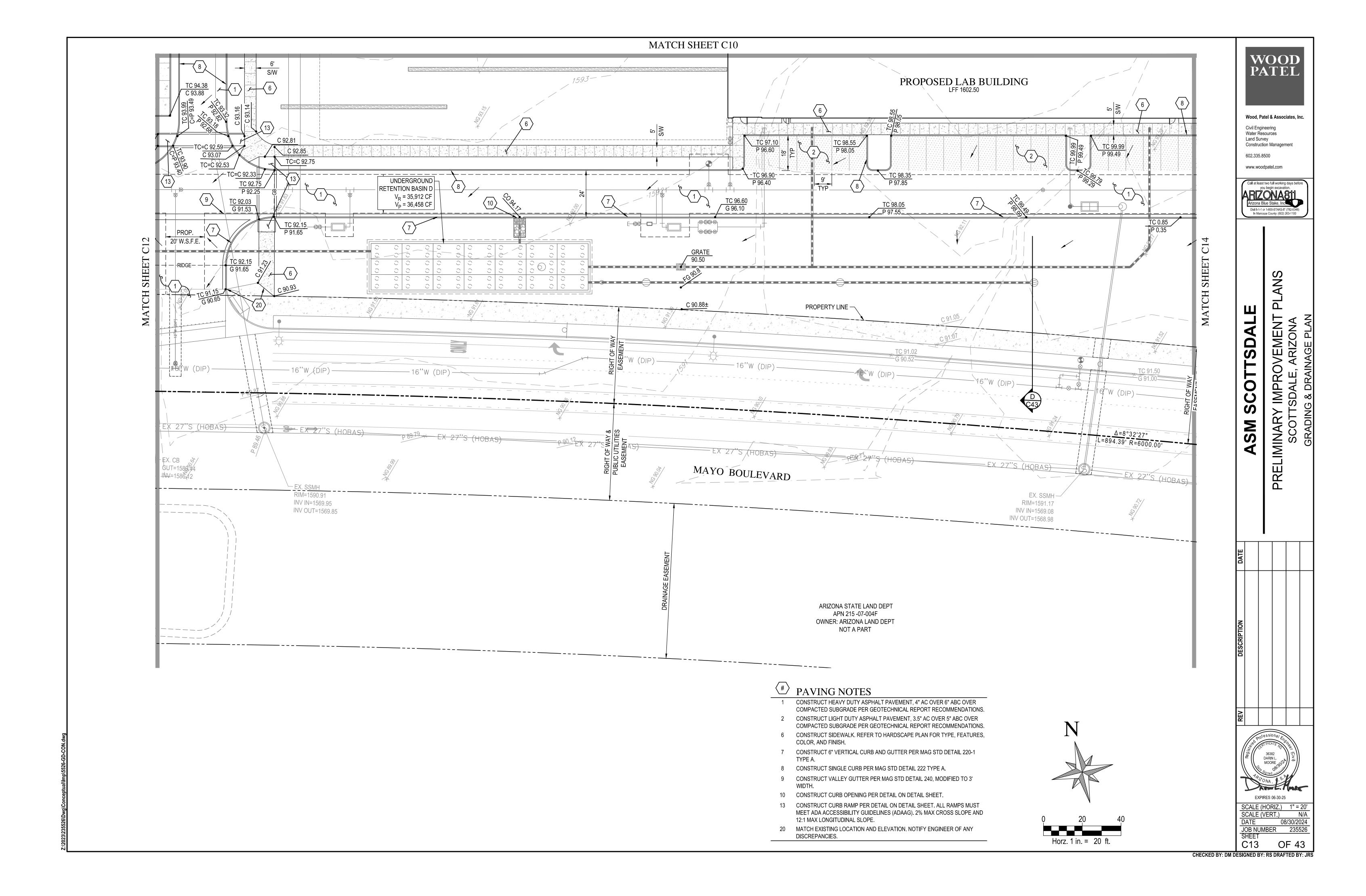


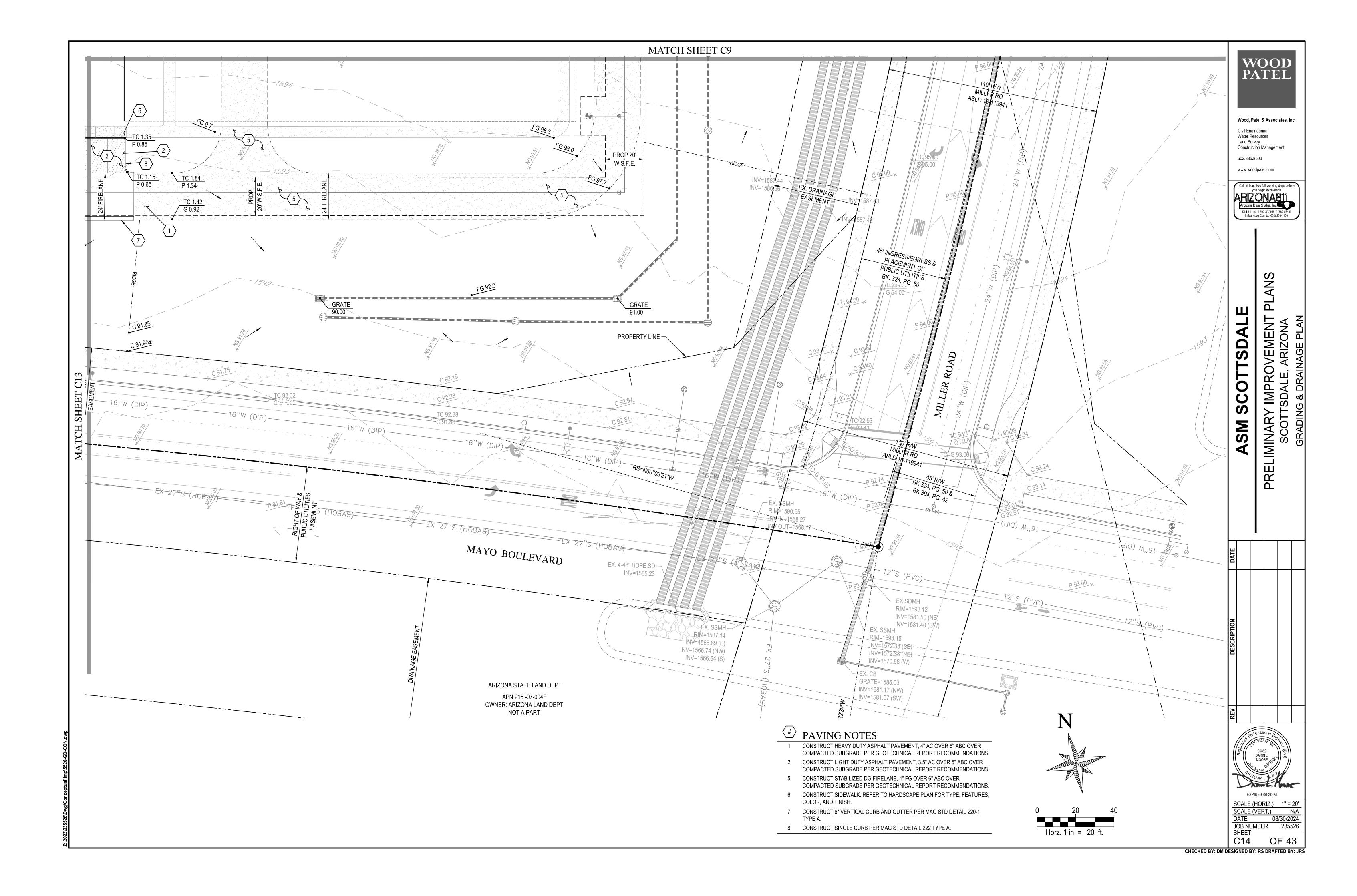


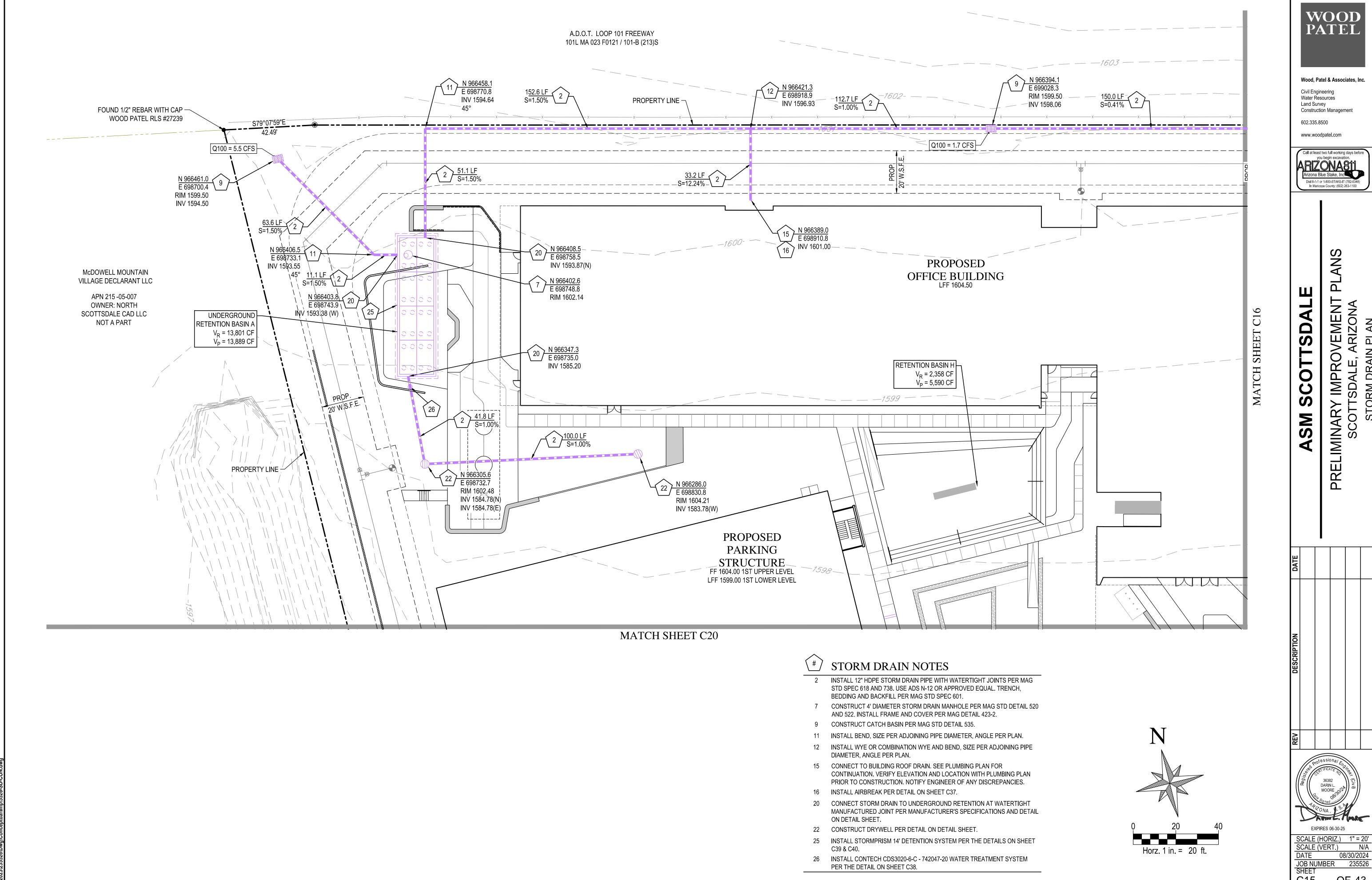




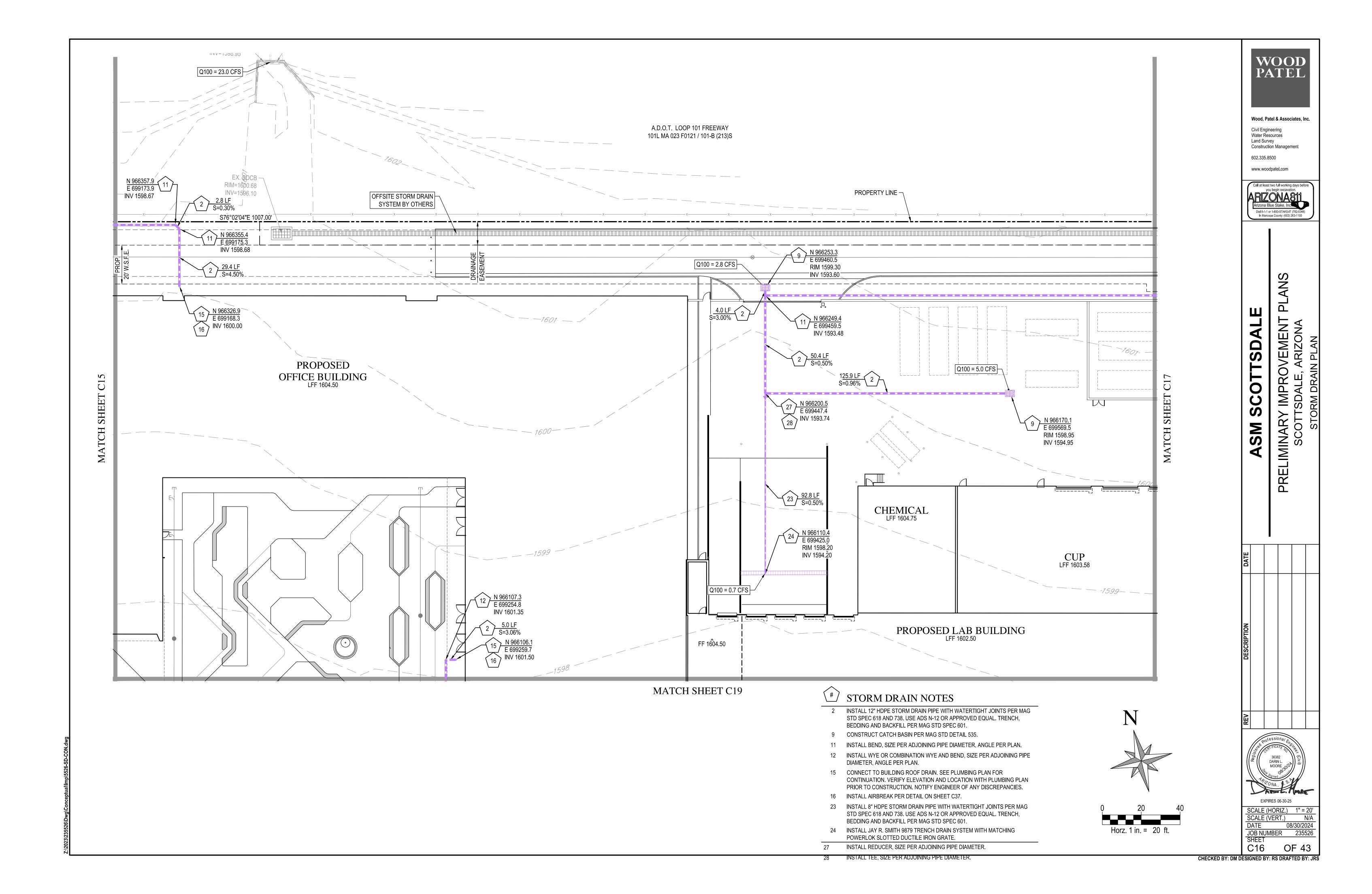


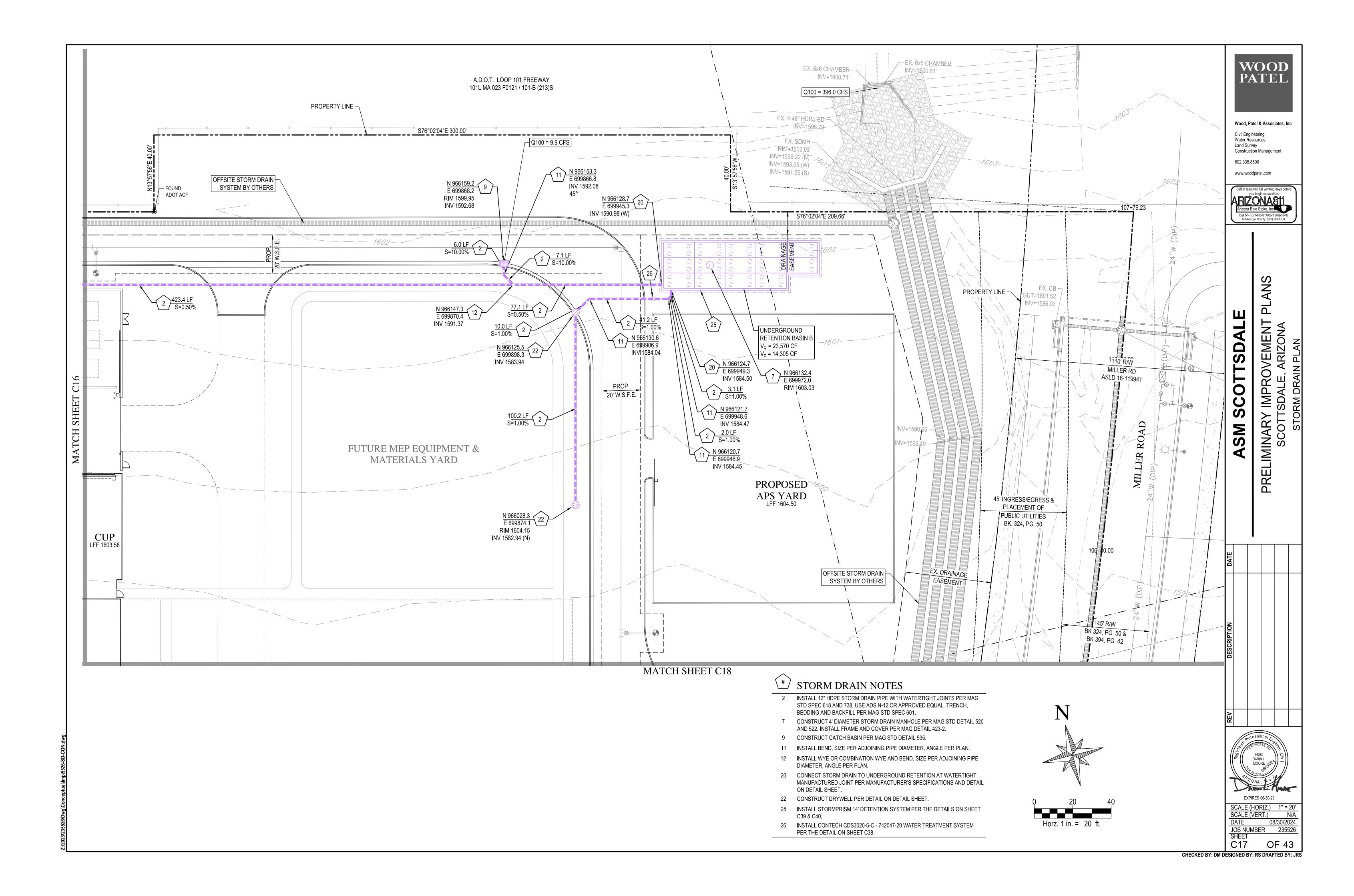


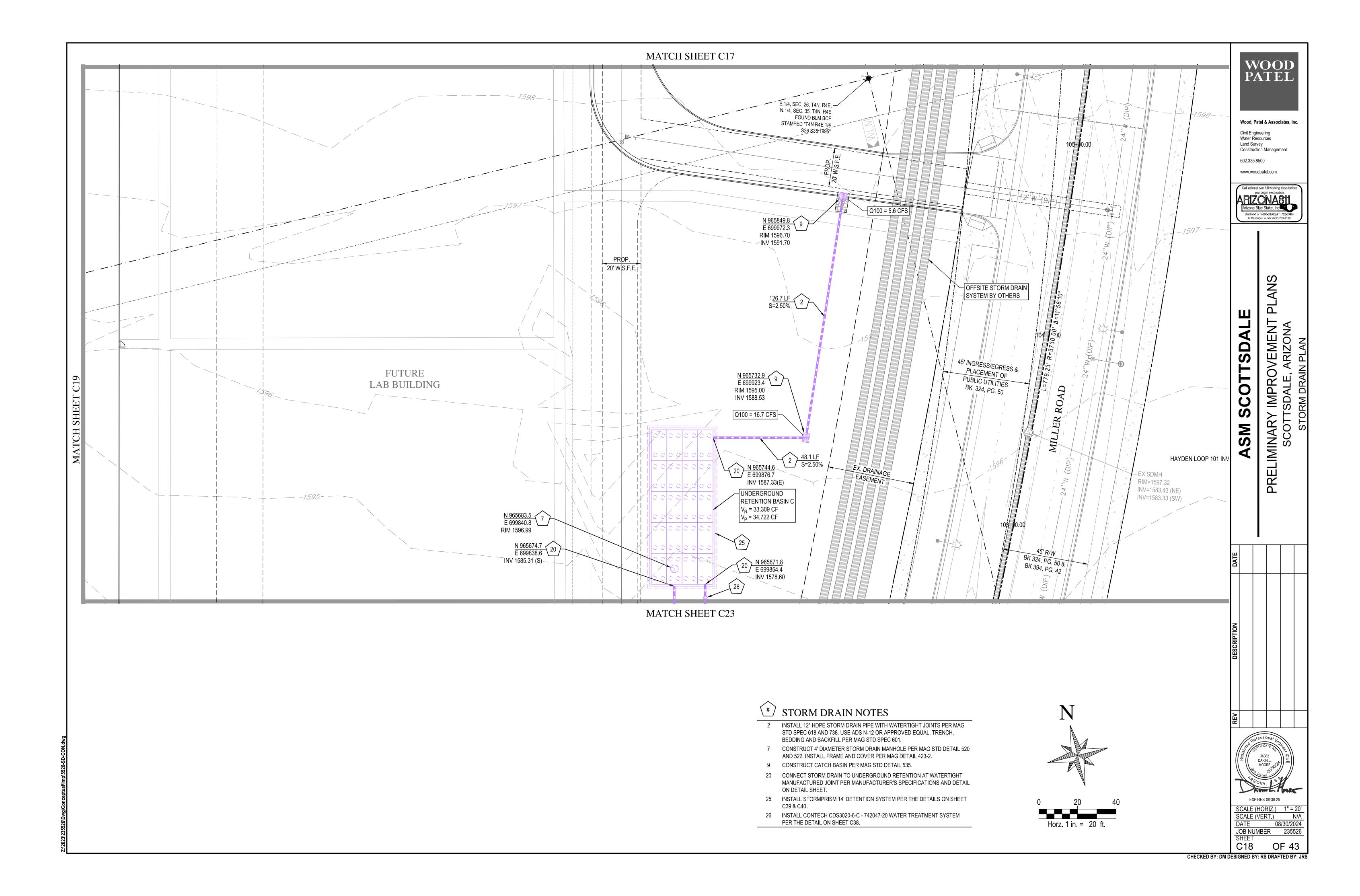


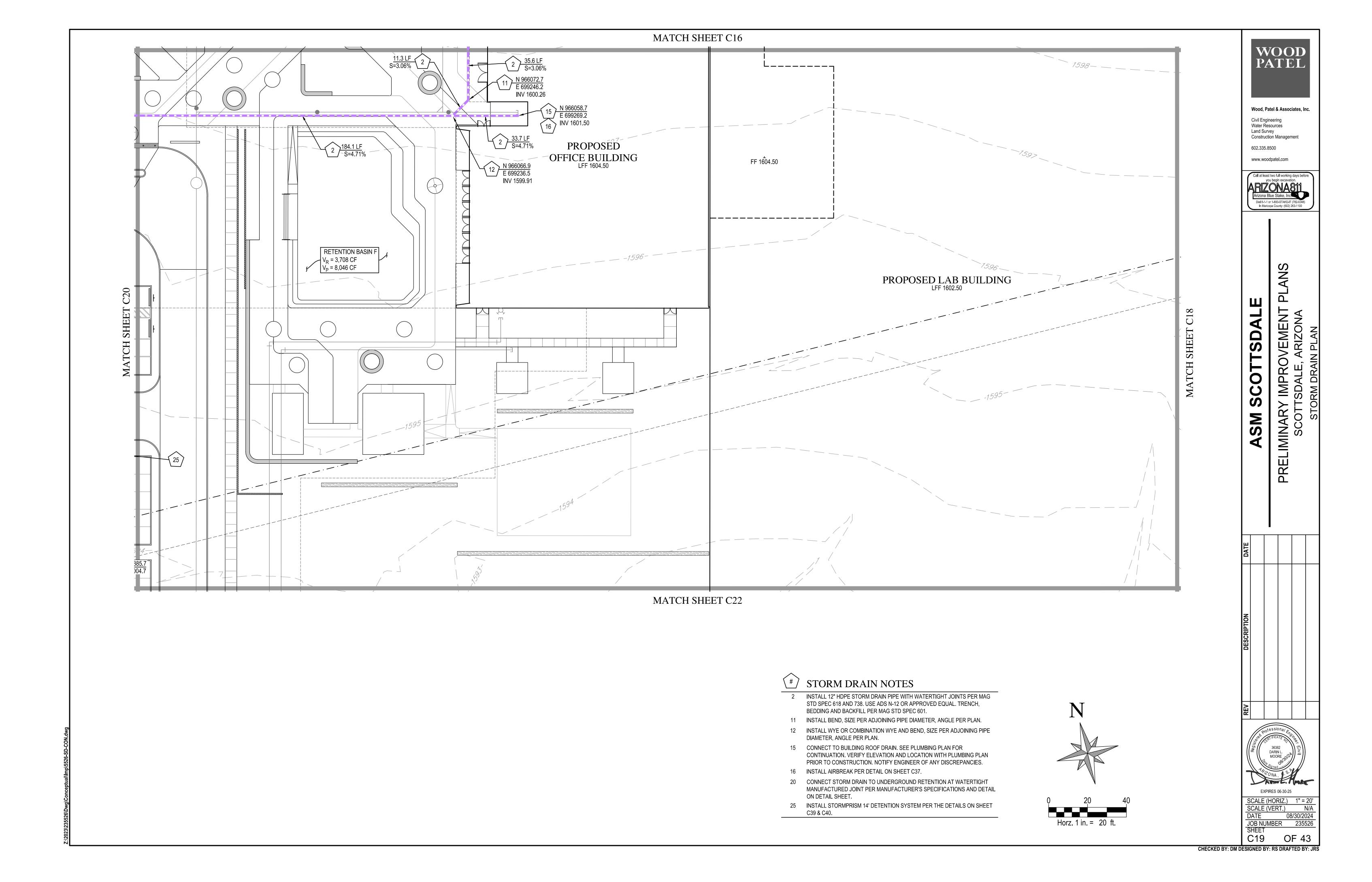


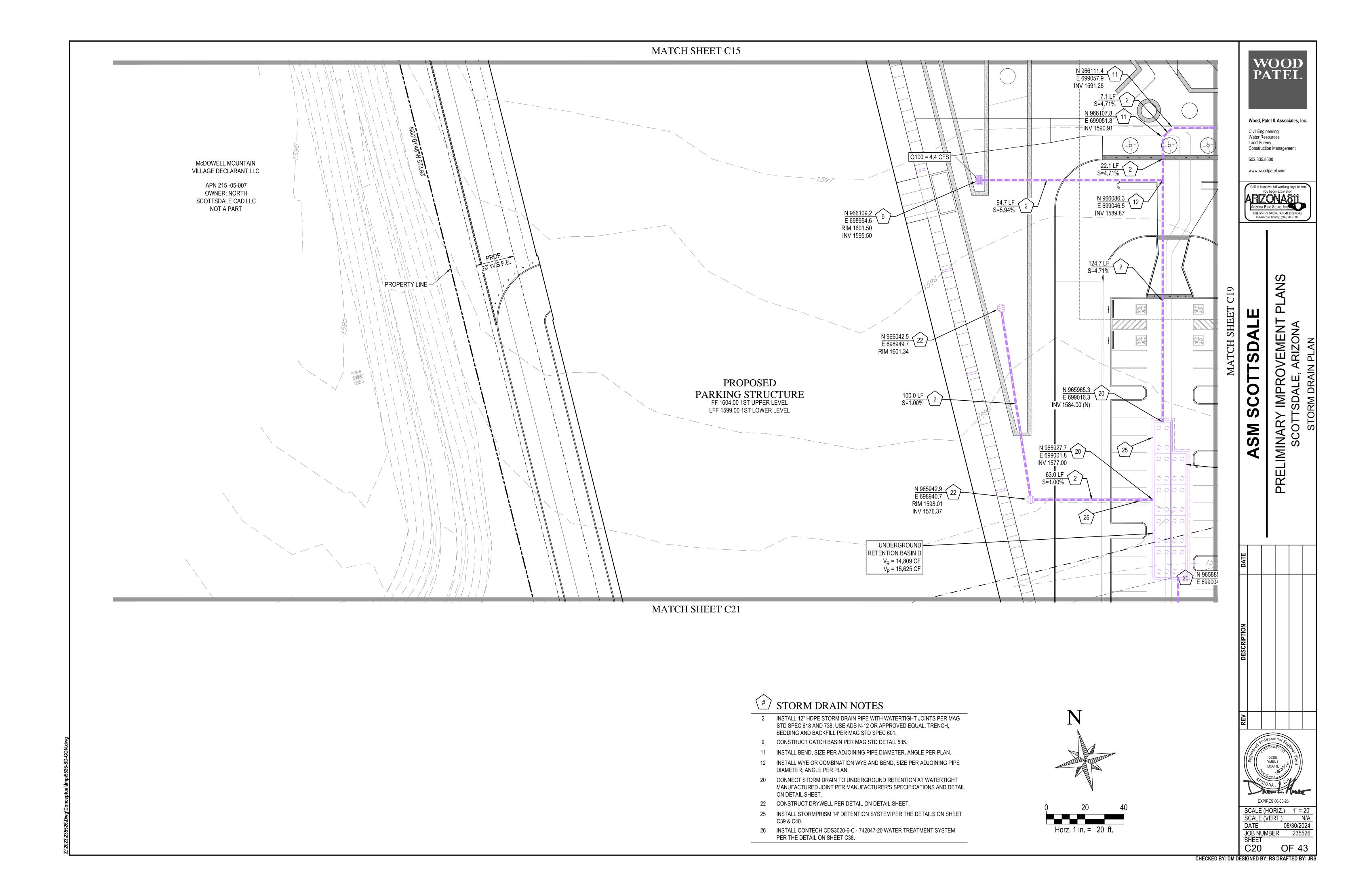
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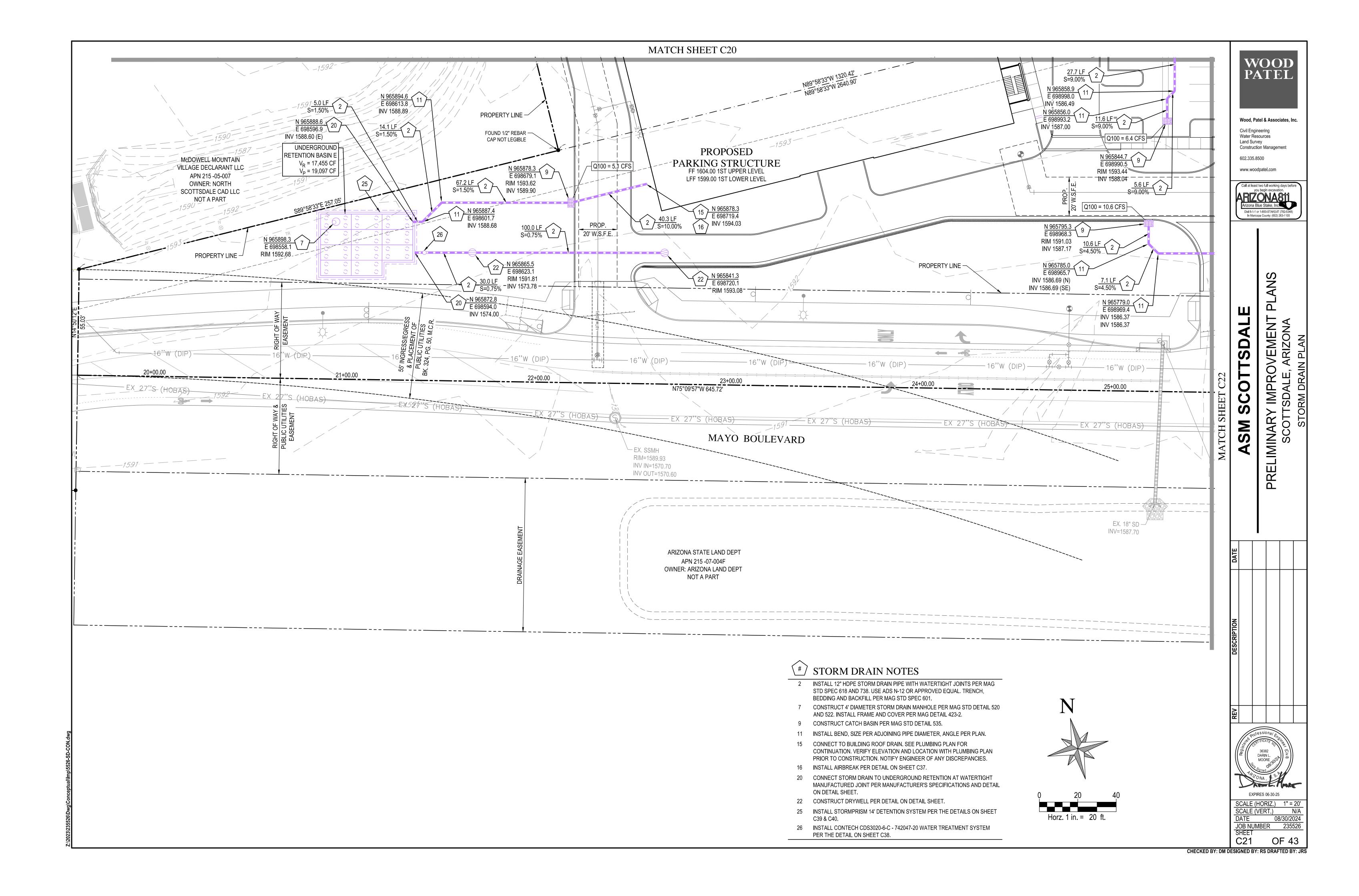


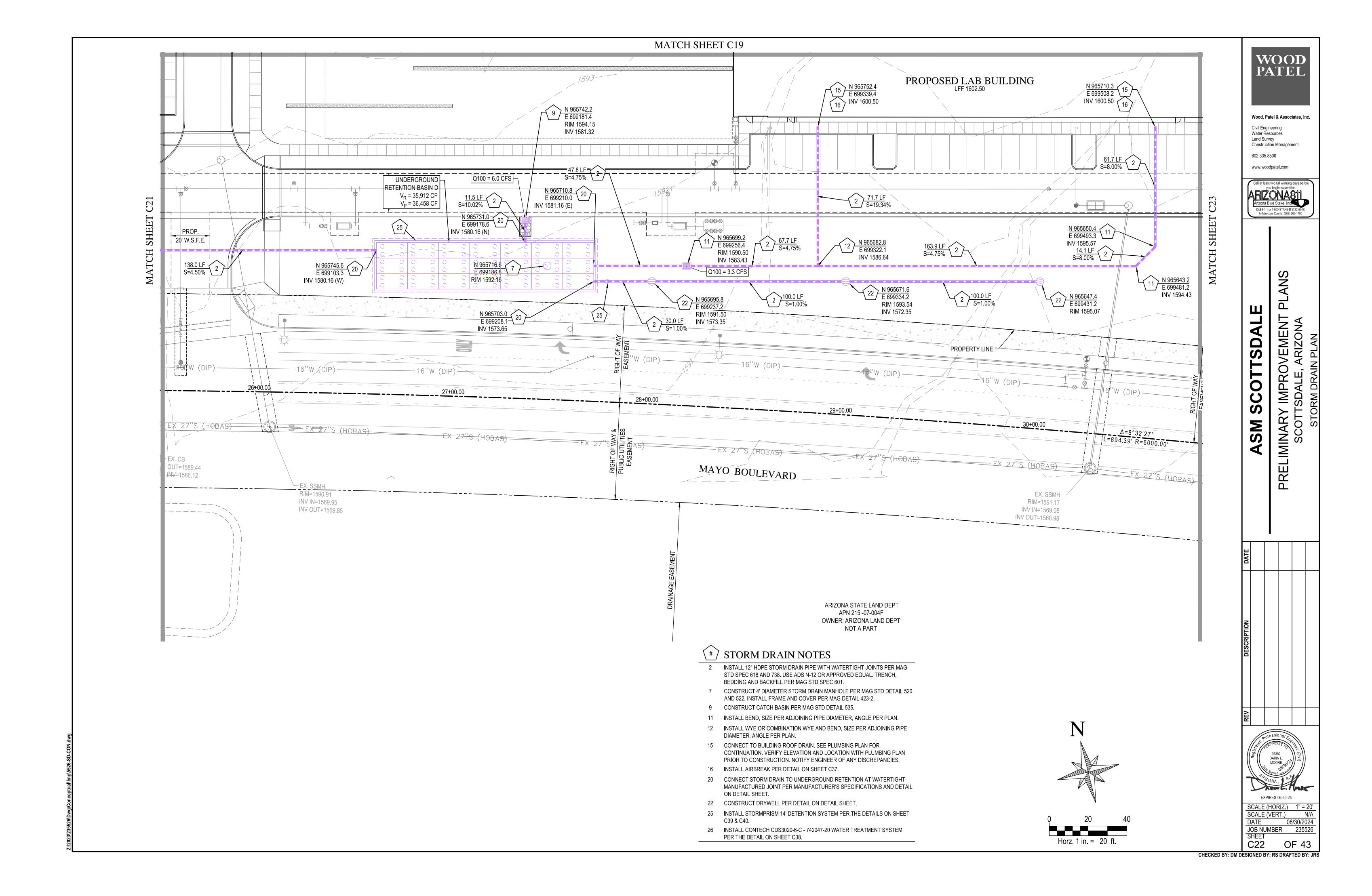


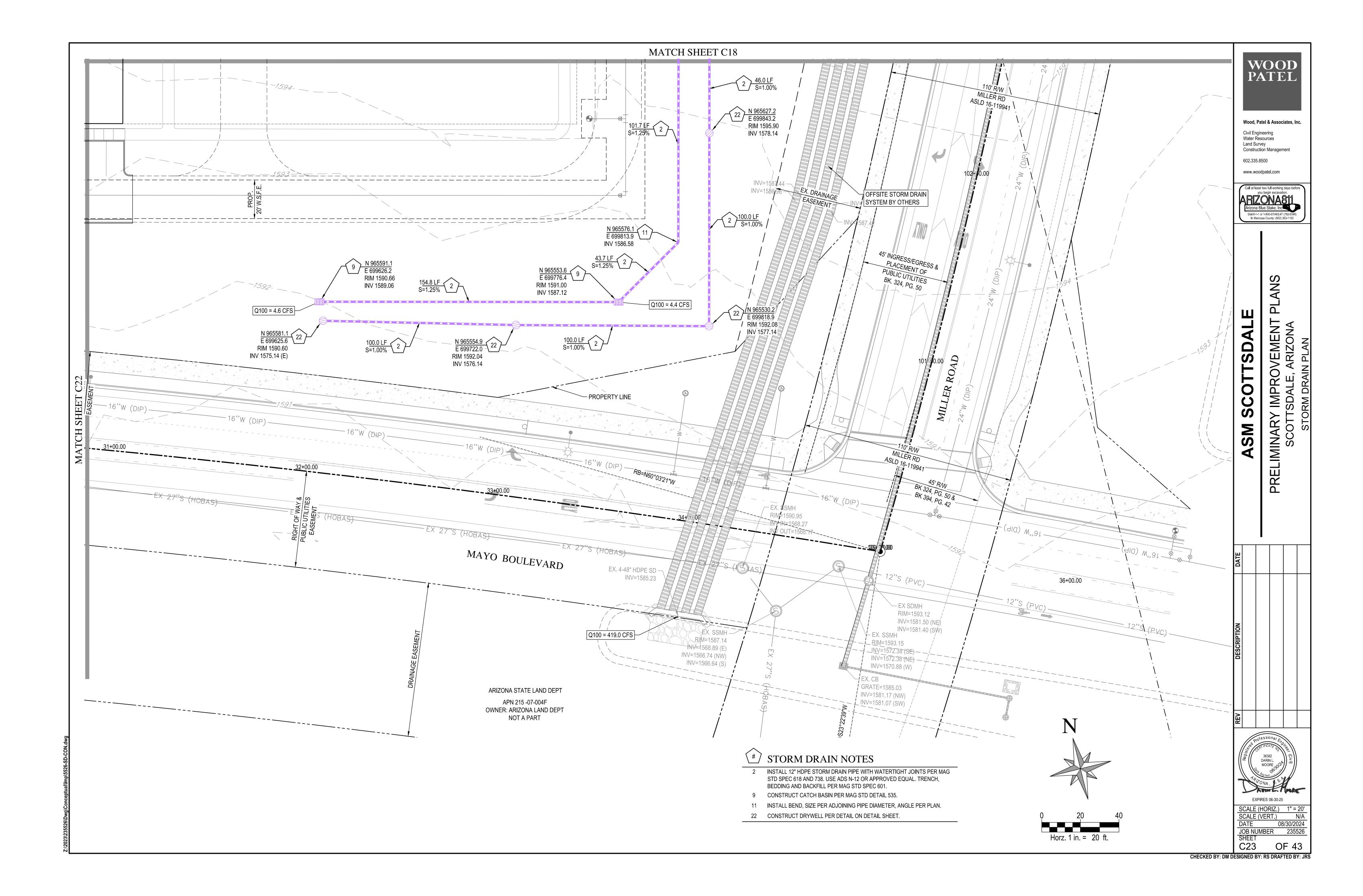


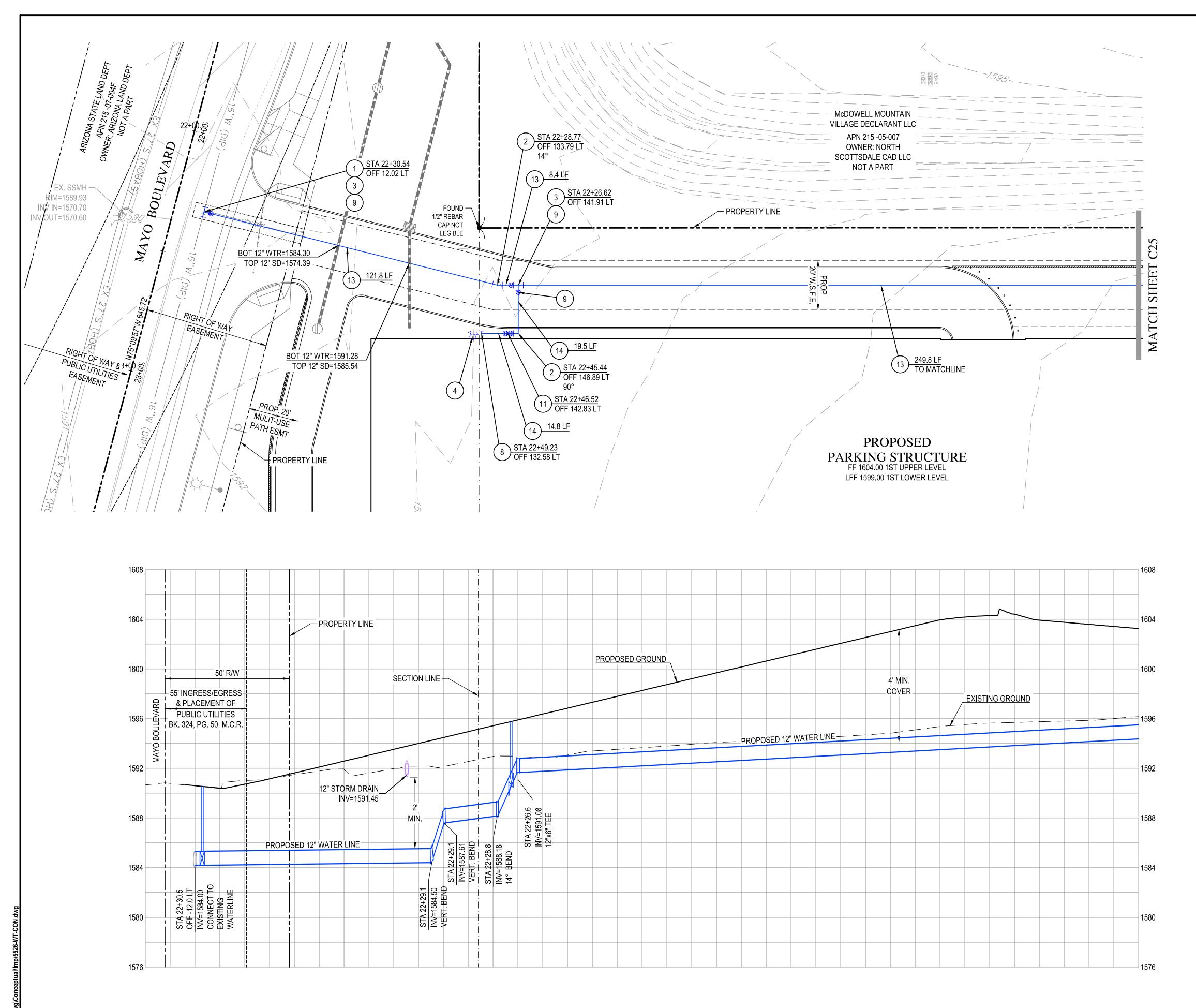


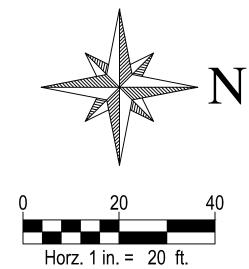












Vert. 1 in. = 4 ft.

WATER NOTES

- CUT AND CONNECT TO EXISTING WATER LINE. VERIFY ELEVATION AND LOCATION PRIOR TO CONSTRUCTION. NOTIFY ENGINEER OF ANY DISCREPANCIES.
- INSTALL BEND, SIZE PER ADJOINING PIPE DIAMETER, ANGLE PER PLAN.
- INSTALL TEE, SIZE PER ADJOINING PIPE DIAMETER.
- PROPOSED FIRE DEPARTMENT CONNECTION. FINAL LOCATION PER FIRE SPRINKLER PLAN.
- 8 CONNECT TO BUILDING FIRE SPRINKLER SERVICE. SEE FIRE SPRINKLER PLAN FOR CONTINUATION. VERIFY ELEVATION AND LOCATION WITH FIRE SPRINKLER PLAN PRIOR TO CONSTRUCTION. NOTIFY ENGINEER OF ANY
- INSTALL GATE VALVE PER MAG STD SPEC 630, SIZE PER ADJOINING PIPE DIAMETER. INSTALL BLOCKING PER MAG STD DETAIL 301. INSTALL VALVE BOX AND COVER AND ADJUST TO GRADE WITH CONCRETE COLLAR PER MAG STD DETAIL 391-1, TYPE C. INSTALL DEBRIS CAP PER MAG DTL 392.
- 11 INSTALL 6" DOUBLE CHECK VALVE BACKFLOW PREVENTION ASSEMBLY PER COS STD DETAIL 2351.
- 13 INSTALL 12" POLYWRAPPED DIP PRESSURE CLASS 350 WATERLINE WITH 12 AWG TRACER WIRE PER MAG STD SPEC 610 AND 750. 4' MINIMUM COVER. INSTALL MECHANICAL JOINT RESTRAINT PER MAG STD DETAIL 303. TRENCH BEDDING AND BACKFILL PER MAG STD SPEC 601.
- 14 INSTALL 6" POLYWRAPPED DIP PRESSURE CLASS 350 WATERLINE WITH 12 AWG TRACER WIRE PER MAG STD SPEC 610 AND 750. 3' MINIMUM COVER, 4' MINIMUM COVER WITHIN RIGHT-OF-WAY. INSTALL MECHANICAL JOINT RESTRAINT PER MAG STD DETAIL 303. TRENCH, BEDDING AND BACKFILL PER MAG STD SPEC 601.



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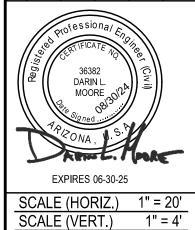
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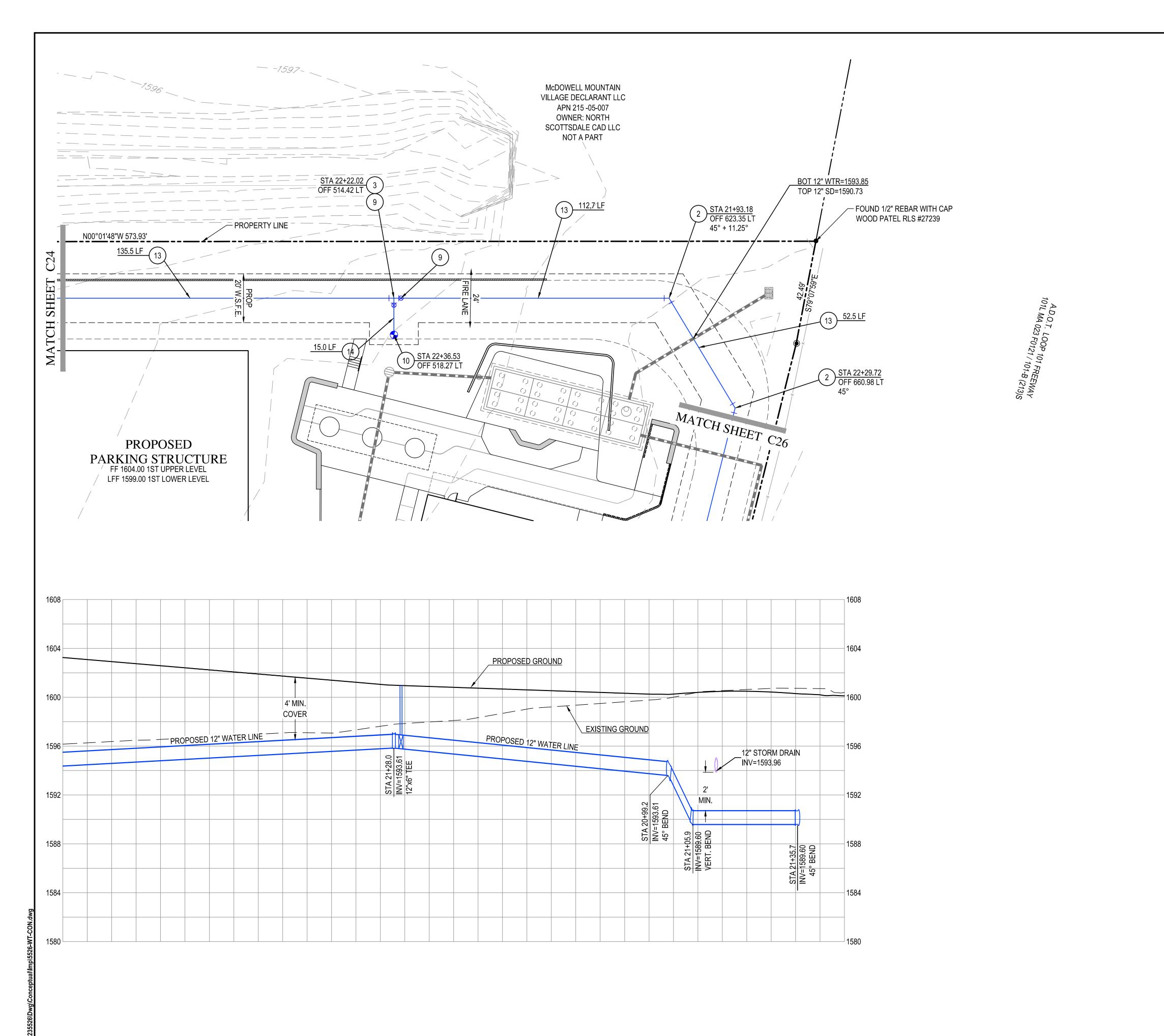


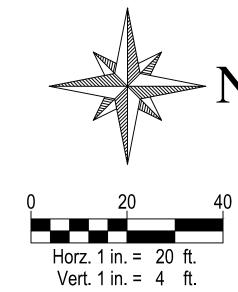
-IMINARY IMPROVEMENT PLANS SCOTTSDALE, ARIZONA SCOTTSDALE ASM

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SCALE (VERT.) 1" = 4'
DATE 08/30/2024 JOB NUMBER 235526 SHEET C24 OF 43





WATER NOTES

- INSTALL BEND, SIZE PER ADJOINING PIPE DIAMETER, ANGLE PER PLAN.
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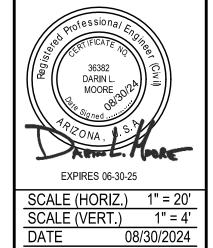
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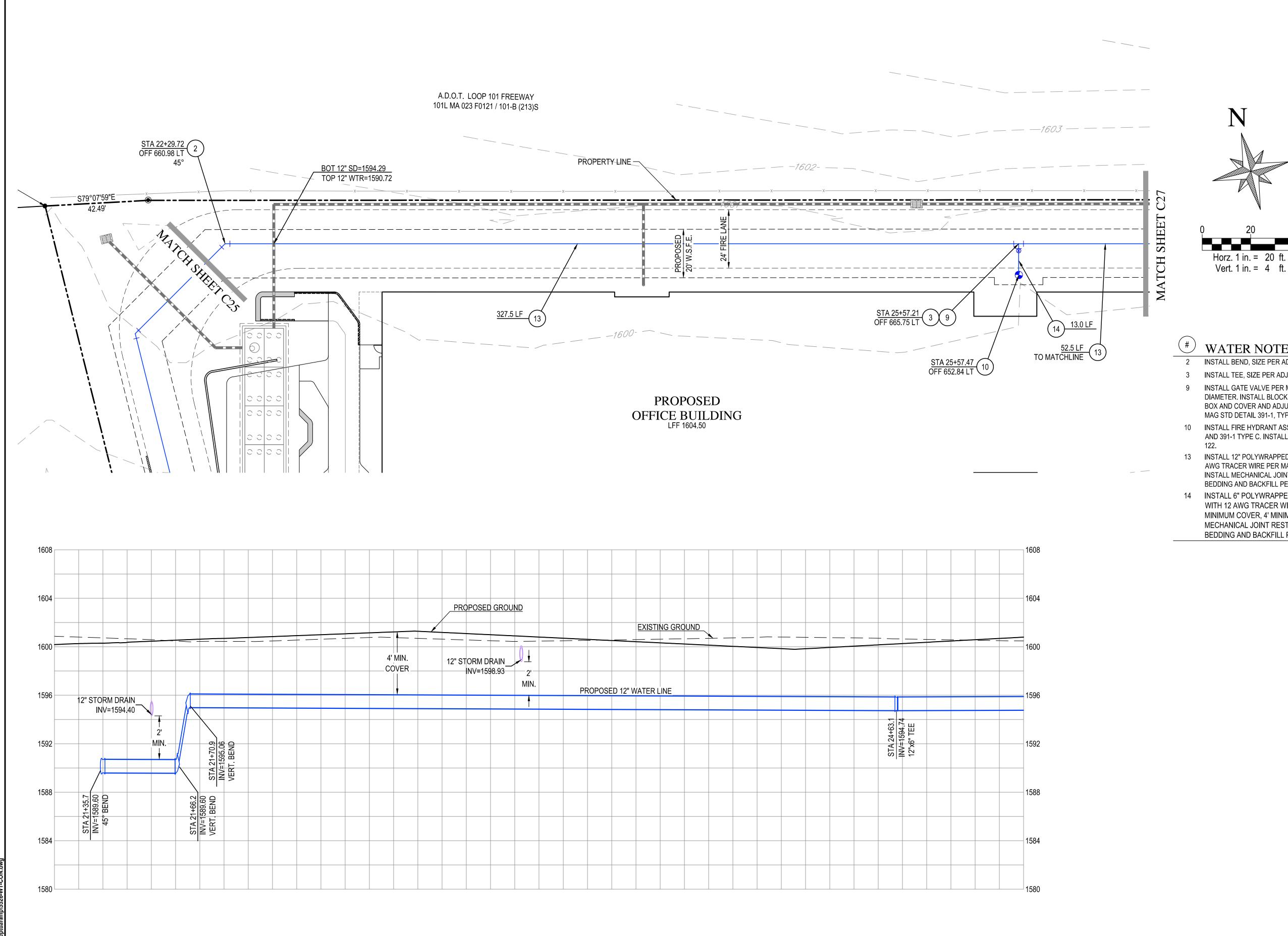
Call at least two full working days before you begin excavation. ARIZONA811
Arizona Blue Stake, Inc.

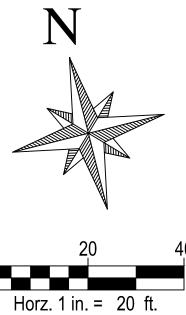
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WATER NOTES

2 INSTALL BEND, SIZE PER ADJOINING PIPE DIAMETER, ANGLE PER PLAN.

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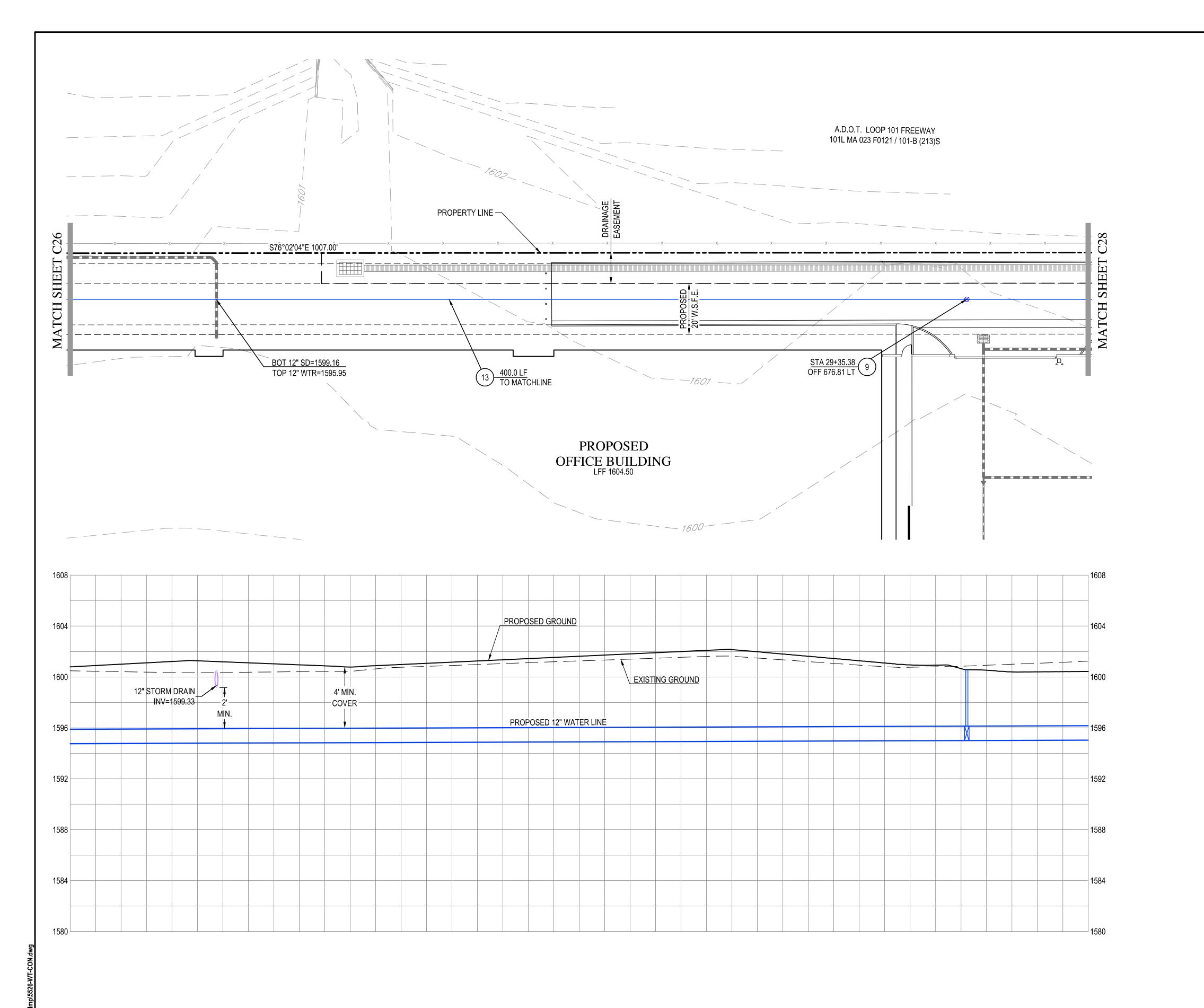
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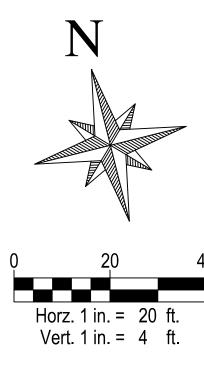


SCALE (HORIZ.) 1" = 20' SCALE (VERT.) 1" = 4' DATE 08/30/2024
 JOB NUMBER
 235526

 SHEET
 C26
 OF 43

CHECKED BY: DM DESIGNED BY: RS DRAFTED BY: JRS





WATER NOTES

- INSTALL GATE VALVE PER MAG STD SPEC 630, SIZE PER ADJOINING PIPE DIAMETER. INSTALL BLOCKING PER MAG STD DETAIL 301. INSTALL VALVE BOX AND COVER AND ADJUST TO GRADE WITH CONCRETE COLLAR PER MAG STD DETAIL 391-1, TYPE C. INSTALL DEBRIS CAP PER MAG DTL 392.
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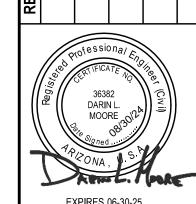
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SCOTTSDALE

-IMINARY IMPROVEMENT PLANS SCOTTSDALE, ARIZONA

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EXPIRES 06-30-25

 SCALE (HORIZ.)
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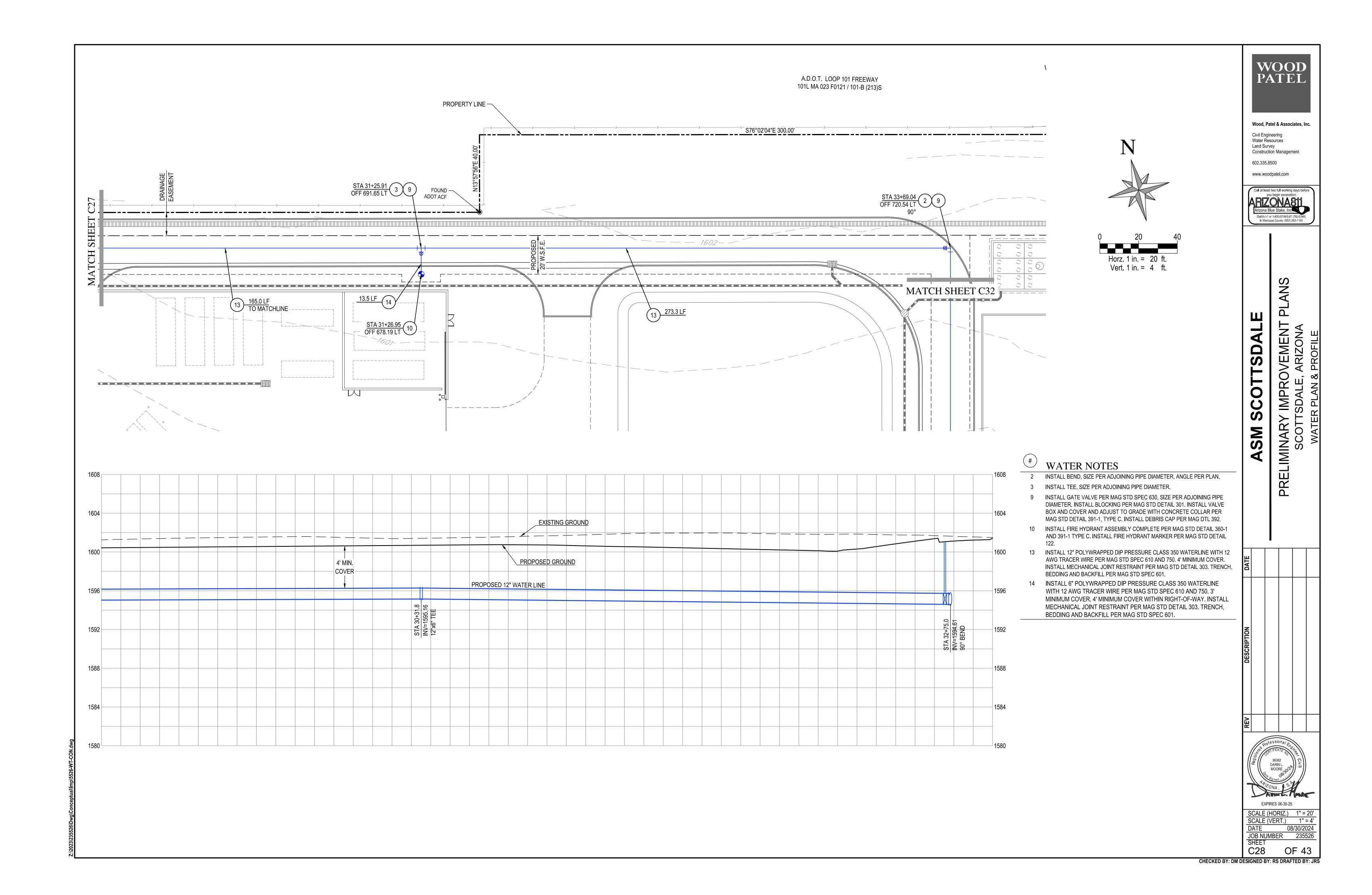
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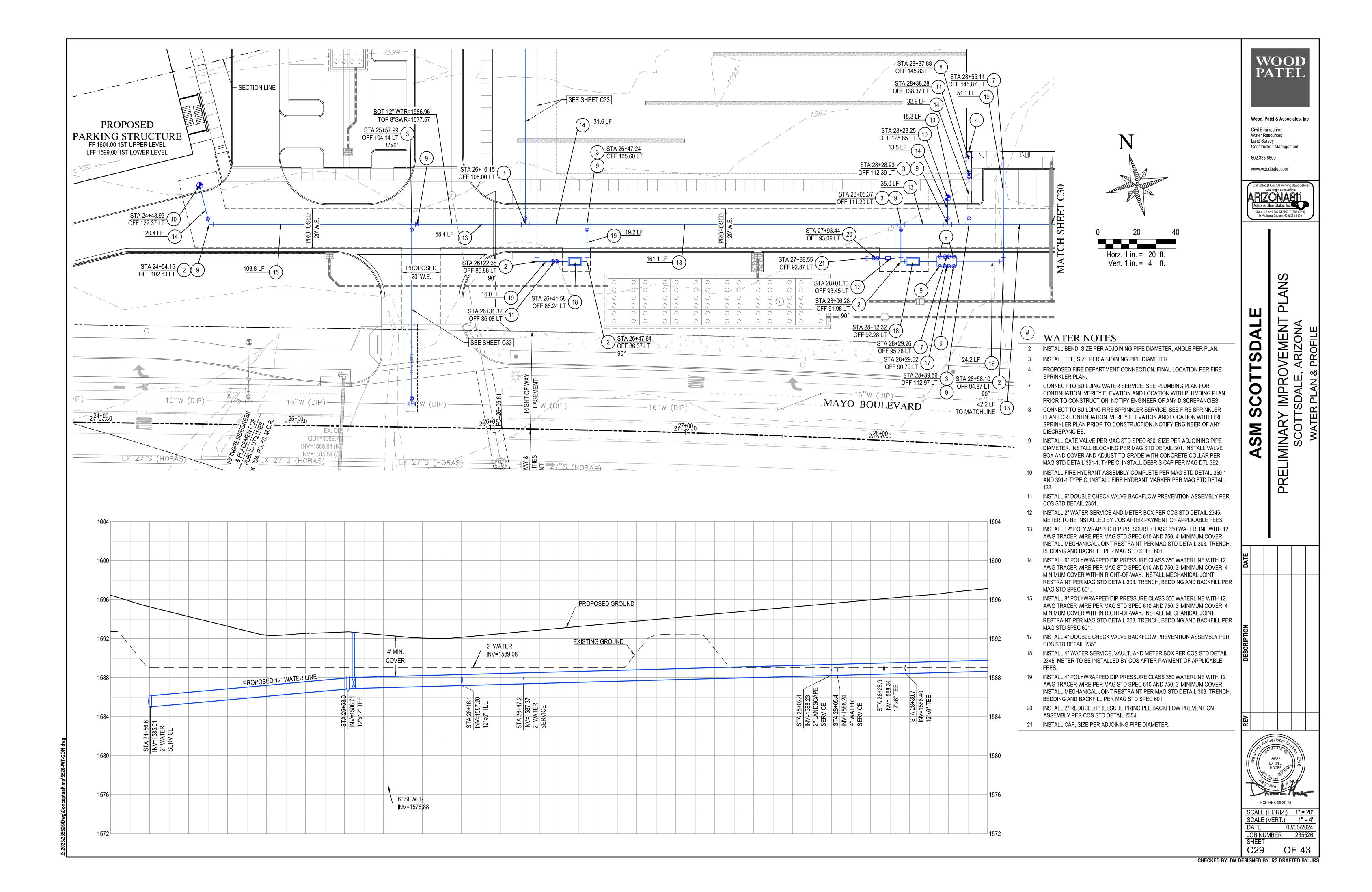
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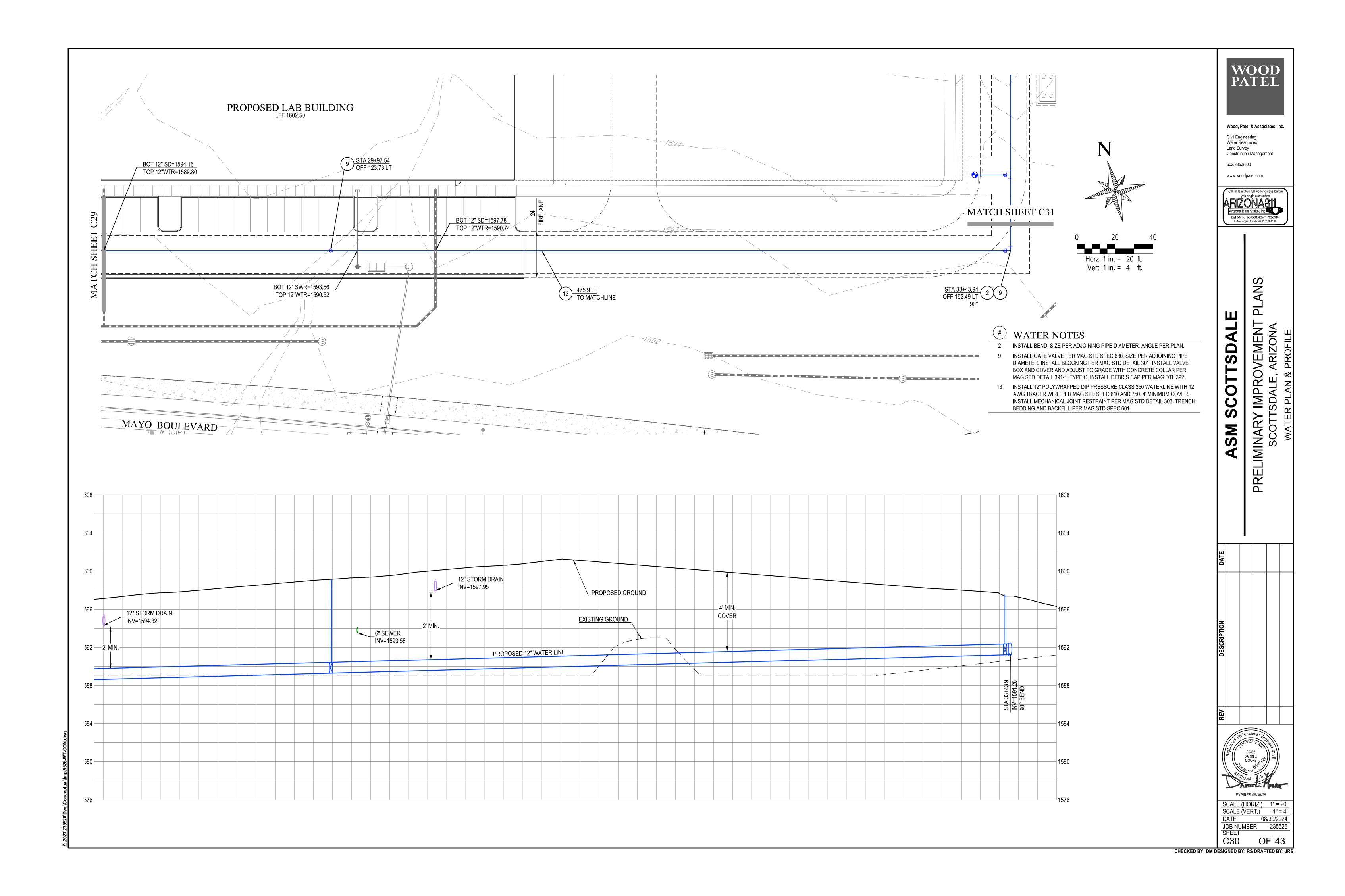
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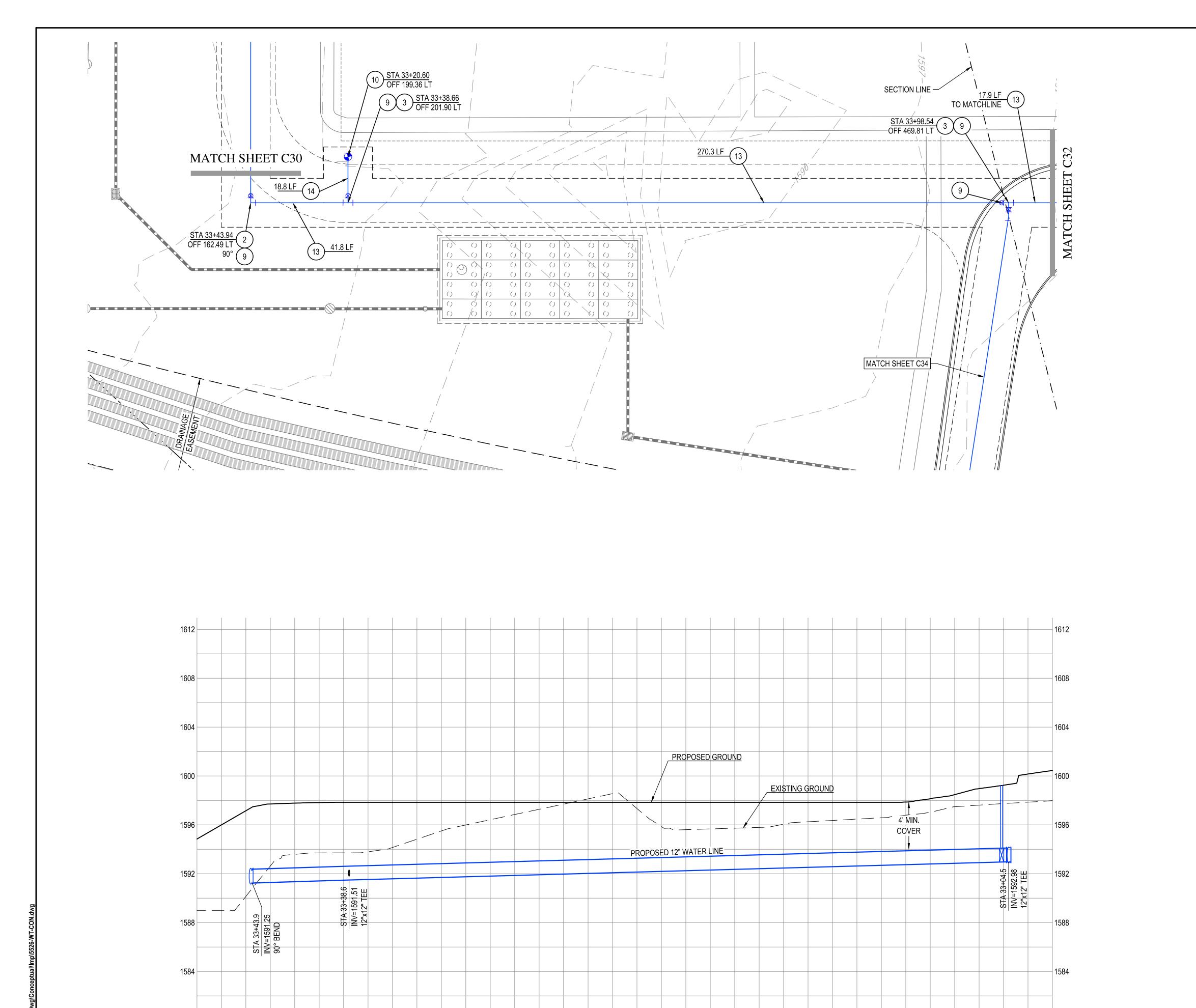
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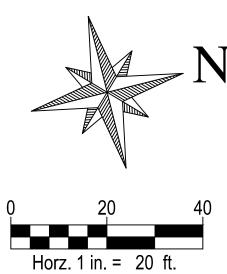
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INSTALL BEND, SIZE PER ADJOINING PIPE DIAMETER, ANGLE PER PLAN.

Vert. 1 in. = 4 ft.

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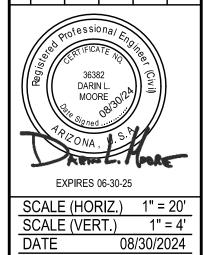


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Arizona Blue Stake, Inc. Dial 8-1-1 or 1-800-STAKE-IT (782-5348 In Maricopa County: (602) 263-1100

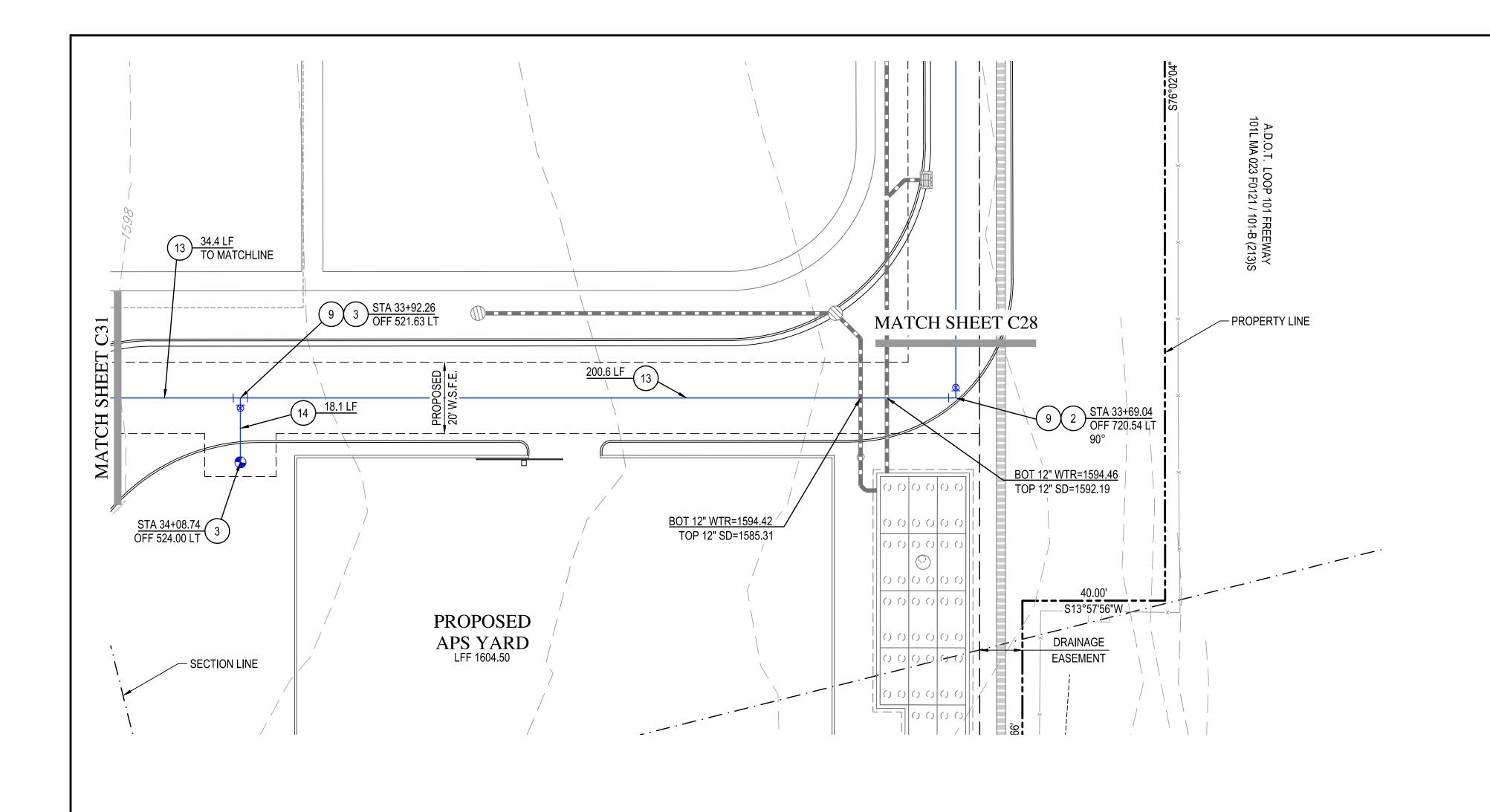
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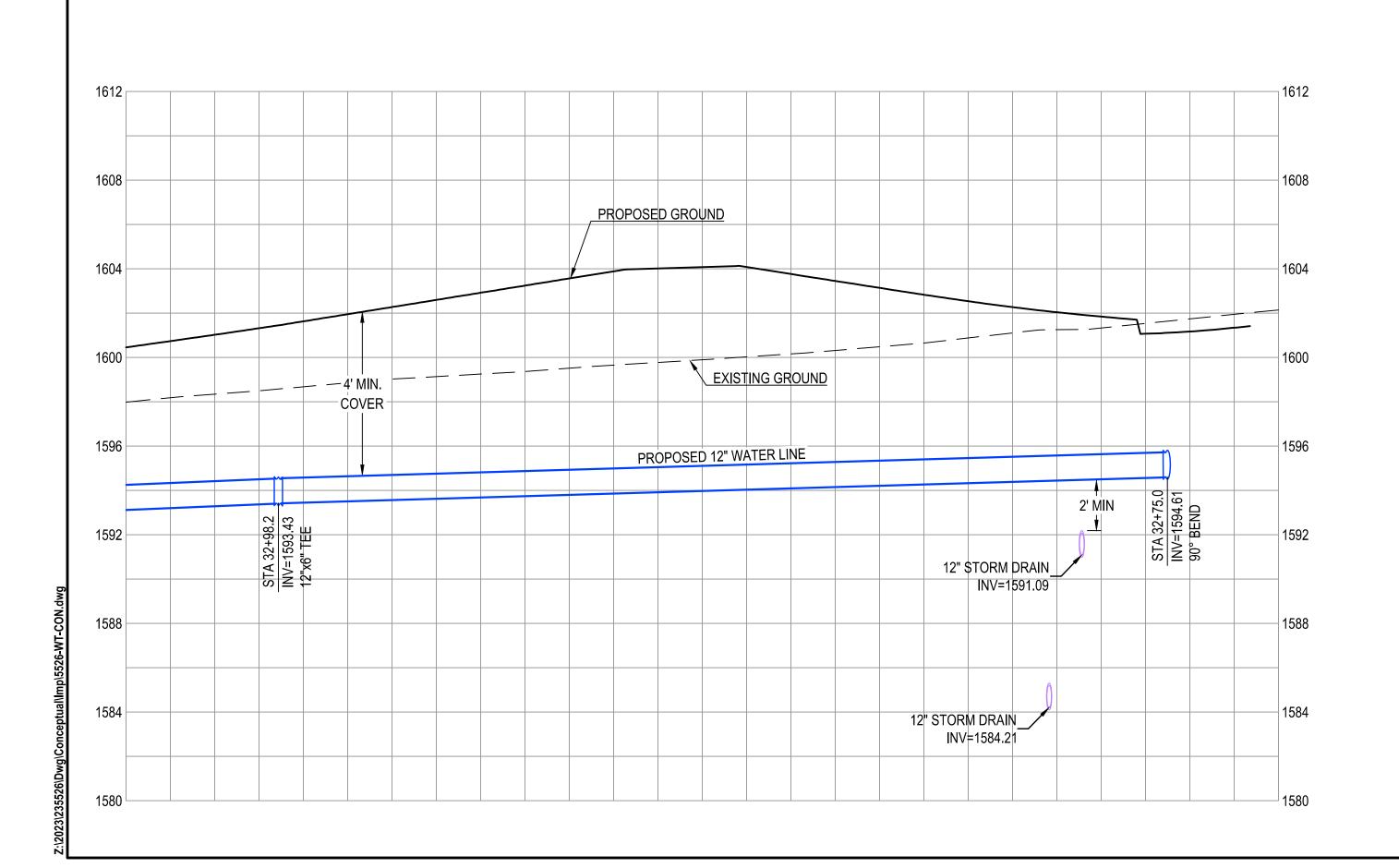
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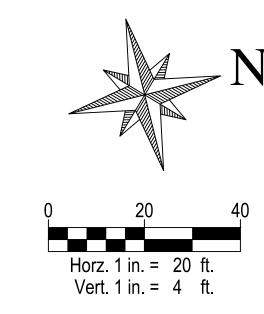
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JOB NUMBER 235526 SHEET C31 OF 43







WATER NOTES

- INSTALL BEND, SIZE PER ADJOINING PIPE DIAMETER, ANGLE PER PLAN.
- 3 INSTALL TEE, SIZE PER ADJOINING PIPE DIAMETER.
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Land Survey

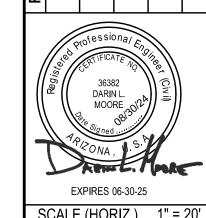
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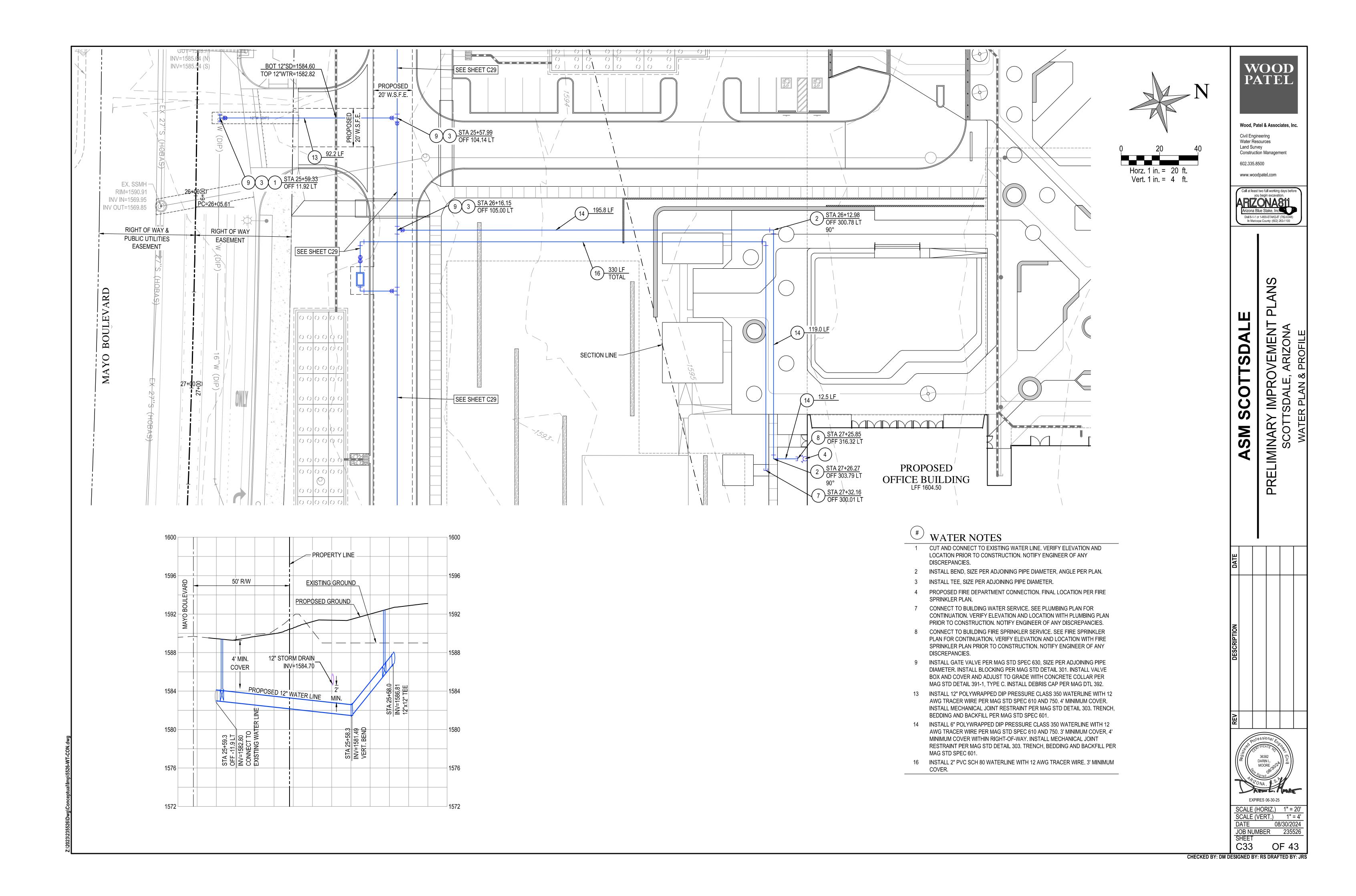
ASM SCOTTSDALE

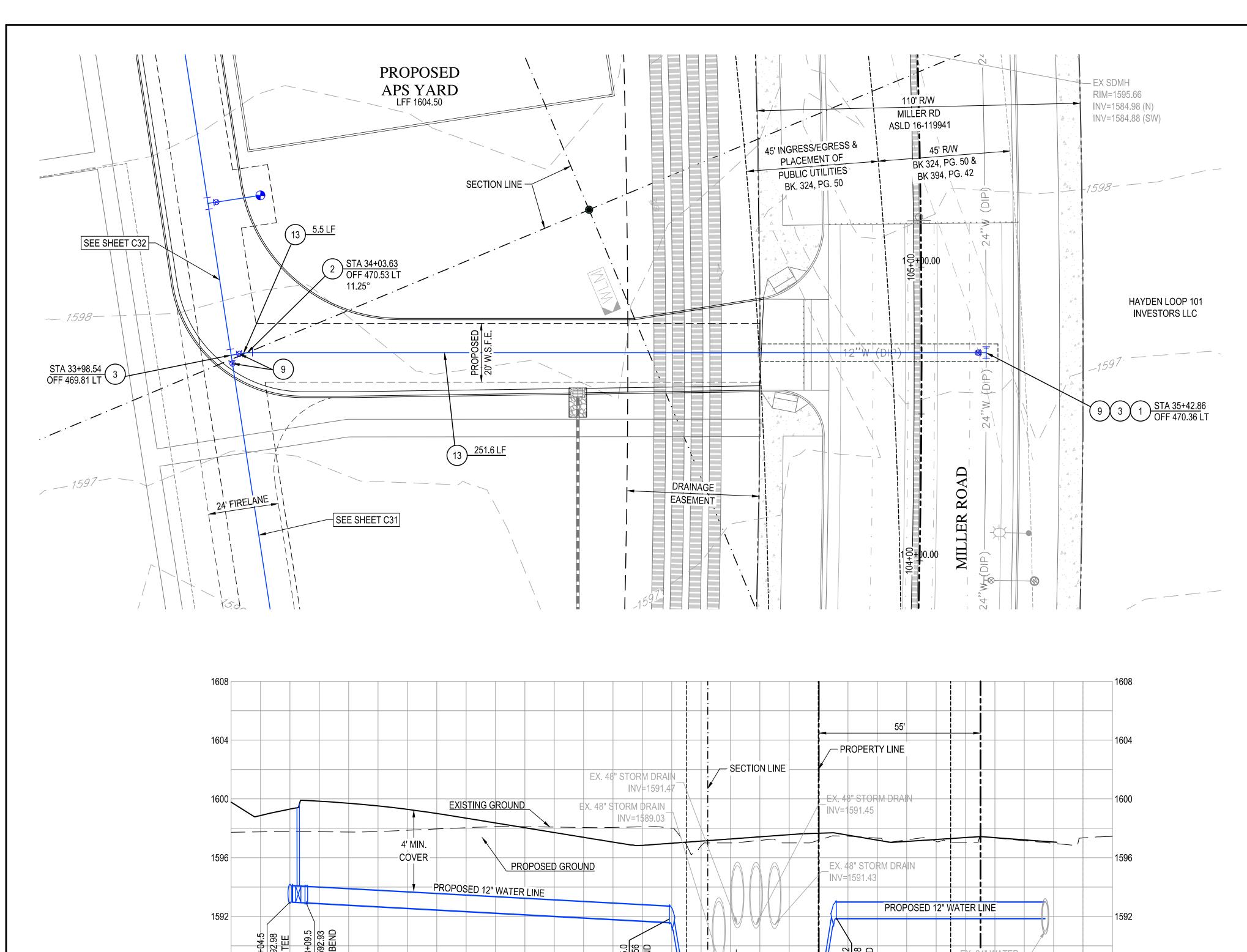
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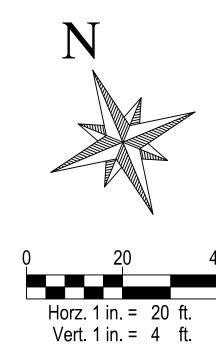


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 JOB NUMBER
 235526

 SHEET
 C32
 OF 43







WATER NOTES

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- 13 INSTALL 12" POLYWRAPPED DIP PRESSURE CLASS 350 WATERLINE WITH 12 AWG TRACER WIRE PER MAG STD SPEC 610 AND 750. 4' MINIMUM COVER. INSTALL MECHANICAL JOINT RESTRAINT PER MAG STD DETAIL 303. TRENCH, BEDDING AND BACKFILL PER MAG STD SPEC 601.



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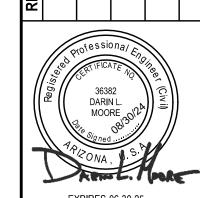
602.335.8500



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LIMINARY IMPROVEMENT PLANS SCOTTSDALE, ARIZONA WATER PLAN & PROFILE SCOTTSDALE ASM

PRE



EXPIRES 06-30-25

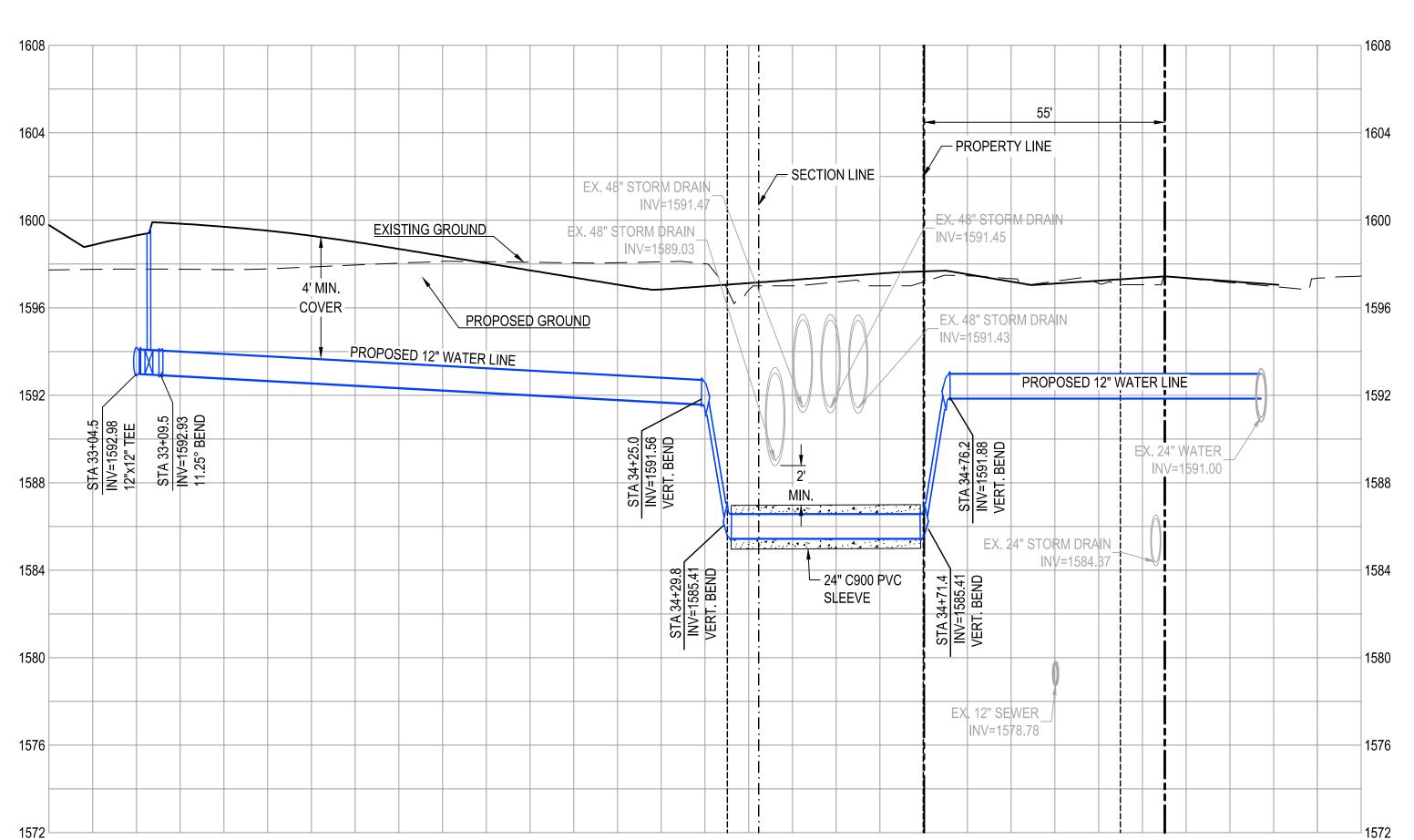
 SCALE (HORIZ.)
 1" = 20'

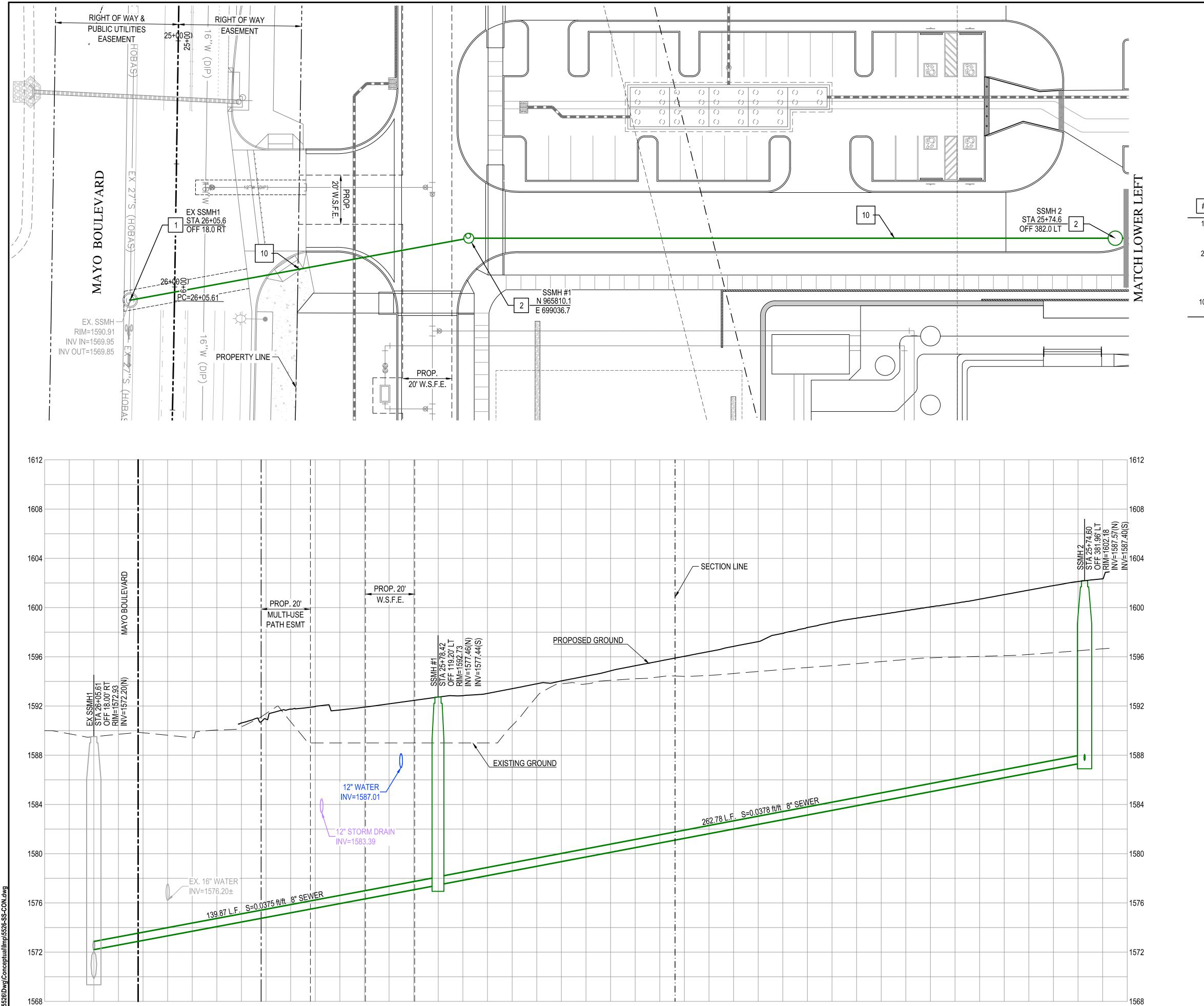
 SCALE (VERT.)
 1" = 4'

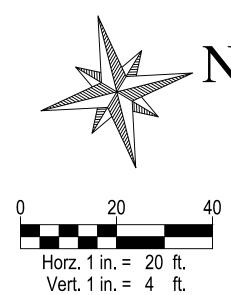
 DATE
 08/30/2024

 JOB NUMBER
 235526

 SHEET
 C34
 OF 43







SEWER NOTES

- CUT AND CONNECT TO EXISTING SEWER LINE. VERIFY ELEVATION AND LOCATION PRIOR TO CONSTRUCTION. NOTIFY ENGINEER OF ANY DISCREPANCIES.
- CONSTRUCT 5' DIAMETER SEWER MANHOLE PER MAG STD DETAIL 420. INSTALL 30" WATERTIGHT MANHOLE FRAME AND COVER PER MAG STD DETAIL 424-2. ADJUST FRAME AND COVER TO GRADE AND CONSTRUCT CONCRETE COLLAR AROUND FRAME PER MAG STD DETAIL 422-1. APPLY AGENCY APPROVED MANHOLE COATING.
- 10 INSTALL 8" VCP SEWER LINE PER MAG STD SPEC 615 AND 625. TRENCH, BEDDING AND BACKFILL PER MAG STD SPEC 601.



Civil Engineering Water Resources Land Survey Construction Management

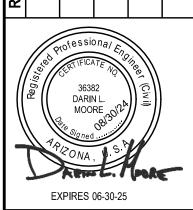
602.335.8500

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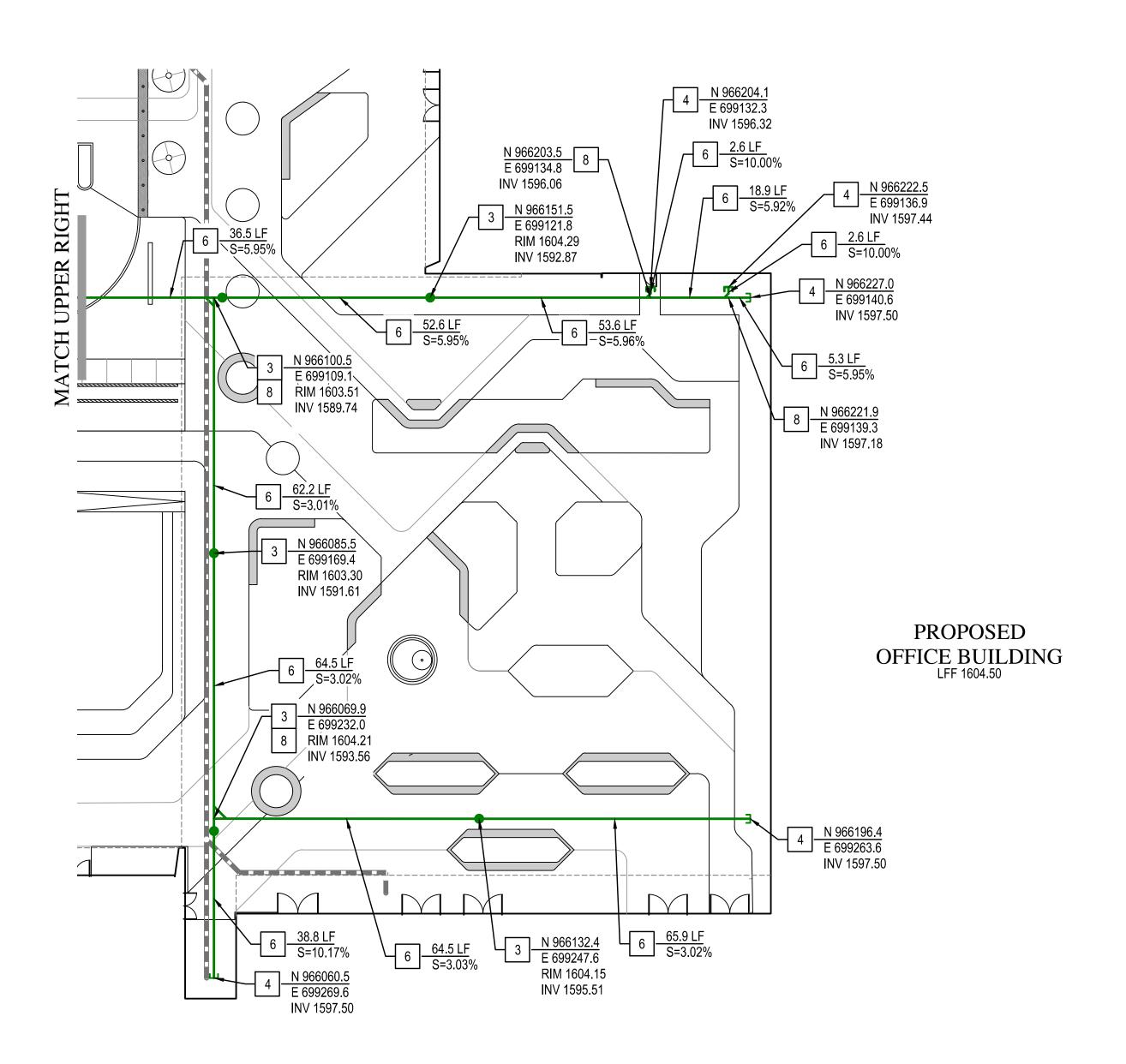
ASM SCOTTSDALE

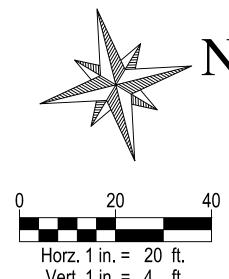
LIMINARY IMPROVEMENT PLANS SCOTTSDALE, ARIZONA SEWER PLAN & PROFILE PREI



SCALE (HORIZ.) 1" = 20' SCALE (VERT.) 1" = 4' DATE 08/30/2024
 JOB NUMBER
 235526

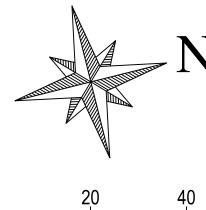
 SHEET
 OF 43

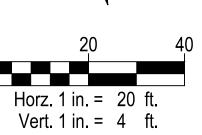




SEWER NOTES

- 3 INSTALL SEWER CLEANOUT PER MAG STD DETAIL 441, SIZE PER ADJOINING
- 4 CONNECT TO BUILDING SEWER. SEE PLUMBING PLAN FOR CONTINUATION. VERIFY ELEVATION AND LOCATION WITH PLUMBING PLAN PRIOR TO CONSTRUCTION. NOTIFY ENGINEER OF ANY DISCREPANCIES.
- 6 INSTALL 6" PVC SDR35 SEWER LINE PER MAG STD SPEC 615 AND 745. TRENCH, BEDDING AND BACKFILL PER MAG STD SPEC 601.
- 8 INSTALL WYE OR COMBINATION WYE AND BEND, SIZE PER ADJOINING PIPE DIAMETER, ANGLE PER PLAN.





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In Maricopa County: (602) 263-1100

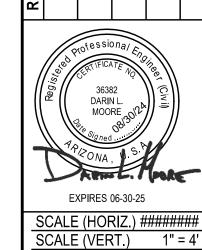
WOOD PATEL

Wood, Patel & Associates, Inc.

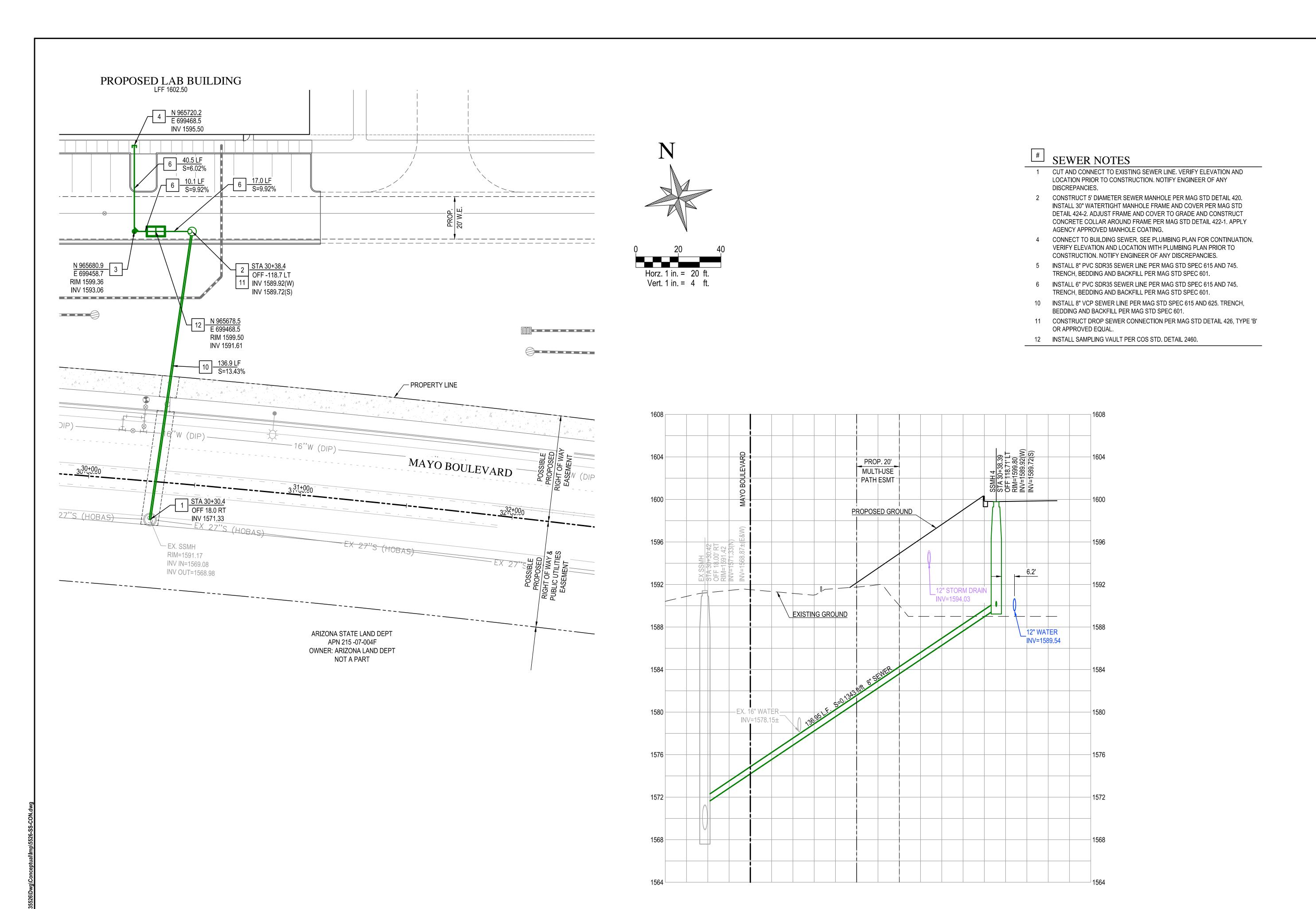
ASM SCOTTSDALE

LIMINARY IMPROVEMENT PLANS SCOTTSDALE, ARIZONA PREI

DATE					
DESCRIPTION					
REV					



SCALE (HORIZ.) ####### SCALE (VERT.) 1" = 4' DATE 08/30/2024 JOB NUMBER 235526 SHEET C36 OF 43





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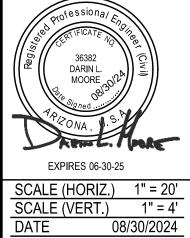


PLANS

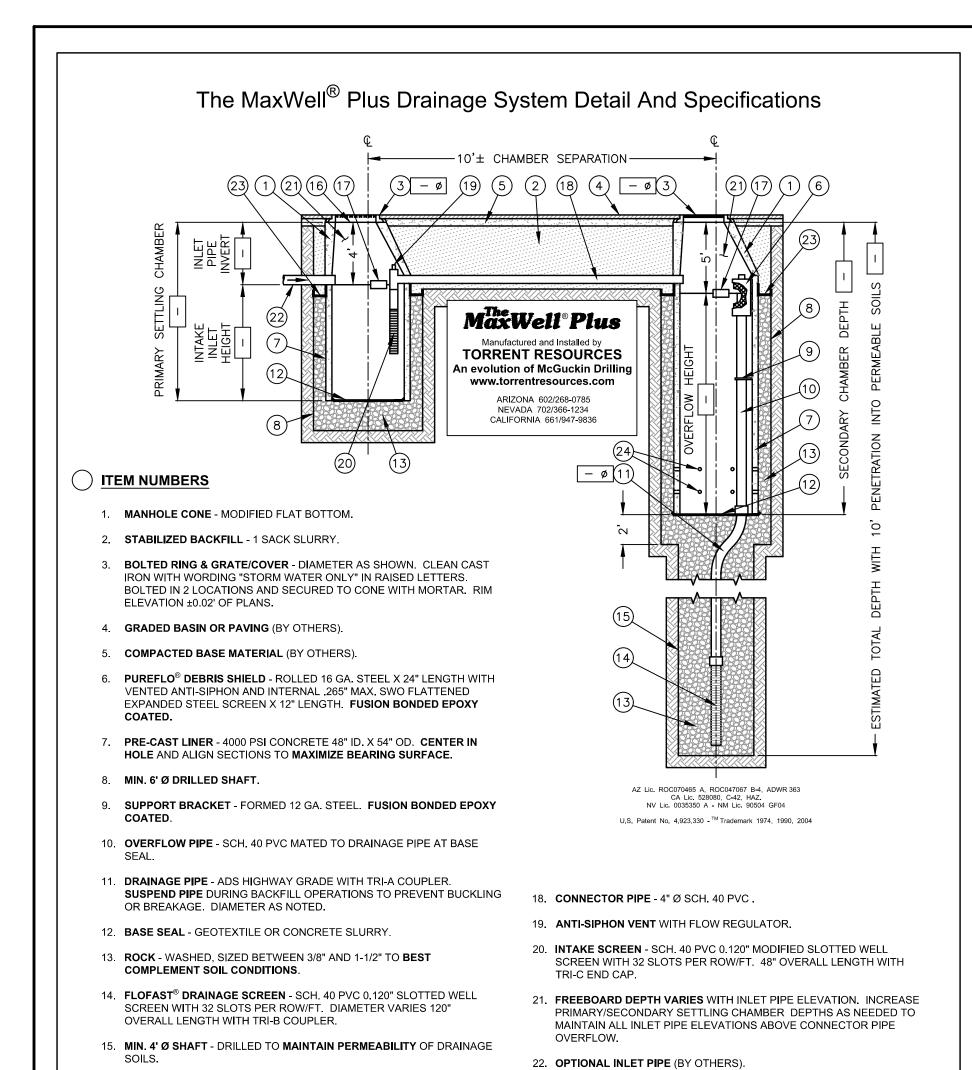
IMINARY IMPROVEMENT SCOTTSDALE, ARIZONA SM

PRE

SCOTTSDALE



JOB NUMBER 235526 SHEET



23. MOISTURE MEMBRANE - 6 MIL. PLASTIC. PLACE SECURELY AGAINST

24. EIGHT (8) PERFORATIONS PER FOOT, 2 ROWS MIN.

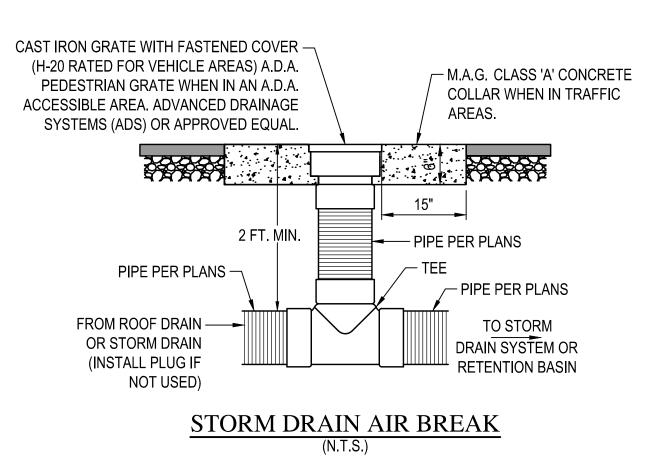
LANDSCAPED AREAS.

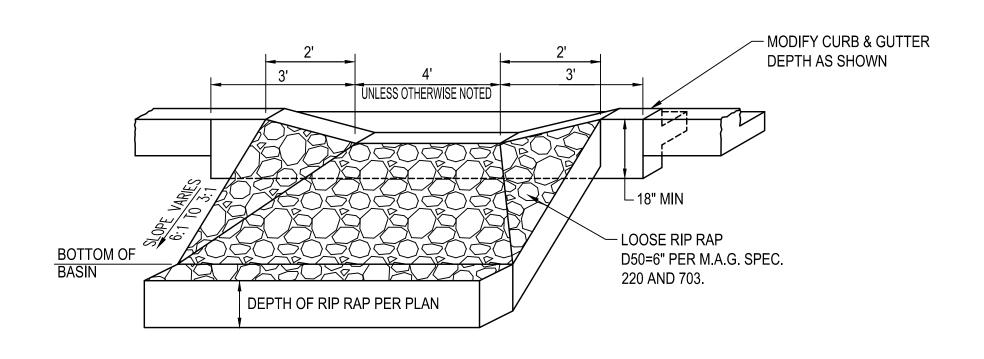
ECCENTRIC CONE AND HOLE SIDEWALL. USED IN LIEU OF SLURRY IN

16. **FABRIC SEAL** - U.V. RESISTANT GEOTEXT**I**LE - **TO BE REMOVED BY**

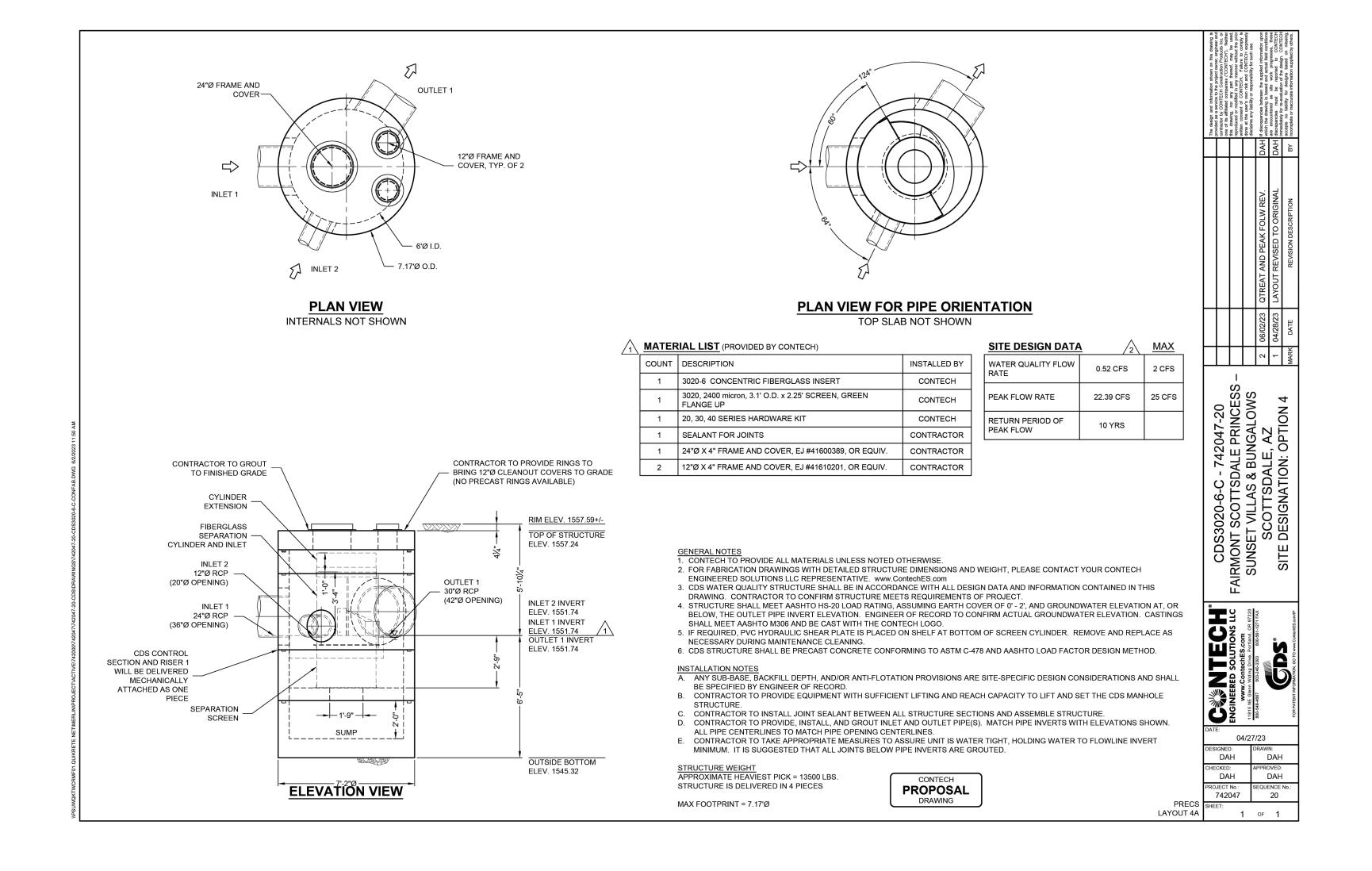
17. ABSORBENT - HYDROPHOBIC PETROCHEMICAL SPONGE. MIN. 128 OZ.

CUSTOMER AT PROJECT COMPLETION.





CURB OPENING AND SPILLWAY DETAIL AT EXTRUDED CURB





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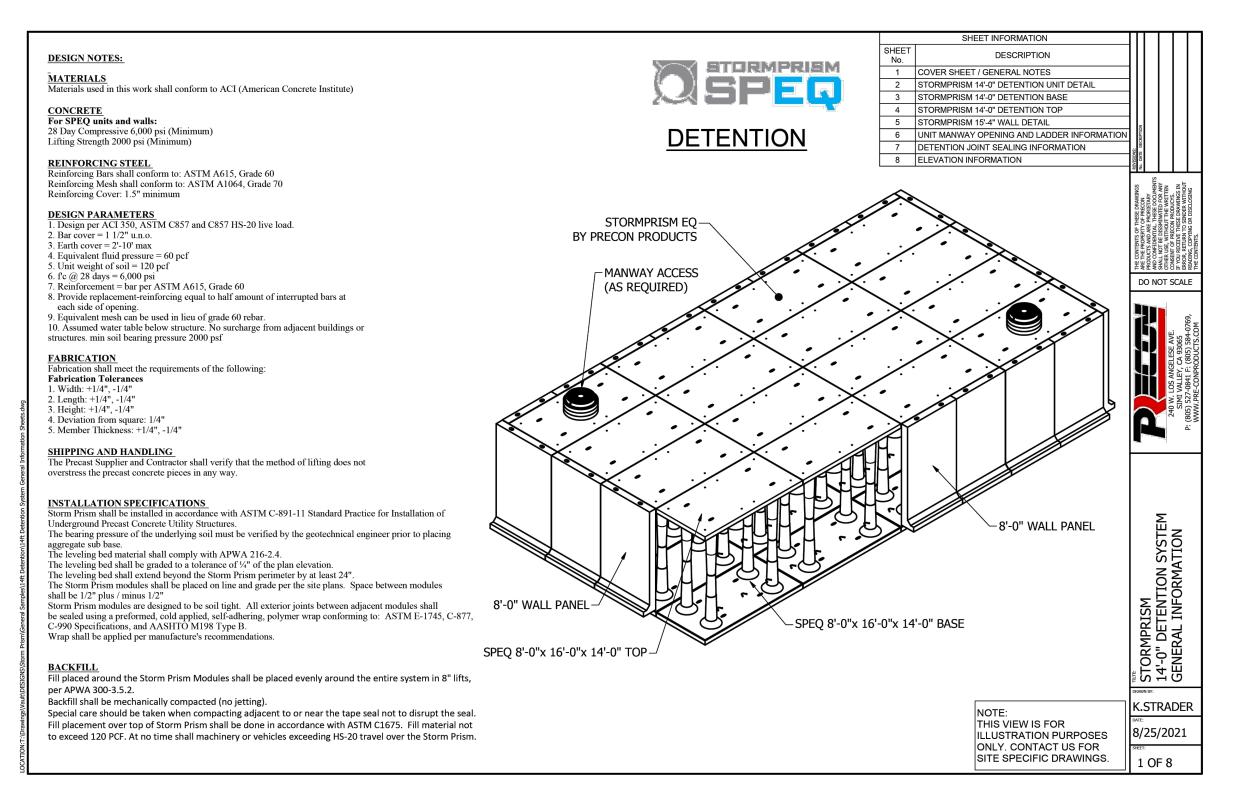
www.woodpatel.com

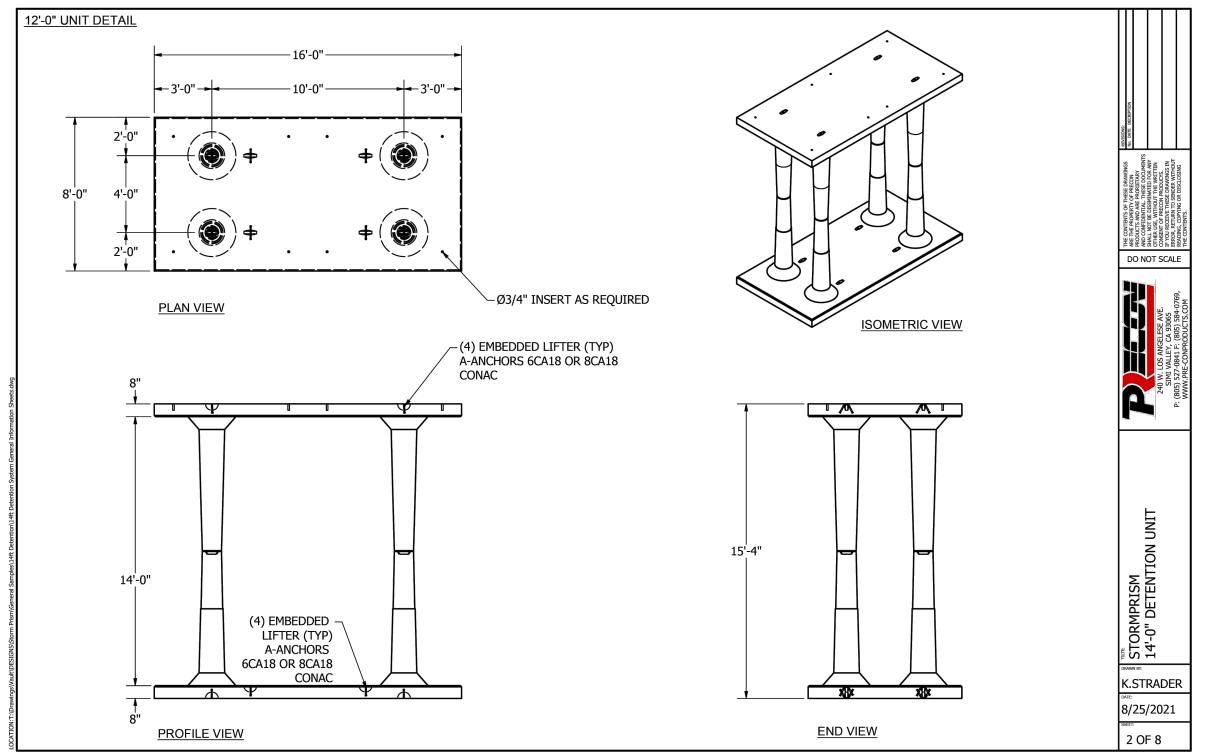


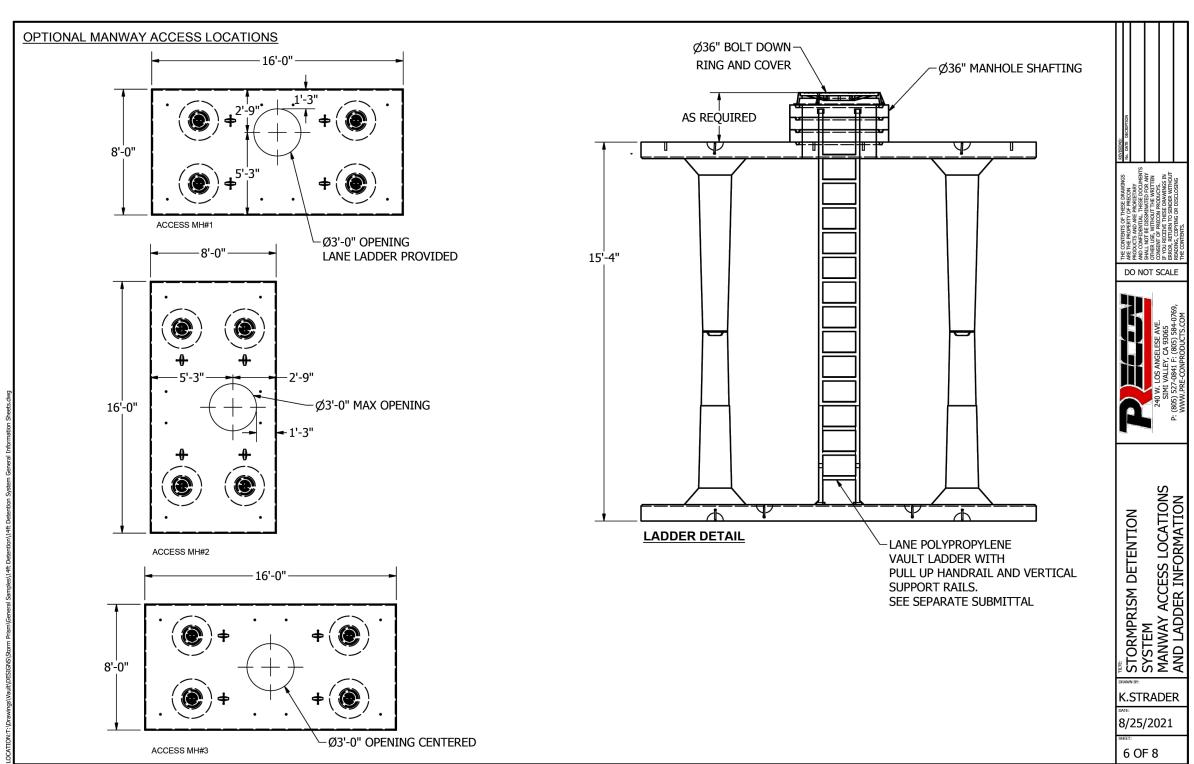
'IMPROVEMENT F ISDALE, ARIZONA SD -IMINARY SCOTTS

EXPIRES 06-30-25 SCALE (HORIZ.) SCALE (VERT.) N/A

08/30/2024 JOB NUMBER 235526









PRELIMINARY IMPROVEMENT PLANS SCOTTSDALE, ARIZONA DETAILS

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/ DESCRIPTION DATE



 SCALE (HORIZ.)
 N/A

 SCALE (VERT.)
 N/A

 DATE
 08/30/2024

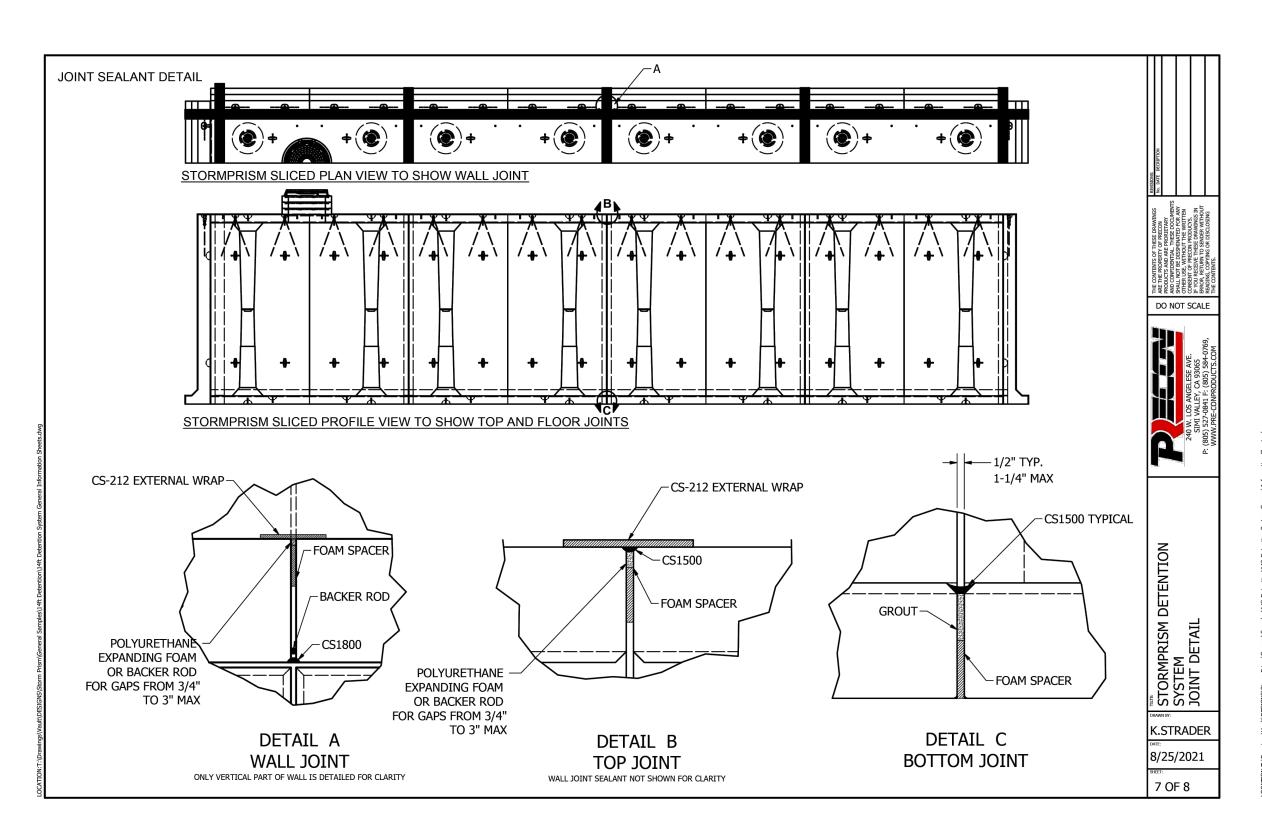
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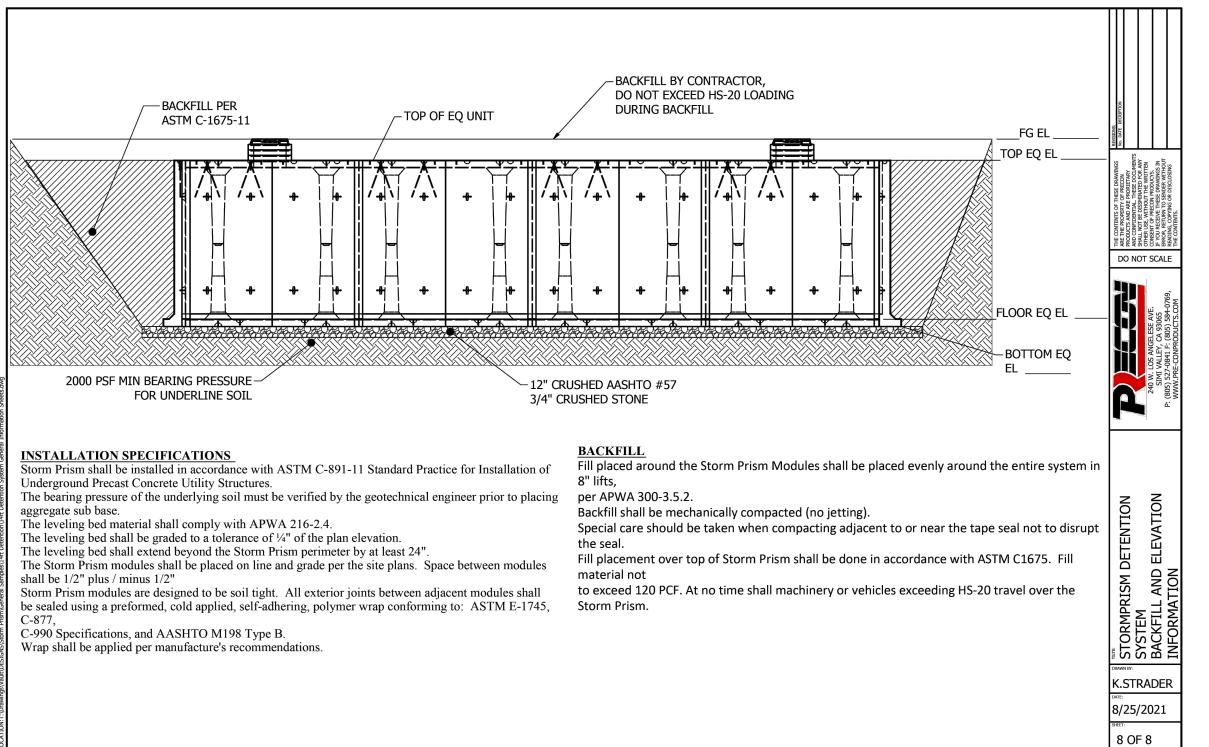
 SHEET
 C38

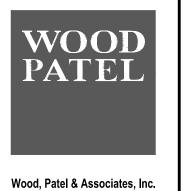
 C38
 OF 43

CHECKED BY: DM DESIGNED BY: RS DRAFTED BY: JRS

:3\235526\Dwg\Conceptual\Imp\5526-DT-CON.dwg







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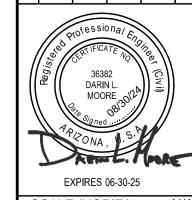
www.woodpatel.com



PLANS

-IMINARY IMPROVEMENT SCOTTSDALE, ARIZONA DETAILS PRE

ASM SCOTTSDALE



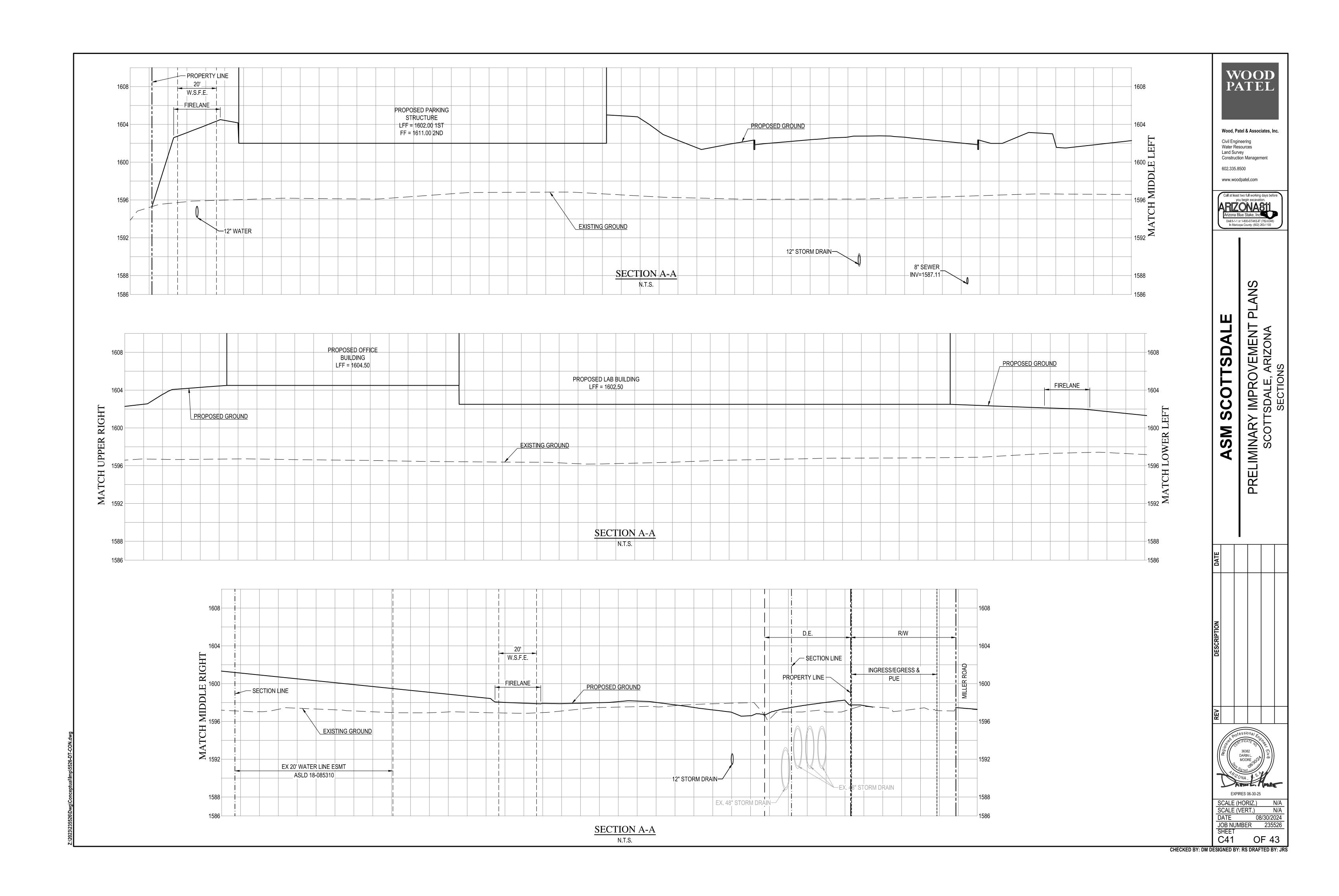
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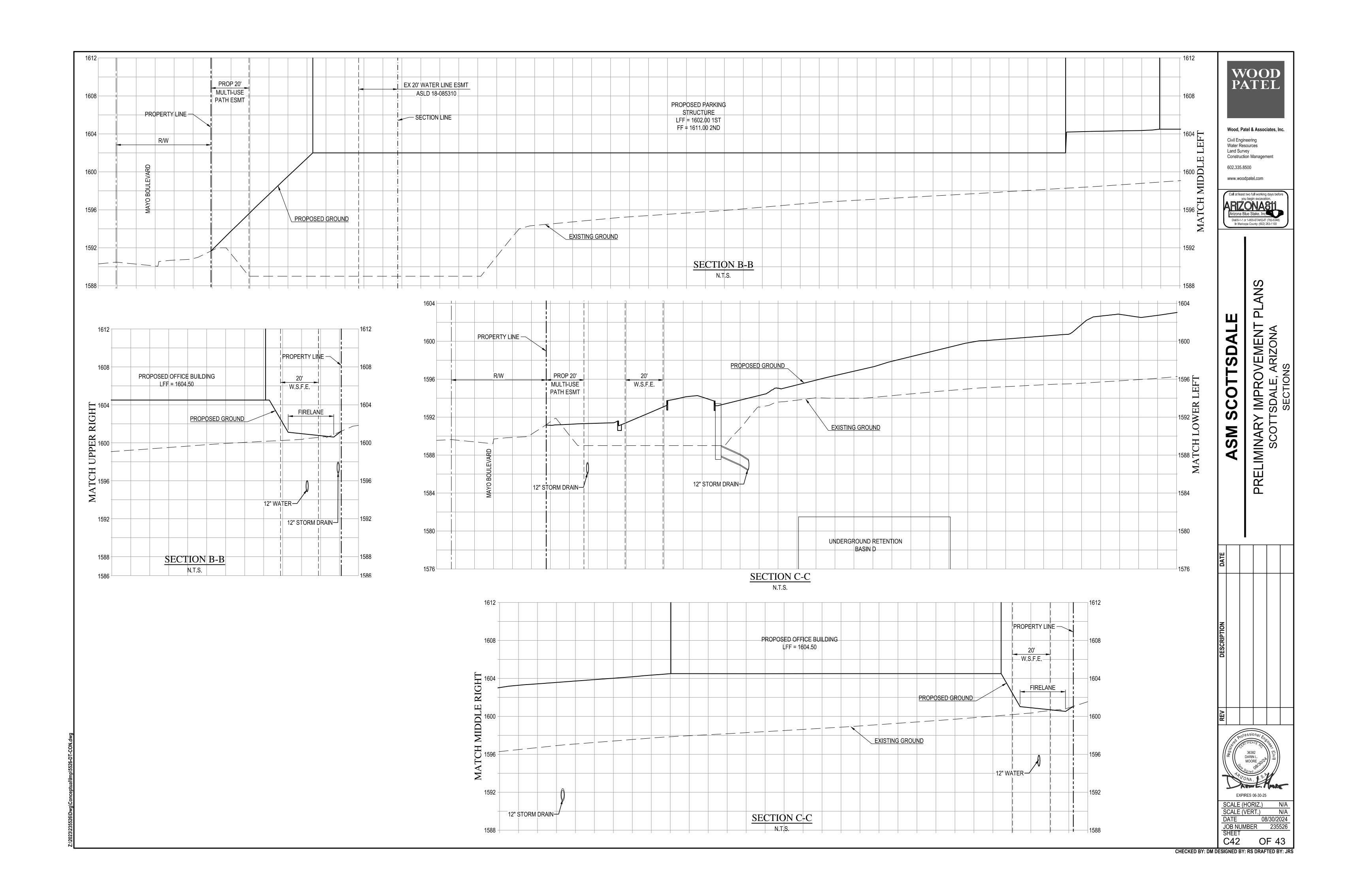
 SCALE (VERT.)
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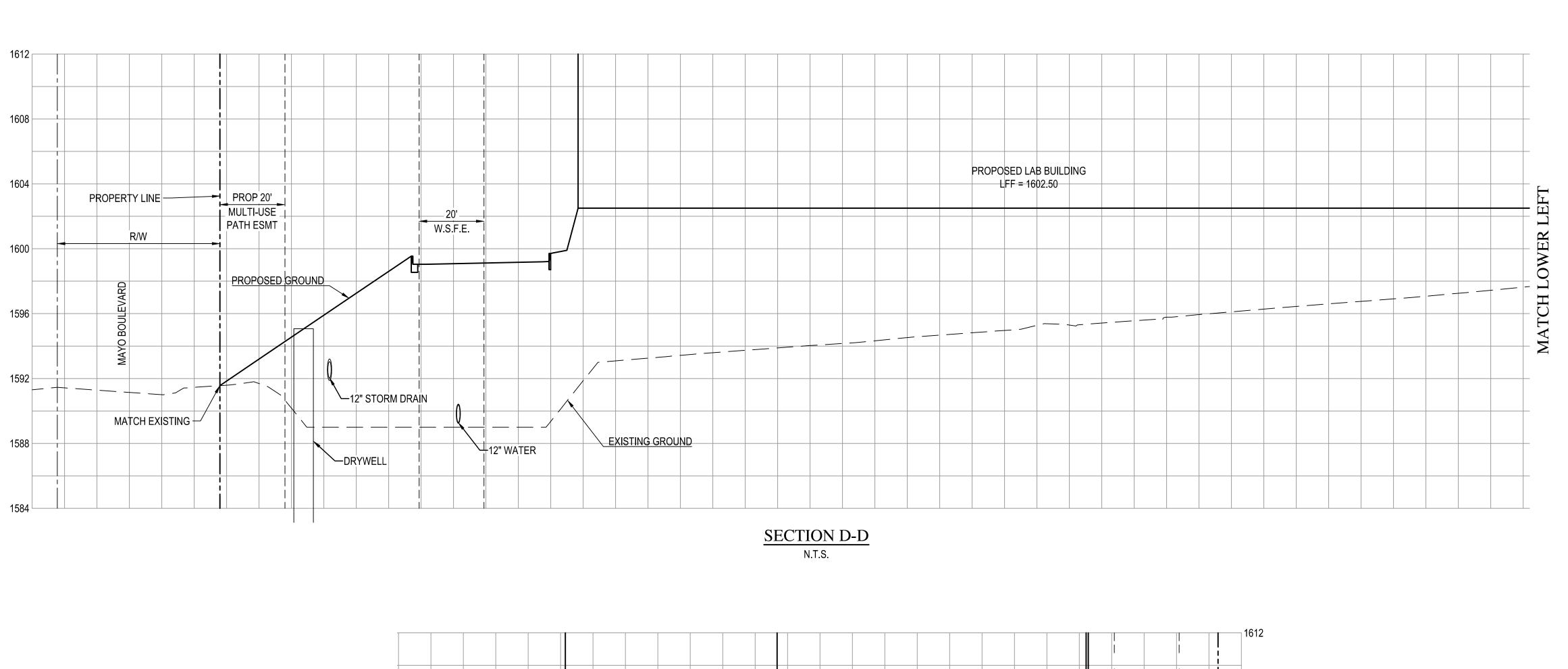
 DATE
 08/30/2024

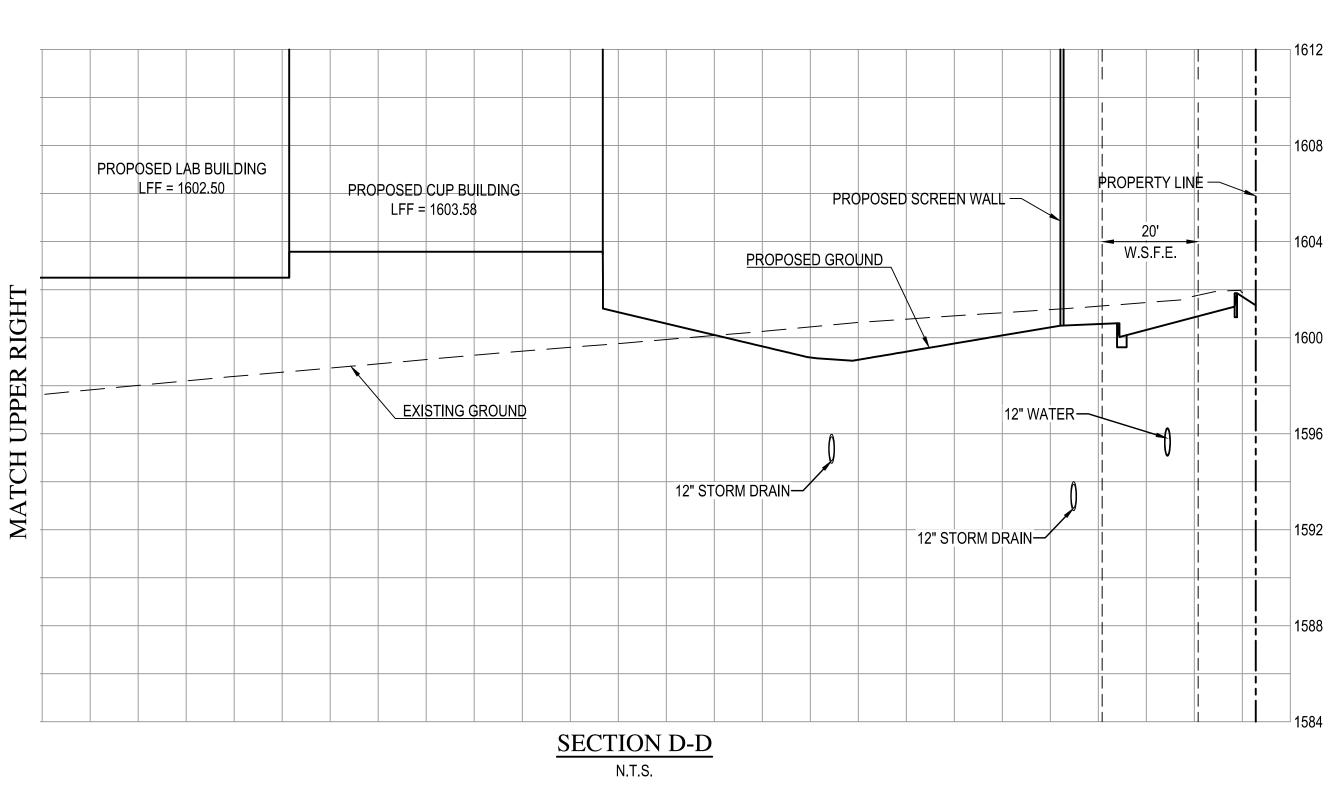
 JOB NUMBER
 235526

 SHEET
 C38
 OF 43











LIMINARY IMPROVEMENT PLANS SCOTTSDALE, ARIZONA SECTIONS PREI

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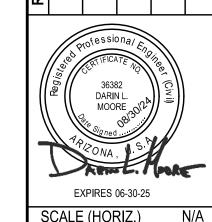
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 SCALE (HORIZ.)
 N/A

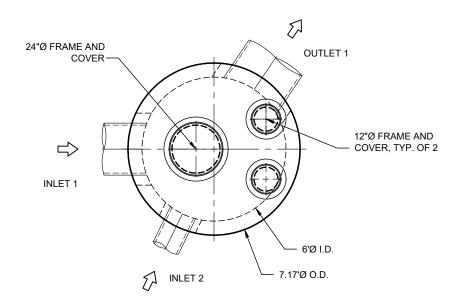
 SCALE (VERT.)
 N/A

 DATE
 08/30/2024

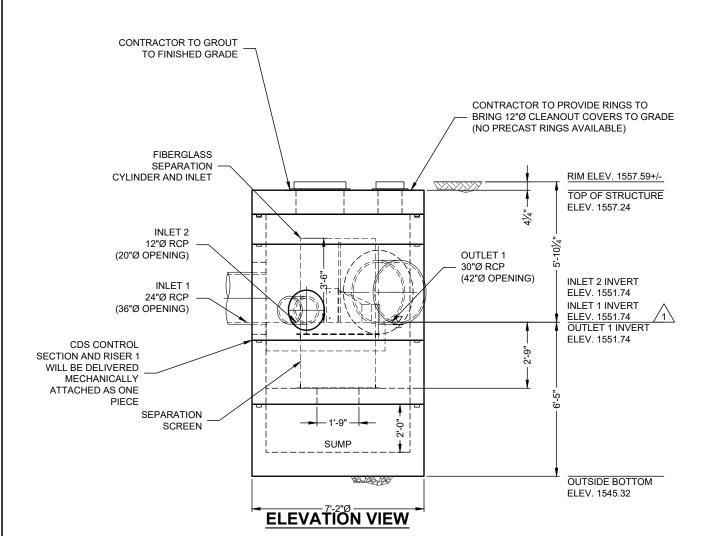
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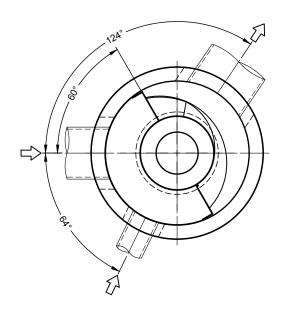
 SHEET
 C43
 OF 43





PLAN VIEW INTERNALS NOT SHOWN





PLAN VIEW FOR PIPE ORIENTATION

TOP SLAB NOT SHOWN

$\sqrt{}$	MATERIAL LIST (PROVIDED BY CONTECH)							
	COUNT	DESCRIPTION	INSTALLED BY					
	1	3020-6 CONCENTRIC FIBERGLASS INSERT	CONTECH					
	1	3020, 2400 micron, 3.1' O.D. x 2.25' SCREEN, GREEN FLANGE UP	CONTECH					
	1	20, 30, 40 SERIES HARDWARE KIT	CONTECH					
	1	SEALANT FOR JOINTS	CONTRACTOR					
	1	24"Ø X 4" FRAME AND COVER, EJ #41600389, OR EQUIV.	CONTRACTOR					
	2	12"Ø X 4" FRAME AND COVER, EJ #41610201, OR EQUIV.	CONTRACTOR					

SITE DESIGN DATA	MAX	
WATER QUALITY FLOW RATE	0.29 CFS	2.8 CFS
PEAK FLOW RATE	15 CFS	20 CFS
RETURN PERIOD OF PEAK FLOW	10 YRS	

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.ContechES.com
- 3. CDS WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
- 4. STRUCTURE SHALL MEET AASHTO HS-20 LOAD RATING, ASSUMING EARTH COVER OF 0' 2', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO.
- 5. IF REQUIRED, PVC HYDRAULIC SHEAR PLATE IS PLACED ON SHELF AT BOTTOM OF SCREEN CYLINDER. REMOVE AND REPLACE AS NECESSARY DURING MAINTENANCE CLEANING.
- 6. CDS STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C-478 AND AASHTO LOAD FACTOR DESIGN METHOD.

- ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE CDS MANHOLE
- CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLE STRUCTURE.
- CONTRACTOR TO PROVIDE, INSTALL, AND GROUT INLET AND OUTLET PIPE(S). MATCH PIPE INVERTS WITH ELEVATIONS SHOWN. ALL PIPE CENTERLINES TO MATCH PIPE OPENING CENTERLINES.
- CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.

STRUCTURE WEIGHT APPROXIMATE HEAVIEST PICK = 13500 LBS. STRUCTURE IS DELIVERED IN 4 PIECES

MAX FOOTPRINT = 7.17'Ø



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				LAYOUT RI	Ы	
				04/28/23	DATE	
				1	MARK	

742047-20 SCOTTSDALE, AZ DESIGNATION: OPTION CDS3020-6-C SITE

DATE:						
04/27/23						
DESIGNED:	DRAWN:					
DAH	DAH					
CHECKED:	APPROVED:					
DAH	DAH					
PROJECT No.:	SEQUENCE No.:					
742047	20					
SHEET:						
1 1	∩⊏ 1					

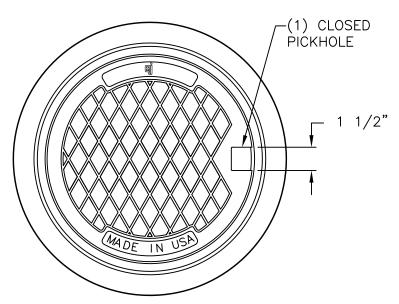
PRECS

LAYOUT 4A

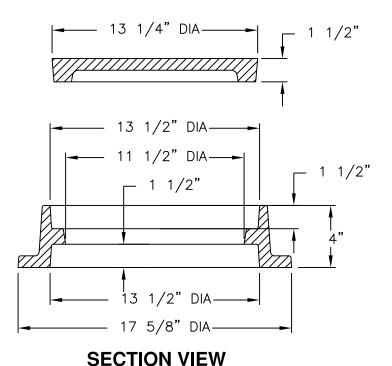
V1610-2BF RG V1610-2CV Assembly

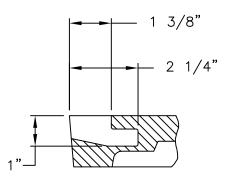






BOTTOM FLANGE VIEW





PICKHOLE DETAIL

NOTE: RING IS REVERSIBLE AND CAN BE INSTALLED AS A TOP FLANGE UNIT

Product Number 41610201

Design Features

-Materials

Cleanout/Monument Box Frame Gray Iron (CL35B) Cleanout/Monument Box Cover Gray Iron (CL35B)

-Design Load

Heavy Duty

-Open Area

n/a

-Coating

Undipped

- V Designates Machined Surface

Certification

- ASTM A48

-

-Country of Origin: USA

Major Components

41610210 41610220

Drawing Revision

06/12/2003 Designer: DAL 09/09/2015 Revised By: DVD

Disclaimer

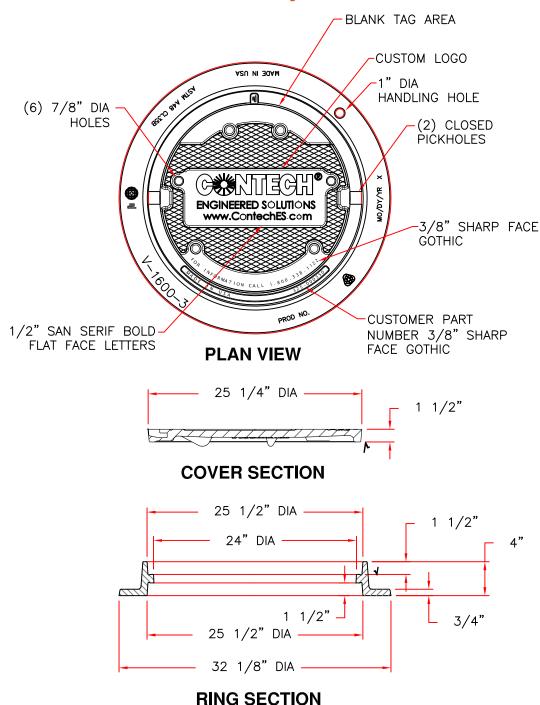
Weights (lbs./kg) dimensions (inches/mm) and drawings provided for your guidance. We reserve the right to modify specifications without prior potics.

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V1600-3 V1610-3 Assembly





Product Number

41600389

Design Features
-Materials
Frame
Gray Iron (CL35B)
Cover
Gray Iron (CL35B)

-Design Load

Heavy Duty

-Open Area

n/a

-Coating

Undipped

- √ Designates Machined Surface

Certification

- ASTM A48

-Country of Origin: USA

Major Components

41600310 41600374

Drawing Revision

05/02/2008 Designer: DEW 6/20/2017 Revised By: DAE

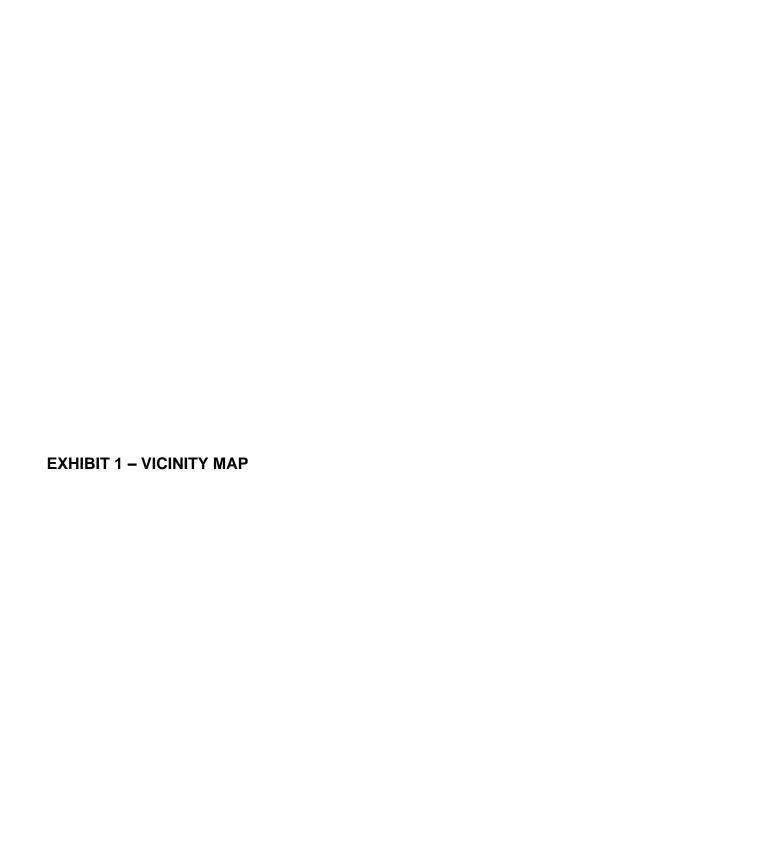
Disclaimer

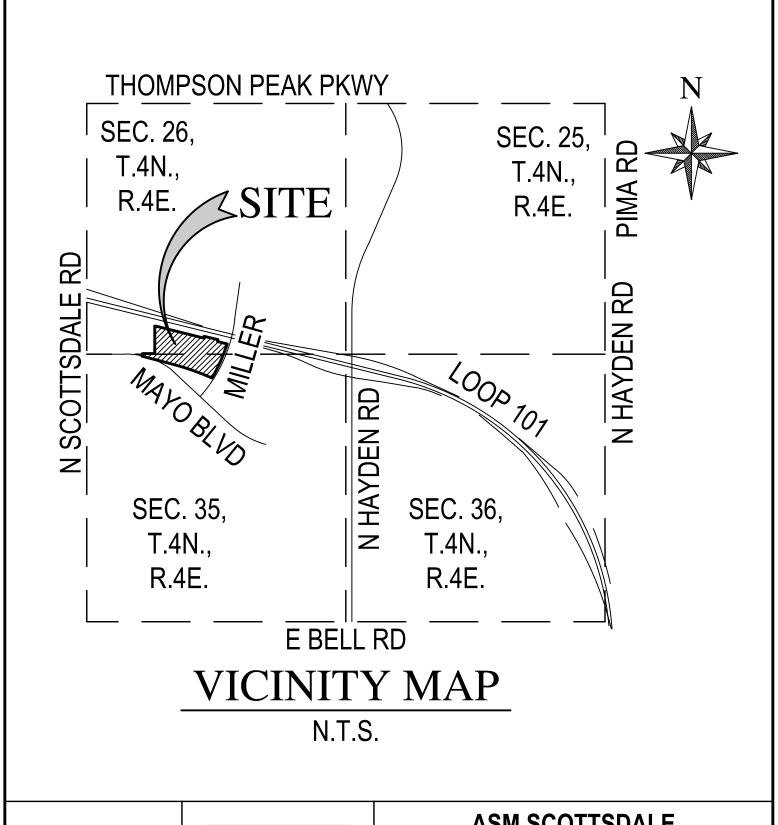
Weights (lbs./kg) dimensions (inches/mm) and drawings provided for your guidance. We reserve the right to modify specifications without prior notice.

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ASM SCOTTSDALE

EXHIBIT 1 - VICINITY MAP

DATE	08/30/2024	SCALE	N.T.S.	SHEET	1 OF 1		
JOB NO.	235526	DESIGN	AJS	CHECK	RGS		
		DRAWN	ВСТ	RFI#	-		

Z:\2023\235526\Project Support\Reports\Drainage\Exhibits\5526-EXH1-VM.dwg



National Flood Hazard Layer FIRMette

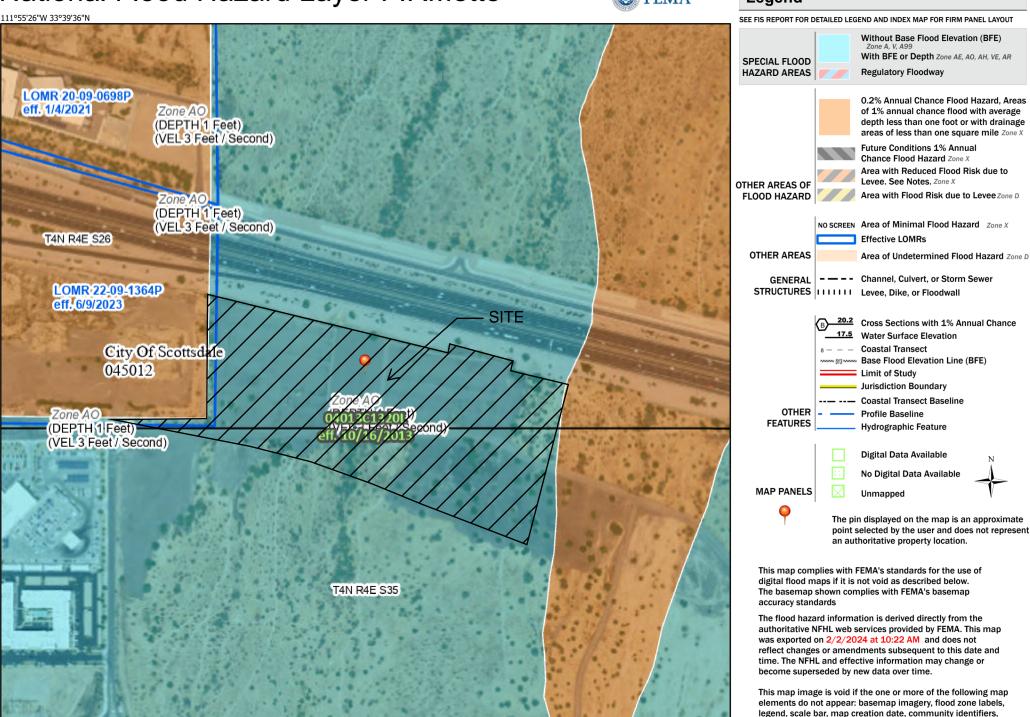
250

500

1,000

1,500





1:6.000

2,000

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

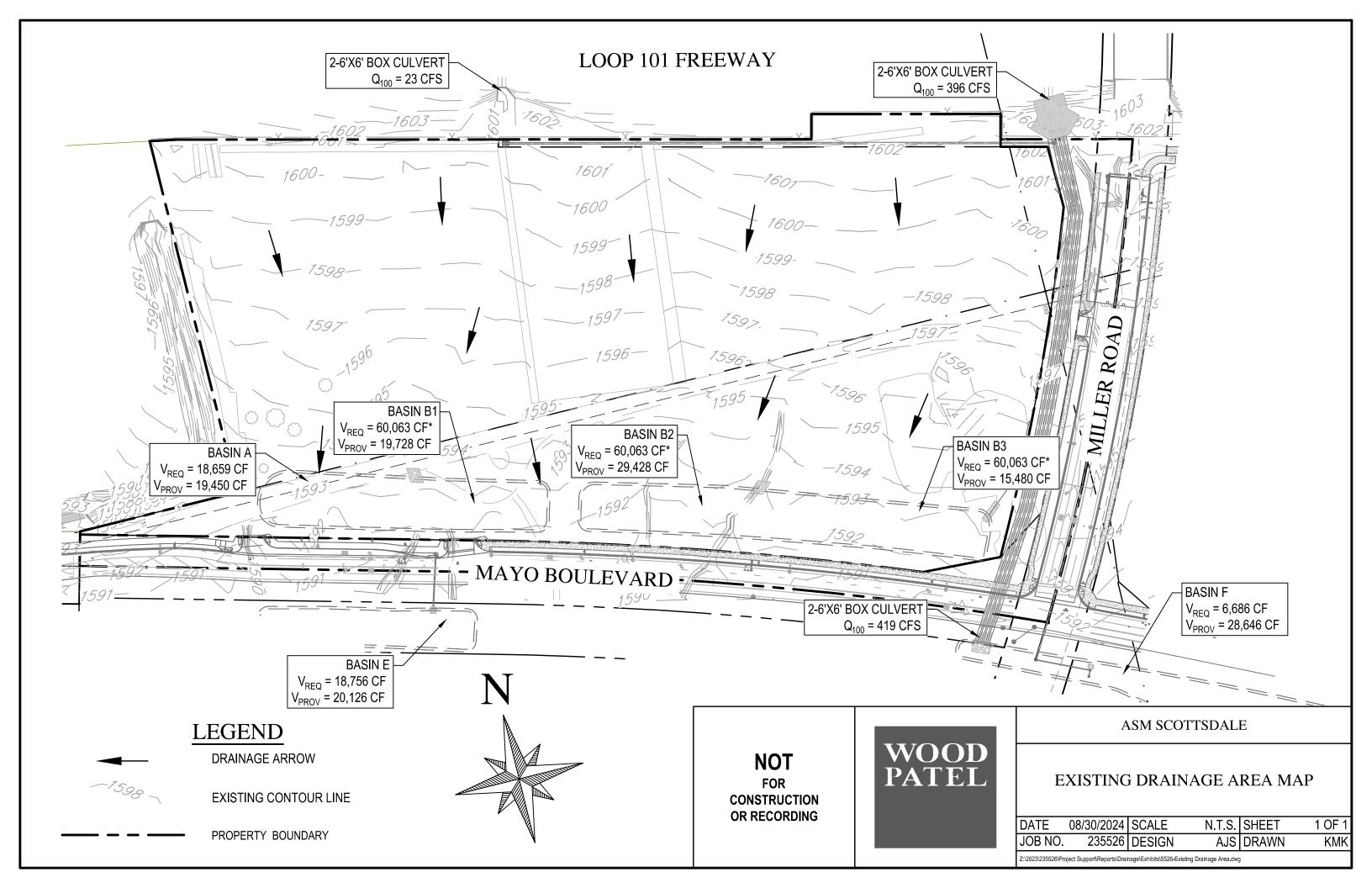
Without Base Flood Elevation (BFE) With BFE or Depth Zone AE, AO, AH, VE, AR Regulatory Floodway 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X **Future Conditions 1% Annual** Chance Flood Hazard Zone X Area with Reduced Flood Risk due to Levee. See Notes. Zone X Area with Flood Risk due to Levee Zone D NO SCREEN Area of Minimal Flood Hazard Zone X Effective LOMRs Area of Undetermined Flood Hazard Zone D - - - Channel, Culvert, or Storm Sewer STRUCTURES | LILLILL Levee, Dike, or Floodwall 20.2 Cross Sections with 1% Annual Chance Water Surface Elevation **Coastal Transect** ₩ 513 W Base Flood Elevation Line (BFE) Limit of Study **Jurisdiction Boundary Coastal Transect Baseline Profile Baseline Hydrographic Feature** Digital Data Available No Digital Data Available

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap

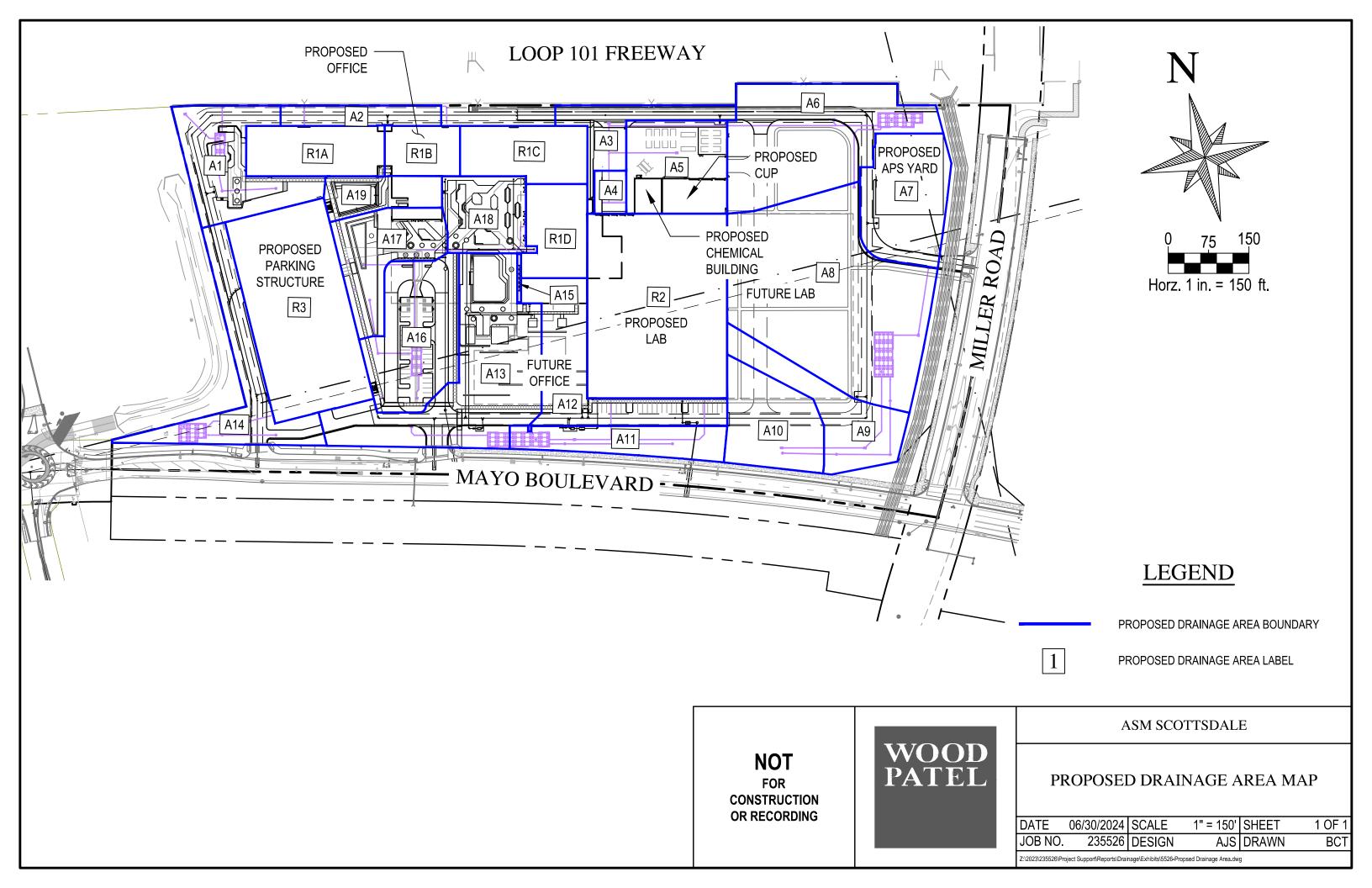
The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 2/2/2024 at 10:22 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

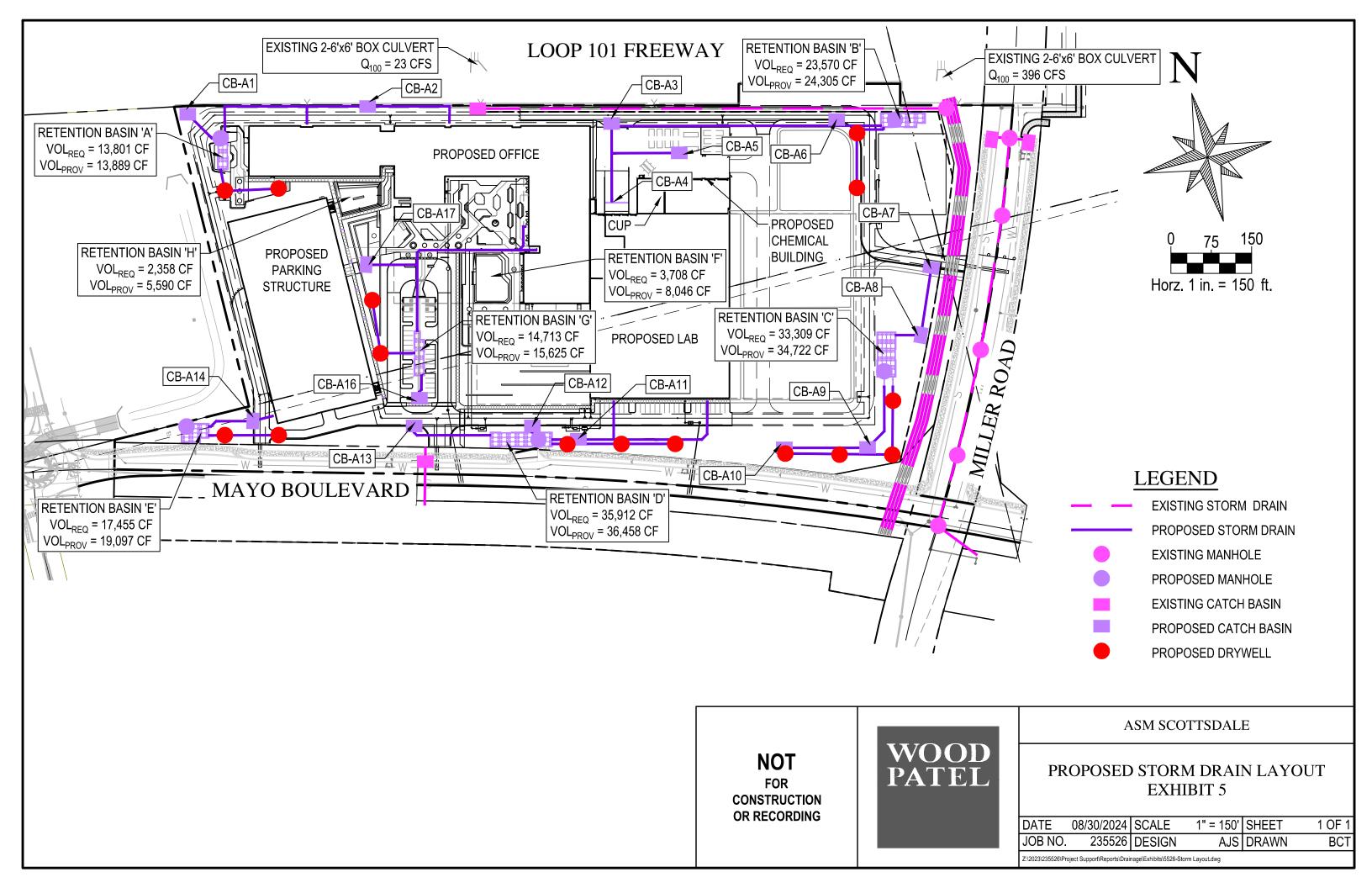




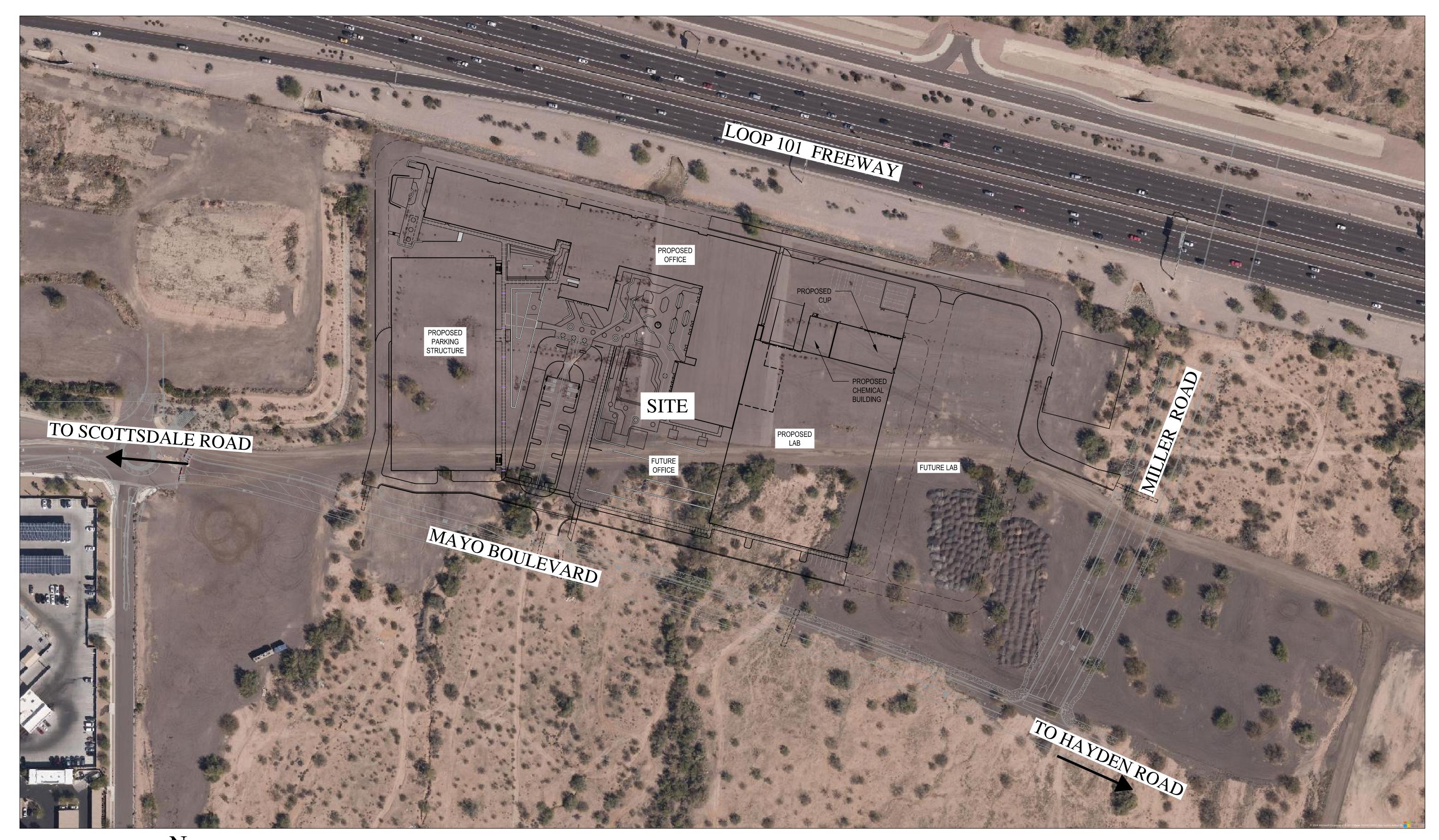


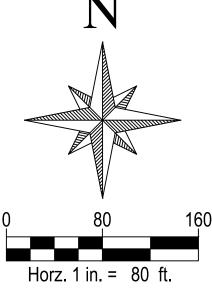












NOT
FOR
CONSTRUCTION
OR RECORDING



ASM SCOTTSDALE

AERIAL MAP EXHIBIT 6

ATE	08/30/2024	SCALE	1" = 80'	SHEET	01 OF 0		
OB NO	235526	DESIGN	AJS	DRAWN	AJS		
2023\235526\Project Support\Reports\Drainage\Eyhibits\5526_Aerial Map dwg							