

CONCEPT DRAINAGE REPORT FOR FAIRMONT SCOTTSDALE PRINCESS CONFERENCE CENTER & EVENT LAWN

August 7, 2023 WP# 215319.30

5-ZN-2015#2



EXPIRES 06-30-25

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EXPIRES 06-30-25

1.0 INTRODUCTION

1.1 General Background

The Fairmont Scottsdale Princess Conference Center & Event Lawn (Site) is a proposed commercial building and lawn event space on an approximate area of 44.4 acres of the Fairmont Scottsdale Princess in the City of Scottsdale (APN#215-08-693, 215-08-695). The proposed development will consist of one (1) commercial building and a large outdoor lawn. The project will include parking, hardscape, landscape, parking, and utility improvements to support the development. The Site is located approximately 1,300-feet to the east of Scottsdale Road and 50-feet north of East Hacienda Way within Section 35, Township 4 North, Range 4 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. Refer to Exhibit 1 – *Vicinity Map* for the project location. The existing property, currently zoned C-2, is primarily developed with buildings, parking lots, pools, sidewalks, and a variety of landscaping (desert and grass).

This concept Drainage Report has been prepared in accordance with Wood, Patel & Associates, Inc.'s (WOODPATEL's) understanding of the City of Scottsdale technical drainage requirements (Ref. 1) and the *Drainage Design Manuals for Maricopa County Hydrology and Hydraulics (2018),* as applicable to the Site.

1.2 FEMA Regulated Flood Zones

The Federal Emergency Management Agency (FEMA) publishes Flood Insurance Rate Map (FIRM) information for communities that adhere to FEMA regulations. The FEMA FIRM panel for this Site is 04013C1320L, effective date October 16, 2013, and indicates the Site falls within "Zone AO" shaded (Refer to Exhibit 2 – *FEMA FIRM*).

"Zone AO" shaded is defined by FEMA as follows:

"Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined".

It is the understanding of WOODPATEL, based on past experience and interpretations of the City of Scottsdale floodplain ordinance that development of land within FEMA Zone "AO" is acceptable as long as, in general, the lowest finish floor elevation is above or properly protected from the anticipated 100year water surface elevations. This Site will be designed in accordance with the City floodplain ordinance to meet Federal and State regulations.

2.0 HYDROLOGY ANALYSIS

2.1 Offsite Hydrology

The proposed Site does not receive offsite flows. An existing regional drainage ditch along the north side of Princess Boulevard in conjunction with an existing flood wall along the south side of Princess Boulevard routes stormwater flows west to an existing regional drainage channel parallel to Scottsdale Road. Stormwater is routed south by the existing regional channel to the TPC Golf Course. Refer to Appendix F - Drainage Report for Fairmont Scottsdale Privado Welcome Building and Parking

Modifications by Wood, Patel & Associates, Inc., dated October 14, 2022, which provides a history of the offsite drainage and retention.

2.2 Onsite Hydrology

Per the existing stormwater waiver approved for the Site, no stormwater retention is provided. Refer to Appendix E - *Storm Water Storage Waiver / Proposed Drainage Improvements Exhibit.* Although retention is not required, the City of Scottsdale does require the First Flush (FF) volume to be treated to meet Federal and State regulations. This will be accomplished with the Contech CDS 2020-5C treatment system. See Appendix D – *Contech CDS 2020-5C Treatment System* for the manufacture's detail. Runoff from the proposed building is designed to direct stormwater away from the proposed building and drain overland to existing catch basins connected to the existing storm drain system. Flows into the existing system are calculated to be 33.2 cfs and 78.0 cfs for the 10-year and 100-year conditions, respectively. See Appendix C - *Hydrologic and Hydraulic Calculations* for the existing and proposed drainage calculations.

Onsite peak flow estimates for the proposed development were generated using the Rational Method, as outlined in the *Drainage Design Manual for Maricopa County, Arizona: Volume I – Hydrology (*Ref 2). NOAA Atlas 14 precipitation data was obtained and utilized to develop Intensity-Duration-Frequency (I-D-F) curves for the Site. Rational Method peak flows were computed at concentration points within the Site at key design locations. Runoff coefficients were estimated to reflect post-development land use conditions for the 2-year, 10-year, and 100-year events (Refer to Appendix C – *Hydrologic and Hydraulic Calculations*).

Ref. 5 - Drainage Report Fairmont Scottsdale Privado Welcome Building and Parking Modifications by Wood, Patel & Associates, Inc., dated October 14, 2022, provides a history of the current onsite drainage and retention. Based on the information above, the proposed site improvements mimic current drainage patterns and areas of retention for onsite with very minimal alteration.

2.3 Establishing Lowest Finish Floor (LF88 Elevations)

The Grading and Drainage Plan has been designed to comply with the City of Scottsdale floodplain ordinance for a Zone "AO" floodplain. It is our understanding, unless other floodproof measures are presented and approved, the proposed Lowest habitable Finished Floor (LFF) elevation must be designed a minimum of 1 foot above the anticipated 100-year flood elevation. Scottsdale currently requires the lowest finished floor elevation of 1 foot above the flood depth, which results in a finished floor elevation of 2 feet above the Highest Adjacent natural Grade (HAG) to the proposed building which would be the regulatory flood elevation. Due to the Site being disturbed after the Zone "AO" Special Flood Hazard was established, the current condition of the Site cannot determine the HAG. Due to this change the HAG must be established using topographical information showing the pre-disturbed condition of the Site.

According to Curry's Corner 7.5-minute Topographic Survey Map by USGS from 1964 with a contour interval of 10 feet, the approximate highest natural grade of this Site prior to development must be changed from the NAVD29 datum to the NAVD88 datum. This change consists of an elevation increase of 1.749 feet determined using surveyed elevations of a nearby monument on both vertical datums.

Using Auto CAD Civil 3D, the quad map was aligned to the Site using common monument lines (section lines) contained within the quad map and previously surveyed by WOODPATEL. The 10-foot interval contours were digitized, adjusting to NAVD88 and applied to a TIN surface model. The surface model was supplemented with break lines at estimated ridge and flowline locations. The surface was used to display interpolated 1-foot contours for the pre-disturbed condition, the proposed building limits were overlaid on the contour map and the HAG was determined for the proposed building. The proposed building lowest finished floor elevation is a minimum of 2 feet above the HAG. Refer to Appendix A - *Regional Contour Map / Opinion of Existing Highest Natural Grade Elevation* and Appendix B - *Curry's Corner Quadrangle Map.* Overlaying the building over the adjusted digitized lowest finish floor elevation map was determined to be 1561.26 making it 1.99-feet above the regulatory flood elevation of 1559.27 calculated by adding 2-feet to the HAG of 1557.27.

In addition, using the same Curry's Corner 7.5-minute Topographic Survey Map, the Lowest Adjacent Grade (LAG) at the building was determined to be 1551.13, 8.14-feet below the lowest finished floor.

2.4 Review of Pinnacle Peak West Area Master Drainage Study

The Site is located within the study limits of the Pinnacle Peak West Drainage Master Study (PPW-ADMS), as prepared by T.Y. Lin International dated July 26, 2013, which utilized FLO-2D to analyze the 100-year, 24-hour storm event for the regional study area. Refer to Appendix G – *121 Pinnacle Peak West Flo-2D Study*.

WOODPATEL reviewed the findings of the PPW-ADMS specific to the proposed Site area and, in our professional opinion, the regional model does not accurately reflect site-specific drainage improvements designed and constructed to protect the Fairmont Scottsdale Princess property. Site-specific improvements included raising a section of Princess Boulevard at the northeast corner of the Site and constructing an east-west flood wall along the north edge of the Site to direct stormwater runoff away from the Fairmont Scottsdale Princess property. Refer to Appendix F - *Fairmont Scottsdale - Ballroom Addition by Wood, Patel & Associates, Inc., dated September 2, 2011,* and Appendix E - *Storm Water Storage Waiver / Proposed Drainage Improvements.*

Based on the above, we believe offsite flows are incorrectly reported in the PPW-ADMS and do not truly impact the proposed Site.

3.0 HYDRAULIC ANALYSIS

The proposed site is designed to convey stormwater by overland flow to existing and proposed catch basins. At the time of this Report, the roof drain locations are not known. However, they will be connected to the existing storm drain system. The contribution of the proposed Site removes 5.24 cfs and 8.79 cfs for the 10- and 100-year conditions, respectively. Refer to Appendix C- *Hydrologic and Hydraulic Calculations*. The proposed drainage values for the Site are 33.2 cfs and 78.01 cfs for the 10- and 100-year conditions, respectively. These flows will be collected in the existing stormwater infrastructure and will be treated as outlined in Section 2.1.

4.0 MAINTENANCE

Ongoing maintenance of the designed or recommended drainage systems is required to preserve the design integrity and purpose of the drainage system. Failure to provide maintenance can prevent the drainage system from performing to its intended design purpose and can result in reduced performance. Maintenance within the public right-of-way is the responsibility of the governing municipality. However, it is the responsibility of landowners (such as private developers or property owners' associations) for facilities on private property. Prior to ultimate condition build-out upstream of drainage structures, additional maintenance may be required due to an increase in sedimentation build-up. A regular maintenance program is required to have drainage systems perform to the level of protection or service, as presented in this report and the projects' plans and specifications.

5.0 CONCLUSIONS

Based on our analysis of the Site, the following conclusions can be made:

- 1. This concept Drainage Report has been prepared in accordance with WOODPATEL's understanding of the City of Scottsdale technical drainage requirements and the *Drainage Design Manuals for Maricopa County Hydrology and Hydraulics (2018)*, as applicable to the Site.
- 2. The Site is within a FEMA designated 100-year floodplain (Zone "AO-Depth 1 foot") in both pre- and postdevelopment conditions.
- 3. The Site is protected from offsite flows from the north by improvements previously designed and constructed specifically to protect this property, as outlined in the approved stormwater storage waiver.
- 4. No stormwater retention has been provided for this project, per the approved stormwater storage waiver.
- 5. The onsite 100-year storm event is to be conveyed south, by existing storm drain and overland flow, to the existing TPC Golf Course.
- 6. The 100-year high water elevation is 1557.27 in the adjacent TPC golf course. This is 3.99 feet below the proposed Conference Center lowest finish floor elevation of 1561.26. It is our understanding this is in compliance with the City of Scottsdale floodplain ordinance, which requires the lowest finished floor to be a minimum of 2 feet above (1 foot for AO and 1 foot of freeboard) the natural highest grade.

- 7. The estimated low natural ground elevation is 1551.13 which is 10.13 feet below the proposed Conference Center lowest finished floor elevation of 1561.26. It is our understanding this is in compliance with the City of Scottsdale floodplain ordinance.
- 8. Ongoing maintenance is required for the existing drainage systems to maintain design performance. Maintenance is the responsibility of the private parties involved.

6.0 REFERENCES

- 1. Design Standards & Policies Manual, City of Scottsdale, 2018
- Drainage Report for Fairmont Scottsdale Hotel Expansion by Wood, Patel & Associates, Inc., dated May 1, 2015
- 3. Drainage Report for Fairmont Scottsdale Western Theme Town by Wood, Patel & Associates, Inc., dated May 15, 2015
- 4. Drainage Memo for Fairmont Scottsdale Sunset Beach Pool by Wood, Patel & Associated, Inc., dated September 11, 2015
- 5. Drainage Report for Fairmont Scottsdale Princess Privado Welcome Building and Parking Modifications by Wood, Patel & Associates, Inc., dated February 21, 2023
- 6. Curry's Corner Quadrangle, 7.5 Minute Series Topographic Map, USGS, 1964.

APPENDIX A – REGIONAL CONTOUR MAP / HIGHEST NATURAL GRADE ELEVATION CALCULATION



- 1) WHEN REQUIRED AS INDICATED ABOVE, FLOOD VENTS SHALL BE PROVIDED ON AT LEAST 2 SEPARATE WALLS. THE FLOOD VENTS SHALL HAVE ONE SQUARE INCH OF OPENING SPACE FOR EVERY SQUARE FOOT OF ENCLOSED SPACE BELOW THE REGULATORY FLOOD ELEVATION, OR AS NOTED ABOVE. SEE ARCHITECTURAL PLANS FOR VENTS OPENINGS. PROPOSED GRADE ADJACENT TO BUILDING MAY EFFECT VENT LOCATIONS, CONSULT ENGINEER PRIOR TO CONSTRUCTION WITH ANY QUESTIONS. 2) WHEN REQUIRED AS INDICATED ABOVE, WET FLOODPROOFING SHALL BE PROVIDED UP TO THE REGULATORY FLOOD DEPTH. WET FLOODPROOFING CONSIST OF
- CONSTRUCTION WITH FLOOD RESISTANT MATERIALS. 3) WHEN REQUIRED AS NOTED ABOVE, ELECTRICAL AND MECHANICAL EQUIPMENT SHALL BE ELEVATED ABOVE THE REGULATORY FLOOD DEPTH.
- 4) PROPOSED BUILDING M1 WILL BE A STRUCTURALLY INDEPENDENT NON-RESIDENTIAL STRUCTURE. 5) FEMA DEFINES DRY FLOODPROOFING AS A COMBINATION OF MEASURES THAT RESULT IN A STRUCTURE, INCLUDING THE ATTENDANT UTILITIES AND EQUIPMENT, BEING WATERTIGHT WITH ALL ELEMENTS SUBSTANTIALLY IMPERMEABLE TO THE ENTRANCE OF FLOODWATER AND WITH STRUCTURAL COMPONENTS HAVING THE CAPACITY TO
- RESIST FLOOD LOADS.

NOT FOR CONSTRUCTION **OR RECORDING**



LEGEND

BOUNDARY LINESECTION LINE1550ESTIMATED 5' CONTOUR NAVD88 DATUM1541ESTIMATED 1' CONTOUR NAVD88 DATUM1551.75ORIGINAL 1964 CURRYS CORNER CONTOURS ON NAVD88 DATUM
SECTION LINE1550ESTIMATED 5' CONTOUR NAVD88 DATUM1541ESTIMATED 1' CONTOUR NAVD88 DATUM1551.75ORIGINAL 1964 CURRYS CORNER CONTOURS ON NAVD88 DATUM
 — 1550 — ESTIMATED 5' CONTOUR NAVD88 DATUM — 1541 — ESTIMATED 1' CONTOUR NAVD88 DATUM 1551.75 ORIGINAL 1964 CURRYS CORNER CONTOURS ON NAVD88 DATUM
 — 1541 — ESTIMATED 1' CONTOUR NAVD88 DATUM 1551.75 ORIGINAL 1964 CURRYS CORNER CONTOURS ON NAVD88 DATUM
1551.75 ORIGINAL 1964 CURRYS CORNER CONTOURS ON NAVD88 DATUM
HAG HIGHEST ADJACENT NATURAL GRADE
LAG LOWEST ADJACENT NATURAL GRADE
RFD REGULATORY FLOOD DEPTH = HAG +2' (ZONE AO DEPTH (1') = 1' FREEBOAF
LGF LOWEST GARAGE FLOOR

ELEVATION STATEMENT

THE WORK PRODUCT PRESENTED IS THE RESULT OF OBTAINING BEST AVAILABLE HISTORICAL ELEVATION INFORMATION, AND EMPLOYING PROFESSIONAL JUDGMENT TO BEST PRESENT IN SITE GROUND ELEVATIONS. ELEVATIONS ARE BASED ON 1964 CURRYS CORNER NGVD29 DATUM CONVERTED TO NAVD88 USING MARICOPA LAND SURVEY CONVERSION OF 1.749 FT

BENCHMARK

THE VERTICAL DATUM FOR THIS EXHIBIT IS BASED ON GDACS POINT 43017-1, 3 INCH CITY OF SCOTTSDALE BRASS CAP FLUSH LOCATED ON SCOTTSDALE ROAD SOUTH OF PRINCESS DRIVE HAVING AN ELEVATION OF 1552.985, CITY OF SCOTTSDALE NAVD88 DATUM.



Horz. 1 in. = 50 ft.



FAIRMONT SCOTTSDALE PRINCESS

CONFERENCE CENTER & EVENT LAWN LOWEST FINISHED FLOOR

08/04/2023 SCALE 1" = 50' SHEET 01 OF 02 DATE JOB NO 215319 DESIGN AJS AJS DRAWN Z:\2021\215319\Project Support\Reports\Rezoning\Conference Center & Event Lawn\Drainage\Exhibits\5319.30-Lowest Floor Elevation Exhibit.dwg





APPENDIX B – CURRY'S CORNER QUADRANGLE MAP



APPENDIX C – HYDROLOGIC AND HYDRAULIC CALCULATIONS

IDF DATA FROM FCDMC NOAA – ATLAS 14 PRECIPITATION DATA



SITE I-D-F CURVE

Fairmont Scottsdale Princess - Conference Center & Event Lawn
Scottsdale AZ
215319
Darin Moore, PE

RAINFALL DEPTHS, INCHES

Duration	Average Reccurence Interval (years)													
Duration	2	5	10	25	50	100								
5-min	0.253	0.341	0.409	0.501	0.571	0.643								
10-min	0.385	0.520	0.623	0.762	0.869	0.978								
15-min	0.478	0.644	0.772	0.945	1.080	1.210								
30-min	0.643	0.867	1.040	1.270	1.450	1.630								
60-min	0.796	1.070	1.290	1.580	1.800	2.020								
2-hr	0.921	1.230	1.460	1.780	2.020	2.270								
3-hr	1.010	1.320	1.560	1.910	2.180	2.460								
6-hr	1.200	1.530	1.800	2.150	2.440	2.730								
12-hr	1.350	1.700	1.970	2.350	2.630	2.930								
24-hr	1.580	2.040	2.410	2.920	3.320	3.740								

RAINFALL INTENSITY, INCHES/HOUR

Duration	Frequency, y	Frequency, years													
minutes	2	5	10	25	50	100									
5	3.04	4.09	4.91	6.01	6.85	7.72									
10	2.31	3.12	3.74	4.57	5.21	5.87									
15	1.91	2.58	3.09	3.78	4.32	4.84									
30	1.29	1.73	2.08	2.54	2.90	3.26									
60	0.80	1.07	1.29	1.58	1.80	2.02									
120	0.46	0.62	0.73	0.89	1.01	1.14									
180	0.34	0.44	0.52	0.64	0.73	0.82									
360	0.20	0.26	0.30	0.36	0.41	0.46									
720	0.11	0.14	0.16	0.20	0.22	0.24									
1440	0.07	0.09	0.10	0.12	0.14	0.16									



5319-Drainage Workbook.xls

TABLE 1 – EXISTING WEIGHTED C VALUES



COMPOSITE WEIGHTED "C" FACTOR CALCULATIONS 100 YEAR

Project	Fairmont Scottsdale Princess - Conference Center & Event Lawn
Location	Scottsdale AZ
Project Number	215319
Project Engineer	Darin Moore, PE

Existing C Factor

Drainage Subbasin D		Desert		Paved		Roof		Grass		Commerc	100 YR Runoff Coefficient	
(Description/ID)	(Acres)	%	"C"	%	"C"	%	"C"	%	"C"	%	"C"	"C"
B1	2.26	93	0.45	7	0.95		0.95		0.30		0.86	0.49
B2	0.88		0.45	100	0.95		0.95		0.30		0.86	0.95
B3	0.93		0.45	100	0.95		0.95		0.30		0.86	0.95
B4	0.94		0.45	100	0.95		0.95		0.30		0.86	0.95
B5	0.91		0.45	100	0.95		0.95		0.30		0.86	0.95
B6	1.99	29	0.45	71	0.95		0.95		0.30		0.86	0.81
B7	0.79		0.45		0.95		0.95		0.30	100	0.86	0.86
N1	0.54	0	0.45	100	0.95		0.95		0.30		0.86	0.95
N2	0.40	0	0.45	100	0.95		0.95		0.30		0.86	0.95

 TABLE 2 – PROPOSED WEIGHTED C VALUES
 PROPOSED WEIGHTED C VALUES



COMPOSITE WEIGHTED "C" FACTOR CALCULATIONS 100 YEAR

Project	Fairmont Scottsdale Princess - Conference Center & Event Lawn
Location	Scottsdale AZ
Project Number	215319
Project Engineer	Darin Moore, PE

Proposed C Factor

Drainage Subbasin ID	ⁿ Area Desert Paved			Roof		Grass	100 YR Runoff Coefficient			
(Description/ID)	(Acres)	%	"C"	%	"C"	%	"C"	%	"C"	"C"
B1	2.16	77.6	0.45	4.4	0.95	18	0.95		0.30	0.56
B2	1.53		0.45	100	0.95		0.95		0.30	0.95
В3	1.17	3	0.45	97	0.95		0.95		0.30	0.94
В4	1.03		0.45	65.7	0.95		0.95	34.3	0.30	0.73
B5	1.44	28.4	0.45	50	0.95		0.95	21.6	0.30	0.67
B6	0.41	20	0.45	80	0.95		0.95		0.30	0.85
R1	2.10		0.45		0.95	100	0.95		0.30	0.95

TABLE 3 – EXISTING RATIONAL METHOD



Fairmont Scottsdale Princess - Conference Center & Event Lawn Project Location Scottsdale AZ Project Number 215319 **Project Engineer** Darin Moore, PE

EXISTING ON-SITE W	EXISTING ON-SITE WATERSHEDS											100 YEAR				10 YEAR			
Drainage Subbasin ID	Longest Watercourse 'L'	Longest Watercourse 'L'	Drainage Area 'A'	Drainage Area 'A'	'K₀' Type¹	Watershed Resistance Coefficient	Top Elevation	Bottom Elevation	Basin Slope 'S'	Calculated Q100 'Tc' (See Note 2)	100 YEAR Intensity 'i'	100 YR Runoff Coefficient	Q100 Flow	Calculated Q10 'Tc' (See Note 2)	10 YEAR Intensity 'i'	10 YR Runoff Coefficient	Q10 Flow		
	(ft)	(mi)	(sf)	(Acres)		'K _{b'}			(ft/mi)	(min)	(in/hr)	'C'	(cfs)	(min)	(in/hr)	'C'	(cfs)		
B1	571	0.108	98439	2.26	А	0.0378	58.5	54.0	41.6	5.5	9.31	0.49	10.2	6.6	5.88	0.49	6.4		
B2	209	0.040	38479	0.88	В	0.0807	59.5	56.3	80.8	5.0	8.03	0.95	6.7	5.1	4.91	0.95	4.1		
В3	209	0.040	40583	0.93	А	0.0402	60.0	57.2	70.7	5.0	8.45	0.95	7.5	5.0	5.22	0.95	4.6		
B4	218	0.041	41039	0.94	A	0.0402	58.0	54.7	79.9	5.0	9.45	0.95	8.5	5.0	5.22	0.95	4.7		
B5	204	0.039	39679	0.91	А	0.0403	59.0	55.5	90.6	5.0	10.45	0.95	9.0	5.0	5.22	0.95	4.5		
B6	397	0.075	86513	1.99	A	0.0381	57.4	53.0	58.5	5.0	11.45	0.95	21.6	5.2	5.22	0.95	9.8		
B7	227	0.043	34517	0.79	А	0.0406	59.0	54.9	95.4	5.0	12.45	0.81	7.9	5.0	5.22	0.81	3.3		
N1	214	0.041	23664	0.54	A	0.0417	59.0	55.0	98.7	5.0	13.45	0.86	6.3	5.0	5.22	0.86	2.4		
N2	177	0.034	17431	0.40	A	0.0425	56.9	54.0	86.5	5.0	14.45	0.95	5.5	5.0	5.22	0.95	2.0		
Total			420344	9.65									83.2				42.0		

Notes

1. Per Drainage Design Manual for Maricopa County, Vol. I, Hydrology (2013), Table 3.1: Equation for Estimating Kb in the Tc Equation

2. Minimum Tc is 5 minutes.

RATIONAL METHOD SUMMARY 100 YEAR, 10 YEAR

TABLE 4 – PROPOSED RATIONAL METHOD

STORMCEPTOR RATIONAL METHOD SUMMARY 100 YEAR, 10 YEAR



ProjectFairmont Scottsdale Princess - Conference Center & Event LawnLocationScottsdale AZProject Number215319Project EngineerDarin Moore, PE

PROPOSED ON-SITE	PROPOSED ON-SITE WATERSHEDS											100 YEAR				10 YEAR			
Drainage Subbasin ID	Longest Watercourse 'L'	Longest Watercourse 'L'	Drainage Area 'A'	Drainage Area 'A'	'K₀' Type¹	Watershed Resistance Coefficient	Top Elevation	Bottom Elevation	Basin Slope 'S'	Calculated Q100 'Tc' (See Note 2)	100 YEAR Intensity 'i'	₹ 100 YR Runoff Coefficient	Q100 Flow	Calculated Q10 'Tc' (See Note 2)	10 YEAR Intensity 'i'	10 YR Runoff Coefficient	Q10 Flow		
	(ft)	(mi)	(sf)	(Acres)		'K _{b'}			(ft/mi)	(min)	(in/hr)	'C'	(cfs)	(min)	(in/hr)	'C'	(cfs)		
B1	317	0.060	94,266	2.16	А	0.0379	61.3	52.0	154.2	5.0	9.31	0.56	11.3	5.0	5.60	0.56	6.8		
B2	269	0.051	66,759	1.53	А	0.0388	61.3	56.4	96.1	5.0	8.03	0.95	11.7	5.0	4.91	0.95	7.1		
В3	184	0.035	50,869	1.17	А	0.0396	61.3	57.1	118.5	5.0	8.45	0.95	9.4	5.0	4.91	0.95	5.4		
B4	226	0.043	44,858	1.03	А	0.0399	61.3	58.0	76.7	5.0	8.78	0.73	6.6	5.0	4.91	0.73	3.7		
B5	247	0.047	62,597	1.44	А	0.0390	61.3	56.0	112.3	5.0	9.78	0.67	9.4	5.0	4.91	0.67	4.7		
B6	159	0.030	17,955	0.41	А	0.0424	61.3	56.4	160.8	5.0	11.78	0.85	4.1	5.0	4.91	0.85	1.7		
R1	289	0.055	91,613	2.10	А	0.0380	0.0	0.0	0.2	18.8	12.78	0.95	25.5	39.2	1.84	0.95	3.7		
Total			428,918	9.85									78.01				33.19		
Nataa																			

Notes

1. Per Drainage Design Manual for Maricopa County, Vol. I, Hydrology (2013), Table 3.1: Equation for Estimating Kb in the Tc Equation

2. Minimum Tc is 5 minutes.

APPENDIX D – CONTECH DSBB-10-20-108



DEBRIS SEPARATING BAFFLE BOX SCREEN FLOW RATE CALCULATOR

Project ID:	742047
Project Name:	Fairmont Scottsdale Princess – Sunset V
Project Location:	Scottsdale, AZ
Unit ID:	
Date:	6/12/2023

Pine Diameter D	36	in.
i ipe Diameter, D	3.00	ft.
Safety Factor, SF	1	unitless
Treatment Flow Rate	1.70	cfs
Treatment now Nate	763	gpm
Water Depth in Pine, d	5	in.
water Deptir in ripe, a	0.42	ft.
Padius r	18	in.
Raulus, I	1.50	ft.
% full	13.89%	
Total Area A	1017.88	in^2
Total Alea, A	7.07	ft^2
Total Perimeter C	113.10	in.
Total Ferineter, C	9.42	ft
Wotted Area Aw	85.62	in^2
Welleu Alea, Aw	0.59	ft^2
Wattad Parimatar P	27.50	in.
Wetted I enimeter, I	2.29	ft
Hydraulic Radius, R	3.11	in.
Tryuraune Radius, R	0.259	ft
Elevation	Below	
¢	1.53	radians
S	27.50	in.

EOR/ Contractor:	Robert Saunders
Designed By:	David Hopkins
CONTECH Rep:	Zach Hubard

Step 1:	
Input Project Information above in Blue	
Step 2:	

Input Design Variables into the Green cells to the left. Input the Pipe Diameter and Safety Factor first followed by the required Treatment Flow Rate last. The Cell for the Treatment Flow Rate initiates a looped calculation once the cell value is changed. This variable should always be the last input.

Constants		
Gravity, g	32.174	ft/s ²
Discharge Coefficient, C _d	0.66	unitless
Screen Open Area, OA	0.37	%

HGL _o , HGL at Entrance of Outlet Pipe
φ, Central Angle (Theta)
T, Top Water Surface Width
A, Area of Section Flow
h _m , Mean Depth of Flow
V_o , Velocity at Entrance of Outlet Pipe
Q _o , Volumetric Flow Rate of Outlet Pipe
Froude Number

0.42	ft
43.76	deg
2.07	ft
0.59	ft²
0.29	ft
3.04	ft/s
1.81	ft³/s
1	unitless

						RES	ULTS					
Model	2.5-4-66	2.5-4-66	2.5-4-66	2.5-4-66	2.5-4-66	4-8-84	5-10-84	6-12-84	8-16-96	10-20-108	11-24-132	11-34-136
HGL (ft)	N/A	N/A	N/A	N/A	N/A	0.46	0.45	0.44	0.43	0.42	0.42	0.42
Rate (ft3/s)	N/A	N/A	N/A	N/A	N/A	1.81	1.81	1.81	1.81	1.81	1.81	1.81

This spreadsheet performs iterative calculations to determine the screened treatment flow rate and the associated maximum HGL inside of the DSBB at this treatment flow rate. The user only needs to input the required pipe size, safety factor and desired volumetric treatment flow rate. The spreadsheet is designed to incrementally increase the water elevation of the outlet pipe until the desired treatment flow rate is achieved. A simultaneous set of calculations is performed during this incremental step to determine the headloss through the DSBB as a result of the water passing through the box and the screen. The basis for these calculations is the Bernoulli Energy Equation combined with an empirically determined equation for the losses associated with the screen. Flow rate, velocity, flow area, and constants are direct factors to the outcome of these calculations.

Limitations and Restrictions on Use (Assumptions required for calculations to be valid)

- 1. Inlet and outlet pipe sizes are the same diameter.
- 2. Inlet-pipe flow is subcritical.
- 3. Flow in outlet-pipe at the exit is critical (no further restrictions down stream).
- 4. Tops of sediment partitions, inlet-pipe inverts, and outlet-pipe inverts are at the same elevation.
- 5. The DSBBB ceiling height is always above the water level.
- 6. Sediment in final chamber does not significantly restrict flow under skimmer panel (if present).
- 7. The DSBB screen channel is not significantly wider than outlet pipe diameter.
- 8. Top of basket is above water height. (This requirement does not affect these head loss calculations, but affects retention of floatable debris.)

Revision: DSBB Screen Flow Calculator Full Capture round pipe.xlsm

Calculation of Head Loss in DSBB Unit

In bypass, if screens are completely clogged.

Project Name	Fairmont Scottsdale Princess – Sunset Villas & Bung
Project #	742047
Location	Scottsdale, AZ
Completed By	DAH

Inputs:		
DSBB Size	DSBB-10-20	(Dropdown)
Inlet		
Flow (cfs)	66.8	
	•	—
Outlet		
		- · · ·

Pipe Material	HDPE	(Dropdown)
Pipe Shape	Round	(Dropdown)
Pipe Diameter (in)	36	
Box Width (in)	24	
Box Height (in)	42	

L	input design information officit in
	blue
	Step 2:
	Change light green cell in Sections
	1, 2 and 3 until "OK"

Step 1:

Section 1: 0	Within 5%?	% Error	
Depth in Pipe (ft)	2.18	ОК	2.98%
Velocity in Pipe (fps)	12.14		
EGL in Pipe (ft)	4.47		
Section 2: Ex	it Chamber	Within 5%?	% Error
HGL in Exit Chamber (ft)	5.52	ОК	1.12%
Velocity in Exit Chmbr (fps)	1.21		
Entrance Loss	1.13		
EGL in Exit Chamber (ft)	5.60		
Section 3: Hea	Within 5%?	% Error	
Length of Weir (ft)	*Note: Must	t be larger than	
Weir Submerged?	Submerged	Section	on 2 HGL
HGL Before Weir (ft)*	5.523	ОК	4.40%
Section 4: Inl	et Chamber		
HGL at Entrance Chamber (ft)	5.52		
Inside Ceiling to Invert (ft)	6.00		
Velocity at Entrance (fps)	1.21		
EGL Start of Box	5.55	1	
Total Head Loss (ft) (EGL Weir-EGL Pipe)*1.3	1.40		



Limitations and Restrictions on Use

(Assumptions required for calculations to be valid)

1. Inlet and outlet pipe sizes are the same diameter.

2. Inlet-pipe flow is subcritical.

3. Flow in outlet-pipe at the exit is critical (no further restrictions down stream).

4. Tops of sediment partitions, inlet-pipe inverts, and outlet-pipe inverts are at the same elevation.

5. Baffle-box ceiling height is always above the water level.

6. Sediment in final chamber does not significantly restrict flow under skimmer panel (if present).

7. Baffle Box is significantly wider than outlet pipe diameter.

8. Top of basket is above water height. (This requirement does not affect these head loss calcuations, but affects retention of flatable debris).

If you have any questions, please contact: Scott Sertich <u>scott.sertich@conteches.com</u>

v8.0 5/23/2023 STS



SITE S	SPECIFI	C DA	ΤA	*		D:	SBB PER	FORMA	NCE DA	ΓA
PROJECT NUMBER		74.	2047			TREATMENT	T FLOW RAT	TE (CFS)		
PROJECT NAME FAIRMONT SCOTTS		TSDALE		FULL CAPTURE TRASH FLOW RATE (CI		TE (CFS)				
PROJECT NAME FAIRMONT SCOTTS PRINCESS PROJECT LOCATION SCOTTSDALE, A STRUCTURE ID 025 WATER QUALITY FLOW RATE (CFS) 1 PROVIDED TREATMENT FLOW RATE (CFS) 2 PEAK FLOW RATE (CFS) 6 DEAK STOPH DUPATION (VEADS) 1		<u>م</u>		SETTLING /	AREA (SF)			4		
	v			, AZ		RATE (GPM/	/SF)			
STRUCTURE ID			125	1 70		SCREEN S	YSTEM STO	RAGE CAPA	CITY (CF)	
WATER QUALITY FLOW RATE (CFS)			1.70		SEDIMENT STORAGE CAPACITY (CF)					
PROVIDED TREATMEN	(OFO)	(65)		25.79		80% TSS .	REMOVAL @	0 231 MICH	RON	
PEAK FLOW RATE	(CFS)			66.80		D	SBB STO	RAGE C	APACITI	ES
PEAK STORM DUR	ATION (YEAR	5)		10.00]		CAGE .	SCREEN CA	APACITY	
PIPE DATA	PE DATA I.E. MATERIAL DIAMETER LENGTH (FT) WIDTH (FT) HEIGHT (FT)		ΤO							
INFLOW PIPE 1	1546.5	546.5 HDPE		36		SCREEN 1	11.50	3.17	2.25	
OUTFLOW PIPE 1	1546.5	HDF	PE	36		SCREEN 2	11.50	3.17	2.25	
RIM ELEVATION 1537.9					SEDIMENT	CHAMBER	CAPACITY			
SURFACE LOADING	REQUIREME	NT		HS20		CHAMBER 1	6.50	10.00	3.00	
FRAME AND COVER	7	(2) 3	6"x72	?" (1) 36"ø		CHAMBER 2	6.42	10.00	3.00	
CORROSIVE SOIL C	CORROSIVE SOIL CONDITIONS		NA		CHAMBER 3	6.42	10.00	3.00		
KNOWN GROUNDWATER ELEVATION			NA				<u> </u>			
NOTES:										
					1					



*PER ENGINEER OF RECORD

GENERAL NOTES

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS OTHERWISE NOTED.
- 2. ALL DIMENSIONS, ELEVATIONS, SPECIFICATIONS, AND CAPACITIES ARE SUBJECT TO CHANGE. FOR PROJECT SPECIFIC DRAWINGS DETAILING EXACT DIMENSIONS, WEIGHTS, AND ACCESSORIES PLEASE CONTACT CONTECH.

INSTALLATION NOTES

- 1. CONTRACTOR TO PROVIDE ALL LABOR, EQUIPMENT, MATERIALS, AND INCIDENTALS REQUIRED TO OFFLOAD AND INSTALL THE DEBRIS SEPARATING BAFFLE BOX AND APPURTENANCES IN ACCORDANCE WITH THIS DRAWING AND THE MANUFACTURER'S SPECIFICATIONS, UNLESS OTHERWISE STATED IN MANUFACTURER'S CONTRACT.
- 2. MANUFACTURER RECOMMENDS A 6" LEVEL ROCK BASE UNLESS SPECIFIED BY THE PROJECT ENGINEER. CONTRACTOR IS RESPONSIBLE FOR VERIFYING PROJECT ENGINEER'S RECOMMENDED BASE SPECIFICATIONS.
- 3. ALL PIPES MUST BE FLUSH WITH INSIDE SURFACE OF CONCRETE (PIPES CANNOT INTRUDE BEYOND FLUSH).
- 4. ALL GAPS AROUND PIPES SHALL BE SEALED WATERTIGHT WITH A NON-SHRINK GROUT PER MANUFACTURER'S STANDARD CONNECTION DETAIL AND SHALL MEET OR EXCEED REGIONAL PIPE CONNECTION STANDARDS.
- 5. CONTRACTOR RESPONSIBLE FOR INSTALLATION OF ALL PIPES, RISERS AND COVERS. ALL COVERS SHALL BE SHIPPED LOOSE. CONTRACTOR TO USE GROUT AND/OR BRICKS TO MATCH COVERS WITH FINISHED SURFACE UNLESS SPECIFIED OTHERWISE.



1.70

0.00

200.00

3.81

163.88

580.00

TOTAL (CF)

81.94

81.94

195.00

192.50

192.50



INLET **ELEVATION VIEW A**

THIS PRODUCT MAY BE PROTECTED BY ONE OR MORE OF THE FOLLOWING US PATENTS: 6,428,692; 7,294,256; 7,846,327; 7,153,417; 7,270,747. RELATED FOREIGN PATENTS OR OTHER PATENTS PENDING PROPRIETARY AND CONFIDENTIAL: THE INFORMATION CONTAINED IN THIS DOCUMENT IS THE SOLE PROPERTY OF CONTECH AND ITS COMPANIES. THIS DOCUMENT, NOR ANY PART THEREOF, MAY BE USED, REPRODUCED OR MODIFIED IN ANY MANNER WITH OUT THE WRITTEN CONSENT OF CONTECH.	CONTECH° INGINEERED SOLUTIONS LLC www.ContechE8.com
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1:60 SCALE

APPENDIX E – STORM STORAGE WAIVER / PROPOSED DRAINAGE IMPROVMENTS EXHIBIT

PA	72.5A - 2007	City of Scottsdale	Case Numbers:	DD	0-5524+20
ate 714/08 Project Name Faimonic Scattadaie Ad 82355 pplicant Contact John Bulka Company Namo Wood Palet & Associates pplicant Contact John Bulka Company Namo Wood Palet & Associates ddress 1355 N. Stepley Mass, AZ 05203 E-mail Jbulka@woodpalet.com ddress 1355 N. Stepley Mass, AZ 05203 E-mail Jbulka@woodpalet.com ddress 101 required stormwater storage. Check the applicable box and provide a signed engineering report and supporting engineering analyses that demonstrate the project mests the orderia and that the effect of a waiver will not increase the potential for flooding on any property. 1 The runoff for the project has been included in a storage facility at another location. The applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility. A The development is adjacent to a watercourse or channel that an engineering analysis shows is designed and constructed to handle the additional runoff without increasing the potential for flood damage to the subject property or to any other property. 3 The development is on a parcel less than one-half acre in size in an area where the engineering analysis demonstrates there is no significant Increase in potential for flood damage to the subject property or to any other property. Such conflicts with ESLO may include: 4 Stormwater storage requirements consti	- PA ZN applicant/developer must complete an <i>omitting Improvement plans</i> . Denial o riew Board.	d submit this form to the c of the walver may require	ity for processing and the developer to subm	f obtain approval of v nit a revised site plan	valver request before to the Development
Project Location 7575 East Princess Drive Bootstadie, A2 8525 Applicant Contact_John Bulka Company Name Wood Patel & Associates Project 4834330 Fax 480-834330 E-mail_bulka@woodpatel.com Address 1855 N. Stepley Mesa, AZ 85203 Waiver Conternal Awaiver is an intentional relinquishment of a claim or right. A project must meet at least one of six criteria listed bolow for the city to consider waiving some or all required stormwater storage. Check the applicable box and provide a signed engineering report and eupporting engineering analyses that demonstrate the project meets the criteria and that the effect of a waiver will not increase the potential for flooding on any property. 1. The runoff for the project has been included in a storage facility at another location. The applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility. 2. The development is adjacent to a watercourse or channel that an engineering analysis shows is designed and constructed to handle the additional runoff without increasing the potential for flood damage to the subject property or to any other property. 3. The development is on a parcel less than one-half acre in size in an area where the engineering analysis demonstrates there is no significant increase in potential for flood damage due to its development. 4. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO). The applicant must demonstrate there is no increased potential for flood damage to the subject property or to any other property. Such conflicts with ESLO may include: 5. The project is located within the Downtown Fee Reduction Area as described and approved by City Council Resolution #6238 (see map). The applicant must demonstrate there is no increased potential for flood damage to any property. Even if the project is located in the	e <u>7/14/08</u> Project Nam	e Fairmont Scottsdale Princ	ess Resort		
Applicant Contract_Joint Data Fax 480-834-332 Company Mater Termination Terminatin Termination Termination Terminati	ject Location 7575 East Princess Drive S	cottsdale, AZ 85255	Wood	Patel & Associates	
 Noted weaking the second stress of the sec	plicant Contact John Duka	Env 480-834-3320	E-mail [bulka@wo	odpatel.com	
 Waiver Griteria Naiver Stan Naiver Stan	Iress 1855 N. Stapley Mesa, AZ 85203				
 Waver is an intentional relinquishment of a claim or right. A project must meet at least one of six criteria listed below for the city to consider waiving some or all required stormwater storage. Check the applicable box and provide a signed engineering report and aupporting engineering analyses that demonstrate the project meets the criteria and that the effect of a waiver will not increase the protential for flooding on any property. 1. The runoff for the project has been included in a storage facility at another location. The applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility. 2. The development is adjacent to a watercourse or channel that an engineering analysis shows is designed and constructed to handle the additional runoff without increasing the potential for flood damage to the subject property or to any other property. 3. The development is on a parcel less than one-half acre in size in an area where the engineering analysis demonstrates there is no significant increase in potential for flood damage due to its development. 4. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO). The applicant must demonstrate there is no increased potential for flood damage to the subject property or to any other property. Such conflicts with ESLO may include: Total land requirements for storage basin, easements, setbacks, and NAOS prevent building allowable footprint per zoning. Tropography prevonts building storage basin. Creating a storage facility requires wash modification. Instances where the Zoning Administrator cannot allow a modification to ESL requirements. S. The project is located within the Downtown Fee Reduction Area as described and ap					
 designed conveyance facility. A.2. The development is adjacent to a watercourse or channel that an engineering analysis shows is designed and constructed to handle the additional runoff without increasing the potential for flood damage to the subject property or to any other property. 3. The development is on a parcel less than one-half acre in size in an area where the engineering analysis demonstrates there is no significant increase in potential for flood damage due to its development. 4. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO). The applicant must demonstrate there is no increased potential for flood damage to the subject property or to any other property. Such conflicts with ESLO may include: Total land requirements for storage basin. Creating a storage facility requires wash modification. Instances where the Zoning Administrator cannot allow a modification to ESL requirements. 5. The project is located within the Downtown Fee Reduction Area as described and approved by City Council Resolution #6238 (see map). The applicant must demonstrate there is no increased potential for flood damage to increased potential for flood damage to increased potential for flood damage to any property. Even if the project is located in the Downtown area, if the project creates additional potential for increased flood damage, the developer must provide alternative mitigation methods to prevent the damage. 6. The project is located within a watershed that drains directly to the Salt River Pima-Maricopa Indian Community (SRPMIC) (see map). The project must provide the pre-development peak discharge flow to the SRPMIC, and attenuate flows over and above pre-development. 	 waiver is an intentional relinquishment insider waiving some or all required sto ipporting engineering analyses that der itential for flooding on any property. 1. The runoff for the project has must demonstrate that the s from the subject property at 	of a claim or right. A proje rmwater storage. Check i nonstrate the project meet as been included in a s stormwater storage fac nd that the runoff will b	act must meet at leasing the applicable box and the applicable box and the storage facility at a solity was specifical the conveyed to this	one of six criteria is d provide a signed er the effect of a waive nother location. T ly designed to acc location through	ted below for the city to ngineering report and ir will not increase the The applicant commodate runolf an adequately
 3. The development is on a parcel less than one-half acre in size in an area where the engineering analysis demonstrates there is no significant increase in potential for flood damage due to its development. 4. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO). The applicant must demonstrate there is no increased potential for flood damage to the subject property or to any other property. Such conflicts with ESLO may include: Total land requirements for storage basin, easements, setbacks, and NAOS prevent building allowable footprint per zoning. Topography prevents building storage basin. Creating a storage facility requires wash modification. Instances where the Zoning Administrator cannot allow a modification to ESL requirements. 5. The project is located within the Downtown Fee Reduction Area as described and approved by City Council Resolution #6238 (see map). The applicant must demonstrate there is no increased potential for flood damage to any property. Even if the project is located in the Downtown area, if the project creates additional potential for increased flood damage, the developer must provide alternative mitigation methods to prevent the damage. G. The project is located within a watershed that drains directly to the Salt River Pima-Maricopa Indian Community (SRPMIC) (see map). The project must provide the pre-development peak discharge flow to the SRPMIC, and attenuate flows over and above pre-development. 	 designed conveyance facility The development is adjaced and constructed to handle to subject property or to any or 	y. ht to a watercourse or he additional runoff wi ther property.	channel that an er thout increasing th	igineering analysi e potential for floc	s shows is designed od damage to the
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 5. The project is located within the Downtown Fee Reduction Area as described and approved by City Council Resolution #6238 (see map). The applicant must demonstrate there is no increased potential for flood damage to any property. Even if the project is located in the Downtown area, if the project creates additional potential for increased flood damage, the developer must provide alternative mitigation methods to prevent the damage. 6. The project is located within a watershed that drains directly to the Salt River Pima-Maricopa Indian Community (SRPMIC) (see map). The project must provide the pre-development peak discharge flow to the SRPMIC, and attenuate flows over and above pre-development. 	 4. Stormwater storage require Ordinance (ESLO). The ap to the subject property or to Total land requirements allowable footprint per z Topography prevents b Creating a storage facil Instances where the Zo 	ments conflict with rec oplicant must demonst o any other property. s for storage basin, eau coning. uilding storage basin. Ity requires wash mod ning Administrator car	uirements of the F rate there is no inc Such conflicts with sements, setbacks lfication. anot allow a modifi	Environmentally S reased potential f ESLO may includ , and NAOS prevent cation to ESL req	ensitive Lands or flood damage le: ent building uirements.
6. The project is located within a watershed that drains directly to the Salt River Pima-Maricopa Indian Community (SRPMIC) (see map). The project must provide the pre-development peak discharge flow to the SRPMIC, and attenuate flows over and above pre-development.	5. The project is located within Council Resolution #6238 (for flood damage to any pro creates additional potential mitigation methods to prevention	n the Downtown Fee F see map). The applic operty. Even if the pro for increased flood da ent the damage.	Reduction Area as ant must demonst ject is located in th mage, the develop	described and ap rate there is no ind ne Downtown area oer must provide a	proved by City creased potential a, if the project alternative
	 The project is located within Community (SRPMIC) (see the SRPMIC, and attenuate 	n a watershed that dra map). The project m a flows over and above	ins directly to the ust provide the pre pre-development	Salt River Pima-M a-development per	laricopa Indian ak discharge flow to
By signing below, I certify that the stated project meets the waiver criteria selected above as demonstrated by the attached documentation. Multiple attached documentation. M	y signing below, I certify that the state tacked documentation.	ated project meets the v	walver criteria selec	ted above as dem $\frac{7 - 16 - 0}{Date}$	onstrated by the
		A second second			



Request for Stormwater Storage Waiver

Revision Date: 18-Jul-07



Revision Date: 18-Jui-07

CITY OF SCOTISDA	Request	for Stor	mwater	Storage	Waiver
292 PA	- <i>S.A. 200</i> 7 ZN	City of Scotts: UP	tale Case Numbers: DR	PP	PC#
	(CITY STAFF TO C	OMPLETE THIS PA	GE	
Project I	Name <u>FAIRMENT</u>	DLOTTBOANS	Princes IZES	onzi	
Check A	ppropriate Boxes:				
	Meets waiver criteria (spe	ecify): 🗆 1 🛛 🖵 2	3 4	□5 □6	
Ø	Recommend approve wa	iver.			
	Recommend <u>deny</u> waive None of waiver criteria Downstream condition Other: Explain:	r: 1 met. s prohibit waiver of	any storage.		
	Return waiver request: Insufficient data provid Other: Explain:	led.			
Recc R C C C Exp	mmended Conditions of M All storage requirements Pre development condition Other: ain: <u>In kmd improv</u>	Waiver: waived. ons must be mainta e <u>mmts ex (eu</u> l	ined. cost of in-lieu	lee	
	Waiver <u>approved</u> per al	bove conditions.			
	Waiver <u>denied</u> .	(mel).		10172/05	
	Floodplain Administrator or Des	signee		Date	
7,	Planning 147 E Indian School Road, S	& Developm Suite 105, Scottsdale	nent Services AZ 85251 • Phone:	Department 480-312-7000 • Fax:	480-312-7088

Request for Stormwater Storage Walver

Page 4 of 5

Revision Date: 18-Jul-07

ETTY OF STATE	Request	for §	Stormv	vater	Storage	Waiver	
292· 3	-7N -	City of	Scottsdale Ca	se Numbers: - DR -	- PP -	PC#	
= F.M.*				A			
		In-Lieu Fe	e and in-Kind	Contribution	IS .		
If the city it would c constructi For FY 20 annually,	grants a waiver, the do ost the city to provide t ion, landscaping, desig 007/2008, this cost is \$ but the city reserves th	eveloper is the waived s in, construct 3.22 per cu ne right to re	required to calc storage volume tion manageme bic foot of stor evise the unit c	culate and cor e, including co ent, and main mwater stored ost at any tim	ntribute an In-Lieu F osts such as land ac- tenance over a 75-y d. This unit cost will e at its sole discretio	ee based on what quisition, ear design life. be updated on.	
The Flood contribution designee	dplain Administrator co on can serve as part o must approve in-lieu f	nsiders in-l f or instead ees and in-l	ind contributio of the calculate kind contributio	ns on a case- ad In-lieu fee. ns.	by-case basis. An li The Floodplain Adi	n-kind ninistrator or	
Project N	ame Fairmont	Scotte	Idale Pri	ncess Re	sort		
The waive	ed stormwater storage	volume is a	calculated as fo	ollows:			
V =CRA; V =storm C =weigh R =100-y A =area c	V =CRA; where V =stormwater storage volume required, in cubic feet, C =weighted average runolf coefficient over disturbed area, R =100-year/2-hour precipitation depth, in feet (2,82 inches, or 0.235 feet, for all regions of Scottsdale), and A = area of disturbed ground, in square feet						
Furtherm $V_{yy} = V - V_{w} = volu$ V = volun $V_{p} = volu$	ore, V _p ; where me waived, ne required, and me provided		C = 0, 9 A = 424 - 100 V = 89, 9 $V_{\mu} = 0$ $V_{w} = 89, 9$	153 36 826			
An In-Lieu Fee will be paid, based on the following calculations and supporting documentation: In-lieu fee (\$) = V _w (cu. ft.) x \$3.22 per cubic foot = <u>254</u> , 240							
An In-Kind Contribution will be made, as follows: <u>See attachment</u> . <u>Princess</u> Drive. Bridge Reconstruction, in quantinge with approved plans.							
No In-Lieu Fee is required. Reason:							
Approve	d by: C. Ashe	izteich			10/23/	<i>⊃</i> ₿	
Floodplain	Administrator or Designee	•0			Date		
7447	Planning E Indian School Road, S	& Deve Suite 105, Sc	elopment ottsdale, AZ 85	Services	Department 480-312-7000 • Fax:	480-312-7088	

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Request for Stormwater Storage Waiver

Page 5 of 5



October 23, 2008 WP# 072910 Sheet 1 of 2

Attachment to Stormwater Storage Waiver Request for Fairmont Scottsdale Princess Resort & Regional Flood Control

The Fairmont Scottsdale Princess Resort (Site) is a 60 acre resort located near the southwest corner of Princess Boulevard and Princess Drive. The Site is bounded by the Princess Blvd, to the north, the Maravilla Scottsdale Senior Living Community to the east, the TPC Golf Course to the south and existing residential developments to the west (see Exhibit 1, attached). The existing Fairmont Scottsdale Princess Resort consists of multiple hotel buildings, a ballroom, spa, tennis cottages, tennis courts, and parking. A majority of the site is developed and portions are being updated and renovated. At the north end of the site there is a 9.75 acre portion of the property that has yet to be developed, and other portions are scheduled for upgrades.

It is Wood/Patel's understanding that the ownership of the Fairmont Scottsdale Princess Resort, Strategic Hotels and Resorts, has agreed to fund regional flood control improvements to the public road/channel crossing at Princess Blvd and Scottsdale Road, in return for the City approving this waiver and it being applicable to the entire site. The improvements consist of removing the existing concrete box culvert crossing and replacing it with a bridge structure. The cost of a new bridge structure is estimated at \$1,053,000.

City of Scottsdale In-Lieu Fees: V(req) Volume required = CRA = (0.90) x (0.235 feet) x (9.75 acres) = 89,826 cu-ft. C (Runoff Coefficient) = 0.90 R (100-year/2-hour precipitation depth) = 0.235 feet Site area = 9.75 acres

City of Scottsdale In-Lieu Fees = $V(req) \times 3.22 = (89,826 \text{ cu-ft}) \times 3.22 = 289,240$

Summary: Public Drainage Improvements = \$1,053,000 (*) City of Scottsdale in Lieu Fee = \$289,240

(*) See Sheet 2 of 2 Engineering Preliminary Opinion of Probable Cost

Attachment to Stormwater Storage Waiver Request

October 23, 2008 WP# 072910 Sheet 2 of 2

Attachment to Stormwater Storage Waiver Request for Fairmont Scottsdale Princess Resort & Regional Flood Control

Engineering Preliminary Opinion of Probable Cost (*)

Prepared Bridge Structure at Princess Drive, just east of Scottsdale Road serving unnamed wash.

Estimated Bridge Surface = 8,100 square fect x \$130/s.f. \$1,053,000

(*) Offered without the benefit of construction documents and specifications.


APPENDIX F – FAIRMONT SCOTTSDALE - BALLROOM ADDITION BY WOOD, PATEL & ASSOCIATES, INC., DATED SEPTEMBER 2, 2011

CITY OF SCOTTSDALE GENERAL NOTES: GENERAL CONSTRUCTION NOTES FOR PUBLIC WORKS CONSTRUCTION

- 1. ALL CONSTRUCTION IN THE PUBLIC RIGHTS-OF-WAY OR IN EASEMENTS GRANTED FOR PUBLIC USE MUST CONFORM TO THE LATEST MARICOPA ASSOCIATION OF GOVERNMENTS (MAG) UNIFORM STANDARD SPECIFICATIONS AND UNIFORM STANDARD DETAILS FOR PUBLIC WORKS CONSTRUCTION AS AMENDED BY THE LATEST VERSION OF THE CITY OF SCOTTSDALE SUPPLEMENTAL STANDARD SPECIFICATIONS AND SUPPLEMENTAL STANDARD DETAILS. IF THERE IS A CONFLICT, THE CITY'S
- SUPPLEMENTAL STANDARD DETAILS WILL GOVERN. 2. THE CITY ONLY APPROVES THE SCOPE, NOT THE DETAIL, OF ENGINEERING DESIGNS; THEREFORE, IF CONSTRUCTION QUANTITIES ARE
- SHOWN ON THESE PLANS, THEY ARE NOT VERIFIED BY THE CITY. 3. THE APPROVAL OF PLANS IS VALID FOR SIX (6) MONTHS. IF AN ENCROACHMENT PERMIT FOR THE CONSTRUCTION HAS NOT BEEN ISSUED WITHIN SIX MONTHS, THE PLANS MUST BE RESUBMITTED TO THE CITY FOR REAPPROVAL.
- 4. A PUBLIC WORKS INSPECTOR WILL INSPECT ALL WORKS WITHIN THE CITY OF SCOTTSDALE RIGHTS-OF-WAY AND IN EASEMENTS. NOTIFY INSPECTION SERVICES 24 HOURS PRIOR TO BEGINNING CONSTRUCTION BY CALLING 480-312-5750.
- WHENEVER EXCAVATION IS NECESSARY, CALL THE BLUE STAKE CENTER 602-263-1100, TWO WORKING DAYS BEFORE EXCAVATION BEGINS. THE CENTER WILL SEE THAT THE LOCATION OF THE UNDERGROUND UTILITY
- LINES IS IDENTIFIED FOR THE PROJECT. CALL "COLLECT" IF NECESSARY ENCROACHMENT PERMITS ARE REQUIRED FOR ALL WORK IN PUBLIC RIGHTS-OF-WAY AND EASEMENTS GRANTED FOR PUBLIC PURPOSES. AN ENCROACHMENT PERMIT WILL BE ISSUED BY THE CITY ONLY AFTER THE REGISTRANT HAS PAID A BASE FEE PLUS A FEE FOR INSPECTION SERVICES. COPIES OF ALL PERMITS MUST BE RETAINED ON-SITE AND BE AVAILABLE FOR INSPECTION AT ALL TIMES. FAILURE TO PRODUCE THE REQUIRED PERMITS WILL RESULT IN IMMEDIATE SUSPENSION OF ALL
- WORK UNTIL THE PROPER PERMIT DOCUMENTATION IS OBTAINED. ALL EXCAVATION AND GRADING THAT IS NOT IN THE PUBLIC RIGHTS-OF-WAY OR NOT IN EASEMENTS GRANTED FOR PUBLIC USE MUST CONFORM TO CHAPTER 70, EXCAVATION AND GRADING, OF THE LATEST EDITION OF THE UNIFORM BUILDING CODE PREPARED BY THE INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS. A PERMIT FOR THIS GRADING MUST BE SECURED FROM THE CITY FOR A FEE ESTABLISHED
- BY THE UNIFORM BUILDING CODE. 8. SIGNS REQUIRE SEPARATE APPROVALS AND PERMITS.

FIRE NOTE:

ALL PRIVATE STREETS AND DRIVES SHALL CONFORM TO THE FIRE DEPARTMENT GUIDELINES FOR EMERGENCY VEHICLE ACCESS.

SEWER NOTE:

1. THE SEWER SYSTEM CONSTRUCTED BY THIS PLAN SET IS A PRIVATE SYSTEM AND WILL NOT BE MAINTAINED BY THE CITY OF SCOTTSDALE. 2. MAINTENANCE IS THE RESPONSIBILITY OF THE OWNER.

WATER NOTE:

1. THE WATER SYSTEM SHOWN HEREIN HAS BEEN DESIGNED TO ADEQUATELY SUPPLY WATER IN SUFFICIENT QUANTITY AND PRESSURE TO MEET LOCAL FIRE REQUIREMENTS.

UTILITY NOTES:

- 1. THESE PLANS HAVE BEEN SUBMITTED TO THE FOLLOWING UTILITY COMPANIES AND THE WORK CONTAINED IN THESE PLANS HAS BEEN APPROVED BY THESE COMPANIES WITHIN THEIR AREA OF INTEREST. THE SIZE AND LOCATIONS, AS SHOWN, OF THE GAS, TELEPHONE AND POWER LINES, AND CONNECTIONS AGREE WITH THE INFORMATION CONTAINED IN THE UTILITY COMPANY'S RECORDS. WHERE THE WORK TO BE DONE CONFLICTS WITH ANY OF THESE UTILITIES, THE CONFLICTS SHALL BE RESOLVED AS SPECIFIED IN THE SPECIAL PROVISIONS AND/OR AS OTHERWISE NOTED ON THESE PLANS. CONFLICTS ARISING DURING THE COURSE OF CONSTRUCTION FROM UNFORESEEN CIRCUMSTANCES SHALL BE REPORTED TO THE INTERESTED UTILITY COMPANY AND BE RESOLVED BY THEM AND THE DESIGN ENGINEER.
- THE CITY WILL NOT PARTICIPATE IN THE COST OF CONSTRUCTION OR UTILITY RELOCATION.

ACCESSIBILITY NOTES:

- 1. ACCESSIBLE ENTRANCES TO THE BUILDING SHALL BE IDENTIFIED BY THE
- INTERNATIONAL SYMBOL OF ACCESSIBILITY. 2. EXTERIOR EXITS WHICH ARE LOCATED ADJACENT TO ACCESSIBLE AREAS
- AND WITHIN 6 FT. OF ADJACENT GROUND LEVEL SHALL BE ACCESSIBLE. 3. ACCESSIBLE RAMPS REQUIRED BY ANSI SHALL NOT HAVE SLOPES THAT
- EXCEED 1FT. IN 12 FT.
- 4. THE SURFACE OF RAMPS AND GROUND SURFACES SHALL BE ROUGHENED OR SHALL BE OF SLIP RESISTANT MATERIALS.
- 5. AN ACCESSIBLE ROUTE OF TRAVEL 3 FT. WIDE MIN. MUST BE PROVIDED TO ALL PORTIONS OF THE BUILDING, BETWEEN THE BUILDING AND THE PUBLIC WAY. ACCESSIBLE ROUTES SHALL HAVE A MAXIMUM SLOPE OF 1:20 AND A MAXIMUM CROSS SLOPE OF 1:50.
- THRESHOLD MUST BE 6 INCHES IN HEIGHT OR LESS.
- 7. THE PRIMARY ENTRANCE TO THE BUILDING MUST BE ACCESSIBLE. ALL OTHER REQUIRED ENTRANCES MUST BE ACCESSIBLE. 8. ALL ACCESSIBLE PARKING SPACES SHALL HAVE A SLOPE NOT
- EXCEEDING 1:50.
- 9. ALL ACCESSIBLE PARKING SPACES SHALL BE OUTLINED ON ALL FOUR SIDES. HAVE A CONTRASTING COLOR AND THE INTERNATIONAL WHEELCHAIR SYMBOL ON THE GROUND WITHIN THE SPACE.
- 10. ALL ACCESSIBLE PARKING SPACES SHALL HAVE A SIGN (MINIMUM 5 FT. ABOVE FINISH GRADE IN FRONT OF THE SPACEO WHICH INCLUDED THE INTERNATIONAL SYMBOL OF ACCESSIBILITY. 11. SIGNS DESIGNATING PERMANENT ROOMS AND SPACES SHALL MEET
- ACCESSIBILITY REQUIREMENTS.
- 12. ALL ELECTRICAL RECEPTACLES AND CONTROLS SHALL BE 18 INCHES MINIMUM AND 48 INCHES MAXIMUM ABOVE FINISHED FLOOR.
- 13. ALL ALARMS SHALL BE AUDIBLE AND VISUAL, MEETING ACCESSIBILITY
- **REQUIREMENTS.** 14. ACCESSIBLE ROUTE SHALL BE WITHOUT STEPS OF CHANGES IN LEVEL GREATER THAN 6 INCHES WITHOUT AN APPROVED RAMP.
- 15. ACCESSIBLE ROUTES SHALL SERVE AS EXITS OR CONNECT TO AREAS OF RESCUE ASSISTANCE.

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	W.B.P.	WATER BACKFLOW PREVENTOR
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FAIRMONT SCOTTSDALE **BALLROOM ADDITION**

LOCATED IN

A PORTION OF SECTION 35, T.4.N., R.4.E., OF THE G. & S.R.M., MARICOPA COUNTY, ARIZONA 7501 EAST PRINCESS BOULEVARD, SCOTTSDALE, AZ. 85255

INDEMNITY

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SE

- CITY OF SCOTTSDALE WILL NOT BE RESPONSIBLE FOR REMOVAL REPAIR, OR REPLACEMENT OF SIDEWALKS, LANDSCAPING OR ANY OTHER IMPROVEMENTS LOCATED WITHIN CITY EASEMENT(S) AS A RESULT OF ACCESS TO MAINTENANCE OF, OR REPAIRS TO THE WATERLINE SHOWN ON THESE PLANS.
- 2. CITY OF SCOTTSDALE WILL NOT BE RESPONSIBLE FOR REMOVAL REPAIR, OR REPLACEMENT OF THE RETAINING WALLS OR OTHER IMPROVEMENTS WITHIN CITY EASEMENT(S) AS A RESULT OF ACCESS TO, MAINTENANCE OF, OR REPAIRS TO THE RETAINING WALLS SURROUNDING THE DETENTION BASINS SHOWN ON THESE PLANS.

AS-BUILT CERTIFICATION

I HEREBY CERTIFY THAT THE "AS-BUILT" MEASUREMENTS AS SHOWN OR NOTED HEREON WERE MADE BY MYSELF OF UNDER MY SUPERVISION AND ARE CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF.

NATURE			DATE	
AL				

ENGINEER'S CERTIFICATION:

ENGINEER'S CERTIFICATION: THE LOWEST FLOOR ELEVATION(S) AND/OR FLOOD PROOFING ELEVATION(S) ON THIS PLAN ARE SUFFICIENTLY HIGH TO PROVIDE PROTECTION FROM FLOODING CAUSED BY ONE-HUNDRED YEAR STORM, AND ARE IN ACCORDANCE WITH CITY OF SCOTTSDALE REVISED CODE, CHAPTER 37-FLOODWAYS & FLOOD PLAINS ORDINANCE.

"THE ENGINEER OF RECORD ON THESE PLANS HAS RECEIVED A COPY OF THE APPROVED STIPULATIONS FOR THIS PROJECT AND HAS DESIGNED THESE PLANS IN CONFORMANCE WITH THE APPROVED STIPULATIONS."

ENGINGER: SCOTTGAUDSLEY, P.E.

	NO CONFLICT	SIGNATURE	BLOCK		
UTILITY	UTILITY COMPANY	NAME OF COMPANY REPRESENTATIVE	TELEPHONE NUMBER	DA' SIGI	
ELECTRIC	ARIZONA PUBLIC SERVICE	SCOTT TIMAR	602-493-4421	8-1	
TELEPHONE	QWEST COMMUNICATIONS	JOHN NEVLIS	602-630-6891	8-3	
NATURAL GAS	SOUTHWEST GAS	ZACH STEVENSON	602-861-1899	7-2	
CABLE TV	COX COMMUNICATIONS	TRAFFIC MANAGEMENT	623-322-7086	7-2	
-	SALT RIVER PROJECT	MATT STREEPER	-	7-2	
		-	-	-	

ENGINEER'S CERTIFICATION

SCOTT AUDSLEY, P.E. , BEING THE ENGINEER OF RECORD FOR THIS DEVELOPMENT, HEREBY CERTIFY THAT ALL UTILITY COMPANIES LISTED ABOVE HAVE BEEN PROVIDED FINAL IMPROVEMENT PLANS FOR REVIEW, AND THAT ALL CONFLICTS IDENTIFIED BY THE UTILITIES HAVE BEEN RESOLVED. IN ADDITION, "NO CONFLICT" FORMS HAVE BEEN OBTAINED FROM EACH UTILITY COMPANY AND ARE INCLUDED IN THIS SUBMITTAL. HTA.

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EVIEW & RI	ECOMMENDED APPROVAL BY:			
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& D	- Jot the 9/2/11	PLANNING	Th. Nucleon	9/2/11

W & S	Joseph O monio	FIRE	NO	for 3/2/11
Flood RET. WALLS	peph D monio 9/2/11			
ENGINEERING	COORDINATOR MGR. (OR DES	SIGNEE)		9/2/11 DATE

COMMUNITY 0401

FLOOD ACCORDING TO FEMA FLOOD INSURANCE RATE MAP NO. 04013C1245H DATED SEPTEMBER 30, 2005, THE SUBJECT PROPERTY IS LOCATED IN ZONE AO. ZONE AO IS DESCRIBED AS: "FLOOD DEPTHS OF 1 TO 3 FEET (USUALLY SHEET FLOW ON SLOPING TERRAIN); AVERAGE DEPTHS TO BE DETERMINED. FOR AREAS OF ALLUVIAL FAN FLOODING VELOCITIES ALSO DETERMINED."



M.C.E.S.D. APPROVAL:

ALL POTABLE WATERLINES AND FITTINGS SHALL HAVE A NSF-PW SEAL. ALL MATERIALS AND PRODUCTS USED IN THE POTABLE WATER SYSTEM SHALL CONFORM TO NSF STANDARDS 60 AND 61 IN ACCORDANCE WITH AAC R18-4-213. ALL MATERIALS SHALL BE LEAD FREE AS DEFINED IN AAC R18-5-504 AND R18-4-101. DWR-11-00195

left # wwR-11-00113 MARICOPA COUNTY ENVIRONMENTAL SERVICES DEPARTMENT

15/11 DATE :



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DEVELOPER:

STRATEGIC HOTELS AND RESORTS 200 W. MADISON, SUITE 1700 CHICAGO, ILLINOIS 60606 CONTACT: MR. MICHAEL DALTON PH: (312) 658–6016

ARCHITECT:

KOLLIN ALTOMARE ARCHITECTS 1350 CORONADO AVENUE LONG BEACH, CALIFORNIA 90804 CONTACT: MR. PAUL ALTOMARE, AIA PH: (562) 597-8760

ENGINEER: WOOD/PATEL AND ASSOCIATES

2051 WEST NORTHERN AVENUE PHOENIX, ARIZONA 85021 CONTACT: DARIN L. MOORE, P.E.

SCOTTSDALE, AZ 85255

APN # - 215-08-001 Y, 215-08-001 X 215-08-133, 215-08-001 M

GROSS LOT (PARCEL) AREA - 34.87 ACRES NET PROJECT AREA - 14± ACRES

BEFORE YOU DIG

(602) 263-1100

H-800-STAKE-IT

(OUTSIDE MARICOPA COUNTY)



ENGINEERS NOTES

- 1. MARICOPA ASSOCIATION OF GOVERNMENTS (M.A.G.) UNIFORM STANDARD SPECIFICATIONS AND DETAILS FOR PUBLIC WORKS CONSTRUCTION (LATEST EDITION INCLUDING LATEST REVISION AND CURRENT SUPPLEMENTALS THEREOF PER THE LOCAL TOWN OR CITY) ARE
- INCORPORATED INTO THIS PLAN IN THEIR ENTIRETY. ALL WORK REQUIRED TO COMPLETE THE CONSTRUCTION COVERED BY THIS PLAN SHALL BE IN ACCORDANCE WITH THE M.A.G. STANDARD SPECIFICATIONS AND DETAILS AND CURRENT SUPPLEMENTS THEREOF PER THE LOCAL CITY OR TOWN UNLESS SPECIFIED OTHERWISE IN THESE PLANS OR ELSEWHERE IN THE CONTRACT DOCUMENTS. CONTRACTORS SHALL FAMILIARIZE THEMSELVES WITH ALL REQUIRED STANDARD SPECIFICATIONS, DETAILS AND SUPPLEMENTS PRIOR TO BIDDING THE
- WORK FOR THE CONSTRUCTION COVERED BY THIS PLAN. 3. THE CONTRACTOR IS RESPONSIBLE FOR ALL METHODS, SEQUENCING, AND SAFETY CONCERNS ASSOCIATED WITH THIS PROJECT DURING CONSTRUCTION, UNLESS SPECIFICALLY ADDRESSED OTHERWISE IN THIS PLAN OR ELSEWHERE IN THE CONTRACT.
- 4. THE CONTRACTOR IS TO COMPLY WITH ALL LOCAL, STATE, AND FEDERAL LAWS AND REGULATIONS APPLICABLE TO THE CONSTRUCTION COVERED BY THIS PLAN.
- 5. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND COMPLYING WITH ALL PERMITS REQUIRED TO COMPLETE ALL WORK COVERED BY THIS
- 6. THE QUANTITIES AND SITE CONDITIONS DEPICTED IN THESE PLANS ARE FOR INFORMATIONAL PURPOSES ONLY AND ARE SUBJECT TO ERROR AND OMISSION. CONTRACTORS SHALL SATISFY THEMSELVES AS TO ACTUAL QUANTITIES AND SITE CONDITIONS PRIOR TO BIDDING THE WORK FOR THE CONSTRUCTION COVERED BY THIS PLAN.
- 7. A REASONABLE EFFORT HAS BEEN MADE TO SHOW THE LOCATIONS OF EXISTING UNDERGROUND FACILITIES AND UTILITIES IN THE CONSTRUCTION AREA. THE CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE TO UTILITIES AND/OR FACILITIES CAUSED DURING THEIR CONSTRUCTION OPERATIONS. THE CONTRACTOR SHALL CALL 48 HOURS IN ADVANCE FOR BLUE STAKE (1-800-STAKE-IT) PRIOR TO ANY EXCAVATION.
- 8. THE CONTRACTOR IS RESPONSIBLE FOR ALL COORDINATION OF CONSTRUCTION AFFECTING UTILITIES AND THE COORDINATION OF ANY NECESSARY UTILITY RELOCATION WORK.
- 9. ALL PAVING, GRADING, EXCAVATION, TRENCHING, PIPE BEDDING, CUT FILL AND BACKFILL SHALL COMPLY WITH THE RECOMMENDATIONS SET FORTH IN THE SOILS (GEOTECHNICAL) REPORT FOR THIS PROJECT IN ADDITION TO THE REFERENCED REQUIRED SPECIFICATIONS AND DETAILS. THE CONTRACTOR SHALL BE AWARE THAT CERTAIN UTILITIES REQUIRE PROPER ATTENTION AND CAREFUL PLANNING DURING SITE CONSTRUCTION. PLEASE NOTE THAT UTILITIES ON THESE PLANS MAY NOT EXHIBIT THE FULL PROTECTIVE COVER REQUIRED DURING THE SUBGRADE PREPARATION PHASE OF THE CONSTRUCTION. IN SUCH INSTANCES, THE CONTRACTOR SHALL PROVIDE ADDITIONAL PROTECTION (SUCH AS RAMPING) OR INCREASED PIPE STRENGTH TO PROVIDE THE NECESSARY PROTECTION REQUIRED TO PREVENT DAMAGE DURING THE CONSTRUCTION OF THIS PROJECT. THE CONTRACTOR SHALL HOLD THE ENGINEER HARMLESS IN ALL CASES FOR DAMAGES TO UTILITIES WHERE INADEQUATE PROTECTIVE MEASURES OCCUR.
- 10. THE CONTRACTOR IS TO VERIFY THE LOCATION AND THE ELEVATIONS OF ALL EXISTING UTILITIES AT POINTS OF TIE-IN PRIOR TO COMMENCING ANY NEW CONSTRUCTION. SHOULD ANY LOCATION OR ELEVATION DIFFER FROM THAT SHOWN ON THESE PLANS, THE CONTRACTOR SHALL CONTACT THE OWNER'S AGENT.
- 11. CONTRACTOR TO VERIFY AND COORDINATE ALL DIMENSIONS AND SITE LAYOUT WITH ARCHITECT'S FINAL SITE PLAN AND FINAL BUILDING DIMENSIONS BEFORE STARTING WORK. REPORT DISCREPANCIES TO OWNER'S AGENT.
- 12. COORDINATION BETWEEN ALL PARTIES IS ESSENTIAL PART OF CONTRACT. 13. CONTRACTOR IS RESPONSIBLE FOR PROJECT AND SITE CONDITIONS, AND TO WORK WITH WEATHER CONDITIONS AS THE PROJECT SITE MAY BE LOCATED IN A FLOOD PRONE AREA AND SUBJECT TO FLOODING AND ITS HAZARDS.
- 14. THE CONTRACTOR IS TO VERIFY THE LOCATION, ELEVATION, CONDITION, AND PAVEMENT CROSS-SLOPE OF ALL EXISTING SURFACES AT POINTS OF TIE-IN AND MATCHING, PRIOR TO COMMENCEMENT OF GRADING, PAVING, CURB AND GUTTER, OR OTHER SURFACE CONSTRUCTION. SHOULD EXISTING LOCATIONS, ELEVATIONS, CONDITION, OR PAVEMENT CROSS-SLOPE DIFFER FROM THAT SHOWN ON THESE PLANS, RESULTING IN THE DESIGN INTENT REFLECTED ON THESE PLANS NOT ABLE TO BE CONSTRUCTED, THE CONTRACTOR SHALL NOTIFY THE OWNER'S AGENT IMMEDIATELY FOR DIRECTION ON HOW TO PROCEED PRIOR TO COMMENCEMENT OF CONSTRUCTION. THE CONTRACTOR ACCEPTS RESPONSIBILITY FOR ALL COSTS ASSOCIATED WITH CORRECTIVE ACTION IF THESE PROCEDURES ARE NOT FOLLOWED.
- 15. CONTRACTOR IS RESPONSIBLE TO COORDINATE UTILITY CROSSINGS AT CULVERT CROSSINGS BEFORE STARTING WORK ON CULVERT. COORDINATE WITH OWNER REPRESENTATIVE. VERIFY UTILITY LINES AND/OR CONDUITS ARE IN PLACE BEFORE STARTING CULVERT WORK.
- 16. CONSTRUCT RETENTION BASIN AS SHOWN. CONTRACTOR TO SCARIFY BOTTOM OF BASIN TWO FEET DEEP AND NOT ALLOW COMPACTION OVER
- 17. THIS PROJECT REQUIRES A REGULAR ONGOING MAINTENANCE PROGRAM FOR THE DESIGNED DRAINAGE SYSTEM(S) TO PRESERVE THE DESIGN INTEGRITY AND THE ABILITY TO PERFORM ITS OPERATIONAL INTENT FAILURE TO PROVIDE MAINTENANCE WILL JEOPARDIZE THE DRAINAGE SYSTEM(S)' PERFORMANCE AND MAY LEAD TO IT'S INABILITY TO PERFORM PROPERLY AND/OR CAUSE DAMAGE ELSEWHERE IN THE PROJECT.
- 18. SEWER LINES DESIGNED IN PROFILE AND PUBLIC WATER LINES ARE REQUIRED TO BE ASBUILT AND THE INSTALLATION AND TESTING WITNESSED BY A PROFESSIONAL ENGINEER IN ACCORDANCE WITH ARIZONA ADMINISTRATIVE CODES R18-9-E301 "4.01 GENERAL PERMIT: SEWAGE COLLECTIONS SYSTEMS" AND R18-5-507 AND 508 "APPROVAL OF CONSTRUCTION" AND "RECORD DRAWINGS", RESPECTIVELY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY OWNER 72 HOURS IN ADVANCE WHEN THOSE SYSTEMS ARE READY TO BE WITNESSED.
- 19. THE WORK PRODUCT PRESENTED IS BELIEVED TO BE COMPLIANT WITH THE INTENT OF THE CURRENT AMERICANS DISABILITIES ACT (ADA) REQUIREMENTS AS INTERPRETED BY THE REVIEWING AGENCY(S). IF CONSTRUCTION OF THE PROJECT IS DELAYED, THIS WORK PRODUCT SHOULD BE UPDATED TO ACCOUNT FOR ANY RELEVANT ADA UPDATES BEFORE CONSTRUCTION BEGINS.
- 20. LOWEST FLOOR (LF) REFERS TO EITHER FLOOR/SLAB ELEVATION OR TOP OF BASEMENT SLAB. LF ELEVATIONS ON THE GRADING AND DRAINAGE PLANS FOR RESIDENTIAL UNITS REFLECT SLAB ON GRADE CONDITIONS AND CANNOT BE LOWERED WITHOUT AGENCY APPROVAL IN LOCATIONS WHERE 'SPECIAL FLOOD HAZARD AREAS' EXIST. IN NON-FLOOD HAZARD LOCATIONS, TO ENSURE THAT ADEQUATE RESIDENTIAL LOT DRAINAGE CAN BE ACHIEVED, A PROFESSIONAL ENGINEER SHOULD BE CONSULTED IF THE LF FOR THE SLAB IS PROPOSED TO BE LOWERED, OR IF A BASEMENT IS TO BE CONSTRUCTED.

SOILS REPORT NOTE

SIGNS & MARKING NOTES

- MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, HTTP: //MUTCD.FHWA.DOT.GOV/INDEX.HTM.
- TUBING POSTS PER COS STANDARD DETAIL NO. 2131,
- WWW.SCOTTSDALEAZ.GOV/DESIGN/COSMAGSUPP.
- CONDITIONS EXIST.
- A PAVED SHOULDER AREA IS PRESENT.
 - B. WHERE ON-STREET PARKING COULD BE EXPECTED TO OCCUR, SUCH AS THE DIRECTION OF THE RESTRICTION SHALL BE INSTALLED
 - SHALL BE .090" (90 MIL) EXTRUDED THERMOPLASTIC, UNLESS OTHERWISE
 - NOTED ON THE PLANS.
 - OTHERWISE ON THE PLANS. THE LINE OR CENTER OF THE DOUBLE LINE.

 - 10. RAISED PAVEMENT MARKERS (RPMS) SHALL BE USED ON ALL STRIPED ADHESIVE.
 - CONNECTIONS, PER COS STANDARD DETAIL NO. 2363, WWW.SCOTTSDALEAZ.GOV/DESIGN/COSMAGSUPP.

 - COMPLY WITH THE COS STANDARD DETAIL NO. 2134-4.
 - INSTALLING PAVEMENT STRIPING, MARKING AND MARKERS.

1. CONTRACTOR SHALL REFER TO THE GEOTECHNICAL REPORT PREPARED BY ALPHA GEOTECHNICAL & MATERIALS, INC. DATED FEBRUARY 4, 2011 & ALL SUBSEQUENT ADDENDUMS FOR ALL PAVING, GRADING. EXCAVATION, TRENCHING, PIPE BEDDING, CUT, FILL AND BACKFILL

1. ALL PAVEMENT MARKINGS, SIGNING AND WORK ZONE TRAFFIC CONTROL TYPE AND LAYOUT NEED TO CONFORM TO THE LATEST EDITION OF THE

2. WORK ZONE TRAFFIC CONTROL NEEDS TO CONFORM TO THE CITY OF PHOENIX TRAFFIC BARRICADE MANUAL AND/OR AS DIRECTED BY THE CITY PUBLIC WORKS INSPECTOR OR TRAFFIC ENGINEERING DIVISION. 3. SIGNS ARE TO BE INSTALLED ON TELESPAR PREPUNCHED SQUARE STEEL

4. DIMENSIONS TO SIGNS NEED TO INCLUDE THE SIGN POST, OR IN THE CASE OF MULTIPLE POSTS, THE PLAN VIEW CENTER OF THE SIGN. 5. 'NO PARKING" SIGNS SHALL ONLY BE USED WHEN THE FOLLOWING SITE

A. WHEN ANY RIGHT HAND LANE (CURB LANE) IS 16 FEET OR WIDER, OR IF

COMMERCIAL AREAS WHERE BUSINESSES HAVE DIRECT FRONTAGE ON THE STREET. WHEN THE ABOVE CRITERIA EXISTS 'NO PARKING' SIGNS (R8-3A 12 INCH X 18 INCH) WITH AN ARROW (SINGLE DIRECTION OR BI-DIRECTIONAL) BELOW THE 'P'SYMBOL ON THE SIGN TO DESIGNATE

APPROXIMATELY EVERY 350-400 FEET ALONG THE LENGTH OF THE PROJECT. NO PARKING SIGNS SHALL BE INSTALLED APPROXIMATELY 5 FEET FROM THE BACK OF CURB AT A 45 DEGREE ANGLE TO THE CURB. STREET LIGHT POLES SHOULD BE USED FOR SIGN MOUNTING WHEN A LIGHT POLE IS WITHIN 50 FEET OF THE PROPOSED SIGN LOCATION. 6. ALL LONGITUDINAL STRIPING (EDGE LINE, LANE LINE AND CENTERLINE)

7. ALL TRANSVERSE STRIPING (STOP LINES, CROSSWALK LINES) SHALL BE A MINIMUM OF .090" (90 MIL) EXTRUDED THERMOPLASTIC, UNLESS NOTED

8. ALL PLAN VIEW STRIPING DIMENSIONS ARE MEASURED TO THE CENTER OF

9. ALL PAVEMENT SYMBOLS, ARROWS AND LEGENDS SHALL BE TYPE 1 PERMANENT, HIGH PERFORMANCE PREFORMED PAVEMENT TAPE. (TAPE MUST PERFORM AS 3M 380I-ES SERIES OR EQUIVALENT.)

STREETS. RPMS SHALL BE INSTALLED PER COS STANDARD DETAIL NO. 2132, WWW.SCOTTSDALEAZ.GOV/DESIGN/COSMAGSUPP, AND ADOT STANDARD DRAWING M-19, WITH A CITY APPROVED BITUMINOUS

11. BLUE TYPE F (2-WAY REFLECTIVE) RPMS SHALL BE USED TO INDICATE THE LOCATION OF ALL FIRE HYDRANTS AND REMOTE FIRE DEPARTMENT

12. ALL EXISTING PAVEMENT MARKINGS THAT CONFLICT WITH PROPOSED MARKINGS SHALL BE REMOVED BY SANDBLASTING. HYDROBLASTING OR GRINDING PRIOR TO THE INSTALLATION OF NEW PAVEMENT MARKINGS. REMOVALS SHALL BE TO THE SATISFACTION OF THE CITY INSPECTOR 13. ASTM TYPE IV SHEETING (MINIMUM) SHALL BE USED FOR ALL WARNING AND REGULATORY AND STREET NAME SIGNS. ALL ADVANCE STREET NAME SIGNS SHALL BE PROPOSED TYPE XI SHEETING. SCHOOL WARNING SIGNS AND ACCOMPANYING PLACARDS MUST BE ASTM PROPOSED TYPE XI FLUORESCENT YELLOW GREEN SHEETING. ALL METRO SIGNS SHALL

14. THE CONTRACTOR IS RESPONSIBLE FOR LAYOUT OF ALL PAVEMENT MARKINGS USING CONTROL POINTS SPACED NO MORE THAN 50 FEET APART. PAVEMENT MARKING LAYOUT SHALL BE APPROVED BY TRAFFIC ENGINEERING PRIOR TO THE APPLICATION OF THE FINAL PRODUCT. ALL PAVEMENT MARKING DRAWINGS ARE SCHEMATIC ONLY. THE CONTRACTOR SHALL FOLLOW ALL DIMENSIONS, DETAILS AND STANDARDS WHEN

CIVIL REAPPROVAL REAPPROVAL # REVISED SHEET NO.(S) DESCRIPTION OF REVISION(S) /1\ 1-3, 8-19 REVISED F.F. ELEVATION, SERVICE AREA, PRIVATE SANITARY SEWER & PRIVATE STORM DRAIN. PAVING TRAFFIC G & D PLANNING W & S FIRE RET. WALLS

ENGINEERING COORDINATOR MGR. (OR DESIGNEE)



EXPIRES 09-30-13 REVISED F.F. ELEVATION, SERVICE AREA, $\angle 1$ PRIVATE SANITARY SEWER & PRIVATE STORM DRAIN. 10-07-2011

DATE

ESTIMATED QUANTITI	ES	
DESCRIPTION	UNITS	
EXCAVATION:		
CUT	CY	20,635
FILL	CY	38,978
SITE WALLS		
SCREEN WALL/FLOODWALL	LF	985
	in the second	
PUBLIC WATER:		
2" DOMESTIC WATER SERVICE LINE		56
2 WATER METER		2
3"x2" REDUCER	ΓΔ ΓΔ	2
3" DOMESTIC WATER SERVICE LINE	LF	62
4" D.I.P. CLASS 350 WATERLINE	LF	138
8" D.I.P. CLASS 350 WATERLINE	LF	1651
12"x8" T.S. & V.	EA	2
W.V.,B. & C.	EA.	8
8"x6" REDUCER	EA.	1
FIRE HYDRANT COMPLETE	EA.	4
FIRE DEPT. CONNECTION (F.D.C.)	EA.	1
SAWCUT, REMOVE & REPLACE EX PAVEMENT	Sĭ .	98
PRIVATE SEWER:		
6" P.V.C. SEWERLINE		164
8" P.V.C. SEWERLINE		189
SEWER CLEANOUT	EA.	1
SEWER MANHOLE	EA.	2
SAWCUT, REMOVE & REPLACE EX PAVEMENT	SY	8
PRIVATE STORM DRAIN:	· · · · ·	
6" HDPE STORM DRAIN PIPE		260
8 HUPE STORM DRAIN PIPE		158
TO FIDE STORM DRAIN FIFE		904
24" HDPE STORM DRAIN PIPE		224
30" HDPE STORM DRAIN PIPE	LF	313
36" HDPE STORM DRAIN PIPE		697
CATCH BASINS (MAG 535)	FA	$\vdash \sim$
CATCH BASINS (MAG 300)	FA	1
STORM DRAIN CLEANOUT	EA.	3
STORM DRAIN MANHOLE	FA	₹
AUS JUNCTION	EA.	
RIPRAP		116
SAWCUT, REMOVE & REPLACE EX PAVEMENT	SY	24
RIBBON CURB (MODIFIED TO 1' WIDTH)	1.5	010
4" ROLL CURB		180
6" VERTICAL CURB		242
6" VERTICAL CURB & GUTTER	LF	441
	<u> </u>	7638
SIDEWALK	3	,000
SIDEWALK SAWCUT & REMOVE AC PAVEMENT	SY SY	1461
SIDEWALK SAWCUT & REMOVE AC PAVEMENT 3" ON 6" AC PAVEMENT	SY SY	1461 1500

U ŧ **Jit** <u>§</u> C J 1350 coronado avenue, long beach. ca 90804 tel 562.597.8760 fax 562.597.8022 www. kollinaitomare.com \sim しらし AUDSLEY EXPIRES 09-30-13 WOOD/PATEL **CIVIL ENGINEERS** HYDROLOGISTS LAND SURVEYORS CONSTRUCTION MANAGERS (602) 335-8500 PHOENIX • MESA • TUCSON ENGINEER S. AUDSLEY DESIGNER AUDSLEY CAD TECHNICIAN P JIROL SCALE (HORIZONTAL) NT SCALE (VERTICAL) DATE 09-02-11 JOB NUMBER <u>103555</u> SHEET 26OF'

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NOTES:

1. QUANTITIES SHOWN HEREON ARE FOR PERMIT PURPOSES ONLY. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL QUANTITIES PRIOR TO BIDDING.

CALL TWO WORKING DAYS

(6()2) 263-1100

H-800-STAKE-IT

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BEFORE YOU DIG

2. EARTHWORK QUANTITIES ARE IN PLACE ESTIMATES, NO SHRINK OR SWELL ASSUMED.













(1) CONSTRUCT 6" VERTICAL CURB & GUTTER PER M.A.G. STD. DETAIL 220-1, TYPE 'A'.

2 CONSTRUCT 6" VERTICAL CURB AND GUTTER PER C.O.S. STD. DET. 2220 TYPE 'A'.

3 CONSTRUCT 2" OF A.C. OVER 6" A.B.C. PER M.A.G. STD. SPEC. 702 & 710 AND GEOTECH REPORT. PARKING STALLS ONLY. SEE ARCHITECTURAL SITE PLAN FOR PARKING STALL LAYOUT.

CONSTRUCT 3" OF A.C. OVER 4" A.B.C. PER M.A.G. STD. SPEC. 702 & 710 AND GEOTECH REPORT. DRIVE LANES ONLY.

6 CONSTRUCT SIDEWALK PER M.A.G. STD. DETAIL 230. MATCH EXISTING SIDEWALK MATERIAL, COLOR AND TEXTURE, SEE ARCHITECTURAL PLANS. WDTH PER PLAN.

(13) MATCH EXISTING ELEVATIONS.

(15) LANDSCAPE AREA PER LANDSCAPE PLANS.

(17) PARKING/ AREA LIGHTS PER ARCHITECTURAL SITE PLAN.

(21) INSTALL 24" ADVANCED DRAINAGE SYSTEMS (ADS) CATCH BASIN WITH M.A.G. STD. DETAIL 535 GRATE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING. (25) INSTALL 18" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.

(29) INSTALL HOPE STORM DRAIN WYE, SIZE PER PLAN. 39 INSTALL PERMEABLE PARKING PAVERS PER DETAIL ON SHEET 26.

(51) CONSTRUCT TEMPORARY SWALE (EAST-WEST) PER PLAN AND TYPICAL SECTION ON SHEET 5. MAXIMUM 4:1 SIDE SLOPES.

52 CONSTRUCT TEMPORARY SWALE (NORTH-SOUTH) PER PLAN AND TYPICAL SECTION ON SHEET 6. MAXIMUM 4:1 SIDE SLOPES.

(58) CONSTRUCT 6" VERTICAL CURB PER M.A.G. STD. DETAIL 222, TYPE 'A'.

(60) CONSTRUCT CLEANOUT PER M.A.G. STD. DET. 441.









- (1) CONSTRUCT 6" VERTICAL CURB & GUTTER PER M.A.G. STD. DETAIL 220-1, TYPE 'A'.
- (2) CONSTRUCT 6" VERTICAL CURB AND GUTTER PER C.O.S. STD. DET. 2220 TYPE 'A'.
- 3 CONSTRUCT 2" OF A.C. OVER 6" A.B.C. PER M.A.G. STD. SPEC. 702 & 710 AND GEOTECH REPORT. PARKING STALLS ONLY. SEE ARCHITECTURAL SITE PLAN FOR PARKING STALL LAYOUT.
- (4) CONSTRUCT 3" OF A.C. OVER 4" A.B.C. PER M.A.G. STD. SPEC. 702 & 710 AND GEOTECH REPORT. DRIVE LANES ONLY.
- CONSTRUCT SIDEWALK PER M.A.G. STD. DETAIL 230. MATCH EXISTING SIDEWALK MATERIAL, COLOR AND TEXTURE, SEE ARCHITECTURAL PLANS. WIDTH PER PLAN.
- (13) MATCH EXISTING ELEVATIONS.
- (15) LANDSCAPE AREA PER LANDSCAPE PLANS.
- $\langle 17 \rangle$ PARKING/ AREA LIGHTS PER ARCHITECTURAL SITE PLAN.
- (18) CONSTRUCT A.D.A. APPROVED SIDEWALK RAMP. ALL RAMPS MUST MEET A.D.A. ACCESSIBILITY GUIDELINES (ADAAG) STANDARDS; 2% MAX CROSS SLOPES AND 12:1 LONGITUDINAL SLOPES. SEE ARCHITECTURAL PLAN FOR DETAILS.
- (21) INSTALL 24" ADVANCED DRAINAGE SYSTEMS (ADS) CATCH BASIN WITH M.A.G. STD. DETAIL 535 GRATE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING.
- (25) INSTALL 18" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
- (29) INSTALL HDPE STORM DRAIN WYE, SIZE PER PLAN.
- (39) INSTALL PERMEABLE PARKING PAVERS PER DETAIL ON SHEET 26.
- (58) CONSTRUCT 6" VERTICAL CURB PER M.A.G. STD. DETAIL 222, TYPE 'A'.
- 59 CONSTRUCT TRAFFIC RATED DECORATIVE CONCRETE PAVERS PER C.O.S. STD. DETAIL 2239, TYPE 'B'. SEE ARCHITECTURAL PLAN FOR PAVER TYPE AND COLOR..
- (60) CONSTRUCT CLEANOUT PER M.A.G. STD. DET. 441.



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- (1) CONSTRUCT 6" VERTICAL CURB & GUTTER PER M.A.G. STD. DETAIL 220-1, TYPE 'A'.
- (2) CONSTRUCT 6" VERTICAL CURB AND GUTTER PER C.O.S. STD. DET. 2220 TYPE 'A'.
- (3) CONSTRUCT 2" OF A.C. OVER 6" A.B.C. PER M.A.G. STD. SPEC. 702 & 710 AND GEOTECH REPORT. PARKING STALLS ONLY. SEE ARCHITECTURAL SITE PLAN FOR PARKING STALL LAYOUT.
- (4) CONSTRUCT 3" OF A.C. OVER 4" A.B.C. PER M.A.G. STD. SPEC. 702 & 710 AND GEOTECH REPORT. DRIVE LANES ONLY
- 5 CONSTRUCT 6" PCC OVER 4" ABC PER M.A.G. STD. SPEC 702 & 725 AND GEOTECH REPORT.
- 6 CONSTRUCT SIDEWALK PER M.A.G. STD. DETAIL 230. MATCH EXISTING SIDEWALK MATERIAL, COLOR AND TEXTURE, SEE ARCHITECTURAL PLANS. WIDTH PER PLAN.
- WALL OPENING FOR DRAINAGE, SEE STRUCTURAL PLAN FOR DETAIL.
- (12) PROVIDE 2' CURB TRANSITION FROM 6" TO 0".
- (15) LANDSCAPE AREA PER LANDSCAPE PLANS.
- (16) SEE ARCHITECTURAL PLANS FOR HARDSCAPE.
- (17) PARKING/ AREA LIGHTS PER ARCHITECTURAL SITE PLAN.
- (18) CONSTRUCT A.D.A. APPROVED SIDEWALK RAMP. ALL RAMPS MUST MEET A.D.A. ACCESSIBILITY GUIDELINES (ADAAG) STANDARDS; 2% MAX CROSS SLOPES AND 12:1 LONGITUDINAL SLOPES. SEE ARCHITECTURAL PLAN FOR DETAILS.
- (21) INSTALL 24" ADVANCED DRAINAGE SYSTEMS (ADS) CATCH BASIN WITH M.A.G. STD. DETAIL 535 GRATE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING.
- (22) INSTALL 6" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
- (25) INSTALL 18" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
- 27 INSTALL 30" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
- (28) INSTALL 36" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SECONDARY LOADING AND WATERTIGHT JOINTS.
- (29) INSTALL HDPE STORM DRAIN WYE, SIZE PER PLAN.
- (30) INSTALL STORM DRAIN AIR BREAK PER DETAIL ON SHEET 26. SEE PLUMBING PLANS FOR CONTINUATION. CONTRACTOR TO VERIFY HORIZONTAL AND VERTICAL LOCATION PRIOR TO CONSTRUCTION. NOTIFY ENGINEER OF ANY DISCREPANCIES.
- 32 CONSTRUCT 4' DIA. STORM DRAIN MANHOLE PER M.A.G. STD. DETAIL 520 & 522. COVER SHALL BE NON-ROCKING EAST JORDAN IRON WORKS #00222459, OR APPROVED EQUAL.
- (39) INSTALL PERMEABLE PARKING PAVERS PER DETAIL ON SHEET 26.
- (43) ACCESSIBLE PARKING, PER DETAIL ON SHEET 26. MAXIMUM SLOPE OF 2% IN ALL DIRECTIONS.
- CONSTRUCT FLUSH A.D.A. APPROVED SIDEWALK CROSSING, 2' CURB TRANSITION ON EACH SIDE OF CROSSING. A.D.A. SIDEWALK CROSSING MUST MEET A.D.A. ACCESSIBILITY GUIDELINES (ADAAG). 2% MAX. CROSS SLOPES.
- (49) CONSTRUCT A.D.A. APPROVED SIDEWALK/ VEHICULAR RAMP. 6" PCC OVER 4" ABC PER M.A.G. STD. SPEC 702 & 725 AND GEOTECH REPORT.
- (53) INSTALL WHEEL STOP PER M.A.G. STD. DETAIL 150.
- $\langle 54 \rangle$ STAIRS, REFERENCE ARCHITECTURAL PLAN FOR DETAILS.
- (58) CONSTRUCT 6" VERTICAL CURB PER M.A.G. STD. DETAIL 222, TYPE 'A'.
- (59) CONSTRUCT TRAFFIC RATED DECORATIVE CONCRETE PAVERS PER C.O.S. STD. DETAIL 2239, TYPE 'B'. SEE ARCHITECTURAL PLAN FOR PAVER TYPE AND COLOR ...
- (61) CONSTRUCT 2' GARDEN WALL PER ARCHITECTURAL PLAN. SCOTT A AUDSLE
- EXPIRES 09-30-13 REVISED F.F. ELEVATION, SERVICE AREA, PRIVATE SANITARY SEWER & PRIVATE STORM DRAIN. 10-07-2011





CURVE TABLE

DELTA RADIUS ARC TANGENT

36°18'39" 25.00' 15.84' 8.20'

CONSTRUCTION NOTES

- (1) CONSTRUCT 6" VERTICAL CURB & GUTTER PER M.A.G. STD. DETAIL 220-1, TYPE 'A'.
- (2) CONSTRUCT 6" VERTICAL CURB AND GUTTER PER C.O.S. STD. DET. 2220 TYPE 'A'.
- 3 CONSTRUCT 2" OF A.C. OVER 6" A.B.C. PER M.A.G. STD. SPEC. 702 & 710 AND GEOTECH REPORT. PARKING STALLS ONLY. SEE ARCHITECTURAL SITE PLAN FOR PARKING STALL LAYOUT.
- (4) CONSTRUCT 3" OF A.C. OVER 4" A.B.C. PER M.A.G. STD. SPEC. 702 & 710 AND GEOTECH REPORT. DRIVE LANES ONLY.
- 8 SAWCUT, REMOVE & REPLACE EXISTING PAVEMENT 2' MIN. PER C.O.S. STD. DETAIL 2200.
- $\langle 13 \rangle$ MATCH EXISTING ELEVATIONS.
- (15) LANDSCAPE AREA PER LANDSCAPE PLANS.
- (19) CONSTRUCT CURB OPENING PER DETAIL ON SHEET 26. WIDTH PER PLAN.
- (21) INSTALL 24" ADVANCED DRAINAGE SYSTEMS (ADS) CATCH BASIN WITH M.A.G. STD. DETAIL 535 GRATE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING.
- (25) INSTALL 18" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20
- PIPE PER MANUFACTORER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
 INSTALL 24" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
- 28 INSTALL 36" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
- (32) CONSTRUCT 4' DIA. STORM DRAIN MANHOLE PER M.A.G. STD. DETAIL 520 & 522. COVER SHALL BE NON-ROCKING EAST JORDAN IRON WORKS #00222459, OR APPROVED EQUAL.
- (33) INSTALL 2' THICK LOOSE ROCK RIP-RAP, D50=8" PER GRADATION TABLE ON SHEET 26. ALL RIP-RAP TO BE NATIVE INDIGENOUS STONE.
- 38 CONSTRUCT 1' DEEP TEMPORARY RETENTION/ SEDIMENTATION BASIN. CONTRACTOR TO SCARIFY BOTTOM OF BASIN 2' DEEP TO MAXIMUM 80% COMPACTION.
- $\overline{39}$ INSTALL PERMEABLE PARKING PAVERS PER DETAIL ON SHEET 26.
- (46) INSTALL STEEL BOLLARD PER M.A.G. STD. DETAIL 140, TYPE '1'.
- (47) INSTALL HDPE STORM DRAIN PLUG, SIZE PER PLAN.
- 52 CONSTRUCT TEMPORARY SWALE (NORTH-SOUTH) PER PLAN AND TYPICAL SECTION ON SHEET 6. MAXIMUM 4:1 SIDE SLOPES.
- (58) CONSTRUCT 6" VERTICAL CURB PER M.A.G. STD. DETAIL 222, TYPE 'A'.



REVISED F.F. ELEVATION, SERVICE AREA, PRIVATE SANITARY SEWER & PRIVATE STORM DRAIN. 10-07-2011

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(OUTSIDE MARICOPA COUNTY)





Horz. 1 in. = 20 ft.

W INV=49.87 SD INV=45.94

CONSTRUCTION NOTES

(1) CONSTRUCT 6" VERTICAL CURB & GUTTER PER M.A.G. STD. DETAIL 220-1, TYPE 'A'.

kollin altomare hitects

U

1350 coronado avenue, long beach. ca 90804 tel 562.597.8760

HAR

46844 SCOTT A. AUDSLEY

EXPIRES 09-30-13

WOOD/PATEL

CIVIL ENGINEERS HYDROLOGISTS LAND SURVEYORS CONSTRUCTION MANAGERS

(602) 335-8500 PHOENIX • MESA • TUCSON

5. AUDSLEY

S. AUDSLEY

P. JIROU

1" = 20

09-02-11

103555

26

N/A

ENGINEER

DESIGNER

DATE

SHEET

CALL TWO WORKING DAYS BEFORE YOU DIG

(602) 263-1100 1-800-STAKE-IT

(OUTSIDE MARICOPA COUNTY)

CAD TECHNICIAN

SCALE (HORIZONTAL)

SCALE (VERTICAL)

JOB NUMBER

NP#

201

00

fax 562.597.8022 www.kollinaltomare.com

- (2) CONSTRUCT 6" VERTICAL CURB AND GUTTER PER C.O.S. STD. DET. 2220 TYPE 'A'.
- (4) CONSTRUCT 3" OF A.C. OVER 4" A.B.C. PER M.A.G. STD. SPEC. 702 & 710 AND GEOTECH REPORT. DRIVE LANES ONLY.
- 6 CONSTRUCT SIDEWALK PER M.A.G. STD. DETAIL 230. MATCH EXISTING SIDEWALK MATERIAL, COLOR AND TEXTURE, SEE ARCHITECTURAL PLANS. WIDTH PER PLAN.
- 8 SAWCUT, REMOVE & REPLACE EXISTING PAVEMENT 2' MIN. PER C.O.S. STD. DETAIL 2200.
- (13) MATCH EXISTING ELEVATIONS.
- (25) INSTALL 18" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
- (28) INSTALL 36" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
- CONSTRUCT 4' DIA. STORM DRAIN MANHOLE PER M.A.G.
 STD. DETAIL 520 & 522. COVER SHALL BE NON-ROCKING EAST JORDAN IRON WORKS #00222459, OR APPROVED EQUAL.
- CONNECT TO EXISTING STORM DRAIN MANHOLE.
 CONTRACTOR TO FIELD VERIFY EXACT LOCATION/INVERT PRIOR TO CONSTRUCTION. NOTIFY ENGINEER OF ANY DISCREPANCIES.
- CONSTRUCT DRIVEWAY PER C.O.S. STD. DETAIL 2257 TYPE 'CH-1' (MODIFIED FOR 30' WIDTH).
- (55) INSTALL RIBBON CURB PER M.A.G. STD. DETAIL 220-1, TYPE 'B'. MODIFIED WIDTH TO MATCH EXISTING.



REVISED F.F. ELEVATION, SERVICE AREA, PRIVATE SANITARY SEWER & PRIVATE STORM DRAIN. 10-07-2011





- (1) CONSTRUCT 6" VERTICAL CURB & GUTTER PER M.A.G. STD. DETAIL 220-1, TYPE 'A'.
- (2) CONSTRUCT 6" VERTICAL CURB AND GUTTER PER C.O.S. STD. DET. 2220 TYPE 'A'.
- (4) CONSTRUCT 3" OF A.C. OVER 4" A.B.C. PER M.A.G. STD. SPEC. 702 & 710 AND GEOTECH REPORT. DRIVE LANES ONLY.
- 6 CONSTRUCT SIDEWALK PER M.A.G. STD. DETAIL 230. MATCH EXISTING SIDEWALK MATERIAL, COLOR AND TEXTURE, SEE ARCHITECTURAL PLANS. WIDTH PER PLAN. (7) WALL OPENING FOR DRAINAGE, SEE STRUCTURAL PLAN FOR DETAIL.
- 8 SAWCUT, REMOVE & REPLACE EXISTING PAVEMENT 2' MIN. PER C.O.S. STD. DETAIL 2200.
- (10) CONSTRUCT MID-BLOCK SIDEWALK RAMP PER C.O.S. STD. DETAIL 2235-2.
- $\langle 13 \rangle$ MATCH EXISTING ELEVATIONS.
- (15) LANDSCAPE AREA PER LANDSCAPE PLANS.
- (16) SEE ARCHITECTURAL PLANS FOR HARDSCAPE.
- (23) INSTALL 8" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
- (25) INSTALL 18" ADS H.D.P.E. (OR EQUAL) STORM DRAIN PIPE PER MANUFACTURER'S SPECIFICATIONS FOR H20 LOADING AND WATERTIGHT JOINTS.
- $\langle 29 \rangle$ INSTALL HDPE STORM DRAIN WYE, SIZE PER PLAN.
- $\langle 31 \rangle$ INSTALL HDPE 45° BEND, SIZE PER PLAN.
- (36) INSTALL 12" ADVANCED DRAINAGE SYSTEMS (ADS) CATCH BASIN WITH PEDESTRIAN RATED GRATE PER MANUFACTURER SPECIFICATIONS.
- (40) CONSTRUCT 4' CONCRETE SCUPPER PER M.A.G. STD. DETAIL 206-1, MODIFIED TO OUTLET ONTO PAVEMENT.
- 50 INSTALL 24" NYLOPLAST DRAIN BASIN BY ADVANCED DRAINAGE SYSTEMS (A.D.S) WITH SOLID, BOLTED, WATER-TIGHT COVER AT GRADE. ADAPTER ANGLE PER
- PLAN.
- 56 BICYCLE PARKING, SEE ARCHITECTURAL PLAN FOR DETAILS.
- (60) CONSTRUCT CLEANOUT PER M.A.G. STD. DET. 441.



EXPIRES 09-30-13

BEFORE YOU DIG

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(OUTSIDE MARICOPA COUNTY)

REVISED F.F. ELEVATION, SERVICE AREA, PRIVATE SANITARY SEWER & PRIVATE STORM DRAIN. 10-07-2011

















PRIVATE SANITARY SEWER NOTES

- 1 CONNECT TO EXISTING SANITARY SEWER MANHOLE WITH ADEKA ULTRA SEAL MC-2005T GASKET. CONTRACTOR TO VERIFY HORIZONTAL AND VERTICAL LOCATION PRIOR TO CONSTRUCTION AND NOTIFY OWNERS AGENT OF ANY DISCREPANCIES.
- 2 REMOVE EXISTING 8" SEWER STUB AND PATCH EXISTING MANHOLE SIDEWALL. CONNECT NEW 8" SEWER STUB FOR FUTURE DEVELOPMENT TO EXISTING MANHOLE WITH ADEKA ULTRA SEAL MC-2005T GASKET. CONTRACTOR TO VERIFY HORIZONTAL AND VERTICAL LOCATION PRIOR TO CONSTRUCTION AND NOTIFY OWNERS AGENT OF ANY DISCREPANCIES.
- 3 INSTALL 8" PVC SDR35 SANITARY SEWER PIPE. LENGTH PER PLAN. SEE PROFILE ON SHT. 18 FOR SLOPE.
- 4 CONSTRUCT 4' DIA. MANHOLE PER M.A.G. STD. DETAIL 420 & 424 WITH SEWERSHIELD 100 EPOXY LINING BY ENVIRONMENTAL COATINGS OR APPROVED EQUAL.
- 11 SAWCUT, REMOVE 2' MIN. OF PAVEMENT, CURB & GUTTER, AND REPLACE PER C.O.S. STD. DET. 2200 & 2201.
- 12 PROVIDE WATER & SEWER SEPARATION & PROTECTION PER M.A.G. STD. DETAIL 404-1.
- 14 STUB OUT AND PLUG SEWER PER M.A.G. STD. DET. 427.

WATER CONSTRUCTION NOTES

- 1 INSTALL 8" D.I.P. PRESSURE CLASS 350 FIRELINE WITH RESTRAINED JOINTS (3' MIN. COVER).
- INSTALL 12"x 8" TAPPING SLEEVE AND VALVE ON EXISTING 12" WATERLINE PER M.A.G. STD. DETAIL 340 & 391-1, TYPE 'C' WITH JOINT RESTRAINT PER M.A.G. STD. DETAIL 303-1 & 303-2. CONTRACTOR SHALL VERIFY HORIZONTAL AND VERTICAL LOCATION PRIOR TO CONSTRUCTION AND NOTIFY OWNERS AGENT OF ANY DISCREPANCIES.
- (19) SAWCUT, REMOVE & REPLACE EXISTING PAVEMENT PER C.O.S. STD. DET. 2200 & 2201.
- (24) CUT EXISTING 6" WATERLINE AND INSTALL 6" CAP.
- (27) REMOVE EXISTING 6" WATERLINE.
- (33) INSTALL 11.25° BEND WITH JOINT RESTRAINT PER M.A.G. STD. DETAIL 303-1 AND 303-2.



REVISED F.F. ELEVATION, SERVICE AREA, PRIVATE SANITARY SEWER & PRIVATE STORM DRAIN. 10-07-2011 CALL TWO WORKING DAYS

BEFORE YOU DIG

(602) 263-1100 1-800-STAKE-IT

(OUTSIDE MARICOPA COUNTY)





















CONSTRUCT SIDEWALK PER M.A.G. STD. DETAIL 230.
 MATCH EXISTING SIDEWALK MATERIAL, COLOR AND TEXTURE, SEE ARCHITECTURAL PLANS. WIDTH PER PLAN.
 LANDSCAPE AREA PER LANDSCAPE PLANS.

57 CONSTRUCT 3' FLOOD/ SCREEN WALL PER DETAIL ON SHEET 26. SEE ARCHITECTURAL PLAN FOR COLOR AND FINISH.









FINISH.

 CONSTRUCT SIDEWALK PER M.A.G. STD. DETAIL 230. MATCH EXISTING SIDEWALK MATERIAL, COLOR AND TEXTURE, SEE ARCHITECTURAL PLANS. WIDTH PER PLAN.
 LANDSCAPE AREA PER LANDSCAPE PLANS.
 CONSTRUCT 3' FLOOD/ SCREEN WALL PER DETAIL ON SHEET 26. SEE ARCHITECTURAL PLAN FOR COLOR AND

kollin altomare hitects U G 1350 coronado avenue, long beach. ca 90804 tel 562.597.8760 fax 562.597.8022 www. kollinaltomare.com S **Served** 20 HAA 46844 SCOTT A AUDSLEY 9.2 237 EXPIRES 09-30-13 DR WOOD/PATEL CIVIL ENGINEERS HYDROLOGISTS LAND SURVEYORS CONSTRUCTION MANAGERS (602) 335-8500 PHOENIX · MESA · TUCSON ENGINEER S. AUDSLEY DESIGNER S. AUDSLEY CAD TECHNICIAN P. JIROUT 0 SCALE (HORIZONTAL) 1" = 20' SCALE (VERTICAL) N/A DATE 09-02-11 JOB NUMBER APP: 103555 SHEET OF 22 26

CALL TWO WORKING DAYS BEFORE YOU DIG (602) 263-1100 1-800-STAKE-IT (OUTSIDE MARICOPA COUNTY)



in a farmer a	1		e de suid				
CURVE TABLE							
CURVE	DELTA	RADI	US	ARC	TANGEI	NT	CHORD
3C1	46*13'22"	140.0) () ()	112.94'	59.7	75'	109.91'
3C2	02*40'24"	140.0	20'	6.53'	3.2	27'	6.53'
3C3	22*28'24"	167.0	20'	65.50'	33.1	18'	65.08'
3C4	120'01'51"	1.5	50'	3.14'	2.6	60'	2.60'
3C5	15*54'35"	111.0)0 '	30.82	15.5	51'	30.72
3C6	121'06'05"	1.50'		3.17'	2.6	66'	2.61'
3C7	22*44'57"	170.0	20'	67.50'	34.2	20'	67.06'
3C8	45*28'50"	144.(20'	114.31'	60.3	36'	111.33'
3C9	02*35'54"	469.5	50'	21.29'	10.6	55'	21.29'
3C10	02*35'54"	494.	50'	22.43	11.2	21'	22.42'
este s Socialista de la				LIN	VE TABL	_E	n, na ^k ang
			LINE	: В	EARING	DI	STANCE
n na 1975 - Angelander 1976 - Angelander			L1	N00"	02'36"E		101.39'
			L2	N00"	02'36"E		101.39'
			L3	N00*	02'36"E		99.70'
			L4	N00"	02 ' 36"E	line en	99.70'



PAVING NOTES

- (1) SAWCUT EXISTING PAVEMENT, CURB & GUTTER.
- $\langle 2 \rangle$ REMOVE EXISTING PAVEMENT, CURB & GUTTER.
- 5 CONSTRUCT 4" ROLL CURB AND GUTTER PER M.A.G. STD. DET. 220-1 TYPE 'D'.
- 6 CONSTRUCT 4" ROLL CURB AND GUTTER PER M.A.G. STD. DET. 220-1 TYPE 'C'.
- (7) CONSTRUCT RIBBON CURB PER C.O.S. STD. DET. 2220 TYPE 'B', MODIFIED TO 1' WIDTH.
- (8) TRANSITION ROLL CURB FROM TYPE 'C' TO TYPE 'D' WITHIN 5'.
- (9) TRANSITION CURB FROM RIBBON TO ROLL CURB AND GUTTER WITHIN 5'.
- (10) CONSTRUCT 6' VALLEY GUTTER AND APRON PER C.O.S. STD. DET. 2240.
- (1) CONSTRUCT 3" A.C. PAVEMENT OVER 4" A.B.C. BASE MATERIAL PER M.A.G. SPEC. 702 & 710, MATCH EXISTING SECTION OR WHICH EVER IS GRATER.
- (12) CONSTRUCT 8' WIDE SCUPPER PER M.A.G. STD. DET. 206-1 WITH STORM DRAIN MARKER PER C.O.S. STD. DET. 2560-3.
- (14) MATCH EXISTING PAVEMENT LOCATION AND ELEVATION.
- $\langle 15 \rangle$ MATCH EXISTING CURB LOCATION AND ELEVATION.
- TRANSITION VERTICAL CURB AND GUTTER FROM M.A.G. STD (16) DET. 220-1 TYPE 'A' TO C.O.S. STD. DET. 2220 TYPE 'A' WITHIN 5'.
- (17) CONSTRUCT 6" VERTICAL CURB AND GUTTER PER C.O.S. STD. DET. 2220 TYPE 'A'.
- (19) ADJUST VALVE BOX PER C.O.S. STD. DET. 2270.
- (20) RELOCATE EXISTING SIGN TO NEW LOCATION.



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OUTSIDE MARICOPA COUNTY

BLVD RINCESS 1 58.34 0+00 STA 10+00.00 - N 963189.55 E 698580.65 PC = 13 + 20.73

> 1560 • •

• • • 1560

10+20 10+40 10+00





CURVE TABLE

LINE TABLE



PAVING NOTES

 $\langle 1 \rangle$ SAWCUT EXISTING PAVEMENT, CURB & GUTTER.

- 2 REMOVE EXISTING PAVEMENT, CURB & GUTTER.
- (4) CONSTRUCT 6" VERTICAL CURB PER M.A.G. STD. DET. 222 TYPE 'A'.
- (1) CONSTRUCT 3" A.C. PAVEMENT OVER 4" A.B.C. BASE MATERIAL PER M.A.G. SPEC. 702 & 710, MATCH EXISTING SECTION OR WHICH EVER IS GRATER.
- (14) MATCH EXISTING PAVEMENT LOCATION AND ELEVATION. $\langle 15 \rangle$ match existing curb location and elevation.
- (19) ADJUST VALVE BOX PER C.O.S. STD. DET. 2270.



CALL TWO WORKING DAYS

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(OUTSIDE MARICOPA COUNTY)



	· · · · ·	CURV	ΈT	ABLE			
IRVE	DELTA	RADI	JS	ARC	TANGE	NT	CHORD
21	46'13'22"	140.0	00' 1	12.94'	59.7	75'	109.91'
2	02*40'24"	140.0	0'	6.53'	3.2	27'	6.53'
3	22*28'24"	167.0	0' (65.50 '	33.1	18'	65.08'
24	120'01'51"	1.5	50'	3.14'	2.6	SO'	2.60'
25	15*54'35"	111.C)0'	30.82'	15.	51'	30.72'
6	121*06'05"	1.5	50'	3.17'	2.6	66'	2.61'
:7	22*44`57"	170.0	0' (67.50 '	34.2	20'	67.06'
8	45*28'50"	144.C	00' 1	14.31'	60.3	36'	111.33'
;9	02*35'54"	469.5	50'	21.29'	10.6	§5'	21.29'
:10	02*35'54"	495.0	0'	22.45'	11.2	23'	22.45'
				LIN	NE TABI	E	
			INE	B	EARING	DI	STANCE
		Ĩ	_1	N00*(02'36"E		101.39'
		Ē	.2	N00*(02 ' 36"E		101.39'
		<u> </u>	_3	N00*	02'36"E		99.70'
		· · · · [_4	N00*(02 ' 36"E		99.70 '





103555

20

sheet 25

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<u>LEGEND</u>



SOLID WHITE FISH-HOOK ARROW

YIELD LINE

4" SOLID DOUBLE YELLOW LINE

4" BROKEN WHITE LINE WITH A 10' LINE SEGMENT AND A 30' GAP





					1
*	PERCENT	D ₅₀ =6" (LAYER THICKNESS=12")	D50=8"	(LAYER THICKNESS=16")	D50=1
	PASSING	ROCK SIZE (IN.)		ROCK SIZE (IN.)	
	100 TO 90	12		16	
	85 TO 70	9	n en	12	
	50 TO 30	6		8	
· .	15 TO 5	4		5	
	5 TO 0	2		3	
	· · · · · · · · · · · · · · · · · · ·		··· .		and the second



APPENDIX G – 121 Pinnacle Peak West FLO-2D STUDY



August 4, 2023







August 4, 2023



August 4, 2023







August 4, 2023





Flood Control District of Maricopa County Web Access Tool Data Legend



Floodplain X-Section

EXHIBIT 1 – VICINITY MAP



Z:\2021\215319\Project Support\Reports\Rezoning\Conference Center & Event Lawn\Drainage\Exhibits\5319.30-EXH1-VM.dwg
EXHIBIT 2 – FEMA FIRM

National Flood Hazard Layer FIRMette

250

n

500

1,000

1.500



Legend

regulatory purposes.

111°55'40"W 33°39'4"N SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT Without Base Flood Elevation (BFE) Zone A. V. A9 With BFE or Depth Zone AE, AO, AH, VE, AR SPECIAL FLOOD HAZARD AREAS **Regulatory Floodway** 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X Future Conditions 1% Annual Chance Flood Hazard Zone X Area with Reduced Flood Risk due to Levee. See Notes. Zone X OTHER AREAS OF FLOOD HAZARD Area with Flood Risk due to Levee Zone D CITY OF PHOENIX NO SCREEN Area of Minimal Flood Hazard Zone X 040051 **Project Site** Effective LOMRs OTHER AREAS Area of Undetermined Flood Hazard Zone D - — – – Channel, Culvert, or Storm Sewer GENERAL STRUCTURES LIIII Levee, Dike, or Floodwall 20.2 Cross Sections with 1% Annual Chance 17.5 Water Surface Elevation CITY OF SCOTTSDALE Zone AO **Coastal Transect** (DEPTH 1 Feet) Mase Flood Elevation Line (BFE) 045012 Limit of Study (VEL 3 Feet / Second) Jurisdiction Boundary **Coastal Transect Baseline** OTHER **Profile Baseline** 04013C1320L FEATURES Hydrographic Feature eff. 10/16/2013 **Digital Data Available** No Digital Data Available MAP PANELS Unmapped The pin displayed on the map is an approximate CITY OF PHOENIX point selected by the user and does not represent an authoritative property location. 040051 This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 3/31/2021 at 1:10 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for 111°55'2"W 33°38'34"N Feet 1:6.000 unmapped and unmodernized areas cannot be used for

2,000 Baseman: USGS National Man: Orthoima

Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

EXHIBIT 3 – EXISTING DRAINAGE MAP



LEGEND

1

EXISTING DRAINAGE AREA EXISTING DRAINAGE AREA



FAIRMONT SCOTTSDALE PRINCESS

CONFERENCE CENTER & EVENT LAWN EXISTING DRAINAGE MAP

DATE	08/04/2023	SCALE	1" = 60'	SHEET	1 OF 1
JOB NO.	215319	DESIGN	AJS	DRAWN	AJS
Z:\2021\215319\Pr	oject Support\Reports\Re	zoning\Conference Ce	nter & Event Lawn\Drai	inage\Exhibits\EXH3- E	Existing Drainage Area. w

WOOD PATEL

EXHIBIT 4 – PROPOSED DRAINAGE MAP



1	



EXHIBIT 5 – STORM PIPE LAYOUT





LEGEND

 EXISTING STORM DRAIN PIPE
PROPOSED 6" STORM DRAIN PIPE
PROPOSED 15" STORM DRAIN PIPE
PROPOSED 18" STORM DRAIN PIPE
 PROPOSED 36" STORM DRAIN PIPE

FAIRMONT SCOTTSDALE PRINCESS

CONFERENCE CENTER & EVENT LAWN STORM PIPE LAYOUT - EXHIBIT 5

DATE	08/04/2023	SCALE	1" = 100'	SHEET	1 OF 1
JOB NO.	215319	DESIGN	AJS	DRAWN	AJS

Z:\2021\215319\Project Support\Reports\Rezoning\Conference Center & Event Lawn\Drainage\Exhibits\5319-Storm Layout.dwg

EXHIBIT 6 – AERIAL MAP



Ν



NOT FOR CONSTRUCTION OR RECORDING

