

PRELIMINARY DRAINAGE REPORT

For
MERCADO VILLAGE
10301 N. 92nd Street
Scottsdale, AZ 85254

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Project Number: 210414

1st Submittal Date: January 9, 2024 (Zoning)
2nd Submittal Date: March 18, 2024 (Zoning)

Case No: 1-ZN-2024

Plan Check No.: TBD

Case #: 1-ZN-2024

Review Cycle: 2

Status: Accepted

Reviewed By: VM

Date: 04/12/2024



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1. INTRODUCTION

This Preliminary Drainage Report represents the storm water analysis for Mercado Village development proposed in Scottsdale, Arizona. Mercado Village is a proposed multi-family residential development located south and east of the Shea Boulevard and 92nd Street intersection. The purpose of this report is to provide the hydrologic and hydraulic analysis, required by the City of Scottsdale, to support the proposed rezoning for said development. This report includes discussions and calculations defining the storm water management concepts for the collection and conveyance necessary to comply with the drainage requirements of the City of Scottsdale and Maricopa County. Preparation of this report has been done in accordance with the requirements of the City of Scottsdale Design Standards & Policies Manual (DS&PM) 2018 ¹, and the Drainage Design Manuals for Maricopa County, Arizona, Volumes I² and Volume II³.

2. LOCATION AND PROJECT DESCRIPTION

2.1 LOCATION:

The subject property consists of five contiguous parcels located south and east of Shea Boulevard and 92nd Street in Scottsdale, AZ.

- A Portion of the Northeast Quarter of Section 30, Township 3 North, Range 5 East of The Gila and Salt River Base and Meridian, Maricopa County, Arizona.
- Parcel ID: Parcel 217-39-536, Zoning is PUD
Parcel 217-39-537A, Zoning is PUD
Parcel 217-39-537B, Zoning is PUD
Parcel 217-39-537C, Zoning is PUD
Parcel 217-36-989B, Zoning is C-O PCD
- Address: 10301 N. 92nd Street. Scottsdale, Arizona 85258.

Refer to **FIGURE 1 - Vicinity Map** for the project's location with respect to major cross streets.

2.2 EXISTING AND PROPOSED DEVELOPMENTS SURROUNDING THE SITE:

- South: Parcel with APN 217-74-038, a medical office development; Zoning is C-O PCD.
- North: Parcel with APN 217-36-960L, Sprouts shopping center; Zoning is C-3 PCD.
- East: Parcel 217-36-001P; paved area and medical offices, Zoning is C-O PCD.
- West: Across N. 92nd St. is Parcel 217-36-962G; medical offices; Zoning is SC PCD.

2.3 EXISTING SITE DESCRIPTION:

The project area includes approximately 263,945 sq. ft. (6.06 acres) of land designated as PUD, with the exception of parcel with APN 217-36-989B at the east which has a zoning of C-O PCD. The site is partially developed, the west side includes medical offices with associated parking lot, driveways and landscape areas; the east area is mainly undeveloped but has a small parking lot constructed at the south which connects to the eastern area of the site. The parcels generally slope towards the southwest corner. Disturbed area of the site is approximately 277,578 sq. ft. (6.37 acres).

Refer to **FIGURE 2** attached for an aerial of the site.

2.4 PROPOSED SITE DEVELOPMENT:

The proposed project will require the demolition of existing structures and associated parking lots. The proposed site will include a new high-density residential facility with 255 units, the building will have a central at-grade parking garage and rise to five floors.

Refer to **Appendix III** – Preliminary Grading and Drainage Plan for site layout.

2.5 FLOOD HAZARD ZONE:

FIRM Map Number 04013C1760L dated October 16, 2013 indicates the site has a Zone X-Shaded designation. Zone X-Shaded is defined as 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile.

Refer to **FIGURE 3** for the FIRM and FIRMette Maps.

3. EXISTING DRAINAGE CONDITIONS

3.1 OFF-SITE DRAINAGE PATTERNS

The topographic survey provides the following information for offsite drainage:

- The north of the site is bounded by Sprouts Shopping center, which has its own drainage system. A ridgeline prevents stormwater from Sprouts Shopping Center parking lot to enter the site. No offsite flows from the north affect the site.
- The west of the site is bounded by N. 92nd Street. Half of the street run-off drains towards the property and is conveyed via curb and gutter into an existing catch basin (EX-OFF-CB-1) adjacent to the property that discharges to an existing 36" CIP storm drain running along the east side of the road. No offsite flows from the west affect the site.
- The south of the site is bounded by a medical office development. The medical office development has its own drainage system and a wall separating the two properties prevents stormwater from entering the site. No offsite flows from the south affect the site.
- The east of the site is adjacent to a paved area and medical offices. The medical office has its own drainage system and grades on the paved area go away from the site. No offsite flows from the east affect the site.

3.3 ON-SITE DRAINAGE

Most of the runoff from the site is collected by existing catch basins (EX. CB-1 and EX. CB-2) and slotted drains (EX. SD-1, EX. SD-2, EX. SD-3) ultimately conveyed to the public network storm system at 92nd Street. The majority of stormwater from drainage area EX-1 flows easterly into a gutter and then proceeds to flow south into EX. CB-1. A small portion of runoff from drainage area EX-1 flows towards the northeast into EX. CB-1. Runoff from drainage area EX-2 flows towards EX. CB-2 located in the mid-west of the drainage area. Runoff from drainage area EX-3 flows westerly into a wall opening which leads stormwater into a slotted drain (EX. SD-1). Stormwater on drainage area EX-4 follows the same pattern as drainage area EX-3, flowing westerly into an opening and discharging into a slotted drain (EX. SD-2). Runoff from drainage

area EX-5 flows southerly into an on-site curb and gutter, and then proceeds to flow westerly into a wall opening leading stormwater into a slotted drain (EX-SD-3). Stormwater from EX-6 flows westerly into N. 92nd Street, where half of the runoff flows northwesterly into EX-OFF-CB-1 and the other half flows southeasterly along the street via curb and gutter. Stormwater collected on EX-R-1 drains through roof drains towards drainage area EX-1 ultimately draining into EX-CB-1. Stormwater collected on EX-R-2 drains through roof drains towards drainage area EX-4 ultimately draining into EX. SD-2. Open retention basin, EX. Basin 1, collects stormwater on drainage area EX-7. Existing storm drains are ultimately connected to the existing 36" public drain along 92nd Street.

Refer to **Appendix II for Existing Conditions Drainage Area Map.**

Existing runoff coefficients and flows for the 100-year, 5 min. event are presented in the tables below. Coefficients of 0.95 and 0.45 were chosen for building or concrete and desert landscape respectively, in accordance with the city of Scottsdale DS&PM.

Table 1. Existing Weighted Runoff Coefficients.

Weighted Runoff Coefficient-Calculations (Cw)				
EXISTING OVERALL SITE C_w				
	BUILDING or CONCRETE	DESERT LANDSCAPE	TOTAL AREA	Cwt
C-VALUE	0.95	0.45		
AREA (ac)	3.33	2.73	6.06	0.72
EX-1	1.39	0.44	1.83	0.83
EX-2	0.17	0.25	0.42	0.65
EX-3	0.29	0.08	0.37	0.84
EX-4	0.31	0.21	0.52	0.75
EX-5	0.23	0.16	0.39	0.74
EX-6	0.05	0.00	0.05	0.95
EX-7	0.00	1.58	1.58	0.45
EX-R-1	0.69	0.00	0.69	0.95
EX-R-2	0.20	0.00	0.20	0.95

The open retention Ex. Basin 1 provided volume is displayed in the table below:

Table 2. EX. Basin 1 Provided Volume

EX. Basin 1					
ELEV.	AREA	DEPTH	AVG V	SUM V	COMMENT
(FT)	(SF)	(FT)	(CF)	(CF)	
1369.0	382			0.00	Bottom
		2.00	1,704.33		
1371.0	1,314			1,704.33	Volume

Table 3. Existing Onsite Flows

EXISTING ONSITE 100 YR 5 MIN FLOWS						
Drainage Area	Area (ac)	C _w	i (in/hr)	Q (cfs)	Q Total (cfs)	Control Point
EX-1	1.83	0.83	7.54	11.45	27.75	Public Storm Drain at 92nd Street
EX-2	0.42	0.65	7.54	2.04		
EX-3	0.37	0.84	7.54	2.37		
EX-4	0.52	0.75	7.54	2.94		
EX-5	0.39	0.74	7.54	2.17		
EX-6	0.05	0.95	7.54	0.39		
EX-R-1	0.69	0.95	7.54	4.93		
EX-R-2	0.20	0.95	7.54	1.47	5.37	EX-BASIN 1
EX-7	1.58	0.45	7.54	5.37		

Overall existing project area includes **6.06 Acres at C_{wt} = 0.72**

Refer to **Appendix II** for 10-year event calculations.

4. PROPOSED STORM WATER MANAGEMENT

4.1 DESIGN INTENT:

Most of the on-site drainage will be directed off-site through a storm drain system to the historical outlets. The drainage proposal will be consistent with previous conditions of the site to avoid disrupting existing drainage patterns. Flows exposed to contaminated surfaces will also be treated prior to being released as a measure to control the quality of stormwater. Drainage areas A1 through A3 will be directed towards Basin 1 via catch basins and storm pipes. Drainage areas B1 through B6 will be directed to the city's public storm system. Proposed drainage patterns are as follow:

- Drainage area A1 will flow southerly into CB-1.
- Stormwater from drainage area A2 will flow towards its center into CB-2 (CMP riser).
- Runoff from drainage area A3 will flow via roof drain into Basin A

- Drainage area B1 will discharge into CB-3.
- Drainage area B2 will discharge runoff into the existing EX. SD-1 via a wall opening.
- Stormwater from Drainage area B3 will discharge into the EX. SD-2 via a wall opening.
- Runoff from drainage area B4 will flow southwesterly into CB-4. EX. SD-3 has been replaced due to increase in flow.
- Drainage area B5 will flow into CB-5.

Flows discharging to the existing slotted drains are reduced. The table below summarizes the flow reduction in the slotted drains and overall flows to the public storm drain system.

Table 4. Proposed and Existing Flows Comparison

P. DAM	Ex. DAM	Proposed Q100 CFS	Existing Q100 CFS	Difference	Inlet
B-2	EX-3	0.42	2.37	-1.96	EX. SD 1
B-3	EX-4	0.38	2.94	-2.56	EX. SD 2
Total On-site flows		27.10	27.75	-0.65	Public Storm Drain

Since the site has been previously developed, on-site retention shall be calculated per City of Scottsdale DSPM 4-1.201.

4.2 DESIGN STORM REQUIREMENTS:

In accordance with City of Scottsdale requirements, stormwater storage for the 100-year 2-hour storm event is required based on maintaining existing retention volume plus the difference between the pre vs. post development runoff from the 100-year 2-hour storm event if increased or first flush, whichever is greater. Per topographic information, there is an existing open retention basin in the site. As such, retention shall be provided by the sum of the existing retention volume plus the pre vs. post difference in volume or first flush.

4.3 LAND CHARACTERISTICS:

The proposed project site consists of residential spaces with a main drive and landscape areas. Based on the City of Scottsdale Design Standards & Policies Manual (DSPM), runoff coefficients for the 100-year storm event used are as follows:

- C=0.95 for building or concrete
- C=0.95 for paved surface
- C=0.45 for undisturbed natural desert or desert landscape

HYDROLOGIC ANALYSIS: The hydrologic analysis is determined using the procedures in the DSPM and the Drainage Design Manual for Maricopa County, Arizona, Volume I.

Table 5: Proposed Conditions Runoff Calculations

Weighted Runoff Coefficient-Calculations (C _w)				
PROPOSED OVERALL SITE C _w				
	BUILDING or CONCRETE	DESERT LANDSCAPE	TOTAL AREA	C _{wt}
C-VALUE	0.95	0.45		
AREA (ac)	4.34	1.72	6.06	0.81
A1	0.29	0.37	0.66	0.67
A2	0.31	0.14	0.44	0.80
A3	0.51	0.06	0.57	0.90
B1	0.44	0.25	0.69	0.77
B2	0.03	0.06	0.09	0.61
B3	0.01	0.10	0.10	0.48
B4	0.36	0.29	0.65	0.73
B5	0.06	0.26	0.32	0.54
B6	2.34	0.19	2.53	0.91

Overall project area includes **6.06 Acres** at **C_{wt} = 0.81**.

Table 6. Proposed Onsite Flows.

PROPOSED ONSITE 100 YR 5 MIN FLOWS						
Drainage Area	Area (ac)	C _w	i (in/hr)	Q (cfs)	Q Total (cfs)	Control Point
A1	0.66	0.67	7.54	3.31	9.81	BASIN A
A2	0.44	0.80	7.54	2.65		
A3	0.57	0.90	7.54	3.85		
B1	0.69	0.77	7.54	4.01	27.10	Public Storm Drain at 92 nd Street
B2	0.09	0.61	7.54	0.42		
B3	0.10	0.48	7.54	0.38		
B4	0.65	0.73	7.54	3.58		
B5	0.32	0.54	7.54	1.30		
B6	2.53	0.91	7.54	17.41		

Refer to the **Proposed C_{wt} Exhibit (Exhibit B)**, **Proposed Conditions Drainage Area Map (Exhibit D)** and Calculations in **Appendix II**.

4.4 STORMWATER RETENTION:

100-YR, 2-HR STORM: Per City of Scottsdale DSPM 4-1.201, development storage requirements for the 100-yr, 2-hr storm event are calculated as follows:

$$V_r = \Delta C \left(\frac{R}{12} \right) A$$

where:

V_r = Required storage (cf)

R = Precipitation amount = 2.22 in per NOAA Atlas 14 Precipitation Frequency Estimates

A = Total area of site (sf)

$\Delta C = C_{post} - C_{pre}$

$$V_r = (0.81 - 0.72) \left(\frac{2.22}{12} \right) (263,945) = 4,394.68 \text{ cf}$$

Since the difference of the weighted coefficient is greater than 0, an increase in stormwater flows will be generated. Therefore, stormwater retention is required for the development following the pre vs. post analysis. The volume provided by EX. Basin 1 must be added to the pre vs. post analysis volume to compare it with the first flush volume. The addition of the volumes is 1,704.33 cf + 4,394.68 cf = 6,099.01 cf.

FIRST FLUSH: First Flush storage required is calculated in accordance with City of Scottsdale DSPM 4-1.201. Only the areas where runoff could be affected by vehicular contact are considered in the first flush calculation. The roof drainage is considered to be free of heavy traffic pollutants, therefore, on-site driveway areas and sidewalks will be considered for the calculation (277,578 sf (total area) – 75,047 sf (landscape) – 124,050 sf (roof) = 78,481 sf.

$$FF_r = C \left(\frac{P}{12} \right) A$$

where:

FF_r = First Flush required storage volume (cf)

A = Area of site excluding roofs and landscape (sf)

C = The weighted average runoff coefficient = 0.95

$$FF_r = (0.95) \left(\frac{0.5}{12} \right) 78,481 = 3,106 \text{ cf}$$

The above assessment indicates that First Flush storage is required (3,106 cf). Since the volume from the pre vs. post analysis is greater than the volume from the first flush analysis, retention must be provided for a minimum of 6,099.01 cf per the City of Scottsdale DS&PM.

Given that stormwater volume will be increased, underground storage (Basin A) is proposed to retain the required pre vs post volume plus existing required volume.

To comply with the City of Scottsdale DS&PM, flows from drainage areas A1 and A2 will be directed to Basin A. Below is a summary of their required volumes.

Table 7. Proposed Conditions Required Storage Volume

Required Storage Volume					
$V_r = 1 * (P/12) * C_w * A$ $P = 100\text{-yr, } 2\text{-hr} = 2.22 \text{ in.}$					
Drainage	Area	C _w	Precipitation	Volume Req.	Volume Req.
<u>Area ID</u>	<u>(acres)</u>	<u>(-)</u>	<u>(in)</u>	<u>(acre-ft)</u>	<u>(CF)</u>
A1	0.66	0.67	2.22	0.08	3,534.66
A2	0.44	0.80	2.22	0.07	2,836.78
A3	0.57	0.90	2.22	0.09	4,113.86
BASIN A			Total Retention:		10,485.29

A volume of 10,485 cf will be retained by the proposed underground storage Basin A in order to comply with the pre vs post analysis and reduce flows to historical outfalls.

UNDERGROUND STORMWATER STORAGE POLICY

Per Section 4-1.202 of the City of Scottsdale's Design Standards & Policies Manual (DSPM), the proposed underground stormwater storage tanks (USSTs) must meet the City's Underground Stormwater Storage Policy, which states the following items:

- The owner must dedicate a public drainage easement over the USST and a minimum of 10' wide vehicular access easements to the basin, with no major vegetation such as trees within the easement. At a minimum, the easement should extend at a projected slope of 1:1 from the bottom of the pipe.
- The USST must have at least a 75-year life, including the lining and coating.
- The USST must drain by gravity.
- Specify MAG supplemental standard detail 2554 for corrugated metal pipes.
- A minimum of two access points must be provided for each USST.
- An Operations and Maintenance (O&M) Manual must be prepared for the system prior to approval of final plans.
- Final plans must include signs at each end of the USST.
- A signed and notarized Ownership and Responsibility Statement must be provided prior to approval of final plans.
- Add the required warning signs.

PROVIDED STORMWATER STORAGE

The proposed development will provide storage for the pre vs post analysis and existing required volume through a corrugated metal pipe underground system. Stormwater will be discharged by the use of drywells.

Basin A provided storage:

Basin A will consist of 10' diameter corrugated metal pipe and will have a length of 135 LF.

$$V_P = \pi * \text{Pipe radius}^2 * \text{Pipe length}$$

$$V_P = (\pi * 5^2) * (135) = 10,603 \text{ cf}$$

Refer to **Appendix II** for existing and proposed volume calculations.

4.6 STORMWATER DISCHARGE

For basins with no direct bleed-off available, drywells are proposed in the on-site storage facilities to dispose of stormwater within thirty six (36) hours. The calculation is as follows:

- Minimum percolating rate of a drywell (for planning purposes) = 0.1 cfs
- Volume to be drained in 36 hours = $0.1 \text{ cfs} * 36 \text{ hours} * 3600 \text{ sec/hour} = 12,960 \text{ cf} = 0.298 \text{ acre-feet}$.
- The number of drywells will be reduced if geotechnical testing for percolation rates determine adequate infiltration is available in the native soils at lower depths. If the percolation rate of the drywells is less than 0.1 cfs the number of drywells may have to be increased.

Basin A:

$$\text{Total provided storage} = 10,603 \text{ CF}$$

$$10,603 \text{ CF} / 12,960 \text{ CF per drywell} = 0.82 = 1 \text{ drywell required.}$$

4.7 PIPE HYDRAULICS CALCULATIONS

The proposed drainage system consists of HDPE pipes ($n=0.013$) conveying a portion of the runoff flows towards the Public Storm Drain System at 92nd Street and the remainder runoff towards Basin A. Capacity of the system was evaluated for the 100-year event scenario using StormCAD, the storm pipes have the capacity to convey the total runoff of the drainage areas.

Refer to Appendix IV for StormCAD Results.

4.8 ADEQ WATER QUALITY REQUIREMENTS

The Arizona Department of Environmental Quality (ADEQ) requires that any site disturbance over an acre is required to submit an NOI. An NOI will be submitted to ADEQ for this site after the first submittal of the construction documents as this site disturbance is over 1 acre and has flows going off-site.

5. FLOOD SAFETY FOR DWELLINGS

5.1 FINISHED FLOOR ELEVATIONS

Since project lies in an "X-Shaded" Flood Zone, finished floor elevations will also comply with the minimum elevation of 14 inches above the Ultimate Outfall of the site (1,368.66'), located on the southwest corner of the site and at least 12 inches above adjacent HWE's. The minimum proposed finish floor elevation of 1,371.25' within the project is in accordance with the previous criteria, ensuring that the building will be safe from flooding during a 100-year storm.

6. CONCLUSIONS

6.1 OVERALL PROJECT:

1. The finished floor elevations will be designed a minimum of 14 inches above the lot Ultimate Outfall.
2. Historical outfalls will be maintained at proposed conditions and no detrimental effects will be posed to existing drainage patterns.
3. On-site retention facilities will be provided to account for pre vs. post analysis plus existing retention.

7. WARNING AND DISCLAIMER OF LIABILITY

RE: Following page.

8. REFERENCES

1. *Design Standards & Policies Manual, City of Scottsdale – January 2018*
2. *Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology, Flood Control District of Maricopa County, Fourth Edition, December 14, 2018*
3. *Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics, Flood Control District of Maricopa County, December 14, 2018*

GRADING & DRAINAGE LANGUAGE

WARNING AND DISCLAIMER OF LIABILITY

The City's Stormwater and Floodplain Management Ordinance is intended to minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding. The Stormwater and Floodplain Management Ordinance identifies floodplains, floodways, flood fringes and special flood hazard areas. However, a property outside these areas could be inundated by floods. Also, much of the city is a dynamic flood area; floodways, floodplains, flood fringes and special flood hazard areas may shift from one location to another, over time, due to natural processes.

WARNING AND DISCLAIMER OF LIABILITY

The flood protection provided by the Stormwater and Floodplain Management Ordinance is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by constructed or natural causes. The Stormwater and Floodplain Management Ordinance does not create liability on the part of the city, any officer or employee thereof, or the federal, state or county government for any flood damages that result from reliance on the Ordinance or any administrative decision lawfully made thereunder.

Compliance with the Stormwater and Floodplain Management Ordinance does not ensure complete protection from flooding. Flood-related problems such as natural erosion, streambed meander, or constructed obstructions and diversions may occur and have an adverse effect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations.

I have read and understand the above.

Plan Check #

Owner

Date

FIGURES

Figure 1. Vicinity Map

Figure 2. Aerial

Figure 3. FIRM

Figure 4. FIRMette



Figure 1. Vicinity Map



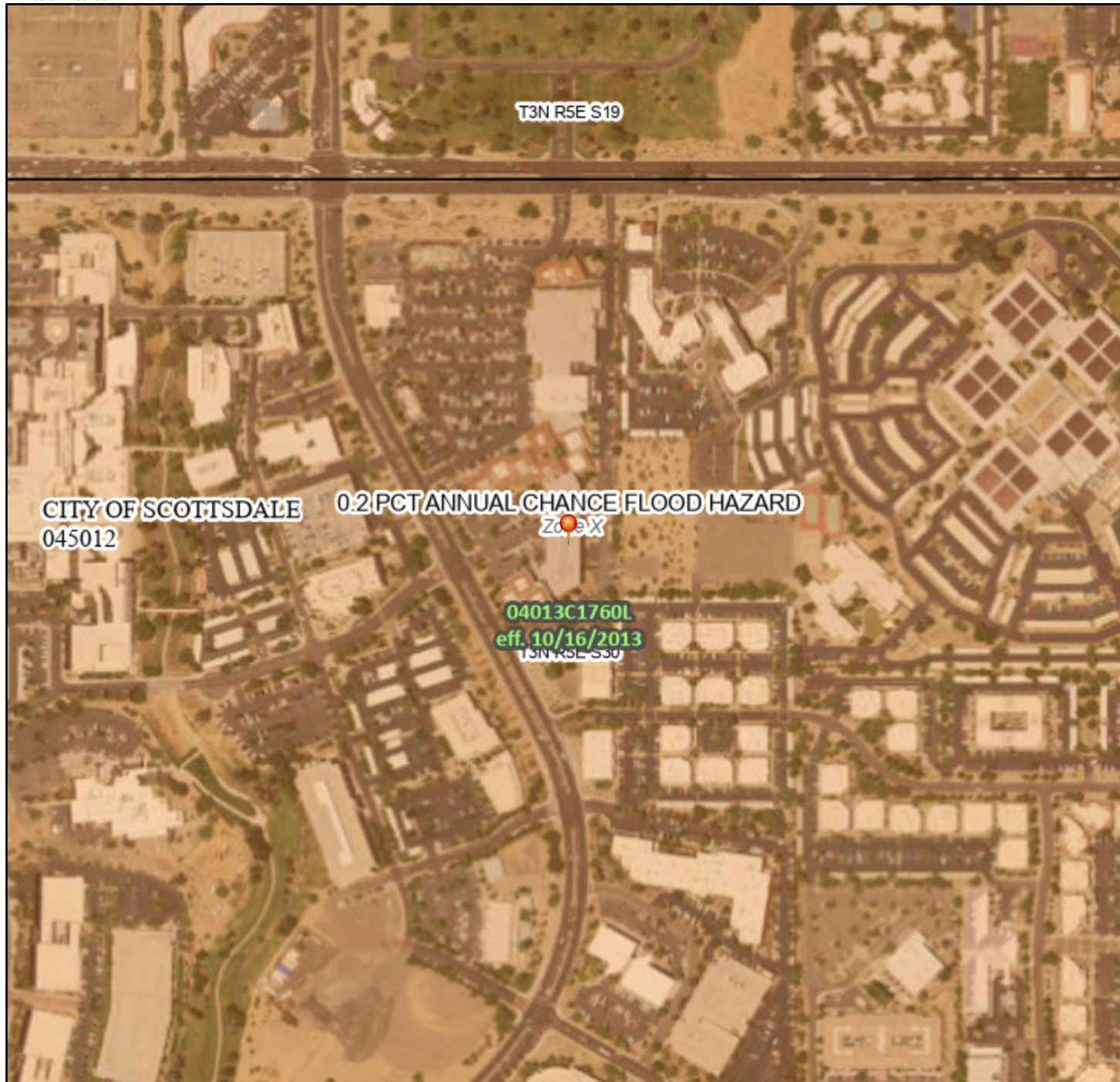
Figure 2. Aerial



National Flood Hazard Layer FIRMette



111°53'8"W 33°35'2"N



0 250 500 1,000 1,500 2,000 Feet

1:6,000

111°52'31"W 33°34'32"N

Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
		Area of Undetermined Flood Hazard Zone D
GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance Water Surface Elevation
		17.5
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
		Coastal Transect Baseline
MAP PANELS		Profile Baseline
		Hydrographic Feature
		Digital Data Available
		No Digital Data Available
		Unmapped



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 4/28/2021 at 12:52 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

FIGURE 4. FIRMette

APPENDIX I

RAINFALL DATA



NOAA Atlas 14, Volume 1, Version 5
Location name: Scottsdale, Arizona, USA*
Latitude: 33.5798°, Longitude: -111.8816°
Elevation: 1370.67 ft**
 * source: ESRI Maps
 ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps_&_aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	2.26 (1.87-2.77)	2.94 (2.46-3.61)	3.98 (3.29-4.87)	4.78 (3.94-5.83)	5.86 (4.74-7.13)	6.68 (5.35-8.08)	7.54 (5.93-9.08)	8.40 (6.49-10.1)	9.56 (7.20-11.5)	10.4 (7.70-12.6)
10-min	1.72 (1.43-2.11)	2.24 (1.87-2.75)	3.03 (2.51-3.71)	3.64 (2.99-4.44)	4.46 (3.61-5.42)	5.09 (4.07-6.14)	5.74 (4.51-6.92)	6.39 (4.94-7.69)	7.28 (5.48-8.76)	7.95 (5.87-9.58)
15-min	1.42 (1.18-1.74)	1.85 (1.55-2.28)	2.50 (2.07-3.06)	3.00 (2.47-3.67)	3.68 (2.98-4.48)	4.20 (3.36-5.08)	4.74 (3.73-5.72)	5.28 (4.08-6.36)	6.01 (4.53-7.24)	6.57 (4.85-7.92)
30-min	0.954 (0.792-1.17)	1.25 (1.04-1.53)	1.69 (1.39-2.06)	2.02 (1.67-2.47)	2.48 (2.01-3.02)	2.83 (2.27-3.42)	3.19 (2.51-3.85)	3.56 (2.75-4.28)	4.05 (3.05-4.87)	4.42 (3.26-5.33)
60-min	0.591 (0.491-0.725)	0.771 (0.645-0.948)	1.04 (0.863-1.28)	1.25 (1.03-1.53)	1.53 (1.24-1.87)	1.75 (1.40-2.12)	1.98 (1.55-2.38)	2.20 (1.70-2.65)	2.51 (1.89-3.02)	2.74 (2.02-3.30)
2-hr	0.346 (0.292-0.415)	0.448 (0.378-0.538)	0.596 (0.501-0.714)	0.710 (0.590-0.850)	0.866 (0.714-1.03)	0.984 (0.800-1.17)	1.11 (0.884-1.31)	1.23 (0.966-1.46)	1.40 (1.07-1.65)	1.53 (1.15-1.81)
3-hr	0.256 (0.215-0.313)	0.328 (0.277-0.404)	0.429 (0.360-0.524)	0.509 (0.423-0.618)	0.621 (0.508-0.750)	0.710 (0.573-0.853)	0.803 (0.636-0.964)	0.900 (0.701-1.08)	1.03 (0.781-1.24)	1.14 (0.842-1.37)
6-hr	0.154 (0.132-0.184)	0.195 (0.168-0.232)	0.249 (0.213-0.295)	0.293 (0.247-0.345)	0.351 (0.293-0.412)	0.397 (0.326-0.465)	0.445 (0.360-0.519)	0.494 (0.393-0.578)	0.561 (0.434-0.655)	0.613 (0.464-0.718)
12-hr	0.085 (0.074-0.100)	0.107 (0.093-0.126)	0.136 (0.117-0.159)	0.158 (0.135-0.184)	0.188 (0.159-0.219)	0.210 (0.176-0.245)	0.234 (0.193-0.272)	0.258 (0.210-0.299)	0.290 (0.230-0.338)	0.315 (0.245-0.370)
24-hr	0.050 (0.044-0.057)	0.063 (0.056-0.073)	0.082 (0.072-0.094)	0.096 (0.085-0.111)	0.117 (0.102-0.134)	0.133 (0.115-0.152)	0.149 (0.128-0.171)	0.167 (0.142-0.190)	0.191 (0.160-0.218)	0.209 (0.173-0.240)
2-day	0.027 (0.024-0.031)	0.034 (0.030-0.039)	0.045 (0.039-0.051)	0.053 (0.047-0.061)	0.065 (0.056-0.074)	0.074 (0.064-0.085)	0.084 (0.072-0.096)	0.094 (0.080-0.108)	0.109 (0.091-0.125)	0.120 (0.099-0.138)
3-day	0.019 (0.017-0.022)	0.025 (0.022-0.028)	0.032 (0.028-0.037)	0.038 (0.034-0.044)	0.047 (0.041-0.054)	0.054 (0.047-0.062)	0.062 (0.053-0.070)	0.069 (0.059-0.079)	0.080 (0.067-0.092)	0.089 (0.074-0.102)
4-day	0.015 (0.014-0.018)	0.020 (0.017-0.022)	0.026 (0.023-0.030)	0.031 (0.027-0.035)	0.038 (0.033-0.043)	0.044 (0.038-0.050)	0.050 (0.043-0.057)	0.057 (0.048-0.065)	0.066 (0.056-0.075)	0.074 (0.062-0.084)
7-day	0.010 (0.009-0.011)	0.013 (0.011-0.014)	0.017 (0.015-0.019)	0.020 (0.017-0.023)	0.025 (0.021-0.028)	0.028 (0.024-0.032)	0.032 (0.028-0.037)	0.037 (0.031-0.042)	0.043 (0.036-0.049)	0.047 (0.039-0.054)
10-day	0.007 (0.007-0.009)	0.010 (0.008-0.011)	0.013 (0.011-0.014)	0.015 (0.013-0.017)	0.019 (0.016-0.021)	0.021 (0.018-0.024)	0.024 (0.021-0.027)	0.027 (0.023-0.031)	0.032 (0.027-0.036)	0.035 (0.029-0.040)
20-day	0.005 (0.004-0.005)	0.006 (0.005-0.007)	0.008 (0.007-0.009)	0.009 (0.008-0.010)	0.011 (0.010-0.013)	0.013 (0.011-0.014)	0.014 (0.012-0.016)	0.016 (0.014-0.018)	0.018 (0.015-0.020)	0.019 (0.016-0.022)
30-day	0.004 (0.003-0.004)	0.005 (0.004-0.005)	0.006 (0.005-0.007)	0.007 (0.006-0.008)	0.009 (0.008-0.010)	0.010 (0.009-0.011)	0.011 (0.010-0.012)	0.012 (0.011-0.014)	0.014 (0.012-0.016)	0.015 (0.013-0.017)
45-day	0.003 (0.002-0.003)	0.004 (0.003-0.004)	0.005 (0.004-0.005)	0.006 (0.005-0.006)	0.007 (0.006-0.008)	0.008 (0.007-0.008)	0.008 (0.007-0.009)	0.009 (0.008-0.010)	0.010 (0.009-0.012)	0.011 (0.010-0.013)
60-day	0.002 (0.002-0.003)	0.003 (0.003-0.003)	0.004 (0.003-0.004)	0.005 (0.004-0.005)	0.005 (0.005-0.006)	0.006 (0.005-0.007)	0.007 (0.006-0.008)	0.007 (0.007-0.008)	0.008 (0.007-0.009)	0.009 (0.008-0.010)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

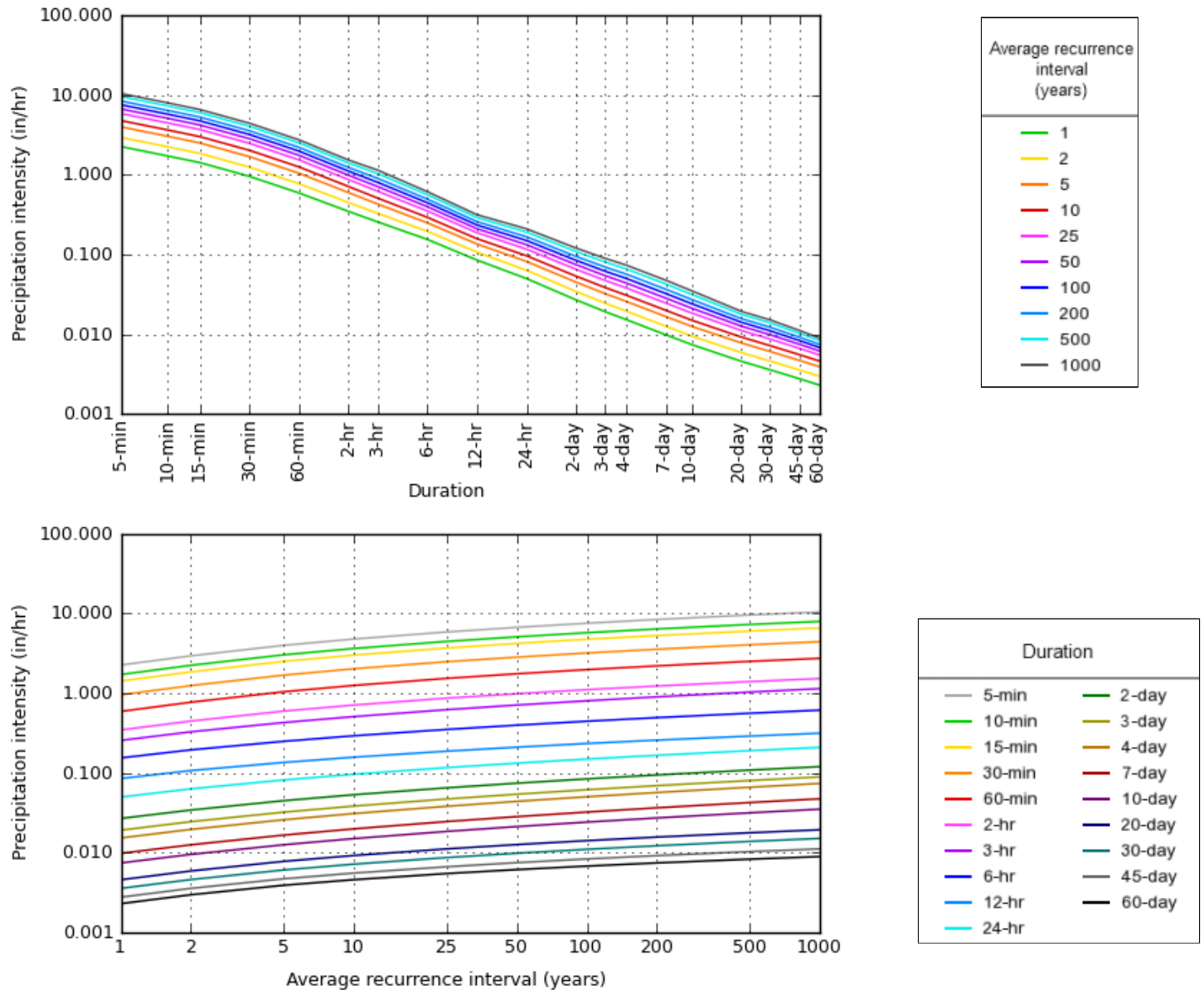
Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based intensity-duration-frequency (IDF) curves

Latitude: 33.5798°, Longitude: -111.8816°



NOAA Atlas 14, Volume 1, Version 5

Created (GMT): Tue Apr 27 19:35:19 2021

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NOAA Atlas 14, Volume 1, Version 5
Location name: Scottsdale, Arizona, USA*
Latitude: 33.5798°, Longitude: -111.8816°
Elevation: 1370.67 ft**

* source: ESRI Maps

** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

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NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps_&_aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.188 (0.156-0.231)	0.245 (0.205-0.301)	0.332 (0.274-0.406)	0.398 (0.328-0.486)	0.488 (0.395-0.594)	0.557 (0.446-0.673)	0.628 (0.494-0.757)	0.700 (0.541-0.842)	0.797 (0.600-0.959)	0.870 (0.642-1.05)
10-min	0.286 (0.238-0.351)	0.373 (0.312-0.459)	0.505 (0.418-0.618)	0.606 (0.499-0.740)	0.743 (0.602-0.903)	0.848 (0.679-1.02)	0.956 (0.752-1.15)	1.07 (0.823-1.28)	1.21 (0.913-1.46)	1.33 (0.978-1.60)
15-min	0.355 (0.295-0.435)	0.463 (0.387-0.569)	0.626 (0.518-0.766)	0.751 (0.618-0.917)	0.921 (0.746-1.12)	1.05 (0.841-1.27)	1.19 (0.932-1.43)	1.32 (1.02-1.59)	1.50 (1.13-1.81)	1.64 (1.21-1.98)
30-min	0.477 (0.396-0.586)	0.623 (0.521-0.766)	0.843 (0.697-1.03)	1.01 (0.833-1.23)	1.24 (1.00-1.51)	1.42 (1.13-1.71)	1.60 (1.25-1.92)	1.78 (1.37-2.14)	2.03 (1.52-2.44)	2.21 (1.63-2.67)
60-min	0.591 (0.491-0.725)	0.771 (0.645-0.948)	1.04 (0.863-1.28)	1.25 (1.03-1.53)	1.53 (1.24-1.87)	1.75 (1.40-2.12)	1.98 (1.55-2.38)	2.20 (1.70-2.65)	2.51 (1.89-3.02)	2.74 (2.02-3.30)
2-hr	0.693 (0.583-0.830)	0.895 (0.756-1.08)	1.19 (1.00-1.43)	1.42 (1.18-1.70)	1.73 (1.43-2.06)	1.97 (1.60-2.34)	2.22 (1.77-2.62)	2.46 (1.93-2.91)	2.80 (2.14-3.31)	3.05 (2.29-3.63)
3-hr	0.769 (0.647-0.941)	0.985 (0.831-1.21)	1.29 (1.08-1.58)	1.53 (1.27-1.86)	1.87 (1.53-2.25)	2.13 (1.72-2.56)	2.41 (1.91-2.90)	2.70 (2.11-3.24)	3.10 (2.35-3.72)	3.43 (2.53-4.11)
6-hr	0.925 (0.793-1.10)	1.17 (1.00-1.39)	1.49 (1.27-1.77)	1.75 (1.48-2.06)	2.10 (1.76-2.47)	2.38 (1.95-2.78)	2.67 (2.16-3.11)	2.96 (2.35-3.46)	3.36 (2.60-3.92)	3.67 (2.78-4.30)
12-hr	1.03 (0.887-1.21)	1.29 (1.12-1.52)	1.63 (1.41-1.91)	1.90 (1.62-2.22)	2.26 (1.91-2.63)	2.54 (2.12-2.95)	2.82 (2.32-3.27)	3.11 (2.53-3.61)	3.49 (2.77-4.08)	3.80 (2.95-4.45)
24-hr	1.20 (1.06-1.38)	1.52 (1.34-1.75)	1.96 (1.73-2.26)	2.31 (2.03-2.65)	2.80 (2.44-3.21)	3.19 (2.75-3.64)	3.59 (3.08-4.10)	4.00 (3.40-4.57)	4.57 (3.83-5.23)	5.03 (4.16-5.77)
2-day	1.29 (1.14-1.48)	1.65 (1.45-1.89)	2.16 (1.89-2.47)	2.56 (2.24-2.93)	3.13 (2.71-3.57)	3.57 (3.08-4.08)	4.05 (3.45-4.63)	4.54 (3.84-5.19)	5.22 (4.36-5.99)	5.76 (4.76-6.63)
3-day	1.38 (1.22-1.58)	1.77 (1.55-2.02)	2.32 (2.04-2.65)	2.77 (2.42-3.16)	3.40 (2.96-3.87)	3.90 (3.37-4.44)	4.44 (3.80-5.05)	5.00 (4.25-5.71)	5.79 (4.86-6.61)	6.43 (5.33-7.36)
4-day	1.47 (1.30-1.68)	1.88 (1.66-2.15)	2.49 (2.19-2.83)	2.98 (2.61-3.39)	3.67 (3.20-4.17)	4.23 (3.66-4.80)	4.83 (4.15-5.48)	5.46 (4.66-6.22)	6.36 (5.35-7.23)	7.09 (5.91-8.08)
7-day	1.65 (1.45-1.90)	2.11 (1.85-2.42)	2.80 (2.45-3.20)	3.35 (2.92-3.83)	4.13 (3.58-4.72)	4.76 (4.11-5.43)	5.44 (4.65-6.20)	6.15 (5.22-7.02)	7.16 (6.00-8.18)	7.97 (6.61-9.13)
10-day	1.79 (1.57-2.04)	2.29 (2.01-2.61)	3.02 (2.65-3.44)	3.61 (3.16-4.11)	4.44 (3.86-5.04)	5.11 (4.42-5.79)	5.81 (4.99-6.59)	6.56 (5.59-7.44)	7.60 (6.40-8.63)	8.44 (7.04-9.60)
20-day	2.20 (1.95-2.51)	2.84 (2.50-3.22)	3.75 (3.30-4.25)	4.44 (3.89-5.03)	5.37 (4.69-6.07)	6.08 (5.30-6.88)	6.81 (5.90-7.72)	7.55 (6.51-8.57)	8.55 (7.31-9.72)	9.32 (7.90-10.6)
30-day	2.58 (2.27-2.93)	3.32 (2.93-3.77)	4.38 (3.86-4.96)	5.19 (4.56-5.86)	6.27 (5.48-7.08)	7.10 (6.19-8.01)	7.96 (6.90-8.97)	8.82 (7.61-9.95)	9.99 (8.56-11.3)	10.9 (9.26-12.3)
45-day	3.00 (2.65-3.39)	3.86 (3.42-4.37)	5.09 (4.51-5.75)	6.01 (5.30-6.78)	7.21 (6.34-8.13)	8.11 (7.11-9.16)	9.03 (7.87-10.2)	9.95 (8.63-11.3)	11.2 (9.61-12.7)	12.1 (10.3-13.7)
60-day	3.31 (2.95-3.74)	4.28 (3.81-4.83)	5.64 (5.00-6.35)	6.62 (5.86-7.46)	7.90 (6.98-8.89)	8.85 (7.79-9.97)	9.80 (8.59-11.1)	10.7 (9.37-12.1)	12.0 (10.4-13.5)	12.9 (11.1-14.6)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

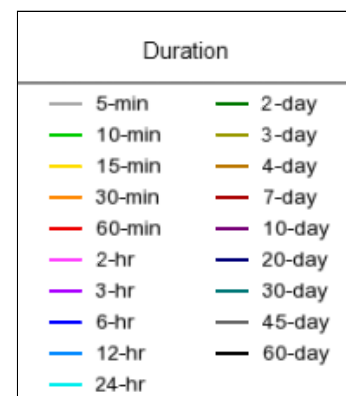
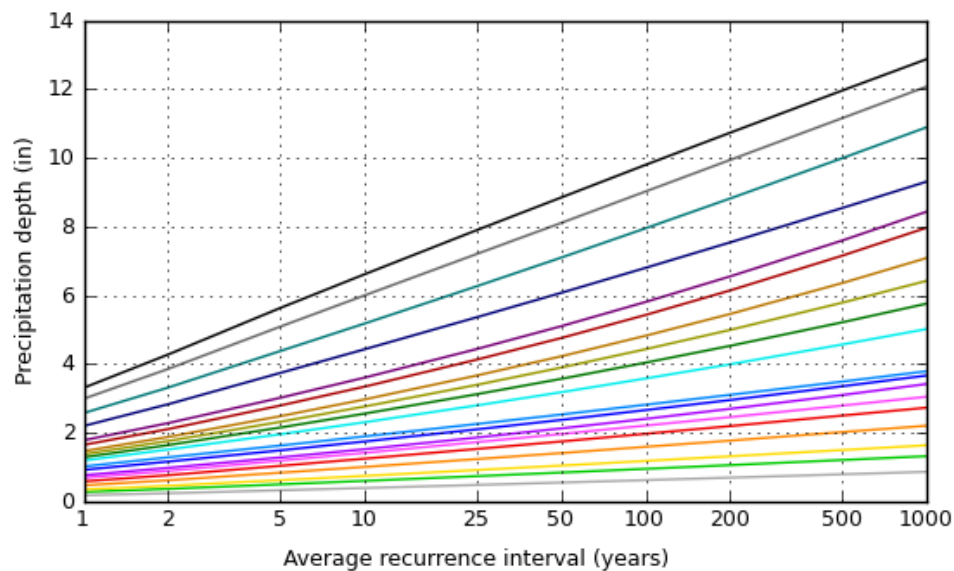
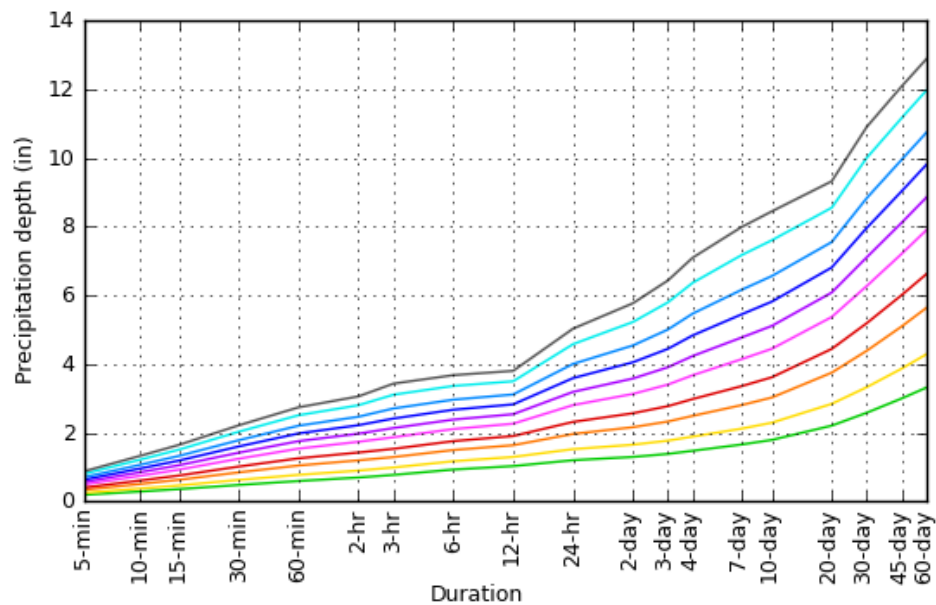
Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves

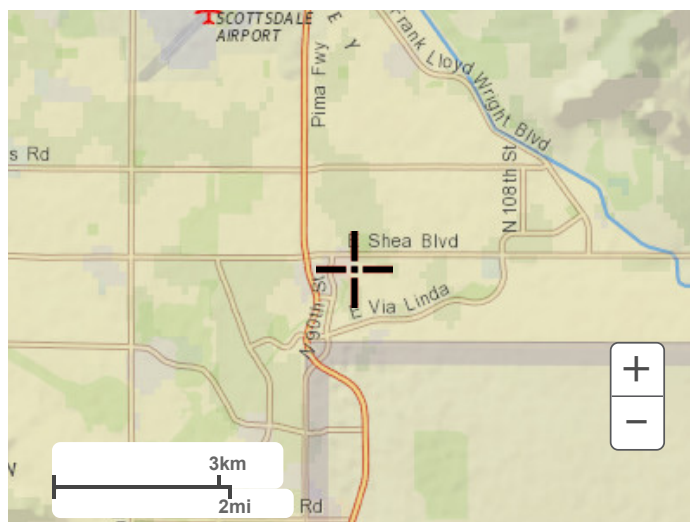
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Created (GMT): Tue Apr 27 19:33:20 2021

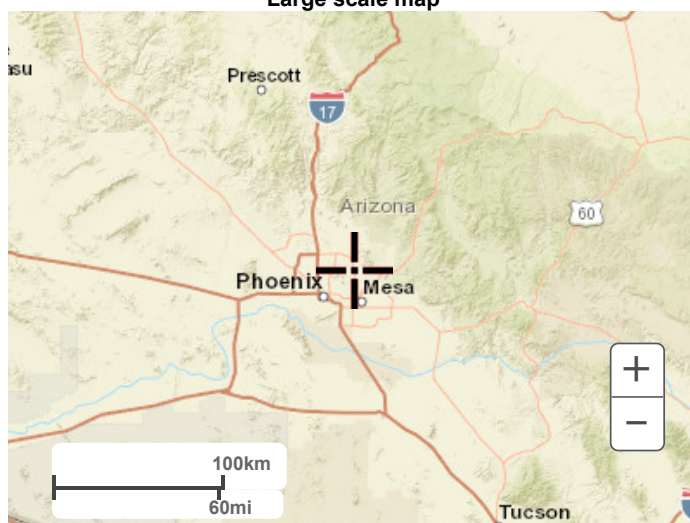
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Large scale terrain



Large scale map



Large scale aerial

APPENDIX II

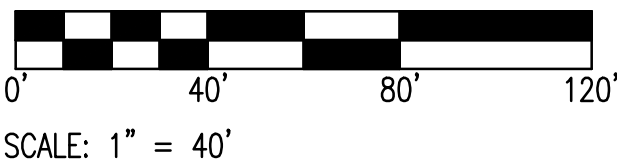
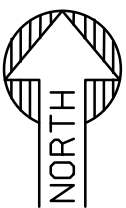
CALCULATIONS

MERCADO VILLAGE
EXISTING CONDITIONS C_{WT} EXHIBIT

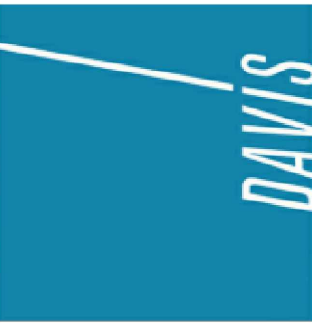
92ND & SHEA, SCOTTSDALE, ARIZONA, 85258



---	PROPERTY LINE			
	BUILDING/PAVED SURFACE =	145,053 SF (3.33 AC)	@ CWT=0.95	
	UNDISTURBED NATURAL DESERT =	118,895 SF (2.73 AC)	@ CWT=0.45	
	TOTAL ON-SITE CWT =	263,948 SF (6.06 AC)	@ CWT=0.72	



PRELIMINARY
NOT FOR
CONSTRUCTION



PROJECT	MERCADO VILLAGE	LOCATION	92ND & SHEA, SCOTTSDALE, AZ 85258
DRAWN	JC	03/18/2024	
DESIGNED	JC	03/18/2024	
QC	SC	01/05/2024	
FINAL QC	BC	03/11/2024	
PROJ. MGR.	AF	03/18/2024	

DATE: 03/18/2024
ISSUED FOR: ZONING

REVISION NO.:	DATE:
1	
2	
3	

JOB NO.: 210414

SHEET TITLE:

EXISTING CONDITIONS
C_{WT} EXHIBIT

PAGE NO.: 1 OF 1
SHEET NO.: X-CWT

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MERCADO VILLAGE
POST CONDITIONS C_{WT} EXHIBIT

92ND & SHEA, SCOTTSDALE, ARIZONA, 85258

OWNER: 94 HUNDRED SHEA LLLP
APN: 217-36-001M

OWNER: SHEA AND 92ND OPCO LLC
APN: 217-36-960L

EXISTING
BUILDING

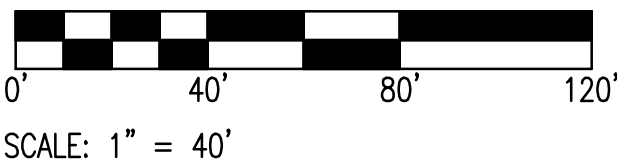
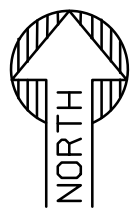
EXISTING
BUILDING

OWNER: 9221 ICON LLC
APN: 217-36-989B

OWNER: JANICEK MIKE F/PATRICIA B
APN: 217-74-038

N 92ND STREET

---	PROPERTY LINE			
	BUILDING/PAVED SURFACE =	188,898 SF (4.34 AC)	⊙ CWT=0.95 AC	
	UNDISTURBED NATURAL DESERT =	75,047 SF (1.72 AC)	⊙ CWT=0.45 AC	
	TOTAL ON-SITE CWT =	263,945 SF (6.06 AC)	⊙ CWT=0.81	
	BUILDING ROOF AREA =	124,050 SF (2.85 AC)	⊙ CWT=0.95 AC	



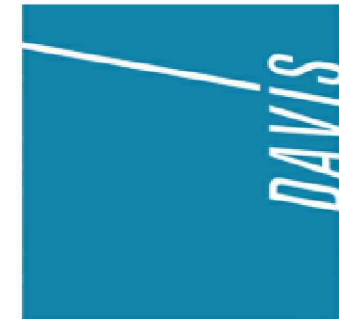
PRELIMINARY
NOT FOR
CONSTRUCTION

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5240 N. 16TH STREET SUITE 105, PHOENIX, ARIZONA 85016
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PROJECT
MERCADO VILLAGE

LOCATION
92ND & SHEA,
SCOTTSDALE, AZ 85258

DRAWN _____ JC 03/18/2024
DESIGNED _____ JC 03/18/2024
QC _____ SC 01/05/2024
FINAL QC _____ BC 03/11/2024
PROJ. MGR. _____ AF 03/18/2024

DATE: 03/18/2024

ISSUED FOR: ZONING

REVISION NO.:	DATE:
1	
2	
3	
4	

JOB NO.: 210414

SHEET TITLE:

POST CONDITIONS
C_{WT} EXHIBIT

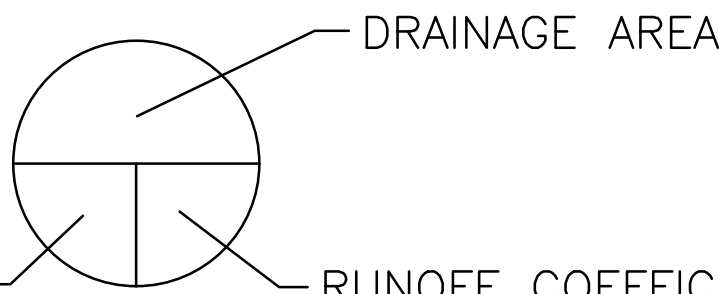
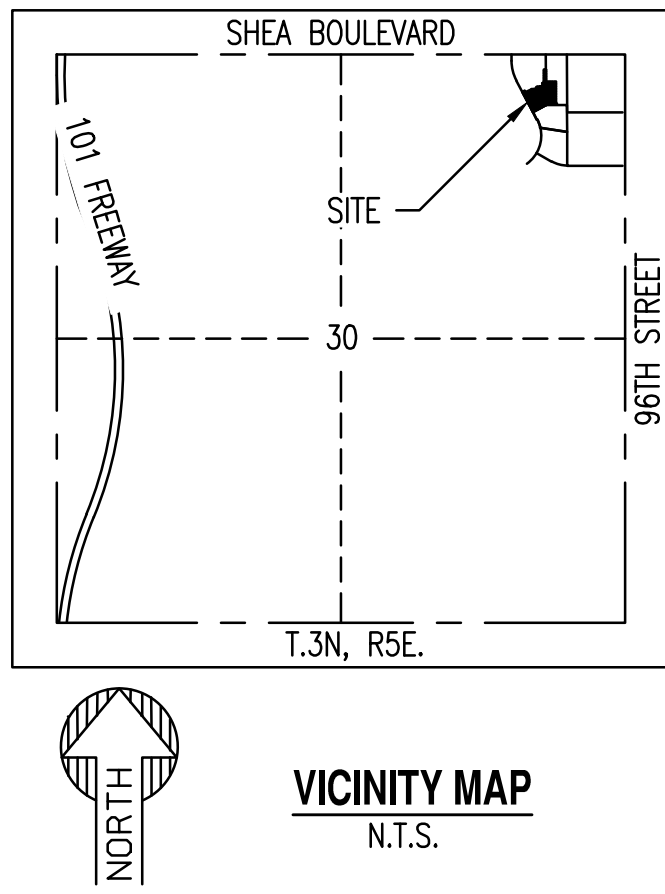
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MERCADO VILLAGE
EXISTING CONDITIONS DRAINAGE AREA MAP

10301 N. 92ND STREET. SCOTTSDALE, AZ 85258



AREA IN ACRES

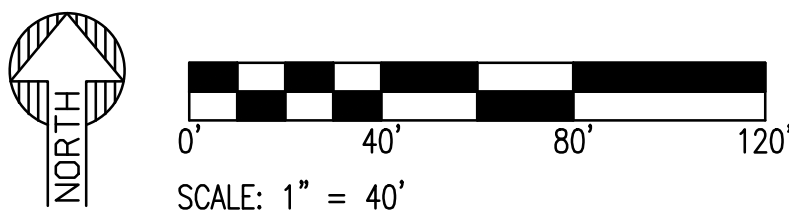
DRAINAGE AREA KEY

PROPOSED LEGEND

- DRAINAGE AREAS DISCHARGING TO PUBLIC STORM DRAIN SYSTEM AT 92ND STREET.
- DRAINAGE AREAS DISCHARGING TO EXISTING OPEN RETENTION BASIN.
- FLOW ARROW

EXISTING ONSITE 100 YR 5 MIN FLOWS

Drainage Area	Area (ac)	Cw	i (in/hr)	Q (cfs)	Q Total (cfs)	Control Point
EX-1	1.83	0.83	7.54	11.45	27.75	Public Storm Drain
EX-2	0.42	0.65	7.54	2.04		
EX-3	0.37	0.84	7.54	2.37		
EX-4	0.52	0.75	7.54	2.94		
EX-5	0.39	0.74	7.54	2.17		
EX-6	0.05	0.95	7.54	0.39		
EX-R-1	0.69	0.95	7.54	4.93		
EX-R-2	0.20	0.95	7.54	1.47	5.37	EX-BASIN 1
EX-7	1.58	0.45	7.54	5.37		



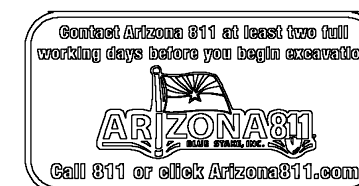
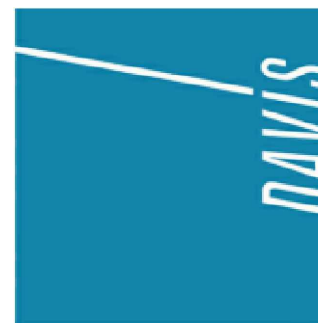
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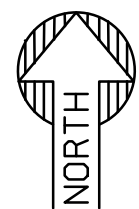
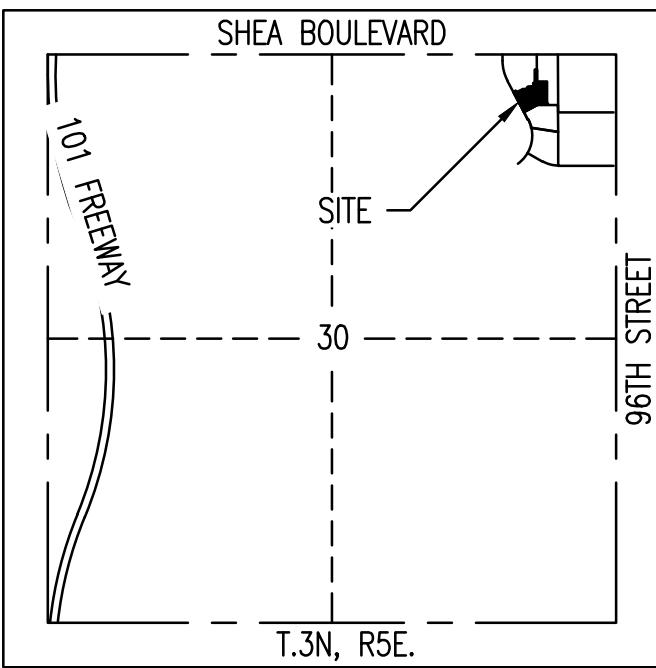
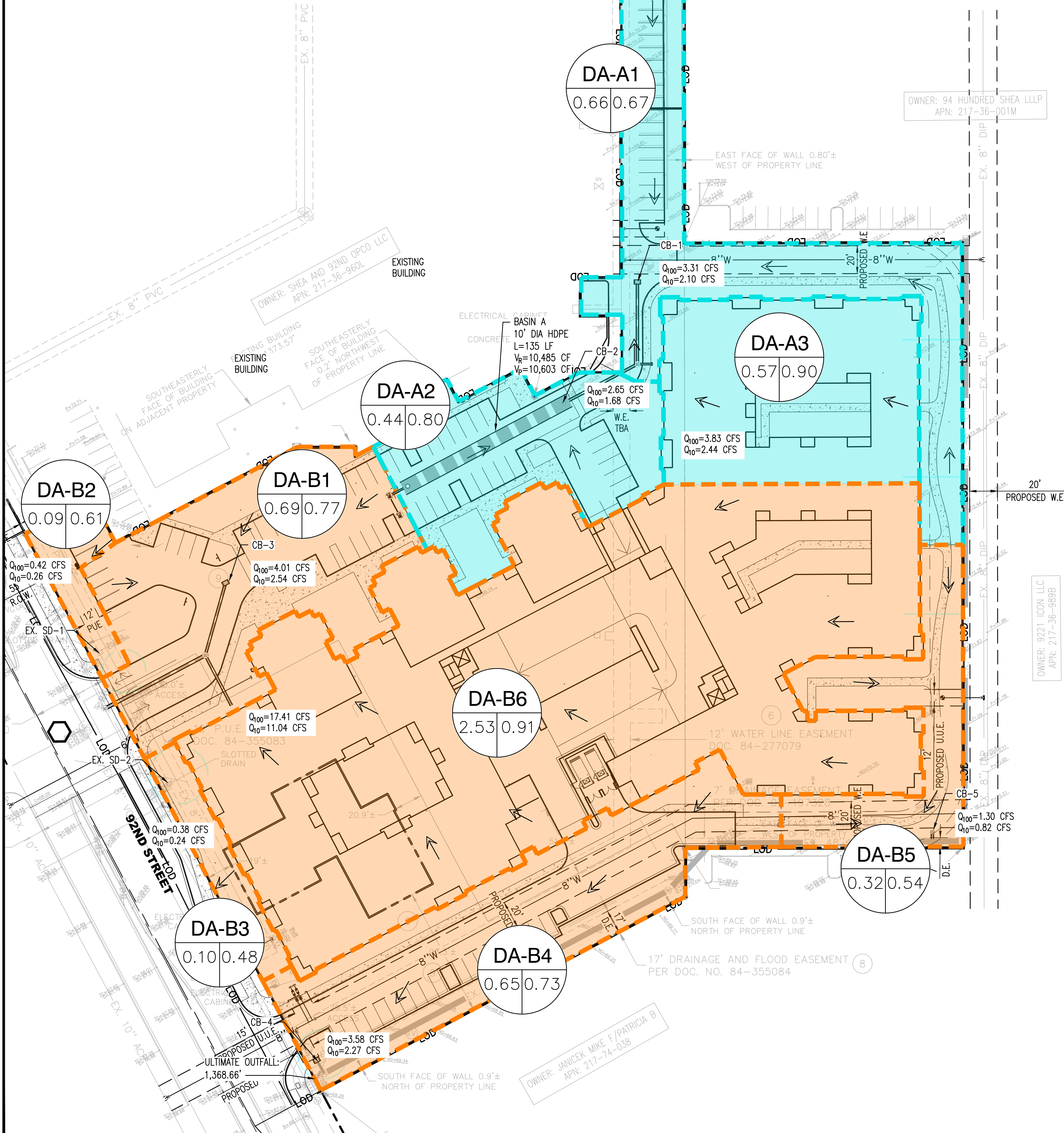


PROJECT	MERCADO VILLAGE	LOCATION	92ND & SHEA, SCOTTSDALE, AZ 85258
DRAWN	JC	03/18/2024	
DESIGNED	JC	03/18/2024	
QC	SC	01/05/2024	
FINAL QC	BC	03/11/2024	
PROJ. MGR.	AF	03/18/2024	
DATE:	03/18/2024		
ISSUED FOR:	ZONING		
REVISION NO.:		DATE:	
JOB NO.:	210414		
SHEET TITLE:			
PAGE NO.:	1 OF 1	SHEET NO.:	X-DAM

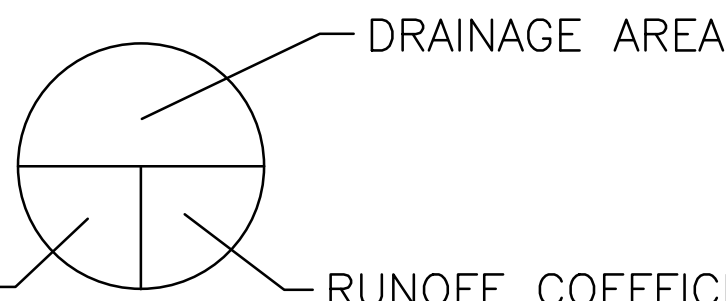
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MERCADO VILLAGE
POST CONDITIONS DRAINAGE AREA MAP

10301 N. 92ND STREET. SCOTTSDALE, AZ 85258



VICINITY MAP
N.T.S.



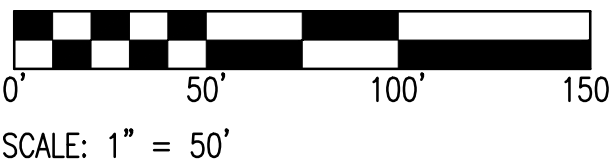
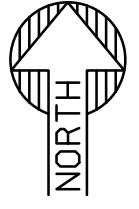
AREA IN ACRES RUNOFF COEFFICIENT

DRAINAGE AREA KEY

- DRAINAGE AREAS DISCHARGING TO BASIN A
- DRAINAGE AREAS DISCHARGING TO PUBLIC STORM DRAIN SYSTEM AT 92ND STREET
- FLOW ARROW

PROPOSED ONSITE 100 YR 5 MIN				
Drainage Area	Area (ac)	Cw	i (in/hr)	Q (cfs)
A1	0.66	0.67	7.54	3.31
A2	0.44	0.80	7.54	2.65
A3	0.11	0.95	7.54	0.76
B1	0.69	0.77	7.54	4.01
B2	0.09	0.61	7.54	0.42
B3	0.10	0.48	7.54	0.38
B4	0.65	0.73	7.54	3.58
B5	0.32	0.54	7.54	1.30
B6	3.00	0.91	7.54	20.50

Required Storage Volume					
Vr=1*(P/12)*Cw*A					
P=100-yr, 2-hr=2.22 in.					
Drainage Area ID	Area (acres)	C _w (-)	Precipitation (in)	Volume Req. (acre-ft)	Volume Req. (CF)
A1	0.66	0.67	2.22	0.08	3,534.66
A2	0.44	0.80	2.22	0.07	2,836.78
A3	0.57	0.90	2.22	0.09	4,113.86
BASIN A				Total Retention:	10,485.29



SCALE: 1" = 50'

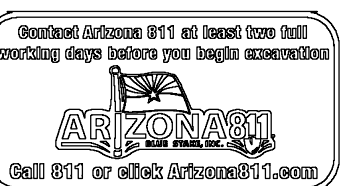
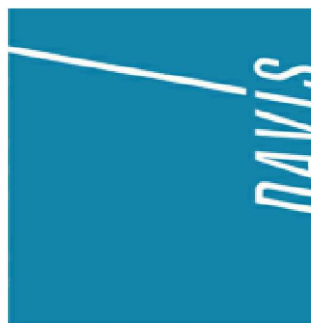
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PROJECT
MERCADO VILLAGE

LOCATION
92ND & SHEA,
SCOTTSDALE, AZ 85258

DRAWN: JC 03/18/2024
DESIGNED: JC 03/18/2024
QC: SC 01/05/2024
FINAL QC: BC 03/11/2024
PROJ. MGR: AF 03/18/2024

DATE: 03/18/2024

ISSUED FOR: ZONING

REVISION NO.: DATE:

1	
2	
3	

JOB NO.: 210414

SHEET TITLE:

POST CONDITIONS
DRAINAGE AREA MAP

PAGE NO.:

1 OF 1

SHEET NO.:

P-DM

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EXISTING ONSITE 10 YR 5 MIN FLOWS						
Drainage Area	Area (ac)	Cw	i (in/hr)	Q (cfs)	Q Total (cfs)	Control Point
EX-1	1.83	0.83	4.78	7.26	17.59	Public Storm Drain
EX-2	0.42	0.65	4.78	1.29		
EX-3	0.37	0.84	4.78	1.50		
EX-4	0.52	0.75	4.78	1.86		
EX-5	0.39	0.74	4.78	1.37		
EX-6	0.05	0.95	4.78	0.25		
EX-R-1	0.69	0.95	4.78	3.13		
EX-R-2	0.20	0.95	4.78	0.93		
EX-7	1.58	0.45	4.78	3.40	3.40	EX-BASIN 1

PROPOSED ONSITE 10 YR 5 MIN FLOWS						
Drainage Area	Area (ac)	Cw	i (in/hr)	Q (cfs)	Q Total (cfs)	Control Point
A1	0.66	0.67	4.78	2.10	6.22	BASIN A
A2	0.44	0.80	4.78	1.68		
A3	0.57	0.90	4.78	2.44		
B1	0.69	0.77	4.78	2.54	17.18	Public Storm Drain
B2	0.09	0.61	4.78	0.26		
B3	0.10	0.48	4.78	0.24		
B4	0.65	0.73	4.78	2.27		
B5	0.32	0.54	4.78	0.82		
B6	2.53	0.91	4.78	11.04		

APPENDIX III

Grading & Drainage Plans

MERCADO VILLAGE

PRELIMINARY GRADING AND DRAINAGE PLAN

10301 N. 92ND STREET. SCOTTSDALE, AZ 85258

ENGINEER'S CERTIFICATION:

THE LOWEST FLOOR ELEVATION(S) AND/OR FLOOD PROOFING ELEVATION(S) ON THIS PLAN ARE SUFFICIENTLY HIGH TO PROVIDE PROTECTION FROM FLOODING CAUSED BY A 100-YEAR STORM, AND ARE IN ACCORDANCE WITH SCOTTSDALE REVISED CODE, CHAPTER 37 - FLOODPLAIN AND STORMWATER REGULATION.

BENCHMARK:

THE BENCHMARK USED FOR THIS SURVEY IS THE CITY OF SCOTTSDALE BRASS CAP IN HANDHOLE AT THE INTERSECTION OF SHEA BOULEVARD AND 92ND STREET HAVING AN ELEVATION OF 1372.19', NAVD 88.

BASIS OF BEARING:

THE NORTH LINE OF NORTHWEST QUARTER OF SECTION 30, TOWNSHIP 3 NORTH, RANGE 5 EAST OF THE GILA AND SALT RIVER BASE AND MERIDIAN, MARICOPA COUNTY, ARIZONA, SAID LINE BEARS NORTH 89 DEGREES 56 MINUTES 30 SECONDS EAST.

FEMA

MAP NUMBER	COMMUNITY NUMBER	PANEL # PANEL DATE	SUFFIX	FIRM ZONE
04013C1760L	040037/ 040049/ 040051/ 045012	1760 09/18/2020	L	X SHADED

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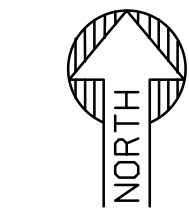
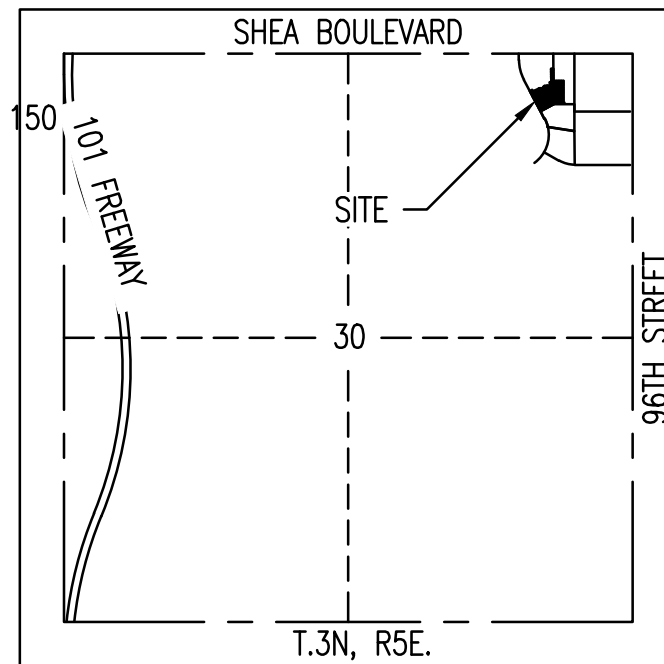
CALIBER
8901 E. MOUNTAIN VIEW ROAD, SUITE
SCOTTSDALE, ARIZONA 85258
PHONE: 480-455-6480
EMAIL: INVEST@CALIBERCO.COM

CIVIL ENGINEER:

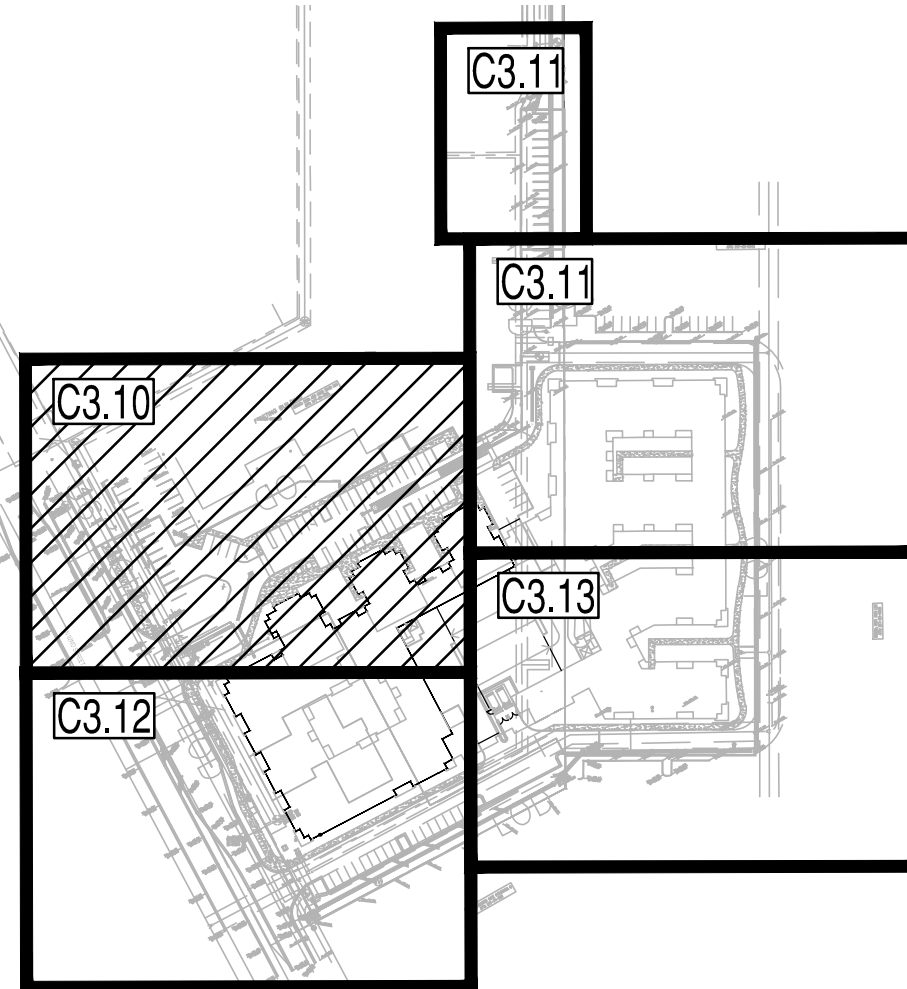
SUSTAINABILITY ENGINEERING GROUP
5240 N. 16TH STREET, SUITE 105
PHOENIX, ARIZONA 85260
PHONE: 480-237-2507
ATTN: ALI FAKIH
EMAIL: ALI@AZSEG.COM

ARCHITECT:

DAVIS GROUP
3033 N. CENTRAL AVENUE, SUITE 800
PHOENIX, ARIZONA 85012
PHONE: 480-638-1100



VICINITY MAP
N.T.S.

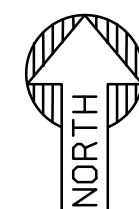


PRELIMINARY GRADING NOTES

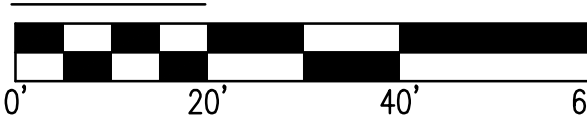
- 1 MATCH EXISTING GRADE.
- 2 6" VERTICAL CURB.
- 3 CONCRETE SIDEWALK.
- 4 UNDERGROUND STORAGE.
- 5 DRYWELL.
- 6 CATCH BASIN.
- 6B 30" CMP RISER WITH STANDARD SOLID LID.
- 7 ROOF DRAIN BUILDING CONNECTION.
- 8 4' DIAMETER STORM MANHOLE STRUCTURE.
- 10 HDPE DOUBLE WALL PIPE. LENGTH, SIZE AND SLOPE PER PLAN.

ABBREVIATIONS

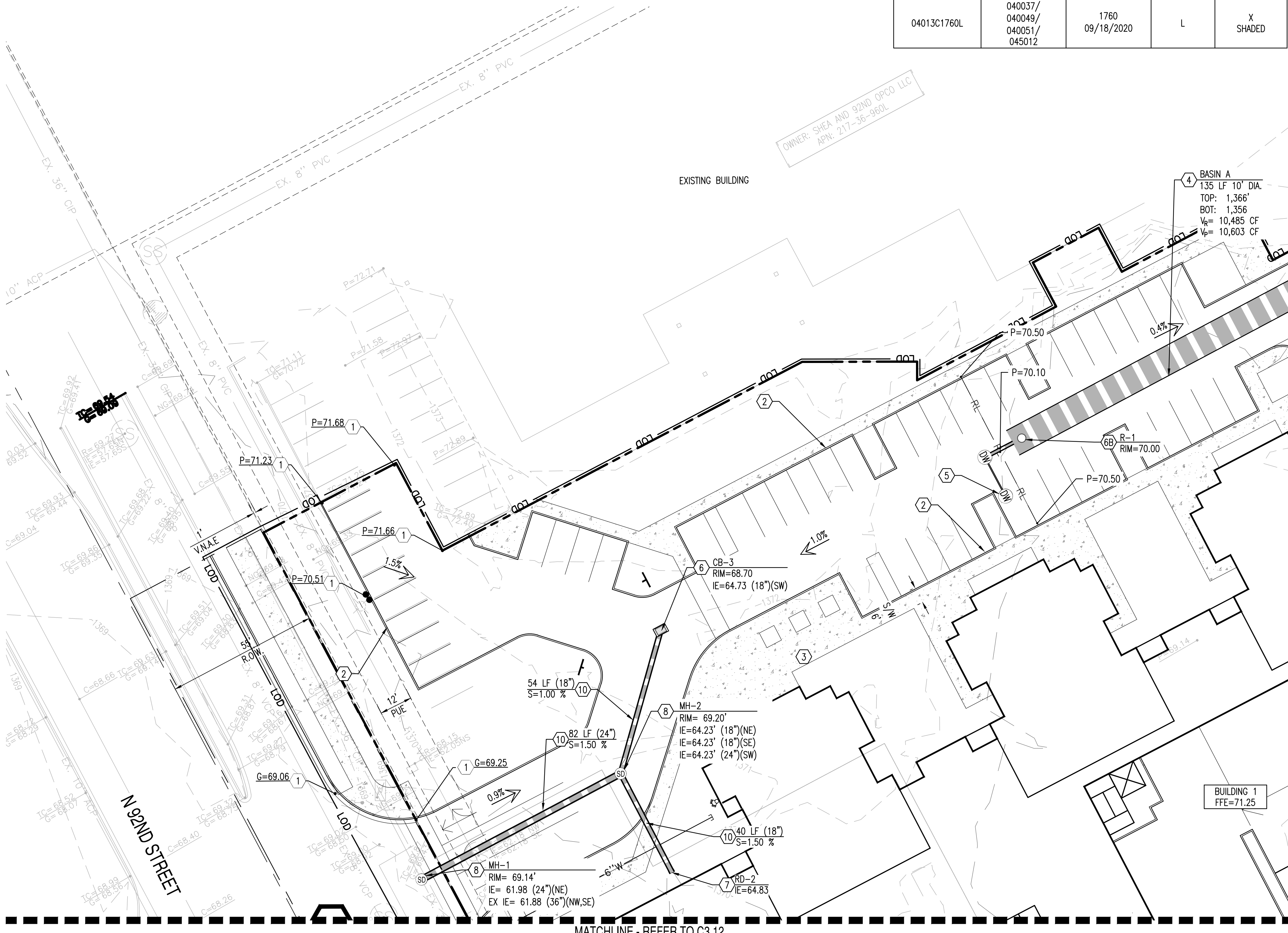
APN	ASSESSOR'S PARCEL NAME	P	PAVEMENT
BOT	BOTTOM	PUE	PUBLIC UTILITY EASEMENT
C	CONCRETE	R	RISER
CB	CATCH BASIN	RD	ROOF DRAIN
C.F.	CUBIC FEET	ROW	RIGHTS-OF-WAY
DE	DRAINAGE EASEMENT	S	SLOPE
DIA	DIAMETER	STA	STATION
EX	EXISTING	S/W	SIDEWALK
FFE	FINISHED FLOOR ELEVATION	TBA	TO BE ABANDONED
FG	FINISHED GRADE	UUE	UNDERGROUND UTILITY EASEMENT
G	GUTTER	VNAE	VEHICULAR NON-ACCESS EASEMENT
HDPE	HIGH DENSITY POLYETHYLENE	VP	VOLUME PROVIDED
IE	INVERT ELEVATION	VR	VOLUME REQUIRED
LF	LINEAR FEET	WE	WATER EASEMENT
MH	MANHOLE		
NG	NATURAL GROUND		



PLAN VIEW



SCALE: 1" = 20'



PROPOSED GRADING LEGEND:

G=XX.XX	GUTTER ELEVATION, TC = G+0.5'	---	PROPERTY LINE	---	LOC	---	LIMIT OF ONSITE CONSTRUCTION
P=XX.XX	PAVEMENT ELEVATION	==	CURB AND GUTTER	XX	---	---	MAJOR CONTOUR
C=XX.XX	CONCRETE ELEVATION	---	VERTICAL CURB	XX	---	---	MINOR CONTOUR
		---	RL	---	---	---	FLOW ARROW

	CATCH BASIN		DRYWELL		RIP-RAP
	STORM PIPE		WATER METER		CONCRETE PAVEMENT
	END SECTION		GATE VALVE		HEAVY DUTY PAVEMENT
	STORM MANHOLE		FIRE HYDRANT		LIGHT DUTY PAVEMENT

EXISTING LEGEND:

--- XXXX ---	EX. MAJOR CONTOURS	EX. S	SEWER LINE		STORM DRAIN LINE
--- XXXX ---	EX. MINOR CONTOURS		SEWER MANHOLE		STORM CATCH BASIN
TC:XX.XX GE:XX.XX	EX. SPOT ELEVATION	EX. W	WATER LINE		STORM MANHOLE
---	EASEMENT LINE AS NOTED	WV	WATER VALVE	GAS	GAS LINE
			FIRE HYDRANT	---	FENCE
					SIGN
					STREET LIGHT
					TREE
				---	ROAD CENTERLINE

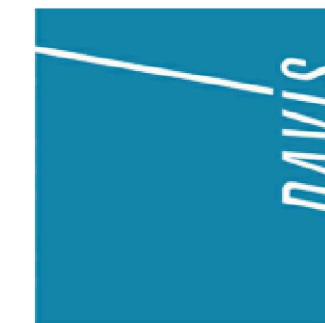
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PROJECT	LOCATION
MERCADO VILLAGE	92ND & SHEA, SCOTTSDALE, AZ 85258

DATE: 03/20/2024

ISSUED FOR: ZONING

REVISION NO.: DATE:

JOB NO.: 210414

SHEET TITLE:

PRELIMINARY
GRADING &
DRAINAGE PLAN

PAGE NO.: 1 OF 5

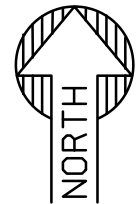
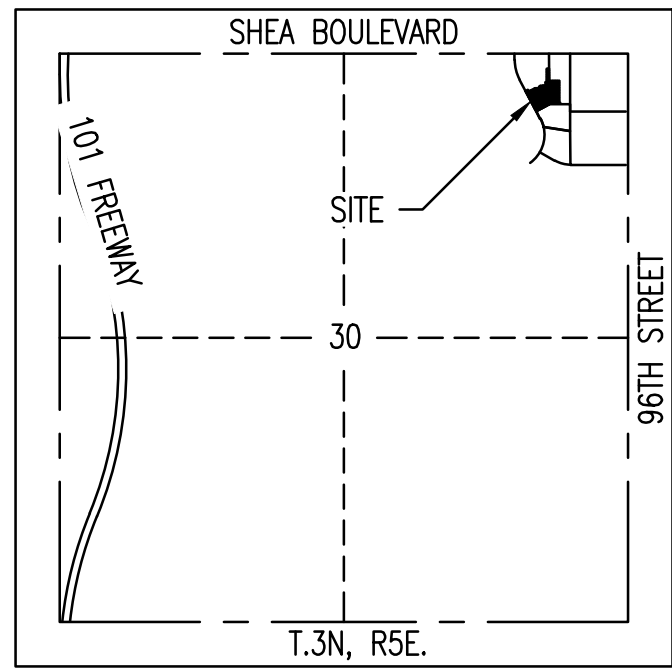
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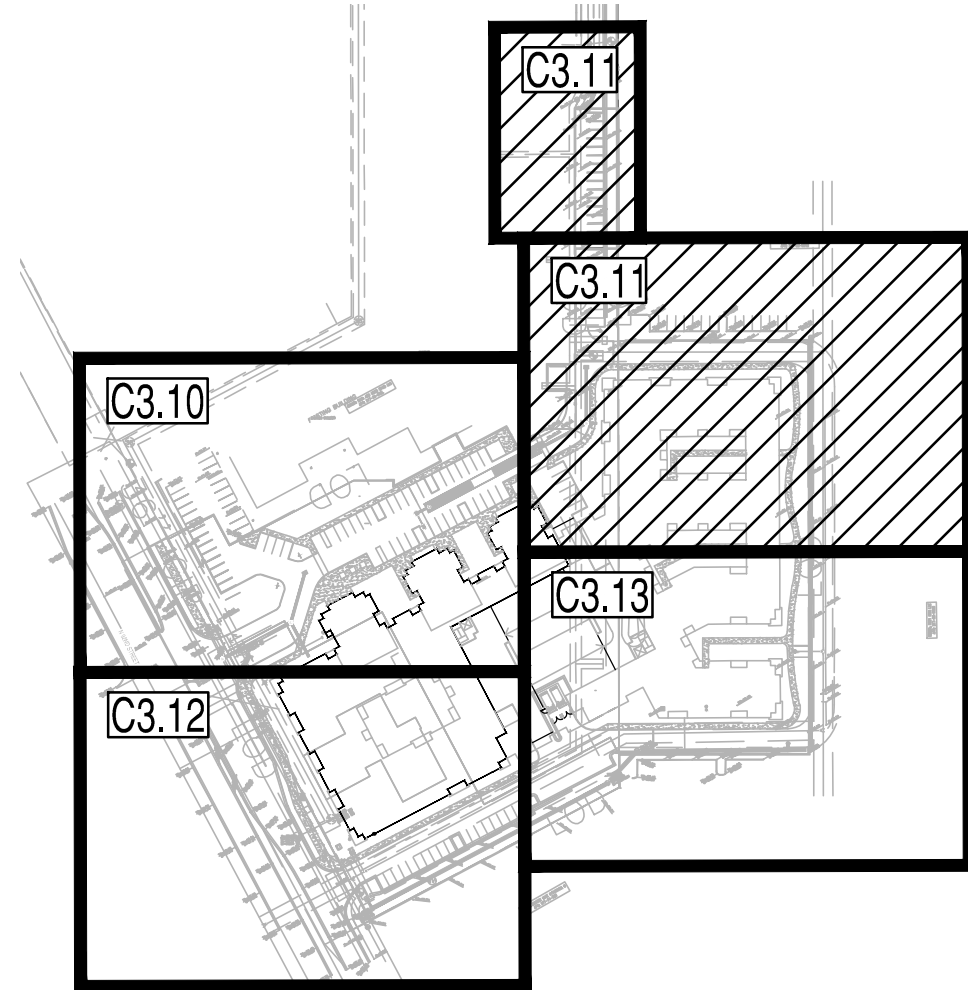
MERCADO VILLAGE
PRELIMINARY GRADING AND DRAINAGE PLAN

10301 N. 92ND STREET. SCOTTSDALE, AZ 85258

OWNER: 94 HUNDRED SHEA LLLP
APN: 217-36-001M

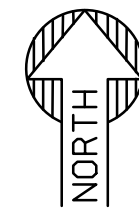


VICINITY MAP
N.T.S.

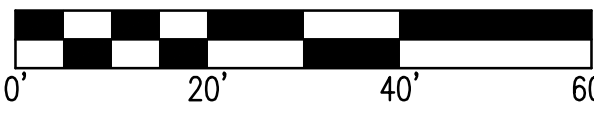


PRELIMINARY GRADING NOTES

- 1 MATCH EXISTING GRADE.
- 2 6" VERTICAL CURB.
- 3 CONCRETE SIDEWALK.
- 4 UNDERGROUND STORAGE.
- 6 CATCH BASIN.
- 6A 30" CMP RISER WITH STANDARD GRATED LID.
- 7 ROOF DRAIN BUILDING CONNECTION.
- 10 HDPE DOUBLE WALL PIPE. LENGTH, SIZE AND SLOPE PER PLAN.



PLAN VIEW



SCALE: 1" = 20'

PROPOSED GRADING LEGEND:

G=XX.XX	GUTTER ELEVATION, TC = G+0.5'	--- LOC ---	LIMIT OF ONSITE CONSTRUCTION
P=XX.XX	PAVEMENT ELEVATION	--- XX ---	MAJOR CONTOUR
C=XX.XX	CONCRETE ELEVATION	--- XX ---	MINOR CONTOUR
		--- RL ---	RIDGELINE
		--- XX ---	FLOW ARROW

	CATCH BASIN
	STORM PIPE
	END SECTION
	STORM MANHOLE

	DRYWELL
	WATER METER
	GATE VALVE
	FIRE HYDRANT

	RIP-RAP
	CONCRETE PAVEMENT
	HEAVY DUTY PAVEMENT
	LIGHT DUTY PAVEMENT

EXISTING LEGEND:

--- XXXX ---	EX. MAJOR CONTOURS	--- EX. S ---	SEWER LINE		STORM DRAIN LINE
--- XXXX ---	EX. MINOR CONTOURS		SEWER MANHOLE		STORM CATCH BASIN
TC:XX.XX GE:XX.XX	EX. SPOT ELEVATION	--- EX. W ---	WATER LINE		STORM MANHOLE
---	EASEMENT LINE AS NOTED		WATER VALVE	---	GAS LINE
			FIRE HYDRANT	---	FENCE
			SIGN	---	ROAD CENTERLINE

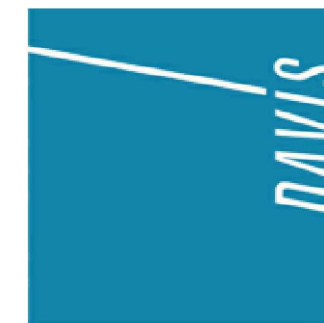
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PROJECT	MERCADO VILLAGE	LOCATION	92ND & SHEA, SCOTTSDALE, AZ 85258
DRAWN	JC	03/18/2024	
DESIGNED	JC	03/18/2024	
QC	SC	01/05/2024	
FINAL QC	BC	03/11/2024	
PROJ. MGR.	AF	03/20/2024	

DATE: 03/20/2024

ISSUED FOR:

ZONING

REVISION NO.:

DATE:

JOB NO.:

210414

SHEET TITLE:

PRELIMINARY
GRADING &
DRAINAGE PLAN

PAGE NO.:

2 OF 5

SHEET NO.:

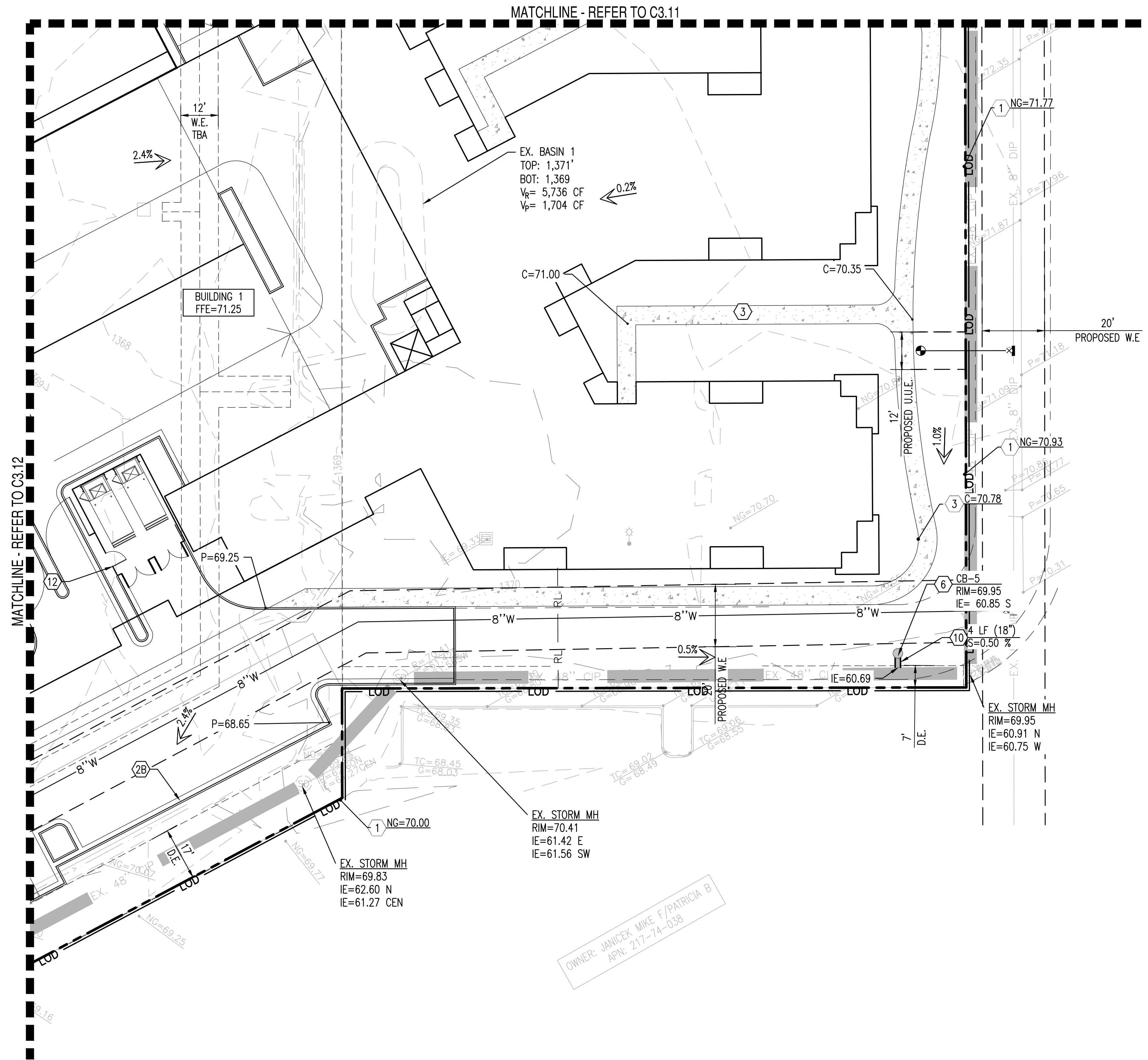
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MERCADO VILLAGE













PRELIMINARY GRADING AND DRAINAGE PLAN





10301 N. 92ND STREET. SCOTTSDALE, AZ 85258











PROPOSED GRADING LEGEND:

G=XX.XX	GUTTER ELEVATION, TC = G+0.5'
P=XX.XX	PAVEMENT ELEVATION
C=XX.XX	CONCRETE ELEVATION

	PROPERTY LINE		LOC		EXIST'G ON-SITE CONSTRUCTION
	CURB AND GUTTER		XX		MAJOR CONTOUR
	VERTICAL CURB		XX		MINOR CONTOUR
	RIDGELINE				FLOW ARROW

	CATCH BASIN
	STORM PIPE
	END SECTION
	STORM MANHOLE

	DRYWELL
	WATER METER
	GATE VALVE
	FIRE HYDRANT


	RIP-RAP
	CONCRETE PAVEMENT
	HEAVY DUTY PAVEMENT
	LIGHT DUTY PAVEMENT

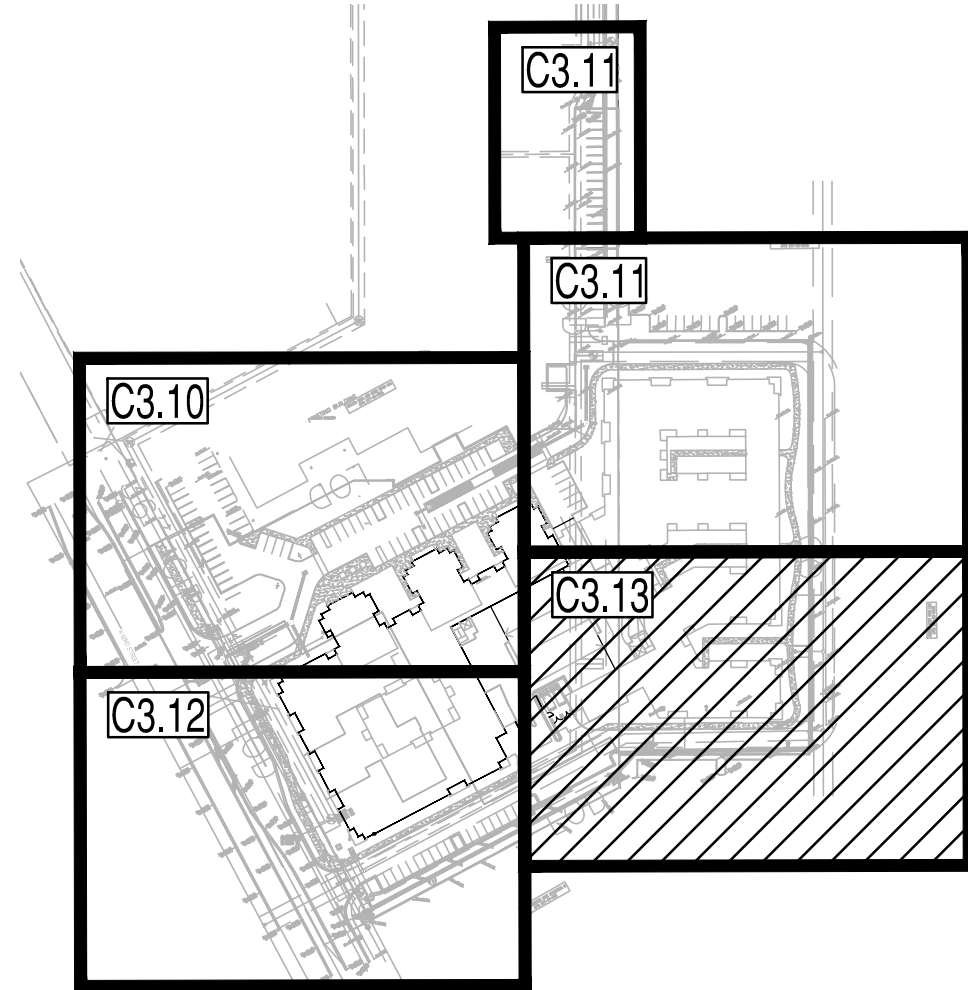
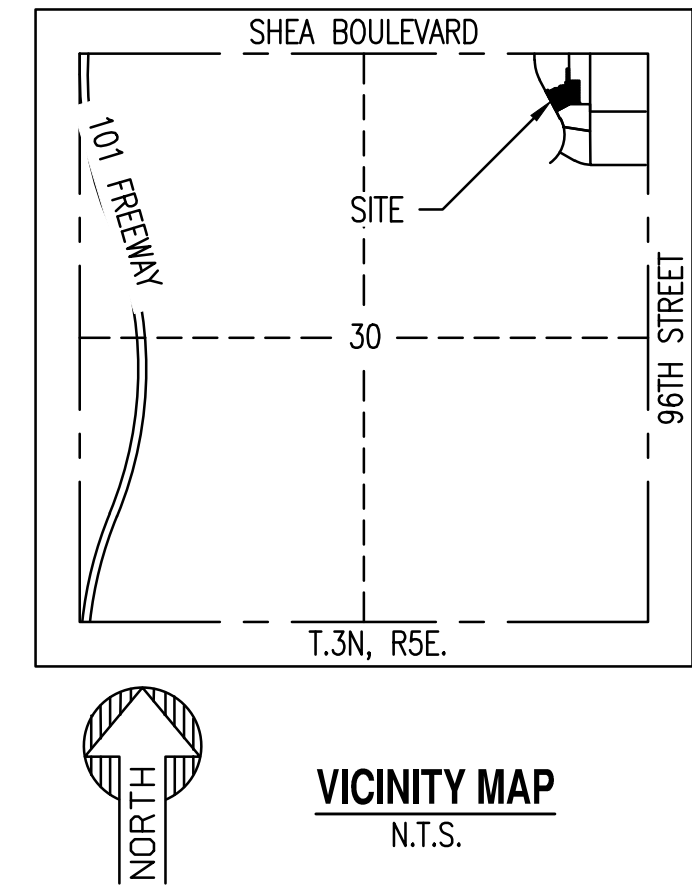
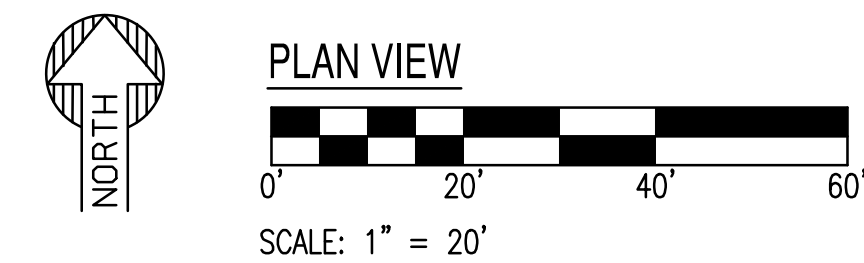
EXISTING LEGEND:

--- XXXX ---	EX. MAJOR CONTOURS
--- XXXX ---	EX. MINOR CONTOURS
TC: XX.XX GE: XX.XX	EX. SPOT ELEVATION
--- ---	EASEMENT LINE AS NOTED

— EX. S —	SEWER LINE
(S)	SEWER MANHOLE
— EX. W —	WATER LINE
WV ⊗	WATER VALVE
⊕	FIRE HYDRANT

	STORM DRAIN LINE		SIGN
	CB		STREET LIGHT
	STORM MANHOLE		TREE
	GAS		
	FENCE		ROAD CENTERLINE

	SIGN
	STREET LIGHT
	TREE
	ROAD CENTERLINE



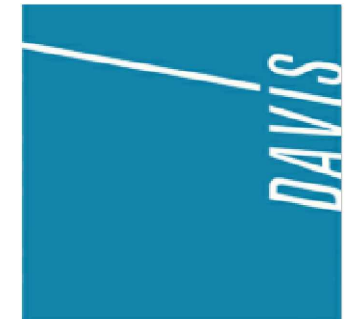
PRELIMINARY GRADING NOTES

- ① MATCH EXISTING GRADE.
- ② 6" VERTICAL CURB.
- ②A 6" VERTICAL CURB & GUTTER.
- ③ CONCRETE SIDEWALK.
- ⑥ CATCH BASIN
- ⑩ HDPE DOUBLE WALL PIPE. LENGTH, SIZE AND SLOPE PER PLAN.
- ⑪ SAWCUT EXISTING PAVEMENT TO PROVIDE STRAIGHT VERTICAL EDGES, FREE FROM IRREGULARITIES.
- ⑫ TRASH ENCLOSURE

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CONSTRUCTION

SUSTAINABILITY
ENGINEERING
GROUP

5240 N. 16TH STREET SUITE 105, PHOENIX, ARIZONA 85016
WWW.AZSEG.COM TEL. 480.588.7226 FAX 480.259.3534



PROJECT		LOCATION	
MERCADO VILLAGE		92ND & SHEA, SCOTTSDALE, AZ 85258	
DRAWN	_____	JC	03/18/2024
DESIGNED	_____	JC	03/18/2024
QC	_____	SC	01/05/2024
FINAL QC	_____	BC	03/11/2024
PROJ. MGR.	_____	AF	03/20/2024

DATE: 03/20/2024

ISSUED FOR: ZONING

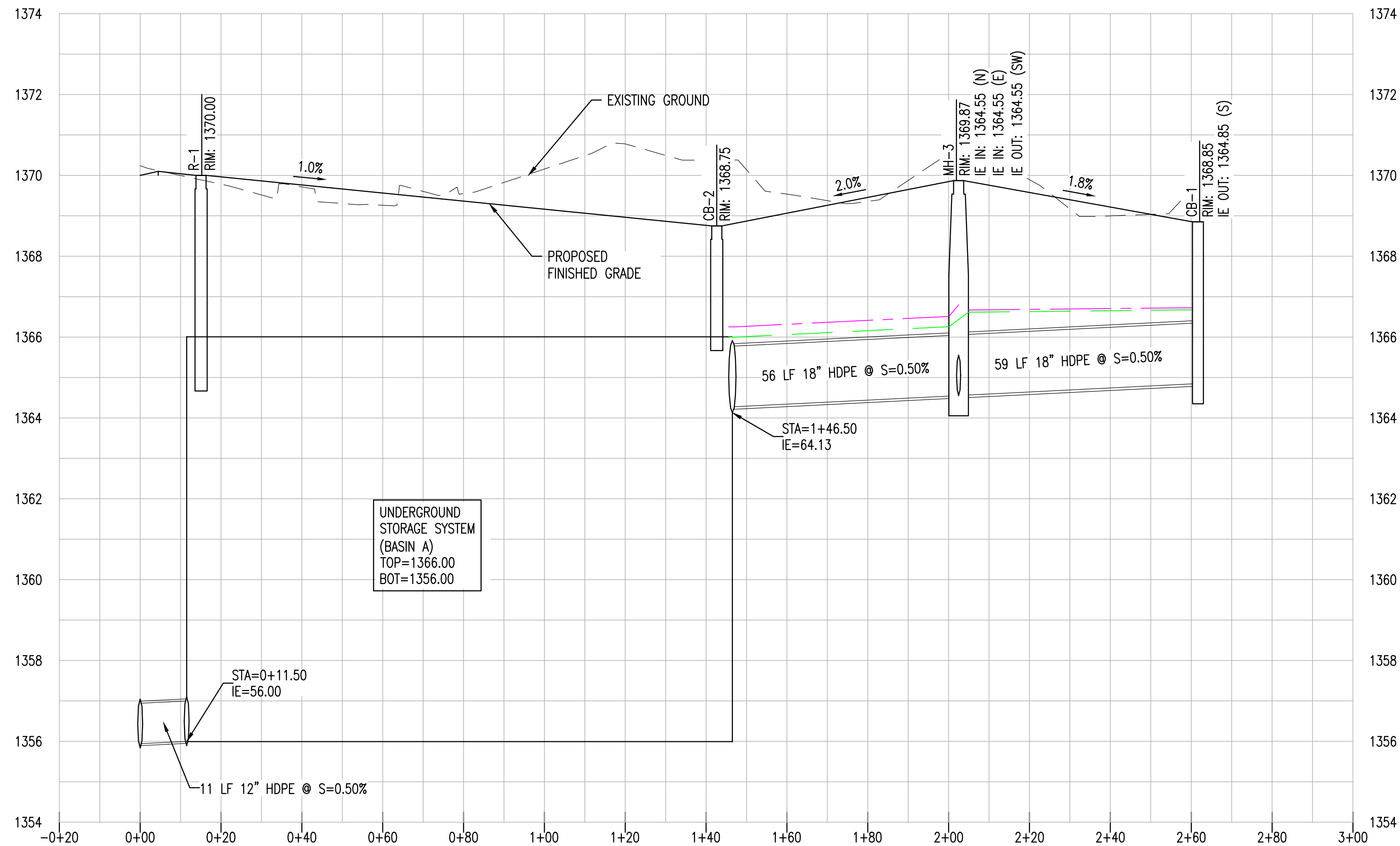
REVISION NO.:		DATE:
1		
2		
3		
4		

JOB NO.: 210414

**PRELIMINARY
GRADING &
DRAINAGE PLAN**

PAGE NO.:	SHEET NO.:
4 OF 5	C3.13

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CB-1 TO BASIN A

HORIZONTAL SCALE: 1" = 20'
VERTICAL SCALE: 1" = 2'

----- ENERGY GRADE LINE
----- HYDRAULIC GRADE LINE

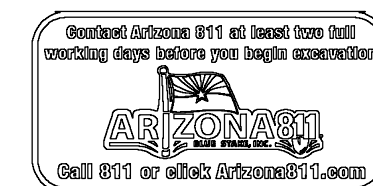
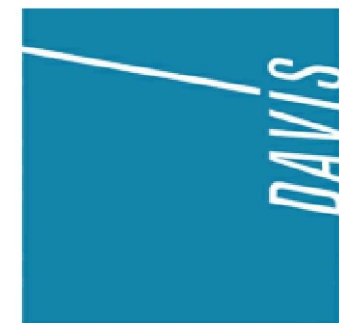
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5240 N. 16TH STREET SUITE 105, PHOENIX, ARIZONA 85016
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PROJECT
MERCADO VILLAGE

LOCATION
92ND & SHEA,
SCOTTSDALE, AZ 85258

DATE: 03/20/2024

ISSUED FOR: ZONING

REVISION NO.:	DATE:
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2	
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JOB NO.: 210414

SHEET TITLE:
**PRELIMINARY GRADING
& DRAINAGE CROSS
SECTIONS**

PAGE NO.:
5 OF 5

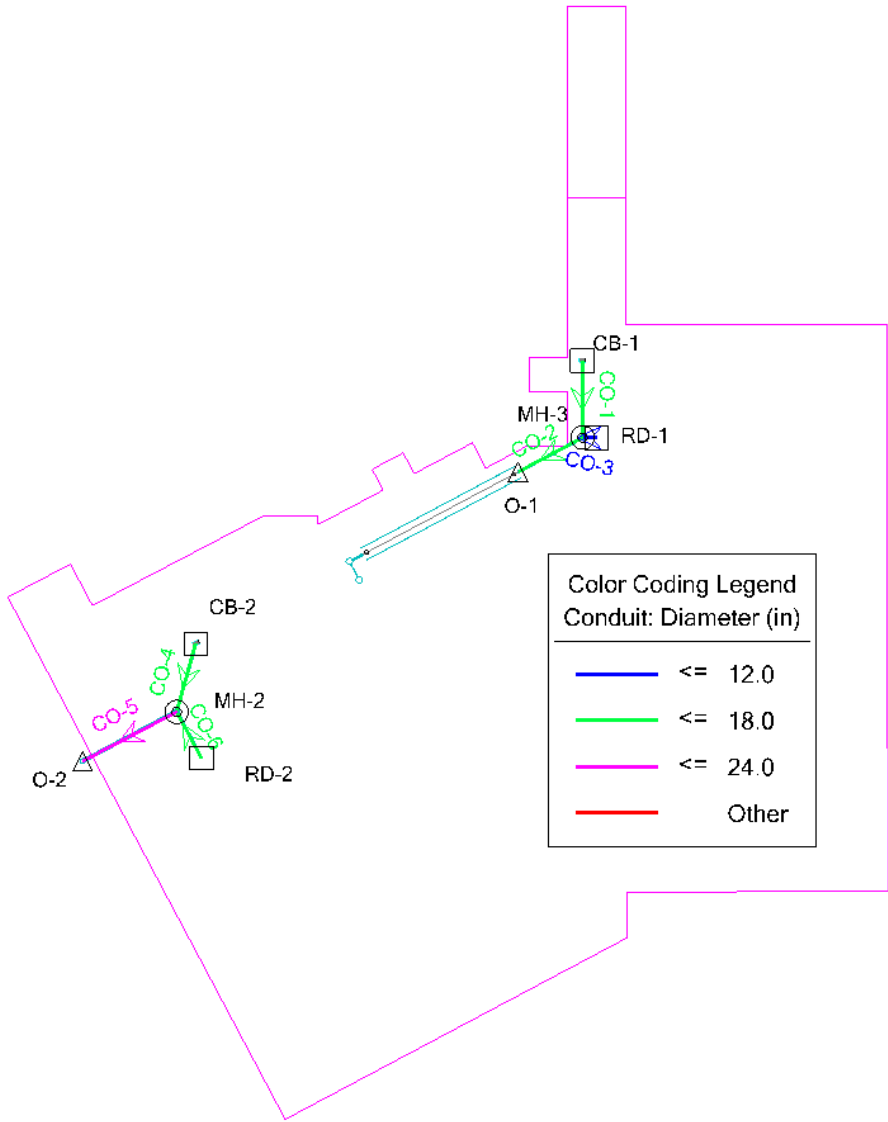
SHEET NO.:
C3.20

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APPENDIX IV

StormCAD Results

Scenario: Q100



FlexTable: Conduit Table

Active Scenario: Q100

Label	Start Node	Invert (Start) (ft)	Stop Node	Invert (Stop) (ft)	Length (Scaled) (ft)	Slope (Calculated) (ft/ft)	Section Type	Diameter (in)	Manning's n	Flow (cfs)	Velocity (ft/s)	Depth (Out) (ft)	Capacity (Full Flow) (cfs)	Flow / Capacity (Design) (%)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
CO-1	CB-1	1,364.85	MH-3	1,364.55	59.0	0.005	Circle	18.0	0.013	3.31	4.09	1.71	7.44	44.5	1,366.32	1,366.26
CO-2	MH-3	1,364.55	O-1	1,364.27	56.2	0.005	Circle	18.0	0.013	7.14	4.04	1.73	7.41	96.3	1,366.26	1,366.00
CO-3	RD-1	1,364.66	MH-3	1,364.55	10.7	0.010	Circle	12.0	0.013	3.83	4.88	1.71	3.57	107.1	1,366.38	1,366.26
CO-4	CB-2	1,364.02	MH-2	1,363.21	54.4	0.015	Circle	18.0	0.013	4.01	2.27	2.40	12.83	31.2	1,365.69	1,365.61
CO-5	MH-2	1,363.21	O-2	1,361.98	81.5	0.015	Circle	24.0	0.013	21.42	6.82	2.90	27.78	77.1	1,365.61	1,364.88
CO-6	RD-2	1,364.83	MH-2	1,363.21	40.1	0.040	Circle	18.0	0.013	17.41	9.85	2.40	21.11	82.5	1,366.71	1,365.61

FlexTable: Catch Basin Table

Active Scenario: Q100

Label	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Elevation (Invert) (ft)	Inlet Type	Capture Efficiency (Calculated) (%)	Flow (Captured) (cfs)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
CB-1	1,368.85	1,368.85	1,364.85	Full Capture	100.0	3.31	1,366.32	1,366.32
CB-2	1,369.90	1,369.90	1,364.02	Full Capture	100.0	4.01	1,365.69	1,365.69
RD-1	1,371.50	1,371.50	1,364.66	Full Capture	100.0	3.83	1,366.38	1,366.38
RD-2	1,371.25	1,371.25	1,364.83	Full Capture	100.0	17.41	1,366.71	1,366.71

FlexTable: Manhole Table

Active Scenario: Q100

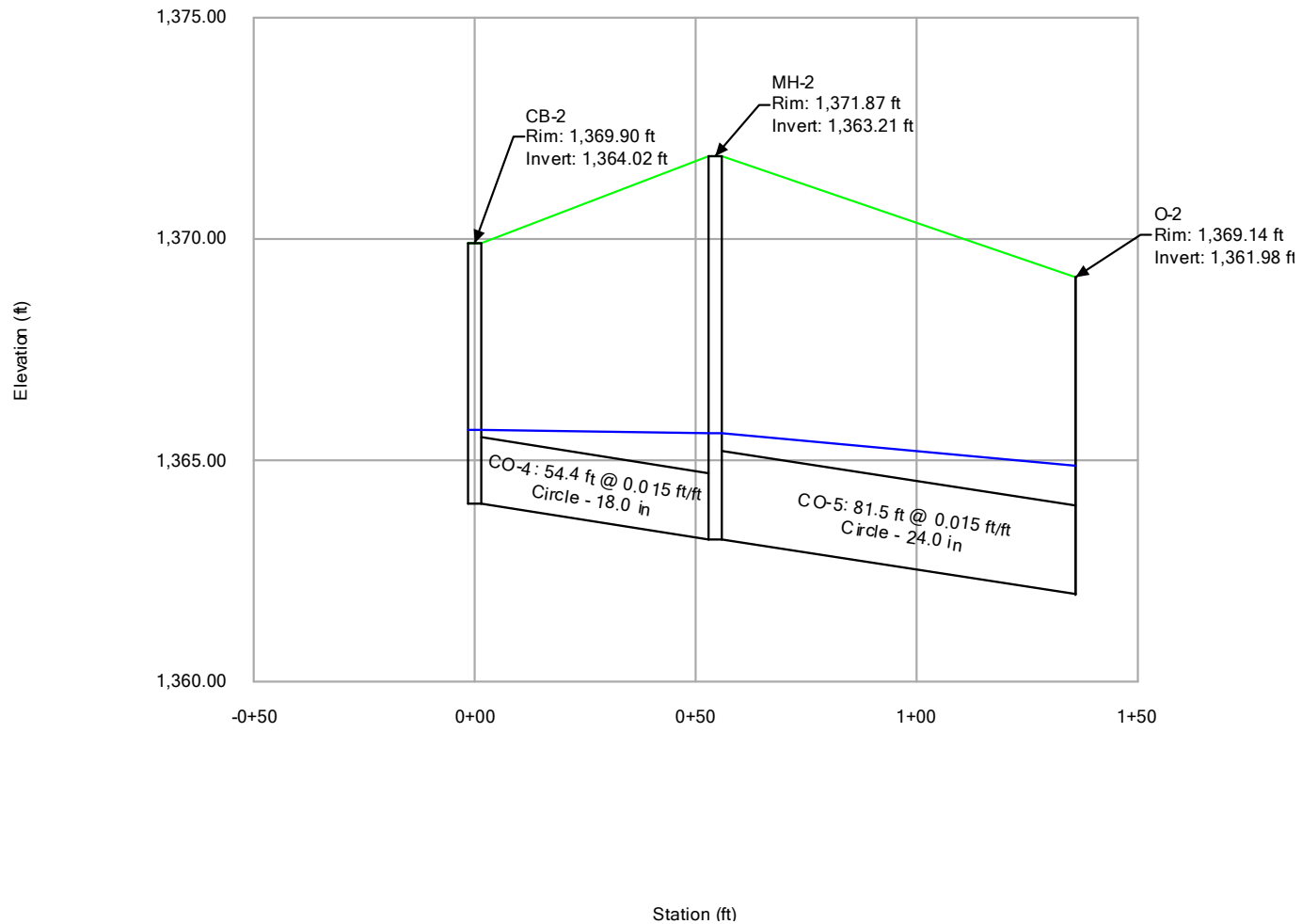
Label	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Elevation (Invert in 1) (ft)	Flow (Total Out) (cfs)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Specific Energy (In) (ft)	Specific Energy (Out) (ft)
MH-2	1,371.87	1,371.87	1,363.21	21.42	2.40	1,365.61	1,365.61	3.91	3.12
MH-3	1,369.87	1,369.87	1,364.55	7.14	1.71	1,366.26	1,366.26	1.76	1.96

FlexTable: Outfall Table

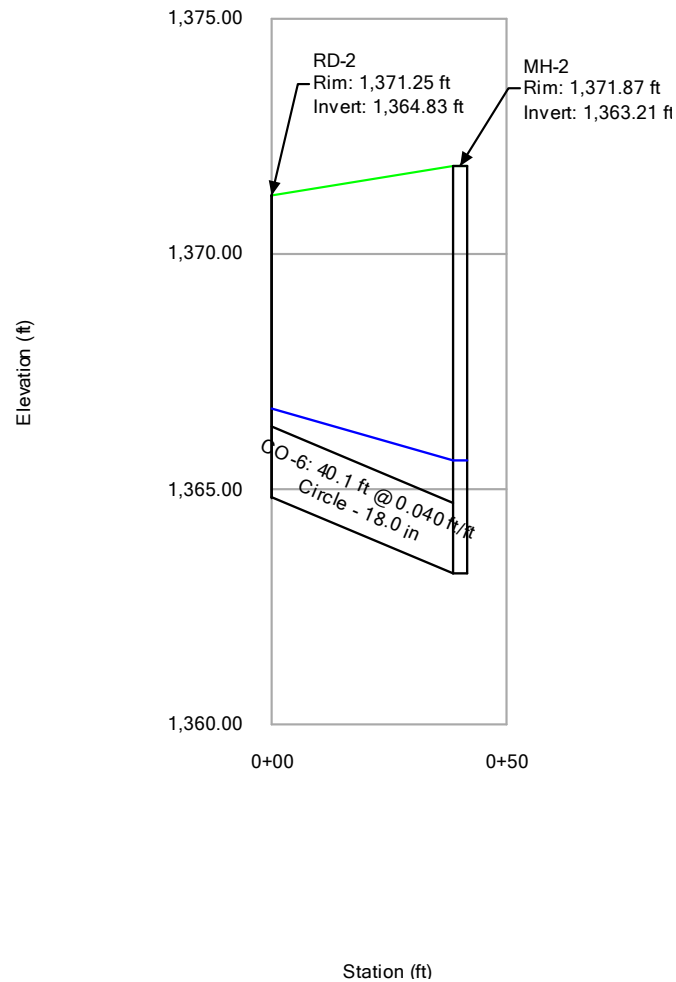
Active Scenario: Q100

Label	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Boundary Condition Type	Elevation (User Defined Tailwater) (ft)	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
O-1	1,368.80	1,364.27	User Defined Tailwater	1,366.00	1,366.00	7.14
O-2	1,369.14	1,361.98	User Defined Tailwater	1,364.88	1,364.88	21.42

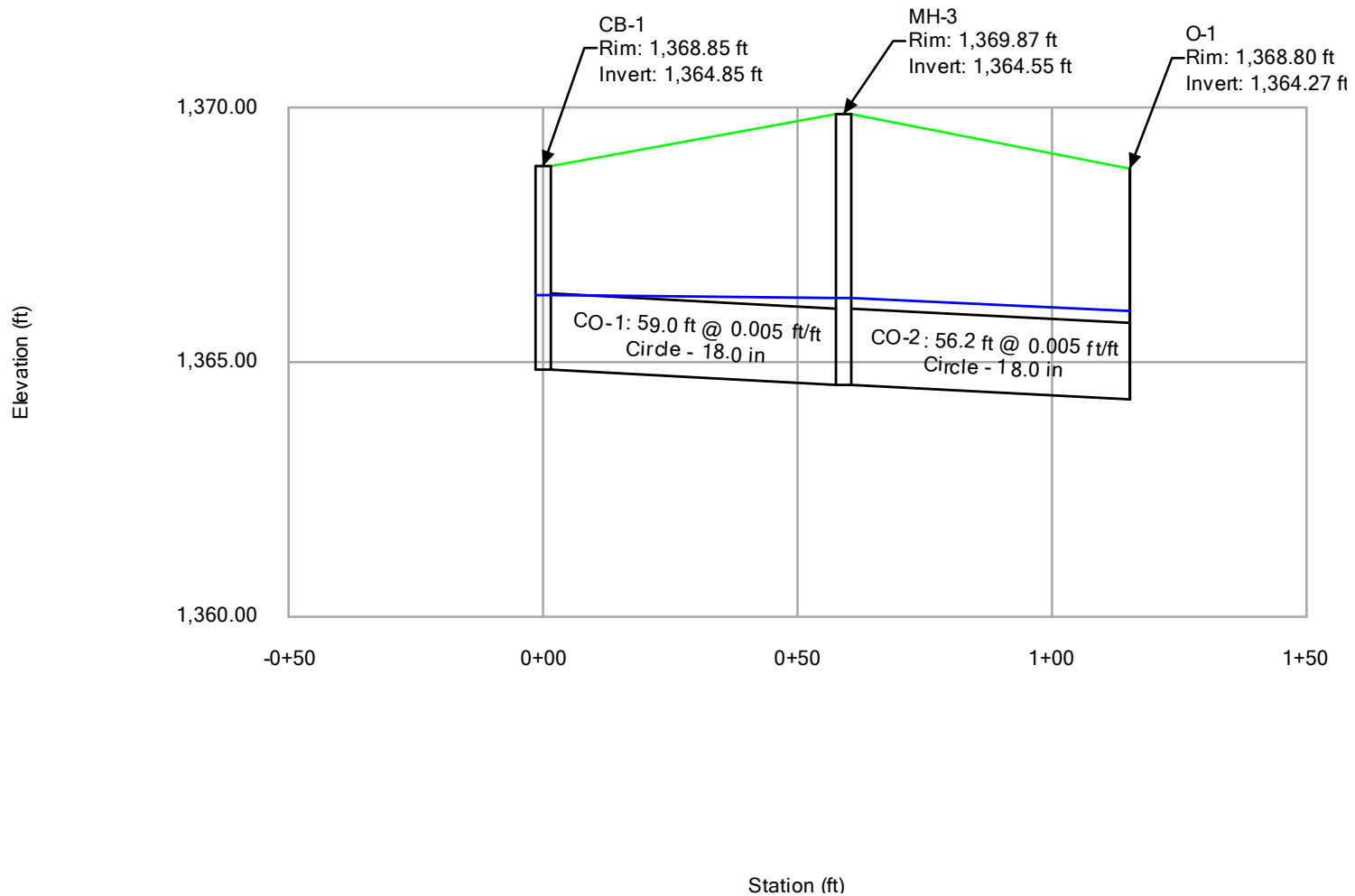
Profile Report **Engineering Profile - CB-2 TO O-2 (Mercado Courtyard StormCAD.stsw)** **Active Scenario: Q100**



Profile Report **Engineering Profile - RD-2 TO MH-2 (Mercado Courtyard StormCAD.stsw)** **Active Scenario: Q100**



Profile Report **Engineering Profile - CB-1 TO BASIN A (Mercado Courtyard StormCAD.stsw)** **Active Scenario: Q100**



Profile Report **Engineering Profile - RD-1 TO MH-3 (Mercado Courtyard StormCAD.stsw)** **Active Scenario: Q100**

