CAVASSON

LOCATED NEAR THE NORTHWEST CORNER OF NORTH HAYDEN ROAD AND LOOP 101
FRONTAGE ROAD INTERSECTION

PRELIMINARY DRAINAGE REPORT- TOLL AT CAVASSON

Basis of Design

October 4, 2024

Project No.: 18114-750

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1. INTRODUCTION

1.1 Project Scope

This report presents the results of a *Preliminary Drainage Study* conducted by Hubbard Engineering at the request of Toll Brothers (“client”), for the Toll at Cavasson development (“site”). The purpose of this report is to provide a hydrologic evaluation for the site as required by the City of Scottsdale per Ordinances 4346 and 4347. This report addresses off-site and on-site conditions. Drainage calculations and methodologies conform to the City of Scottsdale requirements and standards and to Hubbard Engineering’s submitted *Master Drainage Report* for the Cavasson development.

This report is focused on providing practical design information, evaluation, and calculations for statistical flood events up to and including the 100-year frequency flood. The procedures used herein are derived from and performed with currently accepted engineering methodologies and practices. Additionally, the criteria for this evaluation are designed to conform to currently applicable ordinances, regulations and policies affected by the appropriate jurisdictional regulatory authorities for the site.

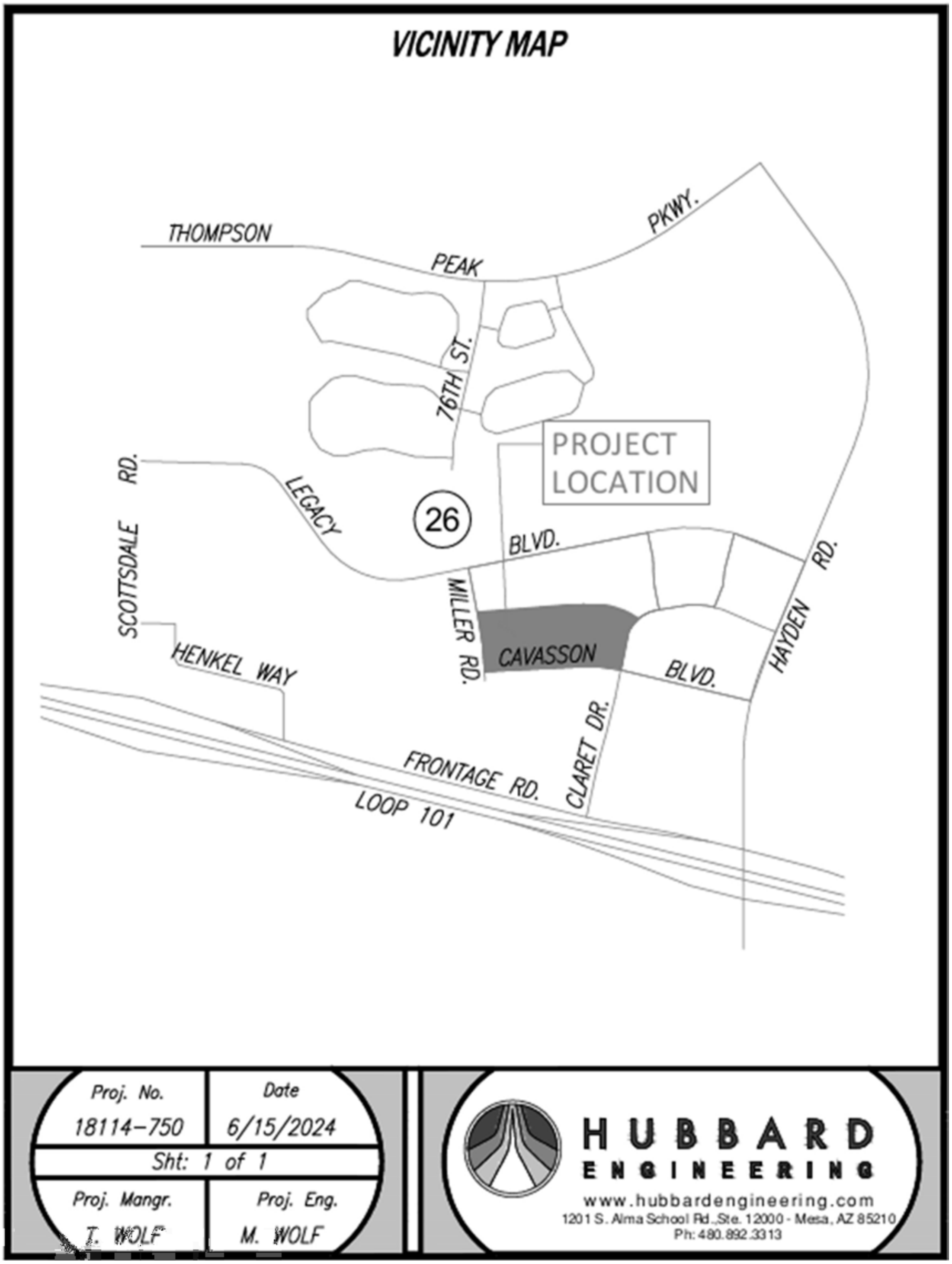
The analysis presented herein focuses on developing design estimates of storm water runoff resulting from a statistical evaluation of storm events of a particular duration and frequency, up to and including a 100-year frequency event. A storm event exceeding the 100-year frequency event may cause or create the risk of greater flood impact than is addressed and presented herein. The scope of this assessment does not include evaluation of storm water runoff resulting from storm events exceeding the 100-year frequency event. Hubbard assumes no responsibility for actual flood damage, increased risks of flood damage, or increased construction or development costs resulting from or related to any such events. Nor shall Hubbard be responsible for any changes in, or additions to, regulatory requirements which may result from, or be related to, any such events or changes in hydrologic or hydraulic conditions within the watershed.

1.2 Site Description

The project site is located in the southeast quarter of Section 26, Township 4N, Range 4E of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. The site is currently undeveloped, and prior to Nationwide Realty Investor’s acquisition, was held in trust by the Arizona State Land Department (ASLD) as a portion of the overall Crossroads East development, which encompasses approximately 883 gross acres. The land naturally falls from northeast to southwest.

Toll at Cavasson is specifically located in the northwest portion of the Cavasson Development and is bounded by Cavasson Boulevard to the south, Claret Drive to the east, Miller Road to the west, and undeveloped desert to the north. The site location is shown in **Figure 1.1 – Vicinity Map**.

Figure 1.1 – Vicinity Map



1.3 Project Type

The Cavasson development is being developed by Nationwide Reality Investors as a master planned mixed use development with office, retail, hotels, and multifamily residential parcels with public and private roadways that run adjacent and through the development.

Toll at Cavasson is proposing a 136-dwelling unit, townhouse style single family development located on Lot 1 of Cavasson Phase 3. Improvements will include private streets throughout the development, multiple amenity spaces throughout the development, as well as proposed public water main and private sewer located within the proposed private streets.

The analyses of pre-development and post-development peak discharges were addressed in Hubbard's *Master Drainage Report Phase 3C update* and will be referenced and further discussed as it applies to Toll at Cavasson.

1.4 Special Conditions- 404 Washes

Located throughout the project site are waters of the United States 404 washes. The handling of the 404 washes has been coordinated with engineering consultants and the City of Scottsdale. The Section 404 Certification form has been completed by consultants. As discussed in the *Master Drainage Report*, Hubbard has met with the Army Corps of Engineer (USACE) and worked with consultants and the Corps to complete a mitigation plan. Approval of the 404 permit was granted on April 2, 2019. That permit number is SPL-2018-00704. ADEQ has issued the 401 permit. This permit has been attached for reference in **Appendix G.**

1.5 Regulatory Criteria

The criterion used in the drainage design and analysis of the site was established using the guidelines as described in the following:

- City of Scottsdale, *Design Standards & Policies Manual*, January 18, 2018.
- City of Scottsdale, *Ordinance No. 4346,* June 17, 2018.
- *Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology*.
- *Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics*.

2. EXISTING DRAINAGE CONDITIONS

2.1 Existing Off-Site Conditions Characteristics

Development in the surrounding area has increased in the last decade and many of the surrounding properties which sit in the Reata Pass basin, part of the Pinnacle Peak South Area, have installed infrastructure to route the offsite flows through their developments. As-built plans for the developments show that channels and culverts have been constructed to divert and route off-site flows. Historical runoff for the areas on and surrounding the project site flow south towards collections points (i.e. existing culverts) where the off-site storm runoff is ultimately conveyed to the TPC golf course just north of the Central Arizona Project canal. Existing culverts located at the edges of Hayden Road are currently utilized to route off-site runoff. Off-site routing for the developed surrounding areas has been verified with their corresponding as-builts, including Hayden Road Improvement Plans, 76th Street & Infrastructure Design, Center Drive (now Legacy Boulevard) Improvements, and One Scottsdale Civic Center Improvement Plans.

Hubbard Engineering completed a *Master Drainage Report* which included an extensive analysis of the off-site flow conditions affecting the entire Crossroads East area, including Cavasson and Hayden Road, for the existing site and proposed development. Results from the study indicate that the storm event affecting the Toll at Cavasson development is a 100-year, 6-hour storm event. Since the submittal of the *Master Drainage Report,* Hubbard has met with the City of Scottsdale and the developers south of the Cavasson development. It was determined in the meeting that the HEC-1 models be adjusted to reflect a time interval of 15 minutes rather than 3 minutes, as was originally modeled in TY Lin's report. The flows affecting the site will be discussed in section 3.1. Details on Hubbard's HEC-1 methodology and analyses, in addition to the modeling results, can be referenced in the approved *Cavasson Master Drainage Report Phase 3C Update* dated August 1, 2023.

The Toll at Cavasson site specifically is being impacted by the undeveloped desert to the north in the existing condition. Prior to construction of the Offsite Storm Drain, there were additionally two culverts impacting the project site. These culverts are conveying flow from basins SB01-B and SB01-C north of Legacy Boulevard. The runoff is now conveyed into underground storage tanks that function as sediment traps for the Cavasson Offsite Storm Drain.

2.2 On-site Drainage

As the Cavasson project site was undeveloped prior to Phase 1, there were no on-site drainage structures. The project site was included in two previous studies, Bob Ward's *Core North/ Core South Drainage Study* and TY Lin's *Pinnacle Peak South Area Drainage Master Study.* The TY Lin report analyzed the Crossroads East area and included FLO2d models combined with HEC-1 analyses on localized basins. The TY Lin report provided a basis on which Hubbard completed its hydrologic studies for the Cavasson project development.

The project site naturally falls from north to south at approximately 1.2% and approximately 0.7% east to west. The site outfall for the Toll at Cavasson development occurs at the southern boundary of the project at the intersection of Cavasson Boulevard and Miller Road at an elevation of 1621.5'.

2.2.1 HEC-1 Analysis

The TY-Lin *Pinnacle Peak South Area Drainage Master Study* report provides an analysis of the entire Pinnacle Peak South Area, which covers an approximate area of 40 square miles. According to the report, the project site is located within the Dobson Wash Watershed, which includes inflow from the southwestern flow split of the Reata Pass Wash. TY Lin's existing and proposed exhibits and results can be found in **Appendices A** and **B**, respectively. Hubbard replicated TY-Lin's model in application to the approximate nine square miles including and surrounding the project site, shown in **Exhibit 1,** and included the inflow hydrographs from TY Lin's Master Drainage study to account for run-off affecting the project site from the 76th Street Channel and the Powerline for both the 100-year, 6-hour and 100-year, 24-hour storm events. Data collected for the model includes the precipitation, soil, and land use for the existing site conditions in application to the delineated sub-basins. All data Hubbard obtained and used in the model was compared to the existing TY Lin model for accuracy and adjusted accordingly based on current site conditions at maximum densities per approved zoning cases. Precipitation data was obtained from the NOAA Atlas 14 precipitation database. The map index used in DDMSW is 64, cells 687-689. Existing soil data was obtained from the United States Department of Agriculture's Natural Resources Conservation Service. Additionally, land use data was determined based on current zoning of the project site. Data for the HEC-1 analysis are in **Appendix C**. The results of the ultimate condition at full build out (after the powerline channel is constructed) are included in **Appendix D**.

Hubbard has met with the City of Scottsdale and the developers south of the Cavasson development in order to coordinate the analyses for the Crossroads East development. It was determined in the meeting that the HEC-1 models be adjusted to reflect a time interval of 15 minutes rather than 3 minutes, as was originally modeled in TY Lin's report. Thus, the inflow hydrographs provided by TY Lin were convoluted to accurately represent this change in time interval resulting in more accurate time to peak values. In addition, it was also decided in the meeting that Hubbard's analysis for the existing, proposed, and ultimate conditions will be used by developers south of Hayden Road. Hubbard's sub-basin delineation, shown in **Exhibit 1**, deviates from TY Lin's analysis in that Hubbard added collection points at existing culverts along Legacy Boulevard and Hayden road to model the peak flows coming through each infrastructure for utilization in routing design. Additionally, detailed analysis of the existing topography and as-builts along Hayden road and Legacy Boulevard revealed that the basins were not segmented by Hayden Road or Legacy Boulevard, in the TY Lin Report. These larger basins were subdivided in the Hubbard analysis and a similar naming convention was used to compare baseline flows.

Hubbard's analysis split the original TY Lin sub-basins 7 and 8 into east and west components to more accurately model the conveyance of the flows on either side of Hayden Road. An additional basin, SB09N was also added per the city's request as this was not included in the TY Lin analysis. In Hubbard's routing, flows from SB07E and SB08E are routed east, following the topography. Since the TY-Lin model does not delineate a SB-08W, the TY-Lin model shows the flow from SB08 flowing from the southwest corner of the basin east across Hayden Road. This analysis is not correct considering that the eastern elevation is approximately 20 feet higher than the western elevation (CP-08 in TY-Lin's exhibit). This routing also neglects the flow being conveyed across the Loop 101 from SB07W and SB08W. Hubbard therefore routed the flow from SB07W and SB08W to the existing ADOT culvert along Frontage road and across the Loop 101, thus affecting the developments south of the Loop 101, a condition not represented in the TY-Lin report.

In TY-Lin's analysis, the 100-year, 24-hour storm event controlled for the entire area, with the 100-year, 6-hour storm event controlling the localized flows. TY-Lin recommended the creation of the Powerline Channel to intercept and route the 100-year, 24-hour flows to Basin 53R. Since it was determined in meetings with the City of Scottsdale that the channel will be built during construction of the Cavasson development, Hubbard's existing model includes this channel and results for the 100-year, 24-hour, 100-year, 6-hour, and 10-year, 6-hour storm events are shown in **Appendix D.** The Existing 100-year, 6-hour flows prior to the offsite channel being constructed were 196 cfs entering the site and an Existing condition 100-Year, 6-hour discharge exiting the site at the Loop 101 of 740 cfs. With the addition of the channel and routing to Basin 53R, a significant amount of flow is being diverted from the affected study area to Basin 53R. The addition of the channel results in higher peak flows for the Crossroads East development from the 100-year, 6-hour storm event rather than the 100-year, 24-hour storm.

In addition to the existing model post-channel and proposed model, Hubbard completed an ultimate condition model considering the surrounding lots will be developed. In the model, this was represented by increasing the imperviousness of SB01-B, SB01-C, and SB07W to reflect maximum assumed densities per the zoning stipulations pertinent to the Cavasson development and Cross Roads East approved zoning cases. To model the effect of stormwater retention on the basin, the required first flush retention was diverted out of the model for each of the onsite subbasins as well as subbasins SB01-B, SB01-C, and SB07W since these three basins will directly impact the Cavasson development. While diverting out the first flush retention does not do anything to the timing of the storm event or reduce the peak discharges in HEC-1, it does properly remove the volume of water from the overall system to help verify the water balance of the watershed.

As part of the Phase 3 Update for Cavasson Phase 3C, subbasin SB02E has been split into SB02E1 and SB02E2 to more accurately model discharge across State Route 101. SB02E2 is the west split while SB02E1 is the east split. CP08W is now routed across State Route 101 to CP-4N where it combines with the discharge from SB02E1. The routing for R8W-4N is stage storage discharge to more accurately model the detention basin that was constructed adjacent to the ADOT culverts as part of Phase 1 and the Claret Monument Sign development.

2.3 Flood Zone Information

The Maricopa County, Arizona and Incorporated Areas Flood Insurance Rate Map (F.I.R.M.) number 04013C1320L, Panel number 1320 of 4425, dated July 21, 2021 indicates that the Toll at Cavasson development falls within Zone AO (Depth 1 foot) on the west side of the project with the remainder classified as Zone X.

Zone AO is defined as:

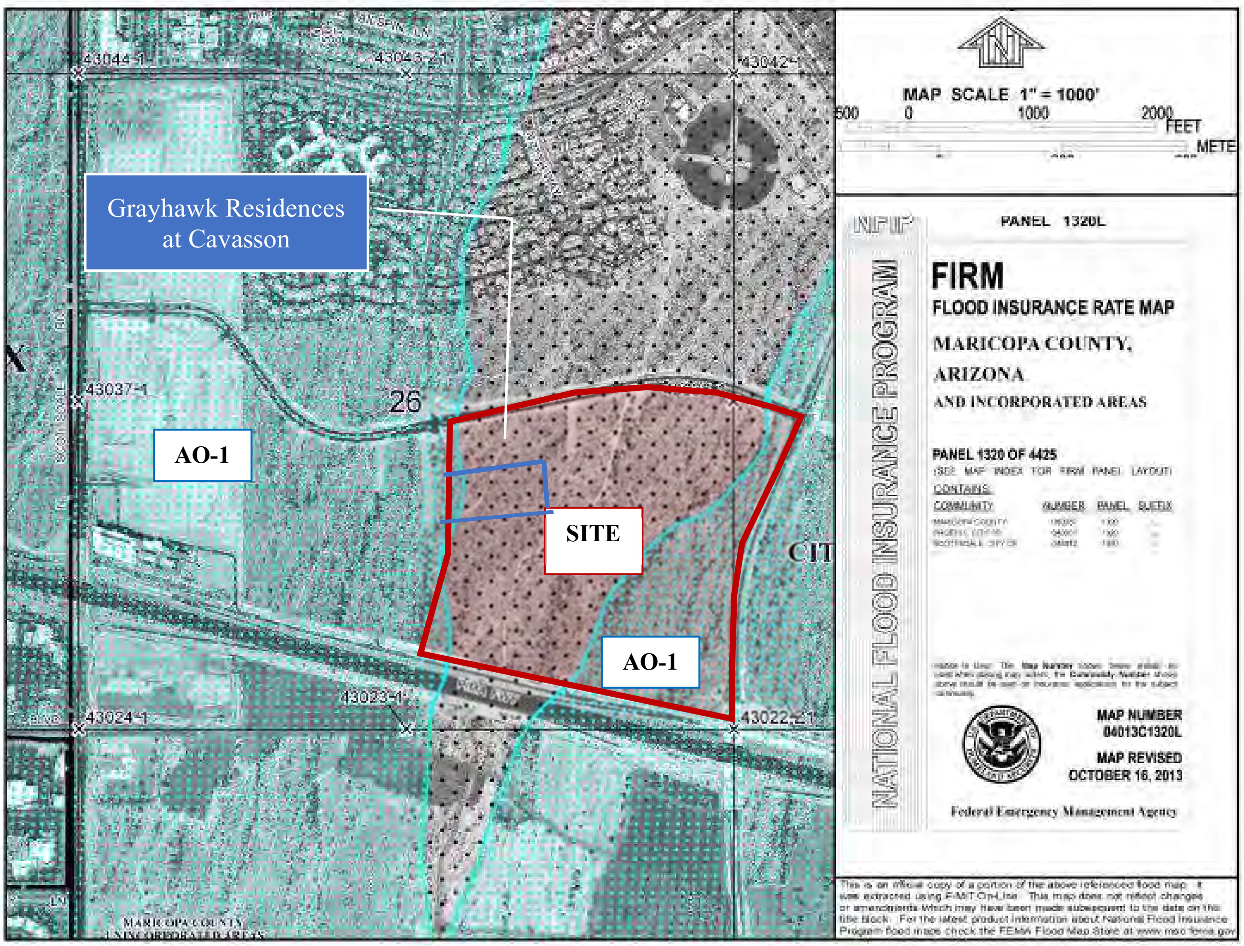
"*Areas subject to inundation by 1-percent-annual-chance shallow flooding (usually sheet flow on sloping terrain) where average depths are between one and three feet. Average flood depths derived from detailed hydraulic analyses are shown in this zone. Mandatory flood insurance purchase requirements and floodplain management standards apply.*"

Zone X is defined as:

"Areas of 0.2% annual chance flood; areas of 1.0% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood."

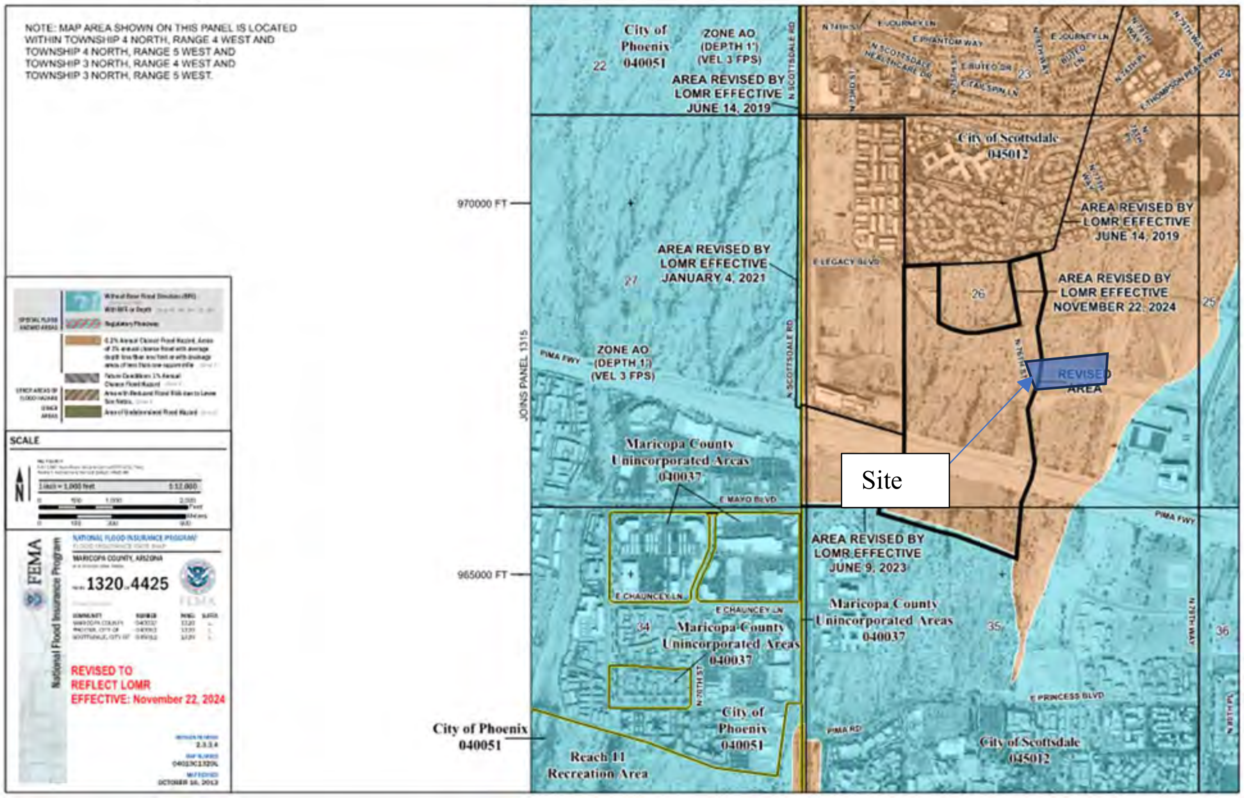
Refer to FEMA Firmette in **Figure 2.1.**

Figure 2.1 – FIRM 04013C1320L



It should be noted that FEMA has approved a LOMR that removes the Zone AO area from the west portion of the property. This LOMR will become effective on November 22, 2024 when the comment period has ended. This revised FIRM is shown below in Figure 2.2.

Figure 2.2 – FIRM 04013C1320L Effective November 22, 2024



3. PROPOSED DRAINAGE PLAN

3.1 Off-Site Flows

Off-site flows for the site were analyzed and addressed in Hubbard's *Master Drainage Report Phase 3C Update.* Results from the analysis show that after the powerline Channel is constructed 19 cfs is currently being conveyed across Legacy from the existing elliptical culvert located north of the Toll at Cavasson development within Legacy Boulevard. When the parcel north of Legacy is developed, these flows are anticipated to increase to 50 cfs. These flows are now, however, being captured in the Offsite Storm Drain Network (Plan Check #1838-21) designed by Hubbard Engineering, construction concluded in December of 2023, that routes all of these offsite flows around the outside perimeter of the site prior to crossing Cavasson Boulevard. This allows the offsite flows to maintain their existing flow patterns without impacting the Toll at Cavasson development. In the event that the Offsite Storm Drain System infrastructure fails, the offsite flow from north of Legacy would be contained within an existing swale to the north of Legacy Boulevard before entering an existing 10'x3' curved box culvert that discharges the flow to the south of Legacy Boulevard and west of Miller Road. This flow would then continue along the west side of Miller Road before joining its historic flow pattern just south of Cavasson Boulevard. It should be noted that due to sedimentation within the existing culverts crossing Legacy Boulevard, this overflow route is already being used in this manner for some of the flow. This Offsite Storm Drain System was discussed in further detail within Hubbard's *Master Drainage Report Phase 3C Update.*

The Toll at Cavasson development is additionally impacted by offsite runoff from the undeveloped desert immediately north of the project site. This runoff generally enters the Toll at Cavasson development site through 5 minor washes crossing the north property line in the existing condition where it flows to the existing sediment basin along the south property line adjacent to Cavasson Boulevard. It then exits the site through the existing 42-inch culvert that crosses under Cavasson Boulevard before resuming its historical flow pattern just south of Cavasson Boulevard. This offsite flow will be blocked by site walls proposed to be constructed as part of the Toll at Cavasson development. As a result, a swale will be graded within a proposed drainage easement on the lot north of Toll at Cavasson, currently being negotiated with the property to the north, which will reroute the runoff to flow west along the north property line towards Miller Road. This swale will terminate in a small sedimentation basin once it reaches the proposed drive entrance. At this point, a negligible portion will be captured in the onsite drainage system within the drive entrance, an additional portion will be captured by the Cavasson Offsite Storm Drain by a proposed temporary inlet within the sedimentation basin, and the remainder will discharge to Miller Road. The runoff will then resume its existing flow pattern after crossing Cavasson Boulevard. The hydraulic calculation for this swale is included in Appendix F.

A portion of the runoff from the undeveloped desert to the north enters the site as concentrated flow on the northeast corner of the site as documented in the Phase 3 Roads Drainage Report written by Hubbard Engineering as DA-OFFA. This runoff is then routed through a small channel to the existing sediment basin adjacent to Cavasson Boulevard. Once the site is developed, this runoff will enter the Toll at Cavasson developments proposed storm drain system and in small storm events will be captured in the proposed retention tanks. In the event of back-to-back storm events or events that exceed the first flush, this runoff will be surcharged through the tank rims to the site ultimate outfall and cross Cavasson Boulevard as sheet flow to resume its historic flow pattern. Once the site to the north is developed, that site will retain its first flush runoff volume and will route the excess flows to the provided emergency overflow stub within the Cavasson Offsite Storm Drain.

The offsite flow from the adjacent Claret Drive will be conveyed to the site through an existing catch basin that will be tied directly into the onsite storm drain system. This offsite flow was calculated as part of the *Phase 3 Roads Drainage Report* written by Hubbard Engineering. The offsite runoff from the adjacent Cavasson Boulevard will be partially conveyed to the site through existing catch basin CB-07 that will be tied directly into the onsite storm drain system. The offsite flow from Cavasson CB-08 however will be rerouted to directly discharge to the existing culvert through a proposed storm drain manhole that will be constructed when the existing inlet headwall is removed. This offsite flow was calculated as part of the *Final Drainage Report – Cavasson Boulevard* written by Hubbard Engineering. Offsite flows from the adjacent half street of Miller Road will not impact the site because Miller Road was designed to convey all of its flow to two curb openings south of Cavasson Boulevard. The Toll at Cavasson development will still however retain the appropriate volume for the 100 year 2-hour storm event for the adjacent half street to remove those flows from the system. A summary of the offsite flows impacting the Toll at Cavasson development can be found in **Table 1** below. See **Exhibit 2** for the offsite drainage areas impacting the site and **Exhibit 5** for the emergency overflow locations and routes for the site.

Table 1: Offsite Flow Summary Table		
Contributing Area	Pre-Development Q100 (cfs)	Post Development Q100 (cfs)
SB01-B	19	0
SB01-C	19	0
DA-OFF-1	9.36	0
DA-OFF-2	7.22	0
DA-OFF-3	7.05	0
DA-OFF-4	8.52	0
DA-OFF-5	7.07	0
Claret Drive DA-OFFA	3.38	3.38
Claret Drive CB-A	3.05	3.05
Cavasson CB-08	2.70	2.7
Cavasson CB-07	3.55	3.55

3.2 On-Site Drainage Configuration

The storm event affecting the site was determined to be a 100-year, 6-hour storm, as discussed in Hubbard's *Master Drainage Report Phase 3 Update.* The Toll at Cavasson development will include 135 single family townhouses with private streets and amenity areas throughout the site. Additionally, the Offsite Storm Drain System runs along the west boundaries of the site within an existing drainage easement.

See **Exhibit 2** for the drainage map. The majority of the site is designed with inverted crowns and valley gutters that serve as the primary method of conveyance. These valley gutters generally flow south and west to grated inlets located on the proposed underground retention system or on the proposed storm drain system in order to follow the natural topography of the site. The proposed gutters for the building roofs will discharge in various locations around the proposed buildings with the majority discharging to either the side or front of the units. Isolated roofs will discharge to backyard areas but these roofs have been minimized as much as possible. Every roof drain downspout discharges to either a proposed concrete/paver walk or to a riprap splash block that directs the discharge to the proposed valley gutters. The backyards are proposed to drain via v-ditch swales directing runoff south and west, crossing between backyards through turned block wall openings. Secondary turned block openings will be provided wherever feasible in the backyards and side yard walls to minimize the risk of ponding in the backyards. There will additionally be blanket drainage easements covering the site as part of the CC&Rs of the association to allow for lot to lot drainage and to stipulate that drainage patterns in the backyards must be maintained at all times. The onsite storm drain system consists of six equalized underground storage tanks located on the southwest corner of the site and extending along the south most private street. These underground retention tanks have flow by grated inlets located periodically within the valley gutter in order to maximize the capture of the storm runoff while minimizing ponding throughout the site. From the underground retention tanks, there are three separate storm drain runs that capture runoff through the remainder of the site.

The first storm drain run will extend north from the north side of UGST-3 for approximately 200 linear feet before reaching a proposed catch basin with a 2’x3’ steel bar grate located in a sump condition. This inlet will pond for a maximum 0.45 feet before breaking over to the south. From this catch basin, the storm drain will continue north for approximately 37 linear feet before reaching a maintenance catch basin to allow the storm drain to realign roughly northwest. The storm drain will then continue northwest for approximately 16 linear feet before reaching a proposed combination curb inlet catch basin. This inlet will pond for a maximum of 0.50 feet before breaking over to the south. From this catch basin the storm drain will turn west and run for approximately 87 linear feet to the termination of this storm drain run in a proposed combination curb inlet catch basin. This inlet will pond for a maximum of 0.30 feet before breaking over to the east to the adjacent combination curb inlet, and 0.40 feet total before breaking over to the south.

The second storm drain run will extend south from the south side of UGST-6 for approximately 61 linear feet before reaching a proposed combination curb inlet catch basin. This inlet will pond for a maximum of 0.5 feet before breaking over the adjacent curb to the south. From this catch basin, the storm drain will continue south for approximately 10 linear feet before reaching a maintenance catch basin to allow the storm drain to realign to head west. The storm drain will then head west for approximately 28 linear feet to a maintenance catch basin where the runoff from Cavasson Boulevard CB-07 will enter the proposed storm drain system. From this catch basin, the storm drain will then run for approximately 243 linear feet to a proposed catch basin with an open grate. This catch basin will include a connection to the existing 42-inch culvert crossing under Cavasson Boulevard and this culvert will serve as a primary emergency overflow for the proposed storm drain system. This storm drain will then run for an additional approximately 88 linear feet to the termination of this storm drain run in a proposed maintenance catch basin. This final maintenance catch basin is where the runoff from Cavasson Boulevard CB-08 will enter the proposed storm drain system.

The third and final storm drain run for the Toll at Cavasson development will extend north from the north side of UGST-6 for approximately 119 linear feet before reaching a proposed maintenance catch basin. From this catch basin, the storm drain will turn and run east for approximately 155 linear feet to a proposed catch basin with a 2’x3’ steel bar grate in a flow-by condition. From this catch basin, the storm drain will turn and head south for approximately 124 linear feet before reaching a proposed catch basin with a 2’x3’ steel bar grate in a flow-by condition. This catch basin will accept flow from two different storm drain runs. The first will be a lateral that is proposed to run south from this proposed catch basin for approximately 70 linear feet before reaching a proposed combination curb inlet catch basin. This inlet will pond for a maximum of 0.5 feet before breaking over the adjacent curb to the south. The second storm drain run will run east for approximately 86 linear feet to a proposed catch basin with a 2'x3' steel bar grate assembly in a flow-by condition. From this catch basin, the storm drain will head northeast for approximately 214 linear feet to a proposed catch basin with a 2'x3' steel bar grate assembly in a flow-by condition. This catch basin will also accept flow from two different storm drain runs. The first will be a lateral extension on the northeast alignment for approximately 18 linear feet to a proposed catch basin with a 2'x3' steel bar grate assembly in a flow-by condition. The second will extend southeast to a proposed maintenance catch basin approximately 25 linear feet away. The storm drain will then slightly change direction to east south east and run for approximately 78 linear feet to an additional proposed maintenance catch basin. The storm drain will then head north for approximately 19 linear feet to a proposed maintenance catch basin where the runoff from Claret Drive CB-A will enter the proposed storm drain system. The storm drain will then run north for approximately 180 linear feet with one additional maintenance catch basin along that length to allow for a slight realignment in the storm drain to follow the curvature of Claret Drive to a proposed maintenance catch basin. From this maintenance catch basin, the storm drain will run for approximately 20 linear feet to a proposed combination curb inlet catch basin. This inlet will pond for a maximum of 0.3 feet before breaking over to the east into Claret Drive. The storm drain will then run for approximately 48 linear feet to terminate in a proposed combination curb inlet catch basin. This inlet will pond for a maximum of 0.3 feet before breaking over to the east into Claret Drive.

Due to the preliminary nature of this report, no hydraulic analysis has been performed on the proposed storm drain systems, nor has sizing been conducted at this time. When the hydraulic analysis is performed, the tailwater starting HGL will be set at the top of the highest equalized tank. Head loss for the pipes will be calculated using Manning's Equation and head loss through the structures will be calculated the equation $k(\frac{v^2}{2g})$ where k is the junction loss coefficient, v is the velocity exiting the junction and g is the acceleration of gravity. See **Appendix E** for hydrologic calculations. The surface overflow location for the proposed equalized underground retention tanks will be the grated access ports for UGST-1, which will be designed to pond for less than 0.5 feet prior to breaking over to the south towards Cavasson Boulevard and the site ultimate surface overflow location. The maximum ponding depth for any inlet has been designed at 0.50 feet and is shown for all sump condition inlets in **Exhibit 4** along with breakover location and elevation. Inlet capacity calculations can be found in **Appendix F** for significant sump and flow-by condition catch basins. Inlet capacity has not been checked for flow-by inlets on the tanks at this time. Street Capacity calculations can also be found in Appendix F to demonstrate that adequate street capacity exists for runoff that is not captured in the flow-by inlets without inundating any of the building garages.

3.3 Maintenance Responsibility

The project development will be responsible for maintenance of all onsite storm drainage systems within the property. Since all onsite storm drainage structures are located within common areas, the subdivision association will be responsible for this maintenance. The City of Scottsdale will be responsible for maintenance of drainage structures within its right-of-way. At the two locations where the offsite, publicly owned storm drains within the right of way, connect to the onsite, privately owned and maintained storm drain system, a catch basin is proposed immediately adjacent to the PUE that will be maintained by the subdivision association and marks the end of the City of Scottsdale's maintenance responsibility.

3.4 Storm Water Storage

The project development is required to provide retention for the first flush storm event for onsite and the 100 year 2-hour storm event for the adjacent half streets. The required first flush volume determined for each drainage area site will be designed per City of Scottsdale *Drainage Policies and Standards for Maricopa County, Arizona* (Reference 1). The first flush retention required for the site will be 18,899 cubic feet. The required retention for the adjacent half streets for the site is 18,166 cubic feet. This results in a total required retention of 37,064 cubic feet which will be retained in 475 lf of 10' diameter underground retention tanks. These tanks provide a total of 37,306 cubic feet of retention which meets the requirement. The required and provided retention calculations can be found in **Appendix G**.

3.4.1 Time to Drain

Each site will be required to dewater the required storm water retention through natural percolation and/or drywells. The City of Scottsdale requires all retention facilities be completely drained within a 36-hour time frame. For design purposes, it was assumed that each drywell will have a percolation rate of 0.1 cfs. During construction, the first drywell constructed will be tested to verify this percolation rate. In the event that the tested percolation rate plus a clogging factor exceed 0.1 cfs, the number of drywells may be reduced while ensuring all retention facilities fully drain within 36 hours. Using this drywell discharge rate, it was determined that 3 separate drywells will be provided to drain the total volume of the underground retention tanks. This will allow the retention tanks to fully dewater in a total time of 34.5 hours. See **Appendix G** for drywell calculations. It should be noted that a static head percolation test was performed on the existing drywell located on the west property line of the site, constructed as part of the Cavasson Offsite Storm Drain, which found a raw percolation rate of 0.5 cfs so it is expected that the proposed drywells will perform similarly which will allow for the elimination of at least one drywell once derating factors are applied.

4. SUMMARY AND CONCLUSION

- The site is located in Section 26 of Township 4N, Range 4E of the Gila and Salt River Base and Meridian, Maricopa County, Arizona.
- The site is partially located in the Flood Plain Zone AO-1 on the west side and in Flood Plain Zone X on the remainder based on the current effective FIRM Map.
- There is an approved LOMR that will become effective on November 22, 2024 that will remove the Zone AO-1 Floodplain from the property.
- The Finish Floor Elevations, Garage Lips, and Electromechanical equipment of the proposed buildings meet the requirement of 2 feet higher than the natural grade (HAG) within the Zone AO-1 flood zone.
- The storm event affecting the site was modeled as a 100-yr, 6-hour event in Hubbard's *Master Drainage Report* due to being more conservative.
- The Offsite Storm Drain System mitigates the offsite flows from north of Legacy Boulevard. The sites onsite storm drain system is directly accepting the offsite flows from Claret Drive and Cavasson Boulevard.
- The site will be required to provide retention for the first flush storm event for onsite and provide retention for the 100-year 2-hour storm event for the adjacent half street roads.
- 475 lf of 10' diameter underground storage tanks will provide 37,306 cf of retention, for the required retention of 37,064 cf.

5. REFERENCES

1) City of Scottsdale. *Drainage Policies & Standards for Maricopa County, Arizona.* January 18, 2018.

2) Flood Control District of Maricopa County. *Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology*. November 2003.

3) Flood Control District of Maricopa County. *Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics*. November 2003.

4) Flood Control District of Maricopa County. *Drainage Design Manual for Maricopa County, Arizona, Volume III, Erosion.* July 2018.

5) City of Scottsdale. *Ordinance No. 4346.* June 17, 2018.

6) TY Lin, *Pinnacle Peak South Area Drainage Master Study,* 2014.

7) City of Scottsdale, *Ordinance No. 4346,* June 17, 2018.

8) Hubbard Engineering, *Master Drainage Report*. December 23, 2018

9) Hubbard Engineering, *Master Drainage Report Phase 3 Update,* August 19, 2021

10) Hubbard Engineering, *Master Drainage Report Phase 3C Update,* August 1, 2023

11) Hubbard Engineering, *Final Drainage Report – Cavasson Boulevard,* August 6, 2019

12) Hubbard Engineering, *Phase 3A Roadways Final Drainage Report,* March 9, 2022

6. LIMITATIONS

This report is focused on providing practical design information, evaluation, and calculations for statistical flood events up to and including the 100-year frequency flood. The procedures used herein are derived from, and performed with, currently accepted engineering methodologies and practices. Additionally, the criteria for this evaluation is designed to conform to currently applicable ordinances, regulations and policies effected by the appropriate jurisdictional regulatory authorities for the site.

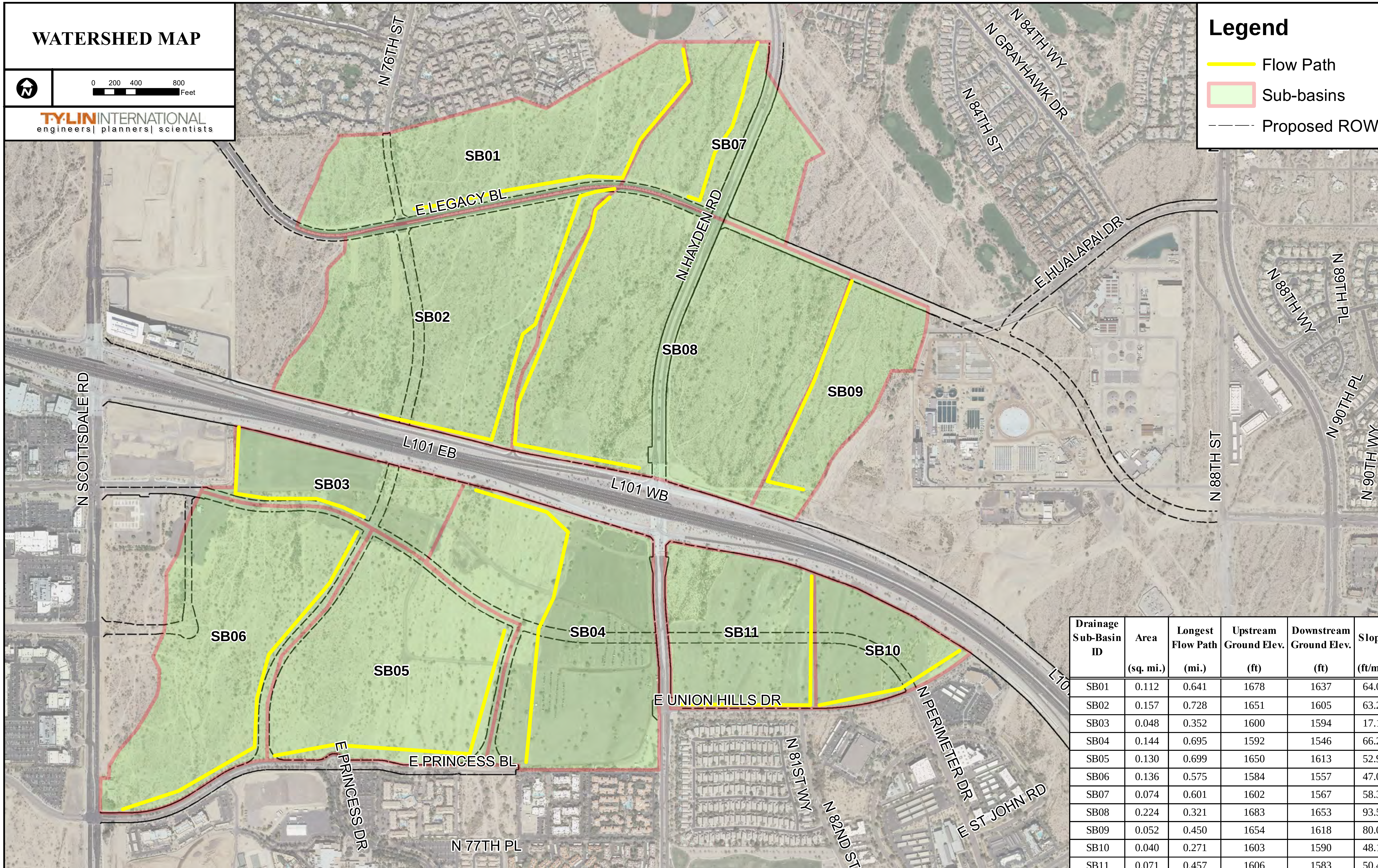
The analysis presented herein focuses on developing design estimates of storm water runoff resulting from a statistical evaluation of storm events of particular duration and frequency up to and including a 100-year frequency event. A storm event exceeding the 100-year frequency event may cause or create the risk of greater flood impact than is addressed and presented herein. However, the scope of this assessment does not include evaluation of storm water runoff resulting from storm events exceeding the 100-year frequency event. Hubbard Engineering assumes no responsibility for actual flood damage, increased risks of flood damage, or increased construction or development cost resulting from or related to any such events.

Nor shall Hubbard Engineering be responsible for any changes in, or additions to, regulatory requirements which may result from, or be related to, any such events or changes in hydrologic or hydraulic conditions within the watershed.

In performing the services contained herein, Hubbard Engineering has received or will receive information prepared or compiled by others. Hubbard Engineering, as engineering professionals, are not required to verify the information, but may rely on the information unless actual knowledge concerning the validity of the information is known or is obvious to the professional. Therefore, Hubbard Engineering is entitled to rely upon the accuracy and completeness of this information without independent evaluation or verification.

Appendix A
TY Lin HEC-1 Existing
Toll at Cavasson

WATERSHED MAP



Drainage Sub-Basin ID	Area (sq. mi.)	Longest Flow Path (mi.)	Upstream Ground Elev. (ft)	Downstream Ground Elev. (ft)	Slope (ft/mi)
SB01	0.112	0.641	1678	1637	64.0
SB02	0.157	0.728	1651	1605	63.2
SB03	0.048	0.352	1600	1594	17.1
SB04	0.144	0.695	1592	1546	66.2
SB05	0.130	0.699	1650	1613	52.9
SB06	0.136	0.575	1584	1557	47.0
SB07	0.074	0.601	1602	1567	58.3
SB08	0.224	0.321	1683	1653	93.5
SB09	0.052	0.450	1654	1618	80.0
SB10	0.040	0.271	1603	1590	48.1
SB11	0.071	0.457	1606	1583	50.4

Path: Q:\Latest Crossroads East Files from San Diego\GIS\Mathew's Figures for Report\Watershed Map\Watershed Map.mxd

Date Saved: 9/12/2014 10:41:59 AM

Appendix B
TY Lin HEC-1 Proposed
Toll at Cavasson

```
1*****************************************
***************************************
 *                                      *                                                *
  *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                *    U.S. ARMY CORPS OF ENGINEERS
  *
 *              JUN   1998              *                                                *    HYDROLOGIC ENGINEERING CENTER
  *
 *           VERSION 4.1                *                                                *         609 SECOND STREET
  *
 *                                      *                                                *       DAVIS, CALIFORNIA 95616
  *
 *  RUN DATE   12SEP14  TIME  11:45:27   *                                                *           (916) 756-1104
  *
 *                                      *                                                *
  *
 *****************************************
***************************************


                                    X     X  XXXXXXX   XXXXX          X
                                    X     X  X        X     X        XX
                                    X     X  X        X               X
                                    XXXXXXX  XXXX     X        XXXXX  X
                                    X     X  X        X               X
                                    X     X  X        X     X         X
                                    X     X  XXXXXXX   XXXXX         XXX


          THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

          THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
          THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
          NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
          DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
          KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1                                                       HEC-1 INPUT                                               PAGE  1

         LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

           1           ID        CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
           2           ID        RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
           3           ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
           4           ID       100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
           5           ID           PIMA ROAD FLOWS)
           6           ID
           7           ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
           8           ID       MODELERS: RK, MW
           9           ID
          10           IT       3       0       0    1000
          11           IN       3
          12           IO       5
                       *DIAGRAM
                       *
          13           JD   3.849  0.0001
          14           PC   0.000   0.002   0.005   0.008   0.011   0.014   0.017   0.020   0.023   0.026
          15           PC   0.029   0.032   0.035   0.038   0.041   0.044   0.048   0.052   0.056   0.060
          16           PC   0.064   0.068   0.072   0.076   0.080   0.085   0.090   0.095   0.100   0.105
          17           PC   0.110   0.115   0.120   0.126   0.133   0.140   0.147   0.155   0.163   0.172
          18           PC   0.181   0.191   0.203   0.218   0.236   0.257   0.283   0.387   0.663   0.707
          19           PC   0.735   0.758   0.776   0.791   0.804   0.815   0.825   0.834   0.842   0.849
          20           PC   0.856   0.863   0.869   0.875   0.881   0.887   0.893   0.898   0.903   0.908
          21           PC   0.913   0.918   0.922   0.926   0.930   0.934   0.938   0.942   0.946   0.950
          22           PC   0.953   0.956   0.959   0.962   0.965   0.968   0.971   0.974   0.977   0.980
          23           PC   0.983   0.986   0.989   0.992   0.995   0.998   1.000
          24           JD   3.657    10.0
          25           JD   3.533    20.0
                       *

          26           KK  76THST
          27           KM        76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
          28           KM        100-YR, 6-HR FLO-2D MODEL (XS 98)
          29           BA    0.24
          30           QI       0       0       0       0       0       0       0       0       0       0
          31           QI       0       0       0       0       0       0       0       0       0       0
          32           QI       0       0       0       0       0       0       0       0       0       0
          33           QI       0       0       0       0       0       0       0       0       0       0
          34           QI       0       0       0       0       0       0       0       0       0       0
          35           QI       0       0       0       0       0       0       0       0       0       0
          36           QI       0       0       0       0       0       0       0       0       0       0
          37           QI       0       0       0       0       0       0       0       0       0       0
          38           QI       0       0       0       0       0       0       0       0       0       0
          39           QI       0       0       0       0       0       0       0       0       0       0
          40           QI       0       0    0.01       0       0       0       0       0       0       0
          41           QI       0       0       0       0       0       0       0       0       0       0
          42           QI       0       0       0       0       0       0       0       0       0       0
          43           QI       0       0       0       0       0       0       0       0       0       0
```

```
    44          QI       0       0       0       0    0.01    0.01       0       0       0       0
    45          QI       0       0       0       0       0       0       0       0       0       0
    46          QI       0       0       0       0       0       0       0       0       0       0
    47          QI    0.01       0       0    0.01       0       0       0       0    0.01    0.01
    48          QI       0    0.01    0.01    0.02    0.02    0.02    0.02    0.02    0.03    0.03
    49          QI    0.03    0.04    0.04    0.05    0.05    0.05    0.06    0.06    0.07    0.08
    50          QI    0.09    0.09     0.1     0.1    0.11    0.11    0.12    0.12    0.13    0.14
    51          QI    0.14    0.15    0.16    0.18    0.19    0.21    0.23    0.25    0.27     0.3
1                                         HEC-1 INPUT                                                PAGE  2

  LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

    52          QI    0.34    0.36     0.4    0.44    0.47    0.51    0.56     0.6    0.66    0.71
    53          QI    0.76    0.95    1.61    1.73    1.95     3.7    4.26    6.53    8.52   10.41
    54          QI   17.69   31.39   35.75   41.23   44.49   47.79   51.13   56.33   62.34   66.18
    55          QI   68.57   67.36   65.05   61.38   58.37   54.61   52.38   50.75    48.4   44.37
    56          QI   42.75    40.4   38.91   37.61   36.75   35.61   34.75   34.25   33.52   32.65
    57          QI   32.34   31.59   31.31   30.73   30.02   29.25   28.51   28.42   27.67   26.83
    58          QI   26.13   25.37   24.53   23.95   23.37   22.83   22.17   21.48   20.81   20.94
    59          QI   19.41   19.05   18.62    18.3   18.17   18.11   17.52    17.1    16.7    16.5
    60          QI   16.15   15.74   15.37   15.07   14.74   14.54   14.51   14.24   14.07   13.65
    61          QI   13.75   13.24   12.99   13.09   12.94   12.53   12.53   12.38   12.22   12.18
    62          QI      12   11.59   11.89   11.53   11.85   11.44   11.16   11.22   11.21   10.82
    63          QI   10.52   10.43   10.88   10.42    10.1    9.81    9.72    9.66    9.78    9.55
    64          QI    9.34    9.36    9.33    9.23     9.1    9.09    9.48     9.3    9.02    9.07
    65          QI    9.11     9.1    8.93    8.81    8.73    8.71    8.57     8.5     8.5    8.43
    66          QI    8.37    8.31    8.26    8.17    8.14    8.05    7.98    7.97    7.83    7.62
    67          QI    7.67    7.67    7.57    7.66    7.74    7.75    7.91    7.88    7.62    7.55
    68          QI    7.47     7.3    7.22    7.25     7.2    7.08    7.02    7.01    6.99    6.96
    69          QI    6.97    6.92    6.89    6.85    6.81    6.77    6.75     6.7    6.67    6.62
    70          QI    6.59    6.56    6.51    6.48    6.47    6.44    6.44    6.47    6.43    6.37
    71          QI    6.32    6.27    6.23    6.22    6.16    6.12    6.08    6.05    6.02    5.98
    72          QI    5.95    5.95    5.95    5.93     5.9    5.87    5.84    5.76    5.72    5.64
    73          QI     5.7    5.76    5.62    5.65    5.74    5.84    5.95     4.5    4.58    5.32
    74          QI    5.41    5.43    4.49    5.32    5.36    5.44    5.48     5.6    5.67    4.68
    75          QI    4.56    4.55    4.53    4.82    4.84    4.81    4.81    4.83    4.82     4.6
    76          QI    4.75    4.82    4.66    4.85    4.81    4.78    4.77    4.75    4.73    4.72
    77          QI    4.69    4.64    4.62    4.61    4.63    4.67    4.71    4.72    4.71    4.66
    78          QI    4.61    4.53    4.43    4.39    4.37    4.36    4.33    4.33    4.28    4.26
    79          QI     4.2    4.17    4.13    4.07    4.03       4    3.96    3.92    3.86     3.8
    80          QI    3.75    3.71    3.65     3.6    3.56    3.51    3.45    3.42    3.39    2.43
    81          QI    2.49    2.39    2.38    3.55    3.52    3.43    2.35     2.4    2.46    3.65
    82          QI    3.61     3.3    2.37     2.4    2.46    3.65    3.59    3.59    3.61    3.67
    83          QI    2.48    2.53    2.53    2.47    2.41    2.36    2.32    2.28    2.25    2.22
    84          QI    2.19    2.16    2.14    2.12    2.09    2.07    2.04    2.02       2    1.97
    85          QI    1.94    1.91    1.88    1.85     1.8    1.72    1.63    1.56    1.58    1.65
    86          QI    1.71    1.72    1.67    1.57    1.49    1.36    1.49    1.56    1.58    1.57
    87          QI    1.48    1.35    1.21    1.15    1.13    1.13    1.13    1.13    1.13    1.13
    88          QI    1.13    1.13    1.12    1.14    1.17     1.2    1.23    1.22    1.18    1.09
    89          QI     1.1    1.11    1.13    1.18    1.26    1.06     1.1    1.08    1.06    1.05
    90          QI    1.06    1.05    1.04    1.02    1.01       1    0.98    0.97    0.96    0.95
    91          QI    0.94    0.93    0.92    0.91     0.9    0.89    0.88    0.87    0.86    0.86
    92          QI    0.85    0.84    0.84    0.83    0.82    0.81     0.8     0.8    0.79    0.78
    93          QI    0.78    0.77    0.76    0.76    0.75    0.74    0.73    0.73    0.72    0.72
    94          QI    0.72    0.71    0.71    0.71     0.7     0.7    0.69    0.69    0.68    0.67
    95          QI    0.67    0.66    0.66    0.65    0.65    0.64    0.64    0.63    0.63    0.62
    96          QI    0.62    0.61    0.61     0.6     0.6    0.59    0.59    0.59    0.58    0.58
    97          QI    0.57    0.57    0.56    0.56    0.56    0.55    0.55    0.54    0.54    0.53
    98          QI    0.53    0.53    0.52    0.52    0.52    0.51    0.51     0.5     0.5     0.5
    99          QI    0.49    0.49    0.48    0.48    0.48    0.47    0.47    0.47    0.46    0.46
   100          QI    0.45    0.45    0.45    0.44    0.44    0.44    0.43    0.43    0.43    0.42
   101          QI    0.42    0.42    0.41    0.41    0.41     0.4     0.4     0.4     0.4    0.39
   102          QI    0.39    0.39
                *
1                                         HEC-1 INPUT                                                PAGE  3

  LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


   103          KK    SB01   BASIN
   104          BA   0.112
   105          LG    0.25    0.25    4.10    0.55      45
   106          UC   0.318   0.253
   107          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
   108          UA     100
                *

   109          KK    CP-1 COMBINE
   110          KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
   111          HC       2
                *

   112          KK    R1-2   ROUTE
   113          KM        MILLER RD CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
   114          RK    2104  0.0015    0.03            TRAP      66       4
                *

   115          KK    SB02   BASIN
   116          BA   0.157
   117          LG    0.15    0.25    4.15    0.58      56
   118          UC   0.328   0.240
   119          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
   120          UA     100
                *
```

```
 121          KK    CP-2 COMBINE
 122          KM        SR 101L FREEWAY AND 76TH ST (MILLER RD CHANNEL)
 123          HC       2
              *

 124          KK    R2-3   ROUTE
 125          KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
 126          RK    1260  0.0015   0.03             TRAP      92       4
              *

 127          KK    SB03   BASIN
 128          BA   0.048
 129          LG    0.15    0.25    4.50    0.47      55
 130          UC   0.364   0.294
 131          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 132          UA     100
              *

 133          KK    CP-3 COMBINE
 134          KM        MAYO BLVD AND 76TH ST (MILLER RD CHANNEL)
 135          HC       2
              *
1                                       HEC-1 INPUT                                              PAGE  4

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 136          KK    R3-5   ROUTE
 137          KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
 138          RK    2396  0.0015   0.03             TRAP      98       4
              *

 139          KK    SB04   BASIN
 140          BA   0.144
 141          LG    0.14    0.25    4.60    0.44      61
 142          UC   0.305   0.199
 143          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 144          UA     100
              *

 145          KK    R4-5   ROUTE
 146          KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 147          RK    2005  0.0013   0.03             TRAP      39       4
              *

 148          KK    SB05   BASIN
 149          BA   0.126
 150          LG    0.22    0.25    4.50    0.44      48
 151          UC   0.327   0.226
 152          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 153          UA     100
              *

 154          KK    CP-5 COMBINE
 155          KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 156          HC       3
              *

 157          KK    R5-6   ROUTE
 158          KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 159          RK    1550  0.0015   0.03             TRAP     188       4
              *

 160          KK    SB06   BASIN
 161          BA   0.136
 162          LG    0.16    0.25    4.55    0.45      53
 163          UC   0.321   0.246
 164          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 165          UA     100
              *

 166          KK    CP-6 COMBINE
 167          KM        PRINCESS BLVD AND SCOTTSDALE RD (PRINCESS BLVD CHANNEL)
 168          HC       2
              *
1                                       HEC-1 INPUT                                              PAGE  5

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 169          KK    SB07   BASIN
 170          BA   0.074
 171          LG    0.21    0.25    4.00    0.58      49
 172          UC   0.206   0.113
 173          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 174          UA     100
              *

 175          KK    R7-8   ROUTE
 176          KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 177          RK    2778  0.0014   0.03             TRAP      46       4
              *
```

```
 178          KK    SB08   BASIN
 179          BA   0.224
 180          LG    0.14    0.25    4.10    0.59      59
 181          UC   0.338   0.195
 182          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 183          UA     100
              *

 184          KK    CP-8 COMBINE
 185          KM        SR 101L FREEWAY AND HAYDEN ROAD (HAYDEN ROAD NORTH CHANNEL)
 186          HC       2
              *

 187          KK    R8-9   ROUTE
 188          KM        HAYDEN ROAD NORTH CHANNEL FROM HAYDEN ROAD TO BASIN 53R
 189          RK    1250  0.0013   0.03            TRAP      67       4
              *

 190          KK    SB09   BASIN
 191          BA   0.052
 192          LG    0.15    0.25    4.00    0.61      55
 193          UC   0.254   0.230
 194          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 195          UA     100
              *

 196          KK    CP-9 COMBINE
 197          KM        SR 101L FREEWAY AND BASIN 53R (HAYDEN ROAD NORTH CHANNEL)
 198          HC       2
              *

 199          KK   PWRCH
 200          KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
 201          KM        FROM PINNACLE PEAK SOUTH ADMS 100-YR, 24-HR FLO-2D MODEL
 202          BA     7.0
 203          QI       0       0       0       0       0       0       0       0       0       0
 204          QI       0       0       0       0       0       0       0       0       0       0
 205          QI       0       0       0       0       0       0       0       0       0       0
 206          QI       0       0       0       0       0       0       0       0       0       0
 207          QI       0       0       0       0       0       0       0       0       0       0
 208          QI       0       0       0       0       0       0       0       0       0       0
1                                         HEC-1 INPUT                                            PAGE  6

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 209          QI       0       0       0       0       0       0       0       0       0       0
 210          QI       0       0       0       0       0       0       0       0       0       0
 211          QI       0       0       0       0       0       0       0       0       0       0
 212          QI       0       0       0       0       0       0       0       0       0       0
 213          QI       0       0       0       0       0       0       0       0       0       0
 214          QI       0       0       0       0       0       0       0       0       0       0
 215          QI       0       0       0       0       0       0       0       0       0       0
 216          QI       0       0       0       0       0       0       0       0       0       0
 217          QI       0       0       0       0       0       0       0       0       0       0
 218          QI       0       0       0       0       0       0       0       0       0       0
 219          QI       0       0       0       0       0       0       0       0       0       0
 220          QI       0       0       0       0       0       0       0       0       0       0
 221          QI       0       0       0       0       0       0       0       0       0       0
 222          QI       0       0       0       0       0       0       0       0       0       0
 223          QI       0       0       0       0       0       0       0       0       0       0
 224          QI       0       0       0       0       0       0       0       0       0       0
 225          QI       0       0       1       1       1       1       1       1       1       1
 226          QI       1       2       2       2       3       3       5       9      18      28
 227          QI      43      50      65      87     194     495     872     967    1051    1095
 228          QI    1099    1091    1093    1100    1117    1198    1243    1370    1495    1777
 229          QI    2055    2422    2906    3246    3546    3747    3874    3884    3837    3741
 230          QI    3658    3562    3456    3370    3270    3175    3085    3024    2955    2888
 231          QI    2822    2744    2640    2543    2461    2378    2309    2239    2183    2132
 232          QI    2084    2045    2005    1973    1933    1898    1860    1821    1779    1740
 233          QI    1699    1659    1619    1576    1544    1499    1461    1432    1397    1364
 234          QI    1331    1298    1268    1237    1206    1179    1152    1126    1100    1074
 235          QI    1047    1023    1000     979     959     940     918     901     882     867
 236          QI     851     834     818     803     789     773     759     744     731     719
 237          QI     705     693     683     670     657     645     635     627     617     608
 238          QI     597     588     581     573     566     559     553     545     537     529
 239          QI     521     514     507     501     495     488     482     476     471     465
 240          QI     460     456     450     444     440     435     429     424     419     414
 241          QI     410     404     400     396     393     387     385     381     377     375
 242          QI     370     367     364     362     360     356     352     349     346     343
 243          QI     339     338     334     333     330     328     326     322     318     315
 244          QI     317     313     308     306     305     303     300     298     295     294
 245          QI     293     291     288     285     286     281     278     276     275     276
 246          QI     270     265     269     267     264     257     265     257     255     255
 247          QI     258     251     251     253     245     248     247     242     246     239
 248          QI     240     241     236     238     232     237     231     231     228     231
 249          QI     223     227     223     223     221     220     219     215     219     215
 250          QI     214     215     211     215     211     209     213     209     207     208
 251          QI     205     201     207     200     200     201     201     199     197     193
 252          QI     192     190     190     191     189     188     187     185     186     184
 253          QI     180     179     180     173     173     179     169     172     164     169
 254          QI     164     165     163     161     156     158     155     158     153     158
 255          QI     149     153     146     144     146     142     149     139     136     138
 256          QI     137     143     131     130     132     126     126     124     123     133
 257          QI     120     119     124     117     116     117     114     111     113     109
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 258           QI    107     109     105     106     104     102     104     100      99     103
 259           QI     97      96     100      94      93      99      91      90      97      88
 260           QI     86      90      84      85      87      83      83      85      80      79
 261           QI     82      78      75      82      74      74      82      72      71      76
 262           QI     70      71      70      68      72      66      67      65      64      69
 263           QI     62      65      61      61      63      59      65      58      58      60
1                                             HEC-1 INPUT                                        PAGE  7

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 264           QI     57      63      56      55      58      55      58      54      52      58
 265           QI     52      52      53      50      57      50      50      55      48      48
 266           QI     50      46      47      47      44      48      45      43      49      43
 267           QI     42      48      42      41      45      41      40      44      40      38
 268           QI     40      40      38      37      40      37      36      42      36      36
 269           QI     35      41      35      34      34      40      34      33      33      33
 270           QI     36      33      32      32      33      32      31      31      30      33
 271           QI     31      30      29      29      31      30      28      29      28      29
 272           QI     31      28      28      27      27      30      27      26      26      26
 273           QI     31      26      25      25      25      30      26      25      25      24
 274           QI     24      28      23      23      24      23      24      25      24      23
 275           QI     23      22

               *

 276           KK  BINFLO
 277           KM         TOTAL INFLOW INTO BASIN 53R.
 278           HC       2
               *

 279           KK   BASIN
 280           KM    BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
 281           KM    ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
 282           KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
 283           KM        THE OUTLET PIPES ARE INLET CONTROLLED.
 284           RS       1    STOR       0
 285           SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
 286           SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
 287           SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
 288           SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
 289           SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
 290           SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
               *

 291           KK  BSNRT1
 292           KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
 293           KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
 294           KM        EXCESS CAPACITY.
 295           RK     550  0.0052   0.024            CIRC     7.0
               *

 296           KK  BSNRT2
 297           KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BASIN 53R OUTFAL
 298           RK    1200  0.0077   0.013            CIRC     7.0
               *

 299           KK    SB10   BASIN
 300           BA   0.040
 301           LG    0.15    0.25    4.25    0.55      55
 302           UC   0.233   0.161
 303           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 304           UA     100
               *
1                                             HEC-1 INPUT                                        PAGE  8

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 305           KK   CP-10 COMBINE
 306           KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
 307           HC       2
               *

 308           KK  R10-11   ROUTE
 309           KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
 310           RK    1277  0.0014   0.03             TRAP     220       4
               *

 311           KK    SB11   BASIN
 312           BA   0.071
 313           LG    0.15    0.25    4.15    0.58      55
 314           UC   0.296   0.232
 315           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 316           UA     100
               *

 317           KK   CP-11 COMBINE
 318           KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
 319           HC       2
               *
 320           ZZ
1
          SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
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```
LINE      (V) ROUTING           (--->) DIVERSION OR PUMP FLOW

 NO.      (.) CONNECTOR         (<---) RETURN OF DIVERTED OR PUMPED FLOW

  26      76THST
               .
               .
 103           .        SB01
               .           .
               .           .
 109        CP-1............
               V
               V
 112        R1-2
               .
               .
 115           .        SB02
               .           .
               .           .
 121        CP-2............
               V
               V
 124        R2-3
               .
               .
 127           .        SB03
               .           .
               .           .
 133        CP-3............
               V
               V
 136        R3-5
               .
               .
 139           .        SB04
               .           V
               .           V
 145           .        R4-5
               .           .
               .           .
 148           .           .        SB05
               .           .           .
               .           .           .
 154        CP-5........................
               V
               V
 157        R5-6
               .
               .
 160           .        SB06
               .           .
               .           .
 166        CP-6............
               .
               .
 169           .        SB07
               .           V
               .           V
 175           .        R7-8
               .           .
               .           .
 178           .           .        SB08
               .           .           .
               .           .           .
 184           .        CP-8............
               .           V
               .           V
 187           .        R8-9
               .           .
               .           .
 190           .           .        SB09
               .           .           .
               .           .           .
 196           .        CP-9............
               .           .
               .           .
 199           .           .       PWRCH
               .           .           .
               .           .           .
 276           .      BINFLO............
               .           V
               .           V
 279           .       BASIN
               .           V
               .           V
 291           .      BSNRT1
               .           V
               .           V
 296           .      BSNRT2
               .           .
               .           .
 299           .           .        SB10
               .           .           .
               .           .           .
```

```
  305          .       CP-10...........
               .           V
               .           V
  308          .      R10-11
               .           .
               .           .
  311          .           .        SB11
               .           .           .
               .           .           .
  317          .       CP-11...........

 (***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1*****************************************
***************************************
 *                                       *                                                   *
  *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                   *    U.S. ARMY CORPS OF ENGINEERS
  *
 *             JUN   1998                *                                                   *    HYDROLOGIC ENGINEERING CENTER
  *
 *           VERSION 4.1                 *                                                   *         609 SECOND STREET
  *
 *                                       *                                                   *       DAVIS, CALIFORNIA 95616
  *
 *  RUN DATE   12SEP14  TIME  11:45:27   *                                                   *           (916) 756-1104
  *
 *                                       *                                                   *
  *
 *****************************************
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```
                            CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
                            RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                            INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                              100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                               PIMA ROAD FLOWS)

                            PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                              MODELERS: RK, MW

  12 IO         OUTPUT CONTROL VARIABLES
                      IPRNT          5  PRINT CONTROL
                      IPLOT          0  PLOT CONTROL
                      QSCAL         0.  HYDROGRAPH PLOT SCALE

     IT         HYDROGRAPH TIME DATA
                       NMIN          3  MINUTES IN COMPUTATION INTERVAL
                      IDATE      1    0  STARTING DATE
                      ITIME       0000  STARTING TIME
                         NQ       1000  NUMBER OF HYDROGRAPH ORDINATES
                     NDDATE      3    0  ENDING DATE
                     NDTIME       0157  ENDING TIME
                     ICENT          19  CENTURY MARK

                  COMPUTATION INTERVAL    0.05 HOURS
                       TOTAL TIME BASE   49.95 HOURS

         ENGLISH UNITS
              DRAINAGE AREA         SQUARE MILES
              PRECIPITATION DEPTH   INCHES
              LENGTH, ELEVATION     FEET
              FLOW                  CUBIC FEET PER SECOND
              STORAGE VOLUME        ACRE-FEET
              SURFACE AREA          ACRES
              TEMPERATURE           DEGREES FAHRENHEIT

  13 JD         INDEX STORM NO. 1
                       STRM       3.85  PRECIPITATION DEPTH
                       TRDA       0.00  TRANSPOSITION DRAINAGE AREA

  14 PI           PRECIPITATION PATTERN
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.01      0.01      0.00      0.01      0.00      0.01
                     0.01      0.00      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                     0.01      0.01      0.02      0.02      0.02      0.03      0.10      0.28      0.04      0.03
                     0.02      0.02      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                     0.01      0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00

  24 JD         INDEX STORM NO. 2
                       STRM       3.66  PRECIPITATION DEPTH
                       TRDA      10.00  TRANSPOSITION DRAINAGE AREA

   0 PI           PRECIPITATION PATTERN
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
```

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                 0.00      0.00      0.00      0.00      0.01      0.01      0.00      0.01      0.00      0.01
                 0.01      0.00      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                 0.01      0.01      0.02      0.02      0.02      0.03      0.10      0.28      0.04      0.03
                 0.02      0.02      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                 0.01      0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00

25 JD          INDEX STORM NO. 3
                   STRM       3.53  PRECIPITATION DEPTH
                   TRDA      20.00  TRANSPOSITION DRAINAGE AREA

 0 PI            PRECIPITATION PATTERN
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.01      0.01      0.00      0.01      0.00      0.01
                 0.01      0.00      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                 0.01      0.01      0.02      0.02      0.02      0.03      0.10      0.28      0.04      0.03
                 0.02      0.02      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                 0.01      0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00

       *** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT


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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+										
	HYDROGRAPH AT									
+		76THST	69.	12.50	23.	8.	4.	0.24		
	HYDROGRAPH AT									
+		SB01	278.	2.55	30.	8.	4.	0.11		
	2 COMBINED AT									

+		CP-1	276.	2.55	30.	15.	8.	0.35
+	ROUTED TO	R1-2	273.	2.70	30.	15.	8.	0.35
+	HYDROGRAPH AT	SB02	410.	2.55	46.	12.	6.	0.16
+	2 COMBINED AT	CP-2	627.	2.65	76.	26.	13.	0.51
+	ROUTED TO	R2-3	618.	2.70	76.	26.	13.	0.51
+	HYDROGRAPH AT	SB03	111.	2.60	14.	4.	2.	0.05
+	2 COMBINED AT	CP-3	720.	2.70	90.	30.	15.	0.56
+	ROUTED TO	R3-5	713.	2.80	90.	30.	15.	0.56
+	HYDROGRAPH AT	SB04	440.	2.55	45.	11.	5.	0.14
+	ROUTED TO	R4-5	432.	2.65	45.	11.	5.	0.14
+	HYDROGRAPH AT	SB05	340.	2.55	36.	9.	4.	0.13
+	3 COMBINED AT	CP-5	1265.	2.75	170.	50.	25.	0.83
+	ROUTED TO	R5-6	1242.	2.80	169.	50.	25.	0.83
+	HYDROGRAPH AT	SB06	355.	2.55	40.	10.	5.	0.14
+	2 COMBINED AT	CP-6	1466.	2.80	208.	60.	29.	0.96
+	HYDROGRAPH AT	SB07	322.	2.50	21.	5.	2.	0.07
+	ROUTED TO	R7-8	310.	2.65	21.	5.	3.	0.07
+	HYDROGRAPH AT	SB08	660.	2.55	68.	17.	8.	0.22
+	2 COMBINED AT	CP-8	903.	2.65	89.	22.	11.	0.30
+	ROUTED TO	R8-9	880.	2.70	89.	22.	11.	0.30
+	HYDROGRAPH AT	SB09	147.	2.55	15.	4.	2.	0.05
+	2 COMBINED AT	CP-9	985.	2.70	104.	26.	12.	0.35
+	HYDROGRAPH AT	PWRCH	3884.	13.35	1547.	503.	248.	7.00
+	2 COMBINED AT	BINFLO	3884.	13.35	1547.	503.	260.	7.35
+	ROUTED TO	BASIN	463.	18.45	458.	399.	251.	7.35
+	ROUTED TO	BSNRT1	463.	18.50	458.	399.	251.	7.35
+	ROUTED TO	BSNRT2	463.	18.50	458.	399.	251.	7.35
+	HYDROGRAPH AT	SB10	141.	2.50	12.	3.	1.	0.04
+	2 COMBINED AT	CP-10	463.	18.50	458.	399.	252.	7.39
+	ROUTED TO	R10-11	463.	18.55	458.	399.	252.	7.39
+	HYDROGRAPH AT	SB11	195.	2.55	21.	5.	3.	0.07
+	2 COMBINED AT	CP-11	463.	18.55	458.	399.	254.	7.46

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SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING

(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	INTERPOLATED TO COMPUTATION INTERVAL DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)
FOR STORM = 1 STORM AREA (SQ MI) = 0.00									
R1-2	MANE	3.00	287.35	160.24	1.72	3.00	284.38	162.00	1.72

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3222E+02 EXCESS=0.0000E+00 OUTFLOW=0.3226E+02 BASIN STORAGE=0.5145E-01 PERCENT ERROR= -0.3

FOR STORM = 2 STORM AREA (SQ MI) = 10.00									
R1-2	MANE	3.00	270.30	162.31	1.67	3.00	267.71	162.00	1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3132E+02 EXCESS=0.0000E+00 OUTFLOW=0.3135E+02 BASIN STORAGE=0.5145E-01 PERCENT ERROR= -0.3

FOR STORM = 3 STORM AREA (SQ MI) = 20.00									
R1-2	MANE	3.00	259.93	162.15	1.64	3.00	259.40	162.00	1.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3073E+02 EXCESS=0.0000E+00 OUTFLOW=0.3080E+02 BASIN STORAGE=0.5496E-01 PERCENT ERROR= -0.4

FOR STORM = 1 STORM AREA (SQ MI) = 0.00									
R2-3	MANE	1.51	651.07	161.96	2.07	3.00	651.05	162.00	2.07

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5609E+02 EXCESS=0.0000E+00 OUTFLOW=0.5610E+02 BASIN STORAGE=0.3635E-01 PERCENT ERROR= -0.1

FOR STORM = 2 STORM AREA (SQ MI) = 10.00									
R2-3	MANE	1.68	617.22	162.52	1.98	3.00	607.09	162.00	1.98

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5389E+02 EXCESS=0.0000E+00 OUTFLOW=0.5385E+02 BASIN STORAGE=0.3635E-01 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 20.00									
R2-3	MANE	1.57	606.67	162.84	1.93	3.00	594.12	162.00	1.94

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5252E+02 EXCESS=0.0000E+00 OUTFLOW=0.5252E+02 BASIN STORAGE=0.3881E-01 PERCENT ERROR= -0.1

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R3-5	MANE	2.90	746.61	169.74	2.14	3.00	740.89	168.00	2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6349E+02 EXCESS=0.0000E+00 OUTFLOW=0.6368E+02 BASIN STORAGE=0.7132E-01 PERCENT ERROR= -0.4

FOR STORM = 2 STORM AREA (SQ MI) = 10.00

R3-5	MANE	2.99	705.69	168.12	2.05	3.00	703.45	168.00	2.05

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6077E+02 EXCESS=0.0000E+00 OUTFLOW=0.6099E+02 BASIN STORAGE=0.7609E-01 PERCENT ERROR= -0.5

FOR STORM = 3 STORM AREA (SQ MI) = 20.00

R3-5	MANE	2.97	683.83	168.92	2.00	3.00	669.67	168.00	2.00

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5920E+02 EXCESS=0.0000E+00 OUTFLOW=0.5943E+02 BASIN STORAGE=0.7619E-01 PERCENT ERROR= -0.5

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R4-5	MANE	2.44	447.26	159.45	3.02	3.00	444.07	159.00	3.02

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2308E+02 EXCESS=0.0000E+00 OUTFLOW=0.2320E+02 BASIN STORAGE=0.4656E-04 PERCENT ERROR= -0.5

FOR STORM = 2 STORM AREA (SQ MI) = 10.00

R4-5	MANE	2.59	429.78	158.16	2.87	3.00	425.33	159.00	2.86

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2182E+02 EXCESS=0.0000E+00 OUTFLOW=0.2203E+02 BASIN STORAGE=0.4483E-04 PERCENT ERROR= -1.0

FOR STORM = 3 STORM AREA (SQ MI) = 20.00

R4-5	MANE	2.60	410.95	158.84	2.76	3.00	410.03	159.00	2.76

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2100E+02 EXCESS=0.0000E+00 OUTFLOW=0.2116E+02 BASIN STORAGE=0.3691E-04 PERCENT ERROR= -0.8

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          FOR STORM = 1  STORM AREA (SQ MI) =       0.00

              R5-6  MANE         1.77   1321.25    168.17      2.39      3.00   1316.26    168.00        2.39



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1051E+03 EXCESS=0.0000E+00 OUTFLOW=0.1053E+03 BASIN STORAGE=0.6328E-01 PERCENT ERROR=
-0.3


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00

              R5-6  MANE         1.94   1236.19    169.97      2.28      3.00   1225.73    171.00        2.28



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1002E+03 EXCESS=0.0000E+00 OUTFLOW=0.1004E+03 BASIN STORAGE=0.6753E-01 PERCENT ERROR=
-0.2


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00

              R5-6  MANE         1.82   1180.28    168.70      2.20      3.00   1169.55    171.00        2.20



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9721E+02 EXCESS=0.0000E+00 OUTFLOW=0.9718E+02 BASIN STORAGE=0.6757E-01 PERCENT ERROR=
0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00

              R7-8  MANE         2.98    320.52    158.47      2.78      3.00    316.88    159.00        2.78



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1062E+02 EXCESS=0.0000E+00 OUTFLOW=0.1097E+02 BASIN STORAGE=0.1432E-03 PERCENT ERROR=
-3.3


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00

              R7-8  MANE         2.87    305.35    159.01      2.61      3.00    304.85    159.00        2.62



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1001E+02 EXCESS=0.0000E+00 OUTFLOW=0.1032E+02 BASIN STORAGE=0.1568E-03 PERCENT ERROR=
-3.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00

              R7-8  MANE         3.00    287.36    160.28      2.59      3.00    278.18    159.00        2.59



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9623E+01 EXCESS=0.0000E+00 OUTFLOW=0.1023E+02 BASIN STORAGE=0.1386E-03 PERCENT ERROR=
-6.3


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00

              R8-9  MANE         1.46    914.50    162.26      2.88      3.00    911.54    162.00        2.88



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4580E+02 EXCESS=0.0000E+00 OUTFLOW=0.4583E+02 BASIN STORAGE=0.2113E-03 PERCENT ERROR=
-0.1
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          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R8-9  MANE         1.32    877.55    161.38      2.73      3.00    866.18    162.00        2.73



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4326E+02 EXCESS=0.0000E+00 OUTFLOW=0.4337E+02 BASIN STORAGE=0.2285E-03 PERCENT ERROR=
-0.3


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R8-9  MANE         1.49    824.02    161.14      2.64      3.00    818.74    162.00        2.64



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4190E+02 EXCESS=0.0000E+00 OUTFLOW=0.4192E+02 BASIN STORAGE=0.2053E-03 PERCENT ERROR=
0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT1  MANE         0.33    462.77   1107.64      2.65      3.00    462.77   1110.00        2.65



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1038E+04 EXCESS=0.0000E+00 OUTFLOW=0.1037E+04 BASIN STORAGE=0.9252E-01 PERCENT ERROR=
0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            BSNRT1  MANE         0.33    462.63   1107.74      2.64      3.00    462.63   1110.00        2.64



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1035E+04 EXCESS=0.0000E+00 OUTFLOW=0.1035E+04 BASIN STORAGE=0.9241E-01 PERCENT ERROR=
0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            BSNRT1  MANE         0.34    462.54   1107.73      2.64      3.00    462.54   1110.00        2.64



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1033E+04 EXCESS=0.0000E+00 OUTFLOW=0.1033E+04 BASIN STORAGE=0.9237E-01 PERCENT ERROR=
0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE         0.48    462.77   1110.07      2.65      3.00    462.77   1110.00        2.65



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1038E+04 EXCESS=0.0000E+00 OUTFLOW=0.1037E+04 BASIN STORAGE=0.1054E+00 PERCENT ERROR=
0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            BSNRT2  MANE         0.37    462.63   1110.22      2.64      3.00    462.63   1110.00        2.64
```

```
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1035E+04 EXCESS=0.0000E+00 OUTFLOW=0.1035E+04 BASIN STORAGE=0.1056E+00 PERCENT ERROR=
0.0


           FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             BSNRT2  MANE         0.33    462.54   1110.65      2.64      3.00    462.54   1110.00        2.63


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1033E+04 EXCESS=0.0000E+00 OUTFLOW=0.1033E+04 BASIN STORAGE=0.1056E+00 PERCENT ERROR=
0.0


           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R10-11  MANE         2.40    462.77   1113.47      2.64      3.00    462.77   1113.00        2.64


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1043E+04 EXCESS=0.0000E+00 OUTFLOW=0.1042E+04 BASIN STORAGE=0.1560E+01 PERCENT ERROR=
0.0


           FOR STORM = 2  STORM AREA (SQ MI) =      10.00
             R10-11  MANE         2.40    462.63   1113.70      2.64      3.00    462.62   1116.00        2.64


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1040E+04 EXCESS=0.0000E+00 OUTFLOW=0.1039E+04 BASIN STORAGE=0.1557E+01 PERCENT ERROR=
0.0

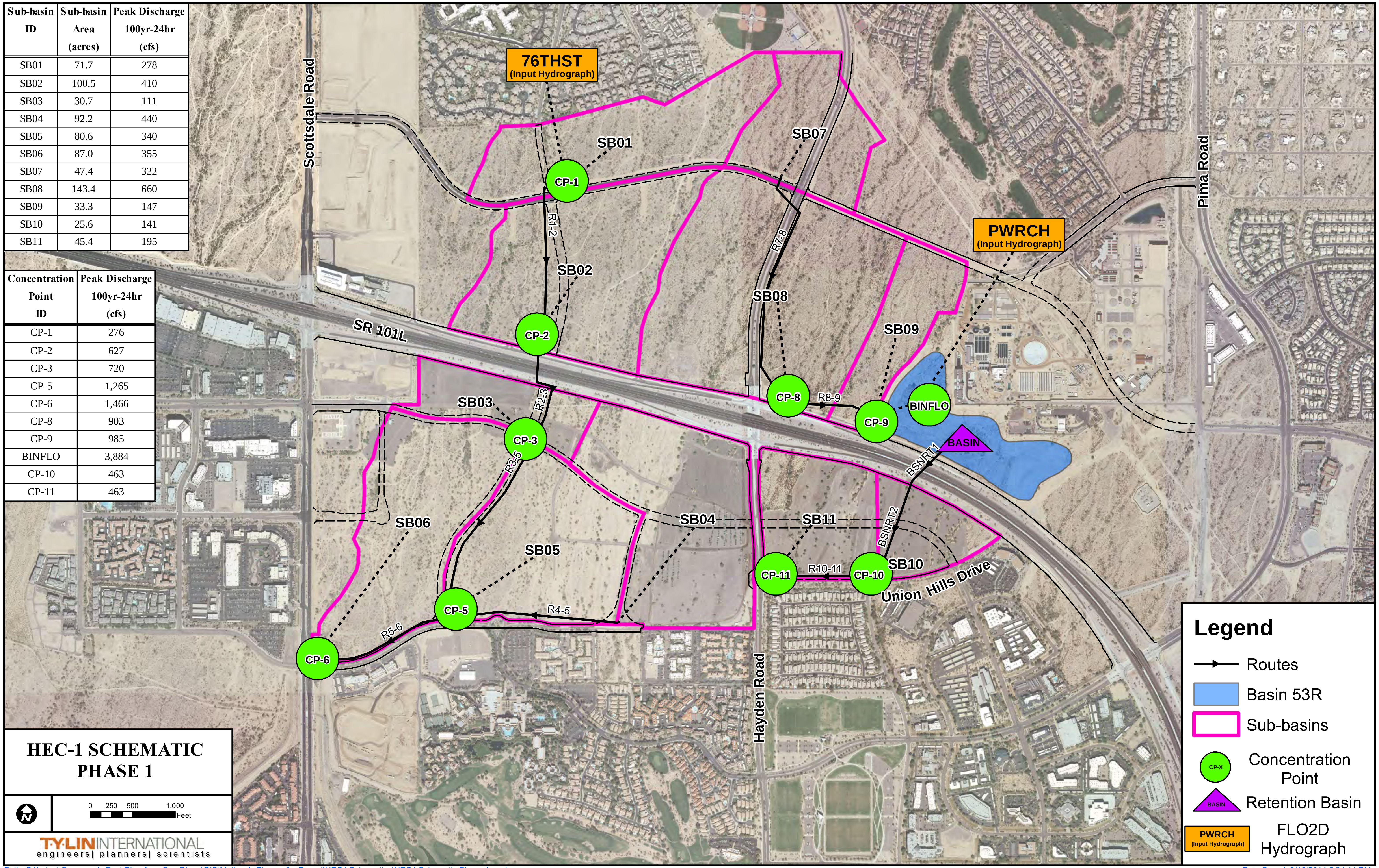

           FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             R10-11  MANE         2.47    462.54   1114.58      2.63      3.00    462.54   1113.00        2.63


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1038E+04 EXCESS=0.0000E+00 OUTFLOW=0.1037E+04 BASIN STORAGE=0.1559E+01 PERCENT ERROR=
0.0


 *** NORMAL END OF HEC-1 ***
```

Sub-basin ID	Sub-basin Area (acres)	Peak Discharge 100yr-24hr (cfs)
SB01	71.7	278
SB02	100.5	410
SB03	30.7	111
SB04	92.2	440
SB05	80.6	340
SB06	87.0	355
SB07	47.4	322
SB08	143.4	660
SB09	33.3	147
SB10	25.6	141
SB11	45.4	195

Concentration Point ID	Peak Discharge 100yr-24hr (cfs)
CP-1	276
CP-2	627
CP-3	720
CP-5	1,265
CP-6	1,466
CP-8	903
CP-9	985
BINFLO	3,884
CP-10	463
CP-11	463



HEC-1 SCHEMATIC PHASE 1

TYLIN INTERNATIONAL
engineers| planners| scientists

Path: Q:\Latest Crossroads East Files from San Diego\GIS\Mathew's Figures for Report\HEC1 Schematics\HEC1 Schematic Phase 1.mxd

Date Saved: 9/10/2014 5:34:14 PM

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1*****************************************
***************************************
 *                                       *                                                *
  *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                *    U.S. ARMY CORPS OF ENGINEERS
  *
 *             JUN   1998                *                                                *    HYDROLOGIC ENGINEERING CENTER
  *
 *           VERSION 4.1                 *                                                *          609 SECOND STREET
  *
 *                                       *                                                *       DAVIS, CALIFORNIA 95616
  *
 *  RUN DATE   12SEP14  TIME  11:47:04   *                                                *           (916) 756-1104
  *
 *                                       *                                                *
  *
 *****************************************
***************************************
```

```
X     X  XXXXXXX   XXXXX          X
X     X  X         X     X       XX
X     X  X         X              X
XXXXXXX  XXXX      X       XXXXX  X
X     X  X         X              X
X     X  X         X     X        X
X     X  XXXXXXX   XXXXX         XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```
1                                                 HEC-1 INPUT                                              PAGE  1

         LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

           1           ID        CROSS ROADS EAST DRAINAGE ALTERNATIVES
           2           ID        RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
           3           ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
           4           ID       100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
           5           ID           PIMA ROAD FLOWS)
           6           ID        PHASE 2 BASIN - SIDESLOPES STEEPENED FROM ~5:1 TO 3:1
           7           ID
           8           ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
           9           ID       MODELERS: RK, MW
          10           ID
          11           IT       3       0       0    1000
          12           IO       3
                       *DIAGRAM
                       *
          13           JD   2.755  0.0001
          14           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
          15           PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
          16           PC   0.962   0.972   0.983   0.991   1.000
          17           JD   2.738  0.5000
          18           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
          19           PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
          20           PC   0.962   0.972   0.983   0.991   1.000
          21           JD   2.686     2.8
          22           PC   0.000   0.009   0.016   0.025   0.034   0.042   0.051   0.059   0.068   0.077
          23           PC   0.088   0.101   0.121   0.164   0.253   0.451   0.694   0.836   0.900   0.938
          24           PC   0.950   0.963   0.975   0.988   1.000
          25           JD   2.540    16.0
          26           PC   0.000   0.015   0.020   0.030   0.048   0.063   0.076   0.090   0.105   0.119
          27           PC   0.135   0.152   0.175   0.222   0.304   0.472   0.670   0.796   0.868   0.912
          28           PC   0.946   0.960   0.973   0.987   1.000
          29           JD   2.232    90.0
          30           PC   0.000   0.021   0.035   0.051   0.071   0.087   0.105   0.125   0.143   0.160
          31           PC   0.179   0.201   0.232   0.281   0.364   0.500   0.658   0.773   0.841   0.888
          32           PC   0.927   0.945   0.964   0.982   1.000
                       *

          33           KK  76THST
          34           KM        76TH STREET CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
          35           KM        100-YR, 6-HR FLO-2D MODEL (XS 98)
          36           BA    0.24
          37           QI       0       0       0       0       0       0       0       0       0       0
          38           QI       0       0       0       0       0       0       0       0       0       0
          39           QI       0       0       0       0       0       0       0       0       0       0
          40           QI       0       0       0       0       0       0       0       0       0       0
          41           QI       0       0       0       0       0       0       0       0       0       0
          42           QI       0       0    0.01       0       0       0       0       0       0       0
          43           QI    0.01       0       0    0.01    0.01    0.01    0.02    0.03    0.05    0.07
```

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   44          QI    0.09    0.16    0.27    0.42    0.63     0.9    2.34    3.89    7.48   10.26
   45          QI    12.7   14.07   17.67   27.43   38.25   42.26   42.56   42.35   43.77   48.39
   46          QI   49.92   51.19   50.98   50.52   49.38    48.6   44.64   42.62   39.72   36.96
   47          QI   34.49   32.83   31.02    29.6   28.15   26.72   25.66   24.48   23.53   22.56
   48          QI   21.55   20.83   19.46   20.34   18.32   17.51   16.55   16.22   15.59   15.15
   49          QI   14.81   14.23   13.72   13.18   13.21   12.84   12.36   12.34   12.16      12
   50          QI   11.63   11.31   11.37   11.23   11.13   10.56   10.63   10.27    9.98    9.86
   51          QI    9.58    9.02    9.34    8.67    8.51    8.32    8.08    7.75    7.43    7.37
1                                            HEC-1 INPUT                                             PAGE  2

 LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

   52          QI    7.29    7.19    7.08       7    6.89    6.73    6.55     6.4    6.23    6.02
   53          QI    5.83    5.66    5.06    4.57    4.75    4.76    4.65    4.55    4.44    4.32
   54          QI    4.23    4.15    4.05    3.95    3.87    3.81    3.68    3.54    3.48    3.42
   55          QI     3.4    2.42    2.38     3.4    3.61    3.55    3.49    2.89    2.36    2.36
   56          QI    2.36    2.37    2.44    3.67    2.44    2.51    2.49    2.47    2.42    2.37
   57          QI    2.32    2.29    2.26    2.23    2.19    2.16    2.13     2.1    2.07    2.05
   58          QI    2.02    1.99    1.96    1.91    1.86    1.83    1.78    1.69    1.58    1.54
   59          QI    1.58    1.65    1.69    1.67    1.59     1.5    1.36    1.41    1.51    1.54
   60          QI    1.52    1.35    1.43    1.37    1.25    1.21     1.2     1.2    1.23    1.28
   61          QI    1.31    1.34    1.35    1.35    1.34    1.34    1.33    1.33    1.26    1.09
   62          QI    1.09    1.11    1.16    1.22    1.06    1.07    1.15    1.06    1.05    1.05
   63          QI    1.04    1.02    1.01       1    0.98    0.97    0.96    0.95    0.94    0.92
   64          QI    0.91     0.9    0.89    0.88    0.87    0.86    0.85    0.85    0.84    0.84
   65          QI    0.83    0.82    0.81     0.8    0.79    0.79    0.78    0.77    0.77    0.76
   66          QI    0.76    0.75    0.74    0.74    0.73    0.72    0.71    0.71     0.7    0.69
   67          QI    0.69    0.68    0.68    0.67    0.67    0.67    0.66    0.66    0.65    0.65
   68          QI    0.65    0.64    0.64    0.63    0.63    0.62    0.62    0.62    0.61    0.61
   69          QI     0.6     0.6    0.59    0.59    0.58    0.58    0.57    0.57    0.56    0.56
   70          QI    0.55    0.55    0.54    0.54    0.54    0.53    0.53    0.52    0.52    0.52
   71          QI    0.51    0.51     0.5     0.5     0.5    0.49    0.49    0.48    0.48    0.48
   72          QI    0.47    0.47    0.46    0.46    0.46    0.45    0.45    0.45    0.44    0.44
   73          QI    0.44
               *

   74          KK    SB01   BASIN
   75          BA   0.112
   76          LG    0.25    0.25    4.10    0.55      45
   77          UC   0.317   0.252
   78          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
   79          UA     100
               *

   80          KK    CP-1 COMBINE
   81          KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
   82          HC       2
               *

   83          KK    R1-2   ROUTE
   84          KM        MILLER RD CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
   85          RK    2104  0.0015   0.03            TRAP      66       4
               *

   86          KK    SB02   BASIN
   87          BA   0.157
   88          LG    0.15    0.25    4.15    0.58      56
   89          UC   0.324   0.237
   90          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
   91          UA     100
               *
1                                            HEC-1 INPUT                                             PAGE  3

 LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


   92          KK    CP-2 COMBINE
   93          KM        SR 101L FREEWAY AND 76TH ST (MILLER RD CHANNEL)
   94          HC       2
               *

   95          KK    R2-3   ROUTE
   96          KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
   97          RK    1260  0.0015   0.03            TRAP      92       4
               *

   98          KK    SB03   BASIN
   99          BA   0.048
  100          LG    0.15    0.25    4.50    0.47      55
  101          UC   0.360   0.290
  102          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
  103          UA     100
               *

  104          KK    CP-3 COMBINE
  105          KM        MAYO BLVD AND 76TH ST (MILLER RD CHANNEL)
  106          HC       2
               *

  107          KK    R3-5   ROUTE
  108          KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
  109          RK    2396  0.0015   0.03            TRAP      98       4
               *
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 110           KK    SB04   BASIN
 111           BA   0.144
 112           LG    0.14    0.25    4.60    0.44      61
 113           UC   0.301   0.196
 114           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 115           UA     100
               *

 116           KK    R4-5   ROUTE
 117           KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 118           RK    2005  0.0013   0.03             TRAP      39       4
               *

 119           KK    SB05   BASIN
 120           BA   0.126
 121           LG    0.22    0.25    4.50    0.44      48
 122           UC   0.326   0.224
 123           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 124           UA     100
               *
1                                           HEC-1 INPUT                                             PAGE  4

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 125           KK    CP-5 COMBINE
 126           KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 127           HC       3
               *

 128           KK    R5-6   ROUTE
 129           KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 130           RK    1550  0.0015   0.03             TRAP     188       4
               *

 131           KK    SB06   BASIN
 132           BA   0.136
 133           LG    0.16    0.25    4.55    0.45      53
 134           UC   0.318   0.243
 135           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 136           UA     100
               *

 137           KK    CP-6 COMBINE
 138           KM        PRINCESS BLVD AND SCOTTSDALE RD (PRINCESS BLVD CHANNEL)
 139           HC       2
               *

 140           KK    SB07   BASIN
 141           BA   0.074
 142           LG    0.21    0.25    4.00    0.58      49
 143           UC   0.204   0.113
 144           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 145           UA     100
               *

 146           KK    R7-8   ROUTE
 147           KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 148           RK    2778  0.0014   0.03             TRAP      46       4
               *

 149           KK    SB08   BASIN
 150           BA   0.224
 151           LG    0.14    0.25    4.10    0.59      59
 152           UC   0.333   0.193
 153           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 154           UA     100
               *

 155           KK    CP-8 COMBINE
 156           KM        SR 101L FREEWAY AND HAYDEN ROAD (HAYDEN ROAD NORTH CHANNEL)
 157           HC       2
               *
1                                           HEC-1 INPUT                                             PAGE  5

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 158           KK    R8-9   ROUTE
 159           KM        HAYDEN ROAD NORTH CHANNEL FROM HAYDEN ROAD TO BASIN 53R
 160           RK    1250  0.0013   0.03             TRAP      67       4
               *

 161           KK    SB09   BASIN
 162           BA   0.052
 163           LG    0.15    0.25    4.00    0.61      55
 164           UC   0.251   0.227
 165           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 166           UA     100
               *

 167           KK    CP-9 COMBINE
 168           KM        SR 101L FREEWAY AND BASIN 53R (HAYDEN ROAD NORTH CHANNEL)
 169           HC       2
```

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              *

 170          KK  PIMACH
 171          KM        PIMA ROAD CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
 172          KM        100-YR, 6-HR FLO-2D MODEL (XS 183)
 173          BA    12.3
 174          QI       0       0       0       0       0       0       0       0       0       0
 175          QI       0       0       0       0       0       0       0       0       0       0
 176          QI       0       0       0       0       0       0       0       0       0       0
 177          QI       0       0       0       0       0       0       0       0       0       0
 178          QI       0       0       0       0       0       0       0       0       0       0
 179          QI       0       0       0       0       0       0       0       0       0       0
 180          QI       0       0       0       0       0       0    0.01    0.01    0.01    0.01
 181          QI    0.02    0.06    0.12    0.19    0.29    0.43    1.62    3.73    6.28   13.12
 182          QI   19.17   18.99   20.23   23.44    37.1    65.6  118.28  536.07  740.49  784.96
 183          QI  795.73  792.08  785.39  784.16  826.86  835.79  839.66  856.83  890.85 1074.56
 184          QI  1522.8 1924.46 2612.09 2761.62 3125.86 3450.94 3565.35 3786.18 3953.42 4005.23
 185          QI  4017.2 3956.42 3828.66 3704.96 3584.74 3457.47 3327.91 3209.52  3068.2 2902.83
 186          QI  2698.1 2519.92 2353.91 2218.47 2091.25 1970.95 1868.74 1749.96 1656.67 1564.23
 187          QI  1473.2 1392.42 1326.33 1253.44 1190.18 1121.44 1068.33  1012.4  961.04  907.15
 188          QI     859  817.16  777.05  741.05  708.67  676.64  650.15     620  595.68  569.46
 189          QI   552.9  528.08  508.87  488.92  471.15  456.27  440.43  424.35  410.65  396.26
 190          QI  384.05   374.5  363.39  351.31  341.86  332.93  325.13  317.66  308.91  300.01
 191          QI  291.96  282.64   276.3  270.44  264.03  257.52  251.57  244.49  238.62  232.96
 192          QI  227.38  223.46  217.91  210.73  207.24  202.22  197.15  192.34  189.04  184.57
 193          QI  178.93   173.7  170.95   166.9  162.57  158.97  155.56  151.97  149.46  146.92
 194          QI   143.8  141.62  138.05  136.38   130.4  128.32  123.98  120.84   117.3  114.09
 195          QI  113.28  110.56  108.01  104.98  103.24  100.82   98.69   97.77    95.8   93.56
 196          QI    92.8   91.19   88.35   86.44   84.49   82.18    80.7   78.31   76.07   74.85
 197          QI   73.81   73.17   71.94   70.66   69.66   69.54   67.38   67.61   65.17   62.74
 198          QI   61.41   60.29   61.36   59.06   56.97   56.32   55.01   53.88   52.91   52.23
 199          QI   52.11   50.68   50.15    49.6   48.36    47.6   46.82    46.4   45.54   44.69
 200          QI   43.67   43.34   42.77    41.9   41.04   40.86   40.39   39.72   38.49   38.26
 201          QI   37.01   36.83   36.64   35.35   35.64   34.94   35.17    33.9    33.7   33.71
 202          QI   33.78   32.55   32.35   31.97   31.61   31.38   31.21   30.84   30.41      30
 203          QI   29.61   29.27   28.72   28.17   27.62   27.26   26.95   26.26   25.93   25.66
 204          QI    25.7   25.62    25.7   25.39   24.94   24.57   24.28   23.92   23.55   22.97
 205          QI    22.7   22.79   22.23   21.94   21.87    21.6    21.3   21.18   21.05   20.75
1                                           HEC-1 INPUT                                           PAGE  6

 LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 206          QI   20.64    20.4   20.19   19.93    19.9   19.01   18.72    18.7   18.37   18.27
 207          QI    18.1   18.05    17.3   17.39   17.27   17.09   16.86   16.81   16.73    16.6
 208          QI   16.55   16.33   16.05   15.82   15.63   15.43   15.38   15.42   15.17   14.98
 209          QI   14.82   14.78   14.76   14.57   14.24   14.01   13.85   13.94   13.77   13.77
 210          QI    12.6
              *

 211          KK  PIMAIN
 212          KM    BYPASS ALONG SE SIDE OF BASIN 53R TO ADOT CULVERT NEAR UNION HILLS DR
 213          DT  PIMABY
 214          DI       0    1000   10000
 215          DQ       0    1000    1000
              *

 216          KK   PWRCH
 217          KM        POWERLINE CHANNEL HYDROGRAPH FROM FLO-2D
 218          KM        100-YR, 6-HR FLO-2D MODEL (XS 107)
 219          BA     7.0
 220          QI       0       0       0       0       0       0       0       0       0       0
 221          QI       0       0       0       0       0       0       0       0       0       0
 222          QI       0       0       0       0       0       0       0       0       0       0
 223          QI       0       0       0       0       0       0       0       0       0       0
 224          QI       0       0       0       0       0       0       0       0       0       0
 225          QI       0       0       0       0       0       0       0       0       0       0
 226          QI       0       0       0       0       0       0       0       0       0       0
 227          QI       0       0    0.03    0.05    0.08    0.11    0.58    1.13    1.62    2.35
 228          QI   17.74   24.61   32.43    41.1      46   50.91  131.64  355.54   399.7  422.85
 229          QI  433.53  433.73  422.44  409.12  397.71  391.08  388.91   381.3  371.02  361.84
 230          QI  359.13  372.83  390.54  400.38  398.98  391.64  390.01  419.65  505.31  577.51
 231          QI  616.52   654.6  689.73   713.7  727.81  737.85  739.81  733.87  725.12  714.86
 232          QI  699.64  690.12  677.33  666.62  655.55  645.41  637.05  633.81  630.71  627.88
 233          QI  621.52  616.56  611.87  606.82  603.07  599.48  598.68  597.46  596.17  595.28
 234          QI  594.12  590.26  586.44  582.19  576.08  570.84  564.08  555.83  546.74  537.07
 235          QI  525.86  516.15  505.83  495.17  484.12  472.45  462.13  451.31  441.83  429.35
 236          QI  419.09  410.42  401.94   394.3  384.43  375.56  366.51  358.68  350.24   342.9
 237          QI     335  326.95  320.66  313.52  306.21  299.42  294.88  279.13  274.41  272.37
 238          QI   270.9  258.49  256.83  252.87  239.19  235.68  236.09  226.97  221.44  220.04
 239          QI  211.13  213.73  201.19  203.88   194.6  196.81  183.61  188.47  178.11  181.39
 240          QI  172.83  170.18  168.28  163.33  169.45  155.83  153.79  154.42  148.45  145.47
 241          QI  146.04  140.33  138.63  140.19  133.49  130.23  135.24  122.64  121.61  121.05
 242          QI   120.9  117.47  115.13  113.28   116.6  110.32  107.03  105.71  109.02  101.22
 243          QI   99.25   97.86   99.48  103.28   92.95    90.9   89.54   87.83   91.59   84.19
 244          QI   82.93    81.6   80.75   79.42   82.41   77.68   75.61   74.44   74.51   72.29
 245          QI    79.3   70.93   70.15   69.48   68.44   67.58   66.72   72.25   65.21   64.67
 246          QI    63.5   62.83    62.6   62.06   61.31   64.86   60.25   59.05   58.48   55.87
 247          QI   56.01   55.72    54.8   54.06   53.38   51.64   51.71   50.75   49.83    49.2
 248          QI   48.75   46.82   45.85   45.55    47.8   43.99   43.77   43.28   42.64   42.31
 249          QI   42.04   41.65   41.41   40.69    40.3   40.09   39.93   39.53   42.18   38.64
 250          QI   37.99   37.44   37.13   36.96    36.8   36.19   35.56   35.28   34.71    33.9
 251          QI   33.42   33.14   32.73   39.25   32.39   31.56   30.95   30.66    30.3      30
 252          QI   29.22   28.72   29.47   28.39   28.32   28.13   28.24   28.22   27.67   27.34
 253          QI   27.15   27.03   26.51   25.94   25.77   31.57   25.38   25.06   25.04   24.52
```

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          254           QI   24.31   24.43   24.27   23.35   23.93    23.1   22.66   22.71   22.57   22.79
          255           QI   22.09   21.91   21.94   21.84   21.71   21.66   21.87   21.85   21.36   21.06
          256           QI   20.95
                        *
1                                                        HEC-1 INPUT                                              PAGE  7

         LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


          257           KK  BINFLO
          258           KM         TOTAL INFLOW INTO BASIN 53R.
          259           HC       3
                        *

          260           KK   BASIN
          261           KM    BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
          262           KM    ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
          263           KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
          264           KM        THE OUTLET PIPES ARE INLET CONTROLLED.
          265           RS       1    STOR       0
          266           SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
          267           SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
          268           SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
          269           SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
          270           SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
          271           SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
                        *

          272           KK  BSNRT1
          273           KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
          274           KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
          275           KM        EXCESS CAPACITY.
          276           RK     550  0.0052   0.024            CIRC     7.0
                        *

          277           KK  BSNRT2
          278           KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BASIN 53R OUTFAL
          279           RK    1200  0.0077   0.013            CIRC     7.0
                        *

          280           KK  PIMABY
          281           KM    RETRIEVE PIMA BYPASS CHANNEL HYDROGRAPH
          282           DR  PIMABY
                        *

          283           KK  PMB-RT
          284           KM        UNION HILLS DR CHANNEL FROM SR 101L FREEWAY TO 82ND ST
          285           RK    3157  0.0082   0.013       0    TRAP      24       4
                        *

          286           KK    SB10   BASIN
          287           BA   0.040
          288           LG    0.15    0.25    4.25    0.55      55
          289           UC   0.230   0.159
          290           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          291           UA     100
                        *
1                                                        HEC-1 INPUT                                              PAGE  8

         LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


          292           KK   CP-10 COMBINE
          293           KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
          294           HC       3
                        *

          295           KK  R10-11   ROUTE
          296           KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
          297           RK    1277  0.0014   0.03             TRAP     220       4
                        *

          298           KK    SB11   BASIN
          299           BA   0.071
          300           LG    0.15    0.25    4.15    0.58      55
          301           UC   0.292   0.229
          302           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          303           UA     100
                        *

          304           KK   CP-11 COMBINE
          305           KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
          306           HC       2
                        *
          307           ZZ
1
                 SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

   NO.      (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

    33      76THST
                 .
```

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              .
 74           .        SB01
              .           .
              .           .
 80        CP-1............
              V
              V
 83        R1-2
              .
              .
 86           .        SB02
              .           .
              .           .
 92        CP-2............
              V
              V
 95        R2-3
              .
              .
 98           .        SB03
              .           .
              .           .
104        CP-3............
              V
              V
107        R3-5
              .
              .
110           .        SB04
              .           V
              .           V
116           .        R4-5
              .           .
              .           .
119           .           .        SB05
              .           .           .
              .           .           .
125        CP-5........................
              V
              V
128        R5-6
              .
              .
131           .        SB06
              .           .
              .           .
137        CP-6............
              .
              .
140           .        SB07
              .           V
              .           V
146           .        R7-8
              .           .
              .           .
149           .           .        SB08
              .           .           .
              .           .           .
155           .        CP-8............
              .           V
              .           V
158           .        R8-9
              .           .
              .           .
161           .           .        SB09
              .           .           .
              .           .           .
167           .        CP-9............
              .           .
              .           .
170           .           .      PIMACH
              .           .           .
              .           .           .
213           .           .           .------->  PIMABY
211           .           .      PIMAIN
              .           .           .
              .           .           .
216           .           .           .       PWRCH
              .           .           .           .
              .           .           .           .
257           .      BINFLO........................
              .           V
              .           V
260           .       BASIN
              .           V
              .           V
272           .      BSNRT1
              .           V
              .           V
277           .      BSNRT2
              .           .
              .           .
282           .           .           .<-------  PIMABY
280           .           .      PIMABY
```

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              .           .           V
              .           .           V
  283         .           .       PMB-RT
              .           .           .
              .           .           .
  286         .           .           .        SB10
              .           .           .           .
              .           .           .           .
  292         .       CP-10.......................
              .           V
              .           V
  295         .      R10-11
              .           .
              .           .
  298         .           .       SB11
              .           .           .
              .           .           .
  304         .       CP-11...........

 (***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1*****************************************
***************************************
 *                                       *                                                   *
  *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                   *    U.S. ARMY CORPS OF ENGINEERS
  *
 *             JUN   1998                *                                                   *    HYDROLOGIC ENGINEERING CENTER
  *
 *           VERSION 4.1                 *                                                   *         609 SECOND STREET
  *
 *                                       *                                                   *       DAVIS, CALIFORNIA 95616
  *
 *  RUN DATE   12SEP14  TIME  11:47:04   *                                                   *           (916) 756-1104
  *
 *                                       *                                                   *
  *
 *****************************************
***************************************



                                CROSS ROADS EAST DRAINAGE ALTERNATIVES
                                RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                                INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                                  100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                                   PIMA ROAD FLOWS)
                                PHASE 2 BASIN - SIDESLOPES STEEPENED FROM ~5:1 TO 3:1

                                PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                                  MODELERS: RK, MW


   12 IO          OUTPUT CONTROL VARIABLES
                        IPRNT           3  PRINT CONTROL
                        IPLOT           0  PLOT CONTROL
                        QSCAL          0.  HYDROGRAPH PLOT SCALE

      IT          HYDROGRAPH TIME DATA
                         NMIN           3  MINUTES IN COMPUTATION INTERVAL
                        IDATE      1    0  STARTING DATE
                        ITIME        0000  STARTING TIME
                           NQ        1000  NUMBER OF HYDROGRAPH ORDINATES
                       NDDATE      3    0  ENDING DATE
                       NDTIME        0157  ENDING TIME
                       ICENT           19  CENTURY MARK

                    COMPUTATION INTERVAL    0.05 HOURS
                         TOTAL TIME BASE   49.95 HOURS

          ENGLISH UNITS
               DRAINAGE AREA         SQUARE MILES
               PRECIPITATION DEPTH   INCHES
               LENGTH, ELEVATION     FEET
               FLOW                  CUBIC FEET PER SECOND
               STORAGE VOLUME        ACRE-FEET
               SURFACE AREA          ACRES
               TEMPERATURE           DEGREES FAHRENHEIT

   13 JD          INDEX STORM NO. 1
                         STRM        2.76  PRECIPITATION DEPTH
                         TRDA        0.00  TRANSPOSITION DRAINAGE AREA

   14 PI            PRECIPITATION PATTERN
                      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                      0.01      0.02      0.02      0.08      0.16      0.46      0.08      0.02      0.02      0.01
                      0.01      0.01      0.01      0.01

   17 JD          INDEX STORM NO. 2
                         STRM        2.74  PRECIPITATION DEPTH
                         TRDA        0.50  TRANSPOSITION DRAINAGE AREA

   18 PI            PRECIPITATION PATTERN
```

```
                 0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01
                 0.01     0.02     0.02     0.08     0.16     0.46     0.08     0.02     0.02     0.01
                 0.01     0.01     0.01     0.01

  21 JD        INDEX STORM NO. 3
                   STRM       2.69  PRECIPITATION DEPTH
                   TRDA       2.80  TRANSPOSITION DRAINAGE AREA

  22 PI          PRECIPITATION PATTERN
                 0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01
                 0.01     0.02     0.04     0.09     0.20     0.24     0.14     0.06     0.04     0.01
                 0.01     0.01     0.01     0.01

  25 JD        INDEX STORM NO. 4
                   STRM       2.54  PRECIPITATION DEPTH
                   TRDA      16.00  TRANSPOSITION DRAINAGE AREA

  26 PI          PRECIPITATION PATTERN
                 0.01     0.00     0.01     0.02     0.02     0.01     0.01     0.01     0.01     0.02
                 0.02     0.02     0.05     0.08     0.17     0.20     0.13     0.07     0.04     0.03
                 0.01     0.01     0.01     0.01

  29 JD        INDEX STORM NO. 5
                   STRM       2.23  PRECIPITATION DEPTH
                   TRDA      90.00  TRANSPOSITION DRAINAGE AREA

  30 PI          PRECIPITATION PATTERN
                 0.02     0.01     0.02     0.02     0.02     0.02     0.02     0.02     0.02     0.02
                 0.02     0.03     0.05     0.08     0.14     0.16     0.12     0.07     0.05     0.04
                 0.02     0.02     0.02     0.02


 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
***

            **************
            *            *
  33 KK     *   76THST   *
            *            *
            **************
                      76TH STREET CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
                      100-YR, 6-HR FLO-2D MODEL (XS 98)

             SUBBASIN RUNOFF DATA

  36 BA        SUBBASIN CHARACTERISTICS
                  TAREA       0.24  SUBBASIN AREA

                                                                     ***

      ***             ***             ***             ***             ***

                   HYDROGRAPH AT STATION   76THST
                  TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                 (CFS)
+      51.      4.55                  15.          4.          2.            2.
                  (INCHES)      0.582       0.664       0.728         0.728
                   (AC-FT)         7.          9.          9.            9.

                   CUMULATIVE AREA =    0.24 SQ MI


      ***             ***             ***             ***             ***

                   HYDROGRAPH AT STATION   76THST
                  TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                 (CFS)
+      51.      4.55                  15.          4.          2.            2.
                  (INCHES)      0.582       0.664       0.728         0.728
                   (AC-FT)         7.          9.          9.            9.

                   CUMULATIVE AREA =    0.24 SQ MI


      ***             ***             ***             ***             ***

                   HYDROGRAPH AT STATION   76THST
                  TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                 (CFS)
+      51.      4.55                  15.          4.          2.            2.
```

```
                    (INCHES)     0.582       0.664       0.728        0.728
                     (AC-FT)        7.          9.          9.           9.

                     CUMULATIVE AREA =    0.24 SQ MI


        ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION   76THST
                    TRANSPOSITION AREA     16.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                       (CFS)
 +      51.      4.55                 15.          4.          2.           2.
                    (INCHES)     0.582       0.664       0.728        0.728
                     (AC-FT)        7.          9.          9.           9.

                     CUMULATIVE AREA =    0.24 SQ MI


        ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION   76THST
                    TRANSPOSITION AREA     90.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                       (CFS)
 +      51.      4.55                 15.          4.          2.           2.
                    (INCHES)     0.582       0.664       0.728        0.728
                     (AC-FT)        7.          9.          9.           9.

                     CUMULATIVE AREA =    0.24 SQ MI

        ***             ***             ***             ***             ***



                     INTERPOLATED HYDROGRAPH AT   76THST

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                       (CFS)
 +      51.      4.55                 15.          4.          2.           2.
                    (INCHES)     0.582       0.664       0.728        0.728
                     (AC-FT)        7.          9.          9.           9.

                     CUMULATIVE AREA =    0.24 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
***


             **************
             *            *
   74 KK     *      SB01  *        BASIN
             *            *
             **************

                 SUBBASIN RUNOFF DATA

   75 BA           SUBBASIN CHARACTERISTICS
                         TAREA        0.11  SUBBASIN AREA

   76 LG           GREEN AND AMPT LOSS RATE
                         STRTL        0.25  STARTING LOSS
                           DTH        0.25  MOISTURE DEFICIT
                          PSIF        4.10  WETTING FRONT SUCTION
                         XKSAT        0.55  HYDRAULIC CONDUCTIVITY
                         RTIMP       45.00  PERCENT IMPERVIOUS AREA

   77 UC           CLARK UNITGRAPH
                            TC        0.32  TIME OF CONCENTRATION
                             R        0.25  STORAGE COEFFICIENT

   78 UA           ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                         0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                       100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.32 HR,       R=  0.25 HR
                                                       SNYDER   TP=  0.19 HR,      CP=  0.47

                                                                   UNIT HYDROGRAPH
                                                              30 END-OF-PERIOD ORDINATES
```

```
             15.       58.      129.      172.      167.      153.      134.      112.       91.       75.
             61.       50.       41.       34.       28.       23.       19.       15.       12.       10.
              8.        7.        6.        5.        4.        3.        3.        2.        2.        1.

     ***            ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB01
                            TRANSPOSITION AREA      0.0 SQ MI

   TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.59, TOTAL EXCESS =    2.17

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     326.      0.95                  26.          7.          3.           3.
                            (INCHES)     2.159       2.159       2.159        2.159
                             (AC-FT)      13.         13.         13.          13.

                             CUMULATIVE AREA =    0.11 SQ MI


     ***            ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB01
                            TRANSPOSITION AREA      0.5 SQ MI

   TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.59, TOTAL EXCESS =    2.15

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     323.      0.95                  26.          6.          3.           3.
                            (INCHES)     2.143       2.143       2.143        2.143
                             (AC-FT)      13.         13.         13.          13.

                             CUMULATIVE AREA =    0.11 SQ MI


     ***            ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB01
                            TRANSPOSITION AREA      2.8 SQ MI

   TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.63, TOTAL EXCESS =    2.05

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     287.      1.00                  25.          6.          3.           3.
                            (INCHES)     2.044       2.044       2.044        2.044
                             (AC-FT)      12.         12.         12.          12.

                             CUMULATIVE AREA =    0.11 SQ MI


     ***            ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB01
                            TRANSPOSITION AREA     16.0 SQ MI

   TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.68, TOTAL EXCESS =    1.86

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     241.      1.00                  22.          6.          3.           3.
                            (INCHES)     1.850       1.850       1.850        1.850
                             (AC-FT)      11.         11.         11.          11.

                             CUMULATIVE AREA =    0.11 SQ MI


     ***            ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB01
                            TRANSPOSITION AREA     90.0 SQ MI

   TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.72, TOTAL EXCESS =    1.52

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     182.      1.00                  18.          5.          2.           2.
                            (INCHES)     1.510       1.510       1.510        1.510
                             (AC-FT)       9.          9.          9.           9.

                             CUMULATIVE AREA =    0.11 SQ MI

     ***            ***             ***             ***             ***
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                         INTERPOLATED HYDROGRAPH AT     SB01

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     324.      0.95                      26.          6.          3.           3.
                            (INCHES)     2.146       2.146       2.146        2.146
                             (AC-FT)       13.         13.         13.          13.

                         CUMULATIVE AREA =    0.11 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             *            *
   80 KK     *      CP-1  *      COMBINE
             *            *
             **************
                                 LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)

   82 HC          HYDROGRAPH COMBINATION
                        ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                          ***

       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-1
                        TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     326.      0.95                      37.         11.          5.           5.
                            (INCHES)     0.987       1.133       1.183        1.183
                             (AC-FT)       19.         21.         22.          22.

                         CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-1
                        TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     323.      0.95                      37.         11.          5.           5.
                            (INCHES)     0.982       1.128       1.178        1.178
                             (AC-FT)       18.         21.         22.          22.

                         CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-1
                        TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     287.      1.00                      36.         10.          5.           5.
                            (INCHES)     0.951       1.096       1.147        1.147
                             (AC-FT)       18.         21.         22.          22.

                         CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-1
                        TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     241.      1.00                      34.         10.          5.           5.
                            (INCHES)     0.886       1.035       1.085        1.085
                             (AC-FT)       17.         19.         20.          20.
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                      CUMULATIVE AREA =    0.35 SQ MI


      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION     CP-1
                     TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     182.      1.00                      29.          9.          4.           4.
                     (INCHES)     0.776       0.927       0.977        0.977
                      (AC-FT)       15.         17.         18.          18.

                      CUMULATIVE AREA =    0.35 SQ MI

      ***             ***             ***             ***             ***



                      INTERPOLATED HYDROGRAPH AT     CP-1

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     323.      0.95                      37.         11.          5.           5.
                     (INCHES)     0.982       1.128       1.178        1.178
                      (AC-FT)       18.         21.         22.          22.

                      CUMULATIVE AREA =    0.35 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             *            *
   83 KK     *      R1-2  *        ROUTE
             *            *
             **************
                                   MILLER RD CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY

                HYDROGRAPH ROUTING DATA

   85 RK          KINEMATIC WAVE STREAM ROUTING
                            L       2104.  CHANNEL LENGTH
                            S      0.0015  SLOPE
                            N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                           CA        0.00  CONTRIBUTING AREA
                        SHAPE        TRAP  CHANNEL SHAPE
                           WD       66.00  BOTTOM WIDTH OR DIAMETER
                            Z        4.00  SIDE SLOPE
                       NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                         ***
                                          COMPUTED KINEMATIC PARAMETERS
                                                VARIABLE TIME STEP
                                              (DT SHOWN IS A MINIMUM)

                          ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                          PEAK                CELERITY
                                                          (MIN)      (FT)     (CFS)     (MIN)      (IN)      (FPS)

                         MAIN          0.17      1.56       3.00    701.33    324.42     63.63      1.19      3.89


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2221E+02  EXCESS=0.0000E+00 OUTFLOW=0.2234E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=
-0.8


                                                  INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                         MAIN          0.17      1.56       3.00              317.42     66.00      1.19

      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION     R1-2
                     TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     317.      1.10                      38.         11.          5.           5.
                     (INCHES)     0.995       1.140       1.189        1.189
                      (AC-FT)       19.         21.         22.          22.
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                    CUMULATIVE AREA =    0.35 SQ MI

                                 COMPUTED KINEMATIC PARAMETERS
                                       VARIABLE TIME STEP
                                     (DT SHOWN IS A MINIMUM)

              ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                            PEAK                CELERITY
                                              (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

             MAIN          0.17      1.56      3.00    701.33    321.94     63.70      1.18      3.88

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2212E+02  EXCESS=0.0000E+00 OUTFLOW=0.2223E+02 BASIN STORAGE=0.5936E-01 PERCENT ERROR=
-0.8

                                    INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

             MAIN          0.17      1.56      3.00              314.95     66.00      1.18

      ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     R1-2
                   TRANSPOSITION AREA      0.5 SQ MI

   PEAK FLOW     TIME                      MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     315.      1.10              37.         11.         5.          5.
                      (INCHES)   0.988       1.134       1.184       1.184
                       (AC-FT)    19.         21.         22.         22.

                    CUMULATIVE AREA =    0.35 SQ MI

                                 COMPUTED KINEMATIC PARAMETERS
                                       VARIABLE TIME STEP
                                     (DT SHOWN IS A MINIMUM)

              ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                            PEAK                CELERITY
                                              (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

             MAIN          0.17      1.56      3.00    701.33    281.88     67.46      1.15      3.72

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2153E+02  EXCESS=0.0000E+00 OUTFLOW=0.2160E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=
-0.6

                                    INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

             MAIN          0.17      1.56      3.00              280.31     66.00      1.15

      ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     R1-2
                   TRANSPOSITION AREA      2.8 SQ MI

   PEAK FLOW     TIME                      MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     280.      1.10              36.         10.         5.          5.
                      (INCHES)   0.955       1.100       1.149       1.149
                       (AC-FT)    18.         21.         22.         22.

                    CUMULATIVE AREA =    0.35 SQ MI

                                 COMPUTED KINEMATIC PARAMETERS
                                       VARIABLE TIME STEP
                                     (DT SHOWN IS A MINIMUM)

              ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                            PEAK                CELERITY
                                              (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

             MAIN          0.17      1.56      3.00    701.33    239.43     66.99      1.09      3.49

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2036E+02  EXCESS=0.0000E+00 OUTFLOW=0.2044E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=
-0.7

                                    INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

             MAIN          0.17      1.56      3.00              234.05     69.00      1.09

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                       HYDROGRAPH AT STATION     R1-2
                     TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     234.      1.15                    34.         10.          5.           5.
                     (INCHES)     0.887       1.038       1.087        1.087
                      (AC-FT)       17.         19.         20.          20.

                      CUMULATIVE AREA =    0.35 SQ MI

                                     COMPUTED KINEMATIC PARAMETERS
                                           VARIABLE TIME STEP
                                         (DT SHOWN IS A MINIMUM)

                  ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                 PEAK                 CELERITY
                                                 (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                 MAIN          0.17      1.56       2.91     526.00    179.26     70.19      0.98      3.16


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1834E+02  EXCESS=0.0000E+00 OUTFLOW=0.1842E+02 BASIN STORAGE=0.6416E-01 PERCENT ERROR=
-0.8


                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                 MAIN          0.17      1.56       3.00               178.38     69.00      0.98

       ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION     R1-2
                     TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     178.      1.15                    30.          9.          4.           4.
                     (INCHES)     0.780       0.932       0.981        0.981
                      (AC-FT)       15.         17.         18.          18.

                      CUMULATIVE AREA =    0.35 SQ MI

       ***             ***             ***             ***             ***



                      INTERPOLATED HYDROGRAPH AT     R1-2

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     315.      1.10                    37.         11.          5.           5.
                     (INCHES)     0.989       1.134       1.184        1.184
                      (AC-FT)       19.         21.         22.          22.

                      CUMULATIVE AREA =    0.35 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             **************
             *            *
   86 KK     *      SB02  *        BASIN
             *            *
             **************

                SUBBASIN RUNOFF DATA

   87 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.16  SUBBASIN AREA

   88 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.15  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.15  WETTING FRONT SUCTION
                        XKSAT        0.58  HYDRAULIC CONDUCTIVITY
                        RTIMP       56.00  PERCENT IMPERVIOUS AREA

   89 UC          CLARK UNITGRAPH
                           TC        0.32  TIME OF CONCENTRATION
                            R        0.24  STORAGE COEFFICIENT
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  90 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                          0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                        100.0

                                                                        ***

                                                            UNIT HYDROGRAPH PARAMETERS
                                                      CLARK   TC=  0.32 HR,       R=  0.24 HR
                                                     SNYDER   TP=  0.19 HR,      CP=  0.49

                                                                 UNIT HYDROGRAPH
                                                            28 END-OF-PERIOD ORDINATES
                     21.       81.      185.      250.      242.      220.      192.      160.      129.      104.
                     85.       68.       55.       45.       36.       29.       24.       19.       16.       13.
                     10.        8.        7.        5.        4.        4.        3.        2.

        ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION     SB02
                           TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.45, TOTAL EXCESS =    2.30

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                              (CFS)
 +     492.      0.95                   39.         10.          5.           5.
                           (INCHES)    2.292       2.292       2.292        2.292
                            (AC-FT)      19.         19.         19.          19.

                            CUMULATIVE AREA =    0.16 SQ MI


        ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION     SB02
                           TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.45, TOTAL EXCESS =    2.29

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                              (CFS)
 +     488.      0.95                   38.         10.          5.           5.
                           (INCHES)    2.276       2.276       2.276        2.276
                            (AC-FT)      19.         19.         19.          19.

                            CUMULATIVE AREA =    0.16 SQ MI


        ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION     SB02
                           TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.49, TOTAL EXCESS =    2.19

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                              (CFS)
 +     433.      1.00                   37.          9.          4.           4.
                           (INCHES)    2.183       2.183       2.183        2.183
                            (AC-FT)      18.         18.         18.          18.

                            CUMULATIVE AREA =    0.16 SQ MI


        ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION     SB02
                           TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.54, TOTAL EXCESS =    2.00

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                              (CFS)
 +     365.      1.00                   34.          8.          4.           4.
                           (INCHES)    1.994       1.994       1.994        1.994
                            (AC-FT)      17.         17.         17.          17.

                            CUMULATIVE AREA =    0.16 SQ MI


        ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION     SB02
                           TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.56, TOTAL EXCESS =    1.67
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  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     278.      1.00                   28.          7.          3.           3.
                           (INCHES)     1.660       1.660       1.660        1.660
                            (AC-FT)       14.         14.         14.          14.

                            CUMULATIVE AREA =    0.16 SQ MI

       ***            ***            ***            ***            ***


                            INTERPOLATED HYDROGRAPH AT     SB02

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     489.      0.95                   38.         10.          5.           5.
                           (INCHES)     2.278       2.278       2.278        2.278
                            (AC-FT)       19.         19.         19.          19.

                            CUMULATIVE AREA =    0.16 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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           **************
           *            *
   92 KK   *     CP-2   *      COMBINE
           *            *
           **************
                                   SR 101L FREEWAY AND 76TH ST (MILLER RD CHANNEL)

   94 HC         HYDROGRAPH COMBINATION
                           ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                  ***

       ***            ***            ***            ***            ***

                             HYDROGRAPH AT STATION     CP-2
                           TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     740.      1.05                   75.         20.         10.          10.
                           (INCHES)     1.372       1.495       1.529        1.529
                            (AC-FT)       37.         41.         42.          42.

                            CUMULATIVE AREA =    0.51 SQ MI


       ***            ***            ***            ***            ***

                             HYDROGRAPH AT STATION     CP-2
                           TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     732.      1.05                   75.         20.         10.          10.
                           (INCHES)     1.363       1.486       1.521        1.521
                            (AC-FT)       37.         40.         41.          41.

                            CUMULATIVE AREA =    0.51 SQ MI


       ***            ***            ***            ***            ***

                             HYDROGRAPH AT STATION     CP-2
                           TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     654.      1.05                   72.         20.         10.          10.
                           (INCHES)     1.311       1.433       1.468        1.468
                            (AC-FT)       36.         39.         40.          40.

                            CUMULATIVE AREA =    0.51 SQ MI
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      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-2
                       TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     556.      1.10                      66.         18.          9.            9.
                       (INCHES)       1.207       1.332       1.367         1.367
                        (AC-FT)         33.         36.         37.           37.

                        CUMULATIVE AREA =    0.51 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-2
                       TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     422.      1.10                      56.         16.          8.            8.
                       (INCHES)       1.028       1.156       1.190         1.190
                        (AC-FT)         28.         31.         32.           32.

                        CUMULATIVE AREA =    0.51 SQ MI

      ***             ***             ***             ***             ***



                        INTERPOLATED HYDROGRAPH AT     CP-2

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     731.      1.05                      75.         20.         10.           10.
                       (INCHES)       1.362       1.485       1.520         1.520
                        (AC-FT)         37.         40.         41.           41.

                        CUMULATIVE AREA =    0.51 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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           **************
           *            *
   95 KK   *      R2-3  *        ROUTE
           *            *
           **************
                             MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD

              HYDROGRAPH ROUTING DATA

   97 RK        KINEMATIC WAVE STREAM ROUTING
                          L        1260.  CHANNEL LENGTH
                          S      0.0015  SLOPE
                          N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                         CA        0.00  CONTRIBUTING AREA
                      SHAPE        TRAP  CHANNEL SHAPE
                         WD       92.00  BOTTOM WIDTH OR DIAMETER
                          Z        4.00  SIDE SLOPE
                     NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                               ***
                                  COMPUTED KINEMATIC PARAMETERS
                                        VARIABLE TIME STEP
                                      (DT SHOWN IS A MINIMUM)

                    ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                CELERITY
                                                   (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                   MAIN         0.13      1.58       1.61     420.00    726.00     65.69      1.53      4.85


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4152E+02  EXCESS=0.0000E+00 OUTFLOW=0.4152E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.1

                                     INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                   MAIN         0.13      1.58       3.00               725.57     66.00      1.53
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      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     R2-3
                        TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     726.      1.10                  75.         20.         10.          10.
                          (INCHES)     1.373       1.495       1.529        1.529
                           (AC-FT)      37.         41.         42.          42.

                           CUMULATIVE AREA =    0.51 SQ MI

                                             COMPUTED KINEMATIC PARAMETERS
                                                   VARIABLE TIME STEP
                                               (DT SHOWN IS A MINIMUM)

                     ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO      VOLUME    MAXIMUM
                                                                                     PEAK                 CELERITY
                                                      (MIN)      (FT)     (CFS)     (MIN)        (IN)      (FPS)

                    MAIN          0.13      1.58       1.61     420.00    719.73     65.75       1.52      4.83

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4128E+02  EXCESS=0.0000E+00 OUTFLOW=0.4128E+02 BASIN STORAGE=0.4197E-01 PERCENT ERROR=
-0.1

                                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                    MAIN          0.13      1.58       3.00               719.25     66.00       1.52

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     R2-3
                        TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     719.      1.10                  75.         20.         10.          10.
                          (INCHES)     1.364       1.486       1.521        1.521
                           (AC-FT)      37.         40.         41.          41.

                           CUMULATIVE AREA =    0.51 SQ MI

                                             COMPUTED KINEMATIC PARAMETERS
                                                   VARIABLE TIME STEP
                                               (DT SHOWN IS A MINIMUM)

                     ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO      VOLUME    MAXIMUM
                                                                                     PEAK                 CELERITY
                                                      (MIN)      (FT)     (CFS)     (MIN)        (IN)      (FPS)

                    MAIN          0.13      1.58       1.65     420.00    650.44     66.16       1.47      4.64

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3985E+02  EXCESS=0.0000E+00 OUTFLOW=0.3986E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.1

                                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                    MAIN          0.13      1.58       3.00               647.31     66.00       1.47

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     R2-3
                        TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     647.      1.10                  72.         20.         10.          10.
                          (INCHES)     1.313       1.435       1.469        1.469
                           (AC-FT)      36.         39.         40.          40.

                           CUMULATIVE AREA =    0.51 SQ MI

                                             COMPUTED KINEMATIC PARAMETERS
                                                   VARIABLE TIME STEP
                                               (DT SHOWN IS A MINIMUM)

                     ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO      VOLUME    MAXIMUM
                                                                                     PEAK                 CELERITY
                                                      (MIN)      (FT)     (CFS)     (MIN)        (IN)      (FPS)

                    MAIN          0.13      1.58       1.61     420.00    553.64     68.36       1.37      4.37
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 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3711E+02  EXCESS=0.0000E+00 OUTFLOW=0.3710E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.1

                                             INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                 MAIN          0.13      1.58       3.00              550.82     69.00      1.37

      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     R2-3
                       TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     551.      1.15                   66.         18.         9.          9.
                         (INCHES)    1.206       1.332       1.366       1.366
                          (AC-FT)      33.         36.        37.         37.

                          CUMULATIVE AREA =    0.51 SQ MI

                                          COMPUTED KINEMATIC PARAMETERS
                                                VARIABLE TIME STEP
                                              (DT SHOWN IS A MINIMUM)

                  ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                               PEAK                 CELERITY
                                                  (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

                 MAIN          0.13      1.58       1.77    420.00    421.93     69.46      1.19      3.95

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3232E+02  EXCESS=0.0000E+00 OUTFLOW=0.3234E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.2

                                             INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                 MAIN          0.13      1.58       3.00              418.11     69.00      1.19

      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     R2-3
                       TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     418.      1.15                   56.         16.         8.          8.
                         (INCHES)    1.029       1.157       1.192       1.192
                          (AC-FT)      28.         31.        32.         32.

                          CUMULATIVE AREA =    0.51 SQ MI

      ***             ***             ***             ***             ***


                          INTERPOLATED HYDROGRAPH AT     R2-3

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     719.      1.10                   75.         20.        10.         10.
                         (INCHES)    1.363       1.486       1.520       1.520
                          (AC-FT)      37.         40.        41.         41.

                          CUMULATIVE AREA =    0.51 SQ MI



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             *            *
   98 KK     *      SB03  *        BASIN
             *            *
             **************

                SUBBASIN RUNOFF DATA

   99 BA          SUBBASIN CHARACTERISTICS
                       TAREA        0.05  SUBBASIN AREA
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  100 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.15  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.50  WETTING FRONT SUCTION
                        XKSAT        0.47  HYDRAULIC CONDUCTIVITY
                        RTIMP       55.00  PERCENT IMPERVIOUS AREA

  101 UC          CLARK UNITGRAPH
                           TC        0.36  TIME OF CONCENTRATION
                            R        0.29  STORAGE COEFFICIENT

  102 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.36 HR,       R=  0.29 HR
                                                       SNYDER   TP=  0.21 HR,      CP=  0.43

                                                                   UNIT HYDROGRAPH
                                                              34 END-OF-PERIOD ORDINATES
                   5.        17.       43.       62.       64.       61.       56.       49.       42.       35.
                  30.        25.       21.       18.       15.       12.       10.        9.        7.        6.
                   5.         4.        4.        3.        3.        2.        2.        2.        1.        1.
                   1.         1.        1.        1.

       ***              ***              ***              ***              ***

                                HYDROGRAPH AT STATION     SB03
                              TRANSPOSITION AREA      0.0 SQ MI

    TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.44, TOTAL EXCESS =    2.31

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     130.      1.00                   12.          3.          1.           1.
                            (INCHES)    2.302       2.302       2.302        2.302
                             (AC-FT)       6.          6.          6.           6.

                             CUMULATIVE AREA =    0.05 SQ MI


       ***              ***              ***              ***              ***

                                HYDROGRAPH AT STATION     SB03
                              TRANSPOSITION AREA      0.5 SQ MI

    TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.44, TOTAL EXCESS =    2.30

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     129.      1.00                   12.          3.          1.           1.
                            (INCHES)    2.286       2.286       2.286        2.286
                             (AC-FT)       6.          6.          6.           6.

                             CUMULATIVE AREA =    0.05 SQ MI


       ***              ***              ***              ***              ***

                                HYDROGRAPH AT STATION     SB03
                              TRANSPOSITION AREA      2.8 SQ MI

    TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.48, TOTAL EXCESS =    2.21

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     117.      1.00                   11.          3.          1.           1.
                            (INCHES)    2.198       2.198       2.198        2.198
                             (AC-FT)       6.          6.          6.           6.

                             CUMULATIVE AREA =    0.05 SQ MI


       ***              ***              ***              ***              ***

                                HYDROGRAPH AT STATION     SB03
                              TRANSPOSITION AREA     16.0 SQ MI

    TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.52, TOTAL EXCESS =    2.02

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
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+      99.      1.05                          10.           3.           1.            1.
                        (INCHES)     2.014        2.014        2.014         2.014
                         (AC-FT)        5.           5.           5.            5.

                         CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB03
                        TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.54, TOTAL EXCESS =    1.69

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+      77.      1.05                           9.           2.           1.            1.
                        (INCHES)     1.679        1.679        1.679         1.679
                         (AC-FT)        4.           4.           4.            4.

                         CUMULATIVE AREA =    0.05 SQ MI

       ***             ***             ***             ***             ***



                         INTERPOLATED HYDROGRAPH AT     SB03

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     129.      1.00                          12.           3.           1.            1.
                        (INCHES)     2.290        2.290        2.290         2.290
                         (AC-FT)        6.           6.           6.            6.

                         CUMULATIVE AREA =    0.05 SQ MI



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             *            *
   104 KK    *      CP-3  *      COMBINE
             *            *
             **************
                                 MAYO BLVD AND 76TH ST (MILLER RD CHANNEL)

   106 HC          HYDROGRAPH COMBINATION
                        ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                          ***

       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-3
                        TRANSPOSITION AREA      0.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     840.      1.10                          87.          23.          11.           11.
                        (INCHES)     1.449        1.564        1.596         1.596
                         (AC-FT)       43.          46.          47.           47.

                         CUMULATIVE AREA =    0.56 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-3
                        TRANSPOSITION AREA      0.5 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     833.      1.10                          86.          23.          11.           11.
                        (INCHES)     1.439        1.555        1.587         1.587
                         (AC-FT)       43.          46.          47.           47.

                         CUMULATIVE AREA =    0.56 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-3
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                         TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     755.      1.10                  83.         22.         11.          11.
                         (INCHES)    1.385       1.500       1.532        1.532
                          (AC-FT)     41.         45.         46.          46.

                          CUMULATIVE AREA =    0.56 SQ MI


      ***            ***             ***             ***             ***

                           HYDROGRAPH AT STATION     CP-3
                         TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     638.      1.15                  76.         21.         10.          10.
                         (INCHES)    1.271       1.390       1.422        1.422
                          (AC-FT)     38.         41.         42.          42.

                          CUMULATIVE AREA =    0.56 SQ MI


      ***            ***             ***             ***             ***

                           HYDROGRAPH AT STATION     CP-3
                         TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     487.      1.15                  65.         18.          9.           9.
                         (INCHES)    1.080       1.202       1.234        1.234
                          (AC-FT)     32.         36.         37.          37.

                          CUMULATIVE AREA =    0.56 SQ MI

      ***            ***             ***             ***             ***



                          INTERPOLATED HYDROGRAPH AT     CP-3

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     828.      1.10                  86.         23.         11.          11.
                         (INCHES)    1.435       1.552       1.583        1.583
                          (AC-FT)     43.         46.         47.          47.

                          CUMULATIVE AREA =    0.56 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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           *            *
  107 KK   *     R3-5  *        ROUTE
           *            *
           **************
                                 MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD

              HYDROGRAPH ROUTING DATA

  109 RK         KINEMATIC WAVE STREAM ROUTING
                           L       2396.  CHANNEL LENGTH
                           S      0.0015  SLOPE
                           N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                          CA        0.00  CONTRIBUTING AREA
                       SHAPE        TRAP  CHANNEL SHAPE
                          WD       98.00  BOTTOM WIDTH OR DIAMETER
                           Z        4.00  SIDE SLOPE
                      NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                       ***
                                              COMPUTED KINEMATIC PARAMETERS
                                                    VARIABLE TIME STEP
                                                  (DT SHOWN IS A MINIMUM)

                        ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                       PEAK                CELERITY
                                                        (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)
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                 MAIN          0.12      1.58       2.78    798.67    817.36     73.14      1.60      5.01

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4741E+02  EXCESS=0.0000E+00 OUTFLOW=0.4763E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.6

                                                 INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                 MAIN          0.12      1.58       3.00              816.94     72.00      1.60

       ***            ***            ***            ***            ***

                    HYDROGRAPH AT STATION     R3-5
                  TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     817.      1.20                   87.         24.         12.          12.
                          (INCHES)    1.450       1.571       1.602        1.602
                           (AC-FT)     43.         47.         48.          48.

                           CUMULATIVE AREA =    0.56 SQ MI

                                             COMPUTED KINEMATIC PARAMETERS
                                                   VARIABLE TIME STEP
                                                 (DT SHOWN IS A MINIMUM)

                  ELEMENT    ALPHA       M          DT         DX        PEAK     TIME TO     VOLUME    MAXIMUM
                                                                                    PEAK                 CELERITY
                                                   (MIN)      (FT)      (CFS)      (MIN)       (IN)      (FPS)

                 MAIN          0.12      1.58       2.79    798.67    810.60     70.45      1.59      4.99

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4713E+02  EXCESS=0.0000E+00 OUTFLOW=0.4736E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.7

                                                 INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                 MAIN          0.12      1.58       3.00              810.54     72.00      1.59

       ***            ***            ***            ***            ***

                    HYDROGRAPH AT STATION     R3-5
                  TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     811.      1.20                   86.         23.         11.          11.
                          (INCHES)    1.440       1.562       1.593        1.593
                           (AC-FT)     43.         46.         47.          47.

                           CUMULATIVE AREA =    0.56 SQ MI

                                             COMPUTED KINEMATIC PARAMETERS
                                                   VARIABLE TIME STEP
                                                 (DT SHOWN IS A MINIMUM)

                  ELEMENT    ALPHA       M          DT         DX        PEAK     TIME TO     VOLUME    MAXIMUM
                                                                                    PEAK                 CELERITY
                                                   (MIN)      (FT)      (CFS)      (MIN)       (IN)      (FPS)

                 MAIN          0.12      1.58       2.90    798.67    742.52     73.90      1.54      4.81

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4551E+02  EXCESS=0.0000E+00 OUTFLOW=0.4571E+02 BASIN STORAGE=0.8226E-01 PERCENT ERROR=
-0.6

                                                 INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                 MAIN          0.12      1.58       3.00              740.16     72.00      1.54

       ***            ***            ***            ***            ***

                    HYDROGRAPH AT STATION     R3-5
                  TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     740.      1.20                   83.         23.         11.          11.
                          (INCHES)    1.385       1.506       1.537        1.537
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                    (AC-FT)       41.          45.          46.           46.

                    CUMULATIVE AREA =    0.56 SQ MI

                                     COMPUTED KINEMATIC PARAMETERS
                                           VARIABLE TIME STEP
                                         (DT SHOWN IS A MINIMUM)

                 ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                              PEAK                 CELERITY
                                                 (MIN)     (FT)     (CFS)    (MIN)       (IN)       (FPS)

                MAIN          0.12      1.58       3.00    798.67    634.55     73.70      1.43      4.52

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4224E+02  EXCESS=0.0000E+00 OUTFLOW=0.4240E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.6

                                          INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                MAIN          0.12      1.58       3.00              626.49     75.00      1.43

       ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     R3-5
                   TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+     626.      1.25                  76.         21.         10.          10.
                   (INCHES)       1.271       1.396       1.427        1.427
                    (AC-FT)         38.         41.         42.          42.

                    CUMULATIVE AREA =    0.56 SQ MI

                                     COMPUTED KINEMATIC PARAMETERS
                                           VARIABLE TIME STEP
                                         (DT SHOWN IS A MINIMUM)

                 ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                              PEAK                 CELERITY
                                                 (MIN)     (FT)     (CFS)    (MIN)       (IN)       (FPS)

                MAIN          0.12      1.58       3.00    798.67    478.66     74.64      1.24      4.10

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3665E+02  EXCESS=0.0000E+00 OUTFLOW=0.3688E+02 BASIN STORAGE=0.8233E-01 PERCENT ERROR=
-0.8

                                          INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                MAIN          0.12      1.58       3.00              478.05     75.00      1.24

       ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     R3-5
                   TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+     478.      1.25                  65.         18.          9.           9.
                   (INCHES)       1.082       1.210       1.242        1.242
                    (AC-FT)         32.         36.         37.          37.

                    CUMULATIVE AREA =    0.56 SQ MI

       ***             ***             ***             ***             ***

                    INTERPOLATED HYDROGRAPH AT     R3-5

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+     806.      1.20                  86.         23.         11.          11.
                   (INCHES)       1.437       1.558       1.589        1.589
                    (AC-FT)         43.         46.         47.          47.

                    CUMULATIVE AREA =    0.56 SQ MI


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            *            *
  110 KK    *     SB04   *       BASIN
            *            *
            **************

               SUBBASIN RUNOFF DATA

  111 BA         SUBBASIN CHARACTERISTICS
                       TAREA        0.14  SUBBASIN AREA

  112 LG         GREEN AND AMPT LOSS RATE
                       STRTL        0.14  STARTING LOSS
                         DTH        0.25  MOISTURE DEFICIT
                        PSIF        4.60  WETTING FRONT SUCTION
                       XKSAT        0.44  HYDRAULIC CONDUCTIVITY
                       RTIMP       61.00  PERCENT IMPERVIOUS AREA

  113 UC         CLARK UNITGRAPH
                          TC        0.30  TIME OF CONCENTRATION
                           R        0.20  STORAGE COEFFICIENT

  114 UA         ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                       0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                     100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.30 HR,       R=  0.20 HR
                                                       SNYDER   TP=  0.18 HR,      CP=  0.54

                                                                   UNIT HYDROGRAPH
                                                              24 END-OF-PERIOD ORDINATES
                 26.      107.      218.      267.      245.      215.      177.      137.      106.       82.
                 63.       49.       38.       29.       23.       18.       14.       11.        8.        6.
                  5.        4.        3.        2.

      ***             ***             ***             ***             ***

                                HYDROGRAPH AT STATION     SB04
                              TRANSPOSITION AREA      0.0 SQ MI

    TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.38, TOTAL EXCESS =    2.38

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     528.      0.95                       37.          9.          4.           4.
                              (INCHES)    2.369       2.369       2.369        2.369
                               (AC-FT)      18.         18.         18.          18.

                               CUMULATIVE AREA =    0.14 SQ MI


      ***             ***             ***             ***             ***

                                HYDROGRAPH AT STATION     SB04
                              TRANSPOSITION AREA      0.5 SQ MI

    TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.37, TOTAL EXCESS =    2.36

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     524.      0.95                       36.          9.          4.           4.
                              (INCHES)    2.353       2.353       2.353        2.353
                               (AC-FT)      18.         18.         18.          18.

                               CUMULATIVE AREA =    0.14 SQ MI


      ***             ***             ***             ***             ***

                                HYDROGRAPH AT STATION     SB04
                              TRANSPOSITION AREA      2.8 SQ MI

    TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.40, TOTAL EXCESS =    2.28

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     452.      0.95                       35.          9.          4.           4.
                              (INCHES)    2.272       2.272       2.272        2.272
                               (AC-FT)      17.         17.         17.          17.

                               CUMULATIVE AREA =    0.14 SQ MI
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      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB04
                       TRANSPOSITION AREA     16.0 SQ MI

    TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.44, TOTAL EXCESS =    2.10

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     382.      1.00                  32.          8.          4.           4.
                       (INCHES)     2.095       2.095       2.095        2.095
                        (AC-FT)       16.         16.         16.          16.

                        CUMULATIVE AREA =    0.14 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB04
                       TRANSPOSITION AREA     90.0 SQ MI

    TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.46, TOTAL EXCESS =    1.77

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     293.      1.00                  27.          7.          3.           3.
                       (INCHES)     1.763       1.763       1.763        1.763
                        (AC-FT)       14.         14.         14.          14.

                        CUMULATIVE AREA =    0.14 SQ MI

      ***             ***             ***             ***             ***



                        INTERPOLATED HYDROGRAPH AT     SB04

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     525.      0.95                  36.          9.          4.           4.
                       (INCHES)     2.356       2.356       2.356        2.356
                        (AC-FT)       18.         18.         18.          18.

                        CUMULATIVE AREA =    0.14 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             *            *
  116 KK     *      R4-5  *        ROUTE
             *            *
             **************
                                PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST

               HYDROGRAPH ROUTING DATA

  118 RK         KINEMATIC WAVE STREAM ROUTING
                           L       2005.  CHANNEL LENGTH
                           S      0.0013  SLOPE
                           N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                          CA        0.00  CONTRIBUTING AREA
                       SHAPE        TRAP  CHANNEL SHAPE
                          WD       39.00  BOTTOM WIDTH OR DIAMETER
                           Z        4.00  SIDE SLOPE
                      NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                        ***
                                      COMPUTED KINEMATIC PARAMETERS
                                            VARIABLE TIME STEP
                                          (DT SHOWN IS A MINIMUM)

                     ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO      VOLUME    MAXIMUM
                                                                                    PEAK                  CELERITY
                                                     (MIN)      (FT)     (CFS)     (MIN)        (IN)       (FPS)

                    MAIN         0.23      1.51       2.41     668.33    520.58     62.34       2.39       4.81


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1819E+02  EXCESS=0.0000E+00 OUTFLOW=0.1838E+02 BASIN STORAGE=0.3179E-04 PERCENT ERROR=
-1.0
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INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		513.38	63.00	2.40

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 513.	1.05		37.	9.	4.	4.
		(INCHES)	2.401	2.402	2.402	2.402
		(AC-FT)	18.	18.	18.	18.

CUMULATIVE AREA = 0.14 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.23	1.51	2.42	668.33	516.56	62.40	2.38	4.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1807E+02 EXCESS=0.0000E+00 OUTFLOW=0.1825E+02 BASIN STORAGE=0.4744E-04 PERCENT ERROR= -1.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		510.06	63.00	2.39

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 510.	1.05		37.	9.	4.	4.
		(INCHES)	2.386	2.386	2.386	2.386
		(AC-FT)	18.	18.	18.	18.

CUMULATIVE AREA = 0.14 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.23	1.51	2.55	668.33	451.64	62.77	2.29	4.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1745E+02 EXCESS=0.0000E+00 OUTFLOW=0.1762E+02 BASIN STORAGE=0.4583E-04 PERCENT ERROR= -1.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		451.16	63.00	2.30

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 451.	1.05		36.	9.	4.	4.
		(INCHES)	2.301	2.302	2.302	2.302
		(AC-FT)	18.	18.	18.	18.

CUMULATIVE AREA = 0.14 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.23	1.51	2.73	668.33	379.50	64.90	2.12	4.31

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1609E+02 EXCESS=0.0000E+00 OUTFLOW=0.1629E+02 BASIN STORAGE=0.4132E-04 PERCENT ERROR= -1.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		374.76	63.00	2.12

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 375.	1.05		33.	8.	4.	4.
		(INCHES)	2.115	2.116	2.116	2.116
		(AC-FT)	16.	16.	16.	16.

CUMULATIVE AREA = 0.14 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.23	1.51	2.87	668.33	291.80	65.24	1.78	3.94

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1354E+02 EXCESS=0.0000E+00 OUTFLOW=0.1370E+02 BASIN STORAGE=0.4337E-04 PERCENT ERROR= -1.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		288.81	66.00	1.78

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 289.	1.10		28.	7.	3.	3.
		(INCHES)	1.782	1.782	1.782	1.782
		(AC-FT)	14.	14.	14.	14.

CUMULATIVE AREA = 0.14 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT R4-5

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 511.	1.05		37.	9.	4.	4.
		(INCHES)	2.388	2.388	2.388	2.388
		(AC-FT)	18.	18.	18.	18.

CUMULATIVE AREA = 0.14 SQ MI

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              *            *
 119 KK       *     SB05   *        BASIN
              *            *
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              SUBBASIN RUNOFF DATA

 120 BA          SUBBASIN CHARACTERISTICS
                       TAREA        0.13  SUBBASIN AREA

 121 LG          GREEN AND AMPT LOSS RATE
                       STRTL        0.22  STARTING LOSS
                         DTH        0.25  MOISTURE DEFICIT
                        PSIF        4.50  WETTING FRONT SUCTION
                       XKSAT        0.44  HYDRAULIC CONDUCTIVITY
                       RTIMP       48.00  PERCENT IMPERVIOUS AREA

 122 UC          CLARK UNITGRAPH
                          TC        0.33  TIME OF CONCENTRATION
                           R        0.22  STORAGE COEFFICIENT

 123 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                       0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                     100.0

                                                                      ***

                                                            UNIT HYDROGRAPH PARAMETERS
                                                      CLARK   TC=  0.33 HR,       R=  0.22 HR
                                                     SNYDER   TP=  0.19 HR,      CP=  0.50

                                                                   UNIT HYDROGRAPH
                                                            27 END-OF-PERIOD ORDINATES
               18.       67.      154.      208.      200.      181.      156.      129.      103.       82.
               66.       53.       42.       34.       27.       21.       17.       14.       11.        9.
                7.        6.        4.        4.        3.        2.        2.

      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB05
                       TRANSPOSITION AREA      0.0 SQ MI

    TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.52, TOTAL EXCESS =    2.23

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     402.      0.95                  30.          8.          4.           4.
                        (INCHES)    2.223       2.223       2.223        2.223
                         (AC-FT)      15.         15.         15.          15.

                         CUMULATIVE AREA =    0.13 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB05
                       TRANSPOSITION AREA      0.5 SQ MI

    TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.52, TOTAL EXCESS =    2.22

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     399.      0.95                  30.          7.          4.           4.
                        (INCHES)    2.208       2.208       2.208        2.208
                         (AC-FT)      15.         15.         15.          15.

                         CUMULATIVE AREA =    0.13 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB05
                       TRANSPOSITION AREA      2.8 SQ MI

    TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.56, TOTAL EXCESS =    2.13

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     352.      1.00                  29.          7.          3.           3.
                        (INCHES)    2.118       2.118       2.118        2.118
                         (AC-FT)      14.         14.         14.          14.

                         CUMULATIVE AREA =    0.13 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB05
                       TRANSPOSITION AREA     16.0 SQ MI

    TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.60, TOTAL EXCESS =    1.94
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  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     296.      1.00                 26.          7.          3.           3.
                         (INCHES)     1.930       1.930       1.930        1.930
                          (AC-FT)      13.         13.         13.          13.

                          CUMULATIVE AREA =    0.13 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB05
                         TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.63, TOTAL EXCESS =    1.60

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     225.      1.00                 22.          5.          3.           3.
                         (INCHES)     1.593       1.593       1.593        1.593
                          (AC-FT)      11.         11.         11.          11.

                          CUMULATIVE AREA =    0.13 SQ MI

       ***             ***             ***             ***             ***



                          INTERPOLATED HYDROGRAPH AT     SB05

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     399.      0.95                 30.          7.          4.           4.
                         (INCHES)     2.210       2.210       2.210        2.210
                          (AC-FT)      15.         15.         15.          15.

                          CUMULATIVE AREA =    0.13 SQ MI



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             *            *
  125 KK     *      CP-5  *      COMBINE
             *            *
             **************
                                 PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)

  127 HC          HYDROGRAPH COMBINATION
                         ICOMP           3  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                          ***

       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     CP-5
                         TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+    1452.      1.15                153.         40.         20.          20.
                         (INCHES)     1.715       1.814       1.836        1.836
                          (AC-FT)      76.         80.         81.          81.

                          CUMULATIVE AREA =    0.83 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     CP-5
                         TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+    1440.      1.15                152.         40.         19.          19.
                         (INCHES)     1.703       1.803       1.824        1.824
                          (AC-FT)      75.         80.         80.          80.

                          CUMULATIVE AREA =    0.83 SQ MI
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      ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     CP-5
                            TRANSPOSITION AREA      2.8 SQ MI

   PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                               (CFS)
 +    1335.      1.15                     146.         39.         19.          19.
                            (INCHES)     1.638       1.737       1.759        1.759
                             (AC-FT)       72.         77.         78.          78.

                             CUMULATIVE AREA =    0.83 SQ MI


      ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     CP-5
                            TRANSPOSITION AREA     16.0 SQ MI

   PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                               (CFS)
 +    1121.      1.15                     133.         36.         17.          17.
                            (INCHES)     1.499       1.602       1.623        1.623
                             (AC-FT)       66.         71.         72.          72.

                             CUMULATIVE AREA =    0.83 SQ MI


      ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     CP-5
                            TRANSPOSITION AREA     90.0 SQ MI

   PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                               (CFS)
 +     844.      1.20                     112.         30.         15.          15.
                            (INCHES)     1.262       1.368       1.389        1.389
                             (AC-FT)       56.         60.         61.          61.

                             CUMULATIVE AREA =    0.83 SQ MI

      ***             ***             ***             ***             ***



                             INTERPOLATED HYDROGRAPH AT     CP-5

   PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                               (CFS)
 +    1410.      1.15                     150.         40.         19.          19.
                            (INCHES)     1.684       1.784       1.805        1.805
                             (AC-FT)       74.         79.         80.          80.

                             CUMULATIVE AREA =    0.83 SQ MI



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             *            *
   128 KK    *      R5-6  *        ROUTE
             *            *
             **************
                                     PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD

                HYDROGRAPH ROUTING DATA

   130 RK         KINEMATIC WAVE STREAM ROUTING
                             L       1550.  CHANNEL LENGTH
                             S      0.0015  SLOPE
                             N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                            CA        0.00  CONTRIBUTING AREA
                         SHAPE        TRAP  CHANNEL SHAPE
                            WD      188.00  BOTTOM WIDTH OR DIAMETER
                             Z        4.00  SIDE SLOPE
                        NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                          ***
                                            COMPUTED KINEMATIC PARAMETERS
                                                  VARIABLE TIME STEP
                                                (DT SHOWN IS A MINIMUM)
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                ELEMENT    ALPHA       M        DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                             PEAK                CELERITY
                                               (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

               MAIN          0.07      1.62      1.71    516.67   1430.15     73.26      1.84      5.13

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8096E+02  EXCESS=0.0000E+00 OUTFLOW=0.8101E+02 BASIN STORAGE=0.7278E-01 PERCENT ERROR=
-0.1

                                              INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

               MAIN          0.07      1.62      3.00             1428.13     72.00      1.84

      ***             ***            ***             ***             ***

                    HYDROGRAPH AT STATION     R5-6
                  TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+    1428.      1.20                    153.         40.         20.          20.
                          (INCHES)     1.715       1.815       1.836        1.836
                           (AC-FT)       76.         80.         81.          81.

                           CUMULATIVE AREA =    0.83 SQ MI

                                        COMPUTED KINEMATIC PARAMETERS
                                             VARIABLE TIME STEP
                                          (DT SHOWN IS A MINIMUM)

                ELEMENT    ALPHA       M        DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                             PEAK                CELERITY
                                               (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

               MAIN          0.07      1.62      1.72    516.67   1417.75     71.63      1.82      5.12

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8047E+02  EXCESS=0.0000E+00 OUTFLOW=0.8049E+02 BASIN STORAGE=0.7282E-01 PERCENT ERROR=
-0.1

                                              INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

               MAIN          0.07      1.62      3.00             1417.68     72.00      1.82

      ***             ***            ***             ***             ***

                    HYDROGRAPH AT STATION     R5-6
                  TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+    1418.      1.20                    151.         40.         19.          19.
                          (INCHES)     1.703       1.803       1.824        1.824
                           (AC-FT)       75.         80.         80.          80.

                           CUMULATIVE AREA =    0.83 SQ MI

                                        COMPUTED KINEMATIC PARAMETERS
                                             VARIABLE TIME STEP
                                          (DT SHOWN IS A MINIMUM)

                ELEMENT    ALPHA       M        DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                             PEAK                CELERITY
                                               (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

               MAIN          0.07      1.62      1.83    516.67   1320.58     72.30      1.76      4.97

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7757E+02  EXCESS=0.0000E+00 OUTFLOW=0.7762E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR=
-0.2

                                              INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

               MAIN          0.07      1.62      3.00             1310.29     72.00      1.76

      ***             ***            ***             ***             ***

                    HYDROGRAPH AT STATION     R5-6
                  TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
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+   (CFS)       (HR)
                              (CFS)
+    1310.      1.20                    146.          39.          19.           19.
                           (INCHES)     1.639        1.739        1.760         1.760
                            (AC-FT)       72.          77.          78.           78.

                            CUMULATIVE AREA =    0.83 SQ MI

                                           COMPUTED KINEMATIC PARAMETERS
                                                 VARIABLE TIME STEP
                                               (DT SHOWN IS A MINIMUM)

                       ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                       PEAK                CELERITY
                                                        (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                      MAIN          0.07      1.62       1.92     516.67   1120.07     73.97      1.62      4.65

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7160E+02  EXCESS=0.0000E+00 OUTFLOW=0.7164E+02 BASIN STORAGE=0.7274E-01 PERCENT ERROR=
-0.2

                                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                      MAIN          0.07      1.62       3.00              1117.54     75.00      1.62

       ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION     R5-6
                           TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                        6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+    1118.      1.25                    133.          36.          17.           17.
                           (INCHES)     1.500        1.603        1.624         1.624
                            (AC-FT)       66.          71.          72.           72.

                            CUMULATIVE AREA =    0.83 SQ MI

                                           COMPUTED KINEMATIC PARAMETERS
                                                 VARIABLE TIME STEP
                                               (DT SHOWN IS A MINIMUM)

                       ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                       PEAK                CELERITY
                                                        (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                      MAIN          0.07      1.62       2.16     516.67    837.38     77.35      1.39      4.17

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6127E+02  EXCESS=0.0000E+00 OUTFLOW=0.6143E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR=
-0.4

                                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                      MAIN          0.07      1.62       3.00               834.70     75.00      1.39

       ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION     R5-6
                           TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                        6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     835.      1.25                    112.          30.          15.           15.
                           (INCHES)     1.264        1.371        1.392         1.392
                            (AC-FT)       56.          60.          61.           61.

                            CUMULATIVE AREA =    0.83 SQ MI

       ***             ***             ***             ***             ***


                            INTERPOLATED HYDROGRAPH AT     R5-6

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                        6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+    1386.      1.20                    150.          40.          19.           19.
                           (INCHES)     1.684        1.785        1.806         1.806
                            (AC-FT)       74.          79.          80.           80.

                            CUMULATIVE AREA =    0.83 SQ MI
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 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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            *            *
  131 KK    *     SB06   *       BASIN
            *            *
            **************

               SUBBASIN RUNOFF DATA

  132 BA         SUBBASIN CHARACTERISTICS
                       TAREA        0.14  SUBBASIN AREA

  133 LG         GREEN AND AMPT LOSS RATE
                       STRTL        0.16  STARTING LOSS
                         DTH        0.25  MOISTURE DEFICIT
                        PSIF        4.55  WETTING FRONT SUCTION
                       XKSAT        0.45  HYDRAULIC CONDUCTIVITY
                       RTIMP       53.00  PERCENT IMPERVIOUS AREA

  134 UC         CLARK UNITGRAPH
                          TC        0.32  TIME OF CONCENTRATION
                           R        0.24  STORAGE COEFFICIENT

  135 UA         ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                       0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                     100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.32 HR,       R=  0.24 HR
                                                       SNYDER   TP=  0.19 HR,      CP=  0.48

                                                                   UNIT HYDROGRAPH
                                                              29 END-OF-PERIOD ORDINATES
                 18.       72.      161.      214.      206.      189.      164.      136.      111.       90.
                 73.       60.       49.       39.       32.       26.       21.       17.       14.       11.
                  9.        8.        6.        5.        4.        3.        3.        2.        2.

       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB06
                       TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.30

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     422.      0.95                   33.          8.          4.          4.
                       (INCHES)     2.285       2.285       2.285       2.285
                        (AC-FT)       17.         17.         17.         17.

                        CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB06
                       TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.28

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     419.      0.95                   33.          8.          4.          4.
                       (INCHES)     2.269       2.269       2.269       2.269
                        (AC-FT)       16.         16.         16.         16.

                        CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB06
                       TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.50, TOTAL EXCESS =    2.19

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     373.      1.00                   32.          8.          4.          4.
                       (INCHES)     2.181       2.181       2.181       2.181
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                    (AC-FT)       16.         16.         16.          16.

                     CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     SB06
                    TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.53, TOTAL EXCESS =    2.01

   PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                         (CFS)
 +     316.      1.00                  29.          7.          4.           4.
                    (INCHES)     1.997       1.997       1.997        1.997
                     (AC-FT)       14.         14.         14.          14.

                     CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     SB06
                    TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.56, TOTAL EXCESS =    1.67

   PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                         (CFS)
 +     241.      1.00                  24.          6.          3.           3.
                    (INCHES)     1.661       1.661       1.661        1.661
                     (AC-FT)       12.         12.         12.          12.

                     CUMULATIVE AREA =    0.14 SQ MI

       ***             ***             ***             ***             ***


                     INTERPOLATED HYDROGRAPH AT     SB06

   PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                         (CFS)
 +     420.      0.95                  33.          8.          4.           4.
                    (INCHES)     2.272       2.272       2.272        2.272
                     (AC-FT)       16.         16.         16.          16.

                     CUMULATIVE AREA =    0.14 SQ MI



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             *            *
   137 KK    *     CP-6   *     COMBINE
             *            *
             **************
                        PRINCESS BLVD AND SCOTTSDALE RD (PRINCESS BLVD CHANNEL)

   139 HC          HYDROGRAPH COMBINATION
                    ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                            ***

       ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     CP-6
                    TRANSPOSITION AREA      0.0 SQ MI

   PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                         (CFS)
 +    1662.      1.20                 185.         49.         24.          24.
                    (INCHES)     1.782       1.881       1.900        1.900
                     (AC-FT)       92.         97.         98.          98.

                     CUMULATIVE AREA =    0.96 SQ MI


       ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     CP-6
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                    TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1650.      1.20                  183.         48.         23.          23.
                       (INCHES)      1.769       1.869       1.887        1.887
                        (AC-FT)        91.         96.         97.          97.

                        CUMULATIVE AREA =    0.96 SQ MI


       ***            ***            ***            ***            ***

                      HYDROGRAPH AT STATION     CP-6
                    TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1542.      1.20                  176.         47.         23.          23.
                       (INCHES)      1.702       1.801       1.820        1.820
                        (AC-FT)        87.         93.         93.          93.

                        CUMULATIVE AREA =    0.96 SQ MI


       ***            ***            ***            ***            ***

                      HYDROGRAPH AT STATION     CP-6
                    TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1308.      1.20                  161.         43.         21.          21.
                       (INCHES)      1.555       1.659       1.677        1.677
                        (AC-FT)        80.         85.         86.          86.

                        CUMULATIVE AREA =    0.96 SQ MI


       ***            ***            ***            ***            ***

                      HYDROGRAPH AT STATION     CP-6
                    TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     982.      1.25                  135.         37.         18.          18.
                       (INCHES)      1.305       1.412       1.430        1.430
                        (AC-FT)        67.         73.         73.          73.

                        CUMULATIVE AREA =    0.96 SQ MI

       ***            ***            ***            ***            ***


                        INTERPOLATED HYDROGRAPH AT     CP-6

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1609.      1.20                  181.         48.         23.          23.
                       (INCHES)      1.743       1.843       1.862        1.862
                        (AC-FT)        90.         95.         96.          96.

                        CUMULATIVE AREA =    0.96 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             *            *
  140 KK     *      SB07  *        BASIN
             *            *
             **************

                SUBBASIN RUNOFF DATA

  141 BA          SUBBASIN CHARACTERISTICS
                      TAREA        0.07  SUBBASIN AREA
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  142 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.21  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.00  WETTING FRONT SUCTION
                        XKSAT        0.58  HYDRAULIC CONDUCTIVITY
                        RTIMP       49.00  PERCENT IMPERVIOUS AREA

  143 UC          CLARK UNITGRAPH
                           TC        0.20  TIME OF CONCENTRATION
                            R        0.11  STORAGE COEFFICIENT

  144 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.20 HR,       R=  0.11 HR
                                                       SNYDER   TP=  0.13 HR,      CP=  0.62

                                                                   UNIT HYDROGRAPH
                                                              14 END-OF-PERIOD ORDINATES
                   39.      156.      219.      180.      130.       84.       53.       34.       22.       14.
                    9.        6.        4.        2.

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB07
                        TRANSPOSITION AREA      0.0 SQ MI

    TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.54, TOTAL EXCESS =    2.22

  PEAK FLOW     TIME                       MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     386.      0.90                  18.          4.          2.           2.
                        (INCHES)     2.209       2.209       2.209        2.209
                         (AC-FT)        9.          9.          9.           9.

                         CUMULATIVE AREA =    0.07 SQ MI


      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB07
                        TRANSPOSITION AREA      0.5 SQ MI

    TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.54, TOTAL EXCESS =    2.20

  PEAK FLOW     TIME                       MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     384.      0.90                  17.          4.          2.           2.
                        (INCHES)     2.193       2.193       2.193        2.193
                         (AC-FT)        9.          9.          9.           9.

                         CUMULATIVE AREA =    0.07 SQ MI


      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB07
                        TRANSPOSITION AREA      2.8 SQ MI

    TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.58, TOTAL EXCESS =    2.10

  PEAK FLOW     TIME                       MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     308.      0.90                  17.          4.          2.           2.
                        (INCHES)     2.095       2.095       2.095        2.095
                         (AC-FT)        8.          8.          8.           8.

                         CUMULATIVE AREA =    0.07 SQ MI


      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB07
                        TRANSPOSITION AREA     16.0 SQ MI

    TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.63, TOTAL EXCESS =    1.91

  PEAK FLOW     TIME                       MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     249.      0.90                  15.          4.          2.           2.
                        (INCHES)     1.901       1.901       1.901        1.901
                         (AC-FT)        8.          8.          8.           8.
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                     CUMULATIVE AREA =    0.07 SQ MI


      ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     SB07
                    TRANSPOSITION AREA     90.0 SQ MI

    TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.66, TOTAL EXCESS =    1.57

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     181.      0.90                 12.          3.          1.           1.
                    (INCHES)      1.563       1.563       1.563        1.563
                     (AC-FT)         6.          6.          6.           6.

                     CUMULATIVE AREA =    0.07 SQ MI

      ***             ***             ***             ***             ***



                     INTERPOLATED HYDROGRAPH AT     SB07

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     384.      0.90                 17.          4.          2.           2.
                    (INCHES)      2.197       2.197       2.197        2.197
                     (AC-FT)         9.          9.          9.           9.

                     CUMULATIVE AREA =    0.07 SQ MI



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           *            *
  146 KK   *      R7-8  *        ROUTE
           *            *
           **************
                             HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY

               HYDROGRAPH ROUTING DATA

  148 RK         KINEMATIC WAVE STREAM ROUTING
                          L       2778.  CHANNEL LENGTH
                          S      0.0014  SLOPE
                          N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                         CA        0.00  CONTRIBUTING AREA
                      SHAPE        TRAP  CHANNEL SHAPE
                         WD       46.00  BOTTOM WIDTH OR DIAMETER
                          Z        4.00  SIDE SLOPE
                     NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                               ***
                                        COMPUTED KINEMATIC PARAMETERS
                                              VARIABLE TIME STEP
                                            (DT SHOWN IS A MINIMUM)

                    ELEMENT    ALPHA       M          DT        DX       PEAK    TIME TO       VOLUME    MAXIMUM
                                                                                   PEAK                 CELERITY
                                                     (MIN)      (FT)     (CFS)     (MIN)         (IN)      (FPS)

                   MAIN          0.21      1.53       2.79    694.50    374.54     61.97      2.32      4.32

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8717E+01  EXCESS=0.0000E+00 OUTFLOW=0.9160E+01 BASIN STORAGE=0.1297E-03 PERCENT ERROR=
-5.1


                                                 INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                   MAIN          0.21      1.53       3.00              360.59     63.00      2.31

      ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     R7-8
                    TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     361.      1.05                 18.          5.          2.           2.
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                (INCHES)     2.311       2.314       2.314       2.314
                 (AC-FT)        9.          9.          9.          9.

                 CUMULATIVE AREA =    0.07 SQ MI

                              COMPUTED KINEMATIC PARAMETERS
                                    VARIABLE TIME STEP
                                  (DT SHOWN IS A MINIMUM)

              ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                             PEAK                CELERITY
                                              (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

             MAIN          0.21      1.53       2.80     694.50    373.06     62.02      2.31      4.31

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8655E+01  EXCESS=0.0000E+00 OUTFLOW=0.9098E+01 BASIN STORAGE=0.1282E-03 PERCENT ERROR=
-5.1


                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


             MAIN          0.21      1.53       3.00               359.23     63.00      2.30

      ***             ***             ***             ***             ***

                  HYDROGRAPH AT STATION     R7-8
                TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                   (CFS)
+     359.      1.05                 18.          5.          2.          2.
                (INCHES)     2.295       2.298       2.298       2.298
                 (AC-FT)        9.          9.          9.          9.

                 CUMULATIVE AREA =    0.07 SQ MI

                              COMPUTED KINEMATIC PARAMETERS
                                    VARIABLE TIME STEP
                                  (DT SHOWN IS A MINIMUM)

              ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                             PEAK                CELERITY
                                              (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

             MAIN          0.21      1.53       3.00     694.50    300.60     62.65      2.18      4.00

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8267E+01  EXCESS=0.0000E+00 OUTFLOW=0.8591E+01 BASIN STORAGE=0.1511E-03 PERCENT ERROR=
-3.9


                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


             MAIN          0.21      1.53       3.00               299.77     63.00      2.18

      ***             ***             ***             ***             ***

                  HYDROGRAPH AT STATION     R7-8
                TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                   (CFS)
+     300.      1.05                 17.          4.          2.          2.
                (INCHES)     2.176       2.178       2.179       2.179
                 (AC-FT)        9.          9.          9.          9.

                 CUMULATIVE AREA =    0.07 SQ MI

                              COMPUTED KINEMATIC PARAMETERS
                                    VARIABLE TIME STEP
                                  (DT SHOWN IS A MINIMUM)

              ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                             PEAK                CELERITY
                                              (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

             MAIN          0.21      1.53       3.00     694.50    243.19     65.55      1.99      3.72

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7502E+01  EXCESS=0.0000E+00 OUTFLOW=0.7866E+01 BASIN STORAGE=0.1118E-03 PERCENT ERROR=
-4.8

                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


             MAIN          0.21      1.53       3.00               238.95     66.00      2.00
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      ***              ***              ***              ***              ***

                          HYDROGRAPH AT STATION     R7-8
                        TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW      TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR        24-HR       72-HR     49.95-HR
+   (CFS)        (HR)
                             (CFS)
+     239.       1.10                    16.          4.          2.           2.
                          (INCHES)      1.995       1.997       1.997        1.997
                           (AC-FT)        8.          8.          8.           8.

                           CUMULATIVE AREA =    0.07 SQ MI

                                            COMPUTED KINEMATIC PARAMETERS
                                                  VARIABLE TIME STEP
                                                (DT SHOWN IS A MINIMUM)

                        ELEMENT    ALPHA       M          DT         DX        PEAK    TIME TO      VOLUME    MAXIMUM
                                                                                         PEAK                 CELERITY
                                                         (MIN)      (FT)      (CFS)     (MIN)        (IN)      (FPS)

                      MAIN          0.21      1.53       2.82     555.60     178.83     67.24       1.61      3.33

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6168E+01  EXCESS=0.0000E+00 OUTFLOW=0.6362E+01 BASIN STORAGE=0.1260E-03 PERCENT ERROR=
-3.1


                                                 INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                      MAIN          0.21      1.53       3.00                176.29     66.00       1.61

      ***              ***              ***              ***              ***

                          HYDROGRAPH AT STATION     R7-8
                        TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW      TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR        24-HR       72-HR     49.95-HR
+   (CFS)        (HR)
                             (CFS)
+     176.       1.10                    13.          3.          2.           2.
                          (INCHES)      1.609       1.611       1.611        1.611
                           (AC-FT)        6.          6.          6.           6.

                           CUMULATIVE AREA =    0.07 SQ MI

      ***              ***              ***              ***              ***



                           INTERPOLATED HYDROGRAPH AT     R7-8

  PEAK FLOW      TIME                          MAXIMUM AVERAGE FLOW
                                        6-HR        24-HR       72-HR     49.95-HR
+   (CFS)        (HR)
                             (CFS)
+     360.       1.05                    18.          5.          2.           2.
                          (INCHES)      2.299       2.301       2.301        2.301
                           (AC-FT)        9.          9.          9.           9.

                           CUMULATIVE AREA =    0.07 SQ MI



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             *            *
  149 KK     *      SB08  *        BASIN
             *            *
             **************

                SUBBASIN RUNOFF DATA

  150 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.22  SUBBASIN AREA

  151 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.14  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.10  WETTING FRONT SUCTION
                        XKSAT        0.59  HYDRAULIC CONDUCTIVITY
                        RTIMP       59.00  PERCENT IMPERVIOUS AREA

  152 UC          CLARK UNITGRAPH
                           TC        0.33  TIME OF CONCENTRATION
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                         R        0.19  STORAGE COEFFICIENT

  153 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                                         ***

                                                          UNIT HYDROGRAPH PARAMETERS
                                                    CLARK   TC=  0.33 HR,       R=  0.19 HR
                                                   SNYDER   TP=  0.19 HR,      CP=  0.55

                                                                UNIT HYDROGRAPH
                                                          24 END-OF-PERIOD ORDINATES
               35.      127.      298.      409.      385.      339.      288.      232.      179.      138.
              106.       82.       63.       49.       37.       29.       22.       17.       13.       10.
                8.        6.        5.        4.

      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB08
                       TRANSPOSITION AREA      0.0 SQ MI

    TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.42, TOTAL EXCESS =    2.34

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     796.      0.95                   56.         14.          7.           7.
                       (INCHES)     2.325       2.325       2.325        2.325
                        (AC-FT)       28.         28.         28.          28.

                        CUMULATIVE AREA =    0.22 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB08
                       TRANSPOSITION AREA      0.5 SQ MI

    TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.42, TOTAL EXCESS =    2.32

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     791.      0.95                   56.         14.          7.           7.
                       (INCHES)     2.309       2.309       2.309        2.309
                        (AC-FT)       28.         28.         28.          28.

                        CUMULATIVE AREA =    0.22 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB08
                       TRANSPOSITION AREA      2.8 SQ MI

    TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.23

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     686.      1.00                   53.         13.          6.           6.
                       (INCHES)     2.218       2.218       2.218        2.218
                        (AC-FT)       26.         26.         26.          26.

                        CUMULATIVE AREA =    0.22 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB08
                       TRANSPOSITION AREA     16.0 SQ MI

    TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.50, TOTAL EXCESS =    2.04

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     577.      1.00                   49.         12.          6.           6.
                       (INCHES)     2.032       2.032       2.032        2.032
                        (AC-FT)       24.         24.         24.          24.

                        CUMULATIVE AREA =    0.22 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB08
                       TRANSPOSITION AREA     90.0 SQ MI
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     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.53, TOTAL EXCESS =    1.71

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     439.      1.00                 41.         10.          5.           5.
                         (INCHES)   1.699       1.699       1.699        1.699
                          (AC-FT)    20.         20.         20.          20.

                          CUMULATIVE AREA =    0.22 SQ MI

       ***             ***             ***             ***             ***



                          INTERPOLATED HYDROGRAPH AT     SB08

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     792.      0.95                 56.         14.          7.           7.
                         (INCHES)   2.311       2.311       2.311        2.311
                          (AC-FT)    28.         28.         28.          28.

                          CUMULATIVE AREA =    0.22 SQ MI



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            *            *
  155 KK    *      CP-8  *      COMBINE
            *            *
            **************
                                SR 101L FREEWAY AND HAYDEN ROAD (HAYDEN ROAD NORTH CHANNEL)

  157 HC         HYDROGRAPH COMBINATION
                         ICOMP          2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                         ***

       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     CP-8
                         TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+    1041.      1.05                 74.         19.          9.           9.
                         (INCHES)   2.322       2.322       2.322        2.322
                          (AC-FT)    37.         37.         37.          37.

                          CUMULATIVE AREA =    0.30 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     CP-8
                         TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+    1035.      1.05                 74.         18.          9.           9.
                         (INCHES)   2.306       2.307       2.307        2.307
                          (AC-FT)    37.         37.         37.          37.

                          CUMULATIVE AREA =    0.30 SQ MI


       ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     CP-8
                         TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     937.      1.05                 71.         18.          9.           9.
                         (INCHES)   2.208       2.208       2.208        2.208
                          (AC-FT)    35.         35.         35.          35.

                          CUMULATIVE AREA =    0.30 SQ MI
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      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-8
                       TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                          6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     783.      1.05                       65.          16.          8.           8.
                       (INCHES)     2.023       2.023       2.023        2.023
                        (AC-FT)       32.          32.         32.          32.

                        CUMULATIVE AREA =    0.30 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-8
                       TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                          6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     561.      1.10                       54.          13.          6.           6.
                       (INCHES)     1.676       1.677       1.677        1.677
                        (AC-FT)       27.          27.         27.          27.

                        CUMULATIVE AREA =    0.30 SQ MI

      ***             ***             ***             ***             ***


                        INTERPOLATED HYDROGRAPH AT     CP-8

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                          6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1035.      1.05                       74.          18.          9.           9.
                       (INCHES)     2.307       2.308       2.308        2.308
                        (AC-FT)       37.          37.         37.          37.

                        CUMULATIVE AREA =    0.30 SQ MI



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             *            *
  158 KK     *     R8-9  *        ROUTE
             *            *
             **************
                                    HAYDEN ROAD NORTH CHANNEL FROM HAYDEN ROAD TO BASIN 53R

                HYDROGRAPH ROUTING DATA

  160 RK          KINEMATIC WAVE STREAM ROUTING
                             L        1250.  CHANNEL LENGTH
                             S      0.0013  SLOPE
                             N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                            CA        0.00  CONTRIBUTING AREA
                         SHAPE        TRAP  CHANNEL SHAPE
                            WD       67.00  BOTTOM WIDTH OR DIAMETER
                             Z        4.00  SIDE SLOPE
                        NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                        ***
                                         COMPUTED KINEMATIC PARAMETERS
                                               VARIABLE TIME STEP
                                             (DT SHOWN IS A MINIMUM)

                          ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                       PEAK                 CELERITY
                                                          (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                         MAIN          0.15      1.56       1.32     416.67   1031.83     65.57      2.33      5.61


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3691E+02  EXCESS=0.0000E+00 OUTFLOW=0.3702E+02 BASIN STORAGE=0.1702E-03 PERCENT ERROR=
-0.3


                                              INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL
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MAIN	0.15	1.56	3.00		1013.13	66.00	2.33

*** *** *** *** ***

HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1013.	1.10		75.	19.	9.	9.
		(INCHES)	2.329	2.330	2.330	2.330
		(AC-FT)	37.	37.	37.	37.

CUMULATIVE AREA = 0.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.15	1.56	1.33	416.67	1027.12	65.64	2.31	5.60

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3666E+02 EXCESS=0.0000E+00 OUTFLOW=0.3678E+02 BASIN STORAGE=0.2024E-03 PERCENT ERROR= -0.3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.15	1.56	3.00		1010.29	66.00	2.32

*** *** *** *** ***

HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1010.	1.10		74.	19.	9.	9.
		(INCHES)	2.314	2.315	2.316	2.316
		(AC-FT)	37.	37.	37.	37.

CUMULATIVE AREA = 0.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.15	1.56	1.45	416.67	913.69	66.38	2.21	5.40

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3510E+02 EXCESS=0.0000E+00 OUTFLOW=0.3519E+02 BASIN STORAGE=0.1735E-03 PERCENT ERROR= -0.3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.15	1.56	3.00		911.34	66.00	2.21

*** *** *** *** ***

HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 911.	1.10		71.	18.	9.	9.
		(INCHES)	2.212	2.214	2.214	2.214
		(AC-FT)	35.	35.	35.	35.

CUMULATIVE AREA = 0.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)

```
                MAIN          0.15      1.56       1.48    416.67    772.14     65.64      2.03      5.07


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3216E+02  EXCESS=0.0000E+00 OUTFLOW=0.3228E+02 BASIN STORAGE=0.1954E-03 PERCENT ERROR=
-0.4


                                         INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                MAIN          0.15      1.56       3.00              769.23     66.00      2.03

      ***            ***             ***             ***             ***

                       HYDROGRAPH AT STATION     R8-9
                     TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     769.      1.10                  65.         16.         8.           8.
                        (INCHES)     2.031       2.033       2.033        2.033
                         (AC-FT)       32.         32.         32.          32.

                         CUMULATIVE AREA =    0.30 SQ MI

                                     COMPUTED KINEMATIC PARAMETERS
                                           VARIABLE TIME STEP
                                         (DT SHOWN IS A MINIMUM)

                 ELEMENT    ALPHA       M          DT         DX        PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                CELERITY
                                                 (MIN)       (FT)      (CFS)     (MIN)       (IN)      (FPS)

                MAIN          0.15      1.56       1.59    416.67    554.15     67.90      1.68      4.50


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2666E+02  EXCESS=0.0000E+00 OUTFLOW=0.2676E+02 BASIN STORAGE=0.1898E-03 PERCENT ERROR=
-0.4


                                         INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                MAIN          0.15      1.56       3.00              554.05     69.00      1.68

      ***            ***             ***             ***             ***

                       HYDROGRAPH AT STATION     R8-9
                     TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     554.      1.15                  54.         13.         6.           6.
                        (INCHES)     1.682       1.684       1.684        1.684
                         (AC-FT)       27.         27.         27.          27.

                         CUMULATIVE AREA =    0.30 SQ MI

      ***            ***             ***             ***             ***



                         INTERPOLATED HYDROGRAPH AT     R8-9

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1010.      1.10                  74.         19.         9.           9.
                        (INCHES)     2.315       2.316       2.316        2.316
                         (AC-FT)       37.         37.         37.          37.

                         CUMULATIVE AREA =    0.30 SQ MI



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          *            *
  161 KK  *      SB09  *        BASIN
          *            *
          **************

            SUBBASIN RUNOFF DATA
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  162 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.05  SUBBASIN AREA

  163 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.15  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.00  WETTING FRONT SUCTION
                        XKSAT        0.61  HYDRAULIC CONDUCTIVITY
                        RTIMP       55.00  PERCENT IMPERVIOUS AREA

  164 UC          CLARK UNITGRAPH
                           TC        0.25  TIME OF CONCENTRATION
                            R        0.23  STORAGE COEFFICIENT

  165 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                                          ***

                                                             UNIT HYDROGRAPH PARAMETERS
                                                     CLARK   TC=  0.25 HR,       R=  0.23 HR
                                                    SNYDER   TP=  0.15 HR,      CP=  0.42

                                                                  UNIT HYDROGRAPH
                                                             27 END-OF-PERIOD ORDINATES
                 11.       51.       86.       89.       82.       70.       56.       45.       36.       29.
                 23.       19.       15.       12.       10.        8.        6.        5.        4.        3.
                  3.        2.        2.        1.        1.        1.        1.

       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB09
                        TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.47, TOTAL EXCESS =    2.29

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     174.      0.95                   13.          3.          2.           2.
                        (INCHES)    2.280       2.280       2.280        2.280
                         (AC-FT)       6.          6.          6.           6.

                         CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB09
                        TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.27

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     173.      0.95                   13.          3.          2.           2.
                        (INCHES)    2.264       2.264       2.264        2.264
                         (AC-FT)       6.          6.          6.           6.

                         CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB09
                        TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.51, TOTAL EXCESS =    2.18

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     153.      0.95                   12.          3.          1.           1.
                        (INCHES)    2.168       2.168       2.168        2.168
                         (AC-FT)       6.          6.          6.           6.

                         CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB09
                        TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.55, TOTAL EXCESS =    1.99

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
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                          (CFS)
+     127.      0.95                 11.          3.          1.          1.
                       (INCHES)    1.978       1.978       1.978       1.978
                        (AC-FT)       5.          5.          5.          5.

                        CUMULATIVE AREA =    0.05 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB09
                       TRANSPOSITION AREA     90.0 SQ MI

    TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.58, TOTAL EXCESS =    1.65

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+      96.      0.95                  9.          2.          1.          1.
                       (INCHES)    1.642       1.642       1.642       1.642
                        (AC-FT)       5.          5.          5.          5.

                        CUMULATIVE AREA =    0.05 SQ MI

      ***             ***             ***             ***             ***



                        INTERPOLATED HYDROGRAPH AT     SB09

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     173.      0.95                 13.          3.          2.          2.
                       (INCHES)    2.269       2.269       2.269       2.269
                        (AC-FT)       6.          6.          6.          6.

                        CUMULATIVE AREA =    0.05 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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            **************
            *            *
  167 KK    *      CP-9  *      COMBINE
            *            *
            **************
                                SR 101L FREEWAY AND BASIN 53R (HAYDEN ROAD NORTH CHANNEL)

  169 HC          HYDROGRAPH COMBINATION
                       ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                    ***

      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-9
                       TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1127.      1.10                 87.         22.         11.         11.
                       (INCHES)    2.322       2.323       2.323       2.323
                        (AC-FT)      43.         43.         43.         43.

                        CUMULATIVE AREA =    0.35 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-9
                       TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1124.      1.10                 87.         22.         10.         10.
                       (INCHES)    2.307       2.308       2.308       2.308
                        (AC-FT)      43.         43.         43.         43.

                        CUMULATIVE AREA =    0.35 SQ MI


      ***             ***             ***             ***             ***
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                     HYDROGRAPH AT STATION     CP-9
                   TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1023.      1.10                     83.         21.         10.          10.
                       (INCHES)     2.206       2.207       2.207        2.207
                        (AC-FT)       41.         41.         41.          41.

                        CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     CP-9
                   TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     869.      1.10                     76.         19.          9.           9.
                       (INCHES)     2.023       2.024       2.025        2.025
                        (AC-FT)       38.         38.         38.          38.

                        CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     CP-9
                   TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     622.      1.15                     63.         16.          8.           8.
                       (INCHES)     1.676       1.678       1.678        1.678
                        (AC-FT)       31.         31.         31.          31.

                        CUMULATIVE AREA =    0.35 SQ MI

       ***             ***             ***             ***             ***



                        INTERPOLATED HYDROGRAPH AT     CP-9

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1124.      1.10                     87.         22.         10.          10.
                       (INCHES)     2.307       2.309       2.309        2.309
                        (AC-FT)       43.         43.         43.          43.

                        CUMULATIVE AREA =    0.35 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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            **************
            *            *
  170 KK    *    PIMACH  *
            *            *
            **************
                                PIMA ROAD CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
                                100-YR, 6-HR FLO-2D MODEL (XS 183)

               SUBBASIN RUNOFF DATA

  173 BA         SUBBASIN CHARACTERISTICS
                   TAREA       12.30  SUBBASIN AREA

                                                                  ***

       ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION   PIMACH
                   TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    4017.      5.50                   1092.        292.        146.         146.
                       (INCHES)     0.825       0.884       0.919        0.919
```

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                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION   PIMACH
                    TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
+    4017.      5.50             1092.        292.        146.         146.
                   (INCHES)     0.825       0.884       0.919        0.919
                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION   PIMACH
                    TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
+    4017.      5.50             1092.        292.        146.         146.
                   (INCHES)     0.825       0.884       0.919        0.919
                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION   PIMACH
                    TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
+    4017.      5.50             1092.        292.        146.         146.
                   (INCHES)     0.825       0.884       0.919        0.919
                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION   PIMACH
                    TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
+    4017.      5.50             1092.        292.        146.         146.
                   (INCHES)     0.825       0.884       0.919        0.919
                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI

      ***             ***             ***             ***             ***


                    INTERPOLATED HYDROGRAPH AT   PIMACH

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
+    4017.      5.50             1092.        292.        146.         146.
                   (INCHES)     0.825       0.884       0.919        0.919
                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             **************
             *            *
  211 KK     *    PIMAIN  *
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          *            *
          **************
                                 BYPASS ALONG SE SIDE OF BASIN 53R TO ADOT CULVERT NEAR UNION HILLS DR

     DT          DIVERSION
                       ISTAD      PIMABY  DIVERSION HYDROGRAPH IDENTIFICATION

     DI             INFLOW        0.00   1000.00  10000.00

     DQ      DIVERTED FLOW        0.00   1000.00   1000.00
                                                                       ***

      ***             ***             ***             ***             ***

                          DIVERSION HYDROGRAPH   PIMABY
                        TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1000.      4.95                      605.        171.         88.          88.
                        (INCHES)     0.458       0.516       0.551        0.551
                         (AC-FT)      300.        338.        361.         361.

                         CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION   PIMAIN
                        TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    3017.      5.50                      486.        122.         58.          58.
                        (INCHES)     0.368       0.368       0.368        0.368
                         (AC-FT)      241.        241.        241.         241.

                         CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                          DIVERSION HYDROGRAPH   PIMABY
                        TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1000.      4.95                      605.        171.         88.          88.
                        (INCHES)     0.458       0.516       0.551        0.551
                         (AC-FT)      300.        338.        361.         361.

                         CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION   PIMAIN
                        TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    3017.      5.50                      486.        122.         58.          58.
                        (INCHES)     0.368       0.368       0.368        0.368
                         (AC-FT)      241.        241.        241.         241.

                         CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                          DIVERSION HYDROGRAPH   PIMABY
                        TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1000.      4.95                      605.        171.         88.          88.
                        (INCHES)     0.458       0.516       0.551        0.551
                         (AC-FT)      300.        338.        361.         361.

                         CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***
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HYDROGRAPH AT STATION PIMAIN
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 3017.	5.50		486.	122.	58.	58.
		(INCHES)	0.368	0.368	0.368	0.368
		(AC-FT)	241.	241.	241.	241.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

DIVERSION HYDROGRAPH PIMABY
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1000.	4.95		605.	171.	88.	88.
		(INCHES)	0.458	0.516	0.551	0.551
		(AC-FT)	300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMAIN
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 3017.	5.50		486.	122.	58.	58.
		(INCHES)	0.368	0.368	0.368	0.368
		(AC-FT)	241.	241.	241.	241.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

DIVERSION HYDROGRAPH PIMABY
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1000.	4.95		605.	171.	88.	88.
		(INCHES)	0.458	0.516	0.551	0.551
		(AC-FT)	300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION PIMAIN
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 3017.	5.50		486.	122.	58.	58.
		(INCHES)	0.368	0.368	0.368	0.368
		(AC-FT)	241.	241.	241.	241.

CUMULATIVE AREA = 12.30 SQ MI

*** *** *** *** ***

INTERPOLATED DIVERSION HYDROGRAPH AT PIMABY

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1000.	4.95		605.	171.	88.	88.
		(INCHES)	0.458	0.516	0.551	0.551
		(AC-FT)	300.	338.	361.	361.

CUMULATIVE AREA = 12.30 SQ MI

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      ***             ***             ***             ***             ***


                      INTERPOLATED HYDROGRAPH AT   PIMAIN

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+    3017.      5.50                  486.        122.         58.          58.
                     (INCHES)        0.368       0.368       0.368        0.368
                      (AC-FT)         241.        241.        241.         241.

                      CUMULATIVE AREA =   12.30 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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            **************
            *            *
  216 KK    *    PWRCH   *
            *            *
            **************
                              POWERLINE CHANNEL HYDROGRAPH FROM FLO-2D
                              100-YR, 6-HR FLO-2D MODEL (XS 107)

               SUBBASIN RUNOFF DATA

  219 BA         SUBBASIN CHARACTERISTICS
                     TAREA        7.00  SUBBASIN AREA

                                                                        ***

      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION    PWRCH
                     TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     740.      5.80                  432.        136.         75.          75.
                     (INCHES)        0.574       0.724       0.826        0.826
                      (AC-FT)         214.        270.        308.         308.

                      CUMULATIVE AREA =    7.00 SQ MI


      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION    PWRCH
                     TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     740.      5.80                  432.        136.         75.          75.
                     (INCHES)        0.574       0.724       0.826        0.826
                      (AC-FT)         214.        270.        308.         308.

                      CUMULATIVE AREA =    7.00 SQ MI


      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION    PWRCH
                     TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     740.      5.80                  432.        136.         75.          75.
                     (INCHES)        0.574       0.724       0.826        0.826
                      (AC-FT)         214.        270.        308.         308.

                      CUMULATIVE AREA =    7.00 SQ MI


      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION    PWRCH
                     TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
```

```
                    (CFS)
+     740.    5.80               432.       136.        75.        75.
                 (INCHES)      0.574      0.724      0.826      0.826
                  (AC-FT)       214.       270.       308.       308.

                  CUMULATIVE AREA =    7.00 SQ MI


       ***            ***            ***            ***            ***

                   HYDROGRAPH AT STATION    PWRCH
                 TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW    TIME                        MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)      (HR)
                    (CFS)
+     740.    5.80               432.       136.        75.        75.
                 (INCHES)      0.574      0.724      0.826      0.826
                  (AC-FT)       214.       270.       308.       308.

                  CUMULATIVE AREA =    7.00 SQ MI

       ***            ***            ***            ***            ***



                  INTERPOLATED HYDROGRAPH AT    PWRCH

  PEAK FLOW    TIME                        MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)      (HR)
                    (CFS)
+     740.    5.80               432.       136.        75.        75.
                 (INCHES)      0.574      0.724      0.826      0.826
                  (AC-FT)       214.       270.       308.       308.

                  CUMULATIVE AREA =    7.00 SQ MI



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             **************
             *            *
  257 KK     *   BINFLO   *
             *            *
             **************
                             TOTAL INFLOW INTO BASIN 53R.

  259 HC          HYDROGRAPH COMBINATION
                     ICOMP            3  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                ***

       ***            ***            ***            ***            ***

                   HYDROGRAPH AT STATION   BINFLO
                 TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW    TIME                        MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)      (HR)
                    (CFS)
+    3634.    5.50               919.       277.       144.       144.
                 (INCHES)      0.435      0.523      0.566      0.566
                  (AC-FT)       456.       549.       593.       593.

                  CUMULATIVE AREA =   19.65 SQ MI


       ***            ***            ***            ***            ***

                   HYDROGRAPH AT STATION   BINFLO
                 TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW    TIME                        MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)      (HR)
                    (CFS)
+    3634.    5.50               919.       276.       144.       144.
                 (INCHES)      0.435      0.523      0.565      0.565
                  (AC-FT)       456.       548.       593.       593.

                  CUMULATIVE AREA =   19.65 SQ MI


       ***            ***            ***            ***            ***

                   HYDROGRAPH AT STATION   BINFLO
                 TRANSPOSITION AREA      2.8 SQ MI
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  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR      72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+    3634.      5.50                    919.        275.       143.         143.
                           (INCHES)    0.435       0.521      0.564        0.564
                            (AC-FT)     456.        546.       591.         591.

                            CUMULATIVE AREA =   19.65 SQ MI


       ***            ***            ***            ***            ***

                             HYDROGRAPH AT STATION   BINFLO
                           TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR      72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+    3634.      5.50                    919.        274.       142.         142.
                           (INCHES)    0.435       0.518      0.560        0.560
                            (AC-FT)     456.        543.       587.         587.

                            CUMULATIVE AREA =   19.65 SQ MI


       ***            ***            ***            ***            ***

                             HYDROGRAPH AT STATION   BINFLO
                           TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR      72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+    3634.      5.50                    919.        270.       141.         141.
                           (INCHES)    0.435       0.512      0.554        0.554
                            (AC-FT)     456.        536.       581.         581.

                            CUMULATIVE AREA =   19.65 SQ MI

       ***            ***            ***            ***            ***



                            INTERPOLATED HYDROGRAPH AT   BINFLO

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR      72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+    3634.      5.50                    919.        273.       142.         142.
                           (INCHES)    0.435       0.517      0.560        0.560
                            (AC-FT)     456.        542.       587.         587.

                            CUMULATIVE AREA =   19.65 SQ MI



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             **************
             *            *
  260 KK     *     BASIN  *
             *            *
             **************
                                  BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
                                  ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
                                   OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
                                   THE OUTLET PIPES ARE INLET CONTROLLED.

                HYDROGRAPH ROUTING DATA

  265 RS          STORAGE ROUTING
                        NSTPS           1  NUMBER OF SUBREACHES
                         ITYP        STOR  TYPE OF INITIAL CONDITION
                       RSVRIC        0.00  INITIAL CONDITION
                            X        0.00 WORKING R AND D COEFFICIENT

  266 SV           STORAGE          0.0       44.9       76.8      108.7      140.6      171.5      202.5      233.5      264.5
295.5
                                  328.7      362.0      395.2      428.5      461.7      497.2      532.7      568.3      603.8
639.3

  268 SE          ELEVATION     1594.00   1597.00   1598.00   1599.00   1600.00   1601.00   1602.00   1603.00   1604.00
1605.00
                                1606.00   1607.00   1608.00   1609.00   1610.00   1611.00   1612.00   1613.00   1614.00
1615.00
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  270 SQ         DISCHARGE          0.       95.      153.      210.      244.      265.      284.      303.      320.
337.
                                  353.      369.      384.      398.      412.      426.      439.      452.      465.
477.

                                                                          ***

       ***             ***             ***             ***             ***

                                 HYDROGRAPH AT STATION    BASIN
                               TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW      TIME                                MAXIMUM AVERAGE FLOW
                                            6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                 (CFS)
+     363.       8.30                        355.         250.        141.         141.
                               (INCHES)     0.168        0.474       0.556        0.556
                                (AC-FT)      176.         496.        582.         582.

 PEAK STORAGE   TIME                                 MAXIMUM AVERAGE STORAGE
                                            6-HR        24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      351.       8.30                        332.         189.        101.         101.

  PEAK STAGE     TIME                                MAXIMUM AVERAGE STAGE
                                            6-HR        24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.68       8.30                     1606.11      1601.55     1598.29      1598.29

                                CUMULATIVE AREA =   19.65 SQ MI


       ***             ***             ***             ***             ***

                                 HYDROGRAPH AT STATION    BASIN
                               TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW      TIME                                MAXIMUM AVERAGE FLOW
                                            6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                 (CFS)
+     363.       8.30                        355.         250.        141.         141.
                               (INCHES)     0.168        0.474       0.555        0.555
                                (AC-FT)      176.         496.        582.         582.

 PEAK STORAGE   TIME                                 MAXIMUM AVERAGE STORAGE
                                            6-HR        24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      351.       8.30                        332.         189.        100.         100.

  PEAK STAGE     TIME                                MAXIMUM AVERAGE STAGE
                                            6-HR        24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.67       8.30                     1606.11      1601.55     1598.28      1598.28

                                CUMULATIVE AREA =   19.65 SQ MI


       ***             ***             ***             ***             ***

                                 HYDROGRAPH AT STATION    BASIN
                               TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW      TIME                                MAXIMUM AVERAGE FLOW
                                            6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                 (CFS)
+     363.       8.30                        354.         250.        141.         141.
                               (INCHES)     0.168        0.473       0.553        0.553
                                (AC-FT)      176.         495.        580.         580.

 PEAK STORAGE   TIME                                 MAXIMUM AVERAGE STORAGE
                                            6-HR        24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      350.       8.30                        331.         188.        100.         100.

  PEAK STAGE     TIME                                MAXIMUM AVERAGE STAGE
                                            6-HR        24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.65       8.30                     1606.08      1601.53     1598.27      1598.27

                                CUMULATIVE AREA =   19.65 SQ MI


       ***             ***             ***             ***             ***

                                 HYDROGRAPH AT STATION    BASIN
                               TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW      TIME                                MAXIMUM AVERAGE FLOW
                                            6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                 (CFS)
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+     362.      8.30                  353.        249.        140.         140.
                        (INCHES)     0.167       0.471       0.550        0.550
                         (AC-FT)      175.        494.        577.         577.

 PEAK STORAGE   TIME                            MAXIMUM AVERAGE STORAGE
                                      6-HR       24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      349.      8.35                  330.        187.         99.          99.

  PEAK STAGE    TIME                             MAXIMUM AVERAGE STAGE
                                      6-HR       24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.61      8.30               1606.04     1601.50     1598.24      1598.24

                         CUMULATIVE AREA =   19.65 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    BASIN
                        TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     361.      8.35                  352.        247.        138.         138.
                        (INCHES)     0.167       0.468       0.544        0.544
                         (AC-FT)      175.        491.        570.         570.

 PEAK STORAGE   TIME                            MAXIMUM AVERAGE STORAGE
                                      6-HR       24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      346.      8.35                  327.        185.         98.          98.

  PEAK STAGE    TIME                             MAXIMUM AVERAGE STAGE
                                      6-HR       24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.52      8.35               1605.96     1601.44     1598.18      1598.18

                         CUMULATIVE AREA =   19.65 SQ MI

       ***             ***             ***             ***             ***



                         INTERPOLATED HYDROGRAPH AT    BASIN

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     362.      8.30                  353.        249.        140.         140.
                        (INCHES)     0.167       0.471       0.550        0.550
                         (AC-FT)      175.        493.        576.         576.

                         CUMULATIVE AREA =   19.65 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             **************
             *            *
  272 KK     *    BSNRT1  *
             *            *
             **************
                                 2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
                                 DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
                                 EXCESS CAPACITY.

                HYDROGRAPH ROUTING DATA

  276 RK          KINEMATIC WAVE STREAM ROUTING
                             L        550.  CHANNEL LENGTH
                             S      0.0052  SLOPE
                             N       0.024  CHANNEL ROUGHNESS COEFFICIENT
                            CA        0.00  CONTRIBUTING AREA
                         SHAPE        CIRC  CHANNEL SHAPE
                            WD        7.00  BOTTOM WIDTH OR DIAMETER
                             Z        0.00  SIDE SLOPE
                        NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS
                                                                          ***

         *** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT


         *** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT
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*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.29	183.33	363.48	498.70	0.56	10.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03 EXCESS=0.0000E+00 OUTFLOW=0.5822E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR=
0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00		363.48	501.00	0.56

***   ***   ***   ***   ***

HYDROGRAPH AT STATION   BSNRT1
TRANSPOSITION AREA   0.0 SQ MI

	PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
				6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)					
			(CFS)				
+	363.	8.35		355.	250.	141.	141.
			(INCHES)	0.168	0.474	0.556	0.556
			(AC-FT)	176.	496.	582.	582.

CUMULATIVE AREA = 19.65 SQ MI

*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

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*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

*** FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION USED - ITERATION= 1

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.29	183.33	363.42	498.84	0.56	10.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03 EXCESS=0.0000E+00 OUTFLOW=0.5819E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00		363.42	501.00	0.56

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW 6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 363.	8.35		355.	250.	141.	141.
		(INCHES)	0.168	0.474	0.555	0.555
		(AC-FT)	176.	496.	582.	582.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.45	183.33	363.06	498.87	0.55	10.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5801E+03 EXCESS=0.0000E+00 OUTFLOW=0.5800E+03 BASIN STORAGE=0.3288E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00		363.06	501.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 363.	8.35		354.	250.	141.	141.
		(INCHES)	0.168	0.473	0.553	0.553
		(AC-FT)	176.	495.	580.	580.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.38	183.33	362.38	500.88	0.55	10.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5767E+03 EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3287E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00		362.38	501.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 362.	8.35		353.	249.	140.	140.
		(INCHES)	0.167	0.471	0.550	0.550
		(AC-FT)	175.	494.	577.	577.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.34	183.33	361.10	501.33	0.54	10.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03 EXCESS=0.0000E+00 OUTFLOW=0.5702E+03 BASIN STORAGE=0.3284E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00		361.10	501.00	0.54

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 90.0 SQ MI

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  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     361.      8.35                     352.        248.        138.         138.
                             (INCHES)    0.167       0.468       0.544        0.544
                              (AC-FT)     175.        491.        570.         570.

                              CUMULATIVE AREA =   19.65 SQ MI

       ***             ***             ***             ***             ***


                              INTERPOLATED HYDROGRAPH AT   BSNRT1

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     362.      8.35                     353.        249.        140.         140.
                             (INCHES)    0.167       0.471       0.549        0.549
                              (AC-FT)     175.        494.        576.         576.

                              CUMULATIVE AREA =   19.65 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             **************
             *            *
  277 KK     *    BSNRT2  *
             *            *
             **************
                                     2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BASIN 53R OUTFAL

                HYDROGRAPH ROUTING DATA

  279 RK          KINEMATIC WAVE STREAM ROUTING
                            L       1200.  CHANNEL LENGTH
                            S      0.0077  SLOPE
                            N       0.013  CHANNEL ROUGHNESS COEFFICIENT
                           CA        0.00  CONTRIBUTING AREA
                        SHAPE        CIRC  CHANNEL SHAPE
                           WD        7.00  BOTTOM WIDTH OR DIAMETER
                            Z        0.00  SIDE SLOPE
                       NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                            ***
                                            COMPUTED KINEMATIC PARAMETERS
                                                  VARIABLE TIME STEP
                                                (DT SHOWN IS A MINIMUM)

                         ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                       PEAK                CELERITY
                                                         (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                        MAIN          7.51      1.25       0.44    400.00    363.48    501.11      0.56     20.39


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03  EXCESS=0.0000E+00 OUTFLOW=0.5821E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR=
0.0


                                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                        MAIN          7.51      1.25       3.00              363.48    501.00      0.56

       ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION   BSNRT2
                             TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     363.      8.35                     355.        250.        141.         141.
                             (INCHES)    0.168       0.474       0.555        0.555
                              (AC-FT)     176.        496.        582.         582.

                              CUMULATIVE AREA =   19.65 SQ MI

                                            COMPUTED KINEMATIC PARAMETERS
                                                  VARIABLE TIME STEP
                                                (DT SHOWN IS A MINIMUM)

                         ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
```

```
                                                                           PEAK                  CELERITY
                                                (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                    MAIN          7.51      1.25       0.44    400.00    363.42    501.35       0.56      20.39


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03  EXCESS=0.0000E+00 OUTFLOW=0.5818E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR=
0.0


                                             INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                    MAIN          7.51      1.25       3.00              363.42    501.00       0.56

      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION   BSNRT2
                     TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     363.      8.35                    355.        250.        141.         141.
                         (INCHES)     0.168       0.474       0.555        0.555
                          (AC-FT)      176.        496.        582.         582.

                          CUMULATIVE AREA =   19.65 SQ MI

                                          COMPUTED KINEMATIC PARAMETERS
                                                VARIABLE TIME STEP
                                              (DT SHOWN IS A MINIMUM)

                     ELEMENT    ALPHA       M          DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                   PEAK                  CELERITY
                                                (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                    MAIN          7.51      1.25       0.36    400.00    363.06    501.42       0.55      20.38


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5800E+03  EXCESS=0.0000E+00 OUTFLOW=0.5799E+03 BASIN STORAGE=0.3755E-01 PERCENT ERROR=
0.0


                                             INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                    MAIN          7.51      1.25       3.00              363.06    501.00       0.55

      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION   BSNRT2
                     TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     363.      8.35                    354.        250.        140.         140.
                         (INCHES)     0.168       0.473       0.553        0.553
                          (AC-FT)      176.        495.        580.         580.

                          CUMULATIVE AREA =   19.65 SQ MI

                                          COMPUTED KINEMATIC PARAMETERS
                                                VARIABLE TIME STEP
                                              (DT SHOWN IS A MINIMUM)

                     ELEMENT    ALPHA       M          DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                   PEAK                  CELERITY
                                                (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                    MAIN          7.51      1.25       0.48    400.00    362.38    501.44       0.55      20.37


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5766E+03  EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3753E-01 PERCENT ERROR=
0.0


                                             INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                    MAIN          7.51      1.25       3.00              362.38    501.00       0.55

      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION   BSNRT2
                     TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
```

```
+     362.      8.35                353.        249.        140.        140.
                      (INCHES)     0.167       0.471       0.550       0.550
                       (AC-FT)      175.        494.        577.        577.

                       CUMULATIVE AREA =   19.65 SQ MI

                                     COMPUTED KINEMATIC PARAMETERS
                                          VARIABLE TIME STEP
                                        (DT SHOWN IS A MINIMUM)

                  ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                 PEAK                 CELERITY
                                                  (MIN)      (FT)      (CFS)    (MIN)       (IN)      (FPS)

                 MAIN          7.51      1.25       0.38    400.00    361.10    501.56      0.54     20.36


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03  EXCESS=0.0000E+00 OUTFLOW=0.5701E+03 BASIN STORAGE=0.3750E-01 PERCENT ERROR=
0.0

                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                 MAIN          7.51      1.25       3.00              361.09    501.00      0.54

       ***             ***             ***             ***             ***

                        HYDROGRAPH AT STATION   BSNRT2
                      TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                         (CFS)
+     361.      8.35                352.        248.        138.        138.
                      (INCHES)     0.167       0.468       0.544       0.544
                       (AC-FT)      175.        491.        570.        570.

                       CUMULATIVE AREA =   19.65 SQ MI

       ***             ***             ***             ***             ***


                       INTERPOLATED HYDROGRAPH AT   BSNRT2

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                         (CFS)
+     362.      8.35                353.        249.        139.        139.
                      (INCHES)     0.167       0.471       0.549       0.549
                       (AC-FT)      175.        494.        576.        576.

                       CUMULATIVE AREA =   19.65 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             *            *
  280 KK     *    PIMABY  *
             *            *
             **************
                              RETRIEVE PIMA BYPASS CHANNEL HYDROGRAPH

  282 DR          RETRIEVE DIVERSION HYDROGRAPH
                      ISTAD      PIMABY  DIVERSION HYDROGRAPH IDENTIFICATION

                                                                        ***

       ***             ***             ***             ***             ***

                        HYDROGRAPH AT STATION   PIMABY
                      TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                         (CFS)
+    1000.      4.95                605.        171.         88.         88.
                      (INCHES)     0.286       0.323       0.345       0.345
                       (AC-FT)      300.        338.        361.        361.

                       CUMULATIVE AREA =   12.30 SQ MI


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                    HYDROGRAPH AT STATION   PIMABY
                  TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1000.      4.95                     605.        171.         88.          88.
                       (INCHES)     0.286       0.323       0.345        0.345
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI


       ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION   PIMABY
                  TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1000.      4.95                     605.        171.         88.          88.
                       (INCHES)     0.286       0.323       0.345        0.345
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI


       ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION   PIMABY
                  TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1000.      4.95                     605.        171.         88.          88.
                       (INCHES)     0.286       0.323       0.345        0.345
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI


       ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION   PIMABY
                  TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1000.      4.95                     605.        171.         88.          88.
                       (INCHES)     0.286       0.323       0.345        0.345
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI

       ***             ***             ***             ***             ***


                        INTERPOLATED HYDROGRAPH AT   PIMABY

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1000.      4.95                     605.        171.         88.          88.
                       (INCHES)     0.458       0.516       0.551        0.551
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI



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             *            *
  283 KK     *    PMB-RT  *
             *            *
             **************
                                UNION HILLS DR CHANNEL FROM SR 101L FREEWAY TO 82ND ST

              HYDROGRAPH ROUTING DATA

  285 RK        KINEMATIC WAVE STREAM ROUTING
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                 L       3157.  CHANNEL LENGTH
                 S      0.0082  SLOPE
                 N       0.013  CHANNEL ROUGHNESS COEFFICIENT
                CA        0.00  CONTRIBUTING AREA
             SHAPE        TRAP  CHANNEL SHAPE
                WD       24.00  BOTTOM WIDTH OR DIAMETER
                 Z        4.00  SIDE SLOPE
            NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                ***
                             COMPUTED KINEMATIC PARAMETERS
                                   VARIABLE TIME STEP
                                 (DT SHOWN IS A MINIMUM)

            ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                            PEAK                CELERITY
                                            (MIN)       (FT)     (CFS)     (MIN)       (IN)      (FPS)

           MAIN          1.87      1.47       0.99   1052.33   1000.00    299.31      0.55     20.34


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03  EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0


                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


           MAIN          1.87      1.47       3.00             1000.00    300.00      0.55

      ***             ***             ***             ***             ***

              HYDROGRAPH AT STATION   PMB-RT
            TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1000.      5.00               605.        171.        87.          87.
                        (INCHES)  0.457       0.516       0.551        0.551
                         (AC-FT)   300.        338.        361.         361.

                         CUMULATIVE AREA =   12.30 SQ MI

                             COMPUTED KINEMATIC PARAMETERS
                                   VARIABLE TIME STEP
                                 (DT SHOWN IS A MINIMUM)

            ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                            PEAK                CELERITY
                                            (MIN)       (FT)     (CFS)     (MIN)       (IN)      (FPS)

           MAIN          1.87      1.47       0.99   1052.33   1000.00    299.31      0.55     20.34


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03  EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0


                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


           MAIN          1.87      1.47       3.00             1000.00    300.00      0.55

      ***             ***             ***             ***             ***

              HYDROGRAPH AT STATION   PMB-RT
            TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1000.      5.00               605.        171.        87.          87.
                        (INCHES)  0.457       0.516       0.551        0.551
                         (AC-FT)   300.        338.        361.         361.

                         CUMULATIVE AREA =   12.30 SQ MI

                             COMPUTED KINEMATIC PARAMETERS
                                   VARIABLE TIME STEP
                                 (DT SHOWN IS A MINIMUM)

            ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                            PEAK                CELERITY
                                            (MIN)       (FT)     (CFS)     (MIN)       (IN)      (FPS)

           MAIN          1.87      1.47       0.99   1052.33   1000.00    299.31      0.55     20.34


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03  EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0
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                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

             MAIN          1.87      1.47       3.00            1000.00    300.00      0.55

      ***           ***           ***            ***            ***

                   HYDROGRAPH AT STATION   PMB-RT
                  TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                         (CFS)
+    1000.      5.00               605.        171.         87.          87.
                      (INCHES)    0.457       0.516       0.551        0.551
                       (AC-FT)     300.        338.        361.         361.

                       CUMULATIVE AREA =   12.30 SQ MI

                                   COMPUTED KINEMATIC PARAMETERS
                                         VARIABLE TIME STEP
                                      (DT SHOWN IS A MINIMUM)

              ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                              PEAK                CELERITY
                                              (MIN)       (FT)     (CFS)     (MIN)       (IN)      (FPS)

             MAIN          1.87      1.47       0.99    1052.33   1000.00    299.31      0.55      20.34

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03  EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

             MAIN          1.87      1.47       3.00            1000.00    300.00      0.55

      ***           ***           ***            ***            ***

                   HYDROGRAPH AT STATION   PMB-RT
                  TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                         (CFS)
+    1000.      5.00               605.        171.         87.          87.
                      (INCHES)    0.457       0.516       0.551        0.551
                       (AC-FT)     300.        338.        361.         361.

                       CUMULATIVE AREA =   12.30 SQ MI

                                   COMPUTED KINEMATIC PARAMETERS
                                         VARIABLE TIME STEP
                                      (DT SHOWN IS A MINIMUM)

              ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                              PEAK                CELERITY
                                              (MIN)       (FT)     (CFS)     (MIN)       (IN)      (FPS)

             MAIN          1.87      1.47       0.99    1052.33   1000.00    299.31      0.55      20.34

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03  EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

             MAIN          1.87      1.47       3.00            1000.00    300.00      0.55

      ***           ***           ***            ***            ***

                   HYDROGRAPH AT STATION   PMB-RT
                  TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                         (CFS)
+    1000.      5.00               605.        171.         87.          87.
                      (INCHES)    0.457       0.516       0.551        0.551
                       (AC-FT)     300.        338.        361.         361.

                       CUMULATIVE AREA =   12.30 SQ MI

      ***           ***           ***            ***            ***
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                    INTERPOLATED HYDROGRAPH AT   PMB-RT

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1000.      5.00               605.        171.         87.          87.
                       (INCHES)    0.457       0.516       0.551        0.551
                        (AC-FT)     300.        338.        361.         361.

                    CUMULATIVE AREA =   12.30 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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            *            *
  286 KK    *      SB10  *        BASIN
            *            *
            **************

               SUBBASIN RUNOFF DATA

  287 BA         SUBBASIN CHARACTERISTICS
                       TAREA        0.04  SUBBASIN AREA

  288 LG         GREEN AND AMPT LOSS RATE
                       STRTL        0.15  STARTING LOSS
                         DTH        0.25  MOISTURE DEFICIT
                        PSIF        4.25  WETTING FRONT SUCTION
                       XKSAT        0.55  HYDRAULIC CONDUCTIVITY
                       RTIMP       55.00  PERCENT IMPERVIOUS AREA

  289 UC         CLARK UNITGRAPH
                          TC        0.23  TIME OF CONCENTRATION
                           R        0.16  STORAGE COEFFICIENT

  290 UA         ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                       0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                     100.0

                                                                        ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.23 HR,       R=  0.16 HR
                                                       SNYDER   TP=  0.14 HR,      CP=  0.51

                                                                   UNIT HYDROGRAPH
                                                              19 END-OF-PERIOD ORDINATES
                 13.        58.        90.        85.        71.        54.        40.        29.        21.        15.
                 11.         8.         6.         4.         3.         2.         2.         1.         1.

       ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     SB10
                   TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.30

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     170.      0.90                10.          2.          1.           1.
                       (INCHES)    2.287       2.287       2.287        2.287
                        (AC-FT)       5.          5.          5.           5.

                    CUMULATIVE AREA =    0.04 SQ MI


       ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     SB10
                   TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.28

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                  6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     169.      0.90                10.          2.          1.           1.
                       (INCHES)    2.272       2.272       2.272        2.272
                        (AC-FT)       5.          5.          5.           5.

                    CUMULATIVE AREA =    0.04 SQ MI


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                               HYDROGRAPH AT STATION     SB10
                             TRANSPOSITION AREA      2.8 SQ MI

            TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.50, TOTAL EXCESS =    2.19

         PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                            6-HR       24-HR       72-HR     49.95-HR
    +   (CFS)       (HR)
                                (CFS)
    +     142.      0.95                      9.          2.          1.           1.
                             (INCHES)     2.179       2.179       2.179        2.179
                              (AC-FT)        5.          5.          5.           5.

                              CUMULATIVE AREA =    0.04 SQ MI


              ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB10
                             TRANSPOSITION AREA     16.0 SQ MI

            TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.54, TOTAL EXCESS =    2.00

         PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                            6-HR       24-HR       72-HR     49.95-HR
    +   (CFS)       (HR)
                                (CFS)
    +     118.      0.95                      9.          2.          1.           1.
                             (INCHES)     1.990       1.990       1.990        1.990
                              (AC-FT)        4.          4.          4.           4.

                              CUMULATIVE AREA =    0.04 SQ MI


              ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB10
                             TRANSPOSITION AREA     90.0 SQ MI

            TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.57, TOTAL EXCESS =    1.66

         PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                            6-HR       24-HR       72-HR     49.95-HR
    +   (CFS)       (HR)
                                (CFS)
    +      88.      0.95                      7.          2.          1.           1.
                             (INCHES)     1.655       1.655       1.655        1.655
                              (AC-FT)        4.          4.          4.           4.

                              CUMULATIVE AREA =    0.04 SQ MI

              ***             ***             ***             ***             ***



                              INTERPOLATED HYDROGRAPH AT     SB10

         PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                            6-HR       24-HR       72-HR     49.95-HR
    +   (CFS)       (HR)
                                (CFS)
    +     169.      0.90                     10.          2.          1.           1.
                             (INCHES)     2.276       2.276       2.276        2.276
                              (AC-FT)        5.          5.          5.           5.

                              CUMULATIVE AREA =    0.04 SQ MI



     *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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                    *            *
         292 KK     *     CP-10  *      COMBINE
                    *            *
                    **************
                                        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)

         294 HC          HYDROGRAPH COMBINATION
                             ICOMP           3  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                         ***

              ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION    CP-10
                             TRANSPOSITION AREA      0.0 SQ MI

         PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                            6-HR       24-HR       72-HR     49.95-HR
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```
+   (CFS)       (HR)
                               (CFS)
+    1355.      6.85                    909.        420.        230.         230.
                            (INCHES)    0.429       0.794       0.903        0.903
                             (AC-FT)     451.        834.        948.         948.

                             CUMULATIVE AREA =   19.69 SQ MI


       ***              ***              ***              ***              ***

                              HYDROGRAPH AT STATION    CP-10
                            TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                               MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1355.      6.85                    909.        420.        230.         230.
                            (INCHES)    0.429       0.794       0.903        0.903
                             (AC-FT)     451.        834.        948.         948.

                             CUMULATIVE AREA =   19.69 SQ MI


       ***              ***              ***              ***              ***

                              HYDROGRAPH AT STATION    CP-10
                            TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                               MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1355.      6.85                    909.        420.        229.         229.
                            (INCHES)    0.429       0.793       0.901        0.901
                             (AC-FT)     451.        833.        946.         946.

                             CUMULATIVE AREA =   19.69 SQ MI


       ***              ***              ***              ***              ***

                              HYDROGRAPH AT STATION    CP-10
                            TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                               MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1354.      6.85                    907.        419.        228.         228.
                            (INCHES)    0.429       0.791       0.897        0.897
                             (AC-FT)     450.        831.        942.         942.

                             CUMULATIVE AREA =   19.69 SQ MI


       ***              ***              ***              ***              ***

                              HYDROGRAPH AT STATION    CP-10
                            TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                               MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1353.      6.85                    905.        417.        226.         226.
                            (INCHES)    0.427       0.788       0.890        0.890
                             (AC-FT)     449.        828.        935.         935.

                             CUMULATIVE AREA =   19.69 SQ MI

       ***              ***              ***              ***              ***



                             INTERPOLATED HYDROGRAPH AT    CP-10

  PEAK FLOW     TIME                               MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1354.      6.85                    907.        419.        228.         228.
                            (INCHES)    0.428       0.791       0.896        0.896
                             (AC-FT)     450.        831.        941.         941.

                             CUMULATIVE AREA =   19.69 SQ MI


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            **************
            *            *
  295 KK    *    R10-11  *        ROUTE
            *            *
            **************
                                UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD

               HYDROGRAPH ROUTING DATA

  297 RK         KINEMATIC WAVE STREAM ROUTING
                           L       1277.  CHANNEL LENGTH
                           S      0.0014  SLOPE
                           N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                          CA        0.00  CONTRIBUTING AREA
                       SHAPE        TRAP  CHANNEL SHAPE
                          WD      220.00  BOTTOM WIDTH OR DIAMETER
                           Z        4.00  SIDE SLOPE
                      NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                         ***
                                       COMPUTED KINEMATIC PARAMETERS
                                             VARIABLE TIME STEP
                                           (DT SHOWN IS A MINIMUM)

                      ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                     PEAK                CELERITY
                                                      (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                     MAIN          0.06      1.62       1.62     425.67   1355.03    412.64      0.90      4.66


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9482E+03  EXCESS=0.0000E+00 OUTFLOW=0.9474E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR=
0.0


                                          INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                     MAIN          0.06      1.62       3.00              1354.77    411.00      0.90

       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION   R10-11
                        TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1355.      6.85                  909.        420.        229.         229.
                        (INCHES)     0.429       0.794       0.902        0.902
                         (AC-FT)      451.        834.        947.         947.

                         CUMULATIVE AREA =   19.69 SQ MI

                                       COMPUTED KINEMATIC PARAMETERS
                                             VARIABLE TIME STEP
                                           (DT SHOWN IS A MINIMUM)

                      ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                     PEAK                CELERITY
                                                      (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                     MAIN          0.06      1.62       1.56     425.67   1354.98    412.72      0.90      4.66


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9479E+03  EXCESS=0.0000E+00 OUTFLOW=0.9473E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR=
0.0


                                          INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                     MAIN          0.06      1.62       3.00              1354.72    411.00      0.90

       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION   R10-11
                        TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1355.      6.85                  909.        420.        229.         229.
                        (INCHES)     0.429       0.794       0.902        0.902
                         (AC-FT)      451.        834.        947.         947.

                         CUMULATIVE AREA =   19.69 SQ MI

                                       COMPUTED KINEMATIC PARAMETERS
                                             VARIABLE TIME STEP
                                           (DT SHOWN IS A MINIMUM)
```

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.06	1.62	1.57	425.67	1354.58	412.38	0.90	4.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9458E+03 EXCESS=0.0000E+00 OUTFLOW=0.9450E+03 BASIN STORAGE=0.9164E+00 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.06	1.62	3.00		1354.33	411.00	0.90

*** *** *** *** ***

HYDROGRAPH AT STATION R10-11
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1354.	6.85		908.	420.	229.	229.
		(INCHES)	0.429	0.793	0.900	0.900
		(AC-FT)	450.	833.	945.	945.

CUMULATIVE AREA = 19.69 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.06	1.62	1.67	425.67	1353.83	412.54	0.90	4.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9420E+03 EXCESS=0.0000E+00 OUTFLOW=0.9412E+03 BASIN STORAGE=0.9165E+00 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.06	1.62	3.00		1353.59	411.00	0.90

*** *** *** *** ***

HYDROGRAPH AT STATION R10-11
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1354.	6.85		907.	419.	228.	228.
		(INCHES)	0.428	0.791	0.896	0.896
		(AC-FT)	450.	831.	941.	941.

CUMULATIVE AREA = 19.69 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.06	1.62	1.66	425.67	1352.40	412.19	0.89	4.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9349E+03 EXCESS=0.0000E+00 OUTFLOW=0.9341E+03 BASIN STORAGE=0.9161E+00 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.06	1.62	3.00		1352.18	411.00	0.89

*** *** *** *** ***

HYDROGRAPH AT STATION R10-11
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	49.95-HR

```
+   (CFS)       (HR)
                                (CFS)
+    1352.      6.85                       904.        417.        226.         226.
                             (INCHES)     0.427       0.788       0.889        0.889
                              (AC-FT)      448.        828.        934.         934.

                              CUMULATIVE AREA =   19.69 SQ MI

       ***             ***             ***             ***             ***



                              INTERPOLATED HYDROGRAPH AT   R10-11

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                           6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+    1353.      6.85                       906.        419.        228.         228.
                             (INCHES)     0.428       0.791       0.895        0.895
                              (AC-FT)      449.        831.        940.         940.

                              CUMULATIVE AREA =   19.69 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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              **************
              *            *
  298 KK      *      SB11  *        BASIN
              *            *
              **************

                 SUBBASIN RUNOFF DATA

  299 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.07  SUBBASIN AREA

  300 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.15  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.15  WETTING FRONT SUCTION
                        XKSAT        0.58  HYDRAULIC CONDUCTIVITY
                        RTIMP       55.00  PERCENT IMPERVIOUS AREA

  301 UC          CLARK UNITGRAPH
                           TC        0.29  TIME OF CONCENTRATION
                            R        0.23  STORAGE COEFFICIENT

  302 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.29 HR,       R=  0.23 HR
                                                       SNYDER   TP=  0.19 HR,      CP=  0.50

                                                                   UNIT HYDROGRAPH
                                                              27 END-OF-PERIOD ORDINATES
                  12.        50.        99.       119.       112.       100.        84.        67.        54.        43.
                  35.        28.        23.        18.        15.        12.         9.         8.         6.         5.
                   4.         3.         3.         2.         2.         1.         1.

       ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB11
                             TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.29

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                           6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     233.      0.95                        17.          4.          2.           2.
                             (INCHES)     2.282       2.282       2.282        2.282
                              (AC-FT)        9.          9.          9.           9.

                              CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB11
                             TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.28
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  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     231.      0.95                 17.          4.          2.           2.
                        (INCHES)    2.267       2.267       2.267        2.267
                         (AC-FT)       9.          9.          9.           9.

                         CUMULATIVE AREA =    0.07 SQ MI


       ***             ***            ***            ***            ***

                          HYDROGRAPH AT STATION     SB11
                        TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.50, TOTAL EXCESS =    2.18

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     201.      1.00                 17.          4.          2.           2.
                        (INCHES)    2.172       2.172       2.172        2.172
                         (AC-FT)       8.          8.          8.           8.

                         CUMULATIVE AREA =    0.07 SQ MI


       ***             ***            ***            ***            ***

                          HYDROGRAPH AT STATION     SB11
                        TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.55, TOTAL EXCESS =    1.99

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     169.      1.00                 15.          4.          2.           2.
                        (INCHES)    1.982       1.982       1.982        1.982
                         (AC-FT)       8.          8.          8.           8.

                         CUMULATIVE AREA =    0.07 SQ MI


       ***             ***            ***            ***            ***

                          HYDROGRAPH AT STATION     SB11
                        TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.58, TOTAL EXCESS =    1.65

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     129.      1.00                 13.          3.          2.           2.
                        (INCHES)    1.647       1.647       1.647        1.647
                         (AC-FT)       6.          6.          6.           6.

                         CUMULATIVE AREA =    0.07 SQ MI

       ***             ***            ***            ***            ***


                         INTERPOLATED HYDROGRAPH AT     SB11

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     231.      0.95                 17.          4.          2.           2.
                        (INCHES)    2.270       2.270       2.270        2.270
                         (AC-FT)       9.          9.          9.           9.

                         CUMULATIVE AREA =    0.07 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             **************
             *            *
  304 KK     *    CP-11   *      COMBINE
             *            *
             **************
                                 UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
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  306 HC          HYDROGRAPH COMBINATION
                        ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                          ***

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    CP-11
                        TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1355.      6.85                   909.        420.        232.         232.
                        (INCHES)      0.428       0.791       0.907        0.907
                         (AC-FT)       451.        834.        956.         956.

                         CUMULATIVE AREA =   19.76 SQ MI


      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    CP-11
                        TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1355.      6.85                   909.        420.        232.         232.
                        (INCHES)      0.428       0.791       0.907        0.907
                         (AC-FT)       451.        834.        956.         956.

                         CUMULATIVE AREA =   19.76 SQ MI


      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    CP-11
                        TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1354.      6.85                   908.        420.        231.         231.
                        (INCHES)      0.427       0.790       0.904        0.904
                         (AC-FT)       450.        833.        953.         953.

                         CUMULATIVE AREA =   19.76 SQ MI


      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    CP-11
                        TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1354.      6.85                   907.        419.        230.         230.
                        (INCHES)      0.427       0.789       0.900        0.900
                         (AC-FT)       450.        831.        949.         949.

                         CUMULATIVE AREA =   19.76 SQ MI


      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    CP-11
                        TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1352.      6.85                   904.        417.        228.         228.
                        (INCHES)      0.425       0.785       0.892        0.892
                         (AC-FT)       448.        828.        940.         940.

                         CUMULATIVE AREA =   19.76 SQ MI

      ***             ***             ***             ***             ***


                         INTERPOLATED HYDROGRAPH AT    CP-11

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
```

+	1353.	6.85		906.	419.	230.	230.
			(INCHES)	0.426	0.788	0.899	0.899
			(AC-FT)	449.	831.	948.	948.

CUMULATIVE AREA = 19.76 SQ MI

1

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+	HYDROGRAPH AT	76THST	51.	4.55	15.	4.	2.	0.24		
+	HYDROGRAPH AT	SB01	324.	0.95	26.	6.	3.	0.11		
+	2 COMBINED AT	CP-1	323.	0.95	37.	11.	5.	0.35		
+	ROUTED TO	R1-2	315.	1.10	37.	11.	5.	0.35		
+	HYDROGRAPH AT	SB02	489.	0.95	38.	10.	5.	0.16		
+	2 COMBINED AT	CP-2	731.	1.05	75.	20.	10.	0.51		
+	ROUTED TO	R2-3	719.	1.10	75.	20.	10.	0.51		
+	HYDROGRAPH AT	SB03	129.	1.00	12.	3.	1.	0.05		
+	2 COMBINED AT	CP-3	828.	1.10	86.	23.	11.	0.56		
+	ROUTED TO	R3-5	806.	1.20	86.	23.	11.	0.56		
+	HYDROGRAPH AT	SB04	525.	0.95	36.	9.	4.	0.14		
+	ROUTED TO	R4-5	511.	1.05	37.	9.	4.	0.14		
+	HYDROGRAPH AT	SB05	399.	0.95	30.	7.	4.	0.13		
+	3 COMBINED AT	CP-5	1410.	1.15	150.	40.	19.	0.83		
+	ROUTED TO	R5-6	1386.	1.20	150.	40.	19.	0.83		
+	HYDROGRAPH AT	SB06	420.	0.95	33.	8.	4.	0.14		
+	2 COMBINED AT	CP-6	1609.	1.20	181.	48.	23.	0.96		
+	HYDROGRAPH AT	SB07	384.	0.90	17.	4.	2.	0.07		
+	ROUTED TO	R7-8	360.	1.05	18.	5.	2.	0.07		
+	HYDROGRAPH AT	SB08	792.	0.95	56.	14.	7.	0.22		
+	2 COMBINED AT	CP-8	1035.	1.05	74.	18.	9.	0.30		
+	ROUTED TO	R8-9	1010.	1.10	74.	19.	9.	0.30		
+	HYDROGRAPH AT	SB09	173.	0.95	13.	3.	2.	0.05		
+	2 COMBINED AT	CP-9	1124.	1.10	87.	22.	10.	0.35		
+	HYDROGRAPH AT	PIMACH	4017.	5.50	1092.	292.	146.	12.30		
+	DIVERSION TO	PIMABY	1000.	4.95	605.	171.	88.	12.30		
	HYDROGRAPH AT									

+		PIMAIN	3017.	5.50	486.	122.	58.	12.30
	HYDROGRAPH AT							
+		PWRCH	740.	5.80	432.	136.	75.	7.00
	3 COMBINED AT							
+		BINFLO	3634.	5.50	919.	273.	142.	19.65
	ROUTED TO							
+		BASIN	362.	8.30	353.	249.	140.	19.65
	ROUTED TO							
+		BSNRT1	362.	8.35	353.	249.	140.	19.65
	ROUTED TO							
+		BSNRT2	362.	8.35	353.	249.	139.	19.65
	HYDROGRAPH AT							
+		PIMABY	1000.	4.95	605.	171.	88.	12.30
	ROUTED TO							
+		PMB-RT	1000.	5.00	605.	171.	87.	12.30
	HYDROGRAPH AT							
+		SB10	169.	0.90	10.	2.	1.	0.04
	3 COMBINED AT							
+		CP-10	1354.	6.85	907.	419.	228.	19.69
	ROUTED TO							
+		R10-11	1353.	6.85	906.	419.	228.	19.69
	HYDROGRAPH AT							
+		SB11	231.	0.95	17.	4.	2.	0.07
	2 COMBINED AT							
+		CP-11	1353.	6.85	906.	419.	230.	19.76

1

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING

(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

						INTERPOLATED TO COMPUTATION INTERVAL			
ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR STORM = 1 STORM AREA (SQ MI) =   0.00

R1-2	MANE	3.00	324.42	63.63	1.19	3.00	317.42	66.00	1.19

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2221E+02 EXCESS=0.0000E+00 OUTFLOW=0.2234E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR= -0.8

FOR STORM = 2 STORM AREA (SQ MI) =   0.50

R1-2	MANE	3.00	321.94	63.70	1.18	3.00	314.95	66.00	1.18

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2212E+02 EXCESS=0.0000E+00 OUTFLOW=0.2223E+02 BASIN STORAGE=0.5936E-01 PERCENT ERROR= -0.8

FOR STORM = 3 STORM AREA (SQ MI) =   2.80

R1-2	MANE	3.00	281.88	67.46	1.15	3.00	280.31	66.00	1.15

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2153E+02 EXCESS=0.0000E+00 OUTFLOW=0.2160E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=

-0.6

FOR STORM = 4 STORM AREA (SQ MI) =   16.00

R1-2	MANE	3.00	239.43	66.99	1.09	3.00	234.05	69.00	1.09

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2036E+02 EXCESS=0.0000E+00 OUTFLOW=0.2044E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR= -0.7

FOR STORM = 5 STORM AREA (SQ MI) =   90.00

R1-2	MANE	2.91	179.26	70.19	0.98	3.00	178.38	69.00	0.98

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1834E+02 EXCESS=0.0000E+00 OUTFLOW=0.1842E+02 BASIN STORAGE=0.6416E-01 PERCENT ERROR= -0.8

FOR STORM = 1 STORM AREA (SQ MI) =   0.00

R2-3	MANE	1.61	726.00	65.69	1.53	3.00	725.57	66.00	1.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4152E+02 EXCESS=0.0000E+00 OUTFLOW=0.4152E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= -0.1

FOR STORM = 2 STORM AREA (SQ MI) =   0.50

R2-3	MANE	1.61	719.73	65.75	1.52	3.00	719.25	66.00	1.52

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4128E+02 EXCESS=0.0000E+00 OUTFLOW=0.4128E+02 BASIN STORAGE=0.4197E-01 PERCENT ERROR= -0.1

FOR STORM = 3 STORM AREA (SQ MI) =   2.80

R2-3	MANE	1.65	650.44	66.16	1.47	3.00	647.31	66.00	1.47

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3985E+02 EXCESS=0.0000E+00 OUTFLOW=0.3986E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= -0.1

FOR STORM = 4 STORM AREA (SQ MI) =   16.00

R2-3	MANE	1.61	553.64	68.36	1.37	3.00	550.82	69.00	1.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3711E+02 EXCESS=0.0000E+00 OUTFLOW=0.3710E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= -0.1

FOR STORM = 5 STORM AREA (SQ MI) =   90.00

R2-3	MANE	1.77	421.93	69.46	1.19	3.00	418.11	69.00	1.19

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3232E+02 EXCESS=0.0000E+00 OUTFLOW=0.3234E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= -0.2

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R3-5	MANE	2.78	817.36	73.14	1.60	3.00	816.94	72.00	1.60

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4741E+02 EXCESS=0.0000E+00 OUTFLOW=0.4763E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR= -0.6

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R3-5	MANE	2.79	810.60	70.45	1.59	3.00	810.54	72.00	1.59

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4713E+02 EXCESS=0.0000E+00 OUTFLOW=0.4736E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR= -0.7

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R3-5	MANE	2.90	742.52	73.90	1.54	3.00	740.16	72.00	1.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4551E+02 EXCESS=0.0000E+00 OUTFLOW=0.4571E+02 BASIN STORAGE=0.8226E-01 PERCENT ERROR= -0.6

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R3-5	MANE	3.00	634.55	73.70	1.43	3.00	626.49	75.00	1.43

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4224E+02 EXCESS=0.0000E+00 OUTFLOW=0.4240E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR= -0.6

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R3-5	MANE	3.00	478.66	74.64	1.24	3.00	478.05	75.00	1.24

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3665E+02 EXCESS=0.0000E+00 OUTFLOW=0.3688E+02 BASIN STORAGE=0.8233E-01 PERCENT ERROR= -0.8

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R4-5	MANE	2.41	520.58	62.34	2.39	3.00	513.38	63.00	2.40

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1819E+02 EXCESS=0.0000E+00 OUTFLOW=0.1838E+02 BASIN STORAGE=0.3179E-04 PERCENT ERROR= -1.0

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

```
             R4-5  MANE         2.42    516.56     62.40      2.38      3.00    510.06     63.00        2.39



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1807E+02 EXCESS=0.0000E+00 OUTFLOW=0.1825E+02 BASIN STORAGE=0.4744E-04 PERCENT ERROR=
-1.0


         FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R4-5  MANE         2.55    451.64     62.77      2.29      3.00    451.16     63.00        2.30



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1745E+02 EXCESS=0.0000E+00 OUTFLOW=0.1762E+02 BASIN STORAGE=0.4583E-04 PERCENT ERROR=
-1.0


         FOR STORM = 4  STORM AREA (SQ MI) =      16.00
             R4-5  MANE         2.73    379.50     64.90      2.12      3.00    374.76     63.00        2.12



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1609E+02 EXCESS=0.0000E+00 OUTFLOW=0.1629E+02 BASIN STORAGE=0.4132E-04 PERCENT ERROR=
-1.2


         FOR STORM = 5  STORM AREA (SQ MI) =      90.00
             R4-5  MANE         2.87    291.80     65.24      1.78      3.00    288.81     66.00        1.78



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1354E+02 EXCESS=0.0000E+00 OUTFLOW=0.1370E+02 BASIN STORAGE=0.4337E-04 PERCENT ERROR=
-1.2


         FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R5-6  MANE         1.71   1430.15     73.26      1.84      3.00   1428.13     72.00        1.84



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8096E+02 EXCESS=0.0000E+00 OUTFLOW=0.8101E+02 BASIN STORAGE=0.7278E-01 PERCENT ERROR=
-0.1


         FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R5-6  MANE         1.72   1417.75     71.63      1.82      3.00   1417.68     72.00        1.82



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8047E+02 EXCESS=0.0000E+00 OUTFLOW=0.8049E+02 BASIN STORAGE=0.7282E-01 PERCENT ERROR=
-0.1


         FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R5-6  MANE         1.83   1320.58     72.30      1.76      3.00   1310.29     72.00        1.76



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7757E+02 EXCESS=0.0000E+00 OUTFLOW=0.7762E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR=
-0.2
```

FOR STORM = 4 STORM AREA (SQ MI) =   16.00

R5-6	MANE	1.92	1120.07	73.97	1.62	3.00	1117.54	75.00	1.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7160E+02 EXCESS=0.0000E+00 OUTFLOW=0.7164E+02 BASIN STORAGE=0.7274E-01 PERCENT ERROR= -0.2

FOR STORM = 5 STORM AREA (SQ MI) =   90.00

R5-6	MANE	2.16	837.38	77.35	1.39	3.00	834.70	75.00	1.39

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6127E+02 EXCESS=0.0000E+00 OUTFLOW=0.6143E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR= -0.4

FOR STORM = 1 STORM AREA (SQ MI) =   0.00

R7-8	MANE	2.79	374.54	61.97	2.32	3.00	360.59	63.00	2.31

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8717E+01 EXCESS=0.0000E+00 OUTFLOW=0.9160E+01 BASIN STORAGE=0.1297E-03 PERCENT ERROR= -5.1

FOR STORM = 2 STORM AREA (SQ MI) =   0.50

R7-8	MANE	2.80	373.06	62.02	2.31	3.00	359.23	63.00	2.30

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8655E+01 EXCESS=0.0000E+00 OUTFLOW=0.9098E+01 BASIN STORAGE=0.1282E-03 PERCENT ERROR= -5.1

FOR STORM = 3 STORM AREA (SQ MI) =   2.80

R7-8	MANE	3.00	300.60	62.65	2.18	3.00	299.77	63.00	2.18

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8267E+01 EXCESS=0.0000E+00 OUTFLOW=0.8591E+01 BASIN STORAGE=0.1511E-03 PERCENT ERROR= -3.9

FOR STORM = 4 STORM AREA (SQ MI) =   16.00

R7-8	MANE	3.00	243.19	65.55	1.99	3.00	238.95	66.00	2.00

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7502E+01 EXCESS=0.0000E+00 OUTFLOW=0.7866E+01 BASIN STORAGE=0.1118E-03 PERCENT ERROR= -4.8

FOR STORM = 5 STORM AREA (SQ MI) =   90.00

R7-8	MANE	2.82	178.83	67.24	1.61	3.00	176.29	66.00	1.61

```
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6168E+01 EXCESS=0.0000E+00 OUTFLOW=0.6362E+01 BASIN STORAGE=0.1260E-03 PERCENT ERROR=
-3.1


           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
               R8-9  MANE         1.32   1031.83     65.57      2.33      3.00   1013.13     66.00        2.33


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3691E+02 EXCESS=0.0000E+00 OUTFLOW=0.3702E+02 BASIN STORAGE=0.1702E-03 PERCENT ERROR=
-0.3


           FOR STORM = 2  STORM AREA (SQ MI) =       0.50
               R8-9  MANE         1.33   1027.12     65.64      2.31      3.00   1010.29     66.00        2.32


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3666E+02 EXCESS=0.0000E+00 OUTFLOW=0.3678E+02 BASIN STORAGE=0.2024E-03 PERCENT ERROR=
-0.3


           FOR STORM = 3  STORM AREA (SQ MI) =       2.80
               R8-9  MANE         1.45    913.69     66.38      2.21      3.00    911.34     66.00        2.21


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3510E+02 EXCESS=0.0000E+00 OUTFLOW=0.3519E+02 BASIN STORAGE=0.1735E-03 PERCENT ERROR=
-0.3


           FOR STORM = 4  STORM AREA (SQ MI) =      16.00
               R8-9  MANE         1.48    772.14     65.64      2.03      3.00    769.23     66.00        2.03


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3216E+02 EXCESS=0.0000E+00 OUTFLOW=0.3228E+02 BASIN STORAGE=0.1954E-03 PERCENT ERROR=
-0.4


           FOR STORM = 5  STORM AREA (SQ MI) =      90.00
               R8-9  MANE         1.59    554.15     67.90      1.68      3.00    554.05     69.00        1.68


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2666E+02 EXCESS=0.0000E+00 OUTFLOW=0.2676E+02 BASIN STORAGE=0.1898E-03 PERCENT ERROR=
-0.4


           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             BSNRT1  MANE         0.29    363.48    498.70      0.56      3.00    363.48    501.00        0.56


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03 EXCESS=0.0000E+00 OUTFLOW=0.5822E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR=
0.0


           FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             BSNRT1  MANE         0.29    363.42    498.84      0.56      3.00    363.42    501.00        0.56
```

```
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03 EXCESS=0.0000E+00 OUTFLOW=0.5819E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR=
0.0

           FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             BSNRT1  MANE         0.45    363.06    498.87      0.55      3.00    363.06    501.00        0.55

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5801E+03 EXCESS=0.0000E+00 OUTFLOW=0.5800E+03 BASIN STORAGE=0.3288E-01 PERCENT ERROR=
0.0

           FOR STORM = 4  STORM AREA (SQ MI) =      16.00
             BSNRT1  MANE         0.38    362.38    500.88      0.55      3.00    362.38    501.00        0.55

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5767E+03 EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3287E-01 PERCENT ERROR=
0.0

           FOR STORM = 5  STORM AREA (SQ MI) =      90.00
             BSNRT1  MANE         0.34    361.10    501.33      0.54      3.00    361.10    501.00        0.54

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03 EXCESS=0.0000E+00 OUTFLOW=0.5702E+03 BASIN STORAGE=0.3284E-01 PERCENT ERROR=
0.0

           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             BSNRT2  MANE         0.44    363.48    501.11      0.56      3.00    363.48    501.00        0.56

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03 EXCESS=0.0000E+00 OUTFLOW=0.5821E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR=
0.0

           FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             BSNRT2  MANE         0.44    363.42    501.35      0.56      3.00    363.42    501.00        0.56

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03 EXCESS=0.0000E+00 OUTFLOW=0.5818E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR=
0.0

           FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             BSNRT2  MANE         0.36    363.06    501.42      0.55      3.00    363.06    501.00        0.55

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5800E+03 EXCESS=0.0000E+00 OUTFLOW=0.5799E+03 BASIN STORAGE=0.3755E-01 PERCENT ERROR=
0.0
```

FOR STORM = 4 STORM AREA (SQ MI) =   16.00

BSNRT2 MANE   0.48   362.38   501.44   0.55   3.00   362.38   501.00   0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5766E+03 EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3753E-01 PERCENT ERROR= 0.0

FOR STORM = 5 STORM AREA (SQ MI) =   90.00

BSNRT2 MANE   0.38   361.10   501.56   0.54   3.00   361.09   501.00   0.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03 EXCESS=0.0000E+00 OUTFLOW=0.5701E+03 BASIN STORAGE=0.3750E-01 PERCENT ERROR= 0.0

FOR STORM = 1 STORM AREA (SQ MI) =   0.00

PMB-RT MANE   0.99   1000.00   299.31   0.55   3.00   1000.00   300.00   0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) =   0.50

PMB-RT MANE   0.99   1000.00   299.31   0.55   3.00   1000.00   300.00   0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) =   2.80

PMB-RT MANE   0.99   1000.00   299.31   0.55   3.00   1000.00   300.00   0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR= 0.0

FOR STORM = 4 STORM AREA (SQ MI) =   16.00

PMB-RT MANE   0.99   1000.00   299.31   0.55   3.00   1000.00   300.00   0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR= 0.0

FOR STORM = 5 STORM AREA (SQ MI) =   90.00

PMB-RT MANE   0.99   1000.00   299.31   0.55   3.00   1000.00   300.00   0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=

0.0

FOR STORM = 1 STORM AREA (SQ MI) =   0.00

R10-11	MANE	1.62	1355.03	412.64	0.90	3.00	1354.77	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9482E+03 EXCESS=0.0000E+00 OUTFLOW=0.9474E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR= 0.0

FOR STORM = 2 STORM AREA (SQ MI) =   0.50

R10-11	MANE	1.56	1354.98	412.72	0.90	3.00	1354.72	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9479E+03 EXCESS=0.0000E+00 OUTFLOW=0.9473E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) =   2.80

R10-11	MANE	1.57	1354.58	412.38	0.90	3.00	1354.33	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9458E+03 EXCESS=0.0000E+00 OUTFLOW=0.9450E+03 BASIN STORAGE=0.9164E+00 PERCENT ERROR= 0.0

FOR STORM = 4 STORM AREA (SQ MI) =   16.00

R10-11	MANE	1.67	1353.83	412.54	0.90	3.00	1353.59	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9420E+03 EXCESS=0.0000E+00 OUTFLOW=0.9412E+03 BASIN STORAGE=0.9165E+00 PERCENT ERROR= 0.0

FOR STORM = 5 STORM AREA (SQ MI) =   90.00

R10-11	MANE	1.66	1352.40	412.19	0.89	3.00	1352.18	411.00	0.89

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9349E+03 EXCESS=0.0000E+00 OUTFLOW=0.9341E+03 BASIN STORAGE=0.9161E+00 PERCENT ERROR= 0.0

*** NORMAL END OF HEC-1 ***

Appendix C
Cavasson HEC-1 Data
Toll at Cavasson

RAINFALL DATA

Project Reference: 17114 - DATA

ID	Method	Duration	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
DEFAULT	NOAA14	5 MIN	0.258	0.349	0.418	0.511	0.583	0.656
	NOAA14	10 MIN	0.393	0.530	0.636	0.778	0.887	0.998
	NOAA14	15 MIN	0.487	0.658	0.789	0.965	1.099	1.237
	NOAA14	30 MIN	0.656	0.886	1.063	1.299	1.480	1.666
	NOAA14	1 HOUR	0.812	1.096	1.315	1.608	1.832	2.062
	NOAA14	2 HOUR	0.939	1.251	1.490	1.814	2.060	2.315
	NOAA14	3 HOUR	1.024	1.339	1.588	1.936	2.212	2.498
	NOAA14	6 HOUR	1.215	1.549	1.817	2.182	2.465	2.760
	NOAA14	12 HOUR	1.374	1.734	2.016	2.399	2.690	2.993
	NOAA14	24 HOUR	1.622	2.097	2.476	3.005	3.425	3.865

Soil Map—Aguila-Carefree Area, Arizona, Parts of Maricopa and Pinal Counties

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Aguila-Carefree Area, Arizona, Parts of Maricopa and Pinal Counties
Survey Area Data: Version 11, Sep 11, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Oct 31, 2014—Dec 7, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
55	Gilman loams	768.3	30.2%
90	Momoli gravelly sandy loam, 1 to 5 percent slopes	1,778.8	69.8%
Totals for Area of Interest		**2,547.2**	**100.0%**

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

The following spreadsheets were used to prepare input data for HEC-1 Green and Ampt Method described in The Drainage Design Manual for Maricopa County

Spreadsheet equations and assumptions were as follows:

SHEET 1: *Summary* — Summary of HEC-1 Input.

SHEET 2: *Basin Geometry* — Drainage areas, watercourse length, slope, adjusted slope, land type, and K_b.

Slopes were adjusted according to Figure 5.4.

Land type and K_b were determined using Table 5.1.

SHEET 3: *Green-Ampt Parameters* — Soil characteristics, XKSAT, PSIF, DTHETA, and RTEMP.

Soil type and percentages determined from Flood Control soils map.

XKSAT determined using Appendix A. If applicable a logrithmic weighted value was determined for multiple soils within a basin according to the following equation:

$$\overline{\mathrm{XSAT}} = \mathrm{ALOG}\sum\left[\%\,\mathrm{SOIL\ TYPE(IN\ DECIMAL)} \cdot \mathrm{LOG(XSAT)}\right]$$

Where: XSAT det er min ed from Appendix A

PSIF and DTHETA determined using Figure 4.3.

RTEMP is a weighted value based on soil data. The following equation was used to determine the weighted value:

$$\mathrm{RTEMP} = \sum \%\,\mathrm{SOIL\ TYPE} \cdot \left[\%\,\mathrm{ROCK\ OUTCROP(IN\ DECIMAL)}\right]$$

These values may be used for predeveloped conditions.

SHEET 4: *Land Use Parameters* — Land use categories, IA, and RTEMP. IA and RTEMP are weighted based on percentage of land use. These values may be used for post developement conditions.

SHEET 5: Retention Required — This is the Proposed Condition Retention that will be required for each SubBasin. It is calculated as the FIRST FLUSH per the Development Agreement (Construction of Offsite Powerline Channel)
The Formula used is from COS: Vol = Cw*(P/12)*Area
In HEC-1 this will be input as a Divert and not brought back
The Divert will be labeled: DIVXXX, where XXX is the Basin ID

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

	SEE SHEET _ FOR DETERMINATION OF THE FOLLOWING INPUT								
Sheet # →	2	2	4	2	4	3	3	3	4
BASIN NUMBER	BASIN AREA (Miles^2)	WATERCOURSE LENGTH (Miles)	Kb WEIGHTED	SLOPE ADJUSTED (Ft/Mile)	IA WEIGHTED	(in)	PSIF (in)	$\overline{\text{XKSAT}}$ (in/hr)	GREATER OF RTEMP SOIL OR WEIGHTED
SB01-C	0.0270	0.3286	0.032	96	0.25	0.35	4.3	0.5778	45%
SB01-B	0.0240	0.2750	0.033	115	0.25	0.35	4.3	0.5778	45%
SB01-A	0.0610	0.2750	0.030	121	0.25	0.35	4.3	0.5778	45%
SB02NE	0.0335	0.3873	0.032	72	0.25	0.35	4.3	0.5778	45%
SB02NW	0.0193	0.1919	0.033	86	0.25	0.35	4.3	0.5778	45%
SB02W	0.0571	0.3483	0.111	88	0.35	0.35	4.3	0.5111	0%
SB02E1	0.0252	0.2788	0.032	86	0.10	0.35	4.3	0.6222	80%
SB02E2	0.0189	0.2973	0.033	77	0.10	0.35	4.3	0.6222	80%
SB8NW	0.0147	0.1445	0.034	83	0.10	0.35	4.3	0.6222	80%
SB8NE	0.0150	0.1405	0.034	85	0.10	0.35	4.3	0.6222	80%
SB8SW	0.0129	0.1383	0.034	116	0.10	0.35	4.3	0.6222	80%
SB8SE	0.0131	0.1580	0.034	104	0.10	0.35	4.3	0.6222	80%
SB08W	0.0589	0.3428	0.030	64	0.10	0.35	4.3	0.6222	80%
SB07W	0.0468	0.3138	0.031	99	0.25	0.35	4.3	0.5778	45%

SUM= 0.4272

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

						BASIN ELEVATION			SLOPE		
BASIN	BASIN AREA			WATER COURSE LENGTH			Low	Δ Elevation			ADJUSTED
NUMBER	(Feet^2)	(ACRES)	(Miles^2)	(Feet)	(Miles)	(Feet)	(Feet)	(Feet)	(Ft/Ft)	(Ft/Mile)	(Ft/Mile)
SB01-C	752,914	17.28	0.0270	1735	0.329	1681.0	1649.3	31.7	0.0183	96.47	96.5
SB01-B	668,111	15.34	0.0240	1452	0.275	1668.5	1637.0	31.5	0.0217	114.55	114.5
SB01-A	1,700,557	39.04	0.0610	1452	0.275	1659.3	1626.0	33.3	0.0229	121.09	121.1
SB02NE	934,725	21.46	0.0335	2045	0.387	1652.0	1624.0	28.0	0.0137	72.29	72.3
SB02NW	537,015	12.33	0.0193	1013	0.192	1640.5	1624.0	16.5	0.0163	86.00	86.0
SB02W	1,592,151	36.55	0.0571	1839	0.348	1634.5	1604.0	30.5	0.0166	87.57	87.6
SB02E1	701,285	16.10	0.0252	1472	0.279	1631.0	1607.0	24.0	0.0163	86.09	86.1
SB02E2	526,856	12.09	0.0189	1570	0.297	1627.0	1604.0	23.0	0.0146	77.35	77.4
SB8NW	408,919	9.39	0.0147	763	0.145	1654.0	1642.0	12.0	0.0157	83.04	83.0
SB8NE	417,750	9.59	0.0150	742	0.141	1654.0	1642.0	12.0	0.0162	85.39	85.4
SB8SW	358,807	8.24	0.0129	730	0.138	1642.0	1626.0	16.0	0.0219	115.73	115.7
SB8SE	365,480	8.39	0.0131	834	0.158	1642.5	1626.0	16.5	0.0198	104.46	104.5
SB08W	1,641,602	37.69	0.0589	1810	0.343	1634.0	1612.0	22.0	0.0122	64.18	64.2
SB07W	1,303,601	29.93	0.0468	1657	0.314	1685.0	1654.0	31.0	0.0187	98.78	98.8
SUM=	11,909,774	273	0.4272								

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA (Miles^2)	SOIL TYPE	% OF SOIL TYPE	ROCK OUTCROP % OF SOIL IMPERVIOUS	XKSAT (in/hr)	XKSAT ADJUSTED FOR 60% VEGATATION*	$\overline{XKSAT}$ (in/hr)	PSIF (in)	DTHETA (in)	RTEMP SOIL IN SUBBASIN
SB01-C	0.0270	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB01-B	0.0240	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB01-A	0.0610	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB02NE	0.0335	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB02NW	0.0193	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB02W	0.0571	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02E1	0.0252	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB02E2	0.0189	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8NW	0.0147	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8NE	0.0150	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8SW	0.0129	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8SE	0.0131	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB08W	0.0589	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB07W	0.0468	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%

*60% vegetation was used for C2 land uses, MFR land uses used 50% and NDR used 35% vegetation

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA ACRES	LAND USE	% OF BASIN	SUB AREA ACRES	LAND TYPE	K_b	K_b WEIGHTED	IA FOR LAND USE	IA WEIGHTED	RTEMP FOR SOIL IN BASIN	RTEMP FOR LAND USE	RTEMP WEIGHTED	RTEMP MAX
SB01-C	17.28	MFR	100%	17.28	A	0.0323	0.0323	0.25	0.25	0%	45%	45%	45%
SB01-B	15.34	MFR	100%	15.34	A	0.0326	0.0326	0.25	0.25	0%	45%	45%	45%
SB01-A	39.04	MFR	100%	39.04	A	0.0301	0.0301	0.25	0.25	0%	45%	45%	45%
SB02NE	21.46	MFR	100%	21.46	A	0.0317	0.0317	0.25	0.25	0%	45%	45%	45%
SB02NW	12.33	MFR	100%	12.33	A	0.0332	0.0332	0.25	0.25	0%	45%	45%	45%
SB02W	36.55	NDR	100%	36.55	C	0.1109	0.1109	0.35	0.35	0%	0%	0%	0%
SB02E1	16.10	C2	100%	16.10	A	0.0325	0.0325	0.1	0.10	0%	80%	80%	80%
SB02E2	12.09	C2	100%	12.09	A	0.0332	0.0332	0.1	0.10	0%	80%	80%	80%
SB8NW	9.39	C2	100%	9.39	A	0.0339	0.0339	0.1	0.10	0%	80%	80%	80%
SB8NE	9.59	C2	100%	9.59	A	0.0339	0.0339	0.1	0.10	0%	80%	80%	80%
SB8SW	8.24	C2	100%	8.24	A	0.0343	0.0343	0.1	0.10	0%	80%	80%	80%
SB8SE	8.39	C2	100%	8.39	A	0.0342	0.0342	0.1	0.10	0%	80%	80%	80%
SB08W	37.69	C2	100%	37.69	A	0.0301	0.0301	0.1	0.10	0%	80%	80%	80%
SB07W	29.93	MFR	100%	29.93	A	0.0308	0.0308	0.25	0.25	0%	45%	45%	45%
SUM=	273			273									

Retention Required

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET

BASIN NUMBER	BASIN AREA ($Feet^2$)	(ACRES)	($Miles^2$)	Weighted C Proposed Cond.	Volume Required [acre-ft]	[ft^3]
SB01-C	752,914	17.28	0.0270	0.85	0.612	26,665.7
SB01-B	668,111	15.34	0.0240	0.85	0.543	23,662.3
SB01-A	1,700,557	39.04	0.0610	0.85	1.383	60,228.1
SB02NE	934,725	21.46	0.0335	0.85	0.760	33,104.9
SB02NW	537,015	12.33	0.0193	0.85	0.437	19,019.3
SB02W	1,592,151	36.55	0.0571	0.85	1.295	56,388.7
SB02E1	701,285	16.10	0.0252	0.85	0.570	24,837.2
SB02E2	526,856	12.09	0.0189	0.85	0.428	18,659.5
SB8NW	408,919	9.39	0.0147	0.85	0.332	14,482.5
SB8NE	417,750	9.59	0.0150	0.85	0.340	14,795.3
SB8SW	358,807	8.24	0.0129	0.85	0.292	12,707.8
SB8SE	365,480	8.39	0.0131	0.85	0.297	12,944.1
SB08W	1,641,602	37.69	0.0589	0.85	1.335	58,140.1
SB07W	1,303,601	29.93	0.0468	0.85	1.060	46,169.2
SUM=	11,909,774	273	0.43		9.683	421,804.5

PROJECT NAME:	Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # :	18114-221
PREPARED BY:	TSW
DATE:	7/20/2023

The following spreadsheets were used to prepare input data for HEC-1 Green and Ampt Method described in The Drainage Design Manual for Maricopa County

Spreadsheet equations and assumptions were as follows:

SHEET 1: *Summary* — Summary of HEC-1 Input.

SHEET 2: *Basin Geometry* — Drainage areas, watercourse length, slope, adjusted slope, land type, and K_b.

Slopes were adjusted according to Figure 5.4.

Land type and K_b were determined using Table 5.1.

SHEET 3: *Green-Ampt Parameters* — Soil characteristics, XKSAT, PSIF, DTHETA, and RTEMP.

Soil type and percentages determined from Flood Control soils map.

XKSAT determined using Appendix A. If applicable a logrithmic weighted value was determined for multiple soils within a basin according to the following equation:

$$\overline{\mathrm{XSAT}} = \mathrm{ALOG}\sum\left[\%\,\mathrm{SOIL\ TYPE(IN\ DECIMAL)} \cdot \mathrm{LOG(XSAT)}\right]$$

Where: XSAT det er min ed from Appendix A

PSIF and DTHETA determined using Figure 4.3.

RTEMP is a weighted value based on soil data. The following equation was used to determine the weighted value:

$$\mathrm{RTEMP} = \sum \%\,\mathrm{SOIL\ TYPE} \cdot \left[\%\mathrm{ROCK\ OUTCROP(IN\ DECIMAL)}\right]$$

These values may be used for predeveloped conditions.

SHEET 4: *Land Use Parameters* — Land use categories, IA, and RTEMP. IA and RTEMP are weighted based on percentage of land use. These values may be used for post developement conditions.

SHEET 5: Retention Required — This is the Proposed Condition Retention that will be required for each SubBasin. It is calculated as the FIRST FLUSH per the Development Agreement (Construction of Offsite Powerline Channel)
The Formula used is from COS: Vol = Cw*(P/12)*Area
In HEC-1 this will be input as a Divert and not brought back
The Divert will be labeled: DIVXXX, where XXX is the Basin ID

Summary

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

	SEE SHEET _ FOR DETERMINATION OF THE FOLLOWING INPUT								
Sheet # →	2	2	4	2	4	3	3	3	4
BASIN NUMBER	BASIN AREA (Miles^2)	WATERCOURSE LENGTH (Miles)	Kb WEIGHTED	SLOPE ADJUSTED (Ft/Mile)	IA WEIGHTED	DTHETA (in)	PSIF (in)	$\overline{XKSAT}$ (in/hr)	GREATER OF RTEMP SOIL OR WEIGHTED
SB01-C	0.0270	0.3286	0.119	96	0.35	0.35	4.3	0.5111	0%
SB01-B	0.0240	0.2750	0.120	115	0.35	0.35	4.3	0.5111	0%
SB01-A	0.0610	0.2750	0.110	121	0.35	0.35	4.3	0.5111	0%
SB02NE	0.0335	0.3873	0.117	72	0.35	0.35	4.3	0.5111	0%
SB02NW	0.0193	0.1919	0.123	86	0.35	0.35	4.3	0.5111	0%
SB02W	0.0571	0.3483	0.111	88	0.35	0.35	4.3	0.5111	0%
SB02E1	0.0252	0.2788	0.120	86	0.35	0.35	4.3	0.5111	0%
SB02E2	0.0189	0.2973	0.126	77	0.35	0.35	4.3	0.5111	0%
SB8NW	0.0147	0.1445	0.126	83	0.35	0.35	4.3	0.5111	0%
SB8NE	0.0150	0.1405	0.125	85	0.35	0.35	4.3	0.5111	0%
SB8SW	0.0129	0.1383	0.127	116	0.35	0.35	4.3	0.5111	0%
SB8SE	0.0131	0.1580	0.127	104	0.35	0.35	4.3	0.5111	0%
SB08W	0.0589	0.3428	0.030	64	0.10	0.35	4.3	0.5111	80%
SB07W	0.0468	0.3138	0.113	99	0.35	0.35	4.3	0.5111	0%

SUM= 0.4272

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA (Feet^2)	BASIN AREA (ACRES)	BASIN AREA (Miles^2)	WATER COURSE LENGTH (Feet)	WATER COURSE LENGTH (Miles)	BASIN ELEVATION High (Feet)	BASIN ELEVATION Low (Feet)	BASIN ELEVATION Δ Elevation (Feet)	SLOPE (Ft/Ft)	SLOPE (Ft/Mile)	SLOPE ADJUSTED (Ft/Mile)
SB01-C	752,914	17.28	0.0270	1735	0.329	1681.0	1649.3	31.7	0.0183	96.47	96.5
SB01-B	668,111	15.34	0.0240	1452	0.275	1668.5	1637.0	31.5	0.0217	114.55	114.5
SB01-A	1,700,557	39.04	0.0610	1452	0.275	1659.3	1626.0	33.3	0.0229	121.09	121.1
SB02NE	934,725	21.46	0.0335	2045	0.387	1652.0	1624.0	28.0	0.0137	72.29	72.3
SB02NW	537,015	12.33	0.0193	1013	0.192	1640.5	1624.0	16.5	0.0163	86.00	86.0
SB02W	1,592,151	36.55	0.0571	1839	0.348	1634.5	1604.0	30.5	0.0166	87.57	87.6
SB02E1	701,285	16.10	0.0252	1472	0.279	1631.0	1607.0	24.0	0.0163	86.09	86.1
SB02E2	526,856	12.09	0.0189	1570	0.297	1627.0	1604.0	23.0	0.0146	77.35	77.4
SB8NW	408,919	9.39	0.0147	763	0.145	1654.0	1642.0	12.0	0.0157	83.04	83.0
SB8NE	417,750	9.59	0.0150	742	0.141	1654.0	1642.0	12.0	0.0162	85.39	85.4
SB8SW	358,807	8.24	0.0129	730	0.138	1642.0	1626.0	16.0	0.0219	115.73	115.7
SB8SE	365,480	8.39	0.0131	834	0.158	1642.5	1626.0	16.5	0.0198	104.46	104.5
SB08W	1,641,602	37.69	0.0589	1810	0.343	1634.0	1612.0	22.0	0.0122	64.18	64.2
SB07W	1,303,601	29.93	0.0468	1657	0.314	1685.0	1654.0	31.0	0.0187	98.78	98.8
SUM=	11,909,774	273	0.4272								

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA ($Miles^2$)	SOIL TYPE	% OF SOIL TYPE	ROCK OUTCROP % OF SOIL IMPERVIOUS	XKSAT (in/hr)	XKSAT ADJUSTED FOR 35% VEGATATION	$\overline{XKSAT}$ (in/hr)	PSIF (in)	DTHETA (in)	RTEMP SOIL IN SUBBASIN
SB01-C	0.0270	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB01-B	0.0240	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB01-A	0.0610	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02NE	0.0335	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02NW	0.0193	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02W	0.0571	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02E1	0.0252	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02E2	0.0189	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8NW	0.0147	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8NE	0.0150	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8SW	0.0129	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8SE	0.0131	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB08W	0.0589	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB07W	0.0468	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%

Check: 0.4272

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA ACRES	LAND USE	% OF BASIN	SUB AREA ACRES	LAND TYPE	K_b	K_b WEIGHTED	IA FOR LAND USE	IA WEIGHTED	RTEMP FOR SOIL IN BASIN	RTEMP FOR LAND USE	RTEMP WEIGHTED	RTEMP MAX
SB01-C	17.28	NDR	100%	17.28	C	0.1191	0.1191	0.35	0.35	0%	0%	0%	0%
SB01-B	15.34	NDR	100%	15.34	C	0.1204	0.1204	0.35	0.35	0%	0%	0%	0%
SB01-A	39.04	NDR	100%	39.04	C	0.1102	0.1102	0.35	0.35	0%	0%	0%	0%
SB02NE	21.46	NDR	100%	21.46	C	0.1167	0.1167	0.35	0.35	0%	0%	0%	0%
SB02NW	12.33	NDR	100%	12.33	C	0.1227	0.1227	0.35	0.35	0%	0%	0%	0%
SB02W	36.55	NDR	100%	36.55	C	0.1109	0.1109	0.35	0.35	0%	0%	0%	0%
SB02E1	16.10	NDR	100%	16.10	C	0.1198	0.1198	0.35	0.35	0%	0%	0%	0%
SB02E2	12.09	NDR	100%	12.09	C	0.1229	0.1229	0.35	0.35	0%	0%	0%	0%
SB8NW	9.39	NDR	100%	9.39	C	0.1257	0.1257	0.35	0.35	0%	0%	0%	0%
SB8NE	9.59	NDR	100%	9.59	C	0.1255	0.1255	0.35	0.35	0%	0%	0%	0%
SB8SW	8.24	NDR	100%	8.24	C	0.1271	0.1271	0.35	0.35	0%	0%	0%	0%
SB8SE	8.39	NDR	100%	8.39	C	0.1269	0.1269	0.35	0.35	0%	0%	0%	0%
SB08W	37.69	C2	100%	37.69	A	0.0301	0.0301	0.1	0.10	0%	80%	80%	80%
SB07W	29.93	NDR	100%	29.93	C	0.1131	0.1131	0.35	0.35	0%	0%	0%	0%

SUM= 273 273

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-221
PREPARED BY: TSW
DATE: 7/20/2023

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN	BASIN AREA			Weighted C	Volume Required		
NUMBER	(Feet^2)	(ACRES)	(Miles^2)	Proposed Cond.	[acre-ft]	[ft^3]	eet)
SB01-C	752,914	17.28	0.0270	0.85	0.612	26,665.7	
SB01-B	668,111	15.34	0.0240	0.85	0.543	23,662.3	
SB01-A	1,700,557	39.04	0.0610	0.85	1.383	60,228.1	
SB02NE	934,725	21.46	0.0335	0.85	0.760	33,104.9	
SB02NW	537,015	12.33	0.0193	0.85	0.437	19,019.3	
SB02W	1,592,151	36.55	0.0571	0.85	1.295	56,388.7	
SB02E1	701,285	16.10	0.0252	0.85	0.570	24,837.2	
SB02E2	526,856	12.09	0.0189	0.85	0.428	18,659.5	
SB8NW	408,919	9.39	0.0147	0.85	0.332	14,482.5	
SB8NE	417,750	9.59	0.0150	0.85	0.340	14,795.3	
SB8SW	358,807	8.24	0.0129	0.85	0.292	12,707.8	
SB8SE	365,480	8.39	0.0131	0.85	0.297	12,944.1	
SB08W	1,641,602	37.69	0.0589	0.85	1.335	58,140.1	
SB07W	1,303,601	29.93	0.0468	0.85	1.060	46,169.2	
SUM=	11,909,774	273	0.43		9.683	421,804.5	

Weir Report

Stage Discharge at 1609, Weir Controls

Trapezoidal Weir

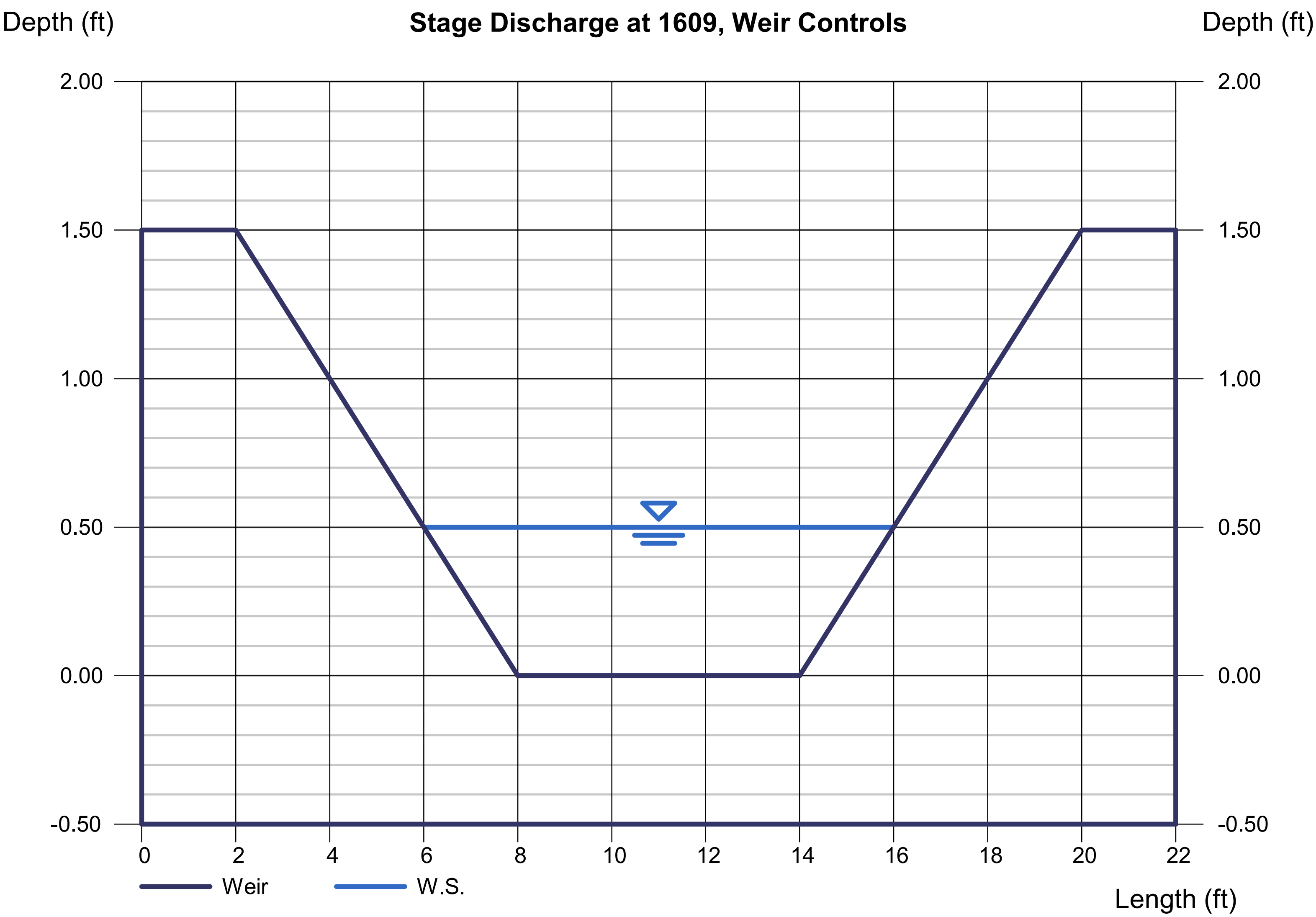
Crest	= Sharp
Bottom Length (ft)	= 6.00
Total Depth (ft)	= 1.50
Side Slope (z:1)	= 4.00

Calculations

Weir Coeff. Cw	= 3.10
Compute by:	Q vs Depth
No. Increments	= 15

Highlighted

Depth (ft)	= 0.50
Q (cfs)	= 8.330
Area (sqft)	= 4.00
Velocity (ft/s)	= 2.08
Top Width (ft)	= 10.00



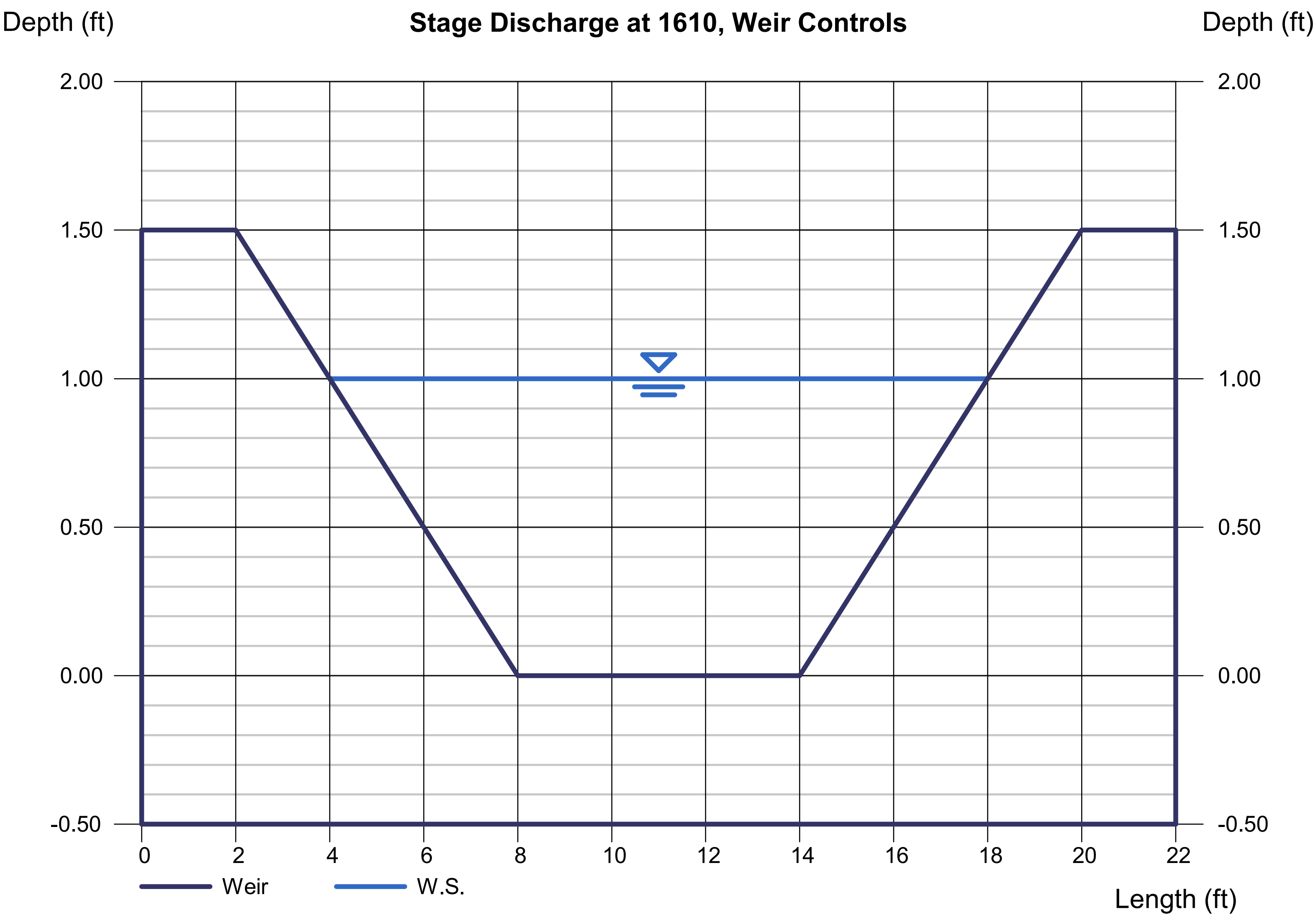
Weir Report

Stage Discharge at 1610, Weir Controls

Trapezoidal Weir		**Highlighted**	
Crest	= Sharp	Depth (ft)	= 1.00
Bottom Length (ft)	= 6.00	Q (cfs)	= 28.52
Total Depth (ft)	= 1.50	Area (sqft)	= 10.00
Side Slope (z:1)	= 4.00	Velocity (ft/s)	= 2.85
		Top Width (ft)	= 14.00

Calculations

Weir Coeff. Cw	= 3.10
Compute by:	Q vs Depth
No. Increments	= 15



Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Jul 20 2023

Stage Discharge 1611, Culvert Controls

Invert Elev Dn (ft)	=	1602.90
Pipe Length (ft)	=	334.00
Slope (%)	=	1.23
Invert Elev Up (ft)	=	1607.00
Rise (in)	=	36.0
Shape	=	Circular
Span (in)	=	36.0
No. Barrels	=	3
n-Value	=	0.012
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment

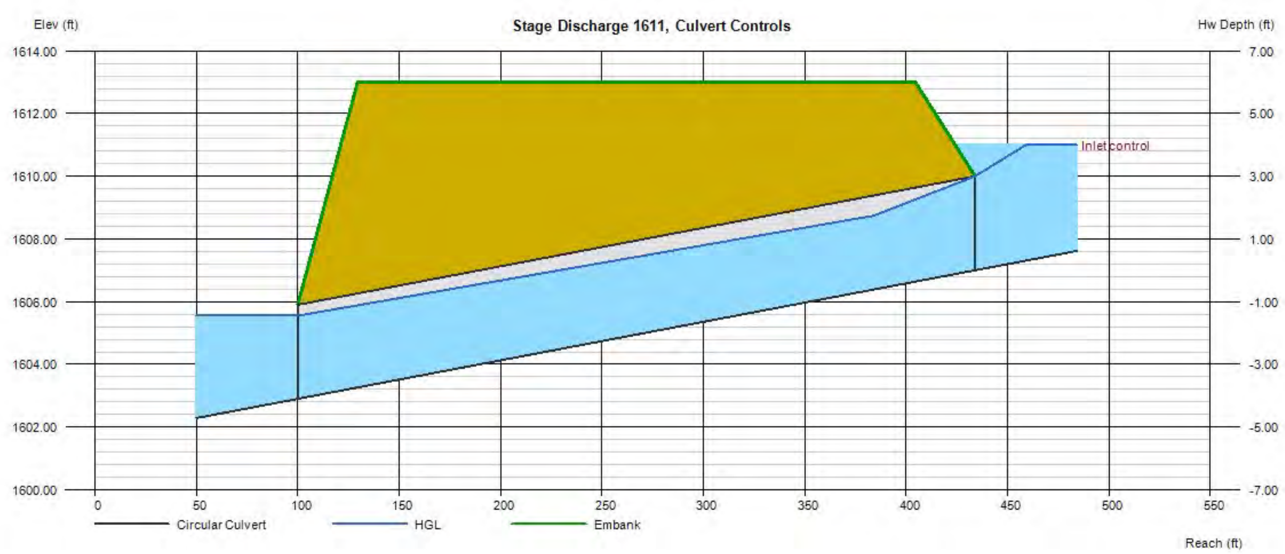
Top Elevation (ft)	=	1613.00
Top Width (ft)	=	275.00
Crest Width (ft)	=	307.00

Calculations

Qmin (cfs)	=	150.50
Qmax (cfs)	=	150.50
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	150.50
Qpipe (cfs)	=	150.50
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	7.59
Veloc Up (ft/s)	=	8.61
HGL Dn (ft)	=	1605.55
HGL Up (ft)	=	1609.30
Hw Elev (ft)	=	1611.00
Hw/D (ft)	=	1.33
Flow Regime	=	Inlet Control



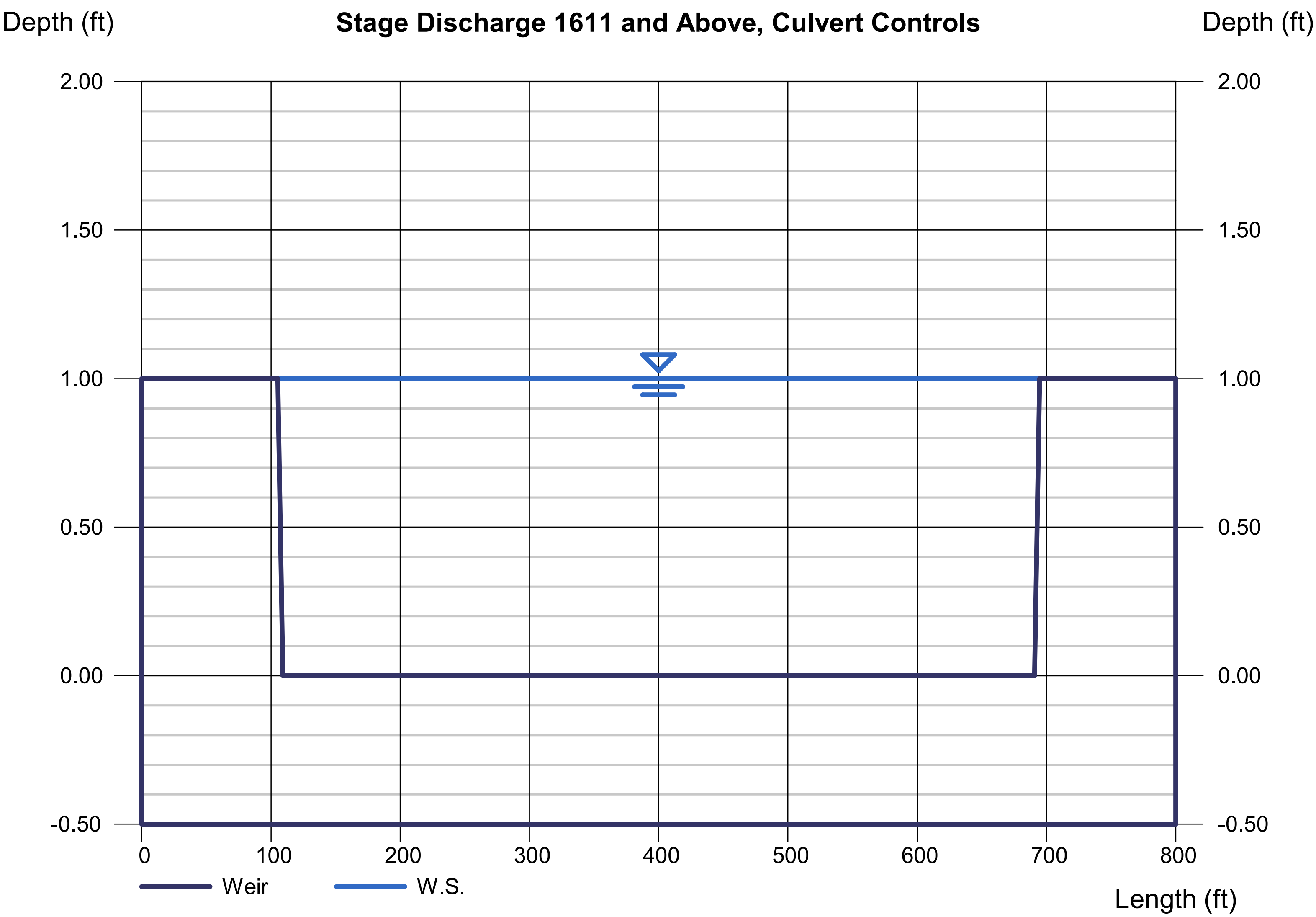
Weir Report

Stage Discharge 1611 and Above, Culvert Controls

Trapezoidal Weir		**Highlighted**	
Crest	= Sharp	Depth (ft)	= 1.00
Bottom Length (ft)	= 581.41	Q (cfs)	= 1812.29
Total Depth (ft)	= 1.00	Area (sqft)	= 585.41
Side Slope (z:1)	= 4.00	Velocity (ft/s)	= 3.10
		Top Width (ft)	= 589.41

Calculations

Weir Coeff. Cw	= 3.10
Compute by:	Q vs Depth
No. Increments	= 10



Culvert Report

Stage Discharge 1613.2, Culvert Controls

Invert Elev Dn (ft)	=	1602.90
Pipe Length (ft)	=	334.00
Slope (%)	=	1.23
Invert Elev Up (ft)	=	1607.00
Rise (in)	=	36.0
Shape	=	Circular
Span (in)	=	36.0
No. Barrels	=	3
n-Value	=	0.012
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment

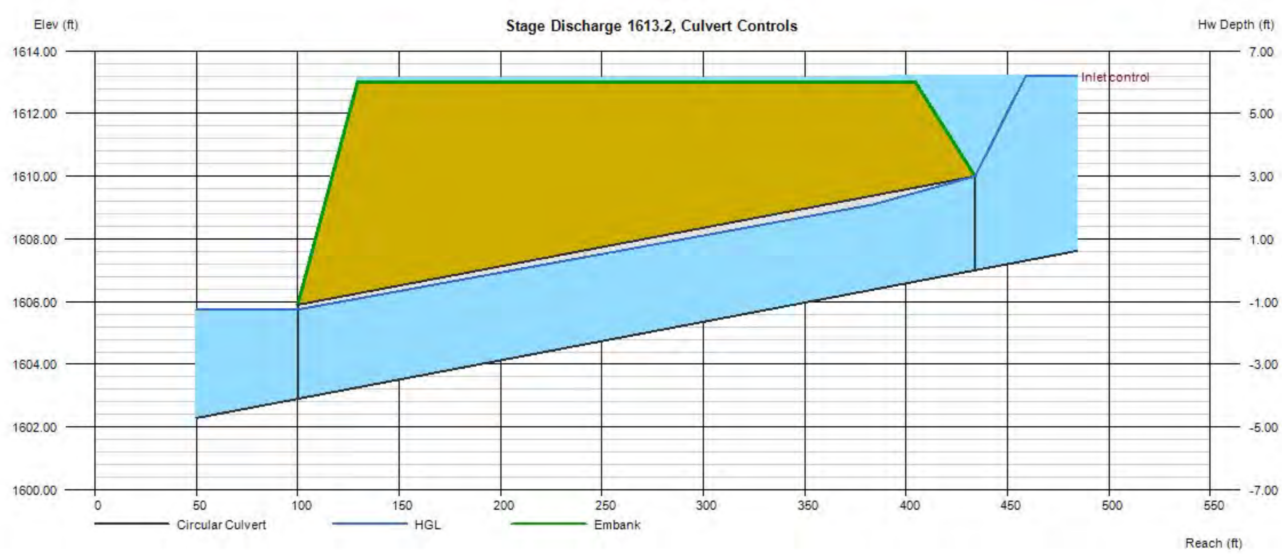
Top Elevation (ft)	=	1613.00
Top Width (ft)	=	275.00
Crest Width (ft)	=	307.00

Calculations

Qmin (cfs)	=	300.00
Qmax (cfs)	=	300.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	300.00
Qpipe (cfs)	=	218.12
Qovertop (cfs)	=	81.88
Veloc Dn (ft/s)	=	10.49
Veloc Up (ft/s)	=	10.88
HGL Dn (ft)	=	1605.75
HGL Up (ft)	=	1609.69
Hw Elev (ft)	=	1613.20
Hw/D (ft)	=	2.07
Flow Regime	=	Inlet Control



Culvert Report

Stage Discharge 1613.3, Culvert Controls

Invert Elev Dn (ft)	=	1602.90
Pipe Length (ft)	=	334.00
Slope (%)	=	1.23
Invert Elev Up (ft)	=	1607.00
Rise (in)	=	36.0
Shape	=	Circular
Span (in)	=	36.0
No. Barrels	=	3
n-Value	=	0.012
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment

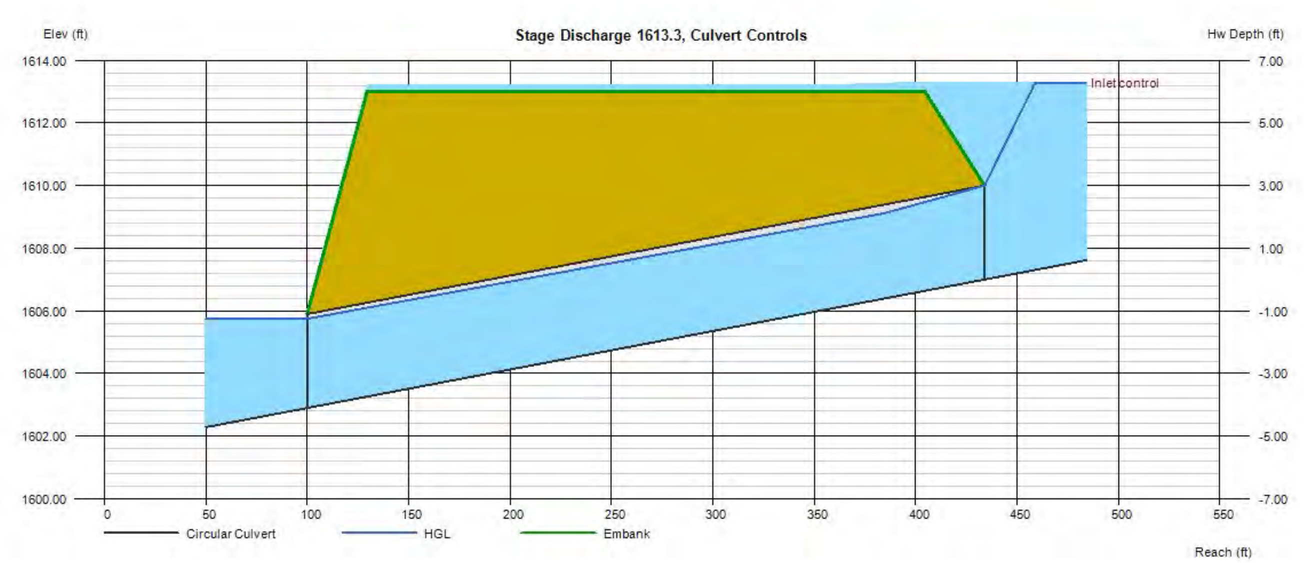
Top Elevation (ft)	=	1613.00
Top Width (ft)	=	275.00
Crest Width (ft)	=	307.00

Calculations

Qmin (cfs)	=	360.00
Qmax (cfs)	=	360.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	360.00
Qpipe (cfs)	=	220.25
Qovertop (cfs)	=	139.75
Veloc Dn (ft/s)	=	10.58
Veloc Up (ft/s)	=	10.96
HGL Dn (ft)	=	1605.75
HGL Up (ft)	=	1609.70
Hw Elev (ft)	=	1613.29
Hw/D (ft)	=	2.10
Flow Regime	=	Inlet Control



HYDRAULIC CALCULATION SHEET
Stage Storage Calculation for State Route 101 Detention Basin at CP08W for R8W-4N
Hubbard Engineering
Project No. 18114-221

Project Name: Cavasson Phase 3C **Prepared By:** TSW **Date:** 7/20/2023
Project No.: 18114-221 **Revised By:** **Date:**

RETENTION BASINS							
Drainage Area	HW Area ft^2	Bottom ft^2	H ft	Volume Provided ft^3	Volume Provided yd^3	Volume Provided acre-feet	Cumulative acre-feet
1608-1609 EX	12,929.14	5,702.11	1	9,072	336	0.208	
1608-1609 P1	23,264.29	14,812.53	1	18,880	699	0.433	0.642
1609-1610 P1	33,504.04	23,264.29	1	28,229	1,046	0.648	1.290
1610-1611 P1	46,797.22	33,504.04	1	39,966	1,480	0.917	2.207
1611-1612 P1	120,932.18	46,797.22	1	80,986	2,999	1.859	4.066
			Total ->	**177,133**	**6,560**	4.066	

Drainage Areas describe the one foot slice calculated for each stage of detention. The EX drainage area is the ADOT concrete basin and the P1 drainage areas denote the detention basin constructed as part of Cavasson Phase 1 and Claret Monument Sign

Volume Provided = $H/3*(A_{H.W.} + A_{BOTTOM} + (A_{H.W.} * A_{BOTTOM})^{0.5})$

Appendix D
Cavasson HEC-1 Ultimate
Toll at Cavasson

10yr-6hr-Ultimate

```
1***************************************                                              **************************************
 *                                      *                                              *                                    *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                              *    U.S. ARMY CORPS OF ENGINEERS    *
 *              JUN   1998              *                                              *    HYDROLOGIC ENGINEERING CENTER   *
 *           VERSION 4.1                *                                              *         609 SECOND STREET          *
 *                                      *                                              *      DAVIS, CALIFORNIA 95616       *
 *  RUN DATE   20JUL23  TIME  09:20:32   *                                              *          (916) 756-1104            *
 *                                      *                                              *                                    *
 ***************************************                                              **************************************




                    X     X  XXXXXXX   XXXXX          X
                    X     X  X        X     X        XX
                    X     X  X        X               X
                    XXXXXXX  XXXX     X        XXXXX  X
                    X     X  X        X               X
                    X     X  X        X     X         X
                    X     X  XXXXXXX   XXXXX         XXX




          THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

          THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
          THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
          NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
          DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
          KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


1                                                      HEC-1 INPUT                                                PAGE  1

         LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

            1          ID        CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
            2          ID        POWERLINE CORRIDOR
            3          ID        RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
            4          ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
            5          ID       10-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
            6          ID           PIMA ROAD FLOWS.
            7          ID
            8          ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
            9          ID       MODELERS: RK, MW
           10          ID
           11          ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
           12          ID       MODELERS: MSW, ES
           13          ID       REVISIONS NOTED WITH HE
           14          ID           ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
           15          ID           THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
           16          ID           UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.
           17          ID
           18          ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/20/23
           19          ID           MODELERS: MSW, TSW
           20          ID           REVISIONS NOTED WITH HE (HUBBARD ENGINEERING)
           21          ID           BASIN 2E WAS SPLIT AT CENTER POINT DUE TO FLOW UNDER 101
           22          ID
           23          IT       3       0       0    1000
           24          IN      15
           25          IO       5
                       *DIAGRAM
                       *
           26          JD   1.817  0.0001
           27          PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
           28          PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
           29          PC   0.962   0.972   0.983   0.991   1.000
           30          JD   1.806  0.5000
           31          PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
           32          PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
           33          PC   0.962   0.972   0.983   0.991   1.000
           34          JD   1.772     2.8
           35          PC   0.000   0.009   0.016   0.025   0.034   0.042   0.051   0.059   0.068   0.077
           36          PC   0.088   0.101   0.121   0.164   0.253   0.451   0.694   0.836   0.900   0.938
           37          PC   0.950   0.963   0.975   0.988   1.000
                       *

           38          KK  76THST
```

```
  39            KM        76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
  40            KM        10-YR, 6-HR, DERIVED FROM 100-YR DATA HYDROGRAPHS
  41            BA    0.24
  42            QI       0       0       0       0       0       0       0       0       0       0
  43            QI       0       0    0.01    0.01    0.06    0.58    8.13   27.04   31.94    31.1
  44            QI   22.07   17.10   13.79   11.20    9.48    8.22    7.44    6.76    6.13    5.32
  45            QI    4.66    4.31    3.73    3.05    2.71    2.44    2.18    2.27    1.51    1.61
  46            QI    1.48    1.38    1.29    1.17    1.01    0.96    0.97    0.77    0.84    0.86
  47            QI    0.70    0.68    0.67    0.62    0.58    0.55    0.53    0.51    0.49    0.46
  48            QI    0.44    0.43    0.42    0.40    0.38    0.37    0.35    0.34    0.33    0.31
  49            QI    0.30    0.29    0.28

                *
```

```
LINE            ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  50            KK   SB01A   BASIN
  51            BA   0.061
  52            PB   2.721
  53            PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.075
  54            PC   0.087   0.099   0.119   0.150   0.234   0.413   0.766   0.875   0.916   0.944
  55            PC   0.956   0.968   0.979   0.990   1.000
  56            LG    0.25    0.25    4.03    0.56      45
  57            UC   0.439   0.231
  58            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  59            UA     100
                *

  60            KK   SB01A  DIVERT
  61            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01A
  62            KM        FIRST FLUSH VOL = 1.383 AC-FT
  63            DT  RET01A   1.383
  64            DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  65            DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                *

  66            KK    CP-1 COMBINE
  67            KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
  68            HC       2   0.301
                *

  69            KK  R1A-2W   ROUTE
  70            KM        ROUTE CP-1 TO CP-2W
  71            RK    1114  0.0200   0.018            TRAP   2.000    0.00
                *

  72            KK  SB02NW   BASIN
  73            KM        BASIN SB02E WAS SPLIT INTO SUB BASINS NORTH OF CAVASSON
  74            KM        AND SOUTH OF CAVASSON
  75            BA  0.0193
  76            LG    0.25    0.25    4.03    0.56      45
  77            UC   0.211   0.167
  78            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  79            UA     100
                *

  80            KK  SB02NW  DIVERT
  81            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NW
  82            KM        FIRST FLUSH VOL = 0.437 AC-FT
  83            DT  RET2NW   0.437
  84            DI     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
  85            DI    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
  86            DQ     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
  87            DQ    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
                *
```

```
LINE            ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  88            KK  CP-02W COMBINE
  89            KM        COMBINE ROUTE FROM 1A-2W WITH NEW SUB BASIN SB02NW
  90            HC       2   0.305
                *

  91            KK   R2W-2   ROUTE
  92            RK    1380  0.0140   0.018            TRAP   2.000    0.00
                *
```

```
  93           KK   SB01C   BASIN
  94           BA   0.027
  95           LG    0.25    0.25    4.03    0.56      45
  96           UC   0.264   0.272
  97           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  98           UA     100
               *

  99           KK   SB01C  DIVERT
 100           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01C
 101           KM        FIRST FLUSH VOL = 0.612 AC-FT
 102           DT  RET01C   0.612
 103           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 104           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 105           KK  R1C-1B   ROUTE
 106           RK     542   0.005   0.013            CIRC   5.000
               *

 107           KK   SB01B   BASIN
 108           BA   0.024
 109           LG    0.25    0.25    4.03    0.56      45
 110           UC   0.234   0.224
 111           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 112           UA     100
               *

 113           KK   SB01B  DIVERT
 114           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01B
 115           KM        FIRST FLUSH VOL = 0.543 AC-FT
 116           DT  RET01B   0.543
 117           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 118           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 119           KK  CP-01B COMBINE
 120           KM        COMBINE ROUTE FROM 1C-1B WITH NEW SUB BASIN SB01B
 121           HC       2   0.051
               *
```



```
LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 122           KK  R1B-2N   ROUTE
 123           RK    2144   0.008   0.013            CIRC   5.000
               *

 124           KK  SB02NE   BASIN
 125           BA  0.0335
 126           LG    0.25    0.25    4.03    0.56      45
 127           UC   0.314   0.336
 128           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 129           UA     100
               *

 130           KK  SB02NE  DIVERT
 131           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NE
 132           KM        FIRST FLUSH VOL = 0.760 AC-FT
 133           DT  RET2NE   0.760
 134           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 135           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 136           KK  CP-02N COMBINE
 137           KM        COMBINE ROUTE 1B-2N (BASINS SB01-B AND SB01-C) WITH SB02NE
 138           HC       2   0.085
               *

 139           KK   R2N-2   ROUTE
 140           RK    1380   0.014   0.018            TRAP   2.000    0.00
               *

 141           KK  SB02E2   BASIN
 142           KM        BASIN 2E HAD TO BE SPLIT AT CENTER POINT DUE TO FLOW UNDER LOOP 101
 143           KM        THIS IS THE WEST SPLIT -HE
 144           BA   0.019
 145           LG    0.10    0.25    4.03    0.61      80
```

```
 146           UC   0.244   0.304
 147           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 148           UA     100
               *

 149           KK  SB02E2  DIVERT
 150           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E
 151           KM        FIRST FLUSH VOL = 0.428 AC-FT
 152           DT  RET2E2   0.428
 153           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 154           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 155           KK   SB02W   BASIN
 156           BA  0.0571
 157           LG    0.35    0.35    4.33    0.51       0
 158           UC   0.686   0.535
 159           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 160           UA     100
               *
```



```
LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 161           KK   CP-02 COMBINE
 162           HC       4  0.1816
               *

 163           KK    R2-3   ROUTE
 164           KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
 165           RK    1260  0.0015   0.030            TRAP      92       4
               *

 166           KK    SB03   BASIN
 167           BA   0.048
 168           LG    0.15    0.25    4.50    0.47      55
 169           UC   0.364   0.294
 170           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 171           UA     100
               *

 172           KK   CP-03 COMBINE
 173           HC       2
               *

 174           KK    R3-5   ROUTE
 175           KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
 176           RK    2396  0.0015   0.03             TRAP      98       4
               *

 177           KK    SB05   BASIN
 178           BA   0.126
 179           LG    0.22    0.25    4.50    0.44      48
 180           UC   0.327   0.226
 181           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 182           UA     100
               *

 183           KK   CP-5A COMBINE
 184           HC       2
               *

 185           KK   SB07W   BASIN
 186           BA  0.0468
 187           LG    0.25    0.25    4.03    0.56      45
 188           UC   0.250   0.178
 189           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 190           UA     100
               *

 191           KK   SB07W  DIVERT
 192           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB07W
 193           KM        FIRST FLUSH VOL = 1.060 AC-FT
 194           DT  RET07W   1.060
 195           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 196           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *
```

```
LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 197          KK  R7W-8W   ROUTE
 198          KM       HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 199          KM       HE MODIFIED ROUTE TO LOOP 101 W OF HAYDEN
 200          RK    3754  0.0013   0.03            TRAP      46       4
              *

 201          KK   SB8NW   BASIN
 202          KM       BASIN JUST SOUTH OF LEGACY - FUTURE MOB SITE
 203          BA  0.0147
 204          LG    0.10    0.25    4.03    0.61      80
 205          UC   0.171   0.131
 206          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 207          UA     100
              *

 208          KK   SB8NW  DIVERT
 209          KM       DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NW
 210          KM       FIRST FLUSH VOL = 0.332 AC-FT
 211          DT  RET8NW   0.332
 212          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 213          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 214          KK  R8NW8N   ROUTE
 215          KM       ROUTE NEW SUB BASIN SB8NW TO OUTLET AT CAVASSON BLVD
 216          KM       NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 217          RK     643  0.0113   0.018           TRAP      46       4
              *

 218          KK   SB8NE   BASIN
 219          KM       SITE JUST SOUTH OF LEGACY AND WEST OF HAYDEN - FUTURE RETAIL
 220          BA   0.015
 221          LG    0.10    0.25    4.03    0.61      80
 222          UC   0.164   0.113
 223          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 224          UA     100
              *

 225          KK   SB8NE  DIVERT
 226          KM       DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NE
 227          KM       FIRST FLUSH VOL = 0.340 AC-FT
 228          DT  RET8NE   0.340
 229          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 230          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 231          KK  R8NE8N   ROUTE
 232          KM       ROUTE NEW SUB BASIN SB8NE TO OUTLET AT CAVASSON BLVD
 233          KM       NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 234          RK     622   0.011   0.018           TRAP      46       4
              *
```



```
LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 235          KK   SB8SW   BASIN
 236          KM       SITE JUST NORTH OF CAVASSON BLVD EAST OF CLARET DR - FUTURE MF
 237          BA  0.0129
 238          LG    0.10    0.25    4.03    0.61      80
 239          UC   0.149   0.110
 240          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 241          UA     100
              *

 242          KK   SB8SW  DIVERT
 243          KM       DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SW
 244          KM       FIRST FLUSH VOL = 0.292 AC-FT
 245          DT  RET8SW   0.292
 246          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 247          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 248          KK   SB8SE   BASIN
 249          KM       SITE JUST NORTH OF CAVASSON AND WEST OF HAYDEN - FUTURE HOTEL OR RETAIL
 250          BA  0.0131
 251          LG    0.10    0.25    4.03    0.61      80
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```
 252           UC   0.165   0.137
 253           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 254           UA     100
               *

 255           KK   SB8SE  DIVERT
 256           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SE
 257           KM        FIRST FLUSH VOL = 0.297 AC-FT
 258           DT  RET8SE   0.297
 259           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 260           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 261           KK  CP-08N COMBINE
 262           KM        CP AT INLET ON NORTH SIDE OF CAVASSON - ENTERS EXIST STORM DRAIN
 263           HC       4  0.0557
               *

 264           KK  R8N-8W   ROUTE
 265           KM        ROUTE NEW SUB BASINS SB8NE SB8NW SB8SW SB8SE TO CP-08W
 266           KM        ROUTE IN STORM DRAIN
 267           RK    1592   0.015   0.013            CIRC       3
               *

 268           KK   SB08W   BASIN
 269           BA  0.0589
 270           LG    0.10    0.25    4.03    0.61      80
 271           UC   0.262   0.177
 272           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 273           UA     100
               *
```



```
LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 274           KK   SB08W  DIVERT
 275           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB08W
 276           KM        FIRST FLUSH VOL REQ = 1.335 AC-FT, PROVIDED = 1.415
 277           KM        THIS IS EXISTING AND WAS BUILT IN PHASE 1
 278           DT   RET8W   1.415
 279           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 280           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 281           KK  CP-08W COMBINE
 282           HC       3  0.1614
               *

 283           KK  R8W-4N   ROUTE
 284           KM         STAGE STORAGE ROUTING UNDER THE LOOP 101 -HE
 285           RS       1    STOR    0.00       0
 286           SV   0.000   0.642   1.290   2.207   4.066   4.066
 287           SE  1608.0  1609.0  1610.0  1611.0  1612.0  1613.0
 288           SQ   0.000   8.330   28.52   150.5   300.0   360.0
 289           SE  1608.0  1609.0  1610.0  1611.0  1613.2  1613.3
               *

 290           KK  SB02E1   BASIN
 291           KM        BASIN 2E HAD TO BE SPLIT AT CENTER POINT DUE TO FLOW UNDER LOOP 101
 292           KM        THIS IS THE EAST SPLIT -HE
 293           BA  0.0252
 294           LG    0.10    0.25    4.03    0.61      80
 295           UC   0.224   0.208
 296           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 297           UA     100
               *

 298           KK  SB02E1  DIVERT
 299           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E1
 300           KM        FIRST FLUSH VOL REQ = 0.570 AC-FT -HE
 301           DT  RET2E1   0.570
 302           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 303           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 304           KK   CP-4N COMBINE
 305           HC       2  0.1866
               *
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```
 306           KK   R4N-4   ROUTE
 307           RK    2840  0.0130   0.030            TRAP   2.000    0.00
               *

 308           KK    SB04   BASIN
 309           BA   0.144
 310           LG    0.14    0.25    4.60    0.44      61
 311           UC   0.305   0.199
 312           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 313           UA     100
               *
1                                          HEC-1 INPUT                                          PAGE  9

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 314           KK    CP-4 COMBINE
 315           HC       2
               *

 316           KK    R4-5   ROUTE
 317           KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 318           RK    1992  0.0013   0.03             TRAP      39       4
               *

 319           KK   CP-05 COMBINE
 320           KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 321           KM        COMBINE CP-05A AND R4-5
 322           HC       2
               *

 323           KK    R5-6   ROUTE
 324           KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 325           RK    1694  0.0015   0.03             TRAP     188       4
               *

 326           KK    SB06   BASIN
 327           BA   0.136
 328           LG    0.16    0.25    4.55    0.45      53
 329           UC   0.321   0.246
 330           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 331           UA     100
               *

 332           KK   CP-06 COMBINE
 333           HC       2
               *

 334           KK   SB07E   BASIN
 335           BA   0.026
 336           LG    0.10    0.25    4.03    0.74       0
 337           UC   0.574   0.527
 338           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 339           UA     100
               *

 340           KK  R7E-8E   ROUTE
 341           RK    3662  0.0130   0.030            TRAP   2.000    0.00
               *

 342           KK   SB08E   BASIN
 343           BA   0.078
 344           LG    0.10    0.25    4.08    0.72       0
 345           UC   0.798   0.652
 346           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 347           UA     100
               *
1                                          HEC-1 INPUT                                          PAGE 10

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 348           KK   SB09N   BASIN
 349           BA   0.028
 350           LG    0.10    0.25    4.79    0.49       0
 351           UC   0.740   1.111
 352           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 353           UA     100
               *
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 354            KK  R9NR8E   ROUTE
 355            RK    2586  0.0013   0.020            TRAP   2.000    0.00
                *

 356            KK  CP-08E COMBINE
 357            HC       3
                *

 358            KK  R8E-R9   ROUTE
 359            RK     501  0.0013   0.020            TRAP   2.000    0.00
                *

 360            KK    SB09   BASIN
 361            BA   0.086
 362            LG    0.10    0.25    4.79    0.49       0
 363            UC   0.683   0.518
 364            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 365            UA     100
                *

 366            KK   CP-09 COMBINE
 367            HC       2
                *

 368            KK   PWRCH
 369            KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
 370            KM        10-YR, 6-HR, DERIVED FROM 100-YR DATA HYDROGRAPHS
 371            BA     7.0
 372            QI       0       0       0       0       0       0       0       0       0       0
 373            QI       0       0       0       0       0    0.07   11.70   33.57  285.55  257.89
 374            QI  236.82  258.26  406.55  486.55  461.36  425.60  409.84  395.31  391.77  376.42
 375            QI  346.76  376.76  276.32  247.65  220.91  197.44  178.64  155.41  139.22  129.78
 376            QI  113.97  102.76   96.30   85.88    79.7   72.75   65.45   59.94   54.69   51.22
 377            QI    52.3   44.56   41.87   42.77   36.93   34.05   32.15   29.01   27.72   27.03
 378            QI   25.05   23.86   22.04   20.81   19.27   18.55   17.90   20.82   16.03   15.23
                *

 379            KK  BINFLO
 380            KM         TOTAL INFLOW INTO BASIN 53R.
 381            HC       2
                *
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```
 382            KK   BASIN
 383            KM    BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
 384            KM    ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
 385            KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
 386            KM        THE OUTLET PIPES ARE INLET CONTROLLED.
 387            RS       1    STOR       0
 388            SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
 389            SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
 390            SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
 391            SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
 392            SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
 393            SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
                *

 394            KK  BSNRT1
 395            KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
 396            KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
 397            KM        EXCESS CAPACITY.
 398            RK     550  0.0052   0.024            CIRC     7.0
                *

 399            KK  BSNRT2
 400            KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BAS 53R OUTFALL)
 401            RK    1200  0.0077   0.013            CIRC     7.0
                *

 402            KK    SB10   BASIN
 403            BA   0.040
 404            LG    0.15    0.25    4.25    0.55      55
 405            UC   0.233   0.161
 406            UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 407            UA     100
                *
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          408           KK   CP-10 COMBINE
          409           KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
          410           HC       2
                        *

          411           KK  R10-11   ROUTE
          412           KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
          413           RK    1277  0.0014   0.03             TRAP     220       4
                        *

          414           KK    SB11   BASIN
          415           BA   0.071
          416           LG    0.15    0.25    4.15    0.58      55
          417           UC   0.296   0.232
          418           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          419           UA     100
                        *
1                                                     HEC-1 INPUT                                          PAGE 12

         LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


          420           KK   CP-11 COMBINE
          421           KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
          422           HC       2
                        *
          423           ZZ
1
                 SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

   NO.      (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

    38      76THST
                 .
                 .
    50           .       SB01A
                 .           .
                 .           .
    63           .           .------->  RET01A
    60           .       SB01A
                 .           .
                 .           .
    66        CP-1............
                 V
                 V
    69      R1A-2W
                 .
                 .
    72           .      SB02NW
                 .           .
                 .           .
    83           .           .------->  RET2NW
    80           .      SB02NW
                 .           .
                 .           .
    88      CP-02W............
                 V
                 V
    91       R2W-2
                 .
                 .
    93           .       SB01C
                 .           .
                 .           .
   102           .           .------->  RET01C
    99           .       SB01C
                 .           V
                 .           V
   105           .      R1C-1B
                 .           .
                 .           .
   107           .           .       SB01B
                 .           .           .
                 .           .           .
   116           .           .           .------->  RET01B
   113           .           .       SB01B
                 .           .           .
                 .           .           .
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119           .      CP-01B............
              .           V
              .           V
122           .      R1B-2N
              .           .
              .           .
124           .           .      SB02NE
              .           .           .
              .           .           .
133           .           .           .------->  RET2NE
130           .           .      SB02NE
              .           .           .
              .           .           .
136           .      CP-02N............
              .           V
              .           V
139           .       R2N-2
              .           .
              .           .
141           .           .      SB02E2
              .           .           .
              .           .           .
152           .           .           .------->  RET2E2
149           .           .      SB02E2
              .           .           .
              .           .           .
155           .           .           .       SB02W
              .           .           .           .
              .           .           .           .
161       CP-02....................................
              V
              V
163        R2-3
              .
              .
166           .        SB03
              .           .
              .           .
172       CP-03............
              V
              V
174        R3-5
              .
              .
177           .        SB05
              .           .
              .           .
183       CP-5A............
              .
              .
185           .       SB07W
              .           .
              .           .
194           .           .------->  RET07W
191           .       SB07W
              .           V
              .           V
197           .      R7W-8W
              .           .
              .           .
201           .           .       SB8NW
              .           .           .
              .           .           .
211           .           .           .------->  RET8NW
208           .           .       SB8NW
              .           .           V
              .           .           V
214           .           .      R8NW8N
              .           .           .
              .           .           .
218           .           .           .       SB8NE
              .           .           .           .
              .           .           .           .
228           .           .           .           .------->  RET8NE
225           .           .           .       SB8NE
              .           .           .           V
              .           .           .           V
231           .           .           .      R8NE8N
              .           .           .           .
              .           .           .           .
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235           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
245           .           .           .           .           .------->  RET8SW
242           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
248           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
258           .           .           .           .           .           .------->  RET8SE
255           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
261           .           .      CP-08N....................................
              .           .           V
              .           .           V
264           .           .      R8N-8W
              .           .           .
              .           .           .
268           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
278           .           .           .           .------->   RET8W
274           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
281           .      CP-08W........................
              .           V
              .           V
283           .      R8W-4N
              .           .
              .           .
290           .           .      SB02E1
              .           .           .
              .           .           .
301           .           .           .------->  RET2E1
298           .           .      SB02E1
              .           .           .
              .           .           .
304           .       CP-4N............
              .           V
              .           V
306           .       R4N-4
              .           .
              .           .
308           .           .        SB04
              .           .           .
              .           .           .
314           .        CP-4............
              .           V
              .           V
316           .        R4-5
              .           .
              .           .
319       CP-05............
              V
              V
323        R5-6
              .
              .
326           .        SB06
              .           .
              .           .
332       CP-06............
              .
              .
334           .       SB07E
              .           V
              .           V
340           .      R7E-8E
              .           .
              .           .
342           .           .       SB08E
              .           .           .
              .           .           .
348           .           .           .       SB09N
              .           .           .           V
              .           .           .           V
354           .           .           .      R9NR8E
```

```
              .           .           .           .
              .           .           .           .
  356         .      CP-08E.......................
              .           V
              .           V
  358         .      R8E-R9
              .           .
              .           .
  360         .           .         SB09
              .           .            .
              .           .            .
  366         .       CP-09............
              .           .
              .           .
  368         .           .        PWRCH
              .           .            .
              .           .            .
  379         .      BINFLO............
              .           V
              .           V
  382         .       BASIN
              .           V
              .           V
  394         .      BSNRT1
              .           V
              .           V
  399         .      BSNRT2
              .           .
              .           .
  402         .           .         SB10
              .           .            .
              .           .            .
  408         .       CP-10............
              .           V
              .           V
  411         .      R10-11
              .           .
              .           .
  414         .           .         SB11
              .           .            .
              .           .            .
  420         .       CP-11............

 (***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1*****************************************                                                *****************************************
 *                                       *                                                *                                       *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                *    U.S. ARMY CORPS OF ENGINEERS       *
 *              JUN   1998               *                                                *    HYDROLOGIC ENGINEERING CENTER      *
 *           VERSION 4.1                 *                                                *          609 SECOND STREET            *
 *                                       *                                                *       DAVIS, CALIFORNIA 95616         *
 *  RUN DATE   20JUL23  TIME  09:20:32   *                                                *           (916) 756-1104              *
 *                                       *                                                *                                       *
 *****************************************                                                *****************************************
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                                    CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
                                    POWERLINE CORRIDOR
                                    RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                                    INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                                      10-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                                       PIMA ROAD FLOWS.

                                    PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                                      MODELERS: RK, MW

                                    REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
                                      MODELERS: MSW, ES
                                      REVISIONS NOTED WITH HE
                                       ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
                                       THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
                                       UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.

                                    REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/20/23
                                       MODELERS: MSW, TSW
                                       REVISIONS NOTED WITH HE (HUBBARD ENGINEERING)
                                       BASIN 2E WAS SPLIT AT CENTER POINT DUE TO FLOW UNDER 101
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25 IO         OUTPUT CONTROL VARIABLES
                    IPRNT          5  PRINT CONTROL
                    IPLOT          0  PLOT CONTROL
                    QSCAL         0.  HYDROGRAPH PLOT SCALE

   IT         HYDROGRAPH TIME DATA
                     NMIN          3  MINUTES IN COMPUTATION INTERVAL
                    IDATE      1    0  STARTING DATE
                    ITIME       0000  STARTING TIME
                       NQ       1000  NUMBER OF HYDROGRAPH ORDINATES
                   NDDATE      3    0  ENDING DATE
                   NDTIME       0157  ENDING TIME
                   ICENT          19  CENTURY MARK

              COMPUTATION INTERVAL    0.05 HOURS
                   TOTAL TIME BASE   49.95 HOURS

       ENGLISH UNITS
            DRAINAGE AREA         SQUARE MILES
            PRECIPITATION DEPTH   INCHES
            LENGTH, ELEVATION     FEET
            FLOW                  CUBIC FEET PER SECOND
            STORAGE VOLUME        ACRE-FEET
            SURFACE AREA          ACRES
            TEMPERATURE           DEGREES FAHRENHEIT

26 JD         INDEX STORM NO. 1
                     STRM       1.82  PRECIPITATION DEPTH
                     TRDA       0.00  TRANSPOSITION DRAINAGE AREA

27 PI           PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                   0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
                   0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

30 JD         INDEX STORM NO. 2
                     STRM       1.81  PRECIPITATION DEPTH
                     TRDA       0.50  TRANSPOSITION DRAINAGE AREA

31 PI           PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                   0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
                   0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

34 JD         INDEX STORM NO. 3
                     STRM       1.77  PRECIPITATION DEPTH
                     TRDA       2.80  TRANSPOSITION DRAINAGE AREA

35 PI           PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                   0.04      0.04      0.04      0.04      0.04      0.05      0.05      0.05      0.05      0.05
                   0.03      0.03      0.03      0.03      0.03      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+	HYDROGRAPH AT	76THST	32.	4.50	10.	3.	1.	0.24		
+	HYDROGRAPH AT	SB01A	59.	4.25	7.	2.	1.	0.06		
+	DIVERSION TO	RET01A	48.	4.15	3.	1.	0.	0.06		
+	HYDROGRAPH AT	SB01A	59.	4.25	4.	1.	0.	0.06		
+	2 COMBINED AT	CP-1	86.	4.25	14.	4.	2.	0.30		
+	ROUTED TO	R1A-2W	86.	4.30	14.	4.	2.	0.30		
+	HYDROGRAPH AT	SB02NW	23.	4.10	2.	1.	0.	0.02		
+	DIVERSION TO	RET2NW	18.	4.00	1.	0.	0.	0.02		
+	HYDROGRAPH AT	SB02NW	23.	4.10	1.	0.	0.	0.02		
+	2 COMBINED AT	CP-02W	97.	4.30	15.	4.	2.	0.31		
+	ROUTED TO	R2W-2	97.	4.30	15.	4.	2.	0.31		
+	HYDROGRAPH AT	SB01C	26.	4.15	3.	1.	0.	0.03		
+	DIVERSION TO	RET01C	24.	4.10	1.	0.	0.	0.03		
+	HYDROGRAPH AT	SB01C	26.	4.15	2.	0.	0.	0.03		
+	ROUTED TO	R1C-1B	25.	4.20	2.	0.	0.	0.03		
+	HYDROGRAPH AT	SB01B	25.	4.15	3.	1.	0.	0.02		
	DIVERSION TO									

+		RET01B	22.	4.05	1.	0.	0.	0.02
+	HYDROGRAPH AT	SB01B	25.	4.15	2.	0.	0.	0.02
+	2 COMBINED AT	CP-01B	48.	4.20	3.	1.	0.	0.05
+	ROUTED TO	R1B-2N	47.	4.20	3.	1.	0.	0.05
+	HYDROGRAPH AT	SB02NE	29.	4.20	4.	1.	0.	0.03
+	DIVERSION TO	RET2NE	27.	4.15	2.	0.	0.	0.03
+	HYDROGRAPH AT	SB02NE	27.	4.25	2.	1.	0.	0.03
+	2 COMBINED AT	CP-02N	73.	4.25	6.	1.	1.	0.09
+	ROUTED TO	R2N-2	72.	4.25	6.	1.	1.	0.09
+	HYDROGRAPH AT	SB02E2	22.	4.15	3.	1.	0.	0.02
+	DIVERSION TO	RET2E2	12.	3.95	1.	0.	0.	0.02
+	HYDROGRAPH AT	SB02E2	22.	4.15	2.	1.	0.	0.02
+	HYDROGRAPH AT	SB02W	16.	4.50	2.	1.	0.	0.06
+	4 COMBINED AT	CP-02	189.	4.30	25.	7.	3.	0.18
+	ROUTED TO	R2-3	187.	4.35	25.	7.	3.	0.18
+	HYDROGRAPH AT	SB03	47.	4.15	6.	2.	1.	0.05
+	2 COMBINED AT	CP-03	220.	4.35	31.	8.	4.	0.23
+	ROUTED TO	R3-5	214.	4.55	31.	8.	4.	0.23
+	HYDROGRAPH AT	SB05	134.	4.10	15.	4.	2.	0.13
+	2 COMBINED AT	CP-5A	250.	4.55	45.	12.	6.	0.36
+	HYDROGRAPH AT	SB07W	54.	4.15	5.	1.	1.	0.05
+	DIVERSION TO	RET07W	41.	4.05	2.	1.	0.	0.05
+	HYDROGRAPH AT	SB07W	54.	4.15	3.	1.	0.	0.05
+	ROUTED TO	R7W-8W	51.	4.65	5.	1.	1.	0.05
+	HYDROGRAPH AT	SB8NW	24.	4.05	2.	1.	0.	0.01
+	DIVERSION TO	RET8NW	8.	3.80	1.	0.	0.	0.01
+	HYDROGRAPH AT	SB8NW	24.	4.05	2.	0.	0.	0.01

	ROUTED TO							
+		R8NW8N	24.	4.10	2.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8NE	25.	4.05	2.	1.	0.	0.01
	DIVERSION TO							
+		RET8NE	8.	3.80	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8NE	25.	4.05	2.	0.	0.	0.01
	ROUTED TO							
+		R8NE8N	25.	4.10	2.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SW	22.	4.05	2.	1.	0.	0.01
	DIVERSION TO							
+		RET8SW	7.	3.75	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SW	22.	4.05	2.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SE	21.	4.05	2.	1.	0.	0.01
	DIVERSION TO							
+		RET8SE	7.	3.80	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SE	21.	4.05	2.	0.	0.	0.01
	4 COMBINED AT							
+		CP-08N	90.	4.05	7.	2.	1.	0.06
	ROUTED TO							
+		R8N-8W	89.	4.10	7.	2.	1.	0.06
	HYDROGRAPH AT							
+		SB08W	85.	4.15	10.	2.	1.	0.06
	DIVERSION TO							
+		RET8W	36.	3.90	3.	1.	0.	0.06
	HYDROGRAPH AT							
+		SB08W	85.	4.15	7.	2.	1.	0.06
	3 COMBINED AT							
+		CP-08W	172.	4.10	18.	5.	2.	0.16
	ROUTED TO							
+		R8W-4N	137.	4.20	18.	5.	2.	0.16
	HYDROGRAPH AT							
+		SB02E1	35.	4.10	4.	1.	0.	0.03
	DIVERSION TO							
+		RET2E1	13.	3.85	1.	0.	0.	0.03
	HYDROGRAPH AT							
+		SB02E1	35.	4.10	3.	1.	0.	0.03
	2 COMBINED AT							
+		CP-4N	167.	4.20	21.	5.	3.	0.19
	ROUTED TO							
+		R4N-4	164.	4.20	21.	5.	3.	0.19
	HYDROGRAPH AT							
+		SB04	178.	4.10	20.	5.	2.	0.14
	2 COMBINED AT							
+		CP-4	312.	4.15	40.	10.	5.	0.33
	ROUTED TO							
+		R4-5	310.	4.25	40.	10.	5.	0.33
	2 COMBINED AT							
+		CP-05	397.	4.50	82.	22.	11.	0.69

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          ROUTED TO
+                        R5-6       387.    4.60          82.         22.         11.       0.69

          HYDROGRAPH AT
+                        SB06       145.    4.10          17.          4.          2.       0.14

          2 COMBINED AT
+                       CP-06       440.    4.35          96.         25.         12.       0.82

          HYDROGRAPH AT
+                       SB07E         9.    4.40           1.          0.          0.       0.03

          ROUTED TO
+                      R7E-8E         8.    4.50           1.          0.          0.       0.03

          HYDROGRAPH AT
+                       SB08E        21.    4.55           3.          1.          0.       0.08

          HYDROGRAPH AT
+                       SB09N         6.    4.55           2.          0.          0.       0.03

          ROUTED TO
+                      R9NR8E         6.    4.65           2.          0.          0.       0.03

          3 COMBINED AT
+                      CP-08E        34.    4.60           6.          2.          1.       0.13

          ROUTED TO
+                      R8E-R9        34.    4.60           6.          2.          1.       0.13

          HYDROGRAPH AT
+                        SB09        34.    4.45           5.          1.          1.       0.09

          2 COMBINED AT
+                       CP-09        66.    4.55          11.          3.          1.       0.22

          HYDROGRAPH AT
+                       PWRCH       487.    5.75         285.         91.         50.       7.00

          2 COMBINED AT
+                      BINFLO       484.    5.75         283.         90.         50.       7.22

          ROUTED TO
+                       BASIN       171.    9.10         156.         85.         48.       7.22

          ROUTED TO
+                      BSNRT1       171.    9.10         156.         85.         48.       7.22

          ROUTED TO
+                      BSNRT2       171.    9.10         156.         85.         48.       7.22

          HYDROGRAPH AT
+                        SB10        52.    4.05           5.          1.          1.       0.04

          2 COMBINED AT
+                       CP-10       171.    9.10         156.         85.         49.       7.26

          ROUTED TO
+                      R10-11       171.    9.20         156.         85.         48.       7.26

          HYDROGRAPH AT
+                        SB11        77.    4.10           9.          2.          1.       0.07

          2 COMBINED AT
+                       CP-11       171.    9.20         156.         85.         49.       7.33
1
                                              SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
                                                  (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)
                                                                                 INTERPOLATED TO
                                                                              COMPUTATION INTERVAL
            ISTAQ    ELEMENT      DT       PEAK    TIME TO     VOLUME       DT       PEAK    TIME TO     VOLUME
                                                    PEAK                                      PEAK

                                 (MIN)     (CFS)     (MIN)      (IN)      (MIN)     (CFS)     (MIN)       (IN)

           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R1A-2W  MANE         0.28     86.74    255.73      0.51      3.00     86.50    258.00       0.51
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8110E+01 EXCESS=0.0000E+00 OUTFLOW=0.8108E+01 BASIN STORAGE=0.2428E-02 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1A-2W  MANE         0.22     86.23    255.60      0.50      3.00     85.99    258.00        0.50


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8082E+01 EXCESS=0.0000E+00 OUTFLOW=0.8081E+01 BASIN STORAGE=0.2428E-02 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1A-2W  MANE         0.23     56.86    261.41      0.46      3.00     56.60    261.00        0.46


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7389E+01 EXCESS=0.0000E+00 OUTFLOW=0.7388E+01 BASIN STORAGE=0.2428E-02 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2W-2  MANE         0.41     97.65    258.39      0.54      3.00     97.55    258.00        0.54


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8773E+01 EXCESS=0.0000E+00 OUTFLOW=0.8771E+01 BASIN STORAGE=0.3356E-02 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R2W-2  MANE         0.41     96.96    258.16      0.54      3.00     96.86    258.00        0.54


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8738E+01 EXCESS=0.0000E+00 OUTFLOW=0.8735E+01 BASIN STORAGE=0.3354E-02 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R2W-2  MANE         0.36     63.48    261.95      0.48      3.00     62.70    264.00        0.48


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7822E+01 EXCESS=0.0000E+00 OUTFLOW=0.7820E+01 BASIN STORAGE=0.3350E-02 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1C-1B  MANE         0.47     26.07    249.94      0.64      3.00     25.45    252.00        0.64


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9235E+00 EXCESS=0.0000E+00 OUTFLOW=0.9234E+00 BASIN STORAGE=0.1159E-13 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1C-1B  MANE         0.40     25.48    250.16      0.63      3.00     25.16    252.00        0.64


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9114E+00 EXCESS=0.0000E+00 OUTFLOW=0.9112E+00 BASIN STORAGE=0.1161E-13 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1C-1B  MANE         0.40     12.02    255.91      0.42      3.00     11.53    258.00        0.42


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6045E+00 EXCESS=0.0000E+00 OUTFLOW=0.6050E+00 BASIN STORAGE=0.1171E-13 PERCENT ERROR=  -0.1


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1B-2N  MANE         1.00     48.05    253.91      0.64      3.00     47.04    252.00        0.64


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1748E+01 EXCESS=0.0000E+00 OUTFLOW=0.1752E+01 BASIN STORAGE=0.3293E-11 PERCENT ERROR=  -0.2


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1B-2N  MANE         0.96     47.49    253.51      0.63      3.00     46.32    252.00        0.63


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1727E+01 EXCESS=0.0000E+00 OUTFLOW=0.1726E+01 BASIN STORAGE=0.3440E-11 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1B-2N  MANE         1.20     21.35    259.25      0.42      3.00     21.16    258.00        0.42


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1144E+01 EXCESS=0.0000E+00 OUTFLOW=0.1146E+01 BASIN STORAGE=0.3398E-11 PERCENT ERROR=  -0.2
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          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2N-2  MANE         0.38     73.38    255.29      0.64      3.00     73.19    255.00        0.64

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2890E+01 EXCESS=0.0000E+00 OUTFLOW=0.2895E+01 BASIN STORAGE=0.5835E-06 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R2N-2  MANE         0.42     72.58    255.46      0.63      3.00     72.21    255.00        0.63

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2852E+01 EXCESS=0.0000E+00 OUTFLOW=0.2855E+01 BASIN STORAGE=0.4437E-06 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R2N-2  MANE         0.51     34.33    261.97      0.42      3.00     34.12    261.00        0.42

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1894E+01 EXCESS=0.0000E+00 OUTFLOW=0.1895E+01 BASIN STORAGE=0.4997E-06 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R2-3  MANE         2.37    190.46    262.56      1.43      3.00    187.15    261.00        1.43

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1391E+02 EXCESS=0.0000E+00 OUTFLOW=0.1387E+02 BASIN STORAGE=0.3148E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R2-3  MANE         2.39    188.45    262.33      1.42      3.00    187.10    261.00        1.42

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1380E+02 EXCESS=0.0000E+00 OUTFLOW=0.1377E+02 BASIN STORAGE=0.3148E-01 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R2-3  MANE         2.98    108.61    268.44      1.11      3.00    107.22    270.00        1.11

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1081E+02 EXCESS=0.0000E+00 OUTFLOW=0.1078E+02 BASIN STORAGE=0.3148E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R3-5  MANE         3.00    218.33    271.57      1.39      3.00    214.58    273.00        1.39

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1702E+02 EXCESS=0.0000E+00 OUTFLOW=0.1699E+02 BASIN STORAGE=0.6955E-01 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R3-5  MANE         3.00    214.12    272.95      1.38      3.00    213.94    273.00        1.39

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1688E+02 EXCESS=0.0000E+00 OUTFLOW=0.1696E+02 BASIN STORAGE=0.6968E-01 PERCENT ERROR=  -0.9

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R3-5  MANE         3.32    124.97    281.91      1.09      3.00    124.88    282.00        1.10

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1344E+02 EXCESS=0.0000E+00 OUTFLOW=0.1341E+02 BASIN STORAGE=0.7428E-01 PERCENT ERROR=  -0.3

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7W-8W  MANE         1.68     52.79    276.79      0.96      3.00     51.01    279.00        0.96

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1600E+01 EXCESS=0.0000E+00 OUTFLOW=0.2399E+01 BASIN STORAGE=0.2981E-03 PERCENT ERROR= -50.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R7W-8W  MANE         1.69     52.24    276.99      0.95      3.00     50.66    279.00        0.95

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1579E+01 EXCESS=0.0000E+00 OUTFLOW=0.2373E+01 BASIN STORAGE=0.2851E-03 PERCENT ERROR= -50.4

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R7W-8W  MANE         0.57     23.36    290.76      0.62      3.00     23.27    291.00        0.63
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1047E+01 EXCESS=0.0000E+00 OUTFLOW=0.1550E+01 BASIN STORAGE=0.2509E-03 PERCENT ERROR= -48.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NW8N  MANE         0.90     23.67    245.12      1.12      3.00     23.61    246.00        1.12

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8762E+00 EXCESS=0.0000E+00 OUTFLOW=0.8785E+00 BASIN STORAGE=0.1427E-06 PERCENT ERROR=  -0.3

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8NW8N  MANE         0.89     23.51    245.06      1.11      3.00     23.45    246.00        1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8684E+00 EXCESS=0.0000E+00 OUTFLOW=0.8709E+00 BASIN STORAGE=0.1495E-06 PERCENT ERROR=  -0.3

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8NW8N  MANE         0.99     12.76    245.64      1.01      3.00     12.75    246.00        1.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7895E+00 EXCESS=0.0000E+00 OUTFLOW=0.7947E+00 BASIN STORAGE=0.1506E-06 PERCENT ERROR=  -0.7

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NE8N  MANE         0.90     25.10    244.37      1.12      3.00     24.86    246.00        1.12

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8929E+00 EXCESS=0.0000E+00 OUTFLOW=0.8983E+00 BASIN STORAGE=0.1026E-06 PERCENT ERROR=  -0.6

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8NE8N  MANE         0.87     24.95    244.34      1.11      3.00     24.69    246.00        1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8849E+00 EXCESS=0.0000E+00 OUTFLOW=0.8866E+00 BASIN STORAGE=0.1149E-06 PERCENT ERROR=  -0.2

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8NE8N  MANE         1.02     13.29    245.08      1.01      3.00     13.20    246.00        1.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8044E+00 EXCESS=0.0000E+00 OUTFLOW=0.8070E+00 BASIN STORAGE=0.1185E-06 PERCENT ERROR=  -0.3

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8N-8W  MANE         0.50     90.26    244.05      1.12      3.00     89.79    246.00        1.12

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3323E+01 EXCESS=0.0000E+00 OUTFLOW=0.3326E+01 BASIN STORAGE=0.1332E-07 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8N-8W  MANE         0.50     89.65    244.04      1.11      3.00     89.18    246.00        1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3296E+01 EXCESS=0.0000E+00 OUTFLOW=0.3298E+01 BASIN STORAGE=0.1340E-07 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8N-8W  MANE         0.71     48.48    244.61      1.01      3.00     48.33    246.00        1.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2995E+01 EXCESS=0.0000E+00 OUTFLOW=0.2995E+01 BASIN STORAGE=0.1327E-07 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R4N-4  MANE         0.60    167.98    253.40      1.07      3.00    165.34    252.00        1.07

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1065E+02 EXCESS=0.0000E+00 OUTFLOW=0.1066E+02 BASIN STORAGE=0.1165E-03 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R4N-4  MANE         0.60    166.44    252.95      1.06      3.00    163.74    252.00        1.06

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1055E+02 EXCESS=0.0000E+00 OUTFLOW=0.1056E+02 BASIN STORAGE=0.1190E-03 PERCENT ERROR=  -0.1
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          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R4N-4  MANE         0.77     98.89    256.01      0.91      3.00     98.41    255.00        0.90

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9006E+01 EXCESS=0.0000E+00 OUTFLOW=0.9007E+01 BASIN STORAGE=0.1090E-03 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R4-5  MANE         2.76    313.86    256.89      1.18      3.00    311.81    255.00        1.18

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2075E+02 EXCESS=0.0000E+00 OUTFLOW=0.2078E+02 BASIN STORAGE=0.3871E-03 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R4-5  MANE         2.76    311.07    257.07      1.17      3.00    309.97    255.00        1.17

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2057E+02 EXCESS=0.0000E+00 OUTFLOW=0.2061E+02 BASIN STORAGE=0.3769E-03 PERCENT ERROR=  -0.2

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R4-5  MANE         3.00    182.72    261.31      1.01      3.00    182.52    261.00        1.01

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1783E+02 EXCESS=0.0000E+00 OUTFLOW=0.1784E+02 BASIN STORAGE=0.3987E-03 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R5-6  MANE         2.96    426.24    261.67      1.24      3.00    421.86    261.00        1.24

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4537E+02 EXCESS=0.0000E+00 OUTFLOW=0.4529E+02 BASIN STORAGE=0.5641E-01 PERCENT ERROR=   0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R5-6  MANE         2.97    428.96    276.76      1.23      3.00    420.18    276.00        1.23

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4515E+02 EXCESS=0.0000E+00 OUTFLOW=0.4505E+02 BASIN STORAGE=0.5641E-01 PERCENT ERROR=   0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R5-6  MANE         2.64    264.03    286.62      1.02      3.00    262.76    288.00        1.02

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3753E+02 EXCESS=0.0000E+00 OUTFLOW=0.3745E+02 BASIN STORAGE=0.6479E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7E-8E  MANE         2.48      8.47    267.86      0.41      3.00      8.45    270.00        0.41

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5640E+00 EXCESS=0.0000E+00 OUTFLOW=0.5642E+00 BASIN STORAGE=0.1424E-04 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R7E-8E  MANE         2.49      8.36    268.11      0.40      3.00      8.32    270.00        0.40

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5555E+00 EXCESS=0.0000E+00 OUTFLOW=0.5565E+00 BASIN STORAGE=0.1227E-04 PERCENT ERROR=  -0.2

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R7E-8E  MANE         3.00      0.79    281.23      0.04      3.00      0.79    282.00        0.04

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5136E-01 EXCESS=0.0000E+00 OUTFLOW=0.5564E-01 BASIN STORAGE=0.1218E-04 PERCENT ERROR=  -8.4

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R9NR8E  MANE         3.00      6.31    281.03      0.51      3.00      6.30    279.00        0.51

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7576E+00 EXCESS=0.0000E+00 OUTFLOW=0.7564E+00 BASIN STORAGE=0.1663E-04 PERCENT ERROR=   0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R9NR8E  MANE         3.00      6.24    281.18      0.50      3.00      6.22    279.00        0.50
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7487E+00 EXCESS=0.0000E+00 OUTFLOW=0.7475E+00 BASIN STORAGE=0.1891E-04 PERCENT ERROR=   0.2


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R9NR8E  MANE         3.00      1.69    287.45      0.14      3.00      1.68    288.00        0.14


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2051E+00 EXCESS=0.0000E+00 OUTFLOW=0.2049E+00 BASIN STORAGE=0.1591E-04 PERCENT ERROR=   0.1


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8E-R9  MANE         0.31     34.65    276.05      0.43      3.00     34.65    276.00        0.43


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3043E+01 EXCESS=0.0000E+00 OUTFLOW=0.3043E+01 BASIN STORAGE=0.7386E-05 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8E-R9  MANE         0.31     34.16    276.08      0.43      3.00     34.16    276.00        0.43


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3001E+01 EXCESS=0.0000E+00 OUTFLOW=0.3002E+01 BASIN STORAGE=0.7554E-05 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8E-R9  MANE         0.73      4.44    286.10      0.06      3.00      4.42    285.00        0.06


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4419E+00 EXCESS=0.0000E+00 OUTFLOW=0.4419E+00 BASIN STORAGE=0.7013E-05 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT1  MANE         0.43    176.85    541.04      0.53      3.00    176.85    543.00        0.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2055E+03 EXCESS=0.0000E+00 OUTFLOW=0.2055E+03 BASIN STORAGE=0.2436E-01 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            BSNRT1  MANE         0.43    176.79    541.14      0.53      3.00    176.79    543.00        0.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2054E+03 EXCESS=0.0000E+00 OUTFLOW=0.2054E+03 BASIN STORAGE=0.2436E-01 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            BSNRT1  MANE         0.47    172.81    544.06      0.52      3.00    172.80    543.00        0.52


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2012E+03 EXCESS=0.0000E+00 OUTFLOW=0.2012E+03 BASIN STORAGE=0.2435E-01 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE         0.44    176.85    543.47      0.53      3.00    176.85    543.00        0.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2055E+03 EXCESS=0.0000E+00 OUTFLOW=0.2054E+03 BASIN STORAGE=0.2781E-01 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            BSNRT2  MANE         0.44    176.78    543.55      0.53      3.00    176.78    543.00        0.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2054E+03 EXCESS=0.0000E+00 OUTFLOW=0.2054E+03 BASIN STORAGE=0.2781E-01 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            BSNRT2  MANE         0.48    172.80    544.14      0.52      3.00    172.80    546.00        0.52


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2012E+03 EXCESS=0.0000E+00 OUTFLOW=0.2011E+03 BASIN STORAGE=0.2781E-01 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R10-11  MANE         3.46    176.84    548.85      0.54      3.00    176.84    549.00        0.54


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2080E+03 EXCESS=0.0000E+00 OUTFLOW=0.2075E+03 BASIN STORAGE=0.5529E+00 PERCENT ERROR=   0.0
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          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R10-11  MANE         3.45    176.78    548.95      0.54      3.00    176.78    549.00        0.54


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2079E+03 EXCESS=0.0000E+00 OUTFLOW=0.2074E+03 BASIN STORAGE=0.5529E+00 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R10-11  MANE         3.43    172.80    553.00      0.52      3.00    172.80    552.00        0.52


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2033E+03 EXCESS=0.0000E+00 OUTFLOW=0.2028E+03 BASIN STORAGE=0.5528E+00 PERCENT ERROR=   0.0



*** NORMAL END OF HEC-1 ***
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100yr-6hr-Ultimate

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1***************************************                                                 **************************************
 *                                      *                                                *                                    *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                *   U.S. ARMY CORPS OF ENGINEERS     *
 *              JUN   1998              *                                                *   HYDROLOGIC ENGINEERING CENTER    *
 *           VERSION 4.1                *                                                *         609 SECOND STREET          *
 *                                      *                                                *      DAVIS, CALIFORNIA 95616       *
 *  RUN DATE   20JUL23  TIME  08:45:55  *                                                *          (916) 756-1104            *
 *                                      *                                                *                                    *
 ****************************************                                                 **************************************



                                                    X     X  XXXXXXX   XXXXX          X
                                                    X     X  X        X     X        XX
                                                    X     X  X        X               X
                                                    XXXXXXX  XXXX     X        XXXXX  X
                                                    X     X  X        X               X
                                                    X     X  X        X     X         X
                                                    X     X  XXXXXXX   XXXXX         XXX



                THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

                THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
                THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
                NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
                DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
                KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


1                                                           HEC-1 INPUT                                               PAGE  1

               LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

                  1           ID         CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
                  2           ID         POWERLINE CORRIDOR
                  3           ID         RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                  4           ID         INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                  5           ID        100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                  6           ID            PIMA ROAD FLOWS.
                  7           ID
                  8           ID         PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                  9           ID        MODELERS: RK, MW
                 10           ID
                 11           ID         REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
                 12           ID        MODELERS: MSW, ES
                 13           ID        REVISIONS NOTED WITH HE
                 14           ID            ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
                 15           ID            THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
                 16           ID            UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.
                 17           ID
                 18           ID         REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
                 19           ID        MODELERS: MSW
                 20           ID        XKSAT ADJUSTMENT
                 21           ID
                 22           ID         REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
                 23           ID        MODELERS: MSW, TSW
                 24           ID        REVISIONS NOTED WITH HE (Hubbard Engineering)
                 25           ID            NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
                 26           ID            CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
                 27           ID            ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
                 28           ID            FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED
                 29           ID
                 30           ID         REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/20/23
                 31           ID            MODELERS: MSW, TSW
                 32           ID            REVISIONS NOTED WITH HE (HUBBARD ENGINEERING)
                 33           ID            BASIN 2E WAS SPLIT AT CENTER POINT DUE TO FLOW UNDER 101
                 34           ID
                 35           IT        3       0       0    1000
                 36           IN       15
                 37           IO        5
                              *DIAGRAM
                              *
                 38           JD    2.755  0.0001
                 39           PC    0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
                 40           PC    0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
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  41           PC   0.962   0.972   0.983   0.991   1.000
  42           JD   2.738  0.5000
  43           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
  44           PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
  45           PC   0.962   0.972   0.983   0.991   1.000
  46           JD   2.686     2.8
  47           PC   0.000   0.009   0.016   0.025   0.034   0.042   0.051   0.059   0.068   0.077
  48           PC   0.088   0.101   0.121   0.164   0.253   0.451   0.694   0.836   0.900   0.938
  49           PC   0.950   0.963   0.975   0.988   1.000
  50           JD   2.540    16.0
  51           PC   0.000   0.015   0.020   0.030   0.048   0.063   0.076   0.090   0.105   0.119
  52           PC   0.135   0.152   0.175   0.222   0.304   0.472   0.670   0.796   0.868   0.912
  53           PC   0.946   0.960   0.973   0.987   1.000
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LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

  54           JD   2.232    90.0
  55           PC   0.000   0.021   0.035   0.051   0.071   0.087   0.105   0.125   0.143   0.160
  56           PC   0.179   0.201   0.232   0.281   0.364   0.500   0.658   0.773   0.841   0.888
  57           PC   0.927   0.945   0.964   0.982   1.000
  58           JD   3.657    10.0
  59           JD   3.533    20.0
               *

  60           KK  76THST
  61           KM       76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
  62           KM       100-YR, 6-HR FLO-2D MODEL (XS 98)
  63           BA    0.24
  64           QI       0       0       0       0       0       0       0       0       0       0
  65           QI       0       0    0.01    0.01    0.09     0.9    12.7   42.26   49.92    48.6
  66           QI   34.49   26.72   21.55   17.51   14.81   12.84   11.63   10.56    9.58    8.32
  67           QI    7.29    6.73    5.83    4.76    4.23    3.81     3.4    3.55    2.36    2.51
  68           QI    2.32    2.16    2.02    1.83    1.58     1.5    1.52    1.21    1.31    1.34
  69           QI    1.09    1.07    1.04    0.97    0.91    0.86    0.83    0.79    0.76    0.72
  70           QI    0.69    0.67    0.65    0.62     0.6    0.58    0.55    0.53    0.51    0.49
  71           QI    0.47    0.45    0.44

               *

  72           KK   SB01A   BASIN
  73           BA   0.061
  74           PB   2.721
  75           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.075
  76           PC   0.087   0.099   0.119   0.150   0.234   0.413   0.766   0.875   0.916   0.944
  77           PC   0.956   0.968   0.979   0.990   1.000
  78           LG    0.25    0.25    4.03    0.56      45
  79           UC   0.439   0.231
  80           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  81           UA     100
               *

  82           KK   SB01A  DIVERT
  83           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01A
  84           KM        FIRST FLUSH VOL = 1.383 AC-FT -HE
  85           DT  RET01A   1.383
  86           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  87           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

  88           KK    CP-1 COMBINE
  89           KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
  90           HC       2   0.301
               *

  91           KK  R1A-2W   ROUTE
  92           KM        ROUTE CP-1 TO CP-2W
  93           RK    1114  0.0200   0.018           TRAP   2.000    0.00
               *
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LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  94           KK  SB02NW   BASIN
  95           KM        BASIN SB02E WAS SPLIT INTO SUB BASINS NORTH OF CAVASSON
  96           KM        AND SOUTH OF CAVASSON -HE
  97           BA  0.0193
  98           LG    0.25    0.25    4.03    0.56      45
  99           UC   0.172   0.133
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 100           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 101           UA     100
               *

 102           KK  SB02NW  DIVERT
 103           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NW
 104           KM        FIRST FLUSH VOL = 0.437 AC-FT -HE
 105           DT  RET2NW   0.437
 106           DI     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
 107           DI    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
 108           DQ     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
 109           DQ    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
               *

 110           KK  CP-02W COMBINE
 111           KM        COMBINE ROUTE FROM 1A-2W WITH NEW SUB BASIN SB02NW
 112           HC       2   0.305
               *

 113           KK   R2W-2   ROUTE
 114           RK    1380  0.0140   0.018            TRAP   2.000    0.00
               *

 115           KK   SB01C   BASIN
 116           BA   0.027
 117           LG    0.25    0.25    4.03    0.56      45
 118           UC   0.215   0.216
 119           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 120           UA     100
               *

 121           KK   SB01C  DIVERT
 122           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01C
 123           KM        FIRST FLUSH VOL = 0.612 AC-FT -HE
 124           DT  RET01C   0.612
 125           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 126           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 127           KK  R1C-1B   ROUTE
 128           KM        KO     2       2
 129           RK     542   0.005   0.013            CIRC   5.000
               *
1                                         HEC-1 INPUT                                           PAGE  4

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 130           KK   SB01B   BASIN
 131           BA   0.024
 132           LG    0.25    0.25    4.03    0.56      45
 133           UC   0.191   0.178
 134           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 135           UA     100
               *

 136           KK   SB01B  DIVERT
 137           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01B
 138           KM        FIRST FLUSH VOL = 0.543 AC-FT -HE
 139           DT  RET01B   0.543
 140           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 141           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 142           KK  CP-01B COMBINE
 143           KM        COMBINE ROUTE FROM 1C-1B WITH NEW SUB BASIN SB01B
 144           HC       2   0.051
               *

 145           KK  R1B-2N   ROUTE
 146           KM    KO     2       2
 147           RK    2144   0.008   0.013            CIRC   5.000
               *

 148           KK  SB02NE   BASIN
 149           BA  0.0335
 150           LG    0.25    0.25    4.03    0.56      45
 151           UC   0.255   0.267
 152           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 153           UA     100
```

```
              *

 154          KK  SB02NE  DIVERT
 155          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NE
 156          KM        FIRST FLUSH VOL = 0.760 AC-FT -HE
 157          DT  RET2NE   0.760
 158          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 159          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 160          KK  CP-02N COMBINE
 161          KM        COMBINE ROUTE 1B-2N (BASINS SB01-B AND SB01-C) WITH SB02NE
 162          HC       2   0.085
              *

 163          KK   R2N-2   ROUTE
 164          RK    1380   0.014   0.018            TRAP   2.000    0.00
              *
1                                            HEC-1 INPUT                                            PAGE  5

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 165          KK  SB02E2   BASIN
 166          KM         WEST SPLIT OF BASIN SBO2E DUE TO HIGH POINT -HE
 167          BA   0.019
 168          LG    0.10    0.25    4.03    0.61      80
 169          UC   0.205   0.233
 170          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 171          UA     100
              *

 172          KK  SB02E2  DIVERT
 173          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E2
 174          KM        FIRST FLUSH VOL = 0.428 AC-FT -HE
 175          DT  RET2E2   0.428
 176          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 177          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 178          KK   SB02W   BASIN
 179          BA  0.0571
 180          LG    0.35    0.35    4.33    0.51       0
 181          UC   0.686   0.535
 182          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 183          UA     100
              *

 184          KK   CP-02 COMBINE
 185          HC       4  0.1816
              *

 186          KK    R2-3   ROUTE
 187          KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
 188          RK    1260  0.0015   0.030            TRAP      92       4
              *

 189          KK    SB03   BASIN
 190          BA   0.048
 191          LG    0.15    0.25    4.50    0.47      55
 192          UC   0.364   0.294
 193          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 194          UA     100
              *

 195          KK   CP-03 COMBINE
 196          HC       2
              *

 197          KK    R3-5   ROUTE
 198          KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
 199          RK    2396  0.0015   0.03             TRAP      98       4
              *
1                                            HEC-1 INPUT                                            PAGE  6

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 200          KK    SB05   BASIN
 201          BA   0.126
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```
 202           LG    0.22    0.25    4.50    0.44      48
 203           UC   0.327   0.226
 204           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 205           UA     100
               *

 206           KK   CP-5A COMBINE
 207           HC       2
               *

 208           KK   SB07W   BASIN
 209           BA  0.0468
 210           LG    0.25    0.25    4.03    0.56      45
 211           UC   0.203   0.141
 212           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 213           UA     100
               *

 214           KK   SB07W  DIVERT
 215           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB07W
 216           KM        FIRST FLUSH VOL = 1.060 AC-FT -HE
 217           DT  RET07W   1.060
 218           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 219           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 220           KK  R7W-8W   ROUTE
 221           KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 222           KM        HE MODIFIED ROUTE TO LOOP 101 W OF HAYDEN
 223           RK    3754  0.0013   0.03            TRAP      46       4
               *

 224           KK   SB8NW   BASIN
 225           KM        BASIN JUST SOUTH OF LEGACY - FUTURE MOB SITE -HE
 226           BA  0.0147
 227           LG    0.10    0.25    4.03    0.61      80
 228           UC   0.144   0.108
 229           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 230           UA     100
               *

 231           KK   SB8NW  DIVERT
 232           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NW
 233           KM        FIRST FLUSH VOL = 0.332 AC-FT -HE
 234           DT  RET8NW   0.332
 235           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 236           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *
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LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 237           KK  R8NW8N   ROUTE
 238           KM        ROUTE NEW SUB BASIN SB8NW TO OUTLET AT CAVASSON BLVD  -HE
 239           KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 240           RK     643  0.0113   0.018           TRAP      46       4
               *

 241           KK   SB8NE   BASIN
 242           KM        SITE JUST SOUTH OF LEGACY AND WEST OF HAYDEN - FUTURE RETAIL
 243           BA   0.015
 244           LG    0.10    0.25    4.03    0.61      80
 245           UC   0.138   0.094
 246           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 247           UA     100
               *

 248           KK   SB8NE  DIVERT
 249           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NE
 250           KM        FIRST FLUSH VOL = 0.340 AC-FT -HE
 251           DT  RET8NE   0.340
 252           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 253           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 254           KK  R8NE8N   ROUTE
 255           KM        ROUTE NEW SUB BASIN SB8NE TO OUTLET AT CAVASSON BLVD -HE
 256           KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 257           RK     622   0.011   0.018           TRAP      46       4
```

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                 *

  258            KK   SB8SW   BASIN
  259            KM        SITE JUST NORTH OF CAVASSON BLVD EAST OF CLARET DR - FUTURE MF
  260            BA  0.0129
  261            LG    0.10    0.25    4.03    0.61      80
  262            UC   0.126   0.091
  263            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  264            UA     100
                 *

  265            KK   SB8SW  DIVERT
  266            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SW
  267            KM        FIRST FLUSH VOL = 0.292 AC-FT -HE
  268            DT  RET8SW   0.292
  269            DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  270            DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                 *

  271            KK   SB8SE   BASIN
  272            KM        SITE JUST NORTH OF CAVASSON AND WEST OF HAYDEN - FUTURE HOTEL OR RETAIL
  273            BA  0.0131
  274            LG    0.10    0.25    4.03    0.61      80
  275            UC   0.139   0.113
  276            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  277            UA     100
                 *
```



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 LINE            ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  278            KK   SB8SE  DIVERT
  279            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SE
  280            KM        FIRST FLUSH VOL = 0.297 AC-FT -HE
  281            DT  RET8SE   0.297
  282            DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  283            DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                 *

  284            KK  CP-08N COMBINE
  285            KM        CP AT INLET ON NORTH SIDE OF CAVASSON - ENTERS EXIST STORM DRAIN
  286            HC       4  0.0557
                 *

  287            KK  R8N-8W   ROUTE
  288            KM        ROUTE NEW SUB BASINS SB8NE SB8NW SB8SW SB8SE TO CP-08W
  289            KM        ROUTE IN STORM DRAIN    -HE
  290            RK    1592   0.015   0.013            CIRC       3
                 *

  291            KK   SB08W   BASIN
  292            BA  0.0589
  293            LG    0.10    0.25    4.03    0.61      80
  294            UC   0.221   0.146
  295            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  296            UA     100
                 *

  297            KK   SB08W  DIVERT
  298            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB08W
  299            KM        FIRST FLUSH VOL REQ = 1.335 AC-FT, PROVIDED = 1.415
  300            KM        THIS IS EXISTING AND WAS BUILT IN PHASE 1 -HE
  301            DT   RET8W   1.415
  302            DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  303            DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                 *

  304            KK  CP-08W COMBINE
  305            HC       3  0.1614
                 *

  306            KK  R8W-4N   ROUTE
  307            KM         STAGE STORAGE ROUTING UNDER THE LOOP 101 -HE
  308            RS       1    STOR    0.00       0
  309            SV   0.000   0.642   1.290   2.207   4.066   4.066
  310            SE  1608.0  1609.0  1610.0  1611.0  1612.0  1613.0
  311            SQ   0.000   8.330   28.52   150.5   300.0   360.0
  312            SE  1608.0  1609.0  1610.0  1611.0  1613.2  1613.3
                 *
```

```
 LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  313          KK  SB02E1   BASIN
  314          KM          EAST SPLIT OF BASIN SBO2E DUE TO HIGH POINT -HE
  315          BA  0.0252
  316          LG    0.10    0.25    4.03    0.61      80
  317          UC   0.189   0.172
  318          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  319          UA     100
               *

  320          KK  SB02E1  DIVERT
  321          KM         DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E1
  322          KM         FIRST FLUSH VOL REQ = 0.570 AC-FT -HE
  323          DT  RET2E1   0.570
  324          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  325          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

  326          KK   CP-4N COMBINE
  327          HC       2  0.1866
               *

  328          KK   R4N-4   ROUTE
  329          RK    2840  0.0130   0.030            TRAP   2.000    0.00
               *

  330          KK    SB04   BASIN
  331          BA   0.144
  332          LG    0.14    0.25    4.60    0.44      61
  333          UC   0.305   0.199
  334          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
  335          UA     100
               *

  336          KK    CP-4 COMBINE
  337          HC       2
               *

  338          KK    R4-5   ROUTE
  339          KM         PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
  340          RK    1992  0.0013   0.03             TRAP      39       4
               *

  341          KK   CP-05 COMBINE
  342          KM         PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
  343          KM         COMBINE CP-05A AND R4-5
  344          HC       2
               *

  345          KK    R5-6   ROUTE
  346          KM         PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
  347          RK    1694  0.0015   0.03             TRAP     188       4
               *
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 LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  348          KK    SB06   BASIN
  349          BA   0.136
  350          LG    0.16    0.25    4.55    0.45      53
  351          UC   0.321   0.246
  352          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
  353          UA     100
               *

  354          KK   CP-06 COMBINE
  355          HC       2
               *

  356          KK   SB07E   BASIN
  357          BA   0.026
  358          LG    0.10    0.25    4.03    0.74       0
  359          UC   0.574   0.527
  360          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  361          UA     100
```

```
              *

 362          KK  R7E-8E   ROUTE
 363          RK    3662  0.0130   0.030           TRAP   2.000    0.00
              *

 364          KK   SB08E   BASIN
 365          BA   0.078
 366          LG    0.10    0.25    4.08    0.72       0
 367          UC   0.798   0.652
 368          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 369          UA     100
              *

 370          KK   SB09N   BASIN
 371          BA   0.028
 372          LG    0.10    0.25    4.79    0.49       0
 373          UC   0.740   1.111
 374          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 375          UA     100
              *

 376          KK  R9N-R9   ROUTE
 377          RK    2586  0.0013   0.020           TRAP   2.000    0.00
              *

 378          KK  CP-08E COMBINE
 379          HC       3
              *

 380          KK  R8E-R9   ROUTE
 381          RK     501  0.0013   0.020           TRAP   2.000    0.00
              *
```



```
 382          KK    SB09   BASIN
 383          BA   0.086
 384          LG    0.10    0.25    4.79    0.49       0
 385          UC   0.683   0.518
 386          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 387          UA     100
              *

 388          KK   CP-09 COMBINE
 389          HC       2
              *

 390          KK   PWRCH
 391          KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
 392          KM        100-YR, 6-HR FLO-2D MODEL (XS 107)
 393          BA     7.0
 394          QI       0       0       0       0       0       0       0       0       0       0
 395          QI       0       0       0       0       0       0    0.11   17.74   50.91  433.53  391.08
 396          QI  359.13  391.64  616.52  737.85  699.64  645.41  621.52  599.48  594.12  570.84
 397          QI  525.86  472.45  419.04  375.56     335  299.45   270.9  235.68  211.13  196.81
 398          QI  172.83  155.83  146.04  130.32   120.9  110.32   99.25    90.9   82.93   77.68
 399          QI    79.3   67.58    63.5   64.86   56.01   51.64   48.75   43.99   42.04   40.99
 400          QI   37.99   36.19   33.42   31.56   29.22   28.13   27.15   31.57   24.31    23.1
 401          QI   22.09   21.66   20.95
              *

 402          KK  BINFLO
 403          KM         TOTAL INFLOW INTO BASIN 53R.
 404          HC       2
              *

 405          KK   BASIN
 406          KM    BASIN 53R STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
 407          KM    ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
 408          KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
 409          KM        THE OUTLET PIPES ARE INLET CONTROLLED.
 410          RS       1    STOR       0
 411          SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
 412          SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
 413          SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
 414          SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
 415          SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
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```
         416           SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
                       *

         417           KK  BSNRT1
         418           KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
         419           KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
         420           KM        EXCESS CAPACITY.
         421           RK     550  0.0052   0.024            CIRC     7.0
                       *
1                                                     HEC-1 INPUT                                             PAGE 12

        LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


         422           KK  BSNRT2
         423           KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BAS 53R OUTFALL)
         424           RK    1200  0.0077   0.013            CIRC     7.0
                       *

         425           KK    SB10   BASIN
         426           BA   0.040
         427           LG    0.15    0.25    4.25    0.55      55
         428           UC   0.233   0.161
         429           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
         430           UA     100
                       *

         431           KK   CP-10 COMBINE
         432           KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
         433           HC       2   0.229
                       *

         434           KK  R10-11   ROUTE
         435           KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
         436           RK    1277  0.0014   0.03             TRAP     220       4
                       *

         437           KK    SB11   BASIN
         438           BA   0.071
         439           LG    0.15    0.25    4.15    0.58      55
         440           UC   0.296   0.232
         441           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
         442           UA     100
                       *

         443           KK   CP-11 COMBINE
         444           KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
         445           HC       2     0.3
                       *
         446           ZZ
1
             SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

   NO.      (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

    60      76THST
                 .
                 .
    72           .       SB01A
                 .           .
                 .           .
    85           .           .------->  RET01A
    82           .       SB01A
                 .           .
                 .           .
    88        CP-1............
                 V
                 V
    91      R1A-2W
                 .
                 .
    94           .      SB02NW
                 .           .
                 .           .
   105           .           .------->  RET2NW
   102           .      SB02NW
                 .           .
                 .           .
```

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110      CP-02W............
              V
              V
113       R2W-2
              .
              .
115           .       SB01C
              .           .
              .           .
124           .           .------->  RET01C
121           .       SB01C
              .           V
              .           V
127           .      R1C-1B
              .           .
              .           .
130           .           .       SB01B
              .           .           .
              .           .           .
139           .           .           .------->  RET01B
136           .           .       SB01B
              .           .           .
              .           .           .
142           .      CP-01B............
              .           V
              .           V
145           .      R1B-2N
              .           .
              .           .
148           .           .      SB02NE
              .           .           .
              .           .           .
157           .           .           .------->  RET2NE
154           .           .      SB02NE
              .           .           .
              .           .           .
160           .      CP-02N............
              .           V
              .           V
163           .       R2N-2
              .           .
              .           .
165           .           .      SB02E2
              .           .           .
              .           .           .
175           .           .           .------->  RET2E2
172           .           .      SB02E2
              .           .           .
              .           .           .
178           .           .           .       SB02W
              .           .           .           .
              .           .           .           .
184       CP-02....................................
              V
              V
186        R2-3
              .
              .
189           .        SB03
              .           .
              .           .
195       CP-03............
              V
              V
197        R3-5
              .
              .
200           .        SB05
              .           .
              .           .
206       CP-5A............
              .
              .
208           .       SB07W
              .           .
              .           .
217           .           .------->  RET07W
214           .       SB07W
              .           V
              .           V
```

```
220           .      R7W-8W
              .           .
              .           .
224           .           .       SB8NW
              .           .           .
              .           .           .
234           .           .           .------->  RET8NW
231           .           .       SB8NW
              .           .           V
              .           .           V
237           .           .      R8NW8N
              .           .           .
              .           .           .
241           .           .           .       SB8NE
              .           .           .           .
              .           .           .           .
251           .           .           .           .------->  RET8NE
248           .           .           .       SB8NE
              .           .           .           V
              .           .           .           V
254           .           .           .      R8NE8N
              .           .           .           .
              .           .           .           .
258           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
268           .           .           .           .           .------->  RET8SW
265           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
271           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
281           .           .           .           .           .           .------->  RET8SE
278           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
284           .           .      CP-08N....................................
              .           .           V
              .           .           V
287           .           .      R8N-8W
              .           .           .
              .           .           .
291           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
301           .           .           .           .------->   RET8W
297           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
304           .      CP-08W........................
              .           V
              .           V
306           .      R8W-4N
              .           .
              .           .
313           .           .      SB02E1
              .           .           .
              .           .           .
323           .           .           .------->  RET2E1
320           .           .      SB02E1
              .           .           .
              .           .           .
326           .       CP-4N............
              .           V
              .           V
328           .       R4N-4
              .           .
              .           .
330           .           .        SB04
              .           .           .
              .           .           .
336           .        CP-4............
              .           V
              .           V
338           .        R4-5
              .           .
              .           .
341       CP-05............
              V
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                 V
   345        R5-6
                 .
                 .
   348           .       SB06
                 .           .
                 .           .
   354       CP-06............
                 .
                 .
   356           .      SB07E
                 .           V
                 .           V
   362           .     R7E-8E
                 .           .
                 .           .
   364           .           .      SB08E
                 .           .           .
                 .           .           .
   370           .           .           .      SB09N
                 .           .           .           V
                 .           .           .           V
   376           .           .           .     R9N-R9
                 .           .           .           .
                 .           .           .           .
   378           .     CP-08E........................
                 .           V
                 .           V
   380           .     R8E-R9
                 .           .
                 .           .
   382           .           .       SB09
                 .           .           .
                 .           .           .
   388           .      CP-09............
                 .           .
                 .           .
   390           .           .      PWRCH
                 .           .           .
                 .           .           .
   402           .     BINFLO............
                 .           V
                 .           V
   405           .      BASIN
                 .           V
                 .           V
   417           .     BSNRT1
                 .           V
                 .           V
   422           .     BSNRT2
                 .           .
                 .           .
   425           .           .       SB10
                 .           .           .
                 .           .           .
   431           .      CP-10............
                 .           V
                 .           V
   434           .     R10-11
                 .           .
                 .           .
   437           .           .       SB11
                 .           .           .
                 .           .           .
   443           .      CP-11............

 (***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1*****************************************                                                 ***************************************
 *                                       *                                                 *                                     *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                 *    U.S. ARMY CORPS OF ENGINEERS     *
 *              JUN   1998               *                                                 *    HYDROLOGIC ENGINEERING CENTER    *
 *           VERSION 4.1                 *                                                 *          609 SECOND STREET          *
 *                                       *                                                 *       DAVIS, CALIFORNIA 95616       *
 *  RUN DATE   20JUL23  TIME  08:45:55   *                                                 *           (916) 756-1104            *
 *                                       *                                                 *                                     *
 *****************************************                                                 ***************************************
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```
                                     CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
                                     POWERLINE CORRIDOR
                                     RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                                     INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                                       100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                                        PIMA ROAD FLOWS.

                                     PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                                       MODELERS: RK, MW

                                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
                                       MODELERS: MSW, ES
                                       REVISIONS NOTED WITH HE
                                        ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
                                        THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
                                        UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.

                                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
                                       MODELERS: MSW
                                       XKSAT ADJUSTMENT

                                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
                                       MODELERS: MSW, TSW
                                       REVISIONS NOTED WITH HE (Hubbard Engineering)
                                        NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
                                        CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
                                        ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
                                        FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED

                                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/20/23
                                        MODELERS: MSW, TSW
                                        REVISIONS NOTED WITH HE (HUBBARD ENGINEERING)
                                        BASIN 2E WAS SPLIT AT CENTER POINT DUE TO FLOW UNDER 101


37 IO          OUTPUT CONTROL VARIABLES
                     IPRNT          5  PRINT CONTROL
                     IPLOT          0  PLOT CONTROL
                     QSCAL         0.  HYDROGRAPH PLOT SCALE

   IT          HYDROGRAPH TIME DATA
                      NMIN          3  MINUTES IN COMPUTATION INTERVAL
                     IDATE      1    0  STARTING DATE
                     ITIME       0000  STARTING TIME
                        NQ       1000  NUMBER OF HYDROGRAPH ORDINATES
                    NDDATE      3    0  ENDING DATE
                    NDTIME       0157  ENDING TIME
                    ICENT          19  CENTURY MARK

                 COMPUTATION INTERVAL    0.05 HOURS
                      TOTAL TIME BASE   49.95 HOURS

       ENGLISH UNITS
            DRAINAGE AREA         SQUARE MILES
            PRECIPITATION DEPTH   INCHES
            LENGTH, ELEVATION     FEET
            FLOW                  CUBIC FEET PER SECOND
            STORAGE VOLUME        ACRE-FEET
            SURFACE AREA          ACRES
            TEMPERATURE           DEGREES FAHRENHEIT

38 JD          INDEX STORM NO. 1
                      STRM       2.76  PRECIPITATION DEPTH
                      TRDA       0.00  TRANSPOSITION DRAINAGE AREA

39 PI            PRECIPITATION PATTERN
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                    0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
                    0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
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42 JD         INDEX STORM NO. 2
                     STRM        2.74  PRECIPITATION DEPTH
                     TRDA        0.50  TRANSPOSITION DRAINAGE AREA

43 PI           PRECIPITATION PATTERN
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                  0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
                  0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

46 JD         INDEX STORM NO. 3
                     STRM        2.69  PRECIPITATION DEPTH
                     TRDA        2.80  TRANSPOSITION DRAINAGE AREA

47 PI           PRECIPITATION PATTERN
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                  0.04      0.04      0.04      0.04      0.04      0.05      0.05      0.05      0.05      0.05
                  0.03      0.03      0.03      0.03      0.03      0.01      0.01      0.01      0.01      0.01
                  0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

50 JD         INDEX STORM NO. 4
                     STRM        2.54  PRECIPITATION DEPTH
                     TRDA       16.00  TRANSPOSITION DRAINAGE AREA

51 PI           PRECIPITATION PATTERN
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                  0.03      0.03      0.03      0.03      0.03      0.04      0.04      0.04      0.04      0.04
                  0.03      0.03      0.03      0.03      0.03      0.01      0.01      0.01      0.01      0.01
                  0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

54 JD         INDEX STORM NO. 5
                     STRM        2.23  PRECIPITATION DEPTH
                     TRDA       90.00  TRANSPOSITION DRAINAGE AREA

55 PI           PRECIPITATION PATTERN
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                  0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                  0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03
                  0.02      0.02      0.02      0.02      0.02      0.01      0.01      0.01      0.01      0.01
                  0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

58 JD         INDEX STORM NO. 6
                     STRM        3.66  PRECIPITATION DEPTH
                     TRDA       10.00  TRANSPOSITION DRAINAGE AREA

 0 PI           PRECIPITATION PATTERN
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                  0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
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                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                 0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                 0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03
                 0.02      0.02      0.02      0.02      0.02      0.01      0.01      0.01      0.01      0.01
                 0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

59 JD          INDEX STORM NO. 7
                   STRM        3.53  PRECIPITATION DEPTH
                   TRDA       20.00  TRANSPOSITION DRAINAGE AREA

 0 PI            PRECIPITATION PATTERN
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                 0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                 0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03
                 0.02      0.02      0.02      0.02      0.02      0.01      0.01      0.01      0.01      0.01
                 0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

       *** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT


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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+	HYDROGRAPH AT	76THST	50.	4.50	15.	4.	2.	0.24		
+	HYDROGRAPH AT	SB01A	103.	4.25	12.	3.	1.	0.06		
+	DIVERSION TO	RET01A	44.	4.05	3.	1.	0.	0.06		
+	HYDROGRAPH AT	SB01A	103.	4.25	9.	2.	1.	0.06		
+	2 COMBINED AT	CP-1	145.	4.25	24.	7.	3.	0.30		
+	ROUTED TO	R1A-2W	144.	4.30	24.	7.	3.	0.30		
+	HYDROGRAPH AT	SB02NW	43.	4.05	4.	1.	0.	0.02		
+	DIVERSION TO	RET2NW	14.	3.85	1.	0.	0.	0.02		
+	HYDROGRAPH AT	SB02NW	43.	4.05	3.	1.	0.	0.02		
+	2 COMBINED AT	CP-02W	164.	4.25	27.	7.	4.	0.31		
+	ROUTED TO	R2W-2	163.	4.25	27.	7.	4.	0.31		
+	HYDROGRAPH AT	SB01C	51.	4.10	5.	1.	1.	0.03		
+	DIVERSION TO	RET01C	20.	3.90	1.	0.	0.	0.03		
+	HYDROGRAPH AT	SB01C	51.	4.10	4.	1.	0.	0.03		
+	ROUTED TO	R1C-1B	50.	4.10	4.	1.	0.	0.03		

+	HYDROGRAPH AT	SB01B	50.	4.10	5.	1.	1.	0.02
+	DIVERSION TO	RET01B	17.	3.90	1.	0.	0.	0.02
+	HYDROGRAPH AT	SB01B	50.	4.10	3.	1.	0.	0.02
+	2 COMBINED AT	CP-01B	100.	4.10	7.	2.	1.	0.05
+	ROUTED TO	R1B-2N	97.	4.15	7.	2.	1.	0.05
+	HYDROGRAPH AT	SB02NE	57.	4.15	6.	2.	1.	0.03
+	DIVERSION TO	RET2NE	27.	3.95	2.	0.	0.	0.03
+	HYDROGRAPH AT	SB02NE	57.	4.15	5.	1.	1.	0.03
+	2 COMBINED AT	CP-02N	154.	4.15	12.	3.	1.	0.09
+	ROUTED TO	R2N-2	154.	4.15	12.	3.	1.	0.09
+	HYDROGRAPH AT	SB02E2	40.	4.10	5.	1.	1.	0.02
+	DIVERSION TO	RET2E2	8.	3.70	1.	0.	0.	0.02
+	HYDROGRAPH AT	SB02E2	40.	4.10	4.	1.	0.	0.02
+	HYDROGRAPH AT	SB02W	39.	4.50	6.	1.	1.	0.06
+	4 COMBINED AT	CP-02	346.	4.20	49.	13.	6.	0.18
+	ROUTED TO	R2-3	346.	4.25	49.	13.	6.	0.18
+	HYDROGRAPH AT	SB03	78.	4.15	10.	3.	1.	0.05
+	2 COMBINED AT	CP-03	412.	4.25	58.	15.	8.	0.23
+	ROUTED TO	R3-5	408.	4.35	58.	15.	8.	0.23
+	HYDROGRAPH AT	SB05	229.	4.10	25.	6.	3.	0.13
+	2 COMBINED AT	CP-5A	536.	4.30	81.	22.	11.	0.36
+	HYDROGRAPH AT	SB07W	104.	4.10	9.	2.	1.	0.05
+	DIVERSION TO	RET07W	29.	3.85	2.	1.	0.	0.05
+	HYDROGRAPH AT	SB07W	104.	4.10	7.	2.	1.	0.05
+	ROUTED TO	R7W-8W	104.	4.45	8.	2.	1.	0.05
+	HYDROGRAPH AT	SB8NW	40.	4.05	4.	1.	0.	0.01
	DIVERSION TO							

+		RET8NW	6.	3.55	1.	0.	0.	0.01
+	HYDROGRAPH AT	SB8NW	40.	4.05	3.	1.	0.	0.01
+	ROUTED TO	R8NW8N	39.	4.05	3.	1.	0.	0.01
+	HYDROGRAPH AT	SB8NE	42.	4.05	4.	1.	0.	0.01
+	DIVERSION TO	RET8NE	6.	3.55	1.	0.	0.	0.01
+	HYDROGRAPH AT	SB8NE	42.	4.05	3.	1.	0.	0.01
+	ROUTED TO	R8NE8N	42.	4.05	3.	1.	0.	0.01
+	HYDROGRAPH AT	SB8SW	37.	4.05	3.	1.	0.	0.01
+	DIVERSION TO	RET8SW	5.	3.55	1.	0.	0.	0.01
+	HYDROGRAPH AT	SB8SW	37.	4.05	3.	1.	0.	0.01
+	HYDROGRAPH AT	SB8SE	36.	4.05	3.	1.	0.	0.01
+	DIVERSION TO	RET8SE	5.	3.55	1.	0.	0.	0.01
+	HYDROGRAPH AT	SB8SE	36.	4.05	3.	1.	0.	0.01
+	4 COMBINED AT	CP-08N	153.	4.05	12.	3.	1.	0.06
+	ROUTED TO	R8N-8W	150.	4.05	12.	3.	1.	0.06
+	HYDROGRAPH AT	SB08W	145.	4.10	15.	4.	2.	0.06
+	DIVERSION TO	RET8W	27.	3.65	3.	1.	0.	0.06
+	HYDROGRAPH AT	SB08W	145.	4.10	12.	3.	1.	0.06
+	3 COMBINED AT	CP-08W	287.	4.10	32.	8.	4.	0.16
+	ROUTED TO	R8W-4N	203.	4.20	32.	8.	4.	0.16
+	HYDROGRAPH AT	SB02E1	60.	4.10	6.	2.	1.	0.03
+	DIVERSION TO	RET2E1	12.	3.65	1.	0.	0.	0.03
+	HYDROGRAPH AT	SB02E1	60.	4.10	5.	1.	1.	0.03
+	2 COMBINED AT	CP-4N	254.	4.15	37.	9.	4.	0.19
+	ROUTED TO	R4N-4	252.	4.15	37.	9.	4.	0.19
+	HYDROGRAPH AT	SB04	292.	4.10	32.	8.	4.	0.14
+	2 COMBINED AT	CP-4	533.	4.10	68.	17.	8.	0.33

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          ROUTED TO
+                           R4-5       528.    4.20          68.         17.          8.       0.33

          2 COMBINED AT
+                          CP-05       934.    4.30         146.         38.         19.       0.69

          ROUTED TO
+                           R5-6       916.    4.40         146.         38.         19.       0.69

          HYDROGRAPH AT
+                           SB06       243.    4.10          29.          7.          3.       0.14

          2 COMBINED AT
+                          CP-06       996.    4.40         171.         45.         22.       0.82

          HYDROGRAPH AT
+                          SB07E        19.    4.40           3.          1.          0.       0.03

          ROUTED TO
+                         R7E-8E        19.    4.45           3.          1.          0.       0.03

          HYDROGRAPH AT
+                          SB08E        46.    4.55           8.          2.          1.       0.08

          HYDROGRAPH AT
+                          SB09N        13.    4.55           3.          1.          0.       0.03

          ROUTED TO
+                         R9N-R9        13.    4.65           3.          1.          0.       0.03

          3 COMBINED AT
+                         CP-08E        76.    4.55          13.          3.          2.       0.13

          ROUTED TO
+                         R8E-R9        76.    4.55          13.          3.          2.       0.13

          HYDROGRAPH AT
+                           SB09        71.    4.45          10.          2.          1.       0.09

          2 COMBINED AT
+                          CP-09       144.    4.50          23.          6.          3.       0.22

          HYDROGRAPH AT
+                          PWRCH       738.    5.75         429.        135.         74.       7.00

          2 COMBINED AT
+                         BINFLO       749.    5.75         439.        138.         76.       7.22

          ROUTED TO
+                          BASIN       241.    9.20         228.        130.         73.       7.22

          ROUTED TO
+                         BSNRT1       241.    9.25         228.        130.         73.       7.22

          ROUTED TO
+                         BSNRT2       241.    9.25         228.        130.         73.       7.22

          HYDROGRAPH AT
+                           SB10        87.    4.05           8.          2.          1.       0.04

          2 COMBINED AT
+                          CP-10       244.    9.20         232.        133.         76.       0.23

          ROUTED TO
+                         R10-11       244.    9.30         232.        133.         76.       0.23

          HYDROGRAPH AT
+                           SB11       130.    4.10          15.          4.          2.       0.07

          2 COMBINED AT
+                          CP-11       244.    9.30         232.        136.         77.       0.30
1
                                              SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
                                                  (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)
                                                                                    INTERPOLATED TO
                                                                                 COMPUTATION INTERVAL
            ISTAQ    ELEMENT      DT       PEAK    TIME TO     VOLUME      DT       PEAK    TIME TO     VOLUME
                                                     PEAK                                     PEAK

                                 (MIN)     (CFS)      (MIN)     (IN)     (MIN)     (CFS)     (MIN)       (IN)
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          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1A-2W  MANE         0.31    145.99    255.52      0.86      3.00    145.05    258.00        0.86

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1385E+02 EXCESS=0.0000E+00 OUTFLOW=0.1385E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1A-2W  MANE         0.31    145.24    255.66      0.86      3.00    144.23    258.00        0.86

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1380E+02 EXCESS=0.0000E+00 OUTFLOW=0.1380E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1A-2W  MANE         0.30    102.74    258.17      0.82      3.00    102.70    258.00        0.82

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1311E+02 EXCESS=0.0000E+00 OUTFLOW=0.1311E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R1A-2W  MANE         0.30     86.26    258.55      0.77      3.00     86.14    258.00        0.77

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1240E+02 EXCESS=0.0000E+00 OUTFLOW=0.1240E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R1A-2W  MANE         0.35     70.99    270.31      0.71      3.00     70.97    270.00        0.71

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1143E+02 EXCESS=0.0000E+00 OUTFLOW=0.1143E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R1A-2W  MANE         0.22    101.82    261.19      0.92      3.00    101.82    261.00        0.92

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1483E+02 EXCESS=0.0000E+00 OUTFLOW=0.1483E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R1A-2W  MANE         0.27     99.04    261.08      0.90      3.00     99.04    261.00        0.90

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1451E+02 EXCESS=0.0000E+00 OUTFLOW=0.1451E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2W-2  MANE         0.36    164.36    255.38      0.94      3.00    163.85    255.00        0.94

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1526E+02 EXCESS=0.0000E+00 OUTFLOW=0.1526E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R2W-2  MANE         0.27    163.57    255.50      0.93      3.00    162.94    255.00        0.93

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1520E+02 EXCESS=0.0000E+00 OUTFLOW=0.1520E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R2W-2  MANE         0.35    117.20    258.14      0.88      3.00    117.19    258.00        0.88

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1428E+02 EXCESS=0.0000E+00 OUTFLOW=0.1428E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
             R2W-2  MANE         0.38     96.83    258.51      0.82      3.00     96.72    258.00        0.82

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1335E+02 EXCESS=0.0000E+00 OUTFLOW=0.1335E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
             R2W-2  MANE         0.37     75.72    261.93      0.74      3.00     75.66    261.00        0.74
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1207E+02 EXCESS=0.0000E+00 OUTFLOW=0.1206E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0


          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
             R2W-2  MANE        0.24    117.87    258.45      1.02      3.00    117.68    258.00        1.02


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1655E+02 EXCESS=0.0000E+00 OUTFLOW=0.1655E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0


          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
             R2W-2  MANE        0.24    114.22    258.50      0.99      3.00    114.02    258.00        0.99


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1613E+02 EXCESS=0.0000E+00 OUTFLOW=0.1613E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1C-1B  MANE        0.30     51.03    246.76      1.36      3.00     50.29    246.00        1.37


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1961E+01 EXCESS=0.0000E+00 OUTFLOW=0.1962E+01 BASIN STORAGE=0.1121E-13 PERCENT ERROR=  -0.1


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1C-1B  MANE        0.40     50.59    246.72      1.35      3.00     49.89    246.00        1.35


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1941E+01 EXCESS=0.0000E+00 OUTFLOW=0.1942E+01 BASIN STORAGE=0.1138E-13 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1C-1B  MANE        0.42     27.84    246.95      1.13      3.00     27.64    249.00        1.14


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1632E+01 EXCESS=0.0000E+00 OUTFLOW=0.1631E+01 BASIN STORAGE=0.1119E-13 PERCENT ERROR=   0.1


          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R1C-1B  MANE        0.40     19.93    246.90      0.91      3.00     19.81    249.00        0.92


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1321E+01 EXCESS=0.0000E+00 OUTFLOW=0.1317E+01 BASIN STORAGE=0.1123E-13 PERCENT ERROR=   0.3


          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R1C-1B  MANE        0.50     11.56    247.25      0.62      3.00     11.55    249.00        0.62


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8898E+00 EXCESS=0.0000E+00 OUTFLOW=0.8904E+00 BASIN STORAGE=0.1117E-13 PERCENT ERROR=  -0.1


          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R1C-1B  MANE        0.35     26.68    246.72      1.66      3.00     26.64    249.00        1.66


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2396E+01 EXCESS=0.0000E+00 OUTFLOW=0.2395E+01 BASIN STORAGE=0.1140E-13 PERCENT ERROR=   0.0


          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R1C-1B  MANE        0.32     25.38    246.62      1.57      3.00     25.34    249.00        1.57


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2256E+01 EXCESS=0.0000E+00 OUTFLOW=0.2254E+01 BASIN STORAGE=0.1122E-13 PERCENT ERROR=   0.1


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1B-2N  MANE        0.92     99.26    247.53      1.36      3.00     98.01    249.00        1.36


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3711E+01 EXCESS=0.0000E+00 OUTFLOW=0.3711E+01 BASIN STORAGE=0.3220E-11 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1B-2N  MANE        0.83     98.83    247.91      1.35      3.00     97.26    249.00        1.35


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3673E+01 EXCESS=0.0000E+00 OUTFLOW=0.3684E+01 BASIN STORAGE=0.3207E-11 PERCENT ERROR=  -0.3
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          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1B-2N  MANE         1.03     53.55    247.69      1.14      3.00     53.37    249.00        1.13

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3088E+01 EXCESS=0.0000E+00 OUTFLOW=0.3089E+01 BASIN STORAGE=0.3137E-11 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R1B-2N  MANE         1.09     38.34    248.42      0.92      3.00     38.21    249.00        0.92

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2500E+01 EXCESS=0.0000E+00 OUTFLOW=0.2500E+01 BASIN STORAGE=0.3264E-11 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R1B-2N  MANE         1.19     22.19    248.72      0.62      3.00     22.17    249.00        0.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1684E+01 EXCESS=0.0000E+00 OUTFLOW=0.1686E+01 BASIN STORAGE=0.3255E-11 PERCENT ERROR=  -0.1

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R1B-2N  MANE         0.90     51.28    247.95      1.67      3.00     51.12    249.00        1.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4529E+01 EXCESS=0.0000E+00 OUTFLOW=0.4530E+01 BASIN STORAGE=0.3257E-11 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R1B-2N  MANE         1.05     48.69    248.70      1.57      3.00     48.64    249.00        1.57

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4264E+01 EXCESS=0.0000E+00 OUTFLOW=0.4266E+01 BASIN STORAGE=0.3118E-11 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2N-2  MANE         0.31    154.68    249.16      1.36      3.00    154.46    249.00        1.36

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6138E+01 EXCESS=0.0000E+00 OUTFLOW=0.6147E+01 BASIN STORAGE=0.5105E-06 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R2N-2  MANE         0.31    153.62    249.29      1.34      3.00    153.29    249.00        1.35

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6080E+01 EXCESS=0.0000E+00 OUTFLOW=0.6088E+01 BASIN STORAGE=0.5465E-06 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R2N-2  MANE         0.42     85.54    249.46      1.13      3.00     85.38    249.00        1.13

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5110E+01 EXCESS=0.0000E+00 OUTFLOW=0.5111E+01 BASIN STORAGE=0.4442E-06 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
             R2N-2  MANE         0.42     61.35    249.48      0.91      3.00     61.19    249.00        0.91

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4137E+01 EXCESS=0.0000E+00 OUTFLOW=0.4138E+01 BASIN STORAGE=0.5190E-06 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
             R2N-2  MANE         0.47     35.73    249.69      0.62      3.00     35.56    249.00        0.61

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2788E+01 EXCESS=0.0000E+00 OUTFLOW=0.2789E+01 BASIN STORAGE=0.5140E-06 PERCENT ERROR=  -0.1

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
             R2N-2  MANE         0.30     82.46    249.44      1.65      3.00     82.26    249.00        1.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7500E+01 EXCESS=0.0000E+00 OUTFLOW=0.7502E+01 BASIN STORAGE=0.4816E-06 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
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            R2N-2  MANE         0.39     78.41    249.52      1.56      3.00     78.24    249.00        1.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7064E+01 EXCESS=0.0000E+00 OUTFLOW=0.7065E+01 BASIN STORAGE=0.4315E-06 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2-3  MANE         2.04    347.37    255.29      2.70      3.00    347.37    255.00        2.70

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2619E+02 EXCESS=0.0000E+00 OUTFLOW=0.2617E+02 BASIN STORAGE=0.4195E-01 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R2-3  MANE         1.91    345.65    255.87      2.68      3.00    345.42    255.00        2.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2603E+02 EXCESS=0.0000E+00 OUTFLOW=0.2597E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R2-3  MANE         2.25    225.57    259.44      2.35      3.00    224.41    261.00        2.34

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2276E+02 EXCESS=0.0000E+00 OUTFLOW=0.2272E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
             R2-3  MANE         2.53    172.53    260.84      2.05      3.00    172.47    261.00        2.05

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1991E+02 EXCESS=0.0000E+00 OUTFLOW=0.1985E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
             R2-3  MANE         2.96    118.90    263.24      1.68      3.00    118.75    264.00        1.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1630E+02 EXCESS=0.0000E+00 OUTFLOW=0.1624E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.1

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
             R2-3  MANE         2.34    231.88    262.66      2.98      3.00    231.53    261.00        2.97

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2888E+02 EXCESS=0.0000E+00 OUTFLOW=0.2882E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.1

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
             R2-3  MANE         2.34    221.80    263.10      2.85      3.00    221.59    261.00        2.85

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2770E+02 EXCESS=0.0000E+00 OUTFLOW=0.2764E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R3-5  MANE         3.00    412.68    261.29      2.56      3.00    410.64    261.00        2.57

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3128E+02 EXCESS=0.0000E+00 OUTFLOW=0.3139E+02 BASIN STORAGE=0.7696E-01 PERCENT ERROR=  -0.6

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R3-5  MANE         3.00    410.61    261.45      2.55      3.00    407.61    261.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3106E+02 EXCESS=0.0000E+00 OUTFLOW=0.3119E+02 BASIN STORAGE=0.7696E-01 PERCENT ERROR=  -0.7

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R3-5  MANE         3.00    268.85    266.94      2.23      3.00    268.82    267.00        2.23

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2736E+02 EXCESS=0.0000E+00 OUTFLOW=0.2730E+02 BASIN STORAGE=0.8862E-01 PERCENT ERROR=  -0.1

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
             R3-5  MANE         3.00    204.70    270.05      1.95      3.00    204.66    270.00        1.95
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2395E+02 EXCESS=0.0000E+00 OUTFLOW=0.2390E+02 BASIN STORAGE=0.9259E-01 PERCENT ERROR=  -0.2


          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
              R3-5  MANE        3.00    139.39    275.21      1.59      3.00    139.08    276.00        1.59


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1956E+02 EXCESS=0.0000E+00 OUTFLOW=0.1945E+02 BASIN STORAGE=0.9871E-01 PERCENT ERROR=   0.0


          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
              R3-5  MANE        3.00    276.07    269.91      2.86      3.00    276.00    270.00        2.86


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3506E+02 EXCESS=0.0000E+00 OUTFLOW=0.3497E+02 BASIN STORAGE=0.9253E-01 PERCENT ERROR=   0.0


          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
              R3-5  MANE        3.00    263.45    268.98      2.73      3.00    263.27    270.00        2.73


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3361E+02 EXCESS=0.0000E+00 OUTFLOW=0.3346E+02 BASIN STORAGE=0.9261E-01 PERCENT ERROR=   0.2


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7W-8W  MANE        2.73    102.63    267.82      1.79      3.00    102.19    267.00        1.79


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3402E+01 EXCESS=0.0000E+00 OUTFLOW=0.4461E+01 BASIN STORAGE=0.3428E-03 PERCENT ERROR= -31.1


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R7W-8W  MANE        2.59    104.73    266.79      1.55      3.00    104.06    267.00        1.55


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3367E+01 EXCESS=0.0000E+00 OUTFLOW=0.3871E+01 BASIN STORAGE=0.3311E-03 PERCENT ERROR= -15.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R7W-8W  MANE        2.99     52.62    270.98      1.33      3.00     52.50    273.00        1.33


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2832E+01 EXCESS=0.0000E+00 OUTFLOW=0.3326E+01 BASIN STORAGE=0.3101E-03 PERCENT ERROR= -17.5


          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R7W-8W  MANE        3.00     37.32    275.61      1.13      3.00     37.24    276.00        1.13


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2292E+01 EXCESS=0.0000E+00 OUTFLOW=0.2822E+01 BASIN STORAGE=0.3108E-03 PERCENT ERROR= -23.1


          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R7W-8W  MANE        3.00     21.45    284.63      0.78      3.00     21.39    282.00        0.78


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1544E+01 EXCESS=0.0000E+00 OUTFLOW=0.1942E+01 BASIN STORAGE=0.3316E-03 PERCENT ERROR= -25.8


          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R7W-8W  MANE        3.00     49.46    273.94      1.67      3.00     49.38    273.00        1.67


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4156E+01 EXCESS=0.0000E+00 OUTFLOW=0.4179E+01 BASIN STORAGE=0.3376E-03 PERCENT ERROR=  -0.6


          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R7W-8W  MANE        3.00     47.01    271.91      1.58      3.00     46.97    273.00        1.58


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3912E+01 EXCESS=0.0000E+00 OUTFLOW=0.3956E+01 BASIN STORAGE=0.3474E-03 PERCENT ERROR=  -1.1


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NW8N  MANE        0.75     40.02    244.34      1.98      3.00     39.44    243.00        1.98


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1550E+01 EXCESS=0.0000E+00 OUTFLOW=0.1552E+01 BASIN STORAGE=0.1215E-06 PERCENT ERROR=  -0.1
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          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8NW8N  MANE         0.77     39.68    244.02      1.96      3.00     39.17    243.00        1.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1538E+01 EXCESS=0.0000E+00 OUTFLOW=0.1540E+01 BASIN STORAGE=0.1352E-06 PERCENT ERROR=  -0.2

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8NW8N  MANE         0.97     21.41    244.47      1.86      3.00     21.33    243.00        1.86

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1449E+01 EXCESS=0.0000E+00 OUTFLOW=0.1455E+01 BASIN STORAGE=0.1154E-06 PERCENT ERROR=  -0.4

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R8NW8N  MANE         1.05     16.17    244.69      1.68      3.00     16.11    243.00        1.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1313E+01 EXCESS=0.0000E+00 OUTFLOW=0.1317E+01 BASIN STORAGE=0.1153E-06 PERCENT ERROR=  -0.2

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R8NW8N  MANE         1.03     10.82    245.03      1.37      3.00     10.72    243.00        1.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1074E+01 EXCESS=0.0000E+00 OUTFLOW=0.1077E+01 BASIN STORAGE=0.1398E-06 PERCENT ERROR=  -0.3

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R8NW8N  MANE         0.91     19.33    244.37      2.65      3.00     19.25    243.00        2.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2076E+01 EXCESS=0.0000E+00 OUTFLOW=0.2081E+01 BASIN STORAGE=0.1314E-06 PERCENT ERROR=  -0.2

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R8NW8N  MANE         0.88     18.61    244.50      2.54      3.00     18.51    243.00        2.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1987E+01 EXCESS=0.0000E+00 OUTFLOW=0.1989E+01 BASIN STORAGE=0.1316E-06 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NE8N  MANE         0.69     42.06    244.33      1.98      3.00     41.68    243.00        1.98

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1579E+01 EXCESS=0.0000E+00 OUTFLOW=0.1583E+01 BASIN STORAGE=0.1124E-06 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8NE8N  MANE         0.68     41.82    244.25      1.96      3.00     41.41    243.00        1.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1567E+01 EXCESS=0.0000E+00 OUTFLOW=0.1569E+01 BASIN STORAGE=0.1055E-06 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8NE8N  MANE         0.92     22.13    243.92      1.85      3.00     22.08    243.00        1.86

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1476E+01 EXCESS=0.0000E+00 OUTFLOW=0.1484E+01 BASIN STORAGE=0.1034E-06 PERCENT ERROR=  -0.5

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R8NE8N  MANE         0.97     16.69    243.96      1.68      3.00     16.65    243.00        1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1338E+01 EXCESS=0.0000E+00 OUTFLOW=0.1340E+01 BASIN STORAGE=0.1343E-06 PERCENT ERROR=  -0.2

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R8NE8N  MANE         1.12     11.10    244.46      1.37      3.00     11.06    243.00        1.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1094E+01 EXCESS=0.0000E+00 OUTFLOW=0.1095E+01 BASIN STORAGE=0.1098E-06 PERCENT ERROR=  -0.1

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R8NE8N  MANE         0.90     19.88    244.18      2.65      3.00     19.84    243.00        2.65
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2116E+01 EXCESS=0.0000E+00 OUTFLOW=0.2117E+01 BASIN STORAGE=0.9623E-07 PERCENT ERROR=  -0.1


          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R8NE8N  MANE        0.86     19.14    244.71      2.54      3.00     19.09    243.00        2.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2025E+01 EXCESS=0.0000E+00 OUTFLOW=0.2028E+01 BASIN STORAGE=0.1302E-06 PERCENT ERROR=  -0.2


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8N-8W  MANE        0.52    152.44    243.65      1.98      3.00    150.95    243.00        1.98


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5871E+01 EXCESS=0.0000E+00 OUTFLOW=0.5874E+01 BASIN STORAGE=0.1353E-07 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8N-8W  MANE        0.51    151.61    243.84      1.96      3.00    149.94    243.00        1.96


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+01 EXCESS=0.0000E+00 OUTFLOW=0.5823E+01 BASIN STORAGE=0.1343E-07 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8N-8W  MANE        0.53     81.55    243.95      1.85      3.00     81.15    243.00        1.85


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5500E+01 EXCESS=0.0000E+00 OUTFLOW=0.5501E+01 BASIN STORAGE=0.1336E-07 PERCENT ERROR=   0.0


          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R8N-8W  MANE        0.65     61.43    243.75      1.68      3.00     61.23    243.00        1.68


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4978E+01 EXCESS=0.0000E+00 OUTFLOW=0.4978E+01 BASIN STORAGE=0.1320E-07 PERCENT ERROR=   0.0


          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R8N-8W  MANE        0.74     40.89    244.25      1.37      3.00     40.71    243.00        1.37


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4070E+01 EXCESS=0.0000E+00 OUTFLOW=0.4071E+01 BASIN STORAGE=0.1350E-07 PERCENT ERROR=   0.0


          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R8N-8W  MANE        0.57     73.39    244.05      2.65      3.00     73.13    243.00        2.65


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7867E+01 EXCESS=0.0000E+00 OUTFLOW=0.7867E+01 BASIN STORAGE=0.1321E-07 PERCENT ERROR=   0.0


          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R8N-8W  MANE        0.64     70.52    244.05      2.53      3.00     70.34    243.00        2.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7526E+01 EXCESS=0.0000E+00 OUTFLOW=0.7527E+01 BASIN STORAGE=0.1341E-07 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R4N-4  MANE        0.64    254.10    249.72      1.92      3.00    252.86    249.00        1.92


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1911E+02 EXCESS=0.0000E+00 OUTFLOW=0.1914E+02 BASIN STORAGE=0.1270E-03 PERCENT ERROR=  -0.1


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R4N-4  MANE        0.55    253.13    250.14      1.85      3.00    251.46    249.00        1.85


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1840E+02 EXCESS=0.0000E+00 OUTFLOW=0.1841E+02 BASIN STORAGE=0.1340E-03 PERCENT ERROR=  -0.1


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R4N-4  MANE        0.69    184.86    247.60      1.71      3.00    184.57    249.00        1.71


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1702E+02 EXCESS=0.0000E+00 OUTFLOW=0.1703E+02 BASIN STORAGE=0.1322E-03 PERCENT ERROR=   0.0
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          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
             R4N-4  MANE         0.69    142.09    249.64      1.53      3.00    141.96    249.00        1.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1522E+02 EXCESS=0.0000E+00 OUTFLOW=0.1523E+02 BASIN STORAGE=0.1308E-03 PERCENT ERROR=  -0.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
             R4N-4  MANE         0.75     95.95    250.50      1.21      3.00     95.49    249.00        1.21

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1207E+02 EXCESS=0.0000E+00 OUTFLOW=0.1207E+02 BASIN STORAGE=0.1249E-03 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
             R4N-4  MANE         0.67    173.19    249.80      2.39      3.00    173.09    249.00        2.39

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2383E+02 EXCESS=0.0000E+00 OUTFLOW=0.2383E+02 BASIN STORAGE=0.1314E-03 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
             R4N-4  MANE         0.66    166.58    249.65      2.29      3.00    166.45    249.00        2.29

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2276E+02 EXCESS=0.0000E+00 OUTFLOW=0.2276E+02 BASIN STORAGE=0.1294E-03 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R4-5  MANE         2.45    529.86    252.44      2.01      3.00    529.34    252.00        2.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3538E+02 EXCESS=0.0000E+00 OUTFLOW=0.3544E+02 BASIN STORAGE=0.4077E-03 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R4-5  MANE         2.35    527.86    252.26      1.96      3.00    527.49    252.00        1.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3453E+02 EXCESS=0.0000E+00 OUTFLOW=0.3453E+02 BASIN STORAGE=0.3991E-03 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R4-5  MANE         2.78    353.24    251.22      1.81      3.00    352.97    252.00        1.81

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3197E+02 EXCESS=0.0000E+00 OUTFLOW=0.3199E+02 BASIN STORAGE=0.4213E-03 PERCENT ERROR=  -0.1

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
              R4-5  MANE         3.00    268.61    253.06      1.63      3.00    266.78    252.00        1.63

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2865E+02 EXCESS=0.0000E+00 OUTFLOW=0.2866E+02 BASIN STORAGE=0.4088E-03 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
              R4-5  MANE         3.00    177.35    253.32      1.31      3.00    177.27    255.00        1.31

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2309E+02 EXCESS=0.0000E+00 OUTFLOW=0.2313E+02 BASIN STORAGE=0.3969E-03 PERCENT ERROR=  -0.2

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
              R4-5  MANE         2.81    333.62    253.57      2.50      3.00    332.61    252.00        2.50

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4397E+02 EXCESS=0.0000E+00 OUTFLOW=0.4403E+02 BASIN STORAGE=0.4063E-03 PERCENT ERROR=  -0.1

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
              R4-5  MANE         2.77    320.55    252.33      2.39      3.00    319.94    252.00        2.39

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4208E+02 EXCESS=0.0000E+00 OUTFLOW=0.4214E+02 BASIN STORAGE=0.4005E-03 PERCENT ERROR=  -0.2

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R5-6  MANE         2.16    991.74    263.54      2.17      3.00    984.16    264.00        2.17
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7955E+02 EXCESS=0.0000E+00 OUTFLOW=0.7943E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R5-6  MANE         2.15    982.67    263.70      2.14      3.00    977.43    264.00        2.14

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7838E+02 EXCESS=0.0000E+00 OUTFLOW=0.7827E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R5-6  MANE         2.55    669.02    267.79      1.93      3.00    667.64    267.00        1.93

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7065E+02 EXCESS=0.0000E+00 OUTFLOW=0.7061E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
              R5-6  MANE         2.89    496.65    268.90      1.71      3.00    495.45    270.00        1.70

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6247E+02 EXCESS=0.0000E+00 OUTFLOW=0.6240E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
              R5-6  MANE         3.00    319.50    275.61      1.37      3.00    319.38    276.00        1.38

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5039E+02 EXCESS=0.0000E+00 OUTFLOW=0.5032E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
              R5-6  MANE         2.50    684.58    266.23      2.57      3.00    683.78    264.00        2.57

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9427E+02 EXCESS=0.0000E+00 OUTFLOW=0.9421E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
              R5-6  MANE         2.52    653.44    264.51      2.46      3.00    652.59    264.00        2.46

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9018E+02 EXCESS=0.0000E+00 OUTFLOW=0.9012E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7E-8E  MANE         1.76     19.03    266.67      0.91      3.00     19.02    267.00        0.91

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1268E+01 EXCESS=0.0000E+00 OUTFLOW=0.1268E+01 BASIN STORAGE=0.1101E-04 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R7E-8E  MANE         1.77     18.85    266.81      0.90      3.00     18.84    267.00        0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1254E+01 EXCESS=0.0000E+00 OUTFLOW=0.1254E+01 BASIN STORAGE=0.1092E-04 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R7E-8E  MANE         2.41      8.57    268.50      0.49      3.00      8.54    270.00        0.49

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6750E+00 EXCESS=0.0000E+00 OUTFLOW=0.6756E+00 BASIN STORAGE=0.1319E-04 PERCENT ERROR=  -0.1

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R7E-8E  MANE         3.00      3.99    269.13      0.21      3.00      3.99    270.00        0.21

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2917E+00 EXCESS=0.0000E+00 OUTFLOW=0.2928E+00 BASIN STORAGE=0.1256E-04 PERCENT ERROR=  -0.4

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R7E-8E  MANE         3.00      0.26    292.33      0.01      3.00      0.25    294.00        0.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1646E-01 EXCESS=0.0000E+00 OUTFLOW=0.2054E-01 BASIN STORAGE=0.1096E-04 PERCENT ERROR= -24.8
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          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R7E-8E  MANE         2.36      9.61    272.07      0.60      3.00      9.60    273.00        0.60

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8313E+00 EXCESS=0.0000E+00 OUTFLOW=0.8321E+00 BASIN STORAGE=0.1454E-04 PERCENT ERROR=  -0.1

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R7E-8E  MANE         2.43      8.70    271.22      0.54      3.00      8.69    273.00        0.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7492E+00 EXCESS=0.0000E+00 OUTFLOW=0.7500E+00 BASIN STORAGE=0.1049E-04 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R9N-R9  MANE         2.26     13.31    278.00      1.08      3.00     13.26    279.00        1.08

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1613E+01 EXCESS=0.0000E+00 OUTFLOW=0.1611E+01 BASIN STORAGE=0.2018E-04 PERCENT ERROR=   0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R9N-R9  MANE         2.27     13.16    278.20      1.07      3.00     13.12    279.00        1.07

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1595E+01 EXCESS=0.0000E+00 OUTFLOW=0.1593E+01 BASIN STORAGE=0.1743E-04 PERCENT ERROR=   0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R9N-R9  MANE         2.76      8.13    284.79      0.74      3.00      8.13    285.00        0.74

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1103E+01 EXCESS=0.0000E+00 OUTFLOW=0.1102E+01 BASIN STORAGE=0.2044E-04 PERCENT ERROR=   0.1

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R9N-R9  MANE         3.00      5.05    284.63      0.46      3.00      5.05    285.00        0.46

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6843E+00 EXCESS=0.0000E+00 OUTFLOW=0.6838E+00 BASIN STORAGE=0.2050E-04 PERCENT ERROR=   0.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R9N-R9  MANE         3.00      1.59    293.56      0.14      3.00      1.59    294.00        0.14

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2079E+00 EXCESS=0.0000E+00 OUTFLOW=0.2079E+00 BASIN STORAGE=0.1894E-04 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R9N-R9  MANE         2.65      9.46    288.90      0.92      3.00      9.46    288.00        0.92

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1380E+01 EXCESS=0.0000E+00 OUTFLOW=0.1379E+01 BASIN STORAGE=0.1805E-04 PERCENT ERROR=   0.1

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R9N-R9  MANE         2.68      8.74    288.39      0.84      3.00      8.73    288.00        0.84

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1261E+01 EXCESS=0.0000E+00 OUTFLOW=0.1260E+01 BASIN STORAGE=0.1932E-04 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8E-R9  MANE         0.32     77.02    273.53      0.96      3.00     76.82    273.00        0.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6728E+01 EXCESS=0.0000E+00 OUTFLOW=0.6728E+01 BASIN STORAGE=0.7838E-05 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8E-R9  MANE         0.32     76.18    273.66      0.95      3.00     75.99    273.00        0.95

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6652E+01 EXCESS=0.0000E+00 OUTFLOW=0.6653E+01 BASIN STORAGE=0.7629E-05 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
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          R8E-R9  MANE         0.45     38.17    276.54      0.55      3.00     38.13    276.00        0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3882E+01 EXCESS=0.0000E+00 OUTFLOW=0.3882E+01 BASIN STORAGE=0.7462E-05 PERCENT ERROR=   0.0

        FOR STORM = 4  STORM AREA (SQ MI) =      16.00
          R8E-R9  MANE         0.52     19.07    276.61      0.27      3.00     19.04    276.00        0.27

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1925E+01 EXCESS=0.0000E+00 OUTFLOW=0.1925E+01 BASIN STORAGE=0.7540E-05 PERCENT ERROR=   0.0

        FOR STORM = 5  STORM AREA (SQ MI) =      90.00
          R8E-R9  MANE         0.94      2.52    293.35      0.04      3.00      2.51    294.00        0.04

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3008E+00 EXCESS=0.0000E+00 OUTFLOW=0.3008E+00 BASIN STORAGE=0.6726E-05 PERCENT ERROR=   0.0

        FOR STORM = 6  STORM AREA (SQ MI) =      10.00
          R8E-R9  MANE         0.36     44.12    279.67      0.68      3.00     44.08    279.00        0.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4782E+01 EXCESS=0.0000E+00 OUTFLOW=0.4782E+01 BASIN STORAGE=0.7472E-05 PERCENT ERROR=   0.0

        FOR STORM = 7  STORM AREA (SQ MI) =      20.00
          R8E-R9  MANE         0.43     40.11    279.56      0.62      3.00     40.07    279.00        0.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4334E+01 EXCESS=0.0000E+00 OUTFLOW=0.4334E+01 BASIN STORAGE=0.7666E-05 PERCENT ERROR=   0.0

        FOR STORM = 1  STORM AREA (SQ MI) =       0.00
          BSNRT1  MANE         0.39    244.41    552.63      0.80      3.00    244.41    552.00        0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3084E+03 EXCESS=0.0000E+00 OUTFLOW=0.3084E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR=   0.0

        FOR STORM = 2  STORM AREA (SQ MI) =       0.50
          BSNRT1  MANE         0.39    244.36    552.54      0.80      3.00    244.36    552.00        0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3083E+03 EXCESS=0.0000E+00 OUTFLOW=0.3082E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR=   0.0

        FOR STORM = 3  STORM AREA (SQ MI) =       2.80
          BSNRT1  MANE         0.41    241.71    552.95      0.79      3.00    241.71    552.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3040E+03 EXCESS=0.0000E+00 OUTFLOW=0.3039E+03 BASIN STORAGE=0.3152E-01 PERCENT ERROR=   0.0

        FOR STORM = 4  STORM AREA (SQ MI) =      16.00
          BSNRT1  MANE         0.33    239.65    552.95      0.78      3.00    239.64    555.00        0.78

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3008E+03 EXCESS=0.0000E+00 OUTFLOW=0.3007E+03 BASIN STORAGE=0.3152E-01 PERCENT ERROR=   0.0

        FOR STORM = 5  STORM AREA (SQ MI) =      90.00
          BSNRT1  MANE         0.44    237.69    555.51      0.77      3.00    237.69    555.00        0.77

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2977E+03 EXCESS=0.0000E+00 OUTFLOW=0.2976E+03 BASIN STORAGE=0.3151E-01 PERCENT ERROR=   0.0

        FOR STORM = 6  STORM AREA (SQ MI) =      10.00
          BSNRT1  MANE         0.46    242.84    552.79      0.79      3.00    242.84    552.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3057E+03 EXCESS=0.0000E+00 OUTFLOW=0.3057E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR=   0.0

        FOR STORM = 7  STORM AREA (SQ MI) =      20.00
          BSNRT1  MANE         0.37    242.32    552.68      0.79      3.00    242.32    552.00        0.79
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3049E+03 EXCESS=0.0000E+00 OUTFLOW=0.3049E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE         0.50    244.41    552.74      0.80      3.00    244.41    552.00        0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3084E+03 EXCESS=0.0000E+00 OUTFLOW=0.3083E+03 BASIN STORAGE=0.3601E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            BSNRT2  MANE         0.43    244.36    552.89      0.80      3.00    244.36    552.00        0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3082E+03 EXCESS=0.0000E+00 OUTFLOW=0.3082E+03 BASIN STORAGE=0.3601E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            BSNRT2  MANE         0.49    241.71    553.29      0.79      3.00    241.71    555.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3040E+03 EXCESS=0.0000E+00 OUTFLOW=0.3039E+03 BASIN STORAGE=0.3600E-01 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            BSNRT2  MANE         0.36    239.64    555.76      0.78      3.00    239.64    555.00        0.78

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3007E+03 EXCESS=0.0000E+00 OUTFLOW=0.3007E+03 BASIN STORAGE=0.3599E-01 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            BSNRT2  MANE         0.37    237.69    555.79      0.77      3.00    237.69    555.00        0.77

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2976E+03 EXCESS=0.0000E+00 OUTFLOW=0.2976E+03 BASIN STORAGE=0.3598E-01 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            BSNRT2  MANE         0.50    242.84    553.18      0.79      3.00    242.83    552.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3057E+03 EXCESS=0.0000E+00 OUTFLOW=0.3057E+03 BASIN STORAGE=0.3600E-01 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            BSNRT2  MANE         0.36    242.32    552.92      0.79      3.00    242.31    555.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3049E+03 EXCESS=0.0000E+00 OUTFLOW=0.3048E+03 BASIN STORAGE=0.3600E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R10-11  MANE         3.04    244.41    557.55     25.54      3.00    244.41    558.00       25.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3125E+03 EXCESS=0.0000E+00 OUTFLOW=0.3119E+03 BASIN STORAGE=0.6746E+00 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R10-11  MANE         3.07    244.36    558.85     25.53      3.00    244.35    558.00       25.52

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3124E+03 EXCESS=0.0000E+00 OUTFLOW=0.3117E+03 BASIN STORAGE=0.6746E+00 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R10-11  MANE         3.00    241.71    559.89     25.15      3.00    241.70    561.00       25.13

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3077E+03 EXCESS=0.0000E+00 OUTFLOW=0.3071E+03 BASIN STORAGE=0.6744E+00 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R10-11  MANE         3.00    239.64    561.16     24.84      3.00    239.64    561.00       24.84

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3040E+03 EXCESS=0.0000E+00 OUTFLOW=0.3034E+03 BASIN STORAGE=0.6743E+00 PERCENT ERROR=   0.0
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          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R10-11  MANE         3.07    237.68    560.85     24.53      3.00    237.68    561.00       24.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3003E+03 EXCESS=0.0000E+00 OUTFLOW=0.2996E+03 BASIN STORAGE=0.6743E+00 PERCENT ERROR=   0.0


          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R10-11  MANE         3.07    242.83    560.81     25.40      3.00    242.83    558.00       25.38


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3108E+03 EXCESS=0.0000E+00 OUTFLOW=0.3102E+03 BASIN STORAGE=0.6744E+00 PERCENT ERROR=   0.0


          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R10-11  MANE         3.00    242.31    558.78     25.30      3.00    242.31    561.00       25.30


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3097E+03 EXCESS=0.0000E+00 OUTFLOW=0.3090E+03 BASIN STORAGE=0.6745E+00 PERCENT ERROR=   0.0




*** NORMAL END OF HEC-1 ***
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100yr-24hr-Ultimate

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1***************************************                              **************************************
 *                                     *                              *                                    *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                              *    U.S. ARMY CORPS OF ENGINEERS    *
 *             JUN   1998               *                              *    HYDROLOGIC ENGINEERING CENTER   *
 *           VERSION 4.1                *                              *         609 SECOND STREET          *
 *                                     *                              *       DAVIS, CALIFORNIA 95616      *
 *  RUN DATE   20JUL23  TIME  10:25:33   *                              *           (916) 756-1104           *
 *                                     *                              *                                    *
 ***************************************                              **************************************



                    X     X  XXXXXXX   XXXXX          X
                    X     X  X        X     X        XX
                    X     X  X        X               X
                    XXXXXXX  XXXX     X        XXXXX  X
                    X     X  X        X               X
                    X     X  X        X     X         X
                    X     X  XXXXXXX   XXXXX         XXX



      THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

      THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
      THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
      NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
      DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
      KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


1                                        HEC-1 INPUT                                        PAGE  1

          LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

             1          ID         CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
             2          ID         POWERLINE CORRIDOR
             3          ID         RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
             4          ID         INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
             5          ID        100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
             6          ID            PIMA ROAD FLOWS.
             7          ID
             8          ID         PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
             9          ID        MODELERS: RK, MW
            10          ID
            11          ID         REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 08/07/18
            12          ID        MODELERS: MSW, ES
            13          ID        REVISIONS NOTED WITH HE
            14          ID            NEW PROP CONDITION MODEL,WITH POWERLINE AND BASIN 53R
            15          ID            THIS IS THE ULTIMATE CONDITION MODEL
            16          ID
            17          ID         REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
            18          ID        MODELERS: MSW
            19          ID        XKSAT ADJUSTMENT
            20          ID
            21          ID         REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
            22          ID        MODELERS: MSW, TSW
            23          ID        REVISIONS NOTED WITH HE (Hubbard Engineering)
            24          ID            NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
            25          ID            CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
            26          ID            ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
            27          ID            FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED
            28          ID
            29          ID         REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/20/23
            30          ID            MODELERS: MSW, TSW
            31          ID            REVISIONS NOTED WITH HE (HUBBARD ENGINEERING)
            32          ID            BASIN 2E WAS SPLIT AT CENTER POINT DUE TO FLOW UNDER 101
            33          ID
            34          IT       3       0       0    1000
            35          IN      15
            36          IO       5
                        *DIAGRAM
                        *
            37          JD   3.849  0.0001
            38          PC   0.000   0.002   0.005   0.008   0.011   0.014   0.017   0.020   0.023   0.026
            39          PC   0.029   0.032   0.035   0.038   0.041   0.044   0.048   0.052   0.056   0.060
            40          PC   0.064   0.068   0.072   0.076   0.080   0.085   0.090   0.095   0.100   0.105
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   41          PC   0.110   0.115   0.120   0.126   0.133   0.140   0.147   0.155   0.163   0.172
   42          PC   0.181   0.191   0.203   0.218   0.236   0.257   0.283   0.387   0.663   0.707
   43          PC   0.735   0.758   0.776   0.791   0.804   0.815   0.825   0.834   0.842   0.849
   44          PC   0.856   0.863   0.869   0.875   0.881   0.887   0.893   0.898   0.903   0.908
   45          PC   0.913   0.918   0.922   0.926   0.930   0.934   0.938   0.942   0.946   0.950
   46          PC   0.953   0.956   0.959   0.962   0.965   0.968   0.971   0.974   0.977   0.980
   47          PC   0.983   0.986   0.989   0.992   0.995   0.998   1.000
   48          JD   3.657    10.0
   49          JD   3.533    20.0
               *
1                                         HEC-1 INPUT                                         PAGE  2

 LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


   50          KK  76THST
   51          KM        76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
   52          KM        100-YR, 24-HR FLO-2D MODEL (XS 98)
   53          BA    0.24
   54          QI    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00
   55          QI    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00
   56          QI    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.01
   57          QI    0.00    0.00    0.00    0.00    0.01    0.00    0.00    0.02    0.03    0.05
   58          QI    0.09    0.11    0.14    0.21    0.34    0.51    0.76    3.70   17.69   47.79
   59          QI   68.57   54.61   42.75   35.61   32.34   29.25   26.13   22.83   19.41   18.11
   60          QI   16.15   14.54   13.75   12.53   12.00   11.44   10.52    9.81    9.34    9.09
   61          QI    9.11    8.71    8.37    8.05    7.67    7.75    7.47    7.08    6.97    6.77
   62          QI    6.59    6.44    6.32    6.12    5.95    5.87    5.70    5.84    5.41    5.44
   63          QI    4.56    4.81    4.75    4.78    4.69    4.67    4.61    4.36    4.20    4.00
   64          QI    3.75    3.51    2.49    3.43    3.61    3.65    2.48    2.36    2.19    2.07
   65          QI    1.94    1.72    1.71    1.36    1.48    1.13    1.13    1.20    1.10    1.06
   66          QI    1.06    1.00    0.94    0.89    0.85    0.81    0.78    0.74    0.72    0.70
   67          QI    0.67    0.64    0.62    0.59    0.57    0.55    0.53    0.51    0.49    0.47
   68          QI    0.45    0.44    0.42    0.40    0.39
               *

   69          KK   SB01A   BASIN
   70          BA   0.061
   71          PB   2.721
   72          PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.075
   73          PC   0.087   0.099   0.119   0.150   0.234   0.413   0.766   0.875   0.916   0.944
   74          PC   0.956   0.968   0.979   0.990   1.000
   75          LG    0.25    0.25    4.03    0.56      45
   76          UC   0.439   0.231
   77          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
   78          UA     100
               *

   79          KK   SB01A  DIVERT
   80          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01A
   81          KM        FIRST FLUSH VOL = 1.383 AC-FT -HE
   82          DT  RET01A   1.383
   83          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
   84          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

   85          KK    CP-1 COMBINE
   86          KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
   87          HC       2   0.301
               *

   88          KK  R1A-2W   ROUTE
   89          KM        ROUTE CP-1 TO CP-2W
   90          RK    1114  0.0200   0.018           TRAP   2.000    0.00
               *
1                                         HEC-1 INPUT                                         PAGE  3

 LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


   91          KK  SB02NW   BASIN
   92          KM        BASIN SB02E WAS SPLIT INTO SUB BASINS NORTH OF CAVASSON
   93          KM        AND SOUTH OF CAVASSON    -HE
   94          BA  0.0193
   95          LG    0.25    0.25    4.03    0.56      45
   96          UC   0.276   0.224
   97          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
   98          UA     100
               *
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 99           KK  SB02NW  DIVERT
100           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NW
101           KM        FIRST FLUSH VOL = 0.437 AC-FT -HE
102           DT  RET2NW   0.437
103           DI     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
104           DI    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
105           DQ     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
106           DQ    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
              *

107           KK  CP-02W COMBINE
108           KM        COMBINE ROUTE FROM 1A-2W WITH NEW SUB BASIN SB02NW
109           HC       2   0.305
              *

110           KK   R2W-2   ROUTE
111           RK    1380  0.0140   0.018            TRAP   2.000    0.00
              *

112           KK   SB01C   BASIN
113           BA   0.027
114           LG    0.25    0.25    4.03    0.56      45
115           UC   0.345   0.366
116           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
117           UA     100
              *

118           KK   SB01C  DIVERT
119           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01C
120           KM        FIRST FLUSH VOL = 0.612 AC-FT -HE
121           DT  RET01C   0.612
122           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
123           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

124           KK  R1C-1B   ROUTE
125           RK     542   0.005   0.013            CIRC   5.000
              *
```



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LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


126           KK   SB01B   BASIN
127           BA   0.024
128           LG    0.25    0.25    4.03    0.56      45
129           UC   0.304   0.298
130           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
131           UA     100
              *

132           KK   SB01B  DIVERT
133           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01B
134           KM        FIRST FLUSH VOL = 0.543 AC-FT -HE
135           DT  RET01B   0.543
136           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
137           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

138           KK  CP-01B COMBINE
139           KM        COMBINE ROUTE 1C-1B WITH SB01B
140           HC       2   0.051
              *

141           KK  R1B-2N   ROUTE
142           RK    1268   0.015   0.025            TRAP   80.00    0.00
              *

143           KK  SB02NE   BASIN
144           BA  0.0335
145           LG    0.25    0.25    4.03    0.56      45
146           UC   0.406   0.448
147           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
148           UA     100
              *

149           KK  SB02NE  DIVERT
150           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NE
151           KM        FIRST FLUSH VOL = 0.760 AC-FT -HE
152           DT  RET2NE   0.760
```

```
       153          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
       154          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                    *

       155          KK  CP-02N COMBINE
       156          KM        COMBINE ROUTE 1B-2N (BASINS SB01-B AND SB01-C) WITH SB02NE
       157          HC       2   0.085
                    *

       158          KK   R2N-2   ROUTE
       159          RK    1380   0.014   0.018            TRAP   2.000    0.00
                    *
1                                          HEC-1 INPUT                                         PAGE  5

      LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


       160          KK  SB02E2   BASIN
       161          KM        BASIN 2E HAD TO BE SPLIT AT CENTER POINT DUE TO FLOW UNDER LOOP 101
       162          KM        THIS IS THE WEST SPLIT -HE
       163          BA   0.019
       164          LG    0.10    0.25    4.03    0.61      80
       165          UC   0.210   0.239
       166          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
       167          UA     100
                    *

       168          KK  SB02E2  DIVERT
       169          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E
       170          KM        FIRST FLUSH VOL = 0.428 AC-FT -HE
       171          DT  RET2E2   0.428
       172          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
       173          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                    *

       174          KK   SB02W   BASIN
       175          BA  0.0571
       176          LG    0.35    0.35    4.33    0.51       0
       177          UC   0.686   0.535
       178          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
       179          UA     100
                    *

       180          KK   CP-02 COMBINE
       181          HC       4  0.1816
                    *

       182          KK    R2-3   ROUTE
       183          KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
       184          RK    1260  0.0015   0.030            TRAP      92       4
                    *

       185          KK    SB03   BASIN
       186          BA   0.048
       187          LG    0.15    0.25    4.50    0.47      55
       188          UC   0.364   0.294
       189          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
       190          UA     100
                    *

       191          KK   CP-03 COMBINE
       192          HC       2
                    *

       193          KK    R3-5   ROUTE
       194          KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
       195          RK    2396  0.0015   0.03             TRAP      98       4
                    *
1                                          HEC-1 INPUT                                         PAGE  6

      LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


       196          KK    SB05   BASIN
       197          BA   0.126
       198          LG    0.22    0.25    4.50    0.44      48
       199          UC   0.327   0.226
       200          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
       201          UA     100
                    *
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```
 202            KK   CP-5A COMBINE
 203            HC       2
                *

 204            KK   SB07W   BASIN
 205            BA  0.0468
 206            LG    0.25    0.25    4.03    0.56      45
 207            UC   0.323   0.237
 208            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 209            UA     100
                *

 210            KK   SB07W  DIVERT
 211            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB07W
 212            KM        FIRST FLUSH VOL = 1.060 AC-FT -HE
 213            DT  RET07W   1.060
 214            DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 215            DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                *

 216            KK  R7W-8W   ROUTE
 217            KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 218            KM        HE MODIFIED ROUTE TO LOOP 101 W OF HAYDEN
 219            RK    3754  0.0013   0.03            TRAP      46       4
                *

 220            KK   SB8NW   BASIN
 221            KM        BASIN JUST SOUTH OF LEGACY - FUTURE MOB SITE -HE
 222            BA  0.0147
 223            LG    0.10    0.25    4.03    0.61      80
 224            UC   0.252   0.200
 225            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 226            UA     100
                *

 227            KK   SB8NW  DIVERT
 228            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NW
 229            KM        FIRST FLUSH VOL = 0.332 AC-FT -HE
 230            DT  RET8NW   0.332
 231            DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 232            DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                *
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LINE            ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 233            KK  R8NW8N   ROUTE
 234            KM        ROUTE NEW SUB BASIN SB8NW TO OUTLET AT CAVASSON BLVD  -HE
 235            KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 236            RK     643  0.0113   0.018            TRAP      46       4
                *

 237            KK   SB8NE   BASIN
 238            KM        SITE JUST SOUTH OF LEGACY AND WEST OF HAYDEN - FUTURE RETAIL
 239            BA   0.015
 240            LG    0.10    0.25    4.03    0.61      80
 241            UC   0.239   0.172
 242            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 243            UA     100
                *

 244            KK   SB8NE  DIVERT
 245            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NE
 246            KM        FIRST FLUSH VOL = 0.340 AC-FT -HE
 247            DT  RET8NE   0.340
 248            DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 249            DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                *

 250            KK  R8NE8N   ROUTE
 251            KM        ROUTE NEW SUB BASIN SB8NE TO OUTLET AT CAVASSON BLVD -HE
 252            KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 253            RK     622   0.011   0.018            TRAP      46       4
                *

 254            KK   SB8SW   BASIN
 255            KM        SITE JUST NORTH OF CAVASSON BLVD EAST OF CLARET DR - FUTURE MF
 256            BA  0.0129
```

```
         257           LG    0.10    0.25    4.03    0.61      80
         258           UC   0.219   0.169
         259           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
         260           UA     100
                       *

         261           KK   SB8SW  DIVERT
         262           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SW
         263           KM        FIRST FLUSH VOL = 0.292 AC-FT -HE
         264           DT  RET8SW   0.292
         265           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
         266           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                       *

         267           KK   SB8SE   BASIN
         268           KM        SITE JUST NORTH OF CAVASSON AND WEST OF HAYDEN - FUTURE HOTEL OR RETAIL
         269           BA  0.0131
         270           LG    0.10    0.25    4.03    0.61      80
         271           UC   0.242   0.210
         272           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
         273           UA     100
                       *
1                                            HEC-1 INPUT                                            PAGE  8

        LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


         274           KK   SB8SE  DIVERT
         275           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SE
         276           KM        FIRST FLUSH VOL = 0.297 AC-FT -HE
         277           DT  RET8SE   0.297
         278           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
         279           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                       *

         280           KK  CP-08N COMBINE
         281           KM        CP AT INLET ON NORTH SIDE OF CAVASSON - ENTERS EXIST STORM DRAIN   -HE
         282           HC       4  0.0557
                       *

         283           KK  R8N-8W   ROUTE
         284           KM        ROUTE NEW SUB BASINS SB8NE SB8NW SB8SW SB8SE TO CP-08W
         285           KM        ROUTE IN STORM DRAIN     -HE
         286           RK    1592   0.015   0.013            CIRC       3
                       *

         287           KK   SB08W   BASIN
         288           BA  0.0589
         289           LG    0.10    0.25    4.03    0.61      80
         290           UC   0.225   0.150
         291           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
         292           UA     100
                       *

         293           KK   SB08W  DIVERT
         294           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB08W
         295           KM        FIRST FLUSH VOL REQ = 1.335 AC-FT, PROVIDED = 1.415
         296           KM        THIS IS EXISTING AND WAS BUILT IN PHASE 1 -HE
         297           DT   RET8W   1.415
         298           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
         299           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                       *

         300           KK  CP-08W COMBINE
         301           HC       3  0.1614
                       *

         302           KK  R8W-4N   ROUTE
         303           KM         STAGE STORAGE ROUTING UNDER THE LOOP 101 -HE
         304           RS       1    STOR    0.00       0
         305           SV   0.000   0.642   1.290   2.207   4.066   4.066
         306           SE  1608.0  1609.0  1610.0  1611.0  1612.0  1613.0
         307           SQ   0.000   8.330   28.52   150.5   300.0   360.0
         308           SE  1608.0  1609.0  1610.0  1611.0  1613.2  1613.3
                       *
1                                            HEC-1 INPUT                                            PAGE  9

        LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10
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 309           KK  SB02E1   BASIN
 310           KM         EAST SPLIT OF BASIN SBO2E DUE TO HIGH POINT -HE
 311           BA  0.0252
 312           LG    0.10    0.25    4.03    0.61      80
 313           UC   0.193   0.176
 314           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 315           UA     100
               *

 316           KK  SB02E1  DIVERT
 317           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E1
 318           KM        FIRST FLUSH VOL REQ = 0.570 AC-FT -HE
 319           DT  RET2E1   0.570
 320           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 321           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 322           KK   CP-4N COMBINE
 323           HC       2  0.1866
               *

 324           KK   R4N-4   ROUTE
 325           RK    2840  0.0130   0.030            TRAP   2.000    0.00
               *

 326           KK    SB04   BASIN
 327           BA   0.144
 328           LG    0.14    0.25    4.60    0.44      61
 329           UC   0.305   0.199
 330           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 331           UA     100
               *

 332           KK    CP-4 COMBINE
 333           HC       2
               *

 334           KK    R4-5   ROUTE
 335           KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 336           RK    1992  0.0013   0.03             TRAP      39       4
               *

 337           KK   CP-05 COMBINE
 338           KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 339           KM        COMBINE CP-05A AND R4-5
 340           HC       2
               *

 341           KK    R5-6   ROUTE
 342           KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 343           RK    1694  0.0015   0.03             TRAP     188       4
               *
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LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 344           KK    SB06   BASIN
 345           BA   0.136
 346           LG    0.16    0.25    4.55    0.45      53
 347           UC   0.321   0.246
 348           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 349           UA     100
               *

 350           KK   CP-06 COMBINE
 351           HC       2
               *

 352           KK   SB07E   BASIN
 353           BA   0.026
 354           LG    0.10    0.25    4.03    0.74       0
 355           UC   0.574   0.527
 356           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 357           UA     100
               *

 358           KK  R7E-8E   ROUTE
 359           RK    3662  0.0130   0.030            TRAP   2.000    0.00
               *
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```
 360            KK   SB08E   BASIN
 361            BA   0.078
 362            LG    0.10    0.25    4.08    0.72       0
 363            UC   0.798   0.652
 364            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 365            UA     100
                *

 366            KK   SB09N   BASIN
 367            BA   0.028
 368            LG    0.10    0.25    4.79    0.49       0
 369            UC   0.740   1.111
 370            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 371            UA     100
                *

 372            KK  R9N-R9   ROUTE
 373            RK    2586  0.0013   0.020           TRAP   2.000    0.00
                *

 374            KK  CP-08E COMBINE
 375            HC       3
                *

 376            KK  R8E-R9   ROUTE
 377            RK     501  0.0013   0.020           TRAP   2.000    0.00
                *
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LINE            ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 378            KK    SB09   BASIN
 379            BA   0.086
 380            LG    0.10    0.25    4.79    0.49       0
 381            UC   0.683   0.518
 382            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 383            UA     100
                *

 384            KK   CP-09 COMBINE
 385            HC       2
                *

 386            KK   PWRCH
 387            KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
 388            KM        100-YR, 24-HR FLO-2D MODEL (XS 107)
 389            KM        FROM PINNACLE PEAK SOUTH ADMS 100-YR, 24-HR FLO-2D MODEL
 390            BA     7.0
 391            QI       0       0       0       0       0       0       0       0       0       0
 392            QI       0       0       0       0       0       0       0       0       0       0
 393            QI       0       0       0       0       0       0       0       0       0       0
 394            QI       0       0       0       0       0       0       0       0       0       0
 395            QI       0       0       0       0       0       1       1       3      43     495
 396            QI    1099    1198    2055    3747    3658    3175    2822    2378    2084    1898
 397            QI    1699    1499    1331    1179    1047     940     851     773     705     645
 398            QI     597     559     521     488     460     435     410     387     370     356
 399            QI     339     328     317     303     293     281     270     257     258     248
 400            QI     240     237     223     220     214     209     205     201     192     188
 401            QI     180     179     164     158     149     142     137     126     120     117
 402            QI     107     102      97      99      86      83      82      74      70      66
 403            QI      62      59      57      55      52      50      50      48      42      41
 404            QI      40      37      35      40      36      32      31      30      31      30
 405            QI      31      30      24      23      23
                *

 406            KK  BINFLO
 407            KM         TOTAL INFLOW INTO BASIN 53R.
 408            HC       2
                *

 409            KK   BASIN
 410            KM    BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
 411            KM    ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
 412            KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
 413            KM        THE OUTLET PIPES ARE INLET CONTROLLED.
 414            RS       1    STOR       0
 415            SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
 416            SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
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```
         417           SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
         418           SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
         419           SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
         420           SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
                       *
1                                                 HEC-1 INPUT                                                 PAGE 12

        LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


         421           KK  BSNRT1
         422           KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
         423           KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
         424           KM        EXCESS CAPACITY.
         425           RK     550  0.0052   0.024           CIRC     7.0
                       *

         426           KK  BSNRT2
         427           KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BAS 53R OUTFALL)
         428           RK    1200  0.0077   0.013           CIRC     7.0
                       *

         429           KK    SB10   BASIN
         430           BA   0.040
         431           LG    0.15    0.25    4.25    0.55      55
         432           UC   0.233   0.161
         433           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
         434           UA     100
                       *

         435           KK   CP-10 COMBINE
         436           KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
         437           HC       2
                       *

         438           KK  R10-11   ROUTE
         439           KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
         440           RK    1277  0.0014   0.03            TRAP     220       4
                       *

         441           KK    SB11   BASIN
         442           BA   0.071
         443           LG    0.15    0.25    4.15    0.58      55
         444           UC   0.296   0.232
         445           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
         446           UA     100
                       *

         447           KK   CP-11 COMBINE
         448           KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
         449           HC       2
                       *
         450           ZZ
1
                SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

   NO.      (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

    50      76THST
                 .
                 .
    69           .       SB01A
                 .           .
                 .           .
    82           .           .------->  RET01A
    79           .       SB01A
                 .           .
                 .           .
    85        CP-1............
                 V
                 V
    88      R1A-2W
                 .
                 .
    91           .      SB02NW
                 .           .
                 .           .
   102           .           .------->  RET2NW
```

```
 99           .      SB02NW
              .           .
              .           .
107      CP-02W............
              V
              V
110       R2W-2
              .
              .
112           .       SB01C
              .           .
              .           .
121           .           .------->  RET01C
118           .       SB01C
              .           V
              .           V
124           .      R1C-1B
              .           .
              .           .
126           .           .       SB01B
              .           .           .
              .           .           .
135           .           .           .------->  RET01B
132           .           .       SB01B
              .           .           .
              .           .           .
138           .      CP-01B............
              .           V
              .           V
141           .      R1B-2N
              .           .
              .           .
143           .           .      SB02NE
              .           .           .
              .           .           .
152           .           .           .------->  RET2NE
149           .           .      SB02NE
              .           .           .
              .           .           .
155           .      CP-02N............
              .           V
              .           V
158           .       R2N-2
              .           .
              .           .
160           .           .      SB02E2
              .           .           .
              .           .           .
171           .           .           .------->  RET2E2
168           .           .      SB02E2
              .           .           .
              .           .           .
174           .           .           .       SB02W
              .           .           .           .
              .           .           .           .
180       CP-02....................................
              V
              V
182        R2-3
              .
              .
185           .        SB03
              .           .
              .           .
191       CP-03............
              V
              V
193        R3-5
              .
              .
196           .        SB05
              .           .
              .           .
202       CP-5A............
              .
              .
204           .       SB07W
              .           .
              .           .
213           .           .------->  RET07W
```

```
210           .       SB07W
              .           V
              .           V
216           .      R7W-8W
              .           .
              .           .
220           .           .       SB8NW
              .           .           .
              .           .           .
230           .           .           .------->  RET8NW
227           .           .       SB8NW
              .           .           V
              .           .           V
233           .           .      R8NW8N
              .           .           .
              .           .           .
237           .           .           .       SB8NE
              .           .           .           .
              .           .           .           .
247           .           .           .           .------->  RET8NE
244           .           .           .       SB8NE
              .           .           .           V
              .           .           .           V
250           .           .           .      R8NE8N
              .           .           .           .
              .           .           .           .
254           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
264           .           .           .           .           .------->  RET8SW
261           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
267           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
277           .           .           .           .           .           .------->  RET8SE
274           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
280           .           .      CP-08N....................................
              .           .           V
              .           .           V
283           .           .      R8N-8W
              .           .           .
              .           .           .
287           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
297           .           .           .           .------->   RET8W
293           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
300           .      CP-08W........................
              .           V
              .           V
302           .      R8W-4N
              .           .
              .           .
309           .           .      SB02E1
              .           .           .
              .           .           .
319           .           .           .------->  RET2E1
316           .           .      SB02E1
              .           .           .
              .           .           .
322           .       CP-4N............
              .           V
              .           V
324           .       R4N-4
              .           .
              .           .
326           .           .        SB04
              .           .           .
              .           .           .
332           .        CP-4............
              .           V
              .           V
334           .        R4-5
              .           .
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                .           .
  337       CP-05............
                V
                V
  341        R5-6
                .
                .
  344           .        SB06
                .           .
                .           .
  350       CP-06............
                .
                .
  352           .       SB07E
                .           V
                .           V
  358           .      R7E-8E
                .           .
                .           .
  360           .           .       SB08E
                .           .           .
                .           .           .
  366           .           .           .       SB09N
                .           .           .           V
                .           .           .           V
  372           .           .           .      R9N-R9
                .           .           .           .
                .           .           .           .
  374           .      CP-08E........................
                .           V
                .           V
  376           .      R8E-R9
                .           .
                .           .
  378           .           .        SB09
                .           .           .
                .           .           .
  384           .       CP-09............
                .           .
                .           .
  386           .           .       PWRCH
                .           .           .
                .           .           .
  406           .      BINFLO............
                .           V
                .           V
  409           .       BASIN
                .           V
                .           V
  421           .      BSNRT1
                .           V
                .           V
  426           .      BSNRT2
                .           .
                .           .
  429           .           .        SB10
                .           .           .
                .           .           .
  435           .       CP-10............
                .           V
                .           V
  438           .      R10-11
                .           .
                .           .
  441           .           .        SB11
                .           .           .
                .           .           .
  447           .       CP-11............

 (***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1*****************************************
 *                                       *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *
 *             JUN   1998                *
 *           VERSION 4.1                 *
 *                                       *
 *  RUN DATE   20JUL23  TIME  10:25:33   *
 *                                       *
 *****************************************

 ***************************************
 *                                     *
 *    U.S. ARMY CORPS OF ENGINEERS     *
 *    HYDROLOGIC ENGINEERING CENTER    *
 *          609 SECOND STREET          *
 *       DAVIS, CALIFORNIA 95616       *
 *           (916) 756-1104            *
 *                                     *
 ***************************************
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                     CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
                     POWERLINE CORRIDOR
                     RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                     INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                       100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                        PIMA ROAD FLOWS.

                     PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                       MODELERS: RK, MW

                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 08/07/18
                       MODELERS: MSW, ES
                       REVISIONS NOTED WITH HE
                        NEW PROP CONDITION MODEL,WITH POWERLINE AND BASIN 53R
                        THIS IS THE ULTIMATE CONDITION MODEL

                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
                       MODELERS: MSW
                       XKSAT ADJUSTMENT

                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
                       MODELERS: MSW, TSW
                       REVISIONS NOTED WITH HE (Hubbard Engineering)
                        NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
                        CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
                        ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
                        FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED

                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/20/23
                        MODELERS: MSW, TSW
                        REVISIONS NOTED WITH HE (HUBBARD ENGINEERING)
                        BASIN 2E WAS SPLIT AT CENTER POINT DUE TO FLOW UNDER 101


36 IO          OUTPUT CONTROL VARIABLES
                     IPRNT           5  PRINT CONTROL
                     IPLOT           0  PLOT CONTROL
                     QSCAL          0.  HYDROGRAPH PLOT SCALE

   IT          HYDROGRAPH TIME DATA
                      NMIN           3  MINUTES IN COMPUTATION INTERVAL
                     IDATE      1    0  STARTING DATE
                     ITIME        0000  STARTING TIME
                        NQ        1000  NUMBER OF HYDROGRAPH ORDINATES
                    NDDATE      3    0  ENDING DATE
                    NDTIME        0157  ENDING TIME
                    ICENT           19  CENTURY MARK

                 COMPUTATION INTERVAL    0.05 HOURS
                      TOTAL TIME BASE   49.95 HOURS

       ENGLISH UNITS
            DRAINAGE AREA         SQUARE MILES
            PRECIPITATION DEPTH   INCHES
            LENGTH, ELEVATION     FEET
            FLOW                  CUBIC FEET PER SECOND
            STORAGE VOLUME        ACRE-FEET
            SURFACE AREA          ACRES
            TEMPERATURE           DEGREES FAHRENHEIT

37 JD          INDEX STORM NO. 1
                      STRM        3.85  PRECIPITATION DEPTH
                      TRDA        0.00  TRANSPOSITION DRAINAGE AREA

38 PI            PRECIPITATION PATTERN
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
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                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                0.02      0.02      0.02      0.02      0.02      0.06      0.06      0.06      0.06      0.06
                0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

 48 JD          INDEX STORM NO. 2
                    STRM        3.66  PRECIPITATION DEPTH
                    TRDA       10.00  TRANSPOSITION DRAINAGE AREA

  0 PI            PRECIPITATION PATTERN
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                0.02      0.02      0.02      0.02      0.02      0.06      0.06      0.06      0.06      0.06
                0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
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                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

49 JD         INDEX STORM NO. 3
                   STRM        3.53  PRECIPITATION DEPTH
                   TRDA       20.00  TRANSPOSITION DRAINAGE AREA

 0 PI           PRECIPITATION PATTERN
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                 0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+	HYDROGRAPH AT	76THST	69.	12.50	23.	8.	4.	0.24		

+	HYDROGRAPH AT	SB01A	86.	12.25	11.	4.	2.	0.06
+	DIVERSION TO	RET01A	6.	11.55	2.	1.	0.	0.06
+	HYDROGRAPH AT	SB01A	86.	12.25	10.	3.	1.	0.06
+	2 COMBINED AT	CP-1	135.	12.30	33.	11.	5.	0.30
+	ROUTED TO	R1A-2W	134.	12.30	33.	11.	5.	0.30
+	HYDROGRAPH AT	SB02NW	29.	12.15	3.	1.	1.	0.02
+	DIVERSION TO	RET2NW	2.	11.45	1.	0.	0.	0.02
+	HYDROGRAPH AT	SB02NW	29.	12.15	3.	1.	0.	0.02
+	2 COMBINED AT	CP-02W	156.	12.25	36.	12.	6.	0.31
+	ROUTED TO	R2W-2	155.	12.25	36.	12.	6.	0.31
+	HYDROGRAPH AT	SB01C	32.	12.20	5.	2.	1.	0.03
+	DIVERSION TO	RET01C	3.	11.60	1.	0.	0.	0.03
+	HYDROGRAPH AT	SB01C	32.	12.20	5.	1.	1.	0.03
+	ROUTED TO	R1C-1B	31.	12.25	5.	1.	1.	0.03
+	HYDROGRAPH AT	SB01B	32.	12.20	4.	1.	1.	0.02
+	DIVERSION TO	RET01B	2.	11.50	1.	0.	0.	0.02
+	HYDROGRAPH AT	SB01B	32.	12.20	4.	1.	1.	0.02
+	2 COMBINED AT	CP-01B	63.	12.20	9.	2.	1.	0.05
+	ROUTED TO	R1B-2N	62.	12.25	9.	2.	1.	0.05
+	HYDROGRAPH AT	SB02NE	35.	12.25	6.	2.	1.	0.03
+	DIVERSION TO	RET2NE	3.	11.70	1.	0.	0.	0.03
+	HYDROGRAPH AT	SB02NE	35.	12.25	6.	2.	1.	0.03
+	2 COMBINED AT	CP-02N	96.	12.25	14.	4.	2.	0.09
+	ROUTED TO	R2N-2	95.	12.25	14.	4.	2.	0.09
+	HYDROGRAPH AT	SB02E2	33.	12.10	5.	2.	1.	0.02
+	DIVERSION TO	RET2E2	1.	9.00	1.	0.	0.	0.02
	HYDROGRAPH AT							

+		SB02E2	33.	12.10	5.	1.	1.	0.02
	HYDROGRAPH AT							
+		SB02W	34.	12.45	5.	1.	1.	0.06
	4 COMBINED AT							
+		CP-02	292.	12.30	59.	18.	9.	0.18
	ROUTED TO							
+		R2-3	291.	12.35	59.	18.	9.	0.18
	HYDROGRAPH AT							
+		SB03	65.	12.15	10.	3.	2.	0.05
	2 COMBINED AT							
+		CP-03	334.	12.35	68.	21.	10.	0.23
	ROUTED TO							
+		R3-5	332.	12.45	68.	21.	10.	0.23
	HYDROGRAPH AT							
+		SB05	190.	12.10	24.	8.	4.	0.13
	2 COMBINED AT							
+		CP-5A	404.	12.45	89.	28.	14.	0.36
	HYDROGRAPH AT							
+		SB07W	68.	12.20	8.	3.	1.	0.05
	DIVERSION TO							
+		RET07W	4.	11.50	2.	1.	0.	0.05
	HYDROGRAPH AT							
+		SB07W	68.	12.20	8.	2.	1.	0.05
	ROUTED TO							
+		R7W-8W	66.	12.60	8.	2.	1.	0.05
	HYDROGRAPH AT							
+		SB8NW	27.	12.15	4.	1.	1.	0.01
	DIVERSION TO							
+		RET8NW	1.	9.00	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8NW	27.	12.15	4.	1.	1.	0.01
	ROUTED TO							
+		R8NW8N	26.	12.15	4.	1.	1.	0.01
	HYDROGRAPH AT							
+		SB8NE	29.	12.10	4.	1.	1.	0.01
	DIVERSION TO							
+		RET8NE	1.	8.95	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8NE	29.	12.10	4.	1.	1.	0.01
	ROUTED TO							
+		R8NE8N	28.	12.15	4.	1.	1.	0.01
	HYDROGRAPH AT							
+		SB8SW	25.	12.10	3.	1.	1.	0.01
	DIVERSION TO							
+		RET8SW	1.	8.95	0.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SW	25.	12.10	3.	1.	0.	0.01
	HYDROGRAPH AT							
+		SB8SE	23.	12.15	3.	1.	1.	0.01
	DIVERSION TO							
+		RET8SE	1.	9.00	0.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SE	23.	12.15	3.	1.	0.	0.01

	4 COMBINED AT							
+		CP-08N	102.	12.15	14.	4.	2.	0.06
	ROUTED TO							
+		R8N-8W	101.	12.15	14.	4.	2.	0.06
	HYDROGRAPH AT							
+		SB08W	118.	12.10	14.	5.	2.	0.06
	DIVERSION TO							
+		RET8W	3.	9.20	2.	1.	0.	0.06
	HYDROGRAPH AT							
+		SB08W	118.	12.10	14.	4.	2.	0.06
	3 COMBINED AT							
+		CP-08W	214.	12.10	36.	11.	5.	0.16
	ROUTED TO							
+		R8W-4N	170.	12.25	35.	11.	5.	0.16
	HYDROGRAPH AT							
+		SB02E1	49.	12.10	6.	2.	1.	0.03
	DIVERSION TO							
+		RET2E1	1.	8.95	1.	0.	0.	0.03
	HYDROGRAPH AT							
+		SB02E1	49.	12.10	6.	2.	1.	0.03
	2 COMBINED AT							
+		CP-4N	209.	12.15	41.	12.	6.	0.19
	ROUTED TO							
+		R4N-4	208.	12.15	41.	12.	6.	0.19
	HYDROGRAPH AT							
+		SB04	241.	12.10	31.	10.	5.	0.14
	2 COMBINED AT							
+		CP-4	438.	12.10	71.	22.	11.	0.33
	ROUTED TO							
+		R4-5	435.	12.20	71.	22.	11.	0.33
	2 COMBINED AT							
+		CP-05	734.	12.35	159.	51.	25.	0.69
	ROUTED TO							
+		R5-6	730.	12.45	159.	51.	25.	0.69
	HYDROGRAPH AT							
+		SB06	202.	12.10	27.	9.	4.	0.14
	2 COMBINED AT							
+		CP-06	819.	12.40	185.	59.	29.	0.82
	HYDROGRAPH AT							
+		SB07E	16.	12.40	2.	1.	0.	0.03
	ROUTED TO							
+		R7E-8E	16.	12.45	2.	1.	0.	0.03
	HYDROGRAPH AT							
+		SB08E	39.	12.55	6.	2.	1.	0.08
	HYDROGRAPH AT							
+		SB09N	11.	12.55	3.	1.	0.	0.03
	ROUTED TO							
+		R9N-R9	11.	12.65	3.	1.	0.	0.03
	3 COMBINED AT							
+		CP-08E	63.	12.55	11.	3.	1.	0.13
	ROUTED TO							
+		R8E-R9	63.	12.55	11.	3.	1.	0.13
	HYDROGRAPH AT							
+		SB09	60.	12.45	9.	2.	1.	0.09

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          2 COMBINED AT
+                          CP-09       120.   12.50          20.          5.          2.       0.22

          HYDROGRAPH AT
+                          PWRCH      3747.   13.25        1540.        502.        248.       7.00

          2 COMBINED AT
+                         BINFLO      3788.   13.25        1558.        507.        250.       7.22

          ROUTED TO
+                          BASIN       462.   18.45         458.        399.        241.       7.22

          ROUTED TO
+                         BSNRT1       462.   18.50         458.        399.        241.       7.22

          ROUTED TO
+                         BSNRT2       462.   18.50         458.        399.        241.       7.22

          HYDROGRAPH AT
+                           SB10        72.   12.05           8.          3.          1.       0.04

          2 COMBINED AT
+                          CP-10       463.   18.50         459.        400.        242.       7.26

          ROUTED TO
+                         R10-11       463.   18.55         459.        400.        241.       7.26

          HYDROGRAPH AT
+                           SB11       107.   12.10          14.          5.          2.       0.07

          2 COMBINED AT
+                          CP-11       465.   18.55         460.        401.        244.       7.33
1
                                              SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
                                                  (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)
                                                                                     INTERPOLATED TO
                                                                                  COMPUTATION INTERVAL
            ISTAQ    ELEMENT      DT       PEAK    TIME TO     VOLUME      DT       PEAK    TIME TO     VOLUME
                                                     PEAK                                     PEAK

                                 (MIN)     (CFS)      (MIN)     (IN)     (MIN)     (CFS)     (MIN)       (IN)

           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R1A-2W  MANE         0.27    138.22    738.13      1.40      3.00    138.18    738.00       1.40


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2249E+02 EXCESS=0.0000E+00 OUTFLOW=0.2249E+02 BASIN STORAGE=0.2963E-02 PERCENT ERROR=   0.0


           FOR STORM = 2  STORM AREA (SQ MI) =      10.00
             R1A-2W  MANE         0.23    132.78    738.11      1.37      3.00    132.73    738.00       1.37


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2206E+02 EXCESS=0.0000E+00 OUTFLOW=0.2206E+02 BASIN STORAGE=0.2963E-02 PERCENT ERROR=   0.0


           FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             R1A-2W  MANE         0.23    129.25    738.12      1.36      3.00    129.19    738.00       1.36


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2178E+02 EXCESS=0.0000E+00 OUTFLOW=0.2178E+02 BASIN STORAGE=0.2963E-02 PERCENT ERROR=   0.0


           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R2W-2  MANE         0.20    160.62    735.42      1.50      3.00    159.58    735.00       1.50


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2435E+02 EXCESS=0.0000E+00 OUTFLOW=0.2435E+02 BASIN STORAGE=0.4089E-02 PERCENT ERROR=   0.0


           FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R2W-2  MANE         0.28    153.29    735.70      1.46      3.00    152.40    738.00       1.46


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2379E+02 EXCESS=0.0000E+00 OUTFLOW=0.2378E+02 BASIN STORAGE=0.4089E-02 PERCENT ERROR=   0.0


           FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R2W-2  MANE         0.22    148.68    735.55      1.44      3.00    147.99    738.00       1.44
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2342E+02 EXCESS=0.0000E+00 OUTFLOW=0.2342E+02 BASIN STORAGE=0.4090E-02 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1C-1B  MANE         0.43     32.64    732.90      1.80      3.00     32.39    735.00        1.81


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2599E+01 EXCESS=0.0000E+00 OUTFLOW=0.2599E+01 BASIN STORAGE=0.9756E-13 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R1C-1B  MANE         0.39     30.53    732.80      1.67      3.00     30.31    735.00        1.67


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2408E+01 EXCESS=0.0000E+00 OUTFLOW=0.2408E+01 BASIN STORAGE=0.9547E-13 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R1C-1B  MANE         0.44     29.15    732.86      1.59      3.00     28.96    735.00        1.59


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2284E+01 EXCESS=0.0000E+00 OUTFLOW=0.2285E+01 BASIN STORAGE=0.9767E-13 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1B-2N  MANE         1.55     64.38    735.51      1.81      3.00     64.19    735.00        1.81


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4913E+01 EXCESS=0.0000E+00 OUTFLOW=0.4913E+01 BASIN STORAGE=0.5632E-04 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R1B-2N  MANE         1.51     60.32    734.68      1.67      3.00     60.22    735.00        1.67


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4553E+01 EXCESS=0.0000E+00 OUTFLOW=0.4552E+01 BASIN STORAGE=0.5975E-04 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R1B-2N  MANE         1.46     57.82    735.22      1.59      3.00     57.66    735.00        1.59


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4319E+01 EXCESS=0.0000E+00 OUTFLOW=0.4320E+01 BASIN STORAGE=0.5659E-04 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2N-2  MANE         0.26     99.78    735.68      1.80      3.00     99.10    735.00        1.80


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8138E+01 EXCESS=0.0000E+00 OUTFLOW=0.8139E+01 BASIN STORAGE=0.1854E-04 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
             R2N-2  MANE         0.41     93.34    735.52      1.66      3.00     92.74    735.00        1.66


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7542E+01 EXCESS=0.0000E+00 OUTFLOW=0.7543E+01 BASIN STORAGE=0.1946E-04 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             R2N-2  MANE         0.30     89.45    735.69      1.58      3.00     88.67    735.00        1.58


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7158E+01 EXCESS=0.0000E+00 OUTFLOW=0.7159E+01 BASIN STORAGE=0.1834E-04 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R2-3  MANE         2.00    302.20    740.66      3.91      3.00    301.87    741.00        3.91


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3789E+02 EXCESS=0.0000E+00 OUTFLOW=0.3783E+02 BASIN STORAGE=0.3881E-01 PERCENT ERROR=   0.1


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R2-3  MANE         2.15    284.82    740.61      3.74      3.00    284.60    741.00        3.74


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3629E+02 EXCESS=0.0000E+00 OUTFLOW=0.3619E+02 BASIN STORAGE=0.3635E-01 PERCENT ERROR=   0.2
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          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R2-3  MANE         2.18    273.91    740.89      3.63      3.00    273.83    741.00        3.63

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3526E+02 EXCESS=0.0000E+00 OUTFLOW=0.3517E+02 BASIN STORAGE=0.3635E-01 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R3-5  MANE         2.78    347.19    747.80      3.61      3.00    345.18    747.00        3.61

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4436E+02 EXCESS=0.0000E+00 OUTFLOW=0.4421E+02 BASIN STORAGE=0.8213E-01 PERCENT ERROR=   0.2

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R3-5  MANE         2.83    327.37    749.00      3.45      3.00    325.41    747.00        3.45

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4237E+02 EXCESS=0.0000E+00 OUTFLOW=0.4224E+02 BASIN STORAGE=0.8213E-01 PERCENT ERROR=   0.1

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R3-5  MANE         2.98    313.61    748.50      3.35      3.00    311.40    750.00        3.34

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4111E+02 EXCESS=0.0000E+00 OUTFLOW=0.4096E+02 BASIN STORAGE=0.8213E-01 PERCENT ERROR=   0.2

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7W-8W  MANE         3.00     68.93    754.57      1.91      3.00     68.50    756.00        1.91

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4511E+01 EXCESS=0.0000E+00 OUTFLOW=0.4779E+01 BASIN STORAGE=0.9374E-03 PERCENT ERROR=  -6.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R7W-8W  MANE         3.00     64.32    754.67      1.83      3.00     64.21    756.00        1.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4179E+01 EXCESS=0.0000E+00 OUTFLOW=0.4572E+01 BASIN STORAGE=0.8368E-03 PERCENT ERROR=  -9.4

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R7W-8W  MANE         3.00     63.27    756.75      1.63      3.00     59.66    759.00        1.63

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3965E+01 EXCESS=0.0000E+00 OUTFLOW=0.4073E+01 BASIN STORAGE=0.8362E-03 PERCENT ERROR=  -2.8

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NW8N  MANE         0.83     27.24    730.38      2.83      3.00     27.08    729.00        2.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2215E+01 EXCESS=0.0000E+00 OUTFLOW=0.2217E+01 BASIN STORAGE=0.3350E-06 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R8NW8N  MANE         0.90     25.74    730.14      2.66      3.00     25.61    729.00        2.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2082E+01 EXCESS=0.0000E+00 OUTFLOW=0.2084E+01 BASIN STORAGE=0.2981E-06 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R8NW8N  MANE         0.87     24.80    730.23      2.55      3.00     24.66    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1996E+01 EXCESS=0.0000E+00 OUTFLOW=0.1998E+01 BASIN STORAGE=0.2840E-06 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NE8N  MANE         0.80     29.24    728.00      2.83      3.00     29.14    729.00        2.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2259E+01 EXCESS=0.0000E+00 OUTFLOW=0.2263E+01 BASIN STORAGE=0.3312E-06 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
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            R8NE8N  MANE         0.87     27.62    728.44      2.66      3.00     27.56    729.00        2.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2124E+01 EXCESS=0.0000E+00 OUTFLOW=0.2127E+01 BASIN STORAGE=0.3201E-06 PERCENT ERROR=  -0.2

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R8NE8N  MANE         0.87     26.65    727.97      2.55      3.00     26.55    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2036E+01 EXCESS=0.0000E+00 OUTFLOW=0.2038E+01 BASIN STORAGE=0.3381E-06 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8N-8W  MANE         0.63    104.30    729.26      2.83      3.00    104.19    729.00        2.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8393E+01 EXCESS=0.0000E+00 OUTFLOW=0.8394E+01 BASIN STORAGE=0.4795E-07 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R8N-8W  MANE         0.63     98.64    729.78      2.66      3.00     98.54    729.00        2.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7889E+01 EXCESS=0.0000E+00 OUTFLOW=0.7890E+01 BASIN STORAGE=0.4709E-07 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R8N-8W  MANE         0.51     95.24    729.83      2.55      3.00     94.91    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7563E+01 EXCESS=0.0000E+00 OUTFLOW=0.7564E+01 BASIN STORAGE=0.4620E-07 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R4N-4  MANE         0.62    213.12    730.47      2.59      3.00    211.87    729.00        2.59

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2575E+02 EXCESS=0.0000E+00 OUTFLOW=0.2576E+02 BASIN STORAGE=0.3243E-03 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
             R4N-4  MANE         0.55    206.27    730.30      2.44      3.00    205.23    729.00        2.44

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2428E+02 EXCESS=0.0000E+00 OUTFLOW=0.2429E+02 BASIN STORAGE=0.3185E-03 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             R4N-4  MANE         0.61    201.64    730.38      2.31      3.00    200.92    729.00        2.31

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2296E+02 EXCESS=0.0000E+00 OUTFLOW=0.2297E+02 BASIN STORAGE=0.3174E-03 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R4-5  MANE         2.44    450.25    732.20      2.65      3.00    449.62    732.00        2.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4676E+02 EXCESS=0.0000E+00 OUTFLOW=0.4671E+02 BASIN STORAGE=0.1241E-02 PERCENT ERROR=   0.1

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R4-5  MANE         2.46    430.99    731.20      2.51      3.00    429.42    732.00        2.51

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4415E+02 EXCESS=0.0000E+00 OUTFLOW=0.4417E+02 BASIN STORAGE=0.1156E-02 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R4-5  MANE         2.59    413.08    733.30      2.39      3.00    412.54    732.00        2.39

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4210E+02 EXCESS=0.0000E+00 OUTFLOW=0.4212E+02 BASIN STORAGE=0.1072E-02 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R5-6  MANE         2.40    767.39    744.77      2.92      3.00    764.40    744.00        2.92
```

```
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1068E+03 EXCESS=0.0000E+00 OUTFLOW=0.1067E+03 BASIN STORAGE=0.6946E-01 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R5-6  MANE         2.50    720.70    746.65      2.77      3.00    719.61    747.00        2.77


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1014E+03 EXCESS=0.0000E+00 OUTFLOW=0.1013E+03 BASIN STORAGE=0.6948E-01 PERCENT ERROR=   0.1


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R5-6  MANE         2.49    687.18    747.03      2.66      3.00    686.98    747.00        2.66


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9744E+02 EXCESS=0.0000E+00 OUTFLOW=0.9736E+02 BASIN STORAGE=0.6948E-01 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7E-8E  MANE         1.93     16.43    748.32      0.80      3.00     16.39    747.00        0.80


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1113E+01 EXCESS=0.0000E+00 OUTFLOW=0.1112E+01 BASIN STORAGE=0.1951E-04 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R7E-8E  MANE         1.94     14.90    748.28      0.72      3.00     14.82    747.00        0.72


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9964E+00 EXCESS=0.0000E+00 OUTFLOW=0.9968E+00 BASIN STORAGE=0.1628E-04 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R7E-8E  MANE         2.13     13.91    748.07      0.67      3.00     13.81    747.00        0.67


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9248E+00 EXCESS=0.0000E+00 OUTFLOW=0.9254E+00 BASIN STORAGE=0.1626E-04 PERCENT ERROR=  -0.1


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R9N-R9  MANE         2.43     11.87    758.35      0.97      3.00     11.83    759.00        0.97


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1453E+01 EXCESS=0.0000E+00 OUTFLOW=0.1451E+01 BASIN STORAGE=0.2431E-04 PERCENT ERROR=   0.1


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R9N-R9  MANE         2.57     10.88    757.62      0.89      3.00     10.84    759.00        0.89


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1330E+01 EXCESS=0.0000E+00 OUTFLOW=0.1328E+01 BASIN STORAGE=0.2146E-04 PERCENT ERROR=   0.1


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R9N-R9  MANE         2.61     10.25    758.60      0.84      3.00     10.23    759.00        0.84


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1249E+01 EXCESS=0.0000E+00 OUTFLOW=0.1248E+01 BASIN STORAGE=0.2338E-04 PERCENT ERROR=   0.1


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8E-R9  MANE         0.31     67.27    753.42      0.85      3.00     67.12    753.00        0.85


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5955E+01 EXCESS=0.0000E+00 OUTFLOW=0.5955E+01 BASIN STORAGE=0.1051E-04 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R8E-R9  MANE         0.38     60.94    753.41      0.76      3.00     60.79    753.00        0.76


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5368E+01 EXCESS=0.0000E+00 OUTFLOW=0.5368E+01 BASIN STORAGE=0.1067E-04 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R8E-R9  MANE         0.36     56.87    753.51      0.71      3.00     56.68    753.00        0.71


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4995E+01 EXCESS=0.0000E+00 OUTFLOW=0.4995E+01 BASIN STORAGE=0.1021E-04 PERCENT ERROR=   0.0
```

```
          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT1  MANE         0.38    462.76   1109.87      2.58      3.00    462.76   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9944E+03 EXCESS=0.0000E+00 OUTFLOW=0.9943E+03 BASIN STORAGE=0.9346E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            BSNRT1  MANE         0.38    462.49   1110.02      2.58      3.00    462.49   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9936E+03 EXCESS=0.0000E+00 OUTFLOW=0.9935E+03 BASIN STORAGE=0.9328E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            BSNRT1  MANE         0.38    462.32   1110.01      2.58      3.00    462.32   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9930E+03 EXCESS=0.0000E+00 OUTFLOW=0.9929E+03 BASIN STORAGE=0.9316E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE         0.38    462.76   1110.48      2.58      3.00    462.76   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9943E+03 EXCESS=0.0000E+00 OUTFLOW=0.9941E+03 BASIN STORAGE=0.1069E+00 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            BSNRT2  MANE         0.38    462.49   1110.30      2.58      3.00    462.49   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9935E+03 EXCESS=0.0000E+00 OUTFLOW=0.9933E+03 BASIN STORAGE=0.1067E+00 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            BSNRT2  MANE         0.38    462.32   1110.49      2.58      3.00    462.32   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9929E+03 EXCESS=0.0000E+00 OUTFLOW=0.9927E+03 BASIN STORAGE=0.1065E+00 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R10-11  MANE         2.34    463.63   1114.77      2.58      3.00    463.63   1113.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9994E+03 EXCESS=0.0000E+00 OUTFLOW=0.9982E+03 BASIN STORAGE=0.1212E+01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R10-11  MANE         2.39    463.32   1113.19      2.58      3.00    463.32   1113.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9983E+03 EXCESS=0.0000E+00 OUTFLOW=0.9972E+03 BASIN STORAGE=0.1068E+01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R10-11  MANE         2.35    463.12   1115.43      2.57      3.00    463.12   1113.00        2.57

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9977E+03 EXCESS=0.0000E+00 OUTFLOW=0.9964E+03 BASIN STORAGE=0.1562E+01 PERCENT ERROR=   0.0


 *** NORMAL END OF HEC-1 ***
```

Appendix E
Hydrology Calculations
Toll at Cavasson

HYDROLOGIC CALCULATION SUMMARY SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Project Name: Toll at Cavasson **Prepared by:** TSW **Date:** 06/22/24
Project No.: 18114-750 **Revised by:** TSW **Date:** 10/02/24

Complete calculations for each concentration point are presented in the attached hydrologic calculation sheets.

Sub-Basin & Concentration Point ID	C Runoff Coefficient Frequency			I Intensity Frequency [in/hr]			A Area [acres]	Q Peak Discharge Frequency [cfs]		
	10-year	50-year	100-year	10-year	50-year	100-year		10-year	50-year	100-year
DA-A	0.80	0.85	0.90	3.66	5.13	5.78	10.96	32.09	47.79	57.02
DA-OFF1	0.40	0.48	0.50	3.66	5.13	5.78	3.24	4.74	7.97	9.36
DA-OFF-2	0.40	0.48	0.50	3.66	5.13	5.78	2.50	3.66	6.15	7.22
DA-OFF-3	0.40	0.48	0.50	3.66	5.13	5.78	2.44	3.57	6.01	7.05
DA-OFF-4	0.40	0.48	0.50	3.66	5.13	5.78	2.95	4.32	7.26	8.52
DA-OFF-5	0.40	0.48	0.50	3.66	5.13	5.78	2.45	3.58	6.02	7.07
SA-A1	0.85	0.90	0.90	3.66	5.13	5.78	0.13	0.39	0.59	0.66
SA-A2	0.85	0.90	0.90	3.66	5.13	5.78	0.21	0.66	0.98	1.10
SA-A3	0.85	0.90	0.90	3.66	5.13	5.78	1.17	3.64	5.41	6.09
SA-A4	0.85	0.90	0.90	3.66	5.13	5.78	4.36	13.57	20.13	22.68
SA-A5	0.85	0.90	0.90	3.66	5.13	5.78	1.22	3.78	5.61	6.32
SA-A6	0.85	0.90	0.90	3.66	5.13	5.78	3.87	12.03	17.85	20.11
SA-A7	0.85	0.90	0.90	3.66	5.13	5.78	3.71	11.55	17.14	19.31
SA-A8	0.85	0.90	0.90	3.66	5.13	5.78	1.90	5.91	8.77	9.89
SA-A9	0.85	0.90	0.90	3.66	5.13	5.78	1.27	3.96	5.88	6.62
SA-A10	0.85	0.90	0.90	3.66	5.13	5.78	0.64	1.99	2.95	3.33
SA-A11	0.85	0.90	0.90	3.66	5.13	5.78	0.04	0.12	0.18	0.21
SA-A12	0.85	0.90	0.90	3.66	5.13	5.78	0.04	0.11	0.17	0.19

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: DA-A

Project Name: Toll at Cavasson **Prepared by:** TSW **Date:** 06/22/24
Project No.: 18114-750 **Revised by:** TSW **Date:** 10/02/24

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	550.37	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1634.80	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1329.68	[ft]
Drainage Area (A)=	10.96	[acres]	Elevation $_{Difference}$:	305.12	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.55439	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[1]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.033501	10.0	3.66
50	-0.00625	0.04	0.033501	10.0	5.1
100	-0.00625	0.04	0.033501	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}[1] = 0.80
C_{50}[1] = 0.85
C_{100}[1] = 0.90

Q_{10} = 32.09 **[cfs]**
Q_{50} = 47.79 **[cfs]**
Q_{100} = 57.02 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: DA-OFF1

Project Name: Toll at Cavasson **Prepared by:** TSW **Date:** 06/22/24
Project No.: 18114-750 **Revised by:** TSW **Date:** 10/02/24

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	700.04	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1640.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1628.00	[ft]
Drainage Area (A)=	3.24	[acres]	Elevation $_{Difference}$:	12.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01714	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group: --
Vegetation Cover: <25 [%]
Classification Type: B (Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.01375
i	[in/hr]	b=	0.08
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.01375	0.08	0.072983	10.0	3.66
50	-0.01375	0.08	0.072983	10.0	5.1
100	-0.01375	0.08	0.072983	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

$C_{10}^{(1)}$ = 0.40
$C_{50}^{(1)}$ = 0.48
$C_{100}^{(1)}$ = 0.50

Q_{10} = 4.74 **[cfs]**
Q_{50} = 7.97 **[cfs]**
Q_{100} = 9.36 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: DA-OFF-2

Project Name: Toll at Cavasson
Project No.: 18114-750
Prepared by: TSW
Revised by: TSW
Date: 06/22/24
Date: 10/02/24

Location Data

State: Arizona
County: Maricopa
Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	557.46	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1641.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1631.00	[ft]
Drainage Area (A)=	2.50	[acres]	Elevation $_{Difference}$:	10.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01794	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	B	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.01375
i	[in/hr]	b=	0.08
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.01375	0.08	0.074532	10.0	3.66
50	-0.01375	0.08	0.074532	10.0	5.1
100	-0.01375	0.08	0.074532	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.40
C_{50}[1] = 0.48
C_{100}[1] = 0.50

Q_{10} = 3.66 **[cfs]**
Q_{50} = 6.15 **[cfs]**
Q_{100} = 7.22 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: DA-OFF-3

Project Name: Toll at Cavasson — **Prepared by:** TSW — **Date:** 06/22/24
Project No.: 18114-750 — **Revised by:** TSW — **Date:** 10/02/24

Location Data

State: Arizona — County: Maricopa — Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	552.20	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1642.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.00	[ft]
Drainage Area (A)=	2.44	[acres]	Elevation $_{Difference}$:	10.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01811	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	B	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.01375	0.08	0.074674	10.0	3.66
50	-0.01375	0.08	0.074674	10.0	5.1
100	-0.01375	0.08	0.074674	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.40
C_{50}[1] = 0.48
C_{100}[1] = 0.50

Q_{10} = 3.57 **[cfs]**
Q_{50} = 6.01 **[cfs]**
Q_{100} = 7.05 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: DA-OFF-4

Project Name: Toll at Cavasson | **Prepared by:** TSW | **Date:** 06/22/24
Project No.: 18114-750 | **Revised by:** TSW | **Date:** 10/02/24

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	658.23	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1645.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1635.00	[ft]
Drainage Area (A)=	2.95	[acres]	Elevation $_{Difference}$:	10.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01519	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	B	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.01375	0.08	0.073544	10.0	3.66
50	-0.01375	0.08	0.073544	10.0	5.1
100	-0.01375	0.08	0.073544	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}[1] = 0.40
C_{50}[1] = 0.48
C_{100}[1] = 0.50

Q_{10} = 4.32 **[cfs]**
Q_{50} = 7.26 **[cfs]**
Q_{100} = 8.52 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: DA-OFF-5

Project Name: Toll at Cavasson
Project No.: 18114-750
Prepared by: TSW
Revised by: TSW
Date: 06/22/24
Date: 10/02/24

Location Data

State: Arizona
County: Maricopa
Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]
Check Frequency:	50	[yrs]
Check Frequency:	10	[yrs]
Drainage Area (A)=	2.45	[acres]

Flow Path Length (L)=	645.08	[ft]
Elevation $_{Max}$:	1647.00	[ft]
Elevation $_{Min}$:	1636.00	[ft]
Elevation $_{Difference}$:	11.00	[ft]
Flow Path Slope$_{Average}$ (S)=	0.01705	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	B	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]
S	[ft/mi]
i	[in/hr]

Kb=m Log A + b
m= -0.01375
b= 0.08
A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.01375	0.08	0.074661	10.0	3.66
50	-0.01375	0.08	0.074661	10.0	5.1
100	-0.01375	0.08	0.074661	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)} =$ 0.40
$C_{50}^{(1)} =$ 0.48
$C_{100}^{(1)} =$ 0.50

Q_{10} = 3.58 **[cfs]**
Q_{50} = 6.02 **[cfs]**
Q_{100} = 7.07 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A1

Project Name:	Toll at Cavasson	**Prepared by:**	TSW	**Date:**	06/22/24
Project No.:	18114-750	**Revised by:**	TSW	**Date:**	10/02/24

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	79.20	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1628.15	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1627.15	[ft]
Drainage Area (A)=	0.13	[acres]	Elevation $_{Difference}$:	1.00	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01263	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045605	10.0	3.66
50	-0.00625	0.04	0.045605	10.0	5.1
100	-0.00625	0.04	0.045605	10.0	5.78

Peak Discharge (Q):
Q = C*i*A   [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 0.39 **[cfs]**
Q_{50} = 0.59 **[cfs]**
Q_{100} = 0.66 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A2

Project Name: Toll at Cavasson | **Prepared by:** TSW | **Date:** 06/22/24
Project No.: 18114-750 | **Revised by:** TSW | **Date:** 10/02/24

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	441.15	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1632.93	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1627.05	[ft]
Drainage Area (A)=	0.21	[acres]	Elevation $_{Difference}$:	5.88	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01333	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[1]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044209	10.0	3.66
50	-0.00625	0.04	0.044209	10.0	5.1
100	-0.00625	0.04	0.044209	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.85
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 0.66 **[cfs]**
Q_{50} = 0.98 **[cfs]**
Q_{100} = 1.10 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A3

Project Name:	Toll at Cavasson	**Prepared by:**	TSW	**Date:**	06/22/24
Project No.:	18114-750	**Revised by:**	TSW	**Date:**	10/02/24

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	592.47	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.50	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1627.25	[ft]
Drainage Area (A)=	1.17	[acres]	Elevation $_{Difference}$:	6.25	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01055	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.039572	10.0	3.66
50	-0.00625	0.04	0.039572	10.0	5.1
100	-0.00625	0.04	0.039572	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 3.64 **[cfs]**
Q_{50} = 5.41 **[cfs]**
Q_{100} = 6.09 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A4

Project Name: Toll at Cavasson　**Prepared by:** TSW　**Date:** 06/22/24
Project No.: 18114-750　**Revised by:** TSW　**Date:** 10/02/24

Location Data

State: Arizona　County: Maricopa　Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	789.60	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.50	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1624.65	[ft]
Drainage Area (A)=	4.36	[acres]	Elevation $_{Difference}$:	8.85	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01121	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group: --
Vegetation Cover: <25 [%]
Classification Type: A (Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)(2):
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.01375
i	[in/hr]	b=	0.08
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.036003	10.0	3.66
50	-0.00625	0.04	0.036003	10.0	5.1
100	-0.00625	0.04	0.036003	10.0	5.78

Peak Discharge (Q):
Q = C*i*A　[cfs]

A　[acres]
i　[in/hr]
C_2　Runoff Coefficient

$C_{10}{}^{(1)}$ = 0.85
$C_{50}{}^{(1)}$ = 0.90
$C_{100}{}^{(1)}$ = 0.90

Q_{10} = 13.57 **[cfs]**
Q_{50} = 20.13 **[cfs]**
Q_{100} = 22.68 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A5

Project Name:	Toll at Cavasson	**Prepared by:**	TSW	**Date:**	06/22/24
Project No.:	18114-750	**Revised by:**	TSW	**Date:**	10/02/24

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	437.29	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.50	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1628.95	[ft]
Drainage Area (A)=	1.22	[acres]	Elevation $_{Difference}$:	4.55	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01041	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.039471	10.0	3.66
50	-0.00625	0.04	0.039471	10.0	5.1
100	-0.00625	0.04	0.039471	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 3.78 **[cfs]**
Q_{50} = 5.61 **[cfs]**
Q_{100} = 6.32 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A6

Project Name:	Toll at Cavasson	**Prepared by:**	TSW	**Date:**	06/22/24
Project No.:	18114-750	**Revised by:**	TSW	**Date:**	10/02/24

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	792.29	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1635.80	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1629.68	[ft]
Drainage Area (A)=	3.87	[acres]	Elevation $_{Difference}$:	6.12	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00772	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.03633	10.0	3.66
50	-0.00625	0.04	0.03633	10.0	5.1
100	-0.00625	0.04	0.03633	10.0	5.78

Peak Discharge (Q):
Q = C*i*A   [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 12.03 **[cfs]**
Q_{50} = 17.85 **[cfs]**
Q_{100} = 20.11 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A7

Project Name:	Toll at Cavasson	**Prepared by:**	TSW	**Date:**	06/22/24
Project No.:	18114-750	**Revised by:**	TSW	**Date:**	10/02/24

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	713.88	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1635.80	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1630.35	[ft]
Drainage Area (A)=	3.71	[acres]	Elevation $_{Difference}$:	5.45	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00763	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.036439	10.0	3.66
50	-0.00625	0.04	0.036439	10.0	5.1
100	-0.00625	0.04	0.036439	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 11.55 **[cfs]**
Q_{50} = 17.14 **[cfs]**
Q_{100} = 19.31 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A8

Project Name: Toll at Cavasson **Prepared by:** TSW **Date:** 06/22/24
Project No.: 18114-750 **Revised by:** TSW **Date:** 10/02/24

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	485.24	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1635.80	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1631.04	[ft]
Drainage Area (A)=	1.90	[acres]	Elevation $_{Difference}$:	4.76	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00981	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group: --
Vegetation Cover: <25 [%]
Classification Type: A (Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.01375
i	[in/hr]	b=	0.08
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.038257	10.0	3.66
50	-0.00625	0.04	0.038257	10.0	5.1
100	-0.00625	0.04	0.038257	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}[1] = 0.85
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 5.91 **[cfs]**
Q_{50} = 8.77 **[cfs]**
Q_{100} = 9.89 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A9

Project Name: Toll at Cavasson | **Prepared by:** TSW | **Date:** 06/22/24
Project No.: 18114-750 | **Revised by:** TSW | **Date:** 10/02/24

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	552.77	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1635.80	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1631.45	[ft]
Drainage Area (A)=	1.27	[acres]	Elevation $_{Difference}$:	4.35	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00787	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[1]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.01375
i	[in/hr]	b=	0.08
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.039344	10.0	3.66
50	-0.00625	0.04	0.039344	10.0	5.1
100	-0.00625	0.04	0.039344	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.85
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 3.96 **[cfs]**
Q_{50} = 5.88 **[cfs]**
Q_{100} = 6.62 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A10

Project Name:	Toll at Cavasson	**Prepared by:**	TSW	**Date:**	06/22/24
Project No.:	18114-750	**Revised by:**	TSW	**Date:**	10/02/24

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	271.18	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1635.80	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.90	[ft]
Drainage Area (A)=	0.64	[acres]	Elevation $_{Difference}$:	2.90	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01069	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.041214	10.0	3.66
50	-0.00625	0.04	0.041214	10.0	5.1
100	-0.00625	0.04	0.041214	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 1.99 **[cfs]**
Q_{50} = 2.95 **[cfs]**
Q_{100} = 3.33 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A11

Project Name:	Toll at Cavasson	**Prepared by:**	TSW	**Date:**	06/22/24
Project No.:	18114-750	**Revised by:**	TSW	**Date:**	10/02/24

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	26.02	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1635.55	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1635.25	[ft]
Drainage Area (A)=	0.04	[acres]	Elevation $_{Difference}$:	0.30	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01153	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.048767	10.0	3.66
50	-0.00625	0.04	0.048767	10.0	5.1
100	-0.00625	0.04	0.048767	10.0	5.78

Peak Discharge (Q):
Q = C*i*A   [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 0.12 **[cfs]**
Q_{50} = 0.18 **[cfs]**
Q_{100} = 0.21 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-750

Concentration Point ID: SA-A12

Project Name:	Toll at Cavasson	**Prepared by:**	TSW	**Date:**	06/22/24
Project No.:	18114-750	**Revised by:**	TSW	**Date:**	10/02/24

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	34.18	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1635.40	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1634.80	[ft]
Drainage Area (A)=	0.04	[acres]	Elevation $_{Difference}$:	0.60	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01756	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.01375
i	[in/hr]	b= 0.08
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.049006	10.0	3.66
50	-0.00625	0.04	0.049006	10.0	5.1
100	-0.00625	0.04	0.049006	10.0	5.78

Peak Discharge (Q):
Q = C*i*A   [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}{}^{(1)} =$ 0.85
$C_{50}{}^{(1)} =$ 0.90
$C_{100}{}^{(1)} =$ 0.90

Q_{10} = 0.11 **[cfs]**
Q_{50} = 0.17 **[cfs]**
Q_{100} = 0.19 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

Appendix F

Hydraulic Calculations

Toll at Cavasson

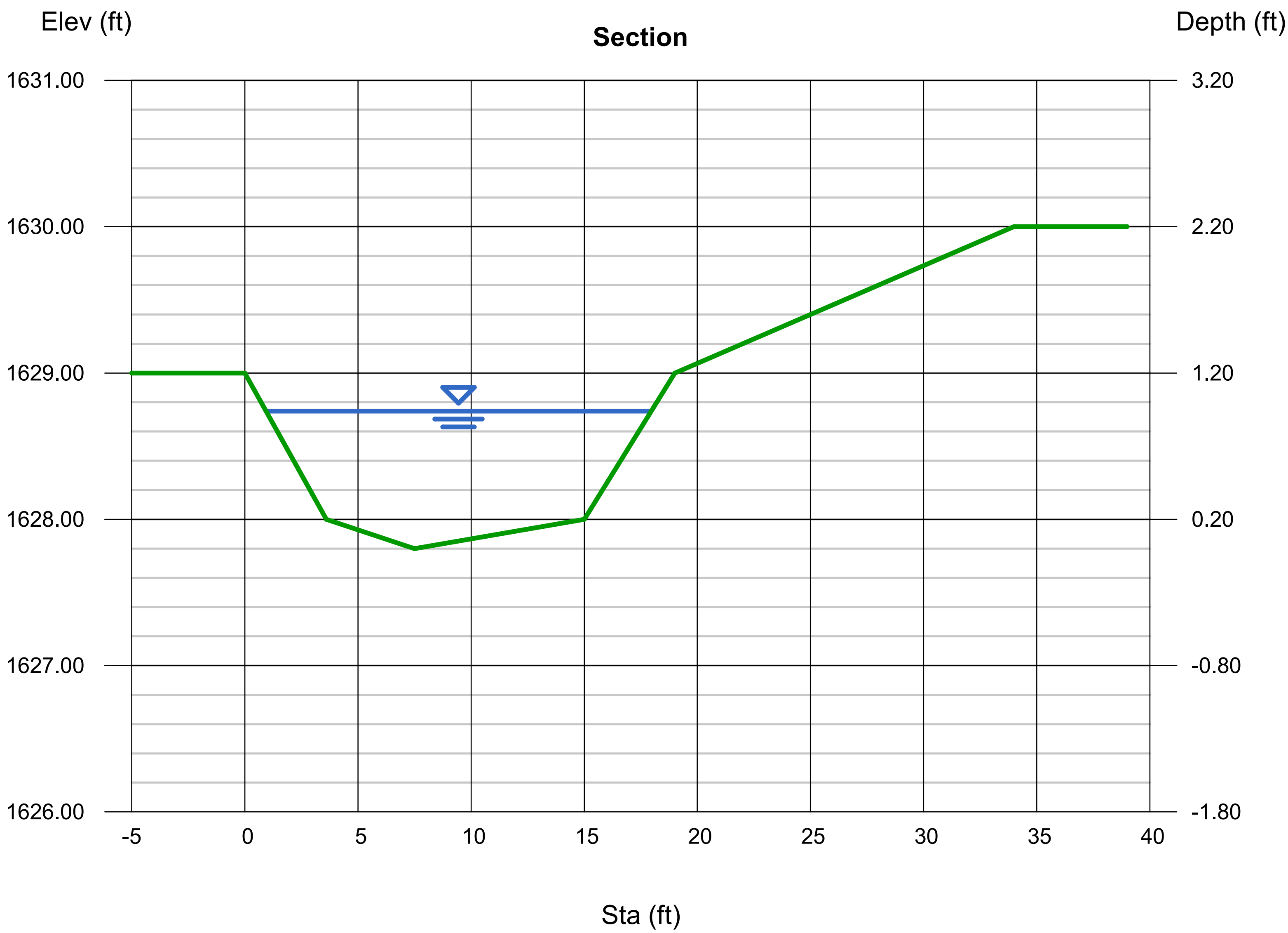
Channel Report

Offsite Runoff Rerout Swale

User-defined		**Highlighted**	
Invert Elev (ft)	= 1627.80	Depth (ft)	= 0.94
Slope (%)	= 0.79	Q (cfs)	= 39.22
N-Value	= 0.030	Area (sqft)	= 11.66
		Velocity (ft/s)	= 3.36
Calculations		Wetted Perim (ft)	= 17.22
Compute by:	Known Q	Crit Depth, Yc (ft)	= 0.79
Known Q (cfs)	= 39.22	Top Width (ft)	= 17.02
		EGL (ft)	= 1.12

(Sta, El, n)-(Sta, El, n)...

(0.00, 1629.00)-(3.60, 1628.00, 0.030)-(7.50, 1627.80, 0.030)-(15.00, 1628.00, 0.030)-(19.00, 1629.00, 0.030)-(34.00, 1630.00, 0.030)



HYDRAULIC CALCULATION SHEET
Flow-by Inlet Summary
Hubbard Engineering
Project No. 18114-750

Project Name: Toll at Cavasson
Project No.: 18114-750

Prepared by: TSW
Revised By:

Date: 10/02/24
Date:

Purpose: Summarize Flow-By Inlet Capture and Bypass Q100's

Methodology: Summarize the catch basin inlet capture and bypass calculations for the onsite flow-by configuration catch basins. Capacity Calculations are based on HEC-22 methodology.

Criteria:

References:

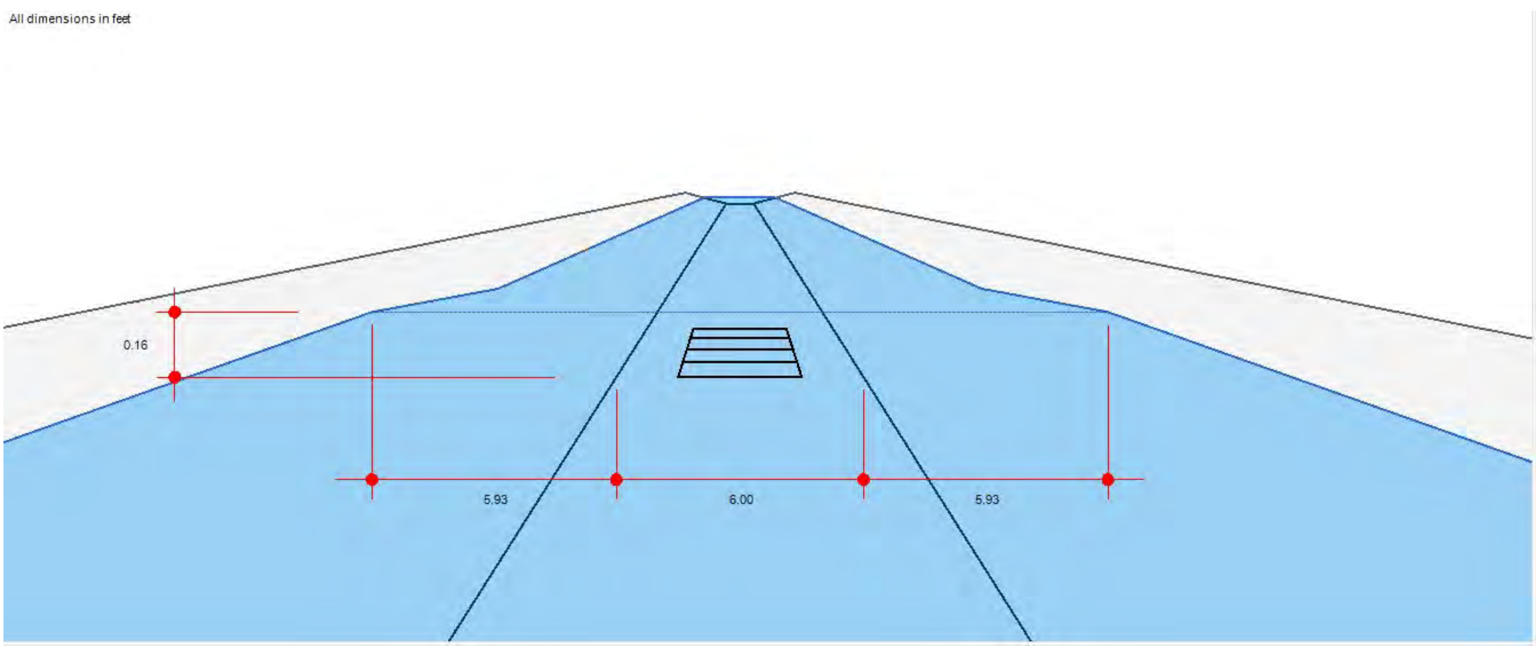
Results:

Identifiers		Catch Basin Parameters		Results		Total
Contributory Area ID	Inlet ID	Catch Basin Inlet Type	Open Area [ft^2]	Q100 Captured [cfs]	Q100 Bypass [cfs]	Q100 [cfs]
SA-A7	CB-13	2'x3' Steel Bar	3.32	3.07	11.12	19.31
SA-A8	CB-12	2'x3' Steel Bar	3.32	2.21	6.57	9.89
SA-A9	CB-14	2'x3' Steel Bar	3.32	1.80	4.82	6.62
SA-A10	CB-10	2'x3' Steel Bar	3.32	1.11	2.22	3.33

Inlet Report

SA-A10 Flow-By Inlet (Q100)

Drop Grate Inlet		**Calculations**	
Location	= On grade	Compute by:	Known Q
Curb Length (ft)	= -0-	Q (cfs)	= 3.33
Throat Height (in)	= -0-		
Grate Area (sqft)	= -0-	**Highlighted**	
Grate Width (ft)	= 3.00	Q Total (cfs)	= 3.33
Grate Length (ft)	= 2.00	Q Capt (cfs)	= 1.11
		Q Bypass (cfs)	= 2.22
Gutter		Depth at Inlet (in)	= 1.92
Slope, Sw (ft/ft)	= 0.027	Efficiency (%)	= 33
Slope, Sx (ft/ft)	= 0.027	Gutter Spread (ft)	= 17.85
Local Depr (in)	= -0-	Gutter Vel (ft/s)	= 1.75
Gutter Width (ft)	= 6.00	Bypass Spread (ft)	= 15.63
Gutter Slope (%)	= 0.79	Bypass Depth (in)	= 1.56
Gutter n-value	= 0.016		

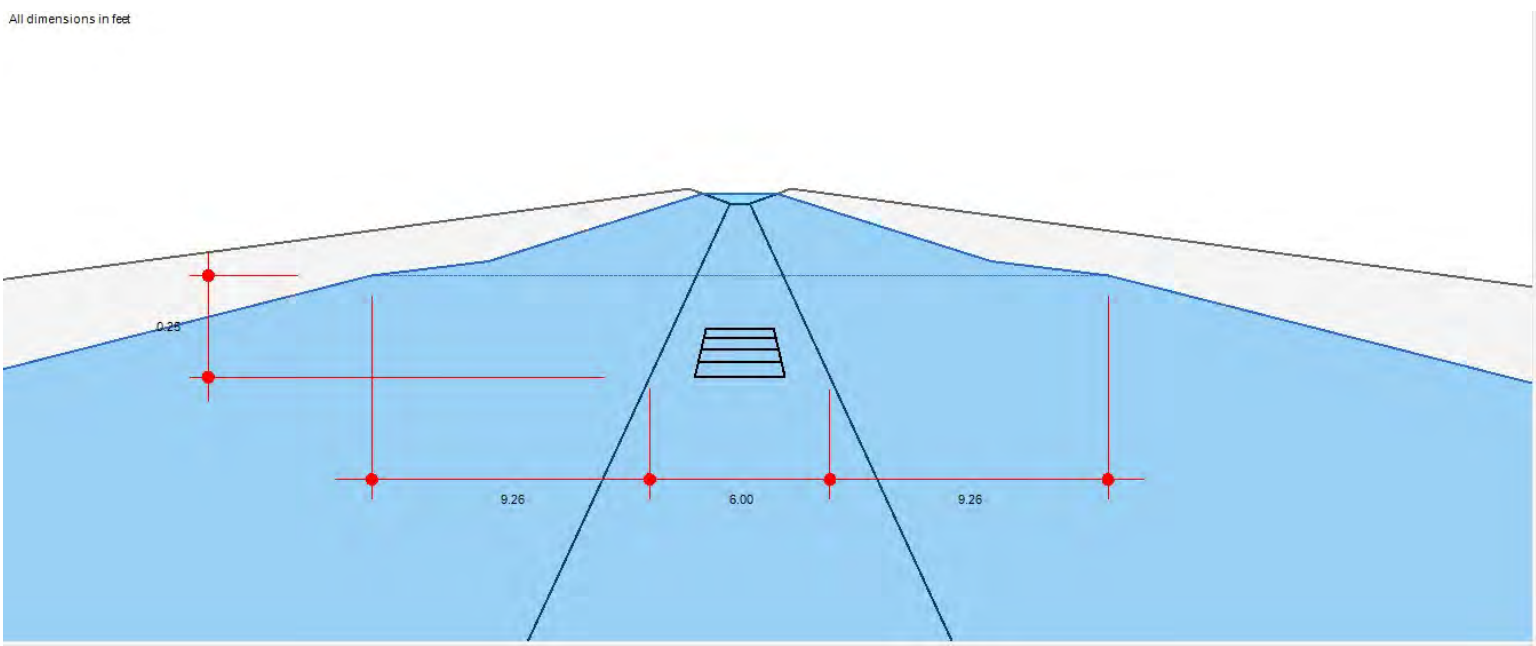


Inlet Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Wednesday, Oct 2 2024

SA-A8 Flow-By Inlet (Q100 - SA-A10 Capture)

Drop Grate Inlet		**Calculations**	
Location	= On grade	Compute by:	Known Q
Curb Length (ft)	= -0-	Q (cfs)	= 8.78
Throat Height (in)	= -0-		
Grate Area (sqft)	= -0-	**Highlighted**	
Grate Width (ft)	= 3.00	Q Total (cfs)	= 8.78
Grate Length (ft)	= 2.00	Q Capt (cfs)	= 2.21
		Q Bypass (cfs)	= 6.57
Gutter		Depth at Inlet (in)	= 3.00
Slope, Sw (ft/ft)	= 0.027	Efficiency (%)	= 25
Slope, Sx (ft/ft)	= 0.027	Gutter Spread (ft)	= 24.52
Local Depr (in)	= -0-	Gutter Vel (ft/s)	= 2.30
Gutter Width (ft)	= 6.00	Bypass Spread (ft)	= 22.30
Gutter Slope (%)	= 0.79	Bypass Depth (in)	= 2.64
Gutter n-value	= 0.016		

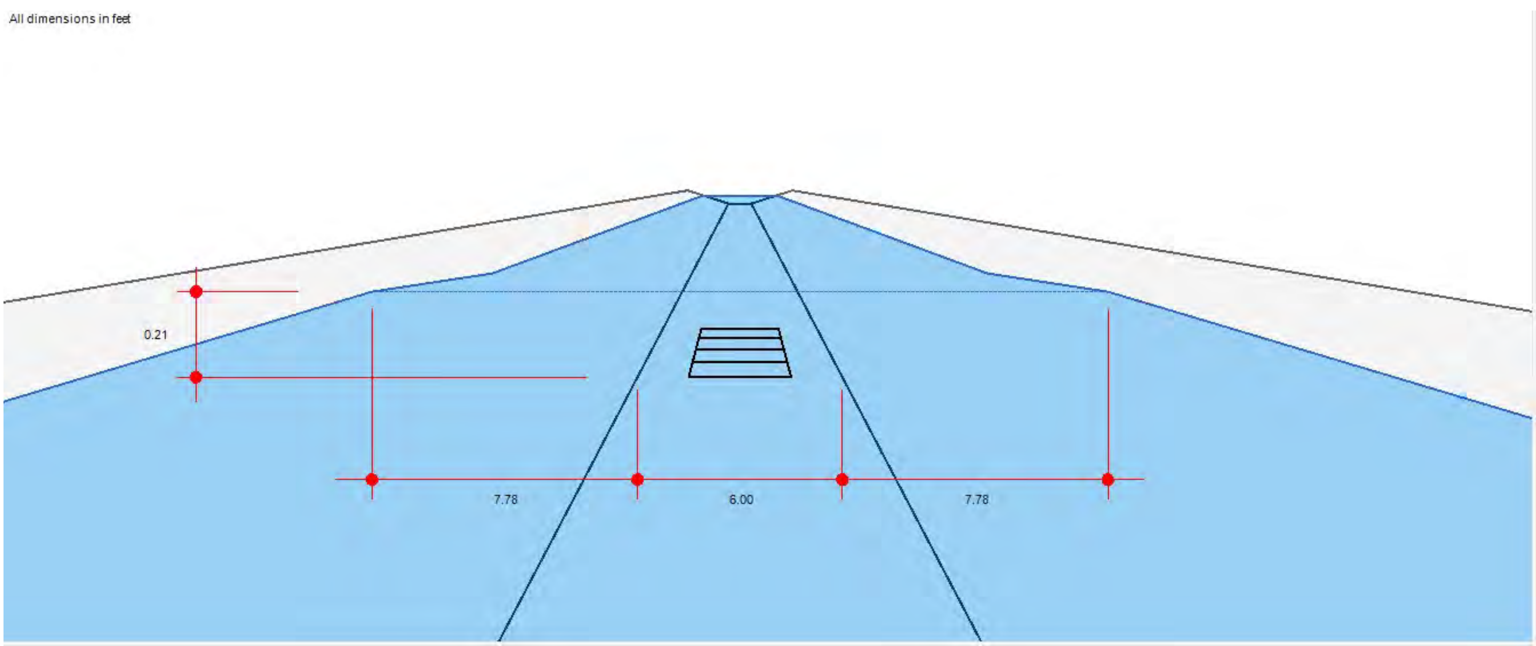


Inlet Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Wednesday, Oct 2 2024

SA-A9 Flow-By Inlet (Q100)

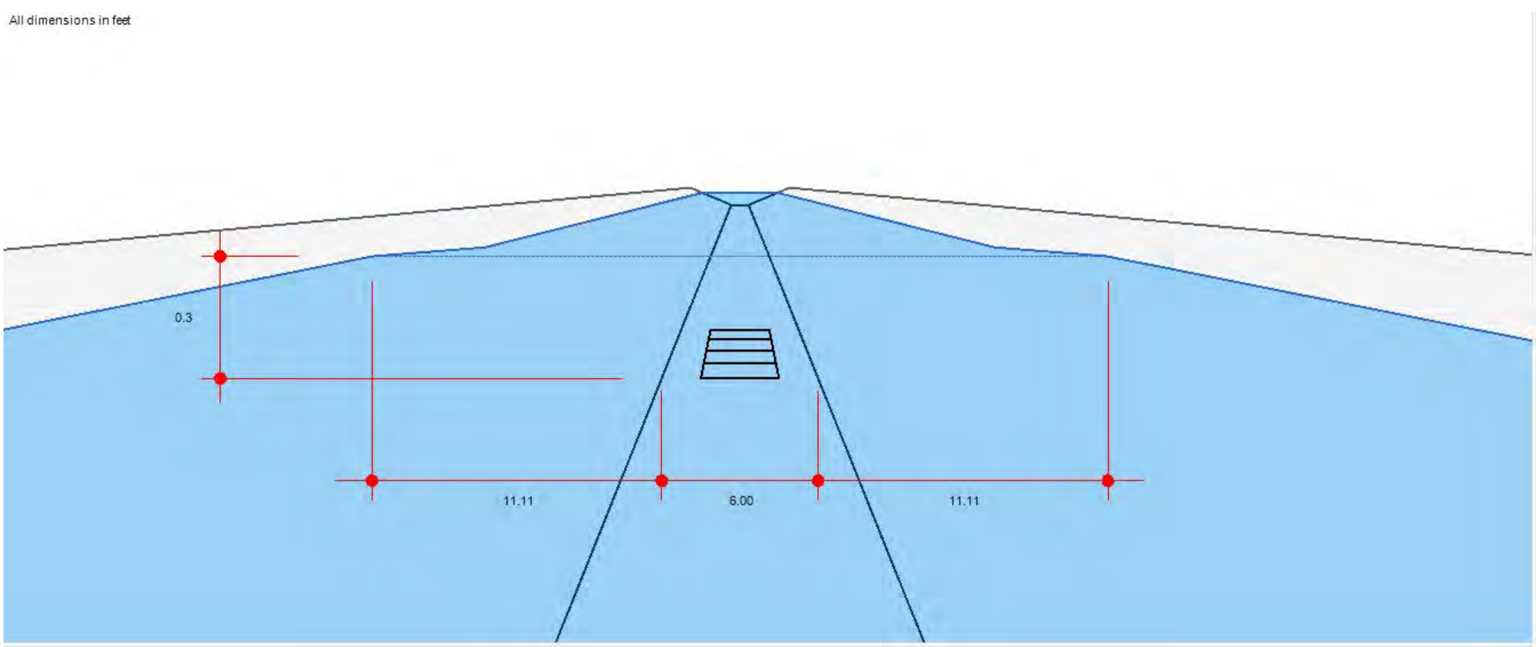
Drop Grate Inlet		**Calculations**	
Location	= On grade	Compute by:	Known Q
Curb Length (ft)	= -0-	Q (cfs)	= 6.62
Throat Height (in)	= -0-		
Grate Area (sqft)	= -0-	**Highlighted**	
Grate Width (ft)	= 3.00	Q Total (cfs)	= 6.62
Grate Length (ft)	= 2.00	Q Capt (cfs)	= 1.80
		Q Bypass (cfs)	= 4.82
Gutter		Depth at Inlet (in)	= 2.52
Slope, Sw (ft/ft)	= 0.027	Efficiency (%)	= 27
Slope, Sx (ft/ft)	= 0.027	Gutter Spread (ft)	= 21.56
Local Depr (in)	= -0-	Gutter Vel (ft/s)	= 2.29
Gutter Width (ft)	= 6.00	Bypass Spread (ft)	= 19.33
Gutter Slope (%)	= 0.93	Bypass Depth (in)	= 2.16
Gutter n-value	= 0.016		



Inlet Report

SA-A7 Flow-By Inlet (Q100 - SA-A10, SA-A8, SA-A9 Capture)

Drop Grate Inlet		Calculations	
Location	= On grade	Compute by:	Known Q
Curb Length (ft)	= -0-	Q (cfs)	= 14.19
Throat Height (in)	= -0-		
Grate Area (sqft)	= -0-	**Highlighted**	
Grate Width (ft)	= 3.00	Q Total (cfs)	= 14.19
Grate Length (ft)	= 2.00	Q Capt (cfs)	= 3.07
		Q Bypass (cfs)	= 11.12
Gutter		Depth at Inlet (in)	= 3.60
Slope, Sw (ft/ft)	= 0.027	Efficiency (%)	= 22
Slope, Sx (ft/ft)	= 0.027	Gutter Spread (ft)	= 28.22
Local Depr (in)	= -0-	Gutter Vel (ft/s)	= 2.76
Gutter Width (ft)	= 6.00	Bypass Spread (ft)	= 26.00
Gutter Slope (%)	= 0.93	Bypass Depth (in)	= 3.24
Gutter n-value	= 0.016		

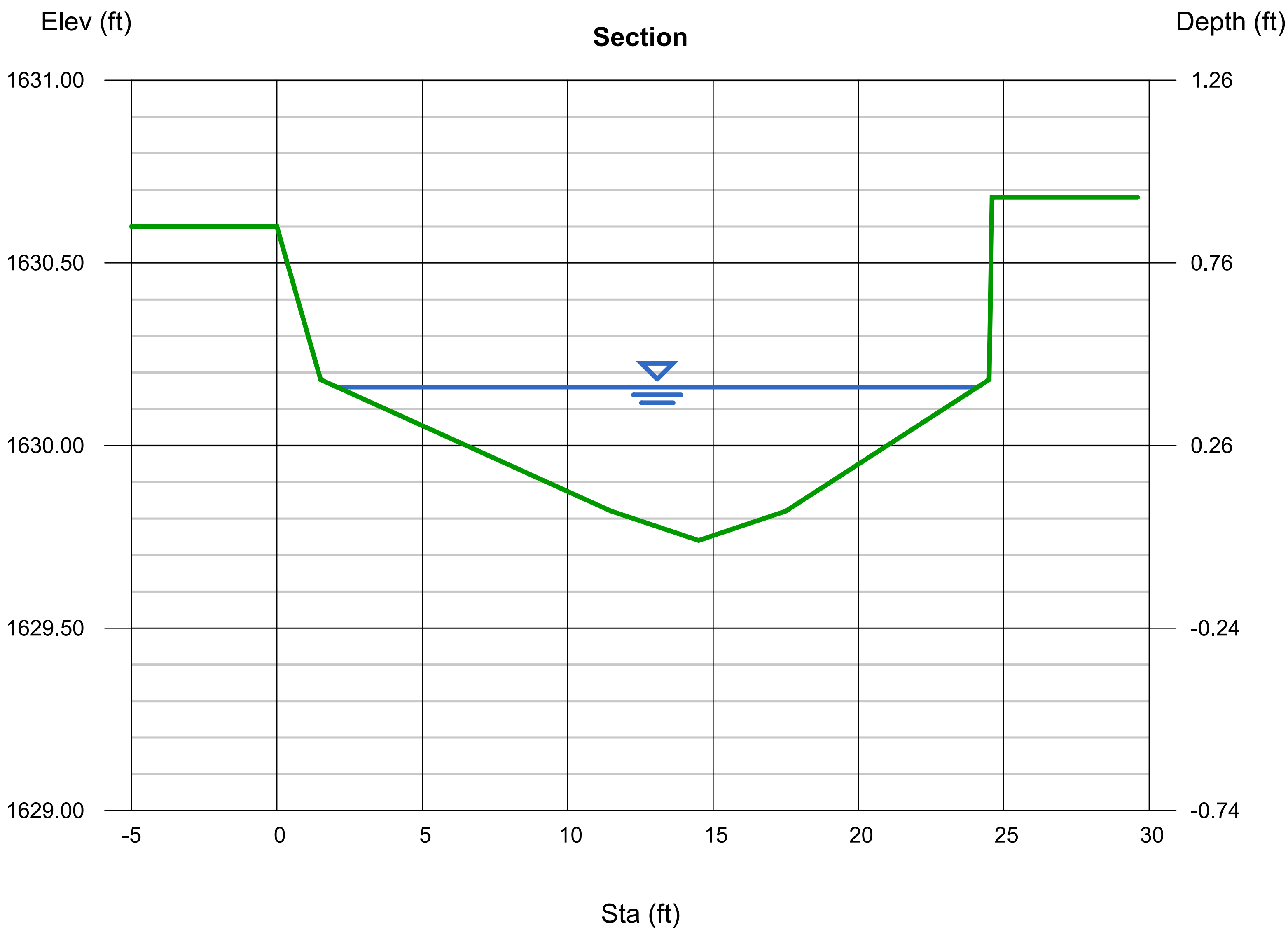


Channel Report

SA-A6 Q100 Street Capacity (SA-A7 through SA-A10 Capture Neglected)

User-defined		**Highlighted**	
Invert Elev (ft)	= 1629.74	Depth (ft)	= 0.42
Slope (%)	= 0.93	Q (cfs)	= 20.11
N-Value	= 0.013	Area (sqft)	= 5.01
		Velocity (ft/s)	= 4.01
Calculations		Wetted Perim (ft)	= 22.07
Compute by:	Known Q	Crit Depth, Yc (ft)	= 0.50
Known Q (cfs)	= 20.11	Top Width (ft)	= 22.05
		EGL (ft)	= 0.67

(Sta, El, n)-(Sta, El, n)...
(0.00, 1630.60)-(1.50, 1630.18, 0.013)-(11.50, 1629.82, 0.013)-(14.50, 1629.74, 0.013)-(17.50, 1629.82, 0.013)-(24.50, 1630.18, 0.013)-(24.60, 1630.68, 0.013)



HYDRAULIC CALCULATION SHEET
Inlet Calculations
Hubbard Engineering
Project No. 18114-750

Project Name: Toll at Cavasson **Prepared by:** TSW **Date:** 10/02/24
Project No.: 18114-750 **Revised By:** **Date:**

Purpose: Evaluate inlet capacities using Orifice Discharging Freely into atmosphere

Methodology: Calculate the capacity of an inlet using the equation for Orifice Discharging Freely into Atmosphere, assuming that h is equal to the depth of ponding over the orifice and Cd is the coefficient of discharge for the orifice. Sharp Edged will be used for most inlet conditions. Combination inlets neglect curb opening capacity

Criteria: Inlet Capacity after using a 50% clogging factor must equal or exceed the flows from 100-YR 2-HR duration storm event.

References:

Calculations: Orifice Discharging Freely Into Atmosphere $Q = C_d*A_0*Sqrt(2*g*h)$ [ft^3]

C_d= 0.61 Sharp Edged
C_d= 0.98 Rounded
C_d= 0.80 Short Tube

Results:

Identifiers		**Calculation Parameters**			**Results**		**Compare**
Contributory Area ID	**Inlet ID**	**Cross Sectional Area [ft^2]**	**Ponding Depth [ft]**	**Configuration**	**Inlet Capacity [cfs]**	**Clogged Inlet Capacity [cfs]**	**Q100** [cfs]
SA-A1	CB-1	3.32	0.40	Sharp Edged	10.27	5.14	0.66
SA-A2	CB-2	3.32	0.50	Sharp Edged	11.49	5.74	1.10
SA-A3	CB-3	3.32	0.45	Sharp Edged	10.90	5.45	6.09
SA-A4	CB-4	2.60	0.50	Sharp Edged	9.00	4.50	17.23
SA-A5	CB-5	3.32	0.50	Sharp Edged	11.49	5.74	6.32
SA-A6	CB-6	3.32	0.50	Sharp Edged	11.49	5.74	11.92
SA-A11	CB-8	3.32	0.30	Sharp Edged	8.90	4.45	3.59
SA-A12	CB-7	3.32	0.30	Sharp Edged	8.90	4.45	0.19

SA-A4 includes the area for SA-A3, Clogged Capture for SA-A3 has been removed from the Q100 for SA-A4, SA-A11 includes the offsite flow from Claret Drive DA-OFFA

Appendix G
Retention Dissipation Calculations
Toll at Cavasson

HYDRAULIC CALCULATION SHEET
Retention Calculations
Hubbard Engineering
Project No. 18114-750

Project Name: Toll at Cavasson **Prepared by:** TSW **Date:** 06/22/24
Project No.: 18114-750 **Revised By:** **Date:**

Purpose: Evaluate the required and provided retention volumes in order to assess conformance to project criteria.

Methodology: Calculate the volume of stormwater required to be retained using City of Scottsdale criteria. Calculate the estimated volume of stormwater retained using retention basin geometry.

Criteria: Retain the calculated stormwater run-off for the first flush condition and 100-YR 2-HR duration storm event.

References: 2. Design Standards and Policies Manual, City of Scottsdale

Calculations: First Flush Volume Required = $C_{Composite}$ *D/12* A [ft^3] (Reference 1)

D = 0.5	[in]	(First Flush)	(Reference 1)
C = 0.95	(Paved Parking)		(Reference 1)
P = 2.30	[in]	(100 YR 2 HR)	

Composite C= (C1*A1 + C2*A2…+….)/(A1+A2+….)

Volume Required = Composite C*P/12*A

Results:

Identifiers	CALCULATE RETENTION VOLUME REQUIRED		Volume Required		
Contributory Area ID	**Area [acres]**	**C**	**[acre-ft]**	**[ft^3]**	**[yd^3]**
Adjacent Half Streets	2.29	0.95	0.42	18,166	673
DA-A	10.96	0.95	0.43	18,899	700
Total Area:	**47.14**	**TOTAL VOLUME REQUIRED:**	**0.85**	**37,064.42**	**1,372.76**

Hubbard Engineering
Project No. 18114-750

Project Name: Toll at Cavasson
Project No.: 18114-750

Prepared by: TSW
Revised By:

Date: 06/22/24
Date:

VOLUME PROVIDED

$V_{U.P.} = \pi * r^2 * L$

$V_{U.P.}$	*Volume of underground pipe provided in cubic feet*
π	*pi*
r	*Underground pipe radius, ft* 5
L	*Length of underground pipe in feet*

UNDERGROUND RETENTION/DETENTION COMBINED

Retention Method	*Length*	*Volume Provided*
(ID)	*(ft)*	*(cf)*
UGST-1	45	3,534
UGST-2	25	1,963
UGST-3	55	4,320
UGST-4	105	8,247
UGST-5	105	8,247
UGST-6	140	10,996
Total	**475**	**37,306**

HYDRAULIC CALCULATION SHEET
Retention Basin Drain Time Calculations
Hubbard Engineering
Project No. 18114-750

Project Name: Toll at Cavasson | **Prepared by:** TSW | **Date:** 06/22/24
Project No.: 18114-750 | **Revised by:** | **Date:**

Purpose: Calculate the number of drywells required to facilitate drainage of the required volume within 36 hours.

Methodology: Calculate the number of drywells necessary to drain the retention basin and undeground storage tanks within 36 hours, assuming each drywell has an infiltration rate of 0.1 cfs

Criteria:

1. Drywell drainage capacity must discharge the retention volume provided within 36 hours.
2. No percolation tests have been done on site. A drywell infiltration rate of 0.1 cfs will be used in accordance with reference 2

References: 2. Design Standards and Policies Manual, City of Scottsdale

Calculations:

Number of Drywells Required = V_{DW} / (Drywell Infiltration Rate * 3600 * 36 hours)

Drywell Infiltration Rate = 0.1 [cfs] (Reference 1)

Results:

Identifiers	Infiltration Drainage Capacity Calculations					V_{DW}	Number of
Retention Basin ID	Volume(Ref. 1) Required [ft^3]	Bottom(Ref 1) Area [ft^2]	Percolation Rate(Ref. 2) Test Location*	Percolation Rate(Ref. 2) [$ft^3/hr/ft^2$]^2	Infiltration Drain Capacity [ft^3]	[ft^3]	Drywells Required
UGST-1	3,534.3	0.0	--	0.000	0.0	3,534.3	1
UGST-2	1,963.5	0.0	--	0.000	0.0	1,963.5	1
UGST-3	4,319.7	0.0	--	0.000	0.0	4,319.7	1
UGST-4	8,246.7	0.0	--	0.000	0.0	8,246.7	1
UGST-5	8,246.7	0.0	--	0.000	0.0	8,246.7	1
UGST-6	10,995.6	0.0	--	0.000	0.0	10,995.6	1
					Total Number of Drywells =		**3**

PHOENIX
1102 West Southern Avenue, Suite 4
Tempe, Arizona 85282
(O) 602-272-PTX1 (7891)
Dispatch 602-272-7890
(F) 602-272-7892
WWW.PROTEX-AZ.COM

TUCSON
916 West Grant Road
Tucson, Arizona 85705
(O) 520-352-1050 (ext 157)
Dispatch 520-352-0150
(F) 520-352-0150
WWW.PROTEX-AZ.COM

August 29, 2024

Toll Brothers Inc (Land)

Re: **Drywell Percolation Testing Report**

Job: Miller Road and Cavasson Boulevard
Hayden Road and Loop 101
Pinal County, Arizona

ProTeX Job No.: 15349

Attention: Mr. Aaron Insco

The project consists of one- or two-story townhome residential structures. The requested testing consisted of one (1) constructed drywell located within the graded retention basins (please see the attached site map for drywell location). The drywell was tested using the following methodology.

Test Methodology:

1. Water is discharged into a drywell using a 3-inch diameter fire hose. The water level is raised to within a few feet of the rim of the drywell.
2. The rate of flow is adjusted until the water level is at a relatively constant elevation.
3. The flow of water through the meter was measured for a given period of time while holding the water level as constant as possible.
4. The measured flow of water for the given period of time is then divided by the time period to determine the flow rate.
5. Steps 3 and 4 are repeated for an extended period of time.

Discussion of Results:
Water was discharged into the drywell and tested for a period of approximately 1.5 hours. The following rate of percolation is assigned to the drywell:

Test Location	Percolation Rate (CFS)
DW 1	0.49

Please contact the undersigned at Engineering@ProTeX-AZ.com if there are any questions.

Respectfully submitted,
ProTeX - the PT Xperts, LLC

Registered Professional Engineer (Civil)
CERTIFICATE NO. 47662
JONES TEMBO
Date Signed 8.29.24
ARIZONA, U.S.A.
Expires 03/31/2026

Ryan Ellwanger
Staff Geologist

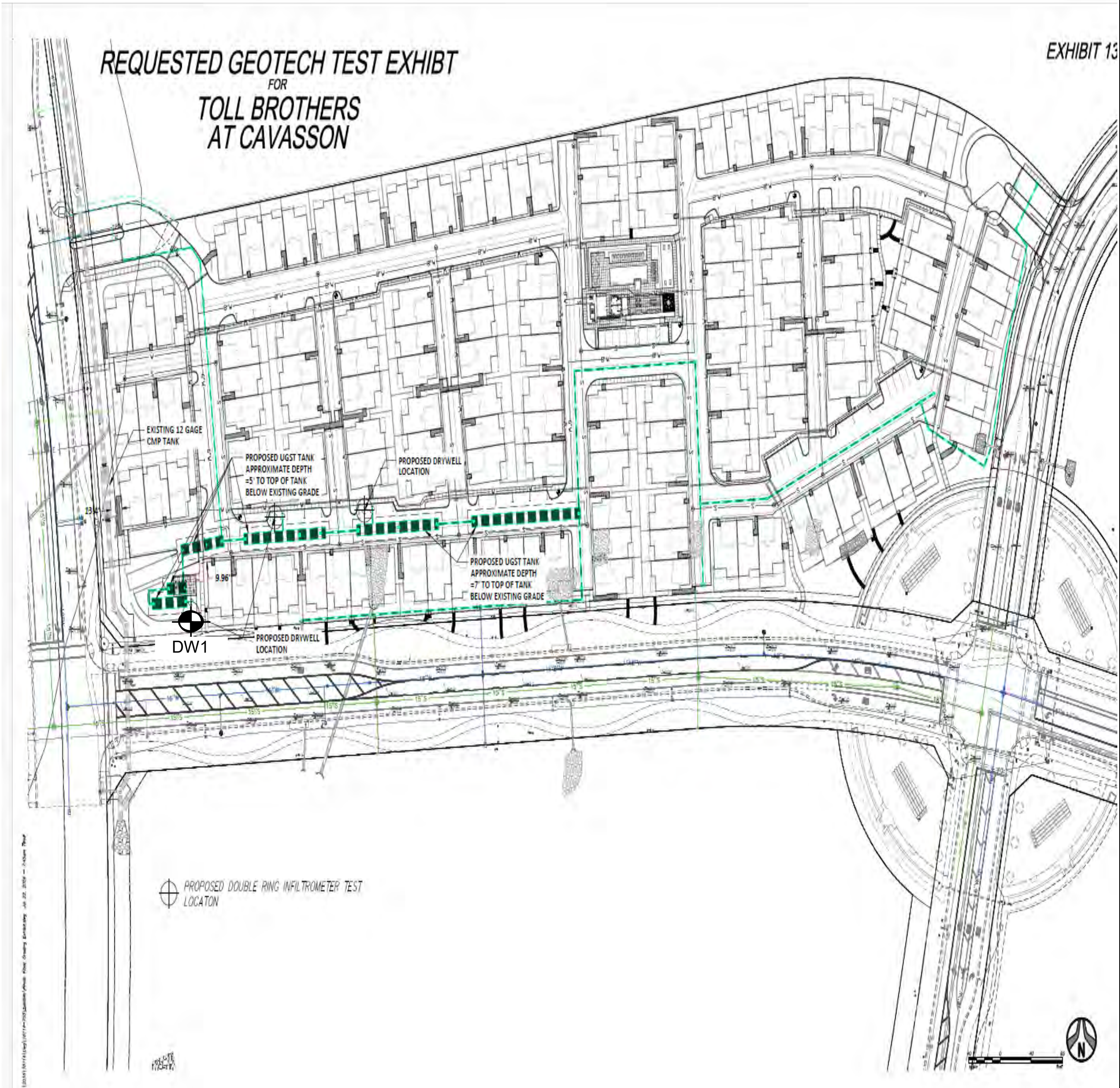
Jones Tembo, PE

ATTACHMENTS: (2) pages: (1) Drywell Field Test Data and (1) Site Map

Client:	Toll Brothers Inc (Land)
Job Name:	Miller Rd and Cavasson Blvd
Location:	Hayden Rd and Loop 101
Date:	8/23/2024
Tested By:	JM
Drywell:	DW# 1

	Time	**Gauge Reading (Gal)**
Test Start:	7:10	5625034.0
Test End:	9:00	5649459.0

T-Initial (min)	**T-final (min)**	**G-initial (Gal)**	**G-final (Gal)**	**Rate (Gal/min)**	**Water Depth From Rim (ft)**
7:40	7:50	5632063.0	5634267.0	220.4	6.0
7:50	8:00	5634267.0	5636459.0	219.2	6.0
8:00	8:10	5636459.0	5638599.0	214.0	6.0
8:10	8:20	5638599.0	5640635.0	203.6	6.0
8:20	8:30	5640635.0	5642810.0	217.5	6.0
8:30	8:40	5642810.0	5645003.0	219.3	6.0
8:40	8:50	5645003.0	5647296.0	229.3	6.0
8:50	9:00	5647296.0	5649459.0	216.3	6.0
				221.6	**Avg (Gal/min)**
				0.49	**Avg (CFS)**



Legend:

Approximate Drywell Location

Site Plan

Scale: N.T.S.	Drawn by: Ryan Ellwanger	Date:08/23/2024

Miller Rd and Cavasson Blvd

Hayden Road and Loop 101

Scottsdale, Arizona

ProTeX Job No.:15349

Appendix H
404 Permit
Toll at Cavasson

March 27, 2019

U.S. Army Corps of Engineers
Los Angeles District – Regulatory Division
915 Wilshire Blvd.
Los Angeles, CA 90017

Subject: Statement of Sale of 0.89 Credits for the Cavasson Mixed-Use Development project, Corps permit number SPL-2018-00704, from the Arizona Game and Fish Department In-Lieu Fee Program to Nationwide Reality Investors Ltd.

The Arizona Game and Fish Department has an agreement with the U.S. Army Corps of Engineers – Los Angeles District to operate an In-Lieu-Fee Program. This letter confirms the sale of 0.89 credits of Advance Credits for establishment/enhancement of wetlands and Mesquite Bosque within the Arlington Wildlife Area; an Arizona Game and Fish Commission owned property. These credits are being used as compensatory mitigation for 0.89 acres of impact calculated at a 1:1 ratio for impacts to Waters of The U.S. associated with the Cavasson Mixed-Use Development Project. By selling credits to the above permittee, AGFD is the party responsible for fulfilling the mitigation aspect of Special Condition of the Permit listed above.

Signed,

Shawn F. Lowery
Statewide ILF Restoration Program Manager
Wildlife Contracts Branch
Arizona Game and Fish Department

Exhibits

Toll at Cavasson

HEC -1 ULTIMATE CONDITION

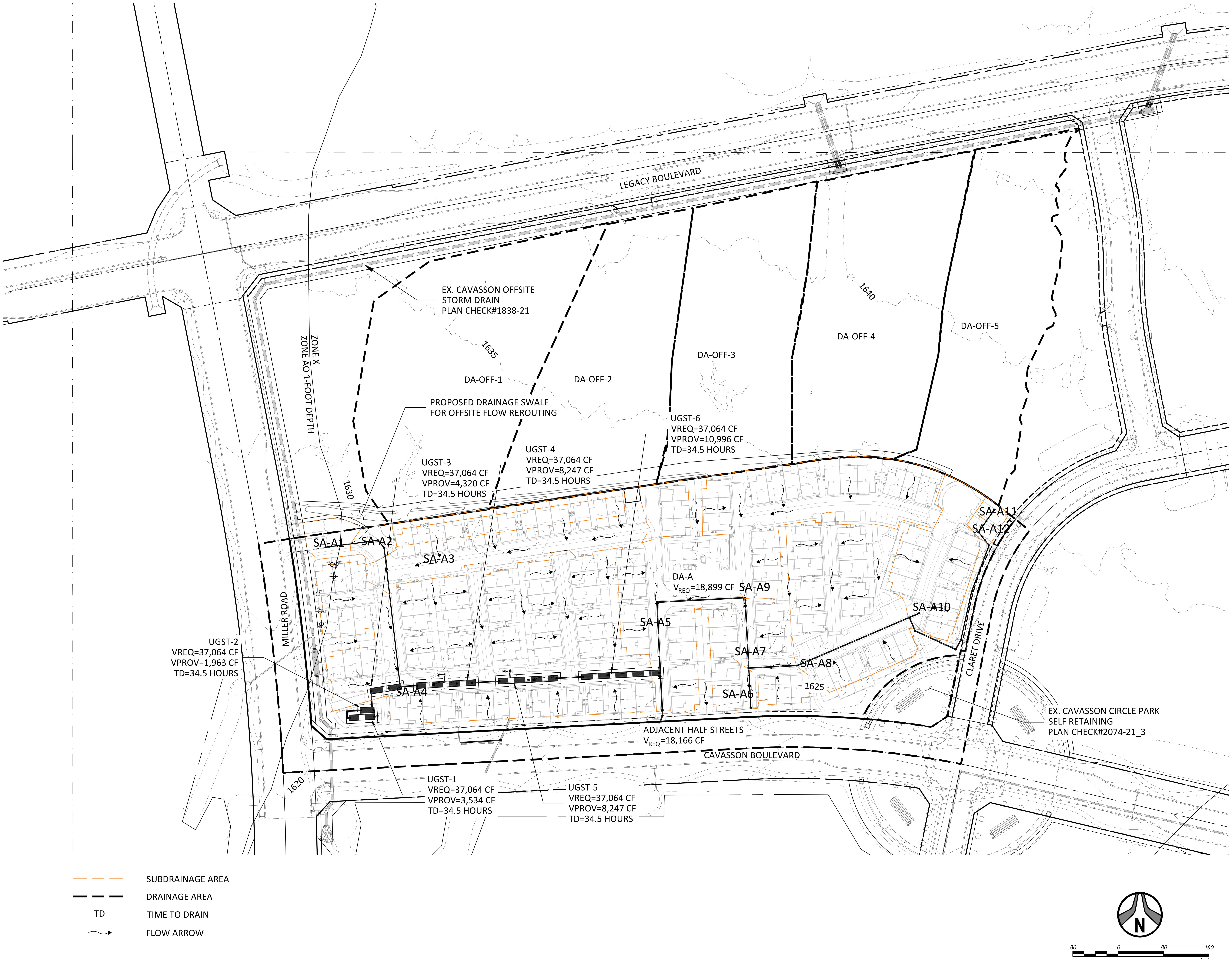
HEC-1 ULTIMATE ROUTING

ASLD CORE NORTH

Project No.	Date	
18114-221	07/21/23	EX-07

HUBBARD
ENGINEERING
www.hubbardengineering.com
1201 S. Alma School Rd., Ste. 12000 - Mesa, AZ 85210
Ph: 480.892.3313

P:\2018\18114\Design-Reports_MP\18114-101_Phase 3C Update\Drainage\Exhibits\DWG\Working\HEC1-CrossRoadsEast_Future_Mk.dwg Jul 21, 2023 - 9:55am TWolf



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HUBBARD ENGINEERING
1201 S. Alma School Rd.
Suite 12000
Mesa, AZ 85210
Ph: 480.892.3313
www.hubbardengineering.com

DRAINAGE MAP
TOLL AT CAVASSON

Date 10-2-2024
Project Eng. M. WOLF
Project No. 18114-750
Project Mgr. T. WOLF

NOT FOR CONSTRUCTION
PRELIMINARY
OR RECORDING
BID AT RISK

811 Call before you dig.
1.800.STAKE.IT
602.263.1100

EX 02

NOTE: ALL CONTOURS SHOWN ARE PREDEVELOPMENT

20 0 20 40
scale feet

N

HUBBARD ENGINEERING
1201 S. Alma School Rd.
Suite 12000
Mesa, AZ 85210
Ph: 480.892.3313
www.hubbardengineering.com

HAG/LAG EXHIBIT
TOLL AT CAVASSON

Project No. 18114-750	Date 06-24-2024
Project Mgr. T. WOLF	Project Eng. M. WOLF

NOT FOR CONSTRUCTION
PRELIMINARY
OR RECORDING
BID AT RISK

811
Call before you dig.
1.800.STAKE.IT
602.263.1100

EX 03

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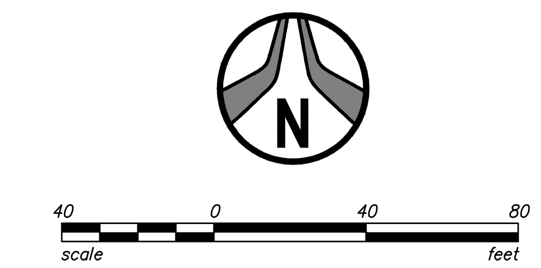
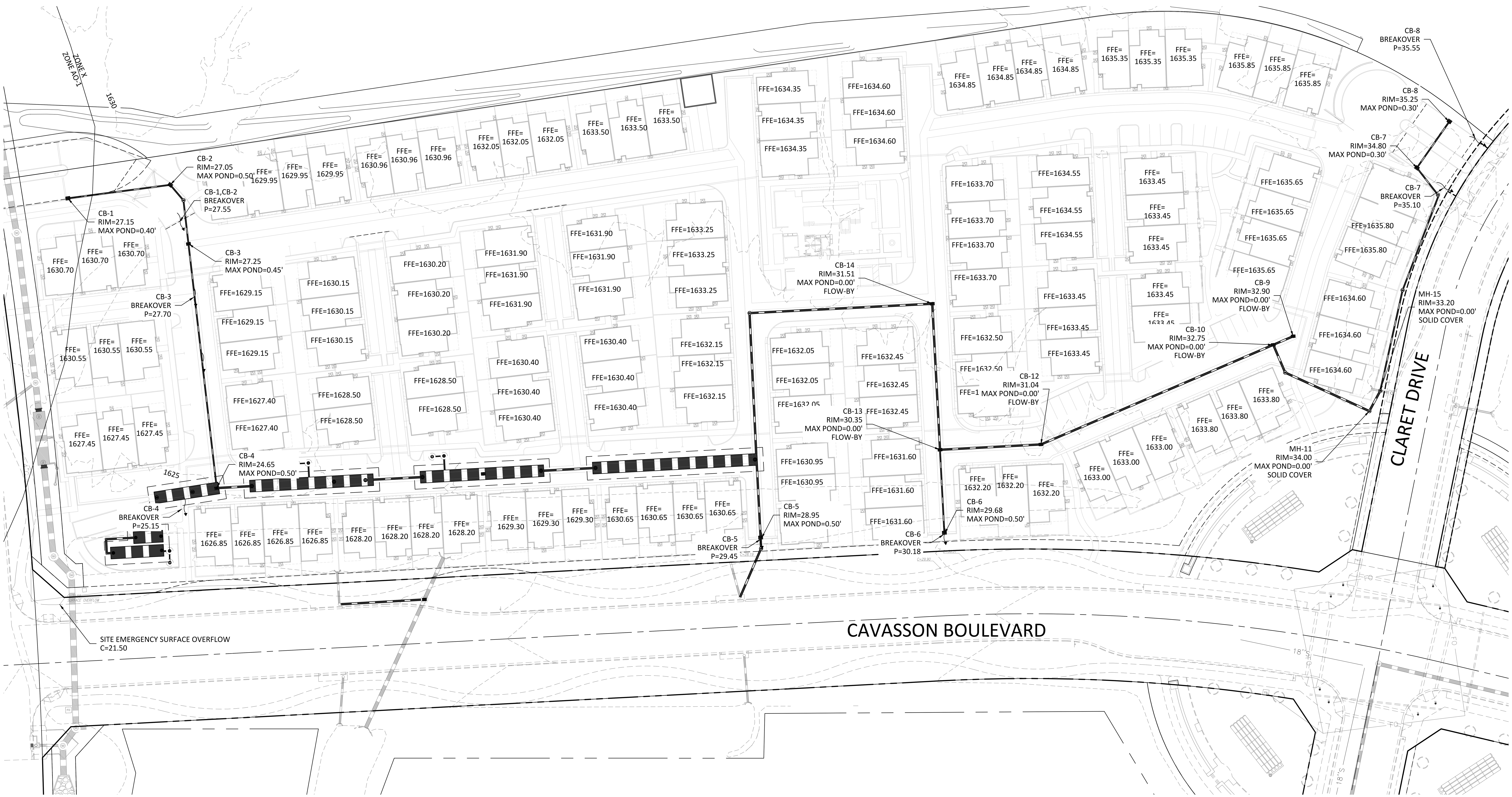
NOTE: MAX PONDING IS TO THE SURFACE OVERFLOW

NOTE: CB-1, CB-2, CB-3, CB-4, CB-5, CB-6, CB-7, AND CB-8 ARE THE ONLY CATCH BASINS LOCATED IN A SUMP CONDITION. ALL REMAINING CATCH BASINS, MAINTENANCE STRUCTURES, TANK ACCESS PORTS, AND STORM DRAIN MANHOLES WILL HAVE MAX PONDING DEPTH = 0.0'





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CATCH BASIN PONDING EXHIBIT
TOLL AT CAVASSON

Date 10-2-2024	Project Eng. M. WOLF
Project No. 18114-750	Project Mgr. T. WOLF

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EX 04

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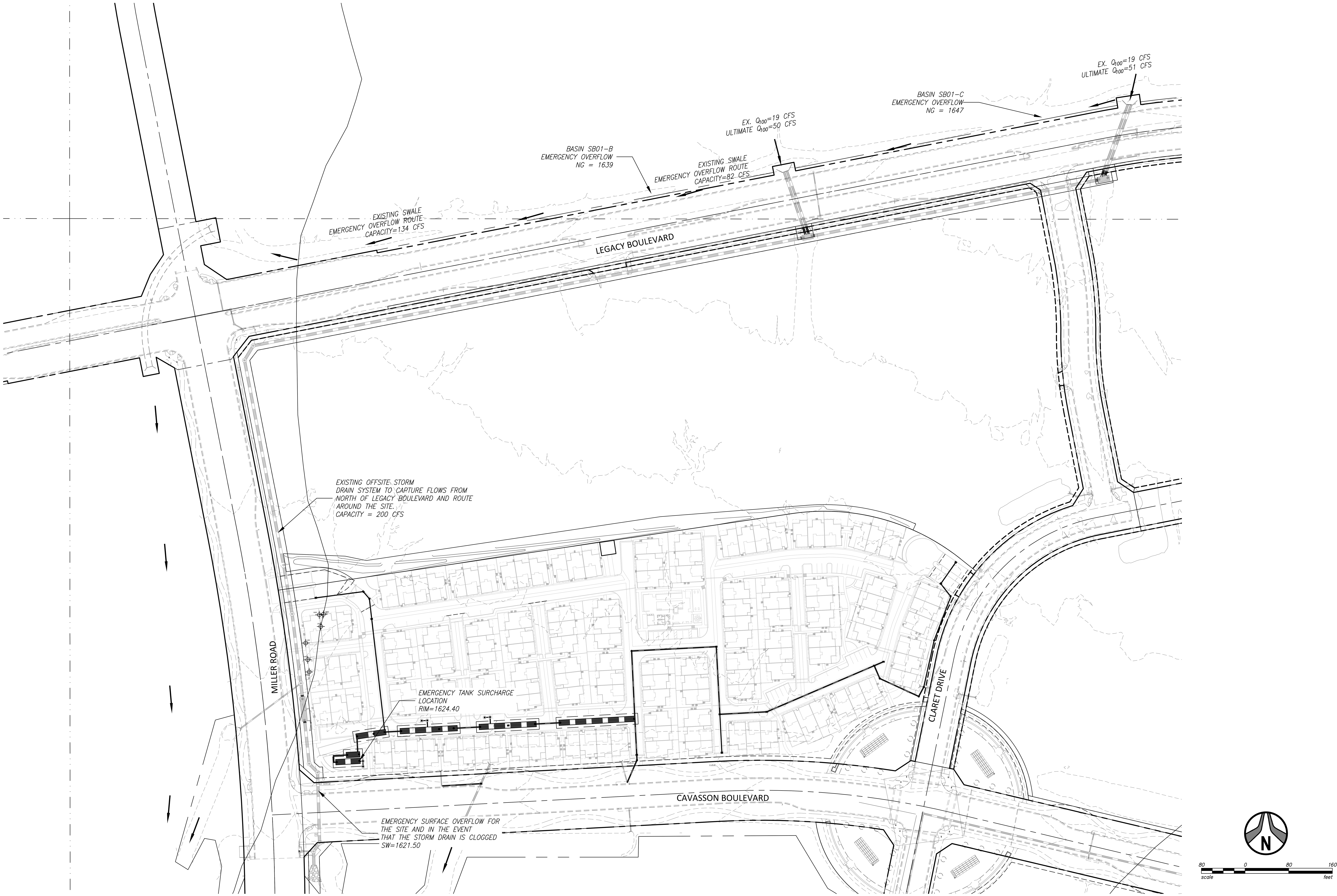

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EMERGENCY OVERFLOW EXHIBIT
TOLL AT CAVASSON

Project No.	Date
18114-750	10-2-2024
Project Mgr.	Project Eng.
T. WOLF	M. WOLF

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EX 05