DRAINAGE REPORTS

ABBREVEATED WATER & SEWER NEED REPORTS

WATER STUDY

WASTERWATER STUDY

STORMWATER WAIVER APPLICATION

PRELIMINARY DRAINAGE REPORT

CATTLETRAK TRUE HOMES



JUNE 2016 DEA PROJECT NO. TRUH0001

PRELIMINARY DRAINAGE REPORT FOR CATTLETRACK TRUE HOMES

PREPARED FOR

TRUE HOMES 7831 E. BUENA TERRA WAY SCOTTSDALE, AZ 85250

PREPARED BY

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JULY 2016 DEA PROJECT NO. TRUH0001



TABLE OF CONTENTS

| 1. INTRODU | JCTION | 3 |
|-------------------------|---|-------------|
| 2. EXISTING | G DRAINAGE CONDITIONS | 3 |
| 3. PROPOSI | ED DRAINAGE CONCEPT | 4 |
| 3.2. OFF- | SITE DRAINAGE CONVEYANCESITE DRAINAGE CONVEYANCESITE STORM WATER STORAGE | 4 |
| 4. HYDROL | OGIC ANALYSIS | 5 |
| 5. HYDRAU | LIC ANALYSIS | 6 |
| 6. CONCLU | SIONS | 6 |
| 7. REFEREN | NCES | |
| FIGURES | TITLE | LOCATION |
| 1 | Vicinity Map | Appendix A |
| <u>EXHIBITS</u> | TITLE | LOCATION |
| A | Drainage Area Map | Back Pocket |
| APPENDIX A B C | TITLE Figures FEMA Flood Insurance Rate Map Volume Calculations and Data Sheets | .Appendix B |



1. INTRODUCTION

This Preliminary drainage report has been prepared under a contract with True Homes, owner/developer of the proposed subdivision. The purpose of this report is to provide preliminary drainage analyses, required by the City of Scottsdale, to support the proposed site plan for this project. This report and design follows the procedures outlined in the City of Scottsdale, Design Standards and Policy Manual (Reference 1) and the Drainage Design Manuals for Maricopa County, Arizona, Volumes I and II (References 2 and 3).

The overall project is located in Section 14, Township 2 North, Range 4 East of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. The site is bounded by Cattletrack Road to the west, residential development to the north, open space/tennis courts to the south, and Arizona Trail/Canal to the east. Figure 1, located in Appendix A, illustrates the location of the project in relation to the City of Scottsdale's street system.

The Cattletrack Subdivision project is a proposed development of approximately 1.9 acres. Onsite improvements include; street, graded pads, and open space/retention to the east.

2. EXISTING DRAINAGE CONDITIONS

This project is currently an existing single residential home. The site drains in an easterly direction towards the canal. There are few feet drop to the east. There is an open space to the south with tennis court and it is depressed several fee from the elevations onsite.

Cattletrack Road, to the west, is developed road with curb and gutter and slopes in a southerly direction.

Offsite runoff impacts the eastern portion of the site and a floodplain (Zone A) occupies the eastern portion of the site as shown in the FIRM panel 04013C1770L. Zone A floodplain is defined as areas with a 1% annual chance of flooding and a 26% chance of flooding over the life of a 30-year mortgage. Because detailed analyses are not performed for such areas; no depths or base flood elevations are shown within these zones.

The remainder of the site to the west is located in Zone D. Zone D floodplain is defined as Areas with possible but undetermined flood hazards. No flood hazard analysis has been conducted. Flood insurance rates are commensurate with the uncertainty of the flood risk.

The general lay of the land is in an easterly direction. Runoff flows towards the canal and ponds next to it and eventually flows in a southern direction when the ponding next to the canal recedes.

DEA drainage team researched available drainage information and coordinated with the Flood

control district and their consultant for the overall drainage study taken place in the area. This drainage report is based on existing floodplain conditions and not the new drainage study underway.

3. PROPOSED DRAINAGE CONCEPT

The proposed drainage concept is presented in three parts: on-site drainage conveyance, off-site drainage conveyance and on-site storm water storage. The hydrology analysis is summarized in section 4.0 and the hydraulic analysis is summarized in section 5.0. See Exhibit A, located in the back pocket, for a graphical illustration of the proposed drainage conditions map.

3.1. On-Site Drainage Conveyance

The intent of the grading design is to convey the runoff generated onsite during a 100-year 2-hour storm event or smaller into retention basins, along the lots and the eastern open space of the overall project.

The road conveys street runoff to the east into the cul-de-sac and then spills into the open space east of the project along the canal. The eastern portion of the site is where the existing floodplain is located.

The extreme outfall of the site is over the canal, at an elevation of 1979.40. The finish floor elevations of the proposed subdivision are being designed to be above this elevation by a foot. Refer to Exhibit A in the back pocket of the report for reference and to the Concept Grading Plans also included in the back pocket.

A CLOMR application will be filed for this project that will show that the proposed grading and reduction in the floodplain area is based on providing compensatory storage (820 ft³) where the lots are being filled. The project will provide volume generated during the 100-year 2 hour storm event onsite. The compensatory volume will be stored above the retention volume in the same basin and will be allowed to enter and leave as needed since it is a ponding situation next to the canal. The volume information is also reflected in the retention calculations section. The compensatory storage is calculated based on the difference between the proposed finish grade elevations and what currently exists in the floodplain. The difference between the two surfaces, using Civil 3D software, was accounted for in the compensatory storage calculations.

3.2. Off-Site Drainage Conveyance

The only runoff that impacts the site is from the floodplain to the east. The floodplain is currently being studied by Flood Control District of Maricopa County (FCDMC) in collaboration with the City of Scottsdale (COS). The current FEMA floodplain is changing based on the new study, however the results are preliminary and not being used for this project. The current effective firm panel information is being used for the purpose of this drainage report.

Through discussions with the consultant for FCDMC, although the new study reflects a larger potential floodplain for the project, the high water surface elevations that are being analyzed through modeling of the overall area are not higher than the adjacent existing grade near the canal. Hence the finish floor elevations are being designed based on the spill elevations along the adjacent Arizona Canal (existing floodplain) and it will also work for the proposed floodplain as well.

The volume that is being filled in the floodplain is being compensated for with open space at the eastern portion of the site in addition to the 100 year volume as mentioned above.

3.3. On-Site Storm Water Storage

The City of Scottsdale requires new development to store the on-site runoff generated during a 100-year, 2-hour storm event. The required storage volume for the project site is estimated as follows:

$$V_R = (P/12)*A*C$$

Where:

V_R = Calculated volume in acre-feet

C_{wt} = Weighted Runoff coefficient (C= .76)

P = Precipitation amount in inches (2.18 inches)

A = Drainage area in acres, including one-half of all abutting streets

A drywell will be utilized to assist in percolating the stored runoff for the 100-year 2 hour volume only into the ground, as there is no adjacent City storm drain system. The number of drywells will be determined after the first drywell is installed and a percolation test is done. The testing of the drywells will be performed and completed by a licensed geotechnical engineer.

4. HYDROLOGIC ANALYSIS

The hydrologic analysis for the site will be determined using the procedures set in the *City of Scottsdale Design Standards and Policies Manual* and the *Drainage Design Manual for Maricopa County, Arizona, Volume I.* Rational for Windows will be utilized to compute the on-site peak discharges. The program is based on the Flood Control District of Maricopa County methodology as explained in Volume 1. The following establishes the Rational Method equation and the basic input data required:

$$Q = C_{wt} I A$$

Where:

 C_{wt} = The runoff coefficient relating runoff to rainfall

I = Average rainfall intensity in inches/hour, lasting for Tc

 T_c = The time of concentration (minutes)

A = The contributing drainage area in acres (from Exhibit A).

Upon acceptance of this conceptual design by the City of Scottsdale, on-site peak discharges will be performed using the methodology above in order to size inlets and storm drain pipes as part of the final drainage report.

5. HYDRAULIC ANALYSIS

The hydraulic analysis of the proposed stormwater management facilities will be determined according to the City of Scottsdale Design Standards and Policies Manual and the Maricopa County Drainage Design Manual, Volume II, Hydraulics.

6. CONCLUSIONS

Based on the results of this study, it can be concluded that:

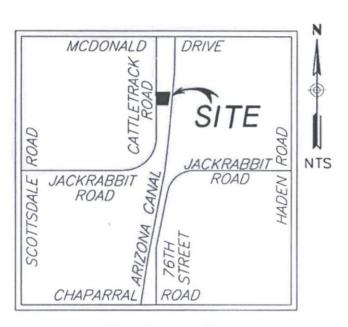
- * The site will be developed according to the City of Scottsdale Design Standards and Policies Manual and the Drainage Design Manuals for Maricopa County.
- * The 100-year 2-hour volume is retained on-site.
- * Compensatory storage will be provided for filling in the floodplain portion of the site.
- * The buildings will not be inundated during a 100-year storm event.
- * A CLOMR application will be filed after the acceptance of this drainage report, followed by a LOMR application after the project is built.

7. REFERENCES

- 1) City of Scottsdale, Design Standards and Policy Manual, January 2010.
- 2) Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology, Flood Control District of Maricopa County, August 15, 2013.
- 3) Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics, Flood Control District of Maricopa County, August 15, 2013.

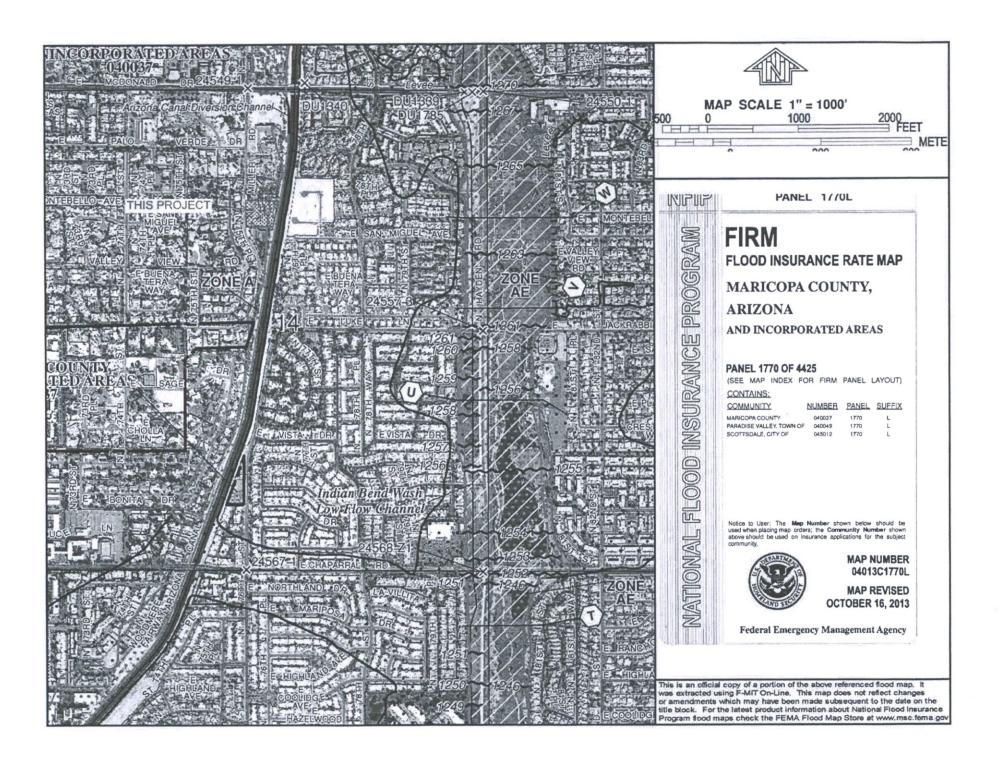


FIGURE 1 (Vicinity Map)



VICINITY MAP

APPENDIX B (FEMA Flood Insurance Rate Map)



APPENDIX C
(Volume Calculations and Data Sheets)

| Contributing Drainage Areas: | A | | | | | | | | |
|------------------------------|---------------|--------------------|-----------------|---------------|-----------------------------|---------------------------|---|---------------|--------------------------------|
| Basin: | Need surplus | of 820 FT3 in co | | | | | | | |
| VOLUME REQUIRED CALCULAT | TIONS | | | | | | | | |
| | A | rea | 'C' Coefficient | Precipitation | Rete | ntion Req | uired | | |
| Туре | (ft) | (Ac) | С | (Inches) | (ft ³) | The second Transferration | (Ac-ft) | | |
| Residential | 48,188 | 1.11 | | _ | 6,653 | | 0.16 | | |
| | | | | | | | | | |
| RETENTION BASIN CALCULATI | ONS (100-year | 2 Hours Only) | | | | | | | |
| Elevation | Delta Depth | Surface Area | | | Volume Prov | ided | perceptual des l'articles de la company | | |
| | (ft) | (ft ²) | | (ft³) | Σ (ft ³) | (Ac-ft) | Σ (Ac-ft) | | |
| 1279.4 | 1.4 | 8,843 | 8,692 | 11,425 | 22,159 | 0.26 | 0.51 | 11,425 | Compensatory Storage |
| 1278.0 | 1.0 | 7,497 | 8,692 | 6,579 | 10,734 | 0.15 | 0.25 | | |
| 1277.0 | 1.0 | 5,702 | 5,750 | 4,154 | 4,154 | 0.10 | 0.10 | 10,734 | Provided for 100-year, 2 hours |
| 1276.0 | | 2,780 | 3,161 | | 10,734 | | 0.25 | 4,080 | Excess |
| | _ | | | | | | | | |
| | | | Provided | | 10,734 | | 0.25 | | Total for Compensatory |
| | | | Required | | 6,653 | | 0.18 | 4,080+11,425= | 15,505 > 820 FT3 |
| | Basin Depth | | Balance | | 4,080 | | 0.07 | | 0.36 ac-ft |
| | 1.38 | | | | | | | | |

| Elevation | Delta Depth | Surface Area | | Volume Provided | | | |
|-----------|-------------|--------------------|----------|--------------------|-----------------------------|---------|-----------|
| | (ft) | (ft ²) | | (ft ³) | Σ (ft ³) | (Ac-ft) | Σ (Ac-ft) |
| 1278.8 | 0.8 | 8,843 | 8,692 | 6,529 | 17,262 | 0.15 | 0.40 |
| 1278.0 | 1.0 | 7,497 | 8,692 | 6,579 | 10,734 | 0.15 | 0.25 |
| 1277.0 | 1.0 | 5,702 | 5,750 | 4,154_ | 4,154 | 0.10 | 0.10 |
| 1276.0 | | 2,780 | 3,161 | | 10,734 | | 0.25 |
| | | | Provided | | 10,734 | | 0.25 |
| | | | Required | _ | 6,653 | | 0.00 |
| | Basin Depth | | Balance | | 4,080 | | 0.25 |
| | 1.38 | | | - | | | |

| Contributing Drainage Areas: Basin: | В | | | | | | |
|---|-------------|--------------------|-----------------|--------------------|---|-----------|------------------------|
| Dasiii. | | | | | | | |
| VOLUME REQUIRED CALCULA | | | | | | | may - El please Clarke |
| | | rea | 'C' Coefficient | Precipitation | | | - Annual Control |
| Туре | (ft) | (Ac) | С | (Inches) | (ft³) | | (Ac-ft) |
| Residential | 9,170 | 0.21 | | - | 1,266 | | 0.02 |
| RETENTION BASIN CALCULAT | | | | | | | |
| Elevation | Delta Depth | Surface Area | | | Volume Prov | | |
| | (ft) | (ft ²) | | (ft³) | Σ (ft ³) | (Ac-ft) | Σ (Ac-ft) |
| 1280.4 | 1.1 | 2,489 | | 2,112 | 2,065 | 0.05 | 0.05 |
| 1279.3 | -0.1 | 1,402 | | -47_ | -47 | 0.00 | 0.00 |
| 1279.4 | | | | | 2,065 | | 0.05 |
| | 7 | | Provided | | 2,065 | | 0.05 |
| | | | Required | | 1,266 | | 0.02 |
| | Basin Depth | | Balance | | 799 | | 0.03 |
| | 0.58 | | | - | | | |
| Contributing Drainage Areas: Basin: VOLUME REQUIRED CALCULA | | rea | 'C' Coefficient | Precipitation | Poto | ntion Req | uirad |
| Tues | (ft) | (Ac) | C | (Inches) | (ft ³) | ntion req | (Ac-ft) |
| Type Residential | 8,875 | | - | (inches) | 1,225 | | 0.02 |
| RETENTION BASIN CALCULAT | | 0.20 | | - | 1,225 | | 0.02 |
| Elevation | Delta Depth | Surface Area | | | Volume Prov | hahir | |
| Lievation | (ft) | (ft²) | | (ft ³) | Σ (ft ³) | (Ac-ft) | Σ (Ac-ft) |
| 1279.8 | 1.0 | 2,661 | | 2,111 | 2,111 | 0.05 | 0.05 |
| 1278.8 | 0.0 | 1,606 | | 0_ | 0 | 0.00 | 0.00 |
| 1278.8 | | | | | 2,111 | | 0.05 |
| | 7 | | Provided | | 2,111 | | 0.05 |
| | | | Required | _ | 1,225 | | 0.02 |
| | Basin Depth | | Balance | - | 886 | | 0.03 |
| | 0.50 | | | - 2 | *************************************** | | |

0.58

| Contributing Drainage Areas: Basin: | D | | | | | | |
|--|---|---|----------------------|--|---|-------------------------|---|
| VOLUME REQUIRED CALCUL | | rea | 'C' Coefficient | Precipitation | Poto | ntion Pos | uirod |
| Time | (ft) | (Ac) | C | (Inches) | Retention Requ (ft ³) | | (Ac-ft) |
| Type Residential | 6,750 | 0.15 | C | (inches) | 932 | | 0.02 |
| Residential | 0,730 | 0.13 | | - | 332 | | 0.02 |
| RETENTION BASIN CALCULA | | · | | | | | |
| Elevation | Delta Depth | Surface Area | | | Volume Prov | ided | |
| | (ft) | (ft ²) | | (ft ³) | Σ (ft ³) | (Ac-ft) | Σ (Ac-ft) |
| 1279.8 | 1.3 | 1,750 | | 1,421 | 1,421 | 0.03 | 0.03 |
| 1278.5 | 0.0 | 549 | | 0_ | 0 | 0.00 | 0.00 |
| 1278.5 | | 0 | | | 1,421 | | 0.03 |
| | | | Provided | | 1,421 | | 0.03 |
| | | | Required | | 932 | | 0.02 |
| | | | | The same of the sa | | | |
| | Basin Depth | | Balance | | 489 | | 0.01 |
| Contributing Drainage Areas: | Basin Depth 0.85 | | Balance | - | 489 | | 0.01 |
| Basin: | 0.85 | | | | | | |
| Basin: VOLUME REQUIRED CALCUL | E ATIONS | rea | 'C' Coefficient | Precipitation | Rete | ntion Req | uired |
| Basin: VOLUME REQUIRED CALCUL Type | E ATIONS A (ft) | (Ac) | | Precipitation (Inches) | Rete (ft³) | ntion Req | uired (Ac-ft) |
| VOLUME REQUIRED CALCUL Type Residential | 0.85 E ATIONS (ft) 6,990 | | 'C' Coefficient | | Rete | ntion Req | uired |
| Basin: VOLUME REQUIRED CALCUL Type Residential | 0.85 E ATIONS (ft) 6,990 ATIONS | (Ac) 0.16 | 'C' Coefficient | | Rete (ft³) | | uired (Ac-ft) |
| Basin: VOLUME REQUIRED CALCUL Type Residential RETENTION BASIN CALCULA | 0.85 E ATIONS (ft) 6,990 | (Ac) | 'C' Coefficient | | Rete (ft³) 965 | ided | uired (Ac-ft) 0.02 |
| Basin: VOLUME REQUIRED CALCUL Type Residential RETENTION BASIN CALCULA | E ATIONS A (ft) 6,990 ATIONS Delta Depth | (Ac) 0.16 Surface Area (ft²) | 'C' Coefficient | (Inches) | Rete (ft³) 965 Volume Prov | | uired (Ac-ft) |
| Basin: VOLUME REQUIRED CALCUL Type Residential RETENTION BASIN CALCULA Elevation | E ATIONS A (ft) 6,990 ATIONS Delta Depth (ft) | O.16 Surface Area (ft²) 1,748 | 'C' Coefficient | (Inches) | Rete (ft^3) 965 Volume Prov $\Sigma (ft^3)$ | ided (Ac-ft) | uired (Ac-ft) 0.02 Σ (Ac-ft) |
| Basin: VOLUME REQUIRED CALCUL Type Residential RETENTION BASIN CALCULA Elevation 1280.4 | 0.85 E ATIONS (ft) 6,990 ATIONS Delta Depth (ft) 1.3 | O.16 Surface Area (ft²) 1,748 | 'C' Coefficient | (ft ³) | Rete (ft^3) 965 Volume Prov $\Sigma (ft^3)$ 1,481 | ided (Ac-ft) 0.03 | uired (Ac-ft) 0.02 Σ (Ac-ft) 0.03 0.00 |
| Basin: VOLUME REQUIRED CALCUL Type Residential RETENTION BASIN CALCULA Elevation 1280.4 1279.1 | 0.85 E ATIONS (ft) 6,990 ATIONS Delta Depth (ft) 1.3 | (Ac) 0.16 Surface Area (ft²) 1,748 625 | 'C' Coefficient | (ft ³) | Rete (ft³) 965 Volume Prov Σ (ft³) 1,481 0 | ided (Ac-ft) 0.03 | uired (Ac-ft) 0.02 Σ (Ac-ft) 0.03 0.00 |
| Basin: VOLUME REQUIRED CALCUL Type Residential RETENTION BASIN CALCULA Elevation 1280.4 1279.1 | 0.85 E ATIONS (ft) 6,990 ATIONS Delta Depth (ft) 1.3 | (Ac) 0.16 Surface Area (ft²) 1,748 625 | 'C' Coefficient C | (ft ³) | Rete (ft³) 965 Volume Prov Σ (ft³) 1,481 0 1,481 | ided (Ac-ft) 0.03 | uired (Ac-ft) 0.02 Σ (Ac-ft) 0.03 0.03 0.03 |
| Basin: VOLUME REQUIRED CALCUL Type Residential RETENTION BASIN CALCULA Elevation 1280.4 1279.1 | 0.85 E ATIONS (ft) 6,990 ATIONS Delta Depth (ft) 1.3 | (Ac) 0.16 Surface Area (ft²) 1,748 625 | 'C' Coefficient C | (ft ³) | Rete (ft³) 965 Volume Prov Σ (ft³) 1,481 0 1,481 | ided (Ac-ft) 0.03 | uired (Ac-ft) 0.02 Σ (Ac-ft) 0.03 0.00 |

DEVELOPER/OWNER

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SCOTTSDALE, AZ 85250

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LEGAL DESCRIPTION

COMMENDING AT THE NORTHWEST CORNER OF THE NORTHEAST QUARTER OF SAID SECTION 14:

HEMIC SOUTH BY DEDREES SO MANUES 34 SECONDS EAS! A DISTANCE OF 13/23 TET. HEMIC ECONDS MEST, S WANTES S SECONDS MEST, S SECONDS MEST, A DISTANCE OF 12/25 FET! SECONDS MEST, A DISTANCE OF 700 TO FET MEST OF AND PARALLEL TO THE WEST LINE OF 300 MORTHEAST DIAMETER, A DISTANCE OF 700 TO FET.

THENCE WEST A DISTANCE OF 12.30 FEET, THENCE NORTH A DISTANCE OF 77.01 FEET. THENCE SOUTH 89 DEGREES 58 WINUTES 34 SECONDS EAST, A DISTANCE OF 16.30 FEET TO THE POINT OF BECRAPING, AND

TOGETHER WITH AN EASEMENT FOR INGRESS AND EGRESS OVER THE EAST 16 FEET OF THE SOUTHEAST QUARTER OF THE SOUTHEAST QUARTER OF THE MORPHEAST QUARTER OF THE OWNERST QUARTER OF SECTION 1, TOWNSHOP 2 MORTH, RANCE 4 EAST OF THE CLE AND SALT BURCH BASE AND MERDIAN, WARDOWN ACOUNTY, ARZONA, EXCEPT THE SOUTH 77.01 FEET, AND

TOGETHER WITH AN EASEMENT FOR INCRESS AND ECRESS OVER THE EAST 16 FEET OF THE NORTHEAST QUARTER OF THE SOUTHEAST QUARTER OF THE NORTHEAST QUARTER OF THE OFFICENT AND THE OF SECTION 1, TOWNSHIP 2 NORTH, RANCE 4 EAST OF THE CILA AND SALT RIVER BASE AND MERIDIAN, MARCOPA COUNTY, ARZONA, AND

THAT PORTION OF THE NORTHEAST QUARTER OF SECTION 14, TOWNSHIP 2 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER BASE AND MERIDIAN. MARICOPA COUNTY, ARIZONA, DESCRIBED AS FOLLOWS:

BEDINNING AT THE NORTHEAST CORNER OF THE LAND DESCRIBED IN THE DEED RECORDED IN DOCKET 10821, PAGE 112 OF OFFICIAL RECORDS IN THE OFFICE OF THE COUNTY RECORDER OF SAID COUNTY;

THENCE SOUTH OF DEGREES 20 MINUTES 13 SECONDS WEST (SOUTH OF DEGREES 49 MINUTES 15 SECONDS WEST RECORD) ALONG THE EAST LINE OF SAID LAND, A DISTANCE OF 287.21 FEET;

THENCE SOUTH 89 DEGREES 56 MINUTES 55 SECONDS EAST, A DISTANCE OF 172.77 FEET TO THE WEST RIGHT-OF-WAY LINE OF THE ARIZONA CANAL.

THEMEE NORTH OB DEGREES 49 MINUTES OB SECONDS EAST ALONG SAD WEST RIGHT-OF-WAY LINE, A DISTANCE OF 288 34 FEET TO THE LASTERLY PROLONGATION OF THE NORTH LINE OF SAID LAND DESCRIBED IN DOCKET 10821, PAGE 1122.

THENCE SOUTH 89 DEGREES 32 MINUTES 24 SECONDS WEST (NORTH 89 DEGREES 58 MINUTES 34 SECONDS WEST RECORD) ALONG SAID EASTERLY PROLONGATION, A DISTANCE OF 170.38 FEET 10 THE POINT OF BEGINNING

LEGEND



CONCEPTUAL GRADING & DRAINAGE PLAN FOR 5713 N. CATTLETRACK ROAD CITY OF SCOTTSDALE, ARIZONA

APN 173-03-0075

PRIVATE STREET

920



PROPOSED PEDESTRIAN GATE

SIDEWALK

-BASIN A Vr=6.653 CF Vp=10,734 CF HWE=1277.38 BTM=1276.0

for 1279.9±

TOP 1279.4±

TOP 1279.5±

CANAL

x TOP 1280.0±

TOP 1280.0±

TOP 1280.2±

WALL OPENINGS

DRYWELL A

EXIST. FLOODPLAIN LIMITS

- BASIN_D Vr=932 CF Vp=1,421 CF HWE=1279.35 BTM=1278.5

60

PROP. FLOODPLAIN LIMIT -LOT 3 FF=1280.47

G PRIVADO VILLAGE
BOOK 202, PAGE 31, MCR



VICINITY MAP

RETENTION AND SITE DATA

GROSS AREA: 82,522 SF / 1.89 AC
NCT AREA: 71,107 SF / 1.62 AC
COMPENSATORY RETENTION REQUIRED: 8,020 CF / 0.18 AC-FT
COMPENSATORY RETENTION PROVINCE: 15,500 CF / 0.36 AC-FT
100 YR-2 R. RETENTION PROVINCE: 10,402 CF / 0.25 AC-FT
100 YR-2 HR. RETENTION PROVINCE: 17,7812 CF / 0.41 AC-FT

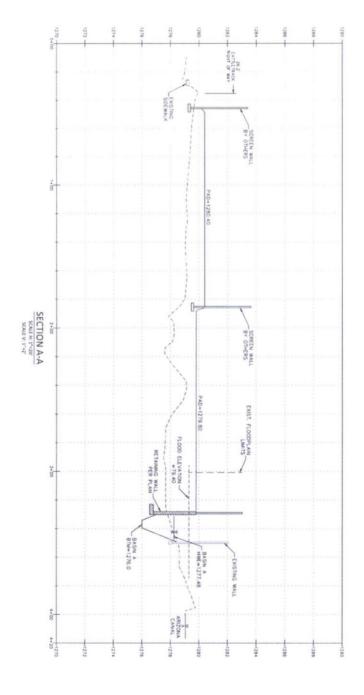
CONCEPT GRADING & DRAINAGE 5713 N. CATTLETRACK ROAD SCOTTSDALE, ARIZONA

SECTION: 14 TWNSHP: 2N RANGE: 4E

J08 NO.: TRUH0001 SHEET 1 OF 2









CONCEPT GRADING & DRAINAGE 5713 N. CATTLETRACK ROAD SCOTTSDALE, ARIZONA





| Т | DRAWN BY: | | | |
|---|------------------|------|----------|----|
| | | | | |
| | DESIGN BY | | | |
| | | | | |
| | CHECKED BY | | | |
| | | 1 | | |
| | DATE: 06-29-2016 | DATE | REVISION | BY |

