

***PRELIMINARY DRAINAGE REPORT***

**K. Hovanian Great West Homes, LLC  
Scottsdale, Arizona**

Case No. \_\_\_\_\_

**Prepared For:**

*K. Hovanian Great West Homes, LLC*

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Review Cycle \_\_\_\_\_ Date *5/10/16*

*Approved*

191948001  
April 2016  
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**Kimley»Horn**

**9-PP-2015  
04/22/16**

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Scottsdale, Arizona**

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***K. Hovanian Great West Homes, LLC***

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April 2016  
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- E Exhibits
- F Request for Stormwater Storage Waiver

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- Figure 1: Context Aerial Plan
- Figure 2: Preliminary Grading and Drainage Plan
- Figure 3: Existing Conditions Map Parcel F
- Figure 4: Developed Condition Map Parcel F
- Figure 5: Highest Adjacent Grade (HAG) Exhibit
- Figure 6: Offsite Watershed Exhibit

## 1.0 Introduction

### 1.1 Project Description

K. Hovnanian Great Western Homes is proposing to construct a single-family housing development on Parcel F of Silverstone at Pinnacle Peak, which is located at the southeast corner of Scottsdale Road and Silverstone Drive. The project is anticipated to consist of a single-family housing development and associated site infrastructure, including drive aisles, parking areas, and underground utilities.

### 1.2 Site Location

The proposed development encompasses approximately 22.1± net acres in a portion of the Northwest Quarter of Section 14, Township 4 North, Range 4 East of the Gila and Salt River Meridian in Maricopa County, Arizona. The parcel is bounded by Silverstone Drive to the south, 74<sup>th</sup> Street to the east, Rawhide Wash channel to the west, and Pinnacle Peak Road to the north. See Appendix A for the site location map and legal description. See Figure 1 in Appendix F for a Context Aerial Map.

The site is located at the former “Rawhide” western theme park area. Based on site observations, the structures associated with the theme park have been demolished, however, grading features including a raised race track, local drainage ditches, and unpaved roadways are present on the site.

### 1.3 Purpose

This Preliminary Drainage Report is intended to satisfy City of Scottsdale requirements and demonstrate conformance to the overall drainage characteristics of the area. This report provides a description of the current storm water drainage patterns and systems and a description of the required and proposed drainage improvements.

### 1.4 Objectives

This report provides a drainage plan for the subject site that is intended to meet the drainage standards and guidelines of the City of Scottsdale and the Flood Control District of Maricopa County (FCDMC). In particular, this report will demonstrate the following:

1. Buildings and site amenities will be elevated such that regional storm water flows from Rawhide Wash do not adversely impact the development.
2. Permanent drainage facilities will have a positive outfall, and any detained storm water will be disposed of within 36 hours.
3. Drainage facilities will be designed such that the 100-year post-development flows are collected and conveyed in such a manner so as to not cause damage to buildings and property.
4. Storm water detention is provided for the difference between the pre-development and post-development storm water volume.

5. Building finished floor elevations are in compliance with FEMA and City of Scottsdale requirements.
6. The proposed drainage plan for the site will be in general conformance to the Master Drainage Report for Silverstone, prepared by Wood, Patel & Associates, March 2007 and subsequent revisions by Wood, Patel & Associates and Kimley-Horn and Associates, Inc.

## 2.0 Description of Existing Drainage Conditions and Characteristics

### 2.1 Existing Drainage Conditions

The site currently consists of vacant land. The site is a portion of the previously demolished "Rawhide" western theme park. The general topography of the site slopes from the northeast to the southwest at approximately 2%. However, grading features including several small on-site drainage ditches, and unpaved access roads remain from the theme park use. Desert vegetation exists throughout the site.

Three on-site sub-basins drain to the south and exit the site at three locations along the southern property line. These sub-basins and corresponding 100-year discharges are shown on Figure 3 Existing Conditions Map in Appendix F.

### 2.2 Existing Off-Site Drainage Conditions

The Rawhide Wash regional drainage channel improvements were recently completed to the west of the project site. These improvements consist of a 300 foot wide, eight foot deep drainage channel that is designed to convey a peak flow of the Rawhide Wash from the Pinnacle Peak Road Bridge to the north of the site to the Scottsdale Road Bridge to the west of the site. This channel discharges storm water onto open desert on the west side of Scottsdale Road where it continues as overland flow to the southwest.

It is our understanding that a Letter of Map Revision (LOMR) has been submitted to FEMA in conjunction with the Rawhide Wash regional drainage improvements. This LOMR would remove significant areas surrounding Rawhide Wash from Flood Zone AO. At the time of this report, the LOMR is under review by FEMA and it is not anticipated that this review will be completed prior to the construction of the proposed development.

Based on the Letter of Map Revision Technical Data Notebook (TDN), prepared by Wood, Patel & Associates in May 2014, the Rawhide Wash outfall on the west side of Scottsdale Road is currently in an interim condition. This interim condition prevents the 100-year design flow of Rawhide Wash from passing under the Scottsdale Road Bridge and onto the open land on the west side of Scottsdale Road. As a result of this restriction, a portion of the design flow overtops Scottsdale Road and Silverstone Drive. Although this backwater and overtopping affects other parcels within Silverstone, adjacent to Parcel F the TDN shows the Rawhide Wash contained in the channel. Refer to Appendix E for the LOMR Technical Data Notebook.

The adjacent portions of Silverstone Drive and 74<sup>th</sup> Street consist of paved roadways with curb and gutters and storm drain infrastructure. Storm water from Silverstone Drive is collected in catch basins and conveyed to the Rawhide Wash channel. Storm water from 74<sup>th</sup> Street is conveyed via surface flow between the curbs before being collected in catch basins north of its intersection with Silverstone drive.

The adjacent portion of Pinnacle Peak Road to the north consists of a paved roadway with curb and gutter and a median. Runoff in Pinnacle Peak Road is collected in catch basins and conveyed via storm drain into the Rawhide Wash. Drainage channels paralleling both the north and south sides of the roadway, convey offsite runoff from the north into the Rawhide Wash, preventing it from reaching the site.

The land to the south of the site consists of the Appaloosa Library. Silverstone Drive separates the project site from the Library.

Based on the adjacent site conditions and the LOMR TDN, the only offsite runoff impacting the proposed development is the Rawhide Wash. Adjacent to this project, the Rawhide Wash is contained in the channel. Proposed structures shall be elevated above the Base Flood Elevations shown in the LOMR TDN. Refer to Figure 3 in Appendix F for the Existing Conditions Topographic Map.

### **2.3 Context Relative to Adjacent Projects and Improvements**

The improvements to the Rawhide Wash channel, Scottsdale Road, and Silverstone Drive have recently been completed. See Figure 1 in Appendix F for the Context Aerial Plan.

### **2.4 FEMA Flood Hazard Areas**

The site is located in Flood Zone "AO" according to the Flood Insurance Rate Map 04013C1310L, dated October 16, 2013. Zone "AO" is designated by FEMA as "areas of flood depths of 1 foot (usually sheet flow on sloping terrain) average depths determined for areas of alluvial fan flooding, velocities also determined." Refer to Appendix B for the FEMA FIRMette map for the site. An elevation certificate will be required for each building constructed in this flood hazard designation. Approval of the LOMR will remove the proposed development from the FEMA Special Flood Hazard Area.



## 3.0 Proposed Drainage Plan

### 3.1 General Description

In the analysis of the proposed drainage conditions the following items are considered:

- Area Types (pavement, building, and desert landscaping)
- Magnitude of areas
- Slopes
- Storm Drain
- Detention Basins

### 3.2 Proposed Site Conditions

Site-generated storm water will be collected in multiple catch basins and conveyed to one underground storage basin and four surface detention basins via underground storm drain pipes and street flow.

Of the detention basins, four are interconnected and upon reaching the last detention basin the outflow discharges via a weir into the Rawhide Wash channel near the southwest corner of the site. The only detention basin that doesn't discharge into Rawhide Wash is Basin 3A and it discharges east into the stormdrain system in 74<sup>th</sup> Street. Basin 1A is the underground storage basin and consists of 10' CMP chambers providing an equivalent storage of 0.63 ac-feet. Refer to Exhibit 4 – Proposed Conditions Map for proposed sub-basins and detention basin locations.

The proposed site was modeled hydrologically in two fashions: 1) as a contributing sub-basin to the Rawhide Wash during a 24-hour rainfall event; 2) as an isolated site using the Rational Method. HEC-1 was used to perform the sub-basin and detention basin routing analysis. The Rawhide Wash models, both with and without the proposed site, are included in the Addendum 3 to the Master Drainage Report (MDR) for Silverstone. The routing analysis for the site, by itself, using Rational Method and synthetic unit hydrographs is included in Appendix C.

As stated in the Addendum 3 to the MDR, the proposed drainage concept does not increase flows in the Rawhide Wash during the 100-year event. Flow in the Rawhide Wash remains at 9865 cfs after development of the site. Since the adjacent Rawhide channel is designed for a 100-year event of 10,900 cfs (Patel 2006) and the Rawhide Wash Model results in a discharge less than that, it is demonstrated that the channel meets the criteria of providing sufficient capacity for the site and therefore the 2-year and 10-year events were not included in the Rawhide Wash Model.

The on-site sub-basins were modeled using the Rational Method to confirm that the detention basins could handle the sharper peaking and more localized storm. DDMSW was used to generate Rational Method results and output hydrographs that could be imported into HEC-1 for routing purposes. For the lone detention basin that does not drain to the Rawhide Wash, Basin 3A, an outflow of 1 cfs will leave the basin during the 100-Year event. The existing discharge at that same point is approximately 6 cfs. The “Site Only” HEC-1 model is located in Appendix C.

Only one proposed Sub-basin discharges unregulated from the site and that is Sub-Basin On-3 which discharges to 74<sup>th</sup> Street. Table 1 shows a comparison of discharge reduction at detention basins.

Table 1: 100-Yr Peak Discharge Reduction from Detention Basins

Detention Basin	24-Hr Storm		Rational Method	
	Inflow (cfs)	Outflow (cfs)	Inflow (cfs)	Outflow (cfs)
Basin 1A	5	1	13	1
Basin 1B	22	22	23	18
Basin 2A	40	34	38	6
Basin 2B	50	45	26	21
Basin 3A	*	*	12	1

\* not included in Rawhide Wash Routing Analysis

Of the 45 cfs that leaves Detention Basin 2B during the 100-year, 24hr event, 41 cfs weirs into Rawhide Wash Channel and 4 cfs flows through a bleeder orifice into the park area. This was modeled in HEC-1 as a diversion.

### 3.3 Proposed Off-Site Conditions

No changes to the off-site conditions are proposed with these improvements. Storm water from adjacent portions of Silverstone Drive and 74<sup>th</sup> Street will continue to be collected in catch basins and conveyed to regional drainage facilities.

Administratively this site will remain in the FEMA floodplain and comply with AO zone requirements; however the Rawhide Wash Channel has been demonstrated to contain the 100-year flow event.

### 3.4 Future Conditions

The roadways to the north and east of the site are in their built-out conditions; therefore, these areas are not anticipated to have any future impacts to the project site. The area to the south is undeveloped, and any future developments in this area will be responsible for on-site management of storm water runoff.

It is also anticipated that at some point in the future the Rawhide Wash outfall will be completed to its final condition. At this point, off-site flows would no longer be anticipated to affect the site. The completion of this outfall would involve coordination between the City of Scottsdale, the City of Phoenix, the Arizona State Land Department,

### 3.5 Storm Water Storage Requirements

According to Addendum 2 to the Master Drainage Report for Silverstone, approved in April 2015, the required detention volume for the parcel is approximately 147,000 cubic feet. This volume contemplates detention for the 100-year, 2-hour rainfall event associated with the NOAA Atlas 14 rainfall depths. Due to a portion of the site's previous development (9 ac), storm water storage will be provided for the difference between the pre-development and post-development storm water runoff volume on that portion and full detention for the remaining 13 acres. An addendum to the Master Drainage Report that reflects this change will be submitted along with this Preliminary Drainage Report for the proposed development.

Table 2 below summarizes the contributing area, runoff coefficient for the contributing area, and the required and provided retention volumes.

Table 2: Storm Water Volume Required

Proposed Storage Volume Summary - Silverstone Parcel F						
Drainage Area	Area [A]		Runoff Coefficient [C]	Precipitation Depth [P]	Required Storage ( $V_{REQ} = CPA/12$ )	
	sf	ac			cf	ac-ft
Roadway	207,028	4.753	0.88	2.30	34,919	0.80
Pavement	21,399	0.491	0.95	2.30	3,896	0.09
Landscaping	232,504	5.338	0.45	2.30	20,053	0.46
Building	156,852	3.601	0.95	2.30	28,560	0.66
Duplex	343,201	7.879	0.81	2.30	53,282	1.22
<b>TOTAL</b>	<b>960,984</b>	<b>22.061</b>	<b>0.76</b>	-	<b>140,711</b>	<b>3.23</b>
Existing Storage Volume Summary - Silverstone Parcel F						
Drainage Area	Area [A]		Runoff Coefficient [C]	Precipitation Depth [P]	Required Storage ( $V_{REQ} = CPA/12$ )	
	sf	ac			cf	ac-ft
Roof	10,940	0.250	0.95	2.30	1,992	0.05
Disturbed Desert	379,843	8.720	0.55	2.30	40,042	0.92
Undist Desert (no credit)	569,765	13.090	0.00	2.30	0	0.00
<b>TOTAL</b>	<b>960,548</b>	<b>22.061</b>			<b>42,034</b>	<b>(0.96)</b>
					<b>Total Required</b>	<b>2.27</b>

Table 3: Stormwater Storage Functional Volume Provided

Detention Basin	Peak Volume
	AC-FT
Basin 1A/1C	0.61
Basin 1B	0.19
Basin 2A	0.93
Basin 2B	0.21
Basin 3A	0.35
<b>Total</b>	<b>2.29</b>

The storage volume summary reveals that 2.27 ac-ft of peak storage will be required to comply with pre versus post runoff volume storage requirements. Table 3, shows actual peak volume from the On-site HEC-1 model and provides slightly more than the required volume.

Refer to Figures 2 and 4 in Appendix E for the Preliminary Grading and Drainage Plan and the Developed Conditions Map, respectively. Refer to Appendix C for the Hydrologic/Hydraulic Calculations.

**3.6 Pre- and Post-Development Runoff Characteristics at Concentration Points**

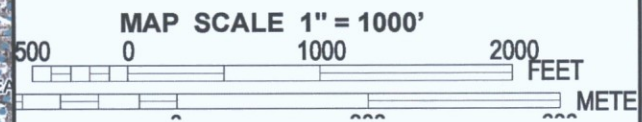
The existing site consists of approximately 22.1 acres of vacant land that drains from the northeast to the southwest. The existing drainage patterns have three concentration points on the south perimeter of the site, reference Figure 3. In the pre-developed condition approximately 50 cfs combined exits the site on the south and enters the park.

The post-development flow at the south property line will be approximately 5 cfs combined. The bulk of the post development flow will discharge into the Rawhide Wash on the west where it presently does not; however, due to the difference in times of concentration of the Rawhide Wash and the site, the drainage from the site has no impact on the peak of the Rawhide Wash.

**3.7 Proposed Drainage Structures or Special Drainage Facilities**

Storm water quality will be maintained by elevating the outfall headwalls 3-6 inches above the basin bottom. This will allow the first flush of storm water to remain in the basin, where it will be disposed of via natural percolation. Orifice plates or pipe sizing will be used to meter the storm water outflow; sizing will be included with the Final Drainage Report.

Basin 1A (underground storage) will have a 4” orifice plate rather than the standard 6” orifice plate provided in the surface basins. This was verbally agreed to by City of Scottsdale.



NFIP

PANEL 1310L

NATIONAL FLOOD INSURANCE PROGRAM

**FIRM**  
**FLOOD INSURANCE RATE MAP**  
**MARICOPA COUNTY,**  
**ARIZONA**  
**AND INCORPORATED AREAS**

**PANEL 1310 OF 4425**  
 (SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
MARICOPA COUNTY	040037	1310	L
PHOENIX, CITY OF	040051	1310	L
SCOTTSDALE, CITY OF	045012	1310	L

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.



**MAP NUMBER**  
**04013C1310L**  
**MAP REVISED**  
**OCTOBER 16, 2013**

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date of the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at [www.fema.gov](http://www.fema.gov)

*Appendix C*

Hydrologic/Hydraulic Calculations

**Silverstone Parcel F**

**100 Year 2 Hour Full Retention Calculation**

Drainage Area	Area [A]		Runoff Coefficient [C]	Precipitation Depth [P] in	Required Storage (V <sub>REQ</sub> = CPA/12)	
	sf	ac			cf	ac-ft
	Roadway	207,028			4.753	0.88
Pavement	21,399	0.491	0.95	2.30	3,896	0.09
Landscaping	232,504	5.338	0.45	2.30	20,053	0.46
Building	156,852	3.601	0.95	2.30	28,560	0.66
Duplex	343,201	7.879	0.81	2.30	53,282	1.22
<b>TOTAL</b>	<b>960,984</b>	<b>22.061</b>	<b>0.76</b>	<b>-</b>	<b>140,711</b>	<b>3.23</b>

**Retention Credit for Disturbed Areas**

Drainage Area	Area [A]		Runoff Coefficient [C]	Precipitation Depth [P] in	Required Storage (V <sub>REQ</sub> = CPA/12)	
	sf	ac			cf	ac-ft
	Roof	10,940			0.250	0.95
Disturbed Desert	379,843	8.720	0.55	2.30	40,042	0.92
Undisturbed Desert (no credit)	569,765	13.080	0.00	2.30	0	0.00
<b>TOTAL</b>	<b>960,548</b>	<b>22.051</b>			<b>42,034</b>	<b>0.96</b>

**Pre Vs. Post Retention Calculations**

Drainage Area	Required Storage (V <sub>REQ</sub> )	
	cf	ac-ft
Post	140,711	3.2
Pre	42,034	1.0
Required	98,677	2.27

**Silverstone Weighted C Average**

46' Roadway Tract			
Area	C		
Pavement	40	0.95	
Landscape	6	0.45	
<b>Weighted Average</b>	<b>46</b>	<b>0.88</b>	

Duplex Lots			
Area	C		
Building	6962	0.95	
Landscape	2786.5	0.45	
<b>Weighted Average</b>	<b>9748.5</b>	<b>0.81</b>	

Flood Control District of Maricopa County  
 Drainage Design Management System  
 SUB BASINS  
 Project Reference: SILVERSTONE F RATION

ID	Sub Basin Data						Sub Basin Hydrology Summary						
	Area (acres)	Length (ft)	USGE	DSGE	Slope (ft/mi)	Kb	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	
<b>Major Basin ID: ON</b>													
ON01	2.7	503	1,870.30	1,863.00	76.6	0.037	Q (cfs)	4.1	5.5	6.6	8.8	10.9	13.0
							C	0.62	0.62	0.62	0.68	0.74	0.78
							CA (ac)	1.64	1.64	1.64	1.80	1.96	2.07
							Tc (min)	10	10	10	10	10	10
							i (in/hr)	2.48	3.35	4.01	4.90	5.57	6.26
ON02	4.6	1,029	1,870.10	1,859.20	55.9	0.036	Q (cfs)	6.9	9.6	11.5	15.4	19.0	22.5
							C	0.62	0.62	0.62	0.68	0.74	0.78
							CA (ac)	2.86	2.86	2.86	3.14	3.42	3.60
							Tc (min)	11	10	10	10	10	10
							i (in/hr)	2.40	3.35	4.01	4.90	5.57	6.26
ON03	0.7	78	1,870.10	1,860.50	649.8	0.041	Q (cfs)	1.0	1.4	1.7	2.3	2.8	3.3
							C	0.62	0.62	0.62	0.68	0.74	0.78
							CA (ac)	0.42	0.42	0.42	0.46	0.50	0.53
							Tc (min)	10	10	10	10	10	10
							i (in/hr)	2.48	3.35	4.01	4.90	5.57	6.26
ON04	5.5	897	1,860.00	1,847.00	76.5	0.035	Q (cfs)	8.5	11.5	13.7	18.4	22.7	26.9
							C	0.62	0.62	0.62	0.68	0.74	0.78
							CA (ac)	3.42	3.42	3.42	3.75	4.08	4.30
							Tc (min)	10	10	10	10	10	10
							i (in/hr)	2.48	3.35	4.01	4.90	5.57	6.26
ON05	2.5	428	1,848.20	1,841.30	85.1	0.038	Q (cfs)	3.8	5.1	6.1	8.2	10.2	12.1
							C	0.62	0.62	0.62	0.68	0.74	0.78
							CA (ac)	1.53	1.53	1.53	1.68	1.83	1.93
							Tc (min)	10	10	10	10	10	10
							i (in/hr)	2.48	3.35	4.01	4.90	5.57	6.26
ON06	2.8	509	1,854.70	1,839.90	153.5	0.037	Q (cfs)	4.3	5.8	7.0	9.4	11.6	13.7
							C	0.62	0.62	0.62	0.68	0.74	0.78
							CA (ac)	1.74	1.74	1.74	1.91	2.08	2.19

\* Non default value



Flood Control District of Maricopa County  
 Drainage Design Management System  
 SUB BASINS  
 Project Reference: SILVERSTONE F RATION

ID	Sub Basin Data						Sub Basin Hydrology Summary					
	Area (acres)	Length (ft)	USGE	DSGE	Slope (ft/mi)	Kb	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
<b>Major Basin ID: ON</b>												
							Tc (min)	10	10	10	10	10
							i (in/hr)	2.48	3.35	4.01	4.90	5.57
ON07	2.5	665	1,855.10	1,837.00	143.7	0.038	Q (cfs)	3.8	5.2	6.2	8.3	10.2
							C	0.62	0.62	0.62	0.68	0.74
							CA (ac)	1.54	1.54	1.54	1.69	1.84
							Tc (min)	10	10	10	10	10
							i (in/hr)	2.48	3.35	4.01	4.90	5.57

\* Non default value

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 19APR16 TIME 10:18:42 *
*****
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* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104

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X X X X X XX
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X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Flood Control District of Maricopa County
2 ID SILVERSTONE F - Silverstone F onsite
3 ID 100 YEAR
4 ID Rational Method
5 ID Synthetic Unit Hydrograph
6 ID 11/30/2015
*DIAGRAM
7 IT 2 1JAN99 0 750
8 IO 2
9 IN 5
*
10 KK ON01 BASIN
11 BA 0.004
12 QI 0 6.5 13.0 9.1 5.2 1.3 0
*
13 KK RET1A
14 KM UG RETENTION BASIN 1A
15 RS 1 STOR 0
16 SV 0 0.03 0.16 0.32 0.47 0.60 0.63
17 SE 0 1 3 5 7 9 10
18 SS 9.9 1 2.3 1.5
19 SL 0.17 0.0871 0.6 0.5
*
20 KK ON02 BASIN
21 BA 0.007
22 QI 0 11.3 22.5 15.8 9.0 2.3 0
*
23 KK 1BIN
24 KM INFLOW TO RETENTION BASIN 1B
25 HC 2
*
26 KK RET1B
27 KM RETENTION BASIN 1B
28 RS 1 STOR 0
29 SV 0 0.037 0.105 0.205
30 SE 0 1 2 3
31 SS 2.3 15 2.3 1.5
32 SL 1.25 0.1963 0.6 0.5
*
33 KK ON04 BASIN
34 BA 0.009

```

35 QI 0 13.5 26.9 18.8 10.8 2.7 0  
\*

1

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

36 KK 2AIN
37 KM INFLOW TO RETENTION BASIN 2A
38 HC 2
*
39 KK RET2A
40 KM RETENTION BASIN 2A
41 RS 1 STOR 0
42 SV 0 0.339 0.678 1.017
43 SE 0 1 2 3
44 SS 2.6 50 2.3 1.5
45 SL 1.25 0.1963 0.6 0.5
*
46 KK ON05 BASIN
47 BA 0.004
48 QI 0 6.1 12.1 8.5 4.9 1.2 0
*
49 KK ON06 BASIN
50 BA 0.004
51 QI 0 6.9 13.7 9.6 5.5 1.4 0
*
52 KK 2BIN
53 KM INFLOW TO RETENTION BASIN 2B
54 HC 3
*
55 KK RET2B
56 KM RETENTION BASIN 2B
57 RS 1 STOR 0
58 SV 0 0.05 0.125 0.229
59 SE 0 1 2 3
60 SS 2.4 40 2.3 1.5
61 SL 0.25 0.393 0.6 0.5
*
62 KK ON07 BASIN
63 BA 0.004
64 QI 0 6.1 12.1 8.5 4.9 1.2 0
*
65 KK RET3A
66 KM RETENTION BASIN 3A
67 RS 1 STOR 0
68 SV 0 0.096 0.216 0.364
69 SE 0 1 2 3
70 SS 2.9 10 2.3 1.5
71 SL 1.25 0.1963 0.6 0.5
*
72 ZZ

```

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

```

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW
10 ON01
V
V
13 RET1A
.
.
20 . ON02
.
.
23 1BIN.....
V
V
26 RET1B
.
.
33 . ON04
.
.
36 2AIN.....
V
V
39 RET2A
.
.
46 . ON05
.
.
49 . ON06
.

```

```

52      2BIN.....
      V
      V
55      RET2B
      .
      .
62      .          ON07
      .          V
      .          V
65      .          RET3A

```

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
*****
*
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*
*   JUN 1998 *
*
*   VERSION 4.1 *
*
*
* RUN DATE 19APR16 TIME 10:18:42 *
*
*
*****
*****

```

```

*
*
* U.S. ARMY CORPS OF ENGINEERS
*
* HYDROLOGIC ENGINEERING CENTER
*
*   609 SECOND STREET
*
*   DAVIS, CALIFORNIA 95616
*
*   (916) 756-1104
*
*

```

Flood Control District of Maricopa County  
SILVERSTONE F - Silverstone F Onsite  
100 YEAR  
Rational Method  
Synthetic Unit Hydrograph  
11/30/2015

```

8 IO      OUTPUT CONTROL VARIABLES
          IPRNT      2  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE

```

```

IT        HYDROGRAPH TIME DATA
          NMIN      2  MINUTES IN COMPUTATION INTERVAL
          IDATE     1JAN99  STARTING DATE
          ITIME     0000  STARTING TIME
          NQ        750  NUMBER OF HYDROGRAPH ORDINATES
          NDDATE    2JAN99  ENDING DATE
          NDTIME    0058  ENDING TIME
          ICENT     19  CENTURY MARK

```

```

          COMPUTATION INTERVAL .03 HOURS
          TOTAL TIME BASE     24.97 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH  INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME     ACRE-Feet
SURFACE AREA       ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

\*\*\* \*\*

```

*****
*
*
10 KK    *   ON01   *   BASIN
*
*
*****

```

```

9 IN      TIME DATA FOR INPUT TIME SERIES
          JXMIN     5  TIME INTERVAL IN MINUTES
          JKDATE    1JAN99  STARTING DATE
          JXTIME    0  STARTING TIME

```

SUBBASIN RUNOFF DATA

```

11 BA     SUBBASIN CHARACTERISTICS
          TAREA     .00  SUBBASIN AREA

```

\*\*\*

HYDROGRAPH AT STATION ON01

DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*
1	JAN	0000	1	0.	*	1	JAN	0616	189	1.	*	1	JAN	1232	377	1.	*	1	JAN	1848	565	1.	*
1	JAN	0002	2	3.	*	1	JAN	0618	190	1.	*	1	JAN	1234	378	1.	*	1	JAN	1850	566	1.	*
1	JAN	0004	3	5.	*	1	JAN	0620	191	1.	*	1	JAN	1236	379	1.	*	1	JAN	1852	567	1.	*
1	JAN	0006	4	8.	*	1	JAN	0622	192	1.	*	1	JAN	1238	380	1.	*	1	JAN	1854	568	1.	*
1	JAN	0008	5	10.	*	1	JAN	0624	193	1.	*	1	JAN	1240	381	1.	*	1	JAN	1856	569	1.	*
1	JAN	0010	6	13.	*	1	JAN	0626	194	1.	*	1	JAN	1242	382	1.	*	1	JAN	1858	570	1.	*
1	JAN	0012	7	11.	*	1	JAN	0628	195	1.	*	1	JAN	1244	383	1.	*	1	JAN	1900	571	1.	*
1	JAN	0014	8	10.	*	1	JAN	0630	196	1.	*	1	JAN	1246	384	1.	*	1	JAN	1902	572	1.	*
1	JAN	0016	9	8.	*	1	JAN	0632	197	1.	*	1	JAN	1248	385	1.	*	1	JAN	1904	573	1.	*
1	JAN	0018	10	7.	*	1	JAN	0634	198	1.	*	1	JAN	1250	386	1.	*	1	JAN	1906	574	1.	*
1	JAN	0020	11	5.	*	1	JAN	0636	199	1.	*	1	JAN	1252	387	1.	*	1	JAN	1908	575	1.	*
1	JAN	0022	12	4.	*	1	JAN	0638	200	1.	*	1	JAN	1254	388	1.	*	1	JAN	1910	576	1.	*
1	JAN	0024	13	2.	*	1	JAN	0640	201	1.	*	1	JAN	1256	389	1.	*	1	JAN	1912	577	1.	*
1	JAN	0026	14	1.	*	1	JAN	0642	202	1.	*	1	JAN	1258	390	1.	*	1	JAN	1914	578	1.	*
1	JAN	0028	15	1.	*	1	JAN	0644	203	1.	*	1	JAN	1300	391	1.	*	1	JAN	1916	579	1.	*
1	JAN	0030	16	1.	*	1	JAN	0646	204	1.	*	1	JAN	1302	392	1.	*	1	JAN	1918	580	1.	*
1	JAN	0032	17	1.	*	1	JAN	0648	205	1.	*	1	JAN	1304	393	1.	*	1	JAN	1920	581	1.	*
1	JAN	0034	18	1.	*	1	JAN	0650	206	1.	*	1	JAN	1306	394	1.	*	1	JAN	1922	582	1.	*
1	JAN	0036	19	1.	*	1	JAN	0652	207	1.	*	1	JAN	1308	395	1.	*	1	JAN	1924	583	1.	*
1	JAN	0038	20	1.	*	1	JAN	0654	208	1.	*	1	JAN	1310	396	1.	*	1	JAN	1926	584	1.	*
1	JAN	0040	21	1.	*	1	JAN	0656	209	1.	*	1	JAN	1312	397	1.	*	1	JAN	1928	585	1.	*
1	JAN	0042	22	1.	*	1	JAN	0658	210	1.	*	1	JAN	1314	398	1.	*	1	JAN	1930	586	1.	*
1	JAN	0044	23	1.	*	1	JAN	0700	211	1.	*	1	JAN	1316	399	1.	*	1	JAN	1932	587	1.	*
1	JAN	0046	24	1.	*	1	JAN	0702	212	1.	*	1	JAN	1318	400	1.	*	1	JAN	1934	588	1.	*
1	JAN	0048	25	1.	*	1	JAN	0704	213	1.	*	1	JAN	1320	401	1.	*	1	JAN	1936	589	1.	*
1	JAN	0050	26	1.	*	1	JAN	0706	214	1.	*	1	JAN	1322	402	1.	*	1	JAN	1938	590	1.	*
1	JAN	0052	27	1.	*	1	JAN	0708	215	1.	*	1	JAN	1324	403	1.	*	1	JAN	1940	591	1.	*
1	JAN	0054	28	1.	*	1	JAN	0710	216	1.	*	1	JAN	1326	404	1.	*	1	JAN	1942	592	1.	*
1	JAN	0056	29	1.	*	1	JAN	0712	217	1.	*	1	JAN	1328	405	1.	*	1	JAN	1944	593	1.	*
1	JAN	0058	30	1.	*	1	JAN	0714	218	1.	*	1	JAN	1330	406	1.	*	1	JAN	1946	594	1.	*
1	JAN	0100	31	1.	*	1	JAN	0716	219	1.	*	1	JAN	1332	407	1.	*	1	JAN	1948	595	1.	*
1	JAN	0102	32	1.	*	1	JAN	0718	220	1.	*	1	JAN	1334	408	1.	*	1	JAN	1950	596	1.	*
1	JAN	0104	33	1.	*	1	JAN	0720	221	1.	*	1	JAN	1336	409	1.	*	1	JAN	1952	597	1.	*
1	JAN	0106	34	1.	*	1	JAN	0722	222	1.	*	1	JAN	1338	410	1.	*	1	JAN	1954	598	1.	*
1	JAN	0108	35	1.	*	1	JAN	0724	223	1.	*	1	JAN	1340	411	1.	*	1	JAN	1956	599	1.	*
1	JAN	0110	36	1.	*	1	JAN	0726	224	1.	*	1	JAN	1342	412	1.	*	1	JAN	1958	600	1.	*
1	JAN	0112	37	1.	*	1	JAN	0728	225	1.	*	1	JAN	1344	413	1.	*	1	JAN	2000	601	1.	*
1	JAN	0114	38	1.	*	1	JAN	0730	226	1.	*	1	JAN	1346	414	1.	*	1	JAN	2002	602	1.	*
1	JAN	0116	39	1.	*	1	JAN	0732	227	1.	*	1	JAN	1348	415	1.	*	1	JAN	2004	603	1.	*
1	JAN	0118	40	1.	*	1	JAN	0734	228	1.	*	1	JAN	1350	416	1.	*	1	JAN	2006	604	1.	*
1	JAN	0120	41	1.	*	1	JAN	0736	229	1.	*	1	JAN	1352	417	1.	*	1	JAN	2008	605	1.	*
1	JAN	0122	42	1.	*	1	JAN	0738	230	1.	*	1	JAN	1354	418	1.	*	1	JAN	2010	606	1.	*
1	JAN	0124	43	1.	*	1	JAN	0740	231	1.	*	1	JAN	1356	419	1.	*	1	JAN	2012	607	1.	*
1	JAN	0126	44	1.	*	1	JAN	0742	232	1.	*	1	JAN	1358	420	1.	*	1	JAN	2014	608	1.	*
1	JAN	0128	45	1.	*	1	JAN	0744	233	1.	*	1	JAN	1400	421	1.	*	1	JAN	2016	609	1.	*
1	JAN	0130	46	1.	*	1	JAN	0746	234	1.	*	1	JAN	1402	422	1.	*	1	JAN	2018	610	1.	*
1	JAN	0132	47	1.	*	1	JAN	0748	235	1.	*	1	JAN	1404	423	1.	*	1	JAN	2020	611	1.	*
1	JAN	0134	48	1.	*	1	JAN	0750	236	1.	*	1	JAN	1406	424	1.	*	1	JAN	2022	612	1.	*
1	JAN	0136	49	1.	*	1	JAN	0752	237	1.	*	1	JAN	1408	425	1.	*	1	JAN	2024	613	1.	*
1	JAN	0138	50	1.	*	1	JAN	0754	238	1.	*	1	JAN	1410	426	1.	*	1	JAN	2026	614	1.	*
1	JAN	0140	51	1.	*	1	JAN	0756	239	1.	*	1	JAN	1412	427	1.	*	1	JAN	2028	615	1.	*
1	JAN	0142	52	1.	*	1	JAN	0758	240	1.	*	1	JAN	1414	428	1.	*	1	JAN	2030	616	1.	*
1	JAN	0144	53	1.	*	1	JAN	0800	241	1.	*	1	JAN	1416	429	1.	*	1	JAN	2032	617	1.	*
1	JAN	0146	54	1.	*	1	JAN	0802	242	1.	*	1	JAN	1418	430	1.	*	1	JAN	2034	618	1.	*
1	JAN	0148	55	1.	*	1	JAN	0804	243	1.	*	1	JAN	1420	431	1.	*	1	JAN	2036	619	1.	*
1	JAN	0150	56	1.	*	1	JAN	0806	244	1.	*	1	JAN	1422	432	1.	*	1	JAN	2038	620	1.	*
1	JAN	0152	57	1.	*	1	JAN	0808	245	1.	*	1	JAN	1424	433	1.	*	1	JAN	2040	621	1.	*
1	JAN	0154	58	1.	*	1	JAN	0810	246	1.	*	1	JAN	1426	434	1.	*	1	JAN	2042	622	1.	*
1	JAN	0156	59	1.	*	1	JAN	0812	247	1.	*	1	JAN	1428	435	1.	*	1	JAN	2044	623	1.	*
1	JAN	0158	60	1.	*	1	JAN	0814	248	1.	*	1	JAN	1430	436	1.	*	1	JAN	2046	624	1.	*
1	JAN	0200	61	1.	*	1	JAN	0816	249	1.	*	1	JAN	1432	437	1.	*	1	JAN	2048	625	1.	*
1	JAN	0202	62	1.	*	1	JAN	0818	250	1.	*	1	JAN	1434	438	1.	*	1	JAN	2050	626	1.	*
1	JAN	0204	63	1.	*	1	JAN	0820	251	1.	*	1	JAN	1436	439	1.	*	1	JAN	2052	627	1.	*
1	JAN	0206	64	1.	*	1	JAN	0822	252	1.	*	1	JAN	1438	440	1.	*	1	JAN	2054	628	1.	*
1	JAN	0208	65	1.	*	1	JAN	0824	253	1.	*	1	JAN	1440	441	1.	*	1	JAN	2056	629	1.	*
1	JAN	0210	66	1.	*	1	JAN	0826	254	1.	*	1	JAN	1442	442	1.	*	1	JAN	2058	630	1.	*
1	JAN	0212	67	1.	*	1	JAN	0828	255	1.	*	1	JAN	1444	443	1.	*	1	JAN	2100	631	1.	*
1	JAN	0214	68	1.	*	1	JAN	0830	256	1.	*	1	JAN	1446	444	1.	*	1	JAN	2102	632	1.	*
1	JAN	0216	69	1.	*	1	JAN	0832	257	1.	*	1	JAN	1448	445	1.	*	1	JAN	2104	633	1.	*
1	JAN	0218	70	1.	*	1	JAN	0834	258	1.	*	1	JAN	1450	446	1.	*	1	JAN	2106	634	1.	*
1	JAN	0220	71	1.	*	1	JAN	0836	259	1.	*	1	JAN	1452	447	1.	*	1	JAN	2108	635	1.	*
1	JAN	0222	72	1.	*	1	JAN	0838	260	1.	*	1	JAN	1454	448	1.	*	1	JAN	2110	636	1.	*
1	JAN	0224	73	1.	*	1	JAN	0840	261	1.	*	1	JAN	1456	449	1.	*	1	JAN	2112	637	1.	*
1	JAN	0226	74	1.	*	1	JAN	0842	262	1.	*	1	JAN	1458	450	1.	*	1	JAN	2114	638	1.	*
1	JAN	0228	75	1.	*	1	JAN	0844	263	1.	*	1	JAN	1500	451	1.	*	1	JAN	2116	639	1.	*
1	JAN	0230	76	1.	*	1	JAN	0846	264	1.	*	1	JAN	1502	452	1.	*	1	JAN	2118	640</		



1 JAN 0556	179	1.	*	1 JAN 1212	367	1.	*	1 JAN 1828	555	1.	*	2 JAN 0044	743	1.
1 JAN 0558	180	1.	*	1 JAN 1214	368	1.	*	1 JAN 1830	556	1.	*	2 JAN 0046	744	1.
1 JAN 0600	181	1.	*	1 JAN 1216	369	1.	*	1 JAN 1832	557	1.	*	2 JAN 0048	745	1.
1 JAN 0602	182	1.	*	1 JAN 1218	370	1.	*	1 JAN 1834	558	1.	*	2 JAN 0050	746	1.
1 JAN 0604	183	1.	*	1 JAN 1220	371	1.	*	1 JAN 1836	559	1.	*	2 JAN 0052	747	1.
1 JAN 0606	184	1.	*	1 JAN 1222	372	1.	*	1 JAN 1838	560	1.	*	2 JAN 0054	748	1.
1 JAN 0608	185	1.	*	1 JAN 1224	373	1.	*	1 JAN 1840	561	1.	*	2 JAN 0056	749	1.
1 JAN 0610	186	1.	*	1 JAN 1226	374	1.	*	1 JAN 1842	562	1.	*	2 JAN 0058	750	1.
1 JAN 0612	187	1.	*	1 JAN 1228	375	1.	*	1 JAN 1844	563	1.	*			
1 JAN 0614	188	1.	*	1 JAN 1230	376	1.	*	1 JAN 1846	564	1.	*			

\*\*\*\*\*

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.97-HR
13.	.17	2.	3.927	12.992	13.478	13.478
		(INCHES)	1.	3.	3.	3.
		(AC-FT)				
CUMULATIVE AREA =			.00 SQ MI			

\*\*\*\*\*

13 KK \*\*\*\*\*  
 \* RET1A \*  
 \*\*\*\*\*  
 UG RETENTION BASIN 1A

HYDROGRAPH ROUTING DATA

15 RS	STORAGE ROUTING								
	NSTPS	1	NUMBER OF SUBREACHES						
	ITYP	STOR	TYPE OF INITIAL CONDITION						
	RSVVIC	.00	INITIAL CONDITION						
	X	.00	WORKING R AND D COEFFICIENT						
16 SV	STORAGE	.0	.0	.2	.3	.5	.6	.6	
17 SE	ELEVATION	.00	1.00	3.00	5.00	7.00	9.00	10.00	
19 SL	LOW-LEVEL OUTLET								
	ELEVL	.17	ELEVATION AT CENTER OF OUTLET						
	CAREA	.09	CROSS-SECTIONAL AREA						
	COQL	.60	COEFFICIENT						
	EXPL	.50	EXPONENT OF HEAD						
18 SS	SPILLWAY								
	CREL	9.90	SPILLWAY CREST ELEVATION						
	SPWID	1.00	SPILLWAY WIDTH						
	COQW	2.30	WEIR COEFFICIENT						
	EXPW	1.50	EXPONENT OF HEAD						

\*\*\*

COMPUTED OUTFLOW-ELEVATION DATA

OUTFLOW	.00	.00	.18	.20	.23	.28	.35	.46	.68	1.31
ELEVATION	.00	.17	.35	.40	.48	.62	.86	1.39	2.83	9.90
OUTFLOW	1.31	1.31	1.31	1.31	1.32	1.33	1.34	1.35	1.37	1.39
ELEVATION	9.90	9.91	9.91	9.92	9.93	9.94	9.95	9.97	9.98	10.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.01	.01	.01	.01	.02	.03	.03	.06	.15
OUTFLOW	.00	.00	.18	.20	.23	.28	.35	.46	.68	1.31
ELEVATION	.00	.17	.35	.40	.48	.62	.86	1.00	1.39	2.83
STORAGE	.16	.32	.47	.60	.63	.63	.63	.63	.63	.63
OUTFLOW	.71	.92	1.10	1.25	1.31	1.31	1.32	1.33	1.34	1.35
ELEVATION	3.00	5.00	7.00	9.00	9.90	9.91	9.93	9.94	9.95	9.97
STORAGE	.63	.63								
OUTFLOW	1.37	1.39								
ELEVATION	9.98	10.00								

\*\*\*\*\*

HYDROGRAPH AT STATION RET1A

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE
1	JAN	0000	1	0.	.0	.0	*	1	JAN	0820	251	1.	.4	6.7	*	1	JAN	1640	501	1.	.6
8.3																					
1	JAN	0002	2	0.	.0	.1	*	1	JAN	0822	252	1.	.4	6.7	*	1	JAN	1642	502	1.	.6
8.3																					
1	JAN	0004	3	0.	.0	.5	*	1	JAN	0824	253	1.	.4	6.7	*	1	JAN	1644	503	1.	.6
8.3																					
1	JAN	0006	4	0.	.0	1.0	*	1	JAN	0826	254	1.	.4	6.7	*	1	JAN	1646	504	1.	.6
8.4																					
1	JAN	0008	5	0.	.1	1.4	*	1	JAN	0828	255	1.	.5	6.7	*	1	JAN	1648	505	1.	.6
8.4																					
1	JAN	0010	6	1.	.1	1.9	*	1	JAN	0830	256	1.	.5	6.7	*	1	JAN	1650	506	1.	.6
8.4																					
1	JAN	0012	7	1.	.1	2.4	*	1	JAN	0832	257	1.	.5	6.8	*	1	JAN	1652	507	1.	.6
8.4																					
1	JAN	0014	8	1.	.1	2.8	*	1	JAN	0834	258	1.	.5	6.8	*	1	JAN	1654	508	1.	.6
8.4																					
1	JAN	0016	9	1.	.2	3.1	*	1	JAN	0836	259	1.	.5	6.8	*	1	JAN	1656	509	1.	.6
8.4																					
1	JAN	0018	10	1.	.2	3.3	*	1	JAN	0838	260	1.	.5	6.8	*	1	JAN	1658	510	1.	.6
8.4																					
1	JAN	0020	11	1.	.2	3.5	*	1	JAN	0840	261	1.	.5	6.8	*	1	JAN	1700	511	1.	.6
8.4																					
1	JAN	0022	12	1.	.2	3.6	*	1	JAN	0842	262	1.	.5	6.8	*	1	JAN	1702	512	1.	.6
8.4																					
1	JAN	0024	13	1.	.2	3.7	*	1	JAN	0844	263	1.	.5	6.8	*	1	JAN	1704	513	1.	.6
8.4																					
1	JAN	0026	14	1.	.2	3.8	*	1	JAN	0846	264	1.	.5	6.8	*	1	JAN	1706	514	1.	.6
8.4																					
1	JAN	0028	15	1.	.2	3.8	*	1	JAN	0848	265	1.	.5	6.8	*	1	JAN	1708	515	1.	.6
8.4																					
1	JAN	0030	16	1.	.2	3.8	*	1	JAN	0850	266	1.	.5	6.8	*	1	JAN	1710	516	1.	.6
8.4																					
1	JAN	0032	17	1.	.2	3.8	*	1	JAN	0852	267	1.	.5	6.8	*	1	JAN	1712	517	1.	.6
8.4																					
1	JAN	0034	18	1.	.2	3.8	*	1	JAN	0854	268	1.	.5	6.8	*	1	JAN	1714	518	1.	.6
8.4																					
1	JAN	0036	19	1.	.2	3.8	*	1	JAN	0856	269	1.	.5	6.8	*	1	JAN	1716	519	1.	.6
8.4																					
1	JAN	0038	20	1.	.2	3.9	*	1	JAN	0858	270	1.	.5	6.9	*	1	JAN	1718	520	1.	.6
8.4																					
1	JAN	0040	21	1.	.2	3.9	*	1	JAN	0900	271	1.	.5	6.9	*	1	JAN	1720	521	1.	.6
8.4																					
1	JAN	0042	22	1.	.2	3.9	*	1	JAN	0902	272	1.	.5	6.9	*	1	JAN	1722	522	1.	.6
8.4																					
1	JAN	0044	23	1.	.2	3.9	*	1	JAN	0904	273	1.	.5	6.9	*	1	JAN	1724	523	1.	.6
8.4																					
1	JAN	0046	24	1.	.2	3.9	*	1	JAN	0906	274	1.	.5	6.9	*	1	JAN	1726	524	1.	.6
8.4																					
1	JAN	0048	25	1.	.2	3.9	*	1	JAN	0908	275	1.	.5	6.9	*	1	JAN	1728	525	1.	.6
8.4																					
1	JAN	0050	26	1.	.2	4.0	*	1	JAN	0910	276	1.	.5	6.9	*	1	JAN	1730	526	1.	.6
8.4																					
1	JAN	0052	27	1.	.2	4.0	*	1	JAN	0912	277	1.	.5	6.9	*	1	JAN	1732	527	1.	.6
8.5																					
1	JAN	0054	28	1.	.2	4.0	*	1	JAN	0914	278	1.	.5	6.9	*	1	JAN	1734	528	1.	.6
8.5																					
1	JAN	0056	29	1.	.2	4.0	*	1	JAN	0916	279	1.	.5	6.9	*	1	JAN	1736	529	1.	.6
8.5																					
1	JAN	0058	30	1.	.2	4.0	*	1	JAN	0918	280	1.	.5	6.9	*	1	JAN	1738	530	1.	.6
8.5																					
1	JAN	0100	31	1.	.2	4.0	*	1	JAN	0920	281	1.	.5	6.9	*	1	JAN	1740	531	1.	.6
8.5																					
1	JAN	0102	32	1.	.2	4.1	*	1	JAN	0922	282	1.	.5	7.0	*	1	JAN	1742	532	1.	.6
8.5																					
1	JAN	0104	33	1.	.2	4.1	*	1	JAN	0924	283	1.	.5	7.0	*	1	JAN	1744	533	1.	.6
8.5																					
1	JAN	0106	34	1.	.2	4.1	*	1	JAN	0926	284	1.	.5	7.0	*	1	JAN	1746	534	1.	.6
8.5																					
1	JAN	0108	35	1.	.2	4.1	*	1	JAN	0928	285	1.	.5	7.0	*	1	JAN	1748	535	1.	.6
8.5																					
1	JAN	0110	36	1.	.3	4.1	*	1	JAN	0930	286	1.	.5	7.0	*	1	JAN	1750	536	1.	.6
8.5																					
1	JAN	0112	37	1.	.3	4.1	*	1	JAN	0932	287	1.	.5	7.0	*	1	JAN	1752	537	1.	.6
8.5																					
1	JAN	0114	38	1.	.3	4.2	*	1	JAN	0934	288	1.	.5	7.0	*	1	JAN	1754	538	1.	.6
8.5																					
1	JAN	0116	39	1.	.3	4.2	*	1	JAN	0936	289	1.	.5	7.0	*	1	JAN	1756	539	1.	.6
8.5																					
1	JAN	0118	40	1.	.3	4.2	*	1	JAN	0938	290	1.	.5	7.0	*	1	JAN	1758	540	1.	.6
8.5																					
1	JAN	0120	41	1.	.3	4.2	*	1	JAN	0940	291	1.	.5	7.0	*	1	JAN	1800	541	1.	.6
8.5																					
1	JAN	0122	42	1.	.3																



8.5	1 JAN 0128	45	1.	.3	4.3 *	1 JAN 0948 295	1.	.5	7.1 *	1 JAN 1808 545	1.	.6
8.5	1 JAN 0130	46	1.	.3	4.3 *	1 JAN 0950 296	1.	.5	7.1 *	1 JAN 1810 546	1.	.6
8.5	1 JAN 0132	47	1.	.3	4.3 *	1 JAN 0952 297	1.	.5	7.1 *	1 JAN 1812 547	1.	.6
8.5	1 JAN 0134	48	1.	.3	4.3 *	1 JAN 0954 298	1.	.5	7.1 *	1 JAN 1814 548	1.	.6
8.5	1 JAN 0136	49	1.	.3	4.3 *	1 JAN 0956 299	1.	.5	7.1 *	1 JAN 1816 549	1.	.6
8.5	1 JAN 0138	50	1.	.3	4.3 *	1 JAN 0958 300	1.	.5	7.1 *	1 JAN 1818 550	1.	.6
8.5	1 JAN 0140	51	1.	.3	4.4 *	1 JAN 1000 301	1.	.5	7.1 *	1 JAN 1820 551	1.	.6
8.5	1 JAN 0142	52	1.	.3	4.4 *	1 JAN 1002 302	1.	.5	7.1 *	1 JAN 1822 552	1.	.6
8.6	1 JAN 0144	53	1.	.3	4.4 *	1 JAN 1004 303	1.	.5	7.1 *	1 JAN 1824 553	1.	.6
8.6	1 JAN 0146	54	1.	.3	4.4 *	1 JAN 1006 304	1.	.5	7.1 *	1 JAN 1826 554	1.	.6
8.6	1 JAN 0148	55	1.	.3	4.4 *	1 JAN 1008 305	1.	.5	7.1 *	1 JAN 1828 555	1.	.6
8.6	1 JAN 0150	56	1.	.3	4.4 *	1 JAN 1010 306	1.	.5	7.1 *	1 JAN 1830 556	1.	.6
8.6	1 JAN 0152	57	1.	.3	4.5 *	1 JAN 1012 307	1.	.5	7.2 *	1 JAN 1832 557	1.	.6
8.6	1 JAN 0154	58	1.	.3	4.5 *	1 JAN 1014 308	1.	.5	7.2 *	1 JAN 1834 558	1.	.6
8.6	1 JAN 0156	59	1.	.3	4.5 *	1 JAN 1016 309	1.	.5	7.2 *	1 JAN 1836 559	1.	.6
8.6	1 JAN 0158	60	1.	.3	4.5 *	1 JAN 1018 310	1.	.5	7.2 *	1 JAN 1838 560	1.	.6
8.6	1 JAN 0200	61	1.	.3	4.5 *	1 JAN 1020 311	1.	.5	7.2 *	1 JAN 1840 561	1.	.6
8.6	1 JAN 0202	62	1.	.3	4.5 *	1 JAN 1022 312	1.	.5	7.2 *	1 JAN 1842 562	1.	.6
8.6	1 JAN 0204	63	1.	.3	4.5 *	1 JAN 1024 313	1.	.5	7.2 *	1 JAN 1844 563	1.	.6
8.6	1 JAN 0206	64	1.	.3	4.6 *	1 JAN 1026 314	1.	.5	7.2 *	1 JAN 1846 564	1.	.6
8.6	1 JAN 0208	65	1.	.3	4.6 *	1 JAN 1028 315	1.	.5	7.2 *	1 JAN 1848 565	1.	.6
8.6	1 JAN 0210	66	1.	.3	4.6 *	1 JAN 1030 316	1.	.5	7.2 *	1 JAN 1850 566	1.	.6
8.6	1 JAN 0212	67	1.	.3	4.6 *	1 JAN 1032 317	1.	.5	7.2 *	1 JAN 1852 567	1.	.6
8.6	1 JAN 0214	68	1.	.3	4.6 *	1 JAN 1034 318	1.	.5	7.2 *	1 JAN 1854 568	1.	.6
8.6	1 JAN 0216	69	1.	.3	4.6 *	1 JAN 1036 319	1.	.5	7.3 *	1 JAN 1856 569	1.	.6
8.6	1 JAN 0218	70	1.	.3	4.6 *	1 JAN 1038 320	1.	.5	7.3 *	1 JAN 1858 570	1.	.6
8.6	1 JAN 0220	71	1.	.3	4.7 *	1 JAN 1040 321	1.	.5	7.3 *	1 JAN 1900 571	1.	.6
8.6	1 JAN 0222	72	1.	.3	4.7 *	1 JAN 1042 322	1.	.5	7.3 *	1 JAN 1902 572	1.	.6
8.6	1 JAN 0224	73	1.	.3	4.7 *	1 JAN 1044 323	1.	.5	7.3 *	1 JAN 1904 573	1.	.6
8.6	1 JAN 0226	74	1.	.3	4.7 *	1 JAN 1046 324	1.	.5	7.3 *	1 JAN 1906 574	1.	.6
8.6	1 JAN 0228	75	1.	.3	4.7 *	1 JAN 1048 325	1.	.5	7.3 *	1 JAN 1908 575	1.	.6
8.6	1 JAN 0230	76	1.	.3	4.7 *	1 JAN 1050 326	1.	.5	7.3 *	1 JAN 1910 576	1.	.6
8.6	1 JAN 0232	77	1.	.3	4.7 *	1 JAN 1052 327	1.	.5	7.3 *	1 JAN 1912 577	1.	.6
8.6	1 JAN 0234	78	1.	.3	4.8 *	1 JAN 1054 328	1.	.5	7.3 *	1 JAN 1914 578	1.	.6
8.6	1 JAN 0236	79	1.	.3	4.8 *	1 JAN 1056 329	1.	.5	7.3 *	1 JAN 1916 579	1.	.6
8.6	1 JAN 0238	80	1.	.3	4.8 *	1 JAN 1058 330	1.	.5	7.3 *	1 JAN 1918 580	1.	.6
8.7	1 JAN 0240	81	1.	.3	4.8 *	1 JAN 1100 331	1.	.5	7.3 *	1 JAN 1920 581	1.	.6
8.7	1 JAN 0242	82	1.	.3	4.8 *	1 JAN 1102 332	1.	.5	7.4 *	1 JAN 1922 582	1.	.6
8.7	1 JAN 0244	83	1.	.3	4.8 *	1 JAN 1104 333	1.	.5	7.4 *	1 JAN 1924 583	1.	.6
8.7	1 JAN 0246	84	1.	.3	4.8 *	1 JAN 1106 334	1.	.5	7.4 *	1 JAN 1926 584	1.	.6
8.7	1 JAN 0248	85	1.	.3	4.9 *	1 JAN 1108 335	1.	.5	7.4 *	1 JAN 1928 585	1.	.6
8.7	1 JAN 0250	86	1.	.3	4.9 *	1 JAN 1110 336	1.	.5	7.4 *	1 JAN 1930 586	1.	.6
8.7	1 JAN 0252	87	1.	.3	4.9 *	1 JAN 1112 337	1.	.5	7.4 *	1 JAN 1932 587	1.	.6
8.7	1 JAN 0254	88	1.	.3	4.9 *	1 JAN 1114 338	1.	.5	7.4 *	1 JAN 1934 588	1.	.6
8.7	1 JAN 0256	89	1.	.3	4.9 *	1 JAN 1116 339	1.	.5	7.4 *	1 JAN 1936 589	1.	.6
8.7	1 JAN 0258	90	1.	.3	4.9 *	1 JAN 1118 340	1.	.5	7.4 *	1 JAN 1938 590	1.	.6
8.7	1 JAN 0300	91	1.	.3	4.9 *	1 JAN 1120 341	1.	.5	7.4 *	1 JAN 1940 591	1.	.6

DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW			
1	JAN	0000	1	0.	*	1	JAN	0616	189	2.	*	1	JAN	1232	377	2.	*	1	JAN	1848	565	2.
1	JAN	0002	2	5.	*	1	JAN	0618	190	2.	*	1	JAN	1234	378	2.	*	1	JAN	1850	566	2.
1	JAN	0004	3	9.	*	1	JAN	0620	191	2.	*	1	JAN	1236	379	2.	*	1	JAN	1852	567	2.
1	JAN	0006	4	14.	*	1	JAN	0622	192	2.	*	1	JAN	1238	380	2.	*	1	JAN	1854	568	2.
1	JAN	0008	5	18.	*	1	JAN	0624	193	2.	*	1	JAN	1240	381	2.	*	1	JAN	1856	569	2.
1	JAN	0010	6	23.	*	1	JAN	0626	194	2.	*	1	JAN	1242	382	2.	*	1	JAN	1858	570	2.
1	JAN	0012	7	20.	*	1	JAN	0628	195	2.	*	1	JAN	1244	383	2.	*	1	JAN	1900	571	2.
1	JAN	0014	8	17.	*	1	JAN	0630	196	2.	*	1	JAN	1246	384	2.	*	1	JAN	1902	572	2.
1	JAN	0016	9	14.	*	1	JAN	0632	197	2.	*	1	JAN	1248	385	2.	*	1	JAN	1904	573	2.
1	JAN	0018	10	12.	*	1	JAN	0634	198	2.	*	1	JAN	1250	386	2.	*	1	JAN	1906	574	2.
1	JAN	0020	11	9.	*	1	JAN	0636	199	2.	*	1	JAN	1252	387	2.	*	1	JAN	1908	575	2.
1	JAN	0022	12	6.	*	1	JAN	0638	200	2.	*	1	JAN	1254	388	2.	*	1	JAN	1910	576	2.
1	JAN	0024	13	4.	*	1	JAN	0640	201	2.	*	1	JAN	1256	389	2.	*	1	JAN	1912	577	2.
1	JAN	0026	14	2.	*	1	JAN	0642	202	2.	*	1	JAN	1258	390	2.	*	1	JAN	1914	578	2.
1	JAN	0028	15	2.	*	1	JAN	0644	203	2.	*	1	JAN	1300	391	2.	*	1	JAN	1916	579	2.
1	JAN	0030	16	2.	*	1	JAN	0646	204	2.	*	1	JAN	1302	392	2.	*	1	JAN	1918	580	2.
1	JAN	0032	17	2.	*	1	JAN	0648	205	2.	*	1	JAN	1304	393	2.	*	1	JAN	1920	581	2.
1	JAN	0034	18	2.	*	1	JAN	0650	206	2.	*	1	JAN	1306	394	2.	*	1	JAN	1922	582	2.
1	JAN	0036	19	2.	*	1	JAN	0652	207	2.	*	1	JAN	1308	395	2.	*	1	JAN	1924	583	2.
1	JAN	0038	20	2.	*	1	JAN	0654	208	2.	*	1	JAN	1310	396	2.	*	1	JAN	1926	584	2.
1	JAN	0040	21	2.	*	1	JAN	0656	209	2.	*	1	JAN	1312	397	2.	*	1	JAN	1928	585	2.
1	JAN	0042	22	2.	*	1	JAN	0658	210	2.	*	1	JAN	1314	398	2.	*	1	JAN	1930	586	2.
1	JAN	0044	23	2.	*	1	JAN	0700	211	2.	*	1	JAN	1316	399	2.	*	1	JAN	1932	587	2.
1	JAN	0046	24	2.	*	1	JAN	0702	212	2.	*	1	JAN	1318	400	2.	*	1	JAN	1934	588	2.
1	JAN	0048	25	2.	*	1	JAN	0704	213	2.	*	1	JAN	1320	401	2.	*	1	JAN	1936	589	2.
1	JAN	0050	26	2.	*	1	JAN	0706	214	2.	*	1	JAN	1322	402	2.	*	1	JAN	1938	590	2.
1	JAN	0052	27	2.	*	1	JAN	0708	215	2.	*	1	JAN	1324	403	2.	*	1	JAN	1940	591	2.
1	JAN	0054	28	2.	*	1	JAN	0710	216	2.	*	1	JAN	1326	404	2.	*	1	JAN	1942	592	2.
1	JAN	0056	29	2.	*	1	JAN	0712	217	2.	*	1	JAN	1328	405	2.	*	1	JAN	1944	593	2.
1	JAN	0058	30	2.	*	1	JAN	0714	218	2.	*	1	JAN	1330	406	2.	*	1	JAN	1946	594	2.
1	JAN	0100	31	2.	*	1	JAN	0716	219	2.	*	1	JAN	1332	407	2.	*	1	JAN	1948	595	2.
1	JAN	0102	32	2.	*	1	JAN	0718	220	2.	*	1	JAN	1334	408	2.	*	1	JAN	1950	596	2.
1	JAN	0104	33	2.	*	1	JAN	0720	221	2.	*	1	JAN	1336	409	2.	*	1	JAN	1952	597	2.
1	JAN	0106	34	2.	*	1	JAN	0722	222	2.	*	1	JAN	1338	410	2.	*	1	JAN	1954	598	2.
1	JAN	0108	35	2.	*	1	JAN	0724	223	2.	*	1	JAN	1340	411	2.	*	1	JAN	1956	599	2.
1	JAN	0110	36	2.	*	1	JAN	0726	224	2.	*	1	JAN	1342	412	2.	*	1	JAN	1958	600	2.
1	JAN	0112	37	2.	*	1	JAN	0728	225	2.	*	1	JAN	1344	413	2.	*	1	JAN	2000	601	2.
1	JAN	0114	38	2.	*	1	JAN	0730	226	2.	*	1	JAN	1346	414	2.	*	1	JAN	2002	602	2.
1	JAN	0116	39	2.	*	1	JAN	0732	227	2.	*	1	JAN	1348	415	2.	*	1	JAN	2004	603	2.
1	JAN	0118	40	2.	*	1	JAN	0734	228	2.	*	1	JAN	1350	416	2.	*	1	JAN	2006	604	2.
1	JAN	0120	41	2.	*	1	JAN	0736	229	2.	*	1	JAN	1352	417	2.	*	1	JAN	2008	605	2.
1	JAN	0122	42	2.	*	1	JAN	0738	230	2.	*	1	JAN	1354	418	2.	*	1	JAN	2010	606	2.
1	JAN	0124	43	2.	*	1	JAN	0740	231	2.	*	1	JAN	1356	419	2.	*	1	JAN	2012	607	2.
1	JAN	0126	44	2.	*	1	JAN	0742	232	2.	*	1	JAN	1358	420	2.	*	1	JAN	2014	608	2.
1	JAN	0128	45	2.	*	1	JAN	0744	233	2.	*	1	JAN	1400	421	2.	*	1	JAN	2016	609	2.
1	JAN	0130	46	2.	*	1	JAN	0746	234	2.	*	1	JAN	1402	422	2.	*	1	JAN	2018	610	2.
1	JAN	0132	47	2.	*	1	JAN	0748	235	2.	*	1	JAN	1404	423	2.	*	1	JAN	2020	611	2.
1	JAN	0134	48	2.	*	1	JAN	0750	236	2.	*	1	JAN	1406	424	2.	*	1	JAN	2022	612	2.
1	JAN	0136	49	2.	*	1	JAN	0752	237	2.	*	1	JAN	1408	425	2.	*	1	JAN	2024	613	2.
1	JAN	0138	50	2.	*	1	JAN	0754	238	2.	*	1	JAN	1410	426	2.	*	1	JAN	2026	614	2.
1	JAN	0140	51	2.	*	1	JAN	0756	239	2.	*	1	JAN	1412	427	2.	*	1	JAN	2028	615	2.
1	JAN	0142	52	2.	*	1	JAN	0758	240	2.	*	1	JAN	1414	428	2.	*	1	JAN	2030	616	2.
1	JAN	0144	53	2.	*	1	JAN	0800	241	2.	*	1	JAN	1416	429	2.	*	1	JAN	2032	617	2.
1	JAN	0146	54	2.	*	1	JAN	0802	242	2.	*	1	JAN	1418	430	2.	*	1	JAN	2034	618	2.
1	JAN	0148	55	2.	*	1	JAN	0804	243	2.	*	1	JAN	1420	431	2.	*	1	JAN	2036	619	2.
1	JAN	0150	56	2.	*	1	JAN	0806	244	2.	*	1	JAN	1422	432	2.	*	1	JAN	2038	620	2.
1	JAN	0152	57	2.	*	1	JAN	0808	245	2.	*	1	JAN	1424	433	2.	*	1	JAN	2040	621	2.
1	JAN	0154	58	2.	*	1	JAN	0810	246	2.	*	1	JAN	1426	434	2.	*	1	JAN	2042	622	2.
1	JAN	0156	59	2.	*	1	JAN	0812	247	2.	*	1	JAN	1428	435	2.	*	1	JAN	2044	623	2.
1	JAN	0158	60	2.	*	1	JAN	0814	248	2.	*	1	JAN	1430	436	2.	*	1	JAN	2046	624	2.
1	JAN	0200	61	2.	*	1	JAN	0816	249	2.	*	1	JAN	1432	437	2.	*	1	JAN	2048	625	2.
1	JAN	0202	62	2.	*	1	JAN	0818	250	2.	*	1	JAN	1434	438	2.	*	1	JAN	2050	626	2.
1	JAN	0204	63	2.	*	1	JAN	0820	251	2.	*	1	JAN	1436	439	2.	*	1	JAN	2052	627	2.
1	JAN	0206	64	2.	*	1	JAN	0822	252	2.	*	1	JAN	1438	440	2.	*	1	JAN	2054	628	2.
1	JAN	0208	65	2.	*	1	JAN	0824	253	2.	*	1	JAN	1440	441	2.	*	1	JAN	2056	629	2.
1	JAN	0210	66	2.	*	1	JAN	0826	254	2.	*	1	JAN	1442	442	2.	*	1	JAN	2058	630	2.
1	JAN	0212	67	2.	*	1	JAN	0828	255	2.	*	1	JAN	1444	443	2.	*	1	JAN	2100	631	2.
1	JAN	0214	68	2.	*	1	JAN	0830	256	2.	*	1	JAN	1446	444	2.	*	1	JAN	2102	632	2.
1	JAN	0216	69	2.	*	1	JAN	0832	257	2.	*	1	JAN	1448	445	2.	*	1	JAN	2104	633	2.
1	JAN	0218	70	2.	*	1	JAN	0834	258	2.	*	1	JAN	1450	446	2.	*	1	JAN	2106	634	2.
1	JAN	0220	71	2.	*	1	JAN	0836	259	2.	*	1	JAN	1452	447	2.	*	1	JAN	2108	635	2.
1	JAN	0222	72	2.	*	1	JAN	0838	260	2.	*	1	JAN	1454	448	2.	*	1	JAN	2110	636	2.
1	JAN	0224	73	2.	*	1	JAN	0840	261	2.	*	1	JAN	1456	449	2.	*	1	JAN	2112	637	2.
1	JAN	0226	74	2.	*	1	JAN	0842	262	2.	*	1	JAN	1458	450	2.	*	1	JAN	2114	638	2.
1	JAN	0228	75	2.	*	1	JAN	0844	263	2.	*	1	JAN	1500	451	2.	*	1	JAN	2116	639	2.
1	JAN	0230	76	2.	*	1	JAN	0846	264	2.	*	1	JAN	1502	452	2.	*	1	JAN	2118	640	2.
1	JAN	0232	77	2.	*	1	JAN	0848	265	2.	*	1	JAN	1504	453	2.	*	1	JAN	2120	641	2.
1	JAN	0234	78	2.	*	1	JAN	0850	266	2.	*	1	JAN	1506	454	2.	*	1	JAN	2122	642	2.
1	JAN	0236	79	2.	*																	



1 JAN 0614 188 2. \* 1 JAN 1230 376 2. \* 1 JAN 1846 564 2. \*

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PEAK FLOW TIME MAXIMUM AVERAGE FLOW  
 + (CFS) (HR) 6-HR 24-HR 72-HR 24.97-HR  
 + 23. .17 (CFS) 3. 2. 2. 2.  
 (INCHES) 3.948 13.112 13.604 13.604  
 (AC-FT) 1. 5. 5. 5.  
 CUMULATIVE AREA = .01 SQ MI

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23 KK \*\*\*\*\*  
 \* 1BIN \*  
 \* \*\*\*\*\*

INFLOW TO RETENTION BASIN 1B

25 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION 1BIN  
 SUM OF 2 HYDROGRAPHS

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DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW
1	JAN	0000	1	0.	*	1	JAN	0616	189	3.	*	1	JAN	1232	377	3.	*	1	JAN	1848	565	4.
1	JAN	0002	2	5.	*	1	JAN	0618	190	3.	*	1	JAN	1234	378	3.	*	1	JAN	1850	566	4.
1	JAN	0004	3	9.	*	1	JAN	0620	191	3.	*	1	JAN	1236	379	3.	*	1	JAN	1852	567	4.
1	JAN	0006	4	14.	*	1	JAN	0622	192	3.	*	1	JAN	1238	380	3.	*	1	JAN	1854	568	4.
1	JAN	0008	5	18.	*	1	JAN	0624	193	3.	*	1	JAN	1240	381	3.	*	1	JAN	1856	569	4.
1	JAN	0010	6	23.	*	1	JAN	0626	194	3.	*	1	JAN	1242	382	3.	*	1	JAN	1858	570	4.
1	JAN	0012	7	20.	*	1	JAN	0628	195	3.	*	1	JAN	1244	383	3.	*	1	JAN	1900	571	4.
1	JAN	0014	8	18.	*	1	JAN	0630	196	3.	*	1	JAN	1246	384	3.	*	1	JAN	1902	572	4.
1	JAN	0016	9	15.	*	1	JAN	0632	197	3.	*	1	JAN	1248	385	3.	*	1	JAN	1904	573	4.
1	JAN	0018	10	12.	*	1	JAN	0634	198	3.	*	1	JAN	1250	386	3.	*	1	JAN	1906	574	4.
1	JAN	0020	11	10.	*	1	JAN	0636	199	3.	*	1	JAN	1252	387	3.	*	1	JAN	1908	575	4.
1	JAN	0022	12	7.	*	1	JAN	0638	200	3.	*	1	JAN	1254	388	3.	*	1	JAN	1910	576	4.
1	JAN	0024	13	4.	*	1	JAN	0640	201	3.	*	1	JAN	1256	389	3.	*	1	JAN	1912	577	4.
1	JAN	0026	14	3.	*	1	JAN	0642	202	3.	*	1	JAN	1258	390	3.	*	1	JAN	1914	578	4.
1	JAN	0028	15	3.	*	1	JAN	0644	203	3.	*	1	JAN	1300	391	3.	*	1	JAN	1916	579	4.
1	JAN	0030	16	3.	*	1	JAN	0646	204	3.	*	1	JAN	1302	392	3.	*	1	JAN	1918	580	4.
1	JAN	0032	17	3.	*	1	JAN	0648	205	3.	*	1	JAN	1304	393	3.	*	1	JAN	1920	581	4.
1	JAN	0034	18	3.	*	1	JAN	0650	206	3.	*	1	JAN	1306	394	3.	*	1	JAN	1922	582	4.
1	JAN	0036	19	3.	*	1	JAN	0652	207	3.	*	1	JAN	1308	395	3.	*	1	JAN	1924	583	4.
1	JAN	0038	20	3.	*	1	JAN	0654	208	3.	*	1	JAN	1310	396	3.	*	1	JAN	1926	584	4.
1	JAN	0040	21	3.	*	1	JAN	0656	209	3.	*	1	JAN	1312	397	3.	*	1	JAN	1928	585	4.
1	JAN	0042	22	3.	*	1	JAN	0658	210	3.	*	1	JAN	1314	398	3.	*	1	JAN	1930	586	4.
1	JAN	0044	23	3.	*	1	JAN	0700	211	3.	*	1	JAN	1316	399	3.	*	1	JAN	1932	587	4.
1	JAN	0046	24	3.	*	1	JAN	0702	212	3.	*	1	JAN	1318	400	3.	*	1	JAN	1934	588	4.
1	JAN	0048	25	3.	*	1	JAN	0704	213	3.	*	1	JAN	1320	401	3.	*	1	JAN	1936	589	4.
1	JAN	0050	26	3.	*	1	JAN	0706	214	3.	*	1	JAN	1322	402	3.	*	1	JAN	1938	590	4.
1	JAN	0052	27	3.	*	1	JAN	0708	215	3.	*	1	JAN	1324	403	3.	*	1	JAN	1940	591	4.
1	JAN	0054	28	3.	*	1	JAN	0710	216	3.	*	1	JAN	1326	404	3.	*	1	JAN	1942	592	4.
1	JAN	0056	29	3.	*	1	JAN	0712	217	3.	*	1	JAN	1328	405	3.	*	1	JAN	1944	593	4.
1	JAN	0058	30	3.	*	1	JAN	0714	218	3.	*	1	JAN	1330	406	3.	*	1	JAN	1946	594	4.
1	JAN	0100	31	3.	*	1	JAN	0716	219	3.	*	1	JAN	1332	407	3.	*	1	JAN	1948	595	4.
1	JAN	0102	32	3.	*	1	JAN	0718	220	3.	*	1	JAN	1334	408	3.	*	1	JAN	1950	596	4.
1	JAN	0104	33	3.	*	1	JAN	0720	221	3.	*	1	JAN	1336	409	3.	*	1	JAN	1952	597	4.
1	JAN	0106	34	3.	*	1	JAN	0722	222	3.	*	1	JAN	1338	410	3.	*	1	JAN	1954	598	4.
1	JAN	0108	35	3.	*	1	JAN	0724	223	3.	*	1	JAN	1340	411	3.	*	1	JAN	1956	599	4.
1	JAN	0110	36	3.	*	1	JAN	0726	224	3.	*	1	JAN	1342	412	3.	*	1	JAN	1958	600	4.
1	JAN	0112	37	3.	*	1	JAN	0728	225	3.	*	1	JAN	1344	413	3.	*	1	JAN	2000	601	4.
1	JAN	0114	38	3.	*	1	JAN	0730	226	3.	*	1	JAN	1346	414	3.	*	1	JAN	2002	602	4.
1	JAN	0116	39	3.	*	1	JAN	0732	227	3.	*	1	JAN	1348	415	3.	*	1	JAN	2004	603	4.
1	JAN	0118	40	3.	*	1	JAN	0734	228	3.	*	1	JAN	1350	416	3.	*	1	JAN	2006	604	4.
1	JAN	0120	41	3.	*	1	JAN	0736	229	3.	*	1	JAN	1352	417	3.	*	1	JAN	2008	605	4.
1	JAN	0122	42	3.	*	1	JAN	0738	230	3.	*	1	JAN	1354	418	3.	*	1	JAN	2010	606	4.
1	JAN	0124	43	3.	*	1	JAN	0740	231	3.	*	1	JAN	1356	419	3.	*	1	JAN	2012	607	4.
1	JAN	0126	44	3.	*	1	JAN	0742	232	3.	*	1	JAN	1358	420	3.	*	1	JAN	2014	608	4.
1	JAN	0128	45	3.	*	1	JAN	0744	233	3.	*	1	JAN	1400	421	3.	*	1	JAN	2016	609	4.
1	JAN	0130	46	3.	*	1	JAN	0746	234	3.	*	1	JAN	1402	422	3.	*	1	JAN	2018	610	4.
1	JAN	0132	47	3.	*	1	JAN	0748	235	3.	*	1	JAN	1404	423	3.	*	1	JAN	2020	611	4.

1 JAN 0134	48	3.	*	1 JAN 0750	236	3.	*	1 JAN 1406	424	3.	*	1 JAN 2022	612	4.
1 JAN 0136	49	3.	*	1 JAN 0752	237	3.	*	1 JAN 1408	425	3.	*	1 JAN 2024	613	4.
1 JAN 0138	50	3.	*	1 JAN 0754	238	3.	*	1 JAN 1410	426	3.	*	1 JAN 2026	614	4.
1 JAN 0140	51	3.	*	1 JAN 0756	239	3.	*	1 JAN 1412	427	3.	*	1 JAN 2028	615	4.
1 JAN 0142	52	3.	*	1 JAN 0758	240	3.	*	1 JAN 1414	428	3.	*	1 JAN 2030	616	4.
1 JAN 0144	53	3.	*	1 JAN 0800	241	3.	*	1 JAN 1416	429	3.	*	1 JAN 2032	617	4.
1 JAN 0146	54	3.	*	1 JAN 0802	242	3.	*	1 JAN 1418	430	3.	*	1 JAN 2034	618	4.
1 JAN 0148	55	3.	*	1 JAN 0804	243	3.	*	1 JAN 1420	431	3.	*	1 JAN 2036	619	4.
1 JAN 0150	56	3.	*	1 JAN 0806	244	3.	*	1 JAN 1422	432	3.	*	1 JAN 2038	620	4.
1 JAN 0152	57	3.	*	1 JAN 0808	245	3.	*	1 JAN 1424	433	3.	*	1 JAN 2040	621	4.
1 JAN 0154	58	3.	*	1 JAN 0810	246	3.	*	1 JAN 1426	434	3.	*	1 JAN 2042	622	4.
1 JAN 0156	59	3.	*	1 JAN 0812	247	3.	*	1 JAN 1428	435	3.	*	1 JAN 2044	623	4.
1 JAN 0158	60	3.	*	1 JAN 0814	248	3.	*	1 JAN 1430	436	3.	*	1 JAN 2046	624	4.
1 JAN 0200	61	3.	*	1 JAN 0816	249	3.	*	1 JAN 1432	437	3.	*	1 JAN 2048	625	4.
1 JAN 0202	62	3.	*	1 JAN 0818	250	3.	*	1 JAN 1434	438	3.	*	1 JAN 2050	626	4.
1 JAN 0204	63	3.	*	1 JAN 0820	251	3.	*	1 JAN 1436	439	3.	*	1 JAN 2052	627	4.
1 JAN 0206	64	3.	*	1 JAN 0822	252	3.	*	1 JAN 1438	440	3.	*	1 JAN 2054	628	4.
1 JAN 0208	65	3.	*	1 JAN 0824	253	3.	*	1 JAN 1440	441	3.	*	1 JAN 2056	629	4.
1 JAN 0210	66	3.	*	1 JAN 0826	254	3.	*	1 JAN 1442	442	3.	*	1 JAN 2058	630	4.
1 JAN 0212	67	3.	*	1 JAN 0828	255	3.	*	1 JAN 1444	443	3.	*	1 JAN 2100	631	4.
1 JAN 0214	68	3.	*	1 JAN 0830	256	3.	*	1 JAN 1446	444	3.	*	1 JAN 2102	632	4.
1 JAN 0216	69	3.	*	1 JAN 0832	257	3.	*	1 JAN 1448	445	3.	*	1 JAN 2104	633	4.
1 JAN 0218	70	3.	*	1 JAN 0834	258	3.	*	1 JAN 1450	446	3.	*	1 JAN 2106	634	4.
1 JAN 0220	71	3.	*	1 JAN 0836	259	3.	*	1 JAN 1452	447	3.	*	1 JAN 2108	635	4.
1 JAN 0222	72	3.	*	1 JAN 0838	260	3.	*	1 JAN 1454	448	3.	*	1 JAN 2110	636	4.
1 JAN 0224	73	3.	*	1 JAN 0840	261	3.	*	1 JAN 1456	449	3.	*	1 JAN 2112	637	4.
1 JAN 0226	74	3.	*	1 JAN 0842	262	3.	*	1 JAN 1458	450	3.	*	1 JAN 2114	638	4.
1 JAN 0228	75	3.	*	1 JAN 0844	263	3.	*	1 JAN 1500	451	3.	*	1 JAN 2116	639	4.
1 JAN 0230	76	3.	*	1 JAN 0846	264	3.	*	1 JAN 1502	452	3.	*	1 JAN 2118	640	4.
1 JAN 0232	77	3.	*	1 JAN 0848	265	3.	*	1 JAN 1504	453	3.	*	1 JAN 2120	641	4.
1 JAN 0234	78	3.	*	1 JAN 0850	266	3.	*	1 JAN 1506	454	3.	*	1 JAN 2122	642	4.
1 JAN 0236	79	3.	*	1 JAN 0852	267	3.	*	1 JAN 1508	455	3.	*	1 JAN 2124	643	4.
1 JAN 0238	80	3.	*	1 JAN 0854	268	3.	*	1 JAN 1510	456	3.	*	1 JAN 2126	644	4.
1 JAN 0240	81	3.	*	1 JAN 0856	269	3.	*	1 JAN 1512	457	3.	*	1 JAN 2128	645	4.
1 JAN 0242	82	3.	*	1 JAN 0858	270	3.	*	1 JAN 1514	458	3.	*	1 JAN 2130	646	4.
1 JAN 0244	83	3.	*	1 JAN 0900	271	3.	*	1 JAN 1516	459	3.	*	1 JAN 2132	647	4.
1 JAN 0246	84	3.	*	1 JAN 0902	272	3.	*	1 JAN 1518	460	3.	*	1 JAN 2134	648	4.
1 JAN 0248	85	3.	*	1 JAN 0904	273	3.	*	1 JAN 1520	461	3.	*	1 JAN 2136	649	4.
1 JAN 0250	86	3.	*	1 JAN 0906	274	3.	*	1 JAN 1522	462	3.	*	1 JAN 2138	650	4.
1 JAN 0252	87	3.	*	1 JAN 0908	275	3.	*	1 JAN 1524	463	3.	*	1 JAN 2140	651	4.
1 JAN 0254	88	3.	*	1 JAN 0910	276	3.	*	1 JAN 1526	464	3.	*	1 JAN 2142	652	4.
1 JAN 0256	89	3.	*	1 JAN 0912	277	3.	*	1 JAN 1528	465	3.	*	1 JAN 2144	653	4.
1 JAN 0258	90	3.	*	1 JAN 0914	278	3.	*	1 JAN 1530	466	3.	*	1 JAN 2146	654	4.
1 JAN 0300	91	3.	*	1 JAN 0916	279	3.	*	1 JAN 1532	467	3.	*	1 JAN 2148	655	4.
1 JAN 0302	92	3.	*	1 JAN 0918	280	3.	*	1 JAN 1534	468	3.	*	1 JAN 2150	656	4.
1 JAN 0304	93	3.	*	1 JAN 0920	281	3.	*	1 JAN 1536	469	3.	*	1 JAN 2152	657	4.
1 JAN 0306	94	3.	*	1 JAN 0922	282	3.	*	1 JAN 1538	470	3.	*	1 JAN 2154	658	4.
1 JAN 0308	95	3.	*	1 JAN 0924	283	3.	*	1 JAN 1540	471	3.	*	1 JAN 2156	659	4.
1 JAN 0310	96	3.	*	1 JAN 0926	284	3.	*	1 JAN 1542	472	3.	*	1 JAN 2158	660	4.
1 JAN 0312	97	3.	*	1 JAN 0928	285	3.	*	1 JAN 1544	473	3.	*	1 JAN 2200	661	4.
1 JAN 0314	98	3.	*	1 JAN 0930	286	3.	*	1 JAN 1546	474	3.	*	1 JAN 2202	662	4.
1 JAN 0316	99	3.	*	1 JAN 0932	287	3.	*	1 JAN 1548	475	3.	*	1 JAN 2204	663	4.
1 JAN 0318	100	3.	*	1 JAN 0934	288	3.	*	1 JAN 1550	476	3.	*	1 JAN 2206	664	4.
1 JAN 0320	101	3.	*	1 JAN 0936	289	3.	*	1 JAN 1552	477	3.	*	1 JAN 2208	665	4.
1 JAN 0322	102	3.	*	1 JAN 0938	290	3.	*	1 JAN 1554	478	3.	*	1 JAN 2210	666	4.
1 JAN 0324	103	3.	*	1 JAN 0940	291	3.	*	1 JAN 1556	479	3.	*	1 JAN 2212	667	4.
1 JAN 0326	104	3.	*	1 JAN 0942	292	3.	*	1 JAN 1558	480	3.	*	1 JAN 2214	668	4.
1 JAN 0328	105	3.	*	1 JAN 0944	293	3.	*	1 JAN 1600	481	3.	*	1 JAN 2216	669	4.
1 JAN 0330	106	3.	*	1 JAN 0946	294	3.	*	1 JAN 1602	482	3.	*	1 JAN 2218	670	4.
1 JAN 0332	107	3.	*	1 JAN 0948	295	3.	*	1 JAN 1604	483	3.	*	1 JAN 2220	671	4.
1 JAN 0334	108	3.	*	1 JAN 0950	296	3.	*	1 JAN 1606	484	3.	*	1 JAN 2222	672	4.
1 JAN 0336	109	3.	*	1 JAN 0952	297	3.	*	1 JAN 1608	485	3.	*	1 JAN 2224	673	4.
1 JAN 0338	110	3.	*	1 JAN 0954	298	3.	*	1 JAN 1610	486	3.	*	1 JAN 2226	674	4.
1 JAN 0340	111	3.	*	1 JAN 0956	299	3.	*	1 JAN 1612	487	3.	*	1 JAN 2228	675	4.
1 JAN 0342	112	3.	*	1 JAN 0958	300	3.	*	1 JAN 1614	488	3.	*	1 JAN 2230	676	4.
1 JAN 0344	113	3.	*	1 JAN 1000	301	3.	*	1 JAN 1616	489	3.	*	1 JAN 2232	677	4.
1 JAN 0346	114	3.	*	1 JAN 1002	302	3.	*	1 JAN 1618	490	3.	*	1 JAN 2234	678	4.
1 JAN 0348	115	3.	*	1 JAN 1004	303	3.	*	1 JAN 1620	491	3.	*	1 JAN 2236	679	4.
1 JAN 0350	116	3.	*	1 JAN 1006	304	3.	*	1 JAN 1622	492	3.	*	1 JAN 2238	680	4.
1 JAN 0352	117	3.	*	1 JAN 1008	305	3.	*	1 JAN 1624	493	3.	*	1 JAN 2240	681	4.
1 JAN 0354	118	3.	*	1 JAN 1010	306	3.	*	1 JAN 1626	494	3.	*	1 JAN 2242	682	4.
1 JAN 0356	119	3.	*	1 JAN 1012	307	3.	*	1 JAN 1628	495	3.	*	1 JAN 2244	683	4.
1 JAN 0358	120	3.	*	1 JAN 1014	308	3.	*	1 JAN 1630	496	3.	*	1 JAN 2246	684	4.
1 JAN 0400	121	3.	*	1 JAN 1016	309	3.	*	1 JAN 1632	497	3.	*	1 JAN 2248	685	4.
1 JAN 0402	122	3.	*	1 JAN 1018	310	3.	*	1 JAN 1634	498	3.	*	1 JAN 2250	686	4.
1 JAN 0404	123	3.	*	1 JAN 1020	311	3.	*	1 JAN 1636	499	3.	*	1 JAN 2252	687	4.
1 JAN 0406	124	3.	*	1 JAN 1022	312	3.	*	1 JAN 1638	500	3.	*	1 JAN 2254	688	4.
1 JAN 0408	125	3.	*	1 JAN 1024	313	3.	*	1 JAN 1640	501	3.	*	1 JAN 2256	689	4.
1 JAN 0410	126	3.	*	1 JAN 1026	314	3.	*	1 JAN 1642	502	3.	*	1 JAN 2258	690	4.
1 JAN 0412	127	3.	*	1 JAN 1028	315	3.	*	1 JAN 1644	503	3.	*	1 JAN 2300	691	4.
1 JAN 0414	128	3.	*	1 JAN 1030	316	3.	*	1 JAN 1646	504	3.	*	1 JAN 2302	692	4.
1 JAN 0416	129	3.	*	1 JAN 1032	317	3.	*	1 JAN 1648	505	3.	*	1 JAN 2304	693	4.
1 JAN 0418	130	3.	*	1 JAN 1034	318	3.	*	1 JAN 1650	506	3.	*	1 JAN 2306	694	4.
1 JAN 0420	131	3.	*	1 JAN 1036	319	3.	*	1 JAN 1652	507	3.	*	1 JAN 2308	695	4.
1 JAN 0422	132	3.	*	1 JAN 1038	320	3.	*	1 JAN 1654	508	3.	*	1 JAN 2310	696	4.
1 JAN 0424	133	3.	*	1 JAN 1040	321	3.	*	1 JAN 1656	509	3.	*	1 JAN 2312	697	4.
1 JAN 0426	134	3.	*	1 JAN 1042	322	3.	*	1 JAN 1658	510	3.	*	1 JAN 2314	698	4.
1 JAN 0428	135	3.	*	1 JAN 1044	323	3.	*	1 JAN 1700	511	3.	*	1 JAN 2316	699	4.
1 JAN 0430	136	3.	*	1 JAN 1046	324	3.	*	1 JAN 1702	512	3.	*	1 JAN 2318	700	4.
1 JAN 0432	137	3.	*	1 JAN 1048	325	3.	*	1 JAN 1704	513	3.	*	1 JAN 2320	701	4.
1 JAN 0434	138	3.	*	1 JAN 1050	326	3.	*	1 JAN 1706	514	4.	*	1 JAN 2322	702	4.
1 JAN 0436	139	3.	*	1 JAN 1052	327	3.	*	1 JAN 1708	515	4.	*	1 JAN 2324	703	4.
1 JAN 0438	140	3.	*	1 JAN 1054	328	3.	*	1 JAN 1710	516	4.	*	1 JAN 2326	704	4.
1 JAN 0440	141	3.	*	1 JAN 1056	329	3.	*	1 JAN 1712	517	4.	*	1 JAN 2328	705	4.
1 JAN 0442	142	3.	*	1 JAN 1058	330	3.	*	1 JAN 1714	518	4.	*	1 JAN 2330	706	4.

SEWID 15.00 SPILLWAY WIDTH  
 COQW 2.30 WEIR COEFFICIENT  
 EXPW 1.50 EXPONENT OF HEAD

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COMPUTED OUTFLOW-ELEVATION DATA

OUTFLOW ELEVATION	.00	.00	.47	.50	.55	.60	.66	.74	.84	.97
	.00	1.25	1.49	1.53	1.59	1.65	1.74	1.86	2.04	2.30
OUTFLOW ELEVATION	1.00	1.17	1.59	2.38	3.65	5.54	8.14	11.58	15.98	21.45
	2.31	2.33	2.37	2.42	2.48	2.56	2.65	2.75	2.87	3.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.04	.05	.07	.07	.08	.08	.09	.10	.10
OUTFLOW	.00	.00	.00	.47	.50	.55	.60	.66	.74	.82
ELEVATION	.00	1.00	1.25	1.49	1.53	1.59	1.65	1.74	1.86	2.00
STORAGE	.11	.13	.14	.14	.15	.15	.16	.17	.18	.19
OUTFLOW	.84	.97	1.17	1.59	2.38	3.65	5.54	8.14	11.58	15.98
ELEVATION	2.04	2.30	2.33	2.37	2.42	2.48	2.56	2.65	2.75	2.87
STORAGE	.20									
OUTFLOW	21.45									
ELEVATION	3.00									

HYDROGRAPH AT STATION RET1B

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE
1	JAN	0000	1	0.	.0	.0	1	JAN	0820	251	3.	.2	2.5	1	JAN	1640	501	3.	.2
2.5							2.5						2.5						
1	JAN	0002	2	0.	.0	.2	1	JAN	0822	252	3.	.2	2.5	1	JAN	1642	502	3.	.2
2.5							2.5						2.5						
1	JAN	0004	3	0.	.0	.7	1	JAN	0824	253	3.	.2	2.5	1	JAN	1644	503	3.	.2
2.5							2.5						2.5						
1	JAN	0006	4	0.	.1	1.3	1	JAN	0826	254	3.	.2	2.5	1	JAN	1646	504	3.	.2
2.5							2.5						2.5						
1	JAN	0008	5	1.	.1	1.9	1	JAN	0828	255	3.	.2	2.5	1	JAN	1648	505	3.	.2
2.5							2.5						2.5						
1	JAN	0010	6	3.	.2	2.5	1	JAN	0830	256	3.	.2	2.5	1	JAN	1650	506	3.	.2
2.5							2.5						2.5						
1	JAN	0012	7	14.	.2	2.8	1	JAN	0832	257	3.	.2	2.5	1	JAN	1652	507	3.	.2
2.5							2.5						2.5						
1	JAN	0014	8	18.	.2	2.9	1	JAN	0834	258	3.	.2	2.5	1	JAN	1654	508	3.	.2
2.5							2.5						2.5						
1	JAN	0016	9	17.	.2	2.9	1	JAN	0836	259	3.	.2	2.5	1	JAN	1656	509	3.	.2
2.5							2.5						2.5						
1	JAN	0018	10	15.	.2	2.8	1	JAN	0838	260	3.	.2	2.5	1	JAN	1658	510	3.	.2
2.5							2.5						2.5						
1	JAN	0020	11	12.	.2	2.8	1	JAN	0840	261	3.	.2	2.5	1	JAN	1700	511	3.	.2
2.5							2.5						2.5						
1	JAN	0022	12	10.	.2	2.7	1	JAN	0842	262	3.	.2	2.5	1	JAN	1702	512	3.	.2
2.5							2.5						2.5						
1	JAN	0024	13	7.	.2	2.6	1	JAN	0844	263	3.	.2	2.5	1	JAN	1704	513	3.	.2
2.5							2.5						2.5						
1	JAN	0026	14	5.	.2	2.5	1	JAN	0846	264	3.	.2	2.5	1	JAN	1706	514	3.	.2
2.5							2.5						2.5						
1	JAN	0028	15	4.	.2	2.5	1	JAN	0848	265	3.	.2	2.5	1	JAN	1708	515	3.	.2
2.5							2.5						2.5						
1	JAN	0030	16	4.	.2	2.5	1	JAN	0850	266	3.	.2	2.5	1	JAN	1710	516	4.	.2
2.5							2.5						2.5						
1	JAN	0032	17	3.	.2	2.5	1	JAN	0852	267	3.	.2	2.5	1	JAN	1712	517	4.	.2
2.5							2.5						2.5						
1	JAN	0034	18	3.	.2	2.5	1	JAN	0854	268	3.	.2	2.5	1	JAN	1714	518	4.	.2
2.5							2.5						2.5						
1	JAN	0036	19	3.	.2	2.5	1	JAN	0856	269	3.	.2	2.5	1	JAN	1716	519	4.	.2
2.5							2.5						2.5						
1	JAN	0038	20	3.	.2	2.5	1	JAN	0858	270	3.	.2	2.5	1	JAN	1718	520	4.	.2
2.5							2.5						2.5						
1	JAN	0040	21	3.	.2	2.5	1	JAN	0900	271	3.	.2	2.5	1	JAN	1720	521	4.	.2
2.5							2.5						2.5						
1	JAN	0042	22	3.	.2	2.5	1	JAN	0902	272	3.	.2	2.5	1	JAN	1722	522	4.	.2
2.5							2.5						2.5						
1	JAN	0044	23	3.	.2	2.5	1	JAN	0904	273	3.	.2	2.5	1	JAN	1724	523	4.	.2
2.5							2.5						2.5						
1	JAN	0046	24	3.	.2	2.5	1	JAN	0906	274	3.	.2	2.5	1	JAN	1726	524	4.	.2
2.5							2.5						2.5						
1	JAN	0048	25	3.	.2	2.5	1	JAN	0908	275	3.	.2	2.5	1	JAN	1728	525	4.	.2
2.5							2.5						2.5						
1	JAN	0050	26	3.	.2	2.5	1	JAN	0910	276	3.	.2	2.5	1	JAN	1730	526	4.	.2
2.5							2.5						2.5						







1 JAN 0400 121	3.	.2	2.5 *	1 JAN 1220 371	3.	.2	2.5 *	1 JAN 2040 621	4.	.2
2.5 1 JAN 0402 122	3.	.2	2.5 *	1 JAN 1222 372	3.	.2	2.5 *	1 JAN 2042 622	4.	.2
2.5 1 JAN 0404 123	3.	.2	2.5 *	1 JAN 1224 373	3.	.2	2.5 *	1 JAN 2044 623	4.	.2
2.5 1 JAN 0406 124	3.	.2	2.5 *	1 JAN 1226 374	3.	.2	2.5 *	1 JAN 2046 624	4.	.2
2.5 1 JAN 0408 125	3.	.2	2.5 *	1 JAN 1228 375	3.	.2	2.5 *	1 JAN 2048 625	4.	.2
2.5 1 JAN 0410 126	3.	.2	2.5 *	1 JAN 1230 376	3.	.2	2.5 *	1 JAN 2050 626	4.	.2
2.5 1 JAN 0412 127	3.	.2	2.5 *	1 JAN 1232 377	3.	.2	2.5 *	1 JAN 2052 627	4.	.2
2.5 1 JAN 0414 128	3.	.2	2.5 *	1 JAN 1234 378	3.	.2	2.5 *	1 JAN 2054 628	4.	.2
2.5 1 JAN 0416 129	3.	.2	2.5 *	1 JAN 1236 379	3.	.2	2.5 *	1 JAN 2056 629	4.	.2
2.5 1 JAN 0418 130	3.	.2	2.5 *	1 JAN 1238 380	3.	.2	2.5 *	1 JAN 2058 630	4.	.2
2.5 1 JAN 0420 131	3.	.2	2.5 *	1 JAN 1240 381	3.	.2	2.5 *	1 JAN 2100 631	4.	.2
2.5 1 JAN 0422 132	3.	.2	2.5 *	1 JAN 1242 382	3.	.2	2.5 *	1 JAN 2102 632	4.	.2
2.5 1 JAN 0424 133	3.	.2	2.5 *	1 JAN 1244 383	3.	.2	2.5 *	1 JAN 2104 633	4.	.2
2.5 1 JAN 0426 134	3.	.2	2.5 *	1 JAN 1246 384	3.	.2	2.5 *	1 JAN 2106 634	4.	.2
2.5 1 JAN 0428 135	3.	.2	2.5 *	1 JAN 1248 385	3.	.2	2.5 *	1 JAN 2108 635	4.	.2
2.5 1 JAN 0430 136	3.	.2	2.5 *	1 JAN 1250 386	3.	.2	2.5 *	1 JAN 2110 636	4.	.2
2.5 1 JAN 0432 137	3.	.2	2.5 *	1 JAN 1252 387	3.	.2	2.5 *	1 JAN 2112 637	4.	.2
2.5 1 JAN 0434 138	3.	.2	2.5 *	1 JAN 1254 388	3.	.2	2.5 *	1 JAN 2114 638	4.	.2
2.5 1 JAN 0436 139	3.	.2	2.5 *	1 JAN 1256 389	3.	.2	2.5 *	1 JAN 2116 639	4.	.2
2.5 1 JAN 0438 140	3.	.2	2.5 *	1 JAN 1258 390	3.	.2	2.5 *	1 JAN 2118 640	4.	.2
2.5 1 JAN 0440 141	3.	.2	2.5 *	1 JAN 1300 391	3.	.2	2.5 *	1 JAN 2120 641	4.	.2
2.5 1 JAN 0442 142	3.	.2	2.5 *	1 JAN 1302 392	3.	.2	2.5 *	1 JAN 2122 642	4.	.2
2.5 1 JAN 0444 143	3.	.2	2.5 *	1 JAN 1304 393	3.	.2	2.5 *	1 JAN 2124 643	4.	.2
2.5 1 JAN 0446 144	3.	.2	2.5 *	1 JAN 1306 394	3.	.2	2.5 *	1 JAN 2126 644	4.	.2
2.5 1 JAN 0448 145	3.	.2	2.5 *	1 JAN 1308 395	3.	.2	2.5 *	1 JAN 2128 645	4.	.2
2.5 1 JAN 0450 146	3.	.2	2.5 *	1 JAN 1310 396	3.	.2	2.5 *	1 JAN 2130 646	4.	.2
2.5 1 JAN 0452 147	3.	.2	2.5 *	1 JAN 1312 397	3.	.2	2.5 *	1 JAN 2132 647	4.	.2
2.5 1 JAN 0454 148	3.	.2	2.5 *	1 JAN 1314 398	3.	.2	2.5 *	1 JAN 2134 648	4.	.2
2.5 1 JAN 0456 149	3.	.2	2.5 *	1 JAN 1316 399	3.	.2	2.5 *	1 JAN 2136 649	4.	.2
2.5 1 JAN 0458 150	3.	.2	2.5 *	1 JAN 1318 400	3.	.2	2.5 *	1 JAN 2138 650	4.	.2
2.5 1 JAN 0500 151	3.	.2	2.5 *	1 JAN 1320 401	3.	.2	2.5 *	1 JAN 2140 651	4.	.2
2.5 1 JAN 0502 152	3.	.2	2.5 *	1 JAN 1322 402	3.	.2	2.5 *	1 JAN 2142 652	4.	.2
2.5 1 JAN 0504 153	3.	.2	2.5 *	1 JAN 1324 403	3.	.2	2.5 *	1 JAN 2144 653	4.	.2
2.5 1 JAN 0506 154	3.	.2	2.5 *	1 JAN 1326 404	3.	.2	2.5 *	1 JAN 2146 654	4.	.2
2.5 1 JAN 0508 155	3.	.2	2.5 *	1 JAN 1328 405	3.	.2	2.5 *	1 JAN 2148 655	4.	.2
2.5 1 JAN 0510 156	3.	.2	2.5 *	1 JAN 1330 406	3.	.2	2.5 *	1 JAN 2150 656	4.	.2
2.5 1 JAN 0512 157	3.	.2	2.5 *	1 JAN 1332 407	3.	.2	2.5 *	1 JAN 2152 657	4.	.2
2.5 1 JAN 0514 158	3.	.2	2.5 *	1 JAN 1334 408	3.	.2	2.5 *	1 JAN 2154 658	4.	.2
2.5 1 JAN 0516 159	3.	.2	2.5 *	1 JAN 1336 409	3.	.2	2.5 *	1 JAN 2156 659	4.	.2
2.5 1 JAN 0518 160	3.	.2	2.5 *	1 JAN 1338 410	3.	.2	2.5 *	1 JAN 2158 660	4.	.2
2.5 1 JAN 0520 161	3.	.2	2.5 *	1 JAN 1340 411	3.	.2	2.5 *	1 JAN 2200 661	4.	.2
2.5 1 JAN 0522 162	3.	.2	2.5 *	1 JAN 1342 412	3.	.2	2.5 *	1 JAN 2202 662	4.	.2
2.5 1 JAN 0524 163	3.	.2	2.5 *	1 JAN 1344 413	3.	.2	2.5 *	1 JAN 2204 663	4.	.2
2.5 1 JAN 0526 164	3.	.2	2.5 *	1 JAN 1346 414	3.	.2	2.5 *	1 JAN 2206 664	4.	.2
2.5 1 JAN 0528 165	3.	.2	2.5 *	1 JAN 1348 415	3.	.2	2.5 *	1 JAN 2208 665	4.	.2
2.5 1 JAN 0530 166	3.	.2	2.5 *	1 JAN 1350 416	3.	.2	2.5 *	1 JAN 2210 666	4.	.2
2.5 1 JAN 0532 167	3.	.2	2.5 *	1 JAN 1352 417	3.	.2	2.5 *	1 JAN 2212 667	4.	.2

1 JAN 0038	20	1.	*	1 JAN 0654	208	1.	*	1 JAN 1310	396	1.	*	1 JAN 1926	584	1.
1 JAN 0040	21	1.	*	1 JAN 0656	209	1.	*	1 JAN 1312	397	1.	*	1 JAN 1928	585	1.
1 JAN 0042	22	1.	*	1 JAN 0658	210	1.	*	1 JAN 1314	398	1.	*	1 JAN 1930	586	1.
1 JAN 0044	23	1.	*	1 JAN 0700	211	1.	*	1 JAN 1316	399	1.	*	1 JAN 1932	587	1.
1 JAN 0046	24	1.	*	1 JAN 0702	212	1.	*	1 JAN 1318	400	1.	*	1 JAN 1934	588	1.
1 JAN 0048	25	1.	*	1 JAN 0704	213	1.	*	1 JAN 1320	401	1.	*	1 JAN 1936	589	1.
1 JAN 0050	26	1.	*	1 JAN 0706	214	1.	*	1 JAN 1322	402	1.	*	1 JAN 1938	590	1.
1 JAN 0052	27	1.	*	1 JAN 0708	215	1.	*	1 JAN 1324	403	1.	*	1 JAN 1940	591	1.
1 JAN 0054	28	1.	*	1 JAN 0710	216	1.	*	1 JAN 1326	404	1.	*	1 JAN 1942	592	1.
1 JAN 0056	29	1.	*	1 JAN 0712	217	1.	*	1 JAN 1328	405	1.	*	1 JAN 1944	593	1.
1 JAN 0058	30	1.	*	1 JAN 0714	218	1.	*	1 JAN 1330	406	1.	*	1 JAN 1946	594	1.
1 JAN 0100	31	1.	*	1 JAN 0716	219	1.	*	1 JAN 1332	407	1.	*	1 JAN 1948	595	1.
1 JAN 0102	32	1.	*	1 JAN 0718	220	1.	*	1 JAN 1334	408	1.	*	1 JAN 1950	596	1.
1 JAN 0104	33	1.	*	1 JAN 0720	221	1.	*	1 JAN 1336	409	1.	*	1 JAN 1952	597	1.
1 JAN 0106	34	1.	*	1 JAN 0722	222	1.	*	1 JAN 1338	410	1.	*	1 JAN 1954	598	1.
1 JAN 0108	35	1.	*	1 JAN 0724	223	1.	*	1 JAN 1340	411	1.	*	1 JAN 1956	599	1.
1 JAN 0110	36	1.	*	1 JAN 0726	224	1.	*	1 JAN 1342	412	1.	*	1 JAN 1958	600	1.
1 JAN 0112	37	1.	*	1 JAN 0728	225	1.	*	1 JAN 1344	413	1.	*	1 JAN 2000	601	1.
1 JAN 0114	38	1.	*	1 JAN 0730	226	1.	*	1 JAN 1346	414	1.	*	1 JAN 2002	602	1.
1 JAN 0116	39	1.	*	1 JAN 0732	227	1.	*	1 JAN 1348	415	1.	*	1 JAN 2004	603	1.
1 JAN 0118	40	1.	*	1 JAN 0734	228	1.	*	1 JAN 1350	416	1.	*	1 JAN 2006	604	1.
1 JAN 0120	41	1.	*	1 JAN 0736	229	1.	*	1 JAN 1352	417	1.	*	1 JAN 2008	605	1.
1 JAN 0122	42	1.	*	1 JAN 0738	230	1.	*	1 JAN 1354	418	1.	*	1 JAN 2010	606	1.
1 JAN 0124	43	1.	*	1 JAN 0740	231	1.	*	1 JAN 1356	419	1.	*	1 JAN 2012	607	1.
1 JAN 0126	44	1.	*	1 JAN 0742	232	1.	*	1 JAN 1358	420	1.	*	1 JAN 2014	608	1.
1 JAN 0128	45	1.	*	1 JAN 0744	233	1.	*	1 JAN 1400	421	1.	*	1 JAN 2016	609	1.
1 JAN 0130	46	1.	*	1 JAN 0746	234	1.	*	1 JAN 1402	422	1.	*	1 JAN 2018	610	1.
1 JAN 0132	47	1.	*	1 JAN 0748	235	1.	*	1 JAN 1404	423	1.	*	1 JAN 2020	611	1.
1 JAN 0134	48	1.	*	1 JAN 0750	236	1.	*	1 JAN 1406	424	1.	*	1 JAN 2022	612	1.
1 JAN 0136	49	1.	*	1 JAN 0752	237	1.	*	1 JAN 1408	425	1.	*	1 JAN 2024	613	1.
1 JAN 0138	50	1.	*	1 JAN 0754	238	1.	*	1 JAN 1410	426	1.	*	1 JAN 2026	614	1.
1 JAN 0140	51	1.	*	1 JAN 0756	239	1.	*	1 JAN 1412	427	1.	*	1 JAN 2028	615	1.
1 JAN 0142	52	1.	*	1 JAN 0758	240	1.	*	1 JAN 1414	428	1.	*	1 JAN 2030	616	1.
1 JAN 0144	53	1.	*	1 JAN 0800	241	1.	*	1 JAN 1416	429	1.	*	1 JAN 2032	617	1.
1 JAN 0146	54	1.	*	1 JAN 0802	242	1.	*	1 JAN 1418	430	1.	*	1 JAN 2034	618	1.
1 JAN 0148	55	1.	*	1 JAN 0804	243	1.	*	1 JAN 1420	431	1.	*	1 JAN 2036	619	1.
1 JAN 0150	56	1.	*	1 JAN 0806	244	1.	*	1 JAN 1422	432	1.	*	1 JAN 2038	620	1.
1 JAN 0152	57	1.	*	1 JAN 0808	245	1.	*	1 JAN 1424	433	1.	*	1 JAN 2040	621	1.
1 JAN 0154	58	1.	*	1 JAN 0810	246	1.	*	1 JAN 1426	434	1.	*	1 JAN 2042	622	1.
1 JAN 0156	59	1.	*	1 JAN 0812	247	1.	*	1 JAN 1428	435	1.	*	1 JAN 2044	623	1.
1 JAN 0158	60	1.	*	1 JAN 0814	248	1.	*	1 JAN 1430	436	1.	*	1 JAN 2046	624	1.
1 JAN 0200	61	1.	*	1 JAN 0816	249	1.	*	1 JAN 1432	437	1.	*	1 JAN 2048	625	1.
1 JAN 0202	62	1.	*	1 JAN 0818	250	1.	*	1 JAN 1434	438	1.	*	1 JAN 2050	626	1.
1 JAN 0204	63	1.	*	1 JAN 0820	251	1.	*	1 JAN 1436	439	1.	*	1 JAN 2052	627	1.
1 JAN 0206	64	1.	*	1 JAN 0822	252	1.	*	1 JAN 1438	440	1.	*	1 JAN 2054	628	1.
1 JAN 0208	65	1.	*	1 JAN 0824	253	1.	*	1 JAN 1440	441	1.	*	1 JAN 2056	629	1.
1 JAN 0210	66	1.	*	1 JAN 0826	254	1.	*	1 JAN 1442	442	1.	*	1 JAN 2058	630	1.
1 JAN 0212	67	1.	*	1 JAN 0828	255	1.	*	1 JAN 1444	443	1.	*	1 JAN 2100	631	1.
1 JAN 0214	68	1.	*	1 JAN 0830	256	1.	*	1 JAN 1446	444	1.	*	1 JAN 2102	632	1.
1 JAN 0216	69	1.	*	1 JAN 0832	257	1.	*	1 JAN 1448	445	1.	*	1 JAN 2104	633	1.
1 JAN 0218	70	1.	*	1 JAN 0834	258	1.	*	1 JAN 1450	446	1.	*	1 JAN 2106	634	1.
1 JAN 0220	71	1.	*	1 JAN 0836	259	1.	*	1 JAN 1452	447	1.	*	1 JAN 2108	635	1.
1 JAN 0222	72	1.	*	1 JAN 0838	260	1.	*	1 JAN 1454	448	1.	*	1 JAN 2110	636	1.
1 JAN 0224	73	1.	*	1 JAN 0840	261	1.	*	1 JAN 1456	449	1.	*	1 JAN 2112	637	1.
1 JAN 0226	74	1.	*	1 JAN 0842	262	1.	*	1 JAN 1458	450	1.	*	1 JAN 2114	638	1.
1 JAN 0228	75	1.	*	1 JAN 0844	263	1.	*	1 JAN 1500	451	1.	*	1 JAN 2116	639	1.
1 JAN 0230	76	1.	*	1 JAN 0846	264	1.	*	1 JAN 1502	452	1.	*	1 JAN 2118	640	1.
1 JAN 0232	77	1.	*	1 JAN 0848	265	1.	*	1 JAN 1504	453	1.	*	1 JAN 2120	641	1.
1 JAN 0234	78	1.	*	1 JAN 0850	266	1.	*	1 JAN 1506	454	1.	*	1 JAN 2122	642	1.
1 JAN 0236	79	1.	*	1 JAN 0852	267	1.	*	1 JAN 1508	455	1.	*	1 JAN 2124	643	1.
1 JAN 0238	80	1.	*	1 JAN 0854	268	1.	*	1 JAN 1510	456	1.	*	1 JAN 2126	644	1.
1 JAN 0240	81	1.	*	1 JAN 0856	269	1.	*	1 JAN 1512	457	1.	*	1 JAN 2128	645	1.
1 JAN 0242	82	1.	*	1 JAN 0858	270	1.	*	1 JAN 1514	458	1.	*	1 JAN 2130	646	1.
1 JAN 0244	83	1.	*	1 JAN 0900	271	1.	*	1 JAN 1516	459	1.	*	1 JAN 2132	647	1.
1 JAN 0246	84	1.	*	1 JAN 0902	272	1.	*	1 JAN 1518	460	1.	*	1 JAN 2134	648	1.
1 JAN 0248	85	1.	*	1 JAN 0904	273	1.	*	1 JAN 1520	461	1.	*	1 JAN 2136	649	1.
1 JAN 0250	86	1.	*	1 JAN 0906	274	1.	*	1 JAN 1522	462	1.	*	1 JAN 2138	650	1.
1 JAN 0252	87	1.	*	1 JAN 0908	275	1.	*	1 JAN 1524	463	1.	*	1 JAN 2140	651	1.
1 JAN 0254	88	1.	*	1 JAN 0910	276	1.	*	1 JAN 1526	464	1.	*	1 JAN 2142	652	1.
1 JAN 0256	89	1.	*	1 JAN 0912	277	1.	*	1 JAN 1528	465	1.	*	1 JAN 2144	653	1.
1 JAN 0258	90	1.	*	1 JAN 0914	278	1.	*	1 JAN 1530	466	1.	*	1 JAN 2146	654	1.
1 JAN 0300	91	1.	*	1 JAN 0916	279	1.	*	1 JAN 1532	467	1.	*	1 JAN 2148	655	1.
1 JAN 0302	92	1.	*	1 JAN 0918	280	1.	*	1 JAN 1534	468	1.	*	1 JAN 2150	656	1.
1 JAN 0304	93	1.	*	1 JAN 0920	281	1.	*	1 JAN 1536	469	1.	*	1 JAN 2152	657	1.
1 JAN 0306	94	1.	*	1 JAN 0922	282	1.	*	1 JAN 1538	470	1.	*	1 JAN 2154	658	1.
1 JAN 0308	95	1.	*	1 JAN 0924	283	1.	*	1 JAN 1540	471	1.	*	1 JAN 2156	659	1.
1 JAN 0310	96	1.	*	1 JAN 0926	284	1.	*	1 JAN 1542	472	1.	*	1 JAN 2158	660	1.
1 JAN 0312	97	1.	*	1 JAN 0928	285	1.	*	1 JAN 1544	473	1.	*	1 JAN 2200	661	1.
1 JAN 0314	98	1.	*	1 JAN 0930	286	1.	*	1 JAN 1546	474	1.	*	1 JAN 2202	662	1.
1 JAN 0316	99	1.	*	1 JAN 0932	287	1.	*	1 JAN 1548	475	1.	*	1 JAN 2204	663	1.
1 JAN 0318	100	1.	*	1 JAN 0934	288	1.	*	1 JAN 1550	476	1.	*	1 JAN 2206	664	1.
1 JAN 0320	101	1.	*	1 JAN 0936	289	1.	*	1 JAN 1552	477	1.	*	1 JAN 2208	665	1.
1 JAN 0322	102	1.	*	1 JAN 0938	290	1.	*	1 JAN 1554	478	1.	*	1 JAN 2210	666	1.
1 JAN 0324	103	1.	*	1 JAN 0940	291	1.	*	1 JAN 1556	479	1.	*	1 JAN 2212	667	1.
1 JAN 0326	104	1.	*	1 JAN 0942	292	1.	*	1 JAN 1558	480	1.	*	1 JAN 2214	668	1.
1 JAN 0328	105	1.	*	1 JAN 0944	293	1.	*	1 JAN 1600	481	1.	*	1 JAN 2216	669	1.
1 JAN 0330	106	1.	*	1 JAN 0946	294	1.	*	1 JAN 1602	482	1.	*	1 JAN 2218	670	1.
1 JAN 0332	107	1.	*	1 JAN 0948	295	1.	*	1 JAN 1604	483	1.	*	1 JAN 2220	671	1.
1 JAN 0334	108	1.	*	1 JAN 0950	296	1.	*	1 JAN 1606	484	1.	*	1 JAN 2222	672	1.
1 JAN 0336	109	1.	*	1 JAN 0952	297	1.	*	1 JAN 1608	485	1.	*	1 JAN 2224	673	1.
1 JAN 0338	110	1.	*	1 JAN 0954	298	1.	*	1 JAN 1610	486	1.	*	1 JAN 2226	674	1.
1 JAN 0340	111	1.	*	1 JAN 0956	299	1.	*	1 JAN 1612	487	1.	*	1 JAN 2228	675	1.
1 JAN 0342	112	1.	*	1 JAN 0958	300	1.	*	1 JAN 1614	488	1.	*	1 JAN 2230	676	1.
1 JAN 0344	113	1.	*	1 JAN 1000	301	1.	*	1 JAN 1616	489	1.	*	1 JAN 2232	677	1.
1 JAN 0346	114	1.	*	1 JAN 1002	302	1.	*	1 JAN 1618	490	1.	*	1 JAN 2234	678	1.

1 JAN 0348	115	1.	*	1 JAN 1004	303	1.	*	1 JAN 1620	491	1.	*	1 JAN 2236	679	1.
1 JAN 0350	116	1.	*	1 JAN 1006	304	1.	*	1 JAN 1622	492	1.	*	1 JAN 2238	680	1.
1 JAN 0352	117	1.	*	1 JAN 1008	305	1.	*	1 JAN 1624	493	1.	*	1 JAN 2240	681	1.
1 JAN 0354	118	1.	*	1 JAN 1010	306	1.	*	1 JAN 1626	494	1.	*	1 JAN 2242	682	1.
1 JAN 0356	119	1.	*	1 JAN 1012	307	1.	*	1 JAN 1628	495	1.	*	1 JAN 2244	683	1.
1 JAN 0358	120	1.	*	1 JAN 1014	308	1.	*	1 JAN 1630	496	1.	*	1 JAN 2246	684	1.
1 JAN 0400	121	1.	*	1 JAN 1016	309	1.	*	1 JAN 1632	497	1.	*	1 JAN 2248	685	1.
1 JAN 0402	122	1.	*	1 JAN 1018	310	1.	*	1 JAN 1634	498	1.	*	1 JAN 2250	686	1.
1 JAN 0404	123	1.	*	1 JAN 1020	311	1.	*	1 JAN 1636	499	1.	*	1 JAN 2252	687	1.
1 JAN 0406	124	1.	*	1 JAN 1022	312	1.	*	1 JAN 1638	500	1.	*	1 JAN 2254	688	1.
1 JAN 0408	125	1.	*	1 JAN 1024	313	1.	*	1 JAN 1640	501	1.	*	1 JAN 2256	689	1.
1 JAN 0410	126	1.	*	1 JAN 1026	314	1.	*	1 JAN 1642	502	1.	*	1 JAN 2258	690	1.
1 JAN 0412	127	1.	*	1 JAN 1028	315	1.	*	1 JAN 1644	503	1.	*	1 JAN 2300	691	1.
1 JAN 0414	128	1.	*	1 JAN 1030	316	1.	*	1 JAN 1646	504	1.	*	1 JAN 2302	692	1.
1 JAN 0416	129	1.	*	1 JAN 1032	317	1.	*	1 JAN 1648	505	1.	*	1 JAN 2304	693	1.
1 JAN 0418	130	1.	*	1 JAN 1034	318	1.	*	1 JAN 1650	506	1.	*	1 JAN 2306	694	1.
1 JAN 0420	131	1.	*	1 JAN 1036	319	1.	*	1 JAN 1652	507	1.	*	1 JAN 2308	695	1.
1 JAN 0422	132	1.	*	1 JAN 1038	320	1.	*	1 JAN 1654	508	1.	*	1 JAN 2310	696	1.
1 JAN 0424	133	1.	*	1 JAN 1040	321	1.	*	1 JAN 1656	509	1.	*	1 JAN 2312	697	1.
1 JAN 0426	134	1.	*	1 JAN 1042	322	1.	*	1 JAN 1658	510	1.	*	1 JAN 2314	698	1.
1 JAN 0428	135	1.	*	1 JAN 1044	323	1.	*	1 JAN 1700	511	1.	*	1 JAN 2316	699	1.
1 JAN 0430	136	1.	*	1 JAN 1046	324	1.	*	1 JAN 1702	512	1.	*	1 JAN 2318	700	1.
1 JAN 0432	137	1.	*	1 JAN 1048	325	1.	*	1 JAN 1704	513	1.	*	1 JAN 2320	701	1.
1 JAN 0434	138	1.	*	1 JAN 1050	326	1.	*	1 JAN 1706	514	1.	*	1 JAN 2322	702	1.
1 JAN 0436	139	1.	*	1 JAN 1052	327	1.	*	1 JAN 1708	515	1.	*	1 JAN 2324	703	1.
1 JAN 0438	140	1.	*	1 JAN 1054	328	1.	*	1 JAN 1710	516	1.	*	1 JAN 2326	704	1.
1 JAN 0440	141	1.	*	1 JAN 1056	329	1.	*	1 JAN 1712	517	1.	*	1 JAN 2328	705	1.
1 JAN 0442	142	1.	*	1 JAN 1058	330	1.	*	1 JAN 1714	518	1.	*	1 JAN 2330	706	1.
1 JAN 0444	143	1.	*	1 JAN 1100	331	1.	*	1 JAN 1716	519	1.	*	1 JAN 2332	707	1.
1 JAN 0446	144	1.	*	1 JAN 1102	332	1.	*	1 JAN 1718	520	1.	*	1 JAN 2334	708	1.
1 JAN 0448	145	1.	*	1 JAN 1104	333	1.	*	1 JAN 1720	521	1.	*	1 JAN 2336	709	1.
1 JAN 0450	146	1.	*	1 JAN 1106	334	1.	*	1 JAN 1722	522	1.	*	1 JAN 2338	710	1.
1 JAN 0452	147	1.	*	1 JAN 1108	335	1.	*	1 JAN 1724	523	1.	*	1 JAN 2340	711	1.
1 JAN 0454	148	1.	*	1 JAN 1110	336	1.	*	1 JAN 1726	524	1.	*	1 JAN 2342	712	1.
1 JAN 0456	149	1.	*	1 JAN 1112	337	1.	*	1 JAN 1728	525	1.	*	1 JAN 2344	713	1.
1 JAN 0458	150	1.	*	1 JAN 1114	338	1.	*	1 JAN 1730	526	1.	*	1 JAN 2346	714	1.
1 JAN 0500	151	1.	*	1 JAN 1116	339	1.	*	1 JAN 1732	527	1.	*	1 JAN 2348	715	1.
1 JAN 0502	152	1.	*	1 JAN 1118	340	1.	*	1 JAN 1734	528	1.	*	1 JAN 2350	716	1.
1 JAN 0504	153	1.	*	1 JAN 1120	341	1.	*	1 JAN 1736	529	1.	*	1 JAN 2352	717	1.
1 JAN 0506	154	1.	*	1 JAN 1122	342	1.	*	1 JAN 1738	530	1.	*	1 JAN 2354	718	1.
1 JAN 0508	155	1.	*	1 JAN 1124	343	1.	*	1 JAN 1740	531	1.	*	1 JAN 2356	719	1.
1 JAN 0510	156	1.	*	1 JAN 1126	344	1.	*	1 JAN 1742	532	1.	*	1 JAN 2358	720	1.
1 JAN 0512	157	1.	*	1 JAN 1128	345	1.	*	1 JAN 1744	533	1.	*	2 JAN 0000	721	1.
1 JAN 0514	158	1.	*	1 JAN 1130	346	1.	*	1 JAN 1746	534	1.	*	2 JAN 0002	722	1.
1 JAN 0516	159	1.	*	1 JAN 1132	347	1.	*	1 JAN 1748	535	1.	*	2 JAN 0004	723	1.
1 JAN 0518	160	1.	*	1 JAN 1134	348	1.	*	1 JAN 1750	536	1.	*	2 JAN 0006	724	1.
1 JAN 0520	161	1.	*	1 JAN 1136	349	1.	*	1 JAN 1752	537	1.	*	2 JAN 0008	725	1.
1 JAN 0522	162	1.	*	1 JAN 1138	350	1.	*	1 JAN 1754	538	1.	*	2 JAN 0010	726	1.
1 JAN 0524	163	1.	*	1 JAN 1140	351	1.	*	1 JAN 1756	539	1.	*	2 JAN 0012	727	1.
1 JAN 0526	164	1.	*	1 JAN 1142	352	1.	*	1 JAN 1758	540	1.	*	2 JAN 0014	728	1.
1 JAN 0528	165	1.	*	1 JAN 1144	353	1.	*	1 JAN 1800	541	1.	*	2 JAN 0016	729	1.
1 JAN 0530	166	1.	*	1 JAN 1146	354	1.	*	1 JAN 1802	542	1.	*	2 JAN 0018	730	1.
1 JAN 0532	167	1.	*	1 JAN 1148	355	1.	*	1 JAN 1804	543	1.	*	2 JAN 0020	731	1.
1 JAN 0534	168	1.	*	1 JAN 1150	356	1.	*	1 JAN 1806	544	1.	*	2 JAN 0022	732	1.
1 JAN 0536	169	1.	*	1 JAN 1152	357	1.	*	1 JAN 1808	545	1.	*	2 JAN 0024	733	1.
1 JAN 0538	170	1.	*	1 JAN 1154	358	1.	*	1 JAN 1810	546	1.	*	2 JAN 0026	734	1.
1 JAN 0540	171	1.	*	1 JAN 1156	359	1.	*	1 JAN 1812	547	1.	*	2 JAN 0028	735	1.
1 JAN 0542	172	1.	*	1 JAN 1158	360	1.	*	1 JAN 1814	548	1.	*	2 JAN 0030	736	1.
1 JAN 0544	173	1.	*	1 JAN 1200	361	1.	*	1 JAN 1816	549	1.	*	2 JAN 0032	737	1.
1 JAN 0546	174	1.	*	1 JAN 1202	362	1.	*	1 JAN 1818	550	1.	*	2 JAN 0034	738	1.
1 JAN 0548	175	1.	*	1 JAN 1204	363	1.	*	1 JAN 1820	551	1.	*	2 JAN 0036	739	1.
1 JAN 0550	176	1.	*	1 JAN 1206	364	1.	*	1 JAN 1822	552	1.	*	2 JAN 0038	740	1.
1 JAN 0552	177	1.	*	1 JAN 1208	365	1.	*	1 JAN 1824	553	1.	*	2 JAN 0040	741	1.
1 JAN 0554	178	1.	*	1 JAN 1210	366	1.	*	1 JAN 1826	554	1.	*	2 JAN 0042	742	1.
1 JAN 0556	179	1.	*	1 JAN 1212	367	1.	*	1 JAN 1828	555	1.	*	2 JAN 0044	743	1.
1 JAN 0558	180	1.	*	1 JAN 1214	368	1.	*	1 JAN 1830	556	1.	*	2 JAN 0046	744	1.
1 JAN 0600	181	1.	*	1 JAN 1216	369	1.	*	1 JAN 1832	557	1.	*	2 JAN 0048	745	1.
1 JAN 0602	182	1.	*	1 JAN 1218	370	1.	*	1 JAN 1834	558	1.	*	2 JAN 0050	746	1.
1 JAN 0604	183	1.	*	1 JAN 1220	371	1.	*	1 JAN 1836	559	1.	*	2 JAN 0052	747	1.
1 JAN 0606	184	1.	*	1 JAN 1222	372	1.	*	1 JAN 1838	560	1.	*	2 JAN 0054	748	1.
1 JAN 0608	185	1.	*	1 JAN 1224	373	1.	*	1 JAN 1840	561	1.	*	2 JAN 0056	749	1.
1 JAN 0610	186	1.	*	1 JAN 1226	374	1.	*	1 JAN 1842	562	1.	*	2 JAN 0058	750	1.
1 JAN 0612	187	1.	*	1 JAN 1228	375	1.	*	1 JAN 1844	563	1.	*			
1 JAN 0614	188	1.	*	1 JAN 1230	376	1.	*	1 JAN 1846	564	1.	*			

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PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
12.	.17	2.	1.	1.	1.
		3.637 (INCHES)	12.005 (AC-FT)	12.455 3.	12.455 3.
CUMULATIVE AREA =		.00 SQ MI			

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49 KK \*\*\*\*\*  
 \* ON06 \* BASIN  
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9 IN TIME DATA FOR INPUT TIME SERIES  
 JXMIN 5 TIME INTERVAL IN MINUTES  
 JXDATE 1JAN99 STARTING DATE  
 JXTIME 0 STARTING TIME

SUBBASIN RUNOFF DATA

50 BA SUBBASIN CHARACTERISTICS  
 TAREA .00 SUBBASIN AREA

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 HYDROGRAPH AT STATION ON06  
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DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW
1	JAN	0000	1	0.	1	JAN	0616	189	1.	1	JAN	1232	377	1.	1	JAN	1848	565	1.
1	JAN	0002	2	3.	1	JAN	0618	190	1.	1	JAN	1234	378	1.	1	JAN	1850	566	1.
1	JAN	0004	3	6.	1	JAN	0620	191	1.	1	JAN	1236	379	1.	1	JAN	1852	567	1.
1	JAN	0006	4	8.	1	JAN	0622	192	1.	1	JAN	1238	380	1.	1	JAN	1854	568	1.
1	JAN	0008	5	11.	1	JAN	0624	193	1.	1	JAN	1240	381	1.	1	JAN	1856	569	1.
1	JAN	0010	6	14.	1	JAN	0626	194	1.	1	JAN	1242	382	1.	1	JAN	1858	570	1.
1	JAN	0012	7	12.	1	JAN	0628	195	1.	1	JAN	1244	383	1.	1	JAN	1900	571	1.
1	JAN	0014	8	10.	1	JAN	0630	196	1.	1	JAN	1246	384	1.	1	JAN	1902	572	1.
1	JAN	0016	9	9.	1	JAN	0632	197	1.	1	JAN	1248	385	1.	1	JAN	1904	573	1.
1	JAN	0018	10	7.	1	JAN	0634	198	1.	1	JAN	1250	386	1.	1	JAN	1906	574	1.
1	JAN	0020	11	6.	1	JAN	0636	199	1.	1	JAN	1252	387	1.	1	JAN	1908	575	1.
1	JAN	0022	12	4.	1	JAN	0638	200	1.	1	JAN	1254	388	1.	1	JAN	1910	576	1.
1	JAN	0024	13	2.	1	JAN	0640	201	1.	1	JAN	1256	389	1.	1	JAN	1912	577	1.
1	JAN	0026	14	1.	1	JAN	0642	202	1.	1	JAN	1258	390	1.	1	JAN	1914	578	1.
1	JAN	0028	15	1.	1	JAN	0644	203	1.	1	JAN	1300	391	1.	1	JAN	1916	579	1.
1	JAN	0030	16	1.	1	JAN	0646	204	1.	1	JAN	1302	392	1.	1	JAN	1918	580	1.
1	JAN	0032	17	1.	1	JAN	0648	205	1.	1	JAN	1304	393	1.	1	JAN	1920	581	1.
1	JAN	0034	18	1.	1	JAN	0650	206	1.	1	JAN	1306	394	1.	1	JAN	1922	582	1.
1	JAN	0036	19	1.	1	JAN	0652	207	1.	1	JAN	1308	395	1.	1	JAN	1924	583	1.
1	JAN	0038	20	1.	1	JAN	0654	208	1.	1	JAN	1310	396	1.	1	JAN	1926	584	1.
1	JAN	0040	21	1.	1	JAN	0656	209	1.	1	JAN	1312	397	1.	1	JAN	1928	585	1.
1	JAN	0042	22	1.	1	JAN	0658	210	1.	1	JAN	1314	398	1.	1	JAN	1930	586	1.
1	JAN	0044	23	1.	1	JAN	0700	211	1.	1	JAN	1316	399	1.	1	JAN	1932	587	1.
1	JAN	0046	24	1.	1	JAN	0702	212	1.	1	JAN	1318	400	1.	1	JAN	1934	588	1.
1	JAN	0048	25	1.	1	JAN	0704	213	1.	1	JAN	1320	401	1.	1	JAN	1936	589	1.
1	JAN	0050	26	1.	1	JAN	0706	214	1.	1	JAN	1322	402	1.	1	JAN	1938	590	1.
1	JAN	0052	27	1.	1	JAN	0708	215	1.	1	JAN	1324	403	1.	1	JAN	1940	591	1.
1	JAN	0054	28	1.	1	JAN	0710	216	1.	1	JAN	1326	404	1.	1	JAN	1942	592	1.
1	JAN	0056	29	1.	1	JAN	0712	217	1.	1	JAN	1328	405	1.	1	JAN	1944	593	1.
1	JAN	0058	30	1.	1	JAN	0714	218	1.	1	JAN	1330	406	1.	1	JAN	1946	594	1.
1	JAN	0100	31	1.	1	JAN	0716	219	1.	1	JAN	1332	407	1.	1	JAN	1948	595	1.
1	JAN	0102	32	1.	1	JAN	0718	220	1.	1	JAN	1334	408	1.	1	JAN	1950	596	1.
1	JAN	0104	33	1.	1	JAN	0720	221	1.	1	JAN	1336	409	1.	1	JAN	1952	597	1.
1	JAN	0106	34	1.	1	JAN	0722	222	1.	1	JAN	1338	410	1.	1	JAN	1954	598	1.
1	JAN	0108	35	1.	1	JAN	0724	223	1.	1	JAN	1340	411	1.	1	JAN	1956	599	1.
1	JAN	0110	36	1.	1	JAN	0726	224	1.	1	JAN	1342	412	1.	1	JAN	1958	600	1.
1	JAN	0112	37	1.	1	JAN	0728	225	1.	1	JAN	1344	413	1.	1	JAN	2000	601	1.
1	JAN	0114	38	1.	1	JAN	0730	226	1.	1	JAN	1346	414	1.	1	JAN	2002	602	1.
1	JAN	0116	39	1.	1	JAN	0732	227	1.	1	JAN	1348	415	1.	1	JAN	2004	603	1.
1	JAN	0118	40	1.	1	JAN	0734	228	1.	1	JAN	1350	416	1.	1	JAN	2006	604	1.
1	JAN	0120	41	1.	1	JAN	0736	229	1.	1	JAN	1352	417	1.	1	JAN	2008	605	1.
1	JAN	0122	42	1.	1	JAN	0738	230	1.	1	JAN	1354	418	1.	1	JAN	2010	606	1.
1	JAN	0124	43	1.	1	JAN	0740	231	1.	1	JAN	1356	419	1.	1	JAN	2012	607	1.
1	JAN	0126	44	1.	1	JAN	0742	232	1.	1	JAN	1358	420	1.	1	JAN	2014	608	1.
1	JAN	0128	45	1.	1	JAN	0744	233	1.	1	JAN	1400	421	1.	1	JAN	2016	609	1.
1	JAN	0130	46	1.	1	JAN	0746	234	1.	1	JAN	1402	422	1.	1	JAN	2018	610	1.
1	JAN	0132	47	1.	1	JAN	0748	235	1.	1	JAN	1404	423	1.	1	JAN	2020	611	1.
1	JAN	0134	48	1.	1	JAN	0750	236	1.	1	JAN	1406	424	1.	1	JAN	2022	612	1.
1	JAN	0136	49	1.	1	JAN	0752	237	1.	1	JAN	1408	425	1.	1	JAN	2024	613	1.
1	JAN	0138	50	1.	1	JAN	0754	238	1.	1	JAN	1410	426	1.	1	JAN	2026	614	1.
1	JAN	0140	51	1.	1	JAN	0756	239	1.	1	JAN	1412	427	1.	1	JAN	2028	615	1.
1	JAN	0142	52	1.	1	JAN	0758	240	1.	1	JAN	1414	428	1.	1	JAN	2030	616	1.
1	JAN	0144	53	1.	1	JAN	0800	241	1.	1	JAN	1416	429	1.	1	JAN	2032	617	1.
1	JAN	0146	54	1.	1	JAN	0802	242	1.	1	JAN	1418	430	1.	1	JAN	2034	618	1.
1	JAN	0148	55	1.	1	JAN	0804	243	1.	1	JAN	1420	431	1.	1	JAN	2036	619	1.
1	JAN	0150	56	1.	1	JAN	0806	244	1.	1	JAN	1422	432	1.	1	JAN	2038	620	1.
1	JAN	0152	57	1.	1	JAN	0808	245	1.	1	JAN	1424	433	1.	1	JAN	2040	621	1.
1	JAN	0154	58	1.	1	JAN	0810	246	1.	1	JAN	1426	434	1.	1	JAN	2042	622	1.
1	JAN	0156	59	1.	1	JAN	0812	247	1.	1	JAN	1428	435	1.	1	JAN	2044	623	1.
1	JAN	0158	60	1.	1	JAN	0814	248	1.	1	JAN	1430	436	1.	1	JAN	2046	624	1.
1	JAN	0200	61	1.	1	JAN	0816	249	1.	1	JAN	1432	437	1.	1	JAN	2048	625	1.
1	JAN	0202	62	1.	1	JAN	0818	250	1.	1	JAN	1434	438	1.	1	JAN	2050	626	1.
1	JAN	0204	63	1.	1	JAN	0820	251	1.	1	JAN	1436	439	1.	1	JAN	2052	627	1.
1	JAN	0206	64	1.	1	JAN	0822	252	1.	1	JAN	1438	440	1.	1	JAN	2054	628	1.



1 JAN 0518	160	1.	*	1 JAN 1134	348	1.	*	1 JAN 1750	536	1.	*	2 JAN 0006	724	1.
1 JAN 0520	161	1.	*	1 JAN 1136	349	1.	*	1 JAN 1752	537	1.	*	2 JAN 0008	725	1.
1 JAN 0522	162	1.	*	1 JAN 1138	350	1.	*	1 JAN 1754	538	1.	*	2 JAN 0010	726	1.
1 JAN 0524	163	1.	*	1 JAN 1140	351	1.	*	1 JAN 1756	539	1.	*	2 JAN 0012	727	1.
1 JAN 0526	164	1.	*	1 JAN 1142	352	1.	*	1 JAN 1758	540	1.	*	2 JAN 0014	728	1.
1 JAN 0528	165	1.	*	1 JAN 1144	353	1.	*	1 JAN 1800	541	1.	*	2 JAN 0016	729	1.
1 JAN 0530	166	1.	*	1 JAN 1146	354	1.	*	1 JAN 1802	542	1.	*	2 JAN 0018	730	1.
1 JAN 0532	167	1.	*	1 JAN 1148	355	1.	*	1 JAN 1804	543	1.	*	2 JAN 0020	731	1.
1 JAN 0534	168	1.	*	1 JAN 1150	356	1.	*	1 JAN 1806	544	1.	*	2 JAN 0022	732	1.
1 JAN 0536	169	1.	*	1 JAN 1152	357	1.	*	1 JAN 1808	545	1.	*	2 JAN 0024	733	1.
1 JAN 0538	170	1.	*	1 JAN 1154	358	1.	*	1 JAN 1810	546	1.	*	2 JAN 0026	734	1.
1 JAN 0540	171	1.	*	1 JAN 1156	359	1.	*	1 JAN 1812	547	1.	*	2 JAN 0028	735	1.
1 JAN 0542	172	1.	*	1 JAN 1158	360	1.	*	1 JAN 1814	548	1.	*	2 JAN 0030	736	1.
1 JAN 0544	173	1.	*	1 JAN 1200	361	1.	*	1 JAN 1816	549	1.	*	2 JAN 0032	737	1.
1 JAN 0546	174	1.	*	1 JAN 1202	362	1.	*	1 JAN 1818	550	1.	*	2 JAN 0034	738	1.
1 JAN 0548	175	1.	*	1 JAN 1204	363	1.	*	1 JAN 1820	551	1.	*	2 JAN 0036	739	1.
1 JAN 0550	176	1.	*	1 JAN 1206	364	1.	*	1 JAN 1822	552	1.	*	2 JAN 0038	740	1.
1 JAN 0552	177	1.	*	1 JAN 1208	365	1.	*	1 JAN 1824	553	1.	*	2 JAN 0040	741	1.
1 JAN 0554	178	1.	*	1 JAN 1210	366	1.	*	1 JAN 1826	554	1.	*	2 JAN 0042	742	1.
1 JAN 0556	179	1.	*	1 JAN 1212	367	1.	*	1 JAN 1828	555	1.	*	2 JAN 0044	743	1.
1 JAN 0558	180	1.	*	1 JAN 1214	368	1.	*	1 JAN 1830	556	1.	*	2 JAN 0046	744	1.
1 JAN 0600	181	1.	*	1 JAN 1216	369	1.	*	1 JAN 1832	557	1.	*	2 JAN 0048	745	1.
1 JAN 0602	182	1.	*	1 JAN 1218	370	1.	*	1 JAN 1834	558	1.	*	2 JAN 0050	746	1.
1 JAN 0604	183	1.	*	1 JAN 1220	371	1.	*	1 JAN 1836	559	1.	*	2 JAN 0052	747	1.
1 JAN 0606	184	1.	*	1 JAN 1222	372	1.	*	1 JAN 1838	560	1.	*	2 JAN 0054	748	1.
1 JAN 0608	185	1.	*	1 JAN 1224	373	1.	*	1 JAN 1840	561	1.	*	2 JAN 0056	749	1.
1 JAN 0610	186	1.	*	1 JAN 1226	374	1.	*	1 JAN 1842	562	1.	*	2 JAN 0058	750	1.
1 JAN 0612	187	1.	*	1 JAN 1228	375	1.	*	1 JAN 1844	563	1.	*			
1 JAN 0614	188	1.	*	1 JAN 1230	376	1.	*	1 JAN 1846	564	1.	*			

\*\*\*\*\*

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW				
(CFS)	(HR)	6-HR	24-HR	72-HR	24.97-HR	
14.	.17	2.	2.	1.	1.	
		(INCHES)	4.206	13.969	14.493	14.493
		(AC-FT)	1.	3.	3.	3.
CUMULATIVE AREA =		.00 SQ MI				

\*\*\*\*\*

52 KK \*\*\*\*\*  
 \* 2BIN \*  
 \* \*  
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INFLOW TO RETENTION BASIN 2B

54 HC HYDROGRAPH COMBINATION  
 ICOMP 3 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION 2BIN  
 SUM OF 3 HYDROGRAPHS

DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW
1	JAN	0000	1	0.	1	JAN	0616	189	9.	1	JAN	1232	377	9.	1	JAN	1848	565	9.
1	JAN	0002	2	5.	1	JAN	0618	190	9.	1	JAN	1234	378	9.	1	JAN	1850	566	9.
1	JAN	0004	3	10.	1	JAN	0620	191	9.	1	JAN	1236	379	9.	1	JAN	1852	567	9.
1	JAN	0006	4	16.	1	JAN	0622	192	9.	1	JAN	1238	380	9.	1	JAN	1854	568	9.
1	JAN	0008	5	21.	1	JAN	0624	193	9.	1	JAN	1240	381	9.	1	JAN	1856	569	9.
1	JAN	0010	6	26.	1	JAN	0626	194	9.	1	JAN	1242	382	9.	1	JAN	1858	570	9.
1	JAN	0012	7	23.	1	JAN	0628	195	9.	1	JAN	1244	383	9.	1	JAN	1900	571	9.
1	JAN	0014	8	20.	1	JAN	0630	196	9.	1	JAN	1246	384	9.	1	JAN	1902	572	9.
1	JAN	0016	9	17.	1	JAN	0632	197	9.	1	JAN	1248	385	9.	1	JAN	1904	573	9.
1	JAN	0018	10	14.	1	JAN	0634	198	9.	1	JAN	1250	386	9.	1	JAN	1906	574	9.
1	JAN	0020	11	11.	1	JAN	0636	199	9.	1	JAN	1252	387	9.	1	JAN	1908	575	9.
1	JAN	0022	12	8.	1	JAN	0638	200	9.	1	JAN	1254	388	9.	1	JAN	1910	576	9.
1	JAN	0024	13	5.	1	JAN	0640	201	9.	1	JAN	1256	389	9.	1	JAN	1912	577	9.
1	JAN	0026	14	4.	1	JAN	0642	202	9.	1	JAN	1258	390	9.	1	JAN	1914	578	9.
1	JAN	0028	15	4.	1	JAN	0644	203	9.	1	JAN	1300	391	9.	1	JAN	1916	579	9.
1	JAN	0030	16	4.	1	JAN	0646	204	9.	1	JAN	1302	392	9.	1	JAN	1918	580	9.
1	JAN	0032	17	4.	1	JAN	0648	205	9.	1	JAN	1304	393	9.	1	JAN	1920	581	9.
1	JAN	0034	18	4.	1	JAN	0650	206	9.	1	JAN	1306	394	9.	1	JAN	1922	582	9.
1	JAN	0036	19	4.	1	JAN	0652	207	9.	1	JAN	1308	395	9.	1	JAN	1924	583	9.

1 JAN 0038	20	4.	*	1 JAN 0654	208	9.	*	1 JAN 1310	396	9.	*	1 JAN 1926	584	9.
1 JAN 0040	21	4.	*	1 JAN 0656	209	9.	*	1 JAN 1312	397	9.	*	1 JAN 1928	585	9.
1 JAN 0042	22	4.	*	1 JAN 0658	210	9.	*	1 JAN 1314	398	9.	*	1 JAN 1930	586	9.
1 JAN 0044	23	4.	*	1 JAN 0700	211	9.	*	1 JAN 1316	399	9.	*	1 JAN 1932	587	9.
1 JAN 0046	24	5.	*	1 JAN 0702	212	9.	*	1 JAN 1318	400	9.	*	1 JAN 1934	588	9.
1 JAN 0048	25	6.	*	1 JAN 0704	213	9.	*	1 JAN 1320	401	9.	*	1 JAN 1936	589	9.
1 JAN 0050	26	7.	*	1 JAN 0706	214	9.	*	1 JAN 1322	402	9.	*	1 JAN 1938	590	9.
1 JAN 0052	27	7.	*	1 JAN 0708	215	9.	*	1 JAN 1324	403	9.	*	1 JAN 1940	591	9.
1 JAN 0054	28	8.	*	1 JAN 0710	216	9.	*	1 JAN 1326	404	9.	*	1 JAN 1942	592	9.
1 JAN 0056	29	8.	*	1 JAN 0712	217	9.	*	1 JAN 1328	405	9.	*	1 JAN 1944	593	9.
1 JAN 0058	30	8.	*	1 JAN 0714	218	9.	*	1 JAN 1330	406	9.	*	1 JAN 1946	594	9.
1 JAN 0100	31	8.	*	1 JAN 0716	219	9.	*	1 JAN 1332	407	9.	*	1 JAN 1948	595	9.
1 JAN 0102	32	8.	*	1 JAN 0718	220	9.	*	1 JAN 1334	408	9.	*	1 JAN 1950	596	9.
1 JAN 0104	33	8.	*	1 JAN 0720	221	9.	*	1 JAN 1336	409	9.	*	1 JAN 1952	597	9.
1 JAN 0106	34	8.	*	1 JAN 0722	222	9.	*	1 JAN 1338	410	9.	*	1 JAN 1954	598	9.
1 JAN 0108	35	8.	*	1 JAN 0724	223	9.	*	1 JAN 1340	411	9.	*	1 JAN 1956	599	9.
1 JAN 0110	36	8.	*	1 JAN 0726	224	9.	*	1 JAN 1342	412	9.	*	1 JAN 1958	600	9.
1 JAN 0112	37	8.	*	1 JAN 0728	225	9.	*	1 JAN 1344	413	9.	*	1 JAN 2000	601	9.
1 JAN 0114	38	8.	*	1 JAN 0730	226	9.	*	1 JAN 1346	414	9.	*	1 JAN 2002	602	9.
1 JAN 0116	39	8.	*	1 JAN 0732	227	9.	*	1 JAN 1348	415	9.	*	1 JAN 2004	603	9.
1 JAN 0118	40	8.	*	1 JAN 0734	228	9.	*	1 JAN 1350	416	9.	*	1 JAN 2006	604	9.
1 JAN 0120	41	8.	*	1 JAN 0736	229	9.	*	1 JAN 1352	417	9.	*	1 JAN 2008	605	9.
1 JAN 0122	42	8.	*	1 JAN 0738	230	9.	*	1 JAN 1354	418	9.	*	1 JAN 2010	606	9.
1 JAN 0124	43	8.	*	1 JAN 0740	231	9.	*	1 JAN 1356	419	9.	*	1 JAN 2012	607	9.
1 JAN 0126	44	8.	*	1 JAN 0742	232	9.	*	1 JAN 1358	420	9.	*	1 JAN 2014	608	9.
1 JAN 0128	45	8.	*	1 JAN 0744	233	9.	*	1 JAN 1400	421	9.	*	1 JAN 2016	609	9.
1 JAN 0130	46	8.	*	1 JAN 0746	234	9.	*	1 JAN 1402	422	9.	*	1 JAN 2018	610	9.
1 JAN 0132	47	8.	*	1 JAN 0748	235	9.	*	1 JAN 1404	423	9.	*	1 JAN 2020	611	9.
1 JAN 0134	48	8.	*	1 JAN 0750	236	9.	*	1 JAN 1406	424	9.	*	1 JAN 2022	612	9.
1 JAN 0136	49	8.	*	1 JAN 0752	237	9.	*	1 JAN 1408	425	9.	*	1 JAN 2024	613	9.
1 JAN 0138	50	8.	*	1 JAN 0754	238	9.	*	1 JAN 1410	426	9.	*	1 JAN 2026	614	9.
1 JAN 0140	51	8.	*	1 JAN 0756	239	9.	*	1 JAN 1412	427	9.	*	1 JAN 2028	615	9.
1 JAN 0142	52	8.	*	1 JAN 0758	240	9.	*	1 JAN 1414	428	9.	*	1 JAN 2030	616	9.
1 JAN 0144	53	8.	*	1 JAN 0800	241	9.	*	1 JAN 1416	429	9.	*	1 JAN 2032	617	9.
1 JAN 0146	54	8.	*	1 JAN 0802	242	9.	*	1 JAN 1418	430	9.	*	1 JAN 2034	618	9.
1 JAN 0148	55	8.	*	1 JAN 0804	243	9.	*	1 JAN 1420	431	9.	*	1 JAN 2036	619	9.
1 JAN 0150	56	8.	*	1 JAN 0806	244	9.	*	1 JAN 1422	432	9.	*	1 JAN 2038	620	9.
1 JAN 0152	57	8.	*	1 JAN 0808	245	9.	*	1 JAN 1424	433	9.	*	1 JAN 2040	621	9.
1 JAN 0154	58	8.	*	1 JAN 0810	246	9.	*	1 JAN 1426	434	9.	*	1 JAN 2042	622	9.
1 JAN 0156	59	8.	*	1 JAN 0812	247	9.	*	1 JAN 1428	435	9.	*	1 JAN 2044	623	9.
1 JAN 0158	60	8.	*	1 JAN 0814	248	9.	*	1 JAN 1430	436	9.	*	1 JAN 2046	624	9.
1 JAN 0200	61	8.	*	1 JAN 0816	249	9.	*	1 JAN 1432	437	9.	*	1 JAN 2048	625	9.
1 JAN 0202	62	8.	*	1 JAN 0818	250	9.	*	1 JAN 1434	438	9.	*	1 JAN 2050	626	9.
1 JAN 0204	63	8.	*	1 JAN 0820	251	9.	*	1 JAN 1436	439	9.	*	1 JAN 2052	627	9.
1 JAN 0206	64	8.	*	1 JAN 0822	252	9.	*	1 JAN 1438	440	9.	*	1 JAN 2054	628	9.
1 JAN 0208	65	8.	*	1 JAN 0824	253	9.	*	1 JAN 1440	441	9.	*	1 JAN 2056	629	9.
1 JAN 0210	66	8.	*	1 JAN 0826	254	9.	*	1 JAN 1442	442	9.	*	1 JAN 2058	630	9.
1 JAN 0212	67	8.	*	1 JAN 0828	255	9.	*	1 JAN 1444	443	9.	*	1 JAN 2100	631	9.
1 JAN 0214	68	8.	*	1 JAN 0830	256	9.	*	1 JAN 1446	444	9.	*	1 JAN 2102	632	9.
1 JAN 0216	69	8.	*	1 JAN 0832	257	9.	*	1 JAN 1448	445	9.	*	1 JAN 2104	633	9.
1 JAN 0218	70	8.	*	1 JAN 0834	258	9.	*	1 JAN 1450	446	9.	*	1 JAN 2106	634	9.
1 JAN 0220	71	8.	*	1 JAN 0836	259	9.	*	1 JAN 1452	447	9.	*	1 JAN 2108	635	9.
1 JAN 0222	72	8.	*	1 JAN 0838	260	9.	*	1 JAN 1454	448	9.	*	1 JAN 2110	636	9.
1 JAN 0224	73	8.	*	1 JAN 0840	261	9.	*	1 JAN 1456	449	9.	*	1 JAN 2112	637	9.
1 JAN 0226	74	8.	*	1 JAN 0842	262	9.	*	1 JAN 1458	450	9.	*	1 JAN 2114	638	9.
1 JAN 0228	75	8.	*	1 JAN 0844	263	9.	*	1 JAN 1500	451	9.	*	1 JAN 2116	639	9.
1 JAN 0230	76	8.	*	1 JAN 0846	264	9.	*	1 JAN 1502	452	9.	*	1 JAN 2118	640	9.
1 JAN 0232	77	8.	*	1 JAN 0848	265	9.	*	1 JAN 1504	453	9.	*	1 JAN 2120	641	9.
1 JAN 0234	78	8.	*	1 JAN 0850	266	9.	*	1 JAN 1506	454	9.	*	1 JAN 2122	642	9.
1 JAN 0236	79	8.	*	1 JAN 0852	267	9.	*	1 JAN 1508	455	9.	*	1 JAN 2124	643	9.
1 JAN 0238	80	8.	*	1 JAN 0854	268	9.	*	1 JAN 1510	456	9.	*	1 JAN 2126	644	9.
1 JAN 0240	81	8.	*	1 JAN 0856	269	9.	*	1 JAN 1512	457	9.	*	1 JAN 2128	645	9.
1 JAN 0242	82	8.	*	1 JAN 0858	270	9.	*	1 JAN 1514	458	9.	*	1 JAN 2130	646	9.
1 JAN 0244	83	8.	*	1 JAN 0900	271	9.	*	1 JAN 1516	459	9.	*	1 JAN 2132	647	9.
1 JAN 0246	84	8.	*	1 JAN 0902	272	9.	*	1 JAN 1518	460	9.	*	1 JAN 2134	648	9.
1 JAN 0248	85	8.	*	1 JAN 0904	273	9.	*	1 JAN 1520	461	9.	*	1 JAN 2136	649	9.
1 JAN 0250	86	9.	*	1 JAN 0906	274	9.	*	1 JAN 1522	462	9.	*	1 JAN 2138	650	9.
1 JAN 0252	87	9.	*	1 JAN 0908	275	9.	*	1 JAN 1524	463	9.	*	1 JAN 2140	651	9.
1 JAN 0254	88	9.	*	1 JAN 0910	276	9.	*	1 JAN 1526	464	9.	*	1 JAN 2142	652	9.
1 JAN 0256	89	9.	*	1 JAN 0912	277	9.	*	1 JAN 1528	465	9.	*	1 JAN 2144	653	9.
1 JAN 0258	90	9.	*	1 JAN 0914	278	9.	*	1 JAN 1530	466	9.	*	1 JAN 2146	654	9.
1 JAN 0300	91	9.	*	1 JAN 0916	279	9.	*	1 JAN 1532	467	9.	*	1 JAN 2148	655	9.
1 JAN 0302	92	9.	*	1 JAN 0918	280	9.	*	1 JAN 1534	468	9.	*	1 JAN 2150	656	9.
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1 JAN 0306	94	9.	*	1 JAN 0922	282	9.	*	1 JAN 1538	470	9.	*	1 JAN 2154	658	9.
1 JAN 0308	95	9.	*	1 JAN 0924	283	9.	*	1 JAN 1540	471	9.	*	1 JAN 2156	659	9.
1 JAN 0310	96	9.	*	1 JAN 0926	284	9.	*	1 JAN 1542	472	9.	*	1 JAN 2158	660	9.
1 JAN 0312	97	9.	*	1 JAN 0928	285	9.	*	1 JAN 1544	473	9.	*	1 JAN 2200	661	9.
1 JAN 0314	98	9.	*	1 JAN 0930	286	9.	*	1 JAN 1546	474	9.	*	1 JAN 2202	662	9.
1 JAN 0316	99	9.	*	1 JAN 0932	287	9.	*	1 JAN 1548	475	9.	*	1 JAN 2204	663	9.
1 JAN 0318	100	9.	*	1 JAN 0934	288	9.	*	1 JAN 1550	476	9.	*	1 JAN 2206	664	9.
1 JAN 0320	101	9.	*	1 JAN 0936	289	9.	*	1 JAN 1552	477	9.	*	1 JAN 2208	665	9.
1 JAN 0322	102	9.	*	1 JAN 0938	290	9.	*	1 JAN 1554	478	9.	*	1 JAN 2210	666	9.
1 JAN 0324	103	9.	*	1 JAN 0940	291	9.	*	1 JAN 1556	479	9.	*	1 JAN 2212	667	9.
1 JAN 0326	104	9.	*	1 JAN 0942	292	9.	*	1 JAN 1558	480	9.	*	1 JAN 2214	668	9.
1 JAN 0328	105	9.	*	1 JAN 0944	293	9.	*	1 JAN 1600	481	9.	*	1 JAN 2216	669	9.
1 JAN 0330	106	9.	*	1 JAN 0946	294	9.	*	1 JAN 1602	482	9.	*	1 JAN 2218	670	9.
1 JAN 0332	107	9.	*	1 JAN 0948	295	9.	*	1 JAN 1604	483	9.	*	1 JAN 2220	671	9.
1 JAN 0334	108	9.	*	1 JAN 0950	296	9.	*	1 JAN 1606	484	9.	*	1 JAN 2222	672	9.
1 JAN 0336	109	9.	*	1 JAN 0952	297	9.	*	1 JAN 1608	485	9.	*	1 JAN 2224	673	9.
1 JAN 0338	110	9.	*	1 JAN 0954	298	9.	*	1 JAN 1610	486	9.	*	1 JAN 2226	674	9.
1 JAN 0340	111	9.	*	1 JAN 0956	299	9.	*	1 JAN 1612	487	9.	*	1 JAN 2228	675	9.
1 JAN 0342	112	9.	*	1 JAN 0958	300	9.	*	1 JAN 1614	488	9.	*	1 JAN 2230	676	9.
1 JAN 0344	113	9.	*	1 JAN 1000	301	9.	*	1 JAN 1616	489	9.	*	1 JAN 2232	677	9.
1 JAN 0346	114	9.	*	1 JAN 1002	302	9.	*	1 JAN 1618	490	9.	*	1 JAN 2234	678	9.

1 JAN 0348	115	9.	*	1 JAN 1004	303	9.	*	1 JAN 1620	491	9.	*	1 JAN 2236	679	9.
1 JAN 0350	116	9.	*	1 JAN 1006	304	9.	*	1 JAN 1622	492	9.	*	1 JAN 2238	680	9.
1 JAN 0352	117	9.	*	1 JAN 1008	305	9.	*	1 JAN 1624	493	9.	*	1 JAN 2240	681	9.
1 JAN 0354	118	9.	*	1 JAN 1010	306	9.	*	1 JAN 1626	494	9.	*	1 JAN 2242	682	9.
1 JAN 0356	119	9.	*	1 JAN 1012	307	9.	*	1 JAN 1628	495	9.	*	1 JAN 2244	683	9.
1 JAN 0358	120	9.	*	1 JAN 1014	308	9.	*	1 JAN 1630	496	9.	*	1 JAN 2246	684	9.
1 JAN 0400	121	9.	*	1 JAN 1016	309	9.	*	1 JAN 1632	497	9.	*	1 JAN 2248	685	9.
1 JAN 0402	122	9.	*	1 JAN 1018	310	9.	*	1 JAN 1634	498	9.	*	1 JAN 2250	686	9.
1 JAN 0404	123	9.	*	1 JAN 1020	311	9.	*	1 JAN 1636	499	9.	*	1 JAN 2252	687	9.
1 JAN 0406	124	9.	*	1 JAN 1022	312	9.	*	1 JAN 1638	500	9.	*	1 JAN 2254	688	9.
1 JAN 0408	125	9.	*	1 JAN 1024	313	9.	*	1 JAN 1640	501	9.	*	1 JAN 2256	689	9.
1 JAN 0410	126	9.	*	1 JAN 1026	314	9.	*	1 JAN 1642	502	9.	*	1 JAN 2258	690	9.
1 JAN 0412	127	9.	*	1 JAN 1028	315	9.	*	1 JAN 1644	503	9.	*	1 JAN 2300	691	9.
1 JAN 0414	128	9.	*	1 JAN 1030	316	9.	*	1 JAN 1646	504	9.	*	1 JAN 2302	692	9.
1 JAN 0416	129	9.	*	1 JAN 1032	317	9.	*	1 JAN 1648	505	9.	*	1 JAN 2304	693	9.
1 JAN 0418	130	9.	*	1 JAN 1034	318	9.	*	1 JAN 1650	506	9.	*	1 JAN 2306	694	9.
1 JAN 0420	131	9.	*	1 JAN 1036	319	9.	*	1 JAN 1652	507	9.	*	1 JAN 2308	695	9.
1 JAN 0422	132	9.	*	1 JAN 1038	320	9.	*	1 JAN 1654	508	9.	*	1 JAN 2310	696	9.
1 JAN 0424	133	9.	*	1 JAN 1040	321	9.	*	1 JAN 1656	509	9.	*	1 JAN 2312	697	9.
1 JAN 0426	134	9.	*	1 JAN 1042	322	9.	*	1 JAN 1658	510	9.	*	1 JAN 2314	698	9.
1 JAN 0428	135	9.	*	1 JAN 1044	323	9.	*	1 JAN 1700	511	9.	*	1 JAN 2316	699	9.
1 JAN 0430	136	9.	*	1 JAN 1046	324	9.	*	1 JAN 1702	512	9.	*	1 JAN 2318	700	9.
1 JAN 0432	137	9.	*	1 JAN 1048	325	9.	*	1 JAN 1704	513	9.	*	1 JAN 2320	701	9.
1 JAN 0434	138	9.	*	1 JAN 1050	326	9.	*	1 JAN 1706	514	9.	*	1 JAN 2322	702	9.
1 JAN 0436	139	9.	*	1 JAN 1052	327	9.	*	1 JAN 1708	515	9.	*	1 JAN 2324	703	9.
1 JAN 0438	140	9.	*	1 JAN 1054	328	9.	*	1 JAN 1710	516	9.	*	1 JAN 2326	704	9.
1 JAN 0440	141	9.	*	1 JAN 1056	329	9.	*	1 JAN 1712	517	9.	*	1 JAN 2328	705	9.
1 JAN 0442	142	9.	*	1 JAN 1058	330	9.	*	1 JAN 1714	518	9.	*	1 JAN 2330	706	9.
1 JAN 0444	143	9.	*	1 JAN 1100	331	9.	*	1 JAN 1716	519	9.	*	1 JAN 2332	707	9.
1 JAN 0446	144	9.	*	1 JAN 1102	332	9.	*	1 JAN 1718	520	9.	*	1 JAN 2334	708	9.
1 JAN 0448	145	9.	*	1 JAN 1104	333	9.	*	1 JAN 1720	521	9.	*	1 JAN 2336	709	9.
1 JAN 0450	146	9.	*	1 JAN 1106	334	9.	*	1 JAN 1722	522	9.	*	1 JAN 2338	710	9.
1 JAN 0452	147	9.	*	1 JAN 1108	335	9.	*	1 JAN 1724	523	9.	*	1 JAN 2340	711	9.
1 JAN 0454	148	9.	*	1 JAN 1110	336	9.	*	1 JAN 1726	524	9.	*	1 JAN 2342	712	9.
1 JAN 0456	149	9.	*	1 JAN 1112	337	9.	*	1 JAN 1728	525	9.	*	1 JAN 2344	713	9.
1 JAN 0458	150	9.	*	1 JAN 1114	338	9.	*	1 JAN 1730	526	9.	*	1 JAN 2346	714	9.
1 JAN 0500	151	9.	*	1 JAN 1116	339	9.	*	1 JAN 1732	527	9.	*	1 JAN 2348	715	9.
1 JAN 0502	152	9.	*	1 JAN 1118	340	9.	*	1 JAN 1734	528	9.	*	1 JAN 2350	716	9.
1 JAN 0504	153	9.	*	1 JAN 1120	341	9.	*	1 JAN 1736	529	9.	*	1 JAN 2352	717	9.
1 JAN 0506	154	9.	*	1 JAN 1122	342	9.	*	1 JAN 1738	530	9.	*	1 JAN 2354	718	9.
1 JAN 0508	155	9.	*	1 JAN 1124	343	9.	*	1 JAN 1740	531	9.	*	1 JAN 2356	719	9.
1 JAN 0510	156	9.	*	1 JAN 1126	344	9.	*	1 JAN 1742	532	9.	*	1 JAN 2358	720	9.
1 JAN 0512	157	9.	*	1 JAN 1128	345	9.	*	1 JAN 1744	533	9.	*	2 JAN 0000	721	9.
1 JAN 0514	158	9.	*	1 JAN 1130	346	9.	*	1 JAN 1746	534	9.	*	2 JAN 0002	722	9.
1 JAN 0516	159	9.	*	1 JAN 1132	347	9.	*	1 JAN 1748	535	9.	*	2 JAN 0004	723	9.
1 JAN 0518	160	9.	*	1 JAN 1134	348	9.	*	1 JAN 1750	536	9.	*	2 JAN 0006	724	9.
1 JAN 0520	161	9.	*	1 JAN 1136	349	9.	*	1 JAN 1752	537	9.	*	2 JAN 0008	725	9.
1 JAN 0522	162	9.	*	1 JAN 1138	350	9.	*	1 JAN 1754	538	9.	*	2 JAN 0010	726	9.
1 JAN 0524	163	9.	*	1 JAN 1140	351	9.	*	1 JAN 1756	539	9.	*	2 JAN 0012	727	9.
1 JAN 0526	164	9.	*	1 JAN 1142	352	9.	*	1 JAN 1758	540	9.	*	2 JAN 0014	728	9.
1 JAN 0528	165	9.	*	1 JAN 1144	353	9.	*	1 JAN 1800	541	9.	*	2 JAN 0016	729	9.
1 JAN 0530	166	9.	*	1 JAN 1146	354	9.	*	1 JAN 1802	542	9.	*	2 JAN 0018	730	9.
1 JAN 0532	167	9.	*	1 JAN 1148	355	9.	*	1 JAN 1804	543	9.	*	2 JAN 0020	731	9.
1 JAN 0534	168	9.	*	1 JAN 1150	356	9.	*	1 JAN 1806	544	9.	*	2 JAN 0022	732	9.
1 JAN 0536	169	9.	*	1 JAN 1152	357	9.	*	1 JAN 1808	545	9.	*	2 JAN 0024	733	9.
1 JAN 0538	170	9.	*	1 JAN 1154	358	9.	*	1 JAN 1810	546	9.	*	2 JAN 0026	734	9.
1 JAN 0540	171	9.	*	1 JAN 1156	359	9.	*	1 JAN 1812	547	9.	*	2 JAN 0028	735	9.
1 JAN 0542	172	9.	*	1 JAN 1158	360	9.	*	1 JAN 1814	548	9.	*	2 JAN 0030	736	9.
1 JAN 0544	173	9.	*	1 JAN 1200	361	9.	*	1 JAN 1816	549	9.	*	2 JAN 0032	737	9.
1 JAN 0546	174	9.	*	1 JAN 1202	362	9.	*	1 JAN 1818	550	9.	*	2 JAN 0034	738	9.
1 JAN 0548	175	9.	*	1 JAN 1204	363	9.	*	1 JAN 1820	551	9.	*	2 JAN 0036	739	9.
1 JAN 0550	176	9.	*	1 JAN 1206	364	9.	*	1 JAN 1822	552	9.	*	2 JAN 0038	740	9.
1 JAN 0552	177	9.	*	1 JAN 1208	365	9.	*	1 JAN 1824	553	9.	*	2 JAN 0040	741	9.
1 JAN 0554	178	9.	*	1 JAN 1210	366	9.	*	1 JAN 1826	554	9.	*	2 JAN 0042	742	9.
1 JAN 0556	179	9.	*	1 JAN 1212	367	9.	*	1 JAN 1828	555	9.	*	2 JAN 0044	743	9.
1 JAN 0558	180	9.	*	1 JAN 1214	368	9.	*	1 JAN 1830	556	9.	*	2 JAN 0046	744	9.
1 JAN 0600	181	9.	*	1 JAN 1216	369	9.	*	1 JAN 1832	557	9.	*	2 JAN 0048	745	9.
1 JAN 0602	182	9.	*	1 JAN 1218	370	9.	*	1 JAN 1834	558	9.	*	2 JAN 0050	746	9.
1 JAN 0604	183	9.	*	1 JAN 1220	371	9.	*	1 JAN 1836	559	9.	*	2 JAN 0052	747	9.
1 JAN 0606	184	9.	*	1 JAN 1222	372	9.	*	1 JAN 1838	560	9.	*	2 JAN 0054	748	9.
1 JAN 0608	185	9.	*	1 JAN 1224	373	9.	*	1 JAN 1840	561	9.	*	2 JAN 0056	749	9.
1 JAN 0610	186	9.	*	1 JAN 1226	374	9.	*	1 JAN 1842	562	9.	*	2 JAN 0058	750	9.
1 JAN 0612	187	9.	*	1 JAN 1228	375	9.	*	1 JAN 1844	563	9.	*			
1 JAN 0614	188	9.	*	1 JAN 1230	376	9.	*	1 JAN 1846	564	9.	*			

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PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
26.	.17	9.	9.	9.	9.	
		(INCHES)	2.934	11.588	12.048	12.048
		(AC-FT)	4.	17.	18.	18.
CUMULATIVE AREA =		.03 SQ MI				

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1 JAN 0046	24	1.	*	1 JAN 0702	212	1.	*	1 JAN 1318	400	1.	*	1 JAN 1934	588	1.
1 JAN 0048	25	1.	*	1 JAN 0704	213	1.	*	1 JAN 1320	401	1.	*	1 JAN 1936	589	1.
1 JAN 0050	26	1.	*	1 JAN 0706	214	1.	*	1 JAN 1322	402	1.	*	1 JAN 1938	590	1.
1 JAN 0052	27	1.	*	1 JAN 0708	215	1.	*	1 JAN 1324	403	1.	*	1 JAN 1940	591	1.
1 JAN 0054	28	1.	*	1 JAN 0710	216	1.	*	1 JAN 1326	404	1.	*	1 JAN 1942	592	1.
1 JAN 0056	29	1.	*	1 JAN 0712	217	1.	*	1 JAN 1328	405	1.	*	1 JAN 1944	593	1.
1 JAN 0058	30	1.	*	1 JAN 0714	218	1.	*	1 JAN 1330	406	1.	*	1 JAN 1946	594	1.
1 JAN 0100	31	1.	*	1 JAN 0716	219	1.	*	1 JAN 1332	407	1.	*	1 JAN 1948	595	1.
1 JAN 0102	32	1.	*	1 JAN 0718	220	1.	*	1 JAN 1334	408	1.	*	1 JAN 1950	596	1.
1 JAN 0104	33	1.	*	1 JAN 0720	221	1.	*	1 JAN 1336	409	1.	*	1 JAN 1952	597	1.
1 JAN 0106	34	1.	*	1 JAN 0722	222	1.	*	1 JAN 1338	410	1.	*	1 JAN 1954	598	1.
1 JAN 0108	35	1.	*	1 JAN 0724	223	1.	*	1 JAN 1340	411	1.	*	1 JAN 1956	599	1.
1 JAN 0110	36	1.	*	1 JAN 0726	224	1.	*	1 JAN 1342	412	1.	*	1 JAN 1958	600	1.
1 JAN 0112	37	1.	*	1 JAN 0728	225	1.	*	1 JAN 1344	413	1.	*	1 JAN 2000	601	1.
1 JAN 0114	38	1.	*	1 JAN 0730	226	1.	*	1 JAN 1346	414	1.	*	1 JAN 2002	602	1.
1 JAN 0116	39	1.	*	1 JAN 0732	227	1.	*	1 JAN 1348	415	1.	*	1 JAN 2004	603	1.
1 JAN 0118	40	1.	*	1 JAN 0734	228	1.	*	1 JAN 1350	416	1.	*	1 JAN 2006	604	1.
1 JAN 0120	41	1.	*	1 JAN 0736	229	1.	*	1 JAN 1352	417	1.	*	1 JAN 2008	605	1.
1 JAN 0122	42	1.	*	1 JAN 0738	230	1.	*	1 JAN 1354	418	1.	*	1 JAN 2010	606	1.
1 JAN 0124	43	1.	*	1 JAN 0740	231	1.	*	1 JAN 1356	419	1.	*	1 JAN 2012	607	1.
1 JAN 0126	44	1.	*	1 JAN 0742	232	1.	*	1 JAN 1358	420	1.	*	1 JAN 2014	608	1.
1 JAN 0128	45	1.	*	1 JAN 0744	233	1.	*	1 JAN 1400	421	1.	*	1 JAN 2016	609	1.
1 JAN 0130	46	1.	*	1 JAN 0746	234	1.	*	1 JAN 1402	422	1.	*	1 JAN 2018	610	1.
1 JAN 0132	47	1.	*	1 JAN 0748	235	1.	*	1 JAN 1404	423	1.	*	1 JAN 2020	611	1.
1 JAN 0134	48	1.	*	1 JAN 0750	236	1.	*	1 JAN 1406	424	1.	*	1 JAN 2022	612	1.
1 JAN 0136	49	1.	*	1 JAN 0752	237	1.	*	1 JAN 1408	425	1.	*	1 JAN 2024	613	1.
1 JAN 0138	50	1.	*	1 JAN 0754	238	1.	*	1 JAN 1410	426	1.	*	1 JAN 2026	614	1.
1 JAN 0140	51	1.	*	1 JAN 0756	239	1.	*	1 JAN 1412	427	1.	*	1 JAN 2028	615	1.
1 JAN 0142	52	1.	*	1 JAN 0758	240	1.	*	1 JAN 1414	428	1.	*	1 JAN 2030	616	1.
1 JAN 0144	53	1.	*	1 JAN 0800	241	1.	*	1 JAN 1416	429	1.	*	1 JAN 2032	617	1.
1 JAN 0146	54	1.	*	1 JAN 0802	242	1.	*	1 JAN 1418	430	1.	*	1 JAN 2034	618	1.
1 JAN 0148	55	1.	*	1 JAN 0804	243	1.	*	1 JAN 1420	431	1.	*	1 JAN 2036	619	1.
1 JAN 0150	56	1.	*	1 JAN 0806	244	1.	*	1 JAN 1422	432	1.	*	1 JAN 2038	620	1.
1 JAN 0152	57	1.	*	1 JAN 0808	245	1.	*	1 JAN 1424	433	1.	*	1 JAN 2040	621	1.
1 JAN 0154	58	1.	*	1 JAN 0810	246	1.	*	1 JAN 1426	434	1.	*	1 JAN 2042	622	1.
1 JAN 0156	59	1.	*	1 JAN 0812	247	1.	*	1 JAN 1428	435	1.	*	1 JAN 2044	623	1.
1 JAN 0158	60	1.	*	1 JAN 0814	248	1.	*	1 JAN 1430	436	1.	*	1 JAN 2046	624	1.
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1 JAN 0614	188	1.	*	1 JAN 1230	376	1.	*	1 JAN 1846	564	1.	*			

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
12.	.17	2.	1.	1.	1.	
		(INCHES)	3.637	12.005	12.455	12.455
		(AC-FT)	1.	3.	3.	3.
CUMULATIVE AREA =		.00 SQ MI				



***Appendix D***

Master Drainage Report for Silverstone, Addendums No. 1 - 3 to the Master Drainage Report for Silverstone



**MASTER DRAINAGE REPORT  
FOR  
SILVERSTONE**

March 2007  
WP #042309

*Prepared for:*

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Attn: Mr. Mike Pacheco

Reviewer: 

Date: 3/18/07

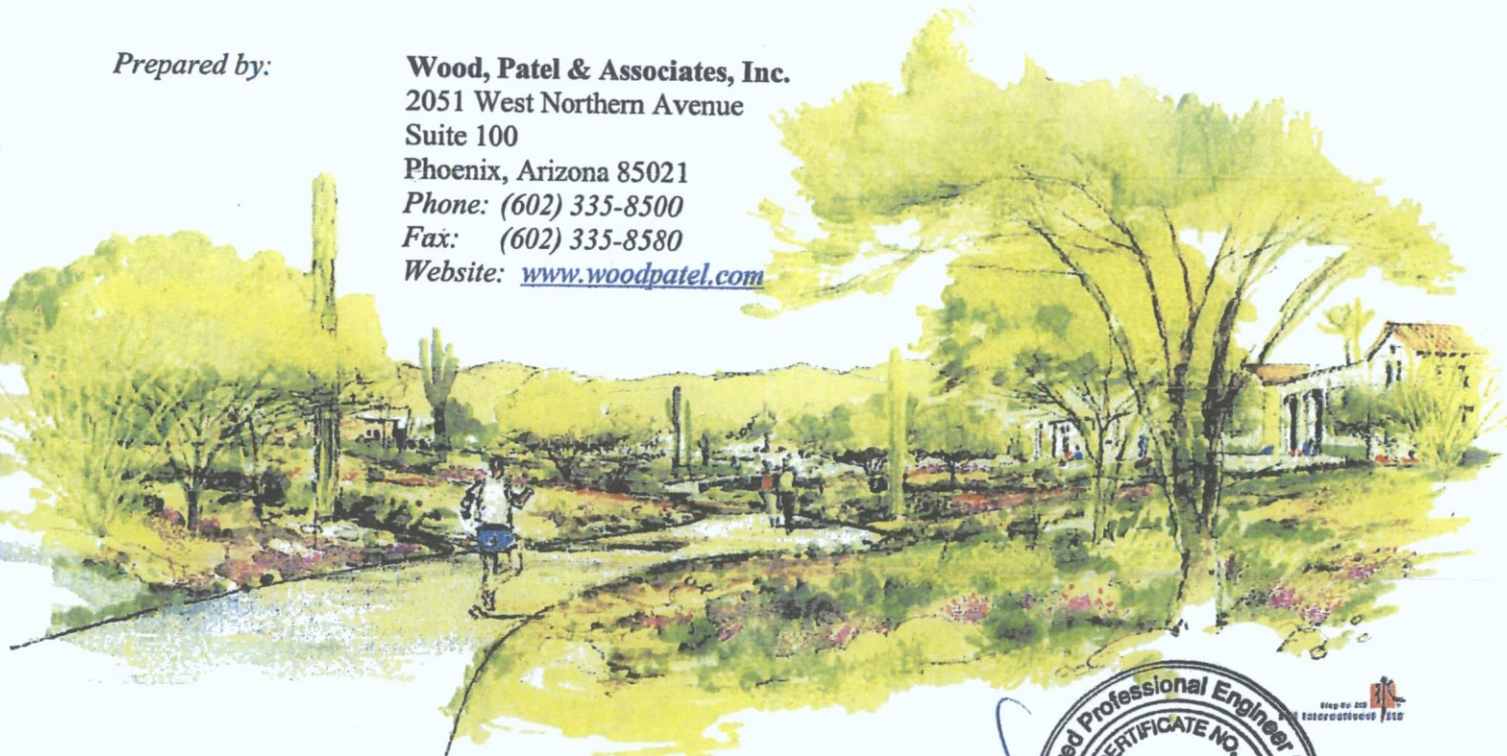
**Stormwater Management Division  
City of Scottsdale**

*Submitted to:*

**City of Scottsdale**  
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**Engineer-in-Training  
Darren Forstie**



Registered Professional Engineer (CIVIL)  
CERTIFICATE NO. 13138  
DARREL E. WOOD  
Date Signed 3/18/07  
ARIZONA, U.S.A.  
Engineer

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## PLATES

Plate 1	Vicinity Map
Plate 2	Flood Insurance Rate Map (FIRM)
Plate 3	Silverstone Drainage Map

## TABLES

Table 4.1	Parcel Detention Requirements
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## 1.0 INTRODUCTION

### 1.1 General Background

This report has been prepared to address drainage requirements and provide a Master Drainage Plan for the development known as Silverstone. The Silverstone site (hereafter referred to as the Site) located in north Scottsdale, is approximately 160 acres and is bounded by Pinnacle Peak Road on the north, Miller Road on the east, Williams Drive on the south, and Scottsdale Road on the west. More specifically, the site is located in Section 14, Township 4 North, Range 4 East of the Gila and Salt River Meridian. Plate 1 – *Vicinity Map*, illustrates the Site's location.

The proposed Silverstone consist of mixed use land including municipal, retail, office, residential, open space, and public streets as applicable per City of Scottsdale Zoning Case 15-ZN-2005, 13-UP-2005.

As per the City of Scottsdale Stipulations, this master drainage plan endeavors to address the following:

1. *DRAINAGE MASTER PLAN: The developer shall submit a master drainage report and plan subject to city manager or designee approval. The master drainage report and plan shall conform the approved Drainage Design Report (Plan Check #3678-05) and to the Design Standards and Policies Manual – Drainage Report Preparation. In addition, the master drainage report and plan shall:
  - A. *Include a complete description of requirements relating to project phasing.*
  - B. *Identify the timing of and parties responsible for construction of all storm water management facilities.*
  - C. *Identify improvements to the Rawhide Wash, including but not limited to retaining walls, scour walls, head walls, bridges, control structures, street and pedestrian crossing, and open space amenities.*
  - D. *Correspondence with State Lands/City of Phoenix to west*
  - E. *Bridge timing/responsibility (Scottsdale Rd. and Pinnacle Peak), with possible grade separated crossing for pedestrian access under Pinnacle Peak Road and under Scottsdale Road.**

- F. *Before master drainage report approval, the developer shall, when requested by City Manager or Designee, submit two (2) hard copies and one (1) disc copy of the complete master drainage report.*
- G. *Before the improvement plan submittal, the developer shall have obtained approval of the master drainage report.*

## **1.2 Drainage Background**

The Site is situated on an alluvial plain within upland Sonoran Desert containing moderate slopes. The entire Site is located within the Rawhide Wash watershed and its Federal Emergency Management Agency (FEMA) designated 100 year floodplain (see Section 1.4 FEMA Floodplain). The Site consists of undeveloped desert terrain with vegetation and a mixture of buildings, dirt paths and drives from the now abandoned Rawhide development. Rawhide has disturbed the Site from historic conditions. The Site generally slopes toward the southwest at approximately 2.4 percent. Some exceptions are a raised dirt track located in the southwest corner of the site and a raised dirt dike bordering the site on the north and east sides. A major named wash, Rawhide Wash traverses the north western portion of the site.

In a rainfall/runoff event, Site is believed to receive offsite flows from the upstream Rawhide Wash watershed. The offsite flows from Rawhide Wash are believed to currently continue through the Site in Rawhide Wash and the small drainage corridors created by the previous owner. This offsite flow leaves the Site at a dip section in Scottsdale Road. The raised berms on the north and east boundaries of the Site are believed to protect the Site from any other offsite flow potential. Therefore, the Site is not impacted from offsite flows other than previously mentioned and only generates local onsite flows which generally start at the north portion of the Site and travel south-southwest through the Site.

## **1.3 Drainage Concept**

This Drainage Master Plan presents a drainage investigation of the Site and addresses several areas: the Site, the proposed public roadways within the Site, and Rawhide Wash.

The Silverstone Development consists of proposed public roadways (Silverstone Drive and 74<sup>th</sup> Street) and private parcels. The north-south public roadway 74<sup>th</sup> Street, will

contain a proposed storm drain system to capture roadway flows and provide a bleed off option to parcel detention basins. This storm drain system will outlet into the proposed channel along Williams Drive while the storm drain system on Silverstone Drive will outlet into the proposed scenic corridor along Scottsdale Road (see Plate 3 – *Silverstone Drainage Map*).

The proposed parcels will retain the 100-year, 2-hour volume as required by the City of Scottsdale development guidelines. The detention basins will bleed off within 36 hours into the roadway storm drain system or existing drainage corridors per the design of the parcel developer and their engineer.

Rawhide Wash will be channelized from Pinnacle Peak Road to Scottsdale Road. The channel will be designed in two phases, an interim and ultimate condition.

#### 1.4 **FEMA Floodplain**

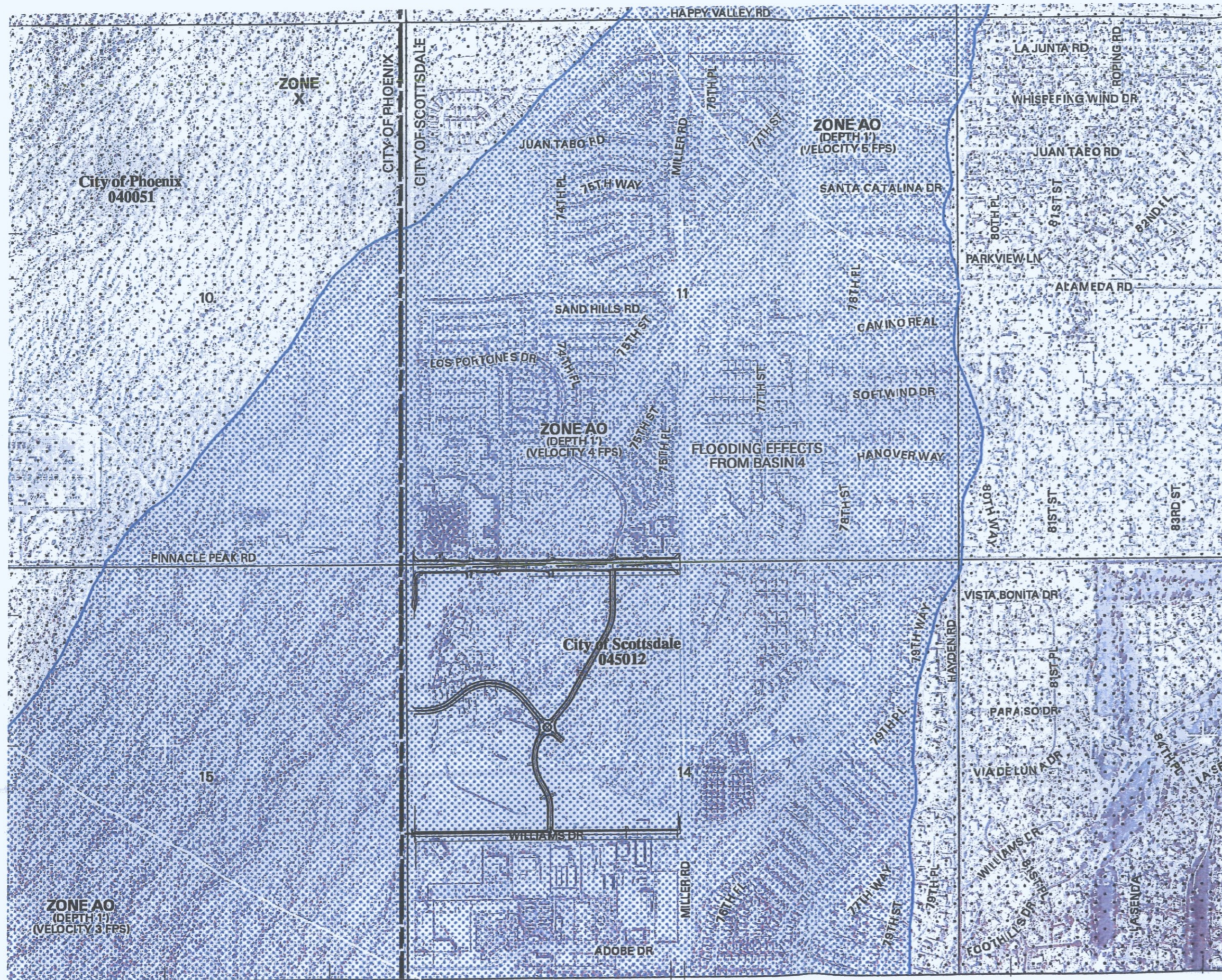
The Site lies within a Federal Emergency Management Agency (FEMA) designated Zone AO, per Flood Insurance Rate Map (FIRM) Panel 1235 of 4350, number 04013C1235G, dated September 30, 2005 (See Plate 2 – Flood Insurance Rate Map). Zone AO is defined by FEMA and per the FIRM Panel as follows:

*Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain), average depths determined. For areas of alluvial fan flooding, velocities also determined.*

The average depth displayed on the FEMA FIRM for the Project Area is one (1) foot with a velocity shown as four (4) feet per second.

**PLATE 2**

**Flood Insurance Rate Map (FIRM)**



**NFIP** PANEL 1235G


**FIRM**  
**FLOOD INSURANCE RATE MAP**  
**MARICOPA COUNTY,**  
**ARIZONA**  
**AND INCORPORATED AREAS**

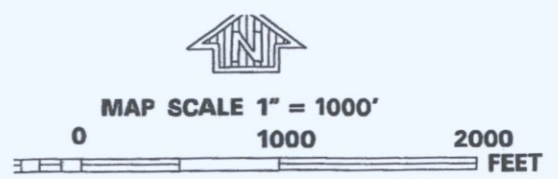
PANEL 1235 OF 4350  
 (SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
MARICOPA COUNTY	040037	1235	0
PHOENIX, CITY OF	040051	1235	0
SCOTTSDALE, CITY OF	045012	1235	0

Notice to User: The Map Number shown below should be used when placing map orders. The Community Number shown above should be used on insurance applications for the subject community.

 **MAP NUMBER**  
**04013C1235G**  
**MAP REVISED**  
**SEPTEMBER 30, 2005**  
 Federal Emergency Management Agency



Flood Insurance Rate Map (FIRM)

Wood, Patel & Associates, Inc.  
 2051 West Northern, Suite 100  
 Phoenix, Arizona 85021 (602) 335-8500

DRAWN BY: D FORSTIE  
 JOB NO: 042309

PLATE 2

**PLATE 3**

**Project Area Drainage Map**

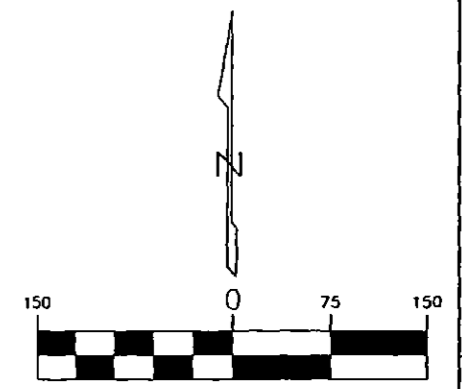
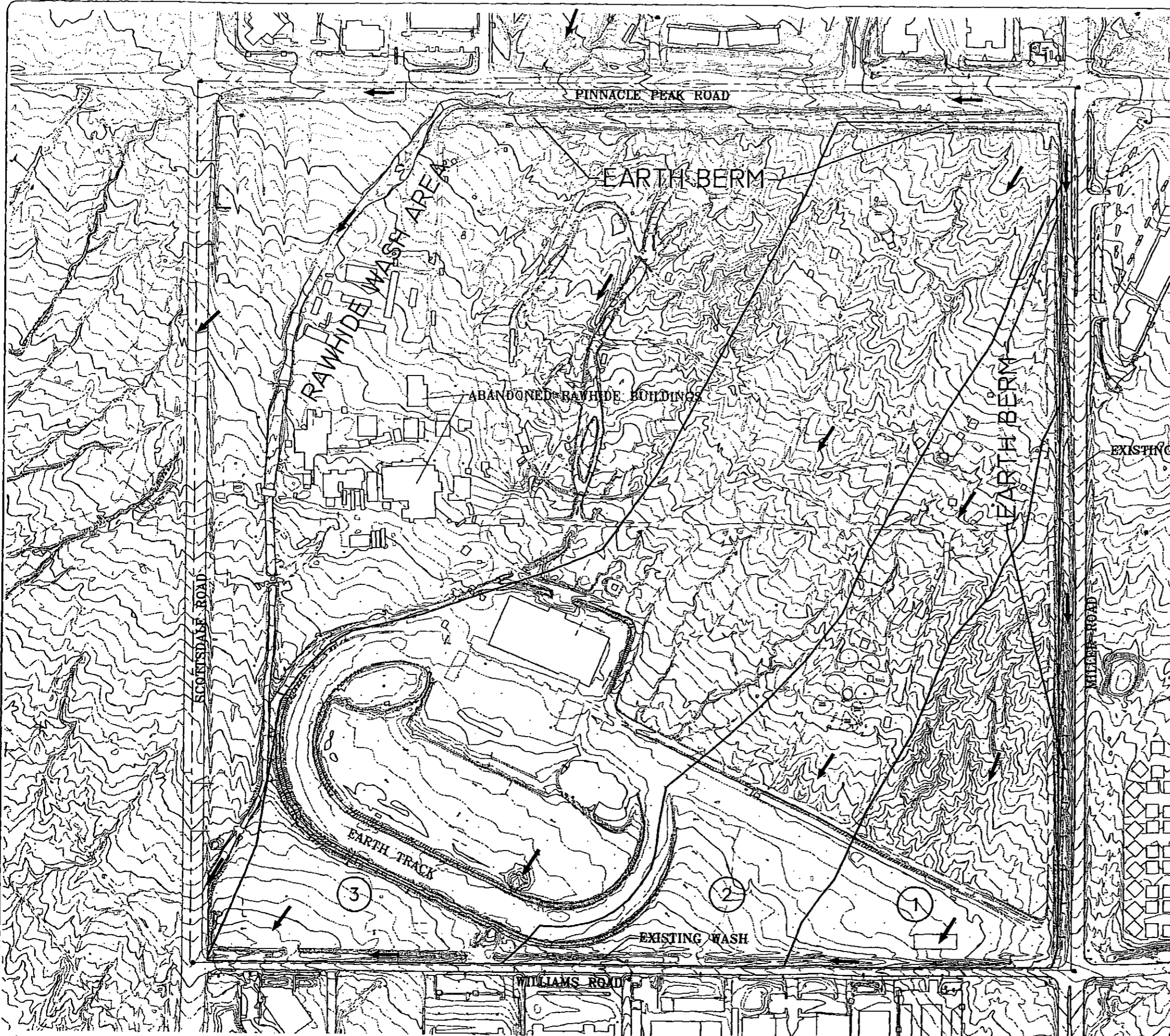


**SILVERSTONE**  
 SCOTTSDALE  
 DRAINAGE MAP



PRELIMINARY  
 NOT  
 FOR  
 CONSTRUCTION  
 OR RECORDING

DRAWN	DF
CHECKED	
DATE	MARCH 07
SCALE	1"=150'
JOB NO.	042309.1C
SHEET	1 OF 1





1 inch = 150 ft.

- LEGEND**
-  PRE-EXISTING FLOW DIRECTION
  -  PRE-EXISTING/PRE-DEVELOPED SUBBASIN

**WOODPATEL**  
 LAND DEVELOPMENT • WATER RESOURCES  
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 (602) 335-8600  
 www.woodpatel.com  
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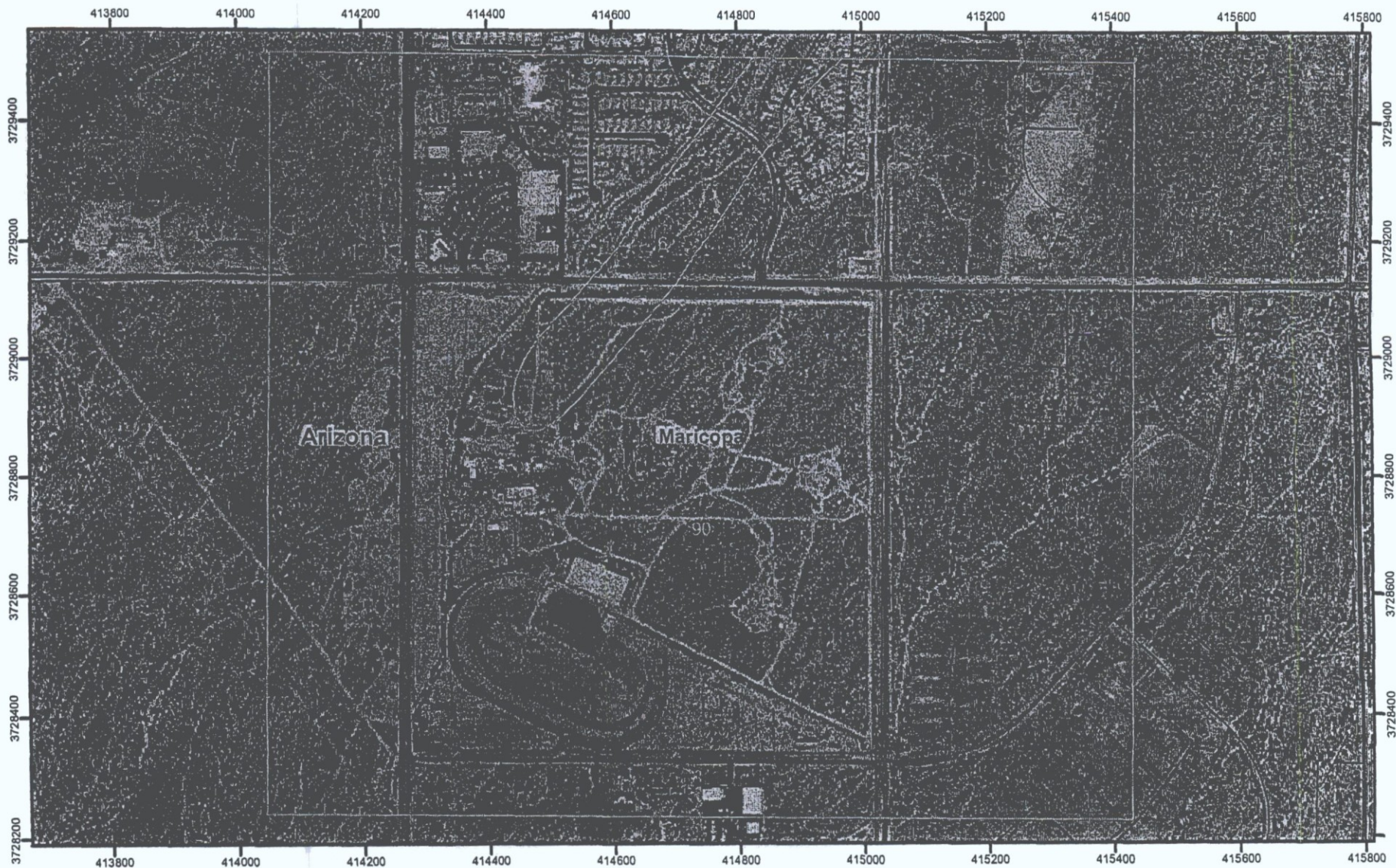
**SILVERSTONE**  
 SCOTTSDALE  
 PRE-EXISTING DRAINAGE MAP

PRELIMINARY  
**NOT**  
 FOR  
 CONSTRUCTION  
 OR RECORDING

DRAWN	DF
CHECKED	
DATE	MARCH 07
SCALE	1" = 150'
JOB NO.	042309.10
SHEET	

**SOIL MAP**

SOIL SURVEY OF AGUILA-CAREFREE AREA, ARIZONA, PARTS OF MARICOPA AND PINAL COUNTIES









































0 100 200 400 Meters

0 300 600 1,200 1,800 2,400 Feet

# SOIL SURVEY OF AGUILA-CAREFREE AREA, ARIZONA, PARTS OF MARICOPA AND PINAL COUNTIES

## MAP LEGEND

-  Soil Map Units
-  Cities
-  Detailed Counties
-  Detailed States
-  Interstate Highways
-  Roads
-  Rails
-  Water
-  Hydrography
-  Oceans
-  Escarpment, bedrock
-  Escarpment, non-bedrock
-  Gulley
-  Levee
-  Slope
-  Blowout
-  Borrow Pit
-  Clay Spot
-  Depression, closed
-  Eroded Spot
-  Gravel Pit
-  Gravelly Spot
-  Gulley
-  Lava Flow
-  Landfill
-  Marsh or Swamp
-  Miscellaneous Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Slide or Slip
-  Sinkhole
-  Sodic Spot
-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Perennial Water
-  Wet Spot

## MAP INFORMATION

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL: <http://websoilsurvey.nrcs.usda.gov>

Coordinate System: UTM Zone 12

Soil Survey Area: Aguila-Carefree Area, Arizona, Parts of  
 Maricopa and Pinal Counties

Spatial Version of Data: 1

Soil Map Compilation Scale: 1:24000

Map comprised of aerial images photographed on these dates:  
 4/30/1997

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# Engineering Properties

Agula-Carefree Area, Arizona, Parts of Maricopa and Pinal Counties

Map symbol and soil name	Depth	USDA texture	Classification		Fragments		Percent passing sieve number--				Liquid limit	Plasticity Index
			Unified	AASHTO	>10 Inches	3-10 Inches	4	10	40	200		
	<i>In</i>				<i>Pct</i>	<i>Pct</i>					<i>Pct</i>	
6:												
Anthony	0-2	Sandy loam	ML, SM	A-4	0	0	95-100	90-100	55-85	35-65	20-30	NP-5
	2-40	Gravelly sandy loam; Very gravelly sandy loam	GM, SM	A-1, A-2	0	15-20	45-65	40-60	25-35	10-30	0-14	NP
	40-60	Loam	CL, CL-ML, ML	A-4	0	0	100	100	95-100	70-80	20-30	NP-10
Arizo	0-1	Gravelly sandy loam	SM	A-2	0	0-5	65-80	60-75	35-50	20-30	15-20	NP-5
	1-8	Very gravelly sandy loam	GM, SM	A-1	0	0-5	45-65	35-50	25-35	10-20	15-20	NP-5
	8-60	Very cobbly loamy sand, Very gravelly loamy sand	GM, GP-GM, SM, SP-SM	A-1	0	25-30	50-60	40-55	20-40	10-15	0-14	NP
90:												
Momoli	0-3	Gravelly sandy loam	SM	A-1, A-2	0	0-5	65-80	60-75	35-50	20-30	20-30	NP-5
	3-60	Very gravelly fine sandy loam; Very gravelly loam; Very gravelly sandy loam	GC-GM, GM	A-1, A-2	0	0-5	40-65	25-50	20-40	10-35	20-30	NP-10

This report shows only the major soils in each map unit. Others may exist.

**Pre-Existing Hydrology**

# WOOD/PATEL

CIVIL ENGINEERS • HYDROLOGISTS • LAND SURVEYORS • CONSTRUCTION MANAGERS

## ON-SITE WEIGHTED "C" FACTOR - 10 and 100 YEAR STORM EVENT

Site: Pre-existing Silverstone  
Location: Scottsdale, Arizona  
Description: Pre-existing conditions of subbasins that leave the Silverstone Site  
References: Design Standards and Policies Manual, Rev. Jan., 1993, City of Scottsdale  
Fig. 2.2-17, Runoff Coefficients (C) for Use with the Rational Formula  
Drainage Design Manual for Maricopa County, Volume I, Hydrology, Table 3.2  
Date: 3/13/2007

Known Values:

Land Type	"C" Value	Subbasin 1	Subbasin 2	Subbasin 3
		ac	ac	ac
Undisturbed Desert	0.31	15	15	30
Commercial	0.90			
Gravel Rds/Dirt/Corrals	0.60		7	27
Sum		15	22	57

Weighted "C" Factor (10 yr) =	0.31	0.40	0.45
Weighted "C" Factor (100 yr) =	0.39	0.50	0.56

Note: Soil C values based on hydrologic soil group B.



Flood Control District of Maricopa County  
 Drainage Design Manual Rational Method

Computed by: DF

Date: 9/26/06

LOCATION DATA

Location: Silverstone

Project Name:

Subarea id: 1

Drainage Area Cover:

DESIGN DATA

Drainage Area 15.0000 acres

Watercourse Length 1683.0000 feet

Top Elevation 1856.0000 feet

Bottom Elevation 1810.0000 feet

Slope 0.027 feet/feet

Roughness Coefficient (Kb) 0.0302

10-year, 6-Hour Rainfall 2.2000 inches

Hydrological Summary Table

Parameter	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Q (cfs)	16	22	26	35	44	53
C	0.310	0.310	0.310	0.340	0.370	0.390
Tc (min)	8.5	7.5	7.0	6.4	6.1	5.8
i (in/hr)	3.4	4.6	5.5	6.9	7.9	9.1

Flood Control District of Maricopa County  
 Drainage Design Manual Rational Method

Computed by: DF

Date: 9/26/06

LOCATION DATA

Location: Silverstone

Project Name:

Subarea id: 2

Drainage Area Cover:

DESIGN DATA

Drainage Area 22.0000 acres

Watercourse Length 2591.0000 feet

Top Elevation 1869.0000 feet

Bottom Elevation 1802.0000 feet

Slope 0.026 feet/feet

Roughness Coefficient (Kb) 0.0316

10-year, 6-Hour Rainfall 2.2000 inches

Hydrological Summary Table

Parameter	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Q (cfs)	26	36	43	60	75	90
C	0.400	0.400	0.400	0.440	0.480	0.500
Tc (min)	11.5	10.1	9.5	8.7	8.2	7.8
i (in/hr)	3.0	4.1	4.9	6.2	7.1	8.2

Flood Control District of Maricopa County  
 Drainage Design Manual Rational Method

Computed by: DF

Date: 9/26/06

LOCATION DATA

Location: Silverstone

Project Name:

Subarea id: 3

Drainage Area Cover:

DESIGN DATA

Drainage Area 57.0000 acres

Watercourse Length 3720.0000 feet

Top Elevation 1876.0000 feet

Bottom Elevation 1790.0000 feet

Slope 0.023 feet/feet

Roughness Coefficient (Kb) 0.0290

10-year, 6-Hour Rainfall 2.2000 inches

Hydrological Summary Table

Parameter	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Q (cfs)	69	96	114	161	202	243
C	0.450	0.450	0.450	0.500	0.540	0.560
Tc (min)	14.1	12.5	11.7	10.6	10.1	9.5
i (in/hr)	2.7	3.7	4.5	5.7	6.6	7.6

**Parcel Detention Volumes**

# SILVERSTONE

## Parcel Detention Requirements

### Known Values:

$$V = (P/12)AC$$

$A(\text{sf}) = \text{Contributing Area}$   
 $P(\text{in}) = 2.82 \quad (\text{from COS Drainage guidelines})$   
 $C_{100} = \text{Weighted Runoff Coefficient}$

Basin	Tributary Area	Comparable Zoning (from Stips.)	Weighted runoff coefficient <sup>(1)</sup>	Required Volume	Required Volume
	ac			cf	ac-ft
<b>A&amp;B</b>	4.5	C-O	0.9	41274	0.95
<b>C</b>	12.4	C-2	0.9	113964	2.62
<b>D</b>	13.5	C-O	0.9	124559	2.86
<b>E</b>	16.7	R-5	0.76	130001	2.98
<b>F</b>	22.1	R-5	0.76	171623	3.94
<b>G</b>	23.8	R-5	0.76	184771	4.24
<b>H</b>	32.8	R-5	0.76	254867	5.85
<b>Park</b>	1.9	-	0.33	6998	0.16

1. Weighted runoff coefficient values based of COS grading and drainage manuel and hydrologic soil group B.

February 13, 2014

City of Scottsdale  
Stormwater/Drainage Division  
7447 East Indian School Road  
Scottsdale, Arizona

Ph: (480) 312-2500

Re: **Silverstone Parcel C**  
*Addendum No. 1 Master Drainage Report*  
Comment Response Letter  
WP# 042309

Dear Mr. Anderson:

Following are our responses to Addendum No. 1 of the Master Drainage Report 1<sup>st</sup> Review Comments, dated January 7, 2014:

1. Comment #1: Provide a cover sheet similar to the original master plan cover sheet that clearly identifies this report as addendum #1 to the original report.  
*Response #1: As requested, a cover sheet has been added to the report identifying it as Addendum No.1 to the Master Drainage Report for Silverstone.*
2. Comment #2: Provide a full .pdf copy of the report on disc for City records by approval.  
*Response #2: As requested, a CD containing a PDF of the MDR addendum has been included in the back of the revised report.*
3. Comment #3: Please call me to discuss the including parcels D and hopefully E in the pre versus post C analysis as part of the master plan update. Ideally, the City would like to include the entire development in the update. A pre versus post C analysis for these parcels would result in a substantial reduction in storage. (If amenable to your client.)  
*Response #3: Wood/Patel has been directed by our client only to pursue the Pre vs. Post C analysis for Parcel C of the Silverstone Development. It is our understanding that our client would prefer to pursue pre. Vs. post C analysis of the other undeveloped parcels at time of parcel development, and would submit additional addendums to the MDR at that time.*

Darrel E. Wood, P.E., R.L.S.  
Ashok C. Patel, P.E., R.L.S., CFM  
James S. Campbell, P.E.  
Thomas R. Gettings, R.L.S.  
Michael T. Young, P.E.  
Jeffrey R. Minch, P.E.  
Robert D. Gofonia, P.E., R.L.S.  
Patrick W. Marunt, P.E.  
Kenneth L. Knickerbocker, P.E., R.L.S.  
Darin L. Moore, P.E.  
John M. Bulka, P.E.  
Daniel J. Cronin, PMP, LEED AP, CDT  
James G. Taillon, CFM  
Daniel W. Matthews, P.E.  
Christopher A. Salas, P.E.  
R. Stuart Barney, P.E.  
Kathy M. Svecovsky, R.L.S.  
Joseph C. Daconta, P.E.  
Shane D. McClara, P.E.  
Ken S. Snow, P.E.  
Ethan A. Boyle, P.E.  
Michael R. Havill, P.E., R.L.S.  
Cesar Castillo, P.E.  
Edward M. Rajnovich, P.E.  
James L. Kary, P.E.  
Mark A. Everett, P.E., CFM  
Ronald F. Martinez, P.E.  
Stefanie M. Thrush, P.E.



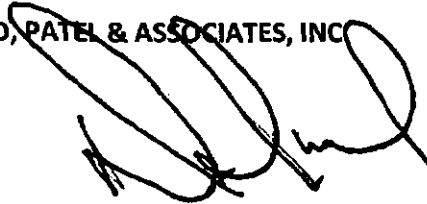
Mr. Richard Anderson  
City of Scottsdale  
Silverstone Parcel C  
Addendum No. 1\_Master Drainage Report - 1<sup>st</sup> Review  
Plan Check No. 3476-06-17  
WP# 042309

February 13, 2014  
Page 2 of 2

Please contact our office with questions regarding the above responses.

Sincerely,

WOOD, PATEL & ASSOCIATES, INC



Darrel E. Wood, P.E., R.L.S  
Principal



DEW/bm

Y:\WP\General Correspondence\134000 Silverstone Parcel C Add No. 1 Master Drainage Report 1st Review Comment Response COS R Anderson 2-13-14.docx

**ADDENDUM NO.1  
TO THE  
MASTER DRAINAGE REPORT  
FOR  
SILVERSTONE**

Revised February 2014  
March 2007

WP# 042309

*Prepared for:*

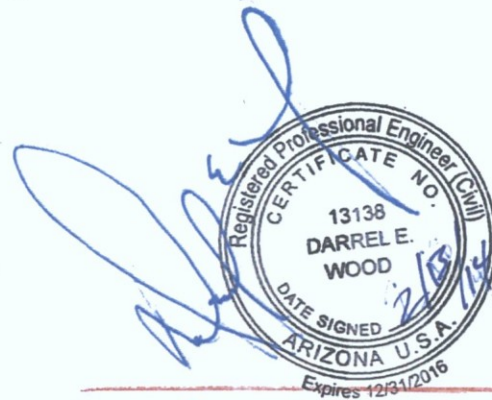
**Silverstone Development, Inc.**  
1550 East Missouri Avenue  
Suite 300  
Phoenix, Arizona 85011  
*Phone: (602) 230-1051*  
*Fax: (602) 230-2826*  
Attn: Mr. Mike Pacheco

*Submitted to:*

**City of Scottsdale**  
7447 East Indian School Road  
Scottsdale, Arizona 85251

*Prepared by:*

**Wood, Patel & Associates, Inc.**  
2051 West Northern Avenue  
Suite 100  
Phoenix, Arizona 85021  
*Phone: (602) 335-8500*  
*Fax: (602) 335-8580*  
*Website: [www.woodpatel.com](http://www.woodpatel.com)*



**Stormwater Review By:**

**Richard Anderson**

**Phone 480-312-2729**

**FAX 480-312-9202**

**E-MAIL [rianderson@ScottsdaleAZ.gov](mailto:rianderson@ScottsdaleAZ.gov)**

**Review Cycle \_\_\_\_\_ Date 3/6/14**





February 13, 2014

**ADDENDUM No.1**

**COS # 425-SA-2006 #15-ZN-2006, 124-NP-2006, PC# 3476-06-2**

- Darrel E. Wood, P.E., R.L.S.
- Ashok C. Patel, P.E., R.L.S., CFM
- James S. Campbell, P.E.
- Thomas R. Gettings, R.L.S.
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- James L. Kary, P.E.
- Mark A. Everett, P.E., CFM
- Ronald F. Martinez, P.E.
- Stefanie M. Thrush, P.E.

- Addendum No.1 to the Master Drainage Report Silverstone - March 2007, WP# 042309
- City of Scottsdale, Stormwater Management Division, Approved 3/18/07
- Justification #1: In January 2010, City of Scottsdale updated "Design Standards and Policies Manual, Chapter 4. Grading & Drainage, NOAA Atlas 14, Volume 1, Version 5, Point Precipitation Frequency Estimate. The updated rainfall volumes are lower in this area of north Scottsdale. (P=2.41 inches 100-year, 2-hour event)
- Justification #2: During recent correspondence with the City of Scottsdale Stormwater Management Division (COS), Wood, Patel & Associates, Inc. (Wood/Patel) was informed of a recent revision to the COS stormwater storage policy. The policy revision is valid for previously developed sites, such as Silverstone Parcel C (previously a portion of the Rawhide development). Per the revised policy, areas of the proposed development that were previously developed are required to provide a storage volume equal to the increase in runoff volume generated by the proposed development during the design storm event (100-year, 2-hour storm). Areas of the proposed development that were previously undeveloped are required to provide a storage volume equal to the entire runoff volume generated by the proposed development during the design storm event.
- Revisions to the Master Drainage Report (MDR) are as follows:
  - Table 4.1 – *Parcel Detention Requirements* is revised as shown below:

**Table 4.1 – Parcel Detention Requirements**

Parcel 1 Basin	Tributary Area	Weighted runoff coefficient	Required Volume	Required Volume
			cf	ac-ft
	ac			
<b>A&amp;B*</b>	4.5	0.9	41274	0.95
<b>C</b>	12.35	<b>See Appendix B</b>	<b>47766</b>	<b>1.10</b>
<b>D**</b>	13.5	0.9	106291	2.44
<b>E**</b>	16.7	0.76	111033	2.54
<b>F**</b>	22.1	0.76	146936	3.37
<b>G**</b>	23.8	0.76	158239	3.63
<b>H*</b>	32.8	0.76	254867	5.85
<b>Park*</b>	1.9	0.33	6998	0.16

\* These parcels are built out. Volumes are not updated.



# ADDENDUM No.1

COS # 425-SA-2006 #15-ZN-2006, 124-NP-2006, PC# 3476-06-2

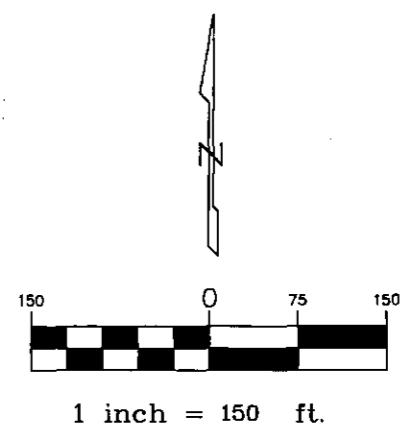
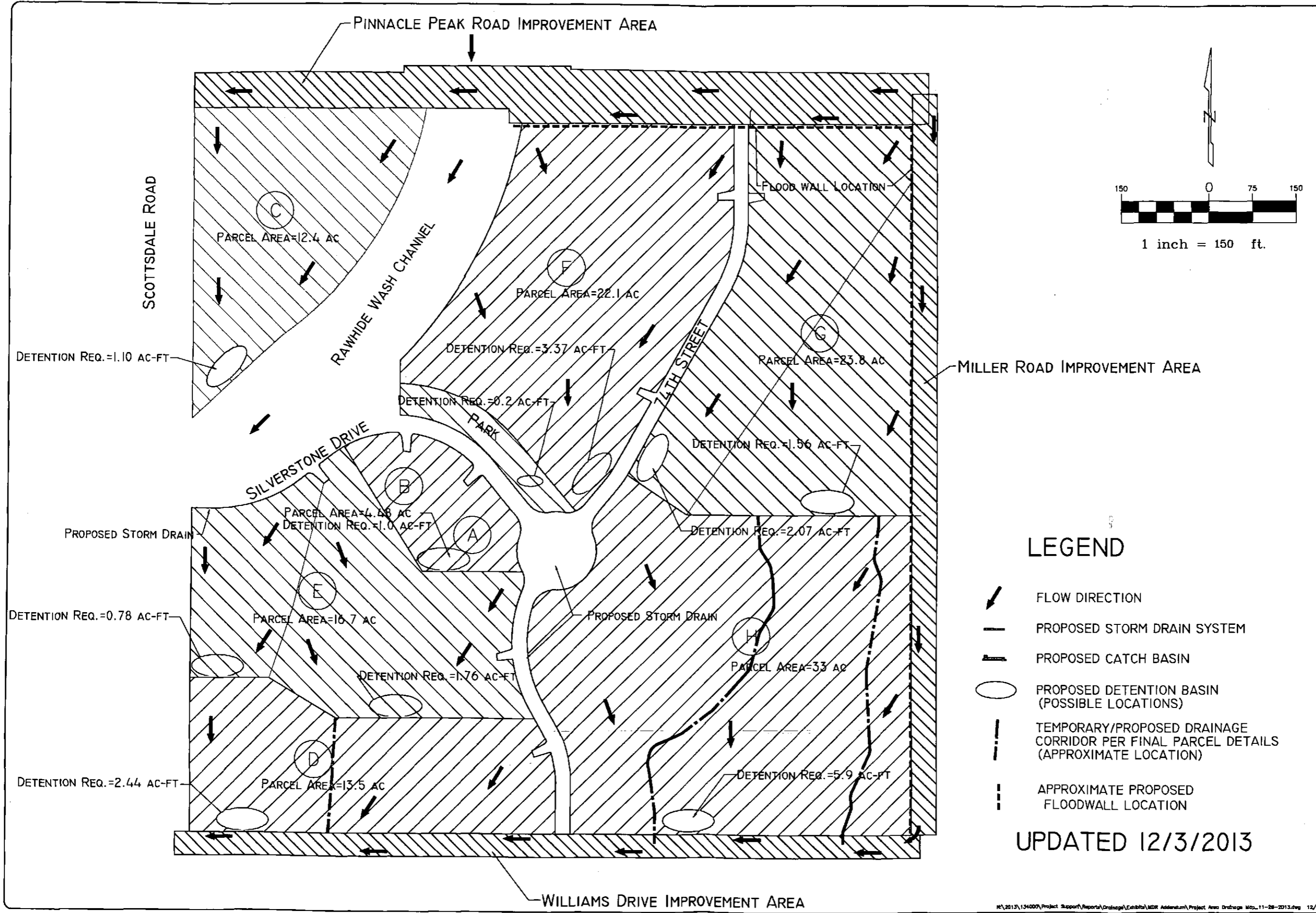
\*\* Required detention volumes for these parcels are updated based upon updated rainfall depth data. Recent changes in C.O.S. stormwater storage policy for previously developed sites may allow for a reduction in required storage volume, but this reduction was not explored for these parcels in this addendum.

- Plate 3 – *Silverstone Drainage Map*, which displays possible locations of proposed detention basins, is revised. The revised plate is attached and dated December 3, 2013.
- Additions to MDR are as follows:
  - Appendix B is added to the MDR and includes the following:
    - Table B1 – *Parcel C Required Stormwater Detention Volume*
    - Table B2 – *Parcel C Pre-Existing Condition Weighted Runoff Coefficients*
    - Table B3 – *Parcel C Post-Developed Condition Weighted Runoff Coefficients*
    - Exhibit 1 – *Pre-Existing Condition Map*
    - Exhibit 2 – *Post-Developed Condition Map*



Darrel E. Wood, P.E., R.L.S.  
Principal

**PLATE 3**  
**Silverstone Drainage Map**



**LEGEND**

- FLOW DIRECTION
- PROPOSED STORM DRAIN SYSTEM
- PROPOSED CATCH BASIN
- PROPOSED DETENTION BASIN (POSSIBLE LOCATIONS)
- TEMPORARY/PROPOSED DRAINAGE CORRIDOR PER FINAL PARCEL DETAILS (APPROXIMATE LOCATION)
- APPROXIMATE PROPOSED FLOODWALL LOCATION

UPDATED 12/3/2013

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PHOTO: BSA • ELPAP

**SILVERSTONE  
SCOTTSDALE  
DRAINAGE MAP**

PRELIMINARY  
**NOT**  
FOR  
CONSTRUCTION  
OR RECORDING

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CHECKED	
DATE	MARCH 07
SCALE	1"=150'
JOB NO.	042309.10
SHEET	1 OF 1

## **APPENDIX B**

**Table B1 – Parcel C Required Stormwater Detention Volume**

**Table B2 – Parcel C Pre-Existing Condition Weighted Runoff Coefficients**

**Table B3 – Parcel C Post-Developed Condition Weighted Runoff Coefficients**

**Exhibit 1 – Pre-Existing Condition Map**

**Exhibit 2 – Post-Developed Condition Map**

**Table B1 - Parcel C Required Stormwater Detention Volume,  $V_{REQ'D}$** 

**Project:** Addendum to Master Drainage Report for Silverstone  
**Location:** Scottsdale, Arizona  
**Description:** Calculation of required stormwater detention volume  
**References:** Design Standards and Policies Manual, Chapter 4 - Grading & Drainage, City of Scottsdale, January 2010  
 NOAA Atlas 14, Volume 1, Version 5, Point Precipitation Frequency Estimates

**Known Values:** Design storm: 100-year, 2-hour  
 Rainfall, P: 2.41 inches

**Calc. Values<sup>1</sup>:**  $V_{REQ'D} = \frac{P}{12} \times A \times C$

Where:  $V_{REQ'D}$  = Required stormwater detention volume  
 P = Rainfall depth for design storm event  
 A = Area of watershed contributing  
 C = Runoff coefficient (see below for calculation)

**For Pre-Existing Developed Areas<sup>2</sup>:**  $C = C_{POST} - C_{PRE}$

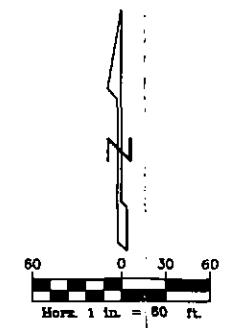
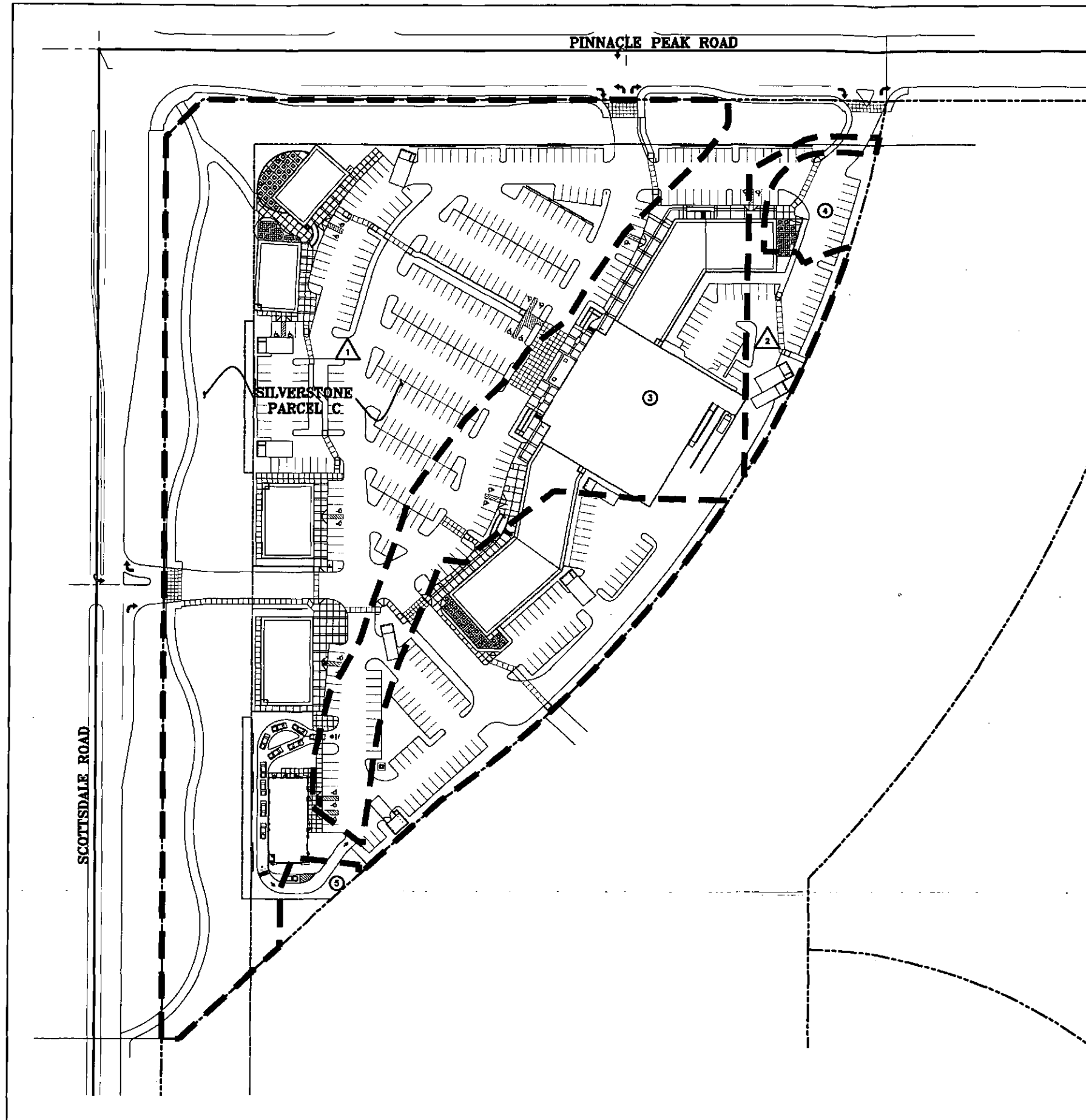
**For Pre-Existing Undeveloped Areas<sup>3</sup>:**  $C = C_{POST}$





Where:  $C_{PRE}$  = Weighted runoff coefficient for pre-existing condition  
 $C_{POST}$  = Weighted runoff coefficient for post-developed condition

**Notes:**

1. Based upon current C.O.S. stormwater storage requirements, areas of the proposed development that were previously developed are required to provide a storage volume equal to the increase in runoff volume generated by the proposed development during the design storm event (100-year, 2-hr storm). Areas of the proposed development that were previously undeveloped are required to provide storage volume equal to the entire runoff volume generated by the proposed development during the design storm event.
2. For previously developed areas, the required storage volume is calculated using a runoff coefficient equal to the increase in the runoff coefficient from the pre-existing to the post-developed condition ( $C=C_{POST}-C_{PRE}$ )
3. For previously undeveloped areas, the required storage volume is calculated using a runoff coefficient equal to the post-developed runoff coefficient ( $C=C_{POST}$ )





- LEGEND**
-  PRE-EXISTING DEVELOPED AREA ID
  -  PRE-EXISTING DEVELOPED AREA BOUNDARY
  -  PRE-EXISTING UNDEVELOPED AREA ID
  -  PROPERTY LINE

**SILVERSTONE**  
PARCEL C  
EXHIBIT 2 - POST-DEVELOPED CONDITION MAP

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ENGINEER D. WOOD  
DESIGNER D. NICHOLS  
CAD TECHNICIAN D. NICHOLS  
SCALE (HORZ) 1"=60'  
SCALE (VERT) N/A  
DATE 12/2/13  
JOB NUMBER 134000

SHEET  
**1 OF 1**





***ADDENDUM No. 2 TO THE  
MASTER DRAINAGE REPORT  
FOR SILVERSTONE***

**Prepared For:**

*Mark-Taylor Residential  
6623 North Scottsdale Road  
Scottsdale, Arizona 85250*

**Prepared By:**

*Kimley-Horn and Associates, Inc.  
7740 North 16<sup>th</sup> Street  
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Phoenix, Arizona 85020*

191769002  
April 2015  
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**Kimley»»Horn**

***ADDENDUM No. 2 TO THE  
MASTER DRAINAGE REPORT  
FOR SILVERSTONE***

**Prepared For:**

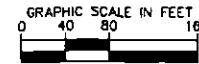
*Mark-Taylor Residential  
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**Prepared By:**

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April 2015  
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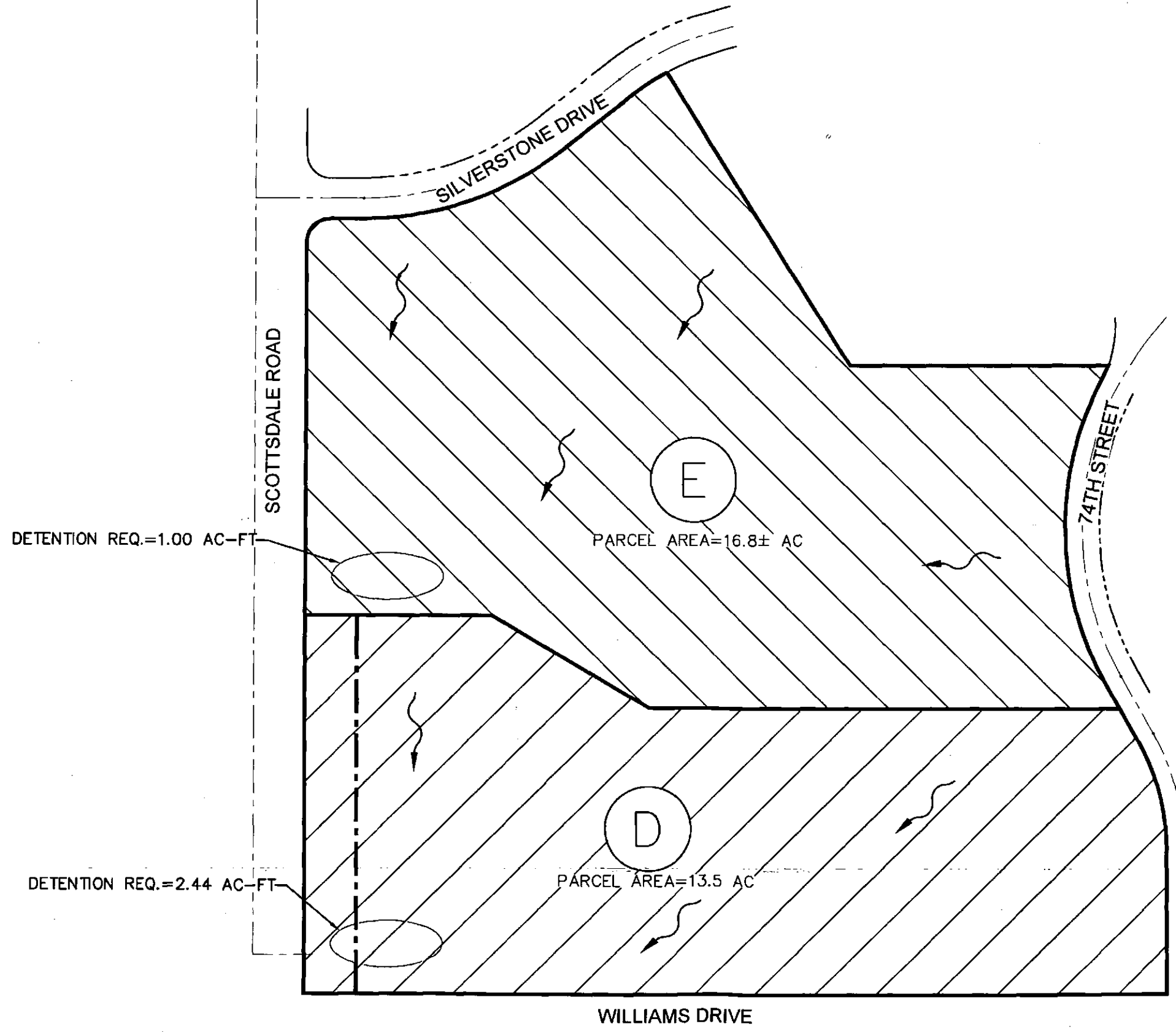
**Kimley»Horn**

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**PLATE 3A - SILVERSTONE DRAINAGE MAP  
(ENLARGED PARCEL E AND D)**

**LEGEND**

-  PROPOSED DETENTION BASIN (POSSIBLE LOCATIONS)
-  PROPOSED FLOW DIRECTION
-  TEMPORARY/PROPOSED DRAINAGE CORRIDOR PER FINAL PARCEL DETAILS (APPROXIMATE LOCATION)



K:\PHX - Civil\Projects\2015\2015-04-23 Master DR Amend Exhibit - Plate 3a.dwg Apr 28, 2015 gerrit.horne  
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APPENDIX B

Table B4- Parcel E Required Stormwater Detention Volume

Table B5- Parcel E Pre-Existing Runoff Coefficients

Table B6- Parcel E Post-Developed Condition Runoff Coefficients

Exhibit 3- Pre-Existing Condition Map Parcel E

Exhibit 4- Post-Developed Condition Map Parcel E

Drainage Area <sup>1</sup>	Surface Type <sup>2</sup>	Area (SF)	Area (AC)	100-YR C Value <sup>2</sup> (C <sub>POST</sub> )
385	Pavement	5,608	0.129	0.95
390	Desert Landscaping (no impervious weed barrier)	2,235	0.051	0.45
395	Pavement	4,186	0.096	0.95
400	Desert Landscaping (no impervious weed barrier)	1,059	0.024	0.45
405	Roof	7,424	0.170	0.95
410	Roof	7,997	0.184	0.95
415	Desert Landscaping (no impervious weed barrier)	28,662	0.658	0.45
420	Desert Landscaping (no impervious weed barrier)	1,290	0.030	0.45
425	Pavement	11,797	0.271	0.95
430	Pavement	33,776	0.775	0.95
435	Desert Landscaping (no impervious weed barrier)	14,014	0.322	0.45
440	Desert Landscaping (no impervious weed barrier)	6,102	0.140	0.45
445	Desert Landscaping (no impervious weed barrier)	58,293	1.338	0.45
OS1	Pavement	14,368	0.330	0.95
OS2	Pavement	12,874	0.296	0.95



SCOTTSDALE ROAD

SILVERSTONE DRIVE

ENTIRE SITE

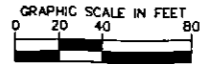
74TH STREET



GRAPHIC SCALE IN FEET  
0 20 40 80

EXHIBIT 3 - PRE-EXISTING  
CONDITION MAP  
PARCEL E

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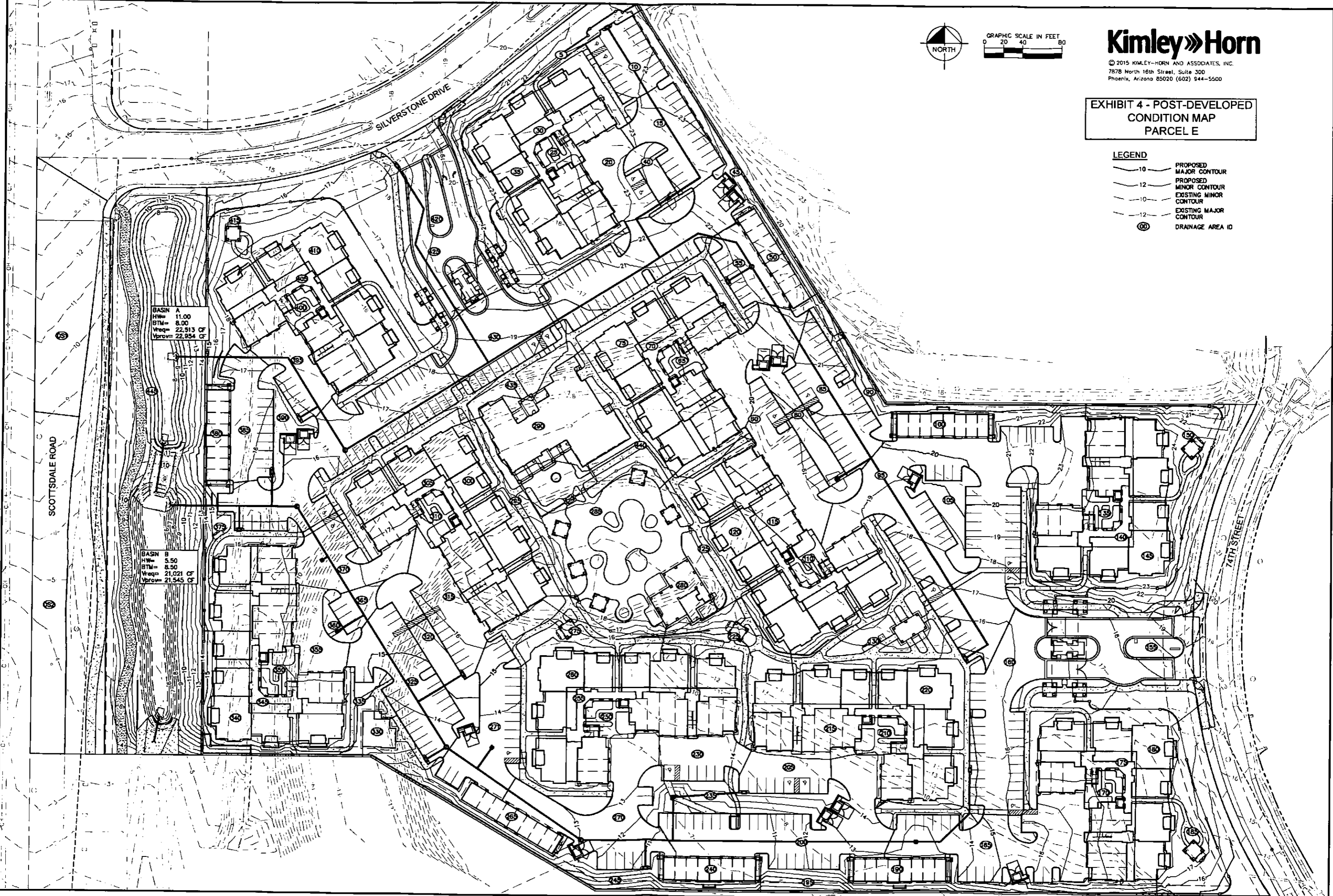
## EXHIBIT 4 - POST-DEVELOPED CONDITION MAP PARCEL E

- LEGEND**
- 10 PROPOSED MAJOR CONTOUR
  - 12 PROPOSED MINOR CONTOUR
  - 10 EXISTING MINOR CONTOUR
  - 12 EXISTING MAJOR CONTOUR
  - DRAINAGE AREA ID

**BASIN A**  
 HW= 11.00  
 STM= 8.00  
 Wreq= 22,313 CF  
 Vreq= 22,954 CF

**BASIN B**  
 HW= 9.50  
 STM= 8.50  
 Wreq= 21,021 CF  
 Vreq= 21,545 CF

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***ADDENDUM No. 3 TO THE  
MASTER DRAINAGE REPORT  
FOR SILVERSTONE***

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**Prepared By:**

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Phoenix, Arizona 85020*



***ADDENDUM No. 3 TO THE  
MASTER DRAINAGE REPORT  
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**Prepared For:**

*K. Hovanian Great West Homes, LLC  
20830 N Tatum Blvd. Suite 350  
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**Prepared By:**

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191948001  
Feb 2016  
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## ADDENDUM No.3

### COS #425-SA-2006, 315-ZN-2006, PC#3476-06-2

- Addendum No.3 to the Master Drainage Report Silverstone- March 2007
- City of Scottsdale, Stormwater Management Division, Approved 3/18/07 (Original), Addendum No. 1 approved 3/6/14, Addendum No. 2 approved April 2015.
- Justification #1: In 2013/2014 a large drainage channel was constructed to convey the Rawhide Wash from the recently constructed box culverts under Pinnacle Peak Road to the recently constructed box culverts under Scottsdale Road. This drainage channel comprises the eastern edge of Parcel F of Silverstone. The design discharge for the Rawhide Wash channel was estimated by URS utilizing USGS Regional Regression Equation #13. The design discharge was 10,900 cfs. The Flood Control District of Maricopa County previously completed a Drainage Study for Rawhide Wash upstream of Happy Valley Road. The Hydrology for this study was completed utilizing HEC-1, following Maricopa County standards. Kimley-Horn extended the HEC-1 model to Scottsdale Road. The 100-year discharge at Scottsdale Road per the extended HEC-1 model is 9,865 cfs. Although the HEC-1 discharge is less than the Regional Regression discharge, it is within the standard error of the Regression Equation. The extended HEC-1 model for Rawhide Wash was then updated to include the proposed runoff from Parcel F. The results show that the onsite discharge does not impact the discharge within the Rawhide Wash.
- Justification #2: The series of retention basins for Parcel F will provide detention for the design storm (100-year 2-hr) runoff volume. Per discussion with the City of Scottsdale areas of the proposed development that were previously developed are only required to provide a storage volume equal to the increase in the runoff volume generated by the proposed development during the design storm event (100-year 2-hr). Silverstone is part of the previously developed Rawhide development of which approximately 9 acres included rooftop and disturbed desert. These 9-acres were credited as existing development and the volume provided is only the increase in development runoff while the remaining 13 acres were calculated to provide the 100% design storm retention requirements.

Revisions to the Master Drainage Report (MDR) are as follows:

- Table 4.1 – Parcel Detention Requirements (Revisions shown in **BOLD**)

*Table 4.1- Parcel Detention Requirements*

Parcel 1 Basin	Tributary Area (ac)	Weighted runoff coefficient	Required Volume (cf)	Required Volume (ac-ft)
A&B	4.5	0.9	41,274	0.95
C*	12.35	See Appendix B	47,766	1.10
D*	13.5	0.9	106,291	2.44
E*	16.7	See Appendix B	43,534	0.99
F	<b>22.1</b>	<b>See Appendix B</b>	<b>98,677</b>	<b>2.27</b>
G*	23.8	0.76	158,239	3.63
H	32.8	0.76	254,867	5.85
Park	1.9	0.33	6,998	0.16

\* Revised in a previous Addendum

Additions to the Master Drainage Report are as follows:

- Plate 3B – Silverstone Drainage Map (Enlarged for Parcel F)
- Table B7 – Parcel F Required Stormwater Detention Volume.
- Exhibit 3 – Existing Condition Map Parcel F
- Exhibit 4 – Developed Condition Map Parcel F
- Rawhide Extended, Existing Condition HEC-1 Model
- Rawhide Extended, Developed Condition HEC-1 Model

PLATE 3B  
Silverstone Drainage Map  
(Enlarged Parcel F)





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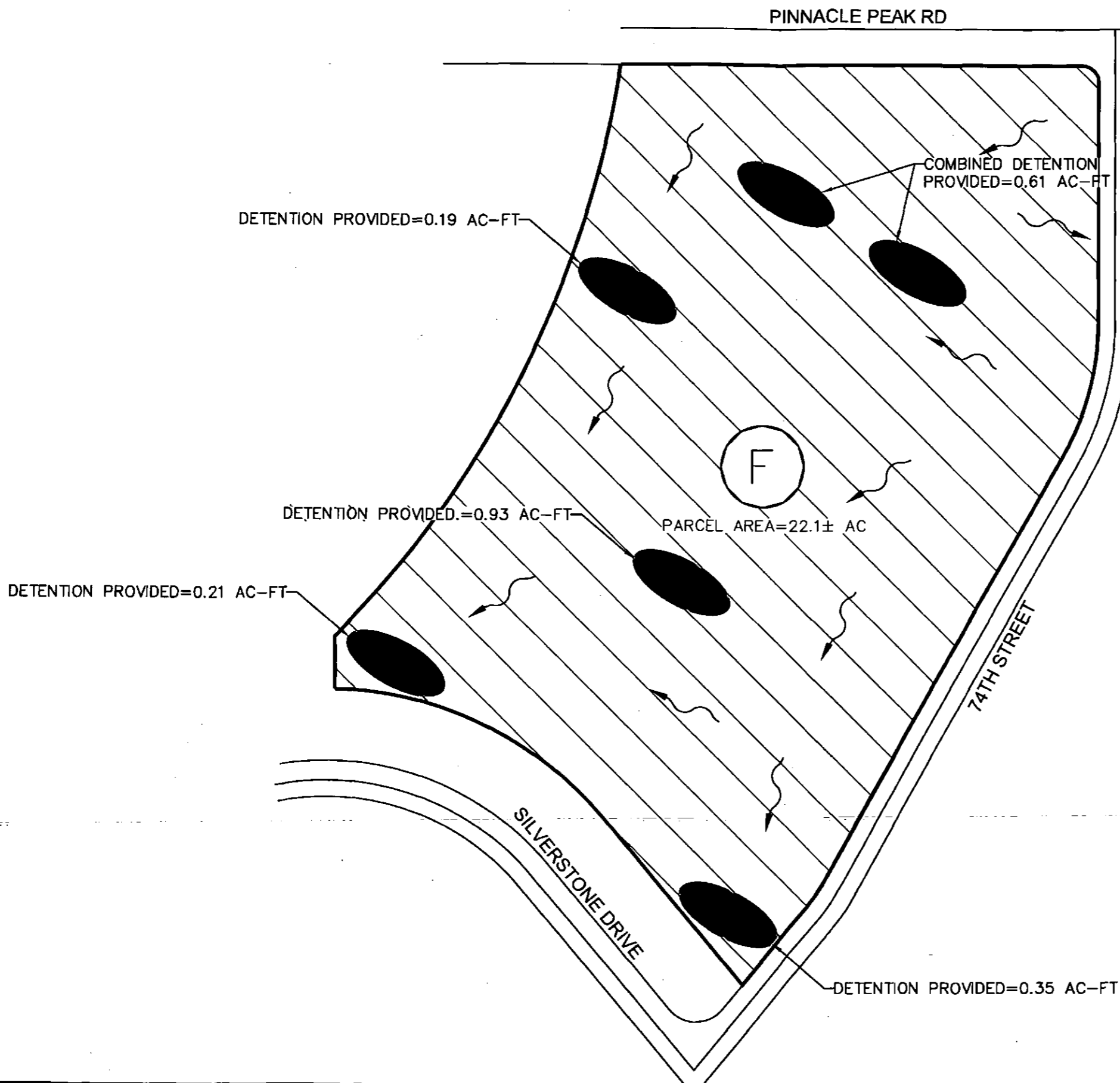
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**PLATE 3B - SILVERSTONE DRAINAGE MAP  
(ENLARGED PARCEL F)**

**LEGEND**

-  PROPOSED DETENTION BASIN (POSSIBLE LOCATIONS)
-  PROPOSED FLOW DIRECTION



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DRAWN BY: bryan.kys  
CHECKED BY: bryan.kys  
SCALE: AS SHOWN  
PROJECT: SILVERSTONE DRIVE AND ADJACENT LOTS  
SHEET: 3B OF 3  
REVISIONS:  
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APPENDIX B

Table B7- Parcel F Required Stormwater Detention Volume

Exhibit 3- Existing Condition Map Parcel F

Exhibit 4- Developed Condition Map Parcel F

Rawhide Extended, Existing Condition HEC-1 Model

Rawhide Extended, Developed Condition HEC-1 Model

**Silverstone Parcel F**

**100 Year 2 Hour Full Retention Calculation**

Drainage Area	Area [A]		Runoff Coefficient [C]	Precipitation Depth [P] in	Required Storage (V <sub>REQ</sub> = CPA/12)	
	sf	ac			cf	ac-ft
	Roadway	207,028			4.753	0.88
Pavement	21,399	0.491	0.95	2.30	3,896	0.09
Landscaping	232,504	5.338	0.45	2.30	20,053	0.46
Building	156,852	3.601	0.95	2.30	28,560	0.66
Duplex	343,201	7.879	0.81	2.30	53,282	1.22
<b>TOTAL</b>	<b>960,984</b>	<b>22.061</b>	<b>0.76</b>	<b>-</b>	<b>140,711</b>	<b>3.23</b>

**Retention Credit for Disturbed Areas**

Drainage Area	Area [A]		Runoff Coefficient [C]	Precipitation Depth [P] in	Required Storage (V <sub>REQ</sub> = CPA/12)	
	sf	ac			cf	ac-ft
	Roof	10,940			0.250	0.95
Disturbed Desert	379,843	8.720	0.55	2.30	40,042	0.92
Undisturbed Desert (no credit)	569,765	13.080	0.00	2.30	0	0.00
<b>TOTAL</b>	<b>960,548</b>	<b>22.051</b>			<b>42,034</b>	<b>0.96</b>

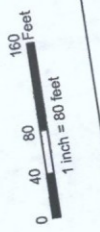
**Pre Vs. Post Retention Calculations**

Drainage Area	Required Storage (V <sub>REQ</sub> = CPA/12)	
	cf	ac-ft
Post	140,711	3.2
Pre	42,034	1.0
Required	98,677	2.27

**Silverstone Weighted C Average**

46' Roadway Tract		
Area	C	
Pavement	40	0.95
Landscape	6	0.45
<b>Weighted Average</b>	<b>46</b>	<b>0.88</b>

Duplex Lots		
Area	C	
Building	6962	0.95
Landscape	2786.5	0.45
<b>Weighted Average</b>	<b>9748.5</b>	<b>0.81</b>



**Kimley & Horn**

Kimley & Horn and Associates, Inc.  
7740 North 16th Street, Suite 300  
Phoenix, Arizona 85020 (602) 944-5500  
Engineering, Planning and Environmental Consultants

SCALE: NONE  
DESIGNED BY: SJA  
DRAWN BY: SJA  
CHECKED BY: SJA  
DATE: DEC 2015

EXISTING CONDITIONS MAP PARCEL F  
SILVERSTONE PARCEL F  
SCOTTSDALE, ARIZONA

PROJECT N  
19194800  
DRAWING F  
EXISTING CON



Flood Control District of Maricopa County  
 Drainage Design Management System  
 SUB BASINS

Area ID	Sub Basin Parameters						Rainfall Losses					Return Period Parameters						
	Area (sq mi)	Length (mi)	Slope (ft/mi)	Adj Slope	Time-Area	Kb	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr	
<b>Major Basin ID: B2</b>																		
CP72	0.502	0.87	119.3	119.3	HEC1 DEFAULT	0.043	0.93*	0.25 *	4.00*	0.500*	28*	Tc (Hrs)	0.627	0.567	0.507	0.446	0.415	0.389
												Vel (f/s)	2.04	2.25	2.52	2.86	3.07	3.28
												R (Hrs)	0.292	0.261	0.231	0.200	0.185	0.172

\* Non default value or value out of range

Flood Control District of Maricopa County  
 Drainage Design Management System  
 SUB BASINS

Area ID	Sub Basin Parameters								Rainfall Losses				
	Area (sq mi)	Length (mi)	Slope (ft/mi)	S-Graph	Lca (mi)	Lag (min)	Velocity (ft/s)	Kn	IA (in)	DTHETA	PSIF (in)	XKSAT (in/hr)	RTIMP (%)
<b>Major Basin ID: ON</b>													
ON01	0.002	0.10	76.8	VALLEY	0.05	1.70	5.06	0.020	0.25	0.25	4.03	0.562	40
ON02	0.008	0.20	55.9	VALLEY	0.10	3.00	5.71	0.020	0.25	0.25	3.90	0.603	40
ON04	0.007	0.17	76.5	VALLEY	0.09	2.60	5.80	0.020	0.25	0.25	4.03	0.566	40
ON05	0.004	0.08	85.2	VALLEY	0.04	1.40	5.09	0.020	0.25	0.25	4.03	0.562	40
ON06	0.004	0.10	158.3	VALLEY	0.05	1.40	5.84	0.020	0.25	0.25	4.03	0.562	40

\* Non default value

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1+-----+
|-----+
| *      *
| *      *
| * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
| *      *
| *   JUN 1998 *
| *      *
| *   VERSION 4.1 *
| *      *
| *      *
| * RUN DATE 19APR16 TIME 10:31:21 *
| *      *
|-----+

```

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*
* U.S. ARMY CORPS OF ENGINEERS
*
* HYDROLOGIC ENGINEERING CENTER
*
* 609 SECOND STREET
*
* DAVIS, CALIFORNIA 95616
*
* (916) 756-1104
*

```

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X   X   XXXXXXXX   XXXXX   X
X   X   X   X   X   XX
X   X   X   X   X   X
XXXXXXX   XXXX   X   XXXXX   X
X   X   X   X   X   X
X   X   X   X   X   X
X   X   XXXXXXXX   XXXXX   XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID DDM MCHUP1 Upper Rawhide Wash - Existing Conditions 100 yr, 24 hr storm
.....
Flood Control District of Maricopa County
Upper Rawhide Wash Floodplain Delineation Study
FCD98-12
DEVELOPED CONDITIONS HYDROLOGY W/ CONTRIBUTION FROM SILVERSTONE PARCEL F
100 yr, 24 hr storm event
Prepared by Kimley-Horn and Associates, Inc.
KHA Job No. 191948001
UPDATED DECEMBER 1, 2015
This hydrology represents conditions of the Upper Rawhide Wash and its
tributaries in existing conditions.
There are two existing flow splits at CP062 and CP066, they will be cut off
and routed 100% to the dam site as part of the dam project. This model
accounts for the proposed diversion works with Rawhide Wash Dam.
Input parameters for the model were determined using the Flood Control
District of Maricopa County's (FCDMC) Drainage Design Menu System (DDMS).
The values entered into DDMS were based on the following:
Rainfall depths Point precipitation values were determined using
the isopiuvial maps in the FCDMC Hydrology Manual
Rainfall excess The Green and Ampt Methodology was used for
estimation of rainfall losses. Digital soil
maps for the City of Scottsdale were used to
determine soil distributions.
Existing land use conditions were determined from
the City of Scottsdale General Plan digital
zoning map and aerial photos.
Unit hydrographs The Clark Unit Hydrograph methodology was applied.
City of Scottsdale digital topographic mapping
with 1' and 2' contour intervals was used.
Routing Normal Depth Channel Routing was used with 8 point
cross sections approximated from the BOSS River
Modeling System.

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* .....
*
*DIAGRAM
IT      2          750
3      IO      3
4      IN      15
* DDM ***** Preserved *****

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1

HEC-1 INPUT

PAGE 2

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
5      KK      005
6      KM      SUB-BASIN 005
7      KM      24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN
8      KM      THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928
9      KM      L = 2.10 Kb = .047 Adj. Slope = 130.0
10     BA      .417
11     IN      15
12     KM      RAINFALL DEPTH OF 4.60 WAS SPACIALLY REDUCED AS SHOWN BY THE PB RECORD
13     PB      4.269
14     KM      THE FOLLOWING PC RECORD USED A 24-HR SCS TYPE II STORM
15     PC      .000 .002 .005 .008 .011 .014 .017 .020 .023 .026
16     PC      .029 .032 .035 .038 .041 .044 .048 .052 .056 .060
17     PC      .064 .068 .072 .076 .080 .085 .090 .095 .100 .105
18     PC      .110 .115 .120 .126 .133 .140 .147 .155 .163 .172
19     PC      .181 .191 .203 .218 .236 .257 .283 .387 .663 .707
20     PC      .735 .758 .776 .791 .804 .815 .825 .834 .842 .849
21     PC      .856 .863 .869 .875 .881 .887 .893 .898 .903 .908
22     PC      .913 .918 .922 .926 .930 .934 .938 .942 .946 .950
23     PC      .953 .956 .959 .962 .965 .968 .971 .974 .977 .980
24     PC      .983 .986 .989 .992 .995 .998 1.000
25     LG      .150 .390 6.200 .150 3.000
26     UC      .475 .483
27     UA      0      3      5      8      12      20      43      75      90      96
28     UA      100
* DDM ***** Preserved *****

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29     KK      R010-1
30     KM      Route the runoff hydrograph from CP005 to CP010
31     RS      1      FLOW      -1
32     RC      0.045 0.035 0.045 884 0.024
33     RX      0      40      156      172      201      253      546      571
34     RY      2870 2868 2868 2866 2866 2868 2868 2870
* DDM ***** Preserved *****

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35     KK      010
36     KM      SUB-BASIN 010
37     KM      24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN
38     KM      THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928
39     KM      L = 1.93 Kb = .048 Adj. Slope = 146.0
40     BA      .336
41     LG      .150 .390 5.800 .170 3.000
42     UC      .442 .471
43     UA      0      3      5      8      12      20      43      75      90      96
44     UA      100
* DDM ***** Preserved *****

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45     KK      CP010
46     KM      Combine routed hydrograph from 005 and runoff hydrograph from 010
47     HC      2
* DDM ***** Preserved *****

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1

HEC-1 INPUT

PAGE 3

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
48     KK      R012-1
49     KM      Route the combined hydrograph from CP010 to CP012
50     RS      5      FLOW      -1
51     RC      0.045 0.035 0.045 3716 0.019
52     RX      261 486 555 684 700 716 743 778
53     RY      2794 2792 2792 2790 2788 2790 2792 2794
* DDM ***** Preserved *****

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54     KK      012
55     KM      SUB-BASIN 012
56     KM      24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN
57     KM      THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928
58     KM      L = 1.68 Kb = .049 Adj. Slope = 117.0
59     BA      .297
60     LG      .150 .370 5.200 .220 3.000
61     UC      .450 .461
62     UA      0      3      5      8      12      20      43      75      90      96
63     UA      100
* DDM ***** Preserved *****

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64     KK      CP012
65     KM      Combine runoff hydrograph from 012 with routed hydrograph from CP010
66     HC      2
* DDM ***** Preserved *****

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67     KK      D012
68     KM      Divert flow away from main Rawhide Wash to the West.
69     DT      012OUT

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70 DI 0 43 152 330 586 929 1365  
 71 DQ 0 10 43 105 203 344 533  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*  
 72 KK R030-1  
 73 KM Route the combined hydrograph from CP012 to CP030A.  
 74 RS 26 FLOW -1  
 75 RC 0.045 0.035 0.045 10780 0.018  
 76 RX 428 497 753 780 835 868 897 992  
 77 RY 2684 2680 2680 2682 2682 2680 2680 2684  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

78 KK 030  
 79 KM SUB-BASIN 030  
 80 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 81 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 82 KM L = 2.38 Kb = .048 Adj. Slope = 100.0  
 83 BA .355  
 84 LG .150 .390 5.700 .180 8.000  
 85 UC .600 .758  
 86 UA 0 3 5 8 12 20 43 75 90 96  
 87 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

1

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

88 KK CP030A  
 89 KM Combine routed hydrograph from CP012 with runoff hydrograph 030.  
 90 KM This combination was made for use in the hydraulic modeling of a  
 91 KM tributary to Rawhide Wash.  
 92 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

93 KK 015  
 94 KM SUB-BASIN 015  
 95 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 96 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 97 KM L = 2.54 Kb = .040 Adj. Slope = 108.0  
 98 BA 1.268  
 99 LG .150 .390 5.800 .170 3.000  
 100 UC .525 .333  
 101 UA 0 3 5 8 12 20 43 75 90 96  
 102 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

103 KK R022-1  
 104 KM Route the runoff hydrograph from 015 to CP022A.  
 105 RS 6 FLOW -1  
 106 RC 0.045 0.035 0.045 4466 0.018  
 107 RX 9714 9798 9905 9943 10002 10037 10299 10412  
 108 RY 2740 2734 2734 2728 2726 2728 2730 2740  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

109 KK 017  
 110 KM SUB-BASIN 017  
 111 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 112 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 113 KM L = 1.34 Kb = .051 Adj. Slope = 113.0  
 114 BA .216  
 115 LG .150 .390 6.200 .150 5.000  
 116 UC .400 .405  
 117 UA 0 3 5 8 12 20 43 75 90 96  
 118 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

119 KK CP022A  
 120 KM Combine routed hydrograph from 015 with runoff hydrograph from 017.  
 121 KM This combination was made for use in the hydraulic modeling of a  
 122 KM tributary to Rawhide Wash.  
 123 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

124 KK 020  
 125 KM SUB-BASIN 020  
 126 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 127 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 128 KM L = 1.74 Kb = .043 Adj. Slope = 199.0  
 129 BA .761  
 130 LG .150 .380 6.400 .140 12.000  
 131 UC .338 .202

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HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

132 UA 0 3 5 8 12 20 43 75 90 96  
 133 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

134 KK R022-2  
 135 KM Route the runoff hydrograph from 020 to CP022B  
 136 RS 6 FLOW -1  
 137 RC 0.045 0.035 0.045 4146 0.017  
 138 RX 9805 9902 10081 10090 10098 10102 10253 10264

139 RY 2690 2686 2688 2686 2686 2688 2688 2690  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*  
 140 KK 022  
 141 KM SUB-BASIN 022  
 142 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 143 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 144 KM L = 2.71 Kb = .047 Adj. Slope = 128.0  
 145 BA .381  
 146 LG .150 .360 6.800 .120 8.000  
 147 UC .563 .752  
 148 UA 0 3 5 8 12 20 43 75 90 96  
 149 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

150 KK CP022B  
 151 KM Combine routed hydrograph from 020 with runoff hydrograph from 022.  
 152 KM This combination was made for use in the hydraulic modeling of a  
 153 KM tributary to Rawhide Wash.  
 154 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

155 KK CP022  
 156 KM Combine hydrographs from CP022A and CP022B.  
 157 KM This combination was made for use in the hydraulic modeling of a  
 158 KM tributary to Rawhide Wash.  
 159 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

160 KK 024  
 161 KM SUB-BASIN 024  
 162 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 163 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 164 KM L = 1.35 Kb = .041 Adj. Slope = 294.0  
 165 BA 1.020  
 166 LG .150 .380 6.400 .140 12.000  
 167 UC .242 .096  
 168 UA 0 3 5 8 12 20 43 75 90 96  
 169 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

170 KK R026-1  
 171 KM Route the runoff hydrograph from 024 to CP026A.  
 172 RS 6 FLOW -1  
 173 RC 0.045 0.035 0.045 6097 0.016  
 174 RX 9712 9782 9851 9931 9950 10050 10176 10237  
 175 RY 2700 2692 2688 2688 2686 2686 2696 2700  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

176 KK 026  
 177 KM SUB-BASIN 026  
 178 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 179 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 180 KM L = 2.41 Kb = .049 Adj. Slope = 249.8  
 181 BA .269  
 182 LG .150 .350 7.000 .110 24.000  
 183 UC .404 .578  
 184 UA 0 3 5 8 12 20 43 75 90 96  
 185 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

186 KK CP026A  
 187 KM Combine routed hydrograph from 024 with runoff hydrograph from 026.  
 188 KM This combination was made for use in the hydraulic modeling of a  
 189 KM tributary to Rawhide Wash.  
 190 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

191 KK CP026  
 192 KM Combine hydrographs from CP022 and CP026A.  
 193 KM This combination was made for use in the hydraulic modeling of a  
 194 KM tributary to Rawhide Wash.  
 195 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

196 KK R030-2  
 197 KM Route the combined hydrograph from CP026 to CP030  
 198 RS 1 FLOW -1  
 199 RC 0.045 0.035 0.045 1354 0.016  
 200 RX 214 314 479 518 654 771 852 939  
 201 RY 2604 2598 2598 2596 2596 2594 2594 2604  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

202 KK CP030  
 203 KM Combine hydrograph from CP026 and CP030A.  
 204 KM This combination was made for use in the hydraulic modeling of a  
 205 KM tributary to Rawhide Wash.  
 206 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

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HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

207 KK R038-1  
208 KM Route combined hydrograph from CP030 to CP038  
209 RS 2 FLOW -1  
210 RC 0.045 0.035 0.045 1605 0.012  
211 RX 290 378 536 538 607 650 727 753  
212 RY 2582 2576 2574 2572 2572 2580 2580 2582  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

213 KK 034  
214 KM SUB-BASIN 034  
215 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
216 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
217 KM L = 1.32 Kb = .049 Adj. Slope = 97.0  
218 BA .298  
219 LG .150 .380 6.400 .140 16.000  
220 UC .408 .341  
221 UA 0 3 5 8 12 20 43 75 90 96  
222 UA 100  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

223 KK R036-1  
224 KM Route runoff hydrograph from 034 to CP036  
225 RS 8 FLOW -1  
226 RC 0.045 0.035 0.045 5295 0.019  
227 RX 9725 9842 9891 9970 10030 10032 10143 10241  
228 RY 2652 2646 2646 2642 2642 2644 2644 2652  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

229 KK 036  
230 KM SUB-BASIN 036  
231 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
232 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
233 KM L = 1.37 Kb = .050 Adj. Slope = 278.2  
234 BA .250  
235 LG .150 .350 7.000 .110 19.000  
236 UC .275 .250  
237 UA 0 3 5 8 12 20 43 75 90 96  
238 UA 100  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

239 KK CP036  
240 KM Combine routed hydrograph from 034 with runoff hydrograph from 036  
241 HC 2  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

242 KK R038-2  
243 KM Route combined hydrograph from CP036 to CP038A.  
244 RS 1 FLOW -1  
245 RC 0.045 0.035 0.045 483 0.025  
246 RX 9837 9891 9971 9988 10012 10126 10148 10177  
247 RY 2578 2574 2574 2572 2572 2574 2576 2578  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

248 KK 038  
249 KM SUB-BASIN 038  
250 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
251 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
252 KM L = 1.27 Kb = .052 Adj. Slope = 303.0  
253 BA .183  
254 LG .150 .370 6.600 .130 16.000  
255 UC .262 .267  
256 UA 0 3 5 8 12 20 43 75 90 96  
257 UA 100  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

258 KK CP038A  
259 KM Combine routed hydrograph from CP036 with runoff hydrograph from 038.  
260 KM This combination was made for use in the hydraulic modeling of a  
261 KM tributary to Rawhide Wash.  
262 HC 2  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

263 KK 032  
264 KM SUB-BASIN 032  
265 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
266 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
267 KM L = .40 Kb = .062 Adj. Slope = 100.0  
268 BA .033  
269 LG .150 .350 4.550 .310 3.000  
270 UC .233 .247  
271 UA 0 3 5 8 12 20 43 75 90 96  
272 UA 100  
\* DDM \*\*\*\*\* Preserved \*\*\*\*\*

273 KK CP038  
274 KM Combine hydrograph from CP038a with runoff hydrographs from 032.  
275 KM This combination was made for use in the hydraulic modeling of a  
276 KM tributary to Rawhide Wash.

277 HC 3  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

278 KK R040-1  
 279 KM Route combined hydrograph from CP038 to CP040  
 280 RS 4 FLOW -1  
 281 RC 0.045 0.035 0.045 3775 0.019  
 282 RX 47 75 224 433 448 523 609 678  
 283 RY 2540 2536 2530 2530 2528 2528 2534 2540  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

284 KK 040  
 285 KM SUB-BASIN 040  
 286 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 287 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 288 KM L = 1.56 Kb = .048 Adj. Slope = 245.0  
 289 BA .328  
 290 LG .150 .380 5.600 .190 5.000  
 291 UC .317 .278

HEC-1 INPUT

PAGE 9

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

292 UA 0 3 5 8 12 20 43 75 90 96  
 293 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

294 KK CP040  
 295 KM Combine routed hydrographs from CP038 with runoff hydrograph from 040  
 296 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

297 KK R042-1  
 298 KM Route combined hydrograph from CP040 to CP042  
 299 RS 2 FLOW -1  
 300 RC 0.045 0.035 0.045 2074 0.018  
 301 RX 263 349 381 450 522 756 847 973  
 302 RY 2486 2482 2476 2482 2476 2476 2482 2486  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

303 KK 041  
 304 KM SUB-BASIN 041  
 305 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 306 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 307 KM L = .69 Kb = .058 Adj. Slope = 133.0  
 308 BA .064  
 309 LG .150 .350 4.800 .260 3.000  
 310 UC .271 .309  
 311 UA 0 3 5 8 12 20 43 75 90 96  
 312 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

313 KK 042  
 314 KM SUB-BASIN 042  
 315 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 316 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 317 KM L = 1.51 Kb = .048 Adj. Slope = 296.4  
 318 BA .325  
 319 LG .150 .400 6.000 .160 8.000  
 320 UC .283 .241  
 321 UA 0 3 5 8 12 20 43 75 90 96  
 322 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

323 KK CP042  
 324 KM Combine routed hydrograph from CP040 with runoff hydrographs from 041 & 042  
 325 HC 3  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

326 KK R043-1  
 327 KM Route combined hydrograph from CP040 to CP042  
 328 RS 7 FLOW -1  
 329 RC 0.045 0.035 0.045 7504 0.017  
 330 RX 263 349 381 450 522 756 847 973  
 331 RY 2486 2482 2476 2482 2476 2476 2482 2486  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

HEC-1 INPUT

PAGE 10

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

332 KK 043  
 333 KM SUB-BASIN 043  
 334 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 335 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 336 KM L = 1.92 Kb = .049 Adj. Slope = 99.0  
 337 BA .286  
 338 LG .150 .350 4.650 .290 3.000  
 339 UC .538 .639  
 340 UA 0 3 5 8 12 20 43 75 90 96  
 341 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

342 KK CP043



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343 KM Combine routed hydrograph from CP042 with runoff hydrograph from 043
344 HC 2
* DDM ***** Preserved *****

345 KK R046-1
346 KM Route combined hydrograph from CP043 to CP046
347 RS 4 FLOW -1
348 RC 0.045 0.035 0.045 2845 0.015
349 RX 9425 9601 9923 9945 10019 10043 10267 10435
350 RY 2300 2288 2288 2286 2286 2288 2288 2302
* DDM ***** Preserved *****

351 KK 044
352 KM SUB-BASIN 044
353 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN
354 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928
355 KM L = 2.99 Kb = .039 Adj. Slope = 240.0
356 BA 1.400
357 LG .150 .390 5.800 .170 5.000
358 UC .417 .278
359 UA 0 3 5 8 12 20 43 75 90 96
360 UA 100
* DDM ***** Preserved *****

361 KK R046-2
362 KM Route runoff hydrograph from 044 to CP046
363 RS 4 FLOW -1
364 RC 0.045 0.035 0.045 1882 0.016
365 RX 9425 9601 9923 9945 10019 10043 10267 10435
366 RY 2300 2288 2288 2286 2286 2288 2288 2302
* DDM ***** Preserved *****

367 KK 046
368 KM SUB-BASIN 046
369 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN
370 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928
371 KM L = .67 Kb = .055 Adj. Slope = 101.0
372 BA .111
373 LG .150 .360 5.100 .230 3.000
374 UC .287 .236
375 UA 0 3 5 8 12 20 43 75 90 96
HEC-1 INPUT

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PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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376 UA 100
* DDM ***** Preserved *****

377 KK CP046
378 KM Combine routed hydrographs from CP042 and 044 with runoff hydrograph from
379 KM 046
380 HC 3
* DDM ***** Preserved *****

381 KK R051-1
382 KM Route combined hydrograph from CP046 to CP051
383 RS 2 FLOW -1
384 RC 0.045 0.035 0.045 1557 0.017
385 RX 9822 9952 9976 9977 10023 10046 10356 10509
386 RY 2275 2261 2262 2260 2260 2263 2264 2275
* DDM ***** Preserved *****

387 KK 048
388 KM SUB-BASIN 048
389 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN
390 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928
391 KM L = .43 Kb = .059 Adj. Slope = 112.0
392 BA .056
393 LG .150 .350 4.900 .250 3.000
394 UC .225 .186
395 UA 0 3 5 8 12 20 43 75 90 96
396 UA 100
* DDM ***** Preserved *****

397 KK 051
398 KM SUB-BASIN 051
399 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN
400 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928
401 KM L = 1.41 Kb = .052 Adj. Slope = 128.0
402 BA .164
403 LG .150 .390 6.200 .150 3.000
404 UC .400 .494
405 UA 0 3 5 8 12 20 43 75 90 96
406 UA 100
* DDM ***** Preserved *****

407 KK CP051
408 KM Combine routed hydrograph from CP046 with runoff hydrographs from 048 and 051
409 HC 3
* DDM ***** Preserved *****

410 KK R053-1
411 KM Route combined hydrograph from CP051 to CP053
412 RS 1 FLOW -1
413 RC 0.045 0.035 0.045 1681 0.030

```

414 RX 9910 9935 9963 9987 10041 10046 10075 10085  
 415 RY 2249 2245 2245 2238 2238 2239 2239 2242  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

416 KK 053  
 417 KM SUB-BASIN 053  
 418 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 419 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 420 KM L = .66 Kb = .056 Adj. Slope = 126.0  
 421 BA .094  
 422 LG .150 .370 6.600 .130 3.000  
 423 UC .258 .227  
 424 UA 0 3 5 8 12 20 43 75 90 96  
 425 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

426 KK CP053  
 427 KM Combine routed hydrographs from CP051 with runoff hydrograph from 053  
 428 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

429 KK R055-1  
 430 KM Route combined hydrograph from CP053 to CP055  
 431 RS 1 FLOW -1  
 432 RC 0.045 0.035 0.045 784 0.019  
 433 RX 111 154 304 389 510 725 836 937  
 434 RY 2208 2202 2200 2197 2199 2200 2199 2208  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

435 KK 055  
 436 KM SUB-BASIN 055  
 437 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 438 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 439 KM L = 2.43 Kb = .047 Adj. Slope = 125.0  
 440 BA .366  
 441 LG .150 .400 6.000 .160 5.000  
 442 UC .538 .670  
 443 UA 0 3 5 8 12 20 43 75 90 96  
 444 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

445 KK CP055  
 446 KM Combine routed hydrograph from CP053 with runoff hydrograph from 055  
 447 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

448 KK R068-1  
 449 KM Route combined hydrograph from CP055 to CP068  
 450 RS 2 FLOW -1  
 451 RC 0.045 0.035 0.045 1728 0.017  
 452 RX 111 154 304 389 510 725 836 937  
 453 RY 2208 2202 2200 2197 2199 2200 2199 2208  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

454 KK 058  
 455 KM SUB-BASIN 058  
 456 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 457 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 458 KM L = 2.86 Kb = .038 Adj. Slope = 120.0  
 459 BA 1.756  
 460 LG .150 .400 6.000 .160 4.000  
 461 UC .525 .304  
 462 UA 0 3 5 8 12 20 43 75 90 96  
 463 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

464 KK R064-1  
 465 KM Route runoff hydrograph from 058  
 466 RS 7 FLOW -1  
 467 RC 0.045 0.035 0.045 7694 0.023  
 468 RX 0 76 102 111 117 118 168 205  
 469 RY 2323 2321 2319 2319 2320 2321 2323 2325  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

470 KK 060  
 471 KM SUB-BASIN 060  
 472 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 473 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 474 KM L = 3.39 Kb = .043 Adj. Slope = 152.0  
 475 BA .778  
 476 LG .150 .390 6.200 .140 6.000  
 477 UC .583 .623  
 478 UA 0 3 5 8 12 20 43 75 90 96  
 479 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

480 KK R062-1

481 KM Route runoff hydrograph from 060  
 482 RS 1 FLOW -1  
 483 RC 0.045 0.035 0.045 441 0.018  
 484 RX 0 20 38 41 45 60 94 128  
 485 RY 2328.5 2328 2327 2327 2326 2326 2327 2328.5  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*  
 486 KK 062  
 487 KM SUB-BASIN 062  
 488 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 489 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 490 KM L = 2.26 Kb = .043 Adj. Slope = 158.0  
 491 BA .730  
 492 LG .150 .400 6.000 .150 8.000  
 493 UC .438 .340  
 494 UA 0 3 5 8 12 20 43 75 90 96  
 495 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

HEC-1 INPUT

PAGE 14

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

496 KK CP062  
 497 KM Combine routed hydrograph from 060 with runoff hydrograph from 062  
 498 HC 2  
 \*  
 \* Currently flow splits at this location. With the Rawhide Wash Dam project  
 \* these flows will be cut off and routed to the dam site. The routing in this  
 \* model indicates this.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*  
 499 KK R064-2  
 500 KM Route combined hydrograph from CP062  
 501 RS 1 FLOW -1  
 502 RC 0.045 0.035 0.045 1554 0.024  
 503 RX 0 17 23 73 83 90 99 107  
 504 RY 2316 2316 2315 2315 2316 2317 2318 2319  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

505 KK 064  
 506 KM SUB-BASIN 064  
 507 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 508 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 509 KM L = 1.80 Kb = .046 Adj. Slope = 149.0  
 510 BA .473  
 511 LG .150 .390 6.200 .150 5.000  
 512 UC .404 .332  
 513 UA 0 3 5 8 12 20 43 75 90 96  
 514 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

515 KK CP064  
 516 KM Combine routed hydrographs from 058 and CP062 with runoff hydrograph from  
 517 KM 064  
 518 HC 3  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

519 KK R066-1  
 520 KM Route combined hydrograph from CP064  
 521 RS 1 FLOW -1  
 522 RC 0.045 0.035 0.045 934 0.021  
 523 RX 0 31 66 104 111 121 129 149  
 524 RY 2281 2281 2280 2280 2281 2282 2283 2284  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

525 KK 066  
 526 KM SUB-BASIN 066  
 527 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 528 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 529 KM L = .18 Kb = .072 Adj. Slope = 122.0  
 530 BA .006  
 531 LG .150 .350 4.200 .360 3.000  
 532 UC .158 .224  
 533 UA 0 3 5 8 12 20 43 75 90 96  
 534 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

HEC-1 INPUT

PAGE 15

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

535 KK CP066  
 536 KM Combine routed hydrograph from CP064 with runoff hydrograph from 066  
 537 HC 2  
 \*  
 \* Currently flow splits at this location. With the Rawhide Wash Dam project  
 \* these flows will be cut off and routed to the dam site. The routing in this  
 \* model indicates this.  
 \*  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

538 KK R068-2  
 539 KM Route combined hydrograph from CP066

540 RS 4 FLOW -1  
 541 RC 0.045 0.035 0.045 3234 0.025  
 542 RX 198 224 270 283 684 688 721 769  
 543 RY 2228 2227 2227 2226.5 2226.5 2227 2227 2228  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

544 KK 068  
 545 KM SUB-BASIN 068  
 546 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 547 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 548 KM L = 1.05 Kb = .050 Adj. Slope = 133.0  
 549 BA .248  
 550 LG .150 .360 5.100 .220 3.000  
 551 UC .321 .241  
 552 UA 0 3 5 8 12 20 43 75 90 96  
 553 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

554 KK CP068  
 555 KM Combine routed hydrographs from CP055 and CP066 with runoff hydrograph from  
 556 KM 068  
 557 HC 3  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

558 KK R070-1  
 559 KM Route combined hydrograph from CP068  
 560 RS 7 FLOW -1  
 561 RC 0.045 0.035 0.045 7672 0.019  
 562 RX 65.2 145.6 307.4 320.0 550.3 564.7 809.8 963.5  
 563 RY 2130 2127 2127 2125.5 2125.5 2124 2124 2130  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

564 KK 070  
 565 KM SUB-BASIN 070  
 566 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 567 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 568 KM L = 2.68 Kb = .043 Adj. Slope = 131.0  
 569 BA .794  
 570 LG .170 .340 4.500 .310 3.000  
 571 UC .550 .478  
 572 UA 0 3 5 8 12 20 43 75 90 96  
 573 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

1

HEC-1 INPUT

PAGE 16

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

574 KK CP070  
 575 KM Combine routed hydrograph from CP068 with runoff hydrograph from 070  
 576 HC 2  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

577 KK R072-1  
 578 KM Route combined hydrograph from CP070  
 579 RS 8 FLOW -1  
 580 RC 0.045 0.035 0.045 8409 0.023  
 581 RX 0 38.0 77.0 109.0 196.5 270.0 328.5 364.5  
 582 RY 1902 1900 1898 1897.0 1899.0 1897 1896 1901  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

583 KK 072  
 584 KM SUB-BASIN 072  
 585 KM 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 586 KM THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 587 KM L = 0.87 Kb = .043 Adj. Slope = 119.3  
 588 BA .502  
 589 LG .110 .250 4.000 .500 28.200  
 590 UC .389 .172  
 591 UA 0 3 5 8 12 20 43 75 90 96  
 592 UA 100  
 \* DDM \*\*\*\*\* Preserved \*\*\*\*\*

593 KK CP072  
 594 KM Combine routed hydrograph from CP070 with runoff hydrograph from 072  
 595 HC 2  
 \*

596 KK ON01 BASIN  
 597 BA 0.004  
 598 LG 0.25 0.25 4.03 0.56 40  
 599 UI 0 25 13 0  
 \*

600 KK RET1A  
 601 KM UG RETENTION BASIN 1A  
 602 RS 1 STOR 0  
 603 SV 0 0.03 0.16 0.32 0.47 0.60 0.63  
 604 SE 0 1 3 5 7 9 10  
 605 SS 9.9 1 2.3 1.5  
 606 SL 0.17 0.0871 0.6 0.5  
 \*

607 KK ON02 BASIN  
 608 BA 0.007

609	LG	0.25	0.25	3.90	0.60	40	
610	UI	0	32	85	32	5	0

1

HEC-1 INPUT

PAGE 17

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

611	KK	1BIN					
612	KM	INFLOW TO RETENTION BASIN 1B					
613	HC	2					
	*						
614	KK	RET1B					
615	KM	RETENTION BASIN 1B					
616	RS	1 STOR	0				
617	SV	0	0.037	0.105	0.205		
618	SE	0	1	2	3		
619	SS	2.3	15	2.3	1.5		
620	SL	1.25	0.1963	0.6	0.5		
	*						
621	KK	ON04 BASIN					
622	BA	0.009					
623	LG	0.25	0.25	4.03	0.57	40	
624	UI	0	37	79	17	0	
	*						

625	KK	2AIN					
626	KM	INFLOW TO RETENTION BASIN 2A					
627	HC	2					
	*						

628	KK	RET2A					
629	KM	RETENTION BASIN 2A					
630	RS	1 STOR	0				
631	SV	0	0.339	0.678	1.017		
632	SE	0	1	2	3		
633	SS	2.6	50	2.3	1.5		
634	SL	1.25	0.1963	0.6	0.5		
	*						

635	KK	ON05 BASIN					
636	BA	0.004					
637	LG	0.25	0.25	4.03	0.56	40	
638	UI	0	62	15	0		
	*						

639	KK	ON06 BASIN					
640	BA	0.004					
641	LG	0.25	0.25	4.03	0.56	40	
642	UI	0	62	15	0		
	*						

643	KK	2BIN					
644	KM	INFLOW TO RETENTION BASIN 2B					
645	HC	3					
	*						

1

HEC-1 INPUT

PAGE 18

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

646	KK	RET2B					
647	KO	2					
648	KM	RETENTION BASIN 2B					
649	RS	1 STOR	0				
650	SV	0	0.05	0.125	0.229		
651	SE	0	1	2	3		
652	SS	2.4	40	2.3	1.5		
653	SL	0.25	0.393	0.6	0.5		
	*						

654	KK	DIV1					
655	KM	Diversion to Park					
656	DT	DIV1					
657	DI	0	3	40			
658	DQ	0	3	4			
	*						

659	KK	CP072A					
660	KM	Combine CP072 with onsite runoff					
661	HC	2					
	*						
662	ZZ						

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW  
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

5 005  
 V  
 V

```

29 R010-1
   .
35 . 010
   .
45 CP010.....
   V
48 R012-1
   .
54 . 012
   .
64 CP012.....
   .
69 .-----> 012OUT
67 D012
   V
72 R030-1
   .
78 . 030
   .
88 CP030A.....
   .
93 . 015
   V
103 . R022-1
   V
109 . . 017
   .
119 . CP022A.....
   .
124 . . 020
   V
134 . . R022-2
   V
140 . . . 022
   .
150 . . CP022B.....
   .
155 . CP022.....
   .
160 . . 024
   V
170 . . R026-1
   V
176 . . . 026
   .
186 . . CP026A.....
   .
191 . CP026.....
   V
196 . R030-2
   V
202 . CP030.....
   V
207 R038-1
   .
213 . 034
   V
223 . R036-1
   V
229 . . 036
   .
239 . CP036.....
   V
242 . R038-2

```







STRTL .15 STARTING LOSS  
 DTH .39 MOISTURE DEFICIT  
 PSIF 5.80 WETTING FRONT SUCTION  
 XKSAT .17 HYDRAULIC CONDUCTIVITY  
 RTIMP 3.00 PERCENT IMPERVIOUS AREA

100 UC CLARK UNITGRAPH  
 TC .52 TIME OF CONCENTRATION  
 R .33 STORAGE COEFFICIENT

101 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 .0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0  
 100.0

\*\*\*

UNIT HYDROGRAPH PARAMETERS

CLARK TC= .52 HR, R= .33 HR  
 SNYDER TP= .44 HR, CP= .81

UNIT HYDROGRAPH

63 END-OF-PERIOD ORDINATES

22.	62.	90.	117.	152.	191.	253.	353.	563.	888.
1249.	1482.	1540.	1523.	1454.	1370.	1262.	1142.	1033.	934.
845.	765.	692.	626.	566.	512.	463.	419.	379.	343.
310.	281.	254.	230.	208.	188.	170.	154.	139.	126.
114.	103.	93.	84.	76.	69.	62.	57.	51.	46.
42.	38.	34.	31.	28.	25.	23.	21.	19.	17.
15.	14.	13.							

\*\*\* \*\*\* \*\*\* \*\*\* \*\*\*

HYDROGRAPH AT STATION 015

TOTAL RAINFALL = 4.27, TOTAL LOSS = 2.72, TOTAL EXCESS = 1.55

PEAK FLOW	TIME	6-HR	24-HR	72-HR	24.97-HR
(CFS)	(HR)	(CFS)	(INCHES)	(AC-FT)	
1766.	12.33	205.	1.500	101.	
		52.	1.537	104.	
		50.	1.538	104.	
		50.	1.538	104.	

CUMULATIVE AREA = 1.27 SQ MI

\*\*\* \*\*

\*\*\*\*\*  
 \* R022-1 \*  
 \* \*  
 \*\*\*\*\*

Route the runoff hydrograph from 015 to CP022A.

HYDROGRAPH ROUTING DATA

105 RS STORAGE ROUTING  
 NSTPS 6 NUMBER OF SUBREACHES  
 ITYP FLOW TYPE OF INITIAL CONDITION  
 RSVRIC -1.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

106 RC NORMAL DEPTH CHANNEL  
 ANL .045 LEFT OVERBANK N-VALUE  
 ANCH .035 MAIN CHANNEL N-VALUE  
 ANR .045 RIGHT OVERBANK N-VALUE  
 RLNTH 4466. REACH LENGTH  
 SEL .0180 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVERBANK ---	+	----- MAIN CHANNEL -----	+	--- RIGHT OVERBANK ---
108 RY ELEVATION	2740.00	2734.00	2734.00	2728.00	2726.00
107 RX DISTANCE	9714.00	9798.00	9905.00	9943.00	10002.00
					10037.00
					10299.00
					10412.00

\*\*\*

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	1.31	5.23	11.98	25.08	45.83	73.14	102.04	131.93	162.79
OUTFLOW	.00	37.42	237.63	735.05	1761.95	3478.22	6207.14	9915.32	14419.45	19678.21
ELEVATION	2726.00	2726.74	2727.47	2728.21	2728.95	2729.68	2730.42	2731.16	2731.89	2732.63
STORAGE	194.64	228.62	270.78	314.36	359.34	405.73	453.53	502.74	553.35	605.38
OUTFLOW	25666.15	32426.68	40529.64	49673.28	59794.69	70868.67	82882.85	95831.55	109713.20	124529.00
ELEVATION	2733.37	2734.10	2734.84	2735.58	2736.32	2737.05	2737.79	2738.53	2739.26	2740.00

\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 6207. TO 124529.













TOTAL RAINFALL = 4.27, TOTAL LOSS = 2.46, TOTAL EXCESS = 1.81

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
340.	12.40	70.	18.	18.	18.
		(INCHES) 1.699	1.798	1.799	1.799
		(AC-FT) 35.	37.	37.	37.

CUMULATIVE AREA = .38 SQ MI

\*\*\*\*\*

150 KK  
\*\*\*\*\*  
\* CP022B \*  
\*\*\*\*\*

Combine routed hydrograph from 020 with runoff hydrograph from 022.  
This combination was made for use in the hydraulic modeling of a  
tributary to Rawhide Wash.

154 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\* \*\*

HYDROGRAPH AT STATION CP022B

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
1633.	12.40	209.	56.	54.	54.
		(INCHES) 1.698	1.829	1.829	1.829
		(AC-FT) 103.	111.	111.	111.

CUMULATIVE AREA = 1.14 SQ MI

\*\*\*\*\*

155 KK  
\*\*\*\*\*  
\* CP022 \*  
\*\*\*\*\*

Combine hydrographs from CP022A and CP022B.  
This combination was made for use in the hydraulic modeling of a  
tributary to Rawhide Wash.

159 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\* \*\*

HYDROGRAPH AT STATION CP022

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
3445.	12.43	449.	118.	113.	113.
		(INCHES) 1.591	1.671	1.672	1.672
		(AC-FT) 223.	234.	234.	234.

CUMULATIVE AREA = 2.63 SQ MI

\*\*\*\*\*

160 KK  
\*\*\*\*\*  
\* 024 \*  
\*\*\*\*\*





167 UC CLARK UNITGRAPH  
 TC .24 TIME OF CONCENTRATION  
 R .10 STORAGE COEFFICIENT

168 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 .0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0  
 100.0

\*\*\*

UNIT HYDROGRAPH PARAMETERS  
 CLARK TC= .24 HR, R= .10 HR  
 SNYDER TP= .20 HR, CP= 1.03

UNIT HYDROGRAPH  
 20 END-OF-PERIOD ORDINATES  
 110. 289. 476. 1049. 2443. 3469. 3237. 2525. 1820. 1281.  
 902. 635. 447. 315. 222. 156. 110. 77. 55. 38.

\*\*\* \*\*

HYDROGRAPH AT STATION 024

TOTAL RAINFALL = 4.27, TOTAL LOSS = 2.41, TOTAL EXCESS = 1.85

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW				
(CFS)	(HR)	6-HR	24-HR	72-HR	24.97-HR	
+	2481.	186.	51.	49.	49.	
	12.10	(INCHES)	1.698	1.846	1.846	1.846
		(AC-FT)	92.	100.	100.	100.

CUMULATIVE AREA = 1.02 SQ MI

\*\*\*\*\*

170 KK  
 \*\*\*\*\*  
 \* R026-1 \*  
 \*\*\*\*\*

Route the runoff hydrograph from 024 to CP026A.

HYDROGRAPH ROUTING DATA

172 RS STORAGE ROUTING  
 NSTPS 6 NUMBER OF SUBREACHES  
 ITYP FLOW TYPE OF INITIAL CONDITION  
 RSVRIC -1.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

173 RC NORMAL DEPTH CHANNEL  
 ANL .045 LEFT OVERBANK N-VALUE  
 ANCH .035 MAIN CHANNEL N-VALUE  
 ANR .045 RIGHT OVERBANK N-VALUE  
 RLNTH 6097. REACH LENGTH  
 SEL .0160 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVERBANK ---	+	----- MAIN CHANNEL -----	+	--- RIGHT OVERBANK ---
175 RY ELEVATION	2700.00	2692.00	2688.00	2688.00	2686.00
174 RY DISTANCE	9712.00	9782.00	9851.00	9931.00	10050.00

\*\*\*

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	11.15	23.98	40.88	65.78	92.95	122.39	154.10	188.08	224.09
OUTFLOW	.00	334.93	1100.22	1820.00	3853.22	6532.27	9844.59	13788.80	18369.50	23637.11
ELEVATION	2686.00	2686.74	2687.47	2688.21	2688.95	2689.68	2690.42	2691.16	2691.89	2692.63
STORAGE	261.72	300.98	341.86	384.37	428.51	474.45	522.21	571.79	623.20	676.43
OUTFLOW	29545.92	36086.96	43261.93	51074.35	59496.30	68532.05	78237.41	88621.69	99694.59	111466.10
ELEVATION	2693.37	2694.10	2694.84	2695.58	2696.32	2697.05	2697.79	2698.53	2699.26	2700.00

\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 9845. TO 111466.  
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.  
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

\*\*\* \*\*

HYDROGRAPH AT STATION R026-1

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	6-HR	24-HR	72-HR	24.97-HR
+		(CFS)			





PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.97-HR
2481.	12.27	243.	67.	64.	64.	
		(INCHES)	1.752	1.931	1.932	1.932
		(AC-FT)	120.	133.	133.	133.

CUMULATIVE AREA = 1.29 SQ MI

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191 KK  
\*\*\*\*\*  
\* CP026 \*  
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Combine hydrographs from CP022 and CP026A.  
This combination was made for use in the hydraulic modeling of a  
tributary to Rawhide Wash.

195 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\* \*\*

HYDROGRAPH AT STATION CP026

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.97-HR
5412.	12.40	692.	185.	178.	178.	
		(INCHES)	1.644	1.757	1.757	1.757
		(AC-FT)	343.	367.	367.	367.

CUMULATIVE AREA = 3.91 SQ MI

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196 KK  
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\* R030-2 \*  
\*\*\*\*\*

Route the combined hydrograph from CP026 to CP030

HYDROGRAPH ROUTING DATA

198 RS STORAGE ROUTING  
NSTPS 1 NUMBER OF SUBREACHES  
ITYP FLOW TYPE OF INITIAL CONDITION  
RSVRIC -1.00 INITIAL CONDITION  
X .00 WORKING R AND D COEFFICIENT

199 RC NORMAL DEPTH CHANNEL  
ANL .045 LEFT OVERBANK N-VALUE  
ANCH .035 MAIN CHANNEL N-VALUE  
ANR .045 RIGHT OVERBANK N-VALUE  
RLNTH 1354. REACH LENGTH  
SEL .0160 ENERGY SLOPE  
ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVERBANK ---	+	----- MAIN CHANNEL -----	+	--- RIGHT OVERBANK ---		
201 RY ELEVATION	2604.00	2598.00	2598.00	2596.00	2594.00	2594.00	2604.00
200 RX DISTANCE	214.00	314.00	479.00	518.00	654.00	771.00	852.00

\*\*\*

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	1.61	3.81	6.58	10.37	16.29	22.45	28.86	36.59	46.15
OUTFLOW	.00	135.45	490.76	1084.49	1751.86	3365.03	5493.72	8096.17	11260.71	15276.39
ELEVATION	2594.00	2594.53	2595.05	2595.58	2596.11	2596.63	2597.16	2597.68	2598.21	2598.74
STORAGE	55.94	65.95	76.17	86.61	97.27	108.15	119.25	130.57	142.10	153.85
OUTFLOW	19974.87	25303.44	31232.77	37743.36	44821.39	52456.68	60641.67	69370.69	78639.52	88446.43
ELEVATION	2599.26	2599.79	2600.32	2600.84	2601.37	2601.90	2602.42	2602.95	2603.47	2604.00

\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 52457. TO 88446.

THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.  
THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

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***          ***          ***          ***          ***
HYDROGRAPH AT STATION  R030-2

PEAK FLOW      TIME          MAXIMUM AVERAGE FLOW
+ (CFS)        (HR)          6-HR      24-HR      72-HR      24.97-HR
+ 5366.        12.43          (CFS)
                (INCHES)    692.      185.      178.      178.
                (AC-FT)    1.644    1.757    1.757    1.757
                343.      367.      367.      367.

PEAK STORAGE   TIME          MAXIMUM AVERAGE STORAGE
+ (AC-FT)      (HR)          6-HR      24-HR      72-HR      24.97-HR
+ 22.          12.43          4.        1.        1.        1.

PEAK STAGE     TIME          MAXIMUM AVERAGE STAGE
+ (FEET)       (HR)          6-HR      24-HR      72-HR      24.97-HR
+ 2597.13      12.43          2594.78   2594.23   2594.22   2594.22

CUMULATIVE AREA = 3.91 SQ MI

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* 202 KK CP030 *
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Combine hydrograph from CP026 and CP030A.  
This combination was made for use in the hydraulic modeling of a  
tributary to Rawhide Wash.

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206 HC          HYDROGRAPH COMBINATION
                ICOMP          2  NUMBER OF HYDROGRAPHS TO COMBINE

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***          ***          ***          ***          ***
HYDROGRAPH AT STATION  CP030

PEAK FLOW      TIME          MAXIMUM AVERAGE FLOW
+ (CFS)        (HR)          6-HR      24-HR      72-HR      24.97-HR
+ 5666.        12.43          (CFS)
                (INCHES)    861.      229.      220.      220.
                (AC-FT)    1.504    1.602    1.602    1.602
                427.      454.      455.      455.

CUMULATIVE AREA = 5.32 SQ MI

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* 207 KK R038-1 *
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Route combined hydrograph from CP030 to CP038

HYDROGRAPH ROUTING DATA

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209 RS          STORAGE ROUTING
                NSTPS          2  NUMBER OF SUBREACHES
                ITYP          FLOW TYPE OF INITIAL CONDITION
                RSVRIC        -1.00 INITIAL CONDITION
                X              .00 WORKING R AND D COEFFICIENT

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210 RC          NORMAL DEPTH CHANNEL
                ANL          .045 LEFT OVERBANK N-VALUE
                ANCH         .035 MAIN CHANNEL N-VALUE
                ANR          .045 RIGHT OVERBANK N-VALUE
                RLNTH        1605. REACH LENGTH
                SEL          .0120 ENERGY SLOPE
                ELMAX         .0  MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

```

CROSS-SECTION DATA

```

212 RY          --- LEFT OVERBANK --- + --- MAIN CHANNEL --- + --- RIGHT OVERBANK ---
ELEVATION      2582.00  2576.00  2574.00  2572.00  2572.00  2580.00  2580.00  2582.00

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223 KK  
 R036-1

Route runoff hydrograph from 034 to CP036

HYDROGRAPH ROUTING DATA

225 RS STORAGE ROUTING  
 NSTPS 8 NUMBER OF SUBREACHES  
 ITYP FLOW TYPE OF INITIAL CONDITION  
 RSVRIC -1.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

226 RC NORMAL DEPTH CHANNEL  
 ANL .045 LEFT OVERBANK N-VALUE  
 ANCH .035 MAIN CHANNEL N-VALUE  
 ANR .045 RIGHT OVERBANK N-VALUE  
 RLNTH 5295. REACH LENGTH  
 SEL .0190 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

--- LEFT OVERBANK --- + --- MAIN CHANNEL --- + --- RIGHT OVERBANK ---  
 228 RY ELEVATION 2652.00 2646.00 2646.00 2642.00 2642.00 2644.00 2644.00 2652.00  
 227 RX DISTANCE 9725.00 9842.00 9891.00 9970.00 10030.00 10032.00 10143.00 10241.00

\*\*\*

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	4.19	9.08	14.66	22.38	36.73	52.16	68.66	87.50	109.29
OUTFLOW	.00	124.69	410.56	838.99	1423.58	2375.94	3679.40	5308.21	7367.13	9992.12
ELEVATION	2642.00	2642.53	2643.05	2643.58	2644.11	2644.63	2645.16	2645.68	2646.21	2646.74
STORAGE	132.15	156.08	181.08	207.15	234.28	262.49	291.76	322.11	353.52	386.00
OUTFLOW	13022.68	16447.67	20263.81	24471.14	29071.57	34068.16	39464.79	45265.89	51476.34	58102.60
ELEVATION	2647.26	2647.79	2648.32	2648.84	2649.37	2649.90	2650.42	2650.95	2651.47	2652.00

\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 3679. TO 58103.  
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.  
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE. (USE A LONGER REACH.)

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HYDROGRAPH AT STATION R036-1

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.97-HR
+	(CFS)	(HR)				
+	415.	12.47				
		(CFS)				
		(INCHES)	56.	16.	15.	15.
		(AC-FT)	1.757	1.953	1.954	1.954
			28.	31.	31.	31.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
			6-HR	24-HR	72-HR	24.97-HR
+	(AC-FT)	(HR)				
+	1.	12.47				
			0.	0.	0.	0.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
			6-HR	24-HR	72-HR	24.97-HR
+	(FEET)	(HR)				
+	2643.06	12.47				
			2642.19	2642.04	2642.04	2642.04

CUMULATIVE AREA = .30 SQ MI

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229 KK  
 036

SUB-BASIN 036  
 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 L = 1.37 Kb = .050 Adj. Slope = 278.2

SUBBASIN RUNOFF DATA



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 UNIT HYDROGRAPH PARAMETERS  
 CLARK TC= .28 HR, R= .25 HR  
 SNYDER TP= .24 HR, CP= .65

UNIT HYDROGRAPH  
 45 END-OF-PERIOD ORDINATES

10.	28.	46.	78.	172.	333.	437.	442.	408.	361.
316.	276.	242.	211.	185.	162.	142.	124.	108.	95.
83.	73.	64.	56.	49.	43.	37.	33.	29.	25.
22.	19.	17.	15.	13.	11.	10.	9.	8.	7.
6.	5.	4.	4.	3.					

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HYDROGRAPH AT STATION      036

TOTAL RAINFALL =    4.27, TOTAL LOSS =    2.13, TOTAL EXCESS =    2.14

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
+	463.	51.	14.	14.	14.
	12.17	1.893	2.127	2.128	2.128
		(INCHES)			
		(AC-FT)	25.	28.	28.

CUMULATIVE AREA =    .25 SQ MI

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 239 KK            CP036            \*  
 \*                    \*  
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Combine routed hydrograph from 034 with runoff hydrograph from 036

241 HC            HYDROGRAPH COMBINATION  
 ICOMP            2    NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION      CP036

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
+	648.	107.	30.	29.	29.
	12.40	1.818	2.032	2.033	2.033
		(INCHES)			
		(AC-FT)	53.	59.	59.

CUMULATIVE AREA =    .55 SQ MI

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 \*                    \*  
 242 KK            R038-2            \*  
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 \*                    \*  
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Route combined hydrograph from CP036 to CP038A.

HYDROGRAPH ROUTING DATA

244 RS            STORAGE ROUTING  
 NSTPS            1    NUMBER OF SUBREACHES  
 ITYP            FLOW    TYPE OF INITIAL CONDITION  
 RSVRIC        -1.00    INITIAL CONDITION  
 X                .00    WORKING R AND D COEFFICIENT

245 RC            NORMAL DEPTH CHANNEL  
 ANL            .045    LEFT OVERBANK N-VALUE  
 ANCH          .035    MAIN CHANNEL N-VALUE  
 ANR            .045    RIGHT OVERBANK N-VALUE  
 RLNTH        483.    REACH LENGTH  
 SEL            .0250    ENERGY SLOPE  
 ELMAX        .0      MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA









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Combine hydrograph from CP038a with runoff hydrographs from 032.  
This combination was made for use in the hydraulic modeling of a  
tributary to Rawhide Wash.

277 HC HYDROGRAPH COMBINATION  
ICOMP 3 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION CP038  
PEAK FLOW TIME MAXIMUM AVERAGE FLOW  
+ (CFS) (HR) 6-HR 24-HR 72-HR 24.97-HR  
+ 6445. 12.43 (CFS) 1007. 270. 260. 260.  
(INCHES) 1.539 1.650 1.651 1.651  
(AC-FT) 499. 536. 536. 536.  
CUMULATIVE AREA = 6.08 SQ MI

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278 KK  
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\* R040-1 \*  
\*\*\*\*\*

Route combined hydrograph from CP038 to CP040

HYDROGRAPH ROUTING DATA

280 RS STORAGE ROUTING  
NSTPS 4 NUMBER OF SUBREACHES  
ITYP FLOW TYPE OF INITIAL CONDITION  
RSVRIC -1.00 INITIAL CONDITION  
X .00 WORKING R AND D COEFFICIENT  
281 RC NORMAL DEPTH CHANNEL  
ANL .045 LEFT OVERBANK N-VALUE  
ANCH .035 MAIN CHANNEL N-VALUE  
ANR .045 RIGHT OVERBANK N-VALUE  
RLNTH 3775. REACH LENGTH  
SEL .0190 ENERGY SLOPE  
ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

--- LEFT OVERBANK --- + --- MAIN CHANNEL --- + --- RIGHT OVERBANK ---  
283 RY ELEVATION 2540.00 2536.00 2530.00 2530.00 2528.00 2528.00 2534.00 2540.00  
282 RX DISTANCE 47.00 75.00 224.00 433.00 448.00 523.00 609.00 678.00

\*\*\*

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

	STORAGE	4.48	9.72	15.71	32.20	51.94	73.04	95.48	119.29	144.44
OUTFLOW	.00	212.85	702.51	1434.46	2238.57	4782.60	8128.00	12242.12	17109.96	22725.81
ELEVATION	2528.00	2528.63	2529.26	2529.89	2530.53	2531.16	2531.79	2532.42	2533.05	2533.68
STORAGE	170.94	198.71	227.73	257.97	288.99	320.65	352.95	385.89	419.47	453.69
OUTFLOW	29105.71	36248.01	44138.50	52852.61	62452.03	72781.60	83832.40	95597.65	108072.30	121252.80
ELEVATION	2534.32	2534.95	2535.58	2536.21	2536.84	2537.47	2538.11	2538.74	2539.37	2540.00

\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 8128. TO 121253.  
THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.  
THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

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HYDROGRAPH AT STATION R040-1

PEAK FLOW TIME MAXIMUM AVERAGE FLOW  
+ (CFS) (HR) 6-HR 24-HR 72-HR 24.97-HR  
+ 6379. 12.53 (CFS) 1007. 270. 260. 260.  
(INCHES) 1.539 1.650 1.650 1.650  
(AC-FT) 499. 535. 535. 535.  
PEAK STORAGE TIME MAXIMUM AVERAGE STORAGE  
+ (AC-FT) (HR) 6-HR 24-HR 72-HR 24.97-HR  
+ 16. 12.53 3. 1. 1. 1.  
PEAK STAGE TIME MAXIMUM AVERAGE STAGE





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***          ***          ***          ***          ***
HYDROGRAPH AT STATION  CP042
PEAK FLOW      TIME          MAXIMUM AVERAGE FLOW
+ (CFS)        (HR)          6-HR      24-HR      72-HR      24.97-HR
+ 6755.        12.53          (CFS)
                              1125.      301.      289.      289.
                              (INCHES)  1.537    1.645    1.645    1.645
                              (AC-FT)   558.     597.     597.     597.
CUMULATIVE AREA = 6.80 SQ MI

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* R043-1 *
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Route combined hydrograph from CP040 to CP042

HYDROGRAPH ROUTING DATA

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328 RS      STORAGE ROUTING
            NSTPS          7  NUMBER OF SUBREACHES
            ITYP           FLOW TYPE OF INITIAL CONDITION
            RSVRIC        -1.00 INITIAL CONDITION
            X              .00 WORKING R AND D COEFFICIENT

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329 RC      NORMAL DEPTH CHANNEL
            ANL           .045 LEFT OVERBANK N-VALUE
            ANCH          .035 MAIN CHANNEL N-VALUE
            ANR           .045 RIGHT OVERBANK N-VALUE
            RLNTH         7504. REACH LENGTH
            SEL           .0170 ENERGY SLOPE
            ELMAX          .0  MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

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CROSS-SECTION DATA
--- LEFT OVERBANK --- + ----- MAIN CHANNEL ----- + --- RIGHT OVERBANK ---
331 RY      ELEVATION  2486.00  2482.00  2476.00  2482.00  2476.00  2476.00  2482.00  2486.00
330 RX      DISTANCE  263.00  349.00  381.00  450.00  522.00  756.00  847.00  973.00

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

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STORAGE      .00  22.27  46.64  73.11  101.67  132.34  165.11  199.98  236.95  276.02
OUTFLOW      .00  454.75  1473.44  2956.16  4874.13  7216.91  9982.34  13172.81  16793.50  20851.32
ELEVATION    2476.00  2476.53  2477.05  2477.58  2478.11  2478.63  2479.16  2479.68  2480.21  2480.74

STORAGE      317.19  360.46  405.90  453.84  504.32  557.32  612.85  670.91  731.50  794.62
OUTFLOW      25354.43  30311.79  35963.23  42334.57  49282.25  56802.84  64899.93  73580.38  82852.70  92727.70
ELEVATION    2481.26  2481.79  2482.32  2482.84  2483.37  2483.90  2484.42  2484.95  2485.47  2486.00

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\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 16793. TO 92728.  
THE Routed HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.  
THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

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***          ***          ***          ***          ***
HYDROGRAPH AT STATION  R043-1
PEAK FLOW      TIME          MAXIMUM AVERAGE FLOW
+ (CFS)        (HR)          6-HR      24-HR      72-HR      24.97-HR
+ 6555.        12.73          (CFS)
                              1124.      300.      289.      289.
                              (INCHES)  1.537    1.643    1.643    1.643
                              (AC-FT)   557.     596.     596.     596.

PEAK STORAGE   TIME          MAXIMUM AVERAGE STORAGE
+ (AC-FT)      (HR)          6-HR      24-HR      72-HR      24.97-HR
+ 18.          12.73          4.        1.        1.        1.

PEAK STAGE     TIME          MAXIMUM AVERAGE STAGE
+ (FEET)       (HR)          6-HR      24-HR      72-HR      24.97-HR
+ 2478.48      12.73          2476.67  2476.18  2476.18  2476.18

CUMULATIVE AREA = 6.80 SQ MI

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.00 .00 .00 .00 .00 .00 .00 .00 .00 .00

338 LG GREEN AND AMPT LOSS RATE  
 STRTL .15 STARTING LOSS  
 DTH .35 MOISTURE DEFICIT  
 PSIF 4.65 WETTING FRONT SUCTION  
 XKSAT .29 HYDRAULIC CONDUCTIVITY  
 RTIMP 3.00 PERCENT IMPERVIOUS AREA

339 UC CLARK UNITGRAPH  
 TC .54 TIME OF CONCENTRATION  
 R .64 STORAGE COEFFICIENT

340 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 .0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0  
 100.0

\*\*\*

UNIT HYDROGRAPH PARAMETERS  
 CLARK TC= .54 HR, R= .64 HR  
 SNYDER TP= .48 HR, CP= .54

UNIT HYDROGRAPH  
 112 END-OF-PERIOD ORDINATES

3.	7.	11.	15.	19.	24.	32.	43.	67.	105.
150.	188.	208.	215.	214.	211.	204.	194.	184.	175.
166.	158.	150.	142.	135.	128.	121.	115.	109.	104.
99.	94.	89.	84.	80.	76.	72.	68.	65.	62.
59.	56.	53.	50.	47.	45.	43.	41.	39.	37.
35.	33.	31.	30.	28.	27.	25.	24.	23.	22.
21.	20.	19.	18.	17.	16.	15.	14.	14.	13.
12.	12.	11.	10.	10.	9.	9.	8.	8.	8.
7.	7.	7.	6.	6.	6.	5.	5.	5.	5.
4.	4.	4.	4.	3.	3.	3.	3.	3.	3.
3.	2.	2.	2.	2.	2.	2.	2.	2.	2.
2.	1.								

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HYDROGRAPH AT STATION 043

TOTAL RAINFALL = 4.27, TOTAL LOSS = 2.86, TOTAL EXCESS = 1.41

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
253.	12.37	42.	11.	10.	10.
		1.362 (INCHES) (AC-FT)	1.399 21.	1.399 21.	1.399 21.

CUMULATIVE AREA = .29 SQ MI

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 \* \*  
 342 KK \* CP043 \*  
 \* \*  
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Combine routed hydrograph from CP042 with runoff hydrograph from 043

344 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION CP043

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
6720.	12.70	1166.	311.	299.	299.
		1.530 (INCHES) (AC-FT)	1.633 617.	1.633 617.	1.633 617.

CUMULATIVE AREA = 7.09 SQ MI

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345 KK \*  
 \* R046-1 \*  
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Route combined hydrograph from CP043 to CP046

HYDROGRAPH ROUTING DATA

347 RS STORAGE ROUTING  
 NSTPS 4 NUMBER OF SUBREACHES  
 ITYP FLOW TYPE OF INITIAL CONDITION  
 RSVRIC -1.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

348 RC NORMAL DEPTH CHANNEL  
 ANL .045 LEFT OVBANK N-VALUE  
 ANCH .035 MAIN CHANNEL N-VALUE  
 ANR .045 RIGHT OVBANK N-VALUE  
 RLNTH 2845. REACH LENGTH  
 SEL .0150 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

--- LEFT OVBANK --- + --- MAIN CHANNEL --- + --- RIGHT OVBANK ---  
 350 RY ELEVATION 2300.00 2288.00 2288.00 2286.00 2286.00 2288.00 2288.00 2302.00  
 349 RX DISTANCE 9425.00 9601.00 9923.00 9945.00 10019.00 10043.00 10267.00 10435.00

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	4.60	10.27	35.80	73.81	113.06	153.55	195.27	238.23	282.42
OUTFLOW	.00	304.39	1022.54	2988.66	7644.51	14316.07	22761.18	32851.81	44508.63	57679.07
ELEVATION	2286.00	2286.84	2287.68	2288.53	2289.37	2290.21	2291.05	2291.89	2292.74	2293.58
STORAGE	327.84	374.50	422.40	471.53	521.89	573.49	626.33	680.35	735.06	790.33
OUTFLOW	72327.22	88428.42	105966.00	124929.20	145311.60	167110.60	190325.90	215531.00	243137.20	272184.30
ELEVATION	2294.42	2295.26	2296.10	2296.95	2297.79	2298.63	2299.47	2300.31	2301.16	2302.00

\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 14316. TO 272184.  
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.  
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

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HYDROGRAPH AT STATION R046-1

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	6-HR	24-HR	72-HR	24.97-HR
+	6563.	12.83	1166.	311.	299.
		(INCHES)	1.530	1.632	1.633
		(AC-FT)	578.	617.	617.
PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)	6-HR	24-HR	72-HR	24.97-HR
+	16.	12.83	3.	1.	1.
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE			
(FEET)	(HR)	6-HR	24-HR	72-HR	24.97-HR
+	2289.18	12.83	2287.19	2286.35	2286.34

CUMULATIVE AREA = 7.09 SQ MI

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351-KK \*  
 \* 044 \*  
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SUB-BASIN 044  
 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 L = 2.99 Kb = .039 Adj. Slope = 240.0

SUBBASIN RUNOFF DATA

356 BA SUBBASIN CHARACTERISTICS  
 TAREA 1.40 SUBBASIN AREA

PRECIPITATION DATA

14 PB STORM 4.27 BASIN TOTAL PRECIPITATION  
 14 PI INCREMENTAL PRECIPITATION PATTERN









.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

373 LG GREEN AND AMPT LOSS RATE  
 STRTL .15 STARTING LOSS  
 DTH .36 MOISTURE DEFICIT  
 PSIF 5.10 WETTING FRONT SUCTION  
 XKSAT .23 HYDRAULIC CONDUCTIVITY  
 RTIMP 3.00 PERCENT IMPERVIOUS AREA

374 UC CLARK UNITGRAPH  
 TC .29 TIME OF CONCENTRATION  
 R .24 STORAGE COEFFICIENT

375 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 .0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0  
 100.0

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UNIT HYDROGRAPH PARAMETERS  
 CLARK TC= .29 HR, R= .24 HR  
 SNYDER TP= .24 HR, CP= .69

UNIT HYDROGRAPH  
 43 END-OF-PERIOD ORDINATES

5.	13.	20.	33.	70.	141.	196.	203.	190.	169.
146.	127.	110.	96.	83.	72.	63.	54.	47.	41.
36.	31.	27.	23.	20.	18.	15.	13.	11.	10.
9.	8.	7.	6.	5.	4.	4.	3.	3.	2.
2.	2.	2.							

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HYDROGRAPH AT STATION 046

TOTAL RAINFALL = 4.27, TOTAL LOSS = 2.80, TOTAL EXCESS = 1.47

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
192.	12.17	(CFS)	17.	4.	4.	4.
		(INCHES)	1.429	1.466	1.466	1.466
		(AC-FT)	8.	9.	9.	9.

CUMULATIVE AREA = .11 SQ MI

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377 KK \*\*\*\*\*  
 \* CP046 \*  
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Combine routed hydrographs from CP042 and 044 with runoff hydrograph from 046

380 HC HYDROGRAPH COMBINATION  
 ICOMP 3 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION CP046

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
7150.	12.80	(CFS)	1412.	375.	361.	361.
		(INCHES)	1.527	1.624	1.624	1.624
		(AC-FT)	700.	745.	745.	745.

CUMULATIVE AREA = 8.60 SQ MI

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 \* R051-1 \*  
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Route combined hydrograph from CP046 to CP051

HYDROGRAPH ROUTING DATA

383 RS STORAGE ROUTING  
 NSTPS 2 NUMBER OF SUBREACHES  
 ITYP FLOW TYPE OF INITIAL CONDITION  
 RSVRIC -1.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

384 RC NORMAL DEPTH CHANNEL  
 ANL .045 LEFT OVERBANK N-VALUE  
 ANCH .035 MAIN CHANNEL N-VALUE  
 ANR .045 RIGHT OVERBANK N-VALUE  
 RLNTH 1557. REACH LENGTH  
 SEL .0170 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

--- LEFT OVERBANK --- + ----- MAIN CHANNEL ----- + --- RIGHT OVERBANK ---  
 386 RY ELEVATION 2275.00 2261.00 2262.00 2260.00 2260.00 2263.00 2264.00 2275.00  
 385 RX DISTANCE 9822.00 9952.00 9976.00 9977.00 10023.00 10046.00 10356.00 10509.00

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	1.39	3.16	5.77	8.97	17.13	29.52	42.45	55.90	69.86
OUTFLOW	.00	175.45	580.60	1264.40	2280.04	4032.57	7248.55	11658.77	17126.96	23592.90
ELEVATION	2260.00	2260.79	2261.58	2262.37	2263.16	2263.95	2264.74	2265.53	2266.32	2267.11
STORAGE	84.34	99.34	114.85	130.88	147.43	164.49	182.07	200.17	218.78	237.91
OUTFLOW	31022.00	39393.51	48695.23	58920.66	70067.29	82135.57	95128.18	109049.50	123905.30	139706.00
ELEVATION	2267.90	2268.69	2269.47	2270.26	2271.05	2271.84	2272.63	2273.42	2274.21	2275.00

\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 11659. TO 139706.  
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.  
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

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HYDROGRAPH AT STATION R051-1

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
			6-HR	24-HR	72-HR
+	(CFS)	(HR)			
+	7079.	12.87			
		(CFS)			
		(INCHES)	1412.	375.	361.
		(AC-FT)	1.526	1.624	1.624
			700.	744.	745.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
			6-HR	24-HR	72-HR
+	(AC-FT)	(HR)			
+	14.	12.87			
			3.	1.	1.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
			6-HR	24-HR	72-HR
+	(FEET)	(HR)			
+	2264.70	12.87			
			2261.89	2260.57	2260.55

CUMULATIVE AREA = 8.60 SQ MI

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 \* 048 \*  
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SUB-BASIN 048  
 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 L = .43 Kb = .059 Adj. Slope = 112.0

SUBBASIN RUNOFF DATA

392 BA SUBBASIN CHARACTERISTICS  
 TAREA .06 SUBBASIN AREA

PRECIPITATION DATA

14 PB STORM 4.27 BASIN TOTAL PRECIPITATION







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HYDROGRAPH AT STATION    CP051

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	24.97-HR
+ 7153.	12.87	(CFS)	1447.	384.	370.	370.
		(INCHES)	1.526	1.621	1.622	1.622
		(AC-FT)	717.	763.	763.	763.

CUMULATIVE AREA =    8.82 SQ MI

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 \* R053-1 \*  
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Route combined hydrograph from CP051 to CP053

HYDROGRAPH ROUTING DATA

412 RS            STORAGE ROUTING

NSTPS	1	NUMBER OF SUBREACHES
ITYP		FLOW TYPE OF INITIAL CONDITION
RSVRC	-1.00	INITIAL CONDITION
X	.00	WORKING R AND D COEFFICIENT

413 RC            NORMAL DEPTH CHANNEL

ANL	.045	LEFT OVERBANK N-VALUE
ANCH	.035	MAIN CHANNEL N-VALUE
ANR	.045	RIGHT OVERBANK N-VALUE
RLNTH	1681.	REACH LENGTH
SEL	.0300	ENERGY SLOPE
ELMAX	.0	MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	---	LEFT OVERBANK	---	+	-----	MAIN CHANNEL	-----	+	---	RIGHT OVERBANK	---
415 RY	ELEVATION	2249.00	2245.00	2245.00	2238.00	2238.00	2239.00	2239.00	2242.00		
414 RX	DISTANCE	9910.00	9935.00	9963.00	9987.00	10041.00	10046.00	10075.00	10085.00		

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	1.26	2.81	4.92	7.11	9.40	11.77	14.23	16.75	19.32
OUTFLOW	.00	162.41	535.30	1165.96	2016.62	3065.61	4301.85	5722.16	7355.02	9162.64
ELEVATION	2238.00	2238.58	2239.16	2239.74	2240.32	2240.89	2241.47	2242.05	2242.63	2243.21
STORAGE	21.93	24.58	27.28	30.61	34.07	37.62	41.24	44.95	48.74	52.61
OUTFLOW	11140.55	13285.33	15594.36	18301.48	21279.04	24488.79	27923.77	31580.25	35456.09	39550.02
ELEVATION	2243.79	2244.37	2244.95	2245.53	2246.10	2246.68	2247.26	2247.84	2248.42	2249.00

\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 9163. TO 39550.  
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.  
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

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HYDROGRAPH AT STATION    R053-1

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	24.97-HR
+ 7142.	12.87	(CFS)	1447.	384.	370.	370.
		(INCHES)	1.525	1.621	1.621	1.621
		(AC-FT)	717.	762.	762.	762.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
+ (AC-FT)	(HR)		6-HR	24-HR	72-HR	24.97-HR
+ 16.	12.87		5.	1.	1.	1.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
+ (FEET)	(HR)		6-HR	24-HR	72-HR	24.97-HR
+ 2242.56	12.87		2239.58	2238.48	2238.46	2238.46

CUMULATIVE AREA =    8.82 SQ MI



.00 .00 .00 .00 .00 .00 .00 .00 .00 .00

422 LG GREEN AND AMPT LOSS RATE  
 STRTL .15 STARTING LOSS  
 DTH .37 MOISTURE DEFICIT  
 PSIF 6.60 WETTING FRONT SUCTION  
 XKSAT .13 HYDRAULIC CONDUCTIVITY  
 RTIMP 3.00 PERCENT IMPERVIOUS AREA

423 UC CLARK UNITGRAPH  
 TC .26 TIME OF CONCENTRATION  
 R .23 STORAGE COEFFICIENT

424 UA ACCUMULATED-AREA VS. TIME, 11 ORDINATES  
 .0 3.0 5.0 8.0 12.0 20.0 43.0 75.0 90.0 96.0  
 100.0

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UNIT HYDROGRAPH PARAMETERS  
 CLARK TC= .26 HR, R= .23 HR  
 SNYDER TP= .23 HR, CP= .69

UNIT HYDROGRAPH  
 41 END-OF-PERIOD ORDINATES

4.	12.	20.	39.	91.	156.	183.	175.	156.	134.
116.	100.	86.	75.	64.	56.	48.	41.	36.	31.
27.	23.	20.	17.	15.	13.	11.	10.	8.	7.
6.	5.	5.	4.	3.	3.	3.	2.	2.	2.
1.									

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HYDROGRAPH AT STATION 053

TOTAL RAINFALL = 4.27, TOTAL LOSS = 2.63, TOTAL EXCESS = 1.64

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
175.	12.13	16.	4.	4.	4.	
		(INCHES) (AC-FT)	1.594 8.	1.631 8.	1.631 8.	1.631 8.

CUMULATIVE AREA = .09 SQ MI

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 \* CP053 \*  
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Combine routed hydrographs from CP051 with runoff hydrograph from 053

428 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION CP053

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
7157.	12.87	1463.	389.	373.	373.	
		(INCHES) (AC-FT)	1.526 725.	1.621 771.	1.621 771.	1.621 771.

CUMULATIVE AREA = 8.91 SQ MI

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 \* R055-1 \*  
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Route combined hydrograph from CP053 to CP055

HYDROGRAPH ROUTING DATA







28.	27.	26.	24.	23.	22.	21.	20.	19.	18.
17.	16.	16.	15.	14.	13.	13.	12.	12.	11.
11.	10.	10.	9.	9.	8.	8.	7.	7.	7.
6.	6.	6.	5.	5.	5.	5.	5.	4.	4.
4.	4.	4.	3.	3.	3.	3.	3.	3.	2.
2.	2.	2.	2.	2.	2.	2.	2.	2.	2.

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HYDROGRAPH AT STATION                    055

TOTAL RAINFALL = 4.27, TOTAL LOSS = 2.66, TOTAL EXCESS = 1.61

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
338.	12.37	61.	16.	15.	15.	
		(INCHES)	1.540	1.601	1.602	1.602
		(AC-FT)	30.	31.	31.	31.

CUMULATIVE AREA = .37 SQ MI

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445 KK                    \*\*\*\*\*  
                             \*                    CP055                    \*  
                             \*                    \*                    \*  
                             \*\*\*\*\*

Combine routed hydrograph from CP053 with runoff hydrograph from 055

447 HC                    HYDROGRAPH COMBINATION  
                             ICCOMP                    2                    NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION                    CP055

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
7319.	12.90	1523.	404.	389.	389.	
		(INCHES)	1.526	1.620	1.620	1.620
		(AC-FT)	755.	802.	802.	802.

CUMULATIVE AREA = 9.28 SQ MI

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448 KK                    \*\*\*\*\*  
                             \*                    R068-1                    \*  
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                             \*\*\*\*\*

Route combined hydrograph from CP055 to CP068

HYDROGRAPH ROUTING DATA

450 RS                    STORAGE ROUTING  
                             NSTPS                    2                    NUMBER OF SUBREACHES  
                             ITYP                    FLOW                    TYPE OF INITIAL CONDITION  
                             RSVRIC                    -1.00                    INITIAL CONDITION  
                             X                    .00                    WORKING R AND D COEFFICIENT

451 RC                    NORMAL DEPTH CHANNEL  
                             ANL                    .045                    LEFT OVERBANK N-VALUE  
                             ANCH                    .035                    MAIN CHANNEL N-VALUE  
                             ANR                    .045                    RIGHT OVERBANK N-VALUE  
                             RLNTH                    1728.                    REACH LENGTH  
                             SEL                    .0170                    ENERGY SLOPE  
                             ELMAX                    .0                    MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	---	LEFT OVERBANK	---	+	-----	MAIN CHANNEL	-----	+	---	RIGHT OVERBANK	---
453 RY	ELEVATION	2208.00	2202.00	2200.00	2197.00	2199.00	2200.00	2199.00	2208.00		
452 RX	DISTANCE	111.00	154.00	304.00	389.00	510.00	725.00	836.00	937.00		

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA





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 \* R064-1 \*  
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Route runoff hydrograph from 058

HYDROGRAPH ROUTING DATA

466 RS STORAGE ROUTING  
 NSTPS 7 NUMBER OF SUBREACHES  
 ITYP FLOW TYPE OF INITIAL CONDITION  
 RSVRIC -1.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

467 RC NORMAL DEPTH CHANNEL  
 ANL .045 LEFT OVERBANK N-VALUE  
 ANCH .035 MAIN CHANNEL N-VALUE  
 ANR .045 RIGHT OVERBANK N-VALUE  
 RLNTH 7694. REACH LENGTH  
 SEL .0230 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

--- LEFT OVERBANK --- + --- MAIN CHANNEL --- + --- RIGHT OVERBANK ---  
 469 RY ELEVATION 2323.00 2321.00 2319.00 2319.00 2320.00 2321.00 2323.00 2325.00  
 468 RX DISTANCE .00 76.00 102.00 111.00 117.00 118.00 168.00 205.00

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.67	1.67	3.01	4.65	6.54	8.68	11.25	14.88	19.62
OUTFLOW	.00	9.78	35.31	78.03	143.67	230.48	338.65	459.52	618.19	829.03
ELEVATION	2319.00	2319.32	2319.63	2319.95	2320.26	2320.58	2320.89	2321.21	2321.53	2321.84
STORAGE	25.48	32.44	40.50	49.64	59.28	69.24	79.53	90.14	101.08	112.35
OUTFLOW	1099.51	1437.19	1849.40	2371.41	3034.28	3778.27	4602.60	5506.89	6491.07	7555.25
ELEVATION	2322.16	2322.47	2322.79	2323.10	2323.42	2323.74	2324.05	2324.37	2324.68	2325.00

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HYDROGRAPH AT STATION R064-1

PEAK FLOW	TIME	6-HR	24-HR	72-HR	24.97-HR
+ (CFS)	(HR)				
+ 2367.	12.53	(CFS)	288.	74.	71.
		(INCHES)	1.525	1.574	1.574
		(AC-FT)	143.	147.	147.
PEAK STORAGE	TIME	6-HR	24-HR	72-HR	24.97-HR
+ (AC-FT)	(HR)				
+ 7.	12.53		1.	0.	0.
PEAK STAGE	TIME	6-HR	24-HR	72-HR	24.97-HR
+ (FEET)	(HR)				
+ 2323.10	12.53		2320.02	2319.34	2319.32

CUMULATIVE AREA = 1.76 SQ MI

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 \* 060 \*  
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SUB-BASIN 060  
 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 L = 3.39 Kb = .043 Adj. Slope = 152.0

SUBBASIN RUNOFF DATA

475 BA SUBBASIN CHARACTERISTICS  
 TAREA .78 SUBBASIN AREA

PRECIPITATION DATA

14 PB STORM 4.27 BASIN TOTAL PRECIPITATION

14 PI INCREMENTAL PRECIPITATION PATTERN  
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00



478.	453.	430.	407.	386.	366.	347.	329.	312.	295.
280.	265.	252.	238.	226.	214.	203.	193.	182.	173.
164.	155.	147.	140.	132.	125.	119.	113.	107.	101.
96.	91.	86.	82.	78.	73.	70.	66.	63.	59.
56.	53.	51.	48.	45.	43.	41.	39.	37.	35.
33.	31.	30.	28.	27.	25.	24.	23.	21.	20.
19.	18.	17.	16.	16.	15.	14.	13.	13.	12.
11.	11.	10.	10.	9.	9.	8.	8.	7.	7.
7.	6.	6.	6.	5.	5.	5.	5.	4.	4.

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HYDROGRAPH AT STATION 060

TOTAL RAINFALL = 4.27, TOTAL LOSS = 2.58, TOTAL EXCESS = 1.69

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
+ 764.	12.40	135.	35.	34.	34.
		(INCHES) (AC-FT)	1.609 67.	1.682 70.	1.683 70.
CUMULATIVE AREA =		.78 SQ MI			

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480 KK                    +  
                          + R062-1 +  
                          +  
                          +

Route runoff hydrograph from 060

HYDROGRAPH ROUTING DATA

482 RS	STORAGE ROUTING		
	NSTPS	1	NUMBER OF SUBREACHES
	ITYP	FLOW	TYPE OF INITIAL CONDITION
	RSVRIC	-1.00	INITIAL CONDITION
	X	.00	WORKING R AND D COEFFICIENT
483 RC	NORMAL DEPTH CHANNEL		
	ANL	.045	LEFT OVERBANK N-VALUE
	ANCH	.035	MAIN CHANNEL N-VALUE
	ANR	.045	RIGHT OVERBANK N-VALUE
	RLNTH	441.	REACH LENGTH
	SEL	.0180	ENERGY SLOPE
	ELMAX	.0	MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

		--- LEFT OVERBANK ---	+	----- MAIN CHANNEL -----	+	--- RIGHT OVERBANK ---
485 RY	ELEVATION	2328.50		2327.00		2328.50
484 RX	DISTANCE	.00		41.00		128.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.02	.05	.09	.13	.18	.24	.30	.37	.46
OUTFLOW	.00	3.15	10.71	22.50	38.67	59.49	85.22	116.17	146.03	192.97
ELEVATION	2326.00	2326.13	2326.26	2326.39	2326.53	2326.66	2326.79	2326.92	2327.05	2327.18
STORAGE	.54	.64	.74	.85	.97	1.09	1.22	1.37	1.52	1.69
OUTFLOW	246.61	307.28	375.26	450.85	534.33	625.98	724.30	831.98	949.90	1078.53
ELEVATION	2327.32	2327.45	2327.58	2327.71	2327.84	2327.97	2328.11	2328.24	2328.37	2328.50

\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 534. TO 1079.  
THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.  
THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

HYDROGRAPH AT STATION R062-1

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
+ 761.	12.43	135.	35.	34.	34.
		(INCHES) (AC-FT)	1.609 67.	1.682 70.	1.683 70.
PEAK STORAGE + (AC-FT)	TIME (HR)	MAXIMUM AVERAGE STORAGE			
+ 1.	12.43	6-HR	24-HR	72-HR	24.97-HR
		0.	0.	0.	0.







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 \* R064-2 \*  
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Route combined hydrograph from CP062

HYDROGRAPH ROUTING DATA

501 RS STORAGE ROUTING  
 NSTPS 1 NUMBER OF SUBREACHES  
 ITYP FLOW TYPE OF INITIAL CONDITION  
 RSVRIC -1.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

502 RC NORMAL DEPTH CHANNEL  
 ANL .045 LEFT OVERBANK N-VALUE  
 ANCH .035 MAIN CHANNEL N-VALUE  
 ANR .045 RIGHT OVERBANK N-VALUE  
 RLNTH 1554. REACH LENGTH  
 SEL .0240 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVERBANK ---	+	----- MAIN CHANNEL -----	+	--- RIGHT OVERBANK ---
504 RY ELEVATION	2316.00	2316.00	2315.00	2315.00	2316.00 2317.00 2318.00 2319.00
503 RX DISTANCE	.00	17.00	23.00	73.00	83.00 90.00 99.00 107.00

\*\*\*

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.39	.80	1.24	1.70	2.22	2.86	3.50	4.15	4.82
OUTFLOW	.00	24.88	80.05	159.50	261.21	380.70	537.92	721.34	929.32	1160.80
ELEVATION	2315.00	2315.21	2315.42	2315.63	2315.84	2316.05	2316.26	2316.47	2316.68	2316.89
STORAGE	5.49	6.18	6.89	7.60	8.34	9.08	9.84	10.61	11.40	12.20
OUTFLOW	1423.55	1718.28	2036.46	2377.85	2742.27	3129.69	3539.77	3972.29	4427.11	4904.13
ELEVATION	2317.10	2317.31	2317.53	2317.74	2317.95	2318.16	2318.37	2318.58	2318.79	2319.00

\*\*\* \*\*

HYDROGRAPH AT STATION R064-2

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
+ (CFS)	(HR)				
+ 1694.	12.37	261.	69.	66.	66.
		(INCHES)	1.610	1.696	1.697
		(AC-FT)	129.	136.	136.
PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	24.97-HR
+ (AC-FT)	(HR)				
+ 6.	12.37	1.	0.	0.	0.
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	24.97-HR
+ (FEET)	(HR)				
+ 2317.30	12.37	2315.59	2315.17	2315.16	2315.16

CUMULATIVE AREA = 1.51 SQ MI

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 \* 064 \*  
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SUB-BASIN 064  
 24-HOUR SCS TYPE II RAINFALL WAS USED TO FIND TC & R FOR THIS BASIN  
 THIS BASIN USED RAINFALL REDUCTION FACTOR OF .928  
 L = 1.80 Kb = .046 Adj. Slope = 149.0

SUBBASIN RUNOFF DATA

510 BA SUBBASIN CHARACTERISTICS  
 TAREA .47 SUBBASIN AREA

PRECIPITATION DATA

14 PB STORM 4.27 BASIN TOTAL PRECIPITATION



UNIT HYDROGRAPH  
61 END-OF-PERIOD ORDINATES

11.	29.	43.	61.	84.	121.	218.	384.	538.	615.
621.	597.	556.	505.	457.	413.	373.	338.	305.	276.
250.	226.	204.	185.	167.	151.	137.	124.	112.	101.
91.	83.	75.	68.	61.	55.	50.	45.	41.	37.
33.	30.	27.	25.	22.	20.	18.	17.	15.	14.
12.	11.	10.	9.	8.	7.	7.	6.	5.	5.
4.									

\*\*\*                    \*\*\*                    \*\*\*                    \*\*\*                    \*\*\*

HYDROGRAPH AT STATION    064

TOTAL RAINFALL =    4.27, TOTAL LOSS =    2.63, TOTAL EXCESS =    1.63

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
694.	12.27	80.	21.	20.	20.
		(INCHES) 1.565	1.627	1.627	1.627
		(AC-FT) 39.	41.	41.	41.

CUMULATIVE AREA =    .47 SQ MI

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515 KK                    \*                    \*  
                             \*                    \*  
                             \*                    \*  
                             \*                    \*

Combine routed hydrographs from 058 and CP062 with runoff hydrograph from 064

518 HC                    HYDROGRAPH COMBINATION  
                             ICOMP                    3    NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\*                    \*\*\*                    \*\*\*                    \*\*\*                    \*\*\*

HYDROGRAPH AT STATION    CP064

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
4244.	12.50	629.	164.	157.	157.
		(INCHES) 1.565	1.630	1.630	1.630
		(AC-FT) 312.	325.	325.	325.

CUMULATIVE AREA =    3.74 SQ MI

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519 KK                    \*                    \*  
                             \*                    \*  
                             \*                    \*  
                             \*                    \*

Route combined hydrograph from CP064

HYDROGRAPH ROUTING DATA

521 RS                    STORAGE ROUTING  
                             NSTPS                    ---1--- NUMBER OF SUBREACHES  
                             ITYP                    FLOW TYPE OF INITIAL CONDITION  
                             RSVRIC                    -1.00 INITIAL CONDITION  
                             X                    .00 WORKING R AND D COEFFICIENT

522 RC                    NORMAL DEPTH CHANNEL  
                             ANL                    .045 LEFT OVERBANK N-VALUE  
                             ANCH                    .035 MAIN CHANNEL N-VALUE  
                             ANR                    .045 RIGHT OVERBANK N-VALUE  
                             RLNTH                    934. REACH LENGTH  
                             SEL                    .0210 ENERGY SLOPE  
                             ELMAX                    .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	---	LEFT OVERBANK	---	+	-----	MAIN CHANNEL	-----	+	---	RIGHT OVERBANK	---
524 RY	ELEVATION	2281.00	2281.00	2280.00	2280.00	2281.00	2282.00	2283.00	2284.00		
523 RX	DISTANCE	.00	31.00	66.00	104.00	111.00	121.00	129.00	149.00		





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 \* CP066 \*  
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Combine routed hydrograph from CP064 with runoff hydrograph from 066

537 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION CP066

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
4243.	12.50	630.	164.	158.	158.	
		(INCHES)	1.564	1.629	1.630	1.630
		(AC-FT)	312.	325.	325.	325.

CUMULATIVE AREA = 3.74 SQ MI

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 \* R068-2 \*  
 \* \*  
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Route combined hydrograph from CP066

HYDROGRAPH ROUTING DATA

540 RS STORAGE ROUTING  
 NSTPS 4 NUMBER OF SUBREACHES  
 ITYP FLOW TYPE OF INITIAL CONDITION  
 RSVRIC -1.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

541 RC NORMAL DEPTH CHANNEL  
 ANL .045 LEFT OVERBANK N-VALUE  
 ANCH .035 MAIN CHANNEL N-VALUE  
 ANR .045 RIGHT OVERBANK N-VALUE  
 RLNTH 3234. REACH LENGTH  
 SEL .0250 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVERBANK ---	--- + ---	----- MAIN CHANNEL -----	+ ---	--- RIGHT OVERBANK ---
543 RY ELEVATION	2228.00	2227.00	2227.00	2226.50	2227.00
542 RX DISTANCE	198.00	224.00	270.00	283.00	688.00
				684.00	721.00
				688.00	769.00

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

	.00	2.36	4.73	7.11	9.52	11.93	14.37	17.13	20.08	23.06
STORAGE	.00	2.36	4.73	7.11	9.52	11.93	14.37	17.13	20.08	23.06
OUTFLOW	.00	39.18	124.54	245.07	396.30	575.50	780.76	1016.19	1284.93	1582.74
ELEVATION	2226.50	2226.58	2226.66	2226.74	2226.82	2226.89	2226.97	2227.05	2227.13	2227.21
STORAGE	26.08	29.13	32.22	35.34	38.49	41.68	44.90	48.16	51.45	54.77
OUTFLOW	1908.11	2260.02	2637.71	3040.54	3468.02	3919.71	4395.25	4894.34	5416.68	5962.06
ELEVATION	2227.29	2227.37	2227.45	2227.53	2227.60	2227.68	2227.76	2227.84	2227.92	2228.00

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HYDROGRAPH AT STATION R068-2

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
4181.	12.60	629.	164.	158.	158.	
		(INCHES)	1.564	1.628	1.628	1.628
		(AC-FT)	312.	325.	325.	325.

PEAK STORAGE + (AC-FT)	TIME (HR)	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	24.97-HR
11.	12.60	2.	1.	1.	1.

PEAK STAGE + (AC-FT)	TIME (HR)	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	24.97-HR
11.	12.60	2.	1.	1.	1.





UNIT HYDROGRAPH  
87 END-OF-PERIOD ORDINATES

9.	27.	39.	51.	66.	84.	108.	144.	219.	340.
493.	636.	717.	740.	733.	714.	685.	645.	601.	561.
523.	488.	455.	424.	396.	369.	344.	321.	299.	279.
260.	243.	226.	211.	197.	184.	171.	160.	149.	139.
130.	121.	113.	105.	98.	91.	85.	80.	74.	69.
64.	60.	56.	52.	49.	46.	42.	40.	37.	34.
32.	30.	28.	26.	24.	23.	21.	20.	18.	17.
16.	15.	14.	13.	12.	11.	11.	10.	9.	9.
8.	7.	7.	6.	6.	6.	5.			

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HYDROGRAPH AT STATION      070

TOTAL RAINFALL =    4.27, TOTAL LOSS =    2.88, TOTAL EXCESS =    1.39

+ PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
+ 833.	12.37	(CFS)	115.	29.	28.	28.
		(INCHES)	1.344	1.381	1.381	1.381
		(AC-FT)	57.	58.	58.	58.

CUMULATIVE AREA =    .79 SQ MI

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574 KK

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*          *
*   CP070   *
*          *
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Combine routed hydrograph from CP068 with runoff hydrograph from 070

576 HC

HYDROGRAPH COMBINATION  
ICOMP                    2    NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION      CP070

+ PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.97-HR	
+ 9940.	13.07	(CFS)	2293.	606.	583.	583.
		(INCHES)	1.516	1.603	1.603	1.603
		(AC-FT)	1137.	1202.	1202.	1202.

CUMULATIVE AREA =    14.06 SQ MI

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577 KK

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*   R072-1   *
*          *
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Route combined hydrograph from CP070

HYDROGRAPH ROUTING DATA

579 RS

STORAGE ROUTING  
NSTPS                    8    NUMBER OF SUBREACHES  
ITYP                    FLOW    TYPE OF INITIAL CONDITION  
RSVRIC                  -1.00    INITIAL CONDITION  
X                        .00    WORKING R AND D COEFFICIENT

580 RC

NORMAL DEPTH CHANNEL  
ANL                    .045    LEFT OVERBANK N-VALUE  
ANCH                   .035    MAIN CHANNEL N-VALUE  
ANR                    .045    RIGHT OVERBANK N-VALUE  
RLNTH                  8409.    REACH LENGTH  
SEL                    .0230    ENERGY SLOPE  
ELMAX                  .0        MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVERBANK ---	+	----- MAIN CHANNEL -----	+	--- RIGHT OVERBANK ---				
582 RY	ELEVATION	1902.00	1900.00	1898.00	1897.00	1899.00	1897.00	1896.00	1901.00

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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 HYDROGRAPH AT STATION 1BIN  
 PEAK FLOW TIME MAXIMUM AVERAGE FLOW  
 + (CFS) (HR) 6-HR 24-HR 72-HR 24.97-HR  
 + 22. 12.03 (CFS) 2. 1. 1. 1.  
 (INCHES) 1.656 2.118 2.119 2.119  
 (AC-FT) 1. 1. 1. 1.  
 CUMULATIVE AREA = .01 SQ MI

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614 KK  
 \* RET1B \*  
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 RETENTION BASIN 1B

HYDROGRAPH ROUTING DATA

616 RS STORAGE ROUTING  
 NSTPS 1 NUMBER OF SUBREACHES  
 ITYP STOR TYPE OF INITIAL CONDITION  
 RSVRIC .00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT  
 617 SV STORAGE .0 .0 .1 .2  
 618 SE ELEVATION .00 1.00 2.00 3.00  
 620 SL LOW-LEVEL OUTLET  
 ELEV 1.25 ELEVATION AT CENTER OF OUTLET  
 CAREA .20 CROSS-SECTIONAL AREA  
 COQL .60 COEFFICIENT  
 EXPL .50 EXPONENT OF HEAD  
 619 SS SPILLWAY  
 CREL 2.30 SPILLWAY CREST ELEVATION  
 SPWID 15.00 SPILLWAY WIDTH  
 COQW 2.30 WEIR COEFFICIENT  
 EXPW 1.50 EXPONENT OF HEAD

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COMPUTED OUTFLOW-ELEVATION DATA

OUTFLOW	.00	.00	.47	.50	.55	.60	.66	.74	.84	.97
ELEVATION	.00	1.25	1.49	1.53	1.59	1.65	1.74	1.86	2.04	2.30
OUTFLOW	1.00	1.17	1.59	2.38	3.65	5.54	8.14	11.58	15.98	21.45
ELEVATION	2.31	2.33	2.37	2.42	2.48	2.56	2.65	2.75	2.87	3.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.04	.05	.07	.07	.08	.08	.09	.10	.10
OUTFLOW	.00	.00	.00	.47	.50	.55	.60	.66	.74	.82
ELEVATION	.00	1.00	1.25	1.49	1.53	1.59	1.65	1.74	1.86	2.00
STORAGE	.11	.13	.14	.14	.15	.15	.16	.17	.18	.19
OUTFLOW	.84	.97	1.17	1.59	2.38	3.65	5.54	8.14	11.58	15.98
ELEVATION	2.04	2.30	2.33	2.37	2.42	2.48	2.56	2.65	2.75	2.87
STORAGE	.20									
OUTFLOW	21.45									
ELEVATION	3.00									

WARNING --- ROUTED OUTFLOW ( 22.) IS GREATER THAN MAXIMUM OUTFLOW ( 21.) IN STORAGE-OUTFLOW TABLE  
 WARNING --- ROUTED OUTFLOW ( 22.) IS GREATER THAN MAXIMUM OUTFLOW ( 21.) IN STORAGE-OUTFLOW TABLE

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HYDROGRAPH AT STATION RET1B

PEAK FLOW TIME MAXIMUM AVERAGE FLOW  
 + (CFS) (HR) 6-HR 24-HR 72-HR 24.97-HR  
 + 22. 12.03 (CFS) 2. 1. 1. 1.  
 (INCHES) 1.633 2.026 2.026 2.026



.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
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.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
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.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
.00	.00	.00	.00	.00	.00	.00	.00	.00	.00

623 LG GREEN AND AMPT LOSS RATE  
 STRTL .25 STARTING LOSS  
 DTH .25 MOISTURE DEFICIT  
 PXSIF 4.03 WETTING FRONT SUCTION  
 XKSAT .57 HYDRAULIC CONDUCTIVITY  
 RTIMP 40.00 PERCENT IMPERVIOUS AREA

622 UI INPUT UNITGRAPH, 4 ORDINATES, VOLUME = .76  
 .0 37.0 79.0 17.0

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HYDROGRAPH AT STATION ON94

TOTAL RAINFALL = 4.27, TOTAL LOSS = 1.90, TOTAL EXCESS = 2.37

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
18.	12.03	1.	0.	0.	0.
		(INCHES) 1.428	1.807	1.807	1.807
		(AC-FT) 1.	1.	1.	1.

CUMULATIVE AREA = .01 SQ MI

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625 XK  
 \*\*\*\*\*  
 \* 2AIN \*  
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INFLOW TO RETENTION BASIN 2A

627 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION 2AIN

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
40.	12.03	3.	1.	1.	1.
		(INCHES) 1.528	1.924	1.927	1.927
		(AC-FT) 2.	2.	2.	2.

CUMULATIVE AREA = .02 SQ MI

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628 HK  
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 \* RET2A \*  
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RETENTION BASIN 2A

HYDROGRAPH ROUTING DATA

630 RS STORAGE ROUTING  
 NSTPS 1 NUMBER OF SUBREACHES  
 ITYP STOR TYPE OF INITIAL CONDITION









.00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00  
 .00 .00 .00 .00 .00 .00 .00 .00 .00 .00

641 LG GREEN AND AMPT LOSS RATE  
 STRTL .25 STARTING LOSS  
 DTH .25 MOISTURE DEFICIT  
 PSIF 4.03 WETTING FRONT SUCTION  
 XKSAT .56 HYDRAULIC CONDUCTIVITY  
 RTIMP 40.00 PERCENT IMPERVIOUS AREA

640 UI INPUT UNITGRAPH, 3 ORDINATES, VOLUME = .99  
 .0 62.0 15.0

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HYDROGRAPH AT STATION ON06

TOTAL RAINFALL = 4.27, TOTAL LOSS = 1.90, TOTAL EXCESS = 2.37

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
+	11.	12.03			
		(CFS)	1.	0.	0.
		(INCHES)	1.865	2.359	2.359
		(AC-FT)	0.	1.	1.
CUMULATIVE AREA =			.00 SQ MI		

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 643 KK \* 2BIN \*  
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INFLOW TO RETENTION BASIN 2B

645 HC HYDROGRAPH COMBINATION  
 ICOMP 3 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION 2BIN

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.97-HR
+	50.	12.03			
		(CFS)	4.	1.	1.
		(INCHES)	1.220	1.706	1.711
		(AC-FT)	2.	3.	3.
CUMULATIVE AREA =			.03 SQ MI		

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 646 KK \* RET2B \*  
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647 KO OUTPUT CONTROL VARIABLES  
 IPRNT 2 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 RETENTION BASIN 2B

HYDROGRAPH ROUTING DATA

649 RS STORAGE ROUTING  
 NSTPS 1 NUMBER OF SUBREACHES  
 ITYP STOR TYPE OF INITIAL CONDITION  
 RSVRIC .00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

650 SV STORAGE .0 .1 .1 .2

651 SE ELEVATION .00 1.00 2.00 3.00

653 SL LOW-LEVEL OUTLET  
 ELEV .25 ELEVATION AT CENTER OF OUTLET  
 CAREA .39 CROSS-SECTIONAL AREA  
 COQL .60 COEFFICIENT  
 EXPL .50 EXPONENT OF HEAD

652 SS SPILLWAY  
 CREL 2.40 SPILLWAY CREST ELEVATION  
 SPWID 40.00 SPILLWAY WIDTH  
 COQW 2.30 WEIR COEFFICIENT  
 EXPW 1.50 EXPONENT OF HEAD

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COMPUTED OUTFLOW-ELEVATION DATA

OUTFLOW	.00	.00	1.12	1.23	1.35	1.51	1.70	1.95	2.29	2.77
ELEVATION	.00	.25	.60	.67	.76	.88	1.06	1.32	1.72	2.40
OUTFLOW	27.83	3.16	4.01	5.65	8.31	12.26	17.74	25.00	34.30	45.89
ELEVATION	2.41	2.43	2.46	2.50	2.55	2.62	2.70	2.79	2.89	3.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.01	.03	.03	.04	.04	.05	.05	.07	.10
OUTFLOW	.00	.00	1.12	1.23	1.35	1.51	1.64	1.70	1.95	2.29
ELEVATION	.00	.25	.60	.67	.76	.88	1.00	1.06	1.32	1.72
STORAGE	.13	.17	.17	.17	.18	.18	.19	.20	.21	.22
OUTFLOW	2.50	2.77	3.16	4.01	5.65	8.31	12.26	17.74	25.00	34.30
ELEVATION	2.00	2.40	2.43	2.46	2.50	2.55	2.62	2.70	2.79	2.89
STORAGE	.23									
OUTFLOW	45.89									
ELEVATION	3.00									

\*\*\* WARNING \*\*\* MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 18. TO 46.  
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.  
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

HYDROGRAPH AT STATION RET2B

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE
.7	1	0000	1	0.	.0	.0	.7	1	0002	2	0.	.0	.3	.7	1	0004	3	0.	.0
.7	1	0006	4	0.	.0	.0	.7	1	0008	5	0.	.0	.3	.7	1	0010	6	0.	.0
.7	1	0012	7	0.	.0	.0	.7	1	0014	8	0.	.0	.3	.7	1	0016	9	0.	.0
.7	1	0018	10	0.	.0	.0	.7	1	0020	11	0.	.0	.3	.7	1	0022	12	0.	.0
.7	1	0024	13	0.	.0	.0	.7	1	0026	14	0.	.0	.3	.7	1	0028	15	0.	.0
.7	1	0030	16	0.	.0	.1	.7	1	0032	17	0.	.0	.3	.7	1	0034	18	0.	.0
.7	1	0036	19	0.	.0	.1	.7	1	0036	19	0.	.0	.3	.7	1	0036	19	0.	.0



.6	1	0212	67	0.	.0	.3 * 1	1032	317	0.	.0	.4 * 1	1852	567	1.	.0
.6	1	0214	68	0.	.0	.3 * 1	1034	318	0.	.0	.4 * 1	1854	568	1.	.0
.6	1	0216	69	0.	.0	.3 * 1	1036	319	0.	.0	.4 * 1	1856	569	1.	.0
.6	1	0218	70	0.	.0	.3 * 1	1038	320	0.	.0	.4 * 1	1858	570	1.	.0
.6	1	0220	71	0.	.0	.3 * 1	1040	321	0.	.0	.4 * 1	1900	571	1.	.0
.6	1	0222	72	0.	.0	.3 * 1	1042	322	0.	.0	.4 * 1	1902	572	1.	.0
.6	1	0224	73	0.	.0	.3 * 1	1044	323	0.	.0	.4 * 1	1904	573	1.	.0
.6	1	0226	74	0.	.0	.3 * 1	1046	324	0.	.0	.4 * 1	1906	574	1.	.0
.6	1	0228	75	0.	.0	.3 * 1	1048	325	0.	.0	.4 * 1	1908	575	1.	.0
.6	1	0230	76	0.	.0	.3 * 1	1050	326	1.	.0	.4 * 1	1910	576	1.	.0
.6	1	0232	77	0.	.0	.3 * 1	1052	327	1.	.0	.4 * 1	1912	577	1.	.0
.6	1	0234	78	0.	.0	.3 * 1	1054	328	1.	.0	.4 * 1	1914	578	1.	.0
.6	1	0236	79	0.	.0	.3 * 1	1056	329	1.	.0	.4 * 1	1916	579	1.	.0
.6	1	0238	80	0.	.0	.3 * 1	1058	330	1.	.0	.4 * 1	1918	580	1.	.0
.6	1	0240	81	0.	.0	.3 * 1	1100	331	1.	.0	.4 * 1	1920	581	1.	.0
.6	1	0242	82	0.	.0	.3 * 1	1102	332	1.	.0	.4 * 1	1922	582	1.	.0
.6	1	0244	83	0.	.0	.3 * 1	1104	333	1.	.0	.4 * 1	1924	583	1.	.0
.6	1	0246	84	0.	.0	.3 * 1	1106	334	1.	.0	.4 * 1	1926	584	1.	.0
.6	1	0248	85	0.	.0	.3 * 1	1108	335	1.	.0	.5 * 1	1928	585	1.	.0
.6	1	0250	86	0.	.0	.3 * 1	1110	336	1.	.0	.5 * 1	1930	586	1.	.0
.6	1	0252	87	0.	.0	.3 * 1	1112	337	1.	.0	.5 * 1	1932	587	1.	.0
.6	1	0254	88	0.	.0	.3 * 1	1114	338	1.	.0	.5 * 1	1934	588	1.	.0
.6	1	0256	89	0.	.0	.3 * 1	1116	339	1.	.0	.5 * 1	1936	589	1.	.0
.6	1	0258	90	0.	.0	.3 * 1	1118	340	1.	.0	.5 * 1	1938	590	1.	.0
.6	1	0300	91	0.	.0	.3 * 1	1120	341	1.	.0	.5 * 1	1940	591	1.	.0
.6	1	0302	92	0.	.0	.3 * 1	1122	342	1.	.0	.5 * 1	1942	592	1.	.0
.6	1	0304	93	0.	.0	.3 * 1	1124	343	1.	.0	.5 * 1	1944	593	1.	.0
.6	1	0306	94	0.	.0	.3 * 1	1126	344	1.	.0	.5 * 1	1946	594	1.	.0
.6	1	0308	95	0.	.0	.3 * 1	1128	345	1.	.0	.5 * 1	1948	595	1.	.0
.6	1	0310	96	0.	.0	.3 * 1	1130	346	1.	.0	.5 * 1	1950	596	1.	.0
.6	1	0312	97	0.	.0	.3 * 1	1132	347	1.	.0	.5 * 1	1952	597	1.	.0
.6	1	0314	98	0.	.0	.3 * 1	1134	348	1.	.0	.6 * 1	1954	598	1.	.0
.6	1	0316	99	0.	.0	.3 * 1	1136	349	1.	.0	.8 * 1	1956	599	1.	.0
.6	1	0318	100	0.	.0	.3 * 1	1138	350	2.	.1	1.0 * 1	1958	600	1.	.0
.6	1	0320	101	0.	.0	.3 * 1	1140	351	2.	.1	1.2 * 1	2000	601	1.	.0
.6	1	0322	102	0.	.0	.3 * 1	1142	352	2.	.1	1.3 * 1	2002	602	1.	.0
.6	1	0324	103	0.	.0	.3 * 1	1144	353	2.	.1	1.5 * 1	2004	603	1.	.0
.6	1	0326	104	0.	.0	.3 * 1	1146	354	2.	.1	1.6 * 1	2006	604	1.	.0
.6	1	0328	105	0.	.0	.3 * 1	1148	355	2.	.1	1.9 * 1	2008	605	1.	.0
.6	1	0330	106	0.	.0	.3 * 1	1150	356	3.	.2	2.3 * 1	2010	606	1.	.0
.6	1	0332	107	0.	.0	.3 * 1	1152	357	12.	.2	2.6 * 1	2012	607	1.	.0
.6	1	0334	108	0.	.0	.3 * 1	1154	358	22.	.2	2.7 * 1	2014	608	1.	.0
.6	1	0336	109	0.	.0	.3 * 1	1156	359	22.	.2	2.7 * 1	2016	609	1.	.0
.6	1	0338	110	0.	.0	.3 * 1	1158	360	22.	.2	2.8 * 1	2018	610	1.	.0
.5	1	0340	111	0.	.0	.3 * 1	1200	361	30.	.2	2.8 * 1	2020	611	1.	.0
.5	1	0342	112	0.	.0	.3 * 1	1202	362	45.	.2	3.0 * 1	2022	612	1.	.0
.5	1	0344	113	0.	.0	.3 * 1	1204	363	45.	.2	3.0 * 1	2024	613	1.	.0

1	0346	114	0.	.0	.3 * 1	1206 364	33.	.2	2.9 * 1	2026 614	1.	.0	
.5	1	0348	115	0.	.0	.3 * 1	1208 365	25.	.2	2.8 * 1	2028 615	1.	.0
.5	1	0350	116	0.	.0	.3 * 1	1210 366	18.	.2	2.7 * 1	2030 616	1.	.0
.5	1	0352	117	0.	.0	.3 * 1	1212 367	13.	.2	2.6 * 1	2032 617	1.	.0
.5	1	0354	118	0.	.0	.3 * 1	1214 368	10.	.2	2.6 * 1	2034 618	1.	.0
.5	1	0356	119	0.	.0	.3 * 1	1216 369	9.	.2	2.6 * 1	2036 619	1.	.0
.5	1	0358	120	0.	.0	.3 * 1	1218 370	7.	.2	2.5 * 1	2038 620	1.	.0
.5	1	0400	121	0.	.0	.3 * 1	1220 371	6.	.2	2.5 * 1	2040 621	1.	.0
.5	1	0402	122	0.	.0	.3 * 1	1222 372	6.	.2	2.5 * 1	2042 622	1.	.0
.5	1	0404	123	0.	.0	.3 * 1	1224 373	5.	.2	2.5 * 1	2044 623	1.	.0
.5	1	0406	124	0.	.0	.3 * 1	1226 374	5.	.2	2.5 * 1	2046 624	1.	.0
.5	1	0408	125	0.	.0	.3 * 1	1228 375	4.	.2	2.5 * 1	2048 625	1.	.0
.5	1	0410	126	0.	.0	.3 * 1	1230 376	4.	.2	2.5 * 1	2050 626	1.	.0
.5	1	0412	127	0.	.0	.3 * 1	1232 377	4.	.2	2.5 * 1	2052 627	1.	.0
.5	1	0414	128	0.	.0	.3 * 1	1234 378	4.	.2	2.4 * 1	2054 628	1.	.0
.5	1	0416	129	0.	.0	.3 * 1	1236 379	4.	.2	2.4 * 1	2056 629	1.	.0
.5	1	0418	130	0.	.0	.3 * 1	1238 380	4.	.2	2.4 * 1	2058 630	1.	.0
.5	1	0420	131	0.	.0	.3 * 1	1240 381	3.	.2	2.4 * 1	2100 631	1.	.0
.5	1	0422	132	0.	.0	.3 * 1	1242 382	3.	.2	2.4 * 1	2102 632	1.	.0
.5	1	0424	133	0.	.0	.3 * 1	1244 383	3.	.2	2.4 * 1	2104 633	1.	.0
.5	1	0426	134	0.	.0	.3 * 1	1246 384	3.	.2	2.4 * 1	2106 634	1.	.0
.5	1	0428	135	0.	.0	.3 * 1	1248 385	3.	.2	2.4 * 1	2108 635	1.	.0
.5	1	0430	136	0.	.0	.3 * 1	1250 386	3.	.2	2.4 * 1	2110 636	1.	.0
.5	1	0432	137	0.	.0	.3 * 1	1252 387	3.	.2	2.4 * 1	2112 637	1.	.0
.5	1	0434	138	0.	.0	.3 * 1	1254 388	3.	.2	2.4 * 1	2114 638	1.	.0
.5	1	0436	139	0.	.0	.3 * 1	1256 389	3.	.2	2.4 * 1	2116 639	1.	.0
.5	1	0438	140	0.	.0	.3 * 1	1258 390	3.	.2	2.4 * 1	2118 640	1.	.0
.5	1	0440	141	0.	.0	.3 * 1	1300 391	3.	.2	2.4 * 1	2120 641	1.	.0
.5	1	0442	142	0.	.0	.3 * 1	1302 392	3.	.2	2.4 * 1	2122 642	1.	.0
.5	1	0444	143	0.	.0	.3 * 1	1304 393	3.	.2	2.4 * 1	2124 643	1.	.0
.5	1	0446	144	0.	.0	.3 * 1	1306 394	3.	.2	2.4 * 1	2126 644	1.	.0
.5	1	0448	145	0.	.0	.3 * 1	1308 395	3.	.2	2.4 * 1	2128 645	1.	.0
.5	1	0450	146	0.	.0	.3 * 1	1310 396	3.	.2	2.3 * 1	2130 646	1.	.0
.5	1	0452	147	0.	.0	.3 * 1	1312 397	3.	.2	2.3 * 1	2132 647	1.	.0
.5	1	0454	148	0.	.0	.3 * 1	1314 398	3.	.2	2.3 * 1	2134 648	1.	.0
.5	1	0456	149	0.	.0	.3 * 1	1316 399	3.	.2	2.3 * 1	2136 649	1.	.0
.5	1	0458	150	0.	.0	.3 * 1	1318 400	3.	.2	2.3 * 1	2138 650	1.	.0
.5	1	0500	151	0.	.0	.3 * 1	1320 401	3.	.2	2.3 * 1	2140 651	1.	.0
.5	1	0502	152	0.	.0	.3 * 1	1322 402	3.	.2	2.3 * 1	2142 652	1.	.0
.5	1	0504	153	0.	.0	.3 * 1	1324 403	3.	.2	2.2 * 1	2144 653	1.	.0
.5	1	0506	154	0.	.0	.3 * 1	1326 404	3.	.1	2.2 * 1	2146 654	1.	.0
.5	1	0508	155	0.	.0	.3 * 1	1328 405	3.	.1	2.2 * 1	2148 655	1.	.0
.5	1	0510	156	0.	.0	.3 * 1	1330 406	3.	.1	2.2 * 1	2150 656	1.	.0
.5	1	0512	157	0.	.0	.3 * 1	1332 407	3.	.1	2.2 * 1	2152 657	1.	.0
.5	1	0514	158	0.	.0	.3 * 1	1334 408	3.	.1	2.2 * 1	2154 658	1.	.0
.5	1	0516	159	0.	.0	.3 * 1	1336 409	3.	.1	2.1 * 1	2156 659	1.	.0
.5	1	0518	160	0.	.0	.3 * 1	1338 410	3.	.1	2.1 * 1	2158 660	1.	.0



.5	1	0654	208	0.	.0	.3 * 1	1514	458	2.	.1	1.1 * 1	2334	708	1.	.0
.5	1	0656	209	0.	.0	.3 * 1	1516	459	2.	.1	1.1 * 1	2336	709	1.	.0
.5	1	0658	210	0.	.0	.3 * 1	1518	460	2.	.1	1.0 * 1	2338	710	1.	.0
.5	1	0700	211	0.	.0	.3 * 1	1520	461	2.	.1	1.0 * 1	2340	711	1.	.0
.5	1	0702	212	0.	.0	.3 * 1	1522	462	2.	.1	1.0 * 1	2342	712	1.	.0
.5	1	0704	213	0.	.0	.3 * 1	1524	463	2.	.1	1.0 * 1	2344	713	1.	.0
.5	1	0706	214	0.	.0	.3 * 1	1526	464	2.	.0	1.0 * 1	2346	714	1.	.0
.5	1	0708	215	0.	.0	.3 * 1	1528	465	2.	.0	1.0 * 1	2348	715	1.	.0
.5	1	0710	216	0.	.0	.3 * 1	1530	466	2.	.0	1.0 * 1	2350	716	1.	.0
.5	1	0712	217	0.	.0	.3 * 1	1532	467	2.	.0	.9 * 1	2352	717	1.	.0
.5	1	0714	218	0.	.0	.3 * 1	1534	468	2.	.0	.9 * 1	2354	718	1.	.0
.5	1	0716	219	0.	.0	.3 * 1	1536	469	2.	.0	.9 * 1	2356	719	1.	.0
.5	1	0718	220	0.	.0	.3 * 1	1538	470	2.	.0	.9 * 1	2358	720	1.	.0
.5	1	0720	221	0.	.0	.3 * 1	1540	471	2.	.0	.9 * 2	0000	721	1.	.0
.5	1	0722	222	0.	.0	.3 * 1	1542	472	2.	.0	.9 * 2	0002	722	1.	.0
.5	1	0724	223	0.	.0	.3 * 1	1544	473	1.	.0	.9 * 2	0004	723	1.	.0
.4	1	0726	224	0.	.0	.3 * 1	1546	474	1.	.0	.9 * 2	0006	724	1.	.0
.4	1	0728	225	0.	.0	.3 * 1	1548	475	1.	.0	.9 * 2	0008	725	1.	.0
.4	1	0730	226	0.	.0	.3 * 1	1550	476	1.	.0	.8 * 2	0010	726	1.	.0
.4	1	0732	227	0.	.0	.3 * 1	1552	477	1.	.0	.8 * 2	0012	727	1.	.0
.4	1	0734	228	0.	.0	.3 * 1	1554	478	1.	.0	.8 * 2	0014	728	1.	.0
.4	1	0736	229	0.	.0	.3 * 1	1556	479	1.	.0	.8 * 2	0016	729	1.	.0
.4	1	0738	230	0.	.0	.3 * 1	1558	480	1.	.0	.8 * 2	0018	730	1.	.0
.4	1	0740	231	0.	.0	.3 * 1	1600	481	1.	.0	.8 * 2	0020	731	1.	.0
.4	1	0742	232	0.	.0	.3 * 1	1602	482	1.	.0	.8 * 2	0022	732	1.	.0
.4	1	0744	233	0.	.0	.3 * 1	1604	483	1.	.0	.8 * 2	0024	733	1.	.0
.4	1	0746	234	0.	.0	.3 * 1	1606	484	1.	.0	.8 * 2	0026	734	1.	.0
.4	1	0748	235	0.	.0	.3 * 1	1608	485	1.	.0	.8 * 2	0028	735	1.	.0
.4	1	0750	236	0.	.0	.3 * 1	1610	486	1.	.0	.8 * 2	0030	736	1.	.0
.4	1	0752	237	0.	.0	.3 * 1	1612	487	1.	.0	.8 * 2	0032	737	1.	.0
.4	1	0754	238	0.	.0	.3 * 1	1614	488	1.	.0	.8 * 2	0034	738	1.	.0
.4	1	0756	239	0.	.0	.3 * 1	1616	489	1.	.0	.8 * 2	0036	739	1.	.0
.4	1	0758	240	0.	.0	.3 * 1	1618	490	1.	.0	.8 * 2	0038	740	1.	.0
.4	1	0800	241	0.	.0	.3 * 1	1620	491	1.	.0	.8 * 2	0040	741	1.	.0
.4	1	0802	242	0.	.0	.3 * 1	1622	492	1.	.0	.8 * 2	0042	742	1.	.0
.4	1	0804	243	0.	.0	.3 * 1	1624	493	1.	.0	.8 * 2	0044	743	1.	.0
.4	1	0806	244	0.	.0	.3 * 1	1626	494	1.	.0	.8 * 2	0046	744	1.	.0
.4	1	0808	245	0.	.0	.3 * 1	1628	495	1.	.0	.7 * 2	0048	745	1.	.0
.4	1	0810	246	0.	.0	.3 * 1	1630	496	1.	.0	.7 * 2	0050	746	1.	.0
.4	1	0812	247	0.	.0	.3 * 1	1632	497	1.	.0	.7 * 2	0052	747	0.	.0
.4	1	0814	248	0.	.0	.3 * 1	1634	498	1.	.0	.7 * 2	0054	748	0.	.0
.4	1	0816	249	0.	.0	.3 * 1	1636	499	1.	.0	.7 * 2	0056	749	0.	.0
.4	1	0818	250	0.	.0	.3 * 1	1638	500	1.	.0	.7 * 2	0058	750	0.	.0

\*\*\*\*\*  
\*\*

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)	6-HR	24-HR	72-HR	24.97-HR
	(CFS)				

+	45.	12.03	4.	1.	1.	1.
			(INCHES)	1.215	1.697	1.697
			(AC-FT)	2.	3.	3.
	PEAK STORAGE	TIME		6-HR	MAXIMUM AVERAGE STORAGE	24.97-HR
	(AC-FT)	(HR)		24-HR	72-HR	
+	0.	12.03		0.	0.	0.
	PEAK STAGE	TIME		6-HR	MAXIMUM AVERAGE STAGE	24.97-HR
	(FEET)	(HR)		24-HR	72-HR	
+	2.99	12.03		1.52	.68	.66
					.66	.66

CUMULATIVE AREA = .03 SQ MI

\*\*\* \*\*

654 KK \*\*\*\*\*  
 \* DIV1 \*  
 \* \*  
 \*\*\*\*\*

Diversion to Park

DT	DIVERSION	ISTAD	DIV1	DIVERSION	HYDROGRAPH	IDENTIFICATION
DI	INFLOW		.00	3.00	40.00	
DQ	DIVERTED FLOW		.00	3.00	4.00	

\*\*\*

			DIVERSION	HYDROGRAPH	DIV1	
	PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24.97-HR
	(CFS)	(HR)		24-HR	72-HR	
+	4.	12.03	(CFS)	2.	1.	1.
			(INCHES)	.698	1.180	1.180
			(AC-FT)	1.	2.	2.

CUMULATIVE AREA = .03 SQ MI

\*\*\*

			HYDROGRAPH	AT	STATION	DIV1
	PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24.97-HR
	(CFS)	(HR)		24-HR	72-HR	
+	41.	12.03	(CFS)	2.	0.	0.
			(INCHES)	.518	.518	.518
			(AC-FT)	1.	1.	1.

CUMULATIVE AREA = .03 SQ MI

\*\*\* \*\*

659 KK \*\*\*\*\*  
 \* CP072A \*  
 \* \*  
 \*\*\*\*\*

Combine CP072 with onsite runoff

661 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\*

			HYDROGRAPH	AT	STATION	CP072A
	PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24.97-HR
	(CFS)	(HR)		24-HR	72-HR	
+			(CFS)			



+ 9865. 13.20  
 (INCHES) 2371. 633. 608. 608.  
 (AC-FT) 1.511 1.612 1.612 1.612  
 1176. 1255. 1255. 1255.

CUMULATIVE AREA = 14.59 SQ MI

1

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	005	484.	12.30	69.	18.	17.	.42		
ROUTED TO	R010-1	480.	12.33	69.	18.	17.	.42	2867.40	12.33
HYDROGRAPH AT	010	395.	12.30	54.	14.	13.	.34		
2 COMBINED AT	CP010	867.	12.33	123.	32.	30.	.75		
ROUTED TO	R012-1	832.	12.47	123.	31.	30.	.75	2791.28	12.47
HYDROGRAPH AT	012	344.	12.30	46.	12.	11.	.30		
2 COMBINED AT	CP012	1111.	12.43	169.	43.	42.	1.05		
DIVERSION TO	012OUT	423.	12.43	58.	15.	14.	1.05		
HYDROGRAPH AT	D012	688.	12.43	111.	28.	27.	1.05		
ROUTED TO	R030-1	599.	13.17	111.	28.	27.	1.05	2680.62	13.17
HYDROGRAPH AT	030	297.	12.43	60.	16.	15.	.35		
2 COMBINED AT	CP030A	731.	13.17	170.	44.	43.	1.40		
HYDROGRAPH AT	015	1766.	12.33	205.	52.	50.	1.27		
ROUTED TO	R022-1	1674.	12.50	205.	52.	50.	1.27	2728.88	12.50
HYDROGRAPH AT	017	286.	12.27	36.	9.	9.	.22		
2 COMBINED AT	CP022A	1875.	12.47	241.	62.	59.	1.48		
HYDROGRAPH AT	020	1458.	12.20	139.	38.	36.	.76		
ROUTED TO	R022-2	1293.	12.40	139.	38.	36.	.76	2688.05	12.40
HYDROGRAPH AT	022	340.	12.40	70.	18.	18.	.38		
2 COMBINED AT	CP022B	1633.	12.40	209.	56.	54.	1.14		
2 COMBINED AT	CP022	3445.	12.43	449.	118.	113.	2.63		
HYDROGRAPH AT	024	2481.	12.10	186.	51.	49.	1.02		
ROUTED TO	R026-1	2173.	12.27	186.	51.	49.	1.02	2688.34	12.27
HYDROGRAPH AT	026	309.	12.27	57.	16.	16.	.27		
2 COMBINED AT	CP026A	2481.	12.27	243.	67.	64.	1.29		



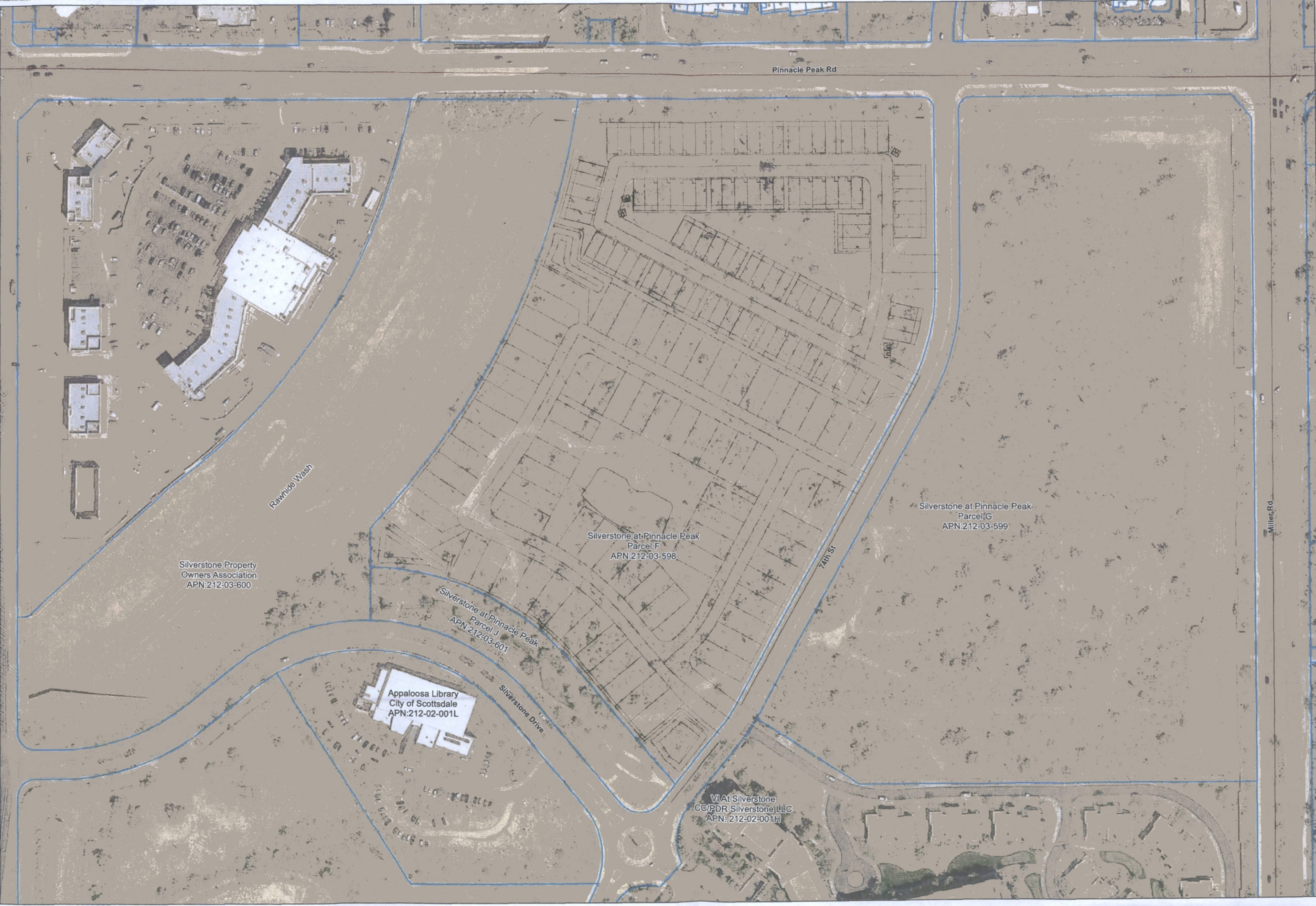
+		R051-1	7079.	12.87	1412.	375.	361.	8.60		
+									2264.70	12.87
		HYDROGRAPH AT								
+		048	108.	12.13	9.	2.	2.	.06		
		HYDROGRAPH AT								
+		051	192.	12.27	27.	7.	7.	.16		
		3 COMBINED AT								
+		CP051	7153.	12.87	1447.	384.	370.	8.82		
		ROUTED TO								
+		R053-1	7142.	12.87	1447.	384.	370.	8.82		
+									2242.56	12.87
		HYDROGRAPH AT								
+		053	175.	12.13	16.	4.	4.	.09		
		2 COMBINED AT								
+		CP053	7157.	12.87	1463.	389.	373.	8.91		
		ROUTED TO								
+		R055-1	7136.	12.90	1463.	388.	373.	8.91		
+									2200.63	12.90
		HYDROGRAPH AT								
+		055	338.	12.37	61.	16.	15.	.37		
		2 COMBINED AT								
+		CP055	7319.	12.90	1523.	404.	389.	9.28		
		ROUTED TO								
+		R068-1	7280.	12.93	1523.	404.	388.	9.28		
+									2200.70	12.93
		HYDROGRAPH AT								
+		058	2569.	12.33	288.	74.	71.	1.76		
		ROUTED TO								
+		R064-1	2367.	12.53	288.	74.	71.	1.76		
+									2323.10	12.53
		HYDROGRAPH AT								
+		060	764.	12.40	135.	35.	34.	.78		
		ROUTED TO								
+		R062-1	761.	12.43	135.	35.	34.	.78		
+									2328.15	12.43
		HYDROGRAPH AT								
+		062	1056.	12.27	127.	34.	32.	.73		
		2 COMBINED AT								
+		CP062	1707.	12.33	261.	69.	66.	1.51		
		ROUTED TO								
+		R064-2	1694.	12.37	261.	69.	66.	1.51		
+									2317.30	12.37
		HYDROGRAPH AT								
+		064	694.	12.27	80.	21.	20.	.47		
		3 COMBINED AT								
+		CP064	4244.	12.50	629.	164.	157.	3.74		
		ROUTED TO								
+		R066-1	4241.	12.50	629.	164.	157.	3.74		
+									2283.57	12.50
		HYDROGRAPH AT								
+		066	10.	12.07	1.	0.	0.	.01		
		2 COMBINED AT								
+		CP066	4243.	12.50	630.	164.	158.	3.74		
		ROUTED TO								
+		R068-2	4181.	12.60	629.	164.	158.	3.74		
+									2227.73	12.60
		HYDROGRAPH AT								
+		068	421.	12.20	39.	10.	10.	.25		
		3 COMBINED AT								
+		CP068	9990.	12.90	2190.	578.	555.	13.27		
		ROUTED TO								
+		R070-1	9712.	13.07	2185.	577.	554.	13.27		
+									2126.90	13.07
		HYDROGRAPH AT								
+		070	833.	12.37	115.	29.	28.	.79		
		2 COMBINED AT								
+		CP070	9940.	13.07	2293.	606.	583.	14.06		

+	ROUTED TO	R072-1	9834.	13.20	2291.	605.	581.	14.06		
+									1900.49	13.20
+	HYDROGRAPH AT	072	919.	12.20	91.	28.	27.	.50		
+	2 COMBINED AT	CP072	9865.	13.20	2370.	632.	608.	14.56		
+	HYDROGRAPH AT	ON01	5.	12.03	0.	0.	0.	.00		
+	ROUTED TO	RET1A	1.	12.10	0.	0.	0.	.00		
+									2.54	12.10
+	HYDROGRAPH AT	ON02	21.	12.03	2.	1.	0.	.01		
+	2 COMBINED AT	1BIN	22.	12.03	2.	1.	1.	.01		
+	ROUTED TO	RET1B	22.	12.03	2.	1.	1.	.01		
+									3.00	12.03
+	HYDROGRAPH AT	ON04	18.	12.03	1.	0.	0.	.01		
+	2 COMBINED AT	2AIN	40.	12.03	3.	1.	1.	.02		
+	ROUTED TO	RET2A	34.	12.07	2.	1.	1.	.02		
+									3.03	12.07
+	HYDROGRAPH AT	ON05	11.	12.03	1.	0.	0.	.00		
+	HYDROGRAPH AT	ON06	11.	12.03	1.	0.	0.	.00		
+	3 COMBINED AT	2BIN	50.	12.03	4.	1.	1.	.03		
+	ROUTED TO	RET2B	45.	12.03	4.	1.	1.	.03		
+									2.99	12.03
+	DIVERSION TO	DIV1	4.	12.03	2.	1.	1.	.03		
+	HYDROGRAPH AT	DIV1	41.	12.03	2.	0.	0.	.03		
+	2 COMBINED AT	CP072A	9865.	13.20	2371.	633.	608.	14.59		

\*\*\* NORMAL END OF HEC-1 \*\*\*

*Appendix E*

Exhibits



Silverstone Property Owners Association  
APN:212-03-600

Appaloosa Library  
City of Scottsdale  
APN:212-02-001L

Silverstone at Pinnacle Peak  
Parcel J  
APN:212-03-601

Silverstone at Pinnacle Peak  
Parcel F  
APN:212-03-598

Silverstone at Pinnacle Peak  
Parcel G  
APN:212-03-599

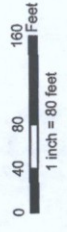
VI At Silverstone  
CC/PDR Silverstone LLC  
APN: 212-02-001H

668/1+	6+21
668/19	121/1
1'6711'1" x 6' x 6'	6' x 6'
1'58'11" x 6' x 6'	6' x 6'
4+16.1' x 6' x 6'	6' x 6'
1'97'	1/16

) \* 85 ( & 217 ( : 7 \$ ( 5 \$ / 0 \$ 3  
3 \$ 5 & ( / )  
6 , / 9 ( 5 6 7 2 1 ( 3 \$ 5 & ( / )  
6 & 2 7 7 6 ' \$ / ( 5 \$ = 2 1 \$

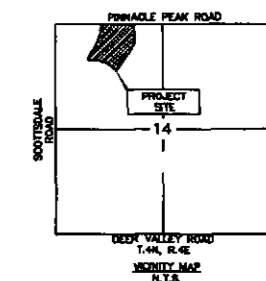
392-(6712  
DRAWING NAME  
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**Kimley»Horn**  
2016 KIMLEY-HORN AND ASSOCIATES, INC.  
(COMPILING) 3/10/2020 DDD  
(CONTRAPTOR) 3/10/2020  
1.15WK K.S. WELSH 637M  
3/10/2020 9:41:00 AM



# PRELIMINARY PLAT GRADING PLAN FOR SILVERSTONE PARCEL F

A PORTION OF THE NORTHWEST QUARTER OF SECTION 14, T-4-N,  
R-4-E, OF THE GILA AND SALT RIVER BASE AND MERIDIAN,  
MARICOPA COUNTY, ARIZONA



**ENGINEER**  
KIMLEY-HORN & ASSOCIATES  
7740 N. 16TH STREET, SUITE 300  
PHOENIX, ARIZONA 85020  
TELEPHONE: (602) 944-5500  
CONTACT: ANDREW JUPP, P.E.

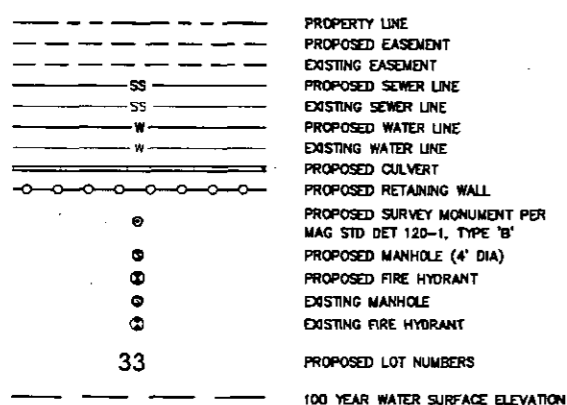
**OWNER/DEVELOPER**  
K. HOVANHIAN GREAT WEST HOMES, LLC  
20830 NORTH TATUM BLVD. SUITE 390  
PHOENIX AZ 85050  
TELEPHONE: (480) 824-4175  
CONTACT: CHUCK CHISHOLM

**SURVEYOR**  
ALLIANCE LAND SURVEYING, LLC.  
7900 N. 70TH AVENUE, SUITE 104  
GLENDALE, AZ 85303  
TELEPHONE: (623) 972-2200  
CONTACT: G. BRYAN GOETZENBERGER

**SITE DATA**  
GROSS AREA 22.07 AC±  
NET AREA 22.07 AC±  
TOTAL NUMBER OF LOTS 174  
MAXIMUM LOT AREA 5,991 S.F.  
MINIMUM LOT AREA 1,407 S.F.  
AVERAGE LOT AREA 2,872 S.F.  
DENSITY 7.99 DU/AC

**UTILITIES**  
WATER & SEWER CITY OF SCOTTSDALE  
ELECTRIC APS  
TELEPHONE CENTURY LINK  
GAS SOUTHWEST GAS CO.  
CABLE T.V. COX COMMUNICATIONS

**LEGEND**



**LEGAL DESCRIPTION**

PARCEL F OF THE NORTHWEST QUARTER OF SECTION 14, TOWNSHIP 3 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER BASE AND MERIDIAN MARICOPA COUNTY, ARIZONA

**BASIS OF BEARING**

THE BASIS OF BEARING IS THE MONUMENT LINE OF SCOTTSDALE ROAD, ALSO BEING THE WEST LINE OF THE NORTHWEST QUARTER OF SECTION 14, USING A BEARING OF NORTH 00 DEGREES 00 MINUTES 24 SECONDS EAST, PER THE MAP OF DEDICATION AND PARCEL MAP FOR SILVERSTONE AT PINNACLE PEAK, RECORDED IN BOOK 883, PAGE 17, M.C.R.

**ZONING**

R-5 (RESIDENTIAL)

**SETBACK TABLE**

FRONT	0 FT
REAR	0 FT
SIDE	0 FT

**ABBREVIATIONS**

AC	ACRE
AE	ACCESS EASEMENT
A.P.N.	ASSESSOR'S PARCEL NUMBER
C/L	CENTERLINE
COR	CORNER
C.O.S.	CITY OF SCOTTSDALE
DE	DRAINAGE EASEMENT
EAE	EMERGENCY ACCESS EASEMENT
ELEV	ELEVATION
EP	EDGE OF PAVEMENT
EX	EXISTING
FND	FOUND
FT	FOOT
G.L.O.	GOVERNMENT LAND OFFICE
NAOS	NATURAL AREA OPEN SPACE
NMPAE	NON-MOTORIZED PUBLIC ACCESS EASEMENT
NVAE	NON-VEHICULAR ACCESS EASEMENT
PUE	PUBLIC UTILITY EASEMENT
P/L	PROPERTY LINE
R/W	RIGHT-OF-WAY
S	SOUTH
SCE	SCENIC CORRIDOR EASEMENT
SE	SOUTHEAST
SEC	SECTION
S.F.	SQUARE FEET
SHLDR	SHOULDER
SS	SANITARY SEWER
SO	SQUARE
SW	SIDEWALK
TYP	TYPICAL
WT	WATER

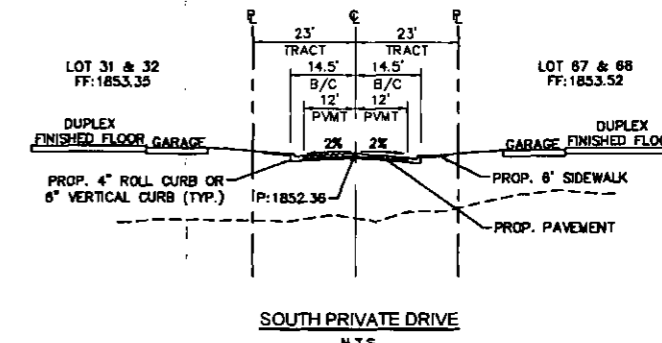
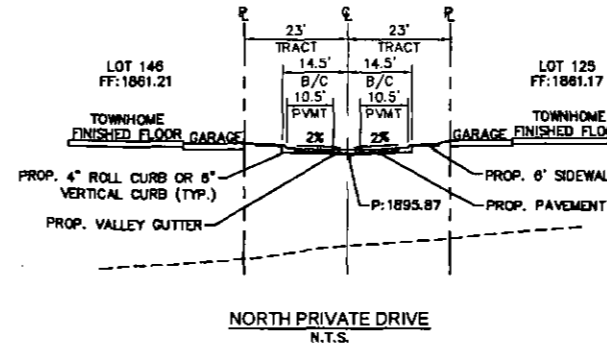
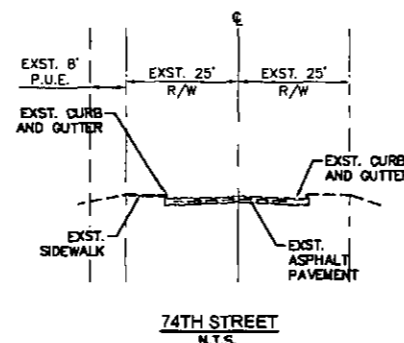
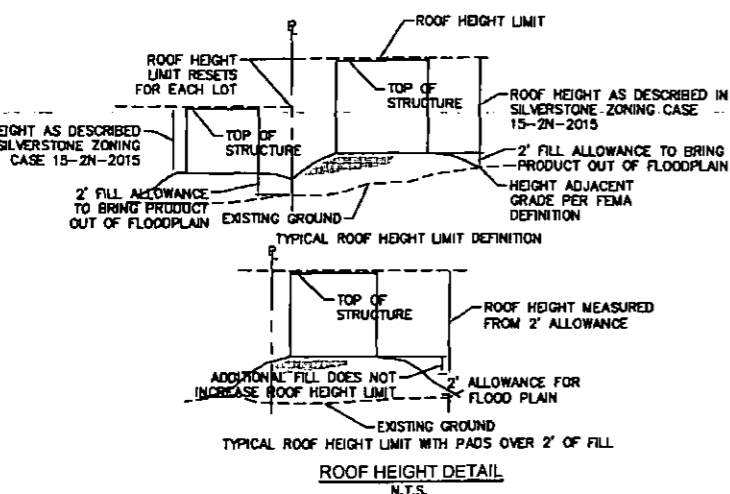
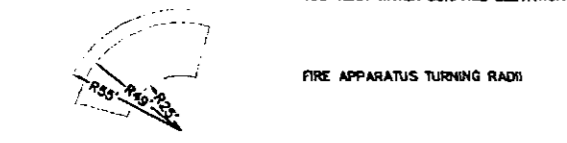
**NOTES**

- ALL STREETS WILL BE PRIVATE. ALL STREETS AND ANY STREET LIGHTS TO BE LOCATED WITHIN THIS PRIVATE TRACT ARE TO BE MAINTAINED BY THE PROPERTY OWNERS ASSOCIATION.
- FOR DISCHARGES AND RETENTION VOLUME CALCULATIONS, REFER TO "PRELIMINARY DRAINAGE REPORT FOR SILVERSTONE PARCEL F" PREPARED BY KIMLEY-HORN AND ASSOCIATES, INC.
- ALL AREA SET ASIDE FOR DETENTION WILL BE WITHIN A DEDICATED DRAINAGE EASEMENT. THESE DRAINAGE EASEMENTS WILL BE CONTIGUOUS TO THE SUBDIVISION'S CIRCULATION TRACTS. THE MAINTENANCE OF THESE DEDICATED DRAINAGE EASEMENTS WILL BE THE RESPONSIBILITY OF THE HOMEOWNER'S ASSOCIATION. THE CITY OF SCOTTSDALE RETAINS THE RIGHT TO ACCESS THE DETENTION BASINS FOR INSPECTION OR MAINTENANCE PURPOSES FROM SAID CIRCULATION TRACTS.
- ALL SITE CONSTRUCTION WILL OCCUR IN ONE PHASE.
- ALL ROOF HEIGHTS SHALL CONFORM TO "HEIGHT ANALYSIS" (ATTACHMENT B) FOR STRUCTURE HEIGHT MEASUREMENT (15-2N-2005) AND ROOF HEIGHT DETAIL ON THIS SHEET.

**FLOOD INFORMATION**

COMMUNITY NUMBER	PANEL NUMBER	SUFFIX	DATE OF FIRM	FIRM ZONE	BASE FLOOD ELEVATION
045012	1310	L	10/18/2013	"A0"	1'

THE LOWEST FLOOR ELEVATIONS AND/OR FLOOD PROOFING ELEVATIONS ON THIS PLAN ARE SUFFICIENTLY HIGH TO PROVIDE PROTECTION FROM FLOODING CAUSED BY A 100-YEAR STORM, AND ARE IN ACCORDANCE WITH SCOTTSDALE REVISED CODE, CHAPTER 37 - FLOODPLAIN AND STORMWATER REGULATIONS.

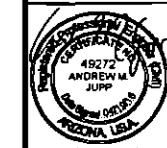


**Kimley-Horn**  
© 2016 KIMLEY-HORN AND ASSOCIATES, INC.  
7740 North 16th Street, Suite 300  
Phoenix, Arizona 85020 (602) 944-5500

SCALE (H): 1"=40'  
SCALE (V): N/A  
DESIGNED BY: KAC  
DRAWN BY: GEX  
CHECKED BY: AMJ  
DATE: 04/19/2016

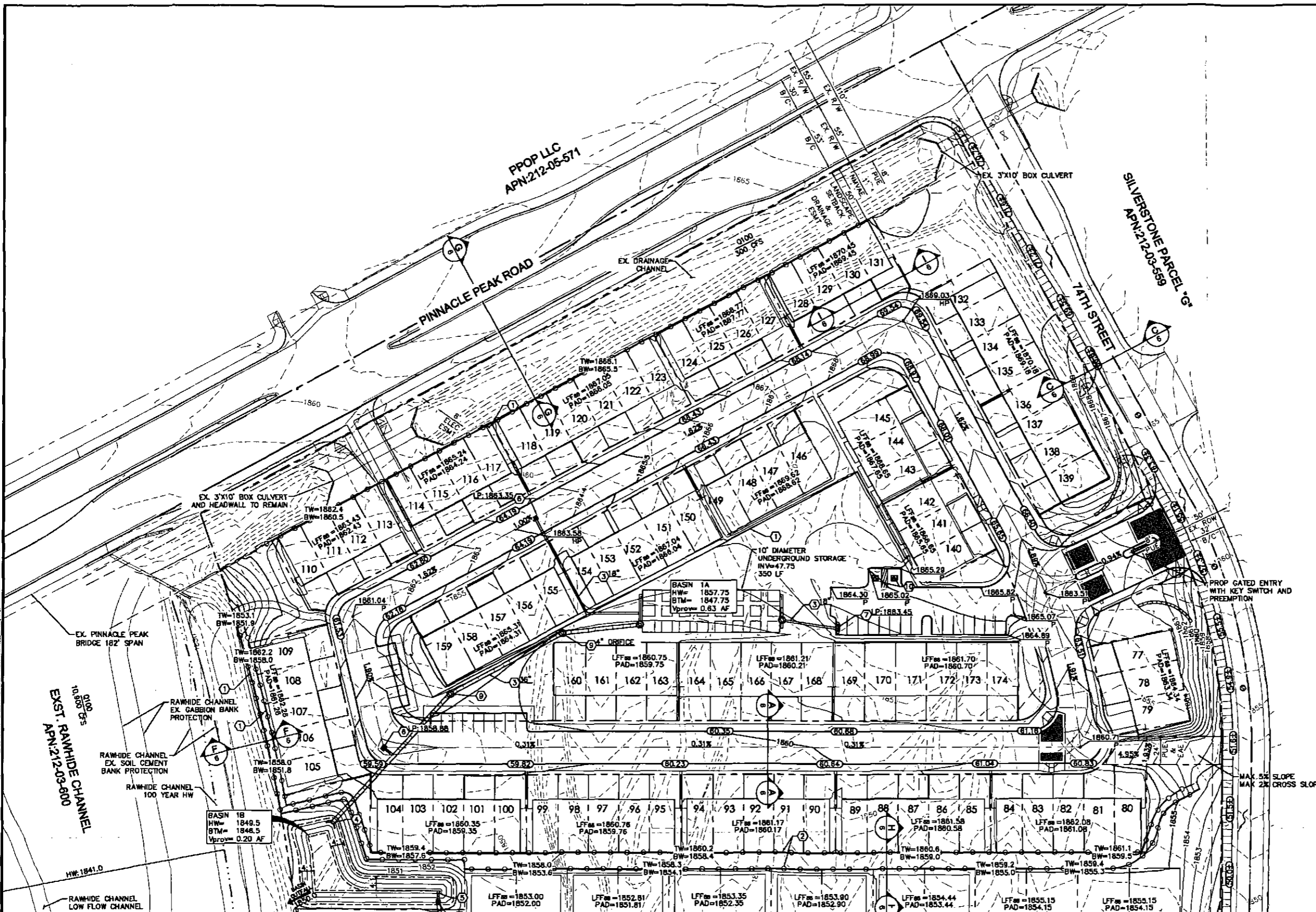
SILVERSTONE PARCEL F  
GRADING AND DRAINAGE  
COVER SHEET  
SCOTTSDALE, ARIZONA

FIGURE 2



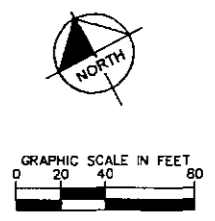
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DRAWING NAME 94800G000

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- CONSTRUCTION NOTES**
- ① PROPOSED RETAINING WALL
  - ② PROPOSED TIERED RETAINING WALL
  - ③ PROPOSED HDPE STORM DRAIN
  - ④ PROPOSED MAG 501 HEADWALL
  - ⑤ PROPOSED MAG 545 FLARED END SECTION
  - ⑥ PROPOSED MAG 535 DROP INLET
  - ⑦ PROPOSED CONTROL STRUCTURE
  - ⑧ PROPOSED MAG 520 STORM DRAIN MAN HOLE
  - ⑨ ADA PARKING STALL MAX 2% IN ANY DIRECTION

NOTES:  
1. REFER TO TABLE ON SHEET 6 FOR GARAGE FLOOR ELEVATIONS



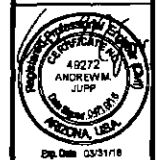
NO.	REVISION	BY	DATE	APPR.

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SCALE (H): 1"=40'  
 SCALE (V): N/A  
 DESIGNED BY: KAC  
 DRAWN BY: CEK  
 CHECKED BY: AMJ  
 DATE: 04/19/2016

**SILVERSTONE PARCEL F  
 GRADING AND DRAINAGE  
 SCOTTSDALE, ARIZONA**

FIGURE 2

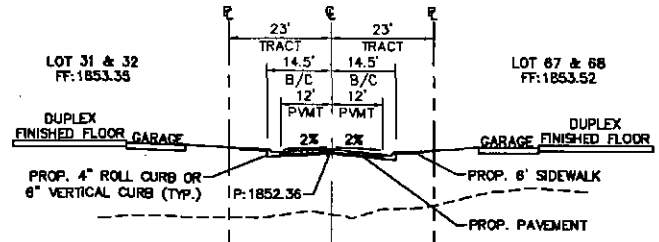
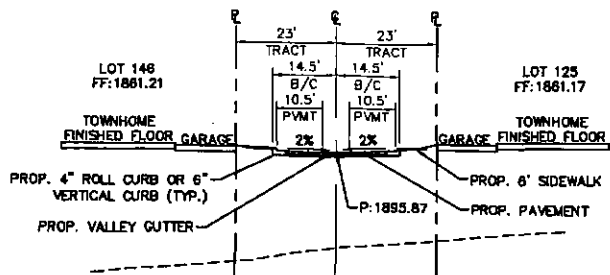


PROJECT NO. 94800GD01  
 DRAWING NAME 94800GD01

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 SHEETS: 1801-1801 (1801-1801) 175-175 (175-175) 1800-1800 (1800-1800) 1800-1800 (1800-1800)  
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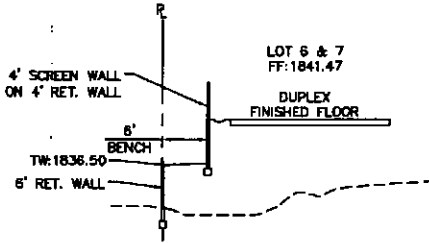
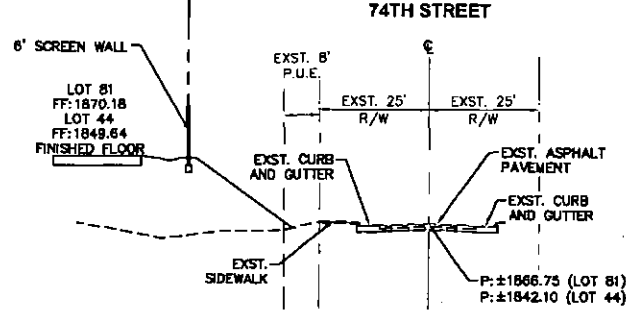






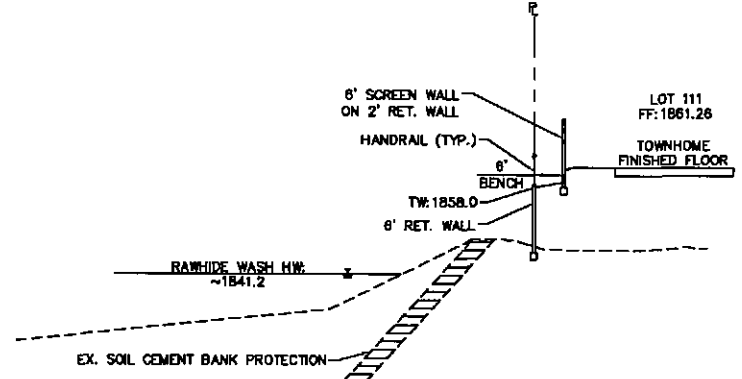
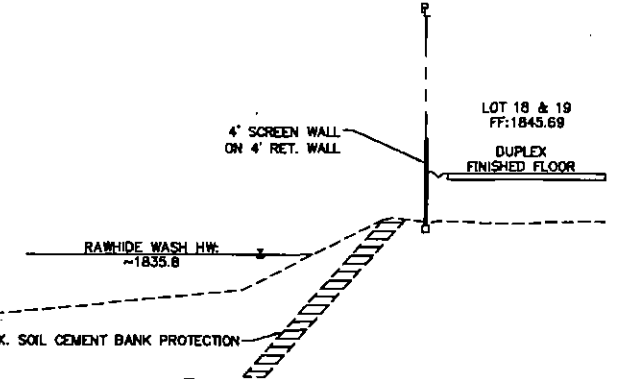
**SECTION A-A**  
HORIZONTAL SCALE: 1"=20'  
VERTICAL SCALE: 1"=10'

**SECTION B-B**  
HORIZONTAL SCALE: 1"=20'  
VERTICAL SCALE: 1"=10'



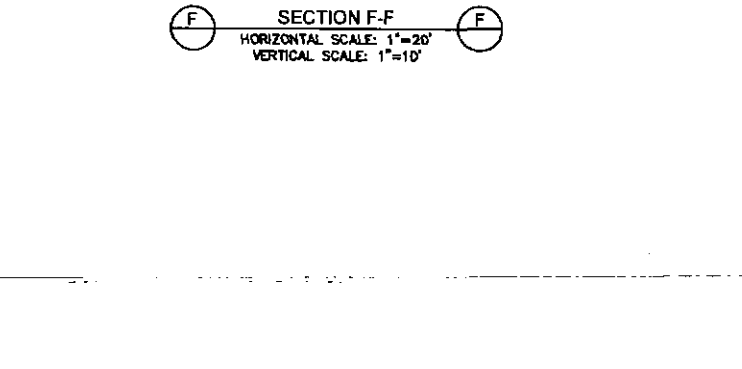
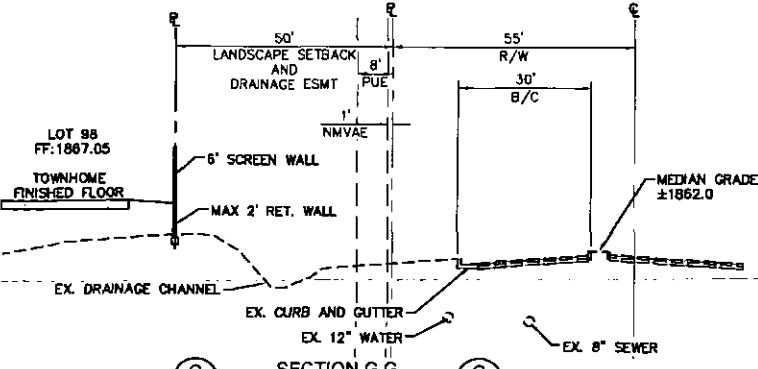
**SECTION C-C**  
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VERTICAL SCALE: 1"=10'

**SECTION D-D**  
HORIZONTAL SCALE: 1"=20'  
VERTICAL SCALE: 1"=10'

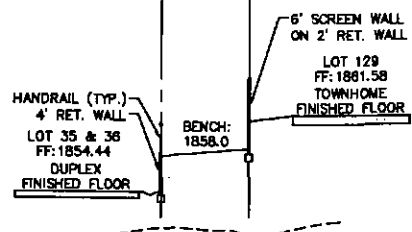


**SECTION E-E**  
HORIZONTAL SCALE: 1"=20'  
VERTICAL SCALE: 1"=10'

**SECTION F-F**  
HORIZONTAL SCALE: 1"=20'  
VERTICAL SCALE: 1"=10'



**SECTION G-G**  
HORIZONTAL SCALE: 1"=20'  
VERTICAL SCALE: 1"=10'



**SECTION H-H**  
HORIZONTAL SCALE: 1"=20'  
VERTICAL SCALE: 1"=10'

K:\VPL\DWG\1948001\CADD\Plan\94800002.dwg Apr 19, 2016 kyc:campbell  
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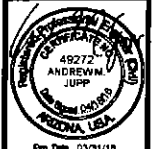
NO.	REVISION	BY	DATE	APPR.

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 SCALE (V): N/A  
 DESIGNED BY: KAC  
 DRAWN BY: CDK  
 CHECKED BY: ANJ  
 DATE: 04/19/2016

SILVERSTONE PARCEL F  
 GRADING AND DRAINAGE  
 DETAILS AND SECTIONS  
 SCOTTSDALE, ARIZONA

FIGURE 2



Exp. Date: 03/31/19  
 PROJECT NO. 94800002  
 DRAWING NAME 94800002

*Appendix F*

Request for Stormwater Storage Waiver



# Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA -      - ZN -      - UP -      - DR -      - PP -      PC#

The applicant/developer must complete and submit this form to the city for processing and obtain approval of waiver request **before submitting improvement plans**. Denial of the waiver may require the developer to submit a revised site plan to the Development Review Board.

Date \_\_\_\_\_ Project Name \_\_\_\_\_  
Project Location \_\_\_\_\_  
Applicant Contact \_\_\_\_\_ Company Name \_\_\_\_\_  
Phone \_\_\_\_\_ Fax \_\_\_\_\_ E-mail \_\_\_\_\_  
Address \_\_\_\_\_

### Waiver Criteria

A project must meet at least one of three criteria listed below for the city to consider waiving some or all required stormwater storage. **However, regardless of the criteria, a waiver will only be granted if the applicant can demonstrate that the effect of a waiver will not increase the potential for flooding on any property.** Check the applicable box and provide a signed engineering report and supporting engineering analysis that demonstrate the project meets the criteria and that the effect of a waiver will not increase the potential for flooding on any property.

If the runoff for the project has been included in a storage facility at another location, the applicant must demonstrate that the stormwater storage facility was specifically designed to accommodate runoff from the subject property and that the runoff will be conveyed to this location through an adequately designed conveyance facility.

- 1. The development is adjacent to a conveyance facility that an engineering analysis shows is designed and constructed to handle the additional runoff from the site as a result of development.
- 2. The development is on a parcel less than one-half acre in size.
- 3. Stormwater storage requirements conflict with requirements of the Environmentally Sensitive Lands Ordinance (ESLO).

For a full storage waiver, a conflict with ESLO is limited to:

- Property located in the hillside landform as defined in the city Zoning Ordinance
- Property in the upper desert landform that has a land slope steeper than 5% as defined in the city Zoning Ordinance
- Property within the ESL zoning overlay district where the only viable location for a stormwater storage basin requires blasting

This full waiver only applies to those portions of property meeting one of these three requirements.

Partial waivers are available for projects or portions of properties within the Environmentally Sensitive Lands Zoning Overlay District, not meeting any of the three full waiver criteria above, if post-development peak discharge rates do not exceed pre-development conditions, based on the 10- and 100-year storm events.

By signing below, I certify that the stated project meets the waiver criteria selected above as demonstrated by the attached documentation.

\_\_\_\_\_  
Engineer

\_\_\_\_\_  
Date

## Planning, Neighborhood & Transportation Division

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781



# Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA - \_\_\_\_\_ - ZN - \_\_\_\_\_ - UP - \_\_\_\_\_ - DR - \_\_\_\_\_ - PP - \_\_\_\_\_ PC# \_\_\_\_\_

## CITY STAFF TO COMPLETE THIS PAGE

Project Name \_\_\_\_\_

### Check Appropriate Boxes:

Meets waiver criteria (specify):  1  2  3

Recommend approve waiver.

Recommend deny waiver:

None of waiver criteria met.

Downstream conditions prohibit waiver of any storage.

Other:

Explain: \_\_\_\_\_  
\_\_\_\_\_

Return waiver request:

Insufficient data provided.

Other: \_\_\_\_\_

Explain: \_\_\_\_\_  
\_\_\_\_\_

### Recommended Conditions of Waiver:

All storage requirements waived.

Post-development peak discharge rates do not exceed pre-development conditions.

Other:

Explain: \_\_\_\_\_  
\_\_\_\_\_

**Waiver approved per above conditions.**

**Waiver denied.**

\_\_\_\_\_  
Floodplain Administrator or Designee

\_\_\_\_\_  
Date

## **Planning, Neighborhood & Transportation Division**

7447 E Indian School Road, Suite 105, Scottsdale, AZ 85251 • Phone: 480-312-2500 • Fax: 480-312-7781



# Request for Stormwater Storage Waiver

City of Scottsdale Case Numbers:

- PA -

- ZN -

- UP -

- DR -

- PP -

PC#

## In-Lieu Fee and In-Kind Contributions

In-lieu fees are only applicable to projects where post-development peak discharge rates exceed pre-development levels, based on the 10- and 100-year storm events. If the city grants a waiver, the developer is required to calculate and contribute an in-lieu fee based on what it would cost the city to provide a storage basin, sized as described below, including costs such as land acquisition, construction, landscaping, design, construction management, and maintenance over a 75-year design life. The fee for this cost is \$1.87 per cubic foot of stormwater storage for a virtual storage basin designed to mitigate the increase in runoff associated with the 100-year/2-hour storm event. The applicant may submit site-specific in-lieu fee calculations subject to the Floodplain Administrator's approval.

The Floodplain Administrator considers in-kind contributions on a case-by-case basis. An in-kind contribution can serve as part of or instead of the calculated in-lieu fee. In-kind contributions must be stormwater related and must constitute a public benefit. In-lieu fees and in-kind contributions are subject to the approval of the Floodplain Administrator or designee.

Project Name \_\_\_\_\_

The waived stormwater storage volume is calculated using a simplified approach as follows:

$V = \Delta CRA$ ; where

V = stormwater storage volume required, in cubic feet,

$\Delta C$  = increase in weighted average runoff coefficient over disturbed area ( $C_{post} - C_{pre}$ ),

R = 100-year/2-hour precipitation depth, in feet (DSPM, Appendix 4-1D, page 11), and

A = area of disturbed ground, in square feet

Furthermore,

R = \_\_\_\_\_

$\Delta C$  = \_\_\_\_\_

$V_w = V - V_p$ ; where

A = \_\_\_\_\_

$V_w$  = volume waived,

V = \_\_\_\_\_

V = volume required, and

$V_p$  = \_\_\_\_\_

$V_p$  = volume provided

$V_w$  = \_\_\_\_\_

An in-lieu fee will be paid, based on the following calculations and supporting documentation:

In-lieu fee (\$) =  $V_w$  (cu. ft.) x \$1.87 per cubic foot = \_\_\_\_\_

An in-kind contribution will be made, as follows:

\_\_\_\_\_  
\_\_\_\_\_

No in-lieu fee is required. Reason:

\_\_\_\_\_  
\_\_\_\_\_

Approved by:

\_\_\_\_\_  
Floodplain Administrator or Designee

\_\_\_\_\_  
Date

## Planning, Neighborhood & Transportation Division

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