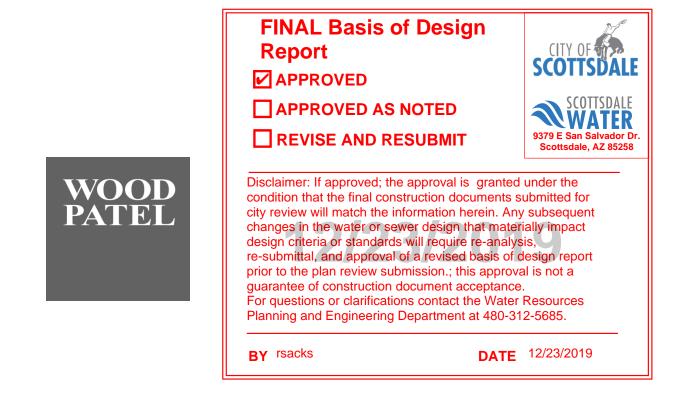


Water and Wastewater Study Combined



WATER DISTRIBUTION SYSTEM BASIS OF DESIGN REPORT FOR RAINTREE PHASE III RESIDENTIAL BUILDING

December 4, 2019 WP# 195063



EXPIRES 03-31-22



2051 W Northern Ave #100 Phoenix AZ 85021 P: 602.335.8500 F: 602.335.8580 www.woodpatel.com

Darrel E. Wood, PE, RLS Ashok C. Patel, PE, RLS, CFM Michael T. Young, PE, LEED AP James S. Campbell, PE, LEED GA Thomas R. Gettings, RLS Darin L. Moore, PE, LEED GA Jeffrey R. Minch, PE, CFM Robert D. Gofonia, PE, RLS Nicholas E. Brown, PE December 4, 2019

Mr. Levi Dillion, PE Senior Water Resources Engineer City of Scottsdale 9379 East San Salvador Drive Scottsdale, Arizona 85258

480.312.5319 Idillion@scottsdaleaz.gov

Re: Raintree Phase III Residential Building

Water Distribution System Basis of Design Report WP# 195063

Mr. Dillion:

This Water Distribution System Basis of Design Report is prepared for Trammell Crow and submitted to the City of Scottsdale. The proposed Raintree Phase III Residential Building (Site) is a 3.12-acre site, located at the southeast corner of North Northsight Boulevard and East Raintree Drive. More specifically, the Site is located in the southwest quarter of Section 12, Township 3 North, Range 4 East of the Gila and Salt River Meridian. Refer to the *Vicinity Map* at the back of this report for project location. The Raintree Phase III Residential Building is a proposed 5-story, 190-room residential building with 5.5-story parking garage. The project is planned to be constructed with associated paving, utility, hardscape, and landscape improvements.

The Site is surrounded by public waterline infrastructure which forms a looped waterline system around the Site. There is one (1) 12-inch C900 water main in Northsight Boulevard. There is one (1) 20-inch SCP and one (1) 12-inch ACP water main in Raintree Drive. There is one (1) 10-inch C900 water main in 87th Street. There is one (1) 8-inch DIP public waterline in the existing Raintree Private Drive. The Raintree Phase III site proposes three (3) connections to the public 8-inch water main in the Raintree Private Drive to service domestic, irrigation and fire demands. Refer to the attached *Water Exhibit* for a depiction of the existing water infrastructure surrounding the Site.

The design criteria used to estimate potable water demands and evaluate system hydraulics are based on Wood, Patel & Associates, Inc.'s (WOODPATEL) understanding of the requirements listed in the *City of Scottsdale Design Standards and Policies Manual*, 2018. The following is a summary of the primary design criteria utilized:

• Average Day Water Demand, High Density Condominium :	0.27 gpm/unit*
Fire Flow Requirements:	min 1,500 gpm**
Maximum Day Demand:	2.0 x ADD
Peak Hour Demand:	3.5 x ADD
Minimum Residual Pressure, Peak Hour:	50 psi
• Minimum Residual Pressure, Maximum Day + Fire Flow:	30 psi
Maximum System Pressure:	120 psi

•	Maximum Pipe Head Loss, Maximum Day Demand:	
٠	Maximum Pipe Head Loss, Peak Hour Demand:	10 ft / 1000 ft
٠	Minimum Pipe Diameter, Public Water Line	8 inches

Abbreviations: gpd = gallons per day; sf = square feet; ADD = average day demand; psi = pounds per square inch *Includes both inside and outside use per Figure 6-1.2, COS Design Standards & Policies Manual

**Fire flow is based on 10% reduction to account for flow measurement inaccuracy (Refer to attached *Calculations* in the appendices)

Potable water service for the proposed Site will be provided by one (1) 3-inch domestic turbo water service, equipped with a 3-inch backflow prevention device and meter, connecting to an existing 3-inch stub on the east side of the Site. In addition, a 1-inch irrigation service, equipped with a 1-inch backflow prevention device and meter, is to be reused in the northeast corner. Refer to the attached *Water Exhibit* for a depiction of the proposed water infrastructure.

Fire protection for the proposed Site will be provided though one (1) 8-inch fire sprinkler service, connecting to the 8-inch public waterline on the south side of the Site and a proposed remote fire department connection. In addition, there are several existing fire hydrants within the Site vicinity. One (1) fire hydrant will be relocated. Refer to the attached *Water Exhibit* for proposed fire hydrant and fire sprinkler service locations.

The design criteria used to estimate water demands and evaluate system hydraulics are based on Wood, Patel & Associates, Inc.'s (WOODPATEL) understanding of locally-accepted design criteria, and the requirements listed in the City of Scottsdale Standards.

The average-day water demand for the Site is projected to be approximately 73,872 gallons per day (gpd), or 51 gallons per minute (gpm). The projected max day demand is 103 gpm. The projected peak demand is 180 gpm (Refer to the attached *Calculation and Hydraulic Modeling Results*).

WaterCAD Version 8i, by Haestad Methods, was utilized to analyze the existing water distribution system and proposed improvements. The hydraulic model was calibrated using data provided from a fire hydrant flow test, performed by the Arizona Flow Testing, LLC., on November 8th, 2019 (Refer to attached *Fire Hydrant Flow Test*).

The hydraulic modeling results indicate the proposed system is capable of delivering peak demands of approximately 180 gpm to the Site, with pressures ranging from 72 -74 psi. Fire-flow results during Max Day demand indicate the required fire flows (1,500 gpm) are available within the proposed development at all existing and proposed fire hydrants, with system residual pressures at or above 30 psi, with the minimum fire flow at 2,704 gpm. It is important to add, a 75-percent reduction was applied to the fire flow requirements, due to proposed sprinkler system. Hydraulic-modeling results, calculations, and exhibits involved in the water system analysis are attached.

Thank you for your review of the Water Distribution System Basis of Design Report provided for the Raintree Phase III Residential Building. Feel free to contact me if you have any questions.

Wood, Patel & Associates, Inc.



EXPIRES 03-31-22

Anthony J. Bueche, PE Project Manager

Attachments:

Calculations and Hydraulic Modeling Results Vicinity Map Water System Exhibit Fire Flow Test Results

AJB/se

Y:\WP\Reports\Commercial\195063 Raintree Phase III Residential Building Water BOD.docx WOODPATEL Raintree Phase III Residential Building

December 4, 2019 Page 2 of 2 19-ZN-2019 12/13/2019 CALCULATIONS AND HYDRAULIC MODELING RESULTS



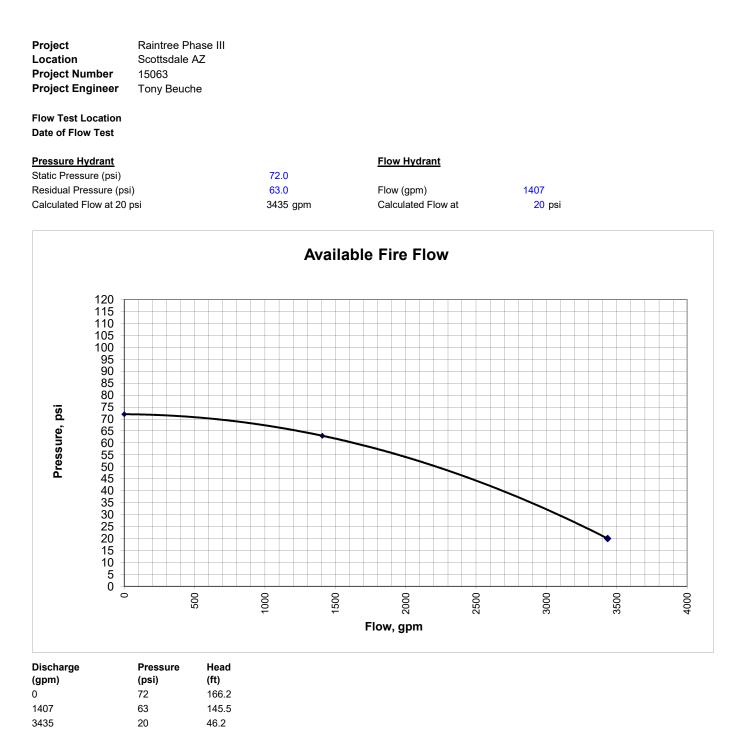
TABLE 1 WATER DEMAND DESIGN FLOWS

Project	Raintree Phase III
Location	Scottsdale AZ
Project Number	15063
Project Engineer	Tony Beuche
References	City of Scottsdale Design Standards and Policies Manual
Project Number Project Engineer	15063 Tony Beuche

LAND USE AND DW	AND USE AND DWELLING UNIT BREAKDOWN BY JUNCTION													
HYDRAULIC ELEVATION		EVATION LAND USE		NG DEMAND VALUE UNITS		AILY DEMAND MAX DAY FLO		X DAY FLOW		PEAK FLOW				
MODEL NODE (ff	(ft)	UNI"	UNITS	DEMAND VALUE	UNITS	(gpm)	(gpd)	Total (gpm)	(gpm)	(gpd)	Total (gpm)	(gpm)	(gpd)	Total (gpm)
J-DOM	1,462.00	High Density Condominium	190	0.27	gpm/Unit	51.30	73,872.00	51.30	102.60	147,744.00	102.60	179.55	258,552.00	179.55
Total			190			51	73,872	51	103	147,744	103	180	258,552	180



EXISTING WATER SYSTEM PRESSURES



Notes

1. Values provided from a flow test by Arizona Fire Flow Testing LLC.

FlexTable: Pipe Table

Label	Diameter (in)	Material	Start Node	Stop Node	Hazen-Williams C	Flow (gpm)	Velocity (ft/s)
P-1	8.0	Ductile Iron	FH-TEST	J-DOM	130.0	0	0.00
P-2	8.0	Ductile Iron	J-DOM	FH-FLOW	130.0	0	0.00
P-4	8.0	Ductile Iron	J-FIRE	J-1	130.0	0	0.00
P-5	12.0	PVC	J-1	J-2	150.0	0	0.00
P-6	12.0	Asbestos Cement	J-2	FH-TEST	140.0	0	0.00
P-PMP	48.0	Ductile Iron	PMP-1	FH-TEST	130.0	0	0.00
P-RES	48.0	Ductile Iron	RES-1	PMP-1	130.0	0	0.00

Active Scenario: Calibration - Static

5063-WaterCAD.wtg 12/4/2019 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 WaterCAD CONNECT Edition Update 1 [10.01.00.72] Page 1 of 1

FlexTable: Junction Table

Active Scenario: Calibration - Static

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
FH-FLOW	1,458.50	0	1,629.20	74
FH-TEST	1,463.00	0	1,629.20	72
J-1	1,460.00	0	1,629.20	73
J-2	1,460.00	0	1,629.20	73
J-DOM	1,462.00	0	1,629.20	72
J-FIRE	1,459.00	0	1,629.20	74

5063-WaterCAD.wtg 12/4/2019 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

FlexTable: Pipe Table

Label	Diameter (in)	Material	Start Node	Stop Node	Hazen-Williams C	Flow (gpm)	Velocity (ft/s)
P-1	8.0	Ductile Iron	FH-TEST	J-DOM	130.0	1,407	8.98
P-2	8.0	Ductile Iron	J-DOM	FH-FLOW	130.0	1,407	8.98
P-4	8.0	Ductile Iron	J-FIRE	J-1	130.0	0	0.00
P-5	12.0	PVC	J-1	J-2	150.0	0	0.00
P-6	12.0	Asbestos Cement	J-2	FH-TEST	140.0	0	0.00
P-PMP	48.0	Ductile Iron	PMP-1	FH-TEST	130.0	1,407	0.25
P-RES	48.0	Ductile Iron	RES-1	PMP-1	130.0	1,407	0.25

Active Scenario: Calibration - Flow

5063-WaterCAD.wtg 12/4/2019 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 WaterCAD CONNECT Edition Update 1 [10.01.00.72] Page 1 of 1

FlexTable: Junction Table

Active Scenario: Calibration - Flow

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
FH-FLOW	1,458.50	1,407	1,595.03	59
FH-TEST	1,463.00	0	1,608.50	63
J-1	1,460.00	0	1,608.50	64
J-2	1,460.00	0	1,608.50	64
J-DOM	1,462.00	0	1,604.34	62
J-FIRE	1,459.00	0	1,608.50	65

5063-WaterCAD.wtg 12/4/2019 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

FlexTable: Pipe Table

Label	Diameter (in)	Material	Start Node	Stop Node	Hazen-Williams C	Flow (gpm)	Velocity (ft/s)
P-1	8.0	Ductile Iron	FH-TEST	J-DOM	130.0	3,435	21.92
P-2	8.0	Ductile Iron	J-DOM	FH-FLOW	130.0	3,435	21.92
P-4	8.0	Ductile Iron	J-FIRE	J-1	130.0	0	0.00
P-5	12.0	PVC	J-1	J-2	150.0	0	0.00
P-6	12.0	Asbestos Cement	J-2	FH-TEST	140.0	0	0.00
P-PMP	48.0	Ductile Iron	PMP-1	FH-TEST	130.0	3,435	0.61
P-RES	48.0	Ductile Iron	RES-1	PMP-1	130.0	3,435	0.61

Active Scenario: Calibration - Max

5063-WaterCAD.wtg 12/4/2019 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

FlexTable: Junction Table

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
FH-FLOW	1,458.50	3,435	1,438.84	-9
FH-TEST	1,463.00	0	1,509.20	20
J-1	1,460.00	0	1,509.20	21
J-2	1,460.00	0	1,509.20	21
J-DOM	1,462.00	0	1,487.47	11
J-FIRE	1,459.00	0	1,509.20	22

Active Scenario: Calibration - Max

5063-WaterCAD.wtg 12/4/2019 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

FlexTable: Pipe Table

Label	Diameter (in)	Material	Start Node	Stop Node	Hazen-Williams	Flow (gpm)	Velocity (ft/s)
				1.5014	<u> </u>		
P-1	8.0	Ductile Iron	FH-TEST	J-DOM	130.0	40	0.26
P-2	8.0	Ductile Iron	J-DOM	FH-FLOW	130.0	-11	0.07
P-3	8.0	Ductile Iron	FH-FLOW	J-FIRE	130.0	-11	0.07
P-4	8.0	Ductile Iron	J-FIRE	J-1	130.0	-11	0.07
P-5	12.0	PVC	J-1	J-2	150.0	-11	0.03
P-6	12.0	Asbestos Cement	J-2	FH-TEST	140.0	-11	0.03
P-PMP	48.0	Ductile Iron	PMP-1	FH-TEST	130.0	51	0.01
P-RES	48.0	Ductile Iron	RES-1	PMP-1	130.0	51	0.01

Active Scenario: Average Day Demand

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FlexTable: Junction Table

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
FH-FLOW	1,458.50	0	1,629.16	74
FH-TEST	1,463.00	0	1,629.17	72
J-1	1,460.00	0	1,629.17	73
J-2	1,460.00	0	1,629.17	73
J-DOM	1,462.00	51	1,629.16	72
J-FIRE	1,459.00	0	1,629.17	74

Active Scenario: Average Day Demand

5063-WaterCAD.wtg 12/4/2019 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

FlexTable: Pipe Table

Label	Diameter (in)	Material	Start Node	Stop Node	Hazen-Williams C	Flow (gpm)	Velocity (ft/s)
P-1	8.0	Ductile Iron	FH-TEST	J-DOM	130.0	141	0.90
P-2	8.0	Ductile Iron	J-DOM	FH-FLOW	130.0	-39	0.25
P-3	8.0	Ductile Iron	FH-FLOW	J-FIRE	130.0	-39	0.25
P-4	8.0	Ductile Iron	J-FIRE	J-1	130.0	-39	0.25
P-5	12.0	PVC	J-1	J-2	150.0	-39	0.11
P-6	12.0	Asbestos Cement	J-2	FH-TEST	140.0	-39	0.11
P-PMP	48.0	Ductile Iron	PMP-1	FH-TEST	130.0	180	0.03
P-RES	48.0	Ductile Iron	RES-1	PMP-1	130.0	180	0.03

Active Scenario: Peak Hour

5063-WaterCAD.wtg 12/4/2019 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

FlexTable: Junction Table

Label	Elevation (ft)	Demand (gpm)	Hydraulic Grade (ft)	Pressure (psi)
FH-FLOW	1,458.50	0	1,628.79	74
FH-TEST	1,463.00	0	1,628.84	72
J-1	1,460.00	0	1,628.83	73
J-2	1,460.00	0	1,628.84	73
J-DOM	1,462.00	180	1,628.78	72
J-FIRE	1,459.00	0	1,628.79	73

Active Scenario: Peak Hour

5063-WaterCAD.wtg 12/4/2019 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

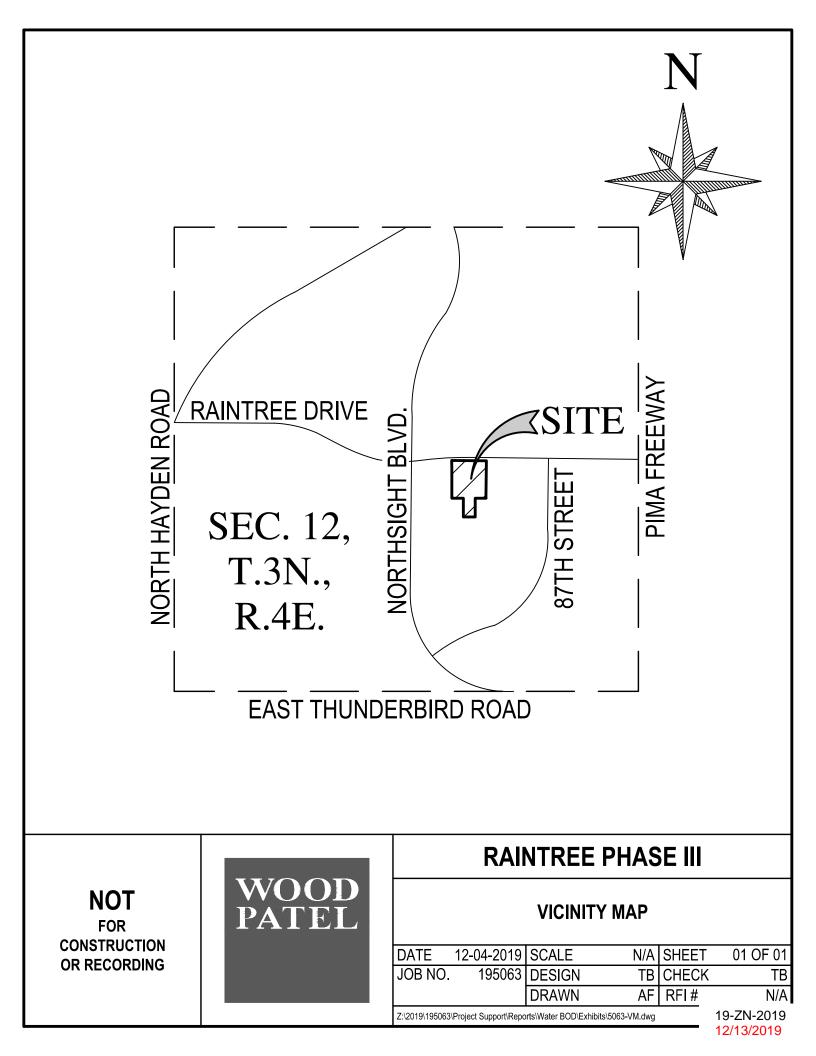
Fire Flow Node FlexTable: Fire Flow Report

					-	
Label	Elevation (ft)	Flow (Total Needed) (gpm)	Flow (Total Available) (gpm)	Pressure (Residual Lower Limit) (psi)	Pressure (Calculated Residual) (psi)	Hydraulic Grade (ft)
FH-FLOW	1,458.50	1,500	2,719	30	30	1,629.06
FH-TEST	1,463.00	1,500	2,978	30	30	1,629.08
J-1	1,460.00	1,500	2,828	30	30	1,629.08
J-2	1,460.00	1,500	2,895	30	30	1,629.08
J-DOM	1,462.00	1,603	2,927	30	30	1,629.06
J-FIRE	1,459.00	1,500	2,704	30	30	1,629.06

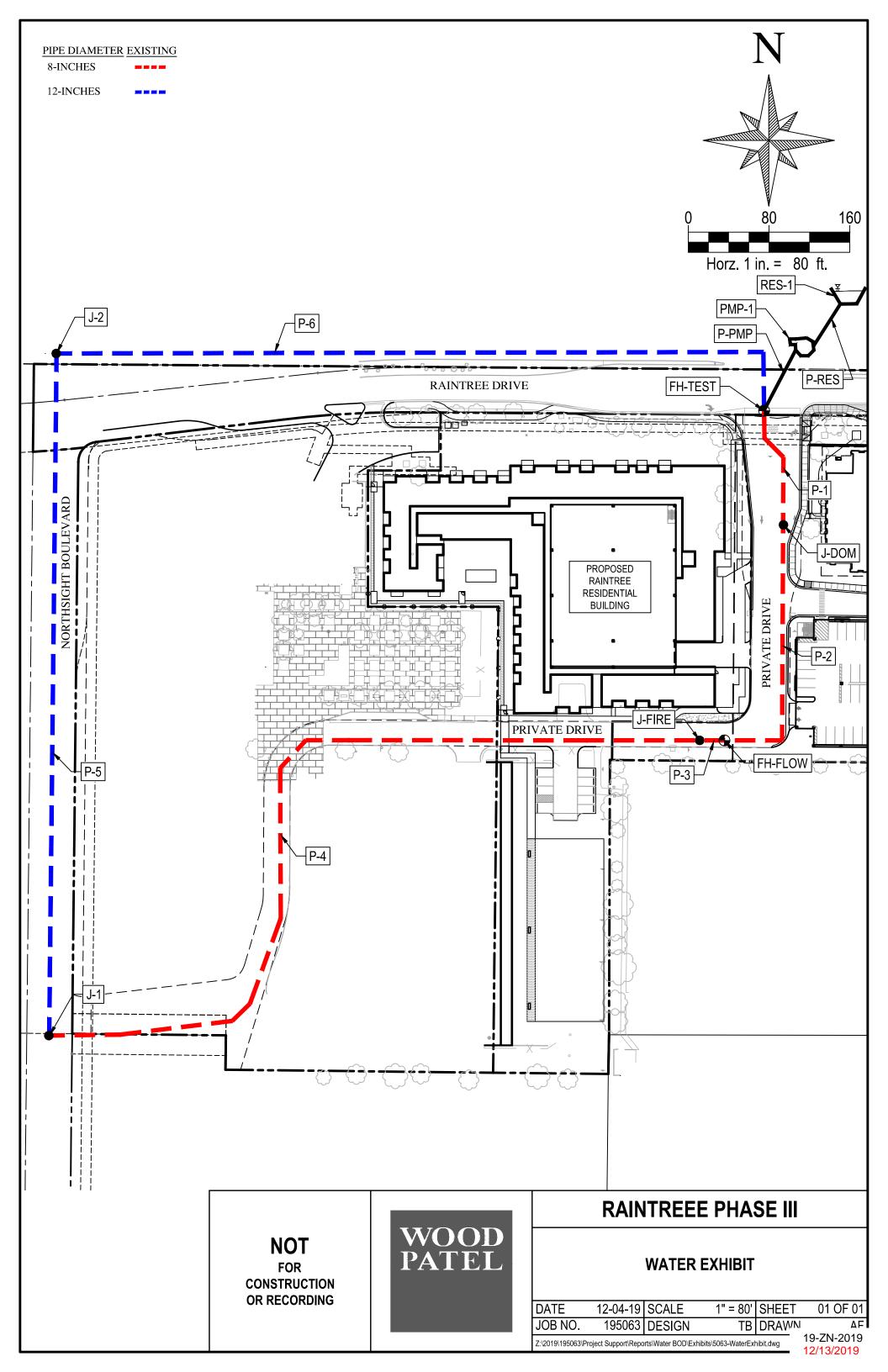
Active Scenario: Fire Flow + Max Day

5063-WaterCAD.wtg 12/4/2019 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 WaterCAD CONNECT Edition Update 1 [10.01.00.72] Page 1 of 1

VICINITY MAP



WATER SYSTEM EXHIBIT



FIRE FLOW TEST RESULTS

Arizona Flow Testing LLC

HYDRANT FLOW TEST REPORT

Project Name: Project Address: Arizona Flow Testing Client Project No.: Flow Test Permit No Date and time flow to Data is current and r Conducted by: Witnessed by:	g Project No.: 1 N :: Cu est conducted: N eliable until: M Fi	7 th Street and 9414 fot Provided 60265 lovember 8, 2 1ay 8, 2020 loyd Vaughar	e 3 Residential Building l Raintree Drive, Scottsdal 2019 at 8:00 AM n – Arizona Flow Testing, I City of Scottsdale-Inspecto	LC (480-250-815	
Raw Test Data			Data with 10 PSI Safet	y Factor	
Static Pressure: (Measured in pounds	82.0 PSI s per square inch)		Static Pressure: (Measured in pounds p	72.0 PSI er square inch)	Scottsdale requires a
Residual Pressure: (Measured in pounds	73.0 PSI s per square inch)		Residual Pressure: (Measured in pounds p	63.0 PSI er square inch)	maximum Static Pressure of 72 PSI for AFES Design.
Pitot Pressure: (Measured in pounds	14.0 PSI s per square inch)		Distance between hydra	ants: Approx. 330	Feet
Diffuser Orifice Diameter: 4- Inch Hose Mon (Measured in inches)		ister	Main size: Not Provided		
Coefficient of Diffuse	r: .7875				
Flowing GPM: (Measured in gallons	1,407 (s per minute)	GPM	Flowing GPM:	1,407	GPM
GPM @ 20 PSI:	3,989 (GPM	GPM @ 20 PSI:	3,627	GPM
Flow Test Location		North	1		
+	Untitled Map			Legend	
Pressure Fire Hydrant				E	ast Raintree Drive
Project Site 87th Street and Raintree Drive					North 87th Street
Flow Fire Hydrant					Private Drive
		. 566576	aandattadhadaa 🗐 🔚 🐻 alaaqaadh 🕷		

Arizona Flow Testing LLC 480-250-8154 www.azflowtest.com floyd@azflowtest.com



WASTEWATER COLLECTION SYSTEM BASIS OF DESIGN REPORT FOR RAINTREE PHASE III RESIDENTIAL BUILDING

December 4, 2019 WP# 195063





EXPIRES 03-31-22



2051 W Northern Ave #100 Phoenix AZ 85021 P: 602.335.8500 F: 602.335.8580 www.woodpatel.com

Darrel E. Wood, PE, RLS Ashok C. Patel, PE, RLS, CFM Michael T. Young, PE, LEED AP James S. Campbell, PE, LEED GA Thomas R. Gettings, RLS Darin L. Moore, PE, LEED GA Jeffrey R. Minch, PE, CFM Robert D. Gofonia, PE, RLS Nicholas E. Brown, PE December 4, 2019

Mr. Levi Dillion, PE Senior Water Resources Engineer City of Scottsdale 9379 East San Salvador Drive Scottsdale, Arizona 85258

480.312.5319 Idillion@scottsdaleaz.gov

Re: Raintree Phase III Residential Building Wastewater Collection System Basis of Design Report WP# 195063

Mr. Dillion:

This Wastewater Collection System Basis of Design Report is prepared for Trammell Crow Company and submitted to the City of Scottsdale in support of the application for a minor General Plan Amendment and rezoning of the property located in the southeast quadrant of Raintree Drive and north Northsight Boulevard.

Phase I consists of the existing office building and improvements on the west side of the property. Phase II consists of an approved office building, parking garage and related improvements to be constructed on the east side of the property.

The proposed Raintree Phase III Residential Building (Site) is a 3.12-acre site, located at the southeast corner of north Northsight Boulevard and east Raintree Drive. More specifically, the Site is located in the southwest quarter of Section 12, Township 3 North, Range 4 East of the Gila and Salt River Meridian. Refer to the *Vicinity Map* at the back of this report for project location. The Raintree Phase III Residential Building is a proposed 5-story, 190-room residential building with 5.5-story parking garage. The project is planned to be constructed with associated paving, utility, hardscape, and landscape improvements.

Wastewater flowing from the proposed Site will discharge to an existing 8-inch stub into the 8-inch public sewer line located in the Site's private drive. The 8-inch public sewer line then discharges into the existing public 8-inch VCP sewer line in Raintree Drive. Refer to the attached *Wastewater Exhibit* for a depiction of the existing sewer infrastructure surrounding the Site.

The design criteria used to estimate wastewater flows and evaluate system hydraulics are based on Wood, Patel & Associates, Inc.'s (WOODPATEL) understanding of the requirements listed in the City of Scottsdale Design Standards and Policies Manual, 2018. The following is a summary of the primary design criteria utilized:

• • • . Minimum Peak Flow d/D Ratio (12-inch diameter or less sewers):d/D = 0.65 Minimum Peak Flow d/D Ratio (12-inch diameter or less sewers):d/D = 0.65

Abbreviations: gpd = gallons per day; fps = feet per second

Based on the above design criteria, the projected average day flow for the proposed project is approximately 26,600 gallons per day (gpd). The peak flow is projected to be 119,700 gpd. The proposed sewer slopes, projected flow velocities, and pipe flow capacities are summarized on the attached spreadsheets.

It is assumed the infiltration and inflow from wet weather has been accounted for in the published design flow rates for the development and the maximum d/D. Therefore, those flows have not been added into the calculations. The proposed sanitary sewer collection system is designed to have adequate capacity to serve the proposed development.

Thank you for your review of the Wastewater Collection System Basis of Design Report provided for the Raintree Phase III Residential Building. Feel free to contact me if you have any questions.

Sincerely,

Wood, Patel & Associates, Inc.



EXPIRES 03-31-22

Anthony J. Beuché, PE **Project Manager**

Attachments:

Wastewater Design Flows & Flowmaster Results Vicinity Map Wastewater Exhibit

AJB/se

Y:\WP\Reports\Commercial\195063 Raintree Phase III Residential Building Wastewater BOD.docx

December 4, 2019 Daga 2 of 2 19-ZN-2019 12/13/2019

WASTEWATER DESIGN FLOWS & FLOWMASTER RESULTS



TABLE 1 WASTEWATER MODEL, FULL BUILD-OUT CONDITION

Project	Raintree Phase III
Location	Scottsdale AZ
Project Number	15063
Project Engineer	Tony Beuche
References	City of Scottsdale Design Standards and Policies Manual
	Arizona Administrative Code, Title 18, Chapter 9

FROM NODE	TO NODE	Multi-Family Residential (DU)	DEMAND VALUE (gpd/DU)	SEWER NODE ADD (gpd)	TOTAL ADD (gpd)	POPULATION	PEAKING FACTOR	PEAK FLOW (gpd)	PEAK FLOW (gpm)
Outfall 1 Priavte D 1	rive 2	190	140	26,600	26,600	266.0	4.5	119,700	83
Total Outfall 1		190		26,600	26,600	266.0	4.5	119,700	83
				-				1	
Total Proposed Flo	ow	190		26,600	26,600	266	4.5	119,700	83

Fickion Method Manning Formula Solve For Normal Depth Input Data 0.013 Roughness Coefficient 0.08000 f/ft Diameter 8.00 in Discharge 119700.0 gal/day Results 12.27 in Flow Area 0.04 ft ² 14970.0 Yetted Perimeter 0.55 ft Yetted Perimeter 0.55 ft Yoy With 0.49 ft Top Width 0.20 ft Percent Full 15.8 % Critical Stope 0.0004 ft ⁴ Velocity 5.23 ft/s Velocity Head 0.42 ft Specific Energy 0.53 ft Froude Number 3.68 ft ⁷ /s Stopa Full 0.0002 ft Velocity Head 4.2 ft Specific Energy 0.58 ft Stope Full 0.000 ft Oxotar Burbeh 0.00 ft Rowing Scharge Full 0.000 ft Stope Full 0.00 ft Length 0.00 ft <th></th> <th>8-Inch a</th> <th>t Peak Fl</th> <th>ow</th>		8-Inch a	t Peak Fl	ow
Solve For Normal Depth Input Data 0.013 Channel Stope 0.08000 fvff Diameter 8.00 in Discharge 11970.00 galday Results	Project Description			
Solve For Normal Depth Input Data 0.013 Channel Stope 0.08000 fvff Diameter 8.00 in Discharge 11970.00 galday Results	Friction Method	Manning Formula		
Rughness Coefficient 0.013 Channel Slope 0.08000 ft/ft Diarder 8.00 in Discharge 11970.00 gal/day Results	Solve For	Normal Depth		
Channel Stope 0.08000 ftft Diacharge 11970.00 gal/day Results Resulta Resulta 0.04 R¹/27 n Flow Area 0.04 R ² Wetted Perimeter 0.55 ft Hydraulic Radius 0.78 in Top Width 0.49 ft Vetted Perimeter 0.55 ft Hydraulic Radius 0.78 in Top Width 0.49 ft Vetted Perimeter 0.55 ft Hydraulic Radius 0.78 in Top Width 0.49 ft Optiduth 0.20 ft Percent Full 15.8 % Critical Stope 0.00641 ft/ft Velocity Head 0.42 ft Specific Energy 0.53 ft Froude Number 3.42 ft/s Discharge Full 0.424 ft Nom Top SuperCritical	Input Data			
Diameter 8.0 in Discharge 119700.00 gal/day Results	Roughness Coefficient		0.013	
Bisharge 11970.00 gal/day Results	Channel Slope		0.08000	ft/ft
Results Normal Depth 1.27 in Flow Area 0.04 ft ² Wetted Perimeter 0.55 ft Hydraulic Radius 0.78 in Top Width 0.49 ft Critical Depth 0.20 ft Percent Full 15.8 % Critical Slope 0.00641 ft/ft Velocity 5.23 ft/s Velocity Head 0.42 ft Specific Energy 0.53 ft Froude Number 3.41 Maximu Discharge 3.68 ft%s Discharge Full 0.0002 ft Specific Energy 0.53 ft Froude Number 3.41 Maximu Discharge 3.68 ft%s Discharge Full 0.0002 ft Fow Type SuperCritical EVF Input Data 1 Downstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 Profile Description 1 Profile Description 1 Profile Description 0.00 ft Average End Depth	Diameter		8.00	in
Normal Depth 1.27 in Flow Area 0.04 ft ² Wetted Perimeter 0.55 ft Hydraulic Radius 0.78 in Top Width 0.49 ft Critical Depth 0.20 ft Percent Full 15.8 % Critical Slope 0.00641 ft/ft Velocity Head 0.42 ft Specific Energy 0.53 ft Froude Number 3.41 Maximum Discharge Slope Full 0.00023 ft/ft Flow Type SuperCritical It Flow Type SuperCritical It Ownstream Depth 0.00 in Length 0.00 ft Number Of Steps 0 It Profile Description It It Profile Description It It Profile Description It It Profile Description It It Normal Depth Over Rise 0.00<	Discharge		119700.00	gal/day
Flow Area 0.04 ft Wetted Perimeter 0.55 ft Hydraulic Radius 0.78 in Top Width 0.49 ft Critical Depth 0.20 ft Percent Full 15.8 % Critical Slope 0.00641 ft/ft Velocity 52.3 ft/s Velocity Head 0.42 ft Specific Energy 0.53 ft Froude Number 3.41 - Maximum Discharge 3.68 ft/s Discharge Full 0.00023 ft/ft Flow Type SuperCritical ft/ft Pownstream Depth 0.00 ift Length 0.00 ift Number Of Steps 0 ift Pofile Description - - Profile Headloss 0.00 ift Average End Depth Over Rise 0.00 %	Results			
Weted Perimeter 0.55 ft Hydraulic Radius 0.78 in Top Width 0.49 ft Critical Depth 0.20 ft Percent Full 15.8 % Critical Slope 0.00641 ft/ft Velocity 5.23 ft/s Velocity 5.23 ft/s Velocity Head 0.42 ft Specific Energy 0.53 ft Froude Number 3.41 T Discharge Full 3.42 ft/s Discharge Full 0.00023 ft/ft Flow Type Super Critical 0.00023 Flow Type Super Critical 0.00 Flow Type Super Critical in Length 0.00 ft Number Of Steps 0 ft Velocity 0.00 ft Profile Description 0.00 ft Profile Headloss 0.00 ft Average End Depth Over Rise 0.00	Normal Depth		1.27	in
Index 0.78 in Top Width 0.49 ft Critical Depth 0.20 ft Percent Full 15.8 % Critical Slope 0.00641 ft/ft Velocity 5.23 ft/s Velocity Head 0.42 ft Specific Energy 0.53 ft Froude Number 3.41	Flow Area		0.04	ft²
Top Width 0.49 ft Critical Depth 0.20 ft Percent Full 15.8 % Critical Slope 0.00641 ft/ft Velocity 5.23 ft/s Velocity Head 0.42 ft Specific Energy 0.53 ft Froude Number 3.41 Maximum Discharge 3.68 ft/s Discharge Full 3.42 ft/s Slope Full 0.00023 ft/ft Flow Type SuperCritical ft Downstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 ft CVF Output Data 0 ft Porfile Description 0 ft Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 %	Wetted Perimeter		0.55	ft
Critical Depth 0.20 f Percent Full 15.8 % Critical Slope 0.00641 ft/f Velocity 5.23 ft/s Velocity Head 0.42 f Specific Energy 0.53 ft Froude Number 3.41 - Maximum Discharge 3.68 ft/s Discharge Full 3.42 ft/s Slope Full 0.0023 ftf Flow Type SuperCritical ft/s Downstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 ft Profile Description - - Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 ft Normal Depth Over Rise 0.00 ft	Hydraulic Radius		0.78	in
Percent Full 15.8 % Critical Stope 0.00641 fr/ft Velocity 5.23 fr/s Velocity Head 0.42 ft Specific Energy 0.53 ft Froude Number 3.41 Maximum Discharge 3.68 ft?s Discharge Full 3.42 ft?s Stope Full 0.00023 ft/ft Flow Type SuperCritical ft Pownstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 ft Profile Description 0.00 ft Profile Description 0.00 ft Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 %	Top Width		0.49	ft
Critical Slope 0.00641 ft/ft Velocity 6.23 ft/s Velocity Head 0.42 ft Specific Energy 0.53 ft Froude Number 3.41 Maximum Discharge 3.68 ft/s Discharge Full 3.42 ft/s Slope Full 0.00023 ft/ft Flow Type SuperCritical tf/ft Slope SuperCritical 0.000 ift Ownstream Depth 0.00 ift Number Of Steps 0 tf OVEF Output Data 0.00 ift Profile Description 0.00 ift Profile Description 0.00 ift Profile Headloss 0.00 ift Average End Depth Over Rise 0.00 ift Normal Depth Over Rise 15.81 %	Critical Depth		0.20	ft
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Specific Energy 0.53 ft Froude Number 3.41 Maximum Discharge 3.68 ft ³ /s Discharge Full 3.42 ft ³ /s Slope Full 0.00023 ft/ft Flow Type SuperCritical trift GVF Input Data Downstream Depth 0.00 in Length 0.00 ft Number Of Steps 0 ft GVF Output Data Profile Description Profile Description 0.00 ft Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 ft Normal Depth Over Rise 15.81 %	Velocity		5.23	ft/s
Four our our our our our our our our our	Velocity Head		0.42	ft
Maximum Discharge3.68ft³/sDischarge Full3.42ft³/sSlope Full0.00023ft/ftFlow TypeSuperCriticalImage: SuperCritical state GVF Input Data Downstream Depth0.00Length0.00ftNumber Of Steps0 GVF Output Data Upstream Depth0.00Profile DescriptioninProfile Headloss0.00Normal Depth Over Rise0.00Normal Depth Over Rise0.00Normal Depth Over Rise15.81	Specific Energy		0.53	ft
Jischarge Full3.42ft³/sSlope Full0.00023ft/ftFlow TypeSuperCriticalGVF Input DataDownstream Depth0.00Length0.00ftNumber Of Steps0GVF Output DataUpstream Depth0.00Profile DescriptioninProfile Headloss0.00ftNormal Depth Over Rise0.00ftNormal Depth Over Rise15.81%	Froude Number		3.41	
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Flow TypeSuperCriticalGVF Input DataDownstream Depth0.00inLength0.00ftNumber Of Steps0GVF Output DataUpstream Depth0.00inProfile Description0.00ftProfile Headloss0.00ftAverage End Depth Over Rise0.00ftNormal Depth Over Rise15.81%	Discharge Full		3.42	ft³/s
GVF Input Data Downstream Depth 0.00 in Length 0.00 ft Number Of Steps 0 0 GVF Output Data 0 0 Upstream Depth 0.00 in Profile Description 0 in Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 %	Slope Full		0.00023	ft/ft
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Number Of Steps0GVF Output Data0.00Upstream Depth0.00Profile Description1Profile Headloss0.00Average End Depth Over Rise0.00Normal Depth Over Rise15.81	Downstream Depth		0.00	in
GVF Output Data Upstream Depth 0.00 in Profile Description Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 % Normal Depth Over Rise 15.81 %	Length		0.00	ft
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Profile Description Profile Headloss Average End Depth Over Rise Normal Depth Over Rise 15.81	GVF Output Data			
Profile Headloss0.00ftAverage End Depth Over Rise0.00%Normal Depth Over Rise15.81%	Upstream Depth		0.00	in
Average End Depth Over Rise0.00%Normal Depth Over Rise15.81%	Profile Description			
Normal Depth Over Rise 15.81 %	Profile Headloss		0.00	ft
	Average End Depth Over Rise		0.00	%
	Normal Depth Over Rise		15.81	%
	Downstream Velocity		Infinity	ft/s

Bentley Systems, Inc. Haestad Methods SoBdittle CEnterMaster V8i (SELECTseries 1) [08.11.01.03] 27 Siemons Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Page 1 of 2

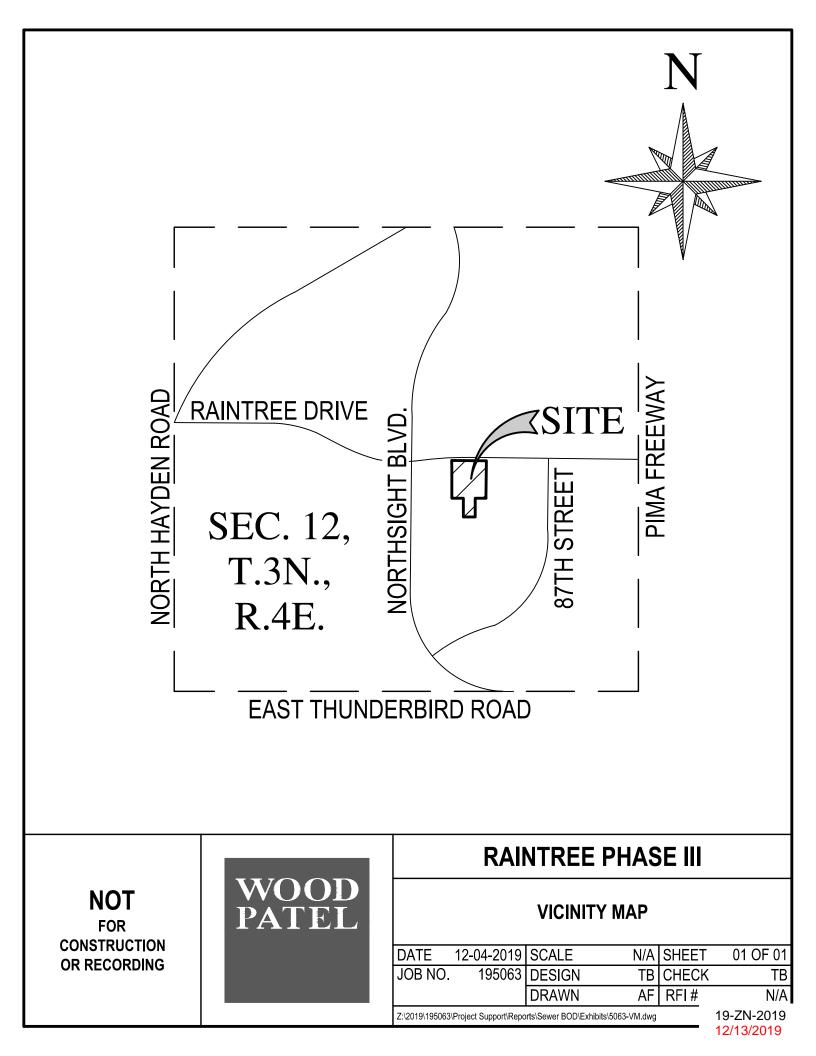
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8-Inch at Peak Flow

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.27	in
Critical Depth	0.20	ft
Channel Slope	0.08000	ft/ft
Critical Slope	0.00641	ft/ft

VICINITY MAP



WASTEWATER EXHIBIT

