



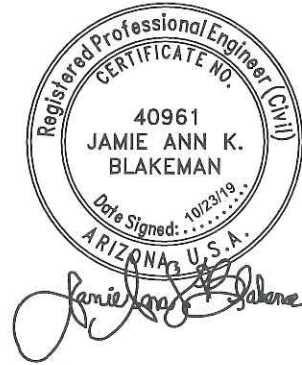
To: Ben Brosseau  
Banyan Residential

Date: October 23, 2019

From: Jamie Blakeman, PE, PTOE

Job Number: 19.5021.01

RE: Scottsdale Entrada  
Traffic Impact & Mitigation Analysis



### INTRODUCTION

Lokahi, LLC (Lokahi) has prepared a Category I Traffic Impact and Mitigation Analysis for the proposed Scottsdale Entrada development located on the northeast corner (NEC) of 64<sup>th</sup> Street and McDowell Road in Scottsdale, Arizona. See **Figure 1** for the vicinity map.

The proposed Scottsdale Entrada development will be comprised of 722 multi-family residential units, 250,000 sf of office, 3,500 sf of retail, 10,000 sf of restaurant, and 1,500 sf of coffee shop. See **Attachment A** and **Figure 2** for the site plan.

The purpose of this Traffic Impact & Mitigation Analysis is to analyze the traffic related impacts of the proposed development to the adjacent roadway network, as well as to provide an update to the 64<sup>th</sup> Street and McDowell Road Traffic Impact Study dated May 20, 2016.



Figure 1 - Vicinity Map

### EXISTING CONDITIONS

The proposed development is bordered by McDowell Road to the south, 64<sup>th</sup> Street to the west, and the Crosscut Canal to the east. The surrounding area consists of single family residential neighborhoods to the north and the east, the Desert Botanical Garden to the south, and Papago Sports Complex to the west.

600 n. 4th street, suite d  
phoenix, az 85004  
480.536.7150  
www.lokahigroup.com

**ACCEPTED**  
**CITY OF SCOTTSDALE**  
**TRANSPORTATION DEPARTMENT**

10/24/19

DATE: \_\_\_\_\_

*Deva OUTLER*  
collaboration

REVIEWER: \_\_\_\_\_

harmony unity cooperation



**McDowell Road** runs east-west and provides three (3) through lanes for each direction of travel with a raised landscaped median. There is a posted speed limit of 45 miles per hour (mph). Per the City of Scottsdale's 2016 *Transportation Master Plan*, McDowell Road is classified as an urban major arterial. The average daily traffic volume along McDowell Road, between 64<sup>th</sup> Street and 68<sup>th</sup> Street is 31,000 vehicles per day (vpd), according to the City of Scottsdale 2016 *Average Daily Traffic Volumes Segment Map*.

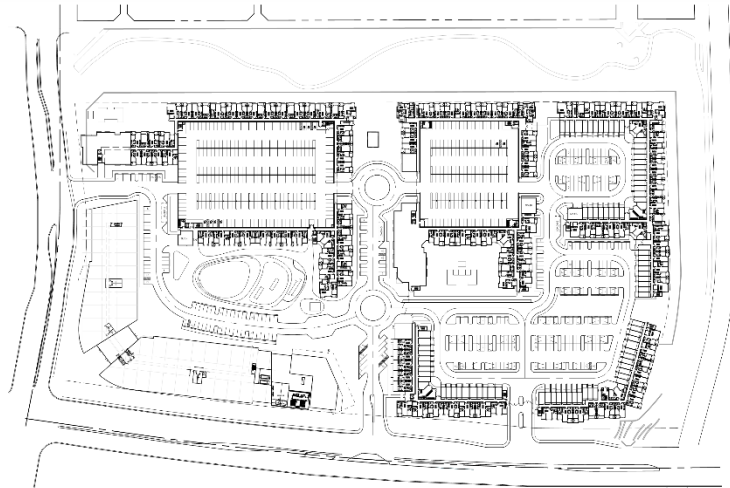


Figure 2 - Site Plan

**64<sup>th</sup> Street/Galvin Parkway** runs north-south and provides two (2) through lanes in each direction of travel with a raised landscaped median. There is a posted speed limit of 40 mph. Per the City of Scottsdale 2016 *Transportation Master Plan*, 64<sup>th</sup> Street is classified as a suburban minor arterial. The average daily traffic volume along 64<sup>th</sup> Street, between McDowell Road and Oak Street is 18,000 vpd, according to the City of Scottsdale 2016 *Average Daily Traffic Volumes Segment Map*.

**McDowell Road and 64<sup>th</sup> Street** currently operates as a signalized intersection under the jurisdiction of the City of Scottsdale. The northbound and the southbound approaches each have an exclusive left turn lane, two (2) through lanes, and an exclusive right turn lane. The eastbound and the westbound approaches each have an exclusive left turn lane, three (3) through lanes, and an exclusive right turn lane. Bike lanes are provided along 64<sup>th</sup> Street for both directions of travel. Approximately 450 feet west of the intersection, for both directions of travel, bike lanes are provided along McDowell Road.

## COLLISION HISTORY

The most recent 3-year collision history, from January 2016 to December 2018, was obtained from the City of Scottsdale. See **Attachment B** for collision data.

### 64<sup>th</sup> Street and McDowell Road

During the three year period, there were a total of 10 crashes, of which there were 3 angle, 3 sideswipe same direction, 2 rear end, 1 single vehicle, and 1 other collision. Violations included speed too fast for conditions (20%), failed to yield right-of-way (20%), and other unsafe passing (20%). The remaining 40% of the crash violations were recorded as followed too closely, drove in





opposite traffic lane, failed to keep in proper lane, and unknown. One crash involved a non-incapacitating injury, one possible injury, one unknown, and the remaining collisions were property damage only.

On October 12, 2017 at 2:31 am, an angle collision occurred with a driver under the influence of alcohol. Both vehicles were headed southbound, and vehicle 1 was cited with speed too fast for conditions. This collision resulted in property damage only.

On December 10, 2017 at 10:14 am, there was a crash involving a bicyclist traveling east who was struck by an eastbound vehicle. This resulted in a possible injury.

**McDowell Road (100 feet east of 64<sup>th</sup> Street to the Crosscut Canal)**

During the three year period, there were a total of 3 crashes, of which there were 2 rear ends and 1 angle collision. There were no reported injuries.

**64<sup>th</sup> Street (100 feet north of McDowell Road to the Papago Sports Complex Driveway)**

During the three year period, there was 1 rear end collision that resulted in no injuries, and a driver being cited for failing to yield to the right-of-way.

**COLLISION RATES**

The City of Scottsdale’s 2016 *Traffic Volume and Collision Rate Data* report provides collision rate and traffic volume information on major roadway segments and at major intersections within the City of Scottsdale. Segment collisions are collisions that occur on a major street more than 100 feet from the major intersections that define the segment, including at minor intersections within the segment. Intersection collisions are collisions that occur at or within 100 feet of a major intersection. The collision rates and city-wide rankings for the study segments and the study intersection are shown in **Table 1** and **Table 2**, respectively.

**Table 1 – Collision Rates - Study Roadway Segments**

Segment	From	To	Collision Rate	Rank
McDowell Road	64th Street	68th Street	0.87	175
64th Street	McDowell Road	Oak Street	0.16	291
2016 City of Scottsdale Average Segment Collision Rate			1.50	

**Table 2 – Collision Rates - Study Intersections**

Intersection	Collision Rate	Rank
McDowell Road and 64th Street	0.09	188
2016 City of Scottsdale Average Intersection Collision Rate	0.65	





## TRIP GENERATION

### TRAFFIC IMPACT STUDY (DATED MAY 20, 2016)

In 2016, the 64<sup>th</sup> Street and McDowell Traffic Impact Study (Zoning Case 5-ZN-2016) was submitted to the City of Scottsdale. See **Attachment C**. The proposed development included 333 multi-family residential units, a 228 key hotel, 566,256 sf of office, 14,400 sf of retail, and 6,400 sf of retail.

The trip generation for the previously approved site is shown in **Table 3** below. This trip generation includes reductions for internal capture and pass-by trips. The trips shown in **Table 3** are taken directly from the 64<sup>th</sup> Street and McDowell Traffic Impact Study dated May 20, 2016.

**Table 3 – Trip Generation – 64<sup>th</sup> Street and McDowell TIS (Dated May 20, 2016)**

Land Use	Weekday	AM Peak Hour			PM Peak Hour			Saturday	Saturday Peak Hour		
	Total	Total	In	Out	Total	In	Out	Total	Total	In	Out
Traffic Impact Study (ZN-5-16), dated May 20, 2016	13,441	1,353	1,023	330	1,225	334	891	9,068	623	337	286

### PROPOSED DEVELOPMENT

The proposed Scottsdale Entrada development will be comprised of 722 multi-family residential units, 250,000 sf of office, 3,500 sf of retail, 10,000 sf of restaurant, and 1,500 sf of coffee shop.

#### Site Access

Three (3) site accesses will be provided. The main access will be located along McDowell Road, approximately 700 feet east of 64<sup>th</sup> Street, and will operate as a full access driveway allowing all movements to and from the proposed development. An additional access will be provided on McDowell Road approximately 1,000 feet east of 64<sup>th</sup> Street. This access will be limited to right-in and right-out movements. The third access will be located on 64<sup>th</sup> Street approximately 470 feet north of McDowell Road. This will operate as a full access driveway allowing all movements to and from the proposed development.

#### Trip Generation

The trip generation for the proposed Scottsdale Entrada was calculated utilizing the Institute of Transportation Engineers (ITE) publication entitled *Trip Generation, 10th Edition*. The ITE rates are based on studies that measure the trip generation characteristics for various types of land uses. The rates are expressed in terms of trips per unit of land use type. This publication is considered to be the standard for the transportation engineering profession.

ITE Land Use 221 – Multifamily Housing (Mid-Rise), ITE Land Use 710 – General Office Building, ITE Land Use 820 – Shopping Center, ITE Land Use 932 – High-Turnover (Sit-Down) Restaurant, and ITE Land Use 936 – Coffee/Donut Shop without Drive-Through were used to calculate the





trips generated by the proposed Scottsdale Entrada development. The internal trip capture rates were applied to the AM and PM peak hour traffic volumes, based on *NCHRP Report 684 – Enhancing Internal Trip Capture of Mixed-Use Developments*. The City of Scottsdale advised to reduce the AM peak hour internal capture rate, therefore 12% was used rather than what is reported in the NCHRP (20%) for only the AM peak hour. The total trip generation is shown in **Table 4**. Detailed trip generation calculations are provided in **Attachment D**.

**Table 4 – Trip Generation - Proposed Scottsdale Entrada Development**

Land Use	Weekday	AM Peak Hour			PM Peak Hour			Saturday	Saturday Peak Hour		
	Total	Total	In	Out	Total	In	Out	Total	Total	In	Out
General Office Building	2,435	290	249	41	288	46	242	553	133	72	61
Multifamily Housing (Mid-Rise)	3,928	260	68	192	318	194	124	3,545	318	156	162
Shopping Center	132	3	2	1	13	6	7	161	16	8	8
High-Turnover (Sit-Down) Restaurant	1,122	99	54	45	98	61	37	1,224	112	57	55
Coffee/Donut Shop without Drive-Through	1,132	152	78	74	54	27	27	1,010	89	44	45
<b>Total</b>	<b>8,749</b>	<b>804</b>	<b>451</b>	<b>353</b>	<b>771</b>	<b>334</b>	<b>437</b>	<b>6,493</b>	<b>668</b>	<b>337</b>	<b>331</b>
<b>Adjusted Internal Capture</b>	<b>1,556</b>	<b>96</b>	<b>48</b>	<b>48</b>	<b>92</b>	<b>46</b>	<b>46</b>	<b>-</b>	<b>-</b>	<b>-</b>	<b>-</b>
<b>Total (After Adjusted Internal Capture)</b>	<b>7,193</b>	<b>708</b>	<b>403</b>	<b>305</b>	<b>679</b>	<b>288</b>	<b>391</b>	<b>6,493</b>	<b>668</b>	<b>337</b>	<b>331</b>

### TRIP GENERATION COMPARISON

A comparison between trips generated by the previously approved site and the proposed Scottsdale Entrada development is shown in **Table 5**.

**Table 5 – Trip Generation Comparison (Proposed Scottsdale Entrada vs. May 2016 TIS)**

Land Use	Weekday	AM Peak Hour			PM Peak Hour			Saturday	Saturday Peak Hour		
	Total	Total	In	Out	Total	In	Out	Total	Total	In	Out
Proposed Entrada Scottsdale	7,193	708	403	305	679	288	391	6,493	668	337	331
Traffic Impact Study (ZN-5-16), dated May 20, 2016	13,441	1,353	1,023	330	1,225	334	891	9,068	623	337	286
<b>Difference</b>	<b>-6,248</b>	<b>-645</b>	<b>-620</b>	<b>-25</b>	<b>-546</b>	<b>-46</b>	<b>-500</b>	<b>-2,575</b>	<b>45</b>	<b>0</b>	<b>45</b>

The proposed Scottsdale Entrada development is expected to generate 6,248 fewer weekday daily trips, 645 fewer weekday AM peak hour trips, and 546 fewer weekday PM peak hour trips than the previously approved site. Additionally, the proposed Scottsdale Entrada development is expected to generate 2,575 fewer Saturday daily trips and 45 more Saturday peak hour trips.





## CONCLUSION

The proposed Scottsdale Entrada development located on the NEC of McDowell Road and 64<sup>th</sup> Street will be comprised of 722 multi-family residential units, 250,000 sf of office, 3,500 sf of retail, 10,000 sf of restaurant, and 1,500 sf of coffee shop.

### Trip Generation

The proposed Scottsdale Entrada development is anticipated to generate 7,193 weekday daily trips, with 708 and 679 occurring during the AM and PM peak hours, respectively. On Saturday it is anticipated to generate 6,493 trips, with 668 occurring during the peak hour.

### Trip Generation Comparison

In 2016, the 64<sup>th</sup> Street and McDowell Traffic Impact Study (Zoning Case 5-ZN-2016) was submitted to the City of Scottsdale which included the analysis of 333 multi-family residential units, a 228 key hotel, 566,256 sf of office, 14,400 sf of retail, and 6,400 sf of retail developed on the site.

The proposed Scottsdale Entrada development is expected to generate 6,248 (46%) fewer weekday daily trips, 645 (48%) fewer weekday AM peak hour trips, and 546 (45%) fewer weekday PM peak hour trips than the previously approved site (Zoning Case 5-ZN-2016). Additionally, the proposed development is expected to generate 2,575 (28%) fewer Saturday daily trips, while generating 45 (7%) more during the Saturday peak hour trips.

The 2016 previously approved site provided three (3) accesses on McDowell Road, similar to the proposed Scottsdale Entrada access, with the exception of the elimination of the west most right-in right-out driveway. Along 64<sup>th</sup> Street, the prior and current site plans proposed a full access driveway. With approximately 50% of the anticipated traffic generated by this new proposed site in comparison to the prior proposed site plan, it is anticipated that these driveways will operate at acceptable levels of service.

The 2016 traffic study also states that a traffic signal is warranted at the main access on McDowell Road between 40% to 50% of the build out of the site. Therefore, with approximately 50% of the site traffic and one less driveway along McDowell Road, it can be concluded that the signal at this location will also be warranted closer to build out of the proposed Scottsdale Entrada development.

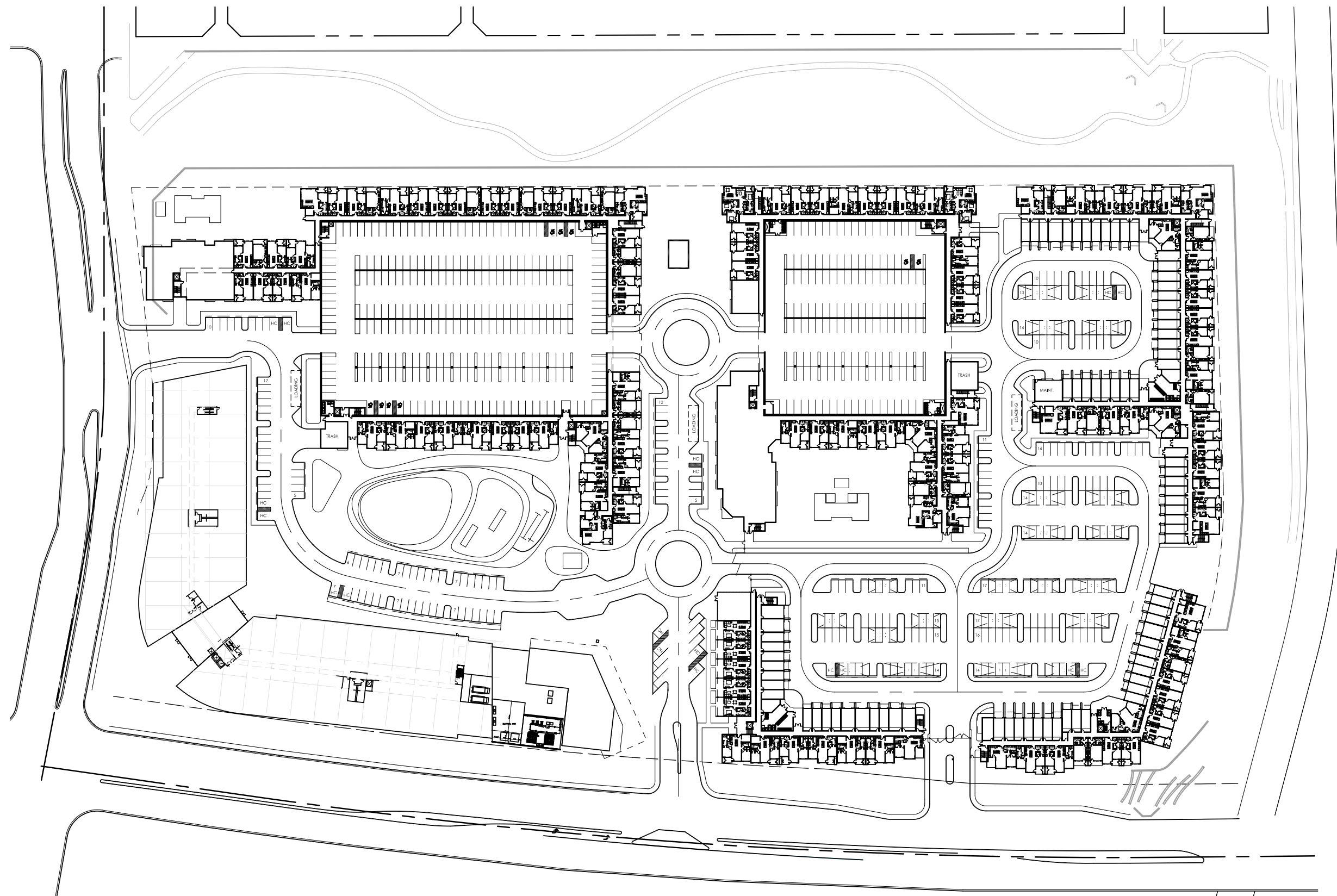
**In conclusion, the proposed Scottsdale Entrada development is anticipated to have significantly less traffic related impacts to the surrounding roadway network as compared to the previously approved May 20, 2016 64<sup>th</sup> Street and McDowell Traffic Impact Study.**





## ATTACHMENT A – PROPOSED SITE PLAN





**SCHEMATIC DESIGN SITE PLAN**

**SCOTTSDALE ENTRADA**



**PROJECT SUMMARY**

UNIT TYPE	UNIT AREA	#DU	RATIO	PS RATIO (PS/DU)	PS REQ'D	PS REQST'D	TARGET MIX
<b>STUDIO</b>		<b>105</b>	<b>15%</b>	1.25	131	105	15%
S01	535	105					
		0					
<b>1 BD</b>		<b>333</b>	<b>46%</b>	1.3	433	333	45%
A01	845	5					
A02	743	265					
A03	861	19					
A04	1,039	44					
<b>2 BD</b>		<b>260</b>	<b>36%</b>	1.7	442	520	35%
B01	1,070	93					
B02	1,128	62					
B03	1,161	76					
B04	1,305	25					
B05	1,243	4					
<b>3 BD</b>		<b>24</b>	<b>3%</b>	1.9	46	72	5%
C01	1,412	24					
		0					
<b>TOTALS</b>		<b>722</b>	<b>100%</b>		<b>1,052</b>	<b>1,030</b>	<b>100%</b>

BLDG	AREA (SF)*	#DU	PS REQST'D
A	71,183	76	110
B	77,028	90	118
C	160,205	175	251
D	153,003	167	248
E	187,118	214	303
<b>TOTAL</b>	<b>648,537</b>	<b>722</b>	<b>1,030</b>

\*unit net rentable s.f. = the unit gross area is measured from the outside face of the exterior stud wall to the center of the units party wall to the outside face of the corridor stud wall. Area does not include building frame-outs, pop-outs, other faux elevation features, patios, balconies, breezeways or patio/ balcony storage rooms.

ENTRADA UNIT MIX  
July 24, 2019  
TAI Project No. 18-2047-01

BUILDING A				LEVEL 1				LEVEL 2				LEVEL 3				LEVEL 4			
UNIT SUMMARY				UNIT				UNIT				UNIT				UNIT			
UNIT TYPE	#DU	RATIO	PS REQST'D	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA
<b>STUDIO</b>	<b>6</b>	<b>8%</b>		S01	535 SF	0	- SF	S01	535 SF	2	1,070 SF	S01	535 SF	2	1,070 SF	S01	535 SF	2	1,070 SF
				S02	595 SF	0	- SF	S02	595 SF	0	- SF	S02	595 SF	0	- SF	S02	595 SF	0	- SF
<b>1 BD</b>	<b>36</b>	<b>47%</b>		<b>1 BD</b>				<b>1 BD</b>				<b>1 BD</b>				<b>1 BD</b>			
				A01	845 SF	5	4,225 SF	A01	845 SF	0	- SF	A01	845 SF	0	- SF	A01	845 SF	0	- SF
				A02	743 SF	0	- SF	A02	743 SF	4	2,972 SF	A02	743 SF	9	6,687 SF	A02	743 SF	8	5,944 SF
				A03	861 SF	1	861 SF	A03	861 SF	1	861 SF	A03	861 SF	1	861 SF	A03	861 SF	1	861 SF
				A04	1,039 SF	0	- SF	A04	1,039 SF	2	2,078 SF	A04	1,039 SF	2	2,078 SF	A04	1,039 SF	2	2,078 SF
<b>2 BD</b>	<b>34</b>	<b>45%</b>		<b>2 BD</b>				<b>2 BD</b>				<b>2 BD</b>				<b>2 BD</b>			
				B01	1,070 SF	2	2,140 SF	B01	1,070 SF	4	4,280 SF	B01	1,070 SF	4	4,280 SF	B01	1,070 SF	4	4,280 SF
				B02	1,128 SF	0	- SF	B02	1,128 SF	0	- SF	B02	1,128 SF	0	- SF	B02	1,128 SF	5	5,640 SF
				B03	1,161 SF	4	4,644 SF	B03	1,161 SF	3	3,483 SF	B03	1,161 SF	5	5,805 SF	B03	1,161 SF	0	- SF
				B04	1,305 SF	0	- SF	B04	1,305 SF	1	1,305 SF	B04	1,305 SF	1	1,305 SF	B04	1,305 SF	1	1,305 SF
				B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF
<b>3 BD</b>	<b>0</b>	<b>0%</b>		<b>3 BD</b>				<b>3 BD</b>				<b>3 BD</b>				<b>3 BD</b>			
				C01	1,412 SF	0	- SF	C01	1,412 SF	0	- SF	C01	1,412 SF	0	- SF	C01	1,412 SF	0	- SF
				C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF
	<b>76</b>	<b>100%</b>	<b>110</b>			<b>12</b>	<b>11,870 SF</b>			<b>17</b>	<b>16,049 SF</b>			<b>24</b>	<b>22,086 SF</b>			<b>23</b>	<b>21,178 SF</b>

BUILDING B				LEVEL 1				LEVEL 2				LEVEL 3				LEVEL 4			
UNIT SUMMARY				UNIT				UNIT				UNIT				UNIT			
UNIT TYPE	#DU	RATIO	PS REQST'D	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA
<b>STUDIO</b>	<b>15</b>	<b>17%</b>		S01	535 SF	3	1,605 SF	S01	535 SF	4	2,140 SF	S01	535 SF	4	2,140 SF	S01	535 SF	4	2,140 SF
				S02	595 SF	0	- SF	S02	595 SF	0	- SF	S02	595 SF	0	- SF	S02	595 SF	0	- SF
<b>1 BD</b>	<b>47</b>	<b>52%</b>		<b>1 BD</b>				<b>1 BD</b>				<b>1 BD</b>				<b>1 BD</b>			
				A01	845 SF	0	- SF	A01	845 SF	0	- SF	A01	845 SF	0	- SF	A01	845 SF	0	- SF
				A02	743 SF	6	4,458 SF	A02	743 SF	10	7,430 SF	A02	743 SF	10	7,430 SF	A02	743 SF	10	7,430 SF
				A03	861 SF	1	861 SF	A03	861 SF	1	861 SF	A03	861 SF	1	861 SF	A03	861 SF	1	861 SF
				A04	1,039 SF	1	1,039 SF	A04	1,039 SF	2	2,078 SF	A04	1,039 SF	2	2,078 SF	A04	1,039 SF	2	2,078 SF
<b>2 BD</b>	<b>28</b>	<b>31%</b>		<b>2 BD</b>				<b>2 BD</b>				<b>2 BD</b>				<b>2 BD</b>			
				B01	1,070 SF	2	2,140 SF	B01	1,070 SF	3	3,210 SF	B01	1,070 SF	3	3,210 SF	B01	1,070 SF	3	3,210 SF
				B02	1,128 SF	0	- SF	B02	1,128 SF	3	3,384 SF	B02	1,128 SF	3	3,384 SF	B02	1,128 SF	3	3,384 SF
				B03	1,161 SF	1	1,161 SF	B03	1,161 SF	1	1,161 SF	B03	1,161 SF	1	1,161 SF	B03	1,161 SF	1	1,161 SF
				B04	1,305 SF	0	- SF	B04	1,305 SF	0	- SF	B04	1,305 SF	0	- SF	B04	1,305 SF	0	- SF
				B05	1,243 SF	1	1,243 SF	B05	1,243 SF	1	1,243 SF	B05	1,243 SF	1	1,243 SF	B05	1,243 SF	1	1,243 SF
<b>3 BD</b>	<b>0</b>	<b>0%</b>		<b>3 BD</b>				<b>3 BD</b>				<b>3 BD</b>				<b>3 BD</b>			
				C01	1,412 SF	0	- SF	C01	1,412 SF	0	- SF	C01	1,412 SF	0	- SF	C01	1,412 SF	0	- SF
				C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF
	<b>90</b>	<b>100%</b>	<b>118</b>			<b>15</b>	<b>12,507 SF</b>			<b>25</b>	<b>21,507 SF</b>			<b>25</b>	<b>21,507 SF</b>			<b>25</b>	<b>21,507 SF</b>

BUILDING C				160,205 SF															
UNIT SUMMARY				LEVEL 1				LEVEL 2				LEVEL 3				LEVEL 4			
UNIT TYPE	#DU	RATIO	PS REQSTD	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA
<b>STUDIO</b>	<b>23</b>	<b>13%</b>	<b>23</b>	<b>STUDIO</b>	<b>535 SF</b>	<b>5</b>	<b>2,675 SF</b>	<b>STUDIO</b>	<b>535 SF</b>	<b>6</b>	<b>3,210 SF</b>	<b>STUDIO</b>	<b>535 SF</b>	<b>6</b>	<b>3,210 SF</b>	<b>STUDIO</b>	<b>535 SF</b>	<b>6</b>	<b>3,210 SF</b>
S01	23	13%	23	S01	535 SF	5	2,675 SF	S01	535 SF	6	3,210 SF	S01	535 SF	6	3,210 SF	S01	535 SF	6	3,210 SF
S02	0	0%	0	S02	595 SF	0	- SF	S02	595 SF	0	- SF	S02	595 SF	0	- SF	S02	595 SF	0	- SF
<b>1 BD</b>	<b>80</b>	<b>46%</b>	<b>0</b>	<b>1 BD</b>	<b>845 SF</b>	<b>0</b>	<b>- SF</b>	<b>1 BD</b>	<b>845 SF</b>	<b>0</b>	<b>- SF</b>	<b>1 BD</b>	<b>845 SF</b>	<b>0</b>	<b>- SF</b>	<b>1 BD</b>	<b>845 SF</b>	<b>0</b>	<b>- SF</b>
A01	0	0%	0	A01	845 SF	0	- SF	A01	845 SF	0	- SF	A01	845 SF	0	- SF	A01	845 SF	0	- SF
A02	62	35%	62	A02	743 SF	8	5,944 SF	A02	743 SF	18	13,374 SF	A02	743 SF	18	13,374 SF	A02	743 SF	18	13,374 SF
A03	3	2%	3	A03	861 SF	0	- SF	A03	861 SF	1	861 SF	A03	861 SF	1	861 SF	A03	861 SF	1	861 SF
A04	15	9%	15	A04	1,039 SF	3	3,117 SF	A04	1,039 SF	4	4,156 SF	A04	1,039 SF	4	4,156 SF	A04	1,039 SF	4	4,156 SF
<b>2 BD</b>	<b>68</b>	<b>39%</b>	<b>0</b>	<b>2 BD</b>	<b>1,070 SF</b>	<b>3</b>	<b>3,210 SF</b>	<b>2 BD</b>	<b>1,070 SF</b>	<b>7</b>	<b>7,490 SF</b>	<b>2 BD</b>	<b>1,070 SF</b>	<b>7</b>	<b>7,490 SF</b>	<b>2 BD</b>	<b>1,070 SF</b>	<b>7</b>	<b>7,490 SF</b>
B01	24	14%	48	B01	1,070 SF	3	3,210 SF	B01	1,070 SF	7	7,490 SF	B01	1,070 SF	7	7,490 SF	B01	1,070 SF	7	7,490 SF
B02	10	6%	20	B02	1,128 SF	1	1,128 SF	B02	1,128 SF	3	3,384 SF	B02	1,128 SF	3	3,384 SF	B02	1,128 SF	3	3,384 SF
B03	23	13%	46	B03	1,161 SF	5	5,805 SF	B03	1,161 SF	6	6,966 SF	B03	1,161 SF	6	6,966 SF	B03	1,161 SF	6	6,966 SF
B04	11	6%	22	B04	1,305 SF	2	2,610 SF	B04	1,305 SF	3	3,915 SF	B04	1,305 SF	3	3,915 SF	B04	1,305 SF	3	3,915 SF
B05	0	0%	0	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF
<b>3 BD</b>	<b>4</b>	<b>2%</b>	<b>12</b>	<b>3 BD</b>	<b>1,412 SF</b>	<b>1</b>	<b>1,412 SF</b>	<b>3 BD</b>	<b>1,412 SF</b>	<b>1</b>	<b>1,412 SF</b>	<b>3 BD</b>	<b>1,412 SF</b>	<b>1</b>	<b>1,412 SF</b>	<b>3 BD</b>	<b>1,412 SF</b>	<b>1</b>	<b>1,412 SF</b>
C01	4	2%	12	C01	1,412 SF	1	1,412 SF	C01	1,412 SF	1	1,412 SF	C01	1,412 SF	1	1,412 SF	C01	1,412 SF	1	1,412 SF
C02	0	0%	0	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF
	<b>175</b>	<b>100%</b>	<b>251</b>		<b>28</b>	<b>25,901 SF</b>		<b>49</b>	<b>44,768 SF</b>		<b>49</b>	<b>44,768 SF</b>		<b>49</b>	<b>44,768 SF</b>		<b>49</b>	<b>44,768 SF</b>	

BUILDING D				153,003 SF															
UNIT SUMMARY				LEVEL 1				LEVEL 2				LEVEL 3				LEVEL 4			
UNIT TYPE	#DU	RATIO	PS REQSTD	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA
<b>STUDIO</b>	<b>23</b>	<b>14%</b>	<b>23</b>	<b>STUDIO</b>	<b>535 SF</b>	<b>2</b>	<b>1,070 SF</b>	<b>STUDIO</b>	<b>535 SF</b>	<b>5</b>	<b>2,675 SF</b>	<b>STUDIO</b>	<b>535 SF</b>	<b>8</b>	<b>4,280 SF</b>	<b>STUDIO</b>	<b>535 SF</b>	<b>8</b>	<b>4,280 SF</b>
S01	23	14%	23	S01	535 SF	2	1,070 SF	S01	535 SF	5	2,675 SF	S01	535 SF	8	4,280 SF	S01	535 SF	8	4,280 SF
S02	0	0%	0	S02	595 SF	0	- SF	S02	595 SF	0	- SF	S02	595 SF	0	- SF	S02	595 SF	0	- SF
<b>1 BD</b>	<b>75</b>	<b>45%</b>	<b>0</b>	<b>1 BD</b>	<b>845 SF</b>	<b>0</b>	<b>- SF</b>	<b>1 BD</b>	<b>845 SF</b>	<b>0</b>	<b>- SF</b>	<b>1 BD</b>	<b>845 SF</b>	<b>0</b>	<b>- SF</b>	<b>1 BD</b>	<b>845 SF</b>	<b>0</b>	<b>- SF</b>
A01	0	0%	0	A01	845 SF	0	- SF	A01	845 SF	0	- SF	A01	845 SF	0	- SF	A01	845 SF	0	- SF
A02	61	37%	61	A02	743 SF	13	9,859 SF	A02	743 SF	14	10,402 SF	A02	743 SF	17	12,631 SF	A02	743 SF	17	12,631 SF
A03	6	4%	6	A03	861 SF	1	861 SF	A03	861 SF	1	861 SF	A03	861 SF	2	1,722 SF	A03	861 SF	2	1,722 SF
A04	8	5%	8	A04	1,039 SF	1	1,039 SF	A04	1,039 SF	1	1,039 SF	A04	1,039 SF	3	3,117 SF	A04	1,039 SF	3	3,117 SF
<b>2 BD</b>	<b>57</b>	<b>34%</b>	<b>0</b>	<b>2 BD</b>	<b>1,070 SF</b>	<b>4</b>	<b>4,280 SF</b>	<b>2 BD</b>	<b>1,070 SF</b>	<b>4</b>	<b>4,280 SF</b>	<b>2 BD</b>	<b>1,070 SF</b>	<b>5</b>	<b>5,350 SF</b>	<b>2 BD</b>	<b>1,070 SF</b>	<b>5</b>	<b>5,350 SF</b>
B01	18	11%	36	B01	1,070 SF	4	4,280 SF	B01	1,070 SF	4	4,280 SF	B01	1,070 SF	5	5,350 SF	B01	1,070 SF	5	5,350 SF
B02	18	11%	36	B02	1,128 SF	4	4,512 SF	B02	1,128 SF	4	4,512 SF	B02	1,128 SF	5	5,640 SF	B02	1,128 SF	5	5,640 SF
B03	14	8%	28	B03	1,161 SF	2	2,322 SF	B03	1,161 SF	2	2,322 SF	B03	1,161 SF	5	5,805 SF	B03	1,161 SF	5	5,805 SF
B04	7	4%	14	B04	1,305 SF	1	1,305 SF	B04	1,305 SF	2	2,610 SF	B04	1,305 SF	2	2,610 SF	B04	1,305 SF	2	2,610 SF
B05	0	0%	0	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF
<b>3 BD</b>	<b>12</b>	<b>7%</b>	<b>36</b>	<b>3 BD</b>	<b>1,412 SF</b>	<b>3</b>	<b>4,236 SF</b>	<b>3 BD</b>	<b>1,412 SF</b>	<b>3</b>	<b>4,236 SF</b>	<b>3 BD</b>	<b>1,412 SF</b>	<b>3</b>	<b>4,236 SF</b>	<b>3 BD</b>	<b>1,412 SF</b>	<b>3</b>	<b>4,236 SF</b>
C01	12	7%	36	C01	1,412 SF	3	4,236 SF	C01	1,412 SF	3	4,236 SF	C01	1,412 SF	3	4,236 SF	C01	1,412 SF	3	4,236 SF
C02	0	0%	0	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF
	<b>167</b>	<b>100%</b>	<b>248</b>		<b>31</b>	<b>29,284 SF</b>		<b>36</b>	<b>32,937 SF</b>		<b>50</b>	<b>45,391 SF</b>		<b>50</b>	<b>45,391 SF</b>				

BUILDING E				187,118 SF															
UNIT SUMMARY				LEVEL 1				LEVEL 2				LEVEL 3				LEVEL 4			
UNIT TYPE	#DU	RATIO	PS REQSTD	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA	UNIT	AREA	#DU	TOTAL AREA
<b>STUDIO</b>	<b>38</b>	<b>18%</b>		<b>STUDIO</b>				<b>STUDIO</b>				<b>STUDIO</b>				<b>STUDIO</b>			
S01	38	18%	38	S01	535 SF	9	4,815 SF	S01	535 SF	9	4,815 SF	S01	535 SF	10	5,350 SF	S01	535 SF	10	5,350 SF
S02	0	0%	0	S02	595 SF	0	- SF	S02	595 SF	0	- SF	S02	595 SF	0	- SF	S02	595 SF	0	- SF
<b>1 BD</b>	<b>95</b>	<b>44%</b>		<b>1 BD</b>				<b>1 BD</b>				<b>1 BD</b>				<b>1 BD</b>			
A01	0	0%	0	A01	845 SF	0	- SF	A01	845 SF	0	- SF	A01	845 SF	0	- SF	A01	845 SF	0	- SF
A02	85	40%	85	A02	743 SF	20	14,860 SF	A02	743 SF	21	15,603 SF	A02	743 SF	22	16,346 SF	A02	743 SF	22	16,346 SF
A03	2	1%	2	A03	861 SF	0	- SF	A03	861 SF	0	- SF	A03	861 SF	1	861 SF	A03	861 SF	1	861 SF
A04	8	4%	8	A04	1,039 SF	1	1,039 SF	A04	1,039 SF	1	1,039 SF	A04	1,039 SF	3	3,117 SF	A04	1,039 SF	3	3,117 SF
<b>2 BD</b>	<b>73</b>	<b>34%</b>		<b>2 BD</b>				<b>2 BD</b>				<b>2 BD</b>				<b>2 BD</b>			
B01	26	12%	52	B01	1,070 SF	6	6,420 SF	B01	1,070 SF	6	6,420 SF	B01	1,070 SF	7	7,490 SF	B01	1,070 SF	7	7,490 SF
B02	20	9%	40	B02	1,128 SF	4	4,512 SF	B02	1,128 SF	4	4,512 SF	B02	1,128 SF	6	6,768 SF	B02	1,128 SF	6	6,768 SF
B03	23	11%	46	B03	1,161 SF	5	5,805 SF	B03	1,161 SF	6	6,966 SF	B03	1,161 SF	6	6,966 SF	B03	1,161 SF	6	6,966 SF
B04	4	2%	8	B04	1,305 SF	1	1,305 SF	B04	1,305 SF	1	1,305 SF	B04	1,305 SF	1	1,305 SF	B04	1,305 SF	1	1,305 SF
B05	0	0%	0	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF	B05	1,243 SF	0	- SF
<b>3 BD</b>	<b>8</b>	<b>4%</b>		<b>3 BD</b>				<b>3 BD</b>				<b>3 BD</b>				<b>3 BD</b>			
C01	8	4%	24	C01	1,412 SF	2	2,824 SF	C01	1,412 SF	2	2,824 SF	C01	1,412 SF	2	2,824 SF	C01	1,412 SF	2	2,824 SF
C02	0	0%	0	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF	C02	1,323 SF	0	- SF
<b>TOTAL</b>	<b>214</b>	<b>100%</b>	<b>303</b>	<b>TOTAL</b>		<b>48</b>	<b>41,580 SF</b>	<b>TOTAL</b>		<b>50</b>	<b>43,484 SF</b>	<b>TOTAL</b>		<b>58</b>	<b>51,027 SF</b>	<b>TOTAL</b>		<b>58</b>	<b>51,027 SF</b>



## ATTACHMENT B – CRASH DATA



64th Street & McDowell Road

REPORT #	YYMMDD	HHMM	NS ST	NS SF	EW ST	EW SF	DIR FROM	DIST FROM	X REF FROM	DOB 1	DOB 2	INJ SEV 1	INJ SEV 2	PHYSICAL C	PHYSICAL C	VIOL 1	VIOL 2	ACTION 1	ACTION 2	RAVEL DIR	RAVEL DIR	MANNER	COMMENTS	DATE ENTERED
16-08502	160410	2123	64	ST	MCDOWELL	RD	AT			12/2/1993		3	0			13		1		EB		1		5/3/2016
1713359	170615	0823	64	ST	MCDOWELL	RD	E	467		5/13/1983	6/27/1994	1	1	0	0	4	1	1	1	WB	WB	4		7/5/2017
1715683	170715	0144	64	ST	MCDOWELL	RD	AT				11/3/1986	99	1	99	0	8	1	4	1	EB	EB	6	HIT AND RUN	7/25/2017
1719931	170908	1841	64	ST	MCDOWELL	RD	AT			4/12/1982	1/22/1993	1	1	0	0	20	1	4	1	EB	WB	2	MULTI VEH 3	9/20/2017
1722564	171012	0231	64	ST	MCDOWELL	RD				1/23/1985	1/1/1990	1	1	4	0	2	1	1	3	SB	SB	2	PHOENIX, DUI	10/31/2017
1725889	171122	1322	64	ST	MCDOWELL	RD	E	437		9/3/1929	1/31/1983	1	1	0	0	12	1	8	1	EB	EB	2		12/27/2017
1727315	171210	1014	64	ST	MCDOWELL	RD	AT			11/7/1975	5/1/1950	1	2	0	0	15	1	1	97	EB	EB	6	CAR/BICYCLE	1/16/2018
1727319	171210	1129	64	ST	MCDOWELL	RD	E	100		2/26/1997	5/11/1995	1	1	0	0	4	97	2	2	WB	WB	4		1/16/2018
1728353	171223	1135	64	ST	MCDOWELL	RD	E	60		12/7/1983	2/26/1978	1	1	0	0	2	1	1	3	WB	WB	4		1/29/2018
1801338	180118	1406	64	ST	MCDOWELL	RD	AT			1/24/1944	6/17/1933	1	1	0	0	15	1	5	1	NB	EB	2		3/2/2018
1802474	180202	0815	64	ST	MCDOWELL	RD	AT			3/17/1996	7/28/1965	1	1	0	0	20	1	5	1	EB	EB	6		3/15/2018
1808111	180411	1626	64	ST	MCDOWELL	RD	E	380		1/31/1990	10/5/1995	1	1	0	0	12	1	8	1	EB	EB	4		4/19/2018
1809501	180428	1250	64	ST	MCDOWELL	RD	N	500		3/31/1969	1/27/1987	1	1	0	0	20	1	2	2	SB	SB	4		5/23/2018
1810698	180514	1008	64	ST	MCDOWELL	RD	E	100		9/26/1995	10/17/1976	1	1	0	0	99	1	97	1	WB	WB	97	MULTI VEH 3	6/6/2018



**ATTACHMENT C – TRIP GENERATION  
REPORT DATED MAY 20, 2016**





# 64<sup>th</sup> St & McDowell Road Draft Traffic Impact Study

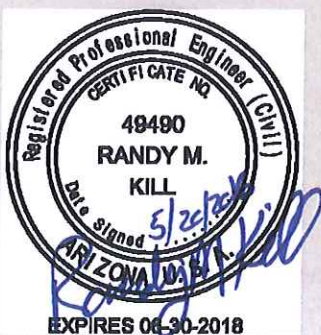
Prepared for SunChase Holdings, Inc.

May 2016

**ACCEPTED**  
CITY OF SCOTTSDALE  
TRANSPORTATION DEPARTMENT

DATE: 6/24/16

REVIEWER: John PBA



**5-ZN-2016**  
**06/20/16**



# Table of Contents

Introduction .....	1
Executive Summary .....	2
McDowell Road and 64 <sup>th</sup> Street .....	2
McDowell Road and Main Access .....	3
64 <sup>th</sup> Street and Road A .....	3
Road A and Road B at McDowell Road .....	3
Proposed Pedestrian and Bicycle Facilities .....	3
Existing Conditions .....	4
Existing Land Use.....	4
Existing Roadway Characteristics.....	4
Existing Transit, Pedestrian and Bicycle Facilities .....	5
Existing Traffic Counts .....	5
Horizon Years .....	5
Annual Growth Rate (Background Traffic) .....	5
Proposed Development .....	9
Land Uses .....	9
Trip Generation .....	9
Internal Capture .....	10
Pass-By Trip Reduction.....	10
Trip Generation Comparison.....	11
Trip Distribution .....	11
Trip Assignment .....	11
Total (Background + Site) Future Traffic .....	12
No Build Traffic Analysis.....	19
Signal Warrant Analysis .....	19
Build Traffic Analysis .....	20
Collision Analysis.....	23
Turn Lane Length Analysis .....	25
McDowell Road and Main Access .....	25
Conclusions and Recommendations.....	27
McDowell Road and 64 <sup>th</sup> Street .....	27
McDowell Road and Main Access .....	27
64 <sup>th</sup> Street and Road A .....	27
Road A and Road B at McDowell Road .....	27
Proposed Pedestrian and Bicycle Facilities .....	27

# Tables

Table 1 – Trip Generation for Former Site .....	4
Table 2 - Trip Generation for Weekday .....	9
Table 3 - Trip Generation for Saturday .....	10
Table 4 – Trip Generation with Internal Capture Reduction .....	10
Table 5 – Trip Generation with Pass-By Reduction.....	11
Table 6 – Trip Generation Comparison .....	11
Table 7 – 2025 No Build Level-Of-Service Results.....	19
Table 8 – Signal Warrant Analysis.....	20
Table 9 – 2025 Build Level-Of-Service Results .....	21
Table 10 - Collision by Type and Direction.....	23
Table 11 - Collision Rate Comparison by Year .....	24
Table 12- Recommended Storage Lengths .....	25

# Figures

Figure 1 – Study Area .....	1
Figure 2 – Conceptual Site Plan .....	2
Figure 3 – Recommended Storage Lengths .....	4
Figure 4 – Existing AM Peak Traffic Volumes.....	6
Figure 5 – Existing PM Peak Traffic Volumes .....	7
Figure 6 – Existing Saturday Peak Traffic Volumes .....	8
Figure 7 – Trip Assignment.....	12
Figure 8 – Site AM Peak Traffic Volumes .....	13
Figure 9 – Site PM Peak Traffic Volumes .....	14
Figure 10 – Site Saturday Peak Traffic Volumes .....	15
Figure 11 – Total 2025 AM Peak Hour Traffic Volumes .....	16
Figure 12 – Total 2025 PM Peak Hour Traffic Volumes .....	17
Figure 13 – Total 2025 Saturday Peak Hour Traffic Volumes .....	18
Figure 14 – Recommended Storage Lengths .....	26

# Appendix

Traffic Counts

Trip Generation for Three Additional Site Plans

*HCS 2010* No Build Analysis Output for Year 2025

Signal Warrant Analysis for Year 2025

*HCS 2010* Build Analysis Output for Year 2025

## Introduction

SunChase Holdings, Inc. proposes to develop approximately 23.5 acres of the former Scottsdale Auto Park dealership site as a mix-use residential, office, retail, hotel and restaurant development. The site is located in the northeast corner of 64<sup>th</sup> Street and McDowell Road in the City of Scottsdale, Arizona, as illustrated in **Figure 1**. Burgess & Niple has been retained to prepare a Traffic Impact Study in accordance with the Category 2 study guidelines of the Design Standards & Policies Manual, Section 5-1, Transportation Impact Studies, 2004 Update.



**Figure 1 – Study Area**

The objectives of this Traffic Impact Study are to:

- Analyze existing traffic conditions
- Estimate new traffic generated by the proposed mix-use development
- Distribute and assign new traffic to adjacent street network
- Determine the need for auxiliary lanes at study intersections
- Evaluate operation of adjacent street network with the new site
- Recommend traffic control measures at study intersections

- Evaluate historical collision data on adjacent street network

The proposed development is shown conceptually on **Figure 2** and includes the following site accesses:

- McDowell Road and Main Access (aligns with existing main entrance to the former dealerships)
- Two additional access points (at Road A and Road B) onto McDowell Road
- 64<sup>th</sup> Street Access and Road A (aligns with existing dealership access)



**Figure 2 – Conceptual Site Plan**

## Executive Summary

### McDowell Road and 64<sup>th</sup> Street

This existing signalized intersection will operate at an acceptable overall LOS D or better in 2025 with the proposed site. It is recommended that the northbound right turn storage length on 64<sup>th</sup> Street be increased from 215' to 265', the southbound left turn storage length be increased from 210' to 235', and the southbound right turn storage length be increased from 150' to 225'. Additionally, it is recommended the existing median between 64<sup>th</sup> Street and Main Access intersections on McDowell Road be modified to provide equal storage lengths for the westbound left turn lane at 64<sup>th</sup> Street and the eastbound left turn lane at Main Access.

## **McDowell Road and Main Access**

This T-intersection warrants a traffic signal with the development between 40% and 50% built out. Therefore, in 2025, under full build conditions, the intersection will warrant a signal. The 2025 analyses of this signalized intersection indicate that it will operate at an acceptable LOS C or better during all three analyzed peak hours. It is recommended that this intersection be monitored to determine at what phase of the development a signal is warranted at this location. In the interim, a “pork chop” median design will be constructed to provide a safer left turn into and out of the site.

It is recommended that if southbound left and right turn length storages be provided, they should be 175’ and 125’, respectively. Additionally, as stated above, it is recommended the existing median between 64<sup>th</sup> Street and Main Access intersections on McDowell Road be modified to provide equal storage lengths for the westbound left turn lane at 64<sup>th</sup> Street and the eastbound left turn lane at Main Access.

## **64<sup>th</sup> Street and Road A**

This T-intersection operates at an acceptable LOS C or better as unsignalized in 2025, with the exception of the westbound left turn movement which operates at lower LOS E and LOS F. However, with the modified median to provide a refuge area for outbound left turning vehicles, the actual operation of this configuration is expected to be better than what is shown in this analysis. The southbound left turn storage length of 100’ is sufficient. It is recommended that an exclusive northbound right turn deceleration lane be provided on 64<sup>th</sup> Street at Road A with a minimum of 100’ of storage to meet the City of Scottsdale standards. Additionally, it is recommended that should westbound left and right turn storage lengths be provided on Road A, they be 100’ each in accordance with City of Scottsdale requirements.

## **Road A and Road B at McDowell Road**

Both of these driveways operate acceptably under all peak hours as right-in/right-out accesses. Exclusive westbound right turn lanes on McDowell Road at Road A and Road B are not required as a result of the operational analysis.

## **Proposed Pedestrian and Bicycle Facilities**

The sidewalks along the frontage of the development will be maintained to provide a connection between the multi-use path parallel to the Cross Cut Canal and to the bike lanes along 64<sup>th</sup> Street, Galvin Parkway, and McDowell Road west of 64<sup>th</sup> Street. Additional pedestrian connections will be provided from the development connecting to the Cross Cut Canal on the north and east sides of the development so that non-vehicular traffic from within the development can easily access the existing multi-use paths.



Figure 3 – Recommended Storage Lengths

## Existing Conditions

### Existing Land Use

The existing vacant site is currently zoned for commercial uses (C-4) and was formerly occupied by car dealerships with total existing building area of 155,900 square feet. The trip generation for the previous site was determined using ITE Land Use Code 841 and is provided in **Table 1**.

Table 1 – Trip Generation for Former Site

	AM Peak Hour		PM Peak Hour		Weekday	Saturday Peak Hour		Saturday
	In	Out	In	Out		In	Out	
Automobile Sales (ITE Land Use Code 841)	224	75	163	245	5,036	313	313	4,636

### Existing Roadway Characteristics

The primary roadways serving the site are McDowell Road and 64<sup>th</sup> Street. McDowell Road is an east-west 6-lane Urban Major Arterial with raised landscape medians. There is an existing median break, located approximately 700' east of 64<sup>th</sup> Street, which served as main full access to the former dealership site. Additionally, an approximately 700-foot long

westbound right-turn deceleration lane exists on McDowell Road from the main full access to the former dealership site to the intersection with 64<sup>th</sup> Street. Posted speed limit on McDowell Road is 45mph.

64<sup>th</sup> Street is a 4-lane north-south Suburban Minor Arterial with raised landscaped medians. An existing median break is located approximately 470' north from McDowell Road and provides a full access to the site. Posted speed limit is 40 mph. 64<sup>th</sup> Street serves as a divider between the City of Scottsdale and City of Phoenix city limits.

The intersection of McDowell Road and 64<sup>th</sup> Street is controlled by a traffic signal while the two existing full access intersections into the site are controlled by stop signs. Exclusive right and left turn lanes are provided at this intersections as well as at each existing site access intersection along McDowell Road and 64<sup>th</sup> Street with exception of northbound right turn lane at the site intersection with 64<sup>th</sup> Street.

Typical ¼-mile or ½-mile signal spacing exist within the City of Scottsdale in the vicinity of the site, with signals at 68<sup>th</sup> Street, 70<sup>th</sup> Street, Scottsdale Road, 74<sup>th</sup> Street, Miller Road, 77<sup>th</sup> Street, and Hayden road to the east and at Oak Street, Thomas Street, and Indian School to the north. Closest signals to the west and the south (City of Phoenix) are located approximately 1 mile from the McDowell Road/64<sup>th</sup> Street intersection at 54<sup>th</sup> Street and Moreland Street, respectively.

## **Existing Transit, Pedestrian and Bicycle Facilities**

Two existing bus routes are currently operating along McDowell Road in the site vicinity. Valley Metro's local bus service (Route 17) runs along McDowell Road with closest stops at 44<sup>th</sup> Street to the west and at Scottsdale Road to the east. Second local bus service (Route 56) is provided from Galvin Parkway turning east on McDowell Road with closest stops at Desert Botanical Garden/Galvin Parkway to the south and at Scottsdale Road/McDowell Road to the east.

According to the Maricopa Association of Government Regional Bike Map, the City of Phoenix provides bike lanes along three legs of the McDowell Road/64<sup>th</sup> Street intersection, i.e. the north leg (64<sup>th</sup> Street), the south leg (Galvin Parkway), and the west leg of McDowell Road. No designated bike lanes exists along the site frontage on the east leg of McDowell Road. Additionally, the City of Scottsdale maintains a paved multi-use path along Arizona Cross Cut Canal (the eastern site border) as well as along the northern border of the existing site. Attached sidewalks are also provided along the site frontage on McDowell Road and 64<sup>th</sup> Street.

## **Existing Traffic Counts**

Traffic Research and Analysis, Inc. was retained to obtain daily traffic counts for the adjacent to site segments of McDowell Road and 64<sup>th</sup> Street as well as turning movement counts at the intersection of McDowell Road and 64<sup>th</sup> Street. The traffic counts were collected beginning Thursday, December 10, 2015 through Saturday, December 12, 2015. Existing morning (AM), evening (PM), and Saturday traffic volumes are illustrated in **Figure 4** through **Figure 6**, respectively. Traffic count data is also presented in the **Appendix**.

## **Horizon Years**

Category 2 study is based on traffic conditions for the build-out or completion year of the development, which according to the developer is year 2025.

## **Annual Growth Rate (Background Traffic)**

Burgess & Niple reviewed the 2035 MAG model. The model showed negative growth within the study area. The historical traffic counts from the City of Scottsdale were then reviewed. According to this data McDowell Road and 64<sup>th</sup>

Street traffic experience net annual decrease from 2000 to 2012. Although two different sources indicated decrease in traffic volumes, with the redevelopment of the McDowell Road corridor occurring, it is reasonable to assume that a portion of the negative growth in recent years will return to the area, especially by 2025. Therefore, a conservative growth rate of 0.5% per year for 10 years was assumed.



Figure 4 – Existing AM Peak Traffic Volumes





Figure 5 – Existing PM Peak Traffic Volumes



Figure 6 – Existing Saturday Peak Traffic Volumes

## Proposed Development

The proposed site is located on the vacant former auto dealership site. The surrounding land consists primarily of single-family residential properties to the north and the east. Desert Botanical Garden is located south of the site with access to Galvin Parkway (south leg of 64<sup>th</sup> Street/McDowell intersection) and no direct access to McDowell Road. The City of Phoenix park land is located to the west. The proposed conceptual plan is a mixture of residential, office, retail, hotel and restaurant parcels. Four conceptual site plans are being presented by SunChase Holdings, Inc. as part of Rezoning Development process. Burgess & Niple has evaluated trip generation scenarios associated with each of the proposed site plans and in coordination with the City of Scottsdale chose one that best represents the potential traffic impacts on the nearby area. This conceptual site plan (Option 1) with its internal roadways is shown on **Figure 2**. It is anticipated that the site will be fully developed in 2025. The **Appendix** contains trip generation for the remaining three site plans.

## Land Uses

The site occupies approximately 30 acres. The following are the assumed land uses for the development plan:

Building B1 (4-level office)	128,060	square feet (SF)
Building B2 (4-level office)	163,800	SF
Building B3 (4-level office)	144,796	SF
Building B4 (4-level office)	129,600	SF
Building D (4-level hotel)	228	rooms
Building E (4-level residential)	165	dwelling units (DU)
Building F (4-level office)	168	DU
Retail (shopping center)	14,400	SF
Restaurant (assumed as high-turnover sit-down)	6,400	SF

## Trip Generation

The traffic volumes generated by the proposed development were estimated using the Institute of Transportation Engineer's (ITE) *Trip Generation Manual, 9th Edition*. The ITE data is based on studies that measured the trip generation characteristics for various types of land uses. The rates are expressed in terms of vehicular trips per unit of land use. **Table 2** and **Table 3** summarize the expected number of trips for each parcel during the Weekday and Saturday, respectively.

**Table 2 - Trip Generation for Weekday**

Description	Units	Amount	ITE Land Use Code	AM		PM		Weekday
				In	Out	In	Out	
Office (Building B1)	SF	128,060	710	205	28	38	184	1,584
Office (Building B2)	SF	163,800	710	250	34	45	217	1,910
Office (Building B3)	SF	144,796	710	226	31	41	200	1,740
Office (Building B4)	SF	129,600	710	207	28	38	186	1,599
Hotel (Building D)	Rooms	228	310	71	50	70	67	1,863
Residential (Building E)	DU	165	230	13	64	61	30	994
Residential (Building F)	DU	168	230	13	65	62	30	1,010
Shopping Center (Retail)	SF	14,400	820	30	18	79	85	1,927
Restaurant (High-Turnover Sit-Down)	SF	6,400	932	38	31	38	25	814
<b>Total Trips for Weekday</b>				<b>1,053</b>	<b>349</b>	<b>472</b>	<b>1,024</b>	<b>13,441</b>

Description	Units	Amount	ITE Land Use Code	Peak Hour		Saturday Day
				In	Out	
Office (Building B1)	SF	128,060	710	30	25	316
Office (Building B2)	SF	163,800	710	38	32	402
Office (Building B3)	SF	144,796	710	34	29	356
Office (Building B4)	SF	129,600	710	30	26	318
Hotel (Building D)	Rooms	228	310	91	71	1,867
Residential (Building E)	DU	165	230	49	41	1,030
Residential (Building F)	DU	168	230	49	42	1,040
Shopping Center (Retail)	SF	14,400	820	129	119	2,725
Restaurant	SF	6,400	932	48	42	1,014
<b>Total Trips for Saturday</b>				<b>498</b>	<b>427</b>	<b>9,068</b>

## Internal Capture

Given the mix-use nature of the proposed development which includes office, retail, restaurant, residential and hotel components, it is expected that several of the estimated trips will be internal to the development. Based on *the NCHRP Report 684 – Enhancing Internal Trip Capture Estimation of Mixed-Use Developments*, the internal capture rates for trip origins and trip destinations within a multi-use development were applied for weekday AM and PM hours. Since Saturday Peak Hour rates were not available, it was assumed that the Saturday peak internal capture rate was the same as the PM peak hour. **Table 4** summarizes the trip reductions for the internal capture rate. Supporting calculations are presented in the **Appendix**.

Description	Reduced AM Peak Hour			Reduced PM Peak Hour			Reduced Saturday Peak Hour		
	In	Out	Total	In	Out	Total	In	Out	Total
Office (Buildings B1, B2, B3, B4)	880	120	1,000	154	752	906	126	107	233
Hotel (Building D)	71	49	120	50	48	98	64	51	115
Residential (Buildings E, F)	26	127	153	73	36	109	58	49	107
Shopping Center	19	12	31	55	60	115	91	84	175
Restaurant	27	22	49	22	15	37	28	25	53
<b>Total Trips After Internal Capture Reduction</b>	<b>1,023</b>	<b>330</b>	<b>1,353</b>	<b>354</b>	<b>911</b>	<b>1,265</b>	<b>367</b>	<b>316</b>	<b>683</b>

## Pass-By Trip Reduction

A number of the trips generated by the retail and restaurant components are expected to be pass-by trips coming from travelers passing the site on their way to their actual destination. Both the McDowell Road corridor and 64<sup>th</sup> Street are convenient and popular commuter routes within the cities of Phoenix and Scottsdale. Based on research and guidance from the *ITE Trip Generation Handbook*, the following pass-by rates were assumed:

- Shopping Center: 25% in Weekly PM and Saturday peak hours

- High-Turnover (Sit-Down) Restaurant: 30% in Weekly PM and Saturday peak hours

**Table 5** summarizes the pass-by reductions taken for this development. The adjusted assigned site trip volumes used for analysis are illustrated in **Figures 8** through **Figure 10**, for the morning (AM), evening (PM), and Saturday peak, respectively.

Table 5 – Trip Generation with Pass-By Reduction									
	Total AM Peak Hour			Total PM Peak Hour			Total Saturday Peak Hour		
	In	Out	Total	In	Out	Total	In	Out	Total
Trip Trips After Internal Capture Reduction	1,023	330	1,353	354	911	1,265	367	316	683
Shopping Center Pass-By Reduction	-	-	-	14	14	28	22	22	44
Restaurant Pass-By Reduction	-	-	-	6	6	12	8	8	16
<b>Total External Trips</b>	<b>1,023</b>	<b>330</b>	<b>1,353</b>	<b>334</b>	<b>891</b>	<b>1,225</b>	<b>337</b>	<b>286</b>	<b>623</b>

## Trip Generation Comparison

**Table 6** compares the trip generation of the previous land use (automobile sales) to the proposed land use.

Table 6 – Trip Generation Comparison									
	Total External Trips AM Peak Hour			Total External Trips PM Peak Hour			Total External Trips Saturday Peak Hour		
	In	Out	Total	In	Out	Total	In	Out	Total
Previous Land Use	224	75	299	163	245	408	313	313	626
Proposed Land Use	1,023	330	1,353	334	891	1,223	337	286	623
<b>Difference</b>	<b>799</b>	<b>255</b>	<b>1,054</b>	<b>171</b>	<b>646</b>	<b>815</b>	<b>24</b>	<b>-27</b>	<b>-3</b>

## Trip Distribution

The trip distribution procedure determines the general pattern of travel for vehicles entering and leaving the proposed development. For this study, the trips will be distributed using existing traffic count data as well as the general knowledge of the major destinations in the area. The assumed trip distribution was determined according to the following:

- 20% to/from the north on 64<sup>th</sup> Street
- 20% to/from the south on 64<sup>th</sup> Street
- 30% to/from the east on McDowell Road
- 30% to/from west on McDowell Road

## Trip Assignment

The site traffic will utilize the Main Access as well as the two additional right-in-right-out access points onto McDowell Road. Additional full access is provided via 64<sup>th</sup> Street. **Figure 7** summarizes the trip assignment at the four access points to the site. **Figures 8** through **10** shown the total site trips for the AM, PM, and Saturday peak hours, respectively.

# Total (Background + Site) Future Traffic

The total future volumes used for analysis are illustrated in **Figure 11** through **Figure 13**. These total volumes were obtained by adding the 2025 background volumes (existing counts grown by 0.5% per year for 10 years) and the total adjusted site trips.



Figure 7 – Trip Assignment



Figure 8 – Site AM Peak Traffic Volumes



Figure 9 – Site PM Peak Traffic Volumes





Figure 10 – Site Saturday Peak Traffic Volumes



Figure 11 – Total 2025 AM Peak Hour Traffic Volumes





Figure 13 – Total 2025 Saturday Peak Hour Traffic Volumes

## No Build Traffic Analysis

To evaluate the proposed development’s impacts on the study intersection of McDowell Road and 64<sup>th</sup> Street, a “no build” analysis was conducted using 2025 background volumes (no site trips). Analysis was conducted utilizing *HCS 2010* software using methodologies from the *Highway Capacity Manual* (HCM). This manual measures the average delay per vehicle to determine the Level-Of-Service (LOS) for signalized and unsignalized intersections. LOS A represents the best operation with least delay, while LOS F represents the worst operation. City of Scottsdale considers LOS D or better an acceptable operation for the overall intersection and LOS E or better an acceptable operation of intersection approaches and turning movements during the peak hours.

Under the No Build analysis, the existing intersection lane configurations and cycle lengths were not modified, however, vehicle splits were optimized. The No Build analysis results for the AM, PM and Saturday peak are illustrated in **Table 7**. The LOS, delay (sec/veh), volume-to-capacity ratios [v/c] are reported from *HCS 2010* output.

Table 7 – 2025 No Build Level-Of-Service Results													
McDowell Road and 64 <sup>th</sup> Street Intersection													
	Overall LOS	Eastbound McDowell Road			Westbound McDowell Road			Northbound 64 <sup>th</sup> Street			Southbound 64 <sup>th</sup> Street		
		LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
AM Peak Hour	C 28.1	C (30.4) [0.91]	C (21.1) [0.38]	B (15.7) [0.02]	B (15.8) [0.68]	C (25.0) [0.60]	B (14.4) [0.22]	D (42.0) [0.10]	D (46.9) [0.48]	C (34.6) [0.20]	D (37.5) [0.49]	D (43.2) [0.64]	C (34.6) [0.63]
		C (23.5)			C (22.5)			D (43.8)			D (39.3)		
PM Peak Hour	D (37.0)	D (38.9) [0.88]	C (34.0) [0.72]	C (22.5) [0.21]	D (41.4) [0.88]	C (32.8) [0.54]	C (20.7) [0.24]	D (35.5) [0.24]	E (56.2) [0.87]	D (41.6) [0.75]	D (39.2) [0.77]	D (37.0) [0.51]	C (23.3) [0.36]
		C (34.2)			C (33.0)			D (49.8)			C (34.0)		
Saturday Peak	C (22.3)	B (12.5) [0.62]	B (14.6) [0.28]	B (10.1) [0.03]	B (13.4) [0.22]	B (18.6) [0.34]	B (13.2) [0.19]	C (34.7) [0.26]	D (40.4) [0.49]	D (37.1) [0.54]	C (33.7) [0.42]	D (37.2) [0.28]	C (32.2) [0.58]
		B (13.9)			B (17.2)			D (38.4)			C (34.0)		

Outputs from *HCS 2010* are provided in the **Appendix**. The signalized intersection of McDowell Road and 64<sup>th</sup> Street and all movements and approaches operate at LOS D or better in 2025 during all three peak hours.

## Signal Warrant Analysis

The Manual on Uniformed Traffic Control Devices (MUTCD) published by the United States Department of Transportation is a source used in determining the need for traffic signal installation through the United States. This document establishes nine (9) separate, related sets of criteria termed “warrants”. If none of the warrants is satisfied then a signal should not be installed. If one or more of the warrants is satisfied, then a signal might be appropriate if the signal is shown to improve the overall safety and/or operation of the intersection.

The MUTCD process was utilized to determine if a signal is warranted with the proposed development at the intersection of McDowell Road and Main Access. 24 hour counts from a similar site were used to determine the volume

distribution by time of day on the southbound leg. A portion of, if not the majority of, the southbound right turns at this driveway would not benefit from signalization. Therefore, analyses were conducted where all (100%) of the right turn volumes were included, 40% of the right turn volumes were included, 25% of the right turn volumes were included, and none of the right turn volumes were included (0%). While there are nine warrants, only Warrant 1 (Eight-Hour Vehicular Volume) was conducted for this study. Because the major street speed limit exceeds 40 mph, the reduced (70% factor) hourly volumes were applied per MUTCD guidance. **Table 8** summarizes the results of the signal warrant analysis for Warrant 1 with the detailed signal warrant analyses provided in the **Appendix**.

Table 8 – Signal Warrant Analysis			
	Condition A	Condition B	Condition A & B
<b>100% Developed</b>			
<b>100% Rights</b>	Yes (14 hours)	Yes (16 hours)	Yes (14 hours)
<b>40% Rights</b>	Yes (11 hours)	Yes (14 hours)	Yes (13 hours)
<b>25% Rights</b>	Yes (10 hours)	Yes (14 hours)	Yes (12 hours)
<b>0% Rights</b>	No (6 hours)	Yes (14 hours)	Yes (10 hours)
<b>50% Developed</b>			
<b>100% Rights</b>	No (6 hours)	Yes (14 hours)	Yes (10 hours)
<b>40% Rights</b>	No (3 hours)	Yes (11 hours)	No (6 hours)
<b>25% Rights</b>	No (3 hours)	Yes (10 hours)	No (5 hours)
<b>0% Rights</b>	No (1 hour)	Yes (8 hours)	No (3 hours)
<b>40% Developed</b>			
<b>100% Rights</b>	No (4 hours)	Yes (12 hours)	No (6 hours)
<b>40% Rights</b>	No (1 hour)	Yes (8 hours)	No (4 hours)
<b>25% Rights</b>	No (1 hour)	No (6 hours)	No (3 hours)
<b>0% Rights</b>	No (0 hours)	No (5 hours)	No (1 hour)

According to the MUTCD, Condition A is intended for locations with a large volume of intersecting traffic while Condition B is used at locations where Condition A is not satisfied and where the traffic volume on the major road is so heavy that traffic on the minor intersecting street suffers excessive delay or conflict in entering or crossing the major street. The combination of Conditions A and B is used when Condition A and Condition B are not satisfied alone. If any one of these conditions (Condition A, Condition B, or a combination) are satisfied, then Warrant 1 is satisfied. In this case, Condition B most fits the location and Warrant 1 is met even with all right turning volumes removed when the development is at 50% of its full build. At 40% development, the warrant is not met when all right turning volume is removed. Therefore, it can be said that the signal will be warranted sometime between a 40% and 50% development level. A signal will definitely be warranted when the development is 100% complete.

## Build Traffic Analysis

The Build traffic analysis analyzes the total future traffic incorporating the trips generated by the proposed development and the background trips. *HCS 2010* was also used in this analysis. The Build analysis assumed the existing intersection lane configurations were not modified, however the signal timing and cycle lengths were optimized. The proposed site full access points on McDowell Road (Main Access) and 64<sup>th</sup> Street (Road A Access) were also analyzed. Because a signal is warranted at full-build, and the 2025 analysis is considered full-build of the development, McDowell Road and Main Access intersection was analyzed as a signal. The HCM does not analyze unsignalized intersections with more than

three through lanes on an approach as is the case with the two unsignalized site driveways along McDowell Road. For analysis purposes, the intersections were coded as having two exclusive through lanes and one shared through and right-turn lane. The westbound through volume was reduced by 20% to account for the traffic that would be traveling in the left-most travel lane that is not included in this analysis.

Per discussions with the City of Scottsdale, the median along 64<sup>th</sup> Street will be modified to provide a refuge area in the median at the main site driveway. This configuration will provide a two-stage left turn movement for outbound traffic from the main site driveway on 64<sup>th</sup> Street. This left turn configuration for 64<sup>th</sup> Street and Road A cannot be modeled in HCM. Therefore, this movement was modeled as a typical left turn from a stop-controlled approach with a median wide enough to store one vehicle. Given the proposed configuration, actual operations for this movement are expected to be much better than summarized herein. The operational results for the three study intersections for the three peak hours are illustrated in **Table 9**. The LOS, delay, and volume-to-capacity ratios (v/c) are reported from the *HCS 2010* output which are provided in the **Appendix**.

Table 9 – 2025 Build Level-Of-Service Results													
McDowell Road and 64 <sup>th</sup> Street Intersection (Signalized)													
	Overall LOS	Eastbound McDowell Road			Westbound McDowell Road			Northbound 64 <sup>th</sup> Street			Southbound 64 <sup>th</sup> Street		
		LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
AM Peak Hour	C (32.7)	D (40.0) [0.92]	C (26.9) [0.43]	B (18.3) [0.02]	D (35.7) [0.92]	C (29.4) [0.69]	B (15.2) [0.24]	D (42.7) [0.09]	D (50.0) [0.73]	D (36.1) [0.44]	D (39.7) [0.67]	D (41.5) [0.61]	C (31.5) [0.61]
PM Peak Hour	D (47.8)	D (47.6) [0.93]	E (55.6) [0.96]	C (28.8) [0.26]	E (72.4) [0.97]	D (35.3) [0.67]	C (22.5) [0.31]	D (36.3) [0.28]	E (66.8) [0.94]	C (30.4) [0.65]	E (79.6) [0.95]	D (39.8) [0.59]	C (24.4) [0.43]
Saturday Peak	C (23.1)	B (13.8) [0.66]	B (16.1) [0.34]	B (10.8) [0.03]	B (14.0) [0.35]	B (19.6) [0.38]	B (13.7) [0.21]	C (34.3) [0.26]	D (40.5) [0.54]	D (37.9) [0.61]	C (33.4) [0.49]	D (36.7) [0.29]	C (32.1) [0.59]
McDowell Road and Main Access Intersection (Signalized)													
	Overall LOS	Eastbound McDowell Road		Westbound McDowell Road		Southbound Main Access							
		LT	TH	TH	RT	LT	RT						
AM Peak Hour	B (19.0)	C (32.9) [0.91]	A (2.9) [0.31]	C (25.1) [0.93]	B (12.0) [0.24]	C (30.2) [0.62]	B (14.4) [0.17]						
PM Peak Hour	B (11.7)	B (10.0) [0.45]	A (7.6) [0.62]	B (14.1) [0.65]	A (9.4) [0.07]	C (24.2) [0.79]	B (17.8) [0.51]						
Saturday Peak	A (7.0)	A (5.0) [0.32]	A (3.1) [0.29]	A (8.9) [0.44]	A (7.0) [0.07]	C (22.1) [0.48]	B (16.7) [0.23]						

64 <sup>th</sup> Road and Road A Intersection (Unsignalized)				
AM Peak Hour		Westbound Road A		Southbound 64 <sup>th</sup> Street
		LT	RT	LT
		E (38.4) [0.25]	B (11.7) [0.09]	B (11.3) [0.23]
		C (22.4)		
PM Peak Hour		F (65.6) [0.65]	C (18.1) [0.35]	B (12.7) [0.10]
		E (37.0)		
		C (18.8) [0.11]	B (11.3) [0.08]	A (9.7) [0.07]
		B (14.3)		
McDowell Road and Road A Intersection (Unsignalized)				
AM Peak Hour				Southbound Road A
				RT
				C (23.3) [0.15]
PM Peak Hour				C (18.1) [0.26]
				B (13.4) [0.07]
Saturday Peak				
McDowell Road and Road B Intersection (Unsignalized)				
AM Peak Hour				Southbound Road B
				RT
				C (23.3) [0.15]
PM Peak Hour				C (21.0) [0.30]
				B (13.5) [0.07]
Saturday Peak				

The signalized intersection of McDowell Road and 64<sup>th</sup> Street will operate at an acceptable LOS D or better under Build Conditions during the 2025 AM, PM and Saturday peak hour with existing intersection lane configurations. The signalized intersection of McDowell Road and Main Access will operate at LOS C or better during all three peak hours. Although the westbound left turn movement at the unsignalized intersection of 64<sup>th</sup> Street and Road A operates at LOS E during the AM peak hour and LOS F during the PM peak hour, no v/c are greater than 1.0 indicating that no movement is



over capacity. As previously stated, with the proposed configuration of the left turn movement, operations are expected to be better than shown in this analysis.

## Collision Analysis

Burgess & Niple obtained the most recent 3-year collision history (2012-2014) near the intersection of McDowell Road and 64<sup>th</sup> Street. This data included crashes at the intersection as well as its proximity along McDowell Road and 64<sup>th</sup> Street (also recorded as Galvin Parkway). There were a total of 12 crashes reported, with five crashes each in 2012 and 2013 and only two in 2014. The intersection collisions are summarized in the charts and **Table 10**. Collision summaries indicate that majority of the collisions were angle type.

Improper turning and speeding accounted for more than half of the crashes. Four of the crashes (33%) resulted in incapacitating injuries and one (8%) in non-incapacitating injury. Additionally, one collision involved alcohol-impaired driver and one ill-impaired driver who failed to keep his vehicle in proper lane striking another vehicle.

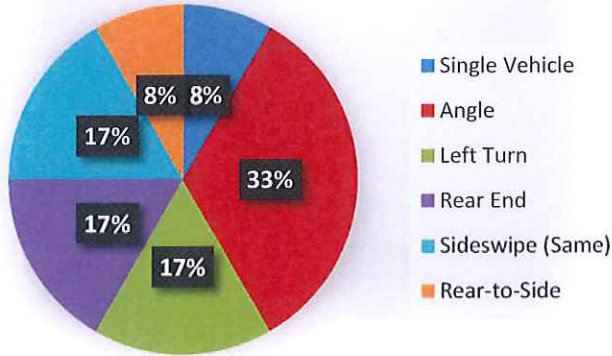
Twelve collisions for the 3-year period for the analyzed intersection of McDowell Road and 64<sup>th</sup> Street is very small with the latest 2014 data only documenting two crashes. For an intersection of this size and traffic volume, twelve crashes in 3-year period is unusually low.

Manner of Collision	McDowell Road		64th Street	
	EB	WB	NB	SB
<i>Single Vehicle</i>	0	0	0	1
<i>Angle</i>	1	0	2	1
<i>Left Turn</i>	0	0	0	2
<i>Rear End</i>	0	2	0	0
<i>Sideswipe (Same)</i>	2	0	0	0
<i>Rear-to-Side</i>	1	0	0	0
<b>Total</b>	<b>4</b>	<b>2</b>	<b>2</b>	<b>4</b>

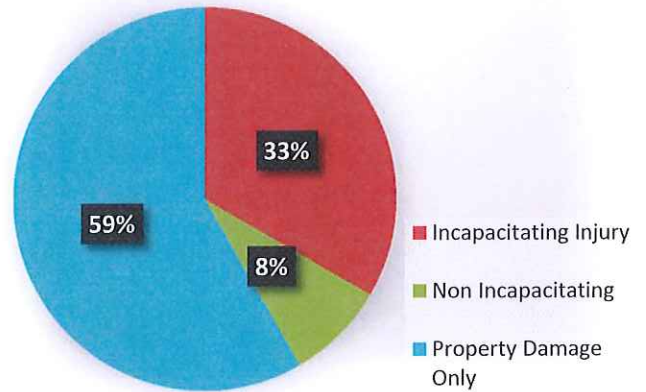
Every two years the City of Scottsdale publishes Traffic Volume and Collision Rate Data report for major intersections and major roadway segments within the city. According to the latest 2014 document, the following 10-year statistics exist for intersections and segments within the study area, listed in **Table 11**.

The intersection of McDowell Road and 64<sup>th</sup> Street as well as the two analyzed segments of McDowell Road and 64<sup>th</sup> Street have lower collision rates than the citywide averages for intersections and segments within the city. Additionally, historical data indicates general downward trends within the study area.

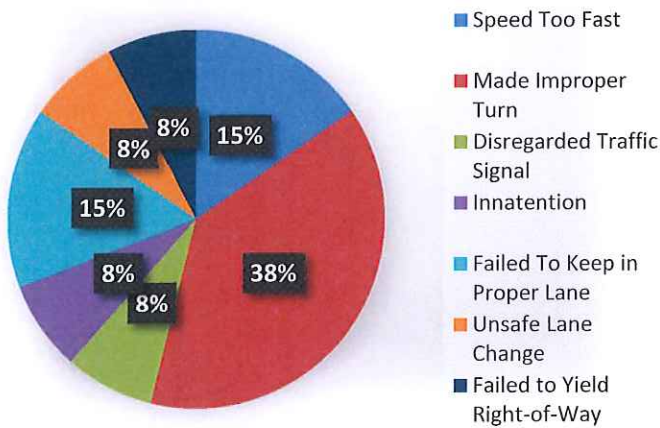
### Collisions by Manner of Collision



### Collisions by Severity



### Collisions by Violation



**Table 11 - Collision Rate Comparison by Year**

		2014	2012	2010	2008	2006	2004
Intersection	Citywide	0.57	0.52	0.48	0.53	0.64	0.66
	McDowell Road and 64 <sup>th</sup> Street Intersection	0.13	0.12	0.18	0.16	0.22	0.22
Segment	Citywide	1.35	1.31	1.27	1.28	1.87	1.84
	McDowell Road between 64 <sup>th</sup> and 68 <sup>th</sup> Streets	0.36	0.89	0.31	0.71	1.03	1.07
	64 <sup>th</sup> Street between McDowell Road and Oak Street	0	0	0.30	0	1.00	0.67

## Turn Lane Length Analysis

The existing right and left turn lanes at the three study intersections were evaluated to determine the storage requirements with the proposed site in year 2025. HCS Software estimates 95<sup>th</sup> percentile queue lengths. The required storage lengths were determined based on the highest peak turning volumes. Average passenger vehicle length is assumed to be 25’.

### McDowell Road and Main Access

The required southbound left turn storage length was determined based on the highest peak turning volumes of 276 vehicles per hour (vph). The 95<sup>th</sup> percentile queue during the PM peak hour is 6.8 vehicles per lane, hence a total required storage is 170’.

The existing westbound right turn storage length is approximately 120’. The required length was determined based on the highest AM peak right turning volumes of 157 vph. The 95<sup>th</sup> percentile queue during the AM and PM peak hours is 1.6 vehicles per lane, hence the available storage of 120’ is sufficient.

Same procedure was used to calculate all other storage lengths at study intersections. Recommended storage length are shown in **Table 12** and on **Figure 14**.

Intersection/ Turn Lane		2025 Design Volumes (vph)			Highest 50 <sup>th</sup> /95 <sup>th</sup> Percentile Queue (length)	Existing Storage	Recommended Storage
		AM	PM	Saturday			
McDowell Road and Main Access	EB LT	<b>460</b>	150	152	10.0 (250’)	120’	*
	WB RT	<b>153</b>	50	51	2.3 (58’)	120’	No change
	SB LT	99	<b>267</b>	86	6.6 (165’)	N/A	175’
	SB RT	83	<b>223</b>	72	4.5(113’)	N/A	125’
McDowell Road and 64 <sup>th</sup> Street	NB RT	180	<b>393</b>	193	10.0 (250’)	215’	265’
	SB LT	190	<b>222</b>	133	8.5 (213’)	210’	235’
	SB RT	<b>329</b>	273	254	8.4 (210’)	150’	225’
	EB LT	311	<b>371</b>	296	9.8 (245’)	280’	No change
	WB LT	342	<b>397</b>	138	11.8 (295’)	280’	*
	WB RT	192	<b>206</b>	161	4.1 (103’)	700’	No change
64 <sup>th</sup> Street and Road A	SB LT	<b>153</b>	50	51	0.9 (23’)	100’	No change
	NB RT	<b>102</b>	33	33	0.0 (0’)	None	100’**
	WB LT	33	<b>89</b>	29	3.6 (90’)	N/A	100’
	WB RT	50	<b>134</b>	43	1.5 (38’)	N/A	100’**

\*Due to proximity of the Main Access and 64<sup>th</sup> Street intersections with McDowell Road, a possibility of extending the eastbound left turn bay at Main Access and the westbound left turn bay at 64<sup>th</sup> Street is limited. Dual left turn lanes were considered, however left turn signal phasing must be changed from permissive + protective to protective only, which reduces signal operations at these locations. It is recommended that the existing median be modified to provide equal storage capacity for these two back-to-back left turn bays.

\*\*100 feet meets minimum storage length requirements for City of Scottsdale



Figure 14 – Recommended Storage Lengths

## Conclusions and Recommendations

### McDowell Road and 64<sup>th</sup> Street

This existing signalized intersection will operate at an acceptable overall LOS D or better in 2025 with the proposed site. It is recommended that the northbound right turn storage length on 64<sup>th</sup> Street be increased from 215' to 265', the southbound left turn storage length be increased from 210' to 235', and the southbound right turn storage length be increased from 150' to 225'. Additionally, it is recommended the existing median between 64<sup>th</sup> Street and Main Access intersections on McDowell Road be modified to provide equal storage lengths for the westbound left turn lane at 64<sup>th</sup> Street and the eastbound left turn lane at Main Access.

### McDowell Road and Main Access

This T-intersection warrants a traffic signal with the development between 40% and 50% built out. Therefore, in 2025, under full build conditions, the intersection will warrant a signal. The 2025 analyses of this signalized intersection indicate that it will operate at an acceptable LOS C or better during all three analyzed peak hours. It is recommended that this intersection be monitored to determine at what phase of the development a signal is warranted at this location. In the interim, a "pork chop" median design will be constructed to provide a safer left turn into and out of the site.

It is recommended that if southbound left and right turn length storages be provided, they should be 175' and 125', respectively. Additionally, as stated above, it is recommended the existing median between 64<sup>th</sup> Street and Main Access intersections on McDowell Road be modified to provide equal storage lengths for the westbound left turn lane at 64<sup>th</sup> Street and the eastbound left turn lane at Main Access.

### 64<sup>th</sup> Street and Road A

This T-intersection operates at an acceptable LOS C or better as unsignalized in 2025, with the exception of the westbound left turn movement which operates at lower LOS E and LOS F. However, with the modified median to provide a refuge area for outbound left turning vehicles, the actual operation of this configuration is expected to be better than what is shown in this analysis. The southbound left turn storage length of 100' is sufficient. It is recommended that an exclusive northbound right turn deceleration lane be provided on 64<sup>th</sup> Street at Road A with a minimum of 100' of storage to meet the City of Scottsdale standards. Additionally, it is recommended that should westbound left and right turn storage lengths be provided on Road A, they be 100' each in accordance with City of Scottsdale requirements.

### Road A and Road B at McDowell Road

Both of these driveways operate acceptably under all peak hours as right-in/right-out accesses. Exclusive westbound right turn lanes on McDowell Road at Road A and Road B are not required as a result of the operational analysis.

### Proposed Pedestrian and Bicycle Facilities

The sidewalks along the frontage of the development will be maintained to provide a connection between the multi-use path parallel to the Cross Cut Canal and to the bike lanes along 64<sup>th</sup> Street, Galvin Parkway, and McDowell Road west of 64<sup>th</sup> Street. Additional pedestrian connections will be provided from the development connecting to the Cross Cut Canal on the north and east sides of the development so that non-vehicular traffic from within the development can easily access the existing multi-use paths.



## ATTACHMENT D – TRIP GENERATION





Trip Generation Calculations

710 General Office Building																						
Land Use	ITE Code	Qty	Unit	Weekday			AM Peak Hour			PM Peak Hour			Weekday			AM Peak Hour			PM Peak Hour			
				Rate	% In	% Out	Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
General Office Building	710	250.0	1000 SF GFA	9.74	50%	50%	1.16	86%	14%	1.15	16%	84%	2,435	1218	1217	290	249	41	288	46	242	Average
General Office Building	710	250.0	1000 SF GFA	2.71	50%	50%	0.37	86%	14%	0.47	16%	84%	678	339	339	93	80	13	118	19	99	Minimum
General Office Building	710	250.0	1000 SF GFA	27.56	50%	50%	4.23	86%	14%	3.23	16%	84%	6,890	3,445	3,445	1,058	910	148	808	129	679	Maximum
Land Use	ITE Code	Qty	Unit	Weekday			AM Peak Hour			PM Peak Hour			Weekday			AM Peak Hour			PM Peak Hour			
General Office Building	710	250.0	1000 SF GFA	Ln(T)=0.97Ln(X)+2.50			T=0.94(X)+26.49			Ln(T)=0.95Ln(X)+0.36			Total	In	Out	Total	In	Out	Total	In	Out	Equation
				50%	50%		86%	14%		16%	84%		2,581	1,291	1,290	261	224	37	272	44	228	

General Office Building		Standard Deviation	5.15	0.47	0.42
		Number of Studies	66	35	32
		Average Size	171	117	114
		R <sup>2</sup>	0.83	0.85	0.88

321 Multifamily Housing																						
Land Use	ITE Code	Qty	Unit	Weekday			AM Peak Hour			PM Peak Hour			Weekday			AM Peak Hour			PM Peak Hour			
				Rate	% In	% Out	Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
Multifamily Housing (Mid-Rise)	221	722	Dwelling Units	5.44	50%	50%	0.36	26%	74%	0.44	61%	39%	3,928	1964	1964	260	68	192	318	194	124	Average
Multifamily Housing (Mid-Rise)	221	722	Dwelling Units	1.27	50%	50%	0.06	26%	74%	0.15	61%	39%	917	459	458	43	11	32	108	66	42	Minimum
Multifamily Housing (Mid-Rise)	221	722	Dwelling Units	12.50	50%	50%	1.61	26%	74%	1.11	61%	39%	9,025	4513	4512	1,162	302	860	801	489	312	Maximum
Land Use	ITE Code	Qty	Unit	Weekday			AM Peak Hour			PM Peak Hour			Weekday			AM Peak Hour			PM Peak Hour			
Multifamily Housing (Mid-Rise)	221	722	Dwelling Units	T=5.45(X)-1.75			Ln(T)=0.98Ln(X)-0.98			Ln(T)=0.96Ln(X)-0.63			Total	In	Out	Total	In	Out	Total	In	Out	Equation
				50%	50%		26%	74%		61%	39%		3,933	1,967	1,966	238	62	176	296	181	115	

Multifamily Housing (Mid-Rise)		Standard Deviation	2.03	0.19	0.19
		Number of Studies	27	53	60
		Average Size	205	207	208
		R <sup>2</sup>	0.77	0.67	0.72

820 Shopping Center																						
Land Use	ITE Code	Qty	Unit	Weekday			AM Peak Hour			PM Peak Hour			Weekday			AM Peak Hour			PM Peak Hour			
				Rate	% In	% Out	Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
Shopping Center	820	3.5	1000 SF GLA	37.75	50%	50%	0.94	62%	38%	3.81	48%	52%	132	66	66	3	2	1	13	6	7	Average
Shopping Center	820	3.5	1000 SF GLA	7.42	50%	50%	0.18	62%	38%	0.74	48%	52%	26	13	13	1	1	0	3	1	2	Minimum
Shopping Center	820	3.5	1000 SF GLA	207.98	50%	50%	23.74	62%	38%	18.69	48%	52%	728	364	364	83	51	32	65	31	34	Maximum
Land Use	ITE Code	Qty	Unit	Weekday			AM Peak Hour			PM Peak Hour			Weekday			AM Peak Hour			PM Peak Hour			
Shopping Center	820	3.5	1000 SF GLA	Ln(T)=0.68Ln(X)+5.57			T=0.50(X)+151.78			Ln(T)=0.74Ln(X)+2.89			Total	In	Out	Total	In	Out	Total	In	Out	Equation
				50%	50%		62%	38%		48%	52%		615	308	307	154	95	59	45	22	23	

Shopping Center		Standard Deviation	16.41	0.87	2.04
		Number of Studies	147	84	261
		Average Size	453	351	327
		R <sup>2</sup>	0.76	0.50	0.82

932 High-Turnover (Sit-Down) Restaurant																						
Land Use	ITE Code	Qty	Unit	Weekday			AM Peak Hour			PM Peak Hour			Weekday			AM Peak Hour			PM Peak Hour			
				Rate	% In	% Out	Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
High-Turnover (Sit-Down) Restaurant	932	10	1000 SF GLA	112.18	50%	50%	9.94	55%	45%	9.77	62%	38%	1,122	561	561	99	54	45	98	61	37	Average
High-Turnover (Sit-Down) Restaurant	932	10	1000 SF GLA	13.04	50%	50%	0.76	55%	45%	0.92	62%	38%	130	65	65	8	4	4	9	6	3	Minimum
High-Turnover (Sit-Down) Restaurant	932	10	1000 SF GLA	742.41	50%	50%	102.39	55%	45%	62.00	62%	38%	7,424	3,712	3,712	1,024	563	461	620	384	236	Maximum
Land Use	ITE Code	Qty	Unit	Weekday			AM Peak Hour			PM Peak Hour			Weekday			AM Peak Hour			PM Peak Hour			
				Equation	% In	% Out	Equation	% In	% Out	Equation	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
High-Turnover (Sit-Down) Restaurant	932	10	1000 SF GLA	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	Equation

High-Turnover (Sit-Down) Restaurant	Standard Deviation	72.51		11.33		7.37	
	Number of Studies	50		39		107	
	Average Size	5		5		6	
	R <sup>2</sup>	N/A		N/A		N/A	

936 Coffee/Donut Shop without Drive-Through																						
Land Use	ITE Code	Qty	Unit	Weekday			AM Peak Hour			PM Peak Hour			Weekday			AM Peak Hour			PM Peak Hour			
				Rate	% In	% Out	Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
Coffee/Donut Shop without Drive-Through	936	1.5	1000 SF GLA	754.55	50%	50%	101.14	51%	49%	36.31	50%	50%	1,132	566	566	152	78	74	54	27	27	Average
Coffee/Donut Shop without Drive-Through	936	1.5	1000 SF GLA	754.55	50%	50%	38.76	51%	49%	15.5	50%	50%	1,132	566	566	58	30	28	23	12	11	Minimum
Coffee/Donut Shop without Drive-Through	936	1.5	1000 SF GLA	754.55	50%	50%	255.48	51%	49%	74.84	50%	50%	1,132	566	566	383	195	188	112	56	56	Maximum
Land Use	ITE Code	Qty	Unit	Weekday			AM Peak Hour			PM Peak Hour			Weekday			AM Peak Hour			PM Peak Hour			
				Equation	% In	% Out	Equation	% In	% Out	Equation	% In	% Out	Total	In	Out	Total	In	Out	Total	In	Out	
Coffee/Donut Shop without Drive-Through	936	1.5	1000 SF GLA	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	Equation

Coffee/Donut Shop without Drive-Through	Standard Deviation	N/A		43.44		13.22	
	Number of Studies	1		21		12	
	Average Size	1		2		2	
	R <sup>2</sup>	N/A		N/A		N/A	





Trip Generation Calculations

710 General Office Building															
Land Use	ITE Code	Qty	Unit	Saturday			Saturday Peak Hour			Saturday			Saturday Peak Hour		
				Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out
General Office Building	710	250.0	1000 SF GFA	2.21	50%	50%	0.53	54%	46%	553	277	276	133	72	61
General Office Building	710	250.0	1000 SF GFA	1.24	50%	50%	0.3	54%	46%	310	155	155	75	41	34
General Office Building	710	250.0	1000 SF GFA	7.46	50%	50%	1.57	54%	46%	1,865	933	932	393	212	181
Land Use	ITE Code	Qty	Unit	Saturday			Saturday Peak Hour			Saturday			Saturday Peak Hour		
General Office Building	710	250.0	1000 SF GFA	Equation	% In	% Out	Equation	% In	% Out	Total	In	Out	Total	In	Out
General Office Building	710	250.0	1000 SF GFA	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

Average  
Minimum  
Maximum  
Equation

General Office Building	Standard Deviation	1.70		0.52	
	Number of Studies	5		3	
	Average Size	94		82	
	R <sup>2</sup>	N/A		N/A	

221 Multifamily Housing (Three to Ten Levels)															
Land Use	ITE Code	Qty	Unit	Saturday			Saturday Peak Hour			Saturday			Saturday Peak Hour		
				Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out
Multifamily Housing (Mid-Rise)	221	722	Dwelling Units	4.91	50%	50%	0.44	49%	51%	3,545	1773	1772	318	156	162
Multifamily Housing (Mid-Rise)	221	722	Dwelling Units	4.03	50%	50%	0.34	49%	51%	2,910	1455	1455	245	120	125
Multifamily Housing (Mid-Rise)	221	722	Dwelling Units	8.51	50%	50%	0.73	49%	51%	6,144	3072	3072	527	258	269
Land Use	ITE Code	Qty	Unit	Saturday			Saturday Peak Hour			Saturday			Saturday Peak Hour		
Multifamily Housing (Mid-Rise)	221	722	Dwelling Units	Equation	% In	% Out	Equation	% In	% Out	Total	In	Out	Total	In	Out
Multifamily Housing (Mid-Rise)	221	722	Dwelling Units	T=3.04(X)+417.11	50%	50%	T=0.42(X)+6.73	49%	51%	2,612	1,306	1,306	310	152	158

Average  
Minimum  
Maximum  
Equation

Multifamily Housing (Mid-Rise)	Standard Deviation	1.26		0.08	
	Number of Studies	6		8	
	Average Size	224		264	
	R <sup>2</sup>	0.73		0.89	

820 Shopping Center															
Land Use	ITE Code	Qty	Unit	Saturday			Saturday Peak Hour			Saturday			Saturday Peak Hour		
				Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out
Shopping Center	820	3.5	1000 SF GLA	46.12	50%	50%	4.5	52%	48%	161	81	80	16	8	8
Shopping Center	820	3.5	1000 SF GLA	13.01	50%	50%	1.42	52%	48%	46	23	23	5	3	2
Shopping Center	820	3.5	1000 SF GLA	167.89	50%	50%	15.1	52%	48%	588	294	294	53	28	25
Land Use	ITE Code	Qty	Unit	Saturday			Saturday Peak Hour			Saturday			Saturday Peak Hour		
Shopping Center	820	3.5	1000 SF GLA	Equation	% In	% Out	Equation	% In	% Out	Total	In	Out	Total	In	Out
Shopping Center	820	3.5	1000 SF GLA	Ln(T)=0.62Ln(X)+6.24	50%	50%	Ln(T)=0.79Ln(X)+2.79	52%	48%	1,115	558	557	44	23	21

Average  
Minimum  
Maximum  
Equation

Shopping Center	Standard Deviation	17.91		1.88	
	Number of Studies	58		119	
	Average Size	602		416	
	R <sup>2</sup>	0.71		0.87	

932 High-Turnover (Sit-Down) Restaurant																
Land Use	ITE Code	Qty	Unit	Saturday			Saturday Peak Hour			Saturday			Saturday Peak Hour			
				Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out	
High-Turnover (Sit-Down) Restaurant	932	10	1000 SF GLA	122.4	50%	50%	11.19	51%	49%	1,224	612	612	112	57	55	Average
High-Turnover (Sit-Down) Restaurant	932	10	1000 SF GLA	101.99	50%	50%	1.63	51%	49%	1,020	510	510	16	8	8	Minimum
High-Turnover (Sit-Down) Restaurant	932	10	1000 SF GLA	173.07	50%	50%	50.4	51%	49%	1,731	866	865	504	257	247	Maximum
Land Use	ITE Code	Qty	Unit	Saturday			Saturday Peak Hour			Saturday			Saturday Peak Hour			
				Equation	% In	% Out	Equation	% In	% Out	Total	In	Out	Total	In	Out	
High-Turnover (Sit-Down) Restaurant	932	10	1000 SF GLA	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	Equation

High-Turnover (Sit-Down) Restaurant	Standard Deviation	36.99		8.3	
	Number of Studies	3		22	
	Average Size	6		5	
	R <sup>2</sup>	N/A		N/A	

936 Coffee/Donut Shop without Drive-Through																
Land Use	ITE Code	Qty	Unit	Saturday			Saturday Peak Hour			Saturday			Saturday Peak Hour			
				Rate	% In	% Out	Rate	% In	% Out	Total	In	Out	Total	In	Out	
Coffee/Donut Shop without Drive-Through	936	1.5	1000 SF GLA	673.64	50%	50%	59.01	49%	51%	1,010	505	505	89	44	45	Average
Coffee/Donut Shop without Drive-Through	936	1.5	1000 SF GLA	673.64	50%	50%	36.87	49%	51%	1,010	505	505	55	27	28	Minimum
Coffee/Donut Shop without Drive-Through	936	1.5	1000 SF GLA	673.64	50%	50%	117.42	49%	51%	1,010	505	505	176	86	90	Maximum
Land Use	ITE Code	Qty	Unit	Saturday			Saturday Peak Hour			Saturday			Saturday Peak Hour			
				Equation	% In	% Out	Equation	% In	% Out	Total	In	Out	Total	In	Out	
Coffee/Donut Shop without Drive-Through	936	1.5	1000 SF GLA	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	Equation

Coffee/Donut Shop without Drive-Through	Standard Deviation	N/A		30.5	
	Number of Studies	1		5	
	Average Size	1		2	
	R <sup>2</sup>	N/A		N/A	

NCHRP 8-51 Internal Trip Capture Estimation Tool						
Project Name:	Entrada Scottsdale			Organization:	Lokahi, LLC	
Project Location:	City of Scottsdale			Performed By:	SAS	
Scenario Description:				Date:	8/7/2019	
Analysis Year:				Checked By:		
Analysis Period:	AM Street Peak Hour			Date:		

Table 1-A: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)						
Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips		
	ITE LUCs <sup>1</sup>	Quantity	Units	Total	Entering	Exiting
Office	710	250	1000 SF GFA	290	249	41
Retail	820	3.5	1000 SF GLA	3	2	1
Restaurant	932/936	11.5	1000 SF GLA	251	132	119
Cinema/Entertainment				0		
Residential	221	722	Dwelling Units	260	68	192
Hotel				0		
All Other Land Uses <sup>2</sup>				0		
<b>Total</b>				<b>804</b>	<b>451</b>	<b>353</b>

Table 2-A: Mode Split and Vehicle Occupancy Estimates						
Land Use	Entering Trips			Exiting Trips		
	Veh. Occ.	% Transit	% Non-Motorized	Veh. Occ.	% Transit	% Non-Motorized
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						
All Other Land Uses <sup>2</sup>						

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						

Table 4-A: Internal Person-Trip Origin-Destination Matrix*						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		1	26	0	0	0
Retail	0		0	0	0	0
Restaurant	35	0		0	3	0
Cinema/Entertainment	0	0	0		0	0
Residential	4	0	26	0		0
Hotel	0	0	0	0	0	

Table 5-A: Computations Summary			
	Total	Entering	Exiting
All Person-Trips	804	451	353
Internal Capture Percentage	24%	21%	27%
External Vehicle-Trips <sup>3</sup>	614	356	258
External Transit-Trips <sup>4</sup>	0	0	0
External Non-Motorized Trips <sup>4</sup>	0	0	0

Table 6-A: Internal Trip Capture Percentages by Land Use		
Land Use	Entering Trips	Exiting Trips
Office	16%	66%
Retail	50%	0%
Restaurant	39%	32%
Cinema/Entertainment	N/A	N/A
Residential	4%	16%
Hotel	N/A	N/A

<sup>1</sup>Land Use Codes (LUCs) from *Trip Generation Informational Report*, published by the Institute of Transportation Engineers.

<sup>2</sup>Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator

<sup>3</sup>Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A

<sup>4</sup>Person-Trips

\*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas Transportation Institute

<b>Project Name:</b>	Entrada Scottsdale
<b>Analysis Period:</b>	AM Street Peak Hour

Table 7-A: Conversion of Vehicle-Trip Ends to Person-Trip Ends						
Land Use	Table 7-A (D): Entering Trips			Table 7-A (O): Exiting Trips		
	Veh. Occ.	Vehicle-Trips	Person-Trips*	Veh. Occ.	Vehicle-Trips	Person-Trips*
Office	1.00	249	249	1.00	41	41
Retail	1.00	2	2	1.00	1	1
Restaurant	1.00	132	132	1.00	119	119
Cinema/Entertainment	1.00	0	0	1.00	0	0
Residential	1.00	68	68	1.00	192	192
Hotel	1.00	0	0	1.00	0	0

Table 8-A (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		11	26	0	0	0
Retail	0		0	0	0	0
Restaurant	37	17		0	5	4
Cinema/Entertainment	0	0	0		0	0
Residential	4	2	38	0		0
Hotel	0	0	0	0	0	

Table 8-A (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		1	30	0	0	0
Retail	10		66	0	1	0
Restaurant	35	0		0	3	0
Cinema/Entertainment	0	0	0		0	0
Residential	7	0	26	0		0
Hotel	7	0	8	0	0	

Table 9-A (D): Internal and External Trips Summary (Entering Trips)						
Destination Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles <sup>1</sup>	Transit <sup>2</sup>	Non-Motorized <sup>2</sup>
Office	39	210	249	210	0	0
Retail	1	1	2	1	0	0
Restaurant	52	80	132	80	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	3	65	68	65	0	0
Hotel	0	0	0	0	0	0
All Other Land Uses <sup>3</sup>	0	0	0	0	0	0

Table 9-A (O): Internal and External Trips Summary (Exiting Trips)						
Origin Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles <sup>1</sup>	Transit <sup>2</sup>	Non-Motorized <sup>2</sup>
Office	27	14	41	14	0	0
Retail	0	1	1	1	0	0
Restaurant	38	81	119	81	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	30	162	192	162	0	0
Hotel	0	0	0	0	0	0
All Other Land Uses <sup>3</sup>	0	0	0	0	0	0

<sup>1</sup>Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A

<sup>2</sup>Person-Trips

<sup>3</sup>Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator

\*Indicates computation that has been rounded to the nearest whole number.

NCHRP 8-51 Internal Trip Capture Estimation Tool						
Project Name:	Entrada Scottsdale			Organization:	Lokahi, LLC	
Project Location:	City of Scottsdale			Performed By:	SAS	
Scenario Description:				Date:	8/7/2019	
Analysis Year:				Checked By:		
Analysis Period:	PM Street Peak Hour			Date:		

Table 1-P: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)						
Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips		
	ITE LUCs <sup>1</sup>	Quantity	Units	Total	Entering	Exiting
Office	710	250	1000 SF GFA	288	46	242
Retail	820	3.5	1000 SF GLA	13	6	7
Restaurant	932/936	11.5	1000 SF GLA	152	88	64
Cinema/Entertainment				0		
Residential	221	722	Dwelling Units	318	194	124
Hotel				0		
All Other Land Uses <sup>2</sup>				0		
<b>Total</b>				<b>771</b>	<b>334</b>	<b>437</b>

Table 2-P: Mode Split and Vehicle Occupancy Estimates						
Land Use	Entering Trips			Exiting Trips		
	Veh. Occ.	% Transit	% Non-Motorized	Veh. Occ.	% Transit	% Non-Motorized
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						
All Other Land Uses <sup>2</sup>						

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						

Table 4-P: Internal Person-Trip Origin-Destination Matrix*						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		0	2	0	5	0
Retail	0		2	0	2	0
Restaurant	2	3		0	12	0
Cinema/Entertainment	0	0	0		0	0
Residential	5	1	12	0		0
Hotel	0	0	0	0	0	

Table 5-P: Computations Summary			
	Total	Entering	Exiting
All Person-Trips	771	334	437
Internal Capture Percentage	12%	14%	11%
External Vehicle-Trips <sup>3</sup>	679	288	391
External Transit-Trips <sup>4</sup>	0	0	0
External Non-Motorized Trips <sup>4</sup>	0	0	0

Table 6-P: Internal Trip Capture Percentages by Land Use		
Land Use	Entering Trips	Exiting Trips
Office	15%	3%
Retail	67%	57%
Restaurant	18%	27%
Cinema/Entertainment	N/A	N/A
Residential	10%	15%
Hotel	N/A	N/A

<sup>1</sup>Land Use Codes (LUCs) from *Trip Generation Informational Report*, published by the Institute of Transportation Engineers.

<sup>2</sup>Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator

<sup>3</sup>Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P

<sup>4</sup>Person-Trips

\*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas Transportation Institute

<b>Project Name:</b>	Entrada Scottsdale
<b>Analysis Period:</b>	PM Street Peak Hour

Table 7-P: Conversion of Vehicle-Trip Ends to Person-Trip Ends						
Land Use	Table 7-P (D): Entering Trips			Table 7-P (O): Exiting Trips		
	Veh. Occ.	Vehicle-Trips	Person-Trips*	Veh. Occ.	Vehicle-Trips	Person-Trips*
Office	1.00	46	46	1.00	242	242
Retail	1.00	6	6	1.00	7	7
Restaurant	1.00	88	88	1.00	64	64
Cinema/Entertainment	1.00	0	0	1.00	0	0
Residential	1.00	194	194	1.00	124	124
Hotel	1.00	0	0	1.00	0	0

Table 8-P (O): Internal Person-Trip Origin-Destination Matrix (Computed at Origin)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		48	10	0	5	0
Retail	0		2	0	2	0
Restaurant	2	26		5	12	4
Cinema/Entertainment	0	0	0		0	0
Residential	5	52	26	0		4
Hotel	0	0	0	0	0	

Table 8-P (D): Internal Person-Trip Origin-Destination Matrix (Computed at Destination)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		0	2	0	8	0
Retail	14		26	0	89	0
Restaurant	14	3		0	31	0
Cinema/Entertainment	3	0	3		8	0
Residential	26	1	12	0		0
Hotel	0	0	4	0	0	

Table 9-P (D): Internal and External Trips Summary (Entering Trips)						
Destination Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles <sup>1</sup>	Transit <sup>2</sup>	Non-Motorized <sup>2</sup>
Office	7	39	46	39	0	0
Retail	4	2	6	2	0	0
Restaurant	16	72	88	72	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	19	175	194	175	0	0
Hotel	0	0	0	0	0	0
All Other Land Uses <sup>3</sup>	0	0	0	0	0	0

Table 9-P (O): Internal and External Trips Summary (Exiting Trips)						
Origin Land Use	Person-Trip Estimates			External Trips by Mode*		
	Internal	External	Total	Vehicles <sup>1</sup>	Transit <sup>2</sup>	Non-Motorized <sup>2</sup>
Office	7	235	242	235	0	0
Retail	4	3	7	3	0	0
Restaurant	17	47	64	47	0	0
Cinema/Entertainment	0	0	0	0	0	0
Residential	18	106	124	106	0	0
Hotel	0	0	0	0	0	0
All Other Land Uses <sup>3</sup>	0	0	0	0	0	0

<sup>1</sup>Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P

<sup>2</sup>Person-Trips

<sup>3</sup>Total estimate for all other land uses at mixed-use development site-not subject to internal trip capture computations in this estimator

\*Indicates computation that has been rounded to the nearest whole number.