

Drainage Reports

Michael P. Leary, LTD

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Date:	November17, 2020
To:	Joe Phillips, Project Manager
	Meredith Tessier, Senior Planner
From:	Mike Leary
Subject:	DC Ranch Park - shared stormwater detention basin

Colleagues, I hope that you view my following comments as nothing more than an opportunity to solve a significant drainage problem to the City's benefit and not an attempt to obfuscate the plans for constructing post-haste the Bell Road Sports Complex. I understand the City's objective to have the fields ready for parking the January 2022 Barrett-Jackson auction. However, I am concerned that the rush might overlook the opportunity to accomplish significantly more than just the proposed lake in DC Ranch Park.

As background I was the development coordinator for the Hampton Group which acquired the adjoining 125-acre ASLD lease back in 2007 (and defaulted in 2009). As part of the EPICENTER infrastructure master planning by the engineering firm of Coe & Van Loo, it was discovered that the existing 100-year flow through the culverts at the northeast corner of Bell Toad and the Pima Freeway far exceeded its capacity. The culvert has a 100-year capacity between 900 and 1,000 cfs but incoming flows between 1,400 and 1,700 cfs. In essence during a 100-year event the intersection and the up and downstream properties will be flooded and the City might be liable for damages as a result of not having rectified the problem that they have been aware of since 2008.

The use of the DC Ranch Park site to mitigate the significant overage was first approved with the Epicenter Master Drainage Plan in 2008. A highlighted abstract of the Executive Report is below:

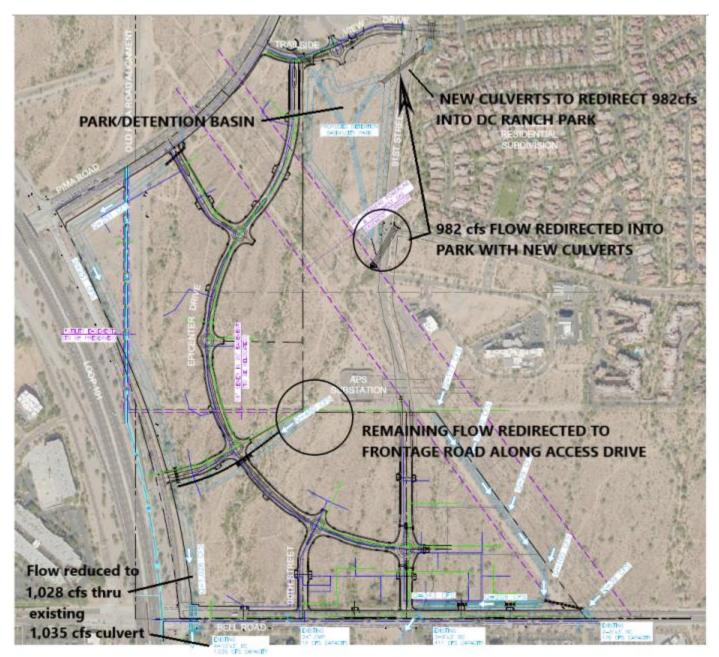
2.0 SCOTTSDALE EPICENTER UPSTREAM DRAINAGE BACKGROUND

In order to gain understanding of the upstream watershed for the site, CVL referred to the drainage reports for DC Ranch (Reference 1) and the Preliminary Drainage Design Report for the Pima Road Realignment Project (Reference 2). According to these reports, the 100-year offsite flows of **491 cfs (CP-C1B)**, **993 cfs (CP-CC2E)**, and **251 cfs (CP1MA3)** are directed through the site toward a four-barrel, 10-foot by 3-foot box culvert located at Bell Road immediately east of the Loop 101 frontage road. In addition, **300 cfs (CP-APII)** from the site itself is directed to the same culvert under the existing non-developed site conditions. Refer to Plate 1 for the location of the HEC-1 concentration points.

In order to calculate the existing 100-year 6-hour flow upstream of the above mentioned box culvert, CVL combined the DC Ranch contributing drainage area, the Pima Road contributing drainage area, and the site's existing condition contributing drainage area in a HEC-1 model and included it within the approved Master Drainage Report for Scottsdale Epicenter (Reference 3). The box culverts, as well as the downstream channel and two additional culvert crossings south of Epicenter, were designed to carry a 100-year flow of 1,035 cfs (References 4 & 5). The 100-year 6-hour flow in the existing condition box culverts is 1,719 cfs (Reference 3). This flow exceeds the capacity of the culvert as well as the channel and three other downstream culverts, causing a potential flood hazard for Scottsdale Epicenter, Bell Road, and properties downstream of Bell Road.

In order to eliminate the flood hazard described above, the City of Scottsdale has agreed to use the future DC Ranch Park (hereafter referred to as the "City of Scottsdale Park"), located at the southwest corner of the intersection of 91st Street and Trailside View Drive, as a detention basin to attenuate the upstream peak flows. Initially, 55 acre-feet of volume was proposed to be detained in this park to reduce the proposed condition 100-year peak flow at the upstream end of the Bell Road culverts to less than 1,035 cfs (Reference 3).

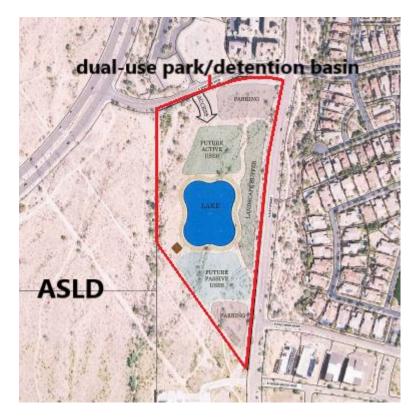
Below is a graphic from Coe & Van Loo illustrating the use of the park as a detention basin to mitigate the existing 100-year flooding (comments in black are mine):



In December 2017 I represented another group wanting to purchase the same ASLD property and had a formal pre-application meeting with City department staff. The use of DC Ranch Park for a drainage basin was reconfirmed by Stormwater's Ashley Couch. If the upstream flow isn't mitigated, the existing 1,400 to 1,700 flow will simply be conveyed through the ASLD property to the undersized Bell Road culverts with the City continuing to have the flooding problem to solve or not to solve.

The latest timeline from ASLD is that the north and south half of the 125-acres will be auctioned the first and second quarter of 2021. For a second time I am the development coordinator for the applicant on the south parcel and part of our the diligence is to confirm again that the DC Ranch Park is available for detention as previously approved.

The point of my letter is to make decision-makers aware that the plan below to construct a first phase lake does not address how the lake and other park improvements work with the long-planned and needed detention basin.



If the park is developed without consideration of the detention basin, the lake/waterlines/site improvements may need to be relocated or reconstructed which would be unnecessary, avoidable and costly. Detention basins for park facilities have been a common and successful dual-use throughout the City. In this case, there might also be another opportunity for a nominal amount of event parking for the January events.

There may be a similar opportunity for dual-use of the Bell Road Fields to absorb some of the flow from the existing wash on the east side thereby reducing the flows into Westworld.

In summary there are multiple benefits to be considered. The City still gets its lake for the Bell Sports Complex. The City eliminates the existing flooding hazard and liability at the Bell/101 culverts. DC Ranch gets greater and earlier park improvements. The Jauary seasonal parking might get to use some portion of the 14-acre park site. All of these benefits could be achieved less expensively if coordinated and not disjointed.

So thems my thoughts. I'm copying quite a few folks so that everyone has the same information. Hope this helps. ML

Cc: City Councilmembers Jim Thompson, City Manager Bill Murphy, Assistant. City Manager Kroy Ekblaw, Special Projects Dan Worth, Executive Director Ashley Couch, Drainage/Flood Control Manager Rich Anderson, Stormwater Engineering Manager Randy Grant, Planning and Development Director Tim Curtis, Current Planning Manager



TECHNICAL MEMORANDUM

To: City of Scottsdale

From: HILGARTWILSON

Date: March 13th, 2017

RE: Great Hearts Academies – Scottsdale Preparatory Field Preliminary Drainage Analysis

Introduction and Background

This technical memorandum has been prepared in support of the Great Hearts Academies (GHA) proposed athletic field (the Project) located southwest of Trailside View and 91st Street, east of Pima Road and the 101 Highway, in the City of Scottsdale (the City). Refer to Figure 1 (Vicinity Map). Specifically, this memorandum discusses the regulatory framework in which the Project will be designed in accordance with as well as preliminary drainage considerations.

Future drainage reports will be prepared in accordance with the most current version of the City of Scottsdale (COS) Design Standards & Policies Manual (DSPM) and the Flood Control District of Maricopa County's (FCDMC) current versions of the Drainage Policies and Standards (DPSM), Drainage Design Manuals (DDM) for Maricopa County, - Hydrology and - Hydraulics.

Hydrologic Flow Determination

In July 2013, T.Y. Lin prepared the *Pinnacle Peak South Area Drainage Master Study Draft* (ADMS) (T.Y. Lin 2013) on behalf of the FCDMC that details the hydrologic and hydraulic conditions of a 45 square mile drainage area extending from Frank Lloyd Wright Boulevard to Dynamite Boulevard and from Scottsdale Road into the McDowell Mountains. The primary goal of the study was to characterize regional flows using the most up to date hydrologic study of the watershed referencing the most current land uses, rainfall data, detailed topography, and analysis. The hydrologic and hydraulic analysis for the study area was completed using FLO-2D, a volume conserving, two-dimensional flood routing modeling program best suited for simulating shallow, distributary flows within a watershed. The ADMS reported a worst case scenario flow of 488 cfs entering the northwest corner of the Project via box culverts beneath, as well as overtopping, Trailside View from the north. The ADMS also reported smaller flows for the 100-year, 6-hour and 24-hour events of 217cfs and 257cfs respectively. FLO-2D results from the ADMS are included as Attachment 1.

Site Location Relative to Known FEMA Flood Hazard Zones

The property is located within the Federal Emergency Management Agency's (FEMA) Special Flood Hazard Area (SFHA) Zone AO as delineated on the FEMA Flood Insurance Rate Maps (FIRMs) panel number 04013C1320L, revised October 16, 2013, which are presented on Figure 2 (FEMA Flood Map). The SFHA Zone associated with this Project is defined below.

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4-ZN- 14-UP-2020 3/14, 10/6/2020

Zone AO:

The flood insurance rate zone that corresponds to areas subject to inundation by 1-percent annual chance (100-year storm event) shallow flooding (usually sheet flow on sloping terrain) where average depths are between one and three feet. Average flood depths derived from detailed hydraulic analyses are shown in this zone.

As the entire Project is located in a FEMA SFHA AO Zone, lowest floor elevations of nonresidential structures shall be elevated to conform to Chapter 37, Article I. Floodplain Management - Special Flood Hazard Areas, Section 37-22 of the Scottsdale Revised Ordinance Code.

Hydraulic Analysis

The 100-year water surface extents of the watercourse through the site were determined in order to identify the anticipated flooding impacts to the proposed field. The preliminary hydraulic analyses for the Project was performed using Hydraflow with parameters referenced from obtained topographic data for the Project. The top widths specified in the Hydraflow results were used to delineate the washes approximate water surface extents. The supporting hydraulic calculations are included as Attachment 2 and the estimated watersurface extents shown on Figure 3 (Drainage Summary Exhibit). It can be seen that based on the current field and parking lot layout, the worst case scenario flows crossing Trailside View do not impact the Project.

Onsite Design Requirements

The proposed Project will comply with the City's required drainage standards as well as Maricopa County Planning and Development Design guidelines and regulations. In accordance with the approved regional drainage report entitled DC Ranch Planning Unit I Part 4 Drainage Plan Study and stormwater storage waiver for the site, onsite stormwater storage is not anticipated to be required.

References

T.Y. Lin, 2013. Pinnacle Peak South Area Drainage Master Study (Draft). Phoenix, Arizona. July 2013.

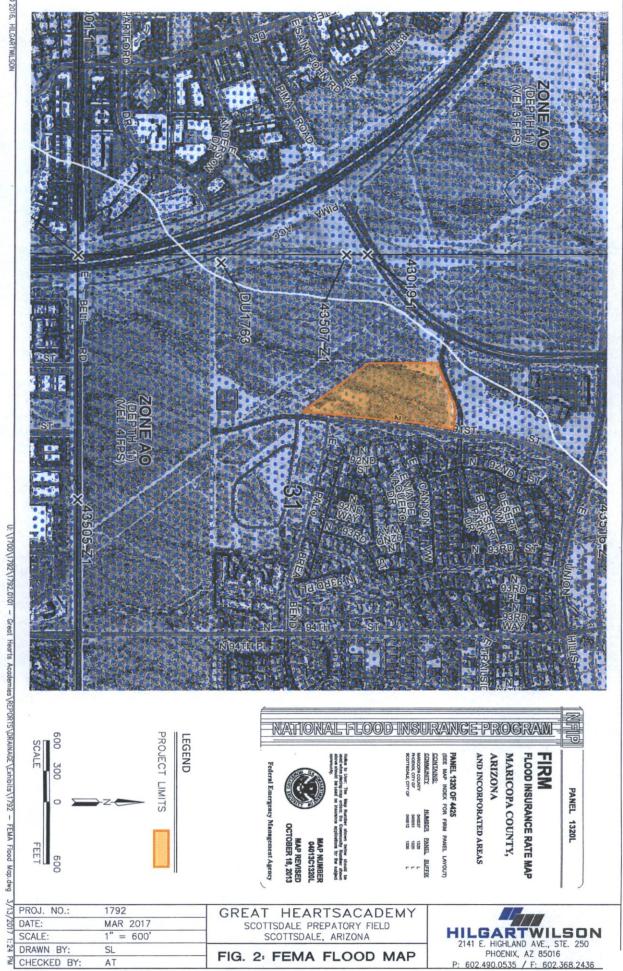
Attachments

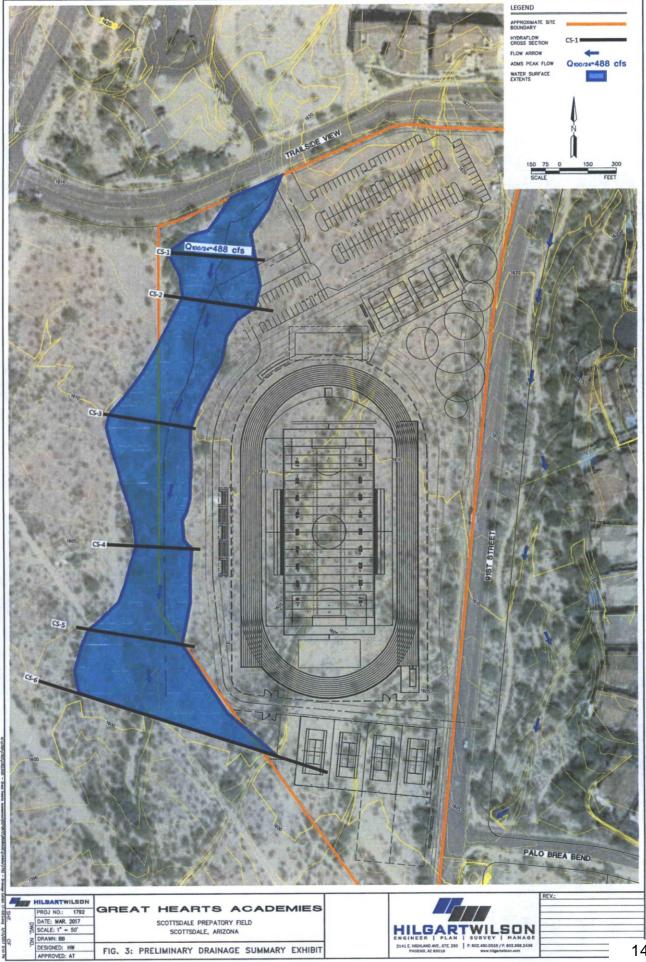
Figure 1: Vicinity Map Figure 2: FEMA Flood Map Figure 3: Drainage Summary Exhibit Attachment 1: Pinnacle Peak South ADMS FLO-2D Results Attachment 2: Hydraflow Calculations

> HILGARTWILSON, LLC 2141 E Highland Avenue, Suite 250 Phoenix, Arizona 85016-4736 Phone: 602-490-0535

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ATTACHMENT 1 PINNACLE PEAK SOUTH ADMS FLO-2D RESULTS

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Flo2D Viewer

Model: 2D_PinnaclePeakSouth/100YR6HR_P

725648	725649	725650	725651	725652	725653	725654	725655	725656	725657	725658	725659	725660 7	25661
88 cfs	1621.34 ft	1622.78 ft	1623.5 ft	1624.56 ft	1625.12 ft	1625.24 ft	1624.9 ft	1622.23 ft	1621.98 ft	4 cfs	9 cfs	13 cfs	15 cfs
726307	726308	726309	726310	726311	726312	726313	726314	726315	726316	726317	726318	726319 7	726320
16 cfs	1622.39 ft	1622.74 ft	1623.51 ft	1624.85 ft	1625.07 ft	1625.22 ft	1624.23 ft	1621.9 ft	1621.09 ft	1 cfs	21 cfs	17 cfs	13 cfs
726967	726968	726969	726970	726971	726972	726973	726974	726975	726976	726977	726978	726979 72	6980
13 cfs	1622.26 ft	1622.56 ft	1622.93 ft	1624.5 ft	1624.94 ft	1624.81 ft	1623.31 ft	1620.62 ft	1619.34 ft	16 cfs	20 cfs	1620.22 ft 162	1.18 fi
727627	727628	727629	727630	727631	727632	727633	727634	727635	727636	727637	727638	727639 72	7640
97 cfs	1620.26 ft	1622.02 ft	1621.93 ft	1622.77 ft	1623.32 ft	1622.06 ft	1621.32 ft	1619.25 ft	4 cfs	25 cfs	3 cfs	1620.36 ft 162	1.01 1
728288	728289	728290	728291	728292	728293	728294	728295	728296	728297	728298	728299	728300 72	8301
84 cfs	74 cfs	1618.59 ft	1620.12 ft	1620 ft	1619.62 ft	1619.6 ft	1618.69 ft	1617.58 ft	17 cfs	16 cfs	1618.56 ft	1619.86 ft 162	0.48 fi
728949	728950	728951	728952	728953	728954	728955	728956	728957	728958	728959	728960	728961 72	8962
1617.54 ft	77 cfs	84 cfs	2 cfs	1616.08 ft	1617.74 ft	1617.53 ft	1617.37 ft	1 cfs	21 cfs	12 cfs	1618.4 ft	1619.42 ft 162	0.03 fi
729611	729612	729613	729614	729615	729616	729617	729618	729619	729620	729621	729622	729623 72	29624
7 cfs	1616.25 ft	65 cfs	123 cfs	56 cfs	1616.48 ft	1616.77 ft	1616.75 ft	9 cfs	23 cfs	1617.2 ft	1618.4 ft	1619.12 ft 16	19.8 fi
730273	730274	E Thaisid	730276	730277	730278	730279	730280	730281	730282	730283	730284	730285 73	0286
6 cfs	1614.99 ft	1614.86 ft	104 cfs	105 cfs	1614.42 ft	1616.42 ft	1616.23 ft	23 cfs	10 cfs	1617.1 ft	1617.98 ft	1618.68 ft 161	.9.31 fi
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5 cfs	1 cfs	1614.85 ft	18 cfs	153 cfs	16 đs	1615.97 ft	1615.75 ft	28 cfs	2 cfs	1616.87 ft	1617.39 ft	1618.06 ft 161	.8.64 fi
731599	731600	731601	731602	731603	731604	731605	731506	731607	731608	731609	731610	731611 73	1612
4 cfs	1613.91 ft	1614.23 ft	5 cfs	126 cfs	55 cfs	1615.46 ft	7 cfs	23 cfs	1615.79 ft	1616.46 ft	1616.75 ft	1617.02 ft 161	.7.64 fi
732263	732264	732265	732266	732267	732268	732269	732270	732271	732272	732273	732274	732275 73	32276
2 cfs	1613.66 ft	1613.98 ft	6 cfs	88 cfs	62 cfs	1614.78 ft	20 cfs	14 cfs	1615.46 ft	1615.65 ft	1615.93 ft	1616.42 ft 1610	.6.94 fi
732927	732928	732929	732930	732931	732932	732933	732934	732935	732936	732937	732938	732939 73	32940
1 cfs	1613.34 ft	1613.71 ft	1613.47 ft	117 cfs	10 cfs	4 cfs	24 cfs	1614.41 ft	1614.84 ft	1614.96 ft	1615.72 ft	1615.92 ft 161	.6.33 fi
733592	733593	733594	733595	733596	733597	733598	733599	733600	733601	733602	733603	733604 73	33605
1612.63 ft	1613.05 ft	1613.33 ft	1 cfs	115 cfs	12 cfs	8 cfs	18 cfs	1614.23 ft	1 cfs	1614.71 ft	1615.54 ft	1615.5 ft 161	15.71 fi
734257	734258	734259	734260	734261	734262	734263	734264	734265	734266	734267	734268	734269 73	34270
1612.44 ft	1612.71 ft	1612.82 ft	5 cfs	114 cfs	12 cfs	16 cfs	10 cfs	1613.73 ft	1 ds	1614.66 ft	1615.47 ft	1615.14 ft 161	4.94 1
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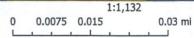
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Flo2D Viewer

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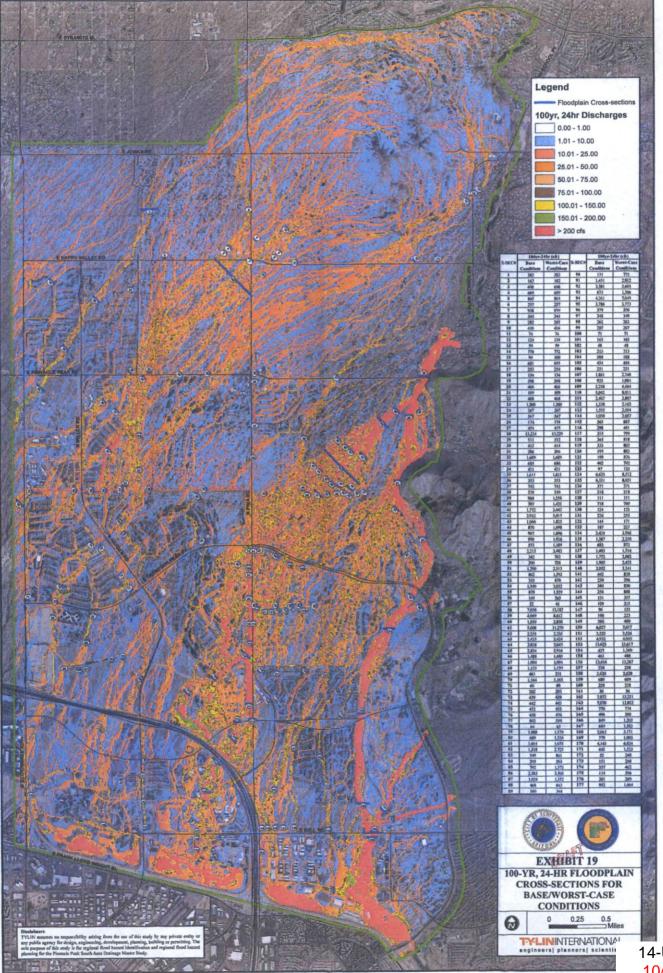
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1615.58 ft	1 cfs	2 cfs	4 cfs	1618.52 ft	97 cfs	86 cfs	1618.59 ft	1620.12 ft	1620 ft	1619,62 ft	1619,59 ft	1618,69 ft	1617.59 ft	22 cfs	20 cfs	1618,58 ft	1619,86 ft	1620.48 ft	1620.56 ft	1620,13 ft	9 cfs	24 cfs	45 cfs	56 cfs	36 cfs	9 cfs
728944	728945	728946	728947	728948	728949	728950	72.8951	728952	728953	728954	728955	728956	728957	728958	728959	728960	728961	728962	728963	728964	728965	728966	728967	728968	728969	728970
1615.06 ft	2 cfs	2 cts	1 cfs	1617.17 ft	1617.54 ft	90 cfs	98 cfs	4 cb	1616.08 ft	1617.74 ft	1617.52 ft	1617.37 ft	2 ds	27 cfs	14 cfs	1618.4 ft	1619,41 ft	1620.03 ft	1620.03 ft	2 cfs	17 cfs	34 cfs	43 cfs	50 cfs	27 cfs	11 cfs
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1 cfs	3 cfs	1 cfs	1615.66 ft	1 cfs	8 cfs	1616.25 ft	77 cfs	139 cfs	51 cfs	1616.48 ft	1616.77 ft	1616.75 ft	13 cfs	27 cfs	1617.2 ft	1618.4 ft	1619.11 ft	1619.8 ft	1619.77 ft	4 cfs	23 efs	36 cfs	41 cfs	47 cfs	20 cfs	11 cfs
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1614.21 ft	3 cfs	1614.77 ft	1615.05 ft	3 cfs	6 cfs	1614.99 ft	1614.87 ft	119 cfs	129 cfs	1614.42 ft	1616.42 ft	1616.26 ft	29 cfs	12 ds	1617.1 ft	1617.97 ft	1618.68 ft	1619.31 ft	1619.32 ft	5 cfs	23 cfs	41 cfs	45 cfs	45 cfs	17 cfs	14 cfs
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2 cfs	3 cfs	1614.39 ft	1614.37 ft	3 cts	5 cfs	1 cfs	1614.85 ft	23 đs	177 ch	18 ds	1615.97 ft	2 cfs	33 cfs	4 cfs	1616.86 ft	1617.38 ft	1618.05 ft	1618.64 ft	1618.83 ft	2 cfs	24 cfs	40 cfs	52 cfs	44 cfs	21 cfs	16 cfs
731594	731595	731596	731597	731598	731599	731600	731601	731602	731603	731604	731605	731606	731607	731608	731609	731610	731611	731612	731613	731614	731615	731616	731617	731618	731619	731620
4 ds	1613.32 ft	1613.71 ft	1613.68 ft	4 cts	5 cfs	1613.91 ft	1614.23 ft	8 cts	143 ds	69 cfs	1615.47 ft	9 cB	28 cts	1615.8 ft	1616.46 ft	1616.74 ft	1617.02 ft	1617.63 ft	1618.12 ft	1 cfs	17 efs	37 ds	73 cfs	39 cfs	21 cfs	21 cfs
732258	732259	732260	732261	732262	732263	732264	732265	732266	732267	732268	732269	732270	732271	732272	732273	732274	732275	732276	732277	732278	732279	732280	732281	732282	732283	732284
2 cfs	1612.83 ft	1612.9 ft	2 cfs	5 cfs	4 cfs	1613.66 ft	1613.98 ft	8 cfs	97 cB	80 cfs	1614.79 ft	25 cfs	17 cfs	1615.46 ft	1615.65 ft	1615.92 ft	1616.41 ft	1616.94 ft	1617.51 ft	1617.78 ft	13 cfs	39 cfs	72 cfs	43 ds	26 cfs	26 cfs
732922	732923	732924	732925	732926	732927	732928	732929	732930	732931	732932	732933	732934	732935	732936	732937	732938	732939	732940	732941	732942	732943	732944	732945	732946	732947	731948
2 0%	1612.11 ft	1 ds	2 cfs	5 0%	1 cfs	1613.34 ft	1613.71 ft	1613.48 ft	136 cfs	14 cfs	6 cfs	28 cfs	3 cfs	1614.83 ft	1614.95 ft	1615.72 ft	1615.92 ft	1616.32 ft	1616.81 ft	1617.04 ft	12 cfs	58 cfs	55 cfs	44 ds	35 cfs	20 cfs
733587	733588	733589	733590	733591	733592	733593	733594	733595	733596	733597	733598	733599	733600	733601	733602	733603	733604	733605	733606	733607	733608	733609	733610	733611	733612	733613
1 ds	1611.78 ft	3 ds	5 cfs	2 cfs	1612.64 ft	1613.06 ft	1613.33 ft	3 ds	132 ds	16 cfs	12 cfs	22 cfs	1614.24 R	1614.31 ft	1614.7 ft	1615.54 ft	1615.49 ft	1615.71 ft	1615.94 ft	1616.05 ft	20 cfs	62 cfs	52 cfs	59 cfs	32 cfs	11 đs
734252	734253	734254	734255	734256	734257	734258	734259	734260	734261	734262	734263	734264	734265	734266	734267	734268	734269	734270	734271	734272	734273	734274	734275	734276	734277	734278
1611.17 ft	1611.15 ft	6 ds	3 cfs	1611.97 ft	1612.45 ft	1612.72 R	1612.83 ft	8 05	128 cfs	16 cfs	20 cB	13 cfs	1613.73 R	1 cħ	1614.65 R	1615.47 ft	1615.13 ft	1614.94 ft	1615.01 ft	2 cfs	44 cts	46 cfs	80 cfs	40 cfs	10 cfs	8 cfs
734918	734919	734920	734921	734922	734923	734924	734925	734926	734927	734928	734929	734930	734931	734932	734933	734934	734935	734936	734937	734938	734939	734940	734941	734942	734943	734944
		and the second se	ALL COMPANY INC.				1					and the second second		C VI DESIGNA				1	The second second second	ALC: CARLES AND A	Very second second		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	and a second	Contraction of the	A COLORADOR



DISCLAIMER: The Flood Control District of Maricopa County (FCD) has made every reasonable effort to obtain and maintain this data as accurately as possible. The FCD assumes no responsibility arising from the use of this information. The data and maps are provided without warranty of any kind, either expressed or implied. The FCD does not guarantee the accuracy, completeness, timeliness or correct sequencing of the data and information requested and hereby expressly disclaims any responsibility for the truth, lack of truth, validity, invalidity, accuracy, inaccuracy, errors or omissions or for the use or results obtained from the use of any said data and information. You, the viewer or user, agree to indemnify the FCD, its officers, and employees from any liability that may arise from any such data or information in its actual or altered form. Any download for commercial intent or resale of this information is prohibited except in accordance with approved FCD policy and Arizona State Statutes 39-121-03. It is ultimately the viewer/users responsibility to verify accuracy prior to acceptance.

2801 W. Durango Street, Phoenix, AZ 85009

PINNACLE PEAK SOUTH ADMS





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ATTACHMENT 2 HYDRAFLOW CALCULATIONS

U:\1700\1792\1792.0101 - Great Hearts Academies\REPORTS\DRAINAGE\Summary Memo\1792 Preliminary Drainage Conditions Memo.docx

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc.

Monday, Mar 13 2017

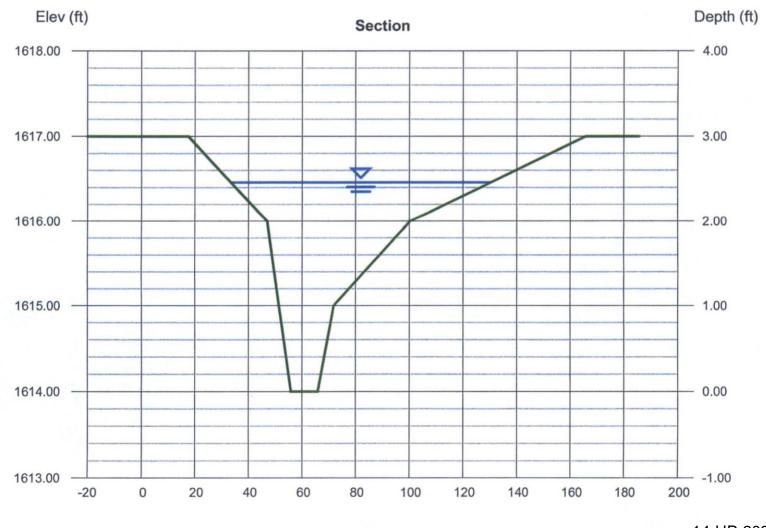
CS-1

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User-defined		Highlighted	
Invert Elev (ft)	= 1614.00	Depth (ft)	= 2.46
Slope (%)	= 1.75	Q (cfs)	= 488.00
N-Value	= 0.032	Area (sqft)	= 86.59
		Velocity (ft/s)	= 5.64
Calculations		Wetted Perim (ft)	= 97.24
Compute by:	Known Q	Crit Depth, Yc (ft)	= 2.51
Known Q (cfs)	= 488.00	Top Width (ft)	= 96.90
		EGL (ft)	= 2.95

(Sta, El, n)-(Sta, El, n)... (0.00, 1617.00)-(17.60, 1617.00, 0.032)-(47.00, 1616.00, 0.032)-(51.40, 1615.00, 0.032)-(55.90, 1614.00, 0.032)-(65.90, 1614.00, 0.032)-(71.80, 1615.00, 0.032) -(100.30, 1616.00, 0.032)-(165.70, 1617.00, 0.032)



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Monday, Mar 13 2017

CS-2

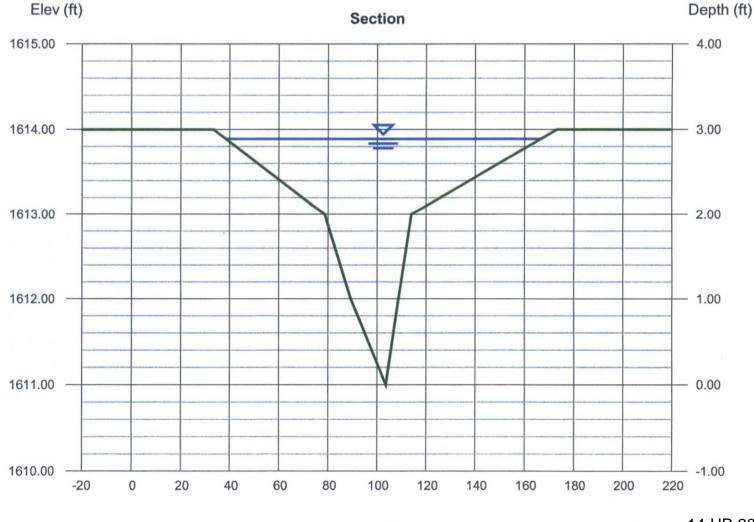
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User-defined		Highlighted	
Invert Elev (ft)	= 1611.00	Depth (ft)	= 2.89
Slope (%)	= 1.13	Q (cfs)	= 488.00
N-Value	= 0.032	Area (sqft)	= 109.68
		Velocity (ft/s)	= 4.45
Calculations		Wetted Perim (ft)	= 128.42
Compute by:	Known Q	Crit Depth, Yc (ft)	= 2.77
Known Q (cfs)	= 488.00	Top Width (ft)	= 128.13
		EGL (ft)	= 3.20

(Sta, El, n)-(Sta, El, n)... (0.00, 1614.00)-(33.40, 1614.00, 0.032)-(78.70, 1613.00, 0.032)-(89.40, 1612.00, 0.032)-(103.60, 1611.00, 0.032)-(108.70, 1612.00, 0.032)-(114.00, 1613.00, 0.032)-(173.00, 1614.00, 0.032)-(199.30, 1614.00, 0.032)



Sta (ft)

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Monday, Mar 13 2017

CS-3

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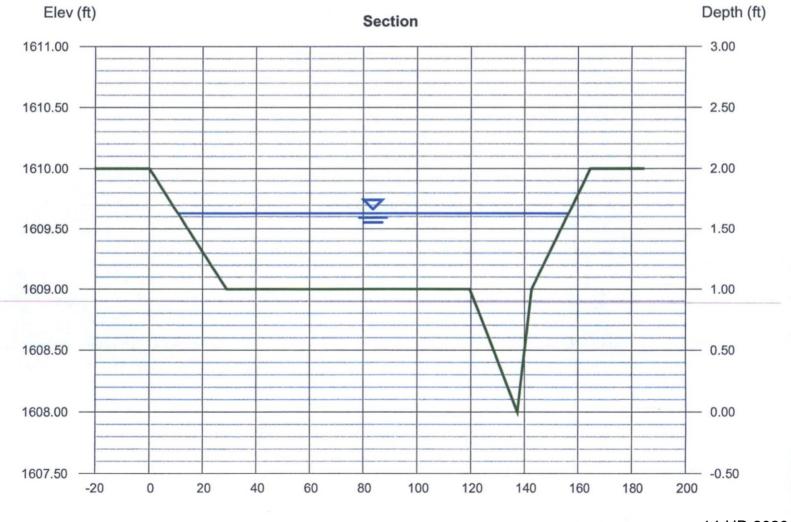
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User-defined		Highlighted	
Invert Elev (ft)	= 1608.00	Depth (ft)	= 1.63
Slope (%)	= 2.33	Q (cfs)	= 488.00
N-Value	= 0.032	Area (sqft)	= 93.31
		Velocity (ft/s)	= 5.23
Calculations		Wetted Perim (ft)	= 145.94
Compute by:	Known Q	Crit Depth, Yc (ft)	= 1.70
Known Q (cfs)	= 488.00	Top Width (ft)	= 145.79
. ,		EGL (ft)	= 2.06

(Sta, El, n)-(Sta, El, n)... (0.00, 1610.00)-(29.10, 1609.00, 0.032)-(119.50, 1609.00, 0.032)-(137.40, 1608.00, 0.032)-(142.70, 1609.00, 0.032)-(164.70, 1610.00, 0.032)



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Monday, Mar 13 2017

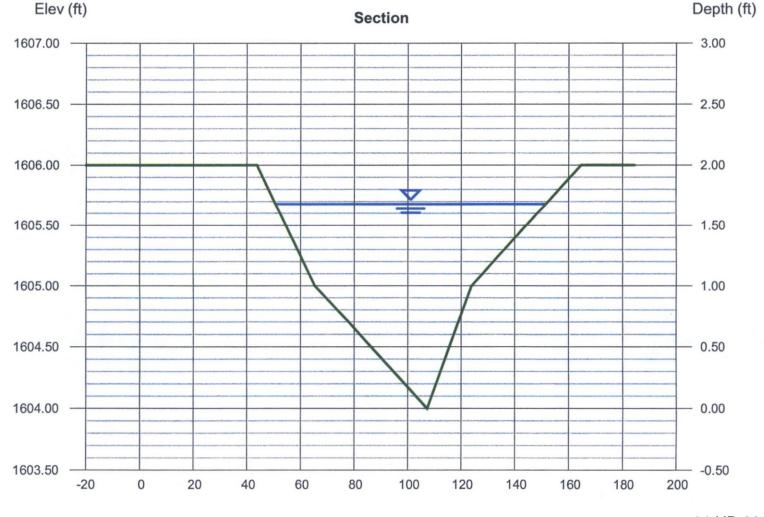
CS-4

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User-defined Invert Elev (ft) Slope (%) N-Value	= 1604.00 = 2.06 = 0.032	Highlighted Depth (ft) Q (cfs) Area (sqft)	= 1.68 = 488.00 = 83.56
Calculations Compute by: Known Q (cfs)	Known Q = 488.00	Velocity (ft/s) Wetted Perim (ft) Crit Depth, Yc (ft) Top Width (ft) EGL (ft)	= 5.84 = 101.03 = 1.77 = 100.97 = 2.21

(Sta, El, n)-(Sta, El, n)... (0.00, 1606.00)-(43.70, 1606.00, 0.032)-(65.30, 1605.00, 0.032)-(107.40, 1604.00, 0.032)-(123.90, 1605.00, 0.032)-(164.60, 1606.00, 0.032)



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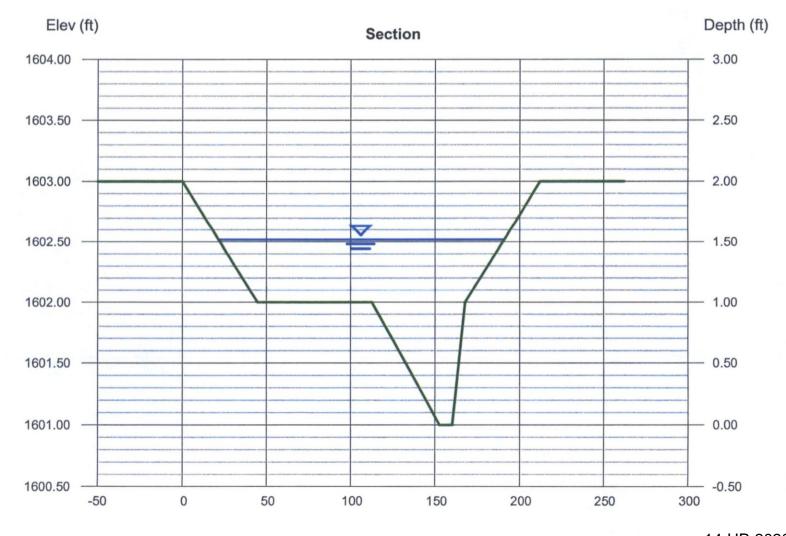
Monday, Mar 13 2017

CS-5

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User-defined		Highlighted		
Invert Elev (ft)	= 1601.00	Depth (ft)	= 1.52	
Slope (%)	= 1.82	Q (cfs)	= 488.00	
N-Value	= 0.032	Area (sqft)	= 107.17	
		Velocity (ft/s)	= 4.55	
Calculations		Wetted Perim (ft)	= 169.48	
Compute by:	Known Q	Crit Depth, Yc (ft)	= 1.53	
Known Q (cfs)	= 488.00	Top Width (ft)	= 169.39	
		EGL (ft)	= 1.84	

(Sta, El, n)-(Sta, El, n)... (0.00, 1603.00)-(44.70, 1602.00, 0.032)-(112.80, 1602.00, 0.032)-(152.80, 1601.00, 0.032)-(160.20, 1601.00, 0.032)-(167.70, 1602.00, 0.032)-(212.20, 1603.00, 0.0



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Monday, Mar 13 2017

CS-6

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User-defined		Highlighted	
Invert Elev (ft)	= 1600.00	Depth (ft)	= 0.45
Slope (%)	= 1.62	Q (cfs)	= 488.00
N-Value	= 0.032	Area (sqft)	= 151.55
		Velocity (ft/s)	= 3.22
Calculations		Wetted Perim (ft)	= 360.69
Compute by:	Known Q	Crit Depth, Yc (ft)	= 0.42
Known Q (cfs)	= 488.00	Top Width (ft)	= 360.65
		EGL (ft)	= 0.61

(Sta, El, n)-(Sta, El, n)... (0.00, 1602.00)-(22.60, 1601.00, 0.032)-(122.10, 1600.00, 0.032)-(435.10, 1600.00, 0.032)-(441.50, 1601.00, 0.032)-(534.40, 1602.00, 0.032)

