CAVASSON

LOCATED NEAR THE SOUTHWEST CORNER OF NORTH HAYDEN ROAD AND EAST LEGACY BOULEVARD

PRELIMINARY DRAINAGE REPORT- GRAYHAWK RESIDENCES AT CAVASSON

Basis of Design

Plan # 24-DR-2021
Case #
Q-S #
☒ Accepted
☐ Corrections
N.Baronas 1/4/2022
Reviewed By Date

June 14, 2021
Revised: August 30, 2021
Revised: December 2, 2021

Project No.: 18114-601

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CERTIFICATE NO.
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Expires 03-31-2024

HUBBARD ENGINEERING

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1. INTRODUCTION

1.1 Project Scope

This report presents the results of a *Final Drainage Study* conducted by Hubbard Engineering at the request of GDI ML Cavasson ("client"), for The Grayhawk Residences at Cavasson development ("site"). The purpose of this report is to provide a hydrologic evaluation for the site as required by the City of Scottsdale per Ordinances 4346 and 4347. This report addresses off-site and on-site conditions. Drainage calculations and methodologies conform to the City of Scottsdale requirements and standards and to Hubbard Engineering's submitted *Master Drainage Report* for the Cavasson development.

This report is focused on providing practical design information, evaluation, and calculations for statistical flood events up to and including the 100-year frequency flood. The procedures used herein are derived from and performed with currently accepted engineering methodologies and practices. Additionally, the criteria for this evaluation are designed to conform to currently applicable ordinances, regulations and policies affected by the appropriate jurisdictional regulatory authorities for the site.

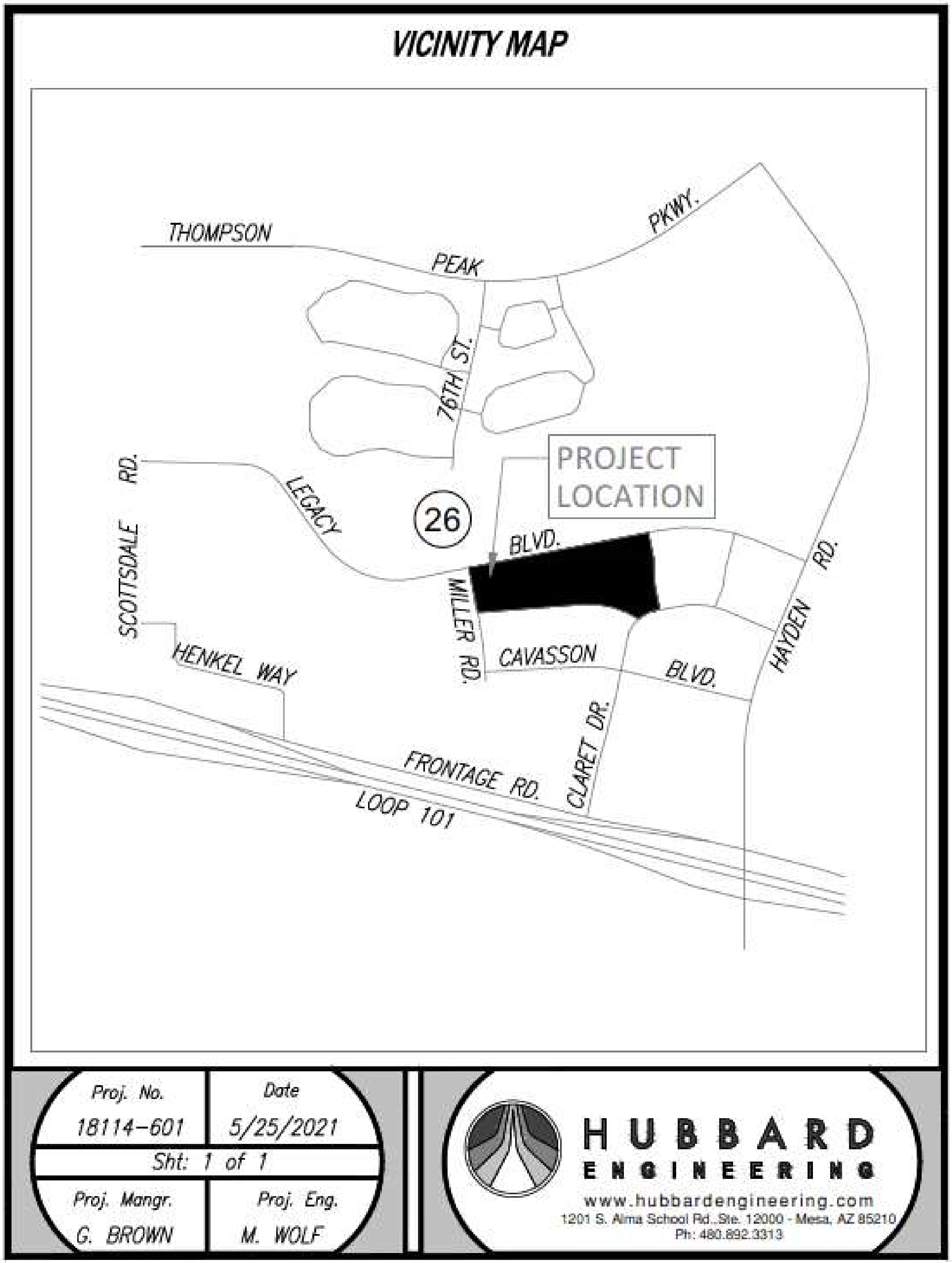
The analysis presented herein focuses on developing design estimates of storm water runoff resulting from a statistical evaluation of storm events of a particular duration and frequency, up to and including a 100-year frequency event. A storm event exceeding the 100-year frequency event may cause or create the risk of greater flood impact than is addressed and presented herein. The scope of this assessment does not include evaluation of storm water runoff resulting from storm events exceeding the 100-year frequency event. Hubbard assumes no responsibility for actual flood damage, increased risks of flood damage, or increased construction or development costs resulting from or related to any such events. Nor shall Hubbard be responsible for any changes in, or additions to, regulatory requirements which may result from, or be related to, any such events or changes in hydrologic or hydraulic conditions within the watershed.

1.2 Site Description

The project site is located in the southeast quarter of Section 26, Township 4N, Range 4E of the Gila and Salt River Base and Meridian, Maricopa County, Arizona. The site is currently undeveloped, and prior to Nationwide Realty Investor's acquisition, was held in trust by the Arizona State Land Department (ASLD) as a portion of the overall Crossroads East development, which encompasses approximately 883 gross acres. The Grayhawk Residences at Cavasson development is located in the Northwest corner of the overall Crossroads East development, near the Legacy Boulevard and Miller Road intersection. The land naturally falls from northeast to southwest.

The project site is bounded by undeveloped desert to the south, Reveille Road to the east, North Miller Road to the west, and Legacy Boulevard to the north. The site location is shown in **Figure 1.1 – Vicinity Map**.

Figure 1.1 – Vicinity Map



1.3 Project Type

The Cavasson development is being developed by Nationwide Reality Investors as a master planned mixed use development with office, retail, hotels, and multifamily residential parcels with public and private roadways that run adjacent and through the development. The Grayhawk Residences at Cavasson will include construction of a new apartment complex with 402 dwelling units over the 18.6 acre site in multiple buildings. Improvements will also include surrounding access drives, parking, and extending existing private utility stubs to service the building.

The analyses of pre-development and post-development peak discharges were addressed in Hubbard's *Master Drainage Report Phase 3 update* and will be referenced and further discussed as it applies to the Grayhawk Residences at Cavasson.

1.4 Special Conditions- 404 Washes

Located throughout the project site are waters of the United States 404 washes. The handling of the 404 washes has been coordinated with engineering consultants and the City of Scottsdale. The Section 404 Certification form has been completed by consultants. As discussed in the *Master Drainage Report*, Hubbard has met with the Army Corps of Engineer (USACE) and worked with consultants and the Corps to complete a mitigation plan. Approval of the 404 permit was granted on April 2, 2019. That permit number is SPL-2018-00704. ADEQ has issued the 401 permit. This permit has been attached for reference in **Appendix G.**

1.5 Regulatory Criteria

The criterion used in the drainage design and analysis of the site was established using the guidelines as described in the following:

- City of Scottsdale, *Design Standards & Policies Manual*, January 18, 2018.
- City of Scottsdale, *Ordinance No. 4346,* June 17, 2018.
- *Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology*.
- *Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics*.

2. EXISTING DRAINAGE CONDITIONS

2.1 Existing Off-Site Conditions Characteristics

Development in the surrounding area has increased in the last decade and many of the surrounding properties which sit in the Reata Pass basin, part of the Pinnacle Peak South Area, have installed infrastructure to route the offsite flows through their developments. As-built plans for the developments show that channels and culverts have been constructed to divert and route off-site flows. Historical runoff for the areas on and surrounding the project site flow south towards collections points (i.e. existing culverts) where the off-site storm runoff is ultimately conveyed to the TPC golf course just north of the Central Arizona Project canal. Existing culverts located at the edges of Hayden Road are currently utilized to route off-site runoff. Off-site routing for the developed surrounding areas has been verified with their corresponding as-builts, including Hayden Road Improvement Plans, 76th Street & Infrastructure Design, Center Drive (now Legacy Boulevard) Improvements, and One Scottsdale Civic Center Improvement Plans.

Hubbard Engineering completed a *Master Drainage Report* which included an extensive analysis of the off-site flow conditions affecting the entire Crossroads East area, including Cavasson and Hayden Road, for the existing site and proposed development. Results from the study indicate that the storm event affecting the Grayhawk Residences at Cavasson is a 100-year, 6-hour storm event. Since the submittal of the *Master Drainage Report,* Hubbard has met with the City of Scottsdale and the developers south of the Cavasson development. It was determined in the meeting that the HEC-1 models be adjusted to reflect a time interval of 15 minutes rather than 3 minutes, as was originally modeled in TY Lin's report. The flows affecting the site will be discussed in section 3.1. Details on Hubbard's HEC-1 methodology and analyses, in addition to the modeling results, can be referenced in the approved *Cavasson Master Drainage Report Phase 3 Update* dated April 30, 2021.

The Grayhawk Residences at Cavasson site specifically is being impacted by two sets of dual barrel elliptical culverts crossing Legacy Boulevard in the existing condition. These culverts are conveying flow from basins SB01-B and SB01-C north of Legacy Boulevard.

2.2 On-site Drainage

As the Cavasson project site was undeveloped prior to Phase 1, there were no on-site drainage structures. The project site was included in two previous studies, Bob Ward's *Core North/ Core South Drainage Study* and TY Lin's *Pinnacle Peak South Area Drainage Master Study.* The TY Lin report analyzed the Crossroads East area and included FLO2d models combined with HEC-1 analyses on localized basins. The TY Lin report provided a basis on which Hubbard completed its hydrologic studies for the Cavasson project development.

The project site naturally falls from north to south at approximately 1.2% and approximately 0.7% east to west. The site outfall for the Grayhawk Residences at Cavasson occurs at the southern boundary of the project along Miller Road at an elevation of 1627'.

2.2.1 HEC-1 Analysis

The TY-Lin *Pinnacle Peak South Area Drainage Master Study* report provides an analysis of the entire Pinnacle Peak South Area, which covers an approximate area of 40 square miles. According to the report, the project site is located within the Dobson Wash Watershed, which includes inflow from the southwestern flow split of the Reata Pass Wash. TY Lin's existing and proposed exhibits and results can be found in **Appendices A** and **B**, respectively. Hubbard replicated TY-Lin's model in application to the approximate nine square miles including and surrounding the project site, shown in **Exhibit 1,** and included the inflow hydrographs from TY Lin's Master Drainage study to account for run-off affecting the project site from the 76th Street Channel and the Powerline for both the 100-year, 6-hour and 100-year, 24-hour storm events. Data collected for the model includes the precipitation, soil, and land use for the existing site conditions in application to the delineated sub-basins. All data Hubbard obtained and used in the model was compared to the existing TY Lin model for accuracy and adjusted accordingly based on current site conditions at maximum densities per approved zoning cases. Precipitation data was obtained from the NOAA Atlas 14 precipitation database. The map index used in DDMSW is 64, cells 687-689. Existing soil data was obtained from the United States Department of Agriculture's Natural Resources Conservation Service. Additionally, land use data was determined based on current zoning of the project site. Data for the HEC-1 analysis are in **Appendix C**. The results of the ultimate condition at full build out (after the powerline channel is constructed) are included in **Appendix D**.

Hubbard has met with the City of Scottsdale and the developers south of the Cavasson development in order to coordinate the analyses for the Crossroads East development. It was determined in the meeting that the HEC-1 models be adjusted to reflect a time interval of 15 minutes rather than 3 minutes, as was originally modeled in TY Lin's report. Thus, the inflow hydrographs provided by TY Lin were convoluted to accurately represent this change in time interval resulting in more accurate time to peak values. In addition, it was also decided in the meeting that Hubbard's analysis for the existing, proposed, and ultimate conditions will be used by developers south of Hayden Road. Hubbard's sub-basin delineation, shown in **Exhibit 1**, deviates from TY Lin's analysis in that Hubbard added collection points at existing culverts along Legacy Boulevard and Hayden road to model the peak flows coming through each infrastructure for utilization in routing design. Additionally, detailed analysis of the existing topography and as-builts along Hayden road and Legacy Boulevard revealed that the basins were not segmented by Hayden Road or Legacy Boulevard, in the TY Lin Report. These larger basins were subdivided in the Hubbard analysis and a similar naming convention was used to compare baseline flows.

Hubbard's analysis split the original TY Lin sub-basins 7 and 8 into east and west components to more accurately model the conveyance of the flows on either side of Hayden Road. An additional basin, SB09N was also added per the city's request as this was not included in the TY Lin analysis. In Hubbard's routing, flows from SB07E and SB08E are routed east, following the topography. Since the TY-Lin model does not delineate a SB-08W, the TY-Lin model shows the flow from SB08 flowing from the southwest corner of the basin east across Hayden Road. This analysis is not correct considering that the eastern elevation is approximately 20 feet higher than the western elevation (CP-08 in TY-Lin's exhibit). This routing also neglects the flow being conveyed across the Loop 101 from SB07W and SB08W. Hubbard therefore routed the flow from

SB07W and SB08W to the existing ADOT culvert along Frontage road and across the Loop 101, thus affecting the developments south of the Loop 101, a condition not represented in the TY-Lin report.

In TY-Lin's analysis, the 100-year, 24-hour storm event controlled for the entire area, with the 100-year, 6-hour storm event controlling the localized flows. TY-Lin recommended the creation of the Powerline Channel to intercept and route the 100-year, 24-hour flows to Basin 53R. Since it was determined in meetings with the City of Scottsdale that the channel will be built during construction of the Cavasson development, Hubbard's existing model includes this channel and results for the 100-year, 24-hour, 100-year, 6-hour, and 10-year, 6-hour storm events are shown in **Appendix D.** The Existing 100-year, 6-hour flows prior to the offsite channel being constructed were 196 cfs entering the site and an Existing condition 100-Year, 6-hour discharge exiting the site at the Loop 101 of 740 cfs. With the addition of the channel and routing to Basin 53R, a significant amount of flow is being diverted from the affected study area to Basin 53R. The addition of the channel results in higher peak flows for the Crossroads East development from the 100-year, 6-hour storm event rather than the 100-year, 24-hour storm.

In addition to the existing model post-channel and proposed model, Hubbard completed an ultimate condition model considering the surrounding lots will be developed. In the model, this was represented by increasing the imperviousness of SB01-B, SB01-C, and SB07W to reflect maximum assumed densities per the zoning stipulations pertinent to the Cavasson development and Cross Roads East approved zoning cases.

2.3 Flood Zone Information

The Maricopa County, Arizona and Incorporated Areas Flood Insurance Rate Map (F.I.R.M.) number 04013C1320L, Panel number 1320 of 4425, dated October 16, 2013 indicates that the Grayhawk Residences at Cavasson falls within Zone AO (Depth 1 foot) on the west side of the project with the remainder classified as Zone X.

Zone AO is defined as:

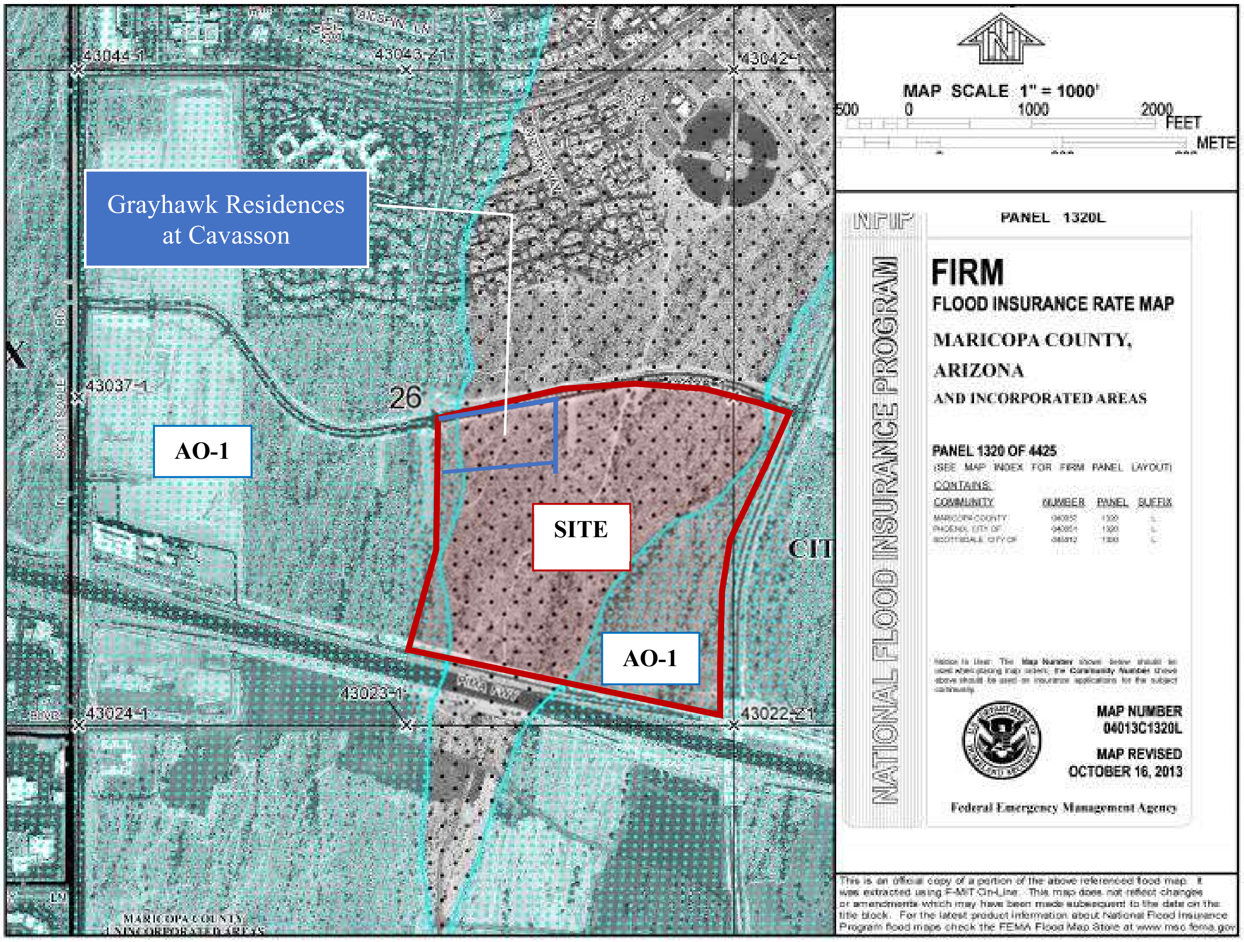
"*Areas subject to inundation by 1-percent-annual-chance shallow flooding (usually sheet flow on sloping terrain) where average depths are between one and three feet. Average flood depths derived from detailed hydraulic analyses are shown in this zone. Mandatory flood insurance purchase requirements and floodplain management standards apply.*"

Zone X is defined as:

"Areas of 0.2% annual chance flood; areas of 1.0% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood."

Refer to FEMA Firmette in **Figure 2.1.**

Figure 2.1 – FIRM 04013C1320L



3. PROPOSED DRAINAGE PLAN

3.1 Off-Site Flows

Off-site flows for the site were analyzed and addressed in Hubbard's *Master Drainage Report Phase 3 Update.* Results from the analysis show that after the powerline Channel is constructed 19 cfs is currently being conveyed across Legacy from the existing elliptical culvert located directly north of the Grayhawk Residences at Cavasson within Legacy Boulevard. When the parcel north of Legacy is developed, these flows are anticipated to increase to 50 cfs. These flows are now, however, being captured in a Offsite Storm Drain Network (Plan Check #1838-21) designed by Hubbard Engineering, construction expected to begin by November of 2021, that routes all of these offsite flows around the outside perimeter of the site prior to crossing Cavasson Boulevard. This allows the offsite flows to maintain their existing flow patterns without impacting the Grayhawk Residences at Cavasson. In the event that the Offsite Storm Drain System infrastructure fails, the offsite flow from north of Legacy would be contained within an existing swale to the north of Legacy Boulevard before entering an existing 10'x3' curved box culvert that discharges the flow to the south of Legacy Boulevard and west of Miller Road. This flow would then continue along the west side of Miller Road before joining its historic flow pattern just south of Cavasson Boulevard. It should be noted that due to sedimentation within the existing culverts crossing Legacy Boulevard, this overflow route is already being used in this manner for some of the flow. This Offsite Storm Drain System was discussed in further detail within Hubbard's *Master Drainage Report Phase 3 Update.*

The offsite flow from the adjacent Reveille Road will be conveyed to the site through an existing catch basin that will be tied directly into the onsite storm drain system. This offsite flow was calculated as part of the *Phase 3 Roads Drainage Report* written by Hubbard Engineering. Offsite flows from the adjacent half street of Miller Road will not impact the site because Miller Road was designed to convey all of its flow to two curb openings south of Cavasson Boulevard. The Grayhawk residences at Cavasson will still however retain the appropriate volume for the 100 year 6-hour storm event for the adjacent half street to remove those flows from the system. Likewise, the offsite flows from the adjacent half street of Legacy boulevard are being captured by the Offsite Storm Drain System and will not impact the site. The site will also still retain the appropriate equivalent volume for the adjacent half street of Legacy Boulevard. A summary of the offsite flows impacting the Grayhawk Residences at Cavasson can be found in **Table 1** below.

Table 1: Offsite Flow Summary Table

Contributing Area	Pre-Development Q100 (cfs)	Post Development Q100 (cfs)
SB01-B	19	0
SB01-C	19	0
Reveille Road	0	2.12

3.2 On-Site Hydraulics

The storm event affecting the site was determined to be a 100-year, 6-hour storm, as discussed in Hubbard's *Master Drainage Report Phase 3 Update.* The Grayhawk Residences at Cavasson will include the installation of several multifamily apartment buildings throughout the site as well as a parking lot and private drives servicing all of the buildings. Additionally, the Offsite Storm Drain System runs along the north and west boundaries of the site within an existing drainage easement.

See **Exhibit 2** for the drainage map. The onsite storm drain system for the Grayhawk Residences at Cavasson consists of two parallel systems running along the north and south drives of the site that are connected by equalized underground retention tanks. The north system ties into the end of one of the underground retention tanks with a 42-inch diameter HDPE pipe running along the north drive. This pipe will then become a 36-inch diameter HDPE pipe after about 300' and continues along the north drive. After another roughly 500' the storm drain bends to follow the main north drive and at this bend converts to a 24-inch diameter HDPE pipe before branching out to service the remainder of the north half of the site. Along this main run, there are several inlets that are teed off of the main run to drain front courtyards of buildings, accept flow from the drainage easement north of the site, and accept the flow from side parking lots for the buildings. The flows from adjacent building roof drains will sheet flow to inlets located along the north storm drain which will then convey them to the underground retention tanks.

The south storm drain system ties into the end of a different underground retention tank with a 48-inch diameter HDPE pipe that runs along the southern drive. The 48-inch diameter pipe becomes a 42-inch diameter HDPE pipe at a manhole structure after about 160 feet. At this same manhole structure, the storm drain system that drains the west half of the central common area enters the south storm drain system. The south storm drain system then continues along the drive for another 300 feet where the system that drains the east half of the central common area then enters the system through another manhole structure. The south storm drain system continues until it reaches a bend where it downsized to a 30-inch HDPE pipe before branching out to drain the remaining areas of the site. One of these branches ties in to the existing manhole that is connected to the Reveille Road catch basin to accept its flow. Similar to the north system, the south system has multiple inlets that are teed off of the main system to capture flow from different parking areas. Additionally, flows from adjacent building roof drains will sheet flow to inlets located along the south storm drain to be conveyed to the underground retention tanks.

There are six underground retention tanks that are all equalized together on the west side of the site near the drive entrance that the north and south storm drain systems drain to. Some of the tanks have inlets that take flow directly from the drainage area over the tanks and from the adjacent buildings roof drain leaders. In greater storm events and back-to-back storm events an emergency overflow has been provided for the retention tanks. A 12-inch diameter HDPE overflow pipe has been designed to connect into the riser of one of the underground retention tanks that will allow excess volume to overflow. This pipe runs down the drive entrance to connect to the Offsite Storm Drain System as an emergency overflow only. The Offsite Storm Drain System was designed to accept up to 20 cfs of emergency overflow from the site. For design purposes, the tailwater for the north and south storm drain systems was set at the top of the equalized tanks and the tailwater for the emergency overflow storm drain system was set at the HGL of the Offsite Storm Drain System at the connection point. Head loss for the pipes was calculated using Manning's Equation and head loss through the structures was calculated the equation $k(\frac{v^2}{2g})$ where k is the junction loss coefficient, v is the velocity exiting the junction and g is the acceleration of gravity. See **Appendix E** for hydrologic calculations and **Appendix F** for hydraulic calculations.

3.3 Storm Water Storage

The project development is required to provide retention for the first flush storm event for onsite and the 100 year 6-hour storm event for the adjacent half streets. The required first flush volume determined for each drainage area site will be designed per City of Scottsdale *Drainage Policies and Standards for Maricopa County, Arizona* (Reference 1). The first flush retention required for the site will be 29,095 cubic feet. The required retention for the adjacent half streets for the site is 30,674 cubic feet. This results in a total required retention of 59,769 which will be retained in 770 lf of 10' CMP underground retention tanks. These tanks provide a total of 60,319 cubic feet of retention which meets the requirement. The required and provided retention calculations can be found in **Appendix F**.

3.3.1 Time to Drain

Each site will be required to dewater the required storm water retention through natural percolation and/or drywells. The City of Scottsdale requires all retention facilities be completely drained within a 36-hour time frame. For design purposes, it was assumed that each drywell will have a percolation rate of 0.1 cfs. During construction, the first drywell constructed will be tested to verify this percolation rate. In the event that the tested percolation rate plus a clogging factor exceed 0.1 cfs, the number of drywell may be reduced while ensuring all retention facilities fully drain within 36 hours. Using this drywell discharge rate, it was determined that 5 separate drywells will be provided to drain the total volume of the underground retention tanks below the emergency outfall pipe located in the riser. This will allow the retention tanks to fully dewater in a total time of 33.5 hours. See **Appendix F** for drywell calculations.

4. SUMMARY AND CONCLUSION

- The site is located in Section 26 of Township 4N, Range 4E of the Gila and Salt River Base and Meridian, Maricopa County, Arizona.
- The site is partially located in the Flood Plain Zone AO-1 on the west side and in Flood Plain Zone X on the remainder.
- The Finish Floor Elevations of the proposed buildings meet the requirement of 2 feet higher than the natural grade (HAG) within the Zone AO-1 flood zone.
- The storm event affecting the site was modeled as a 100-yr, 6-hour event in Hubbard's *Master Drainage Report* due to being more conservative.
- The Offsite Storm Drain System mitigates the offsite flows from north of Legacy Boulevard. The sites onsite storm drain system is directly accepting the offsite flows from Reveille Road.
- The site will be required to provide retention for the first flush storm event for onsite and provide retention for the 100-year 6-hour storm event for the adjacent half street roads.
- 770 lf of 10' CMP retention will provide 60,319 cf of retention, for the required retention of 59,769 cf.

5. REFERENCES

1) City of Scottsdale. *Drainage Policies & Standards for Maricopa County, Arizona.* January 18, 2018.

2) Flood Control District of Maricopa County. *Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology*. November 2003.

3) Flood Control District of Maricopa County. *Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics*. November 2003.

4) Flood Control District of Maricopa County. *Drainage Design Manual for Maricopa County, Arizona, Volume III, Erosion.* July 2018.

5) City of Scottsdale. *Ordinance No. 4346.* June 17, 2018.

6) TY Lin, *Pinnacle Peak South Area Drainage Master Study,* 2014.

7) City of Scottsdale, *Ordinance No. 4346,* June 17, 2018.

8) Hubbard Engineering, *Master Drainage Report*. December 23, 2018

9) Hubbard Engineering, *Master Drainage Report Phase 3 Update,* August 19, 2021

6. LIMITATIONS

This report is focused on providing practical design information, evaluation, and calculations for statistical flood events up to and including the 100-year frequency flood. The procedures used herein are derived from, and performed with, currently accepted engineering methodologies and practices. Additionally, the criteria for this evaluation is designed to conform to currently applicable ordinances, regulations and policies effected by the appropriate jurisdictional regulatory authorities for the site.

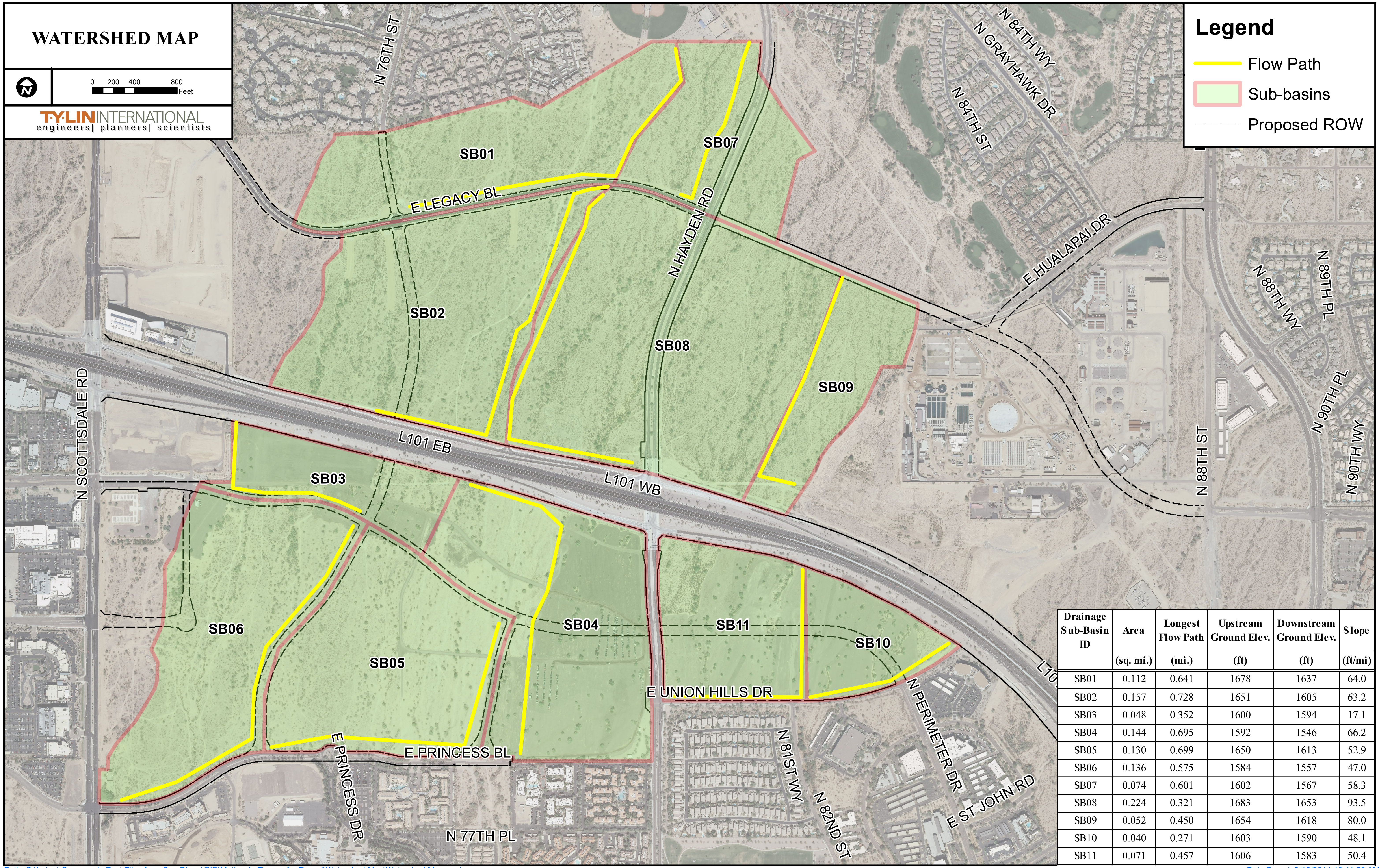
The analysis presented herein focuses on developing design estimates of storm water runoff resulting from a statistical evaluation of storm events of particular duration and frequency up to and including a 100-year frequency event. A storm event exceeding the 100-year frequency event may cause or create the risk of greater flood impact than is addressed and presented herein. However, the scope of this assessment does not include evaluation of storm water runoff resulting from storm events exceeding the 100-year frequency event. Hubbard Engineering assumes no responsibility for actual flood damage, increased risks of flood damage, or increased construction or development cost resulting from or related to any such events.

Nor shall Hubbard Engineering be responsible for any changes in, or additions to, regulatory requirements which may result from, or be related to, any such events or changes in hydrologic or hydraulic conditions within the watershed.

In performing the services contained herein, Hubbard Engineering has received or will receive information prepared or compiled by others. Hubbard Engineering, as engineering professionals, are not required to verify the information, but may rely on the information unless actual knowledge concerning the validity of the information is known or is obvious to the professional. Therefore, Hubbard Engineering is entitled to rely upon the accuracy and completeness of this information without independent evaluation or verification.

Appendix A
TY Lin HEC-1 Existing
Grayhawk Residences at Cavasson

WATERSHED MAP



Drainage Sub-Basin ID	Area (sq. mi.)	Longest Flow Path (mi.)	Upstream Ground Elev. (ft)	Downstream Ground Elev. (ft)	Slope (ft/mi)
SB01	0.112	0.641	1678	1637	64.0
SB02	0.157	0.728	1651	1605	63.2
SB03	0.048	0.352	1600	1594	17.1
SB04	0.144	0.695	1592	1546	66.2
SB05	0.130	0.699	1650	1613	52.9
SB06	0.136	0.575	1584	1557	47.0
SB07	0.074	0.601	1602	1567	58.3
SB08	0.224	0.321	1683	1653	93.5
SB09	0.052	0.450	1654	1618	80.0
SB10	0.040	0.271	1603	1590	48.1
SB11	0.071	0.457	1606	1583	50.4

Appendix B
TY Lin HEC-1 Proposed
Grayhawk Residences at Cavasson

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 *                                      *                                                *
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 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                *    U.S. ARMY CORPS OF ENGINEERS
  *
 *             JUN   1998                *                                                *    HYDROLOGIC ENGINEERING CENTER
  *
 *          VERSION 4.1                  *                                                *         609 SECOND STREET
  *
 *                                       *                                                *       DAVIS, CALIFORNIA 95616
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 *  RUN DATE   12SEP14  TIME  11:45:27   *                                                *          (916) 756-1104
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                         X     X  X        X     X         X
                         X     X  XXXXXXX   XXXXX         XXX


          THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

          THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
          THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
          NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
          DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
          KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1                                     HEC-1 INPUT                                         PAGE  1

         LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

           1           ID        CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
           2           ID        RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
           3           ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
           4           ID       100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
           5           ID           PIMA ROAD FLOWS)
           6           ID
           7           ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
           8           ID       MODELERS: RK, MW
           9           ID
          10           IT       3       0       0    1000
          11           IN       3
          12           IO       5
                       *DIAGRAM
                       *
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          18           PC   0.181   0.191   0.203   0.218   0.236   0.257   0.283   0.387   0.663   0.707
          19           PC   0.735   0.758   0.776   0.791   0.804   0.815   0.825   0.834   0.842   0.849
          20           PC   0.856   0.863   0.869   0.875   0.881   0.887   0.893   0.898   0.903   0.908
          21           PC   0.913   0.918   0.922   0.926   0.930   0.934   0.938   0.942   0.946   0.950
          22           PC   0.953   0.956   0.959   0.962   0.965   0.968   0.971   0.974   0.977   0.980
          23           PC   0.983   0.986   0.989   0.992   0.995   0.998   1.000
          24           JD   3.657    10.0
          25           JD   3.533    20.0
                       *

          26           KK  76THST
          27           KM        76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
          28           KM        100-YR, 6-HR FLO-2D MODEL (XS 98)
          29           BA    0.24
          30           QI       0       0       0       0       0       0       0       0       0       0
          31           QI       0       0       0       0       0       0       0       0       0       0
          32           QI       0       0       0       0       0       0       0       0       0       0
          33           QI       0       0       0       0       0       0       0       0       0       0
          34           QI       0       0       0       0       0       0       0       0       0       0
          35           QI       0       0       0       0       0       0       0       0       0       0
          36           QI       0       0       0       0       0       0       0       0       0       0
          37           QI       0       0       0       0       0       0       0       0       0       0
          38           QI       0       0       0       0       0       0       0       0       0       0
          39           QI       0       0       0       0       0       0       0       0       0       0
          40           QI       0       0    0.01       0       0       0       0       0       0       0
          41           QI       0       0       0       0       0       0       0       0       0       0
          42           QI       0       0       0       0       0       0       0       0       0       0
          43           QI       0       0       0       0       0       0       0       0       0       0
```

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 44          QI      0       0       0       0    0.01    0.01       0       0       0       0
 45          QI      0       0       0       0       0       0       0       0       0       0
 46          QI      0       0       0       0       0       0       0       0       0       0
 47          QI   0.01       0       0    0.01       0       0       0       0    0.01    0.01
 48          QI      0    0.01    0.01    0.02    0.02    0.02    0.02    0.02    0.03    0.03
 49          QI   0.03    0.04    0.04    0.05    0.05    0.05    0.06    0.06    0.07    0.08
 50          QI   0.09    0.09     0.1     0.1    0.11    0.11    0.12    0.12    0.13    0.14
 51          QI   0.14    0.15    0.16    0.18    0.19    0.21    0.23    0.25    0.27     0.3
1                                       HEC-1 INPUT                                              PAGE  2

LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 52          QI   0.34    0.36     0.4    0.44    0.47    0.51    0.56     0.6    0.66    0.71
 53          QI   0.76    0.95    1.61    1.73    1.95     3.7    4.26    6.53    8.52   10.41
 54          QI  17.69   31.39   35.75   41.23   44.49   47.79   51.13   56.33   62.34   66.18
 55          QI  68.57   67.36   65.05   61.38   58.37   54.61   52.38   50.75    48.4   44.37
 56          QI  42.75    40.4   38.91   37.61   36.75   35.61   34.75   34.25   33.52   32.65
 57          QI  32.34   31.59   31.31   30.73   30.02   29.25   28.51   28.42   27.67   26.83
 58          QI  26.13   25.37   24.53   23.95   23.37   22.83   22.17   21.48   20.81   20.94
 59          QI  19.41   19.05   18.62    18.3   18.17   18.11   17.52    17.1    16.7    16.5
 60          QI  16.15   15.74   15.37   15.07   14.74   14.54   14.51   14.24   14.07   13.65
 61          QI  13.75   13.24   12.99   13.09   12.94   12.53   12.53   12.38   12.22   12.18
 62          QI     12   11.59   11.89   11.53   11.85   11.44   11.16   11.22   11.21   10.82
 63          QI  10.52   10.43   10.88   10.42    10.1    9.81    9.72    9.66    9.78    9.55
 64          QI   9.34    9.36    9.33    9.23     9.1    9.09    9.48     9.3    9.02    9.07
 65          QI   9.11     9.1    8.93    8.81    8.73    8.71    8.57     8.5     8.5    8.43
 66          QI   8.37    8.31    8.26    8.17    8.14    8.05    7.98    7.97    7.83    7.62
 67          QI   7.67    7.67    7.57    7.66    7.74    7.75    7.91    7.88    7.62    7.55
 68          QI   7.47     7.3    7.22    7.25     7.2    7.08    7.02    7.01    6.99    6.96
 69          QI   6.97    6.92    6.89    6.85    6.81    6.77    6.75     6.7    6.67    6.62
 70          QI   6.59    6.56    6.51    6.48    6.47    6.44    6.44    6.47    6.43    6.37
 71          QI   6.32    6.27    6.23    6.22    6.16    6.12    6.08    6.05    6.02    5.98
 72          QI   5.95    5.95    5.95    5.93     5.9    5.87    5.84    5.76    5.72    5.64
 73          QI    5.7    5.76    5.62    5.65    5.74    5.84    5.95     4.5    4.58    5.32
 74          QI   5.41    5.43    4.49    5.32    5.36    5.44    5.48     5.6    5.67    4.68
 75          QI   4.56    4.55    4.53    4.82    4.84    4.81    4.81    4.83    4.82     4.6
 76          QI   4.75    4.82    4.66    4.85    4.81    4.78    4.77    4.75    4.73    4.72
 77          QI   4.69    4.64    4.62    4.61    4.63    4.67    4.71    4.72    4.71    4.66
 78          QI   4.61    4.53    4.43    4.39    4.37    4.36    4.33    4.33    4.28    4.26
 79          QI    4.2    4.17    4.13    4.07    4.03       4    3.96    3.92    3.86     3.8
 80          QI   3.75    3.71    3.65     3.6    3.56    3.51    3.45    3.42    3.39    2.43
 81          QI   2.49    2.39    2.38    3.55    3.52    3.43    2.35     2.4    2.46    3.65
 82          QI   3.61     3.3    2.37     2.4    2.46    3.65    3.59    3.59    3.61    3.67
 83          QI   2.48    2.53    2.53    2.47    2.41    2.36    2.32    2.28    2.25    2.22
 84          QI   2.19    2.16    2.14    2.12    2.09    2.07    2.04    2.02       2    1.97
 85          QI   1.94    1.91    1.88    1.85     1.8    1.72    1.63    1.56    1.58    1.65
 86          QI   1.71    1.72    1.67    1.57    1.49    1.36    1.49    1.56    1.58    1.57
 87          QI   1.48    1.35    1.21    1.15    1.13    1.13    1.13    1.13    1.13    1.13
 88          QI   1.13    1.13    1.12    1.14    1.17     1.2    1.23    1.22    1.18    1.09
 89          QI    1.1    1.11    1.13    1.18    1.26    1.06     1.1    1.08    1.06    1.05
 90          QI   1.06    1.05    1.04    1.02    1.01       1    0.98    0.97    0.96    0.95
 91          QI   0.94    0.93    0.92    0.91     0.9    0.89    0.88    0.87    0.86    0.86
 92          QI   0.85    0.84    0.84    0.83    0.82    0.81     0.8     0.8    0.79    0.78
 93          QI   0.78    0.77    0.76    0.76    0.75    0.74    0.73    0.73    0.72    0.72
 94          QI   0.72    0.71    0.71    0.71     0.7     0.7    0.69    0.69    0.68    0.67
 95          QI   0.67    0.66    0.66    0.65    0.65    0.64    0.64    0.63    0.63    0.62
 96          QI   0.62    0.61    0.61     0.6     0.6    0.59    0.59    0.59    0.58    0.58
 97          QI   0.57    0.57    0.56    0.56    0.56    0.55    0.55    0.54    0.54    0.53
 98          QI   0.53    0.53    0.52    0.52    0.52    0.51    0.51     0.5     0.5     0.5
 99          QI   0.49    0.49    0.48    0.48    0.48    0.47    0.47    0.47    0.46    0.46
100          QI   0.45    0.45    0.45    0.44    0.44    0.44    0.43    0.43    0.43    0.42
101          QI   0.42    0.42    0.41    0.41    0.41     0.4     0.4     0.4     0.4    0.39
102          QI   0.39    0.39
             *
1                                       HEC-1 INPUT                                              PAGE  3

LINE         ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


103          KK   SB01   BASIN
104          BA  0.112
105          LG   0.25    0.25    4.10    0.55      45
106          UC  0.318   0.253
107          UA      0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
108          UA    100
             *

109          KK   CP-1 COMBINE
110          KM       LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
111          HC      2
             *

112          KK   R1-2   ROUTE
113          KM       MILLER RD CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
114          RK   2104  0.0015    0.03            TRAP      66       4
             *

115          KK   SB02   BASIN
116          BA  0.157
117          LG   0.15    0.25    4.15    0.58      56
118          UC  0.328   0.240
119          UA      0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
120          UA    100
             *
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          121           KK    CP-2 COMBINE
          122           KM        SR 101L FREEWAY AND 76TH ST (MILLER RD CHANNEL)
          123           HC       2
                        *

          124           KK    R2-3   ROUTE
          125           KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
          126           RK    1260  0.0015   0.03             TRAP      92       4
                        *

          127           KK    SB03   BASIN
          128           BA   0.048
          129           LG    0.15    0.25    4.50    0.47      55
          130           UC   0.364   0.294
          131           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          132           UA     100
                        *

          133           KK    CP-3 COMBINE
          134           KM        MAYO BLVD AND 76TH ST (MILLER RD CHANNEL)
          135           HC       2
                        *
1                                                 HEC-1 INPUT                                          PAGE  4

         LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


          136           KK    R3-5   ROUTE
          137           KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
          138           RK    2396  0.0015   0.03             TRAP      98       4
                        *

          139           KK    SB04   BASIN
          140           BA   0.144
          141           LG    0.14    0.25    4.60    0.44      61
          142           UC   0.305   0.199
          143           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          144           UA     100
                        *

          145           KK    R4-5   ROUTE
          146           KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
          147           RK    2005  0.0013   0.03             TRAP      39       4
                        *

          148           KK    SB05   BASIN
          149           BA   0.126
          150           LG    0.22    0.25    4.50    0.44      48
          151           UC   0.327   0.226
          152           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          153           UA     100
                        *

          154           KK    CP-5 COMBINE
          155           KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
          156           HC       3
                        *

          157           KK    R5-6   ROUTE
          158           KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
          159           RK    1550  0.0015   0.03             TRAP     188       4
                        *

          160           KK    SB06   BASIN
          161           BA   0.136
          162           LG    0.16    0.25    4.55    0.45      53
          163           UC   0.321   0.246
          164           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          165           UA     100
                        *

          166           KK    CP-6 COMBINE
          167           KM        PRINCESS BLVD AND SCOTTSDALE RD (PRINCESS BLVD CHANNEL)
          168           HC       2
                        *
1                                                 HEC-1 INPUT                                          PAGE  5

         LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


          169           KK    SB07   BASIN
          170           BA   0.074
          171           LG    0.21    0.25    4.00    0.58      49
          172           UC   0.206   0.113
          173           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
          174           UA     100
                        *

          175           KK    R7-8   ROUTE
          176           KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
          177           RK    2778  0.0014   0.03             TRAP      46       4
                        *
```

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      178           KK    SB08   BASIN
      179           BA   0.224
      180           LG    0.14    0.25    4.10    0.59      59
      181           UC   0.338   0.195
      182           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
      183           UA     100
                    *

      184           KK    CP-8 COMBINE
      185           KM        SR 101L FREEWAY AND HAYDEN ROAD (HAYDEN ROAD NORTH CHANNEL)
      186           HC       2
                    *

      187           KK    R8-9   ROUTE
      188           KM        HAYDEN ROAD NORTH CHANNEL FROM HAYDEN ROAD TO BASIN 53R
      189           RK    1250  0.0013   0.03             TRAP      67       4
                    *

      190           KK    SB09   BASIN
      191           BA   0.052
      192           LG    0.15    0.25    4.00    0.61      55
      193           UC   0.254   0.230
      194           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
      195           UA     100
                    *

      196           KK    CP-9 COMBINE
      197           KM        SR 101L FREEWAY AND BASIN 53R (HAYDEN ROAD NORTH CHANNEL)
      198           HC       2
                    *

      199           KK   PWRCH
      200           KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
      201           KM        FROM PINNACLE PEAK SOUTH ADMS 100-YR, 24-HR FLO-2D MODEL
      202           BA     7.0
      203           QI       0       0       0       0       0       0       0       0       0       0
      204           QI       0       0       0       0       0       0       0       0       0       0
      205           QI       0       0       0       0       0       0       0       0       0       0
      206           QI       0       0       0       0       0       0       0       0       0       0
      207           QI       0       0       0       0       0       0       0       0       0       0
      208           QI       0       0       0       0       0       0       0       0       0       0
1                                                               HEC-1 INPUT                                       PAGE  6

     LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

      209           QI       0       0       0       0       0       0       0       0       0       0
      210           QI       0       0       0       0       0       0       0       0       0       0
      211           QI       0       0       0       0       0       0       0       0       0       0
      212           QI       0       0       0       0       0       0       0       0       0       0
      213           QI       0       0       0       0       0       0       0       0       0       0
      214           QI       0       0       0       0       0       0       0       0       0       0
      215           QI       0       0       0       0       0       0       0       0       0       0
      216           QI       0       0       0       0       0       0       0       0       0       0
      217           QI       0       0       0       0       0       0       0       0       0       0
      218           QI       0       0       0       0       0       0       0       0       0       0
      219           QI       0       0       0       0       0       0       0       0       0       0
      220           QI       0       0       0       0       0       0       0       0       0       0
      221           QI       0       0       0       0       0       0       0       0       0       0
      222           QI       0       0       0       0       0       0       0       0       0       0
      223           QI       0       0       0       0       0       0       0       0       0       0
      224           QI       0       0       0       0       0       0       0       0       0       0
      225           QI       0       0       1       1       1       1       1       1       1       1
      226           QI       1       2       2       2       3       3       5       9      18      28
      227           QI      43      50      65      87     194     495     872     967    1051    1095
      228           QI    1099    1091    1093    1100    1117    1198    1243    1370    1495    1777
      229           QI    2055    2422    2906    3246    3546    3747    3874    3884    3837    3741
      230           QI    3658    3562    3456    3370    3270    3175    3085    3024    2955    2888
      231           QI    2822    2744    2640    2543    2461    2378    2309    2239    2183    2132
      232           QI    2084    2045    2005    1973    1933    1898    1860    1821    1779    1740
      233           QI    1699    1659    1619    1576    1544    1499    1461    1432    1397    1364
      234           QI    1331    1298    1268    1237    1206    1179    1152    1126    1100    1074
      235           QI    1047    1023    1000     979     959     940     918     901     882     867
      236           QI     851     834     818     803     789     773     759     744     731     719
      237           QI     705     693     683     670     657     645     635     627     617     608
      238           QI     597     588     581     573     566     559     553     545     537     529
      239           QI     521     514     507     501     495     488     482     476     471     465
      240           QI     460     456     450     444     440     435     429     424     419     414
      241           QI     410     404     400     396     393     387     385     381     377     375
      242           QI     370     367     364     362     360     356     352     349     346     343
      243           QI     339     338     334     333     330     328     326     322     318     315
      244           QI     317     313     308     306     305     303     300     298     295     294
      245           QI     293     291     288     285     286     281     278     276     275     276
      246           QI     270     265     269     267     264     257     265     257     255     255
      247           QI     258     251     251     253     245     248     247     242     246     239
      248           QI     240     241     236     238     232     237     231     231     228     231
      249           QI     223     227     223     223     221     220     219     215     219     215
      250           QI     214     215     211     215     211     209     213     209     207     208
      251           QI     205     201     207     200     200     201     201     199     197     193
      252           QI     192     190     190     191     189     188     187     185     186     184
      253           QI     180     179     180     173     173     179     169     172     164     169
      254           QI     164     165     163     161     156     158     155     158     153     158
      255           QI     149     153     146     144     146     142     149     139     136     138
      256           QI     137     143     131     130     132     126     126     124     123     133
      257           QI     120     119     124     117     116     117     114     111     113     109
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 258          QI    107     109     105     106     104     102     104     100      99     103
 259          QI     97      96     100      94      93      99      91      90      97      88
 260          QI     86      90      84      85      87      83      83      85      80      79
 261          QI     82      78      75      82      74      74      82      72      71      76
 262          QI     70      71      70      68      72      66      67      65      64      69
 263          QI     62      65      61      61      63      59      65      58      58      60
1                                                      HEC-1 INPUT                                        PAGE  7

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

 264          QI     57      63      56      55      58      55      58      54      52      58
 265          QI     52      52      53      50      57      50      50      55      48      48
 266          QI     50      46      47      47      44      48      45      43      49      43
 267          QI     42      48      42      41      45      41      40      44      40      38
 268          QI     40      40      38      37      40      37      36      42      36      36
 269          QI     35      41      35      34      34      40      34      33      33      33
 270          QI     36      33      32      32      33      32      31      31      30      33
 271          QI     31      30      29      29      31      30      28      29      28      29
 272          QI     31      28      28      27      27      30      27      26      26      26
 273          QI     31      26      25      25      25      30      26      25      25      24
 274          QI     24      28      23      23      24      23      24      25      24      23
 275          QI     23      22

              *

 276          KK  BINFLO
 277          KM         TOTAL INFLOW INTO BASIN 53R.
 278          HC        2
              *

 279          KK   BASIN
 280          KM    BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
 281          KM    ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
 282          KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
 283          KM        THE OUTLET PIPES ARE INLET CONTROLLED.
 284          RS        1    STOR       0
 285          SV        0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
 286          SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
 287          SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
 288          SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
 289          SQ        0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
 290          SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
              *

 291          KK  BSNRT1
 292          KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
 293          KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
 294          KM        EXCESS CAPACITY.
 295          RK      550  0.0052   0.024            CIRC     7.0
              *

 296          KK  BSNRT2
 297          KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BASIN 53R OUTFAL
 298          RK     1200  0.0077   0.013            CIRC     7.0
              *

 299          KK    SB10   BASIN
 300          BA   0.040
 301          LG    0.15    0.25    4.25    0.55      55
 302          UC   0.233   0.161
 303          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 304          UA     100
              *
1                                                      HEC-1 INPUT                                        PAGE  8

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 305          KK   CP-10 COMBINE
 306          KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
 307          HC        2
              *

 308          KK  R10-11   ROUTE
 309          KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
 310          RK     1277  0.0014   0.03             TRAP     220       4
              *

 311          KK    SB11   BASIN
 312          BA   0.071
 313          LG    0.15    0.25    4.15    0.58      55
 314          UC   0.296   0.232
 315          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 316          UA     100
              *

 317          KK   CP-11 COMBINE
 318          KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
 319          HC        2
              *
 320          ZZ
1
         SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
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```
LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

 NO.      (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

  26      76THST
               .
               .
 103           .        SB01
               .           .
               .           .
 109        CP-1............
               V
               V
 112        R1-2
               .
               .
 115           .        SB02
               .           .
               .           .
 121        CP-2............
               V
               V
 124        R2-3
               .
               .
 127           .        SB03
               .           .
               .           .
 133        CP-3............
               V
               V
 136        R3-5
               .
               .
 139           .        SB04
               .           V
               .           V
 145           .        R4-5
               .           .
               .           .
 148           .           .        SB05
               .           .           .
               .           .           .
 154        CP-5........................
               V
               V
 157        R5-6
               .
               .
 160           .        SB06
               .           .
               .           .
 166        CP-6............
               .
               .
 169           .        SB07
               .           V
               .           V
 175           .        R7-8
               .           .
               .           .
 178           .           .        SB08
               .           .           .
               .           .           .
 184           .        CP-8............
               .           V
               .           V
 187           .        R8-9
               .           .
               .           .
 190           .           .        SB09
               .           .           .
               .           .           .
 196           .        CP-9............
               .           .
               .           .
 199           .           .       PWRCH
               .           .           .
               .           .           .
 276           .      BINFLO............
               .           V
               .           V
 279           .       BASIN
               .           V
               .           V
 291           .      BSNRT1
               .           V
               .           V
 296           .      BSNRT2
               .           .
               .           .
 299           .           .        SB10
               .           .           .
               .           .           .
```

```
  305          .       CP-10............
               .           V
               .           V
  308          .      R10-11
               .           .
               .           .
  311          .           .        SB11
               .           .           .
               .           .           .
  317          .       CP-11............

 (***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1*****************************************
***************************************
 *                                       *                                                    *
  *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                    *   U.S. ARMY CORPS OF ENGINEERS
  *
 *             JUN   1998                *                                                    *   HYDROLOGIC ENGINEERING CENTER
  *
 *           VERSION 4.1                 *                                                    *         609 SECOND STREET
  *
 *                                       *                                                    *      DAVIS, CALIFORNIA 95616
  *
 *  RUN DATE   12SEP14  TIME  11:45:27   *                                                    *          (916) 756-1104
  *
 *                                       *                                                    *
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                                 CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
                                 RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                                 INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                                   100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                                    PIMA ROAD FLOWS)

                                 PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                                   MODELERS: RK, MW


  12 IO          OUTPUT CONTROL VARIABLES
                       IPRNT           5  PRINT CONTROL
                       IPLOT           0  PLOT CONTROL
                       QSCAL          0.  HYDROGRAPH PLOT SCALE

     IT          HYDROGRAPH TIME DATA
                        NMIN           3  MINUTES IN COMPUTATION INTERVAL
                       IDATE      1    0  STARTING DATE
                       ITIME        0000  STARTING TIME
                          NQ        1000  NUMBER OF HYDROGRAPH ORDINATES
                      NDDATE      3    0  ENDING DATE
                      NDTIME        0157  ENDING TIME
                       ICENT          19  CENTURY MARK

                   COMPUTATION INTERVAL    0.05 HOURS
                        TOTAL TIME BASE   49.95 HOURS

        ENGLISH UNITS
             DRAINAGE AREA         SQUARE MILES
             PRECIPITATION DEPTH   INCHES
             LENGTH, ELEVATION     FEET
             FLOW                  CUBIC FEET PER SECOND
             STORAGE VOLUME        ACRE-FEET
             SURFACE AREA          ACRES
             TEMPERATURE           DEGREES FAHRENHEIT

  13 JD          INDEX STORM NO. 1
                        STRM        3.85  PRECIPITATION DEPTH
                        TRDA        0.00  TRANSPOSITION DRAINAGE AREA

  14 PI            PRECIPITATION PATTERN
                      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                      0.00      0.00      0.00      0.00      0.01      0.01      0.00      0.01      0.00      0.01
                      0.01      0.00      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                      0.01      0.01      0.02      0.02      0.02      0.03      0.10      0.28      0.04      0.03
                      0.02      0.02      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                      0.01      0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00
                      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                      0.00      0.00      0.00      0.00      0.00      0.00

  24 JD          INDEX STORM NO. 2
                        STRM        3.66  PRECIPITATION DEPTH
                        TRDA       10.00  TRANSPOSITION DRAINAGE AREA

   0 PI            PRECIPITATION PATTERN
                      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
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                   0.00      0.00      0.00      0.00      0.01      0.01      0.00      0.01      0.00      0.01
                   0.01      0.00      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.02      0.02      0.02      0.03      0.10      0.28      0.04      0.03
                   0.02      0.02      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00

25 JD          INDEX STORM NO. 3
                 STRM        3.53  PRECIPITATION DEPTH
                 TRDA       20.00  TRANSPOSITION DRAINAGE AREA

 0 PI            PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.01      0.01      0.00      0.01      0.00      0.01
                   0.01      0.00      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.02      0.02      0.02      0.03      0.10      0.28      0.04      0.03
                   0.02      0.02      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00

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1
                                                                RUNOFF SUMMARY
                                                        FLOW IN CUBIC FEET PER SECOND
                                                     TIME IN HOURS,  AREA IN SQUARE MILES

                                            PEAK   TIME OF     AVERAGE FLOW FOR MAXIMUM PERIOD      BASIN     MAXIMUM     TIME OF
         OPERATION        STATION           FLOW    PEAK                                             AREA      STAGE     MAX STAGE
+                                                               6-HOUR     24-HOUR     72-HOUR

         HYDROGRAPH AT
+                          76THST           69.   12.50          23.          8.          4.        0.24

         HYDROGRAPH AT
+                            SB01          278.    2.55          30.          8.          4.        0.11

         2 COMBINED AT
```

+		CP-1	276.	2.55	30.	15.	8.	0.35
	ROUTED TO							
+		R1-2	273.	2.70	30.	15.	8.	0.35
	HYDROGRAPH AT							
+		SB02	410.	2.55	46.	12.	6.	0.16
	2 COMBINED AT							
+		CP-2	627.	2.65	76.	26.	13.	0.51
	ROUTED TO							
+		R2-3	618.	2.70	76.	26.	13.	0.51
	HYDROGRAPH AT							
+		SB03	111.	2.60	14.	4.	2.	0.05
	2 COMBINED AT							
+		CP-3	720.	2.70	90.	30.	15.	0.56
	ROUTED TO							
+		R3-5	713.	2.80	90.	30.	15.	0.56
	HYDROGRAPH AT							
+		SB04	440.	2.55	45.	11.	5.	0.14
	ROUTED TO							
+		R4-5	432.	2.65	45.	11.	5.	0.14
	HYDROGRAPH AT							
+		SB05	340.	2.55	36.	9.	4.	0.13
	3 COMBINED AT							
+		CP-5	1265.	2.75	170.	50.	25.	0.83
	ROUTED TO							
+		R5-6	1242.	2.80	169.	50.	25.	0.83
	HYDROGRAPH AT							
+		SB06	355.	2.55	40.	10.	5.	0.14
	2 COMBINED AT							
+		CP-6	1466.	2.80	208.	60.	29.	0.96
	HYDROGRAPH AT							
+		SB07	322.	2.50	21.	5.	2.	0.07
	ROUTED TO							
+		R7-8	310.	2.65	21.	5.	3.	0.07
	HYDROGRAPH AT							
+		SB08	660.	2.55	68.	17.	8.	0.22
	2 COMBINED AT							
+		CP-8	903.	2.65	89.	22.	11.	0.30
	ROUTED TO							
+		R8-9	880.	2.70	89.	22.	11.	0.30
	HYDROGRAPH AT							
+		SB09	147.	2.55	15.	4.	2.	0.05
	2 COMBINED AT							
+		CP-9	985.	2.70	104.	26.	12.	0.35
	HYDROGRAPH AT							
+		PWRCH	3884.	13.35	1547.	503.	248.	7.00
	2 COMBINED AT							
+		BINFLO	3884.	13.35	1547.	503.	260.	7.35
	ROUTED TO							
+		BASIN	463.	18.45	458.	399.	251.	7.35
	ROUTED TO							
+		BSNRT1	463.	18.50	458.	399.	251.	7.35
	ROUTED TO							
+		BSNRT2	463.	18.50	458.	399.	251.	7.35
	HYDROGRAPH AT							
+		SB10	141.	2.50	12.	3.	1.	0.04
	2 COMBINED AT							
+		CP-10	463.	18.50	458.	399.	252.	7.39
	ROUTED TO							
+		R10-11	463.	18.55	458.	399.	252.	7.39
	HYDROGRAPH AT							
+		SB11	195.	2.55	21.	5.	3.	0.07
	2 COMBINED AT							
+		CP-11	463.	18.55	458.	399.	254.	7.46

1

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING

(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	INTERPOLATED TO COMPUTATION INTERVAL DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
FOR STORM = 1 STORM AREA (SQ MI) = 0.00									
R1-2	MANE	3.00	287.35	160.24	1.72	3.00	284.38	162.00	1.72

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3222E+02 EXCESS=0.0000E+00 OUTFLOW=0.3226E+02 BASIN STORAGE=0.5145E-01 PERCENT ERROR= -0.3

FOR STORM = 2 STORM AREA (SQ MI) = 10.00									
R1-2	MANE	3.00	270.30	162.31	1.67	3.00	267.71	162.00	1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3132E+02 EXCESS=0.0000E+00 OUTFLOW=0.3135E+02 BASIN STORAGE=0.5145E-01 PERCENT ERROR= -0.3

FOR STORM = 3 STORM AREA (SQ MI) = 20.00									
R1-2	MANE	3.00	259.93	162.15	1.64	3.00	259.40	162.00	1.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3073E+02 EXCESS=0.0000E+00 OUTFLOW=0.3080E+02 BASIN STORAGE=0.5496E-01 PERCENT ERROR= -0.4

FOR STORM = 1 STORM AREA (SQ MI) = 0.00									
R2-3	MANE	1.51	651.07	161.96	2.07	3.00	651.05	162.00	2.07

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5609E+02 EXCESS=0.0000E+00 OUTFLOW=0.5610E+02 BASIN STORAGE=0.3635E-01 PERCENT ERROR= -0.1

FOR STORM = 2 STORM AREA (SQ MI) = 10.00									
R2-3	MANE	1.68	617.22	162.52	1.98	3.00	607.09	162.00	1.98

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5389E+02 EXCESS=0.0000E+00 OUTFLOW=0.5385E+02 BASIN STORAGE=0.3635E-01 PERCENT ERROR= 0.0

FOR STORM = 3 STORM AREA (SQ MI) = 20.00									
R2-3	MANE	1.57	606.67	162.84	1.93	3.00	594.12	162.00	1.94

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 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5252E+02 EXCESS=0.0000E+00 OUTFLOW=0.5252E+02 BASIN STORAGE=0.3881E-01 PERCENT ERROR=
-0.1


           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
               R3-5  MANE         2.90    746.61    169.74      2.14      3.00    740.89    168.00        2.14


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6349E+02 EXCESS=0.0000E+00 OUTFLOW=0.6368E+02 BASIN STORAGE=0.7132E-01 PERCENT ERROR=
-0.4


           FOR STORM = 2  STORM AREA (SQ MI) =      10.00
               R3-5  MANE         2.99    705.69    168.12      2.05      3.00    703.45    168.00        2.05


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6077E+02 EXCESS=0.0000E+00 OUTFLOW=0.6099E+02 BASIN STORAGE=0.7609E-01 PERCENT ERROR=
-0.5


           FOR STORM = 3  STORM AREA (SQ MI) =      20.00
               R3-5  MANE         2.97    683.83    168.92      2.00      3.00    669.67    168.00        2.00


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5920E+02 EXCESS=0.0000E+00 OUTFLOW=0.5943E+02 BASIN STORAGE=0.7619E-01 PERCENT ERROR=
-0.5


           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
               R4-5  MANE         2.44    447.26    159.45      3.02      3.00    444.07    159.00        3.02


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2308E+02 EXCESS=0.0000E+00 OUTFLOW=0.2320E+02 BASIN STORAGE=0.4656E-04 PERCENT ERROR=
-0.5


           FOR STORM = 2  STORM AREA (SQ MI) =      10.00
               R4-5  MANE         2.59    429.78    158.16      2.87      3.00    425.33    159.00        2.86


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2182E+02 EXCESS=0.0000E+00 OUTFLOW=0.2203E+02 BASIN STORAGE=0.4483E-04 PERCENT ERROR=
-1.0


           FOR STORM = 3  STORM AREA (SQ MI) =      20.00
               R4-5  MANE         2.60    410.95    158.84      2.76      3.00    410.03    159.00        2.76


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2100E+02 EXCESS=0.0000E+00 OUTFLOW=0.2116E+02 BASIN STORAGE=0.3691E-04 PERCENT ERROR=
-0.8
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          FOR STORM = 1  STORM AREA (SQ MI) =       0.00

              R5-6  MANE         1.77   1321.25    168.17      2.39      3.00   1316.26    168.00        2.39


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1051E+03 EXCESS=0.0000E+00 OUTFLOW=0.1053E+03 BASIN STORAGE=0.6328E-01 PERCENT ERROR=
-0.3


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00

              R5-6  MANE         1.94   1236.19    169.97      2.28      3.00   1225.73    171.00        2.28


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1002E+03 EXCESS=0.0000E+00 OUTFLOW=0.1004E+03 BASIN STORAGE=0.6753E-01 PERCENT ERROR=
-0.2


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00

              R5-6  MANE         1.82   1180.28    168.70      2.20      3.00   1169.55    171.00        2.20


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9721E+02 EXCESS=0.0000E+00 OUTFLOW=0.9718E+02 BASIN STORAGE=0.6757E-01 PERCENT ERROR=
0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00

              R7-8  MANE         2.98    320.52    158.47      2.78      3.00    316.88    159.00        2.78


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1062E+02 EXCESS=0.0000E+00 OUTFLOW=0.1097E+02 BASIN STORAGE=0.1432E-03 PERCENT ERROR=
-3.3


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00

              R7-8  MANE         2.87    305.35    159.01      2.61      3.00    304.85    159.00        2.62


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1001E+02 EXCESS=0.0000E+00 OUTFLOW=0.1032E+02 BASIN STORAGE=0.1568E-03 PERCENT ERROR=
-3.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00

              R7-8  MANE         3.00    287.36    160.28      2.59      3.00    278.18    159.00        2.59


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9623E+01 EXCESS=0.0000E+00 OUTFLOW=0.1023E+02 BASIN STORAGE=0.1386E-03 PERCENT ERROR=
-6.3


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00

              R8-9  MANE         1.46    914.50    162.26      2.88      3.00    911.54    162.00        2.88


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4580E+02 EXCESS=0.0000E+00 OUTFLOW=0.4583E+02 BASIN STORAGE=0.2113E-03 PERCENT ERROR=
-0.1
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          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R8-9  MANE         1.32    877.55    161.38      2.73      3.00    866.18    162.00        2.73



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4326E+02 EXCESS=0.0000E+00 OUTFLOW=0.4337E+02 BASIN STORAGE=0.2285E-03 PERCENT ERROR=
-0.3


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R8-9  MANE         1.49    824.02    161.14      2.64      3.00    818.74    162.00        2.64



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4190E+02 EXCESS=0.0000E+00 OUTFLOW=0.4192E+02 BASIN STORAGE=0.2053E-03 PERCENT ERROR=
0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT1  MANE         0.33    462.77   1107.64      2.65      3.00    462.77   1110.00        2.65



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1038E+04 EXCESS=0.0000E+00 OUTFLOW=0.1037E+04 BASIN STORAGE=0.9252E-01 PERCENT ERROR=
0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            BSNRT1  MANE         0.33    462.63   1107.74      2.64      3.00    462.63   1110.00        2.64



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1035E+04 EXCESS=0.0000E+00 OUTFLOW=0.1035E+04 BASIN STORAGE=0.9241E-01 PERCENT ERROR=
0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            BSNRT1  MANE         0.34    462.54   1107.73      2.64      3.00    462.54   1110.00        2.64



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1033E+04 EXCESS=0.0000E+00 OUTFLOW=0.1033E+04 BASIN STORAGE=0.9237E-01 PERCENT ERROR=
0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE         0.48    462.77   1110.07      2.65      3.00    462.77   1110.00        2.65



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1038E+04 EXCESS=0.0000E+00 OUTFLOW=0.1037E+04 BASIN STORAGE=0.1054E+00 PERCENT ERROR=
0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            BSNRT2  MANE         0.37    462.63   1110.22      2.64      3.00    462.63   1110.00        2.64
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 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1035E+04 EXCESS=0.0000E+00 OUTFLOW=0.1035E+04 BASIN STORAGE=0.1056E+00 PERCENT ERROR=
0.0


           FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             BSNRT2  MANE         0.33    462.54   1110.65      2.64      3.00    462.54   1110.00        2.63


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1033E+04 EXCESS=0.0000E+00 OUTFLOW=0.1033E+04 BASIN STORAGE=0.1056E+00 PERCENT ERROR=
0.0


           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R10-11  MANE         2.40    462.77   1113.47      2.64      3.00    462.77   1113.00        2.64


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1043E+04 EXCESS=0.0000E+00 OUTFLOW=0.1042E+04 BASIN STORAGE=0.1560E+01 PERCENT ERROR=
0.0


           FOR STORM = 2  STORM AREA (SQ MI) =      10.00
             R10-11  MANE         2.40    462.63   1113.70      2.64      3.00    462.62   1116.00        2.64


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1040E+04 EXCESS=0.0000E+00 OUTFLOW=0.1039E+04 BASIN STORAGE=0.1557E+01 PERCENT ERROR=
0.0

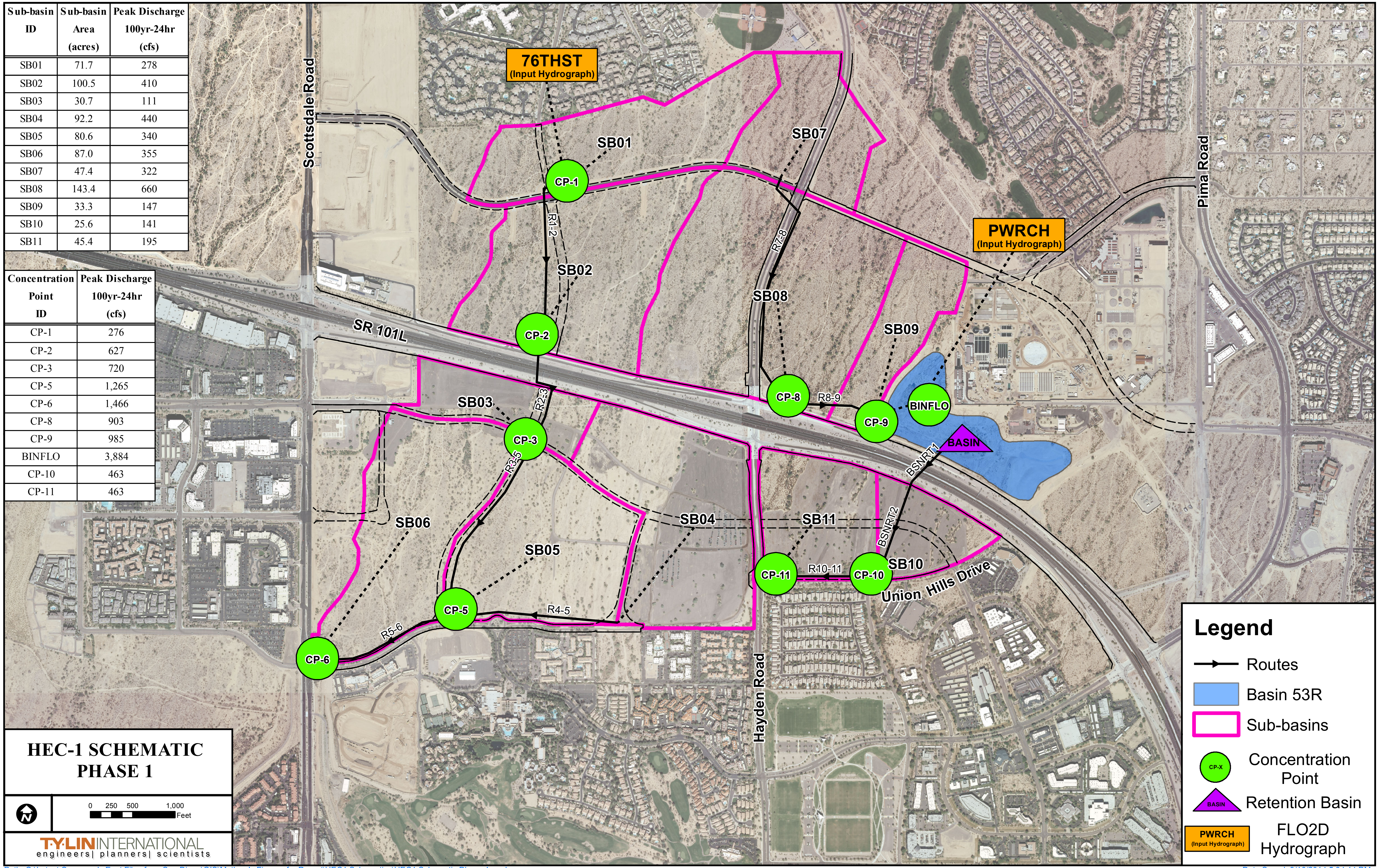

           FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             R10-11  MANE         2.47    462.54   1114.58      2.63      3.00    462.54   1113.00        2.63


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1038E+04 EXCESS=0.0000E+00 OUTFLOW=0.1037E+04 BASIN STORAGE=0.1559E+01 PERCENT ERROR=
0.0


 *** NORMAL END OF HEC-1 ***
```

Sub-basin ID	Sub-basin Area (acres)	Peak Discharge 100yr-24hr (cfs)
SB01	71.7	278
SB02	100.5	410
SB03	30.7	111
SB04	92.2	440
SB05	80.6	340
SB06	87.0	355
SB07	47.4	322
SB08	143.4	660
SB09	33.3	147
SB10	25.6	141
SB11	45.4	195

Concentration Point ID	Peak Discharge 100yr-24hr (cfs)
CP-1	276
CP-2	627
CP-3	720
CP-5	1,265
CP-6	1,466
CP-8	903
CP-9	985
BINFLO	3,884
CP-10	463
CP-11	463



HEC-1 SCHEMATIC PHASE 1

0 250 500 1,000 Feet

TY·LIN INTERNATIONAL
engineers | planners | scientists

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1***************************************
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 *                                       *                                                 *
  *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                 *    U.S. ARMY CORPS OF ENGINEERS
  *
 *             JUN   1998                *                                                 *    HYDROLOGIC ENGINEERING CENTER
  *
 *          VERSION 4.1                  *                                                 *          609 SECOND STREET
  *
 *                                       *                                                 *       DAVIS, CALIFORNIA 95616
  *
 *  RUN DATE   12SEP14  TIME  11:47:04   *                                                 *           (916) 756-1104
  *
 *                                       *                                                 *
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 ***************************************
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                                                  X     X  XXXXXXX   XXXXX         X
                                                  X     X  X        X     X       XX
                                                  X     X  X        X              X
                                                  XXXXXXX  XXXX     X       XXXXX  X
                                                  X     X  X        X              X
                                                  X     X  X        X     X        X
                                                  X     X  XXXXXXX   XXXXX        XXX


           THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

           THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
           THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
           NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
           DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
           KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1                                                           HEC-1 INPUT                                                  PAGE  1

          LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

             1           ID        CROSS ROADS EAST DRAINAGE ALTERNATIVES
             2           ID        RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
             3           ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
             4           ID       100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
             5           ID           PIMA ROAD FLOWS)
             6           ID        PHASE 2 BASIN - SIDESLOPES STEEPENED FROM ~5:1 TO 3:1
             7           ID
             8           ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
             9           ID       MODELERS: RK, MW
            10           ID
            11           IT       3       0       0    1000
            12           IO       3
                         *DIAGRAM
                         *
            13           JD   2.755  0.0001
            14           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
            15           PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
            16           PC   0.962   0.972   0.983   0.991   1.000
            17           JD   2.738  0.5000
            18           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
            19           PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
            20           PC   0.962   0.972   0.983   0.991   1.000
            21           JD   2.686     2.8
            22           PC   0.000   0.009   0.016   0.025   0.034   0.042   0.051   0.059   0.068   0.077
            23           PC   0.088   0.101   0.121   0.164   0.253   0.451   0.694   0.836   0.900   0.938
            24           PC   0.950   0.963   0.975   0.988   1.000
            25           JD   2.540    16.0
            26           PC   0.000   0.015   0.020   0.030   0.048   0.063   0.076   0.090   0.105   0.119
            27           PC   0.135   0.152   0.175   0.222   0.304   0.472   0.670   0.796   0.868   0.912
            28           PC   0.946   0.960   0.973   0.987   1.000
            29           JD   2.232    90.0
            30           PC   0.000   0.021   0.035   0.051   0.071   0.087   0.105   0.125   0.143   0.160
            31           PC   0.179   0.201   0.232   0.281   0.364   0.500   0.658   0.773   0.841   0.888
            32           PC   0.927   0.945   0.964   0.982   1.000
                         *

            33           KK  76THST
            34           KM        76TH STREET CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
            35           KM        100-YR, 6-HR FLO-2D MODEL (XS 98)
            36           BA    0.24
            37           QI       0       0       0       0       0       0       0       0       0       0
            38           QI       0       0       0       0       0       0       0       0       0       0
            39           QI       0       0       0       0       0       0       0       0       0       0
            40           QI       0       0       0       0       0       0       0       0       0       0
            41           QI       0       0       0       0       0       0       0       0       0       0
            42           QI       0       0    0.01       0       0       0       0       0       0       0
            43           QI    0.01       0       0    0.01    0.01    0.01    0.02    0.03    0.05    0.07
```

```
    44          QI    0.09    0.16    0.27    0.42    0.63     0.9    2.34    3.89    7.48   10.26
    45          QI    12.7   14.07   17.67   27.43   38.25   42.26   42.56   42.35   43.77   48.39
    46          QI   49.92   51.19   50.98   50.52   49.38    48.6   44.64   42.62   39.72   36.96
    47          QI   34.49   32.83   31.02    29.6   28.15   26.72   25.66   24.48   23.53   22.56
    48          QI   21.55   20.83   19.46   20.34   18.32   17.51   16.55   16.22   15.59   15.15
    49          QI   14.81   14.23   13.72   13.18   13.21   12.84   12.36   12.34   12.16      12
    50          QI   11.63   11.31   11.37   11.23   11.13   10.56   10.63   10.27    9.98    9.86
    51          QI    9.58    9.02    9.34    8.67    8.51    8.32    8.08    7.75    7.43    7.37
1                                           HEC-1 INPUT                                            PAGE  2

  LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

    52          QI    7.29    7.19    7.08       7    6.89    6.73    6.55     6.4    6.23    6.02
    53          QI    5.83    5.66    5.06    4.57    4.75    4.76    4.65    4.55    4.44    4.32
    54          QI    4.23    4.15    4.05    3.95    3.87    3.81    3.68    3.54    3.48    3.42
    55          QI     3.4    2.42    2.38     3.4    3.61    3.55    3.49    2.89    2.36    2.36
    56          QI    2.36    2.37    2.44    3.67    2.44    2.51    2.49    2.47    2.42    2.37
    57          QI    2.32    2.29    2.26    2.23    2.19    2.16    2.13     2.1    2.07    2.05
    58          QI    2.02    1.99    1.96    1.91    1.86    1.83    1.78    1.69    1.58    1.54
    59          QI    1.58    1.65    1.69    1.67    1.59     1.5    1.36    1.41    1.51    1.54
    60          QI    1.52    1.35    1.43    1.37    1.25    1.21     1.2     1.2    1.23    1.28
    61          QI    1.31    1.34    1.35    1.35    1.34    1.34    1.33    1.33    1.26    1.09
    62          QI    1.09    1.11    1.16    1.22    1.06    1.07    1.15    1.06    1.05    1.05
    63          QI    1.04    1.02    1.01       1    0.98    0.97    0.96    0.95    0.94    0.92
    64          QI    0.91     0.9    0.89    0.88    0.87    0.86    0.85    0.85    0.84    0.84
    65          QI    0.83    0.82    0.81     0.8    0.79    0.79    0.78    0.77    0.77    0.76
    66          QI    0.76    0.75    0.74    0.74    0.73    0.72    0.71    0.71     0.7    0.69
    67          QI    0.69    0.68    0.68    0.67    0.67    0.67    0.66    0.66    0.65    0.65
    68          QI    0.65    0.64    0.64    0.63    0.63    0.62    0.62    0.62    0.61    0.61
    69          QI     0.6     0.6    0.59    0.59    0.58    0.58    0.57    0.57    0.56    0.56
    70          QI    0.55    0.55    0.54    0.54    0.54    0.53    0.53    0.52    0.52    0.52
    71          QI    0.51    0.51     0.5     0.5     0.5    0.49    0.49    0.48    0.48    0.48
    72          QI    0.47    0.47    0.46    0.46    0.46    0.45    0.45    0.45    0.44    0.44
    73          QI    0.44
                *

    74          KK    SB01   BASIN
    75          BA   0.112
    76          LG    0.25    0.25    4.10    0.55      45
    77          UC   0.317   0.252
    78          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
    79          UA     100
                *

    80          KK    CP-1 COMBINE
    81          KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
    82          HC       2
                *

    83          KK    R1-2   ROUTE
    84          KM        MILLER RD CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
    85          RK    2104  0.0015   0.03             TRAP      66       4
                *

    86          KK    SB02   BASIN
    87          BA   0.157
    88          LG    0.15    0.25    4.15    0.58      56
    89          UC   0.324   0.237
    90          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
    91          UA     100
                *
1                                           HEC-1 INPUT                                            PAGE  3

  LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


    92          KK    CP-2 COMBINE
    93          KM        SR 101L FREEWAY AND 76TH ST (MILLER RD CHANNEL)
    94          HC       2
                *

    95          KK    R2-3   ROUTE
    96          KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
    97          RK    1260  0.0015   0.03             TRAP      92       4
                *

    98          KK    SB03   BASIN
    99          BA   0.048
   100          LG    0.15    0.25    4.50    0.47      55
   101          UC   0.360   0.290
   102          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
   103          UA     100
                *

   104          KK    CP-3 COMBINE
   105          KM        MAYO BLVD AND 76TH ST (MILLER RD CHANNEL)
   106          HC       2
                *

   107          KK    R3-5   ROUTE
   108          KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
   109          RK    2396  0.0015   0.03             TRAP      98       4
                *
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 110           KK    SB04   BASIN
 111           BA   0.144
 112           LG    0.14    0.25    4.60    0.44      61
 113           UC   0.301   0.196
 114           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 115           UA     100
               *

 116           KK    R4-5   ROUTE
 117           KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 118           RK    2005  0.0013   0.03             TRAP      39       4
               *

 119           KK    SB05   BASIN
 120           BA   0.126
 121           LG    0.22    0.25    4.50    0.44      48
 122           UC   0.326   0.224
 123           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 124           UA     100
               *
1                                        HEC-1 INPUT                                               PAGE  4

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 125           KK    CP-5 COMBINE
 126           KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 127           HC       3
               *

 128           KK    R5-6   ROUTE
 129           KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 130           RK    1550  0.0015   0.03             TRAP     188       4
               *

 131           KK    SB06   BASIN
 132           BA   0.136
 133           LG    0.16    0.25    4.55    0.45      53
 134           UC   0.318   0.243
 135           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 136           UA     100
               *

 137           KK    CP-6 COMBINE
 138           KM        PRINCESS BLVD AND SCOTTSDALE RD (PRINCESS BLVD CHANNEL)
 139           HC       2
               *

 140           KK    SB07   BASIN
 141           BA   0.074
 142           LG    0.21    0.25    4.00    0.58      49
 143           UC   0.204   0.113
 144           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 145           UA     100
               *

 146           KK    R7-8   ROUTE
 147           KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 148           RK    2778  0.0014   0.03             TRAP      46       4
               *

 149           KK    SB08   BASIN
 150           BA   0.224
 151           LG    0.14    0.25    4.10    0.59      59
 152           UC   0.333   0.193
 153           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 154           UA     100
               *

 155           KK    CP-8 COMBINE
 156           KM        SR 101L FREEWAY AND HAYDEN ROAD (HAYDEN ROAD NORTH CHANNEL)
 157           HC       2
               *
1                                        HEC-1 INPUT                                               PAGE  5

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 158           KK    R8-9   ROUTE
 159           KM        HAYDEN ROAD NORTH CHANNEL FROM HAYDEN ROAD TO BASIN 53R
 160           RK    1250  0.0013   0.03             TRAP      67       4
               *

 161           KK    SB09   BASIN
 162           BA   0.052
 163           LG    0.15    0.25    4.00    0.61      55
 164           UC   0.251   0.227
 165           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 166           UA     100
               *

 167           KK    CP-9 COMBINE
 168           KM        SR 101L FREEWAY AND BASIN 53R (HAYDEN ROAD NORTH CHANNEL)
 169           HC       2
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                *

   170          KK  PIMACH
   171          KM        PIMA ROAD CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
   172          KM        100-YR, 6-HR FLO-2D MODEL (XS 183)
   173          BA    12.3
   174          QI       0       0       0       0       0       0       0       0       0       0
   175          QI       0       0       0       0       0       0       0       0       0       0
   176          QI       0       0       0       0       0       0       0       0       0       0
   177          QI       0       0       0       0       0       0       0       0       0       0
   178          QI       0       0       0       0       0       0       0       0       0       0
   179          QI       0       0       0       0       0       0       0       0       0       0
   180          QI       0       0       0       0       0       0    0.01    0.01    0.01    0.01
   181          QI    0.02    0.06    0.12    0.19    0.29    0.43    1.62    3.73    6.28   13.12
   182          QI   19.17   18.99   20.23   23.44    37.1    65.6  118.28  536.07  740.49  784.96
   183          QI  795.73  792.08  785.39  784.16  826.86  835.79  839.66  856.83  890.85 1074.56
   184          QI  1522.8 1924.46 2612.09 2761.62 3125.86 3450.94 3565.35 3786.18 3953.42 4005.23
   185          QI  4017.2 3956.42 3828.66 3704.96 3584.74 3457.47 3327.91 3209.52  3068.2 2902.83
   186          QI  2698.1 2519.92 2353.91 2218.47 2091.25 1970.95 1868.74 1749.96 1656.67 1564.23
   187          QI  1473.2 1392.42 1326.33 1253.44 1190.18 1121.44 1068.33  1012.4  961.04  907.15
   188          QI     859  817.16  777.05  741.05  708.67  676.64  650.15     620  595.68  569.46
   189          QI   552.9  528.08  508.87  488.92  471.15  456.27  440.43  424.35  410.65  396.26
   190          QI  384.05   374.5  363.39  351.31  341.86  332.93  325.13  317.66  308.91  300.01
   191          QI  291.96  282.64   276.3  270.44  264.03  257.52  251.57  244.49  238.62  232.96
   192          QI  227.38  223.46  217.91  210.73  207.24  202.22  197.15  192.34  189.04  184.57
   193          QI  178.93   173.7  170.95   166.9  162.57  158.97  155.56  151.97  149.46  146.92
   194          QI   143.8  141.62  138.05  136.38   130.4  128.32  123.98  120.84   117.3  114.09
   195          QI  113.28  110.56  108.01  104.98  103.24  100.82   98.69   97.77    95.8   93.56
   196          QI    92.8   91.19   88.35   86.44   84.49   82.18    80.7   78.31   76.07   74.85
   197          QI   73.81   73.17   71.94   70.66   69.66   69.54   67.38   67.61   65.17   62.74
   198          QI   61.41   60.29   61.36   59.06   56.97   56.32   55.01   53.88   52.91   52.23
   199          QI   52.11   50.68   50.15    49.6   48.36    47.6   46.82    46.4   45.54   44.69
   200          QI   43.67   43.34   42.77    41.9   41.04   40.86   40.39   39.72   38.49   38.26
   201          QI   37.01   36.83   36.64   35.35   35.64   34.94   35.17    33.9    33.7   33.71
   202          QI   33.78   32.55   32.35   31.97   31.61   31.38   31.21   30.84   30.41      30
   203          QI   29.61   29.27   28.72   28.17   27.62   27.26   26.95   26.26   25.93   25.66
   204          QI    25.7   25.62    25.7   25.39   24.94   24.57   24.28   23.92   23.55   22.97
   205          QI    22.7   22.79   22.23   21.94   21.87    21.6    21.3   21.18   21.05   20.75
1                                             HEC-1 INPUT                                                PAGE  6

  LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

   206          QI   20.64    20.4   20.19   19.93    19.9   19.01   18.72    18.7   18.37   18.27
   207          QI    18.1   18.05    17.3   17.39   17.27   17.09   16.86   16.81   16.73    16.6
   208          QI   16.55   16.33   16.05   15.82   15.63   15.43   15.38   15.42   15.17   14.98
   209          QI   14.82   14.78   14.76   14.57   14.24   14.01   13.85   13.94   13.77   13.77
   210          QI    12.6
                *

   211          KK  PIMAIN
   212          KM    BYPASS ALONG SE SIDE OF BASIN 53R TO ADOT CULVERT NEAR UNION HILLS DR
   213          DT  PIMABY
   214          DI       0    1000   10000
   215          DQ       0    1000    1000
                *

   216          KK   PWRCH
   217          KM        POWERLINE CHANNEL HYDROGRAPH FROM FLO-2D
   218          KM        100-YR, 6-HR FLO-2D MODEL (XS 107)
   219          BA     7.0
   220          QI       0       0       0       0       0       0       0       0       0       0
   221          QI       0       0       0       0       0       0       0       0       0       0
   222          QI       0       0       0       0       0       0       0       0       0       0
   223          QI       0       0       0       0       0       0       0       0       0       0
   224          QI       0       0       0       0       0       0       0       0       0       0
   225          QI       0       0       0       0       0       0       0       0       0       0
   226          QI       0       0       0       0       0       0       0       0       0       0
   227          QI       0       0    0.03    0.05    0.08    0.11    0.58    1.13    1.62    2.35
   228          QI   17.74   24.61   32.43    41.1      46   50.91  131.64  355.54   399.7  422.85
   229          QI  433.53  433.73  422.44  409.12  397.71  391.08  388.91   381.3  371.02  361.84
   230          QI  359.13  372.83  390.54  400.38  398.98  391.64  390.01  419.65  505.31  577.51
   231          QI  616.52   654.6  689.73   713.7  727.81  737.85  739.81  733.87  725.12  714.86
   232          QI  699.64  690.12  677.33  666.62  655.55  645.41  637.05  633.81  630.71  627.88
   233          QI  621.52  616.56  611.87  606.82  603.07  599.48  598.68  597.46  596.17  595.28
   234          QI  594.12  590.26  586.44  582.19  576.08  570.84  564.08  555.83  546.74  537.07
   235          QI  525.86  516.15  505.83  495.17  484.12  472.45  462.13  451.31  441.83  429.35
   236          QI  419.09  410.42  401.94   394.3  384.43  375.56  366.51  358.68  350.24   342.9
   237          QI     335  326.95  320.66  313.52  306.21  299.42  294.88  279.13  274.41  272.37
   238          QI   270.9  258.49  256.83  252.87  239.19  235.68  236.09  226.97  221.44  220.04
   239          QI  211.13  213.73  201.19  203.88   194.6  196.81  183.61  188.47  178.11  181.39
   240          QI  172.83  170.18  168.28  163.33  169.45  155.83  153.79  154.42  148.45  145.47
   241          QI  146.04  140.33  138.63  140.19  133.49  130.23  135.24  122.64  121.61  121.05
   242          QI   120.9  117.47  115.13  113.28   116.6  110.32  107.03  105.71  109.02  101.22
   243          QI   99.25   97.86   99.48  103.28   92.95    90.9   89.54   87.83   91.59   84.19
   244          QI   82.93    81.6   80.75   79.42   82.41   77.68   75.61   74.44   74.51   72.29
   245          QI    79.3   70.93   70.15   69.48   68.44   67.58   66.72   72.25   65.21   64.67
   246          QI    63.5   62.83    62.6   62.06   61.31   64.86   60.25   59.05   58.48   55.87
   247          QI   56.01   55.72    54.8   54.06   53.38   51.64   51.71   50.75   49.83    49.2
   248          QI   48.75   46.82   45.85   45.55    47.8   43.99   43.77   43.28   42.64   42.31
   249          QI   42.04   41.65   41.41   40.69    40.3   40.09   39.93   39.53   42.18   38.64
   250          QI   37.99   37.44   37.13   36.96    36.8   36.19   35.56   35.28   34.71    33.9
   251          QI   33.42   33.14   32.73   39.25   32.39   31.56   30.95   30.66    30.3      30
   252          QI   29.22   28.72   29.47   28.39   28.32   28.13   28.24   28.22   27.67   27.34
   253          QI   27.15   27.03   26.51   25.94   25.77   31.57   25.38   25.06   25.04   24.52
```

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254            QI   24.31   24.43   24.27   23.35   23.93    23.1   22.66   22.71   22.57   22.79
255            QI   22.09   21.91   21.94   21.84   21.71   21.66   21.87   21.85   21.36   21.06
256            QI   20.95
               *
1                                          HEC-1 INPUT                                       PAGE  7

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


257            KK  BINFLO
258            KM         TOTAL INFLOW INTO BASIN 53R.
259            HC       3
               *

260            KK   BASIN
261            KM    BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
262            KM    ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
263            KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
264            KM        THE OUTLET PIPES ARE INLET CONTROLLED.
265            RS       1    STOR       0
266            SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
267            SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
268            SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
269            SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
270            SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
271            SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
               *

272            KK  BSNRT1
273            KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
274            KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
275            KM        EXCESS CAPACITY.
276            RK     550  0.0052   0.024            CIRC     7.0
               *

277            KK  BSNRT2
278            KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BASIN 53R OUTFAL
279            RK    1200  0.0077   0.013            CIRC     7.0
               *

280            KK  PIMABY
281            KM    RETRIEVE PIMA BYPASS CHANNEL HYDROGRAPH
282            DR  PIMABY
               *

283            KK  PMB-RT
284            KM        UNION HILLS DR CHANNEL FROM SR 101L FREEWAY TO 82ND ST
285            RK    3157  0.0082   0.013       0    TRAP      24       4
               *

286            KK    SB10   BASIN
287            BA   0.040
288            LG    0.15    0.25    4.25    0.55      55
289            UC   0.230   0.159
290            UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
291            UA     100
               *
1                                          HEC-1 INPUT                                       PAGE  8

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


292            KK   CP-10 COMBINE
293            KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
294            HC       3
               *

295            KK  R10-11   ROUTE
296            KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
297            RK    1277  0.0014   0.03             TRAP     220       4
               *

298            KK    SB11   BASIN
299            BA   0.071
300            LG    0.15    0.25    4.15    0.58      55
301            UC   0.292   0.229
302            UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
303            UA     100
               *

304            KK   CP-11 COMBINE
305            KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
306            HC       2
               *
307            ZZ
1
          SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING         (--->) DIVERSION OR PUMP FLOW

   NO.      (.) CONNECTOR       (<---) RETURN OF DIVERTED OR PUMPED FLOW

    33      76THST
              .
```

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              .
  74          .        SB01
              .           .
              .           .
  80       CP-1............
              V
              V
  83       R1-2
              .
              .
  86          .        SB02
              .           .
              .           .
  92       CP-2............
              V
              V
  95       R2-3
              .
              .
  98          .        SB03
              .           .
              .           .
 104       CP-3............
              V
              V
 107       R3-5
              .
              .
 110          .        SB04
              .           V
              .           V
 116          .        R4-5
              .           .
              .           .
 119          .           .        SB05
              .           .           .
              .           .           .
 125       CP-5........................
              V
              V
 128       R5-6
              .
              .
 131          .        SB06
              .           .
              .           .
 137       CP-6............
              .
              .
 140          .        SB07
              .           V
              .           V
 146          .        R7-8
              .           .
              .           .
 149          .           .        SB08
              .           .           .
              .           .           .
 155          .        CP-8............
              .           V
              .           V
 158          .        R8-9
              .           .
              .           .
 161          .           .        SB09
              .           .           .
              .           .           .
 167          .        CP-9............
              .           .
              .           .
 170          .           .      PIMACH
              .           .           .
              .           .           .
 213          .           .           .------->  PIMABY
 211          .           .      PIMAIN
              .           .           .
              .           .           .
 216          .           .           .       PWRCH
              .           .           .           .
              .           .           .           .
 257          .      BINFLO........................
              .           V
              .           V
 260          .       BASIN
              .           V
              .           V
 272          .      BSNRT1
              .           V
              .           V
 277          .      BSNRT2
              .           .
              .           .
 282          .           .           .<-------  PIMABY
 280          .           .      PIMABY
```

```
              .           .           V
              .           .           V
  283         .           .       PMB-RT
              .           .           .
              .           .           .
  286         .           .           .        SB10
              .           .           .           .
              .           .           .           .
  292         .       CP-10.......................
              .           V
              .           V
  295         .      R10-11
              .           .
              .           .
  298         .           .       SB11
              .           .           .
              .           .           .
  304         .       CP-11...........

 (***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1***************************************** 
***************************************
 *                                        *                                                   *
  *
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)    *                                                   *    U.S. ARMY CORPS OF ENGINEERS
  *
 *             JUN   1998                 *                                                   *    HYDROLOGIC ENGINEERING CENTER
  *
 *           VERSION 4.1                  *                                                   *          609 SECOND STREET
  *
 *                                        *                                                   *       DAVIS, CALIFORNIA 95616
  *
 *  RUN DATE   12SEP14  TIME  11:47:04    *                                                   *           (916) 756-1104
  *
 *                                        *                                                   *
  *
 *****************************************
***************************************



                                    CROSS ROADS EAST DRAINAGE ALTERNATIVES
                                    RESERVOIR ROUTING OF BASIN N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                                    INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                                      100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                                       PIMA ROAD FLOWS)
                                    PHASE 2 BASIN - SIDESLOPES STEEPENED FROM ~5:1 TO 3:1

                                    PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                                      MODELERS: RK, MW

   12 IO          OUTPUT CONTROL VARIABLES
                        IPRNT           3  PRINT CONTROL
                        IPLOT           0  PLOT CONTROL
                        QSCAL          0.  HYDROGRAPH PLOT SCALE

      IT          HYDROGRAPH TIME DATA
                         NMIN           3  MINUTES IN COMPUTATION INTERVAL
                        IDATE      1    0  STARTING DATE
                        ITIME        0000  STARTING TIME
                           NQ        1000  NUMBER OF HYDROGRAPH ORDINATES
                       NDDATE      3    0  ENDING DATE
                       NDTIME        0157  ENDING TIME
                       ICENT           19  CENTURY MARK

                    COMPUTATION INTERVAL    0.05 HOURS
                         TOTAL TIME BASE   49.95 HOURS

          ENGLISH UNITS
               DRAINAGE AREA         SQUARE MILES
               PRECIPITATION DEPTH   INCHES
               LENGTH, ELEVATION     FEET
               FLOW                  CUBIC FEET PER SECOND
               STORAGE VOLUME        ACRE-FEET
               SURFACE AREA          ACRES
               TEMPERATURE           DEGREES FAHRENHEIT

   13 JD          INDEX STORM NO. 1
                         STRM        2.76  PRECIPITATION DEPTH
                         TRDA        0.00  TRANSPOSITION DRAINAGE AREA

   14 PI            PRECIPITATION PATTERN
                       0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                       0.01      0.02      0.02      0.08      0.16      0.46      0.08      0.02      0.02      0.01
                       0.01      0.01      0.01      0.01

   17 JD          INDEX STORM NO. 2
                         STRM        2.74  PRECIPITATION DEPTH
                         TRDA        0.50  TRANSPOSITION DRAINAGE AREA

   18 PI            PRECIPITATION PATTERN
```

```
                    0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01
                    0.01     0.02     0.02     0.08     0.16     0.46     0.08     0.02     0.02     0.01
                    0.01     0.01     0.01     0.01

  21 JD          INDEX STORM NO. 3
                      STRM        2.69  PRECIPITATION DEPTH
                      TRDA        2.80  TRANSPOSITION DRAINAGE AREA

  22 PI            PRECIPITATION PATTERN
                    0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01     0.01
                    0.01     0.02     0.04     0.09     0.20     0.24     0.14     0.06     0.04     0.01
                    0.01     0.01     0.01     0.01

  25 JD          INDEX STORM NO. 4
                      STRM        2.54  PRECIPITATION DEPTH
                      TRDA       16.00  TRANSPOSITION DRAINAGE AREA

  26 PI            PRECIPITATION PATTERN
                    0.01     0.00     0.01     0.02     0.02     0.01     0.01     0.01     0.01     0.02
                    0.02     0.02     0.05     0.08     0.17     0.20     0.13     0.07     0.04     0.03
                    0.01     0.01     0.01     0.01

  29 JD          INDEX STORM NO. 5
                      STRM        2.23  PRECIPITATION DEPTH
                      TRDA       90.00  TRANSPOSITION DRAINAGE AREA

  30 PI            PRECIPITATION PATTERN
                    0.02     0.01     0.02     0.02     0.02     0.02     0.02     0.02     0.02     0.02
                    0.02     0.03     0.05     0.08     0.14     0.16     0.12     0.07     0.05     0.04
                    0.02     0.02     0.02     0.02


 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
***


            **************
            *            *
  33 KK     *   76THST   *
            *            *
            **************
                         76TH STREET CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
                         100-YR, 6-HR FLO-2D MODEL (XS 98)

              SUBBASIN RUNOFF DATA

  36 BA          SUBBASIN CHARACTERISTICS
                     TAREA        0.24  SUBBASIN AREA

                                                   ***

       ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION   76THST
                     TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW      TIME                       MAXIMUM AVERAGE FLOW
                                    6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+      51.      4.55                 15.          4.          2.           2.
                     (INCHES)     0.582       0.664       0.728        0.728
                      (AC-FT)        7.          9.          9.           9.

                      CUMULATIVE AREA =    0.24 SQ MI


       ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION   76THST
                     TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW      TIME                       MAXIMUM AVERAGE FLOW
                                    6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+      51.      4.55                 15.          4.          2.           2.
                     (INCHES)     0.582       0.664       0.728        0.728
                      (AC-FT)        7.          9.          9.           9.

                      CUMULATIVE AREA =    0.24 SQ MI


       ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION   76THST
                     TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW      TIME                       MAXIMUM AVERAGE FLOW
                                    6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+      51.      4.55                 15.          4.          2.           2.
```

```
                        (INCHES)     0.582       0.664       0.728        0.728
                         (AC-FT)        7.          9.          9.           9.

                         CUMULATIVE AREA =    0.24 SQ MI


      ***              ***              ***              ***              ***

                          HYDROGRAPH AT STATION   76THST
                        TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+      51.      4.55                      15.          4.          2.           2.
                        (INCHES)     0.582       0.664       0.728        0.728
                         (AC-FT)        7.          9.          9.           9.

                         CUMULATIVE AREA =    0.24 SQ MI


      ***              ***              ***              ***              ***

                          HYDROGRAPH AT STATION   76THST
                        TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+      51.      4.55                      15.          4.          2.           2.
                        (INCHES)     0.582       0.664       0.728        0.728
                         (AC-FT)        7.          9.          9.           9.

                         CUMULATIVE AREA =    0.24 SQ MI

      ***              ***              ***              ***              ***



                         INTERPOLATED HYDROGRAPH AT   76THST

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+      51.      4.55                      15.          4.          2.           2.
                        (INCHES)     0.582       0.664       0.728        0.728
                         (AC-FT)        7.          9.          9.           9.

                         CUMULATIVE AREA =    0.24 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
***


             **************
             *            *
   74 KK     *      SB01  *        BASIN
             *            *
             **************

                SUBBASIN RUNOFF DATA

   75 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.11  SUBBASIN AREA

   76 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.25  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.10  WETTING FRONT SUCTION
                        XKSAT        0.55  HYDRAULIC CONDUCTIVITY
                        RTIMP       45.00  PERCENT IMPERVIOUS AREA

   77 UC          CLARK UNITGRAPH
                           TC        0.32  TIME OF CONCENTRATION
                            R        0.25  STORAGE COEFFICIENT

   78 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.32 HR,       R=  0.25 HR
                                                       SNYDER   TP=  0.19 HR,      CP=  0.47

                                                                   UNIT HYDROGRAPH
                                                              30 END-OF-PERIOD ORDINATES
```

```
              15.       58.      129.      172.      167.      153.      134.      112.       91.       75.
              61.       50.       41.       34.       28.       23.       19.       15.       12.       10.
               8.        7.        6.        5.        4.        3.        3.        2.        2.        1.

     ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB01
                            TRANSPOSITION AREA      0.0 SQ MI

       TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.59, TOTAL EXCESS =    2.17

    PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                              (CFS)
 +     326.      0.95                      26.          7.          3.           3.
                            (INCHES)     2.159       2.159       2.159        2.159
                             (AC-FT)       13.         13.         13.          13.

                             CUMULATIVE AREA =    0.11 SQ MI


     ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB01
                            TRANSPOSITION AREA      0.5 SQ MI

       TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.59, TOTAL EXCESS =    2.15

    PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                              (CFS)
 +     323.      0.95                      26.          6.          3.           3.
                            (INCHES)     2.143       2.143       2.143        2.143
                             (AC-FT)       13.         13.         13.          13.

                             CUMULATIVE AREA =    0.11 SQ MI


     ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB01
                            TRANSPOSITION AREA      2.8 SQ MI

       TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.63, TOTAL EXCESS =    2.05

    PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                              (CFS)
 +     287.      1.00                      25.          6.          3.           3.
                            (INCHES)     2.044       2.044       2.044        2.044
                             (AC-FT)       12.         12.         12.          12.

                             CUMULATIVE AREA =    0.11 SQ MI


     ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB01
                            TRANSPOSITION AREA     16.0 SQ MI

       TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.68, TOTAL EXCESS =    1.86

    PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                              (CFS)
 +     241.      1.00                      22.          6.          3.           3.
                            (INCHES)     1.850       1.850       1.850        1.850
                             (AC-FT)       11.         11.         11.          11.

                             CUMULATIVE AREA =    0.11 SQ MI


     ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB01
                            TRANSPOSITION AREA     90.0 SQ MI

       TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.72, TOTAL EXCESS =    1.52

    PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                              (CFS)
 +     182.      1.00                      18.          5.          2.           2.
                            (INCHES)     1.510       1.510       1.510        1.510
                             (AC-FT)        9.          9.          9.           9.

                             CUMULATIVE AREA =    0.11 SQ MI

     ***             ***             ***             ***             ***
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                         INTERPOLATED HYDROGRAPH AT     SB01

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     324.      0.95                      26.          6.          3.           3.
                          (INCHES)     2.146       2.146       2.146        2.146
                           (AC-FT)       13.         13.         13.          13.

                         CUMULATIVE AREA =    0.11 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
***


              **************
              *            *
   80 KK      *      CP-1  *      COMBINE
              *            *
              **************
                                 LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)

   82 HC          HYDROGRAPH COMBINATION
                        ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                             ***

       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-1
                        TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     326.      0.95                      37.         11.          5.           5.
                          (INCHES)     0.987       1.133       1.183        1.183
                           (AC-FT)       19.         21.         22.          22.

                         CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-1
                        TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     323.      0.95                      37.         11.          5.           5.
                          (INCHES)     0.982       1.128       1.178        1.178
                           (AC-FT)       18.         21.         22.          22.

                         CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-1
                        TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     287.      1.00                      36.         10.          5.           5.
                          (INCHES)     0.951       1.096       1.147        1.147
                           (AC-FT)       18.         21.         22.          22.

                         CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     CP-1
                        TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     241.      1.00                      34.         10.          5.           5.
                          (INCHES)     0.886       1.035       1.085        1.085
                           (AC-FT)       17.         19.         20.          20.
```

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                        CUMULATIVE AREA =    0.35 SQ MI


      ***            ***            ***            ***            ***

                         HYDROGRAPH AT STATION     CP-1
                       TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     182.      1.00                    29.           9.          4.           4.
                         (INCHES)     0.776       0.927       0.977        0.977
                          (AC-FT)       15.         17.         18.          18.

                        CUMULATIVE AREA =    0.35 SQ MI

      ***            ***            ***            ***            ***



                        INTERPOLATED HYDROGRAPH AT     CP-1

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     323.      0.95                    37.          11.          5.           5.
                         (INCHES)     0.982       1.128       1.178        1.178
                          (AC-FT)       18.         21.         22.          22.

                        CUMULATIVE AREA =    0.35 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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            **************
            *            *
   83 KK    *      R1-2  *        ROUTE
            *            *
            **************
                                MILLER RD CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY

               HYDROGRAPH ROUTING DATA

   85 RK          KINEMATIC WAVE STREAM ROUTING
                            L       2104.  CHANNEL LENGTH
                            S      0.0015  SLOPE
                            N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                           CA        0.00  CONTRIBUTING AREA
                        SHAPE        TRAP  CHANNEL SHAPE
                           WD       66.00  BOTTOM WIDTH OR DIAMETER
                            Z        4.00  SIDE SLOPE
                       NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                 ***
                                      COMPUTED KINEMATIC PARAMETERS
                                            VARIABLE TIME STEP
                                          (DT SHOWN IS A MINIMUM)

                    ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                CELERITY
                                                   (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                   MAIN          0.17      1.56       3.00    701.33    324.42     63.63      1.19      3.89


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2221E+02  EXCESS=0.0000E+00 OUTFLOW=0.2234E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=
-0.8

                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                   MAIN          0.17      1.56       3.00              317.42     66.00      1.19

      ***            ***            ***            ***            ***

                         HYDROGRAPH AT STATION     R1-2
                       TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     317.      1.10                    38.          11.          5.           5.
                         (INCHES)     0.995       1.140       1.189        1.189
                          (AC-FT)       19.         21.         22.          22.
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                    CUMULATIVE AREA =    0.35 SQ MI

                                   COMPUTED KINEMATIC PARAMETERS
                                        VARIABLE TIME STEP
                                      (DT SHOWN IS A MINIMUM)

                   ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                PEAK                 CELERITY
                                                   (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

                  MAIN          0.17      1.56       3.00    701.33    321.94     63.70      1.18      3.88

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2212E+02  EXCESS=0.0000E+00 OUTFLOW=0.2223E+02 BASIN STORAGE=0.5936E-01 PERCENT ERROR=
-0.8

                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                  MAIN          0.17      1.56       3.00              314.95     66.00      1.18

      ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     R1-2
                   TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                       MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     315.      1.10                 37.         11.          5.           5.
                       (INCHES)    0.988       1.134       1.184        1.184
                        (AC-FT)      19.         21.         22.          22.

                    CUMULATIVE AREA =    0.35 SQ MI

                                   COMPUTED KINEMATIC PARAMETERS
                                        VARIABLE TIME STEP
                                      (DT SHOWN IS A MINIMUM)

                   ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                PEAK                 CELERITY
                                                   (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

                  MAIN          0.17      1.56       3.00    701.33    281.88     67.46      1.15      3.72

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2153E+02  EXCESS=0.0000E+00 OUTFLOW=0.2160E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=
-0.6

                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                  MAIN          0.17      1.56       3.00              280.31     66.00      1.15

      ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     R1-2
                   TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                       MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     280.      1.10                 36.         10.          5.           5.
                       (INCHES)    0.955       1.100       1.149        1.149
                        (AC-FT)      18.         21.         22.          22.

                    CUMULATIVE AREA =    0.35 SQ MI

                                   COMPUTED KINEMATIC PARAMETERS
                                        VARIABLE TIME STEP
                                      (DT SHOWN IS A MINIMUM)

                   ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                PEAK                 CELERITY
                                                   (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

                  MAIN          0.17      1.56       3.00    701.33    239.43     66.99      1.09      3.49

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2036E+02  EXCESS=0.0000E+00 OUTFLOW=0.2044E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=
-0.7

                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                  MAIN          0.17      1.56       3.00              234.05     69.00      1.09

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                         HYDROGRAPH AT STATION     R1-2
                       TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                               MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     234.      1.15                    34.         10.          5.           5.
                       (INCHES)     0.887       1.038       1.087        1.087
                        (AC-FT)       17.         19.         20.          20.

                        CUMULATIVE AREA =    0.35 SQ MI

                                          COMPUTED KINEMATIC PARAMETERS
                                                VARIABLE TIME STEP
                                              (DT SHOWN IS A MINIMUM)

                    ELEMENT    ALPHA       M          DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                    PEAK                CELERITY
                                                    (MIN)      (FT)     (CFS)     (MIN)       (IN)     (FPS)

                   MAIN          0.17      1.56       2.91     526.00    179.26     70.19      0.98      3.16


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1834E+02  EXCESS=0.0000E+00 OUTFLOW=0.1842E+02 BASIN STORAGE=0.6416E-01 PERCENT ERROR=
-0.8


                                             INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                   MAIN          0.17      1.56       3.00               178.38     69.00      0.98

       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     R1-2
                       TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                               MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     178.      1.15                    30.          9.          4.           4.
                       (INCHES)     0.780       0.932       0.981        0.981
                        (AC-FT)       15.         17.         18.          18.

                        CUMULATIVE AREA =    0.35 SQ MI

       ***             ***             ***             ***             ***



                        INTERPOLATED HYDROGRAPH AT     R1-2

  PEAK FLOW     TIME                               MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     315.      1.10                    37.         11.          5.           5.
                       (INCHES)     0.989       1.134       1.184        1.184
                        (AC-FT)       19.         21.         22.          22.

                        CUMULATIVE AREA =    0.35 SQ MI



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             *            *
   86 KK     *      SB02  *        BASIN
             *            *
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                SUBBASIN RUNOFF DATA

   87 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.16  SUBBASIN AREA

   88 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.15  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.15  WETTING FRONT SUCTION
                        XKSAT        0.58  HYDRAULIC CONDUCTIVITY
                        RTIMP       56.00  PERCENT IMPERVIOUS AREA

   89 UC          CLARK UNITGRAPH
                           TC        0.32  TIME OF CONCENTRATION
                            R        0.24  STORAGE COEFFICIENT
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 90 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                       0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                     100.0

                                                                          ***

                                                               UNIT HYDROGRAPH PARAMETERS
                                                         CLARK   TC=  0.32 HR,       R=  0.24 HR
                                                        SNYDER   TP=  0.19 HR,      CP=  0.49

                                                                    UNIT HYDROGRAPH
                                                               28 END-OF-PERIOD ORDINATES
                21.       81.      185.      250.      242.      220.      192.      160.      129.      104.
                85.       68.       55.       45.       36.       29.       24.       19.       16.       13.
                10.        8.        7.        5.        4.        4.        3.        2.

     ***             ***             ***             ***             ***

                                 HYDROGRAPH AT STATION     SB02
                               TRANSPOSITION AREA      0.0 SQ MI

    TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.45, TOTAL EXCESS =    2.30

   PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                                  (CFS)
 +     492.      0.95                    39.         10.          5.           5.
                                (INCHES)     2.292       2.292       2.292        2.292
                                 (AC-FT)       19.         19.         19.          19.

                                 CUMULATIVE AREA =    0.16 SQ MI


     ***             ***             ***             ***             ***

                                 HYDROGRAPH AT STATION     SB02
                               TRANSPOSITION AREA      0.5 SQ MI

    TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.45, TOTAL EXCESS =    2.29

   PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                                  (CFS)
 +     488.      0.95                    38.         10.          5.           5.
                                (INCHES)     2.276       2.276       2.276        2.276
                                 (AC-FT)       19.         19.         19.          19.

                                 CUMULATIVE AREA =    0.16 SQ MI


     ***             ***             ***             ***             ***

                                 HYDROGRAPH AT STATION     SB02
                               TRANSPOSITION AREA      2.8 SQ MI

    TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.49, TOTAL EXCESS =    2.19

   PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                                  (CFS)
 +     433.      1.00                    37.          9.          4.           4.
                                (INCHES)     2.183       2.183       2.183        2.183
                                 (AC-FT)       18.         18.         18.          18.

                                 CUMULATIVE AREA =    0.16 SQ MI


     ***             ***             ***             ***             ***

                                 HYDROGRAPH AT STATION     SB02
                               TRANSPOSITION AREA     16.0 SQ MI

    TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.54, TOTAL EXCESS =    2.00

   PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                                  (CFS)
 +     365.      1.00                    34.          8.          4.           4.
                                (INCHES)     1.994       1.994       1.994        1.994
                                 (AC-FT)       17.         17.         17.          17.

                                 CUMULATIVE AREA =    0.16 SQ MI


     ***             ***             ***             ***             ***

                                 HYDROGRAPH AT STATION     SB02
                               TRANSPOSITION AREA     90.0 SQ MI

    TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.56, TOTAL EXCESS =    1.67
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  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     278.      1.00                   28.           7.          3.           3.
                          (INCHES)     1.660       1.660       1.660        1.660
                           (AC-FT)       14.         14.         14.          14.

                           CUMULATIVE AREA =    0.16 SQ MI

      ***             ***             ***             ***             ***


                           INTERPOLATED HYDROGRAPH AT     SB02

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     489.      0.95                   38.          10.          5.           5.
                          (INCHES)     2.278       2.278       2.278        2.278
                           (AC-FT)       19.         19.         19.          19.

                           CUMULATIVE AREA =    0.16 SQ MI



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             *            *
   92 KK     *      CP-2  *      COMBINE
             *            *
             **************
                                   SR 101L FREEWAY AND 76TH ST (MILLER RD CHANNEL)

   94 HC          HYDROGRAPH COMBINATION
                          ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                          ***

      ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     CP-2
                          TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     740.      1.05                   75.          20.         10.          10.
                          (INCHES)     1.372       1.495       1.529        1.529
                           (AC-FT)       37.         41.         42.          42.

                           CUMULATIVE AREA =    0.51 SQ MI


      ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     CP-2
                          TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     732.      1.05                   75.          20.         10.          10.
                          (INCHES)     1.363       1.486       1.521        1.521
                           (AC-FT)       37.         40.         41.          41.

                           CUMULATIVE AREA =    0.51 SQ MI


      ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     CP-2
                          TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     654.      1.05                   72.          20.         10.          10.
                          (INCHES)     1.311       1.433       1.468        1.468
                           (AC-FT)       36.         39.         40.          40.

                           CUMULATIVE AREA =    0.51 SQ MI
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      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-2
                       TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     556.      1.10                    66.         18.          9.           9.
                       (INCHES)      1.207       1.332       1.367        1.367
                        (AC-FT)        33.         36.         37.          37.

                        CUMULATIVE AREA =    0.51 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-2
                       TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     422.      1.10                    56.         16.          8.           8.
                       (INCHES)      1.028       1.156       1.190        1.190
                        (AC-FT)        28.         31.         32.          32.

                        CUMULATIVE AREA =    0.51 SQ MI

      ***             ***             ***             ***             ***



                        INTERPOLATED HYDROGRAPH AT     CP-2

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     731.      1.05                    75.         20.         10.          10.
                       (INCHES)      1.362       1.485       1.520        1.520
                        (AC-FT)        37.         40.         41.          41.

                        CUMULATIVE AREA =    0.51 SQ MI



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             *            *
   95 KK     *      R2-3  *        ROUTE
             *            *
             **************
                                MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD

                HYDROGRAPH ROUTING DATA

   97 RK          KINEMATIC WAVE STREAM ROUTING
                            L       1260.  CHANNEL LENGTH
                            S      0.0015  SLOPE
                            N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                           CA        0.00  CONTRIBUTING AREA
                        SHAPE        TRAP  CHANNEL SHAPE
                           WD       92.00  BOTTOM WIDTH OR DIAMETER
                            Z        4.00  SIDE SLOPE
                       NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                         ***
                                       COMPUTED KINEMATIC PARAMETERS
                                             VARIABLE TIME STEP
                                           (DT SHOWN IS A MINIMUM)

                     ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                   PEAK                 CELERITY
                                                     (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

                    MAIN         0.13      1.58       1.61     420.00    726.00     65.69      1.53      4.85


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4152E+02  EXCESS=0.0000E+00 OUTFLOW=0.4152E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.1


                                          INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                    MAIN         0.13      1.58       3.00               725.57     66.00      1.53
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      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     R2-3
                        TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     726.      1.10                  75.         20.         10.          10.
                          (INCHES)     1.373       1.495       1.529        1.529
                           (AC-FT)       37.         41.         42.          42.

                           CUMULATIVE AREA =    0.51 SQ MI

                                         COMPUTED KINEMATIC PARAMETERS
                                               VARIABLE TIME STEP
                                            (DT SHOWN IS A MINIMUM)

                    ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                 CELERITY
                                                   (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                   MAIN         0.13      1.58       1.61     420.00    719.73     65.75      1.52      4.83

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4128E+02  EXCESS=0.0000E+00 OUTFLOW=0.4128E+02 BASIN STORAGE=0.4197E-01 PERCENT ERROR=
-0.1

                                            INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                   MAIN         0.13      1.58       3.00               719.25     66.00      1.52

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     R2-3
                        TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     719.      1.10                  75.         20.         10.          10.
                          (INCHES)     1.364       1.486       1.521        1.521
                           (AC-FT)       37.         40.         41.          41.

                           CUMULATIVE AREA =    0.51 SQ MI

                                         COMPUTED KINEMATIC PARAMETERS
                                               VARIABLE TIME STEP
                                            (DT SHOWN IS A MINIMUM)

                    ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                 CELERITY
                                                   (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                   MAIN         0.13      1.58       1.65     420.00    650.44     66.16      1.47      4.64

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3985E+02  EXCESS=0.0000E+00 OUTFLOW=0.3986E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.1

                                            INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                   MAIN         0.13      1.58       3.00               647.31     66.00      1.47

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     R2-3
                        TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     647.      1.10                  72.         20.         10.          10.
                          (INCHES)     1.313       1.435       1.469        1.469
                           (AC-FT)       36.         39.         40.          40.

                           CUMULATIVE AREA =    0.51 SQ MI

                                         COMPUTED KINEMATIC PARAMETERS
                                               VARIABLE TIME STEP
                                            (DT SHOWN IS A MINIMUM)

                    ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                 CELERITY
                                                   (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                   MAIN         0.13      1.58       1.61     420.00    553.64     68.36      1.37      4.37
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 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3711E+02  EXCESS=0.0000E+00 OUTFLOW=0.3710E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.1

                                                     INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                     MAIN          0.13      1.58       3.00             550.82     69.00      1.37

       ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION     R2-3
                     TRANSPOSITION AREA     16.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                          (CFS)
 +     551.      1.15                   66.         18.          9.           9.
                       (INCHES)      1.206       1.332       1.366        1.366
                        (AC-FT)        33.         36.         37.          37.

                        CUMULATIVE AREA =    0.51 SQ MI

                                                 COMPUTED KINEMATIC PARAMETERS
                                                       VARIABLE TIME STEP
                                                    (DT SHOWN IS A MINIMUM)

                      ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                    PEAK                CELERITY
                                                      (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                     MAIN          0.13      1.58       1.77    420.00    421.93     69.46      1.19      3.95

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3232E+02  EXCESS=0.0000E+00 OUTFLOW=0.3234E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.2

                                                     INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                     MAIN          0.13      1.58       3.00             418.11     69.00      1.19

       ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION     R2-3
                     TRANSPOSITION AREA     90.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                          (CFS)
 +     418.      1.15                   56.         16.          8.           8.
                       (INCHES)      1.029       1.157       1.192        1.192
                        (AC-FT)        28.         31.         32.          32.

                        CUMULATIVE AREA =    0.51 SQ MI

       ***             ***             ***             ***             ***


                        INTERPOLATED HYDROGRAPH AT     R2-3

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                          (CFS)
 +     719.      1.10                   75.         20.         10.          10.
                       (INCHES)      1.363       1.486       1.520        1.520
                        (AC-FT)        37.         40.         41.          41.

                        CUMULATIVE AREA =    0.51 SQ MI



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              *            *
   98 KK      *      SB03  *        BASIN
              *            *
              **************

                 SUBBASIN RUNOFF DATA

   99 BA           SUBBASIN CHARACTERISTICS
                         TAREA        0.05  SUBBASIN AREA
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  100 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.15  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.50  WETTING FRONT SUCTION
                        XKSAT        0.47  HYDRAULIC CONDUCTIVITY
                        RTIMP       55.00  PERCENT IMPERVIOUS AREA

  101 UC          CLARK UNITGRAPH
                           TC        0.36  TIME OF CONCENTRATION
                            R        0.29  STORAGE COEFFICIENT

  102 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                                         ***

                                                             UNIT HYDROGRAPH PARAMETERS
                                                       CLARK   TC=  0.36 HR,       R=  0.29 HR
                                                      SNYDER   TP=  0.21 HR,      CP=  0.43

                                                                  UNIT HYDROGRAPH
                                                             34 END-OF-PERIOD ORDINATES
                    5.        17.        43.        62.        64.        61.        56.        49.        42.        35.
                   30.        25.        21.        18.        15.        12.        10.         9.         7.         6.
                    5.         4.         4.         3.         3.         2.         2.         2.         1.         1.
                    1.         1.         1.         1.

     ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB03
                        TRANSPOSITION AREA      0.0 SQ MI

    TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.44, TOTAL EXCESS =    2.31

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     130.      1.00                   12.          3.          1.           1.
                        (INCHES)     2.302       2.302       2.302        2.302
                         (AC-FT)        6.          6.          6.           6.

                         CUMULATIVE AREA =    0.05 SQ MI


     ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB03
                        TRANSPOSITION AREA      0.5 SQ MI

    TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.44, TOTAL EXCESS =    2.30

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     129.      1.00                   12.          3.          1.           1.
                        (INCHES)     2.286       2.286       2.286        2.286
                         (AC-FT)        6.          6.          6.           6.

                         CUMULATIVE AREA =    0.05 SQ MI


     ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB03
                        TRANSPOSITION AREA      2.8 SQ MI

    TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.48, TOTAL EXCESS =    2.21

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     117.      1.00                   11.          3.          1.           1.
                        (INCHES)     2.198       2.198       2.198        2.198
                         (AC-FT)        6.          6.          6.           6.

                         CUMULATIVE AREA =    0.05 SQ MI


     ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB03
                        TRANSPOSITION AREA     16.0 SQ MI

    TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.52, TOTAL EXCESS =    2.02

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
```

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+      99.      1.05                  10.          3.          1.           1.
                     (INCHES)     2.014       2.014       2.014        2.014
                      (AC-FT)        5.          5.          5.           5.

                      CUMULATIVE AREA =    0.05 SQ MI


      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION     SB03
                     TRANSPOSITION AREA     90.0 SQ MI

    TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.54, TOTAL EXCESS =    1.69

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+      77.      1.05                   9.          2.          1.           1.
                     (INCHES)     1.679       1.679       1.679        1.679
                      (AC-FT)        4.          4.          4.           4.

                      CUMULATIVE AREA =    0.05 SQ MI

      ***             ***             ***             ***             ***



                      INTERPOLATED HYDROGRAPH AT     SB03

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     129.      1.00                  12.          3.          1.           1.
                     (INCHES)     2.290       2.290       2.290        2.290
                      (AC-FT)        6.          6.          6.           6.

                      CUMULATIVE AREA =    0.05 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             *            *
  104 KK     *      CP-3  *      COMBINE
             *            *
             **************
                                 MAYO BLVD AND 76TH ST (MILLER RD CHANNEL)

  106 HC          HYDROGRAPH COMBINATION
                        ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                          ***

      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION     CP-3
                     TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     840.      1.10                  87.         23.         11.          11.
                     (INCHES)     1.449       1.564       1.596        1.596
                      (AC-FT)       43.         46.         47.          47.

                      CUMULATIVE AREA =    0.56 SQ MI


      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION     CP-3
                     TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     833.      1.10                  86.         23.         11.          11.
                     (INCHES)     1.439       1.555       1.587        1.587
                      (AC-FT)       43.         46.         47.          47.

                      CUMULATIVE AREA =    0.56 SQ MI


      ***             ***             ***             ***             ***

                       HYDROGRAPH AT STATION     CP-3
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                    TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     755.      1.10                  83.         22.         11.          11.
                        (INCHES)     1.385       1.500       1.532        1.532
                         (AC-FT)      41.         45.         46.          46.

                         CUMULATIVE AREA =    0.56 SQ MI


      ***            ***            ***            ***            ***

                      HYDROGRAPH AT STATION     CP-3
                    TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     638.      1.15                  76.         21.         10.          10.
                        (INCHES)     1.271       1.390       1.422        1.422
                         (AC-FT)      38.         41.         42.          42.

                         CUMULATIVE AREA =    0.56 SQ MI


      ***            ***            ***            ***            ***

                      HYDROGRAPH AT STATION     CP-3
                    TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     487.      1.15                  65.         18.          9.           9.
                        (INCHES)     1.080       1.202       1.234        1.234
                         (AC-FT)      32.         36.         37.          37.

                         CUMULATIVE AREA =    0.56 SQ MI

      ***            ***            ***            ***            ***



                         INTERPOLATED HYDROGRAPH AT     CP-3

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     828.      1.10                  86.         23.         11.          11.
                        (INCHES)     1.435       1.552       1.583        1.583
                         (AC-FT)      43.         46.         47.          47.

                         CUMULATIVE AREA =    0.56 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             *            *
  107 KK     *      R3-5  *        ROUTE
             *            *
             **************
                                 MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD

                HYDROGRAPH ROUTING DATA

  109 RK          KINEMATIC WAVE STREAM ROUTING
                             L       2396.  CHANNEL LENGTH
                             S      0.0015  SLOPE
                             N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                            CA        0.00  CONTRIBUTING AREA
                         SHAPE        TRAP  CHANNEL SHAPE
                            WD       98.00  BOTTOM WIDTH OR DIAMETER
                             Z        4.00  SIDE SLOPE
                        NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                          ***
                                        COMPUTED KINEMATIC PARAMETERS
                                              VARIABLE TIME STEP
                                            (DT SHOWN IS A MINIMUM)

                       ELEMENT    ALPHA       M         DT         DX        PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                       PEAK                CELERITY
                                                       (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)
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                 MAIN          0.12      1.58       2.78    798.67    817.36     73.14      1.60      5.01

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4741E+02  EXCESS=0.0000E+00 OUTFLOW=0.4763E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.6

                                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                 MAIN          0.12      1.58       3.00              816.94     72.00      1.60

      ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     R3-5
                          TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     817.      1.20                  87.         24.         12.          12.
                          (INCHES)     1.450       1.571       1.602        1.602
                           (AC-FT)     43.         47.         48.          48.

                           CUMULATIVE AREA =    0.56 SQ MI

                                            COMPUTED KINEMATIC PARAMETERS
                                                  VARIABLE TIME STEP
                                                (DT SHOWN IS A MINIMUM)

                  ELEMENT    ALPHA       M          DT         DX        PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                   PEAK                CELERITY
                                                   (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                 MAIN          0.12      1.58       2.79    798.67    810.60     70.45      1.59      4.99

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4713E+02  EXCESS=0.0000E+00 OUTFLOW=0.4736E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.7

                                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                 MAIN          0.12      1.58       3.00              810.54     72.00      1.59

      ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     R3-5
                          TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     811.      1.20                  86.         23.         11.          11.
                          (INCHES)     1.440       1.562       1.593        1.593
                           (AC-FT)     43.         46.         47.          47.

                           CUMULATIVE AREA =    0.56 SQ MI

                                            COMPUTED KINEMATIC PARAMETERS
                                                  VARIABLE TIME STEP
                                                (DT SHOWN IS A MINIMUM)

                  ELEMENT    ALPHA       M          DT         DX        PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                   PEAK                CELERITY
                                                   (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                 MAIN          0.12      1.58       2.90    798.67    742.52     73.90      1.54      4.81

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4551E+02  EXCESS=0.0000E+00 OUTFLOW=0.4571E+02 BASIN STORAGE=0.8226E-01 PERCENT ERROR=
-0.6

                                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                 MAIN          0.12      1.58       3.00              740.16     72.00      1.54

      ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     R3-5
                          TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     740.      1.20                  83.         23.         11.          11.
                          (INCHES)     1.385       1.506       1.537        1.537
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                    (AC-FT)       41.         45.         46.          46.

                    CUMULATIVE AREA =    0.56 SQ MI

                                   COMPUTED KINEMATIC PARAMETERS
                                         VARIABLE TIME STEP
                                       (DT SHOWN IS A MINIMUM)

          ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                        PEAK               CELERITY
                                          (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

         MAIN          0.12      1.58      3.00    798.67    634.55     73.70      1.43      4.52

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4224E+02  EXCESS=0.0000E+00 OUTFLOW=0.4240E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.6

                                         INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

         MAIN          0.12      1.58      3.00              626.49     75.00      1.43

      ***          ***          ***          ***          ***

                     HYDROGRAPH AT STATION     R3-5
                   TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
+     626.      1.25                76.         21.         10.          10.
                   (INCHES)       1.271       1.396       1.427        1.427
                    (AC-FT)         38.         41.         42.          42.

                    CUMULATIVE AREA =    0.56 SQ MI

                                   COMPUTED KINEMATIC PARAMETERS
                                         VARIABLE TIME STEP
                                       (DT SHOWN IS A MINIMUM)

          ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                        PEAK               CELERITY
                                          (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

         MAIN          0.12      1.58      3.00    798.67    478.66     74.64      1.24      4.10

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3665E+02  EXCESS=0.0000E+00 OUTFLOW=0.3688E+02 BASIN STORAGE=0.8233E-01 PERCENT ERROR=
-0.8

                                         INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

         MAIN          0.12      1.58      3.00              478.05     75.00      1.24

      ***          ***          ***          ***          ***

                     HYDROGRAPH AT STATION     R3-5
                   TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
+     478.      1.25                65.         18.          9.           9.
                   (INCHES)       1.082       1.210       1.242        1.242
                    (AC-FT)         32.         36.         37.          37.

                    CUMULATIVE AREA =    0.56 SQ MI

      ***          ***          ***          ***          ***

                    INTERPOLATED HYDROGRAPH AT     R3-5

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
+     806.      1.20                86.         23.         11.          11.
                   (INCHES)       1.437       1.558       1.589        1.589
                    (AC-FT)         43.         46.         47.          47.

                    CUMULATIVE AREA =    0.56 SQ MI

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              *            *
  110 KK      *      SB04  *        BASIN
              *            *
              **************

                 SUBBASIN RUNOFF DATA

  111 BA           SUBBASIN CHARACTERISTICS
                         TAREA        0.14  SUBBASIN AREA

  112 LG           GREEN AND AMPT LOSS RATE
                         STRTL        0.14  STARTING LOSS
                           DTH        0.25  MOISTURE DEFICIT
                          PSIF        4.60  WETTING FRONT SUCTION
                         XKSAT        0.44  HYDRAULIC CONDUCTIVITY
                         RTIMP       61.00  PERCENT IMPERVIOUS AREA

  113 UC           CLARK UNITGRAPH
                            TC        0.30  TIME OF CONCENTRATION
                             R        0.20  STORAGE COEFFICIENT

  114 UA           ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                         0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                       100.0

                                                                         ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.30 HR,       R=  0.20 HR
                                                       SNYDER   TP=  0.18 HR,      CP=  0.54

                                                                   UNIT HYDROGRAPH
                                                              24 END-OF-PERIOD ORDINATES
                    26.      107.      218.      267.      245.      215.      177.      137.      106.       82.
                    63.       49.       38.       29.       23.       18.       14.       11.        8.        6.
                     5.        4.        3.        2.

      ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB04
                         TRANSPOSITION AREA      0.0 SQ MI

    TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.38, TOTAL EXCESS =    2.38

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     528.      0.95                     37.          9.          4.           4.
                         (INCHES)     2.369       2.369       2.369        2.369
                          (AC-FT)       18.         18.         18.          18.

                          CUMULATIVE AREA =    0.14 SQ MI


      ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB04
                         TRANSPOSITION AREA      0.5 SQ MI

    TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.37, TOTAL EXCESS =    2.36

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     524.      0.95                     36.          9.          4.           4.
                         (INCHES)     2.353       2.353       2.353        2.353
                          (AC-FT)       18.         18.         18.          18.

                          CUMULATIVE AREA =    0.14 SQ MI


      ***             ***             ***             ***             ***

                           HYDROGRAPH AT STATION     SB04
                         TRANSPOSITION AREA      2.8 SQ MI

    TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.40, TOTAL EXCESS =    2.28

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     452.      0.95                     35.          9.          4.           4.
                         (INCHES)     2.272       2.272       2.272        2.272
                          (AC-FT)       17.         17.         17.          17.

                          CUMULATIVE AREA =    0.14 SQ MI
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     ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     SB04
                    TRANSPOSITION AREA     16.0 SQ MI

    TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.44, TOTAL EXCESS =    2.10

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                         (CFS)
+     382.      1.00                  32.          8.          4.           4.
                       (INCHES)     2.095       2.095       2.095        2.095
                        (AC-FT)       16.         16.         16.          16.

                        CUMULATIVE AREA =    0.14 SQ MI


     ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     SB04
                    TRANSPOSITION AREA     90.0 SQ MI

    TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.46, TOTAL EXCESS =    1.77

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                         (CFS)
+     293.      1.00                  27.          7.          3.           3.
                       (INCHES)     1.763       1.763       1.763        1.763
                        (AC-FT)       14.         14.         14.          14.

                        CUMULATIVE AREA =    0.14 SQ MI

     ***             ***             ***             ***             ***


                        INTERPOLATED HYDROGRAPH AT     SB04

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                         (CFS)
+     525.      0.95                  36.          9.          4.           4.
                       (INCHES)     2.356       2.356       2.356        2.356
                        (AC-FT)       18.         18.         18.          18.

                        CUMULATIVE AREA =    0.14 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             **************
             *            *
  116 KK     *     R4-5   *        ROUTE
             *            *
             **************
                                PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST

               HYDROGRAPH ROUTING DATA

  118 RK          KINEMATIC WAVE STREAM ROUTING
                           L       2005.  CHANNEL LENGTH
                           S      0.0013  SLOPE
                           N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                          CA        0.00  CONTRIBUTING AREA
                       SHAPE        TRAP  CHANNEL SHAPE
                          WD       39.00  BOTTOM WIDTH OR DIAMETER
                           Z        4.00  SIDE SLOPE
                      NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                         ***
                                      COMPUTED KINEMATIC PARAMETERS
                                            VARIABLE TIME STEP
                                          (DT SHOWN IS A MINIMUM)

                     ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO      VOLUME    MAXIMUM
                                                                                    PEAK                CELERITY
                                                    (MIN)      (FT)     (CFS)     (MIN)        (IN)      (FPS)

                    MAIN         0.23      1.51       2.41     668.33    520.58     62.34       2.39       4.81


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1819E+02  EXCESS=0.0000E+00 OUTFLOW=0.1838E+02 BASIN STORAGE=0.3179E-04 PERCENT ERROR=
-1.0
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                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                 MAIN          0.23      1.51       3.00              513.38     63.00      2.40

      ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION     R4-5
                  TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     513.      1.05               37.          9.          4.           4.
                      (INCHES)    2.401       2.402       2.402        2.402
                       (AC-FT)      18.         18.         18.          18.

                       CUMULATIVE AREA =    0.14 SQ MI

                                COMPUTED KINEMATIC PARAMETERS
                                      VARIABLE TIME STEP
                                    (DT SHOWN IS A MINIMUM)

                  ELEMENT    ALPHA       M          DT         DX        PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                 CELERITY
                                                   (MIN)      (FT)      (CFS)    (MIN)        (IN)      (FPS)

                 MAIN          0.23      1.51       2.42     668.33    516.56     62.40      2.38      4.79

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1807E+02  EXCESS=0.0000E+00 OUTFLOW=0.1825E+02 BASIN STORAGE=0.4744E-04 PERCENT ERROR=
-1.0


                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                 MAIN          0.23      1.51       3.00              510.06     63.00      2.39

      ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION     R4-5
                  TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     510.      1.05               37.          9.          4.           4.
                      (INCHES)    2.386       2.386       2.386        2.386
                       (AC-FT)      18.         18.         18.          18.

                       CUMULATIVE AREA =    0.14 SQ MI

                                COMPUTED KINEMATIC PARAMETERS
                                      VARIABLE TIME STEP
                                    (DT SHOWN IS A MINIMUM)

                  ELEMENT    ALPHA       M          DT         DX        PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                 CELERITY
                                                   (MIN)      (FT)      (CFS)    (MIN)        (IN)      (FPS)

                 MAIN          0.23      1.51       2.55     668.33    451.64     62.77      2.29      4.56

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1745E+02  EXCESS=0.0000E+00 OUTFLOW=0.1762E+02 BASIN STORAGE=0.4583E-04 PERCENT ERROR=
-1.0


                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                 MAIN          0.23      1.51       3.00              451.16     63.00      2.30

      ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION     R4-5
                  TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     451.      1.05               36.          9.          4.           4.
                      (INCHES)    2.301       2.302       2.302        2.302
                       (AC-FT)      18.         18.         18.          18.

                       CUMULATIVE AREA =    0.14 SQ MI

                                COMPUTED KINEMATIC PARAMETERS
                                      VARIABLE TIME STEP
                                    (DT SHOWN IS A MINIMUM)
```

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.23	1.51	2.73	668.33	379.50	64.90	2.12	4.31

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1609E+02 EXCESS=0.0000E+00 OUTFLOW=0.1629E+02 BASIN STORAGE=0.4132E-04 PERCENT ERROR= -1.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		374.76	63.00	2.12

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 375.	1.05		33.	8.	4.	4.
		(INCHES)	2.115	2.116	2.116	2.116
		(AC-FT)	16.	16.	16.	16.

CUMULATIVE AREA = 0.14 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.23	1.51	2.87	668.33	291.80	65.24	1.78	3.94

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1354E+02 EXCESS=0.0000E+00 OUTFLOW=0.1370E+02 BASIN STORAGE=0.4337E-04 PERCENT ERROR= -1.2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.23	1.51	3.00		288.81	66.00	1.78

*** *** *** *** ***

HYDROGRAPH AT STATION R4-5
TRANSPOSITION AREA 90.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 289.	1.10		28.	7.	3.	3.
		(INCHES)	1.782	1.782	1.782	1.782
		(AC-FT)	14.	14.	14.	14.

CUMULATIVE AREA = 0.14 SQ MI

*** *** *** *** ***

INTERPOLATED HYDROGRAPH AT R4-5

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 511.	1.05		37.	9.	4.	4.
		(INCHES)	2.388	2.388	2.388	2.388
		(AC-FT)	18.	18.	18.	18.

CUMULATIVE AREA = 0.14 SQ MI

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           *            *
 119 KK    *     SB05   *       BASIN
           *            *
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              SUBBASIN RUNOFF DATA

  120 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.13  SUBBASIN AREA

  121 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.22  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.50  WETTING FRONT SUCTION
                        XKSAT        0.44  HYDRAULIC CONDUCTIVITY
                        RTIMP       48.00  PERCENT IMPERVIOUS AREA

  122 UC          CLARK UNITGRAPH
                           TC        0.33  TIME OF CONCENTRATION
                            R        0.22  STORAGE COEFFICIENT

  123 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                                           ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.33 HR,       R=  0.22 HR
                                                       SNYDER   TP=  0.19 HR,      CP=  0.50

                                                                    UNIT HYDROGRAPH
                                                              27 END-OF-PERIOD ORDINATES
                 18.       67.      154.      208.      200.      181.      156.      129.      103.       82.
                 66.       53.       42.       34.       27.       21.       17.       14.       11.        9.
                  7.        6.        4.        4.        3.        2.        2.

        ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB05
                        TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.52, TOTAL EXCESS =    2.23

  PEAK FLOW     TIME                       MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     402.      0.95                   30.          8.          4.           4.
                        (INCHES)     2.223       2.223       2.223        2.223
                         (AC-FT)       15.         15.         15.          15.

                         CUMULATIVE AREA =    0.13 SQ MI


        ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB05
                        TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.52, TOTAL EXCESS =    2.22

  PEAK FLOW     TIME                       MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     399.      0.95                   30.          7.          4.           4.
                        (INCHES)     2.208       2.208       2.208        2.208
                         (AC-FT)       15.         15.         15.          15.

                         CUMULATIVE AREA =    0.13 SQ MI


        ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB05
                        TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.56, TOTAL EXCESS =    2.13

  PEAK FLOW     TIME                       MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     352.      1.00                   29.          7.          3.           3.
                        (INCHES)     2.118       2.118       2.118        2.118
                         (AC-FT)       14.         14.         14.          14.

                         CUMULATIVE AREA =    0.13 SQ MI


        ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB05
                        TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.60, TOTAL EXCESS =    1.94
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  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     296.      1.00                    26.          7.          3.           3.
                            (INCHES)   1.930       1.930       1.930        1.930
                             (AC-FT)    13.         13.         13.          13.

                             CUMULATIVE AREA =    0.13 SQ MI


      ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     SB05
                            TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.63, TOTAL EXCESS =    1.60

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     225.      1.00                    22.          5.          3.           3.
                            (INCHES)   1.593       1.593       1.593        1.593
                             (AC-FT)    11.         11.         11.          11.

                             CUMULATIVE AREA =    0.13 SQ MI

      ***             ***             ***             ***             ***



                             INTERPOLATED HYDROGRAPH AT     SB05

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     399.      0.95                    30.          7.          4.           4.
                            (INCHES)   2.210       2.210       2.210        2.210
                             (AC-FT)    15.         15.         15.          15.

                             CUMULATIVE AREA =    0.13 SQ MI



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             *            *
  125 KK     *      CP-5  *      COMBINE
             *            *
             **************
                                     PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)

  127 HC          HYDROGRAPH COMBINATION
                        ICOMP           3  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                         ***

      ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     CP-5
                            TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1452.      1.15                   153.         40.         20.          20.
                            (INCHES)   1.715       1.814       1.836        1.836
                             (AC-FT)    76.         80.         81.          81.

                             CUMULATIVE AREA =    0.83 SQ MI


      ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION     CP-5
                            TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1440.      1.15                   152.         40.         19.          19.
                            (INCHES)   1.703       1.803       1.824        1.824
                             (AC-FT)    75.         80.         80.          80.

                             CUMULATIVE AREA =    0.83 SQ MI
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      ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     CP-5
                          TRANSPOSITION AREA      2.8 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                             (CFS)
 +    1335.      1.15                  146.         39.         19.          19.
                          (INCHES)    1.638       1.737       1.759        1.759
                           (AC-FT)      72.         77.         78.          78.

                           CUMULATIVE AREA =    0.83 SQ MI


      ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     CP-5
                          TRANSPOSITION AREA     16.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                             (CFS)
 +    1121.      1.15                  133.         36.         17.          17.
                          (INCHES)    1.499       1.602       1.623        1.623
                           (AC-FT)      66.         71.         72.          72.

                           CUMULATIVE AREA =    0.83 SQ MI


      ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     CP-5
                          TRANSPOSITION AREA     90.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                             (CFS)
 +     844.      1.20                  112.         30.         15.          15.
                          (INCHES)    1.262       1.368       1.389        1.389
                           (AC-FT)      56.         60.         61.          61.

                           CUMULATIVE AREA =    0.83 SQ MI

      ***             ***             ***             ***             ***


                           INTERPOLATED HYDROGRAPH AT     CP-5

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                             (CFS)
 +    1410.      1.15                  150.         40.         19.          19.
                          (INCHES)    1.684       1.784       1.805        1.805
                           (AC-FT)      74.         79.         80.          80.

                           CUMULATIVE AREA =    0.83 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             *            *
   128 KK    *      R5-6  *        ROUTE
             *            *
             **************
                                   PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD

                HYDROGRAPH ROUTING DATA

   130 RK         KINEMATIC WAVE STREAM ROUTING
                           L        1550.  CHANNEL LENGTH
                           S       0.0015  SLOPE
                           N        0.030  CHANNEL ROUGHNESS COEFFICIENT
                          CA         0.00  CONTRIBUTING AREA
                       SHAPE         TRAP  CHANNEL SHAPE
                          WD       188.00  BOTTOM WIDTH OR DIAMETER
                           Z         4.00  SIDE SLOPE
                      NDXMIN            2  MINIMUM NUMBER OF DX INTERVALS

                                                                          ***
                                          COMPUTED KINEMATIC PARAMETERS
                                                VARIABLE TIME STEP
                                              (DT SHOWN IS A MINIMUM)
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             ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                            PEAK                CELERITY
                                            (MIN)       (FT)     (CFS)     (MIN)       (IN)      (FPS)

            MAIN          0.07      1.62      1.71     516.67   1430.15     73.26      1.84      5.13

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8096E+02  EXCESS=0.0000E+00 OUTFLOW=0.8101E+02 BASIN STORAGE=0.7278E-01 PERCENT ERROR=
-0.1

                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

            MAIN          0.07      1.62      3.00               1428.13     72.00      1.84

      ***             ***             ***             ***             ***

                  HYDROGRAPH AT STATION     R5-6
                TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1428.      1.20                 153.         40.         20.         20.
                        (INCHES)    1.715       1.815       1.836       1.836
                         (AC-FT)      76.         80.         81.         81.

                         CUMULATIVE AREA =    0.83 SQ MI

                                    COMPUTED KINEMATIC PARAMETERS
                                          VARIABLE TIME STEP
                                        (DT SHOWN IS A MINIMUM)

             ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                            PEAK                CELERITY
                                            (MIN)       (FT)     (CFS)     (MIN)       (IN)      (FPS)

            MAIN          0.07      1.62      1.72     516.67   1417.75     71.63      1.82      5.12

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8047E+02  EXCESS=0.0000E+00 OUTFLOW=0.8049E+02 BASIN STORAGE=0.7282E-01 PERCENT ERROR=
-0.1

                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

            MAIN          0.07      1.62      3.00               1417.68     72.00      1.82

      ***             ***             ***             ***             ***

                  HYDROGRAPH AT STATION     R5-6
                TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1418.      1.20                 151.         40.         19.         19.
                        (INCHES)    1.703       1.803       1.824       1.824
                         (AC-FT)      75.         80.         80.         80.

                         CUMULATIVE AREA =    0.83 SQ MI

                                    COMPUTED KINEMATIC PARAMETERS
                                          VARIABLE TIME STEP
                                        (DT SHOWN IS A MINIMUM)

             ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                            PEAK                CELERITY
                                            (MIN)       (FT)     (CFS)     (MIN)       (IN)      (FPS)

            MAIN          0.07      1.62      1.83     516.67   1320.58     72.30      1.76      4.97

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7757E+02  EXCESS=0.0000E+00 OUTFLOW=0.7762E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR=
-0.2

                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

            MAIN          0.07      1.62      3.00               1310.29     72.00      1.76

      ***             ***             ***             ***             ***

                  HYDROGRAPH AT STATION     R5-6
                TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
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+   (CFS)       (HR)
                                (CFS)
+    1310.      1.20                      146.         39.         19.          19.
                             (INCHES)    1.639       1.739       1.760        1.760
                              (AC-FT)      72.         77.         78.          78.

                              CUMULATIVE AREA =    0.83 SQ MI

                                             COMPUTED KINEMATIC PARAMETERS
                                                   VARIABLE TIME STEP
                                                 (DT SHOWN IS A MINIMUM)

                          ELEMENT    ALPHA       M          DT         DX        PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                            PEAK                 CELERITY
                                                          (MIN)       (FT)      (CFS)     (MIN)       (IN)      (FPS)

                         MAIN          0.07      1.62       1.92     516.67   1120.07     73.97      1.62      4.65


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7160E+02  EXCESS=0.0000E+00 OUTFLOW=0.7164E+02 BASIN STORAGE=0.7274E-01 PERCENT ERROR=
-0.2


                                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                         MAIN          0.07      1.62       3.00              1117.54     75.00      1.62

       ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     R5-6
                             TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+    1118.      1.25                      133.         36.         17.          17.
                             (INCHES)    1.500       1.603       1.624        1.624
                              (AC-FT)      66.         71.         72.          72.

                              CUMULATIVE AREA =    0.83 SQ MI

                                             COMPUTED KINEMATIC PARAMETERS
                                                   VARIABLE TIME STEP
                                                 (DT SHOWN IS A MINIMUM)

                          ELEMENT    ALPHA       M          DT         DX        PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                            PEAK                 CELERITY
                                                          (MIN)       (FT)      (CFS)     (MIN)       (IN)      (FPS)

                         MAIN          0.07      1.62       2.16     516.67    837.38     77.35      1.39      4.17


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6127E+02  EXCESS=0.0000E+00 OUTFLOW=0.6143E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR=
-0.4


                                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                         MAIN          0.07      1.62       3.00               834.70     75.00      1.39

       ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     R5-6
                             TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     835.      1.25                      112.         30.         15.          15.
                             (INCHES)    1.264       1.371       1.392        1.392
                              (AC-FT)      56.         60.         61.          61.

                              CUMULATIVE AREA =    0.83 SQ MI

       ***             ***             ***             ***             ***


                              INTERPOLATED HYDROGRAPH AT     R5-6

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+    1386.      1.20                      150.         40.         19.          19.
                             (INCHES)    1.684       1.785       1.806        1.806
                              (AC-FT)      74.         79.         80.          80.

                              CUMULATIVE AREA =    0.83 SQ MI
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            *            *
  131 KK    *     SB06   *        BASIN
            *            *
            **************

               SUBBASIN RUNOFF DATA

  132 BA         SUBBASIN CHARACTERISTICS
                       TAREA        0.14  SUBBASIN AREA

  133 LG         GREEN AND AMPT LOSS RATE
                       STRTL        0.16  STARTING LOSS
                         DTH        0.25  MOISTURE DEFICIT
                        PSIF        4.55  WETTING FRONT SUCTION
                       XKSAT        0.45  HYDRAULIC CONDUCTIVITY
                       RTIMP       53.00  PERCENT IMPERVIOUS AREA

  134 UC         CLARK UNITGRAPH
                          TC        0.32  TIME OF CONCENTRATION
                           R        0.24  STORAGE COEFFICIENT

  135 UA         ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                       0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                     100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.32 HR,       R=  0.24 HR
                                                       SNYDER   TP=  0.19 HR,      CP=  0.48

                                                                   UNIT HYDROGRAPH
                                                              29 END-OF-PERIOD ORDINATES
                 18.       72.      161.      214.      206.      189.      164.      136.      111.       90.
                 73.       60.       49.       39.       32.       26.       21.       17.       14.       11.
                  9.        8.        6.        5.        4.        3.        3.        2.        2.

       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB06
                       TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.30

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     422.      0.95                  33.          8.          4.           4.
                       (INCHES)     2.285       2.285       2.285        2.285
                        (AC-FT)       17.         17.         17.          17.

                        CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB06
                       TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.28

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     419.      0.95                  33.          8.          4.           4.
                       (INCHES)     2.269       2.269       2.269        2.269
                        (AC-FT)       16.         16.         16.          16.

                        CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB06
                       TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.50, TOTAL EXCESS =    2.19

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     373.      1.00                  32.          8.          4.           4.
                       (INCHES)     2.181       2.181       2.181        2.181
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                     (AC-FT)       16.          16.          16.           16.

                     CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     SB06
                    TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.53, TOTAL EXCESS =    2.01

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+     316.      1.00                 29.          7.          4.           4.
                    (INCHES)     1.997       1.997       1.997        1.997
                     (AC-FT)       14.          14.          14.           14.

                     CUMULATIVE AREA =    0.14 SQ MI


       ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     SB06
                    TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.56, TOTAL EXCESS =    1.67

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+     241.      1.00                 24.          6.          3.           3.
                    (INCHES)     1.661       1.661       1.661        1.661
                     (AC-FT)       12.          12.          12.           12.

                     CUMULATIVE AREA =    0.14 SQ MI

       ***             ***             ***             ***             ***



                     INTERPOLATED HYDROGRAPH AT     SB06

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+     420.      0.95                 33.          8.          4.           4.
                    (INCHES)     2.272       2.272       2.272        2.272
                     (AC-FT)       16.          16.          16.           16.

                     CUMULATIVE AREA =    0.14 SQ MI



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             *            *
  137 KK     *      CP-6  *      COMBINE
             *            *
             **************
                                 PRINCESS BLVD AND SCOTTSDALE RD (PRINCESS BLVD CHANNEL)

  139 HC          HYDROGRAPH COMBINATION
                    ICOMP            2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                         ***

       ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     CP-6
                    TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+    1662.      1.20                185.         49.         24.          24.
                    (INCHES)     1.782       1.881       1.900        1.900
                     (AC-FT)       92.          97.          98.           98.

                     CUMULATIVE AREA =    0.96 SQ MI


       ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION     CP-6
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                    TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+    1650.      1.20               183.         48.         23.          23.
                    (INCHES)      1.769       1.869       1.887        1.887
                     (AC-FT)        91.         96.         97.          97.

                     CUMULATIVE AREA =    0.96 SQ MI


       ***            ***            ***            ***            ***

                      HYDROGRAPH AT STATION     CP-6
                    TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+    1542.      1.20               176.         47.         23.          23.
                    (INCHES)      1.702       1.801       1.820        1.820
                     (AC-FT)        87.         93.         93.          93.

                     CUMULATIVE AREA =    0.96 SQ MI


       ***            ***            ***            ***            ***

                      HYDROGRAPH AT STATION     CP-6
                    TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+    1308.      1.20               161.         43.         21.          21.
                    (INCHES)      1.555       1.659       1.677        1.677
                     (AC-FT)        80.         85.         86.          86.

                     CUMULATIVE AREA =    0.96 SQ MI


       ***            ***            ***            ***            ***

                      HYDROGRAPH AT STATION     CP-6
                    TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+     982.      1.25               135.         37.         18.          18.
                    (INCHES)      1.305       1.412       1.430        1.430
                     (AC-FT)        67.         73.         73.          73.

                     CUMULATIVE AREA =    0.96 SQ MI

       ***            ***            ***            ***            ***


                     INTERPOLATED HYDROGRAPH AT     CP-6

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                       (CFS)
+    1609.      1.20               181.         48.         23.          23.
                    (INCHES)      1.743       1.843       1.862        1.862
                     (AC-FT)        90.         95.         96.          96.

                     CUMULATIVE AREA =    0.96 SQ MI



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            *            *
  140 KK    *     SB07   *        BASIN
            *            *
            **************

               SUBBASIN RUNOFF DATA

  141 BA         SUBBASIN CHARACTERISTICS
                    TAREA        0.07  SUBBASIN AREA
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  142 LG         GREEN AND AMPT LOSS RATE
                       STRTL        0.21  STARTING LOSS
                         DTH        0.25  MOISTURE DEFICIT
                        PSIF        4.00  WETTING FRONT SUCTION
                       XKSAT        0.58  HYDRAULIC CONDUCTIVITY
                       RTIMP       49.00  PERCENT IMPERVIOUS AREA

  143 UC         CLARK UNITGRAPH
                          TC        0.20  TIME OF CONCENTRATION
                           R        0.11  STORAGE COEFFICIENT

  144 UA         ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                       0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                     100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.20 HR,       R=  0.11 HR
                                                       SNYDER   TP=  0.13 HR,      CP=  0.62

                                                                   UNIT HYDROGRAPH
                                                              14 END-OF-PERIOD ORDINATES
                 39.      156.      219.      180.      130.       84.       53.       34.       22.       14.
                  9.        6.        4.        2.

       ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     SB07
                          TRANSPOSITION AREA      0.0 SQ MI

    TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.54, TOTAL EXCESS =    2.22

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     386.      0.90                   18.          4.          2.           2.
                          (INCHES)    2.209       2.209       2.209        2.209
                           (AC-FT)       9.          9.          9.           9.

                           CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     SB07
                          TRANSPOSITION AREA      0.5 SQ MI

    TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.54, TOTAL EXCESS =    2.20

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     384.      0.90                   17.          4.          2.           2.
                          (INCHES)    2.193       2.193       2.193        2.193
                           (AC-FT)       9.          9.          9.           9.

                           CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     SB07
                          TRANSPOSITION AREA      2.8 SQ MI

    TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.58, TOTAL EXCESS =    2.10

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     308.      0.90                   17.          4.          2.           2.
                          (INCHES)    2.095       2.095       2.095        2.095
                           (AC-FT)       8.          8.          8.           8.

                           CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                            HYDROGRAPH AT STATION     SB07
                          TRANSPOSITION AREA     16.0 SQ MI

    TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.63, TOTAL EXCESS =    1.91

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     249.      0.90                   15.          4.          2.           2.
                          (INCHES)    1.901       1.901       1.901        1.901
                           (AC-FT)       8.          8.          8.           8.
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                     CUMULATIVE AREA =    0.07 SQ MI


       ***              ***              ***              ***              ***

                      HYDROGRAPH AT STATION     SB07
                     TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.66, TOTAL EXCESS =    1.57

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     181.      0.90                  12.          3.          1.           1.
                     (INCHES)     1.563       1.563       1.563        1.563
                      (AC-FT)        6.          6.          6.           6.

                     CUMULATIVE AREA =    0.07 SQ MI

       ***              ***              ***              ***              ***



                     INTERPOLATED HYDROGRAPH AT     SB07

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     384.      0.90                  17.          4.          2.           2.
                     (INCHES)     2.197       2.197       2.197        2.197
                      (AC-FT)        9.          9.          9.           9.

                     CUMULATIVE AREA =    0.07 SQ MI



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           *            *
  146 KK   *     R7-8   *        ROUTE
           *            *
           **************
                         HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY

              HYDROGRAPH ROUTING DATA

  148 RK          KINEMATIC WAVE STREAM ROUTING
                          L       2778.  CHANNEL LENGTH
                          S      0.0014  SLOPE
                          N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                         CA        0.00  CONTRIBUTING AREA
                      SHAPE        TRAP  CHANNEL SHAPE
                         WD       46.00  BOTTOM WIDTH OR DIAMETER
                          Z        4.00  SIDE SLOPE
                     NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                          ***
                                           COMPUTED KINEMATIC PARAMETERS
                                                 VARIABLE TIME STEP
                                               (DT SHOWN IS A MINIMUM)

                 ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO      VOLUME    MAXIMUM
                                                                                PEAK                 CELERITY
                                                 (MIN)      (FT)     (CFS)     (MIN)        (IN)      (FPS)

                MAIN          0.21      1.53       2.79     694.50    374.54     61.97      2.32      4.32

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8717E+01  EXCESS=0.0000E+00 OUTFLOW=0.9160E+01 BASIN STORAGE=0.1297E-03 PERCENT ERROR=
-5.1


                                                  INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                MAIN          0.21      1.53       3.00               360.59     63.00      2.31

       ***              ***              ***              ***              ***

                      HYDROGRAPH AT STATION     R7-8
                     TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                        (CFS)
+     361.      1.05                  18.          5.          2.           2.
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              (INCHES)     2.311       2.314       2.314        2.314
               (AC-FT)        9.          9.          9.           9.

               CUMULATIVE AREA =    0.07 SQ MI

                              COMPUTED KINEMATIC PARAMETERS
                                    VARIABLE TIME STEP
                                  (DT SHOWN IS A MINIMUM)

           ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                          PEAK                 CELERITY
                                           (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

          MAIN          0.21      1.53       2.80     694.50    373.06     62.02      2.31      4.31


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8655E+01  EXCESS=0.0000E+00 OUTFLOW=0.9098E+01 BASIN STORAGE=0.1282E-03 PERCENT ERROR=
-5.1


                                 INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


          MAIN          0.21      1.53       3.00               359.23     63.00      2.30

     ***             ***             ***             ***             ***

                HYDROGRAPH AT STATION     R7-8
              TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                   (CFS)
+     359.      1.05                18.          5.          2.           2.
              (INCHES)     2.295       2.298       2.298        2.298
               (AC-FT)        9.          9.          9.           9.

               CUMULATIVE AREA =    0.07 SQ MI

                              COMPUTED KINEMATIC PARAMETERS
                                    VARIABLE TIME STEP
                                  (DT SHOWN IS A MINIMUM)

           ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                          PEAK                 CELERITY
                                           (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

          MAIN          0.21      1.53       3.00     694.50    300.60     62.65      2.18      4.00


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8267E+01  EXCESS=0.0000E+00 OUTFLOW=0.8591E+01 BASIN STORAGE=0.1511E-03 PERCENT ERROR=
-3.9


                                 INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


          MAIN          0.21      1.53       3.00               299.77     63.00      2.18

     ***             ***             ***             ***             ***

                HYDROGRAPH AT STATION     R7-8
              TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                   (CFS)
+     300.      1.05                17.          4.          2.           2.
              (INCHES)     2.176       2.178       2.179        2.179
               (AC-FT)        9.          9.          9.           9.

               CUMULATIVE AREA =    0.07 SQ MI

                              COMPUTED KINEMATIC PARAMETERS
                                    VARIABLE TIME STEP
                                  (DT SHOWN IS A MINIMUM)

           ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                          PEAK                 CELERITY
                                           (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

          MAIN          0.21      1.53       3.00     694.50    243.19     65.55      1.99      3.72


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7502E+01  EXCESS=0.0000E+00 OUTFLOW=0.7866E+01 BASIN STORAGE=0.1118E-03 PERCENT ERROR=
-4.8


                                 INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


          MAIN          0.21      1.53       3.00               238.95     66.00      2.00
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     ***             ***             ***             ***             ***

                        HYDROGRAPH AT STATION     R7-8
                      TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     239.      1.10                   16.         4.          2.           2.
                        (INCHES)     1.995       1.997       1.997        1.997
                         (AC-FT)        8.          8.          8.           8.

                         CUMULATIVE AREA =    0.07 SQ MI

                                       COMPUTED KINEMATIC PARAMETERS
                                             VARIABLE TIME STEP
                                           (DT SHOWN IS A MINIMUM)

                       ELEMENT    ALPHA       M         DT         DX        PEAK    TIME TO      VOLUME    MAXIMUM
                                                                                       PEAK                 CELERITY
                                                       (MIN)       (FT)      (CFS)     (MIN)        (IN)      (FPS)

                     MAIN          0.21      1.53       2.82     555.60     178.83     67.24       1.61       3.33

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6168E+01  EXCESS=0.0000E+00 OUTFLOW=0.6362E+01 BASIN STORAGE=0.1260E-03 PERCENT ERROR=
-3.1

                                          INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                     MAIN          0.21      1.53       3.00                176.29     66.00       1.61

     ***             ***             ***             ***             ***

                        HYDROGRAPH AT STATION     R7-8
                      TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     176.      1.10                   13.         3.          2.           2.
                        (INCHES)     1.609       1.611       1.611        1.611
                         (AC-FT)        6.          6.          6.           6.

                         CUMULATIVE AREA =    0.07 SQ MI

     ***             ***             ***             ***             ***


                         INTERPOLATED HYDROGRAPH AT     R7-8

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     360.      1.05                   18.         5.          2.           2.
                        (INCHES)     2.299       2.301       2.301        2.301
                         (AC-FT)        9.          9.          9.           9.

                         CUMULATIVE AREA =    0.07 SQ MI



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             *            *
  149 KK     *      SB08  *        BASIN
             *            *
             **************

               SUBBASIN RUNOFF DATA

  150 BA         SUBBASIN CHARACTERISTICS
                       TAREA        0.22  SUBBASIN AREA

  151 LG         GREEN AND AMPT LOSS RATE
                       STRTL        0.14  STARTING LOSS
                         DTH        0.25  MOISTURE DEFICIT
                        PSIF        4.10  WETTING FRONT SUCTION
                       XKSAT        0.59  HYDRAULIC CONDUCTIVITY
                       RTIMP       59.00  PERCENT IMPERVIOUS AREA

  152 UC         CLARK UNITGRAPH
                          TC        0.33  TIME OF CONCENTRATION
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                    R        0.19  STORAGE COEFFICIENT

 153 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                      0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                    100.0

                                                                     ***

                                                    UNIT HYDROGRAPH PARAMETERS
                                              CLARK   TC=  0.33 HR,       R=  0.19 HR
                                             SNYDER   TP=  0.19 HR,      CP=  0.55

                                                         UNIT HYDROGRAPH
                                                    24 END-OF-PERIOD ORDINATES
              35.      127.      298.      409.      385.      339.      288.      232.      179.      138.
             106.       82.       63.       49.       37.       29.       22.       17.       13.       10.
               8.        6.        5.        4.

      ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB08
                             TRANSPOSITION AREA      0.0 SQ MI

    TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.42, TOTAL EXCESS =    2.34

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     796.      0.95                     56.         14.          7.           7.
                             (INCHES)   2.325       2.325       2.325        2.325
                              (AC-FT)    28.         28.         28.          28.

                              CUMULATIVE AREA =    0.22 SQ MI


      ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB08
                             TRANSPOSITION AREA      0.5 SQ MI

    TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.42, TOTAL EXCESS =    2.32

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     791.      0.95                     56.         14.          7.           7.
                             (INCHES)   2.309       2.309       2.309        2.309
                              (AC-FT)    28.         28.         28.          28.

                              CUMULATIVE AREA =    0.22 SQ MI


      ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB08
                             TRANSPOSITION AREA      2.8 SQ MI

    TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.23

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     686.      1.00                     53.         13.          6.           6.
                             (INCHES)   2.218       2.218       2.218        2.218
                              (AC-FT)    26.         26.         26.          26.

                              CUMULATIVE AREA =    0.22 SQ MI


      ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB08
                             TRANSPOSITION AREA     16.0 SQ MI

    TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.50, TOTAL EXCESS =    2.04

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+     577.      1.00                     49.         12.          6.           6.
                             (INCHES)   2.032       2.032       2.032        2.032
                              (AC-FT)    24.         24.         24.          24.

                              CUMULATIVE AREA =    0.22 SQ MI


      ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB08
                             TRANSPOSITION AREA     90.0 SQ MI
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     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.53, TOTAL EXCESS =    1.71

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     439.      1.00                   41.         10.          5.           5.
                           (INCHES)     1.699       1.699       1.699        1.699
                            (AC-FT)      20.         20.         20.          20.

                            CUMULATIVE AREA =    0.22 SQ MI

       ***             ***             ***             ***             ***



                            INTERPOLATED HYDROGRAPH AT     SB08

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     792.      0.95                   56.         14.          7.           7.
                           (INCHES)     2.311       2.311       2.311        2.311
                            (AC-FT)      28.         28.         28.          28.

                            CUMULATIVE AREA =    0.22 SQ MI



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             *            *
  155 KK     *      CP-8  *      COMBINE
             *            *
             **************
                                    SR 101L FREEWAY AND HAYDEN ROAD (HAYDEN ROAD NORTH CHANNEL)

  157 HC          HYDROGRAPH COMBINATION
                       ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                       ***

       ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION     CP-8
                           TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+    1041.      1.05                   74.         19.          9.           9.
                           (INCHES)     2.322       2.322       2.322        2.322
                            (AC-FT)      37.         37.         37.          37.

                            CUMULATIVE AREA =    0.30 SQ MI


       ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION     CP-8
                           TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+    1035.      1.05                   74.         18.          9.           9.
                           (INCHES)     2.306       2.307       2.307        2.307
                            (AC-FT)      37.         37.         37.          37.

                            CUMULATIVE AREA =    0.30 SQ MI


       ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION     CP-8
                           TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     937.      1.05                   71.         18.          9.           9.
                           (INCHES)     2.208       2.208       2.208        2.208
                            (AC-FT)      35.         35.         35.          35.

                            CUMULATIVE AREA =    0.30 SQ MI
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      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-8
                       TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     783.      1.05                       65.         16.          8.            8.
                       (INCHES)     2.023       2.023       2.023         2.023
                        (AC-FT)       32.         32.         32.           32.

                        CUMULATIVE AREA =    0.30 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-8
                       TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     561.      1.10                       54.         13.          6.            6.
                       (INCHES)     1.676       1.677       1.677         1.677
                        (AC-FT)       27.         27.         27.           27.

                        CUMULATIVE AREA =    0.30 SQ MI

      ***             ***             ***             ***             ***



                        INTERPOLATED HYDROGRAPH AT     CP-8

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+    1035.      1.05                       74.         18.          9.            9.
                       (INCHES)     2.307       2.308       2.308         2.308
                        (AC-FT)       37.         37.         37.           37.

                        CUMULATIVE AREA =    0.30 SQ MI



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             *            *
  158 KK     *      R8-9  *        ROUTE
             *            *
             **************
                                 HAYDEN ROAD NORTH CHANNEL FROM HAYDEN ROAD TO BASIN 53R

                HYDROGRAPH ROUTING DATA

  160 RK          KINEMATIC WAVE STREAM ROUTING
                              L       1250.  CHANNEL LENGTH
                              S      0.0013  SLOPE
                              N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                             CA        0.00  CONTRIBUTING AREA
                          SHAPE        TRAP  CHANNEL SHAPE
                             WD       67.00  BOTTOM WIDTH OR DIAMETER
                              Z        4.00  SIDE SLOPE
                         NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                          ***
                                         COMPUTED KINEMATIC PARAMETERS
                                               VARIABLE TIME STEP
                                             (DT SHOWN IS A MINIMUM)

                      ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                   PEAK                 CELERITY
                                                     (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                     MAIN          0.15      1.56       1.32    416.67   1031.83     65.57      2.33      5.61


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3691E+02  EXCESS=0.0000E+00 OUTFLOW=0.3702E+02 BASIN STORAGE=0.1702E-03 PERCENT ERROR=
-0.3


                                              INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL
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MAIN	0.15	1.56	3.00		1013.13	66.00	2.33

*** *** *** *** ***

HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 0.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1013.	1.10		75.	19.	9.	9.
		(INCHES)	2.329	2.330	2.330	2.330
		(AC-FT)	37.	37.	37.	37.

CUMULATIVE AREA = 0.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.15	1.56	1.33	416.67	1027.12	65.64	2.31	5.60

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3666E+02 EXCESS=0.0000E+00 OUTFLOW=0.3678E+02 BASIN STORAGE=0.2024E-03 PERCENT ERROR= -0.3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.15	1.56	3.00		1010.29	66.00	2.32

*** *** *** *** ***

HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 0.5 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 1010.	1.10		74.	19.	9.	9.
		(INCHES)	2.314	2.315	2.316	2.316
		(AC-FT)	37.	37.	37.	37.

CUMULATIVE AREA = 0.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN	0.15	1.56	1.45	416.67	913.69	66.38	2.21	5.40

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3510E+02 EXCESS=0.0000E+00 OUTFLOW=0.3519E+02 BASIN STORAGE=0.1735E-03 PERCENT ERROR= -0.3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.15	1.56	3.00		911.34	66.00	2.21

*** *** *** *** ***

HYDROGRAPH AT STATION R8-9
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 911.	1.10		71.	18.	9.	9.
		(INCHES)	2.212	2.214	2.214	2.214
		(AC-FT)	35.	35.	35.	35.

CUMULATIVE AREA = 0.30 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)

```
                 MAIN          0.15      1.56       1.48    416.67    772.14     65.64      2.03      5.07


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3216E+02  EXCESS=0.0000E+00 OUTFLOW=0.3228E+02 BASIN STORAGE=0.1954E-03 PERCENT ERROR=
-0.4


                                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                 MAIN          0.15      1.56       3.00              769.23     66.00      2.03

       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     R8-9
                        TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     769.      1.10                     65.         16.          8.           8.
                           (INCHES)    2.031       2.033       2.033        2.033
                            (AC-FT)      32.         32.         32.          32.

                            CUMULATIVE AREA =    0.30 SQ MI

                                             COMPUTED KINEMATIC PARAMETERS
                                                   VARIABLE TIME STEP
                                                 (DT SHOWN IS A MINIMUM)

                  ELEMENT    ALPHA       M          DT         DX        PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                 CELERITY
                                                  (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                 MAIN          0.15      1.56       1.59    416.67    554.15     67.90      1.68      4.50


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2666E+02  EXCESS=0.0000E+00 OUTFLOW=0.2676E+02 BASIN STORAGE=0.1898E-03 PERCENT ERROR=
-0.4


                                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                 MAIN          0.15      1.56       3.00              554.05     69.00      1.68

       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     R8-9
                        TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     554.      1.15                     54.         13.          6.           6.
                           (INCHES)    1.682       1.684       1.684        1.684
                            (AC-FT)      27.         27.         27.          27.

                            CUMULATIVE AREA =    0.30 SQ MI

       ***             ***             ***             ***             ***



                            INTERPOLATED HYDROGRAPH AT     R8-9

  PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+    1010.      1.10                     74.         19.          9.           9.
                           (INCHES)    2.315       2.316       2.316        2.316
                            (AC-FT)      37.         37.         37.          37.

                            CUMULATIVE AREA =    0.30 SQ MI



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             *            *
  161 KK     *      SB09  *        BASIN
             *            *
             **************

                 SUBBASIN RUNOFF DATA
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  162 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.05  SUBBASIN AREA

  163 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.15  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.00  WETTING FRONT SUCTION
                        XKSAT        0.61  HYDRAULIC CONDUCTIVITY
                        RTIMP       55.00  PERCENT IMPERVIOUS AREA

  164 UC          CLARK UNITGRAPH
                           TC        0.25  TIME OF CONCENTRATION
                            R        0.23  STORAGE COEFFICIENT

  165 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                       CLARK   TC=  0.25 HR,       R=  0.23 HR
                                                      SNYDER   TP=  0.15 HR,      CP=  0.42

                                                                   UNIT HYDROGRAPH
                                                              27 END-OF-PERIOD ORDINATES
                   11.       51.       86.       89.       82.       70.       56.       45.       36.       29.
                   23.       19.       15.       12.       10.        8.        6.        5.        4.        3.
                    3.        2.        2.        1.        1.        1.        1.

       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB09
                        TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.47, TOTAL EXCESS =    2.29

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     174.      0.95                    13.          3.          2.           2.
                        (INCHES)     2.280       2.280       2.280        2.280
                         (AC-FT)        6.          6.          6.           6.

                         CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB09
                        TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.27

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     173.      0.95                    13.          3.          2.           2.
                        (INCHES)     2.264       2.264       2.264        2.264
                         (AC-FT)        6.          6.          6.           6.

                         CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB09
                        TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.51, TOTAL EXCESS =    2.18

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     153.      0.95                    12.          3.          1.           1.
                        (INCHES)     2.168       2.168       2.168        2.168
                         (AC-FT)        6.          6.          6.           6.

                         CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB09
                        TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.55, TOTAL EXCESS =    1.99

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
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                          (CFS)
+     127.      0.95                 11.          3.          1.           1.
                       (INCHES)     1.978       1.978       1.978        1.978
                        (AC-FT)        5.          5.          5.           5.

                        CUMULATIVE AREA =    0.05 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB09
                       TRANSPOSITION AREA     90.0 SQ MI

      TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.58, TOTAL EXCESS =    1.65

    PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                         6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+      96.      0.95                  9.          2.          1.           1.
                       (INCHES)     1.642       1.642       1.642        1.642
                        (AC-FT)        5.          5.          5.           5.

                        CUMULATIVE AREA =    0.05 SQ MI

       ***             ***             ***             ***             ***



                        INTERPOLATED HYDROGRAPH AT     SB09

    PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                         6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     173.      0.95                 13.          3.          2.           2.
                       (INCHES)     2.269       2.269       2.269        2.269
                        (AC-FT)        6.          6.          6.           6.

                        CUMULATIVE AREA =    0.05 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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              **************
              *            *
   167 KK     *      CP-9  *      COMBINE
              *            *
              **************
                                  SR 101L FREEWAY AND BASIN 53R (HAYDEN ROAD NORTH CHANNEL)

   169 HC          HYDROGRAPH COMBINATION
                       ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                        ***

       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-9
                       TRANSPOSITION AREA      0.0 SQ MI

    PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                         6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1127.      1.10                 87.         22.         11.          11.
                       (INCHES)     2.322       2.323       2.323        2.323
                        (AC-FT)       43.         43.         43.          43.

                        CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     CP-9
                       TRANSPOSITION AREA      0.5 SQ MI

    PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                         6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1124.      1.10                 87.         22.         10.          10.
                       (INCHES)     2.307       2.308       2.308        2.308
                        (AC-FT)       43.         43.         43.          43.

                        CUMULATIVE AREA =    0.35 SQ MI


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                    HYDROGRAPH AT STATION     CP-9
                  TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+    1023.      1.10                    83.         21.         10.          10.
                          (INCHES)     2.206       2.207       2.207        2.207
                           (AC-FT)      41.         41.         41.          41.

                           CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION     CP-9
                  TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     869.      1.10                    76.         19.          9.           9.
                          (INCHES)     2.023       2.024       2.025        2.025
                           (AC-FT)      38.         38.         38.          38.

                           CUMULATIVE AREA =    0.35 SQ MI


       ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION     CP-9
                  TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     622.      1.15                    63.         16.          8.           8.
                          (INCHES)     1.676       1.678       1.678        1.678
                           (AC-FT)      31.         31.         31.          31.

                           CUMULATIVE AREA =    0.35 SQ MI

       ***             ***             ***             ***             ***



                           INTERPOLATED HYDROGRAPH AT     CP-9

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+    1124.      1.10                    87.         22.         10.          10.
                          (INCHES)     2.307       2.309       2.309        2.309
                           (AC-FT)      43.         43.         43.          43.

                           CUMULATIVE AREA =    0.35 SQ MI



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             *            *
  170 KK     *    PIMACH  *
             *            *
             **************
                                   PIMA ROAD CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
                                   100-YR, 6-HR FLO-2D MODEL (XS 183)

                SUBBASIN RUNOFF DATA

  173 BA          SUBBASIN CHARACTERISTICS
                        TAREA       12.30  SUBBASIN AREA

                                                                      ***

       ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION   PIMACH
                  TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+    4017.      5.50                  1092.        292.        146.         146.
                          (INCHES)     0.825       0.884       0.919        0.919
```

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                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI


       ***            ***            ***            ***            ***

                     HYDROGRAPH AT STATION   PIMACH
                   TRANSPOSITION AREA      0.5 SQ MI

    PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                       (CFS)
 +    4017.      5.50              1092.        292.        146.         146.
                   (INCHES)     0.825       0.884       0.919        0.919
                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI


       ***            ***            ***            ***            ***

                     HYDROGRAPH AT STATION   PIMACH
                   TRANSPOSITION AREA      2.8 SQ MI

    PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                       (CFS)
 +    4017.      5.50              1092.        292.        146.         146.
                   (INCHES)     0.825       0.884       0.919        0.919
                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI


       ***            ***            ***            ***            ***

                     HYDROGRAPH AT STATION   PIMACH
                   TRANSPOSITION AREA     16.0 SQ MI

    PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                       (CFS)
 +    4017.      5.50              1092.        292.        146.         146.
                   (INCHES)     0.825       0.884       0.919        0.919
                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI


       ***            ***            ***            ***            ***

                     HYDROGRAPH AT STATION   PIMACH
                   TRANSPOSITION AREA     90.0 SQ MI

    PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                       (CFS)
 +    4017.      5.50              1092.        292.        146.         146.
                   (INCHES)     0.825       0.884       0.919        0.919
                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI

       ***            ***            ***            ***            ***


                    INTERPOLATED HYDROGRAPH AT   PIMACH

    PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                       (CFS)
 +    4017.      5.50              1092.        292.        146.         146.
                   (INCHES)     0.825       0.884       0.919        0.919
                    (AC-FT)      541.        580.        603.         603.

                    CUMULATIVE AREA =   12.30 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             *            *
   211 KK    *    PIMAIN  *
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            *             *
            **************
                                  BYPASS ALONG SE SIDE OF BASIN 53R TO ADOT CULVERT NEAR UNION HILLS DR

     DT          DIVERSION
                       ISTAD      PIMABY  DIVERSION HYDROGRAPH IDENTIFICATION

     DI             INFLOW        0.00   1000.00  10000.00

     DQ      DIVERTED FLOW        0.00   1000.00   1000.00
                                                                         ***

      ***             ***             ***             ***             ***

                         DIVERSION HYDROGRAPH   PIMABY
                       TRANSPOSITION AREA      0.0 SQ MI

   PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                            (CFS)
 +    1000.      4.95                  605.        171.         88.          88.
                       (INCHES)     0.458       0.516       0.551        0.551
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION   PIMAIN
                       TRANSPOSITION AREA      0.0 SQ MI

   PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                            (CFS)
 +    3017.      5.50                  486.        122.         58.          58.
                       (INCHES)     0.368       0.368       0.368        0.368
                        (AC-FT)      241.        241.        241.         241.

                        CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                         DIVERSION HYDROGRAPH   PIMABY
                       TRANSPOSITION AREA      0.5 SQ MI

   PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                            (CFS)
 +    1000.      4.95                  605.        171.         88.          88.
                       (INCHES)     0.458       0.516       0.551        0.551
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION   PIMAIN
                       TRANSPOSITION AREA      0.5 SQ MI

   PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                            (CFS)
 +    3017.      5.50                  486.        122.         58.          58.
                       (INCHES)     0.368       0.368       0.368        0.368
                        (AC-FT)      241.        241.        241.         241.

                        CUMULATIVE AREA =   12.30 SQ MI


      ***             ***             ***             ***             ***

                         DIVERSION HYDROGRAPH   PIMABY
                       TRANSPOSITION AREA      2.8 SQ MI

   PEAK FLOW     TIME                              MAXIMUM AVERAGE FLOW
                                       6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                            (CFS)
 +    1000.      4.95                  605.        171.         88.          88.
                       (INCHES)     0.458       0.516       0.551        0.551
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI


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                         HYDROGRAPH AT STATION   PIMAIN
                       TRANSPOSITION AREA      2.8 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+    3017.       5.50                486.        122.         58.          58.
                       (INCHES)     0.368       0.368       0.368        0.368
                        (AC-FT)      241.        241.        241.         241.

                        CUMULATIVE AREA =   12.30 SQ MI


       ***             ***             ***             ***             ***

                         DIVERSION HYDROGRAPH   PIMABY
                       TRANSPOSITION AREA     16.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+    1000.       4.95                605.        171.         88.          88.
                       (INCHES)     0.458       0.516       0.551        0.551
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION   PIMAIN
                       TRANSPOSITION AREA     16.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+    3017.       5.50                486.        122.         58.          58.
                       (INCHES)     0.368       0.368       0.368        0.368
                        (AC-FT)      241.        241.        241.         241.

                        CUMULATIVE AREA =   12.30 SQ MI


       ***             ***             ***             ***             ***

                         DIVERSION HYDROGRAPH   PIMABY
                       TRANSPOSITION AREA     90.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+    1000.       4.95                605.        171.         88.          88.
                       (INCHES)     0.458       0.516       0.551        0.551
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION   PIMAIN
                       TRANSPOSITION AREA     90.0 SQ MI

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+    3017.       5.50                486.        122.         58.          58.
                       (INCHES)     0.368       0.368       0.368        0.368
                        (AC-FT)      241.        241.        241.         241.

                        CUMULATIVE AREA =   12.30 SQ MI

       ***             ***             ***             ***             ***


                     INTERPOLATED DIVERSION HYDROGRAPH AT   PIMABY

   PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+    (CFS)       (HR)
                          (CFS)
+    1000.       4.95                605.        171.         88.          88.
                       (INCHES)     0.458       0.516       0.551        0.551
                        (AC-FT)      300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI
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      ***             ***             ***             ***             ***


                       INTERPOLATED HYDROGRAPH AT   PIMAIN

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+    3017.      5.50                486.        122.         58.          58.
                          (INCHES)     0.368       0.368       0.368        0.368
                           (AC-FT)      241.        241.        241.         241.

                       CUMULATIVE AREA =   12.30 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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            **************
            *            *
  216 KK    *     PWRCH  *
            *            *
            **************
                               POWERLINE CHANNEL HYDROGRAPH FROM FLO-2D
                               100-YR, 6-HR FLO-2D MODEL (XS 107)

               SUBBASIN RUNOFF DATA

  219 BA         SUBBASIN CHARACTERISTICS
                         TAREA        7.00  SUBBASIN AREA

                                                                    ***

      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION    PWRCH
                       TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     740.      5.80                432.        136.         75.          75.
                          (INCHES)     0.574       0.724       0.826        0.826
                           (AC-FT)      214.        270.        308.         308.

                       CUMULATIVE AREA =    7.00 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION    PWRCH
                       TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     740.      5.80                432.        136.         75.          75.
                          (INCHES)     0.574       0.724       0.826        0.826
                           (AC-FT)      214.        270.        308.         308.

                       CUMULATIVE AREA =    7.00 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION    PWRCH
                       TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+     740.      5.80                432.        136.         75.          75.
                          (INCHES)     0.574       0.724       0.826        0.826
                           (AC-FT)      214.        270.        308.         308.

                       CUMULATIVE AREA =    7.00 SQ MI


      ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION    PWRCH
                       TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
```

```
                          (CFS)
+     740.      5.80                432.        136.         75.          75.
                       (INCHES)    0.574       0.724       0.826        0.826
                        (AC-FT)     214.        270.        308.         308.

                        CUMULATIVE AREA =    7.00 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION    PWRCH
                       TRANSPOSITION AREA     90.0 SQ MI

   PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     740.      5.80                432.        136.         75.          75.
                       (INCHES)    0.574       0.724       0.826        0.826
                        (AC-FT)     214.        270.        308.         308.

                        CUMULATIVE AREA =    7.00 SQ MI

       ***             ***             ***             ***             ***



                        INTERPOLATED HYDROGRAPH AT    PWRCH

   PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     740.      5.80                432.        136.         75.          75.
                       (INCHES)    0.574       0.724       0.826        0.826
                        (AC-FT)     214.        270.        308.         308.

                        CUMULATIVE AREA =    7.00 SQ MI




 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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              **************
              *            *
   257 KK     *    BINFLO  *
              *            *
              **************
                                 TOTAL INFLOW INTO BASIN 53R.

   259 HC          HYDROGRAPH COMBINATION
                       ICOMP           3  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                        ***

       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION   BINFLO
                       TRANSPOSITION AREA      0.0 SQ MI

   PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    3634.      5.50                919.        277.        144.         144.
                       (INCHES)    0.435       0.523       0.566        0.566
                        (AC-FT)     456.        549.        593.         593.

                        CUMULATIVE AREA =   19.65 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION   BINFLO
                       TRANSPOSITION AREA      0.5 SQ MI

   PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    3634.      5.50                919.        276.        144.         144.
                       (INCHES)    0.435       0.523       0.565        0.565
                        (AC-FT)     456.        548.        593.         593.

                        CUMULATIVE AREA =   19.65 SQ MI


       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION   BINFLO
                       TRANSPOSITION AREA      2.8 SQ MI
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  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    3634.      5.50                     919.         275.        143.         143.
                            (INCHES)    0.435        0.521       0.564        0.564
                             (AC-FT)     456.         546.        591.         591.

                             CUMULATIVE AREA =   19.65 SQ MI


      ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION   BINFLO
                            TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    3634.      5.50                     919.         274.        142.         142.
                            (INCHES)    0.435        0.518       0.560        0.560
                             (AC-FT)     456.         543.        587.         587.

                             CUMULATIVE AREA =   19.65 SQ MI


      ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION   BINFLO
                            TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    3634.      5.50                     919.         270.        141.         141.
                            (INCHES)    0.435        0.512       0.554        0.554
                             (AC-FT)     456.         536.        581.         581.

                             CUMULATIVE AREA =   19.65 SQ MI

      ***             ***             ***             ***             ***



                             INTERPOLATED HYDROGRAPH AT   BINFLO

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    3634.      5.50                     919.         273.        142.         142.
                            (INCHES)    0.435        0.517       0.560        0.560
                             (AC-FT)     456.         542.        587.         587.

                             CUMULATIVE AREA =   19.65 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             **************
             *            *
  260 KK     *     BASIN  *
             *            *
             **************
                                    BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
                                    ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
                                     OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
                                     THE OUTLET PIPES ARE INLET CONTROLLED.

                HYDROGRAPH ROUTING DATA

  265 RS          STORAGE ROUTING
                        NSTPS           1  NUMBER OF SUBREACHES
                         ITYP        STOR  TYPE OF INITIAL CONDITION
                       RSVRIC        0.00  INITIAL CONDITION
                            X        0.00 WORKING R AND D COEFFICIENT

  266 SV            STORAGE          0.0       44.9       76.8      108.7      140.6      171.5      202.5      233.5      264.5
295.5
                                   328.7      362.0      395.2      428.5      461.7      497.2      532.7      568.3      603.8
639.3

  268 SE          ELEVATION     1594.00   1597.00   1598.00   1599.00   1600.00   1601.00   1602.00   1603.00   1604.00
1605.00
                                1606.00   1607.00   1608.00   1609.00   1610.00   1611.00   1612.00   1613.00   1614.00
1615.00
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  270 SQ         DISCHARGE         0.       95.      153.      210.      244.      265.      284.      303.      320.
337.
                                 353.      369.      384.      398.      412.      426.      439.      452.      465.
477.

                                                                   ***

       ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION    BASIN
                           TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                                 MAXIMUM AVERAGE FLOW
                                          6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     363.      8.30                      355.         250.        141.         141.
                           (INCHES)     0.168        0.474       0.556        0.556
                            (AC-FT)       176.         496.        582.         582.

 PEAK STORAGE   TIME                                MAXIMUM AVERAGE STORAGE
                                          6-HR        24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      351.      8.30                      332.         189.        101.         101.

  PEAK STAGE    TIME                                 MAXIMUM AVERAGE STAGE
                                          6-HR        24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.68      8.30                   1606.11      1601.55     1598.29      1598.29

                            CUMULATIVE AREA =   19.65 SQ MI


       ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION    BASIN
                           TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                                 MAXIMUM AVERAGE FLOW
                                          6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     363.      8.30                      355.         250.        141.         141.
                           (INCHES)     0.168        0.474       0.555        0.555
                            (AC-FT)       176.         496.        582.         582.

 PEAK STORAGE   TIME                                MAXIMUM AVERAGE STORAGE
                                          6-HR        24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      351.      8.30                      332.         189.        100.         100.

  PEAK STAGE    TIME                                 MAXIMUM AVERAGE STAGE
                                          6-HR        24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.67      8.30                   1606.11      1601.55     1598.28      1598.28

                            CUMULATIVE AREA =   19.65 SQ MI


       ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION    BASIN
                           TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                                 MAXIMUM AVERAGE FLOW
                                          6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     363.      8.30                      354.         250.        141.         141.
                           (INCHES)     0.168        0.473       0.553        0.553
                            (AC-FT)       176.         495.        580.         580.

 PEAK STORAGE   TIME                                MAXIMUM AVERAGE STORAGE
                                          6-HR        24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      350.      8.30                      331.         188.        100.         100.

  PEAK STAGE    TIME                                 MAXIMUM AVERAGE STAGE
                                          6-HR        24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.65      8.30                   1606.08      1601.53     1598.27      1598.27

                            CUMULATIVE AREA =   19.65 SQ MI


       ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION    BASIN
                           TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                                 MAXIMUM AVERAGE FLOW
                                          6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
```

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+     362.      8.30                 353.        249.        140.         140.
                         (INCHES)    0.167       0.471       0.550        0.550
                          (AC-FT)     175.        494.        577.         577.

 PEAK STORAGE   TIME                            MAXIMUM AVERAGE STORAGE
                                     6-HR       24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      349.      8.35                 330.        187.         99.          99.

  PEAK STAGE    TIME                             MAXIMUM AVERAGE STAGE
                                     6-HR       24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.61      8.30              1606.04     1601.50     1598.24      1598.24

                          CUMULATIVE AREA =   19.65 SQ MI


       ***              ***             ***             ***             ***

                           HYDROGRAPH AT STATION    BASIN
                         TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     361.      8.35                 352.        247.        138.         138.
                         (INCHES)    0.167       0.468       0.544        0.544
                          (AC-FT)     175.        491.        570.         570.

 PEAK STORAGE   TIME                            MAXIMUM AVERAGE STORAGE
                                     6-HR       24-HR       72-HR     49.95-HR
+  (AC-FT)      (HR)
      346.      8.35                 327.        185.         98.          98.

  PEAK STAGE    TIME                             MAXIMUM AVERAGE STAGE
                                     6-HR       24-HR       72-HR     49.95-HR
+   (FEET)      (HR)
   1606.52      8.35              1605.96     1601.44     1598.18      1598.18

                          CUMULATIVE AREA =   19.65 SQ MI

       ***              ***             ***             ***             ***



                          INTERPOLATED HYDROGRAPH AT    BASIN

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+     362.      8.30                 353.        249.        140.         140.
                         (INCHES)    0.167       0.471       0.550        0.550
                          (AC-FT)     175.        493.        576.         576.

                          CUMULATIVE AREA =   19.65 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
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             *            *
  272 KK     *    BSNRT1  *
             *            *
             **************
                                  2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
                                  DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
                                  EXCESS CAPACITY.

                HYDROGRAPH ROUTING DATA

  276 RK          KINEMATIC WAVE STREAM ROUTING
                          L        550.  CHANNEL LENGTH
                          S      0.0052  SLOPE
                          N       0.024  CHANNEL ROUGHNESS COEFFICIENT
                         CA        0.00  CONTRIBUTING AREA
                      SHAPE        CIRC  CHANNEL SHAPE
                         WD        7.00  BOTTOM WIDTH OR DIAMETER
                          Z        0.00  SIDE SLOPE
                     NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS
                                                                        ***

         *** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT


         *** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT
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COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.29	183.33	363.48	498.70	0.56	10.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03 EXCESS=0.0000E+00 OUTFLOW=0.5822E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00		363.48	501.00	0.56

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HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 0.0 SQ MI

	PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
				6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)					
			(CFS)				
+	363.	8.35		355.	250.	141.	141.
			(INCHES)	0.168	0.474	0.556	0.556
			(AC-FT)	176.	496.	582.	582.

CUMULATIVE AREA =   19.65 SQ MI

*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

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*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

*** FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION USED - ITERATION= 1

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.29	183.33	363.42	498.84	0.56	10.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03 EXCESS=0.0000E+00 OUTFLOW=0.5819E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00		363.42	501.00	0.56

***   ***   ***   ***   ***

HYDROGRAPH AT STATION   BSNRT1
TRANSPOSITION AREA   0.5 SQ MI

	PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
				6-HR	24-HR	72-HR	49.95-HR
+	(CFS)	(HR)					
			(CFS)				
+	363.	8.35		355.	250.	141.	141.
			(INCHES)	0.168	0.474	0.555	0.555
			(AC-FT)	176.	496.	582.	582.

CUMULATIVE AREA =   19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.45	183.33	363.06	498.87	0.55	10.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5801E+03 EXCESS=0.0000E+00 OUTFLOW=0.5800E+03 BASIN STORAGE=0.3288E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00		363.06	501.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 2.8 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 363.	8.35		354.	250.	141.	141.
		(INCHES)	0.168	0.473	0.553	0.553
		(AC-FT)	176.	495.	580.	580.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.38	183.33	362.38	500.88	0.55	10.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5767E+03 EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3287E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00		362.38	501.00	0.55

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 16.0 SQ MI

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	49.95-HR
+ (CFS)	(HR)					
		(CFS)				
+ 362.	8.35		353.	249.	140.	140.
		(INCHES)	0.167	0.471	0.550	0.550
		(AC-FT)	175.	494.	577.	577.

CUMULATIVE AREA = 19.65 SQ MI

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	3.34	1.25	0.34	183.33	361.10	501.33	0.54	10.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03 EXCESS=0.0000E+00 OUTFLOW=0.5702E+03 BASIN STORAGE=0.3284E-01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	3.34	1.25	3.00		361.10	501.00	0.54

*** *** *** *** ***

HYDROGRAPH AT STATION BSNRT1
TRANSPOSITION AREA 90.0 SQ MI

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  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     361.      8.35                     352.        248.        138.         138.
                           (INCHES)     0.167       0.468       0.544        0.544
                            (AC-FT)      175.        491.        570.         570.

                            CUMULATIVE AREA =   19.65 SQ MI

       ***             ***             ***             ***             ***


                            INTERPOLATED HYDROGRAPH AT   BSNRT1

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     362.      8.35                     353.        249.        140.         140.
                           (INCHES)     0.167       0.471       0.549        0.549
                            (AC-FT)      175.        494.        576.         576.

                            CUMULATIVE AREA =   19.65 SQ MI



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             *            *
  277 KK     *    BSNRT2  *
             *            *
             **************
                                    2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BASIN 53R OUTFAL

                HYDROGRAPH ROUTING DATA

  279 RK          KINEMATIC WAVE STREAM ROUTING
                            L       1200.  CHANNEL LENGTH
                            S      0.0077  SLOPE
                            N       0.013  CHANNEL ROUGHNESS COEFFICIENT
                           CA        0.00  CONTRIBUTING AREA
                        SHAPE        CIRC  CHANNEL SHAPE
                           WD        7.00  BOTTOM WIDTH OR DIAMETER
                            Z        0.00  SIDE SLOPE
                       NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                  ***
                                             COMPUTED KINEMATIC PARAMETERS
                                                   VARIABLE TIME STEP
                                                 (DT SHOWN IS A MINIMUM)

                         ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                        PEAK                CELERITY
                                                        (MIN)      (FT)      (CFS)     (MIN)       (IN)      (FPS)

                        MAIN          7.51      1.25       0.44    400.00    363.48    501.11      0.56     20.39


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03  EXCESS=0.0000E+00 OUTFLOW=0.5821E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR=
0.0


                                                INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                        MAIN          7.51      1.25       3.00              363.48    501.00      0.56

       ***             ***             ***             ***             ***

                             HYDROGRAPH AT STATION   BSNRT2
                           TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                         6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                              (CFS)
+     363.      8.35                     355.        250.        141.         141.
                           (INCHES)     0.168       0.474       0.555        0.555
                            (AC-FT)      176.        496.        582.         582.

                            CUMULATIVE AREA =   19.65 SQ MI

                                             COMPUTED KINEMATIC PARAMETERS
                                                   VARIABLE TIME STEP
                                                 (DT SHOWN IS A MINIMUM)

                         ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
```

```
                                                    PEAK                 CELERITY
                                  (MIN)     (FT)    (CFS)    (MIN)     (IN)     (FPS)

             MAIN       7.51      1.25      0.44    400.00   363.42    501.35    0.56     20.39


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03  EXCESS=0.0000E+00 OUTFLOW=0.5818E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR=
0.0


                         INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


             MAIN       7.51      1.25      3.00             363.42    501.00    0.56

      ***           ***           ***           ***           ***

              HYDROGRAPH AT STATION   BSNRT2
             TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                  MAXIMUM AVERAGE FLOW
                               6-HR       24-HR      72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
+     363.      8.35            355.       250.       141.        141.
                   (INCHES)    0.168      0.474      0.555       0.555
                    (AC-FT)     176.       496.       582.        582.

                   CUMULATIVE AREA =   19.65 SQ MI

                           COMPUTED KINEMATIC PARAMETERS
                                VARIABLE TIME STEP
                              (DT SHOWN IS A MINIMUM)

              ELEMENT    ALPHA       M         DT        DX       PEAK   TIME TO    VOLUME    MAXIMUM
                                                                         PEAK                 CELERITY
                                  (MIN)     (FT)    (CFS)    (MIN)     (IN)     (FPS)

             MAIN       7.51      1.25      0.36    400.00   363.06    501.42    0.55     20.38


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5800E+03  EXCESS=0.0000E+00 OUTFLOW=0.5799E+03 BASIN STORAGE=0.3755E-01 PERCENT ERROR=
0.0


                         INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


             MAIN       7.51      1.25      3.00             363.06    501.00    0.55

      ***           ***           ***           ***           ***

              HYDROGRAPH AT STATION   BSNRT2
             TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                  MAXIMUM AVERAGE FLOW
                               6-HR       24-HR      72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
+     363.      8.35            354.       250.       140.        140.
                   (INCHES)    0.168      0.473      0.553       0.553
                    (AC-FT)     176.       495.       580.        580.

                   CUMULATIVE AREA =   19.65 SQ MI

                           COMPUTED KINEMATIC PARAMETERS
                                VARIABLE TIME STEP
                              (DT SHOWN IS A MINIMUM)

              ELEMENT    ALPHA       M         DT        DX       PEAK   TIME TO    VOLUME    MAXIMUM
                                                                         PEAK                 CELERITY
                                  (MIN)     (FT)    (CFS)    (MIN)     (IN)     (FPS)

             MAIN       7.51      1.25      0.48    400.00   362.38    501.44    0.55     20.37


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5766E+03  EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3753E-01 PERCENT ERROR=
0.0


                         INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


             MAIN       7.51      1.25      3.00             362.38    501.00    0.55

      ***           ***           ***           ***           ***

              HYDROGRAPH AT STATION   BSNRT2
             TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                  MAXIMUM AVERAGE FLOW
                               6-HR       24-HR      72-HR     49.95-HR
+   (CFS)       (HR)
                      (CFS)
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+     362.      8.35                 353.        249.        140.        140.
                       (INCHES)     0.167       0.471       0.550       0.550
                        (AC-FT)      175.        494.        577.        577.

                        CUMULATIVE AREA =   19.65 SQ MI

                                      COMPUTED KINEMATIC PARAMETERS
                                           VARIABLE TIME STEP
                                          (DT SHOWN IS A MINIMUM)

                     ELEMENT    ALPHA       M         DT         DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                   PEAK                 CELERITY
                                                     (MIN)      (FT)     (CFS)     (MIN)       (IN)      (FPS)

                    MAIN          7.51      1.25       0.38    400.00    361.10    501.56      0.54     20.36


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03  EXCESS=0.0000E+00 OUTFLOW=0.5701E+03 BASIN STORAGE=0.3750E-01 PERCENT ERROR=
0.0

                                             INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                    MAIN          7.51      1.25       3.00              361.09    501.00      0.54

       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION   BSNRT2
                       TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                     6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     361.      8.35                 352.        248.        138.        138.
                       (INCHES)     0.167       0.468       0.544       0.544
                        (AC-FT)      175.        491.        570.        570.

                        CUMULATIVE AREA =   19.65 SQ MI

       ***             ***             ***             ***             ***


                        INTERPOLATED HYDROGRAPH AT   BSNRT2

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                     6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     362.      8.35                 353.        249.        139.        139.
                       (INCHES)     0.167       0.471       0.549       0.549
                        (AC-FT)      175.        494.        576.        576.

                        CUMULATIVE AREA =   19.65 SQ MI



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             **************
             *            *
  280 KK     *    PIMABY  *
             *            *
             **************
                                RETRIEVE PIMA BYPASS CHANNEL HYDROGRAPH

  282 DR          RETRIEVE DIVERSION HYDROGRAPH
                       ISTAD      PIMABY  DIVERSION HYDROGRAPH IDENTIFICATION

                                                                         ***

       ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION   PIMABY
                       TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                            MAXIMUM AVERAGE FLOW
                                     6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1000.      4.95                 605.        171.         88.         88.
                       (INCHES)     0.286       0.323       0.345       0.345
                        (AC-FT)      300.        338.        361.        361.

                        CUMULATIVE AREA =   12.30 SQ MI


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                      HYDROGRAPH AT STATION   PIMABY
                    TRANSPOSITION AREA      0.5 SQ MI

   PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                           (CFS)
 +    1000.      4.95                 605.        171.         88.          88.
                         (INCHES)    0.286       0.323       0.345        0.345
                          (AC-FT)     300.        338.        361.         361.

                          CUMULATIVE AREA =   12.30 SQ MI


        ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION   PIMABY
                    TRANSPOSITION AREA      2.8 SQ MI

   PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                           (CFS)
 +    1000.      4.95                 605.        171.         88.          88.
                         (INCHES)    0.286       0.323       0.345        0.345
                          (AC-FT)     300.        338.        361.         361.

                          CUMULATIVE AREA =   12.30 SQ MI


        ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION   PIMABY
                    TRANSPOSITION AREA     16.0 SQ MI

   PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                           (CFS)
 +    1000.      4.95                 605.        171.         88.          88.
                         (INCHES)    0.286       0.323       0.345        0.345
                          (AC-FT)     300.        338.        361.         361.

                          CUMULATIVE AREA =   12.30 SQ MI


        ***             ***             ***             ***             ***

                      HYDROGRAPH AT STATION   PIMABY
                    TRANSPOSITION AREA     90.0 SQ MI

   PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                           (CFS)
 +    1000.      4.95                 605.        171.         88.          88.
                         (INCHES)    0.286       0.323       0.345        0.345
                          (AC-FT)     300.        338.        361.         361.

                          CUMULATIVE AREA =   12.30 SQ MI

        ***             ***             ***             ***             ***


                          INTERPOLATED HYDROGRAPH AT   PIMABY

   PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
 +   (CFS)       (HR)
                           (CFS)
 +    1000.      4.95                 605.        171.         88.          88.
                         (INCHES)    0.458       0.516       0.551        0.551
                          (AC-FT)     300.        338.        361.         361.

                          CUMULATIVE AREA =   12.30 SQ MI



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             **************
             *            *
   283 KK    *    PMB-RT  *
             *            *
             **************
                                  UNION HILLS DR CHANNEL FROM SR 101L FREEWAY TO 82ND ST

                 HYDROGRAPH ROUTING DATA

   285 RK          KINEMATIC WAVE STREAM ROUTING
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                 L        3157.  CHANNEL LENGTH
                 S       0.0082  SLOPE
                 N        0.013  CHANNEL ROUGHNESS COEFFICIENT
                CA         0.00  CONTRIBUTING AREA
             SHAPE         TRAP  CHANNEL SHAPE
                WD        24.00  BOTTOM WIDTH OR DIAMETER
                 Z         4.00  SIDE SLOPE
            NDXMIN            2  MINIMUM NUMBER OF DX INTERVALS

                                                       ***
                                   COMPUTED KINEMATIC PARAMETERS
                                         VARIABLE TIME STEP
                                       (DT SHOWN IS A MINIMUM)

            ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                          PEAK                CELERITY
                                           (MIN)      (FT)     (CFS)    (MIN)        (IN)      (FPS)

           MAIN          1.87      1.47       0.99   1052.33   1000.00    299.31      0.55     20.34


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03  EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0


                                      INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


           MAIN          1.87      1.47       3.00             1000.00    300.00      0.55

       ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION   PMB-RT
                  TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                      MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1000.      5.00               605.        171.         87.          87.
                       (INCHES)   0.457       0.516       0.551        0.551
                        (AC-FT)    300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI

                                   COMPUTED KINEMATIC PARAMETERS
                                         VARIABLE TIME STEP
                                       (DT SHOWN IS A MINIMUM)

            ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                          PEAK                CELERITY
                                           (MIN)      (FT)     (CFS)    (MIN)        (IN)      (FPS)

           MAIN          1.87      1.47       0.99   1052.33   1000.00    299.31      0.55     20.34


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03  EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0


                                      INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


           MAIN          1.87      1.47       3.00             1000.00    300.00      0.55

       ***             ***             ***             ***             ***

                    HYDROGRAPH AT STATION   PMB-RT
                  TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                      MAXIMUM AVERAGE FLOW
                                   6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1000.      5.00               605.        171.         87.          87.
                       (INCHES)   0.457       0.516       0.551        0.551
                        (AC-FT)    300.        338.        361.         361.

                        CUMULATIVE AREA =   12.30 SQ MI

                                   COMPUTED KINEMATIC PARAMETERS
                                         VARIABLE TIME STEP
                                       (DT SHOWN IS A MINIMUM)

            ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                          PEAK                CELERITY
                                           (MIN)      (FT)     (CFS)    (MIN)        (IN)      (FPS)

           MAIN          1.87      1.47       0.99   1052.33   1000.00    299.31      0.55     20.34


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03  EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0
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                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                MAIN        1.87      1.47      3.00            1000.00    300.00      0.55

      ***            ***            ***            ***            ***

                       HYDROGRAPH AT STATION   PMB-RT
                     TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                     6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+    1000.      5.00                  605.        171.         87.          87.
                         (INCHES)    0.457       0.516       0.551        0.551
                          (AC-FT)     300.        338.        361.         361.

                          CUMULATIVE AREA =   12.30 SQ MI

                                     COMPUTED KINEMATIC PARAMETERS
                                           VARIABLE TIME STEP
                                        (DT SHOWN IS A MINIMUM)

                 ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                               PEAK                CELERITY
                                                 (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

                MAIN        1.87      1.47      0.99   1052.33   1000.00    299.31      0.55     20.34

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03  EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                MAIN        1.87      1.47      3.00            1000.00    300.00      0.55

      ***            ***            ***            ***            ***

                       HYDROGRAPH AT STATION   PMB-RT
                     TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                     6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+    1000.      5.00                  605.        171.         87.          87.
                         (INCHES)    0.457       0.516       0.551        0.551
                          (AC-FT)     300.        338.        361.         361.

                          CUMULATIVE AREA =   12.30 SQ MI

                                     COMPUTED KINEMATIC PARAMETERS
                                           VARIABLE TIME STEP
                                        (DT SHOWN IS A MINIMUM)

                 ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                               PEAK                CELERITY
                                                 (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

                MAIN        1.87      1.47      0.99   1052.33   1000.00    299.31      0.55     20.34

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03  EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

                                        INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

                MAIN        1.87      1.47      3.00            1000.00    300.00      0.55

      ***            ***            ***            ***            ***

                       HYDROGRAPH AT STATION   PMB-RT
                     TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                     6-HR        24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                            (CFS)
+    1000.      5.00                  605.        171.         87.          87.
                         (INCHES)    0.457       0.516       0.551        0.551
                          (AC-FT)     300.        338.        361.         361.

                          CUMULATIVE AREA =   12.30 SQ MI

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                    INTERPOLATED HYDROGRAPH AT   PMB-RT

  PEAK FLOW     TIME                     MAXIMUM AVERAGE FLOW
                                 6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+    1000.      5.00              605.        171.         87.          87.
                        (INCHES)   0.457       0.516       0.551        0.551
                         (AC-FT)    300.        338.        361.         361.

                    CUMULATIVE AREA =   12.30 SQ MI



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           *            *
  286 KK   *      SB10  *        BASIN
           *            *
           **************

              SUBBASIN RUNOFF DATA

  287 BA        SUBBASIN CHARACTERISTICS
                      TAREA        0.04  SUBBASIN AREA

  288 LG        GREEN AND AMPT LOSS RATE
                      STRTL        0.15  STARTING LOSS
                        DTH        0.25  MOISTURE DEFICIT
                       PSIF        4.25  WETTING FRONT SUCTION
                      XKSAT        0.55  HYDRAULIC CONDUCTIVITY
                      RTIMP       55.00  PERCENT IMPERVIOUS AREA

  289 UC        CLARK UNITGRAPH
                         TC        0.23  TIME OF CONCENTRATION
                          R        0.16  STORAGE COEFFICIENT

  290 UA        ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                      0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                    100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.23 HR,       R=  0.16 HR
                                                       SNYDER   TP=  0.14 HR,      CP=  0.51

                                                                   UNIT HYDROGRAPH
                                                              19 END-OF-PERIOD ORDINATES
                 13.       58.       90.       85.       71.       54.       40.       29.       21.       15.
                 11.        8.        6.        4.        3.        2.        2.        1.        1.

       ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     SB10
                   TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.30

  PEAK FLOW     TIME                     MAXIMUM AVERAGE FLOW
                                 6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     170.      0.90               10.          2.          1.           1.
                        (INCHES)   2.287       2.287       2.287        2.287
                         (AC-FT)      5.          5.          5.           5.

                    CUMULATIVE AREA =    0.04 SQ MI


       ***             ***             ***             ***             ***

                     HYDROGRAPH AT STATION     SB10
                   TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.28

  PEAK FLOW     TIME                     MAXIMUM AVERAGE FLOW
                                 6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                          (CFS)
+     169.      0.90               10.          2.          1.           1.
                        (INCHES)   2.272       2.272       2.272        2.272
                         (AC-FT)      5.          5.          5.           5.

                    CUMULATIVE AREA =    0.04 SQ MI


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                         HYDROGRAPH AT STATION     SB10
                       TRANSPOSITION AREA      2.8 SQ MI

        TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.50, TOTAL EXCESS =    2.19

     PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
  +   (CFS)       (HR)
                            (CFS)
  +     142.      0.95                   9.          2.          1.           1.
                         (INCHES)     2.179       2.179       2.179        2.179
                          (AC-FT)        5.          5.          5.           5.

                          CUMULATIVE AREA =    0.04 SQ MI


         ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB10
                       TRANSPOSITION AREA     16.0 SQ MI

        TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.54, TOTAL EXCESS =    2.00

     PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
  +   (CFS)       (HR)
                            (CFS)
  +     118.      0.95                   9.          2.          1.           1.
                         (INCHES)     1.990       1.990       1.990        1.990
                          (AC-FT)        4.          4.          4.           4.

                          CUMULATIVE AREA =    0.04 SQ MI


         ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION     SB10
                       TRANSPOSITION AREA     90.0 SQ MI

        TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.57, TOTAL EXCESS =    1.66

     PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
  +   (CFS)       (HR)
                            (CFS)
  +      88.      0.95                   7.          2.          1.           1.
                         (INCHES)     1.655       1.655       1.655        1.655
                          (AC-FT)        4.          4.          4.           4.

                          CUMULATIVE AREA =    0.04 SQ MI

         ***             ***             ***             ***             ***


                          INTERPOLATED HYDROGRAPH AT     SB10

     PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
  +   (CFS)       (HR)
                            (CFS)
  +     169.      0.90                  10.          2.          1.           1.
                         (INCHES)     2.276       2.276       2.276        2.276
                          (AC-FT)        5.          5.          5.           5.

                          CUMULATIVE AREA =    0.04 SQ MI



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            *            *
   292 KK   *     CP-10  *      COMBINE
            *            *
            **************
                                UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)

   294 HC         HYDROGRAPH COMBINATION
                       ICOMP           3  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                    ***

         ***             ***             ***             ***             ***

                         HYDROGRAPH AT STATION    CP-10
                       TRANSPOSITION AREA      0.0 SQ MI

     PEAK FLOW     TIME                        MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
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+   (CFS)       (HR)
                               (CFS)
+    1355.      6.85                     909.        420.        230.         230.
                            (INCHES)     0.429       0.794       0.903        0.903
                             (AC-FT)      451.        834.        948.         948.

                             CUMULATIVE AREA =   19.69 SQ MI


       ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION    CP-10
                            TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                                 MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1355.      6.85                     909.        420.        230.         230.
                            (INCHES)     0.429       0.794       0.903        0.903
                             (AC-FT)      451.        834.        948.         948.

                             CUMULATIVE AREA =   19.69 SQ MI


       ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION    CP-10
                            TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                                 MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1355.      6.85                     909.        420.        229.         229.
                            (INCHES)     0.429       0.793       0.901        0.901
                             (AC-FT)      451.        833.        946.         946.

                             CUMULATIVE AREA =   19.69 SQ MI


       ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION    CP-10
                            TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                                 MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1354.      6.85                     907.        419.        228.         228.
                            (INCHES)     0.429       0.791       0.897        0.897
                             (AC-FT)      450.        831.        942.         942.

                             CUMULATIVE AREA =   19.69 SQ MI


       ***             ***             ***             ***             ***

                              HYDROGRAPH AT STATION    CP-10
                            TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                                 MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1353.      6.85                     905.        417.        226.         226.
                            (INCHES)     0.427       0.788       0.890        0.890
                             (AC-FT)      449.        828.        935.         935.

                             CUMULATIVE AREA =   19.69 SQ MI

       ***             ***             ***             ***             ***



                             INTERPOLATED HYDROGRAPH AT    CP-10

  PEAK FLOW     TIME                                 MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1354.      6.85                     907.        419.        228.         228.
                            (INCHES)     0.428       0.791       0.896        0.896
                             (AC-FT)      450.        831.        941.         941.

                             CUMULATIVE AREA =   19.69 SQ MI


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             *            *
  295 KK     *    R10-11  *        ROUTE
             *            *
             **************
                                 UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD

                HYDROGRAPH ROUTING DATA

  297 RK          KINEMATIC WAVE STREAM ROUTING
                            L       1277.  CHANNEL LENGTH
                            S      0.0014  SLOPE
                            N       0.030  CHANNEL ROUGHNESS COEFFICIENT
                           CA        0.00  CONTRIBUTING AREA
                        SHAPE        TRAP  CHANNEL SHAPE
                           WD      220.00  BOTTOM WIDTH OR DIAMETER
                            Z        4.00  SIDE SLOPE
                       NDXMIN           2  MINIMUM NUMBER OF DX INTERVALS

                                                                          ***
                                        COMPUTED KINEMATIC PARAMETERS
                                              VARIABLE TIME STEP
                                            (DT SHOWN IS A MINIMUM)

                    ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                CELERITY
                                                    (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

                   MAIN          0.06      1.62       1.62     425.67   1355.03    412.64      0.90      4.66


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9482E+03  EXCESS=0.0000E+00 OUTFLOW=0.9474E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR=
0.0


                                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                   MAIN          0.06      1.62       3.00              1354.77    411.00      0.90

       ***              ***              ***              ***              ***

                              HYDROGRAPH AT STATION   R10-11
                            TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1355.      6.85                    909.        420.        229.         229.
                            (INCHES)   0.429       0.794       0.902        0.902
                             (AC-FT)    451.        834.        947.         947.

                             CUMULATIVE AREA =   19.69 SQ MI

                                        COMPUTED KINEMATIC PARAMETERS
                                              VARIABLE TIME STEP
                                            (DT SHOWN IS A MINIMUM)

                    ELEMENT    ALPHA       M         DT        DX       PEAK    TIME TO     VOLUME    MAXIMUM
                                                                                  PEAK                CELERITY
                                                    (MIN)      (FT)     (CFS)    (MIN)       (IN)      (FPS)

                   MAIN          0.06      1.62       1.56     425.67   1354.98    412.72      0.90      4.66


 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9479E+03  EXCESS=0.0000E+00 OUTFLOW=0.9473E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR=
0.0


                                                   INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL


                   MAIN          0.06      1.62       3.00              1354.72    411.00      0.90

       ***              ***              ***              ***              ***

                              HYDROGRAPH AT STATION   R10-11
                            TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                         MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                               (CFS)
+    1355.      6.85                    909.        420.        229.         229.
                            (INCHES)   0.429       0.794       0.902        0.902
                             (AC-FT)    451.        834.        947.         947.

                             CUMULATIVE AREA =   19.69 SQ MI

                                        COMPUTED KINEMATIC PARAMETERS
                                              VARIABLE TIME STEP
                                            (DT SHOWN IS A MINIMUM)
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                ELEMENT    ALPHA       M         DT         DX        PEAK    TIME TO      VOLUME    MAXIMUM
                                                                               PEAK                  CELERITY
                                                (MIN)      (FT)      (CFS)    (MIN)        (IN)      (FPS)

               MAIN          0.06      1.62       1.57     425.67   1354.58    412.38      0.90      4.66

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9458E+03  EXCESS=0.0000E+00 OUTFLOW=0.9450E+03 BASIN STORAGE=0.9164E+00 PERCENT ERROR=
0.0

                                             INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

               MAIN          0.06      1.62       3.00              1354.33    411.00      0.90

      ***             ***             ***             ***             ***

                   HYDROGRAPH AT STATION   R10-11
                 TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1354.      6.85                 908.        420.        229.         229.
                         (INCHES)   0.429       0.793       0.900        0.900
                          (AC-FT)    450.        833.        945.         945.

                          CUMULATIVE AREA =   19.69 SQ MI

                                          COMPUTED KINEMATIC PARAMETERS
                                                VARIABLE TIME STEP
                                              (DT SHOWN IS A MINIMUM)

                ELEMENT    ALPHA       M         DT         DX        PEAK    TIME TO      VOLUME    MAXIMUM
                                                                               PEAK                  CELERITY
                                                (MIN)      (FT)      (CFS)    (MIN)        (IN)      (FPS)

               MAIN          0.06      1.62       1.67     425.67   1353.83    412.54      0.90      4.66

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9420E+03  EXCESS=0.0000E+00 OUTFLOW=0.9412E+03 BASIN STORAGE=0.9165E+00 PERCENT ERROR=
0.0

                                             INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

               MAIN          0.06      1.62       3.00              1353.59    411.00      0.90

      ***             ***             ***             ***             ***

                   HYDROGRAPH AT STATION   R10-11
                 TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+    1354.      6.85                 907.        419.        228.         228.
                         (INCHES)   0.428       0.791       0.896        0.896
                          (AC-FT)    450.        831.        941.         941.

                          CUMULATIVE AREA =   19.69 SQ MI

                                          COMPUTED KINEMATIC PARAMETERS
                                                VARIABLE TIME STEP
                                              (DT SHOWN IS A MINIMUM)

                ELEMENT    ALPHA       M         DT         DX        PEAK    TIME TO      VOLUME    MAXIMUM
                                                                               PEAK                  CELERITY
                                                (MIN)      (FT)      (CFS)    (MIN)        (IN)      (FPS)

               MAIN          0.06      1.62       1.66     425.67   1352.40    412.19      0.89      4.66

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9349E+03  EXCESS=0.0000E+00 OUTFLOW=0.9341E+03 BASIN STORAGE=0.9161E+00 PERCENT ERROR=
0.0

                                             INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

               MAIN          0.06      1.62       3.00              1352.18    411.00      0.89

      ***             ***             ***             ***             ***

                   HYDROGRAPH AT STATION   R10-11
                 TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                     6-HR       24-HR       72-HR     49.95-HR
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+   (CFS)       (HR)
                                (CFS)
+    1352.      6.85                      904.        417.        226.         226.
                             (INCHES)     0.427       0.788       0.889        0.889
                              (AC-FT)      448.        828.        934.         934.

                              CUMULATIVE AREA =   19.69 SQ MI

       ***             ***             ***             ***             ***



                              INTERPOLATED HYDROGRAPH AT   R10-11

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+    1353.      6.85                      906.        419.        228.         228.
                             (INCHES)     0.428       0.791       0.895        0.895
                              (AC-FT)      449.        831.        940.         940.

                              CUMULATIVE AREA =   19.69 SQ MI



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              *            *
  298 KK      *      SB11  *        BASIN
              *            *
              **************

                 SUBBASIN RUNOFF DATA

  299 BA          SUBBASIN CHARACTERISTICS
                        TAREA        0.07  SUBBASIN AREA

  300 LG          GREEN AND AMPT LOSS RATE
                        STRTL        0.15  STARTING LOSS
                          DTH        0.25  MOISTURE DEFICIT
                         PSIF        4.15  WETTING FRONT SUCTION
                        XKSAT        0.58  HYDRAULIC CONDUCTIVITY
                        RTIMP       55.00  PERCENT IMPERVIOUS AREA

  301 UC          CLARK UNITGRAPH
                           TC        0.29  TIME OF CONCENTRATION
                            R        0.23  STORAGE COEFFICIENT

  302 UA          ACCUMULATED-AREA VS. TIME,  11 ORDINATES
                        0.0       5.0      16.0      30.0      65.0      77.0      84.0      90.0      94.0      97.0
                      100.0

                                                                          ***

                                                              UNIT HYDROGRAPH PARAMETERS
                                                        CLARK   TC=  0.29 HR,       R=  0.23 HR
                                                       SNYDER   TP=  0.19 HR,      CP=  0.50

                                                                   UNIT HYDROGRAPH
                                                              27 END-OF-PERIOD ORDINATES
                 12.       50.       99.      119.      112.      100.       84.       67.       54.       43.
                 35.       28.       23.       18.       15.       12.        9.        8.        6.        5.
                  4.        3.        3.        2.        2.        1.        1.

       ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB11
                             TRANSPOSITION AREA      0.0 SQ MI

     TOTAL RAINFALL =    2.76, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.29

  PEAK FLOW     TIME                           MAXIMUM AVERAGE FLOW
                                          6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                                (CFS)
+     233.      0.95                       17.          4.          2.           2.
                             (INCHES)     2.282       2.282       2.282        2.282
                              (AC-FT)        9.          9.          9.           9.

                              CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                               HYDROGRAPH AT STATION     SB11
                             TRANSPOSITION AREA      0.5 SQ MI

     TOTAL RAINFALL =    2.74, TOTAL LOSS =    0.46, TOTAL EXCESS =    2.28
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  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     231.      0.95                   17.          4.          2.           2.
                        (INCHES)     2.267       2.267       2.267        2.267
                         (AC-FT)        9.          9.          9.           9.

                         CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB11
                        TRANSPOSITION AREA      2.8 SQ MI

     TOTAL RAINFALL =    2.69, TOTAL LOSS =    0.50, TOTAL EXCESS =    2.18

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     201.      1.00                   17.          4.          2.           2.
                        (INCHES)     2.172       2.172       2.172        2.172
                         (AC-FT)        8.          8.          8.           8.

                         CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB11
                        TRANSPOSITION AREA     16.0 SQ MI

     TOTAL RAINFALL =    2.54, TOTAL LOSS =    0.55, TOTAL EXCESS =    1.99

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     169.      1.00                   15.          4.          2.           2.
                        (INCHES)     1.982       1.982       1.982        1.982
                         (AC-FT)        8.          8.          8.           8.

                         CUMULATIVE AREA =    0.07 SQ MI


       ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION     SB11
                        TRANSPOSITION AREA     90.0 SQ MI

     TOTAL RAINFALL =    2.23, TOTAL LOSS =    0.58, TOTAL EXCESS =    1.65

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     129.      1.00                   13.          3.          2.           2.
                        (INCHES)     1.647       1.647       1.647        1.647
                         (AC-FT)        6.          6.          6.           6.

                         CUMULATIVE AREA =    0.07 SQ MI

       ***             ***             ***             ***             ***


                         INTERPOLATED HYDROGRAPH AT     SB11

  PEAK FLOW     TIME                          MAXIMUM AVERAGE FLOW
                                      6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                           (CFS)
+     231.      0.95                   17.          4.          2.           2.
                        (INCHES)     2.270       2.270       2.270        2.270
                         (AC-FT)        9.          9.          9.           9.

                         CUMULATIVE AREA =    0.07 SQ MI



 *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** 
***


             **************
             *            *
  304 KK     *    CP-11   *      COMBINE
             *            *
             **************
                                 UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
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  306 HC          HYDROGRAPH COMBINATION
                        ICOMP           2  NUMBER OF HYDROGRAPHS TO COMBINE

                                                                         ***

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    CP-11
                        TRANSPOSITION AREA      0.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+    1355.      6.85                    909.        420.        232.         232.
                        (INCHES)       0.428       0.791       0.907        0.907
                         (AC-FT)        451.        834.        956.         956.

                         CUMULATIVE AREA =   19.76 SQ MI

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    CP-11
                        TRANSPOSITION AREA      0.5 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+    1355.      6.85                    909.        420.        232.         232.
                        (INCHES)       0.428       0.791       0.907        0.907
                         (AC-FT)        451.        834.        956.         956.

                         CUMULATIVE AREA =   19.76 SQ MI

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    CP-11
                        TRANSPOSITION AREA      2.8 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+    1354.      6.85                    908.        420.        231.         231.
                        (INCHES)       0.427       0.790       0.904        0.904
                         (AC-FT)        450.        833.        953.         953.

                         CUMULATIVE AREA =   19.76 SQ MI

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    CP-11
                        TRANSPOSITION AREA     16.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+    1354.      6.85                    907.        419.        230.         230.
                        (INCHES)       0.427       0.789       0.900        0.900
                         (AC-FT)        450.        831.        949.         949.

                         CUMULATIVE AREA =   19.76 SQ MI

      ***             ***             ***             ***             ***

                          HYDROGRAPH AT STATION    CP-11
                        TRANSPOSITION AREA     90.0 SQ MI

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
+    1352.      6.85                    904.        417.        228.         228.
                        (INCHES)       0.425       0.785       0.892        0.892
                         (AC-FT)        448.        828.        940.         940.

                         CUMULATIVE AREA =   19.76 SQ MI

      ***             ***             ***             ***             ***


                         INTERPOLATED HYDROGRAPH AT    CP-11

  PEAK FLOW     TIME                             MAXIMUM AVERAGE FLOW
                                        6-HR       24-HR       72-HR     49.95-HR
+   (CFS)       (HR)
                             (CFS)
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+    1353.      6.85                  906.        419.        230.        230.
                        (INCHES)     0.426       0.788       0.899       0.899
                         (AC-FT)      449.        831.        948.        948.

                         CUMULATIVE AREA =   19.76 SQ MI
```

1

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+	HYDROGRAPH AT	76THST	51.	4.55	15.	4.	2.	0.24		
+	HYDROGRAPH AT	SB01	324.	0.95	26.	6.	3.	0.11		
+	2 COMBINED AT	CP-1	323.	0.95	37.	11.	5.	0.35		
+	ROUTED TO	R1-2	315.	1.10	37.	11.	5.	0.35		
+	HYDROGRAPH AT	SB02	489.	0.95	38.	10.	5.	0.16		
+	2 COMBINED AT	CP-2	731.	1.05	75.	20.	10.	0.51		
+	ROUTED TO	R2-3	719.	1.10	75.	20.	10.	0.51		
+	HYDROGRAPH AT	SB03	129.	1.00	12.	3.	1.	0.05		
+	2 COMBINED AT	CP-3	828.	1.10	86.	23.	11.	0.56		
+	ROUTED TO	R3-5	806.	1.20	86.	23.	11.	0.56		
+	HYDROGRAPH AT	SB04	525.	0.95	36.	9.	4.	0.14		
+	ROUTED TO	R4-5	511.	1.05	37.	9.	4.	0.14		
+	HYDROGRAPH AT	SB05	399.	0.95	30.	7.	4.	0.13		
+	3 COMBINED AT	CP-5	1410.	1.15	150.	40.	19.	0.83		
+	ROUTED TO	R5-6	1386.	1.20	150.	40.	19.	0.83		
+	HYDROGRAPH AT	SB06	420.	0.95	33.	8.	4.	0.14		
+	2 COMBINED AT	CP-6	1609.	1.20	181.	48.	23.	0.96		
+	HYDROGRAPH AT	SB07	384.	0.90	17.	4.	2.	0.07		
+	ROUTED TO	R7-8	360.	1.05	18.	5.	2.	0.07		
+	HYDROGRAPH AT	SB08	792.	0.95	56.	14.	7.	0.22		
+	2 COMBINED AT	CP-8	1035.	1.05	74.	18.	9.	0.30		
+	ROUTED TO	R8-9	1010.	1.10	74.	19.	9.	0.30		
+	HYDROGRAPH AT	SB09	173.	0.95	13.	3.	2.	0.05		
+	2 COMBINED AT	CP-9	1124.	1.10	87.	22.	10.	0.35		
+	HYDROGRAPH AT	PIMACH	4017.	5.50	1092.	292.	146.	12.30		
+	DIVERSION TO	PIMABY	1000.	4.95	605.	171.	88.	12.30		
	HYDROGRAPH AT									

+		PIMAIN	3017.	5.50	486.	122.	58.	12.30
+	HYDROGRAPH AT	PWRCH	740.	5.80	432.	136.	75.	7.00
+	3 COMBINED AT	BINFLO	3634.	5.50	919.	273.	142.	19.65
+	ROUTED TO	BASIN	362.	8.30	353.	249.	140.	19.65
+	ROUTED TO	BSNRT1	362.	8.35	353.	249.	140.	19.65
+	ROUTED TO	BSNRT2	362.	8.35	353.	249.	139.	19.65
+	HYDROGRAPH AT	PIMABY	1000.	4.95	605.	171.	88.	12.30
+	ROUTED TO	PMB-RT	1000.	5.00	605.	171.	87.	12.30
+	HYDROGRAPH AT	SB10	169.	0.90	10.	2.	1.	0.04
+	3 COMBINED AT	CP-10	1354.	6.85	907.	419.	228.	19.69
+	ROUTED TO	R10-11	1353.	6.85	906.	419.	228.	19.69
+	HYDROGRAPH AT	SB11	231.	0.95	17.	4.	2.	0.07
+	2 COMBINED AT	CP-11	1353.	6.85	906.	419.	230.	19.76

1

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING

(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

						INTERPOLATED TO COMPUTATION INTERVAL			
ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R1-2	MANE	3.00	324.42	63.63	1.19	3.00	317.42	66.00	1.19

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2221E+02 EXCESS=0.0000E+00 OUTFLOW=0.2234E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR= -0.8

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R1-2	MANE	3.00	321.94	63.70	1.18	3.00	314.95	66.00	1.18

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2212E+02 EXCESS=0.0000E+00 OUTFLOW=0.2223E+02 BASIN STORAGE=0.5936E-01 PERCENT ERROR= -0.8

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R1-2	MANE	3.00	281.88	67.46	1.15	3.00	280.31	66.00	1.15

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2153E+02 EXCESS=0.0000E+00 OUTFLOW=0.2160E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR=

-0.6

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R1-2	MANE	3.00	239.43	66.99	1.09	3.00	234.05	69.00	1.09

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2036E+02 EXCESS=0.0000E+00 OUTFLOW=0.2044E+02 BASIN STORAGE=0.5560E-01 PERCENT ERROR= -0.7

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R1-2	MANE	2.91	179.26	70.19	0.98	3.00	178.38	69.00	0.98

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1834E+02 EXCESS=0.0000E+00 OUTFLOW=0.1842E+02 BASIN STORAGE=0.6416E-01 PERCENT ERROR= -0.8

FOR STORM = 1 STORM AREA (SQ MI) = 0.00

R2-3	MANE	1.61	726.00	65.69	1.53	3.00	725.57	66.00	1.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4152E+02 EXCESS=0.0000E+00 OUTFLOW=0.4152E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= -0.1

FOR STORM = 2 STORM AREA (SQ MI) = 0.50

R2-3	MANE	1.61	719.73	65.75	1.52	3.00	719.25	66.00	1.52

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4128E+02 EXCESS=0.0000E+00 OUTFLOW=0.4128E+02 BASIN STORAGE=0.4197E-01 PERCENT ERROR= -0.1

FOR STORM = 3 STORM AREA (SQ MI) = 2.80

R2-3	MANE	1.65	650.44	66.16	1.47	3.00	647.31	66.00	1.47

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3985E+02 EXCESS=0.0000E+00 OUTFLOW=0.3986E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= -0.1

FOR STORM = 4 STORM AREA (SQ MI) = 16.00

R2-3	MANE	1.61	553.64	68.36	1.37	3.00	550.82	69.00	1.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3711E+02 EXCESS=0.0000E+00 OUTFLOW=0.3710E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR= -0.1

FOR STORM = 5 STORM AREA (SQ MI) = 90.00

R2-3	MANE	1.77	421.93	69.46	1.19	3.00	418.11	69.00	1.19

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3232E+02 EXCESS=0.0000E+00 OUTFLOW=0.3234E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=
-0.2

FOR STORM = 1 STORM AREA (SQ MI) =   0.00

R3-5	MANE	2.78	817.36	73.14	1.60	3.00	816.94	72.00	1.60

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4741E+02 EXCESS=0.0000E+00 OUTFLOW=0.4763E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.6

FOR STORM = 2 STORM AREA (SQ MI) =   0.50

R3-5	MANE	2.79	810.60	70.45	1.59	3.00	810.54	72.00	1.59

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4713E+02 EXCESS=0.0000E+00 OUTFLOW=0.4736E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.7

FOR STORM = 3 STORM AREA (SQ MI) =   2.80

R3-5	MANE	2.90	742.52	73.90	1.54	3.00	740.16	72.00	1.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4551E+02 EXCESS=0.0000E+00 OUTFLOW=0.4571E+02 BASIN STORAGE=0.8226E-01 PERCENT ERROR=
-0.6

FOR STORM = 4 STORM AREA (SQ MI) =   16.00

R3-5	MANE	3.00	634.55	73.70	1.43	3.00	626.49	75.00	1.43

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4224E+02 EXCESS=0.0000E+00 OUTFLOW=0.4240E+02 BASIN STORAGE=0.8222E-01 PERCENT ERROR=
-0.6

FOR STORM = 5 STORM AREA (SQ MI) =   90.00

R3-5	MANE	3.00	478.66	74.64	1.24	3.00	478.05	75.00	1.24

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3665E+02 EXCESS=0.0000E+00 OUTFLOW=0.3688E+02 BASIN STORAGE=0.8233E-01 PERCENT ERROR=
-0.8

FOR STORM = 1 STORM AREA (SQ MI) =   0.00

R4-5	MANE	2.41	520.58	62.34	2.39	3.00	513.38	63.00	2.40

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1819E+02 EXCESS=0.0000E+00 OUTFLOW=0.1838E+02 BASIN STORAGE=0.3179E-04 PERCENT ERROR=
-1.0

FOR STORM = 2 STORM AREA (SQ MI) =   0.50

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          R4-5  MANE        2.42    516.56     62.40      2.38      3.00    510.06     63.00        2.39



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1807E+02 EXCESS=0.0000E+00 OUTFLOW=0.1825E+02 BASIN STORAGE=0.4744E-04 PERCENT ERROR=
-1.0


      FOR STORM = 3  STORM AREA (SQ MI) =       2.80
          R4-5  MANE        2.55    451.64     62.77      2.29      3.00    451.16     63.00        2.30



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1745E+02 EXCESS=0.0000E+00 OUTFLOW=0.1762E+02 BASIN STORAGE=0.4583E-04 PERCENT ERROR=
-1.0


      FOR STORM = 4  STORM AREA (SQ MI) =      16.00
          R4-5  MANE        2.73    379.50     64.90      2.12      3.00    374.76     63.00        2.12



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1609E+02 EXCESS=0.0000E+00 OUTFLOW=0.1629E+02 BASIN STORAGE=0.4132E-04 PERCENT ERROR=
-1.2


      FOR STORM = 5  STORM AREA (SQ MI) =      90.00
          R4-5  MANE        2.87    291.80     65.24      1.78      3.00    288.81     66.00        1.78



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1354E+02 EXCESS=0.0000E+00 OUTFLOW=0.1370E+02 BASIN STORAGE=0.4337E-04 PERCENT ERROR=
-1.2


      FOR STORM = 1  STORM AREA (SQ MI) =       0.00
          R5-6  MANE        1.71   1430.15     73.26      1.84      3.00   1428.13     72.00        1.84



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8096E+02 EXCESS=0.0000E+00 OUTFLOW=0.8101E+02 BASIN STORAGE=0.7278E-01 PERCENT ERROR=
-0.1


      FOR STORM = 2  STORM AREA (SQ MI) =       0.50
          R5-6  MANE        1.72   1417.75     71.63      1.82      3.00   1417.68     72.00        1.82



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8047E+02 EXCESS=0.0000E+00 OUTFLOW=0.8049E+02 BASIN STORAGE=0.7282E-01 PERCENT ERROR=
-0.1


      FOR STORM = 3  STORM AREA (SQ MI) =       2.80
          R5-6  MANE        1.83   1320.58     72.30      1.76      3.00   1310.29     72.00        1.76



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7757E+02 EXCESS=0.0000E+00 OUTFLOW=0.7762E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR=
-0.2
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          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
              R5-6  MANE         1.92   1120.07     73.97      1.62      3.00   1117.54     75.00        1.62



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7160E+02 EXCESS=0.0000E+00 OUTFLOW=0.7164E+02 BASIN STORAGE=0.7274E-01 PERCENT ERROR=
-0.2



          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
              R5-6  MANE         2.16    837.38     77.35      1.39      3.00    834.70     75.00        1.39



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6127E+02 EXCESS=0.0000E+00 OUTFLOW=0.6143E+02 BASIN STORAGE=0.6818E-01 PERCENT ERROR=
-0.4



          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R7-8  MANE         2.79    374.54     61.97      2.32      3.00    360.59     63.00        2.31



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8717E+01 EXCESS=0.0000E+00 OUTFLOW=0.9160E+01 BASIN STORAGE=0.1297E-03 PERCENT ERROR=
-5.1



          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R7-8  MANE         2.80    373.06     62.02      2.31      3.00    359.23     63.00        2.30



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8655E+01 EXCESS=0.0000E+00 OUTFLOW=0.9098E+01 BASIN STORAGE=0.1282E-03 PERCENT ERROR=
-5.1



          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R7-8  MANE         3.00    300.60     62.65      2.18      3.00    299.77     63.00        2.18



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8267E+01 EXCESS=0.0000E+00 OUTFLOW=0.8591E+01 BASIN STORAGE=0.1511E-03 PERCENT ERROR=
-3.9



          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
              R7-8  MANE         3.00    243.19     65.55      1.99      3.00    238.95     66.00        2.00



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7502E+01 EXCESS=0.0000E+00 OUTFLOW=0.7866E+01 BASIN STORAGE=0.1118E-03 PERCENT ERROR=
-4.8



          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
              R7-8  MANE         2.82    178.83     67.24      1.61      3.00    176.29     66.00        1.61
```

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6168E+01 EXCESS=0.0000E+00 OUTFLOW=0.6362E+01 BASIN STORAGE=0.1260E-03 PERCENT ERROR=
-3.1

FOR STORM = 1 STORM AREA (SQ MI) =   0.00

R8-9	MANE	1.32	1031.83	65.57	2.33	3.00	1013.13	66.00	2.33

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3691E+02 EXCESS=0.0000E+00 OUTFLOW=0.3702E+02 BASIN STORAGE=0.1702E-03 PERCENT ERROR=
-0.3

FOR STORM = 2 STORM AREA (SQ MI) =   0.50

R8-9	MANE	1.33	1027.12	65.64	2.31	3.00	1010.29	66.00	2.32

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3666E+02 EXCESS=0.0000E+00 OUTFLOW=0.3678E+02 BASIN STORAGE=0.2024E-03 PERCENT ERROR=
-0.3

FOR STORM = 3 STORM AREA (SQ MI) =   2.80

R8-9	MANE	1.45	913.69	66.38	2.21	3.00	911.34	66.00	2.21

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3510E+02 EXCESS=0.0000E+00 OUTFLOW=0.3519E+02 BASIN STORAGE=0.1735E-03 PERCENT ERROR=
-0.3

FOR STORM = 4 STORM AREA (SQ MI) =   16.00

R8-9	MANE	1.48	772.14	65.64	2.03	3.00	769.23	66.00	2.03

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3216E+02 EXCESS=0.0000E+00 OUTFLOW=0.3228E+02 BASIN STORAGE=0.1954E-03 PERCENT ERROR=
-0.4

FOR STORM = 5 STORM AREA (SQ MI) =   90.00

R8-9	MANE	1.59	554.15	67.90	1.68	3.00	554.05	69.00	1.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2666E+02 EXCESS=0.0000E+00 OUTFLOW=0.2676E+02 BASIN STORAGE=0.1898E-03 PERCENT ERROR=
-0.4

FOR STORM = 1 STORM AREA (SQ MI) =   0.00

BSNRT1	MANE	0.29	363.48	498.70	0.56	3.00	363.48	501.00	0.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03 EXCESS=0.0000E+00 OUTFLOW=0.5822E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR=
0.0

FOR STORM = 2 STORM AREA (SQ MI) =   0.50

BSNRT1	MANE	0.29	363.42	498.84	0.56	3.00	363.42	501.00	0.56

```
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03 EXCESS=0.0000E+00 OUTFLOW=0.5819E+03 BASIN STORAGE=0.3289E-01 PERCENT ERROR=
0.0

           FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             BSNRT1  MANE         0.45    363.06    498.87      0.55      3.00    363.06    501.00        0.55

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5801E+03 EXCESS=0.0000E+00 OUTFLOW=0.5800E+03 BASIN STORAGE=0.3288E-01 PERCENT ERROR=
0.0

           FOR STORM = 4  STORM AREA (SQ MI) =      16.00
             BSNRT1  MANE         0.38    362.38    500.88      0.55      3.00    362.38    501.00        0.55

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5767E+03 EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3287E-01 PERCENT ERROR=
0.0

           FOR STORM = 5  STORM AREA (SQ MI) =      90.00
             BSNRT1  MANE         0.34    361.10    501.33      0.54      3.00    361.10    501.00        0.54

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03 EXCESS=0.0000E+00 OUTFLOW=0.5702E+03 BASIN STORAGE=0.3284E-01 PERCENT ERROR=
0.0

           FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             BSNRT2  MANE         0.44    363.48    501.11      0.56      3.00    363.48    501.00        0.56

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+03 EXCESS=0.0000E+00 OUTFLOW=0.5821E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR=
0.0

           FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             BSNRT2  MANE         0.44    363.42    501.35      0.56      3.00    363.42    501.00        0.56

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5819E+03 EXCESS=0.0000E+00 OUTFLOW=0.5818E+03 BASIN STORAGE=0.3756E-01 PERCENT ERROR=
0.0

           FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             BSNRT2  MANE         0.36    363.06    501.42      0.55      3.00    363.06    501.00        0.55

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5800E+03 EXCESS=0.0000E+00 OUTFLOW=0.5799E+03 BASIN STORAGE=0.3755E-01 PERCENT ERROR=
0.0
```

```
          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            BSNRT2  MANE        0.48    362.38    501.44      0.55      3.00    362.38    501.00       0.55



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5766E+03 EXCESS=0.0000E+00 OUTFLOW=0.5766E+03 BASIN STORAGE=0.3753E-01 PERCENT ERROR=
0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            BSNRT2  MANE        0.38    361.10    501.56      0.54      3.00    361.09    501.00       0.54



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5702E+03 EXCESS=0.0000E+00 OUTFLOW=0.5701E+03 BASIN STORAGE=0.3750E-01 PERCENT ERROR=
0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            PMB-RT  MANE        0.99   1000.00    299.31      0.55      3.00   1000.00    300.00       0.55



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            PMB-RT  MANE        0.99   1000.00    299.31      0.55      3.00   1000.00    300.00       0.55



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            PMB-RT  MANE        0.99   1000.00    299.31      0.55      3.00   1000.00    300.00       0.55



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            PMB-RT  MANE        0.99   1000.00    299.31      0.55      3.00   1000.00    300.00       0.55



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            PMB-RT  MANE        0.99   1000.00    299.31      0.55      3.00   1000.00    300.00       0.55



 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3615E+03 EXCESS=0.0000E+00 OUTFLOW=0.3612E+03 BASIN STORAGE=0.1613E+00 PERCENT ERROR=
```

0.0

FOR STORM = 1 STORM AREA (SQ MI) =   0.00

R10-11	MANE	1.62	1355.03	412.64	0.90	3.00	1354.77	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9482E+03 EXCESS=0.0000E+00 OUTFLOW=0.9474E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR=
0.0

FOR STORM = 2 STORM AREA (SQ MI) =   0.50

R10-11	MANE	1.56	1354.98	412.72	0.90	3.00	1354.72	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9479E+03 EXCESS=0.0000E+00 OUTFLOW=0.9473E+03 BASIN STORAGE=0.9166E+00 PERCENT ERROR=
0.0

FOR STORM = 3 STORM AREA (SQ MI) =   2.80

R10-11	MANE	1.57	1354.58	412.38	0.90	3.00	1354.33	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9458E+03 EXCESS=0.0000E+00 OUTFLOW=0.9450E+03 BASIN STORAGE=0.9164E+00 PERCENT ERROR=
0.0

FOR STORM = 4 STORM AREA (SQ MI) =   16.00

R10-11	MANE	1.67	1353.83	412.54	0.90	3.00	1353.59	411.00	0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9420E+03 EXCESS=0.0000E+00 OUTFLOW=0.9412E+03 BASIN STORAGE=0.9165E+00 PERCENT ERROR=
0.0

FOR STORM = 5 STORM AREA (SQ MI) =   90.00

R10-11	MANE	1.66	1352.40	412.19	0.89	3.00	1352.18	411.00	0.89

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9349E+03 EXCESS=0.0000E+00 OUTFLOW=0.9341E+03 BASIN STORAGE=0.9161E+00 PERCENT ERROR=
0.0

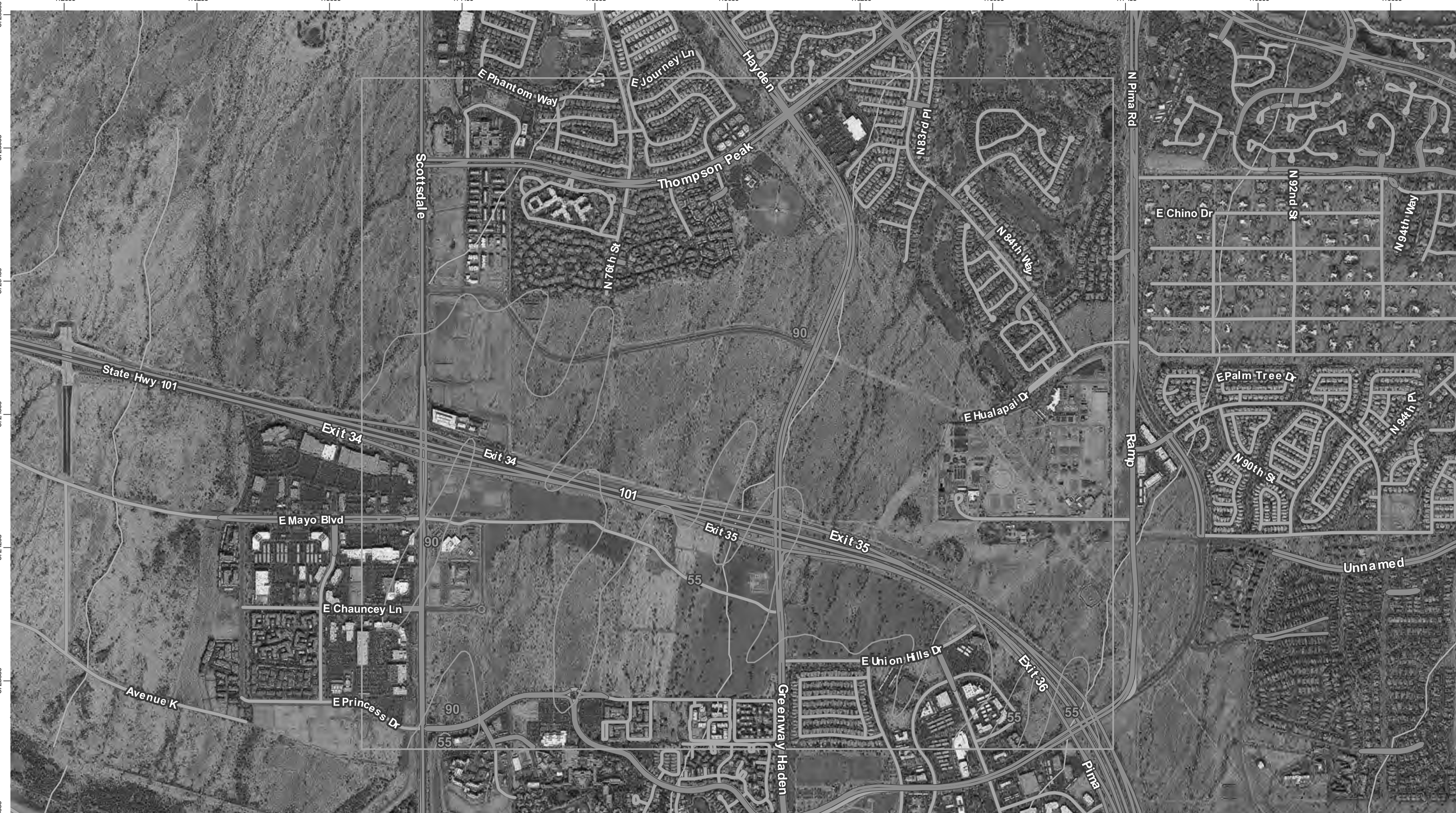
*** NORMAL END OF HEC-1 ***

Appendix C
Cavasson HEC-1 Data
Grayhawk Residences at Cavasson

Flood Control District of Maricopa County
Drainage Design Management System
RAINFALL DATA
Project Reference: 17114 - DATA

ID	Method	Duration	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
DEFAULT	NOAA14	5 MIN	0.258	0.349	0.418	0.511	0.583	0.656
	NOAA14	10 MIN	0.393	0.530	0.636	0.778	0.887	0.998
	NOAA14	15 MIN	0.487	0.658	0.789	0.965	1.099	1.237
	NOAA14	30 MIN	0.656	0.886	1.063	1.299	1.480	1.666
	NOAA14	1 HOUR	0.812	1.096	1.315	1.608	1.832	2.062
	NOAA14	2 HOUR	0.939	1.251	1.490	1.814	2.060	2.315
	NOAA14	3 HOUR	1.024	1.339	1.588	1.936	2.212	2.498
	NOAA14	6 HOUR	1.215	1.549	1.817	2.182	2.465	2.760
	NOAA14	12 HOUR	1.374	1.734	2.016	2.399	2.690	2.993
	NOAA14	24 HOUR	1.622	2.097	2.476	3.005	3.425	3.865

Soil Map—Aguila-Carefree Area, Arizona, Parts of Maricopa and Pinal Counties



Map Scale: 1:17,700 if printed on B landscape (17" x 11") sheet.

N

0 250 500 1000 1500 Meters

0 500 1000 2000 3000 Feet

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 12N WGS84

MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons

Soil Map Unit Lines

Soil Map Unit Points

Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

Spoil Area

Stony Spot

Very Stony Spot

Wet Spot

Other

Special Line Features

Water Features

Streams and Canals

Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Aguila-Carefree Area, Arizona, Parts of Maricopa and Pinal Counties
Survey Area Data: Version 11, Sep 11, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Oct 31, 2014—Dec 7, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
55	Gilman loams	768.3	30.2%
90	Momoli gravelly sandy loam, 1 to 5 percent slopes	1,778.8	69.8%
Totals for Area of Interest		**2,547.2**	**100.0%**

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 8/14/2020

The following spreadsheets were used to prepare input data for HEC-1 Green and Ampt Method described in The Drainage Design Manual for Maricopa County

Spreadsheet equations and assumptions were as follows:

SHEET 1: *Summary* — Summary of HEC-1 Input.

SHEET 2: *Basin Geometry* — Drainage areas, watercourse length, slope, adjusted slope, land type, and K_b.

Slopes were adjusted according to Figure 5.4.

Land type and K_b were determined using Table 5.1.

SHEET 3: *Green-Ampt Parameters* — Soil characteristics, XKSAT, PSIF, DTHETA, and RTEMP.

Soil type and percentages determined from Flood Control soils map.

XKSAT determined using Appendix A. If applicable a logrithmic weighted value was determined for multiple soils within a basin according to the following equation:

$$\overline{\text{XSAT}} = \text{ALOG}\sum\left[\%\,\text{SOIL TYPE(IN DECIMAL)} \cdot \text{LOG(XSAT)}\right]$$

Where: XSAT det er min ed from Appendix A

PSIF and DTHETA determined using Figure 4.3.

RTEMP is a weighted value based on soil data. The following equation was used to determine the weighted value:

$$\text{RTEMP} = \sum \%\,\text{SOIL TYPE} \cdot \left[\%\text{ROCK OUTCROP(IN DECIMAL)}\right]$$

These values may be used for predeveloped conditions.

SHEET 4: *Land Use Parameters* — Land use categories, IA, and RTEMP. IA and RTEMP are weighted based on percentage of land use. These values may be used for post developement conditions.

SHEET 5: Retention Required — This is the Proposed Condition Retention that will be required for each SubBasin. It is calculated as the FIRST FLUSH per the Development Agreement (Construction of Offsite Powerline Channel)
The Formula used is from COS: Vol = Cw*(P/12)*Area
In HEC-1 this will be input as a Divert and not brought back
The Divert will be labeled: DIVXXX, where XXX is the Basin ID

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

	SEE SHEET _ FOR DETERMINATION OF THE FOLLOWING INPUT								
Sheet # →	2	2	4	2	4	3	3	3	4
BASIN NUMBER	BASIN AREA (Miles^2)	WATERCOURSE LENGTH (Miles)	Kb WEIGHTED	SLOPE ADJUSTED (Ft/Mile)	IA WEIGHTED	(in)	PSIF (in)	$\overline{XKSAT}$ (in/hr)	GREATER OF RTEMP SOIL OR WEIGHTED
SB01-C	0.0270	0.3286	0.032	96	0.25	0.35	4.3	0.5778	45%
SB01-B	0.0240	0.2750	0.033	115	0.25	0.35	4.3	0.5778	45%
SB01-A	0.0610	0.2750	0.030	121	0.25	0.35	4.3	0.5778	45%
SB02NE	0.0335	0.3873	0.032	72	0.25	0.35	4.3	0.5778	45%
SB02NW	0.0193	0.1919	0.033	86	0.25	0.35	4.3	0.5778	45%
SB02W	0.0571	0.3483	0.111	88	0.35	0.35	4.3	0.5111	0%
SB02E	0.0440	0.3557	0.031	82	0.10	0.35	4.3	0.6222	80%
SB8NW	0.0147	0.1445	0.034	83	0.10	0.35	4.3	0.6222	80%
SB8NE	0.0150	0.1405	0.034	85	0.10	0.35	4.3	0.6222	80%
SB8SW	0.0129	0.1383	0.034	116	0.10	0.35	4.3	0.6222	80%
SB8SE	0.0131	0.1580	0.034	104	0.10	0.35	4.3	0.6222	80%
SB08W	0.0589	0.3428	0.030	64	0.10	0.35	4.3	0.6222	80%
SB07W	0.0468	0.3138	0.031	99	0.25	0.35	4.3	0.5778	45%

SUM= 0.4272

Basin Geometry

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

						BASIN ELEVATION			SLOPE		
BASIN	BASIN AREA			WATER COURSE LENGTH			Low	Δ Elevation			ADJUSTED
NUMBER	(Feet^2)	(ACRES)	(Miles^2)	(Feet)	(Miles)	(Feet)	(Feet)	(Feet)	(Ft/Ft)	(Ft/Mile)	(Ft/Mile)
SB01-C	752,914	17.28	0.0270	1735	0.329	1681.0	1649.3	31.7	0.0183	96.47	96.5
SB01-B	668,111	15.34	0.0240	1452	0.275	1668.5	1637.0	31.5	0.0217	114.55	114.5
SB01-A	1,700,557	39.04	0.0610	1452	0.275	1659.3	1626.0	33.3	0.0229	121.09	121.1
SB02NE	934,725	21.46	0.0335	2045	0.387	1652.0	1624.0	28.0	0.0137	72.29	72.3
SB02NW	537,015	12.33	0.0193	1013	0.192	1640.5	1624.0	16.5	0.0163	86.00	86.0
SB02W	1,592,151	36.55	0.0571	1839	0.348	1634.5	1604.0	30.5	0.0166	87.57	87.6
SB02E	1,227,661	28.18	0.0440	1878	0.356	1633.2	1604.0	29.2	0.0155	82.10	82.1
SB8NW	408,919	9.39	0.0147	763	0.145	1654.0	1642.0	12.0	0.0157	83.04	83.0
SB8NE	417,750	9.59	0.0150	742	0.141	1654.0	1642.0	12.0	0.0162	85.39	85.4
SB8SW	358,807	8.24	0.0129	730	0.138	1642.0	1626.0	16.0	0.0219	115.73	115.7
SB8SE	365,480	8.39	0.0131	834	0.158	1642.5	1626.0	16.5	0.0198	104.46	104.5
SB08W	1,641,602	37.69	0.0589	1810	0.343	1634.0	1612.0	22.0	0.0122	64.18	64.2
SB07W	1,303,601	29.93	0.0468	1657	0.314	1685.0	1654.0	31.0	0.0187	98.78	98.8
SUM=	11,909,293	273	0.4272								

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA (Miles^2)	SOIL TYPE	% OF SOIL TYPE	ROCK OUTCROP % OF SOIL IMPERVIOUS	XKSAT (in/hr)	XKSAT ADJUSTED FOR 60% VEGATATION*	$\overline{XKSAT}$ (in/hr)	PSIF (in)	DTHETA (in)	RTEMP SOIL IN SUBBASIN
SB01-C	0.0270	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB01-B	0.0240	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB01-A	0.0610	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB02NE	0.0335	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB02NW	0.0193	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%
SB02W	0.0571	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02E	0.0440	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8NW	0.0147	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8NE	0.0150	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8SW	0.0129	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB8SE	0.0131	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB08W	0.0589	90	100%	0%	0.4	0.622	0.622	4.3	0.35	0%
SB07W	0.0468	90	100%	0%	0.4	0.578	0.578	4.3	0.35	0%

*60% vegetation was used for C2 land uses, MFR land uses used 50% and NDR used 35% vegetation

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA ACRES	LAND USE	% OF BASIN	SUB AREA ACRES	LAND TYPE	K_b	K_b WEIGHTED	IA FOR LAND USE	IA WEIGHTED	RTEMP FOR SOIL IN BASIN	RTEMP FOR LAND USE	RTEMP WEIGHTED	RTEMP MAX
SB01-C	17.28	MFR	100%	17.28	A	0.0323	0.0323	0.25	0.25	0%	45%	45%	45%
SB01-B	15.34	MFR	100%	15.34	A	0.0326	0.0326	0.25	0.25	0%	45%	45%	45%
SB01-A	39.04	MFR	100%	39.04	A	0.0301	0.0301	0.25	0.25	0%	45%	45%	45%
SB02NE	21.46	MFR	100%	21.46	A	0.0317	0.0317	0.25	0.25	0%	45%	45%	45%
SB02NW	12.33	MFR	100%	12.33	A	0.0332	0.0332	0.25	0.25	0%	45%	45%	45%
SB02W	36.55	NDR	100%	36.55	C	0.1109	0.1109	0.35	0.35	0%	0%	0%	0%
SB02E	28.18	C2	100%	28.18	A	0.0309	0.0309	0.1	0.10	0%	80%	80%	80%
SB8NW	9.39	C2	100%	9.39	A	0.0339	0.0339	0.1	0.10	0%	80%	80%	80%
SB8NE	9.59	C2	100%	9.59	A	0.0339	0.0339	0.1	0.10	0%	80%	80%	80%
SB8SW	8.24	C2	100%	8.24	A	0.0343	0.0343	0.1	0.10	0%	80%	80%	80%
SB8SE	8.39	C2	100%	8.39	A	0.0342	0.0342	0.1	0.10	0%	80%	80%	80%
SB08W	37.69	C2	100%	37.69	A	0.0301	0.0301	0.1	0.10	0%	80%	80%	80%
SB07W	29.93	MFR	100%	29.93	A	0.0308	0.0308	0.25	0.25	0%	45%	45%	45%
SUM=	273			273									

Retention Required

PROJECT NAME: Cavasson Master Plan Phase 3 Ultimate
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET

BASIN	BASIN AREA			Weighted C	Volume Required	
NUMBER	(Feet^2)	(ACRES)	(Miles^2)	Proposed Cond.	[acre-ft]	[ft^3]
SB01-C	752,914	17.28	0.0270	0.85	0.612	26,665.7
SB01-B	668,111	15.34	0.0240	0.85	0.543	23,662.3
SB01-A	1,700,557	39.04	0.0610	0.85	1.383	60,228.1
SB02NE	934,725	21.46	0.0335	0.85	0.760	33,104.9
SB02NW	537,015	12.33	0.0193	0.85	0.437	19,019.3
SB02W	1,592,151	36.55	0.0571	0.85	1.295	56,388.7
SB02E	1,227,661	28.18	0.0440	0.85	0.998	43,479.6
SB8NW	408,919	9.39	0.0147	0.85	0.332	14,482.5
SB8NE	417,750	9.59	0.0150	0.85	0.340	14,795.3
SB8SW	358,807	8.24	0.0129	0.85	0.292	12,707.8
SB8SE	365,480	8.39	0.0131	0.85	0.297	12,944.1
SB08W	1,641,602	37.69	0.0589	0.85	1.335	58,140.1
SB07W	1,303,601	29.93	0.0468	0.85	1.060	46,169.2
SUM=	11,909,293	273	0.43		9.683	421,787.5

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 8/14/2020

The following spreadsheets were used to prepare input data for HEC-1 Green and Ampt Method described in The Drainage Design Manual for Maricopa County

Spreadsheet equations and assumptions were as follows:

SHEET 1: *Summary* — Summary of HEC-1 Input.

SHEET 2: *Basin Geometry* — Drainage areas, watercourse length, slope, adjusted slope, land type, and K_b.

Slopes were adjusted according to Figure 5.4.

Land type and K_b were determined using Table 5.1.

SHEET 3: *Green-Ampt Parameters* — Soil characteristics, XKSAT, PSIF, DTHETA, and RTEMP.

Soil type and percentages determined from Flood Control soils map.

XKSAT determined using Appendix A. If applicable a logrithmic weighted value was determined for multiple soils within a basin according to the following equation:

$$\overline{\text{XSAT}} = \text{ALOG}\sum\left[\%\ \text{SOIL TYPE(IN DECIMAL)} \cdot \text{LOG(XSAT)}\right]$$

Where: XSAT det er min ed from Appendix A

PSIF and DTHETA determined using Figure 4.3.

RTEMP is a weighted value based on soil data. The following equation was used to determine the weighted value:

$$\text{RTEMP} = \sum \%\ \text{SOIL TYPE} \cdot \left[\%\text{ROCK OUTCROP(IN DECIMAL)}\right]$$

These values may be used for predeveloped conditions.

SHEET 4: *Land Use Parameters* — Land use categories, IA, and RTEMP. IA and RTEMP are weighted based on percentage of land use. These values may be used for post developement conditions.

SHEET 5: Retention Required — This is the Proposed Condition Retention that will be required for each SubBasin. It is calculated as the FIRST FLUSH per the Development Agreement (Construction of Offsite Powerline Channel)
The Formula used is from COS: Vol = Cw*(P/12)*Area
In HEC-1 this will be input as a Divert and not brought back
The Divert will be labeled: DIVXXX, where XXX is the Basin ID

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

	SEE SHEET _ FOR DETERMINATION OF THE FOLLOWING INPUT								
Sheet # →	2	2	4	2	4	3	3	3	4
BASIN NUMBER	BASIN AREA (Miles^2)	WATERCOURSE LENGTH (Miles)	Kb WEIGHTED	SLOPE ADJUSTED (Ft/Mile)	IA WEIGHTED	DTHETA (in)	PSIF (in)	$\overline{XKSAT}$ (in/hr)	GREATER OF RTEMP SOIL OR WEIGHTED
SB01-C	0.0270	0.3286	0.119	96	0.35	0.35	4.3	0.5111	0%
SB01-B	0.0240	0.2750	0.120	115	0.35	0.35	4.3	0.5111	0%
SB01-A	0.0610	0.2750	0.110	121	0.35	0.35	4.3	0.5111	0%
SB02NE	0.0335	0.3873	0.117	72	0.35	0.35	4.3	0.5111	0%
SB02NW	0.0193	0.1919	0.123	86	0.35	0.35	4.3	0.5111	0%
SB02W	0.0571	0.3483	0.111	88	0.35	0.35	4.3	0.5111	0%
SB02E	0.0440	0.3557	0.114	82	0.35	0.35	4.3	0.5111	0%
SB8NW	0.0147	0.1445	0.126	83	0.35	0.35	4.3	0.5111	0%
SB8NE	0.0150	0.1405	0.125	85	0.35	0.35	4.3	0.5111	0%
SB8SW	0.0129	0.1383	0.127	116	0.35	0.35	4.3	0.5111	0%
SB8SE	0.0131	0.1580	0.127	104	0.35	0.35	4.3	0.5111	0%
SB08W	0.0589	0.3428	0.030	64	0.10	0.35	4.3	0.5111	80%
SB07W	0.0468	0.3138	0.113	99	0.35	0.35	4.3	0.5111	0%

SUM= 0.4272

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN	BASIN AREA			WATER COURSE LENGTH		BASIN ELEVATION			SLOPE		
						High	Low	Δ Elevation			ADJUSTED
NUMBER	(Feet^2)	(ACRES)	(Miles^2)	(Feet)	(Miles)	(Feet)	(Feet)	(Feet)	(Ft/Ft)	(Ft/Mile)	(Ft/Mile)
SB01-C	752,914	17.28	0.0270	1735	0.329	1681.0	1649.3	31.7	0.0183	96.47	96.5
SB01-B	668,111	15.34	0.0240	1452	0.275	1668.5	1637.0	31.5	0.0217	114.55	114.5
SB01-A	1,700,557	39.04	0.0610	1452	0.275	1659.3	1626.0	33.3	0.0229	121.09	121.1
SB02NE	934,725	21.46	0.0335	2045	0.387	1652.0	1624.0	28.0	0.0137	72.29	72.3
SB02NW	537,015	12.33	0.0193	1013	0.192	1640.5	1624.0	16.5	0.0163	86.00	86.0
SB02W	1,592,151	36.55	0.0571	1839	0.348	1634.5	1604.0	30.5	0.0166	87.57	87.6
SB02E	1,227,661	28.18	0.0440	1878	0.356	1633.2	1604.0	29.2	0.0155	82.10	82.1
SB8NW	408,919	9.39	0.0147	763	0.145	1654.0	1642.0	12.0	0.0157	83.04	83.0
SB8NE	417,750	9.59	0.0150	742	0.141	1654.0	1642.0	12.0	0.0162	85.39	85.4
SB8SW	358,807	8.24	0.0129	730	0.138	1642.0	1626.0	16.0	0.0219	115.73	115.7
SB8SE	365,480	8.39	0.0131	834	0.158	1642.5	1626.0	16.5	0.0198	104.46	104.5
SB08W	1,641,602	37.69	0.0589	1810	0.343	1634.0	1612.0	22.0	0.0122	64.18	64.2
SB07W	1,303,601	29.93	0.0468	1657	0.314	1685.0	1654.0	31.0	0.0187	98.78	98.8

SUM=	11,909,293	273	0.4272

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA (Miles^2)	SOIL TYPE	% OF SOIL TYPE	ROCK OUTCROP % OF SOIL IMPERVIOUS	XKSAT (in/hr)	XKSAT ADJUSTED FOR 35% VEGATATION	$\overline{\text{XKSAT}}$ (in/hr)	PSIF (in)	DTHETA (in)	RTEMP SOIL IN SUBBASIN
SB01-C	0.0270	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB01-B	0.0240	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB01-A	0.0610	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02NE	0.0335	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02NW	0.0193	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02W	0.0571	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB02E	0.0440	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8NW	0.0147	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8NE	0.0150	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8SW	0.0129	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB8SE	0.0131	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB08W	0.0589	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%
SB07W	0.0468	90	100%	0%	0.4	0.511	0.511	4.3	0.35	0%

Check: 0.4272

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA ACRES	LAND USE	% OF BASIN	SUB AREA ACRES	LAND TYPE	K_b	K_b WEIGHTED	IA FOR LAND USE	IA WEIGHTED	RTEMP FOR SOIL IN BASIN	RTEMP FOR LAND USE	RTEMP WEIGHTED	RTEMP MAX
SB01-C	17.28	NDR	100%	17.28	C	0.1191	0.1191	0.35	0.35	0%	0%	0%	0%
SB01-B	15.34	NDR	100%	15.34	C	0.1204	0.1204	0.35	0.35	0%	0%	0%	0%
SB01-A	39.04	NDR	100%	39.04	C	0.1102	0.1102	0.35	0.35	0%	0%	0%	0%
SB02NE	21.46	NDR	100%	21.46	C	0.1167	0.1167	0.35	0.35	0%	0%	0%	0%
SB02NW	12.33	NDR	100%	12.33	C	0.1227	0.1227	0.35	0.35	0%	0%	0%	0%
SB02W	36.55	NDR	100%	36.55	C	0.1109	0.1109	0.35	0.35	0%	0%	0%	0%
SB02E	28.18	NDR	100%	28.18	C	0.1138	0.1138	0.35	0.35	0%	0%	0%	0%
SB8NW	9.39	NDR	100%	9.39	C	0.1257	0.1257	0.35	0.35	0%	0%	0%	0%
SB8NE	9.59	NDR	100%	9.59	C	0.1255	0.1255	0.35	0.35	0%	0%	0%	0%
SB8SW	8.24	NDR	100%	8.24	C	0.1271	0.1271	0.35	0.35	0%	0%	0%	0%
SB8SE	8.39	NDR	100%	8.39	C	0.1269	0.1269	0.35	0.35	0%	0%	0%	0%
SB08W	37.69	C2	100%	37.69	A	0.0301	0.0301	0.1	0.10	0%	80%	80%	80%
SB07W	29.93	NDR	100%	29.93	C	0.1131	0.1131	0.35	0.35	0%	0%	0%	0%

SUM= 273 273

Retention Required

PROJECT NAME: Cavasson Master Plan Phase 3 Existing
HUBBARD JOB # : 18114-101
PREPARED BY: TSW
DATE: 9/16/2020

NOTE: SHADED HEADING INDICATES USER INPUT SPREADSHEET DATA, OTHERS CALCULATED BY SPREADSHEET.

BASIN NUMBER	BASIN AREA ($Feet^2$)	BASIN AREA (ACRES)	BASIN AREA ($Miles^2$)	Weighted C Proposed Cond.	Volume Required [acre-ft]	Volume Required [ft^3]	eet)
SB01-C	752,914	17.28	0.0270	0.85	0.612	26,665.7	
SB01-B	668,111	15.34	0.0240	0.85	0.543	23,662.3	
SB01-A	1,700,557	39.04	0.0610	0.85	1.383	60,228.1	
SB02NE	934,725	21.46	0.0335	0.85	0.760	33,104.9	
SB02NW	537,015	12.33	0.0193	0.85	0.437	19,019.3	
SB02W	1,592,151	36.55	0.0571	0.85	1.295	56,388.7	
SB02E	1,227,661	28.18	0.0440	0.85	0.998	43,479.6	
SB8NW	408,919	9.39	0.0147	0.85	0.332	14,482.5	
SB8NE	417,750	9.59	0.0150	0.85	0.340	14,795.3	
SB8SW	358,807	8.24	0.0129	0.85	0.292	12,707.8	
SB8SE	365,480	8.39	0.0131	0.85	0.297	12,944.1	
SB08W	1,641,602	37.69	0.0589	0.85	1.335	58,140.1	
SB07W	1,303,601	29.93	0.0468	0.85	1.060	46,169.2	
SUM=	11,909,293	273	0.43		9.683	421,787.5	

Appendix D
Cavasson HEC-1 Ultimate
Grayhawk Residences at Cavasson

10yr-6hr-Ultimate

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1***************************************
**************************************
 *                                     *                                                 *
*
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                 *    U.S. ARMY CORPS OF ENGINEERS
*
 *              JUN   1998               *                                                 *    HYDROLOGIC ENGINEERING CENTER
*
 *           VERSION 4.1                 *                                                 *          609 SECOND STREET
*
 *                                     *                                                 *       DAVIS, CALIFORNIA 95616
*
 *  RUN DATE   30APR21  TIME  13:35:28   *                                                 *           (916) 756-1104
*
 *                                     *                                                 *
*
 ***************************************
**************************************



                                                  X     X  XXXXXXX   XXXXX          X
                                                  X     X  X        X     X        XX
                                                  X     X  X        X               X
                                                  XXXXXXX  XXXX     X        XXXXX  X
                                                  X     X  X        X               X
                                                  X     X  X        X     X         X
                                                  X     X  XXXXXXX   XXXXX         XXX



           THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

           THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
           THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
           NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
           DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
           KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


1                                                     HEC-1 INPUT                                              PAGE  1

          LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

             1          ID        CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
             2          ID        POWERLINE CORRIDOR
             3          ID        RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
             4          ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
             5          ID        10-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
             6          ID           PIMA ROAD FLOWS.
             7          ID
             8          ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
             9          ID        MODELERS: RK, MW
            10          ID
            11          ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
            12          ID        MODELERS: MSW, ES
            13          ID        REVISIONS NOTED WITH HE
            14          ID           ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
            15          ID           THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
            16          ID           UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.
            17          ID
            18          IT       3       0       0    1000
            19          IN      15
            20          IO       5
                        *DIAGRAM
                        *
            21          JD   1.817  0.0001
            22          PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
            23          PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
            24          PC   0.962   0.972   0.983   0.991   1.000
            25          JD   1.806  0.5000
            26          PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
            27          PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
            28          PC   0.962   0.972   0.983   0.991   1.000
            29          JD   1.772     2.8
            30          PC   0.000   0.009   0.016   0.025   0.034   0.042   0.051   0.059   0.068   0.077
            31          PC   0.088   0.101   0.121   0.164   0.253   0.451   0.694   0.836   0.900   0.938
```

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  32          PC   0.950   0.963   0.975   0.988   1.000
              *

  33          KK  76THST
  34          KM        76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
  35          KM        10-YR, 6-HR, DERIVED FROM 100-YR DATA HYDROGRAPHS
  36          BA    0.24
  37          QI       0       0       0       0       0       0       0       0       0       0
  38          QI       0       0    0.01    0.01    0.06    0.58    8.13   27.04   31.94    31.1
  39          QI   22.07   17.10   13.79   11.20    9.48    8.22    7.44    6.76    6.13    5.32
  40          QI    4.66    4.31    3.73    3.05    2.71    2.44    2.18    2.27    1.51    1.61
  41          QI    1.48    1.38    1.29    1.17    1.01    0.96    0.97    0.77    0.84    0.86
  42          QI    0.70    0.68    0.67    0.62    0.58    0.55    0.53    0.51    0.49    0.46
  43          QI    0.44    0.43    0.42    0.40    0.38    0.37    0.35    0.34    0.33    0.31
  44          QI    0.30    0.29    0.28
              *
1                                          HEC-1 INPUT                                           PAGE  2

       LINE   ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  45          KK   SB01A   BASIN
  46          BA   0.061
  47          PB   2.721
  48          PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.075
  49          PC   0.087   0.099   0.119   0.150   0.234   0.413   0.766   0.875   0.916   0.944
  50          PC   0.956   0.968   0.979   0.990   1.000
  51          LG    0.25    0.25    4.03    0.56      45
  52          UC   0.439   0.231
  53          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  54          UA     100
              *

  55          KK   SB01A  DIVERT
  56          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01A
  57          KM        FIRST FLUSH VOL = 1.383 AC-FT
  58          DT  RET01A   1.383
  59          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  60          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

  61          KK    CP-1 COMBINE
  62          KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
  63          HC       2   0.301
              *

  64          KK  R1A-2W   ROUTE
  65          KM        ROUTE CP-1 TO CP-2W
  66          RK    1114  0.0200   0.018           TRAP   2.000    0.00
              *

  67          KK  SB02NW   BASIN
  68          KM        BASIN SB02E WAS SPLIT INTO SUB BASINS NORTH OF CAVASSON
  69          KM        AND SOUTH OF CAVASSON
  70          BA  0.0193
  71          LG    0.25    0.25    4.03    0.56      45
  72          UC   0.211   0.167
  73          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  74          UA     100
              *

  75          KK  SB02NW  DIVERT
  76          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NW
  77          KM        FIRST FLUSH VOL = 0.437 AC-FT
  78          DT  RET2NW   0.437
  79          DI     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
  80          DI    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
  81          DQ     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
  82          DQ    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
              *
1                                          HEC-1 INPUT                                           PAGE  3

       LINE   ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  83          KK  CP-02W COMBINE
  84          KM        COMBINE ROUTE FROM 1A-2W WITH NEW SUB BASIN SB02NW
  85          HC       2   0.305
              *
```

```
 86           KK   R2W-2   ROUTE
 87           RK    1380  0.0140   0.018           TRAP   2.000    0.00
 88
              *

 89           KK   SB01C   BASIN
 90           BA   0.027
 91           LG    0.25    0.25    4.03    0.56      45
 92           UC   0.264   0.272
 93           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 94           UA     100
              *

 95           KK   SB01C  DIVERT
 96           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01C
 97           KM        FIRST FLUSH VOL = 0.612 AC-FT
 98           DT  RET01C   0.612
 99           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
100           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

101           KK  R1C-1B   ROUTE
102           RK     542   0.005   0.013           CIRC   5.000
              *

103           KK   SB01B   BASIN
104           BA   0.024
105           LG    0.25    0.25    4.03    0.56      45
106           UC   0.234   0.224
107           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
108           UA     100
              *

109           KK   SB01B  DIVERT
110           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01B
111           KM        FIRST FLUSH VOL = 0.543 AC-FT
112           DT  RET01B   0.543
113           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
114           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

115           KK  CP-01B COMBINE
116           KM        COMBINE ROUTE FROM 1C-1B WITH NEW SUB BASIN SB01B
117           HC       2   0.051
              *
1                                             HEC-1 INPUT                                                   PAGE  4

        LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


118           KK  R1B-2N   ROUTE
119           RK    2144   0.008   0.013           CIRC   5.000
              *

120           KK  SB02NE   BASIN
121           BA  0.0335
122           LG    0.25    0.25    4.03    0.56      45
123           UC   0.314   0.336
124           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
125           UA     100
              *

126           KK  SB02NE  DIVERT
127           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NE
128           KM        FIRST FLUSH VOL = 0.760 AC-FT
129           DT  RET2NE   0.760
130           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
131           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

132           KK  CP-02N COMBINE
133           KM        COMBINE ROUTE 1B-2N (BASINS SB01-B AND SB01-C) WITH SB02NE
134           HC       2   0.085
              *

135           KK   R2N-2   ROUTE
136           RK    1380   0.014   0.018           TRAP   2.000    0.00
              *

137           KK   SB02E   BASIN
```

```
 138           BA   0.040
 139           LG    0.10    0.25    4.03    0.61      80
 140           UC   0.254   0.212
 141           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 142           UA     100
               *

 143           KK   SB02E  DIVERT
 144           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E
 145           KM        FIRST FLUSH VOL = 0.998 AC-FT
 146           DT   RET2E   0.998
 147           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 148           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 149           KK   SB02W   BASIN
 150           BA  0.0571
 151           LG    0.35    0.35    4.33    0.51       0
 152           UC   0.686   0.535
 153           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 154           UA     100
               *
1                                          HEC-1 INPUT                                            PAGE  5

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 155           KK   CP-02 COMBINE
 156           HC       4  0.1816
               *

 157           KK    R2-3   ROUTE
 158           KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
 159           RK    1260  0.0015   0.030            TRAP      92       4
               *

 160           KK    SB03   BASIN
 161           BA   0.048
 162           LG    0.15    0.25    4.50    0.47      55
 163           UC   0.364   0.294
 164           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 165           UA     100
               *

 166           KK   CP-03 COMBINE
 167           HC       2
               *

 168           KK    R3-5   ROUTE
 169           KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
 170           RK    2396  0.0015   0.03             TRAP      98       4
               *

 171           KK    SB05   BASIN
 172           BA   0.126
 173           LG    0.22    0.25    4.50    0.44      48
 174           UC   0.327   0.226
 175           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 176           UA     100
               *

 177           KK   CP-5A COMBINE
 178           HC       2
               *

 179           KK   SB07W   BASIN
 180           BA  0.0468
 181           LG    0.25    0.25    4.03    0.56      45
 182           UC   0.250   0.178
 183           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 184           UA     100
               *

 185           KK   SB07W  DIVERT
 186           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB07W
 187           KM        FIRST FLUSH VOL = 1.060 AC-FT
 188           DT  RET07W   1.060
 189           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 190           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *
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LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 191           KK  R7W-8W   ROUTE
 192           KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 193           KM        HE MODIFIED ROUTE TO LOOP 101 W OF HAYDEN
 194           RK    3754  0.0013   0.03             TRAP      46       4
               *

 195           KK   SB8NW   BASIN
 196           KM        BASIN JUST SOUTH OF LEGACY - FUTURE MOB SITE
 197           BA  0.0147
 198           LG    0.10    0.25    4.03    0.61      80
 199           UC   0.171   0.131
 200           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 201           UA     100
               *

 202           KK   SB8NW  DIVERT
 203           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NW
 204           KM        FIRST FLUSH VOL = 0.332 AC-FT
 205           DT  RET8NW   0.332
 206           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 207           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 208           KK  R8NW8N   ROUTE
 209           KM        ROUTE NEW SUB BASIN SB8NW TO OUTLET AT CAVASSON BLVD
 210           KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 211           RK     643  0.0113   0.018            TRAP      46       4
               *

 212           KK   SB8NE   BASIN
 213           KM        SITE JUST SOUTH OF LEGACY AND WEST OF HAYDEN - FUTURE RETAIL
 214           BA   0.015
 215           LG    0.10    0.25    4.03    0.61      80
 216           UC   0.164   0.113
 217           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 218           UA     100
               *

 219           KK   SB8NE  DIVERT
 220           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NE
 221           KM        FIRST FLUSH VOL = 0.340 AC-FT
 222           DT  RET8NE   0.340
 223           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 224           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 225           KK  R8NE8N   ROUTE
 226           KM        ROUTE NEW SUB BASIN SB8NE TO OUTLET AT CAVASSON BLVD
 227           KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 228           RK     622   0.011   0.018            TRAP      46       4
               *
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LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 229           KK   SB8SW   BASIN
 230           KM        SITE JUST NORTH OF CAVASSON BLVD EAST OF CLARET DR - FUTURE MF
 231           BA  0.0129
 232           LG    0.10    0.25    4.03    0.61      80
 233           UC   0.149   0.110
 234           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 235           UA     100
               *

 236           KK   SB8SW  DIVERT
 237           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SW
 238           KM        FIRST FLUSH VOL = 0.292 AC-FT
 239           DT  RET8SW   0.292
 240           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 241           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 242           KK   SB8SE   BASIN
 243           KM        SITE JUST NORTH OF CAVASSON AND WEST OF HAYDEN - FUTURE HOTEL OR RETAIL
```

```
 244           BA  0.0131
 245           LG    0.10    0.25    4.03    0.61      80
 246           UC   0.165   0.137
 247           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 248           UA     100
               *

 249           KK   SB8SE  DIVERT
 250           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SE
 251           KM        FIRST FLUSH VOL = 0.297 AC-FT
 252           DT  RET8SE   0.297
 253           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 254           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 255           KK  CP-08N COMBINE
 256           KM        CP AT INLET ON NORTH SIDE OF CAVASSON - ENTERS EXIST STORM DRAIN
 257           HC       4  0.0557
               *

 258           KK  R8N-8W   ROUTE
 259           KM        ROUTE NEW SUB BASINS SB8NE SB8NW SB8SW SB8SE TO CP-08W
 260           KM        ROUTE IN STORM DRAIN
 261           RK    1592   0.015   0.013            CIRC       3
               *

 262           KK   SB08W   BASIN
 263           BA  0.0589
 264           LG    0.10    0.25    4.03    0.61      80
 265           UC   0.262   0.177
 266           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 267           UA     100
               *
1                                        HEC-1 INPUT                                           PAGE  8

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 268           KK   SB08W  DIVERT
 269           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB08W
 270           KM        FIRST FLUSH VOL REQ = 1.335 AC-FT, PROVIDED = 1.415
 271           KM        THIS IS EXISTING AND WAS BUILT IN PHASE 1
 272           DT   RET8W   1.415
 273           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 274           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 275           KK  CP-08W COMBINE
 276           HC       3  0.1614
               *
               *

 277           KK   R8W-4   ROUTE
 278           RK    3137  0.0130   0.030            TRAP   2.000    0.00
               *

 279           KK    SB04   BASIN
 280           BA   0.144
 281           LG    0.14    0.25    4.60    0.44      61
 282           UC   0.305   0.199
 283           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 284           UA     100
               *

 285           KK    CP-4 COMBINE
 286           HC       2
               *

 287           KK    R4-5   ROUTE
 288           KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 289           RK    1992  0.0013   0.03             TRAP      39       4
               *

 290           KK   CP-05 COMBINE
 291           KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 292           KM        COMBINE CP-05A AND R4-5
 293           HC       2
               *

 294           KK    R5-6   ROUTE
```

```
 295           KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 296           RK    1694  0.0015   0.03             TRAP     188       4
               *

 297           KK    SB06   BASIN
 298           BA   0.136
 299           LG    0.16    0.25    4.55    0.45      53
 300           UC   0.321   0.246
 301           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 302           UA     100
               *
1                                        HEC-1 INPUT                                           PAGE  9

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 303           KK   CP-06 COMBINE
 304           HC       2
               *

 305           KK   SB07E   BASIN
 306           BA   0.026
 307           LG    0.10    0.25    4.03    0.74       0
 308           UC   0.574   0.527
 309           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 310           UA     100
               *

 311           KK  R7E-8E   ROUTE
 312           RK    3662  0.0130   0.030            TRAP   2.000    0.00
               *

 313           KK   SB08E   BASIN
 314           BA   0.078
 315           LG    0.10    0.25    4.08    0.72       0
 316           UC   0.798   0.652
 317           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 318           UA     100
               *

 319           KK   SB09N   BASIN
 320           BA   0.028
 321           LG    0.10    0.25    4.79    0.49       0
 322           UC   0.740   1.111
 323           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 324           UA     100
               *

 325           KK  R9NR8E   ROUTE
 326           RK    2586  0.0013   0.020            TRAP   2.000    0.00
               *

 327           KK  CP-08E COMBINE
 328           HC       3
               *

 329           KK  R8E-R9   ROUTE
 330           RK     501  0.0013   0.020            TRAP   2.000    0.00
               *

 331           KK    SB09   BASIN
 332           BA   0.086
 333           LG    0.10    0.25    4.79    0.49       0
 334           UC   0.683   0.518
 335           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 336           UA     100
               *
1                                        HEC-1 INPUT                                           PAGE 10

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 337           KK   CP-09 COMBINE
 338           HC       2
               *

 339           KK   PWRCH
 340           KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
 341           KM        10-YR, 6-HR, DERIVED FROM 100-YR DATA HYDROGRAPHS
 342           BA     7.0
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```
 343          QI       0       0       0       0       0       0       0       0       0       0
 344          QI       0       0       0       0       0    0.07   11.70   33.57  285.55  257.89
 345          QI  236.82  258.26  406.55  486.55  461.36  425.60  409.84  395.31  391.77  376.42
 346          QI  346.76  376.76  276.32  247.65  220.91  197.44  178.64  155.41  139.22  129.78
 347          QI  113.97  102.76   96.30   85.88    79.7   72.75   65.45   59.94   54.69   51.22
 348          QI    52.3   44.56   41.87   42.77   36.93   34.05   32.15   29.01   27.72   27.03
 349          QI   25.05   23.86   22.04   20.81   19.27   18.55   17.90   20.82   16.03   15.23
              *

 350          KK  BINFLO
 351          KM         TOTAL INFLOW INTO BASIN 53R.
 352          HC       2
              *

 353          KK   BASIN
 354          KM     BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
 355          KM     ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
 356          KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
 357          KM        THE OUTLET PIPES ARE INLET CONTROLLED.
 358          RS       1    STOR       0
 359          SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
 360          SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
 361          SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
 362          SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
 363          SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
 364          SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
              *

 365          KK  BSNRT1
 366          KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
 367          KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
 368          KM        EXCESS CAPACITY.
 369          RK     550  0.0052   0.024            CIRC     7.0
              *

 370          KK  BSNRT2
 371          KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BAS 53R OUTFALL)
 372          RK    1200  0.0077   0.013            CIRC     7.0
              *

 373          KK    SB10   BASIN
 374          BA   0.040
 375          LG    0.15    0.25    4.25    0.55      55
 376          UC   0.233   0.161
 377          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 378          UA     100
              *
1                                    HEC-1 INPUT                                              PAGE 11

LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 379          KK   CP-10 COMBINE
 380          KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
 381          HC       2
              *

 382          KK  R10-11   ROUTE
 383          KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
 384          RK    1277  0.0014   0.03             TRAP     220       4
              *

 385          KK    SB11   BASIN
 386          BA   0.071
 387          LG    0.15    0.25    4.15    0.58      55
 388          UC   0.296   0.232
 389          UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 390          UA     100
              *

 391          KK   CP-11 COMBINE
 392          KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
 393          HC       2
              *
 394          ZZ
1
         SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW
```

```
NO.      (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

 33      76THST
              .
              .
 45           .       SB01A
              .           .
              .           .
 58           .           .------->  RET01A
 55           .       SB01A
              .           .
              .           .
 61        CP-1............
              V
              V
 64      R1A-2W
              .
              .
 67           .      SB02NW
              .           .
              .           .
 78           .           .------->  RET2NW
 75           .      SB02NW
              .           .
              .           .
 83      CP-02W............
              V
              V
 86       R2W-2
              .
              .
 89           .       SB01C
              .           .
              .           .
 98           .           .------->  RET01C
 95           .       SB01C
              .           V
              .           V
101           .      R1C-1B
              .           .
              .           .
103           .           .       SB01B
              .           .           .
              .           .           .
112           .           .           .------->  RET01B
109           .           .       SB01B
              .           .           .
              .           .           .
115           .      CP-01B............
              .           V
              .           V
118           .      R1B-2N
              .           .
              .           .
120           .           .      SB02NE
              .           .           .
              .           .           .
129           .           .           .------->  RET2NE
126           .           .      SB02NE
              .           .           .
              .           .           .
132           .      CP-02N............
              .           V
              .           V
135           .       R2N-2
              .           .
              .           .
137           .           .       SB02E
              .           .           .
              .           .           .
146           .           .           .------->   RET2E
143           .           .       SB02E
              .           .           .
              .           .           .
149           .           .           .       SB02W
              .           .           .           .
              .           .           .           .
155       CP-02....................................
              V
              V
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157        R2-3
              .
              .
160           .        SB03
              .           .
              .           .
166       CP-03............
              V
              V
168        R3-5
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              .
171           .        SB05
              .           .
              .           .
177       CP-5A............
              .
              .
179           .       SB07W
              .           .
              .           .
188           .           .------->  RET07W
185           .       SB07W
              .           V
              .           V
191           .      R7W-8W
              .           .
              .           .
195           .           .       SB8NW
              .           .           .
              .           .           .
205           .           .           .------->  RET8NW
202           .           .       SB8NW
              .           .           V
              .           .           V
208           .           .      R8NW8N
              .           .           .
              .           .           .
212           .           .           .       SB8NE
              .           .           .           .
              .           .           .           .
222           .           .           .           .------->  RET8NE
219           .           .           .       SB8NE
              .           .           .           V
              .           .           .           V
225           .           .           .      R8NE8N
              .           .           .           .
              .           .           .           .
229           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
239           .           .           .           .           .------->  RET8SW
236           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
242           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
252           .           .           .           .           .           .------->  RET8SE
249           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
255           .           .      CP-08N....................................
              .           .           V
              .           .           V
258           .           .      R8N-8W
              .           .           .
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262           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
272           .           .           .           .------->   RET8W
268           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
275           .      CP-08W........................
              .           V
              .           V
277           .       R8W-4
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 279        .           .        SB04
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 285        .        CP-4............
            .           V
            .           V
 287        .        R4-5
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 290     CP-05............
            V
            V
 294      R5-6
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 297        .        SB06
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 303     CP-06............
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 305        .       SB07E
            .           V
            .           V
 311        .      R7E-8E
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 313        .           .       SB08E
            .           .           .
            .           .           .
 319        .           .           .       SB09N
            .           .           .           V
            .           .           .           V
 325        .           .           .      R9NR8E
            .           .           .           .
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 327        .      CP-08E.......................
            .           V
            .           V
 329        .      R8E-R9
            .           .
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 331        .           .        SB09
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 337        .       CP-09............
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            .           .
 339        .           .       PWRCH
            .           .           .
            .           .           .
 350        .      BINFLO............
            .           V
            .           V
 353        .       BASIN
            .           V
            .           V
 365        .      BSNRT1
            .           V
            .           V
 370        .      BSNRT2
            .           .
            .           .
 373        .           .        SB10
            .           .           .
            .           .           .
 379        .       CP-10............
            .           V
            .           V
 382        .      R10-11
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 385        .           .        SB11
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 391        .       CP-11............

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION
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 *                                       *                                                    *
*
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                    *    U.S. ARMY CORPS OF ENGINEERS
*
 *              JUN   1998               *                                                    *    HYDROLOGIC ENGINEERING CENTER
*
 *           VERSION 4.1                 *                                                    *          609 SECOND STREET
*
 *                                       *                                                    *       DAVIS, CALIFORNIA 95616
*
 *  RUN DATE   30APR21  TIME  13:35:28   *                                                    *           (916) 756-1104
*
 *                                       *                                                    *
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                                    CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
                                    POWERLINE CORRIDOR
                                    RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                                    INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                                       10-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                                       PIMA ROAD FLOWS.

                                    PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                                       MODELERS: RK, MW

                                    REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
                                       MODELERS: MSW, ES
                                       REVISIONS NOTED WITH HE
                                       ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
                                       THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
                                       UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.


   20 IO          OUTPUT CONTROL VARIABLES
                        IPRNT           5  PRINT CONTROL
                        IPLOT           0  PLOT CONTROL
                        QSCAL          0.  HYDROGRAPH PLOT SCALE

      IT          HYDROGRAPH TIME DATA
                         NMIN           3  MINUTES IN COMPUTATION INTERVAL
                        IDATE      1    0  STARTING DATE
                        ITIME        0000  STARTING TIME
                           NQ        1000  NUMBER OF HYDROGRAPH ORDINATES
                       NDDATE      3    0  ENDING DATE
                       NDTIME        0157  ENDING TIME
                        ICENT          19  CENTURY MARK

                  COMPUTATION INTERVAL    0.05 HOURS
                       TOTAL TIME BASE   49.95 HOURS

           ENGLISH UNITS
                DRAINAGE AREA         SQUARE MILES
                PRECIPITATION DEPTH   INCHES
                LENGTH, ELEVATION     FEET
                FLOW                  CUBIC FEET PER SECOND
                STORAGE VOLUME        ACRE-FEET
                SURFACE AREA          ACRES
                TEMPERATURE           DEGREES FAHRENHEIT

   21 JD          INDEX STORM NO. 1
                         STRM        1.82  PRECIPITATION DEPTH
                         TRDA        0.00  TRANSPOSITION DRAINAGE AREA

   22 PI            PRECIPITATION PATTERN
                       0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                       0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                       0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                       0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                       0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                       0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                       0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                       0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
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                    0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

25 JD          INDEX STORM NO. 2
                      STRM        1.81  PRECIPITATION DEPTH
                      TRDA        0.50  TRANSPOSITION DRAINAGE AREA

26 PI            PRECIPITATION PATTERN
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                    0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
                    0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

29 JD          INDEX STORM NO. 3
                      STRM        1.77  PRECIPITATION DEPTH
                      TRDA        2.80  TRANSPOSITION DRAINAGE AREA

30 PI            PRECIPITATION PATTERN
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                    0.04      0.04      0.04      0.04      0.04      0.05      0.05      0.05      0.05      0.05
                    0.03      0.03      0.03      0.03      0.03      0.01      0.01      0.01      0.01      0.01
                    0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                    0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

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*** HEC-1 ERROR 1 *** INVALID CARD IDENTIFICATION CODE OR CARD OUT OF SEQUENCE
     CARD NO.   88

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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+	HYDROGRAPH AT	76THST	32.	4.50	10.	3.	1.	0.24		
+	HYDROGRAPH AT	SB01A	59.	4.25	7.	2.	1.	0.06		
+	DIVERSION TO	RET01A	48.	4.15	3.	1.	0.	0.06		
+	HYDROGRAPH AT	SB01A	59.	4.25	4.	1.	0.	0.06		
+	2 COMBINED AT	CP-1	86.	4.25	14.	4.	2.	0.30		
+	ROUTED TO	R1A-2W	86.	4.30	14.	4.	2.	0.30		
+	HYDROGRAPH AT	SB02NW	23.	4.10	2.	1.	0.	0.02		
+	DIVERSION TO	RET2NW	18.	4.00	1.	0.	0.	0.02		
+	HYDROGRAPH AT	SB02NW	23.	4.10	1.	0.	0.	0.02		
+	2 COMBINED AT	CP-02W	97.	4.30	15.	4.	2.	0.31		
+	ROUTED TO	R2W-2	97.	4.30	15.	4.	2.	0.31		
+	HYDROGRAPH AT	SB01C	26.	4.15	3.	1.	0.	0.03		
+	DIVERSION TO	RET01C	24.	4.10	1.	0.	0.	0.03		
+	HYDROGRAPH AT	SB01C	26.	4.15	2.	0.	0.	0.03		
+	ROUTED TO	R1C-1B	25.	4.20	2.	0.	0.	0.03		
+	HYDROGRAPH AT	SB01B	25.	4.15	3.	1.	0.	0.02		
+	DIVERSION TO	RET01B	22.	4.05	1.	0.	0.	0.02		
+	HYDROGRAPH AT	SB01B	25.	4.15	2.	0.	0.	0.02		
+	2 COMBINED AT	CP-01B	48.	4.20	3.	1.	0.	0.05		
	ROUTED TO									

+		R1B-2N	47.	4.20	3.	1.	0.	0.05
	HYDROGRAPH AT							
+		SB02NE	29.	4.20	4.	1.	0.	0.03
	DIVERSION TO							
+		RET2NE	27.	4.15	2.	0.	0.	0.03
	HYDROGRAPH AT							
+		SB02NE	27.	4.25	2.	1.	0.	0.03
	2 COMBINED AT							
+		CP-02N	73.	4.25	6.	1.	1.	0.09
	ROUTED TO							
+		R2N-2	72.	4.25	6.	1.	1.	0.09
	HYDROGRAPH AT							
+		SB02E	55.	4.15	7.	2.	1.	0.04
	DIVERSION TO							
+		RET2E	28.	3.95	2.	1.	0.	0.04
	HYDROGRAPH AT							
+		SB02E	55.	4.15	5.	1.	1.	0.04
	HYDROGRAPH AT							
+		SB02W	16.	4.50	2.	1.	0.	0.06
	4 COMBINED AT							
+		CP-02	210.	4.25	28.	7.	4.	0.18
	ROUTED TO							
+		R2-3	209.	4.35	28.	7.	4.	0.18
	HYDROGRAPH AT							
+		SB03	47.	4.15	6.	2.	1.	0.05
	2 COMBINED AT							
+		CP-03	241.	4.35	33.	9.	4.	0.23
	ROUTED TO							
+		R3-5	234.	4.50	33.	9.	4.	0.23
	HYDROGRAPH AT							
+		SB05	134.	4.10	15.	4.	2.	0.13
	2 COMBINED AT							
+		CP-5A	277.	4.50	47.	13.	6.	0.36
	HYDROGRAPH AT							
+		SB07W	54.	4.15	5.	1.	1.	0.05
	DIVERSION TO							
+		RET07W	41.	4.05	2.	1.	0.	0.05
	HYDROGRAPH AT							
+		SB07W	54.	4.15	3.	1.	0.	0.05
	ROUTED TO							
+		R7W-8W	51.	4.65	5.	1.	1.	0.05
	HYDROGRAPH AT							
+		SB8NW	24.	4.05	2.	1.	0.	0.01
	DIVERSION TO							
+		RET8NW	8.	3.80	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8NW	24.	4.05	2.	0.	0.	0.01
	ROUTED TO							
+		R8NW8N	24.	4.10	2.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8NE	25.	4.05	2.	1.	0.	0.01
	DIVERSION TO							
+		RET8NE	8.	3.80	1.	0.	0.	0.01

	HYDROGRAPH AT							
+		SB8NE	25.	4.05	2.	0.	0.	0.01
	ROUTED TO							
+		R8NE8N	25.	4.10	2.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SW	22.	4.05	2.	1.	0.	0.01
	DIVERSION TO							
+		RET8SW	7.	3.75	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SW	22.	4.05	2.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SE	21.	4.05	2.	1.	0.	0.01
	DIVERSION TO							
+		RET8SE	7.	3.80	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SE	21.	4.05	2.	0.	0.	0.01
	4 COMBINED AT							
+		CP-08N	90.	4.05	7.	2.	1.	0.06
	ROUTED TO							
+		R8N-8W	89.	4.10	7.	2.	1.	0.06
	HYDROGRAPH AT							
+		SB08W	85.	4.15	10.	2.	1.	0.06
	DIVERSION TO							
+		RET8W	36.	3.90	3.	1.	0.	0.06
	HYDROGRAPH AT							
+		SB08W	85.	4.15	7.	2.	1.	0.06
	3 COMBINED AT							
+		CP-08W	172.	4.10	18.	5.	2.	0.16
	ROUTED TO							
+		R8W-4	168.	4.15	18.	5.	2.	0.16
	HYDROGRAPH AT							
+		SB04	178.	4.10	20.	5.	2.	0.14
	2 COMBINED AT							
+		CP-4	346.	4.10	38.	10.	5.	0.31
	ROUTED TO							
+		R4-5	338.	4.20	38.	10.	5.	0.31
	2 COMBINED AT							
+		CP-05	432.	4.20	82.	22.	11.	0.66
	ROUTED TO							
+		R5-6	427.	4.30	82.	22.	11.	0.66
	HYDROGRAPH AT							
+		SB06	145.	4.10	17.	4.	2.	0.14
	2 COMBINED AT							
+		CP-06	499.	4.25	96.	25.	12.	0.80
	HYDROGRAPH AT							
+		SB07E	9.	4.40	1.	0.	0.	0.03
	ROUTED TO							
+		R7E-8E	8.	4.50	1.	0.	0.	0.03
	HYDROGRAPH AT							
+		SB08E	21.	4.55	3.	1.	0.	0.08
	HYDROGRAPH AT							
+		SB09N	6.	4.55	2.	0.	0.	0.03
	ROUTED TO							
+		R9NR8E	6.	4.65	2.	0.	0.	0.03

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         3 COMBINED AT
+                        CP-08E        34.    4.60          6.          2.          1.       0.13

         ROUTED TO
+                        R8E-R9        34.    4.60          6.          2.          1.       0.13

         HYDROGRAPH AT
+                          SB09        34.    4.45          5.          1.          1.       0.09

         2 COMBINED AT
+                         CP-09        66.    4.55         11.          3.          1.       0.22

         HYDROGRAPH AT
+                         PWRCH       487.    5.75        285.         91.         50.       7.00

         2 COMBINED AT
+                        BINFLO       484.    5.75        283.         90.         50.       7.22

         ROUTED TO
+                         BASIN       171.    9.10        156.         85.         48.       7.22

         ROUTED TO
+                        BSNRT1       171.    9.10        156.         85.         48.       7.22

         ROUTED TO
+                        BSNRT2       171.    9.10        156.         85.         48.       7.22

         HYDROGRAPH AT
+                          SB10        52.    4.05          5.          1.          1.       0.04

         2 COMBINED AT
+                         CP-10       171.    9.10        156.         85.         49.       7.26

         ROUTED TO
+                        R10-11       171.    9.20        156.         85.         48.       7.26

         HYDROGRAPH AT
+                          SB11        77.    4.10          9.          2.          1.       0.07

         2 COMBINED AT
+                         CP-11       171.    9.20        156.         85.         49.       7.33
1
                                              SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
                                                  (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)
                                                                                    INTERPOLATED TO
                                                                                 COMPUTATION INTERVAL
           ISTAQ    ELEMENT      DT       PEAK    TIME TO     VOLUME      DT       PEAK    TIME TO     VOLUME
                                                   PEAK                                     PEAK

                                (MIN)     (CFS)     (MIN)     (IN)      (MIN)     (CFS)     (MIN)       (IN)

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1A-2W  MANE         0.28     86.74    255.73      0.51      3.00     86.50    258.00        0.51

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8110E+01 EXCESS=0.0000E+00 OUTFLOW=0.8108E+01 BASIN STORAGE=0.2428E-02 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1A-2W  MANE         0.22     86.23    255.60      0.50      3.00     85.99    258.00        0.50

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8082E+01 EXCESS=0.0000E+00 OUTFLOW=0.8081E+01 BASIN STORAGE=0.2428E-02 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1A-2W  MANE         0.23     56.86    261.41      0.46      3.00     56.60    261.00        0.46

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7389E+01 EXCESS=0.0000E+00 OUTFLOW=0.7388E+01 BASIN STORAGE=0.2428E-02 PERCENT ERROR=   0.0


          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2W-2  MANE         0.41     97.65    258.39      0.54      3.00     97.55    258.00        0.54

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8773E+01 EXCESS=0.0000E+00 OUTFLOW=0.8771E+01 BASIN STORAGE=0.3356E-02 PERCENT ERROR=   0.0
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          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R2W-2  MANE         0.41     96.96    258.16      0.54      3.00     96.86    258.00         0.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8738E+01 EXCESS=0.0000E+00 OUTFLOW=0.8735E+01 BASIN STORAGE=0.3354E-02 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R2W-2  MANE         0.36     63.48    261.95      0.48      3.00     62.70    264.00         0.48

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7822E+01 EXCESS=0.0000E+00 OUTFLOW=0.7820E+01 BASIN STORAGE=0.3350E-02 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1C-1B  MANE         0.47     26.07    249.94      0.64      3.00     25.45    252.00         0.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9235E+00 EXCESS=0.0000E+00 OUTFLOW=0.9234E+00 BASIN STORAGE=0.1159E-13 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1C-1B  MANE         0.40     25.48    250.16      0.63      3.00     25.16    252.00         0.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9114E+00 EXCESS=0.0000E+00 OUTFLOW=0.9112E+00 BASIN STORAGE=0.1161E-13 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1C-1B  MANE         0.40     12.02    255.91      0.42      3.00     11.53    258.00         0.42

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6045E+00 EXCESS=0.0000E+00 OUTFLOW=0.6050E+00 BASIN STORAGE=0.1171E-13 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1B-2N  MANE         1.00     48.05    253.91      0.64      3.00     47.04    252.00         0.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1748E+01 EXCESS=0.0000E+00 OUTFLOW=0.1752E+01 BASIN STORAGE=0.3293E-11 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1B-2N  MANE         0.96     47.49    253.51      0.63      3.00     46.32    252.00         0.63

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1727E+01 EXCESS=0.0000E+00 OUTFLOW=0.1726E+01 BASIN STORAGE=0.3440E-11 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1B-2N  MANE         1.20     21.35    259.25      0.42      3.00     21.16    258.00         0.42

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1144E+01 EXCESS=0.0000E+00 OUTFLOW=0.1146E+01 BASIN STORAGE=0.3398E-11 PERCENT ERROR=  -0.2

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2N-2  MANE         0.38     73.38    255.29      0.64      3.00     73.19    255.00         0.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2890E+01 EXCESS=0.0000E+00 OUTFLOW=0.2895E+01 BASIN STORAGE=0.5835E-06 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R2N-2  MANE         0.42     72.58    255.46      0.63      3.00     72.21    255.00         0.63

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2852E+01 EXCESS=0.0000E+00 OUTFLOW=0.2855E+01 BASIN STORAGE=0.4437E-06 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R2N-2  MANE         0.51     34.33    261.97      0.42      3.00     34.12    261.00         0.42

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1894E+01 EXCESS=0.0000E+00 OUTFLOW=0.1895E+01 BASIN STORAGE=0.4997E-06 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R2-3  MANE         2.42    211.40    259.77      1.55      3.00    210.49    261.00         1.55
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1506E+02 EXCESS=0.0000E+00 OUTFLOW=0.1503E+02 BASIN STORAGE=0.3148E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R2-3  MANE         2.38    209.22    260.63      1.54      3.00    208.79    261.00        1.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1494E+02 EXCESS=0.0000E+00 OUTFLOW=0.1492E+02 BASIN STORAGE=0.3148E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R2-3  MANE         2.90    122.62    266.89      1.22      3.00    122.54    267.00        1.22

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1184E+02 EXCESS=0.0000E+00 OUTFLOW=0.1186E+02 BASIN STORAGE=0.3148E-01 PERCENT ERROR=  -0.4

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R3-5  MANE         3.00    238.72    269.68      1.48      3.00    238.21    270.00        1.48

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1817E+02 EXCESS=0.0000E+00 OUTFLOW=0.1812E+02 BASIN STORAGE=0.6963E-01 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R3-5  MANE         3.00    233.59    269.44      1.47      3.00    233.56    270.00        1.47

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1804E+02 EXCESS=0.0000E+00 OUTFLOW=0.1798E+02 BASIN STORAGE=0.6661E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R3-5  MANE         3.00    140.70    278.84      1.18      3.00    140.60    279.00        1.18

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1453E+02 EXCESS=0.0000E+00 OUTFLOW=0.1444E+02 BASIN STORAGE=0.7428E-01 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7W-8W  MANE         1.68     52.79    276.79      0.96      3.00     51.01    279.00        0.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1600E+01 EXCESS=0.0000E+00 OUTFLOW=0.2399E+01 BASIN STORAGE=0.2981E-03 PERCENT ERROR= -50.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R7W-8W  MANE         1.69     52.24    276.99      0.95      3.00     50.66    279.00        0.95

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1579E+01 EXCESS=0.0000E+00 OUTFLOW=0.2373E+01 BASIN STORAGE=0.2851E-03 PERCENT ERROR= -50.4

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R7W-8W  MANE         0.57     23.36    290.76      0.62      3.00     23.27    291.00        0.63

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1047E+01 EXCESS=0.0000E+00 OUTFLOW=0.1550E+01 BASIN STORAGE=0.2509E-03 PERCENT ERROR= -48.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NW8N  MANE         0.90     23.67    245.12      1.12      3.00     23.61    246.00        1.12

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8762E+00 EXCESS=0.0000E+00 OUTFLOW=0.8785E+00 BASIN STORAGE=0.1427E-06 PERCENT ERROR=  -0.3

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8NW8N  MANE         0.89     23.51    245.06      1.11      3.00     23.45    246.00        1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8684E+00 EXCESS=0.0000E+00 OUTFLOW=0.8709E+00 BASIN STORAGE=0.1495E-06 PERCENT ERROR=  -0.3

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8NW8N  MANE         0.99     12.76    245.64      1.01      3.00     12.75    246.00        1.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7895E+00 EXCESS=0.0000E+00 OUTFLOW=0.7947E+00 BASIN STORAGE=0.1506E-06 PERCENT ERROR=  -0.7
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          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NE8N  MANE         0.90     25.10    244.37      1.12      3.00     24.86    246.00        1.12

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8929E+00 EXCESS=0.0000E+00 OUTFLOW=0.8983E+00 BASIN STORAGE=0.1026E-06 PERCENT ERROR=  -0.6

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8NE8N  MANE         0.87     24.95    244.34      1.11      3.00     24.69    246.00        1.11

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8849E+00 EXCESS=0.0000E+00 OUTFLOW=0.8866E+00 BASIN STORAGE=0.1149E-06 PERCENT ERROR=  -0.2

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8NE8N  MANE         1.02     13.29    245.08      1.01      3.00     13.20    246.00        1.01

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8044E+00 EXCESS=0.0000E+00 OUTFLOW=0.8070E+00 BASIN STORAGE=0.1185E-06 PERCENT ERROR=  -0.3

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8N-8W  MANE         0.50     90.26    244.05      1.12      3.00     89.79    246.00        1.12

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3323E+01 EXCESS=0.0000E+00 OUTFLOW=0.3326E+01 BASIN STORAGE=0.1332E-07 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8N-8W  MANE         0.50     89.65    244.04      1.11      3.00     89.18    246.00        1.11

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3296E+01 EXCESS=0.0000E+00 OUTFLOW=0.3298E+01 BASIN STORAGE=0.1340E-07 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8N-8W  MANE         0.71     48.48    244.61      1.01      3.00     48.33    246.00        1.01

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2995E+01 EXCESS=0.0000E+00 OUTFLOW=0.2995E+01 BASIN STORAGE=0.1327E-07 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R8W-4  MANE         0.74    171.86    247.74      1.06      3.00    169.25    249.00        1.06

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9153E+01 EXCESS=0.0000E+00 OUTFLOW=0.9156E+01 BASIN STORAGE=0.1251E-03 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R8W-4  MANE         0.75    170.53    247.11      1.05      3.00    168.12    249.00        1.05

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9058E+01 EXCESS=0.0000E+00 OUTFLOW=0.9060E+01 BASIN STORAGE=0.1245E-03 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R8W-4  MANE         0.85     96.14    247.40      0.89      3.00     95.60    249.00        0.89

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7653E+01 EXCESS=0.0000E+00 OUTFLOW=0.7655E+01 BASIN STORAGE=0.1199E-03 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R4-5  MANE         2.78    341.92    253.01      1.19      3.00    340.48    252.00        1.19

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1923E+02 EXCESS=0.0000E+00 OUTFLOW=0.1934E+02 BASIN STORAGE=0.3703E-03 PERCENT ERROR=  -0.6

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R4-5  MANE         2.78    338.84    253.21      1.18      3.00    337.96    252.00        1.18

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1906E+02 EXCESS=0.0000E+00 OUTFLOW=0.1916E+02 BASIN STORAGE=0.4184E-03 PERCENT ERROR=  -0.5

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R4-5  MANE         3.00    193.48    252.47      1.01      3.00    192.50    252.00        1.01
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1647E+02 EXCESS=0.0000E+00 OUTFLOW=0.1651E+02 BASIN STORAGE=0.3235E-03 PERCENT ERROR=  -0.2

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R5-6  MANE         2.88    465.68    257.00      1.28      3.00    461.39    258.00        1.28

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4510E+02 EXCESS=0.0000E+00 OUTFLOW=0.4501E+02 BASIN STORAGE=0.5641E-01 PERCENT ERROR=   0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R5-6  MANE         2.88    462.64    257.18      1.27      3.00    458.66    258.00        1.27

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4472E+02 EXCESS=0.0000E+00 OUTFLOW=0.4464E+02 BASIN STORAGE=0.5641E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R5-6  MANE         3.00    269.39    286.19      1.06      3.00    264.39    285.00        1.06

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3723E+02 EXCESS=0.0000E+00 OUTFLOW=0.3723E+02 BASIN STORAGE=0.5640E-01 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7E-8E  MANE         2.48      8.47    267.86      0.41      3.00      8.45    270.00        0.41

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5640E+00 EXCESS=0.0000E+00 OUTFLOW=0.5642E+00 BASIN STORAGE=0.1424E-04 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R7E-8E  MANE         2.49      8.36    268.11      0.40      3.00      8.32    270.00        0.40

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5555E+00 EXCESS=0.0000E+00 OUTFLOW=0.5565E+00 BASIN STORAGE=0.1227E-04 PERCENT ERROR=  -0.2

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R7E-8E  MANE         3.00      0.79    281.23      0.04      3.00      0.79    282.00        0.04

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5136E-01 EXCESS=0.0000E+00 OUTFLOW=0.5564E-01 BASIN STORAGE=0.1218E-04 PERCENT ERROR=  -8.4

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R9NR8E  MANE         3.00      6.31    281.03      0.51      3.00      6.30    279.00        0.51

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7576E+00 EXCESS=0.0000E+00 OUTFLOW=0.7564E+00 BASIN STORAGE=0.1663E-04 PERCENT ERROR=   0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R9NR8E  MANE         3.00      6.24    281.18      0.50      3.00      6.22    279.00        0.50

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7487E+00 EXCESS=0.0000E+00 OUTFLOW=0.7475E+00 BASIN STORAGE=0.1891E-04 PERCENT ERROR=   0.2

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R9NR8E  MANE         3.00      1.69    287.45      0.14      3.00      1.68    288.00        0.14

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2051E+00 EXCESS=0.0000E+00 OUTFLOW=0.2049E+00 BASIN STORAGE=0.1591E-04 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8E-R9  MANE         0.31     34.65    276.05      0.43      3.00     34.65    276.00        0.43

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3043E+01 EXCESS=0.0000E+00 OUTFLOW=0.3043E+01 BASIN STORAGE=0.7386E-05 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8E-R9  MANE         0.31     34.16    276.08      0.43      3.00     34.16    276.00        0.43

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3001E+01 EXCESS=0.0000E+00 OUTFLOW=0.3002E+01 BASIN STORAGE=0.7554E-05 PERCENT ERROR=   0.0
```

```
          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8E-R9  MANE        0.73      4.44    286.10      0.06      3.00      4.42    285.00        0.06

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4419E+00 EXCESS=0.0000E+00 OUTFLOW=0.4419E+00 BASIN STORAGE=0.7013E-05 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT1  MANE        0.43    176.85    541.04      0.53      3.00    176.85    543.00        0.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2055E+03 EXCESS=0.0000E+00 OUTFLOW=0.2055E+03 BASIN STORAGE=0.2436E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            BSNRT1  MANE        0.43    176.79    541.14      0.53      3.00    176.79    543.00        0.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2054E+03 EXCESS=0.0000E+00 OUTFLOW=0.2054E+03 BASIN STORAGE=0.2436E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            BSNRT1  MANE        0.47    172.81    544.06      0.52      3.00    172.80    543.00        0.52

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2012E+03 EXCESS=0.0000E+00 OUTFLOW=0.2012E+03 BASIN STORAGE=0.2435E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE        0.44    176.85    543.47      0.53      3.00    176.85    543.00        0.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2055E+03 EXCESS=0.0000E+00 OUTFLOW=0.2054E+03 BASIN STORAGE=0.2781E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            BSNRT2  MANE        0.44    176.78    543.55      0.53      3.00    176.78    543.00        0.53

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2054E+03 EXCESS=0.0000E+00 OUTFLOW=0.2054E+03 BASIN STORAGE=0.2781E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            BSNRT2  MANE        0.48    172.80    544.14      0.52      3.00    172.80    546.00        0.52

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2012E+03 EXCESS=0.0000E+00 OUTFLOW=0.2011E+03 BASIN STORAGE=0.2781E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R10-11  MANE        3.46    176.84    548.85      0.54      3.00    176.84    549.00        0.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2080E+03 EXCESS=0.0000E+00 OUTFLOW=0.2075E+03 BASIN STORAGE=0.5529E+00 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R10-11  MANE        3.45    176.78    548.95      0.54      3.00    176.78    549.00        0.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2079E+03 EXCESS=0.0000E+00 OUTFLOW=0.2074E+03 BASIN STORAGE=0.5529E+00 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R10-11  MANE        3.43    172.80    553.00      0.52      3.00    172.80    552.00        0.52

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2033E+03 EXCESS=0.0000E+00 OUTFLOW=0.2028E+03 BASIN STORAGE=0.5528E+00 PERCENT ERROR=   0.0


***    1 ERROR(S) DETECTED BY HEC-1 ***
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100yr-6hr-Ultimate

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1*****************************************
**************************************
 *                                       *                                                    *
*
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                    *    U.S. ARMY CORPS OF ENGINEERS
*
 *             JUN   1998                *                                                    *    HYDROLOGIC ENGINEERING CENTER
*
 *          VERSION 4.1                  *                                                    *          609 SECOND STREET
*
 *                                       *                                                    *       DAVIS, CALIFORNIA 95616
*
 *  RUN DATE   26APR21  TIME  08:56:05   *                                                    *           (916) 756-1104
*
 *                                       *                                                    *
*
 *****************************************
**************************************



                                                    X     X  XXXXXXX   XXXXX          X
                                                    X     X  X        X     X        XX
                                                    X     X  X        X               X
                                                    XXXXXXX  XXXX     X        XXXXX  X
                                                    X     X  X        X               X
                                                    X     X  X        X     X         X
                                                    X     X  XXXXXXX   XXXXX         XXX



            THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

            THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
            THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
            NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
            DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
            KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM
```

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1                                                   HEC-1 INPUT                                                  PAGE  1

           LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

              1           ID        CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
              2           ID        POWERLINE CORRIDOR
              3           ID        RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
              4           ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
              5           ID       100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
              6           ID           PIMA ROAD FLOWS.
              7           ID
              8           ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
              9           ID       MODELERS: RK, MW
             10           ID
             11           ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
             12           ID       MODELERS: MSW, ES
             13           ID       REVISIONS NOTED WITH HE
             14           ID           ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
             15           ID           THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
             16           ID           UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.
             17           ID
             18           ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
             19           ID       MODELERS: MSW
             20           ID       XKSAT ADJUSTMENT
             21           ID
             22           ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
             23           ID       MODELERS: MSW, TSW
             24           ID       REVISIONS NOTED WITH HE (Hubbard Engineering)
             25           ID           NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
             26           ID           CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
             27           ID           ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
             28           ID           FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED
             29           ID
             30           IT       3       0       0    1000
             31           IN      15
             32           IO       5
                          *DIAGRAM
```

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               *
     33        JD   2.755  0.0001
     34        PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
     35        PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
     36        PC   0.962   0.972   0.983   0.991   1.000
     37        JD   2.738  0.5000
     38        PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.074
     39        PC   0.087   0.099   0.118   0.138   0.216   0.377   0.834   0.911   0.931   0.950
     40        PC   0.962   0.972   0.983   0.991   1.000
     41        JD   2.686     2.8
     42        PC   0.000   0.009   0.016   0.025   0.034   0.042   0.051   0.059   0.068   0.077
     43        PC   0.088   0.101   0.121   0.164   0.253   0.451   0.694   0.836   0.900   0.938
     44        PC   0.950   0.963   0.975   0.988   1.000
     45        JD   2.540    16.0
     46        PC   0.000   0.015   0.020   0.030   0.048   0.063   0.076   0.090   0.105   0.119
     47        PC   0.135   0.152   0.175   0.222   0.304   0.472   0.670   0.796   0.868   0.912
     48        PC   0.946   0.960   0.973   0.987   1.000
     49        JD   2.232    90.0
     50        PC   0.000   0.021   0.035   0.051   0.071   0.087   0.105   0.125   0.143   0.160
     51        PC   0.179   0.201   0.232   0.281   0.364   0.500   0.658   0.773   0.841   0.888
     52        PC   0.927   0.945   0.964   0.982   1.000
     53        JD   3.657    10.0
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   LINE        ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

     54        JD   3.533    20.0
               *

     55        KK  76THST
     56        KM       76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
     57        KM       100-YR, 6-HR FLO-2D MODEL (XS 98)
     58        BA    0.24
     59        QI       0       0       0       0       0       0       0       0       0       0
     60        QI       0       0    0.01    0.01    0.09     0.9    12.7   42.26   49.92    48.6
     61        QI   34.49   26.72   21.55   17.51   14.81   12.84   11.63   10.56    9.58    8.32
     62        QI    7.29    6.73    5.83    4.76    4.23    3.81     3.4    3.55    2.36    2.51
     63        QI    2.32    2.16    2.02    1.83    1.58     1.5    1.52    1.21    1.31    1.34
     64        QI    1.09    1.07    1.04    0.97    0.91    0.86    0.83    0.79    0.76    0.72
     65        QI    0.69    0.67    0.65    0.62     0.6    0.58    0.55    0.53    0.51    0.49
     66        QI    0.47    0.45    0.44
               *

     67        KK   SB01A   BASIN
     68        BA   0.061
     69        PB   2.721
     70        PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.075
     71        PC   0.087   0.099   0.119   0.150   0.234   0.413   0.766   0.875   0.916   0.944
     72        PC   0.956   0.968   0.979   0.990   1.000
     73        LG    0.25    0.25    4.03    0.56      45
     74        UC   0.439   0.231
     75        UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
     76        UA     100
               *

     77        KK   SB01A  DIVERT
     78        KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01A
     79        KM        FIRST FLUSH VOL = 1.383 AC-FT
     80        DT  RET01A   1.383
     81        DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
     82        DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

     83        KK    CP-1 COMBINE
     84        KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
     85        HC       2   0.301
               *

     86        KK  R1A-2W   ROUTE
     87        KM        ROUTE CP-1 TO CP-2W
     88        RK    1114  0.0200   0.018           TRAP   2.000    0.00
               *

     89        KK  SB02NW   BASIN
     90        KM        BASIN SB02E WAS SPLIT INTO SUB BASINS NORTH OF CAVASSON
     91        KM        AND SOUTH OF CAVASSON
     92        BA  0.0193
     93        LG    0.25    0.25    4.03    0.56      45
     94        UC   0.172   0.133
     95        UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
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  96           UA     100
               *
1                                             HEC-1 INPUT                                               PAGE  3

         LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  97           KK  SB02NW  DIVERT
  98           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NW
  99           KM        FIRST FLUSH VOL = 0.437 AC-FT
 100           DT  RET2NW   0.437
 101           DI     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
 102           DI    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
 103           DQ     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
 104           DQ    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
               *

 105           KK  CP-02W COMBINE
 106           KM        COMBINE ROUTE FROM 1A-2W WITH NEW SUB BASIN SB02NW
 107           HC       2   0.305
               *

 108           KK   R2W-2   ROUTE
 109           RK    1380  0.0140   0.018            TRAP   2.000    0.00
               *

 110           KK   SB01C   BASIN
 111           BA   0.027
 112           LG    0.25    0.25    4.03    0.56      45
 113           UC   0.215   0.216
 114           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 115           UA     100
               *

 116           KK   SB01C  DIVERT
 117           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01C
 118           KM        FIRST FLUSH VOL = 0.612 AC-FT
 119           DT  RET01C   0.612
 120           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 121           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 122           KK  R1C-1B   ROUTE
 123           KM        KO     2       2
 124           RK     542   0.005   0.013            CIRC   5.000
               *

 125           KK   SB01B   BASIN
 126           BA   0.024
 127           LG    0.25    0.25    4.03    0.56      45
 128           UC   0.191   0.178
 129           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 130           UA     100
               *

 131           KK   SB01B  DIVERT
 132           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01B
 133           KM        FIRST FLUSH VOL = 0.543 AC-FT
 134           DT  RET01B   0.543
 135           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 136           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *
1                                             HEC-1 INPUT                                               PAGE  4

         LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 137           KK  CP-01B COMBINE
 138           KM        COMBINE ROUTE FROM 1C-1B WITH NEW SUB BASIN SB01B
 139           HC       2   0.051
               *

 140           KK  R1B-2N   ROUTE
 141           KM     KO     2       2
 142           RK    2144   0.008   0.013            CIRC   5.000
               *

 143           KK  SB02NE   BASIN
 144           BA  0.0335
 145           LG    0.25    0.25    4.03    0.56      45
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 146           UC   0.255   0.267
 147           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 148           UA     100
               *

 149           KK  SB02NE  DIVERT
 150           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NE
 151           KM        FIRST FLUSH VOL = 0.760 AC-FT
 152           DT  RET2NE   0.760
 153           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 154           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 155           KK  CP-02N COMBINE
 156           KM        COMBINE ROUTE 1B-2N (BASINS SB01-B AND SB01-C) WITH SB02NE
 157           HC       2   0.085
               *

 158           KK   R2N-2   ROUTE
 159           RK    1380   0.014   0.018            TRAP   2.000    0.00
               *

 160           KK   SB02E   BASIN
 161           BA   0.040
 162           LG    0.10    0.25    4.03    0.61      80
 163           UC   0.214   0.175
 164           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 165           UA     100
               *

 166           KK   SB02E  DIVERT
 167           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E
 168           KM        FIRST FLUSH VOL = 0.998 AC-FT
 169           DT   RET2E   0.998
 170           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 171           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *
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LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 172           KK   SB02W   BASIN
 173           BA  0.0571
 174           LG    0.35    0.35    4.33    0.51       0
 175           UC   0.686   0.535
 176           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 177           UA     100
               *

 178           KK   CP-02 COMBINE
 179           HC       4  0.1816
               *

 180           KK    R2-3   ROUTE
 181           KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
 182           RK    1260  0.0015   0.030            TRAP      92       4
               *

 183           KK    SB03   BASIN
 184           BA   0.048
 185           LG    0.15    0.25    4.50    0.47      55
 186           UC   0.364   0.294
 187           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 188           UA     100
               *

 189           KK   CP-03 COMBINE
 190           HC       2
               *

 191           KK    R3-5   ROUTE
 192           KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
 193           RK    2396  0.0015   0.03             TRAP      98       4
               *

 194           KK    SB05   BASIN
 195           BA   0.126
 196           LG    0.22    0.25    4.50    0.44      48
 197           UC   0.327   0.226
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 198           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 199           UA     100
               *

 200           KK   CP-5A COMBINE
 201           HC       2
               *

 202           KK   SB07W   BASIN
 203           BA  0.0468
 204           LG    0.25    0.25    4.03    0.56      45
 205           UC   0.203   0.141
 206           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 207           UA     100
               *
```



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 208           KK   SB07W  DIVERT
 209           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB07W
 210           KM        FIRST FLUSH VOL = 1.060 AC-FT
 211           DT  RET07W   1.060
 212           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 213           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 214           KK  R7W-8W   ROUTE
 215           KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 216           KM        HE MODIFIED ROUTE TO LOOP 101 W OF HAYDEN
 217           RK    3754  0.0013   0.03             TRAP      46       4
               *

 218           KK   SB8NW   BASIN
 219           KM        BASIN JUST SOUTH OF LEGACY - FUTURE MOB SITE
 220           BA  0.0147
 221           LG    0.10    0.25    4.03    0.61      80
 222           UC   0.144   0.108
 223           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 224           UA     100
               *

 225           KK   SB8NW  DIVERT
 226           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NW
 227           KM        FIRST FLUSH VOL = 0.332 AC-FT
 228           DT  RET8NW   0.332
 229           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 230           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 231           KK  R8NW8N   ROUTE
 232           KM        ROUTE NEW SUB BASIN SB8NW TO OUTLET AT CAVASSON BLVD
 233           KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 234           RK     643  0.0113   0.018            TRAP      46       4
               *

 235           KK   SB8NE   BASIN
 236           KM        SITE JUST SOUTH OF LEGACY AND WEST OF HAYDEN - FUTURE RETAIL
 237           BA   0.015
 238           LG    0.10    0.25    4.03    0.61      80
 239           UC   0.138   0.094
 240           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 241           UA     100
               *

 242           KK   SB8NE  DIVERT
 243           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NE
 244           KM        FIRST FLUSH VOL = 0.340 AC-FT
 245           DT  RET8NE   0.340
 246           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 247           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *
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 248           KK  R8NE8N   ROUTE
 249           KM        ROUTE NEW SUB BASIN SB8NE TO OUTLET AT CAVASSON BLVD
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 250            KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 251            RK     622   0.011   0.018            TRAP      46       4
                *

 252            KK   SB8SW   BASIN
 253            KM        SITE JUST NORTH OF CAVASSON BLVD EAST OF CLARET DR - FUTURE MF
 254            BA  0.0129
 255            LG    0.10    0.25    4.03    0.61      80
 256            UC   0.126   0.091
 257            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 258            UA     100
                *

 259            KK   SB8SW  DIVERT
 260            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SW
 261            KM        FIRST FLUSH VOL = 0.292 AC-FT
 262            DT  RET8SW   0.292
 263            DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 264            DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                *

 265            KK   SB8SE   BASIN
 266            KM        SITE JUST NORTH OF CAVASSON AND WEST OF HAYDEN - FUTURE HOTEL OR RETAIL
 267            BA  0.0131
 268            LG    0.10    0.25    4.03    0.61      80
 269            UC   0.139   0.113
 270            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 271            UA     100
                *

 272            KK   SB8SE  DIVERT
 273            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SE
 274            KM        FIRST FLUSH VOL = 0.297 AC-FT
 275            DT  RET8SE   0.297
 276            DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 277            DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                *

 278            KK  CP-08N COMBINE
 279            KM        CP AT INLET ON NORTH SIDE OF CAVASSON - ENTERS EXIST STORM DRAIN
 280            HC       4  0.0557
                *

 281            KK  R8N-8W   ROUTE
 282            KM        ROUTE NEW SUB BASINS SB8NE SB8NW SB8SW SB8SE TO CP-08W
 283            KM        ROUTE IN STORM DRAIN
 284            RK    1592   0.015   0.013            CIRC       3
                *
1                                            HEC-1 INPUT                                              PAGE  8

LINE            ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 285            KK   SB08W   BASIN
 286            BA  0.0589
 287            LG    0.10    0.25    4.03    0.61      80
 288            UC   0.221   0.146
 289            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 290            UA     100
                *

 291            KK   SB08W  DIVERT
 292            KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB08W
 293            KM        FIRST FLUSH VOL REQ = 1.335 AC-FT, PROVIDED = 1.415
 294            KM        THIS IS EXISTING AND WAS BUILT IN PHASE 1
 295            DT   RET8W   1.415
 296            DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 297            DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                *

 298            KK  CP-08W COMBINE
 299            HC       3  0.1614
                *
                *

 300            KK   R8W-4   ROUTE
 301            RK    3137  0.0130   0.030            TRAP   2.000    0.00
                *

 302            KK    SB04   BASIN
```

```
 303            BA   0.144
 304            LG    0.14    0.25    4.60    0.44      61
 305            UC   0.305   0.199
 306            UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 307            UA     100
                *

 308            KK    CP-4 COMBINE
 309            HC       2
                *

 310            KK    R4-5   ROUTE
 311            KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
 312            RK    1992  0.0013   0.03             TRAP      39       4
                *

 313            KK   CP-05 COMBINE
 314            KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
 315            KM        COMBINE CP-05A AND R4-5
 316            HC       2
                *
```



```
LINE            ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 317            KK    R5-6   ROUTE
 318            KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
 319            RK    1694  0.0015   0.03             TRAP     188       4
                *

 320            KK    SB06   BASIN
 321            BA   0.136
 322            LG    0.16    0.25    4.55    0.45      53
 323            UC   0.321   0.246
 324            UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 325            UA     100
                *

 326            KK   CP-06 COMBINE
 327            HC       2
                *

 328            KK   SB07E   BASIN
 329            BA   0.026
 330            LG    0.10    0.25    4.03    0.74       0
 331            UC   0.574   0.527
 332            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 333            UA     100
                *

 334            KK  R7E-8E   ROUTE
 335            RK    3662  0.0130   0.030            TRAP   2.000    0.00
                *

 336            KK   SB08E   BASIN
 337            BA   0.078
 338            LG    0.10    0.25    4.08    0.72       0
 339            UC   0.798   0.652
 340            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 341            UA     100
                *

 342            KK   SB09N   BASIN
 343            BA   0.028
 344            LG    0.10    0.25    4.79    0.49       0
 345            UC   0.740   1.111
 346            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 347            UA     100
                *

 348            KK  R9N-R9   ROUTE
 349            RK    2586  0.0013   0.020            TRAP   2.000    0.00
                *
```



```
LINE            ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 350            KK  CP-08E COMBINE
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```
 351           HC       3
               *

 352           KK  R8E-R9   ROUTE
 353           RK     501  0.0013   0.020            TRAP   2.000    0.00
               *

 354           KK    SB09   BASIN
 355           BA   0.086
 356           LG    0.10    0.25    4.79    0.49       0
 357           UC   0.683   0.518
 358           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 359           UA     100
               *

 360           KK   CP-09 COMBINE
 361           HC       2
               *

 362           KK   PWRCH
 363           KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
 364           KM        100-YR, 6-HR FLO-2D MODEL (XS 107)
 365           BA     7.0
 366           QI       0       0       0       0       0       0       0       0       0       0
 367           QI       0       0       0       0       0    0.11   17.74   50.91  433.53  391.08
 368           QI  359.13  391.64  616.52  737.85  699.64  645.41  621.52  599.48  594.12  570.84
 369           QI  525.86  472.45  419.04  375.56     335  299.45   270.9  235.68  211.13  196.81
 370           QI  172.83  155.83  146.04  130.32   120.9  110.32   99.25    90.9   82.93   77.68
 371           QI    79.3   67.58    63.5   64.86   56.01   51.64   48.75   43.99   42.04   40.99
 372           QI   37.99   36.19   33.42   31.56   29.22   28.13   27.15   31.57   24.31    23.1
 373           QI   22.09   21.66   20.95
               *

 374           KK  BINFLO
 375           KM         TOTAL INFLOW INTO BASIN 53R.
 376           HC       2
               *

 377           KK   BASIN
 378           KM    BASIN 53R STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
 379           KM    ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
 380           KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
 381           KM        THE OUTLET PIPES ARE INLET CONTROLLED.
 382           RS       1    STOR       0
 383           SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
 384           SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
 385           SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
 386           SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
 387           SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
 388           SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
               *
1                                       HEC-1 INPUT                                            PAGE 11

LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 389           KK  BSNRT1
 390           KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
 391           KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
 392           KM        EXCESS CAPACITY.
 393           RK     550  0.0052   0.024            CIRC     7.0
               *

 394           KK  BSNRT2
 395           KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BAS 53R OUTFALL)
 396           RK    1200  0.0077   0.013            CIRC     7.0
               *

 397           KK    SB10   BASIN
 398           BA   0.040
 399           LG    0.15    0.25    4.25    0.55      55
 400           UC   0.233   0.161
 401           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 402           UA     100
               *

 403           KK   CP-10 COMBINE
 404           KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
 405           HC       2   0.229
               *
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            406           KK  R10-11   ROUTE
            407           KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
            408           RK    1277  0.0014   0.03             TRAP     220       4
                          *

            409           KK    SB11   BASIN
            410           BA   0.071
            411           LG    0.15    0.25    4.15    0.58      55
            412           UC   0.296   0.232
            413           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
            414           UA     100
                          *

            415           KK   CP-11 COMBINE
            416           KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
            417           HC       2     0.3
                          *
            418           ZZ
1
                 SCHEMATIC DIAGRAM OF STREAM NETWORK
 INPUT
  LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

   NO.      (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

    55      76THST
                 .
                 .
    67           .       SB01A
                 .           .
                 .           .
    80           .           .------->  RET01A
    77           .       SB01A
                 .           .
                 .           .
    83        CP-1............
                 V
                 V
    86      R1A-2W
                 .
                 .
    89           .      SB02NW
                 .           .
                 .           .
   100           .           .------->  RET2NW
    97           .      SB02NW
                 .           .
                 .           .
   105      CP-02W............
                 V
                 V
   108       R2W-2
                 .
                 .
   110           .       SB01C
                 .           .
                 .           .
   119           .           .------->  RET01C
   116           .       SB01C
                 .           V
                 .           V
   122           .      R1C-1B
                 .           .
                 .           .
   125           .           .       SB01B
                 .           .           .
                 .           .           .
   134           .           .           .------->  RET01B
   131           .           .       SB01B
                 .           .           .
                 .           .           .
   137           .      CP-01B............
                 .           V
                 .           V
   140           .      R1B-2N
                 .           .
                 .           .
   143           .           .      SB02NE
                 .           .           .
```

```
              .           .           .
152           .           .           .------->  RET2NE
149           .           .      SB02NE
              .           .           .
              .           .           .
155           .      CP-02N............
              .           V
              .           V
158           .       R2N-2
              .           .
              .           .
160           .           .       SB02E
              .           .           .
              .           .           .
169           .           .           .------->   RET2E
166           .           .       SB02E
              .           .           .
              .           .           .
172           .           .           .       SB02W
              .           .           .           .
              .           .           .           .
178       CP-02....................................
              V
              V
180        R2-3
              .
              .
183           .        SB03
              .           .
              .           .
189       CP-03............
              V
              V
191        R3-5
              .
              .
194           .        SB05
              .           .
              .           .
200       CP-5A............
              .
              .
202           .       SB07W
              .           .
              .           .
211           .           .------->  RET07W
208           .       SB07W
              .           V
              .           V
214           .      R7W-8W
              .           .
              .           .
218           .           .       SB8NW
              .           .           .
              .           .           .
228           .           .           .------->  RET8NW
225           .           .       SB8NW
              .           .           V
              .           .           V
231           .           .      R8NW8N
              .           .           .
              .           .           .
235           .           .           .       SB8NE
              .           .           .           .
              .           .           .           .
245           .           .           .           .------->  RET8NE
242           .           .           .       SB8NE
              .           .           .           V
              .           .           .           V
248           .           .           .      R8NE8N
              .           .           .           .
              .           .           .           .
252           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
262           .           .           .           .           .------->  RET8SW
259           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
265           .           .           .           .           .       SB8SE
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              .           .           .           .           .           .
              .           .           .           .           .           .
275           .           .           .           .           .           .------->  RET8SE
272           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
278           .           .      CP-08N....................................
              .           .           V
              .           .           V
281           .           .      R8N-8W
              .           .           .
              .           .           .
285           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
295           .           .           .           .------->   RET8W
291           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
298           .      CP-08W........................
              .           V
              .           V
300           .       R8W-4
              .           .
              .           .
302           .           .        SB04
              .           .           .
              .           .           .
308           .        CP-4............
              .           V
              .           V
310           .        R4-5
              .           .
              .           .
313       CP-05............
              V
              V
317        R5-6
              .
              .
320           .        SB06
              .           .
              .           .
326       CP-06............
              .
              .
328           .       SB07E
              .           V
              .           V
334           .      R7E-8E
              .           .
              .           .
336           .           .       SB08E
              .           .           .
              .           .           .
342           .           .           .       SB09N
              .           .           .           V
              .           .           .           V
348           .           .           .      R9N-R9
              .           .           .           .
              .           .           .           .
350           .      CP-08E........................
              .           V
              .           V
352           .      R8E-R9
              .           .
              .           .
354           .           .        SB09
              .           .           .
              .           .           .
360           .       CP-09............
              .           .
              .           .
362           .           .       PWRCH
              .           .           .
              .           .           .
374           .      BINFLO............
              .           V
              .           V
377           .       BASIN
```

```
               .          V
               .          V
   389         .      BSNRT1
               .          V
               .          V
   394         .      BSNRT2
               .          .
               .          .
   397         .          .        SB10
               .          .           .
               .          .           .
   403         .       CP-10...........
               .          V
               .          V
   406         .      R10-11
               .          .
               .          .
   409         .          .        SB11
               .          .           .
               .          .           .
   415         .       CP-11...........

 (***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1***************************************
***************************************
 *                                       *                                                    *
*
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                    *    U.S. ARMY CORPS OF ENGINEERS
*
 *             JUN   1998                *                                                    *    HYDROLOGIC ENGINEERING CENTER
*
 *          VERSION 4.1                  *                                                    *          609 SECOND STREET
*
 *                                       *                                                    *       DAVIS, CALIFORNIA 95616
*
 *  RUN DATE   26APR21  TIME  08:56:05   *                                                    *           (916) 756-1104
*
 *                                       *                                                    *
*
 ***************************************
***************************************



                                    CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
                                    POWERLINE CORRIDOR
                                    RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                                    INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                                       100-YR, 6-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                                       PIMA ROAD FLOWS.

                                    PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                                       MODELERS: RK, MW

                                    REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 11/14/18
                                       MODELERS: MSW, ES
                                       REVISIONS NOTED WITH HE
                                       ULTIMATE CONDITION MODEL,WITH POWERLINE AND BASIN 53R AND
                                       THIS IS THE POST-DEVELOPMENT MODEL WITH DEVELOPMENT N OF LEGACY
                                       UPDATED INFLOW HYDROGRAPHS TO REFLECT 15MIN INC.

                                    REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
                                       MODELERS: MSW
                                       XKSAT ADJUSTMENT

                                    REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
                                       MODELERS: MSW, TSW
                                       REVISIONS NOTED WITH HE (Hubbard Engineering)
                                       NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
                                       CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
                                       ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
                                       FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED


   32 IO          OUTPUT CONTROL VARIABLES
                        IPRNT          5  PRINT CONTROL
                        IPLOT          0  PLOT CONTROL
                        QSCAL         0.  HYDROGRAPH PLOT SCALE
```

```
   IT          HYDROGRAPH TIME DATA
                      NMIN           3  MINUTES IN COMPUTATION INTERVAL
                     IDATE      1    0  STARTING DATE
                     ITIME        0000  STARTING TIME
                        NQ        1000  NUMBER OF HYDROGRAPH ORDINATES
                    NDDATE      3    0  ENDING DATE
                    NDTIME        0157  ENDING TIME
                    ICENT           19  CENTURY MARK

                COMPUTATION INTERVAL    0.05 HOURS
                     TOTAL TIME BASE   49.95 HOURS

        ENGLISH UNITS
             DRAINAGE AREA         SQUARE MILES
             PRECIPITATION DEPTH   INCHES
             LENGTH, ELEVATION     FEET
             FLOW                  CUBIC FEET PER SECOND
             STORAGE VOLUME        ACRE-FEET
             SURFACE AREA          ACRES
             TEMPERATURE           DEGREES FAHRENHEIT

33 JD          INDEX STORM NO. 1
                      STRM        2.76  PRECIPITATION DEPTH
                      TRDA        0.00  TRANSPOSITION DRAINAGE AREA

34 PI            PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                   0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
                   0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

37 JD          INDEX STORM NO. 2
                      STRM        2.74  PRECIPITATION DEPTH
                      TRDA        0.50  TRANSPOSITION DRAINAGE AREA

38 PI            PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.02      0.02      0.02      0.02      0.02
                   0.03      0.03      0.03      0.03      0.03      0.09      0.09      0.09      0.09      0.09
                   0.02      0.02      0.02      0.02      0.02      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

41 JD          INDEX STORM NO. 3
                      STRM        2.69  PRECIPITATION DEPTH
                      TRDA        2.80  TRANSPOSITION DRAINAGE AREA

42 PI            PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                   0.04      0.04      0.04      0.04      0.04      0.05      0.05      0.05      0.05      0.05
                   0.03      0.03      0.03      0.03      0.03      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

45 JD          INDEX STORM NO. 4
                      STRM        2.54  PRECIPITATION DEPTH
                      TRDA       16.00  TRANSPOSITION DRAINAGE AREA
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46 PI           PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                   0.03      0.03      0.03      0.03      0.03      0.04      0.04      0.04      0.04      0.04
                   0.03      0.03      0.03      0.03      0.03      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

49 JD         INDEX STORM NO. 5
                     STRM        2.23  PRECIPITATION DEPTH
                     TRDA       90.00  TRANSPOSITION DRAINAGE AREA

50 PI           PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                   0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03
                   0.02      0.02      0.02      0.02      0.02      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

53 JD         INDEX STORM NO. 6
                     STRM        3.66  PRECIPITATION DEPTH
                     TRDA       10.00  TRANSPOSITION DRAINAGE AREA

 0 PI           PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                   0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03
                   0.02      0.02      0.02      0.02      0.02      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

54 JD         INDEX STORM NO. 7
                     STRM        3.53  PRECIPITATION DEPTH
                     TRDA       20.00  TRANSPOSITION DRAINAGE AREA

 0 PI           PRECIPITATION PATTERN
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.02      0.02      0.02      0.02      0.02
                   0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03      0.03
                   0.02      0.02      0.02      0.02      0.02      0.01      0.01      0.01      0.01      0.01
                   0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                   0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+	HYDROGRAPH AT	76THST	50.	4.50	15.	4.	2.	0.24		
+	HYDROGRAPH AT	SB01A	103.	4.25	12.	3.	1.	0.06		
+	DIVERSION TO	RET01A	44.	4.05	3.	1.	0.	0.06		
+	HYDROGRAPH AT	SB01A	103.	4.25	9.	2.	1.	0.06		
+	2 COMBINED AT	CP-1	145.	4.25	24.	7.	3.	0.30		
+	ROUTED TO	R1A-2W	144.	4.30	24.	7.	3.	0.30		
+	HYDROGRAPH AT	SB02NW	43.	4.05	4.	1.	0.	0.02		
+	DIVERSION TO	RET2NW	14.	3.85	1.	0.	0.	0.02		

	HYDROGRAPH AT							
+		SB02NW	43.	4.05	3.	1.	0.	0.02
	2 COMBINED AT							
+		CP-02W	164.	4.25	27.	7.	4.	0.31
	ROUTED TO							
+		R2W-2	163.	4.25	27.	7.	4.	0.31
	HYDROGRAPH AT							
+		SB01C	51.	4.10	5.	1.	1.	0.03
	DIVERSION TO							
+		RET01C	20.	3.90	1.	0.	0.	0.03
	HYDROGRAPH AT							
+		SB01C	51.	4.10	4.	1.	0.	0.03
	ROUTED TO							
+		R1C-1B	50.	4.10	4.	1.	0.	0.03
	HYDROGRAPH AT							
+		SB01B	50.	4.10	5.	1.	1.	0.02
	DIVERSION TO							
+		RET01B	17.	3.90	1.	0.	0.	0.02
	HYDROGRAPH AT							
+		SB01B	50.	4.10	3.	1.	0.	0.02
	2 COMBINED AT							
+		CP-01B	100.	4.10	7.	2.	1.	0.05
	ROUTED TO							
+		R1B-2N	97.	4.15	7.	2.	1.	0.05
	HYDROGRAPH AT							
+		SB02NE	57.	4.15	6.	2.	1.	0.03
	DIVERSION TO							
+		RET2NE	27.	3.95	2.	0.	0.	0.03
	HYDROGRAPH AT							
+		SB02NE	57.	4.15	5.	1.	1.	0.03
	2 COMBINED AT							
+		CP-02N	154.	4.15	12.	3.	1.	0.09
	ROUTED TO							
+		R2N-2	154.	4.15	12.	3.	1.	0.09
	HYDROGRAPH AT							
+		SB02E	93.	4.10	10.	3.	1.	0.04
	DIVERSION TO							
+		RET2E	22.	3.70	2.	1.	0.	0.04
	HYDROGRAPH AT							
+		SB02E	93.	4.10	8.	2.	1.	0.04
	HYDROGRAPH AT							
+		SB02W	39.	4.50	6.	1.	1.	0.06
	4 COMBINED AT							
+		CP-02	395.	4.15	53.	14.	7.	0.18
	ROUTED TO							
+		R2-3	388.	4.25	53.	14.	7.	0.18
	HYDROGRAPH AT							
+		SB03	78.	4.15	10.	3.	1.	0.05
	2 COMBINED AT							
+		CP-03	458.	4.20	62.	16.	8.	0.23
	ROUTED TO							
+		R3-5	454.	4.35	62.	16.	8.	0.23
	HYDROGRAPH AT							
+		SB05	229.	4.10	25.	6.	3.	0.13

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          2 COMBINED AT
+                           CP-5A      581.    4.30          85.         23.         11.        0.36

          HYDROGRAPH AT
+                           SB07W      104.    4.10           9.          2.          1.        0.05

          DIVERSION TO
+                          RET07W       29.    3.85           2.          1.          0.        0.05

          HYDROGRAPH AT
+                           SB07W      104.    4.10           7.          2.          1.        0.05

          ROUTED TO
+                          R7W-8W      104.    4.45           8.          2.          1.        0.05

          HYDROGRAPH AT
+                           SB8NW       40.    4.05           4.          1.          0.        0.01

          DIVERSION TO
+                          RET8NW        6.    3.55           1.          0.          0.        0.01

          HYDROGRAPH AT
+                           SB8NW       40.    4.05           3.          1.          0.        0.01

          ROUTED TO
+                          R8NW8N       39.    4.05           3.          1.          0.        0.01

          HYDROGRAPH AT
+                           SB8NE       42.    4.05           4.          1.          0.        0.01

          DIVERSION TO
+                          RET8NE        6.    3.55           1.          0.          0.        0.01

          HYDROGRAPH AT
+                           SB8NE       42.    4.05           3.          1.          0.        0.01

          ROUTED TO
+                          R8NE8N       42.    4.05           3.          1.          0.        0.01

          HYDROGRAPH AT
+                           SB8SW       37.    4.05           3.          1.          0.        0.01

          DIVERSION TO
+                          RET8SW        5.    3.55           1.          0.          0.        0.01

          HYDROGRAPH AT
+                           SB8SW       37.    4.05           3.          1.          0.        0.01

          HYDROGRAPH AT
+                           SB8SE       36.    4.05           3.          1.          0.        0.01

          DIVERSION TO
+                          RET8SE        5.    3.55           1.          0.          0.        0.01

          HYDROGRAPH AT
+                           SB8SE       36.    4.05           3.          1.          0.        0.01

          4 COMBINED AT
+                          CP-08N      153.    4.05          12.          3.          1.        0.06

          ROUTED TO
+                          R8N-8W      150.    4.05          12.          3.          1.        0.06

          HYDROGRAPH AT
+                           SB08W      145.    4.10          15.          4.          2.        0.06

          DIVERSION TO
+                           RET8W       27.    3.65           3.          1.          0.        0.06

          HYDROGRAPH AT
+                           SB08W      145.    4.10          12.          3.          1.        0.06

          3 COMBINED AT
+                          CP-08W      287.    4.10          32.          8.          4.        0.16

          ROUTED TO
+                           R8W-4      287.    4.10          32.          8.          4.        0.16

          HYDROGRAPH AT
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+                         SB04       292.    4.10         32.          8.          4.       0.14

         2 COMBINED AT
+                         CP-4       578.    4.10         64.         16.          8.       0.31

         ROUTED TO
+                         R4-5       567.    4.15         64.         16.          8.       0.31

         2 COMBINED AT
+                        CP-05       951.    4.30        146.         38.         19.       0.66

         ROUTED TO
+                         R5-6       926.    4.35        146.         38.         18.       0.66

         HYDROGRAPH AT
+                         SB06       243.    4.10         29.          7.          3.       0.14

         2 COMBINED AT
+                        CP-06      1020.    4.35        171.         45.         22.       0.80

         HYDROGRAPH AT
+                        SB07E        19.    4.40          3.          1.          0.       0.03

         ROUTED TO
+                       R7E-8E        19.    4.45          3.          1.          0.       0.03

         HYDROGRAPH AT
+                        SB08E        46.    4.55          8.          2.          1.       0.08

         HYDROGRAPH AT
+                        SB09N        13.    4.55          3.          1.          0.       0.03

         ROUTED TO
+                       R9N-R9        13.    4.65          3.          1.          0.       0.03

         3 COMBINED AT
+                       CP-08E        76.    4.55         13.          3.          2.       0.13

         ROUTED TO
+                       R8E-R9        76.    4.55         13.          3.          2.       0.13

         HYDROGRAPH AT
+                         SB09        71.    4.45         10.          2.          1.       0.09

         2 COMBINED AT
+                        CP-09       144.    4.50         23.          6.          3.       0.22

         HYDROGRAPH AT
+                        PWRCH       738.    5.75        429.        135.         74.       7.00

         2 COMBINED AT
+                       BINFLO       749.    5.75        439.        138.         76.       7.22

         ROUTED TO
+                        BASIN       241.    9.20        228.        130.         73.       7.22

         ROUTED TO
+                       BSNRT1       241.    9.25        228.        130.         73.       7.22

         ROUTED TO
+                       BSNRT2       241.    9.25        228.        130.         73.       7.22

         HYDROGRAPH AT
+                         SB10        87.    4.05          8.          2.          1.       0.04

         2 COMBINED AT
+                        CP-10       244.    9.20        232.        133.         76.       0.23

         ROUTED TO
+                       R10-11       244.    9.30        232.        133.         76.       0.23

         HYDROGRAPH AT
+                         SB11       130.    4.10         15.          4.          2.       0.07

         2 COMBINED AT
+                        CP-11       244.    9.30        232.        136.         77.       0.30
1
                                              SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
                                                  (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)
                                                                                    INTERPOLATED TO
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                                                                                  COMPUTATION INTERVAL
          ISTAQ    ELEMENT      DT       PEAK    TIME TO     VOLUME      DT       PEAK    TIME TO     VOLUME
                                                  PEAK                                     PEAK

                               (MIN)     (CFS)     (MIN)      (IN)     (MIN)     (CFS)     (MIN)       (IN)

         FOR STORM = 1  STORM AREA (SQ MI) =       0.00
           R1A-2W  MANE         0.31    145.99    255.52      0.86      3.00    145.05    258.00        0.86


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1385E+02 EXCESS=0.0000E+00 OUTFLOW=0.1385E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0


         FOR STORM = 2  STORM AREA (SQ MI) =       0.50
           R1A-2W  MANE         0.31    145.24    255.66      0.86      3.00    144.23    258.00        0.86


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1380E+02 EXCESS=0.0000E+00 OUTFLOW=0.1380E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0


         FOR STORM = 3  STORM AREA (SQ MI) =       2.80
           R1A-2W  MANE         0.30    102.74    258.17      0.82      3.00    102.70    258.00        0.82


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1311E+02 EXCESS=0.0000E+00 OUTFLOW=0.1311E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0


         FOR STORM = 4  STORM AREA (SQ MI) =      16.00
           R1A-2W  MANE         0.30     86.26    258.55      0.77      3.00     86.14    258.00        0.77


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1240E+02 EXCESS=0.0000E+00 OUTFLOW=0.1240E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0


         FOR STORM = 5  STORM AREA (SQ MI) =      90.00
           R1A-2W  MANE         0.35     70.99    270.31      0.71      3.00     70.97    270.00        0.71


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1143E+02 EXCESS=0.0000E+00 OUTFLOW=0.1143E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0


         FOR STORM = 6  STORM AREA (SQ MI) =      10.00
           R1A-2W  MANE         0.22    101.82    261.19      0.92      3.00    101.82    261.00        0.92


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1483E+02 EXCESS=0.0000E+00 OUTFLOW=0.1483E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0


         FOR STORM = 7  STORM AREA (SQ MI) =      20.00
           R1A-2W  MANE         0.27     99.04    261.08      0.90      3.00     99.04    261.00        0.90


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1451E+02 EXCESS=0.0000E+00 OUTFLOW=0.1451E+02 BASIN STORAGE=0.2984E-02 PERCENT ERROR=   0.0


         FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R2W-2  MANE         0.36    164.36    255.38      0.94      3.00    163.85    255.00        0.94


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1526E+02 EXCESS=0.0000E+00 OUTFLOW=0.1526E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0


         FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R2W-2  MANE         0.27    163.57    255.50      0.93      3.00    162.94    255.00        0.93


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1520E+02 EXCESS=0.0000E+00 OUTFLOW=0.1520E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0


         FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R2W-2  MANE         0.35    117.20    258.14      0.88      3.00    117.19    258.00        0.88


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1428E+02 EXCESS=0.0000E+00 OUTFLOW=0.1428E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0


         FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R2W-2  MANE         0.38     96.83    258.51      0.82      3.00     96.72    258.00        0.82
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1335E+02 EXCESS=0.0000E+00 OUTFLOW=0.1335E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
             R2W-2  MANE         0.37     75.72    261.93      0.74      3.00     75.66    261.00        0.74

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1207E+02 EXCESS=0.0000E+00 OUTFLOW=0.1206E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
             R2W-2  MANE         0.24    117.87    258.45      1.02      3.00    117.68    258.00        1.02

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1655E+02 EXCESS=0.0000E+00 OUTFLOW=0.1655E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
             R2W-2  MANE         0.24    114.22    258.50      0.99      3.00    114.02    258.00        0.99

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1613E+02 EXCESS=0.0000E+00 OUTFLOW=0.1613E+02 BASIN STORAGE=0.4391E-02 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1C-1B  MANE         0.30     51.03    246.76      1.36      3.00     50.29    246.00        1.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1961E+01 EXCESS=0.0000E+00 OUTFLOW=0.1962E+01 BASIN STORAGE=0.1121E-13 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1C-1B  MANE         0.40     50.59    246.72      1.35      3.00     49.89    246.00        1.35

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1941E+01 EXCESS=0.0000E+00 OUTFLOW=0.1942E+01 BASIN STORAGE=0.1138E-13 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1C-1B  MANE         0.42     27.84    246.95      1.13      3.00     27.64    249.00        1.14

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1632E+01 EXCESS=0.0000E+00 OUTFLOW=0.1631E+01 BASIN STORAGE=0.1119E-13 PERCENT ERROR=   0.1

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R1C-1B  MANE         0.40     19.93    246.90      0.91      3.00     19.81    249.00        0.92

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1321E+01 EXCESS=0.0000E+00 OUTFLOW=0.1317E+01 BASIN STORAGE=0.1123E-13 PERCENT ERROR=   0.3

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R1C-1B  MANE         0.50     11.56    247.25      0.62      3.00     11.55    249.00        0.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8898E+00 EXCESS=0.0000E+00 OUTFLOW=0.8904E+00 BASIN STORAGE=0.1117E-13 PERCENT ERROR=  -0.1

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R1C-1B  MANE         0.35     26.68    246.72      1.66      3.00     26.64    249.00        1.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2396E+01 EXCESS=0.0000E+00 OUTFLOW=0.2395E+01 BASIN STORAGE=0.1140E-13 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R1C-1B  MANE         0.32     25.38    246.62      1.57      3.00     25.34    249.00        1.57

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2256E+01 EXCESS=0.0000E+00 OUTFLOW=0.2254E+01 BASIN STORAGE=0.1122E-13 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1B-2N  MANE         0.92     99.26    247.53      1.36      3.00     98.01    249.00        1.36

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3711E+01 EXCESS=0.0000E+00 OUTFLOW=0.3711E+01 BASIN STORAGE=0.3220E-11 PERCENT ERROR=   0.0
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          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R1B-2N  MANE         0.83     98.83    247.91      1.35      3.00     97.26    249.00        1.35

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3673E+01 EXCESS=0.0000E+00 OUTFLOW=0.3684E+01 BASIN STORAGE=0.3207E-11 PERCENT ERROR=  -0.3

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R1B-2N  MANE         1.03     53.55    247.69      1.14      3.00     53.37    249.00        1.13

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3088E+01 EXCESS=0.0000E+00 OUTFLOW=0.3089E+01 BASIN STORAGE=0.3137E-11 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R1B-2N  MANE         1.09     38.34    248.42      0.92      3.00     38.21    249.00        0.92

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2500E+01 EXCESS=0.0000E+00 OUTFLOW=0.2500E+01 BASIN STORAGE=0.3264E-11 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R1B-2N  MANE         1.19     22.19    248.72      0.62      3.00     22.17    249.00        0.62

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1684E+01 EXCESS=0.0000E+00 OUTFLOW=0.1686E+01 BASIN STORAGE=0.3255E-11 PERCENT ERROR=  -0.1

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R1B-2N  MANE         0.90     51.28    247.95      1.67      3.00     51.12    249.00        1.66

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4529E+01 EXCESS=0.0000E+00 OUTFLOW=0.4530E+01 BASIN STORAGE=0.3257E-11 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R1B-2N  MANE         1.05     48.69    248.70      1.57      3.00     48.64    249.00        1.57

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4264E+01 EXCESS=0.0000E+00 OUTFLOW=0.4266E+01 BASIN STORAGE=0.3118E-11 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2N-2  MANE         0.31    154.68    249.16      1.36      3.00    154.46    249.00        1.36

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6138E+01 EXCESS=0.0000E+00 OUTFLOW=0.6147E+01 BASIN STORAGE=0.5105E-06 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R2N-2  MANE         0.31    153.62    249.29      1.34      3.00    153.29    249.00        1.35

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6080E+01 EXCESS=0.0000E+00 OUTFLOW=0.6088E+01 BASIN STORAGE=0.5465E-06 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
             R2N-2  MANE         0.42     85.54    249.46      1.13      3.00     85.38    249.00        1.13

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5110E+01 EXCESS=0.0000E+00 OUTFLOW=0.5111E+01 BASIN STORAGE=0.4442E-06 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
             R2N-2  MANE         0.42     61.35    249.48      0.91      3.00     61.19    249.00        0.91

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4137E+01 EXCESS=0.0000E+00 OUTFLOW=0.4138E+01 BASIN STORAGE=0.5190E-06 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
             R2N-2  MANE         0.47     35.73    249.69      0.62      3.00     35.56    249.00        0.61

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2788E+01 EXCESS=0.0000E+00 OUTFLOW=0.2789E+01 BASIN STORAGE=0.5140E-06 PERCENT ERROR=  -0.1

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
             R2N-2  MANE         0.30     82.46    249.44      1.65      3.00     82.26    249.00        1.65
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7500E+01 EXCESS=0.0000E+00 OUTFLOW=0.7502E+01 BASIN STORAGE=0.4816E-06 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
             R2N-2  MANE         0.39     78.41    249.52      1.56      3.00     78.24    249.00        1.56

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7064E+01 EXCESS=0.0000E+00 OUTFLOW=0.7065E+01 BASIN STORAGE=0.4315E-06 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R2-3  MANE         1.87    394.92    253.51      2.92      3.00    390.70    255.00        2.92

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2831E+02 EXCESS=0.0000E+00 OUTFLOW=0.2825E+02 BASIN STORAGE=0.4180E-01 PERCENT ERROR=   0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R2-3  MANE         1.87    391.85    253.62      2.90      3.00    387.99    255.00        2.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2813E+02 EXCESS=0.0000E+00 OUTFLOW=0.2806E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R2-3  MANE         2.18    249.49    258.70      2.55      3.00    249.46    258.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2474E+02 EXCESS=0.0000E+00 OUTFLOW=0.2468E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.1

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
              R2-3  MANE         2.42    191.22    260.01      2.23      3.00    190.55    261.00        2.23

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2169E+02 EXCESS=0.0000E+00 OUTFLOW=0.2163E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
              R2-3  MANE         2.74    131.58    263.67      1.83      3.00    131.54    261.00        1.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1774E+02 EXCESS=0.0000E+00 OUTFLOW=0.1770E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
              R2-3  MANE         2.27    254.73    261.04      3.27      3.00    254.71    261.00        3.27

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3175E+02 EXCESS=0.0000E+00 OUTFLOW=0.3170E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
              R2-3  MANE         2.30    243.97    260.84      3.14      3.00    243.92    261.00        3.14

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3044E+02 EXCESS=0.0000E+00 OUTFLOW=0.3040E+02 BASIN STORAGE=0.4192E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R3-5  MANE         3.00    458.43    261.68      2.73      3.00    458.12    261.00        2.73

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3337E+02 EXCESS=0.0000E+00 OUTFLOW=0.3338E+02 BASIN STORAGE=0.7696E-01 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R3-5  MANE         3.00    455.77    258.51      2.71      3.00    454.00    261.00        2.71

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3315E+02 EXCESS=0.0000E+00 OUTFLOW=0.3316E+02 BASIN STORAGE=0.7696E-01 PERCENT ERROR=  -0.3

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R3-5  MANE         3.00    294.25    265.05      2.39      3.00    293.16    267.00        2.39

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2933E+02 EXCESS=0.0000E+00 OUTFLOW=0.2923E+02 BASIN STORAGE=0.8862E-01 PERCENT ERROR=   0.0
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          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
              R3-5  MANE         3.00    223.97    267.76      2.10      3.00    223.17    267.00        2.10

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2574E+02 EXCESS=0.0000E+00 OUTFLOW=0.2567E+02 BASIN STORAGE=0.9261E-01 PERCENT ERROR=  -0.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
              R3-5  MANE         3.00    152.87    273.07      1.71      3.00    152.80    273.00        1.71

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2102E+02 EXCESS=0.0000E+00 OUTFLOW=0.2089E+02 BASIN STORAGE=0.9561E-01 PERCENT ERROR=   0.2

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
              R3-5  MANE         2.96    299.06    267.64      3.09      3.00    298.54    270.00        3.09

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3795E+02 EXCESS=0.0000E+00 OUTFLOW=0.3782E+02 BASIN STORAGE=0.9250E-01 PERCENT ERROR=   0.1

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
              R3-5  MANE         2.96    286.26    268.14      2.96      3.00    285.63    270.00        2.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3638E+02 EXCESS=0.0000E+00 OUTFLOW=0.3628E+02 BASIN STORAGE=0.8862E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7W-8W  MANE         2.73    102.63    267.82      1.79      3.00    102.19    267.00        1.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3402E+01 EXCESS=0.0000E+00 OUTFLOW=0.4461E+01 BASIN STORAGE=0.3428E-03 PERCENT ERROR= -31.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R7W-8W  MANE         2.59    104.73    266.79      1.55      3.00    104.06    267.00        1.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3367E+01 EXCESS=0.0000E+00 OUTFLOW=0.3871E+01 BASIN STORAGE=0.3311E-03 PERCENT ERROR= -15.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R7W-8W  MANE         2.99     52.62    270.98      1.33      3.00     52.50    273.00        1.33

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2832E+01 EXCESS=0.0000E+00 OUTFLOW=0.3326E+01 BASIN STORAGE=0.3101E-03 PERCENT ERROR= -17.5

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R7W-8W  MANE         3.00     37.32    275.61      1.13      3.00     37.24    276.00        1.13

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2292E+01 EXCESS=0.0000E+00 OUTFLOW=0.2822E+01 BASIN STORAGE=0.3108E-03 PERCENT ERROR= -23.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R7W-8W  MANE         3.00     21.45    284.63      0.78      3.00     21.39    282.00        0.78

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1544E+01 EXCESS=0.0000E+00 OUTFLOW=0.1942E+01 BASIN STORAGE=0.3316E-03 PERCENT ERROR= -25.8

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R7W-8W  MANE         3.00     49.46    273.94      1.67      3.00     49.38    273.00        1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4156E+01 EXCESS=0.0000E+00 OUTFLOW=0.4179E+01 BASIN STORAGE=0.3376E-03 PERCENT ERROR=  -0.6

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R7W-8W  MANE         3.00     47.01    271.91      1.58      3.00     46.97    273.00        1.58

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3912E+01 EXCESS=0.0000E+00 OUTFLOW=0.3956E+01 BASIN STORAGE=0.3474E-03 PERCENT ERROR=  -1.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NW8N  MANE         0.75     40.02    244.34      1.98      3.00     39.44    243.00        1.98
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1550E+01 EXCESS=0.0000E+00 OUTFLOW=0.1552E+01 BASIN STORAGE=0.1215E-06 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8NW8N  MANE         0.77     39.68    244.02      1.96      3.00     39.17    243.00        1.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1538E+01 EXCESS=0.0000E+00 OUTFLOW=0.1540E+01 BASIN STORAGE=0.1352E-06 PERCENT ERROR=  -0.2

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8NW8N  MANE         0.97     21.41    244.47      1.86      3.00     21.33    243.00        1.86

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1449E+01 EXCESS=0.0000E+00 OUTFLOW=0.1455E+01 BASIN STORAGE=0.1154E-06 PERCENT ERROR=  -0.4

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R8NW8N  MANE         1.05     16.17    244.69      1.68      3.00     16.11    243.00        1.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1313E+01 EXCESS=0.0000E+00 OUTFLOW=0.1317E+01 BASIN STORAGE=0.1153E-06 PERCENT ERROR=  -0.2

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R8NW8N  MANE         1.03     10.82    245.03      1.37      3.00     10.72    243.00        1.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1074E+01 EXCESS=0.0000E+00 OUTFLOW=0.1077E+01 BASIN STORAGE=0.1398E-06 PERCENT ERROR=  -0.3

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R8NW8N  MANE         0.91     19.33    244.37      2.65      3.00     19.25    243.00        2.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2076E+01 EXCESS=0.0000E+00 OUTFLOW=0.2081E+01 BASIN STORAGE=0.1314E-06 PERCENT ERROR=  -0.2

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R8NW8N  MANE         0.88     18.61    244.50      2.54      3.00     18.51    243.00        2.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1987E+01 EXCESS=0.0000E+00 OUTFLOW=0.1989E+01 BASIN STORAGE=0.1316E-06 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8NE8N  MANE         0.69     42.06    244.33      1.98      3.00     41.68    243.00        1.98

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1579E+01 EXCESS=0.0000E+00 OUTFLOW=0.1583E+01 BASIN STORAGE=0.1124E-06 PERCENT ERROR=  -0.2

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8NE8N  MANE         0.68     41.82    244.25      1.96      3.00     41.41    243.00        1.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1567E+01 EXCESS=0.0000E+00 OUTFLOW=0.1569E+01 BASIN STORAGE=0.1055E-06 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8NE8N  MANE         0.92     22.13    243.92      1.85      3.00     22.08    243.00        1.86

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1476E+01 EXCESS=0.0000E+00 OUTFLOW=0.1484E+01 BASIN STORAGE=0.1034E-06 PERCENT ERROR=  -0.5

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R8NE8N  MANE         0.97     16.69    243.96      1.68      3.00     16.65    243.00        1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1338E+01 EXCESS=0.0000E+00 OUTFLOW=0.1340E+01 BASIN STORAGE=0.1343E-06 PERCENT ERROR=  -0.2

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R8NE8N  MANE         1.12     11.10    244.46      1.37      3.00     11.06    243.00        1.37

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1094E+01 EXCESS=0.0000E+00 OUTFLOW=0.1095E+01 BASIN STORAGE=0.1098E-06 PERCENT ERROR=  -0.1
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          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R8NE8N  MANE         0.90     19.88    244.18      2.65      3.00     19.84    243.00        2.65

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2116E+01 EXCESS=0.0000E+00 OUTFLOW=0.2117E+01 BASIN STORAGE=0.9623E-07 PERCENT ERROR=  -0.1

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R8NE8N  MANE         0.86     19.14    244.71      2.54      3.00     19.09    243.00        2.53

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2025E+01 EXCESS=0.0000E+00 OUTFLOW=0.2028E+01 BASIN STORAGE=0.1302E-06 PERCENT ERROR=  -0.2

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8N-8W  MANE         0.52    152.44    243.65      1.98      3.00    150.95    243.00        1.98

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5871E+01 EXCESS=0.0000E+00 OUTFLOW=0.5874E+01 BASIN STORAGE=0.1353E-07 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R8N-8W  MANE         0.51    151.61    243.84      1.96      3.00    149.94    243.00        1.96

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5822E+01 EXCESS=0.0000E+00 OUTFLOW=0.5823E+01 BASIN STORAGE=0.1343E-07 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8N-8W  MANE         0.53     81.55    243.95      1.85      3.00     81.15    243.00        1.85

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5500E+01 EXCESS=0.0000E+00 OUTFLOW=0.5501E+01 BASIN STORAGE=0.1336E-07 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R8N-8W  MANE         0.65     61.43    243.75      1.68      3.00     61.23    243.00        1.68

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4978E+01 EXCESS=0.0000E+00 OUTFLOW=0.4978E+01 BASIN STORAGE=0.1320E-07 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R8N-8W  MANE         0.74     40.89    244.25      1.37      3.00     40.71    243.00        1.37

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4070E+01 EXCESS=0.0000E+00 OUTFLOW=0.4071E+01 BASIN STORAGE=0.1350E-07 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R8N-8W  MANE         0.57     73.39    244.05      2.65      3.00     73.13    243.00        2.65

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7867E+01 EXCESS=0.0000E+00 OUTFLOW=0.7867E+01 BASIN STORAGE=0.1321E-07 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R8N-8W  MANE         0.64     70.52    244.05      2.53      3.00     70.34    243.00        2.53

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7526E+01 EXCESS=0.0000E+00 OUTFLOW=0.7527E+01 BASIN STORAGE=0.1341E-07 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R8W-4  MANE         0.61    288.74    246.26      1.91      3.00    288.70    246.00        1.91

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1646E+02 EXCESS=0.0000E+00 OUTFLOW=0.1645E+02 BASIN STORAGE=0.1371E-03 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
             R8W-4  MANE         0.53    286.94    246.33      1.83      3.00    286.84    246.00        1.83

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1577E+02 EXCESS=0.0000E+00 OUTFLOW=0.1578E+02 BASIN STORAGE=0.1364E-03 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
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       R8W-4  MANE        0.74    161.70    245.00      1.69      3.00    161.56    246.00        1.69

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1454E+02 EXCESS=0.0000E+00 OUTFLOW=0.1454E+02 BASIN STORAGE=0.1408E-03 PERCENT ERROR=   0.0

    FOR STORM = 4  STORM AREA (SQ MI) =      16.00
       R8W-4  MANE        0.78    122.55    244.62      1.51      3.00    122.49    246.00        1.51

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1298E+02 EXCESS=0.0000E+00 OUTFLOW=0.1297E+02 BASIN STORAGE=0.1356E-03 PERCENT ERROR=   0.0

    FOR STORM = 5  STORM AREA (SQ MI) =      90.00
       R8W-4  MANE        0.97     82.08    246.65      1.19      3.00     82.01    246.00        1.19

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1023E+02 EXCESS=0.0000E+00 OUTFLOW=0.1023E+02 BASIN STORAGE=0.1378E-03 PERCENT ERROR=   0.0

    FOR STORM = 6  STORM AREA (SQ MI) =      10.00
       R8W-4  MANE        0.72    147.07    246.77      2.36      3.00    147.06    246.00        2.36

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2027E+02 EXCESS=0.0000E+00 OUTFLOW=0.2028E+02 BASIN STORAGE=0.1355E-03 PERCENT ERROR=   0.0

    FOR STORM = 7  STORM AREA (SQ MI) =      20.00
       R8W-4  MANE        0.85    141.44    246.46      2.25      3.00    141.43    246.00        2.25

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1935E+02 EXCESS=0.0000E+00 OUTFLOW=0.1936E+02 BASIN STORAGE=0.1431E-03 PERCENT ERROR=   0.0

    FOR STORM = 1  STORM AREA (SQ MI) =       0.00
        R4-5  MANE        2.25    581.22    250.37      2.01      3.00    571.59    249.00        2.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3268E+02 EXCESS=0.0000E+00 OUTFLOW=0.3273E+02 BASIN STORAGE=0.4091E-03 PERCENT ERROR=  -0.2

    FOR STORM = 2  STORM AREA (SQ MI) =       0.50
        R4-5  MANE        2.30    574.86    250.76      1.96      3.00    566.30    249.00        1.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3188E+02 EXCESS=0.0000E+00 OUTFLOW=0.3195E+02 BASIN STORAGE=0.4397E-03 PERCENT ERROR=  -0.2

    FOR STORM = 3  STORM AREA (SQ MI) =       2.80
        R4-5  MANE        2.78    331.68    249.80      1.81      3.00    330.74    252.00        1.81

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2948E+02 EXCESS=0.0000E+00 OUTFLOW=0.2950E+02 BASIN STORAGE=0.3881E-03 PERCENT ERROR=  -0.1

    FOR STORM = 4  STORM AREA (SQ MI) =      16.00
        R4-5  MANE        3.00    250.94    251.05      1.62      3.00    249.92    252.00        1.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2640E+02 EXCESS=0.0000E+00 OUTFLOW=0.2643E+02 BASIN STORAGE=0.3813E-03 PERCENT ERROR=  -0.1

    FOR STORM = 5  STORM AREA (SQ MI) =      90.00
        R4-5  MANE        3.00    164.62    254.83      1.31      3.00    164.54    252.00        1.31

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2125E+02 EXCESS=0.0000E+00 OUTFLOW=0.2129E+02 BASIN STORAGE=0.3682E-03 PERCENT ERROR=  -0.2

    FOR STORM = 6  STORM AREA (SQ MI) =      10.00
        R4-5  MANE        2.85    309.77    251.59      2.49      3.00    309.09    252.00        2.49

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4041E+02 EXCESS=0.0000E+00 OUTFLOW=0.4051E+02 BASIN STORAGE=0.3879E-03 PERCENT ERROR=  -0.2

    FOR STORM = 7  STORM AREA (SQ MI) =      20.00
        R4-5  MANE        2.86    295.96    250.39      2.38      3.00    295.45    252.00        2.38
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3868E+02 EXCESS=0.0000E+00 OUTFLOW=0.3875E+02 BASIN STORAGE=0.3882E-03 PERCENT ERROR=  -0.2

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R5-6  MANE         2.20   1010.21    262.68      2.23      3.00   1000.34    261.00        2.23

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7880E+02 EXCESS=0.0000E+00 OUTFLOW=0.7874E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
              R5-6  MANE         2.21    994.60    262.88      2.20      3.00    987.77    261.00        2.20

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7768E+02 EXCESS=0.0000E+00 OUTFLOW=0.7757E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
              R5-6  MANE         2.48    665.14    266.57      1.99      3.00    662.19    267.00        1.99

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7008E+02 EXCESS=0.0000E+00 OUTFLOW=0.7004E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
              R5-6  MANE         2.81    502.74    267.30      1.76      3.00    499.70    267.00        1.76

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6201E+02 EXCESS=0.0000E+00 OUTFLOW=0.6198E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=  -0.1

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
              R5-6  MANE         3.36    325.91    273.89      1.42      3.00    322.56    276.00        1.42

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5001E+02 EXCESS=0.0000E+00 OUTFLOW=0.4998E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=  -0.1

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
              R5-6  MANE         2.61    680.31    263.26      2.65      3.00    679.88    264.00        2.65

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9360E+02 EXCESS=0.0000E+00 OUTFLOW=0.9357E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
              R5-6  MANE         2.52    649.46    264.71      2.54      3.00    649.25    264.00        2.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8961E+02 EXCESS=0.0000E+00 OUTFLOW=0.8960E+02 BASIN STORAGE=0.7457E-01 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7E-8E  MANE         1.76     19.03    266.67      0.91      3.00     19.02    267.00        0.91

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1268E+01 EXCESS=0.0000E+00 OUTFLOW=0.1268E+01 BASIN STORAGE=0.1101E-04 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R7E-8E  MANE         1.77     18.85    266.81      0.90      3.00     18.84    267.00        0.90

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1254E+01 EXCESS=0.0000E+00 OUTFLOW=0.1254E+01 BASIN STORAGE=0.1092E-04 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R7E-8E  MANE         2.41      8.57    268.50      0.49      3.00      8.54    270.00        0.49

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6750E+00 EXCESS=0.0000E+00 OUTFLOW=0.6756E+00 BASIN STORAGE=0.1319E-04 PERCENT ERROR=  -0.1

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R7E-8E  MANE         3.00      3.99    269.13      0.21      3.00      3.99    270.00        0.21

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2917E+00 EXCESS=0.0000E+00 OUTFLOW=0.2928E+00 BASIN STORAGE=0.1256E-04 PERCENT ERROR=  -0.4
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         FOR STORM = 5  STORM AREA (SQ MI) =      90.00
           R7E-8E  MANE         3.00      0.26    292.33      0.01      3.00      0.25    294.00         0.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1646E-01 EXCESS=0.0000E+00 OUTFLOW=0.2054E-01 BASIN STORAGE=0.1096E-04 PERCENT ERROR= -24.8

         FOR STORM = 6  STORM AREA (SQ MI) =      10.00
           R7E-8E  MANE         2.36      9.61    272.07      0.60      3.00      9.60    273.00         0.60

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8313E+00 EXCESS=0.0000E+00 OUTFLOW=0.8321E+00 BASIN STORAGE=0.1454E-04 PERCENT ERROR=  -0.1

         FOR STORM = 7  STORM AREA (SQ MI) =      20.00
           R7E-8E  MANE         2.43      8.70    271.22      0.54      3.00      8.69    273.00         0.54

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7492E+00 EXCESS=0.0000E+00 OUTFLOW=0.7500E+00 BASIN STORAGE=0.1049E-04 PERCENT ERROR=  -0.1

         FOR STORM = 1  STORM AREA (SQ MI) =       0.00
           R9N-R9  MANE         2.26     13.31    278.00      1.08      3.00     13.26    279.00         1.08

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1613E+01 EXCESS=0.0000E+00 OUTFLOW=0.1611E+01 BASIN STORAGE=0.2018E-04 PERCENT ERROR=   0.1

         FOR STORM = 2  STORM AREA (SQ MI) =       0.50
           R9N-R9  MANE         2.27     13.16    278.20      1.07      3.00     13.12    279.00         1.07

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1595E+01 EXCESS=0.0000E+00 OUTFLOW=0.1593E+01 BASIN STORAGE=0.1743E-04 PERCENT ERROR=   0.1

         FOR STORM = 3  STORM AREA (SQ MI) =       2.80
           R9N-R9  MANE         2.76      8.13    284.79      0.74      3.00      8.13    285.00         0.74

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1103E+01 EXCESS=0.0000E+00 OUTFLOW=0.1102E+01 BASIN STORAGE=0.2044E-04 PERCENT ERROR=   0.1

         FOR STORM = 4  STORM AREA (SQ MI) =      16.00
           R9N-R9  MANE         3.00      5.05    284.63      0.46      3.00      5.05    285.00         0.46

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6843E+00 EXCESS=0.0000E+00 OUTFLOW=0.6838E+00 BASIN STORAGE=0.2050E-04 PERCENT ERROR=   0.1

         FOR STORM = 5  STORM AREA (SQ MI) =      90.00
           R9N-R9  MANE         3.00      1.59    293.56      0.14      3.00      1.59    294.00         0.14

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2079E+00 EXCESS=0.0000E+00 OUTFLOW=0.2079E+00 BASIN STORAGE=0.1894E-04 PERCENT ERROR=   0.0

         FOR STORM = 6  STORM AREA (SQ MI) =      10.00
           R9N-R9  MANE         2.65      9.46    288.90      0.92      3.00      9.46    288.00         0.92

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1380E+01 EXCESS=0.0000E+00 OUTFLOW=0.1379E+01 BASIN STORAGE=0.1805E-04 PERCENT ERROR=   0.1

         FOR STORM = 7  STORM AREA (SQ MI) =      20.00
           R9N-R9  MANE         2.68      8.74    288.39      0.84      3.00      8.73    288.00         0.84

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1261E+01 EXCESS=0.0000E+00 OUTFLOW=0.1260E+01 BASIN STORAGE=0.1932E-04 PERCENT ERROR=   0.1

         FOR STORM = 1  STORM AREA (SQ MI) =       0.00
           R8E-R9  MANE         0.32     77.02    273.53      0.96      3.00     76.82    273.00         0.96

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6728E+01 EXCESS=0.0000E+00 OUTFLOW=0.6728E+01 BASIN STORAGE=0.7838E-05 PERCENT ERROR=   0.0

         FOR STORM = 2  STORM AREA (SQ MI) =       0.50
           R8E-R9  MANE         0.32     76.18    273.66      0.95      3.00     75.99    273.00         0.95
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6652E+01 EXCESS=0.0000E+00 OUTFLOW=0.6653E+01 BASIN STORAGE=0.7629E-05 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R8E-R9  MANE         0.45     38.17    276.54      0.55      3.00     38.13    276.00        0.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3882E+01 EXCESS=0.0000E+00 OUTFLOW=0.3882E+01 BASIN STORAGE=0.7462E-05 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R8E-R9  MANE         0.52     19.07    276.61      0.27      3.00     19.04    276.00        0.27

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1925E+01 EXCESS=0.0000E+00 OUTFLOW=0.1925E+01 BASIN STORAGE=0.7540E-05 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R8E-R9  MANE         0.94      2.52    293.35      0.04      3.00      2.51    294.00        0.04

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3008E+00 EXCESS=0.0000E+00 OUTFLOW=0.3008E+00 BASIN STORAGE=0.6726E-05 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R8E-R9  MANE         0.36     44.12    279.67      0.68      3.00     44.08    279.00        0.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4782E+01 EXCESS=0.0000E+00 OUTFLOW=0.4782E+01 BASIN STORAGE=0.7472E-05 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R8E-R9  MANE         0.43     40.11    279.56      0.62      3.00     40.07    279.00        0.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4334E+01 EXCESS=0.0000E+00 OUTFLOW=0.4334E+01 BASIN STORAGE=0.7666E-05 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT1  MANE         0.39    244.41    552.63      0.80      3.00    244.41    552.00        0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3084E+03 EXCESS=0.0000E+00 OUTFLOW=0.3084E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            BSNRT1  MANE         0.39    244.36    552.54      0.80      3.00    244.36    552.00        0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3083E+03 EXCESS=0.0000E+00 OUTFLOW=0.3082E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            BSNRT1  MANE         0.41    241.71    552.95      0.79      3.00    241.71    552.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3040E+03 EXCESS=0.0000E+00 OUTFLOW=0.3039E+03 BASIN STORAGE=0.3152E-01 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            BSNRT1  MANE         0.33    239.65    552.95      0.78      3.00    239.64    555.00        0.78

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3008E+03 EXCESS=0.0000E+00 OUTFLOW=0.3007E+03 BASIN STORAGE=0.3152E-01 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            BSNRT1  MANE         0.44    237.69    555.51      0.77      3.00    237.69    555.00        0.77

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2977E+03 EXCESS=0.0000E+00 OUTFLOW=0.2976E+03 BASIN STORAGE=0.3151E-01 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            BSNRT1  MANE         0.46    242.84    552.79      0.79      3.00    242.84    552.00        0.79

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3057E+03 EXCESS=0.0000E+00 OUTFLOW=0.3057E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR=   0.0
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          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            BSNRT1  MANE         0.37    242.32    552.68      0.79      3.00    242.32    552.00        0.79

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3049E+03 EXCESS=0.0000E+00 OUTFLOW=0.3049E+03 BASIN STORAGE=0.3153E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE         0.50    244.41    552.74      0.80      3.00    244.41    552.00        0.80

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3084E+03 EXCESS=0.0000E+00 OUTFLOW=0.3083E+03 BASIN STORAGE=0.3601E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            BSNRT2  MANE         0.43    244.36    552.89      0.80      3.00    244.36    552.00        0.80

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3082E+03 EXCESS=0.0000E+00 OUTFLOW=0.3082E+03 BASIN STORAGE=0.3601E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            BSNRT2  MANE         0.49    241.71    553.29      0.79      3.00    241.71    555.00        0.79

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3040E+03 EXCESS=0.0000E+00 OUTFLOW=0.3039E+03 BASIN STORAGE=0.3600E-01 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            BSNRT2  MANE         0.36    239.64    555.76      0.78      3.00    239.64    555.00        0.78

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3007E+03 EXCESS=0.0000E+00 OUTFLOW=0.3007E+03 BASIN STORAGE=0.3599E-01 PERCENT ERROR=   0.0

          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            BSNRT2  MANE         0.37    237.69    555.79      0.77      3.00    237.69    555.00        0.77

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2976E+03 EXCESS=0.0000E+00 OUTFLOW=0.2976E+03 BASIN STORAGE=0.3598E-01 PERCENT ERROR=   0.0

          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            BSNRT2  MANE         0.50    242.84    553.18      0.79      3.00    242.83    552.00        0.79

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3057E+03 EXCESS=0.0000E+00 OUTFLOW=0.3057E+03 BASIN STORAGE=0.3600E-01 PERCENT ERROR=   0.0

          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            BSNRT2  MANE         0.36    242.32    552.92      0.79      3.00    242.31    555.00        0.79

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3049E+03 EXCESS=0.0000E+00 OUTFLOW=0.3048E+03 BASIN STORAGE=0.3600E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R10-11  MANE         3.04    244.41    557.55     25.54      3.00    244.41    558.00       25.53

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3125E+03 EXCESS=0.0000E+00 OUTFLOW=0.3119E+03 BASIN STORAGE=0.6746E+00 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =       0.50
            R10-11  MANE         3.07    244.36    558.85     25.53      3.00    244.35    558.00       25.52

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3124E+03 EXCESS=0.0000E+00 OUTFLOW=0.3117E+03 BASIN STORAGE=0.6746E+00 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =       2.80
            R10-11  MANE         3.00    241.71    559.89     25.15      3.00    241.70    561.00       25.13

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3077E+03 EXCESS=0.0000E+00 OUTFLOW=0.3071E+03 BASIN STORAGE=0.6744E+00 PERCENT ERROR=   0.0

          FOR STORM = 4  STORM AREA (SQ MI) =      16.00
            R10-11  MANE         3.00    239.64    561.16     24.84      3.00    239.64    561.00       24.84
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3040E+03 EXCESS=0.0000E+00 OUTFLOW=0.3034E+03 BASIN STORAGE=0.6743E+00 PERCENT ERROR=   0.0


          FOR STORM = 5  STORM AREA (SQ MI) =      90.00
            R10-11  MANE         3.07    237.68    560.85     24.53      3.00    237.68    561.00       24.53


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3003E+03 EXCESS=0.0000E+00 OUTFLOW=0.2996E+03 BASIN STORAGE=0.6743E+00 PERCENT ERROR=   0.0


          FOR STORM = 6  STORM AREA (SQ MI) =      10.00
            R10-11  MANE         3.07    242.83    560.81     25.40      3.00    242.83    558.00       25.38


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3108E+03 EXCESS=0.0000E+00 OUTFLOW=0.3102E+03 BASIN STORAGE=0.6744E+00 PERCENT ERROR=   0.0


          FOR STORM = 7  STORM AREA (SQ MI) =      20.00
            R10-11  MANE         3.00    242.31    558.78     25.30      3.00    242.31    561.00       25.30


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3097E+03 EXCESS=0.0000E+00 OUTFLOW=0.3090E+03 BASIN STORAGE=0.6745E+00 PERCENT ERROR=   0.0



*** NORMAL END OF HEC-1 ***
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100yr-24hr-Ultimate

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1***************************************
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 *                                       *                                                    *
*
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                    *    U.S. ARMY CORPS OF ENGINEERS
*
 *              JUN   1998               *                                                    *    HYDROLOGIC ENGINEERING CENTER
*
 *           VERSION 4.1                 *                                                    *          609 SECOND STREET
*
 *                                       *                                                    *       DAVIS, CALIFORNIA 95616
*
 *  RUN DATE   26APR21  TIME  09:31:12   *                                                    *           (916) 756-1104
*
 *                                       *                                                    *
*
 ***************************************
**************************************



                                                    X     X  XXXXXXX   XXXXX          X
                                                    X     X  X        X     X        XX
                                                    X     X  X        X               X
                                                    XXXXXXX  XXXX     X       XXXXX   X
                                                    X     X  X        X               X
                                                    X     X  X        X     X         X
                                                    X     X  XXXXXXX   XXXXX         XXX



            THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

            THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
            THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
            NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
            DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL   LOSS RATE:GREEN AND AMPT INFILTRATION
            KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


1                                                         HEC-1 INPUT                                                PAGE  1

           LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

              1          ID        CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
              2          ID        POWERLINE CORRIDOR
              3          ID        RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
              4          ID        INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
              5          ID       100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
              6          ID           PIMA ROAD FLOWS.
              7          ID
              8          ID        PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
              9          ID       MODELERS: RK, MW
             10          ID
             11          ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 08/07/18
             12          ID       MODELERS: MSW, ES
             13          ID       REVISIONS NOTED WITH HE
             14          ID           NEW PROP CONDITION MODEL,WITH POWERLINE AND BASIN 53R
             15          ID           THIS IS THE ULTIMATE CONDITION MODEL
             16          ID
             17          ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
             18          ID       MODELERS: MSW
             19          ID       XKSAT ADJUSTMENT
             20          ID
             21          ID        REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
             22          ID       MODELERS: MSW, TSW
             23          ID       REVISIONS NOTED WITH HE (Hubbard Engineering)
             24          ID           NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
             25          ID           CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
             26          ID           ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
             27          ID           FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED
             28          ID
             29          IT       3       0       0    1000
             30          IN      15
             31          IO       5
                         *DIAGRAM
                         *
```

```
  32           JD   3.849  0.0001
  33           PC   0.000   0.002   0.005   0.008   0.011   0.014   0.017   0.020   0.023   0.026
  34           PC   0.029   0.032   0.035   0.038   0.041   0.044   0.048   0.052   0.056   0.060
  35           PC   0.064   0.068   0.072   0.076   0.080   0.085   0.090   0.095   0.100   0.105
  36           PC   0.110   0.115   0.120   0.126   0.133   0.140   0.147   0.155   0.163   0.172
  37           PC   0.181   0.191   0.203   0.218   0.236   0.257   0.283   0.387   0.663   0.707
  38           PC   0.735   0.758   0.776   0.791   0.804   0.815   0.825   0.834   0.842   0.849
  39           PC   0.856   0.863   0.869   0.875   0.881   0.887   0.893   0.898   0.903   0.908
  40           PC   0.913   0.918   0.922   0.926   0.930   0.934   0.938   0.942   0.946   0.950
  41           PC   0.953   0.956   0.959   0.962   0.965   0.968   0.971   0.974   0.977   0.980
  42           PC   0.983   0.986   0.989   0.992   0.995   0.998   1.000
  43           JD   3.657    10.0
  44           JD   3.533    20.0
               *

  45           KK  76THST
  46           KM        76TH ST CHANNEL HYDROGRAPH FROM PINNACLE PEAK SOUTH ADMS
  47           KM        100-YR, 24-HR FLO-2D MODEL (XS 98)
  48           BA    0.24
  49           QI    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00
  50           QI    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00
  51           QI    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.00    0.01
```



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 LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

  52           QI    0.00    0.00    0.00    0.00    0.01    0.00    0.00    0.02    0.03    0.05
  53           QI    0.09    0.11    0.14    0.21    0.34    0.51    0.76    3.70   17.69   47.79
  54           QI   68.57   54.61   42.75   35.61   32.34   29.25   26.13   22.83   19.41   18.11
  55           QI   16.15   14.54   13.75   12.53   12.00   11.44   10.52    9.81    9.34    9.09
  56           QI    9.11    8.71    8.37    8.05    7.67    7.75    7.47    7.08    6.97    6.77
  57           QI    6.59    6.44    6.32    6.12    5.95    5.87    5.70    5.84    5.41    5.44
  58           QI    4.56    4.81    4.75    4.78    4.69    4.67    4.61    4.36    4.20    4.00
  59           QI    3.75    3.51    2.49    3.43    3.61    3.65    2.48    2.36    2.19    2.07
  60           QI    1.94    1.72    1.71    1.36    1.48    1.13    1.13    1.20    1.10    1.06
  61           QI    1.06    1.00    0.94    0.89    0.85    0.81    0.78    0.74    0.72    0.70
  62           QI    0.67    0.64    0.62    0.59    0.57    0.55    0.53    0.51    0.49    0.47
  63           QI    0.45    0.44    0.42    0.40    0.39
               *

  64           KK   SB01A   BASIN
  65           BA   0.061
  66           PB   2.721
  67           PC   0.000   0.008   0.016   0.025   0.033   0.041   0.050   0.058   0.066   0.075
  68           PC   0.087   0.099   0.119   0.150   0.234   0.413   0.766   0.875   0.916   0.944
  69           PC   0.956   0.968   0.979   0.990   1.000
  70           LG    0.25    0.25    4.03    0.56      45
  71           UC   0.439   0.231
  72           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  73           UA     100
               *

  74           KK   SB01A  DIVERT
  75           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01A
  76           KM        FIRST FLUSH VOL = 1.383 AC-FT
  77           DT  RET01A   1.383
  78           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  79           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

  80           KK    CP-1 COMBINE
  81           KM        LEGACY BLVD AND 76TH ST (MILLER RD CHANNEL)
  82           HC       2   0.301
               *

  83           KK  R1A-2W   ROUTE
  84           KM        ROUTE CP-1 TO CP-2W
  85           RK    1114  0.0200   0.018            TRAP   2.000    0.00
               *

  86           KK  SB02NW   BASIN
  87           KM        BASIN SB02E WAS SPLIT INTO SUB BASINS NORTH OF CAVASSON
  88           KM        AND SOUTH OF CAVASSON
  89           BA  0.0193
  90           LG    0.25    0.25    4.03    0.56      45
  91           UC   0.276   0.224
  92           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  93           UA     100
               *
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LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  94          KK  SB02NW  DIVERT
  95          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NW
  96          KM        FIRST FLUSH VOL = 0.437 AC-FT
  97          DT  RET2NW   0.437
  98          DI     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
  99          DI    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
 100          DQ     0.0     5.0    10.0    15.0    20.0    25.0    30.0    35.0    40.0    45.0
 101          DQ    50.0    55.0    60.0    65.0    70.0    80.0    90.0   100.0   150.0   200.0
              *

 102          KK  CP-02W COMBINE
 103          KM        COMBINE ROUTE FROM 1A-2W WITH NEW SUB BASIN SB02NW
 104          HC       2   0.305
              *

 105          KK   R2W-2   ROUTE
 106          RK    1380  0.0140   0.018            TRAP   2.000    0.00
              *

 107          KK   SB01C   BASIN
 108          BA   0.027
 109          LG    0.25    0.25    4.03    0.56      45
 110          UC   0.345   0.366
 111          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 112          UA     100
              *

 113          KK   SB01C  DIVERT
 114          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01C
 115          KM        FIRST FLUSH VOL = 0.612 AC-FT
 116          DT  RET01C   0.612
 117          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 118          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *

 119          KK  R1C-1B   ROUTE
 120          RK     542   0.005   0.013            CIRC   5.000
              *

 121          KK   SB01B   BASIN
 122          BA   0.024
 123          LG    0.25    0.25    4.03    0.56      45
 124          UC   0.304   0.298
 125          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 126          UA     100
              *

 127          KK   SB01B  DIVERT
 128          KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB01B
 129          KM        FIRST FLUSH VOL = 0.543 AC-FT
 130          DT  RET01B   0.543
 131          DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 132          DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
              *
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LINE          ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 133          KK  CP-01B COMBINE
 134          KM        COMBINE ROUTE 1C-1B WITH SB01B
 135          HC       2   0.051
              *

 136          KK  R1B-2N   ROUTE
 137          RK    1268   0.015   0.025            TRAP   80.00    0.00
              *

 138          KK  SB02NE   BASIN
 139          BA  0.0335
 140          LG    0.25    0.25    4.03    0.56      45
 141          UC   0.406   0.448
 142          UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 143          UA     100
              *
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  144           KK  SB02NE  DIVERT
  145           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02NE
  146           KM        FIRST FLUSH VOL = 0.760 AC-FT
  147           DT  RET2NE   0.760
  148           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  149           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                *

  150           KK  CP-02N COMBINE
  151           KM        COMBINE ROUTE 1B-2N (BASINS SB01-B AND SB01-C) WITH SB02NE
  152           HC       2   0.085
                *

  153           KK   R2N-2   ROUTE
  154           RK    1380   0.014   0.018            TRAP   2.000    0.00
                *

  155           KK   SB02E   BASIN
  156           BA   0.040
  157           LG    0.10    0.25    4.03    0.61      80
  158           UC   0.369   0.321
  159           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  160           UA     100
                *

  161           KK   SB02E  DIVERT
  162           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB02E
  163           KM        FIRST FLUSH VOL = 0.998 AC-FT
  164           DT   RET2E   0.998
  165           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
  166           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
                *
1                                            HEC-1 INPUT                                               PAGE  5

 LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


  167           KK   SB02W   BASIN
  168           BA  0.0571
  169           LG    0.35    0.35    4.33    0.51       0
  170           UC   0.686   0.535
  171           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
  172           UA     100
                *

  173           KK   CP-02 COMBINE
  174           HC       4  0.1816
                *

  175           KK    R2-3   ROUTE
  176           KM        MILLER RD CHANNEL FROM SR 101L FREEWAY TO MAYO BLVD
  177           RK    1260  0.0015   0.030            TRAP      92       4
                *

  178           KK    SB03   BASIN
  179           BA   0.048
  180           LG    0.15    0.25    4.50    0.47      55
  181           UC   0.364   0.294
  182           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
  183           UA     100
                *

  184           KK   CP-03 COMBINE
  185           HC       2
                *

  186           KK    R3-5   ROUTE
  187           KM        MILLER RD CHANNEL FROM MAYO BLVD TO PRINCESS BLVD
  188           RK    2396  0.0015   0.03             TRAP      98       4
                *

  189           KK    SB05   BASIN
  190           BA   0.126
  191           LG    0.22    0.25    4.50    0.44      48
  192           UC   0.327   0.226
  193           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
  194           UA     100
                *

  195           KK   CP-5A COMBINE
```

```
 196           HC       2
               *

 197           KK   SB07W   BASIN
 198           BA  0.0468
 199           LG    0.25    0.25    4.03    0.56      45
 200           UC   0.323   0.237
 201           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 202           UA     100
               *
```



```
LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 203           KK   SB07W  DIVERT
 204           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB07W
 205           KM        FIRST FLUSH VOL = 1.060 AC-FT
 206           DT  RET07W   1.060
 207           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 208           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 209           KK  R7W-8W   ROUTE
 210           KM        HAYDEN ROAD NORTH CHANNEL FROM LEGACY BLVD TO SR 101L FREEWAY
 211           KM        HE MODIFIED ROUTE TO LOOP 101 W OF HAYDEN
 212           RK    3754  0.0013   0.03            TRAP      46       4
               *

 213           KK   SB8NW   BASIN
 214           KM        BASIN JUST SOUTH OF LEGACY - FUTURE MOB SITE
 215           BA  0.0147
 216           LG    0.10    0.25    4.03    0.61      80
 217           UC   0.252   0.200
 218           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 219           UA     100
               *

 220           KK   SB8NW  DIVERT
 221           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NW
 222           KM        FIRST FLUSH VOL = 0.332 AC-FT
 223           DT  RET8NW   0.332
 224           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 225           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 226           KK  R8NW8N   ROUTE
 227           KM        ROUTE NEW SUB BASIN SB8NW TO OUTLET AT CAVASSON BLVD
 228           KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 229           RK     643  0.0113   0.018           TRAP      46       4
               *

 230           KK   SB8NE   BASIN
 231           KM        SITE JUST SOUTH OF LEGACY AND WEST OF HAYDEN - FUTURE RETAIL
 232           BA   0.015
 233           LG    0.10    0.25    4.03    0.61      80
 234           UC   0.239   0.172
 235           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 236           UA     100
               *

 237           KK   SB8NE  DIVERT
 238           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8NE
 239           KM        FIRST FLUSH VOL = 0.340 AC-FT
 240           DT  RET8NE   0.340
 241           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 242           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *
```



```
LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 243           KK  R8NE8N   ROUTE
 244           KM        ROUTE NEW SUB BASIN SB8NE TO OUTLET AT CAVASSON BLVD
 245           KM        NORTH HALF OF BASIN 8W WAS BROKEN DOWN INTO 4 SUB-SUBBASINS
 246           RK     622   0.011   0.018           TRAP      46       4
               *

 247           KK   SB8SW   BASIN
```

```
 248           KM        SITE JUST NORTH OF CAVASSON BLVD EAST OF CLARET DR - FUTURE MF
 249           BA  0.0129
 250           LG    0.10    0.25    4.03    0.61      80
 251           UC   0.219   0.169
 252           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 253           UA     100
               *

 254           KK   SB8SW  DIVERT
 255           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SW
 256           KM        FIRST FLUSH VOL = 0.292 AC-FT
 257           DT  RET8SW   0.292
 258           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 259           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 260           KK   SB8SE   BASIN
 261           KM        SITE JUST NORTH OF CAVASSON AND WEST OF HAYDEN - FUTURE HOTEL OR RETAIL
 262           BA  0.0131
 263           LG    0.10    0.25    4.03    0.61      80
 264           UC   0.242   0.210
 265           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 266           UA     100
               *

 267           KK   SB8SE  DIVERT
 268           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB8SE
 269           KM        FIRST FLUSH VOL = 0.297 AC-FT
 270           DT  RET8SE   0.297
 271           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 272           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 273           KK  CP-08N COMBINE
 274           KM        CP AT INLET ON NORTH SIDE OF CAVASSON - ENTERS EXIST STORM DRAIN
 275           HC       4  0.0557
               *

 276           KK  R8N-8W   ROUTE
 277           KM        ROUTE NEW SUB BASINS SB8NE SB8NW SB8SW SB8SE TO CP-08W
 278           KM        ROUTE IN STORM DRAIN
 279           RK    1592   0.015   0.013            CIRC       3
               *
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```
LINE           ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


 280           KK   SB08W   BASIN
 281           BA  0.0589
 282           LG    0.10    0.25    4.03    0.61      80
 283           UC   0.225   0.150
 284           UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
 285           UA     100
               *

 286           KK   SB08W  DIVERT
 287           KM        DIVERSION OF FIRST FLUSH VOLUME FOR BASIN SB08W
 288           KM        FIRST FLUSH VOL REQ = 1.335 AC-FT, PROVIDED = 1.415
 289           KM        THIS IS EXISTING AND WAS BUILT IN PHASE 1
 290           DT   RET8W   1.415
 291           DI     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
 292           DQ     0.0    10.0    20.0    30.0    40.0    50.0    60.0   100.0   150.0   200.0
               *

 293           KK  CP-08W COMBINE
 294           HC       3  0.1614
               *
               *

 295           KK   R8W-4   ROUTE
 296           RK    3137  0.0130   0.030            TRAP   2.000    0.00
               *

 297           KK    SB04   BASIN
 298           BA   0.144
 299           LG    0.14    0.25    4.60    0.44      61
 300           UC   0.305   0.199
 301           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
 302           UA     100
```

```
                  *

   303            KK    CP-4 COMBINE
   304            HC       2
                  *

   305            KK    R4-5   ROUTE
   306            KM        PRINCESS BLVD CHANNEL FROM 77TH ST TO 76TH ST
   307            RK    1992  0.0013   0.03             TRAP      39       4
                  *

   308            KK   CP-05 COMBINE
   309            KM        PRINCESS BLVD AND 76TH ST (PRINCESS BLVD CHANNEL)
   310            KM        COMBINE CP-05A AND R4-5
   311            HC       2
                  *
1                                          HEC-1 INPUT                                             PAGE  9

          LINE            ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


   312            KK    R5-6   ROUTE
   313            KM        PRINCESS BLVD CHANNEL FROM 76TH ST TO SCOTTSDALE RD
   314            RK    1694  0.0015   0.03             TRAP     188       4
                  *

   315            KK    SB06   BASIN
   316            BA   0.136
   317            LG    0.16    0.25    4.55    0.45      53
   318            UC   0.321   0.246
   319            UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
   320            UA     100
                  *

   321            KK   CP-06 COMBINE
   322            HC       2
                  *

   323            KK   SB07E   BASIN
   324            BA   0.026
   325            LG    0.10    0.25    4.03    0.74       0
   326            UC   0.574   0.527
   327            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
   328            UA     100
                  *

   329            KK  R7E-8E   ROUTE
   330            RK    3662  0.0130   0.030            TRAP   2.000    0.00
                  *

   331            KK   SB08E   BASIN
   332            BA   0.078
   333            LG    0.10    0.25    4.08    0.72       0
   334            UC   0.798   0.652
   335            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
   336            UA     100
                  *

   337            KK   SB09N   BASIN
   338            BA   0.028
   339            LG    0.10    0.25    4.79    0.49       0
   340            UC   0.740   1.111
   341            UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
   342            UA     100
                  *

   343            KK  R9N-R9   ROUTE
   344            RK    2586  0.0013   0.020            TRAP   2.000    0.00
                  *
1                                          HEC-1 INPUT                                             PAGE 10

          LINE            ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10


   345            KK  CP-08E COMBINE
   346            HC       3
                  *

   347            KK  R8E-R9   ROUTE
   348            RK     501  0.0013   0.020            TRAP   2.000    0.00
```

```
              *

   349        KK    SB09   BASIN
   350        BA   0.086
   351        LG    0.10    0.25    4.79    0.49       0
   352        UC   0.683   0.518
   353        UA       0     3.0     5.0     8.0    12.0    20.0    43.0    75.0    90.0    96.0
   354        UA     100
              *

   355        KK   CP-09 COMBINE
   356        HC       2
              *

   357        KK   PWRCH
   358        KM        POWERLINE CHANNEL (XS 107) & 50% PIMA ROAD CHANNEL (XS 183) HYDROGRAPHS
   359        KM        100-YR, 24-HR FLO-2D MODEL (XS 107)
   360        KM        FROM PINNACLE PEAK SOUTH ADMS 100-YR, 24-HR FLO-2D MODEL
   361        BA     7.0
   362        QI       0       0       0       0       0       0       0       0       0       0
   363        QI       0       0       0       0       0       0       0       0       0       0
   364        QI       0       0       0       0       0       0       0       0       0       0
   365        QI       0       0       0       0       0       0       0       0       0       0
   366        QI       0       0       0       0       0       1       1       3      43     495
   367        QI    1099    1198    2055    3747    3658    3175    2822    2378    2084    1898
   368        QI    1699    1499    1331    1179    1047     940     851     773     705     645
   369        QI     597     559     521     488     460     435     410     387     370     356
   370        QI     339     328     317     303     293     281     270     257     258     248
   371        QI     240     237     223     220     214     209     205     201     192     188
   372        QI     180     179     164     158     149     142     137     126     120     117
   373        QI     107     102      97      99      86      83      82      74      70      66
   374        QI      62      59      57      55      52      50      50      48      42      41
   375        QI      40      37      35      40      36      32      31      30      31      30
   376        QI      31      30      24      23      23
              *

   377        KK  BINFLO
   378        KM         TOTAL INFLOW INTO BASIN 53R.
   379        HC       2
              *

   380        KK   BASIN
   381        KM     BASIN STAGE/STORAGE FROM PROPOSED CONTOURS BETWEEN
   382        KM     ELEV 1594 AND 1615; BASIN SIDE SLOPES STEEPENED TO 3:1
   383        KM        OUTFLOW RATING CURVE FROM CULVERTMASTER FOR 2-60" PIPES
   384        KM        THE OUTLET PIPES ARE INLET CONTROLLED.
   385        RS       1    STOR       0
   386        SV       0    44.9    76.8   108.7   140.6   171.5   202.5   233.5   264.5   295.5
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  LINE        ID.......1.......2.......3.......4.......5.......6.......7.......8.......9......10

   387        SV   328.7   362.0   395.2   428.5   461.7   497.2   532.7   568.3   603.8   639.3
   388        SE    1594    1597    1598    1599    1600    1601    1602    1603    1604    1605
   389        SE    1606    1607    1608    1609    1610    1611    1612    1613    1614    1615
   390        SQ       0    94.6   153.4   209.8   244.2   264.5   284.0   302.5   320.1   336.9
   391        SQ   353.0   368.5   383.5   398.0   412.0   425.7   438.9   451.9   464.5   476.8
              *

   392        KK  BSNRT1
   393        KM        2-60" CMP OULFLOW PIPES FOR BASIN 53R UNDER SR 101L FREEWAY.
   394        KM        DOWNSTREAM CONNECTING PIPES ARE 60-INCH RCP AND WILL HAVE
   395        KM        EXCESS CAPACITY.
   396        RK     550  0.0052   0.024            CIRC     7.0
              *

   397        KK  BSNRT2
   398        KM        2-60" RCP PIPES FROM SR 101L FREEWAY TO UNION HILLS DR (BAS 53R OUTFALL)
   399        RK    1200  0.0077   0.013            CIRC     7.0
              *

   400        KK    SB10   BASIN
   401        BA   0.040
   402        LG    0.15    0.25    4.25    0.55      55
   403        UC   0.233   0.161
   404        UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
   405        UA     100
              *

   406        KK   CP-10 COMBINE
```

```
                407           KM        UNION HILLS DR AND 82ND ST (UNION HILLS DR CHANNEL)
                408           HC       2
                              *

                409           KK  R10-11   ROUTE
                410           KM        UNION HILLS DR CHANNEL FROM 82ND ST TO HAYDEN ROAD
                411           RK    1277  0.0014   0.03             TRAP     220       4
                              *

                412           KK    SB11   BASIN
                413           BA   0.071
                414           LG    0.15    0.25    4.15    0.58      55
                415           UC   0.296   0.232
                416           UA       0     5.0    16.0    30.0    65.0    77.0    84.0    90.0    94.0    97.0
                417           UA     100
                              *

                418           KK   CP-11 COMBINE
                419           KM        UNION HILLS DR AND HAYDEN ROAD (HAYDEN RD SOUTH CHANNEL)
                420           HC       2
                              *
                421           ZZ
    1
                     SCHEMATIC DIAGRAM OF STREAM NETWORK
     INPUT
      LINE      (V) ROUTING          (--->) DIVERSION OR PUMP FLOW

       NO.      (.) CONNECTOR        (<---) RETURN OF DIVERTED OR PUMPED FLOW

        45      76THST
                     .
                     .
        64           .       SB01A
                     .           .
                     .           .
        77           .           .------->  RET01A
        74           .       SB01A
                     .           .
                     .           .
        80        CP-1............
                     V
                     V
        83      R1A-2W
                     .
                     .
        86           .      SB02NW
                     .           .
                     .           .
        97           .           .------->  RET2NW
        94           .      SB02NW
                     .           .
                     .           .
       102      CP-02W............
                     V
                     V
       105       R2W-2
                     .
                     .
       107           .       SB01C
                     .           .
                     .           .
       116           .           .------->  RET01C
       113           .       SB01C
                     .           V
                     .           V
       119           .      R1C-1B
                     .           .
                     .           .
       121           .           .       SB01B
                     .           .           .
                     .           .           .
       130           .           .           .------->  RET01B
       127           .           .       SB01B
                     .           .           .
                     .           .           .
       133           .      CP-01B............
                     .           V
                     .           V
       136           .      R1B-2N
                     .           .
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              .           .
138           .           .      SB02NE
              .           .           .
              .           .           .
147           .           .           .------->  RET2NE
144           .           .      SB02NE
              .           .           .
              .           .           .
150           .      CP-02N............
              .           V
              .           V
153           .       R2N-2
              .           .
              .           .
155           .           .       SB02E
              .           .           .
              .           .           .
164           .           .           .------->   RET2E
161           .           .       SB02E
              .           .           .
              .           .           .
167           .           .           .       SB02W
              .           .           .           .
              .           .           .           .
173       CP-02....................................
              V
              V
175        R2-3
              .
              .
178           .        SB03
              .           .
              .           .
184       CP-03............
              V
              V
186        R3-5
              .
              .
189           .        SB05
              .           .
              .           .
195       CP-5A............
              .
              .
197           .       SB07W
              .           .
              .           .
206           .           .------->  RET07W
203           .       SB07W
              .           V
              .           V
209           .      R7W-8W
              .           .
              .           .
213           .           .       SB8NW
              .           .           .
              .           .           .
223           .           .           .------->  RET8NW
220           .           .       SB8NW
              .           .           V
              .           .           V
226           .           .      R8NW8N
              .           .           .
              .           .           .
230           .           .           .       SB8NE
              .           .           .           .
              .           .           .           .
240           .           .           .           .------->  RET8NE
237           .           .           .       SB8NE
              .           .           .           V
              .           .           .           V
243           .           .           .      R8NE8N
              .           .           .           .
              .           .           .           .
247           .           .           .           .       SB8SW
              .           .           .           .           .
              .           .           .           .           .
257           .           .           .           .           .------->  RET8SW
254           .           .           .           .       SB8SW
```

```
              .           .           .           .           .
              .           .           .           .           .
260           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
270           .           .           .           .           .           .------->   RET8SE
267           .           .           .           .           .       SB8SE
              .           .           .           .           .           .
              .           .           .           .           .           .
273           .           .      CP-08N....................................
              .           .           V
              .           .           V
276           .           .      R8N-8W
              .           .           .
              .           .           .
280           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
290           .           .           .           .------->   RET8W
286           .           .           .       SB08W
              .           .           .           .
              .           .           .           .
293           .      CP-08W........................
              .           V
              .           V
295           .       R8W-4
              .           .
              .           .
297           .           .        SB04
              .           .           .
              .           .           .
303           .        CP-4............
              .           V
              .           V
305           .        R4-5
              .           .
              .           .
308       CP-05............
              V
              V
312        R5-6
              .
              .
315           .        SB06
              .           .
              .           .
321       CP-06............
              .
              .
323           .       SB07E
              .           V
              .           V
329           .      R7E-8E
              .           .
              .           .
331           .           .       SB08E
              .           .           .
              .           .           .
337           .           .           .       SB09N
              .           .           .           V
              .           .           .           V
343           .           .           .      R9N-R9
              .           .           .           .
              .           .           .           .
345           .      CP-08E........................
              .           V
              .           V
347           .      R8E-R9
              .           .
              .           .
349           .           .        SB09
              .           .           .
              .           .           .
355           .       CP-09............
              .           .
              .           .
357           .           .       PWRCH
              .           .           .
              .           .           .
377           .      BINFLO............
```

```
                .           V
                .           V
   380          .       BASIN
                .           V
                .           V
   392          .      BSNRT1
                .           V
                .           V
   397          .      BSNRT2
                .           .
                .           .
   400          .           .        SB10
                .           .           .
                .           .           .
   406          .       CP-10............
                .           V
                .           V
   409          .      R10-11
                .           .
                .           .
   412          .           .        SB11
                .           .           .
                .           .           .
   418          .       CP-11............

 (***) RUNOFF ALSO COMPUTED AT THIS LOCATION
1*****************************************
***************************************
 *                                       *                                                    *
*
 *   FLOOD HYDROGRAPH PACKAGE  (HEC-1)   *                                                    *    U.S. ARMY CORPS OF ENGINEERS
*
 *              JUN   1998               *                                                    *    HYDROLOGIC ENGINEERING CENTER
*
 *           VERSION 4.1                 *                                                    *          609 SECOND STREET
*
 *                                       *                                                    *       DAVIS, CALIFORNIA 95616
*
 *  RUN DATE   26APR21  TIME  09:31:12   *                                                    *           (916) 756-1104
*
 *                                       *                                                    *
*
 *****************************************
***************************************



                                     CROSS ROADS EAST DRAINAGE INFRASTRUCTURE
                                     POWERLINE CORRIDOR
                                     RESERVE ROUTING OF BASIN 53R N OF SR101 & E OF HAYDEN RD, SCOTTSDALE, AZ
                                     INFLOW HYDROGRAPHS FROM THE PPSADMS DRAFT FLO-2D MODEL:
                                        100-YR, 24-HR BASE W/WALLS (W/ MODIFICATIONS TO CONTAIN POWERLINE AND
                                        PIMA ROAD FLOWS.

                                     PREPARED BY: T.Y.LIN INTERNATIONAL; LAST MODIFIED: 09/14
                                        MODELERS: RK, MW

                                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 08/07/18
                                        MODELERS: MSW, ES
                                        REVISIONS NOTED WITH HE
                                        NEW PROP CONDITION MODEL,WITH POWERLINE AND BASIN 53R
                                        THIS IS THE ULTIMATE CONDITION MODEL

                                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 10/25/19
                                        MODELERS: MSW
                                        XKSAT ADJUSTMENT

                                     REVISED BY: HUBBARD ENGINEERING; LAST MODIFIED: 07/15/20
                                        MODELERS: MSW, TSW
                                        REVISIONS NOTED WITH HE (Hubbard Engineering)
                                        NEW PROPOSED CONDITION MODEL, ACCOUNT FOR WHAT HAS BEEN
                                        CONSTRUCTED IN PHASE 1 INFRASTRUCTURE, DETAIL OUT NEW CP
                                        ACCOUNT FOR HIGHER C FACTOR AND IMPERVIOUSNESS, ADD RETENTION
                                        FIRST FLUSH RETENTION ADDED AS DIVERT CARD WITH VOLUME REQUIRED


   31 IO          OUTPUT CONTROL VARIABLES
                        IPRNT           5  PRINT CONTROL
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                 IPLOT           0  PLOT CONTROL
                 QSCAL          0.  HYDROGRAPH PLOT SCALE

   IT          HYDROGRAPH TIME DATA
                  NMIN           3  MINUTES IN COMPUTATION INTERVAL
                 IDATE      1    0  STARTING DATE
                 ITIME        0000  STARTING TIME
                    NQ        1000  NUMBER OF HYDROGRAPH ORDINATES
                NDDATE      3    0  ENDING DATE
                NDTIME        0157  ENDING TIME
                ICENT           19  CENTURY MARK

            COMPUTATION INTERVAL    0.05 HOURS
                 TOTAL TIME BASE   49.95 HOURS

        ENGLISH UNITS
             DRAINAGE AREA         SQUARE MILES
             PRECIPITATION DEPTH   INCHES
             LENGTH, ELEVATION     FEET
             FLOW                  CUBIC FEET PER SECOND
             STORAGE VOLUME        ACRE-FEET
             SURFACE AREA          ACRES
             TEMPERATURE           DEGREES FAHRENHEIT

32 JD          INDEX STORM NO. 1
                  STRM        3.85  PRECIPITATION DEPTH
                  TRDA        0.00  TRANSPOSITION DRAINAGE AREA

33 PI            PRECIPITATION PATTERN
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                0.02      0.02      0.02      0.02      0.02      0.06      0.06      0.06      0.06      0.06
                0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

43 JD          INDEX STORM NO. 2
                  STRM        3.66  PRECIPITATION DEPTH
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                    TRDA       10.00  TRANSPOSITION DRAINAGE AREA

  0 PI            PRECIPITATION PATTERN
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
                     0.02      0.02      0.02      0.02      0.02      0.06      0.06      0.06      0.06      0.06
                     0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01      0.01
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00

 44 JD          INDEX STORM NO. 3
                    STRM        3.53  PRECIPITATION DEPTH
                    TRDA       20.00  TRANSPOSITION DRAINAGE AREA

  0 PI            PRECIPITATION PATTERN
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00      0.00
                     0.00      0.00      0.00      0.00      0.00      0.01      0.01      0.01      0.01      0.01
```

0.02	0.02	0.02	0.02	0.02	0.06	0.06	0.06	0.06	0.06
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

*** FDKRUT WARNING TIME STEP CALCULATION FAILED TO CONVERGE. STABILITY PROBLEMS MAY RESULT

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RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+	HYDROGRAPH AT	76THST	69.	12.50	23.	8.	4.	0.24		
+	HYDROGRAPH AT	SB01A	86.	12.25	11.	4.	2.	0.06		
+	DIVERSION TO	RET01A	6.	11.55	2.	1.	0.	0.06		
+	HYDROGRAPH AT	SB01A	86.	12.25	10.	3.	1.	0.06		
+	2 COMBINED AT	CP-1	135.	12.30	33.	11.	5.	0.30		
+	ROUTED TO	R1A-2W	134.	12.30	33.	11.	5.	0.30		
+	HYDROGRAPH AT	SB02NW	29.	12.15	3.	1.	1.	0.02		
+	DIVERSION TO	RET2NW	2.	11.45	1.	0.	0.	0.02		
+	HYDROGRAPH AT	SB02NW	29.	12.15	3.	1.	0.	0.02		
+	2 COMBINED AT	CP-02W	156.	12.25	36.	12.	6.	0.31		
+	ROUTED TO	R2W-2	155.	12.25	36.	12.	6.	0.31		
+	HYDROGRAPH AT	SB01C	32.	12.20	5.	2.	1.	0.03		
+	DIVERSION TO	RET01C	3.	11.60	1.	0.	0.	0.03		

+	HYDROGRAPH AT	SB01C	32.	12.20	5.	1.	1.	0.03
+	ROUTED TO	R1C-1B	31.	12.25	5.	1.	1.	0.03
+	HYDROGRAPH AT	SB01B	32.	12.20	4.	1.	1.	0.02
+	DIVERSION TO	RET01B	2.	11.50	1.	0.	0.	0.02
+	HYDROGRAPH AT	SB01B	32.	12.20	4.	1.	1.	0.02
+	2 COMBINED AT	CP-01B	63.	12.20	9.	2.	1.	0.05
+	ROUTED TO	R1B-2N	62.	12.25	9.	2.	1.	0.05
+	HYDROGRAPH AT	SB02NE	35.	12.25	6.	2.	1.	0.03
+	DIVERSION TO	RET2NE	3.	11.70	1.	0.	0.	0.03
+	HYDROGRAPH AT	SB02NE	35.	12.25	6.	2.	1.	0.03
+	2 COMBINED AT	CP-02N	96.	12.25	14.	4.	2.	0.09
+	ROUTED TO	R2N-2	95.	12.25	14.	4.	2.	0.09
+	HYDROGRAPH AT	SB02E	57.	12.25	10.	3.	2.	0.04
+	DIVERSION TO	RET2E	2.	9.65	1.	1.	0.	0.04
+	HYDROGRAPH AT	SB02E	57.	12.25	10.	3.	1.	0.04
+	HYDROGRAPH AT	SB02W	34.	12.45	5.	1.	1.	0.06
+	4 COMBINED AT	CP-02	324.	12.30	64.	20.	10.	0.18
+	ROUTED TO	R2-3	324.	12.35	64.	20.	10.	0.18
+	HYDROGRAPH AT	SB03	65.	12.15	10.	3.	2.	0.05
+	2 COMBINED AT	CP-03	367.	12.35	73.	23.	11.	0.23
+	ROUTED TO	R3-5	365.	12.45	72.	23.	11.	0.23
+	HYDROGRAPH AT	SB05	190.	12.10	24.	8.	4.	0.13
+	2 COMBINED AT	CP-5A	437.	12.45	94.	30.	15.	0.36
+	HYDROGRAPH AT	SB07W	68.	12.20	8.	3.	1.	0.05
+	DIVERSION TO	RET07W	4.	11.50	2.	1.	0.	0.05
+	HYDROGRAPH AT	SB07W	68.	12.20	8.	2.	1.	0.05
+	ROUTED TO	R7W-8W	66.	12.60	8.	2.	1.	0.05

	HYDROGRAPH AT							
+		SB8NW	27.	12.15	4.	1.	1.	0.01
	DIVERSION TO							
+		RET8NW	1.	9.00	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8NW	27.	12.15	4.	1.	1.	0.01
	ROUTED TO							
+		R8NW8N	26.	12.15	4.	1.	1.	0.01
	HYDROGRAPH AT							
+		SB8NE	29.	12.10	4.	1.	1.	0.01
	DIVERSION TO							
+		RET8NE	1.	8.95	1.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8NE	29.	12.10	4.	1.	1.	0.01
	ROUTED TO							
+		R8NE8N	28.	12.15	4.	1.	1.	0.01
	HYDROGRAPH AT							
+		SB8SW	25.	12.10	3.	1.	1.	0.01
	DIVERSION TO							
+		RET8SW	1.	8.95	0.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SW	25.	12.10	3.	1.	0.	0.01
	HYDROGRAPH AT							
+		SB8SE	23.	12.15	3.	1.	1.	0.01
	DIVERSION TO							
+		RET8SE	1.	9.00	0.	0.	0.	0.01
	HYDROGRAPH AT							
+		SB8SE	23.	12.15	3.	1.	0.	0.01
	4 COMBINED AT							
+		CP-08N	102.	12.15	14.	4.	2.	0.06
	ROUTED TO							
+		R8N-8W	101.	12.15	14.	4.	2.	0.06
	HYDROGRAPH AT							
+		SB08W	118.	12.10	14.	5.	2.	0.06
	DIVERSION TO							
+		RET8W	3.	9.20	2.	1.	0.	0.06
	HYDROGRAPH AT							
+		SB08W	118.	12.10	14.	4.	2.	0.06
	3 COMBINED AT							
+		CP-08W	214.	12.10	36.	11.	5.	0.16
	ROUTED TO							
+		R8W-4	213.	12.15	36.	11.	5.	0.16
	HYDROGRAPH AT							
+		SB04	241.	12.10	31.	10.	5.	0.14
	2 COMBINED AT							
+		CP-4	446.	12.10	66.	21.	10.	0.31
	ROUTED TO							
+		R4-5	442.	12.20	66.	21.	10.	0.31
	2 COMBINED AT							
+		CP-05	723.	12.35	158.	50.	25.	0.66
	ROUTED TO							
+		R5-6	717.	12.40	158.	50.	25.	0.66
	HYDROGRAPH AT							

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+                        SB06       202.   12.10          27.          9.          4.       0.14

         2 COMBINED AT
+                       CP-06       817.   12.35         184.         59.         29.       0.80

         HYDROGRAPH AT
+                       SB07E        16.   12.40           2.          1.          0.       0.03

         ROUTED TO
+                      R7E-8E        16.   12.45           2.          1.          0.       0.03

         HYDROGRAPH AT
+                       SB08E        39.   12.55           6.          2.          1.       0.08

         HYDROGRAPH AT
+                       SB09N        11.   12.55           3.          1.          0.       0.03

         ROUTED TO
+                      R9N-R9        11.   12.65           3.          1.          0.       0.03

         3 COMBINED AT
+                      CP-08E        63.   12.55          11.          3.          1.       0.13

         ROUTED TO
+                      R8E-R9        63.   12.55          11.          3.          1.       0.13

         HYDROGRAPH AT
+                        SB09        60.   12.45           9.          2.          1.       0.09

         2 COMBINED AT
+                       CP-09       120.   12.50          20.          5.          2.       0.22

         HYDROGRAPH AT
+                       PWRCH      3747.   13.25        1540.        502.        248.       7.00

         2 COMBINED AT
+                      BINFLO      3788.   13.25        1558.        507.        250.       7.22

         ROUTED TO
+                       BASIN       462.   18.45         458.        399.        241.       7.22

         ROUTED TO
+                      BSNRT1       462.   18.50         458.        399.        241.       7.22

         ROUTED TO
+                      BSNRT2       462.   18.50         458.        399.        241.       7.22

         HYDROGRAPH AT
+                        SB10        72.   12.05           8.          3.          1.       0.04

         2 COMBINED AT
+                       CP-10       463.   18.50         459.        400.        242.       7.26

         ROUTED TO
+                      R10-11       463.   18.55         459.        400.        241.       7.26

         HYDROGRAPH AT
+                        SB11       107.   12.10          14.          5.          2.       0.07

         2 COMBINED AT
+                       CP-11       465.   18.55         460.        401.        244.       7.33
1
                                           SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
                                               (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)
                                                                                  INTERPOLATED TO
                                                                               COMPUTATION INTERVAL
           ISTAQ    ELEMENT      DT       PEAK    TIME TO     VOLUME      DT       PEAK     TIME TO     VOLUME
                                                    PEAK                                      PEAK

                                (MIN)     (CFS)      (MIN)     (IN)     (MIN)     (CFS)     (MIN)       (IN)

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1A-2W  MANE         0.27    138.22    738.13      1.40      3.00    138.18    738.00        1.40

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2249E+02 EXCESS=0.0000E+00 OUTFLOW=0.2249E+02 BASIN STORAGE=0.2963E-02 PERCENT ERROR=   0.0


          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R1A-2W  MANE         0.23    132.78    738.11      1.37      3.00    132.73    738.00        1.37
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2206E+02 EXCESS=0.0000E+00 OUTFLOW=0.2206E+02 BASIN STORAGE=0.2963E-02 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R1A-2W  MANE        0.23    129.25    738.12      1.36      3.00    129.19    738.00        1.36

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2178E+02 EXCESS=0.0000E+00 OUTFLOW=0.2178E+02 BASIN STORAGE=0.2963E-02 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2W-2  MANE        0.20    160.62    735.42      1.50      3.00    159.58    735.00        1.50

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2435E+02 EXCESS=0.0000E+00 OUTFLOW=0.2435E+02 BASIN STORAGE=0.4089E-02 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
             R2W-2  MANE        0.28    153.29    735.70      1.46      3.00    152.40    738.00        1.46

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2379E+02 EXCESS=0.0000E+00 OUTFLOW=0.2378E+02 BASIN STORAGE=0.4089E-02 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             R2W-2  MANE        0.22    148.68    735.55      1.44      3.00    147.99    738.00        1.44

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2342E+02 EXCESS=0.0000E+00 OUTFLOW=0.2342E+02 BASIN STORAGE=0.4090E-02 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1C-1B  MANE        0.43     32.64    732.90      1.80      3.00     32.39    735.00        1.81

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2599E+01 EXCESS=0.0000E+00 OUTFLOW=0.2599E+01 BASIN STORAGE=0.9756E-13 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R1C-1B  MANE        0.39     30.53    732.80      1.67      3.00     30.31    735.00        1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2408E+01 EXCESS=0.0000E+00 OUTFLOW=0.2408E+01 BASIN STORAGE=0.9547E-13 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R1C-1B  MANE        0.44     29.15    732.86      1.59      3.00     28.96    735.00        1.59

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2284E+01 EXCESS=0.0000E+00 OUTFLOW=0.2285E+01 BASIN STORAGE=0.9767E-13 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R1B-2N  MANE        1.55     64.38    735.51      1.81      3.00     64.19    735.00        1.81

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4913E+01 EXCESS=0.0000E+00 OUTFLOW=0.4913E+01 BASIN STORAGE=0.5632E-04 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R1B-2N  MANE        1.51     60.32    734.68      1.67      3.00     60.22    735.00        1.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4553E+01 EXCESS=0.0000E+00 OUTFLOW=0.4552E+01 BASIN STORAGE=0.5975E-04 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R1B-2N  MANE        1.46     57.82    735.22      1.59      3.00     57.66    735.00        1.59

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4319E+01 EXCESS=0.0000E+00 OUTFLOW=0.4320E+01 BASIN STORAGE=0.5659E-04 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
             R2N-2  MANE        0.26     99.78    735.68      1.80      3.00     99.10    735.00        1.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8138E+01 EXCESS=0.0000E+00 OUTFLOW=0.8139E+01 BASIN STORAGE=0.1854E-04 PERCENT ERROR=   0.0
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          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
             R2N-2  MANE        0.41     93.34    735.52      1.66      3.00     92.74    735.00        1.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7542E+01 EXCESS=0.0000E+00 OUTFLOW=0.7543E+01 BASIN STORAGE=0.1946E-04 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
             R2N-2  MANE        0.30     89.45    735.69      1.58      3.00     88.67    735.00        1.58

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7158E+01 EXCESS=0.0000E+00 OUTFLOW=0.7159E+01 BASIN STORAGE=0.1834E-04 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R2-3  MANE        2.04    336.24    739.95      4.22      3.00    335.93    741.00        4.22

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4096E+02 EXCESS=0.0000E+00 OUTFLOW=0.4090E+02 BASIN STORAGE=0.3882E-01 PERCENT ERROR=   0.1

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R2-3  MANE        2.10    317.06    739.89      4.04      3.00    316.94    741.00        4.03

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3916E+02 EXCESS=0.0000E+00 OUTFLOW=0.3908E+02 BASIN STORAGE=0.3878E-01 PERCENT ERROR=   0.1

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R2-3  MANE        2.15    304.76    739.74      3.92      3.00    304.74    741.00        3.92

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3802E+02 EXCESS=0.0000E+00 OUTFLOW=0.3794E+02 BASIN STORAGE=0.3635E-01 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R3-5  MANE        2.78    380.48    747.40      3.86      3.00    378.99    747.00        3.86

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4743E+02 EXCESS=0.0000E+00 OUTFLOW=0.4725E+02 BASIN STORAGE=0.8213E-01 PERCENT ERROR=   0.2

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R3-5  MANE        2.76    359.46    749.07      3.68      3.00    358.16    747.00        3.68

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4525E+02 EXCESS=0.0000E+00 OUTFLOW=0.4507E+02 BASIN STORAGE=0.8573E-01 PERCENT ERROR=   0.2

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R3-5  MANE        2.78    345.68    749.01      3.57      3.00    343.26    747.00        3.57

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4389E+02 EXCESS=0.0000E+00 OUTFLOW=0.4376E+02 BASIN STORAGE=0.8213E-01 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7W-8W  MANE        3.00     68.93    754.57      1.91      3.00     68.50    756.00        1.91

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4511E+01 EXCESS=0.0000E+00 OUTFLOW=0.4779E+01 BASIN STORAGE=0.9374E-03 PERCENT ERROR=  -6.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R7W-8W  MANE        3.00     64.32    754.67      1.83      3.00     64.21    756.00        1.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4179E+01 EXCESS=0.0000E+00 OUTFLOW=0.4572E+01 BASIN STORAGE=0.8368E-03 PERCENT ERROR=  -9.4

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R7W-8W  MANE        3.00     63.27    756.75      1.63      3.00     59.66    759.00        1.63

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3965E+01 EXCESS=0.0000E+00 OUTFLOW=0.4073E+01 BASIN STORAGE=0.8362E-03 PERCENT ERROR=  -2.8

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
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          R8NW8N  MANE         0.83     27.24    730.38      2.83      3.00     27.08    729.00        2.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2215E+01 EXCESS=0.0000E+00 OUTFLOW=0.2217E+01 BASIN STORAGE=0.3350E-06 PERCENT ERROR=  -0.1

        FOR STORM = 2  STORM AREA (SQ MI) =      10.00
          R8NW8N  MANE         0.90     25.74    730.14      2.66      3.00     25.61    729.00        2.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2082E+01 EXCESS=0.0000E+00 OUTFLOW=0.2084E+01 BASIN STORAGE=0.2981E-06 PERCENT ERROR=  -0.1

        FOR STORM = 3  STORM AREA (SQ MI) =      20.00
          R8NW8N  MANE         0.87     24.80    730.23      2.55      3.00     24.66    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1996E+01 EXCESS=0.0000E+00 OUTFLOW=0.1998E+01 BASIN STORAGE=0.2840E-06 PERCENT ERROR=  -0.1

        FOR STORM = 1  STORM AREA (SQ MI) =       0.00
          R8NE8N  MANE         0.80     29.24    728.00      2.83      3.00     29.14    729.00        2.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2259E+01 EXCESS=0.0000E+00 OUTFLOW=0.2263E+01 BASIN STORAGE=0.3312E-06 PERCENT ERROR=  -0.1

        FOR STORM = 2  STORM AREA (SQ MI) =      10.00
          R8NE8N  MANE         0.87     27.62    728.44      2.66      3.00     27.56    729.00        2.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2124E+01 EXCESS=0.0000E+00 OUTFLOW=0.2127E+01 BASIN STORAGE=0.3201E-06 PERCENT ERROR=  -0.2

        FOR STORM = 3  STORM AREA (SQ MI) =      20.00
          R8NE8N  MANE         0.87     26.65    727.97      2.55      3.00     26.55    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2036E+01 EXCESS=0.0000E+00 OUTFLOW=0.2038E+01 BASIN STORAGE=0.3381E-06 PERCENT ERROR=  -0.1

        FOR STORM = 1  STORM AREA (SQ MI) =       0.00
          R8N-8W  MANE         0.63    104.30    729.26      2.83      3.00    104.19    729.00        2.83

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8393E+01 EXCESS=0.0000E+00 OUTFLOW=0.8394E+01 BASIN STORAGE=0.4795E-07 PERCENT ERROR=   0.0

        FOR STORM = 2  STORM AREA (SQ MI) =      10.00
          R8N-8W  MANE         0.63     98.64    729.78      2.66      3.00     98.54    729.00        2.66

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7889E+01 EXCESS=0.0000E+00 OUTFLOW=0.7890E+01 BASIN STORAGE=0.4709E-07 PERCENT ERROR=   0.0

        FOR STORM = 3  STORM AREA (SQ MI) =      20.00
          R8N-8W  MANE         0.51     95.24    729.83      2.55      3.00     94.91    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7563E+01 EXCESS=0.0000E+00 OUTFLOW=0.7564E+01 BASIN STORAGE=0.4620E-07 PERCENT ERROR=   0.0

        FOR STORM = 1  STORM AREA (SQ MI) =       0.00
           R8W-4  MANE         0.71    221.46    727.41      2.55      3.00    220.48    729.00        2.55

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2196E+02 EXCESS=0.0000E+00 OUTFLOW=0.2197E+02 BASIN STORAGE=0.3358E-03 PERCENT ERROR=   0.0

        FOR STORM = 2  STORM AREA (SQ MI) =      10.00
           R8W-4  MANE         0.64    209.50    727.30      2.41      3.00    208.59    729.00        2.41

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2072E+02 EXCESS=0.0000E+00 OUTFLOW=0.2072E+02 BASIN STORAGE=0.3435E-03 PERCENT ERROR=   0.0

        FOR STORM = 3  STORM AREA (SQ MI) =      20.00
           R8W-4  MANE         0.73    201.38    727.98      2.27      3.00    200.85    729.00        2.27
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1955E+02 EXCESS=0.0000E+00 OUTFLOW=0.1955E+02 BASIN STORAGE=0.3555E-03 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R4-5  MANE         2.47    458.39    732.52      2.64      3.00    457.71    732.00        2.64

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4297E+02 EXCESS=0.0000E+00 OUTFLOW=0.4302E+02 BASIN STORAGE=0.1194E-02 PERCENT ERROR=  -0.1

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R4-5  MANE         2.57    435.01    731.91      2.49      3.00    434.59    732.00        2.50

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4059E+02 EXCESS=0.0000E+00 OUTFLOW=0.4063E+02 BASIN STORAGE=0.1021E-02 PERCENT ERROR=  -0.1

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R4-5  MANE         2.59    416.96    732.36      2.38      3.00    415.70    732.00        2.38

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3868E+02 EXCESS=0.0000E+00 OUTFLOW=0.3873E+02 BASIN STORAGE=0.1074E-02 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
              R5-6  MANE         2.34    763.02    745.86      3.01      3.00    762.26    744.00        3.01

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1062E+03 EXCESS=0.0000E+00 OUTFLOW=0.1061E+03 BASIN STORAGE=0.6943E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
              R5-6  MANE         2.51    707.12    745.07      2.85      3.00    703.42    744.00        2.85

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1007E+03 EXCESS=0.0000E+00 OUTFLOW=0.1006E+03 BASIN STORAGE=0.6945E-01 PERCENT ERROR=   0.1

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
              R5-6  MANE         2.52    674.69    746.57      2.74      3.00    673.28    747.00        2.74

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9685E+02 EXCESS=0.0000E+00 OUTFLOW=0.9676E+02 BASIN STORAGE=0.6943E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R7E-8E  MANE         1.93     16.43    748.32      0.80      3.00     16.39    747.00        0.80

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1113E+01 EXCESS=0.0000E+00 OUTFLOW=0.1112E+01 BASIN STORAGE=0.1951E-04 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R7E-8E  MANE         1.94     14.90    748.28      0.72      3.00     14.82    747.00        0.72

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9964E+00 EXCESS=0.0000E+00 OUTFLOW=0.9968E+00 BASIN STORAGE=0.1628E-04 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R7E-8E  MANE         2.13     13.91    748.07      0.67      3.00     13.81    747.00        0.67

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9248E+00 EXCESS=0.0000E+00 OUTFLOW=0.9254E+00 BASIN STORAGE=0.1626E-04 PERCENT ERROR=  -0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R9N-R9  MANE         2.43     11.87    758.35      0.97      3.00     11.83    759.00        0.97

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1453E+01 EXCESS=0.0000E+00 OUTFLOW=0.1451E+01 BASIN STORAGE=0.2431E-04 PERCENT ERROR=   0.1

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R9N-R9  MANE         2.57     10.88    757.62      0.89      3.00     10.84    759.00        0.89

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1330E+01 EXCESS=0.0000E+00 OUTFLOW=0.1328E+01 BASIN STORAGE=0.2146E-04 PERCENT ERROR=   0.1
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          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R9N-R9  MANE        2.61     10.25    758.60      0.84      3.00     10.23    759.00        0.84

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1249E+01 EXCESS=0.0000E+00 OUTFLOW=0.1248E+01 BASIN STORAGE=0.2338E-04 PERCENT ERROR=   0.1

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R8E-R9  MANE        0.31     67.27    753.42      0.85      3.00     67.12    753.00        0.85

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5955E+01 EXCESS=0.0000E+00 OUTFLOW=0.5955E+01 BASIN STORAGE=0.1051E-04 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R8E-R9  MANE        0.38     60.94    753.41      0.76      3.00     60.79    753.00        0.76

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5368E+01 EXCESS=0.0000E+00 OUTFLOW=0.5368E+01 BASIN STORAGE=0.1067E-04 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R8E-R9  MANE        0.36     56.87    753.51      0.71      3.00     56.68    753.00        0.71

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4995E+01 EXCESS=0.0000E+00 OUTFLOW=0.4995E+01 BASIN STORAGE=0.1021E-04 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT1  MANE        0.38    462.76   1109.87      2.58      3.00    462.76   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9944E+03 EXCESS=0.0000E+00 OUTFLOW=0.9943E+03 BASIN STORAGE=0.9346E-01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            BSNRT1  MANE        0.38    462.49   1110.02      2.58      3.00    462.49   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9936E+03 EXCESS=0.0000E+00 OUTFLOW=0.9935E+03 BASIN STORAGE=0.9328E-01 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            BSNRT1  MANE        0.38    462.32   1110.01      2.58      3.00    462.32   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9930E+03 EXCESS=0.0000E+00 OUTFLOW=0.9929E+03 BASIN STORAGE=0.9316E-01 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            BSNRT2  MANE        0.38    462.76   1110.48      2.58      3.00    462.76   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9943E+03 EXCESS=0.0000E+00 OUTFLOW=0.9941E+03 BASIN STORAGE=0.1069E+00 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            BSNRT2  MANE        0.38    462.49   1110.30      2.58      3.00    462.49   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9935E+03 EXCESS=0.0000E+00 OUTFLOW=0.9933E+03 BASIN STORAGE=0.1067E+00 PERCENT ERROR=   0.0

          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            BSNRT2  MANE        0.38    462.32   1110.49      2.58      3.00    462.32   1110.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9929E+03 EXCESS=0.0000E+00 OUTFLOW=0.9927E+03 BASIN STORAGE=0.1065E+00 PERCENT ERROR=   0.0

          FOR STORM = 1  STORM AREA (SQ MI) =       0.00
            R10-11  MANE        2.34    463.63   1114.77      2.58      3.00    463.63   1113.00        2.58

 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9994E+03 EXCESS=0.0000E+00 OUTFLOW=0.9982E+03 BASIN STORAGE=0.1212E+01 PERCENT ERROR=   0.0

          FOR STORM = 2  STORM AREA (SQ MI) =      10.00
            R10-11  MANE        2.39    463.32   1113.19      2.58      3.00    463.32   1113.00        2.58
```

```
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9983E+03 EXCESS=0.0000E+00 OUTFLOW=0.9972E+03 BASIN STORAGE=0.1068E+01 PERCENT ERROR=   0.0


          FOR STORM = 3  STORM AREA (SQ MI) =      20.00
            R10-11  MANE         2.35    463.12   1115.43      2.57      3.00    463.12   1113.00        2.57


CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9977E+03 EXCESS=0.0000E+00 OUTFLOW=0.9964E+03 BASIN STORAGE=0.1562E+01 PERCENT ERROR=   0.0




*** NORMAL END OF HEC-1 ***
```

Appendix E
Hydrology Calculations
Grayhawk Residences at Cavasson

HYDROLOGIC CALCULATION SUMMARY SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Complete calculations for each concentration point are presented in the attached hydrologic calculation sheets.

Sub-Basin & Concentration Point ID	C Runoff Coefficient Frequency			I Intensity Frequency			A Area	Q Peak Discharge Frequency		
	10-year	50-year	100-year	10-year	50-year	100-year		10-year	50-year	100-year
				[in/hr]			[acres]	[cfs]		
DA-A	0.90	0.90	0.90	3.52	5.13	5.78	1.25	3.95	5.75	6.48
DA-B	0.90	0.90	0.90	3.66	5.13	5.78	0.57	1.87	2.63	2.96
DA-C	0.90	0.90	0.90	3.66	5.13	5.78	0.21	0.70	0.98	1.11
DA-D	0.90	0.90	0.90	3.66	5.13	5.78	0.40	1.33	1.86	2.09
DA-E	0.90	0.90	0.90	3.66	5.13	5.78	0.87	2.86	4.01	4.52
DA-F	0.90	0.90	0.90	3.66	5.13	5.78	0.48	1.58	2.22	2.50
DA-G	0.90	0.90	0.90	3.66	5.13	5.78	0.31	1.02	1.43	1.62
DA-H	0.90	0.90	0.90	3.66	5.13	5.78	0.42	1.37	1.92	2.17
DA-I	0.90	0.90	0.90	3.66	5.13	5.78	0.35	1.15	1.61	1.81
DA-J	0.90	0.90	0.90	3.66	5.13	5.78	0.42	1.38	1.93	2.17
DA-K	0.90	0.90	0.90	3.66	5.13	5.78	0.57	1.87	2.63	2.96
DA-L	0.90	0.90	0.90	3.66	5.13	5.78	1.38	4.53	6.35	7.16
DA-M	0.90	0.90	0.90	3.66	5.13	5.78	0.44	1.44	2.01	2.27
DA-N	0.90	0.90	0.90	3.66	5.13	5.78	0.19	0.63	0.89	1.00
DA-O	0.90	0.90	0.90	3.66	5.13	5.78	2.11	6.94	9.72	10.95
DA-P	0.90	0.90	0.90	3.66	5.13	5.78	1.23	4.04	5.66	6.37
DA-Q	0.85	0.90	0.90	3.66	5.13	5.78	0.32	1.01	1.50	1.69
DA-R	0.85	0.90	0.90	3.66	5.13	5.78	0.57	1.77	2.62	2.96
DA-S	0.85	0.90	0.90	3.66	5.13	5.78	0.43	1.32	1.96	2.21
DA-T	0.85	0.90	0.90	3.66	5.13	5.78	0.30	0.93	1.38	1.56
DA-U	0.85	0.90	0.90	3.66	5.13	5.78	0.25	0.77	1.14	1.29
DA-V	0.85	0.90	0.90	3.66	5.13	5.78	0.11	0.35	0.53	0.59
DA-W	0.85	0.90	0.90	3.66	5.13	5.78	0.59	1.83	2.71	3.05
DA-X	0.85	0.90	0.90	3.66	5.13	5.78	0.67	2.08	3.09	3.48
DA-Y	0.85	0.90	0.90	3.66	5.13	5.78	0.25	0.78	1.16	1.31
DA-Z	0.85	0.90	0.90	3.66	5.13	5.78	0.29	0.89	1.33	1.50
DA-AA	0.85	0.90	0.90	3.66	5.13	5.78	0.21	0.66	0.98	1.10
DA-AB	0.85	0.90	0.90	3.66	5.13	5.78	0.28	0.87	1.29	1.45
DA-AC	0.85	0.90	0.90	3.66	5.13	5.78	0.71	2.22	3.30	3.71
DA-AD	0.85	0.90	0.90	3.66	5.13	5.78	0.41	1.28	1.90	2.14
DA-AE	0.85	0.90	0.90	3.66	5.13	5.78	0.50	1.55	2.30	2.59
DA-AF	0.85	0.90	0.90	3.66	5.13	5.78	0.76	2.35	3.49	3.93
PreDevelopment A	0.40	0.48	0.50	3.66	5.13	5.78	3.24	4.74	7.97	9.36
PreDevelopment B	0.40	0.48	0.50	3.66	5.13	5.78	2.50	3.66	6.15	7.22
PreDevelopment C	0.40	0.48	0.50	3.66	5.13	5.78	2.24	3.29	5.53	6.49
PreDevelopment D	0.40	0.48	0.50	3.66	5.13	5.78	2.96	4.33	7.29	8.55
PreDevelopment E	0.40	0.48	0.50	3.66	5.13	5.78	4.63	6.78	11.41	13.39

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-A

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	311.25	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.90	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.50	[ft]
Drainage Area (A)=	1.25	[acres]	Elevation $_{Difference}$:	0.40	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00129	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)(1):
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.039404	11.0	3.52
50	-0.00625	0.04	0.039404	10.0	5.1
100	-0.00625	0.04	0.039404	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}(1) = 0.90
C_{50}(1) = 0.90
C_{100}(1) = 0.90

Q_{10} = 3.95 **[cfs]**
Q_{50} = 5.75 **[cfs]**
Q_{100} = 6.48 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-B

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	47.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.90	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.45	[ft]
Drainage Area (A)=	0.57	[acres]	Elevation $_{Difference}$:	0.45	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00957	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.04153	10.0	3.66
50	-0.00625	0.04	0.04153	10.0	5.1
100	-0.00625	0.04	0.04153	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.90
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 1.87 **[cfs]**
Q_{50} = 2.63 **[cfs]**
Q_{100} = 2.96 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-C

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	43.12	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.90	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.45	[ft]
Drainage Area (A)=	0.21	[acres]	Elevation $_{Difference}$:	0.45	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01044	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044201	10.0	3.66
50	-0.00625	0.04	0.044201	10.0	5.1
100	-0.00625	0.04	0.044201	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.90
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 0.70 **[cfs]**
Q_{50} = 0.98 **[cfs]**
Q_{100} = 1.11 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-D

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	47.30	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.90	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.45	[ft]
Drainage Area (A)=	0.40	[acres]	Elevation $_{Difference}$:	0.45	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00951	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042471	10.0	3.66
50	-0.00625	0.04	0.042471	10.0	5.1
100	-0.00625	0.04	0.042471	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.90
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 1.33 **[cfs]**
Q_{50} = 1.86 **[cfs]**
Q_{100} = 2.09 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-E

Project Name: The Grayhawk Residences at Cavasson | **Prepared by:** TSW | **Date:** 06/03/21
Project No.: 18114-601 | **Revised by:** TSW | **Date:** 08/27/21

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	47.83	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.90	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.45	[ft]
Drainage Area (A)=	0.87	[acres]	Elevation $_{Difference}$:	0.45	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00941	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)(2):
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.040383	10.0	3.66
50	-0.00625	0.04	0.040383	10.0	5.1
100	-0.00625	0.04	0.040383	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}(1) = 0.90
C_{50}(1) = 0.90
C_{100}(1) = 0.90

Q_{10} = 2.86 **[cfs]**
Q_{50} = 4.01 **[cfs]**
Q_{100} = 4.52 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-F

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	63.13	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.90	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.45	[ft]
Drainage Area (A)=	0.48	[acres]	Elevation $_{Difference}$:	0.45	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00713	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group: --
Vegetation Cover: <25 [%]
Classification Type: A (Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.041989	10.0	3.66
50	-0.00625	0.04	0.041989	10.0	5.1
100	-0.00625	0.04	0.041989	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}[1] = 0.90
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 1.58 **[cfs]**
Q_{50} = 2.22 **[cfs]**
Q_{100} = 2.50 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-H

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	58.46	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1634.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.10	[ft]
Drainage Area (A)=	0.42	[acres]	Elevation $_{Difference}$:	0.90	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01540	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group: --
Vegetation Cover: <25 [%]
Classification Type: A (Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency				T_c	i
[yr]	m	b	K_b	[min]	[in/hr]
10	-0.00625	0.04	0.042375	10.0	3.66
50	-0.00625	0.04	0.042375	10.0	5.1
100	-0.00625	0.04	0.042375	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

$C_{10}^{(1)}$ = 0.90
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 1.37 **[cfs]**
Q_{50} = 1.92 **[cfs]**
Q_{100} = 2.17 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-I

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	82.76	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1634.10	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.29	[ft]
Drainage Area (A)=	0.35	[acres]	Elevation $_{Difference}$:	1.81	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02187	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[1]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042861	10.0	3.66
50	-0.00625	0.04	0.042861	10.0	5.1
100	-0.00625	0.04	0.042861	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}[1] = 0.90
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 1.15 **[cfs]**
Q_{50} = 1.61 **[cfs]**
Q_{100} = 1.81 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-J

Project Name: The Grayhawk Residences at Cavasson   **Prepared by:** TSW   **Date:** 06/03/21
Project No.: 18114-601   **Revised by:** TSW   **Date:** 08/27/21

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	71.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.30	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1630.65	[ft]
Drainage Area (A)=	0.42	[acres]	Elevation $_{Difference}$:	2.65	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.03732	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.04237	10.0	3.66
50	-0.00625	0.04	0.04237	10.0	5.1
100	-0.00625	0.04	0.04237	10.0	5.78

Peak Discharge (Q):
Q = C*i*A   [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}{}^{(1)}$ = 0.90
$C_{50}{}^{(1)}$ = 0.90
$C_{100}{}^{(1)}$ = 0.90

Q_{10} = 1.38 **[cfs]**
Q_{50} = 1.93 **[cfs]**
Q_{100} = 2.17 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-K

Project Name:	The Grayhawk Residences at Cavasson	**Prepared by:**	TSW	**Date:**	06/03/21
Project No.:	18114-601	**Revised by:**	TSW	**Date:**	08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	40.79	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1632.50	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.00	[ft]
Drainage Area (A)=	0.57	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01226	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.041532	10.0	3.66
50	-0.00625	0.04	0.041532	10.0	5.1
100	-0.00625	0.04	0.041532	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

$C_{10}^{(1)}$ = 0.90
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 1.87 **[cfs]**
Q_{50} = 2.63 **[cfs]**
Q_{100} = 2.96 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-L

Project Name: The Grayhawk Residences at Cavasson | **Prepared by:** TSW | **Date:** 06/03/21
Project No.: 18114-601 | **Revised by:** TSW | **Date:** 08/27/21

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	21.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1632.25	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1631.92	[ft]
Drainage Area (A)=	1.38	[acres]	Elevation $_{Difference}$:	0.33	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01571	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.039134	10.0	3.66
50	-0.00625	0.04	0.039134	10.0	5.1
100	-0.00625	0.04	0.039134	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}{}^{(1)}$ = 0.90
$C_{50}{}^{(1)}$ = 0.90
$C_{100}{}^{(1)}$ = 0.90

Q_{10} = 4.53 **[cfs]**
Q_{50} = 6.35 **[cfs]**
Q_{100} = 7.16 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-M

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	61.36	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1634.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.70	[ft]
Drainage Area (A)=	0.44	[acres]	Elevation $_{Difference}$:	0.30	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00489	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042252	10.0	3.66
50	-0.00625	0.04	0.042252	10.0	5.1
100	-0.00625	0.04	0.042252	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}[1] = 0.90
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 1.44 **[cfs]**
Q_{50} = 2.01 **[cfs]**
Q_{100} = 2.27 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-N

Project Name:	The Grayhawk Residences at Cavasson	**Prepared by:**	TSW	**Date:**	06/03/21
Project No.:	18114-601	**Revised by:**	TSW	**Date:**	08/27/21

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	63.44	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1636.50	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.65	[ft]
Drainage Area (A)=	0.19	[acres]	Elevation $_{Difference}$:	2.85	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.04492	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044475	10.0	3.66
50	-0.00625	0.04	0.044475	10.0	5.1
100	-0.00625	0.04	0.044475	10.0	5.78

Peak Discharge (Q):
Q = C*i*A   [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)} =$ 0.90
$C_{50}^{(1)} =$ 0.90
$C_{100}^{(1)} =$ 0.90

Q_{10} = 0.63 **[cfs]**
Q_{50} = 0.89 **[cfs]**
Q_{100} = 1.00 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-O

Project Name: The Grayhawk Residences at Cavasson | **Prepared by:** TSW | **Date:** 06/03/21
Project No.: 18114-601 | **Revised by:** TSW | **Date:** 08/27/21

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	126.11	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1636.50	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.45	[ft]
Drainage Area (A)=	2.11	[acres]	Elevation $_{Difference}$:	3.05	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02419	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.037979	10.0	3.66
50	-0.00625	0.04	0.037979	10.0	5.1
100	-0.00625	0.04	0.037979	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient
	$C_{10}^{(1)}$ = 0.90
	$C_{50}^{(1)}$ = 0.90
	$C_{100}^{(1)}$ = 0.90

Q_{10} = 6.94 **[cfs]**
Q_{50} = 9.72 **[cfs]**
Q_{100} = 10.95 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-P

Project Name: The Grayhawk Residences at Cavasson | **Prepared by:** TSW | **Date:** 06/03/21
Project No.: 18114-601 | **Revised by:** TSW | **Date:** 08/27/21

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	107.14	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1636.50	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.90	[ft]
Drainage Area (A)=	1.23	[acres]	Elevation $_{Difference}$:	2.60	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.02427	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[1]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.039448	10.0	3.66
50	-0.00625	0.04	0.039448	10.0	5.1
100	-0.00625	0.04	0.039448	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}[1] = 0.90
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 4.04 **[cfs]**
Q_{50} = 5.66 **[cfs]**
Q_{100} = 6.37 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-Q

Project Name: The Grayhawk Residences at Cavasson | **Prepared by:** TSW | **Date:** 06/03/21
Project No.: 18114-601 | **Revised by:** TSW | **Date:** 08/27/21

Location Data

State: Arizona | County: Maricopa | Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	147.10	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1634.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.50	[ft]
Drainage Area (A)=	0.32	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00340	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group: --
Vegetation Cover: <25 [%]
Classification Type: A (Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043059	10.0	3.66
50	-0.00625	0.04	0.043059	10.0	5.1
100	-0.00625	0.04	0.043059	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient
$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 1.01 **[cfs]**
Q_{50} = 1.50 **[cfs]**
Q_{100} = 1.69 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-R

Project Name:	The Grayhawk Residences at Cavasson	**Prepared by:**	TSW	**Date:**	06/03/21
Project No.:	18114-601	**Revised by:**	TSW	**Date:**	08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	166.88	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1634.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1633.50	[ft]
Drainage Area (A)=	0.57	[acres]	Elevation $_{Difference}$:	0.50	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00300	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.041534	10.0	3.66
50	-0.00625	0.04	0.041534	10.0	5.1
100	-0.00625	0.04	0.041534	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient
C_{10}[1] =	0.85
C_{50}[1] =	0.90
C_{100}[1] =	0.90

Q_{10} = 1.77 **[cfs]**
Q_{50} = 2.62 **[cfs]**
Q_{100} = 2.96 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-S

Project Name: The Grayhawk Residences at Cavasson　**Prepared by:** TSW　**Date:** 06/03/21
Project No.: 18114-601　**Revised by:** TSW　**Date:** 08/27/21

Location Data

State: Arizona　County: Maricopa　Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	106.08	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1634.00	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.63	[ft]
Drainage Area (A)=	0.43	[acres]	Elevation $_{Difference}$:	1.37	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01291	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042319	10.0	3.66
50	-0.00625	0.04	0.042319	10.0	5.1
100	-0.00625	0.04	0.042319	10.0	5.78

Peak Discharge (Q):
Q = C*i*A　[cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.85
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 1.32 **[cfs]**
Q_{50} = 1.96 **[cfs]**
Q_{100} = 2.21 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-T

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	57.29	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1632.25	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1631.55	[ft]
Drainage Area (A)=	0.30	[acres]	Elevation $_{Difference}$:	0.70	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01222	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group: --
Vegetation Cover: <25 [%]
Classification Type: A (Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043268	10.0	3.66
50	-0.00625	0.04	0.043268	10.0	5.1
100	-0.00625	0.04	0.043268	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}[1] = 0.85
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 0.93 **[cfs]**
Q_{50} = 1.38 **[cfs]**
Q_{100} = 1.56 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-U

Project Name:	The Grayhawk Residences at Cavasson	**Prepared by:**	TSW	**Date:**	06/03/21
Project No.:	18114-601	**Revised by:**	TSW	**Date:**	08/27/21

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	32.76	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1632.25	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1631.83	[ft]
Drainage Area (A)=	0.25	[acres]	Elevation $_{Difference}$:	0.42	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01282	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043789	10.0	3.66
50	-0.00625	0.04	0.043789	10.0	5.1
100	-0.00625	0.04	0.043789	10.0	5.78

Peak Discharge (Q):
Q = C*i*A   [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.85
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 0.77 **[cfs]**
Q_{50} = 1.14 **[cfs]**
Q_{100} = 1.29 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-V

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	43.51	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1632.25	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1631.70	[ft]
Drainage Area (A)=	0.11	[acres]	Elevation $_{Difference}$:	0.55	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01264	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.045896	10.0	3.66
50	-0.00625	0.04	0.045896	10.0	5.1
100	-0.00625	0.04	0.045896	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}[1] = 0.85
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 0.35 **[cfs]**
Q_{50} = 0.53 **[cfs]**
Q_{100} = 0.59 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-W

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	68.19	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1632.25	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1631.80	[ft]
Drainage Area (A)=	0.59	[acres]	Elevation $_{Difference}$:	0.45	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.00660	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[1]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.041445	10.0	3.66
50	-0.00625	0.04	0.041445	10.0	5.1
100	-0.00625	0.04	0.041445	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 1.83 **[cfs]**
Q_{50} = 2.71 **[cfs]**
Q_{100} = 3.05 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-X

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	39.35	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1632.16	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1631.50	[ft]
Drainage Area (A)=	0.67	[acres]	Elevation $_{Difference}$:	0.66	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01677	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group: --
Vegetation Cover: <25 [%]
Classification Type: A (Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.041092	10.0	3.66
50	-0.00625	0.04	0.041092	10.0	5.1
100	-0.00625	0.04	0.041092	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 2.08 **[cfs]**
Q_{50} = 3.09 **[cfs]**
Q_{100} = 3.48 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-Y

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	120.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.30	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1631.90	[ft]
Drainage Area (A)=	0.25	[acres]	Elevation $_{Difference}$:	1.40	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.01167	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043751	10.0	3.66
50	-0.00625	0.04	0.043751	10.0	5.1
100	-0.00625	0.04	0.043751	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient
C_{10}[1] =	0.85
C_{50}[1] =	0.90
C_{100}[1] =	0.90

Q_{10} = 0.78 **[cfs]**
Q_{50} = 1.16 **[cfs]**
Q_{100} = 1.31 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-Z

Project Name:	The Grayhawk Residences at Cavasson	**Prepared by:**	TSW	**Date:**	06/03/21
Project No.:	18114-601	**Revised by:**	TSW	**Date:**	08/27/21

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	30.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.30	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.00	[ft]
Drainage Area (A)=	0.29	[acres]	Elevation $_{Difference}$:	1.30	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.04333	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043383	10.0	3.66
50	-0.00625	0.04	0.043383	10.0	5.1
100	-0.00625	0.04	0.043383	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

C_{10}[1] = 0.85
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 0.89 **[cfs]**
Q_{50} = 1.33 **[cfs]**
Q_{100} = 1.50 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-AA

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	30.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.30	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.00	[ft]
Drainage Area (A)=	0.21	[acres]	Elevation $_{Difference}$:	1.30	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.04333	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.044217	10.0	3.66
50	-0.00625	0.04	0.044217	10.0	5.1
100	-0.00625	0.04	0.044217	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 0.66 **[cfs]**
Q_{50} = 0.98 **[cfs]**
Q_{100} = 1.10 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-AB

Project Name:	The Grayhawk Residences at Cavasson	**Prepared by:**	TSW	**Date:**	06/03/21
Project No.:	18114-601	**Revised by:**	TSW	**Date:**	08/27/21

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	30.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.30	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.00	[ft]
Drainage Area (A)=	0.28	[acres]	Elevation $_{Difference}$:	1.30	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.04333	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.043462	10.0	3.66
50	-0.00625	0.04	0.043462	10.0	5.1
100	-0.00625	0.04	0.043462	10.0	5.78

Peak Discharge (Q):
Q = C*i*A   [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

C_{10}[1] = 0.85
C_{50}[1] = 0.90
C_{100}[1] = 0.90

Q_{10} = 0.87 **[cfs]**
Q_{50} = 1.29 **[cfs]**
Q_{100} = 1.45 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-AC

Project Name:	The Grayhawk Residences at Cavasson	**Prepared by:**	TSW	**Date:**	06/03/21
Project No.:	18114-601	**Revised by:**	TSW	**Date:**	08/27/21

Location Data

State: Arizona   County: Maricopa   Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	30.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.30	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.00	[ft]
Drainage Area (A)=	0.71	[acres]	Elevation $_{Difference}$:	1.30	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.04333	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.040914	10.0	3.66
50	-0.00625	0.04	0.040914	10.0	5.1
100	-0.00625	0.04	0.040914	10.0	5.78

Peak Discharge (Q):
Q = C*i*A   [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)} =$ 0.85
$C_{50}^{(1)} =$ 0.90
$C_{100}^{(1)} =$ 0.90

Q_{10} = 2.22 **[cfs]**
Q_{50} = 3.30 **[cfs]**
Q_{100} = 3.71 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-AD

Project Name:	The Grayhawk Residences at Cavasson	**Prepared by:**	TSW	**Date:**	06/03/21
Project No.:	18114-601	**Revised by:**	TSW	**Date:**	08/27/21

Location Data

State: Arizona　County: Maricopa　Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	30.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.30	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.00	[ft]
Drainage Area (A)=	0.41	[acres]	Elevation $_{Difference}$:	1.30	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.04333	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[(2)]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.042415	10.0	3.66
50	-0.00625	0.04	0.042415	10.0	5.1
100	-0.00625	0.04	0.042415	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)} =$ 0.85
$C_{50}^{(1)} =$ 0.90
$C_{100}^{(1)} =$ 0.90

Q_{10} = 1.28 **[cfs]**
Q_{50} = 1.90 **[cfs]**
Q_{100} = 2.14 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-AE

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	30.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.30	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.00	[ft]
Drainage Area (A)=	0.50	[acres]	Elevation $_{Difference}$:	1.30	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.04333	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)[2]:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b
S	[ft/mi]	m= -0.00625
i	[in/hr]	b= 0.04
		A = Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.041896	10.0	3.66
50	-0.00625	0.04	0.041896	10.0	5.1
100	-0.00625	0.04	0.041896	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A [acres]
i [in/hr]
C_2 Runoff Coefficient

$C_{10}^{(1)} =$ 0.85
$C_{50}^{(1)} =$ 0.90
$C_{100}^{(1)} =$ 0.90

Q_{10} = 1.55 **[cfs]**
Q_{50} = 2.30 **[cfs]**
Q_{100} = 2.59 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

HYDROLOGIC DESIGN DATA SHEET
RATIONAL METHOD
Hubbard Engineering
Project No. 18114-601

Concentration Point ID: DA-AF

Project Name: The Grayhawk Residences at Cavasson **Prepared by:** TSW **Date:** 06/03/21
Project No.: 18114-601 **Revised by:** TSW **Date:** 08/27/21

Location Data

State: Arizona County: Maricopa Jurisdiction: City of Scottsdale

Design Data

Design Frequency:	100	[yrs]	Flow Path Length (L)=	30.00	[ft]
Check Frequency:	50	[yrs]	Elevation $_{Max}$:	1633.30	[ft]
Check Frequency:	10	[yrs]	Elevation $_{Min}$:	1632.00	[ft]
Drainage Area (A)=	0.76	[acres]	Elevation $_{Difference}$:	1.30	[ft]
			Flow Path Slope$_{Average}$ (S)=	0.04333	[ft/ft]

Watershed Characteristics

Hydrologic Soil Group:	--	
Vegetation Cover:	<25	[%]
Classification Type:	A	(Reference, Table 3.1, Page 3-3)

Rational Method Computations

Time of Concentration, (Tc)$^{(2)}$:
Tc = 11.4*L0.5*Kb0.52*S-0.31*i-0.38

L	[mi]	Kb=m Log A + b	
S	[ft/mi]	m=	-0.00625
i	[in/hr]	b=	0.04
		A =	Area [acres]

Frequency [yr]	m	b	K_b	T_c [min]	i [in/hr]
10	-0.00625	0.04	0.040762	10.0	3.66
50	-0.00625	0.04	0.040762	10.0	5.1
100	-0.00625	0.04	0.040762	10.0	5.78

Peak Discharge (Q):
Q = C*i*A [cfs]

A	[acres]
i	[in/hr]
C_2	Runoff Coefficient

$C_{10}^{(1)}$ = 0.85
$C_{50}^{(1)}$ = 0.90
$C_{100}^{(1)}$ = 0.90

Q_{10} = 2.35 **[cfs]**
Q_{50} = 3.49 **[cfs]**
Q_{100} = 3.93 **[cfs]**

Reference: 1. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology

Appendix F
Hydraulic Calculations
Grayhawk Residences at Cavasson

HYDRAULIC CALCULATION SHEET
Retention Calculations
Hubbard Engineering
Project No. 18114-601

Project Name: The Grayhawk Residences at Cavasson — **Prepared by:** TSW — **Date:** 06/03/21
Project No.: 18114-601 — **Revised By:** — **Date:**

Purpose: Evaluate the required and provided retention volumes in order to assess conformance to project criteria.

Methodology: Calculate the volume of stormwater required to be retained using City of Scottsdale criteria. Calculate the estimated volume of stormwater retained using retention basin geometry.

Criteria: Retain the calculated stormwater run-off for the first flush condition and 100-YEAR 6-HOUR duration storm event.

References: 1. City of Scottsdale *Design Standards and Policies Manual*

Calculations: First Flush Volume Required = $C_{Composite}$ *D/12* A [ft^3] (Reference 1)

D = 0.5	[in]	(First Flush)	(Reference 1)
C = 0.90	(Paved Parking)		(Reference 1)
P = 2.75	[in]	(100 YR 6 HR)	

Composite C= (C1*A1 + C2*A2…+….)/(A1+A2+….)

Volume Required = Composite C*P/12*A

Results:

Identifiers	CALCULATE RETENTION VOLUME REQUIRED		Volume Required		
Contributory Area ID	Area [acres]	C	[acre-ft]	[ft^3]	[yd^3]
DA-A	1.20	0.90	0.04	1,958	73
DA-B	0.57	0.90	0.02	937	35
DA-C	0.20	0.90	0.01	330	12
DA-D	0.40	0.90	0.02	657	24
DA-E	0.84	0.90	0.03	1,365	51
DA-F	0.48	0.90	0.02	776	29
DA-G	0.27	0.90	0.01	442	16
DA-H	0.42	0.90	0.02	681	25
DA-I	0.34	0.90	0.01	563	21
DA-J	0.42	0.90	0.02	682	25
DA-K	0.61	0.90	0.02	995	37
DA-L	1.38	0.90	0.05	2,248	83
DA-M	0.45	0.90	0.02	736	27
DA-N	0.22	0.90	0.01	352	13
DA-O	2.14	0.90	0.08	3,497	130
DA-P	1.24	0.90	0.05	2,019	75
DA-Q	0.32	0.90	0.01	529	20
DA-R	0.57	0.90	0.02	928	34
DA-S	0.39	0.90	0.01	641	24
DA-T	0.31	0.90	0.01	503	19
DA-U	0.27	0.90	0.01	435	16
DA-V	0.13	0.90	0.00	206	8
DA-W	0.59	0.90	0.02	959	36
DA-X	0.67	0.90	0.03	1,093	40
DA-Y	0.25	0.90	0.01	410	15
DA-Z	0.29	0.90	0.01	470	17
DA-AA	0.21	0.90	0.01	345	13
DA-AB	0.28	0.90	0.01	456	17
DA-AC	0.71	0.90	0.03	1,166	43
DA-AD	0.41	0.90	0.02	671	25
DA-AE	0.50	0.90	0.02	812	30
DA-AF	0.76	0.90	0.03	1,233	46
Phase 3 Roads Adjacent	0.64	0.95	0.14	6,089	226
Legacy BLVD Adjacent	2.07	0.95	0.45	19,656	728
Miller Road Adjacent	0.52	0.95	0.11	4,929	183
Total Area:	**21.04**	**TOTAL VOLUME REQUIRED:**	**1.37**	**59,769.09**	**2,214.60**

Hubbard Engineering
Project No. 18114-601

Project Name: Grayhawk Residences at Cavasson
Project No.: 18114-601

Prepared by: TSW
Revised By:

Date: 06/03/21
Date:

VOLUME PROVIDED

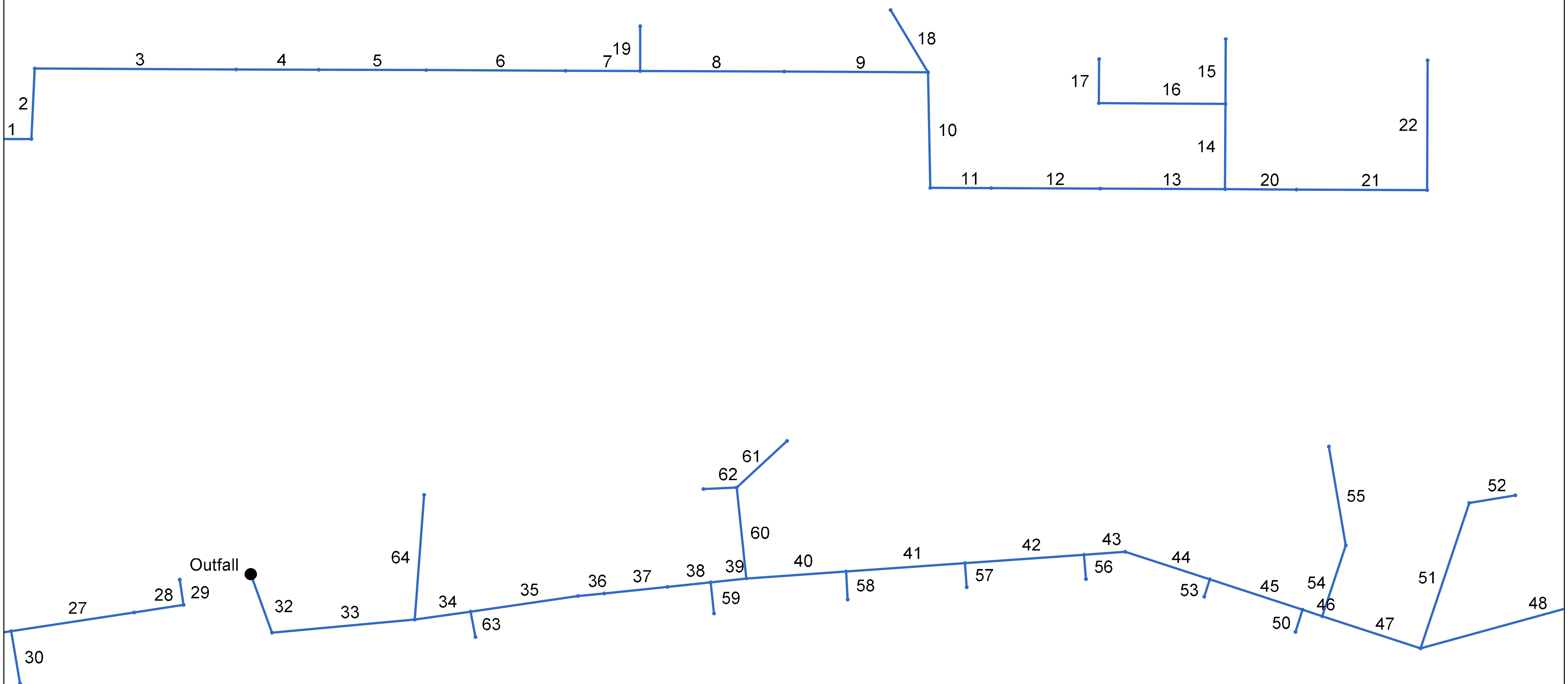
$$V_{U.P.} = \pi * r^2 * L$$

$V_{U.P.}$	*Volume of underground pipe provided in cubic feet*	
π	*pi*	
r	*Underground pipe radius, ft*	*5*
L	*Length of underground pipe in feet*	

UNDERGROUND RETENTION/DETENTION COMBINED

Retention Method	*Length*	*Volume Provided*
(ID)	*(ft)*	*(cf)*
UG-TANK 1	131	10,289
UG-TANK 2	131	10,289
UG-TANK 3	131	10,289
UG-TANK 4	131	10,289
UG-TANK 5	122	9,582
UG-TANK 6	122	9,582
Total	**770**	**60,319**

Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



Project File: GH Storm Drain Network.stm	Number of lines: 64	Date: 8/27/2021

Storm Sewers v2018.30

MyReport

Line No.	Inlet ID	Known Q (cfs)	Flow Rate (cfs)	HGL Dn (ft)	HGL Up (ft)	Gnd/Rim El Dn (ft)	Gnd/Rim El Up (ft)	EGL Dn (ft)	EGL Up (ft)	Vel Ave (ft/s)	Invert Dn (ft)	Invert Up (ft)	Line Size (in)
1	Structure - (2)	6.23	37.42	1630.00	1630.04	1636.00	1636.00	1630.24	1630.28	3.89	1626.00	1626.24	42
2	Structure - (3)	0.00	31.19	1630.28	1630.34	1636.00	1637.00	1630.44	1630.50	3.24	1626.24	1626.68	42
3	Structure - (4)	2.99	31.19	1630.50	1630.58	1637.00	1638.00	1630.66	1630.85	3.70	1626.78	1628.03	42
4	Structure - (5)	1.05	28.20	1630.62	1630.36	1638.00	1638.60	1631.33	1631.06	5.62	1628.13	1628.64	36
5	Structure - (6)	2.09	27.15	1630.36	1631.09	1638.60	1639.00	1631.04	1631.78	6.82	1628.74	1629.41	36
6	Structure - (7)	0.00	25.06	1631.09	1631.99	1639.00	1638.00	1631.74	1632.64	6.55	1629.51	1630.38	36
7	Structure - (8)	4.35	25.06	1631.99	1632.55	1638.00	1639.50	1632.64	1633.20	6.74	1630.48	1630.94	36
8	Structure - (9)	2.47	18.12	1632.55	1633.30 j	1639.50	1641.50	1633.08	1633.83	5.44	1631.04	1631.94	36
9	Structure - (10)	0.00	15.65	1633.30	1634.26	1641.50	1642.00	1633.84	1634.80	6.09	1632.04	1632.93	30
10	Structure - (12)	2.34	14.24	1634.30	1635.11	1642.00	1641.30	1634.91	1635.72	6.53	1633.03	1633.75	24
11	Structure - (13)	0.00	11.90	1635.11	1635.47 j	1641.30	1641.10	1635.64	1636.00	5.77	1633.85	1634.23	24
12	Structure - (14)	1.12	11.90	1635.47	1636.25	1641.10	1641.10	1636.00	1636.78	6.14	1634.33	1635.01	24
13	Structure - (71)	0.00	10.78	1636.25	1637.06	1641.10	1642.00	1636.74	1637.55	5.73	1635.11	1635.88	24
14	Structure - (22)	0.00	3.96	1637.06	1637.49 j	1642.00	1642.75	1637.41	1637.84	4.13	1635.98	1636.69	15
15	Structure - (23)	1.79	1.79	1637.49	1637.94 j	1642.75	1642.75	1637.73	1638.17	3.45	1636.79	1637.37	12
16	Structure - (24)	0.00	2.17	1637.49	1638.47 j	1642.75	164.00	1637.77	1638.74	3.92	1636.79	1637.84	12
17	Structure - (25)	2.17	2.17	1638.50	1638.94	164.00	1644.00	1638.77	1639.21	4.47	1637.94	1638.31	12
18	Structure - (11)	1.41	1.41	1634.26	1634.44 j	1642.00	1645.00	1634.46	1634.64	2.71	1633.33	1633.94	12
19	Structure - (26)	2.59	2.59	1632.55	1632.61	1639.50	1640.50	1632.62	1632.68	2.13	1631.04	1631.41	15
20	Structure - (18)	1.69	6.82	1637.06	1637.43	1642.00	1644.00	1637.51	1637.88	5.21	1635.98	1636.42	18
21	Structure - (19)	2.96	5.13	1637.46	1638.27	1644.00	1640.00	1637.88	1638.69	5.17	1636.52	1637.33	15
22	Structure - (20)	2.17	2.17	1638.69	1638.85	1640.00	1640.00	1638.74	1639.06	2.70	1637.43	1638.24	15
23	Structure - (62)	0.00	9.02	1623.66	1623.76	1629.00	1629.00	1624.09	1624.19	5.30	1622.60	1622.69	24

Project File: GH Storm Drain Network.stm | Number of lines: 64 | Date: 8/27/2021

NOTES: ** Critical depth

MyReport

Line No.	Inlet ID	Known Q (cfs)	Flow Rate (cfs)	HGL Dn (ft)	HGL Up (ft)	Gnd/Rim El Dn (ft)	Gnd/Rim El Up (ft)	EGL Dn (ft)	EGL Up (ft)	Vel Ave (ft/s)	Invert Dn (ft)	Invert Up (ft)	Line Size (in)
24	Structure - (63)	0.00	3.71	1623.76	1623.47	1629.00	1629.00	1624.01	1623.72	3.08	1622.69	1622.80	24
25	Structure - (64)	0.00	3.71	1623.90	1624.60	1629.00	1630.00	1624.25	1624.95	4.72	1622.90	1623.22	12
26	Structure - (65)	0.00	3.71	1624.70	1625.16	1630.00	1632.55	1625.04	1625.51	4.72	1623.32	1623.54	12
27	Structure - (66)	0.00	3.05	1625.51	1626.27	1632.55	1632.00	1625.75	1626.50	3.88	1623.54	1624.06	12
28	Structure - (67)	0.00	3.05	1626.30	1628.75 j	1632.00	1633.00	1626.54	1629.11	4.36	1624.16	1628.00	12
29	Structure - (68)	3.05	3.05	1628.75	1631.75	1633.00	1632.10	1629.11	1632.11	4.84	1628.00	1631.00	12
30	Structure - (69)	0.66	0.66	1625.51	1625.53	1632.55	1634.00	1625.52	1625.54	0.84	1623.64	1623.86	12
31	Structure - (70)	5.31	5.31	1623.76	1623.71	1629.00	1627.30	1624.07	1624.02	3.97	1622.79	1622.90	24
32	Structure - (28)	0.00	44.59	1630.00	1630.04	1635.00	1633.00	1630.20	1630.24	3.57	1626.00	1626.21	48
33	Structure - (29)	3.05	44.59	1630.24	1628.79	1633.00	1633.50	1631.02	1629.57	5.34	1626.31	1626.79	48
34	Structure - (59)	0.00	34.38	1628.79	1628.91	1633.50	1634.00	1629.44	1629.62	6.62	1626.89	1627.08	42
35	Structure - (30)	0.00	32.28	1629.62	1629.66	1634.00	1635.15	1629.91	1630.05	4.66	1627.08	1627.44	42
36	Structure - (31)	0.00	32.28	1629.72	1629.39	1635.15	1634.60	1630.42	1630.08	5.90	1627.54	1627.63	42
37	Structure - (32)	0.00	32.28	1629.50	1629.71 j	1634.60	1634.60	1630.18	1630.39	6.63	1627.73	1627.94	42
38	Structure - (33)	0.00	32.28	1629.82	1629.96	1634.60	1635.00	1630.49	1630.63	6.56	1628.04	1628.18	42
39	Structure - (34)	0.00	30.90	1630.63	1630.02	1635.00	1635.50	1631.30	1630.69	5.44	1628.18	1628.30	42
40	Structure - (35)	0.00	19.76	1630.02	1630.16 j	1635.50	1636.00	1630.42	1630.71	5.51	1628.40	1628.73	36
41	Structure - (36)	0.00	18.45	1630.71	1630.76	1636.00	1636.00	1630.93	1631.10	4.19	1628.73	1629.12	36
42	Structure - (37)	0.00	16.95	1631.10	1631.23	1636.00	1636.50	1631.36	1631.58	4.41	1629.12	1629.52	30
43	Structure - (38)	0.00	15.85	1631.58	1631.61	1636.50	1637.00	1631.79	1631.84	3.77	1629.52	1629.66	30
44	Structure - (39)	0.00	15.85	1631.71	1631.78	1637.00	1637.00	1631.94	1632.08	4.14	1629.76	1630.06	30
45	Structure - (40)	0.00	14.40	1632.08	1632.38	1637.00	1637.00	1632.41	1632.70	4.58	1630.06	1630.38	24
46	Structure - (41)	0.00	13.90	1632.70	1632.77	1637.00	1637.00	1633.01	1633.07	4.42	1630.38	1630.45	24

Project File: GH Storm Drain Network.stm | Number of lines: 64 | Date: 8/27/2021

NOTES: ** Critical depth

MyReport

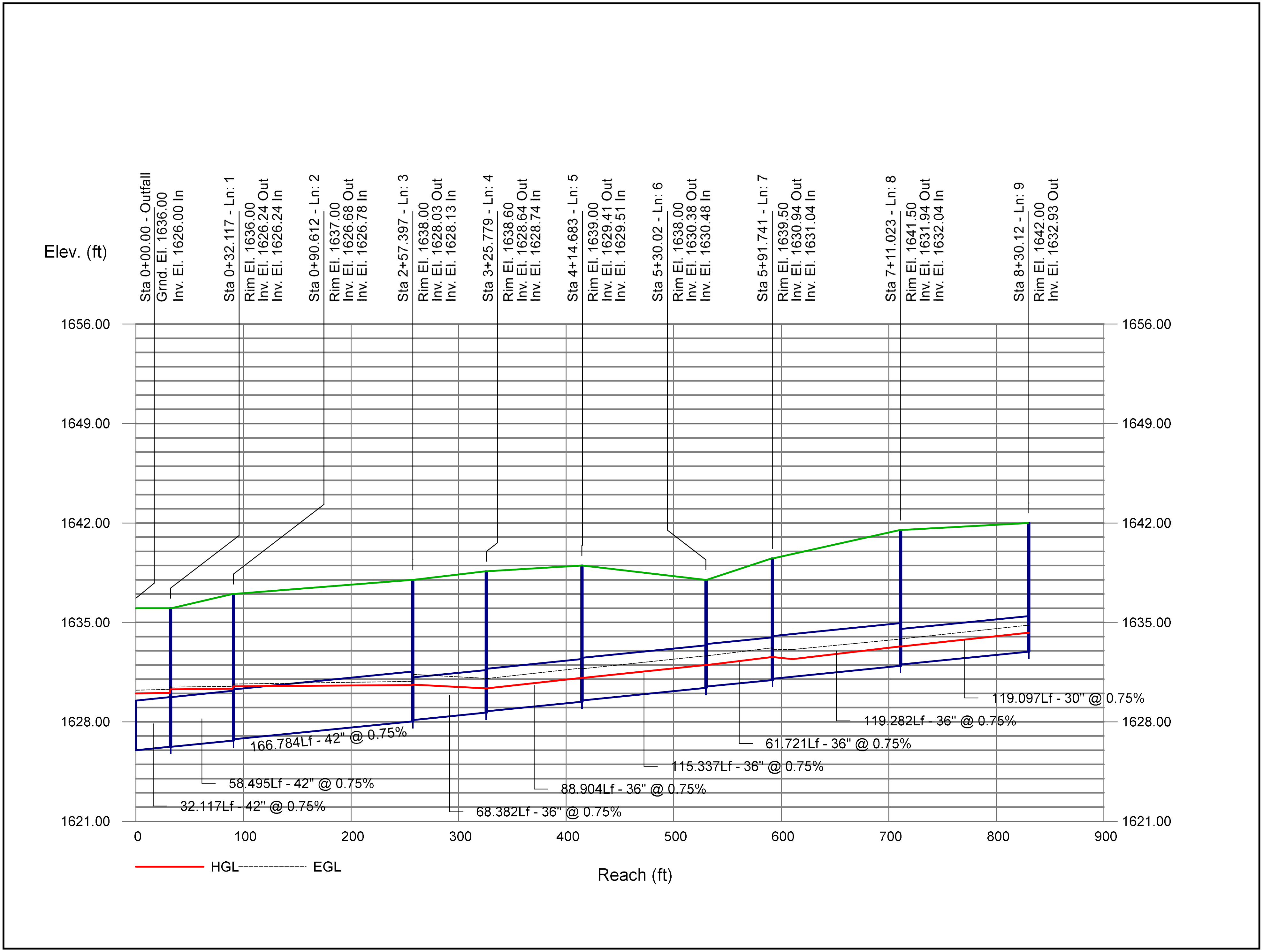
Line No.	Inlet ID	Known Q (cfs)	Flow Rate (cfs)	HGL Dn (ft)	HGL Up (ft)	Gnd/Rim El Dn (ft)	Gnd/Rim El Up (ft)	EGL Dn (ft)	EGL Up (ft)	Vel Ave (ft/s)	Invert Dn (ft)	Invert Up (ft)	Line Size (in)
47	Structure - (42)	0.00	4.26	1633.07	1633.45	1637.00	1640.00	1633.26	1633.63	3.47	1630.55	1631.65	15
48	Structure - (43)	0.00	2.12	1633.63	1633.82	1640.00	1637.00	1633.66	1633.94	2.00	1631.75	1633.15	18
49	Structure - (44)	2.12	2.12	1633.84	1634.10 j	1637.00	1637.60	1634.04	1634.30	3.47	1633.25	1633.55	18
50	Structure - (49)	0.50	0.50	1632.70	1632.71	1637.00	1640.00	1632.71	1632.71	0.64	1630.38	1630.46	12
51	Structure - (45)	0.00	2.14	1633.63	1634.09	1640.00	1635.00	1633.75	1634.21	2.72	1631.75	1632.26	12
52	Structure - (46)	2.14	2.14	1636.17	1636.62	1635.00	1636.00	1636.44	1636.88	4.43	1635.61	1635.99	12
53	Structure - (50)	1.45	1.45	1632.08	1632.11	1637.00	0.00	1632.13	1632.16	1.85	1630.06	1630.12	12
54	Structure - (47)	3.21	9.64	1633.07	1633.19	1637.00	0.00	1633.22	1633.33	3.07	1630.45	1630.70	24
55	Structure - (48)	6.43	6.43	1633.26	1633.58	0.00	1637.00	1633.47	1633.78	3.64	1630.70	1631.53	18
56	Structure - (51)	1.10	1.10	1631.58	1631.60	1636.50	1637.00	1631.61	1631.63	1.40	1629.52	1629.60	12
57	Structure - (52)	1.50	1.50	1631.10	1631.14	1636.00	1637.00	1631.16	1631.19	1.91	1629.12	1629.20	12
58	Structure - (53)	1.31	1.31	1630.71	1630.74	1636.00	1637.00	1630.75	1630.78	1.67	1628.73	1628.82	12
59	Structure - (57)	1.38	1.38	1630.63	1630.67	1635.00	1635.00	1630.68	1630.72	1.76	1628.18	1628.44	12
60	Structure - (54)	0.00	11.14	1630.02	1630.26 j	1635.50	1635.00	1630.52	1630.76	4.78	1628.30	1629.06	24
61	Structure - (55)	8.14	8.14	1630.26	1630.83	1635.00	1636.00	1630.79	1631.36	5.86	1629.16	1629.73	18
62	Structure - (56)	3.00	3.00	1630.26	1630.44	1635.00	1638.00	1630.48	1630.67	3.82	1629.16	1629.44	12
63	Structure - (60)	2.10	2.10	1629.62	1629.69	1634.00	1638.00	1629.73	1629.81	2.67	1627.08	1627.29	12
64	Structure - (58)	7.16	7.16	1628.79	1629.20	1633.50	1638.00	1629.04	1629.51	4.25	1626.89	1627.92	18

Project File: GH Storm Drain Network.stm	Number of lines: 64	Date: 8/27/2021

NOTES: ** Critical depth

Storm Sewer Profile

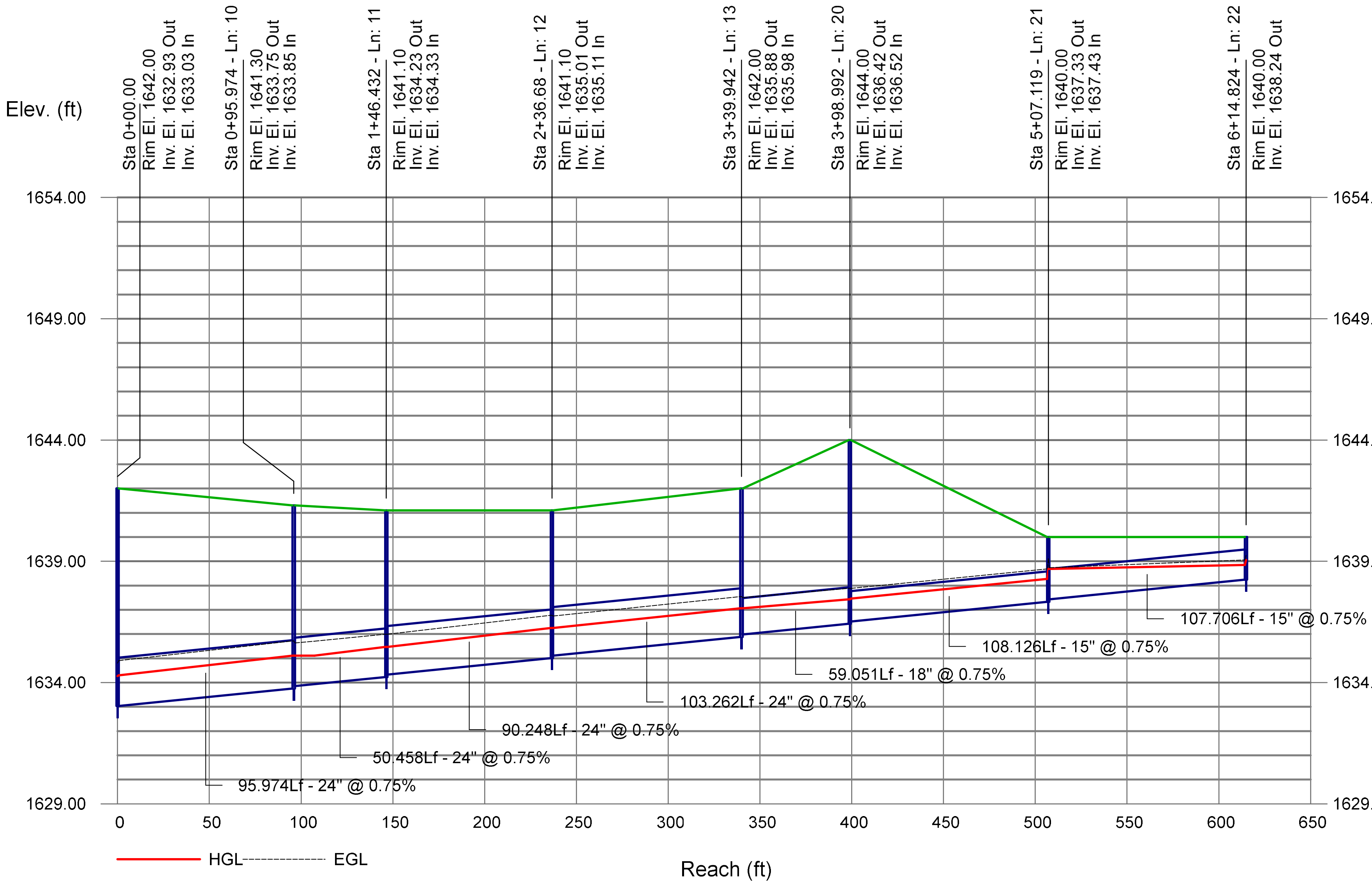
Proj. file: GH Storm Drain Network.stm



Storm Sewers

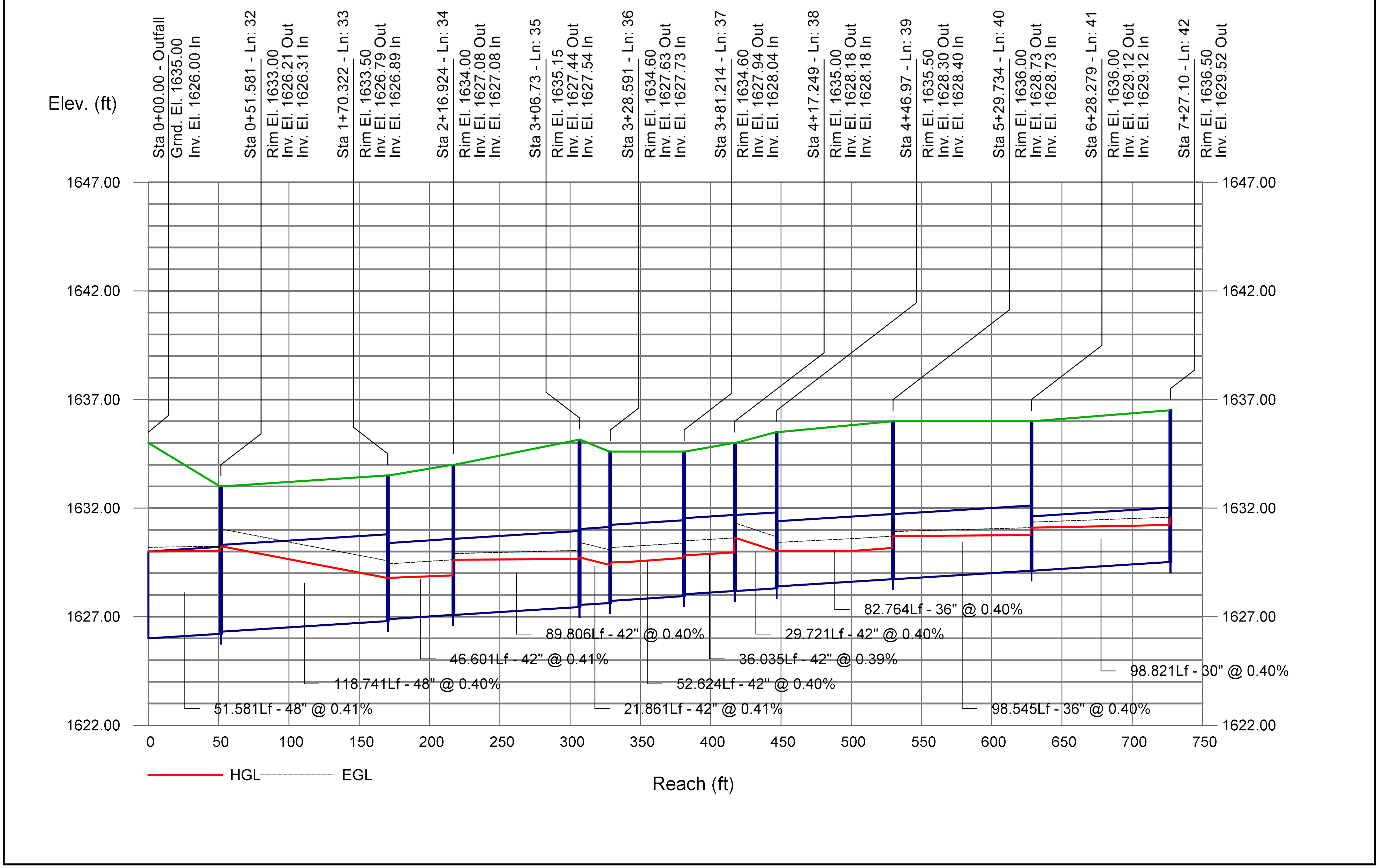
Storm Sewer Profile

Proj. file: GH Storm Drain Network.stm



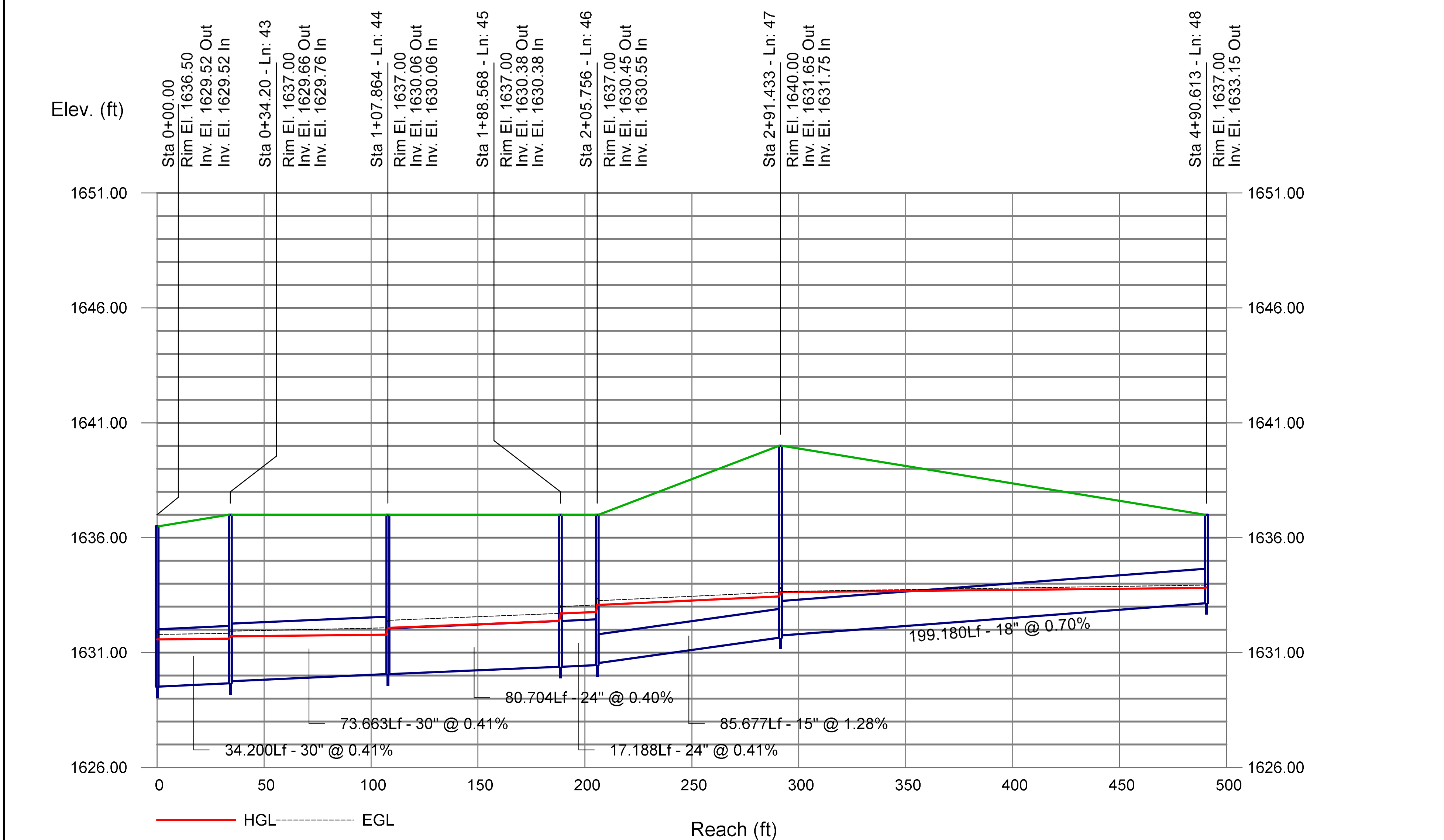
Storm Sewer Profile

Proj. file: GH Storm Drain Network.stm



Storm Sewer Profile

Proj. file: GH Storm Drain Network.stm



HYDRAULIC CALCULATION SHEET
DA-A Drain Time Calculations
Hubbard Engineering
Project No. 18114-601

Project Name: The Grayhawk Residences at Cavasson
Project No.: 18114-601

Prepared by: TSW
Revised by: TSW

Date: 06/03/21
Date: 08/27/21

Purpose: Calculate the number of drywells required to facilitate drainage of the required volume within 36 hours.

Methodology: Calculate the number of drywells necessary to drain the retention basin and undeground storage tanks within 36 hours

Criteria:

1. Drywell drainage capacity must discharge the retention volume provided within 36 hours.
2. Percolation rates have been assumed to be 0.1 cfs and will be field verified during construction

References: 1. Design Standards & Policies Manual, City of Scottsdale, January 2018

Calculations:

Number of Drywells Required = V_{DW} / (Drywell Infiltration Rate * 3600 * 36 hours)

Drywell Infiltration Rate = 0.1 [cfs] (Reference 1)

Results:

Identifiers	Infiltration Drainage Capacity Calculations					V_{DW}	Number of
Retention Basin ID	Volume(Ref. 1) Provided [ft^3]	Bottom(Ref 1) Area [ft^2]	Percolation Rate(Ref. 2) Test Location*	Percolation Rate(Ref. 2) [$ft^3/hr/ft^2$]^2	Infiltration Drain Capacity [ft^3]	[ft^3]	Drywells Required
GH Site	60,319.0	0.0	--	--	0.0	60,319.0	5
					Total Number of Drywells =		**5**

Conclusion: The number of dual chamber drywells computed is sufficient to discharge the provided colume within 36 hours.

Total Vol. =	60,319.0	[ft^3]
D.W. Rate =	0.1	cfs
	603190	seconds
	167.6	*Hours (with no surface infiltration)*
No of DW's	5	
Total System Drain Time=	33.5	Hours

Appendix G
404 Permit
Grayhawk Residences at Cavasson

March 27, 2019

U.S. Army Corps of Engineers
Los Angeles District – Regulatory Division
915 Wilshire Blvd.
Los Angeles, CA 90017

Subject: Statement of Sale of 0.89 Credits for the Cavasson Mixed-Use Development project, Corps permit number SPL-2018-00704, from the Arizona Game and Fish Department In-Lieu Fee Program to Nationwide Reality Investors Ltd.

The Arizona Game and Fish Department has an agreement with the U.S. Army Corps of Engineers – Los Angeles District to operate an In-Lieu-Fee Program. This letter confirms the sale of 0.89 credits of Advance Credits for establishment/enhancement of wetlands and Mesquite Bosque within the Arlington Wildlife Area; an Arizona Game and Fish Commission owned property. These credits are being used as compensatory mitigation for 0.89 acres of impact calculated at a 1:1 ratio for impacts to Waters of The U.S. associated with the Cavasson Mixed-Use Development Project. By selling credits to the above permittee, AGFD is the party responsible for fulfilling the mitigation aspect of Special Condition of the Permit listed above.

Signed,

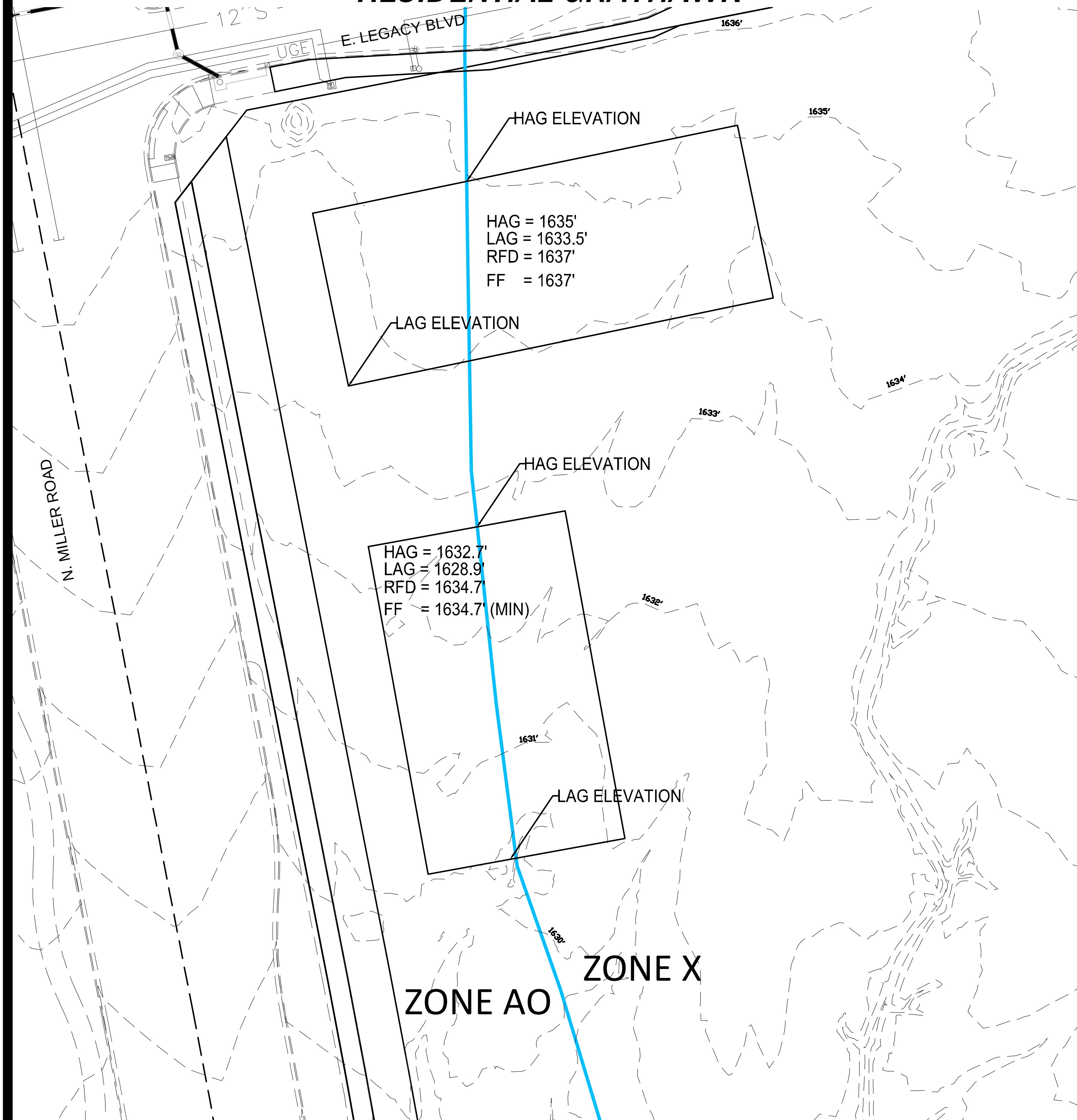
Shawn F. Lowery
Statewide ILF Restoration Program Manager
Wildlife Contracts Branch
Arizona Game and Fish Department

Appendix H

Original HAG/LAG Documentation

Grayhawk Residences at Cavasson

FLOOD PLAIN CLARIFICATION
RESIDENTIAL GRAYHAWK



Proj. No.	Date
18114-601	04-29-2021
Sht: 1 of 1	
Proj. Mangr.	Proj. Eng.
GMB	MSW

HUBBARD
ENGINEERING
www.hubbardengineering.com
1201 S. Alma School Rd.,Ste. 12000 - Mesa, AZ 85210
Ph: 480.892.3313

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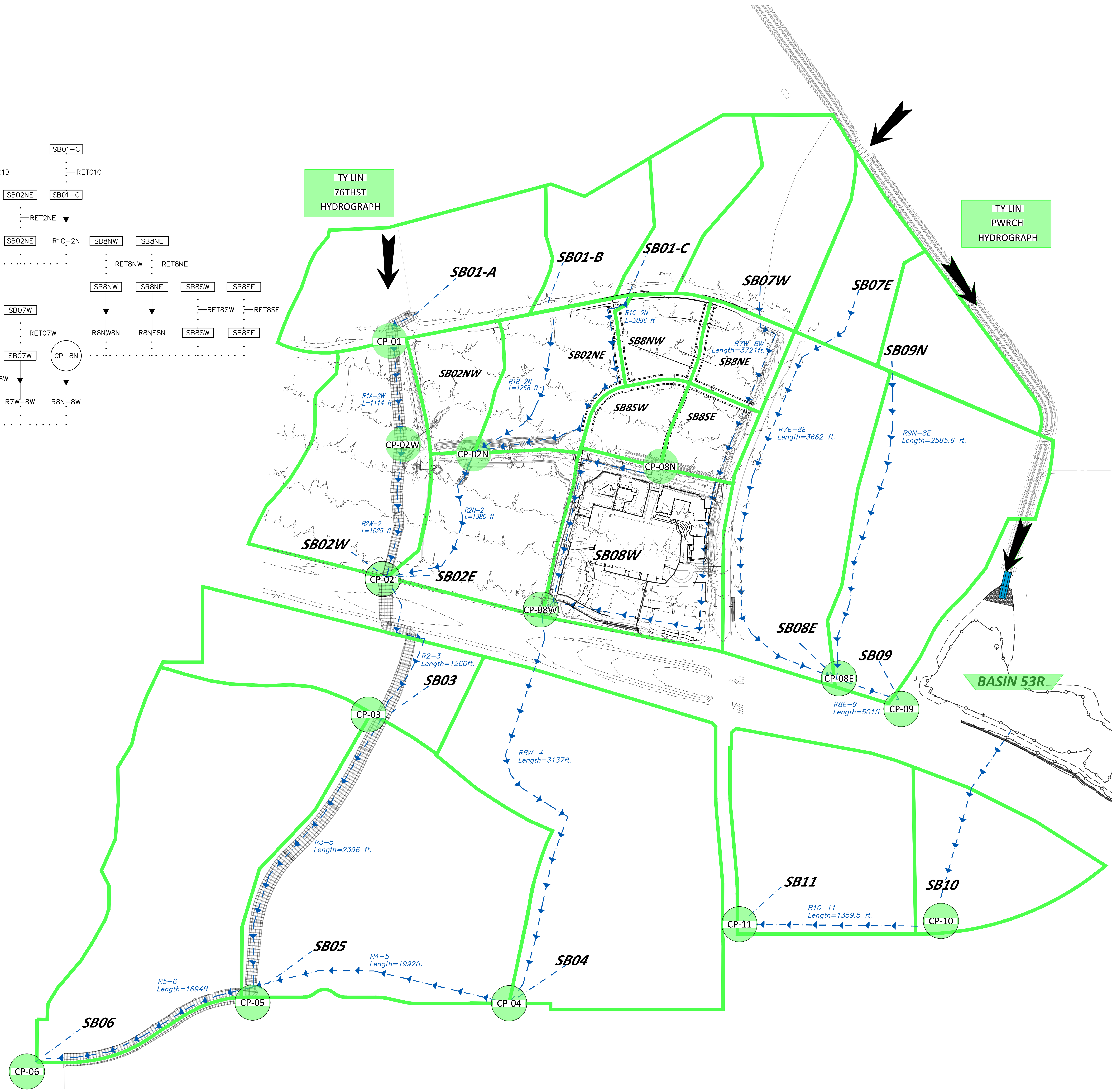
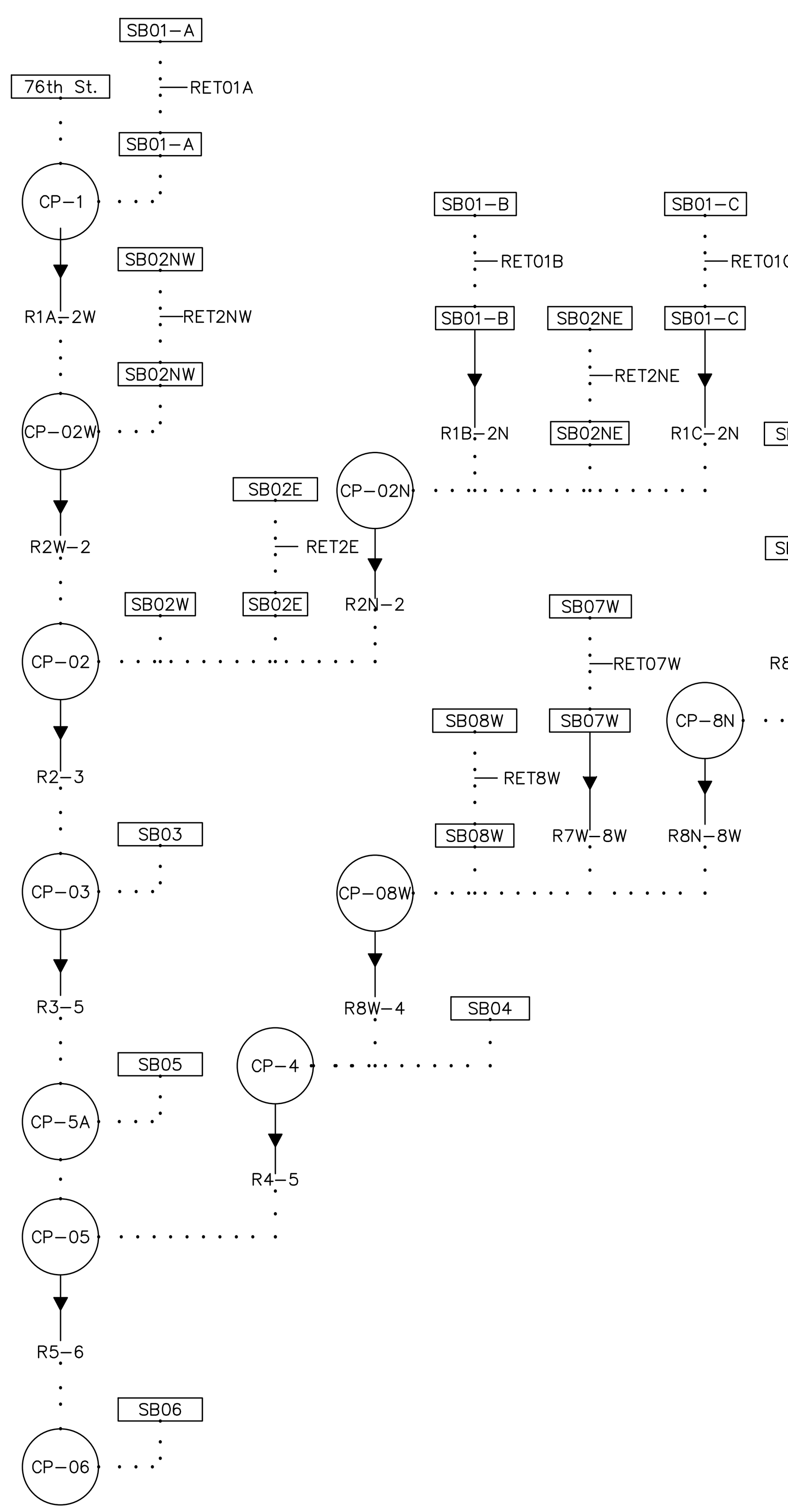
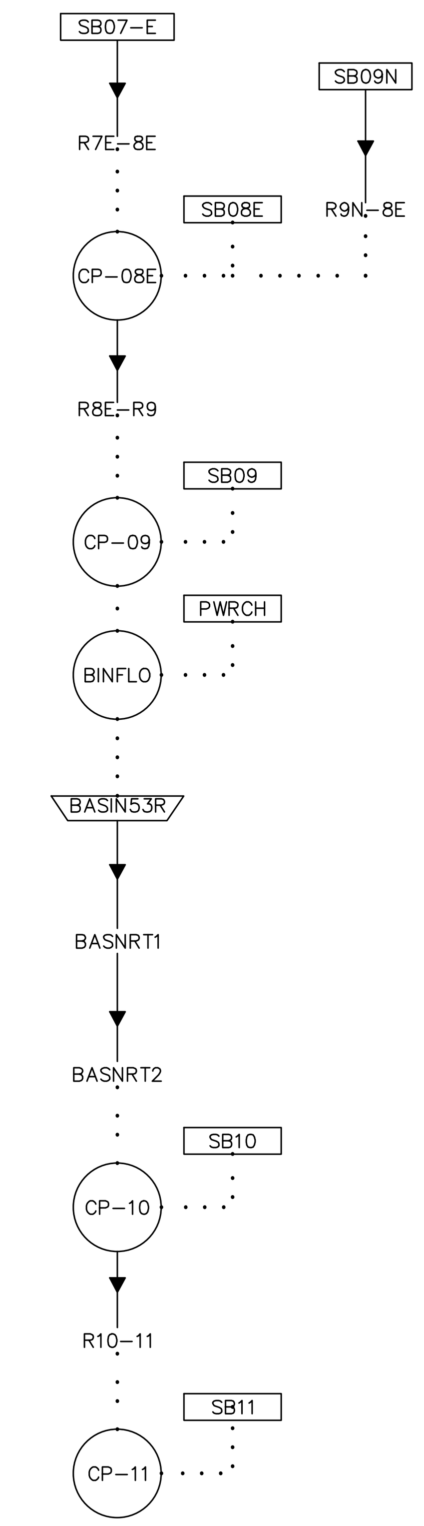
Exhibits

Grayhawk Residences at Cavasson

HEC -1 ULTIMATE CONDITION







400 0 400 800
scale feet

N

HEC-1 ULTIMATE CONDITIONS		
ASLD CORE NORTH		
Project No. 18114-101	Date rev. 07/23/20	EX-04

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DRAINAGE MAP

GRAYHAWK RESIDENCES AT CAVASSON

A PORTION OF THE NORTHWEST QUARTER OF SECTION 26
TOWNSHIP 4 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER
MERIDIAN, MARICOPA COUNTY, ARIZONA

Project No. 18114-601	Date 08/28/2021
Project Mgr. G. BROWN	Project Eng. M. WOLF

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EX 02

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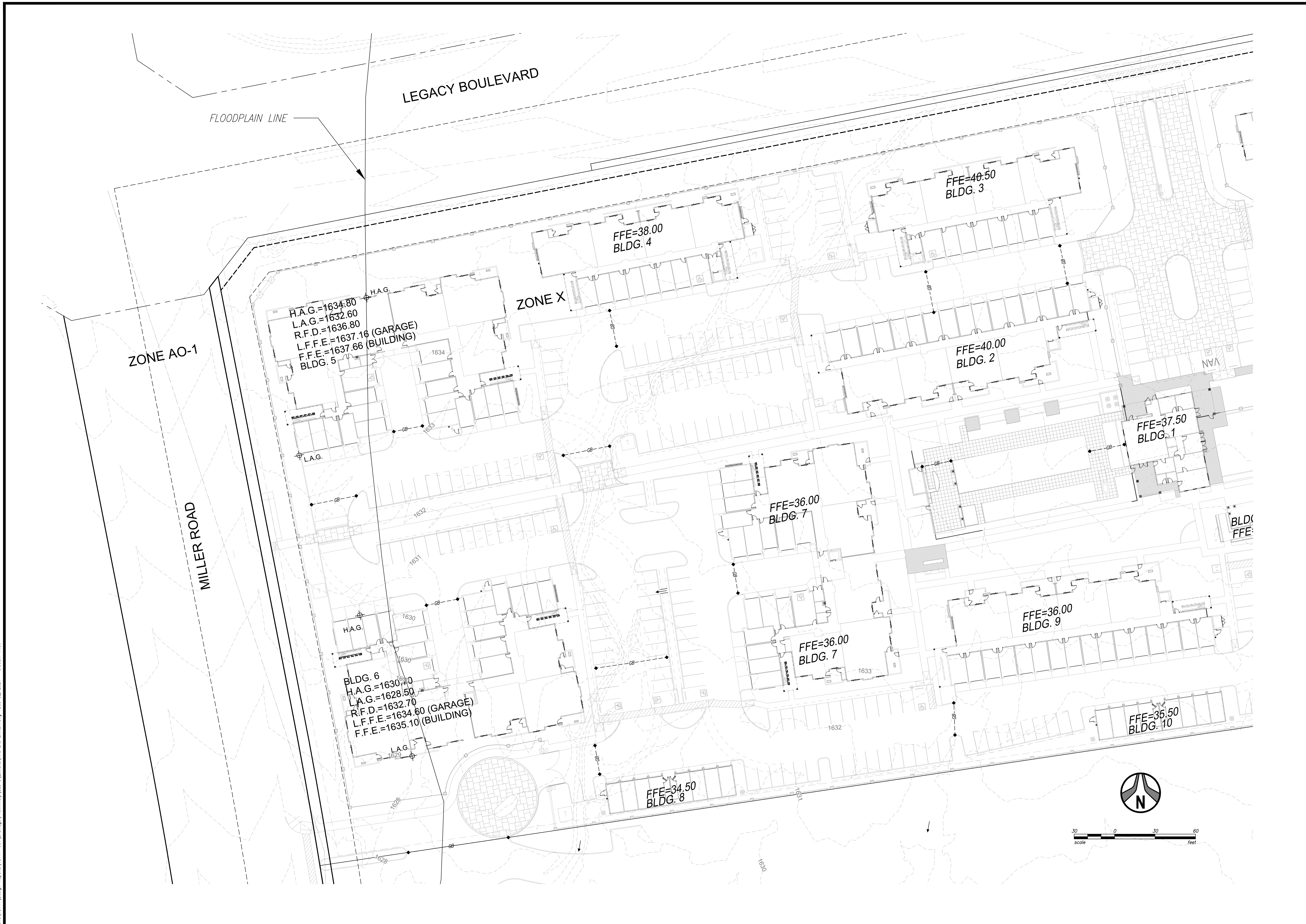

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HAG/LAG EXHIBIT

GRAYHAWK RESIDENCES AT CAVASSON

A PORTION OF THE NORTHWEST QUARTER OF SECTION 26
TOWNSHIP 4 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER
MERIDIAN, MARICOPA COUNTY, ARIZONA

Date 12/2/2021	Project Eng. M. WOLF
Project No. 18114-601	Project Mgr. G. BROWN

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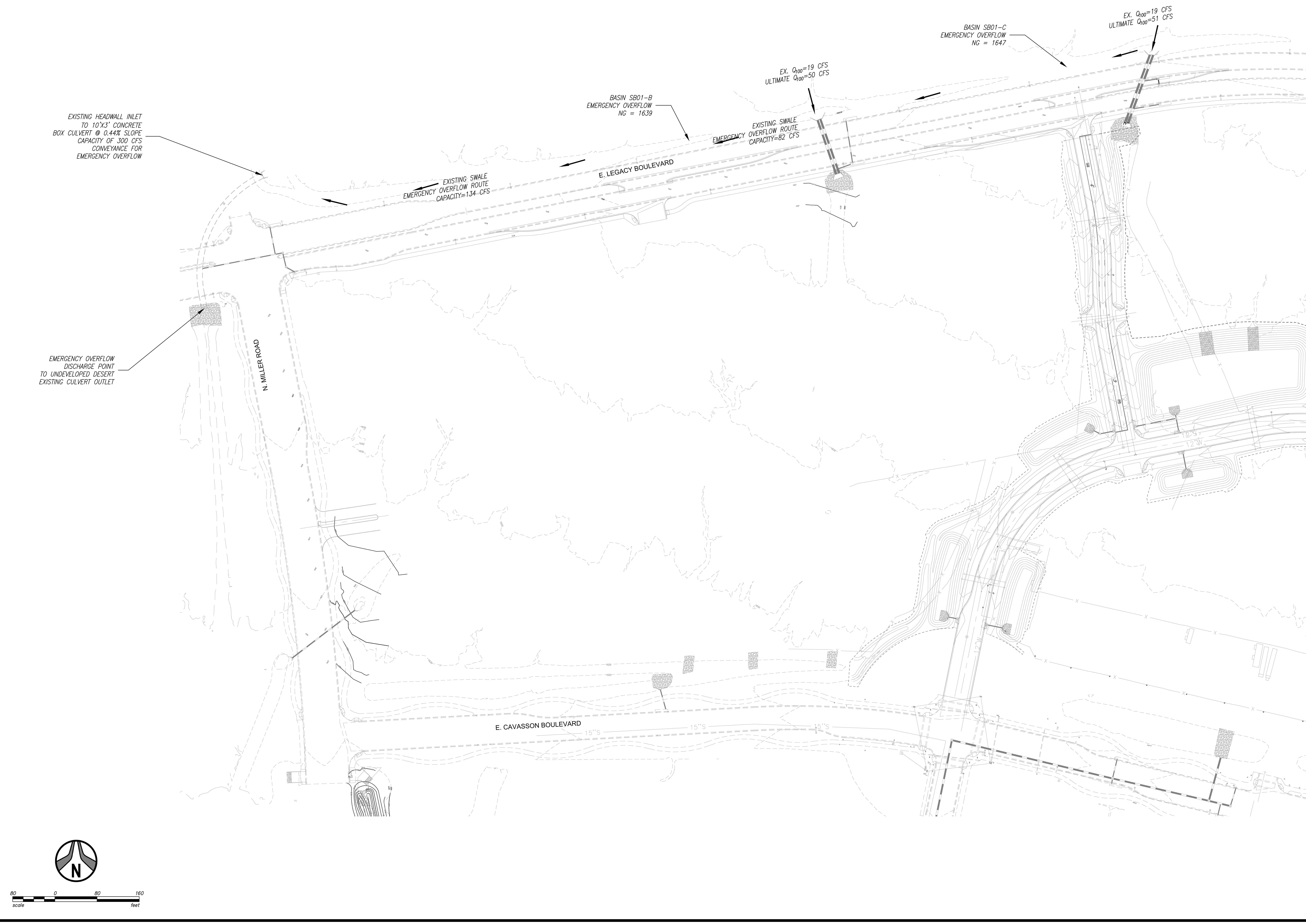
EX 03



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EXISTING OFFSITE FLOW EXHIBIT WITH EXISTING OVERFLOW

GRAYHAWK RESIDENCES AT CAVASSON

Project No. 18114-601	Date 8/27/2021
Project Mgr. GMB	Project Eng. MSW

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EX. 4

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EMERGENCY OVERFLOW EXHIBIT WITH OFFSITE DRAINAGE

GRAYHAWK RESIDENCES AT CAVASSON

Project No.	Date
18114-601	8/27/2021
Project Mgr.	Project Eng.
GMB	MSW

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EX. 5

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SUMMARY OF CATCH BASINS WITH GREATER THAN 0.5' TO BREAKOVER		
CATCH BASIN ID	PONDING DEPTH	AREA DESIGNATION
CB-1	1.10'	OPEN SPACE (LAWN AREA)
CB-2	1.20'	OPEN SPACE (LAWN AREA)

CATCH BASIN EXHIBIT
GRAYHAWK RESIDENCES AT CAVASSON

A PORTION OF THE NORTHWEST QUARTER OF SECTION 26
TOWNSHIP 4 NORTH, RANGE 4 EAST OF THE GILA AND SALT RIVER
MERIDIAN, MARICOPA COUNTY, ARIZONA

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EX 06

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