

# SEWER BASIS OF DESIGN REPORT FOR HUNTINGTON OASIS APARTMENTS

## PRELIMINARY Basis of Design Report

- ACCEPTED  
 ACCEPTED AS NOTED  
 REVISE AND RESUBMIT



Disclaimer: If accepted, the preliminary approval is granted under the condition that a final basis of design report will also be submitted for city review and approval (typically during the DR or PP case). The final report shall incorporate further water or sewer design and analysis requirements as defined in the city design standards and policy manual and address those items noted in the preliminary review comments (both separate and included herein). The final report shall be submitted and approved prior to the plan review submission.  
For questions or clarifications contact the Water Resources Planning and Engineering Department at 480-312-5685.

BY Idillon

DATE 12/28/2020

15-ZN-2020

Scottsdale, Arizona

20 November 2020

PREPARED FOR

Nelson Partners, Inc  
15210 North Scottsdale Road, Suite 300  
Scottsdale, Arizona 85254

DEVELOPER

Village Property Management, LLC  
PO BOX 88  
Beverly Hills, California 90213

SITE ADDRESS

3302 -3388 North Hayden Road  
Scottsdale, Arizona 85251

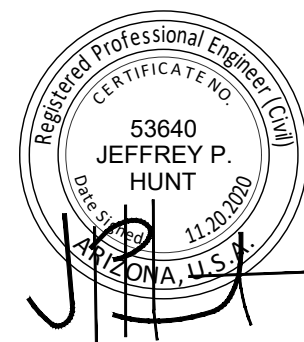
Address comments below and herein within the DR case final BOD:

- 1) **Stipulation:** Provide 20ft combined public water and sewer line easement for new water and sewer lines. Lines to be located only entirely within drive aisles with public line 6ft from any structures. DS&PM 7-1.402
- 2) For all restaurant pads you need to add an accessible grease interceptor DS&PM 7-1.411.
- 3) All new sewer service lines to be 6" minimum and per MAG 440-3 per DS&PM 7-1.409, A&B.
- 4) If water is not separately metered for the restaurant both sewer for the entire building will be billed at the higher commercial rate. Consider financial billing implications. Refer to DS&PM 6-1.416.

PREPARED BY



4450 north 12<sup>th</sup> street, #228  
phoenix, arizona 85014  
CYPRESS # 20.122



15-ZN-2020  
12/04/20

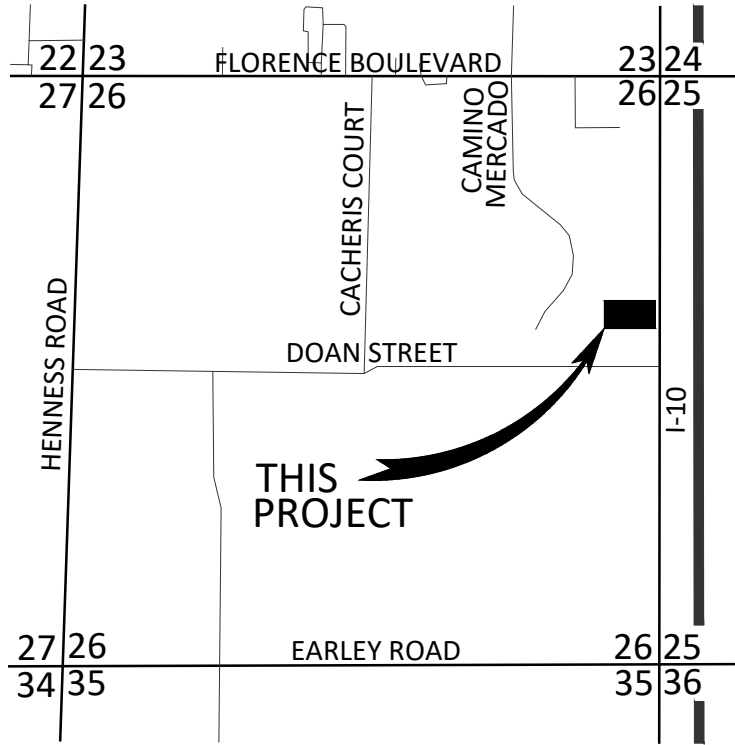
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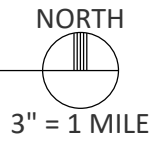
### APPENDICES

A	City of Scottsdale Water and Sewer Quarter Section Map
B	Site Plan
C	Preliminary Improvement Plans
D	Sewer Flow Calculations



IN THE SE 1/4 OF THE NE 1/4 OF SECTION 26,  
 T. 6 S., R. 6 E., G.&S.R.M.,  
 CITY OF CASA GRANDE, PINAL COUNTY, ARIZONA

**LOCATION MAP**



## INTRODUCTION: PROJECT DESCRIPTION AND LOCATION

The Project is known as 'Huntington Oasis Apartments' and is located at 3302 through 3388 North Hayden Road in Scottsdale, Arizona. The Project is located on the west side of Hayden Road, south of Osborn Road. The Project consists of the construction of a new multi-story apartment building, a restaurant building, and a retail building with all required grading and drainage, parking, utility and paving improvements.

The utility provider for sewer facilities is the City of Scottsdale.

## EXISTING CONDITIONS

Per available utility maps and as-built records, the site is currently developed with five existing buildings onsite. The existing buildings are served by individual connections to an existing 8" VCP sewer main, which ties into an existing main in Hayden Road at the southeast and wraps around the site, ending in a terminal manhole. Refer to Appendix A for City of Scottsdale Water and Sewer Quarter Section Map.

## PROPOSED CONDITIONS

The five existing buildings shall be demolished and three new buildings shall be built on site. Building 1 shall be a restaurant. Building 2 shall be a mixed-use restaurant/retail building and Building 3 an apartment building with portions of mixed-use restaurant/retail. The existing 8" sewer main shall be cut in the drive aisle at the south. The entire sewer system, including the existing 8" main and the service connections, shall be abandoned west of this cut. The main to the east that taps Hayden Road and comes onsite shall remain, along with one service to an existing drive-thru restaurant at the southeast corner. **A new manhole shall be installed to connect a new 8" sewer main to the existing 8" sewer main here.** From this manhole, the new sewer main shall run north, where all three buildings shall tap the new main with new 6" services at 2.0% slopes. Refer to Appendix B for Site Plan and Appendix C for Preliminary Improvement Plans.

## REQUIRED COMPUTATIONS

### PROPOSED SEWER DEMAND

#### Building 1

**Average Day Demand (Restaurant):**  $1.2 \text{ GPD/SF} \times 3,309 \text{ SF} = 3,970.8 \text{ GPD}$

**Peak Demand:**  $6 \times 3,970.8 = 23,824.8 \text{ GPD}$

Why 2%? 1% should satisfy IPC. Grease interceptors must be provided for restaurants and shown on final plans.

ok

Using calculations per the AAC Title 18, Chapter 9, with a population less than 100, the velocity for the partial flow in a 6-inch line at 2.0% slope is 4.05 ft/sec and the d/D ratio is 0.17. Thus, the expected maximum demand of 23,824.8 GPD is acceptable in the 6" service. Refer to Appendix D for Sewer Flow Calculations.

#### Building 2

**Average Day Demand (Restaurant):**  $1.2 \text{ GPD/SF} \times 3,600 \text{ SF} = 4,320 \text{ GPD}$   
**(Commercial/Retail):**  $0.5 \text{ GPD/SF} \times 3,600 \text{ SF} = 1,800 \text{ GPD}$   
**Total:** 6,120 GPD  
**Peak Demand (Restaurant):**  $6 \times 4,320 = 25,920 \text{ GPD}$   
**(Commercial/Retail):**  $3 \times 1,800 = 5,400 \text{ GPD}$   
**Total:** 31,320 GPD

Using calculations per the AAC Title 18, Chapter 9, with a population less than 100, the velocity for the partial flow in a 6-inch line at 2.0% slope is 4.05 ft/sec and the d/D ratio is 0.18. Thus, the expected maximum demand of 31,320 GPD is acceptable in the 6" service. Refer to Appendix D for Sewer Flow Calculations.

#### Building 3

**Average Day Demand (Restaurant):**  $1.2 \text{ GPD/SF} \times 7,472 \text{ SF} = 8,966.4 \text{ GPD}$   
**(Commercial/Retail):**  $0.5 \text{ GPD/SF} \times 4,353 \text{ SF} = 2,176.5 \text{ GPD}$   
**(Condo):**  $140 \text{ GPD/Unit} \times 300 \text{ Units} = 42,000 \text{ GPD}$   
**Total:** 53,142.9 GPD  
**Peak Demand (Restaurant):**  $6 \times 8,966.4 = 53,798.4 \text{ GPD}$   
**(Commercial/Retail):**  $3 \times 2,176.5 = 6,529.5 \text{ GPD}$   
**(Condo):**  $4.5 \times 42,000 = 189,000 \text{ GPD}$   
**Total:** 249,327.9 GPD

Using calculations per the AAC Title 18, Chapter 9, with a population less than 1,500, the velocity for the partial flow in a 6-inch line at 2.0% slope is 4.05 ft/sec and the d/D ratio is 0.49. Thus, the expected maximum demand of 249,327.9 GPD is acceptable in the 6" service. Refer to Appendix D for Sewer Flow Calculations.

#### Existing Drive-thru

**Average Day Demand (Restaurant):**  $1.2 \text{ GPD/SF} \times 3,089 \text{ SF} = 3,706.8 \text{ GPD}$   
**Peak Demand (Restaurant):**  $6 \times 3,706.8 = 22,240.8 \text{ GPD}$

#### Total Demand for New/Existing 8" Main:

**Average Day Demand:**  $3,970.8 + 6,120 + 53,142.9 + 3,706.8 = 66,940.5 \text{ GPD}$

227gpm

**Peak Demand:** 23,824.8 + 31,320 + 249,327.9 + 22,240.8 = 326,713.5

Using calculations per the AAC Title 18, Chapter 9, with a population less than 1,700, the velocity for the partial flow in an 8-inch line at 0.5% slope is 2.45 ft/sec and the d/D ratio is 0.55. Thus, the expected maximum demand of 326,713.5 GPD is acceptable in the 8" main. Refer to Appendix D for Sewer Flow Calculations.

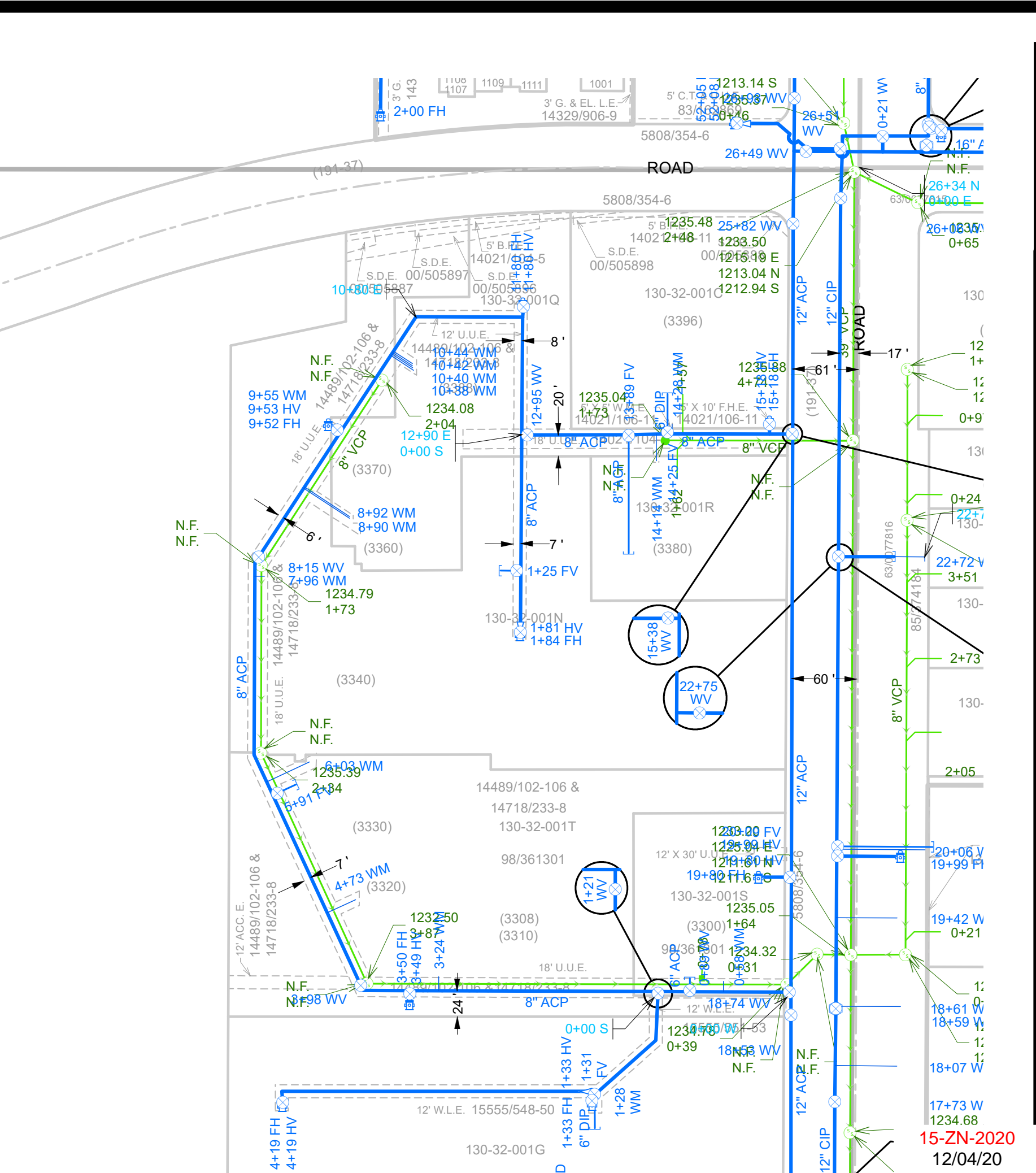
OK capacity 300gpm



**CONCLUSION**

CYPRESS respectfully submits this report as the Wastewater Design Report for the proposed Huntington Oasis Development. The proposed wastewater system shall be designed in accordance with ADEQ, International Building Code, and the City of Scottsdale standards.

Appendix A  
City of Scottsdale Water and Sewer Quarter Section Map





Appendix B  
Site Plan

PRELIMINARY

NOT FOR  
CONSTRUCTION  
OR  
RECORDING

**Huntington Oasis**  
N Hayden Rd and E Osborn Rd  
Scottsdale, AZ 85251

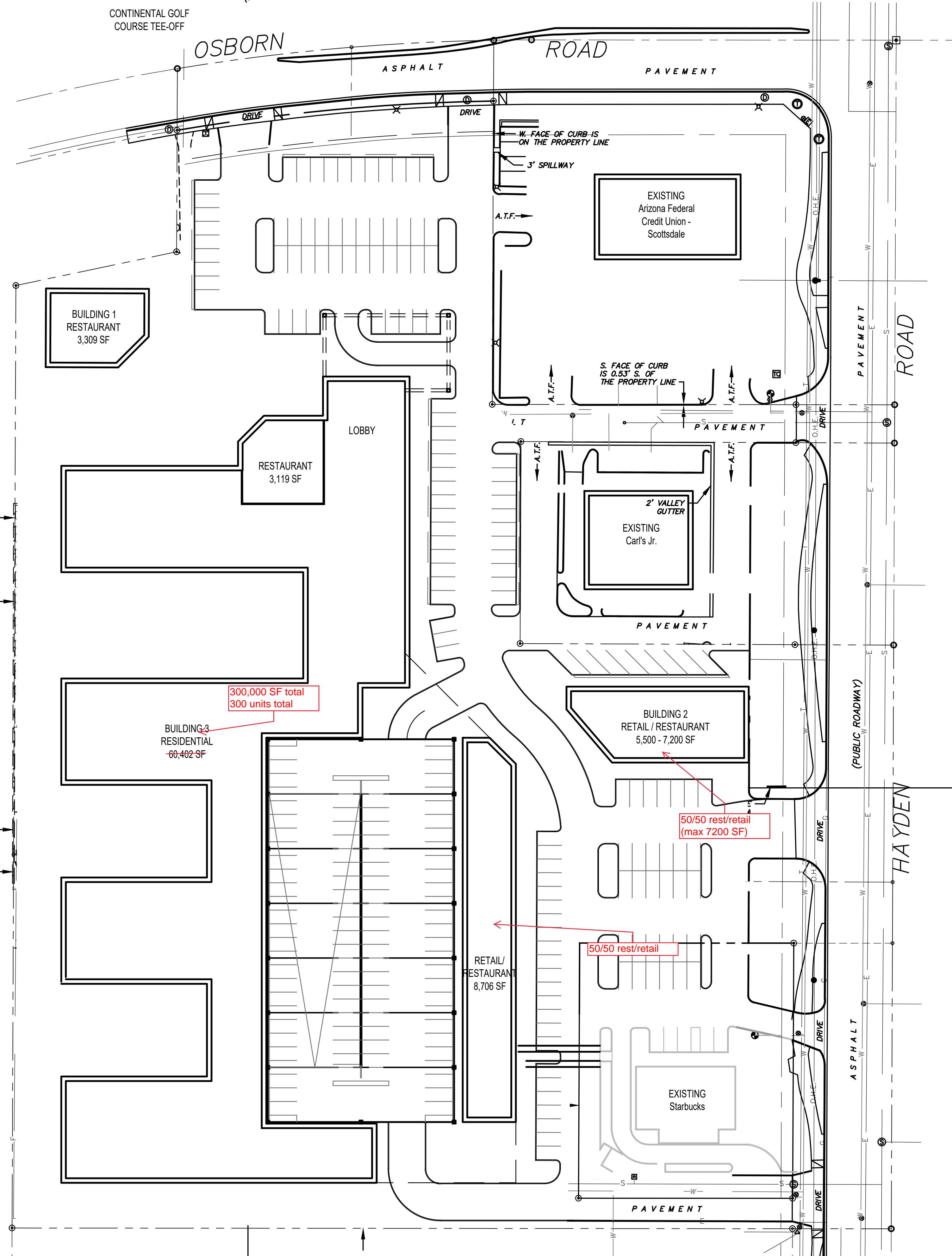
Date  
JUNE 24, 2020

PRE-APP # ZONING # DRB #

Drawings and written material appearing herein constitute original and unpublished work of the architect and may not be duplicated, used, or disclosed without written consent of the architect.

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Project No.  
319078



**PROJECT INFORMATION**

<b>CURRENT ZONING:</b>	PNC
<b>PROPOSED ZONING:</b>	PUD Planned Unit Development
<b>GROSS SITE AREA:</b>	7.00 ACRES (304,920 SF)
<b>NET LOT AREA:</b>	6.6 ACRES (288,288 SF)
<b>PROPOSED GFAR :</b>	308,000 SF (+/- 1,000 SF)
<b>MAX. HEIGHT ALLOWED:</b>	48 FT
<b>PROPOSED HEIGHT:</b>	48 FT
<b>NUMBER OF UNITS:</b>	300 UNITS

**AREA TABULATIONS**

RESTAURANT/RETAIL	21,000 SF (+/- 1,000 SF)
RESIDENTIAL	286,606 SF
850 AVG (EXCLUDING EFF. UNITS /85% EFF	300 UNITS

**PARKING TABULATIONS**

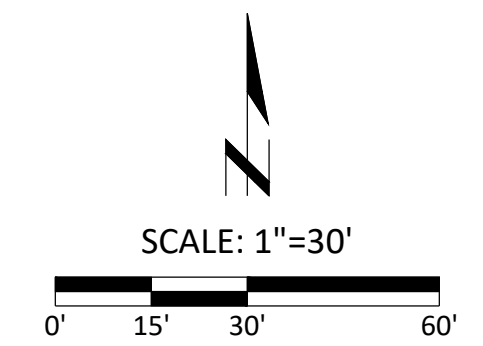
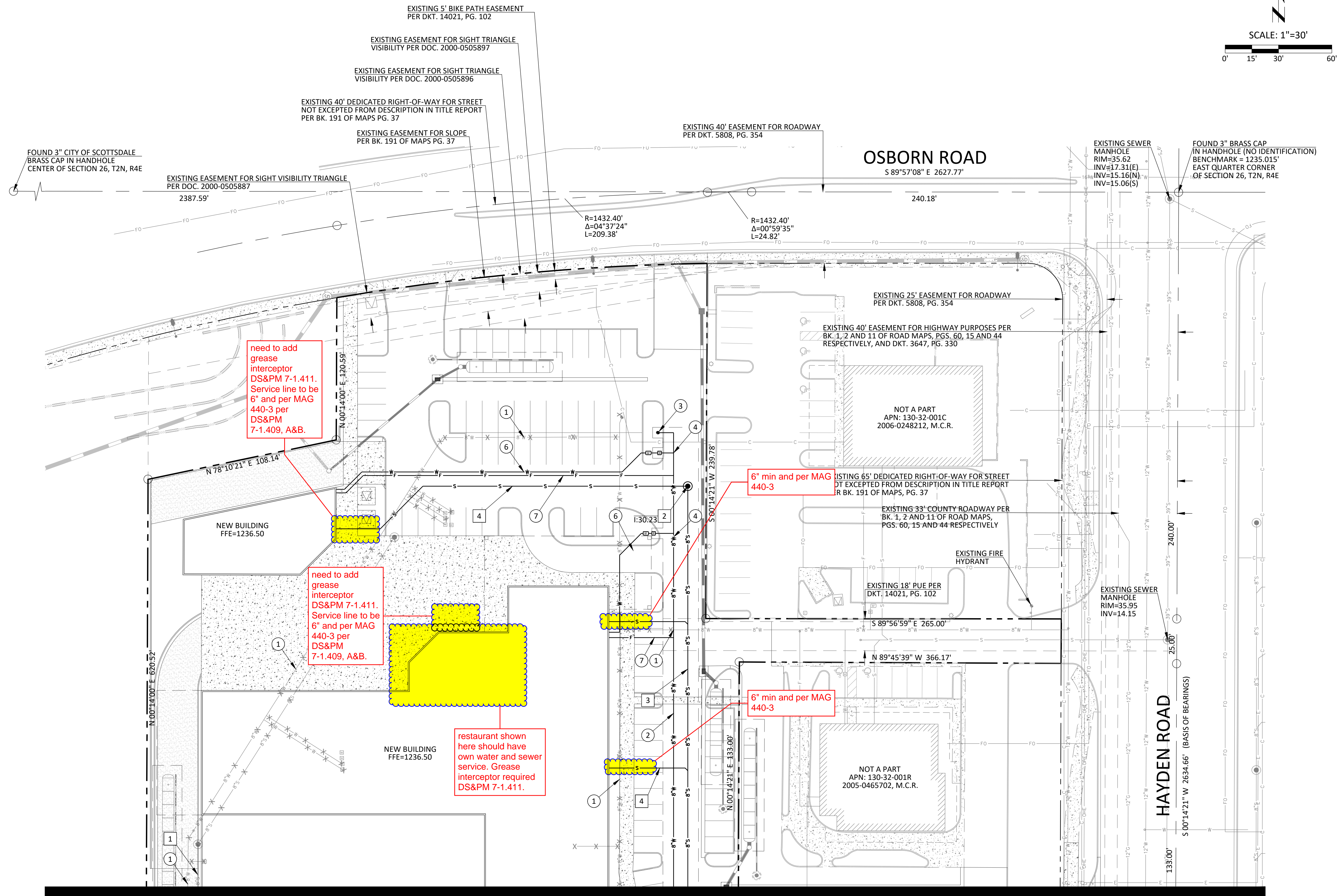
SURFACE PARKING	132 STALLS
STRUCTURED	
LEVEL B1	62 CARS
LEVEL GRADE	98 CARS
LEVEL 2	98 CARS
LEVEL 3	98 CARS
LEVEL 4	98 CARS
TOTAL	454 STALLS
<b>TOTAL</b>	<b>586 STALLS</b>

everything is sprinklered, VB construction



Appendix C  
Preliminary Improvement Plans





- WATER KEYNOTES**
- 1 EXISTING PUBLIC WATER LINE TO BE ABANDONED AND REMOVED.
  - 2 NEW 8" PUBLIC WATER MAIN.
  - 3 NEW PUBLIC FIRE HYDRANT.
  - 4 NEW DOMESTIC SERVICE, METER, AND BACKFLOW PREVENTER.
  - 5 NEW LANDSCAPE SERVICE, METER, AND BACKFLOW PREVENTER.
  - 6 NEW PRIVATE DOMESTIC WATER LINE.
  - 7 NEW PRIVATE FIRE LINE.

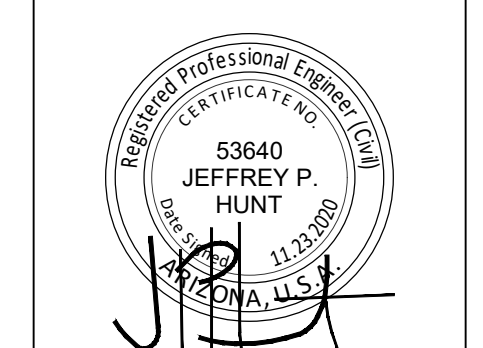
- SEWER KEYNOTES**
- 1 EXISTING PUBLIC SEWER LINE TO BE ABANDONED AND REMOVED.
  - 2 NEW PUBLIC SEWER MANHOLE.
  - 3 NEW 8" PUBLIC SEWER MAIN.
  - 4 NEW PRIVATE SEWER LINE.

**CYPRESS CIVIL**  
 4450 north 12th street  
 suite 228  
 phoenix, arizona 85014  
 p: 623.282.2498  
 e: jphunt@cypresscivil.com

NO.	DATE	REVISION

**PRELIMINARY IMPROVEMENT PLAN for  
 HUNTINGTON OASIS APARTMENTS**  
 3302-3388 NORTH HAYDEN ROAD SCOTTSDALE, ARIZONA

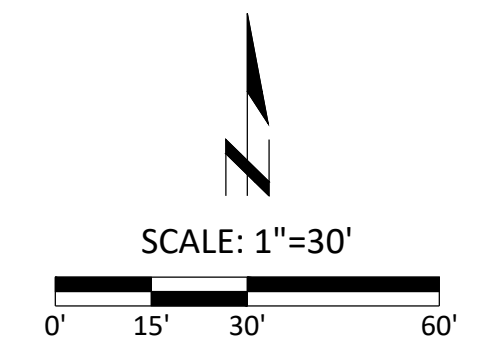
**onsite utility plan**



DRAWN: PT  
 DESIGNED: AS  
 CHECKED: JH  
 DATE: 11-23-2020  
 JOB NO: 20.122  
 SHEET NUMBER



THESE PLANS PRELIMINARY AND ARE NOT FOR CONSTRUCTION OR RECORDING

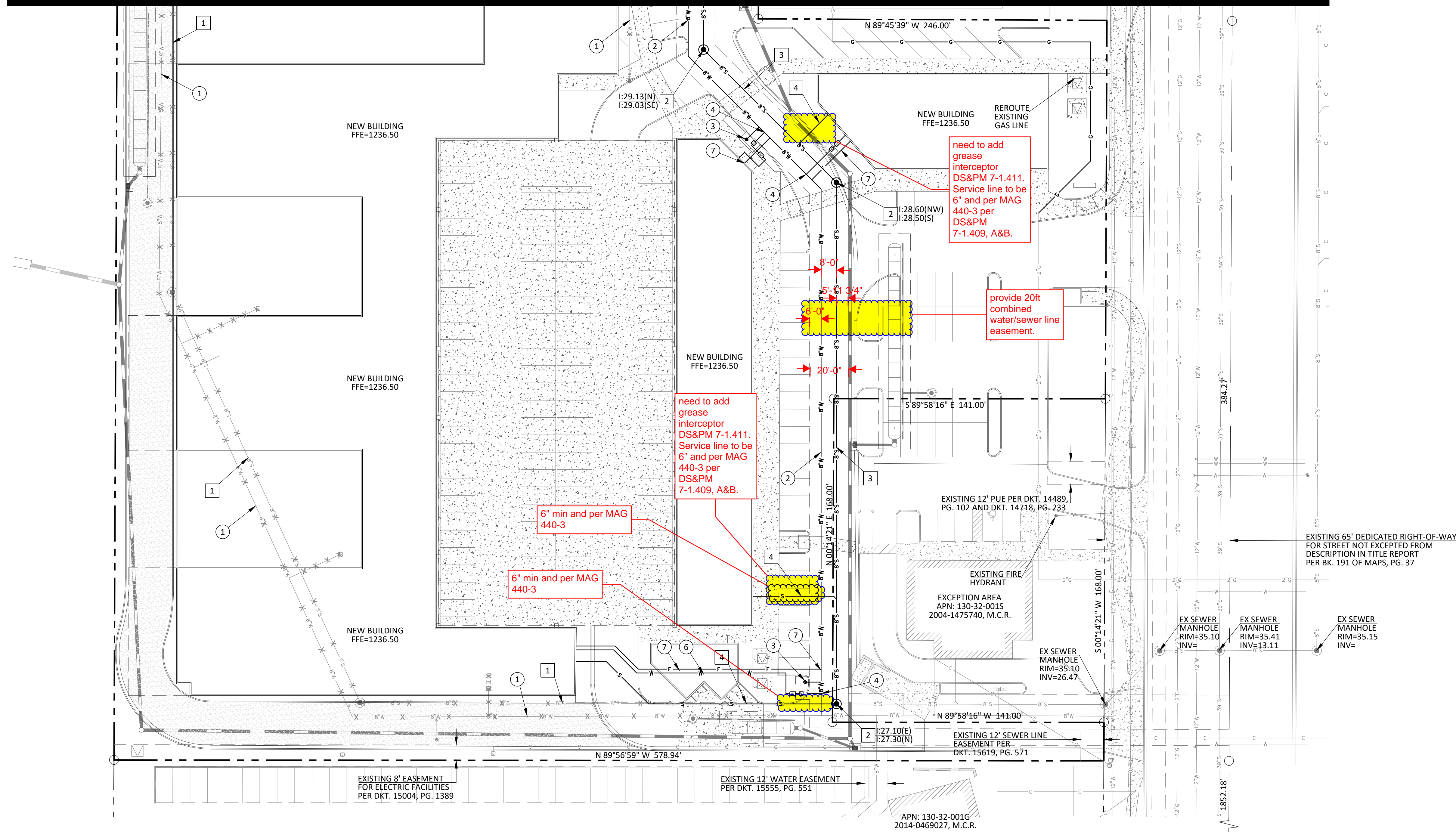


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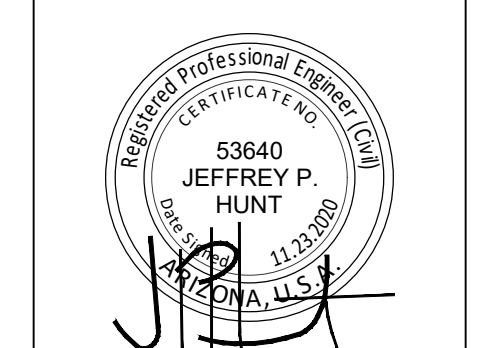
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MATCH LINE - SEE SHEET 4



FOUND 2" ALUMINUM CAP (NO IDENTIFICATION), ALSO FOUND 3" CITY OF SCOTTSDALE BRASS CAP IN HANDHOLE (10.00' W), SOUTHEAST CORNER OF SECTION 26, T2N, R4E

**PRELIMINARY IMPROVEMENT PLAN for HUNTINGTON OASIS APARTMENTS**  
 3302-3388 NORTH HAYDEN ROAD SCOTTSDALE, ARIZONA  
 onsite utility plan



DRAWN: PT  
 DESIGNED: AS  
 CHECKED: JH  
 DATE: 11-23-2020  
 JOB NO: 20.122  
 SHEET NUMBER



THESE PLANS PRELIMINARY AND ARE NOT FOR CONSTRUCTION OR RECORDING

Appendix D  
Sewer Flow Calculations



**Sewer Design Report Calculations**

Huntington Oasis - Building 1 - 6" Sewer Service Lateral

Sewage Flow Per Day (From Table 1, A.A.C. Title 18, Chapter 9)

Total Flow (GPD)	3,971
Upstream Population	100
Dry Peaking Factor	6.0
Wet Weather Peaking Factor	1
Dry Peak Flow (GPD)	23,825
Wet Peak Flow (GPD)	23,825

$$Q = \frac{1.49}{n} AR^{2/3} S^{1/2}$$

Where:  
 Q = flow in cfs  
 n = Manning's Roughness Coefficient  
 A = Cross sectional area of flow  
 R = hydraulic radius  
 S = pipe slope

**EXISTING/NEW 6" LATERAL AT 1.0%**

$n =$	0.013
Pipe diameter (in) =	6
Pipe Slope (ft/ft) =	0.01

**Full Flow\***

Area (in <sup>2</sup> ) =	28.274
R (in) =	1.50
Velocity (ft/sec) =	2.86
Pipe Capacity (GPD)	363,352

**Partial Flow**

$\phi$ (radian) =	1.72
Area (in <sup>2</sup> ) =	3.28
Wetted Perimeter (in) =	5.15
Hydraulic Radius (in) =	0.64
Velocity (ft/sec) =	1.62
d/D ratio =	0.17

**EXISTING/NEW 6" LATERAL AT 2.0%**

$n =$	0.013
Pipe diameter (in) =	6
Pipe Slope (ft/ft) =	0.02

**Full Flow\***

Area (in <sup>2</sup> ) =	28.274
R (in) =	1.50
Velocity (ft/sec) =	4.05
Pipe Capacity (GPD)	513,857

**Partial Flow**

$\phi$ (radian) =	1.57
Area (in <sup>2</sup> ) =	2.57
Wetted Perimeter (in) =	4.71
Hydraulic Radius (in) =	0.55
Velocity (ft/sec) =	2.06
d/D ratio =	0.15

\*per AAC R18-9-E301.4.01.D.2.e





**Sewer Design Report Calculations**

Huntington Oasis - Building 2 - 6" Sewer Service Lateral

Sewage Flow Per Day (From Table 1, A.A.C. Title 18, Chapter 9)

Total Flow (GPD)	6,120
Upstream Population	100
Dry Peaking Factor	Varies
Wet Weather Peaking Factor	1
Dry Peak Flow (GPD)	31,320
Wet Peak Flow (GPD)	31,320

$$Q = \frac{1.49}{n} AR^{2/3} S^{1/2}$$

Where:  
 Q = flow in cfs  
 n = Manning's Roughness Coefficient  
 A = Cross sectional area of flow  
 R = hydraulic radius  
 S = pipe slope

**EXISTING/NEW 6" LATERAL AT 1.0%**

$n =$	0.013
Pipe diameter (in) =	6
Pipe Slope (ft/ft) =	0.01

**EXISTING/NEW 6" LATERAL AT 2.0%**

$n =$	0.013
Pipe diameter (in) =	6
Pipe Slope (ft/ft) =	0.02

**Full Flow\***

Area (in <sup>2</sup> ) =	28.274
R (in) =	1.50
Velocity (ft/sec) =	2.86
Pipe Capacity (GPD)	363,352

**Full Flow\***

Area (in <sup>2</sup> ) =	28.274
R (in) =	1.50
Velocity (ft/sec) =	4.05
Pipe Capacity (GPD)	513,857

**Partial Flow**

$\phi$ (radian) =	1.85
Area (in <sup>2</sup> ) =	3.98
Wetted Perimeter (in) =	5.54
Hydraulic Radius (in) =	0.72
Velocity (ft/sec) =	1.75
d/D ratio =	0.20

**Partial Flow**

$\phi$ (radian) =	1.69
Area (in <sup>2</sup> ) =	3.12
Wetted Perimeter (in) =	5.06
Hydraulic Radius (in) =	0.62
Velocity (ft/sec) =	2.24
d/D ratio =	0.17

\*per AAC R18-9-E301.4.01.D.2.e



**Sewer Design Report Calculations**

Huntington Oasis - Building 3 - 6" Sewer Service Lateral

Sewage Flow Per Day (From Table 1, A.A.C. Title 18, Chapter 9)

Total Flow (GPD)	53,143
Upstream Population	1500
Dry Peaking Factor	Varies
Wet Weather Peaking Factor	1
Dry Peak Flow (GPD)	249,328
Wet Peak Flow (GPD)	249,328

$$Q = \frac{1.49}{n} AR^{2/3} S^{1/2}$$

Where:  
 Q = flow in cfs  
 n = Manning's Roughness Coefficient  
 A = Cross sectional area of flow  
 R = hydraulic radius  
 S = pipe slope

**EXISTING/NEW 6" LATERAL AT 1.0%**

$n =$	0.013
Pipe diameter (in) =	6
Pipe Slope (ft/ft) =	0.01

**Full Flow\***

Area (in <sup>2</sup> ) =	28.274
R (in) =	1.50
Velocity (ft/sec) =	2.86
Pipe Capacity (GPD)	363,352

**Partial Flow**

$\phi$ (radian) =	3.58
Area (in <sup>2</sup> ) =	18.00
Wetted Perimeter (in) =	10.73
Hydraulic Radius (in) =	1.68
Velocity (ft/sec) =	3.09
d/D ratio =	0.61

**EXISTING/NEW 6" LATERAL AT 2.0%**

$n =$	0.013
Pipe diameter (in) =	6
Pipe Slope (ft/ft) =	0.02

**Full Flow\***

Area (in <sup>2</sup> ) =	28.274
R (in) =	1.50
Velocity (ft/sec) =	4.05
Pipe Capacity (GPD)	513,857

**Partial Flow**

$\phi$ (radian) =	3.11
Area (in <sup>2</sup> ) =	13.82
Wetted Perimeter (in) =	9.32
Hydraulic Radius (in) =	1.48
Velocity (ft/sec) =	4.02
d/D ratio =	0.49

\*per AAC R18-9-E301.4.01.D.2.e



**Sewer Design Report Calculations**

Huntington Oasis - 8" Sewer Main

Sewage Flow Per Day (From Table 1, A.A.C. Title 18, Chapter 9)

Total Flow (GPD)	66,941
Upstream Population	1700
Dry Peaking Factor	Varies
Wet Weather Peaking Factor	1
Dry Peak Flow (GPD)	326,714
Wet Peak Flow (GPD)	326,714

$$Q = \frac{1.49}{n} AR^{2/3} S^{1/2}$$

Where:  
 Q = flow in cfs  
 n = Manning's Roughness Coefficient  
 A = Cross sectional area of flow  
 R = hydraulic radius  
 S = pipe slope

**EXISTING/NEW 8" LATERAL AT 0.5%**

$n =$	0.013
Pipe diameter (in) =	8
Pipe Slope (ft/ft) =	0.005

**Full Flow\***

Area (in <sup>2</sup> ) =	50.265
R (in) =	2.00
Velocity (ft/sec) =	2.45
Pipe Capacity (GPD)	553,380

**Partial Flow**

$\phi$ (radian) =	3.35
Area (in <sup>2</sup> ) =	28.49
Wetted Perimeter (in) =	13.41
Hydraulic Radius (in) =	2.12
Velocity (ft/sec) =	2.56
d/D ratio =	0.55

**EXISTING/NEW 8" LATERAL AT 1.0%**

$n =$	0.013
Pipe diameter (in) =	8
Pipe Slope (ft/ft) =	0.01

**Full Flow\***

Area (in <sup>2</sup> ) =	50.265
R (in) =	2.00
Velocity (ft/sec) =	3.47
Pipe Capacity (GPD)	782,598

**Partial Flow**

$\phi$ (radian) =	2.94
Area (in <sup>2</sup> ) =	21.96
Wetted Perimeter (in) =	11.77
Hydraulic Radius (in) =	1.87
Velocity (ft/sec) =	3.31
d/D ratio =	0.45

\*per AAC R18-9-E301.4.01.D.2.e