### PRELIMINARY DRAINAGE REPORT

For **MERCADO COURTYARD** 10301 N. 92<sup>nd</sup> Street Scottsdale, AZ 85254

**Prepared For:** 

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	Date Submitted: 05-03-2021 Resubmitted: 07-28-2022	

Case No: 6-ZN-2021 Plan Check No.: TBD



### **Table of Contents**

LIST OF FIGURES:	2
APPENDIX:	2
1. INTRODUCTION	3
2. LOCATION AND PROJECT DESCRIPTION	3
2.1 LOCATION:	3
2.2 EXISTING AND PROPOSED DEVELOPMENTS SURROUNDING THE SITE:	3
2.3 EXISTING SITE DESCRIPTION:	3
2.4 PROPOSED SITE DEVELOPMENT:	4
2.5 FLOOD HAZARD ZONE:	4
3. EXISTING DRAINAGE CONDITIONS	4
3.1 OFF-SITE DRAINAGE PATTERNS	4
3.3 ON-SITE DRAINAGE	4
4. PROPOSED STORM WATER MANAGEMENT	6
4.1 DESIGN INTENT:	6
4.2 DESIGN STORM REQUIREMENTS:	7
4.3 LAND CHARACTERISTICS:	7
4.4 STORMWATER RETENTION:	9
4.7 ADEQ WATER QUALITY REQUIREMENTS	11
5. FLOOD SAFETY FOR DWELLINGS	11
5.1 FINISHED FLOOR ELEVATIONS	11
6. CONCLUSIONS	11
6.1 OVERALL PROJECT:	11



### **LIST OF FIGURES:**

FIGURE 1	-	Vicinity Map
FIGURE 2	-	Aerial

- FIGURE 2 Aerial FIGURE 3 - FIRM
- FIGURE 4 FIRMette

#### **APPENDIX:**

APPENDIX I	-	Rainfall Data
APPENDIX II	-	Calculations
APPENDIX III	-	Grading and Drainage Plan



#### **1. INTRODUCTION**

This Preliminary Drainage Report represents the storm water analysis for Mercado Courtyard development proposed in Scottsdale, Arizona. Mercado Courtyard is a proposed multi-family residential development located south and east of the Shea Boulevard and 92nd Street intersection. The purpose of this report is to provide the hydrologic and hydraulic analysis, required by the City of Scottsdale, to support the proposed rezoning for said development. This report includes discussions and calculations defining the storm water management concepts for the collection and conveyance necessary to comply with the drainage requirements of the City of Scottsdale Design Standards & Policies Manual (DS&PM) 2018<sup>1</sup>, and the Drainage Design Manuals for Maricopa County, Arizona, Volumes I<sup>2</sup> and Volume II<sup>3</sup>.

#### 2. LOCATION AND PROJECT DESCRIPTION

#### 2.1 LOCATION:

The subject property consists of four contiguous parcels located south and east of Shea Boulevard and 92nd Street in Scottsdale, AZ.

- A Portion of the Northeast Quarter of Section 30, Township 3 North, Range 5 East of The Gila And Salt River Base And Meridian, Maricopa County, Arizona.
- Parcel ID: Parcel 217-39-536, Zoning is PUD Parcel 217-39-537A, Zoning is PUD Parcel 217-39-537B, Zoning is PUD Parcel 217-36-989B, Zoning is C-O PCD
- Address: 10301 N. 92<sup>nd</sup> Street. Scottsdale, Arizona 85258.

Refer to **FIGURE 1** - **Vicinity Map** for the project's location with respect to major cross streets.

#### 2.2 EXISTING AND PROPOSED DEVELOPMENTS SURROUNDING THE SITE:

- South: Parcel with APN 217-74-038, a medical office development; Zoning is C-O PCD.
- North: Parcel with APN 217-36-960L, Sprouts shopping center; Zoning is C-3 PCD.
- East: Parcel 217-36-001P; paved area and medical offices, Zoning is C-O PCD.
- West: Across N. 92<sup>th</sup> St. is Parcel 217-36-962G; medical offices; Zoning is SC PCD.

#### **2.3 EXISTING SITE DESCRIPTION:**

The project area includes approximately 257,362 sq. ft. (5.91 acres) of land designated as PUD, with the exception of parcel with APN 217-36-989B at the east which has a zoning of C-O PCD. The site is partially developed, the west side includes medical offices with associated parking lot, driveways and landscape areas; the east area is mainly undeveloped but has a small parking lot constructed at the south which connects to the eastern area of the site. The parcels generally slope towards the southwest corner.

Refer to **FIGURE 2** attached for an aerial of the site.



#### **2.4 PROPOSED SITE DEVELOPMENT:**

The proposed project will require the demolition of existing structures and associated parking lots. The proposed site will include a new high-density residential facility with 273 units, the building will have a central at-grade parking garage and rise to five floors.

Refer to Appendix III – Preliminary Grading and Drainage Plan for site layout.

#### **2.5 FLOOD HAZARD ZONE:**

FIRM Map Number 04013C1760L dated October 16, 2013 indicates the site has a Zone X-Shaded designation. Zone X-Shaded is defined as 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile.

Refer to **FIGURE 3** for the FIRM and FIRMette.

#### **3. EXISTING DRAINAGE CONDITIONS**

#### **3.1 OFF-SITE DRAINAGE PATTERNS**

The topographic survey provides the following information for offsite drainage:

- The north of the site is bounded by Sprouts Shopping center, which has its own drainage system. A ridgeline prevents stormwater from Sprouts Shopping Center parking lot to enter the site. No offsite flows from the north affect the site.
- The west of the site is bounded by N. 92<sup>nd</sup> Street. Half of the street run-off drains towards the property and is conveyed via curb and gutter into an existing catch basin adjacent to the property. No offsite flows from the west affect the site.
- The south of the site is bounded by a medical office development. The medical office development has its own drainage system and a wall separating the two properties prevents stormwater from entering the site. An existing 48" CIP pipe conveys flows from the southern property without affecting the site. No offsite flows from the south affect the site.
- The east of the site is adjacent to a paved area and medical offices. The medical office has its own drainage system and grades on the paved area go away from the site. The existing 48" CIP pipe conveys flows from the east without affecting the site. No offsite flows from the east affect the site.

#### **3.3 ON-SITE DRAINAGE**

Most of the runoff from the site is collected by existing catch basins (EX. CB-1, EX. CB-2, and EX. CB-3) and slotted drains (EX. SD-1, EX. SD-2, EX. SD-3) ultimately conveyed to the public network storm system at 92<sup>nd</sup> Street. The majority of stormwater from drainage area EX-1 flows easterly into a gutter and then proceeds to flow south into EX. CB-1. A small portion of runoff from drainage area EX-1 flows towards the northeast into EX. CB-1. Runoff from drainage area EX-2 flows westerly into a wall opening which leads stormwater into a slotted drain (EX. SD-1). Stormwater on drainage area EX-3 follows the same pattern as drainage area EX-2, flowing westerly into an opening and discharging into a slotted drain (EX. SD-2). Runoff from drainage area EX-4 flows southerly into an on-site curb and gutter, and then proceeds to flow westerly



into a wall opening leading stormwater into a slotted drain (EX-SD-3). Open retention basin, EX. Basin 1, collects stormwater on drainage area EX-5. Runoff from drainage area EX-6 flows towards EX. CB-2 located in the mid-west of the drainage area. Stormwater from EX-7 flows westerly into N. 92<sup>nd</sup> Street, where half of the runoff flows northwesterly into EX-CB-3 and the other half flows southeasterly along the street via curb and gutter. Stormwater collected on EX-R-1 drains through roof drains towards drainage area EX-1 ultimately draining into EX-CB-1. Stormwater collected on EX-R-2 drains through roof drains towards drainage area EX-3 ultimately draining into EX.SD-2.

#### Refer to Appendix II for Existing Conditions Drainage Area Map.

Existing runoff coefficients and flows for the 100-year, 5 min. event are presented in the tables below. Coefficients of 0.95 and 0.45 were chosen for building or concrete and desert landscape respectively, in accordance with the city of Scottsdale DS&PM.

Weighted Runoff Coefficient-Calculations (Cw)							
EXISTING OVERALL SITE C <sub>w</sub>							
	BUILDING or CONCRETE	DESERT LANDSCAPE	TOTAL AREA	Cwt			
C-VALUE	0.95	0.45					
AREA (ac)	3.33	2.73	6.06	0.72			
EX-1	1.39	0.44	1.83	0.83			
EX-2	0.29	0.08	0.37	0.84			
EX-3	0.31	0.21	0.52	0.75			
EX-4	0.23	0.16	0.39	0.74			
EX-5	0.00	1.58	1.58	0.45			
EX-6	0.17	0.25	0.42	0.65			
EX-7	0.05	0.00	0.05	0.95			
EX-R-1	0.69	0.00	0.69	0.95			
EX-R-2	0.20	0.00	0.20	0.95			

#### Table 1. Existing Weighted Runoff Coefficients.

The open retention Ex. Basin 1 provided volume is displayed in the table below:

#### Table 2. EX. Basin 1 Provided Volume

EX. Basin 1						
ELEV.	AREA	DEPTH	AVG V	SUM V	COMMENT	
(FT)	(SF)	(FT)	(CF)	(CF)		
1369.0	382			0.00	Bottom	
		2.00	1,704.33			
1371.0	1,314			1,704.33	Volume	



#### Table 3. Existing Onsite Flows

EXISTING ONSITE 100 YR 5 MIN							
				Q			
Drainage Area	Area (ac)	Cw	i (in/hr)	(cfs)			
EX-1	1.83	0.83	7.54	11.44			
EX-2	0.37	0.84	7.54	2.37			
EX-3	0.52	0.75	7.54	2.93			
EX-4	0.39	0.74	7.54	2.17			
EX-5	1.58	0.45	7.54	5.37			
EX-6	0.42	0.65	7.54	2.04			
EX-7	0.05	0.95	7.54	0.39			
EX-R-1	0.69	0.95	7.54	4.93			
EX-R-2	0.20	0.95	7.54	1.47			

Overall existing project area includes 6.06 Acres at Cwt = 0.72

Refer to **Appendix II** for 100-year and 10-year event calculations.

#### 4. PROPOSED STORM WATER MANAGEMENT

#### 4.1 DESIGN INTENT:

Most of the on-site drainage will be directed off-site through a storm drain system to the historical outlets. The drainage proposal will be consistent with previous conditions of the site to avoid disrupting existing drainage patterns. Flows exposed to contaminated surfaces will also be treated prior to being released as a measure to control the quality of stormwater. Drainage areas A1 through A3 will be directed towards basin 1 via catch basins and storm pipes. Drainage area A1 will flow southerly into CB-1. Stormwater from drainage area A2 will flow towards its center into CB-2. Stormwater from drainage area A3 will flow into roof drain RD-1. Drainage areas B1 through B6 will be directed to the city's public storm system. Drainage area B1 will discharge into CB-3. Drainage area B2 will discharge runoff into the existing EX. SD-1 via a wall opening. Stormwater from Drainage area B3 will discharge into the EX. SD-2. Runoff from drainage area B4 will flow southwesterly into CB-4. Drainage area B5 will flow into CB-5. Flows discharging to the existing slotted drains are reduced. The table below summarizes the flow reduction in the slotted drains.



I. Proposed	and	Existing	Flows	Comparison	
	I. Proposed	I. Proposed and	I. Proposed and Existing	I. Proposed and Existing Flows	I. Proposed and Existing Flows Comparison

P. DA	Ex. DA	Proposed Q CFS	Existing Q CFS	Difference	Inlet
B2	EX-2	0.42	2.37	-1.95	EX. SD 1
B3	Ex-3	0.98	2.93	-1.95	EX. SD 2

Since the site has been previously developed, on-site retention shall be calculated per City of Scottsdale DSPM 4-1.201.

#### **4.2 DESIGN STORM REQUIREMENTS:**

In accordance with City of Scottsdale requirements, stormwater storage for the 100-year 2hour storm event is required based on maintaining existing retention volume plus the difference between the pre vs. post development runoff from the 100-year 2-hour storm event if increased or first flush, whichever is greater. Per topographic information, there is an existing open retention basin in the site. As such, retention shall be provided by the sum of the existing retention volume plus the pre vs. post difference in volume or first flush.

#### **4.3 LAND CHARACTERISTICS:**

The proposed project site consists of residential spaces with a main drive and landscape areas. Based on the City of Scottsdale Design Standards & Policies Manual (DSPM), runoff coefficients for the 100-year storm event used are as follows:

- C=0.95 for building or concrete
- C=0.95 for paved surface
- C=0.45 for undisturbed natural desert or desert landscape

**HYDROLOGIC ANALYSIS:** The hydrologic analysis is determined using the procedures in the DSPM and the Drainage Design Manual for Maricopa County, Arizona, Volume I.



Weighted Runoff Coefficient-Calculations (Cw)					
	PROPOSEI	D OVERALL SI	TE C <sub>w</sub>		
	BUILDING or CONCRETE	DESERT LANDSCAPE	TOTAL AREA	Cwt	
C-VALUE	0.95	0.45			
AREA (ac)	4.48	1.58	6.06	0.82	
A1	0.29	0.33	0.62	0.68	
A2	0.28	0.15	0.43	0.77	
A3	0.11	0.00	0.11	0.95	
B1	0.42	0.26	0.68	0.76	
B2	0.03	0.06	0.09	0.61	
B3	0.03	0.23	0.26	0.50	
B4	0.34	0.32	0.65	0.71	
B5	0.07	0.23	0.29	0.56	
B6	2.92	0.00	2.92	0.95	

#### TABLE 5: PROPOSED CONDITIONS RUNOFF CALCULATIONS

Overall project area includes 6.06 Acres at Cwt = 0.82.

#### Table 6. Proposed Onsite Flows.

PROPOSED ONSITE 100 YR 5 MIN						
Drainage Area	Area (ac)	Cw	i (in/hr)	Q (cfs)		
A1	0.62	0.68	7.54	3.20		
A2	0.43	0.77	7.54	2.50		
A3	0.11	0.95	7.54	0.81		
B1	0.68	0.76	7.54	3.92		
B2	0.09	0.61	7.54	0.42		
В3	0.26	0.50	7.54	0.98		
B4	0.65	0.71	7.54	3.48		
B5	0.29	0.56	7.54	1.24		
B6	2.92	0.95	7.54	20.91		

Refer to the **Proposed Cwt Exhibit (Exhibit B)**, **Proposed Conditions Drainage Area Map (Exhibit D)** and Calculations in **Appendix II**.



#### 4.4 STORMWATER RETENTION:

100-YR, 2-HR STORM: Per City of Scottsdale DSPM 4-1.201, development storage requirements for the 100-yr, 2-hr storm event are calculated as follows:

$$V_r = \Delta C \, \left(\frac{R}{12}\right) A$$

where:

 $V_r$  = Required storage (cf) R = Precipitation amount =2.22 in per NOAA Atlas 14 Precipitation Frequency Estimates A = Total area of site (sf)  $\Delta C$  =  $C_{post} - C_{pre}$ 

$$V_r = (0.82 - 0.72) \left(\frac{2.22}{12}\right) (263,940.61) = 4,882.90 \ cf$$

Since the difference of the weighted coefficient is greater than 0, an increase in stormwater flows will be generated. Therefore, stormwater retention is required for the development following the pre vs. post analysis. The volume provided by EX. Basin 1 must be added to the pre vs. post analysis volume to compare it with the first flush volume. The addition of the volumes is 1,704.33 cf + 4,882.90 cf = 6,587.23 cf.

**FIRST FLUSH**: First Flush storage required is calculated in accordance with City of Scottsdale DSPM 4-1.201. Only the areas where runoff could be affected by vehicular contact are considered in the first flush calculation. The roof drainage is considered to be free of heavy traffic pollutants, therefore, on-site driveway areas and sidewalks will be considered for the calculation.

$$FF_r = C\left(\frac{P}{12}\right)A$$

where:

 $FF_r$  = First Flush required storage volume (cf)

A = Area of site excluding roofs and landscape (sf)

C = The weighted average runoff coefficient =0.95

$$FF_r = (0.95) \left(\frac{0.5}{12}\right) 62,999.82 = 2,493.74 \ cf$$

The above assessment indicates that First Flush storage is required (2,493.74 cf). Since the volume from the pre vs. post analysis is greater than the volume from the first flush analysis, retention must be provided for 6,587.23 cf per the City of Scottsdale DS&PM.

Given that stormwater volume will be increased, underground storage (Basin 1) is proposed to retain the required pre vs post volume plus existing required volume.

To comply with the City of Scottsdale DS&PM, flows from drainage areas A1 through A3 will be directed to Basin 1. Below is a summary of their required volumes.

Required Storage Volume							
Vr=1*(P/12)*Cw*A							
				P=100-yr, 2-hr=2.24 in.			
Drainage	Area	Cw	Precipitation	Volume Req.	Volume Req.		
<u>Area ID</u>	(acres)	(-)	<u>(in)</u>	(acre-ft)	<u>(CF)</u>		
A1	0.62	0.68	2.22	0.08	3,420.97		
A2	0.43	0.77	2.22	0.06	2,668.47		
A3	0.11	0.95	2.22	0.02	865.33		
BASIN 1 Total Retention: 6,954.78					6,954.78		

#### Table 7. Proposed Conditions Required Storage Volume

A volume of 6,955 cf will be retained by the proposed underground storage Basin 1.

#### **PROVIDED STORMWATER STORAGE**

The proposed development will provide storage for the pre vs post analysis and existing required volume through a corrugated metal pipe underground system. Stormwater will be discharged by the use of drywells.

Basin 1 provided storage: Basin 1 will consist of 10' diameter corrugated metal pipe and will have a length of 110 lf.  $V_P = \pi^*$  Pipe radius<sup>2</sup> \* Pipe length  $V_P = (\pi * 5^2)^*(90) = 7,068$  cf

Refer to **Appendix II** for existing and proposed volume calculations.

#### 4.6 STORMWATER DISCHARGE

For Basins with no direct bleed-off available, drywells are proposed in the on-site storage facilities to dispose of stormwater within thirty six (36) hours. The calculation is as follows:

- Minimum percolating rate of a drywell (for planning purposes)= 0.1 cfs
- Volume to be drained in 36 hours= 0.1 cfs \* 36 hours \* 3600 sec/hour= 12,960 cf = 0.298 acre-feet.
- The number of drywells will be reduced if geotechnical testing for percolation rates determine adequate infiltration is available in the native souls at lower depths. If the percolation rate of the drywells is less than 0.1 cfs the number of drywells may have to e increased.



Basin 1:

Total provided storage = 7,068 CF 7,068 CF / 12,960 CF per drywell = 0.55 = 1 drywell required.

#### **4.7 ADEQ WATER QUALITY REQUIREMENTS**

The Arizona Department of Environmental Quality (ADEQ) requires that any site disturbance over an acre is required to submit an NOI. An NOI will be submitted to ADEQ for this site after the first submittal of the construction documents as this site disturbance is over 1 acre and has flows going off-site.

#### **5. FLOOD SAFETY FOR DWELLINGS**

#### **5.1 FINISHED FLOOR ELEVATIONS**

Since project lies in an "X-Shaded" Flood Zone, finished floor elevations will also comply with the minimum elevation of 14 inches above the Ultimate Outfall of the site (1,368.66'), located on the southwest corner of the site and at least 12 inches above adjacent HWE's. The minimum proposed finish floor elevation of 1,371.25' within the project is in accordance with the previous criteria, ensuring that the building will be safe from flooding during a 100-year storm.

#### 6. CONCLUSIONS

#### **6.1 OVERALL PROJECT:**

- 1. The finish floor elevations will be designed a minimum of 14 inches above the lot Ultimate Outfall.
- 2. Historical outfalls will be maintained at proposed conditions and no detrimental effects will be posed to existing drainage patterns.
- 3. On-site retention facilities will be provided to account for pre vs. post analysis plus existing retention.

#### 7. REFERENCES

- 1. Design Standards & Policies Manual, City of Scottsdale January 2018
- 2. Drainage Design Manual for Maricopa County, Arizona, Volume I, Hydrology, Flood Control District of Maricopa County, Fourth Edition, December 14, 2018
- 3. Drainage Design Manual for Maricopa County, Arizona, Volume II, Hydraulics, Flood Control District of Maricopa County, December 14, 2018

### GRADING & DRAINAGE GRADING & DRAINAGE LANGUAGE

#### WARNING AND DISCLAIMER OF LIABILITY

The City's Stormwater and Floodplain Management Ordinance is intended to minimize the occurrence of losses, hazards and conditions adversely affecting the public health, safety and general welfare which might result from flooding. The Stormwater and Floodplain Management Ordinance identifies floodplains, floodways, flood fringes and special flood hazard areas. However, a property outside these areas could be inundated by floods. Also, much of the city is a dynamic flood area; floodways, floodplains, flood fringes and special flood hazard areas may shift from one location to another, over time, due to natural processes.

WARNING AND DISCLAIMER OF LIABILITY

The flood protection provided by the Stormwater and Floodplain Management Ordinance is considered reasonable for regulatory purposes and is based on scientific and engineering considerations. Floods larger than the base flood can and will occur on rare occasions. Floodwater heights may be increased by constructed or natural causes. The Stormwater and Floodplain Management Ordinance does not create liability on the part of the city, any officer or employee thereof, or the federal, state or county government for any flood damages that result from reliance on the Ordinance or any administrative decision lawfully made thereunder.

Compliance with the Stormwater and Floodplain Management Ordinance does not ensure complete protection from flooding. Flood-related problems such as natural erosion, streambed meander, or constructed obstructions and diversions may occur and have an adverse effect in the event of a flood. You are advised to consult your own engineer or other expert regarding these considerations.

I have read and understand the above.

Plan Check #

Owner

Date



FIGURES Figure 1. Vicinity Map Figure 2. Aerial Figure 3. FIRM Figure 4. FIRMette





Figure 1. Vicinity Map

4/30/2021 8:28:32 AM





Figure 2. Aerial

4/29/2021 5:23:10 PM

#### NOTES TO USERS

This map is for use in administering the Nation Flood Insurance Progra not necessarily identify all areas subject to flooding, particularly from loc sources of small size. The community map repository should be co possible updated or additional flood hazard information. e Program. It doe from local drainag

possed optimized a document in our nazid information. To obtain more detailed information in areas where **Base Flood Elevation** (BFEs) and/or **floodrays** have been determined, users are encountiged to consult tables contained within the Flood instances Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs are intended to flood insurance rainy rounded whet be determined. nded whole-foot elevations. These BFEs are intended or no-poses only and should not be used as the sole source of mastion. Accordingly, flood elevation data presented in the FIS ized in conjunction with the FIRM for purposes of construction a

stal Base Flood Elevations shown on this map apply only lan h American Vertical Datum of 1988 (NAVD 88), Users of this FIF forth American Vertical Datum of 1988 (NAVD 88), Users of this FIHA should wave that coastal flood elevations are also provided in the Summary of Silliw levations table in the Flood Insurance Study report for this jurisdiction. Elevati hown in the Summary of Silliwater Elevations table should be used onstruction and/or floodplain management purposes when they are higher ti e elevations shown on this FIHA.

Soundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study event for the invitational and an and source for the invitation. uted at cross sections and in hased on hydraulic cons

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Floor Insurance Study report for information on flood control structures for this

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This web tool allows users to obtain point-specific datum conversion values by zooming in and howening over a VERTCON checkbox on the layers menu on the left side of the screem. The VERTCON grid referenced in this web application was also used to convert existing flood elevations from NGVD 29 to NAVD 88.

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manufactory groups gamperspectration approximation and a sufficient transmission on this FIRM was deviced from multiple sourced magery was provided in digital format by the Maricopa County Department is Works, Flood Control District. The imagery is dated Cotober 2009 I ber 2009. Additional National Agricultural Imagery Program (NAIP) imager wided by the Arizona State Land Department (ALRIS) and is dated 2001 coordinate system used for the production of the digital FIRM is State ona Central NAD83 HARN, International Feet.

The profile base line depicted on this map represents the hydraulic modelin baselines that match flood profiles in the FIS report. As a result of improve topographic data, the profile base line, in some cases, may deviate significant from the channel centerline or appear outside the SFHA.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this may was published, may users should contact appropriate community officials to verify current corporate limit locations.

ease refer to the separately printed Map Index to an overview map of the unry showing the layout of map panels; community map repeated of a Listing of Communities table containing National Flood insurance Program des for each community, as well as a listing of the panels on which each minumity is location.

tion on available products associated with this FIBM, visit the FEM For Information on available products associated with this FIFM, visit the FEM Map Service Center (MSC) website at http://msc/ema.gov. Available product may include previously issued Letters of Map Change, a Flood Insurance Stud Report, or digital versions of this map. Many of these products can be ordered o obtained directly from the MSC website.

Occasine outsety non time indo vectories. If you have questions about this map, how to order products, or the National Flood Insurance Program in general, please call the FEMA Map Information eXchange (FMN) at 1-377-FEMA MAP (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/.



**FIGURE 3. FIRM** 

### National Flood Hazard Layer FIRMette



#### Legend



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020



# **APPENDIX I** RAINFALL DATA

Precipitation Frequency Data Server



NOAA Atlas 14, Volume 1, Version 5 Location name: Scottsdale, Arizona, USA\* Latitude: 33.5798°, Longitude: -111.8816° Elevation: 1370.67 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF\_tabular | PF\_graphical | Maps\_&\_aerials

#### PF tabular

PDS-	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) <sup>1</sup>									
Duration				Avera	ge recurren	ce interval (	years)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>2.26</b> (1.87-2.77)	<b>2.94</b> (2.46-3.61)	<b>3.98</b> (3.29-4.87)	<b>4.78</b> (3.94-5.83)	<b>5.86</b> (4.74-7.13)	<b>6.68</b> (5.35-8.08)	<b>7.54</b> (5.93-9.08)	<b>8.40</b> (6.49-10.1)	<b>9.56</b> (7.20-11.5)	<b>10.4</b> (7.70-12.6)
10-min	<b>1.72</b> (1.43-2.11)	<b>2.24</b> (1.87-2.75)	<b>3.03</b> (2.51-3.71)	<b>3.64</b> (2.99-4.44)	<b>4.46</b> (3.61-5.42)	<b>5.09</b> (4.07-6.14)	<b>5.74</b> (4.51-6.92)	<b>6.39</b> (4.94-7.69)	<b>7.28</b> (5.48-8.76)	<b>7.95</b> (5.87-9.58)
15-min	<b>1.42</b> (1.18-1.74)	<b>1.85</b> (1.55-2.28)	<b>2.50</b> (2.07-3.06)	<b>3.00</b> (2.47-3.67)	<b>3.68</b> (2.98-4.48)	<b>4.20</b> (3.36-5.08)	<b>4.74</b> (3.73-5.72)	<b>5.28</b> (4.08-6.36)	<b>6.01</b> (4.53-7.24)	<b>6.57</b> (4.85-7.92)
30-min	<b>0.954</b>	<b>1.25</b>	<b>1.69</b>	<b>2.02</b>	<b>2.48</b>	<b>2.83</b>	<b>3.19</b>	<b>3.56</b>	<b>4.05</b>	<b>4.42</b>
	(0.792-1.17)	(1.04-1.53)	(1.39-2.06)	(1.67-2.47)	(2.01-3.02)	(2.27-3.42)	(2.51-3.85)	(2.75-4.28)	(3.05-4.87)	(3.26-5.33)
60-min	<b>0.591</b> (0.491-0.725)	<b>0.771</b> (0.645-0.948)	<b>1.04</b> (0.863-1.28)	<b>1.25</b> (1.03-1.53)	<b>1.53</b> (1.24-1.87)	<b>1.75</b> (1.40-2.12)	<b>1.98</b> (1.55-2.38)	<b>2.20</b> (1.70-2.65)	<b>2.51</b> (1.89-3.02)	<b>2.74</b> (2.02-3.30)
2-hr	<b>0.346</b>	<b>0.448</b>	<b>0.596</b>	<b>0.710</b>	<b>0.866</b>	<b>0.984</b>	<b>1.11</b>	<b>1.23</b>	<b>1.40</b>	<b>1.53</b>
	(0.292-0.415)	(0.378-0.538)	(0.501-0.714)	(0.590-0.850)	(0.714-1.03)	(0.800-1.17)	(0.884-1.31)	(0.966-1.46)	(1.07-1.65)	(1.15-1.81)
3-hr	<b>0.256</b> (0.215-0.313)	<b>0.328</b> (0.277-0.404)	<b>0.429</b> (0.360-0.524)	<b>0.509</b> (0.423-0.618)	<b>0.621</b> (0.508-0.750)	<b>0.710</b> (0.573-0.853)	<b>0.803</b> (0.636-0.964)	<b>0.900</b> (0.701-1.08)	<b>1.03</b> (0.781-1.24)	<b>1.14</b> (0.842-1.37)
6-hr	<b>0.154</b>	<b>0.195</b>	<b>0.249</b>	<b>0.293</b>	<b>0.351</b>	<b>0.397</b>	<b>0.445</b>	<b>0.494</b>	<b>0.561</b>	<b>0.613</b>
	(0.132-0.184)	(0.168-0.232)	(0.213-0.295)	(0.247-0.345)	(0.293-0.412)	(0.326-0.465)	(0.360-0.519)	(0.393-0.578)	(0.434-0.655)	(0.464-0.718)
12-hr	<b>0.085</b>	<b>0.107</b>	<b>0.136</b>	<b>0.158</b>	<b>0.188</b>	<b>0.210</b>	<b>0.234</b>	<b>0.258</b>	<b>0.290</b>	<b>0.315</b>
	(0.074-0.100)	(0.093-0.126)	(0.117-0.159)	(0.135-0.184)	(0.159-0.219)	(0.176-0.245)	(0.193-0.272)	(0.210-0.299)	(0.230-0.338)	(0.245-0.370)
24-hr	<b>0.050</b>	<b>0.063</b>	<b>0.082</b>	<b>0.096</b>	<b>0.117</b>	<b>0.133</b>	<b>0.149</b>	<b>0.167</b>	<b>0.191</b>	<b>0.209</b>
	(0.044-0.057)	(0.056-0.073)	(0.072-0.094)	(0.085-0.111)	(0.102-0.134)	(0.115-0.152)	(0.128-0.171)	(0.142-0.190)	(0.160-0.218)	(0.173-0.240)
2-day	<b>0.027</b>	<b>0.034</b>	<b>0.045</b>	<b>0.053</b>	<b>0.065</b>	<b>0.074</b>	<b>0.084</b>	<b>0.094</b>	<b>0.109</b>	<b>0.120</b>
	(0.024-0.031)	(0.030-0.039)	(0.039-0.051)	(0.047-0.061)	(0.056-0.074)	(0.064-0.085)	(0.072-0.096)	(0.080-0.108)	(0.091-0.125)	(0.099-0.138)
3-day	<b>0.019</b>	<b>0.025</b>	<b>0.032</b>	<b>0.038</b>	<b>0.047</b>	<b>0.054</b>	<b>0.062</b>	<b>0.069</b>	<b>0.080</b>	<b>0.089</b>
	(0.017-0.022)	(0.022-0.028)	(0.028-0.037)	(0.034-0.044)	(0.041-0.054)	(0.047-0.062)	(0.053-0.070)	(0.059-0.079)	(0.067-0.092)	(0.074-0.102)
4-day	<b>0.015</b>	<b>0.020</b>	<b>0.026</b>	<b>0.031</b>	<b>0.038</b>	<b>0.044</b>	<b>0.050</b>	<b>0.057</b>	<b>0.066</b>	<b>0.074</b>
	(0.014-0.018)	(0.017-0.022)	(0.023-0.030)	(0.027-0.035)	(0.033-0.043)	(0.038-0.050)	(0.043-0.057)	(0.048-0.065)	(0.056-0.075)	(0.062-0.084)
7-day	<b>0.010</b>	<b>0.013</b>	<b>0.017</b>	<b>0.020</b>	<b>0.025</b>	<b>0.028</b>	<b>0.032</b>	<b>0.037</b>	<b>0.043</b>	<b>0.047</b>
	(0.009-0.011)	(0.011-0.014)	(0.015-0.019)	(0.017-0.023)	(0.021-0.028)	(0.024-0.032)	(0.028-0.037)	(0.031-0.042)	(0.036-0.049)	(0.039-0.054)
10-day	<b>0.007</b>	<b>0.010</b>	<b>0.013</b>	<b>0.015</b>	<b>0.019</b>	<b>0.021</b>	<b>0.024</b>	<b>0.027</b>	<b>0.032</b>	<b>0.035</b>
	(0.007-0.009)	(0.008-0.011)	(0.011-0.014)	(0.013-0.017)	(0.016-0.021)	(0.018-0.024)	(0.021-0.027)	(0.023-0.031)	(0.027-0.036)	(0.029-0.040)
20-day	<b>0.005</b>	<b>0.006</b>	<b>0.008</b>	<b>0.009</b>	<b>0.011</b>	<b>0.013</b>	<b>0.014</b>	<b>0.016</b>	<b>0.018</b>	<b>0.019</b>
	(0.004-0.005)	(0.005-0.007)	(0.007-0.009)	(0.008-0.010)	(0.010-0.013)	(0.011-0.014)	(0.012-0.016)	(0.014-0.018)	(0.015-0.020)	(0.016-0.022)
30-day	<b>0.004</b>	<b>0.005</b>	<b>0.006</b>	<b>0.007</b>	<b>0.009</b>	<b>0.010</b>	<b>0.011</b>	<b>0.012</b>	<b>0.014</b>	<b>0.015</b>
	(0.003-0.004)	(0.004-0.005)	(0.005-0.007)	(0.006-0.008)	(0.008-0.010)	(0.009-0.011)	(0.010-0.012)	(0.011-0.014)	(0.012-0.016)	(0.013-0.017)
45-day	<b>0.003</b>	<b>0.004</b>	<b>0.005</b>	<b>0.006</b>	<b>0.007</b>	<b>0.008</b>	<b>0.008</b>	<b>0.009</b>	<b>0.010</b>	<b>0.011</b>
	(0.002-0.003)	(0.003-0.004)	(0.004-0.005)	(0.005-0.006)	(0.006-0.008)	(0.007-0.008)	(0.007-0.009)	(0.008-0.010)	(0.009-0.012)	(0.010-0.013)
60-day	<b>0.002</b>	<b>0.003</b>	<b>0.004</b>	<b>0.005</b>	<b>0.005</b>	<b>0.006</b>	<b>0.007</b>	<b>0.007</b>	<b>0.008</b>	<b>0.009</b>
	(0.002-0.003)	(0.003-0.003)	(0.003-0.004)	(0.004-0.005)	(0.005-0.006)	(0.005-0.007)	(0.006-0.008)	(0.007-0.008)	(0.007-0.009)	(0.008-0.010)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

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Back to Top

#### **PF graphical**







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Back to Top

Maps & aerials

Small scale terrain

Precipitation Frequency Data Server



NOAA Atlas 14, Volume 1, Version 5 Location name: Scottsdale, Arizona, USA\* Latitude: 33.5798°, Longitude: -111.8816° Elevation: 1370.67 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

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#### PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) <sup>1</sup>										
Duration		Average recurrence interval (years)								
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.188</b>	<b>0.245</b>	<b>0.332</b>	<b>0.398</b>	<b>0.488</b>	<b>0.557</b>	<b>0.628</b>	<b>0.700</b>	<b>0.797</b>	<b>0.870</b>
	(0.156-0.231)	(0.205-0.301)	(0.274-0.406)	(0.328-0.486)	(0.395-0.594)	(0.446-0.673)	(0.494-0.757)	(0.541-0.842)	(0.600-0.959)	(0.642-1.05)
10-min	<b>0.286</b>	<b>0.373</b>	<b>0.505</b>	<b>0.606</b>	<b>0.743</b>	<b>0.848</b>	<b>0.956</b>	<b>1.07</b>	<b>1.21</b>	<b>1.33</b>
	(0.238-0.351)	(0.312-0.459)	(0.418-0.618)	(0.499-0.740)	(0.602-0.903)	(0.679-1.02)	(0.752-1.15)	(0.823-1.28)	(0.913-1.46)	(0.978-1.60)
15-min	<b>0.355</b>	<b>0.463</b>	<b>0.626</b>	<b>0.751</b>	<b>0.921</b>	<b>1.05</b>	<b>1.19</b>	<b>1.32</b>	<b>1.50</b>	<b>1.64</b>
	(0.295-0.435)	(0.387-0.569)	(0.518-0.766)	(0.618-0.917)	(0.746-1.12)	(0.841-1.27)	(0.932-1.43)	(1.02-1.59)	(1.13-1.81)	(1.21-1.98)
30-min	<b>0.477</b>	<b>0.623</b>	<b>0.843</b>	<b>1.01</b>	<b>1.24</b>	<b>1.42</b>	<b>1.60</b>	<b>1.78</b>	<b>2.03</b>	<b>2.21</b>
	(0.396-0.586)	(0.521-0.766)	(0.697-1.03)	(0.833-1.23)	(1.00-1.51)	(1.13-1.71)	(1.25-1.92)	(1.37-2.14)	(1.52-2.44)	(1.63-2.67)
60-min	<b>0.591</b> (0.491-0.725)	<b>0.771</b> (0.645-0.948)	<b>1.04</b> (0.863-1.28)	<b>1.25</b> (1.03-1.53)	<b>1.53</b> (1.24-1.87)	<b>1.75</b> (1.40-2.12)	<b>1.98</b> (1.55-2.38)	<b>2.20</b> (1.70-2.65)	<b>2.51</b> (1.89-3.02)	<b>2.74</b> (2.02-3.30)
2-hr	<b>0.693</b>	<b>0.895</b>	<b>1.19</b>	<b>1.42</b>	<b>1.73</b>	<b>1.97</b>	<b>2.22</b>	<b>2.46</b>	<b>2.80</b>	<b>3.05</b>
	(0.583-0.830)	(0.756-1.08)	(1.00-1.43)	(1.18-1.70)	(1.43-2.06)	(1.60-2.34)	(1.77-2.62)	(1.93-2.91)	(2.14-3.31)	(2.29-3.63)
3-hr	<b>0.769</b> (0.647-0.941)	<b>0.985</b> (0.831-1.21)	<b>1.29</b> (1.08-1.58)	<b>1.53</b> (1.27-1.86)	<b>1.87</b> (1.53-2.25)	<b>2.13</b> (1.72-2.56)	<b>2.41</b> (1.91-2.90)	<b>2.70</b> (2.11-3.24)	<b>3.10</b> (2.35-3.72)	<b>3.43</b> (2.53-4.11)
6-hr	<b>0.925</b>	<b>1.17</b>	<b>1.49</b>	<b>1.75</b>	<b>2.10</b>	<b>2.38</b>	<b>2.67</b>	<b>2.96</b>	<b>3.36</b>	<b>3.67</b>
	(0.793-1.10)	(1.00-1.39)	(1.27-1.77)	(1.48-2.06)	(1.76-2.47)	(1.95-2.78)	(2.16-3.11)	(2.35-3.46)	(2.60-3.92)	(2.78-4.30)
12-hr	<b>1.03</b>	<b>1.29</b>	<b>1.63</b>	<b>1.90</b>	<b>2.26</b>	<b>2.54</b>	<b>2.82</b>	<b>3.11</b>	<b>3.49</b>	<b>3.80</b>
	(0.887-1.21)	(1.12-1.52)	(1.41-1.91)	(1.62-2.22)	(1.91-2.63)	(2.12-2.95)	(2.32-3.27)	(2.53-3.61)	(2.77-4.08)	(2.95-4.45)
24-hr	<b>1.20</b>	<b>1.52</b>	<b>1.96</b>	<b>2.31</b>	<b>2.80</b>	<b>3.19</b>	<b>3.59</b>	<b>4.00</b>	<b>4.57</b>	<b>5.03</b>
	(1.06-1.38)	(1.34-1.75)	(1.73-2.26)	(2.03-2.65)	(2.44-3.21)	(2.75-3.64)	(3.08-4.10)	(3.40-4.57)	(3.83-5.23)	(4.16-5.77)
2-day	<b>1.29</b>	<b>1.65</b>	<b>2.16</b>	<b>2.56</b>	<b>3.13</b>	<b>3.57</b>	<b>4.05</b>	<b>4.54</b>	<b>5.22</b>	<b>5.76</b>
	(1.14-1.48)	(1.45-1.89)	(1.89-2.47)	(2.24-2.93)	(2.71-3.57)	(3.08-4.08)	(3.45-4.63)	(3.84-5.19)	(4.36-5.99)	(4.76-6.63)
3-day	<b>1.38</b>	<b>1.77</b>	<b>2.32</b>	<b>2.77</b>	<b>3.40</b>	<b>3.90</b>	<b>4.44</b>	<b>5.00</b>	<b>5.79</b>	<b>6.43</b>
	(1.22-1.58)	(1.55-2.02)	(2.04-2.65)	(2.42-3.16)	(2.96-3.87)	(3.37-4.44)	(3.80-5.05)	(4.25-5.71)	(4.86-6.61)	(5.33-7.36)
4-day	<b>1.47</b>	<b>1.88</b>	<b>2.49</b>	<b>2.98</b>	<b>3.67</b>	<b>4.23</b>	<b>4.83</b>	<b>5.46</b>	<b>6.36</b>	<b>7.09</b>
	(1.30-1.68)	(1.66-2.15)	(2.19-2.83)	(2.61-3.39)	(3.20-4.17)	(3.66-4.80)	(4.15-5.48)	(4.66-6.22)	(5.35-7.23)	(5.91-8.08)
7-day	<b>1.65</b>	<b>2.11</b>	<b>2.80</b>	<b>3.35</b>	<b>4.13</b>	<b>4.76</b>	<b>5.44</b>	<b>6.15</b>	<b>7.16</b>	<b>7.97</b>
	(1.45-1.90)	(1.85-2.42)	(2.45-3.20)	(2.92-3.83)	(3.58-4.72)	(4.11-5.43)	(4.65-6.20)	(5.22-7.02)	(6.00-8.18)	(6.61-9.13)
10-day	<b>1.79</b>	<b>2.29</b>	<b>3.02</b>	<b>3.61</b>	<b>4.44</b>	<b>5.11</b>	<b>5.81</b>	<b>6.56</b>	<b>7.60</b>	<b>8.44</b>
	(1.57-2.04)	(2.01-2.61)	(2.65-3.44)	(3.16-4.11)	(3.86-5.04)	(4.42-5.79)	(4.99-6.59)	(5.59-7.44)	(6.40-8.63)	(7.04-9.60)
20-day	<b>2.20</b> (1.95-2.51)	<b>2.84</b> (2.50-3.22)	<b>3.75</b> (3.30-4.25)	<b>4.44</b> (3.89-5.03)	<b>5.37</b> (4.69-6.07)	<b>6.08</b> (5.30-6.88)	<b>6.81</b> (5.90-7.72)	<b>7.55</b> (6.51-8.57)	<b>8.55</b> (7.31-9.72)	<b>9.32</b> (7.90-10.6)
30-day	<b>2.58</b> (2.27-2.93)	<b>3.32</b> (2.93-3.77)	<b>4.38</b> (3.86-4.96)	<b>5.19</b> (4.56-5.86)	<b>6.27</b> (5.48-7.08)	<b>7.10</b> (6.19-8.01)	<b>7.96</b> (6.90-8.97)	<b>8.82</b> (7.61-9.95)	<b>9.99</b> (8.56-11.3)	<b>10.9</b> (9.26-12.3)
45-day	<b>3.00</b> (2.65-3.39)	<b>3.86</b> (3.42-4.37)	<b>5.09</b> (4.51-5.75)	<b>6.01</b> (5.30-6.78)	<b>7.21</b> (6.34-8.13)	<b>8.11</b> (7.11-9.16)	<b>9.03</b> (7.87-10.2)	<b>9.95</b> (8.63-11.3)	<b>11.2</b> (9.61-12.7)	<b>12.1</b> (10.3-13.7)
60-day	<b>3.31</b> (2.95-3.74)	<b>4.28</b> (3.81-4.83)	<b>5.64</b> (5.00-6.35)	<b>6.62</b> (5.86-7.46)	<b>7.90</b> (6.98-8.89)	<b>8.85</b> (7.79-9.97)	<b>9.80</b> (8.59-11.1)	<b>10.7</b> (9.37-12.1)	<b>12.0</b> (10.4-13.5)	<b>12.9</b> (11.1-14.6)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

Back to Top

**PF graphical** 









NOAA Atlas 14, Volume 1, Version 5

Created (GMT): Tue Apr 27 19:33:20 2021

Back to Top

Maps & aerials

Small scale terrain

Precipitation Frequency Data Server



Large scale terrain





Large scale aerial



# APPENDIX II CALCULATIONS

8280 E. Gelding Dr., Suite 101









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X-DAM

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1 OF 1



PROPOSED ONSITE 100 YR 5 MIN									
Drainage Area	Area (ac)	Cw	i (in/hr)	Q (cfs)					
A1	0.62	0.68	7.54	3.20					
A2	0.43	0.77	7.54	2.50					
A3	0.11	0.95	7.54	0.81					
B1	0.68	0.76	7.54	3.92					
B2	0.09	0.61	7.54	0.42					
B3	0.26	0.50	7.54	0.98					
<mark>B</mark> 4	0.65	0.71	7.54	3.48					
B5	0.29	0.56	7.54	1.24					
B6	2.92	0.95	7.54	20.91					

Required Storage Volume											
	Vr=1*(P/12)*Cw*A										
	P=100-yr, 2-hr=2.24 in.										
Drainage	Area	C <sub>w</sub>	Precipitation	Volume Req.	Volume Req.						
<u>Area ID</u>	<u>(acres)</u>	(-)	<u>(in)</u>	(acre-ft)	<u>(CF)</u>						
A1	0.62	0.68	2.22	0.08	3,420.97						
A2	0.43	0.77	2.22	0.06	2,668.47						
A3	0.11	0.95	2.22	0.02	865.33						
	BASIN 1 Total Retention: 6,954.78										







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EXISTING ONSITE 10 YR 5 MIN										
Drainage Area	Area (ac)	Cw	i (in/hr)	Q (cfs)						
EX-1	1.83	0.83	4.78	7.25						
EX-2	0.37	0.84	4.78	1.50						
EX-3	0.52	0.75	4.78	1.86						
EX-4	0.39	0.74	4.78	1.37						
EX-5	1.58	0.45	4.78	3.40						
EX-6	0.42	0.65	4.78	1.29						
EX-7	0.05	0.95	4.78	0.25						
EX-R-1	0.69	0.95	4.78	3.13						
EX-R-2	0.20	0.95	4.78	0.93						

EXISTING ONSITE 100 YR 5 MIN								
Drainage Area	Area (ac)	Cw	i (in/hr)	Q (cfs)				
EX-1	1.83	0.83	7.54	11.44				
EX-2	0.37	0.84	7.54	2.37				
EX-3	0.52	0.75	7.54	2.93				
EX-4	0.39	0.74	7.54	2.17				
EX-5	1.58	0.45	7.54	5.37				
EX-6	0.42	0.65	7.54	2.04				
EX-7	0.05	0.95	7.54	0.39				
EX-R-1	0.69	0.95	7.54	4.93				
EX-R-2	0.20	0.95	7.54	1.47				

Existing Volume Calculations											
							Vr=1*(P/12)*Cw*A				
							P=100-yr, 2-hr=2.22 in.				
Drainage	Area	C <sub>w</sub>	intensity	Precipitation	Q	Volume Req.	Volume Req.				
<u>Area ID</u>	(acres)	<u>(-)</u>	<u>(in/hr)</u>	<u>(in)</u>	<u>(cfs)</u>	(acre-ft)	<u>(CF)</u>				
EXISTING Volum	es										
EX-1	1.83	0.83	7.54	2.22	11.44	0.281	12,229.30				
EX-2	0.37	0.84	7.54	2.22	2.37	0.058	2,536.38				
EX-3	0.52	0.75	7.54	2.22	2.93	0.072	3,135.71				
EX-4	0.39	0.74	7.54	2.22	2.17	0.053	2,314.66				
EX-5	1.58	0.45	7.54	2.22	5.37	0.132	5,735.68				
EX-6	0.42	0.65	7.54	2.22	2.04	0.050	2,179.95				
EX-7	0.05	0.95	7.54	2.22	0.39	0.010	415.06				
EX-R-1	0.69	0.95	7.54	2.22	4.93	0.121	5,268.70				
Total	5.85	0.72			31.64	0.776	33,815.43				

	PROPOSED ONSITE 10 YR 5 MIN									
Drainage Area	Area (ac)	Cw	i (in/hr)	Q (cfs)						
A1	0.62	0.68	4.78	2.03						
A2	0.43	0.77	4.78	1.58						
A3	0.11	0.95	4.78	0.51						
B1	0.68	0.76	4.78	2.48						
B2	0.09	0.61	4.78	0.26						
B3	0.26	0.50	4.78	0.62						
B4	0.65	0.71	4.78	2.21						
B5	0.29	0.56	4.78	0.79						
B6	2.92	0.95	4.78	13.26						

PROPOSED ONSITE 100 YR 5 MIN									
Drainage Area 🛛 Area (ac) 🛛 Cw 🚽 i (in/hr) 👘 Q (cfs)									
A1	0.62	0.68	7.54	3.20					
A2	0.43	0.77	7.54	2.50					
A3	0.11	0.95	7.54	0.81					
B1	0.68	0.76	7.54	3.92					
B2	0.09	0.61	7.54	0.42					
B3	0.26	0.50	7.54	0.98					
B4	0.65	0.71	7.54	3.48					
B5	0.29	0.56	7.54	1.24					
B6	2.92	0.95	7.54	20.91					

	Proposed Volume Calculations										
							Vr=1*(P/12)*Cw*A				
							P=100-yr, 2-hr=2.22 in.				
Drainage	Area	C <sub>w</sub>	intensity	Precipitation	Q	Volume Req.	Volume Req.				
Area ID	(acres)	(-)	<u>(in/hr)</u>	<u>(in)</u>	<u>(cfs)</u>	(acre-ft)	<u>(CF)</u>				
STORM CAPTURE											
A1	0.62	0.68	7.54	2.22	3.20	0.08	3,420.97				
A2	0.43	0.77	7.54	2.22	2.50	0.06	2,668.47				
A3	0.11	0.95	7.54	2.22	0.81	0.02	865.33				
B1	0.68	0.76	7.54	2.22	3.92	0.10	4,185.76				
B2	0.09	0.61	7.54	2.22	0.42	0.01	445.11				
B3	0.26	0.50	7.54	2.22	0.98	0.02	1,049.05				
B4	0.65	0.71	7.54	2.22	3.48	0.09	3,723.59				
B5	0.29	0.56	7.54	2.22	1.24	0.03	1,326.29				
B6	2.92	0.95	7.54	2.22	20.91	0.51	22,349.96				
Basin 1 Totals:	6.06	0.82			37.46	0.919	40,034.55				



## APPENDIX III

## Grading & Drainage Plans







# PRELIMINARY NOT FOR CONSTRUCTION

